

May 12, 2015

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: **Notice of Exempt Modification – Facility Modification
1021 Straights Turnpike, Middlebury, Connecticut**

Dear Ms. Bachman:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains twelve (12) wireless telecommunications antennas at the 169-foot level on an existing 195-foot self-supporting lattice tower at 1021 Straights Turnpike in Middlebury, Connecticut (the “Property”). The tower is owned by T-Mobile. Cellco’s use of the tower was approved by the Council in 2000. Cellco now intends to modify its facility by replacing three (3) of its existing antennas with three (3) model HBXX-6517DS-VTM, 1900 MHz antennas; and adding three (3) model HBXX-6517DS-VTM, 2100 MHz antennas, for a total of fifteen (15) antennas, all at the same 169-foot level on the tower. Cellco also intends to add six (6) remote radio heads (“RRHs”), one (1) each behind its 1900 MHz and 2100 MHz antennas and one (1) HYBRIFLEX™ antenna cable. Included in Attachment 1 are specifications for Cellco’s replacement antennas, RRHs and HYBRIFLEX™ cable.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Edward B. St. John, First Selectman for the Town of Middlebury. The Town of Middlebury is the owner of the Property.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

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1. The proposed modifications will not result in an increase in the height of the existing tower. The replacement antennas and RRHs will be installed on Cellco's existing antenna T-arm at the 169-foot level on the tower.
2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. A cumulative General Power Density table with Cellco's modified facility is included in Attachment 2.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower and its foundation can support Cellco's proposed modifications. (*See Structural Analysis Report included in Attachment 3*).

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

Copy to:

Edward B. St. John, Middlebury First Selectman
Tim Parks

ATTACHMENT 1



HBXX-6517DS-VTM

Andrew® Quad Port Antenna, 1710–2180 MHz, 65° horizontal beamwidth, RET compatible

- Superior azimuth tracking and pattern symmetry with excellent passive intermodulation suppression

Electrical Specifications

Frequency Band, MHz	1710–1880	1850–1990	1920–2180
Gain, dBi	19.0	19.1	19.2
Beamwidth, Horizontal, degrees	67	66	65
Beamwidth, Vertical, degrees	5.0	4.7	4.4
Beam Tilt, degrees	0–6	0–6	0–6
USLS, dB	18	18	18
Front-to-Back Ratio at 180°, dB	30	30	30
CPR at Boresight, dB	21	22	21
CPR at Sector, dB	10	11	9
Isolation, dB	30	30	30
VSWR Return Loss, dB	1.4 15.6	1.4 15.6	1.4 15.6
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153
Input Power per Port, maximum, watts	350	350	350
Polarization	±45°	±45°	±45°
Impedance	50 ohm	50 ohm	50 ohm

Electrical Specifications, BASTA*

Frequency Band, MHz	1710–1880	1850–1990	1920–2180
Gain by all Beam Tilts, average, dBi	18.5	18.6	18.8
Gain by all Beam Tilts Tolerance, dB	±0.4	±0.3	±0.4
	0 ° 18.4	0 ° 18.4	0 ° 18.7
Gain by Beam Tilt, average, dBi	3 ° 18.7	3 ° 18.7	3 ° 18.9
	6 ° 18.4	6 ° 18.5	6 ° 18.6
Beamwidth, Horizontal Tolerance, degrees	±2.4	±1.7	±2.9
Beamwidth, Vertical Tolerance, degrees	±0.3	±0.3	±0.3
USLS, dB	18	19	19
Front-to-Back Total Power at 180° ± 30°, dB	25	26	26
CPR at Boresight, dB	22	23	22
CPR at Sector, dB	10	10	9

* CommScope® supports NGMN recommendations on Base Station Antenna Standards (BASTA). To learn more about the benefits of BASTA, [download the whitepaper Time to Raise the Bar on BSAs.](#)

General Specifications

Antenna Brand	Andrew®
Antenna Type	DualPol® quad
Band	Single band
Brand	DualPol® Teletilt®
Operating Frequency Band	1710 – 2180 MHz

HBXX-6517DS-VTM

POWERED BY



Performance Note

Outdoor usage

Mechanical Specifications

Color	Light gray
Lightning Protection	dc Ground
Radiator Material	Low loss circuit board
Radome Material	PVC, UV resistant
RF Connector Interface	7-16 DIN Female
RF Connector Location	Bottom
RF Connector Quantity, total	4
Wind Loading, maximum	668.0 N @ 150 km/h 150.2 lbf @ 150 km/h
Wind Speed, maximum	241.0 km/h 149.8 mph

Dimensions

Depth	166.0 mm 6.5 in
Length	1903.0 mm 74.9 in
Width	305.0 mm 12.0 in
Net Weight	19.5 kg 43.0 lb

Remote Electrical Tilt (RET) Information

Model with Factory Installed AISG 2.0 Actuator	HBXX-6517DS-A2M
RET System	Teletilt®

Regulatory Compliance/Certifications

Agency

RoHS 2011/65/EU
China RoHS SJ/T 11364-2006
ISO 9001:2008

Classification

Compliant by Exemption
Above Maximum Concentration Value (MCV)
Designed, manufactured and/or distributed under this quality management system



Included Products

600899A-2 — Downtilt Mounting Kit for 2.4 - 4.5 in (60 - 115 mm) OD round members. Kit contains one scissor top bracket set and one bottom bracket set.

* Footnotes

Performance Note	Severe environmental conditions may degrade optimum performance
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HYBRIFLEX™ RRH Hybrid Feeder Cabling Solution, 1-5/8", Single-Mode Fiber

Product Description

RFS' HYBRIFLEX Remote Radio Head (RRH) hybrid feeder cabling solution combines optical fiber and DC power for RRHs in a single lightweight aluminum corrugated cable, making it the world's most innovative solution for RRH deployments.

It was developed to reduce installation complexity and costs at Cellular sites. HYBRIFLEX allows mobile operators deploying an RRH architecture to standardize the RRH installation process and eliminate the need for and cost of cable grounding. HYBRIFLEX combines optical fiber (multi-mode or single-mode) and power in a single corrugated cable. It eliminates the need for junction boxes and can connect multiple RRHs with a single feeder. Standard RFS CELLFLEX® accessories can be used with HYBRIFLEX cable. Both pre-connectorized and on-site options are available.

Features/Benefits

- Aluminum corrugated armor with outstanding bending characteristics - minimizes installation time and enables mechanical protection and shielding
- Same accessories as 1 5/8" coaxial cable
- Outer conductor grounding - Eliminates typical grounding requirements and saves on installation costs
- Lightweight solution and compact design - Decreases tower loading
- Robust cabling - Eliminates need for expensive cable trays and ducts
- Installation of tight bundled fiber optic cable pairs directly to the RRH - Reduces CAPEX and wind load by eliminating need for interconnection
- Optical fiber and power cables housed in single corrugated cable - Saves CAPEX by standardizing RRH cable installation and reducing installation requirements
- Outdoor polyethylene jacket - Ensures long-lasting cable protection



Figure 1: HYBRIFLEX Series

Technical Specifications

Outer Conductor Armor	Corrugated Aluminum	[mm (in)]	46.5 (1.83)
Jacket	Polyethylene, PE	[mm (in)]	50.3 (1.98)
UV-Protection	Individual and External Jacket		Yes
Weight and Bending			
Weight, Approximate		[kg/m (lb/ft)]	1.9 (1.30)
Minimum Bending Radius, Single Bending		[mm (in)]	200 (8)
Minimum Bending Radius, Repeated Bending		[mm (in)]	500 (20)
Recommended/Maximum Clamp Spacing		[m (ft)]	1.0 / 1.2 (3.25 / 4.0)
Electrical Properties			
DC-Resistance Outer Conductor Armor		[Ω/km (Ω/1000ft)]	0.68 (0.205)
DC-Resistance Power Cable, 8.4mm ² (8AWG)		[Ω/km (Ω/1000ft)]	2.1 (0.307)
Optical Properties			
Version			Single-mode OMD
Quantity, Fiber Count			16 (8 pairs)
Core/Clad		[μm]	50/125
Primary Coating (Acrylate)		[μm]	245
Buffer Diameter, Nominal		[μm]	900
Secondary Protection, Jacket, Nominal		[mm (in)]	2.0 (0.08)
Minimum Bending Radius		[mm (in)]	104 (4.1)
Insertion Loss @ wavelength 850nm		dB/km	3.0
Insertion Loss @ wavelength 1310nm		dB/km	1.0
Standards (Meets or exceeds)			UL94-V0, UL1666 RoHS Compliant
Power Cable Properties			
Size (Power)		[mm (AWG)]	8.4 (8)
Quantity, Wire Count (Power)			16 (8 pairs)
Size (Alarm)		[mm (AWG)]	0.8 (18)
Quantity, Wire Count (Alarm)			4 (2 pairs)
Type			UV protected
Strands			19
Primary Jacket Diameter, Nominal		[mm (in)]	6.8 (0.27)
Standards (Meets or exceeds)			NFPA 130, ICEA S-95-658 UL Type XHHV-2, UL 44 UL-LS Limited Smoke, UL VW-1 IEEE-383 (1974), IEEE1202/FT4 RoHS Compliant
Temperature			
Installation Temperature		[°C (°F)]	-40 to +65 (-40 to 149)
Operation Temperature		[°C (°F)]	-40 to +65 (-40 to 149)

* This data is provisional and subject to change

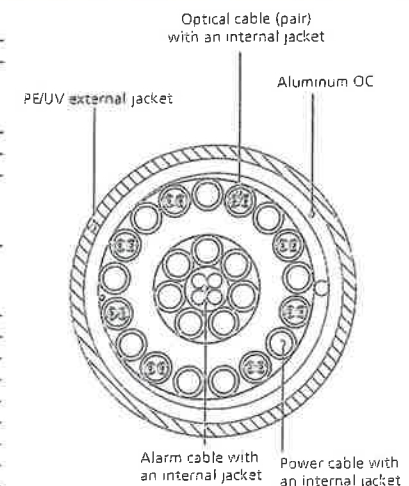


Figure 2: Construction Detail

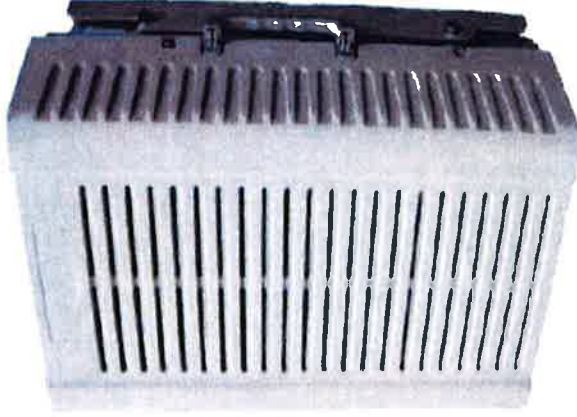
All information contained in the present datasheet is subject to confirmation at time of ordering.

PCS RF MODULES

RRH1900 2X60 - HW CHARACTERISTICS

LA6.0.1/13.3

RRH2x60	
RF Output Power	2x60W
Instantaneous Bandwidth	20MHz
Transmitter	2 TX
Receiver	1900 HW version 1900A HW version
Features	2 Branch RX – LA6.0.1 4 Branch RX – LR13.3 AISG 2.0 for RET/TMA Internal Smart Bias-T
Power	-48VDC
CPRI Ports	2 CPRI Rate 3 Ports
External Alarms	4 External User Alarms
Monitor Ports	TX
Environmental	GR487 Compliance
RF Connectors	7/16 DIN (top mounted)



** Not a Verizon Wireless deployed product

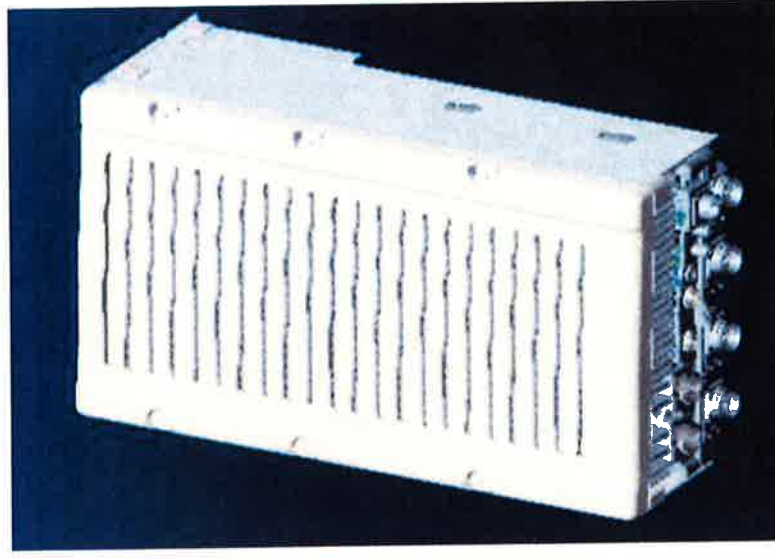
NEW PCS RF MODULES FOR VZW

RRH2X60 - HW CHARACTERISTICS

LR14.3

	RRH2X60
RF Output Power	2x60W (4x30W HW Ready)
Instantaneous Bandwidth	60MHz
Target Reliability (Annual Return Rate)	<2%
Receiver	4 Branch Rx
Features	AISG 2.0 for RET/TMA
Power	-48VDC Internal Smart Bias-T
CPRI Ports	2 CPRI Rate 5 Ports
External Alarms	4 External User Alarms
Monitor Ports	TX, RX
Environmental	GR487 Compliance
RF Connectors	7/16 DIN (downward facing)
Dimensions	22"(h) x 12"(w) x 9.4" (d)**
Weight	55lb**

** - Includes solar shield but not mounting brackets (8 lbs.)



ALCATEL-LUCENT WIRELESS PRODUCT DATASHEET RRH2X60-AWS FOR BAND 4 APPLICATIONS

The Alcatel-Lucent RRH2x60-AWS is a high power, small form factor Remote Radio Head operating in the AWS frequency band (3GPP Band 4) for LTE technology. It is designed with an eco-efficient approach, providing operators with the means to achieve high quality and high capacity coverage with minimum site requirements and efficient operation.



A distributed Node B expands the deployment options by using two components, a Base Band Unit (BBU) containing the digital assets and a separate RRH containing the radio-frequency (RF) elements. This modular design optimizes available space and allows the main components of a Node B to be installed separately, within the same site or several kilometers apart.

The Alcatel-Lucent RRH2x60-AWS is linked to the BBU by an optical-fiber connection carrying downlink and uplink digital radio signals

along with operations, administration and maintenance (OA&M) information.

SUPERIOR RF PERFORMANCE

The Alcatel-Lucent RRH2x60-AWS integrates all the latest technologies. This allows to offer best-in-class characteristics.

It delivers an outstanding 120 watts of total RF power thanks to its two transmit RF paths of 60 W each.

It is ideally suited to support multiple-input multiple-output (MIMO) 2x2 operation.

It includes four RF receivers to natively support 4-way uplink reception diversity. This improves the radio uplink coverage and this can be used to extend the cell radius commensurate with 2x2MIMO 2x60 W for the downlink.

It supports multiple discontinuous LTE carriers within an instantaneous bandwidth of 45 MHz corresponding to the entire AWS B4 spectrum.

The latest generation power amplifiers (PA) used in this product achieve high efficiency (>40%), resulting in improved power consumption figures.

OPTIMIZED TCO

The Alcatel-Lucent RRH2x60-AWS is designed to make available all the benefits of a distributed Node B, with excellent RF characteristics, with low capital expenditures (CAPEX) and low operating expenditures (OPEX).

The Alcatel-Lucent RRH2x60-AWS is a very cost-effective solution to deploy LTE MIMO.

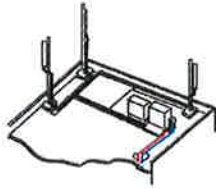
EASY INSTALLATION

The RRH2x60-AWS includes a reversible mounting bracket which allows for ease of installation behind an antenna, or on a rooftop knee wall while providing easy access to the mid body RF connectors.

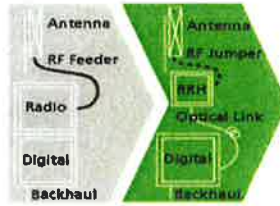
The limited space available in some sites may prevent the installation of traditional single-cabinet BTS equipment. However, many of these sites can host an Alcatel-Lucent RRH2x60-AWS installation, providing more flexible site selection and improved network quality along with greatly reduced installation time and costs.

The Alcatel-Lucent RRH2x60-AWS is a zero-footprint solution and is convection cooled without fans for silent operation, simplifying negotiations with site property owners and minimizing environmental impacts.

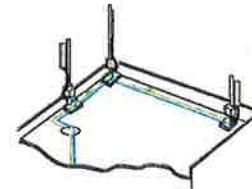
Installation can easily be done by a single person as the Alcatel-Lucent RRH2x60-AWS is compact and weighs about 20 kg, eliminating the need for a crane to hoist the BTS cabinet to the rooftop. A site can be in operation in less than one day.



Macro



RRH for space-constrained cell sites



Distributed

FEATURES

- RRH2x60-AWS integrates two power amplifiers of 60W rating (at each antenna connector)
- Support multiple carriers over the entire 3GPP band 4
- RRH2x60-AWS is optimized for LTE operation
- RRH2x60-AWS is a very compact and lightweight product
- Advanced power management techniques are embedded to provide power savings, such as PA bias control

BENEFITS

- MIMO LTE operation with only one single unit per sector
- Improved uplink coverage with built-in 4-way receive diversity capability
- RRH can be mounted close to the antenna, eliminating nearly all losses in RF cables and thus reducing power consumption by 50% compared to conventional solutions
- Distributed configurations provide easily deployable and cost-effective solutions, near zero footprint and

silent solutions, with minimum impact on the neighborhood, which ease the deployment

- RETA and TMA support without additional hardware thanks to the AISG v2.0 port and the integrated Bias-Tees. Bias-Tees support AISG DC supply and signaling.

TECHNICAL SPECIFICATIONS

Specifications listed are hardware capabilities. Some capabilities depend on support in a specific software release or future release.

