

November 7, 2017

Melanie Bachman  
Acting Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

RE: NOTICE OF EXEMPT MODIFICATION  
60 North Eagleville Road, Mansfield, CT 06268

Dear Ms. Bachman:

Enclosed please find an original and two (2) copies of a Notice of Exempt Modification including drawings, structural analyses, RF emissions reports, parcel maps, and a check in the amount of six hundred twenty five dollars (\$625.00) for the filing fee. In addition, I have included a single copy of each notification letter to the municipality, the Department of Planning and Zoning, and to the property and tower owner. The proof of delivery is likewise enclosed and consists of a copy of the Certified Mail receipt and a copy of the USPS Tracking Results from the USPS website, acknowledging the date and time of delivery.

I have submitted electronic copies of these documents via email to the CSC today.

Please feel free to contact me with any questions or comments. Thank you for your kind cooperation in this matter.

Respectfully submitted,

Jack Andrews  
Zoning Manager, Empire Telecom  
o/b/o AT&T Wireless  
10130 Donleigh Drive  
Columbia, MD 21046  
443-286-4006  
[jandrews@empiretelecomm.com](mailto:jandrews@empiretelecomm.com)

Enclosures

Jack Andrews  
Zoning Manager, Empire Telecom  
o/b/o AT&T Wireless  
10130 Donleigh Drive  
Columbia, MD 21046  
443-286-4007  
jandrews@empiretelecomm.com

November 1, 2017

Melanie Bachman  
Acting Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

**NOTICE OF EXEMPT MODIFICATION**

60 North Eagleville Road, Mansfield, CT 06268

Lat: 41-48-50.62 (41.81406111)  
Long: 72-15-33.88 (-72.25941111)

Dear Ms. Bachman:

AT&T Wireless currently maintains nine (9) antennas at the 185-foot level of an existing 291-foot guyed tower located at 60 North Eagleville Road, in Mansfield, CT. The tower is owned by the University of Connecticut. The property is owned by University of Connecticut. AT&T Wireless now seeks to install three (3) additional antennas at the 291-foot level of the tower, to be mounted on the existing tower, as well as install six (6) new RRUS-32 Remote Radio Units ("RRU"), and install 3 new RRUS-B14 RRUs on the tower, behind the antennas. In addition, a single new surge suppressor will be installed behind the Gamma sector antennas.

The facility was approved by the Connecticut Siting Council in EM-AT&T-078-161021 on November 22, 2016. Eight (8) conditions were enumerated in the Council's decision: 1) AT&T shall ensure that the foundation does not exceed 100 percent of its post-construction structural rating; 2) within 45 days following completion of equipment installation, AT&T shall provide documentation certified by the Professional Engineer that its installation complied with the modifications listed on the Construction Site Plans; 3) Any deviation from the modification as specified in the Notice and supporting documentation shall render the acknowledgement invalid; 4) Any material changes to the modification as proposed shall require the filing of a new Notice with the Council; 5) Within 45 days after the completion of construction the Council shall be notified in writing that the construction has been completed; 6) Any nonfunctioning antenna and associated antenna mounting equipment on this facility owned and operated by AT&T shall be removed within 60 days of the date the antenna ceased to function; 7) the validity of the action shall expire one year from the date of the letter; and 8) the applicant may file a request an extension of time beyond the one year deadline provided that such a request is submitted to the Council not less than 60 days prior to the expiration.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies section 16-50j-73 for construction that constitutes an exempt modification pursuant to RCSA section 16-50j-72(b)(2). In accordance with RCSA section 16-50j-73, a copy of this letter and attachments is being sent to the Honorable Paul Shapiro, Mayor of Mansfield; Linda Painter, the Mansfield Director of Planning & Development, as well as to the University of Connecticut, the tower owner, and to by the University of Connecticut, the property owner. Michael Jednak AVP, of the Facilities Operations & Building Services of the University of Connecticut is our point of contact.

The planned modifications to the facility fall squarely within those activities expressly provided for in RCSA section 50j-72(b)(2).

1. The proposed modifications will not result in an increase in height of the existing structure.
2. The proposed modifications will not require an extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that will exceed state and local limits.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, AT&T Wireless respectfully submits that the proposed modifications to the above referenced telecommunications facility constitute an exempt modification under RCSA section 16-50j-72(b)(2).

Respectfully submitted,

Jack Andrews  
Zoning Manager, Empire Telecom  
o/b/o AT&T Wireless  
10130 Donleigh Drive  
Columbia, MD 21046  
443-286-4007  
[jandrews@empiretelecomm.com](mailto:jandrews@empiretelecomm.com)

Enclosures

cc: Honorable Paul Shapiro, Mayor of Mansfield  
Linda Painter, the Mansfield Director of Planning & Development  
Michael Jednak AVP, University of Connecticut, the tower owner and property owner.

November 1, 2017

Linda Painter, Director of Planning & Development  
Audrey P. Beck Municipal Building  
4 South Eagleville Road, Mansfield, CT 06268

RE: AT&T Wireless Modifications to Telecommunication Facility –  
60 North Eagleville Road, Mansfield, CT 06268

Dear Ms. Painter:

In order to accommodate technological changes, implement the Uniform Mobile Telecommunications System and enhance system performance in the State of Connecticut, AT&T Wireless (“AT&T”) will be changing its equipment configuration at the above referenced telecommunications facility. AT&T Wireless currently maintains nine (9) antennas at the 185-foot level of an existing 291-foot guyed tower located at 60 North Eagleville Road, in Mansfield, CT. The tower is owned by the University of Connecticut. The property is owned by University of Connecticut.

AT&T Wireless now seeks to install three (3) additional antennas at the 291-foot level of the tower, to be mounted on the existing tower, as well as install six (6) new RRUS-32 Remote Radio Units (“RRU”), and install 3 new RRUS-B14 RRUs on the tower, behind the antennas. In addition, a single new surge suppressor will be installed behind the Gamma sector antennas.

This letter is intended to serve as the required notice to the municipality’s Planning and Zoning Department. As required by the Regulations of Connecticut State Agencies (“RCSA”) section 16-50j-73, the Connecticut Siting Council (“CSC”) has been notified of the proposed changes and will review AT&T’s proposal. Please accept this letter as notification under RCSA section 16-50j-73 of construction which constitutes an exempt modification pursuant to RCSA section 16-50j-72(b)(2).



The enclosed letter and documents to the CSC fully describes AT&T's proposal for the above referenced site. However, if you have any questions or require any additional information concerning our plans or the CSC procedures, please contact me at 443-286-4006 or contact Melanie Bachman, Acting Executive Director of the CSC at 860-872-2935.

Respectfully submitted,

Jack Andrews  
Zoning Manager, Empire Telecom  
o/b/o AT&T Wireless  
10130 Donleigh Drive  
Columbia, MD 21046  
443-677-0144  
[jandrews@empiretelecomm.com](mailto:jandrews@empiretelecomm.com)

Enclosures

cc: Melanie Bachman, Connecticut Siting Council

November 1, 2017

The Honorable Paul Shapiro  
Audrey P. Beck Municipal Building  
4 South Eagleville Road, Mansfield, CT 06268

RE: AT&T Wireless Modifications to Telecommunication Facility –  
60 North Eagleville Road, Mansfield, CT 06268

Dear Mayor Shapiro:

In order to accommodate technological changes, implement the Uniform Mobile Telecommunications System and enhance system performance in the State of Connecticut, AT&T Wireless (“AT&T”) will be changing its equipment configuration at the above referenced telecommunications facility. AT&T Wireless currently maintains nine (9) antennas at the 185-foot level of an existing 291-foot guyed tower located at 60 North Eagleville Road, in Mansfield, CT. The tower is owned by the University of Connecticut. The property is owned by University of Connecticut.

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Respectfully submitted,

Jack Andrews  
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10130 Donleigh Drive  
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[jandrews@empiretelecomm.com](mailto:jandrews@empiretelecomm.com)

Enclosures

cc: Melanie Bachman, Connecticut Siting Council

November 1, 2017

University of Connecticut  
Michael Jednak AVP,  
Facilities Operations & Building Services  
25 LeDoyt Road, Unit 3252  
Storrs CT 06269

RE: AT&T Wireless Modifications to Telecommunication Facility –  
60 North Eagleville Road, Mansfield, CT 06268

Dear Mr. Jednak:

In order to accommodate technological changes, implement the Uniform Mobile Telecommunications System and enhance system performance in the State of Connecticut, AT&T Wireless ("AT&T") will be changing its equipment configuration at the above referenced telecommunications facility. AT&T Wireless currently maintains nine (9) antennas at the 185-foot level of an existing 291-foot guyed tower located at 60 North Eagleville Road, in Mansfield, CT. The tower is owned by the University of Connecticut. The property is owned by University of Connecticut.

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This letter is intended to serve as the required notice to the tower owner and the property owner. As required by the Regulations of Connecticut State Agencies ("RCSA") section 16-50j-73, the Connecticut Siting Council ("CSC") has been notified of the proposed changes and will review AT&T's proposal. Please accept this letter as notification under RCSA section 16-50j-73 of construction which constitutes an exempt modification pursuant to RCSA section 16-50j-72(b)(2).

The enclosed letter and documents to the CSC fully describes AT&T's proposal for the above referenced site. However, if you have any questions or require any additional information concerning our plans or the CSC procedures, please contact me at 443-286-4006 or contact Melanie Bachman, Acting Executive Director of the CSC at 860-872-2935.

Respectfully submitted,

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Columbia, MD 21046  
443-677-0144  
[jandrews@empiretelecomm.com](mailto:jandrews@empiretelecomm.com)

Enclosures

cc: Melanie Bachman, Connecticut Siting Council



# Radio Frequency Emissions Analysis Report

AT&T Existing Facility

Site ID: CT1077

Storrs UCONN  
60 North Eagleville Road  
Mansfield, CT 06268

**September 19, 2017**

**Centerline Communications Project Number: 950006-075**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general population allowable limit:	<b>35.11 %</b>



September 19, 2017

AT&T Mobility – New England  
Attn: John Benedetto, RF Manager  
550 Cochituate Road  
Suite 550 – 13&14  
Framingham, MA 06040

### Emissions Analysis for Site: **CT1077 – Storrs UCONN**

Centerline Communications, LLC (“Centerline”) was directed to analyze the proposed AT&T facility located at **60 North Eagleville Road, Mansfield, CT**, for the purpose of determining whether the emissions from the Proposed AT&T Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limits for the 700 and 850 MHz Bands are approximately  $467 \mu\text{W}/\text{cm}^2$  and  $567 \mu\text{W}/\text{cm}^2$  respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 2300 MHz (WCS) bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.





## CALCULATIONS

Calculations were performed for the proposed AT&T Wireless antenna facility located at **60 North Eagleville Road, Mansfield, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since AT&T is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. All power values expressed and analyzed are maximum power levels expected to be used on all radios.

All emissions values for additional carriers were taken from the Connecticut Siting Council (CSC) active MPE database. Values in this database are provided by the individual carriers themselves. For this study the composite MPE values from the two listed adjacent towers are added into the composite value for this site.

For each sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
UMTS	850 MHz	2	30
LTE	850 MHz	2	60
LTE	700 MHz	6	60
LTE	2300 MHz (WCS)	2	60
LTE	2100 MHz (AWS)	2	60
LTE	1900 MHz (PCS)	4	60

*Table 1: Channel Data Table*

The following antennas listed in *Table 2* were used in the modeling for transmission in the 700 MHz, 850 MHz, 1900 MHz (PCS), 2100 MHz and 2300 MHz (WCS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

Sector	Antenna Number	Antenna Make / Model	Antenna Centerline (ft)
A	1	Powerwave 7770	185
A	2	CCI OPA-65R-LCUU-H8	185
A	3	CCI TPA-65R-LCUUUU-H8	185
A	4	CCI HPA-65R-BUU-H8	185
B	1	Powerwave 7770	185
B	2	CCI OPA-65R-LCUU-H6	185
B	3	Quintel QS66512-2	185
B	4	CCI HPA-65R-BUU-H6	185
C	1	Powerwave 7770	185
C	2	CCI OPA-65R-LCUU-H8	185
C	3	CCI TPA-65R-LCUUUU-H8	185
C	4	CCI HPA-65R-BUU-H8	185

*Table 2: Antenna Data*

All calculations were done with respect to uncontrolled / general population threshold limits.



## RESULTS

Per the calculations completed for the proposed AT&T configurations *Table 3* shows resulting emissions power levels and percentages of the FCC's allowable general population limit.

Antenna ID	Antenna Make / Model	Frequency Bands	Antenna Gain (dBi)	Channel Count	Total TX Power (W)	ERP (W)	MPE %
Antenna A1	Powerwave 7770	850 MHz	11.4	2	60	828.23	0.16
Antenna A2	CCI OPA-65R-LCUU-H8	850 MHz / 700 MHz / 2300 MHz (WCS)	13.35 / 12.55 / 14.95	6	360	8,505.20	1.45
Antenna A3	CCI TPA-65R-LCUUUU-H8	700 MHz / 2100 MHz (AWS) / 1900 MHz (PCS)	12.95 / 14.25 / 13.75	6	360	8,405.43	1.25
Antenna A4	CCI HPA-65R-BUU-H8	700 MHz / 1900 MHz (PCS)	13.15 / 14.95	4	240	6,229.75	1.02
Sector A Composite MPE%							<b>3.88</b>
Antenna B1	Powerwave 7770	850 MHz	11.4	2	60	828.23	0.16
Antenna B2	CCI OPA-65R-LCUU-H6	850 MHz / 700 MHz / 2300 MHz (WCS)	12.45 / 11.65 / 15.45	6	360	8,073.14	1.31
Antenna B3	Quintel QS66512-2	700 MHz / 2100 MHz (AWS) / 1900 MHz (PCS)	10.85 / 14.35 / 13.85	6	360	7,638.60	1.04
Antenna B4	CCI HPA-65R-BUU-H6	700 MHz / 1900 MHz (PCS)	11.95 / 14.75	4	240	5,462.56	0.85
Sector B Composite MPE%							<b>3.37</b>
Antenna C1	Powerwave 7770	850 MHz	11.4	2	60	828.23	0.16
Antenna C2	CCI OPA-65R-LCUU-H8	850 MHz / 700 MHz / 2300 MHz (WCS)	13.35 / 12.55 / 14.95	6	360	8,505.20	1.45
Antenna C3	CCI TPA-65R-LCUUUU-H8	700 MHz / 2100 MHz (AWS) / 1900 MHz (PCS)	12.95 / 14.25 / 13.75	6	360	8,405.43	1.25
Antenna C4	CCI HPA-65R-BUU-H8	700 MHz / 1900 MHz (PCS)	13.15 / 14.95	4	240	6,229.75	1.02
Sector C Composite MPE%							<b>3.88</b>

*Table 3: AT&T Emissions Levels*



The Following table (*table 4*) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum AT&T MPE contributions per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, the sectors with the largest calculated MPE% are Sectors A & C. *Table 5* below shows a summary for each AT&T Sector as well as the composite MPE value for the site.

Site Composite MPE%	
Carrier	MPE%
AT&T – Max Sector Value	<b>3.88 %</b>
UConn Police	0.12 %
Existing	14.12 %
Nextel	0.10 %
MetroPCS	0.14 %
Verizon Wireless	6.60 %
Adjacent Tower 1 - 82 No. Eagleville (Sprint)	2.19 %
Adjacent Tower 2 - No. Eagleville (SNET)	7.96 %
<b>Site Total MPE %:</b>	<b>35.11 %</b>

*Table 4: All Carrier MPE Contributions*

AT&T Sector A Total:	3.88 %
AT&T Sector B Total:	3.37 %
AT&T Sector C Total:	3.88 %
Site Total:	35.11 %

*Table 5: Site MPE Summary*



FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated AT&T sector(s). For this site, the sectors with the largest calculated MPE% are Sectors A & C.

AT&T _ Frequency Band / Technology (Sector A & C)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
AT&T 850 MHz UMTS – Antenna 1	2	414.12	185	0.93	850 MHz	567	0.16%
AT&T 850 MHz LTE– Antenna 2	2	1,297.63	185	2.91	850 MHz	567	0.51%
AT&T 700 MHz LTE– Antenna 2	2	1,079.32	185	2.42	700 MHz	467	0.52%
AT&T 2300 MHz (WCS) LTE– Antenna 2	2	1,875.65	185	4.21	2300 MHz (WCS)	1000	0.42%
AT&T 700 MHz LTE– Antenna 3	2	1,183.45	185	2.66	700 MHz	467	0.57%
AT&T 2100 MHz (AWS) LTE– Antenna 3	2	1,596.44	185	3.58	2100 MHz (AWS)	1000	0.36%
AT&T 1900 MHz (PCS) LTE– Antenna 3	2	1,422.82	185	3.19	1900 MHz (PCS)	1000	0.32%
AT&T 700 MHz LTE– Antenna 4	2	1,239.23	185	2.78	700 MHz	467	0.60%
AT&T 1900 MHz (PCS) LTE– Antenna 4	2	1,875.65	185	4.21	1900 MHz (PCS)	1000	0.42%
						<b>Total:</b>	<b>3.88%</b>

*Table 6: AT&T Maximum Sector MPE Power Values*



## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the AT&T facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

AT&T Sector	Power Density Value (%)
Sector A:	3.88 %
Sector B:	3.37 %
Sector C:	3.88 %
AT&T Maximum Total (per sector):	3.88 %
Site Total:	35.11 %
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **35.11 %** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

A handwritten signature in blue ink, appearing to read 'Scott Heffernan', is positioned above the printed name.

Scott Heffernan  
RF Engineering Director  
**Centerline Communications, LLC**  
95 Ryan Drive, Suite 1  
Raynham, MA 02767

## **Structural Analysis Report**

*327' Existing Guyed Lattice Tower*

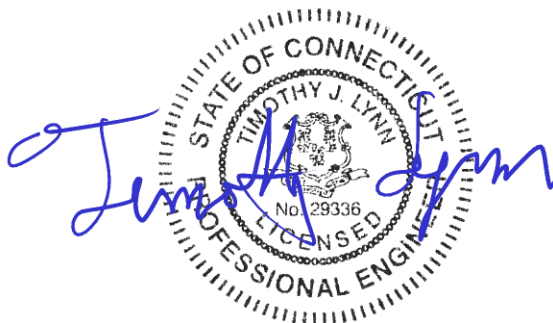
*AT&T Mobility - LTE 6C/7C*

*AT&T Site Ref: CT1077  
Storrs-Uconn*

*60 North Eagleville Road  
Mansfield, CT*

*Centek Project No. 17004.42*

*Date: September 11, 2017*



**Prepared for:**  
AT&T Mobility  
500 Enterprise Drive, Suite 3A  
Rocky Hill, CT 06067

## **Table of Contents**

### **SECTION 1 – REPORT**

- INTRODUCTION
- ANTENNA AND APPURTENANCE SUMMARY
- PRIMARY ASSUMPTIONS USED IN THE ANALYSIS
- ANALYSIS
- TOWER LOADING
- TOWER CAPACITY
- CONCLUSION AND RECOMMENDATIONS

### **SECTION 2 – CONDITIONS & SOFTWARE**

- STANDARD ENGINEERING CONDITIONS
- GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM

### **SECTION 3 – CALCULATIONS**

- tnxTower INPUT/OUTPUT SUMMARY
- tnxTower FEED LINE PLAN
- tnxTower FEED LINE DISTRIBUTION
- tnxTower LEG COMPRESSION DIAGRAM
- tnxTower GLOBAL MAST SHEAR AND MOMENT DIAGRAMS
- tnxTower DEFLECTION DIAGRAMS
- tnxTower STRESS DISTRIBUTION
- tnxTower WIND PRESSURE AND ICE THICKNESS
- tnxTower DETAILED OUTPUT
- TOWER BASE FOUNDATION ANALYSIS
- GUY ANCHOR FOUNDATION ANALYSIS

### **SECTION 4 – REFERENCE MATERIALS**

- RF DATA SHEET



## Introduction

The purpose of this report is to summarize the results of the non-linear, P- $\Delta$  structural analysis of the antenna upgrade proposed by AT&T on the existing guyed lattice tower located in Storrs, CT.

The host tower is a 327-ft, three legged, guyed lattice tower. The original tower design documents were unavailable for use in this report. The tower geometry, structure member sizes and foundation information were obtained from a previous structural report prepared by Destek job no. 1629069 dated October 17, 2016.

Antenna and appurtenance information were obtained from the aforementioned structural report and a AT&T RF data sheet.

The tower consists of one (1) pole section, fifteen (15) straight and one (1) tapered base vertical sections consisting of solid round legs steel grade of ASTM A572-50. Diagonal and horizontal lateral support bracing consists of solid round steel grade of ASTM A36. The vertical tower sections are connected by bolted flanges with the diagonal and horizontal bracing to legs consisting of welded connections. The width of the tower face is 3.67-ft throughout its length.

## Antenna and Appurtenance Summary

The existing and proposed loads considered in the analysis consist of the following:

- UNKNOWN (EXISTING):  
Antennas: One (1) 4-ft lighting rod and one (1) light beacon mounted to the top of the tower.  
Cables: One (1) 1/2" rigid conduit
- UNKNOWN (EXISTING):  
Antennas: One (1) Shively Labs 6813 FM antenna flush mounted with an elevation of 305-ft above grade.  
Cables: One (1) 7/8"  $\varnothing$  coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (FUTURE):  
Antennas: Three (3) 6-ft microwave dishes leg mounted with a RAD center elevation of 290-ft above grade.  
Cables: Three (3) WE65 cables running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: Two (2) Celwave PD1110 omni-directional antennas mounted on one (1) 4-ft sidearm with an elevation of 277-ft above grade.  
Cables: Two (2) 1/2"  $\varnothing$  coax cables running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) Kathrein Scala OGT9-840 and one (1) Decibel DB810K omni-directional antennas leg mounted with an elevation of 267-ft above grade.  
Cables: Two (2) 1-5/8"  $\varnothing$  coax cables running on a leg/face of the existing tower as specified in Section 3 of this report.

- UNKNOWN (EXISTING):  
Antennas: Two (2) Kathrein Scala AP14-850/105 panel antennas mounted on a 3-ft standoff with an elevation of 255-ft above grade.  
Cables: Two (2) 1-5/8" Ø coax cables running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (FUTURE):  
Antennas: Five (5) Sinclair SC479-HF1LDF omni-directional antennas (3 inverted), two (2) Bird 432-83H-01T tower top amplifiers and one (1) Sinclair SE419-SF3P4LDF panel antenna leg mounted with an elevation of 250-ft above grade.  
Cables: Six (6) 1-5/8" Ø and two (2) 1/2" Ø coax cables running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: Two (2) Kathrein Scala OGT9-840 omni-directional antennas (inverted) leg mounted with an elevation of 253-ft above grade.  
Cables: Two (2) 1-5/8" Ø coax cables running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (FUTURE):  
Antennas: Five (5) Sinclair SC479-HF1LDF omni-directional antennas (3 inverted) and two (2) Bird 432-83H-01T tower top amplifiers leg mounted with an elevation of 240-ft above grade.  
Cables: Five (5) 1-5/8" Ø and two (2) 1/2" Ø coax cables running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) Shively Labs 6813 FM antenna flush mounted with an elevation of 211-ft above grade.  
Cables: One (1) 7/8" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) Shively Labs 6813 FM antenna flush mounted with an elevation of 305-ft above grade.  
Cables: One (1) 7/8" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) Shively Labs 6812 FM antenna flush mounted with an elevation of 305-ft above grade.  
Cables: One (1) 1/2" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) 6-ft Yagi antenna flush mounted with an elevation of 190-ft above grade.  
Cables: One (1) 1/2" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.

- UNKNOWN (EXISTING):  
Antennas: One (1) 2'x1'x5" panel antenna flush mounted with an elevation of 172'-2"-ft above grade.  
Cables: One (1) 7/8" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) 8' omni-directional antenna flush mounted with an elevation of 172-ft above grade.  
Cables: One (1) 7/8" Ø coax cables running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) 5' grid dish antenna flush mounted with an elevation of 171'-6"-ft above grade.  
Cables: One (1) 1/2" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) 2'x1'x5" panel antenna flush mounted with an elevation of 158'-10"-ft above grade.  
Cables: One (1) 1/2" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: Three (3) light beacons mounted to the tower with an elevation of 157'.  
Cables: One (1) 1/2" rigid conduit.
- UNKNOWN (FUTURE):  
Antennas: Two (2) 6-ft microwave dishes w/ ice shields pipe mounted with a RAD center elevation of 116-ft above grade.  
Cables: Two (2) EW63 cables running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) Celwave PD1110 omni-directional antenna mounted on one (1) 2-ft sidearm with an elevation of 112-ft above grade.  
Cables: One (1) 7/8" Ø coax cables running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (FUTURE):  
Antennas: One (1) 6-ft microwave dish pipe mounted with a RAD center elevation of 104-ft above grade.  
Cables: One (1) EW63 cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- UNKNOWN (EXISTING):  
Antennas: One (1) Kathrein PR-850 paraflector leg mounted with an elevation of 94-ft above grade.  
Cables: One (1) 1/2" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.

- **UNKNOWN (EXISTING):**  
Antennas: One (1) Decibel ASP-962 yagi leg mounted with an elevation of 94-ft above grade.  
Cables: One (1) 1/2" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- **UNKNOWN (EXISTING):**  
Antennas: One (1) Decibel DB212-1 dipole leg mounted with an elevation of 70-ft above grade.  
Cables: One (1) 7/8" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- **UNKNOWN (EXISTING):**  
Antennas: One (1) 6-ft yagi mounted on a 2-ft sidearm with an elevation of 18-ft above grade.  
Cables: One (1) 1/2" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- **UNKNOWN (EXISTING):**  
Antennas: One (1) 4-ft microwave dish mounted on a 2-ft sidearm with an elevation of 13-ft above grade.  
Cables: One (1) 1/2" Ø coax cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- **VERIZON (EXISTING):**  
Antennas: Three (3) Antel BXA-80063-4CF panel antennas, three (3) JMA X7C-FRO-440 panel antennas, six (6) Andrew HBXX-6517DS panel antennas, three (3) Alcatel-Lucent RRH-2x40-700, three (3) Alcatel-Lucent RRH-2x60-AWS, three (3) Alcatel-Lucent RRH-2x90-PCS one (1) Raycap RC2DC-3315-PF-48 distribution box mounted on (1) 13-ft platform w/ handrails with a rad center elevation of 84-ft above grade level.  
Cables: Twelve (12) 1-1/4" Ø coax cables and one (1) 1-5/8" Ø fiber cable running on a leg/face of the existing tower as specified in Section 3 of this report.
- **AT&T (EXISTING TO REMAIN):**  
Antennas: Three (3) Powerwave 7770 panel antennas, one (1) CCI OPA-65R-LCUU-H6 panel antenna, two (2) CCI OPA-65R-LCUU-H8 panel antennas, one (1) CCI HPA-65R-BUU-H6 panel antenna, two (2) CCI HPA-65R-BUU-H8 panel antennas, three (3) CCI DTMAPB7819VG12A TMAs, six (6) CCI TPX-070821 triplexers, three (3) Ericsson RRUS-11, six (6) Ericsson RRUS-32 and two (2) Raycap DC6-48-60-18-8F surge arrestors mounted on three (3) 12-ft V-Frames with a RAD center elevation of 185-ft above grade level.  
Coax Cables: Twelve (12) 1-5/8" Ø coax cables, two (2) fiber cables and four (4) dc control cables running on the inside of the existing tower.
- **AT&T (PROPOSED):**  
Antennas: Two (2) CCI TPA-65R-LCUUUU-H8 panel antenna, one (1) Qunitel QS66512-2 panel antenna, six (6) Ericsson RRUS-32, three (3) B14 4478 and one (1) Raycap DC6-48-60-18-8F surge arrestor mounted on three (3) 12-ft V-Frames with a RAD center elevation of 185-ft above grade level.  
Coax Cables: One (1) fiber cable and two (2) dc control cables running on the inside of the existing tower.

### Primary Assumptions Used in the Analysis

- The tower structure's theoretical capacity not including any assessment of the condition of the tower.
- The tower carries the horizontal and vertical loads due to the weight of antennas, ice load and wind.
- Tower is properly installed and maintained.
- Tower is in plumb condition.
- Tower loading for antennas and mounts as listed in this report.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds are fabricated with ER-70S-6 electrodes.
- All members are assumed to be as specified in the original tower design documents.
- All members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
- All member protective coatings are in good condition.
- All tower members were properly designed, detailed, fabricated, installed and have been properly maintained since erection.
- Any deviation from the analyzed antenna loading will require a new analysis for verification of structural adequacy.
- All coax cables routed as specified in Section 3 of this report.

## A n a l y s i s

The existing tower was analyzed using a comprehensive computer program entitled tnxTower. The program analyzes the tower, considering the worst case loading condition. The tower is considered as loaded by concentric forces along the tower, and the model assumes that the tower members are subjected to bending, axial, and shear forces.

The existing tower was analyzed for the controlling basic wind speed (3-second gust) with no ice and the applicable wind and ice combination to determine stresses in members as per guidelines of TIA-222-G-2005 entitled "Structural Standard for Antenna Support Structures and Antennas", the American Institute of Steel Construction (AISC) and the Manual of Steel Construction; Load and Resistance Factor Design (LRFD).

The controlling wind speed is determined by evaluating the local available wind speed data as provided in Appendix N of the CSBC<sup>1</sup> and the wind speed data available in the TIA-222-G-2005 Standard.

## T o w e r   L o a d i n g

Tower loading was determined by the basic wind speed as applied to projected surface areas with modification factors per TIA-222-G-2005, gravity loads of the tower structure and its components, and the application of 1.00" radial ice on the tower structure and its components.

Basic Wind Speed:	Tolland County; v = 95-105 mph (3-second gust)	[Annex B of TIA-222-G-2005]
	Storrs; v = 101 mph (3 second gust)	[Appendix N of the 2016 CT Building Code]
Load Cases:	<u>Load Case 1</u> ; 101 mph wind speed w/ no ice plus gravity load – used in calculation of tower stresses and rotation.	[Appendix N of the 2016 CT Building Code]
	<u>Load Case 2</u> ; 40 mph wind speed w/ 1.00" radial ice plus gravity load – used in calculation of tower stresses.	[Annex B of TIA-222-G-2005]

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<sup>1</sup> The 2012 International Building Code as amended by the 2016 Connecticut State Building Code (CSBC).

## Tower Capacity

- Calculated stresses were found to be within allowable limits. In Load Case 2, per tnxTower "Section Capacity Table", this tower was found to be at **89.7%** of its total capacity.

Tower Section	Elevation	Stress Ratio (percentage of capacity)	Result
Leg (T16)	0'-0"-6'-6"	89.7%	<b>PASS</b>
Diagonal (T13)	39'-10"-59'-10"	45.9%	<b>PASS</b>
Top Girt (T16)	0'-0"-6'-6"	25.1%	<b>PASS</b>
Guy A @ 235-ft radius (T7)	166.5-ft	49.7%	<b>PASS</b>

## Foundations and Anchorage

The existing tower base foundation consists of a 3.0-ft diameter x 2.5-ft long reinforced concrete pier on a 10-ft square x 2.0-ft thick reinforced concrete pad bearing directly on the existing sub grade. Additionally, guy wire loading is transferred to three (3) 4.5'x4.0'x24.0' concrete support blocks. The sub-grade conditions used as the basis for the foundation analysis were derived from the aforementioned structural report.

- The worst case tower base and guy anchor reactions developed from the governing Load Case 1 were used in the verification of the anchorage foundations:

Tower Guy Reactions	
Vector	Inner
Horizontal (In Plane of GW)	139 kips
Horizontal (Out of Plane of GW)	5 kips
Vertical	109 kips
Resultant Force at end of Guy Wire	176 kips
Tower Base Reactions	
Vector	Proposed Reaction
Horizontal Shear	4.0 kips
Axial Compression	580.0 kips

Foundation	Design Limit	TIA-222-G Section 9.4 FS <sup>(1)</sup>	Proposed Loading (FS) <sup>(1)</sup>	Result
Reinf. Conc. Anchor Block	Uplift	1.0	2.27	<b>PASS</b>
	Sliding	1.0	2.90	<b>PASS</b>
		<b>Ultimate Bearing</b>	<b>Proposed</b>	
Base Foundation	Bearing	11.0 ksf	6.13 ksf	<b>PASS</b>

| Note 1: FS denotes 'Factor of Safety'.

## Conclusion

This analysis shows that the subject tower **is adequate** to support the proposed modified antenna configuration with the below recommendations.

The analysis is based, in part, on the information provided to this office by AT&T. If the existing conditions are different than the information in this report, Centek Engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:



Timothy J. Lynn, PE  
 Structural Engineer





*Standard Conditions for Furnishing of  
Professional Engineering Services on  
Existing Structures*

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of Centek Engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to Centek Engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the "as new" condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222
- All services performed, results obtained, and recommendations made are in accordance with generally accepted engineering principles and practices. Centek Engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

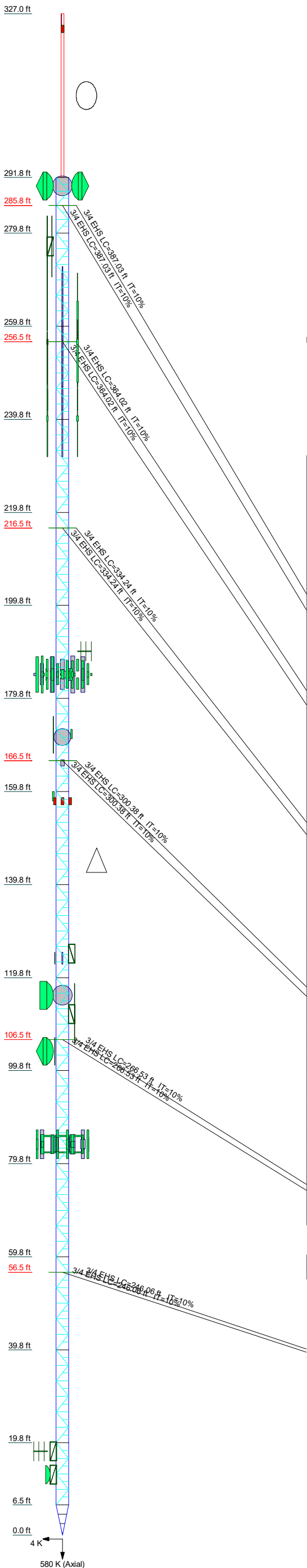
## GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM

tnxTower, is an integrated structural analysis and design software package for Designed specifically for the telecommunications industry, tnxTower, formerly ERITower, automates much of the tower analysis and design required by the TIA/EIA 222 Standard.

### tnxTower Features:

- tnxTower can analyze and design 3- and 4-sided guyed towers, 3- and 4-sided self-supporting towers and either round or tapered ground mounted poles with or without guys.
- The program analyzes towers using the TIA-222-G (2005) standard or any of the previous TIA/EIA standards back to RS-222 (1959). Steel design is checked using the AISC ASD 9th Edition or the AISC LRFD specifications.
- Linear and non-linear (P-delta) analyses can be used in determining displacements and forces in the structure. Wind pressures and forces are automatically calculated.
- Extensive graphics plots include material take-off, shear-moment, leg compression, displacement, twist, feed line, guy anchor and stress plots.
- tnxTower contains unique features such as True Cable behavior, hog rod take-up, foundation stiffness and much more.

Section	T16	T15	T14	T13	T12	T11	T10	T9	T8	T7	T6	T5	T4	T3	T2	T1	L1
Legs				SR 3				SR 2 3/4		SR 2 3/4		SR 2 1/2		SR 2 1/4	SR 2		P1075x0.843
Leg Grade									A572-50								A53-B-35
Diagonals	N.A.			SR 1 1/4		SR 1 3/8	SR 1 1/2	SR 1 1/4	SR 1 3/8	SR 1 1/2		SR 1 1/4		SR 1 1/2		SR 1 3/8	N.A.
Diagonal Grade	N.A.								A36								N.A.
Top Gifts	12x3/8								SR 1								N.A.
Mid Gifts	9x3/8									N.A.							
Bottom Gifts	12x3/8									N.A.							
Horizontals	N.A.									N.A.							
Sec. Horizontals	N.A.								SR 1								N.A.
Top Guy Pull-Offs		N.A.		MC12x35		N.A.		MC12x35	N.A.	MC12x35		N.A.		MC12x35	N.A.	MC12x35	N.A.
Face Width (ft)																3.67	0.895833
# Panels @ (ft)	7 @ 1	4 @ 3.335							78 @ 3.33333							4 @ 3	N.A.
Weight (K)	36.4	1.3	2.0	3.2	2.0	2.1	3.2	1.8	1.7	3.2	1.6	2.9	1.5	2.7	1.3	2.0	3.1



## PLAN

### DESIGNED APPURTENANCE LOADING

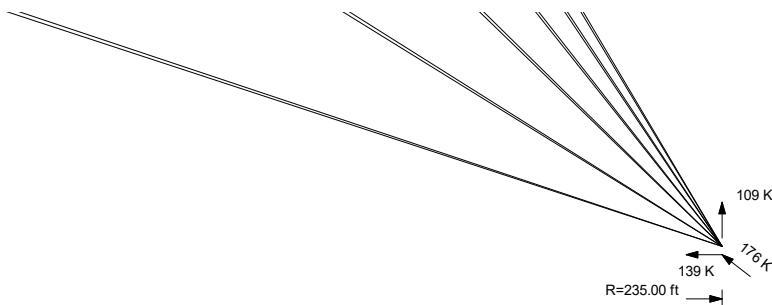
TYPE	ELEVATION	TYPE	ELEVATION
4-ft Lightning Rod	325	RRUS-32 (ATI - Proposed)	185
Flash Beacon Lighting	323	RRUS-32 (ATI - Proposed)	185
6813 1-Bay w/radome (WHUS-34)	305	RRUS-32 (ATI - Proposed)	185
6-ft Dish (CSP - Future)	290	RRUS-32 (ATI - Proposed)	185
6-ft Dish (CSP - Future)	290	B14 4478 (ATI - Proposed)	185
6-ft Dish (CSP - Future)	290	B14 4478 (ATI - Proposed)	185
PD1110 (WHUS-40)	277	B14 4478 (ATI - Proposed)	185
PD1110 (WHUS-39)	277	DC6-48-60-18-8F Surge Arrestor (ATI - Existing)	185
ROHN 4-ft Side Arm (WHUS-40,39)	277	DC6-48-60-18-8F Surge Arrestor (ATI - Existing)	185
OGT9-840 (CSP-9)	267	DC6-48-60-18-8F Surge Arrestor (ATI - Existing)	185
DB810K (CSP-5)	267	12' V-Frame (ATI - Existing)	185
SC479-HF1LDF (CSP-12 Future)	264	12' V-Frame (ATI - Existing)	185
SC479-HF1LDF (CSP-13 Future)	264	12' V-Frame (ATI - Existing)	185
AP14-850/105 (CSP-4)	261	24"x12"x5" Panel	172.17
TTA 432-83H-01T (CSP-19 Future)	256.5	8' x 3" Dia Omni	172
SE419-SF3P4LDF (CSP-16 Future)	256.5	5' Gird Dish	171.5
TTA 432-83H-01T (CSP-15 Future)	256.5	16"x12"x3" TTA	166
SC479-HF1LDF (CSP-20 Future)	254	24"x12"x5" Panel	158.83
SC479-HF1LDF (CSP-21 Future)	254	Beacon	157
OGT9-840 (CSP-7 - Inverted)	253	Beacon	157
OGT9-840 (CSP-11 - Inverted)	253	Beacon	157
AP14-850/105 (CSP-6)	252	Sabre 2' Sidearm	125
SC479-HF1LDF (CSP-18 Future - Inverted)	249	6"x4" Ice Shield	124
SC479-HF1LDF (CSP-14 Future - Inverted)	249	9'x10' Ice Shield	124
SC479-HF1LDF (CSP-17 Future - Inverted)	249	2'6"x4" Pipe Mount	124
TTA 432-83H-01T (CSP-23 Future)	240	2'6"x4" Pipe Mount	124
TTA 432-83H-01T (CSP-26 Future)	240	6-ft Dish	116
SC479-HF1LDF (CSP-22 Future - Inverted)	239	6-ft Dish	116
SC479-HF1LDF (CSP-24 Future - Inverted)	239	Sabre 2' Sidearm	112
SC479-HF1LDF (CSP-25 Future - Inverted)	239	PD1110	112
6813 1-Bay w/radome (WHUS-36)	211	6"x4" Pipe Mount	104
6813 1-Bay w/radome (WHUS-34)	198	6-ft Dish	104
6812 (CPR-32 - Future)	198	PR-850	94
6' Yagi (CPR-33 - Future)	190	ASP-962	94
7770.00 (ATI - Existing)	185	HBXX-6517DS (Verizon)	84
OPA-65R-LCUU-H8 (ATI - Existing)	185	BXA-80063-4CF (Verizon)	84
TPA-65R-LCUUUU-H8 (ATI - Proposed)	185	X7C-FRO-440 (Verizon)	84
HPA-65R-BUUU-H8 (ATI - Existing)	185	HBXX-6517DS (Verizon)	84
7770.00 (ATI - Existing)	185	HBXX-6517DS (Verizon)	84
OPA-65R-LCUU-H6 (ATI - Existing)	185	BXA-80063-4CF (Verizon)	84
QS66512-2 (ATI - Proposed)	185	X7C-FRO-440 (Verizon)	84
HPA-65R-BUUU-H6 (ATI - Existing)	185	HBXX-6517DS (Verizon)	84
7770.00 (ATI - Existing)	185	RRH2x60-AWS (Verizon)	84
OPA-65R-LCUU-H8 (ATI - Existing)	185	RRH2x60-AWS (Verizon)	84
TPA-65R-LCUUUU-H8 (ATI - Proposed)	185	RRH2x60-AWS (Verizon)	84
HPA-65R-BUUU-H8 (ATI - Existing)	185	RRH2x40-07-U (Verizon)	84
DTMABP7819VG12A TMA (ATI - Existing)	185	RRH2x40-07-U (Verizon)	84
DTMABP7819VG12A TMA (ATI - Existing)	185	RRH2x40-07-U (Verizon)	84
DTMABP7819VG12A TMA (ATI - Existing)	185	FD-RRH 4x45 1900 (Verizon)	84
(2) TPX-070821 (ATI - Existing)	185	FD-RRH 4x45 1900 (Verizon)	84
(2) TPX-070821 (ATI - Existing)	185	FD-RRH 4x45 1900 (Verizon)	84
(2) TPX-070821 (ATI - Existing)	185	RC2DC-3315-PF-48 (Verizon)	84
RRUS-11 (ATI - Existing)	185	13' Platform w/rails (Verizon)	84
RRUS-11 (ATI - Existing)	185	HBXX-6517DS (Verizon)	84
RRUS-11 (ATI - Existing)	185	X7C-FRO-440 (Verizon)	84
RRUS-32 (ATI - Existing)	185	HBXX-6517DS (Verizon)	84
RRUS-32 (ATI - Existing)	185	BXA-80063-4CF (Verizon)	84
RRUS-32 (ATI - Existing)	185	DB212-1 (CSP-10)	70
RRUS-32 (ATI - Existing)	185	Sabre 2' Sidearm	18
RRUS-32 (ATI - Existing)	185	6' Yagi	18
RRUS-32 (ATI - Existing)	185	Sabre 2' Sidearm	13
RRUS-32 (ATI - Proposed)	185	4-ft Dish	13
RRUS-32 (ATI - Proposed)	185		

## MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

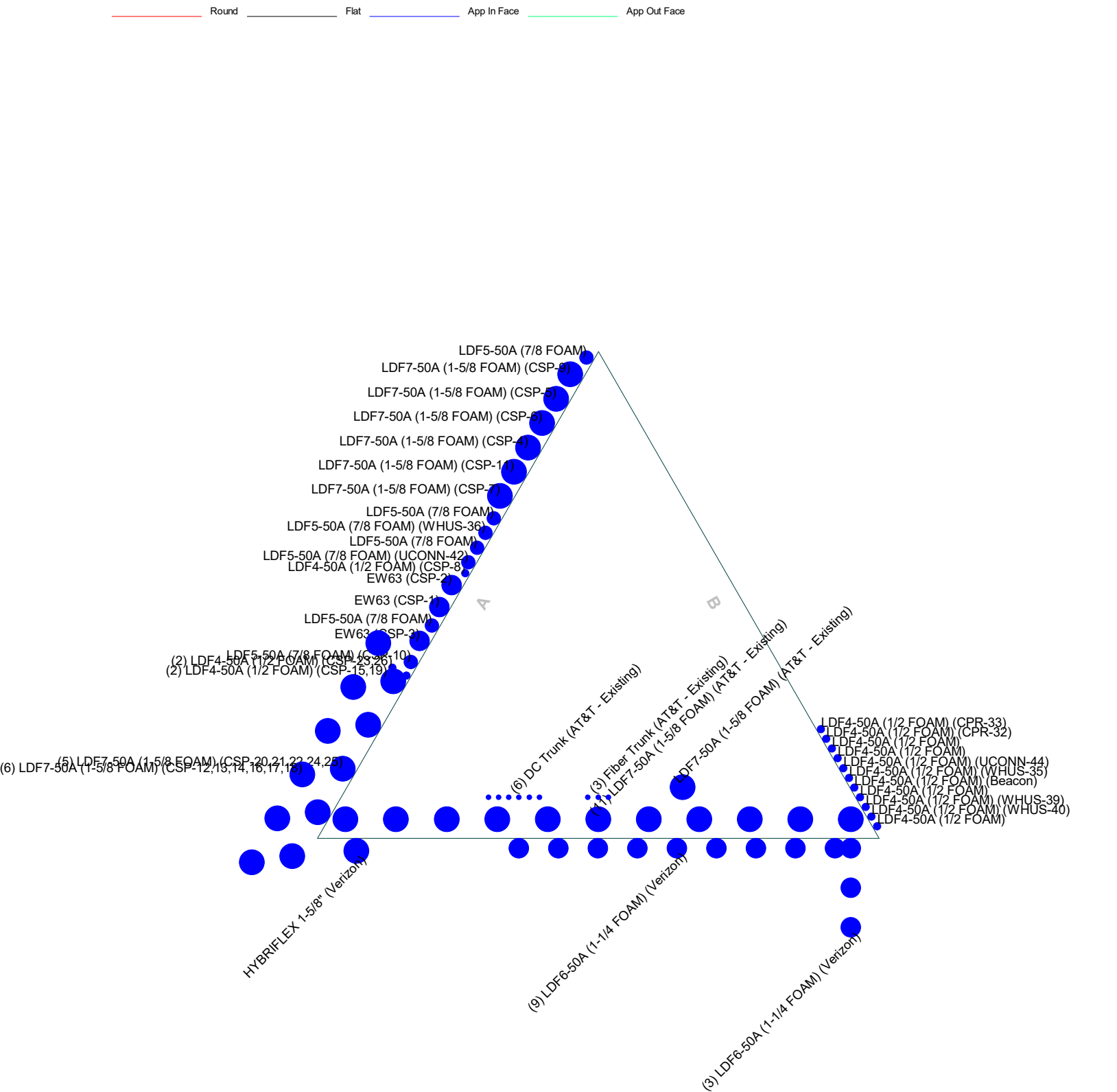
## TOWER DESIGN NOTES

1. Tower designed for Exposure C to the TIA-222-G Standard.
2. Tower designed for a 101 mph basic wind in accordance with the TIA-222-G Standard.
3. Tower is also designed for a 40 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 60 mph wind.
5. Tower Structure Class II.
6. Topographic Category 1 with Crest Height of 0.00 ft
7. TOWER RATING: 89.7%



ALL REACTIONS ARE FACTORED

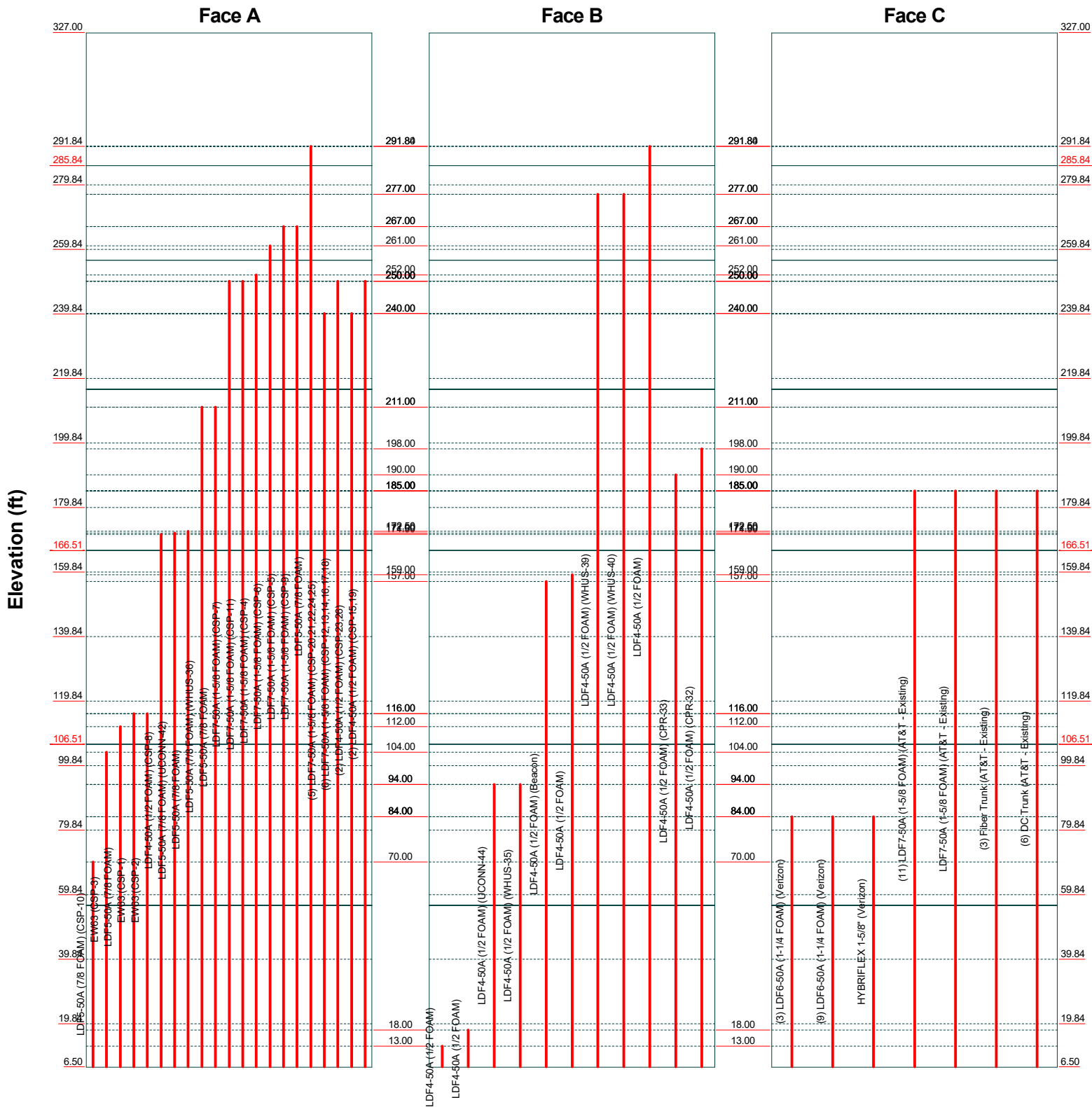
# Feed Line Plan



<b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587		Job: <b>17004.42 - CT1077</b> Project: <b>327' Guyed Tower - N. Eagleville Road Storrs, CT</b>	
Client: AT&T	Drawn by: T.JL	App'd:	
Code: TIA-222-G	Date: 09/11/17	Scale: NTS	
Path:		Dwg No. E-7	

Feed Line Distribution Chart  
6'6" - 327'

Round Flat App In Face App Out Face Truss Leg

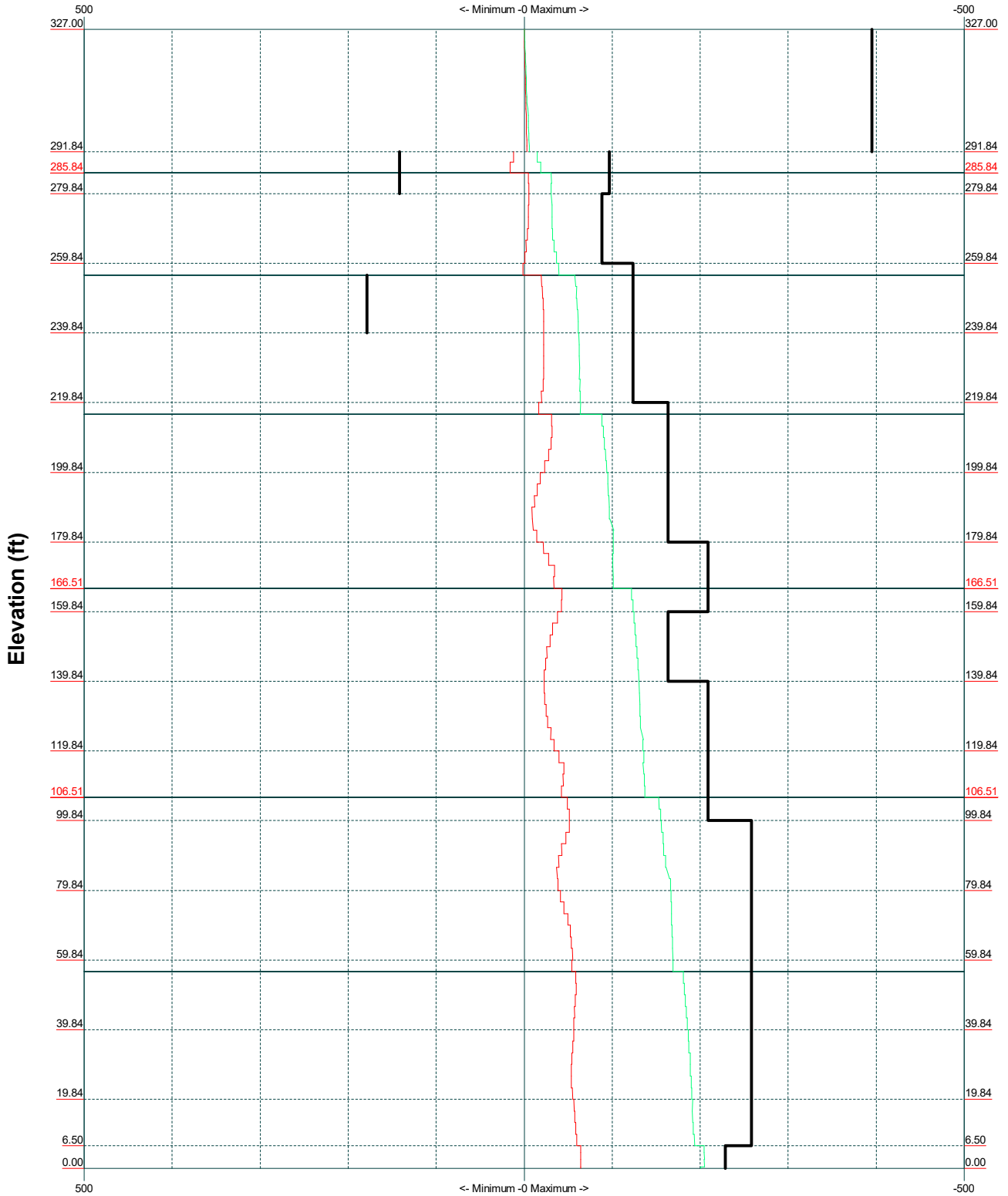


<b>Centek Engineering Inc.</b>		
63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587		
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Project: <b>327' Guyed Tower - N. Eagleville Road Storrs, CT</b>		
Client: AT&T	Drawn by: T.JL	App'd:
Code: TIA-222-G	Date: 09/11/17	Scale: NTS
Path:	Dwg No. E-7	

# TIA-222-G - 101 mph/40 mph 1.0000 in Ice Exposure C

Leg Capacity ———

Leg Compression (K)



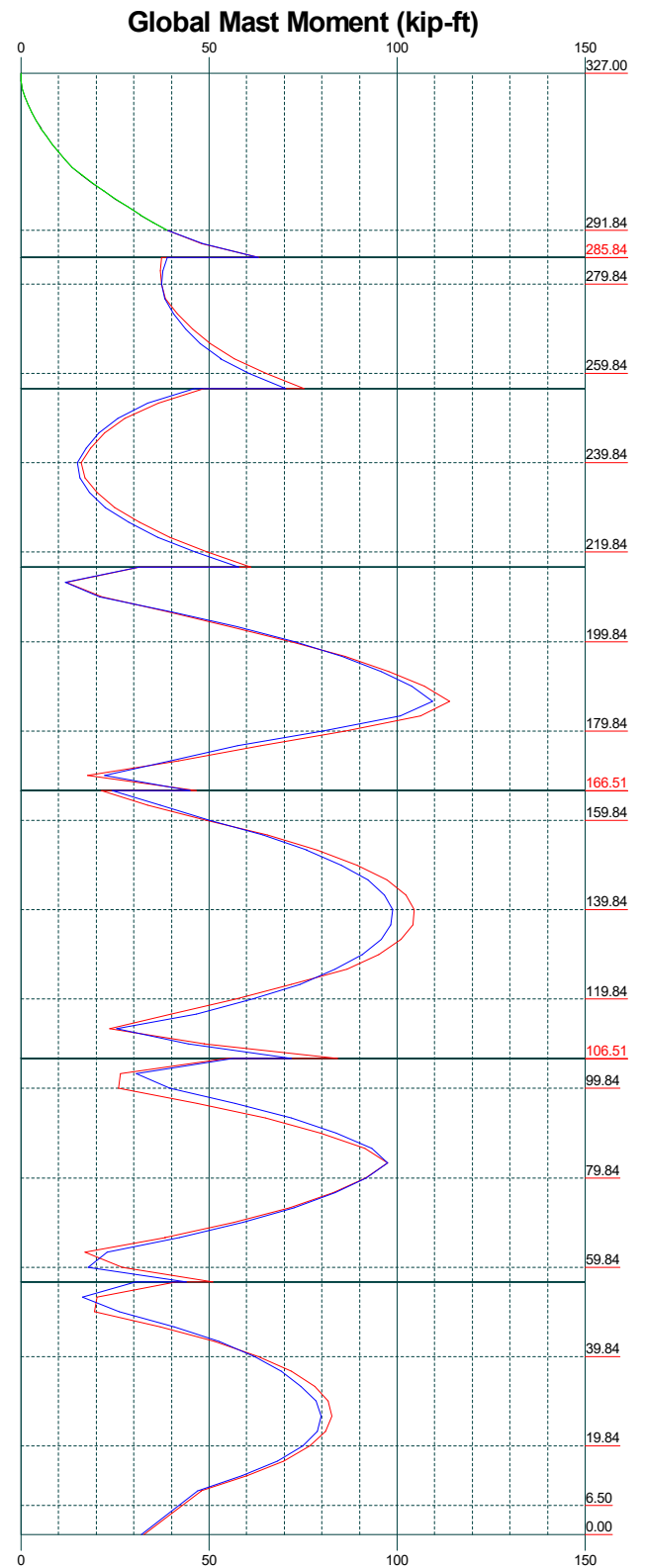
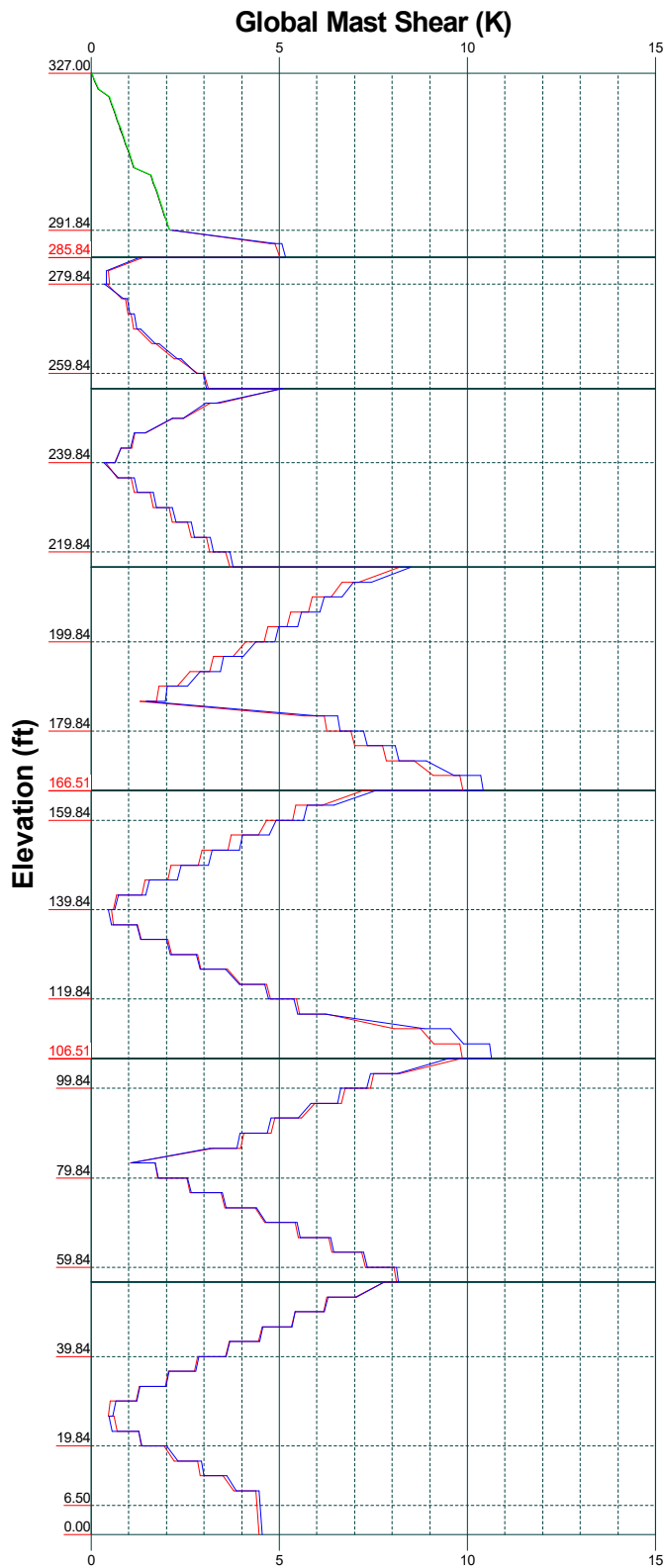
<b>Centek Engineering Inc.</b>		Job: <b>17004.42 - CT1077</b>	
63-2 North Branford Rd.		Project: <b>327' Guyed Tower - N. Eagleville Road Storrs, CT</b>	
Branford, CT 06405		Client: AT&T	Drawn by: T.JL
Phone: (203) 488-0580		Code: TIA-222-G	Date: 09/11/17
FAX: (203) 488-8587		Path:	Scale: NTS
		Dwg No. E-3	

Vx

Vz

Mx

Mz



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Project: **327' Guyed Tower - N. Eagleville Road Storrs, CT**

Client: **AT&T**

Drawn by: **TJL**

App'd:

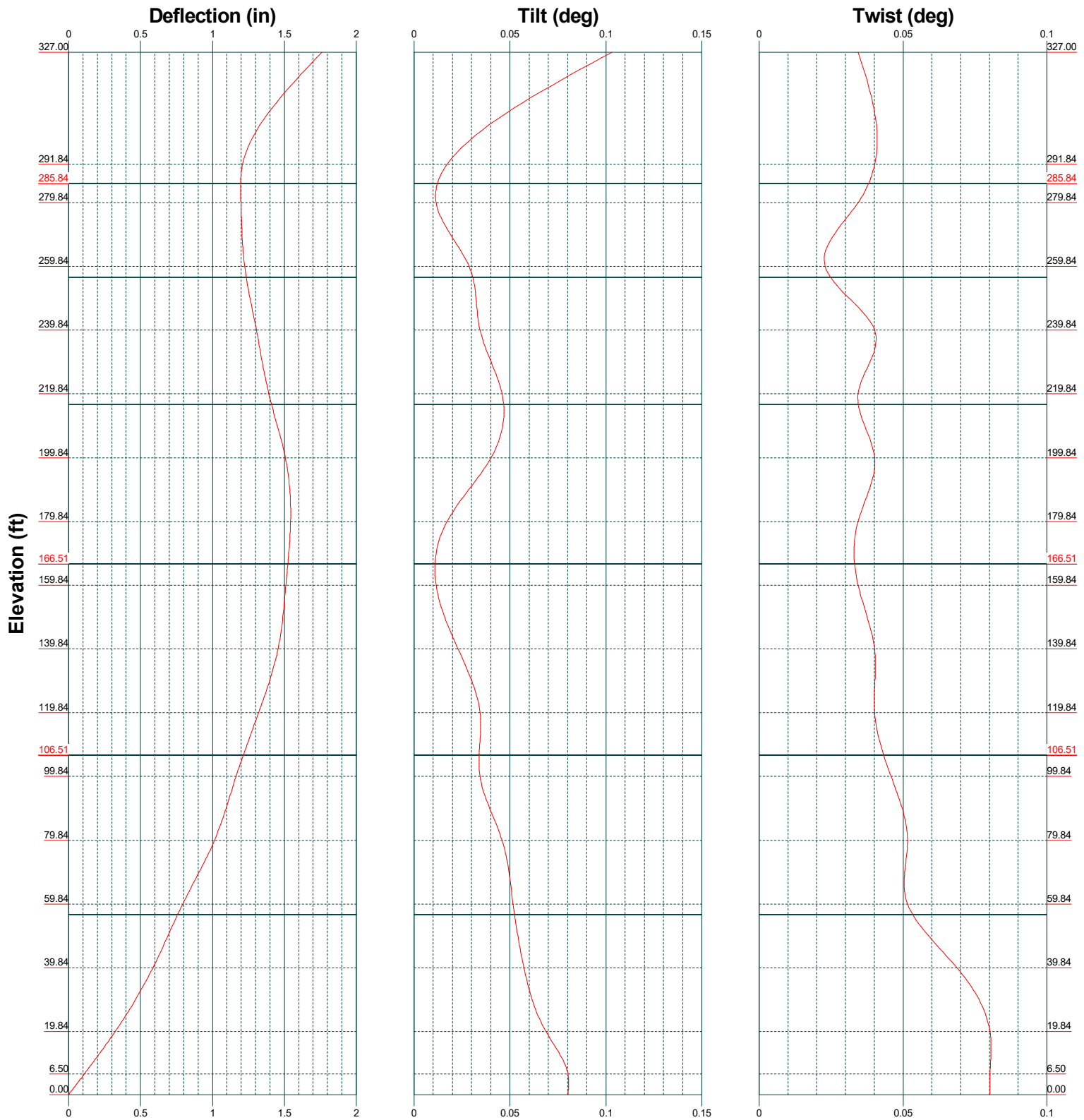
Code: **TIA-222-G**

Date: **09/11/17**

Scale: **NTS**

Path:

