



Crown Castle
3 Corporate Park Drive, Suite 101
Clifton Park, NY 12065

March 15, 2019

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: Notice of Exempt Modification for Crown Site BU: 873633
AT&T Site ID: 10035338
10 Bona Street, Milford, CT 06461
Latitude: 41° 13' 12.27"/ Longitude: -73° 4' 38.56"

Dear Ms. Bachman:

AT&T currently maintains (9) antennas at the 136-foot level of the existing 133-foot monopole at 10 Bona Street in Milford, Connecticut. The tower is owned by Crown Castle. The property is owned by 10 Bona Street LLC. AT&T intends to add (3) antennas, remove (6) TMAs, swap (6) RRUs, add (1) DC6 and (2) DC power cables.

The facility was approved by the City of Milford Planning and Zoning Commission on August 21, 2001. This approval included in the condition that the applicant will be required to pave 200+ of Bona Street from Erna Avenue to City standards for acceptance. This modification complies with those conditions.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.S.C.A. § 16-50j-73, a copy of this letter is being sent to The Honorable Benjamin G. Blake, Mayor, David B. Sulkis, City Planner for the City of Milford, as well as the property owner, and Crown Castle is the tower owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.

5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Sprint respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Anne Marie Zsamba.

Sincerely,

Anne Marie Zsamba, Esq.
Real Estate Specialist
3 Corporate Park Drive, Suite 101, Clifton Park, NY 12065
(201) 236-9224
annemarie.zsamba@crowncastle.com

Attachments:

Exhibit-A: Compound Plan and Elevation Depicting the Planned Changes
Exhibit-B: Structural Modification Report
Exhibit-C: General Power Density Table Report (RF Emissions Analysis Report)

cc: The Honorable Benjamin G. Blake, Mayor
City of Milford
70 West River Street
Milford, CT 06460

David B. Sulkis, City Planner
Planning & Zoning
70 West River Street
Milford, CT 06460

10 Bona Street LLC
92 Tumblebrook Drive
Milford, CT 06460

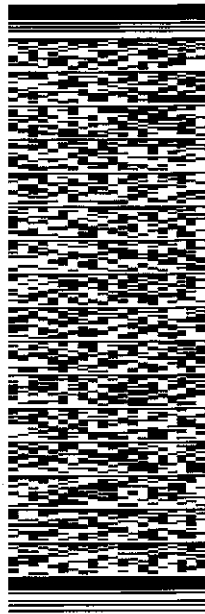
ORIGIN: GFLA (518) 373-3523
WILL STONE
CROWN CASTLE
3 CORPORATE PARK DRIVE
SUITE 101
CLIFTON PARK, NY 12065
UNITED STATES US

SHIP DATE: 18MAR19
ACTWGT: 3.00 LB
CAD: 104924194/NET14100
BILL SENDER

TO **MELANIE BACHMAN**
CONNECTICUT SITING COUNCIL
10 FRANKLIN SQUARE

NEW BRITAIN CT 06051
(860) 827-2951 REF: 1766 6990
PO: DEPT:

565J146D3Z3AD

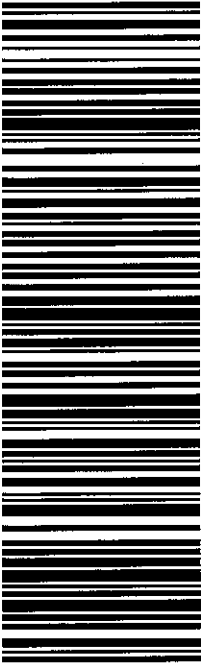


TRK# 7747 1292 3260
0201

MON - 18 MAR 10:30A
PRIORITY OVERNIGHT

SEBDLA

06051
CT-US BDL



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Use of this system constitutes your agreement to the service conditions in the current FedEx Service Guide, available on fedex.com. FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1,000, e.g. jewelry, precious metals, negotiable instruments and other items listed in our Service Guide. Written claims must be filed within strict time limits, see current FedEx Service Guide.

ORIGIN ID: GFLA (518) 373-3523
WILL STONE
CROWN CASTLE
3 CORPORATE PARK DRIVE
SUITE 101
CLIFTON PARK, NY 12065
UNITED STATES US

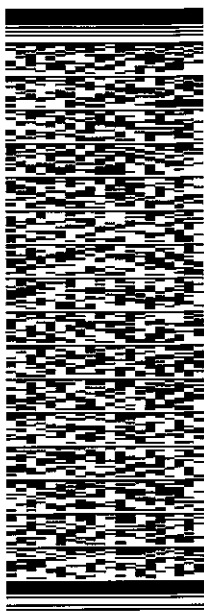
SHIP DATE: 15MAR19
ACTWGT: 1.50 LB
CAD: 104924194NN/ET4100

BILL SENDER

TO THE HONORABLE BENJAMIN G. BLAKE

CITY OF MILFORD
70 WEST RIVER STREET
MILFORD CT 06460
(201) 236-9224
REF: 1734.7690
INV:
PO: DEPT:

565J146D323AD



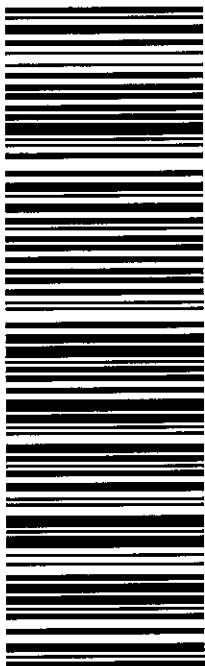
J191018010701uv

TRK# 7747 1293 8344
0201

MON - 18 MAR 10:30A
PRIORITY OVERNIGHT

SE OXCA

06460
CT-US BDL



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ORIGIN:ID-GFLA (518) 373-3523
MILL STONE
CROWN CASTLE
3 CORPORATE PARK DRIVE
SUITE 101
CLIFTON PARK, NY 12065
UNITED STATES US

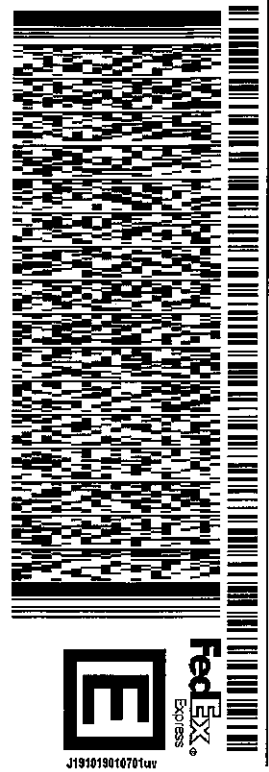
SHIP DATE: 15MAR19
ACTWGT: 1.50 LB
CAD: 104924194/NET4100
BILL SENDER

TO DAVID SULKIS, CITY PLANNER

PLANNING AND ZONING DEPARTMENT
70 WEST RIVER STREET
MILFORD CT 06460

REF: 1734.7880
INVT: (201) 236-9224
PO: DEPT:

565.J1/46D3/23AD



TRK# 7747 1295 1514
0201
MON - 18 MAR 10:30A
PRIORITY OVERNIGHT

SE OXCA
06460
CT-US BDL



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UNITED STATES US

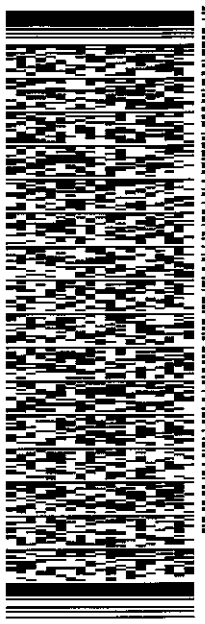
SHIP DATE: 15MAR19
ACTWGT: 1.50 LB
CAD: 10492419#/NET4100
BILL SENDER

TO 10 BONA STREET LLC

92 TUMBLEBROOK DRIVE

MILFORD CT 06460

(201) 236-9224 REF: 17347880
NY DEPT:
PO:

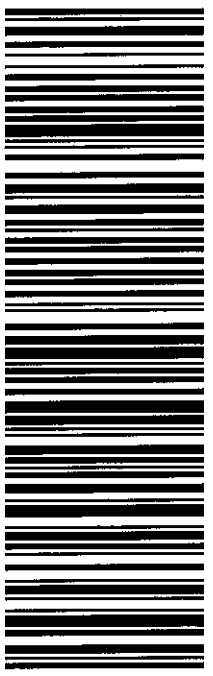


TRK# 7747 1296 5750
0201

MON - 18 MAR 10:30A
PRIORITY OVERNIGHT

SE OXCA

06460
CT-US BDL



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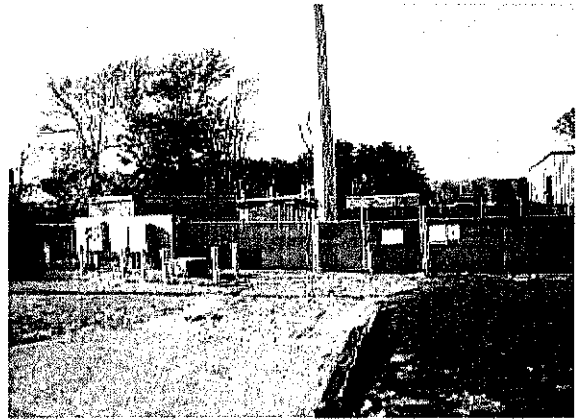


Property Information

Property Location	10 BONA ST
Owner	10 BONA STREET LLC
Co-Owner	C/O CROWN CASTLE
Mailing Address	PMB 353/SITE BU 873633 MCMURRAY PA 15317-2520
Land Use	434V CELL TOWER MDL-00
Land Class	I
Zoning Code	CDD1
Census Tract	1506

Neighborhood	F
Acreage	0.23
Utilities	All Public,Public Sewer
Lot Setting/Desc	Level
Additional Info	

Photo



Sketch

Primary Construction Details

Year Built	
Stories	
Building Style	
Building Use	
Building Condition	
Floors	
Total Rooms	

Bedrooms	
Full Bathrooms	
Half Bathrooms	
Bath Style	
Kitchen Style	
Roof Style	
Roof Cover	

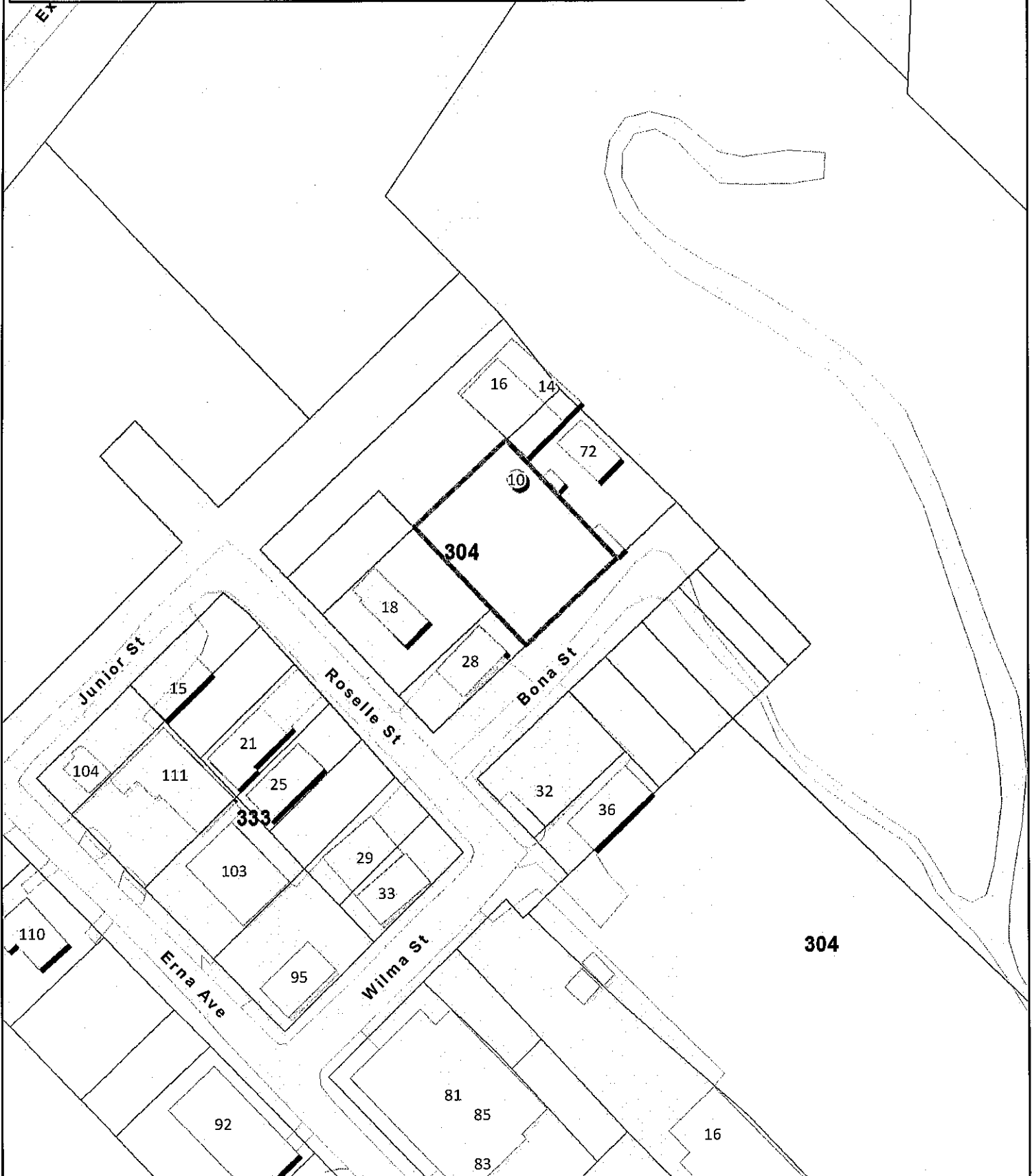
Exterior Walls	
Interior Walls	
Heating Type	
Heating Fuel	
AC Type	
Gross Bldg Area	
Total Living Area	



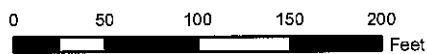
City of Milford, Connecticut. Assessment Parcel Map

Parcel ID: 12894

Address: 10 BONA ST

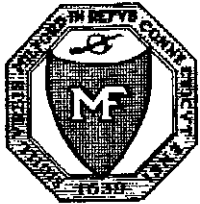


1 inch = 100 feet



Disclaimer: This map is for informational purposes only All information is subject to verification by any user. The City of Milford and its mapping contractors assume no legal responsibility for the information contained herein.

