

STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL

IN RE: :
: :
A PETITION OF CELLCO PARTNERSHIP : PETITION NO. ____
D/B/A VERIZON WIRELESS FOR A :
DECLARATORY RULING ON THE NEED :
TO OBTAIN A SITING COUNCIL :
CERTIFICATE FOR THE INSTALLATION :
OF TWO PORTABLE WIRELESS :
TELECOMMUNICATIONS FACILITIES ON :
PROPERTY AT 139 NORTH MAIN STREET, :
WEST HARTFORD, CONNECTICUT : MAY 2, 2014

PETITION FOR A DECLARATORY RULING:
INSTALLATION OF A TEMPORARY TOWER HAVING NO
SUBSTANTIAL ADVERSE ENVIRONMENTAL EFFECT

I. Introduction

Pursuant to Sections 16-50j-38 and 16-50j-39 of the Regulations of Connecticut State Agencies (“R.C.S.A.”), Cellco Partnership d/b/a Verizon Wireless (“Cellco”) and New Cingular Wireless PCS, LLC (“AT&T”) (collectively the “Petitioners”) hereby petition the Connecticut Siting Council (the “Council”) for a declaratory ruling (“Petition”) that no Certificate of Environmental Compatibility and Public Need (“Certificate”) is required under Section 16-50k(a) of the Connecticut General Statutes (“C.G.S.”) for the temporary installation of a ballast tower and related wireless equipment on property owned by the American School for the Deaf (“ASD”) at 139 North Main Street, West Hartford, Connecticut (the “Property”). Included in Attachment 1 is a letter from ASD authorizing the Petitioners to make this filing. The Petitioners anticipate that the temporary tower will be needed for a period of approximately five (5) months, from June 1, 2014 to October 1, 2014, while construction of the “clock tower” wireless facility, approved in Docket No. 434, is completed.

II. Factual Background

The Petitioners currently maintain wireless telecommunication facilities at the Property. The facilities consist of antennas attached to the cupola of Gallaudet Hall and equipment located inside the same building on the ASD campus. As discussed in the Docket No. 434 Application, ASD is moving forward with significant renovations to its campus which includes the demolition of Gallaudet Hall. The new “clock tower” wireless facility approved in Docket No. 434, will replace the Petitioners’ wireless facilities currently located at Gallaudet Hall.

Construction of the clock tower commenced on or about February 3, 2014 and, due to conditions this past winter, is currently behind schedule. At the same time, ASD’s scheduled demolition of Gallaudet Hall has been accelerated, requiring the Petitioners to remove their existing antennas and related equipment before construction of the clock tower site will be completed.

In order to maintain critical wireless services in the area without interruption, Cellco and AT&T intend to install a temporary ballast tower and associated equipment to the north of the approved clock tower structure. Cellco will install three (3) antennas on the temporary tower at a centerline height of approximately 72 feet above ground level (“AGL”). AT&T will install three (3) antennas on the temporary tower at a centerline height of approximately 82 feet AGL. (See Temporary Tower Install – Project Plans included in Attachment 2). Power and telephone services to the temporary cell site will extend overhead from existing service on the Property. Four (4) temporary utility poles will be installed to support this service. Once the bell tower is constructed and wireless services are activated, all improvements and structures associated with the temporary facility will be removed.

III. Discussion

A. The Installation of the Temporary Wireless Facility Will Not Have A Substantial Adverse Environmental Effect

The Public Utility Environmental Standards Act (the “Act”), C.G.S. § 16-50g et seq., provides for the orderly and environmentally compatible development of telecommunications towers in the state to avoid “a significant impact on the environment and ecology of the State of Connecticut.” C.G.S. § 16-50g. To achieve these goals, the Act established the Council, and requires a Certificate of Environmental Compatibility and Public Need for the construction or modification of telecommunication towers “that may, as determined by the council, have a substantial adverse environmental effect”. C.G.S. § 16-50k(a).

1. Physical Environmental Effects

Cellco respectfully submits that the installation of a temporary ballast tower, associated wireless equipment and related improvements described herein will not have a significant impact in the physical and environmental characteristics of the Property or the surrounding area. The temporary ballast tower and wireless equipment will be installed adjacent to, and north of, the approved clock tower construction area. Use of this area will require minor tree removal and no tree removal.

2. Visual Effects

Visual effects of the temporary wireless facilities will be minimal, temporary and limited, to a large extent, to the Property. (See Attachment 3).

3. FCC Compliance

Radio Frequency (RF) emissions from the Cellco and AT&T antennas on the temporary tower will not exceed the standards adopted by the Federal Communications Commission (FCC). RF emissions calculations for the AT&T and Cellco antennas are included in Attachment 4).

4. FAA Summary Report

Included in Attachment 5 of this filing is a Federal Airways and Airspace Summary Report for the 85-foot temporary tower. According to this report, the temporary tower would not constitute an obstruction or hazard to air navigation.

5. Structural Analysis Report

The 85-foot ballast tower described above is capable of supporting both Cellco's and AT&T's antennas and related improvements. A complete Structural Analysis Report is included in Attachment 6.

In sum, the effect of the temporary tower and related improvements on the environment would be minimal, limited and temporary, rather than significant. The proposed temporary towers would, therefore, not present a substantial adverse environmental effect, and is not a facility for which the General Assembly intended to require a Certificate under C.G.S. § 16-50k(a).

B. Notice to Government Officials, the Property Owner and Abutting Landowners

On May 2, 2014, a copy of this Petition was sent to West Hartford's Mayor Scott Slifka, and Jeffrey S. Bravin, Chief Operating Officer and Assistant Executive Director for ASD. Notice of the filing of this Petition was also sent to the appropriate federal, State and municipal officials as referenced in R.C.S.A. Section 16-50j-40 and the owners of land abutting the Property.

Included in Attachment 7 is a copy of the notice letters sent to government officials and abutting land owners and certifications verifying that the notices were sent.

IV. Conclusion

Based on the information provided above, Cellco respectfully requests that the Council issue a determination in the form of a declaratory ruling that the placement of a temporary

telecommunications facility at the Property will not have a substantial adverse environmental effect and does not require the issuance of a Certificate of Environmental Compatibility and Public Need pursuant to § 16-50k of the General Statutes.

Respectfully submitted,

CELLCO PARTNERSHIP d/b/a VERIZON
WIRELESS

By



Kenneth C. Baldwin, Esq.
Robinson & Cole LLP
280 Trumbull Street
Hartford, CT 06103-3597
(860) 275-8200
Its Attorneys

ATTACHMENT 1



AMERICAN SCHOOL FOR THE DEAF

Edward F. Peltier, Executive Director

March 19, 2014

Melanie Bachman, Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, Connecticut 06

Re: Docket No. 434 - Proposed Temporary Installation of Two Cell on Wheels (COW) on
Property of the American School for the Deaf at 139 North Main Street, West Hartford,
CT

Dear Ms. Bachman:

The American School for the Deaf ("ASD") is the owner of the property at 139 North Main Street in West Hartford, the host property for the wireless telecommunications "clock tower" facility approved by the Siting Council in Docket No. 434.

This letter authorizes Verizon Wireless and AT&T to file for all necessary federal, state or local permits and approvals for the temporary installation of two portable wireless telecommunication facilities on the ASD campus.

Sincerely,

Jeffrey S. Bravin
Chief Operating Officer/Assistant Executive Director

ATTACHMENT 2

Cellco Partnership



d.b.a. **verizon** wireless
WIRELESS COMMUNICATIONS FACILITY
WEST HARTFORD WEST
TEMPORARY TOWER INSTALL
139 NORTH MAIN STREET
WEST HARTFORD, CT 06107

SITE DIRECTIONS

FROM: 99 EAST WINDY DRIVE, EAST HARTFORD, CONNECTICUT **TO:** 139 NORTH MAIN STREET, WEST HARTFORD, CT

1. Start and going EAST on EAST WINDY DRIVE toward DARLON STREET.
2. Turn LEFT onto DARLON STREET.
3. Turn LEFT onto COMMERCIAL BLVD/US-44.
4. Merge onto I-84 N/US-44 N. At the ramp on the left toward Hartford.
5. Turn LEFT onto RAMP ROAD, Exit 43, toward W. Hartford Center.
6. Exit 43 onto 139 N. MAIN STREET, on the left.

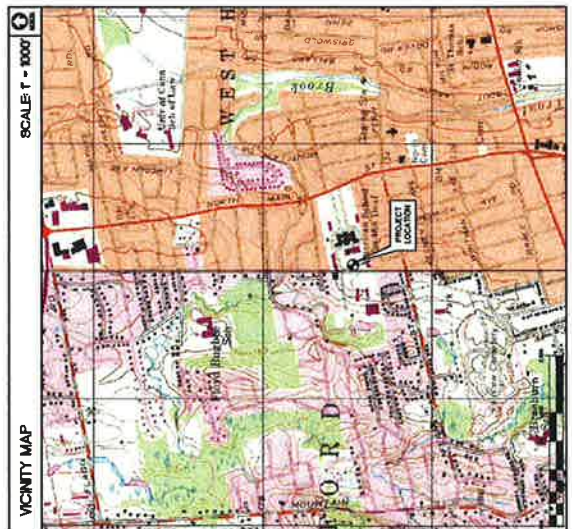
GENERAL NOTES

1. PROPOSED ANTENNA LOCATIONS AND HEIGHTS PROVIDED BY CELLO PARTNERSHIP AND ARE.

SITE INFORMATION

THE SCOPE OF WORK SHALL INCLUDE:

1. THE INSTALLATION OF A SHARED WIRELESS COMMUNICATION TEMPORARY BALLAST TOWER, CELLO PARTNERSHIP RADIO EQUIPMENT TRAILER, & AN AT&T RADIO EQUIPMENT SHELTER ADJACENT TO THE PROPOSED TOWER SITE OF THE AMERICAN SCHOOL FOR THE DEAF LOCATED WITHIN A FENCED AREA.
2. THE PROPOSED WIRELESS FACILITY INSTALLATION WILL BE DESIGNED IN ACCORDANCE WITH THE 2003 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2008 CONNECTICUT SUPPLEMENT.



PROJECT SUMMARY

SITE NAME: WEST HARTFORD WEST
SITE ADDRESS: 139 NORTH MAIN STREET, WEST HARTFORD, CT 06107
PROPERTY OWNER: AMERICAN SCHOOL FOR THE DEAF, 139 NORTH MAIN STREET, WEST HARTFORD, CT 06107
LESSOR/TENANT: CELLO COMMUNICATIONS PARTNERSHIP, 99 EAST WINDY DRIVE, EAST HARTFORD, CT 06109
CONTRACT PERSON: SMOY CARTER PARTNERSHIP (860) 803-0218
ENGINEER: CELLO PARTNERSHIP, 83-2 NORTH BRANFORD ROAD, BRANFORD, CT 06405
TOWER COORDINATES: NAD 83 EASING: 41°-46'-15.907" LONGITUDE: 72°-44'-58.840" ELEVATION: 100.00 FEET
 CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE 2003 SUPPLEMENT TO THE INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2008 SUPPLEMENT PREPARED BY MARTINEZ COUNCIL AND ASSOCIATES DATED APRIL 24, 2014.

SHEET INDEX

SHT. NO.	DESCRIPTION	REV.
T-1	TITLE SHEET	1
C-1.0	FINAL SITE SURVEY PLAN	1
C-2.0	SITE DEVELOPMENT PLAN AND MEC. SITE DETAILS	1
C-3.0	SITE CONSTRUCTION, SAE CONTROL NOTES AND DETAILS	1
C-4.0	ELEVATION AND ANTENNA COORDINATES	1

WEST HARTFORD WEST
 TEMPORARY TOWER INSTALL
 139 NORTH MAIN STREET
 WEST HARTFORD, CT

Central on Site™
 139 North Main Street
 West Hartford, CT 06107
 www.Centek.com

SCALE: AS SHOWN
 JOB NO. 14071000
 TITLE SHEET
T-1
 Sheet No. 1 of 3

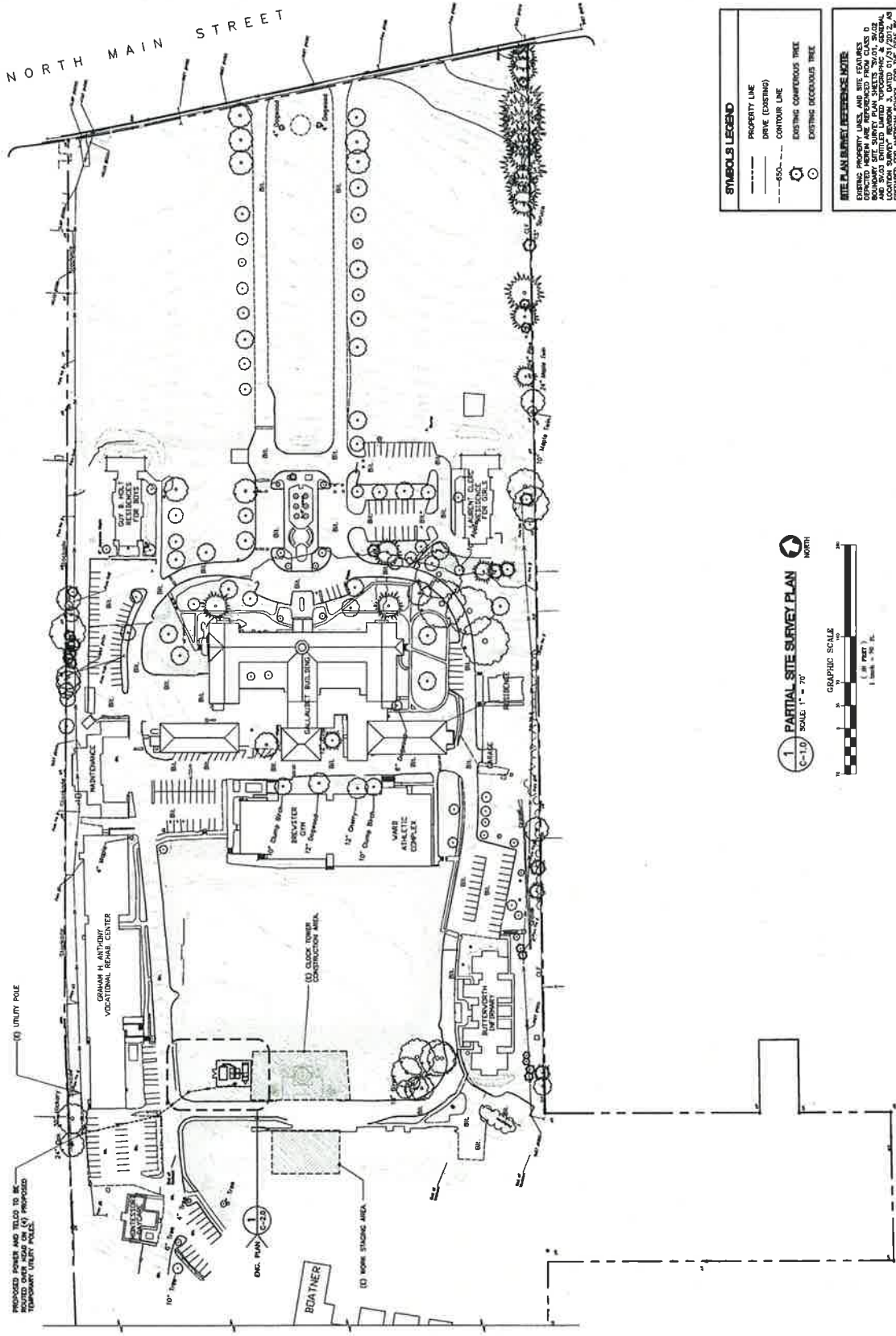
REV.	DATE	BY	DESCRIPTION
1	04/26/14	QAT	ISSUES FOR CDC
2	04/28/14	QAT	ISSUES FOR CDC - BLOCK REVIEW
3	05/01/14	QAT	REVISIONS

Colco Partnership d/b/a Verizon Wireless
WEST HARTFORD WEST
WESTFIELD COMMUNICATIONS FACILITY
TEMPORARY TOWER INSTALL
150 NORTH MAIN STREET
WEST HARTFORD, CT

www.Center.com
1000 Main Street
Westfield, CT 06097
Center of Safety

Colco Partnership
d/b/a Verizon Wireless

REV.	DATE	BY	CHK	DESCRIPTION
1	02/28/14	SK	DC	ISSUED FOR BID
2	06/26/14	SK	DC	ISSUED FOR BIDDING
3	08/13/14	SK	DC	ISSUED FOR CONSTRUCTION



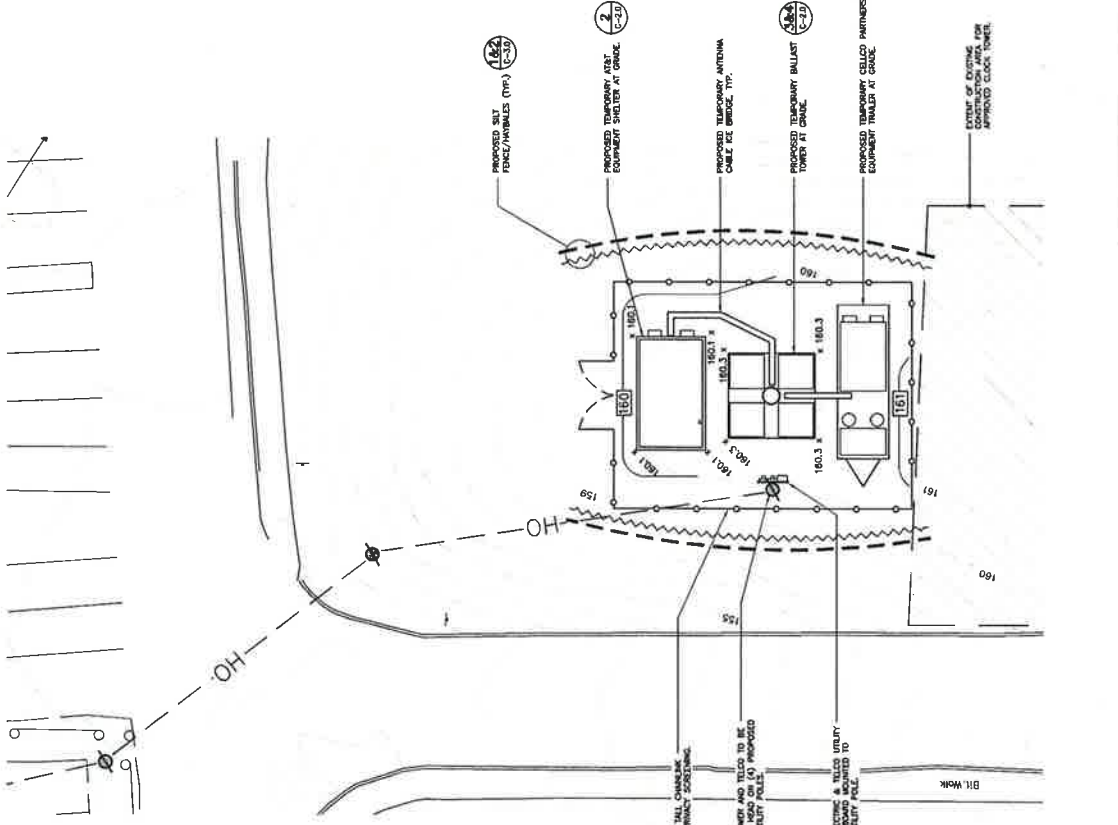
SYMBOLS LEGEND

- PROPERTY LINE
- DRAWN (EXISTING)
- CONTOUR LINE
- EXISTING CONTOUR TREE
- EXISTING DECIDUOUS TREE

SITE PLAN SURVEY REFERENCE NOTE:
EXISTING PROPERTY LINES AND SITE FEATURES SHOWN ON THIS SITE SURVEY PLAN SHEET "C-10" SHALL BE USED TO LOCATE THE TOWER. THE LOCATION SHOWN ON THIS SHEET SHALL BE USED AS A REFERENCE FOR THE TOWER. THE TOWER SHALL BE LOCATED AS SHOWN ON THIS SHEET. THE TOWER SHALL BE CONSTRUCTED AND OPERATED BY THE USER BY PARCELL CORPORATION.

1
PARTIAL SITE SURVEY PLAN
C-10
SCALE: 1" = 70'
GRAPHIC SCALE
1/8" = 10'
1/4" = 20'
1/2" = 40'
1" = 80'
NORTH

PROPOSED SPACES AND TRAILS TO BE
ROUTED OVER ROAD ON (D) PROPOSED
TEMPORARY UTILITY POLES.

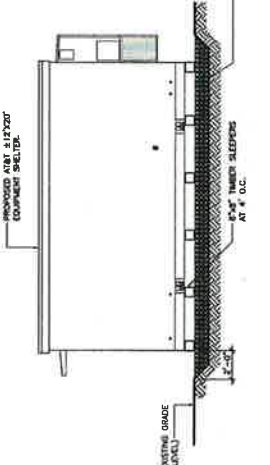
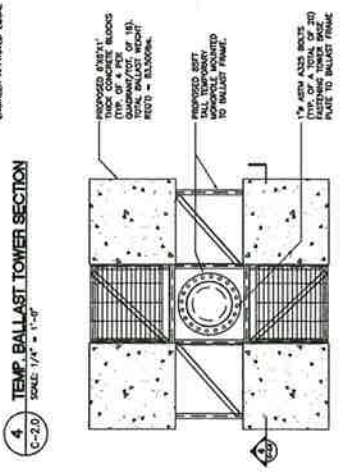
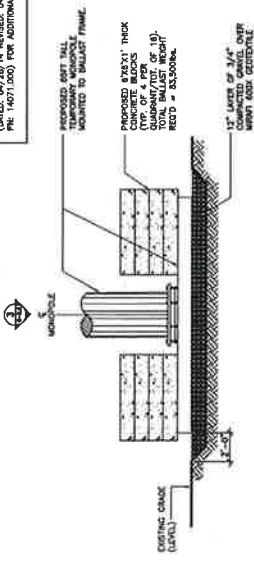


