

STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL Ten Franklin Square, New Britain, CT 06051 Phone: (860) 827-2935 Fax: (860) 827-2950 E-Mail: siting.council@ct.gov www.ct.gov/csc

VIA ELECTRONIC MAIL

May 29, 2018

Paul F. Sagristano Cherundolo Consulting 4 Davis Road West, Suite 5 Old Lyme, CT 06371

RE: **EM-SPRINT-028-180517** - Sprint Spectrum Realty Company, L.P. notice of intent to modify an existing telecommunications facility located at 600 Old Hartford Rd, Colchester, Connecticut.

Dear Mr. Sagristano:

The Connecticut Siting Council (Council) is in receipt of your email correspondence of May 24, 2018 submitted in response to the Council's May 22, 2018 notification of an incomplete request for exempt modification with regard to the above-referenced matter.

The submission renders the request for exempt modification complete and the Council will process the request in accordance with the Federal Communications Commission 60-day timeframe.

Thank you for your attention and cooperation.

Sincerely, Withhet

Melanie A. Bachman Executive Director

MB/FOC/jmb



 $S: \label{eq:start} S: \$

From: Paul Sagristano [mailto:psagristano@lrivassoc.com] Sent: Thursday, May 24, 2018 4:18 PM To: Cunliffe, Fred <<u>Fred.Cunliffe@ct.gov</u>> Cc: CSC-DL Siting Council <<u>Siting.Council@ct.gov</u>> Subject: RE: Council Incomplete Letter for EM-SPRINT-028-180517-OldHartfordRd-Colchester

Fred: Attached please find the Mount analysis as requested by CSC in the incomplete letter. Please advise if this is sufficient to continue the CSC review/approval of this site. Thank you!

Best,

Paul F. Sagristano 917-841-0247

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MASER CONSULTING — Connecticut—

Antenna Mount Analysis

FOR

CT33XC576 – North Colchester

DO Macro Project 600 Old Hartford Road Colchester, CT 06415 New London County

Mount Utilization: 84.6%

May 24, 2018

Prepared For

201 State Route 17 North Rutherford, NJ 07070

Prepared By

Maser Consulting Connecticut 331 Newman Springs Road, Suite 203 Redbank, NJ 07701

T: 732.383.1950



Petros E. Isoukalas. P.E. Geographic Discipline Leader Connecticut License No. PEN.32577

MC Project No. 17924006A

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Objective:

The objective of this report is to determine the capacity of the existing antenna support mount at the subject facility for the final wireless telecommunications configuration, per the applicable codes and standards.

Introduction:

Maser Consulting Connecticut has performed limited field observations on August 30, 2017 to verify the existing condition of the structure and to locate and quantify the existing wireless appurtenances where possible, from ground level. Maser Consulting Connecticut has reviewed the following documents in completing this report:

- Mount Mapping prepared by TEP, dated May 4, 2018.
- RFDS 45854 provided by Sprint, dated April 11, 2017.
- Previous Structural Analysis prepared by Fred A. Nudd Corporation, dated March 20, 214.

The proposed **Sprint** equipment is to be supported on an existing antenna support mount constructed of structural steel antenna support pipes supported by pipes and HSS tubes at a centerline of approximately 180'-0" above ground level. This report is based upon this information, as well as information obtained in the field.

Codes, Standards and Loading:

Maser Consulting Connecticut utilized the following codes and standards:

- 2016 Connecticut State Building Code, Incorporating the 2012 IBC
- Structural Standards for Antenna Supporting Structures and Antennas ANSI/TIA-222-G
 - Ultimate Wind Speed 130 mph
 - Nominal Wind Speed 101 mph
 - Exposure Category C
 - Structural Class II
 - Topographic Category 1
 - o Ice Wind 50 mph
 - Ice Thickness $-\frac{3}{4}$ "
- Specification for Structural Steel Buildings ANSI/AISC 360-10, American Institute of Steel Construction (AISC)

Loading used in this analysis is found in Appendix A of this report.

Analysis Approach & Assumptions:

The analysis approach used in this structural analysis is based on the premise that if the existing antenna support mount is structurally adequate to support the existing and proposed equipment per the aforementioned codes and standards, or if the increase in the forces in the structure is deemed to be negligible or acceptable, then the proposed equipment can be installed as intended.

The existing antenna mount for all sectors has been modeled in RISA-3D, a comprehensive structural analysis program. The program performs design checks of structures under user specified loads. The user specified loads have been calculated separately based on the requirements of the above referenced codes. The program performs an analysis based on the steel code to determine the adequacy of the members and produces the reactions at the connection points of the mounts to the existing structure.



