$\begin{array}{c} C UDDY \& \\ F E D E R^{\text{LLP}} \end{array}$

445 Hamilton Avenue, 14th Floor White Plains, New York 10601 Tel 914.761.1300 Fax 914.761.5372 www.cuddyfeder.com

April 12, 2012

BY EMAIL & FEDEX

Ms. Linda Roberts Executive Director Connecticut Siting Council 10 Franklin Square New Britain, Connecticut 06051

Re: Docket 420 SBA Towers III – Development and Management Plan 350B Cossaduck Hill Road, North Stonington, Connecticut

Dear Ms. Roberts:

On behalf of SBA Towers III please accept for review and Council approval this Development and Management Plan ("D&M Plan") filing for the captioned Facility as approved in Docket 420.

Tower, Compound & Other Equipment

Enclosed are fifteen (15) sets of 11" x 17" construction drawings being filed in accordance with the Council's Decision and Order dated February 2, 2012. Two full-sized sets of the construction drawings will follow under separate cover. The D&M Plan incorporates a 190' monopole as provided for in the Siting Council's Order No. 1 in this Docket. AT&T will mount nine (9) panel antennas on a low profile platform at a centerline height of 186' AGL as depicted on the signed and stamped drawings prepared by Centek Engineering dated March 19, 2012. The proposed D&M Plan also includes construction plans for the site clearing, drainage, and erosion and sedimentation control measures consistent with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control as amended.

Attached please also find a geotechnical study, structural design report for the tower and foundation, specifications for AT&T's antennas and generator as well as a letter from Centek Engineering regarding the designed access drive.

Required Notifications

In accordance with the provisions of RCSA Section 16-50j-77, SBA hereby notifies the Council of its intention to begin site work after Council approval of the D&M Plan. Construction of the tower and other site improvements will commence upon issuance of a local building permit. The construction director on this project is Shawn McCoy of SBA Communications Corporation. Mr. McCoy is located at 5900 Broken Sound Parkway NW, Boca Raton, Florida and can be



reached by telephone at (561) 226-9366, and the local contact is Hollis Redding, Zoning Specialist. Ms. Redding is located at One Research Drive, Suite 200C, Westborough, MA and can be reached by telephone at (203) 464-3623.

We respectfully request that this matter be included on the Council's next available agenda for review and approval.

Thank you for your consideration of the enclosed.

Very truly yours,

1

Daniel M. Laub

Enclosures

 cc: Hon. Michael H. Mullane, II, First Selectman, Town of North Stonington Peter R. and Gisele A. Buehler, Intervenors Hollis Redding, SBA Jason Laskey, SBA Michele Briggs, AT&T Christopher B. Fisher, Esq. Attachment 1

Date: March 9, 2012

Shawn McCoy SBA Communications Corporation 5900 Broken Sound Boca Raton, FL 33487 Office: (561) 226-9366 Tower Engineering Professionals, Inc. 3703 Junction Boulevard Raleigh, NC 27603 (919) 661-6351 <u>Geotech@tepgroup.net</u>

Subject: Subsurface Exploration Report

SBA Designation:	Site Number: Site Name:	CT11796-S N. Stonington 3
Engineering Firm Designation:	TEP Project Number:	121203.10
Site Data:	350 B Cossaduck Hill Road, North (New London County) Latitude N <i>41 ° 29' 57.236''</i> , Longitu 190 Foot - Proposed Monopole To	stonington, CT 06359 de W <i>71 ° 53' 22.277"</i> wer

Dear Mr. McCoy,

Tower Engineering Professionals, Inc. is pleased to submit this **"Subsurface Exploration Report"** to evaluate subsurface conditions in the tower area as they pertain to providing support for the tower foundation.

This report has been prepared in accordance with generally accepted geotechnical engineering practice for specific application to this project. The conclusions in this report are based on the applicable standards of TEP's practice in this geographic area at the time this report was prepared. No other warranty, express or implied, is made.

TEP assumes the current ground surface elevation; tower location and subsequent centerline provided are correct and are consistent with the elevation and centerline to be used for construction of the structure. Should the ground surface elevation be altered and/or the tower location be moved or shifted TEP should be contacted to determine if additional borings are necessary.

The analyses and recommendations submitted herein are based, in part, upon the data obtained from the subsurface exploration. The soil conditions may vary from what is represented in the boring log. While some transitions may be gradual, subsurface conditions in other areas may be quite different. Should actual site conditions vary from those presented in this report, TEP should be provided the opportunity to amend its recommendations as necessary.

We at *Tower Engineering Professionals, Inc.* appreciate the opportunity of providing our continuing professional services to you and SBA Communications Corporation. If you have any questions or need further assistance on this or any other projects please give us a call.

Report Prepared/Reviewed by: Cory A. Bauer / John D. Longest, P.E.

Respectfully submitted by Pete Jernigan, P.E





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1) PROJECT DESCRIPTION

Based on the preliminary drawings, it is understood a monopole communications tower will be constructed at the referenced site. The structure loads will be provided by the tower manufacturer.

2) SITE EXPLORATION

The field exploration included the performance of one soil test boring (B-1) to the auger refusal depth of 8 feet (bgs) at the approximate centerline of the proposed monopole tower. The boring was performed by an ATV mounted drill rig using continuous flight hollow stem augers to advance the hole. Split-spoon samples and Standard Penetration Resistance Values (N-values) were obtained in accordance with ASTM D 1586 at a frequency of 2 samples to auger refusal.

The Split-spoon samples were transported to the TEP laboratory where they were classified by a Geotechnical Engineer in general accordance with the Unified Soil Classification System (USCS), using visual-manual identification procedures (ASTM D 2488).

Diamond-bit core drilling procedures were used to help determine the character and continuity of the rock in boring B-1. The core drilling procedures were in accordance with ASTM Specification D-2113. Rock core samples of the materials penetrated were protected and retained in a swivel-mounted inner tube of the core barrel. Upon completion of the drill run, the core barrel was brought to the surface and samples removed and placed in standard boxes. The samples were classified by a Geotechnical Engineer and the "Recovery" and "Rock Quality Designation" were determined.

The "Recovery" is the ratio of the sample length obtained to the length drilled, expressed as a percent. The "Rock Quality Designation" (RQD) is the percent of the recovered rock samples in lengths of four or more inches, compared to the total length of the core run. This designation is generally applied to samples of NWX size (2-1/8 inch diameter) or larger and to samples described as moderately hard or harder. The percent recovery and RQD are related to rock soundness and continuity. Generalized rock descriptions, percent recovery, and the RQD value are shown on the boring log.

A Boring Location Plan showing the approximate boring location, a Boring Log presenting the subsurface information obtained and a brief guide to interpreting the boring log are included in the Appendix.

3) SITE CONDITIONS

The site is located at 350 B Cossaduck Hill Road in North Stonington, New London County, Connecticut. The proposed tower and compound are to be located in a small clearing surrounded by woodlands. The ground topography is sloping.



4) SUBSURFACE CONDITIONS

The following description of subsurface conditions is brief and general. For more detailed information, the individual Boring Log contained in Appendix B - Boring Log may be consulted.

4.1) Soil

The USCS classification of the materials encountered in the boring include SP, SW and Gneiss. The Standard Penetration Resistance ("N" Values) recorded in the materials ranged from 5 blows per foot to 120 blows per 0 inches of penetration.

4.2) Rock

Gneiss was encountered at a depth of 8 feet (bgs) in the boring. Refusal of auger advancement was encountered at a depth of 8 feet (bgs) in the boring.

4.3) Subsurface Water

Subsurface water was not encountered in the boring at the time of drilling. It should be noted the subsurface water level will fluctuate during the year, due to seasonal variations and construction activity in the area.

4.4) Frost

The TIA frost depth for New London County Connecticut is 40 inches.

5) TOWER FOUNDATION DESIGN

Based on the boring data and the shallow depth of rock, it is the opinion of TEP that a pier extending to a single large mat foundation can be used to support the new tower. The following presents TEP's conclusions and recommendations regarding the foundation type.

5.1) Shallow Foundation

The foundation should bear a minimum of 3.5 feet below the ground surface to penetrate the frost depth and with sufficient depth to withstand the overturning of the tower. To resist the overturning moment, the weight of the concrete and any soil directly above the foundation can be used. A friction factor of 0.50 can be utilized at this depth. The values are based on the current ground surface elevation.

De	pth	Soil	Static	Cohesion ^{2,3}	Friction Angle ²	Effective Unit
Тор	Bottom	5011	(psf)	(psf)	(degrees)	Weight (pcf)
0	3	SP	3200	-	29	116
3	8	SW	17350	-	45	123
8	13	Gneiss ³	18650	_	45	145

Table 1A – Shallow Foundation Analysis Parameters – Boring B-1

Notes:

The bearing values provided are net allowable with a minimum factor of safety of 2 with anticipated settlement less than 1) 1 inch. Bearing may be increased by 1/3 for transient loading (e.g. wind or earthquake loading)

2) These values should be considered ultimate soil parameters

Due to the fractured nature of the rock sample. Cohesion of the rock cannot be relied upon for strength parameters. 3) Indicated layers have been evaluated as a granular material

De	pth	Bock	Recovery	Rock Quality	Unconfined Compressive	Grout/Rock ^{1,2}	Effective Unit
Тор	Bottom	HOUR	(%)	Designation (%)	Strength (psi)	(psi)	Weight (pcf)
8	13	Gneiss ³	100	53	6700	-	145

Table 1B – Rock Parameters – Boring B-1

Notes:

These values should be considered ultimate rock parameters. A minimum factor of safety of 4 should be utilized 1) 2)

The rock encountered is not considered competent, see section 5.2 for design recommendations

In cases where the shear failure is likely to develop along planes of discontinuity or through highly fractured rock 3) masses cohesion cannot be relied upon to provide resistance to failure

5.2) Rock Anchor Foundations

A rock anchor foundation is not recommended at the reference site. Competent rock was not encountered at the time of the exploration. Rock anchor design considerations can be provided upon request.

