



STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@ct.gov

Internet: ct.gov/csc

Daniel F. Caruso

Chairman

September 8, 2010

Thomas J. Regan, Esq.
Brown Rudnick LLP
CityPlace I, 185 Asylum Street
Hartford, CT 06103

RE: **EM-T-MOBILE-018-100819** - Omnipoint Communications, as subsidiary of T-Mobile USA, Inc., notice of intent to modify an existing telecommunications facility located at 20 Vale Road, Brookfield, Connecticut.

Dear Attorney Regan:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

- Prior to the antenna and tower mounted amplifier swap, the mast shall be replaced in accordance with the structural analysis prepared by CENTEK Engineering, Inc. dated May 24, 2010 and stamped by Carlo F. Centore, P.E.; and
- The tower shall not exceed 100 percent of its post-construction structural rating; and
- Prior to the antenna and tower mounted amplifier swap, a signed letter from a Professional Engineer duly licensed in the State of Connecticut shall be submitted to the Council to certify that the mast replacement have been properly completed and the tower does not exceed 100 percent of its post-construction structural rating.

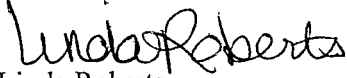
The proposed modifications are to be implemented as specified here and in your notice dated August 19, 2010, including the placement of all necessary equipment and shelters within the tower compound. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any

deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Thank you for your attention and cooperation.

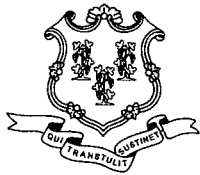
Very truly yours,



Linda Roberts
Executive Director

LR/CDM/laf

- c: The Honorable Bill Davidson, First Selectman, Town of Brookfield
- Heather Paton, Land Use Office, Town of Brookfield
- Clare Ann Walsh, Land Use Enforcement Officer, Town of Brookfield
- Daniel J. Garstka, Senior Engineer, Transmission Projects, Northeast Utilities Service Company



Daniel F. Caruso
Chairman

STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@ct.gov

Internet: ct.gov/csc

August 26, 2010

The Honorable Bill Davidson
First Selectman
Town of Brookfield
Brookfield Municipal Center
Pocono Road
P. O. Box 5106
Brookfield, CT 06804-5106

RE: **EM-T-MOBILE-018-100819** - Omnipoint Communications, as subsidiary of T-Mobile USA, Inc., notice of intent to modify an existing telecommunications facility located at 20 Vale Road, Brookfield, Connecticut.

Dear First Selectman Davidson:

The Connecticut Siting Council (Council) received this request to modify an existing telecommunications facility, pursuant to Regulations of Connecticut State Agencies Section 16-50j-72.

If you have any questions or comments regarding this proposal, please call me or inform the Council by September 9, 2010.

Thank you for your cooperation and consideration.

Very truly yours,

Melanie Bachman
Acting Executive Director

MB/jbw

Enclosure: Notice of Intent

c: Heather Paton, Land Use Office, Town of Brookfield
Clare Ann Walsh, Land Use Enforcement Officer, Town of Brookfield

CONNECT
EM-T-MOBILE-018-100819

In re:

T-Mobile USA, Inc. Notice to Make an Exempt : **EXEMPT MODIFICATION NO.** _____
Modification to an Existing Facility, :
20 Vale Road, Brookfield, Connecticut. : August 19, 2010

ORIGINAL

NOTICE OF EXEMPT MODIFICATION

RECEIVED
AUG 19 2010

Pursuant to Conn. Agencies Regs. §§ 16-50j-73 and 16-50j-72(b), T-Mobile USA, Inc. ("T-Mobile") hereby gives notice to the Connecticut Siting Council ("Council") and the Town of Brookfield of T-Mobile's intent to make an exempt modification to the existing Connecticut Light and Power ("CL&P") tower (the "Tower") located at 20 Vale Road in Brookfield, Connecticut. Specifically, T-Mobile plans to upgrade its wireless system in Connecticut by implementing its Universal Mobile Telecommunications System ("UMTS"). UMTS is a third-generation ("3G") technology that utilizes a code division multiple access ("CDMA") base to allow for fast and large data transfers. To accomplish this upgrade, T-Mobile must modify its antenna and equipment configurations at many of its existing sites.

Once the UMTS upgrade is complete, T-Mobile will operate on a more unified communication system, allowing international wireless telephones to function world-wide. Furthermore, UMTS will enhance GPS navigation capabilities and provide emergency responders with more advanced tracking capabilities. The proposed UMTS technology is compatible with the existing second-generation ("2G") Global System for Mobile Communication ("GSM") currently on the Tower and the proposed upgrade is expected to enhance the existing 2G system. In order to accomplish the upgrade at this site, T-Mobile plans to add UMTS technology and install associated equipment at the base of the Tower.

Under the Council's regulations (Conn. Agencies Regs. § 16-50j-72(b)), T-Mobile's plans do not constitute a modification subject to the Council's review because T-Mobile will not

change the height of the Tower, will not extend the boundaries of the compound, will not increase the noise levels at the site, and will not increase the total radio frequency electromagnetic radiation power density at the site to levels above applicable standards.

The Tower is a 115-foot CL&P pole (Pole No. 10247), with an existing 24' 4" pipe mast extension, located at 20 Vale Road in Brookfield, Connecticut (latitude 41° 25' 51.4" N, longitude 73° 24' 8.5" W). The Tower is owned by CL&P. Currently, T-Mobile has 6 antennas and 3 Ground Mounted Amplifiers ("GMA") located on the Tower with centerlines of 123 feet and 130 feet. A site plan with Tower specifications is attached.

T-Mobile plans to remove its 6 existing antennas and install 3 quad pole GSM antennas and 3 quad pole UMTS antennas on the Tower. T-Mobile also proposes to remove its existing GMAs and install 6 new Tower Mounted Amplifiers ("TMA"). The 6 new TMAs will include 3 PCS TMA and 3 AWS TMA. The proposed antennas and TMAs will have the same centerlines as the existing installation – 123 feet and 130 feet. In order to support its proposed modifications T-Mobile also plans to replace the existing 24' 4" (approximately) pipe mast extension with a new 27' 6" (approximately) pipe mast extension. T-Mobile commissioned Centek Engineering to perform a structural analysis of the Tower (attached). According to the Structural Analysis Report, dated May 24, 2010, the "utility pole with the proposed mast replacement is adequate to support the proposed T-Mobile equipment upgrade" (Section 1-5, Structural Analysis Report).

In addition, T-Mobile plans to route six new 1-1/4 inch coax cables along the outside of the Tower. T-Mobile proposes to install its UMTS equipment cabinet on the existing 12' by 13' (approximately) concrete pad. Hence, no increase in the boundaries of the site is necessary.

Therefore, excluding brief, minor, construction-related noise during the addition of the antennas and the installation of the equipment cabinet, T-Mobile's changes to the Tower will not increase noise levels at the site.

The proposed antennas and TMAs will not adversely impact the health and safety of the surrounding community or the people working on the Tower. The total radio frequency exposure measured around the Tower will be well below the National Council on Radiation Protection and Measurements' ("NCRP") standard adopted by the Federal Communications Commission ("FCC"). The worst-case power density analysis measured at the base of the Tower indicates that T-Mobile's antennas will emit 6.5016% of the NCRP's standard for maximum permissible exposure. A cumulative power density analysis indicates that together, all of the antennas on the Tower will emit only 20.9516% of the NCRP's standard for maximum permissible exposure. Therefore, the power density levels will be well below the FCC mandated radio frequency exposure limits in all locations around the Tower, even with extremely conservative assumptions. The power density analysis is attached.

In conclusion, T-Mobile's proposed plan to remove and replace antennas, add TMAs and add equipment at this site does not constitute a modification subject to the Council's jurisdiction because T-Mobile will not increase the height of the Tower, will not extend the boundaries of the site, will not increase the noise levels at the site, and the total radio frequency electromagnetic radiation power density will stay within all applicable standards. *See Conn. Agencies Regs. § 16-50j-72.*

T-Mobile USA, Inc.

By: Thomas J. Regan

Thomas J. Regan
Brown Rudnick LLP
185 Asylum Street, CityPlace I
Hartford, CT 06103-3402
Email - tregan@brownrudnick.com
Phone - 860.509.6522 /Fax - 860.509.6501

Certificate of Service

This is to certify that on this 19th day of August, 2010, the foregoing Notice of Exempt Modification was sent, via first class mail, to the following:

Town of Brookfield
First Selectman Bill Davidson
Town Hall – Room 201
100 Pocono Road
Brookfield, CT 06804

By: Thomas J. Regan
Thomas J. Regan

40274264 v1 - 025064/0016

T-MOBILE NORTHEAST, LLC

Wireless Communications Facility

CL&P BROOKFIELD / CT11201A

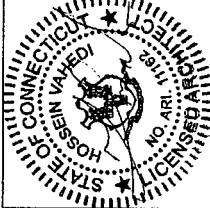
POLE #10247

20 VALE ROAD

BROOKFIELD, CT 06804

TRANSCEND WIRELESS, LLC
 1480 Centre Street, Suite 200
 Hartford, CT 06105
 Office: 817-865-0789
 Fax: 817-865-0522

ATLANTIS GROUP
 1480 Centre Street, Suite 200
 Hartford, CT 06105
 Office: 817-865-0789
 Fax: 817-865-0522



APPROVALS
 LANDLORD _____
 LEASING _____
 R.F. _____
 ZONING _____
 CONSTRUCTION _____
 A/E _____

DRAWN BY: S.B.
 CHECKED BY: S.M.
SUBMITTALS
 A 04/19/10 ISSUED FOR REVIEW
 0 04/22/10 REVISED PER COMMENTS
 1 06/19/10 REVISED PER COMMENTS

THIS DOCUMENT IS THE CREATION OF TRANSCEND WIRELESS, LLC AND IS THE PROPERTY OF TRANSCEND WIRELESS, LLC. IT IS TO BE USED ONLY FOR THE PURPOSES OF OBTAINING PERMITS AND REGULATORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.

CT11201A
CL&P BROOKFIELD
 20 VALE ROAD
 BROOKFIELD, CT 06804

SHEET TITLE:
TITLE SHEET

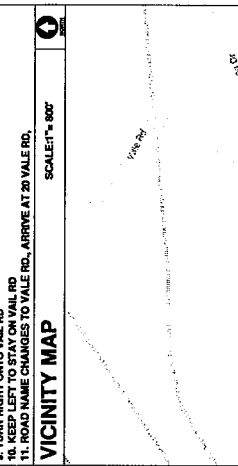
SHEET NUMBER:
01

PROJECT SUMMARY
 SITE NAME: CL&P BROOKFIELD
 SITE ADDRESS: 20 VALE ROAD, BROOKFIELD, CT 06804
 SITE LOCATION: LATITUDE N 41° 25' 51.4" LONGITUDE W 73° 24' 04.5" GROUND E.L. 386.1 (A.M.S.L.)
 PROPERTY OWNER: BERKSHIRE NORTH, LLC
 APPLICANT: TRANSCEND WIRELESS, LLC
 LESSOR/LEASEE: TRANSCEND WIRELESS, LLC
 PROJECT OWNER: 35 GRIFFIN ROAD SOUTH, BLOOMFIELD, CT 06002 (860) 862-7100
 CONTACT: LISA LIN MEDERMEIER, TRANSCEND WIRELESS, LLC, 203) 275-8689
 ASSESSOR'S PARCEL NO.: MAP D18 - LOT 001
 ZONING DISTRICT: I-UR

SHEET INDEX
 SHT. NO. | DESCRIPTION
 01 | TITLE SHEET
 02 | COMPOUND PLAN, NOTES AND DETAILS
 03 | ELEVATION
 04 | EQUIPMENT DETAILS
 05 | ANTENNA DETAILS
 06 | UTILITY ROUTING PLAN
 07 | GROUNDING NOTES, RISERS AND DETAILS
 08 | ELECTRICAL SPECIFICATIONS AND NOTES
 09 | GROUNDING NOTES

GENERAL NOTES
 THE PROPOSED SCOPE OF WORK GENERALLY INCLUDES THE FOLLOWING:
 (1) NEW ERICSSON RIS 3108 UNITS CABINET ON EXISTING CONCRETE PAD WITHIN FENCED COMPOUND.
 A TOTAL OF SIX (6) DIRECTIONAL PANEL ANTENNAS ARE TO BE MOUNTED ON AN EXISTING 17'-4" TOWER EXTENSION MAST ATTACHED TO THE 115' TALL TRANSMISSION LINE TOWER (UTILITY POLE DESIGN BY OTHERS) AT RAD CENTER ELEVATIONS OF 130' AND 123' ABOVE THE TOWER BASE PLATE. SIX (6) NEW CUMMULUS ERICSSON RIS 3108 UNITS CABINET ON EXISTING UTILITY POLE. HOLES OR WELDING SHALL BE MADE TO EXISTING UTILITY POLE. PROPOSED ELECTRICAL AND TEGOS SERVICE WILL BE ROUTED ABOVE GRADE FROM THE TOWER TO THE RISERS. AN ENCLOSURE TO PROPOSED RIS 3108 UNITS CABINET LOCATED WITHIN THE FENCED COMPOUND.

