

December 4, 2017

Melanie A. Bachman, Esq.  
Executive Director/Staff Attorney  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

Re: **Notice of Exempt Modification – Facility Modification  
20 Seles Road, Ashford, Connecticut**

Dear Ms. Bachman:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains twelve (12) antennas at the top level top of an existing 190-foot guyed lattice tower at 20 Seles Road in Ashford, Connecticut (the “Property”). The tower is owned by Cordless Data Transfer (“CDT”). Cellco’s use of the tower was approved by the Council in 2001. Cellco now intends to replace six (6) of its existing antennas with three (3) model JAHH-65B-R3B, 700 MHz antennas and three (3) model JAHH-65B-R3B, 850/2100 MHz antennas, at the same level on the tower. Cellco also intends to install nine (9) remote radio heads (“RRHs”) and two (2) HYBRIFLEX™ fiber optic antenna cables. Included in Attachment 1 are specifications for Cellco’s replacement antennas, RRHs and HYBRIFLEX™ cables.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Michael J. Zambo, First Selectman for the Town of Ashford; Michael Gardner, Ashford’s Land Use Administrator; Raymond and Kathleen Baker, the owners of the Property; and CDT, the tower owner.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco’s replacement antennas and RRHs will be installed at the top of the 190-foot tower.

Melanie A. Bachman, Esq.  
December 4, 2017  
Page 2

2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. A cumulative General Power Density table for Cellco's modified facility is included behind Attachment 2.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower and its foundation can support Cellco's proposed modifications. (See Structural Analysis included in Attachment 3).

A copy of the parcel map and owner information for the Property is included in Attachment 4. A Certificate of Mailing verifying that this filing was sent to municipal officials and the owners of the Property is included in Attachment 5.

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

Copy to:

Michael J. Zambo, Ashford First Selectman  
Michael Gardner, Ashford Land Use Administrator  
Raymond and Kathleen Baker  
Cordless Data Transfer  
Tim Parks

# **ATTACHMENT 1**



## JAHH-65B-R3B

**8-port sector antenna, 2x 698–787, 2x 824–894 and 4x 1695–2360 MHz, 65° HPBW, 3x RET and low bands have diplexers. Internal SBT's on first LB(Port 1) and first HB (Port 5).**

- Internal SBT on low and high band allow remote RET control from the radio over the RF jumper cable
- One RET for 700MHz, one RET for 850MHz, and one RET for both high bands to ensure same tilt level for 4x Rx or 4x MIMO
- Internal filter on low band and interleaved dipole technology providing for attractive, low wind load mechanical package
- Separate RS-485 RET input/output for low and high band

### Electrical Specifications

Frequency Band, MHz	698–787	824–894	1695–1880	1850–1990	1920–2200	2300–2360
Gain, dBi	14.5	15.8	18.0	18.4	18.5	18.8
Beamwidth, Horizontal, degrees	67	65	63	63	65	68
Beamwidth, Vertical, degrees	12.4	10.5	5.7	5.2	4.9	4.4
Beam Tilt, degrees	2–14	2–14	0–10	0–10	0–10	0–10
USLS (First Lobe), dB	18	18	20	20	21	23
Front-to-Back Ratio at 180°, dB	32	34	31	35	36	38
Isolation, dB	25	25	25	25	25	25
Isolation, Intersystem, dB	30	30	30	30	30	30
VSWR   Return Loss, dB	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0	1.5   14.0
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153	-153	-153	-153
Input Power per Port, maximum, watts	350	350	350	350	350	300
Polarization	±45°	±45°	±45°	±45°	±45°	±45°
Impedance	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm

### Electrical Specifications, BASTA\*

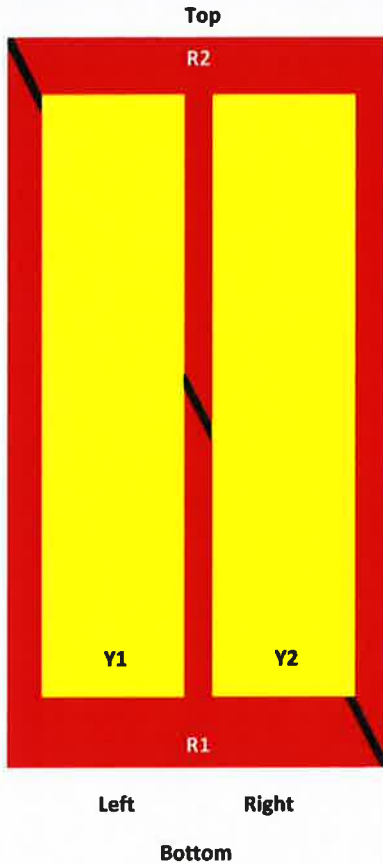
Frequency Band, MHz	698–787	824–894	1695–1880	1850–1990	1920–2200	2300–2360
Gain by all Beam Tilts, average, dBi	14.3	14.9	17.6	18.1	18.2	18.5
Gain by all Beam Tilts Tolerance, dB	±0.3	±0.5	±0.6	±0.4	±0.5	±0.6
Gain by Beam Tilt, average, dBi	2°   14.3	2°   15.0	0°   17.2	0°   17.6	0°   17.7	0°   17.9
	8°   14.3	8°   14.9	5°   17.6	5°   18.2	5°   18.3	5°   18.7
	14°   14.3	14°   15.4	10°   17.6	10°   18.2	10°   18.3	10°   18.7
Beamwidth, Horizontal Tolerance, degrees	±1.2	±1.4	±4	±2.4	±2.9	±2.7
Beamwidth, Vertical Tolerance, degrees	±0.9	±0.5	±0.3	±0.2	±0.3	±0.1
USLS, beampeak to 20° above beampeak, dB	18	17	17	18	19	18
Front-to-Back Total Power at 180° ± 30°, dB	25	24	26	29	27	29
CPR at Boresight, dB	22	23	20	21	21	24
CPR at Sector, dB	11	12	11	11	11	8

\* CommScope® supports NGMN recommendations on Base Station Antenna Standards (BASTA). To learn more about the benefits of BASTA, [download the whitepaper Time to Raise the Bar on BSAs.](#)

### Array Layout

JAHH-65B-R3B

JAHH-65A-R3B JAHH-65B-R3B JAHH-65C-R3B



Array	Freq (MHz)	Conns	RET (SRET)	AISG RET UID
R1	698-798	1-2	1	ANXXXXXXXXXXXXX1
R2	824-894	3-4	2	ANXXXXXXXXXXXXX2
Y1	1695-2360	5-6	3	ANXXXXXXXXXXXXX3
Y2	1695-2360	7-8		

View from the front of the antenna

(Sizes of colored boxes are not true depictions of array sizes)

## General Specifications

Operating Frequency Band	1695 – 2360 MHz   698 – 787 MHz   824 – 894 MHz
Antenna Type	Sector
Band	Multiband
Performance Note	Outdoor usage

## Mechanical Specifications

RF Connector Quantity, total	8
RF Connector Quantity, low band	4
RF Connector Quantity, high band	4
RF Connector Interface	4.3-10 Female
Color	Light gray

JAHH-65B-R3B

Grounding Type	RF connector body grounded to reflector and mounting bracket
Radiator Material	Aluminum   Low loss circuit board
Radome Material	Fiberglass, UV resistant
Reflector Material	Aluminum
RF Connector Location	Bottom
Wind Loading, frontal	746.0 N @ 150 km/h 167.7 lbf @ 150 km/h
Wind Loading, lateral	243.0 N @ 150 km/h 54.6 lbf @ 150 km/h
Wind Loading, rear	776.0 N @ 150 km/h 174.5 lbf @ 150 km/h
Wind Speed, maximum	241 km/h   150 mph

## Dimensions

Length	1828.0 mm   72.0 in
Width	350.0 mm   13.8 in
Depth	208.0 mm   8.2 in
Net Weight, without mounting kit	28.7 kg   63.3 lb

## Remote Electrical Tilt (RET) Information

Input Voltage	10–30 Vdc
Internal Bias Tee	Port 1   Port 5
Internal RET	High band (1)   Low band (2)
Power Consumption, idle state, maximum	2.0 W
Power Consumption, normal conditions, maximum	13.0 W
Protocol	3GPP/AISG 2.0 (Single RET)
RET Interface	8-pin DIN Female   8-pin DIN Male
RET Interface, quantity	2 female   2 male

## Packed Dimensions

Length	1975.0 mm   77.8 in
Width	456.0 mm   18.0 in
Depth	357.0 mm   14.1 in
Shipping Weight	42.0 kg   92.6 lb

## Regulatory Compliance/Certifications

Agency	Classification
RoHS 2011/65/EU	Compliant by Exemption
China RoHS SJ/T 11364-2006	Above Maximum Concentration Value (MCV)
ISO 9001:2008	Designed, manufactured and/or distributed under this quality management system



JAHH-65B-R3B

## Included Products

BSAMNT-1 — Wide Profile Antenna Downtilt Mounting Kit for 2.4 - 4.5 in (60 - 115 mm) OD round members. Kit contains one scissor top bracket set and one bottom bracket set.

## \* Footnotes

Performance Note      Severe environmental conditions may degrade optimum performance

## AirScale RRH 4T4R B5 160W AHCA

Capacity, performance, low total cost of ownership and investment protection

Nokia AirScale Remote Radio Head (RRH) AHCA supports band 5 - full band - along with 4x4 MIMO and 256QAM modulation to deliver higher data rates. It offers Nokia's unique book mounting for faster roll out and radio-integrated Passive Intermodulation (PIM) cancellation for enhanced network performance.

Furthermore, 4TX and 4RX paths in a single radio unit gives the flexibility to support 2T2R-2 sectors or 4T4R-single sector from a single unit, for cost-effective scaling of both coverage and capacity.

### Capacity and performance

AirScale RRH 4T4R delivers 160 W (4x40 W) transmit power and can support 2x2 MIMO, 4x2 MIMO and 4x4 MIMO. The radio supports 256 QAM modulation in the downlink (DL) for up to 30 percent higher throughput. The Virtual Spectrum Analyzer feature enables both uplink and downlink spectrum to be analyzed.

### Low total cost of ownership

With up to two sectors in a single radio, light weight and zero-bolt book mounting, AirScale RRH 4T4R allows operators to achieve faster roll outs and more cost-effective installation and maintenance of radios and tower space.

### Investment protection

AirScale RRH 4T4R complements the AirScale System Module, offering a complete base station solution that is software upgradeable to 5G. AirScale System



Module offers 28 Gbps capacity that can be further enhanced by chaining more modules or through Cloud RAN. AirScale RRH is part of the AirScale Base Station portfolio, the next generation Nokia base station platform, and is backwards-compatible with the Nokia Flexi Multiradio 10 Base Station to best use an operator's existing investments.



Product name	AirScale RRH 4T4R B5 160W AHCA - 473966A
Supported frequency bands	3GPP band 5
Frequencies	DL 869-894MHz, UL 824-849MHz
Number of TX/RX ports	4/4
Instantaneous Bandwidth IBW	25MHz
Occupied Bandwidth OBW	25MHz
Output power	4T4R 40 W/ 2T4R 60W
Dimensions (mm) height x width x depth	337 x 295 x 165
Volume (liters)	16.4
Weight (kg)	16
Supply Voltage / Voltage Range	DC-48V / -36V to -60V
Typical Power Consumption	207 W (ETSI 24h Avg - 4x20W mode)
Antenna ports	4TX/4RX, 4.3-10+
Optical ports	2 x CPRI 9.8 Gbps
ALD control interfaces	AISG3.0 from ANT1, 2, 3, 4 and RET (Power supply ANT1 and ANT3)
Other interfaces	External alarm MDR-26 serial connector (4 inputs, 1 output) DC circular power connector
Operational temperature range	-40°C to 55°C (with no solar load)
Ingress protection class	IP65
Installation options	Pole or wall, RAS, vertical or horizontal book mount
Surge protection	Class II 5kA

Nokia is a registered trademark of Nokia Corporation. Other product and company names mentioned herein may be trademarks or trade names of their respective owners.

Nokia Oyj  
 Karaportti 3  
 FI-02610 Espoo  
 Finland  
 Tel. +358 (0) 10 44 88 000

Product code: SR1611002341EN (April)

# ALCATEL-LUCENT B13 RRH4X30-4R

Alcatel-Lucent B13 Remote Radio Head 4x30-4R is the newest addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering.

**Supporting 2Tx/4Tx MIMO and 4-way Rx diversity**, Alcatel-Lucent B13 RRH4x30-4R allows operators to have a compact radio solution to deploy LTE in the 700U band (700 MHz, 3GPP band 13), providing them with the means to achieve high capacity, high quality and high coverage with minimum site requirements.

The Alcatel-Lucent B13 RRH4x30-4R product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x60 W or 4x30 W RF output power. It supports also 4-way Rx diversity and up to 10MHz instantaneous bandwidth.

The Alcatel-Lucent B13 RRH4x30-4R is a near zero-footprint solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

Its compactness and slim design makes the Alcatel-Lucent B13 RRH4x30-4R easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

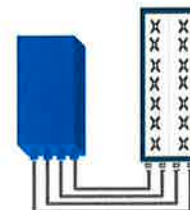


## FEATURES

- Supporting LTE in 700 MHz band (700U, 3GPP band 13)
- LTE 2Tx or 4Tx MIMO (SW switchable)
- Output power: Up to 2x60W or 4x30W
- 10MHz LTE carrier with 4Rx Diversity
- Convection-cooled (fan-less)
- Supports AISG 2.0 ALD devices (RET, TMA) through RS485 or RF ports

## BENEFITS

- Compact to reduce additional footprint when adding LTE in 700U band
- MIMO scheme operation selection (2Tx or 4Tx) by software only
- Improves downlink spectral efficiency through MIMO4
- Increases LTE coverage thanks to 4Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options: Pole or Wall



4x30W with 4T4R  
or  
2x60W with 2T4R

Can be switched between modes via SW w/o site visit

## TECHNICAL SPECIFICATIONS

Features & performance	
Number of TX/RX paths	4 duplexed (either 4T4R or 2T4R by SW)
Frequency band	U700 (C) (3GPP bands 13): DL: 746 - 756 MHz / UL: 777 - 787 MHz
Instantaneous bandwidth - #carriers	10MHz – 1 LTE carrier (in 10MHz occupied bandwidth)
LTE carrier bandwidth	10 MHz
RF output power	2x60W or 4x30W (by SW)
Noise figure – RX Diversity scheme	2 dB typ. (<2.5 dB max) – 2 or 4 way Rx diversity
Sizes (HxWxD) in mm (in.)	550 x 305 x 230 (21.6" x 12.0" x 9") (with solar shield)
Volume in L	38 (with solar shield)
Weight in kg (lb) (w/o mounting HW)	26 (57.2) (with solar shield)
DC voltage range	-40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
DC power consumption	550W typical @100% RF load ( In 2Tx or 4Tx mode)
Environmental conditions	-40°C (-40°F) /+55°C (+131°F) IP65
Wind load (@150km/h or 93mph)	Frontal:<200N / Lateral :<150N
Antenna ports	4 ports 7/16 DIN female (50 ohms) VSWR < 1.5
CPRI ports	2 CPRI ports (HW ready for Rate7, 9.8 Gbps) SFP single mode dual fiber
AISG interfaces	1 AISG2.0 output (RS485) Integrated Smart Bias Tees (x2)
Misc. Interfaces	4 external alarms (1 connector) – 4 RF Tx & 4 RF Rx monitor ports - 1 DC connector (2 pins)
Installation conditions	Pole and wall mounting
Regulatory compliance	3GPP 36.141 / 3GPP 36.113 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27

www.alcatel-lucent.com Alcatel, Lucent, Alcatel-Lucent and the Alcatel-Lucent logo are trademarks of Alcatel-Lucent. All other trademarks are the property of their respective owners. The information presented is subject to change without notice. Alcatel-Lucent assumes no responsibility for inaccuracies contained herein. Copyright © 2014 Alcatel-Lucent. All Rights Reserved

# ALCATEL-LUCENT B66A RRH4X45

The Alcatel-Lucent B66a Remote Radio Head 4x45 is the newest addition of Remote Radio Head to the extended product line of Alcatel-Lucent's distributed Base Station solutions, aimed at facilitating smooth RF site acquisition and related civil engineering. Its operational range covers beyond that of B4 (AWS) and B10 (AWS+).

**Supporting 2Tx/4Tx MIMO and 2-way/4-way Rx diversity**, the Alcatel-Lucent B66a RRH4x45 allows operators to have a compact radio solution to deploy LTE in the 2100 band (3GPP band 4, 10, and 66), providing them with the means to achieve high capacity, high quality, high reliability, large instantaneous bandwidth, and high coverage with minimum site requirements.

The Alcatel-Lucent B66a RRH4x45 product has four transmit RF paths, offering the possibility to **select, via software only, 2Tx or 4Tx MIMO configurations** with either 2x90W or 4x45W RF output power. It also supports 4-way Rx diversity at the 70 MHz instantaneous bandwidth.



The Alcatel-Lucent B66a RRH4x45 is a compact (near zero-footprint) solution and operates noise free, simplifying negotiations with site property owners and minimizing environmental impacts.

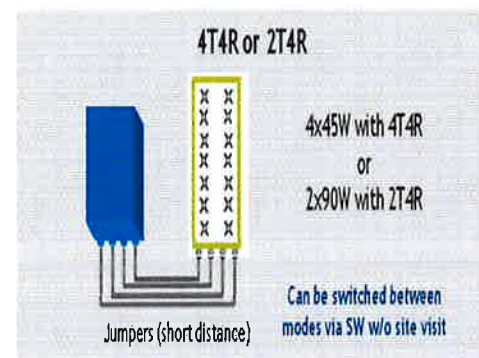
Its compactness and slim design makes the Alcatel-Lucent B66a RRH4x45 easy to install close to the antenna: operators can therefore locate this Remote Radio Head where RF design conditions are deemed ideal, minimizing trade-offs between available sites and RF optimum sites, together with reducing the RF feeder needs and installation costs.

## FEATURES

- Supporting LTE in 2110 - 2180 MHz band/DL, 1710-1780MHz/UL (3GPP band 4, 10, and 66a)
- LTE 2Tx or 4Tx MIMO (SW selectable)
- Configuration: 2T2R/2T4R/4T4R
- Output power: Up to 2x90W or 4x45W (SW configurable)
- 70MHz LTE carrier with 4Rx Diversity
- Convection-cooled (fan-less)
- Supports AISG 2.0 ALD devices (RET, TMA) through RS485 or RF ports

## BENEFITS

- Compact to reduce additional footprint when adding LTE in AWS 1-3 band
- Selection of MIMO configuration (2Tx or 4Tx) by software only
- Improves downlink spectral efficiency through 4Tx MIMO
- Increases LTE coverage thanks to 4Rx diversity capability and best in class Rx sensitivity
- Flexible mounting options: Pole or Wall



## TECHNICAL SPECIFICATIONS

Features & Performance	
Number of TX/RX paths	4 duplexed (either 4T4R or 2T4R selectable by SW)
Frequency band	AWS 1-3, B4/B66a DL: 2110-2180 MHz / UL: 1710-1780 MHz
Instantaneous bandwidth - #carriers	70 MHz – 4 LTE MIMO carriers (in 70 MHz occupied bandwidth)
LTE carrier bandwidth	5, 10, 15, 20 MHz
RF output power	2x90W or 4x45W (selectable by SW)
Noise figure – RX Diversity scheme Receiver Sensitivity (FRC A1-3)	2 dB typical (<2.5 dB max) – 2 or 4 way Rx diversity -104.5 dBm maximum
Sizes (HxWxD) in mm (in.)	655x299x182 (25.8x11.8x7.2) (with solar shield) 640x290x160 (25.2x11.4x6.3) (without solar shield)
Volume in Liters	35.5 (with solar shield) 29.7 (without solar shield)
Weight in kg (lb) (w/o mounting HW)	25.8kg (56.8lb) (with solar shield)
DC voltage range	Nominal: -48V, -40.5 to -57V at full performance, -38 to -57V with relaxation on power consumption
DC power consumption	750W typical @100% RF load (in 2Tx or 4Tx mode); Add 58W for 2A*29V for AISG
Environmental conditions	-40°C (-40°F) / +55°C (+131°F) UL50E Type 4 Enclosure
Wind load (@150km/h or 93mph)	250N (56lb) Frontal/150N (34lb) Lateral
Antenna ports	4 ports 4.3-10 female (50 ohms) VSWR < 1.5
CPRI ports	2 CPRI ports (HW ready for Rate 7, 9.8 Gbps) SFP: SMDF (HW supports also SMSF and MMDF)
AISG interfaces	1 AISG 2.0 output (RS485) Integrated Smart Bias Tees (x2)
Misc. Interfaces	4 external alarms (1 connector) 1 DC connector (2 pins)
Installation conditions	Pole and wall mounting
Regulatory compliance	3GPP 36.141 / 3GPP 36.113 / GR-487 / GR-1089-CORE / GR-3108-CORE / UL 60950-1 / FCC Part 27 / FCC Part 15 / GR-3178-CORE

www.alcatel-lucent.com Alcatel, Lucent, Alcatel-Lucent and the Alcatel-Lucent logo are trademarks of Alcatel-Lucent. All other trademarks are the property of their respective owners. The information presented is subject to change without notice. Alcatel-Lucent assumes no responsibility for inaccuracies contained herein.  
Copyright © 2016 Alcatel-Lucent. All Rights Reserved



**HYBRIFLEX™ RRH Hybrid Feeder Cabling Solution, 1-5/8", Single-Mode Fiber**

**Product Description**

RFS' HYBRIFLEX Remote Radio Head (RRH) hybrid feeder cabling solution combines optical fiber and DC power for RRHs in a single lightweight aluminum corrugated cable, making it the world's most innovative solution for RRH deployments.

It was developed to reduce installation complexity and costs at Cellular sites. HYBRIFLEX allows mobile operators deploying an RRH architecture to standardize the RRH installation process and eliminate the need for and cost of cable grounding. HYBRIFLEX combines optical fiber (multi-mode or single-mode) and power in a single corrugated cable. It eliminates the need for junction boxes and can connect multiple RRHs with a single feeder. Standard RFS CELLFLEX® accessories can be used with HYBRIFLEX cable. Both pre-connectorized and on-site options are available.

**Features/Benefits**

- Aluminum corrugated armor with outstanding bending characteristics - minimizes installation time and enables mechanical protection and shielding
- Same accessories as 1 5/8" coaxial cable
- Outer conductor grounding - Eliminates typical grounding requirements and saves on installation costs
- Lightweight solution and compact design - Decreases tower loading
- Robust cabling - Eliminates need for expensive cable trays and ducts
- Installation of tight bundled fiber optic cable pairs directly to the RRH - Reduces CAPEX and wind load by eliminating need for interconnection
- Optical fiber and power cables housed in single corrugated cable - Saves CAPEX by standardizing RRH cable installation and reducing installation requirements
- Outdoor polyethylene jacket - Ensures long-lasting cable protection



Figure 1: HYBRIFLEX Series

**Technical Specifications**

<b>Dimensions</b>			
Outer Conductor Armor	Corrugated Aluminum	(mm (in))	46.5 (1.83)
Jacket	Polyethylene, PE	(mm (in))	50.3 (1.98)
UV-Protection	Individual and External Jacket		Yes
<b>Mechanical Properties</b>			
Weight, Approximate		(kg/m (lb/ft))	1.9 (1.30)
Minimum Bending Radius, Single Bending		(mm (in))	200 (.8)
Minimum Bending Radius, Repeated Bending		(mm (in))	500 (20)
Recommended/Maximum Clamp Spacing		(m (ft))	1.0 / 1.2 (3.25 / 4.0)
<b>Electrical Properties</b>			
DC-Resistance Outer Conductor Armor		( $\Omega$ /km ( $\Omega$ /1000ft))	068 (0.205)
DC-Resistance Power Cable, 8.4mm <sup>2</sup> (8AWG)		( $\Omega$ /km ( $\Omega$ /1000ft))	2.1 (0.307)
<b>Optical Properties</b>			
Version			Single-mode OM3
Quantity, Fiber Count			16 (8 pairs)
Core/Clad		( $\mu$ m)	50/125
Primary Coating (Acrylate)		( $\mu$ m)	245
Buffer Diameter, Nominal		( $\mu$ m)	900
Secondary Protection, Jacket, Nominal		(mm (in))	2.0 (0.08)
Minimum Bending Radius		(mm (in))	104 (4.1)
Insertion Loss @ wavelength 850nm		dB/km	3.0
Insertion Loss @ wavelength 1310nm		dB/km	1.0
Standards (Meets or exceeds)			UL94-V0, UL1666 RoHS Compliant
<b>DC Power Cable Properties</b>			
Size (Power)		(mm (AWG))	8.4 (8)
Quantity, Wire Count (Power)			16 (8 pairs)
Size (Alarm)		(mm (AWG))	0.8 (18)
Quantity, Wire Count (Alarm)			4 (2 pairs)
Type			UV protected
Strands			19
Primary Jacket Diameter, Nominal		(mm (in))	6.8 (0.27)
Standards (Meets or exceeds)			NFPA 130, IEC 60332-1-2 UL Type XHHW-2, UL 44 UL-LS Limited Smoke, UL VW-1 IEEE-383 (1974), IEEE1202/FT4 RoHS Compliant
<b>Operating Conditions</b>			
Installation Temperature		(°C (°F))	-40 to +65 (-40 to 149)
Operation Temperature		(°C (°F))	-40 to +65 (-40 to 149)

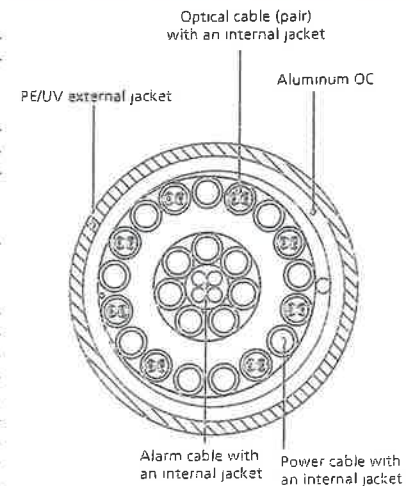


Figure 2: Construction Detail

All information contained in the present datasheet is subject to confirmation at time of ordering.

# **ATTACHMENT 2**

Site Name: Ashford W Tower Height: 190ft		General		Power		Density							
CARRIER	# OF CHAN.	WATTS ERP	HEIGHT	CALC. POWER DENS	FREQ.	MAX. PERMISS. EXP.	FRACTION MPE	Total					
*Nextel	9	100	182	851	0.0104	0.5673	0.18%						
*Sprint	2	557	170.5	1900	0.0148	1.0000	0.15%						
*AT&T	2	565	138	880	0.0233	0.5867	0.40%						
*AT&T	2	875	138	1900	0.0361	1.0000	0.36%						
*AT&T	1	283	138	880	0.0058	0.5867	0.10%						
*AT&T	4	525	138	1900	0.0433	1.0000	0.43%						
*AT&T	1	1771	138	734	0.0366	0.4893	0.75%						
<b>Verizon PCS</b>	<b>1</b>	<b>0</b>	<b>188</b>	<b>0.0000</b>	<b>1970</b>	<b>1.0000</b>	<b>0.00%</b>						
<b>Verizon Cellular</b>	<b>9</b>	<b>400</b>	<b>189</b>	<b>0.0362</b>	<b>869</b>	<b>0.5793</b>	<b>6.26%</b>						
<b>Verizon 850 LTE</b>	<b>1</b>	<b>3710</b>	<b>188</b>	<b>0.0377</b>	<b>869</b>	<b>0.5793</b>	<b>6.52%</b>						
<b>Verizon AWS</b>	<b>1</b>	<b>8326</b>	<b>188</b>	<b>0.0847</b>	<b>2145</b>	<b>1.0000</b>	<b>8.47%</b>						
<b>Verizon 700</b>	<b>1</b>	<b>2063</b>	<b>188</b>	<b>0.0210</b>	<b>746</b>	<b>0.4973</b>	<b>4.22%</b>						<b>27.83%</b>
* Source: Siting Council													



# **ATTACHMENT 3**



# FRED A. NUDD CORPORATION

1743 ROUTE 104, BOX 577  
ONTARIO, NY 14519  
(315) 524-2531 FAX (315) 524-4249

[www.nuddtowers.com](http://www.nuddtowers.com)



Mark LeGault  
Cordless Data Transfer, Inc.  
600 Old Hartford Road  
Colchester, CT 06415  
October 19, 2017

Nudd Job Number: 117-23150

Site Location: 20 Seles Road, Ashford, CT 06258, Windham County (Lat. & Long. = 41-51-48, -72-10-57)

Subject: Structural Analysis of an existing 190 ft Guyed Tower

Fred A. Nudd Corporation has completed a three-dimensional, finite element model structural analysis of the above noted guyed tower. This tower was analyzed considering appurtenance loads noted in the appurtenance loading table on the following page. The design loading criteria and strength design are per the ANSI/TIA-222-G standard, which is the recommended design standard per the 2012 International Building Code (Sec. 1609 & 3108), and the 2016 Connecticut State Building Code. Tower and foundation dimensions have been taken from drawings by Fred A. Nudd, project number 00-6111A-1 & 98-6111-2, dated July 28, 2000 & June 1998, respectively. Geotechnical information was taken from a subsurface exploration report by Tower Engineering Professionals, Inc., project number 090004.13, dated September 22, 2009. Design criteria per each analysis are noted on the following page. The tower is assumed to be in good, undamaged and equivalent to as new condition and has been maintained / inspected per criteria by TIA-222.

