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July 8, 2021

***VIA ELECTRONIC MAIL
AND FEDERAL EXPRESS***

Ms. Melanie A. Bachman, Esq., Executive Director
Connecticut Siting Council
Ten Franklin Square
New Britain, CT 06051

Re: DOCKET NO. 496

Dear Attorney Bachman:

This office represents Tarpon Towers II, LLC (“Tarpon”). On behalf of Tarpon, and pursuant to § 16-50j-75 *et seq.* of the Regulations of Connecticut State Agencies, I have enclosed one original and fifteen copies of Tarpon’s Development and Management (“D&M”) Plan submission in connection with the above-captioned docket. The submission includes the following:

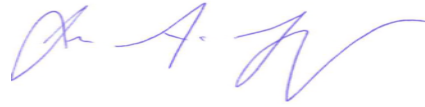
1. Pursuant to Condition 2(a) of the Order, Tarpon has provided a firm commitment from T-Mobile Northeast LLC to install and operate its wireless equipment on the facility approved in Docket No. 496 after completion of construction.
2. Pursuant to Condition 2(b) and (d) of the Order, Tarpon has prepared detailed final site plans, including construction plans in compliance with the *2002 Connecticut Guidelines for Soil Erosion and Sediment Control*.
3. Pursuant to Condition 2(c) of the Order, the enclosed tower and foundation design incorporate a yield point to ensure that the tower setback radius remains within the boundaries of the subject property.
4. Pursuant to Condition 2(e) of the Order, the construction schedule is as follows: Mondays through Friday, 7:30a.m. to 7:30p.m. Tarpon will coordinate with the Town of Windsor, as necessary.

3305216

Ms. Melanie A. Bachman, Esq., Executive Director
Connecticut Siting Council
July 8, 2021
Page 2

Tarpon respectfully requests that the Connecticut Siting Council include this D&M Plan submission for review and approval on the next available agenda. Please do not hesitate to contact me with any questions.

Very truly yours,



Jesse A. Langer

Enclosures

cc: The Honorable Mayor Donald S. Trinks (*via email*)
The Ferraina Company, LLC *c/o* Craig W. Ferraina (*via email*)



*T-Mobile USA
35 Griffin Road South
Bloomfield, CT 06002*

July 7, 2021

***INCLUDED WITH DEVELOPMENT
AND MANAGEMENT PLAN SUBMITTAL***

Ms. Melanie A. Bachman, Esq., Executive Director
Connecticut Siting Council
Ten Franklin Square
New Britain, CT 06051

Re: DOCKET NO. 496

Dear Attorney Bachman:

In accordance with Condition 2(a) of the Connecticut Siting Council's decision in Docket 496, T-Mobile Northeast LLC presents this letter as its firm commitment to install and thereafter operate associated wireless equipment at the facility identified in Docket 496 upon completion of construction.

Brian Paul

Manager, Engineering
Development CT

DocuSigned by:

Brian Paul

6EF34A8D927E49C...

7/7/2021

SURVEY NOTES -

- THIS SURVEY AND MAP HAS BEEN PREPARED IN ACCORDANCE WITH SECTIONS 20-300b-1 THROUGH 20-300b-20 OF THE REGULATIONS OF CONNECTICUT STATE AGENCIES - MINIMUM STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT AS ENDORSED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INC. IT IS A LIMITED PROPERTY/BOUNDARY SURVEY BASED ON A DEPENDENT RESURVEY CONFORMING TO HORIZONTAL ACCURACY CLASS A-2 AND IS INTENDED TO BE USED AS AN IMPROVEMENT LOCATION SURVEY.
EXISTING TOPOGRAPHY, ADJACENT TO THE PROPOSED TOWER, HAS BEEN FIELD VERIFIED ON JULY 11, 2018 AND CONFORMS TO VERTICAL CLASS T-2. UNDERGROUND UTILITIES, WITHIN THIS AREA, HAVE BEEN TAKEN FOR REFERENCE PLAN #2.
- THE PURPOSE OF THIS SURVEY IS TO SUPPORT THE DESIGN AND CONSTRUCTION OF A TELECOMMUNICATION FACILITY. USE OF THIS SURVEY BY ANYONE OTHER THAN TARPON TOWERS II, LLC AND USE OF THIS SURVEY FOR ANY PURPOSE NOT RELATED TO THE DESIGN OF THE INTENDED FACILITY IS STRICTLY PROHIBITED.
- PRIMARY GEODETIC SURVEY CONTROL WAS ESTABLISHED FROM AN ON THE GROUND SURVEY USING THE GLOBAL POSITIONING SYSTEM (GPS) ON JULY 11, 2018. THE HORIZONTAL REFERENCED DATUM IS THE NAD 83 BASED ON THE GRS 80 REFERENCE ELLIPSOID. THE GRID COORDINATES ARE BASED ON THE CONNECTICUT STATE PLANE COORDINATE SYSTEM. ELEVATIONS SHOWN ARE BASED ON NAVD 88. VERTICAL AND HORIZONTAL INFORMATION SHOWN MEETS THE STANDARD CRITERIA FOR AN FAA 1A CERTIFICATION (20± HORIZONTAL AND 3± VERTICAL).
- IN THE EVENT THAT BENCHMARKS, ESTABLISHED FOR THIS PROJECT AND PUBLISHED ON THIS SURVEY, ARE DESTROYED, NOT RECOVERABLE, OR A DISCREPANCY IS FOUND, THE USER SHOULD NOTIFY THIS FIRM IN WRITING PRIOR TO COMMENCING OR CONTINUING ANY WORK.
- A COMMITMENT FOR TITLE INSURANCE PREPARED BY FIDELITY NATIONAL TITLE INSURANCE COMPANY, COMMITMENT 22932067, EFFECTIVE DATE OF COMMITMENT: MAY 16, 2016 WAS USED TO PREPARE THIS SURVEY.
- AREA OF PARCEL "780, 800, 810, 820 & 840 PROSPECT HILL ROAD" = 250,779 SQ. FT. = 5.78 ACRES
- OWNER OF THE SUBJECT PARCEL IS THE FERRAINA COMPANY LLC, DEED REFERENCE VOL. 1212, PAGE 1. PARCEL IS SHOWN ON WINDSOR TAX ASSESSOR'S MAP 17, BLOCK 135, LOT 1, FILE CODE NO. 8300. PARCEL IS ZONED "I"; INDUSTRIAL AS SHOWN ON THE CURRENT ZONING MAP FOR THE TOWN OF WINDSOR.
- NOTHING IN THIS SURVEY IS INTENDED TO EXPRESS AN OPINION REGARDING OWNERSHIP OR TITLE.
- THE SUBJECT PROPERTY IS WITHIN "ZONE X", AN AREA OF MINIMAL FLOOD HAZARD, AS SHOWN ON A PLAN ENTITLED "NATIONAL FLOOD INSURANCE LAYER FIRMETTE 0900300214F, EFFECTIVE DATE: 9/26/2008 TOWN OF WINDSOR 090041". "ZONE X" IS OUTSIDE OF THE 1% ANNUAL CHANCE FLOOD (100 YEAR FLOOD) AND THE 0.2% ANNUAL CHANCE FLOOD AREAS.

REFERENCE PLANS -

- "ALTA/ACSM LAND TITLE SURVEY PREPARED FOR THE FERRAINA COMPANY, LLC 780, 800, 810, 820 AND 840 PROSPECT HILL ROAD, WINDSOR, CONNECTICUT ALFORD ASSOCIATES, INC. CIVIL ENGINEERS 200 PIGEON HILL ROAD, WINDSOR, CT 06095 (860) 688-7288 WILSON M. ALFORD, JR. P.E. & L.S. LICENSE #9344 SCALE: 1 IN.=40 FT. DATE: JUNE 20, 2008"
- "SITE PLAN PREPARED FOR D.M.A.C. LIMITED PARTNERSHIP 780, 800, 820 & 840 PROSPECT HILL ROAD WINDSOR, CONN. SCALE: 1 IN.=40 FT. DATE: MAY 7, 1990 REVISION DATES TO 4/16/99 ALFORD ASSOCIATES, INC., CIVIL ENGINEERS, WINDSOR, CONNECTICUT"

FAA 1-A CERTIFICATION

I HEREBY CERTIFY THAT THE LATITUDE, LONGITUDE, AND ELEVATION PRESENTED HEREON MEET THE REQUIREMENTS OF THE FAA WITH THE FOLLOWING ACCURACIES:

THREE (3) FEET VERTICALLY
TWENTY (20) FEET HORIZONTALLY

Wilson M. Alford, Jr. NOV. 9, 2020
DATE

WILSON M. ALFORD, JR.
CONNECTICUT LICENSED LAND SURVEYOR NO. 9344

NOTE: THE WORD "CERTIFY", AS USED ON THIS PLAN, IS UNDERSTOOD TO BE AN EXPRESSION OF PROFESSIONAL OPINION BY THE LAND SURVEYOR, WHICH IS BASED ON HIS KNOWLEDGE, INFORMATION AND BELIEF. AS SUCH IT CONSTITUTES NEITHER A GUARANTEE OR WARRANTY.

ALONG PROPOSED ACCESS EASEMENT -

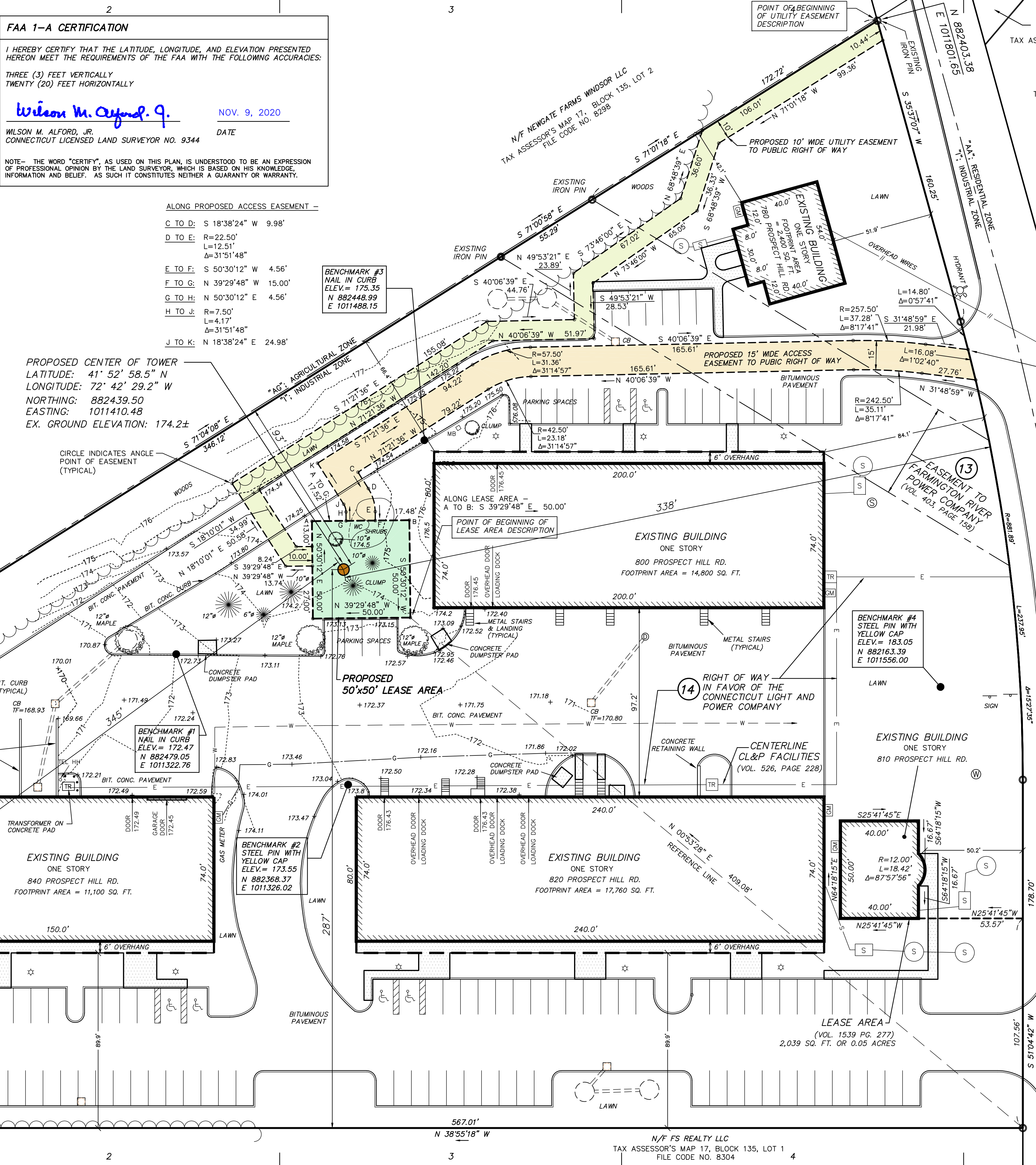
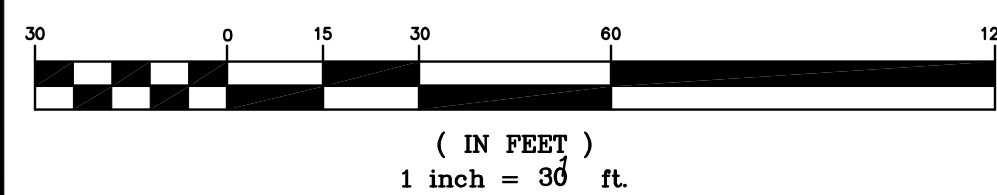
- C TO D: S 18°38'24" W 9.98'
- D TO E: R=22.50'
L=12.51'
Δ=31°51'48"
- E TO F: S 50°30'12" W 4.56'
- F TO G: N 39°29'48" W 15.00'
- G TO H: N 50°30'12" E 4.56'
- H TO J: R=7.50'
L=4.17'
Δ=31°51'48"
- J TO K: N 18°38'24" E 24.98'

PROPOSED CENTER OF TOWER
LATITUDE: 41° 52' 58.5" N
LONGITUDE: 72° 42' 29.2" W
NORTHING: 882439.50
EASTING: 1011410.48
EX. GROUND ELEVATION: 174.2±

CIRCLE INDICATES ANGLE POINT OF EASEMENT (TYPICAL)

THE LOCATIONS OF UNDERGROUND UTILITIES ARE APPROXIMATE AND ALL UTILITIES MAY NOT BE SHOWN. PRIOR TO CONSTRUCTION, CONTRACTOR SHALL CALL 1-800-922-4455 AND HAVE UTILITIES MARKED ON THE GROUND.

GRAPHIC SCALE



- LEGEND -**
- EXISTING IMPROVEMENTS:
- (13) SCHEDULE B - SECTION II EXCEPTION ITEM
 - +171.75 SPOT GRADE GRADES SHOWN AT CURBS REPRESENT GRADE AT BOTTOM OF CURB
 - CONTOUR
 - WC WOOD CHIPS
 - BIT. CONC. BITUMINOUS CONCRETE
 - CONCRETE
 - MB MAIL BOX
 - EDGE OF WOODS
 - 10" HEMLOCK TREE INDICATING CALIPER
 - 10" DECIDUOUS TREE INDICATING CALIPER
 - ☆ LIGHT POLE
 - BOLLARD
 - TR TRANSFORMER ON CONCRETE PAD
 - GM GAS METER
 - GAS SERVICE LINE
 - E ELECTRIC SERVICE
 - o TEL HH TELEPHONE HAND HOLE
 - W WATER SERVICE LINE
 - CB STORM DRAINAGE: CATCH BASIN
 - STORM DRAINAGE LINE
 - TF DRYWELL
 - TF TOP OF FRAME ELEVATION
 - ⊙ STORM MAN HOLE
 - SANITARY SEWER: SEWER LINE
 - SEPTIC TANK

ProTerra
DESIGN GROUP, LLC

4 Bay Road
Bldg A, Suite 200
Hadley, MA 01035
Ph: (413)320-4918

CONSULTANTS:

Alford
ASSOCIATES, INC.

CIVIL ENGINEERS
200 PIGEON HILL ROAD
WINDSOR, CT 06095
860-688-7288
WILSON M. ALFORD, JR., P.E. & L.S.
LICENSE NO. 9344

NO.	DATE	REVISIONS
A	08/31/18	ISSUED FOR REVIEW
B	10/02/18	ISSUED FOR REVIEW
C	10/11/18	ISSUED FOR REVIEW
D	11-10-19	REVISED SURVEY NOTE 1
E	11-9-20	UPDATED OWNERS OF PARCELS ID/FILE CODE #6307 & #6308

SITE NAME: WINDSOR
SITE NUMBER: CT 1209
ADDRESS: 800 PROSPECT HILL ROAD
WINDSOR, CT 06095
TOWER OWNER:
TARPON TOWERS II, LLC
1001 3RD AVENUE WEST
SUITE #20
TARPON TOWERS II, LLC BRADENTON, FL 34205



TO THE BEST OF MY KNOWLEDGE AND BELIEF THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON.
L.S. NO. 9344

DATE: 08/31/18
DRAWN: MRH
CHECK: WMA
SCALE: 1 IN. = 30 FT.
JOB NO.: 18-049

SHEET TITLE:
IMPROVEMENT LOCATION SURVEY


C-1

CONSULTANTS:


NO.	DATE	REVISIONS
A	05/27/21	ISSUED FOR REVIEW
0	06/21/21	DEVELOPMENT & MANAGEMENT PLAN
1	06/25/21	REVISED DEVELOPMENT & MANAGEMENT PLAN

SITE NAME: WINDSOR
SITE NUMBER: CT 1209
ADDRESS: 800 PROSPECT HILL ROAD WINDSOR, CT 06095

TOWER OWNER:
TARPON TOWERS II, LLC
1001 3RD AVENUE WEST SUITE 420
BRADENTON, FL 34205



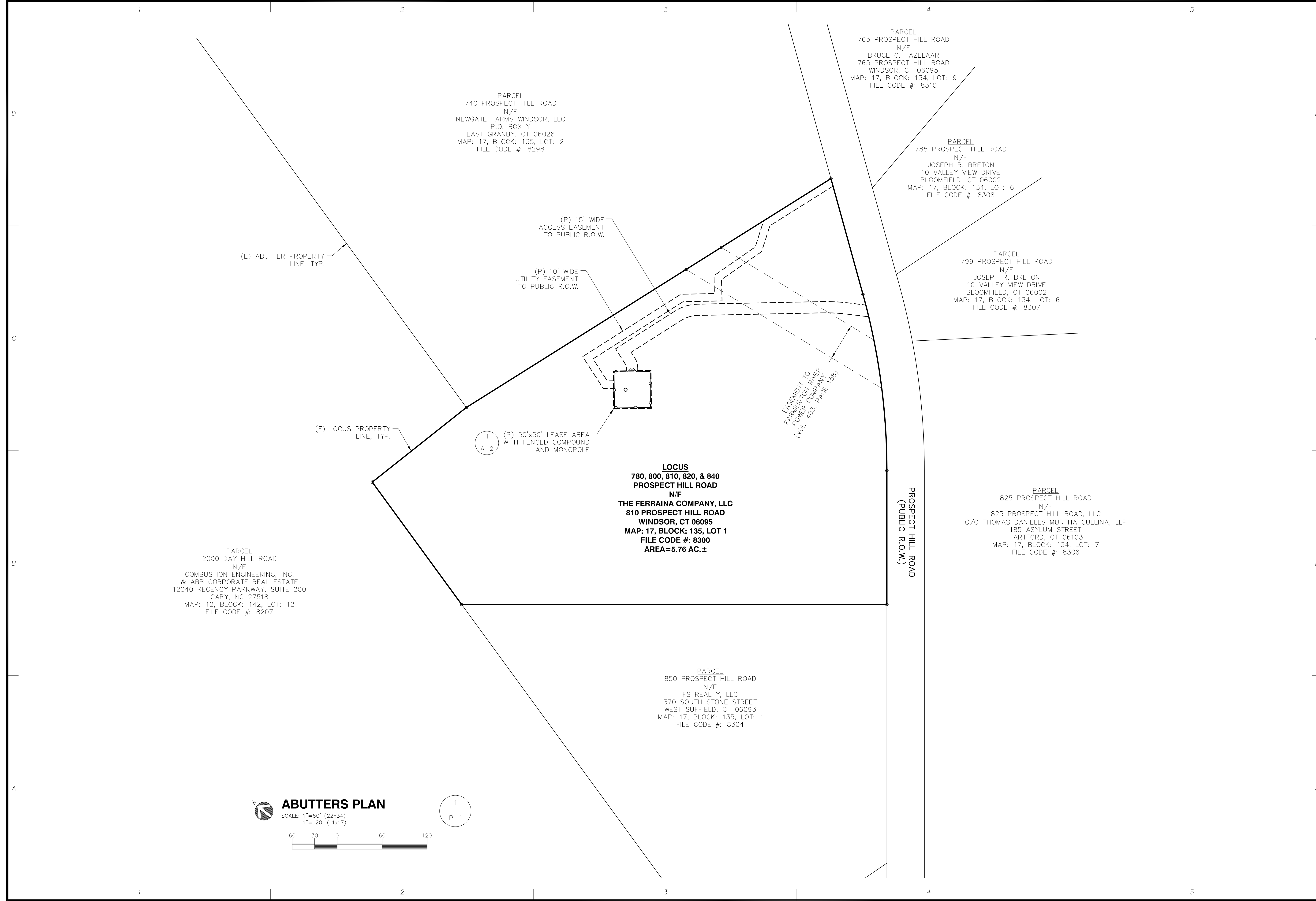
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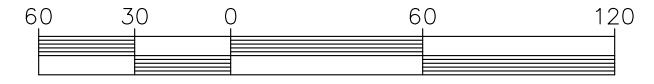
DATE: 06/25/21
DRAWN: BLM
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 18-049
SHEET TITLE:

DATE: 06/25/21
DRAWN: BLM
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 18-049
SHEET TITLE:

ABUTTERS PLAN
P-1



ABUTTERS PLAN
SCALE: 1"=60' (22x34)
1"=120' (11x17)

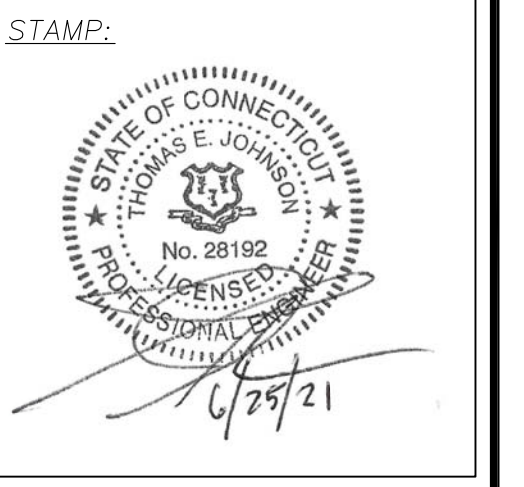


1
P-1

CONSULTANTS:

NO.	DATE	REVISIONS
A	10/5/21	ISSUED FOR REVIEW
0	06/21/21	DEVELOPMENT & MANAGEMENT PLAN
1	06/25/21	REVISED DEVELOPMENT & MANAGEMENT PLAN

TITLE:
SITE NAME: WINDSOR
SITE NUMBER: CT 1209
ADDRESS: 800 PROSPECT HILL ROAD WINDSOR, CT 06095
 TOWER OWNER:
TARPON TOWERS II, LLC
1001 3RD AVENUE WEST SUITE 420 BRADENTON, FL 34205

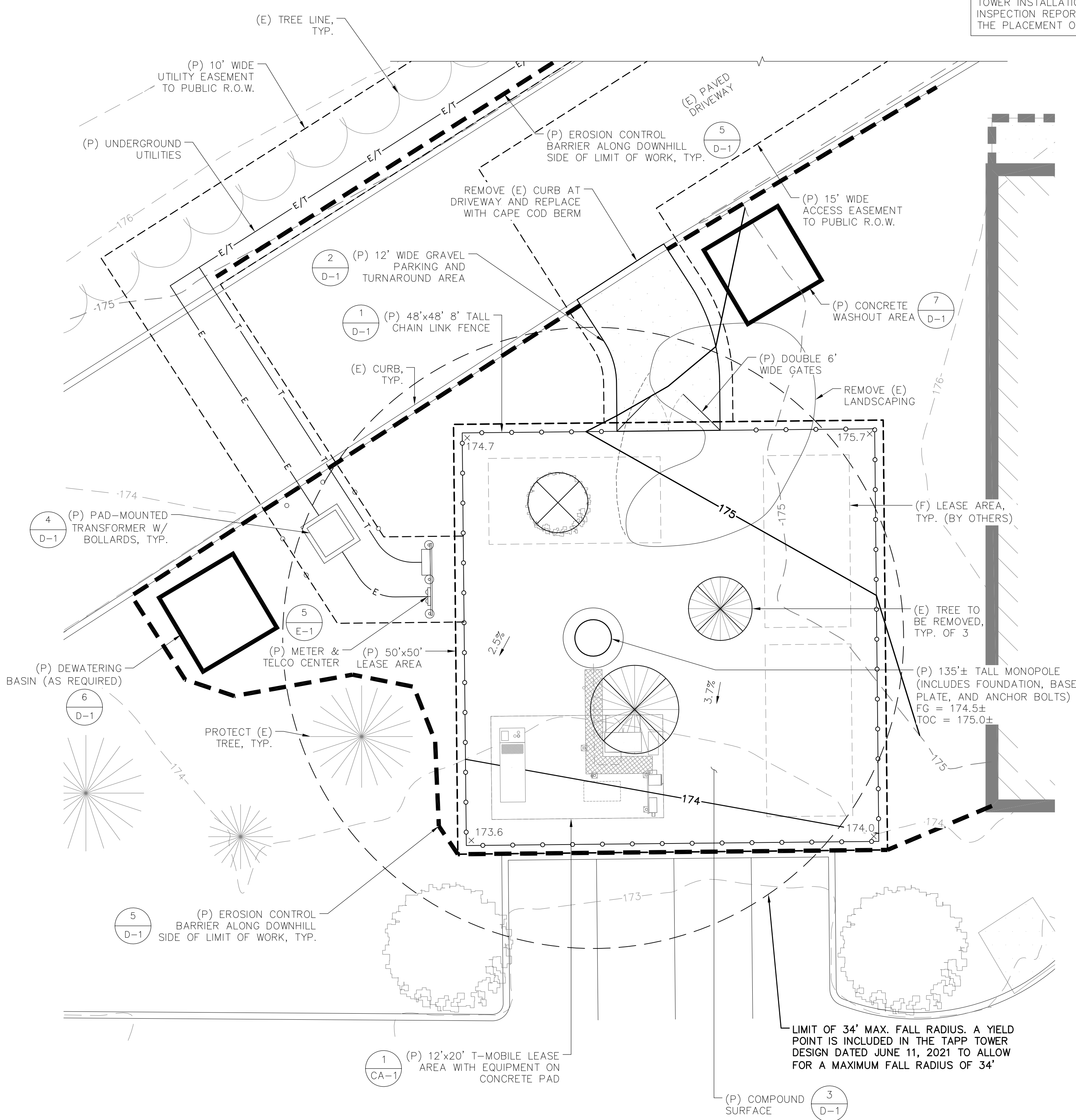


DATE: 06/25/21
 DRAWN: BLM
 CHECK: JMM/TEJ
 SCALE: SEE PLAN
 JOB NO.: 18-049
 SHEET TITLE:

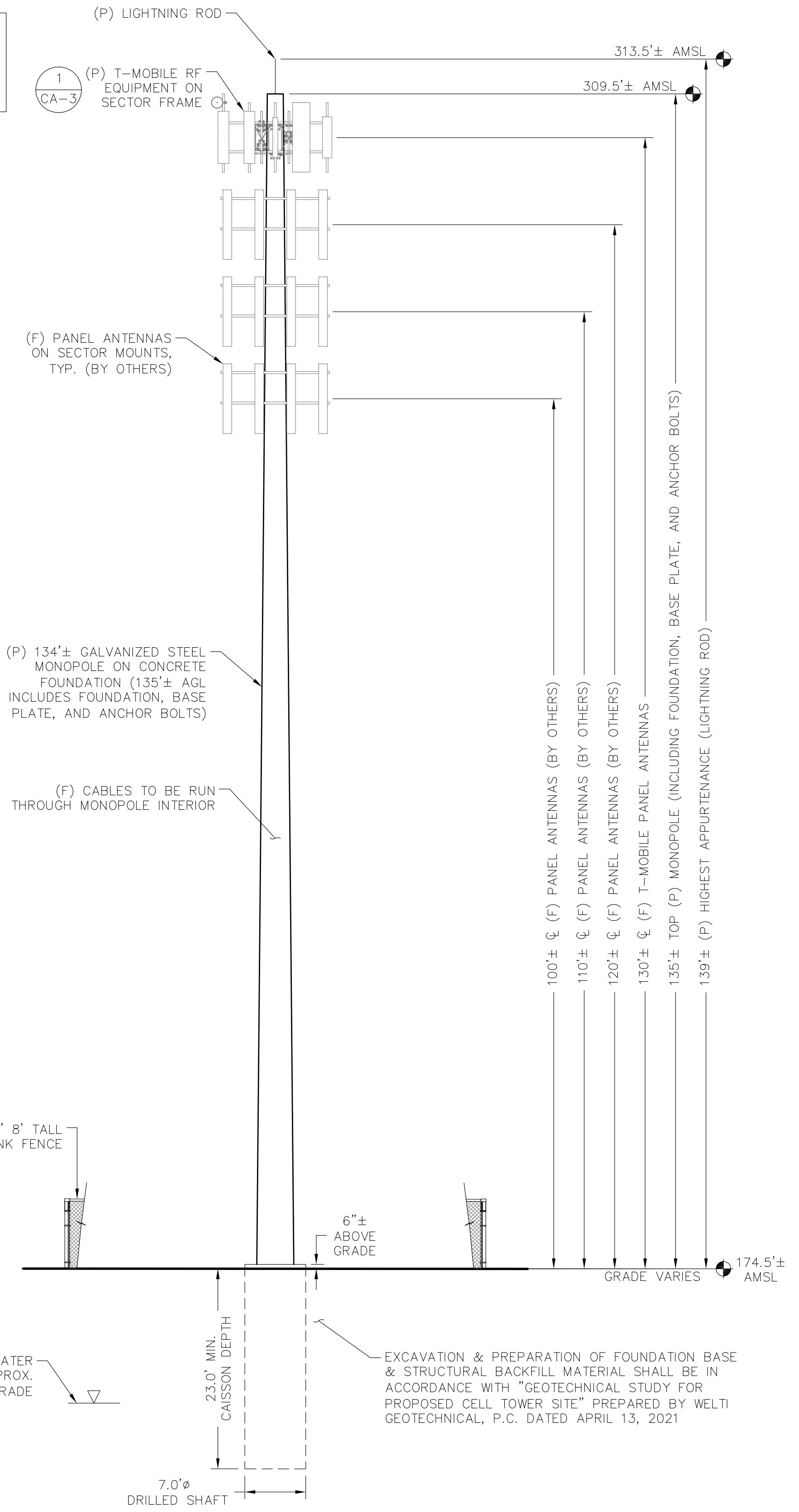
COMPOUND PLAN & ELEVATION

A-2

REFER TO THE NEW TAPP TOWER DESIGN PREPARED FOR THIS PROJECT DATED JUNE 11, 2021 FOR FURTHER INFORMATION ON THE PROPOSED STRUCTURE. TOWER INSTALLATION AND POST MODIFICATION INSPECTION REPORT SHALL BE COMPLETED PRIOR TO THE PLACEMENT OF FUTURE CARRIER EQUIPMENT.



COMPOUND PLAN
 SCALE: 1"=8' (22x34)
 1"=16' (11x17)



SOUTHEAST ELEVATION
 SCALE: 1"=10' (22x34)
 1"=20' (11x17)

LIMIT OF 34' MAX. FALL RADIUS. A YIELD POINT IS INCLUDED IN THE TAPP TOWER DESIGN DATED JUNE 11, 2021 TO ALLOW FOR A MAXIMUM FALL RADIUS OF 34'

EXCAVATION & PREPARATION OF FOUNDATION BASE & STRUCTURAL BACKFILL MATERIAL SHALL BE IN ACCORDANCE WITH "GEOTECHNICAL STUDY FOR PROPOSED CELL TOWER SITE" PREPARED BY WELTI GEOTECHNICAL, P.C. DATED APRIL 13, 2021

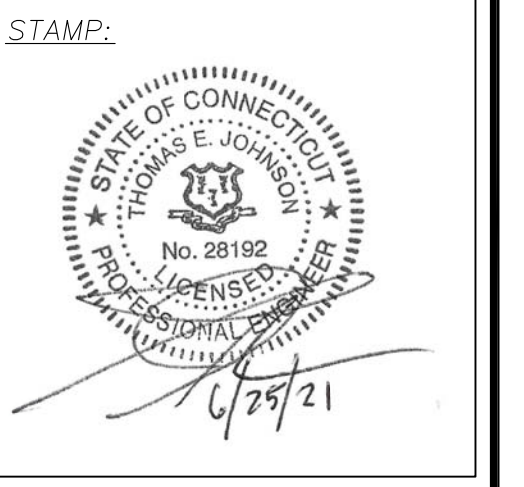
CONSULTANTS:

NO.	DATE	REVISIONS
1	05/27/21	ISSUED FOR REVIEW
0	06/21/21	DEVELOPMENT & MANAGEMENT PLAN
1	06/25/21	REVISED DEVELOPMENT & MANAGEMENT PLAN

TITLE:

SITE NAME: WINDSOR
SITE NUMBER: CT 1209
ADDRESS: 800 PROSPECT HILL ROAD WINDSOR, CT 06095

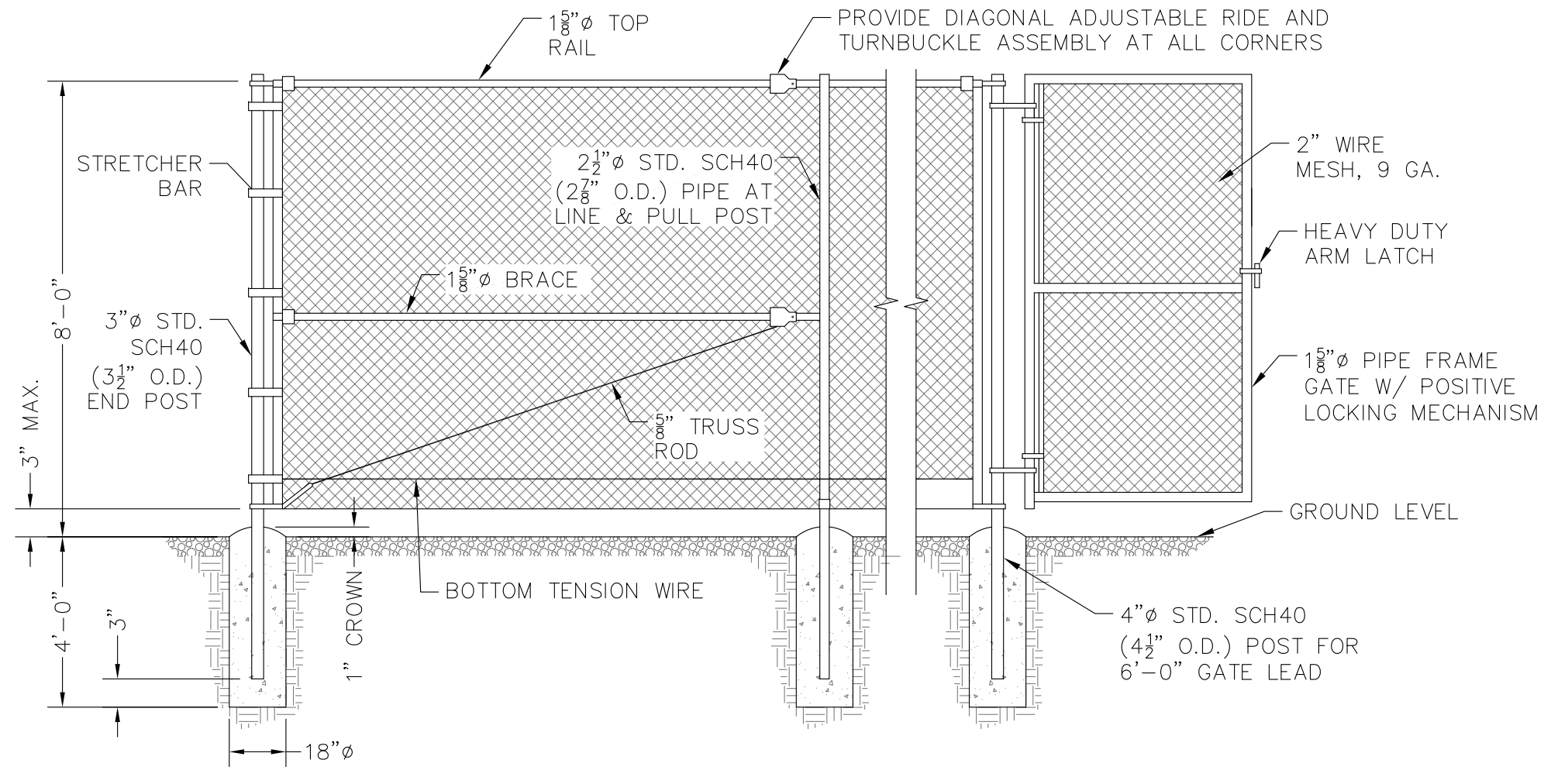
TOWER OWNER:
TARPON TOWERS II, LLC
1001 3RD AVENUE WEST SUITE 420 BRADENTON, FL 34205



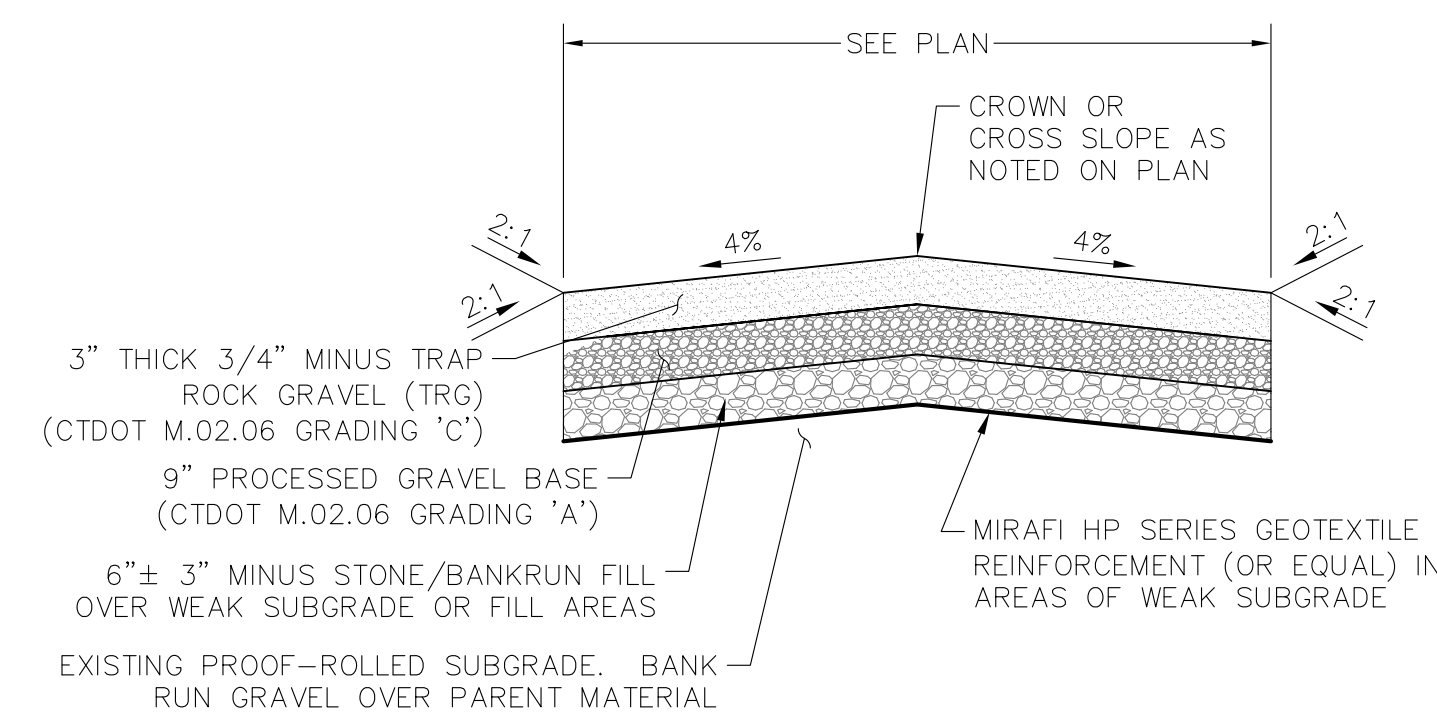
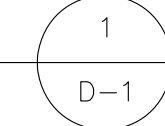
DATE: 06/25/21
DRAWN: BLM
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 18-049
SHEET TITLE:

DETAILS

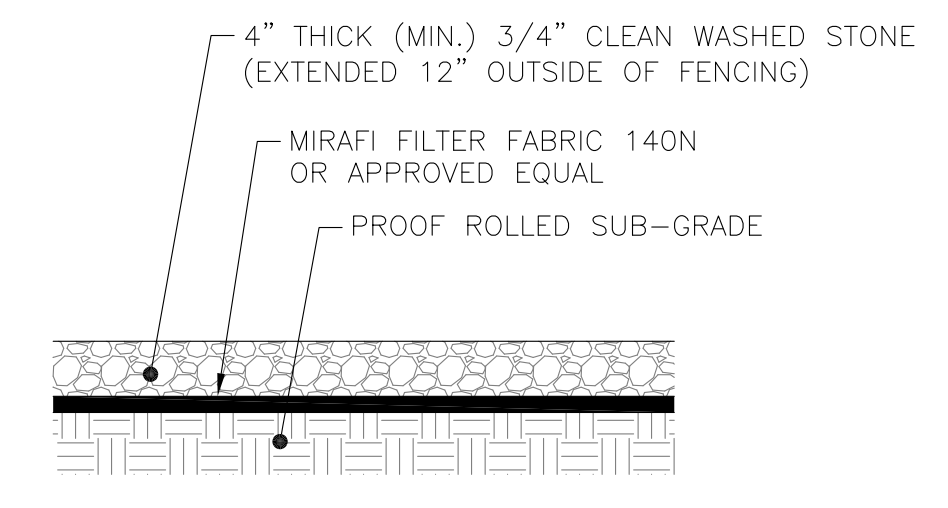
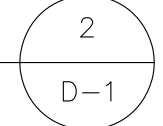
D-1



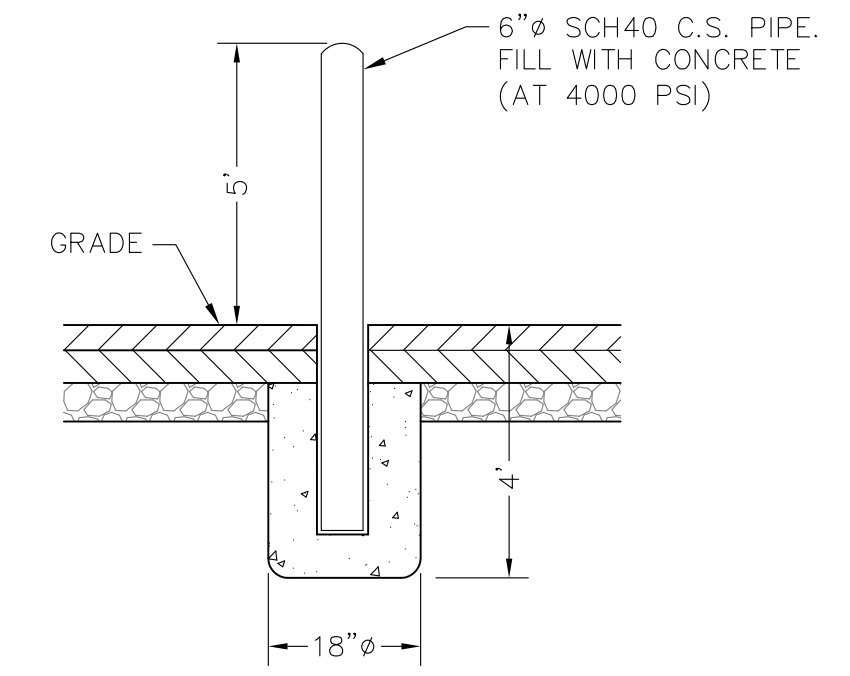
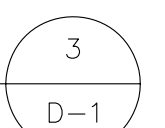
CHAIN LINK FENCE
SCALE: NONE



