Robinson+Cole

KENNETH C. BALDWIN

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Also admitted in Massachusetts and New York

July 31, 2023

Melanie A. Bachman, Esq. Executive Director/Staff Attorney Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

Re: Notice of Exempt Modification – Facility Modification 1300 Hall Boulevard, Bloomfield, Connecticut

Dear Attorney Bachman:

Cellco Partnership d/b/a Verizon Wireless ("Cellco") currently maintains a wireless telecommunications facility at the above-referenced address (the "Property"). Cellco's facility consists of antennas and remote radio heads attached to a rooftop tower. Equipment associated with the facility is located on the roof of the building. The existing facility was approved by the Siting Council ("Council") in August of 2022 (Petition No. 1526). A copy of the Council's Petition No. 1526 approval letter is included in <u>Attachment 1</u>.

Cellco's proposed modification involves the installation of two (2) interference mitigation filters ("filters") on the existing rooftop tower. The specification sheet for the filters is included in Attachment 2.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Bloomfield's Chief Elected Official and Land Use Officer.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing rooftop tower. The filters will be installed on Cellco's existing steel mounting pipes.

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Melanie A. Bachman, Esq. July 31, 2023 Page 2

- 2. The proposed modifications will not require the extension of the site boundary.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The installation of Cellco's new filters will not result any change to radio frequency (RF) emissions from the facility. Therefore, no new RF emissions information is included in this filing.
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. According to the attached Structural Analysis Report ("SA") and Antenna Mount Analysis Report ("MA"), the existing building roof, rooftop tower and antenna support structure can support Cellco's proposed modifications. A copy of the SA and MA are included in Attachment 3.

A copy of the parcel map and Property owner information is included in <u>Attachment 4</u>. A Certificate of Mailing verifying that this filing was sent to municipal officials and the property owner is included in <u>Attachment 5</u>.

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely.

Kenneth C. Baldwin

Enclosures Copy to:

Philip Schenck, Acting Town Manager Justin LaFountain, Director of Land Use The Atrium CT LLC, Property Owner Alex Tyurin, Verizon Wireless

ATTACHMENT 1



STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square. New Britain, CT 06051 Phone: (860) 827-2935 Fax: (860) 827-2950 E-Mail: siting.council@ct.gov Web Site: portal.ct.gov/csc

VIA ELECTRONIC & CERTIFIED MAIL RETURN RECEIPT REQUESTED

August 19, 2022

Kenneth C. Baldwin, Esq. Robinson & Cole LLP 280 Trumbull Street Hartford, CT 06103-3597 kbaldwin@rc.com

RE: **PETITION NO. 1526** – Cellco Partnership d/b/a Verizon Wireless petition for a declaratory ruling, pursuant to Connecticut General Statutes §4-176 and §16-50k, for the proposed installation of a wireless telecommunications facility and associated equipment on the roof of an existing commercial building located at 1300 Hall Boulevard, Bloomfield, Connecticut.

Dear Attorney Baldwin:

At a public meeting held on August 18, 2022, the Connecticut Siting Council (Council) considered and ruled that the above-referenced proposal would not have a substantial adverse environmental effect, and pursuant to Connecticut General Statutes § 16-50k, would not require a Certificate of Environmental Compatibility and Public Need with the following conditions:

- 1. Approval of any project changes be delegated to Council staff;
- 2. Unless otherwise approved by the Council, if the facility authorized herein is not fully constructed within three years from the date of the mailing of the Council's decision, this decision shall be void, and the facility owner/operator shall dismantle the facility and remove all associated equipment or reapply for any continued or new use to the Council before any such use is made. The time between the filing and resolution of any appeals of the Council's decision shall not be counted in calculating this deadline. Authority to monitor and modify this schedule, as necessary, is delegated to the Executive Director. The facility owner/operator shall provide written notice to the Executive Director of any schedule changes as soon as is practicable;
- 3. Any request for extension of the time period to fully construct the facility shall be filed with the Council not later than 60 days prior to the expiration date of this decision and shall be served on all parties and intervenors, if applicable, and the Town of Bloomfield;
- 4. Within 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- 5. Any nonfunctioning antenna and associated antenna mounting equipment on this facility owned and operated by the Petitioner shall be removed within 60 days of the date the antenna ceased to function:
- 6. The facility owner/operator shall remit timely payments associated with annual assessments and invoices submitted by the Council for expenses attributable to the facility under Conn. Gen. Stat. §16-50v;

c:\users\fontaime\appdata\loca\microsoft\windows\inetcache\content outlook\0ulq8lhc\pe1526_dcltr_telcom-rooftoptower (003).docx

- 7. If the facility ceases to provide wireless services for a period of one year the Petitioner shall dismantle the tower and remove all associated equipment or reapply for any continued or new use to the Council within 90 days from the one year period of cessation of service. The Petitioner may submit a written request to the Council for an extension of the 90 day period not later than 60 days prior to the expiration of the 90 day period; and
- 8. This Declaratory Ruling may be transferred or partially transferred, provided both the facility owner/operator/transferor and the transferee are current with payments to the Council for their respective annual assessments and invoices under Conn. Gen. Stat. §16-50v. The Council shall be notified of such sale and/or transfer and of any change in contact information for the individual or representative responsible for management and operations of the facility within 30 days of the sale and/or transfer. Both the facility owner/operator/transferor and the transferee shall provide the Council with a written agreement as to the entity responsible for any quarterly assessment charges under Conn. Gen. Stat. §16-50v(b)(2) that may be associated with this facility.

This decision is under the exclusive jurisdiction of the Council and is not applicable to any other modification or construction. All work is to be implemented as specified in the petition dated June 29, 2022 and additional information received on August 3, 2022.

Enclosed for your information is a copy of the staff report on this project.

Sincerely,

Melanie A. Bachman Executive Director

Matriathal

MAB/RDM/lm

Enclosure: Staff Report dated August 18, 2022

c: The Honorable Danielle Wong, Mayor, Town of Bloomfield (<u>dwong@bloomfieldct.org</u>)
The Honorable Shari Cantor, Mayor, Town of West Hartford (<u>mayor@westhartfordct.org</u>)

ATTACHMENT 2



BSF0020F3V1-1

TWIN BANDSTOP 900MHZ INTERFERENCE MITIGATION FILTER

The BSF0020 is ideal for co-located 700, 850 and 900 networks, Utilising a 2,6MHz guardband the BSF0020 provides rejection of the 900 UL band while passing 700/850 UL and DL bands. Capable of being used in an outdoor environment the BSF0020 contains two identical bandstop filters, suitable for 2x2 MIMO configuration, offering excellent insertion loss, group delay and rejection.

FEATURES

- · Passes full 700 and 850 bands
- Low insertion loss
- Rejection of 900MHz uplink
- DC/AISG pass
- Twin unit
- · Dual twin mounting available



TECHNICAL SPECIFICATIONS

BAND NAME	700 PATH / 850 LIPLINK PATH	850 DOWNLINK PATH
Passband	698 - 849MHz	869 - 891.5MHz
Insertion loss	0_1dB typical / 0.3dB maximum	0.5dB typical, 1.45dB maximum
Return loss	24dB typical, 18dB minimum	
Maximum input power (Per Port)	100W average	200W average and 66W per 5MHz
Rejection	53dB minimum @ 894,1 - 896,5MHz	
ELECTRICAL		
Impedance	500	hms
Intermodulation products	-160dBc maximum in UL Band (assuming 20MHz Signal), with 2 x 43dBm carriers -153dBc maximum with 2 x 43dBm	
DC / AISG	-11	
Passband	0 - 13MHz	
Insertion loss	0,3dB maximum	
Return loss	15dB minimum	
Input voltage range	± 33V	
DC current rating	2A continuous, 4A peak	
Compliance	3GPP TS 25.461	
ENVIRONMENTAL		
For further details of environmental co	ompliance, please contact Kaelus.	
Temperature range	-20°C to +60°C -4°F to +140°F	
Ingress protection	IP67	
Altitude	2600m 8530ft	
Lightning protection	RF port: ±5kA maximum (8/20us), IEC 61000-4-5 – Unit must be terminated with some lightning protection circu	
MTBF	>1,000,000 hours	
Compliance	ETSI EN 300 019 class 4.1H, RoHS, NEBS GR-487-CORE	
MECHANICAL		
Dimensions H x D x W	269 x 277 x 80mm 10,60 x 10,90 x 3,15in (Excluding brackets and connectors)	
Weight	8,0 kg 17,6 lbs (no bracket)	
Finish	Powder coated, light grey (RAL7035)	
Connectors	RF: 4.3-10 (F) x 4	
Mounting	Optional pole/wall bracket supplied with two metal clamps 45-178mm diameter poles or custom bracket. See orderin information.	

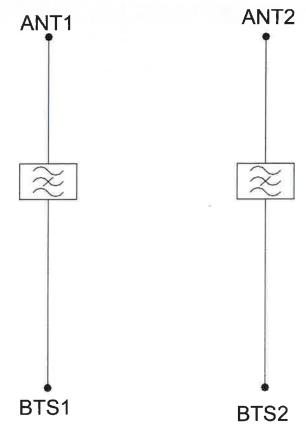


ORDERING INFORMATION

PART NUMBER	CONFIGURATION	OPTIONAL FEATURES	CONNECTORS
BSF0020F3V1	TWIN, 2 in / 2 out	DC/AISG PASS NO BRACKET	4.3-10 (F)
BSF0020F3V1-1	TWIN, 2 in / 2 out	DC/AISG PASS	4.3-10 (F)
BSF0020F3V1-2	QUAD, 4 in / 4 out	DC/AISG PASS	4.3-10 (F)

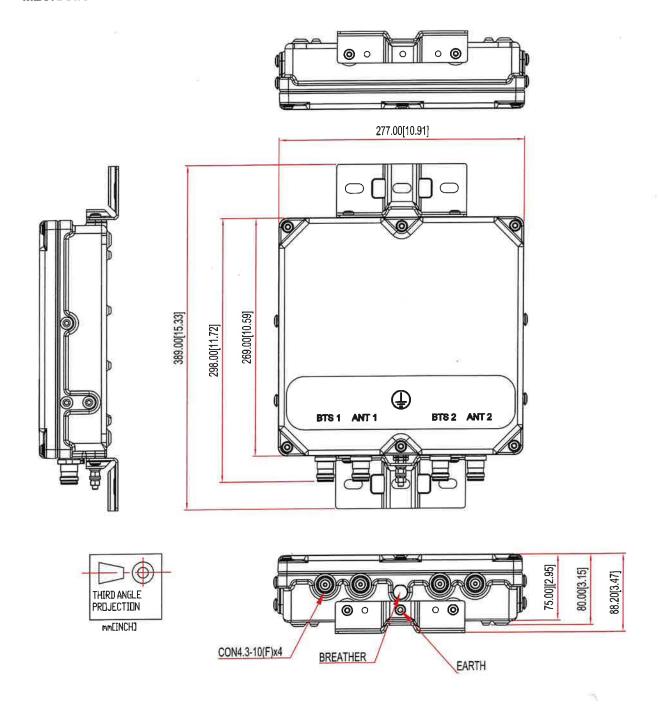


ELECTRICAL BLOCK DIAGRAM





MECHANICAL BLOCK DIAGRAM



ATTACHMENT 3



20 Alexander Drive, 2nd Floor Wallingford, CT 06492

STRUCTURAL ANALYSIS BLOOMFIELD 4 CT



Address:

1300 HALL BOULEVARD BLOOMFIELD, CT 06002

MDG LOCATION ID: 5000160079

Date:

JULY 27, 2023 (REVISION 2)



Civil · Structural · Land Surveying

July 27, 2023

20 Alexander Drive, 2nd Floor Wallingford, CT 06492

RE:

Support Structure Structural Analysis

Verizon Site Name: Bloomfield 4 CT (MDG Location ID: 5000160079)

Site Address: 1300 Hall Boulevard; Bloomfield, CT 06002

CEA Job Number: 1508.208

To whom it may concern:

Chappell Engineering Associates, LLC has performed a structural analysis of the existing triangular roofmounted antenna frame at the above-referenced location. Verizon proposes to upgrade/re-configure the existing antenna arrangement and install two radio filters to service the existing Alpha sector antennas. The proposed radio filters will be secured to the existing antenna frame currently supporting the in-service antennas and radio equipment. Our analysis has been performed in accordance with the 2022 Connecticut State Building Code (2021 International Building Code) with Connecticut Amendments.

The existing roof framing under the existing roof-mounted antenna frame consists of transverse roof beams spaced at approximately 9ft on-center. 3in composite roof decking is laid over the main roof beams. The existing transverse roof beams span approximately 26ft. A rubber membrane roof is laid over the roof deck. We have modeled the existing roof deck under the existing antenna frame to determine the suitability of the existing roof framing to support the proposed equipment.

CEA conducted a site visit on 5/3/23 to investigate the subject antenna mounts and to gather pertinent data and information as it relates to both the existing and proposed antenna and ancillary equipment configurations on the steel mounting frame. Based upon the information obtained during our site visit, our investigation of the existing roof support beams, our consideration of the proposed loads, and our analysis of the existing roof deck, Chappell Engineering Associates, LLC has determined that the existing roof deck steel has adequate capacity to support the proposed Verizon antenna upgrade/re-configuration as shown on the attached drawings. As currently configured, the existing decking is rated at approximately 49% capacity.

If you have any questions regarding this matter, please do not hesitate to call.

Very truly yours,

Clement J. Salek, P.E.

Chappell Engineering Associates, LLC

SONAL ENG

CJS/cjs



R.K. EXECUTIVE CENTRE 201 BOSTON POST RD WEST MARLEDROUGH MA MA 01742 P. (508) 481-7400 F. (508) 481-7406 www.chappellengineering.com TITLE: 1300 Hall Boulevard, Bloomfield, CT 06002

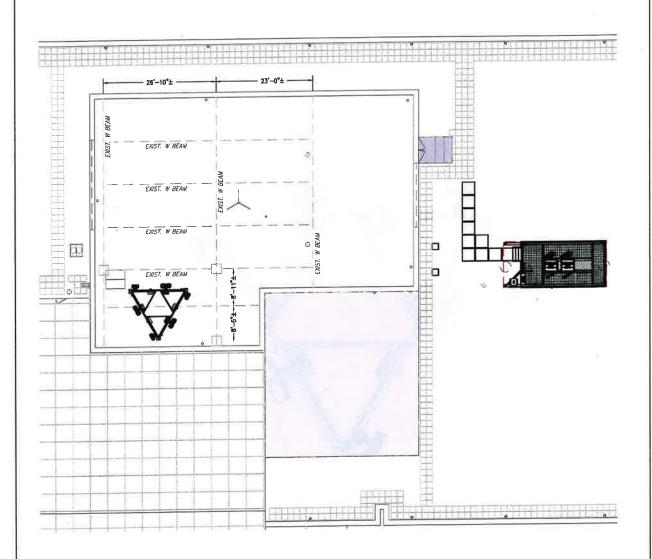
PROJECT #: Bloomfield 4 CT (1508.208)

DATE: July 25, 2023

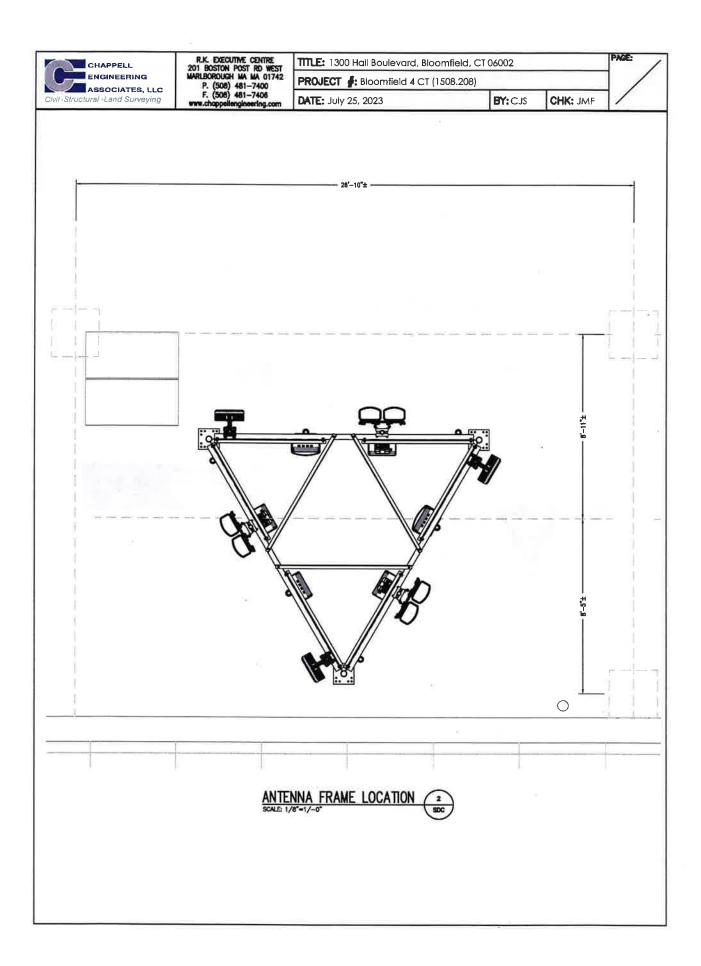
BY: CJS CH

CHK: JMF

PAGE:



PARTIAL ROOF PLAN





R.K. EXECUTIVE CENTRE 201 BOSTON POST RD WEST MARLBOROUGH MA MA 01742 P. (508) 481-7406 F. (508) 481-7406 www.chappellengineering.com

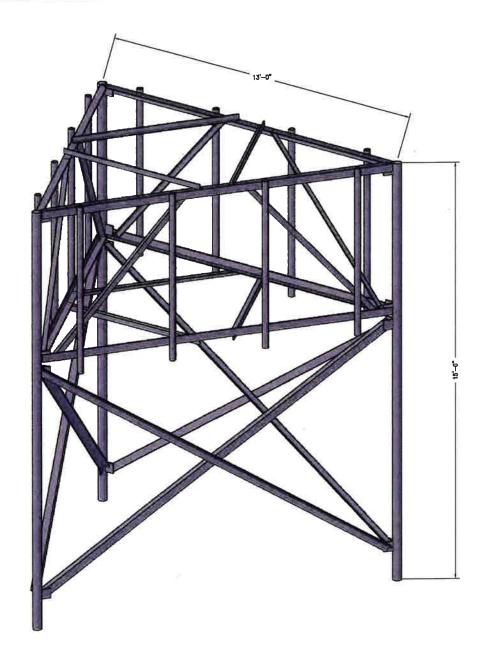
TILE: 1300 Hall Boulevard, Bloomfield, CT 06002

PROJECT #: Bloomfield 4 CT (1508.208)

DATE: July 25, 2023

BY: CJS

CHK: JMF



ANTENNA FRAME ISOMETRIC



SUPPORTING DOCUMENTS

MODO PREQUENCY (RF) DESIGN DATE: NA

ANTENNA MOUNT STRUCTURAL ANALYSIS DATE: 7/27/28

ANTERNA SUPPORT STRUCTURE ((4)-STORY BULLING) STRUCTURAL ANALYSIS DATE: 74772



CHAPPELL.
ENGINEERING
ASSOCIATES, LLC

R.V. ESECUTIVE COSTINE 201 BOSTEN FOST ROAD WEST SUITE 101 IMPEDIOROGICA, IAN 01732 (SOB) AGE-7400

ENGMEER/LAND SURVEYOR

PROJECT TYPE: UPGRADE TO EXISTING WIRELESS TELECOMMUNICATIONS

INSTALLATION ON EXISTING (4)-STORY STEEL FRAMED BUILDING

VICINITY MAP SCALE: 1"-1000

SITE INFORMATION:

PROPERTY OWNER

20 ALEXANDER DRIVE, 2nd FLOOR, WALLINGFORD, CT 06492

BLOOMFIELD 4 CT

1300 HALL BOULEVARD **BLOOMFIELD, CT 06002**

HORTH BULLDRIG ELEVATION AND DILABORD PART NORTH INJURIE ELEVATION

SOUTHAT BOULEVARD LOOMHELD, CT 08002 ниятього соилту, ст

SITE ADDRESS: APPLICANT:

COUNTY

ANTENIA DETAILS AND ANCILLARY ECURACHE SPECIFICATION

DESCRIPTION SHEET INDEX

DWG.

1300 HALL BOULEVARD BLOOMFIELD, CT 08002

BLOOMFIELD 4 CT

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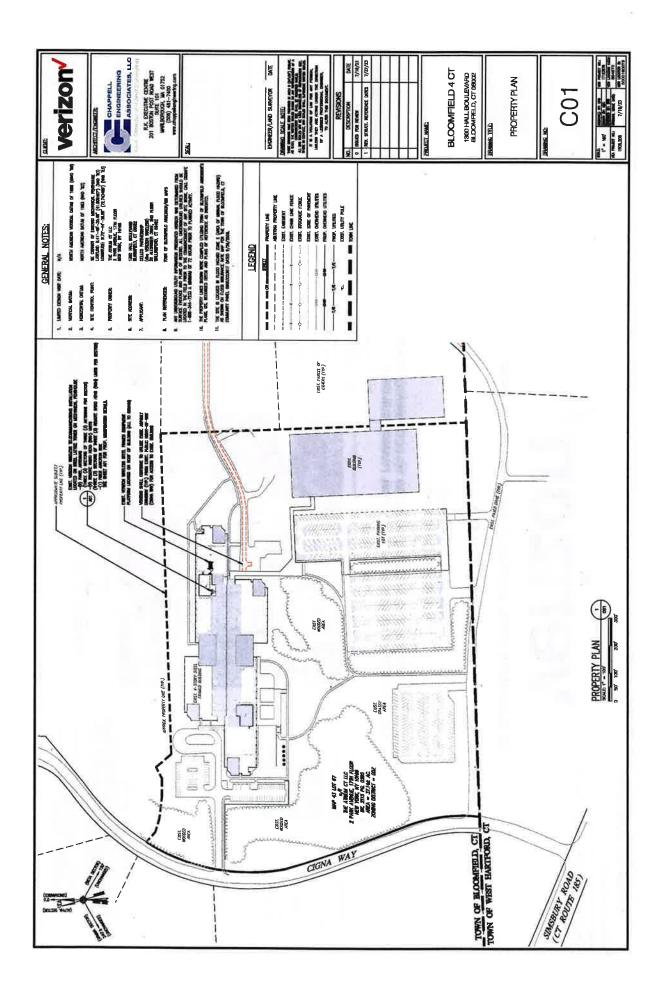
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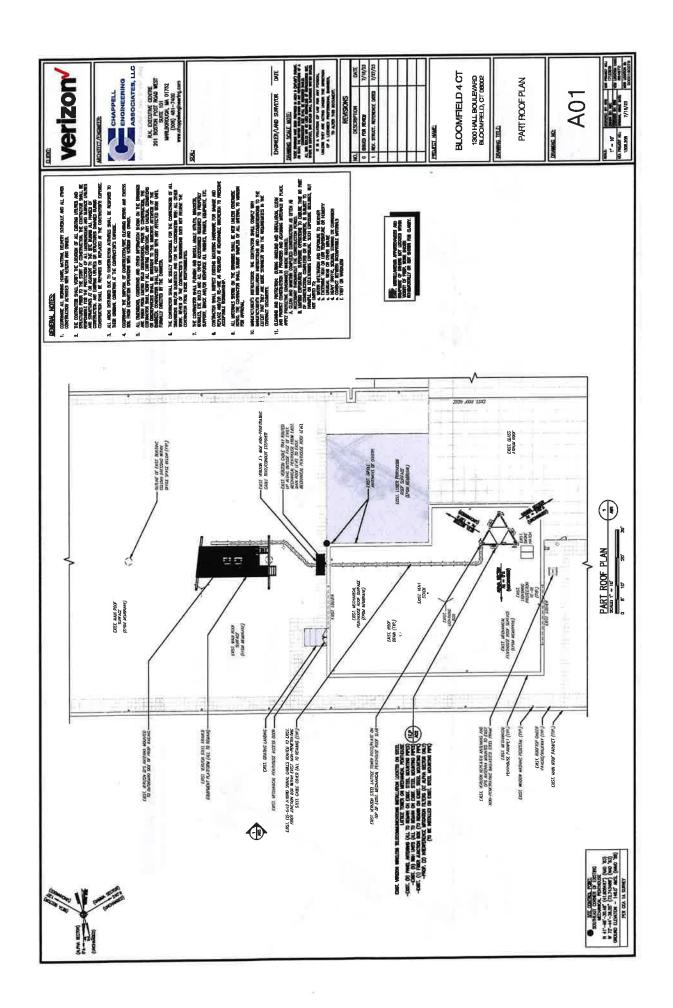
GENERAL NOTES

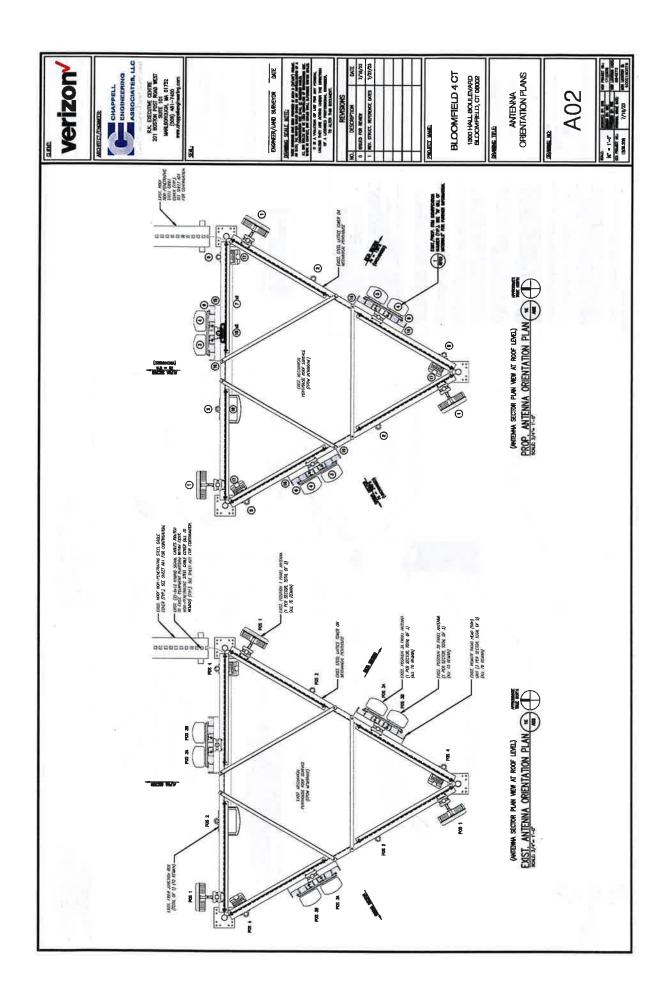
PROJECT DESCRIPTION

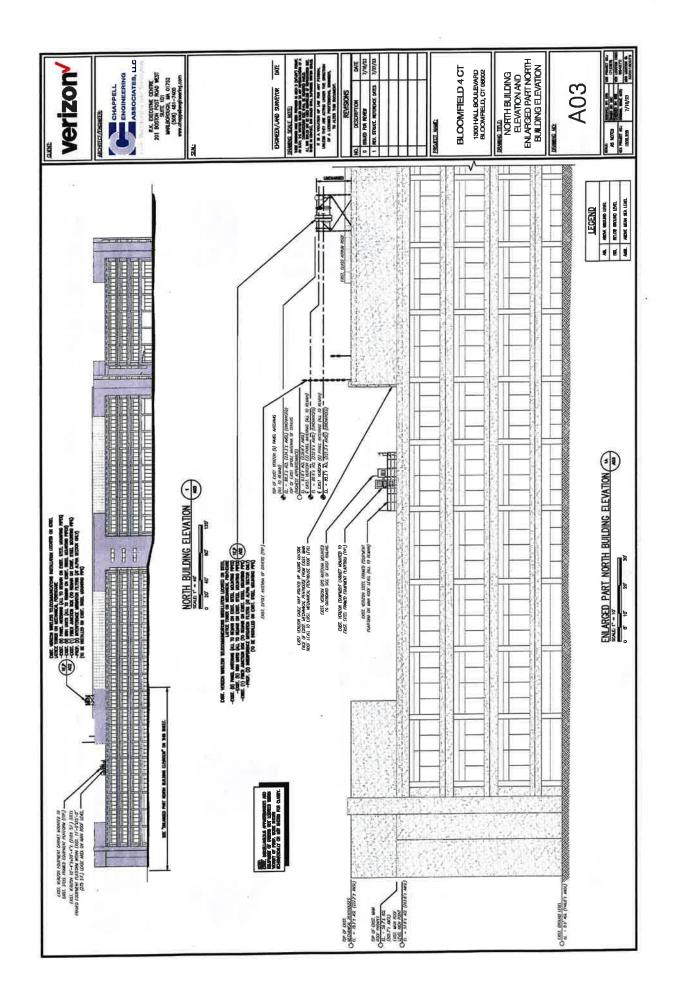
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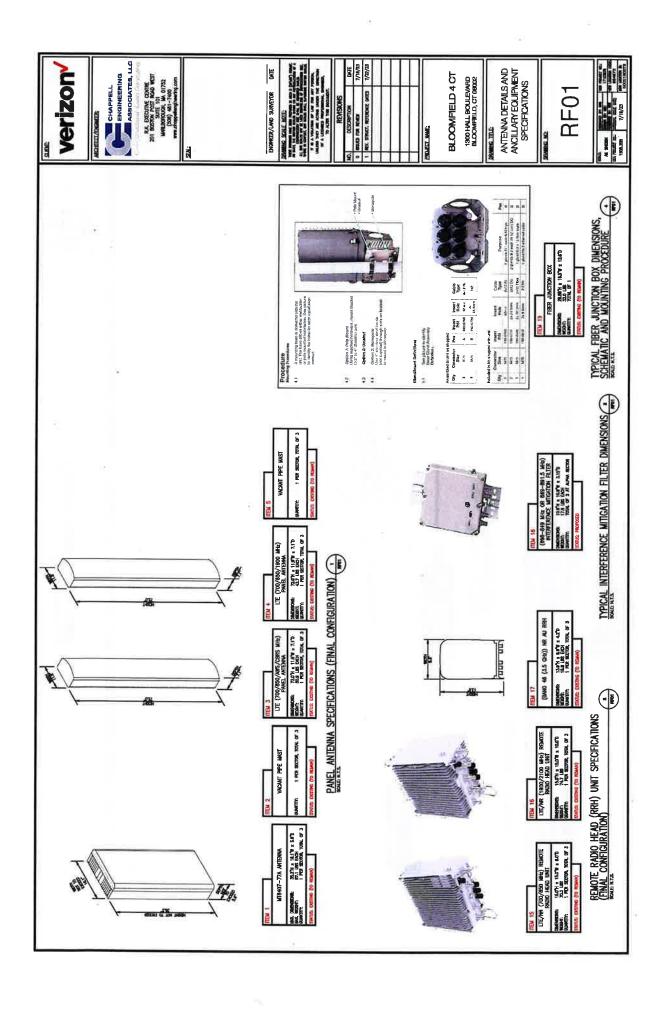
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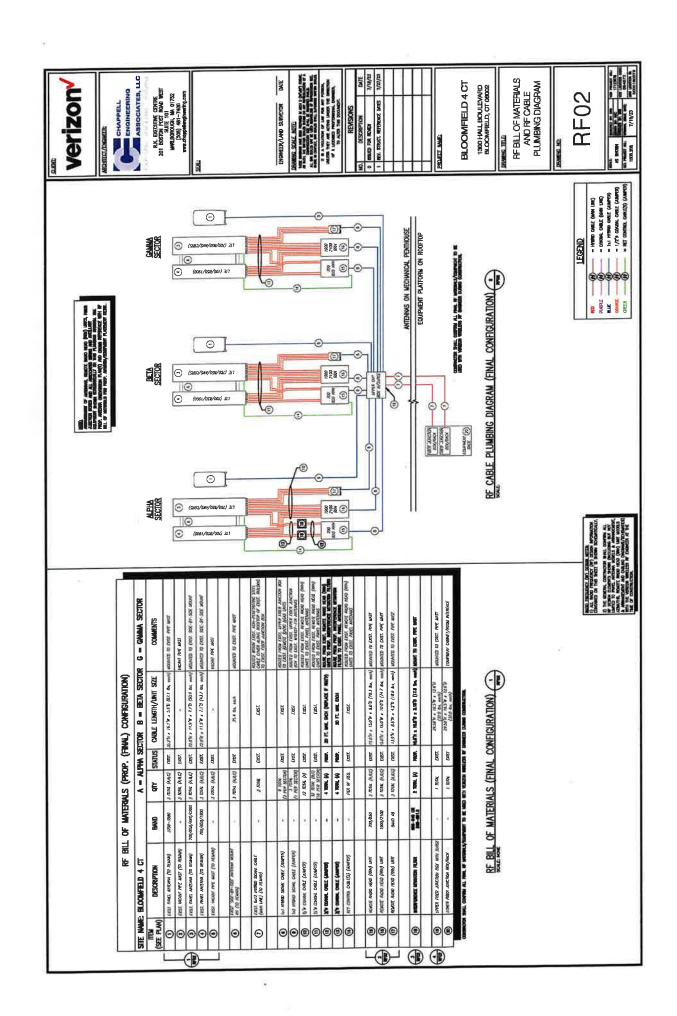




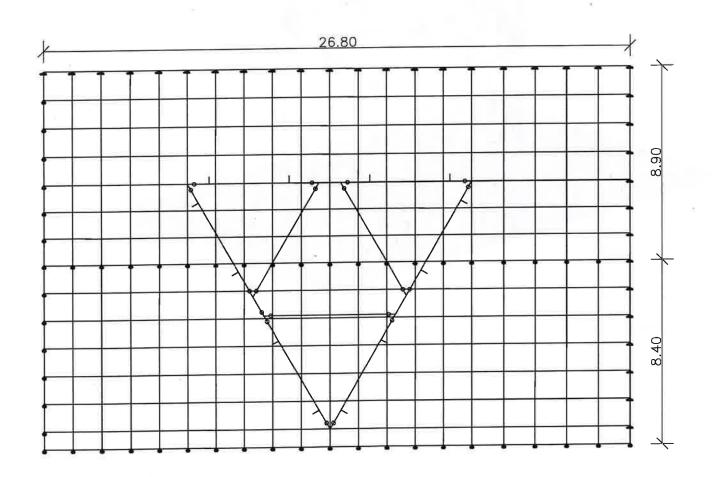








Verizon Bloomfield 4CT MSA 1508	3.208	
1		
a **		<u>X1</u>
SCALE = 1:50	DATE: 7/25/23	X2



Verizon Bloomfield 4CT MSA 1508.208		
/iew: Roof Deck	X1	X3
SCALE = 1:56	DATE: 7/25/23	
	3	

Prepared by:

Page: 4
Date: 7/25/23
17:19

Load no. 1: Front No Ice (units - kips ft.)