Dwg No. **E-4**



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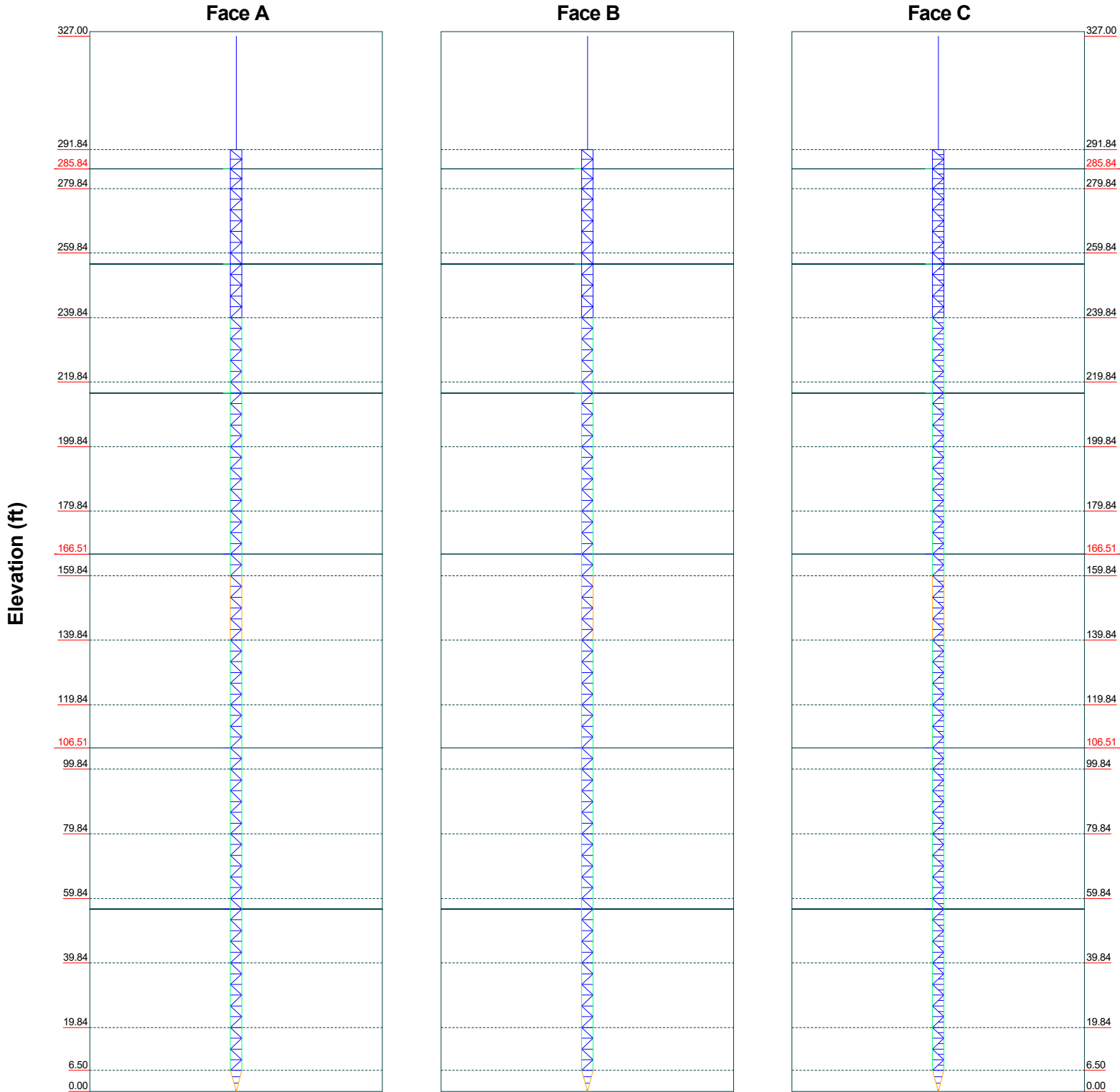
Job: **17004.42 - CT1077**  
Project: **327' Guyed Tower - N. Eagleville Road Storrs, CT**  
Client: **AT&T** Drawn by: **TJL** App'd:  
Code: **TIA-222-G** Date: **09/11/17** Scale: **NTS**  
Path: Dwg No. **E-5**



# Stress Distribution Chart

0' - 327'

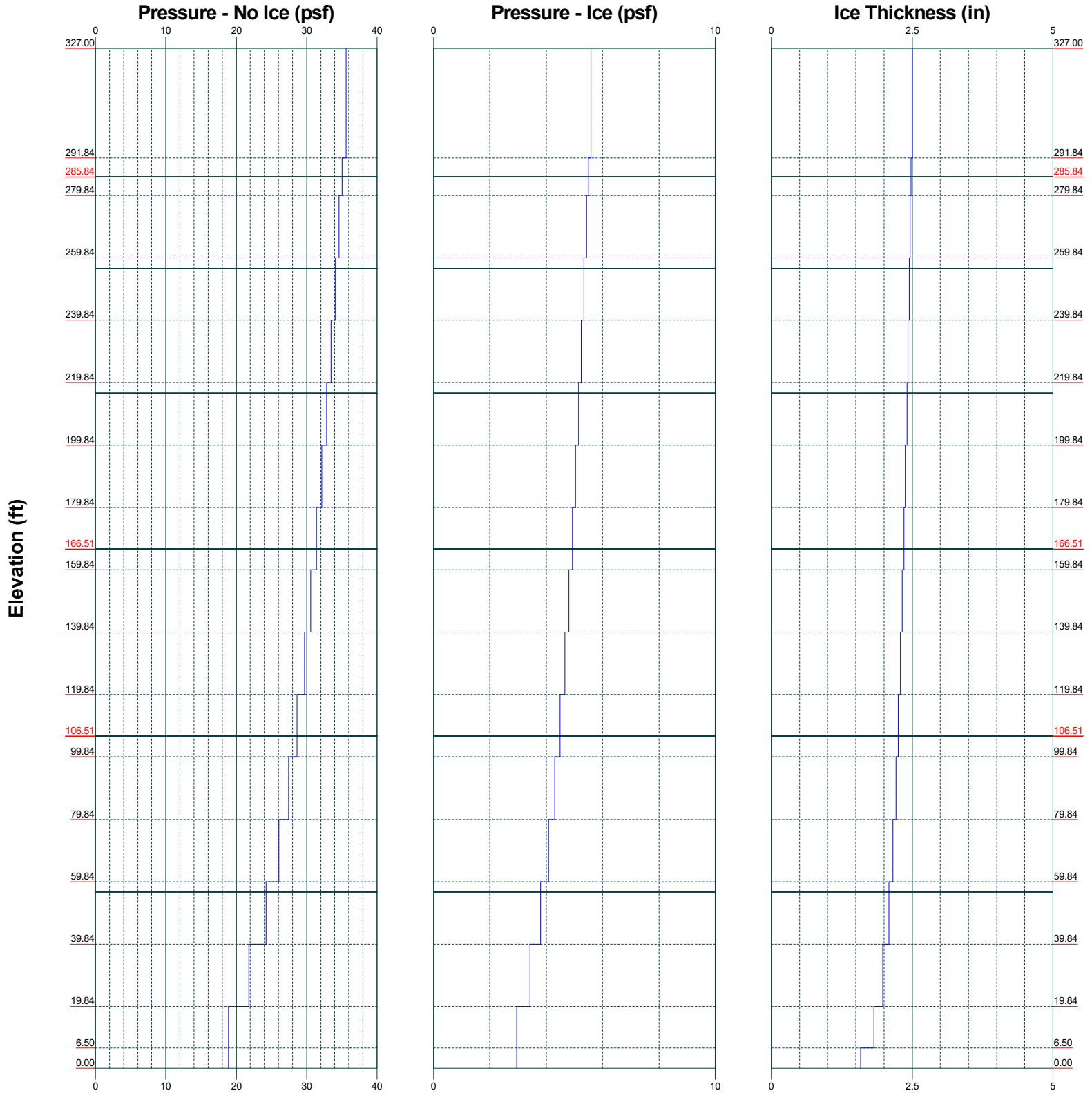
> 100% 90%-100% 75%-90% 50%-75% < 50% Overstress



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Branford, CT 06405		Client: AT&T	Drawn by: T.JL
Phone: (203) 488-0580		Code: TIA-222-G	Date: 09/11/17
FAX: (203) 488-8587		Path:	Scale: NTS
			Dwg No. E-8

# Wind Pressures and Ice Thickness

TIA-222-G - 101 mph/40 mph 1.0000 in Ice Exposure C



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	<b>Project</b>  327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>  15:25:46 09/11/17
	<b>Client</b>  AT&T	<b>Designed by</b>  TJL

## Tower Input Data

The main tower is a 3x guyed tower with an overall height of 327.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 3.67 ft at the top and tapered at the base.

An index plate is provided at the 3x guyed -tower connection.

There is a pole section.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Basic wind speed of 101 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 40 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Safety factor used in guy design is 1.

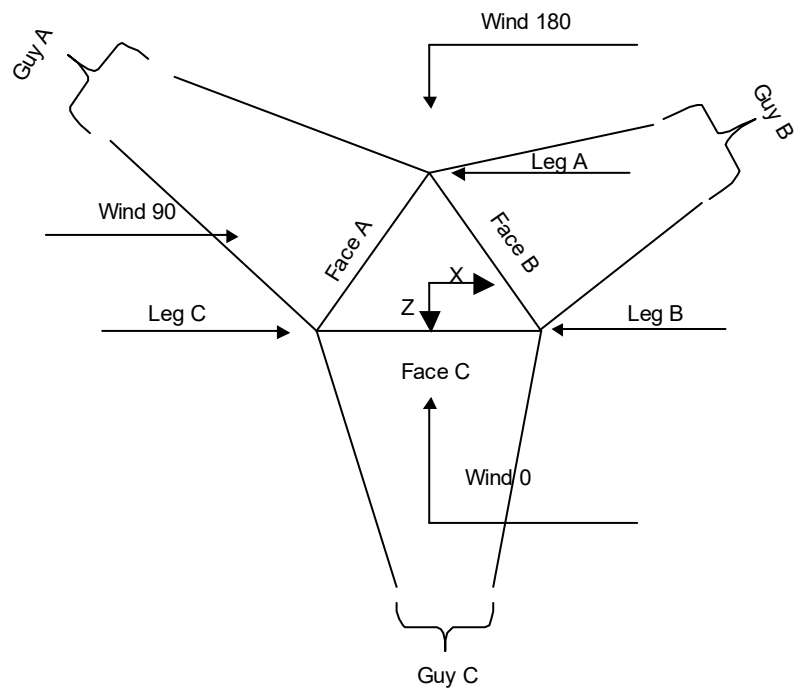
Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Horizontals	Assume Legs Pinned	Calculate Redundant Bracing Forces
Consider Moments - Diagonals	√ Assume Rigid Index Plate	Ignore Redundant Members in FEA
Use Moment Magnification	√ Use Clear Spans For Wind Area	√ SR Leg Bolts Resist Compression
√ Use Code Stress Ratios	√ Use Clear Spans For KL/r	√ All Leg Panels Have Same Allowable
√ Use Code Safety Factors - Guys	√ Retension Guys To Initial Tension	Offset Girt At Foundation
Escalate Ice	Bypass Mast Stability Checks	√ Consider Feed Line Torque
Always Use Max Kz	√ Use Azimuth Dish Coefficients	Include Angle Block Shear Check
Use Special Wind Profile	√ Project Wind Area of Appurt.	Use TIA-222-G Bracing Resist. Exemption
√ Include Bolts In Member Capacity	√ Autocalc Torque Arm Areas	Use TIA-222-G Tension Splice Exemption
√ Leg Bolts Are At Top Of Section	Add IBC .6D+W Combination	Poles
Secondary Horizontal Braces Leg	√ Sort Capacity Reports By Component	Include Shear-Torsion Interaction
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	Always Use Sub-Critical Flow
√ SR Members Have Cut Ends	Treat Feed Line Bundles As Cylinder	Use Top Mounted Sockets
SR Members Are Concentric		

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	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJJ



**Face Guyed**

## Pole Section Geometry

Section	Elevation ft	Section Length ft	Pole Size	Pole Grade	Socket Length ft
L1	327.00-291.84	35.16	P10.75x0.843	A53-B-35 (35 ksi)	

Tower Elevation ft	Gusset Area (per face) ft <sup>2</sup>	Gusset Thickness in	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
L1 327.00-291.84				1	1	1			

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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

### Tower Section Geometry

<i>Tower Section</i>	<i>Tower Elevation</i>	<i>Assembly Database</i>	<i>Description</i>	<i>Section Width</i>	<i>Number of Sections</i>	<i>Section Length</i>
	<i>ft</i>			<i>ft</i>		<i>ft</i>
T1	291.84-279.84			3.67	1	12.00
T2	279.84-259.84			3.67	1	20.00
T3	259.84-239.84			3.67	1	20.00
T4	239.84-219.84			3.67	1	20.00
T5	219.84-199.84			3.67	1	20.00
T6	199.84-179.84			3.67	1	20.00
T7	179.84-159.84			3.67	1	20.00
T8	159.84-139.84			3.67	1	20.00
T9	139.84-119.84			3.67	1	20.00
T10	119.84-99.84			3.67	1	20.00
T11	99.84-79.84			3.67	1	20.00
T12	79.84-59.84			3.67	1	20.00
T13	59.84-39.84			3.67	1	20.00
T14	39.84-19.84			3.67	1	20.00
T15	19.84-6.50			3.67	1	13.34
T16	6.50-0.00			3.67	1	6.50

### Tower Section Geometry (cont'd)

<i>Tower Section</i>	<i>Tower Elevation</i>	<i>Diagonal Spacing</i>	<i>Bracing Type</i>	<i>Has K Brace End Panels</i>	<i>Has Horizontals</i>	<i>Top Girt Offset</i>	<i>Bottom Girt Offset</i>
	<i>ft</i>	<i>ft</i>				<i>in</i>	<i>in</i>
T1	291.84-279.84	3.00	K Brace Left	No	Yes+Steps	0.0000	0.0000
T2	279.84-259.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T3	259.84-239.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T4	239.84-219.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T5	219.84-199.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T6	199.84-179.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T7	179.84-159.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T8	159.84-139.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T9	139.84-119.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T10	119.84-99.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T11	99.84-79.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T12	79.84-59.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T13	59.84-39.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T14	39.84-19.84	3.33	K Brace Left	No	Yes+Steps	0.0000	0.0000
T15	19.84-6.50	3.34	K Brace Left	No	Yes+Steps	0.0000	0.0000
T16	6.50-0.00	1.00	K Brace Left	No	Yes	0.0000	6.0000

### Tower Section Geometry (cont'd)

<i>Tower Elevation</i>	<i>Leg Type</i>	<i>Leg Size</i>	<i>Leg Grade</i>	<i>Diagonal Type</i>	<i>Diagonal Size</i>	<i>Diagonal Grade</i>
<i>ft</i>						
T1 291.84-279.84	Solid Round	2	A572-50 (50 ksi)	Solid Round	1 3/8	A36 (36 ksi)
T2 279.84-259.84	Solid Round	2	A572-50 (50 ksi)	Solid Round	1 3/8	A36 (36 ksi)

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	17004.42 - CT1077	4 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

<i>Tower Elevation ft</i>	<i>Leg Type</i>	<i>Leg Size</i>	<i>Leg Grade</i>	<i>Diagonal Type</i>	<i>Diagonal Size</i>	<i>Diagonal Grade</i>
T3 259.84-239.84	Solid Round	2 1/4	A572-50 (50 ksi)	Solid Round	1 3/8	A36 (36 ksi)
T4 239.84-219.84	Solid Round	2 1/4	A572-50 (50 ksi)	Solid Round	1 3/8	A36 (36 ksi)
T5 219.84-199.84	Solid Round	2 1/2	A572-50 (50 ksi)	Solid Round	1 1/2	A36 (36 ksi)
T6 199.84-179.84	Solid Round	2 1/2	A572-50 (50 ksi)	Solid Round	1 1/4	A36 (36 ksi)
T7 179.84-159.84	Solid Round	2 3/4	A572-50 (50 ksi)	Solid Round	1 1/2	A36 (36 ksi)
T8 159.84-139.84	Solid Round	2 1/2	A572-50 (50 ksi)	Solid Round	1 3/8	A36 (36 ksi)
T9 139.84-119.84	Solid Round	2 3/4	A572-50 (50 ksi)	Solid Round	1 1/4	A36 (36 ksi)
T10 119.84-99.84	Solid Round	2 3/4	A572-50 (50 ksi)	Solid Round	1 1/2	A36 (36 ksi)
T11 99.84-79.84	Solid Round	3	A572-50 (50 ksi)	Solid Round	1 3/8	A36 (36 ksi)
T12 79.84-59.84	Solid Round	3	A572-50 (50 ksi)	Solid Round	1 1/4	A36 (36 ksi)
T13 59.84-39.84	Solid Round	3	A572-50 (50 ksi)	Solid Round	1 1/4	A36 (36 ksi)
T14 39.84-19.84	Solid Round	3	A572-50 (50 ksi)	Solid Round	1 1/4	A36 (36 ksi)
T15 19.84-6.50	Solid Round	3	A572-50 (50 ksi)	Solid Round	1 1/4	A36 (36 ksi)
T16 6.50-0.00	Solid Round	3	A572-50 (50 ksi)	Solid Round		A36 (36 ksi)

### Tower Section Geometry (cont'd)

<i>Tower Elevation ft</i>	<i>Top Girt Type</i>	<i>Top Girt Size</i>	<i>Top Girt Grade</i>	<i>Bottom Girt Type</i>	<i>Bottom Girt Size</i>	<i>Bottom Girt Grade</i>
T1 291.84-279.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T2 279.84-259.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T3 259.84-239.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T4 239.84-219.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T5 219.84-199.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T6 199.84-179.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T7 179.84-159.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T8 159.84-139.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T9 139.84-119.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T10 119.84-99.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T11 99.84-79.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)

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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Elevation ft</i>	<i>Top Girt Type</i>	<i>Top Girt Size</i>	<i>Top Girt Grade</i>	<i>Bottom Girt Type</i>	<i>Bottom Girt Size</i>	<i>Bottom Girt Grade</i>
T12 79.84-59.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T13 59.84-39.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T14 39.84-19.84	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T15 19.84-6.50	Solid Round	1	A36 (36 ksi)	Solid Round	1	A36 (36 ksi)
T16 6.50-0.00	Flat Bar	12x3/8	A36 (36 ksi)	Flat Bar	12x3/8	A36 (36 ksi)

### Tower Section Geometry (cont'd)

<i>Tower Elevation ft</i>	<i>No. of Mid Girts</i>	<i>Mid Girt Type</i>	<i>Mid Girt Size</i>	<i>Mid Girt Grade</i>	<i>Horizontal Type</i>	<i>Horizontal Size</i>	<i>Horizontal Grade</i>
T1 291.84-279.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T2 279.84-259.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T3 259.84-239.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T4 239.84-219.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T5 219.84-199.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T6 199.84-179.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T7 179.84-159.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T8 159.84-139.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T9 139.84-119.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T10 119.84-99.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T11 99.84-79.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T12 79.84-59.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T13 59.84-39.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T14 39.84-19.84	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T15 19.84-6.50	None	Solid Round		A572-50 (50 ksi)	Solid Round	1	A36 (36 ksi)
T16 6.50-0.00	2	Flat Bar	9x3/8	A36 (36 ksi)	Flat Bar		A36 (36 ksi)

### Tower Section Geometry (cont'd)

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	6 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Elevation</i>	<i>Secondary Horizontal Type</i>	<i>Secondary Horizontal Size</i>	<i>Secondary Horizontal Grade</i>	<i>Inner Bracing Type</i>	<i>Inner Bracing Size</i>	<i>Inner Bracing Grade</i>
<i>ft</i>						
T1 291.84-279.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T2 279.84-259.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T3 259.84-239.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T4 239.84-219.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T5 219.84-199.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T6 199.84-179.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T7 179.84-159.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T8 159.84-139.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T9 139.84-119.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T10 119.84-99.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T11 99.84-79.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T12 79.84-59.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T13 59.84-39.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T14 39.84-19.84	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T15 19.84-6.50	Solid Round	1	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

<i>Tower Elevation</i>	<i>Gusset Area (per face)</i>	<i>Gusset Thickness</i>	<i>Gusset Grade</i>	<i>Adjust. Factor A<sub>f</sub></i>	<i>Adjust. Factor A<sub>r</sub></i>	<i>Weight Mult.</i>	<i>Double Angle Stitch Bolt Spacing Diagonals in</i>	<i>Double Angle Stitch Bolt Spacing Horizontals in</i>	<i>Double Angle Stitch Bolt Spacing Redundants in</i>
<i>ft</i>	<i>ft<sup>2</sup></i>	<i>in</i>							
T1 291.84-279.84	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T2 279.84-259.84	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T3 259.84-239.84	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T4 239.84-219.84	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T5 219.84-199.84	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T6 199.84-179.84	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T7 179.84-159.84	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T8 159.84-139.84	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T9	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000



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	17004.42 - CT1077	7 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
ft	ft <sup>2</sup>	in							
139.84-119.84			(36 ksi)						
T10	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
119.84-99.84			(36 ksi)						
T11	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
99.84-79.84			(36 ksi)						
T12	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
79.84-59.84			(36 ksi)						
T13	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
59.84-39.84			(36 ksi)						
T14	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
39.84-19.84			(36 ksi)						
T15 19.84-6.50	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
			(36 ksi)						
T16 6.50-0.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
			(36 ksi)						

## Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	K Factors <sup>1</sup>							
			Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
				X Y	X Y	X Y	X Y	X Y	X Y	X Y
ft										
T1	Yes	Yes	1	1	1	1	1	1	1	1
291.84-279.84				1	1	1	1	1	1	1
T2	Yes	Yes	1	1	1	1	1	1	1	1
279.84-259.84				1	1	1	1	1	1	1
T3	Yes	Yes	1	1	1	1	1	1	1	1
259.84-239.84				1	1	1	1	1	1	1
T4	Yes	Yes	1	1	1	1	1	1	1	1
239.84-219.84				1	1	1	1	1	1	1
T5	Yes	Yes	1	1	1	1	1	1	1	1
219.84-199.84				1	1	1	1	1	1	1
T6	Yes	Yes	1	1	1	1	1	1	1	1
199.84-179.84				1	1	1	1	1	1	1
T7	Yes	Yes	1	1	1	1	1	1	1	1
179.84-159.84				1	1	1	1	1	1	1
T8	Yes	Yes	1	1	1	1	1	1	1	1
159.84-139.84				1	1	1	1	1	1	1
T9	Yes	Yes	1	1	1	1	1	1	1	1
139.84-119.84				1	1	1	1	1	1	1
T10	Yes	Yes	1	1	1	1	1	1	1	1
119.84-99.84				1	1	1	1	1	1	1
T11	Yes	Yes	1	1	1	1	1	1	1	1
99.84-79.84				1	1	1	1	1	1	1
T12	Yes	Yes	1	1	1	1	1	1	1	1
79.84-59.84				1	1	1	1	1	1	1
T13	Yes	Yes	1	1	1	1	1	1	1	1
59.84-39.84				1	1	1	1	1	1	1
T14	Yes	Yes	1	1	1	1	1	1	1	1
39.84-19.84				1	1	1	1	1	1	1
T15	Yes	Yes	1	1	1	1	1	1	1	1
19.84-6.50				1	1	1	1	1	1	1
T16 6.50-0.00	Yes	Yes	1	1	1	1	1	1	1	1

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	8 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	K Factors <sup>1</sup>							
			Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
ft				X Y	X Y	X Y	X Y	X Y	X Y	X Y
				1	1	1	1	1	1	1

<sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
291.84-279.84														
T2	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
279.84-259.84														
T3	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
259.84-239.84														
T4	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
239.84-219.84														
T5	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
219.84-199.84														
T6	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
199.84-179.84														
T7	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
179.84-159.84														
T8	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
159.84-139.84														
T9	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
139.84-119.84														
T10	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
119.84-99.84														
T11	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
99.84-79.84														
T12	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
79.84-59.84														
T13	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
59.84-39.84														
T14	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
39.84-19.84														
T15 19.84-6.50	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T16 6.50-0.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

### Tower Section Geometry (cont'd)

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	9 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
291.84-279.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T2	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
279.84-259.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T3	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
259.84-239.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T4	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
239.84-219.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T5	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
219.84-199.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T6	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
199.84-179.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T7	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
179.84-159.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T8	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
159.84-139.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T9	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
139.84-119.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T10	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
119.84-99.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T11	Flange	1.0000	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
99.84-79.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T12	Flange	1.3750	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
79.84-59.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T13	Flange	1.3750	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
59.84-39.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T14	Flange	1.3750	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
39.84-19.84		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T15 19.84-6.50	Flange	1.3750	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
		A325N		A325N		A325N		A325N		A325X		A325N		A325X	
T16 6.50-0.00	Flange	1.3750	4	0.5000	0	0.5000	0	0.5000	0	0.6250	0	0.5000	0	0.6250	0
		A325N		A325N		A325N		A325N		A325X		A325N		A325X	

## Guy Data

Guy Elevation ft	Guy Grade	Guy Size	Initial Tension K	%	Guy Modulus ksi	Guy Weight plf	L <sub>u</sub> ft	Anchor Radius ft	Anchor Azimuth Adj. °	Anchor Elevation ft	End Fitting Efficiency %
285.84	EHS	A 3/4	5.83	10%	19000	1.155	386.70	235.00	0.0000	-23.40	100%
		B 3/4	5.83	10%	19000	1.155	361.43	235.00	0.0000	8.90	100%
		C 3/4	5.83	10%	19000	1.155	384.07	235.00	0.0000	-20.10	100%
256.507	EHS	A 3/4	5.83	10%	19000	1.155	363.71	235.00	0.0000	-23.40	100%
		B 3/4	5.83	10%	19000	1.155	339.52	235.00	0.0000	8.90	100%
		C 3/4	5.83	10%	19000	1.155	361.18	235.00	0.0000	-20.10	100%
216.507	EHS	A 3/4	5.83	10%	19000	1.155	333.96	235.00	0.0000	-23.40	100%
		B 3/4	5.83	10%	19000	1.155	311.60	235.00	0.0000	8.90	100%
		C 3/4	5.83	10%	19000	1.155	331.60	235.00	0.0000	-20.10	100%
166.507	EHS	A 3/4	5.83	10%	19000	1.155	300.12	235.00	0.0000	-23.40	100%
		B 3/4	5.83	10%	19000	1.155	280.83	235.00	0.0000	8.90	100%
		C 3/4	5.83	10%	19000	1.155	298.05	235.00	0.0000	-20.10	100%
106.507	EHS	A 3/4	5.83	10%	19000	1.155	266.30	235.00	0.0000	-23.40	100%
		B 3/4	5.83	10%	19000	1.155	252.15	235.00	0.0000	8.90	100%
		C 3/4	5.83	10%	19000	1.155	264.71	235.00	0.0000	-20.10	100%

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	10 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

56.5067	EHS	A	3/4	5.83	10%	19000	1.155	245.86	235.00	0.0000	-23.40	100%
		B	3/4	5.83	10%	19000	1.155	237.35	235.00	0.0000	8.90	100%
		C	3/4	5.83	10%	19000	1.155	244.81	235.00	0.0000	-20.10	100%

### Guy Data(cont'd)

Guy Elevation ft	Mount Type	Torque-Arm Spread ft	Torque-Arm Leg Angle °	Torque-Arm Style	Torque-Arm Grade	Torque-Arm Type	Torque-Arm Size
285.84	Torque Arm	8.00	0.0000	Channel	A36 (36 ksi)	Channel	MC12x35
256.507	Torque Arm	8.00	0.0000	Channel	A36 (36 ksi)	Channel	MC12x35
216.507	Torque Arm	8.00	0.0000	Channel	A36 (36 ksi)	Channel	MC12x35
166.507	Torque Arm	8.00	0.0000	Channel	A36 (36 ksi)	Channel	MC12x35
106.507	Torque Arm	8.00	0.0000	Channel	A36 (36 ksi)	Channel	MC12x35
56.5067	Torque Arm	8.00	0.0000	Channel	A36 (36 ksi)	Channel	MC12x35

### Guy Data (cont'd)

Guy Elevation ft	Diagonal Grade	Diagonal Type	Upper Diagonal Size	Lower Diagonal Size	Is Strap.	Pull-Off Grade	Pull-Off Type	Pull-Off Size
285.84	A572-50 (50 ksi)	Solid Round			No	A36 (36 ksi)	Channel	MC12x35
256.51	A572-50 (50 ksi)	Solid Round			No	A36 (36 ksi)	Channel	MC12x35
216.51	A572-50 (50 ksi)	Solid Round			No	A36 (36 ksi)	Channel	MC12x35
166.51	A572-50 (50 ksi)	Solid Round			No	A36 (36 ksi)	Channel	MC12x35
106.51	A572-50 (50 ksi)	Solid Round			No	A36 (36 ksi)	Channel	MC12x35
56.51	A572-50 (50 ksi)	Solid Round			No	A36 (36 ksi)	Channel	MC12x35

### Guy Data (cont'd)

Guy Elevation ft	Cable Weight A K	Cable Weight B K	Cable Weight C K	Cable Weight D K	Tower Intercept A ft	Tower Intercept B ft	Tower Intercept C ft	Tower Intercept D ft
285.84	0.45	0.42	0.44		14.39	12.61	14.20	
256.507	0.42	0.39	0.42		6.5 sec/pulse 12.76	6.1 sec/pulse 11.16	6.5 sec/pulse 12.59	

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	11 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Guy Elevation ft	Cable Weight A K	Cable Weight B K	Cable Weight C K	Cable Weight D K	Tower Intercept A ft	Tower Intercept B ft	Tower Intercept C ft	Tower Intercept D ft
216.507	0.39	0.36	0.38		6.2 sec/pulse 10.80	5.8 sec/pulse 9.43	6.1 sec/pulse 10.65	
166.507	0.35	0.32	0.34		5.7 sec/pulse 8.77	5.3 sec/pulse 7.70	5.6 sec/pulse 8.65	
106.507	0.31	0.29	0.31		5.1 sec/pulse 6.94	4.8 sec/pulse 6.24	5.1 sec/pulse 6.86	
56.5067	0.28	0.27	0.28		4.5 sec/pulse 5.95	4.3 sec/pulse 5.56	4.5 sec/pulse 5.90	
					4.2 sec/pulse	4.1 sec/pulse	4.2 sec/pulse	

### Guy Data (cont'd)

Guy Elevation ft	Calc K Single Angles	Calc K Solid Rounds	Torque Arm		Pull Off		Diagonal	
			K <sub>x</sub>	K <sub>y</sub>	K <sub>x</sub>	K <sub>y</sub>	K <sub>x</sub>	K <sub>y</sub>
285.84	No	No	1	1	1	1	1	1
256.507	No	No	1	1	1	1	1	1
216.507	No	No	1	1	1	1	1	1
166.507	No	No	1	1	1	1	1	1
106.507	No	No	1	1	1	1	1	1
56.5067	No	No	1	1	1	1	1	1

### Guy Data (cont'd)

Guy Elevation ft	Torque-Arm				Pull Off				Diagonal			
	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U
285.84	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75
256.507	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75
216.507	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75
166.507	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75
106.507	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75
56.5067	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75

### Guy Pressures

Guy Elevation ft	Guy Location	z ft	q <sub>z</sub> psf	q <sub>z</sub> Ice psf	Ice Thickness in
285.84	A	131.22	30	5	2.2960

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	12 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Guy Elevation ft	Guy Location	z ft	qz psf	qz Ice psf	Ice Thickness in
256.507	B	147.37	30	5	2.3228
	C	132.87	30	5	2.2989
	A	116.55	29	5	2.2690
	B	132.70	30	5	2.2986
216.507	C	118.20	29	5	2.2722
	A	96.55	28	4	2.2267
	B	112.70	29	5	2.2614
	C	98.20	28	4	2.2304
166.507	A	71.55	26	4	2.1609
	B	87.70	27	4	2.2054
	C	73.20	26	4	2.1659
	A	41.55	23	4	2.0466
106.507	B	57.70	25	4	2.1149
	C	43.20	24	4	2.0546
	A	16.55	19	3	1.8667
	B	32.70	22	3	1.9982
56.5067	C	18.20	20	3	1.8845

## Guy-Tensioning Information

Temperature At Time Of Tensioning																
Guy Elevation ft	H ft	V ft	0 F		20 F		40 F		60 F		80 F		100 F		120 F	
			Initial Tension K	Intercept ft	Initial Tension K	Intercept ft	Initial Tension K	Intercept ft	Initial Tension K	Intercept ft	Initial Tension K	Intercept ft	Initial Tension K	Intercept ft	Initial Tension K	Intercept ft
285.84	A	232.72	309.24	6.615	12.72	6.349	13.24	6.087	13.80	5.830	14.39	5.578	15.02	5.331	15.70	5.090
	B	232.72	276.94	6.731	10.96	6.425	11.47	6.125	12.02	5.830	12.61	5.542	13.25	5.261	13.94	4.989
	C	232.72	305.94	6.626	12.53	6.357	13.05	6.091	13.60	5.830	14.20	5.574	14.83	5.324	15.51	5.080
256.507	A	232.72	279.91	6.719	11.11	6.418	11.62	6.121	12.17	5.830	12.76	5.545	13.40	5.268	14.09	4.999
	B	232.72	247.61	6.853	9.52	6.506	10.02	6.164	10.57	5.830	11.16	5.504	11.80	5.188	12.50	4.884
	C	232.72	276.61	6.732	10.94	6.426	11.45	6.125	12.00	5.830	12.59	5.541	13.23	5.260	13.92	4.988
216.507	A	232.72	239.91	6.889	9.17	6.529	9.67	6.175	10.21	5.830	10.80	5.494	11.45	5.168	12.16	4.854
	B	232.72	207.61	7.051	7.83	6.635	8.31	6.227	8.84	5.830	9.43	5.445	10.09	5.074	10.81	4.720
	C	232.72	236.61	6.904	9.02	6.539	9.52	6.180	10.06	5.830	10.65	5.489	11.30	5.159	12.01	4.841
166.507	A	232.72	189.91	7.149	7.17	6.699	7.65	6.259	8.18	5.830	8.77	5.415	9.43	5.018	10.16	4.640
	B	232.72	157.61	7.338	6.13	6.823	6.59	6.319	7.11	5.830	7.70	5.360	8.37	4.912	9.12	4.491
	C	232.72	186.61	7.165	7.06	6.709	7.53	6.264	8.06	5.830	8.65	5.403	9.32	5.000	10.06	4.619
106.507	A	232.72	129.91	7.512	5.40	6.936	5.85	6.374	6.36	5.830	6.94	5.309	7.62	4.816	8.39	4.358
	B	232.72	97.61	7.713	4.73	7.067	5.16	6.437	5.66	5.830	6.24	5.252	6.93	4.710	7.72	4.213
	C	232.72	126.61	7.533	5.32	6.950	5.77	6.381	6.28	5.830	6.86	5.303	7.54	4.805	8.31	4.343
56.5067	A	232.72	79.91	7.813	4.44	7.132	4.87	6.469	5.36	5.830	5.95	5.223	6.63	4.657	7.43	4.142
	B	232.72	47.61	7.963	4.07	7.229	4.49	6.516	4.98	5.830	5.56	5.181	6.25	4.581	7.07	4.041
	C	232.72	76.61	7.831	4.40	7.144	4.82	6.474	5.31	5.830	5.90	5.218	6.59	4.648	7.39	4.130

## Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
LDF6-50A (1-1/4 FOAM) (Verizon)	C	No	Ar (CaAa)	84.00 - 5.00	0.0000	-0.45	3	1	1.5500	1.5500		0.66
LDF6-50A (1-1/4 FOAM) (Verizon)	C	No	Ar (CaAa)	84.00 - 5.00	0.0000	-0.14	9	9	1.5500	1.5500		0.66
HYBRIFLEX 1-5/8" (Verizon)	C	No	Ar (CaAa)	84.00 - 5.00	0.0000	0.43	1	1	1.9800	1.9800		1.90

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	13 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
LDF7-50A (1-5/8 FOAM) (AT&T - Existing)	C	No	Ar (CaAa)	185.00 - 5.00	-0.5000	0	11	11	1.9800	1.9800		0.82
LDF7-50A (1-5/8 FOAM) (AT&T - Existing)	C	No	Ar (CaAa)	185.00 - 5.00	-3.0000	-0.15	1	1	1.9800	1.9800		0.82
Fiber Trunk (AT&T - Existing)	C	No	Ar (CaAa)	185.00 - 5.00	-3.0000	0	3	3	0.4000	0.4000		1.00
DC Trunk (AT&T - Existing)	C	No	Ar (CaAa)	185.00 - 5.00	-3.0000	0.15	6	6	0.4000	0.4000		0.11
LDF4-50A (1/2 FOAM)	B	No	Ar (CaAa)	13.00 - 5.00	0.0000	0.32	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM)	B	No	Ar (CaAa)	18.00 - 5.00	0.0000	0.34	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM) (UCONN-44)	B	No	Ar (CaAa)	94.00 - 5.00	0.0000	0.36	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM) (WHUS-35)	B	No	Ar (CaAa)	94.00 - 5.00	0.0000	0.38	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM) (Beacon)	B	No	Ar (CaAa)	157.00 - 5.00	0.0000	0.4	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM)	B	No	Ar (CaAa)	159.00 - 5.00	0.0000	0.42	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM) (WHUS-39)	B	No	Ar (CaAa)	277.00 - 5.00	0.0000	0.44	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM) (WHUS-40)	B	No	Ar (CaAa)	277.00 - 5.00	0.0000	0.46	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM)	B	No	Ar (CaAa)	291.80 - 5.00	0.0000	0.48	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM) (CPR-33)	B	No	Ar (CaAa)	190.00 - 5.00	0.0000	0.28	1	1	0.6300	0.6300		0.15
LDF4-50A (1/2 FOAM) (CPR-32)	B	No	Ar (CaAa)	198.00 - 5.00	0.0000	0.3	1	1	0.6300	0.6300		0.15
LDF5-50A (7/8 FOAM) (CSP-10)	A	No	Ar (CaAa)	70.00 - 5.00	0.0000	-0.145	1	1	1.0900	1.0900		0.33
EW63 (CSP-3)	A	No	Ar (CaAa)	104.00 - 5.00	0.0000	-0.105	1	1	1.5742	1.5742		0.51
LDF5-50A (7/8 FOAM)	A	No	Ar (CaAa)	112.00 - 5.00	0.0000	-0.07	1	1	1.0900	1.0900		0.33
EW63 (CSP-1)	A	No	Ar (CaAa)	116.00 - 5.00	0.0000	-0.035	1	1	1.5742	1.5742		0.51
EW63 (CSP-2)	A	No	Ar (CaAa)	116.00 - 5.00	0.0000	0.01	1	1	1.5742	1.5742		0.51
LDF4-50A (1/2 FOAM) (CSP-8)	A	No	Ar (CaAa)	171.50 - 5.00	0.0000	0.04	1	1	0.6300	0.6300		0.15
LDF5-50A (7/8 FOAM) (UCONN-42)	A	No	Ar (CaAa)	172.00 - 5.00	0.0000	0.06	1	1	1.0900	1.0900		0.33
LDF5-50A	A	No	Ar (CaAa)	172.50 - 5.00	0.0000	0.09	1	1	1.0900	1.0900		0.33

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	14 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
(7/8 FOAM) LDF5-50A	A	No	Ar (CaAa)	211.00 - 5.00	0.0000	0.12	1	1	1.0900	1.0900		0.33
(7/8 FOAM) (WHUS-36) LDF5-50A	A	No	Ar (CaAa)	211.00 - 5.00	0.0000	0.15	1	1	1.0900	1.0900		0.33
(7/8 FOAM) LDF7-50A	A	No	Ar (CaAa)	250.00 - 5.00	0.0000	0.19	1	1	1.9800	1.9800		0.82
(1-5/8 FOAM) (CSP-7) LDF7-50A	A	No	Ar (CaAa)	250.00 - 5.00	0.0000	0.24	1	1	1.9800	1.9800		0.82
(1-5/8 FOAM) (CSP-11) LDF7-50A	A	No	Ar (CaAa)	252.00 - 5.00	0.0000	0.29	1	1	1.9800	1.9800		0.82
(1-5/8 FOAM) (CSP-4) LDF7-50A	A	No	Ar (CaAa)	261.00 - 5.00	0.0000	0.34	1	1	1.9800	1.9800		0.82
(1-5/8 FOAM) (CSP-6) LDF7-50A	A	No	Ar (CaAa)	267.00 - 5.00	0.0000	0.39	1	1	1.9800	1.9800		0.82
(1-5/8 FOAM) (CSP-5) LDF7-50A	A	No	Ar (CaAa)	267.00 - 5.00	0.0000	0.44	1	1	1.9800	1.9800		0.82
(1-5/8 FOAM) (CSP-9) LDF5-50A	A	No	Ar (CaAa)	291.84 - 5.00	0.0000	0.48	1	1	1.0900	1.0900		0.33
(7/8 FOAM) LDF7-50A	A	No	Ar (CaAa)	240.00 - 5.00	0.0000	-0.37	5	5	1.9800	1.9800		0.82
(1-5/8 FOAM) (CSP-20,21,22 ,24,25) LDF7-50A	A	No	Ar (CaAa)	250.00 - 5.00	2.5000	-0.37	6	6	1.9800	1.9800		0.82
(1-5/8 FOAM) (CSP-12,13,14 ,16,17,18) LDF4-50A	A	No	Ar (CaAa)	240.00 - 5.00	0.0000	-0.17	2	1	0.6300	0.6300		0.15
(1/2 FOAM) (CSP-23,26) LDF4-50A	A	No	Ar (CaAa)	250.00 - 5.00	0.0000	-0.19	2	1	0.6300	0.6300		0.15
(1/2 FOAM) (CSP-15,19)												

## Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
L1	327.00-291.84	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00
T1	291.84-279.84	A	0.000	0.000	1.308	0.000	0.00
		B	0.000	0.000	0.753	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00
T2	279.84-259.84	A	0.000	0.000	5.245	0.000	0.02
		B	0.000	0.000	3.422	0.000	0.01
		C	0.000	0.000	0.000	0.000	0.00
T3	259.84-239.84	A	0.000	0.000	34.020	0.000	0.14
		B	0.000	0.000	3.780	0.000	0.01



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	17004.42 - CT1077	15 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

<i>Tower Section</i>	<i>Tower Elevation ft</i>	<i>Face</i>	<i>A<sub>R</sub></i>	<i>A<sub>F</sub></i>	<i>C<sub>A</sub>A<sub>A</sub> In Face</i>	<i>C<sub>A</sub>A<sub>A</sub> Out Face</i>	<i>Weight</i>
			<i>ft<sup>2</sup></i>	<i>ft<sup>2</sup></i>	<i>ft<sup>2</sup></i>	<i>ft<sup>2</sup></i>	<i>K</i>
T4	239.84-219.84	C	0.000	0.000	0.000	0.000	0.00
		A	0.000	0.000	74.540	0.000	0.30
		B	0.000	0.000	3.780	0.000	0.01
T5	219.84-199.84	C	0.000	0.000	0.000	0.000	0.00
		A	0.000	0.000	76.973	0.000	0.30
		B	0.000	0.000	3.780	0.000	0.01
T6	199.84-179.84	C	0.000	0.000	0.000	0.000	0.00
		A	0.000	0.000	78.900	0.000	0.31
		B	0.000	0.000	5.564	0.000	0.01
T7	179.84-159.84	C	0.000	0.000	14.118	0.000	0.07
		A	0.000	0.000	82.340	0.000	0.32
		B	0.000	0.000	6.300	0.000	0.01
T8	159.84-139.84	C	0.000	0.000	54.720	0.000	0.27
		A	0.000	0.000	84.520	0.000	0.33
		B	0.000	0.000	8.588	0.000	0.02
T9	139.84-119.84	C	0.000	0.000	54.720	0.000	0.27
		A	0.000	0.000	84.520	0.000	0.33
		B	0.000	0.000	8.820	0.000	0.02
T10	119.84-99.84	C	0.000	0.000	54.720	0.000	0.27
		A	0.000	0.000	91.588	0.000	0.35
		B	0.000	0.000	8.820	0.000	0.02
T11	99.84-79.84	C	0.000	0.000	54.720	0.000	0.27
		A	0.000	0.000	96.145	0.000	0.36
		B	0.000	0.000	10.604	0.000	0.03
T12	79.84-59.84	C	0.000	0.000	63.281	0.000	0.31
		A	0.000	0.000	97.253	0.000	0.37
		B	0.000	0.000	11.340	0.000	0.03
T13	59.84-39.84	C	0.000	0.000	95.880	0.000	0.47
		A	0.000	0.000	98.325	0.000	0.37
		B	0.000	0.000	11.340	0.000	0.03
T14	39.84-19.84	C	0.000	0.000	95.880	0.000	0.47
		A	0.000	0.000	98.325	0.000	0.37
		B	0.000	0.000	11.340	0.000	0.03
T15	19.84-6.50	C	0.000	0.000	95.880	0.000	0.47
		A	0.000	0.000	65.583	0.000	0.25
		B	0.000	0.000	8.698	0.000	0.02
T16	6.50-0.00	C	0.000	0.000	63.952	0.000	0.31
		A	0.000	0.000	7.374	0.000	0.03
		B	0.000	0.000	1.040	0.000	0.00
		C	0.000	0.000	7.191	0.000	0.03

## Feed Line/Linear Appurtenances Section Areas - With Ice

<i>Tower Section</i>	<i>Tower Elevation ft</i>	<i>Face or Leg</i>	<i>Ice Thickness in</i>	<i>A<sub>R</sub></i>	<i>A<sub>F</sub></i>	<i>C<sub>A</sub>A<sub>A</sub> In Face</i>	<i>C<sub>A</sub>A<sub>A</sub> Out Face</i>	<i>Weight</i>
				<i>ft<sup>2</sup></i>	<i>ft<sup>2</sup></i>	<i>ft<sup>2</sup></i>	<i>ft<sup>2</sup></i>	<i>K</i>
L1	327.00-291.84	A	2.502	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.00
T1	291.84-279.84	A	2.482	0.000	0.000	7.265	0.000	0.13
		B		0.000	0.000	6.690	0.000	0.11
		C		0.000	0.000	0.000	0.000	0.00
T2	279.84-259.84	A	2.468	0.000	0.000	22.756	0.000	0.44
		B		0.000	0.000	30.231	0.000	0.52
		C		0.000	0.000	0.000	0.000	0.00
T3	259.84-239.84	A	2.449	0.000	0.000	122.333	0.000	2.38
		B		0.000	0.000	33.165	0.000	0.56
		C		0.000	0.000	0.000	0.000	0.00

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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Section</i>	<i>Tower Elevation ft</i>	<i>Face or Leg</i>	<i>Ice Thickness in</i>	<i>A<sub>R</sub> ft<sup>2</sup></i>	<i>A<sub>F</sub> ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub> In Face ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub> Out Face ft<sup>2</sup></i>	<i>Weight K</i>
T4	239.84-219.84	A	2.428	0.000	0.000	265.380	0.000	5.00
		B		0.000	0.000	32.921	0.000	0.55
		C		0.000	0.000	0.000	0.000	0.00
T5	219.84-199.84	A	2.406	0.000	0.000	277.358	0.000	5.18
		B		0.000	0.000	32.657	0.000	0.54
		C		0.000	0.000	0.000	0.000	0.00
T6	199.84-179.84	A	2.382	0.000	0.000	286.298	0.000	5.30
		B		0.000	0.000	47.647	0.000	0.79
		C		0.000	0.000	45.963	0.000	0.81
T7	179.84-159.84	A	2.356	0.000	0.000	305.283	0.000	5.59
		B		0.000	0.000	53.421	0.000	0.87
		C		0.000	0.000	177.531	0.000	3.12
T8	159.84-139.84	A	2.327	0.000	0.000	316.365	0.000	5.72
		B		0.000	0.000	72.023	0.000	1.17
		C		0.000	0.000	176.842	0.000	3.09
T9	139.84-119.84	A	2.294	0.000	0.000	313.905	0.000	5.62
		B		0.000	0.000	73.041	0.000	1.17
		C		0.000	0.000	176.065	0.000	3.04
T10	119.84-99.84	A	2.256	0.000	0.000	340.088	0.000	6.01
		B		0.000	0.000	71.976	0.000	1.13
		C		0.000	0.000	175.173	0.000	3.00
T11	99.84-79.84	A	2.211	0.000	0.000	354.739	0.000	6.18
		B		0.000	0.000	85.024	0.000	1.32
		C		0.000	0.000	200.134	0.000	3.41
T12	79.84-59.84	A	2.156	0.000	0.000	355.264	0.000	6.07
		B		0.000	0.000	88.945	0.000	1.35
		C		0.000	0.000	296.984	0.000	5.08
T13	59.84-39.84	A	2.084	0.000	0.000	353.839	0.000	5.90
		B		0.000	0.000	86.371	0.000	1.27
		C		0.000	0.000	294.129	0.000	4.91
T14	39.84-19.84	A	1.980	0.000	0.000	344.021	0.000	5.52
		B		0.000	0.000	82.619	0.000	1.16
		C		0.000	0.000	289.977	0.000	4.67
T15	19.84-6.50	A	1.824	0.000	0.000	219.699	0.000	3.32
		B		0.000	0.000	58.987	0.000	0.78
		C		0.000	0.000	189.293	0.000	2.88
T16	6.50-0.00	A	1.586	0.000	0.000	23.024	0.000	0.32
		B		0.000	0.000	6.274	0.000	0.07
		C		0.000	0.000	20.578	0.000	0.29

### Feed Line Center of Pressure

<i>Section</i>	<i>Elevation</i>	<i>CP<sub>X</sub></i>	<i>CP<sub>Z</sub></i>	<i>CP<sub>X</sub></i>	<i>CP<sub>Z</sub></i>
	<i>ft</i>	<i>in</i>	<i>in</i>	<i>Ice in</i>	<i>Ice in</i>
L1	327.00-291.84	0.0000	0.0000	0.0000	0.0000
T1	291.84-279.84	0.2484	-0.3860	0.3843	-0.2816
T2	279.84-259.84	0.6627	-0.9031	1.0787	-0.3670
T3	259.84-239.84	-1.2710	-1.2997	-0.2799	-1.0111
T4	239.84-219.84	-2.7607	-0.4619	-1.4362	-0.6024
T5	219.84-199.84	-2.6012	-0.4969	-1.4024	-0.6502
T6	199.84-179.84	-2.2747	-0.1400	-1.1432	-0.4056
T7	179.84-159.84	-1.6202	0.5226	-0.8654	0.0540
T8	159.84-139.84	-1.5910	0.5268	-0.7491	0.0821
T9	139.84-119.84	-1.5755	0.5272	-0.7379	0.0957
T10	119.84-99.84	-1.5943	0.4374	-0.8328	0.0376
T11	99.84-79.84	-1.4532	0.5658	-0.7211	0.1699

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	17 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section	Elevation	CP <sub>x</sub>	CP <sub>z</sub>	CP <sub>x</sub>	CP <sub>z</sub>
	ft	in	in	Ice in	Ice in
T12	79.84-59.84	-0.9837	1.0034	-0.4412	0.5464
T13	59.84-39.84	-0.9795	0.9802	-0.4652	0.5510
T14	39.84-19.84	-0.9966	0.9973	-0.4865	0.5872
T15	19.84-6.50	-0.9541	1.0014	-0.4298	0.6454
T16	6.50-0.00	-0.3684	0.3817	-0.0531	0.0844

### Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T1	16	LDF4-50A (1/2 FOAM)	279.84 - 291.80	0.6000	0.3400
T1	35	LDF5-50A (7/8 FOAM)	279.84 - 291.84	0.6000	0.3400
T2	14	LDF4-50A (1/2 FOAM)	259.84 - 277.00	0.6000	0.4298
T2	15	LDF4-50A (1/2 FOAM)	259.84 - 277.00	0.6000	0.4298
T2	16	LDF4-50A (1/2 FOAM)	259.84 - 279.84	0.6000	0.4298
T2	32	LDF7-50A (1-5/8 FOAM)	259.84 - 261.00	0.6000	0.4298
T2	33	LDF7-50A (1-5/8 FOAM)	259.84 - 267.00	0.6000	0.4298
T2	34	LDF7-50A (1-5/8 FOAM)	259.84 - 267.00	0.6000	0.4298
T2	35	LDF5-50A (7/8 FOAM)	259.84 - 279.84	0.6000	0.4298
T3	14	LDF4-50A (1/2 FOAM)	239.84 - 259.84	0.6000	0.3898
T3	15	LDF4-50A (1/2 FOAM)	239.84 - 259.84	0.6000	0.3898
T3	16	LDF4-50A (1/2 FOAM)	239.84 - 259.84	0.6000	0.3898
T3	29	LDF7-50A (1-5/8 FOAM)	239.84 - 250.00	0.6000	0.3898
T3	30	LDF7-50A (1-5/8 FOAM)	239.84 - 250.00	0.6000	0.3898
T3	31	LDF7-50A (1-5/8 FOAM)	239.84 - 252.00	0.6000	0.3898
T3	32	LDF7-50A (1-5/8 FOAM)	239.84 - 259.84	0.6000	0.3898
T3	33	LDF7-50A (1-5/8 FOAM)	239.84 - 259.84	0.6000	0.3898
T3	34	LDF7-50A (1-5/8 FOAM)	239.84 - 259.84	0.6000	0.3898
T3	35	LDF5-50A (7/8 FOAM)	239.84 - 259.84	0.6000	0.3898
T3	36	LDF7-50A (1-5/8 FOAM)	239.84 - 240.00	0.6000	0.3898
T3	37	LDF7-50A (1-5/8 FOAM)	239.84 - 250.00	0.6000	0.3898
T3	38	LDF4-50A (1/2 FOAM)	239.84 - 240.00	0.6000	0.3898
T3	39	LDF4-50A (1/2 FOAM)	239.84 -	0.6000	0.3898

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	18 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
			250.00		
T4	14	LDF4-50A (1/2 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	15	LDF4-50A (1/2 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	16	LDF4-50A (1/2 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	29	LDF7-50A (1-5/8 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	30	LDF7-50A (1-5/8 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	31	LDF7-50A (1-5/8 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	32	LDF7-50A (1-5/8 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	33	LDF7-50A (1-5/8 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	34	LDF7-50A (1-5/8 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	35	LDF5-50A (7/8 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	36	LDF7-50A (1-5/8 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	37	LDF7-50A (1-5/8 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	38	LDF4-50A (1/2 FOAM)	219.84 - 239.84	0.6000	0.4305
T4	39	LDF4-50A (1/2 FOAM)	219.84 - 239.84	0.6000	0.4305
T5	14	LDF4-50A (1/2 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	15	LDF4-50A (1/2 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	16	LDF4-50A (1/2 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	27	LDF5-50A (7/8 FOAM)	199.84 - 211.00	0.6000	0.3879
T5	28	LDF5-50A (7/8 FOAM)	199.84 - 211.00	0.6000	0.3879
T5	29	LDF7-50A (1-5/8 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	30	LDF7-50A (1-5/8 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	31	LDF7-50A (1-5/8 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	32	LDF7-50A (1-5/8 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	33	LDF7-50A (1-5/8 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	34	LDF7-50A (1-5/8 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	35	LDF5-50A (7/8 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	36	LDF7-50A (1-5/8 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	37	LDF7-50A (1-5/8 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	38	LDF4-50A (1/2 FOAM)	199.84 - 219.84	0.6000	0.3879
T5	39	LDF4-50A (1/2 FOAM)	199.84 - 219.84	0.6000	0.3879
T6	4	LDF7-50A (1-5/8 FOAM)	179.84 -	0.6000	0.4356

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	19 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T6	5	LDF7-50A (1-5/8 FOAM)	185.00 179.84 - 185.00	0.6000	0.4356
T6	6	Fiber Trunk	179.84 - 185.00	0.6000	0.4356
T6	7	DC Trunk	179.84 - 185.00	0.6000	0.4356
T6	14	LDF4-50A (1/2 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	15	LDF4-50A (1/2 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	16	LDF4-50A (1/2 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	17	LDF4-50A (1/2 FOAM)	179.84 - 190.00	0.6000	0.4356
T6	18	LDF4-50A (1/2 FOAM)	179.84 - 198.00	0.6000	0.4356
T6	27	LDF5-50A (7/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	28	LDF5-50A (7/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	29	LDF7-50A (1-5/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	30	LDF7-50A (1-5/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	31	LDF7-50A (1-5/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	32	LDF7-50A (1-5/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	33	LDF7-50A (1-5/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	34	LDF7-50A (1-5/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	35	LDF5-50A (7/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	36	LDF7-50A (1-5/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	37	LDF7-50A (1-5/8 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	38	LDF4-50A (1/2 FOAM)	179.84 - 199.84	0.6000	0.4356
T6	39	LDF4-50A (1/2 FOAM)	179.84 - 199.84	0.6000	0.4356
T7	4	LDF7-50A (1-5/8 FOAM)	159.84 - 179.84	0.6000	0.3906
T7	5	LDF7-50A (1-5/8 FOAM)	159.84 - 179.84	0.6000	0.3906
T7	6	Fiber Trunk	159.84 - 179.84	0.6000	0.3906
T7	7	DC Trunk	159.84 - 179.84	0.6000	0.3906
T7	14	LDF4-50A (1/2 FOAM)	159.84 - 179.84	0.6000	0.3906
T7	15	LDF4-50A (1/2 FOAM)	159.84 - 179.84	0.6000	0.3906
T7	16	LDF4-50A (1/2 FOAM)	159.84 - 179.84	0.6000	0.3906
T7	17	LDF4-50A (1/2 FOAM)	159.84 - 179.84	0.6000	0.3906
T7	18	LDF4-50A (1/2 FOAM)	159.84 - 179.84	0.6000	0.3906
T7	24	LDF4-50A (1/2 FOAM)	159.84 -	0.6000	0.3906

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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
			171.50		
T7	25	LDF5-50A (7/8 FOAM)	159.84 -	0.6000	0.3906
			172.00		
T7	26	LDF5-50A (7/8 FOAM)	159.84 -	0.6000	0.3906
			172.50		
T7	27	LDF5-50A (7/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	28	LDF5-50A (7/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	29	LDF7-50A (1-5/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	30	LDF7-50A (1-5/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	31	LDF7-50A (1-5/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	32	LDF7-50A (1-5/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	33	LDF7-50A (1-5/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	34	LDF7-50A (1-5/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	35	LDF5-50A (7/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	36	LDF7-50A (1-5/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	37	LDF7-50A (1-5/8 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	38	LDF4-50A (1/2 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T7	39	LDF4-50A (1/2 FOAM)	159.84 -	0.6000	0.3906
			179.84		
T8	4	LDF7-50A (1-5/8 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	5	LDF7-50A (1-5/8 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	6	Fiber Trunk	139.84 -	0.6000	0.4408
			159.84		
T8	7	DC Trunk	139.84 -	0.6000	0.4408
			159.84		
T8	12	LDF4-50A (1/2 FOAM)	139.84 -	0.6000	0.4408
			157.00		
T8	13	LDF4-50A (1/2 FOAM)	139.84 -	0.6000	0.4408
			159.00		
T8	14	LDF4-50A (1/2 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	15	LDF4-50A (1/2 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	16	LDF4-50A (1/2 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	17	LDF4-50A (1/2 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	18	LDF4-50A (1/2 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	24	LDF4-50A (1/2 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	25	LDF5-50A (7/8 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	26	LDF5-50A (7/8 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	27	LDF5-50A (7/8 FOAM)	139.84 -	0.6000	0.4408
			159.84		
T8	28	LDF5-50A (7/8 FOAM)	139.84 -	0.6000	0.4408

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	21 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T8	29	LDF7-50A (1-5/8 FOAM)	159.84 139.84 - 159.84	0.6000	0.4408
T8	30	LDF7-50A (1-5/8 FOAM)	139.84 - 159.84	0.6000	0.4408
T8	31	LDF7-50A (1-5/8 FOAM)	139.84 - 159.84	0.6000	0.4408
T8	32	LDF7-50A (1-5/8 FOAM)	139.84 - 159.84	0.6000	0.4408
T8	33	LDF7-50A (1-5/8 FOAM)	139.84 - 159.84	0.6000	0.4408
T8	34	LDF7-50A (1-5/8 FOAM)	139.84 - 159.84	0.6000	0.4408
T8	35	LDF5-50A (7/8 FOAM)	139.84 - 159.84	0.6000	0.4408
T8	36	LDF7-50A (1-5/8 FOAM)	139.84 - 159.84	0.6000	0.4408
T8	37	LDF7-50A (1-5/8 FOAM)	139.84 - 159.84	0.6000	0.4408
T8	38	LDF4-50A (1/2 FOAM)	139.84 - 159.84	0.6000	0.4408
T8	39	LDF4-50A (1/2 FOAM)	139.84 - 159.84	0.6000	0.4408
T9	4	LDF7-50A (1-5/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	5	LDF7-50A (1-5/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	6	Fiber Trunk	119.84 - 139.84	0.6000	0.4435
T9	7	DC Trunk	119.84 - 139.84	0.6000	0.4435
T9	12	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	13	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	14	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	15	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	16	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	17	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	18	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	24	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	25	LDF5-50A (7/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	26	LDF5-50A (7/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	27	LDF5-50A (7/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	28	LDF5-50A (7/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	29	LDF7-50A (1-5/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	30	LDF7-50A (1-5/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	31	LDF7-50A (1-5/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	32	LDF7-50A (1-5/8 FOAM)	119.84 -	0.6000	0.4435

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	22 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T9	33	LDF7-50A (1-5/8 FOAM)	139.84 119.84 - 139.84	0.6000	0.4435
T9	34	LDF7-50A (1-5/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	35	LDF5-50A (7/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	36	LDF7-50A (1-5/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	37	LDF7-50A (1-5/8 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	38	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T9	39	LDF4-50A (1/2 FOAM)	119.84 - 139.84	0.6000	0.4435
T10	4	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	5	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	6	Fiber Trunk	99.84 - 119.84	0.6000	0.4057
T10	7	DC Trunk	99.84 - 119.84	0.6000	0.4057
T10	12	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	13	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	14	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	15	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	16	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	17	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	18	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	20	EW63	99.84 - 104.00	0.6000	0.4057
T10	21	LDF5-50A (7/8 FOAM)	99.84 - 112.00	0.6000	0.4057
T10	22	EW63	99.84 - 116.00	0.6000	0.4057
T10	23	EW63	99.84 - 116.00	0.6000	0.4057
T10	24	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	25	LDF5-50A (7/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	26	LDF5-50A (7/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	27	LDF5-50A (7/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	28	LDF5-50A (7/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	29	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	30	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	31	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	32	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	33	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	34	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	35	LDF5-50A (7/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	36	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	37	LDF7-50A (1-5/8 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	38	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T10	39	LDF4-50A (1/2 FOAM)	99.84 - 119.84	0.6000	0.4057
T11	1	LDF6-50A (1-1/4 FOAM)	79.84 - 84.00	0.6000	0.4472
T11	2	LDF6-50A (1-1/4 FOAM)	79.84 - 84.00	0.6000	0.4472
T11	3	HYBRIFLEX 1-5/8"	79.84 - 84.00	0.6000	0.4472
T11	4	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	5	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	6	Fiber Trunk	79.84 - 99.84	0.6000	0.4472
T11	7	DC Trunk	79.84 - 99.84	0.6000	0.4472
T11	10	LDF4-50A (1/2 FOAM)	79.84 - 94.00	0.6000	0.4472
T11	11	LDF4-50A (1/2 FOAM)	79.84 - 94.00	0.6000	0.4472
T11	12	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	13	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	14	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	15	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	16	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	17	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	18	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472