6) SOIL RESISTIVITY

Soil resistivity was performed at the TEP laboratory in accordance with ASTM G187-05 (Standard Test Method for Measurement of Soil Resistivity Using the Two Electrode Soil Box Method). Test results indicated a result of 145,000 ohms/cm.

7) CONSTRUCTION CONSIDERATIONS - SHALLOW FOUNDATION

7.1) Excavation

The boring data indicates excavation to the expected subgrade level for the shallow foundation will extend through sand and gneiss bedrock. A large tracked excavator should be able to remove the sand with minimal difficulty. A large tracked excavator with rock teeth and/or a pneumatic hammer will be necessary to remove the gneiss bedrock with difficulty. TEP anticipates the depth to the surface of the rock will vary outside of the boring location. Boulders and bedrock outcroppings are common to this geographic region and may also be encountered in the excavation area.

Excavations should be sloped or shored in accordance with local, state and federal regulations, including OSHA (29 CFR Part 1926) excavation trench safety standards. It is the responsibility of the contractor for site safety. This information is provided as a service and under no circumstance should TEP be assumed responsible for construction site safety.

7.2) Foundation Evaluation/Subgrade Preparation

After excavation to the design elevation for the footing, the materials should be evaluated by a Geotechnical Engineer or a representative of the Geotechnical Engineer prior to reinforcement and concrete placement. This evaluation should include probing, shallow hand auger borings and dynamic cone penetrometer testing (ASTM STP-399) to help verify that suitable residual material lies directly under the foundation and to determine the need for any undercut and replacement of unsuitable materials. Loose surficial material should be compacted in the excavation prior to reinforcement and concrete placement to stabilize surface soil that may have become loose during the excavation process. TEP recommends a 6-inch layer of compacted crushed stone be placed just after excavation to aid in surface stability.

If the foundation excavation shows that only a portion of the foundation will bear on rock, with a portion bearing on soil, then the entire footprint should be over-excavated by a minimum of 4 inches and the bearing elevation should be re-established with a coarse graded aggregate.

7.3) Fill Placement and Compaction

Backfill materials placed above the shallow foundation to the design subgrade elevation should not contain more than 5 percent by weight of organic matter, waste, debris or any otherwise deleterious materials. To be considered for use, backfill materials should have a maximum dry density of at least 100 pounds per cubic foot as determined by standard Proctor (ASTM D 698), a Liquid Limit no greater than 40, a Plasticity Index no greater than 20, a maximum particle size of 4 inches, and 20 percent or less of the material having a particle size between 2 and 4 inches. Because small handheld or walkbehind compaction equipment will most likely be used, backfill should be placed in thin horizontal lifts not exceeding 6 inches (loose).

Fill placement should be monitored by a qualified Materials Technician working under the direction of a Geotechnical Engineer. In addition to the visual evaluation, a sufficient amount of in-place field density tests should be conducted to confirm the required compaction is being attained.

7.4) Reuse of Excavated Soil

The sand that meets the above referenced criteria can be utilized as backfill based on dry soil and site conditions at the time of construction.

If variability in the subsurface materials is encountered, a representative of the Geotechnical Engineer should verify that the design parameters are valid during construction. Modification to the design values presented above may be required in the field.

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4	



APPENDIX A

BORING LAYOUT





APPENDIX B

BORING LOG



Project: CT11796-S N. Stonington 3 Project Location: North Stonington, Connecticut Project Number: 121203.10

Key to Log of Boring

Sheet 1 of 1



- 1. Soil classifications are based on the Unified Soil Classification System. Descriptions and stratum lines are interpretive, and actual lithologic changes may be gradual. Field descriptions may have been modified to reflect results of lab tests.
- 2. Descriptions on these logs apply only at the specific boring locations and at the time the borings were advanced. They are not warranted to be representative of subsurface conditions at other locations or times.

Project: CT11796-S N. Stonington 3 Project Location: North Stonington, Connecticut Project Number: 121203.10

Log of Boring B-1

Sheet 1 of 1

Date(s) Drilled March 1, 2012	Logged By Cory Bauer	Checked By John Longest
Drilling Method Hollow Stem Auger	Drill Bit Size/Type	Total Depth of Borehole 13 feet bgs
Drill Rig Type ATV	Drilling Contractor TEP	Approximate Surface Elevation 444 feet AMSL
Groundwater Level and Date Measured Not Encountered ATD	Sampling Method(s) SPT, Other	Hammer Data 140 lb, 30 in drop, Auto Hammer
Borehole Backfill Cuttings	Location Approximate centerline of the prop	posed monopole tower

Elevention foot	Elevation, reet	Depth, feet	Sample Type	Sample Number	Sampling Resistance, blows/foot	Relative Consistency	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	REMARKS AND OTHER TESTS
3 N Stonington 3/Geotech/Borling B-1:bgs [Basic Borling Log.zb]] 439 439- 429-				S1 S2 S3 R1	5 80 120/0"	Loose Very Dense	SW		Dark brown, fine to medium, poorly graded (SP), trace silt, moist Light brown and light gray, fine to coarse, well graded SAND (SW), with gravel, wet No Recovery Auger refusal - Rock Core - Gray, moderately fractured GNEISS (Recovery 100%, Rock Quality Designation 53%, Unconfined Compressive Strength 6700 psi) Bottom of Boring at 13 feet bgs	Rec=100%, RQD=53%
Q:\1203										Figure 1

Attachment 2

March 26, 2012



Mr. Shawn McCoy SBA Network Services Inc. 5900 Broken Sound Parkway NW Boca Raton, FL 33487

RE: Proposed 190 ft Sabre Monopole for North Stonington 3, CT

Dear Mr. McCoy,

The above referenced Sabre monopole is to be designed for a Basic Wind Speed of 115 mph (no ice), 50 mph (0.75" ice), Structure Class II, Exposure Category B, and Topographic Category 3 with a crest height of 100', in accordance with the Telecommunications Industry Association Standard ANSI/TIA-222-G, "Structural Standard for Antenna Supporting Structures and Antennas".

When designed according to this standard, the wind pressures and steel strength capacities include several safety factors, resulting in an overall minimum safety factor of 25%. Therefore, it is highly unlikely that the monopole will fail structurally in a wind event where the design wind speed is exceeded within the range of the built-in safety factors.

Should the wind speed increase beyond the capacity of the built-in safety factors, to the point of failure of one or more structural elements, the most likely location of the failure would be within one of the monopole shaft sections. This would result in a buckling failure mode, where the steel shaft would bend beyond its elastic limit (beyond the point where the shaft would return to its original shape upon removal of the wind load).

Therefore, the overall effect of an extreme wind event would be localized buckling of the monopole shaft. Assuming that the wind pressure profile is similar to that used to design the monopole, the shaft will buckle at the location of the highest combined stress ratio in the upper portion of the monopole. This would result in the portion of the monopole above "folding over" onto the portion below, essentially collapsing upon itself. *Please note that this letter only applies to a monopole designed and manufactured by Sabre Towers & Poles*. In the unlikely event of total separation, the monopole will be designed to collapse within a radius of 50% of the tower height from the base.

Sincerely,

Robert E. Beacom, P.E. Engineer



 Guyed Towers
 Self-Supporting Towers
 Monopoles
 Concealment Structures
 Turnkey Installations
 Tower Modifications

 Image: Concealment Structure
 Image: Concealment Structure

2101 Murray Street | P.O. Box 658 | Sioux City, IA 51102-0658 | Phone 712.258.6690 | Fax 712.279.0814 | www.SabreTowersandPoles.com

Sabre Industries Towers and Poles
Structural Design Report
190' Monopole
Site Number: CT11796-S
prepared for: SBA NETWORK SERVICES INC
by: Sabre Towers & Poles ™
Job Number: 57617
March 26, 2012
Monopole Profile. 1 Foundation Design Summary. 2 Pole Calculation. C1-C10 Foundation Calculations. A1-A2
Monopole by TRT Foundation by PAR Approved by

and delate rates of

Section	4	e	2	1
Length (ft)	53'-3"	536°	48-9	510.
Number Of Sides			18	
Thickness (in)		-21/16-	-8/6	1/4"
Lap Splice (ft)		7 3"	5 8.	ح ا
Top Diamoter (in)	49.077775°	39.88415"	31.27305"	21.65"
Bottom Diameter (in)	60.6703"	51.5311"	41.885925*	32.7527*
Taper (in/tt)			0.2177	
Grade			A572-85	
Weight (Ibs)	18977	12147	7592	4461



Designed Appurtenance Loading

Tx-Line

(24) 1 5/8"

(18) 1 5/8"

(18) 1 5/8"

(18) 1 5/8"

(12) 1 5/8"

20.2

19.5

4.4

3.1

Bolt Qty

24

Туре

A615-75

1682

1180

Sway (deg)

12.11

11.64

2 66

1.82

Bolt Diameter

2 25"

Finish

Galv-18

Page:

-4' † 10 5' x 25 5' @ 180',380'		3/2 k (C	
Sabre Communications Corporation	Job:	57617	
P.O. Box 658	Customer:		
Sigury City 14 51102-0658	- 252-042-022000-MAC	SBA NETWORK SERVICES INC	

By: TRJ

	Material List	
isplay	Value	
A	4' - 6"	

Α	4' - 6"
	Notes

3) All dimensions are above ground level, unless otherwise specified.