Dimensions and weights

- HxWxD : 510x285x186mm (27 l with solar shield)
- Weight : 20 kg (44 lbs)

Electrical Data

- Power Supply : -48V DC (-40.5 to -57V)
- Power Consumption (ETSI average traffic load reference) : 250W @2x60W

RF Characteristics

- Frequency band: 1710-1755, UL / 2110-2155 MHz, DL (3GPP band 4)
- Output power: 2x60W at antenna connectors
- Technology supported: LTE
- Instantaneous bandwidth: 45 MHz
- Rx diversity: 2-way and 4-way uplink reception
- Typical sensitivity without Rx diversity: -105 dBm for LTE

Connectivity

- Two CPRI optical ports for daisy chaining and up to six RRHs per fiber
- Type of optical fiber: Single-Mode (SM) and Multi-Mode (MM) SFPs
- Optical fiber length: up to 500m using MM fiber, up to 20km using SM fiber
- TMA/RETA : AISG 2.0 (RS485 connector and internal Bias-Tee)
- Six external alarms
- Surge protection for all external ports (DC and RF)

Safety and Regulatory Data

- EMC : 3GPP 25113, EN 301 489-1, EN 301 489-23, GR 1089, GR 3108, OET-65
- Safety : IEC60950-1, EN 60825-1, UL, ANSI/NFPA 70, CAN/CSA-C22.2
- Regulatory : FCC Part 15 Class B, CE Mark – European Directive : 2002/95/EC (ROHS); 2002/96/EC (WEEE); 1999/5/EC (R&TTE)
- Health : EN 50385

Environmental specifications

- Operating temperature: -40°C to 55°C including solar load
- Operating relative humidity: 8% to 100%
- Environmental Conditions : ETS 300 019-1-4 class 4.1E
- Ingress Protection : IEC 60529 IP65
- Acoustic Noise : Noiseless (natural convection cooling)

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AT THE SPEED OF IDEAS™

ATTACHMENT 2

		General		Power		Density							
Site Name: Middlebury Tower Height: 195Ft.													
CARRIER	# OF CHAN.	WATTS ERP	HEIGHT	CALC. POWER DENS	FREQ.	MAX. PERMISS. EXP.	FRACTION MPE	Total					
*AT&T UMTS	2	565	188	0.0115	880	0.5867	1.96%						
*AT&T UMTS	2	875	188	0.0178	1900	1.0000	1.78%						
*AT&T GSM	1	283	188	0.0029	880	0.5867	0.49%						
*AT&T GSM	4	525	188	0.0214	1900	1.0000	2.14%						
*AT&T LTE	1	1615	188	0.0164	734	0.4893	3.36%						
*Pocket (now MetroPCS)	3	631	145	0.0324	2130	1.0000	3.24%						
*T-Mobile PCS/AWS	6	639	195	0.0363	1900	1.0000	3.63%						
*T-Mobile 700 MHz	1	445	193	0.0043	700	0.4667	0.92%						
*Sprint CDMA/LTE	2	693	150.4	0.0220	1900	1.0000	2.20%						
*Sprint CDMA/LTE	1	390	150.4	0.0062	850	0.5667	1.09%						
*Sprint Nextel iDEN	12	100	175	0.0141	851	0.5673	2.48%						
*Sprint Nextel WiMAX	3	562	175	0.0198	2657	1.0000	1.98%						
Verizon PCS	7	366	169	0.0323	1970	1.0000	3.23%						
Verizon Cellular	9	371	169	0.0420	869	0.5793	7.26%						
Verizon AWS	1	2812	169	0.0354	2145	1.0000	3.54%						
Verizon 700	1	788	169	0.0099	746	0.4973	1.99%						41.28%
* Source: Siting Council													

ATTACHMENT 3

REVIEWED

By JACKIE DONAHUE at 3:21 pm, Apr 30, 2015

Date: April 29, 2015

Kenneth Fann
T-Mobile Towers
12920 SE 38th Street
Bellevue, WA 98006
(425) 383-3978



Tower Engineering Professionals
326 Tryon Road
Raleigh, NC 27603
(919) 661-6351
TMOStructural@tepgroup.net

Subject: Structural Analysis Report – Revision 16

Carrier Designation: Verizon Reconfiguration
Carrier Site Name: Middlebury
Carrier Site Number: N/A

T-Mobile Designation: T-Mobile Site Name: Middlebury I-84
T-Mobile Site Number: CT11128E

Engineering Firm Designation: TEP Project Number: 25628_22635

Site Data: 1021 Straits Turnpike, Middlebury, New Haven County, CT 06762
Latitude 41° 32' 8.78", Longitude -73° 05' 21.27"
195 Foot - Self Supporting Tower

Dear Mr. Fann,

Tower Engineering Professionals is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC1: Existing + Proposed Equipment **Sufficient Capacity**
Note: See Table 1 for the existing and proposed loading

Structure Capacity	Controlling Component
97.9%	Diagonal T10 (113.3' - 120')

The analysis has been performed in accordance with the ANSI/TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures, ASCE 7-05 Minimum Design Loads for Buildings and Other Structures and the 2005 Connecticut State Building Code based upon a wind speed of 85 mph fastest mile.

All modifications and equipment proposed in this report shall be installed in accordance with the appurtenances listed in Table 1 and the attached drawing for the determined available structural capacity to be effective.

We at Tower Engineering Professionals appreciate the opportunity of providing our continuing professional services to you and T-Mobile Towers. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Christopher Bean

Respectfully submitted by:

Graham M. Andres, P.E.



Revision #	Date Issued	Description
14	November 24, 2014	Revised T-Mobile Loading
15	January 22, 2015	Revised Sprint and T-Mobile Loading
16	April 29, 2015	Verizon Reconfiguration

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tnxTower Output

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Coax Configuration

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Additional Calculations

1) INTRODUCTION

This tower is a 195-ft self-support tower designed by Fred A. Nudd Corporation in May of 1998. The tower was originally designed for a fastest mile wind speed of 85 mph per ANSI/EIA/TIA-222-F for the appurtenances listed in Table 2. TEP visited the site in June of 2010 to gather existing steel and appurtenance information. This tower has been modified multiple times in the past to accommodate additional loading. All information provided to TEP was assumed to be accurate and complete.

2) ANALYSIS CRITERIA

The analysis has been performed in accordance with the ANSI/TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures and ASCE 7-05 Minimum Design Loads for Buildings and Other Structures using a fastest mile wind speed of 85 mph with no ice, 38 mph with 0.75 inch escalating ice thickness, and 50 mph under service loads.

Table 1 - Existing and Proposed Antenna and Cable Information

Existing/ Proposed	Elevation (ft)	Qty	Antenna Model	Mount Type	Qty Coax	Coax Size (in)	Coax ¹ Location	Owner/ Tenant
Existing	195.0	3	RFS APX16DWV-16DWVS	(3) 12.5' Sector Frames	18	1-5/8	AB Face	T-Mobile
		6	Ericsson KRY-112-71					
	193.0	3	Commscope LNX-6515DS-VTM					
Existing	188.0	6	Powerwave 7770	(3) 15.0' T-Frames with Catwalk	12 1 2	1-5/8 7/16"Ø Fiber 3/8"Ø Power	CA Face	AT&T
		3	Powerwave P65-17-XLH-RR					
		6	Powerwave LGP 13519 Diplexers					
		6	Powerwave LGP 21401 TMA					
		6	Ericsson RRUS-11					
		1	Raycap DC6-48-60-18-8F					
Proposed	169.0	2	Antel BXA-70063-6CF	(3) 15.0' T-Frames with Catwalk	12 1	1-5/8 Fiber	AB Face	Verizon
		4	Decibel DB844G65ZAXY					
		1	Antel BXA 70080/6CF					
		2	Decibel DB846F65ZAXY					
		6	RFS FD9R6004/2C-3L					
		3	ALU RRH2x60-AWS					
		3	ALU RRH2x60-PCS					
		1	RFS DB-T1-6Z-8AB-OZ					
6	HBXX-6517DS-A2M							
Existing	152.0	3	RFS APXVTM14-C-120	(3) 12.0' Sector Frames	7	1-1/4"Ø Fiber	BC Face	Sprint
		3	Decibel DB980H90T3E-M					
		3	RFS APXVSPP18-C-A20					
		3	RFS RRU 800MHz 2x50W					
		3	ALU TD-RRH 8x20-25					
		3	RFS RRU 1900MHz 2x40W					
Existing	75.5	1	GPS Antenna	4.5' Standoff	1	5/8"Ø	BC Face	Unknown

Notes:

1) See "Appendix B – Coax Configuration" for assumed coax configuration.

Table 2 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Qty Coax	Coax Size (in)	Coax Location
195.0	195.0	12	Swedcom	ALP 9011	12	1-5/8	Unknown
185.0	185.0	12	Swedcom	ALP 9011	12	1-5/8	Unknown
175.0	175.0	12	Swedcom	ALP 9011	12	1-5/8	Unknown
165.0	165.0	12	Swedcom	ALP 9011	12	1-5/8	Unknown
155.0	155.0	12	Swedcom	ALP 9011	12	1-5/8	Unknown

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Remarks	Reference	Source
Tower and Foundation Drawings	Fred A. Nudd Corporation, dated May 5, 1998 Drawing No. 98-5974-1	-	T-Mobile
Structural Modification Drawings	Fred A. Nudd Corporation, dated April 30, 1999 Drawing No. 99-6726-1	-	T-Mobile
Steel and Appurtenance Mapping	Tower Engineering Professionals, dated June 3, 2010	102056	TEP
Geotechnical Report	Dr. Clarence Welti, P.E., P.C., dated April 17, 1998	-	T-Mobile
Structural Modification Drawings	Tower Engineering Professionals, Inc. dated August 29, 2011	102056	TEP
Structural Modification Drawings	Tower Engineering Professionals, Inc. dated July 26, 2012	102056	TEP
Structural Modification Analysis	Tower Engineering Professionals, Inc. dated August 1, 2013	25628_4865	TEP
Previous Structural Analysis	Tower Engineering Professionals, Inc. dated January 22, 2015	25628_22635	TEP
Correspondence	Correspondence with T-Mobile with regards to the existing and proposed loading, SAW dated January 26, 2015	-	T-Mobile

3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) The tower and foundation were built in accordance with the manufacturer's specifications.
- 2) The tower and foundation have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Table 1 and "Appendix B – Coax Configuration".
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by the standard.
- 5) All tower components are in sufficient condition to carry their full design capacity.
- 6) Serviceability with respect to antenna twist, tilt, roll, or lateral translation, is not checked and is left to the carrier or tower owner to ensure conformance.
- 7) All antenna mounts and mounting hardware are structurally sufficient to carry the full design capacity requirements of appurtenance wind area and weight as provided by the original manufacturer specifications. It is the carrier's responsibility to ensure compliance to the structural limitations of the existing and/or proposed antenna mounts. TEP did not verify the size, condition or capacity of the antenna mounts and did not analyze antennas supporting mounts as part of this structural analysis report.

This analysis may be affected if any assumptions are not valid or have been made in error. Tower Engineering Professionals should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (lb)	SF*P _{allow} (lb)	% Capacity	Pass / Fail
T1	195 - 180	Leg	PIPE 2.5 STD (SCH 40)	3	-29719.30	61065.26	48.7	Pass
T2	180 - 175	Leg	PIPE 2.5 STD (SCH 40)	45	-32993.60	66232.77	49.8	Pass
T3	175 - 170	Leg	PIPE 2.5 STD (SCH 40)	57	-40983.70	66319.28	61.8	Pass
T4	170 - 160	Leg	2.5SCH40 w/ 3SCH80 Half Sleeve	Note 2	Note 2	Note 2	76.1	Pass
T5	160 - 150	Leg	Pipe 3.5 Std (SCH40)	90	-84070.20	109240.81	77.0	Pass
T6	150 - 140	Leg	3.5SCH40 w/ 4SCH40 Half Sleeve	Note 2	Note 2	Note 2	68.5	Pass
T7	140 - 133.333	Leg	5 STD w/ 6 XH Half Sleeve	Note 2	Note 2	Note 2	46.2	Pass
T8	133.333 - 126.667	Leg	5 STD w/ 6 XH Half Sleeve	Note 2	Note 2	Note 2	51.6	Pass
T9	126.667 - 120	Leg	5 STD w/ 6 XH Half Sleeve	Note 2	Note 2	Note 2	56.8	Pass
T10	120 - 113.333	Leg	PIPE 6 STD (SCH40)	159	-161650.00	232356.55	69.6	Pass
T11	113.333 - 106.667	Leg	PIPE 6 STD (SCH40)	171	-174393.00	232391.21	75.0	Pass
T12	106.667 - 100	Leg	PIPE 6 STD (SCH40)	183	-186243.00	232421.87	80.1	Pass
T13	100 - 80	Leg	6 STD w/ 7 XH Half Sleeve	Note 2	Note 2	Note 2	62.3	Pass
T14	80 - 60	Leg	PIPE 8 STD (SCH40)	225	-256430.00	336506.51	76.2	Pass
T15	60 - 50	Leg	PIPE 8 STD (SCH40)	246	-268742.00	345717.53	77.7	Pass
T16	50 - 40	Leg	PIPE 8 STD (SCH40)	258	-285244.00	345770.85	82.5	Pass
T17	40 - 20	Leg	PIPE 8 X-STR (SCH80)	270	-317855.00	473586.89	67.1	Pass
T18	20 - 0	Leg	PIPE 8 X-STR (SCH80)	285	-357208.00	474086.76	75.3 85.6 (b)	Pass
T1	195 - 180	Diagonal	5/8	12	7269.94	8833.52	82.3	Pass
T2	180 - 175	Diagonal	L1 1/2x1 1/2x3/16	48	-3333.83	6385.59	52.2 67.7 (b)	Pass
T3	175 - 170	Diagonal	L2x2x3/16	60	-2562.46	12345.57	20.8 50.5 (b)	Pass
T4	170 - 160	Diagonal	2L1 1/2x1 1/2x3/16x1/4	75	-4913.61	16655.43	29.5 68.8 (b)	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (lb)	SF*P _{allow} (lb)	% Capacity	Pass / Fail
T5	160 - 150	Diagonal	2L2x2x3/16x1/4	96	-5637.04	25694.51	21.9 73.7 (b)	Pass
T6	150 - 140	Diagonal	2L2x2x3/16x1/4	123	-6289.46	25233.69	24.9 84.2 (b)	Pass
T7	140 - 133.333	Diagonal	L2 1/2x2 1/2x1/4	135	-6542.78	16497.61	39.7 59.2 (b)	Pass
T8	133.333 - 126.667	Diagonal	L2 1/2x2 1/2x1/4	144	-6697.06	14316.29	46.8 60.5 (b)	Pass
T9	126.667 - 120	Diagonal	L2 1/2x2 1/2x3/16	153	-6489.40	9979.96	65.0 94.4 (b)	Pass
T10	120 - 113.333	Diagonal	L3x3x3/16	162	-7262.85	15433.74	47.1 97.9 (b)	Pass
T11	113.333 - 106.667	Diagonal	L3x3x3/16	174	-7119.97	14812.43	48.1 97.4 (b)	Pass
T12	106.667 - 100	Diagonal	L2 1/2x2 1/2x1/4	186	-7580.18	10183.75	74.4	Pass
T13	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	198	-7473.24	27500.05	27.2 44.9 (b)	Pass
T14	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	228	-8019.70	23594.23	34.0 46.7 (b)	Pass
T15	60 - 50	Diagonal	L3x3x5/16	249	-9971.03	12260.08	81.3	Pass
T16	50 - 40	Diagonal	L3x3x5/16	261	-9984.21	11351.93	88.0	Pass
T17	40 - 20	Diagonal	L4x4x3/8	273	-9882.33	25488.16	38.8 57.5 (b)	Pass
T18	20 - 0	Diagonal	L5x5x5/16	288	-11136.30	34335.95	32.4 64.8 (b)	Pass
T1	195 - 180	Horizontal	L1 1/2x1 1/2x3/16	17	-4942.77	6723.25	73.5	Pass
T2	180 - 175	Secondary Horizontal	L2x2x3/16	53	572.18	16652.77	3.4 9.4 (b)	Pass
T3	175 - 170	Secondary Horizontal	L2x2x3/16	65	-710.74	16534.80	4.3 11.7 (b)	Pass
T4	170 - 160	Secondary Horizontal	L2x2x3/16	77	-1079.97	15123.02	7.1 17.8 (b)	Pass
T5	160 - 150	Secondary Horizontal	L2x2x3/16	98	-1457.96	13544.21	10.8 24.0 (b)	Pass
T6	150 - 140	Secondary Horizontal	L2x2x3/16	119	-1884.28	11810.53	16.0 31.0 (b)	Pass
T10	120 - 113.333	Secondary Horizontal	L3x3x3/16	167	-2803.36	19651.09	14.3 41.3 (b)	Pass
T11	113.333 - 106.667	Secondary Horizontal	L3x3x3/16	179	-3024.34	18513.50	16.3 37.1 (b)	Pass
T12	106.667 - 100	Secondary Horizontal	L3x3x3/16	191	-3229.85	6723.76	48.0	Pass
T13	100 - 80	Secondary Horizontal	L3x3x1/4	203	-3857.70	17794.35	21.7 44.9 (b)	Pass
T15	60 - 50	Secondary Horizontal	L4x4x1/4	254	-4660.55	10544.16	44.2	Pass
T16	50 - 40	Secondary Horizontal	L4x4x1/4	266	-4946.73	9490.00	52.1	Pass
T1	195 - 180	Top Girt	L1 1/2x1 1/2x3/16	5	-874.33	6723.25	13.0	Pass
T1	195 - 180	Bottom Girt	L1 1/2x1 1/2x3/16	8	-2434.08	6723.25	36.2	Pass
							Summary	
							Leg (T18)	85.6 Pass
							Diagonal (T10)	97.9 Pass
							Horizontal (T1)	73.5 Pass
							Secondary Horizontal (T16)	52.1 Pass
							Top Girt (T1)	13.0 Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (lb)	SF*P _{allow} (lb)	% Capacity	Pass / Fail
						Bottom Girt (T1)	36.2	Pass
						Bolt Checks	97.9	Pass
						RATING =	97.9	Pass

Table 5 - Tower Component Stresses vs. Capacity

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
-	Anchor Rods	-	81.3	Pass
2	Base Foundation - Soil Interaction	-	43.0	Pass
2	Base Foundation - Structural	-	61.8	Pass

Notes:

- 2) See additional documentation in "Appendix C - Additional Calculations" for calculations supporting the % capacity listed.