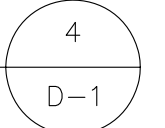
DRIVEWAY SECTION
SCALE: NONE



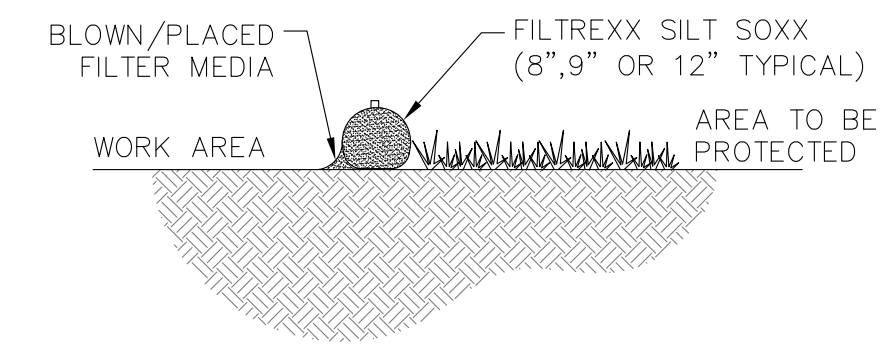
COMPOUND SURFACE
SCALE: NONE



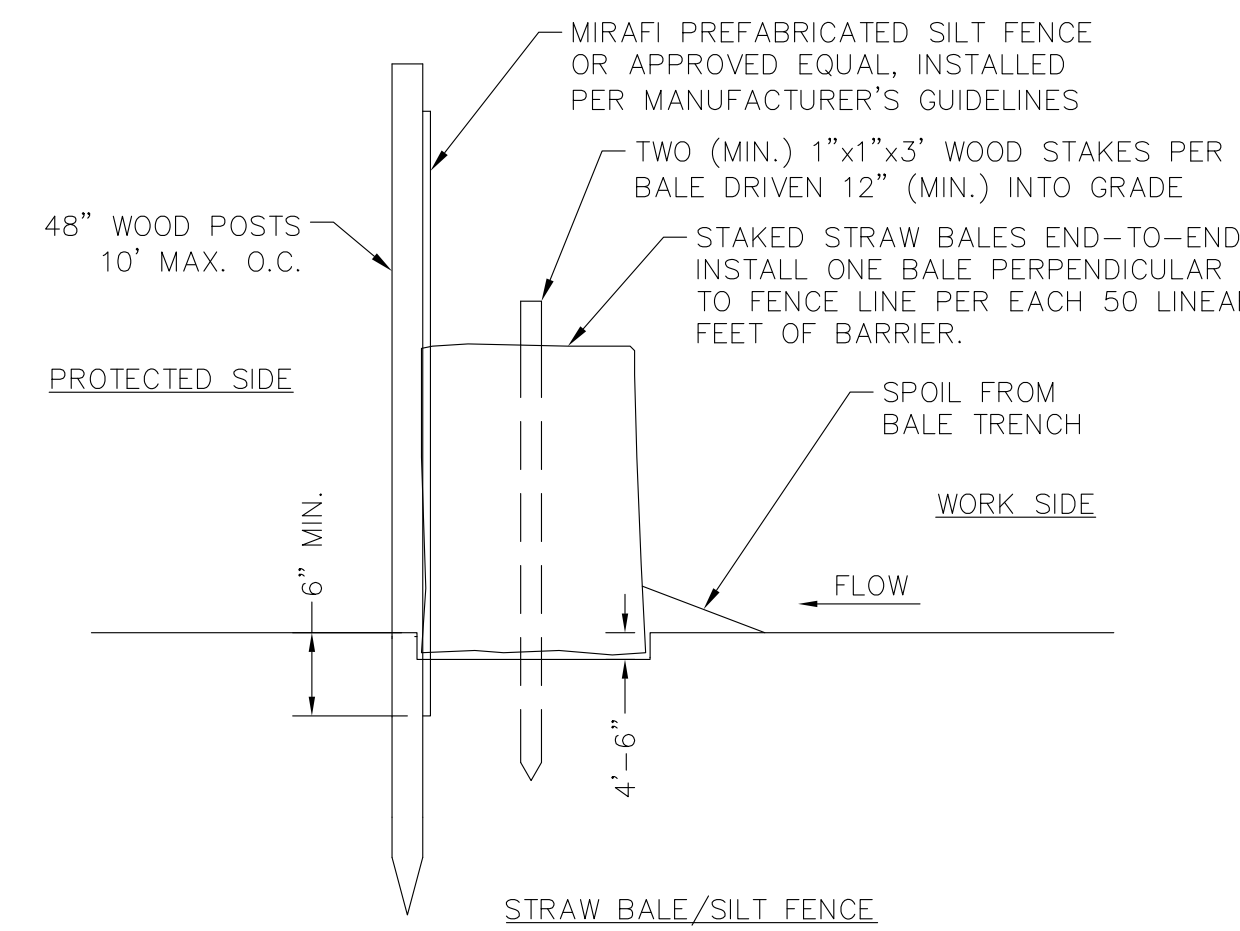
BOLLARD
SCALE: NONE



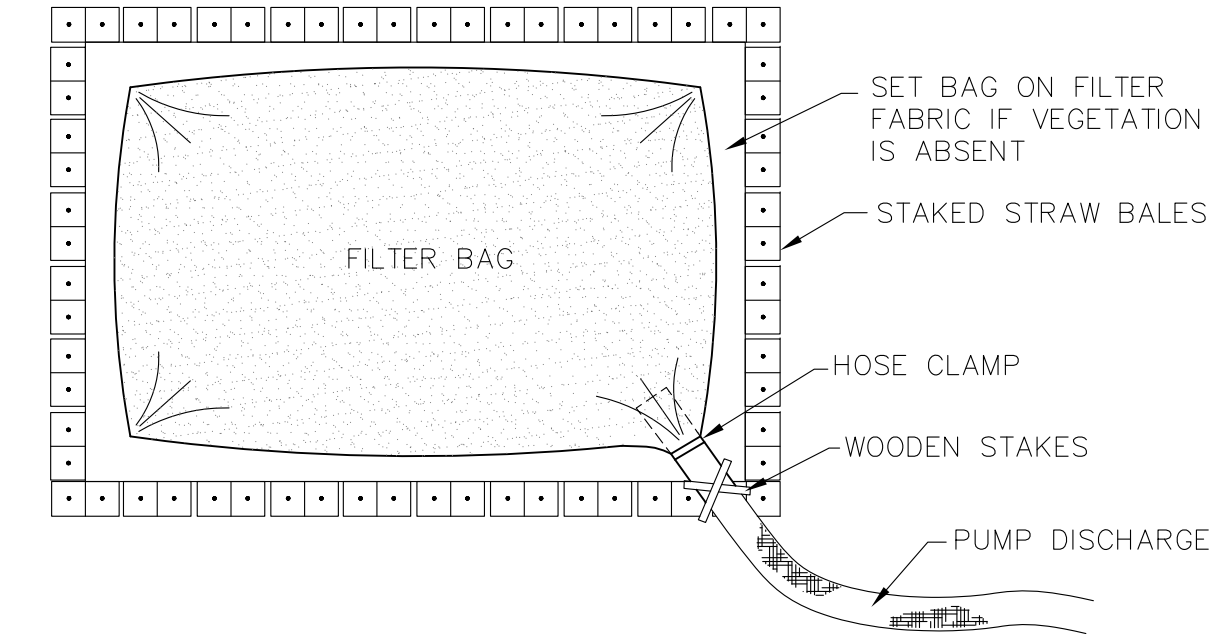
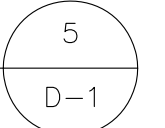
- NOTES:**
- USE SILT SOXX WHERE CONDITIONS DO NOT ALLOW STAKES TO BE DRIVEN.
 - STRAW BALES TO BE TIED W/BIODEGRADABLE TWINE.
 - SILT SOXX FILL TO MEET FILTREXX SPECIFICATIONS AND APPLICATION REQUIREMENTS.
 - SILT SOXX COMPOST MATERIAL TO BE DISPERSED ON SITE, AS DETERMINED BY ENGINEER.



SILT SOXX

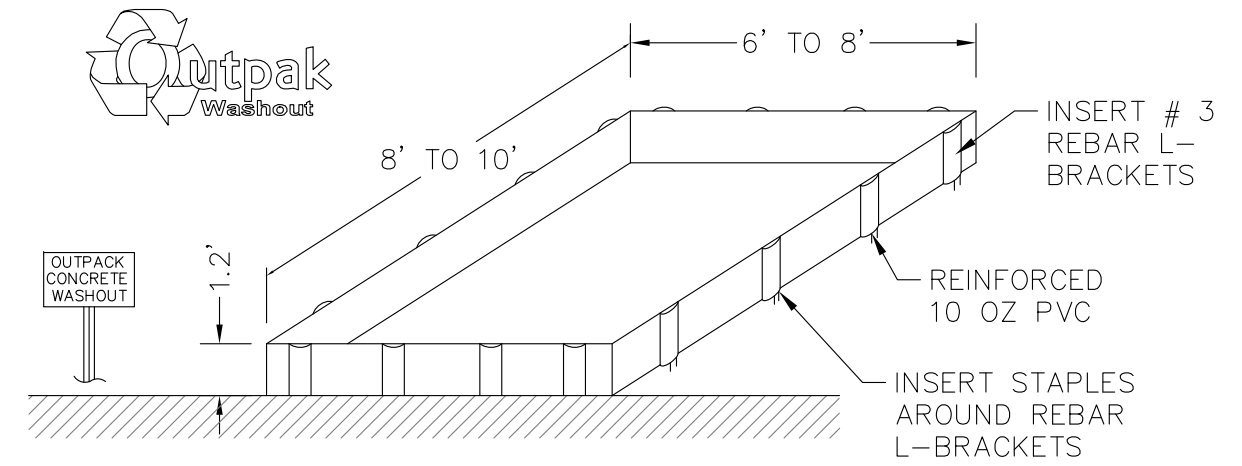
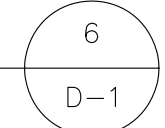


EROSION CONTROL BARRIER
SCALE: NONE



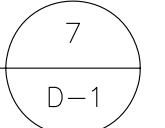
- NOTES:**
- ENSURE DISCHARGE AREA IS COVERED BY STABLE VEGETATION.
 - USE DIFFUSER NOZZLE OR LOW DISCHARGE RATE TO PREVENT SCOURING.
 - TO BE PLACED AT AN UPLAND LOCATION THAT WILL ALLOW WATER TO DRAIN TO THE GROUND.
 - SIZE OF STRAW BALE ENCLOSURE TO BE 10'x10' ADJUSTED TO WATER VOLUME.
 - ADDITIONAL STRAW BALES MAY BE USED TO INCREASE RETENTION & FILTERING.

DEWATERING BASIN
SCALE: NONE



- NOTES:**
- THE CONCRETE WASHOUT AREA SHALL BE INSTALLED PRIOR TO ANY CONCRETE PLACEMENT ON THIS PROJECT.
 - SIGNS SHALL BE PLACED AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF THE CONCRETE WASHOUT.
 - THE CONCRETE WASHOUT AREA WILL BE REPLACED AS NECESSARY TO MAINTAIN CAPACITY FOR WASTE CONCRETE AND OTHER LIQUID WASTE.
 - WASHOUT RESIDUE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF AT AN APPROVED WASTE SITE.
 - DO NOT MIX EXCESS AMOUNTS OF FRESH CONCRETE OR CEMENT ON-SITE.
 - DO NOT WASH OUT CONCRETE TRUCKS INTO STORM DRAINS, OPEN DITCHES, STREETS, OR STREAMS.
 - AVOID DUMPING EXCESS CONCRETE IN NON-DESIGNATED DUMPING AREAS.
 - LOCATE WASHOUT AREA AT LEAST 50' (15 METERS) FROM STORM DRAINS, OPEN DITCHES, OR WATERBODIES.
 - WASH OUT WASTES INTO THE OUTPACK WASHOUT AS SHOWN WHERE THE CONCRETE CAN SET, BE BROKEN UP, AND THEN DISPOSED OF PROPERLY.

CONCRETE WASHOUT AREA
SCALE: NONE



CONSULTANTS:

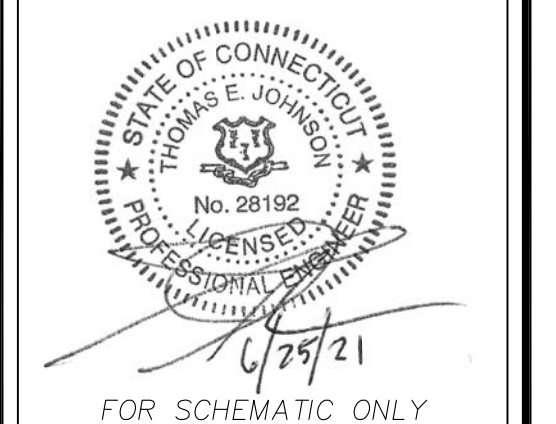
NO.	DATE	REVISIONS
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0	06/21/21	DEVELOPMENT & MANAGEMENT PLAN
1	06/25/21	REVISED DEVELOPMENT & MANAGEMENT PLAN

SITE NAME: WINDSOR
SITE NUMBER: CT 1209
ADDRESS: 800 PROSPECT HILL ROAD WINDSOR, CT 06095

TOWER OWNER:
TARPON TOWERS II, LLC
1001 3RD AVENUE WEST SUITE 420
BRADENTON, FL 34205



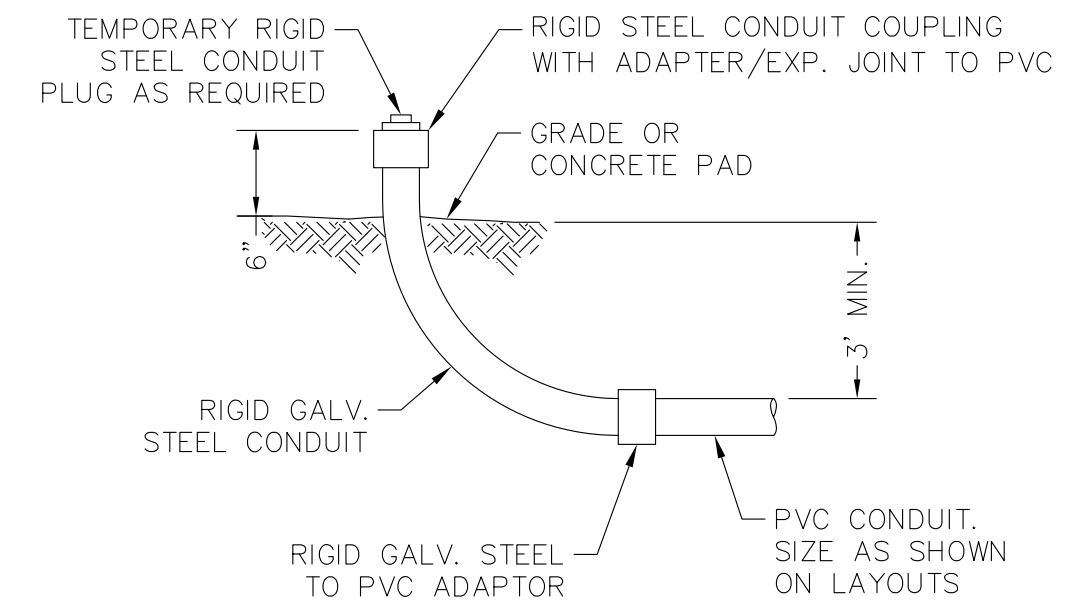
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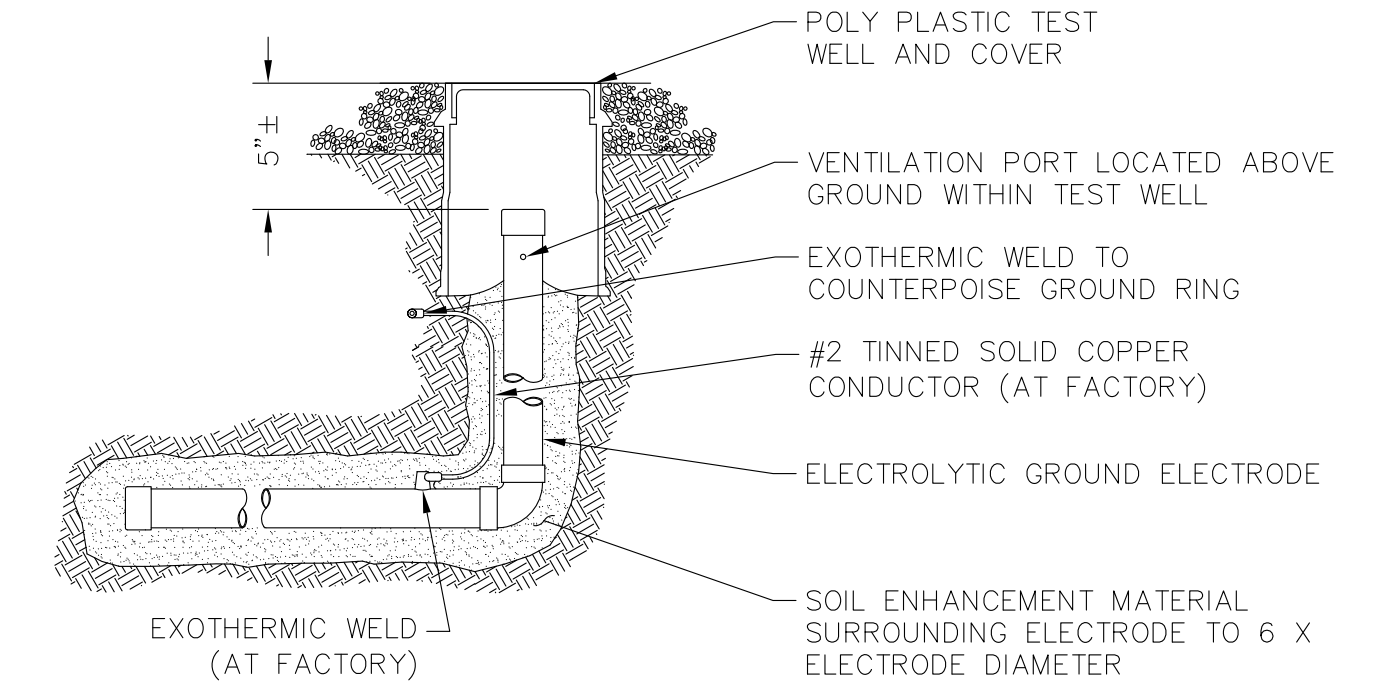
DATE: 06/25/21
DRAWN: BLM
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 18-049
SHEET TITLE:

ELECTRICAL & GROUNDING DETAILS

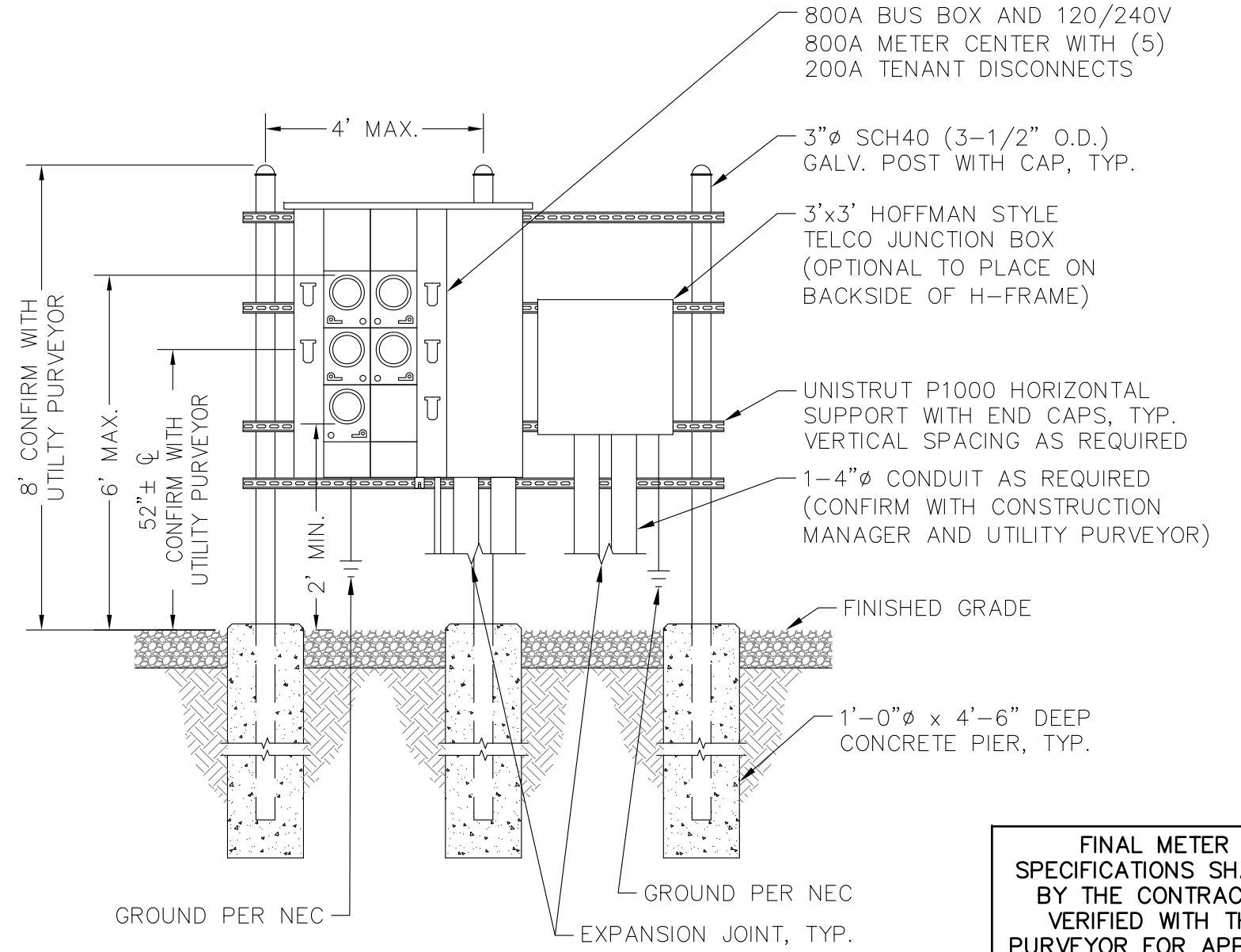
E-1



STUB-UP CONDUIT DETAIL
SCALE: NONE

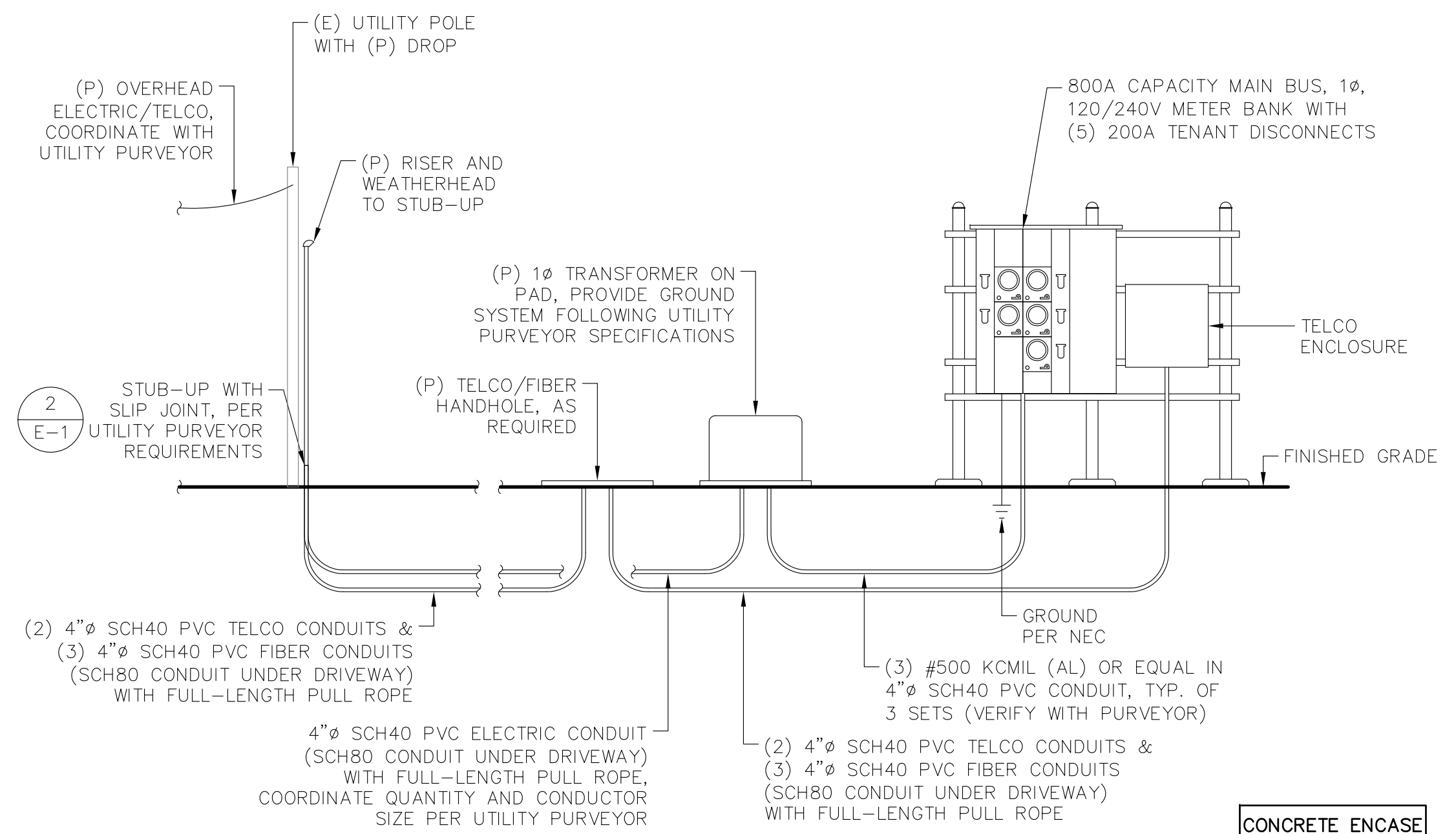


ELECTROLYTIC GROUND ELECTRODE (OPTIONAL)
SCALE: NONE



TENANT METER CENTER
SCALE: NONE

FINAL METER CENTER SPECIFICATIONS SHALL BE SENT BY THE CONTRACTOR TO BE VERIFIED WITH THE UTILITY PURVEYOR FOR APPROVAL PRIOR TO ORDERING EQUIPMENT.