4) The Monopole was designed for a basic wind speed of 115 mph with 0" of radial ice, and 50 mph with 3/4" of radial ice, in accordance with ANSI/TIA-222-G, Structure Class II, Exposure Category B, Topographic Category 3,

5) Weights shown are estimates. Final weights may vary.

Description:

Date:

190' Monopole

3/26/2012



Sabre Industries



No.: 57617 Page: 2 Date: 3/26/12 By: REB

Customer: SBA NETWORK SERVICES INC Site: North Stonington 3, CT CT11796-S

190' Monopole at 115 mph Wind with no ice and 50 mph Wind with 0.75 in. Ice per ANSI/TIA-222-G. Antenna Loading per Page 1



ELEVATION VIEW (72.97 Cu. Yds. each) (1 REQUIRED; NOT TO SCALE)



Typical pier cross-section

Notes:

1). Concrete shall have a minimum 28-day compressive strength of 4000 PSI, in accordance with ACI 318-05

2). Rebar to conform to ASTM specification A615 Grade 60.

3). All rebar to have a minimum of 3" concrete cover.

4). All exposed concrete corners to be chamfered 3/4".

5). The foundation design is based on the geotechnical report by TEP project no. 121203.10, dated: 3/9/12

6). See the geotechnical report for compaction requirements, if specified.

7). The foundation is based on the following factored loads: Moment (kip-ft) = 7779.17 Axial (kips) = 78.74 Shear (kips) = 60.41

	Rebar Schedule per Pad and Pier
Pier	(40) #10 vertical rebar w/hooks at bottom w/#5 ties, two within top 5" of top of pier then 12" C/C
Pad	(57) #9 horizontal rebar evenly spaced each way top and bottom (228 Total)

8). This is a design drawing only. Please see final construction drawings for all installation details.

9). The foundation is designed for a 15% increase in loads shown in note 7.

Information contained herein is the sole property of Sabre Towers & Poles, constitutes a trade secret as defined by Iowa Code Ch. 550 and shall not be reproduced, copied or used in whole or part for any purpose whatsoever without the prior written consent of Sabre Towers & Poles.

						_								
SABRE COM	AUNIC	ATIC	ONS	CORP)		JOB: (00-5761	17		26-	Mar-	12 07:	39
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SABRE COMMUNICATIONS CORP	JOB: 00-57617	26-Mar-12 07:39
2101 Murray Street SBA	NETWORK SERVICES INC	Ph 712.258.6690
Sioux City, IA 51101 No	rth Stonington 3, CT	Fx 712.258.8250
CASE - 1: 3s Gusted Wind		ANSI-TIA-222-G
WIND OLF 1.60 VERTICAL OLF 1.20 DESIGN ICE .00 in GUST FACTOR (Gh) 1.10 FORCE COEFF (Cf) .65 IMPORTANCE FAC (I) 1.00 DIRECTION FAC (Kd) .95 TOPOGRAPHIC CAT 3 APPURTENANCE LOADS	GUSTED WIND (3sec)115.0EXP-CAT/STRUC CLASSB-IIEXP-POWER COEFF28REFERENCE HEIGHT1200.0PRESSURE @ 104.2 ft56.6BASE ABOVE Grd1.0CREST HEIGHT100.0	mph 185.1 kph 57 ft psf 2708.7 Pa ft
C	enter WEIGHT AREA Tx-CABL	E FORCES MOM.
# Qty Description E	lev-Ft Lbs Ft ² Type Qty	#/Ft Psf Kips Kips Ft-K
1 1 14' LP Platform with Handrail (R	187.0 1704 92.0	68.6 6.31 -2.0 -9.5
- 12 DB848H90E-XY. 6 TMA	189.0 28 1 5/8" 24 189.0 8 None 1	1.04 68.6 -6.0 .00 68.61
2 1 14' LP Platform with Handrail (R	179.0 1704 92.0 179.0 28 1 5/8" 18	68.0 6.26 -2.0 -9.4
6 TMA	179.0 8 None 1	.00 67.91
- 3 1 14' LP Platform with Handrall (R 12 DB848H90E-XY.	169.0 1704 92.0 169.0 28 1 5/8" 18	1.04 67.2 -4.2
6 TMA 4 1 14' LP Platform with Handrail (R	169.0 8 None 1 159.0 1704 92.0	$.00\ 67.2\1\ 66.6\ 6.13\ -2.0\ -9.2$
- 12 DB848H90E-XY.	159.0 28 1 5/8" 18 159.0 8 None 1	1.04 66.5 -4.0
5 2 Pipe Mount (up to 6' Dish)	149.0 49 .1 149.0 220 24 4 1 5 (8 12	65.8 .011 .0
2 6' SOLID DISK W/ RADOME	149.0 550 24.4 1 5/6 12	1.04 05.0 5.21 -5.0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	RCES, kips MOMENTS, ft-k BendX BendX BendY 01 -1 0 0 8.22 -6.9 -9.7 0 8.68 -7.2 -51.1 0 16.56 -12.6 -86.6 0 17.04 -13.1 -169.3 0 24.87 -18.5 -263.8 0 32.98 -24.4 -524.1 0 33.41 -25.1 -689.0 0 37.56 -28.4 -855.8 0 37.82 -28.9 -1044.2 0 38.13 -29.8 -1100.8 0 38.65 -31.2 -1272.5 0 39.66 -33.5 -1661.7 0 40.15 -37.0 -2263.3 -1 41.66 -38.2 -2469.2 -1 42.12 -39.5 -2677.5 -1 42.62 -41.0 -2825.0 -1 42.62 -41.0 -2825.0 -1 43.33 -43.2	$\begin{array}{c} \text{ips:} & \text{F'y Inter} \\ \text{TorqZ} & \text{ksi} & 4.8.2 \\ .0 & 82.55 & .000 \\ .0 & 82.55 & .022 \\ .0 & 82.55 & .036 \\ .0 & 82.55 & .138 \\ .0 & 82.55 & .240 \\ .0 & 81.88 & .345 \\ .0 & 80.98 & .467 \\ .0 & 80.08 & .589 \\ .0 & 79.17 & .721 \\ .0 & 78.27 & .840 \\ .0 & 77.37 & .963 \\ .0 & 77.09 & .997 \\ .0 & 82.55 & .703 \\ .0 & 77.09 & .997 \\ .0 & 82.55 & .802 \\ .0 & 82.55 & .802 \\ .0 & 82.55 & .802 \\ .0 & 82.55 & .802 \\ .0 & 82.55 & .802 \\ .0 & 82.55 & .802 \\ .0 & 82.55 & .842 \\ .0 & 82.55 & .877 \\ .0 & 82.55 & .807 \\ .0 & 82.55 & .842 \\ .0 & 82.55 & .877 \\ .0 & 82.55 & .877 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .900 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .890 \\ .0 & 81.41 & .970 \\ .0 & 81.41 & .970 \\ .0 & 81.41 & .970 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .887 \\ .0 & 82.55 & .875 \\ .0 & 82.41 & .924 \\ .0 & 81.38 & .952 \\ .0 & 80.34 & .978 \\ .0 & 79.78 & .991 \\ .0 & 82.45 & .878 \\ .0 & 81.99 & .886 \\ .0 & 81.54 & .893 \\ .0 & 81.99 & .886 \\ .0 & 81.99 & .886 \\ .0 & 81.99 & .915 \\ .0 & 79.73 & .923 \\ .0 & 79.28 & .930 \\ .0 & 78.33 & .945 \\ \end{array}$

SABRE COMMUNICATI	ONS CORP	JOB :	00-576	17		26-M	lar-12	2 07:39
2101 Murray Stree	et	SBA NETW	ORK SER	VICES IN	1C	Ph 7	12.2	58.6690
Sioux City, IA 51	.101	North S	Stoningto	on 3, C1	2	Fx 7	12.25	58.8250
.00 2.15 5	5.47 .00	.0 60.41	-78.7	7779.2	.1	.0 78	8.29	.947
DISPLACEMENTS -							and the second	
ELEV	DEFLECTI	ION feet			-ROTATI	ON, deq:	rees	
X, ft X	Y 20 15 1	Z XY-Re	esult '	X	Y	Z	XY-Re	sult '
109.00 .00	20.12 -1	1,52 20,154	-0.00%> -		.00	.00	12.1	

