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March 9, 2018

EMAIL & FEDEX

Melanie Bachman, Esq.
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, Connecticut 06051

Re: Petition No. 443A - New Cingular Wireless PCS LLC ("AT&T")
Request for Administrative Approval of Temporary Facility
Site: 22 Sound Shore Drive, Greenwich, Connecticut

Dear Executive Director Bachman:

This letter and enclosures are submitted in furtherance of the captioned docket on behalf of New Cingular Wireless PCS, LLP ("AT&T"). I am writing as a follow up regarding AT&T's need for a temporary facility required to accommodate a transmission line reconfiguration planned by Eversource.

Procedural Background

As you will recall, by letter dated December 15, 2015 the Siting Council approved the relocation of AT&T's petition ("Petition 443A") to relocate an existing facility from Eversource Tower 1292 to nearby Tower 1279 (the "Petition 443A Facility"). As part of Petition 443A AT&T advised the Siting Council that a temporary tower facility would be necessary to provide continuity of service while AT&T waited for an outage allowing the facility on Tower 1279 to be installed. In its decision in 443A the Siting Council directed AT&T to submit for your approval the final location of the temporary facility.

Working with Eversource, and in lieu of deploying a temporary tower, AT&T designed a plan to modify the Tower 1292 facility by removing the then existing antenna platform and attaching AT&T antennas directly to Tower 1292. This plan took advantage of existing infrastructure, obviated the need to use a temporary tower and was believed to deter osprey nesting which had occurred in the area in the past. You provided your approval of this option by letter dated December 29, 2015 ("First Temporary Solution").

Subsequently Eversource continued its planning to reconfigure its transmission facilities in the area including Tower 1292 however an outage allowing installation on Tower 1279 was, and still is, unconfirmed. In addition, Metro North and the Connecticut Department of Transportation also required additional time, information and coordination regarding construction details of the permanent Petition 443A Facility. Given these variables and time constraints AT&T sought approval of an alternate temporary site at a nearby racquet club to assure continuity of service to



this area of Greenwich. As this temporary facility was not on the Site reviewed in the Petition 443A Approval, a petition for that second temporary solution ("Second Temporary Solution") was submitted to the Council as Petition 443B ("Petition 443B") and approved on July 7, 2016. Unfortunately, it has since become evident that the planned utility connections cannot be accommodated and the site is not viable without those connections. In addition, Eversource has also since notified AT&T that the antennas attached to Tower 1279 must be removed by April 1st in the interest of completing the planned infrastructure reconfiguration at the site.

New Temporary Siting Location Uses an Existing Transmission Tower

As depicted in the enclosed drawings prepared by Centek Engineering dated February 28, 2018 and included as Attachment 1, AT&T has now been able to arrive at a third temporary solution ("Third Temporary Solution") at the Site. AT&T will mount antennas to Eversource Tower 1281 which currently hosts Sprint and Verizon facilities at centerline heights of 148' and 139' respectively. AT&T's antennas will be mounted to Tower 1281 at a centerline height of 70' and associated equipment including remote radio head units and electrical breaker box will be provided from a cell-on-wheels trailer ("COW") with a cable connection installed on the north leg of Tower 1281. Notably, the telescoping mast on the COW will not be raised or used and as such will not be visible to anyone in the surrounding area. The COW will serve in lieu of a permanent equipment shelter or compound.

It should be noted that the low 70' centerline height above grade is driven by Eversource's requirement that all radio frequency emitting equipment be at least 15' from the bottom of the transmission line conductors. Combined with the limited number of antennas these restrictions severely limit the ability of the Third Temporary Solution to provide reliable service to the area of Cos Cob originally served by AT&T from Tower 1292. While workable as a temporary solution the service that can be provided from Tower 1281 is far inferior to the permanent site approved in Petition 443A which will be constructed once an outage is scheduled.

Materials Included

In support of this request please find the following attachments included with this letter:

1. Drawings and details of the proposed temporary solution prepared by Centek Engineering dated February 28, 2018;
2. Structural analysis prepared by Centek Engineering dated December 19, 2017; and
3. Information demonstrating compliance with federal emissions guidelines.

Conclusion

This temporary facility installation makes use of existing infrastructure at the Site reviewed in Petition 443A, will not require the deployment of a temporary tower mast and is similarly situated as the First Temporary Solution previously approved. Notably, no changes are proposed for the permanent facility approved for Tower 1279 which will be installed when a transmission line outage allows. We respectfully request your approval of this temporary facility as per the Siting Council's direction in the decision for Petition 443A.



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Thank you once again for your time and consideration in this matter. Should you have any questions, please do not hesitate to contact me.

Very truly yours,

A handwritten signature in dark ink, appearing to read "D. Laub", with a long horizontal line extending to the right.

Daniel M. Laub
Attachments

cc: Hon. Peter Tesei, First Selectman, Town of Greenwich
Katie DeLuca, AICP, Director of Planning & Zoning, Town of Greenwich
AT&T



WIRELESS COMMUNICATIONS FACILITY
CT5006
EVERSOURCE STRUCT. NO. 1281
COS COB RELO.
SOUND SHORE DRIVE
GREENWICH, CT 06830

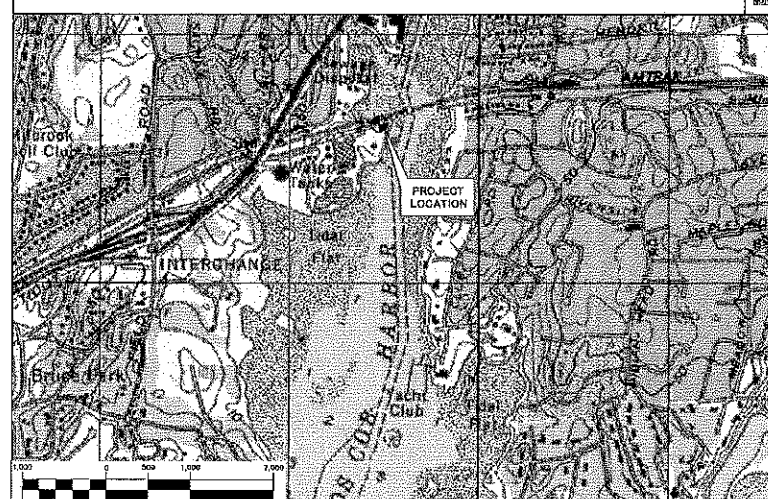
GENERAL NOTES

- ALL WORK SHALL BE IN ACCORDANCE WITH THE 2012 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2016 CONNECTICUT STATE BUILDING CODE, INCLUDING THE 2016 REVISION "G" STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND SUPPORTING STRUCTURES, 2016 CONNECTICUT FIRE SAFETY CODE AND, NATIONAL ELECTRICAL CODE AND LOCAL CODES.
- THE COMPOUND, TOWER, PRIMARY GROUND RING, ELECTRICAL SERVICE TO THE METER BANK AND TELEPHONE SERVICE TO THE DEMARCATION POINT ARE PROVIDED BY SITE OWNER. AS BUILT FIELD CONDITIONS REGARDING THESE ITEMS SHALL BE CONFIRMED BY THE CONTRACTOR. SHOULD ANY FIELD CONDITIONS PRECLUDE COMPLIANCE WITH THE DRAWINGS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER AND SHALL NOT PROCEED WITH ANY AFFECTED WORK.
- CONTRACTOR SHALL REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. CONTRACTOR SHALL COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS TO ALL SUBCONTRACTORS AND ALL RELATED PARTIES. THE SUBCONTRACTORS SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT AFFECTS THEIR WORK.
- CONTRACTOR SHALL PROVIDE A COMPLETE BUILD-OUT WITH ALL FINISHES, STRUCTURAL, MECHANICAL, AND ELECTRICAL COMPONENTS AND PROVIDE ALL ITEMS AS SHOWN OR INDICATED ON THE DRAWINGS OR IN THE WRITTEN SPECIFICATIONS.
- CONTRACTOR SHALL FURNISH ALL MATERIAL, LABOR AND EQUIPMENT TO COMPLETE THE WORK AND FURNISH A COMPLETED JOB ALL IN ACCORDANCE WITH LOCAL AND STATE GOVERNING AUTHORITIES AND OTHER AUTHORITIES HAVING LAWFUL JURISDICTION OVER THE WORK.
- CONTRACTOR SHALL SECURE AND PAY FOR ALL PERMITS AND ALL INSPECTIONS REQUIRED AND SHALL ALSO PAY FEES REQUIRED FOR THE GENERAL CONSTRUCTION, PLUMBING, ELECTRICAL AND HVAC. PERMITS SHALL BE PAID FOR BY THE RESPECTIVE SUBCONTRACTORS.
- CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS ON SITE AT ALL TIMES AND INSURE DISTRIBUTION OF NEW DRAWINGS TO SUBCONTRACTORS AND OTHER RELEVANT PARTIES AS SOON AS THEY ARE MADE AVAILABLE. ALL OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONTRACT AREA. THE CONTRACTOR SHALL FURNISH AN "AS-BUILT" SET OF DRAWINGS TO OWNER UPON COMPLETION OF PROJECT.
- LOCATION OF EQUIPMENT, AND WORK SUPPLIED BY OTHERS THAT IS DIAGRAMMATICALLY INDICATED ON THE DRAWINGS SHALL BE DETERMINED BY THE CONTRACTOR. THE CONTRACTOR SHALL DETERMINE LOCATIONS AND DIMENSIONS SUBJECT TO STRUCTURAL CONDITIONS AND WORK OF THE SUBCONTRACTORS.
- THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING BUILDING'S/PROPERTY'S OPERATIONS, COORDINATE WORK WITH BUILDING/PROPERTY OWNER.
- DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
- ALL UTILITY WORK SHALL BE IN ACCORDANCE WITH LOCAL UTILITY COMPANY REQUIREMENTS AND SPECIFICATIONS.
- ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE REVIEWED BY CONTRACTOR AND ALL APPLICABLE SUBCONTRACTORS FOR ANY CONDITION PER MFR.'S RECOMMENDATIONS. CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST TO OWNER OR CONSTRUCTION MANAGER.
- ANY AND ALL ERRORS, DISCREPANCIES, AND "MISSED" ITEMS ARE TO BE BROUGHT TO THE ATTENTION OF THE AT&T CONSTRUCTION MANAGER DURING THE BIDDING PROCESS BY THE CONTRACTOR. ALL THESE ITEMS ARE TO BE INCLUDED IN THE BID. NO "EXTRA" WILL BE ALLOWED FOR MISSED ITEMS.
- CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ON-SITE SAFETY FROM THE TIME THE JOB IS AWARDED UNTIL ALL WORK IS COMPLETE AND ACCEPTED BY THE OWNER.
- CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE CONSTRUCTION MANAGER FOR REVIEW.
- THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND EXISTING CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA.
- COORDINATION, LAYOUT, FURNISHING AND INSTALLATION OF CONDUIT AND ALL APPURTENANCES REQUIRED FOR PROPER INSTALLATION OF ELECTRICAL AND TELECOMMUNICATION SERVICE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE REVIEWED BY CONTRACTOR AND ALL APPLICABLE SUB-CONTRACTORS FOR ANY CONDITION PER THE MANUFACTURER'S RECOMMENDATIONS. CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST TO OWNER OR CONSTRUCTION MANAGER.
- ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
- THE CONTRACTOR SHALL CONTACT "CALL BEFORE YOU DIG" AT LEAST 48 HOURS PRIOR TO ANY EXCAVATIONS AT 1-800-922-4455. ALL UTILITIES SHALL BE IDENTIFIED AND CLEARLY MARKED PRIOR TO ANY EXCAVATION WORK. CONTRACTOR SHALL MAINTAIN AND PROTECT MARKED UTILITIES THROUGHOUT PROJECT COMPLETION.
- CONTRACTOR SHALL COMPLY WITH OWNERS ENVIRONMENTAL ENGINEER ON ALL METHODS AND PROVISIONS FOR ALL EXCAVATION ACTIVITIES INCLUDING SOIL DISPOSAL. ALL BACKFILL MATERIALS TO BE PROVIDED BY THE CONTRACTOR.

SITE DIRECTIONS

FROM:	TO:
500 ENTERPRISE DRIVE ROCKY HILL, CONNECTICUT	SOUND SHORE DRIVE GREENWICH, CT 06830
1. HEAD NORTHEAST ON ENTERPRISE DR TOWARD CAPITAL BLVD	0.3 MI.
2. TURN LEFT ONTO CAPITAL BLVD	0.2 MI.
3. USE THE LEFT TWO LANES TO TURN LEFT ONTO STATE HWY 411	0.3 MI.
4. TURN LEFT TO MERGE ONTO I-91 S	0.3 MI.
5. MERGE ONTO I-91 S	9.4 MI.
6. TAKE EXIT 17 TO MERGE ONTO CT-155/WILBUR CROSS PKWY	30.2 MI.
7. TAKE EXIT 52 FOR STATE ROUTE 108S/STATE ROUTE 85 TOWARD BRIDGEPORT	0.6 MI.
8. KEEP LEFT AT THE FORK, FOLLOW SIGNS FOR CT-65/BRIDGEPORT AND MERGE ONTO CT-65	5.2 MI.
9. TAKE THE INTERSTATE 95S EXIT TOWARD NY CITY	0.2 MI.
10. MERGE ONTO I-95S	25.0 MI.
11. TAKE EXIT 4 FOR INDIAN FIELD ROAD TOWARD COS COB	0.2 MI.
12. TURN RIGHT ONTO INDIAN FIELD ROAD	62 FT
13. TURN RIGHT ONTO SOUND SHORE DRIVE	0.4 MI.

VICINITY MAP



PROJECT SCOPE

- THE SCOPE OF WORK SHALL INCLUDE:
- THE INSTALLATION OF A WIRELESS COMMUNICATION TOWER SUPPORTED ON AN EXISTING EVERSOURCE TRANSMISSION TOWER WHICH CONSISTS OF THE FOLLOWING:
 - THREE (3) TEMPORARY (3) AT&T PANEL ANTENNAS AND ASSOCIATED APPURTENANCES.
 - ONE (1) TEMPORARY CELL-ON-WHEELS EQUIPMENT CART.
 - THE PROPOSED WIRELESS FACILITY INSTALLATION WILL BE DESIGNED IN ACCORDANCE WITH THE 2012 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2016 CONNECTICUT SUPPLEMENT.

PROJECT INFORMATION

SITE NAME:	COS COB RELO -- CT5006
SITE ADDRESS:	EVERSOURCE STRUCT. NO. 1281 SOUND SHORE DRIVE GREENWICH, CT 06830
PROPERTY OWNER:	STATE OF CONNECTICUT TOWN OF GREENWICH C/O FINANCE DEPARTMENT 101 FIELD POINT ROAD GREENWICH, CT 06830
LESSEE/TENANT:	AT&T MOBILITY 500 ENTERPRISE DRIVE, SUITE 3A ROCKY HILL, CT 06067
CONTACT PERSON:	DAN BILEZIKIAN SAI COMMUNICATIONS (401) 368-0006
ENGINEER:	CENITEK ENGINEERING 63-2 NORTH BRANFORD ROAD, BRANFORD, CT 06405 (203) 488-0580
TOWER COORDINATES:	LATITUDE: 41°-01'-46.955" N LONGITUDE: 73°-35'-54.078" W EX. GROUND ELEVATION: 28.22' ± A.M.S.L. COORDINATES AND GROUND ELEVATION BASED ON FAA 1-A SURVEY CERTIFICATION PREPARED BY CENITEK ENGINEERING, INC., DATED FEBRUARY 28, 2018.

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PROFESSIONAL ENGINEER SEAL

STATE OF CONNECTICUT
JAMES M. SAI
Professional Engineer
No. 12345
Exp. 12/31/2025

at&t

SAI
Structural Analysis Inc.

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BRANFORD, CT 06405
(203) 488-0580
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AT&T MOBILITY
WIRELESS COMMUNICATIONS FACILITY
COS COB RELO
SITE NUMBER: CT5006
SOUND SHORE DRIVE
GREENWICH, CT 06830

DATE: 02/21/18
SCALE: AS NOTED
JOB NO. 16009.03

TITLE SHEET

T-1
Sheet No. 1 of 8

NOTES AND SPECIFICATIONS

DESIGN BASIS:

1. GOVERNING CODE: 2012 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2016 CT STATE BUILDING CODE.
2. ANSI/TIA-222-G, ASCE MANUAL NO. 72 - "DESIGN OF STEEL TRANSMISSION POLE STRUCTURES SECOND EDITION", NESC C2-2012 AND NORTHEAST UTILITIES DESIGN CRITERIA.
3. DESIGN CRITERIA
- WIND LOAD: (PCS MAST)
NOMINAL DESIGN WIND SPEED (V_{50}) = 93 MPH (FASTEST MILE); BASED ON ANSI/TIA-222-G AND NU MAST DESIGN CRITERIA EXCEPTION 1.
- WIND LOAD: (UTILITY POLE & FOUNDATION)
BASIC WIND SPEED (V) = 110 MPH (3-SECOND GUST) BASED ON NESC C2-2012, SECTION 25 RULE 250C.

SPECIAL INSPECTIONS

1. SPECIAL INSPECTIONS ARE TO BE PROVIDED BY AN APPROVED AGENCY HIRED BY AT&T MOBILITY.

GENERAL NOTES:

1. ALL CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE GOVERNING BUILDING CODE.
2. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
3. BEFORE BEGINNING THE WORK, THE CONTRACTOR IS RESPONSIBLE FOR MAKING SUCH INVESTIGATIONS CONCERNING PHYSICAL CONDITIONS (SURFACE AND SUBSURFACE) AT OR CONTIGUOUS TO THE SITE WHICH MAY AFFECT PERFORMANCE AND COST OF THE WORK.
4. DIMENSIONS AND DETAILS SHALL BE CHECKED AGAINST THE PRE MANUFACTURED EQUIPMENT BUILDING SHOP DRAWINGS.
5. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS, SLEEVES AND ANCHOR BOLTS AS REQUIRED BY ALL TRADES.
6. ALL DIMENSIONS, ELEVATIONS, AND OTHER REFERENCES TO EXISTING STRUCTURES, SURFACE, AND SUBSURFACE CONDITIONS ARE APPROXIMATE. NO GUARANTEE IS MADE FOR THE ACCURACY OR COMPLETENESS OF THE INFORMATION SHOWN. THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS, ELEVATIONS, ANGLES WITH EXISTING CONDITIONS AND WITH ARCHITECTURAL AND SITE DRAWINGS BEFORE PROCEEDING WITH ANY WORK.
7. AS THE WORK PROGRESSES, THE CONTRACTOR SHALL NOTIFY THE OWNER OF ANY CONDITIONS WHICH ARE IN CONFLICT OR OTHERWISE NOT CONSISTENT WITH THE CONSTRUCTION DOCUMENTS AND SHALL NOT PROCEED WITH SUCH WORK UNTIL THE CONFLICT IS SATISFACTORILY RESOLVED.
8. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PROVIDING AND MAINTAINING ADEQUATE SHORING, BRACING, AND BARRICADES AS MAY BE REQUIRED FOR THE PROTECTION OF EXISTING PROPERTY, CONSTRUCTION WORKERS, AND FOR PUBLIC SAFETY.
9. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING SITE OPERATIONS, COORDINATE WORK WITH NORTHEAST UTILITIES
10. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER FOUNDATION REMEDIATION WORK IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, GUYS OR TIEDOWNS, WHICH MIGHT BE NECESSARY.
11. THE CONTRACTOR SHALL LIMIT THE DURATION OF ANY FOUNDATION MODIFICATION WORK. THE EXISTING FOUNDATION WITHIN THE SHOWN LIMITS IS STABLE FOR WIND SPEEDS LESS THAN 50MPH WITHOUT ICE LOADING. IF HIGHER WIND SPEED OR ICE EVENT IS EXPECTED, THE EXCAVATION AREA SHALL BE FILLED WITH COMPACT FILL MATERIAL.
12. ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
13. SHOP DRAWINGS, CONCRETE MIX DESIGNS, TEST REPORTS, AND OTHER SUBMITTALS PERTAINING TO STRUCTURAL WORK SHALL BE FORWARDED TO THE OWNER FOR REVIEW BEFORE FABRICATION AND/OR INSTALLATION IS MADE. SHOP DRAWINGS SHALL INCLUDE ERECTION DRAWINGS AND COMPLETE DETAILS OF CONNECTIONS AS WELL AS MANUFACTURER'S SPECIFICATION DATA WHERE APPROPRIATE. SHOP DRAWINGS SHALL BE CHECKED BY THE CONTRACTOR AND BEAR THE CHECKER'S INITIALS BEFORE BEING SUBMITTED FOR REVIEW.
14. NO DRILLING WELDING OR TAPING ON CL&P OWNED EQUIPMENT.
15. REFER TO DRAWING T1 FOR ADDITIONAL NOTES AND REQUIREMENTS.

SITE NOTES

1. THE CONTRACTOR SHALL CALL UTILITIES PRIOR TO THE START OF CONSTRUCTION.
2. ACTIVE EXISTING UTILITIES, WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES. THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY, PRIOR TO PROCEEDING, SHOULD ANY UNCOVERED EXISTING UTILITY PRECLUDE COMPLETION OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
3. ALL RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED OFF SITE AND BE LEGALLY DISPOSED, AT NO ADDITIONAL COST.
4. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE EQUIPMENT AND TOWER AREAS.
5. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.
6. THE SUBGRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
7. THE AREAS OF THE COMPOUND DISTURBED BY THE WORK SHALL BE RETURNED TO THEIR ORIGINAL CONDITION.
8. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
9. IF ANY FIELD CONDITIONS EXIST WHICH PRECLUDE COMPLIANCE WITH THE DRAWINGS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER AND SHALL PROCEED WITH AFFECTED WORK AFTER CONFLICT IS SATISFACTORILY RESOLVED.
10. DIMENSIONS AND DETAILS SHALL BE CHECKED AGAINST THE PRE MANUFACTURED EQUIPMENT BUILDING SHOP DRAWINGS.
11. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS, SLEEVES AND ANCHOR BOLTS AS REQUIRED BY ALL TRADES.

EARTHWORK NOTES

1. COMPACTED GRAVEL FILL SHALL BE FURNISHED AND PLACED AS A FOUNDATION FOR STRUCTURES, WHERE SHOWN ON THE CONTRACT DRAWINGS OR DIRECTED BY THE ENGINEER.
2. CRUSHED STONE FILL SHALL BE PLACED IN 12" MAX. LIFTS AND CONSOLIDATED USING A HAND OPERATED VIBRATORY PLATE COMPACTOR WITH A MINIMUM OF 2 PASSSES OF COMPACTOR PER LIFT.
3. COMPACTED GRAVEL FILL TO BE WELL GRADED BANK RUN GRAVEL MEETING THE FOLLOWING GRADATION REQUIREMENTS:
- | SIEVE DESIGNATION | % PASSING |
|-------------------|-----------|
| 1 1/2" | 100 |
| No. 4 | 40-70 |
| No. 100 | 5-20 |
| No. 200 | 4-8 |
4. CRUSHED STONE TO BE UNIFORMLY GRADED, CLEAN, HARD PROCESS AGGREGATE MEETING THE FOLLOWING GRADATION REQUIREMENTS:
- | SIEVE DESIGNATION | % PASSING |
|-------------------|-----------|
| 1" | 100 |
| 3/4" | 90-100 |
| 2" | 0-15 |
| 3/8" | 0-5 |
5. SELECT BACKFILL FOR FOUNDATION WALLS SHALL BE FREE OF ORGANIC MATERIAL, TOPSOIL, DEBRIS AND BOULDERS LARGER THAN 6".
6. GRAVEL AND GRANULAR FILL SHALL BE INSTALLED IN 10" MAX. LIFTS. COMPACTED TO 95% MIN. AT MAX. DRY DENSITY.
7. NON WOVEN GEOTEXTILE FOR SEPARATION PURPOSES SHALL BE MIRAFI 140N, OR ENGINEER APPROVED EQUAL.

FOUNDATION CONSTRUCTION NOTES

1. ALL FOOTINGS SHALL BE PLACED ON SUITABLE, COMPACTED SOIL HAVING ADEQUATE BEARING CAPACITY AND FREE OF ORGANIC CONTENT, CLAY, OR OTHER UNSUITABLE MATERIAL. ADDITIONAL EXCAVATION MAY BE REQUIRED BELOW FOOTING ELEVATIONS INDICATED IF UNSUITABLE MATERIAL IS ENCOUNTERED.
2. SUBGRADE PREPARATION: IF UNSUITABLE SOIL IS ENCOUNTERED, REMOVE ALL UNSUITABLE MATERIALS FROM BELOW PROPOSED STRUCTURE FOUNDATIONS AND COMPACT EXPOSED SOIL SURFACES. PLACE AND COMPACT APPROVED GRAVEL FILL. PLACEMENT OF ALL COMPACTED FILL MUST BE UNDER SUPERVISION OF AN APPROVED TESTING LABORATORY. FILL SHALL BE COMPACTED IN LAYERS NOT TO EXCEED 10" BEFORE COMPACTION. DETERMINE MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D1557-70 AND MAKE ONE (1) FIELD DENSITY TEST IN ACCORDANCE WITH ASTM D2167-66 FOR EACH 50 CUBIC YARDS OF COMPACTED FILL, BUT NOT LESS THAN ONE (1) PER LAYER, TO INSURE COMPACTION TO 95% OF MAX. DRY DENSITY.
3. ALL SOIL SURROUNDING AND UNDER ALL FOOTINGS SHALL BE KEPT REASONABLY DRY AND PROTECTED FROM FREEZING AND FROST ACTION DURING THE COURSE OF CONSTRUCTION.
4. WHERE GROUNDWATER IS ENCOUNTERED, DEWATERING SHALL BE ACCOMPLISHED CONTINUOUSLY AND COMPLETELY DURING FOUNDATION CONSTRUCTION. PROVIDE CRUSHED STONE AS REQUIRED TO STABILIZE FOOTING SUBGRADE.
5. ALL FOOTINGS ARE TO REST ON FIRM SOIL, REGARDLESS OF ELEVATIONS SHOWN ON THE DRAWINGS, BUT IN NO CASE MAY FOOTING ELEVATIONS BE HIGHER THAN INDICATED ON THE FOUNDATION PLAN, UNLESS SPECIFICALLY DIRECTED BY THE ENGINEER.
6. FOUNDATION WATERPROOFING AND DAMPPROOFING SHALL COMPLY WITH BUILDING CODE REQUIREMENTS UNLESS A MORE SUBSTANTIAL SYSTEM IS INDICATED OR SPECIFIED.

CONCRETE CONSTRUCTION NOTES

1. CONCRETE CONSTRUCTION SHALL CONFORM TO THE FOLLOWING STANDARDS:
- ACI 211 - STANDARD PRACTICE FOR SELECTING PROPORTIONS FOR NORMAL AND HEAVYWEIGHT CONCRETE.
- ACI 301 - SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS.
- ACI 302 - GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION
- ACI 304 - RECOMMENDED PRACTICE FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE.
- ACI 308.1 - STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING
- ACI 318 - BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.
2. CONCRETE SHALL DEVELOP COMPRESSIVE STRENGTH IN 28 DAYS AS FOLLOWS:
- | SLABS ON GRADE | 4,000 PSI |
|--------------------|-----------|
| ALL OTHER CONCRETE | 3,000 PSI |
- PORTLAND CEMENT: ASTM C150, TYPE II, (540 LBS/CUBIC YARD)
- AGGREGATE: ASTM C33, No. 67, TYPICAL
- WATER: POTABLE WITH MAXIMUM WATER CEMENT RATIO OF .55
- SLUMP: 3" TO 4"
- ADMIXTURES: USE AIR ENTRAINING AGENT CONFORMING TO ASTM C260 WITH 4 TO 6% TOTAL AIR. USE WATER REDUCING AGENT CONFORMING TO ASTM C494, TYPE A, IN ALL CONCRETE. CALCIUM CHLORIDE MAY NOT BE USED TO ACCELERATE THE CONCRETE SETTING TIME.
3. REINFORCING STEEL SHALL BE 60,000 PSI YIELD STRENGTH.
4. WELDED WIRE FABRIC SHALL CONFORM TO ASTM- A-185.
5. ALL DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS, UNLESS OTHERWISE NOTED, MUST FOLLOW THE LATEST ACI CODE AND LATEST ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES".
6. CONCRETE COVER OVER REINFORCING SHALL CONFORM TO THE FOLLOWING, UNLESS OTHERWISE SHOWN:
- | CONCRETE CAST AGAINST & PERMANENTLY EXPOSED TO EARTH | 3 INCHES |
|--|--------------|
| CONCRETE EXPOSED TO EARTH OR WEATHER:
#6 THROUGH #18 BARS | 2 INCHES |
| #5 BAR, W31 OR D31 WIRE, AND SMALLER | 1-1/2 INCHES |
| CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND:
#14 THROUGH #18 BARS | 1-1/2 INCHES |
| #11 BAR AND SMALLER | 3/4 INCHES |
7. NO STEEL WIRE, METAL FORM TIES, OR ANY OTHER METAL SHALL REMAIN WITHIN THE REQUIRED COVER OF ANY CONCRETE SURFACE.
8. ALL REINFORCEMENT SHALL BE CONTINUOUS UNLESS OTHERWISE NOTED. SPICES SHALL BE WELL STAGGERED. ADDITIONAL BARS AND SPECIAL BENDING DETAILS ARE REQUIRED AT INTERSECTING WALLS AND AT JOINTS. SUCH DETAILS SHALL COMPLY WITH ACI 315 RECOMMENDATIONS UNLESS OTHERWISE SHOWN.
9. NO TACK WELDING OF REINFORCING WILL BE PERMITTED.
10. NO CALCIUM CHLORIDE OR ADMIXTURES CONTAINING MORE THAN 1% CHLORIDE BY WEIGHT OF ADMIXTURE SHALL BE USED IN THE CONCRETE.
11. UNLESS OTHERWISE NOTED, ALL LAP SPICES SHALL BE 48 BAR DIAMETERS.
12. SLAB ON GRADE FINISHES:
- EXTERIOR SLAB: NON-SLIP BROOM FINISH
INTERIOR SLAB: STEEL TROWEL FINISH
13. INSPECTION AND TESTING OF CONCRETE WORK SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY, PAID BY THE OWNER, AND APPROVED BY THE ENGINEER. THE INSPECTOR SHALL OBSERVE CONDITION OF SOILS AND FORMWORK BEFORE FOOTINGS ARE PLACED, SIZE, SPACING AND LOCATION OF REINFORCEMENT, AND PLACEMENT OF CONCRETE.
14. THE TESTING COMPANY SHALL ALSO OBTAIN A MINIMUM OF THREE (3) COMPRESSIVE STRENGTH TEST SPECIMENS FOR EACH CONCRETE MIX DESIGN. ONE SPECIMEN TESTED AT 7 DAYS, ONE AT 28 DAYS, AND ONE HELD IN RESERVE FOR FUTURE TESTING, IF NEEDED.
15. FOUR COPIES OF ALL INSPECTION TEST REPORTS SHALL BE SUBMITTED TO THE ENGINEER WITHIN TEN (10) WORKING DAYS OF THE DATE OF INSPECTION.

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL IS DESIGNED BY ALLOWABLE STRESS DESIGN (ASD)
- A. STRUCTURAL STEEL (W SHAPES)----ASTM A992 (FY = 50 KSI)
B. STRUCTURAL STEEL (OTHER SHAPES)----ASTM A36 (FY = 36 KSI)
C. STRUCTURAL HSS (RECTANGULAR SHAPES)----ASTM A500 GRADE B, (FY = 46 KSI)
D. STRUCTURAL HSS (ROUND SHAPES)----ASTM A500 GRADE B, (FY = 42 KSI)
E. PIPE----ASTM A53 (FY = 35 KSI)
F. CONNECTION BOLTS----ASTM A325--N
G. U-BOLTS----ASTM A36
H. ANCHOR RODS----ASTM F 1554
I. WELDING ELECTRODE----ASTM E 70XX
2. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE THE FOLLOWING: SECTION PROFILES, SIZES, CONNECTION ATTACHMENTS, REINFORCING, ANCHORAGE, SIZE AND TYPE OF FASTENERS AND ACCESSORIES. INCLUDE ERECTION DRAWINGS, ELEVATIONS AND DETAILS.
3. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST PROVISIONS OF AISC MANUAL OF STEEL CONSTRUCTION.
4. PROVIDE ALL PLATES, CLIP ANGLES, CLOSURE PIECES, STRAP ANCHORS, MISCELLANEOUS PIECES AND HOLES REQUIRED TO COMPLETE THE STRUCTURE.
5. FIT AND SHOP ASSEMBLE FABRICATIONS IN THE LARGEST PRACTICAL SECTIONS FOR DELIVERY TO SITE.
6. INSTALL FABRICATIONS PLUMB AND LEVEL, ACCURATELY FITTED, AND FREE FROM DISTORTIONS OR DEFECTS.
7. AFTER ERECTION OF STRUCTURES, TOUCHUP ALL WELDS, ABRASIONS AND NON-GALVANIZED SURFACES WITH A 95% ORGANIC ZINC RICH PAINT IN ACCORDANCE WITH ASTM 780.
8. ALL STEEL MATERIAL (EXPOSED TO WEATHER) SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT DIPPED GALVANIZED) COATINGS" ON IRONS AND STEEL PRODUCTS.
9. ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING (HOT-DIP) ON IRON AND STEEL HARDWARE".
10. CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES APPEARANCE AND QUALITY OF WELDS, AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES". ALL WELDING SHALL BE DONE USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AWS AND D1.1 WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AWS "MANUAL OF STEEL CONSTRUCTION" 9TH EDITION. AT THE COMPLETION OF WELDING, ALL DAMAGE TO GALVANIZED COATING SHALL BE REPAIRED.
11. THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NON CONFORMING MATERIALS OR CONDITIONS TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW.
12. CONNECTION ANGLES SHALL HAVE A MINIMUM THICKNESS OF 1/4 INCHES.
13. STRUCTURAL CONNECTION BOLTS SHALL CONFORM TO ASTM A325. ALL BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL HAVE A MINIMUM OF TWO BOLTS, UNLESS OTHERWISE ON THE DRAWINGS.
14. LOCK WASHER ARE NOT PERMITTED FOR A325 STEEL ASSEMBLIES.
15. SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED.
16. MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION.
17. FABRICATE BEAMS WITH MILL CAMBER UP.
18. LEVEL AND PLUMB INDIVIDUAL MEMBERS OF THE STRUCTURE TO AN ACCURACY OF 1:500, BUT NOT TO EXCEED 1/4" IN THE FULL HEIGHT OF THE COLUMN.
19. COMMENCEMENT OF STRUCTURAL STEEL WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCIES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.
20. INSPECTION AND TESTING OF ALL WELDING AND HIGH STRENGTH BOLTING SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY.
21. FOUR COPIES OF ALL INSPECTION TEST REPORTS SHALL BE SUBMITTED TO THE ENGINEER WITHIN TEN (10) WORKING DAYS OF THE DATE OF INSPECTION.

PROFESSIONAL ENGINEER SEAL
STATE OF CONNECTICUT
JAMES J. KELLY
REGISTERED PROFESSIONAL ENGINEER
ELECTRICAL
No. 10100
EXPIRATION DATE 12/31/2024

CENTEK engineering
Center at the Core®
1201 484-0883
1201 484-8887 Fax
845 North Bedford Road
Bedford, CT 06030
www.centekeng.com

AT&T MOBILITY
WIRELESS COMMUNICATIONS FACILITY
COS COB RELO
SITE NUMBER: CT5006
SOUND SHORE DRIVE
GREENWICH, CT 06830

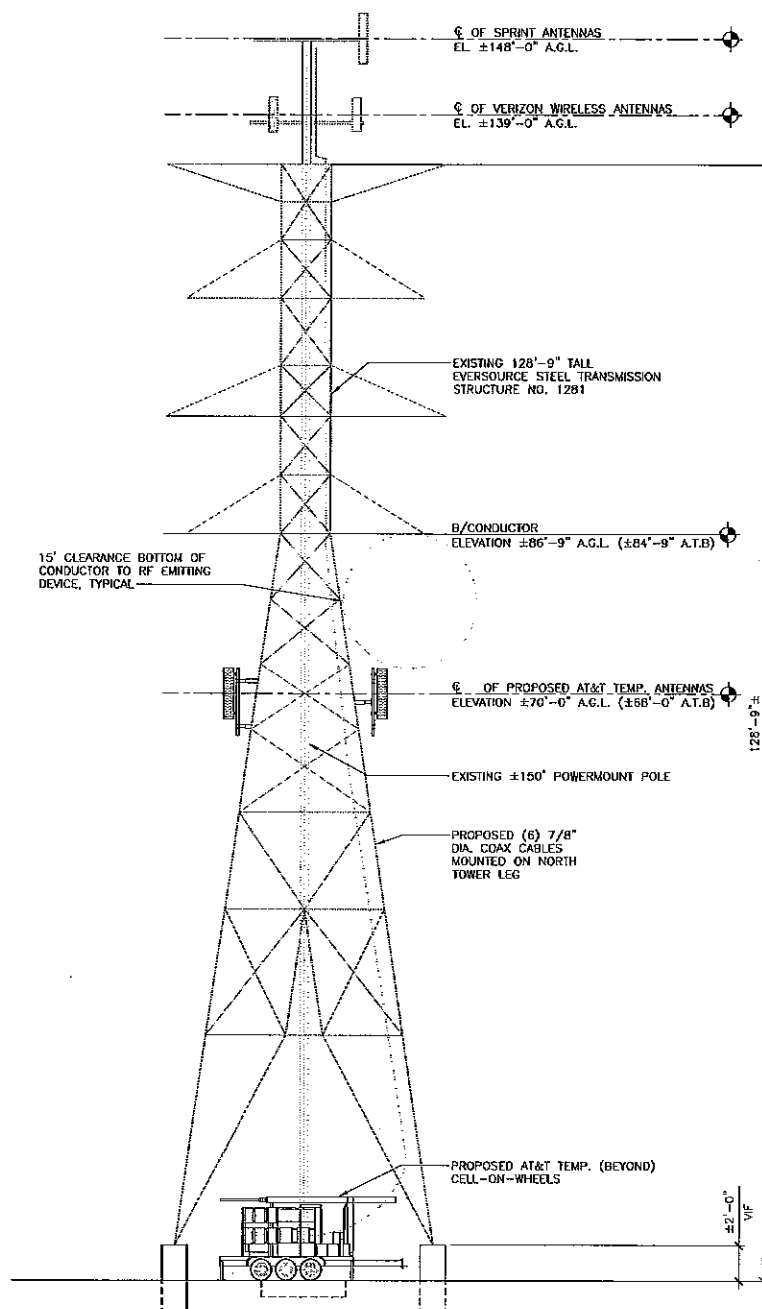
DATE: 02/21/18
SCALE: AS NOTED
JOB NO. 16009.03
GENERAL NOTES AND SPECIFICATIONS
N-1
Sheet No. 2 of 8

CONSTRUCTION DOCUMENTS - ISSUED FOR CONSTRUCTION
DATE: 02/28/18
DRAWN BY: CHD
CHECKED BY: CAC
KEY:

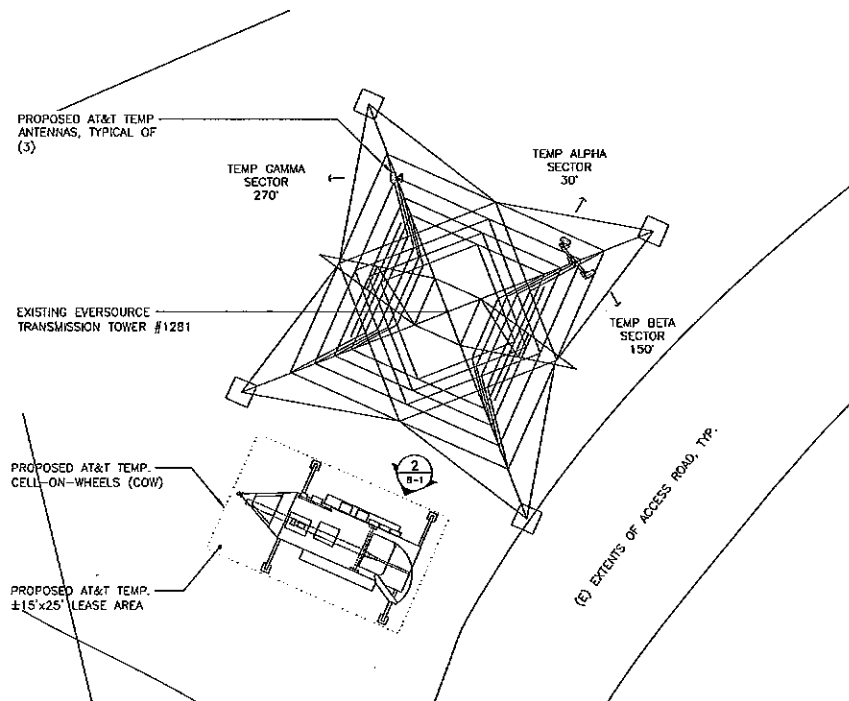
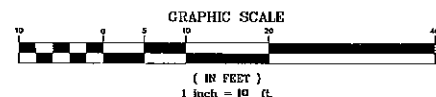
NOTES:

1. INFORMATION SHOWN HEREIN IS FOR USE BY AT&T EQUIPMENT OPERATIONS.
2. THIS B.O.M. DRAWING IS BASED OFF FIELD MEASUREMENTS, CONSTRUCTION DRAWINGS PREPARED BY CENTEK ENGINEERING (REV.0 DATED: 02.28.18), & AT&T RF ANTENNA EQUIPMENT RECOMMENDATION

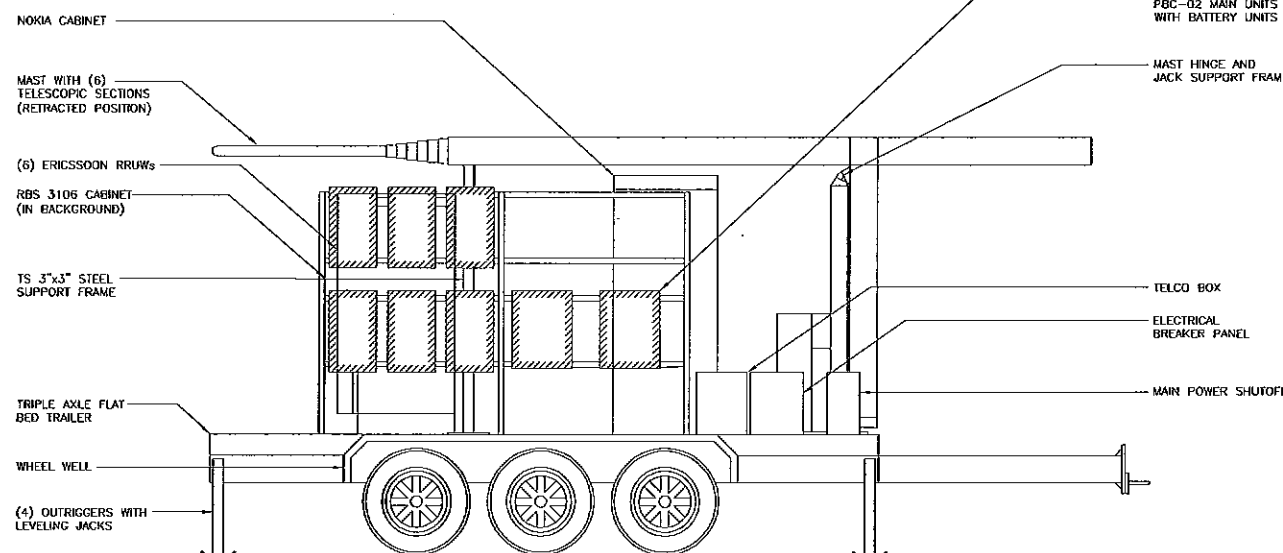
BILL OF MATERIALS - SITE EQUIPMENT				
	QUANTITY	LENGTH	COMMENTS	
① TEMP	3		(KATHREIN B40 10520)	
② 1/2" COAX JUMPERS	6	6 FT	ROUTE FROM 7/8" COAX CABLE TO ANTENNA PORTS	
③ 7/8" COAX CABLE	6	±80 FT	ROUTE FROM COW TO 1/2" JUMPERS	



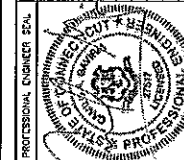
3 TOWER EAST ELEVATION
B-1 SCALE: 1"=10'



1 SITE COMPOUND PLAN
B-1 SCALE: 1"=10'



2 CELL-ON-WHEELS ELEVATION
B-1 SCALE: 1/2"=1'



CENTEK Engineering
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 1201 485-0383
 1201 485-0387 Fax
 1201 485-0388 Mobile
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 www.centekeng.com

AT&T MOBILITY
 WIRELESS COMMUNICATIONS FACILITY
 COS COB RELO
 SITE NUMBER: CT5006
 SOUND SHORE DRIVE
 GREENWICH, CT 06830

DATE: 02/21/18
 SCALE: AS NOTED
 JOB NO. 16009.03

RF BILL OF MATERIALS

B-1
 Sheet No. 3 of 8



STRICKLAND ROAD TOWN HOUSES

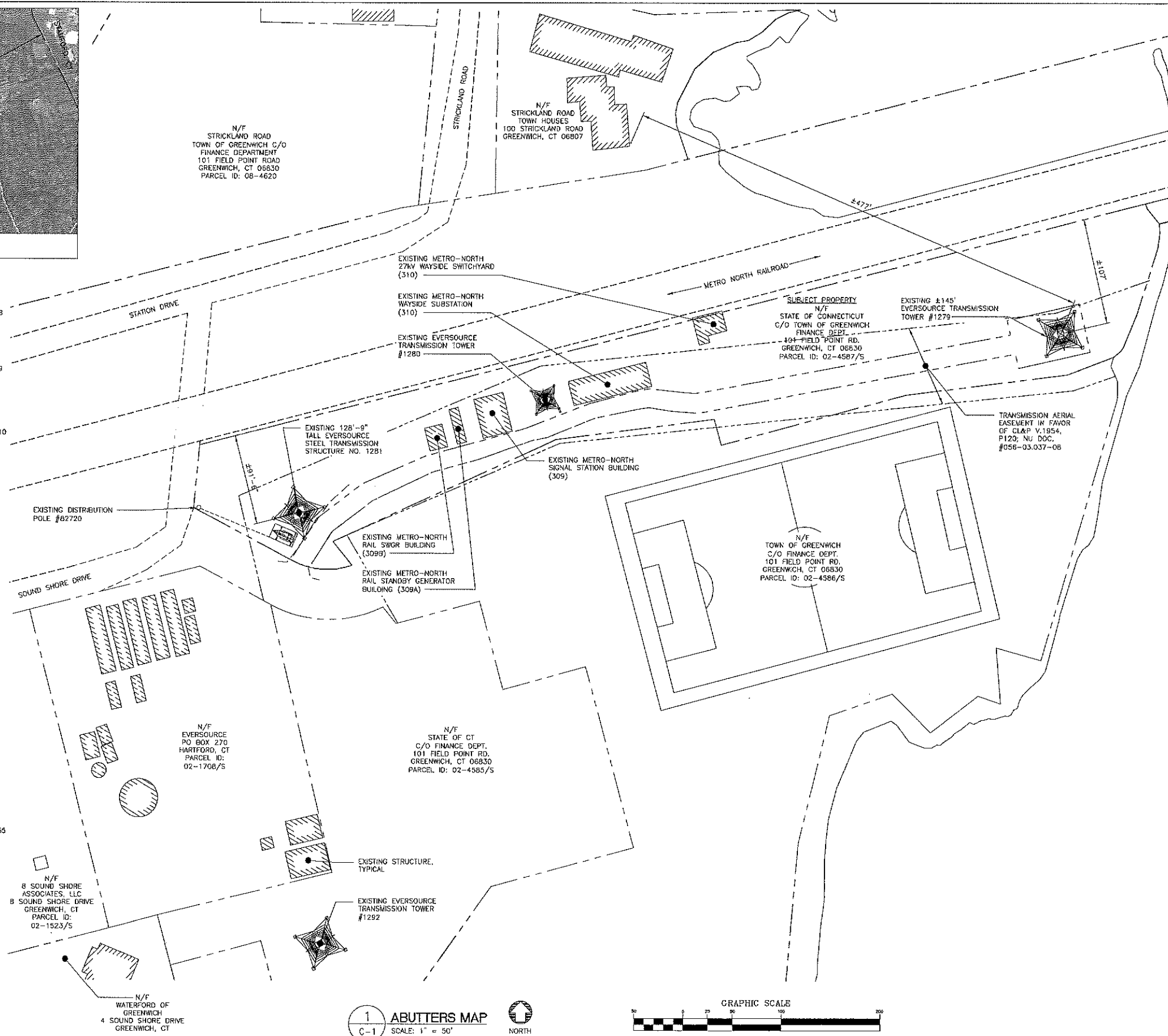
ABUTTERS INFORMATION:

1. N/F: THOMAS P. FONG
100 STRICKLAND ROAD, UNIT 4
GREENWICH, CT 06807
PARCEL ID: 08A-1722/S
MAILING ADDRESS:
68 LEWIS STREET
GREENWICH, CT 06830
2. N/F: GILIAN H. LEVY &
ALEXANDER FINKELSTEIN
100 STRICKLAND ROAD, UNIT 1
GREENWICH, CT 06807
PARCEL ID: 08A-1746/S
MAILING ADDRESS:
SAME
3. N/F: HARRY CHIN
100 STRICKLAND ROAD, UNIT 2
GREENWICH, CT 06807
PARCEL ID: 08A-1716/S
MAILING ADDRESS:
SAME
4. N/F: MARY ELLEN KAVANEWSKY
REVOCABLE TRUST
100 STRICKLAND ROAD, UNIT 3
GREENWICH, CT 06807
PARCEL ID: 08A-1717/S
MAILING ADDRESS:
102 ANGELSH LANE
JUPITER, FL 33477
5. N/F: JODI PELLEGRINO
100 STRICKLAND RD, UNIT 4
GREENWICH, CT 06807
PARCEL ID: 08A-1718/S
MAILING ADDRESS:
SAME
6. N/F: TFF DEVELOPMENT CORP.
100 STRICKLAND ROAD, UNIT 5
GREENWICH, CT 06807
PARCEL ID: 08A-1719/S
MAILING ADDRESS:
64 LEWIS STREET
GREENWICH, CT 06831
7. N/F: TFF DEVELOPMENT CORP.
100 STRICKLAND ROAD, UNIT 6
GREENWICH, CT 06807
PARCEL ID: 08A-1720/S
MAILING ADDRESS:
64 LEWIS STREET
GREENWICH, CT 06831
8. N/F: TFF DEVELOPMENT CORP.
100 STRICKLAND ROAD, UNIT 7
GREENWICH, CT 06807
PARCEL ID: 08A-1721/S
MAILING ADDRESS:
64 LEWIS STREET
GREENWICH, CT 06831
9. N/F: CHRISTINE M. GARDNER
100 STRICKLAND ROAD, UNIT 8
GREENWICH, CT 06807
PARCEL ID: 08A-1722/S
MAILING ADDRESS:
SAME
10. N/F: BRETT L. WILBUR
100 STRICKLAND ROAD, UNIT 9
GREENWICH, CT 06807
PARCEL ID: 08A-1723/S
MAILING ADDRESS:
SAME
11. N/F: CHRISTOPHER A. LEE &
ROBERT VAN CLAHN
100 STRICKLAND ROAD, UNIT 10
GREENWICH, CT 06807
PARCEL ID: 08A-1724/S
MAILING ADDRESS:
SAME

