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Also admitted in Massachusetts

October 16, 2018

Melanie A. Bachman, Esq.
Executive Director/Staff Attorney
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: Docket No. 475 – Cellco Partnership d/b/a Verizon Wireless Telecommunications Facility, Folly Lane, Coventry, Connecticut

Dear Ms. Bachman:

At its meeting of July 19, 2018, the Siting Council approved the Development and Management (“D&M”) Plan submitted for the above-referenced telecommunications facility in Coventry, CT. Enclosed please find the original and fifteen (15) copies of a substitute tower and tower foundation design package for the approved Coventry NW telecommunications facility.

Please let me know if you have any questions or need any additional information.

Sincerely,



Kenneth C. Baldwin

KCB/kmd
Enclosures
Copy to:

Andrew Candiello (*via e-mail*)
John Tierney (*via e-mail*)
Greg Milano (*via e-mail*)
Eric Campbell (*via e-mail*)
Douglas Roberts (*via e-mail*)

18567524-v1



Structural Design Report
140' Extendible to 160' Monopole
Site: Coventry Northwest, CT

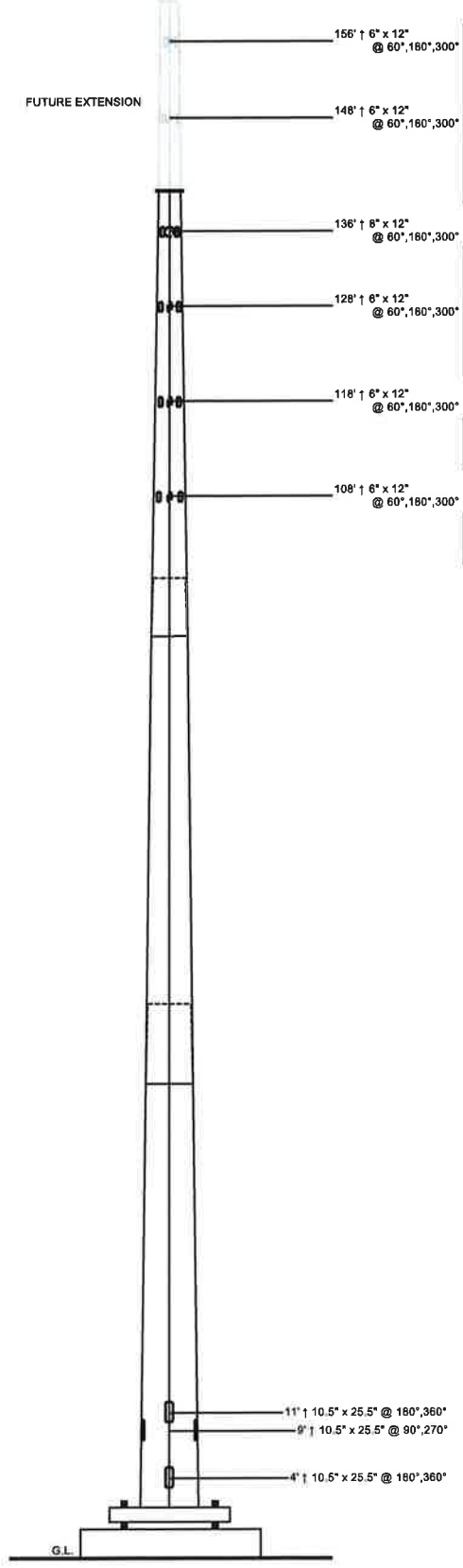
Prepared for: VERIZON WIRELESS
by: Sabre Towers & Poles™

Job Number: 418393
Revision A
October 15, 2018

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Length (ft)	53'-3"	53'-6"	18	47'-0"	20'-0"
Number Of Sides	1/2"	8" - 6"	3/8"	1/4"	
Thickness (in)					
Lap Splice (ft)					
Top Diameter (in)	56.53"	41.77"	28.25"	21"	
Bottom Diameter (in)	75.18"	60.51"	44.71"	28"	
Taper (in/ft)		0.3502			
Grade		A572-65			
Weight (lbs)	22823	15501	7920	1857	
Overall Steel Height (ft)		139			20 (Extension)



Designed Appurtenance Loading

Elev	Description	Tx-Line
160***	(1) 200 sq.ft. (no ice) 225 sq.ft. (ice)	(12) 1 5/8"
150***	(1) 200 sq.ft. (no ice) 225 sq.ft. (ice)	(12) 1 5/8"
140	(1) 250 Sq. Ft. EPA (3000 lbs)	(16) 1 5/8"
130	(1) 200 sq.ft. (no ice) 225 sq.ft. (ice)	(12) 1 5/8"
120	(1) 200 sq.ft. (no ice) 225 sq.ft. (ice)	(12) 1 5/8"
110	(1) 200 sq.ft. (no ice) 225 sq.ft. (ice)	(12) 1 5/8"

Load Case Reactions

Description	Axial (kips)	Shear (kips)	Moment (ft-k)	Deflection (ft)	Sway (deg)
3s Gusted Wind	85.11	94.23	11717.65	11.24	8.07
3s Gusted Wind 0.9 Dead	63.83	94.21	11610	11.1	7.96
3s Gusted Wind&Ice	140.42	28.06	3632.15	3.6	2.55
Service Loads	70.95	18.61	2311.89	2.25	1.6

Base Plate Dimensions

Shape	Width	Thickness	Bolt Circle	Bolt Qty	Bolt Diameter
Square	83.5"	2.75"	82.75"	28	2.25"

Anchor Bolt Dimensions

Length	Diameter	Hole Diameter	Weight	Type	Finish
84"	2.25"	2.625"	3390.8	A615-76	Galv