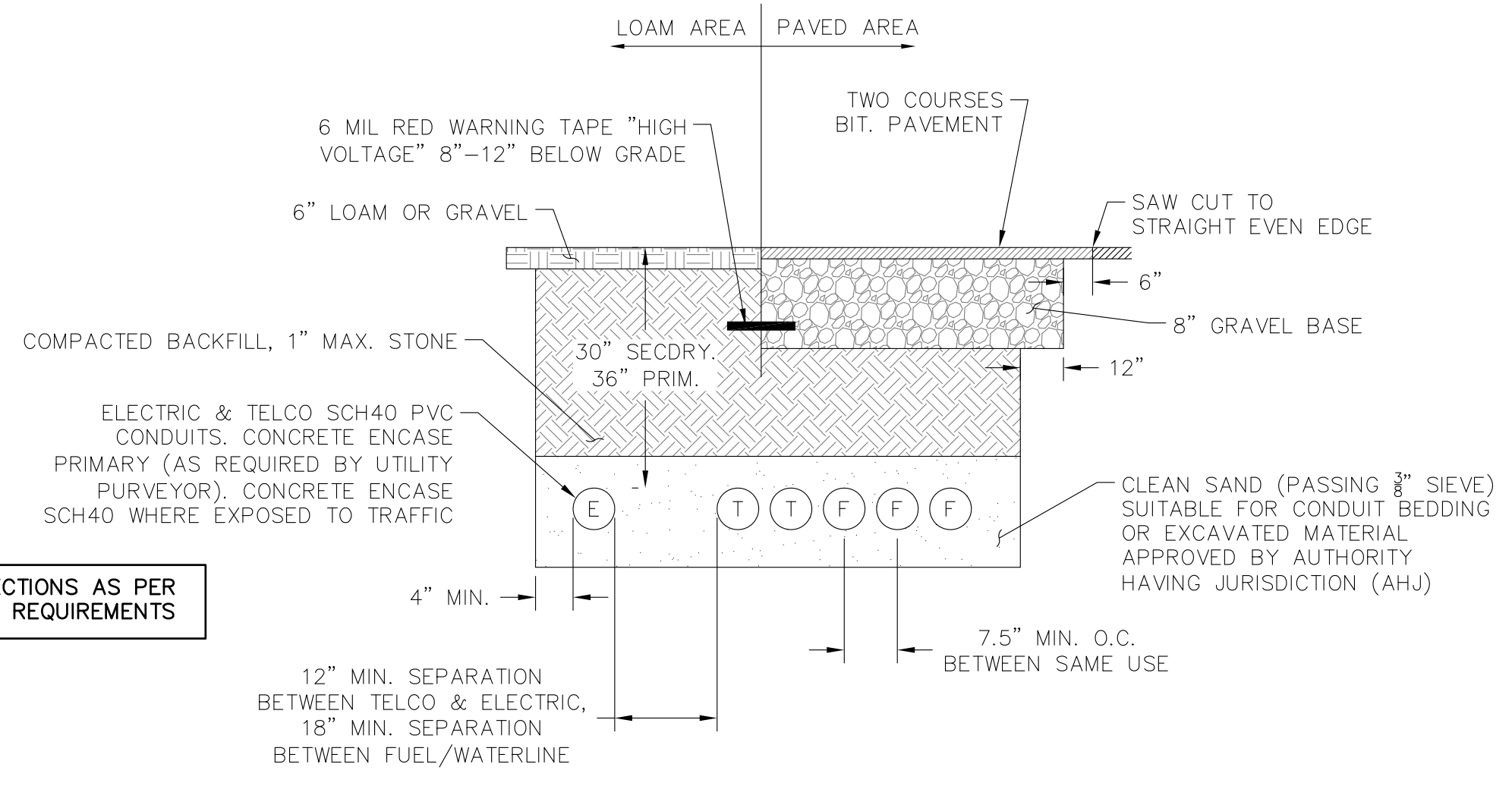
UTILITY RISER SCHEMATIC
SCALE: NONE

SITE WALK AND COORDINATION WITH UTILITY COMPANIES ARE REQUIRED PRIOR TO CONSTRUCTION BY CONSTRUCTION MANAGER

UTILITY RISER SCHEMATIC SUBJECT TO A FINAL UTILITY DESIGN TO BE COMPLETED BY THE LOCAL UTILITY COMPANY

MAKE ALL CONNECTIONS AS PER UTILITY COMPANY REQUIREMENTS AND NEC

CONCRETE ENCASE SCH80 CONDUITS WHERE EXPOSED TO TRAFFIC.



- NOTES
1. MAKE ALL CONNECTIONS AS PER UTILITY COMPANY REQUIREMENTS.
 2. VERIFY CONDUIT SIZE WITH UTILITY COMPANY.
 3. CONTRACTOR SHALL FURNISH AND INSTALL AN APPROVED 2,500 POUND TEST TAPE IN EACH PRIMARY CONDUIT RUN OR PER UTILITY COMPANY REQUIREMENTS.

BURIED CONDUIT SECTION
SCALE: NONE

MAKE ALL CONNECTIONS AS PER UTILITY COMPANY REQUIREMENTS

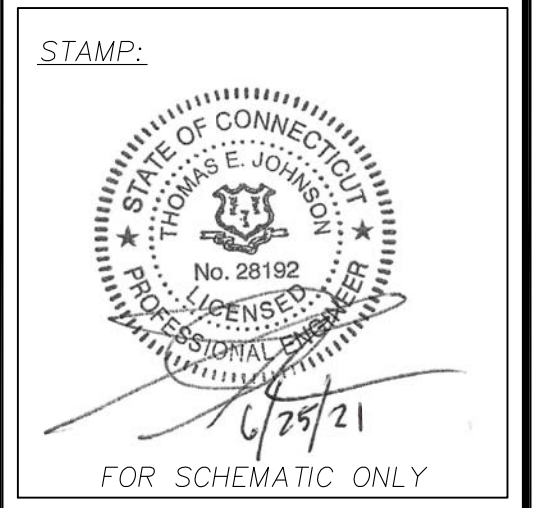
CONSULTANTS:

NO.	DATE	REVISIONS
1	05/27/21	ISSUED FOR REVIEW
0	06/21/21	DEVELOPMENT & MANAGEMENT PLAN
1	06/25/21	REVISED DEVELOPMENT & MANAGEMENT PLAN

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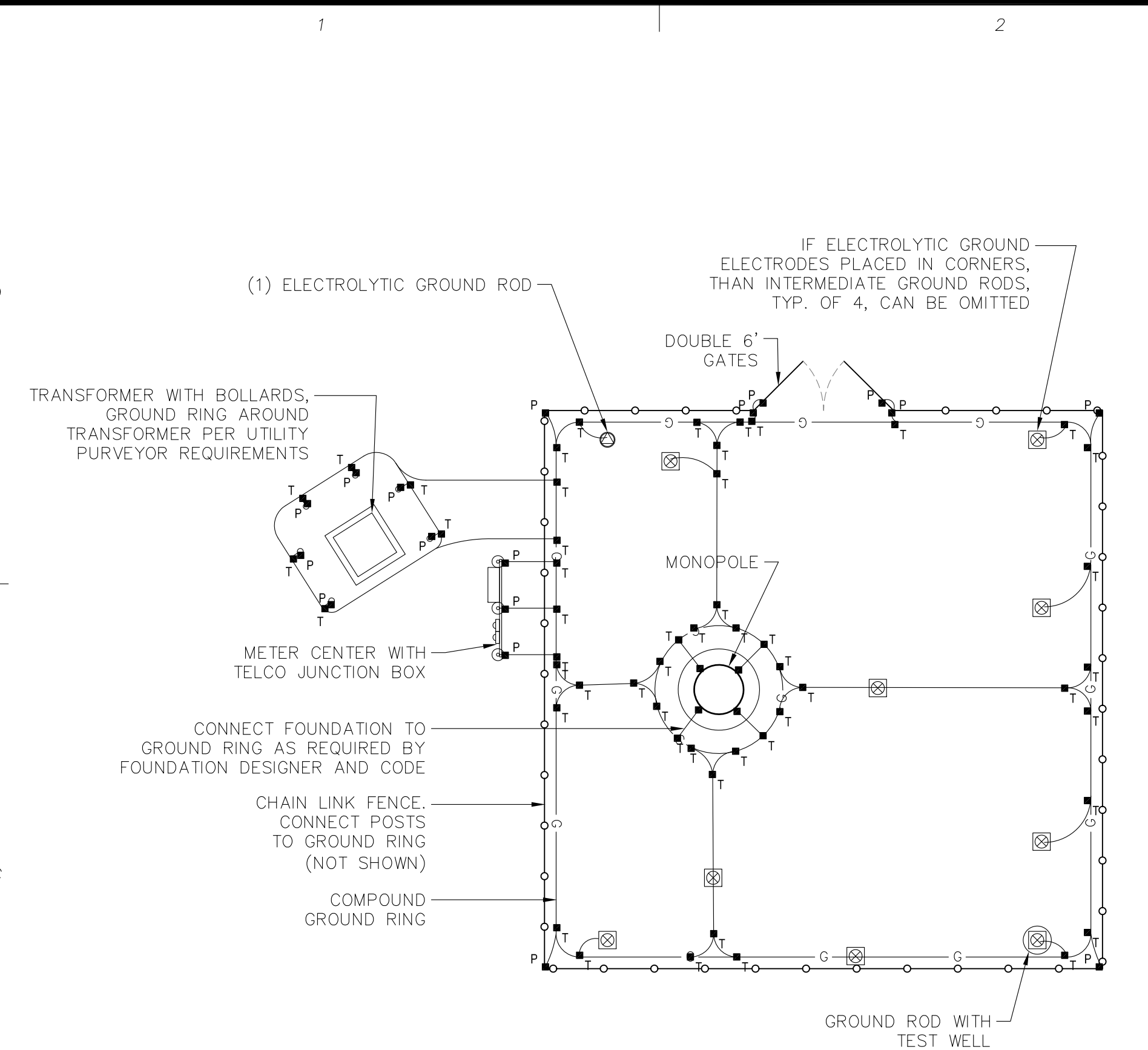
SITE NAME: WINDSOR
SITE NUMBER: CT 1209
ADDRESS: 800 PROSPECT HILL ROAD WINDSOR, CT 06095

TOWER OWNER:
TARPON TOWERS II, LLC
1001 3RD AVENUE WEST SUITE 420 BRADENTON, FL 34205



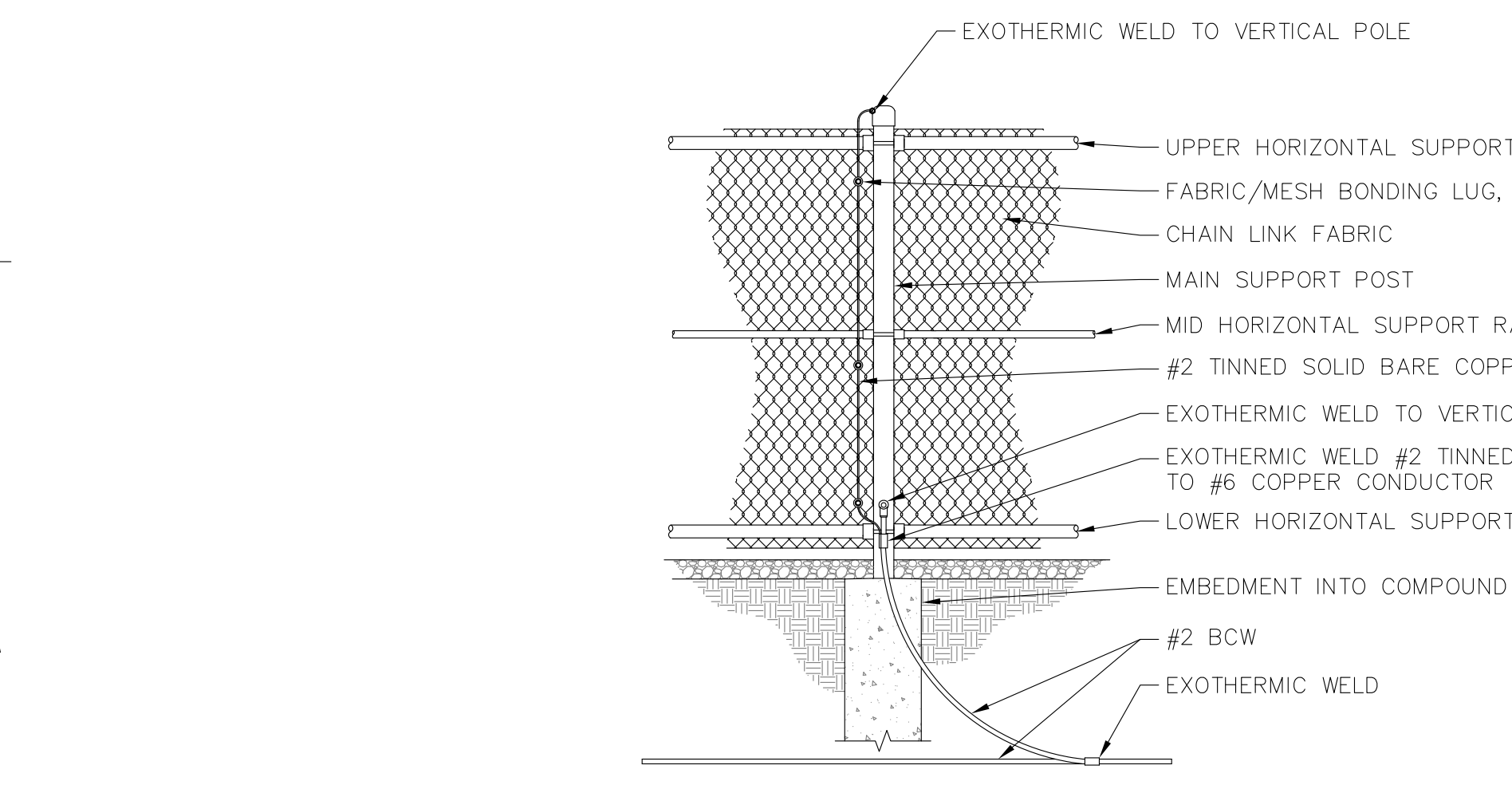
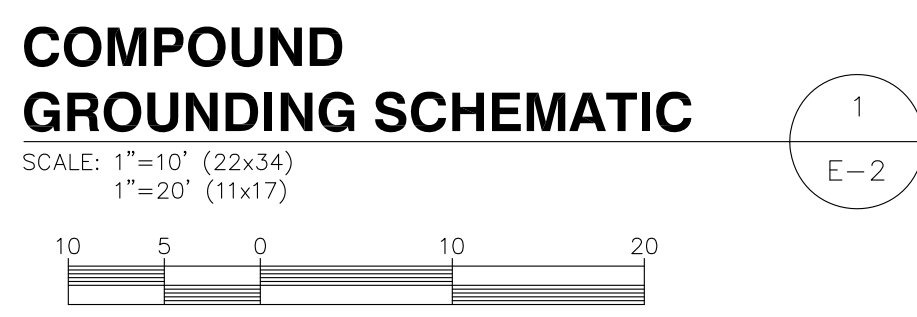
DATE: 06/25/21
DRAWN: BLM
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 18-049