Map Produced: July 2016



City of Milford, Connecticut

Founded 1639

70 West River Street
Milford, CT 06460-3317
Telephone (203) 783-3245
Fax (203) 783-3303

ZONING BOARD OF APPEALS

THIS IS TO CERTIFY THAT, Integrated Mobile Services, Inc., was granted a variance by the Zoning Board of Appeals on March 13, 2001, for the property located at: **10 Bona Street**, Assessor's Map **43 & 53**, Block **304**, Parcel **69, 70, 71 & 72**, in the City of Milford, County of New Haven, State of Connecticut, of which, Joseph N. Clemente, 10 Bona Street, Milford, CT, is the owner.

A VARIANCE WAS GRANTED TO:

Vary Section 4.1.13 exceptions to height requirements to allow 150 ft. high monopole where 50 ft. is permitted.

"NO VARIANCE, SPECIAL PERMIT OR SPECIAL EXCEPTION GRANTED PURSUANT TO CHAPTER 124 OF ANY SPECIAL ACT SHALL BE EFFECTIVE UNTIL A COPY THEREOF...IS RECORDED IN THE LAND RECORDS OF THE TOWN IN WHICH SUCH PREMISES ARE LOCATED." P.A. 75-317

RECORDED: _____
DATE

ZONING BOARD OF APPEALS

CITY CLERK REC. NO. _____

BY: Errol Van Hise 1/3/01
Errol Van Hise, Chairman

Received for record **AUG 21 2001**
at 9:20:56 AM and recorded by me.
Colleen H. Japan
Milford City Clerk

FOR SIGNATURE 9-20-01

009587

FOR REVISION 9-42-1998

CITY OF MILFORD, CONNECTICUT

THIS IS TO CERTIFY THAT INTEGRATED MOBILE SERVICES, LLC

WAS GRANTED A SPECIAL PERMIT BY THE

MILFORD PLANNING & ZONING BOARD ON AUGUST 7, 2001 FOR

PROPERTY LOCATED AT 10 BONA STREET

MAP 43 & 53 BLOCK 304 PARCEL 69-72

IN THE CITY OF MILFORD, COUNTY OF NEW HAVEN, STATE OF

CONNECTICUT FOR WHICH JOSEPH N. CLEMENTE IS THE OWNER.

THE SPECIAL PERMIT WAS GRANTED:

To construct a 150' monopole communication tower with up to 4 equipment buildings (up to 12' x 26' size). A variance was granted March 13, 2001 by the ZBA to increase the allowable height from 50' to 150' in a GI zone. All construction shall be in accordance with plans as follows:

<u>SHEET</u>	<u>ENTITLED</u>	<u>DATED</u>
Title Sheet	Integrated Mobile Services, LLC	11/22/99
C-1	Site Plan	11/22/99; revised to 2/21/00
C-2	Site Details	11/10/99; revised to 2/21/00
C-3	Compound Plan & Elevation	11/10/99; revised to 2/21/00

The following city department reports apply: Letter from B. C. Kolwicz dated February 2, 2000; Police Department memo from Sgt. P. Ellsworth dated December 8, 1999. The applicant will be required to pave 200± of Bona Street from Erna Avenue to city standards for acceptance.

"NO VARIANCE, SPECIAL PERMIT OR SPECIAL EXCEPTION GRANTED PURSUANT TO CHAPTER 124 OF ANY SPECIAL ACT SHALL BE EFFECTIVE UNTIL A COPY THEREOF...IS RECORDED IN THE LAND RECORDS OF THE TOWN IN WHICH SUCH PREMISES ARE LOCATED."

P.A. 75-317
PLANNING & ZONING BOARD

RECORDED _____

CITY CLERK REC. NO. _____

BY:


WADE E. PIERCE
EXECUTIVE SECRETARY

Received for record AUG 21 2001
at 9:42:19 AM and recorded by me.
Alan H. Johnson
Milford City Clerk



OFFICE OF:
TOWN-CITY CLERK

City of Milford, Connecticut

To: Mayor James Richetelli
 Michele Collins, Chmn.
 Bd. of Aldermen
 Chief Louis LaVecchia, Fire Dept.
 Chief Thomas Flaherty, Police Dept.
 Bruce Kolwicz, Public Wks. Dir.
 Anthony Pinto

Marilyn Lipton, City Attorney
 William Gaffney, Assessor
 John Casey, City Engineer
 Wade Pierce, City Planner

From: Alan Jepson
 City Clerk

Date: March 5, 2003

Subject: Board of Alderman Referral Items No. 8a New
 Business

At the Regular Meeting of the Board of Aldermen held on March 3, 2003, the following action was taken:

- 8. New Business
 - a. Board of Aldermen approval is requested for the acceptance of Bona Street (for the length paved) as a City street per the recommendation of the Planning and Zoning Board.

Approved unanimously.



City of Milford, Connecticut

- Founded 1639 -

70 West River Street · Milford, CT 06460-3317
Tel 203-783-3245 FAX 203-783-3303

Planning and Zoning
Office

February 5, 2003

Mr. Carlos Centore
63-2 North Branford Road
Branford, CT 06405

RE: 10 BONA STREET – STREET ACCEPTANCE

Dear Mr. Centore:

At its meeting held on Tuesday, February 4, 2003 the Milford Planning & Zoning Board moved to recommend to the Board of Aldermen that Bona Street (for the length paved) be accepted as a city street; (in conjunction with CGS 8-24 municipal improvements). Letter of recommendation from the Director of Public Works Bruce Kolwicz dated January 24, 2003 is attached.

Very truly yours,

Wade E. Pierce for WEP

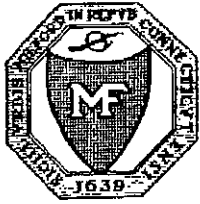
WADE E. PIERCE
Executive Secretary to the
Planning & Zoning Board

WEP/cv

C: Michele Collins, Chair
Board of Aldermen

Marilyn Lipton, City Attorney

Mayor James Richetelli, Jr.



City of Milford, Connecticut

OFFICE OF:
BRUCE C. KOLWICZ
DIRECTOR OF PUBLIC WORKS

RECEIVED
JAN 24 2003
PLANNING & ZONING
MILFORD, CT 06460

Date: January 24, 2003

To: Peter Crabtree, Planning & Zoning

From: Bruce C. Kolwicz, P.W. Director

Re: 10 Bona Street

This street can be accepted as a public street.

BCK:kh

PROJECT INFORMATION

- SCOPE OF WORK:
- ITEMS TO BE REMOVED FROM EXISTING TOWER & ON GROUND:
 - REMOVE (6) RRHs
 - INSTALL AT&T ANTENNA (600-10964) (TYP. OF 1 PER SECTOR, TOTAL OF 3).
 - INSTALL AT&T 4448 35812 (690700) (TYP. OF 1 PER SECTOR, TOTAL OF 3).
 - INSTALL AT&T 4448 35812 (690700) (TYP. OF 1 PER SECTOR, TOTAL OF 3).
 - INSTALL AT&T 4448 35812 (690700) (TYP. OF 1 PER SECTOR, TOTAL OF 3).
 - INSTALL (2) DC TRUNK CABLES.
- ITEMS TO BE MOUNTED INSIDE EXISTING SHELTERS:
- SWAP RR WITH 1216.
 - ADD 2ND XMU.
 - ADD RBS 9830.
- ITEMS TO REMAIN:
- (9) ANTENNAS, (3) RRHs, (2) THAS, (2) SURGE SUPPRESSOR, (2) COAX CABLES, (3) BIAS TEES, (2) FILTERS, (2) FIBER TRUNK CABLES AND (4) DC TRUNK CABLES.

SITE ADDRESS: 10 BONA STREET
MILFORD, CT 06461

LATITUDE (NAD 83): N 41° 13' 12.27"

LONGITUDE (NAD 83): W 73° 04' 38.56"

LANDLORD: CROWN CASTLE INTERNATIONAL
500 W. CLUMMINGS PARK, STE 3800
WOBURN, MA 01801

TYPE OF SITE: MONOPOLE/INDOOR

TOWER HEIGHT: 137'

RAD CENTER: 138'

CURRENT USE: TELECOMMUNICATIONS FACILITY

PROPOSED USE: TELECOMMUNICATIONS FACILITY

DRAWING INDEX

SHEET NO.	SHEET TITLE
T-1	TITLE SHEET
GN-1	GENERAL NOTES I
GN-2	GENERAL NOTES II
C-1	SITE PLAN
C-2	EQUIPMENT LAYOUT & PROPOSED TOWER ELEVATION
C-3	EXISTING & PROPOSED ANTENNA LAYOUT
C-4	EQUIPMENT DETAILS
RF-1	ANTENNA CHART & RF EQUIPMENT SCHEMATIC
G-1	GROUNDING DETAILS

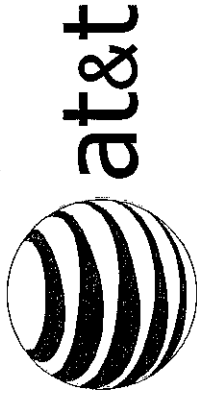
CROWN CASTLE SITE ID #: 873633
CROWN CASTLE SITE NAME: MILFORD

ENGINEERING

- 2018 CONNECTICUT STATE BUILDING CODE
 - 2018 AMENDMENT WITH 2015 INTERNATIONAL BUILDING CODE
 - 2008 CONNECTICUT STATE ELECTRICAL CODE
 - 2008 NATIONAL ELECTRICAL CODE
 - 2015 INTERNATIONAL ENERGY CONSERVATION CODE
 - 2017 NATIONAL ELECTRICAL CODE (NFPA 70 2017)
- ANSI/TIA-222-G

NOTE

ALL CONSTRUCTION ACTIVITIES ARE TO BE COMPLETED DIRECTLY THROUGH CROWN. CONTRACTOR MUST HAVE ALL NECESSARY PERMITS AND APPROVALS IN ORDER TO BEGIN. PRE-APPROVAL TO ENTER THE PROPERTY MUST BE OBTAINED. FOR ACCESS, AUTORIZATION, PLEASE CONTACT CROWN.



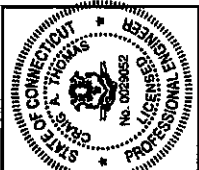
SITE NUMBER: CT2082

FA LOCATION CODE: 10035338
SITE NAME: MILFORD BONA ST
CROWN SITE NAME: MILFORD
PROJECT: LTE 4C/LTE 5C/4TX4RX SOFTWARE RETROFIT
PACE ID: MRCTB033598, MRCTB033681, MRCTB033703
BU #: 873633



CROWN CASTLE
3 CORPORATE PARK DRIVE
CLIFTON PARK, NY 10523

JACOBS
JACOBS ENGINEERING GROUP, INC.
19157 JAMES HOLLOW DR #100
BOZEMAN, WA 99718



PROJECT NO: BU03080

DRAWN BY: JAM
CHECKED BY: DAT

SUBMITTALS	
NO.	ISSUED FOR PERMITTING
1	ISSUED FOR PERMITTING

PERMITTING TO THE CONTRACTOR SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE AT&T REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

FA# 10035338
SITE# CT2082
MILFORD BONA ST
10 BONA STREET
MILFORD, CT 06461

TITLE SHEET

T-1

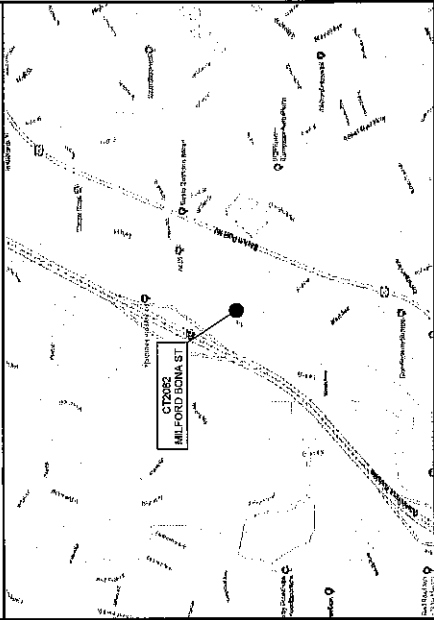
GENERAL NOTES

- THE FACILITY IS AN UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ACCESSIBLE BY TRAINED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES NOT REQUIRE ANY WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.
- CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE AT&T REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.



UNDERGROUND SERVICE ALERT
THE LAW REQUIRES
TWO WORKING DAYS NOTICE PRIOR TO ANY
EXCAVATION ACTIVITIES.
DIAL 811

VICINITY MAP



DIRECTIONS: 2082 MILFORD BONA ST IS NORTH (NEW ENGLAND THRUWAY), GET OFF AT EXIT 36 SCHOOL HOUSE ROAD, LEFT ON W. MAIN ST FOLLOW TO W. MAIN ST TURN RIGHT NEXT LEFT ON ROSSELLE ST ON TURN ON BONA ST. SITE LOCATED AT 68 BONA ST. MONOPOLE IS LOCATED BEHIND BUILDING.



