



**Crown Castle**  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065

October 4, 2016

Melanie A. Bachman  
Acting Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

**RE: Notice of Exempt Modification for AT&T/ LTE 3C Crown Site BU: 876310**  
**AT&T Site ID: CT2154**  
**925 East Center Street, Wallingford, CT 06492**  
**Latitude: 41° 26' 37.36" / Longitude: -72° 47' 46.56"**

Dear Ms. Bachman:

AT&T currently maintains nine (9) antennas at the 112-foot level of the existing 148-foot monopole tower at 925 East Center Street in Wallingford, CT. The tower is owned by Crown Castle. The property is owned by Albert William Beaumont. AT&T now intends to remove three (3) Powerwave antennas and replace three (3) KMW antennas with CCI antennas. These antennas would be installed at the 112-foot level of the tower. AT&T also intends to add three (3) RRU12/A2s and remove three (3) RRU11s, six (6) coax, twelve (12) TMS's and six (6) diplexers.

This facility was approved by the Wallingford Planning & Zoning Commission on September 8, 1997. This approval was given without conditions.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.S.C.A. § 16-50j-73, a copy of this letter is being sent to The Honorable William W. Dickinson, Jr., Mayor, Town of Wallingford, as well as the property owner, and Crown Castle is the tower owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.

**The Foundation for a Wireless World.**  
**CrownCastle.com**

Melanie A. Bachman

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5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, AT&T respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Jeffrey Barbadora.

Sincerely,

Jeffrey Barbadora  
Real Estate Specialist  
12 Gill Street, Suite 5800, Woburn, MA 01801  
781-729-0053  
[Jeff.Barbadora@crowncastle.com](mailto:Jeff.Barbadora@crowncastle.com)

Attachments:

- Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes
- Tab 2: Exhibit-2: Structural Modification Report
- Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Mr. William W. Dickinson, Jr., Mayor  
Town of Wallingford  
45 South Main Street, Room 310  
Wallingford, CT 06492

Albert William Beaumont  
925 East Center Street  
Wallingford, CT 06492

# HARRIS BEACH & WILCOX

A LIMITED LIABILITY PARTNERSHIP

ATTORNEYS AT LAW

147 NORTH BROAD STREET  
PO. BOX 112  
MILFORD, CONNECTICUT 06460-0112  
(203) 877-8000  
(203) 878-9600 (FAX)

## MEMO

To : Steve Paisner, Sprint Spectrum L.P.  
From : Lewis A. Hurwitz, Esq., Harris Beach & Wilcox  
Date : 9/9/97  
Re : Wallingford, Sites 008 and 009  
cc : Steve Crotty, Steve Kotfila, Christine Rosenthal, Jennifer Charland,  
Scott Chasse, Kate Peabody, Tom Flynn

Please be advised that on September 8, 1997, the Wallingford Zoning Board approved our applications to construct monopoles at Beaumont's Farm and the Suzio property. There were conditions in regard to the Beaumont Farm application, details of which will be supplied in the letter of approval. However, it should be noted that a second row of 20' trees is being required. In addition, the Board reserved the right to inform us as to what color to paint the tower. We should have a response from them in a very short period of time in regard to this issue.

This was a very difficult and hard fight. The Beaumont Farm vote was three to two and without the conditions I do not believe we would have prevailed.

End of Memo

— AFFILIATES —  
LIVORNO  
LONDON

PARIS  
OSLO

WASHINGTON, DC  
MILFORD, CT  
HACKENSACK, NJ

— ALBANY —  
BUFFALO

— NEW YORK —  
ITHACA  
NEW YORK CITY

— ROCHESTER —  
SYRACUSE

## **Kotfila, Steve**

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**From:** Chasse, Scott  
**Sent:** Monday, September 29, 1997 7:57 AM  
**To:** Rosenthal, Christine  
**Cc:** Kotfila, Steve  
**Subject:** RE: 008 Lawsuit  
**Importance:** High

PostHC is not necessary at this time. Lets accumulate information first.

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**From:** Rosenthal, Christine  
**Sent:** Monday, September 29, 1997 7:47 AM  
**To:** Chasse, Scott  
**Cc:** Charland, Jennifer  
**Subject:** RE: 008 Lawsuit

At the public hearing, the change in the *application* was cited as being our agreement to maintain the trees as screening. Lew Hurwitz pointed out right then and there that that was a private matter and did not affect our *application* as it stood. Is that was you are asking about? You should bring Lew in on dissecting the lawsuit because he would know how defensible each point is. I don't think that we changed the site plan until after the entire application was heard and decided upon. Confirm with Jenn. Shall I organize a Post-Hearing Conference on this one?

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**From:** Chasse, Scott  
**Sent:** Monday, September 29, 1997 7:40 AM  
**To:** Charland, Jennifer; Flynn, Tom; Johnson, Karen; Knuff, John; Rosenthal, Christine  
**Subject:** 008 Lawsuit  
**Importance:** High

One of the items in the lawsuit against us states that our site plan was changed at the Sept. 8, 1997 hearing. Was this due to the tower foundation size being larger than expected and therefore, necessitating that we move the tower within the compound? If so, at who's direction was this done **prior** to getting the zoning approval? Something as mundane as moving the tower within the compound should have waited until after the approval, then **amend** the site plans of record to conform with the realities of construction.



MEMORANDUM

TO: JEN CHARLAND  
FROM: TOM FLYNN *[Handwritten signature]*  
RE: 03-008 BEAUMONT FARM  
DATE: SEPT. 10, 1997

AS YOU KNOW, THE ABOVE NOTED SPECIAL PERMIT APPLICATION WAS APPROVED ON MONDAY, SEPTEMBER 8, 1997. THERE ARE SEVERAL CONDITIONS THAT WILL EFFECT THE PROCESS OF OBTAINING A BUILDING PERMIT.

1. THE TOWN HAS REQUIRED A \$1000.00 SEDIMENTATION AND EROSION CONTROL BOND. THIS BOND MAY BE IN THE FORM OF CASH, A SURETY BOND OR LETTER OF CREDIT, WHICHEVER IS MOST CONVENIENT FOR SPRINT TO OBTAIN.
2. THE TOWN HAS REQUIRED A REVISED LANDSCAPE PLAN THAT SHOWS A LINE OF EVERGREEN TREES ( 3 DIFFERENT SPECIES AND AT LEAST 20' TALL AT PLANTING) ON THE PERIMETER OF THE LEASE AREA.
3. I WILL NEED A MYLAR FOR RECORDING AND 5 CLEAN COPIES OF THE PLANS, INCLUDING THE REVISED LANDSCAPE PLANS, FOR DELIVERY TO THE PLANNING OFFICE PRIOR TO ISSUANCE OF THE BUILDING PERMIT.
4. WE CAN MAKE APPLICATION FOR THE BUILDING PERMIT PRIOR THE END OF THE APPEAL PERIOD, BUT WILL NEED THE ABOVE NOTED ITEMS PRIOR TO ANY SIGN OFF BY THE PZC.

CVO

SPRINT PCS 9 BARNES INDUSTRIAL ROAD WALLINGFORD, CT. 06429 203-294-5620

## **Kotfila, Steve**

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**From:** Paisner, Steven  
**Sent:** Thursday, November 20, 1997 2:04 PM  
**To:** Lindblad, Ernest; Kotfila, Steve  
**Subject:** Couple of Items

I spoke to Lew Hurwitz...

- 1). WESTBROOK - He agrees that any attorney that tries to exclude another attorney (i.e. Westbrook not allowing Lew to attend today) is up to something no good and non attendance is the way to go.
- 2). WALLINGFORD - Beaumont appeal. KC agrees to stall as long as necessary and make it as tough as possible/expensive on the citizen appeal. Hence, Lew has filed to transfer the case from Superior court to Federal District court...probably the first of several such requests. In the meantime, we are on the air...Ok aside from having to pay more legal bills ourselves.

## **Kotfila, Steve**

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**From:** Kotfila, Steve  
**Sent:** Friday, May 22, 1998 7:05 AM  
**To:** Carrozzella, Bill  
**Cc:** Cashin, Julie; Gelinis, Chris  
**Subject:** RE: Wallingford # 008 - Beaumont  
**Importance:** High

So long as this language does not prohibit us from doing a structural replacement. For that to take place there will be a short period where there would be 2 towers in the compound, but only long enough to effect the swap over of antennas, pulling of a demo permit and dismantling of the old tower. 90 days should cover this evolution.

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**From:** Carrozzella, Bill  
**Sent:** Thursday, May 21, 1998 11:03 AM  
**To:** Kotfila, Steve  
**Cc:** Cashin, Julie; Gelinis, Chris  
**Subject:** Wallingford # 008 - Beaumont

In my discussions with Bill Beaumont he has requested that Sprint not install a second tower within the lease area. Please let me know if it is OK to agree to that.

If we do I will have included in the lease amendment that Sprint still retains the right to replace the existing tower even with a taller tower.

Please advise.

# Memo

**To:** Julie Cashin  
**From:** Bill Carrozzella  
**CC:** C. Gelinas; S. Kotfila  
**Date:** May 20, 1998  
**Re:** Wallingford - Beaumont Farm # 008

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Julie, I have reviewed the Owner Consent and Lease Amendment for the Bell/SNET sublease. Can you please make the following revisions:

**Owner Consent:**

Can the references (and exhibits) to the BANM and SNET Subleases be eliminated? These subleases may not be signed for several weeks or months in the case of Bell. I would like to have the landlord consent finalized ASAP so the additional rent does not go up while we wait for the Bell and SNET agreements to be signed.

**Amendment to Lease:**

Please add a temporary construction easement to this amendment. Bell and SNET may have to access the site for construction over other land owned by landlord as opposed to the existing access easement afforded Sprint. I'd suggest making this temp construction easement broad such as "Landlord shall grant to Subtenants a temporary access easement for the purposes of installing its equipment. This easement shall allow access to the Site through Landlord's adjacent land surrounding the Site in an area to be mutually agreed upon by Landlord and each Subtenant."

Please let me know if you have any questions.

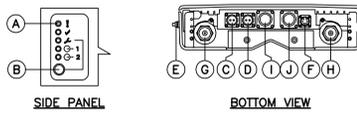
Thanks for your help.



Crown Castle | Earthstar Geographics | Esri, HERE, DeLorme, NGA, USGS, NPS

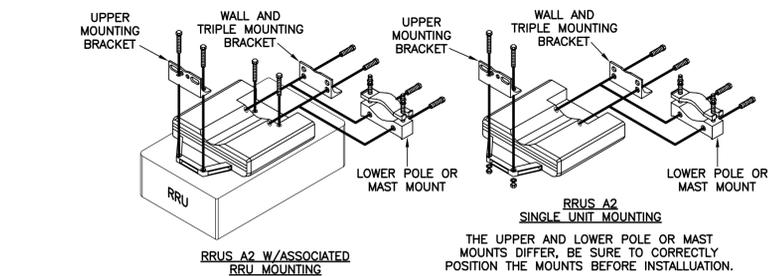
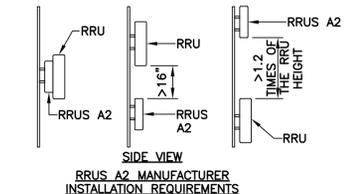
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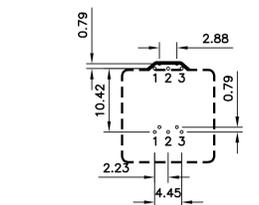


POSITION (ID)	DESCRIPTION	MARKING
A	OPTICAL INDICATORS	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100
B	MAINTENANCE	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100
C	-48V DC POWER SUPPLY	POW IN
D	-48V DC POWER SUPPLY TO RRU	POW OUT
E	GROUNDING	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100
F	RET	RET
G	ANTENNA B	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100
H	ANTENNA A	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100
I	OPTICAL CABLE 1	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100
J	OPTICAL CABLE 2	1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100

- NOTES:**
1. STACKING OF RRU'S IS NOT PERMITTED.
  2. NO PAINTING OF RRU OR THE SOLAR SHIELD IS ALLOWED.
  3. A SINGLE RRU A2 CAN BE INSTALLED AS A STAND ALONE UNIT OR MOUNTED TO THE BACK OF ITS ASSOCIATED RRU.



**1** ERICSSON RRU A2 DETAILS  
N-1 NOT TO SCALE



THE NUMBER OF BOLT HOLES DEPENDS ON THE WALL MATERIAL AS SPECIFIED BY THE SITE ENGINEER. A MINIMUM OF TWO BOLT HOLES ARE RECOMMENDED FOR EACH BRACKET.

ONE OF THE FOLLOWING SOLUTIONS FOR HOLE POSITIONS MUST BE USED:

- 1, 3
- 1, 2, 3

**NOTES AND SPECIFICATIONS**

**DESIGN BASIS:**

- GOVERNING CODE: 2003 INTERNATIONAL BUILDING (IBC) AS MODIFIED BY THE 2005 CT STATE BUILDING CODE AND 2009 AMENDMENTS.
1. DESIGN CRITERIA:
    - WIND LOAD: PER EIA/TIA 222 F-96 (ANTENNA MOUNTS): 85 MPH (FASTEST MILE), EQUIVALENT TO 105 MPH (3 SECOND GUST)
    - BUILDING CLASSIFICATION: II (BASED ON IBC TABLE 1604.5)
    - BASIC WIND SPEED (OTHER STRUCTURE): 105 MPH (3 SECOND GUST) (EXPOSURE B/IMPORTANCE FACTOR 1.0 BASED ON ASCE 7-02) PER 2003 INTERNATIONAL BUILDING CODE (IBC) AS MODIFIED BY THE 2005 CONNECTICUT SUPPLEMENT AND 2009 AMMENDMENT.
    - SEISMIC LOAD (DOES NOT CONTROL): PER ASCE 7-02 MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES.