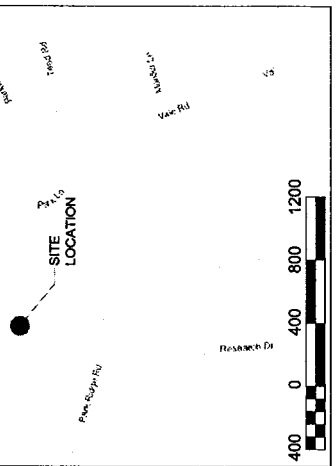
DIRECTIONS: FROM: 35 GRIFFIN DR., BLOOMFIELD, CT TO: 20 VALE ROAD, BROOKFIELD, CT
 1. DEPART GRIFFIN DRIVE SOUTH
 2. TURN RIGHT ONTO SP-107 SOUTH / BLUE HILLS AVE EXIT
 3. KEEP LEFT ONTO DAY HILL RD
 4. TAKE RAMP RIGHT FOR I-91 SOUTH / RICHARD P HORAN MEMORIAL HWY TOWARD HARTFORD COURTYARD BY MARRIOTT ON THE CORNER
 5. TAKE RAMP LEFT FOR SP-25 WEST / US-6 WEST TOWARD WATERBURY
 6. AT EXIT 9, TAKE RAMP RIGHT FOR SP-25 TOWARD BROOKFIELD
 7. TURN LEFT ONTO SP-27 / HAWLEYVILLE RD
 8. TURN RIGHT ONTO US-21 / PIT PLEASANT RD
 9. TURN LEFT ONTO VALE RD
 10. ROAD NAME CHANGES TO VALE RD., ARRIVE AT 20 VALE RD.
 11. KEEP LEFT TO STAY ON VALE RD.



GENERAL NOTES
 1. ALL WORK SHALL BE IN ACCORDANCE WITH THE 2005 INTERNATIONAL BUILDING CODE AS AMENDED AND THE 2005 INTERNATIONAL STANDARDS FOR STEEL ANTENNA TOWERS AND SUPPORTING STRUCTURES.
 2. CONTRACTOR SHALL REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. CONTRACTOR SHALL COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS FOR ALL SUBCONTRACTORS AND ALL RELATED PARTIES. THE SUBCONTRACTORS SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT AFFECTS THEIR WORK.
 3. CONTRACTOR SHALL PROVIDE A COMPLETE BUILD-OUT WITH ALL FINISHES, STRUCTURAL, MECHANICAL, AND ELECTRICAL COMPONENTS AND PROVIDE ALL ITEMS AS SHOWN OR INDICATED ON THE DRAWINGS OR IN THE WRITTEN SPECIFICATIONS.
 4. CONTRACTOR SHALL FURNISH ALL MATERIAL, LABOR AND EQUIPMENT TO COMPLETE THE WORK AND FURNISH A COMPLETED JOB ALL IN ACCORDANCE WITH LOCAL AND STATE GOVERNING AUTHORITIES AND OTHER AUTHORITIES HAVING LAWFUL JURISDICTION OVER THE WORK.
 5. CONTRACTOR SHALL SECURE AND PAY FOR ALL PERMITS AND ALL INSPECTIONS REQUIRED AND SHALL ALSO PAY FEES REQUIRED FOR THE CONSTRUCTION, PLUMBING, ELECTRICAL AND HVAC. PERMITS SHALL BE PAID FOR BY THE RESPECTIVE SUBCONTRACTORS.
 6. CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS ON SITE AT ALL TIMES AND INSURE DISTRIBUTION OF NEW DRAWINGS TO ALL SUBCONTRACTORS. ALL OLD DRAWINGS SHALL BE MARKED VOID AS THEY ARE MADE AVAILABLE. ALL OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONTRACT AREA. THE CONTRACTOR SHALL FURNISH AN "AS-BUILT" SET OF DRAWINGS TO OWNER UPON COMPLETION OF PROJECT.
 7. LOCATION OF EQUIPMENT AND WORK SUPPLIED BY OTHERS THAT IS DIAGRAMMATICALLY INDICATED ON THE DRAWINGS SHALL BE DETERMINED BY THE CONTRACTOR. THE CONTRACTOR SHALL DETERMINE LOCATIONS AND DIMENSIONS SUBJECT TO STRUCTURAL CONDITIONS AND WORK OF THE SUBCONTRACTORS.
 8. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITIONS OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY.
 9. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCE, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE WITH SUCH ORDINANCE, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
 10. ALL UTILITY WORK SHALL BE IN ACCORDANCE WITH LOCAL UTILITY COMPANY REQUIREMENTS AND SPECIFICATIONS.

11. ALL EQUIPMENT PRODUCTS PURCHASED ARE TO BE REVIEWED BY THE CONTRACTOR FOR CONFORMANCE WITH THE SPECIFICATIONS. CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST TO OWNER OR CONSTRUCTION MANAGER.
 12. ANY AND ALL ERRORS, DISCREPANCIES, AND MISSED ITEMS ARE TO BE CORRECTED BY THE CONTRACTOR. ALL THESE ITEMS ARE TO BE INCLUDED IN THE BID. NO "EXTRA" WILL BE ALLOWED FOR MISSED ITEMS.
 13. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ON-SITE SAFETY FROM THE TIME THE JOB IS AWARDED UNTIL ALL WORK IS COMPLETE AND ACCEPTED BY THE OWNER.
 14. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO OWNER AND ARCHITECT. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE CONSTRUCTION MANAGER FOR REVIEW.
 15. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLE, AND EXISTING CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA.
 16. COORDINATION, LAYOUT, FURNISHING AND INSTALLATION OF CONDUIT AND ALL APPURTENANCES REQUIRED FOR PROPER INSTALLATION OF ELECTRICAL AND TELECOMMUNICATION SERVICE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
 17. ALL DAMAGES CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
 18. THE CONTRACTOR SHALL CONTACT "CALL BEFORE YOU DIG" AT LEAST 48 HOURS PRIOR TO ANY EXCAVATIONS AT 1-800-422-4465. ALL UTILITIES SHALL BE PROTECT MARKED UTILITIES THROUGHOUT PROJECT COMPLETION.
 19. CONTRACTOR SHALL COMPLY WITH OWNERS ENVIRONMENTAL ENGINEER ON ALL METHODS AND PROVISIONS FOR ALL EXCAVATION ACTIVITIES INCLUDING BACKFILL. ALL BACKFILL MATERIALS TO BE PROVIDED BY THE CONTRACTOR.

PROJECT SUMMARY
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 SITE ADDRESS: 20 VALE ROAD, BROOKFIELD, CT 06804
 SITE LOCATION: LATITUDE N 41° 25' 51.4" LONGITUDE W 73° 24' 04.5" GROUND E.L. 386.1 (A.M.S.L.)
 PROPERTY OWNER: BERKSHIRE NORTH, LLC
 APPLICANT: TRANSCEND WIRELESS, LLC
 LESSOR/LEASEE: TRANSCEND WIRELESS, LLC
 PROJECT OWNER: 35 GRIFFIN ROAD SOUTH, BLOOMFIELD, CT 06002 (860) 862-7100
 CONTACT: LISA LIN MEDERMEIER, TRANSCEND WIRELESS, LLC, 203) 275-8689
 ASSESSOR'S PARCEL NO.: MAP D18 - LOT 001
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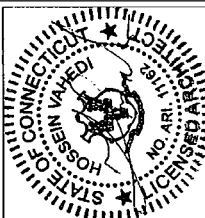
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STRUCTURAL NOTES

- DESIGN REQUIREMENTS ARE PER STATE BUILDING CODE AND ALL OTHER APPLICABLE CODES. ALL STRUCTURAL STEEL SHALL BE FABRICATED IN ACCORDANCE WITH THE AISC "MANUAL OF STEEL CONSTRUCTION" 13TH EDITION.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ERECTION OF ANY MATERIAL. ANY UNUSUAL CONDITIONS SHALL BE REPORTED TO THE ATTENTION OF THE CONSTRUCTION MANAGER.
- DESIGN AND CONSTRUCTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A572 GRADE 50 STRUCTURAL STEEL UNLESS OTHERWISE INDICATED.
- STEEL PIPE SHALL CONFORM TO ASTM A500 COLD-FORMED WELDED & SEAMLESS CARBON STEEL STRUCTURAL TUBING, GRADE A OR ASTM A53 PIPE STRUCTURAL BLACK AND HOT-DIPPED ZINC-COATED WELDED STEEL PIPE. ALL DIMENSIONS SHALL BE AS INDICATED ARE NOMINAL ACTUAL OUTSIDE DIAMETER IS LARGER.
- STRUCTURAL CONNECTION BOLTS SHALL BE HIGH STRENGTH BOLT TO CONFORM TO ASTM A325 HIGH STRENGTH BOLTS FOR STRUCTURAL JOINTS, INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS. ALL BOLTS SHALL BE 5/8" DIA UNLESS OTHERWISE NOTED.
- ALL STEEL MATERIAL SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS", UNLESS OTHERWISE NOTED.
- ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC-COATING (HOT-DIP) ON IRON AND STEEL HARDWARE", UNLESS OTHERWISE NOTED.
- FIELD WELDS, DRILL HOLES, SAW CUTS AND ALL DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED WITH AN ORGANIC ZINC REPAIR PAINT COMPLYING WITH THE REQUIREMENTS OF THE MANUFACTURER. REPAIR PAINT SHALL HAVE 45 PERCENT ZINC BY WEIGHT. ZPP BY DUCAN GALVANIZING, GALVA BRIGHT PREMIUM BY CROWN OR EQUAL THICKNESS OF APPLIED GALVANIZING REPAIR PAINT SHALL BE NOT DRY TO DRY BETWEEN COATS WITH A RESULTING COATING THICKNESS REQUIRED BY ASTM A123 OR A153 AS APPLICABLE.
- CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES, APPEARANCE AND QUALITY OF WELDS AND METHODS USED IN CORRECTING WELDING. ALL WELDERS AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD FOR QUALIFICATION OF WELDERS AND WELDING OPERATORS" AND SHALL BE DONE USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AWS D1.1. WHERE FLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE 2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION" 13TH EDITION.
- NO WELDING, DRILLING, TAPPING OR CUTTING INTO CLIP OR WMECO STRUCTURES.
- INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE UNSUITABLE MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE CONSTRUCTION MANAGER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. SUCH ACTION SHALL REQUIRE CONSTRUCTION MANAGER APPROVAL.
- UNSTRUTS SHALL BE FORMED STEEL CHANNEL STRUT FRAMING AS MANUFACTURED BY UNISTRUT CORP. UNLESS OTHERWISE NOTED, AND SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION.
- EPOXY ANCHOR ASSEMBLY SHALL CONSIST OF 1/4" WASHERS, AN INTERNALLY THREADED INSERT, A SCREEN TUBE AND AN EPOXY ADHESIVE. THE ANCHORING SYSTEM SHALL BE THE HILTI-HY HY-20 AND OR HY-150 SYSTEMS (AS SPECIFIED ON DWG.) OR ENGINEERS APPROVED EQUAL WITH 4-IN MIN. EMBEDMENT DEPTH.
- EXPANSION BOLTS SHALL CONFORM TO FEDERAL SPECIFICATION FF-S-325, GROUP II, TYPE 4, CLASS II, HLTI KWK BOLTS FOR APPROVED EQUAL. MINIMUM EMBEDMENT SHALL BE AS SPECIFIED ON DWG. MINIMUM EMBEDMENT SHALL BE THREE AND ONE HALF (3 1/2) INCHES.