The purpose of this analysis is to determine the structure's ability to support new Verizon equipment installed at a rad center of 190 ft above ground level (AGL). The new equipment to be installed, which includes antennas, diplexers, and associated hardware are listed on the following page in the appurtenance loading table.

Results of the analysis indicate the tower will be able to support the design loads noted in the appurtenance loading table on the following page. Specific section design loads, capacities and stress ratios are provided on the following pages. Maximum member usage was found to be 100%.

The tower base foundation and anchors were analyzed considering onsite soil information from the aforementioned geotechnical report. Based on this analysis, the foundation and anchors will be able support the proposed appurtenance loading, in addition to the existing wireless equipment and tower superstructure. Specific design loads, capacities and stress ratios are provided on the following pages.

In conclusion, the tower superstructure and substructure can support the listed existing and proposed appurtenance loading.

We trust this report satisfies your needs. Please contact us with any questions or concerns regarding this report.

Best Regards,

Fred. A. Nudd Corporation



**Code Design Criteria**

TIA-222-G

Windspeed = 126 mph, 3-second gust,  $V_{ult} = 100$  mph, 3-second gust,  $V_{asd}$

Exposure = B

Radial Ice = 1.00 inch

Ice Windspeed = 50 mph, 3-second gust

**Appurtenance Loading – Existing Equipment to Remain on Tower**

Elevation (ft)	Carrier	Antenna	Mount	Coax (in)
190	Verizon	(2) Antel LPA-80080-4CF (4) Antel LPA-80063-4CF	(3) 12 ft Sector Frame	(6) 1-5/8
178	--	-	(3) 12 ft Boom / Frame	-
170.5	Sprint	(3) RFS APXV9ERR18-C-A20 (3) Alcatel Lucent 2x50W RRH, 800 MHz (3) Alcatel Lucent 4x40W RRH, 1900 MHz	(3) 12 ft Boom / Frame	(3) 1-1/4 Hybriflex
150	--	-	(3) 12 ft Boom / Frame	-
138	AT&T	(6) Powerwave 7770 (6) TMA (6) Diplextors (3) KMW AM-X-CD-17-65-00T-RET (1) Raycap DC6-48-60-18-8F (6) Ericsson RRU11	(3) 12 ft Boom / Frame	(12) 1-5/8 (2) 3/4 DC Cables (1) 3/8 Fiberline

- Note elevation is measured from grade to center of antenna.

**Appurtenance Loading – Proposed Equipment Configuration for Verizon**

Height (ft)	Carrier	Appurtenance	Mount	Coax (in)
190	Verizon	(6) Commscope JAHH-65B-R3B (3) Alcatel Lucent B66A RRH4x45 (AWS) (3) Alcatel Lucent B13 RRH4x30 (700) (3) Alcatel Lucent RRH 4T4R B5 (160) (2) RFS DB-C1-12C-24AB-0Z	--	(2) 1-5/8 Fiber

- Height measurement taken as distance from top of base foundation to center of appurtenance.
- Additional equipment to be installed on the installed mount at 190 ft.
- The additional coax is to be installed alongside the currently installed Verizon coax.

**Maximum Member Usage Results**

Member	Usage (%) <sup>1</sup>
Leg	100
Diagonal	57
Horizontal	49
Guy Wires	52
Bolts	36
Anchor Rod	68

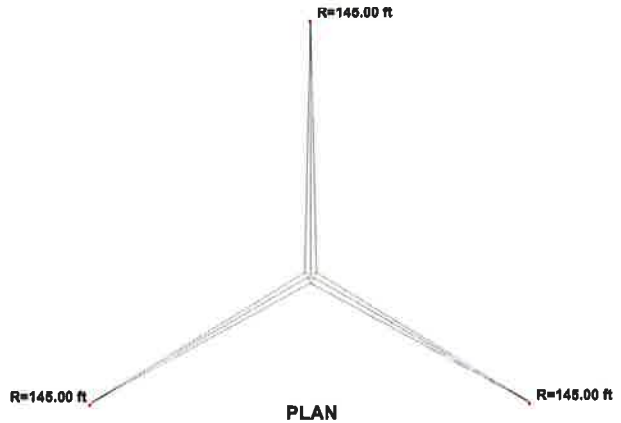
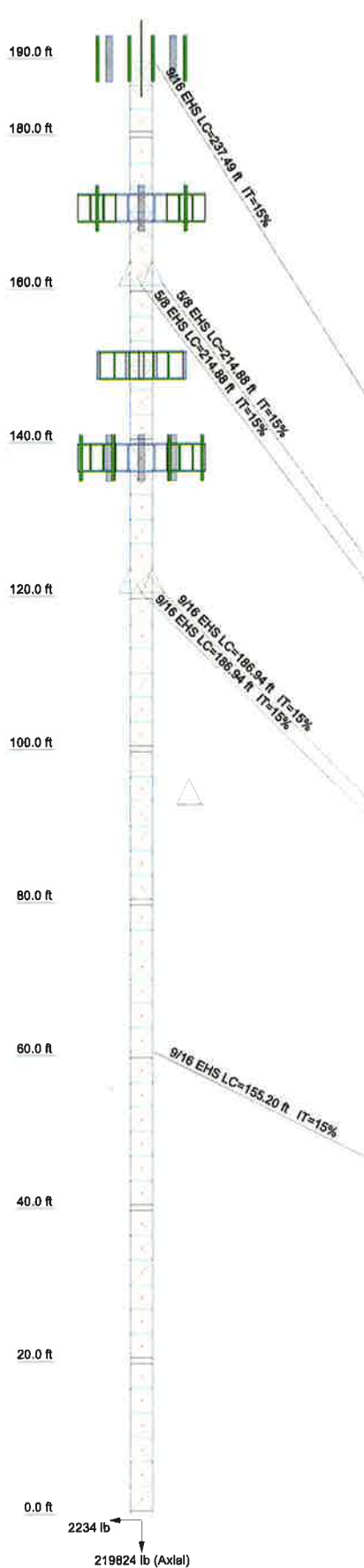
<sup>1</sup>Usage above 100% indicates the applied design load exceeds the member strength capacity and requires strengthening.

**Foundation Usage Results**

Base Reaction	Capacity (kips)	Analysis (kips)	Usage (%) <sup>1</sup>
Compression	254.1	219.8	88
Uplift	92.3	42.0	46
Shear	54.1	47.2	87

<sup>1</sup>Usage above 100% indicates the applied design load exceeds the member strength capacity and requires strengthening.

Legs	P2-5x203	N.A.	N.A.	N.A.	N.A.	3 @ 3.08333
Leg Grade	A500M-58	N.A.	N.A.	N.A.	N.A.	337.0
Diagonals	SR 5/8	N.A.	N.A.	N.A.	N.A.	672.6
Diagonal Grade	A36	N.A.	N.A.	N.A.	N.A.	688.2
Top Girts	L1 1/2x1 1/2x3/16	N.A.	L1 1/2x1 1/2x3/16	L1 1/2x1 1/2x3/16	L1 1/2x1 1/2x3/16	672.6
Bottom Girts	N.A.	N.A.	N.A.	N.A.	N.A.	688.2
Horizontal	L1 1/2x1 1/2x3/16	N.A.	L1 1/2x1 1/2x3/16	L1 1/2x1 1/2x3/16	L1 1/2x1 1/2x3/16	672.6
Top Guy Pull-Offs	N.A.	N.A.	N.A.	N.A.	N.A.	688.2
Bot Guy Pull-Offs	N.A.	N.A.	N.A.	N.A.	N.A.	688.2
Face Width (ft)	3.5					
# Panels @ (ft)	54 @ 3.20833					
Weight (lb)	6689.9					



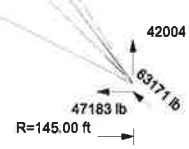
**SYMBOL LIST**

MARK	SIZE	MARK	SIZE
A	L1 1/2x1 1/2x3/16		

**MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A500M-58	58 ksi	70 ksi	A36	36 ksi	58 ksi

- TOWER DESIGN NOTES**
1. Tower is located in Windham County, Connecticut.
  2. Tower designed for Exposure B to the TIA-222-G Standard.
  3. Tower designed for a 100 mph basic wind in accordance with the TIA-222-G Standard.
  4. Tower is also designed for a 50 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
  5. Deflections are based upon a 60 mph wind.
  6. Tower Structure Class II.
  7. Topographic Category 1 with Crest Height of 0.00 ft
  8. Weld together tower sections have flange connections.
  9. TOWER RATING: 99.9%



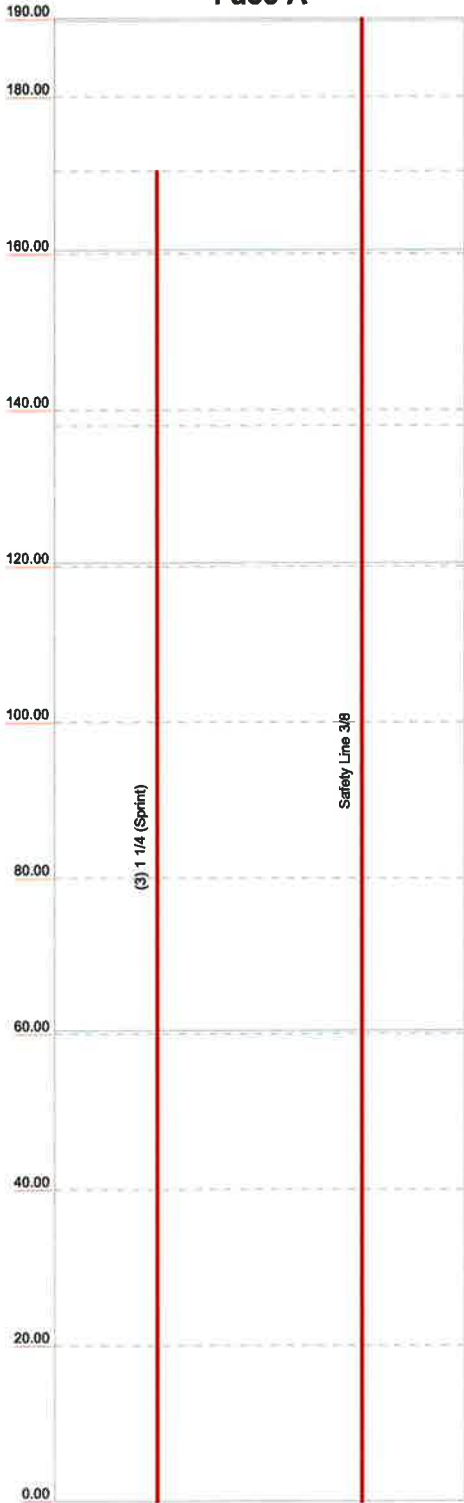
ALL REACTIONS ARE FACTORED

Job:	117-23150		
Project:	Ashford, CT		
Client:	CDT	Drawn by:	FAN
Code:	TIA-222-G	Date:	10/19/17
Path:		Scale:	N
Phone:		Dwg No.:	
FAX:			

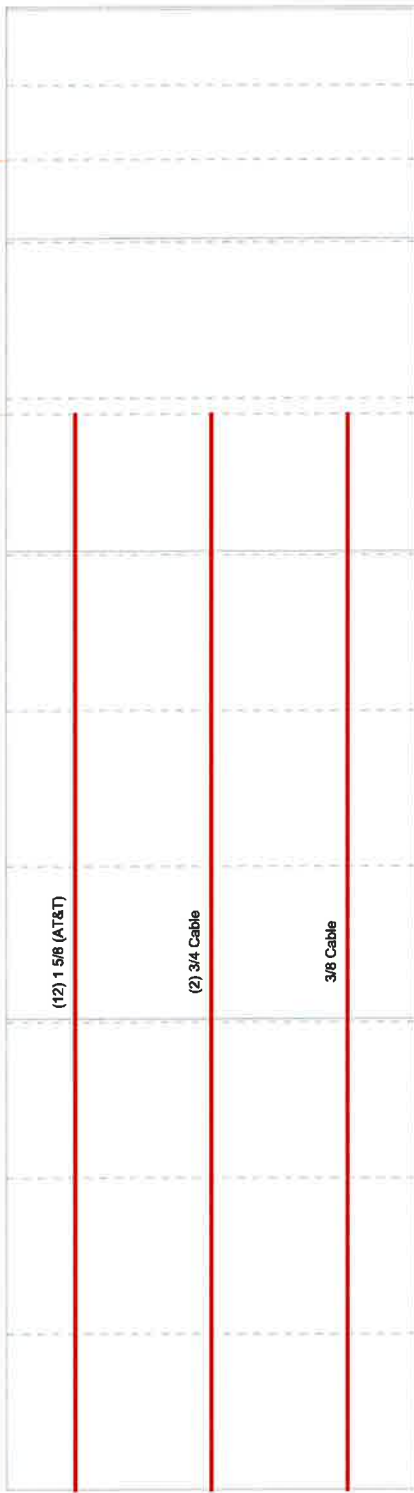
0' - 190'

Round Flat App In Face App Out Face Truss Leg

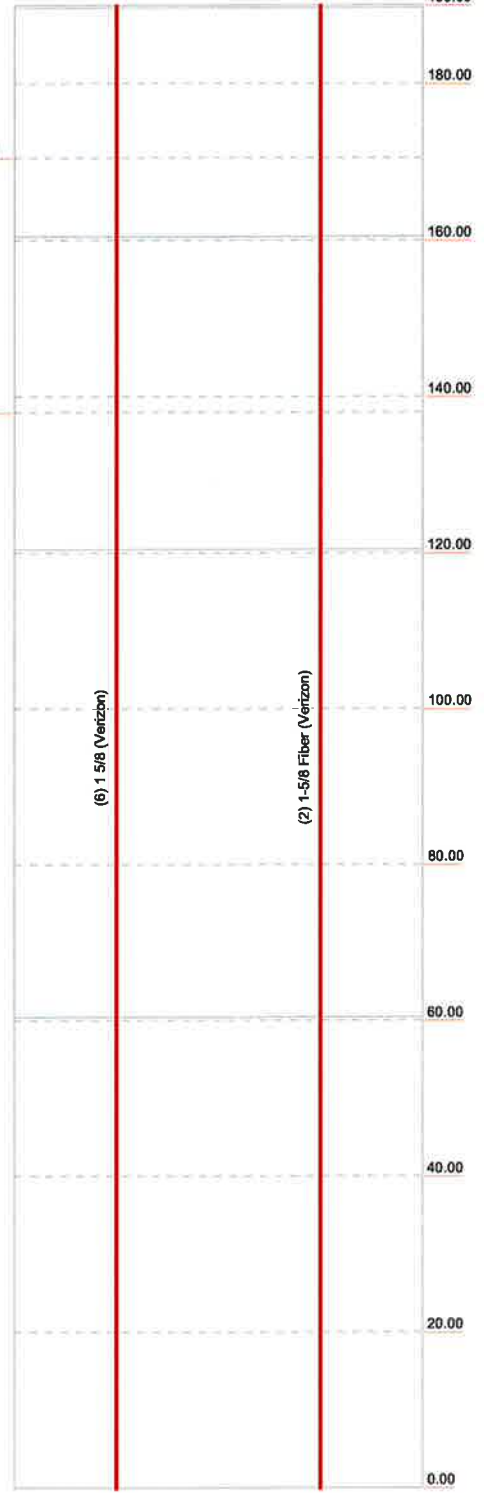
Face A



Face B



Face C



Elevation (ft)

Phone: FAX:	Job: <b>117-23150</b>		
	Project: <b>Ashford, CT</b>		
	Client: CDT	Drawn by: FAN	App'd:
	Code: TIA-222-G	Date: 10/19/17	Scale: N
	Path:		Dwg No.

<b><i>RISATower</i></b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 1 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

## Tower Input Data

The main tower is a 3x guyed tower with an overall height of 190.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 3.50 ft at the top and 3.50 ft at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Windham County, Connecticut.

Basic wind speed of 100 mph.

Structure Class II.

Exposure Category B.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Weld together tower sections have flange connections..

Tension only take-up is 0.0313 in.

Pressures are calculated at each section.

Safety factor used in guy design is 1.

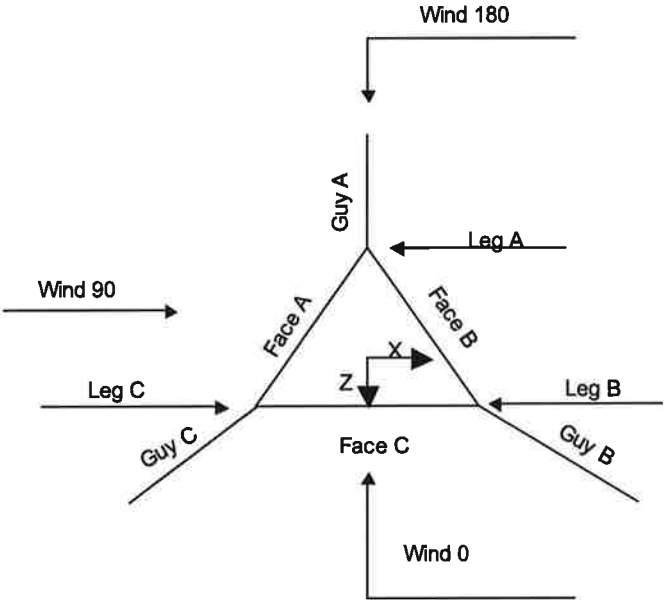
Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

## Options

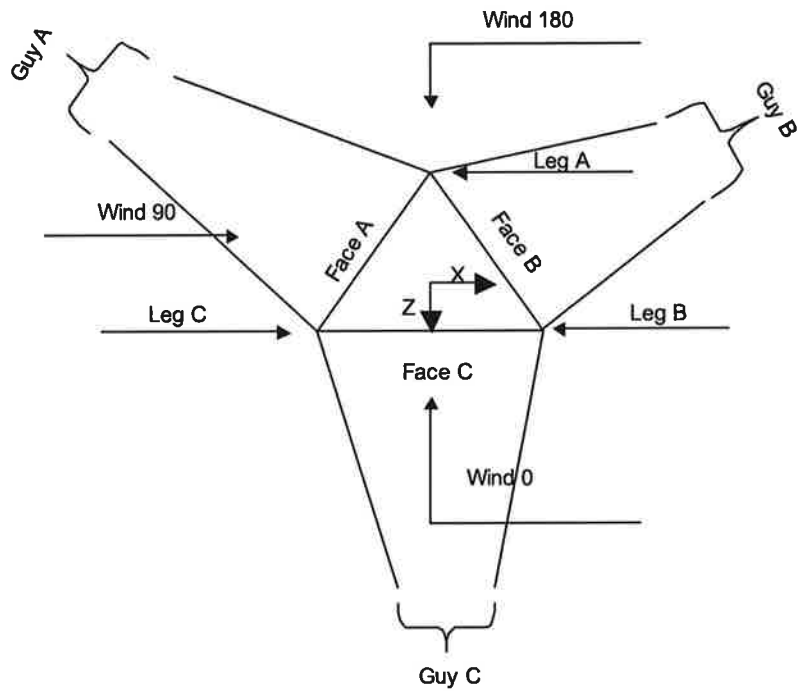
- |  |  |  |
|--|--|--|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>√ Include Bolts In Member Capacity</li> <li>√ Leg Bolts Are At Top Of Section</li> <li>√ Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>Add IBC .6D+W Combination</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>√ Use Clear Spans For KL/r</li> <li>√ Retension Guys To Initial Tension</li> <li>Bypass Mast Stability Checks</li> <li>√ Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>√ Autocalc Torque Arm Areas</li> <li>SR Members Have Cut Ends</li> <li>Sort Capacity Reports By Component</li> <li>√ Triangulate Diamond Inner Bracing</li> </ul> | <ul style="list-style-type: none"> <li>Treat Feedline Bundles As Cylinder</li> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>√ Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>√ All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feedline Torque</li> <li>Include Angle Block Shear Check</li> <li style="text-align: center;"><b>Poles</b></li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul> |
|--|--|--|

<b><i>RISATower</i></b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 2 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN



**Corner & Starmount Guyed Tower**

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 3 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN



**Face Guyed**

**Tower Section Geometry**

<i>Tower Section</i>	<i>Tower Elevation</i>	<i>Assembly Database</i>	<i>Description</i>	<i>Section Width</i>	<i>Number of Sections</i>	<i>Section Length</i>
	<i>ft</i>			<i>ft</i>		<i>ft</i>
T1	190.00-180.00			3.50	1	10.00
T2	180.00-160.00			3.50	1	20.00
T3	160.00-140.00			3.50	1	20.00
T4	140.00-120.00			3.50	1	20.00
T5	120.00-100.00			3.50	1	20.00
T6	100.00-80.00			3.50	1	20.00
T7	80.00-60.00			3.50	1	20.00
T8	60.00-40.00			3.50	1	20.00
T9	40.00-20.00			3.50	1	20.00
T10	20.00-0.00			3.50	1	20.00

**Tower Section Geometry (cont'd)**



<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 4 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Tower Section	Tower Elevation <i>ft</i>	Diagonal Spacing <i>ft</i>	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset <i>in</i>	Bottom Girt Offset <i>in</i>
T1	190.00-180.00	3.08	TX Brace	No	Yes	4.5000	4.5000
T2	180.00-160.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T3	160.00-140.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T4	140.00-120.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T5	120.00-100.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T6	100.00-80.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T7	80.00-60.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T8	60.00-40.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T9	40.00-20.00	3.21	TX Brace	No	Yes	4.5000	4.5000
T10	20.00-0.00	3.21	TX Brace	No	Yes	4.5000	4.5000

### Tower Section Geometry (cont'd)

Tower Elevation <i>ft</i>	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 190.00-180.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)
T2 180.00-160.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)
T3 160.00-140.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)
T4 140.00-120.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)
T5 120.00-100.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)
T6 100.00-80.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)
T7 80.00-60.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)
T8 60.00-40.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)
T9 40.00-20.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)
T10 20.00-0.00	Pipe	P2.5x.203	A500M-58 (58 ksi)	Solid Round	5/8	A36 (36 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation <i>ft</i>	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 190.00-180.00	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T2 180.00-160.00	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T3 160.00-140.00	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T4 140.00-120.00	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T5 120.00-100.00	Equal Angle	L1 1/2x1 1/2x3/16	A36	Equal Angle	L1 1/2x1 1/2x3/16	A36

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 5 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T6 100.00-80.00	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36
T7 80.00-60.00	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36
T8 60.00-40.00	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36
T9 40.00-20.00	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36
T10 20.00-0.00	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36	Equal Angle	L1 1/2x1 1/2x3/16	(36 ksi) A36

### Tower Section Geometry (cont'd)

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T1 190.00-180.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T2 180.00-160.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T3 160.00-140.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T4 140.00-120.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T5 120.00-100.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T6 100.00-80.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T7 80.00-60.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T8 60.00-40.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T9 40.00-20.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
T10 20.00-0.00	None	Flat Bar		A36 (36 ksi)	Single Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	Gusset Area (per face) ft <sup>2</sup>	Gusset Thickness in	Gusset Grade	Adjust. Factor A <sub>f</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontal in
T1 190.00-180.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T2 180.00-160.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T3	0.00	0.0000	A36	1	1	1	36.0000	36.0000

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 6 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
ft	ft <sup>2</sup>	in						
160.00-140.00			(36 ksi)					
T4	0.00	0.0000	A36	1	1	1	36.0000	36.0000
140.00-120.00			(36 ksi)					
T5	0.00	0.0000	A36	1	1	1	36.0000	36.0000
120.00-100.00			(36 ksi)					
T6	0.00	0.0000	A36	1	1	1	36.0000	36.0000
100.00-80.00			(36 ksi)					
T7 80.00-60.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000
80.00-60.00			(36 ksi)					
T8 60.00-40.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000
60.00-40.00			(36 ksi)					
T9 40.00-20.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000
40.00-20.00			(36 ksi)					
T10 20.00-0.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000
20.00-0.00			(36 ksi)					

### Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	K Factors <sup>1</sup>							
			Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
				X Y	X Y	X Y	X Y	X Y	X Y	X Y
T1	No	Yes	1	1	1	1	0.65	0.65	1	1
190.00-180.00				1	1	1	0.65	0.65	1	1
T2	No	Yes	1	1	1	1	0.65	0.65	1	1
180.00-160.00				1	1	1	0.65	0.65	1	1
T3	No	Yes	1	1	1	1	0.65	0.65	1	1
160.00-140.00				1	1	1	0.65	0.65	1	1
T4	No	Yes	1	1	1	1	0.65	0.65	1	1
140.00-120.00				1	1	1	0.65	0.65	1	1
T5	No	Yes	1	1	1	1	0.65	0.65	1	1
120.00-100.00				1	1	1	0.65	0.65	1	1
T6	No	Yes	1	1	1	1	0.65	0.65	1	1
100.00-80.00				1	1	1	0.65	0.65	1	1
T7	No	Yes	1	1	1	1	0.65	0.65	1	1
80.00-60.00				1	1	1	0.65	0.65	1	1
T8	No	Yes	1	1	1	1	0.65	0.65	1	1
60.00-40.00				1	1	1	0.65	0.65	1	1
T9	No	Yes	1	1	1	1	0.65	0.65	1	1
40.00-20.00				1	1	1	0.65	0.65	1	1
T10	No	Yes	1	1	1	1	0.65	0.65	1	1
20.00-0.00				1	1	1	0.65	0.65	1	1

<sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

### Tower Section Geometry (cont'd)

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 7 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
190.00-180.00														
T2	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
180.00-160.00														
T3	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
160.00-140.00														
T4	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
140.00-120.00														
T5	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
120.00-100.00														
T6	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
100.00-80.00														
T7	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
80.00-60.00														
T8	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
60.00-40.00														
T9	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
40.00-20.00														
T10	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	0.75
20.00-0.00														

**Tower Section Geometry (cont'd)**

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
190.00-180.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T2	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
180.00-160.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T3	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
160.00-140.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T4	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
140.00-120.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T5	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
120.00-100.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T6	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
100.00-80.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T7	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
80.00-60.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T8	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
60.00-40.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T9	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
40.00-20.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T10	Flange	0.7500	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
20.00-0.00		A325N		A325N		A325N		A325N		A325N		A325N		A325N	

**Guy Data**



<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 9 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Guy Elevation ft	Cable Weight			Tower Intercept		
	A lb	B lb	C lb	A ft	B ft	C ft
160.375	174.48	174.48	174.48	2.92 2.9 sec/pulse	2.92 2.9 sec/pulse	2.92 2.9 sec/pulse
120.375	125.27	125.27	125.27	2.21 2.6 sec/pulse	2.21 2.6 sec/pulse	2.21 2.6 sec/pulse
60.375	104.01	104.01	104.01	1.53 2.1 sec/pulse	1.53 2.1 sec/pulse	1.53 2.1 sec/pulse
189.625	159.15	159.15	159.15	3.56 3.3 sec/pulse	3.56 3.3 sec/pulse	3.56 3.3 sec/pulse

### Guy Data (cont'd)

Guy Elevation ft	Calc K Single Angles	Calc K Solid Rounds	Torque Arm		Pull Off		Diagonal	
			K <sub>x</sub>	K <sub>y</sub>	K <sub>x</sub>	K <sub>y</sub>	K <sub>x</sub>	K <sub>y</sub>
160.375	No	No	1	1	0.65	0.65	1	1
120.375	No	No	1	1	0.65	0.65	1	1
60.375	No	No			0.65	0.65	1	1
189.625	No	No			0.65	0.65	1	1