**GENERAL NOTES:**

1. ALL CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE GOVERNING BUILDING CODE.
2. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
3. BEFORE BEGINNING THE WORK, THE CONTRACTOR IS RESPONSIBLE FOR MAKING SUCH INVESTIGATIONS CONCERNING PHYSICAL CONDITIONS (SURFACE AND SUBSURFACE) AT OR CONTIGUOUS TO THE SITE WHICH MAY AFFECT PERFORMANCE AND COST OF THE WORK.
4. DIMENSIONS AND DETAILS SHALL BE CHECKED AGAINST EXISTING FIELD CONDITIONS.
5. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS, SLEEVES AND ANCHOR BOLTS AS REQUIRED BY ALL TRADES.
6. ALL DIMENSIONS, ELEVATIONS, AND OTHER REFERENCES TO EXISTING STRUCTURES, SURFACE, AND SUBSURFACE CONDITIONS ARE APPROXIMATE. NO GUARANTEE IS MADE FOR THE ACCURACY OR COMPLETENESS OF THE INFORMATION SHOWN. THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS, ELEVATIONS, ANGLES WITH EXISTING CONDITIONS AND WITH ARCHITECTURAL AND SITE DRAWINGS BEFORE PROCEEDING WITH ANY WORK.
7. AS THE WORK PROGRESSES, THE CONTRACTOR SHALL NOTIFY THE OWNER OF ANY CONDITIONS WHICH ARE IN CONFLICT OR OTHERWISE NOT CONSISTENT WITH THE CONSTRUCTION DOCUMENTS AND SHALL NOT PROCEED WITH SUCH WORK UNTIL THE CONFLICT IS SATISFACTORILY RESOLVED.
8. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PROVIDING AND MAINTAINING ADEQUATE SHORING, BRACING, AND BARRICADES AS MAY BE REQUIRED FOR THE PROTECTION OF EXISTING PROPERTY, CONSTRUCTION WORKERS, AND FOR PUBLIC SAFETY.
9. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING SITE OPERATIONS, COORDINATE WORK WITH NORTHEAST UTILITIES
10. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER FOUNDATION REMEDIATION WORK IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, GUYS OR TIEDOWNS, WHICH MIGHT BE NECESSARY.
11. ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
12. SHOP DRAWINGS, CONCRETE MIX DESIGNS, TEST REPORTS, AND OTHER SUBMITTALS PERTAINING TO STRUCTURAL WORK SHALL BE FORWARDED TO THE OWNER FOR REVIEW BEFORE FABRICATION AND/OR INSTALLATION IS MADE. SHOP DRAWINGS SHALL INCLUDE ERECTION DRAWINGS AND COMPLETE DETAILS OF CONNECTIONS AS WELL AS MANUFACTURER'S SPECIFICATION DATA WHERE APPROPRIATE. SHOP DRAWINGS SHALL BE CHECKED BY THE CONTRACTOR AND BEAR THE CHECKER'S INITIALS BEFORE BEING SUBMITTED FOR REVIEW.
13. NO DRILLING WELDING OR TAPING ON CL&P OWNED EQUIPMENT.
14. REFER TO DRAWING T1 FOR ADDITIONAL NOTES AND REQUIREMENTS.

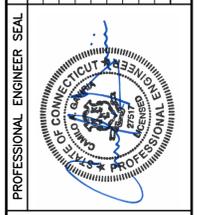
**STRUCTURAL STEEL**

1. ALL STRUCTURAL STEEL IS DESIGNED BY ALLOWABLE STRESS DESIGN (ASD)
  - A. STRUCTURAL STEEL (W SHAPES)---ASTM A992 (FY = 50 KSI)
  - B. STRUCTURAL STEEL (OTHER SHAPES)---ASTM A36 (FY = 36 KSI)
  - C. STRUCTURAL HSS (RECTANGULAR SHAPES)---ASTM A500 GRADE B, (FY = 46 KSI)
  - D. STRUCTURAL HSS (ROUND SHAPES)---ASTM A500 GRADE B, (FY = 42 KSI)
  - E. PIPE---ASTM A53 (FY = 35 KSI)
  - F. CONNECTION BOLTS---ASTM A325-N
  - G. U-BOLTS---ASTM A36
  - H. ANCHOR RODS---ASTM F 1554
  - I. WELDING ELECTRODE---ASTM E 70XX
2. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE THE FOLLOWING: SECTION PROFILES, SIZES, CONNECTION ATTACHMENTS, REINFORCING, ANCHORAGE, SIZE AND TYPE OF FASTENERS AND ACCESSORIES. INCLUDE ERECTION DRAWINGS, ELEVATIONS AND DETAILS.
3. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST PROVISIONS OF AISC MANUAL OF STEEL CONSTRUCTION.
4. PROVIDE ALL PLATES, CLIP ANGLES, CLOSURE PIECES, STRAP ANCHORS, MISCELLANEOUS PIECES AND HOLES REQUIRED TO COMPLETE THE STRUCTURE.
5. FIT AND SHOP ASSEMBLE FABRICATIONS IN THE LARGEST PRACTICAL SECTIONS FOR DELIVERY TO SITE.
6. INSTALL FABRICATIONS PLUMB AND LEVEL, ACCURATELY FITTED, AND FREE FROM DISTORTIONS OR DEFECTS.
7. AFTER ERECTION OF STRUCTURES, TOUCHUP ALL WELDS, ABRASIONS AND NON-GALVANIZED SURFACES WITH A 95% ORGANIC ZINC RICH PAINT IN ACCORDANCE WITH ASTM 780.
8. ALL STEEL MATERIAL (EXPOSED TO WEATHER) SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT DIPPED GALVANIZED) COATINGS" ON IRONS AND STEEL PRODUCTS.
9. ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING (HOT-DIP) ON IRON AND STEEL HARDWARE".
10. THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NON CONFORMING MATERIALS OR CONDITIONS TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW.
11. CONNECTION ANGLES SHALL HAVE A MINIMUM THICKNESS OF 1/4 INCHES.
12. STRUCTURAL CONNECTION BOLTS SHALL CONFORM TO ASTM A325. ALL BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL HAVE A MINIMUM OF TWO BOLTS, UNLESS OTHERWISE ON THE DRAWINGS.
13. LOCK WASHER ARE NOT PERMITTED FOR A325 STEEL ASSEMBLIES.
14. SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED.
15. MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION.
16. FABRICATE BEAMS WITH MILL CAMBER UP.
17. LEVEL AND PLUMB INDIVIDUAL MEMBERS OF THE STRUCTURE TO AN ACCURACY OF 1:500, BUT NOT TO EXCEED 1/4" IN THE FULL HEIGHT OF THE COLUMN.
18. COMMENCEMENT OF STRUCTURAL STEEL WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCIES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.
19. INSPECTION AND TESTING OF ALL WELDING AND HIGH STRENGTH BOLTING SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY.
20. FOUR COPIES OF ALL INSPECTION TEST REPORTS SHALL BE SUBMITTED TO THE ENGINEER WITHIN TEN (10) WORKING DAYS OF THE DATE OF INSPECTION.

**PAINT NOTES**

- PAINTING SCHEDULE:**
1. **ANTENNA PANELS:**
    - A. SHERWIN WILLIAMS POLANE-B
    - B. COLOR TO BE MATCHED WITH EXISTING TOWER STRUCTURE.
  2. **COAXIAL CABLES:**
    - A. ONE COAT OF DTM BONDING PRIMER (2-5 MILS. DRY FINISH)
    - B. TWO COATS OF DTM ACRYLIC PRIMER/FINISH (2.5-5 MILS. DRY FINISH)
    - C. COLOR TO BE FIELD MATCHED WITH EXISTING STRUCTURE.
- EXAMINATION AND PREPARATION:**
1. DO NOT APPLY PAINT IN SNOW, RAIN, FOG OR MIST OR WHEN RELATIVE HUMIDITY EXCEEDS 85%. DO NOT APPLY PAINT TO DAMP OR WET SURFACES.
  2. VERIFY THAT SUBSTRATE CONDITIONS ARE READY TO RECEIVE WORK. EXAMINE SURFACE SCHEDULED TO BE FINISHED PRIOR TO COMMENCEMENT OF WORK. REPORT ANY CONDITION THAT MAY POTENTIALLY AFFECT PROPER APPLICATION.
  3. TEST SHOP APPLIED PRIMER FOR COMPATIBILITY WITH SUBSEQUENT COVER MATERIALS.
  4. PERFORM PREPARATION AND CLEANING PROCEDURE IN STRICT ACCORDANCE WITH COATING MANUFACTURER'S INSTRUCTIONS FOR EACH SUBSTRATE CONDITION.
  5. CORRECT DEFECTS AND CLEAN SURFACES WHICH AFFECT WORK OF THIS SECTION. REMOVE EXISTING COATINGS THAT EXHIBIT LOOSE SURFACE DEFECTS.
  6. IMPERVIOUS SURFACE: REMOVE MILDEW BY SCRUBBING WITH SOLUTION OF TRI-SODIUM PHOSPHATE AND BLEACH. RINSE WITH CLEAN WATER AND ALLOW SURFACE TO DRY.
  7. ALUMINUM SURFACE SCHEDULED FOR PAINT FINISH: REMOVE SURFACE CONTAMINATION BY STEAM OR HIGH-PRESSURE WATER. REMOVE OXIDATION WITH ACID ETCH AND SOLVENT WASHING. APPLY ETCHING PRIMER IMMEDIATELY FOLLOWING CLEANING.
  8. FERROUS METALS: CLEAN UNGALVANIZED FERROUS METAL SURFACES THAT HAVE NOT BEEN SHOP COATED; REMOVE OIL, GREASE, DIRT, LOOSE MILL SCALE, AND OTHER FOREIGN SUBSTANCES. USE SOLVENT OR MECHANICAL CLEANING METHODS THAT COMPLY WITH THE STEEL STRUCTURES PAINTING COUNCIL'S (SSPC) RECOMMENDATIONS. TOUCH UP BARE AREAS AND SHOP APPLIED PRIME COATS THAT HAVE BEEN DAMAGED. WIRE BRUSH, CLEAN WITH SOLVENTS RECOMMENDED BY PAINT MANUFACTURER, AND TOUCH UP WITH THE SAME PRIMER AS THE SHOP COAT.
  9. GALVANIZED SURFACES: CLEAN GALVANIZED SURFACES WITH NON-PETROLEUM-BASED SOLVENTS SO SURFACE IS FREE OF OIL AND SURFACE CONTAMINANTS. REMOVE PRETREATMENT FROM GALVANIZED SHEET METAL FABRICATED FROM COIL STOCK BY MECHANICAL METHODS.
  10. ANTENNA PANELS: REMOVE ALL OIL, DUST, GREASE, DIRT, AND OTHER FOREIGN MATERIAL TO ENSURE ADEQUATE ADHESION. PANELS MUST BE WIPED WITH METHYL ETHYL KETONE (MEK).
  11. COAXIAL CABLES: REMOVE ALL OIL, DUST, GREASE, DIRT, AND OTHER FOREIGN MATERIAL TO ENSURE ADEQUATE ADHESION.
- CLEANING:**
1. COLLECT WASTE MATERIAL, WHICH MAY CONSTITUTE A FIRE HAZARD, PLACE IN CLOSED METAL CONTAINERS AND REMOVE DAILY FROM SITE.
- APPLICATION:**
1. APPLY PRODUCTS IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
  2. DO NOT APPLY FINISHES TO SURFACES THAT ARE NOT DRY.
  3. APPLY EACH COAT TO UNIFORM FINISH.
  4. APPLY EACH COAT OF PAINT SLIGHTLY DARKER THAN PRECEDING COAT UNLESS OTHERWISE APPROVED.
  5. SAND METAL LIGHTLY BETWEEN COATS TO ACHIEVE REQUIRED FINISH.
  6. VACUUM CLEAN SURFACES FREE OF LOOSE PARTICLES. USE TACK CLOTH JUST PRIOR TO APPLYING NEXT COAT.
  7. ALLOW APPLIED COAT TO DRY BEFORE NEXT COAT IS APPLIED.
- COMPLETED WORK:**
1. SAMPLES: PREPARE 24" x 24" SAMPLE AREA FOR REVIEW.
  2. MATCH APPROVED SAMPLES FOR COLOR, TEXTURE AND COVERAGE. REMOVE REFINISH OR REPAINT WORK NOT IN COMPLIANCE WITH SPECIFIED REQUIREMENTS.

REV.	DATE	DESCRIPTION
1	09/19/16	CAC
0	09/14/16	COL



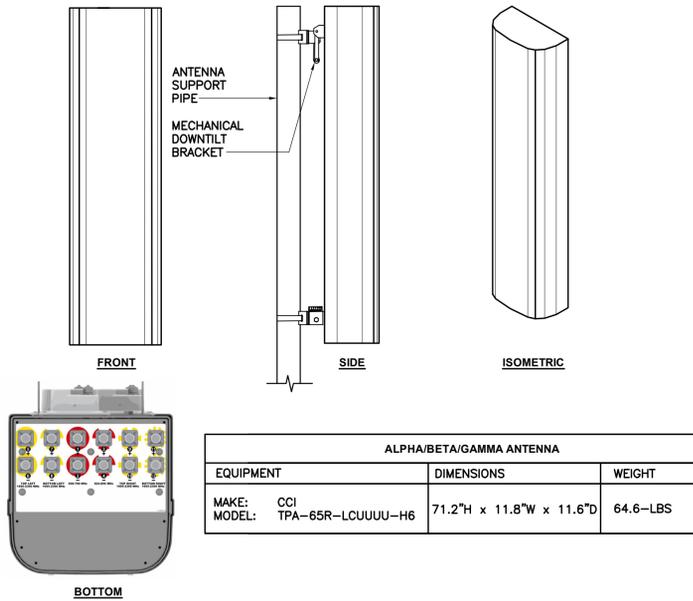
**CENTEK engineering**  
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**WALLINGFORD**  
 CT2154 - LTE 2C  
 945 EAST CENTER STREET  
 WALLINGFORD, CT 06492

DATE: 05/14/16  
 SCALE: AS NOTED  
 JOB NO. 16071.19

NOTES AND SPECIFICATIONS

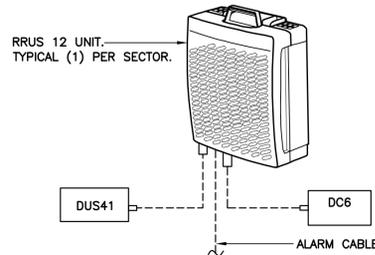




ALPHA/BETA/GAMMA ANTENNA		
EQUIPMENT	DIMENSIONS	WEIGHT
MAKE: CCI MODEL: TPA-65R-LCUUUU-H6	71.2"H x 11.8"W x 11.6"D	64.6-LBS

**5 PROPOSED ANTENNA DETAIL**

- SCALE: NTS
- NOTES:
- INSTALL ANTENNA TO EXISTING PIPE MUST USING MANUFACTURERS SUPPLIED BRACKETS AND MOUNTING HARDWARE
  - SET MECHANICAL DOWNTILT TO VALUE SPECIFIED IN LATEST RFDS

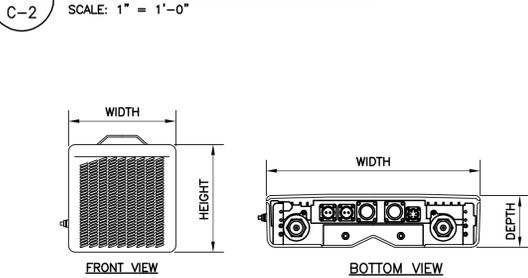


RRU (REMOTE RADIO UNIT)			
EQUIPMENT	DIMENSIONS	WEIGHT	CLEARANCES
MAKE: ERICSSON MODEL: RRU 12	20.4"L x 18.5"W x 7.5"D	50 LBS.	ABOVE: 16" MIN. BELOW: 12" MIN. FRONT: 36" MIN.