* GROUP NONE / JOINT LOADS FX2 0.14 FX3 -0.05 N 16 15 FX2 0.09 FX3 -0.05 N 77 76 51 50 FX2 0.04 FX3 -0.05 N 37 36 FX2 0.02 FX3 -0.05 N 92 91 66 65 FX2 0.03 FX3 -0.03 N 424 421 426 425 / JOINT LOADS FX2 0.06 FX3 -0.15 N 418 TO 420 / END

FORCE SUMMATION

FX1=0 kip FX2=1.1 kip FX3=-1.17 kip

Load no. 2: Side No Ice (units - kips ft.)

* GROUP NONE / JOINT LOADS FX1 0.09 FX3 -0.05 N 16 15 77 76 51 50 FX1 0.02 FX3 -0.05 N 37 36 92 91 65 66 FX1 0.03 FX3 -0.02 N 424 426 425

/ JOINT LOADS FX1 0.06 FX3 -0.15 N 418 TO 420 FX1 0.03 FX3 -0.03 N 421 / END

FORCE SUMMATION

FX1=0.96 kip FX2=0 kip FX3=-1.14 kip

/END

Load no. 3: Front Ice (units - kips ft.)

* GROUP NONE / JOINT LOADS FX2 0.04 FX3 -0.33 N 16 15 77 76 51 50 FX2 0.01 FX3 -0.125 N 37 36 92 91 66 65 FX2 0.02 FX3 -0.5 N 418 TO 420 FX2 0.01 FX3 -0.2 N 424 426 425 / JOINT LOADS FX2 0.03 FX3 -0.06 N 421

Prepared by:

Page: 5 Date: 7/25/23 -17:19-

Load no. 3: Front Ice (units - kips ft.)

FORCE SUMMATION

FX1=0 kip FX2=0.42 kip FX3=-4.89 kip

Load no. 4: Side Ice (units - kips ft.)

GROUP NONE JOINT LOADS

FX1 0.03 FX3 -0.33 N 16 15 77 76 51 50

FX1 0.01 FX3 -0.125 N 37 36 92 91 66 65

FX1 0.02 FX3 -0.42 N 418 TO 420

FX1 0.01 FX3 -0.19 N 424 426 425

FX1 0.02 FX3 -0.06 N 421

/ END

FORCE SUMMATION

FX1=0.35 kip

FX2=0 kip

FX3=-4.62 kip

Load no. 5: Selfweight (units - kips ft.)

GROUP NONE

BEAM LOADS

SELF X3 -1. B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64 69 TO 130

411 TO 415 492 TO 506

/ END

FORCE SUMMATION

FX1=0 kip

FX2=0 kip FX3=-1.8238 kip

Load no. 6: Front Frame Ice (units - kips ft.)

GROUP NONE

/ BEAM LOADS

DIST GL FX2 0.0015 B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64

69 TO 130 411 TO 415 492 TO 506

/ END

FORCE SUMMATION

FX1=0 kip

FX2=0.5644 kip

FX3=0 kip

Prepared by:

Page: 6
Date: 7/25/23
———17:19—

Load no. 7: Side Frame Ice (units - kips ft.)

* GROUP NONE / BEAM LOADS

DIST GL FX1 0.0015 B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64

69 TO 130 411 TO 415 492 TO 506

/END

FORCE SUMMATION

FX1=0.5644 kip FX2=0 kip

FX3=0 kip

Load no. 8: Front Frame No Ice (units - kips ft.)

* GROUP NONE

/ BEAM LOADS

/ BEAM LOADS

DIST GL FX2 0.005 B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64

69 TO 130 411 TO 415 492 TO 506

/ END

FORCE SUMMATION

FX1=0 kip FX2=1.8813 kip FX3=0 kip

Load no. 9: Side Frame No Ice (units - kips ft.)

* GROUP NONE

/ BEAM LOADS

BEAM LOADS

DIST GL FX1 0.005 B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64

69 TO 130 411 TO 415 492 TO 506

/END

FORCE SUMMATION

FX1=1.8813 kip

FX2=0 kip

FX3=0 kip

Prepared by:

Page: 7 Date: 7/25/23 -17:19-

Load no. 10: Roof Deck DL (units - kips ft.)

* GROUP NONE / PRESSURE

FX3P GL -0.044 E 131 TO 410

/ END

FORCE SUMMATION

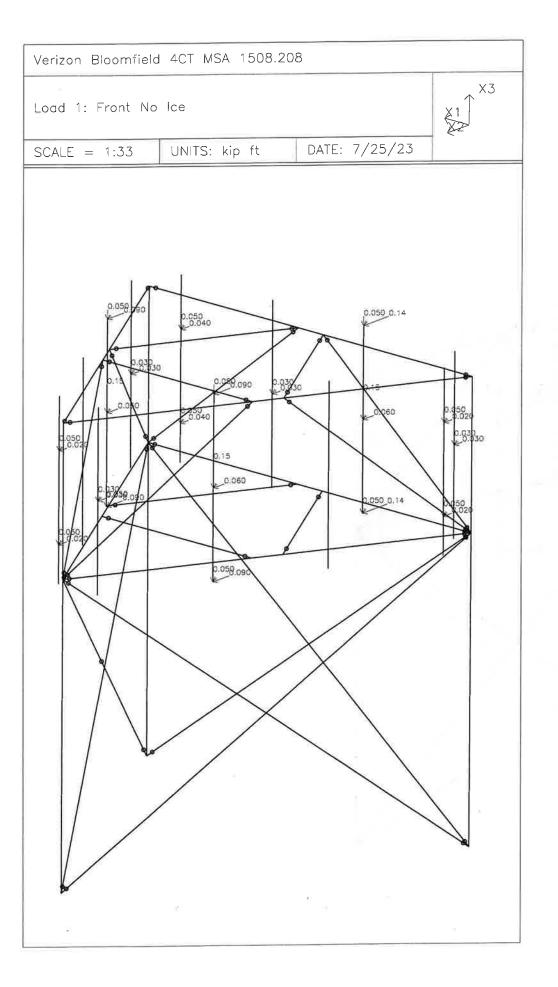
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Load no. 11: Roof Deck Snow (units - kips ft.)

* GROUP NONE / PRESSURE FX3P GL -0.035 E 131 TO 410 / END STATIC

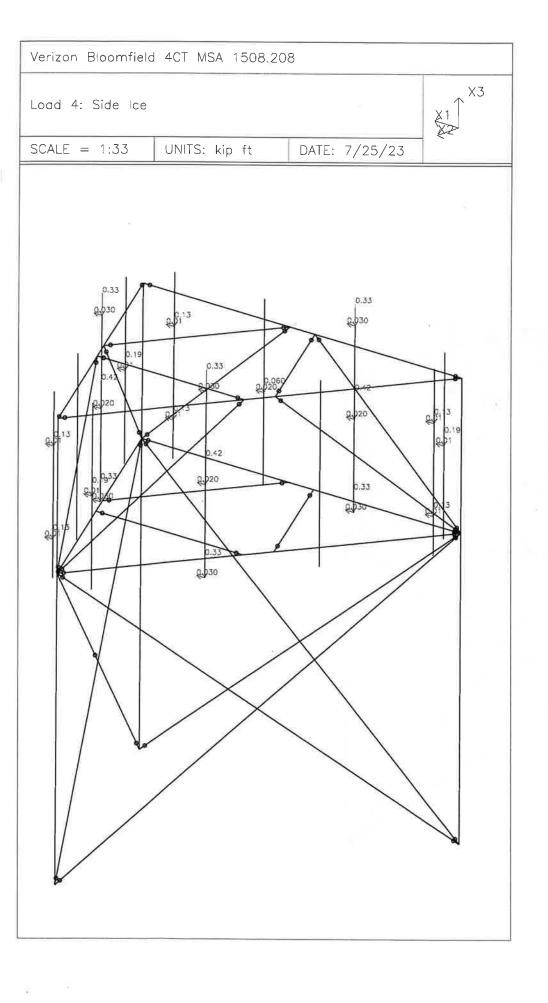
FORCE SUMMATION

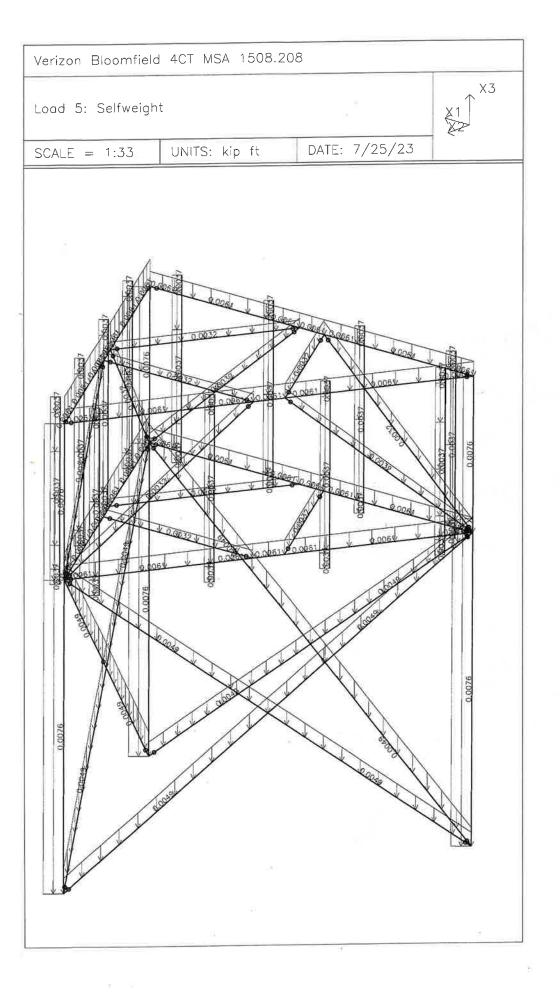
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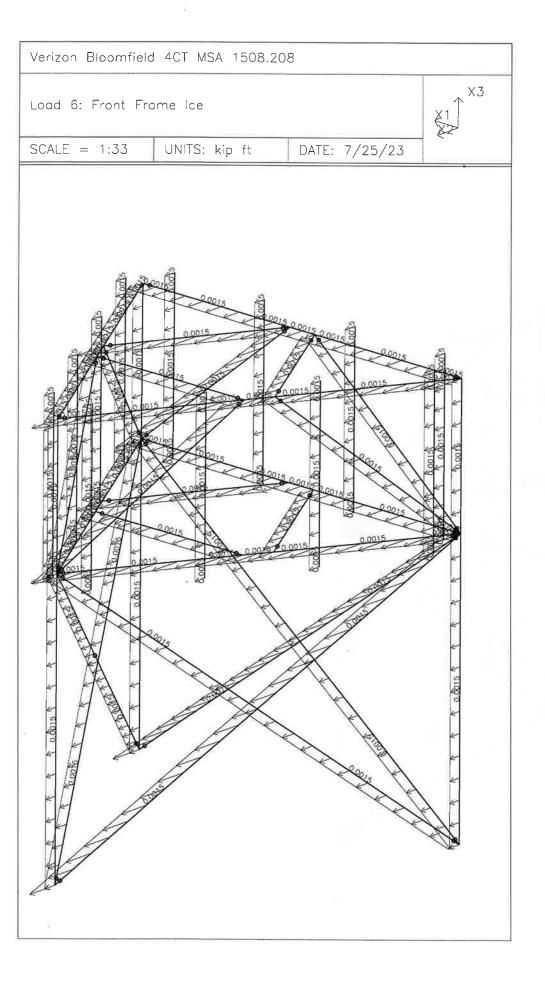


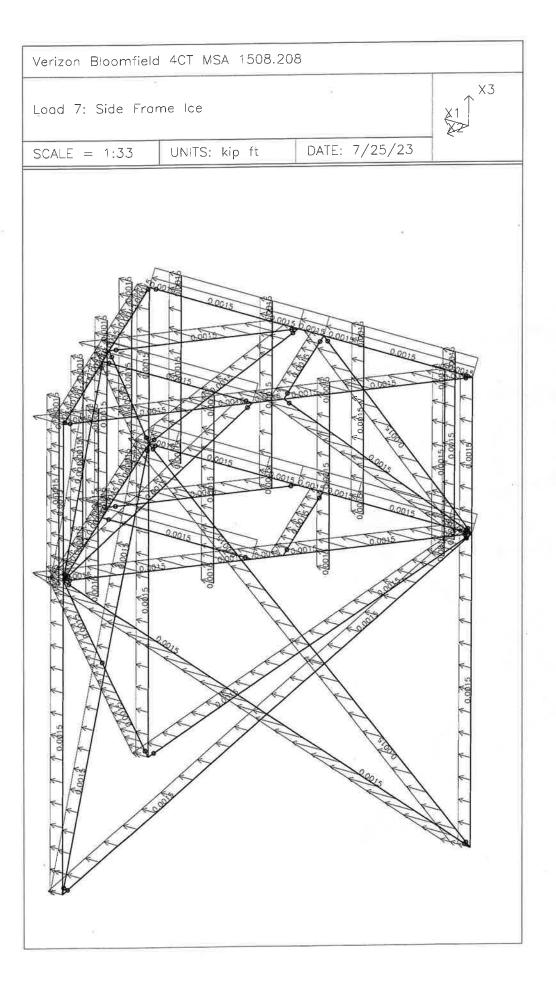
Verizon Bloomfield 4CT MSA 1508.208
Load 2: Side No Ice
SCALE = 1:33 UNITS: kip ft DATE: 7/25/23
2.050 2.050 2.050 2.050 2.050 2.050 2.050 2.050 2.050 2.050 2.050 2.050 2.050 2.050 2.050

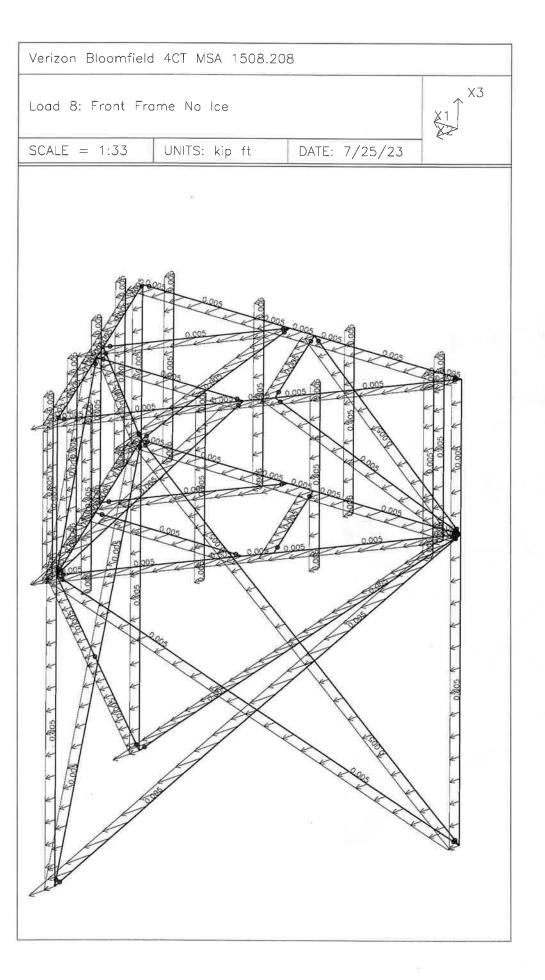
Verizon Bloomfield 4CT MSA 1508.208 Load 3: Front Ice	×3
SCALE = 1:33 UNITS: kip ft DATE: 7/25/23	
0.33 0.00 0.33 0.00 0.33 0.00	0.3301