Notes

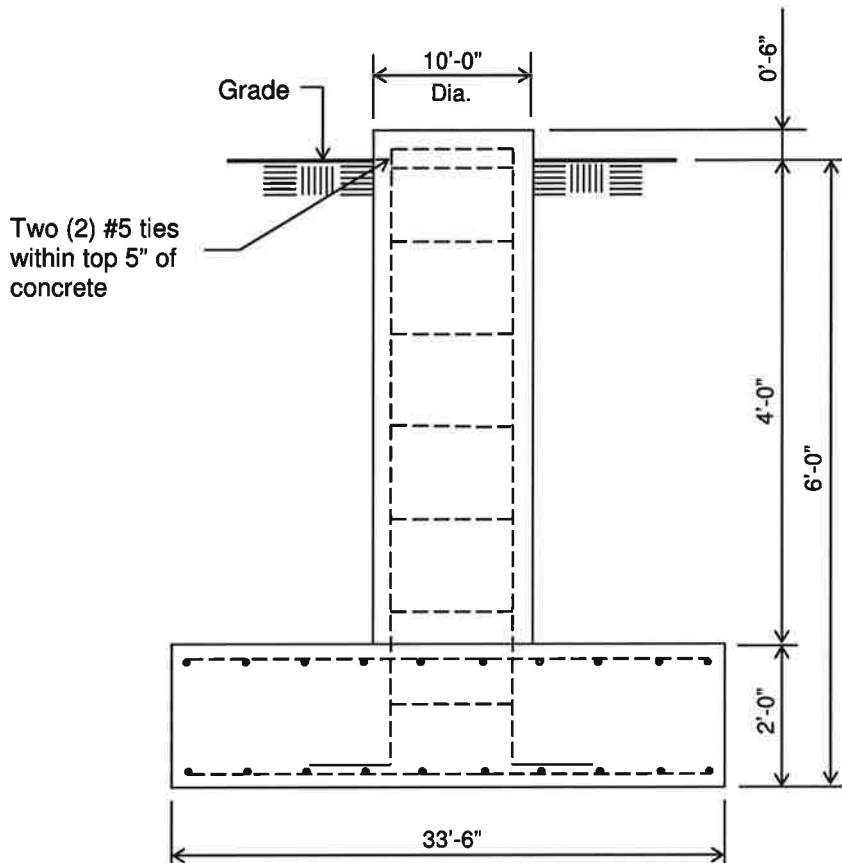
- 1) Antenna Feed Lines Run Inside Pole
 - 2) All dimensions are above ground level, unless otherwise specified.
 - 3) Weights shown are estimates. Final weights may vary.
 - 4) The Monopole was designed for a basic wind speed of 101 mph with 0" of radial ice, and 50 mph with 1" of radial ice, in accordance with ANSI/TIA-222-G, Structure Class II, Exposure Category C, Topographic Category 1.
 - 5) Full Height Step Bolts
 - 6) The tower design meets the requirements for an Ultimate Wind Speed of 130 mph (Risk Category II), in accordance with the 2016 Connecticut Building Code.
 - 7) Tower Rating: 100%
- *** These Appurtenances cannot be installed until the Monopole has been extended.

	Sabre Communications Corporation 7101 Southbridge Drive P.O. Box 658 Sioux City, IA 51102-0658 Phone: (712) 258-8690 Fax: (712) 279-0814	Job: 418393A Customer: VERIZON WIRELESS Site Name: Coventry Northwest, CT Description: 140' ext. 160' Monopole Date: 10/15/2018	By: REB
	<small>Information contained herein is the sole property of Sabre Communications Corporation, constitutes a trade secret as defined by Iowa Code Ch. 550 and shall not be reproduced, copied or used in whole or part for any purpose whatsoever without the prior written consent of Sabre Communications Corporation.</small>		

Customer: VERIZON WIRELESS

Site: Coventry Northwest, CT

140' Monopole Extendible to 160' at
101 mph wind and 50 mph wind with 1" ice per ANSI/TIA-222-G.



ELEVATION VIEW

(96.22 Cu. Yds.)

(1 REQUIRED; NOT TO SCALE)

Notes:

- 1) Concrete shall have a minimum 28-day compressive strength of 4,500 psi, in accordance with ACI 318-11.
- 2) Rebar to conform to ASTM specification A615 Grade 60.
- 3) All rebar to have a minimum of 3" concrete cover.
- 4) All exposed concrete corners to be chamfered 3/4".
- 5) The foundation design is based on the geotechnical report by Hudson Design Group, LLC., Project Name "VZW Coventry NW CT; Rev 1" dated: March 2nd, 2018 and the addendum letter dated: October 18, 2018.
- 6) See the geotechnical report for compaction requirements, if specified.
- 7) 4 ft of soil cover is required over the entire area of the foundation slab.
- 8) The foundation is based on the following factored loads:
Moment = 11,717.65 k-ft
Axial = 85.11 k
Shear = 94.23 k

Rebar Schedule for Pad and Pier	
Pier	(58) #9 vertical rebar w/ hooks at bottom w/ #5 ties, two within top 5" of pier, then 12" C/C
Pad	(74) #9 horizontal rebar evenly spaced each way top and bottom (296 total)

418393A - Extension

(USA 222-G) - Monopole Spatial Analysis (c)2015 Guymast Inc.
 Tel:(416)736-7453 Fax:(416)736-4372 Web:www.guymast.com

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Sabre Towers and Poles on: 15 oct 2018 at: 14:39:02

140' ext. 160' Monopole / Coventry Northwest, CT

* All pole diameters shown on the following pages are across corners.
 See profile drawing for widths across flats.

===== POLE GEOMETRY =====

ELEV ft	SECTION NAME	No. SIDE	OUTSIDE DIAM in	THICK -NESS in	RESISTANCES ♦*Pn ♦*Mn kip ft-kip	SPLICE TYPE	...OVERLAP... LENGTH ft	RATIO	w/t
159.0	A	18	21.32	0.250	1223.3 520.0				13.0
139.0	B	18	28.44	0.250	1590.4 906.9				11.5
98.2	B/C	18	43.17	0.375	2465.3 1406.1	SLIP	6.25	1.71	
92.0	C	18	44.65	0.500	3609.4 3125.3				13.7
53.2	C/D	18	58.41	0.500	5125.9 4567.6	SLIP	8.50	1.71	
44.7	D	18	60.44	0.500	6483.9 7597.7				19.2
0.0			76.34	0.500	6641.9 8059.2				

===== POLE ASSEMBLY =====

SECTION NAME	BASE ELEV ft	BOLTS NUMBER	AT BASE TYPE	OF SECTION DIAM in	STRENGTH ksi	THREADS IN SHEAR PLANE	CALC BASE ELEV ft
A	139.000	0	A325	0.00	92.0	0	139.000
B	92.000	0	A325	0.00	92.0	0	92.000
C	44.750	0	A325	0.00	92.0	0	44.750
D	0.000	0	A325	0.00	92.0	0	0.000

===== POLE SECTIONS =====

SECTION NAME	No.of SIDES	LENGTH ft	OUTSIDE DIAMETER BOT * in	TOP * in	BEND RAD in	MAT- ERIAL ID	FLANGE.ID BOT	TOP	FLANGE.WELD ..GROUP.ID.. BOT	TOP
A	18	20.00	28.44	21.32	0.000	1	0	0	0	0
B	18	47.00	45.40	28.69	0.000	2	0	0	0	0
C	18	53.50	61.44	42.42	0.000	3	0	0	0	0
D	18	53.25	76.34	57.41	0.000	4	0	0	0	0

* - Diameter of circumscribed circle

418393A - Extension

MATERIAL TYPES

TYPE OF SHAPE	TYPE NO	NO OF ELEM.	ORIENT	HEIGHT	WIDTH	.THICKNESS.		IRREGULARITY	
			& deg	in	in	WEB	FLANGE	PROJECTION.	ORIENT
						in	in	% OF AREA	deg
PL	1	1	0.0	28.44	0.25	0.250	0.250	0.00	0.0
PL	2	1	0.0	45.40	0.38	0.375	0.375	0.00	0.0
PL	3	1	0.0	61.44	0.50	0.500	0.500	0.00	0.0
PL	4	1	0.0	76.34	0.50	0.500	0.500	0.00	0.0