SHEET TITLE:
ELECTRICAL & GROUNDING DETAILS
E-2

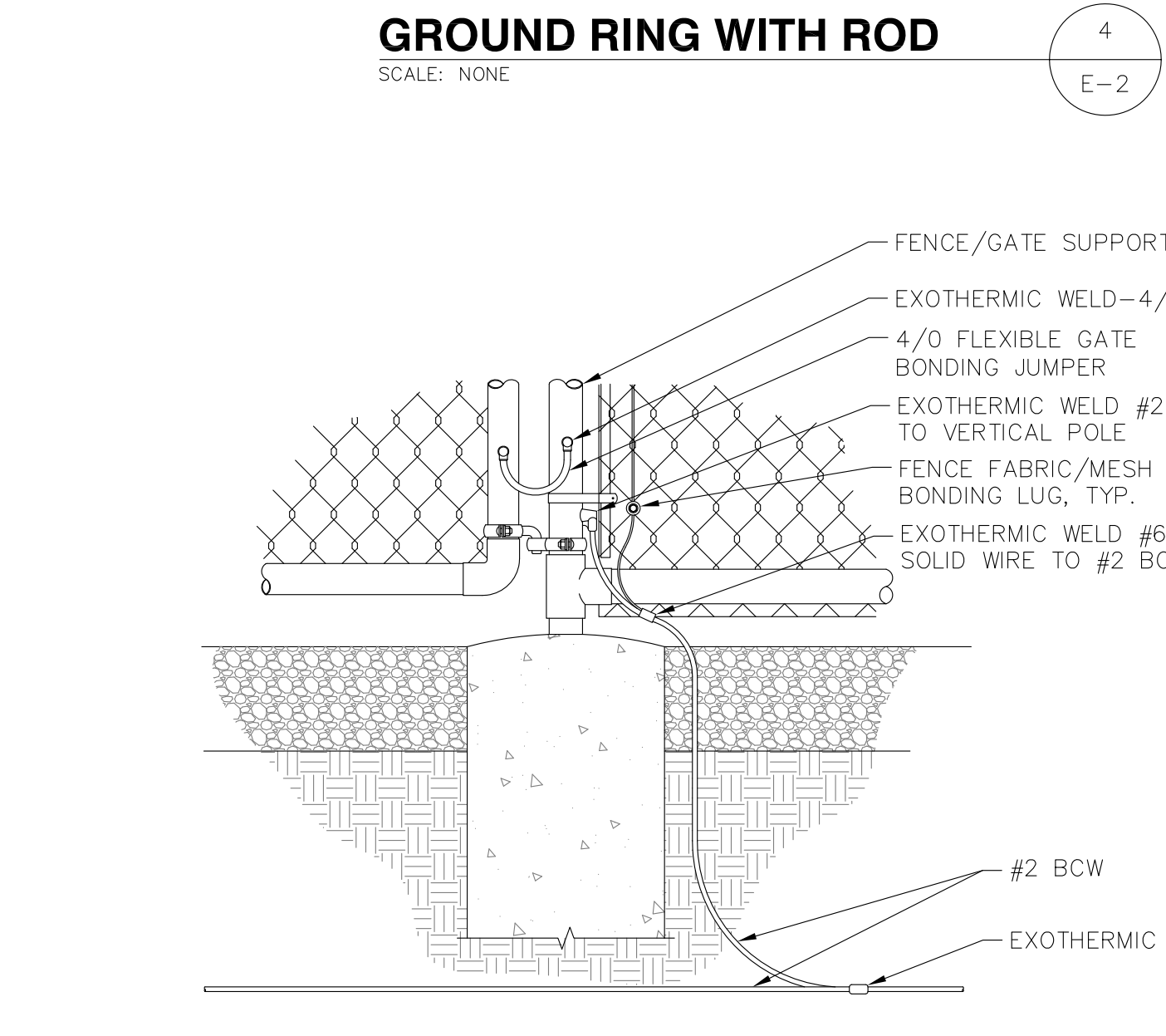
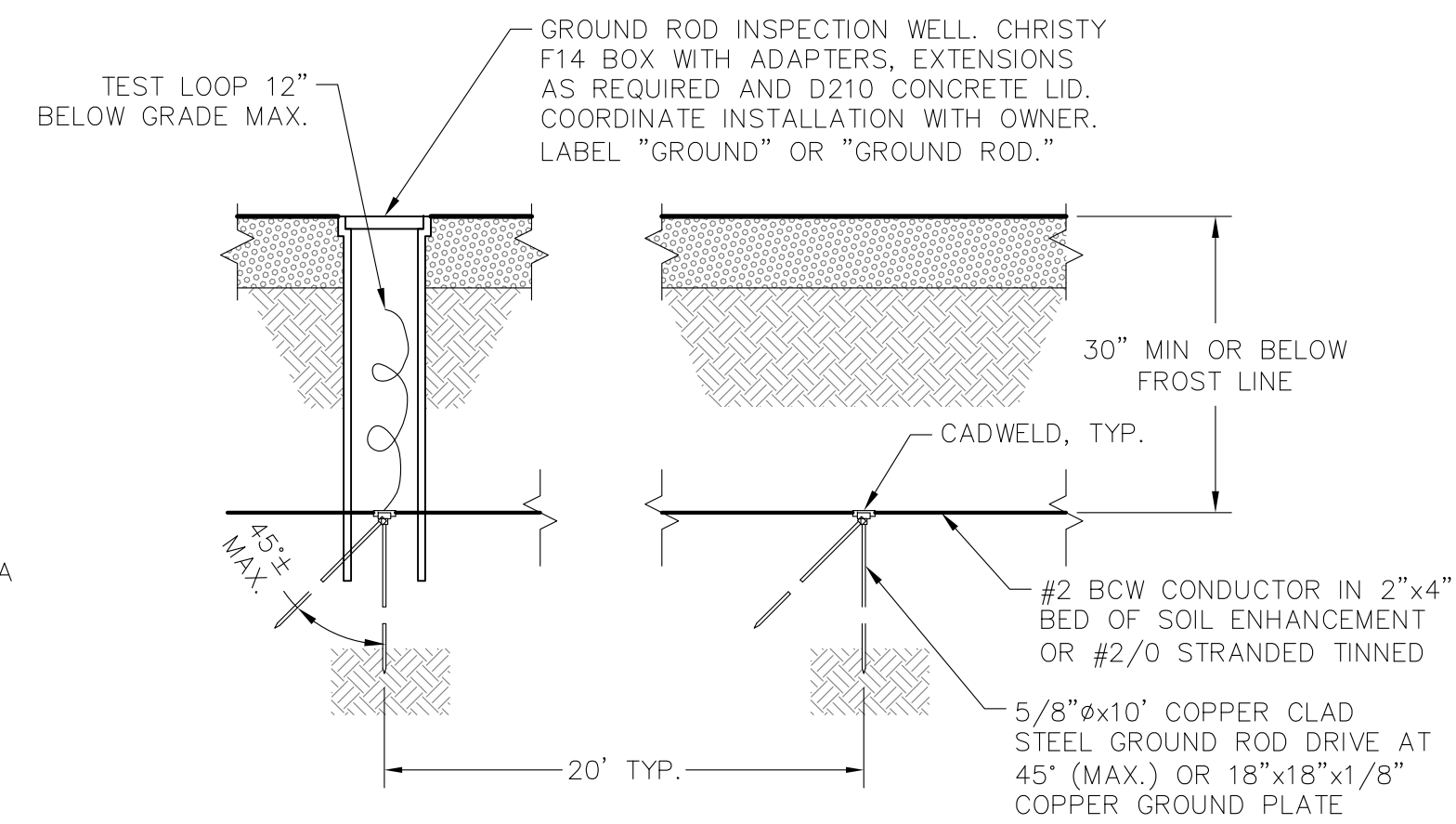
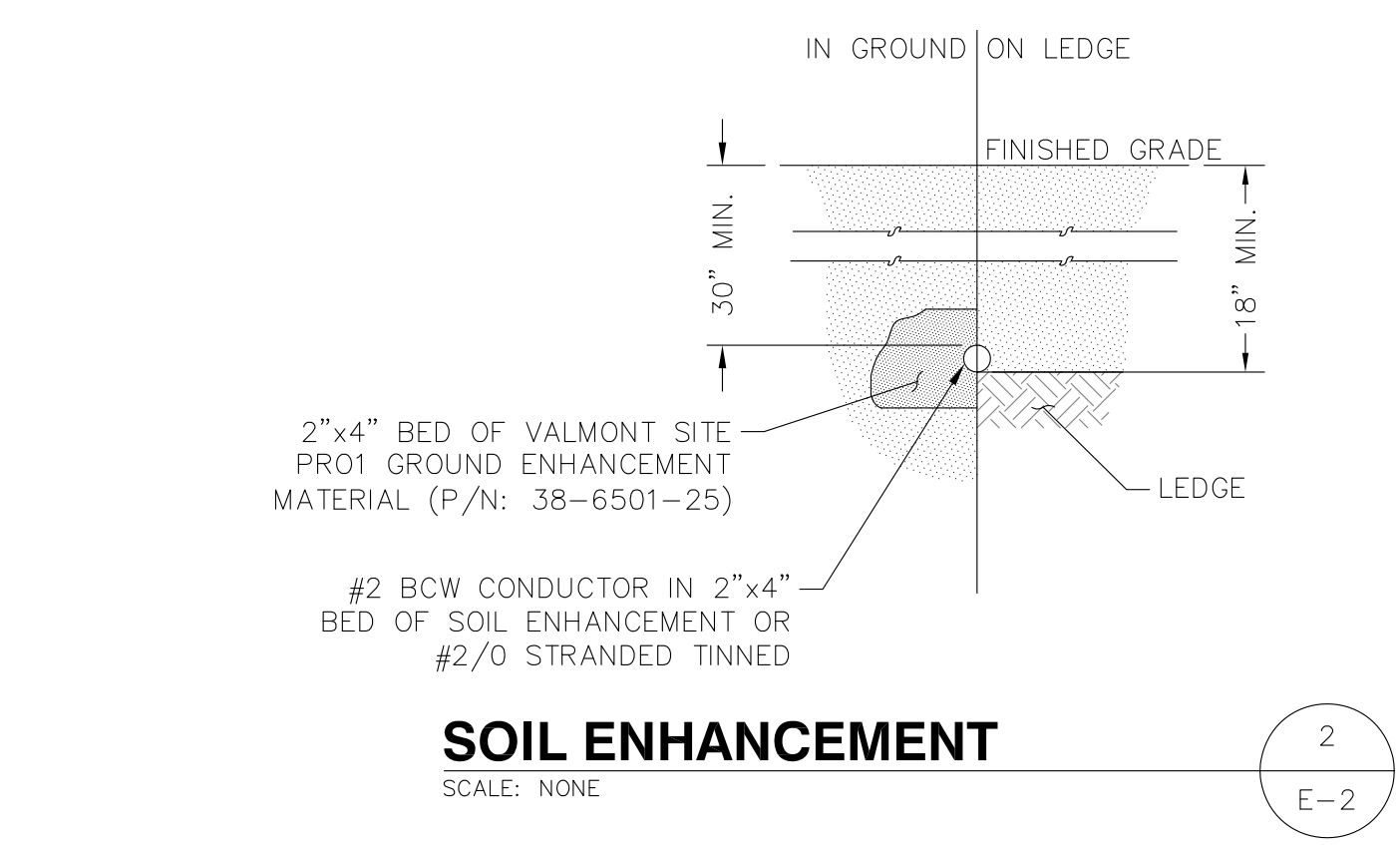


- LEGEND**
- ELECTROLYTIC GROUND ELECTRODE, TYP. 1 MIN. (SEE NOTE)
 - ⊗ 5/8"x10' COPPER CLAD STEEL GROUND ROD, TYP. OF 11
 - EXOTHERMIC WELD #2 BCW TO #2 BCW CONDUCTOR PARALLEL
 - EXOTHERMIC WELD TO #2 BCW CONDUCTOR TO VERTICAL PIPE
 - G — #2 BCW CONDUCTOR IN 2"x4" BED OF SOIL ENHANCEMENT OR #2/0 STRANDED TINNED

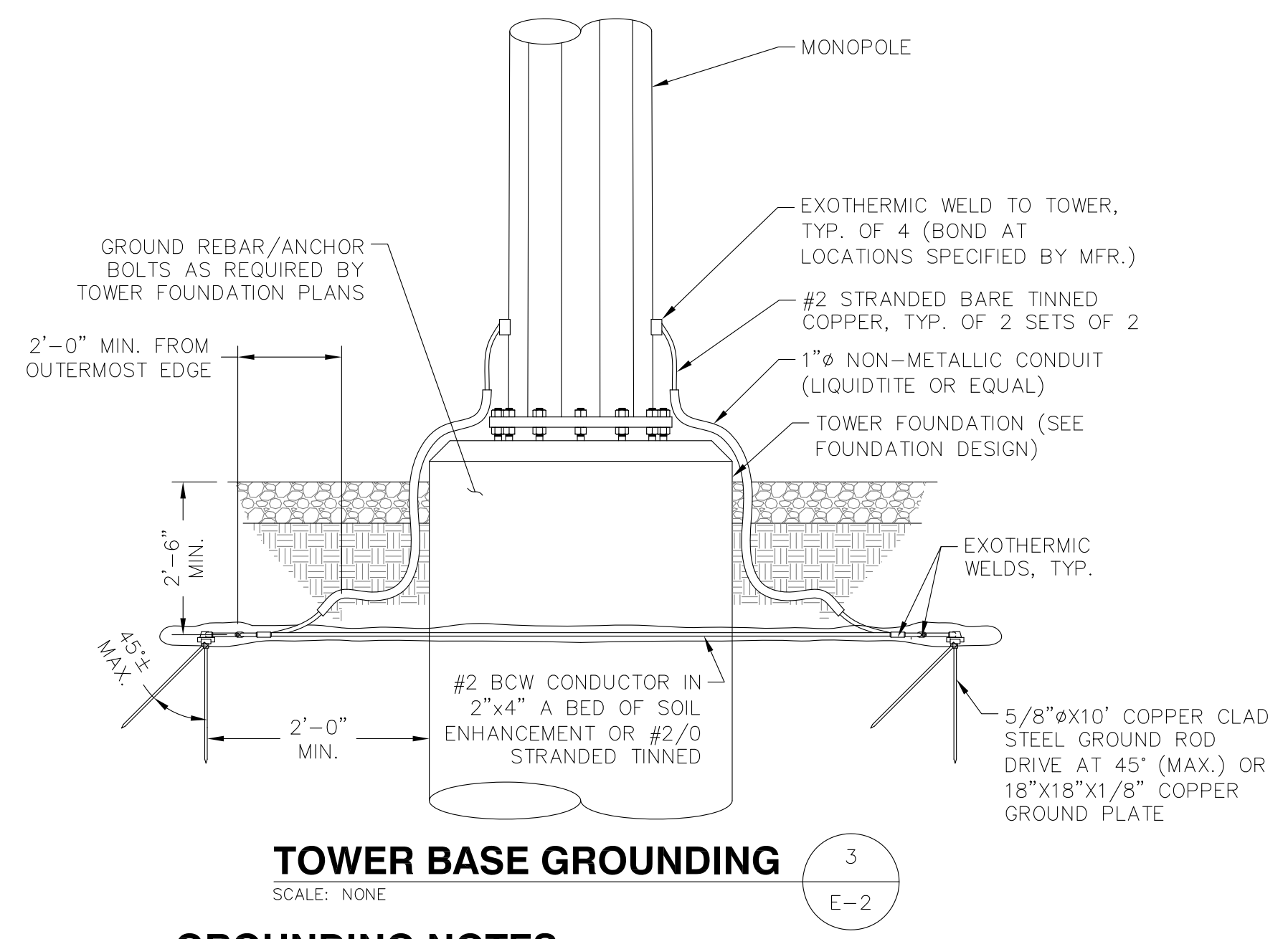
NOTE:
SITE GROUNDING SYSTEM IS A BASIC DESIGN. THE ACTUAL RESISTANCE TO GROUND CANNOT BE CONFIRMED WITHOUT A FIELD TEST. CONTRACTOR TO INSTALL AND PROVIDE DOCUMENTATION AT CLOSEOUT.



FENCE GROUNDING
SCALE: NONE



FENCE GATE GROUNDING
SCALE: NONE



GROUNDING NOTES

- GENERAL**
- GROUNDING SHALL COMPLY WITH ARTICLE (250) OF THE NATIONAL ELECTRIC CODE, EIA/TIA-222 REV.G & MOTOROLA R-56 AS REQUIRED.
 - ALL GROUNDING DEVICES SHALL BE U.L. APPROVED OR LISTED FOR THEIR INTENDED USE.
 - THE CONTRACTOR SHALL SECURE A COPY OF ANY SOIL RESISTIVITY AND/OR SITE RESISTANCE TO EARTH TESTING PREVIOUSLY PERFORMED. IF NO RECORDS EXIST THAN A FIELD SOIL RESISTIVITY TEST SHALL BE PERFORMED TO ASSURE THE GROUNDING SYSTEM PROVIDES 10 OHMS OR LESS IN ACCORDANCE WITH OWNER SPECIFICATIONS.

- GROUND RODS**
- RODS SHALL BE 5/8" DIAMETER, 10' LONG COPPER CLAD STEEL OR SOLID COPPER
 - SHALL BE BURIED 30" (MIN.) OR BELOW PERMANENT MOISTURE LEVEL PENETRATING BELOW FROST LINE
 - RODS SEPARATED 20' (MIN.) TO OTHER GROUND RODS OR ELECTRODES
 - SEPARATION BETWEEN GROUND RODS IN SAME GROUNDING SYSTEM SHALL BE GREATER THAN SUM OF RESPECTIVE LENGTHS
 - GROUND RODS SHALL NOT BE SHORTENED BY CUTTING OR DEFORMED BY DRIVING MACHINERY
 - WHERE CONDITIONS REQUIRE, RODS MAY BE DRIVEN AT ANGLES UP TO 45 DEGREES OR HORIZONTAL ORIENTED PERPENDICULAR TO GROUND RING

- ELECTROLYTIC GROUND RODS**
- WHERE CONDITIONS REQUIRE, 10' LONG ELECTROLYTIC GROUND ELECTRODES (ALLTEC TERRADYNE TG-10L, LYNCOLE XIT K2L-10CS, OR EQUIVALENT) MAY BE USED. INSTALL PER MANUFACTURER'S SPECIFICATIONS
 - L-SHAPED ELECTROLYTIC RODS SHALL BE INSTALLED PERPENDICULAR TO GROUND RING
 - BACKFILL WITH GROUNDING ENCASEMENT MATERIALS (ALLTEC TERRAFILL, LYNCOLE LYCONITE, OR EQUIVALENT)

- GROUND PLATES**
- ONLY TO BE USED WHERE CONDITIONS PROHIBIT USE OF GROUND RODS
 - 1/16" (MIN.) THICKNESS WITH 2 SQUARE FEET (MIN.) AREA UNPAINTED COPPER CLAD STEEL OR SOLID COPPER
 - TOP EDGE BURIED 30" (MIN.) OR BELOW PERMANENT MOISTURE LEVEL
 - PLATES SHALL BE INSTALLED VERTICALLY
 - BACKFILL WITH GROUNDING ENCASEMENT MATERIALS 6" MINIMUM ON ALL SIDES

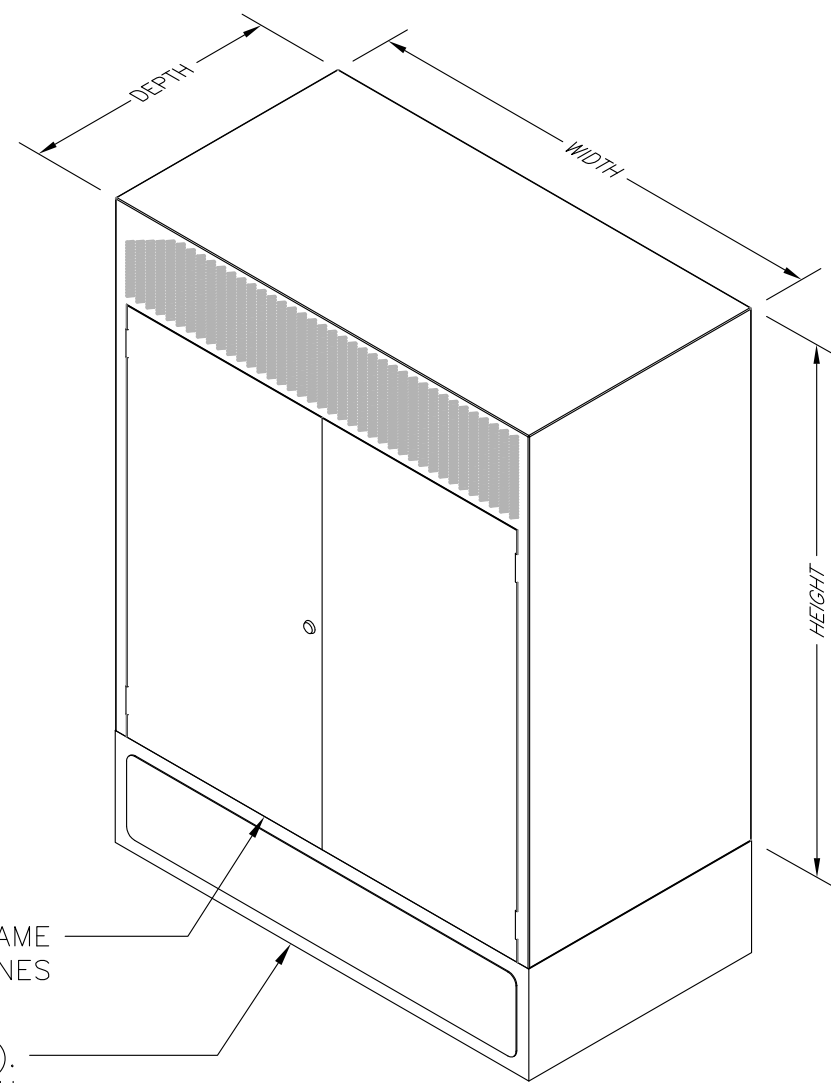
- RADIAL GROUNDING CONDUCTORS**
- #2 BARE COPPER WIRE IN BED OF GROUND ENHANCEMENT MATERIAL OR #2/0 STRANDED TINNED.
 - CONDUCTORS SHALL RADIATE FROM TOWER CENTER
 - SHALL BE BONDED TO GROUND RING AND DIRECTLY TO TOWER
 - WHERE NOT POSSIBLE TO BOND DIRECTLY TO TOWER, ADDITIONAL #2 CONDUCTORS SHALL BE BONDED TO RING TO OBTAIN EQUIVALENT.
 - BURIED 30" WHERE POSSIBLE, 18" (MIN.)
 - EACH RADIAL CONDUCTOR SHALL BE 25' (MIN.), 80' (MAX.) IN LENGTH
 - WHERE MULTIPLE RADIALS ARE USED, VARY CONDUCTOR LENGTHS

- GROUNDING ENCASEMENT MATERIALS**
- PRE-PACKAGED MATERIALS SHALL BE USED
 - ACCEPTABLE MATERIALS: BENTONITE, BENTONITE CONTAINING MATERIALS, CONCRETE, CONDUCTIVE CONCRETE, CEMENT WITH GRADED GRANULAR CARBONACEOUS AGGREGATE IN PLACE OF SAND OR GRAVEL

- CONDUCTORS**
- #2 BCW CONDUCTOR IN GROUND ENHANCEMENT MATERIAL OR #2/0 STRANDED TINNED.
 - #2 OR #6 AWG BCW OR STRANDED COPPER WHERE ABOVE GROUND AS NOTED
 - SPLICES SHALL BE EXOTHERMICALLY WELDED
 - 8" (MIN.) BENDING RADIUS FOR #2 OR SMALLER. 90 DEGREES (MIN.) BEND. ALL BENDS TOWARDS GROUND LOCATION.

- CONNECTORS**
- GROUNDING CONNECTIONS SHALL BE EXOTHERMIC UNLESS OTHERWISE NOTED.
 - EXOTHERMIC WELDS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.
 - PRIOR TO INSTALLING LUGS ON GROUND WIRES, APPLY THOMAS & BETTS KOPR-SHIELD OR EQUAL
 - PREPARE ALL BONDING SURFACES FOR GROUNDING CONNECTIONS BY REMOVING ALL PAINT AND CORROSION DOWN TO SHINY METAL
 - FOLLOWING CONNECTION, APPLY APPROPRIATE CONDUCTIVE ANTI-OXIDIZING PAINT.
 - MECHANICAL CONNECTIONS SHALL BE 3 CRIMP STYLE COMPRESSION FIT CRIMPED WITH HYDRAULIC CRIMPING TOOLS OR EQUAL. NO SLIP BOLTS ARE ACCEPTABLE.

RBS SPECIFICATIONS	
MANUF.	ERICSSON
MODEL #	RBS 6102
HEIGHT	57.1"
WIDTH	51.2"
DEPTH	27.6"
WEIGHT	728± LBS. W/O BATTERIES
MAX WEIGHT	~1600 LBS.



ATTACH RBS CABINET TO BASE FRAME PER MANUFACTURER'S GUIDELINES

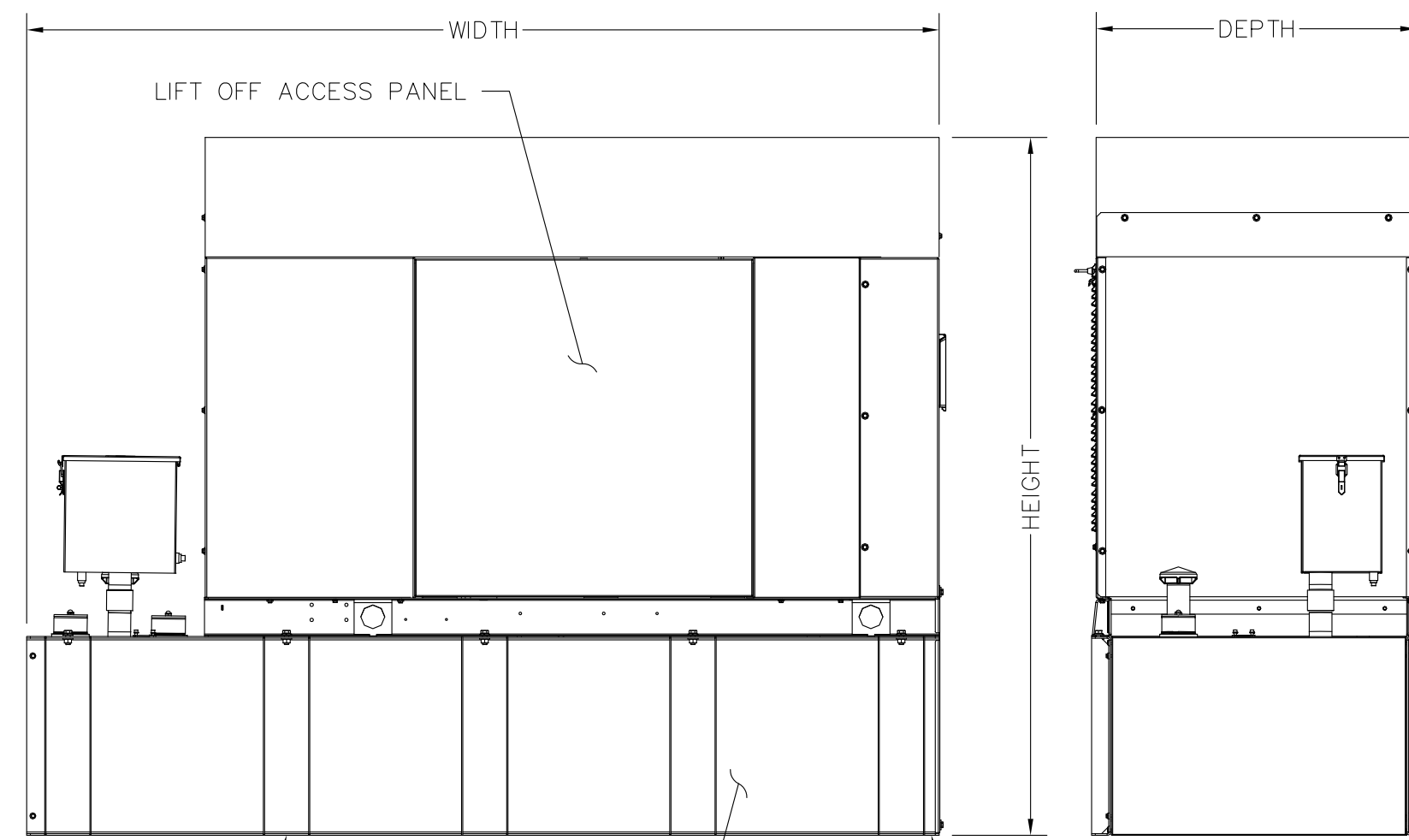
RBS BASE FRAME (DIMENSIONS TBD). ANCHOR TO CONCRETE PAD WITH HILTI HDI 3/8" SS 303 DROP-IN ANCHORS (TYP. OF 8) OR EQUAL PER MANUFACTURER'S GUIDELINES

EQUIPMENT CABINET (RBS)

SCALE: NONE

1
CA-2

DIESEL GENERATOR SPECIFICATIONS	
MANUF.	GENERAC
MODEL #	RD025 (25kW)
HEIGHT	102.6"
WIDTH	89.1"
DEPTH	35"
WEIGHT	2950± LBS.