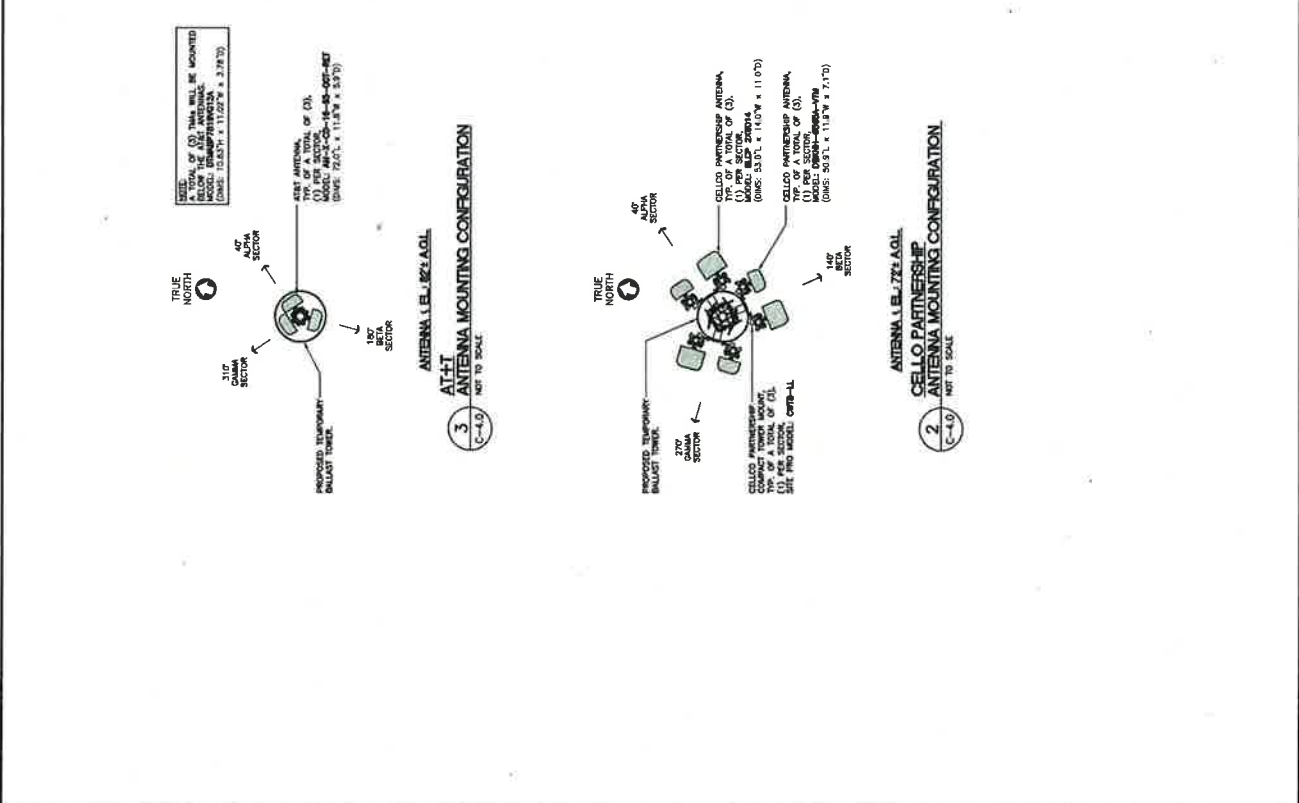
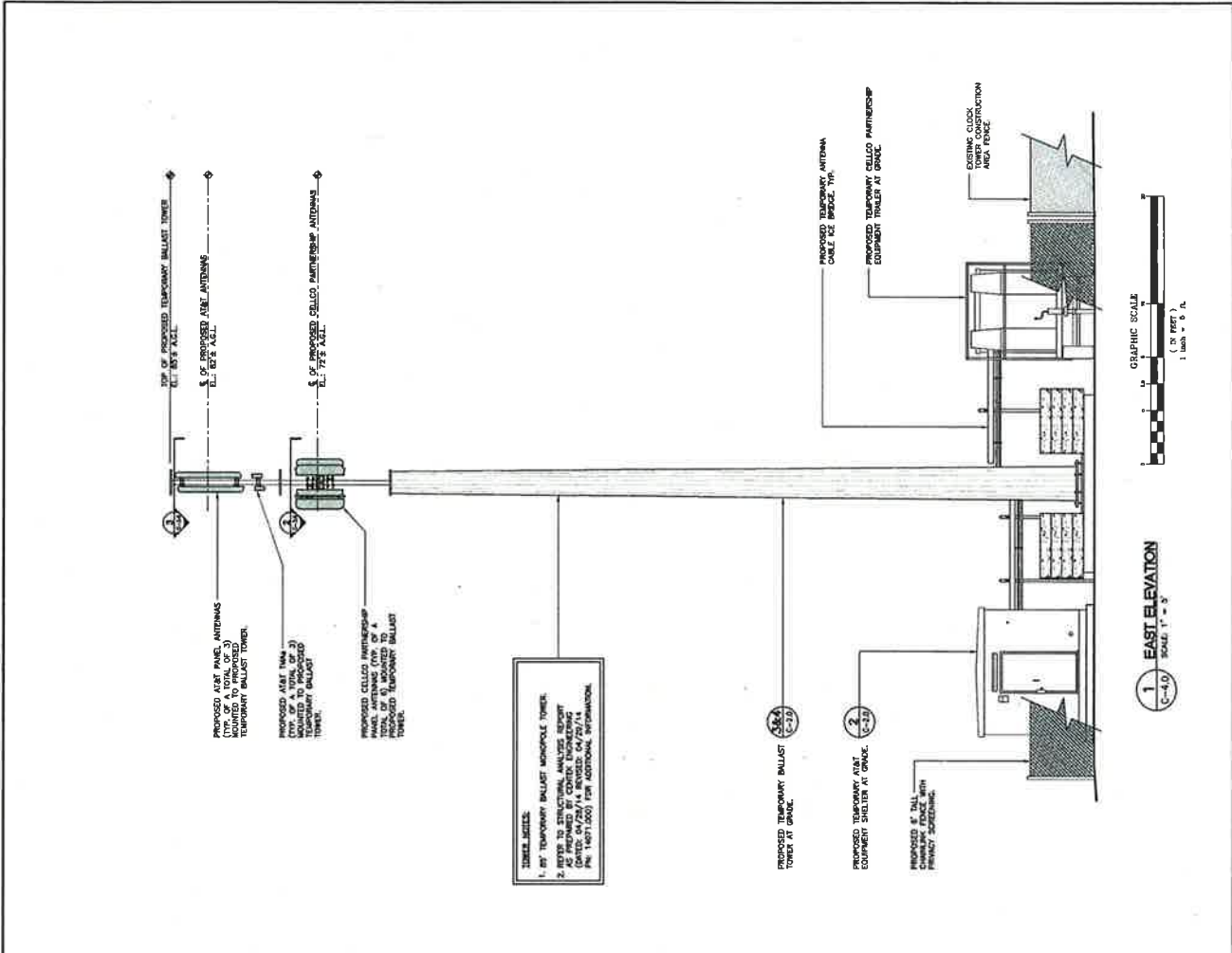
SYMBOLS LEGEND

---	DRIVE (EXISTING)
- - - - -	CONTOUR LINE
150	GRADING LINE
~ ~ ~ ~ ~	STATION FENCE / STRAW BALES / SLEATON FENCE "SANDBART"



NOTE:
 PERFORMED ANALYSIS BASED ON DATA AS PROVIDED BY CENTEX ENGINEERING (DATED 04/28/14 REVISED 04/29/14 PL 1407165) FOR ADDITIONAL INFORMATION





REV.	DATE	CHANGED BY	DESCRIPTION
0	04/14/14	CTC	ISSUED FOR PERMIT
1	04/14/14	CTC	ISSUED FOR PERMIT - CLEAR WORK

ATTACHMENT 3



MEMORANDUM

Date: April 30, 2014

To: Ms. Alexandria Carter
Verizon Wireless
99 East River Drive
East Hartford CT 06108

From: Michael Libertine

Re: Visibility of Temporary Tower
American School for the Deaf
139 North Main Street
West Hartford, Connecticut

In its Decision and Order for Docket 434 (dated June 27, 2013), the Connecticut Siting Council approved an application submitted by Cellco Partnership (d/b/a "Verizon Wireless") for a Certificate of Environmental Compatibility and Public Need in association with the construction, maintenance and operation of a replacement wireless communications facility on the campus of the American School for the Deaf at 139 North Main Street in West Hartford, Connecticut. The approved facility would include a clock tower structure measuring approximately 29 feet by 29 feet in length and width, and rising to a height of 90 feet above ground level ("AGL"), with a 6-foot tall spire affixed to the top of the structure's cupola. The facility has been designed to accommodate Verizon Wireless and New Cingular Wireless PCS LLC (d/b/a "AT&T") both of which are currently located at the campus in a building soon to be razed. The new facilities within the approved clock tower are anticipated to be on air by October of this year.

To accommodate construction sequencing while maintaining existing wireless services at this site, Verizon Wireless requires the installation and operation of a temporary facility. Based on site drawings prepared by Centek Engineering (dated April 28, 2014), the temporary facility would include an 85± foot tall ballast monopole affixed with antennas and tower mounted amplifiers ("TMAs"). The antennas and TMAs would be flush-mounted to the monopole at heights between approximately 70 feet above ground level AGL and 84 feet AGL. The temporary facility would also include equipment shelters for Verizon Wireless and AT&T. The entire temporary facility would be surrounded by a 6-foot tall chain-link fence (with privacy screening) enclosure measuring approximately 40 feet by 60 feet. The temporary facility would be located immediately north of the approved clock tower construction area.

At the request of Verizon Wireless, All-Points Technology Corporation, P.C. ("APT") has evaluated the potential views associated with the proposed temporary tower at the referenced property. The temporary facility is comparable in height and located at similar ground elevation as that of the approved clock tower, but its profile will be much slimmer (diameter of approximately 4 feet at base tapering to about 2.5 feet at top).

Given these characteristics, we anticipate that the view shed of the temporary facility would be comparable to that as presented for the approved clock tower in our Visibility Analysis report (dated November 2012). Views of the temporary facility would be limited to a modest geographic footprint of less than 1,500 feet surrounding the school campus. Since the construction schedule window will occur during the time of year when leaves are on the trees, any views would be substantially obstructed by the intervening mature vegetation in the immediate area, further limiting that footprint. Other than views from immediately in front of the school entrance along North Main Street (looking east), the majority of views would be of the uppermost portion of the temporary tower (above the trees). In addition, with construction apparatus on various portions of the campus for the school's renovation activities (including cranes and man-lifts), the presence of the temporary facility is not likely to be any more noticeable than other similarly tall pieces of equipment.

ATTACHMENT 4

General Power Density

Site Name: W HARTFORD W RELO CT COW
 Cumulative Power Density

Operator	Operating Frequency (MHz)	Number of Trans.	ERP Per Trans. (watts)	Total ERP (watts)	Distance to Target (feet)	Calculated Power Density (mW/cm ²)	Maximum Permissible Exposure* (mW/cm ²)	Fraction of MPE (%)
VZW PCS	1970	2	511	1021	72	0.0708	1.0	7.08%
VZW Cellular	869	8	389	3109	72	0.2157	0.5793333333	37.23%
VZW 700	746	1	475	475	72	0.0330	0.4973333333	6.63%

Total Percentage of Maximum Permissible Exposure

50.94%

*Guidelines adopted by the FCC on August 1, 1996, 47 CFR Part 1 based on NCRP Report 86, 1986 and generally on ANSI/IEEE C95.1-1992

MHz = Megahertz

mW/cm² = milliwatts per square centimeter

ERP = Effective Radiated Power

Absolute worst case maximum values used.

ATTACHMENT 5

W_HARTFORD_W_TEMP_COW.TXT

* Federal Airways & Airspace *
* Summary Report: New Construction *
* Antenna Structure *

Airspace User: Your Name

File: W_HARTFORD_W_TEMP_COW

Location: Hartford, CT
Distance: 3.4 Statute Miles
Direction: 96° (true bearing)

Latitude: 41°-46'-15.31" Longitude: 72°-44'-58.84"

SITE ELEVATION AMSL.....161 ft.
STRUCTURE HEIGHT..... 85 ft.
OVERALL HEIGHT AMSL.....246 ft.

NOTICE CRITERIA

- FAR 77.9(a): NNR (DNE 200 ft AGL)
- FAR 77.9(b): NNR (DNE Notice Slope)
- FAR 77.9(c): NNR (Not a Traverse Way)
- FAR 77.9: NNR FAR 77.9 IFR Straight-In Notice Criteria for HFD
- FAR 77.9: NNR FAR 77.9 IFR Straight-In Notice Criteria for 4B8
- FAR 77.9(d): NNR (Off Airport Construction)

NR = Notice Required
NNR = Notice Not Required
PNR = Possible Notice Required (depends upon actual IFR procedure)
For new construction review Air Navigation Facilities at bottom of this report.

Notice to the FAA is not required at the analyzed location and height for slope, height or Straight-In procedures. Please review the 'Air Navigation' section for notice requirements for offset IFR procedures and EMI.

OBSTRUCTION STANDARDS

- FAR 77.17(a)(1): DNE 499 ft AGL
- FAR 77.17(a)(2): DNE - Airport Surface
- FAR 77.19(a): DNE - Horizontal Surface
- FAR 77.19(b): DNE - Conical Surface
- FAR 77.19(c): DNE - Primary Surface
- FAR 77.19(d): DNE - Approach Surface
- FAR 77.19(e): DNE - Transitional Surface

VFR TRAFFIC PATTERN AIRSPACE FOR: HFD: HARTFORD-BRAINARD

Type: A RD: 28473.03 RE: 13.9
FAR 77.17(a)(1): DNE
FAR 77.17(a)(2): DNE - Height No Greater Than 200 feet AGL.
VFR Horizontal Surface: DNE
VFR Conical Surface: DNE
VFR Approach Slope: DNE
VFR Transitional Slope: DNE

VFR TRAFFIC PATTERN AIRSPACE FOR: 4B8: ROBERTSON FIELD

Type: A RD: 41874.55 RE: 201.6
FAR 77.17(a)(1): DNE
FAR 77.17(a)(2): DNE - Greater Than 5.99 NM.
VFR Horizontal Surface: DNE
VFR Conical Surface: DNE

VFR Approach Slope: DNE
 VFR Transitional Slope: DNE

TERPS DEPARTURE PROCEDURE (FAA Order 8260.3, Volume 4)
 FAR 77.17(a)(3) Departure Surface Criteria (40:1)
 DNE Departure Surface

MINIMUM OBSTACLE CLEARANCE ALTITUDE (MOCA)
 FAR 77.17(a)(4): DNE - No Airway Found

PRIVATE LANDING FACILITIES

FACIL IDENT TYP NAME	BEARING To FACIL	RANGE IN NM	DELTA ARP ELEVATION	FAA IFR
OCT5 HEL ST FRANCIS HOSPITAL No Impact to Private Landing Facility Structure is beyond notice limit by 8914 feet.	84.69	2.29	+62	
OCT9 HEL HARTFORD HOSPITAL No Impact to Private Landing Facility Structure is beyond notice limit by 15294 feet.	107.23	3.34	+35	
CT06 HEL DELTA ONE No Impact to Private Landing Facility Structure is beyond notice limit by 20155 feet.	74.89	4.14	+225	
CT88 HEL RENTSCHLER No Impact to Private Landing Facility Structure is beyond notice limit by 28722 feet.	101	5.55	+198	
CT73 HEL SOUTH MEADOWS No Impact to Private Landing Facility Structure is beyond notice limit by 29816 feet.	239.88	5.73	+46	
CT05 HEL KAMAN AEROSPACE CORP No Impact to Private Landing Facility Structure is beyond notice limit by 30849 feet.	22.3	5.9	+82	

AIR NAVIGATION ELECTRONIC FACILITIES

APCH BEAR	FAC IDNT	ST TYPE	AT	FREQ	VECTOR	DIST (ft)	DELTA ELEVA	ST LOCATION	GRND ANGLE
	HFD	ATCT	Y	A/G	116.11	29825	+171	CT HARTFORD-BRAINARD	.33
	HFD	LOCALIZER	I	109.7	117.47	30210	+235	CT RWY 02 HARTFORD-B	.45
2	BDL	RADAR	ON		16.66	63771	+10	CT BRADLEY INTL	.01
No Impact. This structure does not require Notice based upon EMI. The studied location is within 20 NM of a Radar facility. The calculated Radar Line-Of-Sight (LOS) distance is: 38 NM. This location and height is within the Radar Line-Of-Sight.									
	HFD	VOR/DME	R	114.9	130.65	72724	-603	CT HARTFORD	-.48
	BAF	VORTAC	R	113.0	3.65	142782	-21	MA BARNES	-.01
	CEF	VORTAC	R	114.0	21.26	166901	+5	MA WESTOVER	0.00
	MAD	VOR/DME	R	110.4	174.62	167294	+26	CT MADISON	.01

W_HARTFORD_W_TEMP_COW.TXT

HVN	VOR/DME	R	109.8	191.28	189005	+240	CT NEW HAVEN	.07
CTR	VOR/DME	I	115.1	344.1	197243	-1354	MA CHESTER	-.39

FCC AM PROOF-OF-PERFORMANCE

NOT REQUIRED: Structure is not near a FCC licensed AM radio station Proof-of-Performance is not required. Please review AM Station Report for details.

Nearest AM Station: WTIC @ 4678 meters.

Airspace® Summary Version 14.3.352

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04-29-2014
09:41:32

ATTACHMENT 6

Structural Analysis Report

85-ft Ballast Monopole

Proposed Temporary Antenna Installation

Verizon Site Ref: West Hartford West

AT&T Site Ref: CT1173

*139 North Main Street
West Hartford, CT*

Centek Project No. 14071.000

~~*Date: April 28, 2014*~~

Rev 1: April 29, 2014

Prepared for:



AT&T Mobility
500 Enterprise Drive,
Suite 3A
Rocky Hill, CT 06067



Verizon Wireless
99 East River Road,
9th Floor
East Hartford, CT 06108

CEN TEK Engineering, Inc.
Structural Analysis - 85-ft Ballast Monopole
Verizon Wireless – West Hartford West
AT&T Site Ref – CT1173
West Hartford, CT
Rev 1 ~ April 29, 2014

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- TOWER CAPACITY.
- ANCHORS AND BALLAST.
- CONCLUSION.

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- GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM.

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- tnxTower DETAILED OUTPUT.
- ANCHOR BOLT AND BASE PLATE ANALYSIS.
- MathCAD BALLAST CALCULATION.

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- AT&T EQUIPMENT DATA SHEETS.

CEN TEK Engineering, Inc.
Structural Analysis - 85-ft Ballast Monopole
Verizon Wireless – West Hartford West
AT&T Site Ref – CT1173
West Hartford, CT
Rev 1 ~ April 29, 2014

Introduction

The purpose of this report is to summarize the results of the non-linear, P- Δ structural analysis of the temporary antenna installation proposed by Verizon Wireless and AT&T Mobility on the ballast monopole (tower) located in West Hartford, CT.

The host tower is a 85-ft tall, three-section, ballasted monopole. Original design information was unavailable for use in this report. The tower geometry was obtained from a previous structural analysis prepared by Advanced Engineering Group dated September 19, 2013.

Antenna and appurtenance information were obtained from Verizon and a AT&T RF data sheet.

The tower consists of three (3) vertical steel pipe sections conforming to ASTM A53 Grade B (35ksi). The diameter of the pole (flat-flat) is 6.00-in at the top and 24.00-in at the base.

Verizon proposes the installation of six (6) panel antennas on a temporary ballast monopole. Refer to the Antenna and Appurtenance Summary below for a detailed description of the proposed antenna and appurtenance configuration.

AT&T proposes the installation of three (3) panel antennas and three (3) TMA's on a temporary ballast monopole. Refer to the Antenna and Appurtenance Summary below for a detailed description of the proposed antenna and appurtenance configuration.

Antenna and Appurtenance Summary

The tower was designed to support several communication antennas. The existing, proposed and future loads considered in this analysis consist of the following:

- **AT&T (Proposed):**
Antennas: Three (3) KMW AM-X-CD-16-65-00T panel antennas and three (3) CCI DTMABP7819VG12A TMA's flush mounted with a RAD center elevation of 82-ft above grade level.
Coax Cables: Six (6) 7/8" \varnothing coax cables running on the inside of the monopole.
- **VERIZON (Proposed):**
Antennas: Three (3) Swedcom SLCP 2X6014 and three (3) Andrew DBXNH-6565A panel antennas mounted on three (3) Site Pro WiMax Compact Tower Mounts p/n CWT8-LL with a RAD center elevation of 72-ft above grade level.
Coax Cables: Six (6) 7/8" \varnothing coax cables running on the inside of the monopole and twelve (12) 7/8" \varnothing coax cables running on the exterior of the monopole.
(Note: Contractor to install maximum number of cables permissible within monopole and band remainder to exterior of pole.)

CEN TEK Engineering, Inc.
Structural Analysis - 85-ft Ballast Monopole
Verizon Wireless – West Hartford West
AT&T Site Ref – CT1173
West Hartford, CT
Rev 1 ~ April 29, 2014

Primary Assumptions Used in the Analysis

- The tower structure's theoretical capacity not including any assessment of the condition of the tower.
- The tower carries the horizontal and vertical loads due to the weight of antennas, ice load and wind.
- Tower is properly installed and maintained.
- Tower is in plumb condition.
- Tower loading for antennas and mounts as listed in this report.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds are fabricated with ER-70S-6 electrodes.
- All members are assumed to be as specified in the original tower design documents.
- All members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
- All member protective coatings are in good condition.
- All tower members were properly designed, detailed, fabricated, installed and have been properly maintained since erection.
- Any deviation from the analyzed antenna loading will require a new analysis for verification of structural adequacy.
- **All coax cables routed as specified in Section 3 of this report.**

Analysis

The existing tower was analyzed using a comprehensive computer program entitled trnTower. The program analyzes the tower, considering the worst case loading condition. The tower is considered as loaded by concentric forces along the tower shaft, and the model assumes that the shaft members are subjected to bending, axial, and shear forces.

The existing tower was analyzed for the controlling basic wind speed (fastest mile) with no ice and a 75% reduction of wind force with ½ inch accumulative ice to determine stresses in members as per guidelines of TIA/EIA-222-F-96 entitled "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures", the American Institute of Steel Construction (AISC) and the Manual of Steel Construction; Allowable Stress Design (ASD).

The controlling wind speed is determined by evaluating the local available wind speed data as provided in Appendix K of the CSBC¹ and the wind speed data available in the TIA/EIA-222-F-96 Standard. The higher of the two wind speeds is utilized in preparation on the tower analysis.

Tower Loading

Tower loading was determined by the basic wind speed as applied to projected surface areas with modification factors per TIA/EIA-222-F, gravity loads of the tower structure and its components, and the application of ½" radial ice on the tower structure and its components.

Basic Wind Speed:	Hartford; v = 80 mph (fastest mile) West Hartford; v = 95 mph (3 second gust) equivalent to v = 77.5 mph (fastest mile) <i>TIA/EIA wind speed controls.</i>	[Section 16 of TIA/EIA-222-F-96] [Appendix K of the 2005 CT Building Code Supplement]
Load Cases:	<u>Load Case 1</u> ; 80 mph wind speed w/ no ice plus gravity load – used in calculation of tower stresses and rotation. <u>Load Case 2</u> ; 69 mph wind speed w/ ½" radial ice plus gravity load – used in calculation of tower stresses. The 69 mph wind speed velocity represents 75% of the wind pressure generated by the 80 mph wind speed. <u>Load Case 3</u> ; Seismic – not checked	[Section 2.3.16 of TIA/EIA-222-F-96] [Section 2.3.16 of TIA/EIA-222-F-96] [Section 1614.5 of State Bldg. Code 2005] does not control in the design of this structure type

¹ The 2005 Connecticut State Building Code as amended by the 2009 CT State Supplement. (CSBC)

Tower Capacity

Tower stresses were calculated utilizing the structural analysis software tnxTower. Allowable stresses were determined based on Table 5 of the TIA/EIA code with a 1/3 increase per Section 3.1.1.1 of the same code.

- Calculated stresses were found to be within allowable limits. In Load Case 1, per tnxTower “Section Capacity Table”, this tower was found to be at **83.6%** of its total capacity.

Tower Section	Elevation (AGL)	Stress Ratio (percentage of capacity)	Result
Pole Shaft (L1)	66.5'-86.5'	83.6%	PASS
Pole Shaft (L2)	36.5'-66.5'	32.9%	PASS
Pole Shaft (L3)	1.5'-36.5'	76.5%	PASS

Anchor s and Ballast

The base of the tower is connected to the ballast frame by means of (20) 1.00"Ø, ASTM A325 anchor bolts on a 27" Ø bolt circle.

- The tower reactions developed from the governing Load Case 1 were used in the verification of the foundation:

Reactions	Vector	Proposed Base Reactions
Base	Shear	6 kips
	Compression	9 kips
	Moment	313 kip-ft

- The anchor bolts were found to be within allowable limits.

Tower Component	Component	Stress Ratio (percentage of capacity)	Result
Anchor Bolts	Combined Compression and Shear	83.8%	PASS
Base Plate	Bending	73.2%	PASS

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- The ballast frame was found to be within allowable limits.

Foundation	Design Limit	IBC 2003/2005 CT State Building Code Section 3108.4.2 (FS) ⁽¹⁾	Proposed Loading (FS) ⁽¹⁾	Result
Ballast frame w/ 83,500 lbs of conc. blocks	OTM ⁽²⁾	2.0	2.22	PASS

Note 1: FS denotes Factor of Safety.

Note 2: OTM denotes Overturning Moment

Conclusion

This analysis shows that the subject tower **is adequate** to support the proposed temporary antenna configuration.

The analysis is based, in part, on the information provided to this office by Verizon Wireless and AT&T Mobility. If the existing conditions are different than the information in this report, Centek Engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:



Carlo F. Centore, PE
 Principal ~ Structural Engineer



Prepared by:



Timothy J. Lynn, PE
 Structural Engineer

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Standard Conditions for Furnishing of
Professional Engineering Services on
Existing Structures

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CENTEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provide to CENTEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an uncorroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the “as new” condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222
- All services performed, results obtained, and recommendations made are in accordance with generally accepted engineering principles and practices. CENTEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

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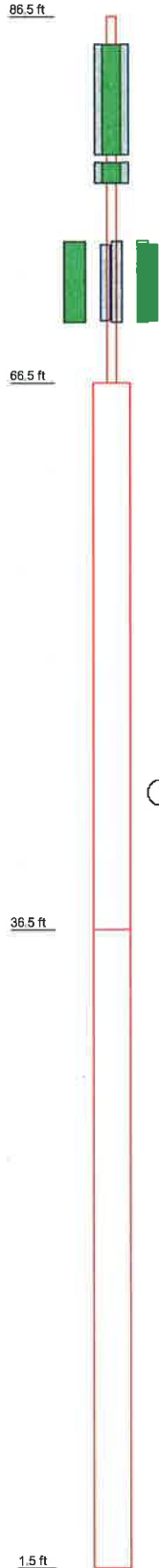
General Description of Structural Analysis Program

tnxTower, is an integrated structural analysis and design software package for Designed specifically for the telecommunications industry, tnxTower, formerly ERITower, automates much of the tower analysis and design required by the TIA/EIA 222 Standard.

tnxTower Features:

- tnxTower can analyze and design 3- and 4-sided guyed towers, 3- and 4-sided self-supporting towers and either round or tapered ground mounted poles with or without guys.
- The program analyzes towers using the TIA-222-G (2005) standard or any of the previous TIA/EIA standards back to RS-222 (1959). Steel design is checked using the AISC ASD 9th Edition or the AISC LRFD specifications.
- Linear and non-linear (P-delta) analyses can be used in determining displacements and forces in the structure. Wind pressures and forces are automatically calculated.
- Extensive graphics plots include material take-off, shear-moment, leg compression, displacement, twist, feed line, guy anchor and stress plots.
- tnxTower contains unique features such as True Cable behavior, hog rod take-up, foundation stiffness and much more.