<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	23 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T11	20	EW63	79.84 - 99.84	0.6000	0.4472
T11	21	LDF5-50A (7/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	22	EW63	79.84 - 99.84	0.6000	0.4472
T11	23	EW63	79.84 - 99.84	0.6000	0.4472
T11	24	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	25	LDF5-50A (7/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	26	LDF5-50A (7/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	27	LDF5-50A (7/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	28	LDF5-50A (7/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	29	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	30	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	31	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	32	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	33	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	34	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	35	LDF5-50A (7/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	36	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	37	LDF7-50A (1-5/8 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	38	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472
T11	39	LDF4-50A (1/2 FOAM)	79.84 - 99.84	0.6000	0.4472
T12	1	LDF6-50A (1-1/4 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	2	LDF6-50A (1-1/4 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	3	HYBRIFLEX 1-5/8"	59.84 - 79.84	0.6000	0.4590
T12	4	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	5	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	6	Fiber Trunk	59.84 - 79.84	0.6000	0.4590
T12	7	DC Trunk	59.84 - 79.84	0.6000	0.4590
T12	10	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	11	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	12	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	13	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	14	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	15	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	16	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	17	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	18	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	19	LDF5-50A (7/8 FOAM)	59.84 - 70.00	0.6000	0.4590
T12	20	EW63	59.84 - 79.84	0.6000	0.4590
T12	21	LDF5-50A (7/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	22	EW63	59.84 - 79.84	0.6000	0.4590
T12	23	EW63	59.84 - 79.84	0.6000	0.4590
T12	24	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	25	LDF5-50A (7/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	26	LDF5-50A (7/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	27	LDF5-50A (7/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	28	LDF5-50A (7/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	29	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	30	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	31	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	32	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	33	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	34	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	35	LDF5-50A (7/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	36	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	37	LDF7-50A (1-5/8 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	38	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T12	39	LDF4-50A (1/2 FOAM)	59.84 - 79.84	0.6000	0.4590
T13	1	LDF6-50A (1-1/4 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	2	LDF6-50A (1-1/4 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	3	HYBRIFLEX 1-5/8"	39.84 - 59.84	0.6000	0.4331
T13	4	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	5	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	24 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T13	6	Fiber Trunk	39.84 - 59.84	0.6000	0.4331
T13	7	DC Trunk	39.84 - 59.84	0.6000	0.4331
T13	10	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	11	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	12	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	13	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	14	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	15	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	16	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	17	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	18	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	19	LDF5-50A (7/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	20	EW63	39.84 - 59.84	0.6000	0.4331
T13	21	LDF5-50A (7/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	22	EW63	39.84 - 59.84	0.6000	0.4331
T13	23	EW63	39.84 - 59.84	0.6000	0.4331
T13	24	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	25	LDF5-50A (7/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	26	LDF5-50A (7/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	27	LDF5-50A (7/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	28	LDF5-50A (7/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	29	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	30	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	31	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	32	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	33	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	34	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	35	LDF5-50A (7/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	36	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	37	LDF7-50A (1-5/8 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	38	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T13	39	LDF4-50A (1/2 FOAM)	39.84 - 59.84	0.6000	0.4331
T14	1	LDF6-50A (1-1/4 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	2	LDF6-50A (1-1/4 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	3	HYBRIFLEX 1-5/8"	19.84 - 39.84	0.6000	0.4857
T14	4	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	5	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	6	Fiber Trunk	19.84 - 39.84	0.6000	0.4857
T14	7	DC Trunk	19.84 - 39.84	0.6000	0.4857
T14	10	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	11	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	12	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	13	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	14	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	15	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	16	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	17	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	18	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	19	LDF5-50A (7/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	20	EW63	19.84 - 39.84	0.6000	0.4857
T14	21	LDF5-50A (7/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	22	EW63	19.84 - 39.84	0.6000	0.4857
T14	23	EW63	19.84 - 39.84	0.6000	0.4857
T14	24	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	25	LDF5-50A (7/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	26	LDF5-50A (7/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	27	LDF5-50A (7/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	28	LDF5-50A (7/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	29	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	30	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	31	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	32	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	25 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T14	33	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	34	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	35	LDF5-50A (7/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	36	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	37	LDF7-50A (1-5/8 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	38	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T14	39	LDF4-50A (1/2 FOAM)	19.84 - 39.84	0.6000	0.4857
T15	1	LDF6-50A (1-1/4 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	2	LDF6-50A (1-1/4 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	3	HYBRIFLEX 1-5/8"	6.50 - 19.84	0.6000	0.5097
T15	4	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	5	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	6	Fiber Trunk	6.50 - 19.84	0.6000	0.5097
T15	7	DC Trunk	6.50 - 19.84	0.6000	0.5097
T15	8	LDF4-50A (1/2 FOAM)	6.50 - 13.00	0.6000	0.5097
T15	9	LDF4-50A (1/2 FOAM)	6.50 - 18.00	0.6000	0.5097
T15	10	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	11	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	12	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	13	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	14	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	15	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	16	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	17	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	18	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	19	LDF5-50A (7/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	20	EW63	6.50 - 19.84	0.6000	0.5097
T15	21	LDF5-50A (7/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	22	EW63	6.50 - 19.84	0.6000	0.5097
T15	23	EW63	6.50 - 19.84	0.6000	0.5097
T15	24	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	25	LDF5-50A (7/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	26	LDF5-50A (7/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	27	LDF5-50A (7/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	28	LDF5-50A (7/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	29	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	30	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	31	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	32	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	33	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	34	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	35	LDF5-50A (7/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	36	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	37	LDF7-50A (1-5/8 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	38	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T15	39	LDF4-50A (1/2 FOAM)	6.50 - 19.84	0.6000	0.5097
T16	1	LDF6-50A (1-1/4 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	2	LDF6-50A (1-1/4 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	3	HYBRIFLEX 1-5/8"	5.00 - 6.50	0.3052	0.0326
T16	4	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	5	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	6	Fiber Trunk	5.00 - 6.50	0.3052	0.0326
T16	7	DC Trunk	5.00 - 6.50	0.3052	0.0326
T16	8	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	9	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	10	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	11	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	12	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	13	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	14	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	15	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	16	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	26 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T16	17	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	18	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	19	LDF5-50A (7/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	20	EW63	5.00 - 6.50	0.3052	0.0326
T16	21	LDF5-50A (7/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	22	EW63	5.00 - 6.50	0.3052	0.0326
T16	23	EW63	5.00 - 6.50	0.3052	0.0326
T16	24	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	25	LDF5-50A (7/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	26	LDF5-50A (7/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	27	LDF5-50A (7/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	28	LDF5-50A (7/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	29	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	30	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	31	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	32	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	33	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	34	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	35	LDF5-50A (7/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	36	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	37	LDF7-50A (1-5/8 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	38	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326
T16	39	LDF4-50A (1/2 FOAM)	5.00 - 6.50	0.3052	0.0326

## Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horiz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K
4-ft Lightning Rod	A	From Face	0.00 0.00 0.00	0.0000	325.00	No Ice 1/2" Ice 1" Ice	0.40 0.81 1.06	0.40 0.81 1.06	0.01 0.01 0.02
Flash Beacon Lighting	B	None		0.0000	323.00	No Ice 1/2" Ice 1" Ice	2.70 3.10 3.50	2.70 3.10 3.50	0.05 0.07 0.09
6813 1-Bay w/radome (WHUS-34)	C	From Leg	2.00 0.00 0.00	0.0000	305.00	No Ice 1/2" Ice 1" Ice	4.90 6.00 7.10	4.90 6.00 7.10	0.10 0.20 0.29
PD1110 (WHUS-40)	C	From Leg	3.00 0.00 0.00	0.0000	277.00	No Ice 1/2" Ice 1" Ice	2.50 3.84 5.20	2.50 3.84 5.20	0.02 0.04 0.07
PD1110 (WHUS-39)	C	From Leg	1.50 0.00 0.00	0.0000	277.00	No Ice 1/2" Ice 1" Ice	2.50 3.84 5.20	2.50 3.84 5.20	0.02 0.04 0.07
ROHN 4-ft Side Arm (WHUS-40,39)	C	From Leg	2.00 0.00 0.00	0.0000	277.00	No Ice 1/2" Ice 1" Ice	5.28 7.88 10.48	5.28 7.88 10.48	0.07 0.08 0.10
OGT9-840 (CSP-9)	A	From Leg	3.00 0.00 0.00	0.0000	267.00	No Ice 1/2" Ice 1" Ice	2.27 3.44 4.61	2.27 3.44 4.61	0.02 0.04 0.06
DB810K	C	From Leg	3.00	0.0000	267.00	No Ice	4.08	4.08	0.04

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	27 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>AA</sub> Front ft<sup>2</sup></i>	<i>C<sub>AA</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>
(CSP-5)			0.00			1/2" Ice	5.73	0.07
			0.00			1" Ice	7.41	0.11
AP14-850/105	B	From Leg	3.00	0.0000	261.00	No Ice	10.61	0.03
(CSP-4)			0.00			1/2" Ice	11.25	0.08
			0.00			1" Ice	11.89	0.14
AP14-850/105	B	From Leg	3.00	0.0000	252.00	No Ice	10.61	0.03
(CSP-6)			0.00			1/2" Ice	11.25	0.08
			0.00			1" Ice	11.89	0.14
SE419-SF3P4LDF	A	From Leg	3.00	0.0000	256.50	No Ice	4.12	0.04
(CSP-16 Future)			0.00			1/2" Ice	5.11	0.05
			0.00			1" Ice	6.08	0.07
TTA 432-83H-01T	B	From Leg	3.00	0.0000	256.50	No Ice	1.40	0.03
(CSP-15 Future)			0.00			1/2" Ice	1.55	0.04
			0.00			1" Ice	1.70	0.05
TTA 432-83H-01T	C	From Leg	3.00	0.0000	256.50	No Ice	1.40	0.03
(CSP-19 Future)			0.00			1/2" Ice	1.55	0.04
			0.00			1" Ice	1.70	0.05
SC479-HF1LDF	A	From Leg	3.00	0.0000	264.00	No Ice	4.41	0.03
(CSP-12 Future)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
SC479-HF1LDF	B	From Leg	3.00	0.0000	264.00	No Ice	4.41	0.03
(CSP-13 Future)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
SC479-HF1LDF	C	From Leg	3.00	0.0000	249.00	No Ice	4.44	0.03
(CSP-14 Future - Inverted)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
SC479-HF1LDF	A	From Leg	3.00	0.0000	249.00	No Ice	4.44	0.03
(CSP-17 Future - Inverted)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
SC479-HF1LDF	B	From Leg	3.00	0.0000	249.00	No Ice	4.44	0.03
(CSP-18 Future - Inverted)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
OGT9-840	C	From Leg	3.00	0.0000	253.00	No Ice	2.27	0.02
(CSP-11 - Inverted)			0.00			1/2" Ice	3.44	0.04
			0.00			1" Ice	4.61	0.06
OGT9-840	B	From Leg	3.00	0.0000	253.00	No Ice	2.27	0.02
(CSP-7 - Inverted)			0.00			1/2" Ice	3.44	0.04
			0.00			1" Ice	4.61	0.06
SC479-HF1LDF	A	From Leg	3.00	0.0000	254.00	No Ice	4.43	0.03
(CSP-20 Future)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
SC479-HF1LDF	B	From Leg	3.00	0.0000	254.00	No Ice	4.43	0.03
(CSP-21 Future)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
SC479-HF1LDF	C	From Leg	3.00	0.0000	239.00	No Ice	4.46	0.03
(CSP-22 Future - Inverted)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
SC479-HF1LDF	A	From Leg	3.00	0.0000	239.00	No Ice	4.46	0.03
(CSP-24 Future - Inverted)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
SC479-HF1LDF	B	From Leg	3.00	0.0000	239.00	No Ice	4.46	0.03
(CSP-25 Future - Inverted)			0.00			1/2" Ice	6.54	0.07
			0.00			1" Ice	8.04	0.11
TTA 432-83H-01T	B	From Leg	3.00	0.0000	240.00	No Ice	1.40	0.03
(CSP-23 Future)			0.00			1/2" Ice	1.55	0.04
			0.00			1" Ice	1.70	0.05
TTA 432-83H-01T	C	From Leg	3.00	0.0000	240.00	No Ice	1.40	0.03

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	28 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>AA</sub> Front ft<sup>2</sup></i>	<i>C<sub>AA</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>
(CSP-26 Future)			0.00			1/2" Ice	1.55	0.04
			0.00			1" Ice	1.70	0.05
6813 1-Bay w/radome (WHUS-36)	C	From Leg	2.00	0.0000	211.00	No Ice	4.90	0.10
			0.00			1/2" Ice	6.00	0.20
			0.00			1" Ice	7.10	0.29
6813 1-Bay w/radome (WHUS-34)	B	From Leg	2.00	0.0000	198.00	No Ice	4.90	0.10
			0.00			1/2" Ice	6.00	0.20
			0.00			1" Ice	7.10	0.29
6812 (CPR-32 - Future)	A	From Leg	3.00	0.0000	198.00	No Ice	0.20	0.00
			0.00			1/2" Ice	0.36	0.00
			0.00			1" Ice	0.52	0.00
6' Yagi (CPR-33 - Future)	B	From Leg	3.00	0.0000	190.00	No Ice	5.00	0.04
			0.00			1/2" Ice	6.50	0.06
			0.00			1" Ice	8.00	0.08
24"x12"x5" Panel	B	From Leg	1.00	0.0000	172.17	No Ice	2.40	0.03
			0.00			1/2" Ice	2.60	0.05
			0.00			1" Ice	2.81	0.07
8' x 3" Dia Omni	C	From Leg	1.00	0.0000	172.00	No Ice	2.40	0.03
			0.00			1/2" Ice	3.19	0.04
			0.00			1" Ice	3.67	0.07
16"x12"x3" TTA	A	From Leg	1.00	0.0000	166.00	No Ice	1.60	0.01
			0.00			1/2" Ice	1.76	0.02
			0.00			1" Ice	1.93	0.03
24"x12"x5" Panel	C	From Leg	1.00	0.0000	158.83	No Ice	2.40	0.03
			0.00			1/2" Ice	2.60	0.05
			0.00			1" Ice	2.81	0.07
Beacon	A	From Leg	0.50	0.0000	157.00	No Ice	0.17	0.01
			0.00			1/2" Ice	0.31	0.01
			0.00			1" Ice	0.39	0.02
Beacon	B	From Leg	0.50	0.0000	157.00	No Ice	0.17	0.01
			0.00			1/2" Ice	0.31	0.01
			0.00			1" Ice	0.39	0.02
Beacon	C	From Leg	0.50	0.0000	157.00	No Ice	0.17	0.01
			0.00			1/2" Ice	0.31	0.01
			0.00			1" Ice	0.39	0.02
Sabre 2' Sidearm	B	From Leg	1.00	0.0000	125.00	No Ice	3.90	0.09
			0.00			1/2" Ice	4.40	0.10
			0.00			1" Ice	4.90	0.11
6'x4' Ice Shield	C	From Leg	1.00	0.0000	124.00	No Ice	0.02	0.28
			0.00			1/2" Ice	0.05	0.40
			0.00			1" Ice	0.08	0.52
9'x10' Ice Shield	A	From Leg	1.00	0.0000	124.00	No Ice	0.88	1.07
			0.00			1/2" Ice	1.20	1.40
			0.00			1" Ice	1.52	1.73
2'6"x4" Pipe Mount	C	From Leg	0.50	0.0000	124.00	No Ice	0.65	0.03
			0.00			1/2" Ice	0.91	0.04
			0.00			1" Ice	1.09	0.05
2'6"x4" Pipe Mount	A	From Leg	0.50	0.0000	124.00	No Ice	0.65	0.03
			0.00			1/2" Ice	0.91	0.04
			0.00			1" Ice	1.09	0.05
PD1110	B	From Leg	2.00	0.0000	112.00	No Ice	2.50	0.02
			0.00			1/2" Ice	3.84	0.04
			0.00			1" Ice	5.20	0.07
Sabre 2' Sidearm	B	From Leg	1.00	0.0000	112.00	No Ice	3.90	0.09
			0.00			1/2" Ice	4.40	0.10
			0.00			1" Ice	4.90	0.11
6'x4" Pipe Mount	C	From Leg	0.50	0.0000	104.00	No Ice	1.81	0.05

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	29 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>AA</sub> Front ft<sup>2</sup></i>	<i>C<sub>AA</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>
			0.00			1/2" Ice	2.46	0.07
			0.00			1" Ice	2.83	0.09
PR-850	C	From Leg	0.50	0.0000	94.00	No Ice	6.35	0.04
			0.00			1/2" Ice	11.43	0.05
			0.00			1" Ice	16.51	0.06
ASP-962	B	From Leg	0.50	0.0000	94.00	No Ice	0.16	0.00
			0.00			1/2" Ice	0.29	0.00
			0.00			1" Ice	0.42	0.00
DB212-1 (CSP-10)	C	From Leg	0.00	0.0000	70.00	No Ice	4.50	0.03
			0.00			1/2" Ice	8.10	0.04
			0.00			1" Ice	11.70	0.05
6' Yagi	C	From Leg	3.00	0.0000	18.00	No Ice	5.00	0.04
			0.00			1/2" Ice	6.50	0.06
			0.00			1" Ice	8.00	0.08
Sabre 2' Sidearm	C	From Leg	1.00	0.0000	18.00	No Ice	3.90	0.09
			0.00			1/2" Ice	4.40	0.10
			0.00			1" Ice	4.90	0.11
Sabre 2' Sidearm	C	From Leg	1.00	0.0000	13.00	No Ice	3.90	0.09
			0.00			1/2" Ice	4.40	0.10
			0.00			1" Ice	4.90	0.11
HBXX-6517DS (Verizon)	A	From Leg	3.00	0.0000	84.00	No Ice	8.53	0.05
			-6.00			1/2" Ice	9.00	0.10
			0.00			1" Ice	9.48	0.16
BXA-80063-4CF (Verizon)	A	From Leg	3.00	0.0000	84.00	No Ice	4.71	0.01
			-3.00			1/2" Ice	5.03	0.04
			0.00			1" Ice	5.35	0.07
X7C-FRO-440 (Verizon)	A	From Leg	3.00	0.0000	84.00	No Ice	7.97	0.03
			3.00			1/2" Ice	8.34	0.08
			0.00			1" Ice	8.72	0.13
HBXX-6517DS (Verizon)	A	From Leg	3.00	0.0000	84.00	No Ice	8.53	0.05
			6.00			1/2" Ice	9.00	0.10
			0.00			1" Ice	9.48	0.16
HBXX-6517DS (Verizon)	B	From Leg	3.00	0.0000	84.00	No Ice	8.53	0.05
			-6.00			1/2" Ice	9.00	0.10
			0.00			1" Ice	9.48	0.16
BXA-80063-4CF (Verizon)	B	From Leg	3.00	0.0000	84.00	No Ice	4.71	0.01
			-3.00			1/2" Ice	5.03	0.04
			0.00			1" Ice	5.35	0.07
X7C-FRO-440 (Verizon)	B	From Leg	3.00	0.0000	84.00	No Ice	7.97	0.03
			3.00			1/2" Ice	8.34	0.08
			0.00			1" Ice	8.72	0.13
HBXX-6517DS (Verizon)	B	From Leg	3.00	0.0000	84.00	No Ice	8.53	0.05
			6.00			1/2" Ice	9.00	0.10
			0.00			1" Ice	9.48	0.16
HBXX-6517DS (Verizon)	C	From Leg	3.00	0.0000	84.00	No Ice	8.53	0.05
			-6.00			1/2" Ice	9.00	0.10
			0.00			1" Ice	9.48	0.16
BXA-80063-4CF (Verizon)	C	From Leg	3.00	0.0000	84.00	No Ice	4.71	0.01
			-3.00			1/2" Ice	5.03	0.04
			0.00			1" Ice	5.35	0.07
X7C-FRO-440 (Verizon)	C	From Leg	3.00	0.0000	84.00	No Ice	7.97	0.03
			3.00			1/2" Ice	8.34	0.08
			0.00			1" Ice	8.72	0.13
HBXX-6517DS (Verizon)	C	From Leg	3.00	0.0000	84.00	No Ice	8.53	0.05
			6.00			1/2" Ice	9.00	0.10
			0.00			1" Ice	9.48	0.16
RRH2x60-AWS	A	From Leg	3.00	0.0000	84.00	No Ice	3.36	0.06



<div><b><i>tnxTower</i></b></div> <div><b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587</div>	<b>Job</b>  17004.42 - CT1077	<b>Page</b>  30 of 95
	<b>Project</b>  327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>  15:25:46 09/11/17
	<b>Client</b>  AT&T	<b>Designed by</b>  TJL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>AA</sub> Front ft<sup>2</sup></i>	<i>C<sub>AA</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>	
(Verizon)			-6.00			1/2" Ice	3.61	2.26	0.08
			0.00			1" Ice	3.88	2.50	0.11
RRH2x60-AWS	B	From Leg	3.00	0.0000	84.00	No Ice	3.36	2.03	0.06
(Verizon)			-6.00			1/2" Ice	3.61	2.26	0.08
			0.00			1" Ice	3.88	2.50	0.11
RRH2x60-AWS	C	From Leg	3.00	0.0000	84.00	No Ice	3.36	2.03	0.06
(Verizon)			-6.00			1/2" Ice	3.61	2.26	0.08
			0.00			1" Ice	3.88	2.50	0.11
RRH2x40-07-U	A	From Leg	3.00	0.0000	84.00	No Ice	1.93	1.05	0.05
(Verizon)			3.00			1/2" Ice	2.10	1.19	0.07
			0.00			1" Ice	2.28	1.33	0.09
RRH2x40-07-U	B	From Leg	3.00	0.0000	84.00	No Ice	1.93	1.05	0.05
(Verizon)			3.00			1/2" Ice	2.10	1.19	0.07
			0.00			1" Ice	2.28	1.33	0.09
RRH2x40-07-U	C	From Leg	3.00	0.0000	84.00	No Ice	1.93	1.05	0.05
(Verizon)			3.00			1/2" Ice	2.10	1.19	0.07
			0.00			1" Ice	2.28	1.33	0.09
FD-RRH 4x45 1900	A	From Leg	3.00	0.0000	84.00	No Ice	2.32	2.38	0.06
(Verizon)			6.00			1/2" Ice	2.52	2.59	0.08
			0.00			1" Ice	2.74	2.80	0.11
FD-RRH 4x45 1900	B	From Leg	3.00	0.0000	84.00	No Ice	2.32	2.38	0.06
(Verizon)			6.00			1/2" Ice	2.52	2.59	0.08
			0.00			1" Ice	2.74	2.80	0.11
FD-RRH 4x45 1900	C	From Leg	3.00	0.0000	84.00	No Ice	2.32	2.38	0.06
(Verizon)			6.00			1/2" Ice	2.52	2.59	0.08
			0.00			1" Ice	2.74	2.80	0.11
RC2DC-3315-PF-48	A	From Leg	3.00	0.0000	84.00	No Ice	3.01	1.96	0.03
(Verizon)			6.00			1/2" Ice	3.23	2.15	0.05
			0.00			1" Ice	3.46	2.35	0.08
13' Platform w/rails	A	None		0.0000	84.00	No Ice	31.30	31.30	1.82
(Verizon)						1/2" Ice	40.20	40.20	2.45
						1" Ice	49.10	49.10	3.08
7770.00	A	From Leg	3.00	0.0000	185.00	No Ice	5.51	2.93	0.04
(AT&T - Existing)			-6.00			1/2" Ice	5.87	3.27	0.07
			0.00			1" Ice	6.23	3.63	0.11
OPA-65R-LCUU-H8	A	From Leg	3.00	0.0000	185.00	No Ice	12.98	7.52	0.09
(AT&T - Existing)			-3.00			1/2" Ice	13.56	8.09	0.16
			0.00			1" Ice	14.15	8.67	0.24
TPA-65R-LCUUUU-H8	A	From Leg	3.00	0.0000	185.00	No Ice	13.30	8.82	0.08
(AT&T - Proposed)			3.00			1/2" Ice	13.90	9.42	0.15
			0.00			1" Ice	14.50	10.03	0.24
HPA-65R-BUU-H8	A	From Leg	3.00	0.0000	185.00	No Ice	12.98	7.52	0.07
(AT&T - Existing)			6.00			1/2" Ice	13.56	8.09	0.14
			0.00			1" Ice	14.15	8.67	0.22
7770.00	B	From Leg	3.00	0.0000	185.00	No Ice	5.51	2.93	0.04
(AT&T - Existing)			-6.00			1/2" Ice	5.87	3.27	0.07
			0.00			1" Ice	6.23	3.63	0.11
OPA-65R-LCUU-H6	B	From Leg	3.00	0.0000	185.00	No Ice	9.66	5.52	0.07
(AT&T - Existing)			-3.00			1/2" Ice	10.13	5.97	0.13
			0.00			1" Ice	10.61	6.43	0.20
QS66512-2	B	From Leg	3.00	0.0000	185.00	No Ice	8.13	6.80	0.11
(AT&T - Proposed)			3.00			1/2" Ice	8.59	7.27	0.17
			0.00			1" Ice	9.05	7.72	0.23
HPA-65R-BUU-H6	B	From Leg	3.00	0.0000	185.00	No Ice	9.66	6.45	0.05
(AT&T - Existing)			6.00			1/2" Ice	10.13	6.91	0.11
			0.00			1" Ice	10.61	7.38	0.18
7770.00	C	From Leg	3.00	0.0000	185.00	No Ice	5.51	2.93	0.04



<div><b><i>tnxTower</i></b></div> <div><b><i>Centek Engineering Inc.</i></b> <i>63-2 North Branford Rd.</i> <i>Branford, CT 06405</i> <i>Phone: (203) 488-0580</i> <i>FAX: (203) 488-8587</i></div>	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	31 of 95	
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT		<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T		<b>Designed by</b>	TJL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>AA</sub> Front ft<sup>2</sup></i>	<i>C<sub>AA</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>
(AT&T - Existing)			-6.00			1/2" Ice	5.87	3.27
			0.00			1" Ice	6.23	3.63
OPA-65R-LCUU-H8	C	From Leg	3.00	0.0000	185.00	No Ice	12.98	7.52
(AT&T - Existing)			-3.00			1/2" Ice	13.56	8.09
			0.00			1" Ice	14.15	8.67
TPA-65R-LCUUUU-H8	C	From Leg	3.00	0.0000	185.00	No Ice	13.30	8.82
(AT&T - Proposed)			3.00			1/2" Ice	13.90	9.42
			0.00			1" Ice	14.50	10.03
HPA-65R-BUU-H8	C	From Leg	3.00	0.0000	185.00	No Ice	12.98	7.52
(AT&T - Existing)			6.00			1/2" Ice	13.56	8.09
			0.00			1" Ice	14.15	8.67
DTMABP7819VG12A TMA	A	From Leg	3.00	0.0000	185.00	No Ice	1.36	0.51
(AT&T - Existing)			-6.00			1/2" Ice	1.51	0.61
			0.00			1" Ice	1.66	0.72
DTMABP7819VG12A TMA	B	From Leg	3.00	0.0000	185.00	No Ice	1.36	0.51
(AT&T - Existing)			-6.00			1/2" Ice	1.51	0.61
			0.00			1" Ice	1.66	0.72
DTMABP7819VG12A TMA	C	From Leg	3.00	0.0000	185.00	No Ice	1.36	0.51
(AT&T - Existing)			-6.00			1/2" Ice	1.51	0.61
			0.00			1" Ice	1.66	0.72
(2) TPX-070821	A	From Leg	3.00	0.0000	185.00	No Ice	0.47	0.10
(AT&T - Existing)			-3.00			1/2" Ice	0.56	0.15
			0.00			1" Ice	0.66	0.20
(2) TPX-070821	B	From Leg	3.00	0.0000	185.00	No Ice	0.47	0.10
(AT&T - Existing)			-3.00			1/2" Ice	0.56	0.15
			0.00			1" Ice	0.66	0.20
(2) TPX-070821	C	From Leg	3.00	0.0000	185.00	No Ice	0.47	0.10
(AT&T - Existing)			-3.00			1/2" Ice	0.56	0.15
			0.00			1" Ice	0.66	0.20
RRUS-11	A	From Leg	1.00	0.0000	185.00	No Ice	2.57	1.07
(AT&T - Existing)			1.00			1/2" Ice	2.76	1.21
			0.00			1" Ice	2.97	1.36
RRUS-11	B	From Leg	1.00	0.0000	185.00	No Ice	2.57	1.07
(AT&T - Existing)			1.00			1/2" Ice	2.76	1.21
			0.00			1" Ice	2.97	1.36
RRUS-11	C	From Leg	1.00	0.0000	185.00	No Ice	2.57	1.07
(AT&T - Existing)			1.00			1/2" Ice	2.76	1.21
			0.00			1" Ice	2.97	1.36
RRUS-32	A	From Leg	2.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Existing)			2.00			1/2" Ice	3.56	2.64
			0.00			1" Ice	3.81	2.86
RRUS-32	B	From Leg	2.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Existing)			2.00			1/2" Ice	3.56	2.64
			0.00			1" Ice	3.81	2.86
RRUS-32	C	From Leg	2.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Existing)			2.00			1/2" Ice	3.56	2.64
			0.00			1" Ice	3.81	2.86
RRUS-32	A	From Leg	3.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Existing)			-3.00			1/2" Ice	3.56	2.64
			0.00			1" Ice	3.81	2.86
RRUS-32	B	From Leg	3.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Existing)			-3.00			1/2" Ice	3.56	2.64
			0.00			1" Ice	3.81	2.86
RRUS-32	C	From Leg	3.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Existing)			-3.00			1/2" Ice	3.56	2.64
			0.00			1" Ice	3.81	2.86
RRUS-32	A	From Leg	3.00	0.0000	185.00	No Ice	3.31	2.42

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	32 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>AA</sub> Front ft<sup>2</sup></i>	<i>C<sub>AA</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>
(AT&T - Proposed)			0.00			1/2" Ice	3.56	2.64
			2.00			1" Ice	3.81	2.86
RRUS-32	B	From Leg	3.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Proposed)			0.00			1/2" Ice	3.56	2.64
			2.00			1" Ice	3.81	2.86
RRUS-32	C	From Leg	3.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Proposed)			0.00			1/2" Ice	3.56	2.64
			2.00			1" Ice	3.81	2.86
RRUS-32	A	From Leg	3.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Proposed)			0.00			1/2" Ice	3.56	2.64
			-2.00			1" Ice	3.81	2.86
RRUS-32	B	From Leg	3.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Proposed)			0.00			1/2" Ice	3.56	2.64
			-2.00			1" Ice	3.81	2.86
RRUS-32	C	From Leg	3.00	0.0000	185.00	No Ice	3.31	2.42
(AT&T - Proposed)			0.00			1/2" Ice	3.56	2.64
			-2.00			1" Ice	3.81	2.86
B14 4478	A	From Leg	3.00	0.0000	185.00	No Ice	1.63	0.91
(AT&T - Proposed)			3.00			1/2" Ice	1.79	1.03
			0.00			1" Ice	1.95	1.17
B14 4478	B	From Leg	3.00	0.0000	185.00	No Ice	1.63	0.91
(AT&T - Proposed)			3.00			1/2" Ice	1.79	1.03
			0.00			1" Ice	1.95	1.17
B14 4478	C	From Leg	3.00	0.0000	185.00	No Ice	1.63	0.91
(AT&T - Proposed)			3.00			1/2" Ice	1.79	1.03
			0.00			1" Ice	1.95	1.17
DC6-48-60-18-8F Surge	A	None		0.0000	185.00	No Ice	1.91	1.91
Arrestor						1/2" Ice	2.10	2.10
(AT&T - Existing)						1" Ice	2.29	2.29
DC6-48-60-18-8F Surge	B	None		0.0000	185.00	No Ice	1.91	1.91
Arrestor						1/2" Ice	2.10	2.10
(AT&T - Existing)						1" Ice	2.29	2.29
DC6-48-60-18-8F Surge	C	None		0.0000	185.00	No Ice	1.91	1.91
Arrestor						1/2" Ice	2.10	2.10
(AT&T - Existing)						1" Ice	2.29	2.29
12' V-Frame	A	None		0.0000	185.00	No Ice	9.22	12.97
(AT&T - Existing)						1/2" Ice	9.22	12.97
						1" Ice	9.22	12.97
12' V-Frame	B	None		0.0000	185.00	No Ice	9.22	12.97
(AT&T - Existing)						1/2" Ice	9.22	12.97
						1" Ice	9.22	12.97
12' V-Frame	C	None		0.0000	185.00	No Ice	9.22	12.97
(AT&T - Existing)						1/2" Ice	9.22	12.97
						1" Ice	9.22	12.97

## Dishes

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	33 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft <sup>2</sup>	Weight K
6-ft Dish (CSP - Future)	A	Paraboloid w/Radome	From Leg	1.00 0.00 0.00	0.0000		290.00	6.00	No Ice 28.27 1/2" Ice 29.07 1" Ice 29.87	0.05 0.06 0.35
6-ft Dish (CSP - Future)	B	Paraboloid w/Radome	From Leg	1.00 0.00 0.00	0.0000		290.00	6.00	No Ice 28.27 1/2" Ice 29.07 1" Ice 29.87	0.05 0.06 0.35
6-ft Dish (CSP - Future)	C	Paraboloid w/Radome	From Leg	1.00 0.00 0.00	0.0000		290.00	6.00	No Ice 28.27 1/2" Ice 29.07 1" Ice 29.87	0.05 0.06 0.35
5' Gird Dish	A	Grid	From Leg	1.00 0.00 0.00	0.0000		171.50	5.00	No Ice 19.63 1/2" Ice 20.29 1" Ice 20.95	0.10 0.20 0.30
6-ft Dish	A	Paraboloid w/o Radome	From Leg	1.00 0.00 0.00	0.0000		116.00	6.00	No Ice 28.27 1/2" Ice 29.07 1" Ice 29.87	0.05 0.06 0.35
6-ft Dish	C	Paraboloid w/Shroud (HP)	From Leg	1.00 0.00 0.00	0.0000		116.00	6.00	No Ice 28.27 1/2" Ice 29.07 1" Ice 29.87	0.05 0.06 0.35
6-ft Dish	C	Paraboloid w/Radome	From Leg	1.00 0.00 0.00	0.0000		104.00	6.00	No Ice 28.27 1/2" Ice 29.07 1" Ice 29.87	0.05 0.06 0.35
4-ft Dish	C	Paraboloid w/o Radome	From Leg	2.00 0.00 0.00	0.0000		13.00	4.00	No Ice 3.14 1/2" Ice 3.41 1" Ice 3.68	0.10 0.10 0.12

## Tower Pressures - No Ice

$G_H = 0.850$  (base tower), 1.350 (upper structure)

Section Elevation ft	z ft	K <sub>Z</sub>	q <sub>z</sub> psf	A <sub>G</sub> ft <sup>2</sup>	F a c e	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft <sup>2</sup>	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
L1 327.00-291.84	309.47	1.605	36	31.497	A	0.000	31.497	31.497	100.00	0.000	0.000
					B	0.000	31.497		100.00	0.000	0.000
					C	0.000	31.497		100.00	0.000	0.000
T1 291.84-279.84	285.84	1.579	35	46.040	A	3.503	6.950	4.000	38.27	1.308	0.000
					B	3.503	6.950		38.27	0.753	0.000
					C	3.503	7.534		36.24	0.000	0.000
T2 279.84-259.84	269.84	1.56	35	76.733	A	0.000	11.672	6.667	57.12	5.245	0.000
					B	0.000	11.672		57.12	3.422	0.000
					C	0.000	12.548		53.13	0.000	0.000
T3 259.84-239.84	249.84	1.535	34	77.150	A	3.482	12.189	7.500	47.86	34.020	0.000
					B	3.482	12.189		47.86	3.780	0.000
					C	3.482	13.059		45.34	0.000	0.000
T4 239.84-219.84	229.84	1.508	33	77.150	A	0.000	12.476	7.500	60.12	74.540	0.000
					B	0.000	12.476		60.12	3.780	0.000
					C	0.000	13.346		56.20	0.000	0.000
T5 219.84-199.84	209.84	1.479	33	77.567	A	3.462	13.286	8.333	49.76	76.973	0.000
					B	3.462	13.286		49.76	3.780	0.000
					C	3.462	14.152		47.31	0.000	0.000
T6 199.84-179.84	189.84	1.448	32	77.567	A	0.000	12.987	8.333	64.17	78.900	0.000
					B	0.000	12.987		64.17	5.564	0.000
					C	0.000	13.852		60.16	14.118	0.000
T7	169.84	1.415	31	77.983	A	3.441	14.090	9.167	52.29	82.340	0.000

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	17004.42 - CT1077	34 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>
ft	ft		psf	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>			
179.84-159.84					B	3.441	14.090		52.29	6.300	0.000
					C	3.441	14.950		49.84	54.720	0.000
T8	149.84	1.378	31	77.567	A	0.000	13.276	8.333	62.77	84.520	0.000
159.84-139.84					B	0.000	13.276		62.77	8.588	0.000
					C	0.000	14.141		58.93	54.720	0.000
T9	129.84	1.337	30	77.983	A	0.000	13.795	9.167	66.45	84.520	0.000
139.84-119.84					B	0.000	13.795		66.45	8.820	0.000
					C	0.000	14.656		62.55	54.720	0.000
T10	109.84	1.291	29	77.983	A	3.441	14.087	9.167	52.30	91.588	0.000
119.84-99.84					B	3.441	14.087		52.30	8.820	0.000
					C	3.441	14.947		49.85	54.720	0.000
T11	89.84	1.237	27	78.400	A	0.000	14.890	10.000	67.16	96.145	0.000
99.84-79.84					B	0.000	14.890		67.16	10.604	0.000
					C	0.000	15.745		63.51	63.281	0.000
T12	69.84	1.173	26	78.400	A	0.000	14.598	10.000	68.50	97.253	0.000
79.84-59.84					B	0.000	14.598		68.50	11.340	0.000
					C	0.000	15.453		64.71	95.880	0.000
T13	49.84	1.093	24	78.400	A	3.420	14.313	10.000	56.39	98.325	0.000
59.84-39.84					B	3.420	14.313		56.39	11.340	0.000
					C	3.420	15.168		53.80	95.880	0.000
T14	29.84	0.981	22	78.400	A	0.000	14.598	10.000	68.50	98.325	0.000
39.84-19.84					B	0.000	14.598		68.50	11.340	0.000
					C	0.000	15.453		64.71	95.880	0.000
T15 19.84-6.50	13.17	0.85	19	52.293	A	0.000	9.735	6.670	68.51	65.583	0.000
					B	0.000	9.735		68.51	8.698	0.000
					C	0.000	10.306		64.72	63.952	0.000
T16 6.50-0.00	3.25	0.85	19	13.616	A	6.042	3.418	3.418	36.13	7.374	0.000
					B	6.042	3.418		36.13	1.040	0.000
					C	6.042	3.418		36.13	7.191	0.000

## Tower Pressure - With Ice

$G_H = 0.850$  (base tower),  $1.350$  (upper structure)

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	t <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>
ft	ft		psf	in	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>			
L1	309.47	1.605	6	2.5017	46.158	A	0.000	46.158	46.158	100.00	0.000	0.000
327.00-291.84						B	0.000	46.158		100.00	0.000	0.000
						C	0.000	46.158		100.00	0.000	0.000
T1	285.84	1.579	5	2.4819	51.004	A	3.503	30.161	13.928	41.37	7.265	0.000
291.84-279.84						B	3.503	30.161		41.37	6.690	0.000
						C	3.503	33.644		37.49	0.000	0.000
T2	269.84	1.56	5	2.4677	84.959	A	0.000	48.447	23.118	47.72	22.756	0.000
279.84-259.84						B	0.000	48.447		47.72	30.231	0.000
						C	0.000	53.646		43.09	0.000	0.000
T3	249.84	1.535	5	2.4488	85.313	A	3.482	48.576	23.825	45.77	122.333	0.000
259.84-239.84						B	3.482	48.576		45.77	33.165	0.000
						C	3.482	53.711		41.66	0.000	0.000
T4	229.84	1.508	5	2.4284	85.245	A	0.000	48.546	23.689	48.80	265.380	0.000
239.84-219.84						B	0.000	48.546		48.80	32.921	0.000
						C	0.000	53.646		44.16	0.000	0.000
T5	209.84	1.479	5	2.4064	85.588	A	3.462	48.927	24.376	46.53	277.358	0.000
219.84-199.84						B	3.462	48.927		46.53	32.657	0.000
						C	3.462	53.958		42.45	0.000	0.000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	Job	Page
	17004.42 - CT1077	35 of 95
	Project 327' Guyed Tower - N. Eagleville Road Storrs, CT	Date 15:25:46 09/11/17
	Client AT&T	Designed by TJL

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	t <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
ft	ft		psf	in	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>			
T6 199.84-179.84	189.84	1.448	5	2.3824	85.508	A	0.000	48.258	24.216	50.18	286.298	0.000
						B	0.000	48.258		50.18	47.647	0.000
						C	0.000	53.247		45.48	45.963	0.000
T7 179.84-159.84	169.84	1.415	5	2.3560	85.837	A	3.441	48.869	24.874	47.55	305.283	0.000
						B	3.441	48.869		47.55	53.421	0.000
						C	3.441	53.783		43.47	177.531	0.000
T8 159.84-139.84	149.84	1.378	5	2.3267	85.322	A	0.000	47.709	23.845	49.98	316.365	0.000
						B	0.000	47.709		49.98	72.023	0.000
						C	0.000	52.602		45.33	176.842	0.000
T9 139.84-119.84	129.84	1.337	5	2.2936	85.629	A	0.000	47.653	24.457	51.32	313.905	0.000
						B	0.000	47.653		51.32	73.041	0.000
						C	0.000	52.459		46.62	176.065	0.000
T10 119.84-99.84	109.84	1.291	4	2.2556	85.502	A	3.441	47.369	24.204	47.64	340.088	0.000
						B	3.441	47.369		47.64	71.976	0.000
						C	3.441	52.110		43.57	175.173	0.000
T11 99.84-79.84	89.84	1.237	4	2.2107	85.769	A	0.000	47.414	24.738	52.17	354.739	0.000
						B	0.000	47.414		52.17	85.024	0.000
						C	0.000	52.050		47.53	200.134	0.000
T12 79.84-59.84	69.84	1.173	4	2.1557	85.586	A	0.000	46.301	24.371	52.64	355.264	0.000
						B	0.000	46.301		52.64	88.945	0.000
						C	0.000	50.843		47.93	296.984	0.000
T13 59.84-39.84	49.84	1.093	4	2.0842	85.347	A	3.420	44.964	23.895	49.39	353.839	0.000
						B	3.420	44.964		49.39	86.371	0.000
						C	3.420	49.384		45.25	294.129	0.000
T14 39.84-19.84	29.84	0.981	3	1.9800	85.000	A	0.000	43.717	23.200	53.07	344.021	0.000
						B	0.000	43.717		53.07	82.619	0.000
						C	0.000	47.958		48.38	289.977	0.000
T15 19.84-6.50	13.17	0.85	3	1.8245	56.349	A	0.000	27.629	14.783	53.51	219.699	0.000
						B	0.000	27.629		53.51	58.987	0.000
						C	0.000	30.279		48.82	189.293	0.000
T16 6.50-0.00	3.25	0.85	3	1.5862	15.402	A	6.042	8.859	7.033	47.20	23.024	0.000
						B	6.042	8.859		47.20	6.274	0.000
						C	6.042	8.859		47.20	20.578	0.000

## Tower Pressure - Service

$G_H = 0.850$  (base tower),  $1.350$  (upper structure)

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
ft	ft		psf	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>			
L1 327.00-291.84	309.47	1.605	13	31.497	A	0.000	31.497	31.497	100.00	0.000	0.000
					B	0.000	31.497		100.00	0.000	0.000
					C	0.000	31.497		100.00	0.000	0.000
T1 291.84-279.84	285.84	1.579	12	46.040	A	3.503	6.950	4.000	38.27	1.308	0.000
					B	3.503	6.950		38.27	0.753	0.000
					C	3.503	7.534		36.24	0.000	0.000
T2 279.84-259.84	269.84	1.56	12	76.733	A	0.000	11.672	6.667	57.12	5.245	0.000
					B	0.000	11.672		57.12	3.422	0.000
					C	0.000	12.548		53.13	0.000	0.000
T3 259.84-239.84	249.84	1.535	12	77.150	A	3.482	12.189	7.500	47.86	34.020	0.000
					B	3.482	12.189		47.86	3.780	0.000
					C	3.482	13.059		45.34	0.000	0.000
T4 229.84	229.84	1.508	12	77.150	A	0.000	12.476	7.500	60.12	74.540	0.000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	Job	17004.42 - CT1077	Page 36 of 95
	Project	327' Guyed Tower - N. Eagleville Road Storrs, CT	Date 15:25:46 09/11/17
	Client	AT&T	Designed by TJL

Section Elevation  ft	z  ft	K <sub>Z</sub>	q <sub>z</sub>  psf	A <sub>G</sub>  ft <sup>2</sup>	F a c e	A <sub>F</sub>  ft <sup>2</sup>	A <sub>R</sub>  ft <sup>2</sup>	A <sub>leg</sub>  ft <sup>2</sup>	Leg %  %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
239.84-219.84					B	0.000	12.476		60.12	3.780	0.000
					C	0.000	13.346		56.20	0.000	0.000
T5	209.84	1.479	12	77.567	A	3.462	13.286	8.333	49.76	76.973	0.000
219.84-199.84					B	3.462	13.286		49.76	3.780	0.000
					C	3.462	14.152		47.31	0.000	0.000
T6	189.84	1.448	11	77.567	A	0.000	12.987	8.333	64.17	78.900	0.000
199.84-179.84					B	0.000	12.987		64.17	5.564	0.000
					C	0.000	13.852		60.16	14.118	0.000
T7	169.84	1.415	11	77.983	A	3.441	14.090	9.167	52.29	82.340	0.000
179.84-159.84					B	3.441	14.090		52.29	6.300	0.000
					C	3.441	14.950		49.84	54.720	0.000
T8	149.84	1.378	11	77.567	A	0.000	13.276	8.333	62.77	84.520	0.000
159.84-139.84					B	0.000	13.276		62.77	8.588	0.000
					C	0.000	14.141		58.93	54.720	0.000
T9	129.84	1.337	10	77.983	A	0.000	13.795	9.167	66.45	84.520	0.000
139.84-119.84					B	0.000	13.795		66.45	8.820	0.000
					C	0.000	14.656		62.55	54.720	0.000
T10	109.84	1.291	10	77.983	A	3.441	14.087	9.167	52.30	91.588	0.000
119.84-99.84					B	3.441	14.087		52.30	8.820	0.000
					C	3.441	14.947		49.85	54.720	0.000
T11	89.84	1.237	10	78.400	A	0.000	14.890	10.000	67.16	96.145	0.000
99.84-79.84					B	0.000	14.890		67.16	10.604	0.000
					C	0.000	15.745		63.51	63.281	0.000
T12	69.84	1.173	9	78.400	A	0.000	14.598	10.000	68.50	97.253	0.000
79.84-59.84					B	0.000	14.598		68.50	11.340	0.000
					C	0.000	15.453		64.71	95.880	0.000
T13	49.84	1.093	9	78.400	A	3.420	14.313	10.000	56.39	98.325	0.000
59.84-39.84					B	3.420	14.313		56.39	11.340	0.000
					C	3.420	15.168		53.80	95.880	0.000
T14	29.84	0.981	8	78.400	A	0.000	14.598	10.000	68.50	98.325	0.000
39.84-19.84					B	0.000	14.598		68.50	11.340	0.000
					C	0.000	15.453		64.71	95.880	0.000
T15 19.84-6.50	13.17	0.85	7	52.293	A	0.000	9.735	6.670	68.51	65.583	0.000
					B	0.000	9.735		68.51	8.698	0.000
					C	0.000	10.306		64.72	63.952	0.000
T16 6.50-0.00	3.25	0.85	7	13.616	A	6.042	3.418	3.418	36.13	7.374	0.000
					B	6.042	3.418		36.13	1.040	0.000
					C	6.042	3.418		36.13	7.191	0.000

### Tower Forces - No Ice - Wind Normal To Face

Section Elevation  ft	Add Weight  K	Self Weight  K	F a c e	e	C <sub>F</sub>	q <sub>z</sub>  psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>  ft <sup>2</sup>	F  K	w  plf	Ctrl. Face
L1	0.00	3.14	A	1	0.6	36	1	1	31.497	0.91	25.86	C
327.00-291.84			B	1	0.6		1	1	31.497			
			C	1	0.6		1	1	31.497			
T1	0.01	1.17	A	0.227	2.508	35	1	1	7.532	0.62	51.43	C
291.84-279.84		TA 0.84	B	0.227	2.508		1	1	7.532			
			C	0.24	2.469		1	1	7.892			
T2	0.03	1.30	A	0.152	2.764	35	1	1	6.627	0.73	36.26	C
279.84-259.84			B	0.152	2.764		1	1	6.627			
			C	0.164	2.722		1	1	7.140			
T3	0.15	1.83	A	0.203	2.586	34	1	1	10.491	1.47	73.50	C
259.84-239.84		TA 0.84	B	0.203	2.586		1	1	10.491			
			C	0.214	2.549		1	1	11.020			

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	37 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
ft	K	K										
T4 239.84-219.84	0.31	1.47	A	0.162	2.729	33	1	1	7.097	1.92	95.96	C
			B	0.162	2.729		1	1	7.097			
			C	0.173	2.689		1	1	7.611			
T5 219.84-199.84	0.31	2.10	A	0.216	2.544	33	1	1	11.134	2.17	108.45	C
		TA 0.84	B	0.216	2.544		1	1	11.134			
			C	0.227	2.508		1	1	11.666			
T6 199.84-179.84	0.39	1.58	A	0.167	2.708	32	1	1	7.397	2.19	109.68	C
			B	0.167	2.708		1	1	7.397			
			C	0.179	2.669		1	1	7.911			
T7 179.84-159.84	0.61	2.31	A	0.225	2.515	31	1	1	11.603	3.10	155.00	C
		TA 0.84	B	0.225	2.515		1	1	11.603			
			C	0.236	2.481		1	1	12.137			
T8 159.84-139.84	0.62	1.66	A	0.171	2.695	31	1	1	7.568	2.86	143.23	C
			B	0.171	2.695		1	1	7.568			
			C	0.182	2.656		1	1	8.083			
T9 139.84-119.84	0.62	1.79	A	0.177	2.675	30	1	1	7.875	2.80	139.97	C
			B	0.177	2.675		1	1	7.875			
			C	0.188	2.637		1	1	8.390			
T10 119.84-99.84	0.64	2.31	A	0.225	2.515	29	1	1	11.600	3.00	150.01	C
		TA 0.84	B	0.225	2.515		1	1	11.600			
			C	0.236	2.481		1	1	12.135			
T11 99.84-79.84	0.70	2.10	A	0.19	2.63	27	1	1	8.529	2.93	146.48	C
			B	0.19	2.63		1	1	8.529			
			C	0.201	2.593		1	1	9.047			
T12 79.84-59.84	0.86	2.02	A	0.186	2.643	26	1	1	8.353	3.23	161.40	C
			B	0.186	2.643		1	1	8.353			
			C	0.197	2.606		1	1	8.869			
T13 59.84-39.84	0.86	2.38	A	0.226	2.511	24	1	1	11.715	3.17	158.45	C
		TA 0.84	B	0.226	2.511		1	1	11.715			
			C	0.237	2.477		1	1	12.247			
T14 39.84-19.84	0.86	2.02	A	0.186	2.643	22	1	1	8.353	2.71	135.54	C
			B	0.186	2.643		1	1	8.353			
			C	0.197	2.606		1	1	8.869			
T15 19.84-6.50	0.58	1.35	A	0.186	2.643	19	1	1	5.571	1.58	118.24	C
			B	0.186	2.643		1	1	5.571			
			C	0.197	2.606		1	1	5.915			
T16 6.50-0.00	0.07	0.81	A	0.695	1.776	19	1	1	8.801	0.33	50.31	C
			B	0.695	1.776		1	1	8.801			
			C	0.695	1.776		1	1	8.801			
Sum Weight:	7.61	36.39								35.71		

### Tower Forces - No Ice - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
ft	K	K										
L1 327.00-291.84	0.00	3.14	A	1	0.6	36	1	1	31.497	0.91	25.86	C
			B	1	0.6		1	1	31.497			
			C	1	0.6		1	1	31.497			
T1 291.84-279.84	0.01	1.17	A	0.227	2.508	35	0.825	1	6.919	0.57	47.68	C
		TA 0.84	B	0.227	2.508		0.825	1	6.919			
			C	0.24	2.469		0.825	1	7.279			
T2	0.03	1.30	A	0.152	2.764	35	0.825	1	6.627	0.73	36.26	C

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	38 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
279.84-259.84			B	0.152	2.764		0.825	1	6.627			
			C	0.164	2.722		0.825	1	7.140			
T3	0.15	1.83	A	0.203	2.586	34	0.825	1	9.882	1.42	71.25	C
259.84-239.84		TA 0.84	B	0.203	2.586		0.825	1	9.882			
			C	0.214	2.549		0.825	1	10.410			
T4	0.31	1.47	A	0.162	2.729	33	0.825	1	7.097	1.92	95.96	C
239.84-219.84			B	0.162	2.729		0.825	1	7.097			
			C	0.173	2.689		0.825	1	7.611			
T5	0.31	2.10	A	0.216	2.544	33	0.825	1	10.528	2.13	106.33	C
219.84-199.84		TA 0.84	B	0.216	2.544		0.825	1	10.528			
			C	0.227	2.508		0.825	1	11.060			
T6	0.39	1.58	A	0.167	2.708	32	0.825	1	7.397	2.19	109.68	C
199.84-179.84			B	0.167	2.708		0.825	1	7.397			
			C	0.179	2.669		0.825	1	7.911			
T7	0.61	2.31	A	0.225	2.515	31	0.825	1	11.001	3.06	153.01	C
179.84-159.84		TA 0.84	B	0.225	2.515		0.825	1	11.001			
			C	0.236	2.481		0.825	1	11.535			
T8	0.62	1.66	A	0.171	2.695	31	0.825	1	7.568	2.86	143.23	C
159.84-139.84			B	0.171	2.695		0.825	1	7.568			
			C	0.182	2.656		0.825	1	8.083			
T9	0.62	1.79	A	0.177	2.675	30	0.825	1	7.875	2.80	139.97	C
139.84-119.84			B	0.177	2.675		0.825	1	7.875			
			C	0.188	2.637		0.825	1	8.390			
T10	0.64	2.31	A	0.225	2.515	29	0.825	1	10.998	2.96	148.19	C
119.84-99.84		TA 0.84	B	0.225	2.515		0.825	1	10.998			
			C	0.236	2.481		0.825	1	11.533			
T11	0.70	2.10	A	0.19	2.63	27	0.825	1	8.529	2.93	146.48	C
99.84-79.84			B	0.19	2.63		0.825	1	8.529			
			C	0.201	2.593		0.825	1	9.047			
T12	0.86	2.02	A	0.186	2.643	26	0.825	1	8.353	3.23	161.40	C
79.84-59.84			B	0.186	2.643		0.825	1	8.353			
			C	0.197	2.606		0.825	1	8.869			
T13	0.86	2.38	A	0.226	2.511	24	0.825	1	11.116	3.14	156.92	C
59.84-39.84		TA 0.84	B	0.226	2.511		0.825	1	11.116			
			C	0.237	2.477		0.825	1	11.648			
T14	0.86	2.02	A	0.186	2.643	22	0.825	1	8.353	2.71	135.54	C
39.84-19.84			B	0.186	2.643		0.825	1	8.353			
			C	0.197	2.606		0.825	1	8.869			
T15	0.58	1.35	A	0.186	2.643	19	0.825	1	5.571	1.58	118.24	C
19.84-6.50			B	0.186	2.643		0.825	1	5.571			
			C	0.197	2.606		0.825	1	5.915			
T16 6.50-0.00	0.07	0.81	A	0.695	1.776	19	0.825	1	7.744	0.30	45.68	C
			B	0.695	1.776		0.825	1	7.744			
			C	0.695	1.776		0.825	1	7.744			
Sum Weight:	7.61	36.39								35.44		

### Tower Forces - No Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L1	0.00	3.14	A	1	0.6	36	1	1	31.497	0.91	25.86	C
327.00-291.84			B	1	0.6		1	1	31.497			



<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	39 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
ft	K	K										
T1 291.84-279.84	0.01	1.17 TA 0.84	C A B	1 0.227 0.227	0.6 2.508 2.508	35	1 0.8 0.8	1 1 1	31.497 6.832 6.832	0.57	47.14	C
T2 279.84-259.84	0.03	1.30	C A B C	0.24 0.152 0.152 0.164	2.469 2.764 2.764 2.722	35	0.8 0.8 0.8 0.8	1 1 1 1	7.191 6.627 6.627 7.140	0.73	36.26	C
T3 259.84-239.84	0.15	1.83 TA 0.84	C A B C	0.203 0.203 0.214	2.586 2.586 2.549	34	0.8 0.8 0.8	1 1 1	9.795 9.795 10.323	1.42	70.93	C
T4 239.84-219.84	0.31	1.47	C A B C	0.162 0.162 0.173	2.729 2.729 2.689	33	0.8 0.8 0.8	1 1 1	7.097 7.097 7.611	1.92	95.96	C
T5 219.84-199.84	0.31	2.10 TA 0.84	C A B C	0.216 0.216 0.227	2.544 2.544 2.508	33	0.8 0.8 0.8	1 1 1	10.442 10.442 10.974	2.12	106.03	C
T6 199.84-179.84	0.39	1.58	C A B C	0.167 0.167 0.179	2.708 2.708 2.669	32	0.8 0.8 0.8	1 1 1	7.397 7.397 7.911	2.19	109.68	C
T7 179.84-159.84	0.61	2.31 TA 0.84	C A B C	0.225 0.225 0.236	2.515 2.515 2.481	31	0.8 0.8 0.8	1 1 1	10.914 10.914 11.449	3.05	152.73	C
T8 159.84-139.84	0.62	1.66	C A B C	0.171 0.171 0.182	2.695 2.695 2.656	31	0.8 0.8 0.8	1 1 1	7.568 7.568 8.083	2.86	143.23	C
T9 139.84-119.84	0.62	1.79	C A B C	0.177 0.177 0.188	2.675 2.675 2.637	30	0.8 0.8 0.8	1 1 1	7.875 7.875 8.390	2.80	139.97	C
T10 119.84-99.84	0.64	2.31 TA 0.84	C A B C	0.225 0.225 0.236	2.515 2.515 2.481	29	0.8 0.8 0.8	1 1 1	10.912 10.912 11.447	2.96	147.93	C
T11 99.84-79.84	0.70	2.10	C A B C	0.19 0.19 0.201	2.63 2.63 2.593	27	0.8 0.8 0.8	1 1 1	8.529 8.529 9.047	2.93	146.48	C
T12 79.84-59.84	0.86	2.02	C A B C	0.186 0.186 0.197	2.643 2.643 2.606	26	0.8 0.8 0.8	1 1 1	8.353 8.353 8.869	3.23	161.40	C
T13 59.84-39.84	0.86	2.38 TA 0.84	C A B C	0.226 0.226 0.237	2.511 2.511 2.477	24	0.8 0.8 0.8	1 1 1	11.031 11.031 11.563	3.13	156.70	C
T14 39.84-19.84	0.86	2.02	C A B C	0.186 0.186 0.197	2.643 2.643 2.606	22	0.8 0.8 0.8	1 1 1	8.353 8.353 8.869	2.71	135.54	C
T15 19.84-6.50	0.58	1.35	C A B C	0.186 0.186 0.197	2.643 2.643 2.606	19	0.8 0.8 0.8	1 1 1	5.571 5.571 5.915	1.58	118.24	C
T16 6.50-0.00	0.07	0.81	C A B C	0.695 0.695 0.695	1.776 1.776 1.776	19	0.8 0.8 0.8	1 1 1	7.593 7.593 7.593	0.29	45.02	C
Sum Weight:	7.61	36.39								35.40		

**Tower Forces - No Ice - Wind 90 To Face**

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	40 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L1 327.00-291.84	0.00	3.14	A	1	0.6	36	1	1	31.497	0.91	25.86	C
			B	1	0.6		1	1	31.497			
			C	1	0.6		1	1	31.497			
T1 291.84-279.84	0.01	1.17	A	0.227	2.508	35	0.85	1	7.007	0.58	48.21	C
		TA 0.84	B	0.227	2.508		0.85	1	7.007			
			C	0.24	2.469		0.85	1	7.367			
T2 279.84-259.84	0.03	1.30	A	0.152	2.764	35	0.85	1	6.627	0.73	36.26	C
			B	0.152	2.764		0.85	1	6.627			
			C	0.164	2.722		0.85	1	7.140			
T3 259.84-239.84	0.15	1.83	A	0.203	2.586	34	0.85	1	9.969	1.43	71.57	C
		TA 0.84	B	0.203	2.586		0.85	1	9.969			
			C	0.214	2.549		0.85	1	10.497			
T4 239.84-219.84	0.31	1.47	A	0.162	2.729	33	0.85	1	7.097	1.92	95.96	C
			B	0.162	2.729		0.85	1	7.097			
			C	0.173	2.689		0.85	1	7.611			
T5 219.84-199.84	0.31	2.10	A	0.216	2.544	33	0.85	1	10.615	2.13	106.64	C
		TA 0.84	B	0.216	2.544		0.85	1	10.615			
			C	0.227	2.508		0.85	1	11.147			
T6 199.84-179.84	0.39	1.58	A	0.167	2.708	32	0.85	1	7.397	2.19	109.68	C
			B	0.167	2.708		0.85	1	7.397			
			C	0.179	2.669		0.85	1	7.911			
T7 179.84-159.84	0.61	2.31	A	0.225	2.515	31	0.85	1	11.087	3.07	153.30	C
		TA 0.84	B	0.225	2.515		0.85	1	11.087			
			C	0.236	2.481		0.85	1	11.621			
T8 159.84-139.84	0.62	1.66	A	0.171	2.695	31	0.85	1	7.568	2.86	143.23	C
			B	0.171	2.695		0.85	1	7.568			
			C	0.182	2.656		0.85	1	8.083			
T9 139.84-119.84	0.62	1.79	A	0.177	2.675	30	0.85	1	7.875	2.80	139.97	C
			B	0.177	2.675		0.85	1	7.875			
			C	0.188	2.637		0.85	1	8.390			
T10 119.84-99.84	0.64	2.31	A	0.225	2.515	29	0.85	1	11.084	2.97	148.45	C
		TA 0.84	B	0.225	2.515		0.85	1	11.084			
			C	0.236	2.481		0.85	1	11.619			
T11 99.84-79.84	0.70	2.10	A	0.19	2.63	27	0.85	1	8.529	2.93	146.48	C
			B	0.19	2.63		0.85	1	8.529			
			C	0.201	2.593		0.85	1	9.047			
T12 79.84-59.84	0.86	2.02	A	0.186	2.643	26	0.85	1	8.353	3.23	161.40	C
			B	0.186	2.643		0.85	1	8.353			
			C	0.197	2.606		0.85	1	8.869			
T13 59.84-39.84	0.86	2.38	A	0.226	2.511	24	0.85	1	11.202	3.14	157.14	C
		TA 0.84	B	0.226	2.511		0.85	1	11.202			
			C	0.237	2.477		0.85	1	11.734			
T14 39.84-19.84	0.86	2.02	A	0.186	2.643	22	0.85	1	8.353	2.71	135.54	C
			B	0.186	2.643		0.85	1	8.353			
			C	0.197	2.606		0.85	1	8.869			
T15 19.84-6.50	0.58	1.35	A	0.186	2.643	19	0.85	1	5.571	1.58	118.24	C
			B	0.186	2.643		0.85	1	5.571			
			C	0.197	2.606		0.85	1	5.915			
T16 6.50-0.00	0.07	0.81	A	0.695	1.776	19	0.85	1	7.895	0.30	46.34	C
			B	0.695	1.776		0.85	1	7.895			
			C	0.695	1.776		0.85	1	7.895			
Sum Weight:	7.61	36.39								35.48		