SABRE COMMUNICATIONS CORP	JOB: 00-57617	26-Mar-12 07:39
2101 Murray Street SBA	NETWORK SERVICES INC	Ph 712.258.6690
Sioux City, IA 51101 No	rth Stonington 3, CT	Fx 712.258.8250
CAGE 2. 3g Custod Wind 0 0 Dood		ANGT TTA 222 C
CASE - 2: 38 Gusted Wind 0.9 Dead		ANS1-11A-222-G
WIND OLF 1.60 VERTICAL OLF .90 DESIGN ICE .00 in GUST FACTOR (Gh) 1.10 FORCE COEFF (Cf) .65 IMPORTANCE FAC (I) 1.00 DIRECTION FAC (Kd) .95 TOPOGRAPHIC CAT 3 APPURTENANCE LOADS	GUSTED WIND (3sec)115.0 tEXP-CAT/STRUC CLASSB-IIEXP-POWER COEFF289REFERENCE HEIGHT1200.0 tPRESSURE @ 104.2 ft56.6 pBASE ABOVE Grd1.0CREST HEIGHT100.0 t	mph 185.1 kph 57 ft psf 2708.7 Pa ft
C	enter WEIGHT AREA TX-CABL	E FORCES MOM.
# Oty Description E	line each each lev-Ft Lbs Ft^2 Type Oty :	WIND Tra-Y Ax-Z Lg-X #/Ft Psf Kips Kips Ft-K
1 1 14 LD Dlatform with Handrail /D		
- 12 DB848H90E-XY.	189.0 28 1 5/8" 24	1.04 68.6 -4.5
6 TMA 2 1 14' LP Platform with Handrail (R 12 DB848H90E-XY.	189.0 8 None 1 179.0 1704 92.0 179.0 28 1 5/8" 18 1	.00 68.6 .0 68.0 6.26 -1.5 -9.4 1.04 67.9 -3.3
6 TMA 3 1 14' LP Platform with Handrail (R 12 DB848H90E-XY.	179.0 8 None 1 169.0 1704 92.0 169.0 28 1 5/8" 18	$.00 \ 67.9 \ .0 \ 67.3 \ 6.19 \ -1.5 \ -9.3 \ 1.04 \ 67.2 \ -3.1$
6 TMA	169.0 8 None 1	.00 67.2 .0
12 DB848H90E-XY.	159.0 28 1 5/8" 18 2	1.04 66.5 -3.0
6 TMA 5 2 Pipe Mount (up to 6' Dish)	159.0 8 None 1 149.0 49 .1	.00 66.5 .0 65.8 .011 .0
2 6' SOLID DISH W/ RADOME	149.0 330 24.4 1 5/8" 12 3	1.04 65.8 3.21 -2.3
RESULTS		
X, ftKztpsfinShearX 189.00 1.02 44.62 $.00$ $.0$ 187.00 1.02 44.23 $.00$ $.0$ 182.00 1.02 44.28 $.00$ $.0$ 179.00 1.03 44.14 $.00$ $.0$ 179.00 1.03 43.90 $.00$ $.0$ 169.00 1.03 43.67 $.00$ $.0$ 169.00 1.03 43.67 $.00$ $.0$ 169.00 1.04 43.22 $.00$ $.0$ 159.00 1.04 43.22 $.00$ $.0$ 149.00 1.05 42.59 $.00$ $.0$ 144.00 1.05 42.59 $.00$ $.0$ 133.00 1.07 42.16 $.00$ $.0$ 133.00 1.07 42.16 $.00$ $.0$ 123.00 1.08 41.82 $.00$ $.0$ 113.00 1.07 41.98 $.00$ $.0$ 113.00 1.12 41.29 $.00$ $.0$ 99.50 1.15 41.14 $.00$ $.0$ 93.75 1.15 41.13 $.00$ $.0$ 73.75 1.23 41.02 $.00$ $.0$ 73.75 1.28 41.00 $.0$ $.0$ 73.75 1.28 41.00 $.0$ $.0$ 73.75 1.28 41.00 $.0$ $.0$ 74.10 1.45 $.41.27$ $.00$ $.0$ 114.125 $.00$	ShearY Axiaz BendX BendY ShearY Axiaz BendX BendY 01 - 1 0 0 0 7.74 - 4.9 - 9.6 0 8.19 -5.1 - 48.7 0 15.68 - 8.9 - 82.7 0 16.16 - 9.3 - 161.1 0 23.64 - 13.1 - 251.2 0 24.12 - 13.5 - 369.4 0 31.92 - 17.9 - 656.5 0 35.96 - 20.3 - 816.1 0 36.25 - 20.7 - 995.8 0 36.58 - 21.4 - 1050.0 0 37.65 - 23.5 - 1400.0 0 38.18 - 24.3 - 1588.3 0 38.71 - 25.2 - 1779.2 0 39.80 - 27.1 - 2169.2 0 40.36 - 28.1 - 2368.31 40.86 - 29.0 - 2570.01 41.39 - 30.2 - 2713.3 - 0 44.13 - 35.7 - 3593.31 44.13 - 35.7 - 3593.31 44.636 - 39.4 - 4265.81 47.17 - 40.7 - 4497.51 47.63 - 41.4 - 4733.31 47.63 - 41.4 - 4733.31 47.63 - 41.4 - 4733.31 49.49 - 45.4 - 5108.31 49.49 - 45.4 - 5108.31 51.50 - 48.7 - 5607.51 51.50 - 48.7 - 5607.51 55.00 - 53.5 - 6396.71 55.00 - 53.5 - 6396.71 55.776 - 56.7 - 6953.31	TorqZ ks1 4.8.2 0 82.55 .000 0 82.55 .021 0 82.55 .021 0 82.55 .026 0 81.88 .325 0 80.98 .441 0 80.08 .557 0 79.17 .683 0 78.27 .797 0 77.37 .915 0 77.09 .947 0 82.55 .764 0 82.55 .764 0 82.55 .803 0 82.55 .867 0 82.55 .864 0 82.55 .850 0 82.41 .888 0 81.89 .903 0 81.38 .917 0 80.86 .930 0 81.38 .917 0 80.64 .880 0 81.99 .856 0 82.55 .845 0 82.55 .845 0 82.55 .845 0 82.45 .848 0 81.99 .856 0 82.55 .845 0 82.917 0 80.64 .880 0 80.19 .888 0 79.73 .896 0 79.28 .904 0 78.83 .912 0 78.83 .912

SABRE COMMUNICATIONS	CORP	JOB: 00-57617		26-Mar-12 07:39
2101 Murray Street		SBA NETWORK SERVICES	INC	Ph 712.258.6690
Sioux City, IA 51101		North Stonington 3,	CT	Fx 712.258.8250
.00 2.15 55.47	.00	.0 60.43 -59.6 7599.2	.1	.0 78.29 .922
DISPLACEMENTS				
ELEV	DEFLECTION	feet	ROTATION	, degrees
X, ft X 189.00 .00 19	Y Z .46 -1.4	XY-Result X 1 19.46<10.30%> -11.64	. 00	Z XY-Result .00 11.64

SABRE COMMUNICATIONS CORP	JOB: 00-57617	26-Mar-12 07:39
2101 Murray Street SBA	NETWORK SERVICES INC	Ph 712.258.6690
Sioux City, IA 51101 No:	rth Stonington 3, CT	Fx 712.258.8250
CASE - 3: 3s Gusted Wind&Ice		ANSI-TIA-222-G
WIND OLF 1.00 VERTICAL OLF 1.20 DESIGN ICE .75 in GUST FACTOR (Gh) 1.10 FORCE COEFF (Cf) 1.20 IMPORTANCE FAC (I) 1.00 DIRECTION FAC (Kd) .95 TOPOGRAPHIC CAT 3	GUSTED WIND (3sec)50.0 mphEXP-CAT/STRUC CLASSB-IIEXP-POWER COEFF2857REFERENCE HEIGHT1200.0 ftPRESSURE @ 104.2 ft6.7 psfBASE ABOVE Grd1.0CREST HEIGHT100.0 ft	80.5 kph 320.0 Pa
APPORIENANCE LOADS	enter WEIGHT AREA TX-CABLE	FORCES MOM.
<pre># Qty Description E 1 1 14' LP Platform with Handrail (R 12 DB848H90E-XY. 6 TMA 2 1 14' LP Platform with Handrail (R 12 DB848H90E-XY.</pre>	Line each each lev-Ft Lbs Ft ² Type Qty #/Ft 187.0 1874 151.0 189.0 55 1 5/8" 24 1.04 189.0 11 None 1 .00 179.0 1874 150.8 179.0 55 1 5/8" 18 1.04	WIND Tra-Y Ax-Z Lg-X Psf Kips Kips Ft-K 8.1 1.22 -7.5 -1.8 8.1 -9.0 8.1 -3 8.0 1.21 -7.5 -1.8 8.0 -7.4
6 TMA 3 1 14' LP Platform with Handrail (R 12 DB848H90E-XY.	179.0 11 None 1 .00 169.0 1874 150.6 169.0 55 1 5/8" 18 1.04	$\begin{array}{rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$
6 TMA 4 1 14' LP Platform with Handrail (R	169.0 11 None 1 .00 159.0 1874 150.3	$\begin{array}{rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$
12 DB848H90E-XY. 6 TMA 5 2 Pipe Mount (up to 6' Dish)	159.0 55 15/8" 18 1.04 159.0 11 None 1 .00 149.0 53 .1	7.9 -6.9 7.93 7.8 .001 .0
2 6' SOLID DISH W/ RADOME	149.0 838 25.1 1 5/8" 12 1.04	7.8 .39 -3.0
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	RCES,kips	: F'Y Inter qZ ksi 4.8.2 .0 82.55 .000 .0 82.55 .013 .0 82.55 .027 .0 82.55 .045 .0 82.55 .068 .0 81.88 .097 .0 80.98 .125 .0 80.08 .157 .0 79.17 .187 .0 78.27 .215 .0 77.37 .241 .0 77.09 .248 .0 82.55 .173 .0 82.55 .193 .0 82.55 .202 .0 82.55 .202 .0 82.55 .209 .0 82.55 .202 .0 82.55 .203 .0 82.55 .203 .0 82.55 .203 .0 82.55 .208 .0 82.55 .213 .0 82.41 .215 .0 81.89 .218 .0 81.89 .218 .0 81.89 .218 .0 81.38 .221 .0 80.86 .224 .0 80.34 .227 .0 79.78 .229 .0 82.55 .202 .0 82.45 .203 .0 81.99 .205 .0 81.99 .205 .0 81.99 .205 .0 81.99 .205 .0 81.99 .205 .0 81.99 .205 .0 81.99 .208 .0 81.99 .208 .0 81.99 .208 .0 81.99 .208 .0 81.99 .208 .0 81.99 .211 .0 79.73 .213 .0 79.78 .214 .0 78.83 .216 .0 78.83 .216

SABRE COMMUNICATION	S CORP	JOB: 00-57	617	26-Mar-12 07:39
2101 Murray Street		SBA NETWORK SE	RVICES INC	Ph 712.258.6690
Sioux City, IA 5110	1	North Stoning	ton 3, CT	Fx 712.258.8250
.00 2.15 12.1	1.38	.0 12.93 -107.4	1681.7 .	0 .0 78.29 .218
DISPLACEMENTS				
ELEV X, ft X 189.00 .00	-DEFLECTIO Y 4.42	N feet Z XY-Result 08 4.42< 2.34%>	X -2.66	ROTATION, degrees Y Z XY-Result 00 .00 2.66