TRANSCEND WIRELESS, LLC
 100 CENTRAL AVE
 SUITE 200
 OFFICE CITY MASSACHUSETTS
 MA 02128

T-MOBILE NORTHEAST, LLC
 1340 Centre Street Suite 203
 Westport, MA 02886
 Tel: 617-853-9000
 Fax: 617-853-9033



APPROVALS

LANDLORD	_____
LEASING	_____
R.F.	_____
ZONING	_____
CONSTRUCTION	_____
A/E	_____

DRAWN BY: S.R.
 CHECKED BY: SM

SUBMITTALS

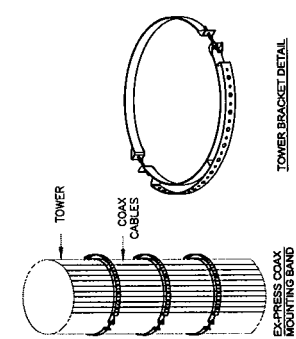
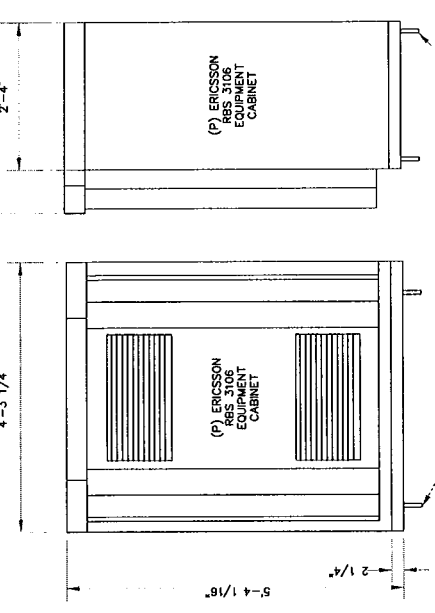
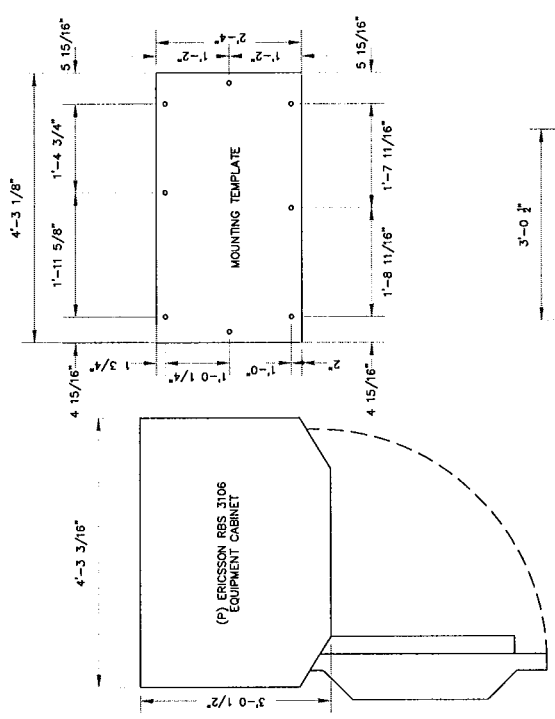
A	04/19/10	ISSUED FOR REVIEW
0	04/22/10	REVISED PER COMMENTS
1	06/19/10	REVISED PER COMMENTS

THIS DOCUMENT IS THE CREATION OF TRANSCEND WIRELESS, LLC AND IS NOT TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, WITHOUT THE EXPRESS WRITTEN PERMISSION OF TRANSCEND WIRELESS, LLC. THIS DOCUMENT IS THE PROPERTY OF TRANSCEND WIRELESS, LLC AND IS TO BE USED ONLY FOR THE PROJECT AND PURPOSES SPECIFICALLY ALLOWED.

CT11201A
CL&P BROOKFIELD
 20 VALE ROAD
 BROOKFIELD, CT 06804

SHEET TITLE:
EQUIPMENT DETAILS

SHEET NUMBER:
04



SECURE (P) ERICSSON RBS 3106 EQUIPMENT CABINET TO (E) CONCRETE WITH (B) 5/8" x HILTI STAINLESS STEEL KWK BOLTS (3 1/2" MIN. EMBEDMENT). (SHIM IF REQUIRED TO LEVEL).

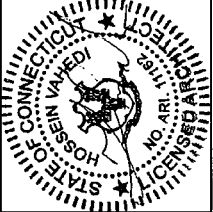
ERICSSON RBS 3106 EQUIPMENT CABINET 1
 SCALE: 1/2" = 1'-0" (11x17)
 SCALE: 1" = 1'-0" (24x36)

COAX MOUNTING DETAIL 2
 N.T.S.

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 FAX: (860) 459-1235

T-MOBILE NORTHEAST, LLC
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LEASING	
R.F.	
ZONING	
CONSTRUCTION	
A/E	

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CHECKED BY: SM

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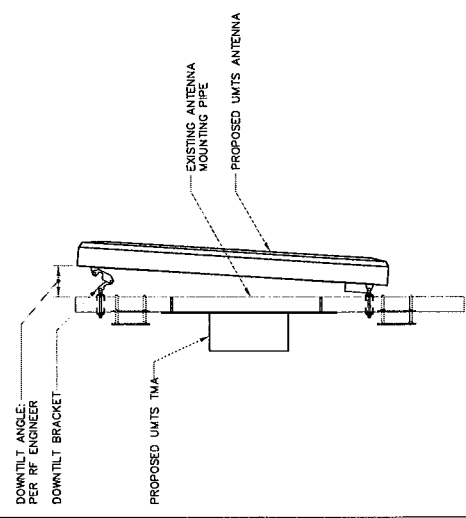
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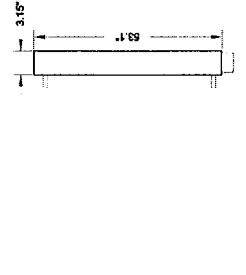
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ANTENNA DETAILS

SHEET NUMBER:
05



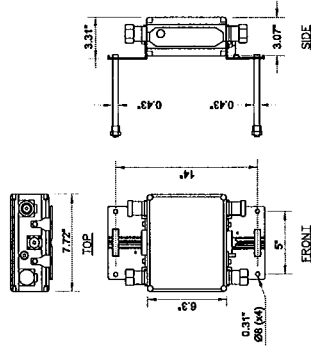
MOUNTING ASSEMBLY (ALL ANTENNAS)

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DUAL POLE ANTENNA DETAIL
 N.T.S.

3 05



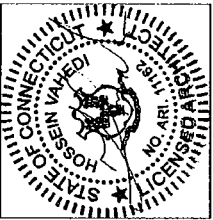
TWIN TMA DETAIL
 N.T.S.

4 05

TRANSCEND WIRELESS, LLC
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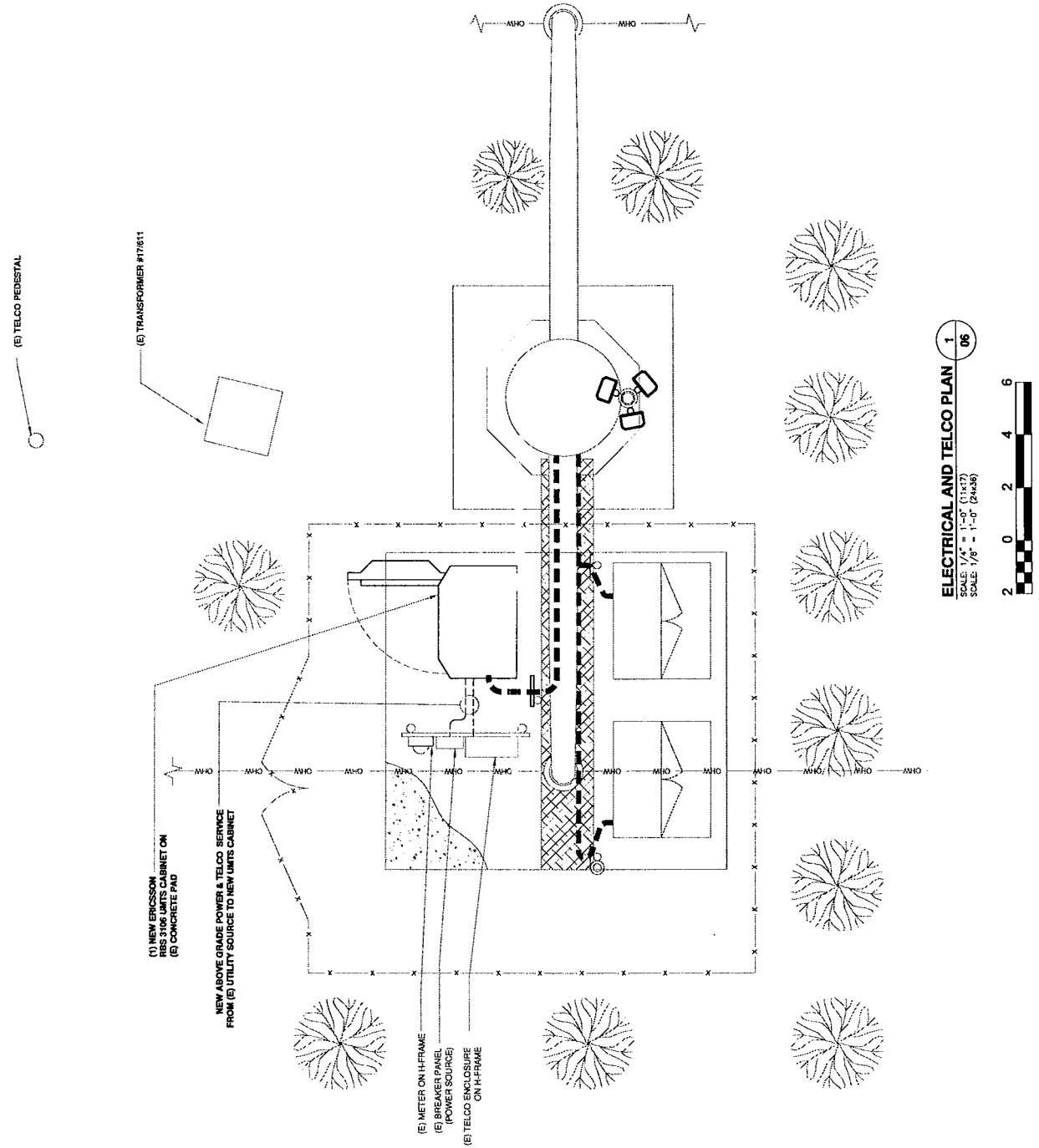
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SHEET TITLE:
UTILITY ROUTING PLAN

SHEET NUMBER:
06



ELECTRICAL AND TELCO PLAN 1/06
 SCALE: 1/4" = 1'-0" (11x17)
 SCALE: 1/8" = 1'-0" (24x36)



NORTHEAST UTILITIES - TOWER GROUNDING SYSTEM NOTES:

GENERAL:

1. T-MOBILE, NORTHEAST, LLC WILL FURNISH THE WIRE, CONNECTORS, AND MISCELLANEOUS MATERIAL ASSOCIATED WITH THE COUNTERPOISE GROUNDING SYSTEM.
2. THE CONTRACTOR SHALL FURNISH ALL LABOR, MATERIALS AND EQUIPMENT NECESSARY TO INSTALL THE GROUNDING SYSTEM AND TO REHABILITATE THE RIGHT-OF-WAY AS CLOSE AS POSSIBLE TO ITS ORIGINAL CONDITION.
3. THE CONTRACTOR SHALL HANDLE AND TRANSPORT THE OWNER SUPPLIED MATERIAL FROM THE OWNERS STORAGE AREA TO THE JOB SITE AND SHALL RETURN SURPLUS MATERIAL AND EMPTY REELS TO DESIGNATED STORAGE AREAS AND YARDS UPON COMPLETION OF THE CONTRACT.
4. NORTHEAST UTILITIES WILL BE RESPONSIBLE FOR PERFORMING TEST FOR SURGE IMPEDANCE AND WAVE IMPEDANCE.