NOTES:

- CONTRACTOR TO COORDINATE FINAL EQUIPMENT MODEL SELECTION WITH AT&T CONSTRUCTION MANAGER PRIOR TO ORDERING.

**6 ERICSSON RRU 12 DETAIL**

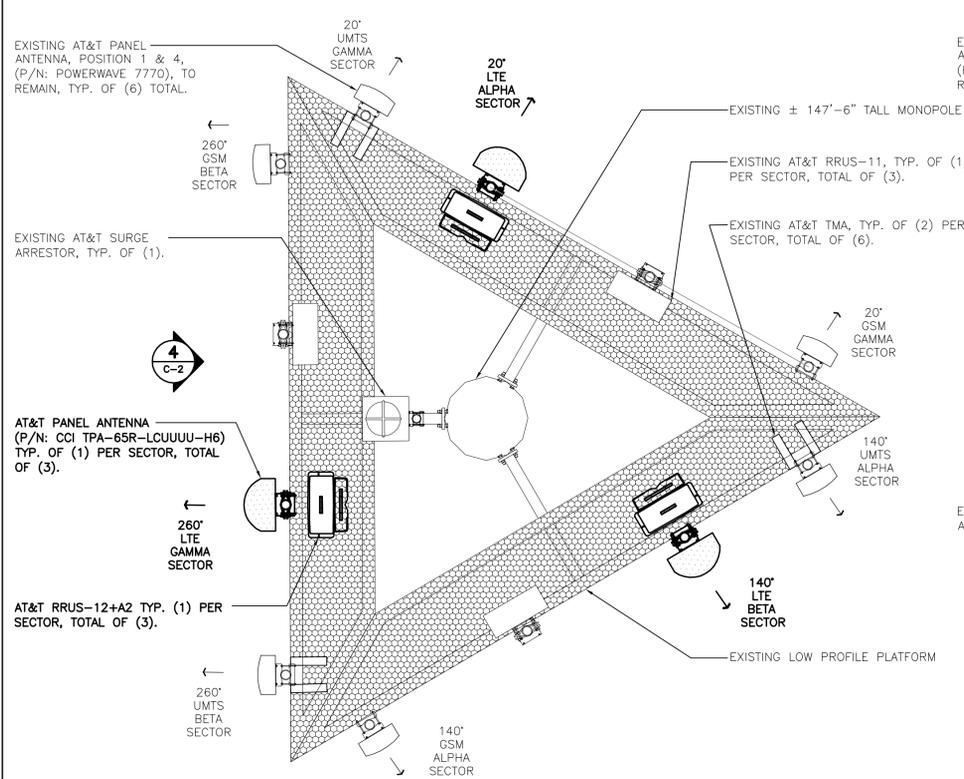


RRU (REMOTE RADIO UNIT)			
EQUIPMENT	DIMENSIONS	WEIGHT	CLEARANCES
MAKE: ERICSSON MODEL: RRU A2	16.42"L x 15.19"W x 3.35"D	22.05 LBS.	ABOVE: 16" MIN. BELOW: 12" MIN. FRONT: 36" MIN.

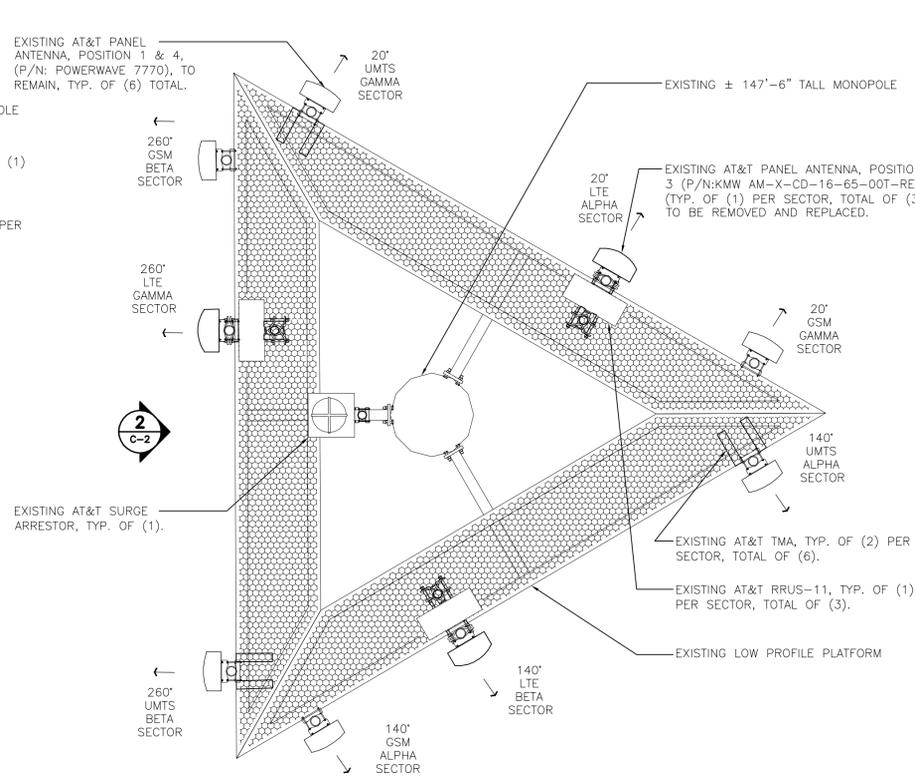
NOTES:

- CONTRACTOR TO COORDINATE FINAL EQUIPMENT MODEL SELECTION WITH AT&T CONSTRUCTION MANAGER PRIOR TO ORDERING.

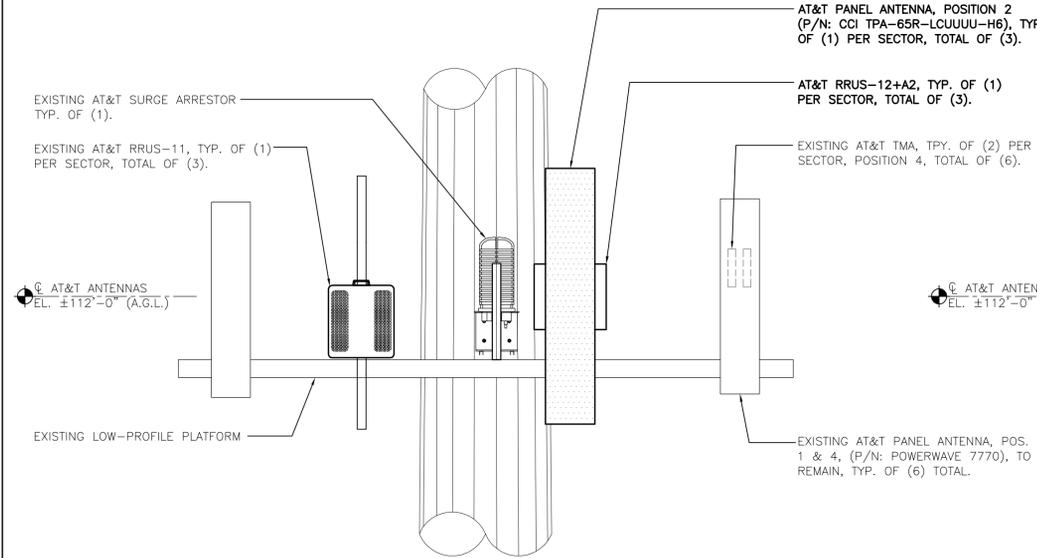
**7 ERICSSON RRU A2 DETAIL**



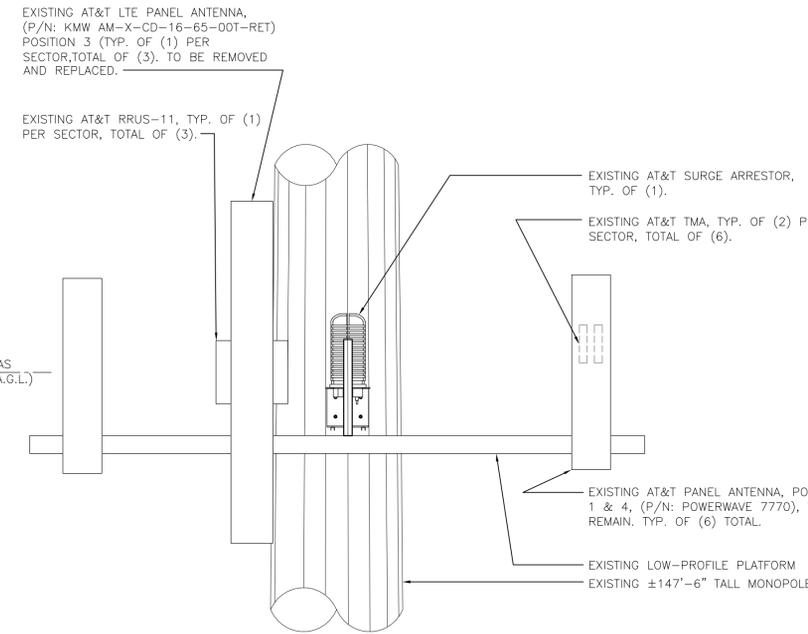
**3 PROPOSED ANTENNA PLAN**  
SCALE: 1/2" = 1'-0"  
APPROX. NORTH



**1 EXISTING ANTENNA PLAN**  
SCALE: 1/2" = 1'-0"  
APPROX. NORTH

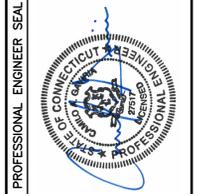


**4 PROPOSED ANTENNA SECTOR ELEVATION**  
SCALE: 1/2" = 1'-0"



**2 EXISTING ANTENNA SECTOR ELEVATION**  
SCALE: 1/2" = 1'-0"

REV.	DATE	BY	CHKD	DESCRIPTION
1	09/18/16	CAC	LOL	CONSTRUCTION DOCUMENTS - REV'D TO MATCH CCI STRUCT.
0	09/14/16	CAC	LOL	CONSTRUCTION DOCUMENTS - ISSUED FOR CLIENT REVIEW



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945 EAST CENTER STREET  
WALLINGFORD, CT 06492

DATE: 05/14/16  
SCALE: AS NOTED  
JOB NO. 16071.19

LTE EQUIPMENT DETAILS







Date: **September 14, 2016**

Charles McGuirt  
Crown Castle  
3530 Toringdon Way, Suite 300  
Charlotte, NC 28277  
(704) 405-6607



Tower Engineering Professionals  
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Raleigh, NC 27603  
(919) 661-6351  
[crown@tepgroup.net](mailto:crown@tepgroup.net)

**Subject: Structural Analysis Report**

<b>Carrier Designation:</b>	<b>AT&amp;T Mobility Co-Locate</b>	
	<b>Carrier Site Number:</b>	CTL02154
	<b>Carrier Site Name:</b>	Wallingford
<b>Crown Castle Designation:</b>	<b>Crown Castle BU Number:</b>	876310
	<b>Crown Castle Site Name:</b>	Beaumont Farm
	<b>Crown Castle JDE Job Number:</b>	384292
	<b>Crown Castle Work Order Number:</b>	1296341
	<b>Crown Castle Application Number:</b>	345702 Rev. 3
<b>Engineering Firm Designation:</b>	<b>TEP Project Number:</b>	72875.96392
<b>Site Data:</b>	<b>945 East Center St., Wallingford, New Haven County, CT 06492</b>	
	<b>Latitude 41° 26' 37.36", Longitude -72° 47' 46.56"</b>	
	<b>147 Foot - Monopole Tower (Including 14-ft extension)</b>	

Dear Charles McGuirt,

Tower Engineering Professionals is pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 945902, in accordance with application 345702, revision 3.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Existing + Proposed Equipment	<b>Sufficient Capacity</b>
Note: See Table I and Table II for the proposed and existing loading, respectively.	

The analysis has been performed in accordance with the TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures, ASCE 7-05 Minimum Design Loads for Buildings and Other Structures and the 2005 Connecticut State Building Code (2006 International Building Code) based upon a wind speed of 85 mph fastest mile.

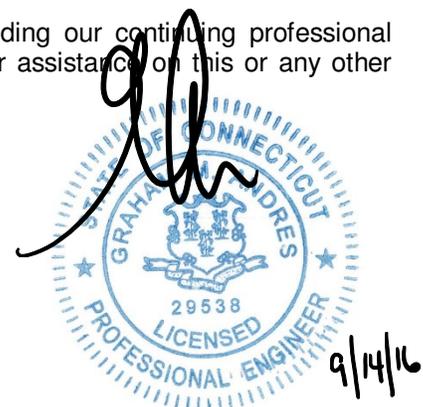
All modifications and equipment proposed in this report shall be installed in accordance with the appurtenances listed in Tables 1 and 2 and the attached drawing for the determined available structural capacity to be effective.

We at Tower Engineering Professionals appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Christopher D. Crook, E.I. / DTS

Respectfully submitted by:

Graham M. Andres, P.E.