WATERFORD OF GREENWICH

ABUTTERS INFORMATION:

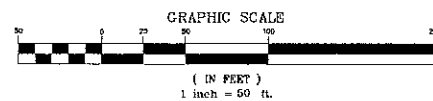
1. N/F: BRUCE L. WARKWICZ TRUST & ANN MARGARET WARKWICZ TRUST
4 SOUND SHORE DR, UNIT 1
GREENWICH, CT 06830
PARCEL ID: 02-1888/S
MAILING ADDRESS:
SAME
2. N/F: IGOR TULCHINSKY
4 SOUND SHORE DR, UNIT 2
GREENWICH, CT 06830
PARCEL ID: 02-1744/S
MAILING ADDRESS:
30 ARCH STREET
RIVERSIDE, CT 06878
3. N/F: TDC JR FAMILY INVESTMENT LLC II & CABOT - WELLINGTON, LLC
4 SOUND SHORE DR, UNIT 3
GREENWICH, CT 06830
PARCEL ID: 02-1745/S
MAILING ADDRESS:
70 FEDERAL STREET, 7TH FLOOR
BOSTON, MA 02110
4. N/F: ALICE P. MELLY & BENISLEY TRUSTEES
4 SOUND SHORE DR, UNIT 4
GREENWICH, CT 06830
PARCEL ID: 02-1746/S
MAILING ADDRESS:
SAME
5. N/F: VHC DEVELOPMENT CORP., & MOTT & PRINCE WOOD INC.
4 SOUND SHORE DR, UNIT 5
GREENWICH, CT 06830
PARCEL ID: 02-1747/S
MAILING ADDRESS:
5 BOWERY, 6/F
NEW YORK, NY 10013
6. N/F: LILLIAN C. ANDERSON
REVOCABLE TRUST
4 SOUND SHORE DR, UNIT 6
GREENWICH, CT 06830
PARCEL ID: 02-1748/S
MAILING ADDRESS:
PMB 8091, 6001 HIGHWAY A1A
INDIAN RIVER SHORES, FL 32963
7. N/F: ROBERT F. FULLER
REVOCABLE TRUST
4 SOUND SHORE DR, UNIT 7
GREENWICH, CT 06830
PARCEL ID: 02-1749/S
MAILING ADDRESS:
SAME
8. N/F: GENSLER ENTERPRISES, LLP
4 SOUND SHORE DR, UNIT 8
GREENWICH, CT 06830
PARCEL ID: 02-1750/S
MAILING ADDRESS:
2602 JUMPER COURT
PALM CITY, FL 34990
9. N/F: ROBERT H. CLARK, JR. & ROSALE C. CLARK
4 SOUND SHORE DR, UNIT 9
GREENWICH, CT 06830
PARCEL ID: 02-1751/S
MAILING ADDRESS:
521 FIFTH AVENUE, 36TH FLOOR
NEW YORK, NY 10175
10. N/F: DAVID J. MACDONALD & DONALD J. MACDONALD
4 SOUND SHORE DR, UNIT 10
GREENWICH, CT 06830
PARCEL ID: 02-1752/S
MAILING ADDRESS:
30 MAPLE DRIVE
NORTH CALDWELL, NJ 07006
11. N/F: HENRY VOLKARDER
4 SOUND SHORE DR, UNIT 11
GREENWICH, CT 06830
PARCEL ID: 02-1753/S
MAILING ADDRESS:
SAME
12. N/F: KELESHIAN INVESTMENTS, LLC
4 SOUND SHORE DR, UNIT 12
GREENWICH, CT 06830
PARCEL ID: 02-1754/S
MAILING ADDRESS:
SAME
13. N/F: HOLLANIN, LLC
4 SOUND SHORE DR, UNIT 13
GREENWICH, CT 06830
PARCEL ID: 02-1755/S
MAILING ADDRESS:
81217 OLD HIGHWAY
ISLAMRAPRA, FL 33036
14. N/F: MAUREN R. SMITH TRUST & JAMES J. RYAN TRUSTEE
4 SOUND SHORE DR, UNIT 14
GREENWICH, CT 06830
PARCEL ID: 02-1756/S
MAILING ADDRESS:
11 CARRIAGE HOUSE LANE
MAMARONECK, NY 10543
15. N/F: JAMES A. LASH & DEBORAH JONES / S
4 SOUND SHORE DR, UNIT 15
GREENWICH, CT 06830
PARCEL ID: 02-1757/S
MAILING ADDRESS:
25 STILES LANE
GREENWICH, CT 06831
16. N/F: LUCY F. GREENE TRUST
4 SOUND SHORE DR, UNIT 16
GREENWICH, CT 06830
PARCEL ID: 02-1758/S
MAILING ADDRESS:
SAME
17. N/F: LINDA M. BOOS
2004 REVOCABLE TRUST
4 SOUND SHORE DR, UNIT 17
GREENWICH, CT 06830
PARCEL ID: 02-1759/S
MAILING ADDRESS:
SAME
18. N/F: JONATHAN P. NELSON & LAURIE P. NELSON W/S
4 SOUND SHORE DR, UNIT 18
GREENWICH, CT 06830
PARCEL ID: 02-1760/S
MAILING ADDRESS:
SAME
19. N/F: FRED H. LANGHAMMER & REGINE LANGHAMMER W/S
4 SOUND SHORE DR, UNIT 19
GREENWICH, CT 06830
PARCEL ID: 02-1761/S
MAILING ADDRESS:
SAME
20. N/F: DONALD C. WAITE III
1998 LINDW TRUST
4 SOUND SHORE DR, UNIT 20
GREENWICH, CT 06830
PARCEL ID: 02-1762/S
MAILING ADDRESS:
24 DOCK SIDE LANE, PMB #155
KEY LARGO, FL 33037
21. N/F: 2121 WATERFOOD, LLC
4 SOUND SHORE DR, UNIT 21
GREENWICH, CT 06830
PARCEL ID: 02-1763/S
MAILING ADDRESS:
675 STEAMBOAT ROAD
GREENWICH, CT 06830
22. N/F: HEINDRICH REAL ESTATE INVESTMENTS, LLC
4 SOUND SHORE DR, UNIT 22
GREENWICH, CT 06830
PARCEL ID: 02-1764/S
MAILING ADDRESS:
SAME
23. N/F: PER HEINDRICH & ASTRID HEINDRICH W/S
4 SOUND SHORE DR, UNIT 23
GREENWICH, CT 06830
PARCEL ID: 02-1765/S
MAILING ADDRESS:
SAME
24. N/F: JOSEPH J. SHOPSHERE TR
4 SOUND SHORE DR, UNIT 24
GREENWICH, CT 06830
PARCEL ID: 02-1766/S
MAILING ADDRESS:
SAME



1
C-1

ABUTTERS MAP

SCALE: 1" = 50'



AT&T MOBILITY WIRELESS COMMUNICATIONS FACILITY COS COB RELO SITE NUMBER: CT5006 SOUND SHORE DRIVE GREENWICH, CT 06830	CENITEK engineering General contractors 1001 48th St 02019-488-9397 Fax 65-5 North Highland Road Framwood, CT 06435 www.CenitekEng.com				PROFESSIONAL ENGINEER SEAL									
					DATE: 02/21/18 SCALE: AS NOTED JOIN NO: 16009-03	ABUTTERS AND SITE ACCESS MAP	C-1	Sheet No. ⁶ of 8						

THIS SURVEY AND MAP HAS BEEN PREPARED IN ACCORDANCE WITH SECTIONS 20-300B-1 THRU 20-300B-20 OF THE REGULATIONS OF CONNECTICUT STATE AGENCIES - "MINIMUM STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT" AS ENDORSED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INC. ON SEPT. 26, 1996. THE LIMITED TOPOGRAPHIC SURVEY PORTION OF THIS PLAN CONFORMS TO A VERTICAL ACCURACY OF CLASS T-2 AND IS INTENDED TO BE USED TO DEPICT A PROPOSED TELECOMMUNICATION SITE.

COORDINATES REFER TO NAD 83

VERTICAL DATUM IS BASED ON NGVD 29

PARCEL OWNER OF RECORD: STATE OF CONNECTICUT

PARCEL AREA = 2.78 ACRES.

PARCEL IS IN R-6 ZONING DISTRICT.

TAX PARCEL NUMBER 02-4585/S.

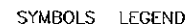
FLOOD ZONE LINE AS SHOWN ON THE FLOOD INSURANCE RATE
MAP, FAIRFIELD COUNTY, CONNECTICUT PANEL 513 OF 626, MAP
NUMBER 09001C0513GF, MAP EFFECTIVE DATE JUNE 8, 2013, BY
FEDERAL EMERGENCY MANAGEMENT AGENCY.

NOT ALL IMPROVEMENTS SHOWN.

REFERENCE IS MADE TO THE FOLLOWING MAPS:

1. MAP OF COS COB POWER PLANT, TOWN OF GREENWICH, SCALE
1"=50', DATED MAY 15, 1989, BY CONNECTICUT DEPARTMENT OF
TRANSPORTATION

2) PROPERTY SURVEY - "EXHIBIT C", DECLARATION OF ENVIRONMENTAL LAND USE RESTRICTION AND GRANT OF EASEMENT, TOWN OF GREENWICH, COS COB PARK, 22 SOUND SHORE DRIVE, GREENWICH, CONNECTICUT, SCALE 1"=50', DATED FEBRUARY 16, 2015, BY MILONE & MACBROOM.



	Drill Hole
	Manhole
	"CL" CB
	Round CB
	Deciduous Tree
WV 	Water Valve
WM 	Water Meter
	Guy Wire
	Property Line
	Contour

TO MY KNOWLEDGE AND BELIEF THIS MAP IS
SUBSTANTIALLY CORRECT AS NOTED HEREON

THIS MAP IS NOT VALID WITHOUT A LIVE SIGNATURE AND SEAL

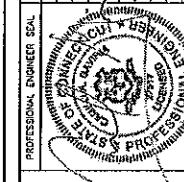
A. RAFAEL MARTINEZ LLS #18633

DATE _____

PROPOSED CONSTRUCTION EQUIPMENT

THE PROPOSED CONSTRUCTION EQUIPMENT SHALL BE AS FOLLOWS:

1. 150-FOOT MAN LIFT
2. 75-TON CRANE
3. 200-FOOT BRONTO SKYLIFT
4. ALL-WHEEL DRIVE FORKLIFT
5. STANDARD CONSTRUCTION VEHICLES
6. APPLICABLE EQUIPMENT FOR INSTALLATION OF PROPOSED TEMPORARY ACCESS DRIVE.

[illegible]

Center

MENTEN

203) 488-0580
203) 488-8587 Fax
43-2 North Branford Road
Branford, CT 06405

AT&T MOBILITY

A & MOBILITY
NATION'S COMMISSION FOR THE FUTURE

WIRELESS COMMUNICATIONS FACILITY
COS COB REL O

SITE NUMBER: CT5006

SOUND SHORE DRIVE

DATE: 02/21/1

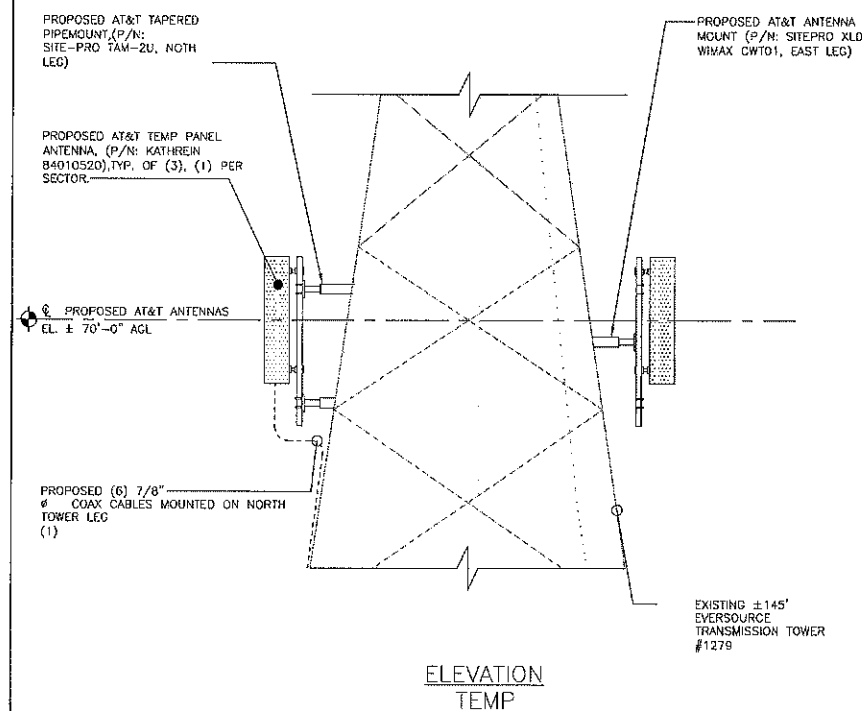
SCALE: AS NOTED

JOB NO. 16009.03

PARTIAL SITE PLAN

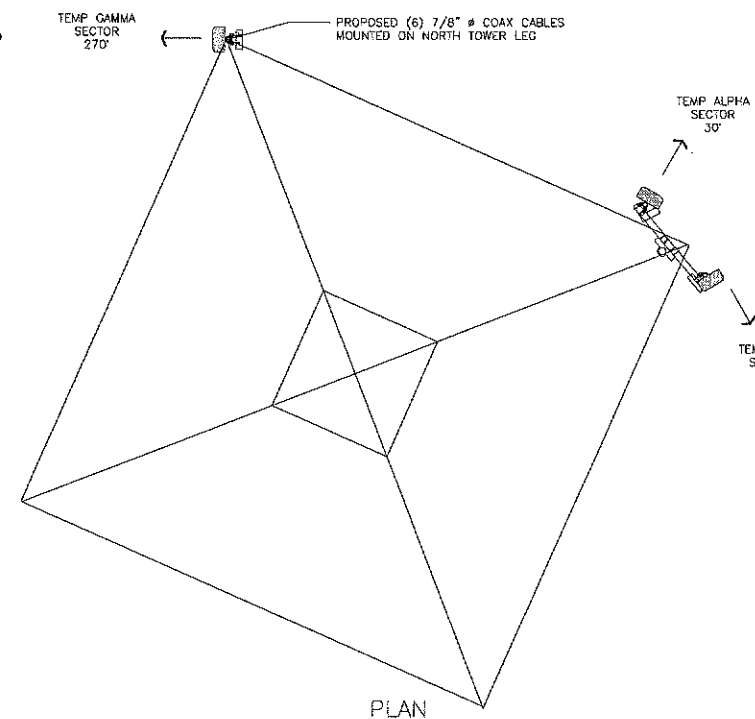
C-2

Sheet No. 5 of 8



ELEVATION
TEMP

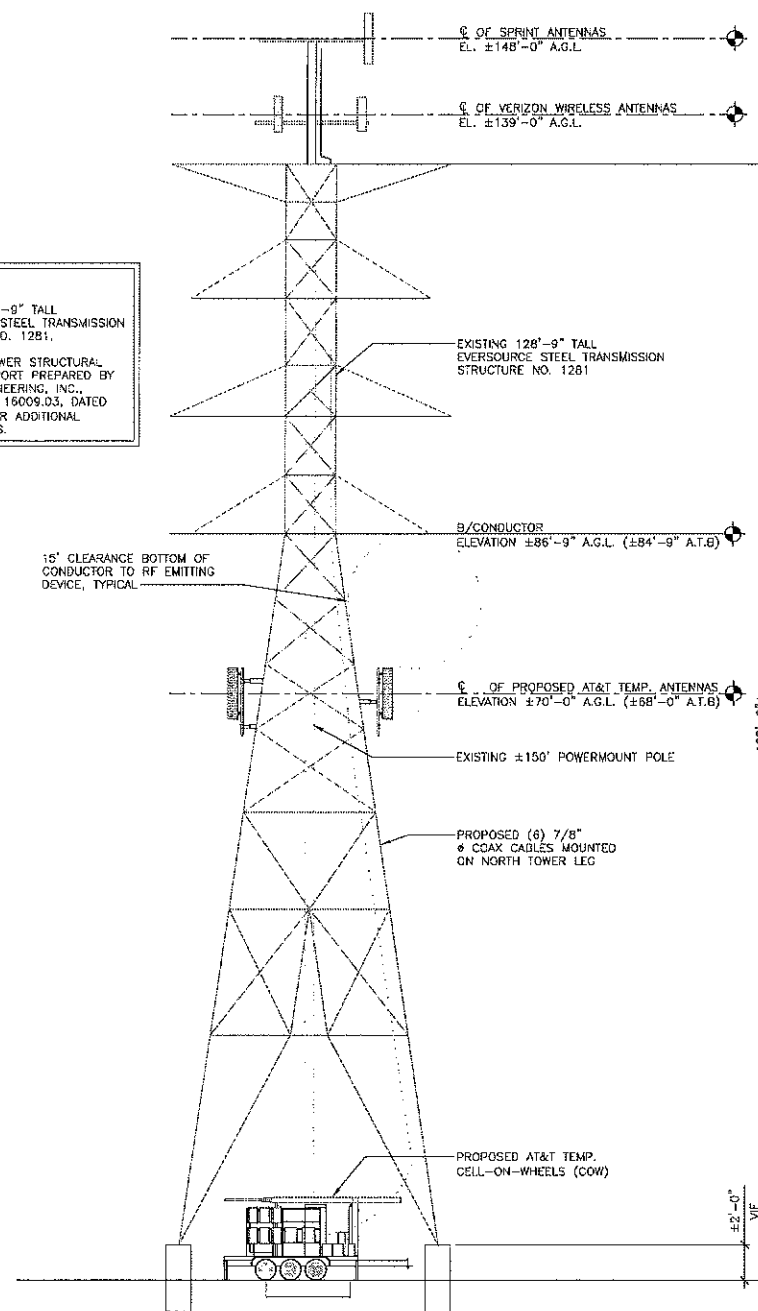
3 ANTENNA MOUNTING CONFIGURATION
C-3 SCALE: 1/4"=1'-0"



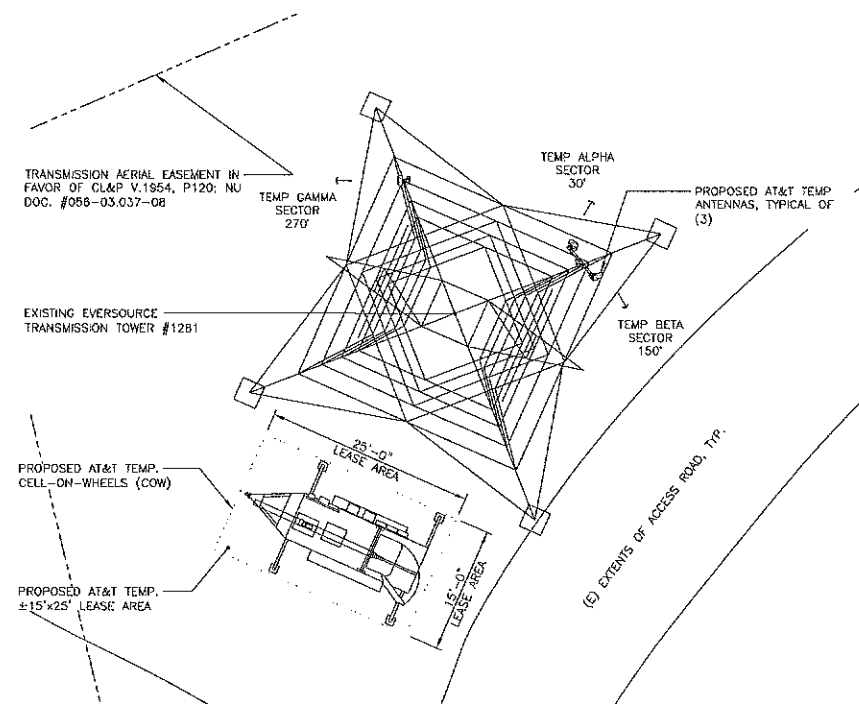
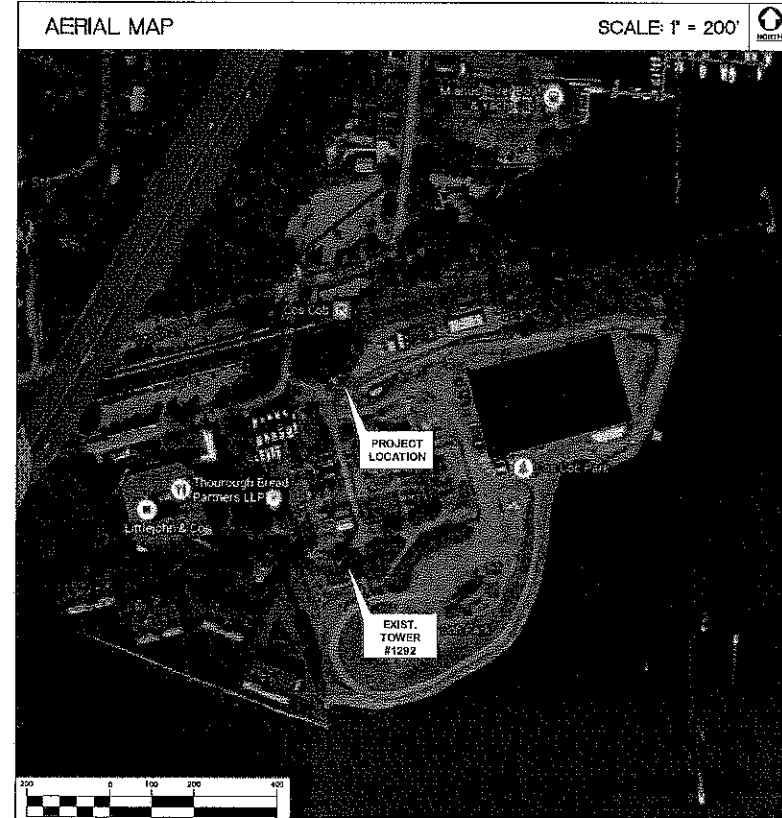
PLAN

TOWER NOTES:

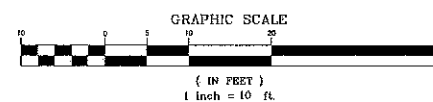
- EXISTING 128'-9" TALL EVERSOURCE STEEL TRANSMISSION STRUCTURE NO. 1281.
- REFER TO TOWER STRUCTURAL ANALYSIS REPORT PREPARED BY CENTEK ENGINEERING, INC., PROJECT NO. 16009.03, DATED 12/19/17 FOR ADDITIONAL REQUIREMENTS.



1 TOWER EAST ELEVATION
C-3 SCALE: 1"=10'

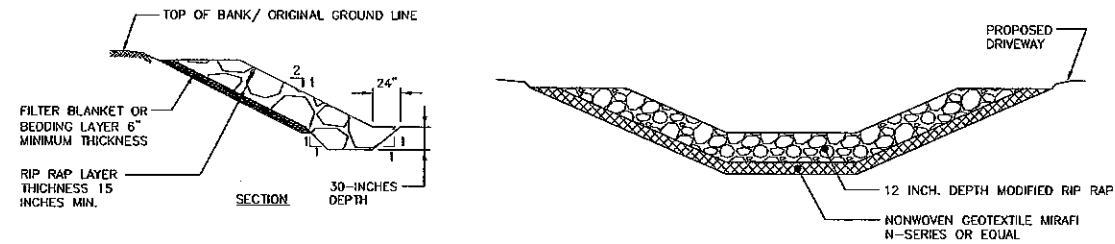


2 SITE COMPOUND PLAN
C-3 SCALE: 1"=10'



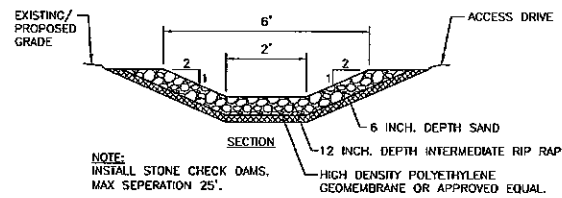
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DATE: 02/21/18		DRAWN BY: DKB	
SCALE: AS NOTED		DATE: 07/28/18	
JOB NO. 16009.03		REV.	
AT&T MOBILITY WIRELESS COMMUNICATIONS FACILITY COS COB RELO SITE NUMBER: CT5006 SOUND SHORE DRIVE GREENWICH, CT 06830			
C-3 Sheet No. 3 of 3			

RIP RAP STABILIZATION



3 RIP RAP SLOPE STABILIZATION
C-4 NOT TO SCALE

2 RIP RAP DRAINAGE SWALE STABILIZATION
C-4 NOT TO SCALE (TYPICAL)



5 RIP RAP SWALE
C-4 NOT TO SCALE

STONE SIZE	% OF MASS
10" AND OVER	0
6" TO 10"	30-50
4" TO 6"	30-50
2" TO 4"	20-30
1" TO 2"	10-20
LESS THAN 1"	0-10

STABILIZATION CRITERIA

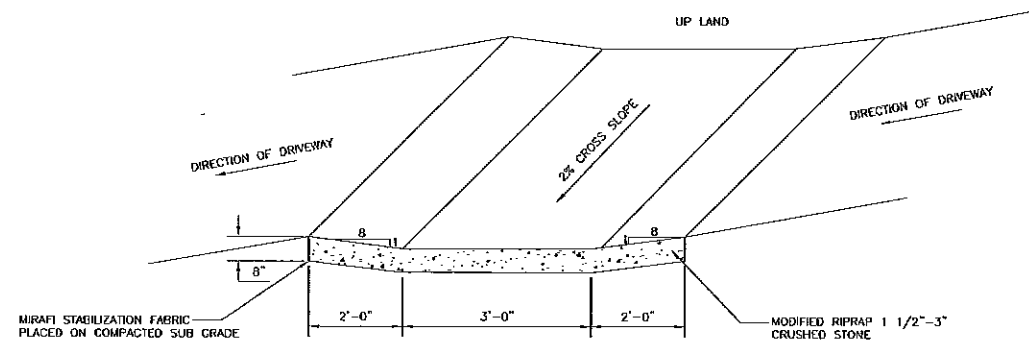
- CONTRACTOR SHALL IMPLEMENT RIP RAP STABILIZATION & SWALE CONSTRUCTION IN LOCATIONS WHERE LEDGE OR UNSTABLE SUBGRADES WITH LARGE AMOUNTS OF ROCK ARE PREVALENT OR AS SPECIFICALLY INDICATED ON THE PLANS.

RIP RAP ON SLOPES AND CHANNELS

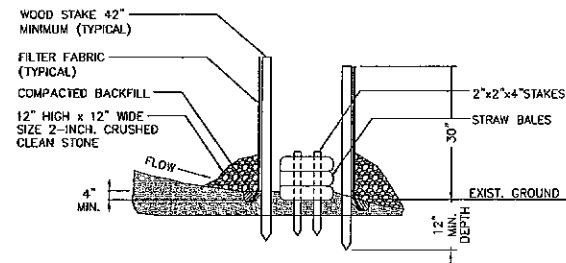
- PREPARE THE SUBGRADE FOR RIP RAP, BEDDING, FILTER OR GEOTEXTILE TO THE REQUIRED LINES AND GRADES. COMPACT ANY FILL REQUIRED IN THE SUBGRADE IN 12-INCHES LIFTS TO 95% OF STANDARD PROCTOR DENSITY. REMOVE BRUSH, TREES, STUMPS, AND OTHER OBJECTIONABLE MATERIAL.
- IMMEDIATELY AFTER SLOPE OR CHANNEL PREPARATION, INSTALL THE FILTER OR BEDDING MATERIALS. SPREAD THE FILTER OR BEDDING MATERIALS IN A UNIFORM LAYER TO THE SPECIFIED DEPTH.
- IMMEDIATELY AFTER PLACEMENT OF THE FILTER BLANKET, BEDDING, PLACE THE RIP RAP TO ITS FULL COURSE THICKNESS IN ONE OPERATION SO THAT IT PRODUCES A DENSE WELL GRADED MASS OF STONE WITH A MINIMUM OF VOIDS. THE DESIRED DISTRIBUTION OF STONES THROUGHOUT THE MASS MAY BE OBTAINED BY SELECTIVE LOADING AT THE QUARRY, CONTROLLED DUMPING OF SUCCESSIVE LOADS DURING THE FINAL PLACING, OR BY A COMBINATION OF THESE METHODS. DO NOT PLACE RIP RAP IN LAYERS OR USE CHUTES OR SIMILAR METHODS TO DUMP THE RIP RAP WHICH ARE LIKELY TO CAUSE SEGREGATION OF THE VARIOUS STONES.
- TAKE CARE NOT TO DISLODGE THE UNDERLYING MATERIAL WHEN PLACING THE STONES. WHEN PLACING RIP RAP ON A FILTER FABRIC TAKE CARE NOT TO DAMAGE THE FABRIC. IF DAMAGE OCCURS, REMOVE AND REPLACE THE DAMAGED SHEET. FOR LARGE STONE, 12 INCHES OR GREATER, USE A 6 INCH LAYER OF FILTER OR BEDDING MATERIAL TO PREVENT DAMAGE TO THE MATERIAL FROM PUNCTURE.
- ENSURE THE FINISHED SLOPE OR CHANNEL IS FREE OF POCKETS OF SMALL STONES OR CLUSTERS OF LARGE STONES. HAND PLACING MAY BE NECESSARY TO ACHIEVE THE REQUIRED GRADES AND A GOOD DISTRIBUTION OF STONE SIZES. ENSURE THE FINAL THICKNESS OF THE RIP RAP BLANKET IS WITHIN PLUS OR MINUS 0.25 OF THE SPECIFIED THICKNESS.

MAINTENANCE

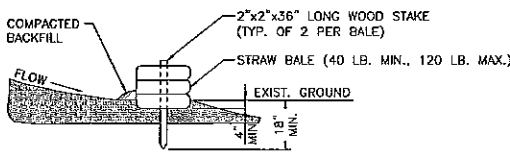
VERIZON WIRELESS SHALL PERIODICALLY INSPECT RIP RAP STABILIZED SLOPES & CHANNELS DETERMINE IF HIGH FLOWS HAVE CAUSED SCOUR BENEATH THE RIP RAP OR FILTER BLANKET MATERIALS. REMOVE TREES THAT DEVELOP IN THE PROTECTED SLOPES.



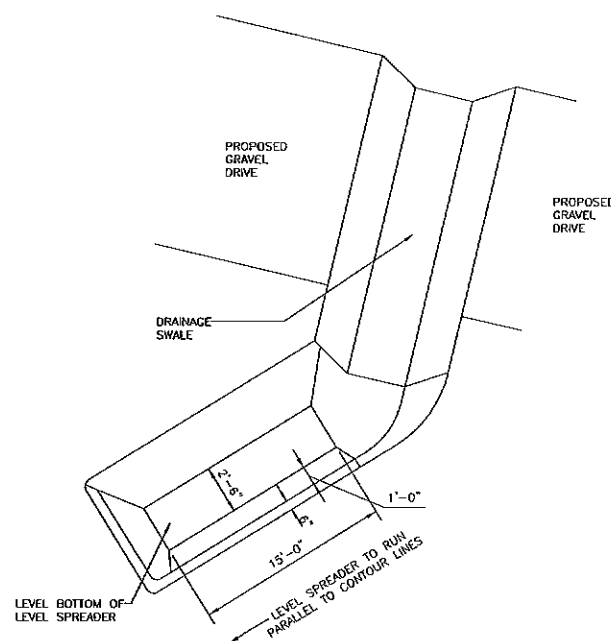
7 CROSS DRAINAGE SWALE
C-4 NOT TO SCALE



1 SILTATION FENCE/STRAW BALE SILTATION FENCE 'SANDWICH' EROSION CONTROL
C-4 NOT TO SCALE



4 TYP. STRAW BALE BARRIER EROSION CONTROL DETAIL
C-4 NOT TO SCALE



ARMORED STONE SURFACE SHALL BE FREE OF ORGANICS AND CONSIST SOLELY OF CRUSHED GRAVEL WITH THE FOLLOWING GRADATION:

SIEVE: 3 1/2"	% PASSING: 100
SIEVE: 2"	% PASSING: 35-70
SIEVE: 1 1/2"	% PASSING: 0-15
SIEVE: 1/2"	% PASSING: 0-5
SIEVE: NO. 4	% PASSING: 0

6 LEVEL SPREADER W/ CROSS DRAINAGE SWALE
C-4 NOT TO SCALE

GENERAL CONSTRUCTION / PRE-CONSTRUCTION NOTES

- PRIOR TO COMMENCEMENT OF ANY CONSTRUCTION ACTIVITIES, A MANDATORY ON-SITE PRE-CONSTRUCTION MEETING SHALL BE CONDUCTED WITH THE VERIZON WIRELESS CONSTRUCTION MANAGER, CONTRACTOR'S CONSTRUCTION MANAGER, THE PROJECT EROSION AND SEDIMENTATION CONTROL/ENVIRONMENTAL MONITOR AND THE ENGINEER OF RECORD.

GENERAL CONSTRUCTION SEQUENCE

THIS IS A GENERAL CONSTRUCTION SEQUENCE OUTLINE SOME ITEMS OF WHICH MAY NOT APPLY TO PARTICULAR SITES.

- CUT AND STUMP AREAS OF PROPOSED CONSTRUCTION.
- INSTALL TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES AS REQUIRED.
- REMOVE AND STOCKPILE TOPSOIL. STOCKPILE SHALL BE SEED TO PREVENT EROSION.
- CONSTRUCT CLOSED DRAINAGE SYSTEM. PRECEPT CULVERT INLETS AND CATCH BASINS WITH SEDIMENTATION BARRIERS.
- CONSTRUCT ROADWAYS AND PERFORM SITE GRADING, PLACING HAY BALES AND SITUATION FENCES AS REQUIRED TO CONTROL SOIL EROSION.
- INSTALL UNDERGROUND UTILITIES.
- BEGIN TEMPORARY AND PERMANENT SEEDING AND MULCHING. ALL CUT AND FILL SLOPES SHALL BE SEED OR MULCHED IMMEDIATELY AFTER THEIR CONSTRUCTION. NO AREA SHALL BE LEFT UNSTABILIZED FOR A TIME PERIOD OF MORE THAN 30 DAYS.
- DAILY, OR AS REQUIRED, CONSTRUCT, INSPECT, AND IF NECESSARY, RECONSTRUCT TEMPORARY BERMS, DRAINS, DITCHES, SILT FENCES AND SEDIMENT TRAPS INCLUDING MULCHING AND SEEDING.
- BEGIN EXCAVATION FOR AND CONSTRUCTION OF TOWERS AND PLATFORMS.
- FINISH PAVING ALL ROADWAYS, DRIVES, AND PARKING AREAS.
- COMPLETE PERMANENT SEEDING AND LANDSCAPING.
- NO FLOW SHALL BE DIVERTED TO ANY WETLANDS UNTIL A HEALTHY STAND OF GRASS HAS BEEN ESTABLISHED IN REGARDED AREAS.
- AFTER GRASS HAS BEEN FULLY GERMINATED IN ALL SEEDER AREAS, REMOVE ALL TEMPORARY EROSION CONTROL MEASURES.

SOIL EROSION AND SEDIMENT CONTROL SEQUENCE

- ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES, SUCH AS CONSTRUCTION ENTRANCE / ANTI TRACKING PAD, SILTATION FENCE, AND SITUATION FENCE / STRAW BALE SHALL BE IN PLACE PRIOR TO ANY GRADING ACTIVITY, INSTALLATION OF PROPOSED STRUCTURES OR UTILITIES. MEASURES SHALL BE LEFT IN PLACE AND MAINTAINED UNTIL CONSTRUCTION IS COMPLETED AND/OR AREA IS STABILIZED.
- THE ENTRANCE TO THE PROJECT SITE IS TO BE PROTECTED BY STONE ANTI TRACKING PAD OF ASTM Z-53, SIZE NO. 2 OR 3, OR D.O.T. 2" CRUSHED GRAVEL. THE STONE ANTI TRACKING PAD IS TO BE MAINTAINED AT ALL TIMES DURING THE CONSTRUCTION PERIOD.
- LAND DISTURBANCE WILL BE KEPT TO A MINIMUM AND RESTABILIZATIONS WILL BE SCHEDULED AS SOON AS PRACTICAL.
- ALL SOIL EROSION AND SEDIMENT CONTROL WORK SHALL BE DONE IN STRICT ACCORDANCE WITH THE CONNECTICUT GUIDELINES FOR EROSION AND SEDIMENT CONTROL INCLUDING THE LATEST DATE FROM THE COUNCIL ON SOIL AND WATER CONSERVATION.
- ANY ADDITIONAL EROSION/SEDIMENTATION CONTROL DEEMED NECESSARY BY TOWN STAFF DURING CONSTRUCTION, SHALL BE INSTALLED BY THE DEVELOPER. IN ADDITION, THE DEVELOPER SHALL BE RESPONSIBLE FOR THE REPAIR/REPLACEMENT/MAINTENANCE OF ALL EROSION CONTROL MEASURES UNTIL ALL DISTURBED AREAS ARE STABILIZED TO THE SATISFACTION OF THE TOWN STAFF.
- IN ALL AREAS, REMOVAL OF TREES, BUSHES AND OTHER VEGETATION AS WELL AS DISTURBANCE OF THE SOIL IS TO BE KEPT TO AN ABSOLUTE MINIMUM WHILE ALLOWING PROPER DEVELOPMENT OF THE SITE. DURING CONSTRUCTION, EXPOSE AS SMALL AN AREA OF SOIL AS POSSIBLE FOR AS SHORT A TIME AS POSSIBLE.
- SILTATION FENCE SHALL BE PLACED AS INDICATED BEFORE A CUT SLOPE HAS BEEN CREATED. SEDIMENT DEPOSITS SHOULD BE PERIODICALLY REMOVED FROM THE UPSTREAM SIDES OF SILTATION FENCE. THIS MATERIAL IS TO BE SPREAD AND STABILIZED IN AREAS NOT SUBJECT TO EROSION, OR TO BE USED IN AREAS WHICH ARE NOT TO BE PAVED OR BUILT ON. SILTATION FENCE IS TO BE REPLACED AS NECESSARY TO PROVIDE PROPER FILTERING ACTION. THE FENCE IS TO REMAIN IN PLACE AND BE MAINTAINED TO INSURE EFFICIENT SILTATION CONTROL UNTIL ALL AREAS ABOVE THE EROSION CHECKS ARE STABILIZED AND VEGETATION HAS BEEN ESTABLISHED.
- SWALE DISCHARGE AREA WILL BE PROTECTED WITH RIP RAP SPLASH PAD/ ENERGY DISSIPATER.
- ALL FILL AREAS SHALL BE COMPACTED SUFFICIENTLY FOR THEIR INTENDED PURPOSE AND AS REQUIRED TO REDUCE SLIPPING, EROSION OR EXCESS SATURATION.
- THE SOIL SHALL NOT BE PLACED WHILE IN A FROZEN OR MUDDY CONDITION, WHEN THE SUBGRADE IS EXCESSIVELY WET, OR IN A CONDITION THAT MAY OTHERWISE BE DETRIMENTAL TO PROPER GRADING OR PROPOSED SODDING OR SEEDING.
- AFTER CONSTRUCTION IS COMPLETE AND GROUND IS STABLE, REMOVE SILTS IN THE RIP RAP ENERGY DISSIPATERS. REMOVE OTHER EROSION AND SEDIMENT DEVICES.

CONSTRUCTION SPECIFICATIONS - SILT FENCE

- THE GEOTEXTILE FABRIC SHALL MEET THE DESIGN CRITERIA FOR SILT FENCES.
- THE FABRIC SHALL BE EMBEDDED A MINIMUM OF 8 INCHES INTO THE GROUND AND THE SOIL COMPACTED OVER THE EMBEDDED FABRIC.
- WOVEN WIRE FENCE SHALL BE FASTENED SECURELY TO THE FENCE POSTS WITH WIRE TIES OR STAPLES.
- FILTER CLOTH SHALL BE FASTENED SECURELY TO THE WOVEN WIRE FENCE WITH TIES SPACED EVERY 24 INCHES AT THE TOP, MID-SECTION AND BOTTOM.
- WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER, THEY SHALL BE OVERLAPPED BY 6 INCHES, FOLDED, AND STAPLED.
- FENCE POSTS SHALL BE A MINIMUM OF 36 INCHES LONG AND DRIVEN A MINIMUM OF 16 INCHES INTO THE GROUND. WOOD POSTS SHALL BE OF SOUND QUALITY HARDWOOD AND SHALL HAVE A MINIMUM CROSS SECTIONAL AREA OF 3.0 SQUARE INCHES.
- MAINTENANCE SHALL BE PERFORMED AS NEEDED TO PREVENT BUILD UP IN THE SILT FENCE DUE TO DEPOSITION OF SEDIMENT.

MAINTENANCE - SILT FENCE

- SILT FENCES SHALL BE INSPECTED IMMEDIATELY AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL. ANY REPAIRS THAT ARE REQUIRED SHALL BE MADE IMMEDIATELY.
- IF THE FABRIC ON A SILT FENCE SHOULD DECOMPOSE OR BECOME INEFFECTIVE DURING THE EXPECTED LIFE OF THE FENCE, THE FABRIC SHALL BE REPLACED PROMPTLY.
- SEDIMENT SHOULD BE INSPECTED AFTER EVERY STORM EVENT. THE DEPOSITS SHOULD BE REMOVED WHEN THEY REACHED APPROXIMATELY ONE-HALF THE HEIGHT OF THE BARRIER.
- SEDIMENT DEPOSITS THAT ARE REMOVED OR LEFT IN PLACE AFTER THE FABRIC HAS BEEN REMOVED SHALL BE GRADED TO CONFORM WITH THE EXISTING TOPOGRAPHY AND VEGETATED.

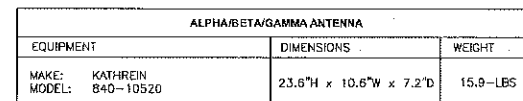
PROFESSIONAL ENGINEER SEAL
VERIZON WIRELESS CONSTRUCTION FACILITY
COS COB RELO
SITE NUMBER: CT5006
SOUND SHORE DRIVE
GREENWICH, CT 06830

AT&T MOBILITY
WIRELESS COMMUNICATIONS FACILITY
COS COB RELO
SITE NUMBER: CT5006
SOUND SHORE DRIVE
GREENWICH, CT 06830

DATE: 02/21/18
SCALE: AS NOTED
JOB NO. 15009.03

SITE CONSTRUCTION, S&E CONTROL
NOTES AND DETAILS

C-4
Sheet No. 7 of 8



5" THICK CRUSHED STONE SURFACE ON STABILIZATION FABRIC.

GRADE TO DRAIN AS SHOWN ON PLAN

MARAFI STABILIZATION FABRIC PLACED ON COMPACTED SUBGRADE.

CHAINLINK FENCE. SEE SEPARATE DETAIL.

LIMITS OF CRUSHED STONE SURFACE.

2" X 6" PRESSURE TREATED WOOD CURB.

GRADE TO DRAIN AWAY FROM TOWER YARD.

2" X 4" X 1'-6" LONG PRESERVATIVE PRESSURE TREATED WOOD STAKE AT 5'-0" O.C.

1'-0"

2 COMPOUND SURFACING DETAIL
C-5 NOT TO SCALE

0	09/28/18	FIP	CAC	CONSTRUCTION DOCUMENTS - ISSUED FOR CONSTRUCTION
				



CENITEK engineering
Centers on solutions™

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Branford, CT 06405

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AT&T MOBILITY
WIRELESS COMMUNICATIONS FACILITY
COS COB RELOC
SITE NUMBER: CT5006
SOUND SHORE DRIVE
GREENWICH, CT 06830

DATE:	02/21/18
SCALE:	AS NOTED
JOB NO.	16009.03

TYPICAL DETAILS

C-5

Sheet No. 8 of 8

Structural Analysis of
Eversource Tower

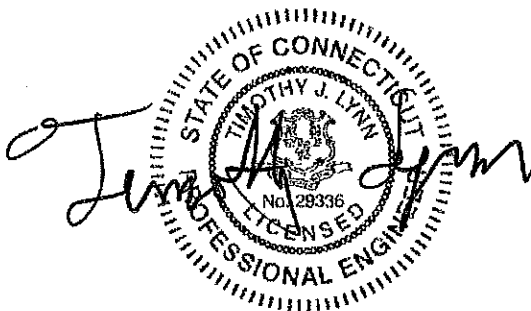
AT&T Site Ref: CT5006 - Temp

Structure No. 1281
129' Electric Transmission Lattice Tower

9 Sound Shore Drive
Greenwich, CT

CEN TEK Project No. 16009.03

Date: December 19, 2017



Prepared for:
AT&T Mobility
500 Enterprise Drive, Suite 3A
Rocky Hill, CT 06067

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Introduction

The purpose of this report is to analyze the existing 128.75' utility tower located at 9 Sound Shore Drive in Greenwich, CT for the temp proposed antenna and equipment installation by AT&T.

The loads considered in this analysis consist of the following:

- **VERIZON WIRELESS (Existing):**
Antennas: Two (2) Decibel DB854DG65ESX panel antennas, three (3) Andrew HBXX-6516DS panel antennas, three (3) Andrew SBNHH-1D65B panel antennas, one (1) Antel BXA-70063-6CF, six (6) RFS FDAP5002/2C-3L Diplexers and three (3) RFS ATSBT-TOP-FM-4G Bias Tee mounted on a (3) T-Arms with a RAD center elevation of 139-ft above grade.
Coax Cables: Eighteen (18) 1-5/8" Ø coax cables running on the outside of the powermount as indicated in section 4 of this report
- **SPRINT (Existing/Reserved):**
Antennas: Three (3) RFS APXVSP18-C panel antennas mounted on an existing 14-ft low profile platform to the powermount with a RAD center elevation of 148-ft above grade.
Coax Cables: Six (6) 1-5/8" Ø coax cables running on the inside of the existing powermount. Twelve (12) 1-5/8" Ø coax cables mounted on Site Pro Super Universal T-Brackets p/n T1200 running on a leg of the existing tower as indicated in section 4 of this report.
- **AT&T MOBILITY (Proposed – Temp Loading):**
Antennas: One (1) Kathrein 840-10520 panel antenna mounted on one (1) Site Pro Tapered Pipe Mount p/n TAM-2U with a RAD center elevation of 70-ft above grade (North Leg). Two (2) Kathrein 840-10520 panel antennas mounted on one (1) Site Pro XLD WIMAX Tower mount p/n CWT01 with a RAD center elevation of 70-ft above grade (East Leg).
Coax Cables: Six (6) 7/8" Ø coax cables running on the North tower leg.

Primary assumptions used in the analysis

- Allowable steel stresses are defined by AISC-ASD 9th edition for design of the Powermount and antenna supporting elements.
- ASCE Manual No. 10-97, "Design of Latticed Steel Transmission Structures", defines allowable steel stresses for evaluation of the CL&P utility tower.
- All utility tower members are adequately protected to prevent corrosion of steel members.
- All proposed antenna mounts are modeled as listed above.
- All coaxial cable will be installed within the powermount unless specified otherwise.
- Powermount will be properly installed and maintained.
- No residual stresses exist due to incorrect tower erection.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds conform to the requirements of AWS D1.1.
- Powermount and utility tower will be in plumb condition.
- Utility tower was properly installed and maintained and all members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
- Any deviation from the analyzed loading will require a new analysis for verification of structural adequacy.

A n a l y s i s

Structural analysis of the existing CL&P tower structure was completed using the current version of PLS-Tower computer program licensed to CENTEK Engineering, Inc. The NESC program contains a library of all AISC angle shapes and corresponding section properties are computed and applied directly within the program. The program's Steel Code Check option was also utilized.

The existing 129-ft tall CL&P lattice tower was analyzed for its ability to resist loads prescribed by the NESC standard. Maximum usage for the tower was calculated considering the additional forces from the powermount and associated appurtenances. Section 7 of this report details these gravity and lateral wind loads.

D e s i g n B a s i s

Our analysis was performed in accordance with EIA-222-F-1996, ASCE Manual No. 10-97, "Design of Latticed Steel Transmission Structures", NESC C2-2007 and Eversource Design Criteria.

The CL&P tower structure, considering existing and future conductor and shield wire loading, with the proposed antenna mast was analyzed under two conditions:

▪ UTILITY TOWER ANALYSIS

The purpose of this analysis is to determine the adequacy of the existing utility structure to support the proposed antenna loads. The loading and design requirements were analyzed in accordance with the EVERSOURCE Design Criteria Table, NESC C2-2007 ~ Construction Grade B, and ASCE Manual No. 10-97, "Design of Latticed Steel Transmission Structures".

Load cases considered:

Load Case 1: NESC Heavy

Wind Pressure.....	4.0 psf
Radial Ice Thickness.....	0.5"
Vertical Overload Capacity Factor.....	1.50
Wind Overload Capacity Factor.....	2.50
Wire Tension Overload Capacity Factor.....	1.65

Load Case 2: NESC Extreme

Wind Speed.....	110 mph ⁽¹⁾
Radial Ice Thickness.....	0"

Note 1: NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading, 1.25 x Gust Response Factor (wind speed: 3-second gust)

Results

■ UTILITY TOWER

This analysis finds that the subject utility structure is adequate to support the proposed antenna mast and related appurtenances. The tower stresses meet the requirements set forth by the ASCE Manual No. 10-97, "Design of Latticed Steel Transmission Structures", for the applied NESC Heavy and Hi-Wind load cases. The detailed analysis results are provided in Section 8 of this report. The analysis results are summarized as follows:

A maximum usage of **90.98%** occurs in the utility structure under the **NESC Extreme** loading condition.