5641 BROOKE STREET
EAST YORVICK, PA 17337



3 CORPORATE PARK DRIVE
SUITE 101
CLIFTON PARK, NY 10525



JACOBS ENGINEERING GROUP, INC.
128 ST. ANDREW AVENUE, 20TH FLOOR
BOSTON, MA 02116



PROJECT NO. 8802001

DRAWN BY: DAP

CHECKED BY: CAT

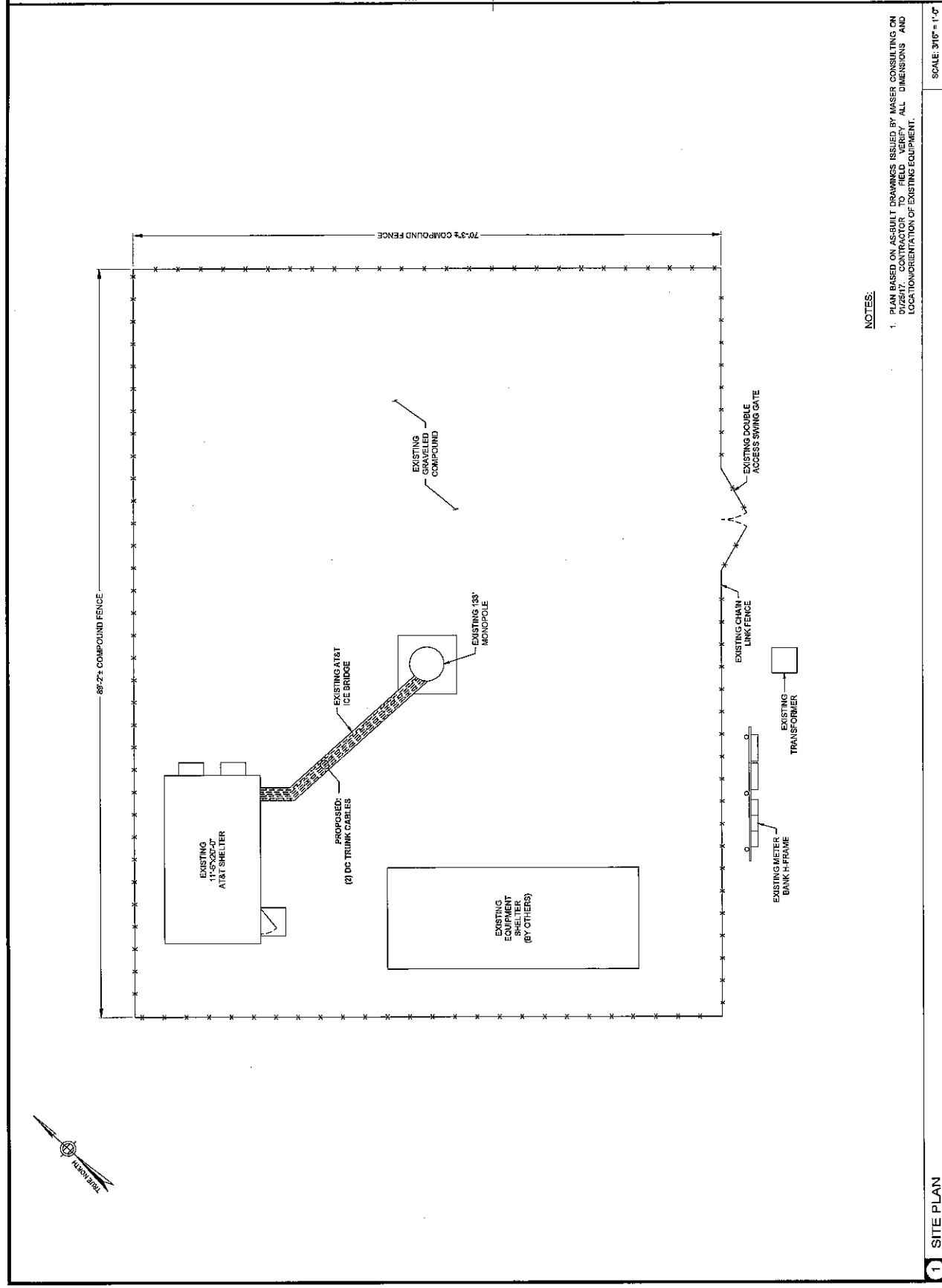
SUBMITTALS	
1	ISSUED FOR PERMITTING

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FA# 10063338
SITE# CT2082
MILFORD BONA ST
MILFORD, CT 06461

SITE PLAN

C-1



NOTES:

1. PLAN BASED ON AS-BUILT DRAWINGS ISSUED BY MASER CONSULTING ON BEHALF OF MASER CONSULTING ENGINEERS. ALL DIMENSIONS AND LOCATIONS OF EXISTING EQUIPMENT.

SCALE: 3/16" = 1'-0"



1000 WEST STREET
EAST WINDSOR, CT 06027

CROWN CASTLE
3 CORPORATE PARKWAY
SUITE 500
CLIFTON PARK, NY 10965

JACOBS
JACOBS ENGINEERING GROUP, INC.
19157 AUSTIN AVENUE, 5TH FLOOR
BOSTON, MA 02108



PROJECT NO: ECR2008

DRAWN BY: DAP

CHECKED BY: CAT

DATE: 11/11/11

SCALE: 1/8" = 1'-0"

DATE: 11/11/11

SCALE: 1/8" = 1'-0"

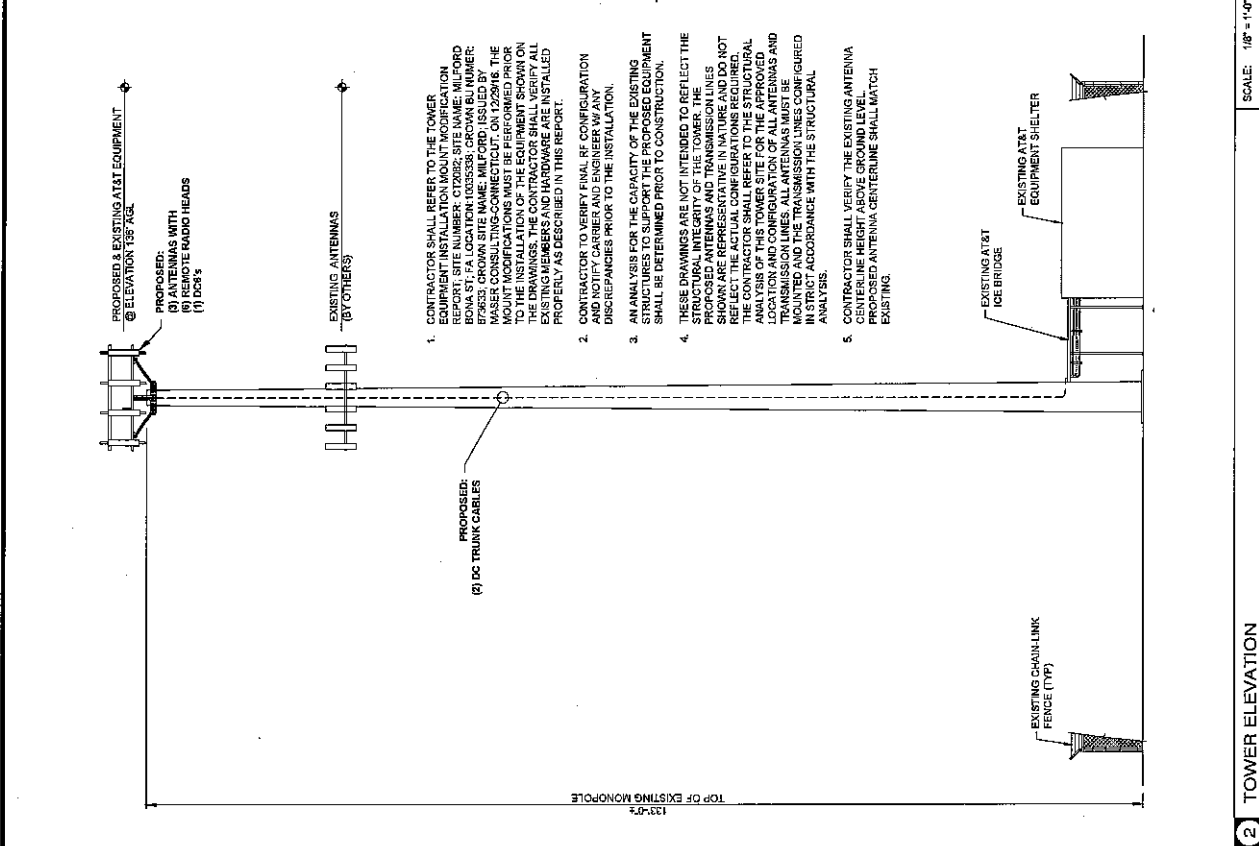
REVISIONS

NO.	DESCRIPTION	DATE
1	ISSUED FOR PERMITTING	

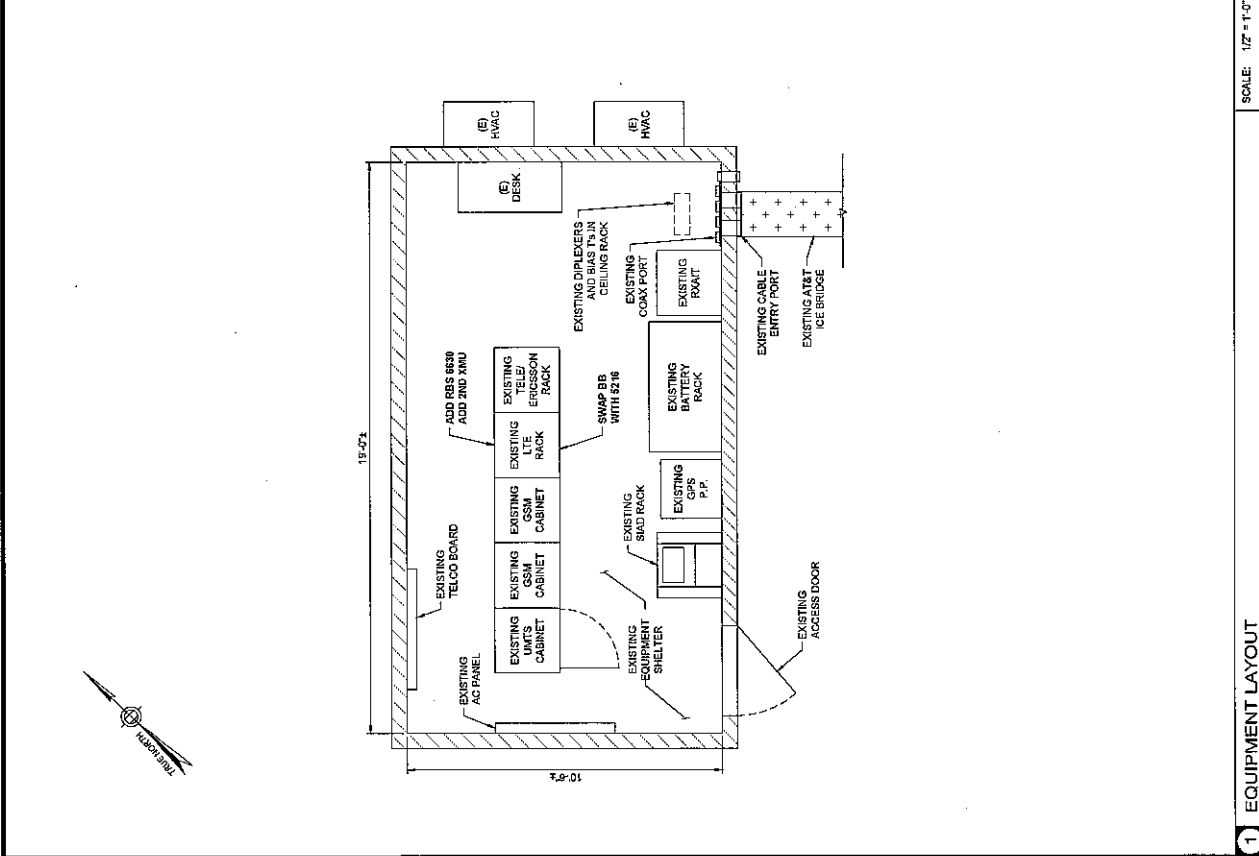
FAX: 10033338
SITE# CT2882
MILFORD BOVA ST
MILFORD, CT 06461

EQUIPMENT LAYOUT & PROPOSED TOWER ELEVATION

C-2



- CONTRACTOR SHALL REFER TO THE TOWER EQUIPMENT INSTALLATION MOUNT MODIFICATION REPORT, SITE NUMBER: CTSSES, SITE NAME: MILFORD BOVA ST, CITY: MILFORD, CT, PROJECT NUMBER: ECR2008, CROWN SITE NAME: MILFORD, ISSUED BY: MASER CONSULTING-CONNECTICUT, ON 12/29/16. THE MOUNT MODIFICATIONS MUST BE PERFORMED PRIOR TO THE INSTALLATION OF THE EQUIPMENT SHOWN ON THESE DRAWINGS. THE CONTRACTOR SHALL VERIFY ALL EXISTING MEMBERS AND HARDWARE ARE INSTALLED PROPERLY AS DESCRIBED IN THIS REPORT.
- CONTRACTOR TO VERIFY FINAL RF CONFIGURATION AND NOTIFY CARRIER AND ENGINEER W/ANY DISCREPANCIES PRIOR TO THE INSTALLATION.
- AN ANALYSIS FOR THE CAPACITY OF THE EXISTING CHAIN-LINK FENCE EQUIPMENT SHALL BE DETERMINED PRIOR TO CONSTRUCTION.
- THESE DRAWINGS ARE NOT INTENDED TO REFLECT THE STRUCTURAL INTEGRITY OF THE TOWER. THE STRUCTURAL INTEGRITY OF THE TOWER, THE FOUNDATION, AND THE ANCHOR BOLTS SHOWN ARE REPRESENTATIVE IN NATURE AND DO NOT REFLECT THE ACTUAL CONFIGURATIONS REQUIRED. THE CONTRACTOR SHALL REFER TO THE STRUCTURAL ANALYSIS REPORT FOR THE DETERMINATION OF THE LOCATION AND CONFIGURATION OF ALL ANTENNAS AND TRANSMISSION LINES. ALL ANTENNAS MUST BE MOUNTED AND THE TRANSMISSION LINES CONFIGURED IN STRICT ACCORDANCE WITH THE STRUCTURAL ANALYSIS.
- CONTRACTOR SHALL VERIFY THE EXISTING ANTENNA CENTERLINE HEIGHT ABOVE GROUND LEVEL. PROPOSED ANTENNA CENTERLINE SHALL MATCH EXISTING.





