# ROBINSON & COLELLP

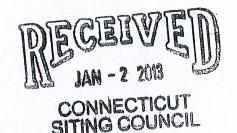
KENNETH C. BALDWIN

280 Trumbull Street Hartford, CT 06103-3597 Main (860) 275-8200 Fax (860) 275-8299 kbaldwin@rc.com Direct (860) 275-8345

Also admitted in Massachusetts

December 28, 2012

Linda Roberts
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051



Re:

EM-VER-082-120829 - 393 Jackson Hill Road, Middlefield, Connecticut

EM-VER-079-120807 – 175 South Main Street, Marlborough,

Connecticut

EM-VER-005-120217B - 127 New Hartford Road, Barkhamsted,

Connecticut

EM-VER-086-120216 – 41 Beckwith Road, Montville, Connecticut

The purpose of this letter is to notify the Siting Council that construction

If you have any questions or need any additional information regarding this

Sincerely,

Kenneth C. Baldwin

activity associated with the above-referenced Cellco Partnership d/b/a Verizon

EM-VER-036-120627 – 15 Pent Road, Deep River, Connecticut

EM-VER-041-120405 – 135 Honey Hill Road, East Haddam, Connecticut

**Completion of Construction Activity** 

Wireless telecommunications facilities has been completed.

facility please do not hesitate to contact me.

Dear Ms. Roberts:

RC

Law Offices

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HARTFORD

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STAMFORD

WHITE PLAINS

NEW YORK CITY

ALBANY

Copy to:

Sandy M. Carter

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### STATE OF CONNECTICUT

### CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051
Phone: (860) 827-2935 Fax: (860) 827-2950
E-Mail: siting.council@ct.gov
www.ct.gov/csc

July 13, 2012

Kenneth C. Baldwin, Esq. Robinson & Cole LLP 280 Trumbull Street Hartford, CT 06103

RE: **EM-VER-036-120627-** Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 15 Pent Road, Deep River, Connecticut.

Dear Attorney Baldwin:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

- Any deviation from the proposed modification as specified in this notice and supporting materials with Council shall render this acknowledgement invalid;
- Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
- Not less than 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- The validity of this action shall expire one year from the date of this letter; and
- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration;

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated June 26, 2012. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

Very truly yours,

Linda Roberts
Executive Director

LR/CDM/ibw

c: The Honorable Richard H. Smith, First Selectman, Town of Deep River Amy Petrone, Zoning Enforcement Officer, Town of Deep River Julie Kohler, Esq., Cohen and Wolf, P.C.

### STATE OF CONNECTICUT

### CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051 Phone: (860) 827-2935 Fax: (860) 827-2950 E-Mail: siting.council@ct.gov www.ct.gov/csc

June 28, 2012

The Honorable Richard H. Smith First Selectman Town of Deep River Town Hall 174 Main Street Deep River, CT 06417

RE:

EM-VER-036-120627- Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 15 Pent Road, Deep River, Connecticut.

Dear First Selectman Smith:

The Connecticut Siting Council (Council) received this request to modify an existing telecommunications facility, pursuant to Regulations of Connecticut State Agencies Section 16-50j-72.

If you have any questions or comments regarding this proposal, please call me or inform the Council by July 13, 2012.

Thank you for your cooperation and consideration.

Very truly yours,

Linda Roberts
Executive Director

LR/jbw

Enclosure: Notice of Intent

c: Amy Petrone, Zoning Enforcement Officer, Town of Deep River



# **ROBINSON & COLE**

EM-VER-036-120627

KENNETH C. BALDWIN

280 Trumbull Street Hartford, CT 06103-3597 Main (860) 275-8200 Fax (860) 275-8299 kbaldwin@rc.com Direct (860) 275-8345

June 26, 2012

Linda Roberts
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: Notice of Exempt Modification – Antenna Swap Pent Road, Deep River, Connecticut

Dear Ms. Roberts:

Cellco Partnership d/b/a Verizon Wireless ("Cellco") currently maintains twelve (12) wireless telecommunications antennas at the 170-foot level on an existing 180-foot tower at the above-referenced address. The tower is owned by T-Mobile. Cellco's use of the tower was approved by the Council in 2004. Cellco now intends to replace all of its existing antennas with six (6) model LPA-80080-4CF cellular antennas; three (3) model BXA-171085-8BF PCS antennas; and three (3) model BXA-70063-6CF LTE antennas, all at the same 170-foot level. Cellco also intends to install six (6) coax cable diplexers to its existing antenna platform. Attached behind Tab 1 are the specifications for the replacement antennas and cable diplexers.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Richard Smith, First Selectman of the Town of Deep River. A copy of this letter is also being sent to Robert Stalsberg, Jr., the owner of the property on which the tower is located.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco's replacement antennas and diplexers will be located at the 170-foot level on the existing 180-foot tower.



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# ROBINSON & COLELLP

Linda Roberts June 26, 2012 Page 2

- 2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundaries.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more.
- 4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative power density table for Cellco's modified facility is included behind <u>Tab 2</u>.

Also attached is a Structural Analysis Report confirming that the tower and foundation can support Cellco's proposed modifications. (See <u>Tab 3</u>).

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

Kenneth C. Baldwin

Enclosures Copy to:

Richard Smith, Deep River First Selectman Robert Stalsberg, Jr. Sandy M. Carter





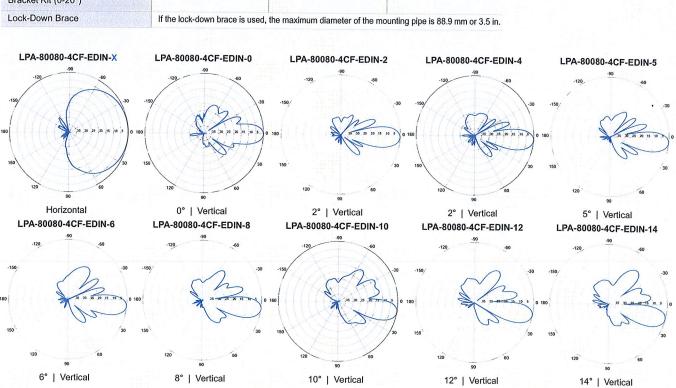
### LPA-80080-4CF-EDIN-X

#### V-Pol | Log Periodic | 80° | 12.5 dBd

Replace "X" with desired electrical downtilt

Antenna is also available with NE connector(s). Replace "EDIN" with "NE" in the model number when ordering.

Frequency bands		806-960 MH:					
Polarization		Vertical		<del></del>			
Horizontal beamwidth		80°					
Vertical beamwidth		15°					
Gain		12.5 dBd (14.6	4D:/				
Electrical downtilt (X)		0, 2, 4, 5, 6, 8, 10,		**************************************			
Impedance		50Ω	12, 14				
VSWR		≤1.4:1					
Upper sidelobe suppression (0°)		-14.2 dB					
Front-to-back ratio (+/-30°)		-34.7 dB					
Null fill		15% (-16.48 d	3)				
Input power		500 W	٥,				
Lightning protection		Direct Ground					
Connector(s)	1 Port / EDIN or NE / Female / Center (Back)						
Mechanical Characteristics			to de				
Dimensions Length x Width x Depth	1200 x 140 x 33	5 mm	47.2	x 5.5 x 13.2 in			
Depth of antenna with z-bracket	37	 '5 mm		14.8 in			
Weight without mounting brackets	5	4 kg		12 lbs			
Survival wind speed	> 20	1 km/hr		> 125 mph			
Wind area	Front: 0.17 m <sup>2</sup> Side: 0.4	0 m <sup>2</sup> Front:	1.8 ft <sup>2</sup>	Side: 4.3 ft <sup>2</sup>			
Wind load @ 161 km/hr (100 mph)	Front: 254 N Side: 57	4 N Front:	57 lbf	Side: 129 lbf			
Mounting Options	Part Number	Fits Pipe Diame	ter	Weigl	nt		
2-Point Mounting & Downtilt Bracket Kit (0-20°)	21699999	50-102 mm 2.0-4	1.0 in	5.4 kg	12 lbs		
Lock-Down Brace	If the lock-down brace is used	the maximum diameter	of the mo	unting ping is 99 0 -	nm or 2		



Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

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### BXA-171085-8BF-EDIN-X

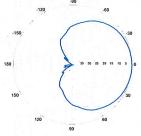
Replace 'X" with desired electrical downtilt.