INSTALLATION:

1. UNLESS OTHERWISE DIRECTED BY THE OWNERS REPRESENTATIVE, COUNTERPOISE SHALL BE BURIED A MINIMUM OF 36" IN CLEARANCE FROM ANY EXISTING UTILITY. IN WOODED OR OTHER AREAS, IN DIVERTED AREAS OR WHERE OBSTRUCTIONS ARE ENCOUNTERED, THE CONTRACTOR SHALL BE RESPONSIBLE FOR IDENTIFYING AND MARKING ALL EXISTING UTILITIES. ALL INSTALLATIONS SHALL INCLUDE CONNECTIONS TO EXISTING OR PROPOSED STRUCTURES, AND SUCH CONNECTIONS SHALL BE MADE BELOW GROUND USING BOLTED PARALLEL GROVE CONNECTORS.
 2. WHERE MULTIPLE STRUCTURE GROUNDS EXIST AT MULTI POLE STRUCTURES, THEY SHALL BE CONNECTED TOGETHER WITH BURIED COPPERWELD WIRE, BUT ONLY IF SUCH GROUNDS HAVE THE SAME TYPE AND SIZE OF WIRE. ALL CONNECTIONS SHALL BE MADE ONLY TO THE POLE GROUNDS, AND THE ANCHOR RODS SHALL BE MADE ONLY TO THE ANCHOR RODS. AT WOOD POLE STRUCTURES WHERE NO SUCH POLES EXIST, THE CONTRACTOR SHALL BE RESPONSIBLE FOR IDENTIFYING AND MARKING ALL EXISTING COUNTERPOISE CONNECTIONS SHALL BE MADE TO THE POLE TOP OUTS.
 3. FOR SINGLE CONTINUOUS (TYPE A) AND SINGLE BROKEN (TYPE B) COUNTERPOISE, THE WIRE SHALL IN GENERAL BE LAYED AT THE CENTER OF THE TRANSMISSION LINE. FOR DOUBLE CONTINUOUS (TYPE C) AND DOUBLE BROKEN (TYPE D) COUNTERPOISE, THE WIRE SHALL IN GENERAL BE LAYED UNDER THE OUTSIDE PHASE WIRES OF THE TRANSMISSION LINE. WHERE THE WIRE IS INSTALLED ACROSS BROOKS, RIVERS, HIGHWAYS, RAILROADS, OR THE VICINITY OF TELEPHONE CABLES OR PRELINES.
 4. AT STEEL POLE STRUCTURES, A BURIED GRADING RING AND SPOKES SHALL ALSO BE INSTALLED AROUND THE STRUCTURE UNLESS THE STRUCTURE HAS A PAD PER FOUNDATION PLACES TO EACH RING, AND COPPERWELD SPOKES SHALL BE CONNECTED AT TWO STRUTS PER FOUNDATION.
 5. AT WOOD POLE STRUCTURES, AN 8' LENGTH OF PLASTIC MOLING SHALL BE STAPLED OVER THE BOTTOM WITH 6" DOWNLEAD.
- GROUND ROADS:
1. WHERE GROUND ROADS ARE REQUIRED, THEY SHALL BE SINGLE OR SECTIONAL WITH THE LENGTH SPECIFIED. THEY SHALL BE INSTALLED AT A DEPTH OF 12" BELOW THE FINISHED SURFACE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR IDENTIFYING AND MARKING ALL EXISTING UTILITIES. ALL INSTALLATIONS SHALL BE CONNECTED TO COUNTERPOISE OR TO POLE GROUNDS USING BOLTED CONNECTORS.
- REHABILITATION:
1. SELECTIVE CLEARING PROCEDURES WERE USED IN THE DEVELOPMENT OF THE RIGHT-OF-WAY, AND GROWTH OF SELECTED SPECIES HAS BEEN SAVED. THE CONTRACTOR SHALL NOT REMOVE ANY OF THESE SPECIES UNLESS THEY ARE DAMAGED OR DESTROYED. THE CONTRACTOR SHALL REVIEW THE ROUTING OF EACH ROAD PRIOR TO THE EXECUTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR IDENTIFYING AND MARKING ALL EXISTING UTILITIES. ALL INSTALLATIONS SHALL BE MADE TO THE OWNER FOR DAMAGES TO THE RIGHT-OF-WAY IN OTHER THAN THE FIELD SPECIFIED LOCATIONS.
 2. ANY BRUSH ALONG THE FIELD SPECIFIED COUNTERPOISE LOCATIONS WHICH IS LEFT IN AN UNSIGHTLY CONDITION BY THE INSTALLATION WORK WILL BE CUT TO THE GROUND BY THE CONTRACTOR AND LEFT IN SMALL, NEAT PILES IN PLACE WHERE CUT.
 3. IN LOCATION WHERE EXCAVATION FOR THE INSTALLATION OF COUNTERPOISE BRINGS TO THE SURFACE ANY SMALL BOULDERS, THEY WILL BE REMOVED OR GRAVEL ON DISPERSED ON THE RIGHT-OF-WAY AS THE OWNERS REPRESENTATIVE MAY DIRECT. INSTALLED COUNTERPOISE SHALL NOT RESULT IN A PATH OF SMALL BOULDERS ON THE FINISHED SURFACE.
 4. THE OWNER ANTICIPATES THAT SEASONAL CONDITIONS MAY NOT ALLOW PERMANENT REHABILITATION OF WORK SITES AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REHABILITATION OF THE COUNTERPOISE, WHERE TEMPORARY REHABILITATION HAS BEEN COMPLETED IN ADVERSE SEASON. THE CONTRACTOR SHALL TAKE THE FOLLOWING STEPS:
 - A. WATERBARS WILL BE CONSTRUCTED ON ACCESS ROADS AND TRENCH LINES TO SHUNT DISTURBED SURFACE.
 - B. ALL DISTURBED SURFACES OF FOUNDATION SITES OR LONG TRENCH LINES OR ACCESS ROADS WILL BE COVERED WITH HAY/MULCH. SUCH DISTURBED SURFACES ON SLOPES GREATER THAN ONE (VERTICAL) ON FOUR (HORIZONTAL) SHALL BE COVERED WITH WOOD CHIPS.
 5. AS DRIVING CONDITIONS PERMIT IN THE SPRING, FOLLOWING COMPLETION OF THE INSTALLATION OF COUNTERPOISE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REHABILITATION OF ALL DISTURBED OR ERODED SURFACES SHALL BE ACCOMPLISHED AS FOLLOWS:

- A. LAWNS, GOLF COURSES, CENTERS, AND OTHER SIMILAR OCCUPANCIES SHALL BE LOAMED, GRADED, FERTILIZED, SEEDED AND WATERED (AS REQUIRED), TO ESTABLISH A REHABILITATION CONSISTENT WITH THE USE ESTABLISHED BY THE OCCUPANT.
- B. GARDENS, OTHER CULTIVATED AREAS AND PLANTINGS, SHALL BE GRADED AND TOPSOILED TO RESTORE THE DEPTH OF FERTILE SOIL TO ORIGINAL CONDITION. WHERE APPROPRIATE, SEEDING SHALL BE DONE IN ACCORDANCE WITH STEP C BELOW.
- C. THE CONTRACTOR SHALL SEED ALL DISTURBED AREAS ALONG THE NEW COUNTERPOISE ROUTES. SEED SHALL BE SPREAD AT THE RATE OF 100 LBS. PER ACRE AND SHALL BE AS FOLLOWS OR APPROVED EQUAL:

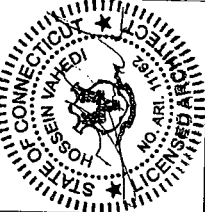
CREEPING RED FESCUE	% BY WEIGHT	% BY GERMINATION	% BY PURITY
DOMESTIC RYE	20	80	98
KENTUCKY TALL FESCUE	50	80	98

- D. ALL OTHER DISTURBED AREAS INCLUDING FOUNDATION SITES, ACCESS ROADS, AND REPAIR OF EROSION OF UTILITIES SHALL BE REHABILITATED IN ACCORDANCE WITH THE SPECIFICATION ABOVE. IN REMOTE AREAS, A CONSERVATION MIX AS LISTED BY THE CONNECTICUT STATE PARKS AND FOREST COMMISSION MAY BE SUBSTITUTED. ALL DISTURBED AREAS SHALL BE REHABILITATED TO ORIGINAL CONDITION. ALL EROSION DAMAGE AND ALL SLOPES OVER ONE (VERTICAL) AND FOUR (HORIZONTAL) WHERE TEMPORARY REHABILITATION WORK HAS BEEN DONE SHALL BE REHABILITATED.
6. IT IS IMPERATIVE THAT PERMANENT REHABILITATION BE ACCOMPLISHED IN GOOD TIME, WHICH WORK BE COMPLETED PRIOR TO THE NEXT FALL, AND UNDISTURBED USE OF THE SITE SUCCEEDING SEASON, AND TO PREVENT UNNECESSARY AND UNREASONABLE SPREADING OF CONTINUATION OF DISTURBED SURFACES.
 7. ANY BRUSH ALONG THE ACCESS ROADS WHICH IS LEFT IN AN UNSIGHTLY CONDITION BY THE WORK WILL BE CUT TO THE GROUND BY THE CONTRACTOR AND LEFT IN SMALL NEAT PILES IN PLACE WHERE CUT.

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 LEASING _____
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 ZONING _____
 CONSTRUCTION _____
 A/E _____

DRAWN BY: S.B.
 CHECKED BY: SM

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 BROOKFIELD, CT 06804

SHEET TITLE:
GROUNDING NOTES

SHEET NUMBER:
09

CEN TEK engineering

Centered on Solutions™

**Structural Analysis of PCS
Structure and CL&P Pole**

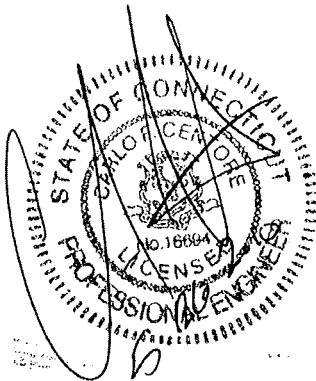
T-Mobile: CT11-201A

*CL&P Structure No. 10247
115' Electric Transmission Pole*

*20 Vale Road
Brookfield, CT*

CEN TEK Project No. 10021.CO10

*~~Date: May 12, 2010~~
Rev 1: May 24, 2010*



Prepared for:
*Transcend Wireless, LLC
113 N Mountain Blvd.
Mountain Top, PA 18707*

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Introduction

The purpose of this report is to analyze the existing 24'-4" long PCS mast and 115' CL&P pole located at 20 Vale Road in Brookfield, CT for the proposed antenna and equipment upgrade by T-Mobile.

The proposed loads consist of the following:

- **T-MOBILE (Existing to be removed):**
Antennas: Six (6) FR90-16-00DP panel antennas and one (1) TMA mounted in two (2) clusters on a PCS mast with RAD center elevations of 123-ft and 130-ft above tower base plate.
- **T-MOBILE (Existing to remain):**
Coax Cables: Twelve (12) 1-5/8" \varnothing coax cables running on the outside of the tower as indicated in section 4 of this report.
- **T-MOBILE (Proposed):**
Antennas: Three (3) RFS APXV18-206516S-C-A20 panel antennas and three (3) TMAs mounted on a PCS mast with a RAD center elevation of 123-ft above tower base plate. Three (3) RFS APX16DWV-16DWVS-E-A20 panel antennas and three (3) TMAs mounted on a PCS mast with a RAD center elevation of 130-ft above tower base plate.
Coax Cables: Six (6) 1-5/8" \varnothing coax cables running on the outside of the tower as indicated in section 4 of this report.

Primary assumptions used in the analysis

- Allowable steel stresses are defined by AISC-ASD 9th edition for design of the PCS Mast and antenna supporting elements.
- ASCE Manual No. 72, "Design of Steel Transmission Pole Structures Second Edition", defines allowable steel stresses for evaluation of the CL&P utility pole.
- All utility pole members are adequately protected to prevent corrosion of steel members.
- All proposed antenna mounts are modeled as listed above.
- All coaxial cable will be installed within the pipe mast unless specified otherwise.
- Pipe mast will be properly installed and maintained.
- No residual stresses exist due to incorrect pole erection.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds conform to the requirements of AWS D1.1.
- All proposed pipe mast members will be as specified in the construction documents to be prepared by CEN TEK engineering, Inc.
- Pipe mast and utility pole will be in plumb condition.
- Utility pole was properly installed and maintained and all members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
- Any deviation from the analyzed loading will require a new analysis for verification of structural adequacy.

A n a l y s i s

Structural analysis of the existing *PCS Mast Structure* was independently completed using the current version of RISA-3D computer program licensed to CEN TEK Engineering, Inc.

The existing mast consisting of a 3-in x 24-ft-4-in long SCH. 40 pipe (O.D. = 3.5") connected at two points to the existing tower was analyzed for its ability to resist loads prescribed by the TIA/EIA standard. Section 5 of this report details these gravity and lateral wind loads. NESC prescribed loads were also applied to the mast structure in order to obtain reactions needed for analyzing the CL&P pole structure. These loads are developed in Section 7 of this report. Load cases and combinations used in RISA-3D for TIA/EIA loading and for NESC/NU loading are listed in report Sections 6 and 8, respectively.