384 BRIDGE STREET
EAST SPRINGFIELD, NY 13507



3 CORPORATE PARK DRIVE
CLIFTON PARK, NY 12045



JACOBS ENGINEERING GROUP, INC.
198 ST. JAMES AVENUE, 5TH FLOOR
EASTON, MA 01028



PROJECT NO: BCC2008

DRAWN BY: DAP

CHECKED BY: CAT

SUBMITTALS

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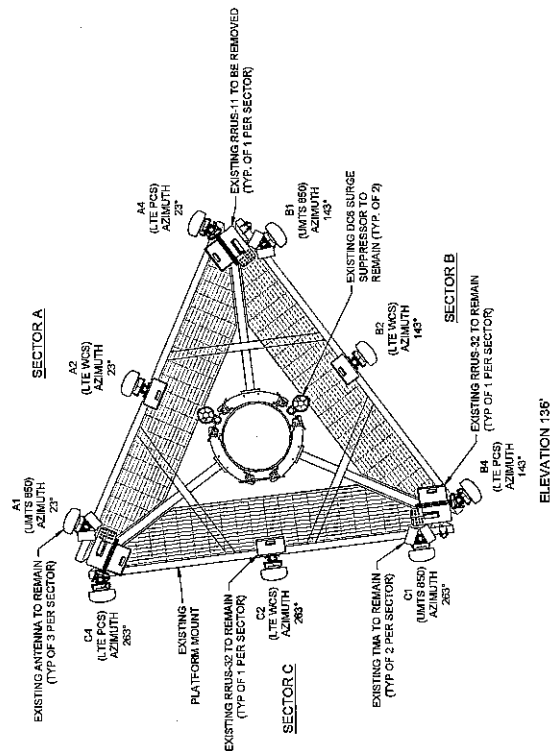
FAF 10063338
SITE: CT202
MILFORD BONA ST
MILFORD STREET
MILFORD, CT 06461

EXISTING & PROPOSED
ANTENNA LAYOUT

C-3

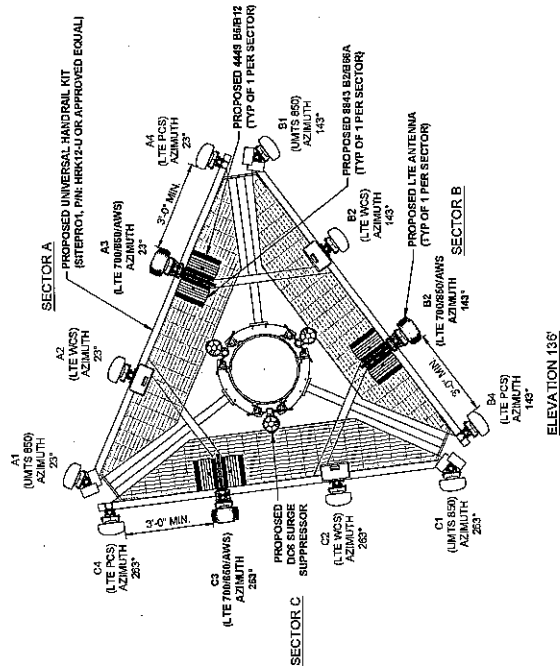
STRUCTURAL NOTE

- CONTRACTOR SHALL REFER TO THE TOWER EQUIPMENT INSTALLATION MOUNT MODIFICATION REPORT, SITE SPECIFIC TO THIS PROJECT, FOR THE ANTENNA MOUNT LOCATION. ADDRESS: CROWN BLDG NUMBER 028331, CROWN SITE NAME: MILFORD, ISSUED BY MASER CONSULTING-CONNECTICUT, ON 1/22/16. THE MOUNT MODIFICATIONS MUST BE PERFORMED PRIOR TO THE INSTALLATION OF THE ANTENNAS. THE CONTRACTOR SHALL VERIFY ALL EXISTING MEMBERS AND HARDWARE ARE INSTALLED PROPERLY AS DESCRIBED IN THIS REPORT.
- CONTRACTOR TO VERIFY FINAL RF CONFIGURATION AND NOTIFY CARRIER AND ENGINEER W/ ANY DISCREPANCIES PRIOR TO THE INSTALLATION.
- CONTRACTOR SHALL NOT USE MOUNTING MORE THAN 10 FEET PER ANTENNA MOUNTING PIPE. RELOCATE TO AN ADJACENT ANTENNA MOUNTING PIPE AS NEEDED.
- CONTRACTOR TO VERIFY FINAL RF CONFIGURATION AND NOTIFY CARRIER AND ENGINEER W/ ANY DISCREPANCIES PRIOR TO THE INSTALLATION.



SCALE: N.T.S.

1 EXISTING ANTENNA LAYOUT



SCALE: N.T.S.

2 PROPOSED ANTENNA LAYOUT

Date: February 04, 2019

Chanhdara Ratsavong
Crown Castle
3530 Toringdon Way Suite 300
Charlotte, NC 28277



Crown Castle
2000 Corporate Drive
Canonsburg, PA 15317
(724) 416-2000

Subject: Structural Analysis Report

Carrier Designation: AT&T Mobility Co-Locate
Carrier Site Number: 10035338
Carrier Site Name: MILFORD BONA ST

Crown Castle Designation: Crown Castle BU Number: 873633
Crown Castle Site Name: Milford
Crown Castle JDE Job Number: 531061
Crown Castle Work Order Number: 1690342
Crown Castle Order Number: 459065 Rev. 1

Engineering Firm Designation: Crown Castle Project Number: 1690342

Site Data: 10 Bona Street, MILFORD, New Haven County, CT
Latitude 41° 13' 12.27", Longitude -73° 4' 38.56"
133 Foot - Monopole Tower

Dear Chanhdara Ratsavong,

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration **Sufficient Capacity**

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Emma McCarty / M.B

Respectfully submitted by:

Maham Barimani, P.E.
Senior Project Engineer

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- Table 1 - Proposed Equipment Configuration
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- 3.2) Assumptions

4) ANALYSIS RESULTS

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- Table 5 – Tower Component Stresses vs. Capacity – LC5
- 4.1) Recommendations

5) APPENDIX A

- tnxTower Output

6) APPENDIX B

- Base Level Drawing

7) APPENDIX C

- Additional Calculations

1) INTRODUCTION

This tower is a 133 ft Monopole tower designed by SUMMIT.

2) ANALYSIS CRITERIA

Building Code: 2018 Connecticut State Building Code
TIA-222 Revision: TIA-222-H
Risk Category: II
Wind Speed: 125 mph
Exposure Category: C
Topographic Factor: 1
Ice Thickness: 1.5 in
Wind Speed with Ice: 50 mph
Service Wind Speed: 60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
133.0	136.0	3	andrew	SBNHH-1D65A w/ Mount Pipe	12 2 1 4 1 2	1-5/8 7/8 17/64 3/4 3/8 Conduit
		3	cci antennas	OPA-65R-LCUU-H4 w/ Mount Pipe		
		2	commscope	WCS-IMFQ-AMT		
		3	ericsson	RRUS 32		
		3	ericsson	RRUS 4449 B5/B12		
		3	ericsson	RRUS 8843 B2/B66A		
		3	kathrein	80010964 w/ Mount Pipe		
		3	powerwave technologies	1001940		
		3	powerwave technologies	7770.00 w/ Mount Pipe		
		6	powerwave technologies	LGP21401		
	3	raycap	DC6-48-60-18-8F			
	133.0	1	tower mounts	Miscellaneous [NA 509-3]		
133.0	1	tower mounts	Platform Mount [LP 1201-1]			

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
115.0	115.0	1	rfs celwave	DB-T1-6Z-8AB-0Z	-	-
		1	tower mounts	Side Arm Mount [SO 201-1]		
113.0	113.0	3	alcatel lucent	9442 RRH2X40-AWS	1 12	1-1/4 1-5/8
		3	amphenol	BXA-171063-8BF-EDIN-4 w/ Mount Pipe		
		3	antel	BXA-171063-8BF-2 w/ Mount Pipe		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
		6	antel	LPA-80090/4CF w/ Mount Pipe		
		3	swedcom	SWCP 2X5514 w/ Mount Pipe		
		1	tower mounts	Platform Mount [LP 303-1]		

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Criscuolo Shepard & Associates	1340372	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Summit	1340388	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Summit	1339622	CCISITES

3.1) Analysis Method

tnxTower (version 8.0.5.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built and maintained in accordance with the manufacturer's specifications.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	133 - 86.5	Pole	TP33.116x24x0.25	1	-11.61	1561.31	43.4	Pass
L2	86.5 - 39.75	Pole	TP41.78x31.7828x0.2813	2	-20.40	2219.08	65.6	Pass
L3	39.75 - 0	Pole	TP49.01x40.1884x0.375	3	-33.38	3555.76	57.5	Pass
							Summary	
						Pole (L2)	65.6	Pass
						Rating =	65.6	Pass

Table 5 - Tower Component Stresses vs. Capacity – LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	46.4	Pass
1	Base Plate	0	48.6	Pass
1,2	Drilled Pier Base Foundation Structure	0	40.0	Pass
1,2	Drilled Pier Base Foundation Soil Interaction	0	31.2	Pass
1,2	Pier and Pad Base Foundation Structure	0	30.8	Pass
1,2	Pier and Pad Base Foundation Soil Interaction	0	44.7	Pass

Structure Rating (max from all components) =	65.6%
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Notes:

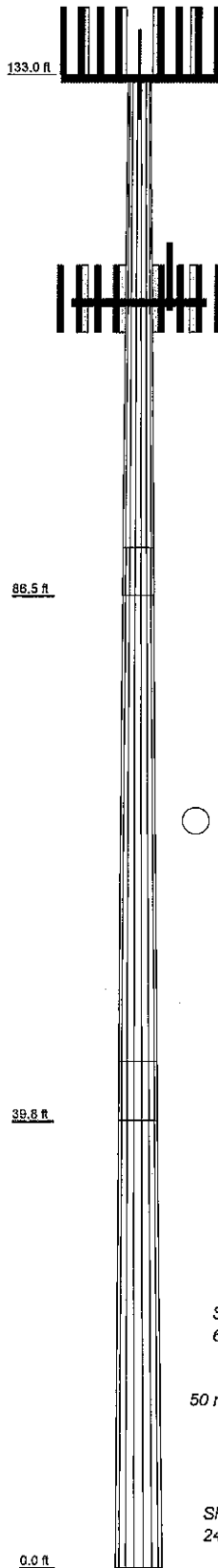
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.
- 2) It is unknown whether the foundation is a drilled shaft or pier and pad. Both designs were analyzed and determined to be sufficient.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

Section	1	2	3
Length (ft)	46.50	51.00	46.00
Number of Sides	18	18	18
Thickness (in)	0.2500	0.2813	0.3750
Socket Length (ft)	4.25	5.25	40.1854
Top Dia (in)	24.0000	31.7828	49.0100
Bot Dia (in)	33.1160	41.7800	
Grade		A607-65	
Weight (K)	3.6	5.7	8.1



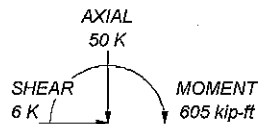
MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A607-65	65 ksi	80 ksi			

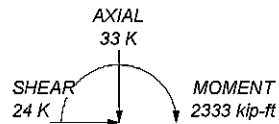
TOWER DESIGN NOTES

1. Tower is located in New Haven County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-H Standard.
3. Tower designed for a 125 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TIA-222-H Annex S
9. TOWER RATING: 65.6%


ALL REACTIONS
ARE FACTORED



TORQUE 0 kip-ft
50 mph WIND - 1.5000 in ICE



TORQUE 0 kip-ft
REACTIONS - 125 mph WIND

 CROWN CASTLE The Pathway to Possible	Crown Castle 2000 Corporate Drive Canonsburg, PA 15317 Phone: (724) 416-2000 FAX:		Job: BU 873633
	Project:	Client: Crown Castle	Drawn by: emccarty
	Code: TIA-222-H	Date: 02/04/19	App'd:
	Path:	Scale: NT	Dwg No. E-

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- 3) Tower is located in New Haven County, Connecticut.
- 4) Tower base elevation above sea level: 69.00 ft.
- 5) Basic wind speed of 125 mph.
- 6) Risk Category II.
- 7) Exposure Category C.
- 8) Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- 9) Topographic Category: 1.
- 10) Crest Height: 0.00 ft.
- 11) Nominal ice thickness of 1.5000 in.
- 12) Ice thickness is considered to increase with height.
- 13) Ice density of 56 pcf.
- 14) A wind speed of 50 mph is used in combination with ice.
- 15) Temperature drop of 50 °F.
- 16) Deflections calculated using a wind speed of 60 mph.
- 17) TIA-222-H Annex S.
- 18) A non-linear (P-delta) analysis was used.
- 19) Pressures are calculated at each section.
- 20) Stress ratio used in pole design is 1.05.
- 21) Tower analysis based on target reliabilities in accordance with Annex S.
- 22) Load Modification Factors used: $K_{es}(F_w) = 0.95$, $K_{es}(t_i) = 0.85$.
- 23) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification Use Code Stress Ratios ✓ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric	Distribute Leg Loads As Uniform Assume Legs Pinned ✓ Assume Rigid Index Plate ✓ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension ✓ Bypass Mast Stability Checks ✓ Use Azimuth Dish Coefficients ✓ Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination ✓ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs	Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation ✓ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption Poles ✓ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known
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Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft		in	in	in	in	

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	133.00-86.50	46.50	4.25	18	24.0000	33.1160	0.2500	1.0000	A607-65 (65 ksi)
L2	86.50-39.75	51.00	5.25	18	31.7828	41.7800	0.2813	1.1250	A607-65 (65 ksi)
L3	39.75-0.00	45.00		18	40.1884	49.0100	0.3750	1.5000	A607-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	I/Q in ²	w in	w/t
L1	24.3317	18.8456	1342.9976	8.4313	12.1920	110.1540	2687.7623	9.4246	3.7840	15.136
	33.5883	26.0792	3558.9750	11.6674	16.8229	211.5550	7122.6329	13.0421	5.3884	21.554
L2	33.0757	28.1211	3525.6028	11.1831	16.1457	218.3621	7055.8447	14.0632	5.0988	18.129
	42.3811	37.0454	8060.1282	14.7321	21.2242	379.7605	16130.862	18.5262	6.8583	24.385
L3	41.7956	47.3879	9489.9239	14.1337	20.4157	464.8347	18992.334	23.6984	6.4132	17.102
	49.7082	57.8878	17299.055	17.2654	24.8971	694.8227	34620.874	28.9494	7.9658	21.242

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _r	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontal in	Double Angle Stitch Bolt Spacing Redundants in
L1 133.00-86.50				1	1	1			
L2 86.50-39.75				1	1	1			
L3 39.75-0.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Number Per Row	Clear Spacing in	Width or Diameter r in	Perimeter r in	Weight klf

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	C _A A _A ft ² /ft	Weight klf
133								
LCF158-50A(1-5/8)	A	No	No	Inside Pole	133.00 - 0.00	12	No Ice	0.00
							1/2" Ice	0.00
							1" Ice	0.00
							2" Ice	0.00
6-8AWG 3 PAIR(7/8)	A	No	No	Inside Pole	133.00 - 0.00	2	No Ice	0.00
							1/2" Ice	0.00
							1" Ice	0.00
							2" Ice	0.00
A-	A	No	No	Inside Pole	133.00 - 0.00	1	No Ice	0.00