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tnxTower Output

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## 1) INTRODUCTION

This tower is 147-ft monopole tower designed by Paul J. Ford and Company in June of 1998. The tower was originally designed for a height of 133-ft for a wind speed of 90 mph per TIA/EIA-222-F for the appurtenances listed in Table 3. The tower was previously extended 14-ft, bringing the overall tower height to 147-ft per reinforcement drawings by URS Greiner Woodward Clyde AES in December of 1999. TEP did not visit the site. All information provided to TEP was assumed to be accurate and complete.

## 2) ANALYSIS CRITERIA

The analysis has been performed in accordance with the TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures and ASCE 7-05 Minimum Design Loads for Buildings and Other Structures using a fastest mile wind speed of 85 mph with no ice, 37.6 mph with 0.75 inch escalating ice thickness and 50 mph under service loads.

**Table 1 - Proposed Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
111.0	112.0	3	CCI Antennas	TPA-65R-LCUUUU-H6 w/ Mount Pipe	-	-	-
		3	CCI Antennas	DTMABP7819VG12A			
		3	Ericsson	RRUS12/RRUS A2			

**Table 2 - Existing Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
130.0	132.0	1	Andrew	VHLP1-23	6	5/16 1-1/4 1/2 7/8	1
		1	Andrew	VHLP2-23			
		1	Andrew	VHLP2.5-23			
	130.0	1	RFS Celwave	APXV9ERR18-C-A20 w/ Mount Pipe			
		1	RFS Celwave	APXVSPP18-C-A20 w/ Mount Pipe			
		3	RFS Celwave	APXVTM14-C-120 w/ Mount Pipe			
		3	Alcatel Lucent	TD-RRH8x20-25			
		6	RFS Celwave	ACU-A20-N			
		2	Alcatel Lucent	800 External Notch Filter			
		2	Alcatel Lucent	800MHZ RRH			
		2	Alcatel Lucent	1900MHz RRH (65MHz)			
	1	Tower Mounts	Miscellaneous [NA 510-1]				
	128.0	3	Argus Technologies	LLPX310R w/ Mount Pipe			
		3	Samsung Telecommunications	FDD_R6_RRH			
	127.0	1	RFS Celwave	APXVSPP18-C-A20 w/ Mount Pipe			
1		Alcatel Lucent	800 External Notch Filter				
1		Alcatel Lucent	800MHZ RRH				
3		RFS Celwave	ACU-A20-N				
1		Alcatel Lucent	1900MHz RRH (65MHz)				
121.0	121.0	3	Antel	BXA-171063/12CF w/ Mount Pipe	12	1-5/8 1-1/4	1
		2	Antel	BXA-70063/6CFx2 w/ Mount Pipe			
		1	Antel	BXA-70063/6CFx4 w/ Mount Pipe			
		2	Antel	LPA-80063/6CF w/ Mount Pipe			
		4	Antel	LPA-80080-6CF-EDIN w/ Mount Pipe			
		3	Commscope	HBX-6516DS-VTM w/ Mount Pipe			
		6	RFS Celwave	FD9R6004/2C-3L			
		1	RFS Celwave	DB-T1-6Z-8AB-0Z			
		3	Alcatel Lucent	RRH2X40-AWS			
		1	Tower Mounts	Platform Mount [LP 1201-1]			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note	
111.0	112.0	3	Powerwave Technologies	7770.00 w/ Mount Pipe	6	1-1/4	2	
		3	KMW Communications	AM-X-CD-16-65-00T-RET w/ Mount Pipe				
		6	Powerwave Technologies	LGP21401				
		6	Powerwave Technologies	LGP21901				
		12	Kathrein	860 10025				
		3	Ericsson	RRUS-11				
	111.0	111.0	3	Powerwave Technologies	7770.00 w/ Mount Pipe	6 2 1	1-1/4 3/4 3/8	1
			3	Ericsson	RRUS-11			
			1	Raycap	DC6-48-60-18-8F			
70.0	70.0	1	Tower Mounts	Platform Mount [LP 1201-1]	1	1/2	1	
		1	Kathrein	OG-860/1920/GPS-A				
		1	Tower Mounts	Side Arm Mount [SO 701-1]				

Notes:

- 1) Existing equipment
- 2) Existing equipment to be removed; not considered in this analysis

**Table 3 - Design Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
130.0	130.0	12	Decibel	DB980H	-	-
110.0	110.0	12	Generic	3.9 sq.ft. Panel Antenna	-	-
95.0	95.0	12	Generic	3.9 sq.ft. Panel Antenna	-	-
70.0	70.0	1	Generic	GPS Antenna	-	-

**3) ANALYSIS PROCEDURE**

**Table 4 - Documents Provided**

Document	Remarks	Reference	Source
Geotechnical Report	Dr. Clarence Welti, P.E., P.C.	1531484	CCISites
Tower Foundation Drawings	Paul J. Ford and Company	1855118	CCISites
Tower Manufacturer Drawings	Paul J. Ford and Company	1855980	CCISites
Tower Reinforcement Drawings	URS Greiner Woodward Clyde	2015154	CCISites

**3.1) Analysis Method**

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

### 3.2) Assumptions

- 1) The tower and foundation were built in accordance with the manufacturer's specifications.
- 2) The tower and foundation have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2, and "Appendix B – Base Level Drawing".
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by the standard.
- 5) All tower components are in sufficient condition to carry their full design capacity.
- 6) Serviceability with respect to antenna twist, tilt, roll, or lateral translation, is not checked and is left to the carrier or tower owner to ensure conformance.
- 7) All antenna mounts and mounting hardware are structurally sufficient to carry the full design capacity requirements of appurtenance wind area and weight as provided by the original manufacturer specifications. It is the carrier's responsibility to ensure compliance to the structural limitations of the existing and/or proposed antenna mounts. TEP did not perform a site visit to verify the size, condition or capacity of the antenna mounts and did not analyze antennas supporting mounts as part of this structural analysis report.

This analysis may be affected if any assumptions are not valid or have been made in error. Tower Engineering Professionals should be notified to determine the effect on the structural integrity of the tower.

### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (lb)	$\phi P_{allow}$ (lb)	% Capacity	Pass / Fail
L1	147 - 133	Pole	TP12.75x12.75x0.5	1	-867.088	538647.949	2.8	Pass
L2	133 - 85.5	Pole	TP29.418x19.537x0.313	2	-13173.500	1481762.739	79.2	Pass
L3	85.5 - 42.75	Pole	TP37.687x27.477x0.375	3	-20915.199	2276763.906	99.9	Pass
L4	42.75 - 0	Pole	TP45.83x35.894x0.438	4	-33230.000	3324395.222	98.7	Pass
							Summary	
						Pole (L3)	99.9	Pass
						<b>RATING =</b>	<b>99.9</b>	<b>Pass</b>

**Table 6 - Tower Component Stresses vs. Capacity**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Flange Connection	133.0	7.5	Pass
1	Anchor Rods	-	84.4	Pass
1	Base Plate	-	93.5	Pass
1	Base Foundation Soil Interaction	-	95.6	Pass
1	Base Foundation Structural	-	28.4	Pass

<b>Structure Rating (max from all components) =</b>	<b>99.9%</b>
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Notes:

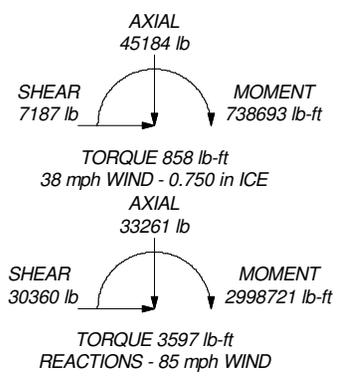
- 1) See additional documentation in "Appendix C - Additional Calculations" for calculations supporting the % capacity listed.

#### **4.1) Recommendations**

- 1) If the load differs from that described in Tables 1 and 2 of this report, "Appendix B – Base Level Drawing" or the provisions of this analysis are found to be invalid, another structural analysis should be performed.
- 2) The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

**APPENDIX A**  
**TNXTOWER OUTPUT**

Section	1	2	3	4
Length (ft)	14.000	47.500	46.500	47.500
Number of Sides	1	12	12	12
Thickness (in)	0.500	0.313	0.375	0.438
Socket Length (ft)		3.750	4.750	
Top Dia (in)	12.750	19.537	27.477	35.884
Bot Dia (in)	12.750	29.418	37.687	45.680
Grade	A53-B-35		A607-65	
Weight (lb)	916.7	3930.3	6153.6	9204.7
			20205.2	



### DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
APXVTM14-C-120 w/ Mount Pipe	130	HBX-6516DS-VTM w/ Mount Pipe	121
APXVTM14-C-120 w/ Mount Pipe	130	HBX-6516DS-VTM w/ Mount Pipe	121
APXVTM14-C-120 w/ Mount Pipe	130	BXA-171063/12CF w/ Mount Pipe	121
LLPX310R w/ Mount Pipe	130	BXA-171063/12CF w/ Mount Pipe	121
LLPX310R w/ Mount Pipe	130	BXA-171063/12CF w/ Mount Pipe	121
LLPX310R w/ Mount Pipe	130	(2) FD9R6004/2C-3L	121
APXV9ERR18-C-A20 w/ Mount Pipe	130	(2) FD9R6004/2C-3L	121
APXVSPP18-C-A20 w/ Mount Pipe	130	(2) FD9R6004/2C-3L	121
APXVSPP18-C-A20 w/ Mount Pipe	130	DB-T1-6Z-8AB-0Z	121
TD-RRH8x20-25	130	Platform Mount [LP 1201-1]	121
TD-RRH8x20-25	130	RRH2X40-AWS	121
TD-RRH8x20-25	130	RRH2X40-AWS	121
FDD_R6_RRH	130	RRH2X40-AWS	121
FDD_R6_RRH	130	(2) LPA-80080-6CF-EDIN w/ Mount Pipe	121
FDD_R6_RRH	130	(2) LPA-80080-6CF-EDIN w/ Mount Pipe	121
800 EXTERNAL NOTCH FILTER	130	(2) LPA-80063/6CF w/ Mount Pipe	121
800 EXTERNAL NOTCH FILTER	130	TPA-65R-LCUUUU-H6 w/ Mount Pipe	111
800 EXTERNAL NOTCH FILTER	130	TPA-65R-LCUUUU-H6 w/ Mount Pipe	111
(3) ACU-A20-N	130	TPA-65R-LCUUUU-H6 w/ Mount Pipe	111
(3) ACU-A20-N	130	TPA-65R-LCUUUU-H6 w/ Mount Pipe	111
(3) ACU-A20-N	130	RRUS-11	111
800MHZ RRH	130	RRUS-11	111
800MHZ RRH	130	RRUS-11	111
800MHZ RRH	130	RRUS-11	111
1900MHz RRH (65MHz)	130	DC6-48-60-18-8F	111
1900MHz RRH (65MHz)	130	DTMABP7819VG12A	111
1900MHz RRH (65MHz)	130	DTMABP7819VG12A	111
2.4" Dia. x 6-ft	130	DTMABP7819VG12A	111
2.4" Dia. x 6-ft	130	RRUS12/RRUS A2	111
Platform Mount [LP 1201-1]	130	RRUS12/RRUS A2	111
Miscellaneous [NA 510-1]	130	RRUS12/RRUS A2	111
VHLP1-23	130	Platform Mount [LP 1201-1]	111
VHLP2-23	130	7770.00 w/ Mount Pipe	111
VHLP2.5-23	130	7770.00 w/ Mount Pipe	111
BXA-70063/6CFx2 w/ Mount Pipe	121	7770.00 w/ Mount Pipe	111
BXA-70063/6CFx2 w/ Mount Pipe	121	Side Arm Mount [SO 701-1]	70
BXA-70063/6CFx4 w/ Mount Pipe	121	OG-860/1920/GPS-A	70
HBX-6516DS-VTM w/ Mount Pipe	121		

### MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A53-B-35	35 ksi	60 ksi	A607-65	65 ksi	80 ksi

### TOWER DESIGN NOTES

1. Tower is located in New Haven County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 99.9%

 Tower Engineering Professionals	<b>Tower Engineering Professionals</b>		<b>Job: Beaumont Farm (BU 876310)</b>		
	326 Tryon Road		Project: <b>TEP No. 72875.96392</b>		
	Raleigh, NC 27603		Client: Crown Castle	Drawn by: Chris D. Crook, E.I.	App'd:
	Phone: (919) 661-6351		Code: TIA/EIA-222-F	Date: 09/13/16	Scale: NTS
	FAX: (919) 661-6350		Path: C:\Users\cdcrook\Desktop\TNX Tower Run\72875\876310_LC5.eri		Dwg No. E-1

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	<b>Project</b> TEP No. 72875.96392	<b>Date</b> 14:54:13 09/13/16
	<b>Client</b> Crown Castle	<b>Designed by</b> Chris D. Crook, E.I.

## Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in New Haven County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.750 in.

Ice thickness is considered to increase with height.

Ice density of 56.000 pcf.

A wind speed of 38 mph is used in combination with ice.

Temperature drop of 50.000 °F.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

- |  |  |   |
|--|--|---|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>√ Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>SR Members Have Cut Ends</li> <li>SR Members Are Concentric</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>√ Bypass Mast Stability Checks</li> <li>√ Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>Autocalc Torque Arm Areas</li> <li>Add IBC .6D+W Combination</li> <li>√ Sort Capacity Reports By Component</li> <li>Triangulate Diamond Inner Bracing</li> <li>Treat Feed Line Bundles As Cylinder</li> </ul> | <ul style="list-style-type: none"> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feed Line Torque</li> <li>Include Angle Block Shear Check</li> <li>Use TIA-222-G Bracing Resist. Exemption</li> <li>Use TIA-222-G Tension Splice Exemption</li> <li style="text-align: center;">Poles</li> <li>√ Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul> |
|--|--|---|

## Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	147.000-133.000	14.000	0.000	Round	12.750	12.750	0.500		A53-B-35 (35 ksi)
L2	133.000-85.500	47.500	3.750	12	19.537	29.418	0.313	1.250	A607-65 (65 ksi)
L3	85.500-42.750	46.500	4.750	12	27.477	37.687	0.375	1.500	A607-65 (65 ksi)
L4	42.750-0.000	47.500		12	35.894	45.830	0.438	1.750	A607-65 (65 ksi)

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	<b>Client</b> Crown Castle	<b>Designed by</b> Chris D. Crook, E.I.