Structure Rating (max from all components) =	97.9%
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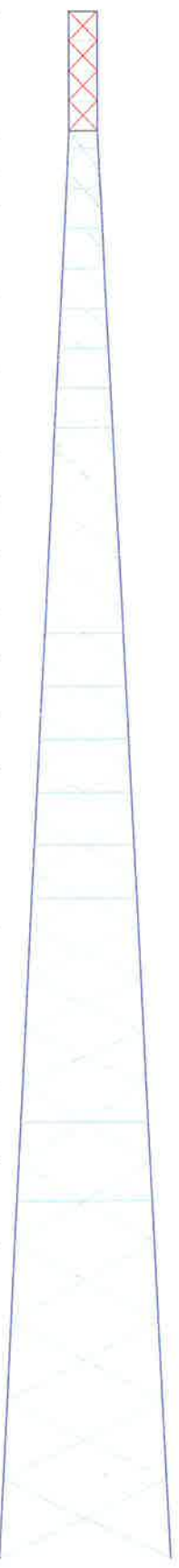
4.1) Recommendations

- 1) If the load differs from that described in Table 1 of this report, "Appendix B – Coax Configuration" or the provisions of this analysis are found to be invalid, another structural analysis should be performed.
- 2) The tower and its foundation have sufficient capacity to carry the existing and proposed loads. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

Legs	PIPE 6 STD (SCH40)	A572-55	L3x3x3/16	J	L3x3x3/16	774.3	754.5	788.4	842.1	895.3	1040.7	817.6	705.9	214.0	181.8	4 @ 3.75	484.2
Diagonals	PIPE 8 X-STR (SCH80)	A572-55	L4x4x3/8		L5x5x5/16												
Diagonal Grade																	
Top Girts	PIPE 8 STD (SCH40)	A572-55	L3x3x3/16		L3x3x3/16												
Bottom Girts																	
Horizontals																	
Sec. Horizontals																	
Face Width (ft)																	
# Panels @ (ft)																	
Weight (lb)																	

195.0 ft
180.0 ft
175.0 ft
170.0 ft
160.0 ft
150.0 ft
140.0 ft
133.3 ft
126.7 ft
120.0 ft
113.3 ft
106.7 ft
100.0 ft
80.0 ft
60.0 ft
50.0 ft
40.0 ft
20.0 ft
0.0 ft



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Sector Mount [SM 802-3]	195	BXA-70080/6CF w/ Mount Pipe	169
HSS Top Mount	195	DBB46F65ZAXY w/Mount Pipe	169
APX16DWV-16DWVS-A	195	DBB46F65ZAXY w/Mount Pipe	169
APX16DWV-16DWVS-A	195	(2) FD9R6004	169
APX16DWV-16DWVS-A	195	(2) FD9R6004	169
(2) KRY 112 71	195	(2) FD9R6004	169
(2) KRY 112 71	195	(3) Sector Mounts 169-ft	169
(2) KRY 112 71	195	(2) BXA-70063/6CF w/ Mount Pipe	169
LNx-6515DS-VTM w/ Mount Pipe	193	(2) DBB44G65ZAXY w/Mount Pipe	169
LNx-6515DS-VTM w/ Mount Pipe	193	(2) DBB44G65ZAXY w/Mount Pipe	169
LNx-6515DS-VTM w/ Mount Pipe	193	DB-B1/T1 w/ Mount Pipe	169
(3) Sector Mounts 188-ft	188	(2) HBXX-6517DS-A2M w/ Mount Pipe	169
(2) 7770.00 w/ Mount Pipe	188	(2) HBXX-6517DS-A2M w/ Mount Pipe	169
(2) 7770.00 w/ Mount Pipe	188	TD-RRH8x20-25	152
(2) 7770.00 w/ Mount Pipe	188	TD-RRH8x20-25	152
P65-17-XLH-RR w/Mount Pipe	188	TD-RRH8x20-25	152
P65-17-XLH-RR w/Mount Pipe	188	1900MHz 2X40W RRH	152
P65-17-XLH-RR w/Mount Pipe	188	DB980H90T2E-M w/ Mount Pipe	152
(2) LGP13519	188	DB980H90T2E-M w/ Mount Pipe	152
(2) LGP13519	188	DB980H90T2E-M w/ Mount Pipe	152
(2) LGP13519	188	APXVTM14-C-120 w/ Mount Pipe	152
(2) LGP21401	188	APXVTM14-C-120 w/ Mount Pipe	152
(2) LGP21401	188	APXVTM14-C-120 w/ Mount Pipe	152
(2) LGP21401	188	800MHZ 2X50W RRH	152
(2) RRUUS-11	188	1900MHz 2X40W RRH	152
(2) RRUUS-11	188	1900MHz 2X40W RRH	152
(2) RRUUS-11	188	Sector Mount [SM 502-3]	152
DC6-48-60-18-8F	188	APXVSP18-C-A20 w/ Mount Pipe	152
(2) HBXX-6517DS-A2M w/ Mount Pipe	169	APXVSP18-C-A20 w/ Mount Pipe	152
RRH2x60-AWS	169	APXVSP18-C-A20 w/ Mount Pipe	152
RRH2x60-AWS	169	800MHZ 2X50W RRH	152
RRH2x60-AWS	169	800MHZ 2X50W RRH	152
RRH2x60-PCS	169	GPS0015	75.5
RRH2x60-PCS	169	1.75" Dia x 5-ft Pipe	75.5
RRH2x60-PCS	169		

SYMBOL LIST

MARK	SIZE	MARK	SIZE
A	2.5SCH40 w/ 3SCH80 Half Sleeve	F	L1 1/2x1 1/2x3/16
B	Pipe 3.5 Std (SCH40)	G	L2x2x3/16
C	3.5SCH40 w/ 4SCH40 Half Sleeve	H	2L1 1/2x1 1/2x3/16x1/4
D	5 STD w/ 6 XH Half Sleeve	I	L2 1/2x2 1/2x3/16
E	6 STD w/ 7 XH Half Sleeve	J	L2 1/2x2 1/2x1/4

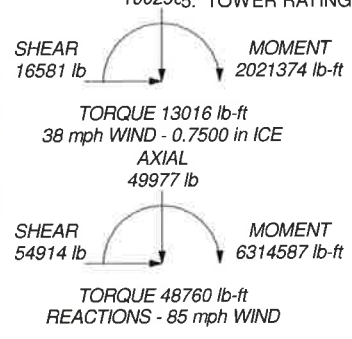
MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-55	55 ksi	70 ksi	A500-50	50 ksi	62 ksi
A36	36 ksi	58 ksi	A500-46	46 ksi	62 ksi
A53-B-35	35 ksi	63 ksi	A53-B-42	42 ksi	63 ksi

MAX. CORN
DOWN: 355796 lb
SHEAR: 34951 lb

TOWER DESIGN NOTES

1. Tower is located in New Haven County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 97.9%





Tower Engineering Professionals, Inc.
326 Tryon Road
Raleigh, NC 27603
Phone: (919) 661-6351
FAX: (919) 661-6350

Job: Middlebury I-84 (CT11128E)

Project: **TEP No. 25628 22635, Revision 16**

Client: T-Mobile Towers Drawn by: Christopher Bear App'd:

Code: TIA/EIA-222-F Date: 04/28/15 Scale: N

Path: Dwg No.

tnxTower	Middlebury I-84 (CT11128E)	Page 1 of 29
Job		
Project	TEP No. 25628_22635, Revision 16	Date 14:24:52 04/28/15
Client	T-Mobile Towers	Designed by Christopher Bean
Tower Engineering Professionals, Inc. 326 Tryon Road Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350		

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 195.00 ft above the ground line. The base of the tower is set at an elevation of 0.00 ft above the ground line. The face width of the tower is 3.50 ft at the top and 21.50 ft at the base. This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in New Haven County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 38 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

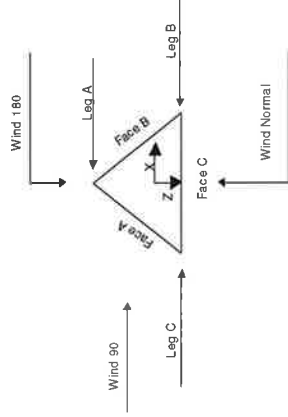
A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feed line supports, and appearance mounts are not considered.

tnxTower	Middlebury I-84 (CT11128E)	Page 2 of 29
Job		
Project	TEP No. 25628_22635, Revision 16	Date 14:24:52 04/28/15
Client	T-Mobile Towers	Designed by Christopher Bean
Tower Engineering Professionals, Inc. 326 Tryon Road Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350		



Triangular Tower

Options

- Consider Moments - Legs
- Consider Moments - Horizontals
- Consider Moments - Diagonals
- Use Moment Magnification
- Use Code Stress Ratios
- Use Code Safety Factors - Guys
- Escalate Ice
- Always Use Max Kz
- Use Special Wind Profile
- Include Bolts In Member Capacity
- Leg Bolts Are At Top Of Section
- Secondary Horizontal Braces Leg
- Use Diamond Inner Bracing (4 Sided)
- Add IBC 6D+W Combination Exemption
- Distribute Leg Loads As Uniform
- Assume Rigid Index Plate
- Use Clear Spans For Wind Area
- Use Clear Spans For KLR
- Retention Guys To Initial Tension
- Bypass Mast Stability Checks
- Use Azimuth Dish Coefficients
- Project Wind Area of Appart.
- Autocalc Torque Arm Areas
- SR Members Have Cut Ends
- Sort Capacity Reports By Component
- Triangulate Diamond Inner Bracing
- Use TIA-222-G Tension Splice Capacity Exemption
- Treat Feedline Bundles As Cylinder
- Use ASCE 10 X-Brace Ly Rules
- Calculate Redundant Bracing Forces
- Ignore Redundant Members In FEA
- SR Leg Bolts Resist Compression
- All Leg Panels Have Same Allowable
- Offset Girt At Foundation
- Consider Feedline Torque
- Include Angle Block Shear Check
- Include Shear-Torsion Interaction
- Always Use Sub-Critical Flow
- Use Top Mounted Sockets

Tower Section Geometry

Tower Section	Tower Elevation f	Assembly Database	Description	Section Width f	Number of Sections	Section Length f
T1	195.00-190.00			3.50	1	15.00
T2	180.00-175.00			3.50	1	5.00
T3	175.00-170.00			4.00	1	5.00
T4	170.00-160.00			4.50	1	10.00
T5	160.00-150.00			5.50	1	10.00
T6	150.00-140.00			6.50	1	10.00
T7	140.00-133.33			7.50	1	6.67
T8	133.33-126.67			8.17	1	6.67
T9	126.67-120.00			8.83	1	6.67
T10	120.00-113.33			9.50	1	6.67
T11	113.33-106.67			10.17	1	6.67
T12	106.67-100.00			10.83	1	6.67
T13	100.00-80.00			11.50	1	20.00
T14	80.00-60.00			13.50	1	10.00
T15	60.00-50.00			15.50	1	10.00
T16	50.00-40.00			16.50	1	10.00
T17	40.00-20.00			17.50	1	20.00
T18	20.00-0.00			19.50	1	20.00

Tower Section Geometry (cont'd)

tnxTower		Middlebury I-84 (CT11128E)		Page	3 of 29
Tower Engineering Professionals, Inc. 326 Tryon Road Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350		Project TEP No. 25628_22635, Revision 16		Date	14:24:52 04/28/15
Client T-Mobile Towers		Designed by Christopher Bean			

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End	Has Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
T1	195.00-180.00	3.75	TX Brace	No	Yes	No	0.0000	0.0000
T2	180.00-175.00	5.00	X Brace	No	Yes	Yes	0.0000	0.0000
T3	175.00-170.00	5.00	X Brace	No	Yes	Yes	0.0000	0.0000
T4	170.00-160.00	5.00	X Brace	No	Yes	Yes	0.0000	0.0000
T5	160.00-150.00	5.00	X Brace	No	Yes	Yes	0.0000	0.0000
T6	150.00-140.00	5.00	X Brace	No	Yes	Yes	0.0000	0.0000
T7	140.00-133.33	6.67	X Brace	No	No	No	0.0000	0.0000
T8	133.33-126.67	6.67	X Brace	No	No	No	0.0000	0.0000
T9	126.67-120.00	6.67	X Brace	No	No	No	0.0000	0.0000
T10	120.00-113.33	6.67	X Brace	No	Yes	Yes	0.0000	0.0000
T11	113.33-106.67	6.67	X Brace	No	Yes	Yes	0.0000	0.0000
T12	106.67-100.00	6.67	X Brace	No	Yes	Yes	0.0000	0.0000
T13	100.00-80.00	6.67	X Brace	No	Yes	Yes	0.0000	0.0000
T14	80.00-60.00	10.00	X Brace	No	Yes	Yes	0.0000	0.0000
T15	60.00-40.00	10.00	X Brace	No	Yes	Yes	0.0000	0.0000
T16	40.00-20.00	10.00	X Brace	No	Yes	Yes	0.0000	0.0000
T17	20.00-0.00	9.36	X Brace	No	No	No	0.0000	0.0000
T18	20.00-0.00	9.36	X Brace	No	No	No	0.0000	1.0000

Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 195.00-180.00	Pipe	PIPE 2.5 STD (SCH 40)	A572-55 (55 ksi)	Solid Round	5/8	A36 (36 ksi)
T2 180.00-175.00	Pipe	PIPE 2.5 STD (SCH 40)	A572-55 (55 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T3 175.00-170.00	Pipe	PIPE 2.5 STD (SCH 40)	A572-55 (55 ksi)	Equal Angle	L2x2x3/16	A36 (36 ksi)
T4 170.00-160.00	Arbitrary Shape	2.5SCH40 w/ 3SCH80 Half Sleeve	A53-B-35 (35 ksi)	Double Equal Angle	2L1 1/2x1 1/2x3/16x1/4	A36 (36 ksi)
T5 160.00-150.00	Pipe	Pipe 3.5 Std (SCH40)	A572-55 (55 ksi)	Double Angle	2L2x2x3/16x1/4	A36 (36 ksi)
T6 150.00-140.00	Arbitrary Shape	3.5SCH40 w/ 4SCH40 Half Sleeve	A500-50 (50 ksi)	Double Angle	2L2x2x3/16x1/4	A36 (36 ksi)
T7 140.00-133.33	Arbitrary Shape	5 STD w/ 6 XH Half Sleeve	A500-46 (46 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T8 133.33-126.67	Arbitrary Shape	5 STD w/ 6 XH Half Sleeve	A500-46 (46 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T9 126.67-120.00	Arbitrary Shape	5 STD w/ 6 XH Half Sleeve	A500-46 (46 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T10	Pipe	PIPE 6 STD (SCH40)	A572-55 (55 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T11	Pipe	PIPE 6 STD (SCH40)	A572-55 (55 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T12	Pipe	PIPE 6 STD (SCH40)	A572-55 (55 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T13 100.00-80.00	Arbitrary Shape	6 STD w/ 7 XH Half Sleeve	A53-B-42 (42 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)
T14 80.00-60.00	Pipe	PIPE 8 STD (SCH40)	A572-55 (55 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)
T15 60.00-40.00	Pipe	PIPE 8 STD (SCH40)	A572-55 (55 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)
T16 50.00-40.00	Pipe	PIPE 8 STD (SCH40)	A572-55 (55 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)
T17 40.00-20.00	Pipe	PIPE 8 STD (SCH40)	A572-55 (55 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)
T17 40.00-20.00	Pipe	PIPE 8 X-STR (SCH80)	A572-55 (55 ksi)	Equal Angle	L4x4x3/8	A36 (36 ksi)

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Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T18 20.00-0.00	Pipe	PIPE 8 X-STR (SCH80)	A572-55 (55 ksi)	Equal Angle	L5x5x5/16	A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 195.00-180.00	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	No. of Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T1 195.00-180.00	None	Flat Bar		A36 (36 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T2 180.00-175.00	Equal Angle	L2x2x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T3 175.00-170.00	Equal Angle	L2x2x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T4 170.00-160.00	Equal Angle	L2x2x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T5 160.00-150.00	Equal Angle	L2x2x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T6 150.00-140.00	Equal Angle	L2x2x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T10	Equal Angle	L3x3x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T11	Equal Angle	L3x3x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T12	Equal Angle	L3x3x3/16	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T13 100.00-80.00	Equal Angle	L3x3x1/4	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T15 60.00-40.00	Equal Angle	L4x4x1/4	A36 (36 ksi)	Solid Round		A36 (36 ksi)

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Tower Elevation	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T16 50.00-40.00	Equal Angle	L4x4x1/4	A36 (16 ksi)	Solid Round		A36 (16 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor	Weight Mult.	Double Angle Spacing	Double Angle Spacing Diagonals	Double Angle Spacing Horizontal
T1	0.00	0.0000	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T2	0.00	0.2500	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T3	0.00	0.2500	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T4	0.00	0.2500	A36 (36 ksi)	1	1	Mid-Pt	0.0000	0.0000
T5	0.00	0.2500	A36 (36 ksi)	1	1	Mid-Pt	0.0000	0.0000
T6	0.00	0.2500	A36 (36 ksi)	1	1	Mid-Pt	0.0000	0.0000
T7	0.00	0.3750	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T8	0.00	0.3750	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T9	0.00	0.3750	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T10	0.00	0.3750	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T11	0.00	0.3750	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T12	0.00	0.3750	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T13	0.00	0.4375	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T14	0.00	0.4375	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T15	0.00	0.4375	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T16	0.00	0.4375	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T17	0.00	0.4375	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000
T18 20.00-0.00	0.00	0.4375	A36 (36 ksi)	1	1	0.0000	0.0000	0.0000

Tower Section Geometry (cont'd)

Tower Elevation	Leg	Diagonal	Top Girt	Bottom Girt	Mid Girt	Long Horizontal	Short Horizontal
T1	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	Net Width Deduct in	Net Width Deduct in
T2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
T3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

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Tower Elevation	Calc Single Angles	Calc K Solid Rounds	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Spec Horiz.	Inner Brace
T1	Yes	Yes	1	Y	Y	Y	Y	0.96	1	Y
T2	No	Yes	1	Y	Y	Y	Y	0.96	0.5	Y
T3	No	Yes	1	1.09	Y	Y	Y	0.5	0.5	Y
T4	No	Yes	1	1.15	Y	Y	Y	0.5	0.5	Y
T5	No	Yes	1	1.23	Y	Y	Y	0.5	0.5	Y
T6	No	Yes	1	1.25	Y	Y	Y	0.5	0.5	Y
T7	Yes	Yes	1	1.22	Y	Y	Y	1	1	Y
T8	No	Yes	1	Y	Y	Y	Y	0.5	0.5	Y
T9	No	Yes	1	Y	Y	Y	Y	0.5	0.5	Y
T10	No	Yes	1	Y	Y	Y	Y	0.5	0.5	Y
T11	No	Yes	1	0.98	Y	Y	Y	0.3	0.3	Y
T12	Yes	Yes	1	0.98	Y	Y	Y	0.5	0.5	Y
T13	No	Yes	1	0.85	Y	Y	Y	0.5	0.5	Y
T14	No	Yes	1	0.85	Y	Y	Y	0.5	0.5	Y
T15	Yes	Yes	1	Y	Y	Y	Y	1	1	Y
T16	Yes	Yes	1	Y	Y	Y	Y	1	1	Y
T17	Yes	Yes	1	Y	Y	Y	Y	1	1	Y
T18	Yes	Yes	1	Y	Y	Y	Y	1	1	Y
T18 20.00-0.00	Yes	Yes	1	Y	Y	Y	Y	1	1	Y

*Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

Tower Section Geometry (cont'd)

Tower Elevation	Leg	Diagonal	Top Girt	Bottom Girt	Mid Girt	Long Horizontal	Short Horizontal
T1	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	Net Width Deduct in	Net Width Deduct in
T2	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
T3	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