General Site Design Assumption:

- All engineering services are performed on the basis that the information used is current and correct.
- It is assumed that the telecommunication equipment supports, antenna supports, and existing structure have been designed by a registered licensed professional engineer for the existing loads acting on the structure, as required by all applicable codes, prior to the proposed modifications listed within this report, if any.
- It is assumed that information provided by the client regarding the structure itself, the antenna models, feed lines, and other relevant information is current and correct.
- It is the responsibility of the client to ensure that the information provided to Maser Consulting Connecticut and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that the original design, material production, fabrication, and erection of the existing structure was performed in accordance with accepted industry design standards and in accordance with all applicable codes. Further, it is assumed that the existing structure and appurtenances have been properly maintained in accordance with all applicable codes and manufacturer's specifications and no structural defects and/or deterioration to the structural members has occurred.
- It is assumed all other existing appurtenances, antennas, cables, etc. belonging to others have been installed and supported per code and per specifications so as not to damage any existing structural support members, and that any contributing loads from adjacent equipment has been taken into consideration for their design.
- All services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. Maser Consulting Connecticut is not responsible for the conclusion, opinions, and recommendations made by others based on the information we supply.

Site Specific Design Parameters:

The following design parameters have been utilized in this report:

- Structural Steel Angles are constructed of A36 Steel
- Structural Steel Pipes are constructed of A53 Grade B Steel
- Structural Steel HSS tubes are constructed of A500 Grade B Steel
- The proposed antenna shall be mounted on a proposed 8' long 3.0 STD pipe mast in all sectors. The pipe masts will attach to the existing horizontal pipe member via pipe support cross plates (SitePro 1 P/N SP216 or approved equivalent). The pipe mast will be attached 8'-4" from the existing antenna pipe mast.
- The proposed RRHs will be mounted behind the proposed antenna on the proposed pipe mast in all sectors.



05/24/2018 Page 4 of 4 Prepared by CL Checked by PET

Calculations:

The calculations are found in Appendix A of this report.

Conclusion:

Maser Consulting Connecticut has determined the existing antenna support mount has **ADEQUATE** structural capacity to support the proposed loading. The existing antenna support mount has been determined to be stressed to a maximum of **84.6%** of its structural capacity with the maximum usage occurring at the HSS support members. Therefore, the proposed **Sprint** installation **CAN** be installed as intended.

The conclusions reached by Maser Consulting Connecticut in this evaluation are only applicable for the existing structural members supporting the proposed **Sprint** telecommunications installation described herein. Further, no structural qualifications are made or implied by this document for the existing structure.

Maser Consulting Connecticut reserves the right to amend this report if additional information about the existing members is provided. The conclusions reached by Maser Consulting Connecticut in this report are only valid for the appurtenances listed in this report. Any change to the installation will require a revision to this structural analysis.

We appreciate the opportunity to be of service on this project. If you should have any questions or require any additional information, please do not hesitate to call our office.

Sincerely,

Maser Consulting Connecticut

Petros E. Tsoukalas, P.E. Geographic Discipline Leader

Carol Luengas, E.I.T. Engineer

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APPENDIX A



Sprint	Computed By:	CL
CT33XC576	Date:	5/24/2018
17924006A	Verified By:	PET
Antenna Mount Analysis	Page:	1

Version 3.3

1. LOADING SUMMARY

Quantity	Manufacturer	Antenna/ Appurtenance	Status	Sector
3	RFS	APXV9ERR18-C-A20	Existing	Alpha, Beta, & Gamma
3	COMMSCOPE	DT465B-2XR-V2	Proposed	Alpha, Beta, & Gamma
3	ALCATEL-LUCENT	1900 MHz RRH	Existing	Alpha, Beta, & Gamma
3	ALCATEL-LUCENT	800 2x50W	Existing	Alpha, Beta, & Gamma
3	ALCATEL-LUCENT	TD-RRH 8x20	Proposed	Alpha, Beta, & Gamma
3	ALCATEL-LUCENT	800 2x50W	Proposed	Alpha, Beta, & Gamma

The worst case loading occurs in the Alpha Sector

Quantity	Manufacturer	Antenna/ Appurtenance	Status
1	RFS	APXV9ERR18-C-A20	Existing
1	COMMSCOPE	DT465B-2XR-V2	Proposed
1	ALCATEL-LUCENT	1900 MHz RRH	Existing
1	ALCATEL-LUCENT	800 2x50W	Existing
1	ALCATEL-LUCENT	TD-RRH 8x20	Proposed
1	ALCATEL-LUCENT	800 2x50W	Proposed