Section	1				
Size	P6x.432				
Length (ft)	20.00				
Grade	A53-B-35				
Weight (K)	0.6				
Section	2				
Size	P24x3/8				
Length (ft)	30.00				
Grade	A53-B-35				
Weight (K)	2.8				
Section	3				
Size	P24x3/8				
Length (ft)	35.00				
Grade	A53-B-35				
Weight (K)	3.3				
Section					6.7
Weight (K)					



DESIGNED APPURTENANCE LOADING

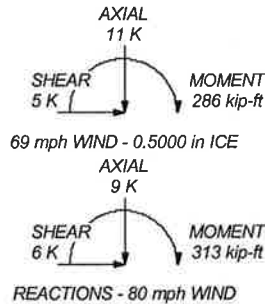
TYPE	ELEVATION	TYPE	ELEVATION
AM-X-CD-16-65-00T-RET(72") (ATI Proposed)	82	DBXNH-6565A-VTM (Verizon Proposed)	72
AM-X-CD-16-65-00T-RET(72") (ATI Proposed)	82	SLCP 2x6014 (Verizon Proposed)	72
AM-X-CD-16-65-00T-RET(72") (ATI Proposed)	82	DBXNH-6565A-VTM (Verizon Proposed)	72
Valmont Uni-Tri Bracket (ATI Proposed)	82	SLCP 2x6014 (Verizon Proposed)	72
DTMABP7819VG12A TMA (ATI Proposed)	78	DBXNH-6565A-VTM (Verizon Proposed)	72
DTMABP7819VG12A TMA (ATI Proposed)	78	Site Pro Compact Tower Mount CWT8 (Verizon Proposed)	72
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SLCP 2x6014 (Verizon Proposed)	72		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A53-B-35	35 ksi	63 ksi			

TOWER DESIGN NOTES

1. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
2. Tower is also designed for a 69 mph basic wind with 0.50 in ice.
3. Deflections are based upon a 50 mph wind.
4. TOWER RATING: 83.6%



Centek Engineering Inc.
 63-2 North Branford Rd.
 Branford, CT 06405
 Phone: (203) 488-0580
 FAX: (203) 488-8587

Job: 14071.000 - West Hartford West	Project: 85-ft Temporary Monopole - West Hartford, CT
Client: Verizon Wireless	Drawn by: TJL
Code: TIA/EIA-222-F	Date: 04/29/14
Path:	Scale: NTS
	Dwg No. E-1

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	Client Verizon Wireless	Designed by TJL

Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Basic wind speed of 80 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 69 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

<ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification √ Use Code Stress Ratios √ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC .6D+W Combination 	<ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension √ Bypass Mast Stability Checks Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas SR Members Have Cut Ends √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing 	<ul style="list-style-type: none"> Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation Consider Feedline Torque Include Angle Block Shear Check <li style="text-align: center;">Poles √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
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Pole Section Geometry

Section	Elevation <i>ft</i>	Section Length <i>ft</i>	Pole Size	Pole Grade	Socket Length <i>ft</i>
L1	86.50-66.50	20.00	P6x.432	A53-B-35 (35 ksi)	
L2	66.50-36.50	30.00	P24x3/8	A53-B-35 (35 ksi)	
L3	36.50-1.50	35.00	P24x3/8	A53-B-35 (35 ksi)	

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Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft ²	in					in	in
L1 86.50-66.50				1	1	1		
L2 66.50-36.50				1	1	1		
L3 36.50-1.50				1	1	1		

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	C_{AA}	Weight
				ft		ft ² /ft	plf
7/8 (Verizon Proposed)	C	No	Inside Pole	73.50 - 4.50	6	No Ice 1/2" Ice	0.00 0.54
7/8 (AT&T Proposed)	C	No	Inside Pole	83.50 - 4.50	6	No Ice 1/2" Ice	0.00 0.54
7/8 (Verizon Proposed)	C	No	CaAa (Out Of Face)	73.50 - 4.50	4	No Ice 1/2" Ice	0.11 1.52
7/8 (Verizon Proposed)	C	No	CaAa (Out Of Face)	73.50 - 4.50	8	No Ice 1/2" Ice	0.00 1.52

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation	Face	A_R	A_F	C_{AA} In Face	C_{AA} Out Face	Weight
	ft		ft ²	ft ²	ft ²	ft ²	K
L1	86.50-66.50	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	3.108	0.12
L2	66.50-36.50	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	13.320	0.39
L3	36.50-1.50	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	14.208	0.41

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation	Face or Leg	Ice Thickness	A_R	A_F	C_{AA} In Face	C_{AA} Out Face	Weight
	ft		in	ft ²	ft ²	ft ²	ft ²	K
L1	86.50-66.50	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	5.908	0.21
L2	66.50-36.50	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	25.320	0.74
L3	36.50-1.50	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	27.007	0.79

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Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment °	Placement ft	CAA		Weight K	
			Horz Lateral ft	Vert ft			Front ft ²	Side ft ²		
AM-X-CD-16-65-00T-RET(7 2") (AT&T Proposed)	A	From Face	0.50 0.00 0.00		0.0000	82.00	No Ice 1/2" Ice	8.26 8.81	4.64 5.09	0.05 0.10
AM-X-CD-16-65-00T-RET(7 2") (AT&T Proposed)	B	From Face	0.50 0.00 0.00		0.0000	82.00	No Ice 1/2" Ice	8.26 8.81	4.64 5.09	0.05 0.10
AM-X-CD-16-65-00T-RET(7 2") (AT&T Proposed)	C	From Face	0.50 0.00 0.00		0.0000	82.00	No Ice 1/2" Ice	8.26 8.81	4.64 5.09	0.05 0.10
DTMABP7819VG12A TMA (AT&T Proposed)	A	From Face	0.50 0.00 0.00		0.0000	78.00	No Ice 1/2" Ice	1.59 1.76	0.58 0.70	0.02 0.03
DTMABP7819VG12A TMA (AT&T Proposed)	B	From Face	0.50 0.00 0.00		0.0000	78.00	No Ice 1/2" Ice	1.59 1.76	0.58 0.70	0.02 0.03
DTMABP7819VG12A TMA (AT&T Proposed)	C	From Face	0.50 0.00 0.00		0.0000	78.00	No Ice 1/2" Ice	1.59 1.76	0.58 0.70	0.02 0.03
Valmont Uni-Tri Bracket (AT&T Proposed)	C	None			0.0000	82.00	No Ice 1/2" Ice	1.75 1.94	1.75 1.94	0.29 0.31
SLCP 2x6014 (Verizon Proposed)	A	From Face	0.50 2.00 0.00		0.0000	72.00	No Ice 1/2" Ice	7.21 7.65	5.67 6.09	0.02 0.07
DBXNH-6565A-VTM (Verizon Proposed)	A	From Face	0.50 -2.00 0.00		0.0000	72.00	No Ice 1/2" Ice	5.89 6.30	3.53 3.89	0.04 0.07
SLCP 2x6014 (Verizon Proposed)	B	From Face	0.50 2.00 0.00		0.0000	72.00	No Ice 1/2" Ice	7.21 7.65	5.67 6.09	0.02 0.07
DBXNH-6565A-VTM (Verizon Proposed)	B	From Face	0.50 -2.00 0.00		0.0000	72.00	No Ice 1/2" Ice	5.89 6.30	3.53 3.89	0.04 0.07
SLCP 2x6014 (Verizon Proposed)	C	From Face	0.50 2.00 0.00		0.0000	72.00	No Ice 1/2" Ice	7.21 7.65	5.67 6.09	0.02 0.07
DBXNH-6565A-VTM (Verizon Proposed)	C	From Face	0.50 -2.00 0.00		0.0000	72.00	No Ice 1/2" Ice	5.89 6.30	3.53 3.89	0.04 0.07
Site Pro Compact Tower Mount CWT8 (Verizon Proposed)	A	None			0.0000	72.00	No Ice 1/2" Ice	2.85 4.05	2.85 4.05	0.15 0.20
Site Pro Compact Tower Mount CWT8 (Verizon Proposed)	B	None			0.0000	72.00	No Ice 1/2" Ice	2.85 4.05	2.85 4.05	0.15 0.20
Site Pro Compact Tower Mount CWT8 (Verizon Proposed)	C	None			0.0000	72.00	No Ice 1/2" Ice	2.85 4.05	2.85 4.05	0.15 0.20

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	Client	Verizon Wireless	Designed by	TJL

Tower Pressures - No Ice

$G_H = 1.690$

Section Elevation	z	K _Z	q _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face	C _A A _A Out Face
ft	ft		psf	ft ²		ft ²	ft ²	ft ²		ft ²	ft ²
L1 86.50-66.50	76.50	1.272	21	11.042	A	0.000	11.042	11.042	100.00	0.000	0.000
					B	0.000	11.042	100.00	0.000	0.000	
					C	0.000	11.042	100.00	0.000	3.108	
L2 66.50-36.50	51.50	1.136	19	60.000	A	0.000	60.000	60.000	100.00	0.000	0.000
					B	0.000	60.000	100.00	0.000	0.000	
					C	0.000	60.000	100.00	0.000	13.320	
L3 36.50-1.50	19.00	1	16	70.000	A	0.000	70.000	70.000	100.00	0.000	0.000
					B	0.000	70.000	100.00	0.000	0.000	
					C	0.000	70.000	100.00	0.000	14.208	

Tower Pressure - With Ice

$G_H = 1.690$

Section Elevation	z	K _Z	q _z	t _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face	C _A A _A Out Face
ft	ft		psf	in	ft ²		ft ²	ft ²	ft ²		ft ²	ft ²
L1 86.50-66.50	76.50	1.272	16	0.5000	12.708	A	0.000	12.708	12.708	100.00	0.000	0.000
						B	0.000	12.708	100.00	0.000	0.000	
						C	0.000	12.708	100.00	0.000	5.908	
L2 66.50-36.50	51.50	1.136	14	0.5000	62.500	A	0.000	62.500	62.500	100.00	0.000	0.000
						B	0.000	62.500	100.00	0.000	0.000	
						C	0.000	62.500	100.00	0.000	25.320	
L3 36.50-1.50	19.00	1	12	0.5000	72.917	A	0.000	72.917	72.917	100.00	0.000	0.000
						B	0.000	72.917	100.00	0.000	0.000	
						C	0.000	72.917	100.00	0.000	27.007	

Tower Pressure - Service

$G_H = 1.690$

Section Elevation	z	K _Z	q _z	A _G	F a c e	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face	C _A A _A Out Face
ft	ft		psf	ft ²		ft ²	ft ²	ft ²		ft ²	ft ²
L1 86.50-66.50	76.50	1.272	8	11.042	A	0.000	11.042	11.042	100.00	0.000	0.000
					B	0.000	11.042	100.00	0.000	0.000	
					C	0.000	11.042	100.00	0.000	3.108	
L2 66.50-36.50	51.50	1.136	7	60.000	A	0.000	60.000	60.000	100.00	0.000	0.000
					B	0.000	60.000	100.00	0.000	0.000	
					C	0.000	60.000	100.00	0.000	13.320	
L3 36.50-1.50	19.00	1	6	70.000	A	0.000	70.000	70.000	100.00	0.000	0.000
					B	0.000	70.000	100.00	0.000	0.000	
					C	0.000	70.000	100.00	0.000	14.208	

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Tower Forces - No Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 86.50-66.50	0.12	0.57	A	1	0.808	1	1	1	11.042	0.42	21.18	C
			B	1	0.808	1	1	11.042				
			C	1	0.808	1	1	11.042				
L2 66.50-36.50	0.39	2.84	A	1	0.59	1	1	1	60.000	1.53	51.06	C
			B	1	0.59	1	1	60.000				
			C	1	0.59	1	1	60.000				
L3 36.50-1.50	0.41	3.31	A	1	0.59	1	1	1	70.000	1.54	43.91	C
			B	1	0.59	1	1	70.000				
			C	1	0.59	1	1	70.000				
Sum Weight:	0.93	6.73						OTM	135.26 kip-ft	3.49		

Tower Forces - No Ice - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 86.50-66.50	0.12	0.57	A	1	0.808	1	1	1	11.042	0.42	21.18	C
			B	1	0.808	1	1	11.042				
			C	1	0.808	1	1	11.042				
L2 66.50-36.50	0.39	2.84	A	1	0.59	1	1	1	60.000	1.53	51.06	C
			B	1	0.59	1	1	60.000				
			C	1	0.59	1	1	60.000				
L3 36.50-1.50	0.41	3.31	A	1	0.59	1	1	1	70.000	1.54	43.91	C
			B	1	0.59	1	1	70.000				
			C	1	0.59	1	1	70.000				
Sum Weight:	0.93	6.73						OTM	135.26 kip-ft	3.49		

Tower Forces - No Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 86.50-66.50	0.12	0.57	A	1	0.808	1	1	1	11.042	0.42	21.18	C
			B	1	0.808	1	1	11.042				
			C	1	0.808	1	1	11.042				
L2 66.50-36.50	0.39	2.84	A	1	0.59	1	1	1	60.000	1.53	51.06	C
			B	1	0.59	1	1	60.000				
			C	1	0.59	1	1	60.000				
L3 36.50-1.50	0.41	3.31	A	1	0.59	1	1	1	70.000	1.54	43.91	C
			B	1	0.59	1	1	70.000				
			C	1	0.59	1	1	70.000				
Sum Weight:	0.93	6.73						OTM	135.26 kip-ft	3.49		

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Tower Forces - No Ice - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 86.50-66.50	0.12	0.57	A	1	0.808	1	1	1	11.042	0.42	21.18	C
			B	1	0.808	1	1	1	11.042			
			C	1	0.808	1	1	1	11.042			
L2 66.50-36.50	0.39	2.84	A	1	0.59	1	1	1	60.000	1.53	51.06	C
			B	1	0.59	1	1	1	60.000			
			C	1	0.59	1	1	1	60.000			
L3 36.50-1.50	0.41	3.31	A	1	0.59	1	1	1	70.000	1.54	43.91	C
			B	1	0.59	1	1	1	70.000			
			C	1	0.59	1	1	1	70.000			
Sum Weight:	0.93	6.73						OTM 135.26 kip-ft	3.49			

Tower Forces - With Ice - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 86.50-66.50	0.21	0.66	A	1	0.974	1	1	1	12.708	0.48	24.15	C
			B	1	0.974	1	1	1	12.708			
			C	1	0.974	1	1	1	12.708			
L2 66.50-36.50	0.74	3.29	A	1	0.59	1	1	1	62.500	1.47	48.89	C
			B	1	0.59	1	1	1	62.500			
			C	1	0.59	1	1	1	62.500			
L3 36.50-1.50	0.79	3.84	A	1	0.59	1	1	1	72.917	1.45	41.55	C
			B	1	0.59	1	1	1	72.917			
			C	1	0.59	1	1	1	72.917			
Sum Weight:	1.74	7.79						OTM 135.01 kip-ft	3.40			

Tower Forces - With Ice - Wind 45 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	R _R	D _F	D _R	A _E ft ²	F K	w plf	Ctrl. Face
L1 86.50-66.50	0.21	0.66	A	1	0.974	1	1	1	12.708	0.48	24.15	C
			B	1	0.974	1	1	1	12.708			
			C	1	0.974	1	1	1	12.708			
L2 66.50-36.50	0.74	3.29	A	1	0.59	1	1	1	62.500	1.47	48.89	C
			B	1	0.59	1	1	1	62.500			
			C	1	0.59	1	1	1	62.500			
L3 36.50-1.50	0.79	3.84	A	1	0.59	1	1	1	72.917	1.45	41.55	C
			B	1	0.59	1	1	1	72.917			
			C	1	0.59	1	1	1	72.917			

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
Sum Weight:	1.74	7.79						OTM	135.01 kip-ft	3.40		

Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 86.50-66.50	0.21	0.66	A	1	0.974	1	1	1	12.708	0.48	24.15	C
			B	1	0.974	1	1	1	12.708			
			C	1	0.974	1	1	1	12.708			
L2 66.50-36.50	0.74	3.29	A	1	0.59	1	1	1	62.500	1.47	48.89	C
			B	1	0.59	1	1	1	62.500			
			C	1	0.59	1	1	1	62.500			
L3 36.50-1.50	0.79	3.84	A	1	0.59	1	1	1	72.917	1.45	41.55	C
			B	1	0.59	1	1	1	72.917			
			C	1	0.59	1	1	1	72.917			
Sum Weight:	1.74	7.79						OTM	135.01 kip-ft	3.40		

Tower Forces - With Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 86.50-66.50	0.21	0.66	A	1	0.974	1	1	1	12.708	0.48	24.15	C
			B	1	0.974	1	1	1	12.708			
			C	1	0.974	1	1	1	12.708			
L2 66.50-36.50	0.74	3.29	A	1	0.59	1	1	1	62.500	1.47	48.89	C
			B	1	0.59	1	1	1	62.500			
			C	1	0.59	1	1	1	62.500			
L3 36.50-1.50	0.79	3.84	A	1	0.59	1	1	1	72.917	1.45	41.55	C
			B	1	0.59	1	1	1	72.917			
			C	1	0.59	1	1	1	72.917			
Sum Weight:	1.74	7.79						OTM	135.01 kip-ft	3.40		

Tower Forces - Service - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 86.50-66.50	0.12	0.57	A	1	1.2	1	1	1	11.042	0.22	11.25	C
			B	1	1.2	1	1	1	11.042			

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L2 66.50-36.50	0.39	2.84	C	1	1.2	1	1	1	11.042	0.60	19.95	C
			A	1	0.59	1	1	1	60.000			
			B	1	0.59	1	1	1	60.000			
L3 36.50-1.50	0.41	3.31	C	1	0.59	1	1	1	60.000	0.60	17.15	C
			A	1	0.59	1	1	1	70.000			
			B	1	0.59	1	1	1	70.000			
Sum Weight:	0.93	6.73	C	1	0.59	1	1	70.000	57.30 kip-ft	1.42		

Tower Forces - Service - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 86.50-66.50	0.12	0.57	A	1	1.2	1	1	1	11.042	0.22	11.25	C
			B	1	1.2	1	1	1	11.042			
			C	1	1.2	1	1	1	11.042			
L2 66.50-36.50	0.39	2.84	A	1	0.59	1	1	1	60.000	0.60	19.95	C
			B	1	0.59	1	1	1	60.000			
			C	1	0.59	1	1	1	60.000			
L3 36.50-1.50	0.41	3.31	A	1	0.59	1	1	1	70.000	0.60	17.15	C
			B	1	0.59	1	1	1	70.000			
			C	1	0.59	1	1	1	70.000			
Sum Weight:	0.93	6.73	C	1	0.59	1	1	70.000	57.30 kip-ft	1.42		

Tower Forces - Service - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 86.50-66.50	0.12	0.57	A	1	1.2	1	1	1	11.042	0.22	11.25	C
			B	1	1.2	1	1	1	11.042			
			C	1	1.2	1	1	1	11.042			
L2 66.50-36.50	0.39	2.84	A	1	0.59	1	1	1	60.000	0.60	19.95	C
			B	1	0.59	1	1	1	60.000			
			C	1	0.59	1	1	1	60.000			
L3 36.50-1.50	0.41	3.31	A	1	0.59	1	1	1	70.000	0.60	17.15	C
			B	1	0.59	1	1	1	70.000			
			C	1	0.59	1	1	1	70.000			
Sum Weight:	0.93	6.73	C	1	0.59	1	1	70.000	57.30 kip-ft	1.42		

Tower Forces - Service - Wind 90 To Face

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 86.50-66.50	0.12	0.57	A B C	1 1 1	1.2 1.2 1.2	1 1 1	1 1 1	1 1 1	11.042 11.042 11.042	0.22	11.25	C
L2 66.50-36.50	0.39	2.84	A B C	1 1 1	0.59 0.59 0.59	1 1 1	1 1 1	1 1 1	60.000 60.000 60.000	0.60	19.95	C
L3 36.50-1.50	0.41	3.31	A B C	1 1 1	0.59 0.59 0.59	1 1 1	1 1 1	1 1 1	70.000 70.000 70.000	0.60	17.15	C
Sum Weight:	0.93	6.73						OTM	57.30 kip-ft	1.42		

Force Totals

Load Case	Vertical Forces	Sum of Forces X	Sum of Forces Z	Sum of Overturning Moments, M _x	Sum of Overturning Moments, M _z	Sum of Torques
	K	K	K	kip-ft	kip-ft	kip-ft
Leg Weight	6.73					
Bracing Weight	0.00					
Total Member Self-Weight	6.73			0.00	0.00	
Total Weight	8.78			0.00	0.00	
Wind 0 deg - No Ice		0.00	-5.82	-307.52	0.00	0.00
Wind 30 deg - No Ice		2.91	-5.04	-266.32	-153.76	0.00
Wind 45 deg - No Ice		4.11	-4.11	-217.45	-217.45	0.00
Wind 60 deg - No Ice		5.04	-2.91	-153.76	-266.32	0.00
Wind 90 deg - No Ice		5.82	0.00	0.00	-307.52	0.00
Wind 120 deg - No Ice		5.04	2.91	153.76	-266.32	0.00
Wind 135 deg - No Ice		4.11	4.11	217.45	-217.45	0.00
Wind 150 deg - No Ice		2.91	5.04	266.32	-153.76	0.00
Wind 180 deg - No Ice		0.00	5.82	307.52	0.00	0.00
Wind 210 deg - No Ice		-2.91	5.04	266.32	153.76	0.00
Wind 225 deg - No Ice		-4.11	4.11	217.45	217.45	0.00
Wind 240 deg - No Ice		-5.04	2.91	153.76	266.32	0.00
Wind 270 deg - No Ice		-5.82	0.00	0.00	307.52	0.00
Wind 300 deg - No Ice		-5.04	-2.91	-153.76	266.32	0.00
Wind 315 deg - No Ice		-4.11	-4.11	-217.45	217.45	0.00
Wind 330 deg - No Ice		-2.91	-5.04	-266.32	153.76	0.00
Member Ice	1.06					
Total Weight Ice	11.24			0.00	0.00	
Wind 0 deg - Ice		0.00	-5.36	-279.82	0.00	0.00
Wind 30 deg - Ice		2.68	-4.64	-242.33	-139.91	0.00
Wind 45 deg - Ice		3.79	-3.79	-197.86	-197.86	0.00
Wind 60 deg - Ice		4.64	-2.68	-139.91	-242.33	0.00
Wind 90 deg - Ice		5.36	0.00	0.00	-279.82	0.00
Wind 120 deg - Ice		4.64	2.68	139.91	-242.33	0.00
Wind 135 deg - Ice		3.79	3.79	197.86	-197.86	0.00
Wind 150 deg - Ice		2.68	4.64	242.33	-139.91	0.00
Wind 180 deg - Ice		0.00	5.36	279.82	0.00	0.00
Wind 210 deg - Ice		-2.68	4.64	242.33	139.91	0.00
Wind 225 deg - Ice		-3.79	3.79	197.86	197.86	0.00
Wind 240 deg - Ice		-4.64	2.68	139.91	242.33	0.00
Wind 270 deg - Ice		-5.36	0.00	0.00	279.82	0.00
Wind 300 deg - Ice		-4.64	-2.68	-139.91	242.33	0.00
Wind 315 deg - Ice		-3.79	-3.79	-197.86	197.86	0.00
Wind 330 deg - Ice		-2.68	-4.64	-242.33	139.91	0.00

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Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M_x kip-ft	Sum of Overturning Moments, M_z kip-ft	Sum of Torques kip-ft
Total Weight	8.78			0.00	0.00	
Wind 0 deg - Service		0.00	-2.33	-124.59	0.00	0.00
Wind 30 deg - Service		1.17	-2.02	-107.90	-62.29	0.00
Wind 45 deg - Service		1.65	-1.65	-88.10	-88.10	0.00
Wind 60 deg - Service		2.02	-1.17	-62.29	-107.90	0.00
Wind 90 deg - Service		2.33	0.00	0.00	-124.59	0.00
Wind 120 deg - Service		2.02	1.17	62.29	-107.90	0.00
Wind 135 deg - Service		1.65	1.65	88.10	-88.10	0.00
Wind 150 deg - Service		1.17	2.02	107.90	-62.29	0.00
Wind 180 deg - Service		0.00	2.33	124.59	0.00	0.00
Wind 210 deg - Service		-1.17	2.02	107.90	62.29	0.00
Wind 225 deg - Service		-1.65	1.65	88.10	88.10	0.00
Wind 240 deg - Service		-2.02	1.17	62.29	107.90	0.00
Wind 270 deg - Service		-2.33	0.00	0.00	124.59	0.00
Wind 300 deg - Service		-2.02	-1.17	-62.29	107.90	0.00
Wind 315 deg - Service		-1.65	-1.65	-88.10	88.10	0.00
Wind 330 deg - Service		-1.17	-2.02	-107.90	62.29	0.00