BASE FRAME (VERIFY BASE DIMENSIONS & ANCHOR LOCATIONS WITH MANUFACTURER). ANCHOR TO CONCRETE PAD WITH 5/8" HDG HILTI-KWIK BOLT 3 SIZED PER MANUFACTURER'S GUIDELINES WITH 3-1/2" MIN. EMBEDMENT, (TYP. OF 10) MAINTAIN 4" MINIMUM EDGE DISTANCE FROM SLAB TO ANCHOR.

211 GALLON UL LISTED DOUBLE WALL FUEL TANK

CONTRACTOR TO VERIFY CONDUIT STUB-UP PLACEMENT WITH MANUFACTURER

GENERATOR DETAIL

SCALE: NONE

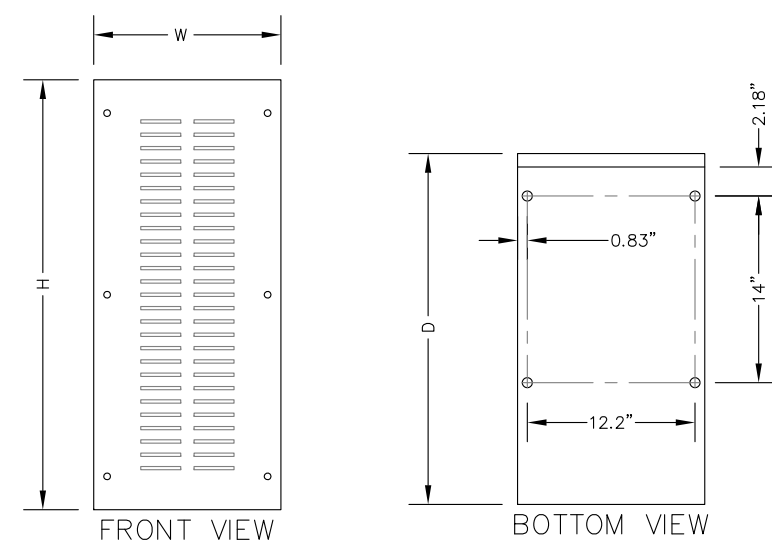
2
CA-2

TENANT INFORMATION

SITE NUMBER: CTHA227A

T-MOBILE NORTHEAST, LLC
35 SOUTH GRIFFIN ROAD
BLOOMFIELD, CT 06002

BBU SPECIFICATIONS	
MANUF.	PTS
MODEL #	PTS8003
HEIGHT	32.3"
WIDTH	14.0"
DEPTH	26.3"
WEIGHT WITHOUT BATTERIES	60 LBS.
MOUNT BASE WITH (4) 1/2" DROP-IN ANCHORS WITH 2" MINIMUM EMBEDMENT (INSTALL PER MANUFACTURER'S INSTALLATION GUIDELINES)	

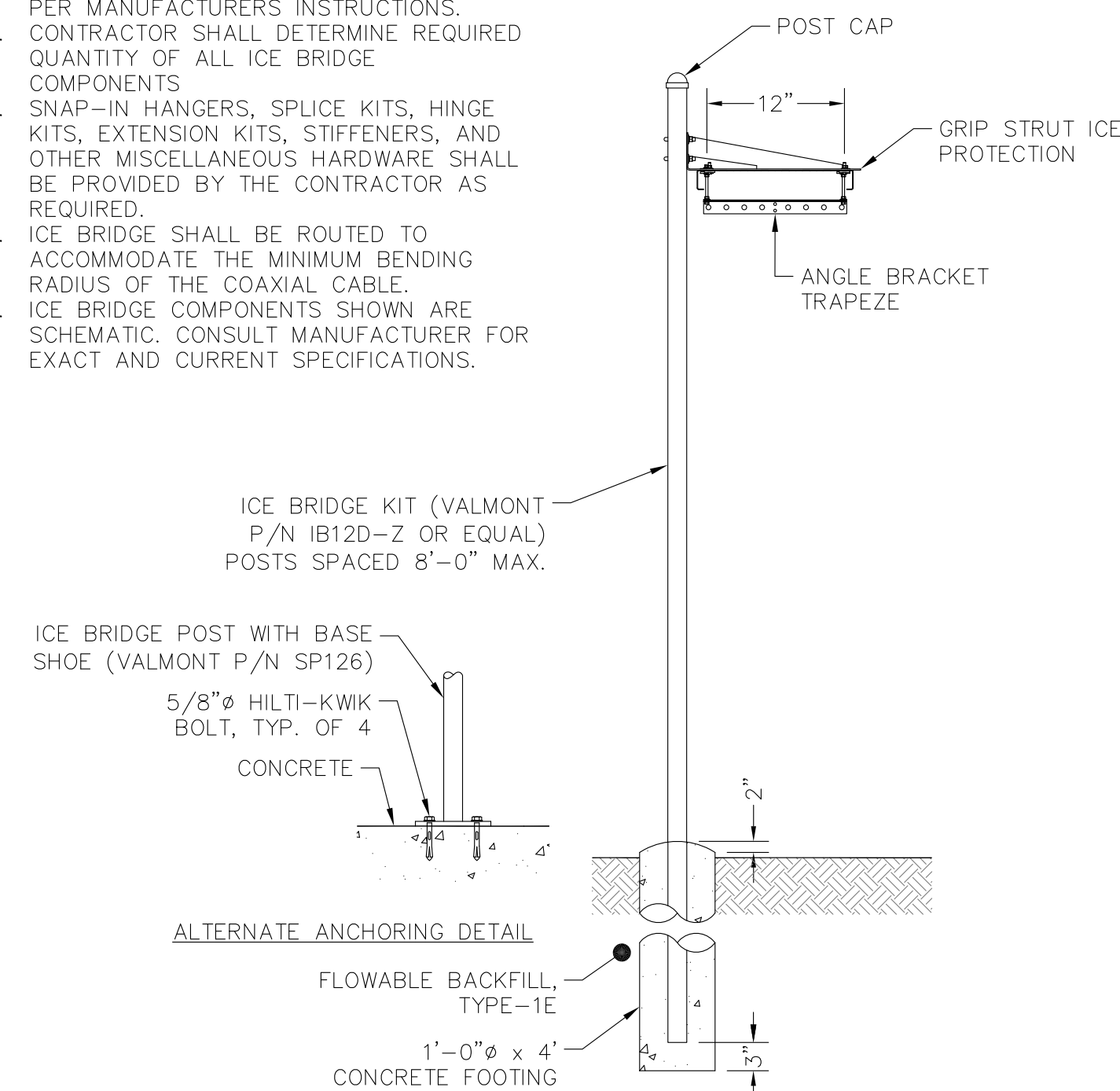


BATTERY BACKUP UNIT (BBU)

SCALE: N.T.S.

3
CA-2

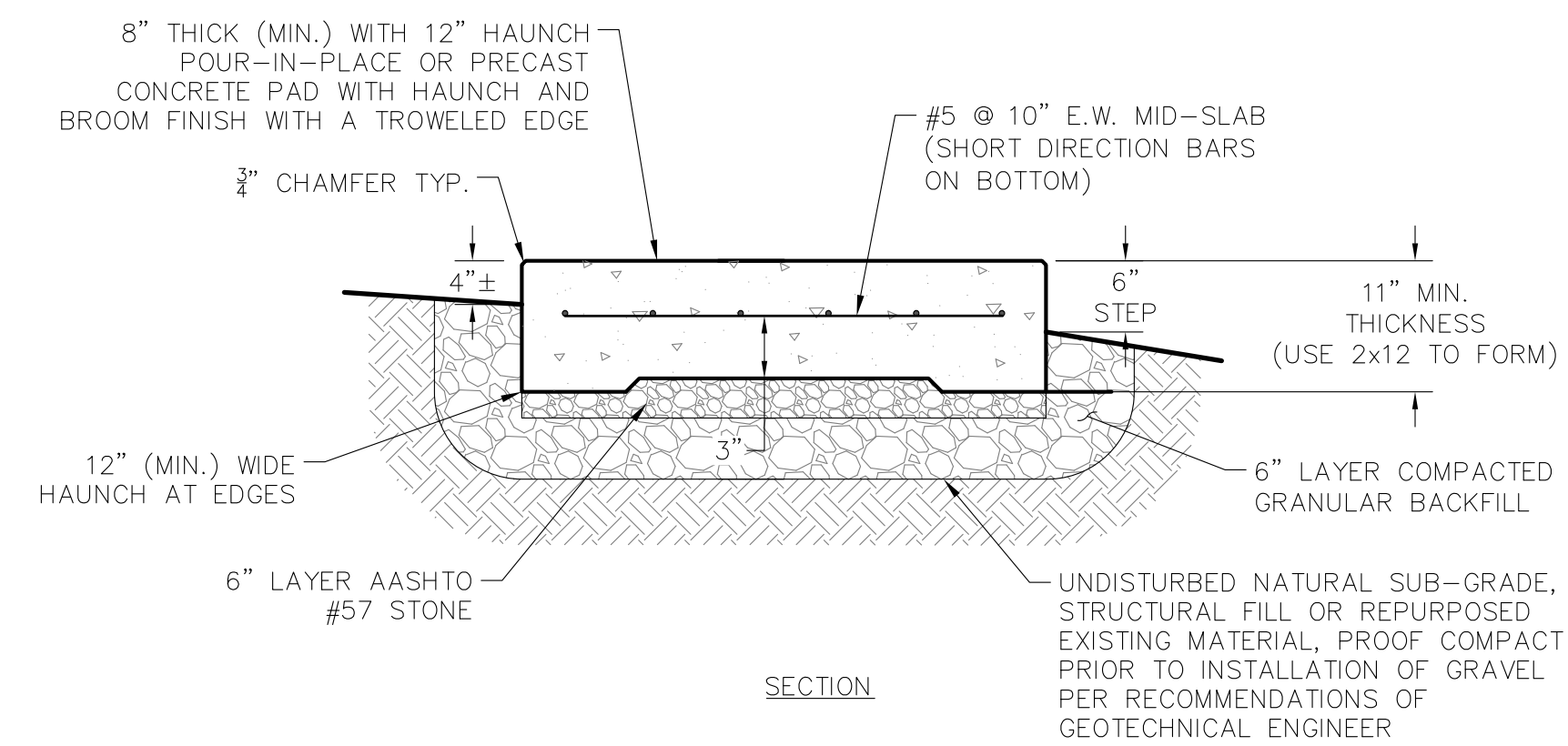
- ALL COMPONENTS SHALL BE INSTALLED PER MANUFACTURER'S INSTRUCTIONS.
- CONTRACTOR SHALL DETERMINE REQUIRED QUANTITY OF ALL ICE BRIDGE COMPONENTS.
- SNAP-IN HANGERS, SPLICE KITS, HINGE KITS, EXTENSION KITS, STIFFENERS, AND OTHER MISCELLANEOUS HARDWARE SHALL BE PROVIDED BY THE CONTRACTOR AS REQUIRED.
- ICE BRIDGE SHALL BE ROUTED TO ACCOMMODATE THE MINIMUM BENDING RADIUS OF THE COAXIAL CABLE.
- ICE BRIDGE COMPONENTS SHOWN ARE SCHEMATIC. CONSULT MANUFACTURER FOR EXACT AND CURRENT SPECIFICATIONS.



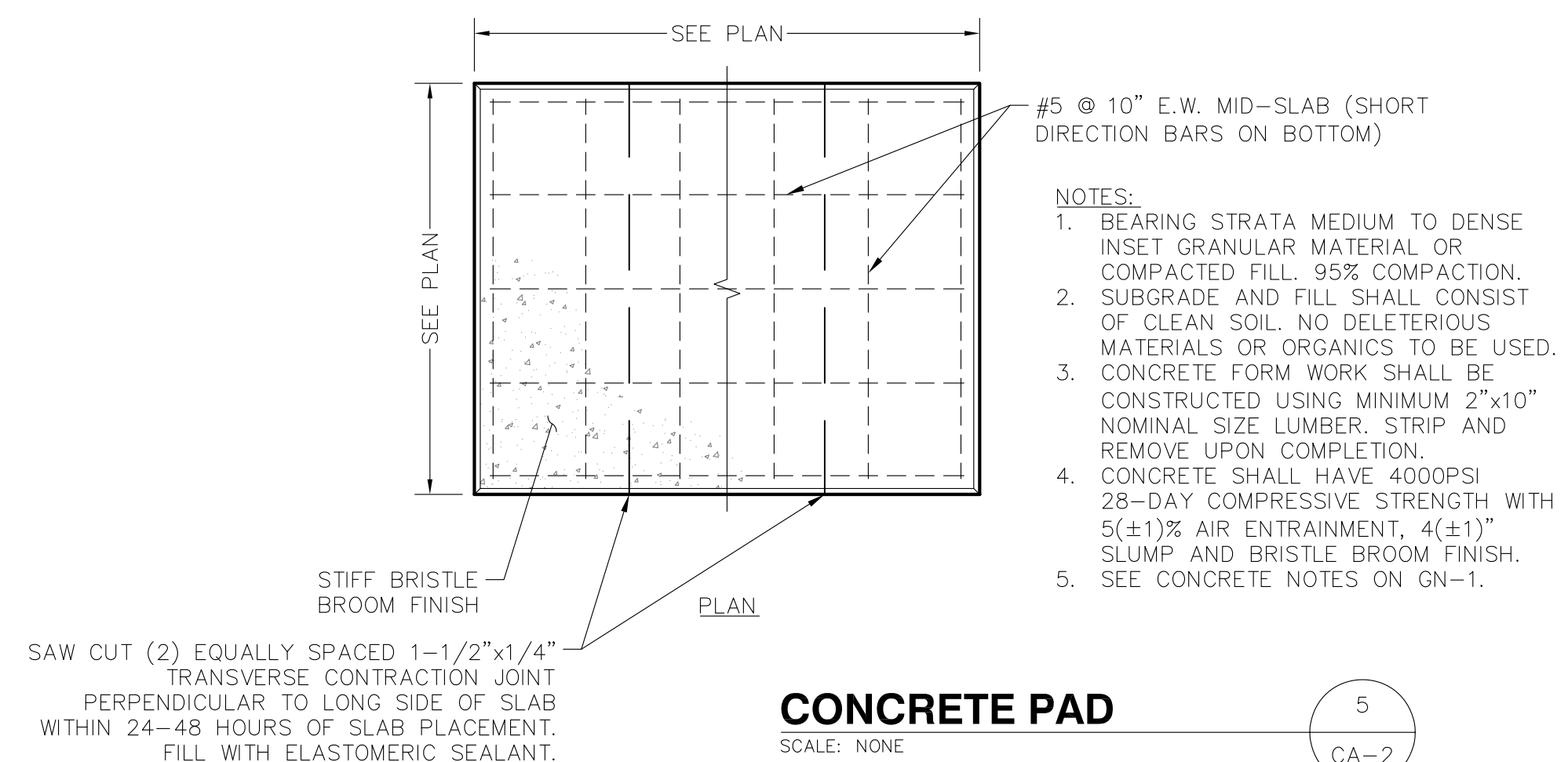
ICE BRIDGE

SCALE: NONE

4
CA-2



SECTION



CONCRETE PAD

SCALE: NONE

5
CA-2

CONSULTANTS:

NO.	DATE	REVISIONS
1	05/27/21	ISSUED FOR REVIEW
0	06/21/21	DEVELOPMENT & MANAGEMENT PLAN
1	06/25/21	REVISED DEVELOPMENT & MANAGEMENT PLAN

TITLE:

SITE NAME: WINDSOR
SITE NUMBER: CT 1209
ADDRESS: 800 PROSPECT HILL ROAD WINDSOR, CT 06095

TOWER OWNER:
TARPON TOWERS II, LLC
1001 3RD AVENUE WEST SUITE 420
BRADENTON, FL 34205

STAMP:



DATE: 06/25/21
DRAWN: BLM
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 18-049
SHEET TITLE:

TENANT DETAILS

CA-2

ANTENNA CONFIGURATION					
SECTOR	ANTENNA MODEL	ANTENNA RAD (SBA DATABASE)	AZIMUTH	RADIOS/TMAS	CABLE FEED LINES
ALPHA	ERICSSON - AIR32 KRD901146-1_B66A_B2A	130'±	70°	PROPOSED RRU'S (1) ERICSSON 4449 B71+B12 (1) ERICSSON 4415 B25	PROPOSED (3) ERICSSON 6x12 HCS (1) ERICSSON 9x18 HCS (SHARED WITH ALL SECTORS)
	RFS - APXVAALL24_43-U-NA20	130'±	70°		
	ERICSSON - AIR6449 B41	130'±	70°		
BETA	ERICSSON - AIR32 KRD901146-1_B66A_B2A	130'±	200°	PROPOSED RRU'S (1) ERICSSON 4449 B71+B12 (1) ERICSSON 4415 B25	CABLING SHARED WITH ANOTHER SECTOR
	RFS - APXVAALL24_43-U-NA20	130'±	200°		
	ERICSSON - AIR6449 B41	130'±	200°		
GAMMA	ERICSSON - AIR32 KRD901146-1_B66A_B2A	130'±	325°	PROPOSED RRU'S (1) ERICSSON 4449 B71+B12 (1) ERICSSON 4415 B25	CABLING SHARED WITH ANOTHER SECTOR
	RFS - APXVAALL24_43-U-NA20	130'±	325°		
	ERICSSON - AIR6449 B41	130'±	325°		

BASED UPON THE CO-LOCATION APPLICATION DATED NOVEMBER 05, 2020

T-MOBILE SITE TECHNICIAN SITE SAFETY NOTES	
SECTOR A:	ACCESS NOT PERMITTED
SECTOR B:	ACCESS NOT PERMITTED
SECTOR C:	ACCESS NOT PERMITTED
SECTOR D:	ACCESS NOT PERMITTED
GPS/LMU:	UNRESTRICTED
RADIO CABINETS:	UNRESTRICTED
PPC DISCONNECT:	UNRESTRICTED
MAIN CIRCUIT D/C:	UNRESTRICTED
NIU/T DEMARC:	UNRESTRICTED
OTHER/SPECIAL:	NONE

CONTRACTOR TO VERIFY PORT LOCATIONS AND NOTIFY ENGINEER PRIOR TO ORDERING PLATFORM.