TOWER SECTION:

The utility structure was found to be within allowable limits.

Tower Member	Stress Ratio (% of capacity)	Result
Angle g3P	90.98%	PASS

■ FOUNDATION AND ANCHORS

The existing foundation consists of a 5-ft square x 8.5-ft long reinforced concrete pier with eight (8) rock anchor groups embedded 12-ft into rock. The base of the tower is connected to the foundation by four (4) 2.00" $\varnothing$ A36 bolts per leg. Foundation information was obtained from NUSCO drawing no. 01037-60010.

BASE REACTIONS:

From PLS-Tower analysis of CL&P tower based on NESC/EVERSOURCE prescribed loads.

Load Case	Shear	Uplift	Compression
NESC Heavy Wind	47.34 kips	188.28 kips	226.21 kips
NESC Extreme Wind	56.57 kips	200.63 kips	207.29 kips

Note 1 – 10% increase to be applied to the above tower base reactions for foundation verification per OTRM 051

ANCHOR BOLTS:

The anchor bolts were found to be within allowable limits.

Component	Design Check	Stress Ratio (percentage of capacity)	Result
Anchor Bolts	Tension	93.0%	PASS

CEN TEK Engineering, Inc.
Structural Analysis – 129-ft CL&P Tower # 1281
AT&T Antenna Upgrade – CT5006 Temp
Greenwich, CT
December 19, 2017

FOUNDATION:

The foundation was found to be within allowable limits.

Foundation	Design Check	Design Limit	Proposed Loading	Result
Reinf. Conc. Pier w/ Rock Anchors	Uplift	1.0 FS ⁽²⁾	1.21 FS ⁽²⁾	PASS
	OTM ⁽¹⁾	1.0 FS ⁽²⁾	1.04 FS ⁽²⁾	PASS
	Soil Bearing	50 ksf	32.19 ksf	PASS

Note 1: OTM denote overturning moment.
Note 2: FS denotes Factor of Safety.

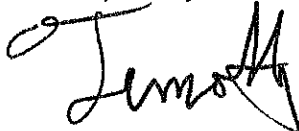
Conclusion

This analysis shows that the subject utility tower with is adequate to support the proposed Temporary AT&T equipment installation.

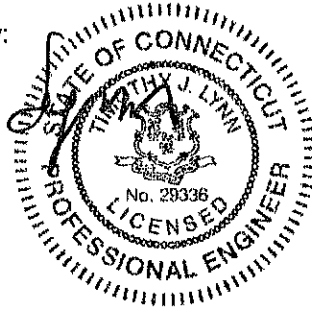
The analysis is based, in part on the information provided to this office by Eversource. If the existing conditions are different than the information in this report, CENTEK engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:



Timothy J. Lynn, PE
Structural Engineer



CEN TEK Engineering, Inc.
Structural Analysis – 129-ft CL&P Tower # 1281
AT&T Antenna Upgrade – CT5006 Temp
Greenwich, CT
December 19, 2017

STANDARD CONDITIONS FOR FURNISHING OF
PROFESSIONAL ENGINEERING SERVICES ON
EXISTING STRUCTURES

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CEN TEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to CEN TEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the "as new" condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222.
- All services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. CEN TEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ RISA - 3 D

RISA-3D Structural Analysis Program is an integrated structural analysis and design software package for buildings, bridges, tower structures, etc.

Modeling Features:

- Comprehensive CAD-like graphic drawing/editing capabilities that let you draw, modify and load elements as well as snap, move, rotate, copy, mirror, scale, split, merge, mesh, delete, apply, etc.
- Versatile drawing grids (orthogonal, radial, skewed)
- Universal snaps and object snaps allow drawing without grids
- Versatile general truss generator
- Powerful graphic select/unselect tools including box, line, polygon, invert, criteria, spreadsheet selection, with locking
- Saved selections to quickly recall desired selections
- Modification tools that modify single items or entire selections
- Real spreadsheets with cut, paste, fill, math, sort, find, etc.
- Dynamic synchronization between spreadsheets and views so you can edit or view any data in the plotted views or in the spreadsheets
- Simultaneous view of multiple spreadsheets
- Constant in-stream error checking and data validation
- Unlimited undo/redo capability
- Generation templates for grids, disks, cylinders, cones, arcs, trusses, tanks, hydrostatic loads, etc.
- Support for all units systems & conversions at any time
- Automatic interaction with RISASection libraries
- Import DXF, RISA-2D, STAAD and ProSteel 3D files
- Export DXF, SDNF and ProSteel 3D files

Analysis Features:

- Static analysis and P-Delta effects
- Multiple simultaneous dynamic and response spectra analysis using Gupta, CQC or SRSS mode combinations
- Automatic inclusion of mass offset (5% or user defined) for dynamic analysis
- Physical member modeling that does not require members to be broken up at intermediate joints
- State of the art 3 or 4 node plate/shell elements
- High-end automatic mesh generation — draw a polygon with any number of sides to create a mesh of well-formed quadrilateral (NOT triangular) elements.
- Accurate analysis of tapered wide flanges - web, top and bottom flanges may all taper independently
- Automatic rigid diaphragm modeling
- Area loads with one-way or two-way distributions
- Multiple simultaneous moving loads with standard AASHTO loads and custom moving loads for bridges, cranes, etc.
- Torsional warping calculations for stiffness, stress and design
- Automatic Top of Member offset modeling
- Member end releases & rigid end offsets
- Joint master-slave assignments
- Joints detachable from diaphragms
- Enforced joint displacements
- 1-Way members, for tension only bracing, slipping, etc.

CEN TEK Engineering, Inc.
Structural Analysis -- 129-ft CL&P Tower # 1281
AT&T Antenna Upgrade -- CT5006 Temp
Greenwich, CT
December 19, 2017

- 1-Way springs, for modeling soils and other effects
- Euler members that take compression up to their buckling load, then turn off.
- Stress calculations on any arbitrary shape
- Inactive members, plates, and diaphragms allows you to quickly remove parts of structures from consideration
- Story drift calculations provide relative drift and ratio to height
- Automatic self-weight calculations for members and plates
- Automatic subgrade soil spring generator

Graphics Features:

- Unlimited simultaneous model view windows
- Extraordinary "true to scale" rendering, even when drawing
- High-speed redraw algorithm for instant refreshing
- Dynamic scrolling stops right where you want
- Plot & print virtually everything with color coding & labeling
- Rotate, zoom, pan, scroll and snap views
- Saved views to quickly restore frequent or desired views
- Full render or wire-frame animations of deflected model and dynamic mode shapes with frame and speed control
- Animation of moving loads with speed control
- High quality customizable graphics printing

Design Features:

- Designs concrete, hot rolled steel, cold formed steel and wood
- ACI 1999/2002, BS 8110-97, CSA A23.3-94, IS456:2000, EC 2-1992 with consistent bar sizes through adjacent spans
- Exact integration of concrete stress distributions using parabolic or rectangular stress blocks
- Concrete beam detailing (Rectangular, T and L)
- Concrete column interaction diagrams
- Steel Design Codes: AISC ASD 9th, LRFD 2nd & 3rd, HSS Specification, CAN/CSA-S16.1-1994 & 2004, BS 5950-1-2000, IS 800-1984, Euro 3-1993 including local shape databases
- AISI 1999 cold formed steel design
- NDS 1991/1997/2001 wood design, including Structural Composite Lumber, multi-ply, full sawn
- Automatic spectra generation for UBC 1997, IBC 2000/2003
- Generation of load combinations: ASCE, UBC, IBC, BOCA, SBC, ACI
- Unbraced lengths for physical members that recognize connecting elements and full lengths of members
- Automatic approximation of K factors
- Tapered wide flange design with either ASD or LRFD codes
- Optimization of member sizes for all materials and all design codes, controlled by standard or user-defined lists of available sizes and criteria such as maximum depths
- Automatic calculation of custom shape properties
- Steel Shapes: AISC, HSS, CAN, ARBED, British, Euro, Indian, Chilean
- Light Gage Shapes: AISI, SSMA, Dale / Incor, Dietrich, Marino\WARE
- Wood Shapes: Complete NDS species/grade database
- Full seamless integration with RISAFoot (Ver 2 or better) for advanced footing design and detailing
- Plate force summation tool

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Structural Analysis -- 129-ft CL&P Tower # 1281
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Greenwich, CT
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Results Features:

- Graphic presentation of color-coded results and plotted designs
- Color contours of plate stresses and forces with quadratic smoothing, the contours may also be animated
- Spreadsheet results with sorting and filtering of: reactions, member & joint deflections, beam & plate forces/stresses, optimized sizes, code designs, concrete reinforcing, material takeoffs, frequencies and mode shapes
- Standard and user-defined reports
- Graphic member detail reports with force/stress/deflection diagrams and detailed design calculations and expanded diagrams that display magnitudes at any dialed location
- Saved solutions quickly restore analysis and design results.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ PLS-TOWER

PLS-TOWER is a Microsoft Windows program for the analysis and design of steel latticed towers used in electric power lines or communication facilities. Both self-supporting and guyed towers can be modeled. The program performs design checks of structures under user specified loads. For electric power structures it can also calculate maximum allowable wind and weight spans and interaction diagrams between different ratios of allowable wind and weight spans.

Modeling Features:

- Powerful graphics module (stress usages shown in different colors)
- Graphical selection of joints and members allows graphical editing and checking
- Towers can be shown as lines, wire frames or can be rendered as 3-d polygon surfaces
- Can extract geometry and connectivity information from a DXF CAD drawing
- CAD design drawings, title blocks, drawing borders or photos can be tied to structure model
- XML based post processor interface
- Steel Detailing Neutral File (SDNF) export to link with detailing packages
- Can link directly to line design program PLS-CADD
- Automatic generation of structure files for PLS-CADD
- Databases of steel angles, rounds, bolts, guys, etc.
- Automatic generation of joints and members by symmetries and interpolations
- Automated mast generation (quickly builds model for towers that have regular repeating sections) via graphical copy/paste
- Steel angles and rounds modeled either as truss, beam or tension-only elements
- Guys are easily handled (can be modeled as exact cable elements)

Analysis Features:

- Automatic handling of tension-only members
- Automatic distribution of loads in 2-part suspension insulators (v-strings, horizontal vees, etc.)
- Automatic calculation of tower dead, ice, and wind loads as well as drag coefficients according to:
 - ASCE 74-1991
 - NESC 2002
 - NESC 2007
 - IEC 60826:2003
 - EN50341-1:2001 (CENELEC)
 - EN50341-3-9:2001 (UK NNA)
 - EN50341-3-17:2001 (Portugal NNA)
 - ESAA C(b)1-2003 (Australia)
 - TPNZ (New Zealand)
 - REE (Spain)
 - EIA/TIA 222-F
 - ANSI/TIA 222-G
 - CSA S37-01
- Automated microwave antenna loading as per EIA/TIA 222-F and ANSI/TIA 222-G
- Minimization of problems caused by unstable joints and mechanisms
- Automatic bandwidth minimization and ability to solve large problems
- Design checks according to (other standards can be added easily):
 - ASCE Standard 10-90

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Structural Analysis -- 129-ft CL&P Tower # 1281
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Greenwich, CT
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- AS 3995 (Australian Standard 3995)
- BS 8100 (British Standard 8100)
- EN50341-1 (CENELEC, both empirical and analytical methods are available)
- ECCS 1985
- NGT-ECCS
- PN-90/B-03200
- EIA/TIA 222-F
- ANSI/TIA 222-G
- CSA S37-01
- EDF/RTE ResaI
- IS 802 (India Standard 802)

Results Features:

- Design summaries printed for each group of members
 - Easy to interpret text, spreadsheet and graphics design summaries
 - Automatic determination of allowable wind and weight spans
 - Automatic determination of interaction diagrams between allowable wind and weight spans
 - Capability to batch run multiple tower configurations and consolidate the results
 - Automated optimum angle member size selection and bolt quantity determination
- Tool for interactive angle member sizing and bolt quantity determination.

Criteria for Design of PCS Facilities On or
Extending Above Metal Electric Transmission
Towers & Analysis of Transmission Towers
Supporting PCS Masts ⁽¹⁾

Introduction

This criteria is the result from an evaluation of the methods and loadings specified by the separate standards, which are used in designing telecommunications towers and electric transmission towers. That evaluation is detailed elsewhere, but in summary; the methods and loadings are significantly different. This criteria specifies the manner in which the appropriate standard is used to design PCS facilities including masts and brackets (hereafter referred to as "masts"), and to evaluate the electric transmission towers to support PCS masts. The intent is to achieve an equivalent level of safety and security under the extreme design conditions expected in Connecticut and Massachusetts.

ANSI Standard TIA/EIA-222 (Rev. F) covering the design of telecommunications structures specifies a working strength/allowable stress design approach. This approach applies the loads from extreme weather loading conditions, and designs the structure so that it does not exceed some defined percentage of failure strength (allowable stress).

ANSI Standard C2-2007 (National Electrical Safety Code) covering the design of electric transmission metal structures is based upon an ultimate strength/yield stress design approach. This approach applies a multiplier (overload capacity factor) to the loads possible from extreme weather loading conditions, and designs the structure so that it does not exceed its ultimate strength (yield stress).

Each standard defines the details of how loads are to be calculated differently. Most of the NU effort in "unifying" both codes was to establish what level of strength each approach would provide, and then increasing the appropriate elements of each to achieve a similar level of security under extreme weather loadings.

Two extreme weather conditions are considered. The first is an extreme wind condition (hurricane) based upon a 50-year recurrence (2% annual probability). The second is a winter condition combining wind and ice loadings.

The following sections describe the design criteria for any PCS mast extending above the top of an electric transmission tower, and the analysis criteria for evaluating the loads on the transmission tower from such a mast from the lower portions of such a mast, and loads on the pre-existing electric lower portions of such a mast, and loads on the pre-existing electric transmission tower and the conductors it supports.

Note 1: Prepared from documentation provide from Northeast Utilities.

PCS Mast

The PCS facility (mast, external cable/trays, including the initial and any planned future support platforms, antennas, etc. extending the full height above the top level of the electric transmission structure) shall be designed in accordance with the provisions of TIA/EIA-222 (Rev. F) with two exceptions:

1. An 85 mph extreme wind speed shall be used for locations in all counties throughout the NU system.
2. The allowable stress increase of TIA Section 3.1.1.1 is allowed for the mast section, but is disallowed for the mast to structure connection design.

The combined wind and ice condition shall consider $\frac{1}{2}$ " radial ice in combination with the wind load (0.75 Wi) as specified in TIA section 2.3.16.

ELECTRIC TRANSMISSION TOWER

The electric transmission tower shall be analyzed using yield stress theory in accordance with the attached table titled "NU Design Criteria". This specifies uniform loadings (different from the TIA loadings) on the each of the following components of the installed facility:

- PCS mast for its total height above ground level, including the initial and planned future support platforms, antennas, etc. above the top of an electric transmission structure.
- Conductors are related devices and hardware.
- Electric transmission structure. The loads from the PCS facility and from the electric conductors shall be applied to the structure at conductor and PCS mast attachment points, where those load transfer to the tower.

The uniform loadings and factors specified for the above components in the table are based upon the National Electrical Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to TIA and its loads and factors with the exceptions noted above. (Note that the NESC does not require the projected wind surfaces of structures and equipment to be increased by the ice covering.)

In the event that the electric transmission tower is not sufficient to support the additional loadings of the PCS mast, reinforcement will be necessary to upgrade the strength of the overstressed members.



Attachment A

NU Design Criteria

		Basic Wind Speed	Pressure	Height Factor	Gust Factor	Load or Stress Factor	Force Coef - Shape Factor
		V (MPH)	Q (PSF)	Kz	Gh		
Ice Condition	TIA/EIA	Antenna Mount	TIA	TIA (.75Wi)	TIA	TIA, Section 3.1.1.1 disallowed for connection design	TIA
	NESC Heavy	Tower/Pole Analysis with antennas extending above top of Tower/Pole (Yield Stress)	4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole (on two faces)	4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor loads provided by NU				
High Wind Condition	TIA/EIA	Antenna Mount	85	TIA	TIA	TIA, Section 3.1.1.1 disallowed for connection design	TIA
	NESC Extreme Wind	Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna				1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading Height above ground level based on top of Tower/Pole				1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor loads provided by NU				
NESC Extreme Ice with Wind Condition*	Tower/Pole Analysis with antennas extending above top of Tower/Pole		Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna				1.6 Flat Surfaces 1.3 Round Surfaces
	Tower/Pole Analysis with Antennas below top of Tower/Pole		Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load Height above ground level based on top of Tower/Pole				1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor loads provided by NU				

* Only for Structures Installed after 2007

Communication Antennas on Transmission Structures (CL&P & WMECo Only)

Northeast Utilities
Approved by: KMS (NU)

Design
NU Confidential Information

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Shape Factor Criteria shall be per TIA Shape Factors.

- 2) STEP 2 - The electric transmission structure analysis and evaluation shall be performed in accordance with NESC requirements and shall include the mast and antenna loads determined from NESC applied loading conditions (not TIA/EIA Loads) on the structure and mount as specified below, and shall include the wireless communication mast and antenna loads per NESC criteria)

The structure shall be analyzed using yield stress theory in accordance with Attachment A, "NU Design Criteria." This specifies uniform loadings (different from the TIA loadings) on each of the following components of the installed facility:

- a) Wireless communication mast for its total height above ground level, including the initial and any planned future equipment (Support Platforms, Antennas, TMA's etc.) above the top of an electric transmission structure.
- b) Conductors and related devices and hardware (wire loads will be provided by NU).
- c) Electric Transmission Structure
 - i) The loads from the wireless communication equipment components based on NESC and NU Criteria in Attachment A, and from the electric conductors shall be applied to the structure at conductor and wireless communication mast attachment points, where those loads transfer to the tower.
 - ii) Shape Factor Multiplier:

NESC Structure Shape	Cd
Polyround (for polygonal steel poles)	1.3
Flat	1.6
Open Lattice	3.2

- iii) When Coaxial Cables are mounted along side the pole structure, the shape multiplier shall be:

Mount Type	Cable Cd	Pole Cd
Coaxial Cables on outside periphery (One layer)	1.45	1.45
Coaxial Cables mounted on stand offs	1.6	1.3

- d) The uniform loadings and factors specified for the above components in Attachment A, "NU Design Criteria" are based upon the National Electric Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to the TIA and its loads and factors with the exceptions noted above.

Note: The NESC does not require ice load be included in the supporting structure. (Ice on conductors and shield wire only, and NU will provide these loads).

- e) Mast reaction loads shall be evaluated for local effects on the transmission structure members at the attachment points.

Communication Antennas on Transmission Structures (CL&P & WMECo Only)



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Description:

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INPUT DATA

TOWER ID: 1281

Structure Height (ft) : 129

Wind Zone : Central CT (green)

Wind Speed : 110 mph

Tower Type : ☐ Suspension
☒ Strain

Extreme Wind Model : PCS Addition

Shield Wire Properties:

	BACK	AHEAD
NAME =	NONE	NONE
DESCRIPTION =	-	-
STRANDING =	--	--
DIAMETER =	0.000 in	0.000 in
WEIGHT =	0.000 lb/ft	0.000 lb/ft

Conductor Properties:

	BACK	AHEAD
NAME =	BITTERN	BITTERN
Number of Conductors per phase	1	1
	1272.000	1272.000
	45/7 ACSR	45/7 ACSR
DIAMETER =	1.345 in	1.345 in
WEIGHT =	1.432 lb/ft	1.432 lb/ft

Insulator Weight = 0 lbs

Broken Wire Side = AHEAD SPAN

Horizontal Line Tensions:

	BACK		AHEAD	
	Shield	Conductor	Shield	Conductor
NESC HEAVY =	na	6,000	na	6,000
EXTREME WIND =	na	5,621	na	4,646
LONG. WIND =	na	na	na	na
250D COMBINED =	na	na	na	na
NESC W/O OLF =	na	na	na	na
60 DEG F NO WIND =	na	2,421	na	2,092

Line Geometry:

	BACK:	45	AHEAD:	45	SUM
LINE ANGLE (deg) =	BACK:	141	AHEAD:	91	232
WIND SPAN (ft) =	BACK:	39	AHEAD:	588	627
WEIGHT SPAN (ft) =					



Job :
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WIRE LOADING AT ATTACHMENTS

TOWER ID: 1281

Wind Span = 232 ft
Weight Span = 627 ft
Total Angle = 90 degrees

Broken Wire Span = AHEAD SPAN
Type of Insulator Attachment = STRAIN

1. NESC RULE 250B Heavy Loading:

	INTACT CONDITION			BROKEN WIRE CONDITION		
	Horizontal	Longitudinal	Vertical	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	292 lb	#VALUE!	#VALUE!	18 lb
Conductor =	14,454 lb	0 lb	2,426 lb	7,276 lb	7,000 lb	151 lb

2. NESC RULE 250C Transverse Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	0 lb
Conductor =	9,332 lb	793 lb	1,033 lb

3. NESC RULE 250C Longitudinal Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	0 lb
Conductor =	#VALUE!	#VALUE!	1,033 lb

4. NESC RULE 250D Extreme Ice & Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	780 lb
Conductor =	#VALUE!	#VALUE!	2,726 lb

5. NESC RULE 250B w/o OLF's

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	195 lb
Conductor =	#VALUE!	#VALUE!	1,617 lb

6. 60 Deg. F. No Wind

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	0 lb
Conductor =	3,191 lb	233 lb	898 lb

7. Construction

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	0 lb
Conductor =	4,787 lb	349 lb	1,347 lb



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NOTE: All loads include required overload factors (OLF's).

LC 1		HORIZONTAL	LONGITUDINAL	VERTICAL
NESC Heavy	shield - back	#VALUE!	#VALUE!	18.18687622
	shield - ahead	#VALUE!	#VALUE!	274.2021338
	SHIELD - SUM	#VALUE!	#VALUE!	292.38901
	conductor - back	7275.894634	7000.357134	150.8815733
	conductor - ahead	7178.1863	-7000.357134	2274.829874
	CONDUCTOR - SUM	14454.08093	0	2425.711447
LC 2		HORIZONTAL	LONGITUDINAL	VERTICAL
Extreme Wind	shield - back	#VALUE!	#VALUE!	0
	shield - ahead	#VALUE!	#VALUE!	0
	SHIELD - SUM	#VALUE!	#VALUE!	0
	conductor - back	5168.274228	4570.8443	64.2252
	conductor - ahead	4163.576165	-3778.000821	968.3184
	CONDUCTOR - SUM	9331.850393	792.8434784	1032.5436
LC 3		HORIZONTAL	LONGITUDINAL	VERTICAL
Long. Wind	shield - back	#VALUE!	#VALUE!	0
	shield - ahead	#VALUE!	#VALUE!	0
	SHIELD - SUM	#VALUE!	#VALUE!	0
	conductor - back	#VALUE!	#VALUE!	64.2252
	conductor - ahead	#VALUE!	#VALUE!	968.3184
	CONDUCTOR - SUM	#VALUE!	#VALUE!	1032.5436
LC 4		HORIZONTAL	LONGITUDINAL	VERTICAL
RULE 250D	shield - back	#VALUE!	#VALUE!	48.49833659
	shield - ahead	#VALUE!	#VALUE!	731.2056901
	SHIELD - SUM	#VALUE!	#VALUE!	779.7040267
	conductor - back	#VALUE!	#VALUE!	169.5765993
	conductor - ahead	#VALUE!	#VALUE!	2556.693343
	CONDUCTOR - SUM	#VALUE!	#VALUE!	2726.269943
LC 5		HORIZONTAL	LONGITUDINAL	VERTICAL
NESC w/o OLF's	shield - back	#VALUE!	#VALUE!	12.12458415
	shield - ahead	#VALUE!	#VALUE!	182.8014225
	SHIELD - SUM	#VALUE!	#VALUE!	194.9260067
	conductor - back	#VALUE!	#VALUE!	100.5877155
	conductor - ahead	#VALUE!	#VALUE!	1516.553249
	CONDUCTOR - SUM	#VALUE!	#VALUE!	1617.140965
LC 6		HORIZONTAL	LONGITUDINAL	VERTICAL
Raking	shield - back	#VALUE!	#VALUE!	0
	shield - ahead	#VALUE!	#VALUE!	0
	SHIELD - SUM	#VALUE!	#VALUE!	0
	conductor - back	1711.905517	1711.905517	55.848
	conductor - ahead	1479.267386	-1479.267386	842.016
	CONDUCTOR - SUM	3191.172903	232.638131	897.864
LC 6		HORIZONTAL	LONGITUDINAL	VERTICAL
60 DEG F NO WIND	shield - back	#VALUE!	#VALUE!	0
	shield - ahead	#VALUE!	#VALUE!	0
	SHIELD - SUM	#VALUE!	#VALUE!	0
	conductor - back	2567.858276	2567.858276	83.772
	conductor - ahead	2218.901079	-2218.901079	1263.024
	CONDUCTOR - SUM	4786.759355	348.9571965	1346.796



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INPUT DATA

TOWER ID: 1281

Structure Height (ft) : 129

Wind Zone : Central CT (green)

Wind Speed : 110 mph

Tower Type : ☐ Suspension
☒ Strain

Extreme Wind Model : PCS Addition

Shield Wire Properties:

	BACK	AHEAD
NAME =	NONE	NONE
DESCRIPTION =	-	-
STRANDING =	-	-
DIAMETER =	0.000 in	0.000 in
WEIGHT =	0.000 lb/ft	0.000 lb/ft

SW (SHIELD WIRE)

Conductor Properties:

	BACK	AHEAD		
NAME =	LINNET	LINNET		
Number of Conductors per phase	336	336	1	Number of Conductors per phase
	26/7 ACSR	26/7 ACSR		
DIAMETER =	0.720 in	0.720 in		
WEIGHT =	0.462 lb/ft	0.462 lb/ft		

Insulator Weight = 0 lbs

Broken Wire Side = AHEAD SPAN

Horizontal Line Tensions:

	BACK SW	AHEAD SW
NESC HEAVY =	na 4,000	na 4,000
EXTREME WIND =	na 3,449	na 3,005
LONG. WIND =	na na	na na
250D COMBINED =	na na	na na
NESC W/O OLF =	na na	na na
60 DEG F NO WIND =	na 2,194	na 1,830

Line Geometry:

	BACK:	45	AHEAD:	45	SUM
LINE ANGLE (deg) =	BACK:	141	AHEAD:	91	232
WIND SPAN (ft) =	BACK:	39	AHEAD:	588	627
WEIGHT SPAN (ft) =					



Job :

Description:

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WIRE LOADING AT ATTACHMENTS

TOWER ID: 1281

Wind Span = 232 ft
Weight Span = 627 ft
Total Angle = 90 degrees

Broken Wire Span = AHEAD SPAN
Type of Insulator Attachment = STRAIN

1. NESC RULE 250B Heavy Loading:

	INTACT CONDITION			BROKEN WIRE CONDITION		
	Horizontal	Longitudinal	Vertical	Horizontal	Longitudinal	Vertical
SW Shield Wire =	#VALUE!	#VALUE!	292 lb	#VALUE!	#VALUE!	18 lb
Conductor =	9,666 lb	0 lb	1,148 lb	4,869 lb	4,667 lb	71 lb

2. NESC RULE 250C Transverse Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
SW Shield Wire =	#VALUE!	#VALUE!	0 lb
Conductor =	5,774 lb	361 lb	333 lb

3. NESC RULE 250C Longitudinal Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	0 lb
Conductor =	#VALUE!	#VALUE!	333 lb

4. NESC RULE 250D Extreme Ice & Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	780 lb
Conductor =	#VALUE!	#VALUE!	1,631 lb

5. NESC RULE 250B w/o OLF's

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	195 lb
Conductor =	#VALUE!	#VALUE!	765 lb

6. 60 Deg. F. No Wind

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	0 lb
Conductor =	2,845 lb	257 lb	290 lb

7. Construction

	Horizontal	Longitudinal	Vertical
SW Shield Wire =	#VALUE!	#VALUE!	0 lb
Conductor =	4,268 lb	386 lb	435 lb



Northeast
Utilities System

Job :

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NOTE: All loads include required overload factors (OLF's).

LC 1		HORIZONTAL	LONGITUDINAL	VERTICAL
NESC Heavy	shield - back	#VALUE!	#VALUE!	3.18687622
	shield - ahead	#VALUE!	#VALUE!	274.202338
	SHIELD - SUM	#VALUE!	#VALUE!	292.38901
	conductor - back	4869.004756	4666.904756	71.40297798
	conductor - ahead	4797.338089	-4666.904756	1076.537206
	CONDUCTOR - SUM	9666.342845	0	1147.940184
LC 2		HORIZONTAL	LONGITUDINAL	VERTICAL
Extreme Wind	shield - back	#VALUE!	#VALUE!	0
	shield - ahead	#VALUE!	#VALUE!	0
	SHIELD - SUM	#VALUE!	#VALUE!	0
	conductor - back	3124.446772	2804.632982	20.7207
	conductor - ahead	2649.988904	-2443.584259	312.4044
	CONDUCTOR - SUM	5774.435676	361.0487225	333.1251
LC 3		HORIZONTAL	LONGITUDINAL	VERTICAL
Long. Wind	shield - back	#VALUE!	#VALUE!	0
	shield - ahead	#VALUE!	#VALUE!	0
	SHIELD - SUM	#VALUE!	#VALUE!	0
	conductor - back	#VALUE!	#VALUE!	20.7207
	conductor - ahead	#VALUE!	#VALUE!	312.4044
	CONDUCTOR - SUM	#VALUE!	#VALUE!	333.1251
LC 4		HORIZONTAL	LONGITUDINAL	VERTICAL
RULE 250D	shield - back	#VALUE!	#VALUE!	48.49833659
	shield - ahead	#VALUE!	#VALUE!	731.2056901
	SHIELD - SUM	#VALUE!	#VALUE!	779.7040267
	conductor - back	#VALUE!	#VALUE!	101.4351389
	conductor - ahead	#VALUE!	#VALUE!	1529.329787
	CONDUCTOR - SUM	#VALUE!	#VALUE!	1630.764926
LC 5		HORIZONTAL	LONGITUDINAL	VERTICAL
NESC w/o OLF's	shield - back	#VALUE!	#VALUE!	12.12458415
	shield - ahead	#VALUE!	#VALUE!	182.8014225
	SHIELD - SUM	#VALUE!	#VALUE!	194.9260067
	conductor - back	#VALUE!	#VALUE!	47.60198532
	conductor - ahead	#VALUE!	#VALUE!	717.691471
	CONDUCTOR - SUM	#VALUE!	#VALUE!	765.2934563
LC 6		HORIZONTAL	LONGITUDINAL	VERTICAL
Raking	shield - back	#VALUE!	#VALUE!	0
	shield - ahead	#VALUE!	#VALUE!	0
	SHIELD - SUM	#VALUE!	#VALUE!	0
	conductor - back	1551.392278	1551.392278	18.018
	conductor - ahead	1294.00541	-1294.00541	271.656
	CONDUCTOR - SUM	2845.397687	257.3868684	289.674
LC 6		HORIZONTAL	LONGITUDINAL	VERTICAL
60 DEG F NO WIND	shield - back	#VALUE!	#VALUE!	0
	shield - ahead	#VALUE!	#VALUE!	0
	SHIELD - SUM	#VALUE!	#VALUE!	0
	conductor - back	2327.088417	2327.088417	27.027
	conductor - ahead	1941.008114	-1941.008114	407.484
	CONDUCTOR - SUM	4268.096531	386.0803025	434.511

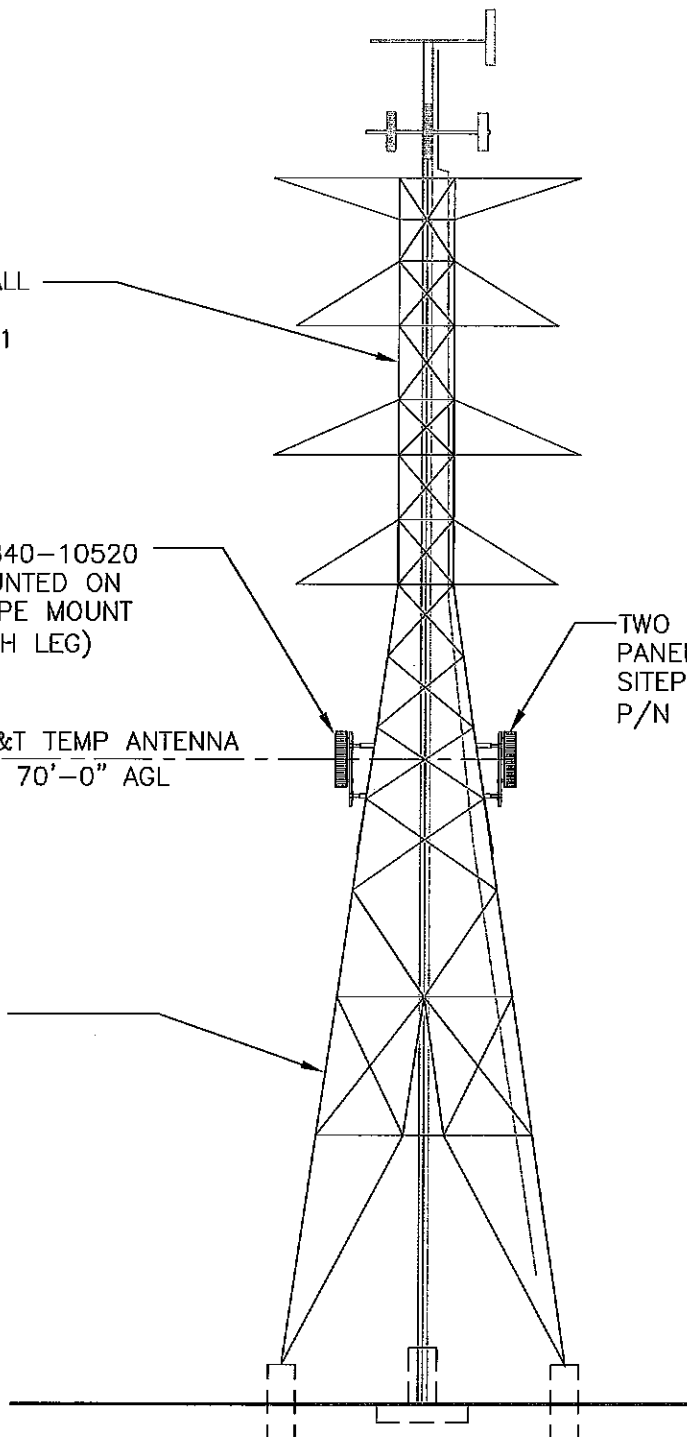
EXISTING 128'-9" TALL
STEEL TRANSMISSION
STRUCTURE NO. 1281

ONE (1) KATHREIN 840-10520
PANEL ANTENNA MOUNTED ON
SITEPRO TAPERED PIPE MOUNT
P/N TAM-2U. (NORTH LEG)

⊕ AT&T TEMP ANTENNA
EL. ± 70'-0" AGL

TWO (2) KATHREIN 840-10520
PANEL ANTENNAS MOUNTED ON
SITEPRO XLD WIMAX TOWER MOUNT
P/N CWT01. (EAST LEG)

PROPOSED (6) 7/8"
DIA. COAX CABLES
MOUNTED ON NORTH
TOWER LEG



1
EL-1

TOWER + MAST ELEVATION

SCALE: NOT TO SCALE

REVISIONS

00	12/19/17	ISSUED FOR REVIEW

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SOUND SHORE DRIVE
GREENWICH, CT 06830

PROJECT NO: 16009.003

DRAWN BY: TJL

CHECKED BY: CFC

SCALE: AS NOTED

DATE: 12/19/17

TOWER
ELEVATION

EL-1

DWG. 1 OF 1

Basic Components

Heavy Wind Pressure =	p := 4.00	psf	(User Input NESC 2007 Figure 250-1 & Table 250-1)
Basic Windspeed =	V := 110	mph	(User Input NESC 2007 Figure 250-2(e))
Radial Ice Thickness =	Ir := 0.50	in	(User Input)
Radial Ice Density =	Id := 56.0	pcf	(User Input)

Factors for Extreme Wind Calculation

Elevation of Top of PCS Mast Above Grade =	TME := 148	ft	(User Input)
Multiplier Gust Response Factor =	m := 1.25		(User Input - Only for NESC Extreme wind case)
NESC Factor =	kv := 1.43		(User Input from NESC 2007 Table 250-3 equation)
Importance Factor =	I := 1.0		(User Input from NESC 2007 Section 250.C.2)
Velocity Pressure Coefficient =	$K_z := 2.01 \cdot \left(\frac{TME}{900} \right)^{\frac{2}{9.5}} = 1.375$		(NESC 2007 Table 250-2)
Exposure Factor =	$E_s := 0.348 \left[\frac{33}{(0.67 \cdot TME)} \right]^{\frac{1}{7}} = 0.296$		(NESC 2007 Table 250-3)
Response Term =	$B_s := \frac{1}{\left(1 + 0.375 \cdot \frac{TME}{220} \right)} = 0.799$		(NESC 2007 Table 250-3)
Gust Response Factor =	$Grf := \frac{\left[1 + \left(2.7 \cdot E_s \cdot B_s^{\frac{1}{2}} \right) \right]}{kv^2} = 0.838$		(NESC 2007 Table 250-3)
Wind Pressure =	$q_z := 0.00256 \cdot K_z \cdot V^2 \cdot Grf \cdot I = 35.7$	psf	(NESC 2007 Section 250.C.2)

Shape Factors

NUS Design Criteria Issued April 12, 2007

Shape Factor for Round Members =	$Cd_R := 1.3$	(User Input)
Shape Factor for Flat Members =	$Cd_F := 1.6$	(User Input)
Shape Factor for Coax Cables Attached to Outside of Pole =	$Cd_{coax} := 1.45$	(User Input)

Overload Factors

NU Design Criteria Table

Overload Factors for Wind Loads:

NESC Heavy Loading =	2.5	(User Input)	Apply in Risa-3D Analysis
NESC Extreme Loading =	1.0	(User Input)	Apply in Risa-3D Analysis

Overload Factors for Vertical Loads:

NESC Heavy Loading =	1.5	(User Input)	Apply in Risa-3D Analysis
NESC Extreme Loading =	1.0	(User Input)	Apply in Risa-3D Analysis

Development of Wind & Ice Load on Antennas**Antenna Data:**

Antenna Model =	RFSAPXVSP18-C
Antenna Shape =	Flat (User Input)
Antenna Height =	$L_{ant} := 72$ in (User Input)
Antenna Width =	$W_{ant} := 11.8$ in (User Input)
Antenna Thickness =	$T_{ant} := 7$ in (User Input)
Antenna Weight =	$WT_{ant} := 57$ lbs (User Input)
Number of Antennas =	$N_{ant} := 3$ (User Input)

Wind Load (NESC Extreme)*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna =	$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 5.9$	sf
Antenna Projected Surface Area =	$A_{ant} := SA_{ant} \cdot N_{ant} = 17.7$	sf
Total Antenna Wind Force =	$F_{ant1} := qz \cdot C_d F \cdot A_{ant} \cdot m = 1263$	lbs

Wind Load (NESC Heavy)*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =	$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 6.5$	sf
Antenna Projected Surface Area w/ Ice =	$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 19.5$	sf
Total Antenna Wind Force w/ Ice =	$F_{ant1} := p \cdot C_d F \cdot A_{ICEant} = 125$	lbs

Gravity Load (without ice)

Weight of All Antennas =	$W_{ant1} := (WT_{ant} \cdot N_{ant}) = 171$	lbs
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Gravity Load (ice only)

Volume of Each Antenna =	$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 5947$	cu in
Volume of Ice on Each Antenna =	$V_{ice} := (L_{ant} + 1)(W_{ant} + 1)(T_{ant} + 1) - V_{ant} = 1528$	cu in
Weight of Ice on Each Antenna =	$W_{ICEant} := \frac{V_{ice}}{1728} \cdot \rho_d = 50$	lbs
Weight of Ice on All Antennas =	$W_{ice,ant1} := W_{ICEant} \cdot N_{ant} = 149$	lbs

Development of Wind & Ice Load on Platform**Platform Data:**

(Sprint)

Platform Model =	FWT 14' Low Profile Platform	(User Input)
Platform Shape =	Flat	(User Input)
Platform Area =	$A_{plt} := 14.2$ sq ft	(User Input from FWT design calcs)
Platform Area w/ Ice =	$A_{ICEplt} := 15.8$ sq ft	(User Input from FWT design calcs)
Platform Weight =	$WT_{plt} := 3020$ lbs	(User Input from FWT design calcs)
Platform Weight w/ Ice =	$WT_{ICEplt} := 4300$ lbs	(User Input from FWT design calcs)

Wind Load (NESC Extreme)

Total Platform Wind Force =	$F_{mnt1} := qz \cdot C_d F \cdot A_{plt} \cdot m = 1013$	lbs
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Wind Load (NESC Heavy)

Total Platform Wind Force w/ Ice =	$F_{mnt1} := p \cdot C_d F \cdot A_{ICEplt} = 101$	lbs
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Gravity Load (without ice)

Weight of Platform =	$Wt_{mnt1} := WT_{plt} = 3020$	lbs
----------------------	--------------------------------	-----

Gravity Load (ice only)

Weight of Ice on Platform =	$Wt_{ice.mnt1} := WT_{ICEplt} - WT_{plt} = 1280$	lbs
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Development of Wind & Ice Load on Antennas
Antenna Data:

Antenna Model =	Andrew HBXX-6516DS	(Verizon)
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 50.9$	in (User Input)
Antenna Width =	$W_{ant} := 12$	in (User Input)
Antenna Thickness =	$T_{ant} := 6.5$	in (User Input)
Antenna Weight =	$WT_{ant} := 31$	lbs (User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)

Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna =	$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 4.2$	sf
Antenna Projected Surface Area =	$A_{ant} := SA_{ant} \cdot N_{ant} = 12.7$	sf
Total Antenna Wind Force =	$F_{ant2} := qz \cdot Cd_F \cdot A_{ant} \cdot m = 908$	lbs

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =	$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 4.7$	sf
Antenna Projected Surface Area w/ Ice =	$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 14.1$	sf
Total Antenna Wind Force w/ Ice =	$F_{ant2} := p \cdot Cd_F \cdot A_{ICEant} = 90$	lbs

Gravity Load (without Ice)

Weight of All Antennas =	$Wt_{ant2} := (WT_{ant} \cdot N_{ant}) = 93$	lbs
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Gravity Load (Ice Only)

Volume of Each Antenna =	$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 3970$	cu in
Volume of Ice on Each Antenna =	$V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 1090$	cu in
Weight of Ice on Each Antenna =	$W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 35$	lbs
Weight of Ice on All Antennas =	$Wt_{ice.ant2} := W_{ICEant} \cdot N_{ant} = 106$	lbs

Development of Wind & Ice Load on Antennas**Antenna Data:**

Antenna Model =	Andrew SBNHH-1D65B	(Verizon)
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 72.0$	in (User Input)
Antenna Width =	$W_{ant} := 11.9$	in (User Input)
Antenna Thickness =	$T_{ant} := 7.1$	in (User Input)
Antenna Weight =	$WT_{ant} := 41$	lbs (User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)

Wind Load (NESC Extreme)*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

$$\text{Surface Area for One Antenna} = SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 6 \quad sf$$

$$\text{Antenna Projected Surface Area} = A_{ant} := SA_{ant} \cdot N_{ant} = 17.9 \quad sf$$

$$\text{Total Antenna Wind Force} = F_{ant3} := qz \cdot C_d F \cdot A_{ant} \cdot m = 1274 \quad lbs$$

Wind Load (NESC Heavy)*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

$$\text{Surface Area for One Antenna w/ Ice} = SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 6.5 \quad sf$$

$$\text{Antenna Projected Surface Area w/ Ice} = A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 19.6 \quad sf$$

$$\text{Total Antenna Wind Force w/ Ice} = F_{ant3} := p \cdot C_d F \cdot A_{ICEant} = 126 \quad lbs$$

Gravity Load (without ice)

$$\text{Weight of All Antennas} = W_{ant3} := (WT_{ant} \cdot N_{ant}) = 123 \quad lbs$$

Gravity Load (ice only)

$$\text{Volume of Each Antenna} = V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 6083 \quad cu \text{ in}$$

$$\text{Volume of Ice on Each Antenna} = V_{ice} := (L_{ant} + 1)(W_{ant} + 1)(T_{ant} + 1) - V_{ant} = 1544 \quad cu \text{ in}$$

$$\text{Weight of Ice on Each Antenna} = W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 50 \quad lbs$$

$$\text{Weight of Ice on All Antennas} = W_{ice,ant3} := W_{ICEant} \cdot N_{ant} = 150 \quad lbs$$

Development of Wind & Ice Load on Antennas
Antenna Data:

Antenna Model =	Decibel DB854DG65ESX	(Verizon)
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 48.5$	in (User Input)
Antenna Width =	$W_{ant} := 12.5$	in (User Input)
Antenna Thickness =	$T_{ant} := 6$	in (User Input)
Antenna Weight =	$WT_{ant} := 18.5$	lbs (User Input)
Number of Antennas =	$N_{ant} := 2$	(User Input)

Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

$$\text{Surface Area for One Antenna} = SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 4.2 \quad sf$$

$$\text{Antenna Projected Surface Area} = A_{ant} := SA_{ant} \cdot N_{ant} = 8.4 \quad sf$$

$$\text{Total Antenna Wind Force} = F_{ant4} := qz \cdot Cd_F \cdot A_{ant} \cdot m = 601 \quad lbs$$

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

$$\text{Surface Area for One Antenna w/ Ice} = SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 4.6 \quad sf$$

$$\text{Antenna Projected Surface Area w/ Ice} = A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 9.3 \quad sf$$

$$\text{Total Antenna Wind Force w/ Ice} = F_{ant4} := p \cdot Cd_F \cdot A_{ICEant} = 59 \quad lbs$$

Gravity Load (without ice)

$$\text{Weight of All Antennas} = W_{ant4} := (WT_{ant} \cdot N_{ant}) = 37 \quad lbs$$

Gravity Load (Ice only)

$$\text{Volume of Each Antenna} = V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 3638 \quad cu \, in$$

$$\text{Volume of Ice on Each Antenna} = V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 1040 \quad cu \, in$$

$$\text{Weight of Ice on Each Antenna} = W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 34 \quad lbs$$

$$\text{Weight of Ice on All Antennas} = W_{ice.ant4} := W_{ICEant} \cdot N_{ant} = 67 \quad lbs$$

Development of Wind & Ice Load on Antennas**Antenna Data:**

Antenna Model =	Antel BXA-70063-6CF	(Verizon)
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 71$	in (User Input)
Antenna Width =	$W_{ant} := 11.2$	in (User Input)
Antenna Thickness =	$T_{ant} := 5.2$	in (User Input)
Antenna Weight =	$WT_{ant} := 17$	lbs (User Input)
Number of Antennas =	$N_{ant} := 1$	(User Input)

Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna =

$$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 5.5 \quad sf$$

Antenna Projected Surface Area =

$$A_{ant} := SA_{ant} \cdot N_{ant} = 5.5 \quad sf$$

Total Antenna Wind Force =

$$F_{ant5} := qz \cdot Cd_F \cdot A_{ant} \cdot m = 394 \quad lbs$$

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =

$$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 6.1 \quad sf$$

Antenna Projected Surface Area w/ Ice =

$$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 6.1 \quad sf$$

Total Antenna Wind Force w/ Ice =

$$F_{ant5} := p \cdot Cd_F \cdot A_{ICEant} = 39 \quad lbs$$

Gravity Load (without ice)

Weight of All Antennas =

$$W_{ant5} := (WT_{ant} \cdot N_{ant}) = 17 \quad lbs$$

Gravity Load (ice only)

Volume of Each Antenna =

$$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 4135 \quad cu\ in$$

Volume of Ice on Each Antenna =

$$V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 1311 \quad cu\ in$$

Weight of Ice on Each Antenna =

$$W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 42 \quad lbs$$

Weight of Ice on All Antennas =

$$W_{ice,ant5} := W_{ICEant} \cdot N_{ant} = 42 \quad lbs$$

Development of Wind & Ice Load on Antennas
Antenna Data:

Antenna Model =	RFS FDAP5002/2C-3L Diplexer	(Verizon)
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 6.5$	in (User Input)
Antenna Width =	$W_{ant} := 8.3$	in (User Input)
Antenna Thickness =	$T_{ant} := 3.3$	in (User Input)
Antenna Weight =	$WT_{ant} := 7$	lbs (User Input)
Number of Antennas =	$N_{ant} := 6$	(User Input)

Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna = $SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 0.4$ sf

Antenna Projected Surface Area = $A_{ant} := SA_{ant} \cdot N_{ant} = 2.2$ sf

Total Antenna Wind Force = $F_{ant6} := qz \cdot Cd_F \cdot A_{ant} \cdot m = 160$ lbs

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
 Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice = $SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 0.5$ sf

Antenna Projected Surface Area w/ Ice = $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 2.9$ sf

Total Antenna Wind Force w/ Ice = $F_{ant6} := p \cdot Cd_F \cdot A_{ICEant} = 19$ lbs

Gravity Load (without ice)

Weight of All Antennas = $W_{ant6} := (WT_{ant} \cdot N_{ant}) = 42$ lbs

Gravity Load (ice only)

Volume of Each Antenna = $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 178$ cu in

Volume of Ice on Each Antenna = $V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 122$ cu in

Weight of Ice on Each Antenna = $W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 4$ lbs

Weight of Ice on All Antennas = $W_{ice,ant6} := W_{ICEant} \cdot N_{ant} = 24$ lbs

Development of Wind & Ice Load on Antennas**Antenna Data:**

Antenna Model =	RFSATSBT-TOP-FM-4G Bias Tee	(Verizon)
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 5.63$	in (User Input)
Antenna Width =	$W_{ant} := 3.7$	in (User Input)
Antenna Thickness =	$T_{ant} := 2.0$	in (User Input)
Antenna Weight =	$WT_{ant} := 2$	lbs (User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)