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 45 deg - No Ice
5	Dead+Wind 60 deg - No Ice
6	Dead+Wind 90 deg - No Ice
7	Dead+Wind 120 deg - No Ice
8	Dead+Wind 135 deg - No Ice
9	Dead+Wind 150 deg - No Ice
10	Dead+Wind 180 deg - No Ice
11	Dead+Wind 210 deg - No Ice
12	Dead+Wind 225 deg - No Ice
13	Dead+Wind 240 deg - No Ice
14	Dead+Wind 270 deg - No Ice
15	Dead+Wind 300 deg - No Ice
16	Dead+Wind 315 deg - No Ice
17	Dead+Wind 330 deg - No Ice
18	Dead+Ice+Temp
19	Dead+Wind 0 deg+Ice+Temp
20	Dead+Wind 30 deg+Ice+Temp
21	Dead+Wind 45 deg+Ice+Temp
22	Dead+Wind 60 deg+Ice+Temp
23	Dead+Wind 90 deg+Ice+Temp
24	Dead+Wind 120 deg+Ice+Temp
25	Dead+Wind 135 deg+Ice+Temp
26	Dead+Wind 150 deg+Ice+Temp
27	Dead+Wind 180 deg+Ice+Temp
28	Dead+Wind 210 deg+Ice+Temp
29	Dead+Wind 225 deg+Ice+Temp
30	Dead+Wind 240 deg+Ice+Temp
31	Dead+Wind 270 deg+Ice+Temp
32	Dead+Wind 300 deg+Ice+Temp
33	Dead+Wind 315 deg+Ice+Temp
34	Dead+Wind 330 deg+Ice+Temp

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Comb. No.	Description
35	Dead+Wind 0 deg - Service
36	Dead+Wind 30 deg - Service
37	Dead+Wind 45 deg - Service
38	Dead+Wind 60 deg - Service
39	Dead+Wind 90 deg - Service
40	Dead+Wind 120 deg - Service
41	Dead+Wind 135 deg - Service
42	Dead+Wind 150 deg - Service
43	Dead+Wind 180 deg - Service
44	Dead+Wind 210 deg - Service
45	Dead+Wind 225 deg - Service
46	Dead+Wind 240 deg - Service
47	Dead+Wind 270 deg - Service
48	Dead+Wind 300 deg - Service
49	Dead+Wind 315 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	86.5 - 66.5	Pole	Max Tension	19	0.00	0.00	-0.00
			Max. Compression	18	-2.58	0.00	0.00
			Max. Mx	6	-1.75	-25.94	0.00
			Max. My	2	-1.75	0.00	25.94
			Max. Vy	6	2.80	-25.94	0.00
			Max. Vx	2	-2.80	0.00	25.94
			Max. Torque	5			0.00
L2	66.5 - 36.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	18	-6.61	0.00	0.00
			Max. Mx	6	-4.97	-133.73	0.00
			Max. My	2	-4.97	0.00	133.73
			Max. Vy	6	4.38	-133.73	0.00
			Max. Vx	2	-4.38	0.00	133.73
			Max. Torque	5			0.00
L3	36.5 - 1.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	18	-11.24	0.00	0.00
			Max. Mx	6	-8.78	-312.96	0.00
			Max. My	2	-8.78	0.00	312.96
			Max. Vy	6	5.82	-312.96	0.00
			Max. Vx	2	-5.82	0.00	312.96
			Max. Torque	5			0.00

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	19	11.24	0.00	5.36
	Max. H _x	14	8.78	5.82	0.00
	Max. H _z	2	8.78	0.00	5.82
	Max. M _x	2	312.96	0.00	5.82
	Max. M _z	6	312.96	-5.82	0.00
	Max. Torsion	5	0.00	-5.04	2.91

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Min. Vert	1	8.78	0.00	0.00
	Min. H _x	6	8.78	-5.82	0.00
	Min. H _z	10	8.78	0.00	-5.82
	Min. M _x	10	-312.96	0.00	-5.82
	Min. M _z	14	-312.96	5.82	0.00
	Min. Torsion	7	-0.00	-5.04	-2.91

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _y K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	8.78	0.00	0.00	0.00	0.00	0.00
Dead+Wind 0 deg - No Ice	8.78	0.00	-5.82	-312.96	0.00	0.00
Dead+Wind 30 deg - No Ice	8.78	2.91	-5.04	-271.03	-156.48	0.00
Dead+Wind 45 deg - No Ice	8.78	4.11	-4.11	-221.29	-221.29	0.00
Dead+Wind 60 deg - No Ice	8.78	5.04	-2.91	-156.48	-271.03	-0.00
Dead+Wind 90 deg - No Ice	8.78	5.82	0.00	0.00	-312.96	0.00
Dead+Wind 120 deg - No Ice	8.78	5.04	2.91	156.48	-271.03	0.00
Dead+Wind 135 deg - No Ice	8.78	4.11	4.11	221.29	-221.29	0.00
Dead+Wind 150 deg - No Ice	8.78	2.91	5.04	271.03	-156.48	-0.00
Dead+Wind 180 deg - No Ice	8.78	0.00	5.82	312.96	0.00	0.00
Dead+Wind 210 deg - No Ice	8.78	-2.91	5.04	271.03	156.48	0.00
Dead+Wind 225 deg - No Ice	8.78	-4.11	4.11	221.29	221.29	0.00
Dead+Wind 240 deg - No Ice	8.78	-5.04	2.91	156.48	271.03	-0.00
Dead+Wind 270 deg - No Ice	8.78	-5.82	0.00	0.00	312.96	0.00
Dead+Wind 300 deg - No Ice	8.78	-5.04	-2.91	-156.48	271.03	0.00
Dead+Wind 315 deg - No Ice	8.78	-4.11	-4.11	-221.29	221.29	0.00
Dead+Wind 330 deg - No Ice	8.78	-2.91	-5.04	-271.03	156.48	-0.00
Dead+Ice+Temp	11.24	0.00	0.00	0.00	0.00	0.00
Dead+Wind 0 deg+Ice+Temp	11.24	0.00	-5.36	-286.38	0.00	0.00
Dead+Wind 30 deg+Ice+Temp	11.24	2.68	-4.64	-248.02	-143.19	0.00
Dead+Wind 45 deg+Ice+Temp	11.24	3.79	-3.79	-202.50	-202.50	0.00
Dead+Wind 60 deg+Ice+Temp	11.24	4.64	-2.68	-143.19	-248.02	-0.00
Dead+Wind 90 deg+Ice+Temp	11.24	5.36	0.00	0.00	-286.38	0.00
Dead+Wind 120 deg+Ice+Temp	11.24	4.64	2.68	143.19	-248.02	0.00
Dead+Wind 135 deg+Ice+Temp	11.24	3.79	3.79	202.50	-202.50	0.00
Dead+Wind 150 deg+Ice+Temp	11.24	2.68	4.64	248.02	-143.19	-0.00
Dead+Wind 180 deg+Ice+Temp	11.24	0.00	5.36	286.38	0.00	0.00
Dead+Wind 210 deg+Ice+Temp	11.24	-2.68	4.64	248.02	143.19	0.00
Dead+Wind 225 deg+Ice+Temp	11.24	-3.79	3.79	202.50	202.50	0.00
Dead+Wind 240 deg+Ice+Temp	11.24	-4.64	2.68	143.19	248.02	-0.00
Dead+Wind 270 deg+Ice+Temp	11.24	-5.36	0.00	0.00	286.38	0.00
Dead+Wind 300 deg+Ice+Temp	11.24	-4.64	-2.68	-143.19	248.02	0.00
Dead+Wind 315 deg+Ice+Temp	11.24	-3.79	-3.79	-202.50	202.50	0.00
Dead+Wind 330 deg+Ice+Temp	11.24	-2.68	-4.64	-248.02	143.19	-0.00
Dead+Wind 0 deg - Service	8.78	0.00	-2.33	-126.82	0.00	0.00
Dead+Wind 30 deg - Service	8.78	1.17	-2.02	-109.83	-63.41	0.00
Dead+Wind 45 deg - Service	8.78	1.65	-1.65	-89.68	-89.68	0.00
Dead+Wind 60 deg - Service	8.78	2.02	-1.17	-63.41	-109.83	-0.00
Dead+Wind 90 deg - Service	8.78	2.33	0.00	0.00	-126.82	0.00
Dead+Wind 120 deg - Service	8.78	2.02	1.17	63.41	-109.83	0.00
Dead+Wind 135 deg - Service	8.78	1.65	1.65	89.68	-89.68	0.00
Dead+Wind 150 deg - Service	8.78	1.17	2.02	109.83	-63.41	-0.00
Dead+Wind 180 deg - Service	8.78	0.00	2.33	126.82	0.00	0.00
Dead+Wind 210 deg - Service	8.78	-1.17	2.02	109.83	63.41	0.00
Dead+Wind 225 deg - Service	8.78	-1.65	1.65	89.68	89.68	0.00

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Load Combination	Vertical	Shear _x	Shear _y	Overturning Moment, M _x	Overturning Moment, M _y	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead+Wind 240 deg - Service	8.78	-2.02	1.17	63.41	109.83	-0.00
Dead+Wind 270 deg - Service	8.78	-2.33	0.00	0.00	126.82	0.00
Dead+Wind 300 deg - Service	8.78	-2.02	-1.17	-63.41	109.83	0.00
Dead+Wind 315 deg - Service	8.78	-1.65	-1.65	-89.68	89.68	0.00
Dead+Wind 330 deg - Service	8.78	-1.17	-2.02	-109.83	63.41	-0.00

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-8.78	0.00	0.00	8.78	0.00	0.000%
2	0.00	-8.78	-5.82	0.00	8.78	5.82	0.000%
3	2.91	-8.78	-5.04	-2.91	8.78	5.04	0.000%
4	4.11	-8.78	-4.11	-4.11	8.78	4.11	0.000%
5	5.04	-8.78	-2.91	-5.04	8.78	2.91	0.000%
6	5.82	-8.78	0.00	-5.82	8.78	0.00	0.000%
7	5.04	-8.78	2.91	-5.04	8.78	-2.91	0.000%
8	4.11	-8.78	4.11	-4.11	8.78	-4.11	0.000%
9	2.91	-8.78	5.04	-2.91	8.78	-5.04	0.000%
10	0.00	-8.78	5.82	0.00	8.78	-5.82	0.000%
11	-2.91	-8.78	5.04	2.91	8.78	-5.04	0.000%
12	-4.11	-8.78	4.11	4.11	8.78	-4.11	0.000%
13	-5.04	-8.78	2.91	5.04	8.78	-2.91	0.000%
14	-5.82	-8.78	0.00	5.82	8.78	0.00	0.000%
15	-5.04	-8.78	-2.91	5.04	8.78	2.91	0.000%
16	-4.11	-8.78	-4.11	4.11	8.78	4.11	0.000%
17	-2.91	-8.78	-5.04	2.91	8.78	5.04	0.000%
18	0.00	-11.24	0.00	0.00	11.24	0.00	0.000%
19	0.00	-11.24	-5.36	0.00	11.24	5.36	0.000%
20	2.68	-11.24	-4.64	-2.68	11.24	4.64	0.000%
21	3.79	-11.24	-3.79	-3.79	11.24	3.79	0.000%
22	4.64	-11.24	-2.68	-4.64	11.24	2.68	0.000%
23	5.36	-11.24	0.00	-5.36	11.24	0.00	0.000%
24	4.64	-11.24	2.68	-4.64	11.24	-2.68	0.000%
25	3.79	-11.24	3.79	-3.79	11.24	-3.79	0.000%
26	2.68	-11.24	4.64	-2.68	11.24	-4.64	0.000%
27	0.00	-11.24	5.36	0.00	11.24	-5.36	0.000%
28	-2.68	-11.24	4.64	2.68	11.24	-4.64	0.000%
29	-3.79	-11.24	3.79	3.79	11.24	-3.79	0.000%
30	-4.64	-11.24	2.68	4.64	11.24	-2.68	0.000%
31	-5.36	-11.24	0.00	5.36	11.24	0.00	0.000%
32	-4.64	-11.24	-2.68	4.64	11.24	2.68	0.000%
33	-3.79	-11.24	-3.79	3.79	11.24	3.79	0.000%
34	-2.68	-11.24	-4.64	2.68	11.24	4.64	0.000%
35	0.00	-8.78	-2.33	0.00	8.78	2.33	0.000%
36	1.17	-8.78	-2.02	-1.17	8.78	2.02	0.000%
37	1.65	-8.78	-1.65	-1.65	8.78	1.65	0.000%
38	2.02	-8.78	-1.17	-2.02	8.78	1.17	0.000%
39	2.33	-8.78	0.00	-2.33	8.78	0.00	0.000%
40	2.02	-8.78	1.17	-2.02	8.78	-1.17	0.000%
41	1.65	-8.78	1.65	-1.65	8.78	-1.65	0.000%
42	1.17	-8.78	2.02	-1.17	8.78	-2.02	0.000%
43	0.00	-8.78	2.33	0.00	8.78	-2.33	0.000%
44	-1.17	-8.78	2.02	1.17	8.78	-2.02	0.000%
45	-1.65	-8.78	1.65	1.65	8.78	-1.65	0.000%
46	-2.02	-8.78	1.17	2.02	8.78	-1.17	0.000%
47	-2.33	-8.78	0.00	2.33	8.78	0.00	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
48	-2.02	-8.78	-1.17	2.02	8.78	1.17	0.000%
49	-1.65	-8.78	-1.65	1.65	8.78	1.65	0.000%
50	-1.17	-8.78	-2.02	1.17	8.78	2.02	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.0000001	0.0000001
2	Yes	4	0.0000001	0.0000001
3	Yes	4	0.0000001	0.00049960
4	Yes	4	0.0000001	0.00057402
5	Yes	4	0.0000001	0.00049960
6	Yes	4	0.0000001	0.0000001
7	Yes	4	0.0000001	0.00049960
8	Yes	4	0.0000001	0.00057402
9	Yes	4	0.0000001	0.00049960
10	Yes	4	0.0000001	0.0000001
11	Yes	4	0.0000001	0.00049960
12	Yes	4	0.0000001	0.00057402
13	Yes	4	0.0000001	0.00049960
14	Yes	4	0.0000001	0.0000001
15	Yes	4	0.0000001	0.00049960
16	Yes	4	0.0000001	0.00057402
17	Yes	4	0.0000001	0.00049960
18	Yes	4	0.0000001	0.0000001
19	Yes	4	0.0000001	0.00093381
20	Yes	5	0.0000001	0.00003883
21	Yes	5	0.0000001	0.00004228
22	Yes	5	0.0000001	0.00003883
23	Yes	4	0.0000001	0.00093381
24	Yes	5	0.0000001	0.00003883
25	Yes	5	0.0000001	0.00004228
26	Yes	5	0.0000001	0.00003883
27	Yes	4	0.0000001	0.00093381
28	Yes	5	0.0000001	0.00003883
29	Yes	5	0.0000001	0.00004228
30	Yes	5	0.0000001	0.00003883
31	Yes	4	0.0000001	0.00093381
32	Yes	5	0.0000001	0.00003883
33	Yes	5	0.0000001	0.00004228
34	Yes	5	0.0000001	0.00003883
35	Yes	4	0.0000001	0.0000001
36	Yes	4	0.0000001	0.00004465
37	Yes	4	0.0000001	0.00005149
38	Yes	4	0.0000001	0.00004465
39	Yes	4	0.0000001	0.0000001
40	Yes	4	0.0000001	0.00004465
41	Yes	4	0.0000001	0.00005149
42	Yes	4	0.0000001	0.00004465
43	Yes	4	0.0000001	0.0000001
44	Yes	4	0.0000001	0.00004465
45	Yes	4	0.0000001	0.00005149
46	Yes	4	0.0000001	0.00004465
47	Yes	4	0.0000001	0.0000001
48	Yes	4	0.0000001	0.00004465

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49	Yes	4	0.00000001	0.00005149
50	Yes	4	0.00000001	0.00004465

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	86.5 - 66.5	9.239	35	1.0434	0.0000
L2	66.5 - 36.5	5.302	35	0.5938	0.0000
L3	36.5 - 1.5	1.901	35	0.4569	0.0000

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
82.00	AM-X-CD-16-65-00T-RET(72")	35	8.289	0.9240	0.0000	10676
78.00	DTMABP7819VG12A TMA	35	7.461	0.8226	0.0000	6280
72.00	SLCP 2x6014	35	6.282	0.6881	0.0000	3681

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	86.5 - 66.5	22.610	2	2.5342	0.0000
L2	66.5 - 36.5	13.033	2	1.4569	0.0000
L3	36.5 - 1.5	4.679	2	1.1235	0.0000

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
82.00	AM-X-CD-16-65-00T-RET(72")	2	20.302	2.2481	0.0000	4470
78.00	DTMABP7819VG12A TMA	2	18.290	2.0051	0.0000	2629
72.00	SLCP 2x6014	2	15.420	1.6828	0.0000	1541

Compression Checks

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Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio P P _a
L1	86.5 - 66.5 (1)	P6x.432	20.00	0.00	0.0	21.000	8.4049	-1.75	176.50	0.010
L2	66.5 - 36.5 (2)	P24x3/8	30.00	0.00	0.0	21.000	27.8325	-4.97	584.48	0.009
L3	36.5 - 1.5 (3)	P24x3/8	35.00	0.00	0.0	21.000	27.8325	-8.78	584.48	0.015

Pole Bending Design Data

Section No.	Elevation ft	Size	Actual M _x kip-ft	Actual f _{bx} ksi	Allow. F _{bx} ksi	Ratio f _{bx} F _{bx}	Actual M _y kip-ft	Actual f _{by} ksi	Allow. F _{by} ksi	Ratio f _{by} F _{by}
L1	86.5 - 66.5 (1)	P6x.432	25.94	25.467	23.100	1.102	0.00	0.000	23.100	0.000
L2	66.5 - 36.5 (2)	P24x3/8	133.73	9.915	23.100	0.429	0.00	0.000	23.100	0.000
L3	36.5 - 1.5 (3)	P24x3/8	312.96	23.202	23.100	1.004	0.00	0.000	23.100	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V K	Actual f _v ksi	Allow. F _v ksi	Ratio f _v F _v	Actual T kip-ft	Actual f _{vt} ksi	Allow. F _{vt} ksi	Ratio f _{vt} F _{vt}
L1	86.5 - 66.5 (1)	P6x.432	2.80	0.666	14.000	0.048	0.00	0.000	14.000	0.000
L2	66.5 - 36.5 (2)	P24x3/8	4.38	0.315	14.000	0.022	0.00	0.000	14.000	0.000
L3	36.5 - 1.5 (3)	P24x3/8	5.82	0.419	14.000	0.030	0.00	0.000	14.000	0.000

Pole Interaction Design Data

Section No.	Elevation ft	Ratio P P _a	Ratio f _{bx} F _{bx}	Ratio f _{by} F _{by}	Ratio f _v F _v	Ratio f _{vt} F _{vt}	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	86.5 - 66.5 (1)	0.010	1.102	0.000	0.048	0.000	1.115	1.333	H1-3+VT ✓
L2	66.5 - 36.5 (2)	0.009	0.429	0.000	0.022	0.000	0.438	1.333	H1-3+VT ✓
L3	36.5 - 1.5 (3)	0.015	1.004	0.000	0.030	0.000	1.020	1.333	H1-3+VT ✓

Section Capacity Table

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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail	
L1	86.5 - 66.5	Pole	P6x.432	1	-1.75	235.28	83.6	Pass	
L2	66.5 - 36.5	Pole	P24x3/8	2	-4.97	779.12	32.9	Pass	
L3	36.5 - 1.5	Pole	P24x3/8	3	-8.78	779.12	76.5	Pass	
							Summary		
							Pole (L1)	83.6	Pass
							RATING =	83.6	Pass

Subject:

Anchor Bolt and Base Plate Analysis

Location:

85-ft Temporary Ballast Monopole
 West Hartford, CT

Rev. 1: 4/29/14

Prepared by: T.J.L. Checked by: C.F.C.
 Job No. 14071.000

Anchor Bolt and Base Plate Analysis:

Input Data:

Tower Reactions:

Overturning Moment = OM := 313-ft-kips (Input From tnxTower)
 Shear Force = Shear := 6-kips (Input From tnxTower)
 Axial Force = Axial := 9-kips (Input From tnxTower)

Anchor Bolt Data:

Use ASTM A325

Number of Anchor Bolts = N := 20 (User Input)
 Diameter of Bolt Circle = D_{bc} := 27.00-in (User Input)
 Bolt "Column" Distance = l := 3.0-in (User Input)
 Bolt Ultimate Strength = F_u := 120-ksi (User Input)
 Bolt Yield Strength = F_y := 92-ksi (User Input)
 Bolt Modulus = E := 29000-ksi (User Input)
 Diameter of Anchor Bolts = D := 1.0-in (User Input)
 Threads per Inch = n := 8 (User Input)

Base Plate Data:

Use ASTM A572-50

Plate Yield Strength = F_{ybp} := 50-ksi (User Input)
 Base Plate Thickness = t_{bp} := 1.00-in (User Input)
 Base Plate Diameter = D_{bp} := 30-in (User Input)
 Outer Pole Diameter = D_{pole} := 24-in (User Input)

Geometric Layout Data:

Distance from Bolts to Centroid of Pole:

Radius of Bolt Circle =: $R_{bc} := \frac{D_{bc}}{2} = 13.5 \text{ in}$

Distance to Bolts = $i := 1..N$

$$d_i := \begin{cases} \theta \leftarrow 2\pi \cdot \left(\frac{i}{N}\right) \\ d \leftarrow R_{bc} \cdot \sin(\theta) \end{cases}$$

$d_1 = 4.17 \text{ in}$	$d_7 = 10.92 \text{ in}$
$d_2 = 7.94 \text{ in}$	$d_8 = 7.94 \text{ in}$
$d_3 = 10.92 \text{ in}$	$d_9 = 4.17 \text{ in}$
$d_4 = 12.84 \text{ in}$	$d_{10} = 0.00 \text{ in}$
$d_5 = 13.50 \text{ in}$	$d_{11} = -4.17 \text{ in}$
$d_6 = 12.84 \text{ in}$	etc.