### Guy Data (cont'd)

Guy Elevation ft	Torque-Arm				Pull Off				Diagonal			
	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U
160.375	0.7500 A325N	2	0.0000	1	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	1
120.375	0.7500 A325N	2	0.0000	1	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	1
60.375	0.6250 A325N	0	0.0000	0.75	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	1
189.625	0.6250 A325N	0	0.0000	0.75	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	1

### Guy Pressures

Guy Elevation ft	Guy Location	z ft	q <sub>z</sub> psf	q <sub>z</sub> Ice psf	Ice Thickness in
160.375	A	80.19	20	5	2.1857
	B	80.19	20	5	2.1857
	C	80.19	20	5	2.1857
120.375	A	60.19	19	5	2.1239
	B	60.19	19	5	2.1239
	C	60.19	19	5	2.1239
60.375	A	30.19	15	4	1.9823
	B	30.19	15	4	1.9823

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 10 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Guy Elevation ft	Guy Location	z ft	q <sub>z</sub> psf	q <sub>z</sub> Ice psf	Ice Thickness in
189.625	C	30.19	15	4	1.9823
	A	94.81	21	5	2.2226
	B	94.81	21	5	2.2226
	C	94.81	21	5	2.2226

### Guy-Mast Forces (Excluding Wind) - No Ice

Guy Elevation ft	Guy Location	Chord Angle °	Guy Tension Top Bottom lb	F <sub>x</sub> lb	F <sub>y</sub> lb	F <sub>z</sub> lb	M <sub>x</sub> lb-ft	M <sub>y</sub> lb-ft	M <sub>z</sub> lb-ft	
160.375	A	48.2735	6490.22	-104.64	4882.39	-4274.84	-9865.97	15173.38	-17088.36	
			6360.00	104.64	4882.39	-4274.84	-9865.97	-15173.38	17088.36	
	B	48.2735	6490.22	3754.44	4882.39	2046.79	19731.94	15173.38	0.00	
			6360.00	3649.79	4882.39	2228.04	-9865.97	-15173.38	-17088.36	
	C	48.2735	6490.22	-3649.79	4882.39	2228.04	-9865.97	15173.38	17088.36	
			6360.00	-3754.44	4882.39	2046.79	19731.94	-15173.38	0.00	
	120.375	A	40.0857	Sum:	0.00	29294.33	0.00	-0.00	0.00	0.00
				5330.67	-99.04	3469.21	-4046.08	-7010.33	14361.42	-12142.24
		B	40.0857	5330.67	99.04	3469.21	-4046.08	-7010.33	-14361.42	12142.24
				5250.00	3553.53	3469.21	1937.27	14020.66	14361.42	0.00
		C	40.0857	5330.67	3454.49	3469.21	2108.82	-7010.33	-14361.42	-12142.24
				5250.00	-3454.49	3469.21	2108.82	-7010.33	14361.42	12142.24
60.375	A	22.8926	Sum:	0.00	20815.27	0.00	-0.00	0.00	0.00	
			5290.46	0.00	2102.12	-4854.90	-4247.81	0.00	0.00	
	B	22.8926	5290.46	4204.47	2102.12	2427.45	2123.90	0.00	-3678.71	
			5250.00	-4204.47	2102.12	2427.45	2123.90	-0.00	3678.71	
	C	22.8926	5290.46	0.00	6306.36	0.00	0.00	0.00	0.00	
			5250.00	0.00	4322.16	-3198.73	-8733.90	0.00	0.00	
189.625	A	52.9833	Sum:	0.00	6306.36	0.00	0.00	0.00	0.00	
			5377.07	0.00	4322.16	-3198.73	-8733.90	0.00	0.00	
	B	52.9833	5377.07	2770.18	4322.16	1599.36	4366.95	0.00	-7563.78	
			5250.00	-2770.18	4322.16	1599.36	4366.95	-0.00	7563.78	
	C	52.9833	5377.07	0.00	12966.47	0.00	0.00	0.00	0.00	
			5250.00	0.00	12966.47	0.00	0.00	0.00	0.00	

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 11 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

### Guy-Mast Forces (Excluding Wind) - Ice

Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom lb	F <sub>x</sub>	F <sub>y</sub>	F <sub>z</sub>	M <sub>x</sub>	M <sub>y</sub>	M <sub>z</sub>
ft		°		lb	lb	lb	lb-ft	lb-ft	lb-ft
160.375	A	48.2735	11257.75 9926.09	-172.02	8793.49	-7027.21	-17769.24	24942.86	-30777.22
	A	48.2735	11257.75 9926.09	172.02	8793.49	-7027.21	-17769.24	-24942.86	30777.22
	B	48.2735	11257.75 9926.09	6171.76	8793.49	3364.63	35538.47	24942.86	0.00
	B	48.2735	11257.75 9926.09	5999.74	8793.49	3662.58	-17769.24	-24942.86	-30777.22
	C	48.2735	11257.75 9926.09	-5999.74	8793.49	3662.58	-17769.24	24942.86	30777.22
	C	48.2735	11257.75 9926.09	-6171.76	8793.49	3364.63	35538.47	-24942.86	0.00
120.375			Sum:	0.00	52760.95	0.00	-0.00	0.00	0.00
	A	40.0857	9516.06 8597.94	-169.12	6541.87	-6908.74	-13219.33	24522.34	-22896.56
	A	40.0857	9516.06 8597.94	169.12	6541.87	-6908.74	-13219.33	-24522.34	22896.56
	B	40.0857	9516.06 8597.94	6067.71	6541.87	3307.91	26438.67	24522.34	0.00
	B	40.0857	9516.06 8597.94	5898.59	6541.87	3600.83	-13219.33	-24522.34	-22896.56
	C	40.0857	9516.06 8597.94	-5898.59	6541.87	3600.83	-13219.33	24522.34	22896.56
60.375			Sum:	0.00	39251.24	0.00	-0.00	0.00	0.00
	A	22.8926	8905.54 8493.69	0.00	3912.05	-8000.28	-7905.18	0.00	0.00
	B	22.8926	8905.54 8493.69	6928.45	3912.05	4000.14	3952.59	0.00	-6846.09
	C	22.8926	8905.54 8493.69	-6928.45	3912.05	4000.14	3952.59	-0.00	6846.09
			Sum:	0.00	11736.15	-0.00	0.00	0.00	0.00
	A	52.9833	10097.77 8539.50	0.00	8411.96	-5586.05	-16998.27	0.00	0.00
189.625	B	52.9833	10097.77 8539.50	4837.66	8411.96	2793.02	8499.13	0.00	-14720.93
	C	52.9833	10097.77 8539.50	-4837.66	8411.96	2793.02	8499.13	-0.00	14720.93
			Sum:	0.00	25235.88	0.00	0.00	0.00	0.00

### Guy-Mast Forces (Excluding Wind) - Service

Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom lb	F <sub>x</sub>	F <sub>y</sub>	F <sub>z</sub>	M <sub>x</sub>	M <sub>y</sub>	M <sub>z</sub>
ft		°		lb	lb	lb	lb-ft	lb-ft	lb-ft
160.375	A	48.2735	6490.22 6360.00	-104.64	4882.39	-4274.84	-9865.97	15173.38	-17088.36
	A	48.2735	6490.22	104.64	4882.39	-4274.84	-9865.97	-15173.38	17088.36



<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 12 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Guy Elevation	Guy Location	Chord Angle	Guy Tension Top Bottom lb	F <sub>x</sub>	F <sub>y</sub>	F <sub>z</sub>	M <sub>x</sub>	M <sub>y</sub>	M <sub>z</sub>
ft		°		lb	lb	lb	lb-ft	lb-ft	lb-ft
			6360.00						
	B	48.2735	6490.22	3754.44	4882.39	2046.79	19731.94	15173.38	0.00
			6360.00						
	B	48.2735	6490.22	3649.79	4882.39	2228.04	-9865.97	-15173.38	-17088.36
			6360.00						
	C	48.2735	6490.22	-3649.79	4882.39	2228.04	-9865.97	15173.38	17088.36
			6360.00						
	C	48.2735	6490.22	-3754.44	4882.39	2046.79	19731.94	-15173.38	0.00
			6360.00						
			Sum:	0.00	29294.33	0.00	-0.00	0.00	0.00
120.375	A	40.0857	5330.67	-99.04	3469.21	-4046.08	-7010.33	14361.42	-12142.24
			5250.00						
	A	40.0857	5330.67	99.04	3469.21	-4046.08	-7010.33	-14361.42	12142.24
			5250.00						
	B	40.0857	5330.67	3553.53	3469.21	1937.27	14020.66	14361.42	0.00
			5250.00						
	B	40.0857	5330.67	3454.49	3469.21	2108.82	-7010.33	-14361.42	-12142.24
			5250.00						
	C	40.0857	5330.67	-3454.49	3469.21	2108.82	-7010.33	14361.42	12142.24
			5250.00						
	C	40.0857	5330.67	-3553.53	3469.21	1937.27	14020.66	-14361.42	0.00
			5250.00						
			Sum:	0.00	20815.27	0.00	-0.00	0.00	0.00
60.375	A	22.8926	5290.46	0.00	2102.12	-4854.90	-4247.81	0.00	0.00
			5250.00						
	B	22.8926	5290.46	4204.47	2102.12	2427.45	2123.90	0.00	-3678.71
			5250.00						
	C	22.8926	5290.46	-4204.47	2102.12	2427.45	2123.90	-0.00	3678.71
			5250.00						
			Sum:	0.00	6306.36	0.00	0.00	0.00	0.00
189.625	A	52.9833	5377.07	0.00	4322.16	-3198.73	-8733.90	0.00	0.00
			5250.00						
	B	52.9833	5377.07	2770.18	4322.16	1599.36	4366.95	0.00	-7563.78
			5250.00						
	C	52.9833	5377.07	-2770.18	4322.16	1599.36	4366.95	-0.00	7563.78
			5250.00						
			Sum:	0.00	12966.47	0.00	0.00	0.00	0.00

### Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
1 5/8 (Verizon)	C	No	Ar (CaAa)	190.00 - 0.00	0.0000	0.25	6	6	1.0000	1.9800		1.04
1 1/4 (Sprint)	A	No	Ar (CaAa)	170.50 - 0.00	0.0000	0.25	3	3	1.0000	1.5500		0.66
1 5/8 (AT&T)	B	No	Ar (CaAa)	138.00 - 0.00	6.0000	0.5	12	6	1.0000	1.9800		1.04
Safety Line 3/8	A	No	Ar (CaAa)	190.00 - 0.00	0.0000	0	1	1	0.3750	0.3750		0.22
3/4 Cable	B	No	Ar (CaAa)	138.00 - 0.00	0.0000	0.1	2	1	0.6300	0.6300		0.19
3/8 Cable	B	No	Ar (CaAa)	138.00 - 0.00	0.0000	0.1	1	1	0.0000	0.3750		0.19
1-5/8 Fiber	C	No	Ar (CaAa)	190.00 - 0.00	0.0000	-0.25	2	2	1.9800	1.9800		0.75

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 13 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	# Per Row	#	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
(Verizon)												

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight lb
T1	190.00-180.00	A	0.000	0.000	0.375	0.000	2.20
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	15.840	0.000	77.40
T2	180.00-160.00	A	0.000	0.000	5.633	0.000	25.19
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	31.680	0.000	154.80
T3	160.00-140.00	A	0.000	0.000	10.050	0.000	44.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	31.680	0.000	154.80
T4	140.00-120.00	A	0.000	0.000	10.050	0.000	44.00
		B	0.000	0.000	45.711	0.000	234.90
		C	0.000	0.000	31.680	0.000	154.80
T5	120.00-100.00	A	0.000	0.000	10.050	0.000	44.00
		B	0.000	0.000	50.790	0.000	261.00
		C	0.000	0.000	31.680	0.000	154.80
T6	100.00-80.00	A	0.000	0.000	10.050	0.000	44.00
		B	0.000	0.000	50.790	0.000	261.00
		C	0.000	0.000	31.680	0.000	154.80
T7	80.00-60.00	A	0.000	0.000	10.050	0.000	44.00
		B	0.000	0.000	50.790	0.000	261.00
		C	0.000	0.000	31.680	0.000	154.80
T8	60.00-40.00	A	0.000	0.000	10.050	0.000	44.00
		B	0.000	0.000	50.790	0.000	261.00
		C	0.000	0.000	31.680	0.000	154.80
T9	40.00-20.00	A	0.000	0.000	10.050	0.000	44.00
		B	0.000	0.000	50.790	0.000	261.00
		C	0.000	0.000	31.680	0.000	154.80
T10	20.00-0.00	A	0.000	0.000	10.050	0.000	44.00
		B	0.000	0.000	50.790	0.000	261.00
		C	0.000	0.000	31.680	0.000	154.80

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight lb
T1	190.00-180.00	A	2.376	0.000	0.000	5.128	0.000	82.07
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	44.114	0.000	752.29
T2	180.00-160.00	A	2.356	0.000	0.000	27.203	0.000	415.52
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	87.966	0.000	1492.14
T3	160.00-140.00	A	2.327	0.000	0.000	42.294	0.000	634.80
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	87.580	0.000	1474.00
T4	140.00-120.00	A	2.294	0.000	0.000	41.937	0.000	623.16
		B		0.000	0.000	80.512	0.000	1775.87

<b>RISATower</b>  Phone: FAX:	<b>Job</b>	117-23150	<b>Page</b>	14 of 56
	<b>Project</b>	Ashford, CT	<b>Date</b>	00:05:19 10/19/17
	<b>Client</b>	CDT	<b>Designed by</b>	FAN

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_{AA}$ In Face ft <sup>2</sup>	$C_{AA}$ Out Face ft <sup>2</sup>	Weight lb
T5	120.00-100.00	C	2.256	0.000	0.000	87.146	0.000	1453.64
		A		0.000	0.000	41.528	0.000	609.91
		B		0.000	0.000	88.788	0.000	1942.37
T6	100.00-80.00	C	2.211	0.000	0.000	86.647	0.000	1430.36
		A		0.000	0.000	41.045	0.000	594.46
		B		0.000	0.000	87.999	0.000	1906.38
T7	80.00-60.00	C	2.156	0.000	0.000	86.059	0.000	1403.09
		A		0.000	0.000	40.453	0.000	575.81
		B		0.000	0.000	87.032	0.000	1862.84
T8	60.00-40.00	C	2.085	0.000	0.000	85.340	0.000	1369.97
		A		0.000	0.000	39.686	0.000	552.01
		B		0.000	0.000	85.776	0.000	1807.13
T9	40.00-20.00	C	1.981	0.000	0.000	84.407	0.000	1327.35
		A		0.000	0.000	38.570	0.000	518.26
		B		0.000	0.000	83.949	0.000	1727.82
T10	20.00-0.00	C	1.775	0.000	0.000	83.052	0.000	1266.22
		A		0.000	0.000	36.359	0.000	454.38
		B		0.000	0.000	80.329	0.000	1576.62
		C		0.000	0.000	80.371	0.000	1147.97

### Feed Line Center of Pressure

Section	Elevation ft	$CP_X$ in	$CP_Z$ in	$CP_X$ Ice in	$CP_Z$ Ice in
T1	190.00-180.00	-0.8400	1.9798	-0.3523	0.4932
T2	180.00-160.00	-0.9022	1.4870	-0.3924	0.4072
T3	160.00-140.00	-0.9443	1.0962	-0.4124	0.3144
T4	140.00-120.00	3.1917	1.4948	1.7390	0.6608
T5	120.00-100.00	3.3976	1.5147	1.8683	0.6863
T6	100.00-80.00	3.3976	1.5147	1.8633	0.6919
T7	80.00-60.00	3.3976	1.5147	1.8574	0.6989
T8	60.00-40.00	3.3976	1.5147	1.8501	0.7085
T9	40.00-20.00	3.3976	1.5147	1.8400	0.7234
T10	20.00-0.00	3.3976	1.5147	1.8218	0.7559

### Shielding Factor $K_a$

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	$K_a$ No Ice	$K_a$ Ice
T1	1	1 5/8	180.00 - 190.00	0.6000	0.2458
T1	4	Safety Line 3/8	180.00 - 190.00	0.6000	0.2458
T1	7	1-5/8 Fiber	180.00 - 190.00	0.6000	0.2458
T2	1	1 5/8	160.00 - 180.00	0.6000	0.2654
T2	2	1 1/4	160.00 - 170.50	0.6000	0.2654
T2	4	Safety Line 3/8	160.00 -	0.6000	0.2654

<h1>RISATower</h1> <p>Phone: FAX:</p>	<b>Job</b> 117-23150	<b>Page</b> 15 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
			180.00		
T2	7	1-5/8 Fiber	160.00 - 180.00	0.6000	0.2654
T3	1	1 5/8	140.00 - 160.00	0.6000	0.2713
T3	2	1 1/4	140.00 - 160.00	0.6000	0.2713
T3	4	Safety Line 3/8	140.00 - 160.00	0.6000	0.2713
T3	7	1-5/8 Fiber	140.00 - 160.00	0.6000	0.2713
T4	1	1 5/8	120.00 - 140.00	0.6000	0.2781
T4	2	1 1/4	120.00 - 140.00	0.6000	0.2781
T4	3	1 5/8	120.00 - 138.00	1.0000	1.0000
T4	4	Safety Line 3/8	120.00 - 140.00	0.6000	0.2781
T4	5	3/4 Cable	120.00 - 138.00	0.6000	0.2781
T4	6	3/8 Cable	120.00 - 138.00	0.6000	0.2781
T4	7	1-5/8 Fiber	120.00 - 140.00	0.6000	0.2781
T5	1	1 5/8	100.00 - 120.00	0.6000	0.2859
T5	2	1 1/4	100.00 - 120.00	0.6000	0.2859
T5	3	1 5/8	100.00 - 120.00	1.0000	1.0000
T5	4	Safety Line 3/8	100.00 - 120.00	0.6000	0.2859
T5	5	3/4 Cable	100.00 - 120.00	0.6000	0.2859
T5	6	3/8 Cable	100.00 - 120.00	0.6000	0.2859
T5	7	1-5/8 Fiber	100.00 - 120.00	0.6000	0.2859
T6	1	1 5/8	80.00 - 100.00	0.6000	0.2952
T6	2	1 1/4	80.00 - 100.00	0.6000	0.2952
T6	3	1 5/8	80.00 - 100.00	1.0000	1.0000
T6	4	Safety Line 3/8	80.00 - 100.00	0.6000	0.2952
T6	5	3/4 Cable	80.00 - 100.00	0.6000	0.2952
T6	6	3/8 Cable	80.00 - 100.00	0.6000	0.2952
T6	7	1-5/8 Fiber	80.00 - 100.00	0.6000	0.2952
T7	1	1 5/8	60.00 - 80.00	0.6000	0.3065
T7	2	1 1/4	60.00 - 80.00	0.6000	0.3065
T7	3	1 5/8	60.00 - 80.00	1.0000	1.0000
T7	4	Safety Line 3/8	60.00 - 80.00	0.6000	0.3065
T7	5	3/4 Cable	60.00 - 80.00	0.6000	0.3065
T7	6	3/8 Cable	60.00 - 80.00	0.6000	0.3065
T7	7	1-5/8 Fiber	60.00 - 80.00	0.6000	0.3065
T8	1	1 5/8	40.00 - 60.00	0.6000	0.3214
T8	2	1 1/4	40.00 - 60.00	0.6000	0.3214
T8	3	1 5/8	40.00 - 60.00	1.0000	1.0000
T8	4	Safety Line 3/8	40.00 - 60.00	0.6000	0.3214
T8	5	3/4 Cable	40.00 - 60.00	0.6000	0.3214
T8	6	3/8 Cable	40.00 - 60.00	0.6000	0.3214
T8	7	1-5/8 Fiber	40.00 - 60.00	0.6000	0.3214
T9	1	1 5/8	20.00 - 40.00	0.6000	0.3431
T9	2	1 1/4	20.00 - 40.00	0.6000	0.3431

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 16 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T9	3	1 5/8	20.00 - 40.00	1.0000	1.0000
T9	4	Safety Line 3/8	20.00 - 40.00	0.6000	0.3431
T9	5	3/4 Cable	20.00 - 40.00	0.6000	0.3431
T9	6	3/8 Cable	20.00 - 40.00	0.6000	0.3431
T9	7	1-5/8 Fiber	20.00 - 40.00	0.6000	0.3431
T10	1	1 5/8	0.00 - 20.00	0.6000	0.3869
T10	2	1 1/4	0.00 - 20.00	0.6000	0.3869
T10	3	1 5/8	0.00 - 20.00	1.0000	1.0000
T10	4	Safety Line 3/8	0.00 - 20.00	0.6000	0.3869
T10	5	3/4 Cable	0.00 - 20.00	0.6000	0.3869
T10	6	3/8 Cable	0.00 - 20.00	0.6000	0.3869
T10	7	1-5/8 Fiber	0.00 - 20.00	0.6000	0.3869

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment	Placement ft	C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight lb	
Sector Frame Mount	A	From Leg	1.50	0.0000	190.00	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
Sector Frame Mount	B	From Leg	1.50	0.0000	190.00	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
Sector Frame Mount	C	From Leg	1.50	0.0000	190.00	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
(2) Antel LPA-80080-4CF (Verizon)	A	From Leg	3.00	0.0000	190.00	No Ice	1.51	6.79	12.00
			0.00			1/2" Ice	1.81	7.14	45.10
			0.00			1" Ice	2.53	7.50	50.65
(2) Antel LPA-80063/4CF (Verizon)	B	From Leg	3.00	0.0000	190.00	No Ice	7.00	3.48	20.00
			0.00			1/2" Ice	7.36	3.82	72.60
			0.00			1" Ice	2.53	3.99	50.65
(2) Antel LPA-80063/4CF (Verizon)	C	From Leg	3.00	0.0000	190.00	No Ice	7.00	3.48	20.00
			0.00			1/2" Ice	7.36	3.82	72.60
			0.00			1" Ice	2.53	3.99	50.65
Sector Frame Mount	A	From Leg	1.50	0.0000	178.00	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
Sector Frame Mount	B	From Leg	1.50	0.0000	178.00	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
Sector Frame Mount	C	From Leg	1.50	0.0000	178.00	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
(2) Allgon 7770.00 (AT&T)	A	From Leg	3.00	0.0000	138.00	No Ice	5.88	2.93	35.00
			0.00			1/2" Ice	6.25	3.29	67.60
			0.00			1" Ice	6.64	3.67	85.58
(2) Allgon 7770.00 (AT&T)	B	From Leg	3.00	0.0000	138.00	No Ice	5.88	2.93	35.00
			0.00			1/2" Ice	6.25	3.29	67.60
			0.00			1" Ice	6.64	3.67	85.58

<h1 style="margin: 0;">RISATower</h1> <p style="font-size: small; margin-top: 10px;">Phone: FAX:</p>	<b>Job</b> 117-23150	<b>Page</b> 17 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub>		Weight
			Horz	Vert			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	lb
(2) Allgon 7770.00 (AT&T)	C	From Leg	3.00	0.0000	138.00	No Ice	5.88	2.93	35.00
			0.00	0.00		1/2" Ice	6.25	3.29	67.60
			0.00	0.00		1" Ice	6.64	3.67	85.58
(2) Powerwave LGP21401 (AT&T)	A	From Leg	3.00	0.0000	138.00	No Ice	1.95	0.53	31.00
			0.00	0.00		1/2" Ice	2.11	0.63	30.30
			0.00	0.00		1" Ice	2.28	0.75	203.29
(2) Powerwave LGP21401 (AT&T)	B	From Leg	3.00	0.0000	138.00	No Ice	1.95	0.53	31.00
			0.00	0.00		1/2" Ice	2.11	0.63	30.30
			0.00	0.00		1" Ice	2.28	0.75	203.29
(2) Powerwave LGP21401 (AT&T)	C	From Leg	3.00	0.0000	138.00	No Ice	1.95	0.53	31.00
			0.00	0.00		1/2" Ice	2.11	0.63	30.30
			0.00	0.00		1" Ice	2.28	0.75	203.29
(2) Powerwave LGP21901 (AT&T)	A	From Leg	3.00	0.0000	138.00	No Ice	0.27	0.13	25.00
			0.00	0.00		1/2" Ice	0.33	0.18	30.30
			0.00	0.00		1" Ice	0.41	0.23	203.29
(2) Powerwave LGP21901 (AT&T)	B	From Leg	3.00	0.0000	138.00	No Ice	0.27	0.13	25.00
			0.00	0.00		1/2" Ice	0.33	0.18	30.30
			0.00	0.00		1" Ice	0.41	0.23	203.29
(2) Powerwave LGP21901 (AT&T)	C	From Leg	3.00	0.0000	138.00	No Ice	0.27	0.13	25.00
			0.00	0.00		1/2" Ice	0.33	0.18	30.30
			0.00	0.00		1" Ice	0.41	0.23	203.29
Sector Frame Mount	A	From Leg	1.50	0.0000	150.00	No Ice	13.60	13.60	465.00
			0.00	0.00		1/2" Ice	18.40	18.40	600.00
			0.00	0.00		1" Ice	23.20	23.20	735.00
Sector Frame Mount	B	From Leg	1.50	0.0000	150.00	No Ice	13.60	13.60	465.00
			0.00	0.00		1/2" Ice	18.40	18.40	600.00
			0.00	0.00		1" Ice	23.20	23.20	735.00
Sector Frame Mount	C	From Leg	1.50	0.0000	150.00	No Ice	13.60	13.60	465.00
			0.00	0.00		1/2" Ice	18.40	18.40	600.00
			0.00	0.00		1" Ice	23.20	23.20	735.00
Lightning Rod	C	None		0.0000	190.00	No Ice	1.00	1.00	40.00
				0.00		1/2" Ice	2.02	2.02	49.26
				0.00		1" Ice	3.05	3.05	64.89
KMW AM-X-CD-17-65-00T-RET (AT&T)	A	From Leg	3.00	0.0000	138.00	No Ice	11.31	6.80	25.00
			0.00	0.00		1/2" Ice	11.93	7.48	86.40
			0.00	0.00		1" Ice	12.53	8.12	155.40
KMW AM-X-CD-17-65-00T-RET (AT&T)	B	From Leg	3.00	0.0000	138.00	No Ice	11.31	6.80	25.00
			0.00	0.00		1/2" Ice	11.93	7.48	86.40
			0.00	0.00		1" Ice	12.53	8.12	155.40
KMW AM-X-CD-17-65-00T-RET (AT&T)	C	From Leg	3.00	0.0000	138.00	No Ice	11.31	6.80	25.00
			0.00	0.00		1/2" Ice	11.93	7.48	86.40
			0.00	0.00		1" Ice	12.53	8.12	155.40
(2) Ericsson RRUS11 (AT&T)	A	From Leg	3.00	0.0000	138.00	No Ice	2.99	0.36	25.00
			0.00	0.00		1/2" Ice	3.19	0.48	38.50
			0.00	0.00		1" Ice	3.41	0.60	54.60
(2) Ericsson RRUS11 (AT&T)	B	From Leg	3.00	0.0000	138.00	No Ice	2.99	0.36	25.00
			0.00	0.00		1/2" Ice	3.19	0.48	38.50
			0.00	0.00		1" Ice	3.41	0.60	54.60
(2) Ericsson RRUS11 (AT&T)	C	From Leg	3.00	0.0000	138.00	No Ice	2.99	0.36	25.00
			0.00	0.00		1/2" Ice	3.19	0.48	38.50
			0.00	0.00		1" Ice	3.41	0.60	54.60
Raycap DC6-48-60-18-8F (AT&T)	A	From Leg	3.00	0.0000	138.00	No Ice	1.47	1.47	25.00
			0.00	0.00		1/2" Ice	1.67	1.67	47.60
			0.00	0.00		1" Ice	1.88	1.88	73.70
Sector Frame Mount (Sprint)	A	From Leg	1.50	0.0000	170.50	No Ice	13.60	13.60	465.00
			0.00	0.00		1/2" Ice	18.40	18.40	600.00
			0.00	0.00		1" Ice	23.20	23.20	735.00