### Tapered Pole Properties

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	It/Q in <sup>2</sup>	w in	w/t
L1	12.750	19.242	361.544	4.335	6.375	56.713	723.088	9.615	0.000	0
	12.750	19.242	361.544	4.335	6.375	56.713	723.088	9.615	0.000	0
L2	20.226	19.345	912.551	6.882	10.120	90.172	1849.075	9.521	4.398	14.075
	30.456	29.287	3166.774	10.420	15.239	207.814	6416.742	14.414	7.047	22.549
L3	29.299	32.726	3068.189	9.703	14.233	215.567	6216.983	16.107	6.359	16.957
	39.016	45.054	8006.057	13.358	19.522	410.107	16222.442	22.174	9.095	24.254
L4	38.189	49.949	8015.109	12.693	18.593	431.079	16240.785	24.584	8.447	19.308
	47.447	63.947	16817.916	16.251	23.740	708.423	34077.658	31.473	11.110	25.394

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>f</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in					in	in	in
L1 147.000-133.000				1	1	1			
L2 133.000-85.500				1	1	1			
L3 85.500-42.750				1	1	1			
L4 42.750-0.000				1	1	1			

### Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	Number Per Row	Clear Spacing	Width or Diameter	Perimeter	Weight
				ft			in	in	in	plf
***										

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	C <sub>AA</sub>	Weight
				ft		ft <sup>2</sup> /ft	plf
**130** 7983A(1/2")	C	No	Inside Pole	130.000 - 0.000	3	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.000 0.084 0.084 0.084 0.084
9207(5/16")	C	No	Inside Pole	130.000 - 0.000	6	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.000 0.600 0.600 0.600 0.600
HB114-08U3M12-xxxF(	C	No	Inside Pole	130.000 - 0.000	1	No Ice	0.000 0.683

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	<b>Client</b> Crown Castle	<b>Designed by</b> Chris D. Crook, E.I.

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number		C <sub>AA</sub> ft <sup>2</sup> /ft	Weight plf
7/8")						1/2" Ice	0.000	0.683
						1" Ice	0.000	0.683
						2" Ice	0.000	0.683
						4" Ice	0.000	0.683
HB114-1-0813U4-M5J(1 1/4")	C	No	Inside Pole	130.000 - 0.000	3	No Ice	0.000	1.200
						1/2" Ice	0.000	1.200
						1" Ice	0.000	1.200
						2" Ice	0.000	1.200
						4" Ice	0.000	1.200
2" Flexible Conduit	C	No	Inside Pole	130.000 - 0.000	1	No Ice	0.000	0.340
						1/2" Ice	0.000	0.340
						1" Ice	0.000	0.340
						2" Ice	0.000	0.340
						4" Ice	0.000	0.340
**121**								
FLC 158-50J(1-5/8")	C	No	Inside Pole	121.000 - 0.000	12	No Ice	0.000	0.920
						1/2" Ice	0.000	0.920
						1" Ice	0.000	0.920
						2" Ice	0.000	0.920
						4" Ice	0.000	0.920
LDF6-50A(1-1/4")	C	No	Inside Pole	121.000 - 0.000	1	No Ice	0.000	0.660
						1/2" Ice	0.000	0.660
						1" Ice	0.000	0.660
						2" Ice	0.000	0.660
						4" Ice	0.000	0.660
**111**								
FLC 114-50J(1-1/4")	C	No	Inside Pole	111.000 - 0.000	6	No Ice	0.000	0.700
						1/2" Ice	0.000	0.700
						1" Ice	0.000	0.700
						2" Ice	0.000	0.700
						4" Ice	0.000	0.700
FB-L98B-002-75000(3/8")	C	No	Inside Pole	111.000 - 0.000	1	No Ice	0.000	0.059
						1/2" Ice	0.000	0.059
						1" Ice	0.000	0.059
						2" Ice	0.000	0.059
						4" Ice	0.000	0.059
WR-VG86ST-BRD(3/4)	C	No	Inside Pole	111.000 - 0.000	2	No Ice	0.000	0.590
						1/2" Ice	0.000	0.590
						1" Ice	0.000	0.590
						2" Ice	0.000	0.590
						4" Ice	0.000	0.590
2" Flexible Conduit	C	No	Inside Pole	111.000 - 0.000	2	No Ice	0.000	0.340
						1/2" Ice	0.000	0.340
						1" Ice	0.000	0.340
						2" Ice	0.000	0.340
						4" Ice	0.000	0.340
**70**								
LDF4-50A(1/2")	C	No	Inside Pole	70.000 - 0.000	1	No Ice	0.000	0.150
						1/2" Ice	0.000	0.150
						1" Ice	0.000	0.150
						2" Ice	0.000	0.150
						4" Ice	0.000	0.150
**Misc**								
Safety Line 3/8	A	No	CaAa (Out Of Face)	133.000 - 0.000	1	No Ice	0.037	0.220
						1/2" Ice	0.137	0.750
						1" Ice	0.238	1.280
						2" Ice	0.437	2.340
						4" Ice	0.838	4.460
Step Pegs (5/8" SR) 7-in. w/30" step	A	No	CaAa (Out Of Face)	147.000 - 0.000	1	No Ice	0.035	0.487
						1/2" Ice	0.135	1.006
						1" Ice	0.235	2.065

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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C <sub>A</sub> A <sub>A</sub> ft <sup>2</sup> /ft	Weight plf
						2" Ice 0.435	6.087
						4" Ice 0.835	21.462
***							

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight lb
L1	147.000-133.000	A	0.000	0.000	0.000	0.490	6.818
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000	0.000
L2	133.000-85.500	A	0.000	0.000	0.000	3.444	33.583
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000	948.512
L3	85.500-42.750	A	0.000	0.000	0.000	3.099	30.224
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000	1128.139
L4	42.750-0.000	A	0.000	0.000	0.000	3.099	30.224
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000	1130.464

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight lb
L1	147.000-133.000	A	0.892	0.000	0.000	0.000	2.988	25.713
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	0.000	0.000	0.000
L2	133.000-85.500	A	0.865	0.000	0.000	0.000	19.873	138.478
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	0.000	0.000	948.512
L3	85.500-42.750	A	0.811	0.000	0.000	0.000	17.885	124.630
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	0.000	0.000	1128.139
L4	42.750-0.000	A	0.750	0.000	0.000	0.000	16.975	117.398
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	0.000	0.000	1130.464

### Feed Line Center of Pressure

Section	Elevation ft	CP <sub>X</sub> in	CP <sub>Z</sub> in	CP <sub>X</sub> Ice in	CP <sub>Z</sub> Ice in
L1	147.000-133.000	0.000	-0.051	0.000	-0.239
L2	133.000-85.500	0.000	-0.105	0.000	-0.492
L3	85.500-42.750	0.000	-0.106	0.000	-0.521
L4	42.750-0.000	0.000	-0.107	0.000	-0.516

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## Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz Lateral	Vert					
**130**									
APXVTM14-C-120 w/ Mount Pipe	A	From Centroid-Le g	4.000	-20.000	130.000	No Ice	6.897	3.607	56.200
			-6.000			1/2" Ice	7.348	3.967	95.726
			0.000			1" Ice	7.807	4.333	140.320
						2" Ice	8.752	5.140	245.492
						4" Ice	10.746	6.971	525.213
APXVTM14-C-120 w/ Mount Pipe	B	From Centroid-Le g	4.000	-30.000	130.000	No Ice	6.897	3.607	56.200
			-6.000			1/2" Ice	7.348	3.967	95.726
			0.000			1" Ice	7.807	4.333	140.320
						2" Ice	8.752	5.140	245.492
						4" Ice	10.746	6.971	525.213
APXVTM14-C-120 w/ Mount Pipe	C	From Centroid-Le g	4.000	-40.000	130.000	No Ice	6.897	3.607	56.200
			-6.000			1/2" Ice	7.348	3.967	95.726
			0.000			1" Ice	7.807	4.333	140.320
						2" Ice	8.752	5.140	245.492
						4" Ice	10.746	6.971	525.213
LLPX310R w/ Mount Pipe	A	From Centroid-Le g	4.000	-40.000	130.000	No Ice	4.982	2.874	43.868
			6.000			1/2" Ice	5.376	3.398	80.948
			-2.000			1" Ice	5.780	3.937	123.321
						2" Ice	6.618	5.125	226.550
						4" Ice	8.437	7.894	531.242
LLPX310R w/ Mount Pipe	B	From Centroid-Le g	4.000	-50.000	130.000	No Ice	4.982	2.874	43.868
			-2.000			1/2" Ice	5.376	3.398	80.948
			-2.000			1" Ice	5.780	3.937	123.321
						2" Ice	6.618	5.125	226.550
						4" Ice	8.437	7.894	531.242
LLPX310R w/ Mount Pipe	C	From Centroid-Le g	4.000	-40.000	130.000	No Ice	4.982	2.874	43.868
			-2.000			1/2" Ice	5.376	3.398	80.948
			-2.000			1" Ice	5.780	3.937	123.321
						2" Ice	6.618	5.125	226.550
						4" Ice	8.437	7.894	531.242
APXV9ERR18-C-A20 w/ Mount Pipe	A	From Centroid-Le g	4.000	-20.000	130.000	No Ice	8.498	7.471	87.550
			2.000			1/2" Ice	9.149	8.656	158.035
			0.000			1" Ice	9.767	9.556	236.537
						2" Ice	11.031	11.388	421.226
						4" Ice	13.679	15.527	935.368
APXVSPP18-C-A20 w/ Mount Pipe	B	From Centroid-Le g	4.000	-30.000	130.000	No Ice	8.498	6.946	82.550
			6.000			1/2" Ice	9.149	8.127	150.561
			-3.000			1" Ice	9.767	9.021	226.532
						2" Ice	11.031	10.844	405.983
						4" Ice	13.679	14.851	908.948
APXVSPP18-C-A20 w/ Mount Pipe	C	From Centroid-Le g	4.000	-40.000	130.000	No Ice	8.498	6.946	82.550
			6.000			1/2" Ice	9.149	8.127	150.561
			0.000			1" Ice	9.767	9.021	226.532
						2" Ice	11.031	10.844	405.983
						4" Ice	13.679	14.851	908.948
TD-RRH8x20-25	A	From Centroid-Le g	4.000	-20.000	130.000	No Ice	4.720	1.703	70.000
			-6.000			1/2" Ice	5.014	1.920	97.151
			0.000			1" Ice	5.316	2.145	127.829
						2" Ice	5.948	2.622	200.542
						4" Ice	7.314	3.680	396.842

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Vert						
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	lb	
TD-RRH8x20-25	B	From Centroid-Le g	4.000	0.000	-30.000	130.000	No Ice	4.720	1.703	70.000
			-6.000				1/2" Ice	5.014	1.920	97.151
			0.000				1" Ice	5.316	2.145	127.829
							2" Ice	5.948	2.622	200.542
							4" Ice	7.314	3.680	396.842
TD-RRH8x20-25	C	From Centroid-Le g	4.000	0.000	-40.000	130.000	No Ice	4.720	1.703	70.000
			-6.000				1/2" Ice	5.014	1.920	97.151
			0.000				1" Ice	5.316	2.145	127.829
							2" Ice	5.948	2.622	200.542
							4" Ice	7.314	3.680	396.842
FDD_R6_RRH	A	From Centroid-Le g	4.000	6.000	-40.000	130.000	No Ice	0.000	0.778	33.000
			-2.000				1/2" Ice	0.000	0.918	44.505
							1" Ice	0.000	1.067	58.310
							2" Ice	0.000	1.391	93.602
							4" Ice	0.000	2.143	200.352
FDD_R6_RRH	B	From Centroid-Le g	4.000	-2.000	-50.000	130.000	No Ice	0.000	0.778	33.000
			-2.000				1/2" Ice	0.000	0.918	44.505
							1" Ice	0.000	1.067	58.310
							2" Ice	0.000	1.391	93.602
							4" Ice	0.000	2.143	200.352
FDD_R6_RRH	C	From Centroid-Le g	4.000	-2.000	-40.000	130.000	No Ice	0.000	0.778	33.000
			-2.000				1/2" Ice	0.000	0.918	44.505
							1" Ice	0.000	1.067	58.310
							2" Ice	0.000	1.391	93.602
							4" Ice	0.000	2.143	200.352
800 EXTERNAL NOTCH FILTER	A	From Centroid-Le g	4.000	2.000	-20.000	130.000	No Ice	0.770	0.375	11.000
			0.000				1/2" Ice	0.890	0.465	16.814
							1" Ice	1.018	0.563	24.257
							2" Ice	1.301	0.787	44.808
							4" Ice	1.970	1.337	114.010
800 EXTERNAL NOTCH FILTER	B	From Centroid-Le g	4.000	6.000	-30.000	130.000	No Ice	0.770	0.375	11.000
			-3.000				1/2" Ice	0.890	0.465	16.814
							1" Ice	1.018	0.563	24.257
							2" Ice	1.301	0.787	44.808
							4" Ice	1.970	1.337	114.010
800 EXTERNAL NOTCH FILTER	C	From Centroid-Le g	4.000	6.000	-40.000	130.000	No Ice	0.770	0.375	11.000
			0.000				1/2" Ice	0.890	0.465	16.814
							1" Ice	1.018	0.563	24.257
							2" Ice	1.301	0.787	44.808
							4" Ice	1.970	1.337	114.010
(3) ACU-A20-N	A	From Centroid-Le g	4.000	2.000	-20.000	130.000	No Ice	0.078	0.136	1.040
			0.000				1/2" Ice	0.121	0.189	2.320
							1" Ice	0.173	0.251	4.410
							2" Ice	0.302	0.400	11.799
							4" Ice	0.665	0.802	44.855
(3) ACU-A20-N	B	From Centroid-Le g	4.000	6.000	-30.000	130.000	No Ice	0.078	0.136	1.040
			-3.000				1/2" Ice	0.121	0.189	2.320
							1" Ice	0.173	0.251	4.410
							2" Ice	0.302	0.400	11.799
							4" Ice	0.665	0.802	44.855
(3) ACU-A20-N	C	From Centroid-Le g	4.000	6.000	-40.000	130.000	No Ice	0.078	0.136	1.040
			0.000				1/2" Ice	0.121	0.189	2.320
							1" Ice	0.173	0.251	4.410
							2" Ice	0.302	0.400	11.799
							4" Ice	0.665	0.802	44.855
800MHZ RRH	A	From Centroid-Le	4.000	2.000	-20.000	130.000	No Ice	2.490	2.068	53.000
							1/2" Ice	2.706	2.271	74.187

<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Road Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b>	Beaumont Farm (BU 876310)	<b>Page</b>	7 of 15
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	<b>Client</b>	Crown Castle	<b>Designed by</b>	Chris D. Crook, E.I.