K Factors

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Description	Face Allow or Shield Leg	Component Type	Placement	Face Offset in	Lateral Offset in	# Rows	Clear Per Spacing Rows	Without Diameter in	Penmeter in	Weight plf	
WC Rail 1.5x1.5x1/8 **** **** ****	A	Yes	At (CAE)	180.00 - 2.00	0.0000	0.3	2	36.0000	1.5000	6.0000	1.23
Safety Line 3/8	A	No	At (Leg)	195.00 - 0.00	0.0000	0	1	0.3750	0.3750	0.22	
Step Pegs (5/8" SR) 7-in. w/30" step	A	No	At (Leg)	195.00 - 0.00	0.0000	0	1	0.3500	0.3500	0.49	
Step Pegs (5/8" SR) 7-in. w/30" step	B	No	At (Leg)	60.00 - 0.00	0.0000	0	1	0.3500	0.3500	0.49	
Step Pegs (5/8" SR) 7-in. w/30" step	C	No	At (Leg)	60.00 - 0.00	0.0000	0	1	0.3500	0.3500	0.49	
**** **** ****	B	Yes	At (CAE)	170.00 - 8.00	-2.0000	-0.35	1	0.5000	0.0001	1.31	
**** **** ****	B	Yes	At (CAE)	181.00 - 0.00	0.0000	0	1	0.5000	0.0001	1.29	
**** **** ****	C	Yes	At (CAE)	160.00 - 0.00	0.0000	0.1	1	0.5000	0.0001	1.05	
**** **** ****	A	Yes	At (CAE)	180.00 - 2.00	0.0000	0.3	1	0.5000	0.0001	1.29	

Feed Line/Linear Appurtenances - Entered As Area						
Description	Face Allow or Shield Leg	Component Type	Placement	Total Number	C _o A _s #/ft	Weight plf

Feed Line/Linear Appurtenances Section Areas						
Tower Section	Tower Elevation ft	Face	A _r ft ²	C _o A _s In Face ft ²	C _o A _s Out Face ft ²	Weight lb
T1	195.00-180.00	A	9.618	0.000	0.000	91.97
		B	15.756	0.250	0.000	226.32
		C	0.000	0.000	0.000	0.00
T2	180.00-175.00	A	5.747	1.250	0.000	73.16
		B	5.252	1.250	0.000	98.38
		C	0.000	0.000	0.000	0.00
T3	175.00-170.00	A	5.747	1.250	0.000	73.16
		B	5.252	1.250	0.000	98.38

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Tower Section	Tower Elevation ft	Face	A _r ft ²	C _o A _s In Face ft ²	C _o A _s Out Face ft ²	Weight lb
T4	170.00-160.00	C	0.000	0.000	0.000	0.00
		A	11.494	0.000	0.000	146.32
		B	28.324	0.000	0.000	358.14
		C	0.000	0.000	0.000	0.00
T5	160.00-150.00	A	11.494	0.000	0.000	146.32
		B	30.304	0.000	0.000	369.28
		C	0.833	2.500	0.000	51.97
T6	150.00-140.00	A	11.494	0.000	0.000	146.32
		B	30.304	0.000	0.000	369.28
		C	4.167	2.500	0.000	73.16
T7	140.00-133.33	A	7.663	1.667	0.000	97.55
		B	20.203	3.333	0.000	246.19
		C	2.778	1.667	0.000	48.77
T8	133.33-126.67	A	7.663	1.667	0.000	97.55
		B	20.203	3.333	0.000	246.19
		C	2.778	1.667	0.000	48.77
T9	126.67-120.00	A	7.663	1.667	0.000	97.55
		B	20.203	3.333	0.000	246.19
		C	2.778	1.667	0.000	48.77
T10	120.00-113.33	A	7.663	1.667	0.000	97.55
		B	20.203	3.333	0.000	246.19
		C	2.778	1.667	0.000	48.77
T11	113.33-106.67	A	7.663	1.667	0.000	97.55
		B	20.203	3.333	0.000	246.19
		C	2.778	1.667	0.000	48.77
T12	106.67-100.00	A	7.663	1.667	0.000	97.55
		B	20.203	3.333	0.000	246.19
		C	2.778	1.667	0.000	48.77
T13	100.00-80.00	A	22.988	5.000	0.000	292.64
		B	60.609	10.000	0.000	738.56
		C	8.334	5.000	0.000	146.32
T14	80.00-60.00	A	22.988	5.000	0.000	292.64
		B	60.609	10.000	0.000	738.56
		C	9.141	5.000	0.000	148.65
T15	60.00-50.00	A	11.786	2.500	0.000	146.32
		B	30.596	5.000	0.000	374.15
		C	5.271	2.500	0.000	79.53
T16	50.00-40.00	A	11.786	2.500	0.000	146.32
		B	30.596	5.000	0.000	374.15
		C	5.271	2.500	0.000	79.53
T17	40.00-20.00	A	23.571	5.000	0.000	292.64
		B	61.192	10.000	0.000	748.30
		C	10.542	5.000	0.000	159.06
T18	20.00-0.00	A	14.859	4.500	0.000	203.77
		B	37.432	8.000	0.000	492.20
		C	5.854	5.000	0.000	131.07

Feed Line/Linear Appurtenances Section Areas - With Ice						
Tower Section	Tower Elevation ft	Face	A _r ft ²	C _o A _s In Face ft ²	C _o A _s Out Face ft ²	Weight lb
T1	195.00-180.00	A	11.083	8.850	0.000	388.97
		B	10.464	15.955	0.000	724.39
		C	0.000	0.000	0.000	0.00
T2	180.00-175.00	A	6.055	7.801	0.000	265.45
		B	4.186	7.436	0.000	300.19
		C	0.000	0.000	0.000	0.00

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Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _g ft ²	A _r ft ²	C _v A _v In Face ft ²	C _v A _v Out Face ft ²	Weight lb
T3	175.00-170.00	A	0.915	6.039	7.798	0.000	0.000	264.82
		B		4.176	7.433	0.000	0.000	299.71
		C		0.000	0.000	0.000	0.000	0.00
T4	170.00-160.00	A	0.910	12.029	15.584	0.000	0.000	527.70
		B		14.050	39.837	0.000	0.000	1118.52
		C		0.000	0.000	0.000	0.000	0.00
T5	160.00-150.00	A	0.903	11.961	15.569	0.000	0.000	524.99
		B		14.439	42.080	0.000	0.000	1156.22
		C		2.014	5.382	0.000	0.000	148.11
T6	150.00-140.00	A	0.896	11.889	15.553	0.000	0.000	522.13
		B		14.355	42.048	0.000	0.000	1151.15
		C		4.028	8.866	0.000	0.000	247.93
T7	140.00-133.33	A	0.889	7.884	10.359	0.000	0.000	346.42
		B		9.521	28.013	0.000	0.000	764.46
		C		2.671	5.901	0.000	0.000	164.40
T8	133.33-126.67	A	0.884	7.848	10.351	0.000	0.000	345.02
		B		9.479	27.997	0.000	0.000	761.97
		C		2.659	5.893	0.000	0.000	163.66
T9	126.67-120.00	A	0.879	7.811	10.343	0.000	0.000	343.56
		B		9.436	27.981	0.000	0.000	759.37
		C		2.647	5.885	0.000	0.000	162.88
T10	120.00-113.33	A	0.873	7.772	10.335	0.000	0.000	342.03
		B		9.391	27.964	0.000	0.000	756.65
		C		2.634	5.876	0.000	0.000	162.07
T11	113.33-106.67	A	0.867	7.731	10.325	0.000	0.000	340.43
		B		9.343	27.945	0.000	0.000	753.79
		C		2.620	5.867	0.000	0.000	161.22
T12	106.67-100.00	A	0.860	7.688	10.316	0.000	0.000	338.75
		B		9.293	27.926	0.000	0.000	750.79
		C		2.606	5.858	0.000	0.000	160.33
T13	100.00-80.00	A	0.846	7.278	30.885	0.000	0.000	1005.29
		B		27.548	83.653	0.000	0.000	2232.75
		C		7.723	17.510	0.000	0.000	475.16
T14	80.00-60.00	A	0.821	22.279	30.773	0.000	0.000	986.01
		B		26.961	83.430	0.000	0.000	2198.12
		C		10.483	17.398	0.000	0.000	489.72
T15	60.00-50.00	A	0.797	12.526	15.335	0.000	0.000	484.15
		B		14.828	41.611	0.000	0.000	1099.12
		C		8.791	8.647	0.000	0.000	259.16
T16	50.00-40.00	A	0.778	12.305	15.292	0.000	0.000	477.05
		B		14.575	41.526	0.000	0.000	1085.81
		C		8.633	8.605	0.000	0.000	254.42
T17	40.00-20.00	A	0.750	23.946	30.458	0.000	0.000	933.10
		B		28.392	82.800	0.000	0.000	2132.07
		C		16.792	17.083	0.000	0.000	494.88
T18	20.00-0.00	A	0.750	18.834	20.775	0.000	0.000	639.41
		B		21.752	53.013	0.000	0.000	1377.27
		C		12.729	12.708	0.000	0.000	367.49

Feed Line Shielding

Section	Elevation ft	Face	A _g ft ²	A _r ft ²	A _r Ice ft ²	C _{Fx} Ice in	C _{Fz} Ice in
T1	195.00-180.00	A	0.355	3.060	0.363	0.600	
		B	0.615	4.459	0.629	0.875	
		C	0.000	0.000	0.000	0.000	

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Section	Elevation ft	Face	A _g ft ²	A _r ft ²	A _r Ice ft ²	C _{Fx} Ice in	C _{Fz} Ice in
T2	180.00-175.00	A	0.000	1.661	0.781	1.462	
		B	0.000	1.365	0.723	1.201	
		C	0.000	0.000	0.000	0.000	
T3	175.00-170.00	A	0.000	1.560	0.912	1.705	
		B	0.000	1.282	0.845	1.402	
		C	0.000	0.000	0.000	0.000	
T4	170.00-160.00	A	0.000	2.904	1.394	2.602	
		B	0.000	6.075	3.407	5.444	
		C	0.000	0.000	0.000	0.000	
T5	160.00-150.00	A	0.000	2.704	1.609	2.995	
		B	0.000	5.959	4.170	6.599	
		C	0.000	0.912	0.401	1.009	
T6	150.00-140.00	A	0.000	2.566	1.544	2.865	
		B	0.000	5.660	4.001	6.319	
		C	0.000	1.434	0.769	1.601	
T7	140.00-133.33	A	0.000	0.965	0.733	1.356	
		B	0.000	2.130	1.898	2.993	
		C	0.000	0.539	0.365	0.757	
T8	133.33-126.67	A	0.000	0.926	0.709	1.309	
		B	0.000	2.046	1.837	2.893	
		C	0.000	0.517	0.353	0.731	
T9	126.67-120.00	A	0.000	0.893	0.690	1.271	
		B	0.000	1.975	1.788	2.810	
		C	0.000	0.499	0.343	0.710	
T10	120.00-113.33	A	0.000	1.223	1.144	2.102	
		B	0.000	2.706	2.964	4.651	
		C	0.000	0.682	0.569	1.173	
T11	113.33-106.67	A	0.000	1.195	1.128	2.068	
		B	0.000	2.645	2.923	4.579	
		C	0.000	0.666	0.561	1.153	
T12	106.67-100.00	A	0.000	1.169	0.984	1.801	
		B	0.000	2.590	2.551	3.990	
		C	0.000	0.651	0.490	1.004	
T13	100.00-80.00	A	0.000	3.366	3.661	6.658	
		B	0.000	7.472	9.489	14.781	
		C	0.000	1.874	1.823	3.707	
T14	80.00-60.00	A	0.000	2.178	2.580	4.643	
		B	0.000	4.851	6.686	10.343	
		C	0.000	1.342	1.362	2.862	
T15	60.00-50.00	A	0.000	1.065	1.236	2.202	
		B	0.000	2.380	3.203	4.922	
		C	0.000	0.673	0.663	1.392	
T16	50.00-40.00	A	0.000	1.020	1.223	2.162	
		B	0.000	2.285	3.170	4.845	
		C	0.000	0.643	0.657	1.363	
T17	40.00-20.00	A	0.000	1.330	2.030	3.546	
		B	0.000	2.991	5.260	7.977	
		C	0.000	0.835	1.090	2.227	
T18	20.00-0.00	A	0.000	1.884	1.628	2.948	
		B	0.000	4.044	4.044	6.314	
		C	0.000	0.582	0.898	1.940	

Feed Line Center of Pressure

Section	Elevation ft	Face	C _{Fx} Ice in	C _{Fz} Ice in	C _{Fx} Ice in	C _{Fz} Ice in
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tnxTower		Middlebury I-84 (CT11128E)		Page	13 of 29
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Client		T-Mobile Towers		Designed by Christopher Bean	

Section	Elevation	CPx	CPz	CPx	CPz
	ft	In	In	In	In
T1	195.00-180.00	2.4370	-4.7977	0.9739	-3.0959
T2	180.00-175.00	1.5865	-6.5234	0.8832	-4.7814
T3	175.00-170.00	1.6191	-6.6923	0.9227	-5.0229
T4	170.00-160.00	1.8936	-12.0895	1.2687	-9.3582
T5	160.00-150.00	1.8421	-12.0717	1.1398	-9.5074
T6	150.00-140.00	1.9450	-12.1640	1.2489	-9.5037
T7	140.00-133.33	2.1393	-13.0677	1.5875	-11.6278
T8	133.33-126.67	2.3423	-14.0993	1.7470	-12.5619
T9	126.67-120.00	2.5364	-15.0802	1.8993	-13.4450
T10	120.00-113.33	2.3140	-13.6337	1.6647	-11.7336
T11	113.33-106.67	2.4550	-14.3331	1.7734	-12.3516
T12	106.67-100.00	2.7195	-15.7450	1.9494	-13.2196
T13	100.00-80.00	2.6160	-14.9594	1.9552	-13.4494
T14	80.00-60.00	3.2563	-18.1833	2.5662	-16.5771
T15	60.00-50.00	3.5382	-19.3106	2.7222	-16.8716
T16	50.00-40.00	3.7191	-20.1985	2.8808	-17.1998
T17	40.00-20.00	4.2430	-22.8945	3.4202	-20.2076
T18	20.00-0.00	2.9814	-15.7858	2.5501	-14.8240

Discrete Tower Loads

Description	Face or Leg	Offset Type	Azimuth Adjustment	Placement	C _A Front	C _A Side	Weight
			°	ft	ft²	ft²	lb
1.75" Dia x 5.4' Pipe	C	From Leg	-30.0000	75.50	0.88	12.00	
			0.00		1.32	19.06	
			0.00		1.63	29.51	
					2.28	61.18	
					3.68	171.86	
GPS0015	C	From Leg	-30.0000	75.50	0.10	0.50	
			0.00		0.15	2.29	
			0.75		0.22	4.89	
					0.38	13.15	
					0.83	46.16	
Sector Mount (SM 502-3)	C	None	0.0000	152.00	33.02	1673.10	
					47.36	2233.90	
					61.70	2774.70	
					90.38	3876.30	
					147.74	6079.50	
APXYSP18-C-A20 w/ Mount Pipe	A	From Leg	0.0000	152.00	6.71	78.90	
			0.00		8.81	144.31	
			0.60		9.36	217.47	
					10.50	390.34	
					12.88	872.84	
APXYSP18-C-A20 w/ Mount Pipe	B	From Leg	0.0000	152.00	6.71	78.90	
			0.00		8.26	99.34	
			0.00		9.36	217.47	
					10.50	390.34	
					12.88	872.84	
APXYSP18-C-A20 w/ Mount Pipe	C	From Leg	0.0000	152.00	6.71	78.90	

tnxTower		Middlebury I-84 (CT11128E)		Page	14 of 29
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Description	Face or Leg	Offset Type	Azimuth Adjustment	Placement	C _A Front	C _A Side	Weight
			°	ft	ft²	ft²	lb
Mount Pipe			0.00		8.81	7.66	14.31
			0.00		9.36	8.49	217.47
					10.50	10.20	390.34
					12.88	13.98	872.84
800MHz 2X50W RRH	A	From Leg	0.0000	152.00	2.49	2.07	53.00
			0.00		2.93	2.48	98.39
					3.41	2.93	156.61
					4.46	3.93	317.77
800MHz 2X50W RRH	B	From Leg	0.0000	152.00	2.49	2.07	53.00
			0.00		2.93	2.48	98.39
					3.41	2.93	156.61
					4.46	3.93	317.77
800MHz 2X50W RRH	C	From Leg	0.0000	152.00	2.49	2.07	53.00
			0.00		2.93	2.48	98.39
					3.41	2.93	156.61
					4.46	3.93	317.77
1900MHz 2X40W RRH	A	From Leg	0.0000	152.00	2.71	2.61	59.50
			0.00		3.20	2.84	82.62
					3.72	3.61	108.98
					4.86	4.74	345.91
1900MHz 2X40W RRH	B	From Leg	0.0000	152.00	2.71	2.61	59.50
			0.00		3.20	2.84	82.62
					3.72	3.61	108.98
					4.86	4.74	345.91
1900MHz 2X40W RRH	C	From Leg	0.0000	152.00	2.71	2.61	59.50
			0.00		3.20	2.84	82.62
					3.72	3.61	108.98
					4.86	4.74	345.91
DB980H90T2E-M w/ Mount Pipe	A	From Leg	5.0000	152.00	4.04	4.48	66.41
			0.00		4.95	5.22	108.76
					5.87	6.74	215.74
					8.05	10.00	549.44
DB980H90T2E-M w/ Mount Pipe	B	From Leg	5.0000	152.00	4.04	4.48	66.41
			0.00		4.95	5.22	108.76
					5.87	6.74	215.74
					8.05	10.00	549.44
DB980H90T2E-M w/ Mount Pipe	C	From Leg	5.0000	152.00	4.04	4.48	66.41
			0.00		4.95	5.22	108.76
					5.87	6.74	215.74
					8.05	10.00	549.44
APXYTM14-C-120 w/ Mount Pipe	A	From Leg	5.0000	152.00	6.90	3.61	36.20
			0.00		7.35	3.97	95.73
			0.00		7.81	4.33	140.32
					10.75	5.14	245.49
APXYTM14-C-120 w/ Mount Pipe	B	From Leg	5.0000	152.00	6.90	3.61	36.20
			0.00		7.35	3.97	95.73
			0.00		7.81	4.33	140.32
					10.75	5.14	245.49