SABRE COMMUNICATIONS CORP	JOB: 00-57617	26-Mar-12 07:39
2101 Murray Street SB	A NETWORK SERVICES INC	Ph 712.258.6690
Sioux City, IA 51101 N	orth Stonington 3, CT	Fx 712.258.8250
CASE - 4: Service Loads		ANSI-TIA-222-G
WIND OLF 1.00 VERTICAL OLF 1.00 DESIGN ICE .00 in GUST FACTOR (Gh) 1.10 FORCE COEFF (Cf) .65 IMPORTANCE FAC (I) 1.00 DIRECTION FAC (Kd) .85 TOPOGRAPHIC CAT 3 APPURTENANCE LOADS	GUSTED WIND (3sec)60.0 mphEXP-CAT/STRUC CLASSB-IIEXP-POWER COEFF2857REFERENCE HEIGHT1200.0 ftPRESSURE @ 104.2 ft8.6 psfBASE ABOVE Grd1.0CREST HEIGHT100.0 ft	96.6 kph 412.3 Pa
	Center WEIGHT AREA TX-CABLE	FORCES MOM.
# Oty Description	Line each each Elev-Ft Lbs Ft ² Type Oty #/Ft	WIND Tra-Y Ax-Z Lg-X Psf Kips Kips Ft-K
_ 1 1 14' LP Platform with Handrail (R 12 DB848H90E-XY.	187.0 1704 92.0 189.0 28 1 5/8" 24 1.04	10.4 .96 -1.7 -1.4 10.4 -5.0
6 TMA 2 1 14' LP Platform with Handrail (R 12 DB848H90E-XY.	189.0 8 None 1 .00 179.0 1704 92.0 179.0 28 1 5/8" 18 1.04	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
 6 TMA 3 1 14' LP Platform with Handrail (R 12 DB848H90E-XY. 	179.0 8 None 1 .00 169.0 1704 92.0 169.0 28 1 5/8" 18 1.04 169.0 8 None 1 00	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
 4 1 14' LP Platform with Handrail (R 12 DB848H90E-XY. 	159.0 1704 92.0 159.0 28 1 5/8" 18 1.04 159.0 8 None 1 00	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
5 2 Pipe Mount (up to 6' Dish) 2 6' SOLID DISH W/ RADOME	149.0 49 .1 149.0 330 24.4 1 5/8" 12 1.04	10.0 .001 .0 10.0 .49 -2.5
RESULTS WIND ICE	OPCES king . MOMENTS ft-king-	Ely Inter
X, ftKztpsfinShear 189.00 1.02 6.79 $.00$ $.0$ 187.00 1.02 6.78 $.00$ $.0$ 182.00 1.02 6.74 $.00$ $.0$ 179.00 1.03 6.72 $.00$ $.0$ 179.00 1.03 6.68 $.00$ $.0$ 169.00 1.03 6.65 $.00$ $.0$ 164.00 1.04 6.61 $.00$ $.0$ 159.00 1.04 6.55 $.00$ $.0$ 149.00 1.05 6.48 $.00$ $.0$ 144.00 1.05 6.48 $.00$ $.0$ 142.50 1.05 6.47 $.00$ $.0$ 138.00 1.07 6.39 $.00$ $.0$ 128.00 1.07 6.39 $.00$ $.0$ 123.00 1.108 6.37 $.00$ $.0$ 113.00 1.12 6.28 $.00$ $.0$ 99.50 1.13 6.27 $.00$ $.0$ 99.50 1.13 6.27 $.00$ $.0$ 93.75 1.16 6.25 $.00$ $.0$ 73.75 1.28 6.26 $.00$ $.0$ 73.75 1.28 6.27 $.00$ $.0$ 73.75 1.34 6.27 $.00$ $.0$ 73.75 1.35 6.24 $.00$ $.0$ 73.75 1.28 6.26 $.00$ $.0$ 73.75 1.28 6.27 $.00$ <td>X ShearY AxiaZ BendX BendY Tor 00 - 1 0 0 1.23 -7.0 - 1.5 0 1.30 -7.2 -7.7 0 2.49 - 12.9 - 13.0 0 2.56 - 13.2 - 25.4 0 3.74 - 18.8 - 39.6 0 3.81 - 19.2 - 58.3 0 4.96 - 24.6 - 78.8 0 5.03 - 25.0 - 103.6 0 5.65 - 28.0 - 128.8 0 5.69 - 28.3 - 157.0 0 5.74 - 28.9 - 165.6 0 5.82 - 29.9 - 191.4 0 5.97 - 31.4 - 250.0 0 6.13 - 32.9 - 310.1 0 6.29 - 34.4 - 371.8 0 6.29 - 34.4 - 371.8 0 6.36 - 35.2 - 403.2 0 6.44 - 36.3 - 425.4 0 6.49 - 37.1 - 457.7 0 6.55 - 37.9 - 462.5 0 6.64 - 39.1 - 495.3 0 6.74 - 40.2 - 528.4 0 6.84 - 41.2 - 562.2 0 6.94 - 42.3 - 596.3 0 7.05 - 43.4 - 631.1 0 7.17 - 44.5 - 666.3 0 7.29 - 45.6 - 702.2 0 7.36 - 46.3 - 738.6 0 7.43 - 47.3 - 742.3 0 7.53 - 48.7 - 779.4 0 7.53 - 50.2 - 796.3 0 7.63 - 50.2 - 796.3 0 7.78 - 51.9 - 834.2 0 7.93 - 53.3 - 873.3 0 8.10 - 54.8 - 913.3 0 8.10 - 54.8 - 913.3 0 8.10 - 54.8 - 913.3 0 8.27 - 56.3 - 955.0 0 8.87 - 60.9 - 1080.8 0 9.10 - 62.5 - 1125.0 0</td> <td>$\begin{array}{c c c c c c c c c c c c c c c c c c c$</td>	X ShearY AxiaZ BendX BendY Tor 00 - 1 0 0 1.23 -7.0 - 1.5 0 1.30 -7.2 -7.7 0 2.49 - 12.9 - 13.0 0 2.56 - 13.2 - 25.4 0 3.74 - 18.8 - 39.6 0 3.81 - 19.2 - 58.3 0 4.96 - 24.6 - 78.8 0 5.03 - 25.0 - 103.6 0 5.65 - 28.0 - 128.8 0 5.69 - 28.3 - 157.0 0 5.74 - 28.9 - 165.6 0 5.82 - 29.9 - 191.4 0 5.97 - 31.4 - 250.0 0 6.13 - 32.9 - 310.1 0 6.29 - 34.4 - 371.8 0 6.29 - 34.4 - 371.8 0 6.36 - 35.2 - 403.2 0 6.44 - 36.3 - 425.4 0 6.49 - 37.1 - 457.7 0 6.55 - 37.9 - 462.5 0 6.64 - 39.1 - 495.3 0 6.74 - 40.2 - 528.4 0 6.84 - 41.2 - 562.2 0 6.94 - 42.3 - 596.3 0 7.05 - 43.4 - 631.1 0 7.17 - 44.5 - 666.3 0 7.29 - 45.6 - 702.2 0 7.36 - 46.3 - 738.6 0 7.43 - 47.3 - 742.3 0 7.53 - 48.7 - 779.4 0 7.53 - 50.2 - 796.3 0 7.63 - 50.2 - 796.3 0 7.78 - 51.9 - 834.2 0 7.93 - 53.3 - 873.3 0 8.10 - 54.8 - 913.3 0 8.10 - 54.8 - 913.3 0 8.10 - 54.8 - 913.3 0 8.27 - 56.3 - 955.0 0 8.87 - 60.9 - 1080.8 0 9.10 - 62.5 - 1125.0 0	$\begin{array}{c c c c c c c c c c c c c c c c c c c $

SABRE	COMM	UNICA	ATIONS	CORP		JOB:	00-57	617		26-	Mar-1	2 07:	39
2101	Murra	y Sti	reet		SBA	A NETW	ORK SE	RVICES	INC	Ph	712.2	58.66	90
Sioux	City	, IA	51101		No	orth S	ltoning	ton 3,	CT	Fx	712.2	58.82	50
	.00	2.15	8.44	.00	.0	9.27	-63.6	1180.0	.0	.0	78.29	.151	
DISP	PLACEN	IENTS	1		a service and the	And the second second	en in der der sone		a Million a disso anna	and a state of the second second	in eta kua		
	ELEV	1	I	DEFLECT	ION feet	5			ROTAT	TION, deg	grees-		Micro
1	X, ft 89.00		X 00 3	Y .05	Z 04	XY-Re 3.05<	sult 1.62%>	-1.82	. 00	. 00	XY-R 1.	esult 82	Allow