An envelope solution was first made to determine maximum and minimum forces, stresses, and deflections to confirm the selected section as adequate. Additional analyses were then made to determine the NESC forces to be applied to the CL&P pole structure.

The RISA-3D program contains a library of all AISC shapes and corresponding section properties are computed and applied directly within the program. The program's Steel Code Check option was also utilized. The forces calculated in RISA-3D using NESC guidelines were then applied to the CL&P pole using PLS-Pole. Maximum usage for the pole was calculated considering the additional forces from the mast and associated appurtenances.

D e s i g n B a s i s

Our analysis was performed in accordance with EIA-222-F-1996, ASCE Manual No. 72 – "Design of Steel Transmission Pole Structures Second Edition", NESC C2-2007 and Northeast Utilities Design Criteria.

The CL&P pole structure, considering existing and future conductor and shield wire loading, with the proposed antenna mast was analyzed under two conditions:

▪ UTILITY POLE ANALYSIS

The purpose of this analysis is to determine the adequacy of the existing utility pole to support the proposed antenna loads. The loading and design requirements were analyzed in accordance with the NU Design Criteria Table, NESC C2-2007 ~ Construction Grade B, and ASCE Manual No. 72.

Load cases considered:

Load Case 1: NESC Heavy

Wind Pressure.....	4.0 psf
Vertical Overload Capacity Factor.....	1.50
Wind Overload Capacity Factor.....	2.50
Wire Tension Overload Capacity Factor.....	1.65

Load Case 2: NESC Extreme

Wind Speed.....	100 mph ⁽¹⁾
Radial Ice Thickness.....	0"

Note 1: NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading, 1.25 x Gust Response Factor (wind speed: 3-second gust)

▪ **PCS MAST ANALYSIS**

The PCS mast, appurtenances and connections to the utility pole were analyzed and designed in accordance with the NU Design Criteria Table, TIA/EIA-222-F, and AISC-ASD standards.

Load cases considered:

Load Case 1:

Wind Speed..... 85 mph ⁽²⁾
 Radial Ice Thickness..... 0"

Load Case 2:

Wind Pressure..... 75% of 85 mph wind pressure
 Radial Ice Thickness..... 0.5"

| Note 2: Per NU Mast Design Criteria Exception 1.

Results

▪ **PCS MAST**

The existing pipe mast was determined to be structurally **inadequate**. Replacement of the existing antenna mast with a **8 SCH. 80 Pipe x 27.5-ft long (O.D. = 8.63")**, conforming to ASTM A53, Grade B, F_y = 35 ksi specifications will be required.

▪ **UTILITY POLE**

This analysis finds that the subject utility pole is adequate to support the proposed antenna mast and related appurtenances. The pole stresses meet the requirements set forth by the ASCE Manual No. 72, "Design of Steel Transmission Pole Structures Second Edition", for the applied NESC Heavy and Hi-Wind load cases. The detailed analysis results are provided in Section 9 of this report. The analysis results are summarized as follows:

A maximum usage of **99.74%** occurs in the utility pole under the **NESC Extreme** loading condition.

POLE SECTION:

The utility pole was found to be within allowable limits.

Tower Section	Elevation	Stress Ratio (% of capacity)	Result
Tube Number 3	0' -25.83' (AGL)	69.44%	PASS

BASE PLATE:

The base plate was found to be within allowable limits from the PLS output based on 24 bend lines.

Tower Component	Design Limit	Stress Ratio (percentage of capacity)	Result
Base Plate	Bending	99.74%	PASS

▪ **FOUNDATION AND ANCHORS**

The existing foundation consists of a 8-ft x 8-ft x 15.0-ft long reinforced concrete pier with twelve (12) rock anchors embedded 22-ft into rock. The base of the tower is connected to the foundation by means of sixteen (16) 2.25"Ø, ASTM A615-75 anchor bolts embedded into the concrete foundation structure. Foundation information was obtained from NUSCO drawing # 01143-60001.

Review of the foundation and anchor design consisted of verification of applied loads obtained from the tower design calculations and code checks of allowable stresses:

BASE REACTIONS:

From PLS-Pole analysis of CL&P pole based on NESC/NU prescribed loads.

Load Case	Transverse	Axial	Overturning Moment
NESC Heavy Wind	21.0 kips	73.9 kips	1758.1 ft-kips
NESC Extreme Wind	36.6 kips	36.8 kips	2758.3 ft-kips
NESC Heavy Broken Wire	21.2 kips	73.8 kips	1820.3 ft-kips

ANCHOR BOLTS:

The anchor bolts were found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (% of capacity)	Result
Anchor Bolts	Tension	55.01%	PASS

FOUNDATION:

The foundation was found to be within allowable limits.

Foundation	Design Limit	Allowable Limit	Proposed Loading ⁽⁴⁾	Result
Reinf. Conc. Pier w/ Rock Anchors	OTM ⁽¹⁾	1.5 FS ⁽²⁾	5.56 FS ⁽²⁾	PASS
	Bearing Pressure	50 ksf ⁽³⁾	45.6 ksf	PASS

Note 1: OTM denote overturning moment.

Note 2: FS denotes Factor of Safety

Note 3: Bearing Capacity based on Weak Rock.

Note 4: 10% increase to PLS base reactions used in foundation analysis per OTRM 051.

CENTEK Engineering, Inc
Structural Analysis – T-Mobile: CT-11-201A
CL&P Structure # 10247
Brookfield, CT
Rev 1 ~ May 24, 2010

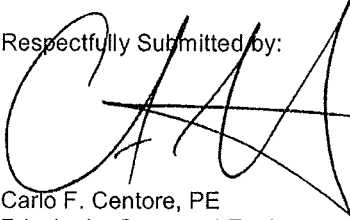
Conclusions and Recommendations

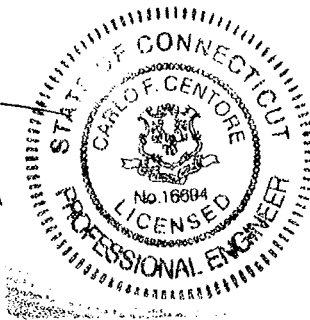
This analysis shows that the subject utility pole with the proposed mast replacement **is adequate** to support the proposed T-Mobile equipment upgrade.

The analysis is based, in part on the information provided to this office by Northeast Utilities and Transcend Wireless. If the existing conditions are different than the information in this report, CENTEK engineering, Inc. must be contacted for resolution of any potential issues.

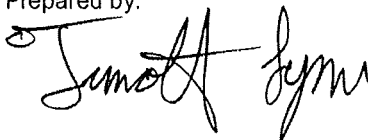
Please feel free to call with any questions or comments.

Respectfully Submitted by:


Carlo F. Centore, PE
Principal ~ Structural Engineer



Prepared by:


Timothy J. Lynn, EIT
Structural Engineer

STANDARD CONDITIONS FOR FURNISHING OF
PROFESSIONAL ENGINEERING SERVICES ON
EXISTING STRUCTURES

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CEN TEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to CEN TEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the "as new" condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222.
- All services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. CEN TEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ RISA - 3D

RISA-3D Structural Analysis Program is an integrated structural analysis and design software package for buildings, bridges, tower structures, etc.

Modeling Features:

- Comprehensive CAD-like graphic drawing/editing capabilities that let you draw, modify and load elements as well as snap, move, rotate, copy, mirror, scale, split, merge, mesh, delete, apply, etc.
- Versatile drawing grids (orthogonal, radial, skewed)
- Universal snaps and object snaps allow drawing without grids
- Versatile general truss generator
- Powerful graphic select/unselect tools including box, line, polygon, invert, criteria, spreadsheet selection, with locking
- Saved selections to quickly recall desired selections
- Modification tools that modify single items or entire selections
- Real spreadsheets with cut, paste, fill, math, sort, find, etc.
- Dynamic synchronization between spreadsheets and views so you can edit or view any data in the plotted views or in the spreadsheets
- Simultaneous view of multiple spreadsheets
- Constant in-stream error checking and data validation
- Unlimited undo/redo capability
- Generation templates for grids, disks, cylinders, cones, arcs, trusses, tanks, hydrostatic loads, etc.
- Support for all units systems & conversions at any time
- Automatic interaction with RISASection libraries
- Import DXF, RISA-2D, STAAD and ProSteel 3D files
- Export DXF, SDNF and ProSteel 3D files

Analysis Features:

- Static analysis and P-Delta effects
- Multiple simultaneous dynamic and response spectra analysis using Gupta, CQC or SRSS mode combinations
- Automatic inclusion of mass offset (5% or user defined) for dynamic analysis
- Physical member modeling that does not require members to be broken up at intermediate joints
- State of the art 3 or 4 node plate/shell elements
- High-end automatic mesh generation — draw a polygon with any number of sides to create a mesh of well-formed quadrilateral (NOT triangular) elements.
- Accurate analysis of tapered wide flanges - web, top and bottom flanges may all taper independently
- Automatic rigid diaphragm modeling
- Area loads with one-way or two-way distributions
- Multiple simultaneous moving loads with standard AASHTO loads and custom moving loads for bridges, cranes, etc.
- Torsional warping calculations for stiffness, stress and design
- Automatic Top of Member offset modeling
- Member end releases & rigid end offsets
- Joint master-slave assignments
- Joints detachable from diaphragms
- Enforced joint displacements
- 1-Way members, for tension only bracing, slipping, etc.

- 1-Way springs, for modeling soils and other effects
- Euler members that take compression up to their buckling load, then turn off.
- Stress calculations on any arbitrary shape
- Inactive members, plates, and diaphragms allows you to quickly remove parts of structures from consideration
- Story drift calculations provide relative drift and ratio to height
- Automatic self-weight calculations for members and plates
- Automatic subgrade soil spring generator

Graphics Features:

- Unlimited simultaneous model view windows
- Extraordinary “true to scale” rendering, even when drawing
- High-speed redraw algorithm for instant refreshing
- Dynamic scrolling stops right where you want
- Plot & print virtually everything with color coding & labeling
- Rotate, zoom, pan, scroll and snap views
- Saved views to quickly restore frequent or desired views
- Full render or wire-frame animations of deflected model and dynamic mode shapes with frame and speed control
- Animation of moving loads with speed control
- High quality customizable graphics printing

Design Features:

- Designs concrete, hot rolled steel, cold formed steel and wood
- ACI 1999/2002, BS 8110-97, CSA A23.3-94, IS456:2000, EC 2-1992 with consistent bar sizes through adjacent spans
- Exact integration of concrete stress distributions using parabolic or rectangular stress blocks
- Concrete beam detailing (Rectangular, T and L)
- Concrete column interaction diagrams
- Steel Design Codes: AISC ASD 9th, LRFD 2nd & 3rd, HSS Specification, CAN/CSA-S16.1-1994 & 2004, BS 5950-1-2000, IS 800-1984, Euro 3-1993 including local shape databases
- AISI 1999 cold formed steel design
- NDS 1991/1997/2001 wood design, including Structural Composite Lumber, multi-ply, full sawn
- Automatic spectra generation for UBC 1997, IBC 2000/2003
- Generation of load combinations: ASCE, UBC, IBC, BOCA, SBC, ACI
- Unbraced lengths for physical members that recognize connecting elements and full lengths of members
- Automatic approximation of K factors
- Tapered wide flange design with either ASD or LRFD codes
- Optimization of member sizes for all materials and all design codes, controlled by standard or user-defined lists of available sizes and criteria such as maximum depths
- Automatic calculation of custom shape properties
- Steel Shapes: AISC, HSS, CAN, ARBED, British, Euro, Indian, Chilean
- Light Gage Shapes: AISI, SSMA, Dale / Incor, Dietrich, Marino\WARE
- Wood Shapes: Complete NDS species/grade database
- Full seamless integration with RISAFoot (Ver 2 or better) for advanced footing design and detailing
- Plate force summation tool

CEN TEK Engineering, Inc.
Structural Analysis – T-Mobile: CT-11-201A
CL&P Structure # 10247
Brookfield, CT
Rev 1 ~ May 24, 2010

Results Features:

- Graphic presentation of color-coded results and plotted designs
- Color contours of plate stresses and forces with quadratic smoothing, the contours may also be animated
- Spreadsheet results with sorting and filtering of: reactions, member & joint deflections, beam & plate forces/stresses, optimized sizes, code designs, concrete reinforcing, material takeoffs, frequencies and mode shapes
- Standard and user-defined reports
- Graphic member detail reports with force/stress/deflection diagrams and detailed design calculations and expanded diagrams that display magnitudes at any dialed location
- Saved solutions quickly restore analysis and design results.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ PLS-POLE

PLS-POLE provides all of the capabilities a structural engineer requires to design transmission, substation or communications structures. It does so using a simple easy to use graphical interface that rests upon our time tested finite element engine. Regardless of whether you want to model a simple wood pole or a guyed steel X-Frame; PLS-POLE can handle the job simply, reliably and efficiently.