Critical Distances For Bending in Plate:

Outer Pole Radius = $R_{pole} := \frac{D_{pole}}{2} = 12 \text{ in}$

Moment Arms of Bolts about Neutral Axis = $MA_i := \text{if}(d_i \geq R_{pole}, d_i - R_{pole}, 0 \text{ in})$

$MA_1 = 0.00 \text{ in}$	$MA_7 = 0.00 \text{ in}$
$MA_2 = 0.00 \text{ in}$	$MA_8 = 0.00 \text{ in}$
$MA_3 = 0.00 \text{ in}$	$MA_9 = 0.00 \text{ in}$
$MA_4 = 0.84 \text{ in}$	$MA_{10} = 0.00 \text{ in}$
$MA_5 = 1.50 \text{ in}$	$MA_{11} = 0.00 \text{ in}$
$MA_6 = 0.84 \text{ in}$	etc

Effective Width of Baseplate for Bending = $B_{eff} := .8 \cdot 2 \cdot \sqrt{\left(\frac{D_{bp}}{2}\right)^2 - \left(\frac{D_{pole}}{2}\right)^2} = 14.4 \text{ in}$

Anchor Bolt Analysis:

Calculated Anchor Bolt Properties:

Polar Moment of Inertia = $I_p := \sum_i (d_i)^2 = 1.822 \times 10^3 \cdot \text{in}^2$

Gross Area of Bolt = $A_g := \frac{\pi}{4} \cdot D^2 = 0.785 \cdot \text{in}^2$

Net Area of Bolt = $A_n := \frac{\pi}{4} \cdot \left(D - \frac{0.9743 \cdot \text{in}}{n} \right)^2 = 0.606 \cdot \text{in}^2$

Net Diameter = $D_n := \frac{2 \cdot \sqrt{A_n}}{\sqrt{\pi}} = 0.878 \cdot \text{in}$

Radius of Gyration of Bolt = $r := \frac{D_n}{4} = 0.22 \cdot \text{in}$

Section Modulus of Bolt = $S_x := \frac{\pi \cdot D_n^3}{32} = 0.066 \cdot \text{in}^3$

Check Anchor Bolt Tension Force:

Maximum Tensile Force = $T_{\text{Max}} := OM \cdot \frac{R_{bc}}{I_p} - \frac{\text{Axial}}{N} = 27.4 \cdot \text{kips}$

Allowable Tensile Force = $T_{\text{ALL.Gross}} := 1.333 \cdot (0.33 \cdot A_g \cdot F_u) = 41.5 \cdot \text{kips}$ (1.333 increase allowed per TIA/EIA)

$T_{\text{ALL.Net}} := 1.333 \cdot (0.60 \cdot A_n \cdot F_y) = 44.572 \cdot \text{kips}$ (1.333 increase allowed per TIA/EIA)

Bolt Tension % of Capacity = $\frac{T_{\text{Max}}}{T_{\text{ALL.Net}}} \cdot 100 = 61$ Bolts are "upset bolts". Use net area per AISC

Condition1 = $\text{if} \left(\frac{T_{\text{Max}}}{T_{\text{ALL.Net}}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$

Condition1 = "OK"

Check Anchor Bolt Bending Stress:

Maximum Bending Moment = $M_x := \left(\frac{\text{Shear}}{N} \right) \cdot l = 0.075 \cdot \text{ft} \cdot \text{kips}$

Maximum Bending Stress = $f_{bx} := \frac{M_x}{S_x} = 13.5 \cdot \text{ksi}$

Allowable Bending Stress = $F_{bx} := 1.333 \cdot 0.6 \cdot F_y = 73.6 \cdot \text{ksi}$ (1.333 increase allowed per TIA/EIA)

Check Combined Stress Requirement:

Per ASCE Manual 72: "If the clearance between the base plate and concrete does not exceed two times the bolt diameter a bending stress analysis of the bolts is NOT normally required."

$$l := \begin{cases} l & \text{if } l > 2 \cdot D_n = 3 \text{ in} \\ 0 & \text{otherwise} \end{cases}$$

$$f_{bx} := \begin{cases} f_{bx} & \text{if } l > 2 \cdot D_n = 13.5 \text{ ksi} \\ 0 & \text{otherwise} \end{cases}$$

Check Anchor Bolt Compression/Combined Stress:

Maximum Compressive Force =

$$C_{Max} := OM \cdot \frac{R_{bc}}{I_p} + \frac{Axial}{N} = 28.3 \text{ kips}$$

Maximum Compressive Stress =

$$f_a := \frac{C_{Max}}{A_n} = 46.7 \text{ ksi}$$

$$K := 0.65$$

$$C_c := \sqrt{\frac{2 \cdot \pi^2 \cdot E}{F_y}} = 78.881$$

$$F_a := \begin{cases} \frac{\left[1 - \frac{\left(\frac{K \cdot l}{r} \right)^2}{2 \cdot C_c^2} \right] \cdot F_y}{\frac{5}{3} + \frac{3 \cdot \left(\frac{K \cdot l}{r} \right)}{8 \cdot C_c} - \frac{\left(\frac{K \cdot l}{r} \right)^3}{8 \cdot C_c^3}} \cdot F_y & \text{if } \frac{K \cdot l}{r} \leq C_c = 53.5 \text{ ksi} \\ \frac{12 \cdot \pi^2 \cdot E}{23 \cdot \left(\frac{K \cdot l}{r} \right)^2} & \text{if } \frac{K \cdot l}{r} > C_c \end{cases}$$

Allowable Compressive Stress =

$$F_a := 1.333 \cdot F_a = 71.3 \text{ ksi} \quad (1.333 \text{ increase allowed per TIA/EIA})$$

Combined Stress % of Capacity =

$$\left(\frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} \right) \cdot 100 = 83.8$$

Condition 2 =

$$\text{Condition2} := \left(\frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition2 = "OK"

Base Plate Analysis:

Force from Bolts =

$$C_i := \frac{OM \cdot d_i}{I_p} + \frac{Axial}{N}$$

$C_1 = 9.0$ -kips

$C_7 = 23.0$ -kips

$C_2 = 16.8$ -kips

$C_8 = 16.8$ -kips

$C_3 = 23.0$ -kips

$C_9 = 9.0$ -kips

$C_4 = 26.9$ -kips

$C_{10} = 0.5$ -kips

$C_5 = 28.3$ -kips

$C_{11} = -8.1$ -kips

$C_6 = 26.9$ -kips

etc.

Maximum Bending Stress in Plate =

$$f_{bp} := \sum_i \frac{6 \cdot C_i \cdot MA_i}{(B_{eff} \cdot t_{bp}^2)} = 36.5 \text{ ksi}$$

Allowable Bending Stress in Plate =

$F_{bp} := 1.33 \cdot 0.75 \cdot F_{ybp} = 49.9 \text{ ksi}$

Plate Bending Stress % of Capacity =

$\frac{f_{bp}}{F_{bp}} \cdot 100 = 73.2$

Condition3 =

Condition3 := $\text{if} \left(\frac{f_{bp}}{F_{bp}} < 1.00, \text{"Ok"}, \text{"Overstressed"} \right)$

Condition3 = "Ok"

Ballast Calculation:

Input Data:

Tower Reactions:

Overturning Moment =	OM := 313-ft-kips	(Input From tnxTower)
Shear Force =	Shear := 6-kips	(Input From tnxTower)
Axial Force =	Axial := 9-kips	(Input From tnxTower)
Overturning Factor of Safety Required =	FS _{req} := 2.0	(User Input)

Ballast Data:

Weight of Base Frame =	WT _{frame} := 3kips	(User Input)
Distance From Center of Monopole to Extreme Edge of Base Frame =	c := 7.5-ft	(User Input)
Distance From Frame Toe to Front Ballast String =	d ₁ := 3.0-ft	(User Input)
Distance From Frame Toe to Back Ballast String =	d ₂ := 12-ft	(User Input)
Depth of Ballast Frame =	d _f := 1.50-ft	(User Input)
Weight of Concrete =	γ _C := 145-pcf	(User Input)
Block Width =	w _b := 6-ft	(User Input)
Block Length =	l _b := 6-ft	(User Input)
Block Thickness =	t _b := 1-ft	(User Input)
Individual Block Weight =	W _b := w _b · l _b · t _b · γ _C = 5220-lb	
Total Overturning Moment About Toe of Base Frame =	M _{tot} := OM + Shear · d _f = 322-ft-kips	
Total Dead Weight =	P _{tot} := Axial + WT _{frame} = 12-kips	
Net Resisting Moment Required =	M _{rnet} := (FS _{req} · M _{tot}) - (P _{tot} · c) = 554-kip-ft	
Ballast Required Per Side of Base Frame =	W _{req} := $\frac{M_{rnet}}{(d_1 + d_2)}$ = 36.93-kips	
Total Ballast Weight Required =	W _{totreq} := W _{req} · 2 = 73.87-kips	
Ballast Weight Required Per Quadrant =	W _{reqquad} := $\frac{W_{req}}{2}$ = 18.47-kips	
Number of Blocks Required per Quadrant =	N _b := $\frac{W_{totreq}}{W_b \cdot (4) \cdot (0.9)}$ = 3.93	
	N _{buse} := 4	

Subject:

Ballast Calculation

Location:

85-ft Temporary Ballast Monopole
West Hartford, CT

Rev. 1: 4/29/14

Prepared by: T.J.L. Checked by: C.F.C.
Job No. 14071.000

Resisting Moment =

$$M_r := [(W_b) \cdot N_{buse} \cdot 2.0 \cdot (d_1 + d_2)] + (P_{tot} \cdot c) = 716.4 \text{ kip}\cdot\text{ft}$$

Actual Factor of Safety =

$$FS := \frac{M_r}{M_{tot}} = 2.22$$

$$FOS_Check := \text{if}(FS \geq FS_{req}, \text{"Okay"}, \text{"No Good"})$$

$$FOS_Check = \text{"Okay"}$$

Product Specifications

COMMScope®

POWERED BY



DBXNH-6565A-VTM

Andrew® Dual Band Teletilt® Antenna, 698–896 MHz and 1710–2180 MHz, 65° horizontal beamwidth, RET compatible

- Interleaved dipole technology providing for attractive, low wind load mechanical package
- The RF connectors are designed for IP67 rating and the radome for IP56 rating
- The values presented on this datasheet have been calculated based on N-P-BASTA White Paper version 9.6 by the NGMN Alliance

Electrical Specifications

Frequency Band, MHz	698–806	806–896	1710–1880	1850–1990	1920–2180
Gain by all Beam Tilts, average, dBi	13.3	13.3	16.8	17.0	17.0
Gain by all Beam Tilts Tolerance, dB	±0.7	±0.4	±0.5	±0.3	±0.7
	0 ° 13.5	0 ° 14.1	0 ° 16.7	0 ° 17.1	0 ° 17.3
Gain by Beam Tilt, average, dBi	8 ° 13.4	8 ° 14.0	4 ° 16.8	4 ° 17.1	4 ° 17.1
	15 ° 12.9	15 ° 13.4	8 ° 16.7	8 ° 16.8	6 ° 16.5
Beamwidth, Horizontal, degrees	69	66	62	57	59
Beamwidth, Horizontal Tolerance, degrees	±2.9	±1.7	±5.7	±2.1	±6.4
Beamwidth, Vertical, degrees	18.8	16.9	7.4	7.0	6.6
Beamwidth, Vertical Tolerance, degrees	±1.5	±0.8	±0.4	±0.3	±0.4
Beam Tilt, degrees	0–15	0–15	0–8	0–8	0–8
USLS, dB	17	15	16	17	18
Front-to-Back Total Power at 180° ± 30°, dB	18	21	28	28	28
CPR at Boresight, dB	15	14	21	22	25
CPR at Sector, dB	5	5	11	9	7
Isolation, dB	30	28	30	30	30
Isolation, Intersystem, dB	35	33	40	40	40
VSWR Return Loss, dB	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0
PIM, 3rd Order, 2 x 20 W, dBc	-153	-153	-153	-153	-153
Input Power per Port, maximum, watts	400	400	300	300	300
Polarization	±45°	±45°	±45°	±45°	±45°
Impedance	50 ohm	50 ohm	50 ohm	50 ohm	50 ohm

General Specifications

Antenna Brand	Andrew®
Antenna Type	DualPol® dual band
Band	Multiband
Brand	DualPol® Teletilt®
Operating Frequency Band	1710 – 2180 MHz 698 – 896 MHz
Number of Ports, all types	4

Mechanical Specifications

Color	Light gray
Lightning Protection	dc Ground
Radiator Material	Aluminum
Radome Material	Fiberglass, UV resistant
RF Connector Interface	7-16 DIN Female
RF Connector Location	Bottom
RF Connector Quantity, total	4

Product Specifications

COMMSCOPE®

DBXNH-6565A-VTM

POWERED BY



Wind Loading, maximum	403.0 N @ 150 km/h 90.6 lbf @ 150 km/h
Wind Speed, maximum	241.0 km/h 149.8 mph

Dimensions

Depth	181.0 mm 7.1 in
Length	1293.0 mm 50.9 in
Width	301.0 mm 11.9 in
Net Weight	15.5 kg 34.2 lb

Remote Electrical Tilt (RET) Information

Model with Factory Installed AISG 1.1 Actuator	DBXNH-6565A-R2M
Model with Factory Installed AISG 2.0 Actuator	DBXNH-6565A-A2M
RET System	Teletilt®

Regulatory Compliance/Certifications

Agency

RoHS 2011/65/EU
China RoHS SJ/T 11364-2006
ISO 9001:2008

Classification

Compliant by Exemption
Above Maximum Concentration Value (MCV)
Designed, manufactured and/or distributed under this quality management system



Included Products

DB380 — Pipe Mounting Kit for 2.4"-4.5" (60-115mm) OD round members on wide panel antennas. Includes 2 clamp sets and double nuts.

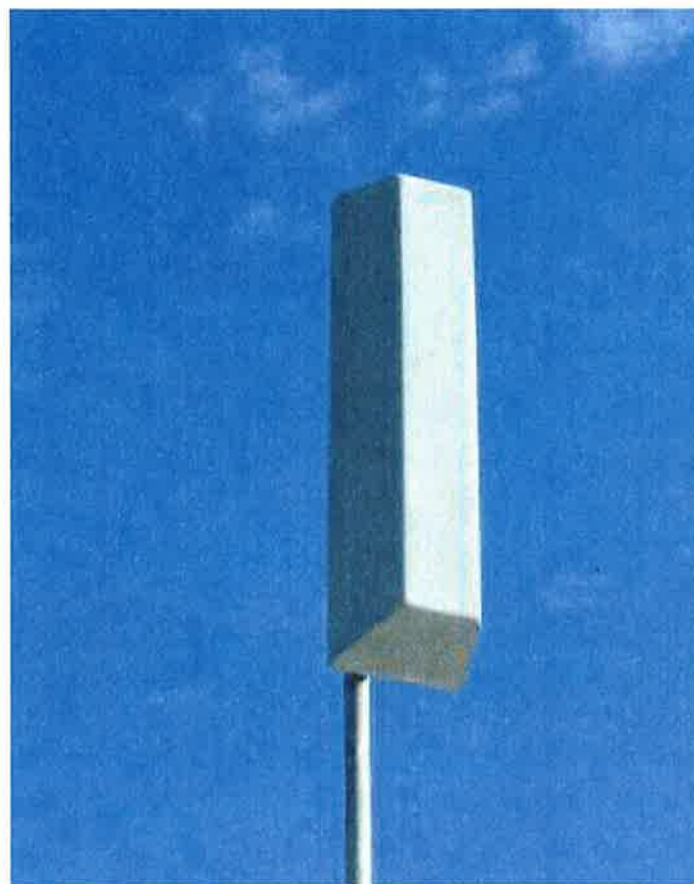
DB5083 — Downtilt Mounting Kit for 2.4"-4.5" (60 - 115 mm) OD round members. Includes a heavy-duty, galvanized steel downtilt mounting bracket assembly and associated hardware. This kit is compatible with the DB380 pipe mount kit for panel antennas that are equipped with two mounting brackets.

SLCP 2x6014

Dual (2x) Circularly Polarized log-periodic antenna

Features

- Transmit Diversity Gain
- Can be configured to combine space & polarization diversity
- Outstanding performance over the entire band (700 - 800 MHz)
- Excellent Axial Ratio
- Optimized for 4G & 3G systems
- Low intermodulation
- Improved Side-to-side rejection
- Fading reduction
- Excellent isolation between ports



Electrical specifications

Frequency range:	700-800 MHz
Impedance:	50 ohm
Connector type:	7/16 Din
Return loss:	18 dB
Polarization:	Circular
Gain ea. port [Circular]:	2x14 dBdC
Gain ea. port [Linear]:	2x11 dBdL
Axial Ratio:	2 dB
Isolation between ports (TX band):	30 dB
Front-to-back ratio:	30 dB
Intermodulation (2x20W):	IM3 150 dB
	IM5 160 dB
	IM7/9 170 dB
Power rating:	2x 500 W
H-plane (-3 dB point):	2x 55°
V-plane (-3 dB point):	2x 16°
Lightning protection:	DC grounded

Mechanical specifications

Overall height:	53 in	[1346 mm]
Width:	14 in	[356 mm]
Depth:	11 in	[279 mm]
Weight (excluding brackets):	20 lbs	[9 Kg]
Wind load measured up to:	150 mph	[240 Km/h]
Wind area (side of antenna):	5.15 sq. ft.	[0.48 sq.m]
Lateral thrust at 113 mph/ 180 Km/h (worst case):	263 lbs	[1171 N]

Materials

Radiating Elements:	Aluminum
Transformer (Power distribution)	Ceramic PCB
Chassis:	Aluminum
Radome:	Grey Fiberglass/PVC
Mounting bolts:	Stainless steel

The SLCP 2x6014 is made in the U.S.A.

Section 15A - CURRENT SECTOR/CELL INFORMATION - ALPHA (MCM)								
ANTENNA CONFIG (FROM BACK)	ANTENNA 1 GSM, UMTS (850 / 1900) or LTE (700 / AWS)		ANTENNA 2 GSM, UMTS (850 / 1900) or LTE (700 / AWS)		ANTENNA 3 GSM, UMTS (850 / 1900) or LTE (700 / AWS)		ANTENNA 4 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	
	TBD	TBD	TBD	TBD	TBD	TBD	TBD	TBD
TECHNOLOGY	UMTS / 850 - GSM / 8	UMTS / 1900					UMTS / 850 - GSM / 8	UMTS / 1900
MHN LOCATION (Top/Bottom/None)	N/A	N/A					N/A	N/A
FEEDER TYPE	1 5/8" - Andrew	1 5/8" - Andrew					1 5/8" - Andrew	1 5/8" - Andrew
Feeder Length (feet)	82"	82"					82"	82"
ANTENNA ATOL	N/A	N/A					N/A	N/A
ANTENNA MAKE - MODEL	BXA-80090-4CF						BXA-80090-4CF	
ANTENNA VENDOR	Antel International Inc.						Antel International Inc.	
ANTENNA SIZE (H x W x D)	TBD						TBD	
ANTENNA WEIGHT	1						1	
Mechanical Gain								
AZIMUTH	20°						33°	
RAZORION CENTER (feet)	62"						62"	
ANTENNA TIP HEIGHT	62"						62"	
ELECTRICAL TILT (150/850/1900 AWS)	N/A						N/A	
MECHANICAL DOWNTILT	N/A						N/A	
FEEDER AMOUNT	1						1	
Antenna RET Motor (QTY/MODEL)	N/A						N/A	
Antenna RET Spitzer (QTY/MODEL)	N/A						N/A	
Antenna RET Earth (Grounding) Clamp (QTY/MODEL)	N/A						N/A	
Antenna RET Surge Arrestor (QTY/MODEL)	N/A						N/A	
Antenna RET CONTROL UNIT (QTY/MODEL) - usually per site	N/A						N/A	
DC BLOCK (QTY/MODEL)	N/A						N/A	
TMALINA (TYP/MODEL)	N/A						N/A	
CURRENT INJECTORS FOR TMA (QTY/MODEL)	Polyphase / 1000860						Polyphase / 1000860	
PDU FOR TMA (QTY/MODEL) - usually per site	LGP 12104 (1900 AND 850 Bypass TMA)						LGP 12104 (1900 AND 850 Bypass TMA)	
SURGE ARRESTOR (QTY/MODEL)	N/A						N/A	
SPLITTER (QTY/MODEL)	1 + N/A / Powerwave / LGP 13519						1 + N/A / Powerwave / LGP 13519	
HYBRID COMBINER (QTY/MODEL)	N/A						N/A	
DUPLEXER (QTY/MODEL)	N/A						N/A	
FILTER (QTY/MODEL)	N/A						N/A	
RAAT KIT MODULE	N/A						N/A	
DUPLEXER or NARROW BAND LDC (QTY/MODEL)	N/A						N/A	
SCRAMBLER MODULE	N/A						N/A	
Additional Component 1	N/A						N/A	
Additional Component 2	N/A						N/A	
Additional Component 3	N/A						N/A	
MAGNETIC DECLINATION	-14°						-14°	
MATCHPLATE POWER (Watts)	N/A						N/A	
ERP (Watts)	N/A						N/A	
Local Market Name 1	N/A						N/A	
Local Market Name 2	N/A						N/A	
Local Market Name 3	N/A						N/A	

Section 15B - CURRENT SECTOR/CELL INFORMATION - BETA								
ANTENNA CONFIG (FROM BACK)	ANTENNA 1 GSM, UMTS (850 / 1900) or LTE (700 / AWS)		ANTENNA 2 GSM, UMTS (850 / 1900) or LTE (700 / AWS)		ANTENNA 3 GSM, UMTS (850 / 1900) or LTE (700 / AWS)		ANTENNA 4 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	
	TBD	TBD	TBD	TBD	TBD	TBD	TBD	TBD
TECHNOLOGY	UMTS / 850 - GSM / 8	UMTS / 1900					UMTS / 850 - GSM / 8	UMTS / 1900
MHN LOCATION (Top/Bottom/None)	N/A	N/A					N/A	N/A
FEEDER TYPE	1 5/8" - Andrew	1 5/8" - Andrew					1 5/8" - Andrew	1 5/8" - Andrew
Feeder Length (feet)	82"	82"					82"	82"
ANTENNA ATOL	N/A	N/A					N/A	N/A
ANTENNA MAKE - MODEL	BXA-80090-4CF						BXA-80090-4CF	
ANTENNA VENDOR	Antel International Inc.						Antel International Inc.	
ANTENNA SIZE (H x W x D)	TBD						TBD	
ANTENNA WEIGHT	1						1	
Mechanical Gain								
AZIMUTH	150°						150°	
RAZORION CENTER (feet)	62"						62"	
ANTENNA TIP HEIGHT	62"						62"	
ELECTRICAL TILT (150/850/1900 AWS)	N/A						N/A	
MECHANICAL DOWNTILT	N/A						N/A	
FEEDER AMOUNT	1						1	
Antenna RET Motor (QTY/MODEL)	N/A						N/A	
Antenna RET Spitzer (QTY/MODEL)	N/A						N/A	
Antenna RET Earth (Grounding) Clamp (QTY/MODEL)	N/A						N/A	
Antenna RET Surge Arrestor (QTY/MODEL)	N/A						N/A	
Antenna RET CONTROL UNIT (QTY/MODEL) - usually per site	N/A						N/A	
DC BLOCK (QTY/MODEL)	N/A						N/A	
TMALINA (TYP/MODEL)	N/A						N/A	
CURRENT INJECTORS FOR TMA (QTY/MODEL)	Polyphase / 1000860						Polyphase / 1000860	
PDU FOR TMA (QTY/MODEL) - usually per site	LGP 12104 (1900 AND 850 Bypass TMA)						LGP 12104 (1900 AND 850 Bypass TMA)	
SURGE ARRESTOR (QTY/MODEL)	N/A						N/A	
SPLITTER (QTY/MODEL)	1 + N/A / Powerwave / LGP 13519						1 + N/A / Powerwave / LGP 13519	
HYBRID COMBINER (QTY/MODEL)	N/A						N/A	
DUPLEXER (QTY/MODEL)	N/A						N/A	
FILTER (QTY/MODEL)	N/A						N/A	
RAAT KIT MODULE	N/A						N/A	
DUPLEXER or NARROW BAND LDC (QTY/MODEL)	N/A						N/A	
SCRAMBLER MODULE	N/A						N/A	
Additional Component 1	N/A						N/A	
Additional Component 2	N/A						N/A	
Additional Component 3	N/A						N/A	
MAGNETIC DECLINATION	-14°						-14°	
MATCHPLATE POWER (Watts)	N/A						N/A	
ERP (Watts)	N/A						N/A	
Local Market Name 1	N/A						N/A	
Local Market Name 2	N/A						N/A	
Local Market Name 3	N/A						N/A	

Section 15C - CURRENT SECTOR/CELL INFORMATION - GAMMA								
ANTENNA CONFIG (FROM BACK)	ANTENNA 1 GSM, UMTS (850 / 1900) or LTE (700 / AWS)		ANTENNA 2 GSM, UMTS (850 / 1900) or LTE (700 / AWS)		ANTENNA 3 GSM, UMTS (850 / 1900) or LTE (700 / AWS)		ANTENNA 4 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	
	TBD	TBD	TBD	TBD	TBD	TBD	TBD	TBD
TECHNOLOGY	UMTS / 850 - GSM / 8	UMTS / 1900					UMTS / 850 - GSM / 8	UMTS / 1900
MHN LOCATION (Top/Bottom/None)	N/A	N/A					N/A	N/A
FEEDER TYPE	1 5/8" - Andrew	1 5/8" - Andrew					1 5/8" - Andrew	1 5/8" - Andrew
Feeder Length (feet)	82"	82"					82"	82"
ANTENNA ATOL	N/A	N/A					N/A	N/A
ANTENNA MAKE - MODEL	BXA-80090-4CF						BXA-80090-4CF	
ANTENNA VENDOR	Antel International Inc.						Antel International Inc.	
ANTENNA SIZE (H x W x D)	TBD						TBD	
ANTENNA WEIGHT	1						1	
Mechanical Gain								
AZIMUTH	270°						270°	
RAZORION CENTER (feet)	62"						62"	
ANTENNA TIP HEIGHT	62"						62"	
ELECTRICAL TILT (150/850/1900 AWS)	N/A						N/A	
MECHANICAL DOWNTILT	N/A						N/A	
FEEDER AMOUNT	1						1	
Antenna RET Motor (QTY/MODEL)	N/A						N/A	
Antenna RET Spitzer (QTY/MODEL)	N/A						N/A	
Antenna RET Earth (Grounding) Clamp (QTY/MODEL)	N/A						N/A	
Antenna RET Surge Arrestor (QTY/MODEL)	N/A						N/A	
Antenna RET CONTROL UNIT (QTY/MODEL) - usually per site	N/A						N/A	
DC BLOCK (QTY/MODEL)	N/A						N/A	
TMALINA (TYP/MODEL)	N/A						N/A	
CURRENT INJECTORS FOR TMA (QTY/MODEL)	Polyphase / 1000860						Polyphase / 1000860	
PDU FOR TMA (QTY/MODEL) - usually per site	LGP 12104 (1900 AND 850 Bypass TMA)						LGP 12104 (1900 AND 850 Bypass TMA)	
SURGE ARRESTOR (QTY/MODEL)	N/A						N/A	
SPLITTER (QTY/MODEL)	1 + N/A / Powerwave / LGP 13519						1 + N/A / Powerwave / LGP 13519	
HYBRID COMBINER (QTY/MODEL)	N/A						N/A	
DUPLEXER (QTY/MODEL)	N/A						N/A	
FILTER (QTY/MODEL)	N/A						N/A	
RAAT KIT MODULE	N/A						N/A	
DUPLEXER or NARROW BAND LDC (QTY/MODEL)	N/A						N/A	
SCRAMBLER MODULE	N/A						N/A	
Additional Component 1	N/A						N/A	
Additional Component 2	N/A						N/A	
Additional Component 3	N/A						N/A	
MAGNETIC DECLINATION	-14°						-14°	
MATCHPLATE POWER (Watts)	N/A						N/A	
ERP (Watts)	N/A						N/A	
Local Market Name 1	N/A						N/A	
Local Market Name 2	N/A						N/A	
Local Market Name 3	N/A						N/A	