#### X-Pol | FET Panel | 85° | 16.4 dBi

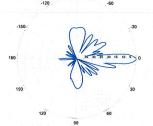
Electrical Characteristics		1710-2	170 MHz		L. Verenia		
Frequency bands	1710-1880 MHz	1850-1	990 MHz	1920-2170	MHz		
Polarization	±45°	±4	45°	±45°			
Horizontal beamwidth	88°	8	5°	80°			
Vertical beamwidth	7°		7°	7°			
Gain	13.5 dBd / 15.6 dBi	13.9 dBd	/ 16.0 dBi	14.3 dBd / 1	6.4 dBi		
Electrical downtilt (X)		0,	2, 4	<del></del>			
Impedance	Ner harbon volt allem fare en less encodinates plates announce generalization and a second and a second annual second	50	0Ω				
VSWR		≤1	.5:1				
First upper sidelobe		< -1	7 dB				
Front-to-back isolation		> 30	0 dB				
In-band isolation		> 28	8 dB				
IM3 (20W carrier)		< -15	0 dBc				
Input power		300 W					
Lightning protection	Direct Ground						
Connector(s)	2 Ports / EDIN / Female / Bottom						
Operating temperature	-40° to +60° C / -40° to +140° F						
Mechanical Characteristics		TWO ISSUED			7,0		
Dimensions Length x Width x Depth	1232 x 154 x 10	05 mm	48.5	x 6.1 x 4.1 in			
Depth with t-brackets	1;	33 mm		5.2 in			
Weight without mounting brackets	4	.8 kg		10.5 lbs			
Survival wind speed	29	96 km/hr	<del></del>	184 mph			
Wind area	Front: 0.19 m <sup>2</sup> Side: 0.1	14 m²	Front: 2.0 ft <sup>2</sup>	Side: 1.5 ft <sup>2</sup>			
Wind load @ 161 km/hr (100 mph)	Front: 281 N Side: 22	23 N	Front: 63 lbf	Side: 50 lbf			
Mounting Options	Part Number	Fits Pipe	Diameter	Weigl	nt 💮		
2-Point Mounting Bracket Kit	26799997	50-102 mm	2.0-4.0 in	2.3 kg	5 lbs		
2-Point Mounting & Downtilt Bracket Kit	26799999	50-102 mm	2.0-4.0 in	3.6 kg	8 lbs		
Concealment Configurations	For concealment configura	tions, order BXA	-171085-8BF-F	DIN-X-FP			



#### BXA-171085-8BF-EDIN-X

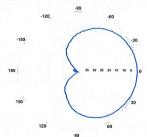


Horizontal | 1710-1880 MHz BXA-171085-8BF-EDIN-0

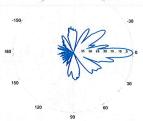


0° | Vertical | 1710-1880 MHz

#### BXA-171085-8BF-EDIN-X

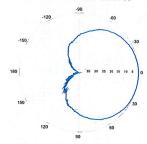


Horizontal | 1850-1990 MHz BXA-171085-8BF-EDIN-0

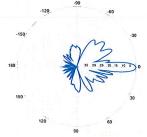


0° | Vertical | 1850-1990 MHz

#### BXA-171085-8BF-EDIN-X



Horizontal | 1920-2170 MHz BXA-171085-8BF-EDIN-0



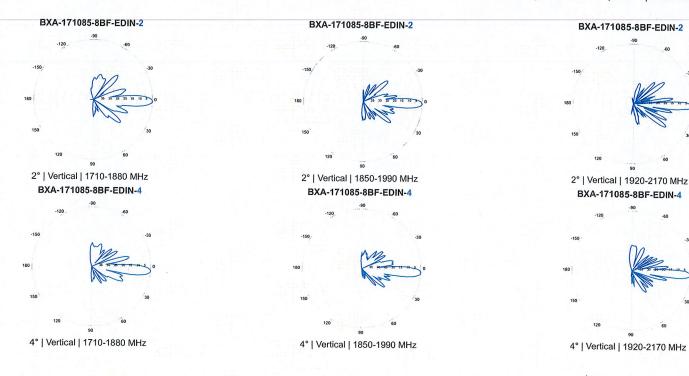
0° | Vertical | 1920-2170 MHz

Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.



# BXA-171085-8BF-EDIN-X

X-Pol | FET Panel | 85° | 16.4 dBi





### BXA-70063-6CF-EDIN-X

#### X-Pol | FET Panel | 63° | 14.5 dBd

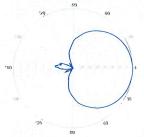
Replace "X" with desired electrical downtilt.

Antenna is also available with NE connector(s). Replace "EDIN" with "NE" in the model number when ordering.

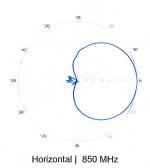
Electrical Characteristics	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	696-900 MHz				
Frequency bands	696-806 MH:	Z	806-900 MHz			
Polarization		±45°				
Horizontal beamwidth	65°		63°			
Vertical beamwidth	13°		11°			
Gain	14.0 dBd (16.1 d	dBi)	14.5 dBd (16.6 dBi)			
Electrical downtilt (X)		0, 2, 3, 4, 5, 6, 8	, 10			
Impedance		50Ω				
VSWR		≤1.35:1				
Upper sidelobe suppression (0°)	-18.3 dB		-18.2 dB			
Front-to-back ratio (+/-30°)	-33.4 dB		-36.3 dB			
Null fill		5% (-26.02 dB	)			
Isolation between ports		< -25 dB				
Input power with EDIN connectors	500 W					
Input power with NE connectors	300 W					
Lightning protection	Direct Ground					
Connector(s)	2 Ports / EDIN or NE / Female / Center (Back)					
Mechanical Characteristics	and the late of the second					
Dimensions Length x Width x Depth	1804 x 285 x 13	2 mm	71.0 x 11.2 x 5.2 in			
Depth with z-brackets	17	'2 mm	6.8 in			
Weight without mounting brackets	7.	9 kg	17 lbs			
Survival wind speed	> 20	1 km/hr	> 125 mph			
Wind area	Front: 0.51 m <sup>2</sup> Side: 0.2	4 m <sup>2</sup> Front:	5.5 ft <sup>2</sup> Side: 2.6 ft <sup>2</sup>			
Wind load @ 161 km/hr (100 mph)	Front: 759 N Side: 39	1 N Front:	169 lbf Side: 89 lbf			
Mounting Options	Part Number	Fits Pipe Diame	ter Weight			
3-Point Mounting & Downtilt Bracket Kit	36210008	40-115 mm 1.57-4	4.5 in 6.9 kg 15.2 lbs			
Concealment Configurations	For concealment configurat	ions, order BXA-70063	8-6CF-EDIN-X-FP			



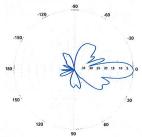
#### BXA-70063-6CF-EDIN-X



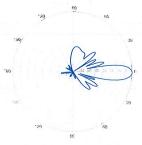
Horizontal | 750 MHz



#### BXA-70063-6CF-EDIN-0

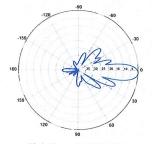


0° | Vertical | 750 MHz

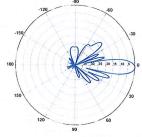


0° | Vertical | 850 MHz

#### BXA-70063-6CF-EDIN-2



2° | Vertical | 750 MHz



2° | Vertical | 850 MHz

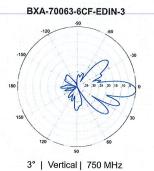
Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

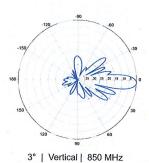
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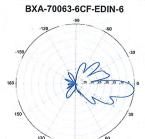


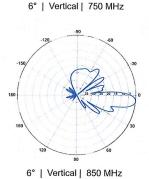
# BXA-70063-6CF-EDIN-X

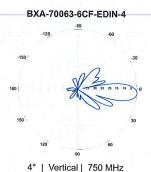
### X-Pol | FET Panel | 63° | 14.5 dBd

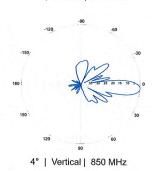


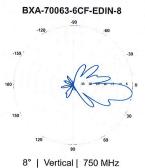


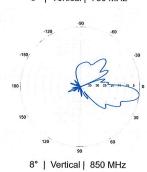


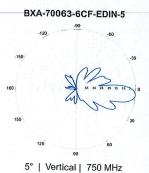


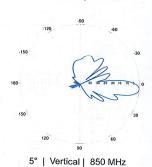


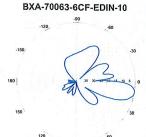


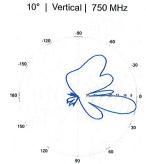












10° | Vertical | 850 MHz

ShareLite Wideband Diplexer - In-line 698-960 MHz/1710-2200 MHz, DC pass in high frequency path