& - with respect to vertical

MATERIAL PROPERTIES

MATERIAL TYPE NO.	ELASTIC MODULUS ksi	UNIT WEIGHT pcf	STRENGTH		THERMAL COEFFICIENT /deg
			Fu ksi	Fy ksi	
1	29000.0	490.0	80.0	65.0	0.00001170
2	29000.0	490.0	80.0	65.0	0.00001170
3	29000.0	490.0	80.0	65.0	0.00001170
4	29000.0	490.0	80.0	65.0	0.00001170

* Only 3 condition(s) shown in full

* Some concentrated wind loads may have been derived from full-scale wind tunnel testing

LOADING CONDITION A

101 mph wind with no ice. Wind Azimuth: 0°

LOADS ON POLE

LOAD TYPE	ELEV ft	APPLY. RADIUS ft	LOAD AT AZI	LOAD AZI	FORCES		MOMENTS	
					HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	159.000	0.00	0.0	0.0	12.2018	3.7200	0.0000	0.0000
C	157.000	0.00	0.0	0.0	0.0000	2.3512	0.0000	0.0000
C	149.000	0.00	0.0	0.0	0.0000	2.2314	0.0000	0.0000
C	149.000	0.00	0.0	0.0	12.0372	3.7200	0.0000	0.0000
C	139.000	0.00	0.0	0.0	14.8295	3.6000	0.0000	0.0000
C	137.000	0.00	0.0	0.0	0.0000	3.0776	0.0000	0.0000
C	129.000	0.00	0.0	0.0	0.0000	1.9319	0.0000	0.0000
C	129.000	0.00	0.0	0.0	11.6799	3.7200	0.0000	0.0000
C	119.000	0.00	0.0	0.0	0.0000	1.7821	0.0000	0.0000
C	119.000	0.00	0.0	0.0	11.4848	3.7200	0.0000	0.0000
C	109.000	0.00	0.0	0.0	0.0000	1.6324	0.0000	0.0000
C	109.000	0.00	0.0	0.0	11.2763	3.7200	0.0000	0.0000
D	159.000	0.00	180.0	0.0	0.0732	0.0702	0.0000	0.0000
D	139.000	0.00	180.0	0.0	0.0889	0.0870	0.0000	0.0000
D	139.000	0.00	180.0	0.0	0.0989	0.1474	0.0000	0.0000
D	125.417	0.00	180.0	0.0	0.0989	0.1474	0.0000	0.0000
D	125.417	0.00	180.0	0.0	0.1117	0.1703	0.0000	0.0000
D	111.833	0.00	180.0	0.0	0.1117	0.1703	0.0000	0.0000
D	111.833	0.00	180.0	0.0	0.1235	0.1931	0.0000	0.0000
D	98.250	0.00	180.0	0.0	0.1235	0.1931	0.0000	0.0000
D	98.250	0.00	180.0	0.0	0.1315	0.4850	0.0000	0.0000
D	92.000	0.00	180.0	0.0	0.1315	0.4850	0.0000	0.0000
D	92.000	0.00	180.0	0.0	0.1363	0.2967	0.0000	0.0000
D	79.083	0.00	180.0	0.0	0.1363	0.2967	0.0000	0.0000
D	79.083	0.00	180.0	0.0	0.1446	0.3257	0.0000	0.0000
D	66.167	0.00	180.0	0.0	0.1446	0.3257	0.0000	0.0000
D	66.167	0.00	180.0	0.0	0.1512	0.3547	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1512	0.3547	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1550	0.7520	0.0000	0.0000
D	44.750	0.00	180.0	0.0	0.1550	0.7520	0.0000	0.0000

418393A - Extension								
D	44.750	0.00	180.0	0.0	0.1542	0.3953	0.0000	0.0000
D	33.562	0.00	180.0	0.0	0.1542	0.3953	0.0000	0.0000
D	33.562	0.00	180.0	0.0	0.1531	0.4205	0.0000	0.0000
D	22.375	0.00	180.0	0.0	0.1531	0.4205	0.0000	0.0000
D	22.375	0.00	180.0	0.0	0.1464	0.4457	0.0000	0.0000
D	11.188	0.00	180.0	0.0	0.1464	0.4457	0.0000	0.0000
D	11.188	0.00	180.0	0.0	0.1495	0.4709	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.1495	0.4709	0.0000	0.0000

LOADING CONDITION M

101 mph wind with no ice. Wind Azimuth: 0♦

LOADS ON POLE

LOAD TYPE	ELEV ft	APPLY. RADIUS ft	LOAD. AT AZI	LOAD AZI	FORCES		MOMENTS	
					HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	159.000	0.00	0.0	0.0	12.2018	2.7900	0.0000	0.0000
C	157.000	0.00	0.0	0.0	0.0000	1.7634	0.0000	0.0000
C	149.000	0.00	0.0	0.0	0.0000	1.6736	0.0000	0.0000
C	149.000	0.00	0.0	0.0	12.0372	2.7900	0.0000	0.0000
C	139.000	0.00	0.0	0.0	14.8295	2.7000	0.0000	0.0000
C	137.000	0.00	0.0	0.0	0.0000	2.3082	0.0000	0.0000
C	129.000	0.00	0.0	0.0	0.0000	1.4489	0.0000	0.0000
C	129.000	0.00	0.0	0.0	11.6799	2.7900	0.0000	0.0000
C	119.000	0.00	0.0	0.0	0.0000	1.3366	0.0000	0.0000
C	119.000	0.00	0.0	0.0	11.4848	2.7900	0.0000	0.0000
C	109.000	0.00	0.0	0.0	0.0000	1.2243	0.0000	0.0000
C	109.000	0.00	0.0	0.0	11.2763	2.7900	0.0000	0.0000
D	159.000	0.00	180.0	0.0	0.0732	0.0526	0.0000	0.0000
D	139.000	0.00	180.0	0.0	0.0889	0.0653	0.0000	0.0000
D	139.000	0.00	180.0	0.0	0.0989	0.1105	0.0000	0.0000
D	125.417	0.00	180.0	0.0	0.0989	0.1105	0.0000	0.0000
D	125.417	0.00	180.0	0.0	0.1117	0.1277	0.0000	0.0000
D	111.833	0.00	180.0	0.0	0.1117	0.1277	0.0000	0.0000
D	111.833	0.00	180.0	0.0	0.1235	0.1449	0.0000	0.0000
D	98.250	0.00	180.0	0.0	0.1235	0.1449	0.0000	0.0000
D	98.250	0.00	180.0	0.0	0.1315	0.3637	0.0000	0.0000
D	92.000	0.00	180.0	0.0	0.1315	0.3637	0.0000	0.0000
D	92.000	0.00	180.0	0.0	0.1363	0.2225	0.0000	0.0000
D	79.083	0.00	180.0	0.0	0.1363	0.2225	0.0000	0.0000
D	79.083	0.00	180.0	0.0	0.1446	0.2443	0.0000	0.0000
D	66.167	0.00	180.0	0.0	0.1446	0.2443	0.0000	0.0000
D	66.167	0.00	180.0	0.0	0.1512	0.2660	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1512	0.2660	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.1550	0.5640	0.0000	0.0000
D	44.750	0.00	180.0	0.0	0.1550	0.5640	0.0000	0.0000
D	44.750	0.00	180.0	0.0	0.1542	0.2965	0.0000	0.0000
D	33.562	0.00	180.0	0.0	0.1542	0.2965	0.0000	0.0000
D	33.562	0.00	180.0	0.0	0.1531	0.3154	0.0000	0.0000
D	22.375	0.00	180.0	0.0	0.1531	0.3154	0.0000	0.0000
D	22.375	0.00	180.0	0.0	0.1464	0.3343	0.0000	0.0000
D	11.188	0.00	180.0	0.0	0.1464	0.3343	0.0000	0.0000
D	11.188	0.00	180.0	0.0	0.1495	0.3532	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.1495	0.3532	0.0000	0.0000