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Ice	C _A A _A ft ² /ft	Weight klf
DQZNB2YN1750 N(17/64)							1/2" Ice	0.00	0.00
							1" Ice	0.00	0.00
							2" Ice	0.00	0.00
FB-L98B-034-XXXXX(3/8)	A	No	No	Inside Pole	133.00 - 0.00	1	No Ice	0.00	0.00
							1/2" Ice	0.00	0.00
							1" Ice	0.00	0.00
							2" Ice	0.00	0.00
WR-VG86ST-BRD(3/4)	A	No	No	Inside Pole	133.00 - 0.00	4	No Ice	0.00	0.00
							1/2" Ice	0.00	0.00
							1" Ice	0.00	0.00
							2" Ice	0.00	0.00
2" Rigid Conduit	A	No	No	Inside Pole	133.00 - 0.00	2	No Ice	0.00	0.00
							1/2" Ice	0.00	0.00
							1" Ice	0.00	0.00
							2" Ice	0.00	0.00
113 561(1-5/8)	C	No	No	Inside Pole	113.00 - 0.00	12	No Ice	0.00	0.00
							1/2" Ice	0.00	0.00
							1" Ice	0.00	0.00
							2" Ice	0.00	0.00
LDF6-50A(1-1/4)	C	No	No	Inside Pole	113.00 - 0.00	1	No Ice	0.00	0.00
							1/2" Ice	0.00	0.00
							1" Ice	0.00	0.00
							2" Ice	0.00	0.00
***** ***									

Feed Line/Linear Appurtenances Section Areas

Tower Section n	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	133.00-86.50	A	0.000	0.000	0.000	0.000	0.88
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.45
L2	86.50-39.75	A	0.000	0.000	0.000	0.000	0.89
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.79
L3	39.75-0.00	A	0.000	0.000	0.000	0.000	0.75
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.67

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section n	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _A A _A In Face ft ²	C _A A _A Out Face ft ²	Weight K
L1	133.00-86.50	A	1.437	0.000	0.000	0.000	0.000	0.88
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.45
L2	86.50-39.75	A	1.359	0.000	0.000	0.000	0.000	0.89
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.79
L3	39.75-0.00	A	1.213	0.000	0.000	0.000	0.000	0.75
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.67

Feed Line Center of Pressure

Section	Elevation	CP _x	CP _z	CP _x	CP _z
	ft	in	in	Ice in	Ice in
L1	133.00-86.50	0.0000	0.0000	0.0000	0.0000
L2	86.50-39.75	0.0000	0.0000	0.0000	0.0000
L3	39.75-0.00	0.0000	0.0000	0.0000	0.0000

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
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Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A _A Front	C _A A _A Side	Weight	
			Horz Lateral	Vert						
			ft	ft		ft	ft ²	ft ²	K	
Lighting Rod 3/4" x 4'	C	None			0.0000	133.00	No Ice	0.30	0.30	0.03
							1/2" Ice	0.71	0.71	0.03
							Ice	1.00	1.00	0.04
							1" Ice	1.52	1.52	0.06
							2" Ice			
8'x2" Antenna Mount Pipe	A	None			0.0000	133.00	No Ice	1.90	1.90	0.03
							1/2" Ice	2.73	2.73	0.04
							Ice	3.40	3.40	0.06
							1" Ice	4.40	4.40	0.12
							2" Ice			
133 7770.00 w/ Mount Pipe	A	From Leg	4.00	0.00	0.0000	133.00	No Ice	5.75	4.25	0.06
							1/2" Ice	6.18	5.01	0.10
							Ice	6.61	5.71	0.16
							1" Ice	7.49	7.16	0.29
							2" Ice			
7770.00 w/ Mount Pipe	B	From Leg	4.00	0.00	0.0000	133.00	No Ice	5.75	4.25	0.06
							1/2" Ice	6.18	5.01	0.10
							Ice	6.61	5.71	0.16
							1" Ice	7.49	7.16	0.29
							2" Ice			
7770.00 w/ Mount Pipe	C	From Leg	4.00	0.00	0.0000	133.00	No Ice	5.75	4.25	0.06
							1/2" Ice	6.18	5.01	0.10
							Ice	6.61	5.71	0.16
							1" Ice	7.49	7.16	0.29
							2" Ice			
(2) LGP21401	A	From Leg	4.00	0.00	0.0000	133.00	No Ice	1.10	0.21	0.01
							1/2" Ice	1.24	0.27	0.02
							Ice	1.38	0.35	0.03
							1" Ice	1.69	0.52	0.05
							2" Ice			
(2) LGP21401	B	From Leg	4.00	0.00	0.0000	133.00	No Ice	1.10	0.21	0.01
							1/2" Ice	1.24	0.27	0.02
							Ice	1.38	0.35	0.03
							1" Ice	1.69	0.52	0.05
							2" Ice			

Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C _A A _A Front	C _A A _A Side	Weight
			Horz	Lateral	Vert					
			ft	ft	ft	°	ft	ft ²	ft ²	K
(2) LGP21401	C	From Leg	4.00	0.0000	133.00		No Ice	1.10	0.21	0.01
			0.00				1/2"	1.24	0.27	0.02
			3.00				Ice	1.38	0.35	0.03
							1" Ice	1.69	0.52	0.05
							2" Ice			
DC6-48-60-18-8F	A	From Leg	4.00	0.0000	133.00		No Ice	0.79	0.79	0.02
			0.00				1/2"	1.27	1.27	0.04
			3.00				Ice	1.45	1.45	0.05
							1" Ice	1.83	1.83	0.10
							2" Ice			
OPA-65R-LCUU-H4 w/ Mount Pipe	A	From Leg	4.00	0.0000	133.00		No Ice	6.18	4.55	0.08
			0.00				1/2"	6.57	5.16	0.13
			3.00				Ice	6.98	5.78	0.19
							1" Ice	7.82	7.07	0.33
							2" Ice			
OPA-65R-LCUU-H4 w/ Mount Pipe	B	From Leg	4.00	0.0000	133.00		No Ice	6.18	4.55	0.08
			0.00				1/2"	6.57	5.16	0.13
			3.00				Ice	6.98	5.78	0.19
							1" Ice	7.82	7.07	0.33
							2" Ice			
OPA-65R-LCUU-H4 w/ Mount Pipe	C	From Leg	4.00	0.0000	133.00		No Ice	6.18	4.55	0.08
			0.00				1/2"	6.57	5.16	0.13
			3.00				Ice	6.98	5.78	0.19
							1" Ice	7.82	7.07	0.33
							2" Ice			
SBNHH-1D65A w/ Mount Pipe	A	From Leg	4.00	0.0000	133.00		No Ice	5.95	5.19	0.06
			0.00				1/2"	6.39	5.96	0.11
			3.00				Ice	6.82	6.66	0.17
							1" Ice	7.71	8.09	0.32
							2" Ice			
SBNHH-1D65A w/ Mount Pipe	B	From Leg	4.00	0.0000	133.00		No Ice	5.95	5.19	0.06
			0.00				1/2"	6.39	5.96	0.11
			3.00				Ice	6.82	6.66	0.17
							1" Ice	7.71	8.09	0.32
							2" Ice			
SBNHH-1D65A w/ Mount Pipe	C	From Leg	4.00	0.0000	133.00		No Ice	5.95	5.19	0.06
			0.00				1/2"	6.39	5.96	0.11
			3.00				Ice	6.82	6.66	0.17
							1" Ice	7.71	8.09	0.32
							2" Ice			
80010964 w/ Mount Pipe	A	From Leg	4.00	0.0000	133.00		No Ice	10.23	5.51	0.11
			0.00				1/2"	10.74	6.37	0.18
			3.00				Ice	11.24	7.12	0.26
							1" Ice	12.25	8.64	0.45
							2" Ice			
80010964 w/ Mount Pipe	B	From Leg	4.00	0.0000	133.00		No Ice	10.23	5.51	0.11
			0.00				1/2"	10.74	6.37	0.18
			3.00				Ice	11.24	7.12	0.26
							1" Ice	12.25	8.64	0.45
							2" Ice			
80010964 w/ Mount Pipe	C	From Leg	4.00	0.0000	133.00		No Ice	10.23	5.51	0.11
			0.00				1/2"	10.74	6.37	0.18
			3.00				Ice	11.24	7.12	0.26
							1" Ice	12.25	8.64	0.45
							2" Ice			
RRUS 32	A	From Leg	4.00	0.0000	133.00		No Ice	2.86	1.78	0.06
			0.00				1/2"	3.08	1.97	0.08
			3.00				Ice	3.32	2.17	0.10
							1" Ice	3.81	2.58	0.16
							2" Ice			
RRUS 32	B	From Leg	4.00	0.0000	133.00		No Ice	2.86	1.78	0.06
			0.00				1/2"	3.08	1.97	0.08
			3.00				Ice	3.32	2.17	0.10
							1" Ice	3.81	2.58	0.16
							2" Ice			

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement		C_{AA}	C_{AA}	Weight
			Horz	Lateral				Front	Side	
			ft	ft			ft ²	ft ²		
RRUS 32	C	From Leg	4.00	0.0000	133.00	No Ice	2.86	1.78	0.06	
			0.00			1/2"	3.08	1.97	0.08	
			3.00			Ice	3.32	2.17	0.10	
						1" Ice	3.81	2.58	0.16	
						2" Ice				
1001940	A	From Leg	4.00	0.0000	133.00	No Ice	0.18	0.08	0.00	
			0.00			1/2"	0.23	0.13	0.00	
			3.00			Ice	0.30	0.18	0.01	
						1" Ice	0.44	0.30	0.01	
						2" Ice				
1001940	B	From Leg	4.00	0.0000	133.00	No Ice	0.18	0.08	0.00	
			0.00			1/2"	0.23	0.13	0.00	
			3.00			Ice	0.30	0.18	0.01	
						1" Ice	0.44	0.30	0.01	
						2" Ice				
1001940	C	From Leg	4.00	0.0000	133.00	No Ice	0.18	0.08	0.00	
			0.00			1/2"	0.23	0.13	0.00	
			3.00			Ice	0.30	0.18	0.01	
						1" Ice	0.44	0.30	0.01	
						2" Ice				
RRUS 8843 B2/B66A	A	From Leg	4.00	0.0000	133.00	No Ice	1.64	1.35	0.07	
			0.00			1/2"	1.80	1.50	0.09	
			3.00			Ice	1.97	1.65	0.11	
						1" Ice	2.32	1.99	0.16	
						2" Ice				
RRUS 8843 B2/B66A	B	From Leg	4.00	0.0000	133.00	No Ice	1.64	1.35	0.07	
			0.00			1/2"	1.80	1.50	0.09	
			3.00			Ice	1.97	1.65	0.11	
						1" Ice	2.32	1.99	0.16	
						2" Ice				
RRUS 8843 B2/B66A	C	From Leg	4.00	0.0000	133.00	No Ice	1.64	1.35	0.07	
			0.00			1/2"	1.80	1.50	0.09	
			3.00			Ice	1.97	1.65	0.11	
						1" Ice	2.32	1.99	0.16	
						2" Ice				
RRUS 4449 B5/B12	A	From Leg	4.00	0.0000	133.00	No Ice	1.97	1.41	0.07	
			0.00			1/2"	2.14	1.56	0.09	
			3.00			Ice	2.33	1.73	0.11	
						1" Ice	2.72	2.07	0.16	
						2" Ice				
RRUS 4449 B5/B12	B	From Leg	4.00	0.0000	133.00	No Ice	1.97	1.41	0.07	
			0.00			1/2"	2.14	1.56	0.09	
			3.00			Ice	2.33	1.73	0.11	
						1" Ice	2.72	2.07	0.16	
						2" Ice				
RRUS 4449 B5/B12	C	From Leg	4.00	0.0000	133.00	No Ice	1.97	1.41	0.07	
			0.00			1/2"	2.14	1.56	0.09	
			3.00			Ice	2.33	1.73	0.11	
						1" Ice	2.72	2.07	0.16	
						2" Ice				
WCS-IMFQ-AMT	A	From Leg	4.00	0.0000	133.00	No Ice	0.99	0.64	0.03	
			0.00			1/2"	1.11	0.75	0.04	
			3.00			Ice	1.25	0.86	0.05	
						1" Ice	1.53	1.11	0.08	
						2" Ice				
WCS-IMFQ-AMT	B	From Leg	4.00	0.0000	133.00	No Ice	0.99	0.64	0.03	
			0.00			1/2"	1.11	0.75	0.04	
			3.00			Ice	1.25	0.86	0.05	
						1" Ice	1.53	1.11	0.08	
						2" Ice				
DC6-48-60-18-8F	B	From Leg	4.00	0.0000	133.00	No Ice	0.79	0.79	0.02	
			0.00			1/2"	1.27	1.27	0.04	
			3.00			Ice	1.45	1.45	0.05	
						1" Ice	1.83	1.83	0.10	
						2" Ice				

