Exhibit G

Structural Drawings of Solar Parking Canopies

(Layout, Structure, Elevations, and Renderings)

(including view of underground utilities; protection of catch basins during construction)

TERMS AND ABBREVIATIONS

ABBRV	TERM
(#)	NUMERICAL QUANTITIES WHEN ENCLOSED IN PARENTHESIES
A/E	ARCHITECT/ENGINEER
AB	ANCHOR BOLT
ABC	AGGREGATE BASE COURSE
ARCH	ARCHITECT
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
CBC	CALIFORNIA BUILDING CODE
CIP	CAST-IN-PLACE
CD	CONTRACT DOCUMENTS
CJ	CONSTRUCTION JOINT CONTROL JOINT
CL	CENTERLINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
D	DEPTH
DIA	DIAMETER
DIM	DIMENSION
DL	DEAD LOAD
EA	EACH
EL	ELEVATION
EQ	EQUAL
EXT	EXTERIOR
EW	EACH WAY
(F)	FUTURE
FF	FINISH FLOOR ELEVATION
FLR	FLOOR
FT	FEET
FTG	FOOTING
GA	GAUGE
GALV	GALVANIZED
GC	GENERAL CONTRACTOR
GSN	GENERAL STRUCTURAL NOTES
HORIZ	HORIZONTAL
HSS	HOLLOW STRUCTURAL
I	SECTION MOMENT OF INERTIA
IBC	INTERNATIONAL BUILDING CODE
ID	INSIDE DIAMETER
KIP, K	ONE THOUSAND POUNDS
KLF	KIP PER LINEAR FOOT
L	STEEL ANGLE
LB	POUND
LL	LIVE LOAD
LLBB	LONG LEG BACK TO BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LSH	LONG SIDE HORIZONTAL
LSV	LONG SIDE VERTICAL
MCJ	MASONRY CONTROL JOINTS
MECH	MECHANICAL
MFR	MANUFACTURER
NA	NOT APPLICABLE
NTS	NOT TO SCALE
OC	ON CENTER
PERP	PERPENDICULAR
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
QA	QUALITY ASSURANCE
QC	QUALITY CONTROL
REINF	REINFORCING
REQD	REQUIRED
RFI	REQUEST FOR INFORMATION
SF	SQUARE FOOT
SIM	SIMILAR
SPEC	SPECIFICATION
STD	STANDARD
T&B	TOP AND BOTTOM
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W/C	WATER TO CEMENT RATIO
W/O	WITHOUT
WL	WINDLOAD

CODE:

2015 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC)

DESIGN LOADS:

1. ROOF:
   - LIVE LOAD _____ 0 PSF
   - DEAD LOAD _____ 20 PSF
2. WIND LOAD:
   - RISK CATEGORY _____ I
   - BASIC WIND SPEED, V : _____ 100 MPH
   - EXPOSURE CATEGORY: _____ C
   - IMPORTANCE FACTOR, Iw: _____ 1.0
   - MEAN ROOF HEIGHT: _____ 15 FT
   - G: _____ 0.85
   - Kd: _____ 0.85
   - Kzt: _____ 1.0
   - Kz: _____ 0.85
   - ENCLOSURE CLASSIFICATION: _____ OPEN BUILDING
3. SEISMIC LOADS:
   - RISK CATEGORY _____ I
   - IMPORTANCE FACTOR, Ie : _____ 1.0
   - SEISMIC SITE CLASS: _____ D
   - Ss: _____ 1.5
   - S1: _____ 0.6
   - SDS: _____ 0.9
   - SD1: _____ 0.96
   - SEISMIC DESIGN CATEGORY: _____ D
   - BASIC SEISMIC FORCE RESISTING SYSTEM: STEEL SPECIAL CANTILEVER COLUMN SYSTEMS
   - R: _____ 2.5
   - Ω: _____ 1.25
   - Cd: _____ 2.5
   - Cs: _____ 0.36
   - BASE SHEAR, V: _____ 0.36W

GENERAL:

1. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES.
2. THE CONTRACTOR IS RESPONSIBLE FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK THAT CONFORMS TO THE REGULATIONS OF THE OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) SAFETY AND HEALTH STANDARDS FOR THE CONSTRUCTION INDUSTRY.
3. WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDUM.
4. OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. HE SHALL BE RESPONSIBLE FOR ALL CHANGES NECESSARY IF HE CHOOSES AN OPTION AND HE SHALL COORDINATE ALL DETAILS.
5. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT.
6. TYPICAL DETAILS ARE NOT CUT ON DRAWINGS, BUT APPLY UNLESS NOTED OTHERWISE.
7. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ACTUAL SITE CONDITIONS AND GENERAL CONTRACTOR PRIOR TO START OF CONSTRUCTION. ALL DIMENSIONS SHOWN ON STRUCTURAL DRAWINGS ARE TO ASSIST CONTRACTOR IN VERIFICATION. DO NOT SCALE DIMENSIONS FROM DRAWINGS.
8. ITEMS SHOWN BY OTHER DISCIPLINES WITH REFERENCE TO STRUCTURAL DRAWINGS BUT NOT SHOWN ON THESE STRUCTURAL DRAWINGS SHALL BE CONSIDERED DESIGN BUILD ITEMS. CONTRACTOR SHALL SUBMIT DESIGN BY OTHERS FOR REVIEW

FOUNDATIONS:

1. GEOTECHNICAL CONSULTANT:
2. REPORT NUMBER:
3. REPORT DATE:
4. DESIGN SOIL BEARING VALUES WERE ASSUMED IN ACCORDANCE WITH SOIL CLASS 5 AS DEFINED IN IBC/CBC TABLE 1806.2 "PRESUMPTIVE LOAD-BEARING VALUES" DESIGN BEARING VALUE OF 1,500 PSF AND LATERAL BEARING VALUE OF 100 PSF/FT WAS USED IN DESIGN. IF ACTUAL SOIL CONDITIONS DIFFER NOTIFY THE STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH WORK.
5. SPREAD FOOTINGS SHALL BEAR ON COMPACTED NATIVE SOILS. BOTTOM OF FOOTINGS SHALL BEAR AT A DEPTH NOT LESS THAN 2.0 FT BELOW LOWEST ADJACENT GRADE WITHIN 5 FEET OF STRUCTURE OR FOUNDATION. FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
6. DRILLED POLE FOUNDATIONS SHALL BEAR ON MACHINE CLEANED, INSPECTED SOIL STRATA. POLE FOUNDATIONS WERE DESIGNED IN ACCORDANCE WITH THE PRESCRIPTIVE METHOD OF IBC/CBC SECTION 1807.3.2. FOR TOP OF POLE FOUNDATION ELEVATIONS, SEE FOUNDATION PLANS AND SECTIONS. IF WATER IS ENCOUNTERED DURING DRILLING, STOP AND CONSULT STRUCTURAL ENGINEER OR GEOTECHNICAL ENGINEER FOR RESOLUTION.

1. SPREAD FOOTINGS SHALL BEAR ON COMPACTED FILL. FOR FILL REQUIREMENTS, SEE SOIL REPORT. DESIGN SOIL BEARING VALUE 1,500 PSF. BOTTOM OF FOOTINGS TO BE 2'-0" MINIMUM BELOW FINISHED GRADE. FINISHED GRADE IS DEFINED AS TOP OF SLAB FOR INTERIOR FOOTINGS AND LOWEST ADJACENT FINISHED GRADE WITHIN 5 FEET FOR PERIMETER FOOTINGS. FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
2. DRILLED POLE FOUNDATIONS SHALL BEAR ON MACHINE CLEANED, INSPECTED SOIL STRATA. DESIGN LATERAL SOIL BEARING VALUE OF 100 PSF/FT WAS USED IN DESIGN. POLE FOUNDATIONS WERE DESIGNED IN ACCORDANCE WITH THE PRESCRIPTIVE METHOD OF IBC/CBC SECTION 1807.3.2. FOR TOP OF POLE FOUNDATION ELEVATIONS, SEE FOUNDATION PLANS AND SECTIONS. IF WATER IS ENCOUNTERED DURING DRILLING, STOP AND CONSULT STRUCTURAL ENGINEER OR GEOTECHNICAL ENGINEER FOR RESOLUTION.