LOADING CONDITION Y

50 mph wind with 1 ice. Wind Azimuth: 0♦

LOADS ON POLE

LOAD TYPE	ELEV ft	APPLY. RADIUS ft	LOAD. AT AZI	LOAD AZI	FORCES		MOMENTS	
					HORIZ kip	DOWN kip	VERTICAL ft-kip	TORSNAL ft-kip
C	159.000	0.00	0.0	0.0	2.9633	9.3408	0.0000	0.0000

418393A - Extension								
C	157.000	0.00	0.0	0.0	0.0000	2.3512	0.0000	0.0000
C	149.000	0.00	0.0	0.0	0.0000	2.2314	0.0000	0.0000
C	149.000	0.00	0.0	0.0	2.9163	9.3047	0.0000	0.0000
C	139.000	0.00	0.0	0.0	7.5207	10.5329	0.0000	0.0000
C	137.000	0.00	0.0	0.0	0.0000	3.0776	0.0000	0.0000
C	129.000	0.00	0.0	0.0	0.0000	1.9319	0.0000	0.0000
C	129.000	0.00	0.0	0.0	2.8150	9.2253	0.0000	0.0000
C	119.000	0.00	0.0	0.0	0.0000	1.7821	0.0000	0.0000
C	119.000	0.00	0.0	0.0	2.7599	9.1814	0.0000	0.0000
C	109.000	0.00	0.0	0.0	0.0000	1.6324	0.0000	0.0000
C	109.000	0.00	0.0	0.0	2.7013	9.1341	0.0000	0.0000
D	159.000	0.00	180.0	0.0	0.0250	0.1403	0.0000	0.0000
D	154.000	0.00	180.0	0.0	0.0250	0.1403	0.0000	0.0000
D	154.000	0.00	180.0	0.0	0.0265	0.1507	0.0000	0.0000
D	149.000	0.00	180.0	0.0	0.0265	0.1507	0.0000	0.0000
D	149.000	0.00	180.0	0.0	0.0279	0.1611	0.0000	0.0000
D	144.000	0.00	180.0	0.0	0.0279	0.1611	0.0000	0.0000
D	144.000	0.00	180.0	0.0	0.0293	0.1715	0.0000	0.0000
D	139.000	0.00	180.0	0.0	0.0293	0.1715	0.0000	0.0000
D	139.000	0.00	180.0	0.0	0.0321	0.2412	0.0000	0.0000
D	125.417	0.00	180.0	0.0	0.0321	0.2412	0.0000	0.0000
D	125.417	0.00	180.0	0.0	0.0356	0.2764	0.0000	0.0000
D	111.833	0.00	180.0	0.0	0.0356	0.2764	0.0000	0.0000
D	111.833	0.00	180.0	0.0	0.0388	0.3113	0.0000	0.0000
D	98.250	0.00	180.0	0.0	0.0388	0.3113	0.0000	0.0000
D	98.250	0.00	180.0	0.0	0.0409	0.6115	0.0000	0.0000
D	92.000	0.00	180.0	0.0	0.0409	0.6115	0.0000	0.0000
D	92.000	0.00	180.0	0.0	0.0422	0.4289	0.0000	0.0000
D	79.083	0.00	180.0	0.0	0.0422	0.4289	0.0000	0.0000
D	79.083	0.00	180.0	0.0	0.0443	0.4679	0.0000	0.0000
D	66.167	0.00	180.0	0.0	0.0443	0.4679	0.0000	0.0000
D	66.167	0.00	180.0	0.0	0.0460	0.5060	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0460	0.5060	0.0000	0.0000
D	53.250	0.00	180.0	0.0	0.0469	0.9099	0.0000	0.0000
D	44.750	0.00	180.0	0.0	0.0469	0.9099	0.0000	0.0000
D	44.750	0.00	180.0	0.0	0.0464	0.5560	0.0000	0.0000
D	33.562	0.00	180.0	0.0	0.0464	0.5560	0.0000	0.0000
D	33.562	0.00	180.0	0.0	0.0454	0.5869	0.0000	0.0000
D	0.000	0.00	180.0	0.0	0.0437	0.6307	0.0000	0.0000

(USA 222-G) - Monopole Spatial Analysis (c)2015 Guymast Inc.

Tel:(416)736-7453 Fax:(416)736-4372 web:www.guymast.com

Processed under license at:

Sabre Towers and Poles on: 15 oct 2018 at: 14:39:02

140' ext. 160' Monopole / Coventry Northwest, CT

MAXIMUM POLE DEFORMATIONS CALCULATED(w.r.t. wind direction)

MAST ELEV ft	DEFLECTIONS (ft)			ROTATIONS (deg)		
	HORIZONTAL ALONG	ACROSS	DOWN	TILT ALONG	ACROSS	TWIST
159.0	11.24C	-0.01R	1.11C	8.07C	-0.01R	0.00I
154.0	10.55C	-0.01R	1.01C	8.03C	-0.01R	0.00I
149.0	9.86C	-0.01R	0.92C	7.92C	-0.01R	0.00I
144.0	9.19C	-0.01R	0.83C	7.75C	-0.01R	0.00I
139.0	8.54C	-0.01R	0.74C	7.51C	-0.01R	0.00I
125.4	6.85C	-0.01R	0.53C	6.88C	-0.01R	0.00I
111.8	5.34C	-0.01R	0.36C	6.04C	-0.01R	0.00I
98.2	4.03C	-0.01R	0.23C	5.06C	-0.01R	0.00I