## Tower Forces - With Ice - Wind Normal To Face

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	41 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
ft	K	K										
L1 327.00-291.84	0.00	4.56	A	1	1.2	6	1	1	46.158	0.42	11.89	C
			B	1	1.2		1	1	46.158			
			C	1	1.2		1	1	46.158			
T1 291.84-279.84	0.25	3.25	A	0.66	1.779	5	1	1	27.127	0.28	23.67	C
		TA 1.94	B	0.66	1.779		1	1	27.127			
			C	0.728	1.78		1	1	31.478			
T2 279.84-259.84	0.96	3.95	A	0.57	1.825	5	1	1	35.139	0.44	22.17	C
			B	0.57	1.825		1	1	35.139			
			C	0.631	1.788		1	1	40.989			
T3 259.84-239.84	2.94	4.87	A	0.61	1.798	5	1	1	39.927	0.65	32.30	C
		TA 1.92	B	0.61	1.798		1	1	39.927			
			C	0.67	1.777		1	1	45.933			
T4 239.84-219.84	5.56	4.09	A	0.569	1.826	5	1	1	35.189	0.80*	39.94	C
			B	0.569	1.826		1	1	35.189			
			C	0.629	1.789		1	1	40.914			
T5 219.84-199.84	5.73	5.14	A	0.612	1.797	5	1	1	40.229	0.79*	39.34	C
		TA 1.90	B	0.612	1.797		1	1	40.229			
			C	0.671	1.777		1	1	46.125			
T6 199.84-179.84	6.91	4.14	A	0.564	1.83	5	1	1	34.831	0.77*	38.49	C
			B	0.564	1.83		1	1	34.831			
			C	0.623	1.792		1	1	40.380			
T7 179.84-159.84	9.58	5.30	A	0.609	1.798	5	1	1	40.080	0.75*	37.74	C
		TA 1.87	B	0.609	1.798		1	1	40.080			
			C	0.667	1.778		1	1	45.809			
T8 159.84-139.84	9.97	4.15	A	0.559	1.835	5	1	1	34.285	0.73*	36.54	C
			B	0.559	1.835		1	1	34.285			
			C	0.617	1.795		1	1	39.678			
T9 139.84-119.84	9.83	4.24	A	0.557	1.837	5	1	1	34.170	0.71*	35.58	C
			B	0.557	1.837		1	1	34.170			
			C	0.613	1.797		1	1	39.440			
T10 119.84-99.84	10.15	5.11	A	0.594	1.808	4	1	1	38.500	0.69*	34.30	C
		TA 1.82	B	0.594	1.808		1	1	38.500			
			C	0.65	1.782		1	1	43.891			
T11 99.84-79.84	10.91	4.48	A	0.553	1.841	4	1	1	33.896	0.66*	32.98	C
			B	0.553	1.841		1	1	33.896			
			C	0.607	1.8		1	1	38.940			
T12 79.84-59.84	12.50	4.28	A	0.541	1.852	4	1	1	32.779	0.62*	31.21	C
			B	0.541	1.852		1	1	32.779			
			C	0.594	1.808		1	1	37.624			
T13 59.84-39.84	12.08	4.84	A	0.567	1.828	4	1	1	35.942	0.58*	28.99	C
		TA 1.74	B	0.567	1.828		1	1	35.942			
			C	0.619	1.794		1	1	40.741			
T14 39.84-19.84	11.36	4.00	A	0.514	1.882	3	1	1	30.288	0.52*	25.91	C
			B	0.514	1.882		1	1	30.288			
			C	0.564	1.831		1	1	34.610			
T15 19.84-6.50	6.98	2.51	A	0.49	1.913	3	1	1	18.783	0.30*	22.31	C
			B	0.49	1.913		1	1	18.783			
			C	0.537	1.856		1	1	21.372			
T16 6.50-0.00	0.68	1.55	A	0.967	2.035	3	1	1	14.900	0.08	12.36	C
			B	0.967	2.035		1	1	14.900			
			C	0.967	2.035		1	1	14.900			
Sum Weight:	116.39	81.63			*2.1A <sub>g</sub> limit					9.79		

## Tower Forces - With Ice - Wind 45 To Face

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>  17004.42 - CT1077	<b>Page</b>  42 of 95
	<b>Project</b>  327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>  15:25:46 09/11/17
	<b>Client</b>  AT&T	<b>Designed by</b>  TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
ft	K	K										
L1 327.00-291.84	0.00	4.56	A	1	1.2	6	1	1	46.158	0.42	11.89	C
			B	1	1.2		1	1	46.158			
			C	1	1.2		1	1	46.158			
T1 291.84-279.84	0.25	3.25	A	0.66	1.779	5	0.825	1	26.514	0.28	23.24	C
		TA 1.94	B	0.66	1.779		0.825	1	26.514			
			C	0.728	1.78		0.825	1	30.865			
T2 279.84-259.84	0.96	3.95	A	0.57	1.825	5	0.825	1	35.139	0.44	22.17	C
			B	0.57	1.825		0.825	1	35.139			
			C	0.631	1.788		0.825	1	40.989			
T3 259.84-239.84	2.94	4.87	A	0.61	1.798	5	0.825	1	39.318	0.64	32.06	C
		TA 1.92	B	0.61	1.798		0.825	1	39.318			
			C	0.67	1.777		0.825	1	45.323			
T4 239.84-219.84	5.56	4.09	A	0.569	1.826	5	0.825	1	35.189	0.80*	39.94	C
			B	0.569	1.826		0.825	1	35.189			
			C	0.629	1.789		0.825	1	40.914			
T5 219.84-199.84	5.73	5.14	A	0.612	1.797	5	0.825	1	39.623	0.79*	39.34	C
		TA 1.90	B	0.612	1.797		0.825	1	39.623			
			C	0.671	1.777		0.825	1	45.519			
T6 199.84-179.84	6.91	4.14	A	0.564	1.83	5	0.825	1	34.831	0.77*	38.49	C
			B	0.564	1.83		0.825	1	34.831			
			C	0.623	1.792		0.825	1	40.380			
T7 179.84-159.84	9.58	5.30	A	0.609	1.798	5	0.825	1	39.478	0.75*	37.74	C
		TA 1.87	B	0.609	1.798		0.825	1	39.478			
			C	0.667	1.778		0.825	1	45.207			
T8 159.84-139.84	9.97	4.15	A	0.559	1.835	5	0.825	1	34.285	0.73*	36.54	C
			B	0.559	1.835		0.825	1	34.285			
			C	0.617	1.795		0.825	1	39.678			
T9 139.84-119.84	9.83	4.24	A	0.557	1.837	5	0.825	1	34.170	0.71*	35.58	C
			B	0.557	1.837		0.825	1	34.170			
			C	0.613	1.797		0.825	1	39.440			
T10 119.84-99.84	10.15	5.11	A	0.594	1.808	4	0.825	1	37.898	0.69*	34.30	C
		TA 1.82	B	0.594	1.808		0.825	1	37.898			
			C	0.65	1.782		0.825	1	43.289			
T11 99.84-79.84	10.91	4.48	A	0.553	1.841	4	0.825	1	33.896	0.66*	32.98	C
			B	0.553	1.841		0.825	1	33.896			
			C	0.607	1.8		0.825	1	38.940			
T12 79.84-59.84	12.50	4.28	A	0.541	1.852	4	0.825	1	32.779	0.62*	31.21	C
			B	0.541	1.852		0.825	1	32.779			
			C	0.594	1.808		0.825	1	37.624			
T13 59.84-39.84	12.08	4.84	A	0.567	1.828	4	0.825	1	35.344	0.58*	28.99	C
		TA 1.74	B	0.567	1.828		0.825	1	35.344			
			C	0.619	1.794		0.825	1	40.142			
T14 39.84-19.84	11.36	4.00	A	0.514	1.882	3	0.825	1	30.288	0.52*	25.91	C
			B	0.514	1.882		0.825	1	30.288			
			C	0.564	1.831		0.825	1	34.610			
T15 19.84-6.50	6.98	2.51	A	0.49	1.913	3	0.825	1	18.783	0.30*	22.31	C
			B	0.49	1.913		0.825	1	18.783			
			C	0.537	1.856		0.825	1	21.372			
T16 6.50-0.00	0.68	1.55	A	0.967	2.035	3	0.825	1	13.843	0.07	11.53	C
			B	0.967	2.035		0.825	1	13.843			
			C	0.967	2.035		0.825	1	13.843			
Sum Weight:	116.39	81.63			*2.1A <sub>g</sub> limit					9.77		

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	Job	17004.42 - CT1077	Page	43 of 95
	Project	327' Guyed Tower - N. Eagleville Road Storrs, CT	Date	15:25:46 09/11/17
	Client	AT&T	Designed by	TJL

## Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
L1 327.00-291.84	0.00	4.56	A	1	1.2	6	1	1	46.158	0.42	11.89	C
			B	1	1.2		1	1	46.158			
			C	1	1.2		1	1	46.158			
T1 291.84-279.84	0.25	3.25	A	0.66	1.779	5	0.8	1	26.426	0.28	23.18	C
		TA 1.94	B	0.66	1.779		0.8	1	26.426			
			C	0.728	1.78		0.8	1	30.777			
T2 279.84-259.84	0.96	3.95	A	0.57	1.825	5	0.8	1	35.139	0.44	22.17	C
			B	0.57	1.825		0.8	1	35.139			
			C	0.631	1.788		0.8	1	40.989			
T3 259.84-239.84	2.94	4.87	A	0.61	1.798	5	0.8	1	39.231	0.64	32.02	C
		TA 1.92	B	0.61	1.798		0.8	1	39.231			
			C	0.67	1.777		0.8	1	45.236			
T4 239.84-219.84	5.56	4.09	A	0.569	1.826	5	0.8	1	35.189	0.80*	39.94	C
			B	0.569	1.826		0.8	1	35.189			
			C	0.629	1.789		0.8	1	40.914			
T5 219.84-199.84	5.73	5.14	A	0.612	1.797	5	0.8	1	39.537	0.79*	39.34	C
		TA 1.90	B	0.612	1.797		0.8	1	39.537			
			C	0.671	1.777		0.8	1	45.432			
T6 199.84-179.84	6.91	4.14	A	0.564	1.83	5	0.8	1	34.831	0.77*	38.49	C
			B	0.564	1.83		0.8	1	34.831			
			C	0.623	1.792		0.8	1	40.380			
T7 179.84-159.84	9.58	5.30	A	0.609	1.798	5	0.8	1	39.392	0.75*	37.74	C
		TA 1.87	B	0.609	1.798		0.8	1	39.392			
			C	0.667	1.778		0.8	1	45.121			
T8 159.84-139.84	9.97	4.15	A	0.559	1.835	5	0.8	1	34.285	0.73*	36.54	C
			B	0.559	1.835		0.8	1	34.285			
			C	0.617	1.795		0.8	1	39.678			
T9 139.84-119.84	9.83	4.24	A	0.557	1.837	5	0.8	1	34.170	0.71*	35.58	C
			B	0.557	1.837		0.8	1	34.170			
			C	0.613	1.797		0.8	1	39.440			
T10 119.84-99.84	10.15	5.11	A	0.594	1.808	4	0.8	1	37.812	0.69*	34.30	C
		TA 1.82	B	0.594	1.808		0.8	1	37.812			
			C	0.65	1.782		0.8	1	43.203			
T11 99.84-79.84	10.91	4.48	A	0.553	1.841	4	0.8	1	33.896	0.66*	32.98	C
			B	0.553	1.841		0.8	1	33.896			
			C	0.607	1.8		0.8	1	38.940			
T12 79.84-59.84	12.50	4.28	A	0.541	1.852	4	0.8	1	32.779	0.62*	31.21	C
			B	0.541	1.852		0.8	1	32.779			
			C	0.594	1.808		0.8	1	37.624			
T13 59.84-39.84	12.08	4.84	A	0.567	1.828	4	0.8	1	35.258	0.58*	28.99	C
		TA 1.74	B	0.567	1.828		0.8	1	35.258			
			C	0.619	1.794		0.8	1	40.057			
T14 39.84-19.84	11.36	4.00	A	0.514	1.882	3	0.8	1	30.288	0.52*	25.91	C
			B	0.514	1.882		0.8	1	30.288			
			C	0.564	1.831		0.8	1	34.610			
T15 19.84-6.50	6.98	2.51	A	0.49	1.913	3	0.8	1	18.783	0.30*	22.31	C
			B	0.49	1.913		0.8	1	18.783			
			C	0.537	1.856		0.8	1	21.372			
T16 6.50-0.00	0.68	1.55	A	0.967	2.035	3	0.8	1	13.692	0.07	11.41	C
			B	0.967	2.035		0.8	1	13.692			
			C	0.967	2.035		0.8	1	13.692			
Sum Weight:	116.39	81.63			*2.1A <sub>g</sub> limit					9.77		

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 17004.42 - CT1077	<b>Page</b> 44 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

## Tower Forces - With Ice - Wind 90 To Face

Section Elevation  ft	Add Weight  K	Self Weight  K	F a c e	e	C <sub>F</sub>	q <sub>z</sub>  psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>  ft <sup>2</sup>	F  K	w  plf	Ctrl. Face
L1 327.00-291.84	0.00	4.56	A B C	1 1 1	1.2 1.2 1.2	6	1 1 1	1 1 1	46.158 46.158 46.158	0.42	11.89	C
T1 291.84-279.84	0.25	3.25 TA 1.94	A B C	0.66 0.66 0.728	1.779 1.779 1.78	5	0.85 0.85 0.85	1 1 1	26.601 26.601 30.953	0.28	23.30	C
T2 279.84-259.84	0.96	3.95	A B C	0.57 0.57 0.631	1.825 1.825 1.788	5	0.85 0.85 0.85	1 1 1	35.139 35.139 40.989	0.44	22.17	C
T3 259.84-239.84	2.94	4.87 TA 1.92	A B C	0.61 0.61 0.67	1.798 1.798 1.777	5	0.85 0.85 0.85	1 1 1	39.405 39.405 45.410	0.64	32.09	C
T4 239.84-219.84	5.56	4.09	A B C	0.569 0.569 0.629	1.826 1.826 1.789	5	0.85 0.85 0.85	1 1 1	35.189 35.189 40.914	0.80*	39.94	C
T5 219.84-199.84	5.73	5.14 TA 1.90	A B C	0.612 0.612 0.671	1.797 1.797 1.777	5	0.85 0.85 0.85	1 1 1	39.710 39.710 45.605	0.79*	39.34	C
T6 199.84-179.84	6.91	4.14	A B C	0.564 0.564 0.623	1.83 1.83 1.792	5	0.85 0.85 0.85	1 1 1	34.831 34.831 40.380	0.77*	38.49	C
T7 179.84-159.84	9.58	5.30 TA 1.87	A B C	0.609 0.609 0.667	1.798 1.798 1.778	5	0.85 0.85 0.85	1 1 1	39.564 39.564 45.293	0.75*	37.74	C
T8 159.84-139.84	9.97	4.15	A B C	0.559 0.559 0.617	1.835 1.835 1.795	5	0.85 0.85 0.85	1 1 1	34.285 34.285 39.678	0.73*	36.54	C
T9 139.84-119.84	9.83	4.24	A B C	0.557 0.557 0.613	1.837 1.837 1.797	5	0.85 0.85 0.85	1 1 1	34.170 34.170 39.440	0.71*	35.58	C
T10 119.84-99.84	10.15	5.11 TA 1.82	A B C	0.594 0.594 0.65	1.808 1.808 1.782	4	0.85 0.85 0.85	1 1 1	37.984 37.984 43.375	0.69*	34.30	C
T11 99.84-79.84	10.91	4.48	A B C	0.553 0.553 0.607	1.841 1.841 1.8	4	0.85 0.85 0.85	1 1 1	33.896 33.896 38.940	0.66*	32.98	C
T12 79.84-59.84	12.50	4.28	A B C	0.541 0.541 0.594	1.852 1.852 1.808	4	0.85 0.85 0.85	1 1 1	32.779 32.779 37.624	0.62*	31.21	C
T13 59.84-39.84	12.08	4.84 TA 1.74	A B C	0.567 0.567 0.619	1.828 1.828 1.794	4	0.85 0.85 0.85	1 1 1	35.429 35.429 40.228	0.58*	28.99	C
T14 39.84-19.84	11.36	4.00	A B C	0.514 0.514 0.564	1.882 1.882 1.831	3	0.85 0.85 0.85	1 1 1	30.288 30.288 34.610	0.52*	25.91	C
T15 19.84-6.50	6.98	2.51	A B C	0.49 0.49 0.537	1.913 1.913 1.856	3	0.85 0.85 0.85	1 1 1	18.783 18.783 21.372	0.30*	22.31	C
T16 6.50-0.00	0.68	1.55	A B C	0.967 0.967 0.967	2.035 2.035 2.035	3	0.85 0.85 0.85	1 1 1	13.994 13.994 13.994	0.08	11.65	C
Sum Weight:	116.39	81.63			*2.1A <sub>g</sub> limit					9.78		



<p><b><i>tnxTower</i></b></p> <p><b><i>Centek Engineering Inc.</i></b>  <i>63-2 North Branford Rd.</i>  <i>Branford, CT 06405</i>  <i>Phone: (203) 488-0580</i>  <i>FAX: (203) 488-8587</i></p>	<p><b>Job</b></p> <p>17004.42 - CT1077</p>	<p><b>Page</b></p> <p>46 of 95</p>
	<p><b>Project</b></p> <p>327' Guyed Tower - N. Eagleville Road Storrs, CT</p>	<p><b>Date</b></p> <p>15:25:46 09/11/17</p>
	<p><b>Client</b></p> <p>AT&amp;T</p>	<p><b>Designed by</b></p> <p>TJL</p>

### Tower Forces - Service - Wind 45 To Face







<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>  17004.42 - CT1077	<b>Page</b>  49 of 95
	<b>Project</b>  327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>  15:25:46 09/11/17
	<b>Client</b>  AT&T	<b>Designed by</b>  TJL

### Force Totals (Does not include forces on guys)

Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Torques kip-ft
Leg Weight	19.66			
Bracing Weight	16.73			
Total Member Self-Weight	36.39			
Guy Weight	12.85			
Total Weight	66.51			
Wind 0 deg - No Ice		0.52	-53.36	-5.72
Wind 30 deg - No Ice		26.53	-45.71	-3.43
Wind 45 deg - No Ice		37.43	-37.46	-1.90
Wind 60 deg - No Ice		45.63	-26.51	-0.68
Wind 90 deg - No Ice		52.75	-0.20	2.52
Wind 120 deg - No Ice		45.61	26.79	6.42
Wind 135 deg - No Ice		37.02	37.61	7.76
Wind 150 deg - No Ice		25.90	45.72	8.71
Wind 180 deg - No Ice		-0.20	52.47	6.69
Wind 210 deg - No Ice		-26.17	45.85	2.76
Wind 225 deg - No Ice		-37.09	38.07	1.30
Wind 240 deg - No Ice		-45.42	27.29	-0.74
Wind 270 deg - No Ice		-52.36	0.26	-3.35
Wind 300 deg - No Ice		-44.86	-25.84	-6.02
Wind 315 deg - No Ice		-36.85	-36.89	-7.03
Wind 330 deg - No Ice		-25.96	-45.31	-7.21
Member Ice	45.25			
Guy Ice	87.39			
Total Weight Ice	338.98			
Wind 0 deg - Ice		0.09	-14.20	-1.25
Wind 30 deg - Ice		7.07	-12.18	-0.77
Wind 45 deg - Ice		10.01	-10.00	-0.41
Wind 60 deg - Ice		12.22	-7.09	-0.09
Wind 90 deg - Ice		14.11	-0.03	0.65
Wind 120 deg - Ice		12.12	7.20	1.59
Wind 135 deg - Ice		9.84	10.07	1.87
Wind 150 deg - Ice		6.91	12.25	2.02
Wind 180 deg - Ice		-0.04	14.07	1.44
Wind 210 deg - Ice		-6.96	12.28	0.45
Wind 225 deg - Ice		-9.91	10.21	0.09
Wind 240 deg - Ice		-12.08	7.29	-0.33
Wind 270 deg - Ice		-14.04	0.05	-0.80
Wind 300 deg - Ice		-12.04	-6.94	-1.35
Wind 315 deg - Ice		-9.85	-9.84	-1.52
Wind 330 deg - Ice		-6.96	-12.10	-1.55
Total Weight	66.51			
Wind 0 deg - Service		0.18	-18.83	-2.02
Wind 30 deg - Service		9.36	-16.13	-1.21
Wind 45 deg - Service		13.21	-13.22	-0.67
Wind 60 deg - Service		16.10	-9.36	-0.24
Wind 90 deg - Service		18.62	-0.07	0.89
Wind 120 deg - Service		16.09	9.46	2.27
Wind 135 deg - Service		13.07	13.27	2.74
Wind 150 deg - Service		9.14	16.14	3.07
Wind 180 deg - Service		-0.07	18.52	2.36
Wind 210 deg - Service		-9.23	16.18	0.97
Wind 225 deg - Service		-13.09	13.44	0.46
Wind 240 deg - Service		-16.03	9.63	-0.26
Wind 270 deg - Service		-18.48	0.09	-1.18

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>  17004.42 - CT1077	<b>Page</b>  50 of 95
	<b>Project</b>  327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>  15:25:46 09/11/17
	<b>Client</b>  AT&T	<b>Designed by</b>  TJL

<i>Load Case</i>	<i>Vertical Forces</i>	<i>Sum of Forces X K</i>	<i>Sum of Forces Z K</i>	<i>Sum of Torques kip-ft</i>
Wind 300 deg - Service		-15.83	-9.12	-2.13
Wind 315 deg - Service		-13.00	-13.02	-2.48
Wind 330 deg - Service		-9.16	-15.99	-2.54

## Load Combinations

<i>Comb. No.</i>	<i>Description</i>
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice+1.0 Guy
3	1.2 Dead+1.6 Wind 30 deg - No Ice+1.0 Guy
4	1.2 Dead+1.6 Wind 45 deg - No Ice+1.0 Guy
5	1.2 Dead+1.6 Wind 60 deg - No Ice+1.0 Guy
6	1.2 Dead+1.6 Wind 90 deg - No Ice+1.0 Guy
7	1.2 Dead+1.6 Wind 120 deg - No Ice+1.0 Guy
8	1.2 Dead+1.6 Wind 135 deg - No Ice+1.0 Guy
9	1.2 Dead+1.6 Wind 150 deg - No Ice+1.0 Guy
10	1.2 Dead+1.6 Wind 180 deg - No Ice+1.0 Guy
11	1.2 Dead+1.6 Wind 210 deg - No Ice+1.0 Guy
12	1.2 Dead+1.6 Wind 225 deg - No Ice+1.0 Guy
13	1.2 Dead+1.6 Wind 240 deg - No Ice+1.0 Guy
14	1.2 Dead+1.6 Wind 270 deg - No Ice+1.0 Guy
15	1.2 Dead+1.6 Wind 300 deg - No Ice+1.0 Guy
16	1.2 Dead+1.6 Wind 315 deg - No Ice+1.0 Guy
17	1.2 Dead+1.6 Wind 330 deg - No Ice+1.0 Guy
18	1.2 Dead+1.0 Ice+1.0 Temp+Guy
19	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp+1.0 Guy
20	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp+1.0 Guy
21	1.2 Dead+1.0 Wind 45 deg+1.0 Ice+1.0 Temp+1.0 Guy
22	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp+1.0 Guy
23	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp+1.0 Guy
24	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp+1.0 Guy
25	1.2 Dead+1.0 Wind 135 deg+1.0 Ice+1.0 Temp+1.0 Guy
26	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp+1.0 Guy
27	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp+1.0 Guy
28	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp+1.0 Guy
29	1.2 Dead+1.0 Wind 225 deg+1.0 Ice+1.0 Temp+1.0 Guy
30	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp+1.0 Guy
31	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp+1.0 Guy
32	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp+1.0 Guy
33	1.2 Dead+1.0 Wind 315 deg+1.0 Ice+1.0 Temp+1.0 Guy
34	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp+1.0 Guy
35	Dead+ Wind 0 deg - Service+Guy
36	Dead+ Wind 30 deg - Service+Guy
37	Dead+ Wind 45 deg - Service+Guy
38	Dead+ Wind 60 deg - Service+Guy
39	Dead+ Wind 90 deg - Service+Guy
40	Dead+ Wind 120 deg - Service+Guy
41	Dead+ Wind 135 deg - Service+Guy
42	Dead+ Wind 150 deg - Service+Guy
43	Dead+ Wind 180 deg - Service+Guy
44	Dead+ Wind 210 deg - Service+Guy
45	Dead+ Wind 225 deg - Service+Guy
46	Dead+ Wind 240 deg - Service+Guy

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	51 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Comb. No.</i>	<i>Description</i>
47	Dead+Wind 270 deg - Service+Guy
48	Dead+Wind 300 deg - Service+Guy
49	Dead+Wind 315 deg - Service+Guy
50	Dead+Wind 330 deg - Service+Guy

## Maximum Member Forces

<i>Section No.</i>	<i>Elevation ft</i>	<i>Component Type</i>	<i>Condition</i>	<i>Gov. Load Comb.</i>	<i>Axial K</i>	<i>Major Axis Moment kip-ft</i>	<i>Minor Axis Moment kip-ft</i>
L1	327 - 291.84	Pole	Max Tension	15	0.00	0.01	-0.02
			Max. Compression	24	-6.02	-7.31	-5.72
			Max. Mx	14	-3.95	38.84	-0.01
			Max. My	10	-3.95	0.49	-38.82
			Max. Vy	14	-2.08	38.84	-0.01
			Max. Vx	10	2.09	0.49	-38.82
			Max. Torque	17			0.93
T1	291.84 - 279.84	Leg	Max Tension	15	15.82	-0.35	-0.18
			Max. Compression	30	-30.91	-0.40	0.41
			Max. Mx	23	-0.12	-2.35	1.25
			Max. My	27	1.09	0.01	-2.71
			Max. Vy	31	-1.03	0.67	0.19
			Max. Vx	27	1.16	0.01	-2.71
		Diagonal	Max Tension	12	3.95	0.00	0.00
			Max. Compression	14	-3.91	0.00	0.00
			Max. Mx	19	0.01	0.04	0.00
			Max. My	26	0.01	0.00	-0.00
			Max. Vy	19	-0.03	0.00	0.00
			Max. Vx	26	0.00	0.00	0.00
		Horizontal	Max Tension	17	1.00	0.00	0.00
			Max. Compression	23	-0.67	0.00	0.00
			Max. Mx	24	-0.54	0.02	0.00
			Max. My	16	0.99	0.00	0.00
			Max. Vy	24	0.03	0.00	0.00
			Max. Vx	16	-0.00	0.00	0.00
		Secondary Horizontal	Max Tension	30	0.00	-0.01	-0.00
			Max. Compression	24	-0.00	-0.01	-0.00
			Max. Mx	23	-0.00	-0.01	-0.00
			Max. My	16	-0.00	-0.00	-0.00
			Max. Vy	23	0.02	-0.01	-0.00
			Max. Vx	16	0.00	-0.00	-0.00
		Top Girt	Max Tension	19	0.00	0.00	0.00
			Max. Compression	19	-0.00	0.00	0.00
			Max. Mx	25	0.00	0.02	0.00
			Max. My	34	0.00	0.00	0.00
			Max. Vy	25	0.03	0.00	0.00
			Max. Vx	34	0.00	0.00	0.00
		Guy A	Bottom Tension	27	13.93		
			Top Tension	10	16.59		
			Top Cable Vert	27	14.06		
			Top Cable Norm	27	8.80		
			Top Cable Tan	27	0.00		
			Bot Cable Vert	10	-10.81		
			Bot Cable Norm	10	8.80		
			Bot Cable Tan	10	0.01		
		Guy B	Bottom Tension	32	13.51		
			Top Tension	15	15.62		

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	52 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T2	279.84 - 259.84	Guy C	Top Cable Vert	32	12.82		
			Top Cable Norm	32	8.92		
			Top Cable Tan	32	0.01		
			Bot Cable Vert	15	-10.02		
			Bot Cable Norm	15	9.05		
			Bot Cable Tan	15	0.01		
			Bottom Tension	22	13.81		
			Top Tension	5	16.47		
			Top Cable Vert	22	13.92		
			Top Cable Norm	22	8.80		
			Top Cable Tan	22	0.01		
			Bot Cable Vert	5	-10.67		
			Bot Cable Norm	5	8.78		
			Bot Cable Tan	5	0.01		
		Top Guy Pull-Off	Max Tension	17	4.78	0.00	0.00
			Max. Compression	9	-4.18	0.00	0.00
			Max. Mx	24	-0.55	0.15	0.00
			Max. My	16	-1.15	0.00	0.00
			Max. Vy	24	-0.16	0.00	0.00
		Torque Arm Top	Max. Vx	16	-0.00	0.00	0.00
			Max Tension	14	7.35	0.00	0.00
			Max. Compression	6	-1.30	0.00	0.00
			Max. Mx	27	3.21	-56.45	0.00
			Max. My	9	0.99	-43.24	0.00
		Leg	Max. Vy	27	14.27	-56.45	0.00
			Max. Vx	9	0.00	-43.24	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	13	-36.55	0.11	-0.12
			Max. Mx	31	-29.58	-0.52	-0.10
		Diagonal	Max. My	27	-29.66	0.17	0.51
			Max. Vy	15	-0.33	0.13	-0.06
			Max. Vx	17	-0.45	0.05	0.18
			Max Tension	12	2.61	0.00	0.00
			Max. Compression	2	-2.94	0.00	0.00
		Horizontal	Max. Mx	30	-0.39	0.04	0.00
			Max. My	26	-0.16	0.00	-0.00
			Max. Vy	30	0.03	0.00	0.00
			Max. Vx	26	0.00	0.00	0.00
			Max Tension	16	0.60	0.00	0.00
		Secondary Horizontal	Max. Compression	8	-0.46	0.00	0.00
			Max. Mx	29	0.24	0.02	0.00
			Max. My	25	0.07	0.00	0.00
			Max. Vy	29	-0.03	0.00	0.00
			Max. Vx	25	-0.00	0.00	0.00
		Top Girt	Max Tension	30	0.00	-0.01	-0.00
			Max. Compression	24	-0.00	-0.01	-0.00
			Max. Mx	23	-0.00	-0.01	0.00
			Max. My	19	-0.00	-0.01	0.00
			Max. Vy	23	0.02	-0.01	0.00
T3	259.84 - 239.84	Leg	Max. Vx	19	-0.00	0.00	0.00
			Max Tension	25	0.40	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	30	0.38	0.02	0.00
			Max. My	16	0.17	0.00	0.00
		Leg	Max. Vy	30	-0.03	0.00	0.00
			Max. Vx	16	-0.00	0.00	0.00
			Max Tension	5	1.75	0.26	-0.31
			Max. Compression	26	-61.43	0.39	-0.02

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	53 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
			Max. Mx	23	-57.14	2.42	-0.87
			Max. My	27	-57.18	-0.36	2.57
			Max. Vy	22	0.99	-0.80	-0.00
			Max. Vx	27	1.01	0.37	-0.69
		Diagonal	Max Tension	9	3.77	0.00	0.00
			Max. Compression	17	-4.50	0.00	0.00
			Max. Mx	30	0.62	0.04	0.00
			Max. My	34	-0.04	0.00	0.00
			Max. Vy	30	-0.03	0.00	0.00
			Max. Vx	34	0.00	0.00	0.00
		Horizontal	Max Tension	10	0.71	0.00	0.00
			Max. Compression	9	-0.95	0.00	0.00
			Max. Mx	18	0.32	0.02	0.00
			Max. My	34	-0.10	0.00	0.00
			Max. Vy	18	0.02	0.00	0.00
			Max. Vx	34	0.00	0.00	0.00
		Secondary Horizontal	Max Tension	30	0.00	-0.01	-0.00
			Max. Compression	24	-0.00	-0.01	-0.00
			Max. Mx	23	-0.00	-0.01	0.00
			Max. My	19	-0.00	-0.01	0.00
			Max. Vy	23	0.02	-0.01	0.00
			Max. Vx	19	-0.00	0.00	0.00
		Top Girt	Max Tension	21	0.80	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	29	0.78	0.02	0.00
			Max. My	34	0.75	0.00	0.00
			Max. Vy	29	0.02	0.00	0.00
			Max. Vx	34	0.00	0.00	0.00
		Guy A	Bottom Tension	27	14.12		
			Top Tension	10	16.31		
			Top Cable Vert	27	13.36		
			Top Cable Norm	27	9.35		
			Top Cable Tan	27	0.00		
			Bot Cable Vert	10	-10.54		
			Bot Cable Norm	10	9.38		
			Bot Cable Tan	10	0.01		
		Guy B	Bottom Tension	32	13.51		
			Top Tension	15	15.26		
			Top Cable Vert	32	12.00		
			Top Cable Norm	32	9.42		
			Top Cable Tan	32	0.00		
			Bot Cable Vert	15	-9.55		
			Bot Cable Norm	15	9.56		
			Bot Cable Tan	15	0.01		
		Guy C	Bottom Tension	22	14.00		
			Top Tension	5	16.17		
			Top Cable Vert	22	13.21		
			Top Cable Norm	22	9.34		
			Top Cable Tan	22	0.01		
			Bot Cable Vert	5	-10.41		
			Bot Cable Norm	5	9.37		
			Bot Cable Tan	5	0.01		
		Top Guy Pull-Off	Max Tension	9	5.90	0.00	0.00
			Max. Compression	17	-5.09	0.00	0.00
			Max. Mx	29	-0.63	0.15	0.00
			Max. My	8	5.66	0.00	-0.00
			Max. Vy	29	-0.16	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
		Torque Arm Top	Max Tension	14	8.30	-14.38	0.00
			Max. Compression	8	-2.30	-37.69	-0.00

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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T4	239.84 - 219.84	Leg	Max. Mx	27	3.43	-53.56	0.00
			Max. My	9	0.82	-41.33	0.00
			Max. Vy	27	13.55	-53.56	0.00
			Max. Vx	9	0.00	-41.33	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	19	-63.52	0.32	-0.28
		Diagonal	Max. Mx	31	-62.42	-0.63	0.15
			Max. My	19	-61.98	0.18	-0.61
			Max. Vy	31	0.35	0.34	-0.29
			Max. Vx	19	0.32	0.10	0.42
			Max Tension	5	2.77	0.00	0.00
			Max. Compression	10	-3.39	0.00	0.00
		Horizontal	Max. Mx	19	-0.24	0.04	0.00
			Max. My	8	-0.90	0.00	-0.00
			Max. Vy	19	0.03	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
			Max Tension	5	0.49	0.00	0.00
			Max. Compression	12	-0.11	0.00	0.00
		Secondary Horizontal	Max. Mx	25	0.45	0.02	0.00
			Max. My	8	-0.09	0.00	-0.00
			Max. Vy	25	-0.02	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
			Max Tension	30	0.00	-0.01	-0.00
			Max. Compression	24	-0.00	-0.01	-0.00
		Top Girt	Max. Mx	23	-0.00	-0.01	0.00
			Max. My	4	0.00	-0.00	0.00
			Max. Vy	23	0.02	-0.01	0.00
			Max. Vx	4	-0.00	-0.00	0.00
			Max Tension	5	0.65	0.00	0.00
			Max. Compression	13	-0.22	0.00	0.00
T5	219.84 - 199.84	Leg	Max. Mx	32	0.30	0.02	0.00
			Max. My	34	0.45	0.00	0.00
			Max. Vy	32	-0.02	0.00	0.00
			Max. Vx	34	-0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	21	-93.71	-0.36	-0.60
		Diagonal	Max. Mx	23	-86.49	2.41	-0.47
			Max. My	27	-87.27	-0.64	2.37
			Max. Vy	23	1.00	-0.90	-0.32
			Max. Vx	27	0.91	0.69	-0.63
			Max Tension	3	7.18	0.00	0.00
			Max. Compression	11	-8.44	0.00	0.00
		Horizontal	Max. Mx	19	0.49	0.04	0.00
			Max. My	8	1.14	0.00	-0.00
			Max. Vy	19	-0.03	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
			Max Tension	34	0.89	0.00	0.00
			Max. Compression	2	-0.47	0.00	0.00
		Secondary Horizontal	Max. Mx	25	0.66	0.02	0.00
			Max. My	8	0.11	0.00	-0.00
			Max. Vy	25	-0.02	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
			Max Tension	30	0.00	-0.01	0.00
			Max. Compression	24	-0.00	-0.01	-0.00
			Max. Mx	23	-0.00	-0.01	0.00
			Max. My	2	-0.00	-0.00	0.00
			Max. Vy	23	0.02	-0.01	0.00



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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T6	199.84 - 179.84	Top Girt	Max. Vx	2	-0.00	0.00	0.00
			Max Tension	26	1.02	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	25	0.93	0.02	0.00
			Max. My	8	0.43	0.00	-0.00
			Max. Vy	25	-0.02	0.00	0.00
		Guy A	Max. Vx	8	0.00	0.00	0.00
			Bottom Tension	27	15.34		
			Top Tension	10	16.02		
			Top Cable Vert	27	12.34		
			Top Cable Norm	27	10.22		
			Top Cable Tan	27	0.00		
			Bot Cable Vert	10	-10.73		
			Bot Cable Norm	10	10.97		
			Bot Cable Tan	10	0.01		
		Guy B	Bottom Tension	32	14.56		
			Top Tension	15	14.91		
			Top Cable Vert	32	10.83		
			Top Cable Norm	32	10.25		
			Top Cable Tan	32	0.00		
			Bot Cable Vert	15	-9.42		
			Bot Cable Norm	15	11.10		
			Bot Cable Tan	15	0.01		
		Guy C	Bottom Tension	22	15.13		
			Top Tension	5	15.87		
			Top Cable Vert	22	12.16		
			Top Cable Norm	22	10.19		
			Top Cable Tan	22	0.01		
			Bot Cable Vert	5	-10.50		
			Bot Cable Norm	5	10.89		
			Bot Cable Tan	5	0.01		
		Top Guy Pull-Off	Max Tension	9	8.91	0.00	0.00
			Max. Compression	17	-7.70	0.00	0.00
			Max. Mx	24	-0.50	0.14	0.00
			Max. My	8	8.56	0.00	-0.00
			Max. Vy	24	-0.16	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
		Torque Arm Top	Max Tension	11	10.65	-10.61	-0.00
			Max. Compression	8	-4.29	-36.62	-0.00
			Max. Mx	27	3.56	-49.47	0.00
			Max. My	8	1.87	-37.07	0.00
			Max. Vy	27	12.52	-49.47	0.00
			Max. Vx	8	0.00	-37.07	0.00
		Leg	Max Tension	1	0.00	0.00	0.00
			Max. Compression	27	-102.03	-0.74	-0.06
			Max. Mx	6	-52.11	1.47	0.04
			Max. My	2	-64.86	0.22	-1.40
			Max. Vy	14	1.54	0.91	0.02
			Max. Vx	2	1.51	-0.21	0.87
		Diagonal	Max Tension	17	5.44	0.00	0.00
			Max. Compression	9	-6.82	0.00	0.00
			Max. Mx	19	0.08	0.04	0.00
			Max. My	8	0.20	0.00	-0.00
			Max. Vy	19	-0.03	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
		Horizontal	Max Tension	10	2.32	0.00	0.00
			Max. Compression	2	-1.60	0.00	0.00
			Max. Mx	26	0.43	0.02	0.00
			Max. My	8	0.09	0.00	-0.00
			Max. Vy	26	0.02	0.00	0.00

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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T7	179.84 - 159.84	Secondary Horizontal	Max. Vx	8	0.00	0.00	0.00
			Max Tension	30	0.00	-0.01	0.00
		Top Girt	Max. Compression	24	-0.00	-0.01	-0.00
			Max. Mx	33	0.00	-0.01	0.00
			Max. My	2	-0.00	-0.00	0.00
			Max. Vy	33	0.02	-0.01	0.00
			Max. Vx	2	-0.00	0.00	0.00
			Max Tension	4	0.73	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	26	0.70	0.02	0.00
			Max. My	8	0.38	0.00	-0.00
			Max. Vy	26	0.02	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
		Diagonal	Max. Compression	24	-123.53	1.03	0.10
			Max. Mx	23	-98.76	-2.28	0.09
			Max. My	10	-40.57	0.39	-2.23
			Max. Vy	31	-1.03	0.99	-0.70
			Max. Vx	19	-0.95	0.12	1.19
			Max Tension	9	7.86	0.00	0.00
			Max. Compression	9	-8.96	0.00	0.00
			Max. Mx	19	-0.44	0.04	0.00
			Max. My	9	-4.26	0.00	-0.00
			Max. Vy	19	-0.03	0.00	0.00
		Horizontal	Max. Vx	9	0.00	0.00	0.00
			Max Tension	23	1.48	0.00	0.00
			Max. Compression	7	-0.31	0.00	0.00
			Max. Mx	25	0.64	0.02	0.00
			Max. My	10	0.90	0.00	-0.00
		Secondary Horizontal	Max. Vy	25	-0.02	0.00	0.00
			Max. Vx	10	0.00	0.00	0.00
			Max Tension	29	0.00	-0.01	-0.00
			Max. Compression	25	-0.00	-0.01	0.00
			Max. Mx	34	0.00	-0.01	0.00
		Top Girt	Max. My	10	-0.00	-0.00	0.00
			Max. Vy	34	0.01	-0.01	0.00
			Max. Vx	10	-0.00	-0.00	0.00
			Max Tension	25	0.78	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
		Guy A	Max. Mx	25	0.73	0.02	0.00
			Max. Vy	25	-0.02	0.00	0.00
			Bottom Tension	10	17.17		
			Top Tension	10	17.39		
			Top Cable Vert	10	11.20		
			Top Cable Norm	10	13.30		
			Top Cable Tan	10	0.00		
			Bot Cable Vert	10	-10.61		
			Bot Cable Norm	10	13.51		
			Bot Cable Tan	10	0.00		
		Guy B	Bottom Tension	15	16.25		
			Top Tension	15	16.44		
			Top Cable Vert	15	9.41		
			Top Cable Norm	15	13.48		
			Top Cable Tan	15	0.00		
		Guy C	Bot Cable Vert	15	-8.88		
			Bot Cable Norm	15	13.61		
			Bot Cable Tan	15	0.00		
			Bottom Tension	5	17.01		

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	57 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T8	159.84 - 139.84	Top Guy Pull-Off	Top Tension	5	17.22		
			Top Cable Vert	5	10.98		
			Top Cable Norm	5	13.27		
			Top Cable Tan	5	0.00		
			Bot Cable Vert	5	-10.39		
			Bot Cable Norm	5	13.47		
			Bot Cable Tan	5	0.00		
			Max Tension	17	12.94	0.00	0.00
			Max. Compression	9	-11.41	0.00	0.00
			Max. Mx	25	2.71	0.14	0.00
			Max. My	9	-11.41	0.00	0.00
			Max. Vy	25	-0.16	0.00	0.00
			Max. Vx	9	-0.00	0.00	0.00
		Torque Arm Top	Max Tension	3	14.34	-7.24	0.00
			Max. Compression	12	-6.72	0.00	0.00
			Max. Mx	27	3.79	-43.40	-0.00
			Max. My	9	-0.81	-40.21	0.00
			Max. Vy	27	11.01	-43.40	-0.00
			Max. Vx	9	0.00	-40.21	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	23	-130.23	-0.52	-0.90
		Diagonal	Max. Mx	24	-129.00	1.05	-0.01
			Max. My	24	-123.99	-0.42	-0.93
			Max. Vy	24	-0.62	-1.00	0.02
			Max. Vx	19	-0.55	0.47	0.85
			Max Tension	7	3.84	0.00	0.00
			Max. Compression	3	-5.20	0.00	0.00
			Max. Mx	27	-0.94	0.04	0.00
			Max. My	9	-1.67	0.00	-0.00
		Horizontal	Max. Vy	27	-0.03	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
			Max Tension	22	0.91	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	33	0.87	0.02	0.00
			Max. My	9	0.72	0.00	-0.00
			Max. Vy	33	-0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
		Secondary Horizontal	Max Tension	22	0.00	-0.01	0.00
			Max. Compression	33	-0.00	-0.01	0.00
			Max. Mx	34	0.00	-0.01	0.00
			Max. My	10	-0.00	-0.00	0.00
			Max. Vy	34	0.01	-0.01	0.00
			Max. Vx	10	-0.00	-0.00	0.00
		Top Girt	Max Tension	31	1.10	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	32	1.10	0.02	0.00
			Max. Vy	32	-0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
T9	139.84 - 119.84	Leg	Max Tension	1	0.00	0.00	0.00
			Max. Compression	25	-135.48	-1.15	0.01
			Max. Mx	24	-134.85	1.17	-0.02
			Max. My	30	-131.84	-0.56	1.02
			Max. Vy	24	0.71	-1.15	0.02
			Max. Vx	29	-0.61	0.55	-0.99
		Diagonal	Max Tension	17	3.55	0.00	0.00
			Max. Compression	9	-5.53	0.00	0.00
			Max. Mx	27	-0.60	0.03	0.00
			Max. My	9	0.66	0.00	-0.00



<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	59 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T11	99.84 - 79.84	Guy C	Top Tension	15	15.73		
			Top Cable Vert	15	6.24		
			Top Cable Norm	15	14.44		
			Top Cable Tan	15	0.00		
			Bot Cable Vert	15	-5.86		
			Bot Cable Norm	15	14.48		
			Bot Cable Tan	15	0.00		
			Bottom Tension	5	16.40		
			Top Tension	5	16.55		
			Top Cable Vert	5	8.07		
			Top Cable Norm	5	14.44		
			Top Cable Tan	5	0.00		
			Bot Cable Vert	5	-7.64		
			Bot Cable Norm	5	14.51		
			Bot Cable Tan	5	0.00		
		Top Guy Pull-Off	Max Tension	17	14.79	0.00	0.00
			Max. Compression	9	-13.28	0.00	0.00
			Max. Mx	20	3.07	0.14	0.00
			Max. My	9	5.83	0.00	-0.00
			Max. Vy	20	-0.15	0.00	0.00
		Torque Arm Top	Max. Vx	9	0.00	0.00	0.00
			Max Tension	3	16.01	0.00	0.00
			Max. Compression	3	-7.81	0.00	0.00
			Max. Mx	27	4.49	-33.16	-0.00
			Max. My	9	-3.08	-16.41	0.00
		Leg	Max. Vy	27	8.45	-33.16	-0.00
			Max. Vx	9	0.00	-16.41	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	25	-166.19	1.57	-0.01
			Max. Mx	24	-164.63	-1.58	0.01
			Max. My	20	-163.28	0.77	1.37
			Max. Vy	6	1.37	-0.87	0.02
			Max. Vx	10	1.33	0.32	-0.76
		Diagonal	Max Tension	6	4.99	0.00	0.00
			Max. Compression	14	-7.48	0.00	0.00
			Max. Mx	23	-1.29	0.04	0.00
			Max. My	9	0.11	0.00	-0.00
			Max. Vy	23	-0.03	0.00	0.00
		Horizontal	Max. Vx	9	0.00	0.00	0.00
			Max Tension	10	2.25	0.00	0.00
			Max. Compression	2	-0.77	0.00	0.00
			Max. Mx	31	1.50	0.02	0.00
			Max. My	9	2.02	0.00	-0.00
		Secondary Horizontal	Max. Vy	31	-0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
			Max Tension	23	0.00	-0.01	-0.00
			Max. Compression	23	-0.00	0.00	0.00
			Max. Mx	19	0.00	-0.01	0.00
T12	79.84 - 59.84	Top Girt	Max. My	2	0.00	-0.00	0.00
			Max. Vy	19	0.01	-0.01	0.00
			Max. Vx	2	-0.00	-0.00	0.00
			Max Tension	31	1.51	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
		Leg	Max. Mx	20	1.47	0.02	0.00
			Max. My	9	0.58	0.00	-0.00
			Max. Vy	20	-0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
		Leg	Max. Compression	25	-168.94	-0.58	-1.45
			Max. Mx	24	-168.27	1.66	0.21

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	60 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T13	59.84 - 39.84	Diagonal	Max. My	19	-166.55	-0.60	-1.48
			Max. Vy	24	-0.98	-1.59	-0.03
			Max. Vx	19	0.87	0.74	1.39
			Max Tension	11	6.02	0.00	0.00
			Max. Compression	3	-8.01	0.00	0.00
			Max. Mx	26	-1.53	0.03	0.00
		Horizontal	Max. My	8	0.84	0.00	-0.00
			Max. Vy	26	-0.03	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
			Max Tension	21	1.58	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	33	1.51	0.02	0.00
		Secondary Horizontal	Max. My	9	1.00	0.00	-0.00
			Max. Vy	33	-0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
			Max Tension	23	0.00	-0.00	-0.00
			Max. Compression	13	-0.00	-0.00	-0.00
			Max. Mx	19	0.00	-0.01	0.00
		Top Girt	Max. My	2	0.00	-0.00	0.00
			Max. Vy	19	0.01	-0.01	0.00
			Max. Vx	2	-0.00	-0.00	0.00
			Max Tension	25	1.54	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	31	1.51	0.02	0.00
		Leg	Max. My	9	0.65	0.00	-0.00
			Max. Vy	31	-0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	25	-185.29	-1.73	0.03
			Max. Mx	23	-169.24	-1.85	-0.89
		Diagonal	Max. My	19	-171.73	0.24	-1.87
			Max. Vy	23	1.06	-1.85	-0.89
			Max. Vx	19	-1.09	-0.64	1.63
			Max Tension	3	5.82	0.00	0.00
			Max. Compression	11	-8.11	0.00	0.00
			Max. Mx	26	-2.45	0.03	0.00
		Horizontal	Max. My	8	-3.07	0.00	-0.00
			Max. Vy	26	-0.02	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
			Max Tension	23	1.76	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	19	1.61	0.02	0.00
		Secondary Horizontal	Max. My	9	1.06	0.00	-0.00
			Max. Vy	19	-0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
			Max Tension	23	0.00	-0.00	-0.00
			Max. Compression	13	-0.00	-0.00	-0.00
			Max. Mx	19	0.00	-0.00	0.00
		Top Girt	Max. My	2	0.00	-0.00	0.00
			Max. Vy	19	0.01	-0.00	0.00
			Max. Vx	2	-0.00	-0.00	0.00
			Max Tension	24	1.86	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	34	1.72	0.02	0.00
		Guy A	Max. My	9	0.93	0.00	-0.00
			Max. Vy	34	0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
			Bottom Tension	27	13.41		
			Top Tension	10	13.73		

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	61 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T14	39.84 - 19.84	Guy B	Top Cable Vert	27	5.25		
			Top Cable Norm	27	12.69		
			Top Cable Tan	27	0.00		
			Bot Cable Vert	10	-4.20		
			Bot Cable Norm	10	12.74		
			Bot Cable Tan	10	0.00		
			Bottom Tension	32	12.68		
			Top Tension	15	13.03		
			Top Cable Vert	32	3.51		
			Top Cable Norm	32	12.55		
		Guy C	Top Cable Tan	32	0.00		
			Bot Cable Vert	15	-2.39		
			Bot Cable Norm	15	12.45		
			Bot Cable Tan	15	0.00		
			Bottom Tension	22	13.09		
			Top Tension	5	13.62		
			Top Cable Vert	22	5.06		
			Top Cable Norm	22	12.65		
			Top Cable Tan	22	0.00		
			Bot Cable Vert	5	-3.94		
		Top Guy Pull-Off	Bot Cable Norm	5	12.48		
			Bot Cable Tan	5	0.00		
			Max Tension	9	12.72	0.00	0.00
			Max. Compression	17	-10.17	0.00	0.00
			Max. Mx	33	3.10	0.13	0.00
			Max. My	9	-2.99	0.00	-0.00
			Max. Vy	33	-0.15	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
		Torque Arm Top	Max Tension	11	13.40	0.00	0.00
			Max. Compression	11	-5.87	0.00	0.00
			Max. Mx	27	5.15	-21.40	0.00
			Max. My	9	-2.37	-10.12	0.00
			Max. Vy	27	5.50	-21.40	0.00
			Max. Vx	9	0.00	-10.12	0.00
		Leg	Max Tension	1	0.00	0.00	0.00
			Max. Compression	25	-190.22	-1.79	0.03
			Max. Mx	25	-190.17	1.81	-0.02
			Max. My	24	-190.02	-0.91	-1.56
			Max. Vy	24	-1.09	-1.79	0.03
			Max. Vx	24	0.94	-0.91	-1.56
		Diagonal	Max Tension	11	2.01	0.00	0.00
			Max. Compression	14	-4.20	0.00	0.00
			Max. Mx	26	-1.86	0.03	0.00
			Max. My	8	-1.80	0.00	-0.00
			Max. Vy	26	-0.02	0.00	0.00
			Max. Vx	8	0.00	0.00	0.00
		Horizontal	Max Tension	23	1.73	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	19	1.64	0.02	0.00
			Max. My	9	1.04	0.00	-0.00
			Max. Vy	19	-0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
		Secondary Horizontal	Max Tension	23	0.00	-0.00	-0.00
			Max. Compression	13	-0.00	-0.00	-0.00
			Max. Mx	19	0.00	-0.00	0.00
			Max. My	2	0.00	-0.00	0.00
			Max. Vy	19	0.01	-0.00	0.00
			Max. Vx	2	-0.00	-0.00	0.00
		Top Girt	Max Tension	26	1.69	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	62 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJJ

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T15	19.84 - 6.5	Leg	Max. Mx	19	1.64	0.02	0.00
			Max. My	17	1.02	0.00	-0.00
			Max. Vy	19	-0.02	0.00	0.00
			Max. Vx	17	0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	24	-193.48	-0.57	-0.51
			Max. Mx	25	-191.26	1.81	-0.02
			Max. My	24	-190.09	-0.91	-1.56
			Max. Vy	29	1.09	1.80	-0.02
			Max. Vx	19	-0.95	0.92	1.55
		Diagonal	Max Tension	6	3.15	0.00	0.00
			Max. Compression	9	-4.40	0.00	0.00
			Max. Mx	26	-1.24	0.03	0.00
			Max. My	8	0.11	0.00	-0.00
			Max. Vy	26	-0.02	0.00	0.00
		Horizontal	Max. Vx	8	0.00	0.00	0.00
			Max Tension	34	1.73	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	19	1.21	0.02	0.00
			Max. My	9	0.78	0.00	-0.00
		Secondary Horizontal	Max. Vy	19	-0.02	0.00	0.00
			Max. Vx	9	0.00	0.00	0.00
			Max Tension	23	0.00	-0.00	-0.00
			Max. Compression	13	-0.00	-0.00	-0.00
			Max. Mx	19	0.00	-0.00	0.00
		Top Girt	Max. My	2	0.00	-0.00	0.00
			Max. Vy	19	0.01	-0.00	0.00
			Max. Vx	2	-0.00	-0.00	0.00
			Max Tension	26	1.74	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
T16	6.5 - 0	Leg	Max. Mx	19	1.67	0.02	0.00
			Max. My	17	0.96	0.00	-0.00
			Max. Vy	19	-0.02	0.00	0.00
			Max. Vx	17	0.00	0.00	0.00
			Max Tension	1	0.00	0.00	0.00
		Top Girt	Max. Compression	24	-204.80	0.22	0.31
			Max. Mx	24	-198.50	2.72	-0.10
			Max. My	15	-82.61	1.26	-0.44
			Max. Vy	24	10.31	-2.70	0.14
			Max. Vx	15	-1.05	-1.16	0.11
		Bottom Girt	Max Tension	24	36.72	0.57	0.05
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	30	36.17	-1.43	-0.07
			Max. My	24	36.01	-1.42	-0.07
			Max. Vy	19	-0.64	-1.42	-0.07
		Mid Girt	Max. Vx	24	-0.05	-1.42	-0.07
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	24	-6.05	-2.19	-0.02
			Max. Mx	23	-5.96	-2.29	-0.03
			Max. My	22	-5.96	-2.28	-0.03
			Max. Vy	6	-3.97	-1.36	-0.02
			Max. Vx	23	-0.18	-2.29	-0.03
			Max Tension	24	0.56	0.00	0.00
			Max. Compression	29	-0.37	0.00	0.00
			Max. Mx	34	0.54	0.03	0.00
			Max. My	28	0.55	0.00	0.00
			Max. Vy	34	-0.04	0.00	0.00
			Max. Vx	28	-0.01	0.00	0.00



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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

## Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Mast	Max. Vert	24	579.91	-0.05	0.34
	Max. H <sub>x</sub>	14	248.60	3.50	0.30
	Max. H <sub>z</sub>	2	246.46	0.02	3.50
	Max. M <sub>x</sub>	1	0.00	-0.04	0.02
	Max. M <sub>z</sub>	1	0.00	-0.04	0.02
	Max. Torsion	1	0.00	-0.04	0.02
	Min. Vert	1	187.82	-0.04	0.02
	Min. H <sub>x</sub>	6	255.08	-3.40	0.19
	Min. H <sub>z</sub>	10	257.56	-0.19	-3.32
	Min. M <sub>x</sub>	1	0.00	-0.04	0.02
	Min. M <sub>z</sub>	1	0.00	-0.04	0.02
	Min. Torsion	1	0.00	-0.04	0.02
	Max. Vert	13	-14.45	-11.89	6.86
	Max. H <sub>x</sub>	13	-14.45	-11.89	6.86
Guy C @ 235 ft Elev -20.1 ft Azimuth 240 deg	Max. H <sub>z</sub>	4	-105.58	-118.24	69.43
	Min. Vert	5	-106.80	-120.03	69.31
	Min. H <sub>x</sub>	5	-106.80	-120.03	69.31
	Min. H <sub>z</sub>	13	-14.45	-11.89	6.86
	Max. Vert	7	-10.54	10.71	6.19
	Max. H <sub>x</sub>	15	-91.63	120.61	69.61
Guy B @ 235 ft Elev 8.9 ft Azimuth 120 deg	Max. H <sub>z</sub>	15	-91.63	120.61	69.61
	Min. Vert	15	-91.63	120.61	69.61
	Min. H <sub>x</sub>	7	-10.54	10.71	6.19
	Min. H <sub>z</sub>	7	-10.54	10.71	6.19
	Max. Vert	2	-14.90	-0.01	-13.91
	Max. H <sub>x</sub>	14	-63.34	5.28	-77.56
Guy A @ 235 ft Elev -23.4 ft Azimuth 0 deg	Max. H <sub>z</sub>	2	-14.90	-0.01	-13.91
	Min. Vert	10	-108.68	0.03	-138.62
	Min. H <sub>x</sub>	6	-60.83	-5.29	-74.73
	Min. H <sub>z</sub>	10	-108.68	0.03	-138.62
	Max. H <sub>x</sub>	14	-63.34	5.28	-77.56
	Max. H <sub>z</sub>	2	-14.90	-0.01	-13.91

## Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	187.82	0.04	-0.02	0.00	0.00	0.00
1.2 Dead+1.6 Wind 0 deg - No Ice+1.0 Guy	246.46	-0.02	-3.50	0.00	0.00	0.00
1.2 Dead+1.6 Wind 30 deg - No Ice+1.0 Guy	253.09	1.87	-2.68	0.00	0.00	0.00
1.2 Dead+1.6 Wind 45 deg - No Ice+1.0 Guy	255.63	2.60	-2.18	0.00	0.00	0.00
1.2 Dead+1.6 Wind 60 deg - No Ice+1.0 Guy	256.81	3.09	-1.62	0.00	0.00	0.00

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	64 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

<i>Load Combination</i>	<i>Vertical K</i>	<i>Shear<sub>x</sub> K</i>	<i>Shear<sub>z</sub> K</i>	<i>Overturning Moment, M<sub>x</sub> kip-ft</i>	<i>Overturning Moment, M<sub>z</sub> kip-ft</i>	<i>Torque kip-ft</i>
1.2 Dead+1.6 Wind 90 deg - No Ice+1.0 Guy	255.08	3.40	-0.19	0.00	0.00	0.00
1.2 Dead+1.6 Wind 120 deg - No Ice+1.0 Guy	252.02	3.07	1.54	0.00	0.00	0.00
1.2 Dead+1.6 Wind 135 deg - No Ice+1.0 Guy	253.57	2.39	2.25	0.00	0.00	0.00
1.2 Dead+1.6 Wind 150 deg - No Ice+1.0 Guy	255.74	1.60	2.82	0.00	0.00	0.00
1.2 Dead+1.6 Wind 180 deg - No Ice+1.0 Guy	257.56	0.19	3.32	0.00	0.00	0.00
1.2 Dead+1.6 Wind 210 deg - No Ice+1.0 Guy	253.72	-1.37	2.81	0.00	0.00	0.00
1.2 Dead+1.6 Wind 225 deg - No Ice+1.0 Guy	249.87	-2.27	2.24	0.00	0.00	0.00
1.2 Dead+1.6 Wind 240 deg - No Ice+1.0 Guy	246.96	-3.05	1.53	0.00	0.00	0.00
1.2 Dead+1.6 Wind 270 deg - No Ice+1.0 Guy	248.60	-3.50	-0.30	0.00	0.00	0.00
1.2 Dead+1.6 Wind 300 deg - No Ice+1.0 Guy	250.84	-3.15	-1.87	0.00	0.00	0.00
1.2 Dead+1.6 Wind 315 deg - No Ice+1.0 Guy	250.15	-2.66	-2.45	0.00	0.00	0.00
1.2 Dead+1.6 Wind 330 deg - No Ice+1.0 Guy	248.60	-1.93	-2.91	0.00	0.00	0.00
1.2 Dead+1.0 Ice+1.0 Temp+Guy	573.04	0.01	-0.32	0.00	0.00	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp+1.0 Guy	576.46	-0.00	-0.37	0.00	0.00	0.00
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp+1.0 Guy	575.32	0.00	-0.37	0.00	0.00	0.00
1.2 Dead+1.0 Wind 45 deg+1.0 Ice+1.0 Temp+1.0 Guy	574.40	0.01	-0.36	0.00	0.00	0.00
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp+1.0 Guy	574.32	0.03	-0.33	0.00	0.00	0.00
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp+1.0 Guy	577.28	0.05	-0.30	0.00	0.00	0.00
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp+1.0 Guy	579.91	0.05	-0.34	0.00	0.00	0.00
1.2 Dead+1.0 Wind 135 deg+1.0 Ice+1.0 Temp+1.0 Guy	579.20	0.06	-0.34	0.00	0.00	0.00
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp+1.0 Guy	577.47	0.06	-0.32	0.00	0.00	0.00
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp+1.0 Guy	574.63	0.02	-0.31	0.00	0.00	0.00
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp+1.0 Guy	575.69	-0.04	-0.32	0.00	0.00	0.00
1.2 Dead+1.0 Wind 225 deg+1.0 Ice+1.0 Temp+1.0 Guy	576.65	-0.05	-0.33	0.00	0.00	0.00
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp+1.0 Guy	576.80	-0.05	-0.33	0.00	0.00	0.00
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp+1.0 Guy	573.94	-0.05	-0.30	0.00	0.00	0.00
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp+1.0 Guy	571.53	-0.02	-0.34	0.00	0.00	0.00
1.2 Dead+1.0 Wind 315 deg+1.0 Ice+1.0 Temp+1.0 Guy	572.16	-0.00	-0.37	0.00	0.00	0.00
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp+1.0 Guy	573.76	0.01	-0.38	0.00	0.00	0.00
Dead+Wind 0 deg - Service+Guy	190.22	0.04	-0.94	0.00	0.00	0.00
Dead+Wind 30 deg -	190.16	0.49	-0.80	0.00	0.00	0.00

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	65 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

<i>Load Combination</i>	<i>Vertical K</i>	<i>Shear<sub>x</sub> K</i>	<i>Shear<sub>z</sub> K</i>	<i>Overturning Moment, M<sub>x</sub> kip-ft</i>	<i>Overturning Moment, M<sub>z</sub> kip-ft</i>	<i>Torque kip-ft</i>
Service+Guy						
Dead+Wind 45 deg -	190.12	0.68	-0.65	0.00	0.00	0.00
Service+Guy						
Dead+Wind 60 deg -	190.20	0.83	-0.45	0.00	0.00	0.00
Service+Guy						
Dead+Wind 90 deg -	190.81	0.97	-0.00	0.00	0.00	0.00
Service+Guy						
Dead+Wind 120 deg -	191.40	0.87	0.42	0.00	0.00	0.00
Service+Guy						
Dead+Wind 135 deg -	191.24	0.72	0.60	0.00	0.00	0.00
Service+Guy						
Dead+Wind 150 deg -	190.86	0.53	0.75	0.00	0.00	0.00
Service+Guy						
Dead+Wind 180 deg -	190.27	0.06	0.86	0.00	0.00	0.00
Service+Guy						
Dead+Wind 210 deg -	190.26	-0.42	0.74	0.00	0.00	0.00
Service+Guy						
Dead+Wind 225 deg -	190.35	-0.62	0.59	0.00	0.00	0.00
Service+Guy						
Dead+Wind 240 deg -	190.32	-0.77	0.41	0.00	0.00	0.00
Service+Guy						
Dead+Wind 270 deg -	189.72	-0.89	-0.02	0.00	0.00	0.00
Service+Guy						
Dead+Wind 300 deg -	189.32	-0.75	-0.48	0.00	0.00	0.00
Service+Guy						
Dead+Wind 315 deg -	189.41	-0.61	-0.67	0.00	0.00	0.00
Service+Guy						
Dead+Wind 330 deg -	189.67	-0.42	-0.82	0.00	0.00	0.00
Service+Guy						