SHOP DRAWINGS:

1. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS AND ITEMS REQUIRED BY ARCHITECTURAL SPECIFICATIONS. UNITED STRUCTURAL DESIGN, LLC. ASSUMES NO RESPONSIBILITY FOR THE FAILURE OF THE CONTRACTOR TO SUBMIT SHOP DRAWINGS FOR REVIEW.
2. ITEMS NOT IN ACCORDANCE WITH CONTRACT DOCUMENTS SHALL BE FLAGGED UPON CONTRACTORS REVIEW
3. THE CONSTRUCTION DOCUMENTS MAY NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS.
4. ELECTRONIC FILES OF CONSTRUCTION DOCUMENTS WILL NOT BE MADE AVAILABLE FOR USE AS SHOP DRAWINGS.
5. FIELD VERIFY ALL DIMENSIONS AND FINISHED GRADE PRIOR TO CONSTRUCTION AND PRIOR TO BEGINNING SHOP DRAWINGS.
6. THE ENGINEER OF RECORD HAS THE RIGHT TO APPROVE OR DISAPPROVE ANY CHANGES TO CONTRACT DOCUMENTS AT ANYTIME BEFORE OR AFTER SHOP DRAWING REVIEW.
7. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT SHALL NOT BE CONSIDERED CHANGES TO THE CONTRACT DOCUMENTS.
8. SHOP DRAWINGS DO NOT REPLACE THE CONTRACT DOCUMENTS. REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE ALL ITEMS ARE CONSTRUCTED ACCORDING TO THE CONTRACT DOCUMENTS.

CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301, "STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE" AND ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".
2. ADDITION OF WATER TO THE BATCH FOR MATERIAL WITH INSUFFICIENT SLUMP WILL NOT BE PERMITTED, UNLESS THE SUPPLIER HAS SPECIFICALLY WITHHELD WATER FROM THE BATCH AT THE PLANT. IN SUCH CASE THE MIX DESIGN AND TRUCK TICKET MUST CLEARLY STATE THE MAXIMUM AMOUNT OF WATER THAT CAN BE ADDED TO THE BATCH ON SITE. IN NO CASE SHALL THE DESIGN WATER TO CEMENTITIOUS MATERIAL RATIO BE EXCEEDED.
3. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED, EXCEPT THAT SLABS ON GRADE NEED BE VIBRATED ONLY AROUND SLAB EDGES, REINFORCING, AND COLUMNS. MECHANICALLY VIBRATE ONLY THE TOP 5 FEET OF DRILLED PIER CONCRETE. REVIBRATE TOP OF DRILLED PIER 15 MINUTES AFTER PLACING CONCRETE.
4. TEST DATA FOR CONCRETE SUBMITTALS SHALL BE SUBMITTED FOR REVIEW PRIOR TO PLACEMENT OF CONCRETE. REFERENCE ACI 318 CHAPTER 5, TABLE 5.3 FOR SPECIFIC REQUIREMENTS.
5. DRILLED PIER CONCRETE SHALL BE CHANNELED TO FREE FALL DOWN THE SHAFT WITHOUT STRIKING THE REINFORCING OR THE SIDES OF THE SHAFT. MAXIMUM HEIGHT OF FREE-FALL IS 15'-0".
6. CONCRETE PROPERTIES:

CONCRETE USE	MINIMUM 28 DAY COMPRESSIVE STRENGTH
UNLESS NOTED OTHERWISE ALL CONCRETE SHALL BE	2,500 PSI

PHOTOVOLTAIC PANELS:

1. THE PANEL MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF THE PANELS AND THE DESIGN OF THE PANEL CONNECTIONS TO THE STRUCTURE INCLUDING ALL COMPONENTS REQUIRED TO MAKE THE CONNECTIONS. PHOTOVOLTAIC PANELS, COMPONENTS AND CONNECTIONS SHALL BE DESIGNED TO SUPPORT PANEL WEIGHT PLUS SNOW, WIND, OR SEISMIC LOADING, WHICHEVER COMBINATION PRODUCES THE MOST SEVERE CONDITION IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE.
2. OWNER TO PROVIDE PANEL CAPABLE OF SUPPORTING IN MANOR IN WHICH IS INTENDED BY THESE DRAWINGS I.E. SUPPORTED BY SHORT END, DUAL SUPPORTS, ETC). SUBMIT PANEL SPEC SHEETS FOR REVIEW PRIOR TO PURCHASING ANY PANELS.
3. CONTRACTOR TO VERIFY PV PANELS WITH OWNER PRIOR TO FABRICATION.
4. THIS IS A DEFERRED SUBMITTAL ITEM.

STRUCTURAL STEEL:

1. LATEST AISC AND AWS CODES APPLY. THE WORD APPROVED INSPECTION 4.4 OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES IS REDEFINED AS REVIEWED.
2. STEEL SHALL BE FINISHED AT LOCATIONS EXPOSED TO WEATHER WITH A CORROSION RESISTANT COATING APPLICABLE TO WEATHER AND EXPOSURE CONDITIONS OF PROJECT LOCATION.
3. WHEN STRUCTURAL STEEL IS FURNISHED TO A SPECIFIED MINIMUM YIELD POINT GREATER THAN 36 KSI, THE ASTM OR OTHER SPECIFICATION DESIGNATION SHALL BE INCLUDED NEAR THE ERECTION MARK ON EACH SHIPPING ASSEMBLY OR IMPORTANT CONSTRUCTION COMPONENT OVER ANY SHOP COAT OF PAINT PRIOR TO SHIPMENT FROM THE FABRICATORS PLANT.
4. IF IT IS NECESSARY TO SPLICE ANY MEMBER, SPLICE LOCATIONS ARE SUBJECT TO REVIEW BY STRUCTURAL ENGINEER. SPLICES SHALL BE FULL PENETRATION WELDED AND TESTED PER THIS SECTION. INDICATE ALL SPLICE LOCATIONS, AND WELDING PROCEDURES ON SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION.
5. ALL BEAMS SHALL BE ERECTED WITH THE NATURAL CAMBER UPWARDS.
6. ALL BOLTS SHALL BE INSTALLED WITH STEEL WASHERS.
7. ALL WELDING BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN INDEPENDENT TESTING AGENCY.
8. ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS. USE E90 SERIES FOR ASTM A706 REINFORCING BARS.
9. ALL WELDING PER AMERICAN WELDING SOCIETY STANDARDS. ALL WELDS ON DRAWINGS ARE SHOWN AS SHOP WELDS. CONTRACTOR MAY SHOP WELD OR FIELD WELD AT THEIR DISCRETION. SHOP WELDS OR FIELD WELDS SHALL BE SHOWN ON SHOP DRAWINGS.
10. SLAG SHALL BE REMOVED FROM ALL COMPLETED WELDS, AND THE WELD AND ADJACENT BASE METAL SHALL BE CLEANED BY BRUSHING OR OTHER SUITABLE MEANS. WELDED JOINTS SHALL NOT BE PAINTED UNTIL AFTER WELDING HAS BEEN COMPLETED AND THE WELD ACCEPTED.
11. ALL STRUCTURAL STEEL SHALL BE FABRICATED BY A FABRICATOR WITH ANY ONE OF THE FOLLOWING MINIMUM QUALIFICATIONS. QUALIFICATIONS SHALL BE IN EFFECT AT TIME OF BID.
    - AISC CERTIFIED FABRICATOR (STD).
12. STEEL PROPERTIES
    - WIDE FLANGE COLUMNS, BEAMS AND TEES: ASTM A992 (Fy = 50 KSI)
    - STEEL PLATES: ASTM A572 (Fy = 50 KSI)
    - CHANNELS AND ANGLES: ASTM A36 (Fy = 36 KSI)
    - HSS RECTANGULAR STEEL: ASTM A500 Gr. B (Fy = 46 KSI)
    - BOLTS: ASTM A325 OR ASTM A F1852 TWIST-OFF TYPE
    - ANCHOR RODS: ASTM F1554 Gr. 55 (Fy = 55 KSI)
13. STEEL BOLTS SHALL BE PRETENSIONED UNLESS OTHERWISE NOTED AS A SNUG-TIGHT CONNECTION ON THE DRAWINGS OR DETAILS. ONE OF THE FOLLOWING METHODS SHALL BE USED TO ASSURE ADEQUATE PRETENSIONING IS ACHIEVED:
    - TURN-OF-NUT METHOD
    - DIRECT TENSION INDICATOR WASHERS
    - CALIBRATED WRENCH
    - TWIST-OFF TYPE BOLT