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92.0	3.50C	0.00R	0.19C	4.70C	-0.01R	0.00I
79.1	2.54C	0.00R	0.11C	3.93C	-0.01R	0.00I
66.2	1.74C	0.00R	0.06C	3.19C	0.00R	0.00I
53.2	1.10C	0.00R	0.03C	2.49C	0.00R	0.00I
44.7	0.77C	0.00R	0.02C	2.05C	0.00R	0.00I
33.6	0.42C	0.00R	0.01C	1.49C	0.00R	0.00I
22.4	0.18C	0.00R	0.00C	0.96C	0.00R	0.00I
11.2	0.04C	0.00R	0.00AC	0.46C	0.00R	0.00I
0.0	0.00A	0.00A	0.00A	0.00A	0.00A	0.00A

MAXIMUM POLE FORCES CALCULATED(w.r.t. to wind direction)

MAST ELEV ft	TOTAL AXIAL kip	SHEAR.w.r.t.WIND.DIR ALONG kip	WIND.DIR ACROSS kip	MOMENT.w.r.t.WIND.DIR ALONG ft-kip	WIND.DIR ACROSS ft-kip	TORSION ft-kip
159.0	9.35 Y	12.24 U	-0.02 N	-0.12 W	-0.04 N	0.01 N
154.0	12.40 Y	12.61 U	-0.02 N	-64.45 E	0.05 N	-0.02 F
149.0	12.39 AI	12.64 I	0.05 L	-64.52 E	0.11 W	0.02 Q
144.0	13.15 AI	13.03 I	0.05 L	-131.69 F	-0.18 L	-0.05 F
139.0	24.68 AI	25.07 H	-0.06 N	-131.74 F	0.26 F	-0.07 F
134.0	25.49 AI	25.48 H	-0.06 N	-264.34 D	0.47 R	-0.08 F
129.0	25.50 AC	25.50 D	-0.10 R	-264.46 D	0.46 R	-0.08 F
124.0	26.35 AC	25.93 D	-0.10 R	-399.58 D	0.94 R	-0.13 R
119.0	36.88 AC	40.83 C	-0.12 C	-399.55 D	0.92 R	-0.13 R
114.0	54.39 AC	53.84 C	-0.12 C	-1032.46 C	2.14 C	-0.28 I
109.0	54.40 AC	53.82 C	-0.09 C	-1032.47 C	2.15 C	-0.28 I
104.0	69.11 AC	66.82 C	-0.09 C	-1894.11 C	3.45 C	-0.48 I
99.0	69.12 AC	66.81 C	0.09 F	-1894.10 C	3.47 C	-0.48 I
94.0	84.11 AC	79.76 C	0.09 F	-2978.19 C	4.67 C	-0.66 I
89.0	84.11 AC	79.81 C	0.15 F	-2978.25 C	4.70 C	-0.66 I
84.0	87.93 AC	80.63 C	0.15 F	-3499.59 C	5.00 C	-0.75 I
79.0	87.93 AC	80.57 C	-0.14 R	-3499.53 C	5.08 C	-0.76 I
74.0	93.47 AC	82.33 C	-0.14 R	-4592.55 C	-5.91 F	-0.88 I
69.0	93.46 AC	82.35 C	0.13 X	-4592.59 C	5.87 R	-0.88 I
64.0	99.51 AC	84.21 C	0.13 X	-5705.93 C	7.40 R	-1.01 I
59.0	99.50 AC	84.20 C	0.14 X	-5705.95 C	7.38 R	-1.00 I
54.0	106.04 AC	86.15 C	0.14 X	-6839.80 C	8.91 R	-1.10 I
49.0	106.04 AC	86.16 C	-0.14 R	-6839.83 C	8.91 R	-1.10 I
44.0	113.77 AC	87.48 C	-0.14 R	-7597.56 C	10.09 R	-1.13 I
39.0	113.77 AC	87.47 C	0.13 X	-7597.53 C	10.08 R	-1.14 I
34.0	119.99 AC	89.19 C	0.13 X	-8608.17 C	11.22 R	-1.16 I

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33.6	119.99 AC	89.21 C	-0.12 R	-8608.17 C	11.21 R	-1.16 I
22.4	126.64 AC	90.92 C	-0.12 R	-9632.47 C	12.54 R	-1.19 I
	126.64 AC	90.92 C	0.13 X	-9632.43 C	12.55 R	-1.19 I
11.2	133.45 AC	92.56 C	0.13 X	-10669.18 C	13.71 R	-1.20 I
	133.45 AC	92.56 C	0.13 X	-10669.16 C	13.69 R	-1.20 I
	140.42 AC	94.23 C	0.13 X	-11717.65 C	15.00 R	-1.21 I
base reaction	140.42 AC	-94.23 C	-0.13 X	11717.65 C	-15.00 R	1.21 I

COMPLIANCE WITH 4.8.2 & 4.5.4

ELEV ft	AXIAL	BENDING	SHEAR + TORSIONAL	TOTAL	SATISFIED	D/t(w/t)	MAX ALLOWED
159.00	0.01Y	0.00W	0.02U	0.01AI	YES	13.05A	45.2
154.00	0.01Y	0.11E	0.02U	0.11E	YES	14.28A	45.2
	0.01AI	0.11E	0.02I	0.11E	YES	14.28A	45.2
149.00	0.01AI	0.19F	0.02I	0.19F	YES	15.52A	45.2
	0.02AI	0.19F	0.04H	0.20F	YES	15.52A	45.2
144.00	0.02AI	0.33D	0.03H	0.34D	YES	16.75A	45.2
	0.02AC	0.33D	0.03D	0.34D	YES	16.75A	45.2
139.00	0.02AC	0.44D	0.03D	0.45D	YES	17.99A	45.2
	0.01AC	0.28D	0.03C	0.29D	YES	11.52A	45.2
125.42	0.02AC	0.53C	0.04C	0.55C	YES	13.76A	45.2
	0.02AC	0.53C	0.04C	0.55C	YES	13.76A	45.2
111.83	0.02AC	0.75C	0.04C	0.76C	YES	16.00A	45.2
	0.02AC	0.75C	0.04C	0.76C	YES	16.00A	45.2
98.25	0.02AC	0.95C	0.04C	0.97C	YES	18.23A	45.2
	0.02AC	0.70C	0.03C	0.71C	YES	13.23A	45.2
92.00	0.02AC	0.74C	0.03C	0.75C	YES	14.01A	45.2
	0.02AC	0.77C	0.03C	0.78C	YES	13.74A	45.2
79.08	0.02AC	0.82C	0.03C	0.83C	YES	15.34A	45.2
	0.02AC	0.82C	0.03C	0.83C	YES	15.34A	45.2
66.17	0.02AC	0.87C	0.03C	0.88C	YES	16.93A	45.2
	0.02AC	0.87C	0.03C	0.88C	YES	16.93A	45.2
53.25	0.02AC	0.90C	0.03C	0.91C	YES	18.53A	45.2
	0.02AC	0.90C	0.03C	0.91C	YES	18.53A	45.2
44.75	0.02AC	0.92C	0.03C	0.93C	YES	19.58A	45.2
	0.02AC	0.94C	0.03C	0.95C	YES	19.22A	45.2
33.56	0.02AC	0.96C	0.03I	0.97C	YES	20.61A	45.2
	0.02AC	0.96C	0.03C	0.97C	YES	20.61A	45.2