Section 16A - NEW PROPOSED SECTOR CELL INFORMATION - ALPHA (OR OMEGA)					
ANTENNA CONFIG (FROM BACK):	ANTENNA 1 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 2 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 3 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 4 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	
TA/RT	TxRx-TxRx	TxRx-TxRx			
TECHNOLOGY	LTE / UMTS / GSM	UMTS / GSM			
RISER LOCATION (Top/Bottom/None)	BOTTOM	N/A			
FEEDER TYPE	1.5B" - Andrew	1.5B" - Andrew			
Feeder Length (feet)	82"	82"			
ANTENNA ATOLL					
ANTENNA MAKE - MODEL	AMX-CD-16-85-001-RET				
ANTENNA VENDOR	KMW				
ANTENNA SIZE (H x W x D)	72.0 x 11.8 x 5.9				
ANTENNA WEIGHT	48.5				
ANTENNA GAIN	1	17.1 cBd			
AZIMUTH	40°				
RADIATION CENTER (feet)	76"				
ANTENNA TIP HEIGHT	79"				
ELECTRICAL TILT (FOR 850/1900/AWS)	0°	0°			
MECHANICAL DOWN TILT	0°				
FEEDER AMOUNT	2				
Antenna RET Mount (QTY/MODEL)	N/A / KMW / Built-in RET Equipment				
Antenna RET Splitter (QTY/MODEL)	N/A				
Antenna RET Earth Grounding Clamp (QTY/MODEL)	N/A				
Antenna RET Surge Arrestor (QTY/MODEL)	N/A				
Antenna RET CONTROL UNIT (QTY/MODEL) - usually per site	N/A				
DC BLOCK (QTY/MODEL)	N/A				
TRAINING (QTY/MODEL)	1 / CCI / DTMABP7819VG12A				
CURRENT INJECTORS FOR TMA (QTY/MODEL)	Andrew / AP-TOC-BDFDM-DBW				
POU FOR TMA (QTY/MODEL) - usually per site	Kathrein / 860-10006				
BURST ARRESTOR (QTY/MODEL)	1 / Kathrein / F82-11055				
DRY-DIELECTRIC (QTY/MODEL)	0 = 2 / Powerwave / LSP 21901				
HYBRID COMBINER (QTY/MODEL)	N/A				
DUPLEXER (QTY/MODEL)	Dual Duplexer / CCI / DUP-850-2 & DUP-				
FILTER (QTY/MODEL)	N/A				
REAR FEED MODULE	RxMT - 850 Band RxMT - 1900 Band				
DUPLEXER or NARROW BAND LLC (QTY/MODEL)	2 / LLC - 850 Band PCS-AWS Triplexer, DC through to PCS-AWS Port, CCI				
SCALING MODULE	N/A				
Additional Component 1					
Additional Component 2					
Additional Component 3					
MAGNETIC DECLINATION	-14°				
MATCHLINE POWER (Watts)	N/A	N/A			
ERP (Watts)	N/A	N/A			
Local Market Notes					
Local Market Notes					
Local Market Notes					
Section 16B - NEW PROPOSED SECTOR CELL INFORMATION - BETA					
ANTENNA CONFIG (FROM BACK):	ANTENNA 1 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 2 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 3 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 4 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	
TA/RT	TxRx-TxRx	TxRx-TxRx			
TECHNOLOGY	LTE / UMTS / GSM	UMTS / GSM			
RISER LOCATION (Top/Bottom/None)	BOTTOM	N/A			
FEEDER TYPE	1.5B" - Andrew	1.5B" - Andrew			
Feeder Length (feet)	82"	82"			
ANTENNA ATOLL					
ANTENNA MAKE - MODEL	AMX-CD-16-85-001-RET				
ANTENNA VENDOR	KMW				
ANTENNA SIZE (H x W x D)	72.0 x 11.8 x 5.9				
ANTENNA WEIGHT	48.5				
ANTENNA GAIN	1	17.1 cBd			
AZIMUTH	180°				
RADIATION CENTER (feet)	76"				
ANTENNA TIP HEIGHT	79"				
ELECTRICAL TILT (FOR 850/1900/AWS)	0°	0°			
MECHANICAL DOWN TILT	0°				
FEEDER AMOUNT	2				
Antenna RET Mount (QTY/MODEL)	N/A / KMW / Built-in RET Equipment				
Antenna RET Splitter (QTY/MODEL)	N/A				
Antenna RET Earth Grounding Clamp (QTY/MODEL)	N/A				
Antenna RET Surge Arrestor (QTY/MODEL)	N/A				
Antenna RET CONTROL UNIT (QTY/MODEL) - usually per site	N/A				
DC BLOCK (QTY/MODEL)	N/A				
TRAINING (QTY/MODEL)	1 / CCI / DTMABP7819VG12A				
CURRENT INJECTORS FOR TMA (QTY/MODEL)	Andrew / AP-TOC-BDFDM-DBW				
POU FOR TMA (QTY/MODEL) - usually per site	Kathrein / 860-10006				
BURST ARRESTOR (QTY/MODEL)	1 / Kathrein / F82-11055				
DRY-DIELECTRIC (QTY/MODEL)	0 = 2 / Powerwave / LSP 21901				
HYBRID COMBINER (QTY/MODEL)	N/A				
DUPLEXER (QTY/MODEL)	Dual Duplexer / CCI / DUP-850-2 & DUP-				
FILTER (QTY/MODEL)	N/A				
REAR FEED MODULE	RxMT - 850 Band RxMT - 1900 Band				
DUPLEXER or NARROW BAND LLC (QTY/MODEL)	2 / LLC - 850 Band PCS-AWS Triplexer, DC through to PCS-AWS Port, CCI				
SCALING MODULE	N/A				
Additional Component 1					
Additional Component 2					
Additional Component 3					
MAGNETIC DECLINATION	-14°				
MATCHLINE POWER (Watts)	N/A	N/A			
ERP (Watts)	N/A	N/A			
Local Market Notes					
Local Market Notes					
Local Market Notes					
Section 16C - NEW PROPOSED SECTOR CELL INFORMATION - GAMMA					
ANTENNA CONFIG (FROM BACK):	ANTENNA 1 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 2 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 3 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 4 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	
TA/RT	TxRx-TxRx	TxRx-TxRx			
TECHNOLOGY	LTE / UMTS / GSM	UMTS / GSM			
RISER LOCATION (Top/Bottom/None)	BOTTOM	N/A			
FEEDER TYPE	1.5B" - Andrew	1.5B" - Andrew			
Feeder Length (feet)	82"	82"			
ANTENNA ATOLL					
ANTENNA MAKE - MODEL	AMX-CD-14-85-001-RET				
ANTENNA VENDOR	KMW				
ANTENNA SIZE (H x W x D)	48.0 x 11.8 x 5.9				
ANTENNA WEIGHT	36.4				
ANTENNA GAIN	1	16.0 cBd			
AZIMUTH	310°				
RADIATION CENTER (feet)	76"				
ANTENNA TIP HEIGHT	78"				
ELECTRICAL TILT (FOR 850/1900/AWS)	0°	0°			
MECHANICAL DOWN TILT	0°				
FEEDER AMOUNT	2				
Antenna RET Mount (QTY/MODEL)	N/A / KMW / Built-in RET Equipment				
Antenna RET Splitter (QTY/MODEL)	N/A				
Antenna RET Earth Grounding Clamp (QTY/MODEL)	N/A				
Antenna RET Surge Arrestor (QTY/MODEL)	N/A				
Antenna RET CONTROL UNIT (QTY/MODEL) - usually per site	N/A				
DC BLOCK (QTY/MODEL)	N/A				
TRAINING (QTY/MODEL)	1 / CCI / DTMABP7819VG12A				
CURRENT INJECTORS FOR TMA (QTY/MODEL)	Andrew / AP-TOC-BDFDM-DBW				
POU FOR TMA (QTY/MODEL) - usually per site	Kathrein / 860-10006				
BURST ARRESTOR (QTY/MODEL)	1 / Kathrein / F82-11055				
DRY-DIELECTRIC (QTY/MODEL)	0 = 2 / Powerwave / LSP 21901				
HYBRID COMBINER (QTY/MODEL)	N/A				
DUPLEXER (QTY/MODEL)	Dual Duplexer / CCI / DUP-850-2 & DUP-				
FILTER (QTY/MODEL)	N/A				
REAR FEED MODULE	RxMT - 850 Band RxMT - 1900 Band				
DUPLEXER or NARROW BAND LLC (QTY/MODEL)	2 / LLC - 850 Band PCS-AWS Triplexer, DC through to PCS-AWS Port, CCI				
SCALING MODULE	N/A				
Additional Component 1					
Additional Component 2					
Additional Component 3					
MAGNETIC DECLINATION	-14°				
MATCHLINE POWER (Watts)	N/A	N/A			
ERP (Watts)	N/A	N/A			
Local Market Notes					
Local Market Notes					
Local Market Notes					

Section 160 - NEW PROPOSED SECTOR CELL INFORMATION - DELTA				
ANTENNA CONFIG (FROM BACK)	ANTENNA 1 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 2 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 3 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 4 GSM, UMTS (850 / 1900) or LTE (700 / AWS)
TRUNK				
TECHNOLOGY				
SRM LOCATION (Top/Bottom/None)				
FEEDER TYPE				
Feeder Length (feet)				
ANTENNA ATOL				
ANTENNA MAKE - MODEL				
ANTENNA VENDOR				
ANTENNA SIZE (H x W x D)				
ANTENNA WEIGHT				
ANTENNA GAIN				
RADIATOR				
RADIATION CENTER (feet)				
ANTENNA EIP HEIGHT				
ELECTRICAL TILT (THRU/20/0/30/0/45)				
MECHANICAL DOWN TILT				
FEEDER ANCHURE				
Antenna RET Mount (QTY/MODEL)				
Antenna RET Spillar (QTY/MODEL)				
Antenna RET Earth Grounding Clamp (QTY/MODEL)				
Antenna RET Surge Arrester (QTY/MODEL)				
Antenna RET CONTROL UNIT (QTY/MODEL) usually per site				
DC BLOCK (QTY/MODEL)				
TMALINA (TYPE/MODEL)				
CURRENT INJECTORS FOR TMA (QTY/MODEL)				
POU FOR TMA (QTY/MODEL) usually per site				
BONDING ARRESTOR (QTY/MODEL)				
DUPLEXER (QTY/MODEL)				
HYBRID COMBINER (QTY/MODEL)				
DUPLEXER or NARROW BAND LLC (QTY/MODEL)				
SCALING MODULE				
Additional Component 1				
Additional Component 2				
Additional Component 3				
MAGNETIC SCREENING				
MATCHPLATE POWER (Watts)				
EIP (Watts)				
Local Market Notes				
Local Market Notes				
Local Market Notes				
Section 168 - NEW PROPOSED SECTOR CELL INFORMATION - EPISILON				
ANTENNA CONFIG (FROM BACK)	ANTENNA 1 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 2 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 3 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 4 GSM, UMTS (850 / 1900) or LTE (700 / AWS)
TRUNK				
TECHNOLOGY				
SRM LOCATION (Top/Bottom/None)				
FEEDER TYPE				
Feeder Length (feet)				
ANTENNA ATOL				
ANTENNA MAKE - MODEL				
ANTENNA VENDOR				
ANTENNA SIZE (H x W x D)				
ANTENNA WEIGHT				
ANTENNA GAIN				
RADIATOR				
RADIATION CENTER (feet)				
ANTENNA EIP HEIGHT				
ELECTRICAL TILT (THRU/20/0/30/0/45)				
MECHANICAL DOWN TILT				
FEEDER ANCHURE				
Antenna RET Mount (QTY/MODEL)				
Antenna RET Spillar (QTY/MODEL)				
Antenna RET Earth Grounding Clamp (QTY/MODEL)				
Antenna RET Surge Arrester (QTY/MODEL)				
Antenna RET CONTROL UNIT (QTY/MODEL) usually per site				
DC BLOCK (QTY/MODEL)				
TMALINA (TYPE/MODEL)				
CURRENT INJECTORS FOR TMA (QTY/MODEL)				
POU FOR TMA (QTY/MODEL) usually per site				
BONDING ARRESTOR (QTY/MODEL)				
DUPLEXER (QTY/MODEL)				
HYBRID COMBINER (QTY/MODEL)				
DUPLEXER or NARROW BAND LLC (QTY/MODEL)				
SCALING MODULE				
Additional Component 1				
Additional Component 2				
Additional Component 3				
MAGNETIC SCREENING				
MATCHPLATE POWER (Watts)				
EIP (Watts)				
Local Market Notes				
Local Market Notes				
Local Market Notes				
Section 16F - NEW PROPOSED SECTOR CELL INFORMATION - ZETA				
ANTENNA CONFIG (FROM BACK)	ANTENNA 1 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 2 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 3 GSM, UMTS (850 / 1900) or LTE (700 / AWS)	ANTENNA 4 GSM, UMTS (850 / 1900) or LTE (700 / AWS)
TRUNK				
TECHNOLOGY				
SRM LOCATION (Top/Bottom/None)				
FEEDER TYPE				
Feeder Length (feet)				
ANTENNA ATOL				
ANTENNA MAKE - MODEL				
ANTENNA VENDOR				
ANTENNA SIZE (H x W x D)				
ANTENNA WEIGHT				
ANTENNA GAIN				
RADIATOR				
RADIATION CENTER (feet)				
ANTENNA EIP HEIGHT				
ELECTRICAL TILT (THRU/20/0/30/0/45)				
MECHANICAL DOWN TILT				
FEEDER ANCHURE				
Antenna RET Mount (QTY/MODEL)				
Antenna RET Spillar (QTY/MODEL)				
Antenna RET Earth Grounding Clamp (QTY/MODEL)				
Antenna RET Surge Arrester (QTY/MODEL)				
Antenna RET CONTROL UNIT (QTY/MODEL) usually per site				
DC BLOCK (QTY/MODEL)				
TMALINA (TYPE/MODEL)				
CURRENT INJECTORS FOR TMA (QTY/MODEL)				
POU FOR TMA (QTY/MODEL) usually per site				
BONDING ARRESTOR (QTY/MODEL)				
DUPLEXER (QTY/MODEL)				
HYBRID COMBINER (QTY/MODEL)				
DUPLEXER or NARROW BAND LLC (QTY/MODEL)				
SCALING MODULE				
Additional Component 1				
Additional Component 2				
Additional Component 3				
MAGNETIC SCREENING				
MATCHPLATE POWER (Watts)				
EIP (Watts)				
Local Market Notes				
Local Market Notes				
Local Market Notes				

AM-X-CD-16-65-00T-RET(6' 65° Dual Broadband Antenna)

Dual Band Electrical DownTilt Antenna

698 ~ 894MHz, X-pol., H65° / V12°

1710 ~ 2170MHz, X-pol., H65° / V6.0°

Electrical Specification

Frequency Range	698~894MHz	1710~2170MHz
Impedance	50Ω	
Polarization	Dual, Slant ±45°	
Gain	15.5dBi / 13.35dBd @ 698-806MHz 16.0dBi / 13.85dBd @ 824-894MHz	17.3dBi / 15.15dBd @ 1710-1755MHz 17.4dBi / 15.25dBd @ 1850-1900MHz 17.1dBi / 14.95dBd @ 2110-2155MHz
Beamwidth	Horizontal	65° @ 698-806MHz 63° @ 824-894MHz
	Vertical	65° @ 1710-1755MHz 67° @ 1850-1900MHz 69° @ 2110-2155MHz
	12.3° @ 698-806MHz 11.5° @ 824-894MHz	6.5° @ 1710-1755MHz 6.0° @ 1850-1900MHz 5.7° @ 2110-2155MHz
VSWR	≤1.5:1	
Front-to-Back Ratio	≥27 dB	
Electrical Downtilt Range	2° ~ 16°	0° ~ 10°
Isolation Between Ports	≥30 dB	
Isolation Between Ports of Different Frequency Elements	≥35 dB	
Cross Pole Discrimination	10.0 dB @ ±60° 15.0 dBi @ 0°	
First Upper Side Lobe Suppression	16dB	
Side Lobe Suppression	> 16 dB @ 0-6° Tilt > 18 dB @ 7-12° Tilt (Up to 10° from Boresight)	> 16 dB @ 0-6° Tilt > 18 dB @ 7-10° Tilt (Up to 10° from Boresight)
Passive Intermodulation	≤ -150 dBc @ 2x20w	
Input Maximum CW Power	500 W	300 W
Environmental Compliance	IP65 for Radome IP67 for Connectors	
RET Motor Configuration	Field Replaceable RET Electronic Control Module / RET Motor is internal to antenna & not field replaceable	
Compliant with AISG 1.1 and 2.0	AISG 1.1 and 2.0	

Mechanical Specification

Dimension (WxDxH)	11.8x5.9x72 inches (300x150x1829mm)
Weight (Without clamp)	48.5 lbs (22.0 kg)
Connector	4 x 7/16 DIN(F), Long Neck
Max Wind Speed	150 mph
Wind Load (@150 mph)	1891 N



Twin Triple Band "Active PCS with 700 and 850 Band Pass-thru" Dual Duplexed TMA

Tel: 201-342-3338

Fax: 201-342-3339

www.cciproduts.com



General Information



CCI's Twin Triple Band (700 Band, Cellular and PCS) TMA contains two triple band TMA's in a single housing. The PCS TMA is full band and fully duplexed, while the 700 Band and Cellular RF is bypassed and combined (Diplexed) with the PCS RF signal. High linearity improves the uplink sensitivity and the receive performance of base stations. The TMA is fully compliant with the latest AISG 2.0 specification. The TMA supports EDGE/GSM, UMTS and LTE BTS equipment. It provides a convenient package for sites upgraded to triple or quad antenna configurations. The twin TMA package reduces tower loading, leasing, and installation costs. Unit count on the tower is cut in half. An excellent match for two branch receive diversity applications using triple polarization antennas. The input and output connectors are located inline for ease of installation in space constrained areas such as uni-pole structures and stealth antennas.

Model
DTMABP7819VG12A

Contents:

General Info and Technical Description	1
Electrical & Mechanical Specs (AISG TMA)	2
Block Diagram & Outline Drawing (AISG TMA)	3

Features:

- Small, lightweight, twin unit
- Triple Band Dual Duplexed (PCS with 700 Band & Cellular Bypass)
- Optional AISG 2.0 compatible unit
- AISG TMA detects BTS port that DC voltage and AISG sampling is applied to, and automatically switches to utilize that port
- AISG TMA operates at constant power
- AISG TMA may be powered by a standard PDU
- High linearity
- Lightning protected
- Fail-safe bypass mode
- High reliability

Technical Description

The TMA system consists of a twin outdoor triple band tower mount unit which combine separate PCS, 700 Band & Cellular antennas onto a single BTS port. The PCS path of the tower mount unit is dual duplexed to separate the low-power uplink signals from the high-power downlink signals at the antenna port, amplifies the low-level uplink signals using an ultra-low noise amplifier (LNA), and recombines the two paths at the BTS port. The 700 Band & Cellular path is ultra low loss and passive. Both paths are diplexed at the BTS port. The tower mount units consist of eight band-pass filters, two redundant low-noise amplifiers, bypass failure circuitry, and bias tee's which are all housed in an IP65 moisture proof enclosure, with IP68 Immersion proof connectors suited to long-life masthead mounting. The unit provides protection against lightning strikes via a multi-stage surge protection circuit. DC power and control is provided via the feeder cable from the BTS or a Power Distribution Unit (PDU). Optional AISG 2.0 DC power and control is provided via the feeder cable from the BTS using the AISG 2.0 and 3GPP standard. The optional AISG TMA detects which BTS port has DC Voltage/AISG Sampling applied and automatically switches to utilize that port. Additionally the AISG TMA operates at constant power when powered by an AISG 2.0 Compatible Site Control Unit, but may be powered by a "Standard Power distribution Unit. A separate AISG connector is also provided to allow direct AISG connection or "Daisy Chaining" to multiple AISG products at the top of the tower.

An optional indoor site control unit (SCU) is available to power up to up to 32 AISG modules per sector and to provide the all the monitoring and alarm functions for the system. The SCU is housed in a single (1U) 1.75" x 19" rack and contains triple redundant power supplies capable of being "hot swapped" that provide a regulated DC supply voltage on the RF coax for the tower mount amplifiers.

Twin Triple Band "Active AWS with 700 and 850 Band Pass-thru" TMA Typical Specifications



Description	Typical Specifications
Electrical Specifications	
700 Band & Cellular Frequency Range	698 to 894 MHz
PCS Receive Frequency Range	1850 – 1910 MHz
PCS Transmit Frequency Range	1930 - 1990 MHz
PCS Amplifier Gain	6 to 12 dB Adjustable in 0.25 dB steps via AISG
PCS Gain Variation	±1.0 dB
PCS System Noise Figure	1.4 dB (@ +25°C), 1.6 dB (@ +65°C), At 1910 MHz: 1.7 dB (@ +25°C), 1.9 dB (@ +65°C)
PCS Input Third Order Intercept Point	+12 dBm Min @ Max. Gain
Input/Output Return Loss	18 dB Min. all ports, 15 dB Min. Bypass Mode
Insertion Loss	
700 Band & Cellular Passband	< 0.2 dB, 0.1 dB typical
PCS Transmit Passband	0.4 dB Typical
PCS Transmit Passband Ripple	±0.2 dB
PCS Bypass Mode, Rx Passband	1.6 dB (@ +25°C), 1.8 dB (@ +65°C), At 1910 MHz: 2.3 dB (@ +25°C), 2.5 dB (@ +65°C)
PCS Bypass Mode, Rx Passband Ripple	±1 dB
Filter Characteristics	
700 Band & Cellular Path Rejection	70 dB @ 1850 - 1990 MHz
PCS Path Rejection	80 dB @ 698 - 894 MHz
Continuous Average Power	200 Watts max
Peak Envelope Power	2 kW max
Intermodulation Performance	
IMD at ANT port in Rx Band	-112 dBm Min. (2 x +43 dBm tones)
Operating Voltage	+10V to +30V DC provided via coax or AISG
Power Consumption	≤ 2.1 Watts
Mechanical Specifications	
Connectors	DIN 7-16 Female (Long Neck) x 6, AISG x 1
Dimensions (Body Only)	10.63" (H) x 11.02" (W) x 3.78" (D); (270 (H) x 280 (W) x 96 (D) mm)
Dimensions (with Bracket)	14.25" (H) x 11.46" (W) x 4.17" (D); (362 (H) x 291 (W) x 106 (D) mm)
Weight (w/o Bracket)	19.18 Lbs. (8.7 Kg)
Mounting	Pole/Wall Mounting Bracket
Environmental Specifications	
Operating Temperature	-40° C to +65°C
Lightning Protection	8/20us, ±2KA max, 10 strikes each, IEC61000-4-5
Enclosure	IP65 (Unit Body), IP68 (Connector)
MTBF	>500,000 hours

All specifications are subject to change. The latest specifications are available at www.cciproducs.com

Communication Components Inc.

Tel: 201-342-3338

CCI Confidential

Fax: 201-342-3339

ATTACHMENT 7

ADJACENT PROPERTY OWNERS

SITE NAME: WEST HARTFORD WEST RELOCATION – TEMPORARY TOWER

OWNER NAME: AMERICAN SCHOOL FOR THE DEAF

OWNER ADDRESS: 139 NORTH MAIN STREET, WEST HARTFORD, CONNECTICUT

ASSESSOR'S REFERENCE: MAP: F7 LOT: 3836

THE FOLLOWING INFORMATION WAS COLLECTED FROM THE TAX ASSESSOR'S RECORDS AND LAND RECORDS OF WEST HARTFORD TOWN HALL. THE INFORMATION IS CURRENT AS OF APRIL 25, 2014.