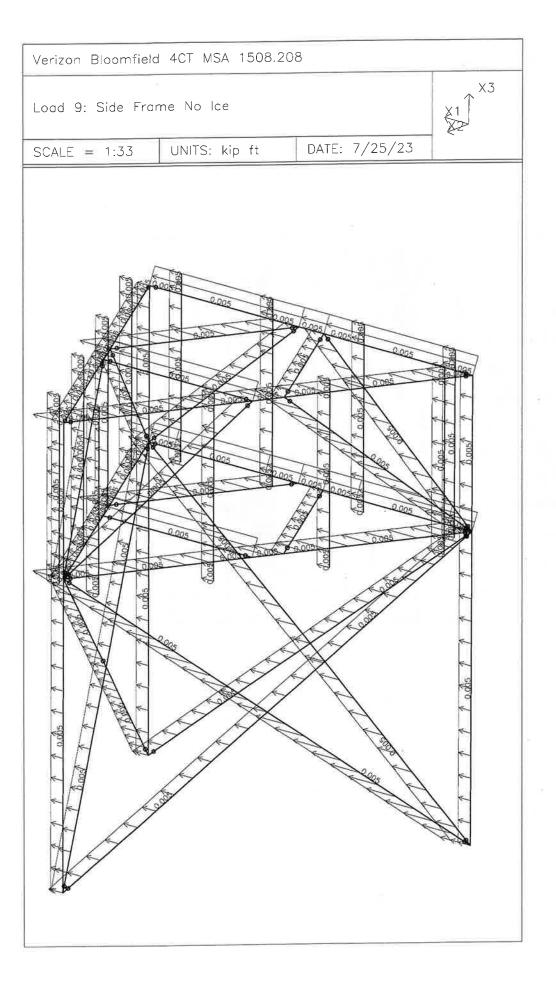




Verizon Bloomfield 4CT M Load 10: Roof Deck DL	SA 1508.208		X3
SCALE = 1:62	UNITS: kip ft	DATE: 7/25/23	

×

/erizon Bloomfield 4CT Load 11: Roof Deck Sr			X1 X3
SCALE = 1:62	UNITS: kip ft	DATE: 7/25/23	AZ
			<i>3</i> ′ • •



Verizon Bloomfield 4CT MSA 1508.208 →X1 SCALE = 1:49UNITS: kip*ft/ft DATE: 7/25/23 0.167 339 8.358 0.291 70.123 -0.043

MOMENTS DIAGRAM

MAXIMUM COMB. ENVELOPE

Deck Utilization Factor = (0.94ft-k) / (Mmax = 1.9ft-k) Deck Utilization Factor = 49%



3VLI COMPOSITE DECK

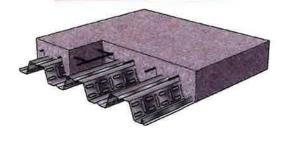
Total DL = 45psf Total LL = 148psf **Total** = 193psf Corresponding Mmax for 192psf:

 $Mmax = (193psf x 9ft^2) / (8)$

Mmax = 1954ft-lbs Mmax = 1.9ft-k

SECTION PROPERTIES

	Design	Deck		Section F	Properties		V _a	F _y
Deck	Thickness	Weight	I _p	Sp	I _n	S _n	a	' y
Gauge	(in.)	(psf)	(in ⁴ /ft)	(in³/ft)	(in ⁴ /ft)	(in³/ft)	(lbs/ft)	(ksi)
22	0.0295	1.71	0.710	0.387	0.715	0.410	1407	50
20	0.0358	2.07	0.907	0.512	0.909	0.538	2485	50
19	0.0418	2.42	1.098	0.639	1.100	0.668	3390	50
18	0.0474	2.75	1.252	0.761	1.252	0.794	4361	50
16	0.0598	3.47	1.582	1.013	1.582	1.013	4901	40



(N = 9.35) NORMAL WEIGHT CONCRETE (145 PCF)

		SDIN	SDI Max. Unshored					Superimposed Live Load (PSF)											
TOTAL SLAB	DECK	Clear Span			Clear Span (ft-in.)														
DEPTH	TYPE	1 SPAN	2 SPAN	3 SPAN	7'-0	7'-6	8'-0	81-6	9,-0	9'-6	10'-0	10'-6	11'-0	11'-6	12'-0	12'-6	13'-0	13'-6	14'-
	3VLI22	10'-0	10'-9	11'-1	216	195	176	161	148	137	127	90	83	76	70	64	59	54	50
5.00 (t=2.00) 45 psf	3VLI20	11'-8	12'-5	12'-10	241	216	196	178	163	150	139	129	121	113	78	72	66	61	57
	3VL 19	12'-3	13'-11	14'-4	265	237	214	194	178	163	151	140	131	122	115	79	73	68	62
	3VLI18	12'-7	15'-2	14'-9	289	261	238	218	201	186	173	161	151	142	134	127	92	86	80
	3VLI16	13'-4	15'-3	15'-7	327	294	267	243	223	206	191	178	167	156	147	139	132	96	89
	3VLI22	9'-6	9'-11	10'-7	247	222	201	184	169	156	113	103	94	87	80	73	67	62	57
5.50 (t=2.50) 51 psf	3VLI20	11'-3	11'-11	12'-4	275	247	223	203	186	171	159	148	138	97	89	82	76	70	65
	3VLI19	11'-10	13'-4	13'-9	302	270	244	222	203	186	172	160	149	139	98	91	84	77	71
	3VLI18	12'-3	14'-6	14'-4	330	298	271	248	229	212	197	184	173	162	153	112	105	98	92
	3VLI16	12'-11	14'-7	15'-1	373	335	304	277	255	235	218	203	190	178	168	159	117	109	10
	3VLI22	9'-2	9'-2	10'-2	277	249	226	206	190	140	127	116	106	97	89	82	76	70	65
6.00 (t=3.00) 57 psf	3VL120	10'-9	11'-5	11'-10	309	277	250	228	209	193	178	166	119	109	100	92	85	79	73
	3VLI19	11'-7	12'-9	13'-2	339	304	274	249	227	209	193	179	167	156	111	102	94	87	80
	3VLI18	11'-11	13'-11	14'-0	370	334	304	279	257	238	221	207	194	182	136	126	118	110	10
	3VLI16	12'-7	14'-0	14'-6	400	376	341	311	286	264	245	228	213	200	189	178	132	123	11:
	3VLI22	8'-9	8'-6	9'-8	307	277	251	229	171	155	141	129	118	108	99	91	84	78	72
	3VLI20	10'-4	11'-0	11'-4	343	307	278	253	232	214	198	144	132	121	111	103	95	87	81
6.50 (t=3.50)	3VLI19	11'-3	12'-4	12'-9	377	337	304	276	252	232	214	199	185	134	123	113	104	96	89
63 psf	3VLI1B	11'-8	13'-5	13'-8	400	371	338	309	285	264	246	229	215	202	151	140	131	123	11:
	3VLI16	12'-4	13'-6	14'-0	400	400	378	345	317	293	272	253	237	222	209	157	146	137	12
	3VLI22	8'-6	7'-11	9'-0	338	304	276	252	188	171	155	142	130	119	109	101	93	86	79
	3VLI20	10'-0	10'-8	11'-0	377	338	305	278	255	235	217	159	145	133	122	113	104	96	89
7.00 (t=4.00)	3VLI19	11'-0	11'-11	12'-3	400	370	334	303	277	255	236	219	204	147	135	124	115	106	98
69 psf	3VLI18	11'-5	13'-0	13'-4	400	400	371	340	313	290	270	252	236	178	166	154	144	135	12
	3VLI16	12'-1	13'-1	13'-6	400	400	400	379	348	322	298	278	260	244	230	172	161	150	14
_	3VLI22	8'-2	7'-5	8'-6	368	331	300	228	205	186	169	154	141	130	119	110	101	93	86
	3VLI20	9'-8	10'-3	10'-7	400	368	333	303	278	256	190	173	158	145	134	123	114	105	97
7.50 (t=4.50)	3VLI19	10'-8	11'-6	11'-11	400	400	364	331	302	278	257	238	175	160	147	136	125	116	10
75 psf	3VLI18	11'-2	12'-7	13'-0	400	400	400	370	341	316	294	275	258	195	181	168	157	147	13
	3VLI16	11'-10	12'-8	13'-1	400	400	400	400	380	351	325	303	283	266	202	188	175	164	15

- 1. Maximum unshored spans do not consider web crippling. Required bearing should be determined based on allowable reactions on page 43 or with the Vulcraft Unshored Span Calculator available at www.vulcraft.com/designtools. The following conditions are required to meet the maximum unshored spans shown:
 - •Minimum exterior bearing length of 1.5" for 19 to 16 gage. Minimum end bearing varies from 1.5" to 3.5" for 22 and 20 gage, depending on slab thickness.
- •Minimum interior bearing length of 2" for 16 gage. Minimum interior bearing varies from 2" to 6.25" for 18 to 22 gage, depending on gage and slab thickness.

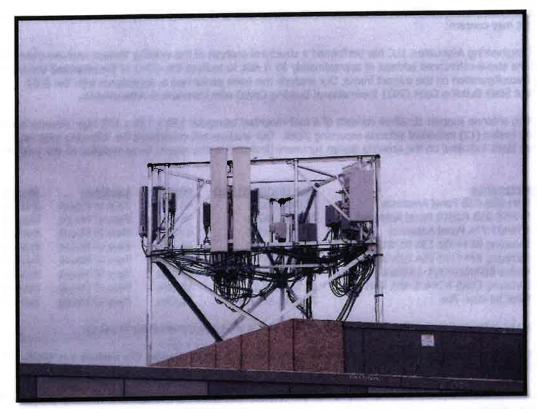
 2. Always contact Vulcraft when using loads in excess of 200 psf. Such loads often result from concentrated, dynamic or "long term" load cases for which reductions due to bond breakage, concrete creep etc. should be evaluated.
- 3. All fire rated assemblies are subject to an upper live load limit of 250 psf.





20 Alexander Drive, 2nd Floor Wallingford, CT 06492

ANTENNA MOUNT STRUCTURAL ANALYSIS BLOOMFIELD 4 CT



Address:

1300 HALL BOULEVARD BLOOMFIELD, CT 06002

MDG LOCATION ID: 5000160079

Date:

JULY 27, 2023 (REVISION 2)



Civil · Structural · Land Surveying

July 27, 2023



20 Alexander Drive, 2nd Floor Wallingford, CT 06492

RE:

Site Name:

Bloomfield 4 CT

MDG Location ID: Site Address:

5000160079 1300 Hall Boulevard, Bloomfield, CT 06002

To whom it may concern:

Chappell Engineering Associates, LLC has performed a structural analysis of the existing Verizon roof-mounted antenna frame at the above-referenced address at approximately 85 ft AGL to analyze the effect of the proposed Verizon antenna upgrade/reconfiguration on the subject frame. Our analysis has been performed in accordance with the 2022 Connecticut State Building Code (2021 International Building Code) with Connecticut Amendments.

The existing antenna support structure consists of a roof-mounted triangular 13ft x 13ft x 15ft high antenna frame supporting twelve (12) individual antenna mounting pipes. Our analysis has considered the following total major equipment loads indicated on the antenna design summary (included in this report) to be installed on the antenna frame.

<u>Appurtenance</u>	Size (HxWxD) (in)	Weight	Location	Status
(3) NHH-65B-R2B Panel Antennas	72.0x11.9x7.1	43.7lbs	Face of Mount	Existing
(3) NHHSS-65B-R2BT0 Panel Antennas	72.0x11.9x7.1	48.1lbs	Face of Mount	Existing
(3) MT6407-77A Panel Antennas	35.2x16.1x5.6	87lbs	Face of Mount	Existing
(3) Samsung RF4440d-13A B5/B13 RRH	15.0x15.0x9.0	70.3lbs	Face of Mount	Existing
(3) Samsung RF4439d-25A B25/B66a RRH	15.0x15.0x10.0	84.4lbs	Face of Mount	Existing
(2) Kaelus BSF0020F3V1-1 Filters	10.6x10.9x3.2	17.6lbs	Face of Mount	Proposed
(3) Samsung CBRS RT4401-48A RRH	13.9x8.6x4.2	18.6lbs	Face of Mount	Existing
(1) Fiber Junction Box	29.6x16.5x12.6	32.0lbs	Face of Mount	Existing

The proposed antennas and ancillary hardware are shown on the enclosed Construction Drawings.

We have modeled the triangular antenna frame under both wind and wind/ice loads. Our analysis and results are included in this report.

Based upon our analysis of the antenna mounts being proposed, the existing triangular anchored antenna roof frame has adequate capacity to support the proposed antenna configuration as shown on the construction drawings. The maximum percentage stress capacity as determined by our analysis are the lower X-bracing L's with a capacity of 51%. Our analysis assumes the existing mount has been installed and will be maintained according to standard industry practices.

If you have any questions regarding this matter, please do not hesitate to call.

Sincerely,

CHAPPELL ENGINEERING ASSOCIATES, LLC

Clement J Salek, P.E.

CJS/cjs

SUPPORTING DOCUMENTS

ANTERNA SUFFORT STRUCTURE ((Q-STORY BUILDING) STRUCTURAL ANALYSIS DATE: 7/27/23 AHTERAVA MOUNT STRUCTURIAL ANALYSIS DATE: 777725 NACIO FREGUENCY (RF) DESIGN DATE: NA



ASSOCIATES, LC

CHAPPELL

Verizon

20 ALEXANDER DRIVE, 2nd FLOOR, WALLINGFORD, CT 06492

BLOOMFIELD 4 CT

1300 HALL BOULEVARD BLOOMFIELD, CT 06002

PROJECT TYPE: UPGRADE TO EXISTING WIRELESS TELECOMMUNICATIONS INSTALLATION ON EXISTING (4)-STORY STEEL FRAMED BUILDING

VICINITY MAP SCALE: 1"-1000

SITE INFORMATION:

DRIVING DIRECTIONS

GENERAL NOTES

FROM WALLENGDRD, TAKE HY NORTH, TAKE THE CT-216 ENT TOWARD WINGSORBLOCKHELD, TURN LEFT CHTO CT-218 WEBT, LIBET PIELEFT 2 LAMED TO THIN LEFT CHTO CT-318 WEBT, LIBET INE BLICKTED STRAIGHT AFEKD,

SHEET INDEX

ENGINEER/LAND SURVEYOR

j Mg	DESCRIPTION	REV.
臣	TILLE OFEET	*
8	PROPERTY PLAN	÷
19	PART RICH PLAN	-
ā	ANTERNA OPPOTATION PLAKS	-
ą	NORTH BURDAND FLENKTION AND DILARGED PART NORTH BURDAND FLENKTION	-
<u>.</u>	ANTENAN DETAILS AND ANCELARY COMPLEYS SPECIFICATIONS	
2	HE BELL OF MATERIALS AND RE CARLE PLANTING DIAGRAM	•