	Sprint	Computed By:	CL
e:	CT33XC576	Date:	5/24/2018
D .	17924006A	Verified By:	PET
	Antenna Mount Analysis	Page:	2

ANALYSIS AND DESIGN

•	Client:	Sprint	Computed By:	CL
MASER CONSULTIN	_G Site Name:	CT33XC576	Date:	5/24/2018
- CONNECTICUT-	Project No.	17924006A	Verified By:	PET
	Title:	Antenna Mount Analysis	Page:	3

I. DESIGN INPUTS

Calculations for gravity and lateral loading on equipment and support mounts are determined as per the ANSI/TIA-222-G Code, Addendum 2

		<u>Reference</u>	<u>Equation</u>
Wind Load Inputs Parameters			
Antenna Centerline	z 180 ft		
Ultimate Wind Speed	V _u 130 mph		
Normal Wind Speed (3 sec. Gust):	V 101 mph	Ref. 1, Eqn. 16-33	
Normal Wind Speed with Ice (3 sec. gust):	Vi 50.0 mph	(Figure a5-2a, p. 233)	
Service Wind Speed:	V _s 60.0 mph	(Figure a5-2a, p. 233)	
Design Ice Thickness:	t _i 0.75 in	(Figure A1-2a, p. 233)	
Exposure Category:	С	Ref. 3, Section 2.6.5.1	
Structure Class:	11	Ref. 3, Table 2-1	
Gust Effect Factor:	G _h 0.85	Ref. 3, Section 2.6.7	
Wind Directionality Factor:	К _d 0.85	Ref. 3, Table 2-2	
Topographic Category:	1	Ref. 3, Section 2.6.6.2	
Wind Load Coefficients			
Importance Factors:			
Non-Iced:	I 1	Ref. 3, Table 2-3	
Iced:	l _{ice} 1	(Table 2-3, P. 39)	
Exposure Category Coefficients:			
3-s Gust-Speed Power Law Exponent:	α 9.5	Ref. 3, Table 2-4	
Nominal Height of the Atmospheric Boundary Layer:	Z g 900 ft	Ref. 3, Table 2-4	
Min. Value for k _z :	Kz _{min} 0.85	Ref. 3, Table 2-4	
Terrain Constant:	K _e 1.00	Ref. 3, Table 2-4	
Velocity Pressure Exposure Coefficient:	K _z 1.432	Ref. 3, Section 2.6.5.2	=2.01 $\cdot (z/z_g)^{2/\alpha}$
Topographic Category Coefficients:			
Topographic Constant:	K _t N/A	Ref. 3, Table 2-5	
Height Attenuation Factor:	f <i>N/A</i>	Ref. 3, Table 2-5	
Height Reduction Factor:	K _h N/A	Ref. 3, Section 2.6.6.4	=e ^(f·z/H)
Topographic Factor:	K _{zt} 1.00	Ref.3, Section 2.6.6.4	=[1+($K_e \cdot K_t/K_h$)] ²
Ice Accumulation:			
Ice Velocity Pressure Exposure Coefficient:	K _{iz} 1.18		$=(z/33)^{0.10}$
Factored Ice Thickness:	t _{iz} 1.78 in	(Section 2.6.8, p. 16)	$= 2.0 \cdot t_i \cdot I \cdot K_{iz} \cdot K_{zt}$
Ice Density:	ρ _i 56.00 pcf		
Design Wind Pressures:			
Velocity Pressure:	q z 31.60 psf	Ref. 3, Section 2.6.9.6	$= 0.00256 \cdot K_z \cdot K_{zt} \cdot K_d \cdot V^2 \cdot I$
Velocity Pressure (With Ice):	q _{zi} 7.79 psf	(Section 2.6.9.6, P. 25)	$=.00256 \cdot K_z \cdot K_{zt} \cdot K_d \cdot V_i^2 \cdot I$
Velocity Pressure (Service):	q _{zs} <u>11.22</u> psf	(Section 2.6.9.6, P. 25)	=.00256· K_z · K_{zt} · K_d · V_i ² · I



Client:	Sprint	Computed By:	CL
Site Name:	CT33XC576	Date:	5/24/2018
Project No.	17924006A	Verified By:	PET
Title:	Antenna Mount Analysis	Page:	4