	0.02AC	0.97C	0.03I	418393A - Extension 0.98C	YES	21.99A	45.2
22.37	0.02AC	0.97C	0.030	0.98C	YES	21.99A	45.2
11.19	0.02AC	0.98C	0.02I	0.99C	YES	23.37A	45.2
0.00	0.02AC	0.99C	0.02I	1.00C	YES	24.75A	45.2

MAXIMUM LOADS ONTO FOUNDATION(w.r.t. wind direction)

DOWN kip	SHEAR.w.r.t.WIND.DIR ALONG kip	WIND.DIR ACROSS kip	MOMENT.w.r.t.WIND.DIR ALONG ft-kip	WIND.DIR ACROSS ft-kip	TORSION ft-kip
140.42 AC	94.23 C	0.13 X	-11717.65 C	15.00 R	-1.21 I

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140' ext. 160' Monopole / Coventry Northwest, CT

 ***** Service Load Condition *****

* Only 1 condition(s) shown in full
 * Some concentrated wind loads may have been derived from full-scale wind tunnel testing

LOADING CONDITION A

60 mph wind with no ice. Wind Azimuth: 0°

LOADS ON POLE

LOAD TYPE	ELEV ft	APPLY. RADIUS ft	LOAD AZI	AT AZI	LOAD AZI	FORCES HORIZ kip	DOWN kip	MOMENTS VERTICAL ft-kip	TORSNAL ft-kip
C	159.000	0.00	0.0	0.0	0.0	2.4080	3.1000	0.0000	0.0000
C	157.000	0.00	0.0	0.0	0.0	0.0000	1.9594	0.0000	0.0000
C	149.000	0.00	0.0	0.0	0.0	0.0000	1.8595	0.0000	0.0000
C	149.000	0.00	0.0	0.0	0.0	2.3755	3.1000	0.0000	0.0000
C	139.000	0.00	0.0	0.0	0.0	2.9266	3.0000	0.0000	0.0000
C	137.000	0.00	0.0	0.0	0.0	0.0000	2.5646	0.0000	0.0000
C	129.000	0.00	0.0	0.0	0.0	0.0000	1.6099	0.0000	0.0000
C	129.000	0.00	0.0	0.0	0.0	2.3050	3.1000	0.0000	0.0000
C	119.000	0.00	0.0	0.0	0.0	0.0000	1.4851	0.0000	0.0000
C	119.000	0.00	0.0	0.0	0.0	2.2665	3.1000	0.0000	0.0000
C	109.000	0.00	0.0	0.0	0.0	0.0000	1.3603	0.0000	0.0000
C	109.000	0.00	0.0	0.0	0.0	2.2254	3.1000	0.0000	0.0000
D	159.000	0.00	180.0	0.0	0.0	0.0144	0.0585	0.0000	0.0000
D	139.000	0.00	180.0	0.0	0.0	0.0175	0.0725	0.0000	0.0000
D	139.000	0.00	180.0	0.0	0.0	0.0195	0.1228	0.0000	0.0000
D	125.417	0.00	180.0	0.0	0.0	0.0195	0.1228	0.0000	0.0000
D	125.417	0.00	180.0	0.0	0.0	0.0220	0.1419	0.0000	0.0000
D	111.833	0.00	180.0	0.0	0.0	0.0220	0.1419	0.0000	0.0000

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D	111.833	0.00	180.0	0.0	0.0244	0.1609	0.0000
D	98.250	0.00	180.0	0.0	0.0244	0.1609	0.0000
D	98.250	0.00	180.0	0.0	0.0259	0.4041	0.0000
D	92.000	0.00	180.0	0.0	0.0259	0.4041	0.0000
D	92.000	0.00	180.0	0.0	0.0269	0.2472	0.0000
D	79.083	0.00	180.0	0.0	0.0269	0.2472	0.0000
D	79.083	0.00	180.0	0.0	0.0285	0.2714	0.0000
D	66.167	0.00	180.0	0.0	0.0285	0.2714	0.0000
D	66.167	0.00	180.0	0.0	0.0298	0.2956	0.0000
D	53.250	0.00	180.0	0.0	0.0298	0.2956	0.0000
D	53.250	0.00	180.0	0.0	0.0306	0.6266	0.0000
D	44.750	0.00	180.0	0.0	0.0306	0.6266	0.0000
D	44.750	0.00	180.0	0.0	0.0304	0.3295	0.0000
D	33.562	0.00	180.0	0.0	0.0304	0.3295	0.0000
D	33.562	0.00	180.0	0.0	0.0302	0.3505	0.0000
D	22.375	0.00	180.0	0.0	0.0302	0.3505	0.0000
D	22.375	0.00	180.0	0.0	0.0289	0.3714	0.0000
D	11.188	0.00	180.0	0.0	0.0289	0.3714	0.0000
D	11.188	0.00	180.0	0.0	0.0295	0.3924	0.0000
D	0.000	0.00	180.0	0.0	0.0295	0.3924	0.0000

MAXIMUM POLE DEFORMATIONS CALCULATED(w.r.t. wind direction)

MAST ELEV ft	DEFLECTIONS (ft)			ROTATIONS (deg)		TWIST
	HORIZONTAL ALONG	ACROSS	DOWN	TILT ALONG	ACROSS	
159.0	2.25D	0.00B	0.05D	1.60D	0.00B	0.00B
154.0	2.11D	0.00B	0.04D	1.59D	0.00B	0.00B
149.0	1.97D	0.00B	0.04D	1.57D	0.00B	0.00B
144.0	1.83D	0.00B	0.04D	1.54D	0.00B	0.00B
139.0	1.70D	0.00B	0.03D	1.49D	0.00B	0.00C
125.4	1.36D	0.00B	0.02D	1.36D	0.00B	0.00C
111.8	1.06D	0.00B	0.02D	1.19D	0.00B	0.00C
98.2	0.80D	0.00B	0.01D	1.00D	0.00B	0.00C
92.0	0.69D	0.00B	0.01D	0.93D	0.00B	0.00C
79.1	0.50D	0.00B	0.01D	0.78D	0.00B	0.00C
66.2	0.34D	0.00B	0.00D	0.63D	0.00B	0.00C
53.2	0.22D	0.00B	0.00D	0.49D	0.00B	0.00C
44.7	0.15D	0.00B	0.00D	0.41D	0.00B	0.00C
33.6	0.08D	0.00B	0.00D	0.29D	0.00B	0.00C
22.4	0.04D	0.00B	0.00D	0.19D	0.00B	0.00C
11.2	0.01D	0.00B	0.00I	0.09D	0.00B	0.00C
0.0	0.00A	0.00A	0.00A	0.00A	0.00A	0.00A

MAXIMUM POLE FORCES CALCULATED(w.r.t. to wind direction)