DO NOT SCALE DRAWINGS

BLOOMFIELD 4 CT

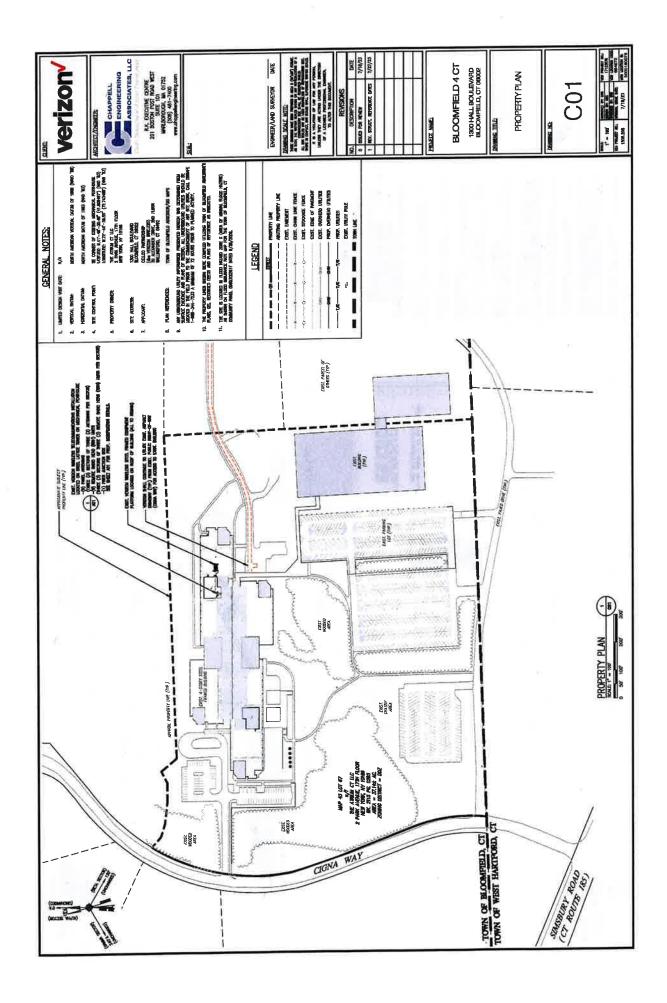
1300 HALL BOULEVARD BLOOMPELD, CT 08002

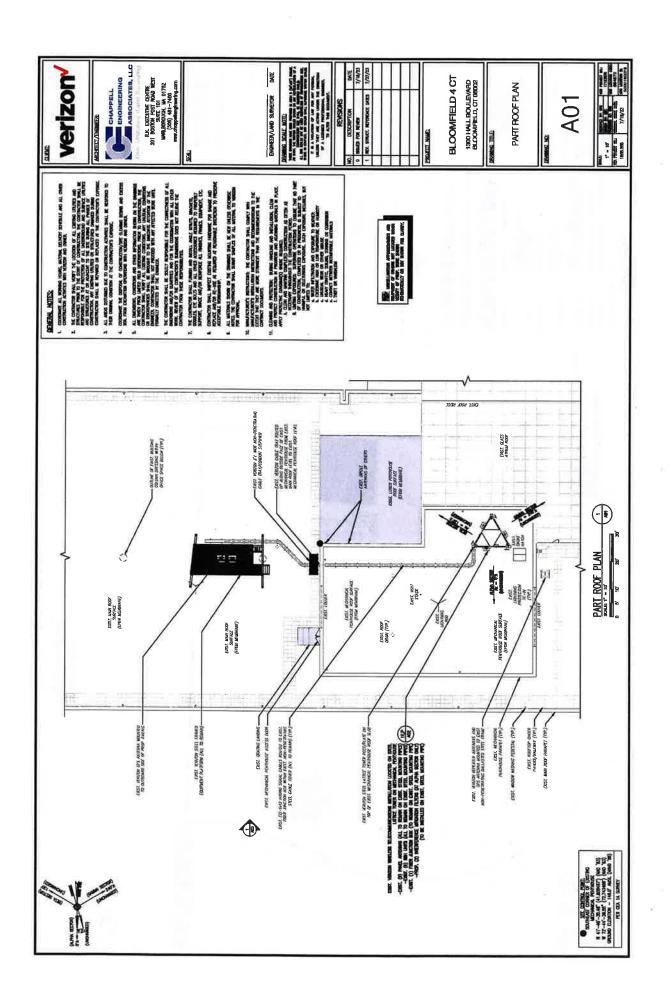
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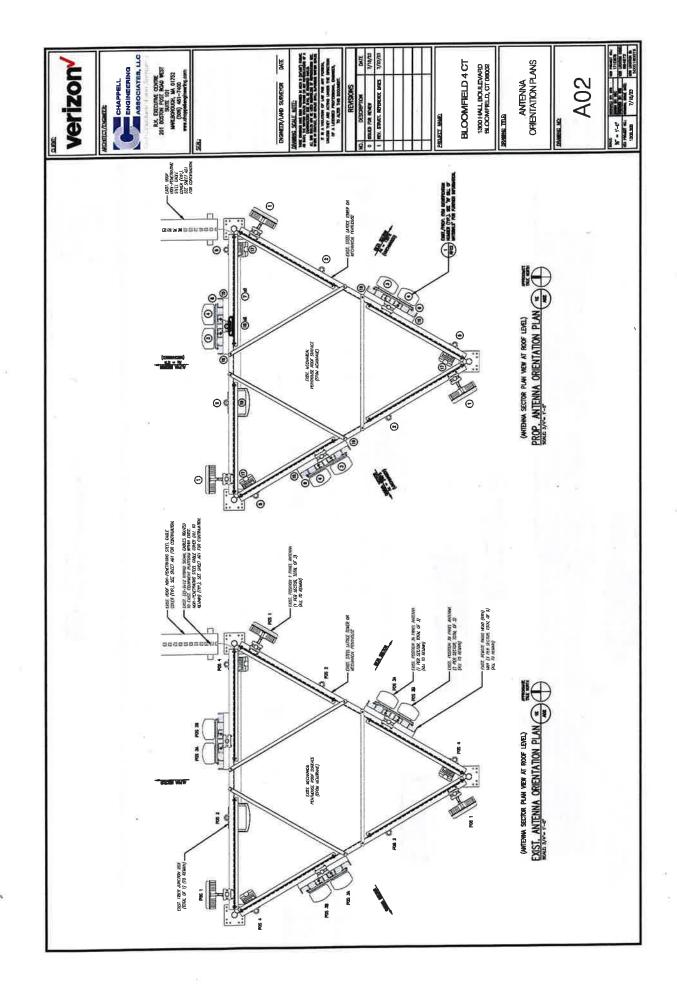
PROJECT DESCRIPTION

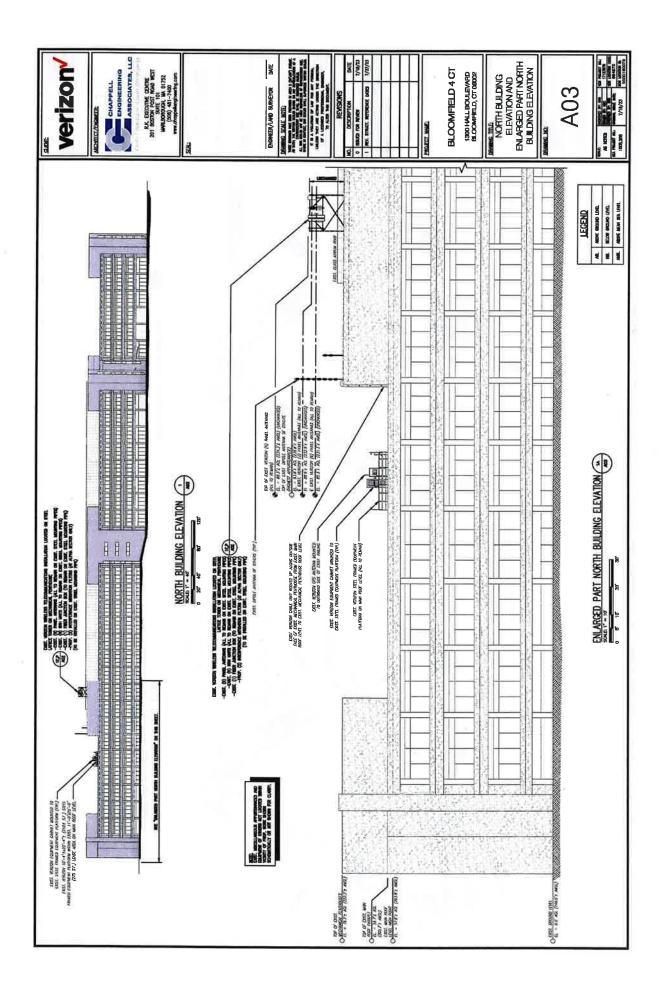
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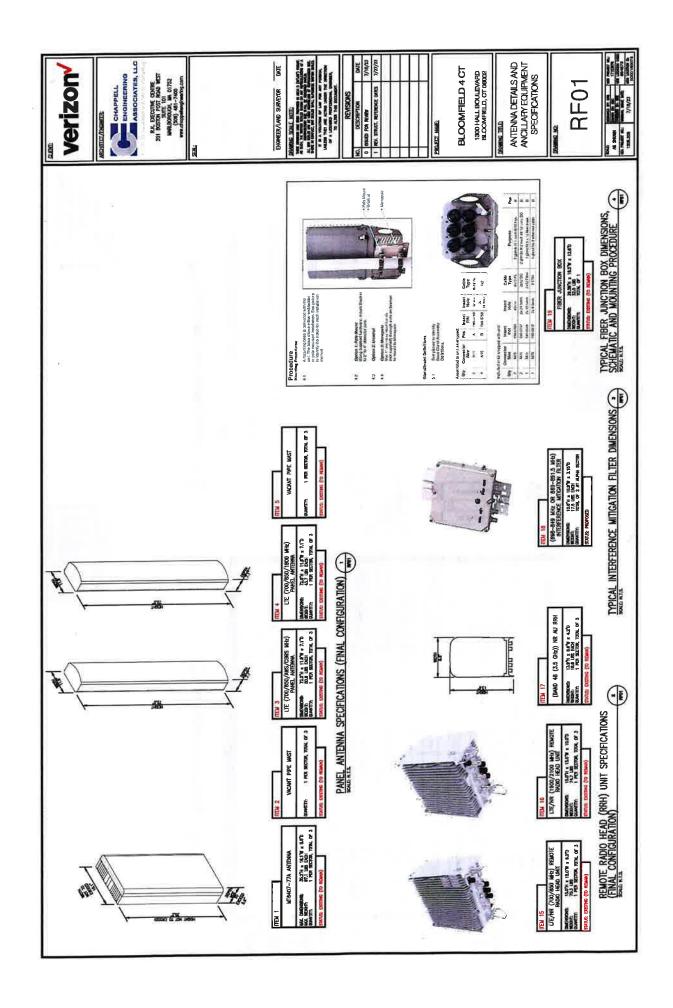
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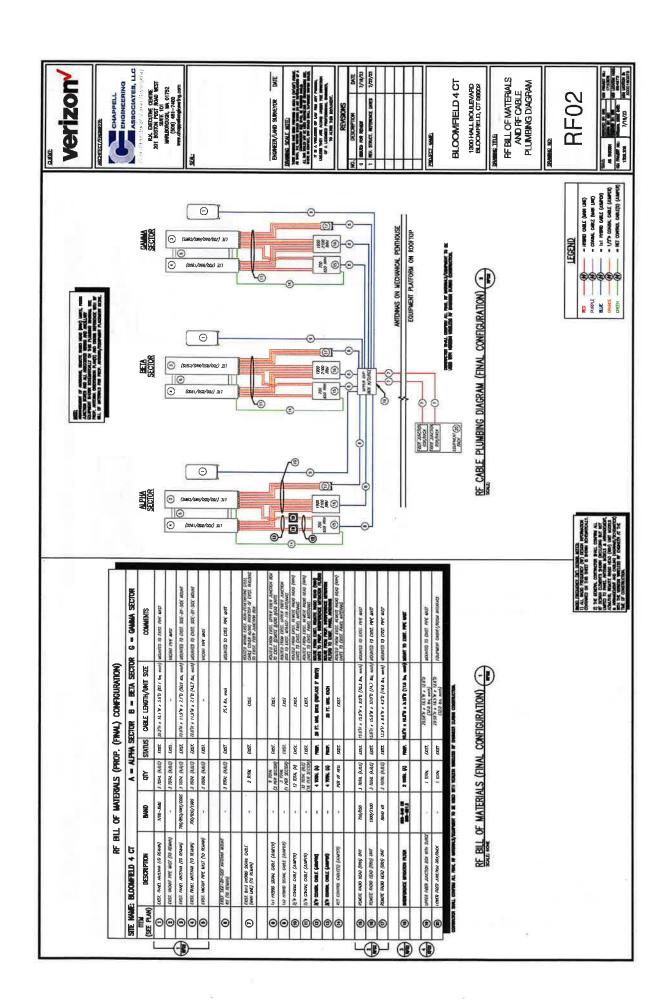


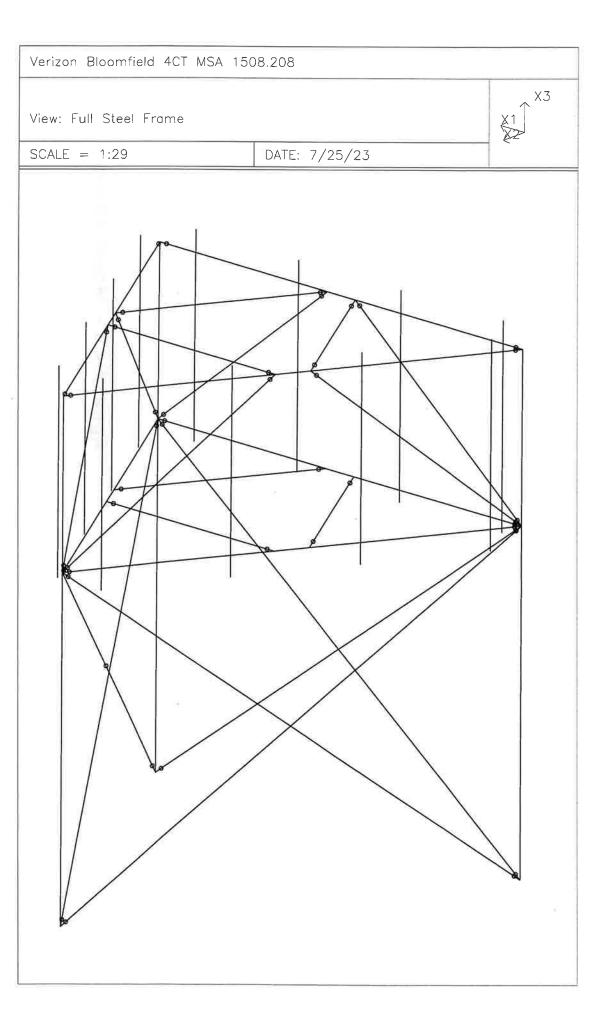


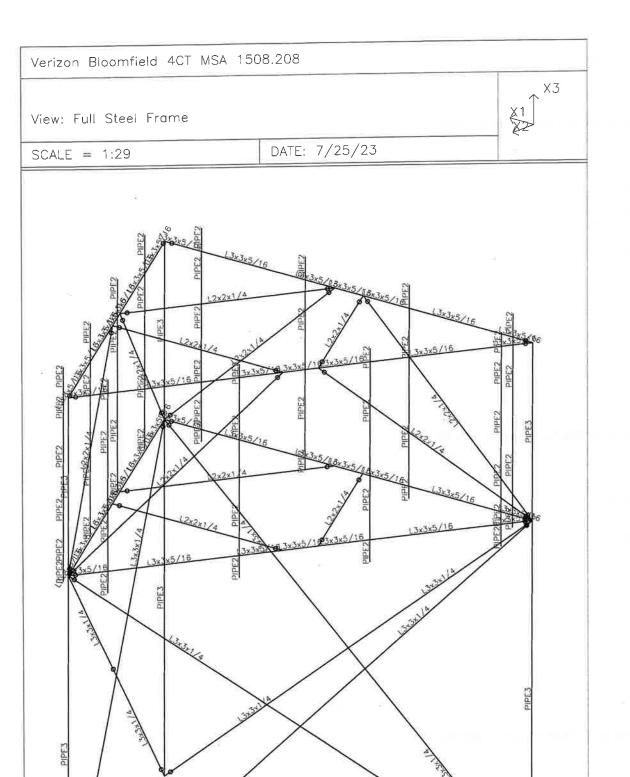












Prepared by:

Page: 1 Date: 7/25/23 -15:27-

Load no. 1: Front No Ice (units - kips ft.)

GROUP NONE JOINT LOADS FX2 0.14 FX3 -0.05 N 16 15 FX2 0.09 FX3 -0.05 N 77 76 51 50 FX2 0.04 FX3 -0.05 N 37 36 FX2 0.02 FX3 -0.05 N 92 91 66 65 FX2 0.03 FX3 -0.03 N 424 421 426 425

/ JOINT LOADS FX2 0.06 FX3 -0.15 N 418 TO 420 / END

FORCE SUMMATION

FX1=0 kip FX2=1.1 kip FX3=-1.17 kip

Load no. 2: Side No Ice (units - kips ft.)

* GROUP NONE / JOINT LOADS

FX1 0.09 FX3 -0.05 N 16 15 77 76 51 50 FX1 0.02 FX3 -0.05 N 37 36 92 91 65 66

FX1 0.03 FX3 -0.02 N 424 426 425

/ JOINT LOADS

FX1 0.06 FX3 -0.15 N 418 TO 420

FX1 0.03 FX3 -0.03 N 421

/ END

FORCE SUMMATION

FX1=0.96 kip FX2=0 kip FX3=-1.14 kip

Load no. 3: Front Ice (units - kips ft.)

* GROUP NONE / JOINT LOADS FX2 0.04 FX3 -0.33 N 16 15 77 76 51 50 FX2 0.01 FX3 -0.125 N 37 36 92 91 66 65 FX2 0.02 FX3 -0.5 N 418 TO 420

FX2 0.01 FX3 -0.2 N 424 426 425 / JOINT LOADS

FX2 0.03 FX3 -0.06 N 421

/ END

FORCE SUMMATION

FX1=0 kip FX2=0.42 kip FX3=-4.89 kip

Prepared by:

Page: 2 Date: 7/25/23 -15:27

Load no. 4: Side Ice (units - kips ft.)

GROUP NONE

/ JOINT LOADS

FX1 0.03 FX3 -0.33 N 16 15 77 76 51 50

FX1 0.01 FX3 -0.125 N 37 36 92 91 66 65

FX1 0.02 FX3 -0.42 N 418 TO 420

FX1 0.01 FX3 -0.19 N 424 426 425

FX1 0.02 FX3 -0.06 N 421

/ END

FORCE SUMMATION

FX1=0.35 kip

FX2=0 kip

FX3=-4.62 kip

Load no. 5: Selfweight (units - kips ft.)

GROUP NONE

/ BEAM LOADS

SELF X3 -1. B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64 69 TO 130

411 TO 415 492 TO 506

/ END

FORCE SUMMATION

FX1=0 kip

FX2=0 kip

FX3=-1.8238 kip

Load no. 6: Front Frame Ice (units - kips ft.)

GROUP NONE

BEAM LOADS

DIST GL FX2 0.0015 B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64

69 TO 130 411 TO 415 492 TO 506

/END

FORCE SUMMATION

FX1=0 kip

FX2=0.5644 kip

FX3=0 kip

Prepared by:

Page: 3 **Date:** 7/25/23 15:27

Load no. 7: Side Frame Ice (units - kips ft.)