STEEL REINFORCING:

1. ALL BARS PER CRSI SPECIFICATIONS AND HANDBOOK. LATEST ACI CODE AND DETAILING MANUAL APPLY. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE. REINFORCING BAR SPACINGS GIVEN ARE MAXIMUM ON CENTERS.
2. ALL REINFORCING TO BE WELDED SHALL BE WELDED IN ACCORDANCE WITH AWS D1.4. NO TACK WELDING OF REINFORCING BARS IS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE BY STRUCTURAL ENGINEER.
3. REINFORCING LAP SPLICES IN CONCRETE SHALL BE PER TYPICAL DETAIL UNLESS NOTED OTHERWISE. ALL SPLICE LOCATIONS ARE SUBJECT TO APPROVAL. PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT CORNERS AND INTERSECTIONS OF FOOTINGS AND WALLS.
4. TYPICAL REINFORCING BAR STRENGTHS
   - REINFORCING (WELDABLE): ASTM A706, DEFORMED, Fy = 60 KSI
5. TYPICAL CLEAR CONCRETE COVERAGES
   - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
   - FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:
     #6 AND LARGER: 2"
     #5 AND SMALLER: 1 1/2"

ALL OTHERS PER LATEST EDITION OF ACI 318.

COLD-FORMED STEEL FRAMING:

1. ALL COLD-FORMED STEEL FRAMING SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS BY THE AMERICAN IRON AND STEEL INSTITUTE AND THE STEEL STUD MANUFACTURERS ASSOCIATION AND ICC ESR-3064P).
2. STEEL FOR ALL MEMBERS AND FOR ALL STRAPS SHALL HAVE A MINIMUM YIELD STRENGTH OF 55,000 PSI.
3. STEEL SHALL BE GALVANIZED AT LOCATIONS EXPOSED TO WEATHER AND WHENEVER NOTED ON THE DRAWINGS.
4. ALL MEMBERS SHALL BE SECURELY SEATED FOR FULL BEARING UNLESS NOTED OTHERWISE.
5. ALL WELDING SHALL BE PERFORMED BY WELDERS EXPERIENCED IN LIGHT GAGE STEEL FRAMING WORK.
6. ALL SCREWS REFERENCED IN THE DRAWINGS FOR LIGHT GAUGE CONNECTIONS SHALL BE DRIL-FLEX BY HILTI OR APPROVED EQUIVALENT (ICC ESR-3332).
7. STEEL STUD SIZES ARE AS INDICATED IN PLANS AND KEYNOTES. THICKNESSES REFERENCED IN THE DRAWINGS ARE AS FOLLOWS:
   - 16 GAUGE MATERIAL - 0.059 INCHES
   - 14 GAUGE MATERIAL - 0.075 INCHES
   - 12 GAUGE MATERIAL - 0.105 INCHES
   - 10 GAUGE MATERIAL - 0.134 INCHES

NOTE: THE UNCOATED MINIMUM STEEL THICKNESS OF THE COLD-FORMED STEEL PRODUCTS AS DELIVERED TO THE JOB SITE SHALL NOT AT ANY LOCATION BE LESS THAN 95 PERCENT OF THE DESIGN THICKNESS INDICATED ABOVE.

1704.2.5 SPECIAL INSPECTION OF FABRICATORS:

SPECIAL INSPECTION OF FABRICATION OF STRUCTURAL STEEL BEING PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP IS REQUIRED.

EXCEPTION: SPECIAL INSPECTIONS OF FABRICATORS WITH ONE OF THE FOLLOWING QUALIFICATIONS IS NOT REQUIRED:
- INTERNATIONAL ACCREDITATION SERVICE, INC. (IAS)APPROVED FABRICATOR.
- AISC CERTIFIED FABRICATOR (STD).

THE SPECIAL INSPECTOR SHALL VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL REVIEW THE PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO THE CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK.

SPECIAL STRUCTURAL INSPECTIONS:

PER IBC SECTION 1704 AND 1705 SPECIAL INSPECTIONS ARE IN ADDITION TO THE REQUIRED INSPECTION CONDUCTED BY THE BUILDING JURISDICTION PER IBC SECTION 110.
THE TYPES OF WORK LISTED BELOW SHALL BE INSPECTED BY A SPECIAL INSPECTOR.

1. ALL SPECIAL INSPECTORS SHALL BE UNDER THE SUPERVISION OF A REGISTERED CIVIL OR STRUCTURAL ENGINEER.
2. THE QUALIFICATIONS OF ALL SPECIAL INSPECTORS SHALL BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
3. THE MINIMUM QUALIFICATIONS FOR THE SPECIAL INSPECTORS ARE AS FOLLOWS:
   - CONCRETE INSPECTION - I.C.C. CERTIFICATION IN REINFORCED CONCRETE OR E.I.T. CERTIFICATION.
   - STRUCTURAL WELDING INSPECTION
   - A. VISUAL TESTING - I.C.C. CERTIFICATION IN STRUCTURAL STEEL AND WELDING OR A.W.S. CERTIFIED WELD INSPECTOR (C.W.I.).
   - B. NON-DESTRUCTIVE TESTING - A.W.S. C.W.I.
   - HIGH STRENGTH BOLTING INSPECTION - I.C.C. CERTIFICATION IN STRUCTURAL STEEL AND WELDING.
   - SPECIAL CASES - EXPERIENCE ACCEPTABLE TO THE STRUCTURAL ENGINEER OF RECORD.
4. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:
   - THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK REQUIRING SPECIAL INSPECTION FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS.
   - THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO BE KEPT AT THE SITE FOR USE BY THE BUILDING OFFICIAL, THE CONTRACTOR, THE STRUCTURAL ENGINEER OF RECORD, AND THE ARCHITECT OF RECORD. IF SPECIAL INSPECTION IS PROVIDED BY ANYONE OTHER THAN THE STRUCTURAL ENGINEER OF RECORD, INSPECTION REPORTS SHALL BE SUBMITTED TO THE OFFICE OF THE STRUCTURAL ENGINEER ON A WEEKLY BASIS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN IF UNCORRECTED, TO THE DESIGN AUTHORITY AND THE BUILDING OFFICIAL.
   - UPON COMPLETION OF THE ASSIGNED WORK, THE SPECIAL INSPECTOR SHALL COMPLETE AND SIGN A FINAL REPORT CERTIFYING THAT TO THE BEST OF HIS KNOWLEDGE, THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.
5. DUTIES AND RESPONSIBILITIES OF THE CONTRACTOR:
   - NOTIFY THE RESPONSIBLE INSPECTOR THAT WORK IS READY FOR INSPECTION AT LEAST ONE WORKING DAY (24 HOURS MINIMUM) BEFORE SUCH INSPECTION IS REQUIRED.
   - ALL WORK REQUIRING SPECIAL STRUCTURAL INSPECTION SHALL REMAIN ACCESSIBLE AND EXPOSED UNTIL IT IS OBSERVED BY THE SPECIAL STRUCTURAL INSPECTOR.
6. SPECIAL INSPECTION
   - INSPECTION OF FABRICATORS
   - INSPECTION OF CONCRETE CONSTRUCTION
   - INSPECTION OF STRUCTURAL STEEL
   - INSPECTION OF SOILS

SEE TABLES ON GSN FOR ADDITIONAL INFORMATION.

1705.6 SPECIAL INSPECTION OF SOILS

SPECIAL INSPECTION FOR EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND LOAD-BEARING REQUIREMENTS SHALL BE AS REQUIRED BY TABLE 1705.6.