Modeling Features:

- Structures are made of standard reusable components that are available in libraries. You can easily create your own libraries or get them from a manufacturer
- Structure models are built interactively using interactive menus and graphical commands
- Automatic generation of underlying finite element model of structure
- Steel poles can have circular, 4, 6, 8, 12, 16, or 18-sided, regular, elliptical or user input cross sections (flat-to-flat or tip-to-tip orientations)
- Steel and concrete poles can be selected from standard sizes available from manufacturers
- Automatic pole class selection
- Cross brace position optimizer
- Capability to specify pole ground line rotations
- Capability to model foundation displacements
- Can optionally model foundation stiffness
- Guys are easily handled (modeled as exact cable elements in nonlinear analysis)
- Powerful graphics module (members color-coded by stress usage)
- Graphical selection of joints and components allows graphical editing and checking
- Poles can be shown as lines, wire frames or can be rendered as 3-d polygon surfaces

Analysis Features:

- Automatic distribution of loads in 2-part suspension insulators (v-strings, horizontal vees, etc.)
- Design checks for ASCE, ANSI/TIA/EIA 222 (Revisions F and G) or other requirements
- Automatic calculation of dead and wind loads
- Automated loading on structure (wind, ice and drag coefficients) according to:
 - ASCE 74-1991
 - NESC 2002
 - NESC 2007
 - IEC 60826:2003
 - EN50341-1:2001 (CENELEC)
 - EN50341-3-9:2001 (UK NNA)
 - EN50341-3-17:2001 (Portugal NNA)
 - ESAA C(b)1-2003 (Australia)
 - TPNZ (New Zealand)
 - REE (Spain)
 - EIA/TIA 222-F
 - ANSI/TIA 222-G
 - CSA S37-01
- Automated microwave antenna loading as per EIA/TIA 222-F and ANSI/TIA 222-G
- Detects buckling by nonlinear analysis

CEN TEK Engineering, Inc.
Structural Analysis – T-Mobile: CT-11-201A
CL&P Structure # 10247
Brookfield, CT
Rev 1 ~ May 24, 2010

Results Features:

- Detects buckling by nonlinear analysis
- Easy to interpret text, spreadsheet and graphics design summaries
- Automatic determination of allowable wind and weight spans
- Automatic determination of interaction diagrams between allowable wind and weight spans
- Automatic tracking of part numbers and costs

Criteria for Design of PCS Facilities On or
Extending Above Metal Electric Transmission
Towers & Analysis of Transmission Towers
Supporting PCS Masts ⁽¹⁾

Introduction

This criteria is the result from an evaluation of the methods and loadings specified by the separate standards, which are used in designing telecommunications towers and electric transmission towers. That evaluation is detailed elsewhere, but in summary; the methods and loadings are significantly different. This criteria specifies the manner in which the appropriate standard is used to design PCS facilities including masts and brackets (hereafter referred to as "masts"), and to evaluate the electric transmission towers to support PCS masts. The intent is to achieve an equivalent level of safety and security under the extreme design conditions expected in Connecticut and Massachusetts.

ANSI Standard TIA/EIA-222 covering the design of telecommunications structures specifies a working strength/allowable stress design approach. This approach applies the loads from extreme weather loading conditions, and designs the structure so that it does not exceed some defined percentage of failure strength (allowable stress).

ANSI Standard C2-2007 (National Electrical Safety Code) covering the design of electric transmission metal structures is based upon an ultimate strength/yield stress design approach. This approach applies a multiplier (overload capacity factor) to the loads possible from extreme weather loading conditions, and designs the structure so that it does not exceed its ultimate strength (yield stress).

Each standard defines the details of how loads are to be calculated differently. Most of the NU effort in "unifying" both codes was to establish what level of strength each approach would provide, and then increasing the appropriate elements of each to achieve a similar level of security under extreme weather loadings.

Two extreme weather conditions are considered. The first is an extreme wind condition (hurricane) based upon a 50-year recurrence (2% annual probability). The second is a winter condition combining wind and ice loadings.

The following sections describe the design criteria for any PCS mast extending above the top of an electric transmission tower, and the analysis criteria for evaluating the loads on the transmission tower from such a mast from the lower portions of such a mast, and loads on the pre-existing electric lower portions of such a mast, and loads on the pre-existing electric transmission tower and the conductors it supports.

| Note 1: Prepared from documentation provide from Northeast Utilities.

PCS Mast

The PCS facility (mast, external cable/trays, including the initial and any planned future support platforms, antennas, etc. extending the full height above the top level of the electric transmission structure) shall be designed in accordance with the provisions of TIA/EIA Standard 222 with two exceptions:

1. An 85 mph extreme wind speed shall be used for locations in all counties throughout the NU system.
2. The stress increase of TIA Section 3.1.1.1 is disallowed. The combined wind and ice condition shall consider ½" radial ice in combination with the wind load (0.75 Wi) as specified in TIA section 2.3.16.

ELECTRIC TRANSMISSION TOWER

The electric transmission tower shall be analyzed using yield stress theory in accordance with the attached table titled "NU Design Criteria". This specifies uniform loadings (different from the TIA loadings) on the each of the following components of the installed facility:

- PCS mast for its total height above ground level, including the initial and planned future support platforms, antennas, etc. above the top of an electric transmission structure.
- Conductors are related devices and hardware.
- Electric transmission structure. The loads from the PCS facility and from the electric conductors shall be applied to the structure at conductor and PCS mast attachment points, where those load transfer to the tower.

The uniform loadings and factors specified for the above components in the table are based upon the National Electrical Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to TIA and its loads and factors with the exceptions noted above. (Note that the NESC does not require the projected wind surfaces of structures and equipment to be increased by the ice covering.)

In the event that the electric transmission tower is not sufficient to support the additional loadings of the PCS mast, reinforcement will be necessary to upgrade the strength of the overstressed members.

Northeast Utilities Overhead Transmission Standards

Attachment A

NU Design Criteria

		Basic Wind Speed	Pressure	Height Factor	Gust Factor	Load or Stress Factor	Force Coef - Shape Factor	
		V (MPH)	Q (PSF)	Kz	Gh			
Ice Condition	TIA/EIA	Antenna Mount	TIA	TIA (75W)	TIA	TIA	TIA Section 3.1.1.1 disallowed for connection design	TIA
	NESC Heavy	Tower/Pole Analysis with antennas extending above top of Tower/Pole (Yield Stress)	---	4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole (on two faces)	---	4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor loads provided by NU					
High Wind Condition	TIA/EIA	Antenna Mount	85	TIA	TIA	TIA	TIA Section 3.1.1.1 disallowed for connection design	TIA
	NESC Extreme Wind	Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna					1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading Height above ground level based on top of Tower/Pole					1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor loads provided by NU					
NESC Extreme Ice with Wind Condition*		Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna					1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load Height above ground level based on top of Tower/Pole					1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor loads provided by NU					

* Only for Structures Installed after 2007

Communication Antennas on Transmission Structures (CL&P & WMECo Only)

Northeast Utilities Approved by: DEH (NU)	Design	OTRM 059 Page 7 of 9	Rev.0 11/17/2009
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Northeast Utilities Overhead Transmission Standards

- 2) STEP 2 - The electric transmission structure analysis and evaluation shall be performed in accordance with NESC requirements and shall include the mast and antenna loads determined from NESC applied loading conditions (not TIA/EIA Loads) on the structure and mount as specified below, and shall include the wireless communication mast and antenna loads per NESC criteria)

The structure shall be analyzed using yield stress theory in accordance with Attachment A, "NU Design Criteria." This specifies uniform loadings (different from the TIA loadings) on each of the following components of the installed facility:

- a) Wireless communication mast for its total height above ground level, including the initial and any planned future equipment (Support Platforms, Antennas, TMA's etc.) above the top of an electric transmission structure.
- b) Conductors and related devices and hardware (wire loads will be provided by NU).
- c) Electric Transmission Structure
 - i) The loads from the wireless communication equipment components based on NESC and NU Criteria in Attachment A, and from the electric conductors shall be applied to the structure at conductor and wireless communication mast attachment points, where those loads transfer to the tower.
 - ii) Shape Factor Multiplier:

NESC Structure Shape	Cd
Polyround (for polygonal steel poles)	1.3
Flat	1.6
Open Lattice	3.2

- iii) When Coaxial Cables are mounted along side the pole structure, the shape multiplier shall be:

Mount Type	Cable Cd	Pole Cd
Coaxial Cables on outside periphery (One layer)	1.45	1.45
Coaxial Cables mounted on stand offs	1.6	1.3

- d) The uniform loadings and factors specified for the above components in Attachment A, "NU Design Criteria" are based upon the National Electric Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to the TIA and its loads and factors with the exceptions noted above.

Note: The NESC does not require ice load be included in the supporting structure. (Ice on conductors and shield wire only, and NU will provide these loads).

- e) Mast reaction loads shall be evaluated for local effects on the transmission structure members at the attachment points.

If the electric transmission structure is not sufficient to support the additional loadings of the wireless communication mast, reinforcement will be required to upgrade the strength of the overstressed members. Any reinforcement design will be reviewed by NU TL&CE to determine the feasibility of construction and its impact on the use of the structure as a transmission structure.

Communication Antennas on Transmission Structures (CL&P & WMECo Only)			
Northeast Utilities Approved by: DEH (NU)	Design	OTRM 059	Rev.0
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Job : T-MOBILE
 Description: BITTERN

Spec. Number
 Computed by
 Checked by

Page of
 Sheet of
 Date 8/20/09
 Date

INPUT DATA

TOWER ID: 10247 ✓
 EAST SIDE ✓

Structure Height (ft) : 115 ✓

Wind Zone : Central CT (green)

Wind Speed : 90.5711047 mph

Tower Type : Suspension ✓
 Strain

Extreme Wind Model : PCS Addition

Shield Wire Properties:

	BACK	AHEAD
NAME =	3/8 AW	3/8 AW
DESCRIPTION =	3/8	3/8
STRANDING =	7 #8 Al Weld	7 #8 Al Weld
DIAMETER =	0.385 in	0.385 in
WEIGHT =	0.262 lb/ft	0.262 lb/ft

Conductor Properties:

		BACK	AHEAD		
Number of Conductors per phase	2	BITTERN	BITTERN	2	Number of Conductors per phase
		1272.000	1272.000		
		45/7 ACSR	45/7 ACSR		
DIAMETER =		1.345 in	1.345 in		
WEIGHT =		1.432 lb/ft	1.432 lb/ft		

Insulator Weight = 0 lbs

Broken Wire Side = AHEAD SPAN

Horizontal Line Tensions:

	BACK		AHEAD	
	Shield	Conductor	Shield	Conductor
NESC HEAVY =	4,200 ✓	10,000 ✓	4,200 ✓	10,000 ✓
EXTREME WIND =	3,440 ✓	10,733 ✓	3,400 ✓	10,733 ✓
LONG. WIND =	na	na	na	na
250D COMBINED =	na	na	na	na
NESC W/O OLF =	na	na	na	na
60 DEG F NO WIND =	1,239 ✓	4,616 ✓	1,239 ✓	4,616 ✓

Line Geometry:

	BACK:	0	AHEAD:	0	SUM
LINE ANGLE (deg) =					0
WIND SPAN (ft) =	BACK:	257	AHEAD:	305	562 ✓
WEIGHT SPAN (ft) =	BACK:	525 ✓	AHEAD:	131 ✓	657 ✓

Job :
Description:

Spec. Number
Computed by
Checked by

Page of
Sheet of
Date 8/20/09
Date

WIRE LOADING AT ATTACHMENTS

TOWER ID: 10247

Wind Span = 562 ft
 Weight Span = 657 ft
 Total Angle = 0 degrees

Broken Wire Span = AHEAD SPAN
 Type of Insulator Attachment = SUSPENSION

1. NESC RULE 250B Heavy Loading:

	INTACT CONDITION			BROKEN WIRE CONDITION		
	Horizontal	Longitudinal	Vertical	Horizontal	Longitudinal	Vertical
Shield Wire =	648 lb	0 lb	800 lb	296 lb	4,830 lb	640 lb
Conductor =	2,195 lb	0 lb	5,082 lb	1,003 lb	23,000 lb	4,065 lb

2. NESC RULE 250C Transverse Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	433 lb	46 lb	198 lb
Conductor =	3,028 lb	0 lb	2,163 lb

3. NESC RULE 250C Longitudinal Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	198 lb
Conductor =	#VALUE!	#VALUE!	2,163 lb

4. NESC RULE 250D Extreme Ice & Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	1,303 lb
Conductor =	#VALUE!	#VALUE!	5,712 lb

5. NESC RULE 250B w/o OLF's

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	533 lb
Conductor =	#VALUE!	#VALUE!	3,388 lb

6. 60 Deg. F, No Wind

	Horizontal	Longitudinal	Vertical
Shield Wire =	0 lb	0 lb	172 lb
Conductor =	0 lb	0 lb	1,881 lb

7. Construction

	Horizontal	Longitudinal	Vertical
Shield Wire =	0 lb	0 lb	258 lb
Conductor =	0 lb	0 lb	2,822 lb

NOTE: All loads include required overload factors (OLF's).