MAST ELEV ft	TOTAL AXIAL kip	SHEAR.w.r.t.WIND.DIR		MOMENT.w.r.t.WIND.DIR		TORSION ft-kip
		ALONG kip	ACROSS kip	ALONG ft-kip	ACROSS ft-kip	
159.0	3.10 L	2.41 L	0.00 C	0.01 F	0.01 E	0.00 E
154.0	5.36 L	2.48 L	0.00 C	-12.84 I	0.02 C	0.00 K
	5.36 D	2.49 F	-0.01 L	-12.84 K	0.01 C	0.00 K

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149.0	5.68 D	2.56 F	-0.01 L	-26.20 F	0.06 L	0.00 L
	10.64 C	4.94 F	-0.01 L	-26.21 F	0.05 L	0.00 L
144.0	10.98 C	5.02 F	-0.01 L	-52.57 F	0.10 L	0.00 L
	10.98 C	5.03 F	-0.01 F	-52.56 F	0.09 L	0.00 L
139.0	11.33 C	5.12 F	-0.01 F	-79.38 F	0.09 L	0.00 L
	14.33 C	8.05 D	0.01 B	-79.37 F	0.09 L	0.00 L
125.4	23.27 C	10.62 D	0.01 B	-204.89 D	-0.23 B	0.01 B
	23.27 B	10.62 D	0.01 H	-204.89 D	-0.23 B	0.01 B
111.8	29.78 B	13.19 D	0.01 B	-375.48 D	-0.41 B	0.01 B
	29.78 I	13.19 D	0.01 B	-375.47 D	-0.42 B	0.01 B
98.2	36.43 I	15.75 D	0.01 B	-589.68 D	-0.58 B	-0.02 E
	36.43 B	15.74 F	0.02 B	-589.68 D	-0.58 B	0.02 C
92.0	38.95 I	15.90 F	0.02 B	-692.51 D	-0.68 B	0.02 C
	38.96 B	15.91 D	0.02 B	-692.50 D	-0.67 B	0.02 C
79.1	42.15 B	16.26 D	0.02 B	-908.01 D	-0.94 B	0.02 C
	42.15 B	16.26 D	0.02 I	-908.00 D	-0.95 B	0.02 B
66.2	45.65 B	16.62 D	0.02 I	-1127.25 D	-1.15 B	-0.03 E
	45.65 B	16.63 D	0.02 I	-1127.26 D	-1.15 B	0.03 C
53.2	49.47 B	17.01 D	0.02 I	-1350.47 D	-1.35 B	0.03 C
	49.47 B	17.01 D	0.01 B	-1350.47 D	-1.35 B	0.03 C
44.7	54.80 B	17.27 D	0.01 B	-1499.64 D	-1.47 B	0.03 C
	54.80 B	17.27 D	0.01 C	-1499.66 D	-1.48 B	0.03 C
33.6	58.48 B	17.61 D	0.01 C	-1698.67 D	-1.57 B	0.04 C
	58.48 B	17.62 D	0.01 C	-1698.67 D	-1.58 B	0.04 C
22.4	62.40 B	17.96 D	0.01 C	-1900.49 D	-1.67 B	0.04 C
	62.40 B	17.95 D	0.01 B	-1900.49 D	-1.67 B	0.04 C
11.2	66.56 B	18.28 D	0.01 B	-2104.91 D	-1.77 B	0.04 C
	66.56 B	18.28 D	0.01 L	-2104.91 D	-1.77 B	0.04 C
	70.95 B	18.61 D	0.01 L	-2311.89 D	-1.88 B	0.04 C
base reaction	70.95 B	-18.61 D	-0.01 L	2311.89 D	1.88 B	-0.04 C

COMPLIANCE WITH 4.8.2 & 4.5.4

ELEV ft	AXIAL	BENDING	SHEAR + TORSIONAL	TOTAL	SATISFIED	D/t(w/t)	MAX ALLOWED
159.00	0.00L	0.00F	0.00L	0.00F	YES	13.05A	45.2
154.00	0.00L	0.02I	0.00L	0.03I	YES	14.28A	45.2
	0.00D	0.02K	0.00F	0.03K	YES	14.28A	45.2
149.00	0.00D	0.04F	0.00F	0.04F	YES	15.52A	45.2

				418393A - Extension			
	0.01C	0.04F	0.01F	0.04F	YES	15.52A	45.2
144.00	0.01C	0.06F	0.01F	0.07F	YES	16.75A	45.2
	0.01C	0.06F	0.01F	0.07F	YES	16.75A	45.2
139.00	0.01C	0.09F	0.01F	0.09F	YES	17.99A	45.2
	0.01C	0.06F	0.01D	0.06F	YES	11.52A	45.2
125.42	0.01C	0.11D	0.01D	0.11D	YES	13.76A	45.2
	0.01B	0.11D	0.01D	0.11D	YES	13.76A	45.2
111.83	0.01B	0.15D	0.01D	0.16D	YES	16.00A	45.2
	0.01I	0.15D	0.01D	0.16D	YES	16.00A	45.2
98.25	0.01I	0.19D	0.01D	0.20D	YES	18.23A	45.2
	0.01B	0.14D	0.01F	0.15D	YES	13.23A	45.2
92.00	0.01I	0.15D	0.01F	0.15D	YES	14.01A	45.2
	0.01B	0.15D	0.01D	0.16D	YES	13.74A	45.2
79.08	0.01B	0.16D	0.01D	0.17D	YES	15.34A	45.2
	0.01B	0.16D	0.01D	0.17D	YES	15.34A	45.2
66.17	0.01B	0.17D	0.01D	0.18D	YES	16.93A	45.2
	0.01B	0.17D	0.01D	0.18D	YES	16.93A	45.2
53.25	0.01B	0.18D	0.01D	0.19D	YES	18.53A	45.2
	0.01B	0.18D	0.01D	0.19D	YES	18.53A	45.2
44.75	0.01B	0.18D	0.01D	0.19D	YES	19.58A	45.2
	0.01B	0.19D	0.01D	0.19D	YES	19.22A	45.2
33.56	0.01B	0.19D	0.01D	0.20D	YES	20.61A	45.2
	0.01B	0.19D	0.01D	0.20D	YES	20.61A	45.2
22.37	0.01B	0.19D	0.00D	0.20D	YES	21.99A	45.2
	0.01B	0.19D	0.00D	0.20D	YES	21.99A	45.2
11.19	0.01B	0.19D	0.00D	0.20D	YES	23.37A	45.2
	0.01B	0.19D	0.00D	0.20D	YES	23.37A	45.2
0.00	0.01B	0.19D	0.00D	0.20D	YES	24.75A	45.2

MAXIMUM LOADS ONTO FOUNDATION(w.r.t. wind direction)

DOWN	SHEAR.w.r.t.WIND.DIR	MOMENT.w.r.t.WIND.DIR	TORSION
kip	ALONG	ALONG	ft-kip
	ACROSS	ACROSS	
	kip	ft-kip	
70.95	18.61	-2311.89	0.04
B	D	D	C