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 18 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A		Weight
			Horz	Vert			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	lb
Sector Frame Mount (Sprint)	B	From Leg	1.50	0.0000	170.50	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
Sector Frame Mount (Sprint)	C	From Leg	1.50	0.0000	170.50	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
RFS APXV9ERR18-C (Sprint)	A	From Leg	3.00	0.0000	170.50	No Ice	8.02	5.81	62.00
			0.00			1/2" Ice	8.50	6.27	114.00
			0.00			1" Ice	4.66	3.02	55.13
Alcatel Lucent 1900 RRH (Sprint)	A	From Leg	3.00	0.0000	170.50	No Ice	2.32	2.24	60.00
			0.00			1/2" Ice	2.53	2.44	83.10
			0.00			1" Ice	4.66	3.02	55.13
Alcatel Lucent 800 RRH (Sprint)	A	From Leg	3.00	0.0000	170.50	No Ice	2.06	1.36	60.00
			0.00			1/2" Ice	2.24	1.52	78.30
			0.00			1" Ice	4.66	3.02	55.13
RFS APXV9ERR18-C (Sprint)	B	From Leg	3.00	0.0000	170.50	No Ice	8.02	5.81	62.00
			0.00			1/2" Ice	8.50	6.27	114.00
			0.00			1" Ice	4.66	3.02	55.13
Alcatel Lucent 1900 RRH (Sprint)	B	From Leg	3.00	0.0000	170.50	No Ice	2.32	2.24	60.00
			0.00			1/2" Ice	2.53	2.44	83.10
			0.00			1" Ice	4.66	3.02	55.13
Alcatel Lucent 800 RRH (Sprint)	B	From Leg	3.00	0.0000	170.50	No Ice	2.06	1.36	60.00
			0.00			1/2" Ice	2.24	1.52	78.30
			0.00			1" Ice	4.66	3.02	55.13
RFS APXV9ERR18-C (Sprint)	C	From Leg	3.00	0.0000	170.50	No Ice	8.02	5.81	62.00
			0.00			1/2" Ice	8.50	6.27	114.00
			0.00			1" Ice	4.66	3.02	55.13
Alcatel Lucent 1900 RRH (Sprint)	C	From Leg	3.00	0.0000	170.50	No Ice	2.32	2.24	60.00
			0.00			1/2" Ice	2.53	2.44	83.10
			0.00			1" Ice	4.66	3.02	55.13
Alcatel Lucent 800 RRH (Sprint)	C	From Leg	3.00	0.0000	170.50	No Ice	2.06	1.36	60.00
			0.00			1/2" Ice	2.24	1.52	78.30
			0.00			1" Ice	4.66	3.02	55.13
Sector Frame Mount	A	From Leg	1.50	0.0000	138.00	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
Sector Frame Mount	B	From Leg	1.50	0.0000	138.00	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
Sector Frame Mount	C	From Leg	1.50	0.0000	138.00	No Ice	13.60	13.60	465.00
			0.00			1/2" Ice	18.40	18.40	600.00
			0.00			1" Ice	23.20	23.20	735.00
(2) Commscope JAHH-65B-R3B	A	From Leg	3.00	0.0000	190.00	No Ice	9.11	3.44	63.30
			0.00			1/2" Ice	10.03	4.36	185.70
			0.00			1" Ice	10.96	5.27	334.10
Alcatel Lucent B66A RRH4x45	A	From Leg	3.00	0.0000	190.00	No Ice	2.54	1.61	51.00
			0.00			1/2" Ice	2.92	1.96	94.30
			0.00			1" Ice	3.35	2.33	150.90
Alcatel Lucent B13 RRH4x30	A	From Leg	3.00	0.0000	190.00	No Ice	2.16	1.62	57.20
			0.00			1/2" Ice	2.51	1.94	99.40
			0.00			1" Ice	2.89	2.29	154.20
Alcatel Lucent RRH 4T4R B5	A	From Leg	3.00	0.0000	190.00	No Ice	1.28	0.72	50.00
			0.00			1/2" Ice	1.54	0.93	74.70
			0.00			1" Ice	1.84	1.18	109.10
RFS DB-C1-12C-24AB-0Z	A	From Leg	3.00	0.0000	190.00	No Ice	2.80	1.17	44.00
			0.00			1/2" Ice	3.29	1.52	120.20
			0.00			1" Ice	3.81	1.91	213.00

<b>RISATower</b>	<b>Job</b>	117-23150	<b>Page</b>	19 of 56
	<b>Project</b>	Ashford, CT	<b>Date</b>	00:05:19 10/19/17
	<b>Client</b>	CDT	<b>Designed by</b>	FAN
Phone: FAX:				

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub>		Weight	
			Horz	Vert			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	lb	
(2) Commscope JAHH-65B-R3B	B	From Leg	3.00	0.00	0.0000	190.00	No Ice	9.11	3.44	63.30
			0.00	0.00			1/2" Ice	10.03	4.36	185.70
			0.00	0.00			1" Ice	10.96	5.27	334.10
Alcatel Lucent B66A RRH4x45	B	From Leg	3.00	0.00	0.0000	190.00	No Ice	2.54	1.61	51.00
			0.00	0.00			1/2" Ice	2.92	1.96	94.30
			0.00	0.00			1" Ice	3.35	2.33	150.90
Alcatel Lucent B13 RRH4x30	B	From Leg	3.00	0.00	0.0000	190.00	No Ice	2.16	1.62	57.20
			0.00	0.00			1/2" Ice	2.51	1.94	99.40
			0.00	0.00			1" Ice	2.89	2.29	154.20
Alcatel Lucent RRH 4T4R B5	B	From Leg	3.00	0.00	0.0000	190.00	No Ice	1.28	0.72	50.00
			0.00	0.00			1/2" Ice	1.54	0.93	74.70
			0.00	0.00			1" Ice	1.84	1.18	109.10
RFS DB-C1-12C-24AB-0Z	B	From Leg	3.00	0.00	0.0000	190.00	No Ice	2.80	1.17	44.00
			0.00	0.00			1/2" Ice	3.29	1.52	120.20
			0.00	0.00			1" Ice	3.81	1.91	213.00
(2) Commscope JAHH-65B-R3B	C	From Leg	3.00	0.00	0.0000	190.00	No Ice	9.11	3.44	63.30
			0.00	0.00			1/2" Ice	10.03	4.36	185.70
			0.00	0.00			1" Ice	10.96	5.27	334.10
Alcatel Lucent B66A RRH4x45	C	From Leg	3.00	0.00	0.0000	190.00	No Ice	2.54	1.61	51.00
			0.00	0.00			1/2" Ice	2.92	1.96	94.30
			0.00	0.00			1" Ice	3.35	2.33	150.90
Alcatel Lucent B13 RRH4x30	C	From Leg	3.00	0.00	0.0000	190.00	No Ice	2.16	1.62	57.20
			0.00	0.00			1/2" Ice	2.51	1.94	99.40
			0.00	0.00			1" Ice	2.89	2.29	154.20
Alcatel Lucent RRH 4T4R B5	C	From Leg	3.00	0.00	0.0000	190.00	No Ice	1.28	0.72	50.00
			0.00	0.00			1/2" Ice	1.54	0.93	74.70
			0.00	0.00			1" Ice	1.84	1.18	109.10

### Tower Pressures - No Ice

$$G_H = 0.850$$

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F <sub>a</sub>	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face
ft	ft		psf	ft <sup>2</sup>	c	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 190.00-180.00	185.00	1.178	26	37.396	A	1.630	6.150	4.792	61.59	0.375	0.000
					B	1.630	6.150		61.59	0.000	0.000
					C	1.630	6.150		61.59	15.840	0.000
T2 180.00-160.00	170.00	1.15	25	74.792	A	2.853	12.348	9.583	63.05	5.633	0.000
					B	2.853	12.348		63.05	0.000	0.000
					C	2.853	12.348		63.05	31.680	0.000
T3 160.00-140.00	150.00	1.11	24	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348		63.05	0.000	0.000
					C	2.853	12.348		63.05	31.680	0.000
T4 140.00-120.00	130.00	1.065	23	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348		63.05	45.711	0.000
					C	2.853	12.348		63.05	31.680	0.000
T5 120.00-100.00	110.00	1.016	22	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348		63.05	50.790	0.000
					C	2.853	12.348		63.05	0.000	0.000



<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 20 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section Elevation ft	z ft	K <sub>z</sub>	q <sub>z</sub> psf	A <sub>G</sub> ft <sup>2</sup>	F a c e	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft <sup>2</sup>	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
T6 100.00-80.00	90.00	0.959	21	74.792	C	2.853	12.348	9.583	63.05	31.680	0.000
					A	2.853	12.348			10.050	0.000
					B	2.853	12.348			50.790	0.000
T7 80.00-60.00	70.00	0.892	19	74.792	C	2.853	12.348	9.583	63.05	31.680	0.000
					A	2.853	12.348			10.050	0.000
					B	2.853	12.348			50.790	0.000
T8 60.00-40.00	50.00	0.811	18	74.792	C	2.853	12.348	9.583	63.05	31.680	0.000
					A	2.853	12.348			10.050	0.000
					B	2.853	12.348			50.790	0.000
T9 40.00-20.00	30.00	0.701	15	74.792	C	2.853	12.348	9.583	63.05	31.680	0.000
					A	2.853	12.348			10.050	0.000
					B	2.853	12.348			50.790	0.000
T10 20.00-0.00	10.00	0.7	15	74.792	C	2.853	12.348	9.583	63.05	31.680	0.000
					A	2.853	12.348			10.050	0.000
					B	2.853	12.348			50.790	0.000
					C	2.853	12.348		63.05	31.680	0.000

### Tower Pressure - With Ice

$G_H = 0.850$

Section Elevation ft	z ft	K <sub>z</sub>	q <sub>z</sub> psf	t <sub>z</sub> in	A <sub>G</sub> ft <sup>2</sup>	F a c e	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft <sup>2</sup>	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	
T1 190.00-180.00	185.00	1.178	6	2.3763	41.356	A	1.630	29.561	12.713	40.76	5.128	0.000	
						B	1.630	29.561			0.000	0.000	
						C	1.630	29.561			40.76	44.114	0.000
T2 180.00-160.00	170.00	1.15	6	2.3563	82.646	A	2.853	57.862	25.292	41.66	27.203	0.000	
						B	2.853	57.862			41.66	0.000	
						C	2.853	57.862			41.66	87.966	0.000
T3 160.00-140.00	150.00	1.11	6	2.3270	82.548	A	2.853	57.296	25.096	41.72	42.294	0.000	
						B	2.853	57.296			41.72	0.000	
						C	2.853	57.296			41.72	87.580	0.000
T4 140.00-120.00	130.00	1.065	6	2.2939	82.438	A	2.853	56.658	24.876	41.80	41.937	0.000	
						B	2.853	56.658			41.80	80.512	0.000
						C	2.853	56.658			41.80	87.146	0.000
T5 120.00-100.00	110.00	1.016	6	2.2559	82.311	A	2.853	55.923	24.623	41.89	41.528	0.000	
						B	2.853	55.923			41.89	88.788	0.000
						C	2.853	55.923			41.89	86.647	0.000
T6 100.00-80.00	90.00	0.959	5	2.2111	82.162	A	2.853	55.058	24.324	42.00	41.045	0.000	
						B	2.853	55.058			42.00	87.999	0.000
						C	2.853	55.058			42.00	86.059	0.000
T7 80.00-60.00	70.00	0.892	5	2.1562	81.979	A	2.853	53.998	23.958	42.14	40.453	0.000	
						B	2.853	53.998			42.14	87.032	0.000
						C	2.853	53.998			42.14	85.340	0.000
T8 60.00-40.00	50.00	0.811	4	2.0849	81.741	A	2.853	52.620	23.482	42.33	39.686	0.000	
						B	2.853	52.620			42.33	85.776	0.000
						C	2.853	52.620			42.33	84.407	0.000
T9 40.00-20.00	30.00	0.701	4	1.9810	81.395	A	2.853	50.614	22.790	42.62	38.570	0.000	
						B	2.853	50.614			42.62	83.949	0.000
						C	2.853	50.614			42.62	83.052	0.000
T10 20.00-0.00	10.00	0.7	4	1.7749	80.708	A	2.853	46.633	21.416	43.28	36.359	0.000	
						B	2.853	46.633			43.28	80.329	0.000
						C	2.853	46.633			43.28	80.371	0.000

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 21 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

**Tower Pressure - Service**

$G_H = 0.850$

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face
ft	ft		psf	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 190.00-180.00	185.00	1.178	9	37.396	A	1.630	6.150	4.792	61.59	0.375	0.000
					B	1.630	6.150	61.59	0.000	0.000	
					C	1.630	6.150	61.59	15.840	0.000	
T2 180.00-160.00	170.00	1.15	9	74.792	A	2.853	12.348	9.583	63.05	5.633	0.000
					B	2.853	12.348	63.05	0.000	0.000	
					C	2.853	12.348	63.05	31.680	0.000	
T3 160.00-140.00	150.00	1.11	9	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348	63.05	0.000	0.000	
					C	2.853	12.348	63.05	31.680	0.000	
T4 140.00-120.00	130.00	1.065	8	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348	63.05	45.711	0.000	
					C	2.853	12.348	63.05	31.680	0.000	
T5 120.00-100.00	110.00	1.016	8	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348	63.05	50.790	0.000	
					C	2.853	12.348	63.05	31.680	0.000	
T6 100.00-80.00	90.00	0.959	8	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348	63.05	50.790	0.000	
					C	2.853	12.348	63.05	31.680	0.000	
T7 80.00-60.00	70.00	0.892	7	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348	63.05	50.790	0.000	
					C	2.853	12.348	63.05	31.680	0.000	
T8 60.00-40.00	50.00	0.811	6	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348	63.05	50.790	0.000	
					C	2.853	12.348	63.05	31.680	0.000	
T9 40.00-20.00	30.00	0.701	5	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348	63.05	50.790	0.000	
					C	2.853	12.348	63.05	31.680	0.000	
T10 20.00-0.00	10.00	0.7	5	74.792	A	2.853	12.348	9.583	63.05	10.050	0.000
					B	2.853	12.348	63.05	50.790	0.000	
					C	2.853	12.348	63.05	31.680	0.000	

**Tower Forces - No Ice - Wind Normal To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	lb	lb				psf			ft <sup>2</sup>	lb	plf	
T1 190.00-180.00	79.60	336.97	A	0.208	2.569	26	1	1	5.172	501.57	50.16	C
			B	0.208	2.569	1	1	5.172				
			C	0.208	2.569	1	1	5.172				
T2 180.00-160.00	179.99	658.24 TA 214.38	A	0.203	2.585	25	1	1	9.953	1023.53	51.18	C
			B	0.203	2.585	1	1	9.953				
			C	0.203	2.585	1	1	9.953				
T3 160.00-140.00	198.80	658.24	A	0.203	2.585	24	1	1	9.953	1041.97	52.10	C
			B	0.203	2.585	1	1	9.953				
			C	0.203	2.585	1	1	9.953				
T4 140.00-120.00	433.70	658.24 TA 214.38	A	0.203	2.585	23	1	1	9.953	1671.00	83.55	B
			B	0.203	2.585	1	1	9.953				
			C	0.203	2.585	1	1	9.953				

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 22 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	lb	lb	e			psf			ft <sup>2</sup>	lb	plf	
T5 120.00-100.00	459.80	658.24	A	0.203	2.585	22	1	1	9.953	1676.06	83.80	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T6 100.00-80.00	459.80	658.24	A	0.203	2.585	21	1	1	9.953	1582.67	79.13	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T7 80.00-60.00	459.80	658.24	A	0.203	2.585	19	1	1	9.953	1473.01	73.65	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T8 60.00-40.00	459.80	658.24	A	0.203	2.585	18	1	1	9.953	1338.00	66.90	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T9 40.00-20.00	459.80	658.24	A	0.203	2.585	15	1	1	9.953	1156.30	57.81	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T10 20.00-0.00	459.80	658.24	A	0.203	2.585	15	1	1	9.953	1155.32	57.77	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
Sum Weight:	3650.89	6689.93								12619.42		

### Tower Forces - No Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	lb	lb	e			psf			ft <sup>2</sup>	lb	plf	
T1 190.00-180.00	79.60	336.97	A	0.208	2.569	26	0.8	1	4.846	483.32	48.33	B
			B	0.208	2.569		0.8	1	4.846			
			C	0.208	2.569		0.8	1	4.846			
T2 180.00-160.00	179.99	658.24	A	0.203	2.585	25	0.8	1	9.383	992.15	49.61	B
		TA 214.38	B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T3 160.00-140.00	198.80	658.24	A	0.203	2.585	24	0.8	1	9.383	1011.69	50.58	B
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T4 140.00-120.00	433.70	658.24	A	0.203	2.585	23	0.8	1	9.383	1641.94	82.10	A
		TA 214.38	B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T5 120.00-100.00	459.80	658.24	A	0.203	2.585	22	0.8	1	9.383	1648.36	82.42	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T6 100.00-80.00	459.80	658.24	A	0.203	2.585	21	0.8	1	9.383	1556.51	77.83	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T7 80.00-60.00	459.80	658.24	A	0.203	2.585	19	0.8	1	9.383	1448.66	72.43	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T8 60.00-40.00	459.80	658.24	A	0.203	2.585	18	0.8	1	9.383	1315.88	65.79	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T9 40.00-20.00	459.80	658.24	A	0.203	2.585	15	0.8	1	9.383	1137.18	56.86	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
T10	459.80	658.24	A	0.203	2.585	15	0.8	1	9.383	1136.22	56.81	A

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 23 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	lb	lb				psf			ft <sup>2</sup>	lb	plf	
20.00-0.00			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
Sum Weight:	3650.89	6689.93								12371.91		

**Tower Forces - No Ice - Wind 90 To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	lb	lb				psf			ft <sup>2</sup>	lb	plf	
T1	79.60	336.97	A	0.208	2.569	26	0.85	1	4.927	487.88	48.79	B
190.00-180.00			B	0.208	2.569		0.85	1	4.927			
			C	0.208	2.569		0.85	1	4.927			
T2	179.99	658.24	A	0.203	2.585	25	0.85	1	9.526	999.99	50.00	B
180.00-160.00		TA 214.38	B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
T3	198.80	658.24	A	0.203	2.585	24	0.85	1	9.526	1019.26	50.96	B
160.00-140.00			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
T4	433.70	658.24	A	0.203	2.585	23	0.85	1	9.526	1629.37	81.47	A
140.00-120.00		TA 214.38	B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
T5	459.80	658.24	A	0.203	2.585	22	0.85	1	9.526	1622.38	81.12	A
120.00-100.00			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
T6	459.80	658.24	A	0.203	2.585	21	0.85	1	9.526	1531.98	76.60	A
100.00-80.00			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
T7	459.80	658.24	A	0.203	2.585	19	0.85	1	9.526	1425.84	71.29	A
80.00-60.00			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
T8	459.80	658.24	A	0.203	2.585	18	0.85	1	9.526	1295.15	64.76	A
60.00-40.00			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
T9	459.80	658.24	A	0.203	2.585	15	0.85	1	9.526	1119.27	55.96	A
40.00-20.00			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
T10	459.80	658.24	A	0.203	2.585	15	0.85	1	9.526	1118.32	55.92	A
20.00-0.00			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
Sum Weight:	3650.89	6689.93								12249.45		

**Tower Forces - With Ice - Wind Normal To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	lb	lb				psf			ft <sup>2</sup>	lb	plf	
T1	834.36	2074.29	A	0.754	1.789	6	1	1	26.776	325.21	32.52	C

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 24 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section Elevation ft	Add Weight lb	Self Weight lb	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F lb	w plf	Ctrl. Face
190.00-180.00			B	0.754	1.789		1	1	26.776			
			C	0.754	1.789		1	1	26.776			
T2	1907.66	3976.13	A	0.735	1.782	6	1	1	51.234	644.47	32.22	C
180.00-160.00			TA	0.735	1.782		1	1	51.234			
		1038.49	C	0.735	1.782		1	1	51.234			
T3	2108.80	3909.50	A	0.729	1.781	6	1	1	50.509	638.76	31.94	C
160.00-140.00			B	0.729	1.781		1	1	50.509			
			C	0.729	1.781		1	1	50.509			
T4	3852.67	3835.10	A	0.722	1.779	6	1	1	49.698	846.41	42.32	B
140.00-120.00			TA	0.722	1.779		1	1	49.698			
		1008.03	C	0.722	1.779		1	1	49.698			
T5	3982.63	3750.58	A	0.714	1.778	6	1	1	48.775	811.67*	40.58	B
120.00-100.00			B	0.714	1.778		1	1	48.775			
			C	0.714	1.778		1	1	48.775			
T6	3903.93	3652.29	A	0.705	1.776	5	1	1	47.699	765.05*	38.25	B
100.00-80.00			B	0.705	1.776		1	1	47.699			
			C	0.705	1.776		1	1	47.699			
T7	3808.63	3533.98	A	0.693	1.776	5	1	1	46.400	710.46*	35.52	B
80.00-60.00			B	0.693	1.776		1	1	46.400			
			C	0.693	1.776		1	1	46.400			
T8	3686.49	3383.50	A	0.679	1.776	4	1	1	44.741	642.59	32.13	B
60.00-40.00			B	0.679	1.776		1	1	44.741			
			C	0.679	1.776		1	1	44.741			
T9	3512.29	3171.29	A	0.657	1.78	4	1	1	42.387	545.32	27.27	B
40.00-20.00			B	0.657	1.78		1	1	42.387			
			C	0.657	1.78		1	1	42.387			
T10	3178.98	2773.77	A	0.613	1.796	4	1	1	37.928	526.04	26.30	B
20.00-0.00			B	0.613	1.796		1	1	37.928			
			C	0.613	1.796		1	1	37.928			
Sum Weight:	30776.44	36106.94			2.1A <sub>g</sub> limit					6455.99		

### Tower Forces - With Ice - Wind 60 To Face

Section Elevation ft	Add Weight lb	Self Weight lb	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F lb	w plf	Ctrl. Face
T1	834.36	2074.29	A	0.754	1.789	6	0.8	1	26.450	322.03	32.20	B
190.00-180.00			B	0.754	1.789		0.8	1	26.450			
			C	0.754	1.789		0.8	1	26.450			
T2	1907.66	3976.13	A	0.735	1.782	6	0.8	1	50.663	639.06	31.95	B
180.00-160.00			TA	0.735	1.782		0.8	1	50.663			
		1038.49	C	0.735	1.782		0.8	1	50.663			
T3	2108.80	3909.50	A	0.729	1.781	6	0.8	1	49.938	633.55	31.68	B
160.00-140.00			B	0.729	1.781		0.8	1	49.938			
			C	0.729	1.781		0.8	1	49.938			
T4	3852.67	3835.10	A	0.722	1.779	6	0.8	1	49.127	841.41	42.07	A
140.00-120.00			TA	0.722	1.779		0.8	1	49.127			
		1008.03	C	0.722	1.779		0.8	1	49.127			
T5	3982.63	3750.58	A	0.714	1.778	6	0.8	1	48.204	811.67*	40.58	A
120.00-100.00			B	0.714	1.778		0.8	1	48.204			
			C	0.714	1.778		0.8	1	48.204			
T6	3903.93	3652.29	A	0.705	1.776	5	0.8	1	47.128	765.05*	38.25	A

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 25 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section Elevation ft	Add Weight lb	Self Weight lb	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F lb	w plf	Ctrl. Face
100.00-80.00			B	0.705	1.776		0.8	1	47.128			
			C	0.705	1.776		0.8	1	47.128			
T7 80.00-60.00	3808.63	3533.98	A	0.693	1.776	5	0.8	1	45.829	710.46*	35.52	A
			B	0.693	1.776		0.8	1	45.829			
			C	0.693	1.776		0.8	1	45.829			
T8 60.00-40.00	3686.49	3383.50	A	0.679	1.776	4	0.8	1	44.170	638.79	31.94	A
			B	0.679	1.776		0.8	1	44.170			
			C	0.679	1.776		0.8	1	44.170			
T9 40.00-20.00	3512.29	3171.29	A	0.657	1.78	4	0.8	1	41.817	542.03	27.10	A
			B	0.657	1.78		0.8	1	41.817			
			C	0.657	1.78		0.8	1	41.817			
T10 20.00-0.00	3178.98	2773.77	A	0.613	1.796	4	0.8	1	37.357	522.73	26.14	A
			B	0.613	1.796		0.8	1	37.357			
			C	0.613	1.796		0.8	1	37.357			
Sum Weight:	30776.44	36106.94			*2.1A <sub>e</sub> limit					6426.79		

### Tower Forces - With Ice - Wind 90 To Face

Section Elevation ft	Add Weight lb	Self Weight lb	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F lb	w plf	Ctrl. Face
T1 190.00-180.00	834.36	2074.29	A	0.754	1.789	6	0.85	1	26.531	316.59	31.66	B
			B	0.754	1.789		0.85	1	26.531			
			C	0.754	1.789		0.85	1	26.531			
T2 180.00-160.00	1907.66	3976.13	A	0.735	1.782	6	0.85	1	50.806	627.28	31.36	B
			TA	0.735	1.782		0.85	1	50.806			
		1038.49	C	0.735	1.782		0.85	1	50.806			
T3 160.00-140.00	2108.80	3909.50	A	0.729	1.781	6	0.85	1	50.081	621.89	31.09	B
			B	0.729	1.781		0.85	1	50.081			
			C	0.729	1.781		0.85	1	50.081			
T4 140.00-120.00	3852.67	3835.10	A	0.722	1.779	6	0.85	1	49.270	835.15	41.76	A
			TA	0.722	1.779		0.85	1	49.270			
		1008.03	C	0.722	1.779		0.85	1	49.270			
T5 120.00-100.00	3982.63	3750.58	A	0.714	1.778	6	0.85	1	48.347	811.67*	40.58	A
			B	0.714	1.778		0.85	1	48.347			
			C	0.714	1.778		0.85	1	48.347			
T6 100.00-80.00	3903.93	3652.29	A	0.705	1.776	5	0.85	1	47.271	765.05*	38.25	A
			B	0.705	1.776		0.85	1	47.271			
			C	0.705	1.776		0.85	1	47.271			
T7 80.00-60.00	3808.63	3533.98	A	0.693	1.776	5	0.85	1	45.972	706.19	35.31	A
			B	0.693	1.776		0.85	1	45.972			
			C	0.693	1.776		0.85	1	45.972			
T8 60.00-40.00	3686.49	3383.50	A	0.679	1.776	4	0.85	1	44.313	634.25	31.71	A
			B	0.679	1.776		0.85	1	44.313			
			C	0.679	1.776		0.85	1	44.313			
T9 40.00-20.00	3512.29	3171.29	A	0.657	1.78	4	0.85	1	41.959	539.42	26.97	A
			B	0.657	1.78		0.85	1	41.959			
			C	0.657	1.78		0.85	1	41.959			
T10 20.00-0.00	3178.98	2773.77	A	0.613	1.796	4	0.85	1	37.500	522.76	26.14	A
			B	0.613	1.796		0.85	1	37.500			
			C	0.613	1.796		0.85	1	37.500			
Sum Weight:	30776.44	36106.94			*2.1A <sub>e</sub>					6380.25		

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 26 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	lb	lb			limit	psf			ft <sup>2</sup>	lb	plf	

### Tower Forces - Service - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	lb	lb				psf			ft <sup>2</sup>	lb	plf	
T1 190.00-180.00	79.60	336.97	A	0.208	2.569	9	1	1	5.172	180.57	18.06	C
			B	0.208	2.569		1	1	5.172			
			C	0.208	2.569		1	1	5.172			
T2 180.00-160.00	179.99	658.24	A	0.203	2.585	9	1	1	9.953	368.47	18.42	C
		TA 214.38	B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T3 160.00-140.00	198.80	658.24	A	0.203	2.585	9	1	1	9.953	375.11	18.76	C
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T4 140.00-120.00	433.70	658.24	A	0.203	2.585	8	1	1	9.953	601.56	30.08	B
		TA 214.38	B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T5 120.00-100.00	459.80	658.24	A	0.203	2.585	8	1	1	9.953	603.38	30.17	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T6 100.00-80.00	459.80	658.24	A	0.203	2.585	8	1	1	9.953	569.76	28.49	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T7 80.00-60.00	459.80	658.24	A	0.203	2.585	7	1	1	9.953	530.28	26.51	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T8 60.00-40.00	459.80	658.24	A	0.203	2.585	6	1	1	9.953	481.68	24.08	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T9 40.00-20.00	459.80	658.24	A	0.203	2.585	5	1	1	9.953	416.27	20.81	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
T10 20.00-0.00	459.80	658.24	A	0.203	2.585	5	1	1	9.953	415.92	20.80	B
			B	0.203	2.585		1	1	9.953			
			C	0.203	2.585		1	1	9.953			
Sum Weight:	3650.89	6689.93								4542.99		