Job : T. MOBILE
Description: ~~BE-THEL~~

Spec. Number
Computed by
Checked by

Page of
Sheet of
Date 8/20/09
Date

INPUT DATA

TOWER ID: 10247

Structure Height (ft) : 115

WEST SIDE

Wind Zone : Central CT (green)

Wind Speed : 90.5711047 mph

Tower Type : Suspension
 Strain

Extreme Wind Model : PCS Addition

Shield Wire Properties:

	BACK	AHEAD
NAME =	3/8 AW	3/8 AW
DESCRIPTION =	3/8	3/8
STRANDING =	7 #8 Al Weld	7 #8 Al Weld
DIAMETER =	0.385 in	0.385 in
WEIGHT =	0.262 lb/ft	0.262 lb/ft

Conductor Properties:

		BACK	AHEAD		
NAME =		BITTERN	BITTERN		
Number of Conductors per phase	2	1272.000	1272.000	2	Number of Conductors per phase
		45/7 ACSR	45/7 ACSR		
DIAMETER =		1.345 in	1.345 in		
WEIGHT =		1.432 lb/ft	1.432 lb/ft		

Insulator Weight = 0 lbs

Broken Wire Side = AHEAD SPAN

Horizontal Line Tensions:

	BACK		AHEAD	
	Shield	Conductor	Shield	Conductor
NESC HEAVY =	4,200	10,000	4,200	10,000
EXTREME WIND =	3,446	10,924	3,446	10,924
LONG. WIND =	na	na	na	na
250D COMBINED =	na	na	na	na
NESC W/O OLF =	na	na	na	na
60 DEG F NO WIND =	1,174	4,638	1,174	4,638

Line Geometry:

	BACK:		AHEAD:		SUM
LINE ANGLE (deg) =	0		0		0
WIND SPAN (ft) =	231		305		536
WEIGHT SPAN (ft) =	447		131		578



Job :
Description:

Spec. Number
Computed by
Checked by

Page of
Sheet of
Date 8/20/09
Date

WIRE LOADING AT ATTACHMENTS

TOWER ID: 10247

Wind Span = 536 ft
Weight Span = 578 ft
Total Angle = 0 degrees

Broken Wire Span = AHEAD SPAN
Type of Insulator Attachment = SUSPENSION

1. NESC RULE 250B Heavy Loading:

	INTACT CONDITION			BROKEN WIRE CONDITION		
	Horizontal	Longitudinal	Vertical	Horizontal	Longitudinal	Vertical
Shield Wire =	619 lb	0 lb	704 lb	267 lb	4,830 lb	544 lb
Conductor =	2,095 lb	0 lb	4,473 lb	903 lb	23,000 lb	3,456 lb

2. NESC RULE 250C Transverse Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	417 lb	0 lb	174 lb
Conductor =	2,915 lb	0 lb	1,904 lb

3. NESC RULE 250C Longitudinal Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	174 lb
Conductor =	#VALUE!	#VALUE!	1,904 lb

4. NESC RULE 250D Extreme Ice & Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	1,147 lb
Conductor =	#VALUE!	#VALUE!	5,027 lb

5. NESC RULE 250B w/o OLF's

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	469 lb
Conductor =	#VALUE!	#VALUE!	2,982 lb

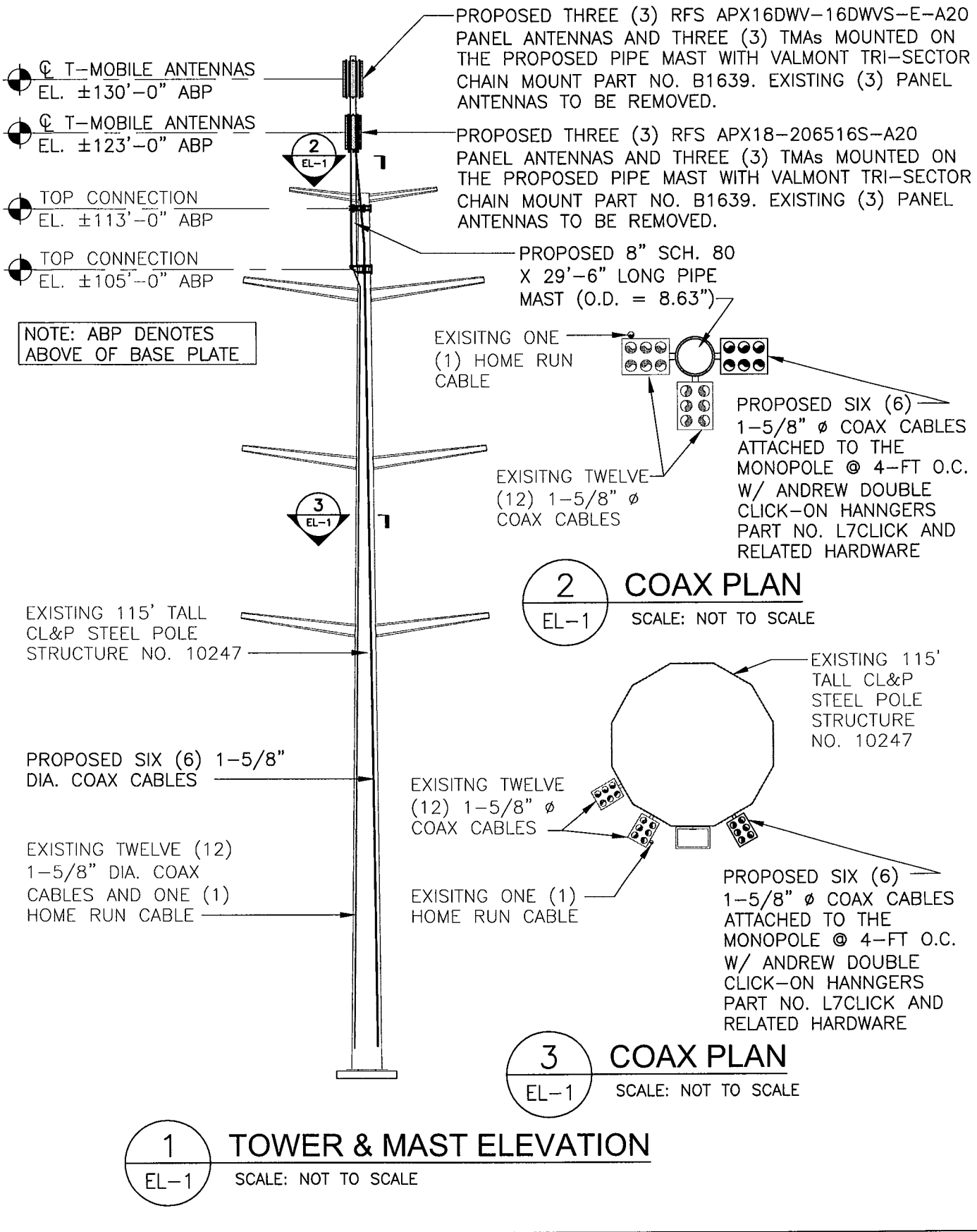
6. 60 Deg. F, No Wind

	Horizontal	Longitudinal	Vertical
Shield Wire =	0 lb	0 lb	151 lb
Conductor =	0 lb	0 lb	1,656 lb

7. Construction

	Horizontal	Longitudinal	Vertical
Shield Wire =	0 lb	0 lb	227 lb
Conductor =	0 lb	0 lb	2,484 lb

NOTE: All loads include required overload factors (OLF's).



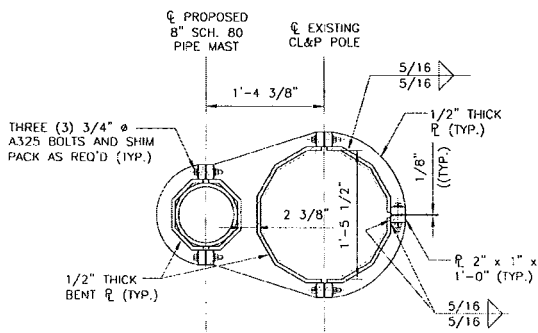
REVISIONS		
00	5/12/10	ISSUED FOR NU REVIEW
01	5/24/10	CONSTRUCTION

CEN TEK engineering
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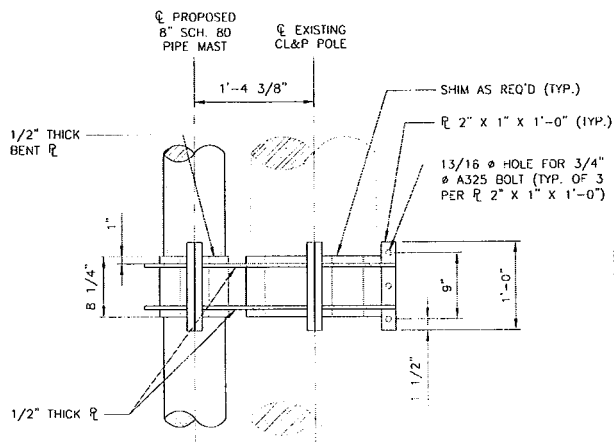
CT11201A
CL&P 10247
20 VALE ROAD
BROOKFIELD, CT 06804

PROJECT NO: 10021.CO10
DRAWN BY: TJL
CHECKED BY: CFC
SCALE: AS NOTED
DATE: 4/29/10

TOWER AND MAST ELEVATION
EL-1
DWG. 1 OF 4

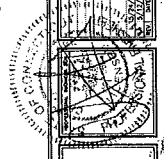


2 TOP PCS BRACKET PLAN VIEW
 S-1 SCALE: 1" = 1'-0"



1 TOP PCS BRACKET DETAIL
 S-1 SCALE: 1" = 1'-0"

DESIGNED BY:	JK
CHECKED BY:	JA
DATE:	05/17/13
PROJECT:	CT11-2011
DESCRIPTION:	CL&P STRUCTURE 10247
SCALE:	AS SHOWN
DATE:	05/21/2013

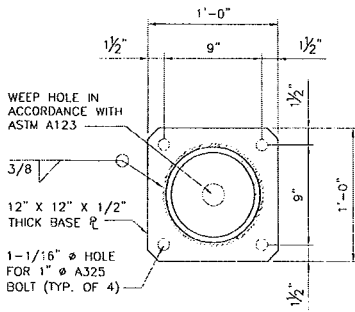


GENEX
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 Fax: 203-261-1101
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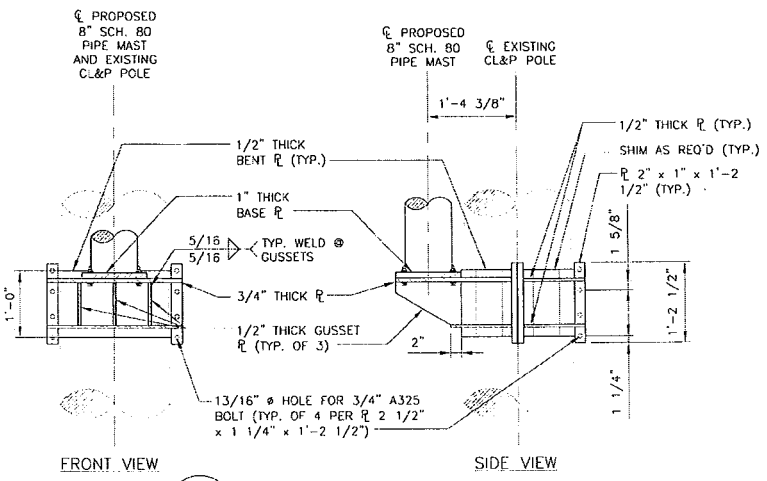
T-MOBILE
 COMMUNICATIONS
 CT11-2011
 CL&P STRUCTURE 10247
 SCALE: AS SHOWN
 DATE: 05/21/2013