Wind Load (NESC Extreme)*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna =

$$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 0.1 \quad sf$$

Antenna Projected Surface Area =

$$A_{ant} := SA_{ant} \cdot N_{ant} = 0.4 \quad sf$$

Total Antenna Wind Force =

$$F_{ant7} := qz \cdot C_d \cdot F \cdot A_{ant} \cdot m = 31 \quad lbs$$

Wind Load (NESC Heavy)*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =

$$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 0.2 \quad sf$$

Antenna Projected Surface Area w/ Ice =

$$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 0.6 \quad sf$$

Total Antenna Wind Force w/ Ice =

$$F_{ant7} := p \cdot C_d \cdot F \cdot A_{ICEant} = 4 \quad lbs$$

Gravity Load (without ice)

Weight of All Antennas =

$$W_{ant7} := (WT_{ant} \cdot N_{ant}) = 6 \quad lbs$$

Gravity Load (ice only)

Volume of Each Antenna =

$$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 42 \quad cu\,in$$

Volume of Ice on Each Antenna =

$$V_{ice} := (L_{ant} + 1) \cdot (W_{ant} + 1) \cdot (T_{ant} + 1) - V_{ant} = 52 \quad cu\,in$$

Weight of Ice on Each Antenna =

$$W_{ICEant} := \frac{V_{ice}}{1728} \cdot \rho \cdot Id = 2 \quad lbs$$

Weight of Ice on All Antennas =

$$W_{ice.ant7} := W_{ICEant} \cdot N_{ant} = 5 \quad lbs$$

Development of Wind & Ice Load on Platform**Platform Data:**

Platform Model =	T-Arm Colocation Mount	(User Input)
Platform Shape =	Flat	(User Input)
Platform Area =	$A_{plt} := 10.65$	sq ft (User Input)
Platform Area w/ Ice =	$A_{ICEplt} := 13.7$	sq ft (User Input)
Platform Weight =	$WT_{plt} := 750$	lbs (User Input)
Platform Weight w/ Ice =	$WT_{ICEplt} := 950$	lbs (User Input)

Wind Load (NESC Extreme)

Total Platform Wind Force =	$F_{mnt2} := qz \cdot Cd_F \cdot A_{plt} \cdot m = 760$	lbs
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Wind Load (NESC Heavy)

Total Platform Wind Force w/ Ice =	$F_{mnt2} := p \cdot Cd_F \cdot A_{ICEplt} = 88$	lbs
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Gravity Load (without ice)

Weight of Platform =	$W_{mnt2} := WT_{plt} = 750$	lbs
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Gravity Load (ice only)

Weight of Ice on Platform =	$W_{ice.mnt2} := WT_{ICEplt} - WT_{plt} = 200$	lbs
-----------------------------	--	-----

Development of Wind & Ice Load on Antennas
Antenna Data:

Antenna Model =	Kathrein 840-10520
Antenna Shape =	Flat (User Input)
Antenna Height =	$L_{ant} := 23.5$ in (User Input)
Antenna Width =	$W_{ant} := 10.3$ in (User Input)
Antenna Thickness =	$T_{ant} := 5.9$ in (User Input)
Antenna Weight =	$WT_{ant} := 19$ lbs (User Input)
Number of Antennas =	$N_{ant} := 1$ (User Input)

Gravity Load (without ice)

Weight of All Antennas =	$W_{ant8} := WT_{ant} \cdot N_{ant} = 19$	lbs
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Gravity Load (ice only)

Volume of Each Antenna =	$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 1428$	cu in
Volume of Ice on Each Antenna =	$V_{ice} := (L_{ant} + 1)(W_{ant} + 1)(T_{ant} + 1) - V_{ant} = 482$	cu in
Weight of Ice on Each Antenna =	$W_{ICEant} := \frac{V_{ice}}{1728} \cdot \rho_{ice} = 16$	lbs
Weight of Ice on All Antennas =	$W_{ice,ant8} := W_{ICEant} \cdot N_{ant} = 16$	lbs

Wind Load (NESC Extreme)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna =	$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 1.7$	sf
Antenna Projected Surface Area =	$A_{ant} := SA_{ant} \cdot N_{ant} = 1.7$	sf
Total Antenna Wind Force =	$F_{ant8} := qz \cdot C_d \cdot F \cdot A_{ant} = 96$	lbs

Wind Load (NESC Heavy)

*Assumes Maximum Possible Wind Pressure
Applied to all Antennas Simultaneously*

Surface Area for One Antenna w/ Ice =	$SA_{ICEant} := \frac{(L_{ant} + 1) \cdot (W_{ant} + 1)}{144} = 1.9$	sf
Antenna Projected Surface Area w/ Ice =	$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 1.9$	sf
Total Antenna Wind Force w/ Ice =	$F_{ant8} := p \cdot C_d \cdot F \cdot A_{ICEant} = 12$	lbs

Development of Wind & Ice Load on Antenna Mounts

Mount Data:

Mount Type:	SitePro Tapered Pipe Mount
Mount Shape =	Round (User Input)
Pipe Mount Length =	$L_{mnt} := 36$ in (User Input)
2 inch Pipe Mount Linear Weight =	$W_{mnt} := 3.66$ plf (User Input)
Pipe Mount Outside Diameter =	$D_{mnt} := 2.375$ in (User Input)
Number of Mounting Pipes =	$N_{mnt} := 1$ (User Input)
Mount Weight =	$W_{tp,mnt} := 77$ lbs (User Input)

Gravity Loads (without ice)

Weight Each Pipe Mount = $WT_{mnt} := W_{mnt} \cdot \frac{L_{mnt}}{12} = 11$ lbs

Weight of All Mounts = $W_{l,mnt3} := WT_{mnt} \cdot N_{mnt} + W_{tp,mnt} = 88$ lbs

Gravity Load (ice)

Volume of Each Pipe = $V_{mnt} := \frac{\pi}{4} \cdot D_{mnt}^2 \cdot L_{mnt} = 159$ cu in

Volume of Ice on Each Pipe = $V_{ice} := \left[\frac{\pi}{4} \cdot \left[(D_{mnt} + 2 \cdot lr)^2 \right] \cdot (L_{mnt} + 2 \cdot lr) \right] - V_{mnt} = 172$ cu in

Weight of Ice each mount (incl. hardware) = $W_{ICEmnt} := \frac{V_{ice}}{1728} \cdot Id = 6$ lbs

Weight of Ice on All Mounts = $W_{ice,mnt3} := (W_{ICEmnt} \cdot N_{mnt} + 5) = 11$ lbs

Wind Load (NESC Extreme Wind)

Assumes Mount is Shielded by Antenna

Mount Projected Surface Area = $A_{mnt} := 0.0$ sf

Total Mount Wind Force = $F_{mnt3} := qz \cdot Cd_F \cdot A_{mnt} \cdot m = 0$ lbs

Wind Load (NESC Heavy Wind)

Assumes Mount is Shielded by Antenna

Mount Projected Surface Area w/ Ice = $A_{ICEmnt} := 0.0$ sf

Total Mount Wind Force = $F_{mnt3} := p \cdot Cd_F \cdot A_{ICEmnt} = 0$ lbs

Development of Wind & Ice Load on Antenna Mounts

Mount Data:

Mount Type:

SitePro XLD WIMax Tower Mount

Platform Area =

$A_{plt} := 4.5$ sq ft (User Input)

Platform Area w/ Ice =

$A_{ICEplt} := 6$ sq ft (User Input)

Platform Weight =

$WT_{plt} := 160$ lbs (User Input)

Platform Weight w/ Ice =

$WT_{ICEplt} := 185$ lbs (User Input)

Wind Load (NESC Extreme)

Total Platform Wind Force =

$F_{mnt4} := qz \cdot C_d \cdot A_{plt} \cdot m = 321$ lbs

Wind Load (NESC Heavy)

Total Platform Wind Force w/ Ice =

$F_{mnt4} := p \cdot C_d \cdot A_{ICEplt} = 38$ lbs

Gravity Load (without ice)

Weight of Platform =

$W_{mnt4} := WT_{plt} = 160$ lbs

Gravity Load (ice only)

Weight of Ice on Platform =

$W_{ice,mnt4} := WT_{ICEplt} - WT_{plt} = 25$ lbs

Total Equipment Loads:

Sprint @ 148-ft AGL

$$\text{NESC Heavy Wind Vertical} = (W_{\text{ant1}} + W_{\text{ice.ant1}} + W_{\text{mnt1}} + W_{\text{ice.mnt1}}) \cdot 1.5 = 6929$$

$$\text{NESC Heavy Wind Transverse} = (F_{\text{ant1}} + F_{\text{mnt1}}) \cdot 2.5 = 564$$

$$\text{NESC Extreme Wind Vertical} = (W_{\text{ant1}} + W_{\text{mnt1}}) = 3191$$

$$\text{NESC Extreme Wind Transverse} = (F_{\text{ant1}} + F_{\text{mnt1}}) = 2276$$

Verizon @ 139-ft AGL

$$\text{NESC Heavy Wind Vertical} =$$

$$\text{NESC_Heavy_Vert} := (W_{\text{ant2}} + W_{\text{ice.ant2}} + W_{\text{ant3}} + W_{\text{ice.ant3}} + W_{\text{ant4}} + W_{\text{ice.ant4}} + W_{\text{ant5}} + W_{\text{ice.ant5}} + W_{\text{ant6}} + W_{\text{ice.ant6}} + W_{\text{ant7}} + W_{\text{ice.ant7}})$$

$$\text{NESC_Heavy_Vert} = 2494$$

$$\text{NESC Heavy Wind Transverse} = (F_{\text{ant2}} + F_{\text{ant3}} + F_{\text{ant4}} + F_{\text{ant5}} + F_{\text{ant6}} + F_{\text{ant7}} + F_{\text{mnt2}}) \cdot 2.5 = 1081$$

$$\text{NESC Extreme Wind Vertical} = (W_{\text{ant2}} + W_{\text{ant3}} + W_{\text{ant4}} + W_{\text{ant5}} + W_{\text{ant6}} + W_{\text{ant7}} + W_{\text{mnt2}}) = 1068$$

$$\text{NESC Extreme Wind Transverse} = (F_{\text{ant2}} + F_{\text{ant3}} + F_{\text{ant4}} + F_{\text{ant5}} + F_{\text{ant6}} + F_{\text{ant7}} + F_{\text{mnt2}}) = 4127$$

AT&T @ 70-ft AGL

$$\text{NESC Heavy Wind Vertical} = (W_{\text{ant8}} + W_{\text{ice.ant8}} + W_{\text{mnt3}} + W_{\text{ice.mnt3}}) \cdot 1.5 = 200$$

North Tower Leg - Alpha Sector

$$\text{NESC Heavy Wind Transverse} = (F_{\text{ant8}} + F_{\text{mnt3}}) \cdot 2.5 = 31$$

$$\text{NESC Extreme Wind Vertical} = (W_{\text{ant8}} + W_{\text{mnt3}}) = 107$$

$$\text{NESC Extreme Wind Transverse} = (F_{\text{ant8}} + F_{\text{mnt3}}) = 96$$

$$\text{NESC Heavy Wind Vertical} = (W_{\text{ant8} \cdot 2} + W_{\text{ice.ant8} \cdot 2} + W_{\text{mnt4}} + W_{\text{ice.mnt4}}) \cdot 1.5 = 381$$

East Tower Leg - Beta/Gamma
Sectors

$$\text{NESC Heavy Wind Transverse} = (F_{\text{ant8} \cdot 2} + F_{\text{mnt4}}) \cdot 2.5 = 158$$

$$\text{NESC Extreme Wind Vertical} = (W_{\text{ant8} \cdot 2} + W_{\text{mnt4}}) = 198$$

$$\text{NESC Extreme Wind Transverse} = (F_{\text{ant8} \cdot 2} + F_{\text{mnt4}}) = 513$$

Coax Cable within Powermount

(Below Top of Tower)

Distance Between Coax Cable Attach Points =

$$\text{CoaxSpan} := \begin{pmatrix} 8 \\ 15 \\ 14 \\ 29.375 \\ 29.875 \\ 32.5 \end{pmatrix} \cdot \text{ft} \quad (\text{User Input})$$

Diameter of Coax Cable =

$$D_{\text{coax}} := 1.98 \cdot \text{in} \quad (\text{User Input})$$

Weight of Coax Cable =

$$W_{\text{coax}} := 1.04 \cdot \text{plf} \quad (\text{User Input})$$

Number of Coax Cables =

$$N_{\text{coax}} := 24 \quad (\text{User Input}) \quad (6 \text{ Cables inside Powermount 18 on Exterior})$$

Number of Projected Coax Cables Transverse =

$$NP_{\text{Tcoax}} := 4 \quad (\text{User Input})$$

Extreme Wind Pressure =

$$q_z := 35.7 \cdot \text{psf} \quad (\text{User Input})$$

Heavy Wind Pressure =

$$p := 4 \cdot \text{psf} \quad (\text{User Input})$$

Radial Ice Thickness =

$$I_r := 0.5 \cdot \text{in} \quad (\text{User Input})$$

Radial Ice Density =

$$I_d := 56 \cdot \text{pcf} \quad (\text{User Input})$$

Shape Factor =

$$C_d_{\text{coax}} := 1.6 \quad (\text{User Input})$$

Overload Factor for NESC Heavy Wind Load =

$$OF_{\text{HW}} := 2.5 \quad (\text{User Input})$$

Overload Factor for NESC Extreme Wind Load =

$$OF_{\text{EW}} := 1.0 \quad (\text{User Input})$$

Overload Factor for NESC Heavy Vertical Load =

$$OF_{\text{HV}} := 1.5 \quad (\text{User Input})$$

Overload Factor for NESC Extreme Vertical Load =

$$OF_{\text{EV}} := 1.0 \quad (\text{User Input})$$

Wind Area with Ice Transverse =

$$A_{\text{Tice}} := (NP_{\text{Tcoax}} \cdot D_{\text{coax}} + 2 \cdot I_r) = 8.92 \cdot \text{in}$$

Wind Area without Ice Transverse =

$$A_T := (NP_{\text{Tcoax}} \cdot D_{\text{coax}}) = 7.92 \cdot \text{in}$$

Ice Area per Liner Ft =

$$A_{i_{\text{coax}}} := \frac{\pi}{4} \left[(D_{\text{coax}} + 2 \cdot I_r)^2 - D_{\text{coax}}^2 \right] = 0.027 \text{ ft}^2$$

Weight of Ice on All Coax Cables =

$$W_{\text{ice}} := A_{i_{\text{coax}}} \cdot I_d \cdot N_{\text{coax}} = 36.359 \cdot \text{plf}$$

Heavy Vertical Load =

$$\text{HeavyVert} := \overline{\left[(N_{\text{coax}} \cdot W_{\text{coax}} + W_{\text{ice}}) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{HV}} \right]}$$

Heavy Transverse Load =

$$\text{HeavyTrans} := \overline{\left(p \cdot A_{\text{Tice}} \cdot C_{d_{\text{coax}}} \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{HW}} \right)}$$

$$\text{HeavyVert} = \begin{pmatrix} 736 \\ 1380 \\ 1288 \\ 2702 \\ 2748 \\ 2989 \end{pmatrix} \text{ lb}$$

$$\text{HeavyTrans} = \begin{pmatrix} 95 \\ 178 \\ 167 \\ 349 \\ 355 \\ 387 \end{pmatrix} \text{ lb}$$

Extreme Vertical Load =

$$\text{ExtremeVert} := \overline{\left(N_{\text{coax}} \cdot W_{\text{coax}} \right) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{EV}}}$$

Extreme Transverse Load =

$$\text{ExtremeTrans} := \overline{\left[(q_z \cdot A_{\text{T}} \cdot C_{d_{\text{coax}}}) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{EW}} \right]}$$

$$\text{ExtremeVert} = \begin{pmatrix} 200 \\ 374 \\ 349 \\ 733 \\ 746 \\ 811 \end{pmatrix} \text{ lb}$$

$$\text{ExtremeTrans} = \begin{pmatrix} 302 \\ 565 \\ 528 \\ 1107 \\ 1126 \\ 1225 \end{pmatrix} \text{ lb}$$

Coax Cable on Powermount

(Above Top of Tower)

Coax Cable Span =	$Coax_{Span} := 15\text{-ft}$	(User Input)	
Diameter of Coax Cable =	$D_{coax} := 1.98\text{-in}$	(User Input)	
Weight of Coax Cable =	$W_{coax} := 1.04\text{-plf}$	(User Input)	
Number of Coax Cables =	$N_{coax} := 18$	(User Input)	(6 Cables Inside Powermount 12 on Exterior)
Number of Projected Coax Cables Transverse =	$NP_{Tcoax} := 2$	(User Input)	
Extreme Wind Pressure =	$qz := 35.7\text{-psf}$	(User Input)	
Heavy Wind Pressure =	$p := 4\text{-psf}$	(User Input)	
Radial Ice Thickness =	$Ir := 0.5\text{-in}$	(User Input)	
Radial Ice Density =	$Id := 56\text{-pcf}$	(User Input)	
Shape Factor =	$Cd_{coax} := 1.6$	(User Input)	
Overload Factor for NESC Heavy Wind Load =	$OF_{HW} := 2.5$	(User Input)	
Overload Factor for NESC Extreme Wind Load =	$OF_{EW} := 1.0$	(User Input)	
Overload Factor for NESC Heavy Vertical Load =	$OF_{HV} := 1.5$	(User Input)	
Overload Factor for NESC Extreme Vertical Load =	$OF_{EV} := 1.0$	(User Input)	
Wind Area with Ice Transverse =	$A_{Tice} := (NP_{Tcoax} \cdot D_{coax} + 2 \cdot Ir) = 4.96\text{-in}$		
Wind Area without Ice Transverse =	$A_T := (NP_{Tcoax} \cdot D_{coax}) = 3.96\text{-in}$		
Ice Area per Liner Ft =	$A_{ice} := \frac{\pi}{4} [(D_{coax} + 2 \cdot Ir)^2 - D_{coax}^2] = 0.027\text{ft}^2$		
Weight of Ice on All Coax Cables =	$W_{ice} := A_{ice} \cdot Id \cdot N_{coax} = 27.269\text{-plf}$		
Heavy Vertical Load =			
$Heavy_{Vert} := \overline{[(N_{coax} \cdot W_{coax} + W_{ice}) \cdot Coax_{Span} \cdot OF_{HV}]}$	$Heavy_{Vert} = 1035\text{lb}$		
Heavy Transverse Load =			
$Heavy_{Trans} := \overline{[p \cdot A_{Tice} \cdot Cd_{coax} \cdot Coax_{Span} \cdot OF_{HW}]}$	$Heavy_{Trans} = 99\text{lb}$		
Extreme Vertical Load =			
$Extreme_{Vert} := \overline{[(N_{coax} \cdot W_{coax}) \cdot Coax_{Span} \cdot OF_{EV}]}$	$Extreme_{Vert} = 281\text{lb}$		
Extreme Transverse Load =			
$Extreme_{Trans} := \overline{[(qz \cdot A_T \cdot Cd_{coax}) \cdot Coax_{Span} \cdot OF_{EW}]}$	$Extreme_{Trans} = 283\text{lb}$		

Coax Cable on CL&P Tower

Sprint Cables on East Leg

Distance Between Coax Cable Attach Points =

Coax Cable Span =

$$\text{CoaxSpan} := \begin{pmatrix} 10.25 \\ 12.775 \\ 14.025 \\ 14.75 \\ 16.585 \\ 14.625 \\ 13.29 \\ 32.5 \end{pmatrix} \cdot \text{ft} \quad (\text{User Input})$$

Diameter of Coax Cable =

$$D_{\text{coax}} := 1.98 \cdot \text{in} \quad (\text{User Input})$$

Weight of Coax Cable =

$$W_{\text{coax}} := 1.04 \cdot \text{plf} \quad (\text{User Input})$$

Number of Coax Cables =

$$N_{\text{coax}} := 12 \quad (\text{User Input})$$

Number of Projected Coax Cables Transverse =

$$NP_{\text{Tcoax}} := 6 \quad (\text{User Input})$$

Extreme Wind Pressure =

$$q_z := 35.7 \cdot \text{psf} \quad (\text{User Input})$$

Heavy Wind Pressure =

$$p := 4 \cdot \text{psf} \quad (\text{User Input})$$

Radial Ice Thickness =

$$I_r := 0.5 \cdot \text{in} \quad (\text{User Input})$$

Radial Ice Density =

$$I_d := 56 \cdot \text{pcf} \quad (\text{User Input})$$

Shape Factor =

$$C_{d_{\text{coax}}} := 1.6 \quad (\text{User Input})$$

Overload Factor for NESC Heavy Wind Load =

$$OF_{\text{HW}} := 2.5 \quad (\text{User Input})$$

Overload Factor for NESC Extreme Wind Load =

$$OF_{\text{EW}} := 1.0 \quad (\text{User Input})$$

Overload Factor for NESC Heavy Vertical Load =

$$OF_{\text{HV}} := 1.5 \quad (\text{User Input})$$

Overload Factor for NESC Extreme Vertical Load =

$$OF_{\text{EV}} := 1.0 \quad (\text{User Input})$$

Wind Area with Ice Transverse =

$$A_{\text{Tice}} := (NP_{\text{Tcoax}} \cdot D_{\text{coax}} + 2 \cdot I_r) = 12.88 \cdot \text{in}$$

Wind Area without Ice Transverse =

$$A_T := (NP_{\text{Tcoax}} \cdot D_{\text{coax}}) = 11.88 \cdot \text{in}$$

Ice Area per Liner Ft =

$$A_{\text{ice}} := \frac{\pi}{4} \cdot \left[(D_{\text{coax}} + 2 \cdot I_r)^2 - D_{\text{coax}}^2 \right] = 0.027 \text{ft}^2$$

Weight of Ice on All Coax Cables =

$$W_{\text{ice}} := A_{\text{ice}} \cdot I_d \cdot N_{\text{coax}} = 18.179 \cdot \text{plf}$$

Heavy Vertical Load =

$$\text{HeavyVert} := \left[\left(N_{\text{coax}} \cdot W_{\text{coax}} + W_{\text{ice}} \right) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{HV}} \right]$$

Heavy Transverse Load =

$$\text{HeavyTrans} := \left(p \cdot A_{\text{Tice}} \cdot C_{d_{\text{coax}}} \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{HW}} \right)$$

$$\text{HeavyVert} = \begin{pmatrix} 471 \\ 588 \\ 645 \\ 678 \\ 763 \\ 673 \\ 611 \\ 1495 \end{pmatrix} \text{ lb}$$

$$\text{HeavyTrans} = \begin{pmatrix} 176 \\ 219 \\ 241 \\ 253 \\ 285 \\ 251 \\ 228 \\ 558 \end{pmatrix} \text{ lb}$$

Extreme Vertical Load =

$$\text{ExtremeVert} := \left[\left(N_{\text{coax}} \cdot W_{\text{coax}} \right) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{EV}} \right]$$

Extreme Transverse Load =

$$\text{ExtremeTrans} := \left[\left(q_z \cdot A_T \cdot C_{d_{\text{coax}}} \right) \cdot \text{CoaxSpan} \cdot \text{OF}_{\text{EW}} \right]$$

$$\text{ExtremeVert} = \begin{pmatrix} 128 \\ 159 \\ 175 \\ 184 \\ 207 \\ 183 \\ 166 \\ 406 \end{pmatrix} \text{ lb}$$

$$\text{ExtremeTrans} = \begin{pmatrix} 580 \\ 722 \\ 793 \\ 834 \\ 938 \\ 827 \\ 752 \\ 1838 \end{pmatrix} \text{ lb}$$

Coax Cable on CL&P Tower

AT&T Cables on North Leg

Distance Between Coax Cable Attach Points =

Coax Cable Span =

$$\text{CoaxSpan} := \begin{pmatrix} 10 \\ 15 \\ 13 \\ 32 \end{pmatrix} \cdot \text{ft} \quad (\text{User Input})$$

Diameter of Coax Cable =

$$D_{\text{coax}} := 1.98 \cdot \text{in} \quad (\text{User Input})$$

Weight of Coax Cable =

$$W_{\text{coax}} := 1.04 \cdot \text{plf} \quad (\text{User Input})$$

Number of Coax Cables =

$$N_{\text{coax}} := 4 \quad (\text{User Input})$$

Number of Projected Coax Cables Transverse =

$$NP_{\text{Tcoax}} := 2 \quad (\text{User Input})$$

Extreme Wind Pressure =

$$qz := 35.7 \cdot \text{psf} \quad (\text{User Input})$$

Heavy Wind Pressure =

$$p := 4 \cdot \text{psf} \quad (\text{User Input})$$

Radial Ice Thickness =

$$Ir := 0.5 \cdot \text{in} \quad (\text{User Input})$$

Radial Ice Density =

$$Id := 56 \cdot \text{pcf} \quad (\text{User Input})$$

Shape Factor =

$$Cd_{\text{coax}} := 1.6 \quad (\text{User Input})$$

Overload Factor for NESC Heavy Wind Load =

$$OF_{\text{HW}} := 2.5 \quad (\text{User Input})$$

Overload Factor for NESC Extreme Wind Load =

$$OF_{\text{EW}} := 1.0 \quad (\text{User Input})$$

Overload Factor for NESC Heavy Vertical Load =

$$OF_{\text{HV}} := 1.5 \quad (\text{User Input})$$

Overload Factor for NESC Extreme Vertical Load =

$$OF_{\text{EV}} := 1.0 \quad (\text{User Input})$$

Wind Area with Ice Transverse =

$$A_{\text{Tice}} := (NP_{\text{Tcoax}} \cdot D_{\text{coax}} + 2 \cdot Ir) = 4.96 \cdot \text{in}$$

Wind Area without Ice Transverse =

$$A_{\text{T}} := (NP_{\text{Tcoax}} \cdot D_{\text{coax}}) = 3.96 \cdot \text{in}$$

Ice Area per Linear Ft =

$$Ai_{\text{coax}} := \frac{\pi}{4} \left[(D_{\text{coax}} + 2 \cdot Ir)^2 - D_{\text{coax}}^2 \right] = 0.027 \text{ ft}^2$$

Weight of Ice on All Coax Cables =

$$W_{\text{ice}} := Ai_{\text{coax}} \cdot Id \cdot N_{\text{coax}} = 6.06 \cdot \text{plf}$$

Heavy Vertical Load =

$$\text{Heavy}_{\text{Vert}} := \left[(N_{\text{coax}} \cdot W_{\text{coax}} + W_{\text{ice}}) \cdot \text{Coax}_{\text{Span}} \cdot \text{OF}_{\text{HV}} \right]$$

Heavy Transverse Load =

$$\text{Heavy}_{\text{Trans}} := \left(p \cdot A_{\text{Tice}} \cdot C_{d_{\text{coax}}} \cdot \text{Coax}_{\text{Span}} \cdot \text{OF}_{\text{HW}} \right)$$

$$\text{Heavy}_{\text{Vert}} = \begin{pmatrix} 153 \\ 230 \\ 199 \\ 491 \end{pmatrix} \text{ lb}$$

$$\text{Heavy}_{\text{Trans}} = \begin{pmatrix} 66 \\ 99 \\ 86 \\ 212 \end{pmatrix} \text{ lb}$$

Extreme Vertical Load =

$$\text{Extreme}_{\text{Vert}} := \left[(N_{\text{coax}} \cdot W_{\text{coax}}) \cdot \text{Coax}_{\text{Span}} \cdot \text{OF}_{\text{EV}} \right]$$

Extreme Transverse Load =

$$\text{Extreme}_{\text{Trans}} := \left[(qz \cdot A_T \cdot C_{d_{\text{coax}}}) \cdot \text{Coax}_{\text{Span}} \cdot \text{OF}_{\text{EW}} \right]$$

$$\text{Extreme}_{\text{Vert}} = \begin{pmatrix} 42 \\ 62 \\ 54 \\ 133 \end{pmatrix} \text{ lb}$$

$$\text{Extreme}_{\text{Trans}} = \begin{pmatrix} 188 \\ 283 \\ 245 \\ 603 \end{pmatrix} \text{ lb}$$

Project Name : 16009.03 - Greenwich, CT
Project Notes: CL&P Structure #1281 / AT&T CTS103
Project File : J:\Jobs\1600900.WI\003.Cos Cob (TEMP) Struct No 1281\04_Structural\Backup Documentation\PLS Tower\CL&P # 1281.tow
Date run : 4:34:33 PM Tuesday, December 19, 2017
by : Tower Version 12.50
Licensed to : Centek Engineering Inc

Successfully performed nonlinear analysis

Angle member "36P" has 1.00 bolt holes, but double angles should have at least 2 holes. ??
Angle member "36X" has 1.00 bolt holes, but double angles should have at least 2 holes. ??
Member "5P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "5X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "5XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "5Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "7P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "7X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "7XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "7Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "10P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "10X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "10XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "10Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "12P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "12X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "12XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "12Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "13P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "13X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "13XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "13Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "14P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "14X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "14XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??

[illegible]

and spacing distances will be checked. ??
 Member "53AY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??
 Member "53BP" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??
 Member "53BY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??
 Member "53CP" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??
 Member "53CY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??
 Unable to calculate rupture capacity for member "g106p" because it has a long and short edge distance of 0. ??
 KL/R value of 200.30 exceeds maximum of 200.00 for member "g118p" ??
 Problem calculating gross area of longitudinal face for section "1": width is zero at elevation 128.80 (ft) which is not the top of the section. ??
 Problem calculating gross area of longitudinal face for section "2": width is zero at elevation -4.25 (ft) which is not the top of the section. ??
 Unusual number of fixed joints found: 5. Towers normally have from between 1 and 4 fixed joints. ??
 The model has 121 warnings. ??

Member check option: ASCE 10
 Connection rupture check: ASCE 10
 Crossing diagonal check: ASCE 10 [Alternate Unsupported RJOUT = 1]
 Included angle check: None
 Climbing load check: None
 Redundant members checked with: Actual Force

Loads from file: j:\jobs\1600900.wi\003_cob cob (temp) struct no 1281\04_structural\backup documentation\pls tower\cl&p # 1281.lca

*** Analysis Results:

Maximum element usage is 90.98% for Angle "3P" in load case "NESC Extreme"
 Maximum insulator usage is 29.57% for Clamp "10" in load case "NESC Heavy"

Summary of Joint Support Reactions For All Load Cases:

Load Case	Joint Label	Long. Force (kips)	Tran. Force (kips)	Vert. Force (kips)	Shear Force (kips)	Tran. Moment (ft-k)	Long. Moment (ft-k)	Bending Moment (ft-k)	Vert. Moment (ft-k)	Vert. Usage %
NESC Heavy	15P	-32.08	-32.98	-220.23	46.01	-2.78	3.51	4.48	0.12	0.00
NESC Heavy	29P	-0.19	-1.19	-34.21	1.20	11.40	-3.54	11.94	0.84	0.00
NESC Heavy	15X	34.21	-29.68	187.35	45.29	-0.36	-0.19	0.41	0.01	0.00
NESC Heavy	15XY	-34.98	-31.90	188.28	47.34	1.41	-0.21	1.42	0.07	0.00
NESC Heavy	15Y	33.04	-33.01	-226.21	46.70	-1.71	-5.98	6.22	-0.37	0.00
NESC Extreme	15P	-29.55	-29.88	-207.29	42.02	-1.27	3.20	3.44	0.38	0.00
NESC Extreme	29P	-0.07	-2.53	-12.94	2.53	30.58	-2.59	30.69	1.16	0.00
NESC Extreme	15X	30.62	-33.84	168.73	45.64	4.46	0.17	4.46	0.74	0.00
NESC Extreme	15XY	-37.01	-42.78	200.63	56.57	6.85	-0.89	6.91	-0.61	0.00
NESC Extreme	15Y	30.53	-30.59	-204.80	43.22	4.44	-6.74	8.07	-1.35	0.00

Summary of Joint Support Reactions For All Load Cases in Direction of Leg:

Load Case	Support Origin	Joint	Member	Leg Dir.	Leg Force (kips)	Perpendicular To Leg (kips)	Residual Shear (kips)	Horizontal To Leg (kips)	Residual Shear (kips)	Horizontal To Leg (kips)	Long. Force (kips)	Total Force (kips)	Total Tran. Force (kips)	Total Vert. Force (kips)
NESC Heavy	15P	14S	14P	224.984	0.695	0.698	-0.247	0.652	-32.08	-32.98	-220.23	-220.23		
NESC Heavy	15X	14X	14X	-192.625	6.941	7.058	-6.713	2.181	34.21	-29.68	187.35			

167.46 9.685 4
 PNBRS L4x4x1/4
 186.27 12.340 4
 moments): g118P ??
 20a H1
 112.48 6.000 3
 Angler L2x2x1/4
 130.21 4.243 4
 BraceR L2.5x2.5x1/4
 100.99 3.354 3
 moments): g110P ??
 Plate 6"x3/4" PL
 101.59 1.501 3

1
 SAE 4X4X0.25 36.0 8.23 Comp 8.23 g120P -1.119NESC Ext 16.004 16.800 13.594 1.000 1.000 1.000 186.27
 1 A potentially damaging moment exists in the following members (make sure your system is well triangulated to minimize
 SAE 1.75X1.75X0.1875 36.0 19.51 Comp 19.51 30AP -1.776NESC Hea 13.441 9.100 10.195 0.500 0.500 0.500 104.96
 1
 SAE 2X2X0.25 36.0 37.80 Tens 34.46 42XY -3.136NESC Ext 15.869 9.100 13.594 1.000 1.000 1.000 130.21
 1
 SAE 2.5X2.5X0.25 36.0 58.05 Comp 58.05 g110X -7.891NESC Ext 29.101 16.800 13.594 1.000 1.000 1.000 81.98
 1 A potentially damaging moment exists in the following members (make sure your system is well triangulated to minimize
 Bar 6x3/4 36.0 4.84 Comp 4.84 g106P -0.813NESC Ext 109.423 16.800 0.000 1.000 1.000 1.000 83.19
 1

Group Summary (Tension Portion):

Group No.	Hole Desc.	Angle Size	Steel Strength	Max Usage	Max Use	Tension Control	Force Control	Section Capacity	Net Tension	Tension Connect.	Tension Connect.	Tension Length	No.
Label	Type												
Of Diameter													
Holes													
(in)			(ksi)	%	Tens.				Case	Capacity (kips)	Capacity (kips)	Capacity (kips)	Tens. (ft)

1	LEG1	SAE	3.5X3.5X0.25	36.0 90.98	Comp 73.56	3X	26.77NESC Ext	48.465	36.400	54.375	60.417	7.000	4
2	LEG2	SAE	6X6X0.3125	36.0 87.73	Comp 78.83	5XY	79.193NESC Ext	100.462	109.200	203.906	226.562	6.050	12
3	LEG3	SAE	8X8X0.5	36.0 68.57	Comp 66.97	7X	157.846NESC Hea	235.687	254.800	380.624	422.916	7.000	14
4	LEG4	SAE	8X8X0.625	36.0 72.74	Comp 67.09	9X	195.772NESC Hea	291.819	0.000	0.000	0.000	7.915	0
5	LEG5	SAE	8X8X0.75	36.0 63.48	Comp 57.26	11X	193.318NESC Hea	337.589	0.000	0.000	0.000	10.131	0
6	X1	SAE	2.5X2.5X0.1875	36.0 86.45	Comp 75.20	15AP	13.584NESC Ext	25.048	18.200	20.391	18.063	10.817	2
7	X2	SAU	2.5X2X0.1875	36.0 85.43	Comp 63.92	16AP	11.586NESC Ext	19.030	18.200	20.391	18.125	9.220	2
8	X3	SAU	4X3X0.25	36.0 57.37	Comp 49.96	17AP	22.516NESC Hea	45.066	45.500	67.969	60.417	10.000	5
9	X4	SAU	3.5X2.5X0.25	36.0 60.32	Comp 54.98	18AY	20.012NESC Hea	40.920	36.400	54.375	42.647	8.521	4
10	X5	SAU	4X3.5X0.3125	36.0 54.28	Comp 54.24	19AP	32.782NESC Hea	60.440	63.700	118.945	112.337	9.220	7
11	X6	SAU	5X3.5X0.25	36.0 26.18	Cross 26.16	21BXY	12.023NESC Hea	45.962	54.600	81.562	72.500	10.597	6
12	X7	SAU	4X3X0.25	36.0 16.00	Comp 11.18	22BY	4.070NESC Hea	41.087	36.400	54.375	44.391	12.246	4
13	X8	SAU	3.5X3X0.25	36.0 8.89	Tens 8.89	23AY	2.427NESC Ext	40.925	27.300	40.781	38.516	14.070	3
14	X9	SAU	5X3X0.25	36.0 18.11	Comp 2.95	24AP	0.805NESC Hea	41.087	27.300	40.781	36.250	17.450	3
15	D1	SAU	5X3X0.25	36.0 78.17	Tens 78.17	25AY	29.029NESC Ext	37.133	63.700	95.156	74.632	14.103	7
16	D2	SAE	2X2X0.1875	36.0 41.26	Tens 41.26	26BX	7.477NESC Ext	18.827	18.200	20.391	18.125	16.853	2

1.000 0.6875 A potentially damaging moment exists in the following members (make sure your system is well triangulated to minimize moments): g106P
 ??
 1.000 Plate 6"x3/4" PL Bar 6x3/4 36.0 4.84 Comp 2.51 g106P 0.421NESC Hea 145.800 16.800 0.000 0.000 1.501 1

*** Maximum Stress Summary for Each Load Case

Summary of Maximum Usages by Load Case:

Load Case Maximum Element Usage & Label		Element Type	
NESC Heavy	87.73	5Y	Angle
NESC Extreme	90.98	3P	Angle

Summary of Insulator Usages:

Insulator Label	Insulator Type	Insulator Maximum Usage &	Load Case Maximum Weight (lbs)
1	Clamp	19.65	NESC Heavy 0.0
2	Clamp	19.51	NESC Heavy 0.0
3	Clamp	20.20	NESC Heavy 0.0
4	Clamp	20.61	NESC Heavy 0.0
5	Clamp	20.20	NESC Heavy 0.0
6	Clamp	20.62	NESC Heavy 0.0
7	Clamp	20.20	NESC Heavy 0.0
8	Clamp	20.61	NESC Heavy 0.0
9	Clamp	29.53	NESC Heavy 0.0
10	Clamp	29.57	NESC Heavy 0.0
11	Clamp	29.53	NESC Heavy 0.0
12	Clamp	1.81	NESC Extreme 0.0
13	Clamp	2.09	NESC Extreme 0.0
14	Clamp	2.32	NESC Heavy 0.0
15	Clamp	4.00	NESC Extreme 0.0
16	Clamp	4.67	NESC Extreme 0.0
17	Clamp	3.69	NESC Heavy 0.0
18	Clamp	3.20	NESC Extreme 0.0
19	Clamp	6.04	NESC Heavy 0.0
20	Clamp	10.21	NESC Heavy 0.0
21	Clamp	10.89	NESC Heavy 0.0
22	Clamp	10.43	NESC Heavy 0.0
23	Clamp	4.98	NESC Heavy 0.0
24	Clamp	5.34	NESC Heavy 0.0
25	Clamp	3.42	NESC Heavy 0.0
26	Clamp	9.14	NESC Extreme 0.0
27	Clamp	16.71	NESC Heavy 0.0
28	Clamp	2.37	NESC Extreme 0.0
29	Clamp	3.08	NESC Heavy 0.0
30	Clamp	2.50	NESC Heavy 0.0
31	Clamp	4.27	NESC Heavy 0.0

*** Weight of structure (lbs):
 Weight of Angles*Section DLF: 38857.7

*
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*

Project Name : 16009.03 - Greenwich, CT
Project Notes: CL&P Structure #1281 / A&T CTS103
Project File : J:\Jobs\1600900.WI\003 Cos Cob (TEMP) Struct No 1281\04_Structural\Backup Documentation\PLS Tower\CL&P # 1281.tow
Date run : 4:34:32 PM Tuesday, December 19, 2017
by : Tower Version 12.50
Licensed to : Centek Engineering Inc

Successfully performed nonlinear analysis

Angle member "36P" has 1.00 bolt holes, but double angles should have at least 2 holes. ??
Angle member "36X" has 1.00 bolt holes, but double angles should have at least 2 holes. ??
Member "5P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "5X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "5XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "5Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "7P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "7X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "7XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "7Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "10P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "10X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "10XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "10Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "12P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "12X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "12XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "12Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "13P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "13X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "13XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "13Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge
and spacing distances will be checked. ??
Member "14P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge

[illegible]

[illegible]

Member "53P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??

Member "53Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??

Member "53AP" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??

Member "53AY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??

Member "53BP" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??

Member "53BY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??

Member "53CP" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??

Member "53CY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked. ??

Unable to calculate rupture capacity for member "g106P" because it has a long and short edge distance of 0. ??

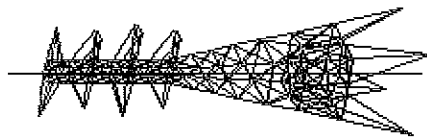
KL/R value of 200.30 exceeds maximum of 200.00 for member "g118P" ??

Problem calculating gross area of longitudinal face for section "1": width is zero at elevation 128.80 (ft) which is not the top of the section. ??

Problem calculating gross area of longitudinal face for section "2": width is zero at elevation -4.25 (ft) which is not the top of the section. ??

Unusual number of fixed joints found: 5. Towers normally have from between 1 and 4 fixed joints. ??

The model has 121 warnings. ??



Nonlinear convergence parameters: Use Standard Parameters

Tension only member maximum compression load as a percent of compression capacity: 100%

Member check option: ASCE 10

Connection rupture check: ASCE 10

Crossing diagonal check: ASCE 10 [Alternate Unsupported R1OUT = 1]

Included angle check: None

Climbing load check: None

Redundant members checked with: Actual Force

Joints Geometry:

Joint Symmetry X Coord. Y Coord. Z Coord. X Disp. Y Disp. Z Disp. X Rot. Y Rot. Z Rot.

Label	Code	(ft)	(ft)	(ft)	Rest.	Rest.	Rest.	Rest.	Rest.
1P	XY-Symmetry	3	3	128.8	Free	Free	Free	Free	Free
2P	XY-Symmetry	3	3	124.3	Free	Free	Free	Free	Free
3P	XY-Symmetry	3	3	119.8	Free	Free	Free	Free	Free
4P	XY-Symmetry	3	3	112.8	Free	Free	Free	Free	Free
5P	XY-Symmetry	3	3	104.8	Free	Free	Free	Free	Free
6P	XY-Symmetry	3	3	98.75	Free	Free	Free	Free	Free
7P	XY-Symmetry	3	3	91.75	Free	Free	Free	Free	Free
8P	XY-Symmetry	3	3	84.75	Free	Free	Free	Free	Free
15P	XY-Symmetry	15.44	15.44	0	Fixed	Fixed	Fixed	Fixed	Fixed
16P	X-Symmetry	0	16.75	128.8	Free	Free	Free	Free	Free
17P	Y-Symmetry	3	14.25	112.8	Free	Free	Free	Free	Free
18P	Y-Symmetry	3	16.75	98.75	Free	Free	Free	Free	Free
19P	Y-Symmetry	3	14.25	84.75	Free	Free	Free	Free	Free
20AP	Y-Symmetry	9.568	0	40	Free	Free	Free	Free	Free
20BP	X-Symmetry	0	9.568	40	Free	Free	Free	Free	Free
21P	XY-Symmetry	11.77	2.208	25	Free	Free	Free	Free	Free
22P	XY-Symmetry	2.208	11.77	25	Free	Free	Free	Free	Free
23P	None	0	-14.25	112.8	Free	Free	Free	Free	Free
24P	None	0	-16.75	98.75	Free	Free	Free	Free	Free
25P	None	0	-14.25	84.75	Free	Free	Free	Free	Free
26P	Y-Symmetry	3	8.625	112.8	Free	Free	Free	Free	Free
27P	Y-Symmetry	3	9.875	98.75	Free	Free	Free	Free	Free
28P	Y-Symmetry	3	8.625	84.75	Free	Free	Free	Free	Free
29P	None	1.5	0	-4.25	Fixed	Fixed	Fixed	Fixed	Fixed
30P	None	1.5	0	25	Free	Free	Free	Free	Free
31P	None	1.5	0	40	Free	Free	Free	Free	Free
32P	None	1.5	0	84.75	Free	Free	Free	Free	Free
33P	None	1.5	0	98.75	Free	Free	Free	Free	Free
34P	None	1.5	0	112.8	Free	Free	Free	Free	Free
35P	None	1.5	0	128.8	Free	Free	Free	Free	Free
36P	None	1.5	0	134.8	Free	Free	Free	Free	Free
37P	None	1.5	0	143.8	Free	Free	Free	Free	Free
1X	X-GenX	3	-3	128.8	Free	Free	Free	Free	Free
1XY	XY-GenX	-3	-3	128.8	Free	Free	Free	Free	Free
1Y	Y-GenX	-3	3	128.8	Free	Free	Free	Free	Free
2X	X-GenX	3	-3	124.3	Free	Free	Free	Free	Free
2XY	XY-GenX	-3	-3	124.3	Free	Free	Free	Free	Free
2Y	Y-GenX	-3	3	124.3	Free	Free	Free	Free	Free
3X	X-GenX	3	-3	119.8	Free	Free	Free	Free	Free
3XY	XY-GenX	-3	-3	119.8	Free	Free	Free	Free	Free
3Y	Y-GenX	-3	3	119.8	Free	Free	Free	Free	Free
4X	X-GenX	3	-3	112.8	Free	Free	Free	Free	Free
4XY	XY-GenX	-3	-3	112.8	Free	Free	Free	Free	Free
4Y	Y-GenX	-3	3	112.8	Free	Free	Free	Free	Free
5X	X-GenX	3	-3	104.8	Free	Free	Free	Free	Free
5XY	XY-GenX	-3	-3	104.8	Free	Free	Free	Free	Free
5Y	Y-GenX	-3	3	104.8	Free	Free	Free	Free	Free
6X	X-GenX	3	-3	98.75	Free	Free	Free	Free	Free
6XY	XY-GenX	-3	-3	98.75	Free	Free	Free	Free	Free
6Y	Y-GenX	-3	3	98.75	Free	Free	Free	Free	Free
7X	X-GenX	3	-3	91.75	Free	Free	Free	Free	Free
7XY	XY-GenX	-3	-3	91.75	Free	Free	Free	Free	Free
7Y	Y-GenX	-3	3	91.75	Free	Free	Free	Free	Free
8X	X-GenX	3	-3	84.75	Free	Free	Free	Free	Free
8XY	XY-GenX	-3	-3	84.75	Free	Free	Free	Free	Free
8Y	Y-GenX	-3	3	84.75	Free	Free	Free	Free	Free
15X	X-GenX	15.44	-15.44	0	Fixed	Fixed	Fixed	Fixed	Fixed
15XY	XY-GenX	-15.44	-15.44	0	Fixed	Fixed	Fixed	Fixed	Fixed

Joint Label	Symmetry Code	Origin Joint	End Joint	Elevation	X Disp.		Y Disp.		Z Disp.		X Rot.		Y Rot.		Z Rot.	
					Rest.	(ft)	Rest.	(ft)	Rest.	(ft)	Rest.	(ft)	Rest.	(ft)	Rest.	(ft)
15Y	Y-GenXY	-15.44	15.44	0	Fixed	0	Fixed	0	Fixed	0	Fixed	0	Fixed	0	Fixed	0
16X	X-Gen	0	-16.75	128.8	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
17Y	Y-Gen	-3	14.25	112.8	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
18Y	Y-Gen	-3	16.75	98.75	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
19Y	Y-Gen	-3	14.25	84.75	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
20AY	Y-Gen	-9.568	0	0	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
20BX	X-Gen	0	-9.568	40	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
21X	X-GenXY	11.77	-2.208	25	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
21XY	XY-GenXY	-11.77	-2.208	25	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
21Y	Y-GenXY	-11.77	2.208	25	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
22X	X-GenXY	2.208	-11.77	25	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
22XY	XY-GenXY	-2.208	-11.77	25	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
22Y	Y-GenXY	-2.208	11.77	25	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
26Y	Y-Gen	-3	8.625	112.8	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
27Y	Y-Gen	-3	9.875	98.75	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
28Y	Y-Gen	-3	8.625	84.75	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0

Secondary Joints:

Joint Label	Symmetry Code	Origin Joint	End Joint	Elevation	X Disp.		Y Disp.		Z Disp.		X Rot.		Y Rot.		Z Rot.	
					Rest.	(ft)	Rest.	(ft)	Rest.	(ft)	Rest.	(ft)	Rest.	(ft)	Rest.	(ft)
9S	XY-Symmetry	8P	15P	0	Free	77	Free	77	Free	77	Free	77	Free	77	Free	77
10S	XY-Symmetry	8P	15P	0	Free	69.25	Free	69.25	Free	69.25	Free	69.25	Free	69.25	Free	69.25
11S	XY-Symmetry	8P	15P	0	Free	61.5	Free	61.5	Free	61.5	Free	61.5	Free	61.5	Free	61.5
12S	XY-Symmetry	8P	15P	0	Free	51.58	Free	51.58	Free	51.58	Free	51.58	Free	51.58	Free	51.58
13S	XY-Symmetry	8P	15P	0	Free	40	Free	40	Free	40	Free	40	Free	40	Free	40
14S	XY-Symmetry	8P	15P	0	Free	25	Free	25	Free	25	Free	25	Free	25	Free	25
10.50E129S	None	1X	1Y	0.5	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
10.50E113S	None	4X	4Y	0.5	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
10.50E99S	None	6X	6Y	0.5	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
10.50E85S	None	8X	8Y	0.5	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
21XF0.50S	Y-Symmetry	21X	21P	0.5	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
22PF0.50S	X-Symmetry	22P	22Y	0.5	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
9X	X-GenXY	8P	15P	0	Free	77	Free	77	Free	77	Free	77	Free	77	Free	77
9XY	XY-GenXY	8P	15P	0	Free	77	Free	77	Free	77	Free	77	Free	77	Free	77
9Y	Y-GenXY	8P	15P	0	Free	77	Free	77	Free	77	Free	77	Free	77	Free	77
10X	X-GenXY	8P	15P	0	Free	69.25	Free	69.25	Free	69.25	Free	69.25	Free	69.25	Free	69.25
10XY	XY-GenXY	8P	15P	0	Free	69.25	Free	69.25	Free	69.25	Free	69.25	Free	69.25	Free	69.25
10Y	Y-GenXY	8P	15P	0	Free	69.25	Free	69.25	Free	69.25	Free	69.25	Free	69.25	Free	69.25
11X	X-GenXY	8P	15P	0	Free	61.5	Free	61.5	Free	61.5	Free	61.5	Free	61.5	Free	61.5
11XY	XY-GenXY	8P	15P	0	Free	61.5	Free	61.5	Free	61.5	Free	61.5	Free	61.5	Free	61.5
11Y	Y-GenXY	8P	15P	0	Free	61.5	Free	61.5	Free	61.5	Free	61.5	Free	61.5	Free	61.5
12X	X-GenXY	8P	15P	0	Free	51.58	Free	51.58	Free	51.58	Free	51.58	Free	51.58	Free	51.58
12XY	XY-GenXY	8P	15P	0	Free	51.58	Free	51.58	Free	51.58	Free	51.58	Free	51.58	Free	51.58
12Y	Y-GenXY	8P	15P	0	Free	40	Free	40	Free	40	Free	40	Free	40	Free	40
13X	X-GenXY	8P	15P	0	Free	40	Free	40	Free	40	Free	40	Free	40	Free	40
13XY	XY-GenXY	8P	15P	0	Free	40	Free	40	Free	40	Free	40	Free	40	Free	40
13Y	Y-GenXY	8P	15P	0	Free	25	Free	25	Free	25	Free	25	Free	25	Free	25
14X	X-GenXY	8P	15P	0	Free	25	Free	25	Free	25	Free	25	Free	25	Free	25
14XY	XY-GenXY	8P	15P	0	Free	25	Free	25	Free	25	Free	25	Free	25	Free	25
14Y	Y-GenXY	8P	15P	0	Free	0.5	Free	0.5	Free	0.5	Free	0.5	Free	0.5	Free	0.5
21XF0.50Y	Y-Gen	21X	21P	0.5	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0
22PF0.50X	X-Gen	22P	22Y	0.5	Free	0	Free	0	Free	0	Free	0	Free	0	Free	0

The model contains 74 primary and 32 secondary joints for a total of 106 joints.