Round Flange Plate and Bolts per ANSI/TIA 222-G
Elevation = 139 feet

Pole Data

Diameter: 28 in
Thickness: 0.25 in
Yield (Fy): 65 ksi
of Sides: 18 "0" IF Round
Strength (Fu): 80 ksi

Reactions

Moment, Mu: 399.58 ft-kips
Axial, Pu: 17.25 kips
Shear, Vu: 40.83 kips

Bolt Data

Quantity: 12
Diameter: 1 in
Bolt Material: A325
Strength (Fu): 120 ksi
Yield (Fy): 92 ksi
BC Diam. (in): 31.5 BC Override:

Flange Bolt Results

Allowable Φ *Rnt: 54.54 kips
Adjusted Φ *Rnt (due to shear): 54.33 kips
Maximum Bolt Tension: 49.30 kips
Bolt Interaction Ratio: **90.7% Pass**

Plate Data

Diameter (in): 34 Dia. Override:
Thickness: 1.5 in
Center Hole Diam.: 18 in
Yield (Fy): 50 ksi
Single-Rod B-eff: 7.41 in
Drain Hole: 1 in. diameter
Drain Location: 13 in. center of pole to center of drain hole

Flange Plate Results

Compression Side Plate (Mu/Z): 15.1 ksi
Allowable Φ *Fy: 45.0 ksi
Compr. Plate Interaction Ratio: **33.5% Pass**

Square Base Plate and Anchor Rods per ANSI/TIA 222-G

Pole Data

Diameter: 75.180 in (flat to flat)
Thickness: 0.5 in
Yield (Fy): 65 ksi
of Sides: 18 "0" IF Round
Strength (Fu): 80 ksi

Reactions

Moment, Mu: 11717.65 ft-kips
Axial, Pu: 85.11 kips
Shear, Vu: 94.23 kips

Anchor Rod Data

Quantity: 28 (multiple of 4)
Diameter: 2.25 in
Rod Material: A615
Strength (Fu): 100 ksi
Yield (Fy): 75 ksi
BC Diam. (in): 82.75 BC Override:
Rod Spacing: 6 in

Anchor Rod Results

Maximum Rod (Pu+ Vu/η): 252.5 Kips
Allowable Φ *Rnt: 260.0 Kips (per 4.9.9)
Anchor Rod Interaction Ratio: **97.1% Pass**

Plate Data

Width (in): 83.5 Width Override:
Thickness: 2.75 in
Yield (Fy): 50 ksi
Eff. Width: 42.91 in
Corner Clip: 21.00 in
Drain Hole: 2.625 in. diameter
Drain Location: 35.5 in. center of pole to center of drain hole
Center Hole: 63 in. diameter

Base Plate Results

Base Plate (Mu/Z): 43.5 ksi
Allowable Φ *Fy: 45 ksi (per AISC)
Base Plate Interaction Ratio: **96.6% Pass**

MAT FOUNDATION DESIGN BY SABRE TOWERS & POLES

160' Monopole VERIZON WIRELESS Coventry Northwest, CT (418393) 10/15/18 NM

Overall Loads:

Factored Moment (ft-kips)	11717.65
Factored Axial (kips)	85.11
Factored Shear (kips)	94.23
Bearing Design Strength (ksf)	6
Water Table Below Grade (ft)	10
Width of Mat (ft)	33.5
Thickness of Mat (ft)	2
Depth to Bottom of Slab (ft)	6
Quantity of Bolts in Bolt Circle	28
Bolt Circle Diameter (in)	82.75
Top of Concrete to Top of Bottom Threads (in)	60
Diameter of Pier (ft)	10
Ht. of Pier Above Ground (ft)	0.5
Ht. of Pier Below Ground (ft)	4
Quantity of Bars in Mat	74
Bar Diameter in Mat (in)	1.128
Area of Bars in Mat (in ²)	73.95
Spacing of Bars in Mat (in)	5.41
Quantity of Bars Pier	58
Bar Diameter in Pier (in)	1.128
Tie Bar Diameter in Pier (in)	0.625
Spacing of Ties (in)	12
Area of Bars in Pier (in ²)	57.96
Spacing of Bars in Pier (in)	6.05
f'c (ksi)	4.5
fy (ksi)	60
Unit Wt. of Soil (kcf)	0.125
Unit Wt. of Concrete (kcf)	0.15

Volume of Concrete (yd³) 96.22

Two-Way Shear Action:

Average d (in)	19.872
ϕV_c (ksi)	0.217
$\phi V_c = \phi(2 + 4/\beta_c)f'_c$	0.342
$\phi V_c = \phi(\alpha_s d/b_o + 2)f'_c$	0.217
$\phi V_c = \phi 4f'_c$	0.228
Shear perimeter, b _o (in)	439.42
β_c	1

One-Way Shear:

ϕV_c (kips) 911.0

Stability:

Overturning Design Strength (ft-k) 14810.7

Max. Net Bearing Press. (ksf) 5.34

Allowable Bearing Pressure (ksf) 4.00

Safety Factor 2.00

Ultimate Bearing Pressure (ksf) 8.00

Bearing Φ_s 0.75

Minimum Pier Diameter (ft) 8.23

Equivalent Square b (ft) 8.86

Square Pier? (Y/N) N

Recommended Spacing (in) 5 to 12

Minimum Pier A_s (in²) 56.55

Recommended Spacing (in) 5 to 12

v_u (ksi) 0.199

V_u (kips) 637.0

Total Applied M (ft-k) 12330.1

Pier Design:

ϕV_n (kips)	1318.7	V_u (kips)	94.2
$\phi V_c = \phi 2(1 + N_u / (2000 A_g)) f'_c{}^{1/2} b_w d$	1318.7		
V_s (kips)	0.0	*** V_s max = $4 f'_c{}^{1/2} b_w d$ (kips)	3091.1
Maximum Spacing (in)	6.10	(Only if Shear Ties are Required)	
Actual Hook Development (in)	18.74	Req'd Hook Development l_{dh} (in)	13.78
		*** Ref. To Spacing Requirements ACI 11.5.4.3	

Flexure in Slab:

ϕM_n (ft-kips)	6132.8	M_u (ft-kips)	6110.7
a (in)	2.89		
Steel Ratio	0.00926		
β_1	0.825		
Maximum Steel Ratio (ρ_t)	0.0197		
Minimum Steel Ratio	0.0018		
Rebar Development in Pad (in)	144.83	Required Development in Pad (in)	31.39

Condition	1 is OK, 0 Fails
Maximum Soil Bearing Pressure	1
Pier Area of Steel	1
Pier Shear	1
Interaction Diagram Visual Check	1
Two-Way Shear Action	1
One-Way Shear Action	1
Overtuning	1
Flexure	1
Steel Ratio	1
Length of Development in Pad	1
Hook Development	1