II. CALCULATIONS

Wind Load on Appurtenances

Dimensions and Force Coefficients

		Non-Iced Condition								Iced Condition						
	Ν	/lounting Pipe	9		Equipment				I	Mounting Pipe Equipment						
Antenna/ Appurtenance	Length	Diameter	Force Coefficient	Height	Width	Depth	Force Co	oefficient	Length	Diameter	Force Coefficient	Height	Width	Depth	Force Co	efficient
	(in) (in)	(in)	C _a	(in)	(IN)	(11)	C _{a Front}	C _{a Side}	(in)	(11)	Ca	(11)	(11)	(111)	C _{a Front}	C _{a Side}
APXV9ERR18-C-A20	72.0	2.400	1.200	72.00	11.80	7.90	1.36	1.47	75.6	6.0	0.926	75.55	15.35	11.45	1.31	1.38
DT465B-2XR-V2	96.0	3.500	1.200	71.90	13.80	8.20	1.32	1.46	99.6	7.1	0.958	75.45	17.35	11.75	1.28	1.37
1900 MHz RRH	0.0	0.000	0.000	25.00	12.40	12.20	1.20	1.20	0.0	0.0	0.000	28.55	15.95	15.75	1.20	1.20
800 2x50W	0.0	0.000	0.000	19.00	13.00	8.61	1.20	1.20	0.0	0.0	0.000	22.55	16.55	12.16	1.20	1.20
TD-RRH 8x20	0.0	0.000	0.000	26.10	18.60	6.70	1.20	1.26	0.0	0.0	0.000	29.65	22.15	10.25	1.20	1.22
800 2x50W	0.0	0.000	0.000	19.00	13.00	8.61	1.20	1.20	0.0	0.0	0.000	22.55	16.55	12.16	1.20	1.20

			Non-Iced	l Condition		Iced Condition				
Antenna/ Appurtenance	# of Brackets	Wind Fo	rce (lbs.)	Controlling Wind Force Gravity (Ib		Wind Force (lbs.)		Controlling Wind Force	Gravity (lbs.)	
		F _N	F _T	(lbs.)		F _N	F _T	(lbs.)		
APXV9ERR18-C-A20	2	107.8	97.4	107.8	34.8	34.9	37.1	37.1	109.2	
DT465B-2XR-V2	2	131.6	117.8	131.6	36.1	43.3	43.5	43.5	121.7	
1900 MHz RRH	1	69.4	68.3	69.4	60.0	25.1	24.8	25.1	99.1	
800 2x50W	1	55.3	36.6	55.3	53.0	20.6	15.1	20.6	70.9	
TD-RRH 8x20	1	108.7	41.2	108.7	70.0	36.3	17.0	36.3	115.6	
800 2x50W	1	55.3	36.6	55.3	53.0	20.6	15.1	20.6	70.9	

* ALL CALCULATED LOADS ARE PER MOUNTING BRACKET. TO GET THE TOTAL EQUIPMENT LOAD, MULTIPLY THE INDIVIDUAL LOADS BY THE NUMBER OF BRACKETS

Wind Load on Framing Members

				Non-Iced Condition			Iced Condition					
Member Category	Member Shape	Length (in)	Member Surface	Exposed Wind	Force Coefficient	Wind Load	Exposed Wind	Depth	Length	Force Coefficient	Wind Load	Ice Weight
				Height (in)	C _a (pii)	Height (in)	(11)	(11)	Ca	(pii)	(pii)	
Pipe	Pipe 3.5	172	Round	4.00	1.20	10.75	7.55	7.55	175.55	1.16	4.84	12.54
Pipe	Pipe 2.0	60	Round	2.38	1.20	6.38	5.93	5.93	63.55	0.88	2.89	9.02
Square HSS	HSS 4X4	44	Square	4.00	1.53	13.73	7.55	7.55	47.55	1.37	5.71	16.14
Square HSS	HSS 4X4	28	Square	4.00	1.40	12.53	7.55	7.55	31.55	1.27	5.31	16.14
Equal Angle	L4x4	43	Square	4.00	1.52	13.65	7.55	7.55	46.55	1.36	5.68	16.14
Equal Angle	L2x2	17	Square	2.00	1.45	6.49	5.55	5.55	20.55	1.25	3.84	10.00
Grating												18.92171015

	Client:	Sprint	Computed By:	CL
MASER CONSULTING	Site Name:	CT33XC576	Date:	5/24/2018
	Project No.	17924006A	Verified By:	PET
	Title:	Antenna Mount Analysis	Page:	5