NO MOUNT SUBSTITUTIONS WITHOUT PRIOR APPROVAL BY ENGINEER

TENANT INFORMATION

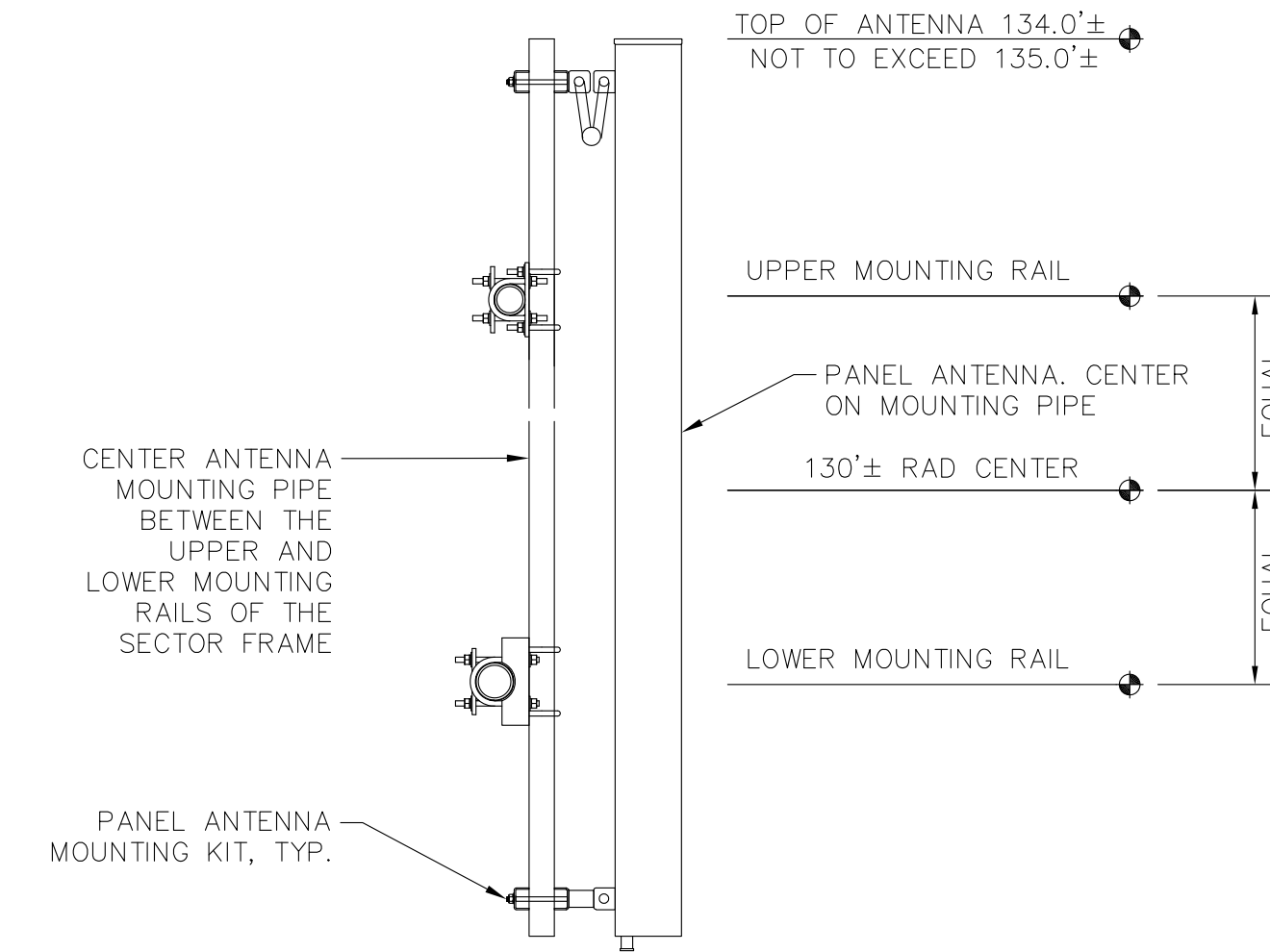


SITE NUMBER: CTHA227A
T-MOBILE NORTHEAST, LLC
35 SOUTH GRIFFIN ROAD
BLOOMFIELD, CT 06002

4 Bay Road
Bldg A, Suite 200
Hodley, MA 01035
Ph: (413)320-4918

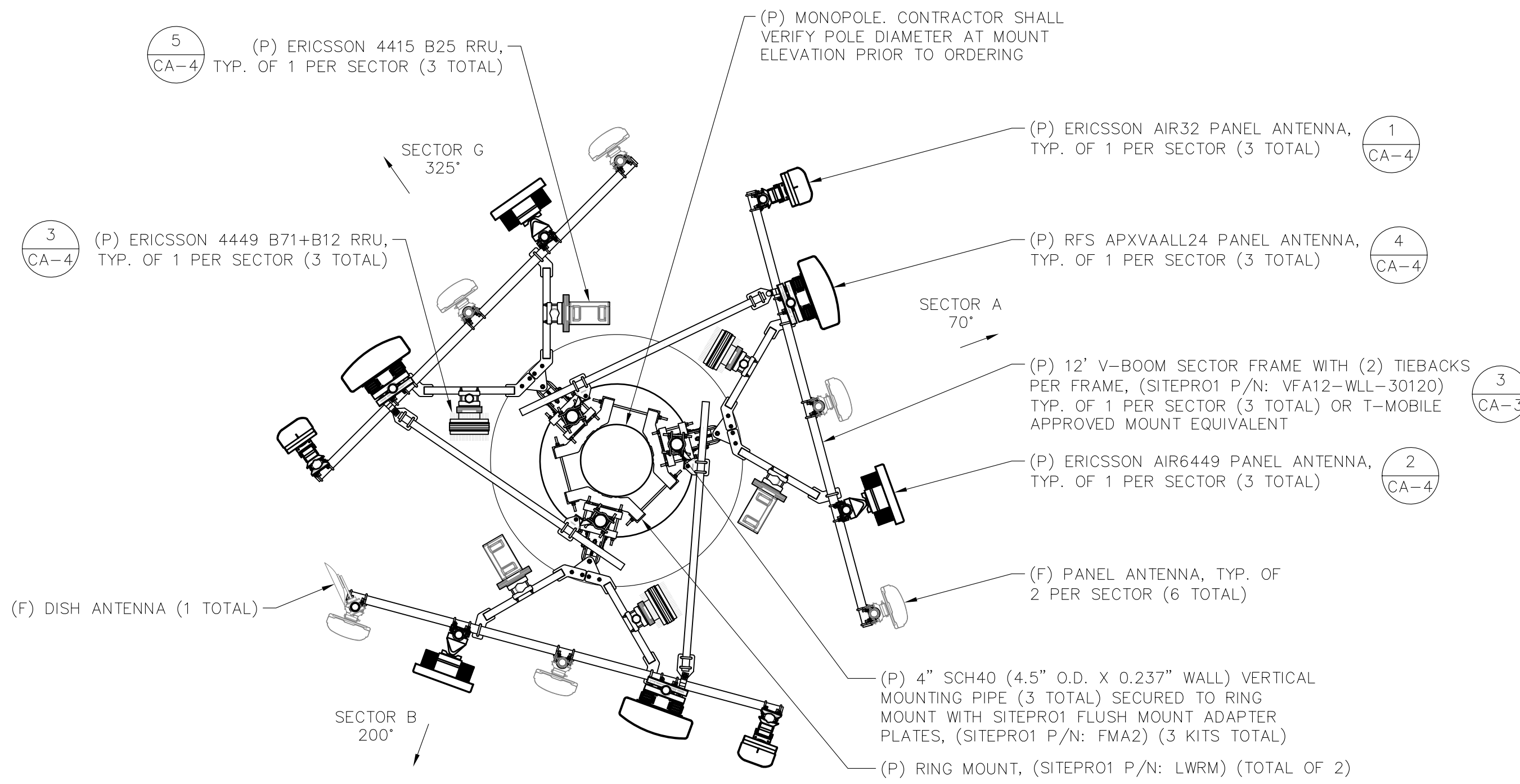
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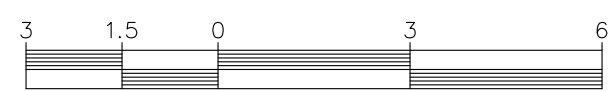
TYPICAL MOUNTING ELEVATION

SCALE: NONE



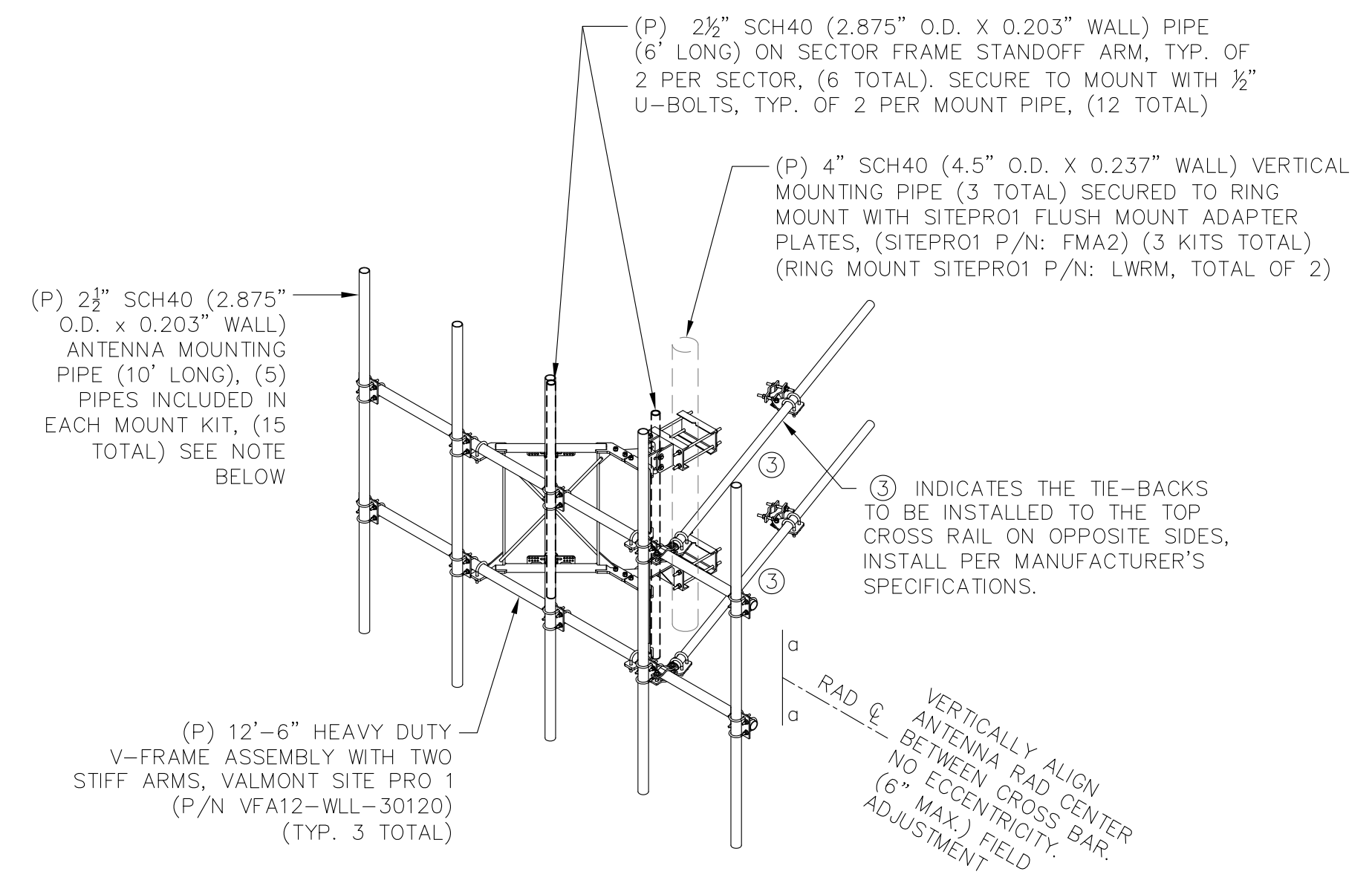
RF ANTENNA PLAN

SCALE: 1"=3' (22x34)
1"=6' (11x17)



PLATFORM FRAME SHALL BE ALIGNED TO MAINTAIN A CLEAR PATH FOR THE SAFETY CLIMB.

EQUIPMENT PLACEMENT NOTE:
ANTENNAS TO BE INSTALLED WITH THE CENTER OF THE ANTENNAS, (RADCN) EQUALLY SPACED BETWEEN THE MOUNTING RAILS, (CENTER OF THE MOUNT) NO VERTICAL ECCENTRICITY ALLOWED.



NOTE: MOUNT CONFIGURATION SHOWN SUBJECT TO FINAL APPROVAL BY T-MOBILE

ANTENNA MOUNT ISOMETRIC

SCALE: NONE

SITE NAME: WINDSOR
SITE NUMBER: CT 1209
ADDRESS: 800 PROSPECT HILL ROAD WINDSOR, CT 06095
TOWER OWNER: TARPON TOWERS II, LLC
1001 3RD AVENUE WEST SUITE 420 BRADENTON, FL 34205

STAMP:

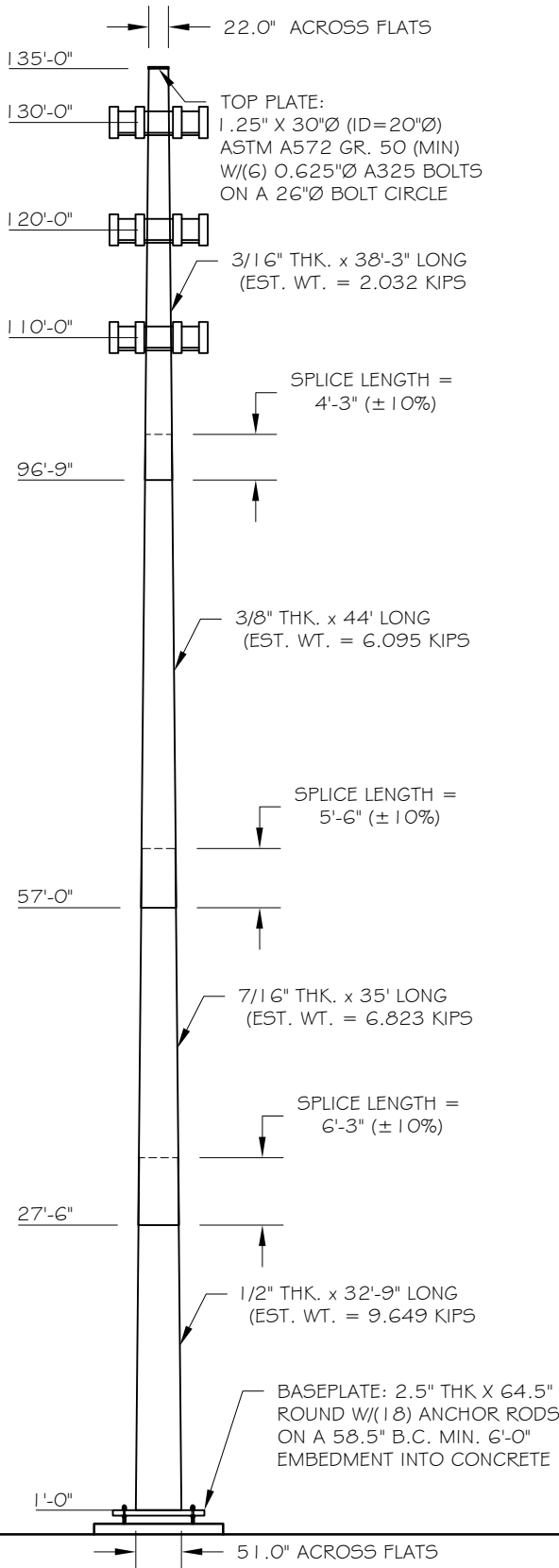


DATE: 06/25/21
DRAWN: BLM
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 18-049
SHEET TITLE:

TENANT
DETAILS

CA-3

Page 1 of 3	Job Number: 23521-150
Eng: MFP	Customer Ref: TP-19977
	Date: 6/11/2021
Structure: 135-FT MONOPOLE	
Site: CT1209 WINDSOR	
Location: HARTFORD CO., CT / 41°52'58.5", -72°42'29.2"	
Owner: TARPON TOWERS	
Revision No.: Revision Date:	



DESIGN			
Building Code: 2016 CONNECTICUT BUILDING CODE			
Design Standard: ANSI/TIA-222-G			
Wind Speed Load Cases: ASCE-7-05 CONVERTED TO ASCE-7-10			
Load Case #1: 94 MPH Design Wind Speed - V_{ASD} ($V_{ULT} = 121$ MPH)			
Load Case #2: 50 MPH Wind with 1" Ice Accumulation			
Load Case #3: 60 MPH Service Wind Speed			
Structure Class Risk Category	Exposure Cat.	Topography Cat.	Crest Height
II	C	I	

STRUCTURE MEETS THE MINIMUM REQUIREMENTS OF TIA-222-H

EQUIPMENT LIST	
Elev.	Description
130	(4) AIR32 + (4) APXVAALL24_43 + (4) AIR6449 B41 + (6) RRU
130	(1) 1-FT DISH + 12-FT PLATFORM WITH HANDRAIL
120	(12) JAHH-45C-R3B + (6) RRH + (2) RAYCAP
120	12-FT PLATFORM WITH HANDRAIL
110	(3) APX1GDWV-1GDWV-S-E-ACU + (3) APXVAARR24-43 + (9) RRU
110	(3) AIR3246 + (1) 1-FT DISH + 12-FT PLATFORM WITH HANDRAIL

ANTENNA FEED LINES ROUTED ON THE INSIDE OF THE POLE
POLE DESIGNED FOR A MAX 34-FT FALL RADIUS

STRUCTURE PROPERTIES					
Cross-Section: 18-Sided			Taper: 0.23134 in/ft		
Shaft Steel: ASTM A572 GR 65			Baseplate Steel: ASTM A572 GR 50		
Anchor Rods: 2.25 in. A615 GR. 75 X 7'-0"					
Sect.	Length (ft)	Thickness (in)	Splice (ft)	Top Dia. (in)	Bot Dia. (in)
1	38.25	0.1875	4.25	22.00	30.85
2	44.00	0.3750	5.50	29.49	39.67
3	35.00	0.4375	6.25	37.65	45.74
4	32.75	0.5000	0.00	43.42	51.00



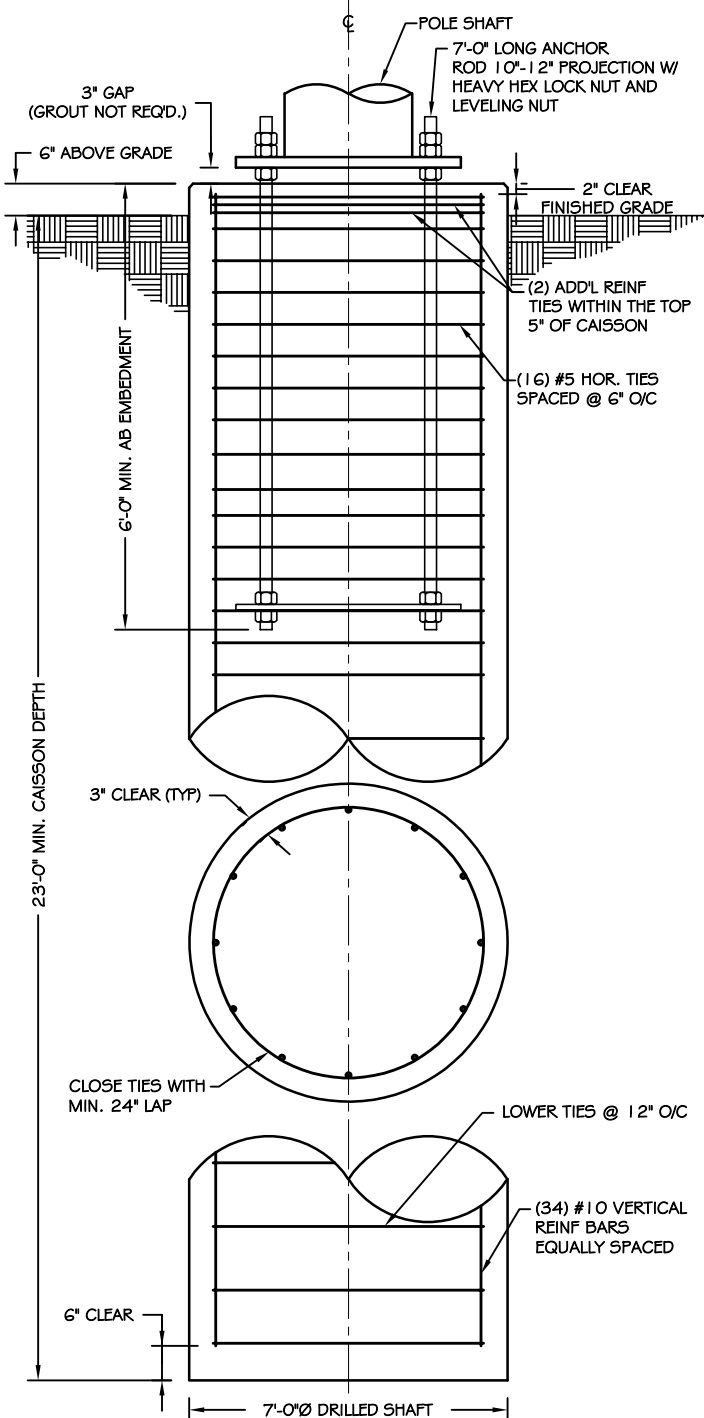
BASE REACTIONS FOR FOUNDATION DESIGN

Moment: 4767 ft-kip
Shear: 46 kip
Axial: 48 kip

Page 2 of 3	Job Number: 23521-150
Eng: MFP	Customer Ref: TP-19977
	Date: 6/11/2021
Structure: 135-FT MONOPOLE	
Site: CT1209 WINDSOR	
Location: HARTFORD CO., CT / 41°52'58.5", -72°42'29.2"	
Owner: TARPON TOWERS	
Revision No.: Revision Date:	

FOUNDATION NOTES:

1. ALL FOUNDATION CONCRETE SHALL USE TYPE II CEMENT AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 4500 PSI AT 28 DAYS. CONCRETE SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45. IN AREAS OF POTENTIAL FREEZING, CONCRETE SHALL BE AIR ENTRAINED 6% (± 1.5%). ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 318, "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", LATEST EDITION.
2. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 VERTICAL BARS SHALL BE GRADE 60, AND TIES OR STIRRUPS SHALL BE A MINIMUM OF GRADE 40. THE PLACEMENT OF ALL REINFORCEMENT SHALL CONFORM TO ACI 315, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", LATEST EDITION.
3. CAISSON FOUNDATION INSTALLATION SHALL BE IN ACCORDANCE WITH ACI 336, "STANDARD SPECIFICATIONS FOR THE CONSTRUCTION OF DRILLED PIERS", LATEST EDITION.
4. THE CONTRACTOR SHALL DETERMINE THE MEANS AND METHODS TO SUPPORT THE EXCAVATION DURING CONSTRUCTION. THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND SHALL CONSULT THE GEOTECHNICAL ENGINEER AS NECESSARY PRIOR TO CONSTRUCTION.
5. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT BY:
 ENGINEER: WELTI GEOTECHNICAL
 REPORT NO.: N/A (DATED 4/13/21)
6. ESTIMATED CONCRETE VOLUME = 33 CUBIC YARDS.
7. THE FOUNDATION HAS BEEN DESIGNED TO RESIST THE FOLLOWING FACTORED LOADS:
 MOMENT: 4767 FT*KIPS
 SHEAR: 46 KIPS
 AXIAL: 48 KIPS
8. GEOTECHNICAL REPORT INDICATES GROUNDWATER MAY BE ENCOUNTERED AT 18'-0" BELOW GRADE.



CAISSON FOUNDATION

NOT TO SCALE



tnxTower Michael Plahovinsak, P.E. 18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Job 135' Monopole - MFP #23521-150 r1	Page 1 of 7
	Project CT1209 Windsor	Date 06:32:36 06/11/21
	Client TP-19977	Designed by JC

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Hartford County, Connecticut.