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Vert						
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	lb	
		g	0.000			1" Ice	2.931	2.481	98.387	
						2" Ice	3.407	2.928	156.608	
						4" Ice	4.462	3.927	317.771	
800MHZ RRH	B	From Centroid-Le	4.000		-30.000	130.000	No Ice	2.490	2.068	53.000
		g	6.000				1/2" Ice	2.706	2.271	74.187
			-3.000				1" Ice	2.931	2.481	98.387
							2" Ice	3.407	2.928	156.608
							4" Ice	4.462	3.927	317.771
800MHZ RRH	C	From Centroid-Le	4.000		-40.000	130.000	No Ice	2.490	2.068	53.000
		g	6.000				1/2" Ice	2.706	2.271	74.187
			0.000				1" Ice	2.931	2.481	98.387
							2" Ice	3.407	2.928	156.608
							4" Ice	4.462	3.927	317.771
1900MHz RRH (65MHz)	A	From Centroid-Le	4.000		-20.000	130.000	No Ice	2.698	2.771	60.000
		g	2.000				1/2" Ice	2.936	3.011	83.902
			0.000				1" Ice	3.183	3.260	111.077
							2" Ice	3.703	3.784	176.024
							4" Ice	4.846	4.935	353.751
1900MHz RRH (65MHz)	B	From Centroid-Le	4.000		-30.000	130.000	No Ice	2.698	2.771	60.000
		g	6.000				1/2" Ice	2.936	3.011	83.902
			-3.000				1" Ice	3.183	3.260	111.077
							2" Ice	3.703	3.784	176.024
							4" Ice	4.846	4.935	353.751
1900MHz RRH (65MHz)	C	From Centroid-Le	4.000		-40.000	130.000	No Ice	2.698	2.771	60.000
		g	6.000				1/2" Ice	2.936	3.011	83.902
			0.000				1" Ice	3.183	3.260	111.077
							2" Ice	3.703	3.784	176.024
							4" Ice	4.846	4.935	353.751
2.4" Dia. x 6-ft	A	From Centroid-Le	4.000		0.000	130.000	No Ice	1.425	1.425	21.960
		g	-2.000				1/2" Ice	1.925	1.925	32.787
			0.000				1" Ice	2.294	2.294	47.674
							2" Ice	3.060	3.060	90.239
							4" Ice	4.702	4.702	230.803
2.4" Dia. x 6-ft	C	From Centroid-Le	4.000		0.000	130.000	No Ice	1.425	1.425	21.960
		g	2.000				1/2" Ice	1.925	1.925	32.787
			0.000				1" Ice	2.294	2.294	47.674
							2" Ice	3.060	3.060	90.239
							4" Ice	4.702	4.702	230.803
Platform Mount [LP 1201-1]	C	None			0.000	130.000	No Ice	23.100	23.100	2100.000
							1/2" Ice	26.800	26.800	2500.000
							1" Ice	30.500	30.500	2900.000
							2" Ice	37.900	37.900	3700.000
							4" Ice	52.700	52.700	5300.000
Miscellaneous [NA 510-1]	C	None			0.000	130.000	No Ice	6.000	6.000	255.700
							1/2" Ice	8.500	8.500	339.500
							1" Ice	11.000	11.000	409.120
							2" Ice	16.000	16.000	562.540
							4" Ice	26.000	26.000	869.380
**121**										
(2) LPA-80080-6CF-EDIN w/ Mount Pipe	A	From Centroid-Le	4.000		30.000	121.000	No Ice	4.560	10.740	46.215
		g	0.000				1/2" Ice	5.102	12.002	112.725
			0.000				1" Ice	5.608	12.981	187.098
							2" Ice	6.648	14.993	363.031
							4" Ice	8.832	19.232	857.357
(2) LPA-80080-6CF-EDIN w/ Mount Pipe	B	From Centroid-Le	4.000		30.000	121.000	No Ice	4.560	10.740	46.215
		g	3.000				1/2" Ice	5.102	12.002	112.725
			0.000				1" Ice	5.608	12.981	187.098



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	<b>Client</b>	Crown Castle	<b>Designed by</b>	Chris D. Crook, E.I.

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	CAAA Front ft <sup>2</sup>	CAAA Side ft <sup>2</sup>	Weight lb	
(2) FD9R6004/2C-3L	B	From Centroid-Le g	4.000 -6.000 0.000	30.000	121.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.000 0.136 0.196 0.343 0.740	3.100 5.399 8.787 19.608 62.872	
(2) FD9R6004/2C-3L	C	From Centroid-Le g	4.000 -6.000 0.000	30.000	121.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	0.000 0.136 0.196 0.343 0.740	3.100 5.399 8.787 19.608 62.872	
DB-T1-6Z-8AB-0Z	C	From Centroid-Le g	4.000 3.000 0.000	30.000	121.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	5.600 5.915 6.240 6.914 8.365	2.333 2.558 2.791 3.284 4.373	44.000 80.134 120.222 213.037 454.667
Platform Mount [LP 1201-1]	C	None		0.000	121.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	23.100 26.800 30.500 37.900 52.700	23.100 26.800 30.500 37.900 52.700	2100.000 2500.000 2900.000 3700.000 5300.000
**119**									
RRH2X40-AWS	A	From Centroid-Le g	4.000 0.000 0.000	30.000	121.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	2.522 2.753 2.993 3.499 4.615	1.589 1.795 2.010 2.465 3.479	44.000 61.396 81.692 131.758 275.237
RRH2X40-AWS	B	From Centroid-Le g	4.000 0.000 0.000	30.000	121.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	2.522 2.753 2.993 3.499 4.615	1.589 1.795 2.010 2.465 3.479	44.000 61.396 81.692 131.758 275.237
RRH2X40-AWS	C	From Centroid-Le g	4.000 0.000 0.000	30.000	121.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	2.522 2.753 2.993 3.499 4.615	1.589 1.795 2.010 2.465 3.479	44.000 61.396 81.692 131.758 275.237
**111**									
7770.00 w/ Mount Pipe	A	From Centroid-Le g	4.000 -6.000 1.000	20.000	111.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.119 6.626 7.128 8.164 10.360	4.254 5.014 5.711 7.155 10.412	55.379 102.815 156.641 286.582 664.786
7770.00 w/ Mount Pipe	B	From Centroid-Le g	4.000 -6.000 1.000	20.000	111.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.119 6.626 7.128 8.164 10.360	4.254 5.014 5.711 7.155 10.412	55.379 102.815 156.641 286.582 664.786
7770.00 w/ Mount Pipe	C	From Centroid-Le g	4.000 -6.000 1.000	20.000	111.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.119 6.626 7.128 8.164 10.360	4.254 5.014 5.711 7.155 10.412	55.379 102.815 156.641 286.582 664.786
TPA-65R-LCUUUU-H6 w/ Mount Pipe	A	From Centroid-Le g	4.000 0.000 1.000	20.000	111.000	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	8.406 9.052 9.664 10.917 13.544	9.676 10.937 11.909 13.907 18.118	96.507 176.353 264.392 468.653 1023.708

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	<b>Client</b>	Crown Castle	<b>Designed by</b>	Chris D. Crook, E.I.

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert</i> <i>ft ft ft</i>	<i>Azimuth Adjustment</i> <i>°</i>	<i>Placement</i> <i>ft</i>	<i>C<sub>AA</sub> Front</i> <i>ft<sup>2</sup></i>	<i>C<sub>AA</sub> Side</i> <i>ft<sup>2</sup></i>	<i>Weight</i> <i>lb</i>	
TPA-65R-LCUUUU-H6 w/ Mount Pipe	B	From Centroid-Le g	4.000	20.000	111.000	No Ice	8.406	9.676	96.507
			0.000			1/2" Ice	9.052	10.937	176.353
			1.000			1" Ice	9.664	11.909	264.392
						2" Ice	10.917	13.907	468.653
						4" Ice	13.544	18.118	1023.708
TPA-65R-LCUUUU-H6 w/ Mount Pipe	C	From Centroid-Le g	4.000	20.000	111.000	No Ice	8.406	9.676	96.507
			0.000			1/2" Ice	9.052	10.937	176.353
			1.000			1" Ice	9.664	11.909	264.392
						2" Ice	10.917	13.907	468.653
						4" Ice	13.544	18.118	1023.708
RRUS-11	A	From Centroid-Le g	4.000	20.000	111.000	No Ice	3.256	1.379	50.000
			0.000			1/2" Ice	3.498	1.558	70.872
			1.000			1" Ice	3.749	1.745	94.783
						2" Ice	4.277	2.146	152.503
						4" Ice	5.435	3.050	312.977
RRUS-11	B	From Centroid-Le g	4.000	20.000	111.000	No Ice	3.256	1.379	50.000
			0.000			1/2" Ice	3.498	1.558	70.872
			1.000			1" Ice	3.749	1.745	94.783
						2" Ice	4.277	2.146	152.503
						4" Ice	5.435	3.050	312.977
RRUS-11	C	From Centroid-Le g	4.000	20.000	111.000	No Ice	3.256	1.379	50.000
			0.000			1/2" Ice	3.498	1.558	70.872
			1.000			1" Ice	3.749	1.745	94.783
						2" Ice	4.277	2.146	152.503
						4" Ice	5.435	3.050	312.977
DC6-48-60-18-8F	A	From Centroid-Le g	4.000	20.000	111.000	No Ice	1.467	1.467	18.900
			0.000			1/2" Ice	1.667	1.667	36.615
			1.000			1" Ice	1.878	1.878	56.825
						2" Ice	2.333	2.333	105.337
						4" Ice	3.378	3.378	239.015
DTMABP7819VG12A	A	From Centroid-Le g	4.000	20.000	111.000	No Ice	1.139	0.391	19.180
			-6.000			1/2" Ice	1.284	0.488	26.485
			1.000			1" Ice	1.437	0.595	35.633
						2" Ice	1.769	0.833	60.234
						4" Ice	2.538	1.414	140.104
DTMABP7819VG12A	B	From Centroid-Le g	4.000	20.000	111.000	No Ice	1.139	0.391	19.180
			-6.000			1/2" Ice	1.284	0.488	26.485
			1.000			1" Ice	1.437	0.595	35.633
						2" Ice	1.769	0.833	60.234
						4" Ice	2.538	1.414	140.104
DTMABP7819VG12A	C	From Centroid-Le g	4.000	20.000	111.000	No Ice	1.139	0.391	19.180
			-6.000			1/2" Ice	1.284	0.488	26.485
			1.000			1" Ice	1.437	0.595	35.633
						2" Ice	1.769	0.833	60.234
						4" Ice	2.538	1.414	140.104
RRUS12/RRUS A2	A	From Centroid-Le g	4.000	20.000	111.000	No Ice	3.667	2.141	71.500
			0.000			1/2" Ice	3.924	2.347	98.979
			1.000			1" Ice	4.189	2.563	129.873
						2" Ice	4.745	3.019	202.684
						4" Ice	5.960	4.035	397.842
RRUS12/RRUS A2	B	From Centroid-Le g	4.000	20.000	111.000	No Ice	3.667	2.141	71.500
			0.000			1/2" Ice	3.924	2.347	98.979
			1.000			1" Ice	4.189	2.563	129.873
						2" Ice	4.745	3.019	202.684
						4" Ice	5.960	4.035	397.842
RRUS12/RRUS A2	C	From Centroid-Le	4.000	20.000	111.000	No Ice	3.667	2.141	71.500
			0.000			1/2" Ice	3.924	2.347	98.979

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	<b>Client</b>	Crown Castle	<b>Designed by</b>	Chris D. Crook, E.I.

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight lb	
		g	1.000						
Platform Mount [LP 1201-1]	C	None		0.000	111.000	1" Ice	4.189	2.563	129.873
						2" Ice	4.745	3.019	202.684
						4" Ice	5.960	4.035	397.842
						No Ice	23.100	23.100	2100.000
						1/2" Ice	26.800	26.800	2500.000
						1" Ice	30.500	30.500	2900.000
						2" Ice	37.900	37.900	3700.000
**70** OG-860/1920/GPS-A	C	From Face	3.000 0.000 0.000	0.000	70.000	No Ice	0.144	0.144	1.650
						1/2" Ice	0.233	0.233	3.534
						1" Ice	0.333	0.333	6.435
						2" Ice	0.567	0.567	15.904
						4" Ice	1.167	1.167	53.777
						No Ice	0.850	1.670	65.000
						1/2" Ice	1.140	2.340	79.000
Side Arm Mount [SO 701-1]	C	From Face	1.500 0.000 0.000	0.000	70.000	1" Ice	1.430	3.010	93.000
						2" Ice	2.010	4.350	121.000
						4" Ice	3.170	7.030	177.000
						***			

## Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft <sup>2</sup>	Weight lb	
**130**											
VHLP1-23	A	Paraboloid w/Shroud (HP)	From Centroid -Leg	4.000 -2.000 2.000	-40.000		130.000	1.275	No Ice	1.280	14.000
									1/2" Ice	1.450	19.340
									1" Ice	1.620	24.680
									2" Ice	1.960	35.360
VHLP2-23	C	Paraboloid w/Shroud (HP)	From Centroid -Leg	4.000 -2.000 2.000	0.000		130.000	2.180	4" Ice	2.640	56.720
									No Ice	3.730	31.000
									1/2" Ice	4.020	51.636
									1" Ice	4.310	72.272
VHLP2.5-23	C	Paraboloid w/Shroud (HP)	From Centroid -Leg	4.000 2.000 2.000	30.000		130.000	2.917	2" Ice	4.900	113.544
									4" Ice	6.060	196.088
									No Ice	6.680	47.600
									1/2" Ice	7.070	83.890
									1" Ice	7.460	120.170
***											

## Load Combinations

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	<b>Client</b>	Crown Castle	<b>Designed by</b>	Chris D. Crook, E.I.