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustmen t	Placement ft		C _A A _A Front ft ²	C _A A _A Side ft ²	Weight K
DC6-48-60-18-8F	C	From Leg	4.00	0.0000	133.00	No Ice	0.79	0.79	0.02
			0.00			1/2"	1.27	1.27	0.04
			3.00			Ice	1.45	1.45	0.05
						1" Ice	1.83	1.83	0.10
						2" Ice			
Platform Mount [LP 1201-1]	C	None		0.0000	133.00	No Ice	23.10	23.10	2.10
						1/2"	26.80	26.80	2.50
						Ice	30.50	30.50	2.90
						1" Ice	37.90	37.90	3.70
						2" Ice			
Miscellaneous [NA 509-3]	C	None		0.0000	133.00	No Ice	11.84	11.84	0.28
						1/2"	16.96	16.96	0.30
						Ice	22.08	22.08	0.32
						1" Ice	32.32	32.32	0.36
						2" Ice			
115 DB-T1-6Z-8AB-0Z	B	From Leg	2.00	0.0000	115.00	No Ice	4.80	2.00	0.04
			0.00			1/2"	5.07	2.19	0.08
			0.00			Ice	5.35	2.39	0.12
						1" Ice	5.93	2.81	0.21
						2" Ice			
Side Arm Mount [SO 201-1]	B	None		0.0000	115.00	No Ice	2.96	2.11	0.10
						1/2"	4.10	2.93	0.12
						Ice	5.24	3.75	0.14
						1" Ice	7.52	5.39	0.18
						2" Ice			
113 BXA-171063-8BF-EDIN-4 w/ Mount Pipe	A	From Leg	4.00	0.0000	113.00	No Ice	3.18	3.35	0.03
			0.00			1/2"	3.56	3.97	0.06
			0.00			Ice	3.93	4.60	0.10
						1" Ice	4.69	5.89	0.19
						2" Ice			
BXA-171063-8BF-EDIN-4 w/ Mount Pipe	B	From Leg	4.00	0.0000	113.00	No Ice	3.18	3.35	0.03
			0.00			1/2"	3.56	3.97	0.06
			0.00			Ice	3.93	4.60	0.10
						1" Ice	4.69	5.89	0.19
						2" Ice			
BXA-171063-8BF-EDIN-4 w/ Mount Pipe	C	From Leg	4.00	0.0000	113.00	No Ice	3.18	3.35	0.03
			0.00			1/2"	3.56	3.97	0.06
			0.00			Ice	3.93	4.60	0.10
						1" Ice	4.69	5.89	0.19
						2" Ice			
BXA-171063-8BF-2 w/ Mount Pipe	A	From Leg	4.00	0.0000	113.00	No Ice	3.18	3.35	0.03
			0.00			1/2"	3.56	3.97	0.06
			0.00			Ice	3.93	4.60	0.10
						1" Ice	4.69	5.89	0.19
						2" Ice			
BXA-171063-8BF-2 w/ Mount Pipe	B	From Leg	4.00	0.0000	113.00	No Ice	3.18	3.35	0.03
			0.00			1/2"	3.56	3.97	0.06
			0.00			Ice	3.93	4.60	0.10
						1" Ice	4.69	5.89	0.19
						2" Ice			
BXA-171063-8BF-2 w/ Mount Pipe	C	From Leg	4.00	0.0000	113.00	No Ice	3.18	3.35	0.03
			0.00			1/2"	3.56	3.97	0.06
			0.00			Ice	3.93	4.60	0.10
						1" Ice	4.69	5.89	0.19
						2" Ice			
(2) LPA-80090/4CF w/ Mount Pipe	A	From Leg	4.00	0.0000	113.00	No Ice	2.86	5.21	0.03
			0.00			1/2"	3.22	5.82	0.07
			0.00			Ice	3.59	6.44	0.11
						1" Ice	4.34	7.74	0.22
						2" Ice			
(2) LPA-80090/4CF w/ Mount Pipe	B	From Leg	4.00	0.0000	113.00	No Ice	2.86	5.21	0.03
			0.00			1/2"	3.22	5.82	0.07
			0.00			Ice	3.59	6.44	0.11
						1" Ice			

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment	Placement ft	C _A A _{Front} ft ²	C _A A _{Side} ft ²	Weight K	
(2) LPA-80090/4CF w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	113.00	1" Ice	4.34	7.74	0.22
						2" Ice			
						No Ice	2.86	5.21	0.03
						1/2"	3.22	5.82	0.07
						Ice	3.59	6.44	0.11
SWCP 2X5514 w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	113.00	1" Ice	4.34	7.74	0.22
						2" Ice			
						No Ice	6.52	6.53	0.04
						1/2"	6.95	7.24	0.10
						Ice	7.37	7.92	0.17
SWCP 2X5514 w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	113.00	1" Ice	8.25	9.33	0.34
						2" Ice			
						No Ice	6.52	6.53	0.04
						1/2"	6.95	7.24	0.10
						Ice	7.37	7.92	0.17
SWCP 2X5514 w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	113.00	1" Ice	8.25	9.33	0.34
						2" Ice			
						No Ice	6.52	6.53	0.04
						1/2"	6.95	7.24	0.10
						Ice	7.37	7.92	0.17
9442 RRH2X40-AWS	A	From Leg	4.00 0.00 0.00	0.0000	113.00	1" Ice	2.99	2.14	0.13
						2" Ice			
						No Ice	2.16	1.42	0.04
						1/2"	2.35	1.59	0.06
						Ice	2.56	1.77	0.08
9442 RRH2X40-AWS	B	From Leg	4.00 0.00 0.00	0.0000	113.00	1" Ice	2.99	2.14	0.13
						2" Ice			
						No Ice	2.16	1.42	0.04
						1/2"	2.35	1.59	0.06
						Ice	2.56	1.77	0.08
9442 RRH2X40-AWS	C	From Leg	4.00 0.00 0.00	0.0000	113.00	1" Ice	2.99	2.14	0.13
						2" Ice			
						No Ice	2.16	1.42	0.04
						1/2"	2.35	1.59	0.06
						Ice	2.56	1.77	0.08
Platform Mount [LP 303-1]	A	None		0.0000	113.00	1" Ice	31.50	31.50	2.18
						2" Ice			
						No Ice	14.66	14.66	1.25
						1/2"	18.87	18.87	1.48
						Ice	23.08	23.08	1.71

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice

Comb. No.	Description
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Sectio n No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	133 - 86.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-22.61	-0.77	-0.09
			Max. Mx	8	-11.62	-508.10	-1.97
			Max. My	2	-11.62	1.70	506.41
			Max. Vy	8	15.62	-508.10	-1.97
			Max. Vx	2	-15.56	1.70	506.41
			Max. Torque	2			
L2	86.5 - 39.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-34.17	-0.77	-0.09
			Max. Mx	8	-20.40	-1327.10	-5.34
			Max. My	2	-20.41	5.05	1322.41
			Max. Vy	8	20.15	-1327.10	-5.34
			Max. Vx	2	-20.08	5.05	1322.41
			Max. Torque	2			
L3	39.75 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-50.02	-0.77	-0.09
			Max. Mx	8	-33.38	-2327.13	-8.58
			Max. My	2	-33.38	8.29	2319.55
			Max. Vy	8	24.12	-2327.13	-8.58
			Max. Vx	2	-24.05	8.29	2319.55
			Max. Torque	2			

Sectio n No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
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Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	31	50.02	-5.43	-3.14
	Max. H _x	20	33.39	24.09	0.07
	Max. H _z	2	33.39	0.07	24.03
	Max. M _x	2	2319.55	0.07	24.03
	Max. M _z	8	2327.13	-24.09	-0.07
	Max. Torsion	14	0.44	-0.07	-24.03
	Min. Vert	17	25.04	11.98	-20.77
	Min. H _x	8	33.39	-24.09	-0.07
	Min. H _z	14	33.39	-0.07	-24.03
	Min. M _x	14	-2319.52	-0.07	-24.03
	Min. M _z	20	-2326.52	24.09	0.07
	Min. Torsion	2	-0.45	0.07	24.03

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	27.83	0.00	0.00	-0.01	-0.24	0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	33.39	-0.07	-24.03	-2319.55	8.29	0.45
0.9 Dead+1.0 Wind 0 deg - No Ice	25.04	-0.07	-24.03	-2298.41	8.29	0.44
1.2 Dead+1.0 Wind 30 deg - No Ice	33.39	11.98	-20.77	-2004.51	-1156.29	0.42
0.9 Dead+1.0 Wind 30 deg - No Ice	25.04	11.98	-20.77	-1986.24	-1145.68	0.42
1.2 Dead+1.0 Wind 60 deg - No Ice	33.39	20.83	-11.95	-1152.34	-2011.12	0.28
0.9 Dead+1.0 Wind 60 deg - No Ice	25.04	20.83	-11.95	-1141.84	-1992.72	0.28
1.2 Dead+1.0 Wind 90 deg - No Ice	33.39	24.09	0.07	8.58	-2327.13	0.07
0.9 Dead+1.0 Wind 90 deg - No Ice	25.04	24.09	0.07	8.50	-2305.86	0.07
1.2 Dead+1.0 Wind 120 deg - No Ice	33.39	20.90	12.08	1167.18	-2019.69	-0.16
0.9 Dead+1.0 Wind 120 deg - No Ice	25.04	20.90	12.08	1156.55	-2001.21	-0.16
1.2 Dead+1.0 Wind 150 deg - No Ice	33.39	12.11	20.85	2013.04	-1171.16	-0.34
0.9 Dead+1.0 Wind 150 deg - No Ice	25.04	12.11	20.85	1994.71	-1160.41	-0.34
1.2 Dead+1.0 Wind 180 deg - No Ice	33.39	0.07	24.03	2319.52	-8.90	-0.44
0.9 Dead+1.0 Wind 180 deg - No Ice	25.04	0.07	24.03	2298.39	-8.74	-0.44
1.2 Dead+1.0 Wind 210 deg - No Ice	33.39	-11.98	20.77	2004.48	1155.68	-0.42
0.9 Dead+1.0 Wind 210 deg - No Ice	25.04	-11.98	20.77	1986.22	1145.23	-0.42
1.2 Dead+1.0 Wind 240 deg - No Ice	33.39	-20.83	11.95	1152.31	2010.51	-0.29
0.9 Dead+1.0 Wind 240 deg - No Ice	25.04	-20.83	11.95	1141.82	1992.27	-0.29

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturing Moment, M _x kip-ft	Overturing Moment, M _z kip-ft	Torque kip-ft
1.2 Dead+1.0 Wind 270 deg - No Ice	33.39	-24.09	-0.07	-8.61	2326.52	-0.07
0.9 Dead+1.0 Wind 270 deg - No Ice	25.04	-24.09	-0.07	-8.53	2305.40	-0.08
1.2 Dead+1.0 Wind 300 deg - No Ice	33.39	-20.90	-12.08	-1167.21	2019.08	0.16
0.9 Dead+1.0 Wind 300 deg - No Ice	25.04	-20.90	-12.08	-1156.57	2000.76	0.16
1.2 Dead+1.0 Wind 330 deg - No Ice	33.39	-12.11	-20.85	-2013.07	1170.55	0.35
0.9 Dead+1.0 Wind 330 deg - No Ice	25.04	-12.11	-20.85	-1994.73	1159.96	0.35
1.2 Dead+1.0 Ice+1.0 Temp	50.02	0.00	0.00	0.09	-0.77	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	50.02	-0.01	-6.26	-601.69	0.68	0.11
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	50.02	3.12	-5.41	-520.29	-301.06	0.10
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	50.02	5.42	-3.12	-299.45	-522.37	0.06
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	50.02	6.27	0.01	1.65	-603.94	0.01
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	50.02	5.43	3.14	302.34	-523.92	-0.04
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	50.02	3.14	5.42	522.04	-303.75	-0.09
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	50.02	0.01	6.26	601.89	-2.42	-0.11
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	50.02	-3.12	5.41	520.49	299.32	-0.10
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	50.02	-5.42	3.12	299.65	520.63	-0.06
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	50.02	-6.27	-0.01	-1.45	602.20	-0.01
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	50.02	-5.43	-3.14	-302.14	522.18	0.04
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	50.02	-3.14	-5.42	-521.84	302.01	0.09
Dead+Wind 0 deg - Service	27.83	-0.02	-5.21	-500.80	1.60	0.10
Dead+Wind 30 deg - Service	27.83	2.60	-4.51	-432.78	-249.83	0.09
Dead+Wind 60 deg - Service	27.83	4.52	-2.59	-248.80	-434.39	0.06
Dead+Wind 90 deg - Service	27.83	5.23	0.02	1.84	-502.62	0.02
Dead+Wind 120 deg - Service	27.83	4.54	2.62	251.99	-436.24	-0.03
Dead+Wind 150 deg - Service	27.83	2.63	4.52	434.61	-253.04	-0.08
Dead+Wind 180 deg - Service	27.83	0.02	5.21	500.78	-2.11	-0.10
Dead+Wind 210 deg - Service	27.83	-2.60	4.51	432.76	249.32	-0.09
Dead+Wind 240 deg - Service	27.83	-4.52	2.59	248.77	433.88	-0.06
Dead+Wind 270 deg - Service	27.83	-5.23	-0.02	-1.87	502.11	-0.02
Dead+Wind 300 deg - Service	27.83	-4.54	-2.62	-252.01	435.74	0.03
Dead+Wind 330 deg - Service	27.83	-2.63	-4.52	-434.64	252.54	0.08

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-27.83	0.00	0.00	27.83	0.00	0.000%
2	-0.07	-33.39	-24.03	0.07	33.39	24.03	0.000%
3	-0.07	-25.04	-24.03	0.07	25.04	24.03	0.000%

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
4	11.98	-33.39	-20.77	-11.98	33.39	20.77	0.000%
5	11.98	-25.04	-20.77	-11.98	25.04	20.77	0.000%
6	20.83	-33.39	-11.95	-20.83	33.39	11.95	0.000%
7	20.83	-25.04	-11.95	-20.83	25.04	11.95	0.000%
8	24.09	-33.39	0.07	-24.09	33.39	-0.07	0.000%
9	24.09	-25.04	0.07	-24.09	25.04	-0.07	0.000%
10	20.90	-33.39	12.08	-20.90	33.39	-12.08	0.000%
11	20.90	-25.04	12.08	-20.90	25.04	-12.08	0.000%
12	12.11	-33.39	20.85	-12.11	33.39	-20.85	0.000%
13	12.11	-25.04	20.85	-12.11	25.04	-20.85	0.000%
14	0.07	-33.39	24.03	-0.07	33.39	-24.03	0.000%
15	0.07	-25.04	24.03	-0.07	25.04	-24.03	0.000%
16	-11.98	-33.39	20.77	11.98	33.39	-20.77	0.000%
17	-11.98	-25.04	20.77	11.98	25.04	-20.77	0.000%
18	-20.83	-33.39	11.95	20.83	33.39	-11.95	0.000%
19	-20.83	-25.04	11.95	20.83	25.04	-11.95	0.000%
20	-24.09	-33.39	-0.07	24.09	33.39	0.07	0.000%
21	-24.09	-25.04	-0.07	24.09	25.04	0.07	0.000%
22	-20.90	-33.39	-12.08	20.90	33.39	12.08	0.000%
23	-20.90	-25.04	-12.08	20.90	25.04	12.08	0.000%
24	-12.11	-33.39	-20.85	12.11	33.39	20.85	0.000%
25	-12.11	-25.04	-20.85	12.11	25.04	20.85	0.000%
26	0.00	-50.02	0.00	0.00	50.02	0.00	0.000%
27	-0.01	-50.02	-6.26	0.01	50.02	6.26	0.000%
28	3.12	-50.02	-5.41	-3.12	50.02	5.41	0.000%
29	5.42	-50.02	-3.12	-5.42	50.02	3.12	0.000%
30	6.27	-50.02	0.01	-6.27	50.02	-0.01	0.000%
31	5.43	-50.02	3.14	-5.43	50.02	-3.14	0.000%
32	3.14	-50.02	5.42	-3.14	50.02	-5.42	0.000%
33	0.01	-50.02	6.26	-0.01	50.02	-6.26	0.000%
34	-3.12	-50.02	5.41	3.12	50.02	-5.41	0.000%
35	-5.42	-50.02	3.12	5.42	50.02	-3.12	0.000%
36	-6.27	-50.02	-0.01	6.27	50.02	0.01	0.000%
37	-5.43	-50.02	-3.14	5.43	50.02	3.14	0.000%
38	-3.14	-50.02	-5.42	3.14	50.02	5.42	0.000%
39	-0.02	-27.83	-5.21	0.02	27.83	5.21	0.000%
40	2.60	-27.83	-4.51	-2.60	27.83	4.51	0.000%
41	4.52	-27.83	-2.59	-4.52	27.83	2.59	0.000%
42	5.23	-27.83	0.02	-5.23	27.83	-0.02	0.000%
43	4.54	-27.83	2.62	-4.54	27.83	-2.62	0.000%
44	2.63	-27.83	4.52	-2.63	27.83	-4.52	0.000%
45	0.02	-27.83	5.21	-0.02	27.83	-5.21	0.000%
46	-2.60	-27.83	4.51	2.60	27.83	-4.51	0.000%
47	-4.52	-27.83	2.59	4.52	27.83	-2.59	0.000%
48	-5.23	-27.83	-0.02	5.23	27.83	0.02	0.000%
49	-4.54	-27.83	-2.62	4.54	27.83	2.62	0.000%
50	-2.63	-27.83	-4.52	2.63	27.83	4.52	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00020756
3	Yes	4	0.00000001	0.00009271
4	Yes	5	0.00000001	0.00041661
5	Yes	5	0.00000001	0.00018349
6	Yes	5	0.00000001	0.00040907
7	Yes	5	0.00000001	0.00017982
8	Yes	4	0.00000001	0.00020720
9	Yes	4	0.00000001	0.00009188
10	Yes	5	0.00000001	0.00041752
11	Yes	5	0.00000001	0.00018315
12	Yes	5	0.00000001	0.00042304
13	Yes	5	0.00000001	0.00018582