TOP CONNECTION DETAILS

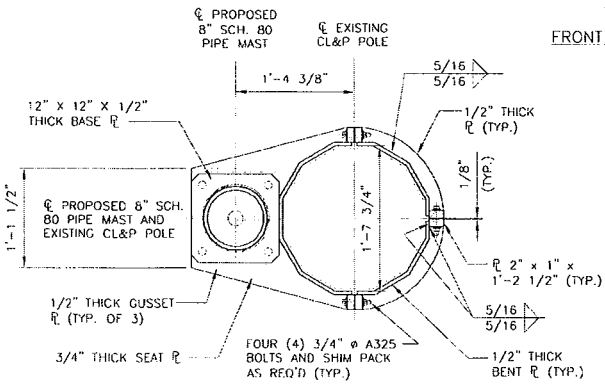
S-1



2 BASE PLATE DETAIL
S-2 SCALE: 1-1/2" = 1'-0"



1 BOTTOM PCS BRACKET DETAIL
S-2 SCALE: 3/4" = 1'-0"



3 BOTTOM PCS BRACKET PLAN VIEW
S-2 SCALE: 1" = 1'-0"

NOTE:
1. CL&P POLE TAPER = 0.2830"/FT (V.I.F.)

DESIGNED BY:	SL
DRAWN BY:	TA
CHECKED BY:	CLC
DATE:	05/13/13
SCALE:	AS SHOWN
APP. NO.:	1007-0013
CT11-2011A CL&P STRUCTURE 10247 IN FULL BOND INCORPORATED IN CT	
BOTTOM CONNECTION DETAILS	
S-2	

DESIGN BASIS:

1. GOVERNING CODE: TIA/EIA-222 REVISION "F" "STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND SUPPORTING STRUCTURES", ASCE MANUAL NO. 72 - "DESIGN OF STEEL TRANSMISSION POLE STRUCTURES SECOND EDITION", NESC C2-2002 AND NORTHEAST UTILITIES DESIGN CRITERIA.

2. DESIGN CRITERIA

WIND LOAD: (PCS MAST)

BASIC WIND SPEED (V) = 85 MPH (FASTEST MILE); BASED ON TIA/EIA-222F AND NO MAST DESIGN CRITERIA EXCEPTION 1.

GENERAL NOTES:

1. ALL WORK SHALL BE IN ACCORDANCE WITH TIA/EIA-222 REVISION "F" "STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND SUPPORTING STRUCTURES", ASCE MACLPAL NO. 72 - "DESIGN OF STEEL TRANSMISSION POLE STRUCTURES SECOND EDITION", NESC C2-2002 AND NORTHEAST UTILITIES DESIGN CRITERIA.
2. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING SITE OPERATIONS, COORDINATE WORK WITH NORTHEAST UTILITIES.
3. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
4. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND EXISTING CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA.
5. ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL IS DESIGNED BY ALLOWABLE STRESS DESIGN (ASD)
 - A. STRUCTURAL STEEL (W SHAPES)---ASTM A992 (FY = 50 KSI)
 - B. STRUCTURAL STEEL (OTHER SHAPES)---ASTM A36 (FY = 36 KSI)
 - C. STRUCTURAL HSS (RECTANGULAR SHAPES)---ASTM A500 GRADE B, (FY = 46 KSI)
 - D. STRUCTURAL HSS (ROUND SHAPES)---ASTM A500 GRADE B, (FY = 42 KSI)
 - E. PIPE---ASTM A53 (FY = 35 KSI)
 - F. CONNECTION BOLTS---ASTM A325-N
 - G. U-BOLTS---ASTM A36
 - H. ANCHOR RODS---ASTM F 1554
 - I. WELDING ELECTRODE---ASTM E 70XX
2. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE THE FOLLOWING: SECTION PROFILES, SIZES, CONNECTION ATTACHMENTS, REINFORCING, ANCHORAGE, SIZE AND TYPE OF FASTENERS AND ACCESSORIES. INCLUDE SECTION DRAWINGS, ELEVATIONS AND DETAILS.

3. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST PROVISIONS OF AISC MANUAL OF STEEL CONSTRUCTION.
4. PROVIDE ALL PLATES, CLIP ANGLES, CLOSURE PIECES, STRAP ANCHORS, MISCELLANEOUS PIECES AND HOLES REQUIRED TO COMPLETE THE STRUCTURE.
5. FIT AND SHOP ASSEMBLE FABRICATIONS IN THE LARGEST PRACTICAL SECTIONS FOR DELIVERY TO SITE.
6. INSTALL FABRICATIONS PLUMB AND LEVEL, ACCURATELY FITTED, AND FREE FROM DISTORTIONS OR DEFECTS.
7. AFTER ERECTION OF STRUCTURES, TOUCHUP ALL WELDS, ABRASIONS AND NON-GALVANIZED SURFACES WITH A 95% ORGANIC ZINC RICH PAINT IN ACCORDANCE WITH ASTM 780.
8. ALL STEEL MATERIAL (EXPOSED TO WEATHER) SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT DIPPED GALVANIZED) COATINGS" ON IRONS AND STEEL PRODUCTS.
9. ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING (HOT-DIP) ON IRON AND STEEL HARDWARE".
10. CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES APPEARANCE AND QUALITY OF WELDS, AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES". ALL WELDING SHALL BE DONE USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC AND D1.1 WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION" 9TH EDITION. AT THE COMPLETION OF WELDING, ALL DAMAGE TO GALVANIZED COATING SHALL BE REPAIRED.
11. THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NON CONFORMING MATERIALS OR CONDITIONS TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW.
12. CONNECTION ANGLES SHALL HAVE A MINIMUM THICKNESS OF 1/4 INCHES.
13. STRUCTURAL CONNECTION BOLTS SHALL CONFORM TO ASTM A325. ALL BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL HAVE A MINIMUM OF TWO BOLTS, UNLESS OTHERWISE ON THE DRAWINGS.
14. LOCK WASHER ARE NOT PERMITTED FOR A325 STEEL ASSEMBLIES.
15. SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED.
16. MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION.
17. FABRICATE BEAMS WITH MILL CAMBER UP.
18. LEVEL AND PLUMB INDIVIDUAL MEMBERS OF THE STRUCTURE TO AN ACCURACY OF 1:500, BUT NOT TO EXCEED 1/4" IN THE FULL HEIGHT OF THE COLUMN.
19. COMMENCEMENT OF STRUCTURAL STEEL WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCIES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.
20. INSPECTION AND TESTING OF ALL WELDING AND HIGH STRENGTH BOLTING SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY.
21. FOUR COPIES OF ALL INSPECTION TEST REPORTS SHALL BE SUBMITTED TO THE ENGINEER WITHIN TEN (10) WORKING DAYS OF THE DATE OF INSPECTION.

DESIGNED BY: TA	DATE: 02/12/10
CHECKED BY: TA	SCALE: AS SHOWN
DRAWN BY: OTC	SHEET NO: N-1
PROJECT: T-MOBILE	
DRAWING NO: CT11-201A	
CLIP STRUCTURE 10247	
DATE: 02/12/10	
SCALE: AS SHOWN	
SHEET NO: N-1	
TOTAL SHEETS: 1	
STRUCTURAL NOTES	

Technical Memo

To: Transcend
From: Amir Uzzaman - Radio Frequency Engineer
cc: Jason Overbey
Subject: Power Density Report for CT11201A
Date: June 8, 2010

1. Introduction:

This report is the result of an Electromagnetic Field Intensities (EMF - Power Densities) study for the T-Mobile antenna installation on a Utility Monopole at 20 Vale Road Tower #10247, Brookfield, CT. This study incorporates the most conservative consideration for determining the practical combined worst case power density levels that would be theoretically encountered from locations surrounding the transmitting location.

2. Discussion:

The following assumptions were used in the calculations:

- 1) The emissions from T-Mobile transmitters are in the (1940-1949.8), (2140-2145), (2110-2120)MHz frequency Band.
- 2) The antenna array consists of three sectors, with 2 antennas per sector.
- 3) The model number for GSM antenna is APX16DWV-16DWV.
- 3) The model number for UMTS antenna is APXV18-206516.
- 4) GSM antenna center line height is 130 ft.
- 4) UMTS antenna center line height is 123 ft.
- 5) The maximum transmit power from any GSM sector is 2104.36 Watts Effective Radiated Power (EiRP) assuming 8 channels per sector.
- 5) The maximum transmit power from any UMTS sector is 2183.18 Watts Effective Radiated Power (EiRP) assuming 2 channels per sector.
- 6) All the antennas are simultaneously transmitting and receiving, 24 hours a day.
- 7) Power levels emitting from the antennas are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 8) The average ground level of the studied area does not change significantly with respect to the transmitting location.

Equations given in "FCC OET Bulletin 65, Edition 97-01" were then used with the above information to perform the calculations.

3. Conclusion:

Based on the above worst case assumptions, the power density calculation from the T-Mobile antenna installation on a Utility Monopole at 20 Vale Road Tower #10247, Brookfield, CT, is 0.06502 mW/cm². This value represents 6.502% of the Maximum Permissible Exposure (MPE) standard of 1 milliwatt per square centimeter (mW/cm²) set forth in the FCC/ANSI/IEEE C95.1-1991. Furthermore, the proposed antenna location for T-Mobile will not interfere with existing public safety communications, AM or FM radio broadcasts, TV, Police Communications, HAM Radio communications or any other signals in the area.

The combined Power Density from other carriers is 14.45%. The combined Power Density for the site is 20.952% of the M.P.E. standard.

Connecticut Market



Worst Case Power Density

Site: **CT11201A**
 Site Address: **20 Vale Road Tower #10247**
 Town: **Brookfield**
 Tower Height: **115 ft.**
 Tower Style: **Utility Monopole**

GSM Data		UMTS Data	
Base Station TX output	20 W	Base Station TX output	40 W
Number of channels	8	Number of channels	2
Antenna Model	APX16DWV-16DWV	Antenna Model	APXV18-206516
Cable Size	1 1/4 in.	Cable Size	1 1/4 in.
Cable Length	150 ft.	Cable Length	150 ft.
Antenna Height	130.0 ft.	Antenna Height	123.0 ft.
Ground Reflection	1.6	Ground Reflection	1.6
Frequency	1945.0 MHz	Frequency	2.1 GHz
Jumper & Connector loss	4.50 dB	Jumper & Connector loss	1.50 dB
Antenna Gain	18.0 dBi	Antenna Gain	17.6 dBi
Cable Loss per foot	0.0154 dB	Cable Loss per foot	0.0116 dB
Total Cable Loss	2.3100 dB	Total Cable Loss	1.7400 dB
Total Attenuation	6.8100 dB	Total Attenuation	3.2400 dB
Total EIRP per Channel (In Watts)	54.20 dBm 263.04 W	Total EIRP per Channel (In Watts)	60.38 dBm 1091.59 W
Total EIRP per Sector (In Watts)	63.23 dBm 2104.36 W	Total EIRP per Sector (In Watts)	63.39 dBm 2183.18 W
nsg	11.1900	nsg	14.3600
Power Density (S) = 0.030026 mW/cm²		Power Density (S) = 0.034990 mW/cm²	
T-Mobile Worst Case % MPE =		6.5016%	
Equation Used : $S = \frac{(1000)(gff)^2 (Power)^{10^{(nsg/10)}}}{4\pi (R)^2}$			
Office of Engineering and Technology (OET) Bulletin 65, Edition 97-01, August 1997			

Co-Location Total	
Carrier	% of Standard
Verizon	
Cingular	
Sprint	14.4500 %
AT&T Wireless	
Pocket	
MetroPCS	
Nextel	
Other Antenna Systems	
Total Excluding T-Mobile	14.4500 %
T-Mobile	6.5016
Total % MPE for Site	20.9516%