#### **Product Description**

The ShareLite FD9R6004 Series of diplexers are designed to enable feeder sharing between systems in the 698-960 MHz range and in the 1710-2200 MHz range. The diplexer is equipped with in-line connector placement so it can be installed in the BTS cabinet or at the tower top. This is especially valuable in crowded sites or when the feeders are not easily accessible. Due to its wideband design, the FD9R6004 Series can accommodate many combining solutions between 698-960 MHz and 1710-2200 MHz systems such as LTE 700 MHz, Cellular 800 MHz with PCS, GSM900 with GSM1800, or GSM900 with UMTS. This diplexer features a highly selective filter. It provides a high level of isolation between ports, while keeping the insertion loss on both paths at an extremely low level. The FD9R6004 diplexers are available with various DC pass options, helpful in configurations with or without the Tower Mount Amplifiers installed.



#### Features/Benefits

- · LTE ready design
- Extremely Low Insertion Loss
- High level of Rejection between bands Protection against interferences
- Extremely High Power Handling Capability
- Integrated DC block/bypass versions available
- Very compact & small size design Easy installation and reduced tower load
- In-line long-neck connectors for easy connection & waterproofing
- Exceptional reliability & environmental protection (IP 67)
- Equipped with 1 \* Breathable Vent Prevent any humidity inside the product
- Mounting hardware for Wall and Pole mount provided (P/N SEM2-1A)
- Grounding already provided through the mounting bracket
- · Kit available for easy dual mount

Technical Specifications	
Product Type	Diplexer/Cross Band Coupler
Frequency Range 1, MHz	698-960
Frequency Range 2, MHz	1710-2200
Application	LTE700, GSM900, UMTS, GSM1800, Cellular 800, PCS
Configuration	Sharelite Single diplexer, outdoor, DC pass in the 1710-2170MHz path, with mounting hardware SEM2-1A
Mounting	Wall Mounting: With 4 screws (maximum 6mm diameter); Pole Mounting: With included clamp set 40-110mm (1.57-4.33)
Return Loss All Ports Min/Typ, dB	19/23
Power Handling Continuous, Max, W	1250 at common port; 750 in low frequency path & 500 in high frequency path
Power Handling Peak, Max, W	15000 in low frequency path & 8000 in high frequency path
Impedance, Ohms	50
Insertion Loss, Path 1, dB	0.07 typ.
Insertion Loss, Path 2, dB	0.13 typ.
Rejection Between Bands Min/Typ, dB	58/64@698-960MHz; 60/70@1710-2200MHz
IMP Level at the COM Port, Typ, dBm	-112 @ 2x43
DC Pass in Low Frequency Path	No
DC Pass in High Frequency Path	Yes
Temperature Range, °C (°F)	-40 to +60 (-40 to +140)
Environmental	ETSI 300-019-2-4 Class 4.1E
Ingress Protection	IP 67
Lightning Protection	EN/IEC61000-4-5 Level 4
Connectors	In-line long-neck 7-16-Female
Weight, kg (lb)	1.2 (2.6)
Shipping Weight, kg (lb)	3.2 (7) for 2 * single units in 1 * box, 9.8 (21.6) for 6 * units = 3 * Boxes in 1 * overwrap
Dimensions, H x W x D, mm (in)	147 x 164 x 37 (5.8 x 6.5 x 1.5)
Shipping Dimensions, H x W x D, mm (in)	254 x 406 x 82 (10 x 16 x 3.2) for 2 * Single Units in 1 * box, 280 x 406 x 241 (11 x 16 x 9.5) for 6 * units = 3 * Boxes in 1 * overwrap
Volume, L	0.43
Housing	Aluminum

RFS The Clear Choice ®

All information contained in the present datasheet is subject to confirmation at time of ordering

FD9R6004/2C-3L

Rev: A / 10/12/2011

Print Date: 13.03.2012

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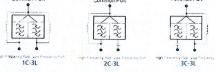


ShareLite Wideband Diplexer - In-line 698-960 MHz/1710-2200 MHz, DC pass in high frequency path

#### Other Documentation

FD9R6004/2C-3L Installation Instructions: Wideband\_Diplexer\_Installation\_Rev5.pdf

FD9R6004/1C-3L	
LD200004/ IC-2F	X
Single FD9R6004/2C-3L	X
FD9R6004/3C-3L	X
KIT-FD9R6004/1C-DL	X
Dual KIT-FD9R6004/2C-DL	X
KIT-FD9R6004/3C-DL	X



Mounting Hard	lware and Ground Cable Ordering Information	
Model Number	Description	
SEM2-1A	Mounting Hardware, Pole mount o40-110mm (Included with the Single and Dual Diplexer) Wall Screws M6 (Not included with the product)	90
SEM2-3	Assembly kit for 2 pcs of FD9R6004/xC-3L (Can be ordered separately but included with the Dual Diplexer Kit)	File
CA020-2	Ground Cable, 2m, includes lugs (Optional)	
CA030-2	Ground Cable, 2m, includes lugs (Optional)	( Spring
SEM6	Mounting Hardware for 6 Diplexers, Tower Base (Optional)	-

	•	

	General	Power	Density					
Site Name: Deep River								
Tower Height: Verizon @ 1	170ft							
				CALC.		MAX.		
				POWER		PERMISS.	FRACTION	
CARRIER	# OF CHAN.	WATTS ERP	HEIGHT	DENS	FREQ.	EXP.		Total
*VoiceStream	ω	245.7	180	0.0218	1930	1.0000	2.18%	
*AT&T UMTS	П	200	160	0.0070	880	0.5867	1.20%	
*AT&T GSM	2	427	160	0.0120	1900	1.0000	1.20%	
*AT&T GSM	4	296	160	0.0166	880	0.5867	2.83%	
*Pocket	3	631	150	0.0303	2130	1.0000	3.03%	
Verizon PCS	11	233	170	0.0319	1970	1.0000	3.19%	
/erizon Cellular	6	246	170	0.0275	869	0.5793	4.76%	
Verizon AWS	1	680	170	0.0085	2145	1.0000	0.85%	
Verizon 700	1	810	170	0.0101	869	0.4653	2.17%	
								21.39%
* Source: Siting Council								
- 6.5								
							_	-



# STRUCTURAL ANALYSIS REPORT



SITE NUMBER:

CT11237C

SITE NAME:

**DEEP RIVER/RT 9** 

**SITE ADDRESS:** 

**15 PENT ROAD** 

DEEP RIVER, CT 06417

NEW ANTENNA INSTALLATION ON AN EXISTING 178' MONOPOLE

BY:



CARRIER SITE NAME: DEEP RIVER CT

April 25, 2012

GPD Project #: 2011711.11

# **MONOPOLE**

### STRUCTURAL ANALYSIS REPORT

CT11237C DEEP RIVER/RT 9 15 Pent Road Deep River, CT 06417 GPD Project #: 2011711.11

New Antenna Installation Existing 178 ft Monopole

For: T-Mobile Towers Bellevue, Washington

Prepared By:

David B. Granger, P.E. Registered Professional Engineer Connecticut #: 17557

April 25, 2012

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#### APPENDICES

- 1. TNX ANALYSIS PRINTOUT
- 2. TOWER ELEVATION DRAWING AND FEEDLINE PLAN
- 3. ANCHOR ROD AND BASE PLATE ANALYSIS
- 4. FOUNDATION ANALYSIS

#### **EXECUTIVE SUMMARY**

The purpose of this analysis is to verify whether the existing tower is structurally capable of carrying the new antenna and coax loads as specified by Verizon to T-Mobile Towers. This report was commissioned by Mr. Kenneth Fann of T-Mobile Towers.

The existing structure meets the requirements of TIA/EIA-222-F and the 2005 Connecticut State Building Code for an 85 mph fastest-mile wind speed with 1/2" of radial ice (w/ 25% wind load reduction) for the proposed antenna configuration.