### Tower Forces - Service - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	lb	lb				psf			ft <sup>2</sup>	lb	plf	
T1 190.00-180.00	79.60	336.97	A	0.208	2.569	9	0.8	1	4.846	173.99	17.40	B
			B	0.208	2.569		0.8	1	4.846			
			C	0.208	2.569		0.8	1	4.846			

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 27 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section Elevation ft	Add Weight lb	Self Weight lb	F a c e	e	C <sub>F</sub>	q <sub>r</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F lb	w plf	Ctrl. Face
180.00-160.00	179.99	658.24 TA 214.38	A	0.203	2.585	9	0.8	1	9.383	357.17	17.86	B
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
160.00-140.00	198.80	658.24	A	0.203	2.585	9	0.8	1	9.383	364.21	18.21	B
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
140.00-120.00	433.70	658.24 TA 214.38	A	0.203	2.585	8	0.8	1	9.383	591.10	29.55	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
120.00-100.00	459.80	658.24	A	0.203	2.585	8	0.8	1	9.383	593.41	29.67	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
100.00-80.00	459.80	658.24	A	0.203	2.585	8	0.8	1	9.383	560.34	28.02	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
80.00-60.00	459.80	658.24	A	0.203	2.585	7	0.8	1	9.383	521.52	26.08	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
60.00-40.00	459.80	658.24	A	0.203	2.585	6	0.8	1	9.383	473.72	23.69	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
40.00-20.00	459.80	658.24	A	0.203	2.585	5	0.8	1	9.383	409.39	20.47	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
20.00-0.00	459.80	658.24	A	0.203	2.585	5	0.8	1	9.383	409.04	20.45	A
			B	0.203	2.585		0.8	1	9.383			
			C	0.203	2.585		0.8	1	9.383			
Sum Weight:	3650.89	6689.93								4453.89		

### Tower Forces - Service - Wind 90 To Face

Section Elevation ft	Add Weight lb	Self Weight lb	F a c e	e	C <sub>F</sub>	q <sub>r</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F lb	w plf	Ctrl. Face
190.00-180.00	79.60	336.97	A	0.208	2.569	9	0.85	1	4.927	175.64	17.56	B
			B	0.208	2.569		0.85	1	4.927			
			C	0.208	2.569		0.85	1	4.927			
180.00-160.00	179.99	658.24 TA 214.38	A	0.203	2.585	9	0.85	1	9.526	360.00	18.00	B
			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
160.00-140.00	198.80	658.24	A	0.203	2.585	9	0.85	1	9.526	366.93	18.35	B
			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
140.00-120.00	433.70	658.24 TA 214.38	A	0.203	2.585	8	0.85	1	9.526	586.57	29.33	A
			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
120.00-100.00	459.80	658.24	A	0.203	2.585	8	0.85	1	9.526	584.06	29.20	A
			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
100.00-80.00	459.80	658.24	A	0.203	2.585	8	0.85	1	9.526	551.51	27.58	A
			B	0.203	2.585		0.85	1	9.526			
			C	0.203	2.585		0.85	1	9.526			
20.00-0.00	459.80	658.24	A	0.203	2.585	7	0.85	1	9.526	513.30	25.67	A





<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 29 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Description	Aiming Azimuth °	Weight lb	Offset <sub>x</sub> ft	Offset <sub>z</sub> ft	z ft	K <sub>z</sub>	q <sub>z</sub> psf	C <sub>AAc</sub> Front ft <sup>2</sup>	C <sub>AAc</sub> Side ft <sup>2</sup>
ET									
Ericsson RRUS11	0.0000	50.00	0.00	-5.02	138.00	1.083	24	5.98	0.72
Ericsson RRUS11	120.0000	50.00	4.35	2.51	138.00	1.083	24	5.98	0.72
Ericsson RRUS11	240.0000	50.00	-4.35	2.51	138.00	1.083	24	5.98	0.72
Raycap	0.0000	25.00	0.00	-5.02	138.00	1.083	24	1.47	1.47
DC6-48-60-18-8F									
Sector Frame Mount	0.0000	465.00	0.00	-3.52	170.50	1.151	25	13.60	13.60
Sector Frame Mount	120.0000	465.00	3.05	1.76	170.50	1.151	25	13.60	13.60
Sector Frame Mount	240.0000	465.00	-3.05	1.76	170.50	1.151	25	13.60	13.60
RFS APXV9ERR18-C	0.0000	62.00	0.00	-5.02	170.50	1.151	25	8.02	5.81
Alcatel Lucent 1900	0.0000	60.00	0.00	-5.02	170.50	1.151	25	2.32	2.24
RRH									
Alcatel Lucent 800 RRH	0.0000	60.00	0.00	-5.02	170.50	1.151	25	2.06	1.36
RFS APXV9ERR18-C	120.0000	62.00	4.35	2.51	170.50	1.151	25	8.02	5.81
Alcatel Lucent 1900	120.0000	60.00	4.35	2.51	170.50	1.151	25	2.32	2.24
RRH									
Alcatel Lucent 800 RRH	120.0000	60.00	4.35	2.51	170.50	1.151	25	2.06	1.36
RFS APXV9ERR18-C	240.0000	62.00	-4.35	2.51	170.50	1.151	25	8.02	5.81
Alcatel Lucent 1900	240.0000	60.00	-4.35	2.51	170.50	1.151	25	2.32	2.24
RRH									
Alcatel Lucent 800 RRH	240.0000	60.00	-4.35	2.51	170.50	1.151	25	2.06	1.36
Sector Frame Mount	0.0000	465.00	0.00	-3.52	138.00	1.083	24	13.60	13.60
Sector Frame Mount	120.0000	465.00	3.05	1.76	138.00	1.083	24	13.60	13.60
Sector Frame Mount	240.0000	465.00	-3.05	1.76	138.00	1.083	24	13.60	13.60
Commscope	0.0000	126.60	0.00	-5.02	190.00	1.187	26	18.22	6.88
JAHH-65B-R3B									
Alcatel Lucent B66A	0.0000	51.00	0.00	-5.02	190.00	1.187	26	2.54	1.61
RRH4x45									
Alcatel Lucent B13	0.0000	57.20	0.00	-5.02	190.00	1.187	26	2.16	1.62
RRH4x30									
Alcatel Lucent RRH	0.0000	50.00	0.00	-5.02	190.00	1.187	26	1.28	0.72
4T4R B5									
RFS	0.0000	44.00	0.00	-5.02	190.00	1.187	26	2.80	1.17
DB-C1-12C-24AB-0Z									
Commscope	120.0000	126.60	4.35	2.51	190.00	1.187	26	18.22	6.88
JAHH-65B-R3B									
Alcatel Lucent B66A	120.0000	51.00	4.35	2.51	190.00	1.187	26	2.54	1.61
RRH4x45									
Alcatel Lucent B13	120.0000	57.20	4.35	2.51	190.00	1.187	26	2.16	1.62
RRH4x30									
Alcatel Lucent RRH	120.0000	50.00	4.35	2.51	190.00	1.187	26	1.28	0.72
4T4R B5									
RFS	120.0000	44.00	4.35	2.51	190.00	1.187	26	2.80	1.17
DB-C1-12C-24AB-0Z									
Commscope	240.0000	126.60	-4.35	2.51	190.00	1.187	26	18.22	6.88
JAHH-65B-R3B									
Alcatel Lucent B66A	240.0000	51.00	-4.35	2.51	190.00	1.187	26	2.54	1.61
RRH4x45									
Alcatel Lucent B13	240.0000	57.20	-4.35	2.51	190.00	1.187	26	2.16	1.62
RRH4x30									
Alcatel Lucent RRH	240.0000	50.00	-4.35	2.51	190.00	1.187	26	1.28	0.72
4T4R B5									
Sum Weight:		9403.40							

**Discrete Appurtenance Pressures - With Ice**       $G_H = 0.850$

<b>RISATower</b>	<b>Job</b>	117-23150	<b>Page</b>	30 of 56
	<b>Project</b>	Ashford, CT	<b>Date</b>	00:05:19 10/19/17
	<b>Client</b>	CDT	<b>Designed by</b>	FAN
Phone:				
FAX:				

Description	Aiming Azimuth °	Weight lb	Offset <sub>x</sub> ft	Offset <sub>y</sub> ft	z ft	K <sub>z</sub>	q <sub>z</sub> psf	C <sub>AAc</sub> Front ft <sup>2</sup>	C <sub>AAc</sub> Side ft <sup>2</sup>	t <sub>z</sub> in
Torque Arm Face C	180.0000	0.00	0.00	2.53	161.26	1.133	6	6.77	9.91	2.3563
Torque Arm Face B	60.0000	0.00	2.19	-1.26	161.26	1.133	6	6.77	9.91	2.3563
Torque Arm Face A	300.0000	0.00	-2.19	-1.26	161.26	1.133	6	6.77	9.91	2.3563
Torque Arm Face C	180.0000	0.00	0.00	2.53	121.26	1.044	6	6.68	9.79	2.2939
Torque Arm Face B	60.0000	0.00	2.19	-1.26	121.26	1.044	6	6.68	9.79	2.2939
Torque Arm Face A	300.0000	0.00	-2.19	-1.26	121.26	1.044	6	6.68	9.79	2.2939
Sector Frame Mount	0.0000	1108.31	0.00	-3.52	190.00	1.187	6	36.47	36.47	2.3826
Sector Frame Mount	120.0000	1108.31	3.05	1.76	190.00	1.187	6	36.47	36.47	2.3826
Sector Frame Mount	240.0000	1108.31	-3.05	1.76	190.00	1.187	6	36.47	36.47	2.3826
Antel LPA-80080-4CF	0.0000	286.09	0.00	-5.02	190.00	1.187	6	6.88	10.15	2.3826
Antel LPA-80063/4CF	120.0000	286.09	4.35	2.51	190.00	1.187	6	6.88	10.15	2.3826
Antel LPA-80063/4CF	240.0000	286.09	-4.35	2.51	190.00	1.187	6	6.88	10.15	2.3826
Sector Frame Mount	0.0000	1104.12	0.00	-3.52	178.00	1.165	6	36.32	36.32	2.3671
Sector Frame Mount	120.0000	1104.12	3.05	1.76	178.00	1.165	6	36.32	36.32	2.3671
Sector Frame Mount	240.0000	1104.12	-3.05	1.76	178.00	1.165	6	36.32	36.32	2.3671
Allgon 7770.00	0.0000	418.14	0.00	-5.02	138.00	1.083	6	16.19	9.71	2.3076
Allgon 7770.00	120.0000	418.14	4.35	2.51	138.00	1.083	6	16.19	9.71	2.3076
Allgon 7770.00	240.0000	418.14	-4.35	2.51	138.00	1.083	6	16.19	9.71	2.3076
Powerwave LGP21401	0.0000	678.91	0.00	-5.02	138.00	1.083	6	15.18	15.18	2.3076
Powerwave LGP21401	120.0000	678.91	4.35	2.51	138.00	1.083	6	15.18	15.18	2.3076
Powerwave LGP21401	240.0000	678.91	-4.35	2.51	138.00	1.083	6	15.18	15.18	2.3076
Powerwave LGP21901	0.0000	678.91	0.00	-5.02	138.00	1.083	6	15.18	15.18	2.3076
Powerwave LGP21901	120.0000	678.91	4.35	2.51	138.00	1.083	6	15.18	15.18	2.3076
Powerwave LGP21901	240.0000	678.91	-4.35	2.51	138.00	1.083	6	15.18	15.18	2.3076
Sector Frame Mount	0.0000	1093.28	0.00	-3.52	150.00	1.110	6	35.94	35.94	2.3270
Sector Frame Mount	120.0000	1093.28	3.05	1.76	150.00	1.110	6	35.94	35.94	2.3270
Sector Frame Mount	240.0000	1093.28	-3.05	1.76	150.00	1.110	6	35.94	35.94	2.3270
Lightning Rod	0.0000	151.24	0.00	0.00	190.00	1.187	6	5.63	5.63	2.3826
KMW	0.0000	209.07	0.00	-5.02	138.00	1.083	6	8.09	4.86	2.3076
AM-X-CD-17-65-00T-R ET										
KMW	120.0000	209.07	4.35	2.51	138.00	1.083	6	8.09	4.86	2.3076
AM-X-CD-17-65-00T-R ET										
KMW	240.0000	209.07	-4.35	2.51	138.00	1.083	6	8.09	4.86	2.3076
AM-X-CD-17-65-00T-R ET										
Ericsson RRUS11	0.0000	418.14	0.00	-5.02	138.00	1.083	6	16.19	9.71	2.3076
Ericsson RRUS11	120.0000	418.14	4.35	2.51	138.00	1.083	6	16.19	9.71	2.3076
Ericsson RRUS11	240.0000	418.14	-4.35	2.51	138.00	1.083	6	16.19	9.71	2.3076
Raycap	0.0000	209.07	0.00	-5.02	138.00	1.083	6	8.09	4.86	2.3076
DC6-48-60-18-8F										
Sector Frame Mount	0.0000	1101.38	0.00	-3.52	170.50	1.151	6	36.23	36.23	2.3570
Sector Frame Mount	120.0000	1101.38	3.05	1.76	170.50	1.151	6	36.23	36.23	2.3570
Sector Frame Mount	240.0000	1101.38	-3.05	1.76	170.50	1.151	6	36.23	36.23	2.3570
RFS APXV9ERR18-C	0.0000	156.47	0.00	-5.02	170.50	1.151	6	5.81	4.07	2.3570
Alcatel Lucent 1900	0.0000	156.47	0.00	-5.02	170.50	1.151	6	5.81	4.07	2.3570
RRH										
Alcatel Lucent 800 RRH	0.0000	156.47	0.00	-5.02	170.50	1.151	6	5.81	4.07	2.3570
RFS APXV9ERR18-C	120.0000	156.47	4.35	2.51	170.50	1.151	6	5.81	4.07	2.3570
Alcatel Lucent 1900	120.0000	156.47	4.35	2.51	170.50	1.151	6	5.81	4.07	2.3570
RRH										
Alcatel Lucent 800 RRH	120.0000	156.47	4.35	2.51	170.50	1.151	6	5.81	4.07	2.3570
RFS APXV9ERR18-C	240.0000	156.47	-4.35	2.51	170.50	1.151	6	5.81	4.07	2.3570
Alcatel Lucent 1900	240.0000	156.47	-4.35	2.51	170.50	1.151	6	5.81	4.07	2.3570
RRH										
Alcatel Lucent 800 RRH	240.0000	156.47	-4.35	2.51	170.50	1.151	6	5.81	4.07	2.3570
Sector Frame Mount	0.0000	1088.06	0.00	-3.52	138.00	1.083	6	35.75	35.75	2.3076
Sector Frame Mount	120.0000	1088.06	3.05	1.76	138.00	1.083	6	35.75	35.75	2.3076
Sector Frame Mount	240.0000	1088.06	-3.05	1.76	138.00	1.083	6	35.75	35.75	2.3076
Commscope	0.0000	1293.13	0.00	-5.02	190.00	1.187	6	26.99	15.65	2.3826

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 31 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Description	Aiming Azimuth °	Weight lb	Offset <sub>x</sub> ft	Offset <sub>z</sub> ft	z ft	K <sub>z</sub>	q <sub>z</sub> psf	C <sub>AAc</sub> Front ft <sup>2</sup>	C <sub>AAc</sub> Side ft <sup>2</sup>	t <sub>z</sub> in
JAHH-65B-R3B										
Alcatel Lucent B66A RRH4x45	0.0000	257.33	0.00	-5.02	190.00	1.187	6	4.35	3.28	2.3826
Alcatel Lucent B13 RRH4x30	0.0000	258.29	0.00	-5.02	190.00	1.187	6	3.83	3.14	2.3826
Alcatel Lucent RRH 4T4R B5	0.0000	167.70	0.00	-5.02	190.00	1.187	6	1.60	1.72	2.3826
RFS	0.0000	407.11	0.00	-5.02	190.00	1.187	6	5.13	2.84	2.3826
DB-C1-12C-24AB-0Z Commscope	120.0000	1293.13	4.35	2.51	190.00	1.187	6	26.99	15.65	2.3826
JAHH-65B-R3B										
Alcatel Lucent B66A RRH4x45	120.0000	257.33	4.35	2.51	190.00	1.187	6	4.35	3.28	2.3826
Alcatel Lucent B13 RRH4x30	120.0000	258.29	4.35	2.51	190.00	1.187	6	3.83	3.14	2.3826
Alcatel Lucent RRH 4T4R B5	120.0000	167.70	4.35	2.51	190.00	1.187	6	1.60	1.72	2.3826
RFS	120.0000	407.11	4.35	2.51	190.00	1.187	6	5.13	2.84	2.3826
DB-C1-12C-24AB-0Z Commscope	240.0000	1293.13	-4.35	2.51	190.00	1.187	6	26.99	15.65	2.3826
JAHH-65B-R3B										
Alcatel Lucent B66A RRH4x45	240.0000	257.33	-4.35	2.51	190.00	1.187	6	4.35	3.28	2.3826
Alcatel Lucent B13 RRH4x30	240.0000	258.29	-4.35	2.51	190.00	1.187	6	3.83	3.14	2.3826
Alcatel Lucent RRH 4T4R B5	240.0000	167.70	-4.35	2.51	190.00	1.187	6	1.60	1.72	2.3826
Sum Weight:		33065.32								

### Discrete Appurtenance Pressures - Service G<sub>H</sub> = 0.850

Description	Aiming Azimuth °	Weight lb	Offset <sub>x</sub> ft	Offset <sub>z</sub> ft	z ft	K <sub>z</sub>	q <sub>z</sub> psf	C <sub>AAc</sub> Front ft <sup>2</sup>	C <sub>AAc</sub> Side ft <sup>2</sup>
Torque Arm Face C	180.0000	0.00	0.00	2.53	161.26	1.133	9	3.54	5.32
Torque Arm Face B	60.0000	0.00	2.19	-1.26	161.26	1.133	9	3.54	5.32
Torque Arm Face A	300.0000	0.00	-2.19	-1.26	161.26	1.133	9	3.54	5.32
Torque Arm Face C	180.0000	0.00	0.00	2.53	121.26	1.044	8	3.54	5.32
Torque Arm Face B	60.0000	0.00	2.19	-1.26	121.26	1.044	8	3.54	5.32
Torque Arm Face A	300.0000	0.00	-2.19	-1.26	121.26	1.044	8	3.54	5.32
Sector Frame Mount	0.0000	465.00	0.00	-3.52	190.00	1.187	9	13.60	13.60
Sector Frame Mount	120.0000	465.00	3.05	1.76	190.00	1.187	9	13.60	13.60
Sector Frame Mount	240.0000	465.00	-3.05	1.76	190.00	1.187	9	13.60	13.60
Antel LPA-80080-4CF	0.0000	24.00	0.00	-5.02	190.00	1.187	9	3.02	13.58
Antel LPA-80063/4CF	120.0000	40.00	4.35	2.51	190.00	1.187	9	14.00	6.96
Antel LPA-80063/4CF	240.0000	40.00	-4.35	2.51	190.00	1.187	9	14.00	6.96
Sector Frame Mount	0.0000	465.00	0.00	-3.52	178.00	1.165	9	13.60	13.60
Sector Frame Mount	120.0000	465.00	3.05	1.76	178.00	1.165	9	13.60	13.60
Sector Frame Mount	240.0000	465.00	-3.05	1.76	178.00	1.165	9	13.60	13.60
Allgon 7770.00	0.0000	70.00	0.00	-5.02	138.00	1.083	8	11.76	5.86
Allgon 7770.00	120.0000	70.00	4.35	2.51	138.00	1.083	8	11.76	5.86
Allgon 7770.00	240.0000	70.00	-4.35	2.51	138.00	1.083	8	11.76	5.86
Powerwave LGP21401	0.0000	62.00	0.00	-5.02	138.00	1.083	8	3.90	1.06
Powerwave LGP21401	120.0000	62.00	4.35	2.51	138.00	1.083	8	3.90	1.06
Powerwave LGP21401	240.0000	62.00	-4.35	2.51	138.00	1.083	8	3.90	1.06
Powerwave LGP21901	0.0000	50.00	0.00	-5.02	138.00	1.083	8	0.54	0.26
Powerwave LGP21901	120.0000	50.00	4.35	2.51	138.00	1.083	8	0.54	0.26

<h1>RISATower</h1> <p>Phone: FAX:</p>	<b>Job</b>	117-23150	<b>Page</b>	32 of 56
	<b>Project</b>	Ashford, CT	<b>Date</b>	00:05:19 10/19/17
	<b>Client</b>	CDT	<b>Designed by</b>	FAN

Description	Aiming Azimuth °	Weight lb	Offset <sub>x</sub> ft	Offset <sub>y</sub> ft	z ft	K <sub>x</sub>	q <sub>x</sub> psf	C <sub>AAc</sub> Front ft <sup>2</sup>	C <sub>AAc</sub> Side ft <sup>2</sup>
Powerwave LGP21901	240.0000	50.00	-4.35	2.51	138.00	1.083	8	0.54	0.26
Sector Frame Mount	0.0000	465.00	0.00	-3.52	150.00	1.110	9	13.60	13.60
Sector Frame Mount	120.0000	465.00	3.05	1.76	150.00	1.110	9	13.60	13.60
Sector Frame Mount	240.0000	465.00	-3.05	1.76	150.00	1.110	9	13.60	13.60
Lightning Rod	0.0000	40.00	0.00	0.00	190.00	1.187	9	1.00	1.00
KMW	0.0000	25.00	0.00	-5.02	138.00	1.083	8	11.31	6.80
AM-X-CD-17-65-00T-R ET									
KMW	120.0000	25.00	4.35	2.51	138.00	1.083	8	11.31	6.80
AM-X-CD-17-65-00T-R ET									
KMW	240.0000	25.00	-4.35	2.51	138.00	1.083	8	11.31	6.80
AM-X-CD-17-65-00T-R ET									
Ericsson RRUS11	0.0000	50.00	0.00	-5.02	138.00	1.083	8	5.98	0.72
Ericsson RRUS11	120.0000	50.00	4.35	2.51	138.00	1.083	8	5.98	0.72
Ericsson RRUS11	240.0000	50.00	-4.35	2.51	138.00	1.083	8	5.98	0.72
Raycap	0.0000	25.00	0.00	-5.02	138.00	1.083	8	1.47	1.47
DC6-48-60-18-8F									
Sector Frame Mount	0.0000	465.00	0.00	-3.52	170.50	1.151	9	13.60	13.60
Sector Frame Mount	120.0000	465.00	3.05	1.76	170.50	1.151	9	13.60	13.60
Sector Frame Mount	240.0000	465.00	-3.05	1.76	170.50	1.151	9	13.60	13.60
RFS APXV9ERR18-C	0.0000	62.00	0.00	-5.02	170.50	1.151	9	8.02	5.81
Alcatel Lucent 1900 RRH	0.0000	60.00	0.00	-5.02	170.50	1.151	9	2.32	2.24
Alcatel Lucent 800 RRH	0.0000	60.00	0.00	-5.02	170.50	1.151	9	2.06	1.36
RFS APXV9ERR18-C	120.0000	62.00	4.35	2.51	170.50	1.151	9	8.02	5.81
Alcatel Lucent 1900 RRH	120.0000	60.00	4.35	2.51	170.50	1.151	9	2.32	2.24
Alcatel Lucent 800 RRH	120.0000	60.00	4.35	2.51	170.50	1.151	9	2.06	1.36
RFS APXV9ERR18-C	240.0000	62.00	-4.35	2.51	170.50	1.151	9	8.02	5.81
Alcatel Lucent 1900 RRH	240.0000	60.00	-4.35	2.51	170.50	1.151	9	2.32	2.24
Alcatel Lucent 800 RRH	240.0000	60.00	-4.35	2.51	170.50	1.151	9	2.06	1.36
Sector Frame Mount	0.0000	465.00	0.00	-3.52	138.00	1.083	8	13.60	13.60
Sector Frame Mount	120.0000	465.00	3.05	1.76	138.00	1.083	8	13.60	13.60
Sector Frame Mount	240.0000	465.00	-3.05	1.76	138.00	1.083	8	13.60	13.60
Commscope	0.0000	126.60	0.00	-5.02	190.00	1.187	9	18.22	6.88
JAHH-65B-R3B									
Alcatel Lucent B66A RRH4x45	0.0000	51.00	0.00	-5.02	190.00	1.187	9	2.54	1.61
Alcatel Lucent B13 RRH4x30	0.0000	57.20	0.00	-5.02	190.00	1.187	9	2.16	1.62
Alcatel Lucent RRH 4T4R B5	0.0000	50.00	0.00	-5.02	190.00	1.187	9	1.28	0.72
RFS	0.0000	44.00	0.00	-5.02	190.00	1.187	9	2.80	1.17
DB-C1-12C-24AB-0Z Commscope	120.0000	126.60	4.35	2.51	190.00	1.187	9	18.22	6.88
JAHH-65B-R3B									
Alcatel Lucent B66A RRH4x45	120.0000	51.00	4.35	2.51	190.00	1.187	9	2.54	1.61
Alcatel Lucent B13 RRH4x30	120.0000	57.20	4.35	2.51	190.00	1.187	9	2.16	1.62
Alcatel Lucent RRH 4T4R B5	120.0000	50.00	4.35	2.51	190.00	1.187	9	1.28	0.72
RFS	120.0000	44.00	4.35	2.51	190.00	1.187	9	2.80	1.17
DB-C1-12C-24AB-0Z Commscope	240.0000	126.60	-4.35	2.51	190.00	1.187	9	18.22	6.88
JAHH-65B-R3B									
Alcatel Lucent B66A RRH4x45	240.0000	51.00	-4.35	2.51	190.00	1.187	9	2.54	1.61

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 33 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Description	Aiming Azimuth °	Weight lb	Offset <sub>x</sub> ft	Offset <sub>y</sub> ft	z ft	K <sub>z</sub>	q <sub>z</sub> psf	C <sub>AAc</sub> Front ft <sup>2</sup>	C <sub>AAc</sub> Side ft <sup>2</sup>
Alcatel Lucent B13 RRH4x30	240.0000	57.20	-4.35	2.51	190.00	1.187	9	2.16	1.62
Alcatel Lucent RRH 4T4R B5	240.0000	50.00	-4.35	2.51	190.00	1.187	9	1.28	0.72
Sum Weight:		9403.40							

### Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice+1.0 Guy
3	1.2 Dead+1.6 Wind 30 deg - No Ice+1.0 Guy
4	1.2 Dead+1.6 Wind 60 deg - No Ice+1.0 Guy
5	1.2 Dead+1.6 Wind 90 deg - No Ice+1.0 Guy
6	1.2 Dead+1.6 Wind 120 deg - No Ice+1.0 Guy
7	1.2 Dead+1.6 Wind 150 deg - No Ice+1.0 Guy
8	1.2 Dead+1.6 Wind 180 deg - No Ice+1.0 Guy
9	1.2 Dead+1.6 Wind 210 deg - No Ice+1.0 Guy
10	1.2 Dead+1.6 Wind 240 deg - No Ice+1.0 Guy
11	1.2 Dead+1.6 Wind 270 deg - No Ice+1.0 Guy
12	1.2 Dead+1.6 Wind 300 deg - No Ice+1.0 Guy
13	1.2 Dead+1.6 Wind 330 deg - No Ice+1.0 Guy
14	1.2 Dead+1.0 Ice+1.0 Temp+Guy
15	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp+1.0 Guy
16	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp+1.0 Guy
17	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp+1.0 Guy
18	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp+1.0 Guy
19	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp+1.0 Guy
20	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp+1.0 Guy
21	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp+1.0 Guy
22	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp+1.0 Guy
23	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp+1.0 Guy
24	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp+1.0 Guy
25	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp+1.0 Guy
26	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp+1.0 Guy
27	Dead+Wind 0 deg - Service+Guy
28	Dead+Wind 30 deg - Service+Guy
29	Dead+Wind 60 deg - Service+Guy
30	Dead+Wind 90 deg - Service+Guy
31	Dead+Wind 120 deg - Service+Guy
32	Dead+Wind 150 deg - Service+Guy
33	Dead+Wind 180 deg - Service+Guy
34	Dead+Wind 210 deg - Service+Guy
35	Dead+Wind 240 deg - Service+Guy
36	Dead+Wind 270 deg - Service+Guy
37	Dead+Wind 300 deg - Service+Guy
38	Dead+Wind 330 deg - Service+Guy