* GROUP NONE / BEAM LOADS DIST GL FX1 0.0015 B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64 69 TO 130 411 TO 415 492 TO 506 / END

FORCE SUMMATION

FX1=0.5644 kip FX2=0 kip FX3=0 kip

Load no. 8: Front Frame No Ice (units - kips ft.)

* GROUP NONE / BEAM LOADS / BEAM LOADS DIST GL FX2 0.005 B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64 69 TO 130 411 TO 415 492 TO 506 / END

FORCE SUMMATION

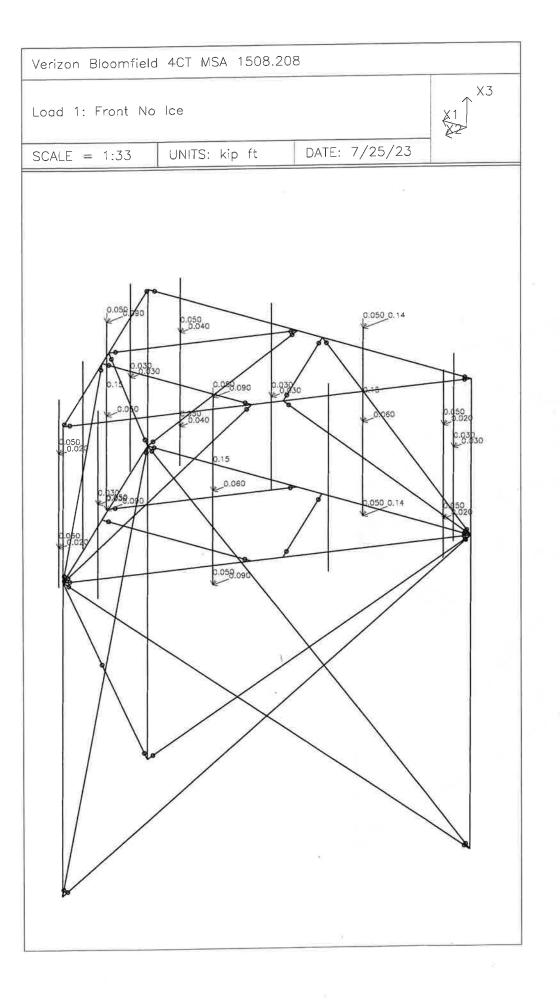
FX1=0 kip FX2=1.8813 kip FX3=0 kip

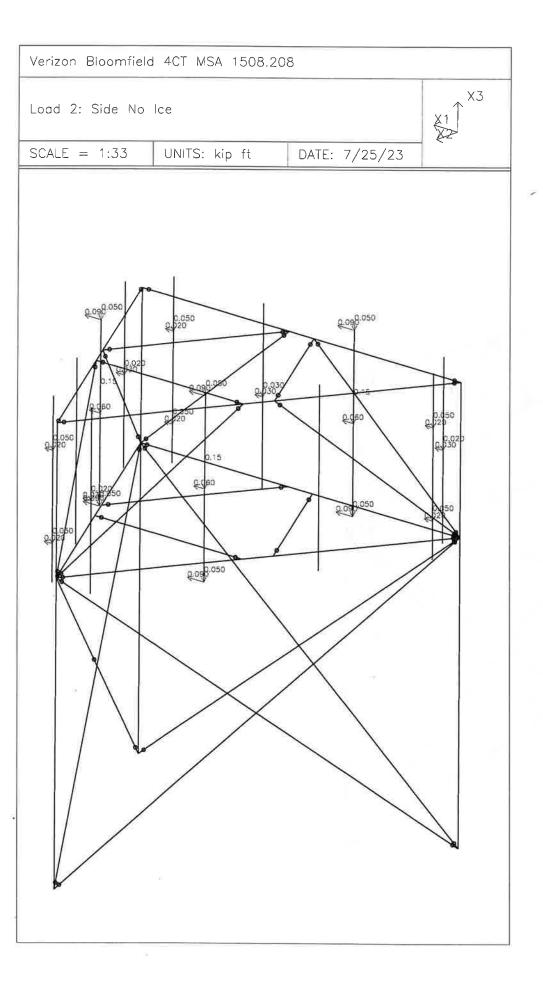
Load no. 9: Side Frame No Ice (units - kips ft.)

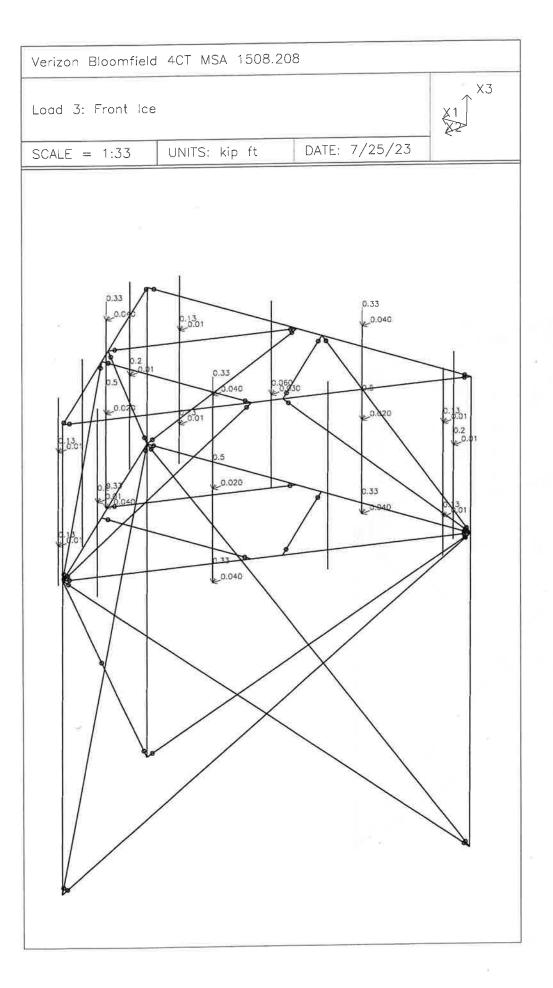
* GROUP NONE / BEAM LOADS / BEAM LOADS DIST GL FX1 0.005 B 1 TO 4 9 TO 14 19 TO 31 36 TO 41 46 TO 54 59 TO 64 69 TO 130 411 TO 415 492 TO 506 / END

FORCE SUMMATION

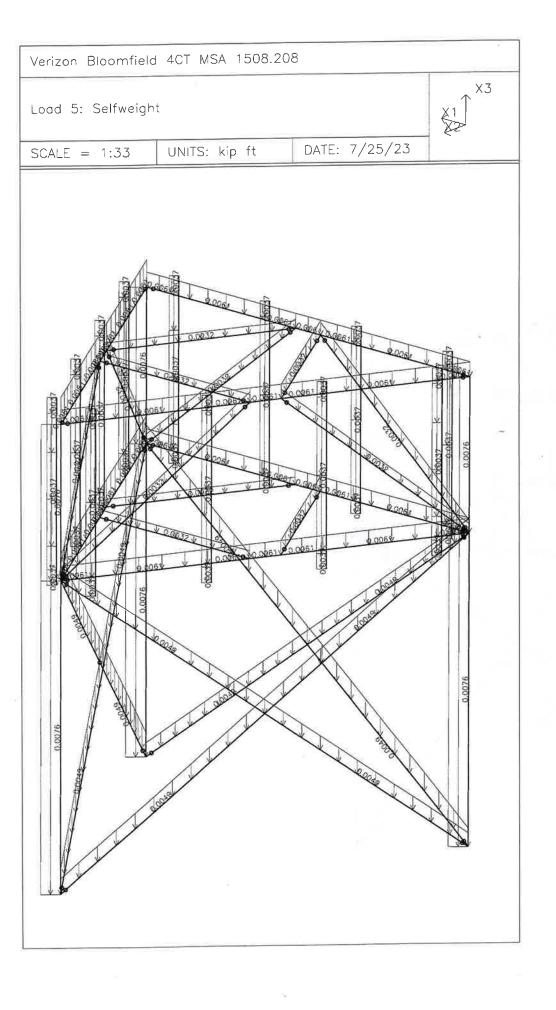
FX1=1.8813 kip FX2=0 kip FX3=0 kip

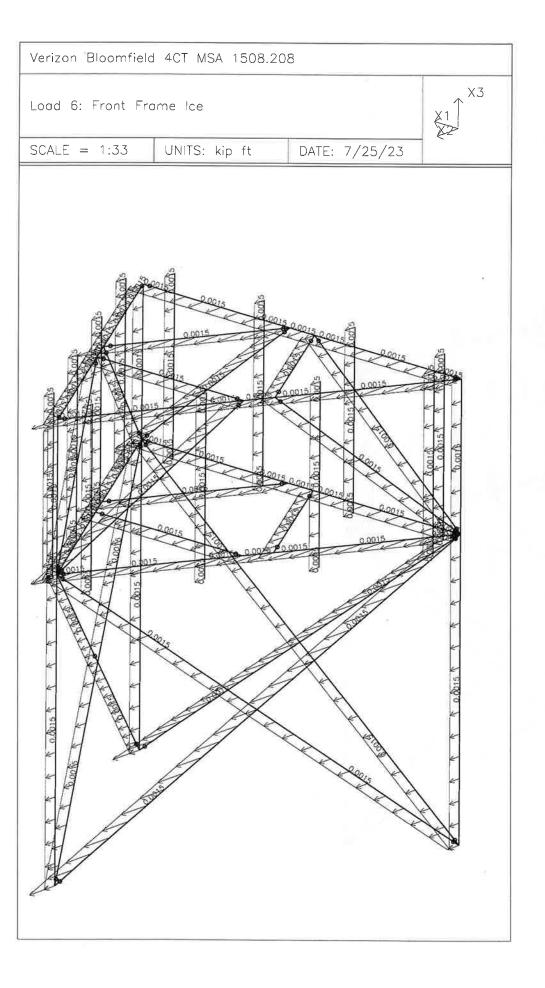


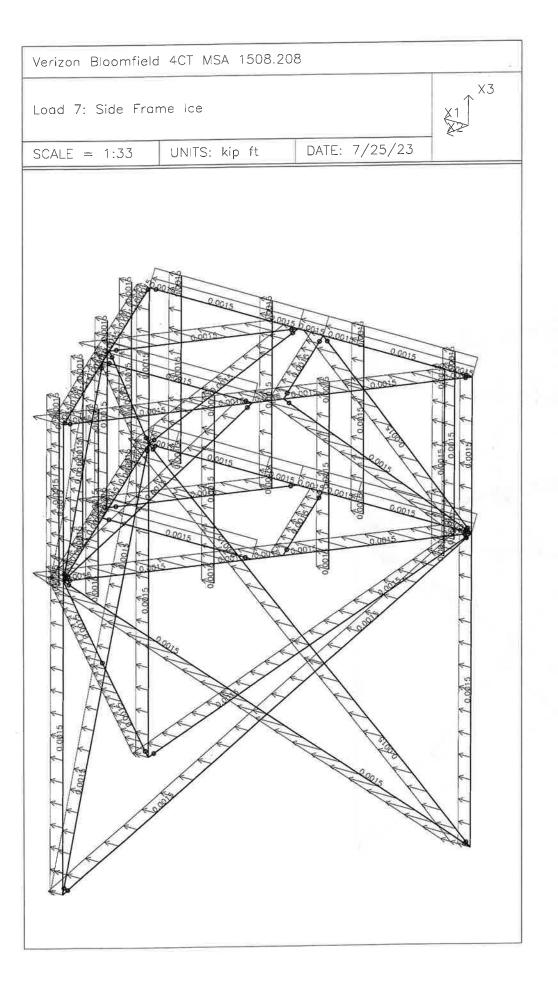


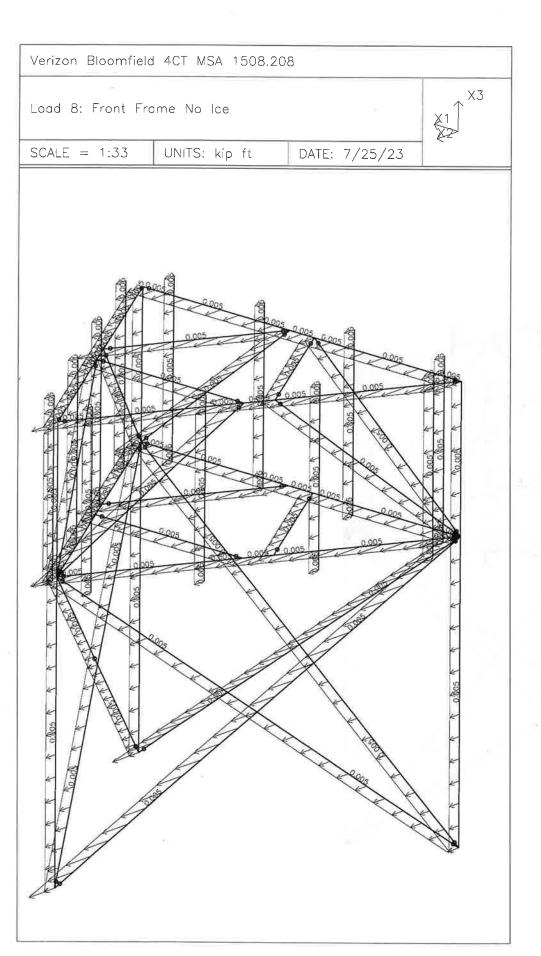


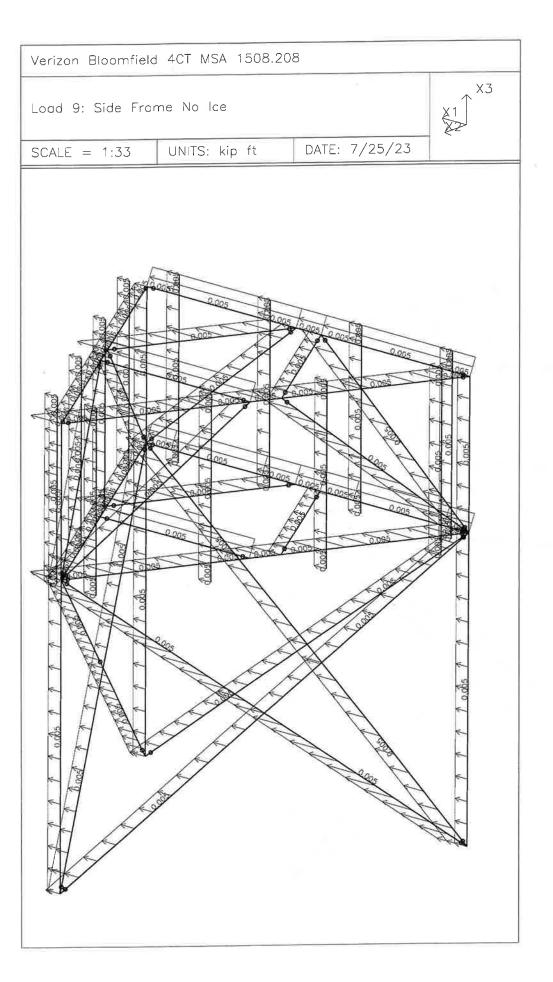
Verizon Bloomfield 4CT MSA 1508.208	
Load 4: Side Ice	X3
SCALE = 1:33 UNITS: kip ft DATE: 7/25/23	
0.33 0.42 0.33	0.19

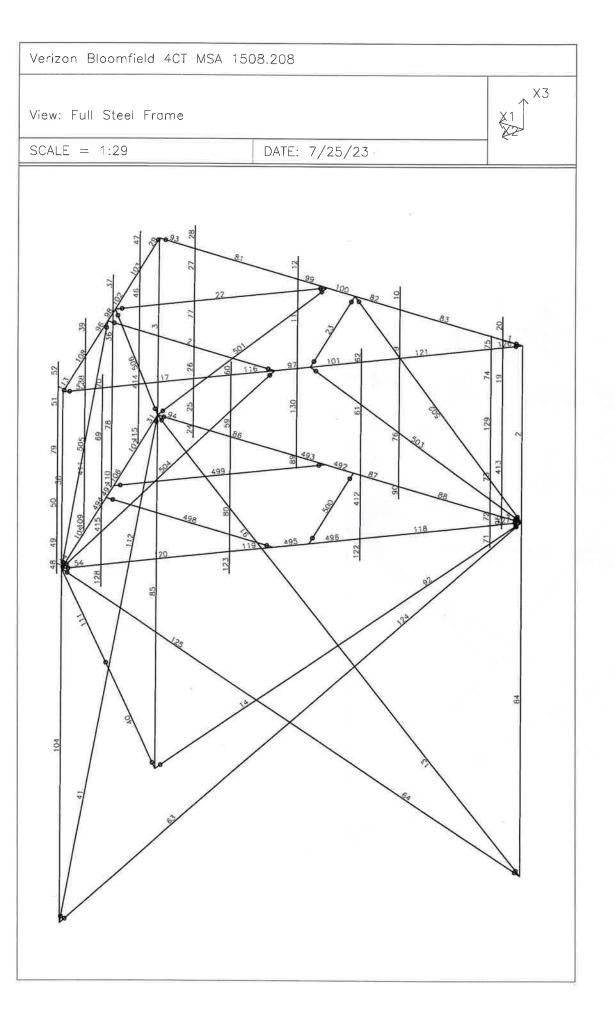












Prepared by:

Code: AISC-LRFD Page: 1 Date: 7/25/23

-15:31-

		Summary Table									
			Defl			Dir		APAC	ITY	Combined	
Beam	Section	Com	L/	Slen	Axial		Shear	Mom	LTB	Axial+Mom	
2	PIPE 3	6	9999	155	-0.07		0.00	0.02 0.01		0.05	
3	PIPE 3	7	7491	155	-0.10		0.00	0.01	0.00	0.10	
					0.02		0.00 0.01	0.03		0.23	
10	PIPE 2	ь	1396	91	0.02		0.01	0.15		0.20	
12	PIPE 2	6	1295	91	0.01		0.00	0.05	0.05	0.13	
13	L 3x3x1/4	1	484	332	-0.05	MJ	0.00	0.04	0.05	0.13	***
	1.0.0.4/4	1	485	332	-0.40		0.00	0.10	0.00	0.40	***
14	L 3x3x1/4					MI	0.00	0.08	0.00		
20	PIPE 2	3	6517	91	-0.01		0.01 0.00	0.10	0.10	0.13	
21	L 2x2x1/4	2	4409	184	0.01	MJ	0.00	0.02	0.02	0.03	_
22	L 2x2x1/4	1	2554	184	-0.03		0.00	0.02		0.04	
				50557A		MI	0.00	0.03	0.00	0.06	
23	L 2x2x1/4	2	1932	184	-0.04		0.00	0.01	0.01	0.00	
28	PIPE 2	4	5601	91	-0.01		0.00	0.02		0.05	
30	PIPE 3	6	6608	155	-0.10		0.01	0.02 0.02	0.00	0.12	
		1		91	0.02		0.00 0 .01		0.00	0.19	
37	PIPE 2	3	1692	91	0.02		0.01	0.03			Т
39	PIPE 2	6	1664	91	0.01		0.00	0.06	0.06	0.12	
40	L 3x3x1/4	2	314	332	-0.26	MJ	0.00	0.01	0.02	0.29	***
- 44	L 3x3x1/4	2	314	332	-0.40		0.00		0.00	0.51	***
						MI	0.00	0.17	0.00		
47	PIPE 2	6	3255	91	-0.01		0.00 0.01	0.08	0.04	0.13	
52	PIPE 2	6	9250	91	-0.01		0.00		0.03	0.04	
53	L 3x3x5/16	3	1198	265	-0.03		0.00 0.05	0.12	0.00	0.45	***
					0.00		0.01 0.01	0.32 0.09	0.00	0.24	
54	L 3x3x5/16	3	1214	265	0.02	MI	0.01	0.14	0.00		
60	PIPE 2	6	1422	91	0.02		0.01		0.10	0.26	
62	PIPE 2	3	1679	91	0.01		0.01	0.06	0.06	0.11	
		1	560	332	-0.39		0.00	0.04	0.00	0.47	***
03	L 3x3x1/4	+ '	300	002	-0.00		0.00	0.08	0.00		
64	L 3x3x1/4	2	561	332	-0.03		0.00	0.04 0.07	0.06	0.13	***
70	PIPE 2	3	5374	91	-0.01	MJ	0.00	0.07	0.07	0.15	
75	DIDE 2	6	9999	91	-0.01		0.01	0.08	0.00	0.05	
7002	PIPE 2		50			MI	0.00	0.03	0.00		***
93	L 3x3x5/16	3	1197	265	-0.03		0.05 0.01	0.13 0.32	0.15	0.46	
94	L 3x3x5/16	4	1227	265	0.02		0.02	0.10	0.11	0.23	-
113	L 3x3x5/16	6	1168	265	-0.03		0.01 0.05	0.14 0.11	0.00	0.45	***
			D. WORLD			MI	0.01 0.03	0.33 0.09	0.00	0.26	
114	L 3x3x5/16	3	1246	265	0.02	MI	0.01	0.17	0.00	0.20	
498	L 2x2x1/4	2	4409	184	-0.01		0.00 0.00	0.02 0.02	0.02	0.03	
499	L 2x2x1/4	1	2554	184	-0.03		0.00		0.02	0.04	

Prepared by:

Code: AISC-LRFD Page: 2 Date: 7/25/23 -15:31-

		Resu	ılts	Sui	mmar	у -	Tabl	е			
							C	4 P A C	ITY		
Beam	Section	Com	Defl L/	Slen	Axial	Dir	Shear	Mom	LTB	Combined Axial+Mom	
						MI	0.00	0.03	0.00	-	
500	L 2x2x1/4	2	1932	184	-0.04	MJ		0.01	0.01	0.06	
			9				0.00	0.04	0.00		
501	L 2x2x1/4	4	882	240	-0.31		0.00	0.01	0.01	0.34	**:
						MI	0.00	0.07	0.00		_
502	L 2x2x1/4	6	1129	240	-0.28	MJ	0.00	0.03	0.04	0.32	**:
			- 1	- 1		MI	0.00	0.05	0.00		
503	L 2x2x1/4	2	787	240	-0.24		0.00	0.03	0.04	0.29	**1
							0.00	0.08	0.00		0.0
504	L 2x2x1/4	6	1772	240	-0.31	MJ	0.00	0.02	0.02	0.35	***
						MI	0.00	0.03	0.00		
505	L 2x2x1/4	6	2297	240	-0.31	MJ	0.00	0.03	0.04	0.33	**
		1 1					0.00	0.03	0.00		
506	L 2x2x1/4	4	850	240	-0.26	MJ		0.01	0.02	0.29	**
						MI	0.00	0.07	0.00		

Prepared by:

Code: AISC-LRFD Page: 1 Date: 7/25/23

Detailed Results Table for Beam 93 - 1

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch

Beam: 2 1 X2 (Major axis)
93, 81, 99,
100, 82, 83, 13.00

CONSTRAINTS

DESIGN DATA

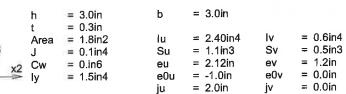
- Sections : Check - Steel Grade: A36 - Kx = 1.00 - Ky = 1.00

- Allow. Slend.: 200 (compr.) 300 (tens.)

- Allowable Deflection: 1/240 - Tension Area Reduction Factor: 1.00

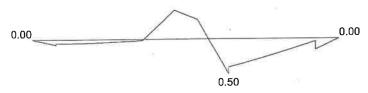
- Building type : Unbraced

Section: L 3x3x5/16

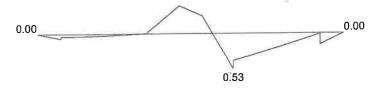


DESIGN COMBINATION = 3

Mu (M2) Moment Diagram



Max. AXIAL Force = 0.05 (tens.), -0.80 (compr.) Max. SHEAR Force = 0.84 Mv (M3) Moment Diagram



Max. AXIAL Force = 0.05 (tens.), -0.80 (compr.) Max. SHEAR Force = 0.02

SECTION CLASSIFICATION: *** COMPACT ***

Limiting Ratios: Compact Non-Compact d/t= 9.65 < 12.8 12.8

(Fy= 36.0)

b/t= 9.65 < 15.3

DESIGN	EQUATION	FACTORS	VALUES	RESULT
M3 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 0.51	M = 0.53 Mn = 1.89	0.31
V3 Shear (F2-1)	Vu/(.9*Vn)<1.00 Vn=0.6*Fy*Av	Av = 0.84	Vu = 0.84 Vn = 18.14	0.05

Prepared by:

Code: AISC-LRFD

Page: 2 Date: 7/25/23 ——15:36—

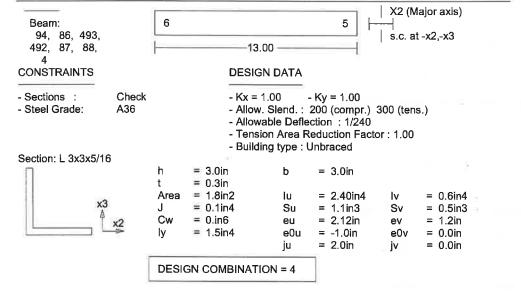
Detailed Results Table for Beam 93 - 1

Moments: kips*foot, Forces: kips, Stresses: ksi, Section prop.: inch

DESIGN	EQUATION	FACTORS	VALUES	RESULT
M2 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 1.13	M = 0.50 Mn = 4.25	0.13
Deflection	defl. < 1.00 L / 240		defl = 0.11890	0.18
Axial Force (4-1),(4-2)	Pu < 1.00 0.90AgFcr	(kL/r)x =72 (kL/r)y =112 λc = 1.26	Pu = 0.80 Ag = 1.78 Fcr = 18.57	0.03
Lateral Torsional Buckling (5-6)	M < 1.00 0.9Mn Critical Segment from at: Long leg tip Segment End Momen		M = 0.50 Mn = 3.59 My = 3.40 Mob = 8.55	0.15
Combined Forces (compress.) (H1-1b)	Pu Mux Muy 2φPn φMnx φMny < 1.00 Critical Segment from at: Long leg tip	Cmx = 1.00 Cmy = 0.85 Pex = 99.07 Pey = 40.51 Mnx = 3.59 (0.01 + 0.15 + 0.00 to 13.00	Mux = 0.50 Muy = 0.53 B1x = 1.01 B1y = 1.00 Mny = 2.07	0.46

Detailed Results Table for Beam 94 - 4

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch



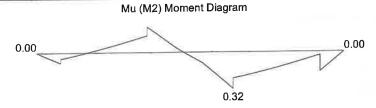
Prepared by:

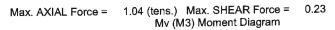
Code: AISC-LRFD Page: 3 Date: 7/25/23

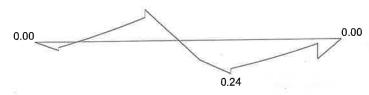
-15:36-

Detailed Results Table for Beam 94 - 4

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch







Max. AXIAL Force = 1.04 (tens.) Max. SHEAR Force =

SECTION CLASSIFICATION: *** COMPACT ***

<

Limiting Ratios:

Compact Non-Compact

d/t= 9.65 b/t= 9.65

12.8 15.3

12.8

(Fy= 36.0)

DESIGN	EQUATION	FACTORS	VALUES	RESULT
M3 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 0.51	M = 0.24 Mn = 1.89	0.14
V3 Shear (F2-1)	Vu/(.9*Vn)<1.00 Vn=0.6*Fy*Av	Av = 0.84	Vu = 0.23 Vn = 18.14	0.01
M2 Moment (A-F1-1) without LTB	M 	Z = 1.13	M = 0.32 Mn = 4.25	0.08
Deflection	defl. < 1.00 L / 240		defl = 0.12715	0.20
Axial Force (D1-1)	Pu 	(kL/r)x =169 (kL/r)y =265	Pu = 1.04 Ag = 1.78 Fy = 36.00	0.02
Lateral Torsional Buckling (5-6)	M < 1.00 0.9Mn Critical Segment from at: Long leg tip Segment End Momen		M = 0.32 Mn = 3.59 My = 3.40 Mob = 8.55	0.10

Prepared by:

Code: AISC-LRFD

Page: 4 Date: 7/25/23 ——15:36—

Detailed Results Table for Beam 94 - 4

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch

DESIGN	EQUATION	FACTORS	VALUES	RESULT
Combined Forces (compress.) (H1-1b)	Pu Mux Muy 2 Pu + + + 2 NMNx NMNy < 1.00 Critical Segment from at: Long leg tip	· ·	Mux = 0.32 Muy = 0.24 B1x = 1.00 B1y = 1.00 Mny = 2.07	0.23

Detailed Results Table for Beam 10 - 90

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch



- Sections : - Steel Grade:

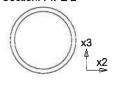
Check A53 -Kx = 1.00 -Ky = 1.00

Allow. Slend.: 200 (compr.) 300 (tens.)Allowable Deflection: 1/240

- Tension Area Reduction Factor : 1.00

- Building type : Unbraced

Section: PIPE 2



D = 2.37in= 0.2int Area = 1.1in2= 0.7in413 = 0.7in4= 1.3in4**Z**2 = 0.8in3**Z**3 = 0.8in3Cw 0.in6 е3 = 1.2in= 1.2in

DESIGN COMBINATION = 6

M2 Moment Diagram



Max. AXIAL Force = 0.63 (tens.), -0.33 (compr.) Max. SHEAR Force = 0.09

Prepared by:

Code: AISC-LRFD Page: 5 Date: 7/25/23

-15:36-

Detailed Results Table for Beam 10 - 90

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch

SECTION CLASSIFICATION: *** COMPACT ***

Limiting Ratios: d/t= 15.46

Compact Non-Compact 94.3

59.1

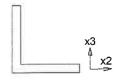
(Fy=35.0 R=-0.017)

DESIGN	EQUATION	FACTORS	VALUES	RESULT
V2 Shear (F2-1)	Vu/(.9*Vn)<1.00 Vn=0.6*Fy*Av	Av = 0.64	Vu = 0.09 Vn = 13.53	0.01
M3 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 0.76	M = 0.29 Mn = 2.22	0.15
M2 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 0.76	M = 0.16 Mn = 2.22	0.08
Deflection	defl. < 1.00 L / 240	(F)	defl = 0.05157	0.17
Axial Force (D1-1)	Pu < 1.00 0.90AgFy	(kL/r)x =15 (kL/r)y =15	Pu = 0.63 Ag = 1.07 Fy = 35.00	0.02
Combined Forces (tension) (H1-1b)	Pu Mux Muy 20Pn 00Mnx 00Mny < 1.00		Mux = 0.16 Muy = 0.29	0.23

Detailed Results Table for Beam 504

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch X2 (Major axis) 29 41 Beam: s.c. at -x2,-x3 504 -7.81 **DESIGN DATA** CONSTRAINTS -Kx = 1.00- Ky = 1.00Check - Sections : - Allow. Slend.: 200 (compr.) 300 (tens.) A36 - Steel Grade: - Allowable Deflection: 1/240 - Tension Area Reduction Factor: 1.00

Section: L 2x2x1/4



= 2.0in = 2.0inb = 0.2in t = 0.6in4= 0.1in4= 0.9in2lu I٧ Area = 0.2in3= 0.0in4Su = 0.4 in 3Sv J = 0.8in = 1.4inev = 0.in6eu Cw = 0.0in= 0.3in4e0u = -0.7in e0v = 0.0in= 1.3inju jν

- Building type : Unbraced

DESIGN COMBINATION = 6

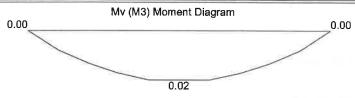
Prepared by:

Code: AISC-LRFD

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Detailed Results Table for Beam 504

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch



Max. AXIAL Force = -1.16 (compr.) Max. SHEAR Force = 0.00

SECTION CLASSIFICATION: *** COMPACT ***

Limiting Ratios:

Compact Non-Compact

d/t= 8.06 b/t= 8.06 12.8 15.3 12.8 (Fy= 36.0)

DESIGN	EQUATION	FACTORS	VALUES	RESULT
M3 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 0.17	M = 0.02 Mn = 0.65	0.03
M2 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 0.39	M = 0.01 Mn = 1.48	0.01
Deflection	defl. L / 240		defl = 0.03387	0.09
Axial Force (4-1),(4-2)	Pu < 1.00 0.90AgFcr	(kL/r)x =154 (kL/r)y =240 λc = 2.69	Pu = 1.16 Ag = 0.94 Fcr = 4.37	0.31
Lateral Torsional Buckling (5-6)	M < 1.00 0.9Mn Critical Segment from at: Long leg tip Segment End Momen		M = 0.01 Mn = 1.25 My = 1.18 Mob = 3.03	0.01
Combined Forces (compress.) (H1-1a)	Pu 8Mux 8Muy — + — + —	Cmy = 0.85 Pex = 11.40 Pey = 4.68 Mnx = 1.25 (0.31 + 0.01 + 0	Mux = 0.01 Muy = 0.02 B1x = 1.11 B1y = 1.13 Mny = 0.70 0.02)	0.35

Prepared by:

Code: AISC-LRFD

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Detailed Results Table for Beam 41 - 112

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch X2 (Major axis) 6 Beam: 40 s.c. at -x2,-x3 41, 112 16.40

CONSTRAINTS

DESIGN DATA

- Sections : Check -Kx = 1.00- Ky = 1.00

- Steel Grade: A36

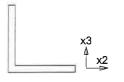
- Allow. Slend.: 200 (compr.) 300 (tens.)