## Solution Summary

<i>Load Comb.</i>	<i>Sum of Applied Forces</i>			<i>Sum of Reactions</i>			<i>% Error</i>
	<i>PX K</i>	<i>PY K</i>	<i>PZ K</i>	<i>PX K</i>	<i>PY K</i>	<i>PZ K</i>	
1	0.00	-66.51	0.00	0.00	66.51	0.00	0.006%
2	1.05	-78.25	-104.95	-1.04	78.25	104.89	0.039%
3	52.59	-77.21	-90.73	-52.59	77.21	90.69	0.028%
4	73.71	-76.45	-73.92	-73.73	76.45	73.86	0.044%
5	89.80	-76.18	-52.39	-89.78	76.18	52.47	0.061%
6	103.57	-77.53	-0.42	-103.53	77.53	0.46	0.045%
7	90.10	-78.83	52.79	-90.07	78.83	-52.78	0.021%
8	72.99	-78.45	73.90	-72.96	78.45	-73.89	0.019%
9	51.25	-77.56	90.38	-51.20	77.55	-90.36	0.046%
10	-0.54	-76.23	104.11	0.61	76.23	-104.12	0.053%
11	-52.01	-77.26	90.95	51.97	77.26	-90.94	0.030%
12	-73.17	-78.02	74.91	73.12	78.02	-74.88	0.038%
13	-89.96	-78.30	53.92	89.92	78.30	-53.90	0.041%
14	-102.95	-76.94	0.51	102.93	76.94	-0.48	0.027%
15	-88.92	-75.64	-51.26	88.95	75.64	51.29	0.030%
16	-72.71	-76.02	-72.74	72.74	76.02	72.70	0.041%
17	-51.36	-76.92	-89.72	51.37	76.92	89.68	0.026%
18	0.00	-349.70	0.00	0.01	349.70	0.01	0.003%
19	0.27	-350.39	-27.49	-0.26	350.39	27.49	0.002%
20	13.82	-349.68	-23.90	-13.81	349.68	23.90	0.004%
21	19.40	-349.17	-19.52	-19.38	349.17	19.52	0.004%
22	23.60	-348.98	-13.88	-23.59	348.98	13.88	0.004%
23	27.07	-349.86	-0.12	-27.06	349.86	0.12	0.004%

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	66 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
24	23.45	-350.72	13.77	-23.44	350.72	-13.76	0.004%
25	19.09	-350.47	19.33	-19.08	350.47	-19.32	0.004%
26	13.39	-349.88	23.68	-13.38	349.88	-23.67	0.004%
27	-0.21	-349.00	27.48	0.22	349.00	-27.47	0.004%
28	-13.71	-349.71	24.00	13.72	349.71	-23.99	0.004%
29	-19.29	-350.23	19.73	19.30	350.23	-19.73	0.003%
30	-23.57	-350.42	14.15	23.57	350.42	-14.14	0.002%
31	-27.00	-349.53	0.14	27.00	349.53	-0.13	0.001%
32	-23.37	-348.67	-13.51	23.37	348.67	13.51	0.001%
33	-19.10	-348.92	-19.10	19.10	348.92	19.10	0.001%
34	-13.45	-349.52	-23.53	13.45	349.52	23.53	0.001%
35	0.23	-66.73	-23.15	-0.23	66.73	23.14	0.006%
36	11.60	-66.50	-20.01	-11.59	66.50	20.00	0.023%
37	16.26	-66.33	-16.30	-16.24	66.33	16.30	0.025%
38	19.81	-66.27	-11.56	-19.79	66.27	11.56	0.028%
39	22.84	-66.57	-0.09	-22.82	66.57	0.10	0.035%
40	19.87	-66.86	11.64	-19.87	66.86	-11.64	0.011%
41	16.10	-66.77	16.30	-16.09	66.77	-16.30	0.010%
42	11.30	-66.58	19.93	-11.29	66.58	-19.92	0.035%
43	-0.12	-66.28	22.96	0.13	66.28	-22.94	0.030%
44	-11.47	-66.51	20.06	11.47	66.51	-20.04	0.025%
45	-16.14	-66.68	16.52	16.14	66.68	-16.52	0.006%
46	-19.84	-66.74	11.89	19.84	66.74	-11.89	0.006%
47	-22.71	-66.44	0.11	22.70	66.44	-0.11	0.019%
48	-19.61	-66.15	-11.31	19.60	66.15	11.30	0.013%
49	-16.04	-66.24	-16.04	16.03	66.24	16.04	0.014%
50	-11.33	-66.44	-19.79	11.33	66.44	19.78	0.017%

## Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	8	0.00104866	0.00010821
2	Yes	11	0.00094455	0.00082159
3	Yes	11	0.00063453	0.00059223
4	Yes	10	0.00096146	0.00088086
5	Yes	8	0.00128301	0.00038215
6	Yes	11	0.00107709	0.00110621
7	Yes	12	0.00058613	0.00078546
8	Yes	12	0.00050555	0.00065985
9	Yes	11	0.00109056	0.00115546
10	Yes	8	0.00114622	0.00033811
11	Yes	11	0.00067483	0.00063900
12	Yes	11	0.00088607	0.00081825
13	Yes	11	0.00100499	0.00089057
14	Yes	11	0.00069487	0.00046288
15	Yes	8	0.00061970	0.00036455
16	Yes	10	0.00101502	0.00055905
17	Yes	11	0.00066263	0.00044735
18	Yes	10	0.00080442	0.00009205
19	Yes	10	0.00120894	0.00019542
20	Yes	10	0.00124510	0.00018353
21	Yes	10	0.00109722	0.00014804
22	Yes	10	0.00087755	0.00010762
23	Yes	10	0.00061253	0.00011874
24	Yes	10	0.00061083	0.00016296

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	67 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

25	Yes	10	0.00062834	0.00015058
26	Yes	10	0.00065943	0.00012205
27	Yes	10	0.00087464	0.00010646
28	Yes	10	0.00119578	0.00018012
29	Yes	10	0.00121072	0.00019486
30	Yes	10	0.00113023	0.00018986
31	Yes	10	0.00079649	0.00013977
32	Yes	9	0.00102954	0.00019628
33	Yes	9	0.00139301	0.00022256
34	Yes	10	0.00090961	0.00014843
35	Yes	8	0.00056177	0.00014180
36	Yes	7	0.00143628	0.00026444
37	Yes	7	0.00128572	0.00019648
38	Yes	7	0.00111871	0.00016146
39	Yes	7	0.00121953	0.00033331
40	Yes	8	0.00048687	0.00019747
41	Yes	8	0.00046616	0.00017751
42	Yes	7	0.00126299	0.00034149
43	Yes	7	0.00115430	0.00016565
44	Yes	7	0.00144252	0.00026777
45	Yes	8	0.00058359	0.00015218
46	Yes	8	0.00054597	0.00014195
47	Yes	7	0.00109776	0.00019983
48	Yes	7	0.00064818	0.00008795
49	Yes	7	0.00084047	0.00013016
50	Yes	7	0.00115642	0.00020346

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	327 - 291.84	1.762	43	0.1055	0.0368
T1	291.84 - 279.84	1.209	43	0.0157	0.0396
T2	279.84 - 259.84	1.198	43	0.0104	0.0368
T3	259.84 - 239.84	1.224	43	0.0272	0.0257
T4	239.84 - 219.84	1.306	43	0.0364	0.0384
T5	219.84 - 199.84	1.392	43	0.0430	0.0362
T6	199.84 - 179.84	1.507	43	0.0395	0.0416
T7	179.84 - 159.84	1.543	43	0.0145	0.0343
T8	159.84 - 139.84	1.510	43	0.0094	0.0332
T9	139.84 - 119.84	1.456	43	0.0234	0.0385
T10	119.84 - 99.84	1.321	43	0.0366	0.0407
T11	99.84 - 79.84	1.164	43	0.0337	0.0431
T12	79.84 - 59.84	1.014	43	0.0444	0.0494
T13	59.84 - 39.84	0.793	43	0.0520	0.0529
T14	39.84 - 19.84	0.586	40	0.0557	0.0686
T15	19.84 - 6.5	0.324	40	0.0705	0.0813
T16	6.5 - 0	0.109	40	0.0779	0.0799

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
325.00	4-ft Lightning Rod	43	1.719	0.0994	0.0425	87415

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	68 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

<i>Elevation</i>	<i>Appurtenance</i>	<i>Gov. Load Comb.</i>	<i>Deflection in</i>	<i>Tilt °</i>	<i>Twist °</i>	<i>Radius of Curvature ft</i>
<i>ft</i>			<i>in</i>			
323.00	Flash Beacon Lighting	43	1.677	0.0933	0.0423	87415
305.00	6813 1-Bay w/radome	43	1.341	0.0423	0.0407	19867
290.00	6-ft Dish	43	1.202	0.0131	0.0394	15022
285.84	Guy	43	1.195	0.0093	0.0388	24241
277.00	PD1110	43	1.200	0.0132	0.0351	168180
267.00	OGT9-840	43	1.209	0.0216	0.0278	62448
264.00	SC479-HF1LDF	43	1.214	0.0241	0.0263	51913
261.00	AP14-850/105	43	1.221	0.0264	0.0257	46339
256.51	Guy	43	1.235	0.0293	0.0267	52836
256.50	SE419-SF3P4LDF	43	1.235	0.0293	0.0267	52860
254.00	SC479-HF1LDF	43	1.244	0.0306	0.0282	64923
253.00	OGT9-840	43	1.248	0.0312	0.0289	71601
252.00	AP14-850/105	43	1.252	0.0316	0.0297	79811
249.00	SC479-HF1LDF	43	1.265	0.0330	0.0321	120519
240.00	TTA 432-83H-01T	43	1.305	0.0364	0.0384	164946
239.00	SC479-HF1LDF	43	1.310	0.0367	0.0387	168471
216.51	Guy	43	1.411	0.0437	0.0366	87643
211.00	6813 1-Bay w/radome	43	1.445	0.0442	0.0384	731702
198.00	6813 1-Bay w/radome	43	1.514	0.0377	0.0415	41714
190.00	6' Yagi	43	1.537	0.0272	0.0390	47764
185.00	7770.00	43	1.542	0.0203	0.0366	50376
172.17	24"x12"x5" Panel	43	1.534	0.0102	0.0326	95551
172.00	8' x 3" Dia Omni	43	1.534	0.0102	0.0325	97254
171.50	5' Gird Dish	43	1.533	0.0101	0.0325	102631
166.51	Guy	43	1.524	0.0096	0.0324	229206
166.00	16"x12"x3" TTA	43	1.523	0.0096	0.0324	261993
158.83	24"x12"x5" Panel	43	1.508	0.0097	0.0334	257659
157.00	Beacon	43	1.505	0.0102	0.0338	513365
125.00	Sabre 2' Sidearm	43	1.361	0.0349	0.0404	86702
124.00	6'x4' Ice Shield	43	1.354	0.0354	0.0405	95688
116.00	6-ft Dish	43	1.290	0.0366	0.0409	301264
112.00	PD1110	43	1.258	0.0358	0.0412	253478
106.51	Guy	43	1.214	0.0343	0.0418	130618
104.00	6-ft Dish	43	1.195	0.0338	0.0422	107081
94.00	PR-850	43	1.124	0.0354	0.0450	275892
84.00	HBXX-6517DS	43	1.050	0.0416	0.0484	46743
70.00	DB212-1	43	0.909	0.0493	0.0505	117035
56.51	Guy	43	0.757	0.0524	0.0547	111273
18.00	6' Yagi	40	0.296	0.0717	0.0815	66547
13.00	4-ft Dish	40	0.217	0.0746	0.0811	101569

## Maximum Tower Deflections - Design Wind

<i>Section No.</i>	<i>Elevation ft</i>	<i>Horz. Deflection in</i>	<i>Gov. Load Comb.</i>	<i>Tilt °</i>	<i>Twist °</i>
L1	327 - 291.84	12.649	10	0.5757	0.2797
T1	291.84 - 279.84	9.322	10	0.1700	0.2652
T2	279.84 - 259.84	8.998	10	0.1226	0.2535
T3	259.84 - 239.84	8.690	10	0.1090	0.1981
T4	239.84 - 219.84	8.682	10	0.1547	0.1932
T5	219.84 - 199.84	8.736	10	0.1975	0.1908
T6	199.84 - 179.84	9.043	11	0.1847	0.1950
T7	179.84 - 159.84	9.163	11	0.0827	0.1839
T8	159.84 - 139.84	9.050	7	0.0970	0.2049
T9	139.84 - 119.84	8.981	7	0.1702	0.2430

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	69 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T10	119.84 - 99.84	8.364	7	0.2387	0.2733
T11	99.84 - 79.84	7.474	7	0.2319	0.2580
T12	79.84 - 59.84	6.457	7	0.2924	0.2274
T13	59.84 - 39.84	5.028	7	0.3437	0.2100
T14	39.84 - 19.84	3.609	7	0.3652	0.2671
T15	19.84 - 6.5	1.945	7	0.4340	0.3249
T16	6.5 - 0	0.651	7	0.4676	0.3298

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
325.00	4-ft Lightning Rod	10	12.409	0.5468	0.2790	19554
323.00	Flash Beacon Lighting	10	12.171	0.5181	0.2782	19554
305.00	6813 1-Bay w/radome	10	10.229	0.2828	0.2712	4444
290.00	6-ft Dish	10	9.248	0.1603	0.2642	3379
285.84	Guy	10	9.122	0.1430	0.2614	5530
277.00	PD1110	10	8.945	0.1124	0.2473	47566
267.00	OGT9-840	10	8.772	0.0970	0.2180	12804
264.00	SC479-HF1LDF	10	8.732	0.0984	0.2090	10406
261.00	AP14-850/105	10	8.700	0.1059	0.2009	9145
256.51	Guy	10	8.672	0.1182	0.1914	10210
256.50	SE419-SF3P4LDF	10	8.672	0.1182	0.1914	10215
254.00	SC479-HF1LDF	10	8.665	0.1253	0.1876	12322
253.00	OGT9-840	10	8.664	0.1279	0.1864	13459
252.00	AP14-850/105	10	8.663	0.1303	0.1853	14827
249.00	SC479-HF1LDF	10	8.665	0.1370	0.1828	21330
240.00	TTA 432-83H-01T	10	8.682	0.1544	0.1931	14782
239.00	SC479-HF1LDF	10	8.683	0.1565	0.1939	14850
216.51	Guy	10	8.770	0.2022	0.1910	10030
211.00	6813 1-Bay w/radome	10	8.838	0.2056	0.1927	19961
198.00	6813 1-Bay w/radome	11	9.075	0.1764	0.1944	9070
190.00	6' Yagi	11	9.163	0.1290	0.1889	9582
185.00	7770.00	11	9.178	0.0961	0.1846	10082
172.17	24"x12"x5" Panel	11	9.092	0.0904	0.1902	16335
172.00	8' x 3" Dia Omni	11	9.090	0.0904	0.1904	16531
171.50	5' Gird Dish	11	9.083	0.0905	0.1908	17133
166.51	Guy	8	9.053	0.0914	0.1961	26940
166.00	16"x12"x3" TTA	8	9.051	0.0916	0.1967	28601
158.83	24"x12"x5" Panel	7	9.054	0.0988	0.2065	46846
157.00	Beacon	7	9.059	0.1029	0.2094	32699
125.00	Sabre 2' Sidearm	7	8.567	0.2277	0.2694	13088
124.00	6'x4' Ice Shield	7	8.529	0.2305	0.2705	14081
116.00	6-ft Dish	7	8.203	0.2407	0.2733	26556
112.00	PD1110	7	8.028	0.2388	0.2712	38097
106.51	Guy	7	7.780	0.2338	0.2659	25477
104.00	6-ft Dish	7	7.665	0.2320	0.2630	21790
94.00	PR-850	7	7.206	0.2399	0.2515	27553
84.00	HBXX-6517DS	7	6.701	0.2752	0.2368	8876
70.00	DB212-1	7	5.781	0.3246	0.2127	21936
56.51	Guy	7	4.789	0.3470	0.2145	18413
18.00	6' Yagi	7	1.773	0.4396	0.3268	14435
13.00	4-ft Dish	7	1.293	0.4525	0.3292	22151

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	70 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

### Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	291.84	Leg	A325N	1.0000	4	3.08	53.01	0.058 ✓	1	Bolt Tension
T2	279.84	Leg	A325N	1.0000	4	2.62	53.01	0.050 ✓	1	Bolt Tension
T3	259.84	Leg	A325N	1.0000	4	3.28	53.01	0.062 ✓	1	Bolt Tension
T4	239.84	Leg	A325N	1.0000	4	5.20	53.01	0.098 ✓	1	Bolt Tension
T5	219.84	Leg	A325N	1.0000	4	5.34	53.01	0.101 ✓	1	Bolt Tension
T6	199.84	Leg	A325N	1.0000	4	7.91	53.01	0.149 ✓	1	Bolt Tension
T7	179.84	Leg	A325N	1.0000	4	8.42	53.01	0.159 ✓	1	Bolt Tension
T8	159.84	Leg	A325N	1.0000	4	10.40	53.01	0.196 ✓	1	Bolt Tension
T9	139.84	Leg	A325N	1.0000	4	10.90	53.01	0.206 ✓	1	Bolt Tension
T10	119.84	Leg	A325N	1.0000	4	11.32	53.01	0.214 ✓	1	Bolt Tension
T11	99.84	Leg	A325N	1.0000	4	13.00	53.01	0.245 ✓	1	Bolt Tension
T12	79.84	Leg	A325N	1.3750	4	13.89	100.23	0.139 ✓	1	Bolt Tension
T13	59.84	Leg	A325N	1.3750	4	14.11	100.23	0.141 ✓	1	Bolt Tension
T14	39.84	Leg	A325N	1.3750	4	15.55	100.23	0.155 ✓	1	Bolt Tension
T15	19.84	Leg	A325N	1.3750	4	15.94	100.23	0.159 ✓	1	Bolt Tension
T16	6.5	Leg	A325N	1.3750	4	17.02	100.23	0.170 ✓	1	Bolt Tension

### Guy Design Data

Section No.	Elevation ft	Size	Initial Tension K	Breaking Load K	Actual $T_u$ K	Allowable $\phi T_n$ K	Required S.F.	Actual S.F.
T1	285.84 (A) (672)	3/4 EHS	5.83	58.30	16.59	34.98	1.000	2.109 ✓
	285.84 (A) (673)	3/4 EHS	5.83	58.30	16.59	34.98	1.000	2.108 ✓
	285.84 (B) (668)	3/4 EHS	5.83	58.30	15.62	34.98	1.000	2.239 ✓
	285.84 (B) (669)	3/4 EHS	5.83	58.30	15.60	34.98	1.000	2.243 ✓
	285.84 (C) (664)	3/4 EHS	5.83	58.30	16.46	34.98	1.000	2.125 ✓
	285.84 (C) (665)	3/4 EHS	5.83	58.30	16.47	34.98	1.000	2.124 ✓
	256.51 (A) (684)	3/4 EHS	5.83	58.30	16.28	34.98	1.000	2.149 ✓
T3	256.51 (A) (685)	3/4 EHS	5.83	58.30	16.31	34.98	1.000	2.145 ✓
	256.51 (B) (680)	3/4 EHS	5.83	58.30	15.25	34.98	1.000	2.294 ✓
	256.51 (B) (681)	3/4 EHS	5.83	58.30	15.26	34.98	1.000	2.293 ✓



<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	71 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	Initial Tension K	Breaking Load K	Actual $T_u$ K	Allowable $\phi T_n$ K	Required S.F.	Actual S.F.
T5	256.51 (C) (676)	3/4 EHS	5.83	58.30	16.14	34.98	1.000	2.168 ✓
	256.51 (C) (677)	3/4 EHS	5.83	58.30	16.17	34.98	1.000	2.163 ✓
	216.51 (A) (696)	3/4 EHS	5.83	58.30	15.99	34.98	1.000	2.187 ✓
	216.51 (A) (697)	3/4 EHS	5.83	58.30	16.02	34.98	1.000	2.183 ✓
	216.51 (B) (692)	3/4 EHS	5.83	58.30	14.90	34.98	1.000	2.348 ✓
	216.51 (B) (693)	3/4 EHS	5.83	58.30	14.91	34.98	1.000	2.345 ✓
	216.51 (C) (688)	3/4 EHS	5.83	58.30	15.82	34.98	1.000	2.211 ✓
T7	216.51 (C) (689)	3/4 EHS	5.83	58.30	15.87	34.98	1.000	2.205 ✓
	166.51 (A) (708)	3/4 EHS	5.83	58.30	17.12	34.98	1.000	2.044 ✓
	166.51 (A) (709)	3/4 EHS	5.83	58.30	17.39	34.98	1.000	2.011 ✓
	166.51 (B) (704)	3/4 EHS	5.83	58.30	16.44	34.98	1.000	2.128 ✓
	166.51 (B) (705)	3/4 EHS	5.83	58.30	16.22	34.98	1.000	2.157 ✓
	166.51 (C) (700)	3/4 EHS	5.83	58.30	17.22	34.98	1.000	2.031 ✓
	166.51 (C) (701)	3/4 EHS	5.83	58.30	17.05	34.98	1.000	2.052 ✓
T10	106.51 (A) (720)	3/4 EHS	5.83	58.30	16.29	34.98	1.000	2.148 ✓
	106.51 (A) (721)	3/4 EHS	5.83	58.30	16.78	34.98	1.000	2.084 ✓
	106.51 (B) (716)	3/4 EHS	5.83	58.30	15.73	34.98	1.000	2.223 ✓
	106.51 (B) (717)	3/4 EHS	5.83	58.30	15.28	34.98	1.000	2.289 ✓
	106.51 (C) (712)	3/4 EHS	5.83	58.30	16.55	34.98	1.000	2.114 ✓
	106.51 (C) (713)	3/4 EHS	5.83	58.30	16.35	34.98	1.000	2.140 ✓
	56.51 (A) (732)	3/4 EHS	5.83	58.30	13.64	34.98	1.000	2.564 ✓
T13	56.51 (A) (733)	3/4 EHS	5.83	58.30	13.73	34.98	1.000	2.547 ✓
	56.51 (B) (728)	3/4 EHS	5.83	58.30	13.02	34.98	1.000	2.687 ✓
	56.51 (B) (729)	3/4 EHS	5.83	58.30	13.03	34.98	1.000	2.684 ✓
	56.51 (C) (724)	3/4 EHS	5.83	58.30	13.56	34.98	1.000	2.580 ✓
	56.51 (C) (725)	3/4 EHS	5.83	58.30	13.62	34.98	1.000	2.568 ✓

## Compression Checks

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	72 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

### Pole Design Data

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>Kl/r</i>	<i>A</i> <i>in<sup>2</sup></i>	<i>P<sub>u</sub></i> <i>K</i>	$\phi P_n$ <i>K</i>	Ratio $\frac{P_u}{\phi P_n}$
L1	327 - 291.84 (1)	P10.75x0.843	35.16	35.16	120.0	26.2373	-3.95	395.38	0.010

### Pole Bending Design Data

Section No.	Elevation <i>ft</i>	Size	<i>M<sub>ux</sub></i> <i>kip-ft</i>	$\phi M_{nx}$ <i>kip-ft</i>	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	<i>M<sub>uy</sub></i> <i>kip-ft</i>	$\phi M_{ny}$ <i>kip-ft</i>	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
L1	327 - 291.84 (1)	P10.75x0.843	38.96	217.72	0.179	0.00	217.72	0.000

### Pole Shear Design Data

Section No.	Elevation <i>ft</i>	Size	Actual <i>V<sub>u</sub></i> <i>K</i>	$\phi V_n$ <i>K</i>	Ratio $\frac{V_u}{\phi V_n}$	Actual <i>T<sub>u</sub></i> <i>kip-ft</i>	$\phi T_n$ <i>kip-ft</i>	Ratio $\frac{T_u}{\phi T_n}$
L1	327 - 291.84 (1)	P10.75x0.843	2.09	413.24	0.005	0.80	316.69	0.003

### Pole Interaction Design Data

Section No.	Elevation <i>ft</i>	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	Ratio $\frac{M_{uy}}{\phi M_{ny}}$	Ratio $\frac{V_u}{\phi V_n}$	Ratio $\frac{T_u}{\phi T_n}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	327 - 291.84 (1)	0.010	0.179	0.000	0.005	0.003	0.189 ✓	1.000	4.8.2 ✓

### Leg Design Data (Compression)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>Kl/r</i>	<i>A</i> <i>in<sup>2</sup></i>	Mast Stability Index	<i>P<sub>u</sub></i> <i>K</i>	$\phi P_n$ <i>K</i>	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	2	12.00	3.00	72.0 K=1.00	3.1416	1.00	-30.91	96.77	0.319 <sup>1</sup>
T2	279.84 - 259.84	2	20.00	3.33	80.0 K=1.00	3.1416	1.00	-36.55	88.54	0.413 <sup>1</sup>

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	73 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	Mast Stability Index	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T3	259.84 - 239.84	2 1/4	20.00	3.33	71.1 K=1.00	3.9761	1.00	-61.43	123.62	0.497 <sup>1</sup>
T4	239.84 - 219.84	2 1/4	20.00	3.33	71.1 K=1.00	3.9761	1.00	-63.52	123.62	0.514 <sup>1</sup>
T5	219.84 - 199.84	2 1/2	20.00	3.33	64.0 K=1.00	4.9087	1.00	-93.71	163.72	0.572 <sup>1</sup>
T6	199.84 - 179.84	2 1/2	20.00	3.33	64.0 K=1.00	4.9087	1.00	-102.03	163.72	0.623 <sup>1</sup>
T7	179.84 - 159.84	2 3/4	20.00	3.33	58.2 K=1.00	5.9396	1.00	-123.53	208.68	0.592 <sup>1</sup>
T8	159.84 - 139.84	2 1/2	20.00	3.33	64.0 K=1.00	4.9087	1.00	-130.23	163.72	0.795 <sup>1</sup>
T9	139.84 - 119.84	2 3/4	20.00	3.33	58.2 K=1.00	5.9396	1.00	-135.48	208.68	0.649 <sup>1</sup>
T10	119.84 - 99.84	2 3/4	20.00	3.33	58.2 K=1.00	5.9396	1.00	-155.11	208.68	0.743 <sup>1</sup>
T11	99.84 - 79.84	3	20.00	3.33	53.3 K=1.00	7.0686	1.00	-166.19	258.36	0.643 <sup>1</sup>
T12	79.84 - 59.84	3	20.00	3.33	53.3 K=1.00	7.0686	1.00	-168.94	258.36	0.654 <sup>1</sup>
T13	59.84 - 39.84	3	20.00	3.33	53.3 K=1.00	7.0686	1.00	-185.29	258.36	0.717 <sup>1</sup>
T14	39.84 - 19.84	3	20.00	3.33	53.3 K=1.00	7.0686	1.00	-190.22	258.36	0.736 <sup>1</sup>
T15	19.84 - 6.5	3	13.34	3.34	53.4 K=1.00	7.0686	1.00	-193.48	258.30	0.749 <sup>1</sup>
T16	6.5 - 0	3	6.84	2.10	67.3 K=2.00	7.0686	1.00	-204.80	228.38	0.897 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Leg Bending Design Data (Compression)

Section No.	Elevation ft	Size	M <sub>ux</sub> kip-ft	φM <sub>ux</sub> kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	M <sub>uy</sub> kip-ft	φM <sub>uy</sub> kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
T1	291.84 - 279.84	2	0.00	5.00	0.000	0.00	5.00	0.000
T2	279.84 - 259.84	2	0.00	5.00	0.000	0.00	5.00	0.000
T3	259.84 - 239.84	2 1/4	0.00	7.12	0.000	0.00	7.12	0.000
T4	239.84 - 219.84	2 1/4	0.00	7.12	0.000	0.00	7.12	0.000
T5	219.84 - 199.84	2 1/2	0.00	9.77	0.000	0.00	9.77	0.000
T6	199.84 - 179.84	2 1/2	0.00	9.77	0.000	0.00	9.77	0.000
T7	179.84 - 159.84	2 3/4	0.00	13.00	0.000	0.00	13.00	0.000
T8	159.84 - 139.84	2 1/2	0.00	9.77	0.000	0.00	9.77	0.000
T9	139.84 - 119.84	2 3/4	0.00	13.00	0.000	0.00	13.00	0.000
T10	119.84 - 99.84	2 3/4	0.00	13.00	0.000	0.00	13.00	0.000
T11	99.84 - 79.84	3	0.00	16.88	0.000	0.00	16.88	0.000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	74 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	$M_{ux}$ kip-ft	$\phi M_{ux}$ kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	$M_{uy}$ kip-ft	$\phi M_{uy}$ kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
T12	79.84 - 59.84	3	0.00	16.88	0.000	0.00	16.88	0.000
T13	59.84 - 39.84	3	0.00	16.88	0.000	0.00	16.88	0.000
T14	39.84 - 19.84	3	0.00	16.88	0.000	0.00	16.88	0.000
T15	19.84 - 6.5	3	0.00	16.88	0.000	0.00	16.88	0.000
T16	6.5 - 0	3	0.00	16.88	0.000	0.00	16.88	0.000

### Leg Interaction Design Data (Compression)

Section No.	Elevation ft	Size	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	Ratio $\frac{M_{uy}}{\phi M_{uy}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
T1	291.84 - 279.84	2	0.319	0.000	0.000	0.319 <sup>1</sup>	1.000	4.8.1 ✓
T2	279.84 - 259.84	2	0.413	0.000	0.000	0.413 <sup>1</sup>	1.000	4.8.1 ✓
T3	259.84 - 239.84	2 1/4	0.497	0.000	0.000	0.497 <sup>1</sup>	1.000	4.8.1 ✓
T4	239.84 - 219.84	2 1/4	0.514	0.000	0.000	0.514 <sup>1</sup>	1.000	4.8.1 ✓
T5	219.84 - 199.84	2 1/2	0.572	0.000	0.000	0.572 <sup>1</sup>	1.000	4.8.1 ✓
T6	199.84 - 179.84	2 1/2	0.623	0.000	0.000	0.623 <sup>1</sup>	1.000	4.8.1 ✓
T7	179.84 - 159.84	2 3/4	0.592	0.000	0.000	0.592 <sup>1</sup>	1.000	4.8.1 ✓
T8	159.84 - 139.84	2 1/2	0.795	0.000	0.000	0.795 <sup>1</sup>	1.000	4.8.1 ✓
T9	139.84 - 119.84	2 3/4	0.649	0.000	0.000	0.649 <sup>1</sup>	1.000	4.8.1 ✓
T10	119.84 - 99.84	2 3/4	0.743	0.000	0.000	0.743 <sup>1</sup>	1.000	4.8.1 ✓
T11	99.84 - 79.84	3	0.643	0.000	0.000	0.643 <sup>1</sup>	1.000	4.8.1 ✓
T12	79.84 - 59.84	3	0.654	0.000	0.000	0.654 <sup>1</sup>	1.000	4.8.1 ✓
T13	59.84 - 39.84	3	0.717	0.000	0.000	0.717 <sup>1</sup>	1.000	4.8.1 ✓
T14	39.84 - 19.84	3	0.736	0.000	0.000	0.736 <sup>1</sup>	1.000	4.8.1 ✓
T15	19.84 - 6.5	3	0.749	0.000	0.000	0.749 <sup>1</sup>	1.000	4.8.1 ✓
T16	6.5 - 0	3	0.897	0.000	0.000	0.897 <sup>1</sup>	1.000	4.8.1 ✓

<sup>1</sup>  $P_u / \phi P_n$  controls

### Diagonal Design Data (Compression)

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	75 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	1 3/8	4.74	4.52	110.6 K=0.70	1.4849	-3.91	25.28	0.155 <sup>1</sup>
T2	279.84 - 259.84	1 3/8	4.96	4.73	115.6 K=0.70	1.4849	-2.94	23.79	0.123 <sup>1</sup>
T3	259.84 - 239.84	1 3/8	4.96	4.70	115.0 K=0.70	1.4849	-4.50	23.99	0.188 <sup>1</sup>
T4	239.84 - 219.84	1 3/8	4.96	4.70	115.0 K=0.70	1.4849	-3.39	23.99	0.141 <sup>1</sup>
T5	219.84 - 199.84	1 1/2	4.96	4.68	104.8 K=0.70	1.7672	-8.44	32.13	0.263 <sup>1</sup>
T6	199.84 - 179.84	1 1/4	4.96	4.68	125.7 K=0.70	1.2272	-6.82	17.31	0.394 <sup>1</sup>
T7	179.84 - 159.84	1 1/2	4.96	4.65	104.1 K=0.70	1.7672	-8.96	32.36	0.277 <sup>1</sup>
T8	159.84 - 139.84	1 3/8	4.96	4.68	114.3 K=0.70	1.4849	-5.20	24.19	0.215 <sup>1</sup>
T9	139.84 - 119.84	1 1/4	4.96	4.65	124.9 K=0.70	1.2272	-5.53	17.48	0.316 <sup>1</sup>
T10	119.84 - 99.84	1 1/2	4.96	4.65	104.1 K=0.70	1.7672	-9.95	32.36	0.307 <sup>1</sup>
T11	99.84 - 79.84	1 3/8	4.96	4.62	112.9 K=0.70	1.4849	-7.48	24.59	0.304 <sup>1</sup>
T12	79.84 - 59.84	1 1/4	4.96	4.62	124.2 K=0.70	1.2272	-8.01	17.65	0.454 <sup>1</sup>
T13	59.84 - 39.84	1 1/4	4.96	4.62	124.2 K=0.70	1.2272	-8.11	17.65	0.459 <sup>1</sup>
T14	39.84 - 19.84	1 1/4	4.96	4.62	124.2 K=0.70	1.2272	-4.20	17.65	0.238 <sup>1</sup>
T15	19.84 - 6.5	1 1/4	4.96	4.62	124.2 K=0.70	1.2272	-4.40	17.65	0.249 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	1	3.67	3.50	117.7 K=0.70	0.7854	-0.67	12.27	0.054 <sup>1</sup>
T2	279.84 - 259.84	1	3.67	3.50	117.7 K=0.70	0.7854	-0.46	12.27	0.038 <sup>1</sup>
T3	259.84 - 239.84	1	3.67	3.48	117.0 K=0.70	0.7854	-0.95	12.38	0.077 <sup>1</sup>
T4	239.84 - 219.84	1	3.67	3.48	117.0 K=0.70	0.7854	-0.11	12.38	0.009 <sup>1</sup>
T5	219.84 - 199.84	1	3.67	3.46	116.3 K=0.70	0.7854	-0.47	12.48	0.038 <sup>1</sup>
T6	199.84 -	1	3.67	3.46	116.3	0.7854	-1.60	12.48	0.128 <sup>1</sup>

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	76 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
	179.84				K=0.70				✓
T7	179.84 - 159.84	1	3.67	3.44	115.6 K=0.70	0.7854	-0.31	12.59	0.025 <sup>1</sup> ✓
T10	119.84 - 99.84	1	3.67	3.44	115.6 K=0.70	0.7854	-0.63	12.59	0.050 <sup>1</sup> ✓
T11	99.84 - 79.84	1	3.67	3.42	114.9 K=0.70	0.7854	-0.77	12.70	0.061 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Secondary Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	1	1.84	1.75	81.5 K=0.97	0.7854	-0.00	17.94	0.000 <sup>1</sup> ✓
T2	279.84 - 259.84	1	1.84	1.75	81.5 K=0.97	0.7854	-0.00	17.94	0.000 <sup>1</sup> ✓
T3	259.84 - 239.84	1	1.84	1.74	81.3 K=0.97	0.7854	-0.00	17.96	0.000 <sup>1</sup> ✓
T4	239.84 - 219.84	1	1.84	1.74	81.3 K=0.97	0.7854	-0.00	17.96	0.000 <sup>1</sup> ✓
T5	219.84 - 199.84	1	1.84	1.73	81.2 K=0.98	0.7854	-0.00	17.99	0.000 <sup>1</sup> ✓
T6	199.84 - 179.84	1	1.84	1.73	81.2 K=0.98	0.7854	-0.00	17.99	0.000 <sup>1</sup> ✓
T7	179.84 - 159.84	1	1.84	1.72	81.0 K=0.98	0.7854	-0.00	18.02	0.000 <sup>1</sup> ✓
T8	159.84 - 139.84	1	1.84	1.73	81.2 K=0.98	0.7854	-0.00	17.99	0.000 <sup>1</sup> ✓
T9	139.84 - 119.84	1	1.84	1.72	81.0 K=0.98	0.7854	-0.00	18.02	0.000 <sup>1</sup> ✓
T10	119.84 - 99.84	1	1.84	1.72	81.0 K=0.98	0.7854	-0.00	18.02	0.000 <sup>1</sup> ✓
T11	99.84 - 79.84	1	1.84	1.71	80.8 K=0.98	0.7854	-0.00	18.05	0.000 <sup>1</sup> ✓
T12	79.84 - 59.84	1	1.84	1.71	80.8 K=0.98	0.7854	-0.00	18.05	0.000 <sup>1</sup> ✓
T13	59.84 - 39.84	1	1.84	1.71	80.8 K=0.98	0.7854	-0.00	18.05	0.000 <sup>1</sup> ✓
T14	39.84 - 19.84	1	1.84	1.71	80.8 K=0.98	0.7854	-0.00	18.05	0.000 <sup>1</sup> ✓
T15	19.84 - 6.5	1	1.84	1.71	80.8 K=0.98	0.7854	-0.00	18.05	0.000 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	77 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

### Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	1	3.67	3.50	117.7 K=0.70	0.7854	-0.00	12.27	0.000 <sup>1</sup> ✓
T4	239.84 - 219.84	1	3.67	3.48	117.0 K=0.70	0.7854	-0.22	12.38	0.018 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Bottom Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T16	6.5 - 0	12x3/8	0.28	0.03	3.6 K=1.00	4.5000	-6.05	145.70	0.041 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Mid Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T16	6.5 - 0	9x3/8	1.41	1.16	128.8 K=1.00	3.3750	-0.37	45.69	0.008 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Guy Pull-Off Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	MC12x35	3.67	3.50	37.9 K=1.00	10.3000	-4.18	309.45	0.014 <sup>1</sup>
T3	259.84 - 239.84	MC12x35	3.67	3.48	37.6 K=1.00	10.3000	-5.09	309.72	0.016 <sup>1</sup>
T5	219.84 - 199.84	MC12x35	3.67	3.46	37.4 K=1.00	10.3000	-7.70	310.00	0.025 <sup>1</sup>
T7	179.84 - 159.84	MC12x35	3.67	3.44	37.2 K=1.00	10.3000	-11.41	310.27	0.037 <sup>1</sup>
T10	119.84 - 99.84	MC12x35	3.67	3.44	37.2	10.3000	-13.28	310.27	0.043 <sup>1</sup>

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	78 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T13	59.84 - 39.84	MC12x35	3.67	3.42	K=1.00 37.0 K=1.00	10.3000	-10.17	310.55	0.033 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Guy Pull-Off Bending Design Data

Section No.	Elevation ft	Size	M <sub>ux</sub> kip-ft	φM <sub>ux</sub> kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	M <sub>uy</sub> kip-ft	φM <sub>uy</sub> kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
T1	291.84 - 279.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000
T3	259.84 - 239.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000
T5	219.84 - 199.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000
T7	179.84 - 159.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000
T10	119.84 - 99.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000
T13	59.84 - 39.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000

### Top Guy Pull-Off Interaction Design Data

Section No.	Elevation ft	Size	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	Ratio $\frac{M_{uy}}{\phi M_{uy}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
T1	291.84 - 279.84	MC12x35	0.014	0.000	0.000	0.014 <sup>1</sup>	1.000	4.8.1 ✓
T3	259.84 - 239.84	MC12x35	0.016	0.000	0.000	0.016 <sup>1</sup>	1.000	4.8.1 ✓
T5	219.84 - 199.84	MC12x35	0.025	0.000	0.000	0.025 <sup>1</sup>	1.000	4.8.1 ✓
T7	179.84 - 159.84	MC12x35	0.037	0.000	0.000	0.037 <sup>1</sup>	1.000	4.8.1 ✓
T10	119.84 - 99.84	MC12x35	0.043	0.000	0.000	0.043 <sup>1</sup>	1.000	4.8.1 ✓
T13	59.84 - 39.84	MC12x35	0.033	0.000	0.000	0.033 <sup>1</sup>	1.000	4.8.1 ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Torque-Arm Top Design Data



<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	79 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84 (666)	MC12x35	4.00	3.92	42.4 K=1.00	10.3000	-0.31	303.60	0.001
T1	291.84 - 279.84 (667)	MC12x35	4.00	3.92	42.4 K=1.00	10.3000	-0.39	303.60	0.001
T1	291.84 - 279.84 (670)	MC12x35	4.00	3.92	42.4 K=1.00	10.3000	-0.35	303.60	0.001
T1	291.84 - 279.84 (671)	MC12x35	4.00	3.92	42.4 K=1.00	10.3000	-0.44	303.60	0.001
T1	291.84 - 279.84 (674)	MC12x35	4.00	3.92	42.4 K=1.00	10.3000	-0.29	303.60	0.001
T1	291.84 - 279.84 (675)	MC12x35	4.00	3.92	42.4 K=1.00	10.3000	-0.30	303.60	0.001
T3	259.84 - 239.84 (678)	MC12x35	4.00	3.91	42.3 K=1.00	10.3000	-1.01	303.75	0.003
T3	259.84 - 239.84 (679)	MC12x35	4.00	3.91	42.3 K=1.00	10.3000	-0.89	303.75	0.003
T3	259.84 - 239.84 (682)	MC12x35	4.00	3.91	42.3 K=1.00	10.3000	-0.93	303.75	0.003
T3	259.84 - 239.84 (683)	MC12x35	4.00	3.91	42.3 K=1.00	10.3000	-0.99	303.75	0.003
T3	259.84 - 239.84 (686)	MC12x35	4.00	3.91	42.3 K=1.00	10.3000	-0.85	303.75	0.003
T3	259.84 - 239.84 (687)	MC12x35	4.00	3.91	42.3 K=1.00	10.3000	-0.94	303.75	0.003
T5	219.84 - 199.84 (690)	MC12x35	4.00	3.90	42.2 K=1.00	10.3000	-2.56	303.90	0.008
T5	219.84 - 199.84 (691)	MC12x35	4.00	3.90	42.2 K=1.00	10.3000	-2.42	303.90	0.008
T5	219.84 - 199.84 (694)	MC12x35	4.00	3.90	42.2 K=1.00	10.3000	-2.34	303.90	0.008
T5	219.84 - 199.84 (695)	MC12x35	4.00	3.90	42.2 K=1.00	10.3000	-2.43	303.90	0.008
T5	219.84 - 199.84 (698)	MC12x35	4.00	3.90	42.2 K=1.00	10.3000	-2.24	303.90	0.007
T5	219.84 - 199.84 (699)	MC12x35	4.00	3.90	42.2 K=1.00	10.3000	-2.37	303.90	0.008
T7	179.84 - 159.84 (702)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-4.75	304.05	0.016
T7	179.84 - 159.84 (703)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-4.84	304.05	0.016
T7	179.84 - 159.84 (706)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-4.53	304.05	0.015
T7	179.84 - 159.84 (707)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-4.54	304.05	0.015
T7	179.84 - 159.84 (710)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-4.30	304.05	0.014
T7	179.84 - 159.84 (711)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-4.52	304.05	0.015
T10	119.84 - 99.84 (714)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-5.60	304.05	0.018
T10	119.84 - 99.84 (715)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-5.82	304.05	0.019
T10	119.84 - 99.84 (718)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-5.30	304.05	0.017
T10	119.84 - 99.84 (719)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-5.34	304.05	0.018
T10	119.84 - 99.84 (722)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-4.75	304.05	0.016
T10	119.84 - 99.84 (723)	MC12x35	4.00	3.89	42.1 K=1.00	10.3000	-5.12	304.05	0.017

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	80 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T13	59.84 - 39.84 (726)	MC12x35	4.00	3.88	41.9 K=1.00	10.3000	-4.01	304.20	0.013
T13	59.84 - 39.84 (727)	MC12x35	4.00	3.88	41.9 K=1.00	10.3000	-5.06	304.20	0.017
T13	59.84 - 39.84 (730)	MC12x35	4.00	3.88	41.9 K=1.00	10.3000	-3.55	304.20	0.012
T13	59.84 - 39.84 (731)	MC12x35	4.00	3.88	41.9 K=1.00	10.3000	-3.49	304.20	0.011
T13	59.84 - 39.84 (734)	MC12x35	4.00	3.88	41.9 K=1.00	10.3000	-2.97	304.20	0.010
T13	59.84 - 39.84 (735)	MC12x35	4.00	3.88	41.9 K=1.00	10.3000	-3.12	304.20	0.010

### Torque-Arm Top Bending Design Data

Section No.	Elevation ft	Size	M <sub>ux</sub> kip-ft	φM <sub>ux</sub> kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	M <sub>uy</sub> kip-ft	φM <sub>uy</sub> kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
T1	291.84 - 279.84 (666)	MC12x35	-45.77	97.47	0.470	0.00	21.36	0.000
T1	291.84 - 279.84 (667)	MC12x35	-46.29	97.47	0.475	0.00	21.36	0.000
T1	291.84 - 279.84 (670)	MC12x35	-45.84	97.47	0.470	0.00	21.36	0.000
T1	291.84 - 279.84 (671)	MC12x35	-42.95	97.47	0.441	-0.00	21.36	0.000
T1	291.84 - 279.84 (674)	MC12x35	-43.05	97.47	0.442	-0.00	21.36	0.000
T1	291.84 - 279.84 (675)	MC12x35	-46.16	97.47	0.474	0.00	21.36	0.000
T3	259.84 - 239.84 (678)	MC12x35	-44.49	97.47	0.456	0.00	21.36	0.000
T3	259.84 - 239.84 (679)	MC12x35	-44.14	97.47	0.453	0.00	21.36	0.000
T3	259.84 - 239.84 (682)	MC12x35	-40.59	97.47	0.416	-0.00	21.36	0.000
T3	259.84 - 239.84 (683)	MC12x35	-43.96	97.47	0.451	0.00	21.36	0.000
T3	259.84 - 239.84 (686)	MC12x35	-40.51	97.47	0.416	-0.00	21.36	0.000
T3	259.84 - 239.84 (687)	MC12x35	-44.64	97.47	0.458	0.00	21.36	0.000
T5	219.84 - 199.84 (690)	MC12x35	-44.29	97.47	0.454	0.00	21.36	0.000
T5	219.84 - 199.84 (691)	MC12x35	-43.85	97.47	0.450	0.00	21.36	0.000
T5	219.84 - 199.84 (694)	MC12x35	-39.28	97.47	0.403	-0.00	21.36	0.000
T5	219.84 - 199.84 (695)	MC12x35	-43.60	97.47	0.447	0.00	21.36	0.000
T5	219.84 - 199.84 (698)	MC12x35	-39.13	97.47	0.401	-0.00	21.36	0.000
T5	219.84 - 199.84 (699)	MC12x35	-44.37	97.47	0.455	0.00	21.36	0.000
T7	179.84 - 159.84 (702)	MC12x35	-42.40	97.47	0.435	-0.00	21.36	0.000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	81 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	$M_{ux}$ kip-ft	$\phi M_{ux}$ kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	$M_{uy}$ kip-ft	$\phi M_{uy}$ kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
T7	179.84 - 159.84 (703)	MC12x35	-43.31	97.47	0.444	0.00	21.36	0.000
T7	179.84 - 159.84 (706)	MC12x35	-42.46	97.47	0.436	-0.00	21.36	0.000
T7	179.84 - 159.84 (707)	MC12x35	-36.34	97.47	0.373	-0.00	21.36	0.000
T7	179.84 - 159.84 (710)	MC12x35	-36.47	97.47	0.374	-0.00	21.36	0.000
T7	179.84 - 159.84 (711)	MC12x35	-43.03	97.47	0.441	0.00	21.36	0.000
T10	119.84 - 99.84 (714)	MC12x35	-31.52	97.47	0.323	-0.00	21.36	0.000
T10	119.84 - 99.84 (715)	MC12x35	-32.45	97.47	0.333	0.00	21.36	0.000
T10	119.84 - 99.84 (718)	MC12x35	-31.64	97.47	0.325	-0.00	21.36	0.000
T10	119.84 - 99.84 (719)	MC12x35	-24.37	97.47	0.250	-0.00	21.36	0.000
T10	119.84 - 99.84 (722)	MC12x35	-24.31	97.47	0.249	-0.00	21.36	0.000
T10	119.84 - 99.84 (723)	MC12x35	-32.15	97.47	0.330	0.00	21.36	0.000
T13	59.84 - 39.84 (726)	MC12x35	-17.59	97.47	0.180	0.00	21.36	0.000
T13	59.84 - 39.84 (727)	MC12x35	-16.79	97.47	0.172	-0.00	21.36	0.000
T13	59.84 - 39.84 (730)	MC12x35	-10.87	97.47	0.112	-0.00	21.36	0.000
T13	59.84 - 39.84 (731)	MC12x35	-16.88	97.47	0.173	0.00	21.36	0.000
T13	59.84 - 39.84 (734)	MC12x35	-10.77	97.47	0.111	0.00	21.36	0.000
T13	59.84 - 39.84 (735)	MC12x35	-17.57	97.47	0.180	0.00	21.36	0.000

### Torque-Arm Top Interaction Design Data

Section No.	Elevation ft	Size	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	Ratio $\frac{M_{uy}}{\phi M_{uy}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
T1	291.84 - 279.84 (666)	MC12x35	0.001	0.470	0.000	0.470	1.000	4.8.1 ✓
T1	291.84 - 279.84 (667)	MC12x35	0.001	0.475	0.000	0.476	1.000	4.8.1 ✓
T1	291.84 - 279.84 (670)	MC12x35	0.001	0.470	0.000	0.471	1.000	4.8.1 ✓
T1	291.84 - 279.84 (671)	MC12x35	0.001	0.441	0.000	0.441	1.000	4.8.1 ✓
T1	291.84 - 279.84 (674)	MC12x35	0.001	0.442	0.000	0.442	1.000	4.8.1 ✓
T1	291.84 - 279.84 (675)	MC12x35	0.001	0.474	0.000	0.474	1.000	4.8.1 ✓
T3	259.84 - 239.84 (678)	MC12x35	0.003	0.456	0.000	0.458	1.000	4.8.1 ✓

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	82 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	Ratio $P_u$ $\phi P_n$	Ratio $M_{ux}$ $\phi M_{nx}$	Ratio $M_{uy}$ $\phi M_{ny}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
T3	259.84 - 239.84 (679)	MC12x35	0.003	0.453	0.000	0.454 ✓	1.000	4.8.1 ✓
T3	259.84 - 239.84 (682)	MC12x35	0.003	0.416	0.000	0.418 ✓	1.000	4.8.1 ✓
T3	259.84 - 239.84 (683)	MC12x35	0.003	0.451	0.000	0.453 ✓	1.000	4.8.1 ✓
T3	259.84 - 239.84 (686)	MC12x35	0.003	0.416	0.000	0.417 ✓	1.000	4.8.1 ✓
T3	259.84 - 239.84 (687)	MC12x35	0.003	0.458	0.000	0.460 ✓	1.000	4.8.1 ✓
T5	219.84 - 199.84 (690)	MC12x35	0.008	0.454	0.000	0.459 ✓	1.000	4.8.1 ✓
T5	219.84 - 199.84 (691)	MC12x35	0.008	0.450	0.000	0.454 ✓	1.000	4.8.1 ✓
T5	219.84 - 199.84 (694)	MC12x35	0.008	0.403	0.000	0.407 ✓	1.000	4.8.1 ✓
T5	219.84 - 199.84 (695)	MC12x35	0.008	0.447	0.000	0.451 ✓	1.000	4.8.1 ✓
T5	219.84 - 199.84 (698)	MC12x35	0.007	0.401	0.000	0.405 ✓	1.000	4.8.1 ✓
T5	219.84 - 199.84 (699)	MC12x35	0.008	0.455	0.000	0.459 ✓	1.000	4.8.1 ✓
T7	179.84 - 159.84 (702)	MC12x35	0.016	0.435	0.000	0.443 ✓	1.000	4.8.1 ✓
T7	179.84 - 159.84 (703)	MC12x35	0.016	0.444	0.000	0.452 ✓	1.000	4.8.1 ✓
T7	179.84 - 159.84 (706)	MC12x35	0.015	0.436	0.000	0.443 ✓	1.000	4.8.1 ✓
T7	179.84 - 159.84 (707)	MC12x35	0.015	0.373	0.000	0.380 ✓	1.000	4.8.1 ✓
T7	179.84 - 159.84 (710)	MC12x35	0.014	0.374	0.000	0.381 ✓	1.000	4.8.1 ✓
T7	179.84 - 159.84 (711)	MC12x35	0.015	0.441	0.000	0.449 ✓	1.000	4.8.1 ✓
T10	119.84 - 99.84 (714)	MC12x35	0.018	0.323	0.000	0.333 ✓	1.000	4.8.1 ✓
T10	119.84 - 99.84 (715)	MC12x35	0.019	0.333	0.000	0.342 ✓	1.000	4.8.1 ✓
T10	119.84 - 99.84 (718)	MC12x35	0.017	0.325	0.000	0.333 ✓	1.000	4.8.1 ✓
T10	119.84 - 99.84 (719)	MC12x35	0.018	0.250	0.000	0.259 ✓	1.000	4.8.1 ✓
T10	119.84 - 99.84 (722)	MC12x35	0.016	0.249	0.000	0.257 ✓	1.000	4.8.1 ✓
T10	119.84 - 99.84 (723)	MC12x35	0.017	0.330	0.000	0.338 ✓	1.000	4.8.1 ✓
T13	59.84 - 39.84 (726)	MC12x35	0.013	0.180	0.000	0.187 ✓	1.000	4.8.1 ✓
T13	59.84 - 39.84 (727)	MC12x35	0.017	0.172	0.000	0.181 ✓	1.000	4.8.1 ✓
T13	59.84 - 39.84 (730)	MC12x35	0.012	0.112	0.000	0.117 ✓	1.000	4.8.1 ✓

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	83 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	Ratio $\frac{M_{uy}}{\phi M_{ny}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
T13	59.84 - 39.84 (731)	MC12x35	0.011	0.173	0.000	0.179 ✓	1.000	4.8.1 ✓
T13	59.84 - 39.84 (734)	MC12x35	0.010	0.111	0.000	0.115 ✓	1.000	4.8.1 ✓
T13	59.84 - 39.84 (735)	MC12x35	0.010	0.180	0.000	0.185 ✓	1.000	4.8.1 ✓

### Tension Checks

### Leg Design Data (Tension)

Section No.	Elevation ft	Size	$L$ ft	$L_u$ ft	$Kl/r$	$A$ in <sup>2</sup>	$P_u$ K	$\phi P_n$ K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	2	12.00	3.00	72.0	3.1416	15.82	141.37	0.112 <sup>1</sup>
T3	259.84 - 239.84	2 1/4	20.00	3.33	71.1	3.9761	1.75	178.92	0.010 <sup>1</sup>

<sup>1</sup>  $P_u / \phi P_n$  controls

### Leg Bending Design Data (Tension)

Section No.	Elevation ft	Size	$M_{ux}$ kip-ft	$\phi M_{nx}$ kip-ft	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	$M_{uy}$ kip-ft	$\phi M_{ny}$ kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
T1	291.84 - 279.84	2	0.00	5.00	0.000	0.00	5.00	0.000
T3	259.84 - 239.84	2 1/4	0.00	7.12	0.000	0.00	7.12	0.000

### Leg Interaction Design Data (Tension)

Section No.	Elevation ft	Size	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	Ratio $\frac{M_{uy}}{\phi M_{ny}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
T1	291.84 - 279.84	2	0.112	0.000	0.000	0.112 <sup>1</sup> ✓	1.000	4.8.1 ✓
T3	259.84 - 239.84	2 1/4	0.010	0.000	0.000	0.010 <sup>1</sup> ✓	1.000	4.8.1 ✓

<sup>1</sup>  $P_u / \phi P_n$  controls

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 17004.42 - CT1077	<b>Page</b> 84 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

### Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	1 3/8	4.74	4.52	158.0	1.4849	3.95	48.11	0.082 <sup>1</sup>
T2	279.84 - 259.84	1 3/8	4.96	4.73	165.2	1.4849	2.61	48.11	0.054 <sup>1</sup>
T3	259.84 - 239.84	1 3/8	4.96	4.70	164.2	1.4849	3.77	48.11	0.078 <sup>1</sup>
T4	239.84 - 219.84	1 3/8	4.96	4.70	164.2	1.4849	2.77	48.11	0.058 <sup>1</sup>
T5	219.84 - 199.84	1 1/2	4.96	4.68	149.6	1.7672	7.18	57.26	0.125 <sup>1</sup>
T6	199.84 - 179.84	1 1/4	4.96	4.68	179.6	1.2272	5.44	39.76	0.137 <sup>1</sup>
T7	179.84 - 159.84	1 1/2	4.96	4.65	148.7	1.7672	7.86	57.26	0.137 <sup>1</sup>
T8	159.84 - 139.84	1 3/8	4.96	4.68	163.2	1.4849	3.84	48.11	0.080 <sup>1</sup>
T9	139.84 - 119.84	1 1/4	4.96	4.65	178.5	1.2272	3.55	39.76	0.089 <sup>1</sup>
T10	119.84 - 99.84	1 1/2	4.96	4.65	148.7	1.7672	8.82	57.26	0.154 <sup>1</sup>
T11	99.84 - 79.84	1 3/8	4.96	4.62	161.3	1.4849	4.99	48.11	0.104 <sup>1</sup>
T12	79.84 - 59.84	1 1/4	4.96	4.62	177.4	1.2272	6.02	39.76	0.151 <sup>1</sup>
T13	59.84 - 39.84	1 1/4	4.96	4.62	177.4	1.2272	5.82	39.76	0.146 <sup>1</sup>
T14	39.84 - 19.84	1 1/4	4.96	4.62	177.4	1.2272	2.01	39.76	0.051 <sup>1</sup>
T15	19.84 - 6.5	1 1/4	4.96	4.62	177.5	1.2272	3.15	39.76	0.079 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	1	3.67	3.50	168.2	0.7854	1.00	25.45	0.039 <sup>1</sup>
T2	279.84 - 259.84	1	3.67	3.50	168.2	0.7854	0.60	25.45	0.024 <sup>1</sup>
T3	259.84 - 239.84	1	3.67	3.48	167.2	0.7854	0.71	25.45	0.028 <sup>1</sup>
T4	239.84 -	1	3.67	3.48	167.2	0.7854	0.49	25.45	0.019 <sup>1</sup>

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	85 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T5	219.84 - 199.84	1	3.67	3.46	166.2	0.7854	0.89	25.45	0.035 <sup>1</sup>
T6	199.84 - 179.84	1	3.67	3.46	166.2	0.7854	2.32	25.45	0.091 <sup>1</sup>
T7	179.84 - 159.84	1	3.67	3.44	165.2	0.7854	1.48	25.45	0.058 <sup>1</sup>
T8	159.84 - 139.84	1	3.67	3.46	166.2	0.7854	0.91	25.45	0.036 <sup>1</sup>
T9	139.84 - 119.84	1	3.67	3.44	165.2	0.7854	1.15	25.45	0.045 <sup>1</sup>
T10	119.84 - 99.84	1	3.67	3.44	165.2	0.7854	2.00	25.45	0.078 <sup>1</sup>
T11	99.84 - 79.84	1	3.67	3.42	164.2	0.7854	2.25	25.45	0.089 <sup>1</sup>
T12	79.84 - 59.84	1	3.67	3.42	164.2	0.7854	1.58	25.45	0.062 <sup>1</sup>
T13	59.84 - 39.84	1	3.67	3.42	164.2	0.7854	1.76	25.45	0.069 <sup>1</sup>
T14	39.84 - 19.84	1	3.67	3.42	164.2	0.7854	1.73	25.45	0.068 <sup>1</sup>
T15	19.84 - 6.5	1	3.67	3.42	164.2	0.7854	1.73	25.45	0.068 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

## Secondary Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	1	1.84	1.75	84.1	0.7854	0.00	25.45	0.000 <sup>1</sup>
T2	279.84 - 259.84	1	1.84	1.75	84.1	0.7854	0.00	25.45	0.000 <sup>1</sup>
T3	259.84 - 239.84	1	1.84	1.74	83.6	0.7854	0.00	25.45	0.000 <sup>1</sup>
T4	239.84 - 219.84	1	1.84	1.74	83.6	0.7854	0.00	25.45	0.000 <sup>1</sup>
T5	219.84 - 199.84	1	1.84	1.73	83.1	0.7854	0.00	25.45	0.000 <sup>1</sup>
T6	199.84 - 179.84	1	1.84	1.73	83.1	0.7854	0.00	25.45	0.000 <sup>1</sup>
T7	179.84 - 159.84	1	1.84	1.72	82.6	0.7854	0.00	25.45	0.000 <sup>1</sup>
T8	159.84 - 139.84	1	1.84	1.73	83.1	0.7854	0.00	25.45	0.000 <sup>1</sup>
T9	139.84 - 119.84	1	1.84	1.72	82.6	0.7854	0.00	25.45	0.000 <sup>1</sup>

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	86 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T10	119.84 - 99.84	1	1.84	1.72	82.6	0.7854	0.00	25.45	0.000 <sup>1</sup>
T11	99.84 - 79.84	1	1.84	1.71	82.1	0.7854	0.00	25.45	0.000 <sup>1</sup>
T12	79.84 - 59.84	1	1.84	1.71	82.1	0.7854	0.00	25.45	0.000 <sup>1</sup>
T13	59.84 - 39.84	1	1.84	1.71	82.1	0.7854	0.00	25.45	0.000 <sup>1</sup>
T14	39.84 - 19.84	1	1.84	1.71	82.1	0.7854	0.00	25.45	0.000 <sup>1</sup>
T15	19.84 - 6.5	1	1.84	1.71	82.1	0.7854	0.00	25.45	0.000 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	1	3.67	3.50	168.2	0.7854	0.00	25.45	0.000 <sup>1</sup>
T2	279.84 - 259.84	1	3.67	3.50	168.2	0.7854	0.40	25.45	0.016 <sup>1</sup>
T3	259.84 - 239.84	1	3.67	3.50	168.2	0.7854	0.80	25.45	0.032 <sup>1</sup>
T4	239.84 - 219.84	1	3.67	3.48	167.2	0.7854	0.65	25.45	0.025 <sup>1</sup>
T5	219.84 - 199.84	1	3.67	3.48	167.2	0.7854	1.02	25.45	0.040 <sup>1</sup>
T6	199.84 - 179.84	1	3.67	3.46	166.2	0.7854	0.73	25.45	0.029 <sup>1</sup>
T7	179.84 - 159.84	1	3.67	3.46	166.2	0.7854	0.78	25.45	0.031 <sup>1</sup>
T8	159.84 - 139.84	1	3.67	3.44	165.2	0.7854	1.10	25.45	0.043 <sup>1</sup>
T9	139.84 - 119.84	1	3.67	3.46	166.2	0.7854	0.98	25.45	0.039 <sup>1</sup>
T10	119.84 - 99.84	1	3.67	3.44	165.2	0.7854	1.13	25.45	0.044 <sup>1</sup>
T11	99.84 - 79.84	1	3.67	3.44	165.2	0.7854	1.51	25.45	0.059 <sup>1</sup>
T12	79.84 - 59.84	1	3.67	3.42	164.2	0.7854	1.54	25.45	0.061 <sup>1</sup>
T13	59.84 - 39.84	1	3.67	3.42	164.2	0.7854	1.86	25.45	0.073 <sup>1</sup>
T14	39.84 - 19.84	1	3.67	3.42	164.2	0.7854	1.69	25.45	0.066 <sup>1</sup>
T15	19.84 - 6.5	1	3.67	3.42	164.2	0.7854	1.74	25.45	0.068 <sup>1</sup>



<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	87 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T16	6.5 - 0	12x3/8	3.67	3.42	379.1	4.5000	36.72	145.80	0.252 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Mid Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T16	6.5 - 0	9x3/8	2.54	2.29	253.9	3.3750	0.56	109.35	0.005 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Guy Pull-Off Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84	MC12x35	3.67	3.50	37.9	10.3000	4.78	333.72	0.014 <sup>1</sup>
T3	259.84 - 239.84	MC12x35	3.67	3.48	37.6	10.3000	5.90	333.72	0.018 <sup>1</sup>
T5	219.84 - 199.84	MC12x35	3.67	3.46	37.4	10.3000	8.91	333.72	0.027 <sup>1</sup>
T7	179.84 - 159.84	MC12x35	3.67	3.44	37.2	10.3000	12.94	333.72	0.039 <sup>1</sup>
T10	119.84 - 99.84	MC12x35	3.67	3.44	37.2	10.3000	14.79	333.72	0.044 <sup>1</sup>
T13	59.84 - 39.84	MC12x35	3.67	3.42	37.0	10.3000	12.72	333.72	0.038 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Guy Pull-Off Bending Design Data

Section No.	Elevation ft	Size	M <sub>ux</sub> kip-ft	φM <sub>nx</sub> kip-ft	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	M <sub>uy</sub> kip-ft	φM <sub>ny</sub> kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
T1	291.84 - 279.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000
T3	259.84 - 239.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000
T5	219.84 - 199.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000
T7	179.84 -	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	88 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	$M_{ux}$ kip-ft	$\phi M_{nx}$ kip-ft	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	$M_{uy}$ kip-ft	$\phi M_{ny}$ kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
	159.84							
T10	119.84 - 99.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000
T13	59.84 - 39.84	MC12x35	0.00	97.47	0.000	0.00	21.36	0.000

### Top Guy Pull-Off Interaction Design Data

Section No.	Elevation ft	Size	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	Ratio $\frac{M_{uy}}{\phi M_{ny}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
T1	291.84 - 279.84	MC12x35	0.014	0.000	0.000	0.014 <sup>1</sup>	1.000	4.8.1 ✓
T3	259.84 - 239.84	MC12x35	0.018	0.000	0.000	0.018 <sup>1</sup>	1.000	4.8.1 ✓
T5	219.84 - 199.84	MC12x35	0.027	0.000	0.000	0.027 <sup>1</sup>	1.000	4.8.1 ✓
T7	179.84 - 159.84	MC12x35	0.039	0.000	0.000	0.039 <sup>1</sup>	1.000	4.8.1 ✓
T10	119.84 - 99.84	MC12x35	0.044	0.000	0.000	0.044 <sup>1</sup>	1.000	4.8.1 ✓
T13	59.84 - 39.84	MC12x35	0.038	0.000	0.000	0.038 <sup>1</sup>	1.000	4.8.1 ✓