Steel Material Properties:

Centek Engineering Inc - CL&P # 1281

Steel Material Label	Modulus of Elasticity (ksi)	Yield Stress (ksi)	Ultimate Stress (ksi)	Member Stress All. (ksi)	Member Stress Rupture (ksi)	Member Bearing (ksi)	Member Bearing (ksi)
A 36	2.9e+004	36	58	0	0	0	0
A500-42	2.9e+004	42	58	0	0	0	0

Bolt Properties:

Bolt Label	Bolt Diameter (in)	Hole Diameter (in)	Ultimate Shear Capacity (kips)	Default End Distance (in)	Default Spacing (in)	Shear Capacity (kips)	Shear Capacity (kips)
5/8 A394	0.625	0.6875	9.1	1.125	1.5	0	0
5/8 A325	0.625	0.6875	16.8	1.25	1.5	0	0

Number Bolts Used By Type:

Bolt Number Type	Bolts
5/8 A394	1248
5/8 A325	22

Angle Properties:

Angle Type	Angle Size	Long Leg (in)	Short Leg (in)	Thick. (in)	Unit Weight (lbs/ft)	Gross Area (in^2)	w/t Ratio	Radius of Gyration (in)	Radius of Gyration (in)	Rz Angles (in)	Number of Angles	Wind Width (in)	Short Edge Dist. (in)	Long Edge Dist. (in)	Factor	Cost	Optimize Sections
SAE	8X8X0.75	8	8	0.75	38.9	11.44	8.83	2.47	2.47	1.58	1	8	4	0	1.0000	0	0
SAE	8X8X0.625	8	8	0.625	32.7	9.61	10.8	2.49	2.49	1.58	1	8	4	0	1.0000	0	0
SAE	8X8X0.5	8	8	0.5	26.4	7.75	13.75	2.5	2.5	1.59	1	8	4	0	1.0000	0	0
SAE	6X6X0.3125	6	6	0.3125	12.5	3.65	16.6	1.89	1.89	1.2	1	6	3	0	1.0000	0	0
SAE	4X4X0.25	4	4	0.25	6.6	1.94	13.5	1.25	1.25	0.795	1	4	2	0	1.0000	0	0
SAE	3.5X3.5X0.25	3.5	3.5	0.25	5.8	1.69	11.5	1.09	1.09	0.694	1	3.5	1.75	0	1.0000	0	0
SAE	3X3X0.25	3	3	0.25	4.9	1.44	9.75	0.93	0.93	0.592	1	3	1.5	0	1.0000	0	0
SAE	3X3X0.1875	3	3	0.1875	3.71	1.09	13.33	0.939	0.939	0.596	1	3	1.5	0	1.0000	0	0
SAE	2.5X2.5X0.25	2.5	2.5	0.25	4.1	1.19	7.75	0.769	0.769	0.491	1	2.5	1.25	0	1.0000	0	0
SAE	2.5X2.5X0.1875	2.5	2.5	0.1875	3.07	0.902	10.67	0.778	0.778	0.495	1	2.5	1.25	0	1.0000	0	0
SAE	2X2X0.25	2	2	0.25	3.19	0.94	5	0.609	0.609	0.391	1	2	1	0	1.0000	0	0
SAE	2X2X0.1875	2	2	0.1875	2.44	0.71	8	0.617	0.617	0.394	1	2	1	0	1.0000	0	0
SAE	1.75X1.75X0.1875	1.75	1.75	0.1875	2.12	0.62	6	0.537	0.537	0.343	1	1.75	0.875	0	1.0000	0	0
SAU	5X3.5X0.25	5	3.5	0.25	7	2.06	17	1.62	1.04	0.77	1	5	1.75	0	1.0000	0	0
SAU	5X3X0.25	5	3	0.25	6.6	1.94	17	1.62	0.861	0.663	1	5	1.5	0	1.0000	0	0
SAU	4X3.5X0.3125	4	3.5	0.3125	7.7	2.25	10.4	1.26	1.07	0.73	1	4	1.75	0	1.0000	0	0
SAU	4X3.5X0.25	4	3.5	0.25	6.2	1.81	13.25	1.27	1.07	0.734	1	4	1.75	0	1.0000	0	0
SAU	4X3X0.25	4	3	0.25	5.8	1.69	13.25	1.28	0.896	0.651	1	4	1.5	0	1.0000	0	0
SAU	3.5X3X0.25	3.5	3	0.25	5.4	1.56	11.25	1.11	0.914	0.631	1	3.5	1.5	0	1.0000	0	0
SAU	3.5X2.5X0.25	3.5	2.5	0.25	4.9	1.44	11.25	1.12	0.735	0.544	1	3.5	1.25	0	1.0000	0	0
SAU	3X2X0.25	3	2	0.25	4.1	1.19	9.75	0.957	0.574	0.435	1	3	1	0	1.0000	0	0
SAU	3X2X0.1875	3	2	0.1875	3.07	0.9	13.33	0.966	0.583	0.439	1	3	1	0	1.0000	0	0
SAU	2.5X2X0.1875	2.5	2	0.1875	2.75	0.81	10.67	0.793	0.6	0.427	1	2.5	1	0	1.0000	0	0
Pwmt	Pipe 12" Std.	12.75	12	0	49.6	13.6	1	4.39	4.39	4.39	1	12.75	0	0	0.0000	0	0
Bar	6x3/4	6	0.75	0	15.3	4.5	8	0.2165	1.732	1.732	1	6	0	0	0.0000	0	0

DAE 1.75X1.75X0.1875 1.75 1.75 0.1875 4.2 1.24 7 0.537 0.738 0.537 2 3.5 0 0 0.0000 0

Angle Groups:

Group Label	Group Description	Angle Type	Angle Size	Material Type	Element Type	Group Type	Optimize Group	Allow. Add. Angle Width For Optimize (in)
1	LEG1	SAE	3.5X3.5X0.25	A 36	Beam	Leg	None	0.000
2	LEG2	SAE	6X6X0.3125	A 36	Beam	Leg	None	0.000
3	LEG3	SAE	8X8X0.5	A 36	Beam	Leg	None	0.000
4	LEG4	SAE	8X8X0.625	A 36	Beam	Leg	None	0.000
5	LEG5	SAE	8X8X0.75	A 36	Beam	Leg	None	0.000
6	X1	SAE	2.5X2.5X0.1875	A 36	Truss	Crossing Diagonal	None	0.000
7	X2	SAU	2.5X2X0.1875	A 36	Truss	Crossing Diagonal	None	0.000
8	X3	SAU	4X3X0.25	A 36	Truss	Crossing Diagonal	None	0.000
9	X4	SAU	3.5X2.5X0.25	A 36	Truss	Crossing Diagonal	None	0.000
10	X5	SAU	4X3.5X0.3125	A 36	Truss	Crossing Diagonal	None	0.000
11	X6	SAU	5X3.5X0.25	A 36	Truss	Crossing Diagonal	None	0.000
12	X7	SAU	4X3X0.25	A 36	Truss	Crossing Diagonal	None	0.000
13	X8	SAU	3.5X3X0.25	A 36	Truss	Crossing Diagonal	None	0.000
14	X9	SAU	5X3X0.25	A 36	Truss	Crossing Diagonal	None	0.000
15	D1	SAU	5X3X0.25	A 36	Truss	Crossing Diagonal	None	0.000
16	D2	SAE	2X2X0.1875	A 36	T-Only	Other	None	0.000
17	D3	SAU	3X2X0.25	A 36	T-Only	Other	None	0.000
18	D4	SAU	4X3X0.25	A 36	T-Only	Other	None	0.000
19	D5	SAU	3.5X3X0.25	A 36	Truss	Other	None	0.000
20	H1	SAE	1.75X1.75X0.1875	A 36	T-Only	Other	None	0.000
21	R2	SAU	4X3.5X0.25	A 36	Truss	Other	None	0.000
22	H3	SAE	3X3X0.1875	A 36	Truss	Other	None	0.000
23	H4	SAU	5X3X0.25	A 36	Truss	Other	None	0.000
24	H5	SAE	3.5X3.5X0.25	A 36	Truss	Other	None	0.000
25	X10	SAE	1.75X1.75X0.1875	A 36	Beam	Other	None	0.000
26	X11	SAE	2X2X0.1875	A 36	Beam	Other	None	0.000
27	X12	SAU	2.5X2X0.1875	A 36	Truss	Other	None	0.000
28	H6	SAU	4X3X0.25	A 36	Truss	Other	None	0.000
29	D6	SAU	3X2X0.1875	A 36	Truss	Other	None	0.000
30	HGR1	SAU	2.5X2X0.1875	A 36	Truss	Other	None	0.000
31	HGR2	SAE	3X3X0.1875	A 36	T-Only	Other	None	0.000
32	A1	SAE	3X3X0.25	A 36	Beam	Other	None	0.000
33	A2	SAU	3.5X3X0.25	A 36	Beam	Other	None	0.000
34	A#	SAE	4X4X0.25	A 36	Beam	Other	None	0.000
35	H7	SAU	2.5X2X0.1875	A 36	Truss	Other	None	0.000
36	H8	DAE	1.75X1.75X0.1875	A 36	T-Only Beam	Other	None	0.000
Pwmt 12" Std. Pipe Pwmt			Pipe 12" Std.	A500-42	Beam	Other	None	0.000
PMBR1	L2x2x3/16	SAE	2X2X0.1875	A 36	Beam	Other	None	12.000
PMBR2	L2.5x2.5x3/16	SAE	2.5X2.5X0.1875	A 36	Beam	Other	None	12.000
PMBR3	L3x3x3/16	SAE	3X3X0.1875	A 36	Beam	Other	None	12.000
PMBR4	L3.5x3.5x1/4	SAE	3.5X3.5X0.25	A 36	Beam	Other	None	12.000
PMBR5	L4x4x1/4	SAE	4X4X0.25	A 36	Beam	Other	None	12.000
20a	H1	SAE	1.75X1.75X0.1875	A 36	Truss	Other	None	0.000
Angler	L2x2x1/4	SAE	2X2X0.25	A 36	Beam	Other	None	0.000
Bracer	L2.5x2.5x1/4	SAE	2.5X2.5X0.25	A 36	Beam	Other	None	0.000
Plate	6"x3/4" PL	Bar	6x3/4	A 36	Beam	Other	None	0.000

Aggregate Angle Information:

Note: Estimate of surface area reported for painting purposes, not wind loading.

Angle Type	Angle Size	Material Type	Length (ft)	Total Surface Area (ft ²)	Total Weight (lbs)
SAE	3.5X3.5X0.25	A 36	212.92	248.41	1234.96
SAE	6X6X0.3125	A 36	56.20	112.40	702.50
SAE	8X8X0.5	A 36	56.00	149.33	1478.40
SAE	8X8X0.625	A 36	94.98	253.29	3105.93
SAE	8X8X0.75	A 36	251.24	669.98	9773.41
SAE	2.5X2.5X0.1875	A 36	106.66	88.88	327.44
SAU	2.5X2X0.1875	A 36	295.39	221.54	812.33
SAU	4X3X0.25	A 36	318.53	371.62	1847.48
SAU	3.5X2.5X0.25	A 36	68.17	68.17	334.01
SAU	4X3.5X0.3125	A 36	147.51	184.39	1135.85
SAU	5X3.5X0.25	A 36	84.78	120.10	593.45
SAU	3.5X3X0.25	A 36	456.32	494.34	2464.10
SAU	5X3X0.25	A 36	346.59	462.12	2287.47
SAE	2X2X0.1875	A 36	153.55	126.82	464.18
SAU	3X2X0.25	A 36	84.00	127.96	629.55
SAE	1.75X1.75X0.1875	A 36	12.00	7.00	50.40
DAE	1.75X1.75X0.1875	A 36	62.95	78.69	390.29
SAU	4X3.5X0.25	A 36	145.30	145.30	539.06
SAE	2X2X0.25	A 36	16.97	11.31	54.14
SAU	3X2X0.1875	A 36	59.10	49.25	181.44
SAE	3X3X0.25	A 36	68.29	68.29	334.64
SAE	4X4X0.25	A 36	105.60	140.80	696.94
Pwmt	Pipe 12" Std.	A500-42	148.00	610.50	7340.80
Bar	6x3/4	A 36	1.50	1.69	22.96
SAE	2.5X2.5X0.25	A 36	6.71	5.59	27.51

Sections:

The adjustment factors below only apply to dead load and wind areas that are calculated for members in the model. They do not apply to equipment or to manually input dead load and drag areas.

Section Label	Joint Defining Section Adjust.	Dead Load Drag x Area Factor	Transverse Longitudinal Drag x Area Factor	Area Factor	AF Flat Ar Round Factor	Transverse Longitudinal Drag x Area Factor	SAPS Angle Drag x Area Factor	SAPS Round Force Drag x Area Solid Face Factor
1	8X	1.050	3.350	3.350	1.100	0.000	0.000	0.000
2	29P	1.050	3.500	3.500	1.200	0.000	0.000	0.000

Angle Member Connectivity:

Member	Group Section	Symmetry	Origin	End Ecc.	Rest.	Ratio	Ratio	Bolt	# Bolt #	Shear	Connect	Short	Long	End
Bolt	Shear Tension Rest.							Type	Bolts	Holes	Planes	Edge	Edge	Dist.
Spacing	Path	Path Coef.	Joint	Joint Code	Code	RUX	RLY	RLZ				Leg		
Length	Length											Dist.	Dist.	
(in)	(in)	(in)										(in)	(in)	(in)

5P	2	99.083	L/r	100.462	Net Sect	60	6.05	99.083	109.200	203.906	100.462	226.562	0.000	0.000	0.000
0.000		0.000	Automatic		Member "5P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked.										
5X	2	99.083	L/r	100.462	Net Sect	60	6.05	99.083	109.200	203.906	100.462	226.562	0.000	0.000	0.000
0.000		0.000	Automatic		Member "5X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked.										
5XY	2	99.083	L/r	100.462	Net Sect	60	6.05	99.083	109.200	203.906	100.462	226.562	0.000	0.000	0.000
0.000		0.000	Automatic		Member "5XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked.										
5Y	2	99.083	L/r	100.462	Net Sect	60	6.05	99.083	109.200	203.906	100.462	226.562	0.000	0.000	0.000
0.000		0.000	Automatic		Member "5Y" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked.										
6P	3	249.636	L/r	235.687	Net Sect	53	7.00	249.636	0.000	0.000	235.687	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
6X	3	249.636	L/r	235.687	Net Sect	53	7.00	249.636	0.000	0.000	235.687	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
6XY	3	249.636	L/r	235.687	Net Sect	53	7.00	249.636	0.000	0.000	235.687	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
6Y	3	249.636	L/r	235.687	Net Sect	53	7.00	249.636	0.000	0.000	235.687	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
7P	3	249.636	L/r	235.687	Net Sect	53	7.00	249.636	0.000	0.000	235.687	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
7X	3	249.636	L/r	235.687	Net Sect	53	7.00	249.636	0.000	0.000	235.687	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
7XY	3	249.636	L/r	235.687	Net Sect	53	7.00	249.636	0.000	0.000	235.687	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
7Y	3	249.636	L/r	235.687	Net Sect	53	7.00	249.636	0.000	0.000	235.687	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
8P	4	306.646	L/r	291.819	Net Sect	60	7.92	306.646	0.000	0.000	291.819	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
8X	4	306.646	L/r	291.819	Net Sect	60	7.92	306.646	0.000	0.000	291.819	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
8XY	4	306.646	L/r	291.819	Net Sect	60	7.92	306.646	0.000	0.000	291.819	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
8Y	4	306.646	L/r	291.819	Net Sect	60	7.92	306.646	0.000	0.000	291.819	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
9P	4	306.646	L/r	291.819	Net Sect	60	7.92	306.646	0.000	0.000	291.819	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
9X	4	306.646	L/r	291.819	Net Sect	60	7.92	306.646	0.000	0.000	291.819	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
9XY	4	306.646	L/r	291.819	Net Sect	60	7.92	306.646	0.000	0.000	291.819	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
9Y	4	306.646	L/r	291.819	Net Sect	60	7.92	306.646	0.000	0.000	291.819	0.000	0.000	0.000	0.000
0.000		0.000	Automatic												
10P	4	291.200	Shear	291.200	Shear	291.200	7.92	306.646	291.200	543.749	291.819	604.166	0.000	0.000	0.000
0.000		0.000	Automatic		Member "10P" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked.										
10X	4	291.200	Shear	291.200	Shear	291.200	7.92	306.646	291.200	543.749	291.819	604.166	0.000	0.000	0.000
0.000		0.000	Automatic		Member "10X" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked.										
10XY	4	291.200	Shear	291.200	Shear	291.200	7.92	306.646	291.200	543.749	291.819	604.166	0.000	0.000	0.000
0.000		0.000	Automatic		Member "10XY" will not be checked for block shear since more than one gage line exists (long edge distance (g) greater than zero); however, end, edge and spacing distances will be checked.										
10Y	4	291.200	Shear	291.200	Shear	291.200	7.92	306.646	291.200	543.749	291.819	604.166	0.000	0.000	0.000
0.000		0.000	Automatic												

0.000	distance (g)	greater than zero)	Automatic	Member "10Y" will not be checked for block shear since more than one gage line exists (long edge				
11P	5	335.162	L/r 337.589	Net Sect 77 10.13 335.162	0.000	0.000	337.589	0.000 0.000 0.000
0.000			Automatic					
0.000	11X	5	335.162	L/r 337.589	Net Sect 77 10.13 335.162	0.000	0.000	337.589 0.000 0.000
0.000			Automatic					
0.000	11XY	5	335.162	L/r 337.589	Net Sect 77 10.13 335.162	0.000	0.000	337.589 0.000 0.000
0.000			Automatic					
0.000	11Y	5	335.162	L/r 337.589	Net Sect 77 10.13 335.162	0.000	0.000	337.589 0.000 0.000
0.000			Automatic					
0.000	12P	5	364.000	Shear 337.589	Net Sect 45 11.83 385.718 364.000	815.624	337.589	906.249 0.000 0.000 0.000
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "12P" will not be checked for block shear since more than one gage line exists (long edge				
12X	5	364.000	Shear 337.589	Net Sect 45 11.83 385.718 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "12X" will not be checked for block shear since more than one gage line exists (long edge				
12XY	5	364.000	Shear 337.589	Net Sect 45 11.83 385.718 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "12XY" will not be checked for block shear since more than one gage line exists (long edge				
12Y	5	364.000	Shear 337.589	Net Sect 45 11.83 385.718 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "12Y" will not be checked for block shear since more than one gage line exists (long edge				
13P	5	364.000	Shear 337.589	Net Sect 58 15.32 368.010 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "13P" will not be checked for block shear since more than one gage line exists (long edge				
13X	5	364.000	Shear 337.589	Net Sect 58 15.32 368.010 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "13X" will not be checked for block shear since more than one gage line exists (long edge				
13XY	5	364.000	Shear 337.589	Net Sect 58 15.32 368.010 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "13XY" will not be checked for block shear since more than one gage line exists (long edge				
13Y	5	364.000	Shear 337.589	Net Sect 58 15.32 368.010 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "13Y" will not be checked for block shear since more than one gage line exists (long edge				
14P	5	364.000	Shear 337.589	Net Sect 48 25.53 381.402 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "14P" will not be checked for block shear since more than one gage line exists (long edge				
14X	5	364.000	Shear 337.589	Net Sect 48 25.53 381.402 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "14X" will not be checked for block shear since more than one gage line exists (long edge				
14XY	5	364.000	Shear 337.589	Net Sect 48 25.53 381.402 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "14XY" will not be checked for block shear since more than one gage line exists (long edge				
14Y	5	364.000	Shear 337.589	Net Sect 48 25.53 381.402 364.000	815.624	337.589	906.249 0.000 0.000 0.000	
0.000			Automatic					
0.000	distance (g)	greater than zero)	Automatic	Member "14Y" will not be checked for block shear since more than one gage line exists (long edge				
15AP	6	15.633	L/r 18.063	Rupture 131 10.82 15.633 18.200	20.391	25.048	18.063 0.000 0.000 0.000	
0.000			Automatic					
0.000	15AX	6	15.633	L/r 18.063	Rupture 131 10.82 15.633 18.200	20.391	25.048	18.063 0.000 0.000
0.000			Automatic					
0.000	15AXY	6	15.633	L/r 18.063	Rupture 131 10.82 15.633 18.200	20.391	25.048	18.063 0.000 0.000
0.000			Automatic					
0.000	15AY	6	15.633	L/r 18.063	Rupture 131 10.82 15.633 18.200	20.391	25.048	18.063 0.000 0.000
0.000			Automatic					
0.000	15BP	6	15.633	L/r 18.063	Rupture 131 10.82 15.633 18.200	20.391	25.048	18.063 0.000 0.000
0.000			Automatic					
0.000	15BX	6	15.633	L/r 18.063	Rupture 131 10.82 15.633 18.200	20.391	25.048	18.063 0.000 0.000
0.000			Automatic					

15BX	6	15.633	L/r	18.063	Rupture	131	10.82	15.633	18.200	20.391	25.048	18.063	0.000	0.000	0.000
0.000		0.000	Automatic												
15BY	6	15.633	L/r	18.063	Rupture	131	10.82	15.633	18.200	20.391	25.048	18.063	0.000	0.000	0.000
0.000		0.000	Automatic												
16AP	7	14.303	L/r	18.125	Rupture	130	9.22	14.303	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000		0.000	Automatic												
16AX	7	14.303	L/r	18.125	Rupture	130	9.22	14.303	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000		0.000	Automatic												
16AXY	7	14.303	L/r	18.125	Rupture	130	9.22	14.303	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000		0.000	Automatic												
16AY	7	14.303	L/r	18.125	Rupture	130	9.22	14.303	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000		0.000	Automatic												
16BP	7	14.303	L/r	18.125	Rupture	130	9.22	14.303	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000		0.000	Automatic												
16BX	7	14.303	L/r	18.125	Rupture	130	9.22	14.303	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000		0.000	Automatic												
16BX	7	14.303	L/r	18.125	Rupture	130	9.22	14.303	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000		0.000	Automatic												
16BY	7	14.303	L/r	18.125	Rupture	130	9.22	14.303	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000		0.000	Automatic												
17AP	8	39.613	L/r	45.066	Net Sect	100	10.00	39.613	45.500	67.969	45.066	60.417	0.000	0.000	0.000
0.000		0.000	Automatic												
distance (g) greater than zero)			Automatic	Member "17AP" will not be checked for block shear since more than one gage line exists (long edge											
17AX	8	39.613	L/r	45.066	Net Sect	100	10.00	39.613	45.500	67.969	45.066	60.417	0.000	0.000	0.000
0.000		0.000	Automatic	Member "17AX" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "17AX" will not be checked for block shear since more than one gage line exists (long edge											
17AXY	8	39.613	L/r	45.066	Net Sect	100	10.00	39.613	45.500	67.969	45.066	60.417	0.000	0.000	0.000
0.000		0.000	Automatic	Member "17AXY" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "17AXY" will not be checked for block shear since more than one gage line exists (long edge											
17AY	8	39.613	L/r	45.066	Net Sect	100	10.00	39.613	45.500	67.969	45.066	60.417	0.000	0.000	0.000
0.000		0.000	Automatic	Member "17AY" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "17AY" will not be checked for block shear since more than one gage line exists (long edge											
17BP	8	39.613	L/r	45.066	Net Sect	100	10.00	39.613	45.500	67.969	45.066	60.417	0.000	0.000	0.000
0.000		0.000	Automatic	Member "17BP" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "17BP" will not be checked for block shear since more than one gage line exists (long edge											
17BX	8	39.613	L/r	45.066	Net Sect	100	10.00	39.613	45.500	67.969	45.066	60.417	0.000	0.000	0.000
0.000		0.000	Automatic	Member "17BX" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "17BX" will not be checked for block shear since more than one gage line exists (long edge											
17BXY	8	39.613	L/r	45.066	Net Sect	100	10.00	39.613	45.500	67.969	45.066	60.417	0.000	0.000	0.000
0.000		0.000	Automatic	Member "17BXY" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "17BXY" will not be checked for block shear since more than one gage line exists (long edge											
18AP	9	32.738	L/r	36.400	Shear	104	8.52	32.738	36.400	54.375	40.920	42.647	0.000	0.000	0.000
0.000		0.000	Automatic	Member "18AP" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "18AP" will not be checked for block shear since more than one gage line exists (long edge											
18AX	9	32.738	L/r	36.400	Shear	104	8.52	32.738	36.400	54.375	40.920	42.647	0.000	0.000	0.000
0.000		0.000	Automatic	Member "18AX" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "18AX" will not be checked for block shear since more than one gage line exists (long edge											
18AXY	9	32.738	L/r	36.400	Shear	104	8.52	32.738	36.400	54.375	40.920	42.647	0.000	0.000	0.000
0.000		0.000	Automatic	Member "18AXY" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "18AXY" will not be checked for block shear since more than one gage line exists (long edge											
18AY	9	32.738	L/r	36.400	Shear	104	8.52	32.738	36.400	54.375	40.920	42.647	0.000	0.000	0.000
0.000		0.000	Automatic	Member "18AY" will not be checked for block shear since more than one gage line exists (long edge											
distance (g) greater than zero)			Automatic	Member "18AY" will not be checked for block shear since more than one gage line exists (long edge											
18BP	9	32.738	L/r	36.400	Shear	104	8.52	32.738	36.400	54.375	40.920	42.647	0.000	0.000	0.000
0.000		0.000	Automatic	Member "18BP" will not be checked for block shear since more than one gage line exists (long edge											

distance (g) greater than zero); however, end, edge and spacing distances will be checked.	??								
21AP 11 44.335	L/r 45.962	Net Sect 96 10.60	44.335	54.600	81.562	45.962	72.500	0.000	0.000
0.000	Automatic	Member "21AP" will not be checked for block shear since more than one gage line exists (long edge greater than zero); however, end, edge and spacing distances will be checked.	??						
21AX 11 44.335	L/r 45.962	Net Sect 96 10.60	44.335	54.600	81.562	45.962	72.500	0.000	0.000
0.000	Automatic	Member "21AX" will not be checked for block shear since more than one gage line exists (long edge greater than zero); however, end, edge and spacing distances will be checked.	??						
21AXY 11 44.335	L/r 45.962	Net Sect 96 10.60	44.335	54.600	81.562	45.962	72.500	0.000	0.000
0.000	Automatic	Member "21AXY" will not be checked for block shear since more than one gage line exists (long edge greater than zero); however, end, edge and spacing distances will be checked.	??						
21AY 11 44.335	L/r 45.962	Net Sect 96 10.60	44.335	54.600	81.562	45.962	72.500	0.000	0.000
0.000	Automatic	Member "21AY" will not be checked for block shear since more than one gage line exists (long edge greater than zero); however, end, edge and spacing distances will be checked.	??						
21BP 11 44.335	L/r 45.962	Net Sect 96 10.60	44.335	54.600	81.562	45.962	72.500	0.000	0.000
0.000	Automatic	Member "21BP" will not be checked for block shear since more than one gage line exists (long edge greater than zero); however, end, edge and spacing distances will be checked.	??						
21BX 11 44.335	L/r 45.962	Net Sect 96 10.60	44.335	54.600	81.562	45.962	72.500	0.000	0.000
0.000	Automatic	Member "21BX" will not be checked for block shear since more than one gage line exists (long edge greater than zero); however, end, edge and spacing distances will be checked.	??						
21BXY 11 44.335	L/r 45.962	Net Sect 96 10.60	44.335	54.600	81.562	45.962	72.500	0.000	0.000
0.000	Automatic	Member "21BXY" will not be checked for block shear since more than one gage line exists (long edge greater than zero); however, end, edge and spacing distances will be checked.	??						
22AP 12 30.985	Shear 126 12.25	30.985	36.400	54.375	41.087	44.391	0.000	0.000	0.000
0.000	Automatic								
22AX 12 30.985	Shear 126 12.25	30.985	36.400	54.375	41.087	44.391	0.000	0.000	0.000
0.000	Automatic								
22AXY 12 30.985	Shear 126 12.25	30.985	36.400	54.375	41.087	44.391	0.000	0.000	0.000
0.000	Automatic								
22AY 12 30.985	Shear 126 12.25	30.985	36.400	54.375	41.087	44.391	0.000	0.000	0.000
0.000	Automatic								
22BP 12 30.985	Shear 126 12.25	30.985	36.400	54.375	41.087	44.391	0.000	0.000	0.000
0.000	Automatic								
22BX 12 30.985	Shear 126 12.25	30.985	36.400	54.375	41.087	44.391	0.000	0.000	0.000
0.000	Automatic								
22BXY 12 30.985	Shear 126 12.25	30.985	36.400	54.375	41.087	44.391	0.000	0.000	0.000
0.000	Automatic								
22BY 12 30.985	Shear 126 12.25	30.985	36.400	54.375	41.087	44.391	0.000	0.000	0.000
0.000	Automatic								
23AP 13 22.542	Shear 147 14.07	22.542	27.300	40.781	40.925	38.516	0.000	0.000	0.000
0.000	Automatic								
23AX 13 22.542	Shear 147 14.07	22.542	27.300	40.781	40.925	38.516	0.000	0.000	0.000
0.000	Automatic								
23AXY 13 22.542	Shear 147 14.07	22.542	27.300	40.781	40.925	38.516	0.000	0.000	0.000
0.000	Automatic								
23AY 13 22.542	Shear 147 14.07	22.542	27.300	40.781	40.925	38.516	0.000	0.000	0.000
0.000	Automatic								
23BP 13 22.542	Shear 147 14.07	22.542	27.300	40.781	40.925	38.516	0.000	0.000	0.000
0.000	Automatic								
23BX 13 22.542	Shear 147 14.07	22.542	27.300	40.781	40.925	38.516	0.000	0.000	0.000
0.000	Automatic								
23BXY 13 22.542	Shear 147 14.07	22.542	27.300	40.781	40.925	38.516	0.000	0.000	0.000
0.000	Automatic								
23BY 13 22.542	Shear 147 14.07	22.542	27.300	40.781	40.925	38.516	0.000	0.000	0.000
0.000	Automatic								
24AP 14 21.431	Shear 174 17.45	21.431	27.300	40.781	41.087	36.250	0.000	0.000	0.000
0.000	Automatic								

0.000	24AX	14	21.431	Automatic	Shear	174	17.45	21.431	27.300	40.781	41.087	36.250	0.000	0.000
0.000			0.000	L/r 27.300										
0.000	24AXY	14	21.431	Automatic	Shear	174	17.45	21.431	27.300	40.781	41.087	36.250	0.000	0.000
0.000			0.000	L/r 27.300										
0.000	24AY	14	21.431	Automatic	Shear	174	17.45	21.431	27.300	40.781	41.087	36.250	0.000	0.000
0.000			0.000	L/r 27.300										
0.000	24BP	14	21.431	Automatic	Shear	174	17.45	21.431	27.300	40.781	41.087	36.250	0.000	0.000
0.000			0.000	L/r 27.300										
0.000	24BX	14	21.431	Automatic	Shear	174	17.45	21.431	27.300	40.781	41.087	36.250	0.000	0.000
0.000			0.000	L/r 27.300										
0.000	24BXY	14	21.431	Automatic	Shear	174	17.45	21.431	27.300	40.781	41.087	36.250	0.000	0.000
0.000			0.000	L/r 27.300										
0.000	24BY	14	21.431	Automatic	Shear	174	17.45	21.431	27.300	40.781	41.087	36.250	0.000	0.000
0.000			0.000	L/r 27.300										
0.000	25AP	15	33.799	Automatic	Net Sect	128	14.10	33.799	63.700	95.156	37.133	74.632	0.000	0.000
0.000			0.000	L/r 37.133										
distance (g)	greater than zero)			Automatic Member "25AP" will not be checked for block shear since more than one gage line exists (long edge										
0.000	25AX	15	33.799	Automatic	Net Sect	128	14.10	33.799	63.700	95.156	37.133	74.632	0.000	0.000
0.000			0.000	L/r 37.133										
distance (g)	greater than zero)			Automatic Member "25AX" will not be checked for block shear since more than one gage line exists (long edge										
0.000	25AXY	15	33.799	Automatic	Net Sect	128	14.10	33.799	63.700	95.156	37.133	74.632	0.000	0.000
0.000			0.000	L/r 37.133										
distance (g)	greater than zero)			Automatic Member "25AXY" will not be checked for block shear since more than one gage line exists (long edge										
0.000	25AY	15	33.799	Automatic	Net Sect	128	14.10	33.799	63.700	95.156	37.133	74.632	0.000	0.000
0.000			0.000	L/r 37.133										
distance (g)	greater than zero)			Automatic Member "25AY" will not be checked for block shear since more than one gage line exists (long edge										
0.000	25BP	15	33.799	Automatic	Net Sect	128	14.10	33.799	63.700	95.156	37.133	74.632	0.000	0.000
0.000			0.000	L/r 37.133										
distance (g)	greater than zero)			Automatic Member "25BP" will not be checked for block shear since more than one gage line exists (long edge										
0.000	25BX	15	33.799	Automatic	Net Sect	128	14.10	33.799	63.700	95.156	37.133	74.632	0.000	0.000
0.000			0.000	L/r 37.133										
distance (g)	greater than zero)			Automatic Member "25BX" will not be checked for block shear since more than one gage line exists (long edge										
0.000	25BXY	15	33.799	Automatic	Net Sect	128	14.10	33.799	63.700	95.156	37.133	74.632	0.000	0.000
0.000			0.000	L/r 37.133										
distance (g)	greater than zero)			Automatic Member "25BXY" will not be checked for block shear since more than one gage line exists (long edge										
0.000	25BY	15	33.799	Automatic	Net Sect	128	14.10	33.799	63.700	95.156	37.133	74.632	0.000	0.000
0.000			0.000	L/r 37.133										
distance (g)	greater than zero)			Automatic Member "25BY" will not be checked for block shear since more than one gage line exists (long edge										
0.000	26AP	16	4.044	Automatic	Rupture	257	16.85	4.044	18.200	20.391	18.827	18.125	0.000	0.000
0.000			0.000	L/r 18.125										
0.000	26AX	16	4.044	Automatic	Rupture	257	16.85	4.044	18.200	20.391	18.827	18.125	0.000	0.000
0.000			0.000	L/r 18.125										
0.000	26AXY	16	4.044	Automatic	Rupture	257	16.85	4.044	18.200	20.391	18.827	18.125	0.000	0.000
0.000			0.000	L/r 18.125										
0.000	26AY	16	4.044	Automatic	Rupture	257	16.85	4.044	18.200	20.391	18.827	18.125	0.000	0.000
0.000			0.000	L/r 18.125										
0.000	26BP	16	4.044	Automatic	Rupture	257	16.85	4.044	18.200	20.391	18.827	18.125	0.000	0.000
0.000			0.000	L/r 18.125										
0.000	26BX	16	4.044	Automatic	Rupture	257	16.85	4.044	18.200	20.391	18.827	18.125	0.000	0.000
0.000			0.000	L/r 18.125										
0.000	26BXY	16	4.044	Automatic	Rupture	257	16.85	4.044	18.200	20.391	18.827	18.125	0.000	0.000
0.000			0.000	L/r 18.125										
0.000	26BY	16	4.044	Automatic	Rupture	257	16.85	4.044	18.200	20.391	18.827	18.125	0.000	0.000
0.000			0.000	L/r 18.125										
0.000	27AP	17	6.420	Automatic	Net Sect	265	19.19	6.420	36.400	54.375	24.887	48.333	0.000	0.000
0.000			0.000	L/r 24.887										
0.000	27AX	17	6.420	Automatic	Net Sect	265	19.19	6.420	36.400	54.375	24.887	48.333	0.000	0.000
				L/r 24.887										

0.000	27AXY	17	0.000	Automatic	Net Sect	265	19.19	6.420	36.400	54.375	24.887	48.333	0.000	0.000	0.000
0.000			6.420	I/r 24.887											
0.000	27AY	17	0.000	Automatic	Net Sect	265	19.19	6.420	36.400	54.375	24.887	48.333	0.000	0.000	0.000
0.000			6.420	I/r 24.887											
0.000	27BP	17	0.000	Automatic	Net Sect	265	19.19	6.420	36.400	54.375	24.887	48.333	0.000	0.000	0.000
0.000			6.420	I/r 24.887											
0.000	27BX	17	0.000	Automatic	Net Sect	265	19.19	6.420	36.400	54.375	24.887	48.333	0.000	0.000	0.000
0.000			6.420	I/r 24.887											
0.000	27BY	17	0.000	Automatic	Net Sect	265	19.19	6.420	36.400	54.375	24.887	48.333	0.000	0.000	0.000
0.000			6.420	I/r 24.887											
0.000	28AP	18	0.000	Automatic	Net Sect	265	19.19	6.420	36.400	54.375	24.887	48.333	0.000	0.000	0.000
0.000			6.420	I/r 24.887											
0.000	distance (g)	greater than zero)	0.000	Automatic	Member "28AP" will not be checked for block shear since more than one gage line exists (long edge										
0.000	28AX	18	0.000	Automatic	Shear	205	15.32	11.489	45.500	67.969	47.739	60.417	0.000	0.000	0.000
0.000			11.489	I/r 45.500											
0.000	distance (g)	greater than zero)	0.000	Automatic	Member "28AX" will not be checked for block shear since more than one gage line exists (long edge										
0.000	28AXY	18	0.000	Automatic	Shear	205	15.32	11.489	45.500	67.969	47.739	60.417	0.000	0.000	0.000
0.000			11.489	I/r 45.500											
0.000	distance (g)	greater than zero)	0.000	Automatic	Member "28AXY" will not be checked for block shear since more than one gage line exists (long edge										
0.000	28AY	18	0.000	Automatic	Shear	205	15.32	11.489	45.500	67.969	47.739	60.417	0.000	0.000	0.000
0.000			11.489	I/r 45.500											
0.000	distance (g)	greater than zero)	0.000	Automatic	Member "28AY" will not be checked for block shear since more than one gage line exists (long edge										
0.000	28BP	18	0.000	Automatic	Shear	205	15.32	11.489	45.500	67.969	47.739	60.417	0.000	0.000	0.000
0.000			11.489	I/r 45.500											
0.000	distance (g)	greater than zero)	0.000	Automatic	Member "28BP" will not be checked for block shear since more than one gage line exists (long edge										
0.000	28BX	18	0.000	Automatic	Shear	205	15.32	11.489	45.500	67.969	47.739	60.417	0.000	0.000	0.000
0.000			11.489	I/r 45.500											
0.000	distance (g)	greater than zero)	0.000	Automatic	Member "28BX" will not be checked for block shear since more than one gage line exists (long edge										
0.000	28BY	18	0.000	Automatic	Shear	205	15.32	11.489	45.500	67.969	47.739	60.417	0.000	0.000	0.000
0.000			11.489	I/r 45.500											
0.000	distance (g)	greater than zero)	0.000	Automatic	Member "28BY" will not be checked for block shear since more than one gage line exists (long edge										
0.000	29AP	19	0.000	Automatic	Net Sect	136	28.52	25.652	45.500	67.969	44.975	53.634	0.000	0.000	0.000
0.000			25.652	I/r 44.975											
0.000	29AX	19	0.000	Automatic	Net Sect	136	28.52	25.652	45.500	67.969	44.975	53.634	0.000	0.000	0.000
0.000			25.652	I/r 44.975											
0.000	29AXY	19	0.000	Automatic	Net Sect	136	28.52	25.652	45.500	67.969	44.975	53.634	0.000	0.000	0.000
0.000			25.652	I/r 44.975											
0.000	29AY	19	0.000	Automatic	Net Sect	136	28.52	25.652	45.500	67.969	44.975	53.634	0.000	0.000	0.000
0.000			25.652	I/r 44.975											
0.000	29BP	19	0.000	Automatic	Net Sect	136	28.52	25.652	45.500	67.969	44.975	53.634	0.000	0.000	0.000
0.000			25.652	I/r 44.975											
0.000	29BX	19	0.000	Automatic	Net Sect	136	28.52	25.652	45.500	67.969	44.975	53.634	0.000	0.000	0.000
0.000			25.652	I/r 44.975											
0.000	29BY	19	0.000	Automatic	Net Sect	136	28.52	25.652	45.500	67.969	44.975	53.634	0.000	0.000	0.000
0.000			25.652	I/r 44.975											
0.000	30AP	20a	0.000	Automatic	Rupture	105	6.00	13.441	9.100	10.195	15.911	7.024	0.000	0.000	0.000
0.000			9.100	Shear 7.024											
0.000	30AY	20a	0.000	Automatic	Rupture	105	6.00	13.441	9.100	10.195	15.911	7.024	0.000	0.000	0.000
0.000			9.100	Shear 7.024											
0.000	30BP	20a	0.000	Automatic	Rupture	105	6.00	13.441	9.100	10.195	15.911	7.024	0.000	0.000	0.000
0.000			9.100	Shear 7.024											

0.000	30BX	20a	0.000	Shear	7.024	Automatic	Rupture	105	6.00	13.441	9.100	10.195	15.911	7.024	0.000	0.000
0.000			9.100	Automatic												
0.000	31P	20	0.000	L/r	7.024	Automatic	Rupture	210	6.00	4.027	9.100	10.195	15.911	7.024	0.000	0.000
0.000			4.027													
0.000	31Y	20	0.000	Automatic			Rupture	210	6.00	4.027	9.100	10.195	15.911	7.024	0.000	0.000
0.000			4.027													
0.000	32P	20	0.000	L/r	7.432	Automatic	Rupture	210	6.00	4.027	9.100	10.195	15.911	7.432	0.000	0.000
0.000			4.027													
0.000	32X	20	0.000	L/r	7.432	Automatic	Rupture	210	6.00	4.027	9.100	10.195	15.911	7.432	0.000	0.000
0.000			4.027													
0.000	33P	20	0.000	Automatic			Rupture	210	6.00	4.027	9.100	10.195	15.911	7.024	0.000	0.000
0.000			4.027													
0.000	33Y	20	0.000	L/r	7.024	Automatic	Rupture	210	6.00	4.027	9.100	10.195	15.911	7.024	0.000	0.000
0.000			4.027													
0.000	34P	20	0.000	Automatic			Rupture	210	6.00	4.027	9.100	10.195	15.911	7.432	0.000	0.000
0.000			4.027													
0.000	34X	20	0.000	L/r	7.432	Automatic	Rupture	210	6.00	4.027	9.100	10.195	15.911	7.432	0.000	0.000
0.000			4.027													
0.000	35P	20	0.000	Automatic			Rupture	210	6.00	4.027	9.100	10.195	15.911	7.024	0.000	0.000
0.000			4.027													
0.000	35Y	20	0.000	L/r	7.024	Automatic	Rupture	210	6.00	4.027	9.100	10.195	15.911	7.024	0.000	0.000
0.000			4.027													
0.000	36P	36	0.000	Automatic			Rupture	210	6.00	4.027	9.100	10.195	15.911	7.024	0.000	0.000
0.000			16.800													
0.000	36P	36	0.000	Shear	14.864	Automatic	Rupture	134	6.00	19.743	16.800	20.391	35.999	14.864	0.000	0.000
0.000			0.000													
0.000	36X	36	0.000	Automatic			Rupture	134	6.00	19.743	16.800	20.391	35.999	14.864	0.000	0.000
0.000			16.800													
0.000	37AP	21	0.000	Automatic			Shear	149	15.74	27.341	27.300	40.781	49.025	36.250	0.000	0.000
0.000			27.300													
0.000	37AY	21	0.000	Automatic			Shear	149	15.74	27.341	27.300	40.781	49.025	36.250	0.000	0.000
0.000			27.300													
0.000	37BP	21	0.000	Automatic			Shear	149	15.74	27.341	27.300	40.781	49.025	36.250	0.000	0.000
0.000			27.300													
0.000	37BX	21	0.000	Automatic			Shear	149	15.74	27.341	27.300	40.781	49.025	36.250	0.000	0.000
0.000			27.300													
0.000	38AP	22	0.000	Automatic			Rupture	193	9.57	11.503	18.200	20.391	31.139	18.125	0.000	0.000
0.000			11.503													
0.000	38AX	22	0.000	L/r	18.125	Automatic	Rupture	193	9.57	11.503	18.200	20.391	31.139	18.125	0.000	0.000
0.000			11.503													
0.000	38XY	22	0.000	Automatic			Rupture	193	9.57	11.503	18.200	20.391	31.139	18.125	0.000	0.000
0.000			11.503													
0.000	38AY	22	0.000	L/r	18.125	Automatic	Rupture	193	9.57	11.503	18.200	20.391	31.139	18.125	0.000	0.000
0.000			11.503													
0.000	38BP	22	0.000	Automatic			Rupture	193	9.57	11.503	18.200	20.391	31.139	18.125	0.000	0.000
0.000			11.503													
0.000	38BX	22	0.000	L/r	18.125	Automatic	Rupture	193	9.57	11.503	18.200	20.391	31.139	18.125	0.000	0.000
0.000			11.503													
0.000	38XY	22	0.000	Automatic			Rupture	193	9.57	11.503	18.200	20.391	31.139	18.125	0.000	0.000
0.000			11.503													
0.000	38BY	22	0.000	L/r	18.125	Automatic	Rupture	193	9.57	11.503	18.200	20.391	31.139	18.125	0.000	0.000
0.000			11.503													
0.000	39AP	23	0.000	Automatic			Shear	173	9.56	21.560	18.200	27.187	41.087	24.167	0.000	0.000
0.000			18.200													
0.000	39AX	23	0.000	Automatic			Shear	173	9.56	21.560	18.200	27.187	41.087	24.167	0.000	0.000
0.000			18.200													
0.000	39XY	23	0.000	Automatic			Shear	173	9.56	21.560	18.200	27.187	41.087	24.167	0.000	0.000
0.000			18.200													
0.000	39AY	23	0.000	Automatic			Shear	173	9.56	21.560	18.200	27.187	41.087	24.167	0.000	0.000
0.000			18.200													
0.000	39BP	23	0.000	Automatic			Shear	173	9.56	21.560	18.200	27.187	41.087	24.167	0.000	0.000
0.000			18.200													

0.000	43BP	26	12.587	L/R 13.170	Rupture	129	4.24	12.587	18.200	20.391	18.827	13.170	0.000	0.000	0.000
0.000	43EX	26	12.587	Automatic	Rupture	129	4.24	12.587	18.200	20.391	18.827	13.170	0.000	0.000	0.000
0.000	43EXY	26	12.587	L/R 13.170	Rupture	129	4.24	12.587	18.200	20.391	18.827	13.170	0.000	0.000	0.000
0.000	43BY	26	12.587	Automatic	Rupture	129	4.24	12.587	18.200	20.391	18.827	13.170	0.000	0.000	0.000
0.000	43CP	26	12.587	L/R 13.170	Rupture	129	4.24	12.587	18.200	20.391	18.827	13.170	0.000	0.000	0.000
0.000	43CX	26	12.587	Automatic	Rupture	129	4.24	12.587	18.200	20.391	18.827	13.170	0.000	0.000	0.000
0.000	43CXY	26	12.587	L/R 13.170	Rupture	129	4.24	12.587	18.200	20.391	18.827	13.170	0.000	0.000	0.000
0.000	43CY	26	12.587	Automatic	Rupture	129	4.24	12.587	18.200	20.391	18.827	13.170	0.000	0.000	0.000
0.000	44P	27	16.678	L/R 18.125	Rupture	116	8.22	16.678	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44Y	27	16.678	Automatic	Rupture	116	8.22	16.678	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44AP	27	16.678	L/R 18.125	Rupture	116	8.22	16.678	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44AY	27	16.678	Automatic	Rupture	116	8.22	16.678	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44BP	27	14.533	L/R 18.125	Rupture	128	9.13	14.533	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44BY	27	14.533	Automatic	Rupture	128	9.13	14.533	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44CP	27	14.533	L/R 18.125	Rupture	128	9.13	14.533	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44CY	27	14.533	Automatic	Rupture	128	9.13	14.533	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44DP	27	16.678	L/R 18.125	Rupture	116	8.22	16.678	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44DY	27	16.678	Automatic	Rupture	116	8.22	16.678	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44EP	27	16.678	L/R 18.125	Rupture	116	8.22	16.678	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	44EY	27	16.678	Automatic	Rupture	116	8.22	16.678	18.200	20.391	19.030	18.125	0.000	0.000	0.000
0.000	45P	28	18.200	Shear 18.200	Shear	111	6.00	35.407	18.200	27.187	41.087	24.167	0.000	0.000	0.000
0.000	45AP	28	18.200	Automatic	Shear	111	6.00	35.407	18.200	27.187	41.087	24.167	0.000	0.000	0.000
0.000	45BP	28	18.200	Automatic	Shear	111	6.00	35.407	18.200	27.187	41.087	24.167	0.000	0.000	0.000
0.000	46P	29	7.736	L/R 13.577	Rupture	202	14.78	7.736	18.200	20.391	18.908	13.577	0.000	0.000	0.000
0.000	46X	29	7.736	Automatic	Rupture	202	14.78	7.736	18.200	20.391	18.908	13.577	0.000	0.000	0.000
0.000	46XY	29	7.736	L/R 13.577	Rupture	202	14.78	7.736	18.200	20.391	18.908	13.577	0.000	0.000	0.000
0.000	46Y	29	7.736	Automatic	Rupture	202	14.78	7.736	18.200	20.391	18.908	13.577	0.000	0.000	0.000
0.000	47P	30	5.767	L/R 13.577	Rupture	201	13.25	5.767	18.200	20.391	19.030	13.577	0.000	0.000	0.000
0.000	47Y	30	5.767	Automatic	Rupture	201	13.25	5.767	18.200	20.391	19.030	13.577	0.000	0.000	0.000
0.000	47AP	30	5.486	L/R 15.209	Rupture	206	13.59	5.486	18.200	20.391	19.030	15.209	0.000	0.000	0.000