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Description	Face or Leg	Offset Type	Offsets: Horiz. Lateral Vert	Azimuth Adjustment	Placement	C _A A _S Front Side	C _A A _S Front Side	Weight
APXVTM14-C-120 w/ Mount Pipe	C	From Leg	5.00	0.0000	152.00	8.75	5.14	245.49
			0.00			10.75	6.97	525.21
			0.00			6.90	3.61	56.20
TD-RRH8x20-25	A	From Leg	5.00	0.0000	152.00	8.75	5.14	245.49
			0.00			10.75	6.97	525.21
			0.00			6.90	3.61	56.20
			5.00	0.0000	152.00	8.75	5.14	245.49
			0.00			10.75	6.97	525.21
			0.00			6.90	3.61	56.20
	B	From Leg	5.00	0.0000	152.00	8.75	5.14	245.49
			0.00			10.75	6.97	525.21
			0.00			6.90	3.61	56.20
			5.00	0.0000	152.00	8.75	5.14	245.49
			0.00			10.75	6.97	525.21
			0.00			6.90	3.61	56.20
TD-RRH8x20-25	C	From Leg	5.00	0.0000	152.00	8.75	5.14	245.49
			0.00			10.75	6.97	525.21
			0.00			6.90	3.61	56.20
			5.00	0.0000	152.00	8.75	5.14	245.49
			0.00			10.75	6.97	525.21
			0.00			6.90	3.61	56.20
****	C	None	5.00	0.0000	169.00	21.56	21.56	1395.40
			0.00			29.77	29.77	2140.10
			0.00			37.98	37.98	2884.80
			5.00	0.0000	169.00	21.56	21.56	1395.40
			0.00			29.77	29.77	2140.10
			0.00			37.98	37.98	2884.80
(2) BXA-70063/GCT w/ Mount Pipe	A	From Leg	5.00	0.0000	169.00	54.40	54.40	4374.20
			0.00			87.24	87.24	7353.00
			0.00			7.75	7.75	61.11
			5.00	0.0000	169.00	54.40	54.40	4374.20
			0.00			87.24	87.24	7353.00
			0.00			7.75	7.75	61.11
(2) DB844G6SZAXY w/ Mount Pipe	B	From Leg	5.00	0.0000	169.00	12.34	12.34	754.05
			0.00			9.97	9.97	8.59
			0.00			5.38	5.38	5.40
			5.00	0.0000	169.00	12.34	12.34	754.05
			0.00			9.97	9.97	8.59
			0.00			5.38	5.38	5.40
(2) DB844G6SZAXY w/ Mount Pipe	C	From Leg	5.00	0.0000	169.00	10.34	10.34	688.90
			0.00			7.83	7.83	8.96
			0.00			5.38	5.38	5.40
			5.00	0.0000	169.00	10.34	10.34	688.90
			0.00			7.83	7.83	8.96
			0.00			5.38	5.38	5.40
DB-B1/T1 w/ Mount Pipe	C	From Leg	5.00	0.0000	169.00	4.56	4.56	57.55
			0.00			6.29	6.29	5.55
			0.00			8.08	8.08	7.69
			5.00	0.0000	169.00	4.56	4.56	57.55
			0.00			6.29	6.29	5.55
			0.00			8.08	8.08	7.69
(2) HBXX-6517DS-A2M w/ Mount Pipe	A	From Leg	5.00	0.0000	169.00	10.79	10.79	674.64
			0.00			8.98	8.98	6.96
			0.00			9.14	9.14	214.64
			5.00	0.0000	169.00	10.79	10.79	674.64
			0.00			8.98	8.98	6.96
			0.00			9.14	9.14	214.64
(2) HBXX-6517DS-A2M w/ Mount Pipe	B	From Leg	5.00	0.0000	169.00	14.32	14.32	913.98
			0.00			8.98	8.98	6.96
			0.00			9.14	9.14	214.64
			5.00	0.0000	169.00	14.32	14.32	913.98
			0.00			8.98	8.98	6.96
			0.00			9.14	9.14	214.64

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Description	Face or Leg	Offset Type	Offsets: Horiz. Lateral Vert	Azimuth Adjustment	Placement	C _A A _S Front Side	C _A A _S Front Side	Weight	
									ft
(2) HBXX-6517DS-A2M w/ Mount Pipe	C	From Leg	5.00	0.0000	169.00	4.32	4.32	15.03	
			0.00			8.98	8.98	6.96	
			0.00			9.14	9.14	214.64	
			5.00	0.0000	169.00	4.32	4.32	15.03	
			0.00			8.98	8.98	6.96	
			0.00			9.14	9.14	214.64	
	RRH2x60-AWS	A	From Leg	5.00	0.0000	169.00	4.32	4.32	15.03
				0.00			8.98	8.98	6.96
				0.00			9.14	9.14	214.64
				5.00	0.0000	169.00	4.32	4.32	15.03
				0.00			8.98	8.98	6.96
				0.00			9.14	9.14	214.64
RRH2x60-AWS	B	From Leg	5.00	0.0000	169.00	4.32	4.32	15.03	
			0.00			8.98	8.98	6.96	
			0.00			9.14	9.14	214.64	
			5.00	0.0000	169.00	4.32	4.32	15.03	
			0.00			8.98	8.98	6.96	
			0.00			9.14	9.14	214.64	
	RRH2x60-AWS	C	From Leg	5.00	0.0000	169.00	4.32	4.32	15.03
				0.00			8.98	8.98	6.96
				0.00			9.14	9.14	214.64
				5.00	0.0000	169.00	4.32	4.32	15.03
				0.00			8.98	8.98	6.96
				0.00			9.14	9.14	214.64
RRH2X60-PCS	A	From Leg	5.00	0.0000	169.00	2.57	2.57	2.01	
			0.00			2.79	2.79	2.22	
			0.00			3.02	3.02	2.43	
			5.00	0.0000	169.00	2.57	2.57	2.01	
			0.00			2.79	2.79	2.22	
			0.00			3.02	3.02	2.43	
	RRH2X60-PCS	B	From Leg	5.00	0.0000	169.00	2.57	2.57	2.01
				0.00			2.79	2.79	2.22
				0.00			3.02	3.02	2.43
				5.00	0.0000	169.00	2.57	2.57	2.01
				0.00			2.79	2.79	2.22
				0.00			3.02	3.02	2.43
RRH2X60-PCS	C	From Leg	5.00	0.0000	169.00	2.57	2.57	2.01	
			0.00			2.79	2.79	2.22	
			0.00			3.02	3.02	2.43	
			5.00	0.0000	169.00	2.57	2.57	2.01	
			0.00			2.79	2.79	2.22	
			0.00			3.02	3.02	2.43	
	BXA-70080/GCF w/ Mount Pipe	A	From Leg	5.00	0.0000	169.00	7.70	7.70	5.54
				0.00			8.79	8.79	7.30
				0.00			9.92	9.92	8.48
				5.00	0.0000	169.00	7.70	7.70	5.54
				0.00			8.79	8.79	7.30
				0.00			9.92	9.92	8.48
DB846F65ZAXY w/ Mount Pipe	B	From Leg	5.00	0.0000	169.00	7.27	7.27	46.55	
			0.00			7.88	7.88	9.01	
			0.00			8.48	8.48	9.91	
			5.00	0.0000	169.00	7.27	7.27	46.55	
			0.00			7.88	7.88	9.01	
			0.00			8.48	8.48	9.91	
DB846F65ZAXY w/ Mount Pipe	C	From Leg	5.00	0.0000	169.00	12.33	12.33	15.98	
			0.00			9.72	9.72	11.81	
			0.00			11.81	11.81	367.34	
			5.00	0.0000	169.00	12.33	12.33	15.98	
			0.00			9.72	9.72	11.81	
			0.00			11.81	11.81	367.34	
(2) FD29R6004	A	From Leg	5.00	0.0000	169.00	0.37	0.37	0.08	
			0.00			0.45	0.45	0.14	
			0.00			0.75	0.75	0.54	
			5.00	0.0000	169.00	0.37	0.37	0.08	
			0.00			0.45	0.45	0.14	
			0.00			0.75	0.75	0.54	
(2) FD29R6004	B	From Leg	5.00	0.0000	169.00	1.28	1.28	0.74	
			0.00			1.28	1.28	0.74	
			0.00			1.28	1.28	0.74	
			5.00	0.0000	169.00	1.28	1.28	0.74	
			0.00			1.28	1.28	0.74	
			0.00			1.28	1.28	0.74	

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	C _A Front	C _A Side	Weight
			ft ft ft	°	ft	ft	ft	lb
(2)LGP21401	A	From Leg	0.00 3.00	-30.00000	188.00	0.44	0.47	12.97
			1/2" Ice					19.68
			1" Ice					39.83
			2" Ice					10.00
			4" Ice					21.26
(2)LGP21401	B	From Leg	5.00 3.00	-30.00000	188.00			30.32
			1/2" Ice					54.89
			1" Ice					135.29
			2" Ice					10.00
			4" Ice					21.26
(2)LGP21401	C	From Leg	5.00 3.00	-30.00000	188.00			30.32
			1/2" Ice					54.89
			1" Ice					135.29
			2" Ice					10.00
			4" Ice					21.26
(2)RRUS-11	A	From Leg	5.00 3.00	-30.00000	188.00			30.32
			1/2" Ice					54.89
			1" Ice					135.29
			2" Ice					10.00
			4" Ice					21.26
(2)RRUS-11	B	From Leg	5.00 3.00	-30.00000	188.00			30.32
			1/2" Ice					54.89
			1" Ice					135.29
			2" Ice					10.00
			4" Ice					21.26
(2)RRUS-11	C	From Leg	5.00 3.00	-30.00000	188.00			30.32
			1/2" Ice					54.89
			1" Ice					135.29
			2" Ice					10.00
			4" Ice					21.26
DC6-48-60-18-8F	A	From Leg	5.00 3.00	-30.00000	188.00			30.32
			1/2" Ice					54.89
			1" Ice					135.29
			2" Ice					10.00
			4" Ice					21.26

Sector Mount (SM 802-3)	C	None	0.00	0.00000	195.00			930.00
			1/2" Ice					31.39
			1" Ice					38.37
			2" Ice					52.33
			4" Ice					80.25
HSS Top Mount	C	None	0.00	0.00000	195.00			808
			1/2" Ice					9.70
			1" Ice					11.32
			2" Ice					14.56
			4" Ice					21.04
LNX-651SDS-VTM w/ Mount Pipe	A	From Leg	3.00 0.00	25.00000	193.00			1068.51
			1/2" Ice					11.83
			1" Ice					15.57
			2" Ice					25.17
			4" Ice					39.83
LNX-651SDS-VTM w/ Mount Pipe	B	From Leg	3.00 0.00	25.00000	193.00			1068.51
			1/2" Ice					11.83
			1" Ice					15.57
			2" Ice					25.17
			4" Ice					39.83

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	C _A Front	C _A Side	Weight
			ft ft ft	°	ft	ft	ft	lb
(3) Sector Mounts 188-ft	C	None	0.00 0.00	0.00000	188.00	0.45	0.14	5.40
			1/2" Ice					8.79
			1" Ice					19.61
			2" Ice					62.87
			4" Ice					3.10
(2)FDR6004	C	From Leg	5.00 0.00	0.00000	169.00			5.40
			1/2" Ice					8.79
			1" Ice					19.61
			2" Ice					62.87
			4" Ice					3.10

(2)7770.00 w/ Mount Pipe	A	From Leg	5.00 3.00	-30.00000	188.00			1395.40
			1/2" Ice					2140.10
			1" Ice					3798
			2" Ice					4374.20
			4" Ice					7353.00
(2)7770.00 w/ Mount Pipe	B	From Leg	5.00 3.00	-30.00000	188.00			1395.40
			1/2" Ice					2140.10
			1" Ice					3798
			2" Ice					4374.20
			4" Ice					7353.00
(2)7770.00 w/ Mount Pipe	C	From Leg	5.00 3.00	-30.00000	188.00			1395.40
			1/2" Ice					2140.10
			1" Ice					3798
			2" Ice					4374.20
			4" Ice					7353.00
P65-17-XL-H-RR w/Mount Pipe	A	From Leg	5.00 0.00 3.00	-30.00000	188.00			679.83
			1" Ice					56.90
			2" Ice					160.42
			4" Ice					293.10
P65-17-XL-H-RR w/Mount Pipe	B	From Leg	5.00 0.00 3.00	-30.00000	188.00			679.83
			1" Ice					56.90
			2" Ice					160.42
			4" Ice					293.10
P65-17-XL-H-RR w/Mount Pipe	C	From Leg	5.00 0.00 3.00	-30.00000	188.00			679.83
			1" Ice					56.90
			2" Ice					160.42
			4" Ice					293.10
P65-17-XL-H-RR w/Mount Pipe	A	From Leg	5.00 0.00 3.00	-30.00000	188.00			679.83
			1" Ice					56.90
			2" Ice					160.42
			4" Ice					293.10
P65-17-XL-H-RR w/Mount Pipe	B	From Leg	5.00 0.00 3.00	-30.00000	188.00			679.83
			1" Ice					56.90
			2" Ice					160.42
			4" Ice					293.10
P65-17-XL-H-RR w/Mount Pipe	C	From Leg	5.00 0.00 3.00	-30.00000	188.00			679.83
			1" Ice					56.90
			2" Ice					160.42
			4" Ice					293.10

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Client T-Mobile Towers		Designed by Christopher Bean			

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Client T-Mobile Towers		Designed by Christopher Bean			

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C/Ax Front	C/Ax Side	Weight
			Horz. Lateral	Vert.					
			f	f	o	f	f ²	f ²	lb
			0.00				12.45	11.71	251.57
				1" Ice			13.74	13.82	469.57
				2" Ice			16.75	18.22	1068.51
LNx-651SDS-VTM w/ Mount Pipe	C	From Leg	3.00		25.0000	193.00	11.21	9.36	75.35
			0.00	1/2" Ice			11.83	10.68	158.69
			0.00	1" Ice			12.45	11.71	251.57
				2" Ice			13.74	13.82	469.57
APX16DWW-16D WVS-A	A	From Leg	3.00		25.0000	195.00	7.07	2.15	40.70
			0.00	1/2" Ice			7.52	2.49	73.65
			0.00	1" Ice			7.98	2.84	111.47
				2" Ice			8.92	3.55	202.47
APX16DWW-16D WVS-A	B	From Leg	3.00		25.0000	195.00	10.91	5.08	451.40
			0.00	1/2" Ice			7.07	2.15	40.70
			0.00	1" Ice			7.98	2.84	111.47
			0.00	2" Ice			8.92	3.55	202.47
APX16DWW-16D WVS-A	C	From Leg	3.00		25.0000	195.00	10.91	5.08	451.40
			0.00	1/2" Ice			7.52	2.49	73.65
			0.00	1" Ice			7.98	2.84	111.47
				2" Ice			8.92	3.55	202.47
(2) KRY 112.71	A	From Leg	3.00		25.0000	195.00	10.91	5.08	451.40
			0.00	1/2" Ice			7.52	2.49	73.65
			0.00	1" Ice			7.98	2.84	111.47
				2" Ice			8.92	3.55	202.47
(2) KRY 112.71	B	From Leg	3.00		25.0000	195.00	10.91	5.08	451.40
			0.00	1/2" Ice			7.52	2.49	73.65
			0.00	1" Ice			7.98	2.84	111.47
				2" Ice			8.92	3.55	202.47
(2) KRY 112.71	C	From Leg	3.00		25.0000	195.00	10.91	5.08	451.40
			0.00	1/2" Ice			7.52	2.49	73.65
			0.00	1" Ice			7.98	2.84	111.47
				2" Ice			8.92	3.55	202.47

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice

Comb. No.	Description
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice
14	Dead+Ice+Temp
15	Dead+Wind 0 deg+Ice+Temp
16	Dead+Wind 30 deg+Ice+Temp
17	Dead+Wind 60 deg+Ice+Temp
18	Dead+Wind 90 deg+Ice+Temp
19	Dead+Wind 120 deg+Ice+Temp
20	Dead+Wind 150 deg+Ice+Temp
21	Dead+Wind 180 deg+Ice+Temp
22	Dead+Wind 210 deg+Ice+Temp
23	Dead+Wind 240 deg+Ice+Temp
24	Dead+Wind 270 deg+Ice+Temp
25	Dead+Wind 300 deg+Ice+Temp
26	Dead+Wind 330 deg+Ice+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37	Dead+Wind 300 deg - Service
38	Dead+Wind 330 deg - Service

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt o	Twist o
T1	195 - 180	8.983	27	0.5205	0.0852
T2	180 - 175	7.287	27	0.4834	0.0502
T3	175 - 170	6.784	27	0.4542	0.0471
T4	170 - 160	6.325	27	0.4228	0.0451
T5	160 - 150	5.465	27	0.3843	0.0435
T6	150 - 140	4.708	27	0.3294	0.0412
T7	140 - 133.333	4.046	27	0.2910	0.0385
T8	133.333 - 126.667	3.640	27	0.2776	0.0361
T9	126.667 - 120	3.252	27	0.2496	0.0304
T10	120 - 113.333	2.880	27	0.2276	0.0275
T11	113.333 - 106.667	2.535	27	0.2053	0.0247
T12	106.667 - 100	2.221	27	0.1828	0.0222
T13	100 - 80	1.940	27	0.1492	0.0165
T14	80 - 60	1.217	27	0.1039	0.0108
T15	60 - 50	0.655	27	0.0813	0.0083
T16	50 - 40	0.446	27	0.0588	0.0059
T17	40 - 20	0.285	27	0.0292	0.0028
T18	20 - 0	0.081	27		