14	Yes	4	0.00000001	0.00034725
15	Yes	4	0.00000001	0.00019680
16	Yes	5	0.00000001	0.00040681
17	Yes	5	0.00000001	0.00017889
18	Yes	5	0.00000001	0.00041476
19	Yes	5	0.00000001	0.00018264
20	Yes	4	0.00000001	0.00017925
21	Yes	4	0.00000001	0.00006401
22	Yes	5	0.00000001	0.00042077
23	Yes	5	0.00000001	0.00018475
24	Yes	5	0.00000001	0.00041483
25	Yes	5	0.00000001	0.00018200
26	Yes	4	0.00000001	0.00000001
27	Yes	5	0.00000001	0.00012412
28	Yes	5	0.00000001	0.00015307
29	Yes	5	0.00000001	0.00015267
30	Yes	5	0.00000001	0.00012479
31	Yes	5	0.00000001	0.00015399
32	Yes	5	0.00000001	0.00015432
33	Yes	5	0.00000001	0.00012422
34	Yes	5	0.00000001	0.00015155
35	Yes	5	0.00000001	0.00015216
36	Yes	5	0.00000001	0.00012400
37	Yes	5	0.00000001	0.00015317
38	Yes	5	0.00000001	0.00015265
39	Yes	4	0.00000001	0.00001920
40	Yes	4	0.00000001	0.00013502
41	Yes	4	0.00000001	0.00012646
42	Yes	4	0.00000001	0.00001623
43	Yes	4	0.00000001	0.00013077
44	Yes	4	0.00000001	0.00013712
45	Yes	4	0.00000001	0.00002050
46	Yes	4	0.00000001	0.00012413
47	Yes	4	0.00000001	0.00013263
48	Yes	4	0.00000001	0.00001596
49	Yes	4	0.00000001	0.00013424
50	Yes	4	0.00000001	0.00012793

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	133 - 86.5	16.099	43	1.0518	0.0009
L2	90.75 - 39.75	7.573	43	0.8139	0.0004
L3	45 - 0	1.782	43	0.3630	0.0001

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
133.00	Lighting Rod 3/4" x 4'	43	16.099	1.0518	0.0009	52152
115.00	DB-T1-6Z-8AB-0Z	43	12.265	0.9681	0.0007	14486
113.00	BXA-171063-8BF-EDIN-4 w/ Mount Pipe	43	11.850	0.9578	0.0006	13038

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	133 - 86.5	74.526	10	4.8736	0.0040
L2	90.75 - 39.75	35.076	10	3.7727	0.0019
L3	45 - 0	8.256	10	1.6821	0.0005

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
133.00	Lighting Rod 3/4" x 4'	10	74.526	4.8736	0.0041	11416
115.00	DB-T1-6Z-8AB-0Z	10	56.788	4.4869	0.0031	3169
113.00	BXA-171063-8BF-EDIN-4 w/ Mount Pipe	10	54.871	4.4391	0.0030	2852

Compression Checks

Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	KI/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
L1	133 - 86.5 (1)	TP33.116x24x0.25	46.50	0.00	0.0	25.418	-11.61	1486.96	0.008
L2	86.5 - 39.75 (2)	TP41.78x31.7828x0.2813	51.00	0.00	0.0	36.126 7	-20.40	2113.41	0.010
L3	39.75 - 0 (3)	TP49.01x40.1884x0.375	45.00	0.00	0.0	57.887 8	-33.38	3386.44	0.010

Pole Bending Design Data

Section No.	Elevation ft	Size	M _{ux} kip-ft	φM _{nx} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	M _{uy} kip-ft	φM _{ny} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
L1	133 - 86.5 (1)	TP33.116x24x0.25	509.41	1140.83	0.447	0.00	1140.83	0.000
L2	86.5 - 39.75 (2)	TP41.78x31.7828x0.2813	1330.58	1961.90	0.678	0.00	1961.90	0.000
L3	39.75 - 0 (3)	TP49.01x40.1884x0.375	2332.69	3928.24	0.594	0.00	3928.24	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V _u K	φV _n K	Ratio $\frac{V_u}{\phi V_n}$	Actual T _u kip-ft	φT _n kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L1	133 - 86.5 (1)	TP33.116x24x0.25	15.67	446.09	0.035	0.16	1251.39	0.000
L2	86.5 - 39.75 (2)	TP41.78x31.7828x0.2813	20.19	634.02	0.032	0.16	2247.06	0.000
L3	39.75 - 0 (3)	TP49.01x40.1884x0.375	24.16	1015.93	0.024	0.16	4327.06	0.000

Pole Interaction Design Data

Section No.	Elevation ft	Ratio P_u ϕP_n	Ratio M_{ux} ϕM_{nx}	Ratio M_{uy} ϕM_{ny}	Ratio V_u ϕV_n	Ratio T_u ϕT_n	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	133 - 86.5 (1)	0.008	0.447	0.000	0.035	0.000	0.456	1.050	4.8.2
L2	86.5 - 39.75 (2)	0.010	0.678	0.000	0.032	0.000	0.689	1.050	4.8.2
L3	39.75 - 0 (3)	0.010	0.594	0.000	0.024	0.000	0.604	1.050	4.8.2

Section Capacity Table

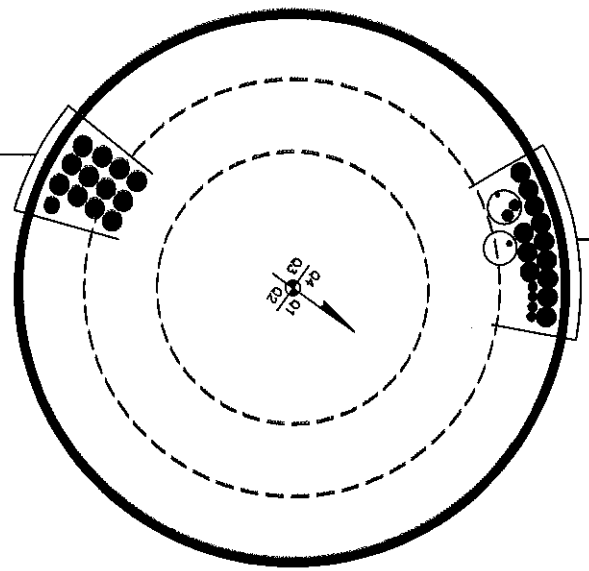
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail	
L1	133 - 86.5	Pole	TP33.116x24x0.25	1	-11.61	1561.31	43.4	Pass	
L2	86.5 - 39.75	Pole	TP41.78x31.7828x0.2813	2	-20.40	2219.08	65.6	Pass	
L3	39.75 - 0	Pole	TP49.01x40.1884x0.375	3	-33.38	3555.76	57.5	Pass	
							Summary		
							Pole (L2)	65.6	Pass
							RATING =	65.6	Pass

APPENDIX B
BASE LEVEL DRAWING



- (PROPOSED EQUIPMENT CONFIGURATION--IN CONDUIT)
- (1) 17/64" TO 133 FT LEVEL
 - (1) 3/8" TO 133 FT LEVEL
 - (2) 7/8" TO 133 FT LEVEL
- (PROPOSED EQUIPMENT CONFIGURATION)
- (4) 3/4" TO 133 FT LEVEL
 - (12) 1-5/8" TO 133 FT LEVEL

- (OTHER CONSIDERED EQUIPMENT)
- (1) 1-1/4" TO 113 FT LEVEL
 - (12) 1-5/8" TO 113 FT LEVEL



BUSINESS UNIT: 873633 TOWER ID: C_BASLEVEL

APPENDIX C
ADDITIONAL CALCULATIONS

Monopole Base Plate Connection

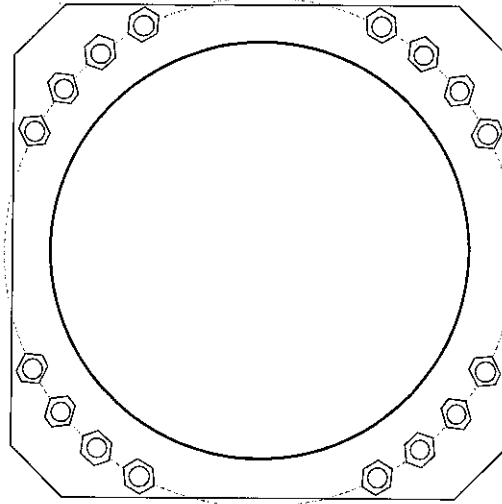


Site Info	
BU #	873633
Site Name	Milford
Order #	459065 Rev. 1

Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	No
l_{gr} (in)	1.25

Applied Loads	
Moment (kip-ft)	2332.69
Axial Force (kips)	33.38
Shear Force (kips)	24.16

*TIA-222-H Section 15.5 Applied



Connection Properties		Analysis Results	
Anchor Rod Data		Anchor Rod Summary	<i>(units of kips, kip-in)</i>
(16) 2-1/4" ϕ bolts (A615-75 N; $F_y=75$ ksi, $F_u=100$ ksi) on 60" BC		$P_{u,c} = 118.65$	$\phi P_{n,c} = 243.75$ Stress Rating
Base Plate Data		$V_u = 1.51$	$\phi V_n = 73.13$ 46.4%
58" OD x 3.25" Plate (A572-55; $F_y=55$ ksi, $F_u=70$ ksi)		$M_u = n/a$	$\phi M_n = n/a$ Pass
Stiffener Data		Base Plate Summary	
N/A		Max Stress (ksi):	25.28 (Flexural)
Pole Data		Allowable Stress (ksi):	49.5
49.01" x 0.375" 18-sided pole (A607-65; $F_y=65$ ksi, $F_u=80$ ksi)		Stress Rating:	48.6% Pass

Drilled Pier Foundation

BU #: 873633
 Site Name: Milford
 Order Number: 459065 Rev. 1

TIA-222 Revision: H
 Tower Type: Monopole



Check Limitation
 Apply TIA-222-H Section 15.5:

Analysis Results

Soil Lateral Capacity	Compression	Uplift
D _{u=0} (ft from TOC)	6.01	-
Soil Safety Factor	4.06	-
Max Moment (kip-ft)	2439.77	-
Rating*	31.2%	-
Soil Vertical Capacity		
Compression		Uplift
Skin Friction (kips)	600.55	-
End Bearing (kips)	288.63	-
Weight of Concrete (kips)	176.64	-
Total Capacity (kips)	889.18	-
Axial (kips)	209.64	-
Rating*	22.5%	-
Reinforced Concrete Capacity		
Compression		Uplift
Critical Depth (ft from TOC)	6.13	-
Critical Moment (kip-ft)	2439.69	-
Critical Moment Capacity	5801.63	-
Rating*	40.0%	-
Soil Interaction Rating*		
		31.2%
Structural Foundation Rating*		
		40.0%

*Rating per TIA-222-H Section 15.5

Soil Profile

of Layers: 2

Layer	Top (ft)	Bottom (ft)	Thickness (ft)	V _{soil} (pcf)	V _{concrete} (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Comp Override (ksf)	Ultimate Skin Friction Uplift Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	3.5	3.5	120	150	0	19	0.000	0.000	0.00	0.00	10	19	Cohesionless
2	3.5	25	21.5	120	150	0	30	1.694	1.694					Cohesionless

Applied Loads	Comp.	Uplift
Moment (kip-ft)	2333	-
Axial Force (kips)	33	-
Shear Force (kips)	24	-

Material Properties	
Concrete Strength, f _c	3 ksi
Rebar Strength, F _y	60 ksi

Pier Design Data	
Depth	25 ft
Ext. Above Grade	0.5 ft
Pier Section 1	
<i>From 0.5' above grade to 25' below grade</i>	
Pier Diameter	7 ft
Rebar Quantity	24
Rebar Size	11
Clear Cover to Ties	4 in
Tie Size	5

Groundwater Depth: n/a ft

Pier and Pad Foundation



BU #: 873633
 Site Name: Milford
 App. Number: 459065 Rev. 1

TIA-222 Revision: H
 Tower Type: Monopole

Top & Bot. Pad Rein. Different?:
 Block Foundation?:

Superstructure Analysis Reactions		
Compression, P_{comp} :	33	kips
Base Shear, V_u_{comp} :	24	kips
Moment, M_u :	2333	ft-kips
Tower Height, H :	133	ft
BP Dist. Above Fdn, bp_{dist} :	3	in

Foundation Analysis Checks				
	Capacity	Demand	Rating*	Check
Lateral (Sliding) (kips)	275.50	24.00	8.3%	Pass
Bearing Pressure (ksf)	8.13	2.01	23.5%	Pass
Overturning (kip*ft)	5630.74	2519.00	44.7%	Pass
Pier Flexure (Comp.) (kip*ft)	7552.24	2441.00	30.8%	Pass
Pier Compression (kip)	23390.64	72.69	0.3%	Pass
Pad Flexure (kip*ft)	4940.35	785.18	15.1%	Pass
Pad Shear - 1-way (kips)	715.56	129.72	17.3%	Pass
Pad Shear - 2-way (Comp) (ksi)	0.164	0.025	14.7%	Pass
Flexural 2-way (Comp) (kip*ft)	6626.53	1464.60	21.0%	Pass