	<u>Map/Lot</u>	<u>Property Address</u>	<u>Property Owner</u>
1.	E8/2021/27	27 Fernridge Road	John S. and Carrie L. Benyei 27 Fernridge Road West Hartford, CT 06107
2.	E8/2021/29	29 Fernridge Road	Matthew and Jill Kenyon 29 Fernridge Road West Hartford, CT 06107
3.	E8/2021/31	31 Fernridge Road	Nancy Bernaiche 31 Fernridge Road West Hartford, CT 06107
4.	E8/2021/35	35 Fernridge Road	Anna-Mae F. Maglaty 35 Fernridge Road West Hartford, CT 06107
5.	E8/1981/572	572 Fern Street	The American School for the Deaf of Hartford 139 North Main Street West Hartford, CT 06107
6.	E8/1131/36	36 Cobbs Road	Patricia A. Farquhar 36 Cobbs Road West Hartford, CT 06107
7.	E8/1131/34	34 Cobbs Road	Paula M. Anderson 34 Cobbs Road West Hartford, CT 06107

8.	E7/1131/32	32 Cobbs Road	James A. Siegel and Holly C. Van Deusen 32 Cobbs Road West Hartford, CT 06107
9.	E7/1131/30	30 Cobbs Road	James A. Kammerer 30 Cobbs Road West Hartford, CT 06107
10.	E7/1131/24	24 Cobbs Road	Robert L. Martin and Angela M. Guadagno 24 Cobbs Road West Hartford, CT 06107
11.	E7/1131/22	22 Cobbs Road	Francis P. and Cathleen F. Laffin 22 Cobbs Road West Hartford, CT 06107
12.	E7/1131/20	20 Cobbs Road	Wendy L. Smith 20 Cobbs Road West Hartford, CT 06107
13.	E7/1131/18	18 Cobbs Road	Milbrey E. Ewing 18 Cobbs Road West Hartford, CT 06107
14.	E7/1131/16	16 Cobbs Road	Martin Murphy, Jr. 16 Cobbs Road West Hartford, CT 06107
15.	E7/1131/14	14 Cobbs Road	Robert E. Bell and Justine Fallon 14 Cobbs Road West Hartford, CT 06107
16.	E8/1131/12	12 Cobbs Road	Steven J. and Rosemary A. Dacunha 12 Cobbs Road West Hartford, CT 06107
17.	E8/1131/10	10 Cobbs Road	Estate of Marion S. Gebelein c/o Edward F. Gebelein, Jr., Exr P.O. Box 52 Winchester Center, Ct 06094-0052
18.	E8/1131/8	8 Cobbs Road	Pamela B. Ponichtera 8 Cobbs Road West Hartford, CT 06107

19.	E8/1981/534	534 Fern Street	Christopher and Nancy Schwartz 534 Fern Street West Hartford, CT 06107
20.	E8/1981/532	532 Fern Street	Kevin C. Quiros and Joyce M. Chung 532 Fern Street West Hartford, CT 06107
21.	E8/1981/528	528 Fern Street	Noel E. Cyr 528 Fern Street West Hartford, CT 06107
22.	E8/2741/73	73 Hilltop Drive	The American School for the Deaf of Hartford 139 North Main Street West Hartford, CT 06107
23.	E7/2741/69	69 Hilltop Drive	Peter C. and Karen M. Lavoie 69 Hilltop Drive West Hartford, CT 06107
24.	E7/2741/67	67 Hilltop Drive	David Ryan Polgar 67 Hilltop Drive West Hartford, CT 06107
25.	E7/2741/65	65 Hilltop Drive	Dorrance E. and Cynthia C. Goodwin 65 Hilltop Drive West Hartford, CT 06107
26.	E7/2741/62	62 Hilltop Drive	Mary Pat Hagar 62 Hilltop Drive West Hartford, CT 06107
27.	E7/2741/60	60 Hilltop Drive	Ethan F. Loiselle 60 Hilltop Drive West Hartford, CT 06107
28.	E7/2741/58	58 Hilltop Drive	Lisa Davidson Nelte 58 Hilltop Drive West Hartford, CT 06107
29.	E7/2741/56	56 Hilltop Drive	Robert E. and Ann K. Tuthill 56 Hilltop Drive West Hartford, CT 06107

30.	E7/2741/54	54 Hilltop Drive	Brian Venable and Raminder J. Singh 54 Hilltop Drive West Hartford, CT 06107
31.	E7/2741/52	52 Hilltop Drive	Robert A. and Stephanie H. Schenkel 52 Hilltop Drive West Hartford, CT 06107
32.	E7/2741/50	50 Hilltop Drive	Nancy Hudson 50 Hilltop Drive West Hartford, CT 06107
33.	E7/2741/46	46 Hilltop Drive	Pamela J. Rosin 46 Hilltop Drive West Hartford, CT 06107
34.	E7/2741/44	44 Hilltop Drive	NICDAN, LLC 250-30 41 st Road Littleneck, NY 11363
35.	E7/2741/42	42 Hilltop Drive	Shane Mulready 42 Hilltop Drive West Hartford, CT 06107
36.	E7/2741/40	40 Hilltop Drive	Kimberly J. Wisner 40 Hilltop Drive West Hartford, CT 06107
37.	F7/2741/38	38 Hilltop Drive	Barbara A. Jarosz and Michael R. Leahy 38 Hilltop Drive West Hartford, CT 06107
38.	F7/2741/34	34 Hilltop Drive	Alexandra F. Zrakas 34 Hilltop Drive West Hartford, CT 06107
39.	F7/2741/32	32 Hilltop Drive	Brian D. Healy 32 Hilltop Drive West Hartford, CT 06107
40.	F7/2741/30	30 Hilltop Drive	Susanne D. Spargo 30 Hilltop Drive West Hartford, CT 06107

41.	F7/2741/28	28 Hilltop Drive	Kristen J. Noone 28 Hilltop Drive West Hartford, CT 06107
42.	F7/2741/26	26 Hilltop Drive	Daniel Tavelli and Elizabeth Murray 26 Hilltop Drive West Hartford, CT 06107
43.	F7/2741/22	22 Hilltop Drive	Carol J. Dingelday 22 Hilltop Drive West Hartford, CT 06107
44.	F7/2741/20	20 Hilltop Drive	Deborah L. Hutton 20 Hilltop Drive West Hartford, CT 06107
45.	F7/2741/18	18 Hilltop Drive	Martha J. Taylor 18 Hilltop Drive West Hartford, CT 06107
46.	F7/2741/16	16 Hilltop Drive	Lei Wang 16 Hilltop Drive West Hartford, CT 06107
47.	F7/2741/14	14 Hilltop Drive	Joseph A. Galluccio 14 Hilltop Drive West Hartford, CT 06107
48.	F7/2741/10	10 Hilltop Drive	Douglas E. and Elaine C. McIntyre c/o Robert and Medora Charette 10 Hilltop Drive West Hartford, CT 06107
49.	F7/2741/8	8 Hilltop Drive	NICDAN 1 LLC 250-30 41 st Road Little Neck, NY 11363
50.	F7/2741/6	6 Hilltop Drive	Elsie J. Rickard 6 Hilltop Drive West Hartford, CT 06107
51.	F7/3836/125	125 North Main Street	Sharon Ashley Roberts 125 North Main Street West Hartford, CT 06107

52.	F7/3836/126	126 North Main Street	Guy L. and Sheryl Cusano 126 North Main Street West Hartford, CT 06107
53.	F7/3836/128	128 North Main Street	Fernando S. and Maria C. Ferreira 128 North Main Street West Hartford, CT 06107
54.	F7/3836/132	132 North Main Street	Jennifer G. Searls 132 North Main Street West Hartford, CT 06107
55.	F7/3836/134	134 North Main Street	David A. and Karen M. Walsh 134 North Main Street West Hartford, CT 06107
56.	F7/3836/136	136 North Main Street	Ryan P. Brown 136 North Main Street West Hartford, CT 06107
57.	F7/3836/138	138 North Main Street	Isaac A. Kamola and Serena Laws 138 North Main Street West Hartford, CT 06107
58.	F7/3836/142	142 North Main Street	Christopher J. Baran and Rebecca Bucha 142 North Main Street West Hartford, CT 06107
59.	F7/3836/144	144 North Main Street	Richard M. and Mary E. Carotenuti 144 North Main Street West Hartford, CT 06107
60.	F7/3261/73	73 Linnard Road	Douglas and Janice N. Larson 73 Linnard Road West Hartford, CT 06107
61.	F7/3836/152	152 North Main Street	Dean G. and Margaret H. Monos 152 North Main Street West Hartford, CT 06107
62.	F7/3836/154	154 North Main Street	Ruby O. Durian 154 North Main Street West Hartford, CT 06107

63.	F7/3836/156	156 North Main Street	Renee Rovelli 156 North Main Street West Hartford, CT 06107
64.	F7/3836/160	160 North Main Street	Anne E. and Peter W. Dehertogh III 160 North Main Street West Hartford, CT 06107
65.	F7/3836/160	160 North Main Street (acct # 3836 2 160 201)	Town of West Hartford 50 South Main Street West Hartford, CT 06107
66.	F7/6361/3	3 Wyndwood Road	Jeffrey E. Balboni 3 Wyndwood Road West Hartford, CT 06107
67.	F7/6361/7	7 Wyndwood Road	Scott C. Merrell 7 Wyndwood Road West Hartford, CT 06107
68.	F7/6361/11	11 Wyndwood Road	Edward J. Raftery 11 Wyndwood Road West Hartford, CT 06107
69.	F7/6361/15	15 Wyndwood Road	Mary Ellen Thibodeau 15 Wyndwood Road West Hartford, CT 06107
70.	F7/6361/17	17 Wyndwood Road	William E. and Brigitte H. Stiles 17 Wyndwood Road West Hartford, CT 06107
71.	F7/6361/21	21 Wyndwood Road	C. Randall Schmidt and Linda B. Domenitz 21 Wyndwood Road West Hartford, CT 06107
72.	E7/6361/25	25 Wyndwood Road	Leo B. Harrington and Karen Libertoff Herrington 25 Wyndwood Road West Hartford, CT 06107
73.	E7/6361/27	27 Wyndwood Road	Robert John and Joan P. Zinky 27 Wyndwood Road West Hartford, CT 06107

74.	E7/6361/31	31 Wyndwood Road	Anne M. Danaher 31 Wyndwood Road West Hartford, CT 06107
75.	E7/6361/33	33 Wyndwood Road	Jonathan and Allison Mason 33 Wyndwood Road West Hartford, CT 06107
76.	E7/6361/37	37 Wyndwood Road	James D. and Elizabeth A. Kuhn 37 Wyndwood Road West Hartford, CT 06107
77.	E7/6361/39	39 Wyndwood Road	Richard R. Phillips and Sylvia Dehaas-Phillips 39 Wyndwood Road West Hartford, CT 06107
78.	E7/6361/43	43 Wyndwood Road	Timothy J. Holsbeke and Stacey H. Rubin 43 Wyndwood Road West Hartford, CT 06107
79.	E7/6361/45	45 Wyndwood Road	Scott and Susan Mangini 45 Wyndwood Road West Hartford, CT 06107
80.	E7/6361/47	47 Wyndwood Road	Joan W. Grenham 47 Wyndwood Road West Hartford, CT 06107
81.	E7/6361/51	51 Wyndwood Road	Handa Xi and Chao Liu 51 Wyndwood Road West Hartford, CT 06107
82.	E7/6361/55	55 Wyndwood Road	Garrett Griffiths and Katherine Leduc 55 Wyndwood Road West Hartford, CT 06107
83.	E7/6361/59	59 Wyndwood Road	Vicki Melchiorre 59 Wyndwood Road West Hartford, CT 06107
84.	E7/6361/63	63 Wyndwood Road	Matthew F. and Leigh C. Eppinger 63 Wyndwood Road West Hartford, CT 06107

85.	E7/6361/67	67 Wyndwood Road	Sally Speer 67 Wyndwood Road West Hartford, CT 06107
86.	E7/1081/115	115 Cliffmore Road	Elton P. and Rosalind S. Katz 115 Cliffmore Road West Hartford, CT 06107
87.	E7/1081/111	111 Cliffmore Road	David N. and Elizabeth N. Sankar 111 Cliffmore Road West Hartford, CT 06107
88.	E7/1081/107	107 Cliffmore Road	George Keith and Rose S. Bernhardt 107 Cliffmore Road West Hartford, CT 06107
89.	E7/4731/20	20 Rustic Lane	James R. and Joanne Farrell 20 Rustic Lane West Hartford, CT 06107
90.	E7/4731/16	16 Rustic Lane	Jesse Delaney 16 Rustic Lane West Hartford, CT 06107
91.	E7/4731/12	12 Rustic Lane	Dorothy F. Elsner 12 Rustic Lane West Hartford, CT 06107
92.	E7/4731/10	10 Rustic Lane	Quang A. and Mai T. Chu 10 Rustic Lane West Hartford, CT 06107
93.	E7/4731/6	6 Rustic Lane	Timothy J. Tierney 6 Rustic Lane West Hartford, CT 06107
94.	E7/4731/2	2 Rustic Lane	Tamika T. Brierley 2 Rustic Lane West Hartford, CT 06107
95.	E7/1626/61	61 East Maxwell Drive	Emiliya Prokopets and Sam Razumnuy 61 East Maxwell Drive West Hartford, CT 06107

CERTIFICATION OF SERVICE

I hereby certify that a copy of the attached letter was sent by first class mail, postage prepage, to each of the parties on the attached list of abutting landowners.

5-2-14

Date



Kenneth C. Baldwin, Esq.
Robinson & Cole LLP
280 Trumbull Street
Hartford, CT 06103

Attorneys for CELLCO PARTNERSHIP d/b/a
VERIZON WIRELESS

CERTIFICATION OF SERVICE

I hereby certify that on this 2nd day of May, 2014, a copy of the attached notice letter was sent first class mail, postage prepaid, to the following:

STATE OFFICIALS:

The Honorable George Jepsen
Attorney General
Office of the Attorney General
55 Elm Street
Hartford, CT 06106

Dora B. Schriro, Commissioner
Department of Emergency Services and Public Protection
Emergency Management and Homeland Security Division
25 Sigourney Street, 6th Floor
Hartford, CT 06106-5042

Rob Klee, Commissioner
Department of Energy and Environmental Protection
79 Elm Street
Hartford, CT 06106

Jewel Mullen, M.D., M.P.H., M.P.A., Commissioner
Department of Public Health
410 Capitol Avenue
P.O. Box 340308, MS 13COM
Hartford, CT 06134-0308

Karl J. Wagener, Executive Director
Council on Environmental Quality
79 Elm Street
Hartford, CT 06106

Arthur House, Chairman
Public Utilities Regulatory Authority
Ten Franklin Square
New Britain, CT 06051

Benjamin Barnes, Secretary
Office of Policy and Management
450 Capitol Avenue
Hartford, CT 06106

Catherine Smith, Commissioner
Department of Economic and Community Development
505 Hudson Street
Hartford, CT 06106

James P. Redeker, Commissioner
Department of Transportation
P.O. Box 317546
2800 Berlin Turnpike
Newington, CT 06131-7546

Daniel T. Forrest
Deputy State Historic Preservation Officer
Connecticut Commission on Culture & Tourism
Historic Preservation and Museum Division
One Constitution Plaza, 2nd Floor
Hartford, CT 06103

Steven K. Reviczky, Commissioner
Department of Agriculture
165 Capital Avenue
Hartford, CT 06106

Capitol Region Council of Governments
241 Main Street, 4th Floor
Hartford, CT 06106-5310

WEST HARTFORD TOWN OFFICIALS:

Scott Slifka
Mayor
Town of West Hartford
50 South Main Street
West Hartford, CT 06107
(Received copy of full Petition)

Ron Van Winkle
Town Manager
Town of West Hartford
50 South Main Street, Room 310
West Hartford, CT 06107

The Honorable Brian Becker
Representative – 19th District
Legislative Office Building
Room 4009
Hartford, CT 06106

The Honorable Beth Bye
Senator – 5th District
Legislative Office Building
Room 3100
Hartford, CT 06106

Kevin Ahern, Chairman
Town Plan & Zoning Commission and
Inland Wetlands and Watercourses Agency
Town of West Hartford
50 South Main Street, Room 214
West Hartford, CT 06107

Todd Dumais, Town Planner
Planning and Zoning Division
Town of West Hartford
50 South Main Street, Room 214
West Hartford, CT 06107

FEDERAL OFFICIALS:

The Honorable Richard Blumenthal
United States Senator
702 Hart Senate Office Building
Washington, DC 20510

The Honorable Christopher Murphy
United States Senator
B40A Dirksen Senate Office Building
Washington, DC 20510

The Honorable John B. Larson
Congressman
221 Main Street, 2nd Floor
Hartford, CT 06106

Federal Communications Commission
445 12th Street SW
Washington, DC 20554



Kenneth C. Baldwin, Esq.
Robinson & Cole LLP
280 Trumbull Street
Hartford, CT 06103
Telephone: (860) 275-8200
Attorneys for Celco Partnership d/b/a Verizon Wireless

KENNETH C. BALDWIN

280 Trumbull Street
Hartford, CT 06103-3597
Main (860) 275-8200
Fax (860) 275-8299
kbaldwin@rc.com
Direct (860) 275-8345

Also admitted in Massachusetts

May 2, 2014

«Name_and_Address»

**Re: Cellco Partnership d/b/a Verizon Wireless
Petition for the Installation of a Temporary Wireless Facility at the
American School for the Deaf Property, 139 North Main Street,
West Hartford, Connecticut**

Dear «Salutation»:

On June 27, 2013, the Connecticut Siting Council (“Council”) approved an application by Cellco Partnership d/b/a Verizon Wireless (“Cellco”) for the installation of a new wireless telecommunications facility on property owned by the American School for the Deaf (“ASD”), 139 North Main Street, West Hartford, Connecticut (Council Docket No. 434). This new facility would replace the existing wireless facilities that both Cellco and AT&T Wireless currently maintain on the Gallaudet Hall cupola on the ASD campus.

ASD plans to demolish Gallaudet Hall prior to the completion and activation of the new wireless facility. In order to maintain critical wireless services in the area, Cellco and AT&T Wireless will be installing a temporary telecommunications tower and related equipment immediately north of the approved replacement telecommunications facility, currently under construction.

May 2, 2014

Page 2

Today, Cellco and AT&T have filed a Petition for Declaratory Ruling with the Council seeking its approval for this temporary wireless facility. The temporary facility would consist of an 85-foot ballast tower and two small equipment shelters. (See attached plans). The temporary facility would be in place for approximately five (5) months from June 1, 2014 to October 1, 2014, and would be removed once the wireless facility approved in Docket No. 434 is completed and activated.

If you have any questions regarding the above-referenced Petition for Declaratory Ruling please feel free to contact me or the Siting Council directly. The Siting Council can be reached at 860-827-2935.

Very truly yours,

A handwritten signature in black ink, appearing to read "Kenneth C. Baldwin". The signature is fluid and cursive, with a long horizontal stroke at the end.

Kenneth C. Baldwin

KCB/kmd

Cellco Partnership

d.b.a. **verizon** wireless
WIRELESS COMMUNICATIONS FACILITY
WEST HARTFORD WEST
TEMPORARY TOWER INSTALL
139 NORTH MAIN STREET
WEST HARTFORD, CT 06107

SITE DIRECTIONS

FROM: 50 EAST WIND BROOK EAST HARTFORD, CONNECTICUT TO: 139 NORTH MAIN STREET WEST HARTFORD, CT

1. Drive east along EAST WIND BROOK DRIVE toward SARGAN STREET.
2. Turn LEFT on SARGAN STREET.
3. Drive east on SARGAN STREET.
4. Turn LEFT on SARGAN STREET.
5. Drive east on SARGAN STREET.
6. Turn RIGHT on SARGAN STREET.
7. Drive east on SARGAN STREET.
8. Turn RIGHT on SARGAN STREET.

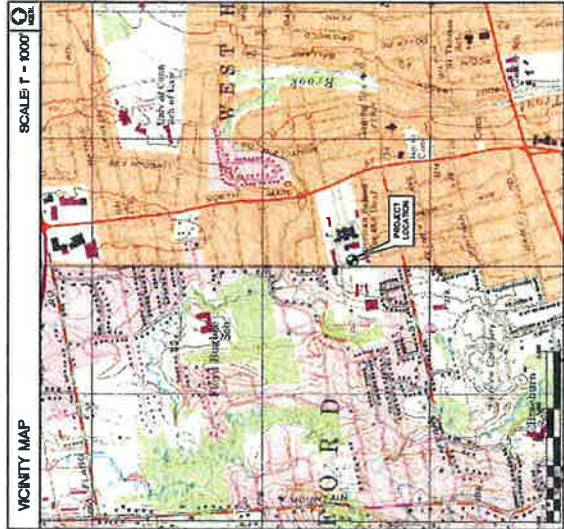
GENERAL NOTES

1. PROPOSED ANTENNA LOCATION AND HEIGHTS PROVIDED BY CELCO PARTNERSHIP AND AIRE.

SITE INFORMATION

THE SCOPE OF WORK SHALL INCLUDE:

1. PARTNERSHIP TO PROVIDE WIRELESS COMMUNICATIONS EQUIPMENT INCLUDING BUT NOT LIMITED TO ANTENNAS, PARALLEL MOUNTED WIRELESS COMMUNICATIONS EQUIPMENT, AND AN ANTENNA RIGID EQUIPMENT SHIELDER ADJACENT TO THE PROPOSED ANTENNA LOCATION.
2. THE PROPOSED WIRELESS FACILITY INSTALLATION WILL BE OCCURRED IN ACCORDANCE WITH THE 2006 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2006 CONNECTICUT SUPPLEMENT.



PROJECT SUMMARY

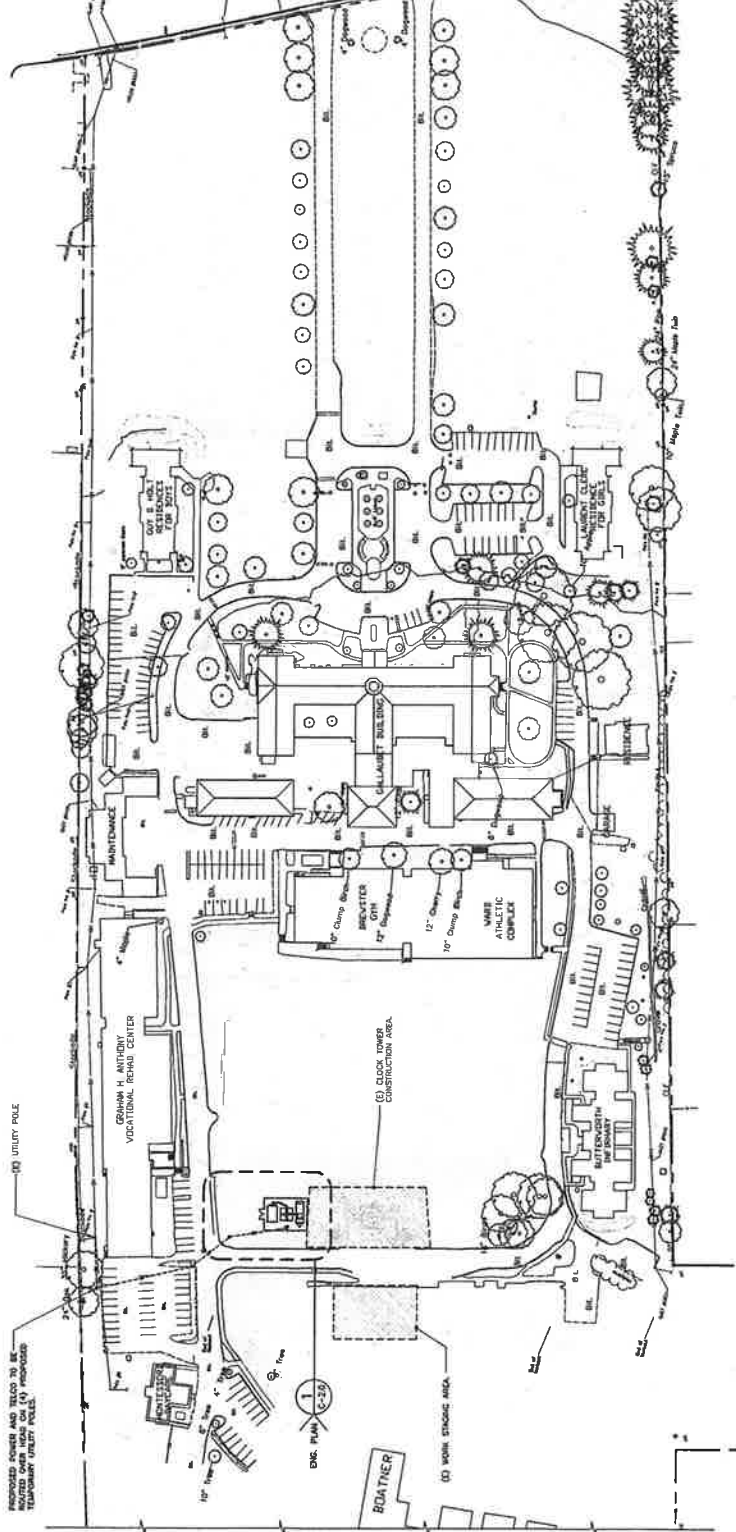
SITE NAME: WEST HARTFORD WEST
 SITE ADDRESS: 139 NORTH MAIN STREET WEST HARTFORD, CT 06107
 PROPERTY OWNER: AMERICAN SCHOOL FOR THE DEAF 139 NORTH MAIN STREET WEST HARTFORD, CT 06107
 LESSEE/tenant: CELCO PARTNERSHIP 69 EAST PASTOR WALKER EAST HARTFORD, CT 06108
 CONTRACT PERSON: SANEY CARTER CELCO PARTNERSHIP (800) 803-8210
 ENGINEER: CHUCKY ENGINEERING 1000 WEST MAIN STREET HARTFORD, CT 06103
 TEMPORARY TOWER COORDINATE: 41°-44'-13.30" N 72°-50'-13.30" W
 COORDINATE AND GROUND ELEVATION BASED ON 1-A
 AND ASSOCIATES DATED APRIL 24, 2014.