TABLE 1705.6: REQUIRED VERIFICATION AND INSPECTION OF SOILS

VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	—	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	—	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	—	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	—
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	—	X

1705.3 SPECIAL INSPECTION OF CONCRETE CONSTRUCTION

SPECIAL INSPECTION AND VERIFICATIONS FOR CONCRETE CONSTRUCTION SHALL BE AS REQUIRED BY TABLE 1705.3.

EXCEPTIONS: SPECIAL INSPECTIONS SHALL NOT BE REQUIRED FOR:
1. ISOLATED SPREAD CONCRETE FOOTINGS OF BUILDING THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK.
2. CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF BUILDINGS THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK WHERE:
   - THE FOOTINGS SUPPORT WALLS OF LIGHT-FRAME CONSTRUCTION;
   - THE STRUCTURAL DESIGN OF THE FOOTING IS BASED ON A SPECIFIED COMPRESSIVE STRENGTH, f'c, NO GREATER THAN 2,500 PSI REGARDLESS OF THE COMPRESSIVE STRENGTH SPECIFIED.
3. CONCRETE SLABS ON GRADE. STEEL REINFORCING STILL REQUIRES SPECIAL INSPECTION..

TABLE 1705.3: REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCE STANDARD	IBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	—	X	ACI 318: Ch. 20, 25.2, 25.3, 26.5.1-26.5.3	1908.4
2. REINFORCING BAR WELDING. a. VERIFY WELDABILITY OF REINFORCING BARS. b. INSPECT SINGLE PASS FILLET WELDS, MAXIMUM 5/16". c. INSPECT ALL OTHER WELDS.	— — X	— X X	AWS D1.4 ACI 318: 26.5.4	—
5. VERIFYING USE OF REQUIRED DESIGN MIX.	—	X	ACI 318: Ch 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	—	ACI 318: 26.4.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	—	X	ACI 318: 26.4.7-26.4.9	1908.9
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	—	X	ACI 318:26.10.1 (b)	—

Sheet List	
Sheet Number	Sheet Name
S0.1	GENERAL STRUCTURAL NOTES
S1.0	OVERALL COLUMN LAYOUT
S1.1	OVERALL STRUCTURES LAYOUT
S3.1	SECTIONS AND RENDERINGS
S4.1	DETAILS

UNITED STRUCTURAL DESIGN LLC
PHOENIX, AZ
(602) 888-1143
www.unitedstr.com

PRELIMINARY NOT FOR CONSTRUCTION

Skyline SOLAR AZ
A DIVISION OF SKYLINE ENTERPRISES

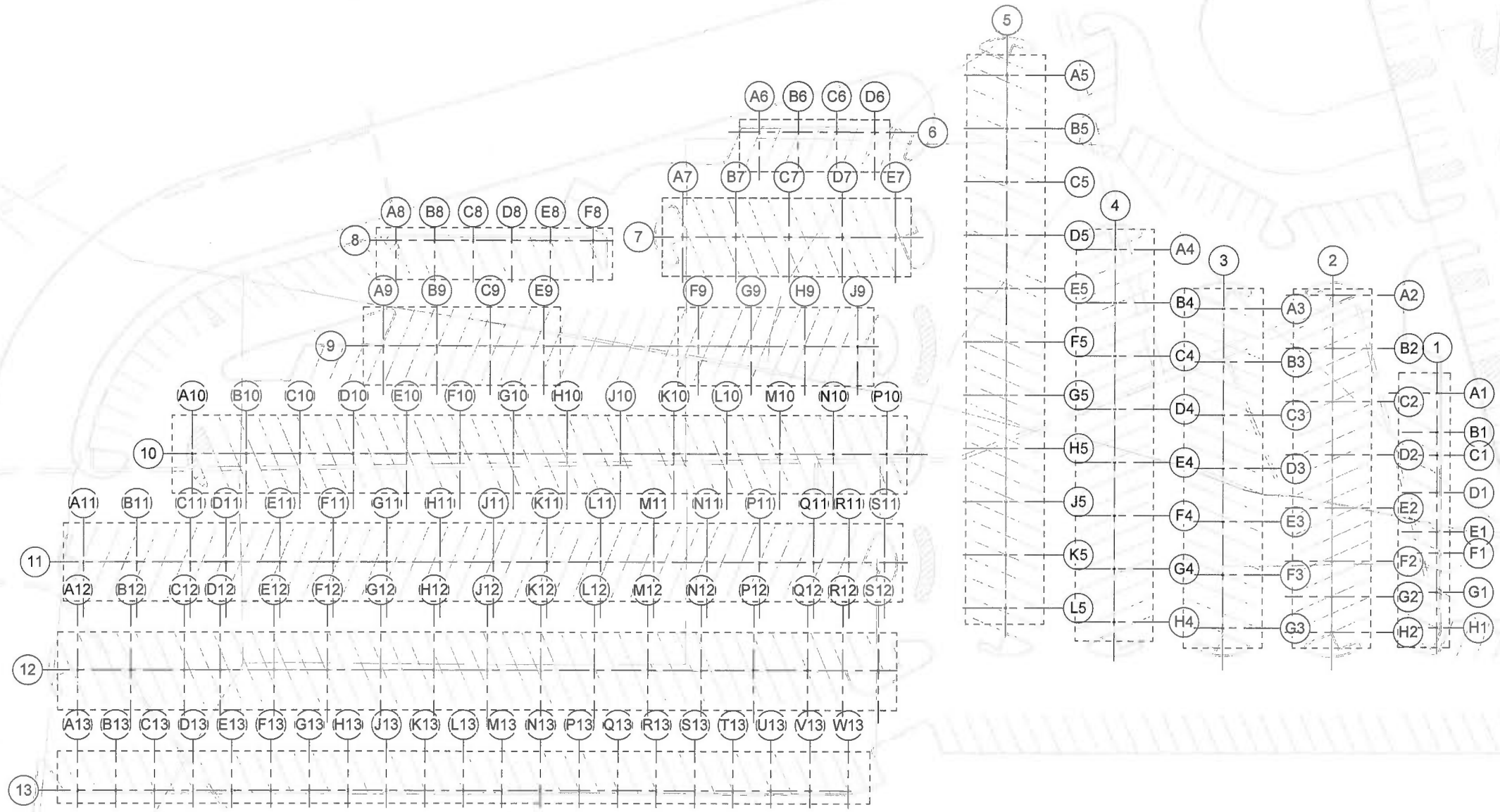
Taubman Westfarms Mall
West Hartford, CT 06110