0.000	47AY	30	5.486	L/r	15.209	Rupture	206	13.59	5.486	18.200	20.391	19.030	15.209	0.000	0.000	0.000
			0.000	Automatic												
0.000	48P	31	8.465	L/r	18.125	Rupture	192	15.02	8.465	18.200	20.391	31.139	18.125	0.000	0.000	0.000
			0.000	Automatic												
0.000	48Y	31	8.465	L/r	18.125	Rupture	192	15.02	8.465	18.200	20.391	31.139	18.125	0.000	0.000	0.000
			0.000	Automatic												
0.000	48AP	31	8.140	L/r	14.801	Rupture	196	15.32	8.140	18.200	20.391	31.139	14.801	0.000	0.000	0.000
			0.000	Automatic												
0.000	48AY	31	8.140	L/r	14.801	Rupture	196	15.32	8.140	18.200	20.391	31.139	14.801	0.000	0.000	0.000
			0.000	Automatic												
0.000	49P	30	5.767	L/r	13.985	Rupture	201	13.25	5.767	18.200	20.391	19.030	13.985	0.000	0.000	0.000
			0.000	Automatic												
0.000	49Y	30	5.767	L/r	13.985	Rupture	201	13.25	5.767	18.200	20.391	19.030	13.985	0.000	0.000	0.000
			0.000	Automatic												
0.000	49AP	30	5.486	L/r	14.801	Rupture	206	13.59	5.486	18.200	20.391	19.030	14.801	0.000	0.000	0.000
			0.000	Automatic												
0.000	49AY	30	5.486	L/r	14.801	Rupture	206	13.59	5.486	18.200	20.391	19.030	14.801	0.000	0.000	0.000
			0.000	Automatic												
0.000	50P	32	18.200	Shear	18.125	Rupture	143	14.07	21.867	18.200	27.187	37.802	18.125	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "50P" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	50X	32	18.200	Shear	18.125	Rupture	143	14.07	21.867	18.200	27.187	37.802	18.125	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "50X" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	50XY	32	18.200	Shear	18.125	Rupture	143	14.07	21.867	18.200	27.187	37.802	18.125	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "50XY" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	50Y	32	18.200	Shear	18.125	Rupture	143	14.07	21.867	18.200	27.187	37.802	18.125	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "50Y" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	50AP	32	18.200	Shear	18.200	Shear	122	6.00	27.975	18.200	27.187	39.417	21.146	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "50AP" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	50AY	32	18.200	Shear	18.200	Shear	122	6.00	27.975	18.200	27.187	39.417	21.146	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "50AY" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	51P	33	32.661	L/r	34.020	Net Sect	111	11.64	32.661	36.400	54.375	34.020	42.647	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "51P" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	51Y	33	32.661	L/r	34.020	Net Sect	111	11.64	32.661	36.400	54.375	34.020	42.647	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "51Y" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	51AP	33	31.965	L/r	36.400	Shear	114	6.00	31.965	36.400	54.375	37.918	42.647	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "51AP" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	51AY	33	31.965	L/r	36.400	Shear	114	6.00	31.965	36.400	54.375	37.918	42.647	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "51AY" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	51BP	33	27.300	Shear	27.300	Shear	107	5.62	33.416	27.300	40.781	34.800	31.985	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "51BP" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	51BY	33	27.300	Shear	27.300	Shear	107	5.62	33.416	27.300	40.781	34.800	31.985	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "51BY" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	51CP	33	33.416	L/r	36.400	Shear	107	5.63	33.416	36.400	54.375	38.683	42.647	0.000	0.000	0.000
			0.000	Automatic												
distance (g)			greater than zero)			Member "51CP" will not be checked for block shear since more than one gage line exists (long edge										
						edge and spacing distances will be checked. ??										
0.000	51CY	33	33.416	L/r	36.400	Shear	107	5.63	33.416	36.400	54.375	38.683	42.647	0.000	0.000	0.000
			0.000	Automatic												

0.000	distance (g) greater than zero)	Automatic	Member "51CY" will not be checked for block shear since more than one gage line exists (long edge	0.000
52P	34 41.627	L/r 45.500	Shear 106 14.07 41.627 45.500 67.969 47.765 60.417 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "52P" will not be checked for block shear since more than one gage line exists (long edge	0.000
52Y	34 41.627	L/r 45.500	Shear 106 14.07 41.627 45.500 67.969 47.765 60.417 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "52Y" will not be checked for block shear since more than one gage line exists (long edge	0.000
52AP	34 45.317	L/r 35.539	Rupture 91 6.00 45.317 45.500 67.969 52.721 35.539 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "52AP" will not be checked for block shear since more than one gage line exists (long edge	0.000
52AY	34 45.317	L/r 35.539	Rupture 91 6.00 45.317 45.500 67.969 52.721 35.539 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "52AY" will not be checked for block shear since more than one gage line exists (long edge	0.000
52BP	34 42.220	L/r 45.500	Shear 104 6.88 42.220 45.500 67.969 51.718 71.078 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "52BP" will not be checked for block shear since more than one gage line exists (long edge	0.000
52BY	34 42.220	L/r 45.500	Shear 104 6.88 42.220 45.500 67.969 51.718 71.078 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "52BY" will not be checked for block shear since more than one gage line exists (long edge	0.000
52CP	34 42.220	L/r 48.043	Net Sect 104 6.88 42.220 54.600 81.562 48.043 63.970 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "52CP" will not be checked for block shear since more than one gage line exists (long edge	0.000
52CY	34 42.220	L/r 48.043	Net Sect 104 6.88 42.220 54.600 81.562 48.043 63.970 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "52CY" will not be checked for block shear since more than one gage line exists (long edge	0.000
53P	33 32.661	L/r 34.020	Net Sect 111 11.64 32.661 36.400 54.375 34.020 42.647 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "53P" will not be checked for block shear since more than one gage line exists (long edge	0.000
53Y	33 32.661	L/r 34.020	Net Sect 111 11.64 32.661 36.400 54.375 34.020 42.647 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "53Y" will not be checked for block shear since more than one gage line exists (long edge	0.000
53AP	33 31.965	L/r 36.400	Shear 114 6.00 31.965 36.400 54.375 40.925 42.647 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "53AP" will not be checked for block shear since more than one gage line exists (long edge	0.000
53AY	33 31.965	L/r 36.400	Shear 114 6.00 31.965 36.400 54.375 40.925 42.647 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "53AY" will not be checked for block shear since more than one gage line exists (long edge	0.000
53BP	33 27.300	Shear 27.300	Shear 107 5.62 33.416 27.300 40.781 35.356 31.985 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "53BP" will not be checked for block shear since more than one gage line exists (long edge	0.000
53BY	33 27.300	Shear 27.300	Shear 107 5.62 33.416 27.300 40.781 35.356 31.985 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "53BY" will not be checked for block shear since more than one gage line exists (long edge	0.000
53CP	33 33.416	L/r 36.400	Shear 107 5.63 33.416 36.400 54.375 38.237 42.647 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "53CP" will not be checked for block shear since more than one gage line exists (long edge	0.000
53CY	33 33.416	L/r 36.400	Shear 107 5.63 33.416 36.400 54.375 38.237 42.647 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "53CY" will not be checked for block shear since more than one gage line exists (long edge	0.000
54P	35 8.154	L/r 9.100	Shear 169 6.00 8.154 9.100 10.195 22.067 9.629 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "54P" will not be checked for block shear since more than one gage line exists (long edge	0.000
54Y	35 8.154	L/r 9.100	Shear 169 6.00 8.154 9.100 10.195 22.067 9.629 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "54Y" will not be checked for block shear since more than one gage line exists (long edge	0.000
g99P	Pwmt 437.242	L/r 571.199	Net Sect 80 29.25 437.242 0.000 0.000 571.199 0.000 0.000 0.000	0.000
0.000	distance (g) greater than zero)	Automatic	Member "g99P" will not be checked for block shear since more than one gage line exists (long edge	0.000
g100P	Pwmt 535.971	L/r 571.199	Net Sect 41 15.00 535.971 0.000 0.000 571.199 0.000 0.000 0.000	0.000

Joint Label	Dead Load (kips)	X-Dray Area (ft^2)	Y-Dray Area (ft^2)
1P	0.117	5.752	4.127
2P	0.0615	3.558	2.426
3P	0.116	7.249	6.160
4P	0.213	10.554	9.179
5P	0.222	10.953	9.394
6P	0.315	12.098	10.143
7P	0.351	11.891	10.802
8P	0.433	14.318	13.381
15P	0.651	16.233	16.233
16P	0.114	7.054	2.102
17P	0.0621	2.786	2.354
18P	0.0805	3.740	2.381
19P	0.0621	2.786	2.354
20AP	0.39	20.837	17.520
20BP	0.403	17.907	21.056
21P	0.22	11.891	8.870
22P	0.22	8.870	11.891
23P	0.1	6.042	2.462
24P	0.15	8.339	2.688
25P	0.1	6.042	2.462
26P	0.053	2.813	1.250
27P	0.0705	3.724	1.250
28P	0.053	2.813	1.250
29P	0.725	15.539	15.539
30P	1.25	27.431	28.453
31P	1.59	34.533	34.802
32P	1.47	31.836	31.648
33P	0.707	15.500	15.312
34P	0.756	16.567	16.375
35P	0.571	12.325	12.375
36P	0.372	7.969	7.969
37P	0.223	4.781	4.781
1X	0.117	5.752	4.127
1Y	0.11	5.439	3.970
1Y	0.11	5.439	3.970
2X	0.0615	3.558	2.426
2X	0.0615	3.558	2.426
2Y	0.0615	3.558	2.426
3X	0.117	7.249	6.224
3X	0.117	7.249	6.224
3Y	0.116	7.249	6.160
4X	0.218	10.789	8.992
4X	0.213	10.476	8.835
4Y	0.208	10.242	9.023
5X	0.222	10.953	9.482
5X	0.222	10.953	9.482
5Y	0.222	10.953	9.394
6X	0.326	12.528	10.018
6X	0.321	12.216	9.861
6Y	0.31	11.786	9.986
7X	0.352	11.891	10.866
7X	0.352	11.891	10.866
7Y	0.351	11.891	10.802
8X	0.438	14.533	13.193

1	10.937	420.113	337.810	139.781	158.944
2	26.071	728.141	730.724	227.007	321.570
Total	37.007	1148.254	1068.534	366.789	480.514

Angle Member Weights and Surface Areas by Section:

Section Label	Unfactored Weight (kips)	Factored Weight (kips)	Unfactored Surface Area (ft^2)	Factored Surface Area (ft^2)
1	10.937	11.483	1715.817	1801.607
2	26.071	27.374	3034.480	3186.204
Total	37.007	38.858	4750.297	4987.812

Section Joint Information:

Section Label	Joint Label	Joint Elevation (ft)
1	1P	128.800
1	2P	124.300
1	1X	128.800
1	2X	124.300
1	1XY	128.800
1	2XY	124.300
1	1Y	128.800
1	2Y	124.300
1	3P	119.800
1	3X	119.800
1	3XY	119.800
1	3Y	119.800
1	4P	112.800
1	4X	112.800
1	4XY	112.800
1	4Y	112.800
1	5P	104.800
1	5X	104.800
1	5XY	104.800
1	5Y	104.800
1	6P	98.750
1	6X	98.750
1	6XY	98.750
1	6Y	98.750
1	7P	91.750
1	7X	91.750
1	7XY	91.750
1	7Y	91.750
1	8P	84.750
1	8X	84.750
1	8XY	84.750
1	8Y	84.750
1	10.50E129S	128.800
1	10.50E113S	112.800
1	10.50E99S	98.750
1	10.50E85S	84.750
1	28Y	84.750
1	28P	84.750

1	19Y	84.750
1	19P	84.750
1	27Y	98.750
1	27P	98.750
1	18Y	98.750
1	18P	98.750
1	26Y	112.800
1	26P	112.800
1	17Y	112.800
1	17P	112.800
1	16P	128.800
1	16X	128.800
1	23P	112.800
1	24P	98.750
1	25P	84.750
1	32P	84.750
1	33P	98.750
1	34P	112.750
1	35P	128.750
1	36P	134.750
1	37P	143.750
2	8P	84.750
2	9S	77.000
2	8X	84.750
2	9X	77.000
2	8XY	84.750
2	9XY	77.000
2	8Y	84.750
2	9Y	77.000
2	10S	69.250
2	10X	69.250
2	10XY	69.250
2	10Y	69.250
2	11S	61.500
2	11X	61.500
2	11XY	61.500
2	11Y	61.500
2	12S	51.580
2	12X	51.580
2	12XY	51.580
2	12Y	51.580
2	13S	40.000
2	13X	40.000
2	13XY	40.000
2	13Y	40.000
2	14S	25.000
2	14X	25.000
2	14XY	25.000
2	14Y	25.000
2	15P	0.000
2	15X	0.000
2	15XY	0.000
2	15Y	0.000
2	20AP	40.000
2	20AY	40.000
2	20BP	40.000
2	20BX	40.000
2	21P	25.000
2	21X	25.000
2	21XY	25.000

2	21Y	25.000
2	22P	25.000
2	22X	25.000
2	22XY	25.000
2	22Y	25.000
2	21XF0.50S	25.000
2	21XF0.50Y	25.000
2	22PF0.50S	25.000
2	22PF0.50X	25.000
2	29P	-4.250
2	30P	25.000
2	31P	40.000
2	32P	84.750

Sections Information:

Section Label	Top Z (ft)	Bottom Z (ft)	Joint Count	Member Count	Tran. Face Top (ft)	Tran. Face Bot (ft)	Tran. Face Gross Area (ft^2)	Long. Face Top (ft)	Long. Face Bot (ft)	Long. Face Gross Area (ft^2)
1	143.750	84.750	59	188	0.00	6.00	309.150	0.00	28.50	817.275
face for section "1": width is zero at elevation 128.80 (ft) which is not the top of the section. ??										
2	84.750	-4.250	52	147	6.00	0.00	1226.814	6.00	0.00	1226.814
face for section "2": width is zero at elevation -4.25 (ft) which is not the top of the section. ??										

Problem calculating gross area of longitudinal

Problem calculating gross area of longitudinal

*** Insulator Data

Clamp Properties:

Label	Stock Holding
Number	Capacity
(lbs)	

C-EX1 5e+004

Clamp Insulator Connectivity:

Clamp Structure Label	Property Min. Attach	Required Set Vertical Load (uplift) (lbs)
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1	16X	C-EX1	No Limit
2	16P	C-EX1	No Limit
3	17P	C-EX1	No Limit
4	17Y	C-EX1	No Limit
5	18P	C-EX1	No Limit
6	18Y	C-EX1	No Limit
7	19P	C-EX1	No Limit
8	19Y	C-EX1	No Limit
9	23P	C-EX1	No Limit
10	24P	C-EX1	No Limit
11	25P	C-EX1	No Limit
12	2Y	C-EX1	No Limit
13	4Y	C-EX1	No Limit
14	6Y	C-EX1	No Limit
15	8Y	C-EX1	No Limit
16	10Y	C-EX1	No Limit
17	12Y	C-EX1	No Limit

18			
19			
20			
21			
22			
23			
24			
25			
26			
27			
28			
29			
30			
31			
13Y	C-EX1	No Limit	
14Y	C-EX1	No Limit	
30P	C-EX1	No Limit	
31P	C-EX1	No Limit	
32P	C-EX1	No Limit	
33P	C-EX1	No Limit	
34P	C-EX1	No Limit	
35P	C-EX1	No Limit	
36P	C-EX1	No Limit	
37P	C-EX1	No Limit	
10XY	C-EX1	No Limit	
12XY	C-EX1	No Limit	
13XY	C-EX1	No Limit	
14XY	C-EX1	No Limit	

Loads from file: j:\jobs\1600900.wi\003_cob (temp) struct no 1281\04_structural\backup documentation\pls tower\cl&p # 1281.1ca

Loading Method Parameters:

Z of ground for wind height adjust	0.00 (ft)	and structure Z coordinate that will be put on the centerline ground profile in PHS-CADD.
Ground elevation shift	0.00 (ft)	
Z of ground with shift	0.00 (ft)	
Z of structure top (highest joint)	143.75 (ft)	
Structure height	148.00 (ft)	
Structure height above ground	143.75 (ft)	
Tower Shape	Rectangular	

Vector Load Cases:

Load Case Description	Dead Load Factor	Wind Area	Steel Poles Tubular Arms and Towers Cables	SF for SF Insuls.	SF for Found.	Point Loads	Wind/Ice Model	Trans. Wind Pressure (psf)	Longit. Wind Pressure (psf)	Ice Thick. (in)	Density (lbs/Ft^3)	Ice Temperature (deg F)	Joint Displ.
NESC Heavy	1.5000	2.5000	1.00000	1.0000	1.0000	1.0000	34 loads	Wind on Face	4	0	0.000	0.000	0.0
NESC Extreme	1.0000	1.0000	1.00000	1.0000	1.0000	1.0000	34 loads	NESC 2007	31	0	0.000	0.000	0.0

Joint Label	Vertical		Transverse		Longitudinal		Load Comment
	Load (lbs)	Load (lbs)	Load (lbs)	Load (lbs)			
16X	1148	9666	0	Shield Wire			
16P	1148	9666	0	Shield Wire			
17P	151	7276	7000	Conductor - Back			
17Y	2275	7178	-7000	Conductor - Ahead			
18P	151	7276	7000	Conductor - Back			
18Y	2275	7178	-7000	Conductor - Ahead			
19P	151	7276	7000	Conductor - Back			
19Y	2275	7178	-7000	Conductor - Ahead			
23P	2426	14454	0	Conductor			
24P	2426	14454	0	Conductor			
25P	2426	14454	0	Conductor			
37P	6929	564	0	Sprint Antennas			
36P	2494	1061	0	Verizon Antennas			
37P	1035	99	0	Coax Cable on Powermount			
35P	736	95	0	Coax Cable on Powermount			
34P	1380	178	0	Coax Cable on Powermount			
33P	1288	167	0	Coax Cable on Powermount			
32P	2702	349	0	Coax Cable on Powermount			
31P	2748	355	0	Coax Cable on Powermount			
30P	2989	387	0	Coax Cable on Powermount			

Section Label	Z of Top	Z of Bottom	Ave. Res. Adj. Wind Pres.	Tran Drag Coef	Tran Wind Load	Long Drag Coef	Long Wind Load	Ice Weight	Total Weight
2Y	471	176	0	0	0	0	0	0	0
4Y	588	219	0	0	0	0	0	0	0
6Y	645	241	0	0	0	0	0	0	0
8Y	678	253	0	0	0	0	0	0	0
10Y	763	285	0	0	0	0	0	0	0
12Y	673	251	0	0	0	0	0	0	0
13Y	611	228	0	0	0	0	0	0	0
14Y	1495	558	0	0	0	0	0	0	0
10XY	200	31	0	0	0	0	0	0	0
10Y	381	158	0	0	0	0	0	0	0
10XY	153	66	0	0	0	0	0	0	0
12XY	230	99	0	0	0	0	0	0	0
13XY	199	86	0	0	0	0	0	0	0
14XY	491	212	0	0	0	0	0	0	0

Section Load Case Information (Standard) for "NESC Heavy":

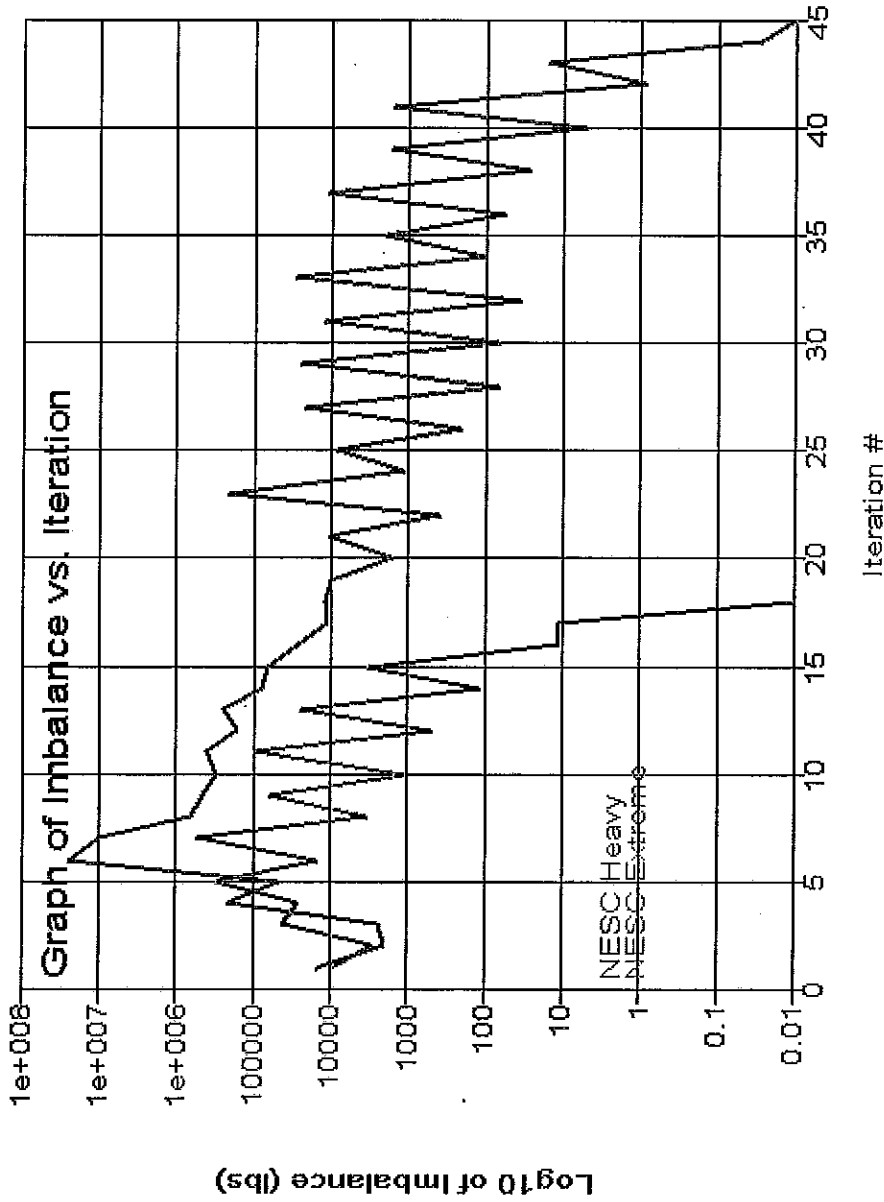
	(ft)	(ft)	(psf)	(psf)	(lbs)	(psf)	(lbs)	(lbs)	(lbs)
1	143.75	84.75	114.25	10.00	10.00	3.350	5324.6	0.00	3.350
2	84.75	-4.25	40.25	10.00	10.00	3.500	11254.9	0.00	3.500

Point Loads for Load Case "NESC Extreme":

Joint Label	Vertical Load (lbs)	Transverse Load (lbs)	Longitudinal Load (lbs)	Load Comment
16X	333	5774	361	Shield Wire
16P	333	5774	361	Shield Wire
17P	64	5168	4571	Conductor - Back
17Y	968	4164	-3778	Conductor - Ahead
18P	64	5168	4571	Conductor - Back
18Y	968	4164	-3778	Conductor - Ahead
19P	64	5168	4571	Conductor - Back
19Y	968	4164	-3778	Conductor - Ahead
23P	1033	9332	793	Conductor
24P	1033	9332	793	Conductor
25P	1033	9332	793	Conductor
37P	3191	2276	0	Sprint Antennas
36P	1068	4127	0	Verizon Antennas
37P	281	283	0	Coax Cable on Powermount
35P	200	302	0	Coax Cable on Powermount
34P	374	565	0	Coax Cable on Powermount
33P	349	528	0	Coax Cable on Powermount
32P	733	1107	0	Coax Cable on Powermount
31P	746	1126	0	Coax Cable on Powermount
30P	811	1225	0	Coax Cable on Powermount
2Y	128	580	0	Coax Cable on Tower
4Y	159	722	0	Coax Cable on Tower
6Y	175	793	0	Coax Cable on Tower
8Y	184	834	0	Coax Cable on Tower
10Y	207	938	0	Coax Cable on Tower
12Y	183	827	0	Coax Cable on Tower
13Y	166	752	0	Coax Cable on Tower

*** Analysis Results:

Maximum element usage is 90.98% for Angle "3P" in load case "NESC Extreme"
Maximum insulator usage is 29.57% for Clamp "10" in load case "NESC Heavy"



Angle Forces For All Load Cases:
Positive for tension - negative for compression

Group Label	Angle Max. Usage	Max. Tens. %	Max. Comp. For All IC	LC 1 (kips)	LC 2 (kips)
1 1P	23.90	0.000	-11.775	-10.562	-11.775
1 1X	23.38	11.331	0.000	10.259	11.331
1 1XY	21.35	10.348	0.000	8.837	10.348
1 1Y	21.39	0.000	-10.537	-9.535	-10.537

1	2P	24.76	0.000	-12.198	-11.217	-12.198
1	2X	22.28	10.800	0.000	9.357	10.800
1	2XY	20.72	10.043	0.000	8.163	10.043
1	2Y	22.47	0.000	-11.066	-10.704	-11.066
1	3P	90.98	0.000	-29.853	-26.038	-29.853
1	3X	73.56	26.775	0.000	23.155	26.775
1	3XY	72.39	26.349	0.000	21.727	26.349
1	3Y	86.24	0.000	-28.300	-27.174	-28.300
2	4P	59.43	0.000	-54.206	-51.658	-54.206
2	4X	48.89	49.120	0.000	47.679	49.120
2	4XY	50.16	50.387	0.000	45.477	50.387
2	4Y	58.12	0.000	-53.007	-53.007	-51.930
2	5P	84.60	0.000	-83.826	-83.826	-83.506
2	5X	77.37	77.724	0.000	77.724	74.711
2	5XY	78.83	79.193	0.000	74.983	79.193
2	5Y	87.73	0.000	-86.923	-86.923	-80.342
3	6P	50.32	0.000	-125.623	-125.623	-120.202
3	6X	50.05	117.971	0.000	117.971	107.501
3	6XY	48.90	115.255	0.000	114.854	115.255
3	6Y	51.64	0.000	-128.908	-128.908	-113.952
3	7P	66.80	0.000	-166.750	-166.750	-156.586
3	7X	66.97	157.846	0.000	157.846	139.715
3	7XY	65.21	153.695	0.000	153.695	150.072
3	7Y	68.57	0.000	-171.178	-171.178	-146.817
4	8P	63.56	0.000	-194.918	-194.918	-181.484
4	8X	62.82	183.309	0.000	183.309	159.004
4	8XY	61.56	179.651	0.000	179.651	171.673
4	8Y	64.97	0.000	-199.232	-199.232	-166.947
4	9P	68.43	0.000	-209.830	-209.830	-193.714
4	9X	67.09	195.772	0.000	195.772	168.577
4	9XY	65.76	191.900	0.000	191.900	181.553
4	9Y	69.38	0.000	-212.751	-212.751	-175.256
4	10P	71.64	0.000	-208.626	-208.626	-192.739
4	10X	65.91	191.938	0.000	191.938	164.633
4	10XY	64.47	187.723	0.000	187.723	176.728
4	10Y	72.74	0.000	-211.817	-211.817	-174.233
5	11P	62.92	0.000	-210.888	-210.888	-194.444
5	11X	57.26	193.318	0.000	193.318	167.364
5	11XY	56.28	189.997	0.000	189.997	181.250
5	11Y	63.48	0.000	-212.763	-212.763	-174.607
5	12P	59.12	0.000	-215.180	-215.180	-208.299
5	12X	51.90	175.217	0.000	175.217	143.203
5	12XY	48.77	164.640	0.000	164.640	147.977
5	12Y	61.65	0.000	-224.407	-224.407	-198.956
5	13P	60.29	0.000	-219.442	-219.442	-214.015
5	13X	48.74	164.552	0.000	164.552	131.057
5	13XY	45.96	155.143	0.000	155.143	138.582
5	13Y	62.19	0.000	-226.376	-226.376	-205.466
5	14P	60.68	0.000	-220.886	-220.886	-214.446
5	14X	50.55	170.648	0.000	170.648	136.989
5	14XY	48.90	165.083	0.000	165.083	155.259
5	14Y	63.01	0.000	-229.360	-229.360	-206.367
6	15AP	75.20	13.584	0.000	11.593	13.584
6	15AX	86.45	0.000	-13.515	-11.292	-13.515
6	15AXY	65.75	0.000	-10.280	-9.532	-10.280
6	15AY	59.39	10.728	0.000	10.366	10.728
6	15BP	9.80	1.771	0.000	0.845	1.771
6	15BX	21.77	0.000	-2.538	-1.310	-2.538
6	15BXY	10.34	0.012	-1.205	-1.205	0.012
6	15BY	3.75	0.678	0.000	0.678	0.358

7	16AP	63.92	11.586	0.000	8.599	11.586
7	16AX	85.43	0.000	-12.218	-9.097	-12.218
7	16AXY	74.02	0.000	-10.586	-10.586	-10.586
7	16AY	58.81	10.659	0.000	9.614	10.659
7	16BP	6.45	0.000	-0.858	-0.858	-0.858
7	16BX	3.03	0.550	0.000	0.550	0.014
7	16BXY	11.81	2.141	0.000	0.718	2.141
7	16BY	13.39	0.000	-1.781	-1.005	-1.781
8	17AP	49.96	22.516	0.000	22.516	19.014
8	17AX	57.37	0.000	-22.726	-22.726	-19.894
8	17AXY	53.38	0.000	-21.144	-21.144	-17.714
8	17AY	47.25	21.292	0.000	21.292	18.628
8	17BP	5.29	0.000	-1.548	-1.067	-1.548
8	17BX	2.02	0.908	-0.356	0.908	-0.356
8	17BXY	6.62	2.986	0.000	0.388	2.986
8	17BY	3.92	0.000	-1.146	-0.883	-1.146
9	18AP	49.25	17.928	0.000	17.928	16.299
9	18AX	55.10	0.000	-18.040	-18.040	-17.276
9	18AXY	60.32	0.000	-19.747	-19.747	-15.932
9	18AY	54.98	20.012	0.000	20.012	17.005
9	18BP	2.59	0.000	-0.614	-0.614	-0.167
9	18BX	5.11	0.278	-1.672	0.278	-1.672
9	18BXY	3.95	1.438	-0.105	-0.105	1.438
9	18BY	1.16	0.155	-0.275	-0.275	0.155
10	19AP	54.24	32.782	0.000	32.782	27.815
10	19AX	54.28	0.000	-33.224	-33.224	-28.829
10	19AXY	51.75	0.000	-31.671	-31.671	-25.659
10	19AY	51.74	31.272	0.000	31.272	26.401
10	19BP	5.71	3.451	0.000	3.451	2.243
10	19BX	9.54	0.000	-4.693	-2.737	-4.693
10	19BXY	6.53	0.000	-3.215	-3.215	-0.491
10	19BY	6.05	3.656	0.000	3.656	3.635
10	20AP	51.43	31.085	0.000	31.085	26.896
10	20AX	51.91	0.000	-31.768	-31.768	-28.990
10	20AXY	53.52	0.000	-32.759	-32.759	-25.625
10	20AY	52.60	31.794	0.000	31.794	26.914
10	20BP	16.12	0.000	-7.936	-7.936	-7.485
10	20BX	12.06	7.289	0.000	7.289	4.431
10	20EXY	14.55	8.795	0.000	6.854	8.795
10	20BY	15.70	0.000	-7.730	-7.730	-6.198
11	21AP	3.22	1.478	0.000	1.478	0.150
11	21AX	6.93	0.000	-3.072	-3.072	-1.665
11	21AXY	5.77	1.926	-2.446	-2.446	1.926
11	21AY	3.71	0.026	-1.645	0.026	-1.645
11	21BP	26.18	0.000	-11.095	-11.095	-9.819
11	21BX	23.65	10.869	0.000	10.869	9.147
11	21EXY	26.16	12.023	0.000	12.023	11.308
11	21BY	25.25	0.000	-10.697	-10.697	-9.976
12	22AP	2.27	0.827	0.000	0.827	0.457
12	22AX	3.94	0.000	-1.220	-1.220	-1.043
12	22AXY	1.51	0.348	-0.469	-0.469	0.348
12	22AY	5.79	0.062	-1.793	0.062	-1.793
12	22BP	10.42	3.793	0.000	3.793	3.775
12	22BX	16.00	0.000	-4.959	-4.959	-4.718
12	22EXY	13.39	0.000	-4.148	-4.148	-3.322
12	22BY	11.18	4.070	0.000	4.070	3.581
13	23AP	6.92	1.890	0.000	0.727	1.890
13	23AX	6.03	0.000	-1.359	-1.359	-0.921
13	23AXY	2.78	0.475	-0.627	-0.627	0.475
13	23AY	8.89	2.427	0.000	0.811	2.427

13	23BP	4.31	0.000	-0.971	-0.971	-0.847
13	23BX	4.22	1.152	0.000	1.152	0.686
13	23BY	6.52	1.779	0.000	1.779	1.753
13	23BY	3.86	0.000	-0.870	-0.542	-0.870
14	24AP	2.95	0.805	0.000	0.805	0.659
14	24AX	14.96	0.000	-3.206	-1.518	-3.206
14	24AXY	18.11	0.000	-3.882	-1.915	-3.882
14	24AY	2.67	0.000	-0.572	-0.189	-0.572
14	24BP	5.71	0.000	-1.223	-1.223	-0.544
14	24BX	4.26	0.000	-0.914	-0.914	-0.854
14	24BY	1.80	0.056	-0.386	-0.386	0.056
14	24BY	4.79	0.000	-1.026	-1.026	-0.866
15	25AP	44.96	16.696	0.000	1.134	16.696
15	25AX	26.66	9.899	-1.975	-1.975	9.899
15	25AXY	57.87	21.490	0.000	6.726	21.490
15	25AY	78.17	29.029	0.000	10.026	29.029
15	25BP	8.93	0.920	-3.020	0.920	-3.020
15	25BX	59.48	22.088	0.000	22.088	18.192
15	25BY	57.78	21.455	0.000	21.455	17.172
15	25BY	11.60	0.000	-3.919	-0.179	-3.919
16	26AP	18.44	3.341	0.000	3.341	3.103
16	26AX	28.74	5.208	0.000	4.050	5.208
16	26AXY	15.81	2.866	0.000	2.866	2.826
16	26AY	7.59	0.000	-0.307	-0.307	0.000
16	26BP	12.72	2.305	0.000	0.039	2.305
16	26BX	41.26	7.477	0.000	6.556	7.477
16	26BY	37.59	6.813	0.000	6.259	6.813
16	26BY	34.28	6.213	0.000	0.479	6.213
17	27AP	0.00	0.000	0.000	0.000	0.000
17	27AX	27.27	6.786	0.000	3.973	6.786
17	27AXY	65.57	16.318	0.000	8.157	16.318
17	27AY	0.00	0.000	0.000	0.000	0.000
17	27BP	0.00	0.000	0.000	0.000	0.000
17	27BX	22.54	5.609	0.000	5.609	1.205
17	27BY	27.41	6.822	0.000	6.822	5.243
17	27BY	0.00	0.000	0.000	0.000	0.000
18	28AP	35.99	16.373	0.000	0.774	16.373
18	28AX	45.89	0.000	-5.272	-5.272	0.000
18	28AXY	0.00	0.000	0.000	0.000	0.000
18	28AY	63.43	28.862	0.000	6.892	28.862
18	28BP	0.00	0.000	0.000	0.000	0.000
18	28BX	26.43	12.025	0.000	12.025	9.658
18	28BY	31.42	14.294	0.000	13.894	14.294
18	28BY	55.12	0.000	-6.332	0.000	-6.332
19	29AP	11.35	2.394	-2.911	-2.911	2.394
19	29AX	50.34	22.640	0.000	4.559	22.640
19	29AXY	76.25	34.293	0.000	10.242	34.293
19	29AY	3.28	0.000	-0.841	-0.671	-0.841
19	29BP	3.06	1.376	-0.371	-0.371	1.376
19	29BX	43.87	19.733	0.000	19.733	17.549
19	29BY	48.90	21.995	0.000	21.372	21.995
19	29BY	6.33	0.013	-1.625	0.013	-1.625
20a	30AP	19.51	0.000	-1.776	-1.776	-0.870
20a	30AY	18.84	0.000	-1.715	-1.715	-0.165
20a	30BP	7.89	0.554	0.000	0.554	0.323
20a	30BX	3.33	0.234	-0.043	0.234	-0.043
20	31P	14.26	1.002	0.000	1.002	0.883
20	31Y	38.25	2.687	0.000	2.687	1.111
20	32P	16.03	1.191	0.000	1.191	1.168
20	32X	16.30	1.212	0.000	1.212	0.175

20	33P	20.87	1.466	0.000	1.466	1.137
20	33Y	51.30	3.603	0.000	3.603	1.401
20	34P	2.00	0.148	0.000	0.082	0.148
20	34X	24.18	1.797	0.000	1.797	0.758
20	35P	17.00	1.194	0.000	1.062	1.194
20	35Y	36.79	2.584	0.000	2.584	0.606
36	36P	67.24	0.000	-11.296	-11.296	-10.067
36	36X	84.81	12.606	0.000	12.606	10.882
21	37AP	27.63	0.145	-7.542	0.145	-7.542
21	37AY	53.92	0.000	-14.720	-5.032	-14.720
21	37BP	14.10	3.850	0.000	1.537	3.850
21	37BX	53.28	0.000	-14.544	-14.544	-11.858
22	38AP	36.07	0.000	-4.149	-3.172	-4.149
22	38AX	22.00	0.000	-2.537	-2.537	-1.962
22	38AY	5.65	1.024	-0.467	-0.467	1.024
22	38BP	24.38	0.000	-2.804	-2.456	-2.804
22	38BX	21.82	0.000	-2.510	-1.192	-2.510
22	38BY	38.58	0.000	-4.438	-3.899	-4.438
22	38BX	35.22	0.000	-4.051	-3.748	-4.051
22	38BY	38.32	0.000	-4.408	-1.376	-4.408
23	39AP	19.80	0.000	-3.604	-3.100	-3.604
23	39AX	4.50	0.819	-0.274	-0.274	0.819
23	39AY	12.81	2.332	0.000	0.131	2.332
23	39BP	48.03	0.000	-8.742	-5.299	-8.742
23	39BX	2.52	0.459	0.000	0.284	0.459
23	39BY	16.81	0.000	-3.060	-3.060	-0.600
23	39BX	20.00	0.000	-3.640	-3.640	-2.538
23	39BY	1.14	0.208	-0.109	-0.109	0.208
23	39C1P	29.97	5.455	0.000	0.567	5.455
23	39C1Y	18.45	3.358	-0.355	-0.355	3.358
23	F39C197P	26.10	4.750	0.000	0.533	4.750
23	F39C197Y	14.55	2.647	-0.378	-0.378	2.647
23	39C2P	15.45	2.812	0.000	0.074	2.812
23	39C2X	47.84	8.707	0.000	7.118	8.707
23	F39C2118P	15.58	2.836	0.000	0.074	2.836
23	F39C2118X	47.94	8.726	0.000	7.119	8.726
24	40P	3.22	0.586	-0.232	0.586	-0.232
24	40X	5.42	0.000	-0.726	-0.726	-0.005
24	40XY	28.24	0.000	-3.780	-1.682	-3.780
24	40Y	21.85	3.977	0.000	1.376	3.977
24	41P	5.43	0.989	-0.080	-0.080	0.989
24	41X	7.39	0.000	-0.990	-0.142	-0.990
24	41XY	2.31	0.000	-0.310	-0.076	-0.310
24	41Y	0.65	0.007	-0.087	-0.087	0.007
24	Angler	26.11	0.000	-2.376	-0.766	-2.376
24	42P	37.80	3.440	0.000	0.743	3.440
24	42X	34.46	0.000	-3.136	-0.481	-3.136
24	42XY	33.55	3.053	0.000	1.055	3.053
24	42Y	11.57	1.523	0.000	0.630	1.523
26	43AP	5.86	0.000	-0.738	-0.217	-0.738
26	43AX	8.77	1.155	0.000	0.721	1.155
26	43AY	7.08	0.000	-0.891	-0.186	-0.891
26	43BP	1.49	0.000	-0.188	-0.141	-0.188
26	43BX	3.52	0.463	0.000	0.336	0.463
26	43BY	4.06	0.000	-0.511	-0.259	-0.511
26	43BY	3.98	0.524	0.000	0.229	0.524
26	43CP	6.98	0.000	-0.879	-0.879	-0.208
26	43CX	3.61	0.475	0.000	0.475	0.097
26	43CXY	9.21	0.000	-1.160	-1.160	-0.519
26	43CY	2.63	0.347	0.000	0.347	0.255

27	44P	8.61	0.000	-1.437	-1.161	-1.437
27	44Y	7.15	0.000	-1.193	-1.193	-0.400
27	44AP	6.56	1.188	0.000	1.188	0.378
27	44AY	7.97	1.444	0.000	1.165	1.444
27	44BP	4.87	0.000	-0.707	-0.255	-0.707
27	44BY	2.57	0.465	-0.284	-0.284	0.465
27	44CP	3.05	0.317	-0.443	0.317	-0.443
27	44CY	4.07	0.738	0.000	0.292	0.738
27	44DP	3.24	0.000	-0.540	-0.166	-0.540
27	44DY	2.83	0.514	-0.195	-0.195	0.514
27	44EP	2.98	0.224	-0.497	0.224	-0.497
27	44EY	3.14	0.569	0.000	0.198	0.569
28	45P	37.58	6.839	0.000	6.839	4.142
28	45AP	37.32	6.793	0.000	6.793	4.071
28	45BP	33.75	6.142	0.000	6.142	3.510
29	46P	23.78	0.000	-1.840	-1.840	-0.762
29	46X	25.73	0.000	-1.991	-1.991	-1.121
29	46XY	24.53	0.000	-1.897	-1.897	-0.377
29	46Y	25.75	0.000	-1.992	-1.992	-0.712
30	47P	3.31	0.449	0.000	0.326	0.449
30	47Y	31.56	4.286	0.000	4.286	2.179
30	47AP	14.40	2.190	0.000	2.190	1.287
30	47AY	16.00	2.433	0.000	2.433	0.889
31	48P	3.50	0.635	0.000	0.573	0.635
31	48Y	31.65	5.737	0.000	5.737	2.883
31	48AP	20.65	3.056	0.000	3.056	0.935
31	48AY	22.09	3.270	0.000	3.270	1.964
30	49P	3.58	0.500	0.000	0.379	0.500
30	49Y	31.08	4.347	0.000	4.347	2.217
30	49AP	16.56	2.451	0.000	2.451	0.875
30	49AY	15.88	2.350	0.000	2.350	1.414
32	50P	37.17	6.737	0.000	6.737	2.986
32	50X	16.84	0.000	-3.065	-3.065	-2.854
32	50XY	17.67	0.000	-3.216	-3.216	-1.905
32	50Y	37.66	6.825	0.000	6.825	4.604
32	50AP	6.65	1.210	0.000	1.210	0.478
32	50AY	7.00	1.274	0.000	1.274	1.196
33	51P	28.71	0.000	-9.378	-9.378	-7.588
33	51Y	29.59	0.000	-9.664	-9.664	-4.225
33	51AP	3.61	0.000	-1.155	-1.102	-1.155
33	51AY	8.97	0.071	-2.868	-2.868	0.071
33	51BP	26.23	7.162	0.000	7.162	4.246
33	51BY	14.47	3.950	0.000	3.663	3.950
33	51CP	18.89	6.874	0.000	6.874	4.671
33	51CY	9.37	3.412	0.000	3.412	2.926
34	52P	24.56	0.000	-10.222	-10.222	-7.600
34	52Y	25.24	0.000	-10.506	-10.506	-4.898
34	52AP	3.23	0.000	-1.465	-1.465	-1.349
34	52AY	8.75	0.000	-3.966	-3.966	-0.719
34	52BP	15.38	6.996	0.000	6.996	3.885
34	52BY	7.65	3.483	0.000	2.124	3.483
34	52CP	13.62	6.542	0.000	6.542	4.304
34	52CY	4.43	2.128	0.000	1.710	2.128
33	53P	29.39	0.000	-9.599	-9.599	-7.230
33	53Y	29.34	0.000	-9.582	-9.582	-4.672
33	53AP	5.28	0.000	-1.688	-1.196	-1.688
33	53AY	9.80	0.000	-3.134	-3.134	-0.176
33	53BP	28.56	7.797	0.000	7.797	4.826
33	53BY	16.57	4.523	0.000	4.281	4.523
33	53CP	16.94	6.167	0.000	6.167	4.029

33	53CY	7.39	2.689	0.000	2.689	2.288
35	54P	20.02	0.189	-1.633	-1.633	0.189
35	54X	7.70	0.701	-0.328	0.701	-0.328
	g99P	7.56	0.000	-33.064	-33.064	-12.411
	g100P	5.20	0.000	-27.894	-27.894	-10.833
	g101P	8.70	0.000	-22.636	-22.636	-9.524
	g102P	3.27	0.000	-17.669	-17.669	-7.610
	g103P	2.84	0.000	-15.347	-15.347	-6.605
	g104P	2.43	0.000	-12.893	-12.893	-5.623
	g105P	2.01	0.000	-11.354	-11.354	-4.759
	g106P	4.84	0.421	-0.813	0.421	-0.813
	g107P	3.67	0.088	-0.374	0.088	-0.374
	g108P	1.80	0.000	-0.183	-0.157	-0.183
	g109P	2.80	0.000	-0.285	-0.285	-0.107
	g110P	44.87	6.100	0.000	2.743	6.100
	g110X	58.05	0.000	-7.891	-1.753	-7.891
	g111P	32.68	0.000	-3.332	-0.507	-3.332
	g111X	24.43	2.491	0.000	0.681	2.491
	g112P	13.21	1.347	0.000	0.561	1.347
	g112X	14.17	0.000	-1.444	-0.825	-1.444
	g113P	5.75	0.586	0.000	0.398	0.586
	g114P	13.65	0.000	-1.392	-1.216	-1.392
	g115P	2.24	0.228	0.000	0.083	0.228
	g116P	1.02	0.138	-0.135	0.138	-0.135
	g116X	13.43	1.826	0.000	0.839	1.826
	g117P	15.25	0.000	-2.073	-1.493	-2.073
	g118P	2.34	0.000	-0.318	-0.013	-0.318
	g119P	0.45	0.010	-0.061	0.010	-0.061
	g120P	8.23	0.000	-1.119	-0.562	-1.119
	g120X	8.07	1.097	-0.005	-0.005	1.097
	g121P	1.49	0.000	-8.298	-8.298	-3.588

Equilibrium Joint Positions and Rotations for Load Case "NESC Heavy":