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
----------------	-----------------	-------------------	-----------	-----------------------	-------------	-------------------------------	-------------------------------

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 34 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft		
T1	190 - 180	Leg	Max Tension	5	0.01	-0.13	0.18		
			Max. Compression	25	-18579.48	-63.25	-35.55		
			Max. Mx	11	-11115.37	534.57	0.19		
			Max. My	2	-12715.02	-6.84	420.03		
			Max. Vy	11	-1426.22	534.42	-0.01		
			Max. Vx	2	-1121.73	-6.94	420.03		
			Diagonal	Max Tension	12	4591.17	0.00	0.00	
				Horizontal	Max Tension	25	321.81	0.00	0.00
					Max. Compression	6	-5639.89	0.00	0.00
				Max. Mx	14	293.13	-23.29	0.00	
				Max. My	11	245.26	0.00	0.00	
				Max. Vy	14	26.62	0.00	0.00	
		Bottom Girt	Max. Vx	11	-0.00	0.00	0.00		
			Max Tension	1	0.00	0.00	0.00		
			Max. Compression	6	-2895.25	0.00	0.00		
			Max. Mx	14	-2562.30	-23.29	0.00		
			Max. My	11	-2811.20	0.00	0.00		
			Max. Vy	14	26.62	0.00	0.00		
			Max. Vx	11	-0.00	0.00	0.00		
			Guy A	Bottom Tension	21	10140.85			
		Top Tension		8	10914.57				
		Top Cable Vert		21	9153.57				
		Top Cable Norm		21	5944.75				
		Top Cable Tan		21	0.15				
		Bot Cable Vert		8	-7982.17				
		Bot Cable Norm		8	6254.74				
		Bot Cable Tan		8	0.19				
		Guy B		Bottom Tension	25	10397.93			
				Top Tension	12	10877.36			
			Top Cable Vert	25	9123.97				
			Top Cable Norm	25	5922.01				
			Top Cable Tan	25	0.05				
			Bot Cable Vert	12	-8186.67				
			Bot Cable Norm	12	6410.57				
			Bot Cable Tan	12	0.59				
		Guy C	Bottom Tension	17	10375.04				
			Top Tension	4	10891.89				
			Top Cable Vert	17	9135.52				
			Top Cable Norm	17	5930.88				
			Top Cable Tan	17	0.20				
			Bot Cable Vert	4	-8168.46				
			Bot Cable Norm	4	6396.69				
			Bot Cable Tan	4	0.82				
		Top Guy Pull-Off	Max Tension	23	709.77	0.00	0.00		
Max. Compression	12		-2584.15	0.00	0.00				
Max. Mx	14		-75.52	-23.29	0.00				
Max. My	11		-933.60	0.00	0.00				
Max. Vy	14		26.62	0.00	0.00				
Max. Vx	11		-0.00	0.00	0.00				
T2	180 - 160		Leg	Max Tension	12	1208.31	32.09	12.63	
				Max. Compression	19	-38535.32	-155.60	-95.92	
				Max. Mx	5	-5923.40	-582.83	-31.55	
				Max. My	2	-15058.39	-9.38	573.36	
		Max. Vy		5	-1204.02	-582.83	-31.55		
		Max. Vx		8	-1265.95	47.37	-558.20		
		Diagonal	Horizontal	Max Tension	13	5665.02	0.00	0.00	
				Max Tension	19	667.45	0.00	0.00	
			Max. Compression	8	-5279.97	0.00	0.00		
			Max. Mx	14	613.88	-23.03	0.00		
			Max. My	3	572.20	0.00	0.00		
			Max. Vy	14	26.33	0.00	0.00		

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 35 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
			Max. Vx	3	-0.00	0.00	0.00
		Top Girt	Max Tension	1	0.00	0.00	0.00
			Max. Compression	12	-2970.82	0.00	0.00
			Max. Mx	14	-2445.87	-23.03	0.00
			Max. My	11	-2732.58	0.00	0.00
			Max. Vy	14	26.33	0.00	0.00
			Max. Vx	11	-0.00	0.00	0.00
		Guy A	Bottom Tension	8	11791.71		
			Top Tension	8	11921.38		
			Top Cable Vert	8	8991.63		
			Top Cable Norm	8	7827.52		
			Top Cable Tan	8	3.04		
			Bot Cable Vert	8	-8680.72		
			Bot Cable Norm	8	7980.56		
			Bot Cable Tan	8	3.68		
		Guy B	Bottom Tension	12	11900.49		
			Top Tension	12	12030.17		
			Top Cable Vert	12	9072.62		
			Top Cable Norm	12	7900.16		
			Top Cable Tan	12	2.60		
			Bot Cable Vert	12	-8761.71		
			Bot Cable Norm	12	8053.20		
			Bot Cable Tan	12	4.12		
		Guy C	Bottom Tension	4	11872.45		
			Top Tension	4	12002.12		
			Top Cable Vert	4	9051.64		
			Top Cable Norm	4	7881.55		
			Top Cable Tan	4	3.30		
			Bot Cable Vert	4	-8740.73		
			Bot Cable Norm	4	8034.59		
			Bot Cable Tan	4	3.42		
		Top Guy Pull-Off	Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-7591.02	0.00	0.00
			Max. Mx	14	-3695.65	-23.03	0.00
			Max. My	9	-4568.95	0.00	0.00
			Max. Vy	14	26.33	0.00	0.00
			Max. Vx	9	-0.00	0.00	0.00
		Bottom Guy Pull-Off	Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-3731.17	0.00	0.00
			Max. Mx	14	-1571.32	-23.03	0.00
			Max. My	9	-2188.64	0.00	0.00
			Max. Vy	14	26.33	0.00	0.00
			Max. Vx	9	-0.00	0.00	0.00
		Torque Arm Top	Max Tension	19	12773.12	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	25	11838.16	-40.77	0.00
			Max. My	22	11718.65	0.00	0.04
			Max. Vy	25	34.34	0.00	0.00
			Max. Vx	22	-0.03	0.00	0.00
		Torque Arm Bottom	Max Tension	10	2205.56	0.00	0.00
			Max. Compression	4	-8435.37	0.00	0.00
			Max. Mx	25	-3713.96	-38.09	0.00
			Max. My	3	-4557.67	0.00	-0.00
			Max. Vy	25	43.53	0.00	0.00
			Max. Vx	3	0.00	0.00	0.00
		Leg	Max Tension	1	0.00	0.00	0.00
T3	160 - 140		Max. Compression	16	-40743.51	-69.78	69.22
			Max. Mx	5	-30774.12	362.57	-30.76
			Max. My	8	-24417.86	-7.35	390.03
			Max. Vy	5	-1201.72	-132.17	9.49



<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 36 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
			Max. Vx	8	-1265.58	20.14	-84.04
		Diagonal	Max Tension	4	4863.25	0.00	0.00
		Horizontal	Max Tension	16	705.70	0.00	0.00
			Max. Compression	6	-4541.07	0.00	0.00
			Max. Mx	14	663.98	-22.66	0.00
			Max. My	9	556.11	0.00	0.00
			Max. Vy	14	-25.90	0.00	0.00
		Top Girt	Max. Vx	9	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-3195.08	0.00	0.00
			Max. Mx	14	-1599.37	-22.66	0.00
			Max. My	9	-2200.73	0.00	0.00
			Max. Vy	14	-25.90	0.00	0.00
		Bottom Girt	Max. Vx	9	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-2385.74	0.00	0.00
			Max. Mx	14	-1501.10	-22.66	0.00
			Max. My	9	-2166.64	0.00	0.00
			Max. Vy	14	-25.90	0.00	0.00
		Leg	Max. Vx	9	-0.00	0.00	0.00
T4	140 - 120		Max Tension	1	0.00	0.00	0.00
			Max. Compression	23	-59744.19	1.01	-6.28
			Max. Mx	10	-22190.85	752.54	-591.54
			Max. My	2	-19287.18	-127.37	920.07
			Max. Vy	10	1577.43	752.17	-592.27
			Max. Vx	2	1974.30	-127.37	920.07
		Diagonal	Max Tension	3	4683.87	0.00	0.00
		Horizontal	Max Tension	23	1034.80	0.00	0.00
			Max. Compression	8	-4460.17	0.00	0.00
			Max. Mx	14	958.01	-22.24	0.00
			Max. My	9	749.61	0.00	0.00
			Max. Vy	14	25.42	0.00	0.00
		Top Girt	Max. Vx	9	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-2308.95	0.00	0.00
			Max. Mx	14	-1414.74	-22.24	0.00
			Max. My	9	-2154.20	0.00	0.00
			Max. Vy	14	25.42	0.00	0.00
			Max. Vx	9	-0.00	0.00	0.00
		Guy A	Bottom Tension	21	9926.67		
			Top Tension	8	10231.44		
			Top Cable Vert	21	7047.93		
			Top Cable Norm	21	7416.80		
			Top Cable Tan	21	2.90		
			Bot Cable Vert	8	-6302.94		
			Bot Cable Norm	8	7668.88		
			Bot Cable Tan	8	2.16		
		Guy B	Bottom Tension	25	9757.59		
			Top Tension	12	10148.17		
			Top Cable Vert	25	6994.54		
			Top Cable Norm	25	7352.66		
			Top Cable Tan	25	2.67		
			Bot Cable Vert	12	-6194.52		
			Bot Cable Norm	12	7539.12		
			Bot Cable Tan	12	2.36		
		Guy C	Bottom Tension	17	9908.18		
			Top Tension	4	10237.74		
			Top Cable Vert	17	7051.97		
			Top Cable Norm	17	7421.65		
			Top Cable Tan	17	2.83		
			Bot Cable Vert	4	-6291.08		

<b>RISATower</b>  Phone: FAX:	<b>Job</b>	117-23150	<b>Page</b>	37 of 56
	<b>Project</b>	Ashford, CT	<b>Date</b>	00:05:19 10/19/17
	<b>Client</b>	CDT	<b>Designed by</b>	FAN

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft	
T5	120 - 100	Top Guy Pull-Off	Bot Cable Norm	4	7654.70			
			Bot Cable Tan	4	2.20			
			Max Tension	1	0.00	0.00	0.00	
			Max. Compression	2	-6201.84	0.00	0.00	
			Max. Mx	14	-1849.15	-22.24	0.00	
			Max. My	9	-3752.07	0.00	0.00	
			Max. Vy	14	25.42	0.00	0.00	
			Max. Vx	9	-0.00	0.00	0.00	
			Max Tension	4	115.10	0.00	0.00	
			Bottom Guy Pull-Off	Max. Compression	6	-3900.01	0.00	0.00
				Max. Mx	14	-680.99	-22.24	0.00
				Max. My	9	-1820.28	0.00	0.00
				Max. Vy	14	25.42	0.00	0.00
				Max. Vx	9	-0.00	0.00	0.00
		Torque Arm Top		Max Tension	19	9834.62	0.00	0.00
				Max. Compression	1	0.00	0.00	0.00
				Max. Mx	21	9250.41	-39.58	0.00
				Max. My	16	8809.62	0.00	-0.04
				Max. Vy	21	-33.34	0.00	0.00
			Max. Vx	16	0.04	0.00	0.00	
		Torque Arm Bottom	Max Tension	10	3429.47	0.00	0.00	
			Max. Compression	7	-6553.19	0.00	0.00	
			Max. Mx	21	-1804.76	-37.05	0.00	
			Max. My	22	-1874.30	0.00	0.00	
			Max. Vy	21	42.35	0.00	0.00	
			Max. Vx	22	-0.00	0.00	0.00	
			Leg	Max Tension	1	0.00	0.00	0.00
				Max. Compression	16	-59647.20	23.40	-11.02
				Max. Mx	4	-35072.29	466.04	-352.55
				Max. My	8	-34520.79	-66.69	567.81
		Max. Vy		10	1577.55	160.62	-102.24	
		Max. Vx		2	1973.92	-6.22	180.67	
		Diagonal Horizontal		Max Tension	10	5230.73	0.00	0.00
				Max Tension	16	1033.12	0.00	0.00
				Max. Compression	6	-3727.72	0.00	0.00
				Max. Mx	14	1002.72	-21.77	0.00
				Max. My	22	1021.64	0.00	0.00
				Max. Vy	14	24.88	0.00	0.00
		Top Girt		Max. Vx	22	-0.00	0.00	0.00
				Max Tension	1	0.00	0.00	0.00
Max. Compression	6			-3101.52	0.00	0.00		
Max. Mx	14			-683.74	-21.77	0.00		
Max. My	9		-1813.27	0.00	0.00			
Max. Vy	14		24.88	0.00	0.00			
Bottom Girt	Max. Vx	9	-0.00	0.00	0.00			
	Max Tension	1	0.00	0.00	0.00			
	Max. Compression	8	-2165.68	0.00	0.00			
	Max. Mx	14	-620.71	-21.77	0.00			
	Max. My	16	-482.77	0.00	-0.00			
	Max. Vy	14	24.88	0.00	0.00			
	Max. Vx	16	0.00	0.00	0.00			
	Max Tension	1	0.00	0.00	0.00			
T6	100 - 80	Leg	Max Tension	1	0.00	0.00	0.00	
			Max. Compression	25	-63051.02	-68.96	-36.97	
			Max. Mx	10	-25492.25	-332.59	22.35	
			Max. My	9	-27634.88	-163.14	336.25	
			Max. Vy	10	810.35	-28.84	19.29	
			Max. Vx	2	894.15	15.52	-0.19	
		Diagonal Horizontal	Max Tension	10	3890.24	0.00	0.00	
			Max Tension	25	1092.08	0.00	0.00	
			Max. Compression	8	-3819.09	0.00	0.00	

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 38 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
			Max. Mx	14	1045.90	-21.22	0.00
			Max. My	3	658.15	0.00	0.00
			Max. Vy	14	24.25	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
		Top Girt	Max Tension	1	0.00	0.00	0.00
			Max. Compression	6	-1947.50	0.00	0.00
			Max. Mx	14	-580.08	-21.22	0.00
			Max. My	16	-569.25	0.00	-0.00
			Max. Vy	14	24.25	0.00	0.00
			Max. Vx	16	0.00	0.00	0.00
		Bottom Girt	Max Tension	1	0.00	0.00	0.00
			Max. Compression	2	-1862.27	0.00	0.00
			Max. Mx	14	-510.25	-21.22	0.00
			Max. My	3	-1800.14	0.00	0.00
			Max. Vy	14	24.25	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
T7	80 - 60	Leg	Max Tension	1	0.00	0.00	0.00
			Max. Compression	19	-67347.70	-10.44	-7.03
			Max. Mx	4	-25793.47	-781.28	317.02
			Max. My	8	-25690.44	-107.48	-836.27
			Max. Vy	4	-1748.06	-781.28	317.02
			Max. Vx	8	-1853.52	-107.48	-836.27
		Diagonal	Max Tension	3	4296.66	0.00	0.00
		Horizontal	Max Tension	19	1166.50	0.00	0.00
			Max. Compression	2	-3855.94	0.00	0.00
			Max. Mx	14	1142.19	-20.55	0.00
			Max. My	3	638.74	0.00	0.00
			Max. Vy	14	-23.49	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
		Top Girt	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-2114.89	0.00	0.00
			Max. Mx	14	-470.40	-20.55	0.00
			Max. My	3	-1797.86	0.00	0.00
			Max. Vy	14	-23.49	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
		Guy A	Bottom Tension	8	10531.51		
			Top Tension	8	10571.87		
			Top Cable Vert	8	4161.61		
			Top Cable Norm	8	9718.31		
			Top Cable Tan	8	1.29		
			Bot Cable Vert	8	-4032.37		
			Bot Cable Norm	8	9728.96		
			Bot Cable Tan	8	1.29		
		Guy B	Bottom Tension	12	10499.66		
			Top Tension	12	10540.02		
			Top Cable Vert	12	4149.27		
			Top Cable Norm	12	9688.94		
			Top Cable Tan	12	0.22		
			Bot Cable Vert	12	-4020.03		
			Bot Cable Norm	12	9699.60		
			Bot Cable Tan	12	0.22		
		Guy C	Bottom Tension	4	10507.90		
			Top Tension	4	10548.26		
			Top Cable Vert	4	4152.46		
			Top Cable Norm	4	9696.54		
			Top Cable Tan	4	1.32		
			Bot Cable Vert	4	-4023.22		
			Bot Cable Norm	4	9707.20		
			Bot Cable Tan	4	1.32		
		Top Guy Pull-Off	Max Tension	19	4010.79	0.00	0.00
			Max. Compression	8	-824.54	0.00	0.00

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 39 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft	
T8	60 - 40	Leg	Max. Mx	14	3289.85	-20.55	0.00	
			Max. My	3	648.64	0.00	0.00	
			Max. Vy	14	-23.49	0.00	0.00	
			Max. Vx	3	-0.00	0.00	0.00	
			Max Tension	1	0.00	0.00	0.00	
			Max. Compression	25	-72782.38	-11.23	-6.26	
			Max. Mx	4	-25794.05	528.93	-191.38	
			Max. My	8	-25691.23	90.62	553.68	
			Max. Vy	4	-1748.97	-126.41	63.26	
			Max. Vx	8	-1855.54	-8.19	-141.72	
			Max Tension	4	4185.45	0.00	0.00	
			Max. Compression	25	1260.63	0.00	0.00	
		Diagonal	Horizontal	Max. Compression	12	-3523.24	0.00	0.00
				Max. Mx	14	1186.68	-19.70	0.00
				Max. My	3	736.25	0.00	0.00
				Max. Vy	14	22.52	0.00	0.00
				Max. Vx	3	-0.00	0.00	0.00
				Max Tension	17	222.82	0.00	0.00
		Top Girt		Max. Compression	6	-1828.84	0.00	0.00
				Max. Mx	14	89.23	-19.70	0.00
				Max. My	3	-1511.00	0.00	0.00
				Max. Vy	14	22.52	0.00	0.00
				Max. Vx	3	-0.00	0.00	0.00
				Max Tension	19	90.11	0.00	0.00
		Bottom Girt		Max. Compression	8	-2090.64	0.00	0.00
				Max. Mx	14	-144.89	-19.70	0.00
				Max. My	3	-1656.49	0.00	0.00
				Max. Vy	14	22.52	0.00	0.00
Max. Vx	3			-0.00	0.00	0.00		
Max Tension	19			90.11	0.00	0.00		
T9	40 - 20	Leg	Max. Compression	8	-2090.64	0.00	0.00	
			Max. Mx	14	-144.89	-19.70	0.00	
			Max. My	3	-1656.49	0.00	0.00	
			Max. Vy	14	22.52	0.00	0.00	
			Max. Vx	3	-0.00	0.00	0.00	
			Max Tension	1	0.00	0.00	0.00	
			Max. Compression	25	-75839.36	-81.49	-45.59	
			Max. Mx	5	-22388.01	235.72	10.94	
			Max. My	8	-29342.36	19.02	242.98	
			Max. Vy	4	-532.02	11.12	-7.44	
			Max. Vx	8	-553.43	20.43	36.01	
			Max Tension	10	3145.10	0.00	0.00	
		Diagonal	Horizontal	Max Tension	25	1313.58	0.00	0.00
				Max. Compression	12	-3719.28	0.00	0.00
				Max. Mx	14	1223.85	-18.50	0.00
				Max. My	3	787.23	0.00	0.00
				Max. Vy	14	21.14	0.00	0.00
				Max. Vx	3	-0.00	0.00	0.00
		Top Girt		Max Tension	1	0.00	0.00	0.00
				Max. Compression	2	-1758.12	0.00	0.00
				Max. Mx	14	-104.60	-18.50	0.00
				Max. My	3	-1684.03	0.00	0.00
				Max. Vy	14	21.14	0.00	0.00
				Max. Vx	3	-0.00	0.00	0.00
		Bottom Girt		Max Tension	1	0.00	0.00	0.00
				Max. Compression	8	-1822.08	0.00	0.00
				Max. Mx	14	-73.02	-18.50	0.00
				Max. My	3	-1646.25	0.00	0.00
Max. Vy	14			21.14	0.00	0.00		
Max. Vx	3			-0.00	0.00	0.00		
T10	20 - 0	Leg	Max Tension	1	0.00	0.00	0.00	
			Max. Compression	25	-75904.01	-31.18	-17.60	
			Max. Mx	11	-32740.55	1199.65	545.51	
			Max. My	8	-32943.69	-19.47	-1362.12	
			Max. Vy	11	-3234.57	1199.65	545.51	
			Max. Vx	8	3699.89	-19.47	-1362.12	
		Diagonal		Max Tension	3	3545.40	0.00	0.00

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 40 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
		Horizontal	Max Tension	25	1314.70	0.00	0.00
			Max. Compression	8	-3577.58	0.00	0.00
			Max. Mx	14	1258.04	-16.23	0.00
			Max. My	3	771.42	0.00	0.00
			Max. Vy	14	18.55	0.00	0.00
		Top Girt	Max. Vx	3	-0.00	0.00	0.00
			Max Tension	22	66.25	0.00	0.00
			Max. Compression	12	-2040.03	0.00	0.00
			Max. Mx	14	-40.73	-16.23	0.00
			Max. My	3	-1650.44	0.00	0.00
		Bottom Girt	Max. Vy	14	18.55	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
			Max Tension	25	1064.58	0.00	0.00
			Max. Compression	8	-353.18	0.00	0.00
			Max. Mx	14	1012.70	-16.23	0.00
		Base Beam	Max. My	3	-332.46	0.00	0.00
			Max. Vy	14	18.55	0.00	0.00
			Max. Vx	3	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	12	-3711.25	1372.00	-0.08
			Max. Mx	19	-73346.72	-147317.74	-457.71
			Max. My	11	-32426.86	-64591.94	-1230.06
			Max. Vy	15	-73360.43	-147284.81	-556.17
			Max. Vx	11	-600.21	-64591.94	-1230.06

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical lb	Horizontal, X lb	Horizontal, Z lb
Guy C @ 145 ft Elev 0 ft Azimuth 240 deg	Max. Vert	10	-5011.90	-4221.69	2444.27
	Max. H <sub>x</sub>	10	-5011.90	-4221.69	2444.27
	Max. H <sub>z</sub>	4	-42003.76	-40867.17	23582.35
	Min. Vert	4	-42003.76	-40867.17	23582.35
	Min. H <sub>x</sub>	4	-42003.76	-40867.17	23582.35
Guy B @ 145 ft Elev 0 ft Azimuth 120 deg	Min. H <sub>z</sub>	10	-5011.90	-4221.69	2444.27
	Max. Vert	6	-4958.91	4181.00	2411.67
	Max. H <sub>x</sub>	12	-41999.22	40844.88	23584.46
	Max. H <sub>z</sub>	12	-41999.22	40844.88	23584.46
	Min. Vert	12	-41999.22	40844.88	23584.46
Guy A @ 145 ft Elev 0 ft Azimuth 0 deg	Min. H <sub>x</sub>	6	-4958.91	4181.00	2411.67
	Min. H <sub>z</sub>	6	-4958.91	4181.00	2411.67
	Max. Vert	2	-5228.49	6.55	-5039.28
	Max. H <sub>x</sub>	24	-29024.92	1203.43	-37011.83
	Max. H <sub>z</sub>	2	-5228.49	6.55	-5039.28
Mast	Min. Vert	8	-41676.38	-10.75	-46934.75
	Min. H <sub>x</sub>	18	-29043.73	-1205.01	-37029.49
	Min. H <sub>z</sub>	8	-41676.38	-10.75	-46934.75
	Max. Vert	19	219823.91	-176.97	-103.74
	Max. H <sub>x</sub>	11	97322.56	2149.55	27.24
	Max. H <sub>z</sub>	2	95958.58	27.98	2233.64

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 41 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Location	Condition	Gov. Load Comb.	Vertical lb	Horizontal, X lb	Horizontal, Z lb
	Max. M <sub>x</sub>	1	0.00	25.45	14.08
	Max. M <sub>z</sub>	1	0.00	25.45	14.08
	Max. Torsion	1	0.00	25.45	14.08
	Min. Vert	1	89118.13	25.45	14.08
	Min. H <sub>x</sub>	5	97345.50	-2087.81	26.44
	Min. H <sub>z</sub>	8	97790.54	27.55	-2132.99
	Min. M <sub>x</sub>	1	0.00	25.45	14.08
	Min. M <sub>z</sub>	1	0.00	25.45	14.08
	Min. Torsion	1	0.00	25.45	14.08

### Tower Mast Reaction Summary

Load Combination	Vertical lb	Shear <sub>x</sub> lb	Shear <sub>z</sub> lb	Overturning Moment, M <sub>x</sub> lb-ft	Overturning Moment, M <sub>z</sub> lb-ft	Torque lb-ft
Dead Only	89118.13	-25.45	-14.08	0.00	0.00	0.00
1.2 Dead+1.6 Wind 0 deg - No Ice+1.0 Guy	95958.58	-27.98	-2233.64	0.00	0.00	0.00
1.2 Dead+1.6 Wind 30 deg - No Ice+1.0 Guy	97140.45	1043.87	-1831.34	0.00	0.00	0.00
1.2 Dead+1.6 Wind 60 deg - No Ice+1.0 Guy	97893.38	1839.46	-1086.70	0.00	0.00	0.00
1.2 Dead+1.6 Wind 90 deg - No Ice+1.0 Guy	97345.50	2087.81	-26.44	0.00	0.00	0.00
1.2 Dead+1.6 Wind 120 deg - No Ice+1.0 Guy	96173.90	1875.53	1079.68	0.00	0.00	0.00
1.2 Dead+1.6 Wind 150 deg - No Ice+1.0 Guy	97213.92	1018.42	1816.28	0.00	0.00	0.00
1.2 Dead+1.6 Wind 180 deg - No Ice+1.0 Guy	97790.54	-27.55	2132.99	0.00	0.00	0.00
1.2 Dead+1.6 Wind 210 deg - No Ice+1.0 Guy	97201.37	-1073.88	1818.33	0.00	0.00	0.00
1.2 Dead+1.6 Wind 240 deg - No Ice+1.0 Guy	96148.00	-1933.35	1081.85	0.00	0.00	0.00
1.2 Dead+1.6 Wind 270 deg - No Ice+1.0 Guy	97322.56	-2149.55	-27.24	0.00	0.00	0.00
1.2 Dead+1.6 Wind 300 deg - No Ice+1.0 Guy	97886.28	-1900.01	-1090.51	0.00	0.00	0.00
1.2 Dead+1.6 Wind 330 deg - No Ice+1.0 Guy	97128.29	-1101.33	-1835.35	0.00	0.00	0.00
1.2 Dead+1.0 Ice+1.0 Temp+Guy	217822.27	-152.68	-84.55	0.00	0.00	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp+1.0 Guy	219794.94	-149.56	-461.75	0.00	0.00	0.00
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp+1.0 Guy	219169.41	44.25	-413.55	0.00	0.00	0.00
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp+1.0 Guy	218606.19	192.77	-280.86	0.00	0.00	0.00
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp+1.0 Guy	219181.29	232.08	-85.59	0.00	0.00	0.00
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp+1.0 Guy	219823.91	176.97	103.74	0.00	0.00	0.00
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp+1.0 Guy	219183.73	38.36	249.76	0.00	0.00	0.00
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp+1.0 Guy	218608.47	-150.93	313.55	0.00	0.00	0.00
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp+1.0 Guy	219168.77	-340.50	251.61	0.00	0.00	0.00