No.	Description	Date

PROJECT NUMBER:	18001.005
DRAWN BY:	DB
CHECKED BY:	JE
DATE:	06/13/2018

SHEET NAME
GENERAL STRUCTURAL NOTES

S0.1



1 OVERALL LAYOUT - COLUMN LOCATIONS (FIELD VERIFY PRIOR TO CONSTRUCTION)
NO SCALE

UNITED
STRUCTURAL DESIGN LLC
PHOENIX, AZ
(602) 888-1143
www.unitedstr.com

PRELIMINARY NOT FOR CONSTRUCTION

Sterling SOLAR

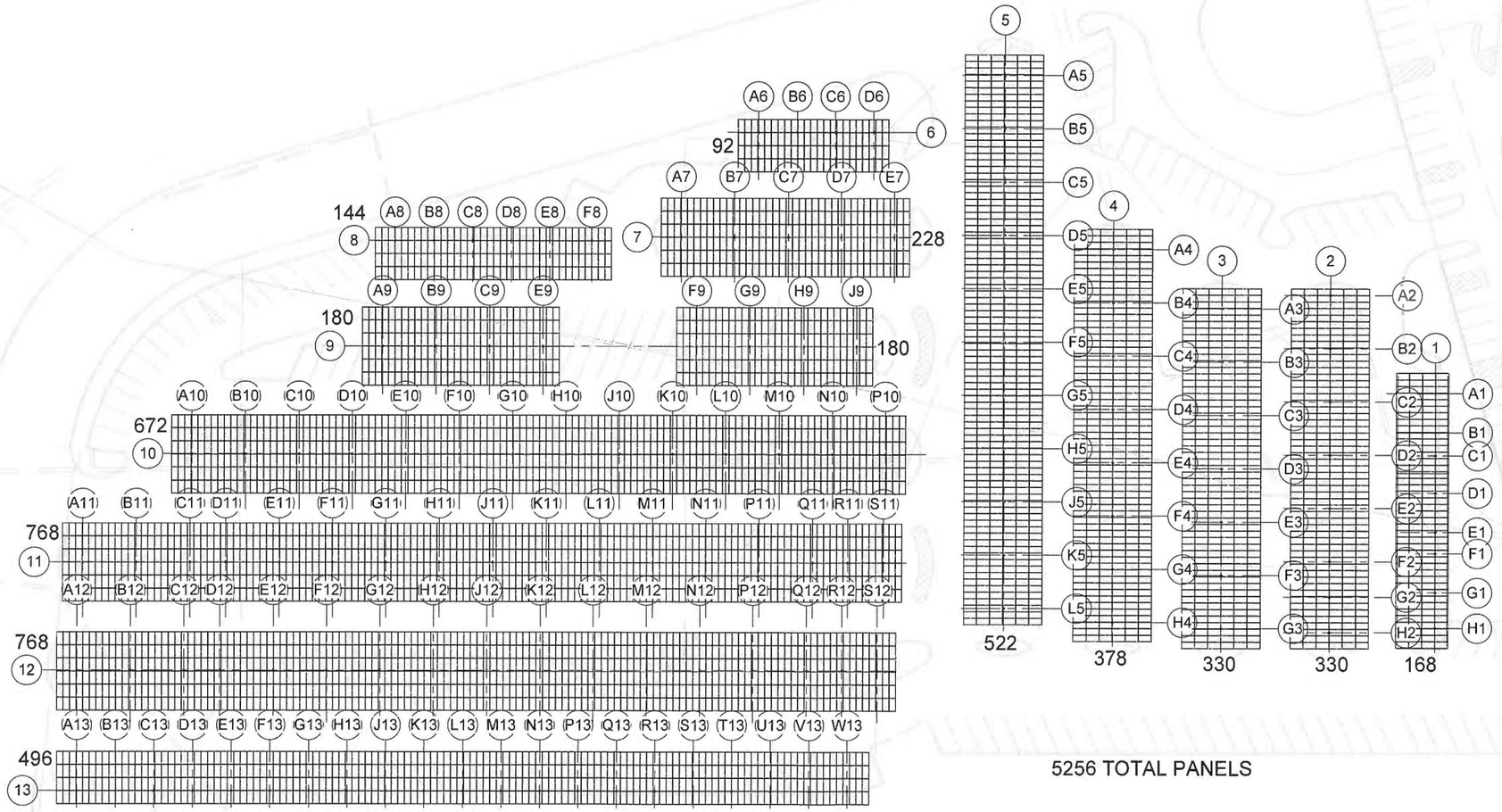
Taubman Westfarms Mall
West Hartford, CT 06110

No.	Description	Date

PROJECT NUMBER:	16001.005
DRAWN BY:	DB
CHECKED BY:	JE
DATE:	06/13/2018

SHEET NAME
OVERALL COLUMN LAYOUT

S1.0



1 OVERALL LAYOUT - STRUCTURES
1" = 30'-0"

UNITED
STRUCTURAL DESIGN LLC
PHOENIX, AZ
(602) 888-1143
www.unitedstr.com

PRELIMINARY NOT FOR CONSTRUCTION

Skyline SOLAR

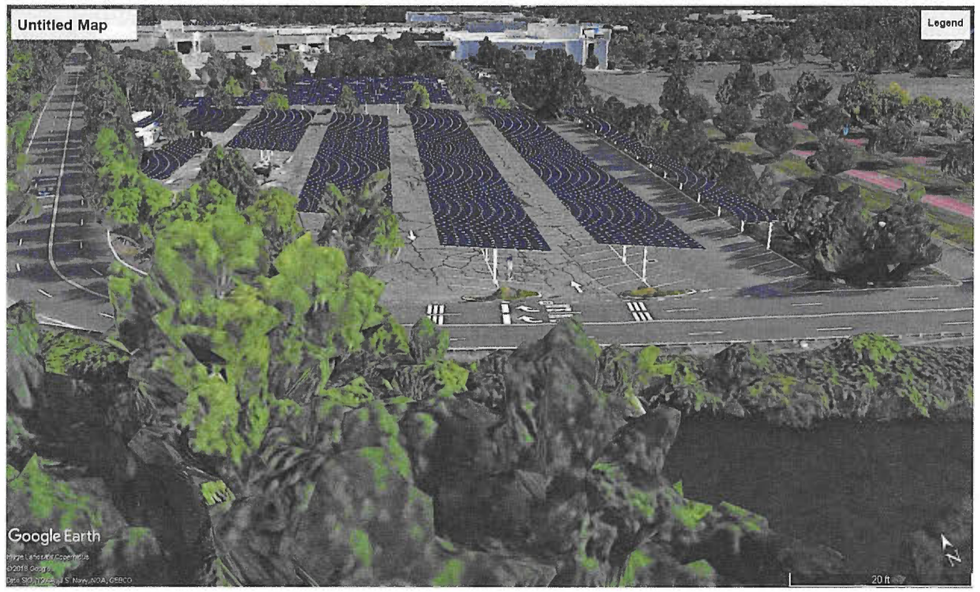
Taubman Westfarms Mall
West Hartford, CT 06110

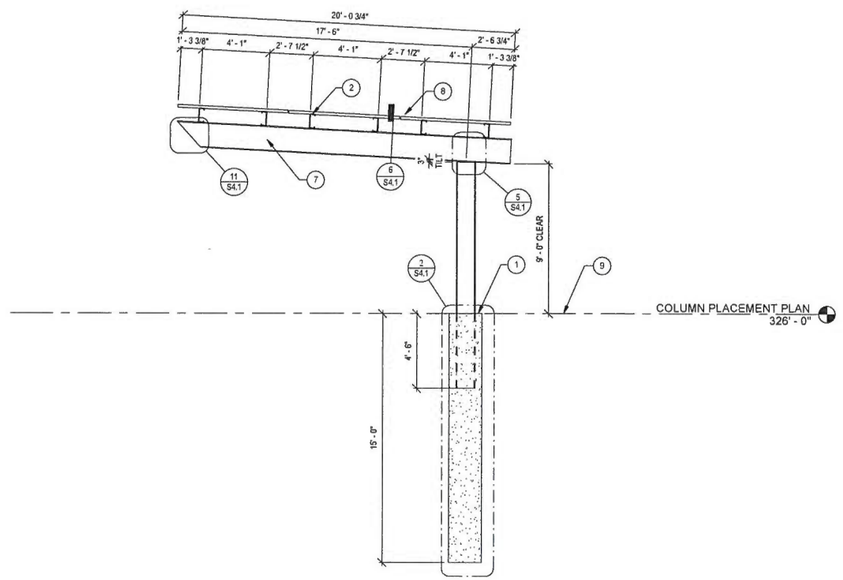
No.	Description	Date

PROJECT NUMBER:	18001.005
DRAWN BY:	DB
CHECKED BY:	JE
DATE:	06/13/2018

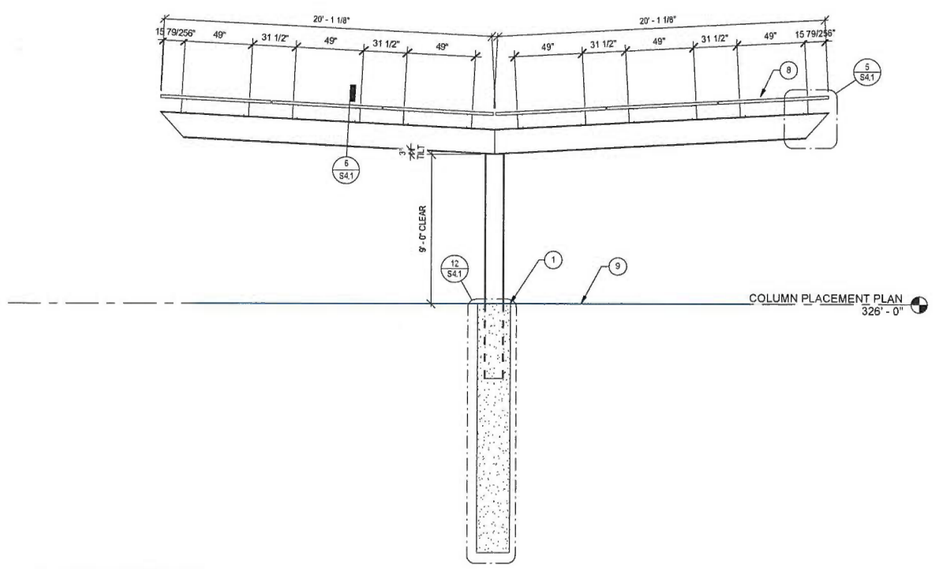
SHEET NAME
OVERALL STRUCTURES LAYOUT

S1.1





1 3 PANEL STRUCTURE SECTION
1/2" SCALE



2 6 PANEL STRUCTURE SECTION
1/2" SCALE

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PRELIMINARY NOT FOR CONSTRUCTION

Skyline SOLAR of Arizona
A Division of Skyline Steel, Inc.