SABRE COMMUNICATIONS CORP JOB: 00-5761	.7 26-Mar-12 07:39					
2101 Murray Street SBA NETWORK SERV	VICES INC Ph 712.258.6690					
Sioux City, IA 51101 North Stoningto	on 3, CT Fx 712.258.8250					
SHAPE: 18 SIDED POLYGON with FL BOLTS: QUADRANT SPACED BOLTS LOCATE:	AT-FLAT ORIENTATION 5,00 in. ON CENTER					
POLE DATA	EOPCE - 78 7 king Vert					
PLATE = .5000 in. BASE AATA PLATE = .5000 in. ACTIONS SHEAD TAPER = .2177 in/ft SHEAD POLE Fy = 65.00 ksi X-AX Y-AX Z-AX	$\begin{array}{rcl} FORCL = & -78.7 \text{ kips} & \text{Vert} \\ \text{R X} & = & 38.8 \text{ kips} & \text{Long} \\ \text{R Y} & = & 46.3 \text{ kips} & \text{Tran} \\ \text{IS MOM} & = & 5499.9 \text{ ft-kips} & \text{Tran} \\ \text{is MOM} & = & 5499.9 \text{ ft-kips} \text{ Long} \\ \text{is MOM} & = & 0 \text{ ft-kips} & \text{Vert} \end{array}$					
Design: ANY Orientation Reactions a	t 45 00 deg to X-AXIS					
BOLT LOADS	t 45.00 deg to X-AXIS					
AXIAL - COMPRESSION AXIAL - TENSION SHEAR AXIAL STRESS SHEAR STRESS YIELD STRENGTH FY ULT. STRENGTH FU ALLOW STRESS FA [.80 x 1.00] SHEAR FV [.80 x .40] TENSION AREA REQUIRED TENSION AREA FURNISHED ROOT AREA FURNISHED A615 ::: ANCHOR BOLT DE	<pre>= 233.77 kips = 227.21 kips = 3.55 kips = 71.93 ksi = 1.15 ksi = 100.00 ksi Interaction = 80.00 ksi .928 TIA-G = 32.00 ksi = 2.92 in^2 = 3.25 in^2 = 3.07 in^2</pre>					
24 Bolts on a 67.500 in.	Bolt Circle SHIP					
2.250 in. Diameter 67.13 in.	Empeaded (1DS)					
12.00 In. Exposed 84.00 In.	TOCAT LENGEN STON					
CONCRETE - Fc= 4000 psi						
ANCHOR BOLTS are STRAIGHT w\ UPLIFT NUT						
BASE PLATE						
$[Bend Model: 1/4 Circ]$ $YIELD STRENGTH = 50.0 ksi$ $BEND LINE WIDTH = 48.1 in.$ $PLATE MOMENT = 4245.7 in-k$ $THICKNESS REQD = 2.800 in.$ $BENDING STRESS = 39.2 ksi$ $ALLOWABLE STRESS = 45.0 ksi$ $[Fy \times .90 \times 1.00]$	BASE PLATE USED3.00 in. THICKSHIP69.75 in. SQUARE(lbs)48.25 in. CENTER HOLE197217.00 in. CORNER CLIP					
LOAD CASE SIMMAR	Y					

							ABol	t-Str	Plate-	Str	
	FO	RCES-(k	ips)	MOME	ENTS-(ft	-k)		Allow	_Actual	Allow	_Design
LC	Axial	ShearX	ShearY	X-axis	Y-axis	TorQ	CSR	ksi	ksi	ksi	Code
1	78.7	38.8	46.3	4989	5967	0	.928	75.00	39.20	45.00	TIA-G
2	59.6	38.8	46.4	4874	5829	0	.904	75.00	38.16	45.00	TIA-G
3	107.4	8.3	9.9	1078	1290	0	.215	75.00	9.14	45.00	TIA-G
4	63.6	5.9	7.1	756	905	0	.149	75.00	6.33	45.00	TIA-G

MAT FOUNDATION DESIGN BY SABRE TOWERS & POLES 190' Monopole SBA NETWORK SERVICES INC North Stonington 3, CT (57617) 3-26-12 REB

Overall Loads:			
Factored Moment (ft-kips)	8946.05		
Factored Axial (kips)	90.55		
Factored Shear (kips)	69.47		
Bearing Design Strength (ksf)	34.70	Max. Net Bearing Press. (ksf)	5.55
Water Table Below Grade (ft)	999		
Width of Mat (ft)	29	Ultimate Bearing Pressure (ksf)	46.27
Thickness of Mat (ft)	2	Bearing Φs	0.75
Depth to Bottom of Slab (ft)	6		
Quantity of Bolts in Bolt Circle	24		
Bolt Circle Diameter (in)	67.5		
Top of Concrete to Top			
of Bottom Threads (in)	60		
Equivalent Diameter of Pier (ft)	9.03	Minimum Pier Diameter (ft)	7.13
Ht. of Pier Above Ground (ft)	0.5	Equivalent Square b (ft)	8.00
Ht. of Pier Below Ground (ft)	4		
Quantity of Bars in Mat	57		
Bar Diameter in Mat (in)	1.128		
Area of Bars in Mat (in ²)	56.96		
Spacing of Bars in Mat (in)	6.09	Recommended Spacing (in)	6 to 12
Quantity of Bars Pier	40	1	
Bar Diameter in Pier (in)	1.27		
Tie Bar Diameter in Pier (in)	0.625		
Spacing of Ties (in)	12		
Area of Bars in Pier (in^2)	50.67	Minimum Pier A _s (in ²)	46.11
Spacing of Bars in Pier (in)	7.84	Recommended Spacing (in)	6 to 12
fc (ksi)	4		0 10 12
fv (ksi)	60		
Unit Wt. of Soil (kcf)	0.116		
Unit Wt. of Concrete (kcf)	0.15		
Volume of Concrete (vd ³)	72 97		
Two-Way Shear Action:	12.01		
Average d (in)	19.872		
ϕV_{e} (kips)	1508 7	V. (kins)	142.4
$\phi V_c = \phi (2 + 4/\beta_c) f_c^{1/2} b_c d$	2278.4		172.7
$dV = d(\alpha d/b + 2)f^{1/2}b d$	1508 7		
$\phi V_c = \phi 4 f_c^{1/2} b_c d$	1518.9		
Shear perimeter b (in)	402.85		
	402.00		
p _c	1		
One-Way Shear:			
ϕV_{c} (kips)	743.5	V,, (kips)	599.0
Stability		- u (···P=/	
Overturning Design Strength (ft-k)	9546.0	Total Applied M (ft-k)	9397.6
	6		

MAT FOUNDATION DESIGN BY SABRE TOWERS & POLES (CONTINUED) 190' Monopole SBA NETWORK SERVICES INC North Stonington 3, CT (57617) 3-26-12 REB

Pier Design:			
φV _n (kips)	1014.9	V _u (kips)	69.5
$\phi V_c = \phi 2(1 + N_u / (2000 A_g)) f_c^{1/2} b_w d$	1014.9	Maral A. I. T. Yuro	
V _s (kips)	0.0	*** V _s max = 4 f' _c ^{1/2} b _w d (kips)	2376.4
Maximum Spacing (in)	6.80	(Only if Shear Ties are Required)	L]
Actual Hook Development (in)	18.74	Req'd Hook Development I _{dh} (in)	16.87
		*** Ref. To Spacing Requirements ACI	11.5.4.3
Flexure in Slab:			
φM _n (ft-kips)	4723.5	M _u (ft-kips)	4421.6
a (in)	2.89		
Steel Ratio	0.00824		
β1	0.85		
Maximum Steel Ratio (.75p _b)	0.0214		
Minimum Steel Ratio	0.0018		
Rebar Development in Pad (in)	171.00	Required Development in Pad (in)	49.82
Condition	1 is OK, 0 Fails		
Maximum Soil Bearing Pressure	1		
Pier Area of Steel	1		
Pier Shear	1		
Interaction Diagram Visual Check	1		
Two-Way Shear Action	1		
One-Way Shear Action	1		
Overturning	1		
Flexure	1		
Steel Ratio	1		
Length of Development in Pad	1		
Hook Development	1		

Attachment 3

Simply Intelligent

AM-X-CD-17-65-00T-RET(8' 65° Dual Broadband Antenna)

KMW Communications

Base Station Antennas For Mobile Communications

Dual Band Electrical DownTilt Antenna

698 ~ 894MHz, X-pol., H65° / V8.0°

1710 ~ 2170MHz, X-pol., H65° / V7.0°

Electrical Specification

Frequency Ra	nge	698~894MHz	1710~2170MHz		
Impedance		50Ω			
Polarization		Dual, S	Slant ±45°		
Gain		16.8dBi / 14.65dBd @ 698-806MHz 17.5dBi / 15.35dBd @ 824-894MHz	17.0dBi / 14.85dBd @ 1710-1755MHz 17.3dBi / 15.15dBd @ 1850-1900MHz 17.5dBi / 15.35dBd @ 2110-2155MHz		
Beamwidth	Horizontal	68° @ 698-806MHz 63° @ 824-894MHz	67° @ 1710-1755MHz 65° @ 1850-1900MHz 62° @ 2110-2155MHz		
Beamwidth	Vertical	9.2° @ 698-806MHz 8.0° @ 824-894MHz	7.3° @ 1710-1755MHz 7.0° @ 1850-1900MHz 6.7° @ 2110-2155MHz		
VSWR		≤1.5:1			
Front-to-Back	Ratio	≥27 dB			
Electrical Dow	ntilt Range	2° ~ 16° 0° ~ 10°			
Isolation Between Ports		≥3	0 dB		
Isolation Betwe	een Ports of Different Frequency Elements	≥3	5 dB		
Cross Pole Discrimination		10.0 dB @ ±60° 15.0 dBi @ 0°			
First Upper Sid	e Lobe Suppression	16dB			
Side Lobe Suppression		> 16dB @ 0-6° Tilt > 18dB @ 7-12° Tilt (Up to 10° from Boresight)	> 16dB @ 0-6° Tilt > 18dB @ 7-10° Tilt (Up to 10° from Boresight)		
Passive Intermodulation		≤ -150 dBc @ 2x20w			
Input Maximum CW Power		500 W	300 W		
Environmental Compliance		IP65 for Radome IP67 for Connectors			
RET Motor Co	nfiguration	Field Replaceable RET Electronic Control Module / RET Motor is internal to antenna & not field replaceable			
Compliant with AISG 1.1 and 2.0		AISG 1	AISG 1.1 and 2.0		