<sup>1</sup>  $P_u / \phi P_n$  controls

### Torque-Arm Top Design Data

Section No.	Elevation ft	Size	$L$ ft	$L_u$ ft	$Kl/r$	$A$ in <sup>2</sup>	$P_u$ K	$\phi P_n$ K	Ratio $\frac{P_u}{\phi P_n}$
T1	291.84 - 279.84 (666)	MC12x35	4.00	3.92	42.4	10.3000	3.24	333.72	0.010
T1	291.84 - 279.84 (667)	MC12x35	4.00	3.92	42.4	10.3000	3.21	333.72	0.010
T1	291.84 - 279.84 (670)	MC12x35	4.00	3.92	42.4	10.3000	3.31	333.72	0.010
T1	291.84 - 279.84 (671)	MC12x35	4.00	3.92	42.4	10.3000	3.13	333.72	0.009
T1	291.84 - 279.84 (674)	MC12x35	4.00	3.92	42.4	10.3000	3.18	333.72	0.010
T1	291.84 - 279.84 (675)	MC12x35	4.00	3.92	42.4	10.3000	3.31	333.72	0.010
T3	259.84 - 239.84 (678)	MC12x35	4.00	3.91	42.3	10.3000	3.31	333.72	0.010
T3	259.84 - 239.84 (679)	MC12x35	4.00	3.91	42.3	10.3000	3.39	333.72	0.010
T3	259.84 - 239.84 (682)	MC12x35	4.00	3.91	42.3	10.3000	3.30	333.72	0.010
T3	259.84 - 239.84 (683)	MC12x35	4.00	3.91	42.3	10.3000	3.39	333.72	0.010
T3	259.84 -	MC12x35	4.00	3.91	42.3	10.3000	3.30	333.72	0.010

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	89 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T3	239.84 (686)								
	259.84 -	MC12x35	4.00	3.91	42.3	10.3000	3.43	333.72	0.010
	239.84 (687)								
T5	219.84 -	MC12x35	4.00	3.90	42.2	10.3000	3.45	333.72	0.010
	199.84 (690)								
T5	219.84 -	MC12x35	4.00	3.90	42.2	10.3000	3.56	333.72	0.011
	199.84 (691)								
T5	219.84 -	MC12x35	4.00	3.90	42.2	10.3000	3.78	333.72	0.011
	199.84 (694)								
T5	219.84 -	MC12x35	4.00	3.90	42.2	10.3000	3.53	333.72	0.011
	199.84 (695)								
T5	219.84 -	MC12x35	4.00	3.90	42.2	10.3000	3.49	333.72	0.010
	199.84 (698)								
T5	219.84 -	MC12x35	4.00	3.90	42.2	10.3000	3.56	333.72	0.011
	199.84 (699)								
T7	179.84 -	MC12x35	4.00	3.89	42.1	10.3000	3.80	333.72	0.011
	159.84 (702)								
T7	179.84 -	MC12x35	4.00	3.89	42.1	10.3000	3.79	333.72	0.011
	159.84 (703)								
T7	179.84 -	MC12x35	4.00	3.89	42.1	10.3000	4.19	333.72	0.013
	159.84 (706)								
T7	179.84 -	MC12x35	4.00	3.89	42.1	10.3000	4.15	333.72	0.012
	159.84 (707)								
T7	179.84 -	MC12x35	4.00	3.89	42.1	10.3000	3.93	333.72	0.012
	159.84 (710)								
T7	179.84 -	MC12x35	4.00	3.89	42.1	10.3000	3.73	333.72	0.011
	159.84 (711)								
T10	119.84 - 99.84	MC12x35	4.00	3.89	42.1	10.3000	4.52	333.72	0.014
	(714)								
T10	119.84 - 99.84	MC12x35	4.00	3.89	42.1	10.3000	4.49	333.72	0.013
	(715)								
T10	119.84 - 99.84	MC12x35	4.00	3.89	42.1	10.3000	4.77	333.72	0.014
	(718)								
T10	119.84 - 99.84	MC12x35	4.00	3.89	42.1	10.3000	5.37	333.72	0.016
	(719)								
T10	119.84 - 99.84	MC12x35	4.00	3.89	42.1	10.3000	5.62	333.72	0.017
	(722)								
T10	119.84 - 99.84	MC12x35	4.00	3.89	42.1	10.3000	5.05	333.72	0.015
	(723)								
T13	59.84 - 39.84	MC12x35	4.00	3.88	41.9	10.3000	5.73	333.72	0.017
	(726)								
T13	59.84 - 39.84	MC12x35	4.00	3.88	41.9	10.3000	5.96	333.72	0.018
	(727)								
T13	59.84 - 39.84	MC12x35	4.00	3.88	41.9	10.3000	5.88	333.72	0.018
	(730)								
T13	59.84 - 39.84	MC12x35	4.00	3.88	41.9	10.3000	5.80	333.72	0.017
	(731)								
T13	59.84 - 39.84	MC12x35	4.00	3.88	41.9	10.3000	5.88	333.72	0.018
	(734)								
T13	59.84 - 39.84	MC12x35	4.00	3.88	41.9	10.3000	5.88	333.72	0.018
	(735)								

## Torque-Arm Top Bending Design Data

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	90 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	$M_{ux}$ kip-ft	$\phi M_{ux}$ kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	$M_{uy}$ kip-ft	$\phi M_{uy}$ kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
T1	291.84 - 279.84 (666)	MC12x35	-55.74	97.47	0.572	-0.00	21.36	0.000
T1	291.84 - 279.84 (667)	MC12x35	-56.45	97.47	0.579	0.00	21.36	0.000
T1	291.84 - 279.84 (670)	MC12x35	-55.82	97.47	0.573	-0.00	21.36	0.000
T1	291.84 - 279.84 (671)	MC12x35	-51.45	97.47	0.528	-0.00	21.36	0.000
T1	291.84 - 279.84 (674)	MC12x35	-51.58	97.47	0.529	-0.00	21.36	0.000
T1	291.84 - 279.84 (675)	MC12x35	-56.27	97.47	0.577	0.00	21.36	0.000
T3	259.84 - 239.84 (678)	MC12x35	-53.40	97.47	0.548	0.00	21.36	0.000
T3	259.84 - 239.84 (679)	MC12x35	-53.02	97.47	0.544	0.00	21.36	0.000
T3	259.84 - 239.84 (682)	MC12x35	-48.33	97.47	0.496	0.00	21.36	0.000
T3	259.84 - 239.84 (683)	MC12x35	-52.75	97.47	0.541	0.00	21.36	0.000
T3	259.84 - 239.84 (686)	MC12x35	-48.15	97.47	0.494	0.00	21.36	0.000
T3	259.84 - 239.84 (687)	MC12x35	-53.56	97.47	0.550	0.00	21.36	0.000
T5	219.84 - 199.84 (690)	MC12x35	-49.33	97.47	0.506	0.00	21.36	0.000
T5	219.84 - 199.84 (691)	MC12x35	-48.87	97.47	0.501	0.00	21.36	0.000
T5	219.84 - 199.84 (694)	MC12x35	-43.73	97.47	0.449	0.00	21.36	0.000
T5	219.84 - 199.84 (695)	MC12x35	-48.52	97.47	0.498	0.00	21.36	0.000
T5	219.84 - 199.84 (698)	MC12x35	-43.55	97.47	0.447	0.00	21.36	0.000
T5	219.84 - 199.84 (699)	MC12x35	-49.47	97.47	0.508	0.00	21.36	0.000
T7	179.84 - 159.84 (702)	MC12x35	-42.37	97.47	0.435	-0.00	21.36	0.000
T7	179.84 - 159.84 (703)	MC12x35	-43.40	97.47	0.445	-0.00	21.36	0.000
T7	179.84 - 159.84 (706)	MC12x35	-42.40	97.47	0.435	-0.00	21.36	0.000
T7	179.84 - 159.84 (707)	MC12x35	-36.65	97.47	0.376	-0.00	21.36	0.000
T7	179.84 - 159.84 (710)	MC12x35	-36.90	97.47	0.379	-0.00	21.36	0.000
T7	179.84 - 159.84 (711)	MC12x35	-42.98	97.47	0.441	-0.00	21.36	0.000
T10	119.84 - 99.84 (714)	MC12x35	-32.12	97.47	0.330	-0.00	21.36	0.000
T10	119.84 - 99.84 (715)	MC12x35	-33.16	97.47	0.340	-0.00	21.36	0.000
T10	119.84 - 99.84 (718)	MC12x35	-32.09	97.47	0.329	-0.00	21.36	0.000
T10	119.84 - 99.84 (719)	MC12x35	-25.75	97.47	0.264	-0.00	21.36	0.000
T10	119.84 - 99.84 (722)	MC12x35	-25.92	97.47	0.266	-0.00	21.36	0.000
T10	119.84 - 99.84 (723)	MC12x35	-32.66	97.47	0.335	-0.00	21.36	0.000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	91 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	$M_{ux}$ kip-ft	$\phi M_{nx}$ kip-ft	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	$M_{uy}$ kip-ft	$\phi M_{ny}$ kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
T13	59.84 - 39.84 (726)	MC12x35	-21.37	97.47	0.219	0.00	21.36	0.000
T13	59.84 - 39.84 (727)	MC12x35	-20.88	97.47	0.214	0.00	21.36	0.000
T13	59.84 - 39.84 (730)	MC12x35	-14.99	97.47	0.154	0.00	21.36	0.000
T13	59.84 - 39.84 (731)	MC12x35	-20.44	97.47	0.210	0.00	21.36	0.000
T13	59.84 - 39.84 (734)	MC12x35	-14.85	97.47	0.152	0.00	21.36	0.000
T13	59.84 - 39.84 (735)	MC12x35	-21.27	97.47	0.218	0.00	21.36	0.000

### Torque-Arm Top Interaction Design Data

Section No.	Elevation ft	Size	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	Ratio $\frac{M_{uy}}{\phi M_{ny}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
T1	291.84 - 279.84 (666)	MC12x35	0.010	0.572	0.000	0.577	1.000	4.8.1 ✓
T1	291.84 - 279.84 (667)	MC12x35	0.010	0.579	0.000	0.584	1.000	4.8.1 ✓
T1	291.84 - 279.84 (670)	MC12x35	0.010	0.573	0.000	0.578	1.000	4.8.1 ✓
T1	291.84 - 279.84 (671)	MC12x35	0.009	0.528	0.000	0.533	1.000	4.8.1 ✓
T1	291.84 - 279.84 (674)	MC12x35	0.010	0.529	0.000	0.534	1.000	4.8.1 ✓
T1	291.84 - 279.84 (675)	MC12x35	0.010	0.577	0.000	0.582	1.000	4.8.1 ✓
T3	259.84 - 239.84 (678)	MC12x35	0.010	0.548	0.000	0.553	1.000	4.8.1 ✓
T3	259.84 - 239.84 (679)	MC12x35	0.010	0.544	0.000	0.549	1.000	4.8.1 ✓
T3	259.84 - 239.84 (682)	MC12x35	0.010	0.496	0.000	0.501	1.000	4.8.1 ✓
T3	259.84 - 239.84 (683)	MC12x35	0.010	0.541	0.000	0.546	1.000	4.8.1 ✓
T3	259.84 - 239.84 (686)	MC12x35	0.010	0.494	0.000	0.499	1.000	4.8.1 ✓
T3	259.84 - 239.84 (687)	MC12x35	0.010	0.550	0.000	0.555	1.000	4.8.1 ✓
T5	219.84 - 199.84 (690)	MC12x35	0.010	0.506	0.000	0.511	1.000	4.8.1 ✓
T5	219.84 - 199.84 (691)	MC12x35	0.011	0.501	0.000	0.507	1.000	4.8.1 ✓
T5	219.84 - 199.84 (694)	MC12x35	0.011	0.449	0.000	0.454	1.000	4.8.1 ✓
T5	219.84 - 199.84 (695)	MC12x35	0.011	0.498	0.000	0.503	1.000	4.8.1 ✓
T5	219.84 -	MC12x35	0.010	0.447	0.000	0.452	1.000	4.8.1 ✓

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	<b>Page</b>
	17004.42 - CT1077	92 of 95
	<b>Project</b> 327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b> 15:25:46 09/11/17
	<b>Client</b> AT&T	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	Ratio $P_u$ $\phi P_n$	Ratio $M_{ux}$ $\phi M_{nx}$	Ratio $M_{uy}$ $\phi M_{ny}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
	199.84 (698)					✓		
T5	219.84 - 199.84 (699)	MC12x35	0.011	0.508	0.000	0.513	1.000	4.8.1 ✓
T7	179.84 - 159.84 (702)	MC12x35	0.011	0.435	0.000	0.440	1.000	4.8.1 ✓
T7	179.84 - 159.84 (703)	MC12x35	0.011	0.445	0.000	0.451	1.000	4.8.1 ✓
T7	179.84 - 159.84 (706)	MC12x35	0.013	0.435	0.000	0.441	1.000	4.8.1 ✓
T7	179.84 - 159.84 (707)	MC12x35	0.012	0.376	0.000	0.382	1.000	4.8.1 ✓
T7	179.84 - 159.84 (710)	MC12x35	0.012	0.379	0.000	0.384	1.000	4.8.1 ✓
T7	179.84 - 159.84 (711)	MC12x35	0.011	0.441	0.000	0.447	1.000	4.8.1 ✓
T10	119.84 - 99.84 (714)	MC12x35	0.014	0.330	0.000	0.336	1.000	4.8.1 ✓
T10	119.84 - 99.84 (715)	MC12x35	0.013	0.340	0.000	0.347	1.000	4.8.1 ✓
T10	119.84 - 99.84 (718)	MC12x35	0.014	0.329	0.000	0.336	1.000	4.8.1 ✓
T10	119.84 - 99.84 (719)	MC12x35	0.016	0.264	0.000	0.272	1.000	4.8.1 ✓
T10	119.84 - 99.84 (722)	MC12x35	0.017	0.266	0.000	0.274	1.000	4.8.1 ✓
T10	119.84 - 99.84 (723)	MC12x35	0.015	0.335	0.000	0.343	1.000	4.8.1 ✓
T13	59.84 - 39.84 (726)	MC12x35	0.017	0.219	0.000	0.228	1.000	4.8.1 ✓
T13	59.84 - 39.84 (727)	MC12x35	0.018	0.214	0.000	0.223	1.000	4.8.1 ✓
T13	59.84 - 39.84 (730)	MC12x35	0.018	0.154	0.000	0.163	1.000	4.8.1 ✓
T13	59.84 - 39.84 (731)	MC12x35	0.017	0.210	0.000	0.218	1.000	4.8.1 ✓
T13	59.84 - 39.84 (734)	MC12x35	0.018	0.152	0.000	0.161	1.000	4.8.1 ✓
T13	59.84 - 39.84 (735)	MC12x35	0.018	0.218	0.000	0.227	1.000	4.8.1 ✓

## Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
L1	327 - 291.84	Pole	P10.75x0.843	1	-3.95	395.38	18.9	Pass
T1	291.84 - 279.84	Leg	2	2	-30.91	96.77	31.9	Pass
T2	279.84 - 259.84	Leg	2	36	-36.55	88.54	41.3	Pass
T3	259.84 - 239.84	Leg	2 1/4	83	-61.43	123.62	49.7	Pass
T4	239.84 - 219.84	Leg	2 1/4	127	-63.52	123.62	51.4	Pass
T5	219.84 - 199.84	Leg	2 1/2	171	-93.71	163.72	57.2	Pass

<b><i>tnxTower</i></b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	17004.42 - CT1077	<b>Page</b>	93 of 95
	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
T6	199.84 - 179.84	Leg	2 1/2	218	-102.03	163.72	62.3	Pass
T7	179.84 - 159.84	Leg	2 3/4	263	-123.53	208.68	59.2	Pass
T8	159.84 - 139.84	Leg	2 1/2	306	-130.23	163.72	79.5	Pass
T9	139.84 - 119.84	Leg	2 3/4	353	-135.48	208.68	64.9	Pass
T10	119.84 - 99.84	Leg	2 3/4	398	-155.11	208.68	74.3	Pass
T11	99.84 - 79.84	Leg	3	443	-166.19	258.36	64.3	Pass
T12	79.84 - 59.84	Leg	3	486	-168.94	258.36	65.4	Pass
T13	59.84 - 39.84	Leg	3	533	-185.29	258.36	71.7	Pass
T14	39.84 - 19.84	Leg	3	578	-190.22	258.36	73.6	Pass
T15	19.84 - 6.5	Leg	3	621	-193.48	258.30	74.9	Pass
T16	6.5 - 0	Leg	3	652	-204.80	228.38	89.7	Pass
T1	291.84 - 279.84	Diagonal	1 3/8	25	-3.91	25.28	15.5	Pass
T2	279.84 - 259.84	Diagonal	1 3/8	44	-2.94	23.79	12.3	Pass
T3	259.84 - 239.84	Diagonal	1 3/8	116	-4.50	23.99	18.8	Pass
T4	239.84 - 219.84	Diagonal	1 3/8	133	-3.39	23.99	14.1	Pass
T5	219.84 - 199.84	Diagonal	1 1/2	207	-8.44	32.13	26.3	Pass
T6	199.84 - 179.84	Diagonal	1 1/4	223	-6.82	17.31	39.4	Pass
T7	179.84 - 159.84	Diagonal	1 1/2	282	-8.96	32.36	27.7	Pass
T8	159.84 - 139.84	Diagonal	1 3/8	349	-5.20	24.19	21.5	Pass
T9	139.84 - 119.84	Diagonal	1 1/4	358	-5.53	17.48	31.6	Pass
T10	119.84 - 99.84	Diagonal	1 1/2	425	-9.95	32.36	30.7	Pass
T11	99.84 - 79.84	Diagonal	1 3/8	482	-7.48	24.59	30.4	Pass
T12	79.84 - 59.84	Diagonal	1 1/4	494	-8.01	17.65	45.4	Pass
T13	59.84 - 39.84	Diagonal	1 1/4	567	-8.11	17.65	45.9	Pass
T14	39.84 - 19.84	Diagonal	1 1/4	617	-4.20	17.65	23.8	Pass
T15	19.84 - 6.5	Diagonal	1 1/4	628	-4.40	17.65	24.9	Pass
T1	291.84 - 279.84	Horizontal	1	15	-0.67	12.27	5.4	Pass
T2	279.84 - 259.84	Horizontal	1	74	-0.46	12.27	3.8	Pass
T3	259.84 - 239.84	Horizontal	1	113	-0.95	12.38	7.7	Pass
T4	239.84 - 219.84	Horizontal	1	166	0.49	25.45	1.9	Pass
T5	219.84 - 199.84	Horizontal	1	204	-0.47	12.48	3.8	Pass
T6	199.84 - 179.84	Horizontal	1	235	-1.60	12.48	12.8	Pass
T7	179.84 - 159.84	Horizontal	1	285	1.48	25.45	5.8	Pass
T8	159.84 - 139.84	Horizontal	1	318	0.91	25.45	3.6	Pass
T9	139.84 - 119.84	Horizontal	1	363	1.15	25.45	4.5	Pass
T10	119.84 - 99.84	Horizontal	1	436	2.00	25.45	7.8	Pass
T11	99.84 - 79.84	Horizontal	1	452	2.25	25.45	8.9	Pass
T12	79.84 - 59.84	Horizontal	1	512	1.58	25.45	6.2	Pass
T13	59.84 - 39.84	Horizontal	1	557	1.76	25.45	6.9	Pass
T14	39.84 - 19.84	Horizontal	1	588	1.73	25.45	6.8	Pass
T15	19.84 - 6.5	Horizontal	1	645	1.73	25.45	6.8	Pass
T1	291.84 - 279.84	Secondary Horizontal	1	28	-0.00	17.94	0.1	Pass
T2	279.84 - 259.84	Secondary Horizontal	1	59	0.00	25.45	0.1	Pass
T3	259.84 - 239.84	Secondary Horizontal	1	118	-0.00	17.96	0.1	Pass
T4	239.84 - 219.84	Secondary Horizontal	1	149	0.00	25.45	0.1	Pass
T5	219.84 - 199.84	Secondary Horizontal	1	208	-0.00	17.99	0.1	Pass
T6	199.84 - 179.84	Secondary Horizontal	1	225	0.00	25.45	0.1	Pass
T7	179.84 - 159.84	Secondary Horizontal	1	284	0.00	25.45	0.1	Pass
T8	159.84 - 139.84	Secondary Horizontal	1	315	0.00	25.45	0.1	Pass
T9	139.84 - 119.84	Secondary Horizontal	1	360	0.00	25.45	0.1	Pass
T10	119.84 - 99.84	Secondary Horizontal	1	419	0.00	25.45	0.1	Pass
T11	99.84 - 79.84	Secondary Horizontal	1	450	0.00	25.45	0.1	Pass
T12	79.84 - 59.84	Secondary Horizontal	1	523	0.00	25.45	0.1	Pass
T13	59.84 - 39.84	Secondary Horizontal	1	568	0.00	25.45	0.1	Pass
T14	39.84 - 19.84	Secondary Horizontal	1	585	0.00	25.45	0.1	Pass
T15	19.84 - 6.5	Secondary Horizontal	1	644	0.00	25.45	0.1	Pass
T1	291.84 - 279.84	Top Girt	1	6	-0.00	12.27	0.2	Pass
T2	279.84 - 259.84	Top Girt	1	10	0.40	25.45	1.6	Pass
T3	259.84 - 239.84	Top Girt	1	41	0.80	25.45	3.2	Pass
T4	239.84 - 219.84	Top Girt	1	84	0.65	25.45	2.5	Pass
T5	219.84 - 199.84	Top Girt	1	130	1.02	25.45	4.0	Pass

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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\sigma P_{allow}$ K	% Capacity	Pass Fail
T6	199.84 - 179.84	Top Girt	1	174	0.73	25.45	2.9	Pass
T7	179.84 - 159.84	Top Girt	1	221	0.78	25.45	3.1	Pass
T8	159.84 - 139.84	Top Girt	1	265	1.10	25.45	4.3	Pass
T9	139.84 - 119.84	Top Girt	1	311	0.98	25.45	3.9	Pass
T10	119.84 - 99.84	Top Girt	1	356	1.13	25.45	4.4	Pass
T11	99.84 - 79.84	Top Girt	1	400	1.51	25.45	5.9	Pass
T12	79.84 - 59.84	Top Girt	1	446	1.54	25.45	6.1	Pass
T13	59.84 - 39.84	Top Girt	1	490	1.86	25.45	7.3	Pass
T14	39.84 - 19.84	Top Girt	1	536	1.69	25.45	6.6	Pass
T15	19.84 - 6.5	Top Girt	1	581	1.74	25.45	6.8	Pass
T16	6.5 - 0	Top Girt	12x3/8	626	36.72	145.80	25.2	Pass
T16	6.5 - 0	Bottom Girt	12x3/8	655	-6.02	145.70	4.5	Pass
T16	6.5 - 0	Mid Girt	9x3/8	659	-0.37	45.69	0.8	Pass
T1	291.84 - 279.84	Guy A@285.84	3/4	673	16.59	34.98	47.4	Pass
T3	259.84 - 239.84	Guy A@256.507	3/4	685	16.31	34.98	46.6	Pass
T5	219.84 - 199.84	Guy A@216.507	3/4	697	16.02	34.98	45.8	Pass
T7	179.84 - 159.84	Guy A@166.507	3/4	709	17.39	34.98	49.7	Pass
T10	119.84 - 99.84	Guy A@106.507	3/4	721	16.78	34.98	48.0	Pass
T13	59.84 - 39.84	Guy A@56.5067	3/4	733	13.73	34.98	39.3	Pass
T1	291.84 - 279.84	Guy B@285.84	3/4	668	15.62	34.98	44.7	Pass
T3	259.84 - 239.84	Guy B@256.507	3/4	681	15.26	34.98	43.6	Pass
T5	219.84 - 199.84	Guy B@216.507	3/4	693	14.91	34.98	42.6	Pass
T7	179.84 - 159.84	Guy B@166.507	3/4	704	16.44	34.98	47.0	Pass
T10	119.84 - 99.84	Guy B@106.507	3/4	716	15.73	34.98	45.0	Pass
T13	59.84 - 39.84	Guy B@56.5067	3/4	729	13.03	34.98	37.3	Pass
T1	291.84 - 279.84	Guy C@285.84	3/4	665	16.47	34.98	47.1	Pass
T3	259.84 - 239.84	Guy C@256.507	3/4	677	16.17	34.98	46.2	Pass
T5	219.84 - 199.84	Guy C@216.507	3/4	689	15.87	34.98	45.4	Pass
T7	179.84 - 159.84	Guy C@166.507	3/4	700	17.22	34.98	49.2	Pass
T10	119.84 - 99.84	Guy C@106.507	3/4	712	16.55	34.98	47.3	Pass
T13	59.84 - 39.84	Guy C@56.5067	3/4	725	13.62	34.98	38.9	Pass
T1	291.84 - 279.84	Top Guy	MC12x35	24	4.78	333.72	1.4	Pass
		Pull-Off@285.84						
T3	259.84 - 239.84	Top Guy	MC12x35	119	5.90	333.72	1.8	Pass
		Pull-Off@256.507						
T5	219.84 - 199.84	Top Guy	MC12x35	209	8.91	333.72	2.7	Pass
		Pull-Off@216.507						
T7	179.84 - 159.84	Top Guy	MC12x35	280	12.94	333.72	3.9	Pass
		Pull-Off@166.507						
T10	119.84 - 99.84	Top Guy	MC12x35	415	14.79	333.72	4.4	Pass
		Pull-Off@106.507						
T13	59.84 - 39.84	Top Guy	MC12x35	569	12.72	333.72	3.8	Pass
		Pull-Off@56.5067						
T1	291.84 - 279.84	Torque Arm	MC12x35	667	3.21	333.72	58.4	Pass
		Top@285.84						
T3	259.84 - 239.84	Torque Arm	MC12x35	687	3.43	333.72	55.5	Pass
		Top@256.507						
T5	219.84 - 199.84	Torque Arm	MC12x35	699	3.56	333.72	51.3	Pass
		Top@216.507						
T7	179.84 - 159.84	Torque Arm	MC12x35	703	-4.84	304.05	45.2	Pass
		Top@166.507						
T10	119.84 - 99.84	Torque Arm	MC12x35	715	-5.82	304.05	34.7	Pass
		Top@106.507						
T13	59.84 - 39.84	Torque Arm	MC12x35	726	5.73	333.72	22.8	Pass
		Top@56.5067						

Summary		
Pole (L1)	18.9	Pass
Leg (T16)	89.7	Pass
Diagonal (T13)	45.9	Pass
Horizontal	12.8	Pass



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	<b>Project</b>	327' Guyed Tower - N. Eagleville Road Storrs, CT	<b>Date</b>	15:25:46 09/11/17
	<b>Client</b>	AT&T	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
						(T6) Secondary Horizontal	0.1	Pass
						(T1) Top Girt	25.2	Pass
						(T16) Bottom Girt	4.5	Pass
						(T16) Mid Girt	0.8	Pass
						(T16) Guy A (T7)	49.7	Pass
						Guy B (T7)	47.0	Pass
						Guy C (T7)	49.2	Pass
						Top Guy Pull-Off (T10)	4.4	Pass
						Torque Arm Top (T1)	58.4	Pass
						Bolt Checks	24.5	Pass
						<b>RATING =</b>	<b>89.7</b>	<b>Pass</b>

### Guyed Tower Base Foundation:

#### Input Data:

##### Tower Data

Shear Force = Shear := 4-kip (User Input from tnxTower)  
 Axial Force = Axial := 580-kip (User Input from tnxTower)  
 Tower Height =  $H_t$  := 327-ft (User Input)

##### Footing Data:

Overall Depth of Footing =  $D_f$  := 4-ft (User Input)  
 Length of Pier =  $L_p$  := 2.5-ft (User Input)  
 Extension of Pier Above Grade =  $L_{pag}$  := 0.5-ft (User Input)  
 Diameter of Pier =  $D_p$  := 3.0-ft (User Input)  
 Width of Pad =  $W_{pad}$  := 10-ft (User Input)  
 Thickness of Pad =  $t_{pad}$  := 2.0-ft (User Input)

##### Material Properties:

Concrete Compressive Strength =  $f_c$  := 3000-psi (User Input)  
 Steel Reinforcement Yield Strength =  $f_y$  := 60000-psi (User Input)  
 Internal Friction Angle of Soil =  $\Phi_s$  := 30-deg (User Input)  
 Ultimate Soil Bearing Capacity =  $q_s$  := 11000-psf (User Input)  
 Unit Weight of Soil =  $\gamma_{soil}$  := 120-pcf (User Input)  
 Unit Weight of Concrete =  $\gamma_{conc}$  := 150-pcf (User Input)  
 Foundation Bouyancy = Bouyancy := 0 (User Input) (Yes=1 / No=0)  
 Depth to Neglect =  $n$  := 0-ft (User Input)  
 Cohesion of Clay Type Soil =  $c$  := 0-ksf (User Input) (Use 0 for Sandy Soil)  
 Seismic Zone Factor =  $Z$  := 2 (User Input)  
 Coefficient of Friction Between Concrete =  $\mu$  := 0.45 (User Input)

#### Calculated Factors:

Coefficient of Lateral Soil Pressure =  $K_p := \frac{1 + \sin(\Phi_s)}{1 - \sin(\Phi_s)} = 3$

Load Factor =  $LF := \begin{cases} 1.333 & \text{if } H_t \leq 700\text{-ft} \\ 1.7 & \text{if } H_t \geq 1200\text{-ft} \\ 1.333 + \left( \frac{H_t - 700\text{ft}}{1200\text{ft} - 700\text{ft}} \right) \cdot 0.4 & \text{otherwise} \end{cases} = 1.333$

### Stability of Footing:

Adjusted Concrete Unit Weight =

$$\gamma_c := \text{if}(\text{Bouyancy} = 1, \gamma_{\text{conc}} - 62.4 \text{pcf}, \gamma_{\text{conc}}) = 150 \text{pcf}$$

Adjusted Soil Unit Weight =

$$\gamma_s := \text{if}(\text{Bouyancy} = 1, \gamma_{\text{soil}} - 62.4 \text{pcf}, \gamma_{\text{soil}}) = 120 \text{pcf}$$

Passive Pressure =

$$P_{\text{top}} := 0$$

$$P_{\text{bot}} := K_p \cdot \gamma_s \cdot D_f + c \cdot 2 \cdot \sqrt{K_p} = 1.44 \text{ksf}$$

$$P_{\text{ave}} := \frac{P_{\text{top}} + P_{\text{bot}}}{2} = 0.72 \text{ksf}$$

$$A_p := D_p \cdot L_p = 7.5$$

Soil Shear Resistance =

$$SI_1 := P_{\text{ave}} \cdot A_p = 5.4 \text{kip}$$

Weight of Concrete =

$$WT_c := \left( \frac{1}{4} \cdot \pi \cdot D_p^2 \cdot L_p + W_{\text{pad}}^2 \cdot t_{\text{pad}} \right) \cdot \gamma_c = 32.65 \text{kip}$$

Total Weight =

$$WT_{\text{tot}} := WT_c + \text{Axial} = 612.65 \text{kip}$$

Soil/Concrete Friction Resistance =

$$SI_2 := \mu \cdot WT_{\text{tot}} = 275.69 \text{kips}$$

Total Sliding Resistance =

$$SI_{\text{tot}} := SI_1 + SI_2 = 281.09 \text{kips}$$

Sliding Resistance Ratio =

$$\text{Sliding\_Resistance\_ratio} := \frac{0.75 SI_{\text{tot}}}{\text{Shear}} = 52.7$$

$$\text{Sliding\_Resistance\_Check} := \text{if} \left( \left( \frac{\text{Shear}}{0.75 SI_{\text{tot}}} < 1.0 \right), \text{"Okay"}, \text{"No Good"} \right)$$

$$\text{Sliding\_Resistance\_Check} = \text{"Okay"}$$

### Bearing Pressure Caused by Footing:

Maximum Pressure in Mat =

$$P_{\text{max}} := \frac{WT_{\text{tot}}}{W_{\text{pad}}} = 6.13 \text{ksf}$$

$$\text{Max\_Pressure\_Check} := \text{if}(P_{\text{max}} < 0.6 q_s, \text{"Okay"}, \text{"No Good"})$$

$$\text{Max\_Pressure\_Check} = \text{"Okay"}$$

**Job :** AT&T ~ CT1077: 327-ft Guyed Lattice Tower  
**Address:** North Eagleville Rd., Storrs, CT  
**Description:** Guy Anchor Evaluation

**Project No.** 17004.42  
**Computed by** TJL  
**Checked by** CFC

**Sheet** 1 of 2  
**Date** 9/11/17  
**Date**

## CHECK UPLIFT RESISTANCE

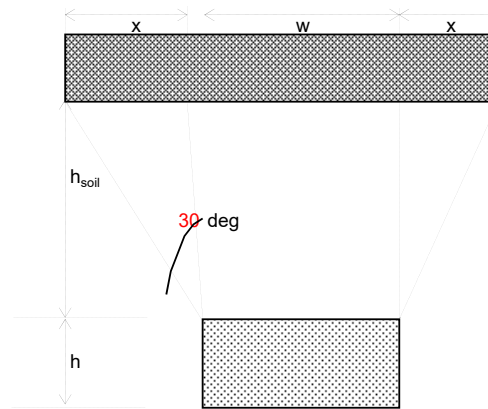
## ANCHOR (A) AT 235.0 ft RADIUS

### RESULTS FROM COMPUTER ANALYSIS:

Uplift = 109 kips  
Sliding = 139 kips  
Wdepth = 50 ft

### CONCRETE PARAMETERS:

$\gamma_{\text{conc}} = 150$  pcf  
 $\gamma_{\text{conc.sub}} = 87.6$  pcf  
 $w = 4.5$  ft  
 $h = 4$  ft  
 $d = 24$  ft  
  
Vol. = 432.00 ft<sup>3</sup>  
Vol.sub = 0.00 ft<sup>3</sup>  
Wc = 64.80 kips  
 $\phi = 0.90$   
58.32



Foundation Section

### SOIL PARAMETERS:

$\gamma_{\text{soil}} = 120$  pcf  
 $\gamma_{\text{soil.sub}} = 57.6$  pcf  
 $h_{\text{soil}} = 8$  ft  
 $x = 4.62$  ft

Soil Weight (Wr):

B1 = 108.00  
B2 = 108.00  
B3 = 456.61

W.soil = 251.73 kips  
W.soil.sub = 0.00 kips  
Total = 251.73 kips  
 $\phi = 0.75$   
188.80

### SF AGAINST SLIDING

2.27 > 1 OK

GUY ANCHORS AGAINST UPLIFT ARE ADEQUATE

**Job :** AT&T ~ CT1077: 327-ft Guyed Lattice Tower  
**Address:** 689 Old Colchester Rd., Montville, CT  
**Description:** Guy Anchor Evaluation

**Project No.** 17004.42  
**Computed by** TJL  
**Checked by** CFC

**Sheet** 2 of 2  
**Date** 9/11/17  
**Date**

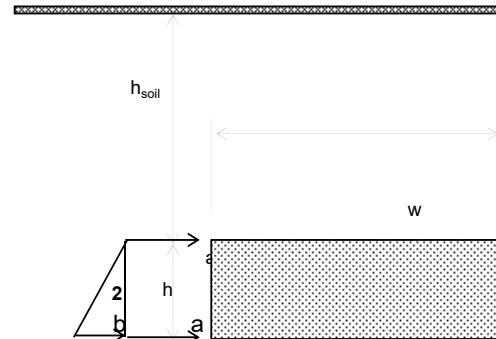
## CHECK SLIDING RESISTANCE

### SOIL PARAMETERS

$\gamma_{soil} = 120$  pcf  
 $\gamma_{soil} = 57.6$  pcf  
 $h_{soil} = 8$  ft  
 $h = 4$  ft  
 $\phi = 30$  degrees

### ANCHOR PARAMETERS

$w = 4.5$  ft  
 $h = 4.0$  ft  
 $d = 24.0$  ft



**Foundation Elevation View**

$$K_p = 3.00$$

### HORIZONTAL FORCES

**RESIST TO SLIDING =**

2.88 ksf  
4.32 ksf  
345.60 k

**SOIL & CONCRETE WEIGHT =**  
**UPLIFT REACTIONS =**  
**SUM =**

$W_r + W_c = 247.12$  k  
-109 k  
138.12 k

**COEF. OF FRICTION, (0.45) =**  
**RESIST TO SLIDING =**  
**SUM =**

62.15 k  
345.60 k  
407.75 k

### SF AGAINST SLIDING

**SF = 2.9 > 1 OK**

**GUY ANCHORS AGAINST SLIDING ARE ADEQUATE**

Section 1 - RFDS GENERAL INFORMATION

RFDS NAME:	CTU1077	DATE:	04/21/2017	RF DESIGN ENG:	Md Mateen	RF PERF ENG:		RFDS PROGRAM TYPE:	2018 LTE Next Carrier		
ISSUE:	Bronze Standard	Approved? (Y/N):	Yes	RF DESIGN PHONE:	8602586382	RF PERF PHONE:		RFDS TECHNOLOGY:	LTE		
REVISION:	Preliminary	RF MANAGER:	John Benedetto	RF DESIGN EMAIL:	mm093q@att.com	RF PERF EMAIL:		STATE/STATUS:	Final/RF Approval		
INITIATIVE /PROJECT:	LTE 6C AWS J, LTE 7C 700 UPPER D, RRH Add 1900 A3-A4 & E & C5					RFDS VERSION:	1.00	RFDS ID:	1736984		
						GSM FREQUENCY:		Created By:	mm093q	Updated By:	rx855w
						UMTS FREQUENCY:	850, 1900	Date Created:	4/20/2017 5:37:46 PM	Date Updated:	7/26/2017 9:44:19 AM
						LTE FREQUENCY:	700, 850, 1900, WCS				
						I-PLAN JOB # 1:	NER-RCTB-17-02148	IPLAN PRD GRP    SUB GRP #1:	LTE Next Carrier    LTE 6C		
						I-PLAN JOB # 2:	NER-RCTB-17-02235	IPLAN PRD GRP    SUB GRP #2:	LTE Next Carrier    LTE 7C		
						I-PLAN JOB # 3:	NER-RCTB-17-02296	IPLAN PRD GRP    SUB GRP #3:	LTE Additional Radio    2nd RRH Add		
						I-PLAN JOB # 4:		IPLAN PRD GRP    SUB GRP #4:			
						I-PLAN JOB # 5:		IPLAN PRD GRP    SUB GRP #5:			
						I-PLAN JOB # 6:		IPLAN PRD GRP    SUB GRP #6:			
I-PLAN JOB # 7:		IPLAN PRD GRP    SUB GRP #7:									
I-PLAN JOB # 8:		IPLAN PRD GRP    SUB GRP #8:									

Section 2 - LOCATION INFORMATION

USID:	59367	FA LOCATION CODE:	10035012	LOCATION NAME:	STORRS-UCONN	ORACLE PTN # 1:	2051A0B91T	PACE JOB # 1:	MRCTB023976	
REGION:	NORTHEAST	MARKET CLUSTER:	NEW ENGLAND	MARKET:	CONNECTICUT	ORACLE PTN # 2:	2051A0B8YP	PACE JOB # 2:	MRCTB023938	
ADDRESS:	1298 STORRS ROAD	CITY:	STORRS	STATE:	CT	ORACLE PTN # 3:	2051A0B98M	PACE JOB # 3:	MRCTB024148	
ZIP CODE:	06268	COUNTY:	TOLLAND	LONG (DEC. DEG.):	-72.2594431	ORACLE PTN # 4:		PACE JOB # 4:		
LATITUDE (D-M-S):	41d 48m50.57316s	LONGITUDE (D-M-S):	-72d -15m-33.99516s	LAT (DEC. DEG.):	41.8140481	ORACLE PTN # 5:		PACE JOB # 5:		
DIRECTIONS, ACCESS AND EQUIPMENT LOCATION:	1077 - STORRS I-84 NORTH TO EXIT 68 RT. 195. TURN RIGHT ON RT. 195 & CONTINUE UNTIL YOU REACH CAMPUS. TAKE RIGHT AT 2ND LIGHT FOR NORTH EAGLEVILLE ROAD APPROX. .5 MILES. TAKE RIGHT INTO DRIVEWAY JUST AFTER CEMETERY, AND BEAR RIGHT UP HILL... DORMS 3/4 WAY UP DRIVEWAY. TAKE RIGHT UP HILL. LOOK FOR CINGULAR LOCK ON DOOR JUST TO THE LEFT OF MAIN GATE FOR ACCESS. THERE IS A SCHEMATIC ON U DRIVE AND ONE IN THE SITE FOR THIS GSM SITE. SITE IS GROUND LEVEL SHELTER, AND SMART CARDS ARE LOCATED INSIDE THE SHELTER. POWER IS PROVIDED BY THE UCONN GRID. GATE COMBO 0043UCONN MAINTAINED POWER OUTAGE INFO 860-486-3113 UCONN POLICE 860-486-4800DOOR COMBO 35214GROUND LEVEL:SHELTERGSM T1 INFO:DHXV238935 (ET 55) HCGS723324 (ET 54)HCGS723325 (ET 62)HCGS715979 (ET 81)DHXV238936 (ET 63)HCGS723323 (ET 35)UMTS T1ON FIBER					ORACLE PTN # 6:		PACE JOB # 6:		
						ORACLE PTN # 7:		PACE JOB # 7:		
						ORACLE PTN # 8:		PACE JOB # 8:		
						BORDER CELL WITH CONTOUR COORD:		SEARCH RING NAME:		
						AM STUDY REQ'D (Y/N):	No	SEARCH RING ID:		
						FREQ COORD:		BTA:		MSA / RSA:
						OPS DISTRICT:	CT-North	LAC(GSM):	05021	
						OPS ZONE:	NE_CT_N_WDHM_N_CS	LAC(UMTS):	05990	
						RF DISTRICT:	NPO Triage	BSC(GSM):	MDTWCTBSC11	
						RF ZONE:	Hotseat	RNC(UMTS):	MDTWCTNICRBR05	
PARENT NAME(GSM):	MIDDLETOWN-GSM MTSO-BSC-11	MME POOL ID(LTE):	FF01							
PARENT NAME(UMTS):	MIDDLETOWN RNC05									

Section 3 - LICENSE COVERAGE/FILING INFORMATION

CGSA - NO FILING TRIGGERED (Yes/No):	No	CGSA LOSS:		PCS REDUCED - UPS ZIP:		CGSA CALL SIGNS:	
CGSA - MINOR FILING NEEDED (Yes/No):	No	CGSA EXT AGMT NEEDED:		PCS POPS REDUCED:			
CGSA - MAJOR FILING NEEDED (Yes/No):	Yes	CGSA SCORECARD UPDATED:					

Section 4 - TOWER/REGULATORY INFORMATION

STRUCTURE AT&T OWNED?:	Yes	GROUND ELEVATION (ft):		STRUCTURE TYPE:	GUYED	MARKET LOCATION 700 MHz Band:			
ADDITIONAL REGULATORY?:	Yes	HEIGHT OVERALL (ft):	0.00	FCC ASR NUMBER:	NR	MARKET LOCATION 850 MHz Band:			
SUB-LEASE RIGHTS?:	Yes	STRUCTURE HEIGHT (ft):	295.90			MARKET LOCATION 1900 MHz Band:			
LIGHTING TYPE:	PAINT AND RED LIGHT								

## Section 5 - E-911 INFORMATION - existing

[illegible][illegible]

## Section 6 - RBS GENERAL INFORMATION - existing

	GSM 1ST RBS	GSM 2ND RBS	UMTS 1ST RBS	UMTS 2ND RBS	LTE 1ST RBS	LTE 2ND RBS						
RBS ID:	96453	96454	172512	241675	366839	489337						
CTS COMMON ID:	184D1077_2	184D1077	CTU1077	CTV1077	CTL01077	CTL00077R						
CELL ID / BCF:	032D1077	032D1077	CTU1077	CTU1077	CTL01077	CTL00077R						
BTA/TID:	184G	184P	184V	184U	184L	184L						
4-9 DIGIT SITE ID:	1077	1077	1077	1077	1077	0077						
COW OR TOY?:	No	No	No	No	No	No						
CELL SITE TYPE:	SECTORIZED	SECTORIZED	SECTORIZED		SECTORIZED	SECTORIZED						
SITE TYPE:	BTS-CONVENTIONAL	BTS-CONVENTIONAL	MACRO-CONVENTIONAL	MACRO-CONVENTIONAL	MACRO-CONVENTIONAL	MACRO-CONVENTIONAL						
BTS LOCATION ID:	GROUND	GROUND	INTERNAL	INTERNAL								
BASE STATION TYPE:	BASE	BASE	BASE	OVERLAY	BASE	OVERLAY						
EQUIPMENT NAME:	STORRS-UCONN	STORRS-UCONN	STORRS-UCONN	STORRS-UCONN	STORRS-UCONN	STORRS-UCONN 2ND LTE NODE						
DISASTER PRIORITY:	0	0	2	0	3	3						

## Section 6 - RBS GENERAL INFORMATION - final

	GSM 1ST RBS	GSM 2ND RBS	UMTS 1ST RBS	UMTS 2ND RBS	LTE 1ST RBS	LTE 2ND RBS						
RBS ID:			172512	241675	366839							
CTS COMMON ID:			CTU1077	CTV1077	CTL01077							
CELL ID / BCF:			CTU1077	CTU1077	CTL01077							
BTA/TID:			184V	184U	184L							
4-9 DIGIT SITE ID:			1077	1077	1077							
COW OR TOY?:			No	No	No							
CELL SITE TYPE:			SECTORIZED	SECTORIZED	SECTORIZED							
SITE TYPE:			MACRO-CONVENTIONAL	MACRO-CONVENTIONAL	MACRO-CONVENTIONAL							
BTS LOCATION ID:			INTERNAL	INTERNAL								
BASE STATION TYPE:			BASE	OVERLAY	BASE							
EQUIPMENT NAME:			STORRS-UCONN	STORRS-UCONN	STORRS-UCONN							
DISASTER PRIORITY:			2	0	3							



## Section 7 - RBS SPECIFIC INFORMATION - existing

	GSM 1ST RBS	GSM 2ND RBS	UMTS 1ST RBS	UMTS 2ND RBS	LTE 1ST RBS	LTE 2ND RBS						
RAC:												
EQUIPMENT VENDOR:	NOKIA	NOKIA	ERICSSON	ERICSSON	ERICSSON	ERICSSON						
EQUIPMENT TYPE:	ULTRASITE	ULTRASITE	3206 INDOOR	3206 INDOOR	6601 INDOOR MU	6601 INDOOR MU						
BASEBAND CONFIGURATION:												
LOCATION:												
CABINET LOCATION:												
MARKET STATE CODE:					CT	CT						
AGPS:	Yes	Yes	Yes	Yes	Yes	Yes						
NODE B NUMBER:	0	0	0	0	1077	77						

## Section 7 - RBS SPECIFIC INFORMATION - final

	GSM 1ST RBS	GSM 2ND RBS	UMTS 1ST RBS	UMTS 2ND RBS	LTE 1ST RBS	LTE 2ND RBS						
RAC:												
EQUIPMENT VENDOR:			ERICSSON	ERICSSON	ERICSSON							
EQUIPMENT TYPE:			3206 INDOOR	3206 INDOOR	6601 INDOOR MU							
BASEBAND CONFIGURATION:					2x6601 / 2x5216 / 2xXMU03 + IDLe							
LOCATION:												
CABINET LOCATION:												
MARKET STATE CODE:					CT							
AGPS:			Yes	Yes	Yes							
NODE B NUMBER:			0	0	1077							

## Section 8 - RBS/SECTOR ASSOCIATION - existing

[illegible]

Section 8 - RBS/SECTOR ASSOCIATION - final

[illegible]

Section 9 - SOFT SECTOR ID - existing

[illegible][illegible]

Section 9 - Cell Number - existing

[illegible][illegible]

Section 10 - CID/SAC - existing

[illegible][illegible]

Section 15A - CURRENT TOWER CONFIGURATION - SECTOR A (OR OMNI)

ANTENNA POSITION is LEFT to RIGHT from BACK OF ANTENNA (unless otherwise specified)		ANTENNA POSITION 1		ANTENNA POSITION 2		ANTENNA POSITION 3		ANTENNA POSITION 4		ANTENNA POSITION 5		ANTENNA POSITION 6		ANTENNA POSITION 7	
ANTENNA MAKE - MODEL		7770		OPA-65R-LCUU-H8				HPA-65R-BUU-H8							
ANTENNA VENDOR		POWERWAVE		CCI Antennas				CCI Products							
ANTENNA SIZE (H x W x D)		55X11X5		92.7X14.4X7				92.4X14.8X7.4							
ANTENNA WEIGHT		35		88				68							
AZIMUTH		143		23				23							
MAGNETIC DECLINATION															
RADIATION CENTER (feet)		185		185				185							
ANTENNA TIP HEIGHT		187		189				189							
MECHANICAL DOWNTILT		0		0				0							
FEEDER AMOUNT		2		Fiber + 2 Coax											
VERTICAL SEPARATION from ANTENNA ABOVE (TIP to TIP)															
VERTICAL SEPARATION from ANTENNA BELOW (TIP to TIP)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # / # of inches)															
Antenna RET Motor (QTY/MODEL)		2	Powerwave 7020		Built-in				Built-in						
SURGE ARRESTOR (QTY/MODEL)				5	DC/Fiber Squid (1) + Andrew APTDC-BDFDM-DB Broadband (4)			1	DC/Fiber Squid						
DIPLEXER (QTY/MODEL)		1	Kaelus DBC2055F1V1-2	4	CCI Triplexer -TPX-070821										
DUPLEXER (QTY/MODEL)															
Antenna RET CONTROL UNIT (QTY/MODEL)		1	Powerwave 7070		RRH Controlled				RRH Controlled						
DC BLOCK (QTY/MODEL)															
TMA/LNA (QTY/MODEL)		1	CCI DTMABP7819VG12A (Twin 700/850 Bypass)												
CURRENT INJECTORS FOR TMA (QTY/MODEL)			AISG Compliant Diplexer												
PDU FOR TMA5 (QTY/MODEL)															
FILTER (QTY/MODEL)															
SQUID (QTY/MODEL)															
FIBER TRUNK (QTY/MODEL)															
DC TRUNK (QTY/MODEL)															
RRH - 700 band (QTY/MODEL)				1	RRUS-E2			1	RRUS-11						
RRH - 850 band (QTY/MODEL)		1	RRUW	1	RRUS-11										
RRH - 1900 band (QTY/MODEL)		1	RRUW					1	RRUS-32 B2						
RRH - AWS band (QTY/MODEL)															
RRH - WCS band (QTY/MODEL)				1	RRUS-32										
Additional RRH #1 - any band (QTY/MODEL)															
Additional RRH #2 - any band (QTY/MODEL)															
Additional Component 1 (QTY/MODEL)															
Additional Component 2 (QTY/MODEL)															
Additional Component 3 (QTY/MODEL)															
Local Market Note 1															
Local Market Note 2															
Local Market Note 3															

PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	ATOLL CELL ID	TX/RX ?	TECHNOLOGY/FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID (CSSNG)
ANTENNA POSITION 1	PORT 1		59367.A.850.3G.1	CTV10771	CTV10771		UMTS 850	7770.00.850.10	13.5	143	10	None	Andrew 1-5/8 (850)	250						264.24		1	
	PORT 2		59367.A.850.3G.2	CTV10771	CTV1077A		UMTS 850	7770.00.850.10	13.5	143	10	Bottom	Andrew 1-5/8 (850)	250						264.24		1	
	PORT 3		59367.A.1900.3G.1	CTU10777	CTU10777		UMTS 1900	7770.00.1900.06	15.5	143	6	None	Andrew 1-5/8 (850)	250						279.9		2	
	PORT 4		59367.A.1900.3G.2	CTU10777	CTU10774		UMTS 1900	7770.00.1900.06	15.5	143	6	Bottom	Andrew 1-5/8 (850)	250						486.41		2	
ANTENNA POSITION 2	PORT 1		59367.A.850.4G.1	CTL01077_8A_1	CTL01077_8A_1		LTE 850	OPA-65R-LCUU-H8_849MHz_02DT	15.4	23	2	Bottom	Andrew 1-5/8 (850)	250						1000		4	
	PORT 2			CTL01077_7A_2_E			LTE 700	OPA-65R-LCUU-	14.7	2	2	Bottom	Andrew 1-5/8 (850)	250						1475.7065		4	

							H8_719MHz_02DT																
	PORT 3		59367.A.2300.4G.1	CTL01077_3A_1	CTL01077_3A_1		LTE WCS	OPA-65R-LCUU- H8_2350MHz_06DT	17.3	23	6	Top	FIBER	0						1145.5129		3	
ANTENNA POSITION 4	PORT 1		59367.A.700.4G.1	CTL01077_7A_1	CTL01077_7A_1		LTE 700	HPA-65R-BUU- H8_719MHz_02DT	15.3	23	2	Top	FIBER	0						1044.7202		7	
	PORT 3		59367.A.1900.4G.1	CTL01077_9A_1	CTL01077_9A_1		LTE 1900	HPA-65R-BUU- H8_1930MHz_01DT	16.7	23	1	Top	FIBER	0						3380.6483		7	

Section 15B - CURRENT TOWER CONFIGURATION - SECTOR B

ANTENNA POSITION is LEFT to RIGHT from BACK OF ANTENNA (unless otherwise specified)		ANTENNA POSITION 1		ANTENNA POSITION 2		ANTENNA POSITION 3		ANTENNA POSITION 4		ANTENNA POSITION 5		ANTENNA POSITION 6		ANTENNA POSITION 7	
ANTENNA MAKE - MODEL		7770		OPA-65R-LCUU-H6				HPA-65R-BUU-H6							
ANTENNA VENDOR		POWERWAVE		CCI Antennas				CCI Products							
ANTENNA SIZE (H x W x D)		55X11X5		72X14.8X7.4				72X14.8X9							
ANTENNA WEIGHT		35		73				50.7							
AZIMUTH		263		143				143							
MAGNETIC DECLINATION															
RADIATION CENTER (feet)		185		185				185							
ANTENNA TIP HEIGHT		187		188				188							
MECHANICAL DOWNTILT		0		0				0							
FEEDER AMOUNT		2		Fiber + 2 Coax											
VERTICAL SEPARATION from ANTENNA ABOVE (TIP to TIP)															
VERTICAL SEPARATION from ANTENNA BELOW (TIP to TIP)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # / # of inches)															
Antenna RET Motor (QTY/MODEL)		2	Powerwave 7020		Built-in				Built-in						
SURGE ARRESTOR (QTY/MODEL)				4	Andrew APTDC-BDFDM-DB										
DIPLEXER (QTY/MODEL)		1	Kaelus DBC2055F1V1-2	4	CCI Triplexer -TPX-070821										
DUPLEXER (QTY/MODEL)															
Antenna RET CONTROL UNIT (QTY/MODEL)					RRH Controlled				RRH Controlled						
DC BLOCK (QTY/MODEL)															
TMA/LNA (QTY/MODEL)		1	CCI DTMAP7819VG12A (Twin 700/850 Bypass)												
CURRENT INJECTORS FOR TMA (QTY/MODEL)			AISG Compliant Diplexer												
PDU FOR TMA5 (QTY/MODEL)															
FILTER (QTY/MODEL)															
SQUID (QTY/MODEL)															
FIBER TRUNK (QTY/MODEL)															
DC TRUNK (QTY/MODEL)															
RRH - 700 band (QTY/MODEL)				1	RRUS-E2			1	RRUS-11						
RRH - 850 band (QTY/MODEL)		1	RRUW	1	RRUS-11										
RRH - 1900 band (QTY/MODEL)		1	RRUW					1	RRUS-32 B2						
RRH - AWS band (QTY/MODEL)															
RRH - WCS band (QTY/MODEL)				1	RRUS-32										
Additional RRH #1 - any band (QTY/MODEL)															
Additional RRH #2 - any band (QTY/MODEL)															
Additional Component 1 (QTY/MODEL)															
Additional Component 2 (QTY/MODEL)															
Additional Component 3 (QTY/MODEL)															
Local Market Note 1															
Local Market Note 2															
Local Market Note 3															

PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	ATOLL CELL ID	TX/RX ?	TECHNOLOGY/FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID (CSSNG)
ANTENNA POSITION 1	PORT 1		59367.B.850.3G.1	CTV10772	CTV10772		UMTS 850	7770.00.850.08	13.5	263	8	None	Andrew 1-5/8 (850)	250						264.24		9	
	PORT 2		59367.B.850.3G.2	CTV10772	CTV1077B		UMTS 850	7770.00.850.08	13.5	263	8	Bottom	Andrew 1-5/8 (850)	250						264.24		9	
	PORT 3		59367.B.1900.3G.1	CTU10778	CTU10778		UMTS 1900	7770.00.1900.04	15.5	263	4	None	Andrew 1-5/8 (850)	250						279.9		10	
	PORT 4		59367.B.1900.3G.2	CTU10778	CTU10775		UMTS 1900	7770.00.1900.04	15.5	263	4	Bottom	Andrew 1-5/8 (850)	250						486.41		10	
ANTENNA POSITION 2	PORT 1		59367.B.850.4G.1	CTL01077_8B_1	CTL01077_8B_1		LTE 850	OPA-65R-LCUU-H6_849MHz_02DT	15.4	143	2	Bottom	Andrew 1-5/8 (850)	250						1000		12	
	PORT 2			CTL01077_7B_2_E	CTL01077_7B_2_E		LTE 700	OPA-65R-LCUU-H6_719MHz_02DT	14	2	2	Bottom	Andrew 1-5/8 (850)	250						1475.7065		12	



	PORT 3		59367.B.2300.4G.1	CTL01077_3B_1	CTL01077_3B_1		LTE WCS	OPA-65R-LCUU-H6_2350MHz_06DT	17.5	143	6	Top	FIBER	0						1227.4392		11	
ANTENNA POSITION 4	PORT 1		59367.B.700.4G.1	CTL01077_7B_1	CTL01077_7B_1		LTE 700	HPA-65R-BUU-H6_719MHz_02DT	14.28	143	2	Top	FIBER	0						827.9421		15	
	PORT 3		59367.B.1900.4G.1	CTL01077_9B_1	CTL01077_9B_1		LTE 1900	HPA-65R-BUU-H6_1930MHz_02DT	16.85	143	2	Top	FIBER	0						3258.367		15	

Section 15C - CURRENT TOWER CONFIGURATION - SECTOR C

ANTENNA POSITION is LEFT to RIGHT from BACK OF ANTENNA (unless otherwise specified)		ANTENNA POSITION 1		ANTENNA POSITION 2		ANTENNA POSITION 3		ANTENNA POSITION 4		ANTENNA POSITION 5		ANTENNA POSITION 6		ANTENNA POSITION 7	
ANTENNA MAKE - MODEL		7770		OPA-65R-LCUU-H8				HPA-65R-BUU-H8							
ANTENNA VENDOR		POWERWAVE		CCI Antennas				CCI Products							
ANTENNA SIZE (H x W x D)		55X11X5		92.7X14.4X7				92.4X14.8X7.4							
ANTENNA WEIGHT		35		88				68							
AZIMUTH		23		263				263							
MAGNETIC DECLINATION															
RADIATION CENTER (feet)		185		185				185							
ANTENNA TIP HEIGHT		187		189				189							
MECHANICAL DOWNTILT		0		0				0							
FEEDER AMOUNT		2		Fiber + 2 Coax											
VERTICAL SEPARATION from ANTENNA ABOVE (TIP to TIP)															
VERTICAL SEPARATION from ANTENNA BELOW (TIP to TIP)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # / # of inches)															
Antenna RET Motor (QTY/MODEL)		2	Powerwave 7020		Built-in				Built-in						
SURGE ARRESTOR (QTY/MODEL)				4	Andrew APTDC-BDFDM-DB										
DIPLEXER (QTY/MODEL)		1	Kaelus DBC2055F1V1-2	4	CCI Triplexer -TPX-070821										
DUPLEXER (QTY/MODEL)															
Antenna RET CONTROL UNIT (QTY/MODEL)					RRH Controlled				RRH Controlled						
DC BLOCK (QTY/MODEL)															
TMA/LNA (QTY/MODEL)		1	CCI DTMABP7819VG12A (Twin 700/850 Bypass)												
CURRENT INJECTORS FOR TMA (QTY/MODEL)			AISG Compliant Diplexer												
PDU FOR TMA5 (QTY/MODEL)															
FILTER (QTY/MODEL)															
SQUID (QTY/MODEL)															
FIBER TRUNK (QTY/MODEL)															
DC TRUNK (QTY/MODEL)															
RRH - 700 band (QTY/MODEL)				1	RRUS-E2			1	RRUS-11						
RRH - 850 band (QTY/MODEL)		1	RRUW	1	RRUS-11										
RRH - 1900 band (QTY/MODEL)		1	RRUW					1	RRUS-32 B2						
RRH - AWS band (QTY/MODEL)															
RRH - WCS band (QTY/MODEL)				1	RRUS-32										
Additional RRH #1 - any band (QTY/MODEL)															
Additional RRH #2 - any band (QTY/MODEL)															
Additional Component 1 (QTY/MODEL)															
Additional Component 2 (QTY/MODEL)															
Additional Component 3 (QTY/MODEL)															
Local Market Note 1															
Local Market Note 2															
Local Market Note 3															

PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	ATOLL CELL ID	TX/RX ?	TECHNOLOGY/FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID (CSSNG)
ANTENNA POSITION 1	PORT 1		59367.C.850.3G.1	CTV10773	CTV10773		UMTS 850	7770.00.850.06	13.5	23	6	None	Andrew 1-5/8 (850)	250						264.24		17	
	PORT 2		59367.C.850.3G.2	CTV10773	CTV1077C		UMTS 850	7770.00.850.06	13.5	23	6	Bottom	Andrew 1-5/8 (850)	250						264.24		17	
	PORT 3		59367.C.1900.3G.1	CTU10779	CTU10779		UMTS 1900	7770.00.1900.04	15.5	23	4	None	Andrew 1-5/8 (850)	250						279.9		18	
	PORT 4		59367.C.1900.3G.2	CTU10779	CTU10776		UMTS 1900	7770.00.1900.04	15.5	23	4	Bottom	Andrew 1-5/8 (850)	250						486.41		18	
ANTENNA POSITION 2	PORT 1		59367.C.850.4G.1	CTL01077_8C_1	CTL01077_8C_1		LTE 850	OPA-65R-LCUU-H8_849MHz_04DT	15.4	263	4	Bottom	Andrew 1-5/8 (850)	250						1000		20	
	PORT 2			CTL01077_7C_2_E			LTE 700	OPA-65R-LCUU-H8_719MHz_04DT	14.8	4	4	Bottom	Andrew 1-5/8 (850)	250						1475.7065		20	

	PORT 3		59367.C.2300.4G.1	CTL01077_3C_1	CTL01077_3C_1		LTE WCS	OPA-65R-LCUU-H8_2350MHz_07DT	17.2	263	7	Top	FIBER	0						1145.5129		19	
ANTENNA POSITION 4	PORT 1		59367.C.700.4G.1	CTL01077_7C_1	CTL01077_7C_1		LTE 700	HPA-65R-BUU-H8_719MHz_04DT	15.2	263	4	Top	FIBER	0						1044.7202		23	
	PORT 3		59367.C.1900.4G.1	CTL01077_9C_1	CTL01077_9C_1		LTE 1900	HPA-65R-BUU-H8_1930MHz_05DT	17.4	263	5	Top	FIBER	0						3380.6483		23	