Taubman Westfarms Mall

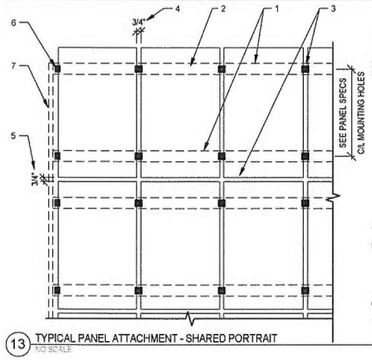
West Hartford, CT 06110

No.	Description	Date

PROJECT NUMBER:	18001.005
DRAWN BY:	DB
CHECKED BY:	JE
DATE:	06/13/2018

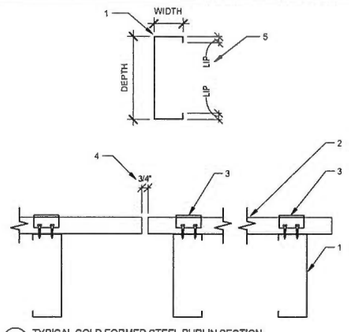
SHEET NAME
SECTIONS AND RENDERINGS

S3.1



1. STEEL PURLIN, SEE PLANS AND GSN FOR SIZE, GAUGE AND FINISH.
2. SOLAR PANELS BY OTHERS.
3. "SKYGRIP" MID CLAMP ALUMINUM BRACKET BY SKYLINE STEEL ATTACHED TO 14 GAUGE THICK PURLINS OR THICKER WITH (2) #12-14 DRIL-FLEX SELF-DRILLING SCREWS BY HILTI (ICC ESR-3332). INSTALL PER MANUFACTURER SPECIFICATIONS. DO NOT OVER TIGHTEN SCREWS WHEN INSTALLING. ANY SCREWS THAT ARE STRIPPED DURING INSTALLATION SHALL BE REMOVED AND REINSTALLED IN NEW HOLES. (4) MINIMUM PER PANEL.
4. SPACING BETWEEN LONG SIDE OF PANELS, TYPICAL.
5. SPACING BETWEEN SHORT SIDE OF PANELS, TYPICAL. CONTRACTOR TO VERIFY PURLIN SPACING WITH PANEL AND BRACKET SPECIFICATIONS PRIOR TO INSTALLATION.
6. SKYGRIP END CLAMP ON ENDS ONLY.
7. END CAP.

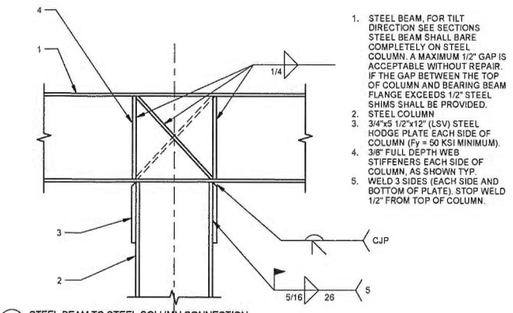
13 TYPICAL PANEL ATTACHMENT - SHARED PORTRAIT
NO SCALE



1. STEEL PURLIN, SEE PLANS AND GSN FOR SIZE, GAUGE AND FINISH.
2. SOLAR PANELS BY OTHERS.
3. "SKYGRIP" ALUMINUM BRACKET BY SKYLINE STEEL ATTACHED TO 14 GAUGE THICK PURLINS OR THICKER WITH (2) #12-14 DRIL-FLEX SELF-DRILLING SCREWS BY HILTI (ICC ESR-3332). INSTALL PER MANUFACTURER SPECIFICATIONS. DO NOT OVER TIGHTEN SCREWS WHEN INSTALLING. ANY SCREWS THAT ARE STRIPPED DURING INSTALLATION SHALL BE REMOVED AND REINSTALLED IN NEW HOLES. (4) MINIMUM PER PANEL.
4. SPACING BETWEEN PANELS, TYPICAL. CONTRACTOR TO VERIFY PURLIN SPACING WITH PANEL AND BRACKET SPECIFICATIONS PRIOR TO INSTALLATION.
5. MINIMUM LIP LENGTH SHALL MEET OR EXCEED THE FOLLOWING:

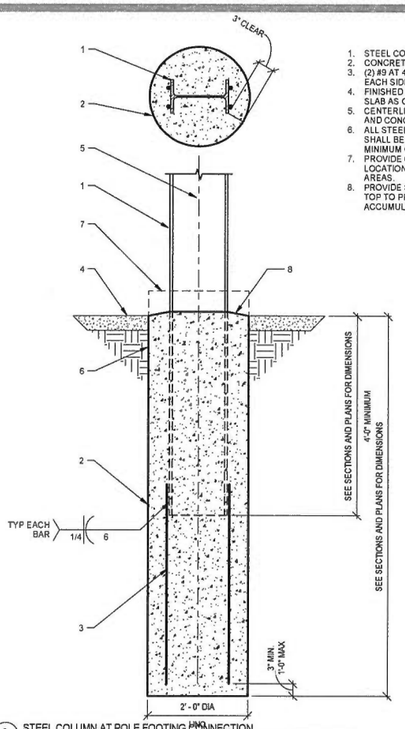
C-SHAPES
- 16 GAUGE - 0.773 INCHES
- 14 GAUGE - 0.800 INCHES
- 12 GAUGE - 0.885 INCHES

9 TYPICAL COLD FORMED STEEL PURLIN SECTION
NO SCALE



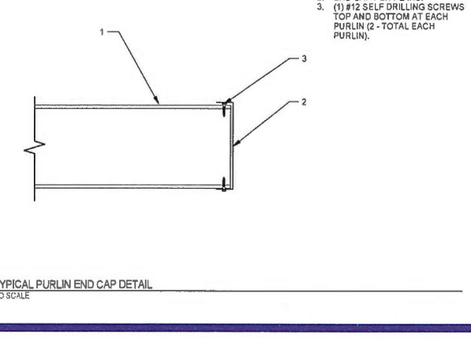
1. STEEL BEAM, FOR TILT DIRECTION SEE SECTIONS STEEL BEAM SHALL BARE COMPLETELY ON STEEL COLUMN. A MAXIMUM 1/2" GAP IS ACCEPTABLE WITHOUT REPAIR. IF THE GAP BETWEEN THE TOP OF COLUMN AND BEARING BEAM FLANGE EXCEEDS 1/2" STEEL SHIMS SHALL BE PROVIDED.
2. STEEL COLUMN
3. 3/4"x5 1/2"x12" (LSV) STEEL HODGE PLATE EACH SIDE OF COLUMN (Fy = 50 KSI MINIMUM).
4. 3/8" FULL DEPTH WEB STIFFENERS EACH SIDE OF COLUMN, AS SHOWN TYP.
5. WELD 3 SIDES (EACH SIDE AND BOTTOM OF PLATE). STOP WELD 1/2" FROM TOP OF COLUMN.