The foundation reactions, with the proposed loading configuration, were found to be less than the existing foundation design. Therefore, the existing foundation is sufficient, assuming it was properly constructed according to original design.

#### Section Results

	occusii Results	
Monopole	% Capacity	Result
164.3′ – 178′	16.0%	Pass
129.8′ – 164.3′	52.9%	Pass
96.1′ – 129.8′	60.5%	Pass
63.3′ – 96.1′	69.1%	Pass
31.3′ – 63.3′	74.2%	Pass
0' - 31.3'	81.3%	Pass
Anchor Rods	72.3%	Pass
Base Plate	74.8%	Pass
<b>Foundation</b>	% Capacity	Result
Bearing	37.7%	Pass
Overturning	73.5%	Pass
Tower Rating: 8	1.3%	

#### **TOWER DESCRIPTION**

The existing 178' monopole is located in Deep River, Connecticut. The tower was originally designed for Voicestream Wireless by PiROD Inc. of Plymouth, Indiana. The original design load for the tower was for an 85 mph basic wind speed with 1/2" radial ice (w/ 25% wind load reduction) in accordance with EIA/TIA-222-F. The tower was originally designed to hold the following:

#### Original Configuration

Antennas:	
Elev. 180'	(9) 1' x 5' Panel Antennas on a LP Platform, w/ 1-5/8" internal coax
Elev. 170'	(9) 1' x 5' Panel Antennas on a LP Platform, w/ 1-5/8" internal coax
Elev. 160'	(12) 1' x 5' Panel Antennas on a LP Platform, w/ 1-5/8" internal coax
Elev. 150'	(12) 1' x 5' Panel Antennas on a LP Platform, w/ 1-5/8" internal coax
Elev. 140'	(12) 1' x 5' Panel Antennas on a LP Platform, w/ 1-5/8" internal coax

The existing monopole has six major sections connected by slip joints. It has 18 sides and is evenly tapered from 62.9375" (flat-flat) at the base to 22.6400" (flat-flat) at the top. The structure is galvanized and has no tower lighting.

All structural information for this analysis was provided by T-Mobile Towers in the form of the original tower and foundation drawings by PiROD Inc. (Eng. File #: A-117035, dated August 29, 2000). Geotechnical information was provided in the form of a soils report by Dr. Clarence Welti, P.E., P.C., dated May 1, 2000. Additional information was provided in the form of a site inspection report by SiteMaster, dated July 24, 2009 and a previous analysis by Semaan Engineering Solutions, LLC, dated December 11, 2008. Existing, reserved, and proposed antenna information was provided by T-Mobile Towers. This analysis and report are based solely on this information.

#### **TOWER MATERIALS**

Data on the steel strength was available from the information provided. The following table details the steel strength used in the analysis.

Monopole	ASTM A572 (65 KSI Yield Strength)
Anchor Rods	ASTM A687 (105 KSI Yield Strength)
Base Plate	ASTM A572 (50 KSI Yield Strength)

### **TOWER LOADING**

The following data shows the major loading that the tower supports. The existing, reserved, and proposed antenna information was provided by T-Mobile Towers.

**Existing and Reserved Configuration** 

Elevation	Carrier	Antennas
1 <i>77</i> .5′	T-Mobile	(12) Andrew TMBXX-6516-R2M Antennas,
		(6) Andrew ETW190VS12UB TMAs & (1) 4' HP MW Dish
		on a 16.5' LP Platform, w/ (25) 1-5/8" internal coax
170′	Verizon	(12) Decibel DB844H90E-XY Antennas on a 12.5' LP Platform,
		w/ (12) 1-5/8" internal coax
		12.5' LP Platform, w/ (12) 1-5/8" internal coax
160′	AT&T	(6) Powerwave 7770 Antennas, (6) Powerwave LGP21401 TMAs
		& (6) Powerwave 21903 Diplexers on a12.5' LP Platform,
		w/ (12) 1-5/8" internal coax
150′	Pocket	(3) Kathrein 742 213 Antennas, flush mounted,
Programme (1912)		w/ (6) 1-5/8" internal coax

**Proposed Configuration** 

Elevation	Carrier	Antennas
177.5'	T-Mobile	(12) Andrew TMBXX-6516-R2M Antennas,
		(6) Andrew ETW190VS12UB TMAs & (1) 4' HP MW Dish
est des son trans		on a 16.5' LP Platform, w/ (25) 1-5/8" internal coax
170'	Verizon	(3) Antel BXA-70063-6CF Antennas,
		(6) Antel LPA-80080-4CF Antennas,
		(3) Antel BXA-171085/8BF Antennas &
		(6) RFS FD9R6004/2C-3L Diplexers on a
		12.5' LP Platform, w/ (12) 1-5/8" internal coax
160′	AT&T	(6) Powerwave 7770 Antennas, (6) Powerwave LGP21401 TMAs
		& (6) Powerwave 21903 Diplexers on a12.5' LP Platform,
		w/ (12) 1-5/8" internal coax
150'	Pocket	(3) Kathrein 742 213 Antennas, flush mounted,
		w/ (6) 1-5/8" internal coax

Notes: - BOLD type indicates proposed carrier's final configuration.

- See Appendix 2 for the proposed coax layout.

The purpose of this independent structural analysis review is to determine if the existing tower, with the proposed configuration, is in conformance to the latest TIA/EIA-222-F and the 2005 Connecticut State Building Code standard requirements.

#### **ANALYSIS**

The purpose of this structural analysis review is to determine if the existing tower is in conformance to the latest TIA/EIA-222-F standard requirements. TnxTower (Version v6.0.2.0), a commercially available software program, was used to create a three-dimensional model of the tower and calculate member stresses for various dead, live, wind, and ice load cases. All loads were computed in accordance with the ANSI/TIA/EIA-222-F standard and all local building code requirements. Selected output from the analysis is included in Appendix 1.

The current requirements of TIA/EIA-222-F and the 2005 Connecticut State Building Code are for a fastest-mile wind speed of 85 mph with 1/2" radial ice. A 25% reduction in wind load is allowed when wind and ice are applied simultaneously. TIA/EIA-222-F requires tower within Middlesex County, Connecticut to be analyzed with an 85 mph fastest-mile wind speed at this tower location.

#### **ANALYSIS FASTEST-MILE WIND SPEED:**

85 MPH

The tower and foundations are assumed, for the purpose of this analysis, to have been properly fabricated, constructed, maintained, and to be in good condition with no structural defects. This is not a condition assessment of the tower and has been provided without the benefit of recent detailed tower photos, a recent detailed tower mapping, or a recent GPD Group site visit. This analysis assumes all antennas and coax have been installed in a neat and orderly fashion. Proposed antennas are assumed to be installed on standard sized mounts. The existing/proposed mounts are assumed to have been verified by the carrier to support the existing/proposed loading for the required various load cases.

### **CONCLUSIONS AND RECOMMENDATIONS**

Based on the computer structural analysis results, the existing 178' monopole meets the requirements of TIA/EIA-222-F and then 2005 Connecticut State Building Code for an 85 mph fastest-mile wind speed with 1/2" of radial ice (w/ 25% wind load reduction) for the proposed antenna configuration.

The foundation reactions, with the proposed loading configuration, were found to be less than the capacity of the existing foundation design. Therefore, the existing foundation is adequate assuming it was properly constructed according to original design.

#### Summary of Findings

Monopole	Satisfactory	
<b>Anchor Rods</b>	Satisfactory	
<b>Base Plate</b>	Satisfactory	
Foundation	Satisfactory	

Therefore, based on our analysis results, the existing structure is structurally satisfactory for the proposed loading configuration.



#### **DISCLAIMER OF WARRANTIES**

GPD GROUP has not performed a recent site visit to the tower to verify the member sizes or antenna/coax loading. If the existing conditions are not as represented on the tower elevation contained in this report, we should be contacted immediately to evaluate the significance of the discrepancy. This is not a condition assessment of the tower or foundation. This report does not replace a full tower inspection. The tower and foundations are assumed to have been properly fabricated, erected, maintained, in good condition, twist free, and plumb.

The engineering services rendered by GPD GROUP in connection with this Structural Analysis are limited to a computer analysis of the tower structure and theoretical capacity of its main structural members. All tower components have been assumed to only resist dead loads when no other loads are applied. No allowance was made for any damaged, bent, missing, loose, or rusted members (above and below ground). No allowance was made for loose bolts or cracked welds.