Basic wind speed of 94 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	135.00-96.75	38.25	4.25	18	22.0000	30.8489	0.1875	0.7500	A572-65 (65 ksi)
L2	96.75-57.00	44.00	5.50	18	29.4907	39.6698	0.3750	1.5000	A572-65 (65 ksi)
L3	57.00-27.50	35.00	6.25	18	37.6474	45.7444	0.4375	1.7500	A572-65 (65 ksi)
L4	27.50-1.00	32.75		18	43.4235	51.0000	0.5000	2.0000	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	It/Q in ²	w in	w/t
L1	22.3105	12.9812	780.3007	7.7434	11.1760	69.8193	1561.6281	6.4918	3.5420	18.891
	31.2958	18.2474	2167.3087	10.8848	15.6712	138.2986	4337.4693	9.1254	5.0994	27.197
L2	30.8861	34.6549	3711.5567	10.3361	14.9813	247.7466	7427.9971	17.3308	4.5304	12.081
	40.2239	46.7706	9123.8911	13.9496	20.1522	452.7481	18259.7876	23.3897	6.3219	16.858
L3	39.4527	51.6706	9038.5241	13.2095	19.1249	472.6057	18088.9412	25.8402	5.8559	13.385
	46.3826	62.9143	16316.0700	16.0840	23.2382	702.1241	32653.6091	31.4631	7.2810	16.642
L4	45.4845	68.1196	15856.2313	15.2378	22.0591	718.8055	31733.3266	34.0663	6.7625	13.525
	51.7096	80.1435	25821.9188	17.9275	25.9080	996.6774	51677.8148	40.0794	8.0960	16.192

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft ²	in					in	in	in
L1				1	1	1			

tnxTower Michael Plahovinsak, P.E. 18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Job	135' Monopole - MFP #23521-150 r1	Page	2 of 7
	Project	CT1209 Windsor	Date	06:32:36 06/11/21
	Client	TP-19977	Designed by	JC

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft ²	in					in	in	in
135.00-96.75									
L2 96.75-57.00				1	1	1			
L3 57.00-27.50				1	1	1			
L4 27.50-1.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		C_{AA} ft ² /ft	Weight plf
1 5/8"	C	No	Yes	Inside Pole	130.00 - 1.00	18	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00	0.92 0.92 0.92
1 5/8"	C	No	Yes	Inside Pole	120.00 - 1.00	18	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00	0.92 0.92 0.92
1 5/8"	C	No	Yes	Inside Pole	110.00 - 1.00	18	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00	0.92 0.92 0.92

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A_R ft ²	A_F ft ²	C_{AA} In Face ft ²	C_{AA} Out Face ft ²	Weight K
L1	135.00-96.75	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1.15
L2	96.75-57.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1.97
L3	57.00-27.50	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1.46
L4	27.50-1.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1.31

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	C_{AA} In Face ft ²	C_{AA} Out Face ft ²	Weight K
L1	135.00-96.75	A	2.266	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	1.15
L2	96.75-57.00	A	2.175	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	1.97
L3	57.00-27.50	A	2.048	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	1.46
L4	27.50-1.00	A	1.835	0.000	0.000	0.000	0.000	0.00

tnxTower Michael Plahovinsak, P.E. 18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Job	135' Monopole - MFP #23521-150 r1	Page	3 of 7
	Project	CT1209 Windsor	Date	06:32:36 06/11/21
	Client	TP-19977	Designed by	JC

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	1.31

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K	
**									
(4) Ericsson AIR 32 w/ mount pipe	A	From Face	3.00 0.00 0.00	0.0000	130.00	No Ice 5.92 1/2" Ice 6.31 1" Ice 6.70	5.60 6.25 6.91	0.13 0.18 0.24	
(4) RFS APXVAALL24_43 w/ mount pipe	B	From Face	3.00 0.00 0.00	0.0000	130.00	No Ice 20.24 1/2" Ice 20.89 1" Ice 21.55	10.63 12.06 13.34	0.15 0.29 0.43	
(4) Ericsson AIR6449-B41 w/ mount pipe	C	From Face	3.00 0.00 0.00	0.0000	130.00	No Ice 6.05 1/2" Ice 6.43 1" Ice 6.82	3.27 3.74 4.23	0.13 0.18 0.23	
(3) Ericsson RRUS-4449	A	From Face	2.00 0.00 0.00	0.0000	130.00	No Ice 1.65 1/2" Ice 1.81 1" Ice 1.98	1.16 1.30 1.45	0.07 0.09 0.10	
(3) Ericsson RRUS-4415/B25	B	From Face	2.00 0.00 0.00	0.0000	130.00	No Ice 1.63 1/2" Ice 1.78 1" Ice 1.95	0.64 0.75 0.86	0.05 0.06 0.08	
12' Platform w/ Handrail	C	None		0.0000	130.00	No Ice 30.00 1/2" Ice 35.00 1" Ice 40.00	30.00 35.00 40.00	1.80 2.60 3.40	
**									
(4) Andrew JAHH-45C-R3B w/ mount pipe	A	From Face	3.00 0.00 0.00	0.0000	120.00	No Ice 15.89 1/2" Ice 16.51 1" Ice 17.14	9.47 10.88 12.15	0.13 0.24 0.36	
(4) Andrew JAHH-45C-R3B w/ mount pipe	B	From Face	3.00 0.00 0.00	0.0000	120.00	No Ice 15.89 1/2" Ice 16.51 1" Ice 17.14	9.47 10.88 12.15	0.13 0.24 0.36	
(4) Andrew JAHH-45C-R3B w/ mount pipe	C	From Face	3.00 0.00 0.00	0.0000	120.00	No Ice 15.89 1/2" Ice 16.51 1" Ice 17.14	9.47 10.88 12.15	0.13 0.24 0.36	
(3) Nokia RRH 4T4R-B13/5	A	From Face	2.00 0.00 0.00	0.0000	120.00	No Ice 2.27 1/2" Ice 2.47 1" Ice 2.67	1.42 1.59 1.76	0.07 0.09 0.11	
(3) Nokia RRH-4T4R-B2/66	B	From Face	2.00 0.00 0.00	0.0000	120.00	No Ice 2.27 1/2" Ice 2.47 1" Ice 2.67	1.42 1.59 1.76	0.07 0.09 0.11	
(2) Raycap RVZDC-6627-PF-48	C	From Face	2.00 0.00 0.00	0.0000	120.00	No Ice 3.79 1/2" Ice 4.04 1" Ice 4.30	2.51 2.73 2.95	0.03 0.06 0.10	
12' Platform w/ Handrail	C	None		0.0000	120.00	No Ice 30.00 1/2" Ice 35.00 1" Ice 40.00	30.00 35.00 40.00	1.80 2.60 3.40	
**									
(3) Ericsson AIR-3246 B66 w/ mount pipe	A	From Face	3.00 0.00 0.00	0.0000	110.00	No Ice 7.96 1/2" Ice 8.37 1" Ice 8.79	6.35 7.02 7.71	0.20 0.27 0.34	
(3) RFS APXVAARR24-43 w/ mount pipe	B	From Face	3.00 0.00	0.0000	110.00	No Ice 20.24 1/2" Ice 20.89	10.79 12.21	0.09 0.22	

tnxTower Michael Plahovinsak, P.E. 18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Job	135' Monopole - MFP #23521-150 r1	Page	4 of 7
	Project	CT1209 Windsor	Date	06:32:36 06/11/21
	Client	TP-19977	Designed by	JC

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz	Lateral						Vert
			ft	ft	°	ft	ft ²	ft ²	K	
(3) RFS APX16DWV-16DWV-S-E-A CU w/Mount Pipe	C	From Face	0.00				1" Ice	21.55	13.49	0.37
			3.00		0.0000	110.00	No Ice	6.38	3.52	0.07
			0.00				1/2" Ice	6.80	4.18	0.12
(3) Ericsson RRUS-2217	A	From Face	0.00				1" Ice	7.23	4.85	0.18
			2.00		0.0000	110.00	No Ice	1.35	0.63	0.03
			0.00				1/2" Ice	1.49	0.73	0.04
(3) Ericsson RRUS-4449	A	From Face	0.00				1" Ice	1.65	0.85	0.05
			2.00		0.0000	110.00	No Ice	1.65	1.16	0.07
			0.00				1/2" Ice	1.81	1.30	0.09
(3) Ericsson RRUS-4415/B25	A	From Face	0.00				1" Ice	1.98	1.45	0.10
			2.00		0.0000	110.00	No Ice	1.63	0.64	0.05
			0.00				1/2" Ice	1.78	0.75	0.06
12' Platform w/ Handrail	C	None	0.00				1" Ice	1.95	0.86	0.08
					0.0000	110.00	No Ice	30.00	30.00	1.80
							1/2" Ice	35.00	35.00	2.60
						1" Ice	40.00	40.00	3.40	

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets:		Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight	
				Horz	Lateral							Vert
			ft	ft	°	°	ft	ft	ft ²	K		
Andrew VHL P1-23 (1' Std. 23.6 GHz)	A	Paraboloid w/o Radome	From Face	1.00		0.0000		130.00	1.00	No Ice	0.79	0.03
				0.00						1/2" Ice	0.92	0.03
				0.00						1" Ice	1.06	0.04
Andrew VHL P1-23 (1' Std. 23.6 GHz)	A	Paraboloid w/o Radome	From Face	1.00		0.0000		110.00	1.00	No Ice	0.79	0.03
				0.00						1/2" Ice	0.92	0.03
				0.00						1" Ice	1.06	0.04

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 90 deg - No Ice
5	0.9 Dead+1.6 Wind 90 deg - No Ice
6	1.2 Dead+1.6 Wind 180 deg - No Ice
7	0.9 Dead+1.6 Wind 180 deg - No Ice
8	1.2 Dead+1.0 Ice+1.0 Temp
9	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
10	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
11	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
12	Dead+Wind 0 deg - Service
13	Dead+Wind 90 deg - Service
14	Dead+Wind 180 deg - Service

tnxTower Michael Plahovinsak, P.E. 18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Job	135' Monopole - MFP #23521-150 r1	Page	5 of 7
	Project	CT1209 Windsor	Date	06:32:36 06/11/21
	Client	TP-19977	Designed by	JC

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	135 - 96.75	Pole	Max Tension	6	0.00	0.00	0.00
			Max. Compression	8	-49.13	-4.34	13.34
			Max. Mx	4	-15.01	-446.18	25.23
			Max. My	2	-15.13	-19.90	433.86
			Max. Vy	4	23.40	-446.18	25.23
			Max. Vx	6	22.45	24.28	-427.48
L2	96.75 - 57	Pole	Max. Torque	4			6.34
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-62.10	-4.65	14.32
			Max. Mx	4	-24.33	-1408.35	61.85
			Max. My	2	-24.40	-58.29	1358.35
			Max. Vy	4	26.58	-1408.35	61.85
L3	57 - 27.5	Pole	Max. Vx	6	25.64	59.02	-1353.06
			Max. Torque	4			6.34
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-75.23	-4.77	14.67
			Max. Mx	4	-34.16	-2206.89	88.79
			Max. My	2	-34.19	-86.63	2128.90
L4	27.5 - 1	Pole	Max. Vy	4	28.90	-2206.89	88.79
			Max. Vx	6	27.97	84.72	-2124.46
			Max. Torque	4			6.32
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-93.06	-4.77	14.70
			Max. Mx	4	-48.15	-3191.80	118.80
			Max. My	2	-48.15	-118.27	3082.45
			Max. Vy	4	31.10	-3191.80	118.80
			Max. Vx	6	30.19	113.42	-3079.01
			Max. Torque	4			6.31

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	135 - 96.75	16.262	13	1.0813	0.0081
L2	101 - 57	8.964	13	0.8781	0.0030
L3	62.5 - 27.5	3.255	13	0.5075	0.0011
L4	33.75 - 1	0.915	13	0.2505	0.0004

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
130.00	Andrew VHL P1-23 (1' Std. 23.6 GHz)	13	15.126	1.0561	0.0115	36120
120.00	(4) Andrew JAHH-45C-R3B w/ mount pipe	13	12.888	1.0032	0.0091	12040
110.00	Andrew VHL P1-23 (1' Std. 23.6 GHz)	13	10.749	0.9429	0.0070	7223

tnxTower Michael Plahovinsak, P.E. 18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Job	135' Monopole - MFP #23521-150 r1	Page	6 of 7
	Project	CT1209 Windsor	Date	06:32:36 06/11/21
	Client	TP-19977	Designed by	JC

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	135 - 96.75	72.138	4	4.8065	0.0343
L2	101 - 57	39.721	4	3.9000	0.0129
L3	62.5 - 27.5	14.410	4	2.2485	0.0046
L4	33.75 - 1	4.049	4	1.1085	0.0018

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
130.00	Andrew VHL P1-23 (1' Std. 23.6 GHz)	4	67.092	4.6941	0.0490	8215
120.00	(4) Andrew JAHH-45C-R3B w/ mount pipe	4	57.149	4.4583	0.0391	2737
110.00	Andrew VHL P1-23 (1' Std. 23.6 GHz)	4	47.649	4.1892	0.0301	1640

Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
L1	135 - 96.75 (1)	TP30.8489x22x0.1875	38.25	0.00	0.0	17.6622	-15.01	1120.64	0.013
L2	96.75 - 57 (2)	TP39.6698x29.4907x0.375	44.00	0.00	0.0	45.2561	-24.33	3351.08	0.007
L3	57 - 27.5 (3)	TP45.7444x37.6474x0.4375	35.00	0.00	0.0	60.9065	-34.16	4522.87	0.008
L4	27.5 - 1 (4)	TP51x43.4235x0.5	32.75	0.00	0.0	80.1435	-48.15	5940.26	0.008

Pole Bending Design Data

Section No.	Elevation ft	Size	M _{ux} kip-ft	φM _{ux} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	M _{uy} kip-ft	φM _{uy} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
L1	135 - 96.75 (1)	TP30.8489x22x0.1875	446.89	684.95	0.652	0.00	684.95	0.000
L2	96.75 - 57 (2)	TP39.6698x29.4907x0.375	1409.71	2614.90	0.539	0.00	2614.90	0.000
L3	57 - 27.5 (3)	TP45.7444x37.6474x0.4375	2208.68	4070.75	0.543	0.00	4070.75	0.000
L4	27.5 - 1 (4)	TP51x43.4235x0.5	3194.01	6156.17	0.519	0.00	6156.17	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V _u K	φV _n K	Ratio $\frac{V_u}{\phi V_n}$	Actual T _u kip-ft	φT _n kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L1	135 - 96.75 (1)	TP30.8489x22x0.1875	23.42	560.32	0.042	6.34	1372.88	0.005
L2	96.75 - 57 (2)	TP39.6698x29.4907x0.375	26.60	1675.54	0.016	6.32	5243.98	0.001
L3	57 - 27.5 (3)	TP45.7444x37.6474x0.4375	28.92	2261.44	0.013	6.31	8163.72	0.001
L4	27.5 - 1 (4)	TP51x43.4235x0.5	31.11	2970.13	0.010	6.31	12345.75	0.001

tnxTower Michael Plahovinsak, P.E. 18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Job 135' Monopole - MFP #23521-150 r1	Page 7 of 7
	Project CT1209 Windsor	Date 06:32:36 06/11/21
	Client TP-19977	Designed by JC

Pole Interaction Design Data

Section No.	Elevation ft	Ratio P_u	Ratio M_{ux}	Ratio M_{uy}	Ratio V_u	Ratio T_u	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
		ϕP_n	ϕM_{nx}	ϕM_{ny}	ϕV_n	ϕT_n			
L1	135 - 96.75 (1)	0.013	0.652	0.000	0.042	0.005	0.668	1.000	4.8.2 ✓
L2	96.75 - 57 (2)	0.007	0.539	0.000	0.016	0.001	0.547	1.000	4.8.2 ✓
L3	57 - 27.5 (3)	0.008	0.543	0.000	0.013	0.001	0.550	1.000	4.8.2 ✓
L4	27.5 - 1 (4)	0.008	0.519	0.000	0.010	0.001	0.527	1.000	4.8.2 ✓

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
L1	135 - 96.75	Pole	TP30.8489x22x0.1875	1	-15.01	1120.64	66.8	Pass
L2	96.75 - 57	Pole	TP39.6698x29.4907x0.375	2	-24.33	3351.08	54.7	Pass
L3	57 - 27.5	Pole	TP45.7444x37.6474x0.4375	3	-34.16	4522.87	55.0	Pass
L4	27.5 - 1	Pole	TP51x43.4235x0.5	4	-48.15	5940.26	52.7	Pass
						Summary		
						Pole (L1)	66.8	Pass
						RATING =	66.8	Pass

Michael F. Plahovinsak, P.E. 18301 State Route 161 W Plain City, OH 43064 Phone: 614-398-6250 email: mike@mfpeng.com	Job 135-ft monopole - MFP #23521-150	Page BP & AB Calc
	Project CT1209 Windsor	Date 6/11/2021
	Client TAPP TP-19977	Designed by Mike

Anchor Rod and Base Plate Calculation

ANSI/TIA-222-G

<i>Factored Base Reactions:</i>	<i>Pole Shape:</i>	<i>Anchor Rods:</i>	<i>Base Plate:</i>
Moment: 3194 ft-kips	18-Sided	(18) 2.25 in. A615 GR. 75	2.5 in. x 64.5 in. Round
Shear: 31 kips	<i>Pole Dia. (D_f):</i>	Anchor Rods Evenly Spaced	fy = 50 ksi
Axial: 48 kips	51.00 in	On a 58.5 in Bolt Circle	

Anchor Rod Calculation According to TIA-222-G section 4.9.9

$\phi_t, \phi_v =$	0.80	TIA 4.9.9
$I_{bolts} =$	7700.06 in ²	Moment of Inertia
$P_u =$	148 kips	Compr Force
$V_u =$	1.7 kips	Shear Force
$R_{nt} =$	325.00 kips	Nominal Tensile Strength
n	0.50	for detail type (d)
Stress Rating =	58.3%	Satisfies TIA-G 4.9.9

Base Plate Calculation According to TIA-222-G

$\phi =$	0.90	TIA 4.7
$M_{PL} =$	344.8 in-kip	Plate Moment
$L =$	8.9 in	Section Length
$Z =$	13.9	Plastic Section Modulus
$M_P =$	695.4 in-kip	Plastic Moment
$\phi M_n =$	625.9 in-kip	Factored Resistance
<i>Calculated Moment vs Factored Resistance</i>		
	344.77 in-kip	\leq 626 in-kip
Stress Rating =	55.1%	

Anchor Rods Are Adequate	58.3%	<input checked="" type="checkbox"/>
Base Plate is Adequate	55.1%	<input checked="" type="checkbox"/>

Michael F. Plahovinsak, P.E. 18301 State Route 161 W Plain City, OH 43064 Phone: 614-398-6250 email: mike@mfpeng.com	Job	135-ft monopole - MFP #23521-150	Page	FND
	Project	CT1209 Windsor	Date	6/11/2021
	Client	TAPP TP-19977	Designed by	Mike

Caisson Calculation

According to ANSI/TIA-222-G

1. Foundation overturning resistance calculated with PLS Caisson, for Brom's method for rigid piles. Soil layers modeled after recommendations from the geotechnical report.
2. Cohesion strength for the upper 21 ft has been reduced by 50%
3. In lieu of a soil resistance factor $f_s = 0.75$ (TIA-9.4.1) an additional safety factor against soil failure of 1.33 has been applied.
4. Foundation has been designed with factored loads per TIA-222-G.
5. Design water table = 18 ft below grade

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*** PIER PROPERTIES      CONCRETE STRENGTH (ksi) = 4.50          STEEL STRENGTH (ksi) = 60.00
                          DIAMETER (ft) = 7.000          DISTANCE FROM TOP OF PIER TO GROUND LEVEL (ft) = 0.50

*** SOIL PROPERTIES
LAYER  TYPE  THICKNESS  DEPTH AT TOP OF LAYER  DENSITY  CU  KP  PHI
      (ft)  (ft)  (pcf)  (psf)  (degrees)
1      S    4.00      0.00      100.0    1.000  3.000  -0.00
2      S   14.00     4.00      120.0    3.000  3.000  30.00
3      S   13.00     18.00     58.0     3.000  3.000  30.00

*** DESIGN (FACTORED) LOADS AT TOP OF PIER  MOMENT (ft-k) = 4767.0  VERTICAL (k) = 48.0  SHEAR (k) = 46.0
                                              ADDITIONAL SAFETY FACTOR AGAINST SOIL FAILURE = 1.33

*** CALCULATED PIER LENGTH (ft) = 23.500

*** CHECK OF SOILS PROPERTIES AND ULTIMATE RESISTING FORCES ALONG PIER

TYPE  TOP OF LAYER BELOW TOP OF PIER  THICKNESS  DENSITY  CU  KP  FORCE  ARM
      (ft)  (ft)  (pcf)  (psf)  (k)  (ft)
S      0.50      4.00      100.0    1.000  16.80  3.17
S      4.50     12.62     120.0    3.000  919.78  12.18
S     17.12     1.38     120.0    3.000  -173.90  17.82
S     18.50     5.00     58.0     3.000  -700.88  21.05

*** SHEAR AND MOMENTS ALONG PIER
DISTANCE BELOW TOP OF PIER (ft)  WITH THE ADDITIONAL SAFETY FACTOR  WITHOUT ADDITIONAL SAFETY FACTOR
SHEAR (k)  MOMENT (ft-k)  SHEAR (k)  MOMENT (ft-k)
0.00      61.8      6594.4      46.4      4946.0
2.35      58.2      6737.5      43.7      5053.2
4.70      39.8      6858.7      29.9      5144.1
7.05     -43.8      6862.1     -32.9      5146.7
9.40    -169.2     6619.9    -126.9     4965.1
11.75   -336.4     6034.0    -252.3     4525.6
14.10   -545.3     5006.2    -409.0     3754.8
16.45   -795.9     3438.5    -597.0     2578.9
18.80   -661.4     1585.9    -496.1     1189.5
21.15   -340.8     404.4     -255.6     303.3
23.50     0.0       -0.0       0.0       -0.0

*** TOTAL REINFORCEMENT PCT = 0.62  REINFORCEMENT AREA (in^2) = 34.36
*** USABLE AXIAL CAP. (k) = 48.0  USABLE MOMENT CAP. (ft-k) = 5305.7

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For Design:

7-ft Diameter caisson x 23.5-ft long (23-ft Embed with 0.5-ft above grade)
Concrete strength = 4500 PSI @ 28 days. Estimated Concrete Volume = 33 CY3.
(34) #10 Vertical Rebar. Steel Cross-Section = 43.18 in ²

Monopole Spread Footing Calculation

ANSI/TIA-222-G

Factored Base Reactions:	Footing Dimensions:		Concrete:
Moment: 4767 ft-kips	25 ft x 25 ft	7 ft Square Pier	f'c = 4500 psi
Shear: 46 kips	x 3.5 ft thick	w/6 in Reveal	Steel fy = 60 ksi
Axial: 48 kips	Bearing 6 ft B.G.	86.5 Yd3 Concrete	f = 0.75
Soil Backfill 100 pcf	Ultimate Bearing:	6000 psf	Water Table n/a

Foundation Weight

Weight of Pole	48.0 kips
Weight of Concrete	350.175 kips
Weight of Soil	144 kips
Bouyancy of Water	0.0 kips
Total	542.2 kips

Overturning Resistance:

Overturning Moment (M_u)	5066 ft-kips	4767 ft-kips + (46 kips x 6.5 ft)
Resisting Moment (R_s)	6777.1875 ft-kips	542.175 kips x 25 ft / 2
$\phi \times R_s > M_u$	$M_{\text{overturning}} / f M_{\text{resist}}$	99.7% OK

Soil Bearing Pressure:

Eccentricity (e)	9.34 ft	5066 ft-kips / 542.175 kips
6(e)	56.1 ft >	25.0 ft 6e > 25
Maximum Soil Bearing	4039.4335 psf	Calculated across corners
Soil Overburden	-600 psf	
Net Soil Bearing	3439.4335 psf	
Resisting Soil Bearing (R_s)	6000 psf	
Net Soil Bearing < $\phi \times R_s$	Net Bearing / f R_s	76.4% OK

Bending Moment in Pier:

Bending Moment	4905 ft-kips	4767 ft-kips + (46 kips x 3 ft)
Pier Steel Req'd (Loads)	56.11 in ²	
Min. Pier Steel	35.28 in ²	1/2% (Based on Square Pier)

Bending Moment in Footing:

Max Bending Moment	3168.5947 ft-kips	Σ Moments about pier face
Footing Steel Req'd (Loads)	1.22 in ² /ft	
Min. Footing Steel	0.91 in ² /ft	0.18%