SHEET INDEX

NO.	DESCRIPTION	REV.
1-1	TITLE SHEET	1
C-1.0	FINAL SITE SURVEY PLAN	1
C-2.0	SITE DEVELOPMENT PLAN AND SPEC. SITE DETAILS	1
C-3.0	SITE CONSTRUCTION, SEE CONSTRUCTION NOTES AND DETAILS	1
C-4.0	ELEVATION AND ANTENNA CONFIGURATION	1

Cellco Partnership d/b/a Verizon Wireless
 WEST HARTFORD WEST TEMPORARY TOWER INSTALL
 139 NORTH MAIN STREET WEST HARTFORD, CT
 DATE: 04/17/14
 SCALE: AS NOTED
 JOB NO.: 139T1000
 TITLE SHEET
 T-1
 Draw No. 1 of 5

NORTH MAIN STREET



PROJECT INFORMATION		DATE		DRAWN BY		CHECKED BY		DATE	
PROJECT NO.	2003-010	DATE	07/11/14	DRAWN BY	JEF	CHECKED BY	JEF	DATE	07/11/14
PROJECT NAME	WEST HARTFORD WEST TEMPORARY TOWER INSTALL	DRAWN BY	JEF	CHECKED BY	JEF	DATE	07/11/14	DRAWN BY	JEF
CAD FILE	C:\projects\2003-010\dwg\01.dwg	CHECKED BY	JEF	DATE	07/11/14	DRAWN BY	JEF	DATE	07/11/14
CAD USER	JEF	CHECKED BY	JEF	DATE	07/11/14	DRAWN BY	JEF	DATE	07/11/14
CAD PLOT	PLOT	CHECKED BY	JEF	DATE	07/11/14	DRAWN BY	JEF	DATE	07/11/14

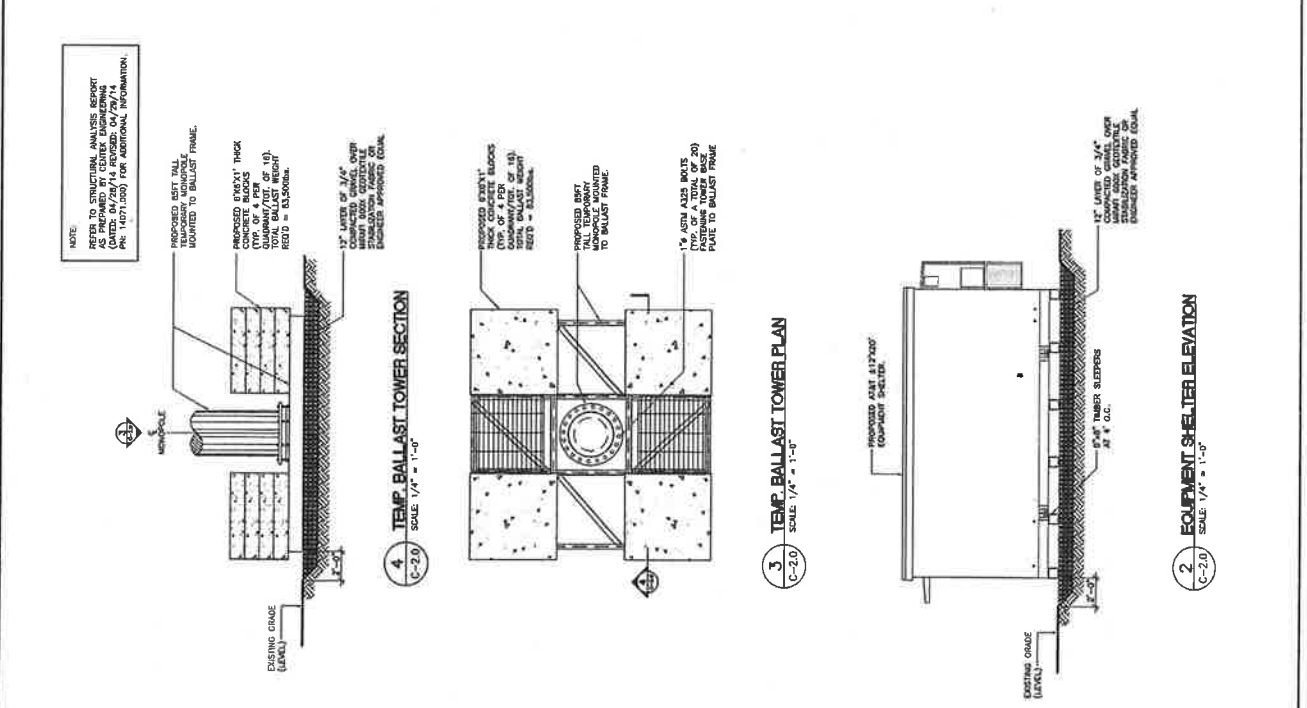
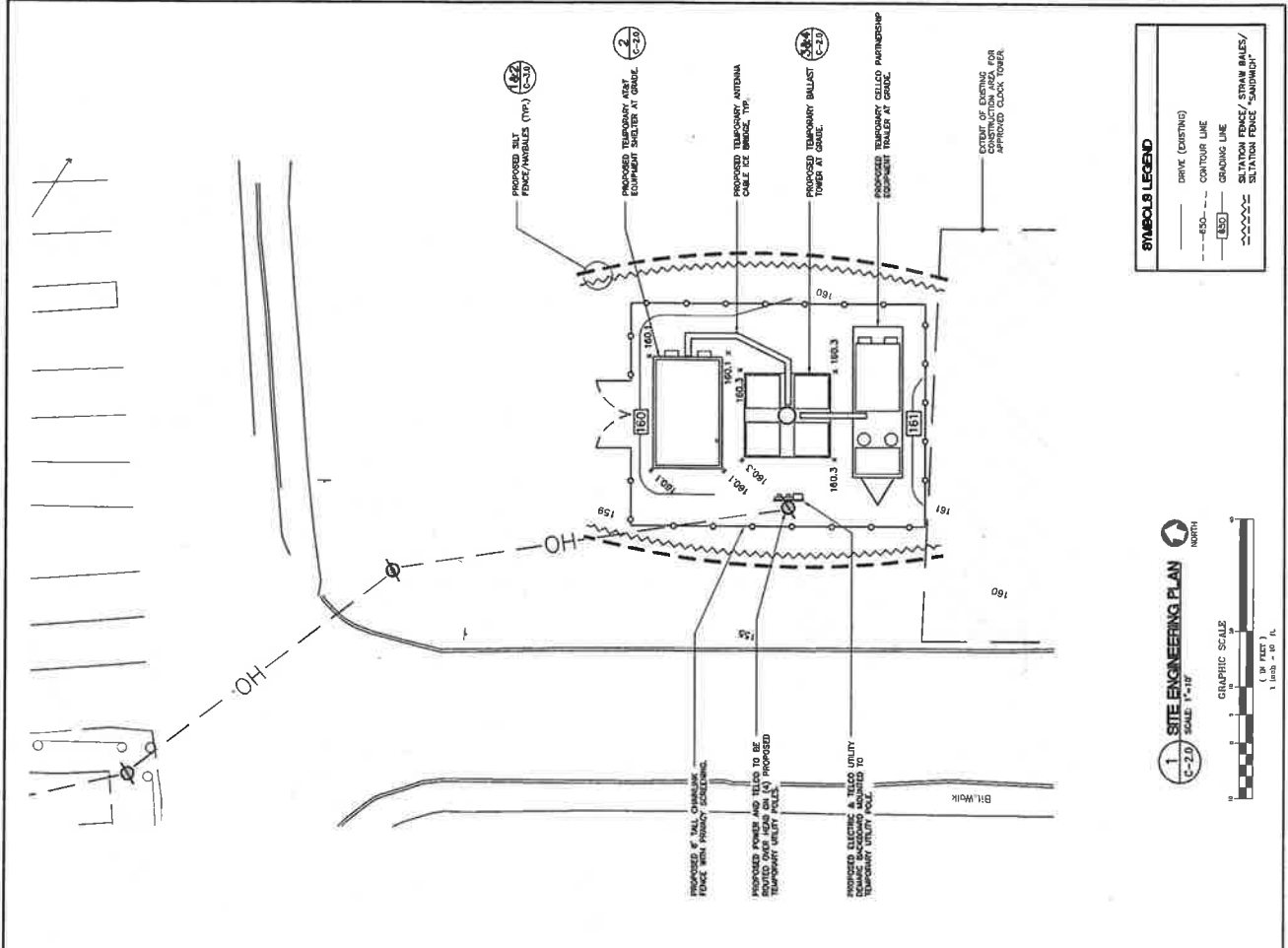
- SYMBOLS LEGEND**
- PROPERTY LINE
 - DRIVE (EXISTING)
 - - - - - CONTOUR LINE
 - ⊗ EXISTING CONSPICUOUS TREE
 - ⊙ EXISTING DECIDUOUS TREE

SITE PLAN SURVEY REFERENCE NOTE:
 EXISTING PROPERTY LINES AND SITE EXPOSURES DETERMINED FROM AERIAL PHOTOGRAPHS, GROUND SURVEY, AND 3-D SURVEY DATA. A GENERAL LOCATIONAL SURVEY WAS CONDUCTED ON 07/07/14 BY PERKINS ASSOCIATES.



INDICATED AREAS AND TREES TO BE MOVED OVER HIGH OR BY PROPOSED TEMPORARY UTILITY POLES.

Calcio Partnership d/b/a Verizon Wireless
WEST HARTFORD WEST
 TEMPORARY TOWER INSTALL
 88 NORTH MAIN STREET
 WEST HARTFORD, CT



NO.	DATE	BY	CHKD.	DESCRIPTION
1	04/29/14	DL	DL	ISSUED FOR PERMITS
2	04/29/14	DL	DL	ISSUED FOR PERMITS
3	04/29/14	DL	DL	ISSUED FOR PERMITS
4	04/29/14	DL	DL	ISSUED FOR PERMITS
5	04/29/14	DL	DL	ISSUED FOR PERMITS
6	04/29/14	DL	DL	ISSUED FOR PERMITS
7	04/29/14	DL	DL	ISSUED FOR PERMITS
8	04/29/14	DL	DL	ISSUED FOR PERMITS
9	04/29/14	DL	DL	ISSUED FOR PERMITS
10	04/29/14	DL	DL	ISSUED FOR PERMITS

GENERAL CONSTRUCTION / PRE-CONSTRUCTION NOTES

1. PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION ACTIVITIES, A MEETINGS ON-SITE PRE-CONSTRUCTION MEETING SHALL BE CONDUCTED WITH THE VERMONT WILDLIFE CONSTRUCTION MANAGER, AT THE CONSTRUCTION MANAGER, LANDSCAPER AND THE OWNER'S OFFICE.
2. THE SOUTHERN PROPERTY USE MANAGER TO THE PROPOSED ACCESS DRIVE IS SHOWN IN RED. THE CONTRACTOR SHALL MAINTAIN THE PROPERTY LINE THRU LOCATING DURING THE DRIVE PERIOD OF CONSTRUCTION. ALL CONSTRUCTION ACTIVITIES SHALL BE CONFINED TO THE SUBJECT PROPERTY.

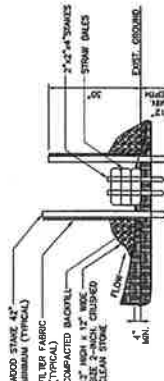
GENERAL CONSTRUCTION SEQUENCE

THIS IS A GENERAL CONSTRUCTION SEQUENCE OUTLINE SOME ITEMS OF WHICH MAY NOT APPLY TO PARTICULAR SITES.

1. CUT AND STUMP AREAS OF PROPOSED CONSTRUCTION.
2. ACTUAL TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES AS REQUIRED.
3. REMOVE AND STOCKPILE TOPSOIL. STOCKPILE SHALL BE SEEDED WITH WHEAT RYE OR ANNUAL RYE TO PREVENT EROSION.
4. CONSTRUCT CLOSED DRAINAGE SYSTEM, PRECEPT COLLECT BULBS AND CATCH BASINS WITH SEDIMENTATION BARRENERS, CONTROL ROADWAYS AND FOOTWAY SHEET PILING, PLACING STRAW BALES AND SILTATION FENCES AS REQUIRED TO CONTROL SOIL EROSION.
5. INSTALL UNDERGROUND UTILITIES.
6. BEGIN TEMPORARY AND PERMANENT SEEDING AND MULCHING. ALL CUT AND FILL SLOPES SHALL BE SEEDED OR MULCHED IMMEDIATELY AFTER THEIR CONSTRUCTION. NO AREA SHALL BE LEFT UNSTABILIZED FOR A TIME PERIOD OF MORE THAN 30 DAYS.
7. BULK, OR AS REQUIRED, CONSTRUCT, INSPECT, AND IF NECESSARY, RECONSTRUCT TEMPORARY BODAS, BODAS, DITCHES, SILT FENCES AND SEDIMENT TRAPS INCLUDING MULCHING AND SEEDING.
8. BEGIN EXCAVATION FOR AND CONSTRUCTION OF TOWERS AND PLATFORMS.
9. FINISH PAVING ALL ROADWAYS, DRIVEWAYS AND PARKING AREAS.
10. COMPLETE PERMANENT SEEDING AND LANDSCAPING. AREA OF DISTURBED SOIL SHALL BE SOWN WITH WHEAT ENRIEAD CONSERVATION/MULCH SEED MIX (NEW ENGLAND WETLAND PLANTS, INC. (413) 248-8680), OR APPROVED EQUIVALENT AT THE MANUFACTURER'S RECOMMENDED SEEDING RATE.
11. RECLAIMED AREAS SHALL BE SEEDING TO ANY METHODS UNTIL A HEALTHY STAND OF GRASS HAS BEEN ESTABLISHED IN ALL SEEDING AREAS. REMOVE ALL TEMPORARY EROSION CONTROL MEASURES.

SOIL EROSION AND SEDIMENT CONTROL SEQUENCE

1. ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES, SUCH AS CONSTRUCTION ENTRANCE / EXIT TRACKING PAD, SILTATION FENCE, AND SILTATION FENCE / STRAW BALE SHALL BE IN PLACE PRIOR TO ANY GRADING ACTIVITY. CONSTRUCTION ENTRANCE / EXIT TRACKING PADS SHALL BE MAINTAINED UNTIL CONSTRUCTION OF THE COMPLETE AND/OR AREA IS ESTABLISHED.
2. THE SILTATION FENCE TO THE PROPOSED SITE IS TO BE PROTECTED BY STONE AND TRACKING PAD IS TO BE MAINTAINED AT ALL TIMES DURING THE CONSTRUCTION PERIOD. THE STONE AND TRACKING PAD IS TO BE MAINTAINED AT ALL TIMES DURING THE CONSTRUCTION PERIOD.
3. THE ENTRANCE TO THE PROPERTY IS TO BE PROTECTED BY STONE AND TRACKING PAD IS TO BE MAINTAINED AT ALL TIMES DURING THE CONSTRUCTION PERIOD.
4. LAND DISTURBANCE WILL BE KEPT TO A MINIMUM AND REVEGETATION WILL BE SCHEDULED AS SOON AS POSSIBLE.
5. CONSTRUCTION AND SEDIMENT CONTROL MEASURES SHALL BE MAINTAINED THROUGHOUT THE CONSTRUCTION PERIOD. MEASURES FOR EROSION AND SEDIMENT CONTROL INCLUDE THE LATEST BEST PRACTICES WITH THE CURRENT SOIL AND WATER CONSERVATION.
6. ANY ADDITIONAL EROSION/SEDIMENTATION CONTROL DEVICES NECESSARY BY TOWN STAFF DURING CONSTRUCTION, REPAIR/REPLACEMENT/MAINTENANCE OF ALL EROSION CONTROL MEASURES UNTIL ALL DISTURBED AREAS ARE STABILIZED TO THE SATISFACTION OF THE TOWN STAFF.
7. IN ALL AREAS, EROSION, OF TREES, BARRIERS AND OTHER MEASURES AS WELL AS DETERMINATION OF THE SOIL IS TO BE MADE IMMEDIATELY AFTER CONSTRUCTION. REPAIR/REPLACEMENT/MAINTENANCE OF ALL EROSION CONTROL MEASURES UNTIL ALL DISTURBED AREAS ARE STABILIZED TO THE SATISFACTION OF THE TOWN STAFF.
8. SILTATION FENCE SHALL BE PLACED AS INDICATED BEFORE A CUT SLOPE HAS BEEN CREATED. SEDIMENT DEPOSITORS SHALL BE INSTALLED AT REGULAR INTERVALS ALONG THE SLOPE. SEDIMENT DEPOSITORS SHALL BE PAINTED OR MARKED WITH HIGHLY VISIBLE ORANGE OR RED. SEDIMENT DEPOSITORS SHALL BE MAINTAINED AS NECESSARY TO PROVIDE PROPER FILTERING ACTION. ABOVE THE EROSION CHECKS ARE STABILIZED AND VEGETATION HAS BEEN ESTABLISHED.
9. SMALL DISCHARGE AREA WILL BE PROTECTED WITH RPP RAY SPLASH PAD/ ENERGY DISSIPATER.
10. ALL FILL AREAS SHALL BE PROTECTED SUFFICIENTLY FOR THEIR INTENDED PURPOSE AND AS REQUIRED TO REDUCE SLIPING, EROSION OR EXCESS SATURATION.
11. THE SOIL SHALL NOT BE PLACED WHILE IN A FROZEN OR MUDDY CONDITION. WHEN THE WINDROCK IS EXCESSIVELY EXPOSED IN A CONDITION THAT WOULD OTHERWISE BE UNDESIRABLE TO PROPER GRADING OR PROPOSED SEEDING OR STABILIZATION.
12. AFTER CONSTRUCTION IS COMPLETE AND GROUND IS STABLE, REMOVE SITS IN THE RPP RAY ENERGY DISSIPATER. REMOVE OTHER EROSION AND SEDIMENT DEVICES.



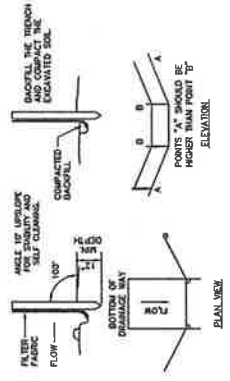
1 SILTATION FENCE/STRAW BALE SILTATION FENCE "SANDWICH" EROSION CONTROL
NOT TO SCALE
C-30

CONSTRUCTION SPECIFICATIONS - SILT FENCE

1. THE GEOTEXTILE FABRIC SHALL MEET THE DESIGN CRITERIA FOR SILT FENCES.
2. THE FABRIC SHALL BE BURIED A MINIMUM OF 8 INCHES INTO THE GROUND AND THE SOIL COMPACTED OVER THE BURIED FABRIC.
3. WOODEN WIRE FENCE SHALL BE FASTENED SECURELY TO THE FENCE POSTS WITH WIRE TIES OR STAPLES.
4. FENCE POSTS SHALL BE FASTENED TO THE WOODEN WIRE FENCE WITH TIES SPACED EVERY 24 INCHES.
5. WHEN TWO SECTIONS OF FENCE MEET AT AN ANGLE, THEY SHALL BE OVERLAPPED BY 8 INCHES, FOLDED, AND STAPLED.
6. FENCE POSTS SHALL BE A MINIMUM OF 36 INCHES LONG AND BURNED A MINIMUM OF 14 INCHES INTO THE GROUND. FENCE POSTS SHALL BE MADE OF HIGH QUALITY HOLLOWWOOD AND SHALL HAVE A MINIMUM CROSS SECTIONAL AREA OF 3.0 SQUARE INCHES.
7. MAINTENANCE SHALL BE PERFORMED AS NEEDED TO PREVENT BUILD UP IN THE SILT FENCE DUE TO DEPOSITION OF SEDIMENT.

MAINTENANCE - SILT FENCE

1. SILT FENCES SHALL BE INSPECTED IMMEDIATELY AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL. ANY DEFECTS THAT ARE REQUIRED SHALL BE MADE IMMEDIATELY.
2. IF THE FABRIC ON A SILT FENCE SHOULD DECOMPOSE OR BECOME DEFECTIVE DURING THE EXPECTED LIFE OF THE FENCE, THE FENCING SHALL BE RE-DESIGNED PROPERLY.
3. SEDIMENT DEPOSITORS SHALL BE MAINTAINED AT ALL TIMES. THE DEPOSITORS SHOULD BE REMOVED WHEN THEY REACHED APPROXIMATELY ONE-HALF THE HEIGHT OF THE BARRIERS.
4. SEDIMENT DEPOSITORS THAT ARE REMOVED OR LEFT IN PLACE AFTER THE FABRIC HAS BEEN REMOVED SHALL BE GRASS TO CONFORM WITH THE EXISTING TOPOGRAPHY AND VEGETATION.



2 PLACEMENT AND CONSTRUCTION OF SILTATION FENCE
NOT TO SCALE
C-30

<p>Colco Partnership d/b/a Vertzon Wreless VERIZON COMMUNICATIONS PROPERTY 190 NORTH MAIN STREET WEST HARTFORD WEST TEMPORARY TOWER INSTALL WEST HARTFORD, CT</p>		<p>DATE: 04/24/24 SCALE: AS SHOWN JOB NO.: 19071400</p>
<p>DATE: 04/24/24 DRAWN BY: CDT CHECKED BY: CDT DESCRIPTION: TOWER PERM PERM - CROSS NUMBER</p>		<p>C-3.0 Sheet No. 3 of 3</p>
<p>CONSTRUCTION NOTES AND DETAILS</p>		<p>SITE CONSTRUCTION GENERAL NOTES AND DETAILS</p>

REV.	DATE	BY	CHK'D	DESCRIPTION
1	04/26/14	CJK	DLA	ISSUED FOR G&S OCCUPATION
0	04/26/14	CJK	DLA	ISSUED FOR G&S OCCUPATION

Coloco Partnership d/b/a Vertzon Wireless
WEST HARTFORD WEST
 WEST HARTFORD STREET
 WEST HARTFORD, CT

TEMPORARY TOWER INSTALL

DATE: 04/14/14
 SCALE: AS NOTED
 JOB NO.: 14071-000

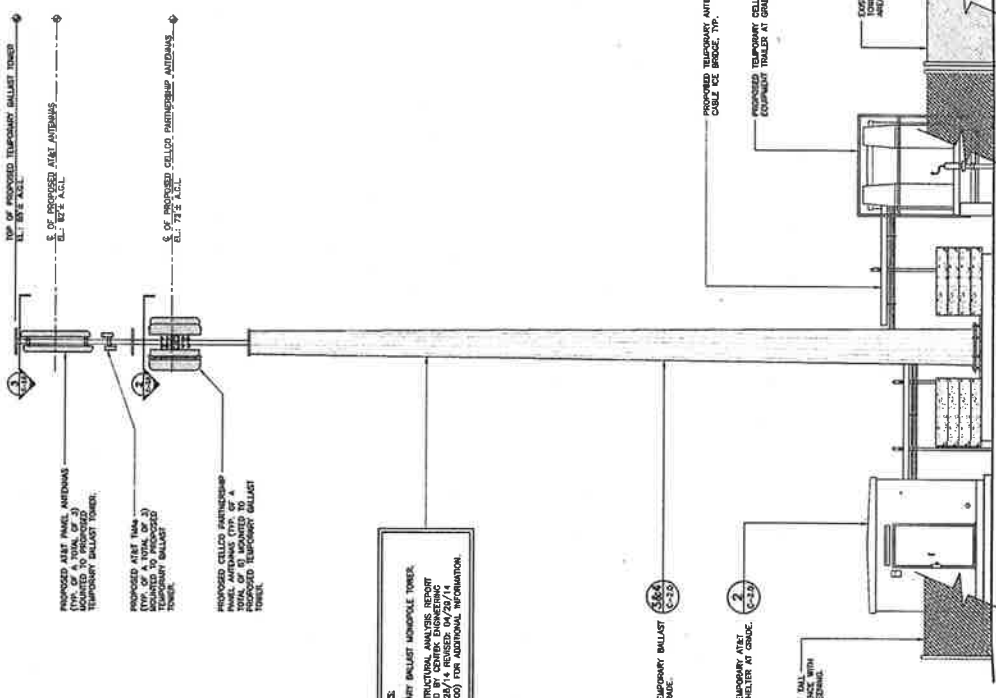
ELEVATION AND ANTENNA CONFIGURATIONS

C-40

Sheet No. 8 of 9

INDEX INDEX:

1. 30' TEMPORARY BALLOAST MONOPOLE TOWER.
 2. REFER TO STRUCTURAL ANALYSIS REPORT FOR STRUCTURAL ANALYSIS OF TOWER (DATED 04/28/14, REVISED 04/28/14, P# 14071-000) FOR ADDITIONAL INFORMATION.



1 EAST ELEVATION
 SCALE 1" = 30'