GPD GROUP does not analyze the fabrication of the structure (including welding). It is not possible to have all the very detailed information needed to perform a thorough analysis of every structural subcomponent and connection of an existing tower. GPD GROUP provides a limited scope of service in that we cannot verify the adequacy of every weld, plate connection detail, etc. The purpose of this report is to assess the feasibility of adding appurtenances usually accompanied by transmission lines to the structure.

It is the owner's responsibility to determine the amount of ice accumulation, if any, that should be considered in the structural analysis.

The attached sketches are a schematic representation of the analyzed tower. If any material is fabricated from these sketches, the contractor shall be responsible for field verifying the existing conditions, proper fit, and clearance in the field. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from GPD GROUP, but are beyond the scope of this report.

Miscellaneous items such as antenna mounts etc. have not been designed or detailed as a part of our work. We recommend that material of adequate size and strength be purchased from a reputable tower manufacturer.

GPD GROUP makes no warranties, expressed and/or implied, in connection with this report and disclaim any liability arising from material, fabrication, and erection of this tower. GPD GROUP will not be responsible whatsoever for, or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of GPD GROUP pursuant to this report will be limited to the total fee received for preparation of this report.

#### CT11237C DEEP RIVER/RT 9

### **APPENDICES**

- 1. TNX Analysis Printout
- 2. Tower Elevation Drawing and Feedline Plan
- 3. Anchor Rod and Base Plate Analysis
- 4. Foundation Analysis

# TNX ANALYSIS PRINTOUT



tnxTower	Job CT11237C Deep River/Rt 9	Page 1 of 6
GPD Group 520 South Main St.	Project 2011711.11	Date 15:44:25 12/13/11
Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101	Client T-Mobile Towers	Designed by uguduru

# **Tower Input Data**

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Middlesex County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 74 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

# Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Component Type	Placement	Total Number	OLENSANDERSON SINGLESSES SERVICES	$C_A A_A$	Weight
	Leg			ft			ft²/ft	plf
LDF7-50A (1-5/8	C	No	Inside Pole	177.50 - 8.00	25	No Ice	0.00	0.82
FOAM)						1/2" Ice	0.00	0.82
LDF7-50A (1-5/8	В	No	Inside Pole	170.00 - 8.00	12	No Ice	0.00	0.82
FOAM)						1/2" Ice	0.00	0.82
LDF7-50A (1-5/8	Α	No	Inside Pole	160.00 - 8.00	12	No Ice	0.00	0.82
FOAM)						1/2" Ice	0.00	0.82
LDF7-50A (1-5/8	В	No	Inside Pole	150.00 - 8.00	6	No Ice	0.00	0.82
FOAM)						1/2" Ice	0.00	0.82
Climbing Pegs	В	No	CaAa (Out Of	178.00 - 8.00	1	No Ice	0.00	0.31
			Face)			1/2" Ice	0.00	0.71
Safety Line 3/8	В	No	CaAa (Out Of	178.00 - 8.00	1	No Ice	0.04	0.22
			Face)			1/2" Ice	0.14	0.75

# **Discrete Tower Loads**

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement	PROPERTY NEWSCOOLS OF THE STATE	C <sub>A</sub> A <sub>A</sub> Front	$C_{\Lambda}A_{\Lambda}$ Side	Weight
	8		Vert						1 26.
			ft	0	ft		$ft^2$	$ft^2$	lb
			ft						
			ft				10-511 ANS		
Pirod 16.5' LP Platform	C	None		0.0000	177.50	No Ice	20.80	20.80	1800.00
						1/2" Ice	28.10	28.10	2066.00
PiROD 12' Lightweight	C	From	4.00	0.0000	177.50	No Ice	10.20	2.94	253.00
T-Frame (GPD)		Centroid-Le	0.00			1/2" Ice	16.20	4.96	355.00
		g	0.00						

# tnxTower

GPD Group 520 South Main St. Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101

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Client	TALL	Designed by
	T-Mobile Towers	uguduru

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	$C_A A_A$ Side	Weight
	200		Vert						
			ft	0	ft		$ft^2$	$ft^2$	lb
			ft ft						
(4) TMBXX-6516-R2M w/ 6'	A	From	4.00	0.0000	177.50	No Ice	7.08	5.24	56.50
Mount Pipe		Centroid-Le g	0.00			1/2" Ice	7.64	6.12	109.72
(4) TMBXX-6516-R2M w/ 6'	В	From	4.00	0.0000	177.50	No Ice	7.08	5.24	56.50
Mount Pipe		Centroid-Le	0.00			1/2" Ice	7.64	6.12	109.72
(4) TMBXX-6516-R2M w/ 6'	C	g From	4.00	0.0000	177.50	No Ice	7.08	5.24	56.50
Mount Pipe		Centroid-Le	0.00	0.0000	177.50	1/2" Ice	7.64	6.12	109.72
		g	0.00					0.12	107.72
(2) ETW190VS12UB	Α	From	4.00	0.0000	177.50	No Ice	0.66	0.35	11.00
		Centroid-Le	0.00			1/2" Ice	0.78	0.44	15.83
(2) ETW190VS12UB	В	g From	0.00 4.00	0.0000	177.50	No Iso	0.66	0.25	11.00
(2) E1 (1) 0 (0120B	ь	Centroid-Le	0.00	0.0000	177.30	No Ice 1/2" Ice	0.66 0.78	0.35 0.44	11.00 15.83
		g	0.00			172 100	0.76	0.44	13.63
(2) ETW190VS12UB	C	From	4.00	0.0000	177.50	No Ice	0.66	0.35	11.00
		Centroid-Le	0.00			1/2" Ice	0.78	0.44	15.83
4" x 4' Mount Pipe	С	g	0.00	0.0000	199.50				
4 X 4 Mount Pipe	C	From Centroid-Le	4.00 0.00	0.0000	177.50	No Ice 1/2" Ice	1.21 1.47	1.21	36.48
		g	0.00			1/2 ICE	1.47	1.47	48.11
MTS 12.5' LP Platform	C	None		0.0000	170.00	No Ice	14.66	14.66	1250.00
						1/2" Ice	18.87	18.87	1481.33
BXA-70063-6CF w/ 72"	Α	From	4.00	0.0000	170.00	No Ice	7.75	5.58	38.90
Mount Pipe		Centroid-Le	0.00			1/2" Ice	8.29	6.52	94.85
BXA-70063-6CF w/ 72"	В	g From	4.00	0.0000	170.00	No Ice	7.75	5.58	38.90
Mount Pipe	ь	Centroid-Le	0.00	0.0000	170.00	1/2" Ice	8.29	6.52	94.85
		g	0.00			172 100	0.27	0.52	74.03
BXA-70063-6CF w/ 72"	C	From	4.00	0.0000	170.00	No Ice	7.75	5.58	38.90
Mount Pipe		Centroid-Le	0.00			1/2" Ice	8.29	6.52	94.85
(2) LPA-80080/4CF w/ 6'	Α	g From	0.00 4.00	0.0000	170.00	No Ice	2.11	7.40	22.00
Mount Pipe	. 101 . 1	Centroid-Le	0.00	0.0000	170.00	1/2" Ice	3.11 3.58	7.48 8.38	33.90 80.49
		g	0.00			172 100	5.50	0.50	00.49
(2) LPA-80080/4CF w/ 6'	В	From	4.00	0.0000	170.00	No Ice	3.11	7.48	33.90
Mount Pipe		Centroid-Le	0.00			1/2" Ice	3.58	8.38	80.49
(2) LPA-80080/4CF w/ 6'	C	g From	0.00 4.00	0.0000	170.00	NI. I.	2.11	7.40	22.00
Mount Pipe	C	Centroid-Le	0.00	0.0000	170.00	No Ice 1/2" Ice	3.11 3.58	7.48 8.38	33.90 80.49
		g	0.00			1/2 100	5.50	0.50	00.49
BXA-171085-8BF w/ 2"x48"	Α	From	4.00	0.0000	170.00	No Ice	2.94	3.02	25.10
Mountpipe		Centroid-Le	0.00			1/2" Ice	3.26	3.57	52.92
BXA-171085-8BF w/ 2"x48"	В	g From	0.00	0.0000	170.00		201		
Mountpipe	В	Centroid-Le	4.00 0.00	0.0000	170.00	No Ice 1/2" Ice	2.94	3.02	25.10
Meantpipe		g g	0.00			1/2 Ice	3.26	3.57	52.92
BXA-171085-8BF w/ 2"x48"	C	From	4.00	0.0000	170.00	No Ice	2.94	3.02	25.10
Mountpipe		Centroid-Le	0.00			1/2" Ice	3.26	3.57	52.92
(a) EDOD (004/2C 27		g	0.00	. 12.					
(2) FD9R6004/2C-3L	Α	From	4.00	0.0000	170.00	No Ice	0.37	0.08	3.10
		Centroid-Le	0.00			1/2" Ice	0.45	0.14	5.40
(2) FD9R6004/2C-3L	В	g From	4.00	0.0000	170.00	No Ice	0.37	0.08	3.10
David.	47.1	Centroid-Le	0.00	0.0000	170.00	1/2" Ice	0.37	0.08	5.40
		g	0.00				danski vista		5.10
(2) FD9R6004/2C-3L	C	From	4.00	0.0000	170.00	No Ice	0.37	0.08	3.10