<i>Comb. No.</i>	<i>Description</i>
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice
14	Dead+Ice+Temp
15	Dead+Wind 0 deg+Ice+Temp
16	Dead+Wind 30 deg+Ice+Temp
17	Dead+Wind 60 deg+Ice+Temp
18	Dead+Wind 90 deg+Ice+Temp
19	Dead+Wind 120 deg+Ice+Temp
20	Dead+Wind 150 deg+Ice+Temp
21	Dead+Wind 180 deg+Ice+Temp
22	Dead+Wind 210 deg+Ice+Temp
23	Dead+Wind 240 deg+Ice+Temp
24	Dead+Wind 270 deg+Ice+Temp
25	Dead+Wind 300 deg+Ice+Temp
26	Dead+Wind 330 deg+Ice+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37	Dead+Wind 300 deg - Service
38	Dead+Wind 330 deg - Service

**Maximum Tower Deflections - Service Wind**

<i>Section No.</i>	<i>Elevation</i>	<i>Horz. Deflection</i>	<i>Gov. Load Comb.</i>	<i>Tilt</i>	<i>Twist</i>
	<i>ft</i>	<i>in</i>		<i>°</i>	<i>°</i>
L1	147 - 133	42.130	30	2.292	0.016
L2	133 - 85.5	35.424	30	2.288	0.016
L3	89.25 - 42.75	16.025	30	1.765	0.005
L4	47.5 - 0	4.373	30	0.860	0.002

**Critical Deflections and Radius of Curvature - Service Wind**

<i>Elevation</i>	<i>Appurtenance</i>	<i>Gov. Load Comb.</i>	<i>Deflection</i>	<i>Tilt</i>	<i>Twist</i>	<i>Radius of Curvature</i>
<i>ft</i>			<i>in</i>	<i>°</i>	<i>°</i>	<i>ft</i>
132.000	VHLP1-23	30	34.946	2.285	0.016	34823

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	<b>Client</b> Crown Castle	<b>Designed by</b> Chris D. Crook, E.I.

Elevation	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
130.000	APXVTM14-C-120 w/ Mount Pipe	30	33.991	2.277	0.016	23643
121.000	(2) LPA-80080-6CF-EDIN w/ Mount Pipe	30	29.726	2.220	0.014	8737
111.000	7770.00 w/ Mount Pipe	30	25.117	2.114	0.011	5127
70.000	OG-860/1920/GPS-A	30	9.605	1.358	0.003	2495

**Maximum Tower Deflections - Design Wind**

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	147 - 133	121.440	5	6.611	0.046
L2	133 - 85.5	102.109	5	6.597	0.046
L3	89.25 - 42.75	46.211	5	5.092	0.016
L4	47.5 - 0	12.618	5	2.483	0.005

**Critical Deflections and Radius of Curvature - Design Wind**

Elevation	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
132.000	VHLP1-23	5	100.731	6.588	0.045	13165
130.000	APXVTM14-C-120 w/ Mount Pipe	5	97.977	6.567	0.045	8764
121.000	(2) LPA-80080-6CF-EDIN w/ Mount Pipe	5	85.687	6.404	0.040	3148
111.000	7770.00 w/ Mount Pipe	5	72.408	6.103	0.033	1828
70.000	OG-860/1920/GPS-A	5	27.708	3.917	0.008	872

**Compression Checks**

**Pole Design Data**

Section No.	Elevation ft	Size	L ft	L <sub>a</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio P/P <sub>a</sub>
L1	147 - 133 (1)	TP12.75x12.75x0.5	14.000	0.000	0.0	21.000	19.242	-867.088	404087.000	0.002
L2	133 - 85.5 (2)	TP29.418x19.537x0.313	47.500	0.000	0.0	39.000	28.503	-13173.500	1111600.000	0.012
L3	85.5 - 42.75 (3)	TP37.687x27.477x0.375	46.500	0.000	0.0	39.000	43.795	-20915.199	1708000.000	0.012
L4	42.75 - 0 (4)	TP45.83x35.894x0.438	47.500	0.000	0.0	39.000	63.947	-33230.000	2493920.000	0.013

<b>tnxTower</b>  <b>Tower Engineering Professionals</b> 326 Tryon Road Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> Beaumont Farm (BU 876310)	<b>Page</b> 14 of 15
	<b>Project</b> TEP No. 72875.96392	<b>Date</b> 14:54:13 09/13/16
	<b>Client</b> Crown Castle	<b>Designed by</b> Chris D. Crook, E.I.

### Pole Bending Design Data

Section No.	Elevation ft	Size	Actual $M_x$ lb-ft	Actual $f_{bx}$ ksi	Allow. $F_{bx}$ ksi	Ratio $\frac{f_{bx}}{F_{bx}}$	Actual $M_y$ lb-ft	Actual $f_{by}$ ksi	Allow. $F_{by}$ ksi	Ratio $\frac{f_{by}}{F_{by}}$
L1	147 - 133 (1)	TP12.75x12.75x0.5	3787.24	0.801	23.100	0.035	0.000	0.000	23.100	0.000
L2	133 - 85.5 (2)	TP29.418x19.537x0.313	667216. 2	40.691	39.000	1.043	0.000	0.000	39.000	0.000
L3	85.5 - 42.75 (3)	TP37.687x27.477x0.375	1661233 .333	51.459	39.000	1.319	0.000	0.000	39.000	0.000
L4	42.75 - 0 (4)	TP45.83x35.894x0.438	2998716 .667	50.795	39.000	1.302	0.000	0.000	39.000	0.000

### Pole Shear Design Data

Section No.	Elevation ft	Size	Actual $V$ lb	Actual $f_v$ ksi	Allow. $F_v$ ksi	Ratio $\frac{f_v}{F_v}$	Actual $T$ lb-ft	Actual $f_{vt}$ ksi	Allow. $F_{vt}$ ksi	Ratio $\frac{f_{vt}}{F_{vt}}$
L1	147 - 133 (1)	TP12.75x12.75x0.5	540.932	0.028	14.000	0.004	1.842	0.000	14.000	0.000
L2	133 - 85.5 (2)	TP29.418x19.537x0.313	21657.6 00	0.760	26.000	0.059	774.660	0.022	26.000	0.001
L3	85.5 - 42.75 (3)	TP37.687x27.477x0.375	25925.8 01	0.592	26.000	0.046	931.675	0.014	26.000	0.001
L4	42.75 - 0 (4)	TP45.83x35.894x0.438	30393.0 00	0.475	26.000	0.037	882.217	0.007	26.000	0.000

### Pole Interaction Design Data

Section No.	Elevation ft	Ratio $P$ $P_a$	Ratio $f_{bx}$ $F_{bx}$	Ratio $f_{by}$ $F_{by}$	Ratio $f_v$ $F_v$	Ratio $f_{vt}$ $F_{vt}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	147 - 133 (1)	0.002	0.035	0.000	0.004	0.000	0.037	1.333	H1-3+VT
L2	133 - 85.5 (2)	0.012	1.043	0.000	0.059	0.001	1.056	1.333	H1-3+VT
L3	85.5 - 42.75 (3)	0.012	1.319	0.000	0.046	0.001	1.332	1.333	H1-3+VT
L4	42.75 - 0 (4)	0.013	1.302	0.000	0.037	0.000	1.316	1.333	H1-3+VT

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	$P$ lb	$SF * P_{allow}$ lb	% Capacity	Pass Fail
L1	147 - 133	Pole	TP12.75x12.75x0.5	1	-867.088	538647.949	2.8	Pass
L2	133 - 85.5	Pole	TP29.418x19.537x0.313	2	-13173.500	1481762.73	79.2	Pass
L3	85.5 - 42.75	Pole	TP37.687x27.477x0.375	3	-20915.199	2276763.90	99.9	Pass
L4	42.75 - 0	Pole	TP45.83x35.894x0.438	4	-33230.000	3324395.22	98.7	Pass

<b><i>tnxTower</i></b>  <b><i>Tower Engineering Professionals</i></b> 326 Tryon Road Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	<b>Job</b> Beaumont Farm (BU 876310)	<b>Page</b> 15 of 15
	<b>Project</b> TEP No. 72875.96392	<b>Date</b> 14:54:13 09/13/16
	<b>Client</b> Crown Castle	<b>Designed by</b> Chris D. Crook, E.I.

<i>Section No.</i>	<i>Elevation ft</i>	<i>Component Type</i>	<i>Size</i>	<i>Critical Element</i>	<i>P lb</i>	<i>SF*P<sub>allow</sub> lb</i>	<i>% Capacity</i>	<i>Pass Fail</i>
						2		
							Summary	
						Pole (L3)	99.9	Pass
						<b>RATING =</b>	<b>99.9</b>	<b>Pass</b>

**APPENDIX B**  
**BASE LEVEL DRAWING**

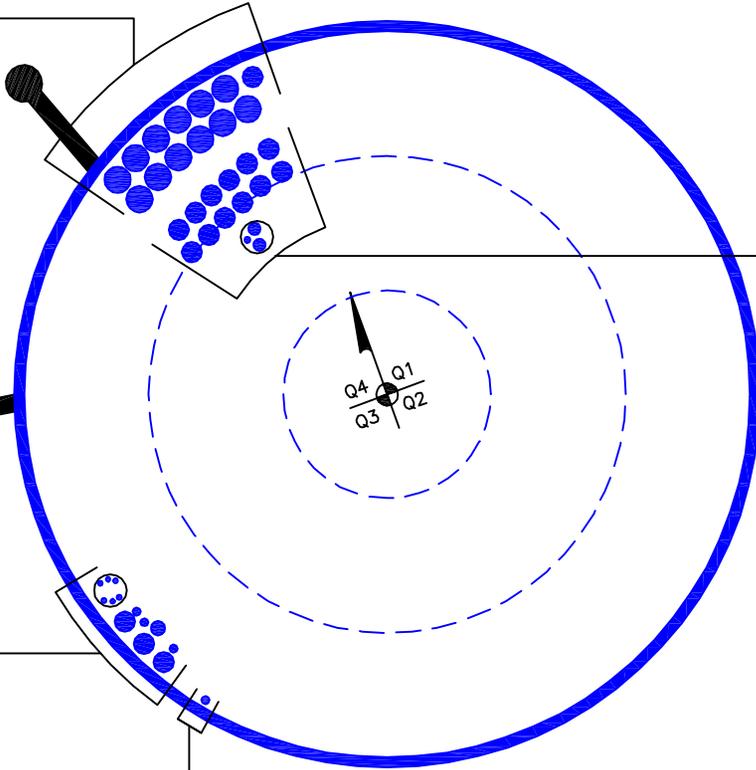


(INSTALLED)  
(1) 1-1/4" TO 121 FT LEVEL  
(12) 1-5/8" TO 121 FT LEVEL

CLIMBING PEGS  
W/SAFETY CLIMB

(INSTALLED-IN CONDUIT)  
(6) 5/16" TO 130 FT LEVEL  
(INSTALLED)  
(3) 1/2" TO 130 FT LEVEL  
(INSTALLED)  
(1) 7/8" TO 130 FT LEVEL  
(3) 1 1/4" TO 130 FT LEVEL

(INSTALLED)  
(1) 1/2" TO 70 FT LEVEL



(INSTALLED-TO BE REMOVED)  
(6) 1-1/4" TO 111 FT LEVEL  
(INSTALLED-IN CONDUIT)  
(1) 3/8" TO 111 FT LEVEL  
(2) 3/4" TO 111 FT LEVEL  
(INSTALLED)  
(6) 1-1/4" TO 111 FT LEVEL

**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

# Stiffened or Unstiffened, Exterior Flange Plate - Any Bolt Material TIA Rev F

## Site Data

BU#: 876310  
 Site Name: *Beaumont Farm*  
 App #: 345702 Rev. 3

Reactions		
Moment:	3.787	ft-kips
Axial:	0.867	kips
Shear:	0.541	kips
Elevation:	133	feet

Pole Manufacturer:	Other
--------------------	-------

If No stiffeners, Criteria: **AISC ASD** <-Only Applicable to Unstiffened Cases

Bolt Data		
Qty:	20	
Diameter (in.):	1.25	Bolt Fu: 105
Bolt Material:	A325	Bolt Fy: 81
N/A:		Bolt Fty: 44.00
N/A:		
Circle (in.):	24	

## Flange Bolt Results

Bolt Tension Capacity, **B**: 71.98 kips  
 Max Bolt directly applied T: 0.34 Kips  
 Min. PL "tc" for **B cap. w/o Pry**: 4.644 in  
 Min PL "treq" for actual **T w/ Pry**: 0.273 in  
 Min PL "t1" for actual **T w/o Pry**: 0.317 in  
 T allowable with Prying: 4.49 kips  
 Prying Force, Q: 0.00 kips  
 Total Bolt Tension=T+Q: 0.34 kips  
 Prying Bolt Stress Ratio=(T+Q)/(B): 0.5% **Pass**

Non-Rigid
Service, ASD
Fty*ASIF

Plate Data		
Diam:	28	in
Thick, t:	1	in
Grade (Fy):	50	ksi
Strength, Fu:	65	ksi
Single-Rod B-eff:	2.00	in

## Exterior Flange Plate Results

Flexural Check  
 Compression Side Plate Stress: 2.7 ksi  
 Allowable Plate Stress: 50.0 ksi  
 Compression Plate Stress Ratio: 5.4% **Pass**  
**No Prying**  
 Tension Side Stress Ratio, (treq/t)^2: 7.5% **Pass**

Non-Rigid
Service ASD
0.75*Fy*ASIF
Comp. Y.L. Length: 20.33

Stiffener Data (Welding at Both Sides)		
Config:	0	*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

**n/a**

## Stiffener Results

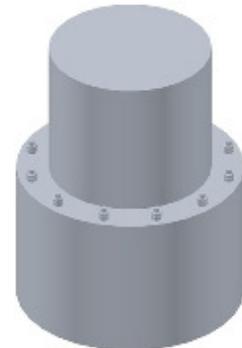
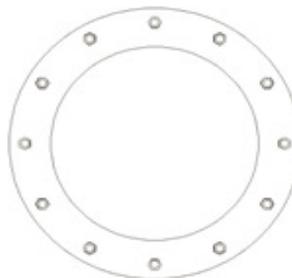
Horizontal Weld : n/a  
 Vertical Weld: n/a  
 Plate Flex+Shear, fb/Fb+(fv/Fv)^2: n/a  
 Plate Tension+Shear, ft/Ft+(fv/Fv)^2: n/a  
 Plate Comp. (AISC Bracket): n/a