## Section 16A - PLANNED/PROPOSED TOWER CONFIGURATION - SECTOR A (OR OMNI)

ANTENNA POSITION IS LEFT to RIGHT from BACK OF ANTENNA (unless otherwise specified)	ANTENNA POSITION 1		ANTENNA POSITION 2		ANTENNA POSITION 3		ANTENNA POSITION 4		ANTENNA POSITION 5		ANTENNA POSITION 6		ANTENNA POSITION 7	
Existing Antenna?														
ANTENNA MAKE - MODEL					TPA-65R-LCUUUU-H8									
ANTENNA VENDOR					CCI									
ANTENNA SIZE (H x W x D)					96X14.4X8.6									
ANTENNA WEIGHT					75									
AZIMUTH					23									
MAGNETIC DECLINATION														
RADIATION CENTER (feet)					185									
ANTENNA TIP HEIGHT					189									
MECHANICAL DOWNTILT					0									
FEEDER AMOUNT														
VERTICAL SEPARATION from ANTENNA ABOVE (TIP to TIP)														
VERTICAL SEPARATION from ANTENNA BELOW (TIP to TIP)														
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)														
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)														
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # / # of inches)														
Antenna RET Motor (QTY/MODEL)						Built-in								
SURGE ARRESTOR (QTY/MODEL)					1	DC Fiber Squid								
DIPLEXER (QTY/MODEL)														
DUPLEXER (QTY/MODEL)														
Antenna RET CONTROL UNIT (QTY/MODEL)						RRH Controlled								
DC BLOCK (QTY/MODEL)														
TMA/LNA (QTY/MODEL)														
CURRENT INJECTORS FOR TMA (QTY/MODEL)														
PDU FOR TMAS (QTY/MODEL)														
FILTER (QTY/MODEL)														
SQUID (QTY/MODEL)														
FIBER TRUNK (QTY/MODEL)														
DC TRUNK (QTY/MODEL)														
RRH - 700 band (QTY/MODEL)					1	B14 4478								
RRH - 850 band (QTY/MODEL)														
RRH - 1900 band (QTY/MODEL)					1	RRUS-32 B2								
RRH - AWS band (QTY/MODEL)					1	RRUS-32 B66								
RRH - WCS band (QTY/MODEL)														
Additional RRH #1 - any band (QTY/MODEL)														
Additional RRH #2 - any band (QTY/MODEL)														
Additional Component 1 (QTY/MODEL)														
Additional Component 2 (QTY/MODEL)														
Additional Component 3 (QTY/MODEL)														
Local Market Note 1	LTE 6C AWS J, LTE 7C 700 UPPER D, RRH Add 1900 A3-A4 & E & C5- Add a 12 port Antenna at Pos3.- Add B14 radio, Add RRUS-32 B66 for AWS, Add RRUS-32 B2 for PCS.- Add 1 DC Fiber Squid { Showing total of 1 Squid per sector }./ ADD 2*5216+2XMU+IDL6													
Local Market Note 2														
Local Market Note 3	2*5216 +2*XMU + IDLe													

PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	ATOLL CELL ID	TX/RX ?	TECHNOLOGY/FREQ UENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/ Integrated/No ne)	FEEDERS TYPE	FEEDER LENGTH (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLAT E POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID (CSSNG)
ANTENNA POSITION 3	PORT 1			CTL00077_7A_3_F	CTL00077_7A_3_F	0	LTE 700	TPA-65R-LCUUUU- H8_776MHz_02DT	15.4	23	2	Top	Fiber	0						1475.7065			
	PORT 3			CTL00077_2A_1	CTL00077_2A_1	0	LTE AWS	TPA-65R-LCUUUU- H8_2133MHz_06DT	16.6	23	6	Top	Fiber	0						2535.1286			
	PORT 4			CTL00077_2A_2	CTL00077_2A_2	0	LTE AWS	TPA-65R-LCUUUU- H8_2133MHz_06DT	16.6	23	6	Top	Fiber	0						2535.1286			
	PORT 7			CTL01077_9A_3	CTL01077_9A_3	0	LTE 1900	TPA-65R-LCUUUU- H8_1930MHz_06DT	16.3	23	6	Top	Fiber	0									





Section 17A - FINAL TOWER CONFIGURATION - SECTOR A (OR OMNI)

ANTENNA POSITION is LEFT to RIGHT from BACK OF ANTENNA (unless otherwise specified)		ANTENNA POSITION 1		ANTENNA POSITION 2		ANTENNA POSITION 3		ANTENNA POSITION 4		ANTENNA POSITION 5		ANTENNA POSITION 6		ANTENNA POSITION 7	
ANTENNA MAKE - MODEL		7770		OPA-65R-LCUU-H8		TPA-65R-LCUUUU-H8		HPA-65R-BUU-H8							
ANTENNA VENDOR		POWERWAVE		CCI Antennas		CCI		CCI Products							
ANTENNA SIZE (H x W x D)		55X11X5		92.7X14.4X7		96X14.4X8.6		92.4X14.8X7.4							
ANTENNA WEIGHT		35		88		75		68							
AZIMUTH		143		23		23		23							
MAGNETIC DECLINATION															
RADIATION CENTER (feet)		185		185		185		185							
ANTENNA TIP HEIGHT		187		189		189		189							
MECHANICAL DOWNTILT		0		0		0		0							
FEEDER AMOUNT		2		Fiber + 2 Coax											
VERTICAL SEPARATION from ANTENNA ABOVE (TIP to TIP)															
VERTICAL SEPARATION from ANTENNA BELOW (TIP to TIP)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # / # of inches)															
Antenna RET Motor (QTY/MODEL)		2	Powerwave 7020		Built-in		Built-in		Built-in						
SURGE ARRESTOR (QTY/MODEL)				5	DC/Fiber Squid (1) + AndrewAndrew APTDC-BDFDM-DB (4)										
DIPLEXER (QTY/MODEL)		1	Kaelus DBC2055F1V1-2	4	CCI Triplexer -TPX-070821										
DUPLEXER (QTY/MODEL)															
Antenna RET CONTROL UNIT (QTY/MODEL)		1	Powerwave 7070		RRH Controlled		RRH Controlled		RRH Controlled						
DC BLOCK (QTY/MODEL)															
TMA/LNA (QTY/MODEL)		1	CCI DTMABP7819VG12A (Twin 700/850 Bypass)												
CURRENT INJECTORS FOR TMA (QTY/MODEL)			AISG Compliant Diplexer												
PDU FOR TMA5 (QTY/MODEL)															
FILTER (QTY/MODEL)															
SQUID (QTY/MODEL)															
FIBER TRUNK (QTY/MODEL)															
DC TRUNK (QTY/MODEL)															
RRH - 700 band (QTY/MODEL)				1	RRUS-E2	1	B14 4478	1	RRUS-11						
RRH - 850 band (QTY/MODEL)		1	RRUW	1	RRUS-11										
RRH - 1900 band (QTY/MODEL)		1	RRUW			1	RRUS-32 B2	1	RRUS-32 B2						
RRH - AWS band (QTY/MODEL)						1	RRUS-32 B66								
RRH - WCS band (QTY/MODEL)				1	RRUS-32										
Additional RRH #1 - any band (QTY/MODEL)															
Additional RRH #2 - any band (QTY/MODEL)															
Additional Component 1 (QTY/MODEL)															
Additional Component 2 (QTY/MODEL)															
Additional Component 3 (QTY/MODEL)															
Local Market Note 1		LTE 6C AWS J, LTE 7C 700 UPPER D, RRH Add 1900 A3-A4 & E & C5 - Add a 12 port Antenna at POs3, - Add B14 radio, Add RRUS-32 B66 for AWS, Add RRUS-32 B2 for PCS. - Add 1 DC Fiber Squid ( Showing total of 1 Squid per sector )// ADD 2*5216+2XMU+IDL6													
Local Market Note 2															
Local Market Note 3		2*5216 +2*XMU + IDL6													

PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	ATOLL CELL ID	TX/RX ?	TECHNOLOGY/FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID (CSSNG)
ANTENNA POSITION 1	PORT 1	59367.A.850.3G.1		CTV10771	CTV10771		UMTS 850	7770.00.850.10	13.5	143	10	None	Andrew 1-5/8	250						264.24		1	
	PORT 2	59367.A.850.3G.1,59367.A.850.3G.2		CTV1077A	CTV1077A		UMTS 850	7770.00.850.10	13.5	143	10	Bottom	Andrew 1-5/8	250						264.24		1	
ANTENNA POSITION 2	PORT 1	59367.A.850.4G.1		CTL00077_8A_1	CTL00077_8A_1		LTE 850	OPA-65R-LCUU-H8_849MHz_02DT	15.4	23	2	Bottom	Andrew 1-5/8	250						1000		3	
	PORT 2	59367.A.700.4G.3		CTL00077_7A_2_E	CTL00077_7A_2_E		LTE 700	OPA-65R-LCUU-	14.7	23	2	Bottom	Andrew 1-5/8	250						1475.7065		3	

							H8_719MHz_02DT																
	PORT 3	59367.A.WCS.4G.1		CTL01077_3A_1	CTL01077_3A_1		LTE WCS	OPA-65R-LCUU-H8_2350MHz_06DT	17.4	23	6	Top	Fiber	0						1285.2866		4	
ANTENNA POSITION 3	PORT 1	59367.A.700.4G.tmp5		CTL00077_7A_3_F	CTL00077_7A_3_F		LTE 700	TPA-65R-LCUUUU-H8_776MHz_02DT	15.4	23	2	Top	Fiber	0						1475.7065		5	
	PORT 3	59367.A.AWS.4G.tmp1		CTL00077_2A_1	CTL00077_2A_1		LTE AWS	TPA-65R-LCUUUU-H8_2133MHz_06DT	16.6	23	6	Top	Fiber	0						2535.1286		6	
	PORT 4	59367.A.AWS.4G.tmp4		CTL00077_2A_2	CTL00077_2A_2		LTE AWS	TPA-65R-LCUUUU-H8_2133MHz_06DT	16.6	23	6	Top	Fiber	0						2535.1286		6	
	PORT 7	59367.A.1900.4G.tmp3		CTL01077_9A_3	CTL01077_9A_3		LTE 1900	TPA-65R-LCUUUU-H8_1930MHz_06DT	16.3	23	6	Top	Fiber	0						3664.3757		6	
ANTENNA POSITION 4	PORT 1	59367.A.700.4G.1		CTL01077_7A_1	CTL01077_7A_1		LTE 700	HPA-65R-BUU-H8_719MHz_02DT	15.3	23	2	Top	Fiber	0						1475.7065		7	
	PORT 3	59367.A.1900.4G.1		CTL01077_9A_1	CTL01077_9A_1		LTE 1900	HPA-65R-BUU-H8_1930MHz_06DT	17.4	23	6	Top	Fiber	0						3664.3757		8	
	PORT 4	59367.A.1900.4G.1,59367.A.1900.4G.tmp4		CTL01077_9A_2	CTL01077_9A_2		LTE 1900	HPA-65R-BUU-H8_1930MHz_06DT	17.4	23	6	Top	Fiber	0						3664.3757		8	



Section 17B - FINAL TOWER CONFIGURATION - SECTOR B

ANTENNA POSITION is LEFT to RIGHT from BACK OF ANTENNA (unless otherwise specified)		ANTENNA POSITION 1		ANTENNA POSITION 2		ANTENNA POSITION 3		ANTENNA POSITION 4		ANTENNA POSITION 5		ANTENNA POSITION 6		ANTENNA POSITION 7	
ANTENNA MAKE - MODEL		7770		OPA-65R-LCUU-H6		QS66512-2		HPA-65R-BUU-H6							
ANTENNA VENDOR		POWERWAVE		CCI Antennas		Quintel		CCI Products							
ANTENNA SIZE (H x W x D)		55X11X5		72X14.8X7.4		72X12X9.6		72X14.8X9							
ANTENNA WEIGHT		35		73		111		50.7							
AZIMUTH		263		143		143		143							
MAGNETIC DECLINATION															
RADIATION CENTER (feet)		185		185		185		185							
ANTENNA TIP HEIGHT		187		188		188		188							
MECHANICAL DOWNTILT		0		0		0		0							
FEEDER AMOUNT		2		Fiber + 2 Coax											
VERTICAL SEPARATION from ANTENNA ABOVE (TIP to TIP)															
VERTICAL SEPARATION from ANTENNA BELOW (TIP to TIP)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)															
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # / # of inches)															
Antenna RET Motor (QTY/MODEL)		2	Powerwave 7020		Built-in		Built-in		Built-in						
SURGE ARRESTOR (QTY/MODEL)				5	DC/Fiber Squid (1) + AndrewAndrew APTDC-BDFDM-DB (4)										
DIPLEXER (QTY/MODEL)		1	Kaelus DBC2055F1V1-2	4	CCI Triplexer -TPX-070821										
DUPLEXER (QTY/MODEL)															
Antenna RET CONTROL UNIT (QTY/MODEL)					RRH Controlled		RRH Controlled		RRH Controlled						
DC BLOCK (QTY/MODEL)															
TMA/LNA (QTY/MODEL)		1	CCI DTMABP7819VG12A (Twin 700/850 Bypass)												
CURRENT INJECTORS FOR TMA (QTY/MODEL)			AISG Compliant Diplexer												
PDU FOR TMA5 (QTY/MODEL)															
FILTER (QTY/MODEL)															
SQUID (QTY/MODEL)															
FIBER TRUNK (QTY/MODEL)															
DC TRUNK (QTY/MODEL)															
RRH - 700 band (QTY/MODEL)				1	RRUS-E2	1	B14 4478	1	RRUS-11						
RRH - 850 band (QTY/MODEL)		1	RRUW	1	RRUS-11										
RRH - 1900 band (QTY/MODEL)		1	RRUW			1	RRUS-32 B2	1	RRUS-32 B2						
RRH - AWS band (QTY/MODEL)						1	RRUS-32 B66								
RRH - WCS band (QTY/MODEL)				1	RRUS-32										
Additional RRH #1 - any band (QTY/MODEL)															
Additional RRH #2 - any band (QTY/MODEL)															
Additional Component 1 (QTY/MODEL)															
Additional Component 2 (QTY/MODEL)															
Additional Component 3 (QTY/MODEL)															
Local Market Note 1		LTE 6C AWS J, LTE 7C 700 UPPER D, RRH Add 1900 A3-A4 & E & C5 - Add a 12 port Antenna at POs3, - Add B14 radio, Add RRUS-32 B66 for AWS, Add RRUS-32 B2 for PCS. - Add 1 DC Fiber Squid ( Showing total of 1 Squid per sector )// ADD 2*5216+2*XMU+1DL6													
Local Market Note 2															
Local Market Note 3		2*5216 +2*XMU + 1DL6													

PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	ATOLL CELL ID	TX/RX ?	TECHNOLOGY/FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID (CSSNG)
ANTENNA POSITION 1	PORT 1	59367.B.850.3G.1		CTV10772	CTV10772		UMTS 850	7770.00.850.08	13.5	263	8	None	Andrew 1-5/8	250						264.24		9	
	PORT 2	59367.B.850.3G.1,59367.B.850.3G.2		CTV1077B	CTV1077B		UMTS 850	7770.00.850.08	13.5	263	8	Bottom	Andrew 1-5/8	250						264.24		9	
ANTENNA POSITION 2	PORT 1	59367.B.850.4G.1		CTL00077_8B_1	CTL00077_8B_1		LTE 850	OPA-65R-LCUU-H6_849MHz_02DT	14.6	143	2	Bottom	Andrew 1-5/8	250						1000		11	
	PORT 2	59367.B.700.4G.3		CTL00077_7B_2_E	CTL00077_7B_2_E		LTE 700	OPA-65R-LCUU-	14	143	2	Bottom	Andrew 1-5/8	250						1475.7065		11	

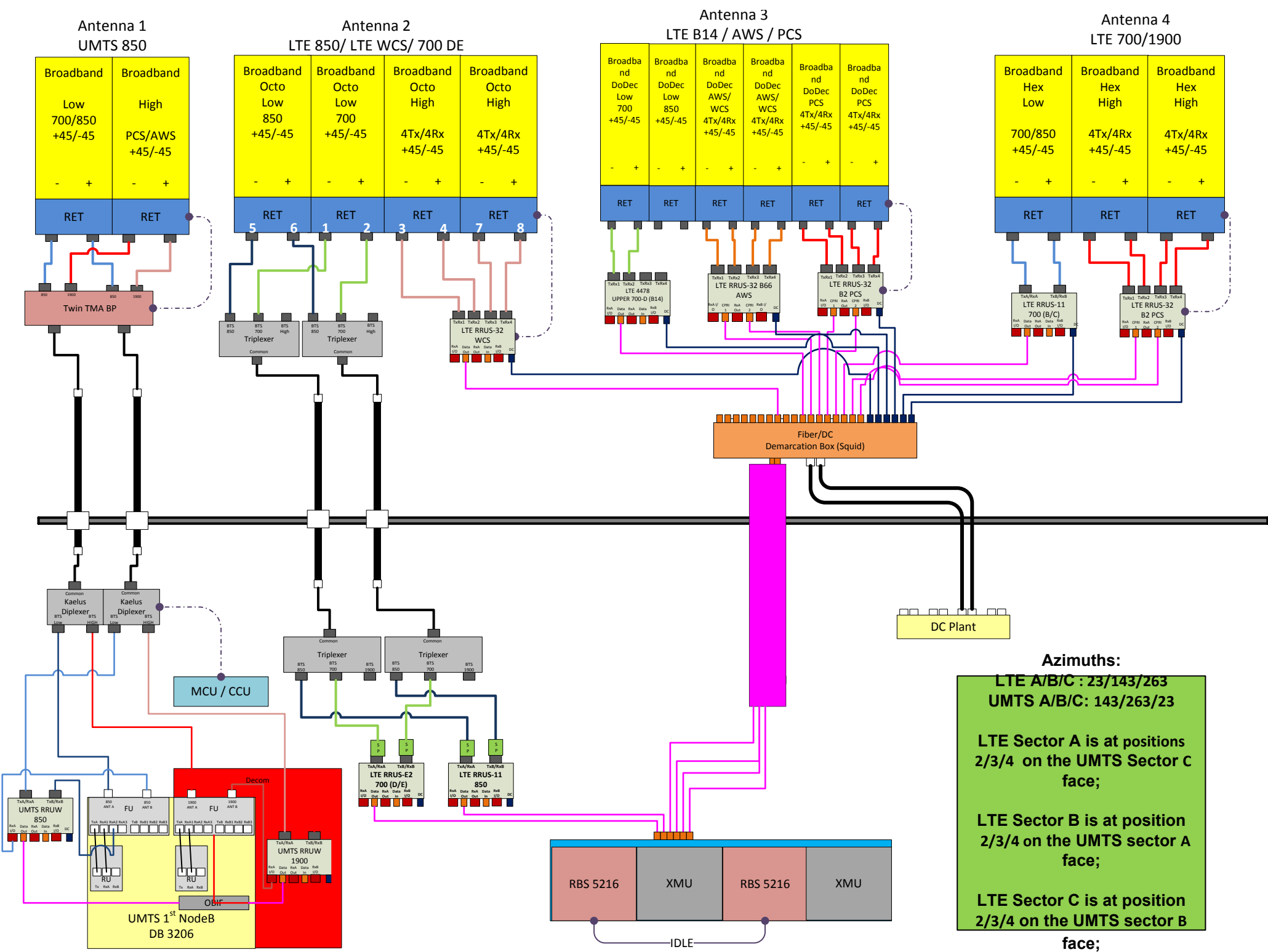
							H6_719MHz_02DT																
	PORT 3	59367.B.WCS.4G.1		CTL01077_3B_1	CTL01077_3B_1		LTE WCS	OPA-65R-LCUU-H6_2350MHz_06DT	17.8	143	6	Top	Fiber	0						1285.2866		12	
ANTENNA POSITION 3	PORT 1	59367.B.700.4G.tmp5		CTL00077_7B_3_F	CTL00077_7B_3_F		LTE 700	QS66512-2_776MHz_02DT	13.7	143	2	Top	Fiber	0						1475.7065		13	
	PORT 3	59367.B.AWS.4G.tmp1		CTL00077_2B_1	CTL00077_2B_1		LTE AWS	QS66512-2_2130MHz_06DT	15.8	143	6	Top	Fiber	0						2535.1286		14	
	PORT 4	59367.B.AWS.4G.tmp4		CTL00077_2B_2	CTL00077_2B_2		LTE AWS	QS66512-2_2130MHz_06DT	15.8	143	6	Top	Fiber	0						2535.1286		14	
	PORT 7	59367.B.1900.4G.tmp3		CTL01077_9B_3	CTL01077_9B_3		LTE 1900	QS66512-2_1930MHz_06DT	15.9	143	6	Top	Fiber	0						3664.3757		14	
ANTENNA POSITION 4	PORT 1	59367.B.700.4G.1		CTL01077_7B_1	CTL01077_7B_1		LTE 700	HPA-65R-BUU-H6_719MHz_02DT	14.28	143	2	Top	Fiber	0						1475.7065		15	
	PORT 3	59367.B.1900.4G.1		CTL01077_9B_1	CTL01077_9B_1		LTE 1900	HPA-65R-BUU-H6_1930MHz_06DT	17.18	143	6	Top	Fiber	0						3664.3757		16	
	PORT 4	59367.B.1900.4G.1,59367.B.1900.4G.tmp4		CTL01077_9B_2	CTL01077_9B_2		LTE 1900	HPA-65R-BUU-H6_1930MHz_06DT	17.18	143	6	Top	Fiber	0						3664.3757		16	

Section 17C - FINAL TOWER CONFIGURATION - SECTOR C

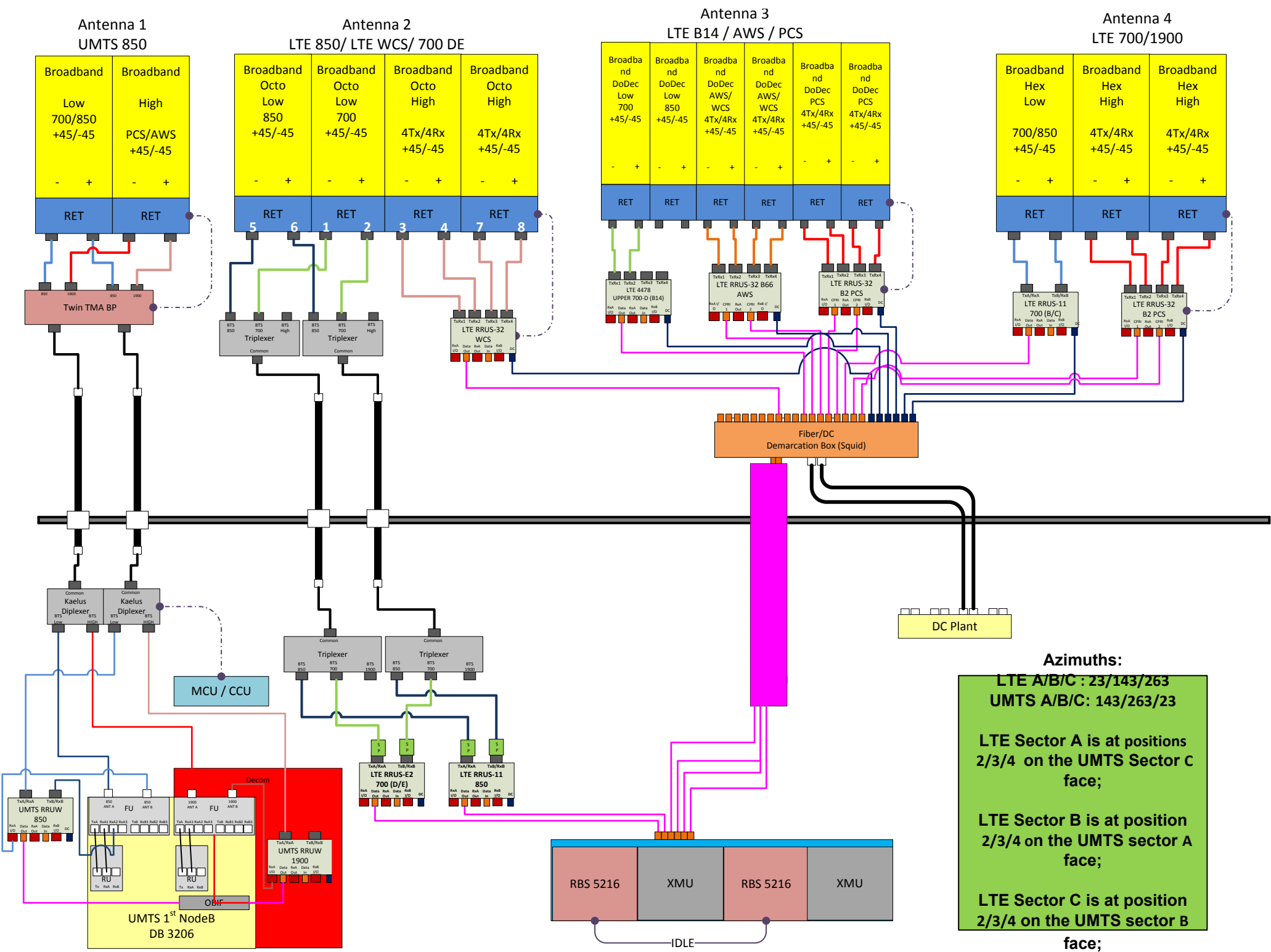
ANTENNA POSITION is LEFT to RIGHT from BACK OF ANTENNA (unless otherwise specified)	ANTENNA POSITION 1		ANTENNA POSITION 2		ANTENNA POSITION 3		ANTENNA POSITION 4		ANTENNA POSITION 5		ANTENNA POSITION 6		ANTENNA POSITION 7	
ANTENNA MAKE - MODEL	7770		OPA-65R-LCUU-H8		TPA-65R-LCUUUU-H8		HPA-65R-BUU-H8							
ANTENNA VENDOR	POWERWAVE		CCI Antennas		CCI		CCI Products							
ANTENNA SIZE (H x W x D)	55X11X5		92.7X14.4X7		96X14.4X8.6		92.4X14.8X7.4							
ANTENNA WEIGHT	35		88		75		68							
AZIMUTH	23		263		263		263							
MAGNETIC DECLINATION														
RADIATION CENTER (feet)	185		185		185		185							
ANTENNA TIP HEIGHT	187		189		189		189							
MECHANICAL DOWNTILT	0		0		0		0							
FEEDER AMOUNT	2		Fiber + 2 Coax											
VERTICAL SEPARATION from ANTENNA ABOVE (TIP to TIP)														
VERTICAL SEPARATION from ANTENNA BELOW (TIP to TIP)														
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)														
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)														
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # / # of inches)														
Antenna RET Motor (QTY/MODEL)	2	Powerwave 7020		Built-in		Built-in		Built-in						
SURGE ARRESTOR (QTY/MODEL)			5	DC/Fiber Squid (1) + AndrewAndrew APTDC-BDFDM-DB (4)										
DIPLEXER (QTY/MODEL)	1	Kaelus DBC2055F1V1-2	4	CCI Triplexer -TPX-070821										
DUPLEXER (QTY/MODEL)														
Antenna RET CONTROL UNIT (QTY/MODEL)				RRH Controlled		RRH Controlled		RRH Controlled						
DC BLOCK (QTY/MODEL)														
TMA/LNA (QTY/MODEL)	1	CCI DTMABP7819VG12A (Twin 700/850 Bypass)												
CURRENT INJECTORS FOR TMA (QTY/MODEL)		AISG Compliant Diplexer												
PDU FOR TMA5 (QTY/MODEL)														
FILTER (QTY/MODEL)														
SQUID (QTY/MODEL)														
FIBER TRUNK (QTY/MODEL)														
DC TRUNK (QTY/MODEL)														
RRH - 700 band (QTY/MODEL)			1	RRUS-E2	1	B14 4478	1	RRUS-11						
RRH - 850 band (QTY/MODEL)	1	RRUW	1	RRUS-11										
RRH - 1900 band (QTY/MODEL)	1	RRUW			1	RRUS-32 B2	1	RRUS-32 B2						
RRH - AWS band (QTY/MODEL)					1	RRUS-32 B66								
RRH - WCS band (QTY/MODEL)			1	RRUS-32										
Additional RRH #1 - any band (QTY/MODEL)														
Additional RRH #2 - any band (QTY/MODEL)														
Additional Component 1 (QTY/MODEL)														
Additional Component 2 (QTY/MODEL)														
Additional Component 3 (QTY/MODEL)														
Local Market Note 1	LTE 6C AWS J, LTE 7C 700 UPPER D, RRH Add 1900 A3-A4 & E & C5 - Add a 12 port Antenna at POs3, - Add B14 radio, Add RRUS-32 B66 for AWS, Add RRUS-32 B2 for PCS. - Add 1 DC Fiber Squid ( Showing total of 1 Squid per sector )// ADD 2*5216+2XMU+IDLe													
Local Market Note 2														
Local Market Note 3	2*5216 +2*XMU + IDLe													

PORT SPECIFIC FIELDS	PORT NUMBER	USEID (CSSng)	USEID (Atoll)	ATOLL TXID	ATOLL CELL ID	TX/RX ?	TECHNOLOGY/FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	RXAIT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPA/MCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID (CSSNG)
ANTENNA POSITION 1	PORT 1	59367.C.850.3G.1		CTV10773	CTV10773		UMTS 850	7770.00.850.06	13.5	23	6	None	Andrew 1-5/8	250						264.24		17	
	PORT 2	59367.C.850.3G.1,59367.C.850.3G.2		CTV1077C	CTV1077C		UMTS 850	7770.00.850.06	13.5	23	6	Bottom	Andrew 1-5/8	250						264.24		17	
ANTENNA POSITION 2	PORT 1	59367.C.850.4G.1		CTL00077_8C_1	CTL00077_8C_1		LTE 850	OPA-65R-LCUU-H8_849MHz_04DT	15.4	263	4	Bottom	Andrew 1-5/8	250						1000		19	
	PORT 2	59367.C.700.4G.4		CTL00077_7C_2_E	CTL00077_7C_2_E		LTE 700	OPA-65R-LCUU-	14.8	263	4	Bottom	Andrew 1-5/8	250						1475.7065		19	

							H8_719MHz_04DT															
	PORT 3	59367.C.WCS.4G.1		CTL01077_3C_1	CTL01077_3C_1		LTE WCS	OPA-65R-LCUU-H8_2350MHz_06DT	17.4	263	6	Top	Fiber	0					1285.2866		20	
ANTENNA POSITION 3	PORT 1	59367.C.700.4G.tmp5		CTL00077_7C_3_F	CTL00077_7C_3_F		LTE 700	TPA-65R-LCUUUU-H8_776MHz_04DT	15.5	263	4	Top	Fiber	0					1475.7065		21	
	PORT 3	59367.C.AWS.4G.tmp1		CTL00077_2C_1	CTL00077_2C_1		LTE AWS	TPA-65R-LCUUUU-H8_2133MHz_07DT	16.5	263	7	Top	Fiber	0					2535.1286		22	
	PORT 4	59367.C.AWS.4G.tmp4		CTL00077_2C_2	CTL00077_2C_2		LTE AWS	TPA-65R-LCUUUU-H8_2133MHz_07DT	16.5	263	7	Top	Fiber	0					2535.1286		22	
	PORT 7	59367.C.1900.4G.tmp3		CTL01077_9C_3	CTL01077_9C_3		LTE 1900	TPA-65R-LCUUUU-H8_1930MHz_07DT	16.3	263	7	Top	Fiber	0					3664.3757		22	
ANTENNA POSITION 4	PORT 1	59367.C.700.4G.1		CTL01077_7C_1	CTL01077_7C_1		LTE 700	HPA-65R-BUU-H8_719MHz_04DT	15.2	263	4	Top	Fiber	0					1475.7065		23	
	PORT 3	59367.C.1900.4G.1		CTL01077_9C_1	CTL01077_9C_1		LTE 1900	HPA-65R-BUU-H8_1930MHz_07DT	17.4	263	7	Top	Fiber	0					3664.3757		24	
	PORT 4	59367.C.1900.4G.1,59367.C.1900.4G.tmp4		CTL01077_9C_2	CTL01077_9C_2		LTE 1900	HPA-65R-BUU-H8_1930MHz_07DT	17.4	263	7	Top	Fiber	0					3664.3757		24	







**Azimuths:**

**LTE A/B/C : 23/143/263**  
**UMTS A/B/C: 143/263/23**

**LTE Sector A is at positions 2/3/4 on the UMTS Sector C face;**

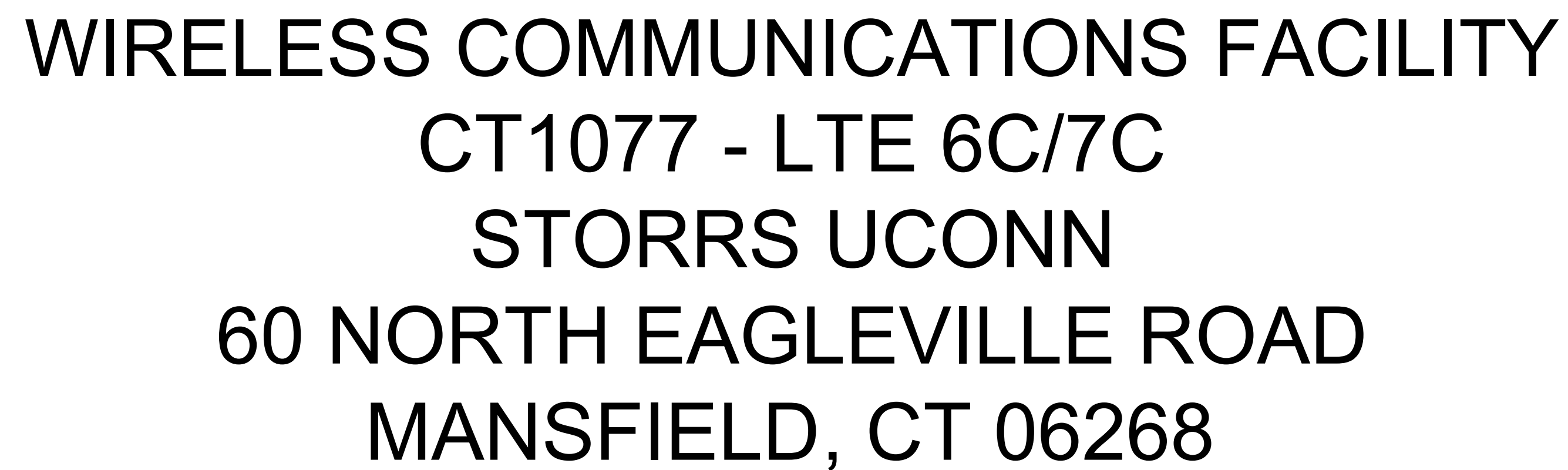
**LTE Sector B is at position 2/3/4 on the UMTS sector A face;**

**LTE Sector C is at position 2/3/4 on the UMTS sector B face;**

WORKFLOW SUMMARY

Date	FROM State / Status	FROM ATTUID	TO State / Status	TO ATTUID	Operation	Comments	PACE Status
06/08/2017	Preliminary In Progress	mm093q	Preliminary Submitted for Approval	RC475S	Promote	Preliminary RFDS	NER-RCTB-17-02148 MRCTB023976 SUCCESS 06/08/2017 10:20:15 AM NER-RCTB-17-02235 MRCTB023938 SUCCESS 06/08/2017 10:20:15 AM NER-RCTB-17-02296 MRCTB024148 SUCCESS 06/08/2017 10:20:15 AM
07/13/2017	Preliminary Submitted for Approval	RC475S	Preliminary Approved	DC5778	Promote		
07/26/2017	Preliminary Approved	DC5778	Preliminary In Progress	rx855w	Pull Back	RFDS Update as per Demotion/ Promotion comments	
07/27/2017	Preliminary In Progress	rx855w	Preliminary Submitted for Approval	RC475S	Promote	RFDS as per scoping session held on "07-27-2017". This RFDS can be promoted for Final RF Approval.	NER-RCTB-17-02148 FAILURE 07/27/2017 4:46:07 PM NER-RCTB-17-02235 FAILURE 07/27/2017 4:46:07 PM NER-RCTB-17-02296 FAILURE 07/27/2017 4:46:07 PM
07/31/2017	Preliminary Submitted for Approval	RC475S	Preliminary Approved	DC5778	Promote		
08/01/2017	Preliminary Approved	DC5778	Final RF Approval	OM636A	Promote	Please promote to final	





1. ALL WORK SHALL BE IN ACCORDANCE WITH THE 2012 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2016 CONNECTICUT STATE BUILDING CODE, INCLUDING THE TIA-222 REVISION "G" STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND SUPPORTING STRUCTURES, 2016 CONNECTICUT FIRE SAFETY CODE AND, NATIONAL ELECTRICAL CODE AND LOCAL CODES.
2. THE COMPOUND, TOWER, PRIMARY GROUND RING, ELECTRICAL SERVICE TO THE METER BANK AND TELEPHONE SERVICE TO THE DEMARCATION POINT ARE PROVIDED BY SITE OWNER. AS BUILT FIELD CONDITIONS REGARDING THESE ITEMS SHALL BE CONFIRMED BY THE CONTRACTOR. SHOULD ANY FIELD CONDITIONS PRECLUDE COMPLIANCE WITH THE DRAWINGS, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER AND SHALL NOT PROCEED WITH ANY AFFECTED WORK.
3. CONTRACTOR SHALL REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. CONTRACTOR SHALL COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS TO ALL SUBCONTRACTORS AND ALL RELATED PARTIES. THE SUBCONTRACTORS SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT AFFECTS THEIR WORK.
4. CONTRACTOR SHALL PROVIDE A COMPLETE BUILD-OUT WITH ALL FINISHES, STRUCTURAL, MECHANICAL, AND ELECTRICAL COMPONENTS AND PROVIDE ALL ITEMS AS SHOWN OR INDICATED ON THE DRAWINGS OR IN THE WRITTEN SPECIFICATIONS.
5. CONTRACTOR SHALL FURNISH ALL MATERIAL, LABOR AND EQUIPMENT TO COMPLETE THE WORK AND FURNISH A COMPLETED JOB ALL IN ACCORDANCE WITH LOCAL AND STATE GOVERNING AUTHORITIES AND OTHER AUTHORITIES HAVING LAWFUL JURISDICTION OVER THE WORK.
6. CONTRACTOR SHALL SECURE AND PAY FOR ALL PERMITS AND ALL INSPECTIONS REQUIRED AND SHALL ALSO PAY FEES REQUIRED FOR THE GENERAL CONSTRUCTION, PLUMBING, ELECTRICAL AND HVAC. PERMITS SHALL BE PAID FOR BY THE RESPECTIVE SUBCONTRACTORS.
7. CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS ON SITE AT ALL TIMES AND INSURE DISTRIBUTION OF NEW DRAWINGS TO SUBCONTRACTORS AND OTHER RELEVANT PARTIES AS SOON AS THEY ARE MADE AVAILABLE. ALL OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONTRACT AREA. THE CONTRACTOR SHALL FURNISH AN "AS-BUILT" SET OF DRAWINGS TO OWNER UPON COMPLETION OF PROJECT.
8. LOCATION OF EQUIPMENT, AND WORK SUPPLIED BY OTHERS THAT IS DIAGRAMMATICALLY INDICATED ON THE DRAWINGS SHALL BE DETERMINED BY THE CONTRACTOR. THE CONTRACTOR SHALL DETERMINE LOCATIONS AND DIMENSIONS SUBJECT TO STRUCTURAL CONDITIONS AND WORK OF THE SUBCONTRACTORS.
9. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING BUILDING'S/PROPERTY'S OPERATIONS, COORDINATE WORK WITH BUILDING/PROPERTY OWNER.
10. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
11. ALL UTILITY WORK SHALL BE IN ACCORDANCE WITH LOCAL UTILITY COMPANY REQUIREMENTS AND SPECIFICATIONS.
12. ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE REVIEWED BY CONTRACTOR AND ALL APPLICABLE SUBCONTRACTORS FOR ANY CONDITION PER MFR.'S RECOMMENDATIONS. CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST TO OWNER OR CONSTRUCTION MANAGER.
13. ANY AND ALL ERRORS, DISCREPANCIES, AND "MISSED" ITEMS ARE TO BE BROUGHT TO THE ATTENTION OF THE AT&T CONSTRUCTION MANAGER DURING THE BIDDING PROCESS BY THE CONTRACTOR. ALL THESE ITEMS ARE TO BE INCLUDED IN THE BID. NO 'EXTRA' WILL BE ALLOWED FOR MISSED ITEMS.
14. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ON-SITE SAFETY FROM THE TIME THE JOB IS AWARDED UNTIL ALL WORK IS COMPLETE AND ACCEPTED BY THE OWNER.
15. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE CONSTRUCTION MANAGER FOR REVIEW.
16. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND EXISTING CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA.
17. COORDINATION, LAYOUT, FURNISHING AND INSTALLATION OF CONDUIT AND ALL APPURTENANCES REQUIRED FOR PROPER INSTALLATION OF ELECTRICAL AND TELECOMMUNICATION SERVICE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
18. ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE REVIEWED BY CONTRACTOR AND ALL APPLICABLE SUB- CONTRACTORS FOR ANY CONDITION PER THE MANUFACTURER'S RECOMMENDATIONS. CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST TO OWNER OR CONSTRUCTION MANAGER.
19. ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD RESPONSIBLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
20. THE CONTRACTOR SHALL CONTACT "CALL BEFORE YOU DIG" AT LEAST 48 HOURS PRIOR TO ANY EXCAVATIONS AT 1-800-922-4455. ALL UTILITIES SHALL BE IDENTIFIED AND CLEARLY MARKED PRIOR TO ANY EXCAVATION WORK. CONTRACTOR SHALL MAINTAIN AND PROTECT MARKED UTILITIES THROUGHOUT PROJECT COMPLETION.
21. CONTRACTOR SHALL COMPLY WITH OWNERS ENVIRONMENTAL ENGINEER ON ALL METHODS AND PROVISIONS FOR ALL EXCAVATION ACTIVITIES INCLUDING SOIL DISPOSAL. ALL BACKFILL MATERIALS TO BE PROVIDED BY

FROM:	500 ENTERPRISE DRIVE ROCKY HILL, CONNECTICUT	TO:	60 N.EAGLEVILLE ROAD MANSFIELD, CONNECTICUT
1.	HEAD NORTHEAST ON ENTERPRISE DR TOWARD CAPITAL BLVD		0.36 MI
2.	TURN LEFT ONTO CAPITAL BLVD		0.27 MI
3.	TURN LEFT ONTO WEST ST		0.16 MI
4.	TURN LEFT ONTO I-91 N VIA THE RAMP ON THE LEFT TOWARD HARTFORD.		0.73 MI
5.	MERGE ONTO CT-15 N/WILBUR CROSS HWY N VIA EXIT 29 TOWARD I-84 E/E HARTFORD/BOSTON.		2.14 MI
6.	CT-15 N/WILBUR CROSS HWY N BECOMES I-84 E/US-6 E/WILBUR CROSS HWY N		1.50 MI
7.	KEEP LEFT TO TAKE I-84 E/WILBUR CROSS HWY N TOWARD BOSTON		14.45 MI
8.	TAKE THE CT-195 EXIT, EXIT 68, TOWARD TOLLAND/MANSFIELD.		0.27 MI
9.	TURN RIGHT ONTO N EAGLEVILLE RD		7.11 MI
10.	N.EAGLEVILLE ROAD IN ON THE LEFT		

1. THE PROPOSED SCOPE OF WORK CONSISTS OF A MODIFICATION TO THE EXISTING UNMANNED TELECOMMUNICATIONS FACILITY INCLUDING THE FOLLOWING:

- A. INSTALL (3) NEW (12) PORT ANTENNAS, (1) PER SECTOR ON POSITION 3.
- B. INSTALL (3) NEW RRUS-32 B66 ON POSITION 3 ANTENNAS, (1) PER SECTOR.
- C. INSTALL (3) NEW RRUS-32 B2 ON POSITION 3 ANTENNAS, (1) PER SECTOR.
- D. INSTALL (3) NEW RRUS-B14 ON POSITION 3 ANTENNAS, (1) PER SECTOR.
- E. INSTALL (1) NEW SURGER ARRESTOR WITHIN GAMMA SECTOR.

AT&T SITE NUMBER:	CT1077
AT&T SITE NAME:	STORRS—UCCONN
SITE ADDRESS:	60 NORTH EAGLEVILLE ROAD MANSFIELD, CT 06268
LESSEE/APPLICANT:	AT&T MOBILITY 500 ENTERPRISE DRIVE, SUITE 3A ROCKY HILL, CT 06067
ENGINEER:	CENTEK ENGINEERING, INC. 63-2 NORTH BRANFORD RD. BRANFORD, CT 06405
PROJECT COORDINATES:	LATITUDE: 41°-48'-50.55" N LONGITUDE: 72°-15'-34.02" W GROUND ELEVATION: ±853' AMSL SITE COORDINATES AND GROUND ELEVATION REFERENCED FROM GOOGLE EARTH.

SHT. NO.	DESCRIPTION	REV.
T-1	TITLE SHEET	1
N-1	NOTES, SPECIFICATIONS AND DETAILS	1
C-1	PLANS AND ELEVATION	1
C-2	LTE 6C/7C EQUIPMENT DETAILS	1
E-1	LTE SCHEMATIC DIAGRAM AND NOTES	1
E-2	LTE WIRING DIAGRAM	1
E-3	TYPICAL ELECTRICAL DETAILS	1



WIRELESS COMMUNICATIONS FACILITY  
**STORRS-UCONN**  
SITE NUMBER: CT1107  
60 NORTH EAGLEVILLE ROAD  
MANSFIELD, CT 06268

TE:	08/31/17
SALE:	AS NOTED
B NO.	17004.42

# TITLE SHEET

**T-1**

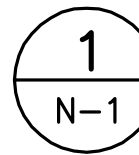
Sheet No. 1 of 7





NOTES:

1. AT&T SHALL SUPPLY RRU, AND RRU POLE-MOUNTING BRACKET. CONTRACTOR SHALL SUPPLY POLE/PIPE AND INSTALL ALL MOUNTING HARDWARE INCLUDING ERICSSON RRU POLE-MOUNTING BRACKET. CONTRACTOR SHALL INSTALLS RRU AND MAKES CABLE TERMINATIONS.
2. NO PAINTING OF THE RRU OR SOLAR SHIELD IS ALLOWED.



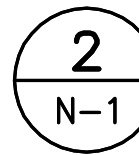
## TYPICAL RRUS MOUNTING DETAILS

SCALE: NTS



NOTES:

1. RAYCAP VIA AT&T SUPPLIES THE DC6 OVER VOLTAGE PROTECTOR AND PIPE MOUNTING BRACKETS. SUBCONTRACTOR SHALL SUPPLY THE PIPE.



### RAYCAP DC6 MOUNTING DETAIL

SCALE: 3" = 1'-0"

## NOTES AND SPECIFICATIONS

**DESIGN BASIS:**

GOVERNING CODE: 2012 INTERNATIONAL BUILDING (IBC) AS MODIFIED BY  
THE 2016 CT STATE BUILDING CODE AND AMENDMENTS.

1. DESIGN CRITERIA:
- WIND LOAD: PER TIA 222 G (ANTENNA MOUNTS): 95–105 MPH (3 SECOND GUST)
  - RISK CATEGORY: II (BASED ON IBC APPEAL N)
  - NOMINAL DESIGN SPEED (TOWER): 101 MPH ( $V_{asd}$ ) (EXPOSURE C/IMPORTANCE FACTOR 1.0 BASED ON ASCE 7–10) PER 2012 INTERNATIONAL BUILDING CODE (IBC) AS MODIFIED BY THE 2016 CONNECTICUT STATE BUILDING CODE.
  - SEISMIC LOAD (DOES NOT CONTROL): PER ASCE 7–10 MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES.

**GENERAL NOTES:**

1. ALL CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE GOVERNING BUILDING CODE.
2. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE IN EXCESS OF STANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
3. BEFORE BEGINNING THE WORK, THE CONTRACTOR IS RESPONSIBLE FOR MAKING SUCH INVESTIGATION CONCERNING PHYSICAL CONDITIONS (SURFACE AND SUBSURFACE) AT OR CONTIGUOUS TO THE SITE WHICH MAY AFFECT PERFORMANCE AND COST OF THE WORK.
4. DIMENSIONS AND DETAILS SHALL BE CHECKED AGAINST EXISTING FIELD CONDITIONS.
5. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS, SLEEVES AND ANCHOR BOLTS AS REQUIRED BY ALL TRADES.
6. ALL DIMENSIONS, ELEVATIONS, AND OTHER REFERENCES TO EXISTING STRUCTURES, SURFACE, AND SUBSURFACE CONDITIONS ARE APPROXIMATE. NO GUARANTEE IS MADE FOR THE ACCURACY OR COMPLETENESS OF THE INFORMATION SHOWN. THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS, ELEVATIONS, ANGLES WITH EXISTING CONDITIONS AND WITH ARCHITECTURAL AND SITE DRAWINGS BEFORE PROCEEDING WITH ANY WORK.
7. AS THE WORK PROGRESSES, THE CONTRACTOR SHALL NOTIFY THE OWNER OF ANY CONDITIONS WHICH ARE IN CONFLICT OR OTHERWISE NOT CONSISTENT WITH THE CONSTRUCTION DOCUMENTS AND SHALL NOT PROCEED WITH SUCH WORK UNTIL THE CONFLICT IS SATISFACTORILY RESOLVED.
8. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR MAINTAINING ADEQUATE SHIELDING, BRACING, AND BARRICADES AS MAY BE REQUIRED FOR THE PROTECTION OF EXISTING PROPERTY, CONSTRUCTION WORKERS, AND FOR PUBLIC SAFETY.
9. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING SITE OPERATIONS, COORDINATE WORK WITH NORTHEAST UTILITIES
10. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER FOUNDATION REMEDIATION WORK IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, GUYS OR TIEDOWNS, WHICH MIGHT BE NECESSARY.
11. ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
12. SHOP DRAWINGS, CONCRETE MIX DESIGNS, TEST REPORTS, AND OTHER SUBMITTALS PERTAINING TO STRUCTURAL WORK SHALL BE FORWARDED TO THE OWNER FOR REVIEW BEFORE FABRICATION AND/OR INSTALLATION IS MADE. SHOP DRAWINGS SHALL INCLUDE ERECTION DRAWINGS AND COMPLETE DETAILS OF CONNECTIONS AS WELL AS MANUFACTURER'S SPECIFICATION DATA WHERE APPROPRIATE. SHOP DRAWINGS SHALL BE CHECKED BY THE CONTRACTOR AND BEAR THE CHECKER'S INITIALS BEFORE BEING SUBMITTED FOR REVIEW.
13. NO DRILLING WELDING OR TAPING ON EVERSOURCE OWNED EQUIPMENT.
14. REFER TO DRAWING T1 FOR ADDITIONAL NOTES AND REQUIREMENTS.
15. SIZE AND TYPE OF FASTENERS AND ACCESSORIES. INCLUDE ERECTION DRAWINGS, ELEVATIONS AND DETAILS.
16. STRUCTURAL STEEL SHALL BE DETAIL, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST PROVISIONS OF AISC MANUAL OF STEEL CONSTRUCTION.
17. PROVIDE ALL PLATES, CLIP ANGLES, CLOSURE PIECES, STRAP ANCHORS, MISCELLANEOUS PIECES AND HOLES REQUIRED TO COMPLETE THE STRUCTURE.
18. FIT AND SHOP ASSEMBLE FABRICATIONS IN THE LARGEST PRACTICAL SECTIONS FOR DELIVERY TO SITE.
19. INSTALL FABRICATIONS PLUMB AND LEVEL, ACCURATELY FITTED, AND FREE FROM DISTORTIONS OR DEFECTS.
20. AFTER ERECTION OF STRUCTURES, TOUCHUP ALL WELDS, ABRASIONS AND NON-GALVANIZED SURFACES WITH A 95% ORGANIC ZINC RICH PAINT IN ACCORDANCE WITH ASTM 780.
21. ALL STEEL MATERIAL (EXPOSED TO WEATHER) SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT DIPPED GALVANIZED) COATINGS" ON IRONS AND STEEL PRODUCTS.
22. ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING (HOT-DIP) ON IRON AND STEEL HARDWARE".
23. THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NON CONFORMING MATERIALS OR CONDITIONS TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW.
24. CONNECTION ANGLES SHALL HAVE A MINIMUM THICKNESS OF 1/4 INCHES.
25. STRUCTURAL CONNECTION BOLTS SHALL CONFORM TO ASTM A325. ALL BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL HAVE A MINIMUM OF TWO BOLTS, UNLESS OTHERWISE ON THE DRAWINGS.
26. LOCK WASHER ARE NOT PERMITTED FOR A325 STEEL ASSEMBLIES.
27. SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED.
28. MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION.
29. FABRICATE BEAMS WITH MILL CAMBER UP.
30. LEVEL AND PLUMB INDIVIDUAL MEMBERS OF THE STRUCTURE TO AN ACCURACY OF 1:500, BUT NOT TO EXCEED 1/4" IN THE FULL HEIGHT OF THE COLUMN.
31. COMMENCEMENT OF STRUCTURAL STEEL WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCIES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.
32. INSPECTION AND TESTING OF ALL WELDING AND HIGH STRENGTH BOLTING SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY.
33. FOUR COPIES OF ALL INSPECTION TEST REPORTS SHALL BE SUBMITTED TO THE ENGINEER WITHIN TEN (10) WORKING DAYS OF THE DATE OF INSPECTION.

## STRUCTURAL STEEL

- ALL STRUCTURAL STEEL IS DESIGNED BY ALLOWABLE STRESS DESIGN (ASD)
- A. STRUCTURAL STEEL (W SHAPES)---ASTM A992 (FY = 50 KSI)
  - B. STRUCTURAL STEEL (OTHER SHAPES)---ASTM A36 (FY = 36 KSI)
  - C. STRUCTURAL HSS (RECTANGULAR SHAPES)---ASTM A500 GRADE B, (FY = 46 KSI)
  - D. STRUCTURAL HSS (ROUND SHAPES)---ASTM A500 GRADE B, (FY = 42 KSI)
  - E. PIPE---ASTM A53 (FY = 35 KSI)
  - F. CONNECTION BOLTS---ASTM A325-N
  - G. U-BOLTS---ASTM A36
  - H. ANCHOR RODS---ASTM F 1554
  - I. WELDING ELECTRODE---ASTM E 70XX
2. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE THE FOLLOWING: SECTION PROFILES, SIZES, CONNECTION ATTACHMENTS, REINFORCING, ANCHORAGE, SIZE AND TYPE OF FASTENERS AND ACCESSORIES. INCLUDE ERECTION DRAWINGS, ELEVATIONS AND DETAILS.
  3. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST PROVISIONS OF AISC MANUAL OF STEEL CONSTRUCTION.
  4. PROVIDE ALL PLATES, CLIP ANGLES, CLOSURE PIECES, STRAP ANCHORS, MISCELLANEOUS PIECES AND HOLES REQUIRED TO COMPLETE THE STRUCTURE.
  5. FIT AND SHOP ASSEMBLE FABRICATIONS IN THE LARGEST PRACTICAL SECTIONS FOR DELIVERY TO SITE.
  6. INSTALL FABRICATIONS PLUMB AND LEVEL, ACCURATELY FITTED, AND FREE FROM DISTORTIONS OR DEFECTS.
  7. AFTER ERECTION OF STRUCTURES, TOUCHUP ALL WELDS, ABRASIONS AND NON-GALVANIZED SURFACES WITH A 95% ORGANIC ZINC RICH PAINT IN ACCORDANCE WITH ASTM 780.
  8. ALL STEEL MATERIAL (EXPOSED TO WEATHER) SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT DIPPED GALVANIZED) COATINGS" ON IRONS AND STEEL PRODUCTS.
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  17. LEVEL AND PLUMB INDIVIDUAL MEMBERS OF THE STRUCTURE TO AN ACCURACY OF 1:500, BUT NOT TO EXCEED 1/4" IN THE FULL HEIGHT OF THE COLUMN.
  18. COMMENCEMENT OF STRUCTURAL STEEL WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCIES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.
  19. INSPECTION AND TESTING OF ALL WELDING AND HIGH STRENGTH BOLTING SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY.
  20. FOUR COPIES OF ALL INSPECTION TEST REPORTS SHALL BE SUBMITTED TO THE ENGINEER WITHIN TEN (10) WORKING DAYS OF THE DATE OF INSPECTION.

## PAINT NOTES

**PAINTING SCHEDULE:**

1. ANTENNA PANELS:
  - A. SHERWIN WILLIAMS POLANE-B
  - B. COLOR TO BE MATCHED WITH EXISTING TOWER STRUCTURE.
2. COAXIAL CABLE:
  - A. ONE COAT OF DTM BONDING PRIMER (2-5 MILS. DRY FINISH)
  - B. TWO COATS OF DTM ACRYLIC PRIMER/FINISH (2.5-5 MILS. DRY FINISH)
  - C. COLOR TO BE FIELD MATCHED WITH EXISTING STRUCTURE.

**EXAMINATION AND PREPARATION:**

1. DO NOT APPLY PAINT IN SNOW, RAIN, FOG OR MIST OR WHEN RELATIVE HUMIDITY EXCEEDS 85%. DO NOT APPLY PAINT TO DAMP OR WET SURFACES.
2. VERIFY THAT SUBSTRATE CONDITIONS ARE READY TO RECEIVE WORK. EXAMINE SURFACE SCHEDULED TO BE FINISHED PRIOR TO COMMENCEMENT OF WORK. REPORT ANY CONDITION THAT MAY POTENTIALLY AFFECT PROPER APPLICATION.
3. TEST SHOP APPLIED PRIMER FOR COMPATIBILITY WITH SUBSEQUENT COVER MATERIALS.
4. PERFORM PREPARATION AND CLEANING PROCEDURE IN STRICT ACCORDANCE WITH COATING MANUFACTURER'S INSTRUCTIONS FOR EACH SUBSTRATE CONDITION.
5. CORRECT DEFECTS AND CLEAN SURFACES WHICH AFFECT WORK OF THIS SECTION. REMOVE EXISTING COATINGS THAT EXHIBIT LOOSE SURFACE DEFECTS.
6. IMPERVIOUS SURFACE: REMOVE MILDEW BY SCRUBBING WITH SOLUTION OF TRI-SODIUM PHOSPHATE AND BLEACH. RINSE WITH CLEAN WATER AND ALLOW SURFACE TO DRY.
7. ALUMINUM SURFACE SCHEDULED FOR PAINT FINISH: REMOVE SURFACE CONTAMINATION BY STEAM OR HIGH-PRESSURE WATER. REMOVE OXIDATION WITH ACID ETCH AND SOLVENT WASHING. APPLY ETCHING PRIMER IMMEDIATELY FOLLOWING CLEANING.
8. FERROUS METALS: CLEAN UNGALVANIZED FERROUS METAL SURFACES THAT HAVE NOT BEEN SHOP COATED; REMOVE OIL, GREASE, DIRT, LOOSE MILL SCALE, AND OTHER FOREIGN SUBSTANCES. USE SOLVENT OR MECHANICAL CLEANING METHODS THAT COMPLY WITH THE STEEL STRUCTURES PAINTING COUNCIL'S (SSPC) RECOMMENDATIONS. TOUCH UP BARE AREAS AND SHOP APPLIED PRIMER COATS THAT HAVE BEEN DAMAGED. WIRE BRUSH, CLEAN WITH SOLVENTS RECOMMENDED BY PAINT MANUFACTURER, AND TOUCH UP WITH THE SAME PRIMER AS THE SHOP COAT.
9. GALVANIZED SURFACES: CLEAN GALVANIZED SURFACES WITH NON-PETROLEUM-BASED SOLVENTS SO SURFACE IS FREE OF OIL AND SURFACE CONTAMINANTS. REMOVE PRETREATMENT FROM GALVANIZED SHEET METAL FABRICATED FROM COIL STOCK BY MECHANICAL METHODS.
10. ANTENNA PANELS: REMOVE ALL OIL, DUST, GREASE, DIRT, AND OTHER FOREIGN MATERIAL TO ENSURE ADEQUATE ADHESION. PANELS MUST BE WIPED WITH METHYL ETHER KETONE (MEK).
11. COAXIAL CABLES: REMOVE ALL OIL, DUST, GREASE, DIRT, AND OTHER FOREIGN MATERIAL TO ENSURE ADEQUATE ADHESION.

**CLEANING:**

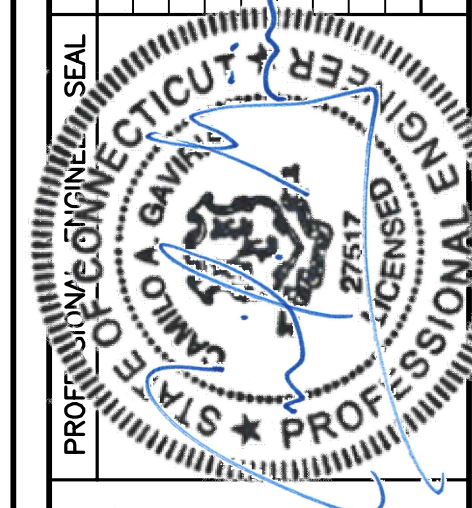
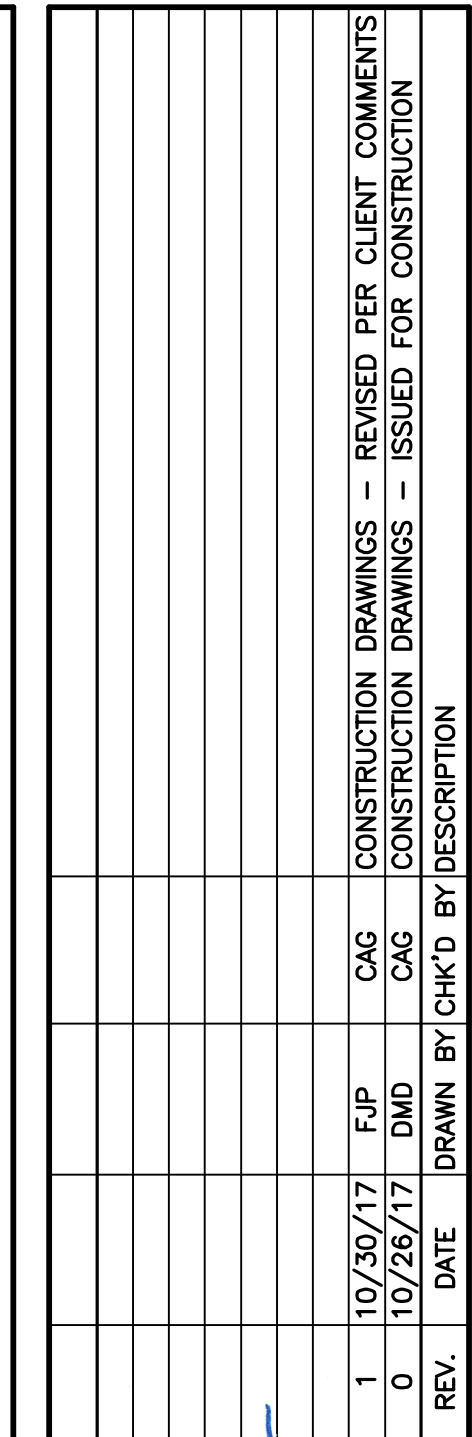
1. COLLECT WASTE MATERIAL, WHICH MAY CONSTITUTE A FIRE HAZARD, PLACE IN CLOSED METAL CONTAINERS AND REMOVE DAILY FROM SITE.

**APPLICATION:**

1. APPLY PRODUCTS IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
2. DO NOT APPLY FINISHES TO SURFACES THAT ARE NOT DRY.
3. APPLY EACH COAT TO UNIFORM FINISH.
4. APPLY EACH COAT OF PAINT SLIGHTLY DARKER THAN PRECEDING COAT UNLESS OTHERWISE APPROVED.
5. SAND METAL LIGHTLY BETWEEN COATS TO ACHIEVE REQUIRED FINISH.
6. VACUUM CLEAN SURFACES FREE OF LOOSE PARTICLES. USE TACK CLOTH JUST PRIOR TO APPLYING NEXT COAT.
7. ALLOW APPLIED COAT TO DRY BEFORE NEXT COAT IS APPLIED.

**COMPLETED WORK:**

1. SAMPLES: PREPARE 24" X 24" SAMPLE AREA FOR REVIEW.
2. MATCH APPROVED SAMPLES FOR COLOR, TEXTURE AND COVERAGE. REMOVE REFINISH OR REPAINT WORK NOT IN COMPLIANCE WITH SPECIFIED REQUIREMENTS.