Joint Label	X-Displ (ft)	Y-Displ (ft)	Z-Displ (ft)	X-Rot (deg)	Y-Rot (deg)	Z-Rot (deg)	X-Pos (ft)	Y-Pos (ft)	Z-Pos (ft)
1P	-0.01286	0.7814	-0.05445	-0.8400	-0.0403	-0.0632	2.987	3.781	128.7
2P	-0.01052	0.7132	-0.05297	-0.9014	-0.0394	-0.0687	2.989	3.713	124.2
3P	-0.004911	0.6417	-0.05136	-0.8914	-0.0322	-0.0689	2.995	3.642	119.7
4P	-0.005221	0.5404	-0.04691	-0.7828	-0.0763	-0.0682	2.995	3.54	112.8
5P	0.00269	0.4333	-0.04229	-0.7340	0.0163	-0.0694	3.003	3.433	104.8
6P	0.0003198	0.3608	-0.03706	-0.6317	-0.0675	-0.0687	3	3.361	98.71
7P	0.008339	0.2862	-0.03275	-0.5851	-0.0044	-0.0710	3.008	3.286	91.72
8P	0.004541	0.2217	-0.02725	-0.4427	-0.0235	-0.0729	3.005	3.222	84.72
15P	0	0	0	0.0000	0.0000	0.0000	15.44	15.44	0
16P	0.004299	0.7857	-0.2739	-0.9367	-0.0379	-0.0689	0.004299	17.54	128.5
17P	0.008909	0.5409	-0.2111	-0.8149	-0.0766	-0.0893	3.009	14.79	112.6
18P	0.01647	0.3614	-0.2037	-0.6652	-0.0678	-0.0763	3.016	17.11	98.55
19P	0.01878	0.223	-0.1337	-0.5411	-0.0233	-0.0696	3.019	14.47	84.62
20AP	0.01479	0.03104	-0.001743	-0.0294	-0.0188	-0.0987	9.583	0.03104	40
20BP	0.02757	0.04399	-0.02979	-0.2162	-0.0298	-0.0929	0.02757	9.612	39.97
21P	-0.175	-0.007307	-0.03228	0.0000	0.0000	0.0000	11.6	2.201	24.97
22P	-0.008963	0.1557	0.01735	0.0000	0.0000	0.0000	2.199	11.93	25.02
23P	-0.02845	0.5488	0.1876	-0.8012	-0.0246	-0.0798	-0.02845	-13.7	113
24P	-0.02533	0.3687	0.1792	-0.6498	-0.0211	-0.0763	-0.02533	-16.38	98.93
25P	-0.01661	0.2291	0.114	-0.5295	-0.0189	-0.0761	-0.01661	-14.02	84.86
26P	0.0008233	0.5406	-0.1298	-0.8547	-0.0762	-0.0672	3.001	9.166	112.7
27P	0.00771	0.3611	-0.1218	-0.7170	-0.0674	-0.0634	3.008	10.24	98.63
28P	0.01182	0.2225	-0.07973	-0.5671	-0.0233	-0.0728	3.012	8.847	84.67
29P	0	0	0	0.0000	0.0000	0.0000	1.5	0	-4.25
30P	0.01251	0.0384	-0.00248	-0.0439	0.0169	-0.0345	1.513	0.0384	25
31P	0.01478	0.04233	-0.003542	-0.0294	0.0006	-0.0432	1.515	0.04233	40
32P	0.001559	0.2235	-0.006479	-0.4787	-0.0222	-0.0535	1.502	0.2235	84.74
33P	-0.003525	0.363	-0.007802	-0.6620	-0.0223	-0.0581	1.496	0.363	98.74
34P	-0.009185	0.5419	-0.009491	-0.7794	-0.0257	-0.0613	1.491	0.5419	112.7
35P	-0.01642	0.7829	-0.01183	-1.0030	-0.0281	-0.0627	1.484	0.7829	128.7
36P	-0.01926	0.8943	-0.01304	-1.1071	-0.0284	-0.0628	1.481	0.8943	134.7
37P	-0.02355	1.073	-0.015	-1.1505	-0.0286	-0.0628	1.476	1.073	143.7
1X	-0.02026	0.7819	0.03497	-0.8415	-0.0356	-0.0761	2.98	-2.218	128.8
1XY	-0.02043	0.7892	0.0319	-0.8457	-0.0094	-0.0736	-3.02	-2.211	128.8
2X	-0.01244	0.7887	-0.05717	-0.8575	-0.0310	-0.0754	-3.012	3.789	128.7
2XY	-0.0182	0.7145	0.03454	-0.9004	0.0076	-0.0770	2.982	-2.286	124.3
2Y	-0.01827	0.7219	0.03159	-0.8909	-0.0672	-0.0717	-3.018	-2.278	124.3
3X	-0.0107	0.7207	-0.05578	-0.8832	0.0101	-0.0771	-3.011	3.721	124.2
3Y	-0.0197	0.642	0.03424	-0.8996	-0.0238	-0.0767	2.98	-2.358	119.8
3XY	-0.01257	0.6503	0.03141	-0.8926	-0.0291	-0.0713	-3.013	-2.35	119.8
4X	-0.01225	0.6506	-0.05425	-0.8848	-0.0233	-0.0775	-3.012	3.651	119.7
4Y	-0.01293	0.541	0.03167	-0.7849	0.0233	-0.0773	2.987	-2.459	112.8
4XY	-0.01333	0.5491	0.02904	-0.7860	-0.0751	-0.0694	-3.013	-2.451	112.8
5X	-0.005613	0.5482	-0.04962	-0.7831	0.0238	-0.0795	-3.006	3.548	112.8
5Y	-0.01448	0.4332	0.02879	-0.7311	-0.0640	-0.0758	2.986	-2.567	104.8
5XY	-0.005118	0.4411	0.02633	-0.7324	0.0158	-0.0676	-3.005	-2.559	104.8
6X	-0.006855	0.4419	-0.04491	-0.7397	-0.0643	-0.0756	-3.007	3.442	104.8
6Y	-0.007137	0.3613	0.02478	-0.6322	0.0208	-0.0761	-2.993	-2.639	98.77
6XY	-0.007731	0.369	0.02248	-0.6347	-0.0652	-0.0647	-3.008	-2.631	98.77
6Y	0.0002975	0.3682	-0.03948	-0.6317	0.0210	-0.0743	-3	3.368	98.71

Joint	X	X	Y	Y H-Shear	Z Comp.	Uplift Result.	Result.	X	X-M.	Y	Y-M. H-Bend-M	Z	Z-M.	Max.
7X	-0.01003	0.2861	0.02151	-0.5856	-0.0362	-0.0769	2.99	-2.714	91.77					
7XY	0.0003277	0.2936	0.01931	-0.5899	-0.0076	-0.0581	-3	-2.706	91.77					
7Y	-0.002417	0.2942	-0.03508	-0.5857	-0.0394	-0.0680	-3.002	3.294	91.71					
8X	-0.001342	0.2221	0.01689	-0.4399	-0.0213	-0.0780	2.999	-2.778	84.77					
8XY	-0.00344	0.2296	0.01482	-0.4397	-0.0158	-0.0512	-3.003	-2.777	84.76					
8Y	0.00643	0.2291	-0.02944	-0.4438	-0.0186	-0.0620	-2.994	3.229	84.72					
15X	0	0	0	0.0000	0.0000	0.0000	15.44	-15.44	0					
15XY	0	0	0	0.0000	0.0000	0.0000	-15.44	15.44	0					
15Y	0	0	0	0.0000	0.0000	0.0000	-15.44	15.44	0					
16X	-0.03732	0.7882	0.238	-0.8588	-0.0228	-0.0681	-0.03732	-15.96	129					
17Y	0.008084	0.5478	-0.2214	-0.8713	0.0245	-0.0466	-2.992	14.8	112.6					
18Y	0.01565	0.3675	-0.2177	-0.7365	0.0215	-0.0463	-2.984	17.12	98.53					
19Y	0.01804	0.2293	-0.1428	-0.5942	-0.0183	-0.0551	-2.982	14.48	84.61					
20AY	0.01476	0.05631	-0.01073	-0.0294	-0.0554	-0.0870	-9.553	0.05631	39.99					
20BX	0.001944	0.0442	0.01195	-0.1223	0.0140	-0.0934	0.001944	-9.524	40.01					
21X	0.1353	0.005169	0.02003	0.0000	0.0000	0.0000	11.91	-2.203	25.02					
21XY	-0.01322	0.02557	-0.00424	0.0000	0.0000	0.0000	-11.78	2.216	24.99					
21Y	0.08732	0.007923	-0.009262	0.0000	0.0000	0.0000	-11.68	2.216	24.99					
22X	0.00217	0.1403	-0.005643	0.0000	0.0000	0.0000	2.21	-11.63	24.99					
22XY	0.002751	0.04161	0.007777	-0.0448	0.0172	-0.0543	-2.205	-11.73	25.01					
22Y	0.04611	0.04892	-0.01731	-0.0896	0.0024	-0.2279	-2.162	11.82	24.98					
26Y	0.002662	0.548	-0.135	-0.8984	0.0241	-0.0736	-2.997	9.173	112.7					
27Y	0.009339	0.3679	-0.1279	-0.7719	0.0213	-0.0663	-2.991	10.24	98.62					
28Y	0.01248	0.2293	-0.08407	-0.6063	-0.0183	-0.0592	-2.988	8.854	84.67					
9S	0.01475	0.1701	-0.0275	-0.3167	-0.0391	-0.0557	4.152	4.308	76.97					
10S	0.01387	0.1291	-0.02746	-0.2597	-0.0147	-0.0434	5.289	5.404	69.22					
11S	0.0208	0.09662	-0.02509	-0.1990	-0.0363	-0.0370	6.434	6.509	61.47					
12S	0.02232	0.06432	-0.02298	-0.1606	-0.0288	-0.0297	7.891	7.933	51.56					
13S	0.0272	0.03025	-0.01938	-0.1682	0.0563	-0.0180	9.596	9.599	39.98					
14S	-0.0104	-0.008784	-0.02018	-0.0671	0.0935	-0.0028	11.76	11.76	24.98					
10.50E129S	-0.01647	0.7851	-0.01309	-0.8495	-0.0243	-0.0707	-0.01647	0.7851	128.8					
10.50E113S	-0.009208	0.5447	-0.01006	-0.7732	-0.0225	-0.0814	-0.009208	0.5447	112.8					
10.50E99S	-0.003512	0.3647	-0.008132	-0.6164	-0.0226	-0.0716	-0.003512	0.3647	98.74					
10.50E85S	0.001581	0.2246	-0.006944	-0.4284	-0.0232	-0.0615	0.001581	0.2246	84.74					
21XF0.50S	0.01233	0.006912	-0.04866	-0.0438	0.3781	-0.2978	11.78	0.006912	24.95					
22PF0.50S	0.0213	0.1024	-0.343	0.0000	0.0000	0.0000	0.0213	11.87	24.66					
9X	-0.066133	0.1701	0.01867	-0.3178	0.0083	-0.0631	4.131	-3.967	77.02					
9XY	0.003203	0.1805	0.016	-0.3098	-0.0386	-0.0544	-4.134	-3.957	77.02					
9Y	0.004224	0.181	-0.03024	-0.3103	0.0020	-0.0667	-4.133	4.319	76.97					
10X	-0.001656	0.1292	0.01376	-0.2587	-0.0283	-0.0471	5.274	-5.146	69.27					
10XY	0.003653	0.1432	0.01642	-0.2526	0.0009	-0.0568	-5.275	-5.132	69.27					
10Y	0.01293	0.1428	-0.03086	-0.2503	-0.0276	-0.0661	-5.262	5.418	69.22					
11X	-0.002135	0.09665	0.01897	-0.1884	0.0079	-0.0420	6.411	-6.316	61.52					
11XY	0.002308	0.1131	0.01516	-0.1885	-0.0320	-0.0492	-6.41	-6.3	61.52					
11Y	0.01363	0.114	-0.02899	-0.1929	-0.0035	-0.0583	-6.399	6.527	61.47					
12X	-0.001213	0.06435	0.01787	-0.1637	-0.0123	-0.0320	7.868	-7.804	51.6					
12XY	0.003163	0.08599	0.01312	-0.1466	-0.0096	-0.0455	-7.866	-7.783	51.59					
12Y	0.02187	0.08442	-0.02786	-0.1471	-0.0270	-0.0543	-7.847	7.953	51.55					
13X	0.0007541	0.03203	0.01656	-0.1296	-0.0050	-0.0259	9.569	-9.537	40.02					
13XY	0.003086	0.05709	0.01142	-0.1353	-0.0104	-0.0317	-9.566	-9.512	40.01					
13Y	0.028	0.05615	-0.02471	-0.1701	-0.0520	-0.0382	-9.541	9.625	39.98					
14X	0.000693	0.00664	0.01254	-0.0495	0.0059	-0.0129	11.77	-11.76	25.01					
14XY	0.003382	0.02649	0.008579	-0.0994	-0.0037	-0.0180	-11.77	-11.74	25.01					
14Y	0.04621	0.007845	-0.02371	-0.1249	0.0229	-0.0128	-11.72	11.78	24.98					
21XF0.50Y	0.01393	0.01706	-0.1958	-0.0446	-1.2602	0.1544	-11.76	0.01706	24.8					
22PF0.50X	0.002338	0.09594	-0.002493	0.0000	0.0000	0.0000	0.002338	-11.67	25					

Joint Support Reactions for Load Case "NESC Heavy":

Joint X X Y Y H-Shear Z Comp. Uplift Result. Result. X X-M. Y Y-M. H-Bend-M Z Z-M. Max.

Centek Engineering Inc - CL&P # 1281

Label	Force Usage (kips)	Force Usage %	Force Usage (kips)	Force Usage %	Force Usage (kips)	Force Usage %	Force (kips)	Usage Moment (ft-k)	Usage Moment %	Usage (ft-k)	Usage %	Usage Moment (ft-k)	Usage %
15P	-32.08	0.0	-32.98	0.0	0.0	-220.23	0.0	224.99	0.0	-2.78	0.0	3.5	0.0
29P	-0.19	0.0	-1.19	0.0	0.0	-34.21	0.0	34.23	0.0	11.40	0.0	-3.5	0.0
15X	34.21	0.0	-29.68	0.0	0.0	187.35	0.0	192.75	0.0	-0.36	0.0	-0.2	0.0
15XY	-34.98	0.0	-31.90	0.0	0.0	188.28	0.0	194.14	0.0	1.41	0.0	-0.2	0.0
15Y	33.04	0.0	-33.01	0.0	0.0	-226.21	0.0	230.99	0.0	-1.71	0.0	-6.0	0.0

Joint Displacements, Loads and Member Forces on Joints for Load Case "NESC Heavy":

Joint X Label	External Y Load (kips)	External Y Load (kips)	External Z Load (kips)	External Z Load (kips)	Member X Force (kips)	Member Y Force (kips)	Member Z Force (kips)	Force (kips)	Disp. (ft)	Y Disp. (ft)	Z Disp. (ft)
1P	0.0000	0.0000	-0.1848	0.0000	-0.0000	0.0000	0.0000	0.1848	-0.0129	0.7814	-0.0545
2P	0.0000	0.0000	-0.0969	-0.0000	0.0000	0.0000	0.0000	0.0969	-0.0105	0.7132	-0.0530
3P	0.0000	0.0000	-0.1835	-0.0000	-0.0000	0.0000	0.0000	0.1835	-0.0049	0.6417	-0.0514
4P	0.0000	0.0000	-0.3355	-0.0000	-0.0000	0.0000	0.0000	0.3355	-0.0052	0.5404	-0.0469
5P	0.0000	0.0000	-0.3493	0.0000	0.0000	0.0000	0.0000	0.3493	0.0027	0.4333	-0.0423
6P	0.0000	0.0000	-0.4956	-0.0000	-0.0000	0.0000	0.0000	0.4956	0.0003	0.3608	-0.0371
7P	0.0000	0.0000	-0.5534	-0.0000	-0.0000	0.0000	0.0000	0.5534	0.0083	0.2862	-0.0327
8P	0.0000	0.0000	-0.6814	-0.0000	0.0000	0.0000	0.0000	0.6814	0.0045	0.2217	-0.0273
15P	0.0000	0.0000	-1.0248	32.0798	32.9790	-219.2062	0.0000	0.0000	0.0000	0.0000	0.0000
16P	0.0000	9.6660	-1.3281	0.0000	-9.6660	1.3281	0.0043	0.7857	-0.0043	0.7857	-0.2739
17P	7.0000	7.2760	-0.2488	-7.0000	-7.2760	0.2488	0.0089	0.5409	-0.0089	0.5409	-0.2111
18P	7.0000	7.2760	-0.2778	-7.0000	-7.2760	0.2778	0.0165	0.3614	-0.0165	0.3614	-0.2037
19P	7.0000	7.2760	-0.2488	-7.0000	-7.2760	0.2488	0.0188	0.2230	-0.0188	0.2230	-0.1337
20AP	0.0000	0.0000	-0.6136	0.0000	-0.0000	0.0000	0.0000	0.6136	0.0148	0.0310	-0.0017
20BP	0.0000	0.0000	-0.6343	-0.0000	-0.0000	0.0000	0.0000	0.6343	0.0276	0.0440	-0.0298
21P	0.0000	0.0000	-0.3466	-0.0000	-0.0000	0.0000	0.0000	0.3466	-0.1750	-0.0073	-0.0323
22P	0.0000	0.0000	-0.3466	-0.0000	-0.0000	0.0000	0.0000	0.3466	-0.0090	0.1557	0.0174
23P	0.0000	14.5365	-2.5839	-0.0000	-0.0000	-14.5365	2.5839	0.0284	0.5488	0.1876	0.1792
24P	0.0000	14.5441	-2.6618	0.0000	-14.5441	2.6618	-0.0253	0.3687	0.0253	0.3687	0.1792
25P	0.0000	14.5365	-2.5839	0.0000	-0.0000	-14.5365	2.5839	-0.0166	0.2291	0.1140	0.1140
26P	0.0000	0.0000	-0.0835	0.0000	-0.0000	0.0000	0.0000	0.0835	0.0008	0.5406	-0.1298
27P	0.0000	0.0000	-0.1110	0.0000	-0.0000	0.0000	0.0000	0.1110	0.0077	0.3611	-0.1218
28P	0.0000	0.0000	-0.0835	0.0000	-0.0000	0.0000	0.0000	0.0835	0.0118	0.2225	-0.0797
29P	0.0000	0.5439	-1.1425	0.1916	-0.6416	0.6416	0.0000	0.0000	0.0000	0.0000	0.0000
30P	0.0000	1.2098	-4.9616	0.0000	-1.2098	4.9616	0.0125	0.0384	-0.0125	0.0384	-0.0025
31P	0.0000	1.4660	-5.2444	-0.0000	-1.4660	5.2444	0.0148	0.0423	-0.0148	0.0423	-0.0035
32P	0.0000	1.4302	-5.0159	0.0000	-1.4302	5.0159	0.0016	0.2335	-0.0016	0.2335	-0.0065
33P	0.0000	0.6653	-2.4008	-0.0000	-0.0000	-0.6653	2.4008	-0.0035	0.3630	-0.0035	0.0078
34P	0.0000	0.7213	-2.5709	-0.0000	-0.0000	-0.7213	2.5709	-0.0092	0.5419	-0.0092	0.0095
35P	0.0000	0.5043	-1.6351	0.0000	-0.5043	1.6351	-0.0164	0.7829	-0.0164	0.7829	-0.0118
36P	0.0000	1.3280	-3.0799	-0.0000	-1.3280	3.0799	-0.0193	0.8943	-0.0193	0.8943	-0.0130
37P	0.0000	0.8232	-8.3155	-0.0000	-0.8232	8.3155	-0.0236	1.0728	-0.0236	1.0728	-0.0150
1X	0.0000	0.0775	-0.1848	-0.0000	-0.0000	0.1848	-0.0203	0.7819	-0.0203	0.7819	0.0350
1XY	0.0000	0.0723	-0.1740	-0.0000	-0.0000	0.1740	-0.0204	0.7892	-0.0204	0.7892	0.0319
1Y	0.0000	0.0000	-0.1740	0.0000	0.0000	0.1740	-0.0124	0.7887	-0.0124	0.7887	-0.0572
2X	0.0000	0.0813	-0.0969	-0.0000	-0.0000	0.0969	-0.0182	0.7145	-0.0182	0.7145	0.0345
2XY	0.0000	0.0813	-0.0969	-0.0000	-0.0000	0.0969	-0.0183	0.7219	-0.0183	0.7219	0.0316
2Y	0.0000	0.1760	-0.5679	-0.0000	-0.0000	0.5679	-0.0107	0.7207	-0.0107	0.7207	-0.0558
3X	0.0000	0.1527	-0.1842	-0.0000	-0.0000	0.1842	-0.0197	0.6420	-0.0197	0.6420	0.0342
3XY	0.0000	0.1527	-0.1842	-0.0000	-0.0000	0.1842	-0.0126	0.6503	-0.0126	0.6503	0.0314
3Y	0.0000	0.0000	-0.1835	-0.0000	-0.0000	0.1835	-0.0122	0.6506	-0.0122	0.6506	-0.0543
4X	0.0000	0.2091	-0.3433	-0.0000	-0.0000	0.3433	-0.0129	0.5410	-0.0129	0.5410	0.0317
4XY	0.0000	0.2039	-0.3352	-0.0000	-0.0000	0.3352	-0.0133	0.5491	-0.0133	0.5491	0.0290

4Y	0.0000	0.2190	-0.9154	-0.0000	-0.2190	0.9154	-0.0056	0.5482	-0.0496
5X	0.0000	0.2434	-0.3502	0.0000	-0.2434	0.3502	-0.0145	0.4332	0.0288
5XY	0.0000	0.2434	-0.3502	-0.0000	-0.2434	0.3502	-0.0051	0.4411	0.0263
5Y	0.0000	0.0000	-0.3493	0.0000	0.0000	0.3493	-0.0069	0.4419	-0.0449
6X	0.0000	0.2387	-0.5133	-0.0000	-0.2387	0.5133	-0.0071	0.3613	0.0248
6XY	0.0000	0.2387	-0.5052	-0.0000	-0.2387	0.5052	-0.0077	0.3690	0.0225
6Y	0.0000	0.2410	-1.1325	-0.0000	-0.2410	1.1325	-0.0003	0.3682	-0.0395
7X	0.0000	0.2859	-0.5541	0.0000	-0.2859	0.5541	-0.0100	0.2861	0.0215
7XY	0.0000	0.2859	-0.5541	-0.0000	-0.2859	0.5541	-0.0003	0.2936	0.0193
8X	0.0000	0.0000	-0.5534	0.0000	0.0000	0.5534	-0.0024	0.2942	-0.0351
8Y	0.0000	0.3125	-0.6891	-0.0000	-0.3125	0.6891	-0.0013	0.2221	0.0169
8XY	0.0000	0.3125	-0.6810	-0.0000	-0.3125	0.6810	-0.0034	0.2296	0.0148
8Y	0.0000	0.2530	-1.3512	-0.0000	-0.2530	1.3512	0.0064	0.2291	-0.0294
15X	0.0000	0.4392	-1.0248	-34.2131	29.2425	188.3774	0.0000	0.0000	0.0000
15XY	0.0000	0.4392	-1.0248	34.9789	31.4603	189.3077	0.0000	0.0000	0.0000
15Y	0.0000	0.0000	-1.0248	-33.0372	33.0080	-225.1900	0.0000	0.0000	0.0000
16X	0.0000	9.7364	-1.3281	0.0000	-9.7364	1.3281	-0.0373	0.7882	0.2380
17Y	-7.0000	7.1780	-2.3728	7.0000	-7.1780	2.3728	0.0081	0.5478	-0.2214
18Y	-7.0000	7.1780	-2.4018	7.0000	-7.1780	2.4018	0.0157	0.3675	-0.2177
19Y	-7.0000	7.1780	-2.3728	7.0000	-7.1780	2.3728	0.0180	0.2293	-0.1428
20AY	0.0000	0.0000	-0.6406	-0.0000	0.0000	0.6406	0.0148	0.0563	-0.0107
20BX	0.0000	0.6316	-0.6343	-0.0000	-0.6316	0.6343	0.0019	0.0442	0.0120
21X	0.0000	0.0000	-0.3466	-0.0000	0.0000	0.3466	0.1353	0.0052	0.0200
21XY	0.0000	0.0000	-0.3466	0.0000	-0.0000	0.3466	-0.0132	0.0296	-0.0042
21Y	0.0000	0.0000	-0.3466	0.0000	0.0000	0.3466	0.0873	0.0079	-0.0093
22X	0.0000	0.3674	-0.3466	0.0000	-0.3674	0.3466	0.0022	0.1403	-0.0056
22XY	0.0000	0.3674	-0.4107	0.0000	-0.3674	0.4107	0.0028	0.0416	0.0078
22Y	0.0000	0.0000	-0.4107	0.0000	0.0000	0.4107	0.0461	0.0489	-0.0173
26Y	0.0000	0.0000	-0.0835	0.0000	0.0000	0.0835	0.0027	0.5480	-0.1350
27Y	0.0000	0.0000	-0.1110	0.0000	-0.0000	0.1110	0.0093	0.3679	-0.1279
28Y	0.0000	0.0000	-0.0835	0.0000	-0.0000	0.0835	0.0125	0.2293	-0.0841
9S	0.0000	0.0000	-0.6364	0.0000	0.0000	0.6364	0.0148	0.1701	-0.0275
10S	0.0000	0.0000	-0.6392	0.0000	0.0000	0.6392	0.0139	0.1291	-0.0275
11S	0.0000	0.0000	-0.8152	0.0000	-0.0000	0.8152	0.0208	0.0966	-0.0251
12S	0.0000	0.0000	-1.1543	0.0000	0.0000	1.1543	0.0223	0.0643	-0.0230
13S	0.0000	0.0000	-0.9523	0.0000	-0.0000	0.9523	0.0272	0.0302	-0.0194
14S	0.0000	0.0000	-1.4748	-0.0000	0.0000	1.4748	-0.0104	-0.0088	-0.0202
i0.50E129S	0.0000	0.0126	-0.0607	0.0000	-0.0126	0.0607	-0.0165	0.7851	-0.0131
i0.50E113S	0.0000	0.0042	-0.0355	0.0000	-0.0042	0.0355	-0.0092	0.5447	-0.0101
i0.50E99S	0.0000	0.0000	-0.0355	0.0000	0.0000	0.0355	-0.0035	0.3647	-0.0081
i0.50E85S	0.0000	0.0000	-0.0355	-0.0000	0.0000	0.0355	0.0016	0.2246	-0.0069
21XF0.50S	0.0000	0.0000	-0.0699	-0.0000	0.0000	0.0699	0.0123	0.0007	-0.0487
22PF0.50S	0.0000	0.0000	-0.0230	-0.0000	0.0000	0.0230	0.0213	0.1024	-0.3430
9X	0.0000	0.3307	-0.6364	-0.0000	-0.3307	0.6364	-0.0061	0.1701	0.0187
9XY	0.0000	0.3307	-0.6364	0.0000	-0.3307	0.6364	0.0032	0.1805	0.0160
9Y	0.0000	0.0000	-0.6364	0.0000	-0.0000	0.6364	0.0042	0.1810	-0.0302
10X	0.0000	0.3255	-0.6392	0.0000	-0.3255	0.6392	-0.0017	0.1292	0.0198
10XY	0.0000	0.4225	-0.9922	-0.0000	-0.4225	0.9922	0.0004	0.1432	0.0164
10Y	0.0000	0.4430	-1.7832	-0.0000	-0.4430	1.7832	0.0129	0.1428	-0.0309
11X	0.0000	0.4067	-0.8152	-0.0000	-0.4067	0.8152	-0.0021	0.0966	0.0190
11XY	0.0000	0.4067	-0.8152	0.0000	-0.4067	0.8152	0.0023	0.1131	0.0150
11Y	0.0000	0.0000	-0.8152	0.0000	-0.0000	0.8152	0.0136	0.1140	-0.0290
12X	0.0000	0.5742	-1.1543	0.0000	-0.5742	1.1543	-0.0012	0.0644	0.0179
12XY	0.0000	0.6732	-1.3843	0.0000	-0.6732	1.3843	0.0032	0.0860	0.0131
12Y	0.0000	0.2510	-1.8273	0.0000	-0.2510	1.8273	0.0219	0.0844	-0.0279
13X	0.0000	0.4040	-0.9523	0.0000	-0.4040	0.9523	0.0008	0.0320	0.0166
13XY	0.0000	0.4900	-1.1513	0.0000	-0.4900	1.1513	0.0031	0.0571	0.0114
13Y	0.0000	0.2280	-1.5633	0.0000	-0.2280	1.5633	0.0280	0.0561	-0.0247
14X	0.0000	0.6248	-1.4748	-0.0000	-0.6248	1.4748	0.0007	0.0066	0.0125
14XY	0.0000	0.8368	-1.9658	-0.0000	-0.8368	1.9658	0.0034	0.0265	0.0086

14Y 0.0000 0.5580 -2.9698 0.0000 -0.5580 2.9698 0.0462 0.0078 -0.0237
 21XF0.50Y 0.0000 0.0000 -0.0919 -0.0000 0.0000 0.0919 0.0139 0.0171 -0.1958
 22FF0.50X 0.0000 0.0322 -0.0230 0.0000 -0.0322 0.0230 0.0023 0.0959 -0.0025

Crossing Diagonal Check for Load Case "NESC Heavy" (RLOUT controls):

Comp. Member Label	Tens. Member Label	Connect Leg for	Force In Comp. Member	Force In Tens. Member	-----Original-----					-----Alternate-----																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																						
					L/R	RLX	RLY	RLZ	L/R	KL/R	Curve No.	L/R	RLOUT	I/R	KL/R	Curve No.																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
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Summary of Clamp Capacities and Usages for Load Case "NESC Heavy":

Clamp Label	Force (kips)	Input Holding Capacity (kips)	Factored Holding Capacity (kips)	Usage %
1	9.827	50.00	50.00	19.65
2	9.757	50.00	50.00	19.51
3	10.100	50.00	50.00	20.20
4	10.303	50.00	50.00	20.61
5	10.100	50.00	50.00	20.20
6	10.310	50.00	50.00	20.62
7	10.100	50.00	50.00	20.20
8	10.303	50.00	50.00	20.61
9	14.764	50.00	50.00	29.53
10	14.786	50.00	50.00	29.57
11	14.764	50.00	50.00	29.53
12	0.595	50.00	50.00	1.19
13	0.941	50.00	50.00	1.88
14	1.158	50.00	50.00	2.32
15	1.375	50.00	50.00	2.75
16	1.837	50.00	50.00	3.67
17	1.844	50.00	50.00	3.69
18	1.580	50.00	50.00	3.16
19	3.022	50.00	50.00	6.04
20	5.107	50.00	50.00	10.21
21	5.445	50.00	50.00	10.89
22	5.216	50.00	50.00	10.43

23	2.491	50.00	50.00	4.98
24	2.670	50.00	50.00	5.34
25	1.711	50.00	50.00	3.42
26	3.354	50.00	50.00	6.71
27	8.356	50.00	50.00	16.71
28	1.078	50.00	50.00	2.16
29	1.539	50.00	50.00	3.08
30	1.251	50.00	50.00	2.50
31	2.137	50.00	50.00	4.27

Equilibrium Joint Positions and Rotations for Load Case "NESC Extreme":

Joint Label	X-Displ (ft)	Y-Displ (ft)	Z-Displ (ft)	X-Rot (deg)	Y-Rot (deg)	Z-Rot (deg)	X-Pos (ft)	Y-Pos (ft)	Z-Pos (ft)
1P	0.01969	0.6965	-0.0498	-0.7072	-0.0072	-0.0700	3.02	3.697	128.8
2P	0.0199	0.6412	-0.04938	-0.8106	-0.0326	-0.0746	3.02	3.641	124.3
3P	0.02339	0.5701	-0.04669	-0.8811	-0.0056	-0.0801	3.023	3.57	119.8
4P	0.01971	0.4793	-0.04184	-0.6880	-0.0410	-0.0879	3.02	3.479	112.8
5P	0.02409	0.3883	-0.03722	-0.6153	0.0352	-0.0932	3.024	3.388	104.8
6P	0.01982	0.3276	-0.03215	-0.5229	-0.0400	-0.0962	3.02	3.328	98.72
7P	0.02462	0.2669	-0.02814	-0.4733	0.0111	-0.0972	3.025	3.267	91.72
8P	0.01949	0.2152	-0.02307	-0.3465	-0.0091	-0.0980	3.019	3.215	84.73
15P	0	0	0	0.0000	0.0000	0.0000	15.44	15.44	0
16P	0.03498	0.6998	-0.2226	-0.7310	-0.0006	-0.0598	0.03492	17.45	128.6
17P	0.03882	0.4794	-0.1908	-0.6851	-0.0414	-0.1094	3.039	14.73	112.6
18P	0.04334	0.3279	-0.1734	-0.5178	-0.0402	-0.1079	3.043	17.08	98.58
19P	0.04144	0.2159	-0.1099	-0.3797	-0.0091	-0.1059	3.041	14.47	84.64
20AP	0.02558	0.07857	-0.007562	0.0394	0.0522	-0.1452	9.594	0.07857	39.99
20BP	0.04481	0.09766	-0.001819	-0.1700	-0.0082	-0.1417	0.04481	9.666	39.98
21P	-0.2942	0.02321	-0.03094	0.0000	0.0000	0.0000	11.48	2.231	24.97
22P	0.001695	0.3103	0.04547	0.0000	0.0000	0.0000	2.21	12.08	25.05
23P	-0.006129	0.4872	0.1686	-0.7512	0.0056	-0.0842	-0.006129	-13.76	113
24P	-0.01219	0.3353	0.1517	-0.5698	0.0002	-0.0878	-0.01219	-16.41	98.9
25P	-0.009552	0.223	0.09111	-0.4246	-0.0076	-0.1000	-0.009552	-14.03	84.84
26P	0.02832	0.4793	-0.12	-0.7943	-0.0410	-0.0943	3.028	9.104	112.7
27P	0.03077	0.3277	-0.1071	-0.6225	-0.0399	-0.0972	3.031	10.2	98.64
28P	0.03089	0.2156	-0.06931	-0.4816	-0.0090	-0.1165	3.031	8.841	84.68
29P	0	0	0	0.0000	0.0000	0.0000	1.5	0	-4.25
30P	0.01482	0.09925	-0.001093	-0.0958	0.0453	-0.0485	1.515	0.09925	25
31P	0.02554	0.09509	-0.001509	0.0394	0.0244	-0.0615	1.526	0.09509	40
32P	0.01478	0.02759	-0.002759	0.3752	-0.0116	-0.0761	1.515	0.2178	84.75
33P	0.0143	0.3306	-0.003484	-0.5582	0.0015	-0.0801	1.514	0.3306	98.75
34P	0.01491	0.481	-0.004526	-0.6200	0.0018	-0.0821	1.515	0.481	112.7
35P	0.01611	0.6988	-0.006237	-1.1598	0.0051	-0.0818	1.516	0.6988	128.7
36P	0.01685	0.8395	-0.007959	-1.4742	0.0051	-0.0818	1.517	0.8395	134.7
37P	0.01801	1.085	-0.01138	-1.6026	0.0052	-0.0818	1.518	1.085	143.7
1X	0.01225	0.697	0.02997	-0.6550	0.0203	-0.0728	3.012	-2.303	128.8
1XY	0.01234	0.703	0.02989	-0.7041	0.0242	-0.0654	-2.988	-2.297	128.8
1Y	0.01964	0.7027	-0.04929	-0.6959	-0.0100	-0.0708	-2.98	3.703	128.8
2X	0.01231	0.642	0.02927	-0.8243	0.0334	-0.0780	3.012	-2.358	124.3
2XY	0.01233	0.6481	0.02928	-0.7812	-0.0344	-0.0694	-2.988	-2.352	124.3
2Y	0.01979	0.6476	-0.04799	-0.7930	0.0390	-0.0787	-2.98	3.648	124.3
3X	0.007412	0.5703	0.02885	-0.8864	0.0201	-0.0823	3.007	-2.43	119.8
3XY	0.01503	0.5792	0.02888	-0.8600	0.0094	-0.0748	-2.985	-2.421	119.8
3Y	0.01554	0.5791	-0.04645	-0.8539	0.0087	-0.0854	-2.984	3.579	119.8
4X	0.0104	0.4799	0.02561	-0.6631	0.0540	-0.0895	3.01	-2.52	112.8
4XY	0.01035	0.4896	0.0257	-0.6676	-0.0433	-0.0819	-2.99	-2.51	112.8
4Y	0.01933	0.4882	-0.04183	-0.6807	0.0463	-0.0966	-2.981	3.489	112.8
5X	0.005006	0.3882	0.02242	-0.6204	-0.0336	-0.0938	3.005	-2.612	104.8
5XY	0.01421	0.3983	0.02241	-0.6183	0.0417	-0.0856	-2.986	-2.602	104.8
5Y	0.01471	0.3984	-0.03738	-0.6165	-0.0382	-0.0977	-2.985	3.398	104.8
6X	0.009322	0.338	0.01846	-0.5174	0.0402	-0.0983	3.009	-2.672	98.77
6XY	0.009078	0.338	0.01818	-0.5159	-0.0370	-0.0872	-2.991	-2.662	98.77
6Y	0.01948	0.3377	-0.03249	-0.5209	0.0355	-0.0997	-2.981	3.338	98.72

7X	0.004357	0.2667	0.01538	-0.4747	-0.0151	-0.0994	3.004	-2.733	91.77
7XY	0.01384	0.2772	0.01486	-0.4721	0.0122	-0.0829	-2.986	-2.723	91.76
7Y	0.0149	0.2773	-0.02867	-0.4698	-0.0184	-0.0951	-2.985	3.277	91.72
8X	0.01023	0.2155	0.01122	-0.3438	-0.0055	-0.1007	3.01	-2.785	84.76
8XY	0.008425	0.2262	0.01037	-0.3377	-0.0019	-0.0785	-2.992	-2.774	84.76
8Y	0.02118	0.2261	-0.02391	-0.3413	-0.0084	-0.0907	-2.979	3.226	84.73
15X	0	0	0	0.0000	0.0000	0.0000	15.44	-15.44	0
15XY	0	0	0	0.0000	0.0000	0.0000	-15.44	15.44	0
15Y	0	0	0	0.0000	0.0000	0.0000	-15.44	15.44	0
16X	-0.0006972	0.7019	0.1978	-0.7099	0.0313	-0.0474	-0.0006972	-16.05	129
17Y	0.03812	0.489	-0.1913	-0.6920	0.0466	-0.0795	-2.962	14.74	112.6
18Y	0.04285	0.3376	-0.177	-0.3377	0.0357	-0.0860	-2.957	17.09	98.57
19Y	0.04102	0.2266	-0.1118	-0.3896	-0.0085	-0.1014	-2.959	14.48	84.64
20AY	0.0256	0.1153	-0.01801	0.0392	-0.1404	-0.1264	-9.542	0.1153	39.98
20BX	0.006458	0.09773	0.001804	-0.0416	0.0372	-0.1401	0.006458	-9.47	40
21X	0.1533	0.05278	0.01033	0.0000	0.0000	0.0000	11.92	-2.155	25.01
21XY	-0.01243	0.08098	-0.0165	0.0000	0.0000	0.0000	-11.78	-2.127	24.98
21Y	0.1032	0.06888	0.02038	0.0000	0.0000	0.0000	-11.67	2.277	25.02
22X	0.002001	0.2069	-0.01755	0.0000	0.0000	0.0000	2.21	-11.56	24.98
22XY	0.008055	0.1016	-0.003567	0.0707	-0.0073	-0.0257	-2.2	-11.67	25
22Y	0.05862	0.1132	-0.01591	-0.0825	0.0451	-0.2959	-2.149	11.88	24.98
26Y	0.02969	0.4891	-0.1199	-0.7886	0.0465	-0.1004	-2.97	9.114	112.7
27Y	0.032	0.3377	-0.1084	-0.6381	0.0357	-0.1001	-2.968	10.21	98.64
28Y	0.03099	0.2265	-0.07033	-0.4884	-0.0083	-0.1034	-2.969	8.851	84.68
9S	0.02831	0.1757	-0.0222	-0.2334	-0.0335	-0.0799	4.166	4.313	76.98
10S	0.02813	0.1447	-0.0211	-0.1795	-0.0133	-0.0645	5.303	5.42	69.23
11S	0.03478	0.1225	-0.01776	-0.1364	-0.0360	-0.0567	6.448	6.535	61.48
12S	0.03842	0.0991	-0.01456	-0.0909	-0.0414	-0.0458	7.907	7.968	51.57
13S	0.04403	0.07795	-0.009238	-0.1572	0.0747	-0.0328	9.613	9.647	39.99
14S	-0.002845	0.01889	-0.01451	-0.1742	0.1366	-0.0188	11.77	11.79	24.99
10.50E129S	0.01589	0.6998	-0.01386	-0.7739	-0.0079	-0.0611	0.01589	0.6998	128.8
10.50E113S	0.01487	0.4846	-0.006699	-0.6558	-0.0568	-0.1091	0.01487	0.4846	112.8
10.50E99S	0.01432	0.3328	-0.005359	-0.5079	-0.0670	-0.0945	0.01432	0.3328	98.74
10.50E85S	0.01477	0.2199	-0.004992	-0.3328	-0.0732	-0.0996	0.01477	0.2199	84.75
21XF0.50S	0.01413	0.04579	-0.1012	-0.0954	0.8151	-0.4216	11.78	0.04579	24.9
22PF0.50S	0.02091	0.4805	-0.1912	0.0000	0.0000	0.0000	0.02091	12.25	24.81
9X	0.003958	0.1751	0.01172	-0.2318	0.147	-0.0823	4.142	-3.962	77.01
9XY	0.01246	0.1913	0.01001	-0.2146	0.0265	-0.0795	-4.125	-3.946	77.01
9Y	0.01916	0.1911	-0.02383	-0.2235	0.0027	-0.0921	-4.118	4.329	76.98
10X	0.0066	0.1452	0.01167	-0.1869	-0.0187	-0.0654	5.282	-5.13	69.26
10XY	0.008292	0.166	0.009105	-0.1774	0.0060	-0.0767	-5.267	-5.109	69.26
10Y	0.0271	0.1653	-0.02365	-0.1637	-0.0268	-0.0905	-5.248	5.44	69.23
11X	0.004814	0.121	0.01021	-0.1302	0.0115	-0.0564	6.418	-6.292	61.51
11XY	0.009072	0.146	0.006825	-0.1129	-0.0254	-0.0683	-6.404	-6.267	61.51
11Y	0.02898	0.1479	-0.02141	-0.1275	-0.0055	-0.0784	-6.384	6.561	61.48
12X	0.004873	0.1014	0.007894	-0.0947	-0.0096	-0.0423	7.874	-7.767	51.59
12XY	0.008461	0.1332	0.003151	-0.0725	-0.0048	-0.0630	-7.86	-7.736	51.58
12Y	0.03729	0.1287	-0.01998	-0.0848	-0.0329	-0.0726	-7.832	7.998	51.56
13X	0.005096	0.07985	0.003897	-0.0865	0.0069	-0.0369	9.574	-9.489	40.01
13XY	0.007701	0.1162	0.0003766	-0.0959	-0.0130	-0.0416	-9.561	-9.452	40
13Y	0.04616	0.1156	-0.01595	-0.1032	-0.0534	-0.0544	-9.522	9.684	39.98
14X	0.0006584	0.05461	0.002791	-0.1220	0.0128	-0.0343	11.77	-11.72	25
14XY	0.008506	0.0825	-0.00129	-0.1947	0.0034	-0.0079	-11.76	-11.69	25
14Y	0.05868	0.0691	-0.01486	-0.2179	0.0574	0.0019	-11.71	11.84	24.99
21XF0.50Y	0.01568	0.07698	-0.1486	-0.0965	-0.9774	0.1667	-11.75	0.07698	24.85
22PF0.50X	0.002736	0.2414	-0.07724	0.0000	0.0000	0.0000	0.002736	-11.53	24.92

Joint Support Reactions for Load Case "NESC Extreme":

Joint X X X Y Y H-Shear Z Comp. Uplift Result. Result. X X-M. Y Y-M. E-Bend-M Z Z-M. Max.

Label	Force Usage (kips)	Force Usage % (kips)	Usage %	Force Usage (kips)	Force Usage % (kips)	Force Usage %	Force (kips)	Usage Moment % (ft-k)	Usage Moment % (ft-k)	Usage %	Usage Moment % (ft-k)	Usage %
15P	-29.55	0.0	-29.88	0.0	0.0	-207.29	0.0	0.0	211.51	0.0	-1.27	0.0
29P	-0.07	0.0	-2.53	0.0	0.0	-12.94	0.0	0.0	13.19	0.0	30.58	0.0
15X	30.62	0.0	-33.84	0.0	0.0	168.73	0.0	0.0	174.79	0.0	4.46	0.0
15XY	-37.01	0.0	-42.78	0.0	0.0	200.63	0.0	0.0	208.45	0.0	6.85	0.0
15Y	30.53	0.0	-30.59	0.0	0.0	-204.80	0.0	0.0	209.31	0.0	4.44	0.0

Joint Displacements, Loads and Member Forces on Joints for Load Case "NESC Extreme":

Joint X Label	External Load (kips)	Y External Load (kips)	Z External Load (kips)	X Member Force (kips)	Y Member Force (kips)	Z Member Force (kips)	X Disp. (ft)	Y Disp. (ft)	Z Disp. (ft)
1P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0197	0.6965	-0.0498
2P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0199	0.6412	-0.0484
3P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0234	0.5701	-0.0467
4P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0197	0.4793	-0.0418
5P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0241	0.3883	-0.0372
6P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0195	0.3276	-0.0321
7P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0246	0.2669	-0.0281
8P	0.0000	0.9518	-0.7211	0.0000	-0.9518	0.7211	0.0195	0.2152	-0.0231
15P	0.0000	0.6885	-0.5264	29.5451	29.1875	-206.7639	0.0000	0.0000	0.0000
16P	0.3610	6.0373	-0.5276	-0.3610	-6.0373	0.5276	0.0349	0.6998	-0.2226
17P	4.5710	5.4313	-0.2586	-4.5710	-5.4313	0.2586	0.0386	0.4794	-0.1908
18P	4.5710	5.4313	-0.2586	-4.5710	-5.4313	0.2586	0.0433	0.3279	-0.1734
19P	4.5710	5.4313	-0.2586	-4.5710	-5.4313	0.2586	0.0414	0.2159	-0.1039
20AP	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0256	0.0786	-0.0076
20BP	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0448	0.0977	-0.0182
21P	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	-0.2942	0.0232	-0.0309
22P	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0017	0.3103	0.0455
23P	0.7930	9.5953	-1.2276	-0.7930	-9.5953	1.2276	-0.0061	0.4872	0.1686
24P	0.7930	9.5953	-1.2276	-0.7930	-9.5953	1.2276	-0.0122	0.3353	0.1517
25P	0.7930	9.5953	-1.2276	-0.7930	-9.5953	1.2276	-0.0096	0.2230	0.0911
26P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0283	0.4793	-0.1200
27P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0308	0.3277	-0.1071
28P	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0307	0.2156	-0.0693
29P	0.0000	0.6885	-0.5264	0.0741	1.8423	-12.4174	0.0000	0.0000	0.0000
30P	0.0000	1.9135	-1.3374	0.0000	-1.9135	1.3374	0.0148	0.0993	-0.0011
31P	0.0000	1.8145	-1.2724	0.0000	-1.8145	1.2724	0.0255	0.0951	-0.0015
32P	0.0000	2.0588	-1.4541	0.0000	-2.0588	1.4541	0.0148	0.2178	-0.0028
33P	0.0000	0.7913	-0.5436	0.0000	-0.7913	0.5436	0.0143	0.3306	-0.0035
34P	0.0000	0.8283	-0.5886	0.0000	-0.8283	0.5886	0.0149	0.4810	-0.0045
35P	0.0000	0.5653	-0.3946	0.0000	-0.5653	0.3946	0.0161	0.6988	-0.0062
36P	0.0000	4.3903	-1.2626	0.0000	-4.3903	1.2626	0.0169	0.8395	-0.0080
37P	0.0000	2.8223	-3.6666	0.0000	-2.8223	3.6666	0.0180	1.0846	-0.0114
1X	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0123	0.6970	0.0300
1XY	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0123	0.7030	0.0299
1Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0196	0.7027	-0.0493
2X	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0123	0.6420	0.0293
2XY	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0123	0.6481	0.0293
2Y	0.0000	0.8433	-0.3226	0.0000	-0.8433	0.3226	0.0198	0.6476	-0.0480
3X	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0074	0.5703	0.0289
3XY	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0150	0.5792	0.0289
3Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0155	0.5791	-0.0464
4X	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0104	0.4799	0.0256
4XY	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0103	0.4896	0.0257

4Y	0.0000	0.9853	-0.3536	0.0000	-0.9853	0.3536	0.0193	0.4892	-0.0418
5X	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0050	0.3882	0.0224
5Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0142	0.3983	0.0224
5Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0147	0.3984	-0.0374
6X	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0093	0.3280	0.0185
6Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0091	0.3380	0.0182
6Y	0.0000	1.0563	-0.3696	0.0000	-1.0563	0.3696	0.0195	0.3377	-0.0325
7X	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0044	0.2667	0.0154
7Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0138	0.2772	0.0149
7Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0149	0.2773	-0.0287
8X	0.0000	0.9518	-0.7211	0.0000	-0.9518	0.7211	0.0102	0.2155	0.0112
8Y	0.0000	0.9518	-0.7211	0.0000	-0.9518	0.7211	0.0084	0.2262	0.0104
8Y	0.0000	1.7858	-0.9051	0.0000	-1.7858	0.9051	0.0212	0.2261	-0.0239
15X	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
15Y	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
15Y	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
16X	0.3610	6.0373	-0.5276	0.0000	-6.0373	0.5276	0.0000	0.0000	0.0000
17Y	-3.7780	4.4273	-1.1626	0.0000	3.7780	-4.4273	0.0000	0.0000	0.0000
18Y	-3.7780	4.4273	-1.1626	0.0000	3.7780	-4.4273	0.0000	0.0000	0.0000
19Y	-3.7780	4.4273	-1.1626	0.0000	3.7780	-4.4273	0.0000	0.0000	0.0000
20AY	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
20BX	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
21X	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
21XY	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
21Y	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
22X	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
22XY	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
22Y	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
26Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0020	0.2069	-0.0176
27Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0020	0.2069	-0.0176
28Y	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0020	0.2069	-0.0176
9S	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
10S	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
11S	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
12S	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
13S	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
14S	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
10.50E129S	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0020	0.2069	-0.0176
10.50E113S	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0020	0.2069	-0.0176
10.50E99S	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0020	0.2069	-0.0176
10.50E85S	0.0000	0.2633	-0.1946	0.0000	-0.2633	0.1946	0.0020	0.2069	-0.0176
21XF0.50S	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
22PF0.50S	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
9X	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
9XY	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
9Y	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
10X	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
10XY	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
10Y	0.0000	2.1395	-0.9314	0.0000	-2.1395	0.9314	0.0020	0.2069	-0.0176
11X	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
11XY	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
11Y	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
12X	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
12XY	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
12Y	0.0000	1.5155	-0.7094	0.0000	-1.5155	0.7094	0.0020	0.2069	-0.0176
13X	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
13XY	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
13Y	0.0000	1.4405	-0.6924	0.0000	-1.4405	0.6924	0.0020	0.2069	-0.0176
14X	0.0000	0.6885	-0.5264	0.0000	-0.6885	0.5264	0.0000	0.0000	0.0000
14XY	0.0000	1.2915	-0.6594	0.0000	-1.2915	0.6594	0.0020	0.2069	-0.0176

14Y 0.0000 2.5265 -0.9324 0.0000 -2.5265 0.9324 0.0587 0.0691 -0.0149
 21XF0.50Y 0.0000 0.6885 -0.5264 -0.0000 -0.6885 0.5264 0.0157 0.0770 -0.1486
 22PF0.50X 0.0000 0.6885 -0.5264 0.0000 -0.6885 0.5264 0.0027 0.2414 -0.0772

Crossing Diagonal Check for Load Case "NESC Extreme" (RLOUT controls):

Comp. Tens. Member Label	Connect Leg for	Force In	Force In	Original				Alternate			
				Comp. Member	Comp. Member	L/R	Cap.	Support	L/R	Cap.	Curve
				(kips)	(kips)	RLZ	RLZ	Cap.	Cap.	Cap.	No.
15BX	15BX	Long only	-2.54	0.01	15.63	0.500	0.500	131.11	128.51	5	5
16BP	16BP	Short only	-0.61	-1.78	14.30	0.500	0.500	129.55	127.32	5	5
16BY	16BP	Short only	-1.78	-0.61	14.30	0.500	0.500	129.55	127.32	5	5
17BP	17BY	Long only	-1.55	-1.15	39.61	0.500	0.500	100.45	105.33	2	2
17BY	17BP	Long only	-1.15	-1.55	39.61	0.500	0.500	100.45	105.33	2	2
19BX	19BX	Long only	-4.69	-0.49	61.20	0.500	0.500	77.55	88.16	2	2
19BX	19BX	Long only	-0.49	-4.69	61.20	0.500	0.500	77.55	88.16	2	2
20BP	20BP	Long only	-7.48	-6.20	61.20	0.500	0.500	77.55	88.16	2	2
20BY	20BP	Long only	-6.20	-7.48	61.20	0.500	0.500	77.55	88.16	2	2
21AX	21AP	Short only	-1.67	0.15	44.33	0.580	0.580	95.79	101.84	2	2
21BP	21BY	Short only	-9.82	-9.98	44.33	0.580	0.580	95.79	101.84	2	2
21BY	21BP	Short only	-9.98	-9.82	44.33	0.580	0.580	95.79	101.84	2	2

Summary of Clamp Capacities and Usages for Load Case "NESC Extreme":

Clamp Force Label	(kips)	Input Holding Capacity (kips)	Factored Holding Capacity (kips)	Usage %
1	6.071	50.00	50.00	12.14
2	6.071	50.00	50.00	12.14
3	7.104	50.00	50.00	14.21
4	5.935	50.00	50.00	11.87
5	7.104	50.00	50.00	14.21
6	5.935	50.00	50.00	11.87
7	7.104	50.00	50.00	14.21
8	5.935	50.00	50.00	11.87
9	9.706	50.00	50.00	19.41
10	9.706	50.00	50.00	19.41
11	9.706	50.00	50.00	19.41
12	0.903	50.00	50.00	1.81
13	1.047	50.00	50.00	2.09
14	1.119	50.00	50.00	2.24
15	2.002	50.00	50.00	4.00
16	2.333	50.00	50.00	4.67
17	1.673	50.00	50.00	3.35
18	1.599	50.00	50.00	3.20
19	2.693	50.00	50.00	5.39
20	2.335	50.00	50.00	4.67
21	2.216	50.00	50.00	4.43
22	2.520	50.00	50.00	5.04
23	0.960	50.00	50.00	1.92
24	1.005	50.00	50.00	2.01
25	0.689	50.00	50.00	1.38

26	4.568	50.00	50.00	9.14
27	4.627	50.00	50.00	9.25
28	1.184	50.00	50.00	2.37
29	1.136	50.00	50.00	2.27
30	1.099	50.00	50.00	2.20
31	1.450	50.00	50.00	2.90

*** Overall summary for all load cases - Usage = Maximum Stress / Allowable Stress
Printed capacities do not include the strength factor entered for each load case.
The Group Summary reports on the member and load case that resulted in maximum usage
which may not necessarily be the same as that which produces maximum force.