5 STEEL BEAM TO STEEL COLUMN CONNECTION
1"/0" SCALE



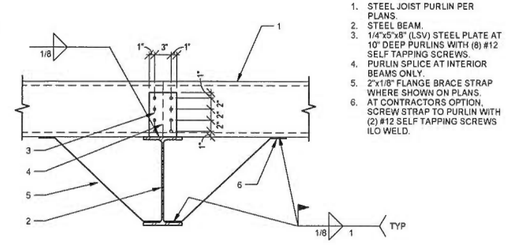
1. STEEL COLUMN.
2. CONCRETE POLE FOOTING
3. (2) #9 AT 4" O.C. WELDED TO EACH SIDE OF COLUMN.
4. FINISHED GRADE, ASPHALT OR SLAB AS OCCURS.
5. CENTERLINE OF STEEL COLUMN AND CONCRETE FOOTING.
6. ALL STEEL BELOW GRADE SHALL BE ENCASED IN 3" MINIMUM CONCRETE.
7. PROVIDE 6" EXPOSED PIER AT LOCATIONS OUTSIDE PAVED AREAS.
8. PROVIDE SLOPPED SURFACE AT TOP TO PREVENT ACCUMULATION OF WATER.

2 STEEL COLUMN AT POLE FOOTING CONNECTION
1"/0" SCALE



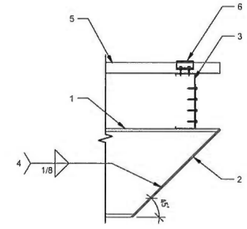
1. COLD FORMED STEEL PURLIN.
2. SOLAR PANELS BY OTHERS.
3. SOLAR CLAMP PER OTHER DETAILS.
4. L2"x2"x14 GAUGE STEEL SAG ANGLE WHERE SHOWN ON PLANS. ATTACH WITH (2) #12 SELF DRILLING SCREWS AT EACH PURLIN.
5. AT CONTRACTORS OPTION, USE 1 1/2" X 1 1/4" X 18 GAUGE STEEL HAT CHANNEL SAG ROD IN LEU OF L2"X2"X14 GAUGE STEEL SAG ROD.

10 SAG ROD AT STEEL PURLINS ATTACHMENT
1"/0" SCALE



1. STEEL JOIST PURLIN PER PLANS.
2. STEEL BEAM.
3. 1/4"x5"x8" (LSV) STEEL PLATE AT 10" DEEP PURLINS WITH (8) #12 SELF TAPPING SCREWS.
4. PURLIN SPLICE AT INTERIOR BEAMS ONLY.
5. 2"x1/8" FLANGE BRACE STRAP WHERE SHOWN ON PLANS.
6. AT CONTRACTORS OPTION, SCREW STRAP TO PURLIN WITH (2) #12 SELF TAPPING SCREWS ILO WELD.

6 TYPICAL PURLIN TO STEEL BEAM CONNECTION
NO SCALE



1. STEEL BEAM.
2. 1/8" STEEL CAP PLATE.
3. STEEL PURLIN, ATTACH TO BEAM PER OTHER DETAILS.
4. WELD 2" MINIMUM EACH FLANGE AND WEB.
5. SOLAR PANELS BY OTHERS.
6. SOLAR CLAMP PER OTHER DETAILS.

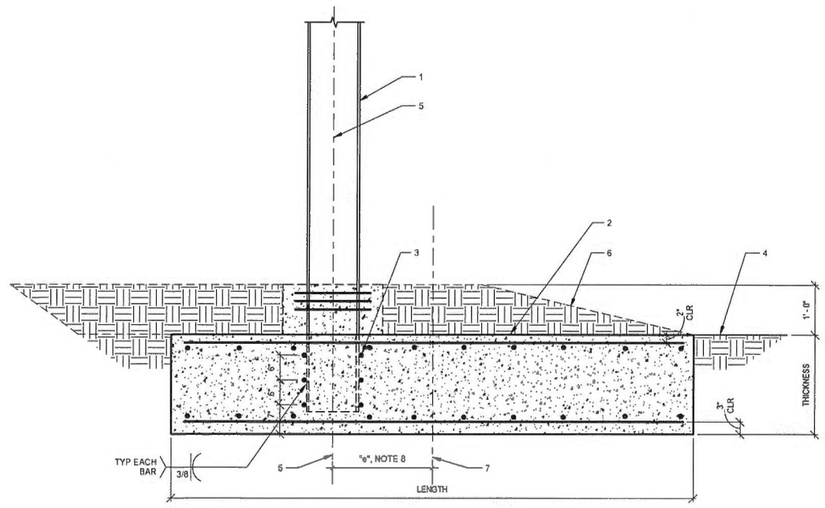
11 TYPICAL PURLIN TO STEEL BEAM CONNECTION
1/0 SCALE

1. COLD FORMED STEEL PURLIN.
2. END CAP PER PLANS.
3. (1) #12 SELF DRILLING SCREWS TOP AND BOTTOM AT EACH PURLIN (2 - TOTAL EACH PURLIN).

12 TYPICAL PURLIN END CAP DETAIL
1/0 SCALE

FOOTING SCHEDULE

STRUCTURE	FOOTING SIZE (LENGTH x WIDTH x THICKNESS)	FOOTING ECCENTRICITY "e"	FOOTING REINFORCING	CONCRETE STRENGTH
STRUCTURE 1	10'-0"x5'-5"x2'-0"	0'-0"	#6 AT 10" O.C. EACH WAY TOP AND BOTTOM	3,000 PSI



1. STEEL COLUMN.
2. CONCRETE SPREAD FOOTING, SEE SCHEDULE THIS DETAIL FOR SIZE AND REINFORCING.
3. (3) #9 x 5'-0" BARS AT 6" O.C. VERTICAL WELDED TO EACH SIDE OF COLUMN FLANGE (6-TOTAL PER COLUMN).
4. FINISHED GRADE OR SLAB AS OCCURS.
5. CENTERLINE OF STEEL COLUMN AND CONCRETE FOOTING WIDTH.
6. LINE OF SLOPED FINISHED GRADE AS OCCURS. WHERE FINISHED GRADE CAUSES THE TOP OF FOOTING TO BE LOWERED, THE STEEL COLUMN SHALL BE ENCASED WITH A CONCRETE CLOSURE POUR, WITH (3) #4 TIES WITHIN TOP 6". PROVIDE 3" MINIMUM CONCRETE COVER FOR ALL STEEL BELOW GRADE.
7. CENTERLINE OF FOOTING LENGTH.
8. OFFSET COLUMN PER FOOTING SCHEDULE. WHERE FOOTING ECCENTRICITY EXISTS, LONGER FOOTING TOE SHALL OCCUR ON THE SAME SIDE OF THE COLUMN.

8 STEEL COLUMN AT CONCRETE SPREAD FOOTING
NO SCALE

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PRELIMINARY NOT FOR CONSTRUCTION

Skyline SOLAR
A Division of Skyline Steel, Inc.