Mechanical Specification

Dimension (W×D×H)	11.8×6.0×96 inches
Weight (Without clamp)	27kg (59.5 lbs)
Connector	4 x 7/16 DIN(F), Long Neck
Max Wind Speed	150mph
Wind Load (@150 mph)	2521 N





Attachment 4





Industrial Diesel Generator Set

EPA Emissions Certification: Tier III

SD050

CUSTOM MODEL

Standby Power Rating 50KW 60 Hz









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Te	a 1	[U	r	e	S

. Engine

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. **Controls**

<u>Alternator</u>

Generator Set

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benefits

PROTOTYPE & TORSIONALLY TESTED	►	PROVIDES A PROVEN UNIT
UL2200 TESTED	►	ENSURES A QUALITY PRODUCT
RHINOCOAT PAINT SYSTEM	►	IMPROVES RESISTANCE TO ELEMENTS
SOUND LEVEL 2 ENCLOSURE	►	71dbA @ 7 METERS (23FT)
EPA TIER CERTIFIED	►	ENVIRONMENTALLY FRIENDLY
INDUSTRIAL TESTED, GENERAC APPROVED	►	ENSURES INDUSTRIAL STANDARDS
POWER-MATCHED OUTPUT	►	ENGINEERED FOR PERFORMANCE
INDUSTRIAL GRADE	►	IMPROVES LONGEVITY AND RELIABILITY
tor		
TWO-THIRDS PITCH	►	ELIMINATES HARMFUL 3RD HARMONIC
LAYER WOUND ROTOR & STATOR	►	IMPROVES COOLING
CLASS H MATERIALS	►	HEAT TOLERANT DESIGN
DIGITAL 3-PHASE VOLTAGE CONTROL	►	FAST AND ACCURATE RESPONSE
ENCADSULATED BOARD W/ SEALED HARNESS	•	
A 20-A VOLTAGE TO CURRENT CENCORC	r N	
4-20MA VOLTAGE-TO-CORRENT SENSORS		NOISE RESISTANT 24/7 MONITORING
SURFACE-MOUNT TECHNOLOGY	▶	PROVIDES VIBRATION RESISTANCE
ADVANCED DIAGNOSTICS & COMMUNICATIONS	►	HARDENED RELIABILITY

primary codes and standards





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ISO

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application and engineering data

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ENGINE SPECIFICATIONS

General			
Make	lveco / FPT		
EPA Emissions Compliance	Tie	r III	
EPA Emissions Reference	See Emission	is Data Sheet	
Cylinder #	4		
Туре	Diesel		
Displacement - L (cu. in.)	4.5	(274)	
Bore - mm (in.)	105	(4.1)	
Stroke - mm (in.)	132	(5.2)	
Compression Ratio	17.5:1		
Intake Air Method	Turbocharged		
Cylinder Head Type	2 Valve		
Piston Type	Aluminum		
Crankshaft Type	Forged Steel		
Engine Block Type	Cast Iron /	Wet Sleeve	

Engine Governing	
Governor	Electronic Isochronous
Frequency Regulation (Steady State)	+/- 0.25%

Lubrication System

Oil Pump Type	Gear
Oil Filter Type	Full Flow
Crankcase Capacity - L (gal)(qts)	13.6 (3.6) (14.4)

Cooling System

Cooling System Type	Closed	
Water Pump	Belt Driven Centrifugal	
Fan Type	Pusher	
Fan Blade Number	2538 (10)	
Fan Diameter (in.)	26	
Coolant Heater Wattage	1500	
Coolant Heater Standard Voltage	120	

Fuel System

Fuel Type	Ultra Low Sulfur Diesel Fuel			
Fuel Specifications	ASTM			
Fuel Filtering (microns)	5			
Fuel Inject Pump Make	Standyne			
Fuel Pump Type	Engine Driven Gear			
Injector Type	Mechanical			
Engine Type	Direct Injection			
Fuel Supply Line - mm (in.)	1/4 inch Npt			
Fuel Return Line - mm (in.)	1/4 inch Npt			

Engine Electrical System

System Voltage	12VDC		
Battery Charging Alternator	90 Amp		
Battery Size (at 0 oC)	Optima Redtop		
Battery Group	34		
Battery Voltage	12VC		
Ground Polarity	Negative		

ALTERNATOR SPECIFICATIONS

Standard Model	390			
Poles	4			
Field Type	Revolving			
Insulation Class - Rotor	н			
Insulation Class - Stator	Н			
Total Harmonic Distortion	< 3.5%			
Telephone Interference Factor (TIF)	< 50			
Standard Excitation	PMG			
Bearings	Single Sealed Cartridge			
Coupling	Direct, Flexible Disc			
Load Capacity - Standby	100%			
Load Capacity - Prime	100%			
Prototype Short Circuit Test	Y			

CODES AND STANDARDS COMPLIANCE (WHERE APPLICABLE)

NFPA 99 NFPA 110 ISO 8528-5 ISO 1708A.5 ISO 3046 BS5514 SAE J1349 DIN6271 IEEE C62.41 TESTING NEMA ICS 1

Rating Definitions:

Standby – Applicable for a varying emergency load for the duration of a utility power outage with no overload capability. (Max. load factor = 70%)

Prime – Applicable for supplying power to a varying load in lieu of utility for an unlimited amount of running time. (Max. load factor = 80%) A 10% overload capacity is available for 1 out of every 12 hours.

Voltage Regulator Type	Digital
Number of Sensed Phases	All
Regulation Accuracy (Steady State)	+/- 0.25%

SD050

operating data (60Hz)

Single-Phase 120	/240VAC @	ຈີ1 0nf			50	Amps	208	1					
Three-Phase 120	208VAC @	0.8pf			-	Amps:	-	1					
Three-Phase 120	240VAC @	0.8pf			-	Amps:	-	1					
Three-Phase 277	480VAC @	0.8pf			-	Amps:	-	1					
Three-Phase 346	600VAC @	0.8pf			-	Amps:	-	1					
					NOTE: Genera	tor output limite	d to 200A.						
ARTING CAPAB	LITIES (s	KVA)											
		_				s	KVA vs. Vo	oltage Dip					
				48	OVAC					208/2	40VAC		
Alternator*	<u>kW</u>	10%	15%	20%	25%	30%	35%	10%	15%	20%	25%	30%	35
Standard	50	-	-	-	-	-	-	26	39	52	65	77	9
Upsize 1		-	-	-	-	-	-	-	-	-	-	-	-
Upsize 2		<u> </u>	-	-	-	-	-	-	-	-	-	-	-
	*All Generac temperature	industrial alter rise. Upsize 2 p	nators utilize Cl rovides less tha	ass H insulatio an or equal	n materials. Sta	ndard alternator	provides less t	than or equal t	o Class B temp	erature rise. U	psize 1 provide	s less than or e	qual to C
EL				-									
					Fuel Co	onsumptior	Rates						
Fuel Pump Lift	- in (m)	7			<u>STAI</u>	NDBY							
36(.9)				Perce	nt Load	gph	lph	1					
				2	5%	1.52	5.75						
				5	0%	2.33	8.82						
				7.	5%	3.08	11.65						
				10	0%	4.15	15./1	l					
OLING													
Coolant System	Capacity	- Gal (L)	1	a				1		STA		1	
4.5	o (17.44)		J	Coolant Flow per Minute					gpm (lpm) 32.7(123.8)				
				Heat rejection to Coolant					BTU/min 123,000				
Maximum Radi	ator Backp	oressure	1	Inlet Air					cfm (m3/min) $6,360$ (180.0)				
1.5" H ₂	O Column		J	Max. Operating Radiator Air Temp					F (C) 122(50)				
				Max. Ope	erating Am	bient Temp	erature		F ^o (C ^o)	122	2(50)	l	
MBUSTION AIR	REQUIR	EMENTS											
						STANDBY							
Intake Flow at Ra	ted Power		cfm	(m3/min)	247		(7.00)	1					
								-					
HAUST													
Exhaust Outle	t Size (Ope	n Set)								STA	NDBY		
	3.0"		1	Exhaust F	low (Rated	d Output)		cfr	m (m3/hr)	534(906.7)	T	
Maximum Backpressure (Post-Silencer)			4	Maximum Backpressure				inHg (Kpa) 1.5 (5.1)					
Maximum Backpr				Exhaust Temp (Bated Output)					°F (°C)	-	. ,	t	
Maximum Backpro	5" Hø			Exhaust T	'emn (Rate	d Outnut)			FILL	u u	30(498 81		
Maximum Backpro 1.5	5" Hg		J	Exhaust T	emp (Rate	ed Output)			F(C)	9.	30(498.8)		
Maximum Backpro 1.5 GINE	5" Hg			Exhaust T	emp (Rate	ed Output)			F (C)	9.	30(498.8)	<u> </u>	
Maximum Backpro 1.5 GINE	5" Hg] 	Exhaust T	emp (Rate	ed Output)			F(C)	9.	30(498.8)	<u> </u>	

* CA units include aftertreatment

Horsepower at Rated kW

Temperature Deration

Altitude Deration

Deration – Operational characteristics consider maximum ambient conditions. Derate factors may apply under atypical site conditions. Please consult a Generac Power Systems Industrial Dealer for additional details. All performance ratings in accordance with ISO3046, BS5514, ISO8528 and DIN6271 standards.