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Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt lb	Allowable Load lb	Ratio Load/Allowable	Allowable Ratio	Criteria
T1	195	Leg	A325N	0.7500	4	4404.88	19438.60	0.227	1.333	Bolt Tension
T2	180	Diagonal	A325N	0.5000	1	2823.53	3126.36	0.903	1.333	Member Block Shear
T3	175	Secondary Horizontal	A325N	0.6250	1	572.18	4553.91	0.126	1.333	Member Block Shear
T4	170	Diagonal	A325X	0.5000	1	2744.96	4078.13	0.673	1.333	Member Bearing
T5	160	Secondary Horizontal	A325N	0.6250	1	710.74	4553.91	0.156	1.333	Member Block Shear
T6	150	Leg	A325N	0.7500	6	9228.08	19438.10	0.475	1.333	Bolt Tension
T7	140	Diagonal	A325N	0.5000	1	4988.61	5437.50	0.917	1.333	Gusset Bearing
T8	133.333	Secondary Horizontal	A325N	0.6250	1	1079.97	4553.91	0.237	1.333	Member Block Shear
T9	126.667	Diagonal	A325X	0.5000	1	5344.07	5437.50	0.983	1.333	Gusset Bearing
T10	120	Secondary Horizontal	A325N	0.6250	1	1457.96	4553.91	0.320	1.333	Member Block Shear
T11	113.333	Leg	A325N	1.0000	6	16141.80	34557.30	0.467	1.333	Bolt Tension
T12	106.667	Diagonal	A325N	0.5000	1	6104.61	5437.50	1.123	1.333	Gusset Bearing
T13	100	Secondary Horizontal	A325N	0.6250	1	1884.28	4553.91	0.414	1.333	Member Block Shear
T14	80	Diagonal	A325X	0.6250	1	6440.41	8156.25	0.790	1.333	Member Bearing
T15	60	Diagonal	A325X	0.6250	1	6576.58	8156.25	0.806	1.333	Member Bearing
T16	50	Leg	A325N	1.0000	8	16781.90	34557.50	0.486	1.333	Bolt Tension
T17	40	Diagonal	A325N	0.6250	1	6415.84	5097.66	1.259	1.333	Member Bearing
T18	20	Secondary Horizontal	A325N	0.6250	1	6649.98	5097.66	1.305	1.333	Member Bearing
T19	113.333	Diagonal	A325N	0.6250	1	2803.36	5097.66	0.550	1.333	Member Bearing
T20	106.667	Secondary Horizontal	A325N	0.6250	1	6618.01	5097.66	1.298	1.333	Member Bearing
T21	100	Leg	A325N	1.0000	8	20835.80	34557.10	0.603	1.333	Bolt Tension
T22	80	Diagonal	A325X	0.6250	1	6889.92	8156.25	0.848	1.333	Member Bearing
T23	60	Leg	A325N	1.2500	8	24667.70	53996.00	0.457	1.333	Bolt Tension
T24	40	Diagonal	A325N	0.6250	2	3853.11	6442.72	0.598	1.333	Bolt Shear
T25	20	Secondary Horizontal	A325N	0.6250	1	3857.70	6442.72	0.599	1.333	Bolt Shear
T26	15	Leg	A325N	1.2500	8	28300.10	53996.10	0.524	1.333	Bolt Tension
T27	10	Diagonal	A325N	0.6250	2	4009.85	6442.72	0.622	1.333	Bolt Shear
T28	5	Leg	A325N	0.6250	2	4985.52	6442.72	0.774	1.333	Bolt Shear
T29	0	Diagonal	A325N	1.2500	8	31318.90	53996.90	0.580	1.333	Bolt Tension
T30	0	Leg	A325N	0.6250	2	4992.11	6442.72	0.775	1.333	Bolt Shear
T31	0	Diagonal	A325N	1.2500	8	34665.60	53996.10	0.642	1.333	Bolt Tension
T32	0	Leg	A325N	0.6250	2	4941.17	6442.72	0.767	1.333	Bolt Shear
T33	0	Secondary Horizontal	A36	1.5000	8	38603.00	33823.20	1.141	1.333	Bolt Tension
T34	0	Diagonal	A325N	0.6250	2	5568.15	6442.72	0.864	1.333	Bolt Shear

Compression Checks

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Client T-Mobile Towers		Designed by Christopher Bean			

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appearance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
195.00	Sector Mount [SM 802-3]	27	8.983	0.5205	0.0852	22657
188.00	LNX-6515DS-VTM w/ Mount Pipe	27	8.749	0.5180	0.0784	22657
169.00	(3) Sector Mounts 188-ft	27	8.169	0.5099	0.0626	16183
152.00	(3) Sector Mounts 169-ft	27	6.235	0.4179	0.0449	17373
75.50	Sector Mount [SM 502-3]	27	4.851	0.3405	0.0417	11789
	1.75" Dia x 5-ft Pipe	27	1.076	0.1407	0.0153	30319

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Tilt °	Twist °
T1	195 - 180	25.707	1.4880	0.2325
T2	180 - 170	20.864	1.3834	0.1448
T3	175 - 170	19.423	1.2907	0.1360
T4	170 - 160	18.110	1.2101	0.1303
T5	160 - 150	15.649	1.0999	0.1255
T6	150 - 140	13.484	0.9427	0.1190
T7	140 - 133.333	11.589	0.8326	0.1112
T8	133.333 - 126.667	10.425	0.7943	0.1044
T9	126.667 - 120	9.314	0.7546	0.0972
T10	120 - 113.333	8.250	0.7143	0.0877
T11	113.333 - 106.667	7.263	0.6513	0.0796
T12	106.667 - 100	6.363	0.5874	0.0715
T13	100 - 80	5.558	0.5232	0.0641
T14	80 - 60	3.490	0.4268	0.0478
T15	60 - 50	1.879	0.2972	0.0312
T16	50 - 40	1.279	0.2326	0.0240
T17	40 - 20	0.818	0.1682	0.0170
T18	20 - 0	0.233	0.0837	0.0082

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appearance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
195.00	Sector Mount [SM 802-3]	2	25.707	1.4889	0.2325	8084
193.00	LNX-6515DS-VTM w/ Mount Pipe	2	25.040	1.4817	0.2154	8084
188.00	(3) Sector Mounts 188-ft	2	23.386	1.4587	0.1753	5774
169.00	(3) Sector Mounts 169-ft	2	17.854	1.1959	0.1295	6074
152.00	Sector Mount [SM 502-3]	2	13.893	0.9745	0.1205	4123
75.50	1.75" Dia x 5-ft Pipe	2	3.085	0.4026	0.0441	10612

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Leg Design Data (Compression)

Section No.	Elevation ft	Size	L		K/U	F _a psi	A in ²	Actual		Allow.		Ratio P _a /P _a
			β	fr				P	lb	P _a	lb	
T1	195 - 180	PIPE 2.5 STD (SCH 40)	15.00	3.75	47.4	26833.100	1.7072	-29719.30	45810.40	0.649		
T2	180 - 175	PIPE 2.5 STD (SCH 40)	5.01	2.67	33.8	29103.699	1.7072	-32993.60	49687.00	0.664		
T3	175 - 170	PIPE 2.5 STD (SCH 40)	5.01	2.65	33.5	29141.699	1.7072	-40983.70	49751.90	0.824		
T4	170 - 160	2.5SCH40 w/ 3SCH180 Half Sleeve	10.02	2.62	34.2	19124.000	3.2120	-62274.10	61426.70	1.014		
T5	160 - 150	PIPE 3.5 Std (SCH40)	10.02	2.60	23.4	30584.000	2.6795	-84070.20	81951.10	1.026		
T6	150 - 140	3.5SCH40 w/ 4SCH140 Half Sleeve	10.02	2.59	23.8	27885.500	4.2666	-108633.00	118976.00	0.913		
T7	140 - 133.333	5 STD w/ 6 XH Half Sleeve	6.68	6.68	45.4	23299.900	8.5023	-122005.00	198103.00	0.616		
T8	133.333 - 126.667	5 STD w/ 6 XH Half Sleeve	6.68	6.68	45.4	23299.900	8.5023	-136220.00	198103.00	0.688		
T9	126.667 - 120	5 STD w/ 6 XH Half Sleeve	6.68	6.68	45.4	23299.900	8.5023	-149868.00	198103.00	0.757		
T10	120 - 113.333	PPE 6 STD (SCH40)	6.68	3.45	18.4	31206.400	5.5858	-161650.00	174311.00	0.927		
T11	113.333 - 106.667	PPE 6 STD (SCH40)	6.68	3.44	18.4	31211.000	5.5858	-174393.00	174377.00	1.000		
T12	106.667 - 100	PPE 6 STD (SCH40)	6.68	3.44	18.4	31215.100	5.5858	-186243.00	174360.00	1.068		
T13	100 - 80	6 STD w/ 7 XH Half Sleeve	20.03	3.42	19.5	23952.801	11.1773	-222447.00	267728.00	0.831		
T14	80 - 60	PPE 8 STD (SCH40)	20.03	6.68	27.3	30055.500	8.3993	-256430.00	257443.00	1.016		
T15	60 - 50	PPE 8 STD (SCH40)	10.02	5.16	21.1	30878.199	8.3993	-268742.00	259353.00	1.036		
T16	50 - 40	PPE 8 STD (SCH40)	10.02	5.16	21.1	30882.900	8.3993	-285244.00	259393.00	1.100		
T17	40 - 20	PIPE 8 X-STR (SCH80)	20.03	10.02	41.7	27820.100	12.7706	-317855.00	353279.00	0.895		
T18	20 - 0	PIPE 8 X-STR (SCH80)	20.03	9.97	41.6	27840.500	12.7706	-357208.00	355654.00	1.004		

Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L		K/U	F _a psi	A in ²	Actual		Allow.		Ratio P _a /P _a
			β	fr				P	lb	P _a	lb	
T2	180 - 175	L1 1/2x1 1/2x3/16	6.25	3.13	128.2	9084.010	0.5273	-3333.83	4790.39	0.696		
T3	175 - 170	L2x2x3/16	6.56	3.29	100.2	12953.100	0.7150	-2562.46	9261.49	0.277		
T4	170 - 160	2L1 1/2x1 1/2x3/16x1/4	7.25	3.60	108.7	11846.800	1.0547	-4913.61	12494.70	0.393		
T5	160 - 150	2L2-2x3/16x1/4	8.01	3.95	96.0	13482.500	1.4297	-5637.04	19275.70	0.292		
T6	150 - 140	2L2-2x3/16x1/4	8.40	4.13	97.9	13240.600	1.4297	-6289.46	18930.00	0.332		

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Client T-Mobile Towers		Designed by Christopher Bean			

Section No.	Elevation ft	Size	L		K/U	F _a psi	A in ²	Actual		Allow.		Ratio P _a /P _a
			β	fr				P	lb	P _a	lb	
T7	140 - 133.333	L1 1/2x1 1/2x1/4	10.29	4.87	119.2	10400.300	1.1900	-6542.78	12376.30	0.529		
T8	133.333 - 126.667	L2 1/2x2 1/2x1/4	10.80	5.26	128.6	9025.160	1.1900	-6697.06	10739.90	0.624		
T9	126.667 - 120	L2 1/2x2 1/2x3/16	11.34	5.53	134.1	8300.260	0.9020	-6489.40	7486.84	0.867		
T10	120 - 113.333	L3x3x3/16	11.88	5.81	116.9	10622.200	1.0900	-7262.85	11578.20	0.627		
T11	113.333 - 106.667	L3x3x3/16	12.44	6.09	120.2	10194.600	1.0900	-7119.97	11112.10	0.641		
T12	106.667 - 100	L2 1/2x2 1/2x1/4	13.01	6.24	152.5	6419.940	1.1900	-7580.18	7639.72	0.992		
T13	100 - 80	L3 1/2x3 1/2x1/4	14.76	7.21	106.0	12207.200	1.6900	-7473.24	20630.20	0.362		
T14	80 - 60	L3 1/2x3 1/2x1/4	16.57	8.07	118.7	10473.400	1.6900	-8019.70	17700.10	0.453		
T15	60 - 50	L3x3x5/16	18.87	9.11	170.0	5167.060	1.7800	-9971.03	9197.36	1.084		
T16	50 - 40	L3x3x5/16	19.73	9.54	176.7	4784.310	1.7800	-9984.21	8516.08	1.172		
T17	40 - 20	L4x4x3/8	21.47	10.41	149.5	6685.630	2.8600	-9882.33	19120.90	0.517		
T18	20 - 0	L5x5x5/16	23.24	11.30	132.5	8501.120	3.0300	-11136.30	25758.40	0.432		

Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L		K/U	F _a psi	A in ²	Actual		Allow.		Ratio P _a /P _a
			β	fr				P	lb	P _a	lb	
T1	195 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	124.9	9564.350	0.5273	-4942.77	5043.70	0.980		

Secondary Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L		K/U	F _a psi	A in ²	Actual		Allow.		Ratio P _a /P _a
			β	fr				P	lb	P _a	lb	
T2	180 - 175	L2x2x3/16	3.73	3.49	53.2	18064.801	0.7150	-572.18	12916.40	0.044		
T3	175 - 170	L2x2x3/16	4.24	4.00	60.8	17448.500	0.7150	-710.74	12404.20	0.057		
T4	170 - 160	L2x2x3/16	5.24	4.95	75.3	15867.200	0.7150	-1079.97	11345.10	0.095		
T5	160 - 150	L2x2x3/16	6.24	5.91	89.9	14210.700	0.7150	-1457.96	10160.70	0.143		
T6	150 - 140	L2x2x3/16	7.24	6.87	104.6	12391.800	0.7150	-1884.28	8860.11	0.213		
T10	120 - 113.333	L3x3x3/16	9.82	9.27	93.3	13524.800	1.0900	-2803.36	14742.00	0.190		
T11	113.333 - 106.667	L3x3x3/16	10.49	9.94	100.0	12741.900	1.0900	-3024.34	13888.60	0.218		

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Section No.	Elevation	Size	L	L _w	K/U _r	F _s	A	Actual	Allow.	Ratio
	f		f	f	f	psi	in ²	P	P _a	P _s
T12	106.667 - 100	L3x3x1/16	11.16	10.60	179.6	4627.600	1.0900	-3229.85	5044.08	0.640
					K=0.84					
T13	100 - 80	L3x3x1/4	13.16	12.52	126.9	9270.190	1.4400	-3857.70	13349.10	0.289
					K=0.50					
T15	60 - 50	L4x4x1/4	15.98	15.27	191.4	4077.370	1.9400	-4660.55	7910.10	0.589
					K=0.83					
T16	50 - 40	L4x4x1/4	16.99	16.27	201.7	3669.730	1.9400	-4946.73	7119.28	0.695
					K=0.82					

Top Girt Design Data (Compression)

Section No.	Elevation	Size	L	L _w	K/U _r	F _s	A	Actual	Allow.	Ratio
	f		f	f	f	psi	in ²	P	P _a	P _s
T1	195 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	124.9	9564.350	0.5273	-874.33	5043.70	0.173
					K=0.94					

Bottom Girt Design Data (Compression)

Section No.	Elevation	Size	L	L _w	K/U _r	F _s	A	Actual	Allow.	Ratio
	f		f	f	f	psi	in ²	P	P _a	P _s
T1	195 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	124.9	9564.350	0.5273	-2434.08	5043.70	0.483
					K=0.94					

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation	Size	L	L _w	K/U _r	F _s	A	Actual	Allow.	Ratio
	f		f	f	f	psi	in ²	P	P _a	P _s
T1	195 - 180	PIPE 3.5 STD (SCH140)	15.00	3.75	47.4	33000.000	1.7072	17619.50	56338.00	0.311
T2	180 - 175	PIPE 2.5 STD (SCH 40)	5.01	2.34	29.5	33000.000	1.7072	30268.40	56338.00	0.531
T3	175 - 170	PIPE 2.5 STD (SCH 40)	5.01	2.36	29.8	33000.000	1.7072	36944.80	56338.00	0.656
T4	170 - 160	2.5SCH140 w/ 3SC180 Half	10.02	2.38	31.1	21000.000	3.2120	55410.10	67452.50	0.821
T5	160 - 150	PIPE 3.5 Std (SCH140)	10.02	2.40	21.6	33000.000	2.6795	74960.30	88424.80	0.848
T6	150 - 140	3.5SCH140 w/ 4SC140 Half	10.02	2.42	22.2	30000.000	4.2666	96925.70	121798.00	0.757
T7	140 - 133.333	5 STD w/ 6 XH Half Sleeve	6.68	6.68	45.4	27600.000	8.5023	109104.00	234663.00	0.465
T8	133.333 - 126.667	5 STD w/ 6 XH Half Sleeve	6.68	6.68	45.4	27600.000	8.5023	122027.00	234663.00	0.520
T9	126.667/100	5 STD w/ 6 XH Half Sleeve	6.68	6.68	45.4	27600.000	8.5023	134255.00	234663.00	0.572
T10	120 - 113.333	PIPE 6 STD (SCH140)	6.68	3.23	17.1	33000.000	5.5858	144900.00	184320.00	0.787
T11	113.333 -	PIPE 6 STD (SCH140)	6.68	3.23	17.1	33000.000	5.5858	156298.00	184320.00	0.848

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Section No.	Elevation	Size	L	L _w	K/U _r	F _s	A	Actual	Allow.	Ratio
	f		f	f	f	psi	in ²	P	P _a	P _s
T12	106.667	PIPE 6 STD (SCH140)	6.68	3.24	17.3	33000.000	5.5858	166839.00	184320.00	0.905
		6 STD w/ 7 XH Half Sleeve								
T13	100 - 80	PIPE 8 STD (SCH140)	20.03	3.25	18.6	25200.000	11.1773	197546.00	281668.00	0.701
T14	80 - 60	PIPE 8 STD (SCH140)	20.03	6.68	27.3	33000.000	8.3993	226401.00	271175.00	0.817
T15	60 - 50	PIPE 8 STD (SCH140)	10.02	4.85	19.8	33000.000	8.3993	236946.00	271175.00	0.855
T16	50 - 40	PIPE 8 X-STR (SCH140)	10.02	4.86	19.9	33000.000	8.3993	250854.00	271175.00	0.905
T17	40 - 20	PIPE 8 X-STR (SCH140)	20.03	10.02	41.7	33000.000	12.7706	277325.00	421429.00	0.658
T18	20 - 0	PIPE 8 X-STR (SCH140)	20.03	0.08	0.3	33000.000	12.7706	308824.00	421429.00	0.733

Diagonal Design Data (Tension)

Section No.	Elevation	Size	L	L _w	K/U _r	F _s	A	Actual	Allow.	Ratio
	f		f	f	f	psi	in ²	P	P _a	P _s
T1	195 - 180	5/8	5.13	4.78	366.9	21600.000	0.3068	7269.94	6626.80	1.097
T2	180 - 175	L1 1/2x1 1/2x3/16	6.25	3.13	82.4	29000.000	0.3076	2823.53	8920.90	0.317
T3	175 - 170	L2x3x3/16	6.56	3.29	64.0	29000.000	0.4484	2744.96	13002.40	0.211
T4	170 - 160	2 1/2x3x1/16x1/4	7.25	3.60	94.5	29000.000	0.6152	4988.61	17841.80	0.280
T5	160 - 150	2 1/2x3x1/16x1/4	8.01	3.95	76.8	29000.000	0.8965	5344.07	25998.00	0.206
T6	150 - 140	L2 1/2x2 1/2x1/4	8.40	4.13	80.3	29000.000	0.8965	6104.61	25998.00	0.235
T7	140 - 133.333	L2 1/2x2 1/2x1/4	10.29	4.87	78.0	29000.000	0.7519	6440.41	21804.40	0.295
T8	133.333 - 126.667	L2 1/2x2 1/2x1/4	10.80	5.26	82.1	29000.000	0.7519	6576.58	21804.40	0.302
T9	126.667 - 120	L2 1/2x2 1/2x3/16	11.34	5.53	85.3	29000.000	0.5710	6415.84	16559.90	0.387
T10	120 - 113.333	L3x3x3/16	11.88	5.81	74.2	29000.000	0.7120	6649.98	20648.90	0.322
T11	113.333 - 106.667	L3x3x3/16	12.44	6.09	77.8	29000.000	0.7120	6618.01	20648.90	0.321
T12	106.667 - 100	L2 1/2x2 1/2x1/4	13.01	6.24	79.5	29000.000	0.7519	6889.92	21804.40	0.316
T13	100 - 80	L3 1/2x3 1/2x1/4	14.76	7.21	99.4	29000.000	1.1269	7388.15	32679.40	0.223
T14	80 - 60	L3 1/2x3 1/2x1/4	16.57	8.07	88.9	29000.000	1.1269	7871.24	32679.40	0.241
T15	60 - 50	L3x3x5/16	18.87	9.11	121.1	29000.000	1.1592	9047.86	33617.30	0.269
T16	50 - 40	L3x3x5/16	19.73	9.54	126.7	29000.000	1.1592	8986.85	33617.30	0.267
T17	40 - 20	L4x4x3/8	21.47	10.41	103.5	29000.000	1.9341	9590.02	56087.80	0.171
T18	20 - 0	L5x5x5/16	23.24	11.30	87.9	29000.000	2.0967	10206.30	60804.80	0.168