# tnxTower

GPD Group 520 South Main St. Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101

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Project		Date
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Client T-Mobile Towers		Designed by
		uguduru

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C <sub>A</sub> A <sub>A</sub> Front	$C_A A_A$ Side	Weight
			Vert						
			ft ft ft	o	ft		ft <sup>2</sup>	ft²	lb
the second section and the second section and the second section and the second section as the second		Centroid-Le	0.00			1/2" Ice	0.45	0.14	5.40
		g	0.00			1/2 100	0.45	0.14	3.40
MTS 12.5' LP Platform	C	None	0.00	0.0000	160.00	No Ice	14.66	14.66	1250.00
				0.0000	100.00	1/2" Ice	18.87	18.87	1481.33
(2) 7770.00 w/ 6' Mount Pipe	Α	From	4.00	0.0000	160.00	No Ice	6.22	4.35	60.90
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		Centroid-Le	0.00	0.0000	100.00	1/2" Ice	6.77	5.20	106.99
		g	0.00			1/2 100	0.77	3.20	100.99
(2) 7770.00 w/ 6' Mount Pipe	В	From	4.00	0.0000	160.00	No Ice	6.22	4.35	60.90
,-,		Centroid-Le	0.00	0.0000	100.00	1/2" Ice	6.77	5.20	106.99
		g	0.00			1/2 100	0.77	3.20	100.99
(2) 7770.00 w/ 6' Mount Pipe	C	From	4.00	0.0000	160.00	No Ice	6.22	4.35	60.90
a) /// olos III o III oliik l'ipe	Ŭ	Centroid-Le	0.00	0.0000	100.00	1/2" Ice	6.77	5.20	
		g g	0.00			1/2 100	0.77	3.20	106.99
(2) LGP21401	Α	From	4.00	0.0000	160.00	No Ice	1.29	0.23	14.10
(2) 20121101		Centroid-Le	0.00	0.0000	100.00	1/2" Ice	1.45	0.23	21.26
		g	0.00			1/2 100	1.43	0.31	21.20
(2) LGP21401	В	From	4.00	0.0000	160.00	No Ice	1.29	0.23	14.10
(2) 23.21.01		Centroid-Le	0.00	0.0000	100.00	1/2" Ice	1.45	0.23	14.10
		g	0.00			1/2 100	1.43	0.31	21.26
(2) LGP21401	C	From	4.00	0.0000	160.00	No Ice	1.29	0.23	14.10
(2) 20121101	Č	Centroid-Le	0.00	0.0000	100.00	1/2" Ice	1.45		14.10
		g g	0.00			1/2 ICE	1.43	0.31	21.26
(2) LGP21903 Diplexer	Α	From	4.00	0.0000	160.00	No Ice	0.27	0.10	11.00
(2) Bot 21903 Biplexer	21	Centroid-Le	0.00	0.0000	100.00	1/2" Ice	0.27	0.18	11.02
		g	0.00			1/2 Ice	0.34	0.25	13.44
(2) LGP21903 Diplexer	В	From	4.00	0.0000	160.00	No Isa	0.07	0.10	11.00
(2) EGI 21903 Dipiexei	ь	Centroid-Le	0.00	0.0000	160.00	No Ice	0.27	0.18	11.02
			0.00			1/2" Ice	0.34	0.25	13.44
(2) LGP21903 Diplexer	C	g From	4.00	0.0000	160.00	NI. T	0.07	0.10	11.00
(2) EGI 21903 Diplexel	C	Centroid-Le	0.00	0.0000	100.00	No Ice	0.27	0.18	11.02
			0.00			1/2" Ice	0.34	0.25	13.44
Valmont Light Duty	C	g None	0.00	0.0000	150.00	NI I	1.77	1.50	<b>7.1.00</b>
Tri-Bracket (1)	C	None		0.0000	150.00	No Ice	1.76	1.76	54.00
742 213 w/ Mount Pipe	Α	From Leg	0.50	0.0000	150.00	1/2" Ice No Ice	2.08	2.08	70.00
742 215 W Would Tipe	Α	110III Leg	0.00	0.0000	130.00		5.37	4.62	48.92
			0.00			1/2" Ice	5.95	6.00	90.56
742 213 w/ Mount Pipe	В	From Leg	0.50	0.0000	150.00	No Ioo	5.27	4.60	40.00
, 12 215 W Mount Tipe	Ь	1 Tolli Leg	0.00	0.0000	130.00	No Ice	5.37	4.62	48.92
			0.00			1/2" Ice	5.95	6.00	90.56
742 213 w/ Mount Pipe	C	From Leg	0.50	0.0000	150.00	N. T.	5 27	1.00	40.00
142 215 W/ Would Tipe	C	1 Tolli Leg		0.0000	150.00	No Ice	5.37	4.62	48.92
			0.00			1/2" Ice	5.95	6.00	90.56

Dishes											
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				Vert ft	0	0	ft	ft		$ft^2$	lb
4' Dish	С	Paraboloid w/Shroud (HP)	From Centroid -Leg	4.00 0.00 0.00	0.0000		177.50	4.00	No Ice 1/2" Ice	12.57 13.10	80.00 90.00

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inalower	CT112	4 of 6	
<b>GPD Group</b> 520 South Main St.	Project	2011711.11	Date 15:44:25 12/13/11
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	Maximum	Tower	<b>Deflections</b>	- Service Wind
Elevation	Horz. Deflection	Gov. Load	remains a conservative conservation and mainly and administration approximately $Tilt$	Twist
ft	in	Comb.	o	o
178 - 164.333	35.769	29	1.8136	0.0091
167.25 - 129.75	31.714	29	1.7855	0.0067
133.583 - 96.0833	20.061	29	1.4639	0.0031
100.75 - 63.25	11.236	29	1.0814	0.0016
68.75 - 31.25	5.162	29	0.7101	0.0009

0.3727

0.0009

0.0004

Section

No.

L2 L3

L4

L5

L6

37.5 - 0

	Critical Deflections and Radius of Curvature - Service Wind									
Elevation	and a section notes in the section of the section	Gov. Load	Deflection	Tilt	Twist	Radius of  Curvature				
ft		Comb.	in	•	0	ft				
177.50	4' Dish	29	35.579	1.8127	0.0090	21517				
170.00	MTS 12.5' LP Platform	29	32.744	1.7953	0.0073	13529				
160.00	MTS 12.5' LP Platform	29	29.040	1.7441	0.0054	8003				
150.00	Valmont Light Duty Tri-Bracket (1)	20	25.490	1.6541	0.0042	6003				

		Maximum	Tower	<b>Deflections</b>	- Design Wind
Section No.	Elevation	Horz. Deflection	Gov. Load	тии Тии	Twist
	ft	in	Comb.	•	0
L1	178 - 164.333	103.254	4	5.2371	0.0261
L2	167.25 - 129.75	91.542	4	5.1652	0.0190
L3	133.583 - 96.0833	57.916	4	4.2299	0.0087
L4	100.75 - 63.25	32.446	4	3.1242	0.0047
L5	68.75 - 31,25	14.911	4	2.0512	0.0025
L6	37.5 - 0	4.469	4	1.0767	0.0011