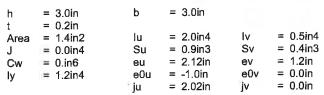
- Allowable Deflection : 1/240

- Tension Area Reduction Factor: 1.00

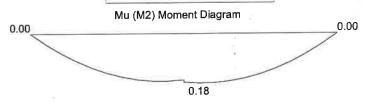
- Building type : Unbraced

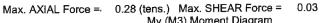
Section: L 3x3x1/4

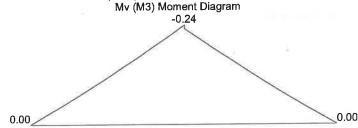




DESIGN COMBINATION = 2







Max. AXIAL Force = 0.28 (tens.) Max. SHEAR Force =

SECTION CLASSIFICATION: *** COMPACT ***

15.3

Limiting Ratios: d/t = 12.10

<

b/t = 12.10

Compact Non-Compact 12.8 12.8

(Fy= 36.0)

DESIGN	EQUATION	FACTORS	VALUES	RESULT
M3 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 0.42	M = 0.24 Mn = 1.57	0.17
M2 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 0.93	M = 0.18 Mn = 3.49	0.06

Prepared by:

Code: AISC-LRFD

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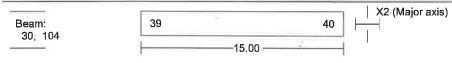
Detailed Results Table for Beam 41 - 112

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch

DESIGN	EQUATION	FACTORS	VALUES	RESULT
Deflection	defl. < 1.00 L / 240		defl = 0.62662	0.76
Axial Force (D1-1)	Pu < 1.00 0.90AgFy	(kL/r)x =212 (kL/r)y =332	Pu = 0.28 Ag = 1.44 Fy = 36.00	0.01
Lateral Torsional Buckling (5-6)	M < 1.00 0.9Mn Critical Segment from at: Long leg tip Segment End Momen		M = 0.18 Mn = 2.27 My = 2.79 Mob = 3.25	0.09
Combined Forces (compress.) (H1-1b)	Pu Mux Muy 2φPn φMnx φMny <1.00 Critical Segment from at: Short leg tip	Pey = 3.73 Mnx = 3.49 (0.00 + -0.06 + -	Mux = 0.18 Muy = 0.24 B1x = 1.00 B1y = 1.00 Mny = 1.73 0.15)	0.21

Detailed Results Table for Beam 30 - 104

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch



CONSTRAINTS

- Sections : Check

DESIGN DATA

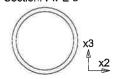
-Kx = 1.00 -Ky = 1.00

- Steel Grade: A53

Check

- Allow. Slend.: 200 (compr.) 300 (tens.)
- Allowable Deflection : 1/240
- Tension Area Reduction Factor: 1.00
- Building type : Unbraced

Section: PIPE 3



D = 3.50in t = 0.2in

= 3.02in4 Area = 2.23in212 13 = 3.02in4= 6.03in4 **Z**2 = 2.33in3 **Z**3 = 2.33in3Cw = 0.in6е3 = 1.8ine2 = 1.8in

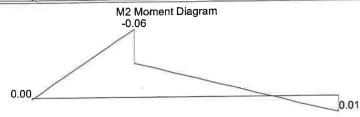
DESIGN COMBINATION = 6

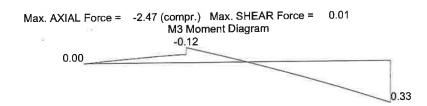
Prepared by:

Code: AISC-LRFD Page: 9 Date: 7/25/23 ——15:36———

Detailed Results Table for Beam 30 - 104

Moments: kips*foot , Forces: kips , Stresses: ksi , Section prop.: inch





Max. AXIAL Force = -2.47 (compr.) Max. SHEAR Force = 0.05

SECTION CLASSIFICATION: *** COMPACT ***

Limiting Ratios:

Compact Non-Compact

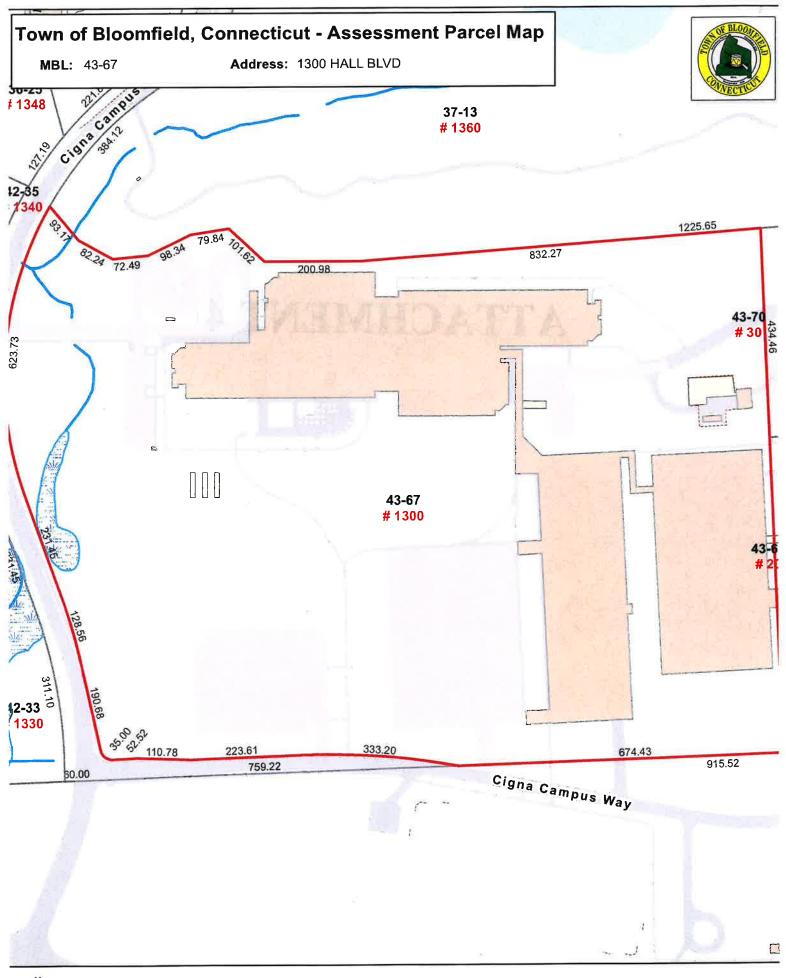
d/t= 16.16

59.1 94.3

(Fy=35.0 R=0.032)

DESIGN	EQUATION	FACTORS	VALUES	RESULT
M3 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 2.33	M = 0.33 Mn = 6.80	0.05
M2 Moment (A-F1-1) without LTB	M 0.9Mn < 1.00	Z = 2.33	M = 0.06 Mn = 6.80	0.01
Deflection	defl. L / 240 < 1.00		defl = 0.02724	0.04
Axial Force (E2-1)	Pu < 1.00 0.85AgFcr	(kL/r)x =141 (kL/r)y =141 λc = 1.56	Pu = 2.47 Ag = 2.23 Fcr = 12.56	0.10
Combined Forces (compress.) (H1-1b)	Pu	Cmx = 1.00 Cmy = 0.85 Pex = 31.93 Pey = 31.93	Mux = 0.06 Muy = 0.33 B1x = 1.08 B1y = 1.00	0.12

ATTACHMENT 4







Map Block Lot

43-67

Building #

PID

1422

Account

Property Information

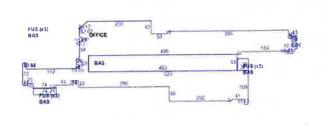
Property Location	1300 HALL BLV	D	
Owner	THE ATRIUM CT	LLC	
Co-Owner			
	2 PARK AVE 17	TH FLOOR	
Mailing Address	NEW YORK	NY	10166
Land Use	200 Cor	nmercial	
Land Class	С		
Zoning Code	DDZ		
Census Tract	4713		

Site Index	С	
Acreage	37.14	
Utilities		
Lot Setting/Desc		
Fire District	С	
Book / Page	2115/0265	

Photo



Sketch



Primary Construction Details

Year Built	1983
Building Desc.	Commercial
Building Style	Office Bldg
Building Grade	В
Stories	4
Occupancy	6.00
Exterior Walls	Stone/Masonry
Exterior Walls 2	Glass/Thermo.
Roof Style	Flat
Roof Cover	Rolled Compos
Interior Walls	Drywall
Interior Walls 2	Minimum
Interior Floors 1	Ceram Clay Til
Interior Floors 2	Carpet

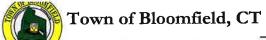
Heating Fuel	Gas
Heating Type	Forced Air
AC Type	100
Bedrooms	0
Full Bathrooms	0
Half Bathrooms	0
Extra Fixtures	0
Total Rooms	0
Bath Style	NA
Kitchen Style	NA
Bsmt Fin Area	0
Rec Rm Area	0
Bsmt Gar	0
Fireplaces	0

(*Industrial / Commercial Details)

Building Use	Commercial
Building Condition	G
Sprinkler %	100
Heat / AC	Heat/AC Pkg
Frame Type	Fireprf Steel
Baths / Plumbing	Average
Ceiling / Wall	Ceil & Wall
Rooms / Prtns	Average
Wall Height	16.00
First Floor Use	
Foundation	NA

Report Created On

7/19/2023



Property Listing Report

Map Block Lot

43-67

Building #

PID

1422

Account

Valuation Sumr	nary (A	ssessed value = 70	% of Appraised Value)	Sub Areas		
Item	Appr	aised	Assessed	Subarea Type	Gross Area (sq ft)	Living Area (sq f
Buildings	1931260	0	13518820	First Floor	158118	158118
Extras	187200		131040	Cathedral Ceiling	72478	0
Improvements				Framed Enclosed Porch	324	0
Outbuildings	628800		440160	Finished Open Porch	2707	0
Land	4871400		3409980	Finished Upper Story	414772	414772
Total	2500000	0	17500000		14	1*/
Outbuilding ar	nd Extra F	eatures				
Туре		Descriptio	n			
Pass Elv > 2000 Ca	p	4 STOPS				
Mezz - Unfin		32352 S.F.				
Load Leveller		2 Units				
Mezz - Unfin		500 S.F.				
Freight Elv > 6000	Сар	5 STOPS				
Freight Elv > 6000	Сар	5 STOPS				
Pass Elv > 2000 Ca	p	4 STOPS				-
Pass Elv > 2000 Ca	р	4 STOPS	¥.			
Pass Elv > 2000 Ca	р	4 STOPS				
Pass Elv > 2000 Ca	p	4 UNITS		Total Area	648399	572890
Sales History						
Owner of Record				Book/ Page S	ale Date Sale Pri	ce
THE ATRIUM CT LL	C			2115/0265 20	021-11-15 104500	00
METROPOLITAN TO	WER LIFE IN	SURANCE CO		1417/0307 20	007-05-16 500000	00
CONN GEN LIFE INS				0082/0231 19	900-01-01 0	



Town of Bloomfield, CT

Property Listing Report

Map Block Lot

43-67

Building #

PID

1422 Account



Sketch



Primary Construction Details

1973 Industrial Warehouse - Storage
Warehouse - Storage
Walehouse - Otoruge
С
1
1.00
Brick Veneer
,
Flat
Rolled Compos
Minimum
Drywall
Concrete
Carpet

Gas
Hot Air-No Duc
100
0
0
0
0
0
NA
NA
0
0
0
0

(*Industrial /	Commercial Details)
Building Use	Industrial
Building Condition	A
Sprinkler %	100
Heat / AC	Heat/AC Pkg
Frame Type	Masonry
Baths / Plumbing	Average
Ceiling / Wall	Sus Ceil & Wal
Rooms / Prtns	Average
Wall Height	19.00
First Floor Use	

NA

Sub Areas

Subarea Type	Gross Area (sq ft)	Living Area (sq ft)
Office Area	14464	14464
First Floor	87208	87208
-		

Subarea Type	Gross Area (sq ft)	Living Area (sq ft)
Total Area	101672	101672

Foundation



Property Listing Report

Map Block Lot

43-67

Building #

PID

1422

Account

Photo



Sketch



Primary Construction Details

Year Built	1983
Building Desc.	Commercial
Building Style	Parking Garage
Building Grade	С
Stories	1
Occupancy	1.00
Exterior Walls	Pre-cast Concr
Exterior Walls 2	NA *
Roof Style	Flat
Roof Cover	Rolled Compos
Interior Walls	Minimum
Interior Walls 2	Drywall
Interior Floors 1	Concrete
Interior Floors 2	Carpet

Heating Fuel	Typical
Heating Type	Typical
АС Туре	25
Bedrooms	0
Full Bathrooms	0
Half Bathrooms	0
Extra Fixtures	0
Total Rooms	0
Bath Style	NA
Kitchen Style	NA
Bsmt Fin Area	0
Rec Room Area	0
Bsmt Gar	0
Fireplaces	0

(*Industrial / Commercial Details)

Commercial A 0 Heat/AC Pkg Reinforced Cnc
0 Heat/AC Pkg
Heat/AC Pkg
Reinforced Cnc
None
Susp Ceil Only
Average
10.00
NA

Sub Areas

Subarea Type	Gross Area (sq ft)	Living Area (sq ft)
First Floor	101107	101107
Covered Loading Platform	n 2092	0
Finished Open Porch	1698	0
Finished Upper Story	98439	98439

Subarea Type	Gross Area (sq ft)	Living Area (sq ft)
Total Area	203336	199546



Property Listing Report

Map Block Lot

43-67

Building #

PID

1422 Account

Photo







Primary Construction Details

Year Built	2003
Building Desc.	Commercial
Building Style	Service Shop
Building Grade	С
Stories	1
Оссиралсу	1.00
Exterior Walls	Pre-finsh Metl
Exterior Walls 2	NA _
Roof Style	Flat
Roof Cover	Enam Mtl Shing
Interior Walls	Minimum
Interior Walls 2	
Interior Floors 1	Concrete
Interior Floors 2	

Heating Fuel	None
Heating Type	None
АС Туре	0
Bedrooms	0
Full Bathrooms	0
Half Bathrooms	0
Extra Fixtures	0
Total Rooms	0
Bath Style	NA
Kitchen Style	NA
Bsmt Fin Area	0
Rec Room Area	0
Bsmt Gar	0
Fireplaces	0

(*Industrial / Commercial Details)

Building Use	Commercial	
Building Condition	A	
Sprinkler %	0	
Heat / AC	None	
Frame Type	Steel	
Baths / Plumbing	None	
Ceiling / Wall	None	
Rooms / Prtns	Average	
Wall Height	19.00	
First Floor Use		
Foundation	NA	

Sub Areas

Gross Area (sq ft)	Living Area (sq ft)
1200	1200
	X
	(sq ft)

Subarea Type	Gross Area (sq ft)	Living Area (sq ft)
	-	
	_	
Total Area	1200	1200



Map Block Lot

43-67

Building #

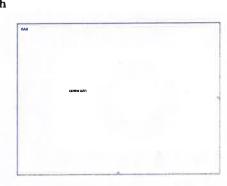
PID

1422 Account

Photo



Sketch



Primary Construction Details

Year Built	1983
Building Desc.	Commercial
Building Style	Office Bldg
Building Grade	В
Stories	1
Occupancy	1.00
Exterior Walls	Stone/Masonry
Exterior Walls 2	Concrete
Roof Style	Flat
Roof Cover	T&G/Rubber
Interior Walls	Minimum
Interior Walls 2	NA
Interior Floors 1	Concrete
Interior Floors 2	

Heating Fuel	Gas
Heating Type	Forced Air
AC Type	100
Bedrooms	0
Full Bathrooms	0
Half Bathrooms	0
Extra Fixtures	0
Total Rooms	0
Bath Style	NA
Kitchen Style	NA
Bsmt Fin Area	0
Rec Room Area	0
Bsmt Gar	0
Fireplaces	0

(*Industrial / Commercial Details)

Commercial
G
100
Heat/AC Pkg
Fireprf Steel
Average
Ceil & Wall
Light
16.00
NA

Sub Areas

Subarea Type	Gross Area (sq ft)	Living Area (sq ft)
First Floor	35104	35104

barea Type	Gross Area (sq ft)	Living Area (sq ft)
9		
7		
Total Area	35104	35104

ATTACHMENT 5



Certificate of Mailing — Firm

Name and Address of Sender Kenneth C. Baldwin, Esq. Robinson & Cole LLP 280 Trumbull Street Hartford, CT 06103	TOTAL NO. of Pieces Listed by Sender TOTAL NO. of Pieces Received at Post Office Postmaster, per (name of receiving employee)	Affix Stamp Here Postmark with Date of Receipt. neopost 07/31/2023 US POSTAGE \$003.19 ZIP 06103 041L12203937
		EX.
USPS® Tracking Number Firm-specific Identifier	Address (Name, Street, City, State, and ZIP Code™)	Postage Fee Special Handling Parcel Airlift
1. 2. 3.	Philip Schenck, Acting Town Manager Town of Bloomfield 800 Bloomfield Avenue Bloomfield, CT 06002 Justin LaFountain, Director of Land Use Town of Bloomfield 800 Bloomfield Avenue Bloomfield, CT 06002 The Atrium CT LLC 2 Park Avenue, 17th Floor New York, NY 10166	
4.		
5.		
6.		