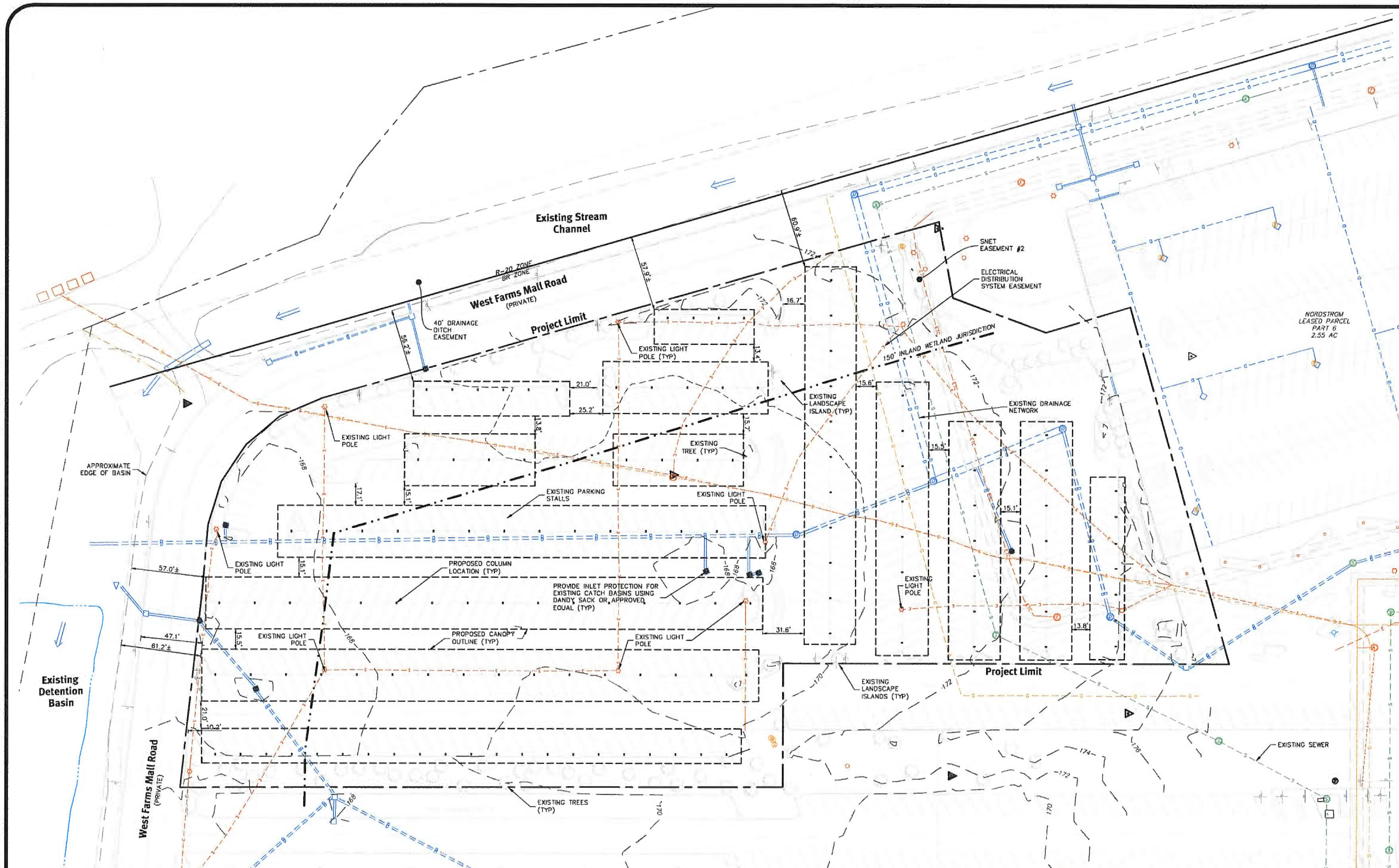
Taubman Westfarms Mall
West Hartford, CT 06110

No.	Description	Date

PROJECT NUMBER:	16001.005
DRAWN BY:	DB
CHECKED BY:	JE
DATE:	05/13/2016

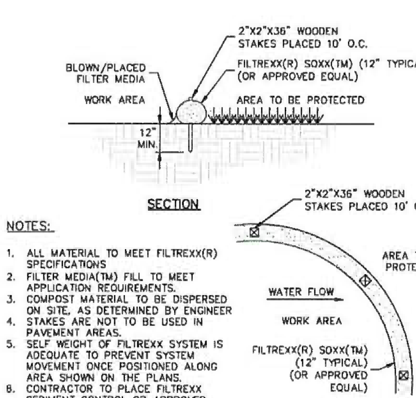
SHEET NAME
DETAILS

S4.1



General Notes:

1. THE PURPOSE OF THIS PLAN IS TO DEMONSTRATE THE CANOPY AND COLUMN LOCATIONS ASSOCIATED WITH THE WESTFARMS MALL SOLAR PROJECT. THE IMPROVEMENTS ASSOCIATED WITH THIS PROJECT INCLUDE:
   - INSTALLATION OF COLUMNS WITHIN EXISTING PAVEMENT AREAS.
   - ERECTION OF ELEVATED CANOPIES ONTO NEW COLUMNS.
   - TRENCHING ASSOCIATED WITH ELECTRICAL CONDUITS.
   - SOIL EROSION AND SEDIMENT CONTROL PROTECTION IN EXISTING CATCH BASIN STRUCTURES AND AROUND POTENTIAL STOCKPILE AREAS.
   - DAILY SWEEPING TO OCCUR WITHIN PROJECT LIMITS DURING CONSTRUCTION TO MINIMIZE SEDIMENTS ONTO ADJACENT ROADWAYS.
2. ALL CATCH BASINS WITHIN PROJECT LIMITS TO BE EQUIPPED WITH INLET PROTECTION UNTIL ALL CONSTRUCTION IS COMPLETE.
3. THE EXISTING SITE FEATURES, UTILITIES, AND TOPOGRAPHY SHOWN ON THIS PLAN WAS PREPARED BY F.A. HESKETH & ASSOCIATES, INC., 6 CREAMERY BROOK, EAST GRANBY, CT 06026 AT A SCALE OF 1" = 100' TITLED 'ALTA/ACSM LAND TITLE SURVEY PREPARED FOR WESTFARMS MALL, L.L.C., NEW BRITAIN AVENUE (CONN RTE. 71) & SOUTH ROAD WEST HARTFORD, NEWINGTON, & FARMINGTON, CT. DATED 04-14-95, REVISED 05-09-2018.
4. DIPRETE ENGINEERING HAS NOT COMPLETED ANY FIELD SURVEY ON THE SUBJECT PROPERTY.

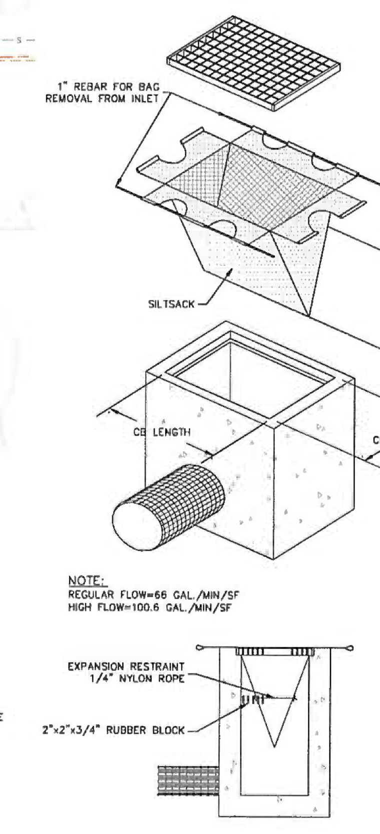


NOTES:

1. ALL MATERIAL TO MEET FILTREXX(R) SPECIFICATIONS
2. FILTER MEDIA(TM) FILL TO MEET APPLICATION REQUIREMENTS.
3. COMPOST MATERIAL TO BE DISPERSED ON SITE, AS DETERMINED BY ENGINEER
4. STAKES ARE NOT TO BE USED IN PAVEMENT AREAS.
5. SELF WEIGHT OF FILTREXX SYSTEM IS ADEQUATE TO PREVENT SYSTEM MOVEMENT ONCE POSITIONED ALONG AREA SHOWN ON THE PLANS.
6. CONTRACTOR TO PLACE FILTREXX SEDIMENT CONTROL OR APPROVED EQUAL AROUND ALL CURB INLET LOCATIONS AS SPECIFIED ON PLANS.

Filtrexx Sediment Control (or Approved Equal)

NOT TO SCALE



NOTE:
REGULAR FLOW=66 GAL./MIN/SF
HIGH FLOW=100.6 GAL./MIN/SF

Silt Sack Detail

NOT TO SCALE

DiPrete Engineering
990 Washington Street Suite 101A Dedham, MA 02026
tel 781-326-0021 fax 401-461-6006 www.diprete-eng.com
Boston • Providence • Newport

This regulatory submission set shall not be used for construction purposes unless stamped "Issued for Construction" and signed by a DiPrete Engineering representative.

The contractor is responsible for all of the means, methods, safety precautions and requirements, and OSHA conformance in the implementation of this plan and design.

Site Layout Plan
Westfarms Mall
West Hartford & Farmington, Connecticut

Applicant
Safari Energy, LLC
989 6th Avenue, 11th Floor, New York, New York, 10018

Scale: 1"=40'
0 20' 40' 80'

SHEET 1 OF 1