=	Critical Deflections and Radius of Curvature - Design Wind									
Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature				
ft		Comb.	in	٥	۰	Curvature ft				
177.50	4' Dish	4	102.706	5.2351	0.0257	7835				
170.00	MTS 12.5' LP Platform	4	94.517	5.1930	0.0207	4925				
160.00	MTS 12.5' LP Platform	4	83.823	5.0429	0.0154	2880				
150.00	Valmont Light Duty Tri-Bracket (1)	4	73.548	4.7807	0.0120	2229				

# tnxTower

GPD Group 520 South Main St. Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101

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Project		Date
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Client	TALL	Designed by
	T-Mobile Towers	uguduru

# Compression Checks

· <del>.</del>	Pole Design Data									
Section No.	Elevation	остинения принцения чин учения сторования от принцения высования в принцения выполняющей выполняющей выполняющ Size	L	L <sub>u</sub>	Kl/r	$F_a$	A	Actual P	Allow. Pa	Ratio P
	ft		ft	fi		ksi	in <sup>2</sup>	lb	lb	$P_a$
L1	178 - 164.333 (1)	TP26x22.64x0.25	13.67	0.00	0.0	39.000	19.8636	-4545.84	774681.00	0.006
L2	164.333 - 129.75 (2)	TP34.0625x24.7829x0.3125	37.50	0.00	0.0	39.000	32.5349	-11035.50	1268860.00	0.009
L3	129.75 - 96.0833 (3)	TP41.75x32.4889x0.375	37.50	0.00	0.0	39.000	47.8748	-17928.80	1867120.00	0.010
L4	96.0833 - 63.25 (4)	TP49.0625x39.8475x0.375	37.50	0.00	0.0	39.000	56.3416	-26054.00	2197320.00	0.012
L5	63.25 - 31.25 (5)	TP56.125x46.961x0.375	37.50	0.00	0.0	39.000	64.5385	-35272.20	2517000.00	0.014
L6	31.25 - 0 (6)	TP62.9375x53.8477x0.375	37.50	0.00	0.0	37.124	74.4650	-47316.60	2764410.00	0.017

		P	ole Be	endin	g Des	ign [	)ata			
Section No.	Elevation ft	Size	Actual M <sub>x</sub> lb-ft	Actual f <sub>bx</sub> ksi	Allow. F <sub>bx</sub> ksi	Ratio  f <sub>bx</sub> F <sub>by</sub>	Actual M <sub>y</sub> lb-ft	Actual f <sub>by</sub> ksi	Allow. F <sub>by</sub> ksi	$\frac{Ratio}{f_{by}}$ $\frac{f_{by}}{F_{by}}$
Li	178 - 164.333 (1)	TP26x22.64x0.25	82231.4	8.059	39.000	0.207	0.00	0.000	39.000	0.000
L2	164.333 - 129.75 (2)	TP34.0625x24.7829x0.3125	594594. 17	27.140	39.000	0.696	0.00	0.000	39.000	0.000
L3	129.75 - 96.0833 (3)	TP41.75x32.4889x0.375	1227383	31.041	39.000	0.796	0.00	0.000	39.000	0.000
L4	96.0833 - 63.25 (4)	TP49.0625x39.8475x0.375	1944375 .00	35.456	39.000	0.909	0.00	0.000	39.000	0.000
L5	63.25 - 31.25 (5)	TP56.125x46.961x0.375	2737983	38.013	39.000	0.975	0.00	0.000	39.000	0.000
L6	31.25 - 0 (6)	TP62.9375x53.8477x0.375	3800400 .00	39.597	37.124	1.067	0.00	0.000	37.124	0.000

			Pole S	hear	Desig	jn Da	nta			
Section No.	Elevation ft	Size	Actual V lb	Actual f <sub>v</sub> ksi	Allow. F, ksi	Ratio  f <sub>v</sub> F <sub>v</sub>	Actual T lb-ft	Actual f <sub>vi</sub> ksi	Allow. F <sub>vi</sub> ksi	Ratio $\frac{f_{vt}}{F_{vt}}$
Ll	178 - 164.333 (1)	TP26x22.64x0.25	11166.9 0	0.562	26.000	0.043	2.85	0.000	26.000	0.000
L2	164.333 - 129.75 (2)	TP34.0625x24.7829x0.3125	17717.3 0	0.545	26.000	0.042	13.19	0.000	26.000	0.000
L3	129.75 - 96.0833 (3)	TP41.75x32.4889x0.375	20837.0	0.435	26.000	0.033	25.03	0.000	26.000	0.000
L4	96.0833 - 63.25 (4)	TP49.0625x39.8475x0.375	23931.1	0.425	26.000	0.033	37.61	0.000	26.000	0.000
L5	63.25 - 31.25 (5)	TP56.125x46.961x0.375	26760.9 0	0.415	26.000	0.032	50.06	0.000	26.000	0.000

# tnxTower

GPD Group 520 South Main St. Akron, OH 44311 Phone: (330) 572-2100 FAX: (330) 572-2101

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Client	T-Mobile Towers	Designed by uguduru

Section	Elevation	Size	Actual	Actual	Allow.	Ratio	Actual	Actual	Allow.	Ratio
No.	r.		V	$f_{\nu}$	$F_{ u}$	$f_v$	T	$f_{vi}$	$F_{vt}$	$f_{vi}$
	Ji		lb	ksi	ksi	$F_{\nu}$	lb-ft	ksi	ksi	$F_{\nu t}$
L6	31.25 - 0 (6)	TP62.9375x53.8477x0.375	29873.5 0	0.401	26.000	0.031	61.85	0.000	26.000	0.000

Pole Interaction De	esign Data
---------------------	------------

Section No.	Elevation	Ratio P	Ratio f <sub>bx</sub>	Ratio f <sub>by</sub>	Ratio f <sub>v</sub>	Ratio f <sub>vi</sub>	Comb. Stress	Allow. Stress	Criteria
	ft	$P_a$	$F_{bx}$	$F_{bv}$	$F_{\nu}$	$F_{vt}$	Ratio	Ratio	
L1	178 - 164.333 (1)	0.006	0.207	0.000	0.043	0.000	0.213	1.333	H1-3+VT 🗸
L2	164.333 - 129.75 (2)	0.009	0.696	0.000	0.042	0.000	0.705	1.333	H1-3+VT 🗸
L3	129.75 - 96.0833 (3)	0.010	0.796	0.000	0.033	0.000	0.806	1.333	H1-3+VT 🗸
L4	96.0833 - 63.25 (4)	0.012	0.909	0.000	0.033	0.000	0.921	1.333	H1-3+VT 🗸
L5	63.25 - 31.25 (5)	0.014	0.975	0.000	0.032	0.000	0.989	1.333	H1-3+VT 🗸
L6	31.25 - 0 (6)	0.017	1.067	0.000	0.031	0.000	1.084	1.333	H1-3+VT 🗸

# **Section Capacity Table**

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P <sub>allow</sub> lb	% Capacity	Pass Fail
LI	178 - 164.333	Pole	TP26x22.64x0.25	1	-4545.84	1032649.73	16.0	Pass
L2	164.333 - 129.75	Pole	TP34.0625x24.7829x0.3125	2	-11035.50	1691390.31	52.9	Pass
L3	129.75 - 96.0833	Pole	TP41.75x32.4889x0.375	3	-17928.80	2488870.86	60.5	Pass
L4	96.0833 - 63.25	Pole	TP49.0625x39.8475x0.375	4	-26054.00	2929027.44	69.1	Pass
L5	63.25 - 31.25	Pole	TP56.125x46.961x0.375	5	-35272.20	3355160.86	74.2	Pass
L6	31.25 - 0	Pole	TP62.9375x53.8477x0.375	6	-47316.60	3684958.38	81.3	Pass
							Summary	
						Pole (L6)	81.3	Pass
		gran to come to be a				RATING =	81.3	Pass