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Consult Factory

Consult Factory

3 of 5



standard features and options

4 of 5

CONTROL SYSTEM	
Control Panel	
Digital H Control Panel - Dual 4x20 Display	Std
Programmable Crank Limiter	Std
7-Day Programmable Exerciser (requires H-Transfer Switch)	Std
Special Applications Programmable PLC	Std
RS-232	Std
RS-485	Std
All-Phase Sensing DVR	Std
Full System Status	Std
Utility Monitoring (Req. H-Transfer Switch)	Std
2-Wire Start Compatible	Std
Power Output (kW)	Std
Power Factor	Std
Reactive Power	Std
All phase AC Voltage	Std
All phase Currents	Std
Oil Pressure	Std
Coolant Temperature	Std
Coolant Level	Std
Low Fuel Pressure Indication	Std
Engine Speed	Std
Battery Voltage	Std
Frequency	Std
Date/Time Fault History (Event Log)	Std
UL2200 GENprotect™	Std
Low-Speed Exercise	Opt
Isochronous Governor Control	Std
-40deg C - 70deg C Operation	Std
Weather Resistant Electrical Connections	Std
Audible Alarms and Shutdowns	Std
Not in Auto (Flashing Light)	Std
On/Off/Manual Switch	Std
E-Stop (Red Mushroom-Type)	Std
Remote E-Stop (Break Glass-Type, Surface Mount)	-
Remote E-Stop (Red Mushroom-Type, Surface Mount)	-
Remote E-Stop (Red Mushroom-Type, Flush Mount)	-
NFPA 110 Level I and II (Programmable)	Std
Remote Communication - RS232	Std

SD050

GENERATOR SET

Genset Vibration Isolation	Std
Factory Testing	Std
Extended warranty	Std
Padlockable Doors	Std
Steel Enclosure (Enclosed Models)	Std
Remote Emergency Shutdown	Opt

	dog-14∰ - "
-	

General

ENGINE SYSTEM

Oil Drain Extension	Std
Air Cleaner	Std
 Industrial Exhaust Silencer (Open Sets, ship loose) 	Std
 Critical Exhaust Silencer (Enclosed Sets) 	Std
 Stainless steel flexible exhaust connection 	Std
Fuel System	
Primary Fuel Filter with Water Separator	Std
Flexible Fuel Lines	Std
UL142 Fuel Tank, 48 Hr Runtime	Std
2 Gal Overflow Containment with Alarm	Std

Cooling System	
 120VAC Coolant Heater (3-wire connection cord) 	Std
50%/50% Coolant	Std
Level 1 Guarding (Open Sets)	Std
Closed Coolant Recovery System	Std
UV/Ozone resistant hoses	Std
Factory-Installed Radiator	Std
Radiator Drain Extension	Std
Fan guard	Std
 Radiator duct adapter (Open Sets) 	Std

Engine Electrical System

Battery charging alternator	Std
Battery cables	Std
Battery tray	Std
75W 120VAC Battery heater	Std
Solenoid activated starter motor	Std
10A UL float/equalize battery charger	Std
Weather Resistant electrical connections	Std
Duplex GFCI Convenience Outlet	Std

ALTERNATOR SYSTEM

C	UL2200 GENprotect [™]	Std
	100% Rated 200A Main Line Circuit Breaker	Std

	Alarms (Programmable Tolerances, Pre-Alarms and Shutdow					
)	Low Fuel	Std				
)	Oil Pressure (Pre-programmed Low Pressure Shutdown)	Std				
)	Coolant Temperature (Pre-programmed High Temp Shutdo	Std				
		C 1 1				

Coolant Level (Pre-programmed Low Level Shutdown)	Sta
Engine Speed (Pre-programmed Overspeed Shutdown)	Std
Voltage (Pre-programmed Overvoltage Shutdown)	Std
Battery Voltage	Std

Other Options

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dimensions, weights and sound levels









	OPEN SET							
		TANK	SIZE					
	RUNTIME	CAPACITY	TANK					
	HOURS	(GAL)	VOLUME	L	W	Н	WT	dBA*
О	-	-	-	-	-	-	-	
О	-	-	-	-	-	-	-	
О	-	-	-	-	-	-	-	
0	-	-	-	-	-	-	-	84
0	-	-	-	-	-	-	-	04
	48	210	210	76	38	87	3400	
0	-	-	-	-	-	-	-	
О	-	-	-	-	-	-	-	

	LEVEL 2 S	OUND ENC	CLOSURE					
		TANK	SIZE					
	RUNTIME	CAPACITY	TANK					
	HOURS	(GAL)	VOLUME	L	W	Н	WT	dBA*
0	-	-	-	-	-	-	-	
0	-	-	-	-	-	-	-	
0	-	-	-	-	-	-	-	
0	-	-	-	-	-	-	-	71
0	-	-	-	-	-	-	-	/1
	48	210	210	94.8	38	99	3935	
0	-	-	-	-	-	-	-	
0	-	-	-	-	-	-	-	

*Required gallons based on 100% of standby rating. Weights consider steel enclosure and are without fuel in tank. Sound levels measured at 23ft (7m) and does not account for ambient site conditions.

YOUR FACTORY RECOGNIZED GENERAC INDUSTRIAL DEALER					

Specification characteristics may change without notice. Dimensions and weights are for preliminary purposes only. Please consult a Generac Power Systems Industrial Dealer for detailed installation drawings.

0J2534

- TANK STUB-UP AREA TOP OR PLAN VIEW _ 36.5 [1.4″] 795 [31.3"] TANK 384 . 90 [3.5*****] 420 B [15.1"] STUB-UP [16.5"] AREA 32 [1.3*****] 202.5 [8″] LOW VOLTAGE CUSTOMER CONNECTION -BOX (NOTE 3)





RECOMMENDED ELECTRICAL STUB-U (SEE TOP VIEW)	IPS
	INSIDE BASE
1) LOW VOLTAGE CUSTOMER CONNECTION BOX FOR 120VAC GFCI OUTLET, (STANDARD BLOCK HEATER, BATTERY CHARGER AND OTHER 120 VAC OPTIONS).	B SEE NOTE 3
2) TRANSFER SWITCH/ COMMUNICATION CONDUITS. COMMUNICATIONS AND 2-WIRE START MUST NOT BE RUN IN CONDUIT WITH AC WRING.	

NOTES:

- 1. THE LEFT SIDE OF THE GENERATOR IS SERVICE ACCESSIBLE.
- 2. 10 AMP BATTERY CHARGER ENCLOSED WITHIN CONTROL PANEL. 3. CONNECTION POINTS FOR CONTROL WIRES. BOTTOM OF LOW VOLTAGE CUSTOMER CONNECTION BOX HAS KNOCKOUTS FOR 1/2" AND 3/4" CONDUIT FITTINGS.
- 4. GENERATOR MUST BE GROUNDED.
- 5. 12 VOLT NEGATIVE GROUND SYSTEM.
- 6. OPTIONAL REMOTE EMERGENCY STOP SHIPPED LOOSE WITH GENERATOR. 7. MAIN LINE CIRCUIT BREAKER (MLCB), AC LOAD LEAD CONNECTION AND
- AUXILIARY 120/240V CONNECTION. 8. LEVEL 2A SOUND ATTENUATED ENCLOSURE STANDARD WITH GENERATOR.
- 9. DOORS MUST BE ABLE TO OPEN 90 DEG. TO BE REMOVED. DOORS ARE LOCATED ON THE LEFT SIDE OF THE GENERATOR ONLY.
- 10. STUB-UPS: BASE TANK REQUIRES ALL STUB-UPS TO BE IN THE REAR TANK STUB-UP AREA.
- 11. 'A' IS THE STUB UP AREA FOR THE MLCB AND NEUTRAL CONNECTION. 12. SEE DRAWING 0C3850 FOR DUCT REMOVAL. REMOVAL OF FRONT
- DUCT WILL PROVIDE ACCESS TO MUFFLER.
- 13. 120VAC ENGINE BLOCK HEATER.

14. 210 GALLON USEABLE CAPACITY BASETANK STANDARD WITH GENERATOR.

15. MUST ALLOW FREE FLOW OF DISCHARGE AIR AND EXHAUST. SEE SPEC SHEET FOR MINIMUM AIR FLOW AND MAXIMUM RESTRICTION REQUIREMENTS

116. MUST ALLOW FILOW AND MAXIMUM RESTRUCTION REQUIREMENTS AIR FLOW AND MAXIMUM RESTRUCTION REQUIREMENTS. 117. IT IS THE RESPONSIBILITY OF THE INSTALLATION TECHNICIAN TO ENSURE THAT THE GENERATOR INSTALLATION COMPLIES WITH ALL APPLICABLE CODES, STANDARDS, AND REGULATIONS.

WEIGHT DATA (INCLUDES WOODEN SHIPPING SKID) ENCLOSED GENERATOR WITH EMPTY FUEL TANK - TO BE DETERMINED

UNITS: mm [INCHES]

PRELIMINARY

0KW	GENERAC POWER SYSTEMS Waukesha P.I. BIX 8 WAUKESHA, WIS. 53187					
	FILE NAME	NAME 0J2534.DWG S			SIZE B	
	SCALE	NTS	FIRST USE	AT&T		
	DWG NO.				RE∨	
		0J2	534		1	

Attachment 5

Centered on Solutions™

April 11, 2012

Ms. Hollis Redding SBA Communications Corporation One Research Drive, Suite 200C Westborough, MA 01581

Re: SBA - N. Stonington 3 - Proposed Communications Facility

Dear Ms. Redding,

Centek has developed the site and access drive design as part of SBA's D&M Plan. As depicted in the Site Development Plan, sheet C-1.2 of the D&M Plan drawing set, a portion of the new site access drive transverses an area of existing ledge. It is anticipated that minimal ledge will require removal in the field for the access drive construction. The access drive is designed keeping the high side of the drive (northeast side) relatively level with the ledge with the addition of fill to the other side thereby minimizing ledge removal. If field conditions arise requiring some ledge removal, chipping with pneumatic equipment will be utilized.

Feel free to contact me should you have any questions or concerns.