Horizontal Design Data (Tension)

Section No.	Elevation	Size	L	L _w	K/U _r	F _s	A	Actual	Allow.	Ratio
	f		f	f	f	psi	in ²	P	P _a	P _s
T1	195 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	21600.000	0.5273	514.75	11390.60	0.045

Secondary Horizontal Design Data (Tension)

Section No.	Elevation	Size	L	L _w	K/U _r	F _s	A	Actual	Allow.	Ratio
	f		f	f	f	psi	in ²	P	P _a	P _s
T2	180 - 175	L2x3x3/16	3.73	3.49	67.9	29000.000	0.4308	572.18	12492.70	0.046

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Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P _{allow} lb	% Capacity	Pass/Fail
T15	60 - 50	Leg	PIPE 8 STD (SCH40)	246	-368742.00	345717.53	77.7	Pass
T16	50 - 40	Leg	PIPE 8 STD (SCH40)	258	-285244.00	345770.85	82.5	Pass
T17	40 - 20	Leg	PIPE 8 X-STR (SCH80)	270	-317855.00	473586.89	67.1	Pass
T18	20 - 0	Leg	PIPE 8 X-STR (SCH80)	285	-357208.00	474086.76	75.3	Pass
T1	195 - 180	Diagonal	5/8 L1 1/2x1 1/2x3/16	12	7269.94	8833.52	82.3	Pass
T2	180 - 175	Diagonal	L1 1/2x1 1/2x3/16	48	-3333.83	6385.59	52.2	Pass
T3	175 - 170	Diagonal	L2x2x3/16	60	-2562.46	12345.57	20.8	Pass
T4	170 - 160	Diagonal	2L1 1/2x1 1/2x3/16x1/4	75	-4913.61	16655.43	29.5	Pass
T5	160 - 150	Diagonal	2L2x2x3/16x1/4	96	-5637.04	25694.51	21.9	Pass
T6	150 - 140	Diagonal	2L2x2x3/16x1/4	123	-6289.46	25233.69	24.9	Pass
T7	140 - 133.333	Diagonal	L2 1/2x2 1/2x1/4	135	-6542.78	16497.61	39.7	Pass
T8	133.333 - 126.667	Diagonal	L2 1/2x2 1/2x1/4	144	-6697.06	14316.29	46.8	Pass
T9	126.667 - 120	Diagonal	L2 1/2x2 1/2x3/16	153	-6489.40	9979.96	65.0	Pass
T10	120 - 113.333	Diagonal	L3x3x3/16	162	-7262.85	15433.74	47.1	Pass
T11	113.333 - 106.667	Diagonal	L3x3x3/16	174	-7119.97	14812.43	48.1	Pass
T12	106.667 - 100	Diagonal	L2 1/2x2 1/2x1/4	186	-7380.18	10183.75	74.4	Pass
T13	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	198	-7473.24	27500.05	27.2	Pass
T14	80 - 60	Diagonal	L3 1/2x3 1/2x1/4	228	-8019.70	23594.23	34.0	Pass
T15	60 - 50	Diagonal	L3x3x5/16	249	-9871.03	12260.08	81.3	Pass
T16	50 - 40	Diagonal	L3x3x5/16	261	-9984.21	11351.93	88.0	Pass
T17	40 - 20	Diagonal	L4x4x3/8	273	-9882.33	25488.16	38.8	Pass
T18	20 - 0	Diagonal	L5x5x5/16	288	-11136.30	34335.95	32.4	Pass
T1	195 - 180	Horizontal	L1 1/2x1 1/2x3/16	17	-4942.77	6723.25	73.5	Pass
T2	180 - 175	Secondary Horizontal	L2x2x3/16	53	572.18	16652.77	9.4	Pass
T3	175 - 170	Secondary Horizontal	L2x2x3/16	65	-710.74	16534.80	4.3	Pass
T4	170 - 160	Secondary Horizontal	L2x2x3/16	77	-1079.97	15123.02	7.1	Pass
T5	160 - 150	Secondary Horizontal	L2x2x3/16	98	-1457.96	13544.21	10.8	Pass
T6	150 - 140	Secondary Horizontal	L2x2x3/16	119	-1884.28	11810.53	16.0	Pass
T10	120 - 113.333	Secondary Horizontal	L3x3x3/16	167	-2803.36	19651.09	14.3	Pass
T11	113.333 - 106.667	Secondary Horizontal	L3x3x3/16	179	-3024.34	18513.50	16.3	Pass
T12	106.667 - 100	Secondary Horizontal	L3x3x3/16	191	-3229.85	6723.76	48.0	Pass
T13	100 - 80	Secondary Horizontal	L3x3x1/4	203	-3857.70	17794.35	21.7	Pass
T15	60 - 50	Secondary Horizontal	L4x4x1/4	254	-4660.55	10544.16	44.2	Pass
T16	50 - 40	Secondary Horizontal	L4x4x1/4	266	-4946.73	9490.00	52.1	Pass
T1	195 - 180	Top Girt	L1 1/2x1 1/2x3/16	5	-874.33	6723.25	13.0	Pass
T1	195 - 180	Bottom Girt	L1 1/2x1 1/2x3/16	8	-2434.08	6723.25	36.2	Pass
Summary								Pass
Leg (T18)								85.6

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Section No.	Elevation ft	Component Type	Size	Kt/R	F _x	A	Actual P lb	Allow. P _a lb	Ratio P _a /P	Pass/Fail	
T3	175 - 170	Leg	L2x2x3/16	4.24	4.00	77.7	29000.000	44308	110.74	12492.70	0.057
T4	170 - 160	Leg	L2x2x3/16	5.24	4.95	96.2	29000.000	0.4308	1079.97	12492.70	0.086
T5	160 - 150	Leg	L2x2x3/16	6.24	5.91	114.9	29000.000	0.4308	1457.96	12492.70	0.117
T6	150 - 140	Leg	L2x2x3/16	7.24	6.87	133.5	29000.000	0.4308	1884.28	12492.70	0.151
T10	120 - 113.333	Leg	L3x3x3/16	9.82	9.27	118.5	29000.000	0.7120	2803.36	20648.90	0.136
T11	113.333 - 106.667	Leg	L3x3x3/16	10.49	9.94	127.0	29000.000	0.6945	3024.34	20139.10	0.150
T12	106.667 - 100	Leg	L3x3x3/16	11.16	10.60	135.5	21600.000	1.0900	3229.85	23544.00	0.137
T13	100 - 80	Leg	L3x3x1/4	13.16	12.52	161.6	29000.000	0.9394	3857.70	27241.90	0.142
T15	60 - 50	Leg	L4x4x1/4	15.98	15.27	146.6	21600.000	1.9400	4660.55	41904.00	0.111
T16	50 - 40	Leg	L4x4x1/4	16.99	16.27	156.2	21600.000	1.9400	4946.73	41904.00	0.118

Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L _s ft	Kt/R	F _x	A	Actual P lb	Allow. P _a lb	Ratio P _a /P	
T1	195 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	21600.000	0.5273	413.84	11390.60	0.036

Bottom Girt Design Data (Tension)

Section No.	Elevation ft	Size	L _s ft	Kt/R	F _x	A	Actual P lb	Allow. P _a lb	Ratio P _a /P	
T1	195 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	21600.000	0.5273	38.84	11390.60	0.003

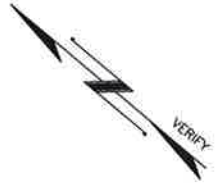
Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P _{allow} lb	% Capacity	Pass/Fail
T1	195 - 180	Leg	PIPE 2.5 STD (SCH 40)	3	-29719.30	61065.26	48.7	Pass
T2	180 - 175	Leg	PIPE 2.5 STD (SCH 40)	45	-32993.60	60322.27	49.8	Pass
T3	175 - 170	Leg	PIPE 2.5 STD (SCH 40)	57	-40983.70	66319.28	61.8	Pass
T4	170 - 160	Leg	2.5SCH40 w/ 3SCH80 Half Sleeve	69	-62274.10	81881.79	76.1	Pass
T5	160 - 150	Leg	PIPE 3.5 STD (SCH40)	90	-84070.20	109240.81	77.0	Pass
T6	150 - 140	Leg	3.5SCH40 w/ 4SCH40 Half Sleeve	111	-108653.00	158595.00	68.5	Pass
T7	140 - 133.333	Leg	5 STD w/ 6 XH Half Sleeve	132	-122005.00	264071.29	46.2	Pass
T8	133.333 - 126.667	Leg	5 STD w/ 6 XH Half Sleeve	141	-136220.00	264071.29	51.6	Pass
T9	126.667 - 120	Leg	5 STD w/ 6 XH Half Sleeve	150	-149868.00	264071.29	56.8	Pass
T10	120 - 113.333	Leg	PIPE 6 STD (SCH40)	159	-161650.00	232566.55	69.6	Pass
T11	113.333 - 106.667	Leg	PIPE 6 STD (SCH40)	171	-174393.00	232391.21	75.0	Pass
T12	106.667 - 100	Leg	PIPE 6 STD (SCH40)	183	-186243.00	232421.87	80.1	Pass
T13	100 - 80	Leg	6 STD w/ 7 XH Half Sleeve	195	-222447.00	356881.41	62.3	Pass
T14	80 - 60	Leg	PIPE 8 STD (SCH40)	225	-256430.00	336506.51	76.2	Pass

mxTower		Job	Middlebury I-84 (CT11128E)	Page	29 of 29
Tower Engineering Professionals, Inc. 326 Iron Road Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350		Project	TEP No. 25628_22635, Revision 16	Date	14:24:52 04/28/15
		Client	T-Mobile Towers	Designed by	Christopher Bean

Section No.	Elevation f	Component Type	Size	Critical Element	P lb	SF*W _{allow} lb	% Capacity	Pass Fail
		Diagonal (T10)					97.9	Pass
		Horizontal (T1)					73.5	Pass
		Secondary Horizontal (T16)					52.1	Pass
		Top Girt (T1)					13.0	Pass
		Bottom Girt (T1)					36.2	Pass
		Boil Checks					97.9	Pass
		BAITING =					97.9	Pass

APPENDIX B
COAX CONFIGURATION



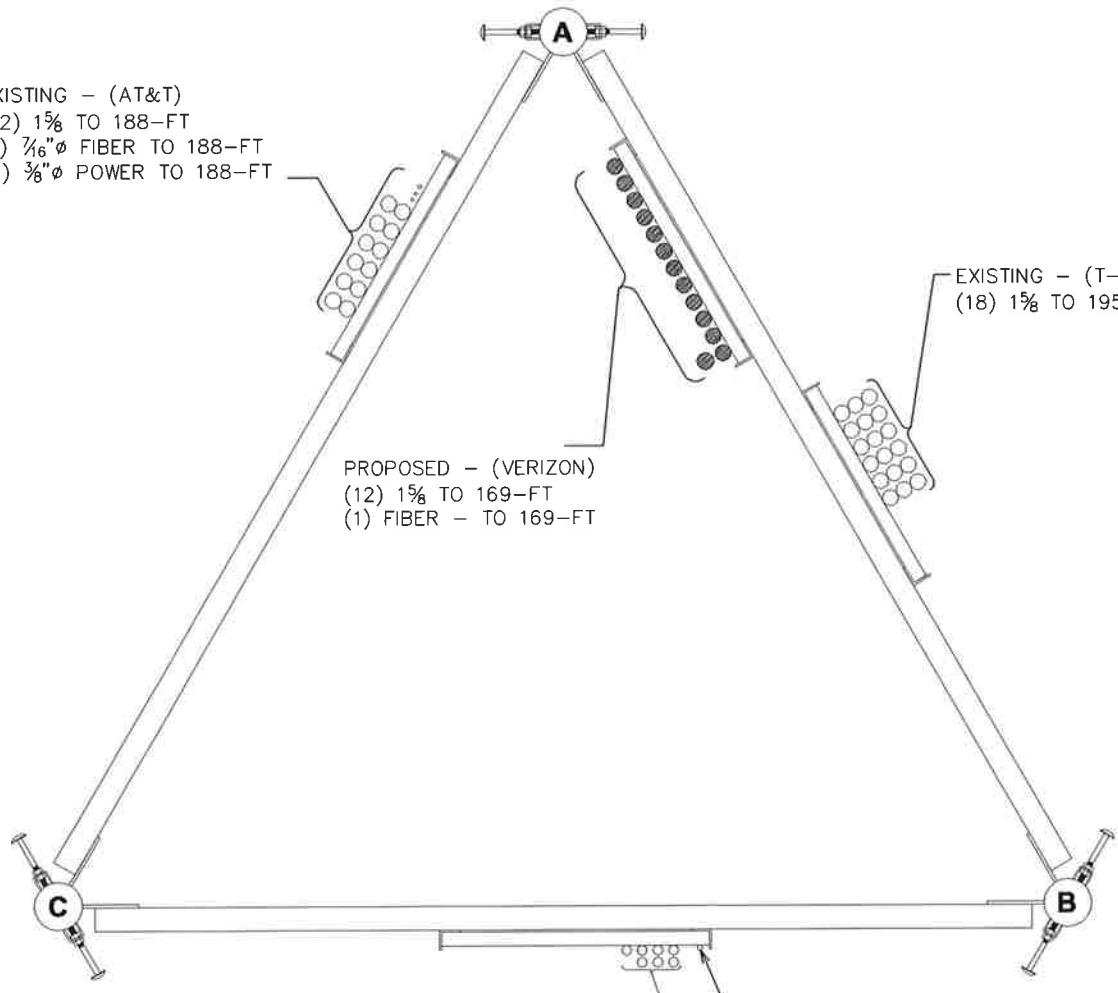
EXISTING - (AT&T)
(12) 1 5/8" TO 188-FT
(1) 7/16" Ø FIBER TO 188-FT
(2) 3/8" Ø POWER TO 188-FT

EXISTING - (T-MOBILE)
(18) 1 5/8" TO 195-FT

PROPOSED - (VERIZON)
(12) 1 5/8" TO 169-FT
(1) FIBER - TO 169-FT

EXISTING - (UNKNOWN)
(1) 5/8" Ø TO 75.5-FT

EXISTING - (SPRINT)
(7) 1 1/4" Ø FIBER TO 152-FT



COAX CONFIGURATION

SCALE: N.T.S.

PREPARED BY:
TOWER ENGINEERING PROFESSIONALS
326 TRYON ROAD
RALEIGH, NC 27603-3530
(919) 661-6351

PREPARED FOR:
T-Mobile
TOWERS
12920 SE 38TH STREET
BELLEVUE, WA 98005

PROJECT INFORMATION:
MIDDLEBURY I-84
SITE#: CT11128E
1021 STRAITS TURNPIKE
MIDDLEBURY, CT 06762
(NEW HAVEN COUNTY)

REVISION: 1G
TEP #: 25628_22635
SHEET NUMBER:
S-1

APPENDIX C
ADDITIONAL CALCULATIONS

Project Name: Middlebury I-84
 Project Number: TEP#25628 22635, Revision 16
 Client Site Number: CT11128E
 Elevation: 80 - 100ft

Engineer: CJB
 Check: RHL
 Date: 4/28/2015
 CODE: TIA-F

Grouted/Un-Grouted Pipe Leg + Half Sleeve R/F

ASIF: 1.33 - allowable stress increase factor (typically 1.33)
 Mast St.: 1.00 - from trxtower

Input - Loads

P_{initial}: 10.226 kips - force from initial load (no wind)
 P_{wind}: 222.447 kips - force due to final loading including reinforcement
 T_u: 197.546 kips - maximum load on leg

Input - Tower Leg 6 STD

K: 1.00 - effective length factor for leg
 L_u: 3.43 ft - unbraced length of tower leg
 F_{y,leg}: 55.00 ksi - minimum specified yield strength of tower leg
 F_{u,leg}: 70.00 ksi - minimum specified ultimate strength of tower leg
 r: 2.25 in - minimum radius of gyration of tower leg
 A_{leg}: 5.58 in² - area of tower leg
 D_i: 6.07 in - inside diameter of tower leg
 t_{leg}: 0.28 in - thickness of tower leg
 f_c: 0.00 ksi - minimum specified compressive strength of grout (if ungrouted enter 0)

Quick Check

Weld Size: OK
 Weld Connection: 47.7%
 Crushing Check: 47.3%
 Leg Comp. Check: 47.9%
 Sleeve Check: 57.9%
 Built-up Check: 54.8%
 Slenderness Check: OK
 Leg Tension Check: 45.6%

Input - Sleeve R/F 7 XH Gap Check: OK

F_{y,sleeve}: 42.00 ksi - minimum specified yield strength of sleeve r/f
 F_{u,sleeve}: 60.00 ksi - minimum specified ultimate strength of sleeve r/f
 r_{x,sleeve}: 1.10 in - minimum radius of gyration of sleeve r/f about the x-axis
 r_{y,sleeve}: 2.53 in - minimum radius of gyration of sleeve r/f about the y-axis
 A_{sleeve}: 5.60 in² - area of sleeve r/f
 t_{sleeve}: 0.50 in - thickness of tower leg

Termination: Connected to Flange

Input - Sleeve Connection to Leg

a: 12.00 in - spacing of connectors connecting the sleeve to the leg
 D: 5.00 - weld size for the weld connecting the sleeve to the leg (unit = # of 16ths)
 Length //: 12.00 in - length of weld on each side of the leg at the termination
 Length ⊥: 11.98 in - length of weld at the bottom/top of the leg sleeve at termination (πD/2)
 No: 2.00 - number of longitudinal welds per end of the leg (typically near side & far side, so 2)
 F_{EXX}: 70.00 ksi - weld electrode classification
 Width: 7.63 in - maximum width of the built-up leg
 Gap: 0.00 in - length of leg considered for crushing