# **TOWER ELEVATION DRAWING AND FEEDLINE PLAN**

2	37.50		9		•			3684.4	164.3 ft	Pirod 16.1 PIROD 1: (4) TMBX Pipe (4) TMBX Pipe (4) TMBX Pipe (2) ETW1 (2) ETW1
E	37.50	18	0.3750	4.67				5580.8	129.8 ft	(2) ETW1 4" x 4" Md 4" Dish BXA-7000 BXA-7000 (2) LPA-8 (2) LPA-8 BXA-1710
4	37.50	81	0.3750	5.50	39.8475	49.0625	A572-65	6694.9	96.1 ft	1. Town 2. Town 3. Town 4. Defile 5. TOW
w	37.50	18	0.3750	6.25	46,9610	56.1250		5.1777	63.3 ft AXI	
9	37.50	18	0.3750		53.8477	62.9375		8811.8	31.3 ft 5478  SHEAR 25021 lb  TORQUE: 74 mph WIND - AXII. 4732  SHEAR 29853 lb	MOMEN 3283817 I 2220 lb-ft 0.5000 in ICE AL
Section	Length (ft)	Number of Sides	Thickness (in)	Socket Length (ft)	Top Dia (in)	Bot Dia (in)	Grade	Weight (lb) 33431.6	TORQUE 2 REACTIONS - 1	

#### **DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION	
Pirod 16.5' LP Platform	177.5	BXA-171085-8BF w/ 2"x48" Mountpipe	170	
PiROD 12' Lightweight T-Frame (GPD)	177.5	BXA-171085-8BF w/ 2"x48" Mountpipe	170	
(4) TMBXX-6516-R2M w/ 6' Mount	177.5	(2) FD9R6004/2C-3L	170	
Pipe		(2) FD9R6004/2C-3L	170	
(4) TMBXX-6516-R2M w/ 6' Mount Pipe	177.5	(2) FD9R6004/2C-3L	170	
		MTS 12.5' LP Platform	170	
(4) TMBXX-6516-R2M w/ 6' Mount Pipe	177.5	(2) 7770.00 w/ 6' Mount Pipe	160	
(2) ETW190VS12UB	177.5	(2) 7770.00 w/ 6' Mount Pipe	160	
(2) ETW190VS12UB	177.5	(2) 7770.00 w/ 6' Mount Pipe	160	
(2) ETW190VS12UB	177.5	(2) LGP21401	160	
4" x 4' Mount Pipe	177.5	(2) LGP21401	160	
4' Dish	177.5	(2) LGP21401	160	
BXA-70063-6CF w/ 72" Mount Pipe	170	(2) LGP21903 Diplexer	160	
BXA-70063-6CF w/ 72" Mount Pipe	170	(2) LGP21903 Diplexer	160	
BXA-70063-6CF w/ 72" Mount Pipe	170	(2) LGP21903 Diplexer	160	
(2) LPA-80080/4CF w/ 6' Mount Pipe	170	MTS 12.5' LP Platform	160	
(2) LPA-80080/4CF w/ 6' Mount Pipe	170	742 213 w/ Mount Pipe	150	
(2) LPA-80080/4CF w/ 6' Mount Pipe	170	742 213 w/ Mount Pipe	150	
BXA-171085-8BF w/ 2"x48" Mountpipe	170	742 213 w/ Mount Pipe	150	
		Valmont Light Duty Tri-Bracket (1)	150	

**MATERIAL STRENGTH** 

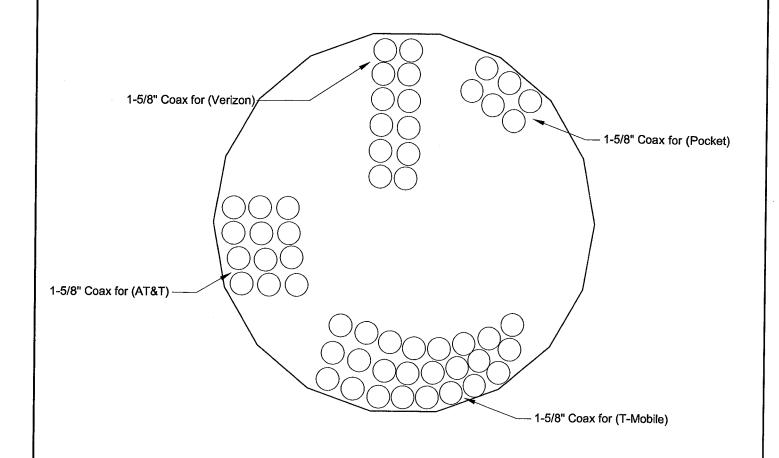
GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

#### **TOWER DESIGN NOTES**

- s located in Middlesex County, Connecticut.
  lesigned for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
  s also designed for a 74 mph basic wind with 0.50 in ice.
  ons are based upon a 50 mph wind.
  R RATING: 81.3%









NOT TO SCALE





CT11237C DEEP RIVER/RT 9

JOB NO.
2011711.11
DATE
12/13/2011
DRAWN BY
UG

### ANCHOR ROD AND BASE PLATE ANALYSIS



### Anchor Rod and Base Plate Stresses CT11237C DEEP RIVER/RT 9 2011711.11

1	Overturning Moment =	3800.40 k*	ft
201	Axial Force =	47.33 k	
8.0	Shear Force =	29.85 k	

Anchor Rods	3	
Number of Rods =	45	
Type =	Bolt	2.41
Rod Ultimate Strength (Fu) =	150	ksi
ASIF =	1.333	
Rod Circle =	68	in
Rod Diameter =	1.25	in
Area =	1.23	in <sup>2</sup>
Max Tension on Rod =	58.55	kips
Max Compression on Rod =	60.66	kips
Allow. Rod Force =	80.99	kips
Anchor Rod Capacity =	72.3%	OK

	Stiffeners	11.111	
	Configuration =	Every F	lod
	Thickness =	0.	75 in
	Width =		4 in
	Notch =		0.5 in
	Height =		12 in
	Stiffener Strength $(F_y) =$		50 ksi
			NAME OF THE PARTY
	Weld Info. Known? =		No
	Weld Info. Known? =		No
	Weld Info. Known? =		No
	Weld Info. Known? =		No 46 kips
		39.	46 kips
	Stiffener Vertical Force =	39. Not Verifië	46 kips
\	Stiffener Vertical Force = Vert. Weld Capacity =	39. Not Verifie Not Verifie	46 kips

Acceptable Stress Ratio	free bases
=	105.0%

Base Plat	е	
Location =	External	
Plate Strength $(F_y) =$	50	ksi
Outside Diameter =	71	in
Plate Thickness =	1.375	in
b =	4.00	
Le =	4.00	in
		16.45
fb =	37.41	ksi
Fb =	50	ksi
BP Capacity =	74.8%	OK

Pole		
Pole Diameter =	62.9375	in
Number of Sides =	18	
Thickness =	0.375	in
Pole Yield Strength =	65	ksi

# **FOUNDATION ANALYSIS**



### Mat Foundation Analysis CT11237C DEEP RIVER/RT 9 2011711.11

General Info		
Code	TIA/EIA-222-F (ASD)	
Bearing On	Rock	
Foundation Type	Mono Pad	
Pier Type	None	
Reinforcing Known	Yes	
Max Capacity	1.05	

Towe	Tower Reactions			
Moment, M	3800.398 k-ft			
Axial, P	47.329 k			
Shear, V	29.853 k			
Shear, V	29.853 k			

Pad & Pier G	eometry	
Pier N/A	0	ft
Pad Length, L	29	ft
Pad Width, W	29	ft
Pad Thickness, t	3.5	ft
Depth, D	3.5	ft
Height Above Grade, HG	0.5	ft

Pad & Pier Reinforcing			
Rebar Fy	60	ksi	
Concrete Fc'	4	ksi	
Clear Cover	3	in	
Reinforced Top & Bottom?	Yes		
Pad Reinforcing Size	#8		
Pad Quantity Per Layer	32		
Pier Rebar Size	#8		
Pier Quantity of Rebar			

Soil Properties			
Soil Type	Cohesive		
Soil Unit Weight	120	pcf	
Cohesion, Cu	0 ksf		
Bearing Type	Gross		
<b>Ultimate Bearing</b>	12	ksf	
Water Table Depth	Below	ft	
Frost Depth	3.333	ft	

GPD Mat Foundation Analysis - V1.01

Bearing S	Summary		Load Case
Qxmax	1.71	ksf	1D+1W
Qymax	1.71	ksf	1D+1W
Qmax @ 45°	2.26	ksf	1D+1W
Q <sub>(all) Gross</sub>	6.00	ksf	
Controlling Capacity	37.7%	Pass	

<b>Overturning Summa</b>	ry (Required	FS=1.5)	Load Case
FS(ot)x	2.04	≥1.5	1D+1W
FS(ot)y	2.04	≥1.5	1D+1W
<b>Controlling Capacity</b>	73.5%	Pass	