AT&T MOBILITY  
WIRELESS COMMUNICATIONS FACILITY  
**STORRS-UCONN**  
SITE NUMBER: CT1107  
60 NORTH EAGLEVILLE ROAD  
MANSFIELD, CT 06268

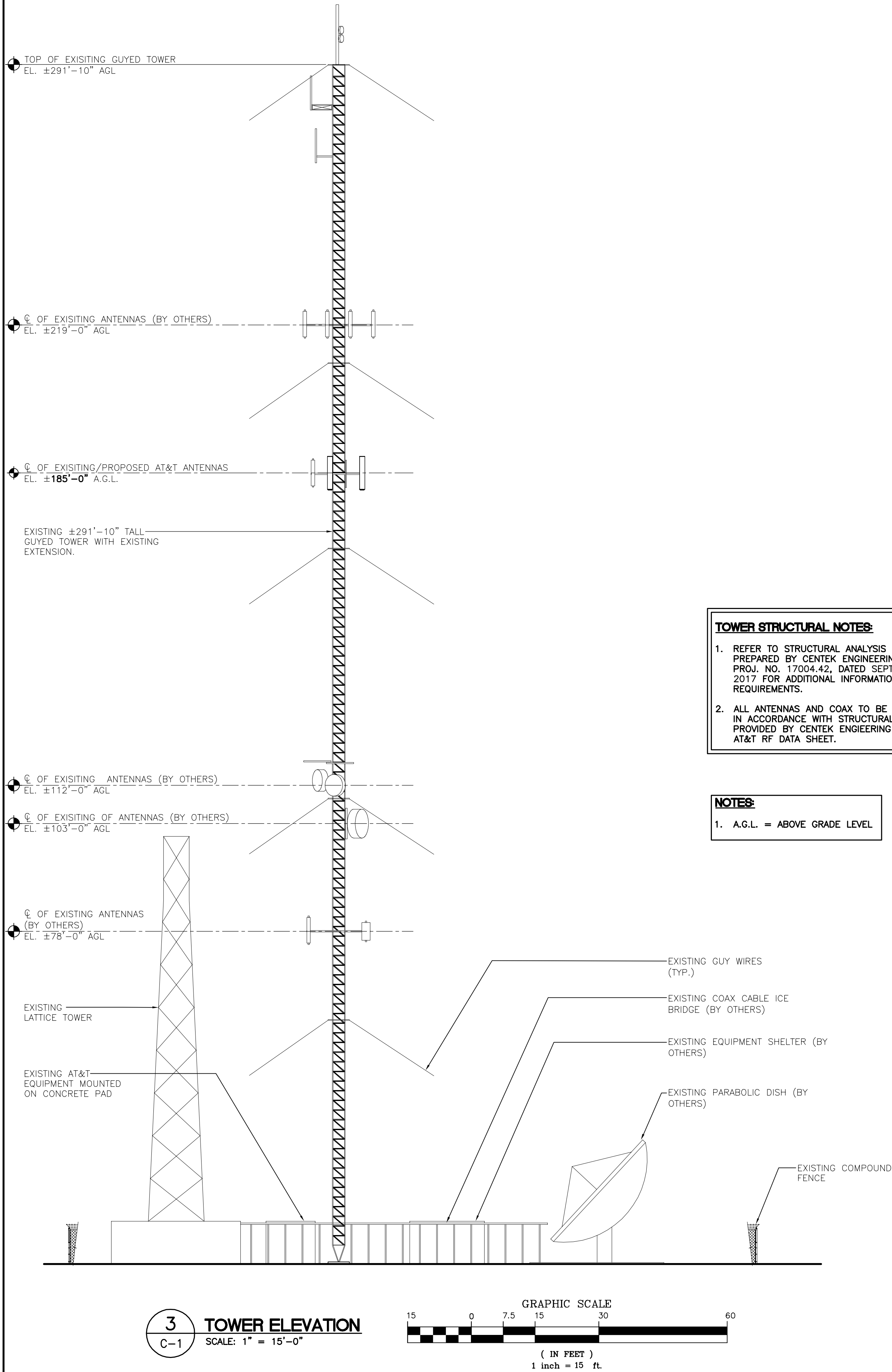
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JOB NO.	17004.42

NOTES,  
SPECIFICATIONS  
AND DETAILS

**N-1**

Sheet No. 2 of 7



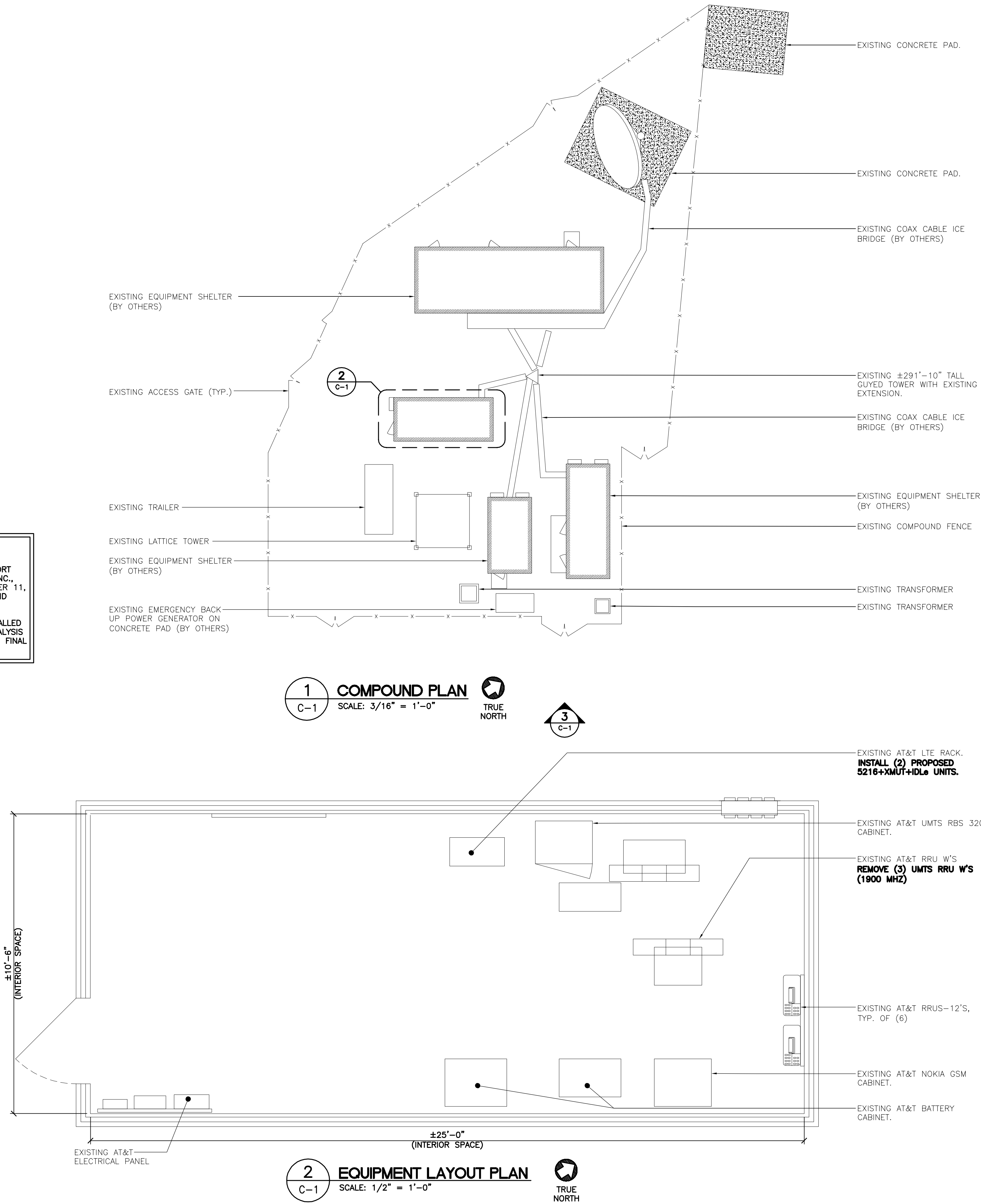


**TOWER STRUCTURAL NOTES:**

1. REFER TO STRUCTURAL ANALYSIS REPORT PREPARED BY CENTEK ENGINEERING, INC., PROJ. NO. 17004.42, DATED SEPTEMBER 11, 2017 FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
2. ALL ANTENNAS AND COAX TO BE INSTALLED IN ACCORDANCE WITH STRUCTURAL ANALYSIS PROVIDED BY CENTEK ENGINEERING AND FINAL AT&T RF DATA SHEET.

**NOTES:**

1. A.G.L. = ABOVE GRADE LEVEL



REV.	DATE	DRAWN BY	CHECKED BY	DESCRIPTION
1	10/30/17	FJP	DMD	
0	10/26/17			

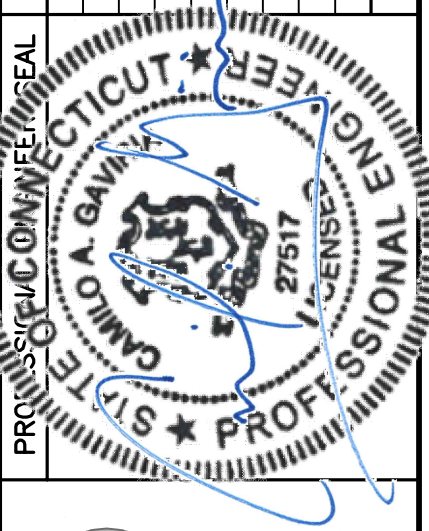
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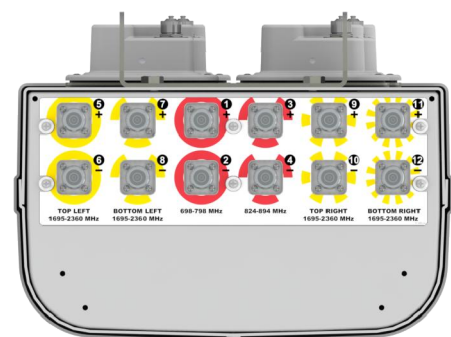
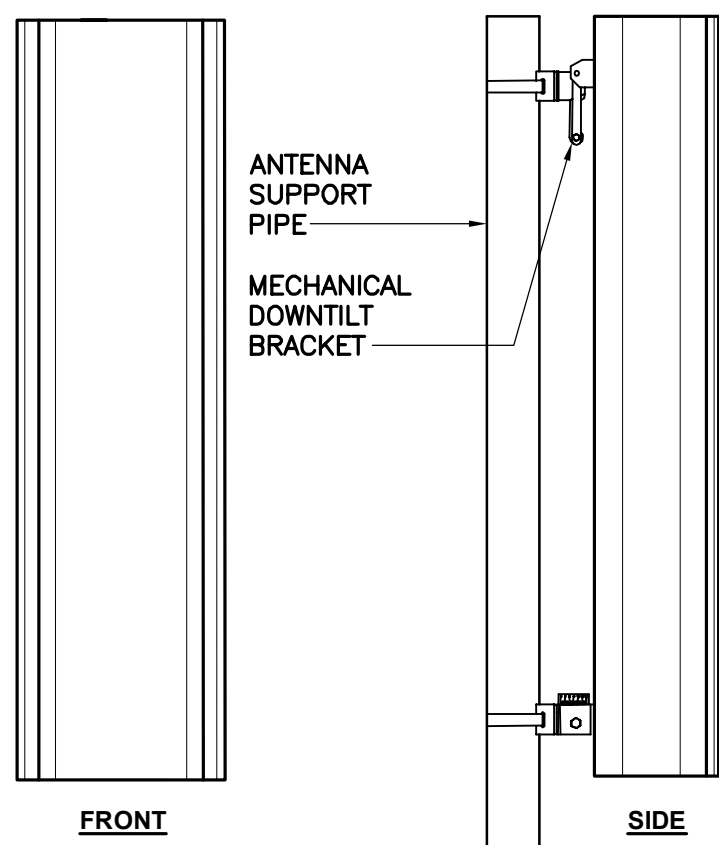
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PLANS AND ELEVATION

**C-1**  
Sheet No. 3 of 7







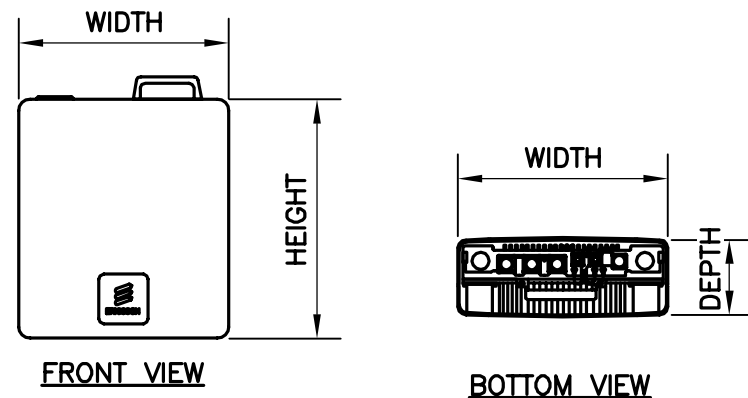
CCI-H8 BOTTOM



QUINTEL BOTTOM

ALPHA/BETA/GAMMA ANTENNA		
EQUIPMENT	DIMENSIONS	WEIGHT
MAKE: CCI MODEL: TPA-65R-LCUUUU-H8	92.4"L x 14.8"W x 7.4"D	68.0 LBS.
MAKE: QUINTEL MODEL: QS66512-2	72"L x 12"W x 9.6"D	111.0 LBS.

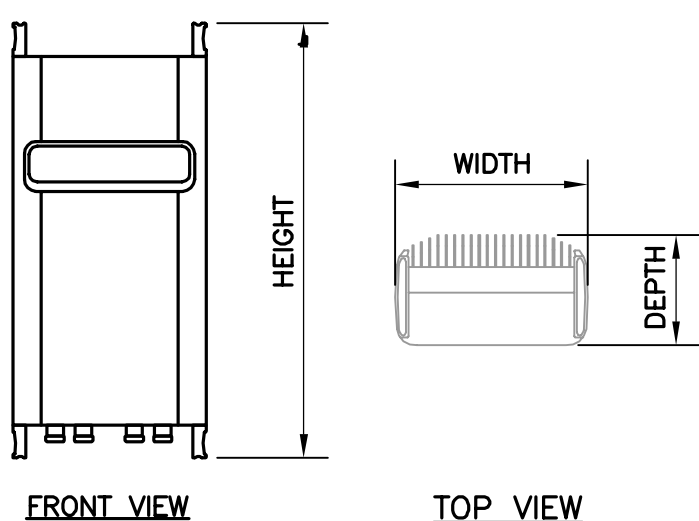
5 PROPOSED ANTENNA DETAIL  
SCALE: 1/2" = 1'-0"



B14 4478

RRU (REMOTE RADIO UNIT)			
EQUIPMENT	DIMENSIONS	WEIGHT	CLEARANCES
MAKE: ERICSSON MODEL: B14 4478	14.9"L x 13.1"W x 7.3"D	60 LBS.	ABOVE: 16" MIN. BELOW: 12" MIN. FRONT: 36" MIN.
NOTES: 1. CONTRACTOR TO COORDINATE FINAL EQUIPMENT MODEL SELECTION WITH AT&T CONSTRUCTION MANAGER PRIOR TO ORDERING.			

6 ERICSSON B14 4478 DETAIL  
SCALE: 1" = 1'-0"



RRUS-32\_B2/B66

RRU (REMOTE RADIO UNIT)			
EQUIPMENT	DIMENSIONS	WEIGHT	CLEARANCES
MAKE: ERICSSON MODEL: RRUS 32 B66	27.17"H x 12.05"W x 7.01"D	52.91 LBS.	ABOVE: 16" MIN. BELOW: 12" MIN. FRONT: 36" MIN.
MAKE: ERICSSON MODEL: RRUS-32 B2	27.17"H x 12.05"W x 7.01"D	52.91 LBS.	ABOVE: 16" MIN. BELOW: 12" MIN. FRONT: 36" MIN.
NOTES: 1. CONTRACTOR TO COORDINATE FINAL EQUIPMENT MODEL SELECTION WITH AT&T CONSTRUCTION MANAGER PRIOR TO ORDERING.			

7 ERICSSON REMOTE RADIO UNITS  
SCALE: 1" = 1'-0"

RELOCATED AT&T SURGE ARRESTOR TO GAMMA SECTOR, TYP. OF (1).

PROPOSED AT&T RRH B14 4478 B2'S, TYP. OF (1) PER SECTOR/(3) TOTAL

EXISTING AT&T RRUS-32'S, TYP. OF (1) PER SECTOR/(3) TOTAL.

PROPOSED AT&T SURGE ARRESTOR, TYP. OF (1)(P/N: RAYCAP DC6-48-60-18-8F)

PROPOSED AT&T PANEL ANTENNA POS.3  
• ALPHA/GAMMA(P/N: CCI TPA-65R-LCUUUU-H8)  
• BETA (P/N: QUINTEL QS66512-2)

PROPOSED AT&T RRUS-32, TYP. OF (2) PER SECTOR/(6) TOTAL, ON ADDED MOUNT.

2 PROPOSED ANTENNA PLAN  
SCALE: 3/8" = 1'-0"



PROPOSED AT&T RRUS-32, TYP. OF (2) PER SECTOR/(6) TOTAL, ON ADDED MOUNT.

EXISTING AT&T PANEL ANTENNA POS.4  
• ALPHA/GAMMA(P/N: CCI HPA-65R-BUU-H8)  
• BETA (P/N: CCI HPA-65R-BUU-H6)

PROPOSED AT&T SURGE ARRESTOR, TYP. OF (1)(P/N: RAYCAP DC6-48-60-18-8F)

EXISTING AT&T RRUS-11'S, TYP. OF (1) PER SECTOR/(3) TOTAL.

PROPOSED AT&T RRH B14 4478 B2'S, TYP. OF (1) PER SECTOR/(3).

PROPOSED AT&T PANEL ANTENNA POS.3  
• ALPHA/GAMMA(P/N: CCI TPA-65R-LCUUUU-H8)  
• BETA (P/N: QUINTEL QS66512-2)

4 PROPOSED ANTENNA ELEVATION  
SCALE: 1/2" = 1'-0"

EXISTING AT&T PANEL ANTENNA POS. 1, (P/N: POWERWAVE 7770), TYP. OF (1) PER SECTOR/(3) TOTAL.

EXISTING AT&T PANEL ANTENNA POS.2  
• ALPHA/GAMMA(P/N: CCI OPA-65R-LCUU-H8)  
• BETA (P/N: CCI OPA-65R-LCUU-H6)

EXISTING ±291'-10" TALL GUYED TOWER WITH EXISTING EXTENSION.

EXISTING AT&T PANEL ANTENNA POS.4  
• ALPHA/GAMMA(P/N: CCI HPA-65R-BUU-H8)  
• BETA (P/N: CCI HPA-65R-BUU-H6)

EXISTING AT&T RRUS-11'S, TYP. OF (1) PER SECTOR/(3) TOTAL.

EXISTING AT&T SURGE ARRESTOR, TYP. OF (1).

EXISTING AT&T TWIN TMA (1)/(3) TOTAL.

EXISTING AT&T TRIPLEXER, TYP. OF (2)/(6) TOTAL.

EXISTING AT&T RRUS-32'S, TYP. OF (1) PER SECTOR/(3) TOTAL.

EXISTING AT&T RRUS-11'S, TYP. OF (1) PER SECTOR/(3) TOTAL.

1 EXISTING ANTENNA PLAN  
SCALE: 3/8" = 1'-0"



EXISTING AT&T PANEL ANTENNA POS.4  
• ALPHA/GAMMA(P/N: CCI HPA-65R-BUU-H8)  
• BETA (P/N: CCI HPA-65R-BUU-H6)

EXISTING AT&T PANEL ANTENNA POS.2

• ALPHA/GAMMA(P/N: CCI OPA-65R-LCUU-H8)  
• BETA (P/N: CCI OPA-65R-LCUU-H6)

EXISTING AT&T PANEL ANTENNA POS. 1, (P/N: POWERWAVE 7770), TYP. OF (1) PER SECTOR/(3) TOTAL.

EXISTING AT&T TWIN TMA (1)/(3) TOTAL.

CL AT&T ANTENNAS EL. ±185' A.G.L.

EXISTING AT&T RRUS-32'S, TYP. OF (1) PER SECTOR/(3) TOTAL.

EXISTING AT&T TRIPLEXER, TYP. OF (2)/(6) TOTAL.

EXISTING ±291'-10" TALL GUYED TOWER WITH EXISTING EXTENSION.

EXISTING AT&T SURGE ARRESTOR, TYP. OF (1).

3 EXISTING ANTENNA ELEVATION  
SCALE: 1/2" = 1'-0"

EXISTING AT&T PANEL ANTENNA POS. 1, (P/N: POWERWAVE 7770), TYP. OF (1) PER SECTOR/(3) TOTAL.

EXISTING AT&T PANEL ANTENNA POS.2  
• ALPHA/GAMMA(P/N: CCI OPA-65R-LCUU-H8)  
• BETA (P/N: CCI OPA-65R-LCUU-H6)

EXISTING ±291'-10" TALL GUYED TOWER WITH EXISTING EXTENSION.

EXISTING AT&T PANEL ANTENNA POS.4  
• ALPHA/GAMMA(P/N: CCI HPA-65R-BUU-H8)  
• BETA (P/N: CCI HPA-65R-BUU-H6)

EXISTING AT&T SURGE ARRESTOR, TYP. OF (2), (1) TO BE RELOCATED TO GAMMA SECTOR.

EXISTING AT&T TWIN TMA (1)/(3) TOTAL.

EXISTING AT&T TRIPLEXER, TYP. OF (2)/(6) TOTAL.

EXISTING AT&T PANEL ANTENNA POS.2  
• ALPHA/GAMMA(P/N: CCI OPA-65R-LCUU-H8)  
• BETA (P/N: CCI OPA-65R-LCUU-H6)

EXISTING AT&T PANEL ANTENNA POS. 1, (P/N: POWERWAVE 7770), TYP. OF (1) PER SECTOR/(3) TOTAL.

EXISTING AT&T TWIN TMA (1)/(3) TOTAL.

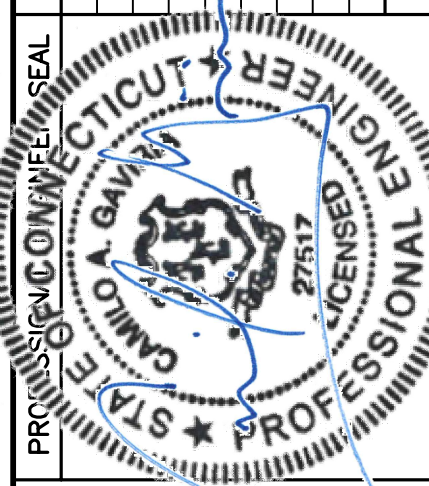
CL AT&T ANTENNAS EL. ±185' A.G.L.

EXISTING AT&T RRUS-32'S, TYP. OF (1) PER SECTOR/(3) TOTAL.

EXISTING AT&T TRIPLEXER, TYP. OF (2)/(6) TOTAL.

EXISTING AT&T SURGE ARRESTOR, TYP. OF (2), (1) TO BE RELOCATED TO GAMMA SECTOR.

CONSTRUCTION DRAWINGS	REVISED PER CLIENT COMMENTS
CAG	CAG
10/30/17	FJP
10/26/17	DMD
DATE	DRAWN BY CHK'D BY DESCRIPTION



CENTEX engineering  
Centered on Solutions™  
(203) 488-0360  
(203) 488-8387 Fax  
63-2 North Branford Road  
Branford, CT 06405  
www.CentexEng.com

AT&T MOBILITY  
WIRELESS COMMUNICATIONS FACILITY  
STORRS-UCONN  
SITE NUMBER: CT1107  
60 NORTH EAGLEVILLE ROAD  
MANSFIELD, CT 06268

DATE: 08/31/17

SCALE: AS NOTED

JOB NO. 17004.42

LTE 6C/7C  
EQUIPMENT  
DETAILS

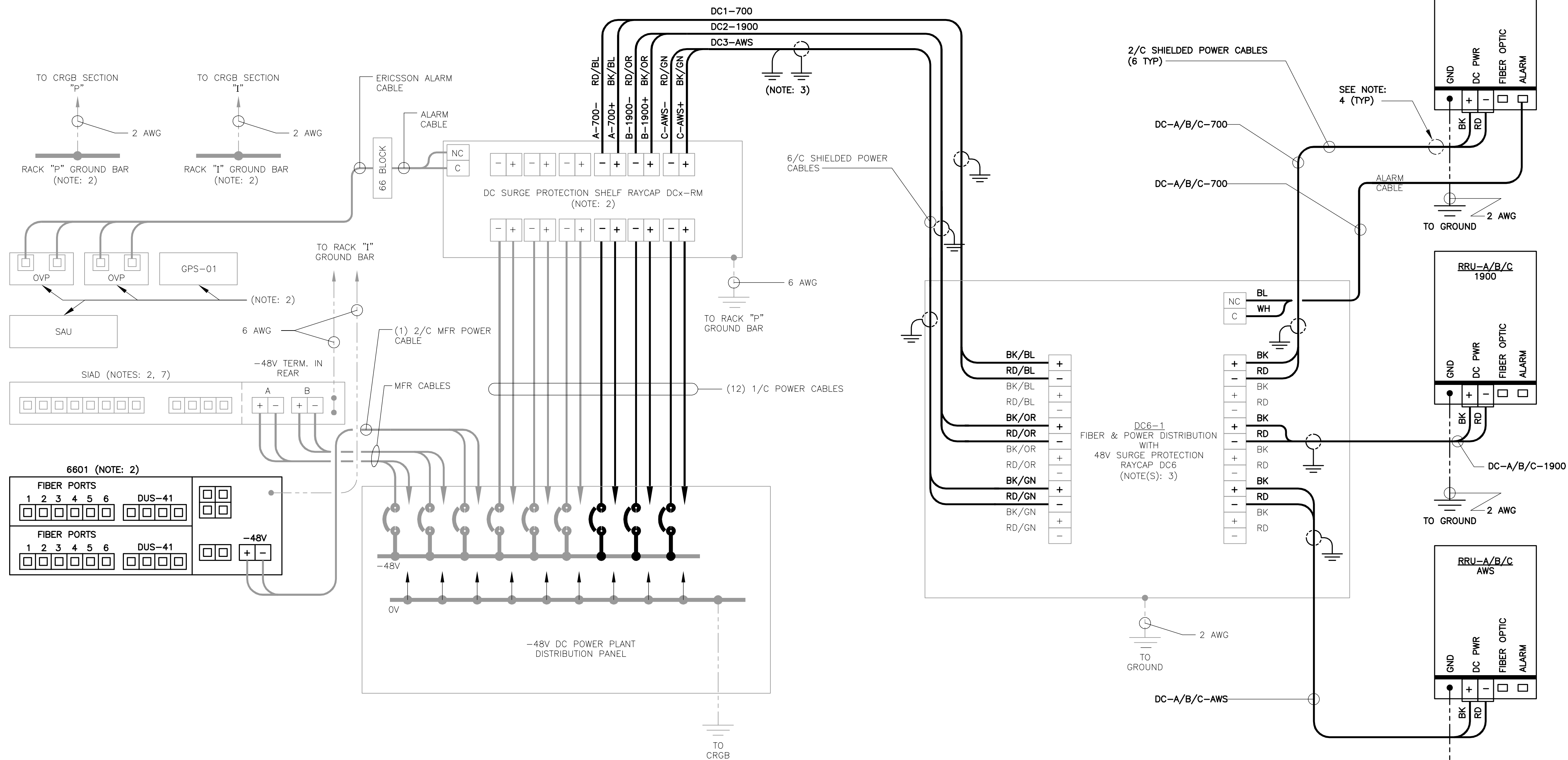
C-2

Sheet No. 4 of 7





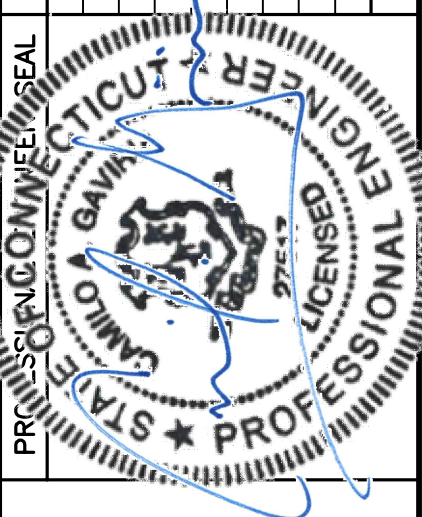




1 LTE WIRING DIAGRAM  
E-2 NOT TO SCALE

LTE WIRING DIAGRAM NOTES:

1. LABEL THE DC POWER CABLES AT BOTH ENDS OF EVERY WIRE AND IN ANY PULL BOX IF USED. LABEL SHALL BE DURABLE, SELF ADHESIVE, WRAPPED LONGITUDINALLY ALONG THE CABLE AND STATE THE SECTOR, FREQUENCY BAND AND POLARITY; I.E. "A-1900+". CABLE AND WIRE LABELS SHOWN ARE REPRESENTATIVE AND MAY BE MODIFIED AS DIRECTED BY AT&T.
2. INSTALL ON BASEBAND EQUIPMENT RACK.
3. THE BARE GROUND WIRE OF EACH MULTI-CONDUCTOR CABLE SHALL BE CONNECTED TO THE "P" GROUND BAR ON THE RACK. WHEN A SHIELDED CABLE IS USED, THE DRAIN WIRE ALSO SHALL BE CONNECTED TO THE "P" GROUND BAR.
4. CABLE GROUND WIRE AND SHIELD DRAIN WIRE TO BE LEFT UN-TERMINATED AT RRU AND DC POWER PLANT.
5. SEE LTE SCHEMATIC DIAGRAM DETAIL 1/E-1 FOR BREAKER RATING.



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Branford, CT 06405  
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AT&T MOBILITY  
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**STORRS-UCONN**  
SITE NUMBER: CT1107  
60 NORTH EAGLEVILLE ROAD  
MANSFIELD, CT 06268

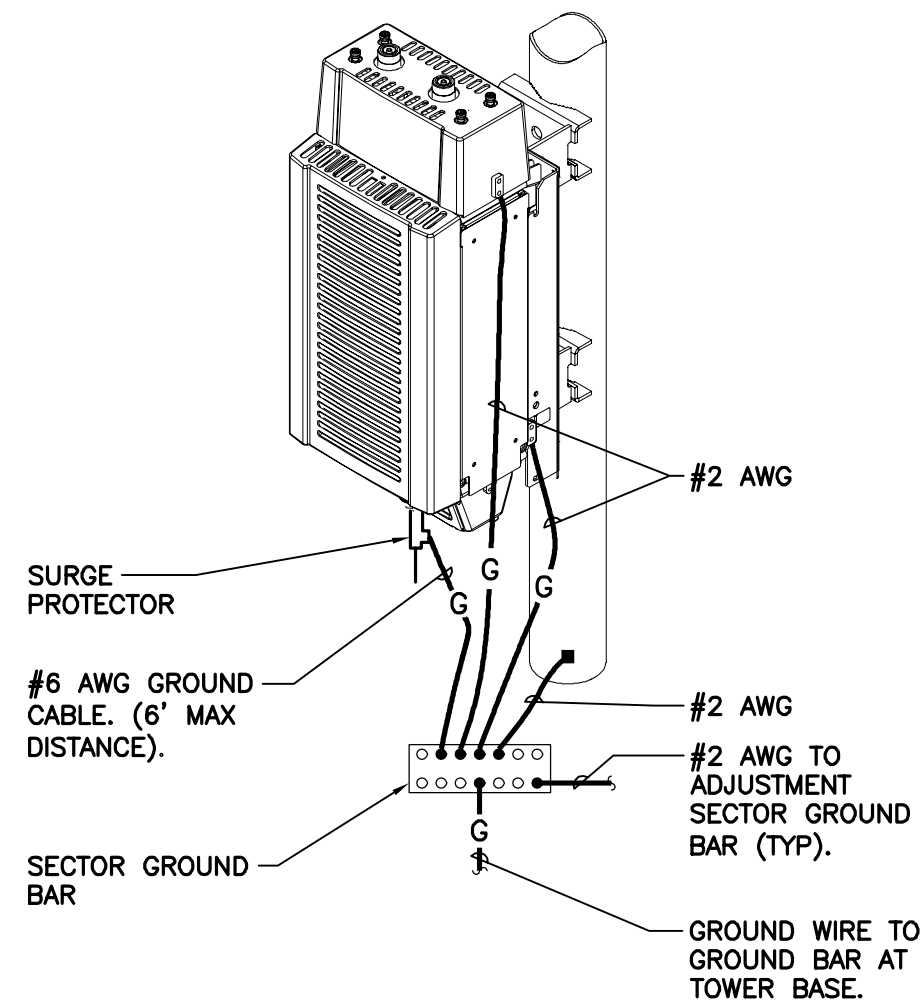
DATE: 08/31/17  
SCALE: AS NOTED  
JOB NO. 17004.42

LTE WIRING  
DIAGRAM

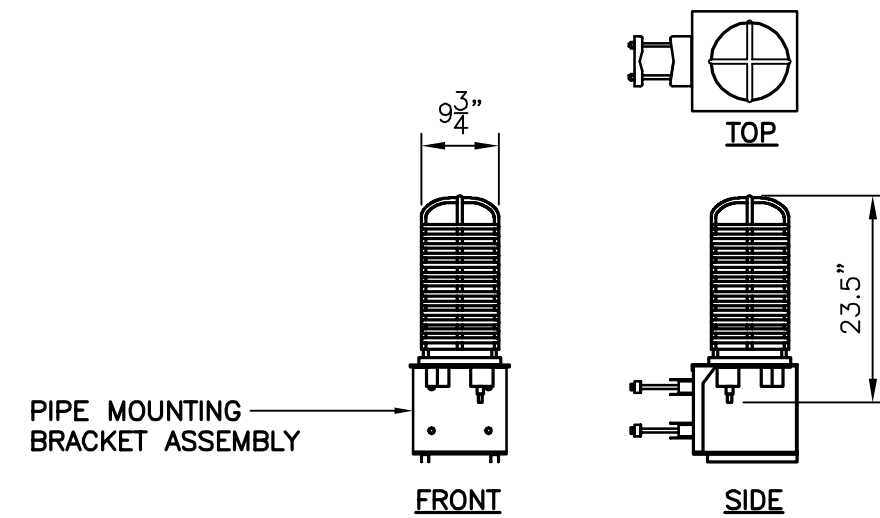
E-2  
Sheet No. 6 of 7



EACH RRH CABINET SHALL BE GROUNDED IN THE FOLLOWING MANNER:  
1. AT TOP OF THE CABINET  
2. AT RIGHT SIDE OF THE CABINET.



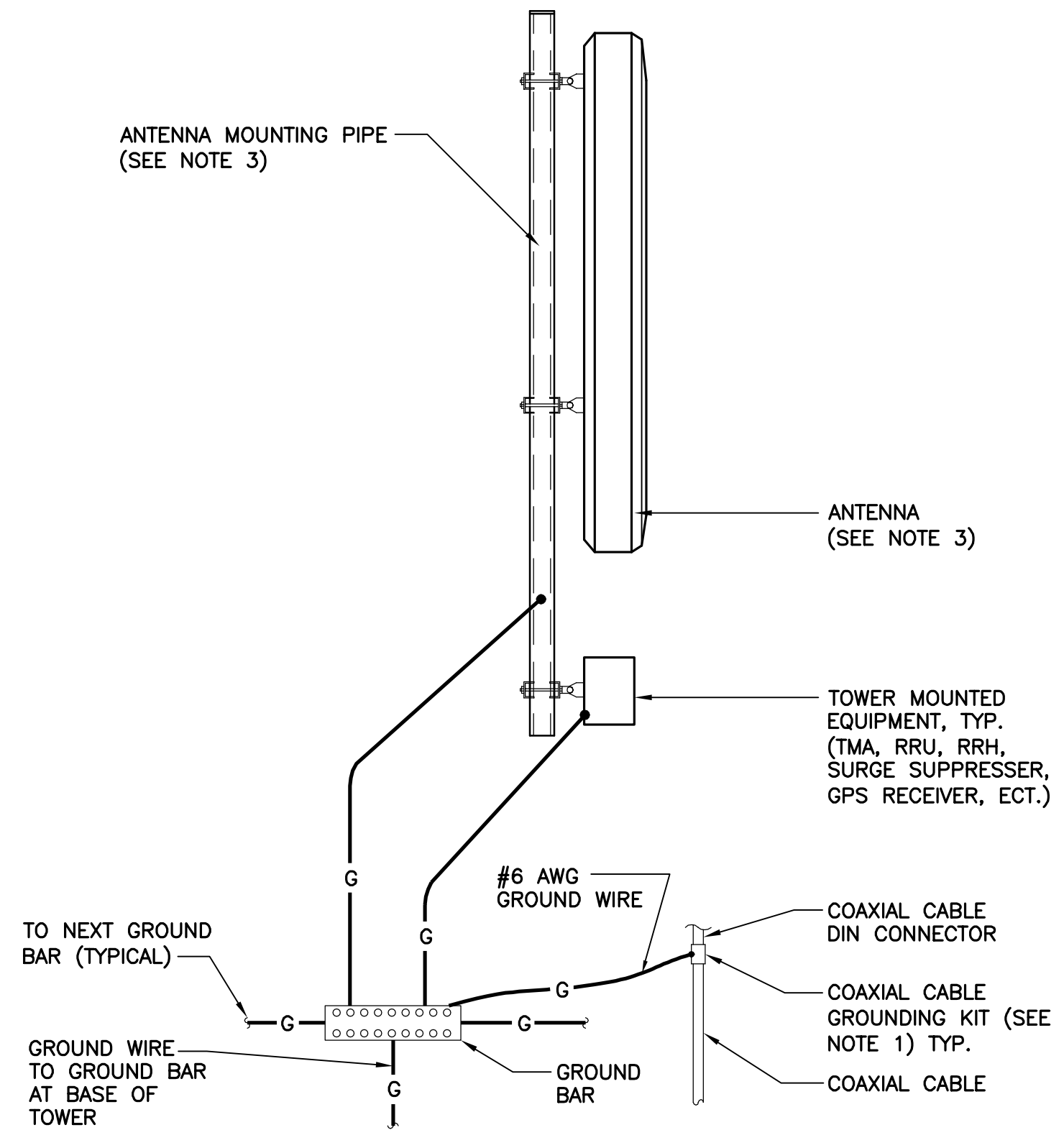
**3 RRU POLE MOUNT GROUNTING**  
E-3 NOT TO SCALE



SITE TYPE	ARRESTOR MAKE/MODEL	QTY REQUIRED	ARRESTOR LOCATION	WEIGHT
	MAKE: RAYCAP (SQUID) MODEL: DC6-48-60-18-BF	(1) PER SITE	TOWER, ADJACENT TO AT&T ANTENNAS AND RRUs.	20 LBS. (WITHOUT MOUNT)

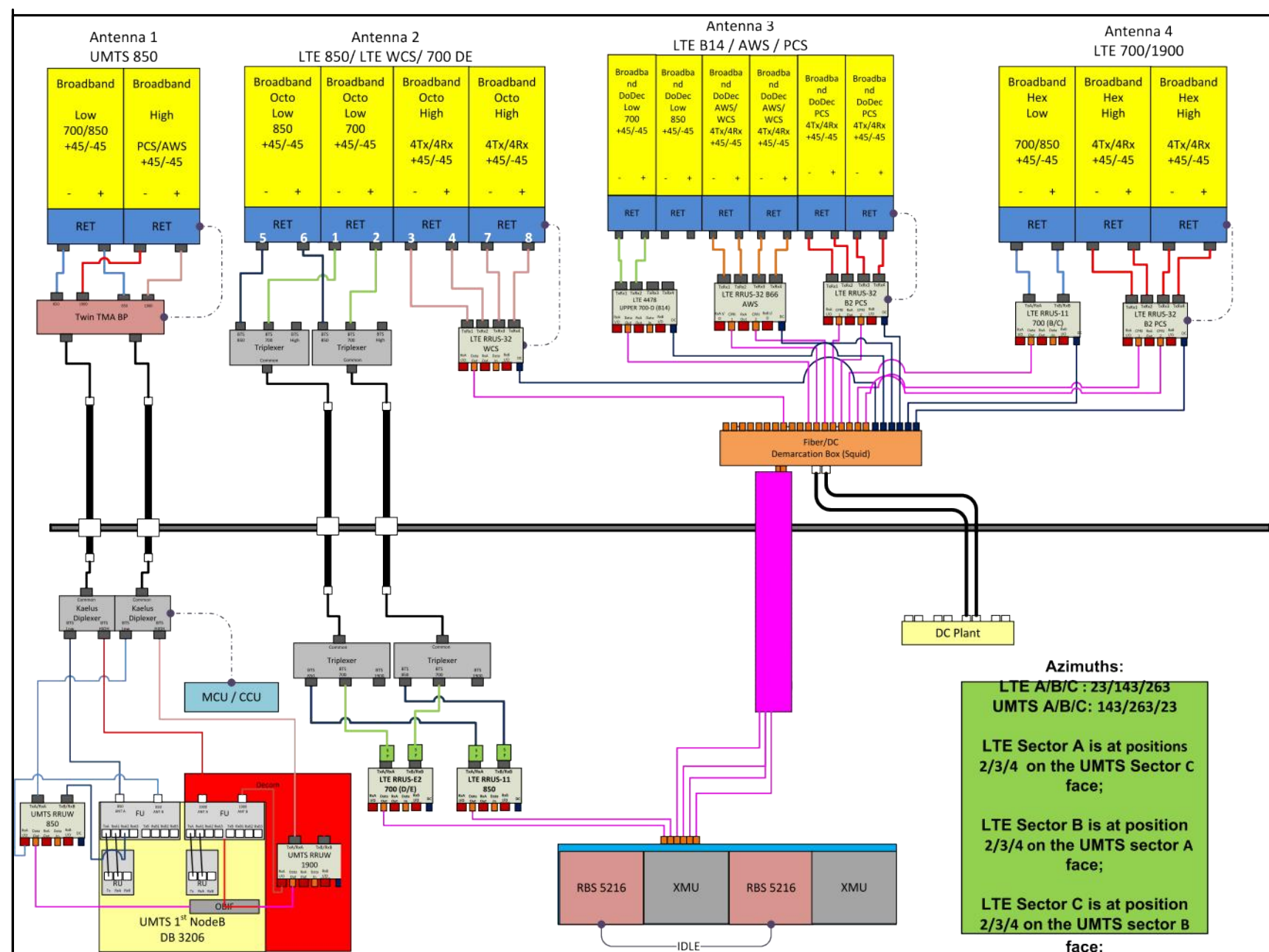
**NOTES:**  
1. CONTRACTOR TO COORDINATE FINAL SURGE ARRESTOR MODEL SELECTION(S) WITH AT&T CONSTRUCTION MANAGER PRIOR TO ORDERING.  
2. CONTRACTOR TO INSTALL ARRESTOR IN CONFORMANCE WITH MANUFACTURERS RECOMMENDATIONS.

**2 SURGE ARRESTOR DETAIL**  
E-3 SCALE: NTS

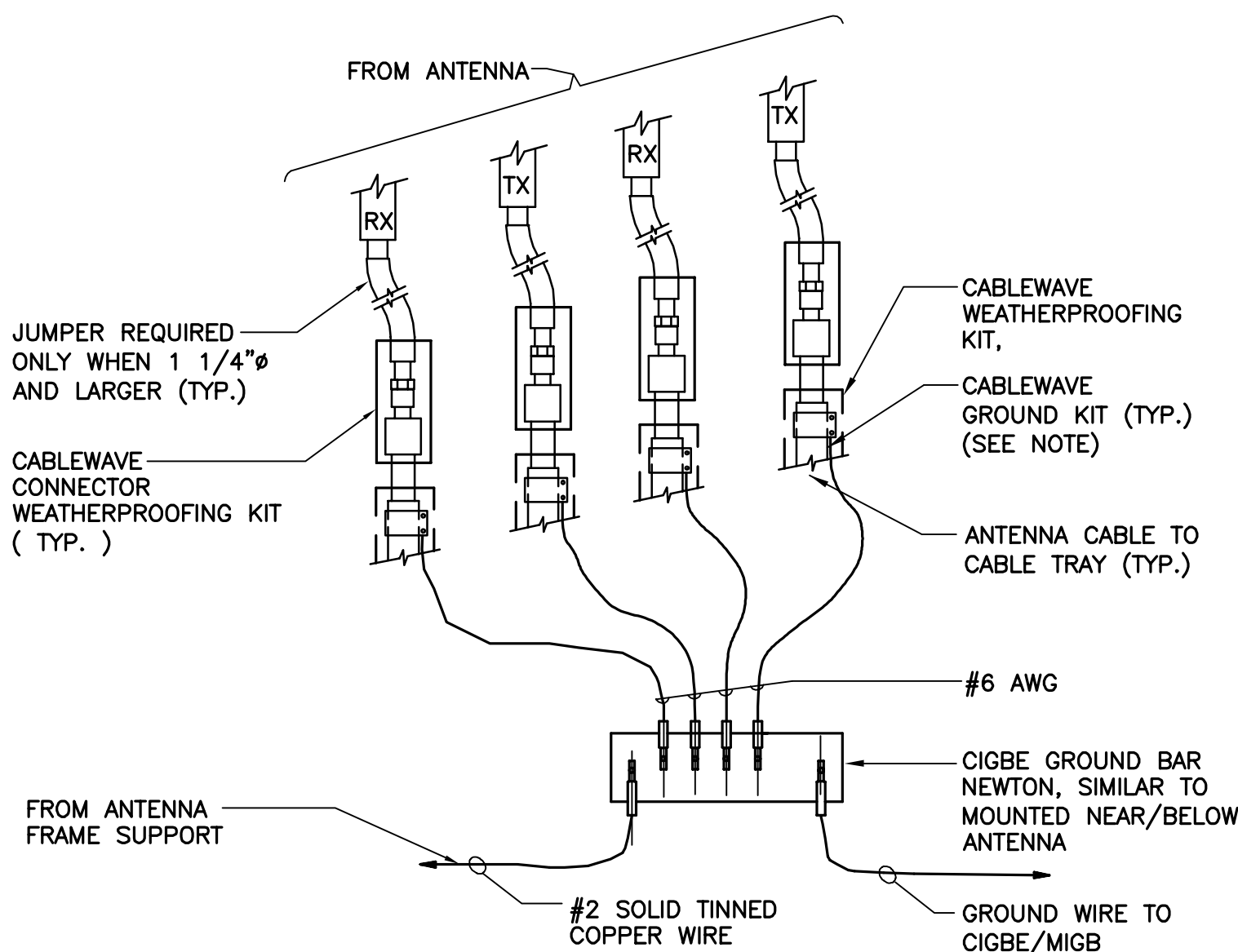


- NOTES:**
- BOND COAXIAL CABLE GROUND KITS TO EACH OWNER'S GROUND BAR ALONG ENTIRE COAX RUN FROM ANTENNA TO SHELTER.
  - BOND ALL EQUIPMENT TO GROUND PER NEC AND MANUFACTURERS SPECIFICATIONS.
  - DETAIL IS TYPICAL FOR ALL ANTENNA SECTORS, INCLUDING GPS ANTENNA.

**1 TYPICAL ANTENNA GROUNTING DETAIL**  
E-3 NOT TO SCALE

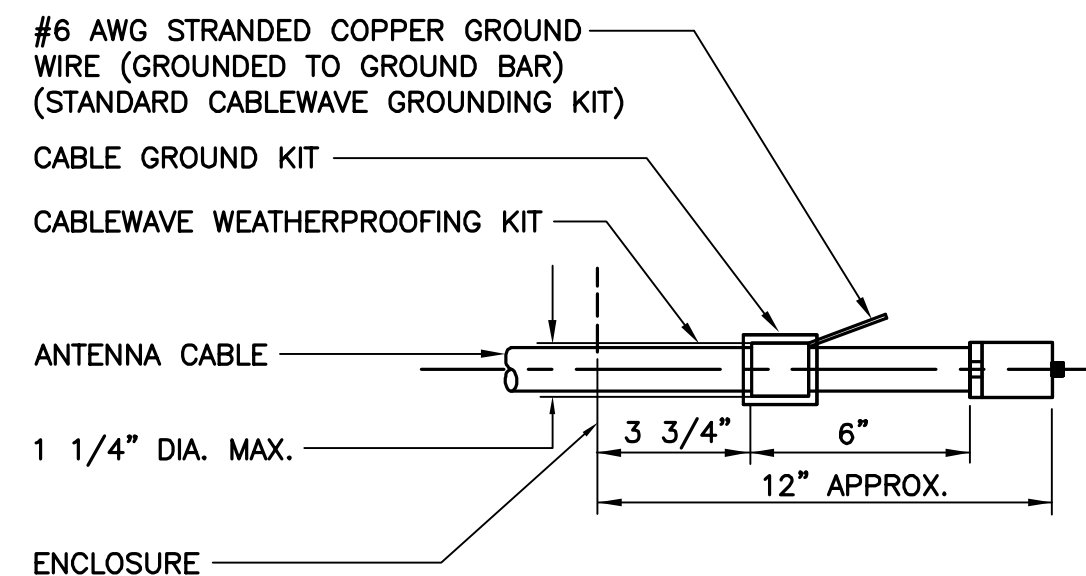


**6 RF PLUMBING DIAGRAM**  
E-3 NOT TO SCALE



- NOTE:**
- DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO CIGBE

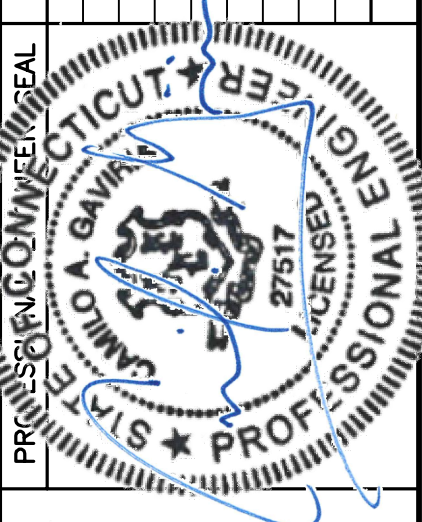
**5 CONNECTION OF GROUND WIRES TO GROUND BAR**  
E-3 NOT TO SCALE



- NOTE:**
- DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.

**4 ANTENNA CABLE GROUNTING DETAIL**  
E-3 NOT TO SCALE

REV.	DATE	DRAWN BY	CHECKED BY	DESCRIPTION
1	10/30/17	FJP	CAG	CONSTRUCTION DRAWINGS - REVISED PER CLIENT COMMENTS
0	10/26/17	DMD	CAG	CONSTRUCTION DRAWINGS - ISSUED FOR CONSTRUCTION



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SITE NUMBER: CT1107  
60 NORTH EAGLEVILLE ROAD  
MANSFIELD, CT 06268

DATE: 08/31/17  
SCALE: AS NOTED  
JOB NO. 17004.42

TYPICAL  
ELECTRICAL  
DETAILS

**E-3**

Sheet No. 7 of 7



November 1, 2017

Linda Painter, Director of Planning & Development  
Audrey P. Beck Municipal Building  
4 South Eagleville Road, Mansfield, CT 06268

RE: AT&T Wireless Modifications to Telecommunication Facility –  
60 North Eagleville Road, Mansfield, CT 06268

Dear Ms. Painter:

In order to accommodate technological changes, implement the Uniform Mobile Telecommunications System and enhance system performance in the State of Connecticut, AT&T Wireless ("AT&T") will be changing its equipment configuration at the above referenced telecommunications facility. AT&T Wireless currently maintains nine (9) antennas at the 185-foot level of an existing 291-foot guyed tower located at 60 North Eagleville Road, in Mansfield, CT. The tower is owned by the University of Connecticut. The property is owned by University of Connecticut.

AT&T Wireless now seeks to install three (3) additional antennas at the 291-foot level of the tower, to be mounted on the existing tower, as well as install six (6) new RRUS-32 Remote Radio Units ("RRU"), and install 3 new RRUS-B14 RRUs on the tower, behind the antennas. In addition, a single new surge suppressor will be installed behind the Gamma sector antennas.

This letter is intended to serve as the required notice to the municipality's Planning and Zoning Department. As required by the Regulations of Connecticut State Agencies ("RCSA") section 16-50j-73, the Connecticut Siting Council ("CSC") has been notified of the proposed changes and will review AT&T's proposal. Please accept this letter as notification under RCSA section 16-50j-73 of construction which constitutes an exempt modification pursuant to RCSA section 16-50j-72(b)(2).



The enclosed letter and documents to the CSC fully describes AT&T's proposal for the above referenced site. However, if you have any questions or require any additional information concerning our plans or the CSC procedures, please contact me at 443-286-4006 or contact Melanie Bachman, Acting Executive Director of the CSC at 860-872-2935.

Respectfully submitted,



Jack Andrews  
Zoning Manager, Empire Telecom  
o/b/o AT&T Wireless  
10130 Donleigh Drive  
Columbia, MD 21046  
443-677-0144  
[jandrews@empiretelecomm.com](mailto:jandrews@empiretelecomm.com)

Enclosures

cc: Melanie Bachman, Connecticut Siting Council

# USPS Tracking® Results

FAC

Track Another Package

Tracking Number: 70161370000084028979

7016 1370 0000 8402 8979

U.S. Postal Service™ CERTIFIED MAIL® RECEIPT Domestic Mail Only	
For delivery information, visit our website at <a href="http://www.usps.com">www.usps.com</a> ®.	
STORRS MANSFIELD, CT 06268	
Certified Mail Fee	\$3.35
Extra Services & Fees (check box, add fee as appropriate)	\$0.00
<input type="checkbox"/> Return Receipt (hardcopy)	\$0.00
<input type="checkbox"/> Return Receipt (electronic)	\$0.00
<input type="checkbox"/> Certified Mail Restricted Delivery	\$0.00
<input type="checkbox"/> Adult Signature Required	\$0.00
<input type="checkbox"/> Adult Signature Restricted Delivery	\$0.00
Postage	\$2.24
Total Postage and Fees	\$5.59
Sent To	Linda Poirier
Street and Apt. No., or PO Box No.	4 S Eagleville Rd
City, State, ZIP+4®	Mansfield CT 06268
PS Form 3800, April 2015 PSN 7530-02-000-9047 See Reverse for Instructions	

0572  
11Postmark  
Here

11/01/2017

Delivered

## Product & Tracking Information

See Available Actions

**Postal Product:**  
First-Class Mail®

**Features:**  
Certified Mail™

DATE &amp; TIME

STATUS OF ITEM

LOCATION

November 6, 2017, 11:33  
am

Delivered, Front  
Desk/Reception

STORRS  
MANSFIELD, CT 06268

Your item was delivered to the front desk or reception area at 11:33 am on November 6, 2017 in STORRS MANSFIELD, CT 06268.

November 6, 2017, 9:43 am

In Transit to Destination

ON ITS WAY TO STORRS  
MANSFIELD, CT 06268

November 5, 2017, 10:43  
pm

Departed USPS Regional  
Destination Facility

SPRINGFIELD MA  
NETWORK DISTRIBUTION  
CENTER

## USPS Tracking® Results

FAQs

Track Another Pac

Tracking Number: 70161370000084028986

7016 1370 0000 8402 8986

**U.S. Postal Service™**  
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 Domestic Mail Only

For delivery information, visit our website at [www.usps.com](http://www.usps.com)®.

**STORRS MANSFIELD, CT 06269**

**CERTIFIED MAIL**

Certified Mail Fee \$ **\$3.35**

Extra Services & Fees (check box, add fee to postage)

<input type="checkbox"/> Return Receipt (hardcopy)	\$ <b>\$0.00</b>
<input type="checkbox"/> Return Receipt (electronic)	\$ <b>\$0.00</b>
<input type="checkbox"/> Certified Mail Restricted Delivery	\$ <b>\$0.00</b>
<input type="checkbox"/> Adult Signature Required	\$ <b>\$0.00</b>
<input type="checkbox"/> Adult Signature Restricted Delivery	\$ <b>\$0.00</b>

Postage \$ **\$2.24**

Total Postage and Fees \$ **\$5.59**

Sent To **Michael Jednak**

Street and Apt. No., or PO Box No. **25 Le Dogt Rd # 3252**

City, State, ZIP+4® **Storrs CT 06269**

Postmark Here **0572 11**

**11/01/2017**

PS Form 3800, April 2015 PSN 7530-02-000-9047 See Reverse for Instructions

Delivered to Agent

## Product &amp; Tracking Information

See Available Actions

**Postal Product:**  
First-Class Mail®

**Features:**  
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DATE & TIME	STATUS OF ITEM	LOCATION
November 6, 2017, 5:32 pm	Tendered to Final Delivery Agent	STORRS MANSFIELD, CT 06269

Your item has been tendered to a final delivery agent in STORRS MANSFIELD, CT 06269 on November 6, 2017 at 5:32 pm. The Postal Service no longer has the item.

November 6, 2017, 12:58 pm	Available for Pickup	STORRS MANSFIELD, CT 06269
November 6, 2017, 12:16 pm	Arrived at Unit	STORRS MANSFIELD, CT 06268



# USPS Tracking® Results

FAQs

Track Another Pac

Tracking Number: 70161370000084028962

7016 1370 0000 8402 8962

U.S. Postal Service™ CERTIFIED MAIL® RECEIPT Domestic Mail Only	
For delivery information, visit our website at <a href="http://www.usps.com">www.usps.com</a> ®.	
STORRS MANSFIELD, CT 06268	
Certified Mail Fee	\$3.35
Extra Services & Fees (check box, add fee to postage)	
<input type="checkbox"/> Return Receipt (hardcopy)	\$0.00
<input type="checkbox"/> Return Receipt (electronic)	\$0.00
<input type="checkbox"/> Certified Mail Restricted Delivery	\$0.00
<input type="checkbox"/> Adult Signature Required	\$0.00
<input type="checkbox"/> Adult Signature Restricted Delivery	\$0.00
Postage	\$2.24
Total Postage and Fees	\$5.59
Sent To	Mayor Shapiro
Street and Apt. No., or PO Box No.	4 S. Eagleville Rd
City, State, ZIP+4®	Mansfield CT 06268
PS Form 3800, April 2015 PSN 7530-02-000-9047	

0572 11  
Postmark Here  
11/01/2017

See Reverse for Instructions

Delivered

See Available Actions

## Product & Tracking Information

**Postal Product:**  
First-Class Mail®

**Features:**  
Certified Mail™

DATE &amp; TIME

STATUS OF ITEM

LOCATION

November 6, 2017, 11:33  
am

Delivered, Front  
Desk/Reception

STORRS  
MANSFIELD, CT 06268

Your item was delivered to the front desk or reception area at 11:33 am on November 6, 2017 in STORRS MANSFIELD, CT 06268.

November 6, 2017, 9:42 am

In Transit to Destination

ON ITS WAY TO STORRS  
MANSFIELD, CT 06268

November 5, 2017, 10:42  
pm

Departed USPS Regional  
Destination Facility

SPRINGFIELD MA  
NETWORK DISTRIBUTION  
CENTER

November 1, 2017

The Honorable Paul Shapiro  
Audrey P. Beck Municipal Building  
4 South Eagleville Road, Mansfield, CT 06268

RE: AT&T Wireless Modifications to Telecommunication Facility –  
60 North Eagleville Road, Mansfield, CT 06268

Dear Mayor Shapiro:

In order to accommodate technological changes, implement the Uniform Mobile Telecommunications System and enhance system performance in the State of Connecticut, AT&T Wireless ("AT&T") will be changing its equipment configuration at the above referenced telecommunications facility. AT&T Wireless currently maintains nine (9) antennas at the 185-foot level of an existing 291-foot guyed tower located at 60 North Eagleville Road, in Mansfield, CT. The tower is owned by the University of Connecticut. The property is owned by University of Connecticut.

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This letter is intended to serve as the required notice to the municipality. As required by the Regulations of Connecticut State Agencies ("RCSA") section 16-50j-73, the Connecticut Siting Council ("CSC") has been notified of the proposed changes and will review AT&T's proposal. Please accept this letter as notification under RCSA section 16-50j-73 of construction which constitutes an exempt modification pursuant to RCSA section 16-50j-72(b)(2).

The enclosed letter and documents to the CSC fully describes AT&T's proposal for the above referenced site. However, if you have any questions or require any additional information concerning our plans or the CSC procedures, please contact me at 443-286-4006 or contact Melanie Bachman, Acting Executive Director of the CSC at 860-872-2935.

Respectfully submitted,



Jack Andrews  
Zoning Manager, Empire Telecom  
o/b/o AT&T Wireless  
10130 Donleigh Drive  
Columbia, MD 21046  
443-677-0144  
[jandrews@empiretelecomm.com](mailto:jandrews@empiretelecomm.com)

Enclosures

cc: Melanie Bachman, Connecticut Siting Council



November 1, 2017

University of Connecticut  
Michael Jednak AVP,  
Facilities Operations & Building Services  
25 LeDoyt Road, Unit 3252  
Storrs CT 06269

RE: AT&T Wireless Modifications to Telecommunication Facility –  
60 North Eagleville Road, Mansfield, CT 06268

Dear Mr. Jednak:

In order to accommodate technological changes, implement the Uniform Mobile Telecommunications System and enhance system performance in the State of Connecticut, AT&T Wireless ("AT&T") will be changing its equipment configuration at the above referenced telecommunications facility. AT&T Wireless currently maintains nine (9) antennas at the 185-foot level of an existing 291-foot guyed tower located at 60 North Eagleville Road, in Mansfield, CT. The tower is owned by the University of Connecticut. The property is owned by University of Connecticut.

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This letter is intended to serve as the required notice to the tower owner and the property owner. As required by the Regulations of Connecticut State Agencies ("RCSA") section 16-50j-73, the Connecticut Siting Council ("CSC") has been notified of the proposed changes and will review AT&T's proposal. Please accept this letter as notification under RCSA section 16-50j-73 of construction which constitutes an exempt modification pursuant to RCSA section 16-50j-72(b)(2).



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Respectfully submitted,



Jack Andrews  
Zoning Manager, Empire Telecom  
o/b/o AT&T Wireless  
10130 Donleigh Drive  
Columbia, MD 21046  
443-677-0144  
[jandrews@empiretelecomm.com](mailto:jandrews@empiretelecomm.com)

Enclosures

cc: Melanie Bachman, Connecticut Siting Council



**Town of Mansfield, Connecticut**  
Web GIS Maps and Online Property Information

by MainStreetGIS, LLC [Town Website](#)  
[User Guide](#) [Feedback](#) [Disclaimer](#)

GIS Map Street View Tax Maps

Base Map: Town Base Map

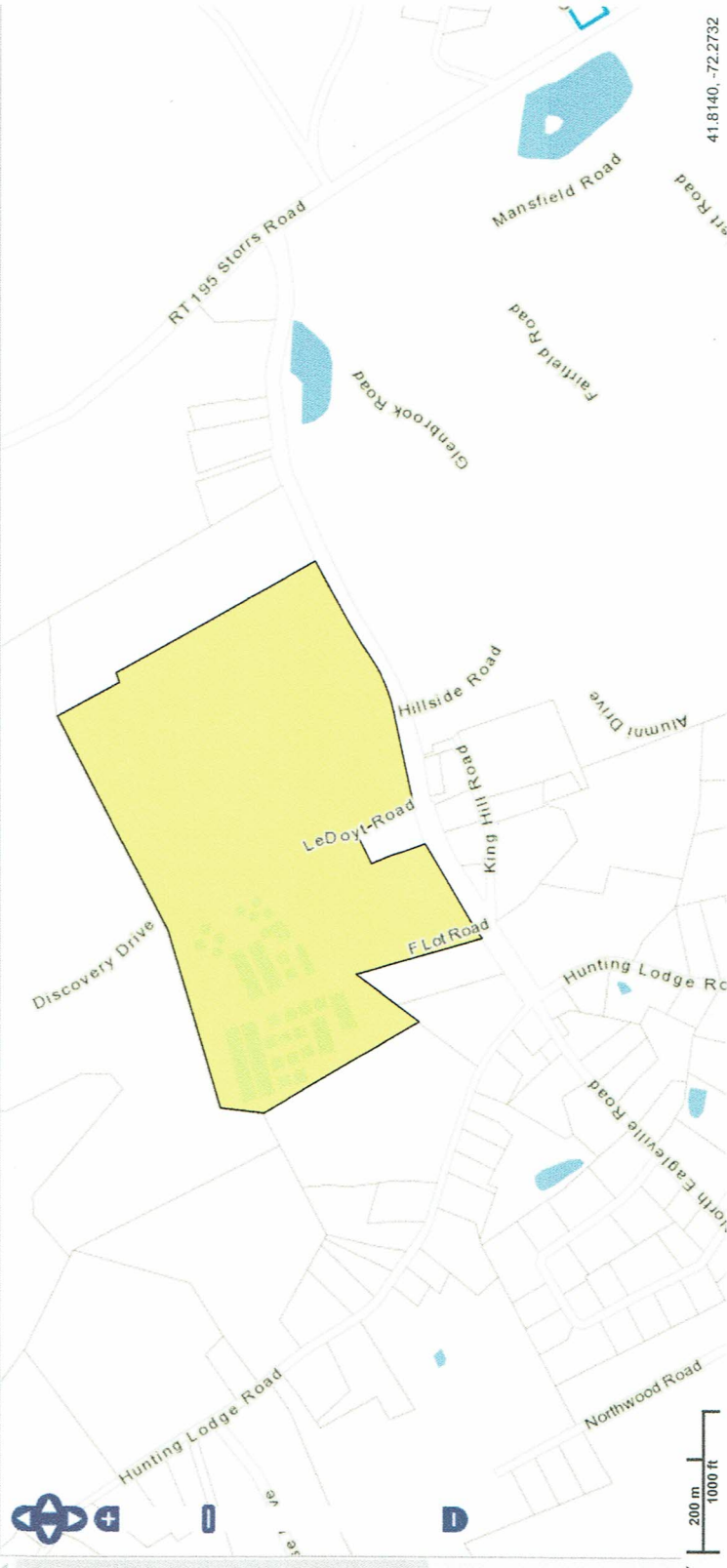
Address  Parcel ID  Google  Owner

**Layers** **Property** **Selection**

- ☒ Road Name Text
- ☒ Parcel Address Text
- ☒ Parcel ID Text
- ☐ Parcel Area Text
- ☐ Parcel Dimension Text
- ☒ New Town Boundary Survey Line
- ☐ Roads (Census 2010)
- ☐ Town Boundaries
- ☐ Natural Diversity Database (CT DEP)
- ☐ Critical Habitat (CT DEP)
- ☐ School District
- ☐ Historic District
- ☐ Historic Village
- ☐ Zoning
- ☐ Hydrant
- ☐ Utility Corridor
- ☐ Sewer Group
- ☐ Drainage Group
- ☐ Watershed (USDA 2001)
- ☐ Watershed (CT DEP)
- ☐ Aquifer Protection Area (CT DEP 2012)
- ☐ Protected Open Space (CT DEP 2011)
- ☐ DEP Property (CT DEP 2010)
- ☐ Trail
- ☐ Trail (CT DEP 2006)



1 : 8819



*11/1/17 - Property will not identify in GIS page*