Input - Built-up Leg Section 6 STD w/7 XH Half Sleeve

r_{x,bu}: 2.10 in - minimum radius of gyration of the built-up section about the x-axis
 r_{y,bu}: 2.39 in - minimum radius of gyration of the built-up section about the y-axis

Project Name: Middlebury I-84
 Project Number: TEP#25628_22635, Revision 1G
 Client Site Number: CT11128E
 Elevation: 120 - 140ft

Engineer: CJB
 Check: RHL
 Date: 4/28/2015
 CODE: TIA-F

Grouted/Un-Grouted Pipe Leg + Half Sleeve R/F

ASIF: 1.33 - allowable stress increase factor (typically 1.33)
 Mast St.: 1.00 - from trnTower

Input - Loads

$P_{initial}$: 6.95659 kips - force from initial load (no wind)
 P_{wind} : 149.868 kips - force due to final loading including reinforcement
 T_U : 134.255 kips - maximum load on leg

Input - Tower Leg 5 STD

K : 1.00 - effective length factor for leg
 L_U : 6.68 ft - unbraced length of tower leg
 $F_{y_{leg}}$: 55.00 ksi - minimum specified yield strength of tower leg
 $F_{u_{leg}}$: 70.00 ksi - minimum specified ultimate strength of tower leg
 r : 1.88 in - minimum radius of gyration of tower leg
 A_{leg} : 4.30 in² - area of tower leg
 D_I : 5.05 in - inside diameter of tower leg
 t_{leg} : 0.26 in - thickness of tower leg
 f'_c : 0.00 ksi - minimum specified compressive strength of grout (if ungrouted enter 0)

Quick Check

Weld Size: OK
 Weld Connection: 33.2%
 Crushing Check: 41.9%
 Leg Comp. Check: 42.8%
 Sleeve Check: 47.7%
 Built-up Check: 53.2%
 Slenderness Check: OK
 Leg Tension Check: 39.0%

Input - Sleeve R/F 6 XH Gap Check: OK

$F_{y_{sleeve}}$: 46.00 ksi - minimum specified yield strength of sleeve r/f
 $F_{u_{sleeve}}$: 62.00 ksi - minimum specified ultimate strength of sleeve r/f
 $r_{x_{sleeve}}$: 0.96 in - minimum radius of gyration of sleeve r/f about the x-axis
 $r_{y_{sleeve}}$: 2.19 in - minimum radius of gyration of sleeve r/f about the y-axis
 A_{sleeve} : 4.20 in² - area of sleeve r/f
 t_{sleeve} : 0.43 in - thickness of tower leg

Termination: Connected to Flange

Input - Sleeve Connection to Leg

a : 15.50 in - spacing of connectors connecting the sleeve to the leg *MI Discrepancy, approved by EOR
 D : 5.00 - weld size for the weld connecting the sleeve to the leg (unit = # of 16ths)
 Length //: 12.00 in - length of weld on each side of the leg at the termination
 Length ⊥: 10.41 in - length of weld at the bottom/top of the leg sleeve at termination ($\pi D/2$)
 N_o : 2.00 - number of longitudinal welds per end of the leg (typically near side & far side, so 2)
 F_{EXX} : 70.00 ksi - weld electrode classification
 Width: 6.63 in - maximum width of the built-up leg
 Gap: 0.00 in - length of leg considered for crushing

Input - Built-up Leg Section 5 STD w/6 XH Half Sleeve

$r_{x_{bu}}$: 1.77 in - minimum radius of gyration of the built-up section about the x-axis
 $r_{y_{bu}}$: 2.04 in - minimum radius of gyration of the built-up section about the y-axis

Project Name: Middlebury I-84
 Project Number: TEP#25628 22635, Revision 1 G
 Client Site Number: CT11128E
 Elevation: 140 - 150ft

Engineer: CJB
 Check: RHL
 Date: 4/28/2015
 CODE: TIA-F

Grouted/Un-Grouted Pipe Leg + Half Sleeve R/F

ASIF: 1.33 - allowable stress increase factor (typically 1.33)
 Mast St.: 1.00 - from tnxTower

Input - Loads

P_{initial}: 5.478 kips - force from initial load (no wind)
 P_{wind}: 108.653 kips - force due to final loading including reinforcement
 T_u: 96.923 kips - maximum load on leg

Input - Tower Leg 3.5 STD

K: 1.00 - effective length factor for leg
 L_u: 2.60 ft - unbraced length of tower leg
 F_{y,leg}: 55.00 ksi - minimum specified yield strength of tower leg
 F_{u,leg}: 70.00 ksi - minimum specified ultimate strength of tower leg
 r: 1.34 in - minimum radius of gyration of tower leg
 A_{leg}: 2.68 in² - area of tower leg
 D_i: 3.55 in - inside diameter of tower leg
 t_{leg}: 0.23 in - thickness of tower leg
 f_c: 0.00 ksi - minimum specified compressive strength of grout (If ungrouted enter 0)

Quick Check

Weld Size: OK
 Weld Connection: 33.3%
 Crushing Check: 59.6%
 Leg Comp. Check: 61.0%
 Sleeve Check: 63.7%
 Built-up Check: 66.5%
 Slenderness Check: OK
 Leg Tension Check: 53.4%

Input - Sleeve R/F 4 STD Gap Check: OK

F_{y,sleeve}: 50.00 ksi - minimum specified yield strength of sleeve r/f
 F_{u,sleeve}: 65.00 ksi - minimum specified ultimate strength of sleeve r/f
 r_{x,sleeve}: 0.66 in - minimum radius of gyration of sleeve r/f about the x-axis
 r_{y,sleeve}: 1.51 in - minimum radius of gyration of sleeve r/f about the y-axis
 A_{sleeve}: 1.59 in² - area of sleeve r/f
 t_{sleeve}: 0.24 in - thickness of tower leg

Termination: Connected to Flange

Input - Sleeve Connection to Leg

a: 12.00 in - spacing of connectors connecting the sleeve to the leg
 D: 3.00 - weld size for the weld connecting the sleeve to the leg (unit = # of 16ths)
 Length //: 12.00 in - length of weld on each side of the leg at the termination
 Length ⊥: 7.07 in - length of weld at the bottom/top of the leg sleeve at termination (πD/2)
 No: 2.00 - number of longitudinal welds per end of the leg (typically near side & far side, so 2)
 F_{EXX}: 70.00 ksi - weld electrode classification
 Width: 4.50 in - maximum width of the built-up leg
 Gap: 0.00 in - length of leg considered for crushing

Input - Built-up Leg Section 3.5 STD w/4 STD Half Sleeve

r_{x,bu}: 1.31 in - minimum radius of gyration of the built-up section about the x-axis
 r_{y,bu}: 1.40 in - minimum radius of gyration of the built-up section about the y-axis

Project Name: Middlebury I-84
 Project Number: TEP#25628_22635, Revision 16
 Client Site Number: CT11128E
 Elevation: 160 - 170ft

Engineer: CJB
 Check: RHL
 Date: 4/28/2015
 CODE: TIA-F

Grouted/Un-Grouted Pipe Leg + Half Sleeve R/F

ASIF: 1.33 - allowable stress increase factor (typically 1.33)
 Mast St.: 1.00 - from tnxTower

Input - Loads

P_{initial}: 3.40447 kips - force from initial load (no wind)
 P_{wind}: 62.274 kips - force due to final loading including reinforcement
 T_v: 55.41 kips - maximum load on leg

Input - Tower Leg 2.5 STD

K: 1.00 - effective length factor for leg
 L_v: 2.64 ft - unbraced length of tower leg
 F_{y,leg}: 55.00 ksi - minimum specified yield strength of tower leg
 F_{u,leg}: 70.00 ksi - minimum specified ultimate strength of tower leg
 r: 0.95 in - minimum radius of gyration of tower leg
 A_{leg}: 1.70 in² - area of tower leg
 D_i: 2.47 in - inside diameter of tower leg
 t_{leg}: 0.20 in - thickness of tower leg
 f_c: 0.00 ksi - minimum specified compressive strength of grout (if ungrouted enter 0)

Quick Check

Weld Size: OK
 Weld Connection: 25.2%
 Crushing Check: 46.2%
 Leg Comp. Check: 47.8%
 Sleeve Check: 69.4%
 Built-up Check: 61.8%
 Slenderness Check: OK
 Leg Tension Check: 47.3%

Input - Sleeve R/F 3 XS Gap Check: OK

F_{y,sleeve}: 35.00 ksi - minimum specified yield strength of sleeve r/f
 F_{u,sleeve}: 60.00 ksi - minimum specified ultimate strength of sleeve r/f
 r_{x,sleeve}: 0.50 in - minimum radius of gyration of sleeve r/f about the x-axis
 r_{y,sleeve}: 1.14 in - minimum radius of gyration of sleeve r/f about the y-axis
 A_{sleeve}: 1.51 in² - area of sleeve r/f
 t_{sleeve}: 0.30 in - thickness of tower leg

Termination: Connected to Flange

Input - Sleeve Connection to Leg

a: 12.00 in - spacing of connectors connecting the sleeve to the leg
 D: 3.00 - weld size for the weld connecting the sleeve to the leg (unit = # of 16ths)
 Length //: 12.00 in - length of weld on each side of the leg at the termination
 Length ⊥: 5.50 in - length of weld at the bottom/top of the leg sleeve at termination (πD/2)
 No: 2.00 - number of longitudinal welds per end of the leg (typically near side & far side, so 2)
 F_{EXX}: 70.00 ksi - weld electrode classification
 Width: 3.50 in - maximum width of the built-up leg
 Gap: 0.00 in - length of leg considered for crushing

Input - Built-up Leg Section 2.5 STD w/3 XS Half Sleeve

r_{x,bu}: 0.92 in - minimum radius of gyration of the built-up section about the x-axis
 r_{y,bu}: 1.04 in - minimum radius of gyration of the built-up section about the y-axis



JOB: TEP#25628_22635, Rev 16
 SHEET #: 1 OF 2
 CALCULATED BY: CJB DATE 4/28/2015
 CHECKED BY: RHL DATE 4/28/2015

Mat Foundation Design for Self Supporting Tower -TIA-222-F

Q_a , ALLOWABLE SOIL PRESS. (ksf)	6	F'_c (ksi)	3
NET OR GROSS BEARING?	NET	F'_y (ksi)	60
SOIL DENSITY (pcf)	125		
TOWER FACE WIDTH (ft.)	21.5		
Tower Eccentricity (ft)	0.00	Distance between tower centroid and the foundation centroid	

Base Reactions LC1: Maximum Wind

M_u , MOMENT (k-ft)	6314.6
P_t , AXIAL (k)	50.0
H , SHEAR (k)	54.9

Base Reactions LC 2: Ice + Ice Wind

M , MOMENT (k-ft)	2021.4
P_t , AXIAL (k)	100.3
H , SHEAR (k)	16.6

Try:

L (ft.)	B (ft.)	t (ft.)	Soil depth to TOP of mat (ft.)	Soil depth to BOT. of mat (ft.)	Pier dia./width (ft.)	Pier Height, h (ft.)	Pier Shape
33	33	4	5.416	9.416	4.00	5.67	Square

W_f , WEIGHT OF FOUNDATION (k) =	694.2	Concrete Volume (cu yd)	171.4
W_s , WEIGHT OF SOIL (k) =	704.8		

CHECK BEARING CAPACITY: LC1 LC2

P = P_t + W_f + W_s =	1449.0 k	1499.3 k
e = (M_{ot} + P_t*e_t)/P =	4.72 ft	1.51 ft
L/6 =	5.50 ft	5.50 ft
90 Axis: q_{max} =	1.30 ksf	0.58 ksf
Diag. Axis: q_{max} =	1.77 ksf	0.73 ksf

Capacity: 29.5%

CHECK OVERTURNING SF: LC1 LC2

M_{ot} = M + H*(t+h) =	6845.6 k-ft	2181.7 k-ft
M_{st} = P*(L/2 - e_t) + (W_{f+s}*L/2) =	23907.8 k-ft	24738.1 k-ft
SF = M_{ot}/M_{st} =	3.49 > 1.5	11.34 > 1.5

Capacity: 43.0%



JOB: TEP#25628_22635, Rev 16
SHEET #: 2 OF 2
CALCULATED BY: CJB DATE 4/28/2015
CHECKED BY: RHL DATE 4/28/2015

CHECK BEAM SHEAR

$V_u = 183.6 \text{ k}$
 $\phi V_c = 1415.3 \text{ k}$ $V_c > V_u$ **O.K.** **Capacity: 13.0%**

CHECK PUNCHING SHEAR

$V_u = 325.9 \text{ k}$
 $\phi V_c = 1404.0 \text{ k}$ $V_c > V_u$ **O.K.** **Capacity: 23.2%**

CALCULATE REINFORCING REQUIRED

$F'_c = 3.0 \text{ ksi}$ $F_y = 60.0 \text{ ksi}$

Temp & Shrinkage Reinforcement, $A_s, \text{temp} = 0.39 \text{ in}^2/\text{ft}$ (ACI 318 Sec. 10.5.4)

BOTTOM REINFORCING

Bar Size = 8
Bar Spacing = 11.8 in.
d = 43.5 in.

$\mu_u = -687.7 \text{ in-k/ft}$

$\phi M_n = 0.9 \cdot A_s \cdot F_y \cdot (d - 1/2 \cdot A_s \cdot F_y / (0.85 \cdot b \cdot F'_c))$

Solution: $A_{s, \text{req}} = 0.29 \text{ in}^2/\text{ft}$
Check, $A_s = 0.80 \text{ in}^2/\text{ft}$ **Capacity: 36.7%**

TOP REINFORCING

Bar Size = 8
Bar Spacing = 11.8 in.
d = 43.5 in.

$\mu_u = 642.5 \text{ in-k/ft}$

$\phi M_n = 0.9 \cdot A_s \cdot F_y \cdot d \cdot (1 - 0.59 \cdot A_s \cdot F_y / (b \cdot d \cdot F'_c))$

Solution: $A_{s, \text{req}} = 0.28 \text{ in}^2/\text{ft}$
Check, $A_s = 0.80 \text{ in}^2/\text{ft}$ **Capacity: 34.3%**



PASS PASS

Middlebury I-84 (CT11128E)

Results Summary: LC1 LC2

TEP #: 25628_22635, Rev 16

Soil Interaction: N/A N/A

Analysis: CJB 4/28/2015

Pier Check

Foundation Structural: 30.5% 61.8%

Check: RHL 4/28/2015

Code Revisions: TIA-222-F ACI 318-05

Tower Type: Self Support

	LC1	LC2	
Moment:	0.00	0.00	kip-ft
Axial (download):	355.80	0.00	kip
Shear:	34.95	30.60	kip
Axial (uplift):	0.00	307.56	kip

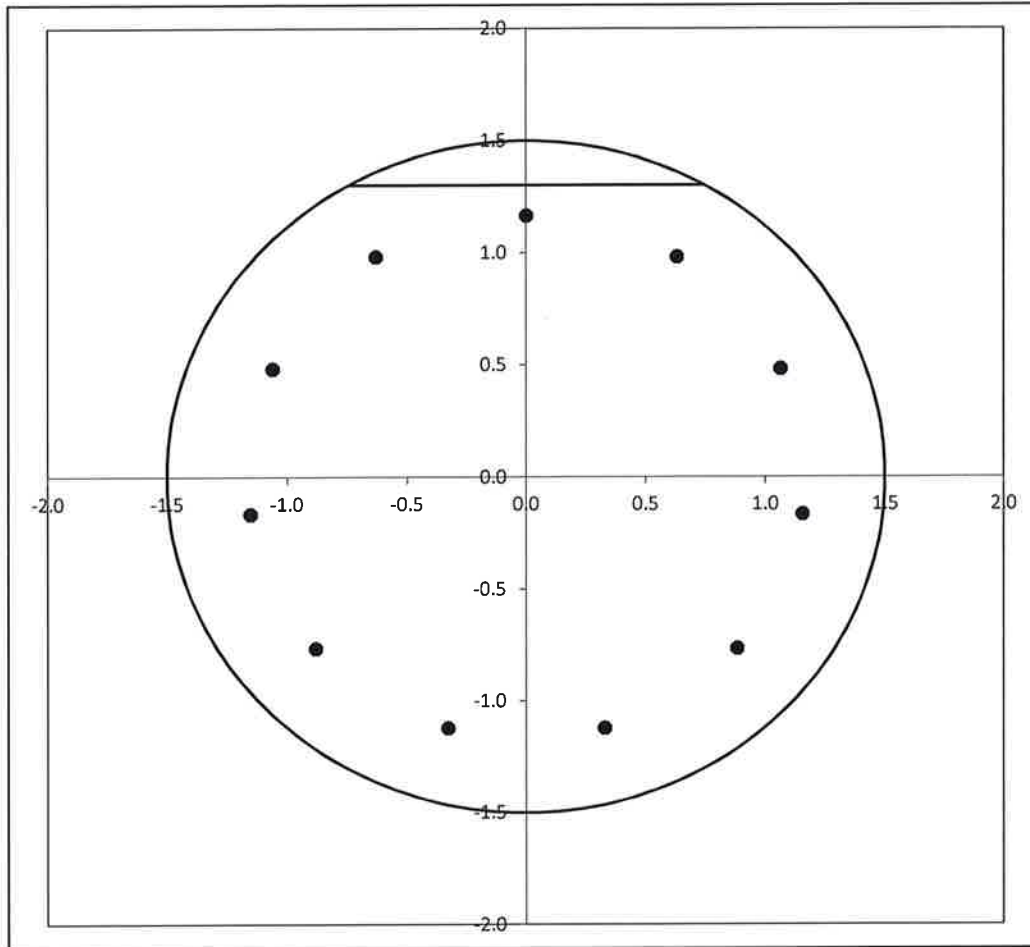
Shaft Information		
Diameter:	3.00	ft
Projection:	0.25	ft
Caisson Length:	5.67	ft
f'c:	4.000	ksi
Max ϵ_c :	0.003	in/in

Cage 1 Reinforcement

Tie Bar Size:	4	($f_y = 60.0$ ksi)
Clear Cover to Tie:	3.00	in (Cage $\varnothing = 28.00$ in)
Tie Bar Spacing:	16.00	in
Vertical Bar Size:	8	
Vertical Bar Quantity:	11	($\rho = 0.854\%$)
f _y :	60.0	ksi
E:	29,000	ksi



Reinforcement Capacity



	LC1	LC2
V_u =	45.4	45.4 kip
V_c =	151.3	50.9 kip
f_y, tie = 60.0 V_s =	47.1	47.1 kip
ϕV_n =	148.8	73.6 kip
Capacity =	30.5%	61.8%
	PASS	PASS

	LC1	LC2
M_u =	0.0	0.0 kip-ft
ϕM_n =	926.4	97.1 kip-ft
Capacity =	0.0%	0.0%
	PASS	PASS