## Pole Results

Pole Punching Shear Check: n/a

Pole Data		
Diam:	12.75	in
Thick:	0.5	in
Grade:	35	ksi
# of Sides:	0	"0" IF Round
Fu	60	ksi
Reinf. Fillet Weld	0	"0" if None

Stress Increase Factor	
ASIF:	1.333



\* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

## Square, Stiffened / Unstiffened Base Plate, Any Rod Material - Rev. F / G

- Assumptions:**
- 1) Rod groups at corners. Total # rods divisible by 4. Maximum total # of rods = 48 (12 per Corner).
  - 2) Rod Spacing = Straight Center-to-Center distance between any (2) adjacent rods (same corner)
  - 3) Clear space between bottom of leveling nut and top of concrete **not** exceeding  $(1) \times (\text{Rod Diameter})$

### Site Data

BU#: 876310  
 Site Name: *Beaumont Farm*  
 App #: 345702 Rev. 3

### Anchor Rod Data

Qty:	16	
Diam:	2.25	in
Rod Material:	A615-J	
Yield, Fy:	75	ksi
Strength, Fu:	100	ksi
Bolt Circle:	54	in
Anchor Spacing:	6	in

### Plate Data

W=Side:	54	in
Thick:	3	in
Grade:	50	ksi
Clip Distance:	6	in

### Stiffener Data (Welding at both sides)

Configuration:	Unstiffened	
Weld Type:		**
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

### Pole Data

Diam:	45.83	in
Thick:	0.4375	in
Grade:	65	ksi
# of Sides:	12	"0" IF Round

### Stress Increase Factor

ASD ASIF:	1.333	
-----------	-------	--

\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

### Base Reactions

TIA Revision:	F	
Unfactored Moment, M:	2998.721	ft-kips
Unfactored Axial, P:	33.261	kips
Unfactored Shear, V:	30.36	kips

### Anchor Rod Results

TIA F --> Maximum Rod Tension: 164.5 Kips  
 Allowable Tension: 195.0 Kips  
 Anchor Rod Stress Ratio: 84.4% **Pass**

### Base Plate Results

Base Plate Stress: 46.7 ksi  
 Allowable PL Bending Stress: 50.0 ksi  
 Base Plate Stress Ratio: 93.5% **Pass**

### Flexural Check

### PL Ref. Data

Yield Line (in):	30.54
Max PL Length:	30.54

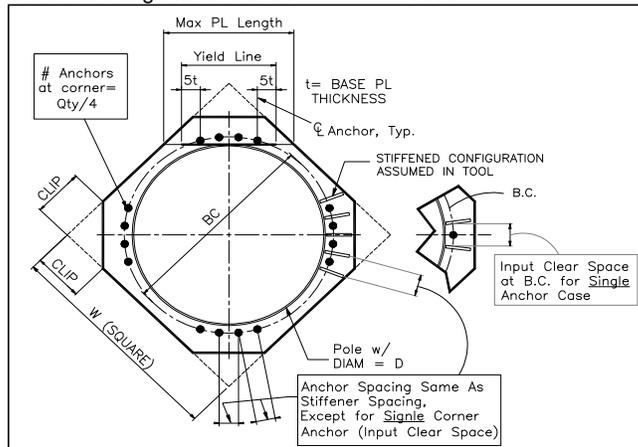
### N/A - Unstiffened

### Stiffener Results

Horizontal Weld : N/A  
 Vertical Weld: N/A  
 Plate Flex+Shear,  $f_b/F_b + (f_v/F_v)^2$ : N/A  
 Plate Tension+Shear,  $f_t/F_t + (f_v/F_v)^2$ : N/A  
 Plate Comp. (AISC Bracket): N/A

### Pole Results

Pole Punching Shear Check: N/A



JOB: Beaumont Farm (BU 876310): TEP# 72875.96392  
 SHEET NUMBER: 1 OF 2  
 CALCULATED BY: CDC DATE 9/13/2016  
 CHECKED BY: DTS DATE 9/13/2016

**Pad and Pier Foundation for Monopole - TIA-222-F**

<b>Q<sub>a</sub></b> , ALLOWABLE SOIL PRESS. (ksf)	20
NET or GROSS	NET
SOIL DENSITY (pcf)	165

<b>F'<sub>c</sub></b> (ksi)	3
<b>F'<sub>y</sub></b> (ksi)	60

**Base Reactions LC1: Maximum Wind**

<b>M</b> , MOMENT (k-ft)	2998.7
<b>P<sub>t</sub></b> , TOTAL DOWNLOAD (k)	33.3
<b>H</b> , HORIZONTAL SHEAR (k)	30.4

**Base Reaction LC 2: Ice Wind + Ice**

<b>M</b> (k-ft)	738.7
<b>P<sub>t</sub></b> (k)	45.2
<b>H</b> (k)	7.2

Try:

L (ft.)	B (ft.)	t (ft.)	Soil depth to TOP of mat (ft.)	Soil depth to BOT. of mat (ft.)	Pier dia./width (ft.)	Pier Height, h (cu.ft.)	Pier Shape
23	23	5	0	4.5	3.82	0.00	Square

<b>W<sub>m</sub></b> , Weight of Mat (k) =	396.8
<b>W<sub>p</sub></b> , Weight of Pier (k) =	0.0
<b>W<sub>s</sub></b> , WEIGHT OF SOIL (k) =	0.0

Concrete Vol. (cu ft) **97.96**

**CHECK DESIGN CRITERIA**

**CHECK STABILITY:**

	LC1	LC2
<b>Mst</b> = $P * (L/2) + (V_{f+s} * L/2) =$	4945.1 k-ft	5082.2 k-ft
<b>Mot</b> = $M + H*(t+h) =$	3150.5 k-ft	775 k-ft
<b>SF</b> = $Mot/Mst =$	1.57 > 1.5	6.56 > 1.5

**Capacity: 95.6%**

**CHECK BEARING PRESSURE**

	LC1	LC2
<b>P</b> = $P_t + W_f + W_s =$	430.0 k	441.9 k
<b>e</b> = $M / P =$	7.33 ft	1.75 ft
<b>L/6</b> =	3.83 ft	3.83 ft
<b>Width of Wedge, L'</b> =	12.52 ft	23.00 ft
<b>0 Deg Wind: Q<sub>max</sub></b> =	2.24 ksf	0.47 ksf
<b>45 Deg Wind: Q<sub>max</sub></b> =	3.32 ksf	0.63 ksf

**Capacity: 16.6%**

JOB: Beaumont Farm (BU 876310): TEP# 72875.96392  
 SHEET NUMBER: 2 OF 2  
 CALCULATED BY: CDC DATE 9/13/2016  
 CHECKED BY: DTS DATE 9/13/2016

**CHECK ONE WAY SHEAR**

$V_u =$    
 $V_c =$

**Capacity:** 28.38%

**CHECK TWO WAY SHEAR: PUNCHING + UNBALANCED MOMENT**

$V_u =$    
 $\phi V_c =$

**Capacity:** 13.97%

**CALCULATE REINFORCING REQUIRED**

$F'_c = 3.0$  ksi       $F'_y = 60.0$  ksi

Temp & Shrinkage reinforcing,  $A_{s,temp} =$   (ACI 318 Sec. 10.5.4)

**BOTTOM REINFORCING**

Bar Size =   
 Bar Spacing, c-c:   
 d = 54.9 in.

$M_u =$

$\phi Mn = 0.9 * A_s * F_y * d * (1 - 0.59 * A_s * F_y / (b * d * F'_c))$

Solution:  $A_{s,req} =$

Check,  $A_s =$

**Capacity:** 23.01%

**TOP REINFORCING**

Bar Size =   
 Bar Spacing, c-c:   
 d = 54.9 in.

$M_u =$

$\phi Mn = 0.9 * A_s * F_y * d * (1 - 0.59 * A_s * F_y / (b * d * F'_c))$

Solution:  $A_{s,req} =$

Bar Spacing, c-c:

**$A_{s,req} < A_{s,t}$ , Use  $A_{s,t}$**

Check,  $A_s =$

**Top Reinforcing O.K.**

**Capacity:** 11.66%



## RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

AT&T Existing Facility

Site ID: CT2154

Wallingford  
945 East Center Street  
Wallingford, CT 06492

**July 11, 2016**

**EBI Project Number: 6216003144**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general public allowable limit:	<b>8.22 %</b>



July 11, 2016

AT&T Mobility – New England  
Attn: Cameron Syme, RF Manager  
550 Cochituate Road  
Suite 550 – 13&14  
Framingham, MA 06040

## Emissions Analysis for Site: **CT2154 – Wallingford**

EBI Consulting was directed to analyze the proposed AT&T facility located at **945 East Center Street, Wallingford, CT**, for the purpose of determining whether the emissions from the Proposed AT&T Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limits for the 700 and 850 MHz Bands are approximately  $467 \mu\text{W}/\text{cm}^2$  and  $567 \mu\text{W}/\text{cm}^2$  respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 2300 MHz (WCS) bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## CALCULATIONS

Calculations were done for the proposed AT&T Wireless antenna facility located at **945 East Center Street, Wallingford, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since AT&T is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 UMTS channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 2 UMTS channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 2 LTE channels (700 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 4) 2 LTE channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 5) 2 GSM channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.



- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the **Powerwave 7770 and the CCI HPA-65R-BUU-H6** for transmission in the 700 MHz, 850 MHz and 1900 MHz (PCS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerlines of the proposed antennas are **112 feet** above ground level (AGL) for **Sector A**, **112 feet** above ground level (AGL) for **Sector B** and **112 feet** above ground level (AGL) for Sector C.
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.



## AT&T Site Inventory and Power Data by Antenna

Sector:	A	Sector:	B	Sector:	C
Antenna #:	<b>1</b>	Antenna #:	<b>1</b>	Antenna #:	<b>1</b>
Make / Model:	Powerwave 7770	Make / Model:	Powerwave 7770	Make / Model:	Powerwave 7770
Gain:	11.4 / 13.4 dBd	Gain:	11.4 / 13.4 dBd	Gain:	11.4 / 13.4 dBd
Height (AGL):	<b>112 feet</b>	Height (AGL):	<b>112 feet</b>	Height (AGL):	<b>112 feet</b>
Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power(W):	120 Watts	Total TX Power(W):	120 Watts	Total TX Power(W):	120 Watts
ERP (W):	2,140.89	ERP (W):	2,140.89	ERP (W):	2,140.89
Antenna A1 MPE%	<b>0.89 %</b>	Antenna B1 MPE%	<b>0.89 %</b>	Antenna C1 MPE%	<b>0.89 %</b>
Antenna #:	<b>2</b>	Antenna #:	<b>2</b>	Antenna #:	<b>2</b>
Make / Model:	CCI HPA-65R-BUU-H6	Make / Model:	CCI HPA-65R-BUU-H6	Make / Model:	CCI HPA-65R-BUU-H6
Gain:	11.95 / 14.75 dBd	Gain:	11.95 / 14.75 dBd	Gain:	11.95 / 14.75 dBd
Height (AGL):	<b>112 feet</b>	Height (AGL):	<b>112 feet</b>	Height (AGL):	<b>112 feet</b>
Frequency Bands	700 MHz / 1900 MHz (PCS)	Frequency Bands	700 MHz / 1900 MHz (PCS)	Frequency Bands	700 MHz / 1900 MHz (PCS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power(W):	240 Watts	Total TX Power(W):	240 Watts	Total TX Power(W):	240 Watts
ERP (W):	5,462.56	ERP (W):	5,462.56	ERP (W):	5,462.56
Antenna A2 MPE%	<b>2.43 %</b>	Antenna B2 MPE%	<b>2.43 %</b>	Antenna C2 MPE%	<b>2.43 %</b>
Antenna #:	<b>3</b>	Antenna #:	<b>3</b>	Antenna #:	<b>3</b>
Make / Model:	Powerwave 7770	Make / Model:	Powerwave 7770	Make / Model:	Powerwave 7770
Gain:	11.4 dBd	Gain:	11.4 dBd	Gain:	11.4 dBd
Height (AGL):	<b>112 feet</b>	Height (AGL):	<b>112 feet</b>	Height (AGL):	<b>112 feet</b>
Frequency Bands	850 MHz	Frequency Bands	850 MHz	Frequency Bands	850 MHz
Channel Count	2	Channel Count	2	Channel Count	2
Total TX Power(W):	60 Watts	Total TX Power(W):	60 Watts	Total TX Power(W):	60 Watts
ERP (W):	828.23	ERP (W):	828.23	ERP (W):	828.23
Antenna A3 MPE%	<b>0.47 %</b>	Antenna B3 MPE%	<b>0.47 %</b>	Antenna C3 MPE%	<b>0.47 %</b>

Site Composite MPE%	
Carrier	MPE%
AT&T – Max per sector	<b>3.79 %</b>
Verizon Wireless	3.10 %
Clearwire	0.12 %
Sprint	1.21 %
<b>Site Total MPE %:</b>	<b>8.22 %</b>

AT&T Sector A Total:	3.79 %
AT&T Sector B Total:	3.79 %
AT&T Sector C Total:	3.79 %
<b>Site Total:</b>	<b>8.22 %</b>

AT&T_ Max Values Per Sector	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
AT&T 850 MHz UMTS	2	414.12	112	2.65	850 MHz	567	0.47 %
AT&T 1900 MHz (PCS) UMTS	2	656.33	112	4.20	1900 MHz (PCS)	1000	0.42 %
AT&T 700 MHz LTE	2	940.05	112	6.02	700 MHz	467	1.29 %
AT&T 1900 MHz (PCS) LTE	2	1,791.23	112	11.46	1900 MHz (PCS)	1000	1.15 %
AT&T 850 MHz GSM	2	414.12	112	2.65	850 MHz	567	0.47 %
						<b>Total:*</b>	<b>3.79 %</b>

Note: Totals may vary by .01% due to summing of remainders



## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the AT&T facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

AT&T Sector	Power Density Value (%)
Sector A:	3.79 %
Sector B:	3.79 %
Sector C:	3.79 %
AT&T Maximum Total (per sector):	3.79 %
Site Total:	8.22 %
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **8.22 %** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.