Pier Properties		
Pier Shape:	Square	
Pier Diameter, $dpier$:	7	ft
Ext. Above Grade, E :	0.5	ft
Pier Rebar Size, Sc :	11	
Pier Rebar Quantity, mc :	32	
Pier Tie/Spiral Size, St :	4	
Pier Tie/Spiral Quantity, mt :	12	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, cc_{pier} :	3	in

*Rating per TIA-222-H Section 15.5

Soil Rating*:	44.7%
Structural Rating*:	30.8%

Pad Properties		
Depth, D :	7	ft
Pad Width, W :	23.5	ft
Pad Thickness, T :	3	ft
Pad Rebar Size (Bottom), Sp :	11	
Pad Rebar Quantity (Bottom), mp :	24	
Pad Clear Cover, cc_{pad} :	3	in

Material Properties		
Rebar Grade, F_y :	60000	psi
Concrete Compressive Strength, F'_c :	3000	psi
Dry Concrete Density, δ_c :	150	pcf

Soil Properties		
Total Soil Unit Weight, γ :	120	pcf
Ultimate Net Bearing, Q_{net} :	10.000	ksf
Cohesion, C_u :	0.000	ksf
Friction Angle, ϕ :	30	degrees
SPT Blow Count, N_{blows} :	19	
Base Friction, μ :	0.45	
Neglected Depth, N :	3.50	ft
Foundation Bearing on Rock?	No	
Groundwater Depth, gw :	None	ft

<--Toggle between Gross and Net



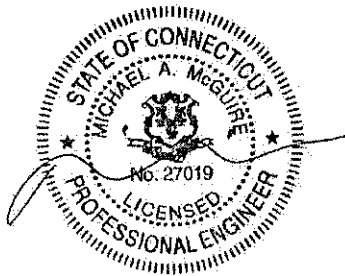
RF EMISSIONS COMPLIANCE REPORT

Crown Castle on Behalf of AT&T Mobility, LLC

Site: Milford
Crown Castle Site ID: 873633
App ID: 459065
10 Bona Street
MILFORD, CT
2/25/2019

Report Status:

AT&T Mobility, LLC Is Compliant



sealed 26feb2019 mike@h2dc.com
H2DC PLLC CT CoA#: 0001714

Prepared By:

Sitesafe, LLC

8618 Westwood Center Drive,
Suite 315

Vienna, VA 22182

Voice 703-276-1100
Fax 703-276-1169

Engineering Statement in Re:
Electromagnetic Energy Analysis
AT&T Mobility, LLC
MILFORD, CT

My signature on the cover of this document indicates:

That I, Michael A McGuire, am currently and actively licensed to provide (in this state/jurisdiction as indicated within the professional electrical engineering seal on the cover of this document) professional electrical engineering services, as an employee of Hurricane Hill Development Company, PLLC, a duly authorized/registered engineering firm (in this state, as applicable) on behalf of SiteSafe, LLC; and

That I am thoroughly familiar with the Rules and Regulations of the Federal Communications Commission ("the FCC" and "the FCC Rules") both in general and specifically as they apply to the FCC's Guidelines for Human Exposure to Radiofrequency Electromagnetic Fields; and

That the technical information serving as the basis for this report was supplied by AT&T Mobility, LLC (See attached Site Summary and Carrier documents), and that AT&T Mobility, LLC's installations involve communications equipment, antennas and associated technical equipment at a location referred to as the "Milford" ("the site"); and

That AT&T Mobility, LLC proposes to operate at the site with transmit antennas listed in the carrier summary and with a maximum effective radiated power as specified by AT&T Mobility, LLC and shown on the worksheet, and that worst-case 100% duty cycle have been assumed; and

That this analysis has been performed with the assumption that the ground immediately surrounding the tower is primarily flat or falling; and

That at this time, the FCC requires that certain licensees address specific levels of radio-frequency energy to which workers or members of the public might possibly be exposed (at §1.1307(b) of the FCC Rules); and

That such consideration of possible exposure of humans to radio-frequency radiation must utilize the standards set by the FCC, which is the Federal Agency having jurisdiction over communications facilities; and

That the FCC rules define two tiers of permissible exposure guidelines: 1) "uncontrolled environments," defined as situations in which persons may not be aware of (the "general public"), or may not be able to control their exposure to a transmission facility; and (2) "controlled environments," which defines situations in which persons are aware of their potential for exposure (industry personnel); and

That this statement specifically addresses the uncontrolled environment (which is more conservative than the controlled environment) and the limit set forth in the FCC rules for licensees of AT&T Mobility, LLC's operating frequency as shown on the attached antenna worksheet; and

That when applying the uncontrolled environment standards, the predicted Maximum Power Density at two meters above ground level from the proposed AT&T Mobility, LLC operation is no more than 1.575% of the maximum in any accessible area on the ground and

That it is understood per FCC Guidelines and OET65 Appendix A, that regardless of the existent radio-frequency environment, only those licenses whose contributions exceed five percent of the exposure limit pertinent to their operation(s) bear any responsibility for bringing any non-compliant area(s) into compliance; and

That when applying the uncontrolled environment standards, the cumulative predicted energy density from the proposed operation is no more than 3.121% of the maximum in any accessible area up to two meters above the ground per OET-65; and

That the calculations provided in this report are based on data provided by the client and antenna pattern data supplied by the antenna manufacturer, in accordance with FCC guidelines listed in OET-65. Horizontal and vertical antenna patterns are combined for modeling purposes to accurately reflect the energy two meters above ground level where on-axis energy refers to maximum energy two meters above the ground along the azimuth of the antenna and where area energy refers to the maximum energy anywhere two meters above the ground regardless of the antenna azimuth, accounting for cumulative energy from multiple antennas for the carrier and frequency range indicated; and

That the Occupational Safety and Health Administration has policies in place which address worker safety in and around communications sites, thus individual companies will be responsible for their employees' training regarding Radio Frequency Safety.

In summary, it is stated here that the proposed operation at the site would not result in exposure of the Public to excessive levels of radio-frequency energy as defined in the FCC Rules and Regulations, specifically 47 CFR 1.1307 and that AT&T Mobility, LLC's proposed operation is completely compliant.

Finally, it is stated that access to the tower should be restricted to communication industry professionals, and approved contractor personnel trained in radio-frequency safety; and that the instant analysis addresses exposure levels at two meters above ground level and does not address exposure levels on the tower, or in the immediate proximity of the antennas.

**AT&T Mobility, LLC
Milford
Site Summary**

Carrier	Area Maximum Percentage MPE
AT&T Mobility, LLC (Proposed)	0.274 %
AT&T Mobility, LLC (Proposed)	0.213 %
AT&T Mobility, LLC (Proposed)	0.27 %
AT&T Mobility, LLC (Proposed)	0.464 %
AT&T Mobility, LLC (Proposed)	0.265 %
AT&T Mobility, LLC	0.089 %
Verizon Wireless	0.649 %
Verizon Wireless	0.327 %
Verizon Wireless	0.569 %
 Composite Site MPE:	 3.121 %

**AT&T Mobility, LLC (Proposed)
Milford
Carrier Summary**

Frequency: 1930 MHz
 Maximum Permissible Exposure (MPE): 1000 $\mu\text{W}/\text{cm}^2$
 Maximum power density at ground level: 2.74165 $\mu\text{W}/\text{cm}^2$
 Highest percentage of Maximum Permissible Exposure: 0.27416 %

Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	On Axis		Area	
					Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE	Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE
Kathrein-Scala	800-10964	136	23	5154	1.253456	0.125346	2.601504	0.26015
Kathrein-Scala	800-10964	136	143	5154	1.252498	0.12525	2.601504	0.26015
Kathrein-Scala	800-10964	136	263	5154	1.252498	0.12525	2.601504	0.26015

**AT&T Mobility, LLC (Proposed)
Milford
Carrier Summary**

Frequency: 850 MHz
 Maximum Permissible Exposure (MPE): 566.67 $\mu\text{W}/\text{cm}^2$
 Maximum power density at ground level: 1.20756 $\mu\text{W}/\text{cm}^2$
 Highest percentage of Maximum Permissible Exposure: 0.2131 %

Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	On Axis		Area	
					Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE	Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE
Kathrein-Scala	800-10964	136	23	2631	0.845074	0.149131	1.017042	0.179478
Kathrein-Scala	800-10964	136	143	2631	0.843138	0.148789	1.017042	0.179478
Kathrein-Scala	800-10964	136	263	2631	0.845074	0.149131	1.017042	0.179478

**AT&T Mobility, LLC (Proposed)
Milford
Carrier Summary**

Frequency: 2300 MHz
 Maximum Permissible Exposure (MPE): 1000 $\mu\text{W}/\text{cm}^2$
 Maximum power density at ground level: 2.69813 $\mu\text{W}/\text{cm}^2$
 Highest percentage of Maximum Permissible Exposure: 0.26981 %

Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	On Axis		Area	
					Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE	Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE
ANDREW	SBNHH-1D65A	136	23	2685	1.696196	0.16962	2.691885	0.269188
ANDREW	SBNHH-1D65A	136	143	2685	1.6846	0.16846	2.691885	0.269189
ANDREW	SBNHH-1D65A	136	263	2685	1.6846	0.16846	2.691885	0.269189

**AT&T Mobility, LLC (Proposed)
Milford
Carrier Summary**

Frequency: 2110 MHz
 Maximum Permissible Exposure (MPE): 1000 $\mu\text{W}/\text{cm}^2$
 Maximum power density at ground level: 4.64339 $\mu\text{W}/\text{cm}^2$
 Highest percentage of Maximum Permissible Exposure: 0.46434 %

Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	On Axis		Area	
					Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE	Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE
CCI Antennas	OPA-65R-LCUU-H4	136	23	4257	3.283655	0.328366	4.599884	0.459988
CCI Antennas	OPA-65R-LCUU-H4	136	143	4257	3.283656	0.328366	4.599883	0.459988
CCI Antennas	OPA-65R-LCUU-H4	136	263	4257	3.339784	0.333978	4.599884	0.459988

**AT&T Mobility, LLC (Proposed)
Milford
Carrier Summary**

Frequency: 734 MHz
 Maximum Permissible Exposure (MPE): 489.33 $\mu\text{W}/\text{cm}^2$
 Maximum power density at ground level: 1.29787 $\mu\text{W}/\text{cm}^2$
 Highest percentage of Maximum Permissible Exposure: 0.26523 %

Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	On Axis		Area	
					Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE	Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE
CCI Antennas	OPA-65R-LCUU-H4	136	23	1902	1.234788	0.252341	1.285713	0.262748
CCI Antennas	OPA-65R-LCUU-H4	136	143	1902	1.235891	0.252566	1.285713	0.262748
CCI Antennas	OPA-65R-LCUU-H4	136	263	1902	1.234789	0.252341	1.285713	0.262748

**AT&T Mobility, LLC
Milford
Carrier Summary**

Frequency: 869 MHz
 Maximum Permissible Exposure (MPE): 579.33 $\mu\text{W}/\text{cm}^2$
 Maximum power density at ground level: 0.51694 $\mu\text{W}/\text{cm}^2$
 Highest percentage of Maximum Permissible Exposure: 0.08923 %

Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	On Axis		Area	
					Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE	Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE
Powerwave	7770	136	23	547	0.288052	0.049721	0.443001	0.076467
Powerwave	7770	136	143	547	0.288052	0.049721	0.443001	0.076467
Powerwave	7770	136	263	547	0.287685	0.049658	0.443001	0.076467

**Verizon Wireless
Milford
Carrier Summary**

Frequency: 1900 MHz
 Maximum Permissible Exposure (MPE): 1000 $\mu\text{W}/\text{cm}^2$
 Maximum power density at ground level: 6.48932 $\mu\text{W}/\text{cm}^2$
 Highest percentage of Maximum Permissible Exposure: 0.64893 %

Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	On Axis		Area	
					Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE	Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE
Antel	BXA-171063-8BF	113	30	3708	2.606999	0.2607	2.900977	0.290098
Antel	BXA-171063-8BF	113	150	3708	2.610039	0.261004	2.900977	0.290098
Antel	BXA-171063-8BF	113	270	3708	2.610039	0.261004	2.900977	0.290098
Antel	BXA-171063-8BF	113	30	3708	3.669449	0.366945	3.66947	0.366947
Antel	BXA-171063-8BF	113	150	3708	3.669449	0.366945	3.66947	0.366947
Antel	BXA-171063-8BF	113	270	3708	3.669449	0.366945	3.66947	0.366947

Verizon Wireless Milford Carrier Summary

Frequency: 751 MHz
Maximum Permissible Exposure (MPE): 500.67 $\mu\text{W}/\text{cm}^2$
Maximum power density at ground level: 1.63879 $\mu\text{W}/\text{cm}^2$
Highest percentage of Maximum Permissible Exposure: 0.32732 %

Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	On Axis		Area	
					Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE	Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE
SWEDCOM	SWCP 2X5514	113	30	2530	1.577105	0.315001	1.637968	0.327157
SWEDCOM	SWCP 2X5514	113	150	2530	1.575096	0.3146	1.637968	0.327157
SWEDCOM	SWCP 2X5514	113	270	2530	1.577105	0.315001	1.637968	0.327157

**Verizon Wireless
Milford
Carrier Summary**

Frequency: 850 MHz
 Maximum Permissible Exposure (MPE): 566.67 $\mu\text{W}/\text{cm}^2$
 Maximum power density at ground level: 3.22623 $\mu\text{W}/\text{cm}^2$
 Highest percentage of Maximum Permissible Exposure: 0.56933 %

Antenna Make	Model	Height (feet)	Orientation (degrees true)	ERP (Watts)	On Axis		Area	
					Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE	Max Power Density ($\mu\text{W}/\text{cm}^2$)	Percent of MPE
Antel	LPA-80090-4CF	113	30	1423	1.118911	0.197455	1.177164	0.207735
Antel	LPA-80090-4CF	113	150	1423	1.118911	0.197455	1.177165	0.207735
Antel	LPA-80090-4CF	113	270	1423	1.118911	0.197455	1.177165	0.207735
Antel	LPA-80090-4CF	113	30	1423	1.118911	0.197455	1.177164	0.207735
Antel	LPA-80090-4CF	113	150	1423	1.118911	0.197455	1.177165	0.207735
Antel	LPA-80090-4CF	113	270	1423	1.118911	0.197455	1.177165	0.207735