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 42 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Load Combination	Vertical lb	Shear <sub>x</sub> lb	Shear <sub>y</sub> lb	Overturning Moment, M <sub>x</sub> lb-ft	Overturning Moment, M <sub>y</sub> lb-ft	Torque lb-ft
deg+1.0 Ice+1.0 Temp+1.0 Guy						
1.2 Dead+1.0 Wind 240	219791.36	-479.38	108.18	0.00	0.00	0.00
deg+1.0 Ice+1.0 Temp+1.0 Guy						
1.2 Dead+1.0 Wind 270	219146.32	-537.86	-85.35	0.00	0.00	0.00
deg+1.0 Ice+1.0 Temp+1.0 Guy						
1.2 Dead+1.0 Wind 300	218575.37	-497.66	-282.67	0.00	0.00	0.00
deg+1.0 Ice+1.0 Temp+1.0 Guy						
1.2 Dead+1.0 Wind 330	219149.21	-346.67	-415.66	0.00	0.00	0.00
deg+1.0 Ice+1.0 Temp+1.0 Guy						
Dead+Wind 0 deg -	89291.30	-25.63	-515.34	0.00	0.00	0.00
Service+Guy						
Dead+Wind 30 deg -	89250.09	216.96	-433.55	0.00	0.00	0.00
Service+Guy						
Dead+Wind 60 deg -	89210.66	401.99	-260.06	0.00	0.00	0.00
Service+Guy						
Dead+Wind 90 deg -	89248.10	460.17	-13.63	0.00	0.00	0.00
Service+Guy						
Dead+Wind 120 deg -	89289.16	409.84	236.55	0.00	0.00	0.00
Service+Guy						
Dead+Wind 150 deg -	89248.59	217.74	405.07	0.00	0.00	0.00
Service+Guy						
Dead+Wind 180 deg -	89210.81	-25.42	478.08	0.00	0.00	0.00
Service+Guy						
Dead+Wind 210 deg -	89249.17	-268.63	405.02	0.00	0.00	0.00
Service+Guy						
Dead+Wind 240 deg -	89289.69	-460.91	236.46	0.00	0.00	0.00
Service+Guy						
Dead+Wind 270 deg -	89248.29	-511.38	-13.81	0.00	0.00	0.00
Service+Guy						
Dead+Wind 300 deg -	89210.41	-453.26	-260.31	0.00	0.00	0.00
Service+Guy						
Dead+Wind 330 deg -	89249.70	-268.22	-433.75	0.00	0.00	0.00
Service+Guy						

## Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX lb	PY lb	PZ lb	PX lb	PY lb	PZ lb	
1	0.00	-22331.48	0.00	0.00	22331.48	0.00	0.000%
2	-19.84	-26486.15	-34838.60	18.88	26484.04	34736.66	0.233%
3	17338.50	-26280.33	-29642.46	-17360.39	26279.00	29556.05	0.206%
4	30226.78	-26074.50	-17204.11	-30223.84	26074.55	17200.91	0.010%
5	34711.37	-26280.33	19.84	-34648.55	26278.84	38.20	0.196%
6	30589.58	-26486.15	17436.48	-30456.93	26482.85	-17359.66	0.348%
7	17372.86	-26280.33	29662.30	-17290.36	26278.91	-29637.13	0.199%
8	19.84	-26074.50	34442.58	-20.23	26074.57	-34438.86	0.009%
9	-17338.50	-26280.33	29642.46	17255.07	26278.94	-29618.13	0.201%
10	-30569.74	-26486.15	17402.12	30435.97	26482.86	-17326.39	0.349%
11	-34711.37	-26280.33	-19.84	34649.14	26278.87	77.48	0.195%
12	-30246.62	-26074.50	-17238.47	30243.39	26074.55	17235.73	0.010%
13	-17372.86	-26280.33	-29662.30	17393.42	26279.00	29577.03	0.203%
14	0.00	-132197.64	0.00	1.28	132197.64	0.69	0.001%
15	-4.37	-132465.44	-15162.91	4.32	132465.02	15115.64	0.035%
16	7526.27	-132197.64	-13055.86	-7526.18	132197.28	13006.82	0.037%
17	13088.38	-131929.84	-7563.07	-13060.04	131929.54	7547.51	0.024%
18	15060.11	-132197.64	4.37	-15017.02	132197.27	20.62	0.037%
19	13118.04	-132465.44	7585.24	-13076.90	132465.03	-7561.33	0.036%

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 43 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX lb	PY lb	PZ lb	PX lb	PY lb	PZ lb	
20	7533.84	-132197.64	13060.23	-7490.57	132197.27	-13035.38	0.038%
21	4.37	-131929.84	15133.70	-3.28	131929.54	-15101.40	0.024%
22	-7526.27	-132197.64	13055.86	7484.28	132197.28	-13031.38	0.037%
23	-13113.67	-132465.44	7577.67	13073.09	132465.03	-7554.28	0.035%
24	-15060.11	-132197.64	-4.37	15017.99	132197.29	28.65	0.037%
25	-13092.75	-131929.84	-7570.63	13065.61	131929.55	7554.56	0.024%
26	-7533.84	-132197.64	-13060.23	7533.94	132197.28	13011.03	0.037%
27	-4.46	-22377.79	-7838.68	4.46	22377.79	7836.28	0.010%
28	3901.16	-22331.48	-6669.55	-3900.79	22331.48	6667.84	0.007%
29	6801.03	-22285.17	-3870.92	-6800.03	22285.17	3870.32	0.005%
30	7810.06	-22331.48	4.46	-7808.42	22331.48	-3.96	0.007%
31	6882.65	-22377.79	3923.21	-6880.62	22377.79	-3922.02	0.010%
32	3908.89	-22331.48	6674.02	-3907.64	22331.48	-6672.83	0.007%
33	4.46	-22285.17	7749.58	-4.47	22285.17	-7748.40	0.005%
34	-3901.16	-22331.48	6669.55	3899.90	22331.48	-6668.37	0.007%
35	-6878.19	-22377.79	3915.48	6876.15	22377.79	-3914.29	0.010%
36	-7810.06	-22331.48	-4.46	7808.42	22331.48	4.97	0.007%
37	-6805.49	-22285.17	-3878.66	6804.50	22285.17	3878.06	0.005%
38	-3908.89	-22331.48	-6674.02	3908.52	22331.48	6672.31	0.007%

## Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	13	0.00000001	0.00000001
2	Yes	14	0.00100347	0.00060622
3	Yes	13	0.00104326	0.00040466
4	Yes	13	0.00030798	0.00017789
5	Yes	13	0.00110861	0.00039250
6	Yes	13	0.00145799	0.00087028
7	Yes	13	0.00107583	0.00038851
8	Yes	13	0.00030673	0.00017753
9	Yes	13	0.00106624	0.00039718
10	Yes	13	0.00146495	0.00087896
11	Yes	13	0.00109224	0.00038997
12	Yes	13	0.00030299	0.00015836
13	Yes	13	0.00103397	0.00039456
14	Yes	13	0.00000001	0.00000315
15	Yes	16	0.00111934	0.00016938
16	Yes	15	0.00121930	0.00017474
17	Yes	14	0.00119991	0.00014426
18	Yes	15	0.00122943	0.00016882
19	Yes	16	0.00111890	0.00016490
20	Yes	15	0.00123040	0.00016882
21	Yes	14	0.00119626	0.00014328
22	Yes	15	0.00120439	0.00017285
23	Yes	16	0.00110650	0.00016737
24	Yes	15	0.00121012	0.00017136
25	Yes	14	0.00119541	0.00014771
26	Yes	15	0.00122662	0.00017386
27	Yes	13	0.00000001	0.00003160
28	Yes	13	0.00000001	0.00002791
29	Yes	13	0.00000001	0.00002563
30	Yes	13	0.00000001	0.00002601
31	Yes	13	0.00000001	0.00003022
32	Yes	13	0.00000001	0.00002663



<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 44 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

33	Yes	13	0.00000001	0.00002603
34	Yes	13	0.00000001	0.00002780
35	Yes	13	0.00000001	0.00003110
36	Yes	13	0.00000001	0.00002699
37	Yes	13	0.00000001	0.00002477
38	Yes	13	0.00000001	0.00002682

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	190 - 180	1.145	30	0.0451	0.0230
T2	180 - 160	1.044	37	0.0490	0.0157
T3	160 - 140	0.850	29	0.0339	0.0157
T4	140 - 120	0.766	29	0.0226	0.0324
T5	120 - 100	0.685	33	0.0063	0.0460
T6	100 - 80	0.713	27	0.0026	0.0943
T7	80 - 60	0.682	27	0.0190	0.1319
T8	60 - 40	0.577	27	0.0208	0.1590
T9	40 - 20	0.502	27	0.0299	0.1802
T10	20 - 0	0.315	27	0.0613	0.1925

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
190.00	Sector Frame Mount	30	1.145	0.0451	0.0230	133823
189.63	Guy	30	1.141	0.0453	0.0227	133823
178.00	Sector Frame Mount	37	1.023	0.0486	0.0144	114030
170.50	Sector Frame Mount	29	0.942	0.0438	0.0123	71776
160.38	Guy	29	0.853	0.0342	0.0155	25984
150.00	Sector Frame Mount	29	0.802	0.0279	0.0241	85486
138.00	(2) Allgon 7770.00	29	0.758	0.0211	0.0335	66025
120.38	Guy	33	0.685	0.0066	0.0454	24968
60.38	Guy	27	0.579	0.0208	0.1586	53008

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	190 - 180	6.037	5	0.2727	0.1157
T2	180 - 160	5.434	5	0.2854	0.0821
T3	160 - 140	4.286	12	0.2039	0.1028
T4	140 - 120	3.765	4	0.1378	0.1802
T5	120 - 100	3.306	8	0.0482	0.2426
T6	100 - 80	3.357	8	0.0311	0.4560
T7	80 - 60	3.252	2	0.0945	0.6214
T8	60 - 40	2.856	2	0.1002	0.7400

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 45 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T9	40 - 20	2.493	2	0.1493	0.8342
T10	20 - 0	1.551	2	0.3037	0.8893

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
190.00	Sector Frame Mount	5	6.037	0.2727	0.1157	32675
189.63	Guy	5	6.015	0.2736	0.1143	32675
178.00	Sector Frame Mount	5	5.309	0.2826	0.0835	30253
170.50	Sector Frame Mount	5	4.836	0.2563	0.0872	13223
160.38	Guy	12	4.301	0.2056	0.1018	5205
150.00	Sector Frame Mount	4	3.992	0.1689	0.1414	13770
138.00	(2) Allgon 7770.00	4	3.713	0.1303	0.1853	16926
120.38	Guy	8	3.310	0.0498	0.2401	4788
60.38	Guy	2	2.863	0.1003	0.7379	11762

### Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt lb	Allowable Load lb	Ratio Load Allowable	Allowable Ratio	Criteria
T1	190	Leg	A325N	0.7500	4	351.24	29820.60	0.012	✓	1 Bolt Tension
T2	180	Leg	A325N	0.7500	4	1173.51	29820.60	0.039	✓	1 Bolt Tension
		Torque Arm Top@160.375	A325N	0.7500	2	6386.56	17892.40	0.357	✓	1 Bolt Shear
		Torque Arm Bottom@160.375	A325N	0.7500	2	4217.68	17892.40	0.236	✓	1 Bolt Shear
T3	160	Leg	A325N	0.7500	4	2970.77	29820.60	0.100	✓	1 Bolt Tension
T4	140	Leg	A325N	0.7500	4	3203.41	29820.60	0.107	✓	1 Bolt Tension
		Torque Arm Top@120.375	A325N	0.7500	2	4917.31	17892.40	0.275	✓	1 Bolt Shear
		Torque Arm Bottom@120.375	A325N	0.7500	2	3276.59	17892.40	0.183	✓	1 Bolt Shear
T5	120	Leg	A325N	0.7500	4	4898.11	29820.60	0.164	✓	1 Bolt Tension
T6	100	Leg	A325N	0.7500	4	4926.86	29820.60	0.165	✓	1 Bolt Tension
T7	80	Leg	A325N	0.7500	4	5183.77	29820.60	0.174	✓	1 Bolt Tension
T8	60	Leg	A325N	0.7500	4	5614.73	29820.60	0.188	✓	1 Bolt Tension
T9	40	Leg	A325N	0.7500	4	6067.53	29820.60	0.203	✓	1 Bolt Tension
T10	20	Leg	A325N	0.7500	4	6293.71	29820.60	0.211	✓	1 Bolt Tension

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 46 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

### Guy Design Data

Section No.	Elevation ft	Size	Initial Tension lb	Breaking Load lb	Actual $T_u$ lb	Allowable $\phi T_n$ lb	Required S.F.	Actual S.F.
T1	189.63 (A) (615)	9/16 EHS	5250.00	35000.04	10914.60	21000.00	1.000	1.924 ✓
	189.63 (B) (614)	9/16 EHS	5250.00	35000.04	10877.40	21000.00	1.000	1.931 ✓
	189.63 (C) (613)	9/16 EHS	5250.00	35000.04	10891.90	21000.00	1.000	1.928 ✓
T2	160.38 (A) (586)	5/8 EHS	6360.00	42399.99	11921.40	25440.00	1.000	2.134 ✓
	160.38 (A) (587)	5/8 EHS	6360.00	42399.99	11863.80	25440.00	1.000	2.144 ✓
	160.38 (B) (580)	5/8 EHS	6360.00	42399.99	11929.20	25440.00	1.000	2.133 ✓
	160.38 (B) (581)	5/8 EHS	6360.00	42399.99	12030.20	25440.00	1.000	2.115 ✓
	160.38 (C) (574)	5/8 EHS	6360.00	42399.99	11952.30	25440.00	1.000	2.128 ✓
	160.38 (C) (575)	5/8 EHS	6360.00	42399.99	12002.10	25440.00	1.000	2.120 ✓
T4	120.38 (A) (604)	9/16 EHS	5250.00	35000.04	10231.40	21000.00	1.000	2.052 ✓
	120.38 (A) (605)	9/16 EHS	5250.00	35000.04	10179.00	21000.00	1.000	2.063 ✓
	120.38 (B) (598)	9/16 EHS	5250.00	35000.04	10134.90	21000.00	1.000	2.072 ✓
	120.38 (B) (599)	9/16 EHS	5250.00	35000.04	10148.20	21000.00	1.000	2.069 ✓
	120.38 (C) (592)	9/16 EHS	5250.00	35000.04	10195.90	21000.00	1.000	2.060 ✓
	120.38 (C) (593)	9/16 EHS	5250.00	35000.04	10237.70	21000.00	1.000	2.051 ✓
T7	60.38 (A) (612)	9/16 EHS	5250.00	35000.04	10571.90	21000.00	1.000	1.986 ✓
	60.38 (B) (611)	9/16 EHS	5250.00	35000.04	10540.00	21000.00	1.000	1.992 ✓
	60.38 (C) (610)	9/16 EHS	5250.00	35000.04	10548.30	21000.00	1.000	1.991 ✓

### Compression Checks

### Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	$L_u$ ft	$Kl/r$	A in <sup>2</sup>	Mast Stability Index	$P_u$ lb	$\phi P_n$ lb	Ratio $\frac{P_u}{\phi P_n}$
T1	190 - 180	P2.5x.203	10.00	3.08	39.1 K=1.00	1.7040	1.00	-18579.50	78157.90	0.238 ✓
T2	180 - 160	P2.5x.203	20.00	3.21	40.6 K=1.00	1.7040	1.00	-38535.30	77325.90	0.498 ✓
T3	160 - 140	P2.5x.203	20.00	3.21	40.6	1.7040	1.00	-40743.50	77325.90	0.527 ✓

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 47 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	Mast Stability Index	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T4	140 - 120	P2.5x.203	20.00	3.21	K=1.00 40.6	1.7040	1.00	-59744.20	77325.90	0.773 <sup>1</sup>
T5	120 - 100	P2.5x.203	20.00	3.21	K=1.00 40.6	1.7040	0.98	-59647.20	76068.80	0.784 <sup>1</sup>
T6	100 - 80	P2.5x.203	20.00	3.21	K=1.00 40.6	1.7040	0.98	-63051.00	76085.20	0.829 <sup>1</sup>
T7	80 - 60	P2.5x.203	20.00	3.21	K=1.00 40.6	1.7040	0.98	-67347.70	76057.80	0.885 <sup>1</sup>
T8	60 - 40	P2.5x.203	20.00	3.21	K=1.00 40.6	1.7040	0.98	-72782.40	75967.60	0.958 <sup>1</sup>
T9	40 - 20	P2.5x.203	20.00	3.21	K=1.00 40.6	1.7040	0.98	-75839.40	75978.80	0.998 <sup>1</sup>
T10	20 - 0	P2.5x.203	20.00	3.21	K=1.00 40.6	1.7040	0.98	-75904.00	75972.40	0.999 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T1	190 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-5639.89	11503.00	0.490 <sup>1</sup>
T2	180 - 160	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-5279.97	11503.00	0.459 <sup>1</sup>
T3	160 - 140	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-4541.07	11503.00	0.395 <sup>1</sup>
T4	140 - 120	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-4460.17	11503.00	0.388 <sup>1</sup>
T5	120 - 100	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-3727.72	11503.00	0.324 <sup>1</sup>
T6	100 - 80	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-3819.09	11503.00	0.332 <sup>1</sup>
T7	80 - 60	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-3855.94	11503.00	0.335 <sup>1</sup>
T8	60 - 40	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-3523.24	11503.00	0.306 <sup>1</sup>
T9	40 - 20	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-3719.28	11503.00	0.323 <sup>1</sup>
T10	20 - 0	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-3577.58	11503.00	0.311 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 48 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

**Top Girt Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r K=0.65	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T2	180 - 160	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-2970.82	11503.00	0.258 <sup>1</sup>
T3	160 - 140	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-3195.08	11503.00	0.278 <sup>1</sup>
T4	140 - 120	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-2308.95	11503.00	0.201 <sup>1</sup>
T5	120 - 100	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-3101.52	11503.00	0.270 <sup>1</sup>
T6	100 - 80	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-1947.50	11503.00	0.169 <sup>1</sup>
T7	80 - 60	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-2114.89	11503.00	0.184 <sup>1</sup>
T8	60 - 40	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-1828.84	11503.00	0.159 <sup>1</sup>
T9	40 - 20	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-1758.12	11503.00	0.153 <sup>1</sup>
T10	20 - 0	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-2040.03	11503.00	0.177 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

**Bottom Girt Design Data (Compression)**

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r K=0.65	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T1	190 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-2895.25	11503.00	0.252 <sup>1</sup>
T3	160 - 140	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-2385.74	11503.00	0.207 <sup>1</sup>
T5	120 - 100	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-2165.68	11503.00	0.188 <sup>1</sup>
T6	100 - 80	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-1862.27	11503.00	0.162 <sup>1</sup>
T8	60 - 40	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-2090.64	11503.00	0.182 <sup>1</sup>
T9	40 - 20	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-1822.08	11503.00	0.158 <sup>1</sup>
T10	20 - 0	L1 1/2x1 1/2x3/16	3.50	3.26	86.7	0.5273	-353.18	11503.00	0.031 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

**Top Guy Pull-Off Design Data (Compression)**

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 49 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r K=0.65	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T1	190 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-2584.15	11503.00	0.225 <sup>1</sup> ✓
T2	180 - 160	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-7591.02	11503.00	0.660 <sup>1</sup> ✓
T4	140 - 120	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-6201.84	11503.00	0.539 <sup>1</sup> ✓
T7	80 - 60	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-824.54	11503.00	0.072 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Bottom Guy Pull-Off Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r K=0.65	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T2	180 - 160	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-3731.17	11503.00	0.324 <sup>1</sup> ✓
T4	140 - 120	L1 1/2x1 1/2x3/16	3.50	3.26	86.7 K=0.65	0.5273	-3900.01	11503.00	0.339 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Torque-Arm Bottom Design Data

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r K=1.00	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T2	180 - 160 (578)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-8157.97	36439.50	0.224 <sup>1</sup> ✓
T2	180 - 160 (579)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-8073.65	36439.50	0.222 <sup>1</sup> ✓
T2	180 - 160 (584)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-8435.37	36439.50	0.231 <sup>1</sup> ✓
T2	180 - 160 (585)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-8396.26	36439.50	0.230 <sup>1</sup> ✓
T2	180 - 160 (590)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-8268.30	36439.50	0.227 <sup>1</sup> ✓
T2	180 - 160 (591)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-8253.67	36439.50	0.227 <sup>1</sup> ✓
T4	140 - 120 (596)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-6005.16	36439.50	0.165 <sup>1</sup> ✓
T4	140 - 120 (597)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-6009.83	36439.50	0.165 <sup>1</sup> ✓
T4	140 - 120 (602)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-6532.83	36439.50	0.179 <sup>1</sup> ✓

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 50 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T4	140 - 120 (603)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-6507.04	36439.50	0.179 <sup>1</sup> ✓
T4	140 - 120 (608)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-6523.02	36439.50	0.179 <sup>1</sup> ✓
T4	140 - 120 (609)	L3x3x1/4	3.50	3.38	68.5 K=1.00	1.4400	-6553.19	36439.50	0.180 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Tension Checks

### Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T1	190 - 180	P2.5x.203	10.00	3.08	39.1	1.7040	0.01	88951.40	0.000 <sup>1</sup> ✓
T2	180 - 160	P2.5x.203	20.00	3.21	40.6	1.7040	1208.31	88951.40	0.014 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T1	190 - 180	5/8	4.66	4.35	333.7	0.3068	4591.17	9940.20	0.462 <sup>1</sup> ✓
T2	180 - 160	5/8	4.75	4.42	339.7	0.3068	5665.02	9940.20	0.570 <sup>1</sup> ✓
T3	160 - 140	5/8	4.75	4.42	339.7	0.3068	4863.25	9940.20	0.489 <sup>1</sup> ✓
T4	140 - 120	5/8	4.75	4.42	339.7	0.3068	4683.87	9940.20	0.471 <sup>1</sup> ✓
T5	120 - 100	5/8	4.75	4.42	339.7	0.3068	5230.73	9940.20	0.526 <sup>1</sup> ✓
T6	100 - 80	5/8	4.75	4.42	339.7	0.3068	3890.24	9940.20	0.391 <sup>1</sup> ✓
T7	80 - 60	5/8	4.75	4.42	339.7	0.3068	4296.66	9940.20	0.432 <sup>1</sup> ✓
T8	60 - 40	5/8	4.75	4.42	339.7	0.3068	4185.45	9940.20	0.421 <sup>1</sup> ✓
T9	40 - 20	5/8	4.75	4.42	339.7	0.3068	3145.10	9940.20	0.316 <sup>1</sup> ✓

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 51 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T10	20 - 0	5/8	4.75	4.42	339.7	0.3068	3545.40	9940.20	0.357 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T1	190 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	321.81	17085.90	0.019 <sup>1</sup>
T2	180 - 160	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	667.45	17085.90	0.039 <sup>1</sup>
T3	160 - 140	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	705.70	17085.90	0.041 <sup>1</sup>
T4	140 - 120	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	1034.80	17085.90	0.061 <sup>1</sup>
T5	120 - 100	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	1033.12	17085.90	0.060 <sup>1</sup>
T6	100 - 80	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	1092.08	17085.90	0.064 <sup>1</sup>
T7	80 - 60	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	1166.50	17085.90	0.068 <sup>1</sup>
T8	60 - 40	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	1260.63	17085.90	0.074 <sup>1</sup>
T9	40 - 20	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	1313.58	17085.90	0.077 <sup>1</sup>
T10	20 - 0	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	1314.70	17085.90	0.077 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T8	60 - 40	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	222.82	17085.90	0.013 <sup>1</sup>
T10	20 - 0	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	66.25	17085.90	0.004 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls



<b>RISATower</b>  Phone: FAX:	Job	117-23150	Page	52 of 56
	Project	Ashford, CT	Date	00:05:19 10/19/17
	Client	CDT	Designed by	FAN

### Bottom Girt Design Data (Tension)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>KI/r</i>	<i>A</i> <i>in<sup>2</sup></i>	<i>P<sub>u</sub></i> <i>lb</i>	$\phi P_n$ <i>lb</i>	Ratio $\frac{P_u}{\phi P_n}$
T8	60 - 40	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	90.11	17085.90	0.005 <sup>1</sup>
T10	20 - 0	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	1064.58	17085.90	0.062 <sup>1</sup>

<sup>1</sup>  $P_u / \phi P_n$  controls

### Top Guy Pull-Off Design Data (Tension)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>KI/r</i>	<i>A</i> <i>in<sup>2</sup></i>	<i>P<sub>u</sub></i> <i>lb</i>	$\phi P_n$ <i>lb</i>	Ratio $\frac{P_u}{\phi P_n}$
T1	190 - 180	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	709.77	17085.90	0.042 <sup>1</sup>
T7	80 - 60	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	4010.79	17085.90	0.235 <sup>1</sup>

<sup>1</sup>  $P_u / \phi P_n$  controls

### Bottom Guy Pull-Off Design Data (Tension)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>KI/r</i>	<i>A</i> <i>in<sup>2</sup></i>	<i>P<sub>u</sub></i> <i>lb</i>	$\phi P_n$ <i>lb</i>	Ratio $\frac{P_u}{\phi P_n}$
T4	140 - 120	L1 1/2x1 1/2x3/16	3.50	3.26	85.7	0.5273	115.10	17085.90	0.007 <sup>1</sup>

<sup>1</sup>  $P_u / \phi P_n$  controls

### Torque-Arm Top Design Data

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>KI/r</i>	<i>A</i> <i>in<sup>2</sup></i>	<i>P<sub>u</sub></i> <i>lb</i>	$\phi P_n$ <i>lb</i>	Ratio $\frac{P_u}{\phi P_n}$
T2	180 - 160 (576)	L2x2x5/16	4.75	4.59	91.6	1.1500	12754.20	37260.00	0.342 <sup>1</sup>
T2	180 - 160 (577)	L2x2x5/16	4.75	4.59	91.6	1.1500	12773.10	37260.00	0.343 <sup>1</sup>
T2	180 - 160 (582)	L2x2x5/16	4.75	4.59	91.6	1.1500	12718.90	37260.00	0.341 <sup>1</sup>

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 53 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T2	180 - 160 (583)	L2x2x5/16	4.75	4.59	91.6	1.1500	12751.80	37260.00	0.342 <sup>1</sup>
T2	180 - 160 (588)	L2x2x5/16	4.75	4.59	91.6	1.1500	12714.50	37260.00	0.341 <sup>1</sup>
T2	180 - 160 (589)	L2x2x5/16	4.75	4.59	91.6	1.1500	12766.00	37260.00	0.343 <sup>1</sup>
T4	140 - 120 (594)	L2x2x5/16	4.75	4.59	91.6	1.1500	9815.32	37260.00	0.263 <sup>1</sup>
T4	140 - 120 (595)	L2x2x5/16	4.75	4.59	91.6	1.1500	9834.62	37260.00	0.264 <sup>1</sup>
T4	140 - 120 (600)	L2x2x5/16	4.75	4.59	91.6	1.1500	9760.31	37260.00	0.262 <sup>1</sup>
T4	140 - 120 (601)	L2x2x5/16	4.75	4.59	91.6	1.1500	9829.40	37260.00	0.264 <sup>1</sup>
T4	140 - 120 (606)	L2x2x5/16	4.75	4.59	91.6	1.1500	9742.36	37260.00	0.261 <sup>1</sup>
T4	140 - 120 (607)	L2x2x5/16	4.75	4.59	91.6	1.1500	9831.75	37260.00	0.264 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Torque-Arm Bottom Design Data

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T2	180 - 160 (578)	L3x3x1/4	3.50	3.38	43.6	1.4400	1972.99	46656.00	0.042 <sup>1</sup>
T2	180 - 160 (579)	L3x3x1/4	3.50	3.38	43.6	1.4400	1878.91	46656.00	0.040 <sup>1</sup>
T2	180 - 160 (584)	L3x3x1/4	3.50	3.38	43.6	1.4400	2205.56	46656.00	0.047 <sup>1</sup>
T2	180 - 160 (585)	L3x3x1/4	3.50	3.38	43.6	1.4400	2152.10	46656.00	0.046 <sup>1</sup>
T2	180 - 160 (590)	L3x3x1/4	3.50	3.38	43.6	1.4400	2073.28	46656.00	0.044 <sup>1</sup>
T2	180 - 160 (591)	L3x3x1/4	3.50	3.38	43.6	1.4400	2008.44	46656.00	0.043 <sup>1</sup>
T4	140 - 120 (596)	L3x3x1/4	3.50	3.38	43.6	1.4400	2859.99	46656.00	0.061 <sup>1</sup>
T4	140 - 120 (597)	L3x3x1/4	3.50	3.38	43.6	1.4400	2880.32	46656.00	0.062 <sup>1</sup>
T4	140 - 120 (602)	L3x3x1/4	3.50	3.38	43.6	1.4400	3429.47	46656.00	0.074 <sup>1</sup>
T4	140 - 120 (603)	L3x3x1/4	3.50	3.38	43.6	1.4400	3296.40	46656.00	0.071 <sup>1</sup>
T4	140 - 120 (608)	L3x3x1/4	3.50	3.38	43.6	1.4400	3305.35	46656.00	0.071 <sup>1</sup>