Group Summary (Compression Portion):

Group Label	No.	Comp.	Member	Bolts	Angle	Steel	Max Usage	Max	Comp.	Comp.	L/R	RLZ	RLY	RLX	Comp.	Comp.	Connect.	L/R																	
																			Curve No.	Type	Size	Strength	Usage Cont-	rol	In Member	Use Control	Force Control	Capacity Connect.	Connect.						
																														(ksi)	%	Comp.	(kips)	(kips)	(kips)

1	LEG1	SAE	3.5X3.5X0.25	36.0	90.98	Comp	90.98	3P	-29.853NESC Ext	32.813	36.400	54.375	1.000	1.000	1.000	121.04		
121.04	7.000	4																
2	LEG2	SAE	6X6X0.3125	36.0	87.73	Comp	87.73	5Y	-86.923NESC Hea	99.083	109.200	203.906	1.000	1.000	1.000	60.50		
60.50	6.050	1																
3	LEG3	SAE	8X8X0.5	36.0	68.57	Comp	68.57	7Y	-171.178NESC Hea	249.636	254.800	380.624	1.000	1.000	1.000	52.83		
52.83	7.000	1																
4	LEG4	SAE	8X8X0.625	36.0	72.74	Comp	72.74	10Y	-211.817NESC Hea	306.646	291.200	543.749	1.000	1.000	1.000	60.12		
60.12	7.915	1																
5	LEG5	SAE	8X8X0.75	36.0	63.48	Comp	63.48	11Y	-212.763NESC Hea	335.162	0.000	0.000	1.000	1.000	1.000	76.95		
76.95	10.131	1																
6	X1	SAE	2.5X2.5X0.1875	36.0	86.45	Comp	86.45	15AX	-13.515NESC Ext	15.633	18.200	20.391	0.500	0.500	0.500	131.11		
128.51	10.817	5																
7	X2	SAU	2.5X2X0.1875	36.0	85.43	Comp	85.43	16AX	-12.218NESC Ext	14.303	18.200	20.391	0.500	0.500	0.500	129.55		
127.32	9.220	5																
8	X3	SAU	4X3X0.25	36.0	57.37	Comp	57.37	17AX	-22.726NESC Hea	39.613	45.500	67.969	0.500	0.750	0.500	100.45		
105.33	10.000	2																
9	X4	SAU	3.5X2.5X0.25	36.0	60.32	Comp	60.32	18AXY	-19.747NESC Hea	32.738	36.400	54.375	0.500	0.750	0.500	104.34		
108.25	8.521	2																
10	X5	SAU	4X3.5X0.3125	36.0	54.28	Comp	54.28	19AX	-33.224NESC Hea	61.204	63.700	118.945	0.500	0.750	0.500	77.55		
88.16	9.220	2																
11	X6	SAU	5X3.5X0.25	36.0	26.18	Cross	26.18	21BP	-11.095NESC Hea	42.374	54.600	81.562	1.000	0.580	0.580	95.79		
107.89	10.597	3																
12	X7	SAU	4X3X0.25	36.0	16.00	Comp	16.00	22BX	-4.959NESC Hea	30.985	36.400	54.375	0.560	0.560	0.560	126.41		
124.92	12.246	5																
13	X8	SAU	3.5X3X0.25	36.0	8.89	Tens	6.03	23AX	-1.359NESC Hea	22.542	27.300	40.781	0.550	0.550	0.550	147.17		
140.74	14.070	5																
14	X9	SAU	5X3X0.25	36.0	18.11	Comp	18.11	24AXY	-3.882NESC Ext	21.431	27.300	40.781	0.550	0.550	0.550	173.71		
160.96	17.450	5																
15	D1	SAU	5X3X0.25	36.0	79.17	Tens	11.60	25BY	-3.919NESC Ext	33.799	63.700	95.156	1.000	0.500	0.500	127.63		
125.86	14.103	5																
16	D2	SAE	2X2X0.1875	36.0	41.26	Tens	7.59	26AY	-0.307NESC Hea	4.044	18.200	20.391	0.500	0.500	0.500	256.65		
224.16	16.833	5																
17	D3	SAU	3X2X0.25	36.0	65.57	Tens	0.00	27BXY	0.000	6.420	36.400	54.375	1.000	0.500	0.500	264.74		
230.33	19.194	5																
18	D4	SAU	4X3X0.25	36.0	63.43	Tens	55.12	28BY	-6.332NESC Ext	11.489	45.500	67.969	0.500	1.000	0.500	205.19		
205.19	15.321	4																
19	D5	SAU	3.5X3X0.25	36.0	76.25	Tens	11.35	29AP	-2.911NESC Hea	25.652	45.500	67.969	0.250	0.250	0.250	135.61		

131.93	28.523	5	5	SAE	1.75X1.75X0.1875	36.0	51.30	Tens	0.00	35Y	0.000	4.027	9.100	10.195	1.000	1.000	1.000	209.91
20		4	H1	SAE	1.75X1.75X0.1875	36.0	51.30	Tens	0.00	35Y	0.000	4.027	9.100	10.195	1.000	1.000	1.000	209.91
209.91	6.000	4	H2	SAU	4X3.5X0.25	36.0	53.92	Comp	53.92	37AY	-14.720NESC Ext	27.341	27.300	40.781	1.000	0.500	0.500	148.70
21		6	H3	SAE	3X3X0.1875	36.0	38.58	Comp	38.58	38BX	-4.438NESC Ext	11.503	18.200	20.391	1.000	1.000	1.000	192.66
137.65	15.738	6	H4	SAU	5X3X0.25	36.0	48.03	Comp	48.03	39AY	-8.742NESC Ext	21.560	18.200	27.187	1.000	1.000	1.000	173.07
164.68	9.569	6	H5	SAE	3.5X3.5X0.25	36.0	28.24	Comp	28.24	40XY	-3.780NESC Ext	13.386	18.200	27.187	1.000	1.000	1.000	233.97
22		5	H6	SAU	4X3X0.25	36.0	37.58	Tens	0.00	45BP	0.000	35.407	18.200	27.187	1.000	1.000	1.000	110.60
160.48	9.562	5	D6	SAU	3X2X0.1875	36.0	25.75	Comp	25.75	46Y	-1.992NESC Hea	7.736	18.200	20.391	1.000	0.500	0.500	201.94
24		4	HGR1	SAU	2.5X2X0.1875	36.0	31.56	Tens	0.00	49AY	0.000	5.486	18.200	20.391	1.000	0.500	0.500	205.58
190.09	13.531	6	HGR2	SAE	3X3X0.1875	36.0	31.65	Tens	0.00	48AY	0.000	8.140	18.200	20.391	1.000	0.500	0.500	195.77
25		4	A1	SAE	3X3X0.25	36.0	37.66	Tens	17.67	50XY	-3.216NESC Hea	21.867	18.200	27.187	0.500	0.500	0.500	142.64
0.00	0.000	0	A2	SAU	3.5X3X0.25	36.0	29.59	Comp	29.59	51Y	-9.664NESC Hea	32.661	36.400	54.375	0.500	0.500	0.500	110.71
26		3	A#	SAE	4X4X0.25	36.0	25.24	Comp	25.24	52Y	-10.506NESC Hea	41.627	45.500	67.969	0.500	0.500	0.500	106.21
127.06	4.243	5	H7	SAU	2.5X2X0.1875	36.0	20.02	Comp	20.02	54P	-1.633NESC Hea	8.154	9.100	10.195	1.000	1.000	1.000	168.62
27		4	H8	DAE	1.75X1.75X0.1875	36.0	84.81	Tens	67.24	36P	-11.296NESC Hea	19.743	16.800	20.391	1.000	1.000	1.000	134.08
116.67	8.224	2	Pmnt	Pmnt	12" Std.	42.0	8.70	Comp	8.70	g101P	-22.636NESC Hea	260.146	0.000	0.000	1.000	1.000	1.000	122.32
115.30	6.000	3	PMER1	SAE	2X2X0.1875	36.0	3.67	Comp	3.67	g107P	-0.374NESC Ext	20.042	16.800	10.195	1.000	1.000	1.000	45.71
182.48	14.775	5	PMER2	SAE	2.5X2.5X0.1875	36.0	32.68	Comp	32.68	g111P	-3.332NESC Ext	22.126	16.800	10.195	1.000	1.000	1.000	81.32
28		3	PMER3	SAE	3X3X0.1875	36.0	2.24	Tens	0.00	g114P	0.000	11.823	16.800	10.195	1.000	1.000	1.000	162.44
29		4	PMER4	SAE	3.5X3.5X0.25	36.0	15.25	Comp	15.25	g116X	-2.073NESC Ext	17.249	16.800	13.594	1.000	1.000	1.000	167.46
30		1	PMER5	SAE	4X4X0.25	36.0	8.23	Comp	8.23	g120P	-1.119NESC Ext	16.004	16.800	13.594	1.000	1.000	1.000	186.27
205.58	13.585	4	1 A	1 A potentially damaging moment exists in the following members (make sure your system is well triangulated to minimize moments): g118P ??														
31		1	H1	SAE	1.75X1.75X0.1875	36.0	19.51	Comp	19.51	30AP	-1.776NESC Hea	13.441	9.100	10.195	0.500	0.500	0.500	104.96
32		3	H2	SAE	2X2X0.25	36.0	37.80	Tens	34.46	42XY	-3.136NESC Ext	15.869	9.100	13.594	1.000	1.000	1.000	130.21
137.29	14.073	4	Angler	SAE	2X2X0.25	36.0	37.80	Tens	34.46	42XY	-3.136NESC Ext	15.869	9.100	13.594	1.000	1.000	1.000	130.21
115.36	11.643	3	Bracer	SAE	2.5X2.5X0.25	36.0	58.05	Comp	58.05	g110X	-7.891NESC Ext	29.101	16.800	13.594	1.000	1.000	1.000	81.98
33		3	1 A	1 A potentially damaging moment exists in the following members (make sure your system is well triangulated to minimize moments): g110P ??														
113.11	14.073	3	Plate	Par	6x3/4	36.0	4.84	Comp	4.84	g106P	-0.813NESC Ext	109.423	16.800	0.000	1.000	1.000	1.000	83.19
34		1	1	Bar	6x3/4	36.0	4.84	Comp	4.84	g106P	-0.813NESC Ext	109.423	16.800	0.000	1.000	1.000	1.000	83.19
35		3	1	Bar	6x3/4	36.0	4.84	Comp	4.84	g106P	-0.813NESC Ext	109.423	16.800	0.000	1.000	1.000	1.000	83.19
168.62	6.000	4	1	Bar	6x3/4	36.0	4.84	Comp	4.84	g106P	-0.813NESC Ext	109.423	16.800	0.000	1.000	1.000	1.000	83.19
36		4	1	Bar	6x3/4	36.0	4.84	Comp	4.84	g106P	-0.813NESC Ext	109.423	16.800	0.000	1.000	1.000	1.000	83.19
134.08	6.000	4	1	Bar	6x3/4	36.0	4.84	Comp	4.84	g106P	-0.813NESC Ext	109.423	16.800	0.000	1.000	1.000	1.000	83.19
Pmnt 12" Std.	Pipe	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt	Pmnt
122.32	44.750	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
82.86	1.501	3	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
100.66	3.354	3	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
162.44	8.068	4	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
167.46	9.685	4	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
186.27	12.340	4	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
moments):	g118P	??	??	??	??	??	??	??	??	??	??	??	??	??	??	??	??	??
112.48	6.000	3	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
130.21	4.243	4	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
100.99	3.354	3	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
moments):	g110P	??	??	??	??	??	??	??	??	??	??	??	??	??	??	??	??	??
101.59	1.501	3	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1

Group Summary (Tension Portion):

Group No.	Group Label	Hole Diameter	Group Angle Desc. Type	Angle Size	Steel Strength	Usage		Tension		Control Member	Tension		Net Section	Tension		Tension Length	No. Of Bolts		
						Max	Cont-rol	Max	Use		Force Control	Load Capacity		Shear Connect.	Bearing Connect.			Capacity (kips)	Capacity (kips)
						rol	Use	(ksi)	%		Tens. %	(kips)		(kips)	(kips)			(kips)	(ft)
Holes																			

1	2.000	0.6875	LEG1	SAE	3.5X3.5X0.25	36.0	90.98	Comp	73.56	3X	26.775	NESC	Ext	48.465	36.400	54.375	60.417	7.000	4
2	4.000	0.6875	LEG2	SAE	6X6X0.3125	36.0	87.73	Comp	78.83	5XY	79.193	NESC	Ext	100.462	109.200	203.906	226.562	6.050	12
3	3.500	0.6875	LEG3	SAE	8X8X0.5	36.0	68.57	Comp	66.97	7X	157.846	NESC	Hea	235.687	254.800	380.624	422.916	7.000	14
4	3.500	0.6875	LEG4	SAE	8X8X0.625	36.0	72.74	Comp	67.09	9X	195.772	NESC	Hea	291.819	0.000	0.000	0.000	7.915	0
5	4.000	0.6875	LEG5	SAE	8X8X0.75	36.0	63.48	Comp	57.26	11X	193.318	NESC	Hea	337.589	0.000	0.000	0.000	10.131	0
6	1.000	0.6875	X1	SAE	2.5X2.5X0.1875	36.0	86.45	Comp	75.20	15AP	13.584	NESC	Ext	25.048	18.200	20.391	18.063	10.817	2
7	1.000	0.6875	X2	SAU	2.5X2X0.1875	36.0	85.43	Comp	63.92	16AP	11.586	NESC	Ext	19.030	18.200	20.391	18.125	9.220	2
8	1.740	0.6875	X3	SAU	4X3X0.25	36.0	57.37	Comp	49.96	17AP	22.516	NESC	Hea	45.066	45.500	67.969	60.417	10.000	5
9	1.030	0.6875	X4	SAU	3.5X2.5X0.25	36.0	60.32	Comp	54.98	18AY	20.012	NESC	Hea	40.920	36.400	54.375	42.647	8.521	4
10	1.790	0.6875	X5	SAU	4X3.5X0.3125	36.0	54.28	Comp	54.24	19AP	32.782	NESC	Hea	60.440	63.700	118.945	112.337	9.220	7
11	1.550	0.6875	X6	SAU	5X3.5X0.25	36.0	26.18	Cross	26.16	21BXY	12.023	NESC	Hea	45.962	54.600	81.562	72.500	10.597	6
12	1.000	0.6875	X7	SAU	4X3X0.25	36.0	16.00	Comp	11.18	22BY	4.070	NESC	Hea	41.087	36.400	54.375	44.391	12.246	4
13	1.000	0.6875	X8	SAU	3.5X3X0.25	36.0	8.89	Tens	8.89	23AY	2.427	NESC	Ext	40.925	27.300	40.781	38.516	14.070	3
14	1.000	0.6875	X9	SAU	5X3X0.25	36.0	18.11	Comp	2.95	24AP	0.805	NESC	Hea	41.087	27.300	40.781	36.250	17.450	3
15	1.710	0.6875	D1	SAU	5X3X0.25	36.0	78.17	Tens	78.17	25AY	29.029	NESC	Ext	37.133	63.700	95.156	74.632	14.103	7
16	1.000	0.6875	D2	SAE	2X2X0.1875	36.0	41.26	Tens	41.26	26BX	7.477	NESC	Ext	18.827	18.200	20.391	18.125	16.853	2
17	1.000	0.6875	D3	SAU	3X2X0.25	36.0	65.57	Tens	65.57	27AXY	16.318	NESC	Ext	24.887	36.400	54.375	48.333	19.194	4
18	1.260	0.6875	D4	SAU	4X3X0.25	36.0	63.43	Tens	63.43	28AY	28.862	NESC	Ext	47.739	45.500	67.969	60.417	15.321	5
19	1.000	0.6875	D5	SAU	3.5X3X0.25	36.0	76.25	Tens	76.25	29AXY	34.293	NESC	Ext	44.975	45.500	67.969	53.634	28.523	5
20	1.000	0.6875	H1	SAE	1.75X1.75X0.1875	36.0	51.30	Tens	51.30	33Y	3.603	NESC	Hea	15.911	9.100	10.195	7.024	6.000	1
21	1.000	0.6875	H2	SAU	4X3.5X0.25	36.0	53.92	Comp	14.10	37BP	3.850	NESC	Ext	49.025	27.300	40.781	36.250	15.738	3
22	1.000	0.6875	H3	SAE	3X3X0.1875	36.0	38.58	Comp	5.65	38AXY	1.024	NESC	Ext	31.139	18.200	20.391	18.125	9.569	2
23	1.000	0.6875	H4	SAU	5X3X0.25	36.0	48.03	Comp	47.94	F39C2118X	8.726	NESC	Ext	41.087	18.200	27.187	24.167	2.208	2
	1.000	0.6875																	

1.000	24	0.6875	H5	SAE	3.5X3.5X0.25	36.0	28.24	Comp	21.85	40Y	3.977NESC Ext	49.187	18.200	27.187	25.677	13.531	2
0.000	25	0	X10	SAE	1.75X1.75X0.1875	36.0	0.00	0.00			0.000	0.000	0.000	0.000	0.000	0.000	0
1.000	26	0.6875	X11	SAE	2X2X0.1875	36.0	11.57	Tens	11.57	43AP	1.523NESC Ext	18.827	18.200	20.391	13.170	4.243	2
1.000	27	0.6875	X12	SAU	2.5X2X0.1875	36.0	8.61	Comp	7.97	44AY	1.444NESC Ext	19.030	18.200	20.391	18.125	8.224	2
1.000	28	0.6875	H6	SAU	4X3X0.25	36.0	37.58	Tens	37.58	45P	6.839NESC Hea	41.087	18.200	27.187	24.167	6.000	2
1.000	29	0.6875	D6	SAU	3X2X0.1875	36.0	25.75	Comp	0.00	46Y	0.000	18.908	18.200	20.391	13.577	14.775	2
1.000	30	0.6875	HGR1	SAU	2.5X2X0.1875	36.0	31.56	Tens	31.56	47Y	4.286NESC Hea	19.030	18.200	20.391	13.577	13.250	2
1.000	31	0.6875	HGR2	SAE	3X3X0.1875	36.0	31.65	Tens	31.65	48Y	5.737NESC Hea	31.139	18.200	20.391	18.125	15.022	2
1.590	32	0.6875	A1	SAE	3X3X0.25	36.0	37.66	Tens	37.66	50Y	6.825NESC Hea	37.802	18.200	27.187	18.125	14.073	2
2.000	33	0.6875	A2	SAU	3.5X3X0.25	36.0	29.59	Comp	28.56	53BP	7.797NESC Hea	35.356	27.300	40.781	31.985	5.625	3
2.000	34	0.6875	A#	SAE	4X4X0.25	36.0	25.24	Comp	15.38	52BP	6.996NESC Hea	51.718	45.500	67.969	71.078	6.875	5
1.000	35	0.6875	H7	SAU	2.5X2X0.1875	36.0	20.02	Comp	7.70	54X	0.701NESC Hea	22.067	9.100	10.195	9.629	6.000	1
1.000	36	0.6875	H8	DAE	1.75X1.75X0.1875	36.0	84.81	Tens	84.81	36X	12.606NESC Hea	35.999	16.800	20.391	14.864	6.000	1
0.000	Pwmt 12" Std.	0	Pipe	Pwmt	Pipe 12" Std.	42.0	8.70	Comp	0.00	g121P	0.000	571.199	0.000	0.000	0.000	9.000	0
1.000	EMBR1	0.6875	L2x2x3/16	SAE	2X2X0.1875	36.0	3.67	Comp	0.87	g107P	0.088NESC Hea	18.827	16.800	10.195	10.343	1.501	1
1.000	EMBR2	0.6875	L2.5x2.5x3/16	SAE	2.5X2.5X0.1875	36.0	32.68	Comp	24.43	g111X	2.491NESC Ext	25.048	16.800	10.195	11.328	3.354	1
1.000	EMBR3	0.6875	L3x3x3/16	SAE	3X3X0.1875	36.0	2.24	Tens	2.24	g114P	0.228NESC Ext	31.139	16.800	10.195	11.328	8.068	1
1.000	EMBR4	0.6875	L3.5x3.5x1/4	SAE	3.5X3.5X0.25	36.0	15.25	Comp	13.43	g116P	1.826NESC Ext	49.187	16.800	13.594	15.104	9.685	1
1.000	EMBR5	0.6875	L4x4x1/4	SAE	4X4X0.25	36.0	8.23	Comp	8.07	g120X	1.097NESC Ext	57.287	16.800	13.594	15.104	12.340	1
??	0.6875	A potentially	damaging moment	exists in the following members		make sure your system is well triangulated to minimize moments): g118P											
20a	1.000	0.6875	H1	SAE	1.75X1.75X0.1875	36.0	19.51	Comp	7.89	30BP	0.554NESC Hea	15.911	9.100	10.195	7.024	6.000	1
AngleR	1.000	0.6875	L2x2x1/4	SAE	2X2X0.25	36.0	37.80	Tens	37.80	42X	3.440NESC Ext	24.887	9.100	13.594	9.366	4.243	1
BraceR	1.000	0.6875	L2.5x2.5x1/4	SAE	2.5X2.5X0.25	36.0	58.05	Comp	44.87	g110P	6.100NESC Ext	32.987	16.800	13.594	15.104	3.354	1
1.000	0.6875	A potentially	damaging moment	exists in the following members		make sure your system is well triangulated to minimize moments): g110P											
??	1.000	0.6875	6"x3/4" PL	Bar	6x3/4	36.0	4.84	Comp	2.51	g106P	0.421NESC Hea	145.800	16.800	0.000	0.000	1.501	1

*** Maximum Stress Summary for Each Load Case

Summary of Maximum Usages by Load Case:

Load Case Maximum Element Usage & Label Type

NESC Heavy 87.73 5Y Angle
NESC Extreme 90.98 3P Angle

Summary of Insulator Usages:

Insulator Label	Insulator Type	Insulator Maximum Usage %	Load Case Weight (lbs)
1	Clamp	19.65	NESC Heavy 0.0
2	Clamp	19.51	NESC Heavy 0.0
3	Clamp	20.20	NESC Heavy 0.0
4	Clamp	20.61	NESC Heavy 0.0
5	Clamp	20.20	NESC Heavy 0.0
6	Clamp	20.62	NESC Heavy 0.0
7	Clamp	20.20	NESC Heavy 0.0
8	Clamp	20.61	NESC Heavy 0.0
9	Clamp	29.53	NESC Heavy 0.0
10	Clamp	29.57	NESC Heavy 0.0
11	Clamp	29.53	NESC Heavy 0.0
12	Clamp	1.81	NESC Extreme 0.0
13	Clamp	2.09	NESC Extreme 0.0
14	Clamp	2.32	NESC Heavy 0.0
15	Clamp	4.00	NESC Extreme 0.0
16	Clamp	4.67	NESC Extreme 0.0
17	Clamp	3.69	NESC Heavy 0.0
18	Clamp	3.20	NESC Extreme 0.0
19	Clamp	6.04	NESC Heavy 0.0
20	Clamp	10.21	NESC Heavy 0.0
21	Clamp	10.89	NESC Heavy 0.0
22	Clamp	10.43	NESC Heavy 0.0
23	Clamp	4.98	NESC Heavy 0.0
24	Clamp	5.34	NESC Heavy 0.0
25	Clamp	3.42	NESC Heavy 0.0
26	Clamp	9.14	NESC Extreme 0.0
27	Clamp	16.71	NESC Heavy 0.0
28	Clamp	2.37	NESC Extreme 0.0
29	Clamp	3.08	NESC Heavy 0.0
30	Clamp	2.50	NESC Heavy 0.0
31	Clamp	4.27	NESC Heavy 0.0

Loads At Insulator Attachments For All Load Cases:

Case	Insulator Label	Insulator Type	Structure Attach				Structure Attach			
			Label	Load X (kips)	Load Y (kips)	Load Z (kips)	Label	Load X (kips)	Load Y (kips)	Load Z (kips)
NESC Heavy	1	Clamp	16X	0.000	9.736	1.328	16X	0.000	9.827	1.328
NESC Heavy	2	Clamp	16P	0.000	9.666	1.328	16P	0.000	9.757	1.328
NESC Heavy	3	Clamp	17P	7.000	7.276	0.249	17P	7.000	7.276	0.249
NESC Heavy	4	Clamp	17Y	-7.000	7.178	2.373	17Y	-7.000	7.178	2.373
NESC Heavy	5	Clamp	18P	7.000	7.276	0.278	18P	7.000	7.276	0.278
NESC Heavy	6	Clamp	18Y	-7.000	7.178	2.402	18Y	-7.000	7.178	2.402
NESC Heavy	7	Clamp	19P	7.000	7.276	0.249	19P	7.000	7.276	0.249
NESC Heavy	8	Clamp	19Y	-7.000	7.178	2.373	19Y	-7.000	7.178	2.373
NESC Heavy	9	Clamp	23P	0.000	14.536	2.584	23P	0.000	14.536	2.584
NESC Heavy	10	Clamp	24P	0.000	14.544	2.662	24P	0.000	14.544	2.662

NESC Heavy	11	Clamp	25P	0.000	14.536	2.584	14.764
NESC Heavy	12	Clamp	2Y	0.000	0.176	0.568	0.595
NESC Heavy	13	Clamp	4Y	0.000	0.219	0.915	0.941
NESC Heavy	14	Clamp	6Y	0.000	0.241	1.133	1.158
NESC Heavy	15	Clamp	8Y	0.000	0.253	1.351	1.375
NESC Heavy	16	Clamp	10Y	0.000	0.443	1.783	1.837
NESC Heavy	17	Clamp	12Y	0.000	0.251	1.827	1.844
NESC Heavy	18	Clamp	13Y	0.000	0.228	1.563	1.580
NESC Heavy	19	Clamp	14Y	0.000	0.558	2.970	3.022
NESC Heavy	20	Clamp	30P	0.000	1.210	4.962	5.107
NESC Heavy	21	Clamp	31P	0.000	1.466	5.244	5.445
NESC Heavy	22	Clamp	32P	0.000	1.430	5.016	5.216
NESC Heavy	23	Clamp	33P	0.000	0.665	2.401	2.491
NESC Heavy	24	Clamp	34P	0.000	0.721	2.571	2.670
NESC Heavy	25	Clamp	35P	0.000	0.504	1.635	1.711
NESC Heavy	26	Clamp	36P	0.000	1.328	3.080	3.354
NESC Heavy	27	Clamp	37P	0.000	0.823	8.316	8.356
NESC Heavy	28	Clamp	10XY	0.000	0.422	0.992	1.078
NESC Heavy	29	Clamp	12XY	0.000	0.673	1.384	1.539
NESC Heavy	30	Clamp	13XY	0.000	0.490	1.151	1.251
NESC Heavy	31	Clamp	14XY	0.000	0.837	1.966	2.137
NESC Extreme	1	Clamp	16X	0.361	6.037	0.528	6.071
NESC Extreme	2	Clamp	16P	0.361	6.037	0.528	6.071
NESC Extreme	3	Clamp	17P	4.571	5.431	0.259	7.104
NESC Extreme	4	Clamp	17Y	-3.778	4.427	1.163	5.935
NESC Extreme	5	Clamp	18P	4.571	5.431	0.259	7.104
NESC Extreme	6	Clamp	18Y	-3.778	4.427	1.163	5.935
NESC Extreme	7	Clamp	19P	4.571	5.431	0.259	7.104
NESC Extreme	8	Clamp	19Y	-3.778	4.427	1.163	5.935
NESC Extreme	9	Clamp	23P	0.793	9.595	1.228	9.706
NESC Extreme	10	Clamp	24P	0.793	9.595	1.228	9.706
NESC Extreme	11	Clamp	25P	0.793	9.595	1.228	9.706
NESC Extreme	12	Clamp	2Y	0.000	0.843	0.323	0.903
NESC Extreme	13	Clamp	4Y	0.000	0.985	0.354	1.047
NESC Extreme	14	Clamp	6Y	0.000	1.056	0.370	1.119
NESC Extreme	15	Clamp	8Y	0.000	1.786	0.905	2.002
NESC Extreme	16	Clamp	10Y	0.000	2.139	0.931	2.333
NESC Extreme	17	Clamp	12Y	0.000	1.515	0.709	1.673
NESC Extreme	18	Clamp	13Y	0.000	1.440	0.692	1.598
NESC Extreme	19	Clamp	14Y	0.000	2.526	0.932	2.693
NESC Extreme	20	Clamp	30P	0.000	1.913	1.337	2.335
NESC Extreme	21	Clamp	31P	0.000	1.814	1.272	2.216
NESC Extreme	22	Clamp	32P	0.000	2.059	1.454	2.520
NESC Extreme	23	Clamp	33P	0.000	0.791	0.544	0.960
NESC Extreme	24	Clamp	34P	0.000	0.828	0.569	1.005
NESC Extreme	25	Clamp	35P	0.000	0.565	0.395	0.689
NESC Extreme	26	Clamp	36P	0.000	4.390	1.263	4.568
NESC Extreme	27	Clamp	37P	0.000	2.822	3.667	4.627
NESC Extreme	28	Clamp	10XY	0.000	0.972	0.675	1.184
NESC Extreme	29	Clamp	12XY	0.000	0.971	0.588	1.136
NESC Extreme	30	Clamp	13XY	0.000	0.933	0.580	1.099
NESC Extreme	31	Clamp	14XY	0.000	1.291	0.659	1.450

Overturning Moments For User Input Concentrated Loads:

Moments are static equivalents based on central axis of 0.0 (i.e. a single pole).

Load Case	Total Tran.	Total Long.	Total Transverse	Longitudinal Torsional
			Vert. Overturning	Moment

	Load (kips)	Load (kips)	Load (kips)	Moment (ft-k)	Moment (ft-k)	Moment (ft-k)
NESC Heavy	112.174	0.000	46.731	11605.566	-38.940	-14.573
NESC Extreme	88.291	5.480	16.818	8855.018	551.181	-38.601
*** Weight of structure (lbs):						
Weight of Angles*Section DLF:				38857.7		
Total:				38857.7		

*** End of Report

Tower Anchor Bolt AnalysisMax Leg Reactions:

Uplift = Uplift := 200.63 kips (User Input)

Shear = Shear := 56.57 kips (User Input)

Compression = Compression := 226.21 kips (User Input)

Anchor Bolt Data:

Use ASTM A36 (Assumed Conservative Value - Actual Grade Unknown)

Number of Anchor Bolts = N := 4 (User Input)

Bolt Ultimate Strength = $F_u := 58 \text{ ksi}$ (User Input)Bolt Yield Strength = $F_y := 36 \text{ ksi}$ (User Input)

Diameter of Bolts = D := 2.0 in (User Input)

Threads per Inch = n := 4.5 (User Input)

Coefficient of Friction = $\mu := 0.55$ (User Input)Anchor Bolt Area:Gross Area of Bolt = $A_g := \frac{\pi}{4} \cdot D^2 = 3.142 \cdot \text{in}^2$ Net Area of Bolt = $A_n := \frac{\pi}{4} \cdot \left(D - \frac{0.9743 \cdot \text{in}}{n} \right)^2 = 2.498 \cdot \text{in}^2$ (AISC 13th Ed. pg. 7-83)

Subject:

Anchor Bolt Analysis for CL&P Tower # 1281

Location:

Greenwich, CT

Rev. 0: 12/19/17

 Prepared by: T.J.L. Checked by: C.F.C.
 Job No. 16009.03
Check Tensile Force:

Maximum Tensile Force (Gross Area) =

$$F_{\text{gross.area}} := 1.0 \cdot (0.33 \cdot A_g \cdot F_u) = 60.1 \text{ kips}$$

Maximum Tensile Force (Net Area) =

$$F_{\text{net.area}} := 1.0 \cdot (0.60 \cdot A_n \cdot F_y) = 54 \text{ kips}$$

Allowable Tension =

$$\text{AllowableTension} := \begin{cases} F_{\text{gross.area}} & \text{if } F_{\text{gross.area}} < F_{\text{net.area}} \\ F_{\text{net.area}} & \text{if } F_{\text{net.area}} < F_{\text{gross.area}} \end{cases}$$

$$\text{AllowableTension} = 54 \text{ kips}$$

Applied Tension =

$$\text{MaxTension} := \frac{\text{Uplift}}{N} = 50.16 \text{ kips}$$

$$\frac{\text{MaxTension}}{F_{\text{net.area}}} = 93.0\%$$

$$\text{Condition1} := \text{if} \left(\frac{\text{MaxTension}}{F_{\text{net.area}}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition1 = "OK"

Check Anchor Bolt Area:

Based on the ASCE 10-97 Design of Latticed Steel Transmission Structures

Required Area =

$$A_{s1} := \frac{\text{Uplift}}{F_y} + \frac{\text{Shear}}{\mu \cdot 85 \cdot F_y} = 8.9 \text{ in}^2$$

$$A_{s2} := \left[\frac{\text{Shear} - (0.3 \cdot \text{Compression})}{\mu \cdot 85 \cdot F_y} \right] = -0.671 \text{ in}^2$$

Provided Area =

$$A_{s\text{provided}} := A_n \cdot N = 10 \text{ in}^2$$

$$\text{Condition2} := \text{if} \left(\frac{A_{s1}}{A_{s\text{provided}}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition2 = "OK"

$$\text{Condition3} := \text{if} \left(\frac{A_{s2}}{A_{s\text{provided}}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition3 = "OK"

Foundation:**Input Data:**Tower Data

Shear (Compression Leg) =	$\text{Shear}_{\text{comp}} := 46.7 \cdot 1.1 \cdot \text{kips} = 51.4 \cdot \text{kips}$	(User Input from PLS Tower)
Shear (Uplift Leg) =	$\text{Shear}_{\text{up}} := 56.8 \cdot 1.1 \cdot \text{kips} = 62.3 \cdot \text{kips}$	(User Input from PLS Tower)
Compression =	$\text{Comp} := 226.2 \cdot 1.1 \cdot \text{kips} = 248.8 \cdot \text{kips}$	(User Input from PLS Tower)
Uplift =	$\text{Uplift} := 200.6 \cdot 1.1 \cdot \text{kips} = 220.7 \cdot \text{kips}$	(User Input from PLS Tower)
Tower Height =	$H_t := 129 \cdot \text{ft}$	(User Input)

Footing Data:

Depth to Bottom of Footing =	$D_f := 8 \cdot \text{ft}$	(User Input)
Length of Pier =	$L_p := 8.5 \cdot \text{ft}$	(User Input)
Extension of Pier Above Grade =	$L_{\text{pag}} := 0.5 \cdot \text{ft}$	(User Input)
Width of Pier =	$W_p := 5 \cdot \text{ft}$	(User Input)
Depth of Soil =	$D_{\text{soil}} := 8 \cdot \text{ft}$	(User Input)
Depth of Rock =	$D_{\text{rock}} := 12 \cdot \text{ft}$	(User Input)

Material Properties:

Concrete Compressive Strength =	$f_c := 3500 \cdot \text{psi}$	(User Input)
Steel Reinforcement Yield Strength =	$f_y := 60000 \cdot \text{psi}$	(User Input)
Anchor Bolt Yield Strength =	$f_{ya} := 75000 \cdot \text{psi}$	(User Input)
Internal Friction Angle of Soil =	$\phi_s := 30 \cdot \text{deg}$	(User Input)
Allowable Soil Bearing Capacity =	$q_s := 4000 \cdot \text{psf}$	(User Input)
Allowable Rock Bearing Capacity =	$q_{\text{rock}} := 50000 \cdot \text{psf}$	(User Input)
Unit Weight of Soil =	$\gamma_{\text{soil}} := 100 \cdot \text{pcf}$	(User Input)
Unit Weight of Concrete =	$\gamma_{\text{conc}} := 150 \cdot \text{pcf}$	(User Input)
Unit Weight of Rock =	$\gamma_{\text{rock}} := 160 \cdot \text{pcf}$	(User Input)
Foundation Bouyancy =	$\text{Bouyancy} := 0$	(User Input) (Yes=1 / No=0)
Depth to Neglect =	$n := 1.0 \cdot \text{ft}$	(User Input)
Cohesion of Clay Type Soil =	$c := 0 \cdot \text{ksf}$	(User Input) (Use 0 for Sandy Soil)
Seismic Zone Factor =	$Z := 2$	(User Input) (UBC-1997 Fig 23-2)
Coefficient of Friction Between Concrete =	$\mu := 0.45$	(User Input)

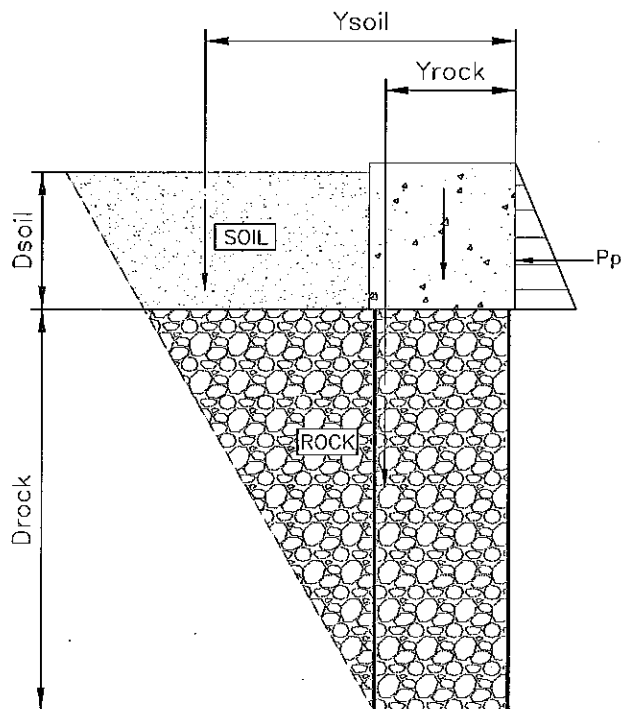
Rock Anchor Properties:

ASTM A615 Grade 60

Bolt Ultimate Strength =	$F_u := 90\text{-ksi}$	(User Input)	
Bolt Yield Strength =	$F_y := 60\text{-ksi}$	(User Input)	
Anchor Diameter =	$d_{ra1} := 1.128\text{-in}$	(User Input)	(1 #9 and 1 #11 per Rock Group)
Anchor Diameter =	$d_{ra2} := 1.41\text{-in}$	(User Input)	
Hole Diameter =	$d_{hole} := 4\text{-in}$	(User Input)	
Grout Strength =	$\tau := 120\text{-psi}$	(User Input)	
Distance to Rock Anchor Group 1 =	$D_{a1} := 24\text{-in}$	(User Input)	
Number of Rock Anchors in Group 1 =	$N_{a1} := 6$	(User Input)	
Total Number of Rock Bolts =	$N_{atot} := 8$	(User Input)	

Check Uplift:

Adjusted Concrete Unit Weight =	$\gamma_c := \text{if}(\text{Bouyancy} = 1, \gamma_{conc} - 62.4\text{pcf}, \gamma_{conc}) = 150\text{-pcf}$
Adjusted Soil Unit Weight =	$\gamma_s := \text{if}(\text{Bouyancy} = 1, \gamma_{soil} - 62.4\text{pcf}, \gamma_{soil}) = 100\text{-pcf}$
Weight of Concrete =	$WT_c := (W_p^2 \cdot L_p) \cdot \gamma_c = 31.875\text{-kip}$
BaseArea 1 of Resisting Pyramid =	$B_1 := (D_{a1} \cdot 2)^2 = 16\text{ft}^2$
BaseArea 2 of Resisting Pyramid =	$B_2 := [\tan(\Phi_s) \cdot (D_{rock} \cdot 0.5) \cdot 2 + D_{a1} \cdot 2]^2 = 119.4\text{ft}^2$
BaseArea 3 of Resisting Pyramid =	$B_3 := [\tan(\Phi_s) \cdot (D_{rock} \cdot 0.5 + D_{soil}) \cdot 2 + D_{a1} \cdot 2]^2 = 406.7\text{ft}^2$
Weight of Soil =	$WT_{soil} := \left[\frac{D_{soil}}{3} \cdot (B_2 + B_3 + \sqrt{B_2 \cdot B_3}) - W_p^2 \cdot L_p \right] \cdot \gamma_s = 177.806\text{-kip}$
Weight of Rock =	$WT_{rock} := \left[\frac{D_{rock} \cdot 0.5}{3} \cdot (B_1 + B_2 + \sqrt{B_1 \cdot B_2}) \right] \cdot \gamma_{rock} = 57.324\text{-kip}$
Total Resistance =	$WT_{tot} := WT_c + WT_{rock} + WT_{soil} = 267\text{-kips}$
Factor of Safety/Actual =	$FS := \frac{WT_{tot}}{\text{Uplift}} = 1.21$
Factor of Safety Required =	$FS_{req} := 1.0$
	$\text{Uplift_Check} := \text{if}(FS \geq FS_{req}, \text{"Okay"}, \text{"No Good"})$
	$\text{Uplift_Check} = \text{"Okay"}$



$$\text{Area 1} = A1 := \frac{1}{2} \cdot \tan(\Phi_s) \cdot D_{\text{soil}}^2 = 18.475 \text{ ft}^2 \quad \text{sf}$$

$$\text{Area 2} = A2 := \tan(\Phi_s) \cdot D_{\text{rock}} \cdot D_{\text{soil}} = 55.426 \text{ ft}^2 \quad \text{sf}$$

$$\text{Distance to Centroid 1} = Y1 := \tan(\Phi_s) \cdot D_{\text{rock}} + \frac{1}{3} \cdot \tan(\Phi_s) \cdot D_{\text{soil}} = 8.468 \text{ ft} \quad \text{ft}$$

$$\text{Distance to Centroid 2} = Y2 := \frac{1}{2} \cdot \tan(\Phi_s) \cdot D_{\text{rock}} = 3.464 \text{ ft} \quad \text{ft}$$

$$\text{Distance from Toe to Centroid of Soil} = Y_{\text{soil}} := \frac{(A1 \cdot Y1 + A2 \cdot Y2)}{(A1 + A2)} + W_p = 9.72 \text{ ft} \quad \text{ft}$$

$$\text{Area 3} = A3 := \frac{1}{2} \cdot \tan(\Phi_s) \cdot D_{\text{rock}}^2 = 41.569 \text{ ft}^2 \quad \text{sf}$$

$$\text{Area 4} = A4 := W_p \cdot D_{\text{rock}} = 60 \text{ ft}^2 \quad \text{sf}$$

$$\text{Distance to Centroid 3} = Y3 := W_p + \frac{1}{3} \cdot \tan(\Phi_s) \cdot D_{\text{rock}} = 7.309 \text{ ft} \quad \text{ft}$$

$$\text{Distance to Centroid 4} = Y4 := \frac{W_p}{2} = 2.5 \text{ ft} \quad \text{ft}$$

$$\text{Distance from Toe to Centroid of Rock} = Y_{\text{rock}} := \frac{(A3 \cdot Y3 + A4 \cdot Y4)}{(A3 + A4)} = 4.47 \text{ ft} \quad \text{ft}$$

Check Overtuning:

Coefficient of Lateral Soil Pressure =

$$K_p := \frac{1 + \sin(\Phi_s)}{1 - \sin(\Phi_s)} = 3$$

Passive Pressure =

$$P_{top} := 0 = 0 \cdot \text{ksf}$$

$$P_{bot} := K_p \cdot \gamma_s \cdot D_f + c \cdot 2 \cdot \sqrt{K_p} = 2.4 \cdot \text{ksf}$$

$$P_{ave} := \frac{P_{top} + P_{bot}}{2} = 1.2 \cdot \text{ksf}$$

$$A_p := W_p \cdot (L_p - L_{pag}) = 40 \text{ ft}^2$$

Ultimate Shear =

$$S_u := P_{ave} \cdot A_p = 48 \cdot \text{kip}$$

Weight of Concrete Pad =

$$WT_c := (W_p^2 \cdot L_p) \cdot \gamma_c = 31.875 \cdot \text{kip}$$

Weight of Soil Wedge at Back Face Corners =

$$WT_{s2} := 2 \cdot \left[(D_{soil})^3 \cdot \frac{\tan(\Phi_s)}{3} \right] \cdot \gamma_s = 19.707 \cdot \text{kips}$$

Total Weight of Soil =

$$WT_{Stot} := (A1 + A2) \cdot W_p \cdot \gamma_s + WT_{s2} = 56.7 \cdot \text{kips}$$

Total Weight of Rock =

$$WT_{Rtot} := (A3 + A4) \cdot W_p \cdot \gamma_{rock} = 81.3 \cdot \text{kips}$$

Resisting Moment =

$$M_r := (WT_c) \cdot \frac{W_p}{2} + S_u \cdot \frac{L_p}{3} + WT_{Stot} \cdot Y_{soil} + WT_{Rtot} \cdot Y_{rock} = 1129 \cdot \text{kip} \cdot \text{ft}$$

Overtuning Moment =

$$M_{ot} := \text{Uplift} \cdot \frac{W_p}{2} + \text{Shear}_{up} \cdot L_p = 1081 \cdot \text{kip} \cdot \text{ft}$$

Factor of Safety Actual =

$$FS := \frac{M_r}{M_{ot}} = 1.04$$

Factor of Safety Required =

$$FS_{req} := 1.0$$

$$\text{OverTurning_Moment_Check} := \text{if}(FS \geq FS_{req}, \text{"Okay"}, \text{"No Good"})$$

$$\text{OverTurning_Moment_Check} = \text{"Okay"}$$

Check Bearing Pressure:

Area of the Pier =

$$A_{mat} := W_p^2 = 25 \text{ ft}^2$$

Section Modulus of Pier =

$$S := \frac{W_p^3}{6} = 20.83 \text{ ft}^3$$

Maximum Bearing Pressure =

$$P_{max} := \frac{WT_c + Comp}{A_{mat}} + \frac{Shear_{comp} \cdot L_p}{S} = 32.187 \text{ ksf}$$

$$Max_Pressure_Check := \text{if}(P_{max} < q_{rock}, \text{"Okay"}, \text{"No Good"})$$

$$Max_Pressure_Check = \text{"Okay"}$$

Check Rock Anchors:

RockAnchor Check:

Polar Moment of Inertia =

$$I_p := (D_{a1}^2 \cdot N_{a1}) = 3456 \text{ in}^2$$

Maximum Tension Force =

$$T_{Max} := \frac{Uplift}{N_{atot}} + \frac{Shear_{up} \cdot L_p \cdot D_{a1}}{I_p} - \frac{WT_c}{N_{atot}} = 67.7 \text{ kips}$$

Gross Area of Bolt Group =

$$A_g := \frac{\pi}{4} \cdot (d_{ra1}^2 + d_{ra2}^2) = 2.561 \text{ in}^2$$

Allowable Tension =

$$T_{all} := A_g \cdot F_y = 153.6 \text{ kips}$$

$$\frac{T_{Max}}{T_{all}} = 44.1\%$$

$$Condition1 := \text{if}(T_{Max} < T_{all}, \text{"OK"}, \text{"NG"})$$

$$Condition1 = \text{"OK"}$$

Check Bond Strength:

Bond Strength =

$$Bond_Strength := d_{Hole} \cdot \pi \cdot (D_{rock}^{0.5}) \cdot \tau = 109 \text{ kips}$$

$$\frac{T_{Max}}{Bond_Strength} = 62.4\%$$

$$Condition2 := \text{if}(T_{Max} < Bond_Strength, \text{"OK"}, \text{"NG"})$$

$$Condition2 = \text{"OK"}$$

Emissions Analysis

Carrier/ Transmission Mode	Centerline Height (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm ²)	Standard Limits (mW/cm ²)	Percent of Limit
Sprint	148	1900	4	693	0.0455	1.0000	4.55
Sprint	148	850	1	390	0.0064	0.5667	1.13
Verizon PCS	139	1970	1	1470	0.0274	1.0000	2.74
Verizon Cellular	139	869	9	290	0.0486	0.5793	8.38
Verizon AWS	139	2145	1	1750	0.0326	1.0000	3.26
Verizon 700	139	698	1	523	0.0097	0.4653	2.09
AT&T UMTS	70	800	1	500	0.0367	0.5867	6.25
AT&T LTE	70	700	1	500	0.0367	0.4667	7.86
AT&T LTE	70	1900	1	500	0.0367	1.0000	3.67
Total							39.95%

* Sprint and Verizon information obtained from Siting Council records.