<b>RISATower</b>  Phone: FAX:	<b>Job</b> 117-23150	<b>Page</b> 54 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> lb	φP <sub>n</sub> lb	Ratio $\frac{P_u}{\phi P_n}$
T4	140 - 120 (609)	L3x3x1/4	3.50	3.38	43.6	1.4400	3394.67	46656.00	0.073 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	φP <sub>allow</sub> lb	% Capacity	Pass Fail	
T1	190 - 180	Leg	P2.5x.203	2	-18579.50	78157.90	23.8	Pass	
		Diagonal	5/8	30	4591.17	9940.20	46.2	Pass	
		Horizontal	L1 1/2x1 1/2x3/16	27	-5639.89	11503.00	49.0	Pass	
		Bottom Girt	L1 1/2x1 1/2x3/16	9	-2895.25	11503.00	25.2	Pass	
		Guy A@189.625	9/16	615	10914.60	21000.00	52.0	Pass	
		Guy B@189.625	9/16	614	10877.40	21000.00	51.8	Pass	
		Guy C@189.625	9/16	613	10891.90	21000.00	51.9	Pass	
		Top Guy	L1 1/2x1 1/2x3/16	6	-2584.15	11503.00	22.5	Pass	
		Pull-Off@189.625							
		T2	180 - 160	Leg	P2.5x.203	35	-38535.30	77325.90	49.8
Diagonal	5/8			54	5665.02	9940.20	57.0	Pass	
Horizontal	L1 1/2x1 1/2x3/16			85	-5279.97	11503.00	45.9	Pass	
Top Girt	L1 1/2x1 1/2x3/16			39	-2970.82	11503.00	25.8	Pass	
Guy A@160.375	5/8			586	11921.40	25440.00	46.9	Pass	
Guy B@160.375	5/8			581	12030.20	25440.00	47.3	Pass	
Guy C@160.375	5/8			575	12002.10	25440.00	47.2	Pass	
Top Guy	L1 1/2x1 1/2x3/16			51	-7591.02	11503.00	66.0	Pass	
Pull-Off@160.375									
Bottom Guy	L1 1/2x1 1/2x3/16			42	-3731.17	11503.00	32.4	Pass	
T3	160 - 140	Torque Arm	L2x2x5/16	577	12773.10	37260.00	34.3	Pass	
		Top@160.375					35.7 (b)		
		Torque Arm	L3x3x1/4	584	-8435.37	36439.50	23.1	Pass	
		Bottom@160.375					23.6 (b)		
		Leg	P2.5x.203	94	-40743.50	77325.90	52.7	Pass	
T4	140 - 120	Diagonal	5/8	149	4863.25	9940.20	48.9	Pass	
		Horizontal	L1 1/2x1 1/2x3/16	147	-4541.07	11503.00	39.5	Pass	
		Top Girt	L1 1/2x1 1/2x3/16	99	-3195.08	11503.00	27.8	Pass	
		Bottom Girt	L1 1/2x1 1/2x3/16	100	-2385.74	11503.00	20.7	Pass	
		Leg	P2.5x.203	154	-59744.20	77325.90	77.3	Pass	
		Diagonal	5/8	177	4683.87	9940.20	47.1	Pass	
		Horizontal	L1 1/2x1 1/2x3/16	205	-4460.17	11503.00	38.8	Pass	
		Top Girt	L1 1/2x1 1/2x3/16	157	-2308.95	11503.00	20.1	Pass	
		Guy A@120.375	9/16	604	10231.40	21000.00	48.7	Pass	
		Guy B@120.375	9/16	599	10148.20	21000.00	48.3	Pass	
T5	120 - 100	Guy C@120.375	9/16	593	10237.70	21000.00	48.8	Pass	
		Top Guy	L1 1/2x1 1/2x3/16	169	-6201.84	11503.00	53.9	Pass	
		Pull-Off@120.375							
		Bottom Guy	L1 1/2x1 1/2x3/16	162	-3900.01	11503.00	33.9	Pass	
		Pull-Off@120.375							
		Torque Arm	L2x2x5/16	595	9834.62	37260.00	26.4	Pass	
		Top@120.375					27.5 (b)		
		Torque Arm	L3x3x1/4	609	-6553.19	36439.50	18.0	Pass	
		Bottom@120.375					18.3 (b)		
		Leg	P2.5x.203	214	-59647.20	76068.80	78.4	Pass	
Diagonal	5/8	268	5230.73	9940.20	52.6	Pass			

<h1>RISATower</h1> <p>Phone: FAX:</p>	<b>Job</b> 117-23150	<b>Page</b> 55 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	$\phi P_{allow}$ lb	% Capacity	Pass Fail	
T6	100 - 80	Horizontal	L1 1/2x1 1/2x3/16	267	-3727.72	11503.00	32.4	Pass	
		Top Girt	L1 1/2x1 1/2x3/16	219	-3101.52	11503.00	27.0	Pass	
		Bottom Girt	L1 1/2x1 1/2x3/16	220	-2165.68	11503.00	18.8	Pass	
		Leg	P2.5x.203	275	-63051.00	76085.20	82.9	Pass	
		Diagonal	5/8	328	3890.24	9940.20	39.1	Pass	
T7	80 - 60	Horizontal	L1 1/2x1 1/2x3/16	289	-3819.09	11503.00	33.2	Pass	
		Top Girt	L1 1/2x1 1/2x3/16	279	-1947.50	11503.00	16.9	Pass	
		Bottom Girt	L1 1/2x1 1/2x3/16	281	-1862.27	11503.00	16.2	Pass	
		Leg	P2.5x.203	335	-67347.70	76057.80	88.5	Pass	
		Diagonal	5/8	348	4296.66	9940.20	43.2	Pass	
T8	60 - 40	Horizontal	L1 1/2x1 1/2x3/16	349	-3855.94	11503.00	33.5	Pass	
		Top Girt	L1 1/2x1 1/2x3/16	337	-2114.89	11503.00	18.4	Pass	
		Guy A@60.375	9/16	612	10571.90	21000.00	50.3	Pass	
		Guy B@60.375	9/16	611	10540.00	21000.00	50.2	Pass	
		Guy C@60.375	9/16	610	10548.30	21000.00	50.2	Pass	
		Top Guy	L1 1/2x1 1/2x3/16	342	4010.79	17085.90	23.5	Pass	
		Pull-Off@60.375							
		Leg	P2.5x.203	395	-72782.40	75967.60	95.8	Pass	
		Diagonal	5/8	449	4185.45	9940.20	42.1	Pass	
		Horizontal	L1 1/2x1 1/2x3/16	411	-3523.24	11503.00	30.6	Pass	
T9	40 - 20	Top Girt	L1 1/2x1 1/2x3/16	399	-1828.84	11503.00	15.9	Pass	
		Bottom Girt	L1 1/2x1 1/2x3/16	400	-2090.64	11503.00	18.2	Pass	
		Leg	P2.5x.203	455	-75839.40	75978.80	99.8	Pass	
		Diagonal	5/8	508	3145.10	9940.20	31.6	Pass	
		Horizontal	L1 1/2x1 1/2x3/16	471	-3719.28	11503.00	32.3	Pass	
T10	20 - 0	Top Girt	L1 1/2x1 1/2x3/16	459	-1758.12	11503.00	15.3	Pass	
		Bottom Girt	L1 1/2x1 1/2x3/16	460	-1822.08	11503.00	15.8	Pass	
		Leg	P2.5x.203	515	-75904.00	75972.40	99.9	Pass	
		Diagonal	5/8	528	3545.40	9940.20	35.7	Pass	
		Horizontal	L1 1/2x1 1/2x3/16	565	-3577.58	11503.00	31.1	Pass	
		Top Girt	L1 1/2x1 1/2x3/16	519	-2040.03	11503.00	17.7	Pass	
		Bottom Girt	L1 1/2x1 1/2x3/16	522	1064.58	17085.90	6.2	Pass	
<b>Summary</b>									
		Leg (T10)				99.9	Pass		
		Diagonal (T2)				57.0	Pass		
		Horizontal (T1)				49.0	Pass		
		Top Girt (T3)				27.8	Pass		
		Bottom Girt (T1)				25.2	Pass		
		Guy A (T1)				52.0	Pass		
		Guy B (T1)				51.8	Pass		
		Guy C (T1)				51.9	Pass		
		Top Guy Pull-Off (T2)				66.0	Pass		
		Bottom Guy Pull-Off (T4)				33.9	Pass		
		Torque Arm Top (T2)				35.7	Pass		
		Torque Arm Bottom (T2)				23.6	Pass		
		Bolt Checks				35.7	Pass		
		<b>RATING =</b>				<b>99.9</b>	<b>Pass</b>		

<b><i>RISATower</i></b>  <i>Phone:</i> <i>FAX:</i>	<b>Job</b> 117-23150	<b>Page</b> 56 of 56
	<b>Project</b> Ashford, CT	<b>Date</b> 00:05:19 10/19/17
	<b>Client</b> CDT	<b>Designed by</b> FAN

---

Site Name:

Ashford, CT

Date:

10/19/2017

**Design Base Loads (Factored) per TIA-222-G**

Moment ( $M_u$ ):	0.0 k-ft	Concrete Compressive Strength ( $f'_c$ ):	3000 psi
Shear/Leg ( $V_u$ ):	2.2 k	Vertical Steel Rebar Size #:	5
Compression/Leg ( $P_u$ ):	219.8 k	Vertical Steel Rebar Area:	0.31 in <sup>2</sup>
Uplift/Leg ( $T_u$ ):	0.0 k	# of Vertical Steel Rebars:	8
Tower Type (GT / SST):	GT	Vertical Steel Rebar Yield Strength ( $F_y$ ):	60 ksi
Diameter of Prismatic Portion of Pier (d):	2.0 ft	Tie / Stirrup Size #:	4
Depth to Base of Foundation:	4.5 ft	Tie / Stirrup Area:	0.20 in <sup>2</sup>
Pier Height Above Ground (h):	1.00 ft	Tie / Stirrup Spacing:	10.0 in
Length / Width of Pad (w):	5.5 ft	Tie / Stirrup Steel Yield Strength ( $F_y$ ):	40 ksi
Thickness of Pad (t):	1.5 ft	Rebar Cage Diameter:	16.0 in
Depth Below Ground Surface to Water Table (w):	20.0 ft	Bending/Tension Reduction Factor ( $\phi_B$ ):	0.90
Unit Weight of Concrete:	150.0 pcf	Shear Reduction Factor ( $\phi_V$ ):	0.75
Unit Weight of Water:	62.4 pcf	Compression Reduction Factor ( $\phi_C$ ):	0.65
Unit Weight of Soil Above Water Table:	115.0 pcf	Steel Elastic Modulus:	29000 ksi
Unit Weight of Soil Below Water Table:	50.0 pcf	Pad Steel Rebar Size #:	5
Friction Angle of Uplift from Top of Pad:	33 Degrees	Pad Steel Rebar Area:	0.31 in <sup>2</sup>
Friction Angle of Uplift from Base of Pad:	33 Degrees	Pad Steel Rebar Yield Strength ( $F_y$ ):	60 ksi
Uplift Angle Started at Top or Base of Pad (T/B):	B	# of Rebar in Top of Pad:	0
Ultimate Skin Friction:	0 psf	# of Rebar in Base of Pad:	5
Ultimate Compressive Bearing Pressure:	14000 psf	Pad Clear Cover:	3 in
Bearing Strength Reduction Factor ( $\phi_s$ ):	0.60		
Uplift Strength Reduction Factor ( $\phi_s$ ):	0.75		

**Axial Capacities and Design Moment**

Nominal Uplift Capacity per Leg ( $\phi_s T_n$ ):	30.4 k
Nominal Compressive Capacity per Leg ( $\phi_s P_n$ ):	254.1 k
$P_u$ :	222.7 k
$T_u / \phi_s T_n$ :	0.00 Result: OK
$P_u / \phi_s P_n$ :	0.88 Result: OK

### Pad Strength Capacity

$\beta$ :	0.85 ACI318-05 - 10.2.7.3
Lower Pad Flexural Reinforcement Ratio:	0.0016 OK - Minimum Reinforcement Ratio Met - /
Upper Pad Flexural Reinforcement Ratio:	0.0000 OK - Minimum Reinforcement Ratio Met - /
Lower Pad Flexural Reinforcement Spacing:	15 in - Pad Reinforcing Spacing OK - ACI7.12.2.2 & 10.5.4
Upper Pad Flexural Reinforcement Spacing:	0 in - Pad Reinforcing Spacing OK - ACI7.12.2.2 & 10.5.4
One Way Design Shear ( $V_u$ ):	21.3 k
One Way Shear Capacity ( $\phi V_c$ ):	81.2 k - ACI318-05 - 11.3.1.1
$V_u / \phi V_c$ :	0.26 Result: OK
Punching Design Shear ( $V_u$ ):	159.9 k
Nominal Punching Shear Capacity ( $\phi_c V_n$ ):	293.3 k - ACI318-05 - 11.12.2.1
$V_u / \phi V_c$ :	0.54 Result: OK
Flexural Loading Due to Soil Pressure ( $M_u$ ):	61.9 k-ft
Lower Steel Pad Moment Capacity ( $\phi M_n$ ):	100.5 k-ft - ACI318-05 - 10.3
$M_u / \phi M_n$ :	0.62 Result: OK
Flexural Loading Due to Uplift ( $M_u$ ):	0.0 k-ft
Upper Steel Pad Moment Capacity ( $\phi M_n$ ):	0.0 k-ft - ACI318-05 - 10.3
$M_u / \phi M_n$ :	0.00 Result: OK

### Pier Strength Capacity

Design Moment ( $M_u$ ):	4.9 k-ft
Nominal Moment Capacity ( $\phi_B M_n$ ):	88.9 k-ft - ACI318-005 - 10.2
$M_u / \phi_B M_n$ :	0.06 Result: OK
Design Shear ( $V_u$ ):	2.2 k
Nominal Shear Capacity ( $\phi_V V_n$ ):	67.9 k - ACI318-05 - 11.3.1.1 or 11.5.7.2
$V_u / \phi_V V_n$ :	0.03 Result: OK
Design Tension ( $T_u$ ):	0.0 k
Nominal Tension Capacity ( $\phi_T T_n$ ):	133.9 k - ACI318-05 - 10.2
$T_u / \phi_T T_n$ :	0.00 Result: OK
Design Compression ( $P_u$ ):	219.8 k
Nominal Compression Capacity ( $\phi_P P_n$ ):	701.9 k - ACI318-05 - 10.3.6.2
$P_u / \phi_P P_n$ :	0.31 Result: OK
Pier Reinforcement Ratio:	0.005 Reinforcement Ratio is Satisfactory - ACI318-05 - 10.9.1 & 10.8.4
$M_u / \phi_B M_n + T_u / \phi_T T_n$ :	0.06 Result: OK

Site Name: **Ashford, CT**  
 Date: **10/19/2017**

**Design Standard per TIA-222-G**

Uplift (Factored - $P_u$ ):	42.0 k
Shear (Factored - $V_u$ ):	47.2 k
Anchor Base Depth (d):	8.0 ft
Width of Anchor (W):	5.5 ft
Length of Anchor (L):	11.5 ft
Thickness of Anchor (t):	2.0 ft
Depth Below Ground Surface to Water Table (w):	20.0 ft
Soil Uplift at Base / Top of Anchor (B/T):	T
Unit Weight of Concrete:	150.0 pcf
Unit Weight of Soil Above Water Table:	115.0 pcf
Unit Weight of Water:	62.4 pcf
Submerged Soil Unit Weight:	50.0 pcf
Internal Angle of Friction:	33 Degrees
Cohesion:	0 psf
Ultimate Skin Friction of Pad Sides to Soil:	450 psf
Ultimate Coefficient of Shear Friction:	0.30
Maximum Top Conical Failure Angle:	33 Degrees
Maximum Base Conical Failure Angle:	33 Degrees
Allowable Capacity Increase:	1.00 (Due to Transient Loads)
Uplift Strength Reduction Factor ( $\phi_u$ ):	0.75
Shear Strength Reduction Factor ( $\phi_v$ ):	0.75
Concrete Uplift Strength Reduction Factor ( $\phi_u$ ):	0.90

**Uplift**

Weight of Concrete (Buoyancy Effect Considered):	19.0 k
Weight of Soil (Buoyancy Effect Considered):	100.3 k
Ultimate Uplift Resistance from Skin Friction:	20.3 k
Nominal Factored Uplift Resistance ( $\phi_u P_n$ ):	92.3 k
$P_u / \phi_u P_n$ :	0.46 Result: OK

**Shear**

Ultimate Shear Friction Resistance Due to Normal Force - Uplift:	9.3 k
Passive Pressure:	2731 psf
Ultimate Passive Pressure Resistance:	62.8 k
Nominal Shear Resistance ( $\phi_v V_n$ ):	54.1 k
$V_u / \phi_v V_n$ :	0.87 Result: OK

**Anchor Rod Capacity**

# of Anchor Rods:	1	Rod $F_y$ :	48 ksi
Anchor Rod Gross Area:	2.41 in <sup>2</sup>	Rod $F_u$ :	62 ksi
Anchor Rod Net Area:	2.41 in <sup>2</sup>	$\phi_y$ :	0.80
Resultant Tensile Load ( $T_u$ ):	63.2 k	$\phi_t$ :	0.65
Anchor Rod Tensile Resistance ( $\phi T_n$ ):	92.5 k		
$T_u / \phi T_n$ :	0.68 Result: OK		



## Strength Analysis of Reinforced Concrete

Concrete Compressive Strength ( $f'_c$ ):	3000 psi
Longitudinal Rebar Yield Strength:	60000 psi
# Longitudinal Rebar (Top):	6
# Longitudinal Rebar (1 Side):	5
Rebar Size:	4
Strength Reduction Factor for Shear ( $\phi_v$ ):	0.75
Strength Reduction Factor for Flexure ( $\phi_b$ ):	0.9
Compression Zone Factor ( $\beta_1$ ):	0.85
Area of Single Rebar:	0.20 in <sup>2</sup>
One Way Shear due to Shear Load ( $V_u$ ):	13.0 k
Nominal One Way Shear Capacity for Shear Load ( $\phi_c V_n$ ):	122.3 k
$V_u/\phi_v V_n$ :	0.11 Result: OK
One Way Shear due to Uplift ( $V_u$ ):	18.0 k
Nominal One Way Shear Capacity for Uplift ( $\phi_c V_n$ ):	108.4 k
$V_u/\phi_v V_n$ :	0.17 Result: OK
Pad Flexure due to Shear Load ( $M_u$ ):	67.8 k-ft
Nominal Flexural Capacity for Shear Load ( $\phi_b M_n$ ):	279.0 k-ft
Pad Flexure due to Uplift ( $M_u$ ):	60.4 k-ft
Nominal Flexural Capacity for Uplift ( $\phi_b M_n$ ):	107.9 k-ft
$M_u/\phi_b M_n$ (Max.):	0.56 Result: OK

# **ATTACHMENT 4**



necog

Necog GIS Site

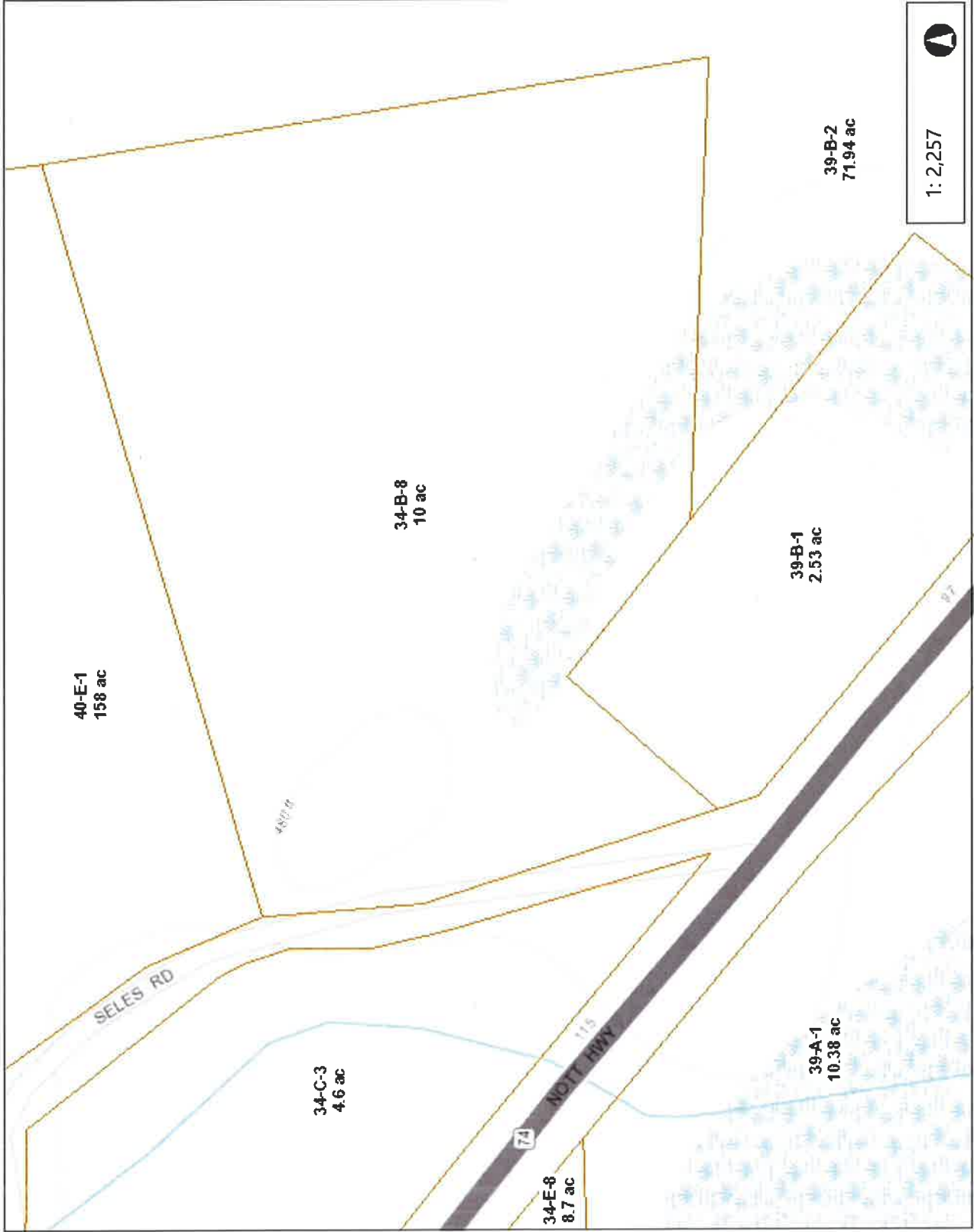


Legend

- Town
- Buildings
- Parcels
- Parcel Label

Notes

Enter Map Description



1: 2,257

Miles



This map is a user generated static output from an Internet mapping site and is for reference only. Data layers that appear on this map may or may not be accurate, current, or otherwise reliable.

**THIS MAP IS NOT TO BE USED FOR NAVIGATION**

## 20 SELES RD

**Location** 20 SELES RD

**Mblu** 34/ B/ 8/ /

**Acct#** 00016400

**Owner** BAKER RAYMOND C &  
KATHLEEN P

**Assessment** \$111,670

**Appraisal** \$178,900

**PID** 152

**Building Count** 1

**Lot Type**

**topoTopography** Above Grade

**Location** Bus. District,Rural

### Current Value

Appraisal					
Valuation Year	Building	Extra Features	Outbuildings	Land	Total
2016	\$108,000	\$3,000	\$2,700	\$65,200	\$178,900

Assessment					
Valuation Year	Building	Extra Features	Outbuildings	Land	Total
2016	\$75,600	\$2,100	\$1,900	\$32,070	\$111,670

### Parcel Addresses

Additional Addresses		
Address	City, State Zip	Type
20 SELES RD		Primary

### Owner of Record

**Owner** BAKER RAYMOND C & KATHLEEN P  
**Co-Owner**  
**Address** 20 SELES RD  
ASHFORD, CT 06278

**Sale Price** \$0  
**Certificate** C  
**Book & Page** 058/ 245  
**Sale Date** 07/15/1968

### Ownership History

Ownership History				
Owner	Sale Price	Certificate	Book & Page	Sale Date
BAKER RAYMOND C & KATHLEEN P	\$0	C	058/ 245	07/15/1968

### Building Information

**Building 1 : Section 1**

**Building Photo**

**Year Built:** 1968  
**Living Area:** 1,581  
**Replacement Cost:** \$142,141  
**Building Percent Good:** 76  
**Replacement Cost Less Depreciation:** \$108,000

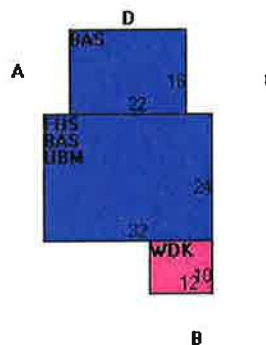
**Building Photo**



(http://images.vgsi.com/photos/AshfordCTPhotos//\00\00\00/7

Building Attributes	
Field	Description
Style	Cape Cod
Model	Residential
Grade:	C
Stories:	1.5
Occupancy	1
Exterior Wall 1	Clapboard
Exterior Wall 2	Vinyl Siding
Roof Structure:	Gable
Roof Cover	Asphalt Shingl
Interior Wall 1	Plywood Panel
Interior Wall 2	Drywall
Interior Flr 1	Carpet
Interior Flr 2	
Heat Fuel	Oil
Heat Type:	Forced Air
AC Type:	None
Total Bedrooms:	3 Bedrooms
Total Bthrms:	1
Total Half Baths:	1
Total Xtra Fixtrs:	0
Total Rooms:	6
Bath Style:	Average
Kitchen Style:	Average
Bsmt. Garages	1

**Building Layout**



Building Sub-Areas (sq ft)			Legend
Code	Description	Gross Area	Living Area
BAS	First Floor	1,120	1,120
FHS	Finished Half Story	768	461
UBM	Unfinished Basement	768	0
WDK	Wood Deck	120	0
		2,776	1,581

**Extra Features**

Extra Features							Legend
Code	Description	Sub Code	Sub Description	Size	Value	Assessed Value	Bldg #
FPL2	Fireplace 1.5 Sty			1 units	\$3,000	\$2,100	1

**Parcel Information**

**Use Code** 101  
**Description** Single Family  
**Deeded Acres** 10

**Land**

**Land Use**

**Use Code** 101  
**Description** Single Family  
**Zone** RA  
**Neighborhood**  
**Alt Land Appr Category** No

**Land Line Valuation**

**Size (Acres)** 10  
**Frontage**  
**Depth**  
**Assessed Value** \$32,070  
**Appraised Value** \$65,200

Special Land			
Land Use Code	Land Use Description	Units	Unit Type
630	Open Space	9	AC

**Outbuildings**

Outbuildings							Legend
Code	Description	Sub Code	Sub Description	Size	Value	Assessed Value	Bldg #
BRN1	Barn 1 Story	FR	Frame	520 S.F.	\$2,400	\$1,700	1
CNP	Canopy			180 S.F.	\$300	\$200	1

**Valuation History**

Appraisal					
Valuation Year	Building	Extra Features	Outbuildings	Land	Total
2016	\$115,100	\$3,200	\$2,700	\$65,200	\$186,200
2015	\$101,300	\$3,300	\$2,700	\$74,800	\$182,100
2014	\$101,300	\$3,300	\$9,000	\$74,800	\$188,400

Assessment					
Valuation Year	Building	Extra Features	Outbuildings	Land	Total
2016	\$80,600	\$2,200	\$1,900	\$32,070	\$116,770
2015	\$70,900	\$2,300	\$1,900	\$36,020	\$111,120
2014	\$70,900	\$2,300	\$6,300	\$36,020	\$115,520

# **ATTACHMENT 5**



# Certificate of Mailing — Firm


Name and Address of Sender  
**Kenneth C. Baldwin, Esq.**  
**Robinson & Cole LLP**  
**280 Trumbull Street**  
**Hartford, CT 06103**

TOTAL NO. of Pieces Listed by Sender  
 3

TOTAL NO. of Pieces Received at Post Office™  
 3

Postmaster, per (name of receiving employee)  
 [Signature]

Affix Stamp Here  
 Postmark with Date of Receipt.



USPS® Tracking Number Firm-specific Identifier	Address (Name, Street, City, State, and ZIP Code™)	Postage	Fee	Special Handling	Parcel Airlift
1.	Michael J. Zambo, First Selectman Town of Ashford 5 Town Hall Road Ashford, CT 06278				
2.	Michael Gardner, Land Use Administrator Town of Ashford 5 Town Hall Road Ashford, CT 06278				
3.	Raymond and Kathleen Baker 20 Seles Road Ashford, CT 06278				
4.					
5.					
6.					