BASIC EQUATIONS

ANSI/TIA-222-G Reference

Table 2-3, Pg. 39

I

Force Coefficient: (Square) $C_{\underline{f}_square}(h, w) := \begin{bmatrix} 1.2 & \text{if } \frac{h}{w} \le 2.5 \\ 1.2 + \frac{0.2}{4.5} \cdot \left(\frac{h}{w} - 2.5\right) \end{bmatrix} & \text{if } \frac{h}{w} > 2.5 \land \frac{h}{w} \le 7 \\ \left[1.4 + \frac{0.6}{18} \cdot \left(\frac{h}{w} - 7\right) \right] & \text{if } \frac{h}{w} > 7 \land \frac{h}{w} \le 25 \\ 2.0 & \text{otherwise} \end{bmatrix}$

Force Coefficient:	$C_{f \text{ round}}(h, w) :=$	0.7 if $\frac{h}{2} \le 2.5$	Table 2-8, P. 42
(Round)	$C_{f_round}(n, w) :=$	$\begin{bmatrix} 0.7 + \frac{1}{w} \leq 2.5 \\ 0.7 + \frac{0.1}{4.5} \cdot \left(\frac{h}{w} - 2.5\right) \end{bmatrix} \text{ if } \frac{h}{w} > 2.5 \land \frac{h}{w} \leq 7$ $\begin{bmatrix} 0.8 + \frac{0.4}{18} \cdot \left(\frac{h}{w} - 7\right) \end{bmatrix} \text{ if } \frac{h}{w} > 7 \land \frac{h}{w} \leq 25$ 1.2 otherwise	

Terrain Exposure Constants:

Table 2-4, P. 40

$$\alpha := \begin{bmatrix} 7.0 & \text{if Exp} = "B" & Z_g := \\ 9.5 & \text{if Exp} = "C" & \\ 11.5 & \text{if Exp} = "D" & \\ \end{bmatrix} \begin{array}{c} 1200 \text{ft if Exp} = "B" & K_{zmin} := \\ 900 \text{ft if Exp} = "C" & \\ 700 \text{ft if Exp} = "C" & \\ 1.03 & \text{if Exp} = "D" & \\ \end{array}$$

MASER CONSULTING — CONNECTICUT—	Client:	Sprint	Computed By:	CL
	Site Name:	CT33XC576	Date:	5/24/2018
	Project No.	17924006A	Verified By:	PET
	Title:	Antenna Mount Analysis	Page:	6
			-	

BASIC EQUATIONS

Velocity Pressure Coefficient:

$Kz(z) := \begin{bmatrix} K_z \leftarrow \max\left[2.01 \cdot \left(\frac{z}{Z_g}\right)^{\alpha}, K_{zmin}\right] \\ K_z \leftarrow \min(K_z, 2.01) \end{bmatrix}$

$$K_z := Kz(z)$$

Section 2.6.5, P. 13

ANSI/TIA-222-G Reference

$$\begin{array}{c|c} \text{Kzt}(z) := \text{K}_{zt} \leftarrow & 1.0 \ \text{if Topo} = "1" & \text{Section 2.6.6.4, p. 14} \\ \text{otherwise} & \text{Table 2-4 p. 40} \\ & \text{I.00 \ if Exp} = "B" & \text{Table 2-4 p. 40} \\ & 1.00 \ \text{if Exp} = "C" & \text{I.10 \ if Exp} = "D" \\ & \text{K}_t \leftarrow & 0.43 \ \text{if Topo} = "2" & \text{Table 2-5 p. 40} \\ & 0.53 \ \text{if Topo} = "3" & \text{I.25 \ if Topo} = "4" \\ & \text{f} \leftarrow & 1.25 \ \text{if Topo} = "3" \\ & 1.50 \ \text{if Topo} = "3" \\ & 1.50 \ \text{if Topo} = "3" \\ & 1.50 \ \text{if Topo} = "4" \\ & \text{K}_h \leftarrow e^{\left(\frac{\text{f} \cdot z}{\text{CH}}\right)} & \text{Section 2.6.6.4, P. 14} \\ & \left(1 + \frac{\text{K}_e \cdot \text{K}_t}{\text{K}_h}\right)^2 & \text{Section 2.6.6.4, P. 14} \end{array}$$

 $K_{zt} := Kzt(z)$

Section 2.6.9.6, P. 25

Velocity Pressure:

 $q_z := 0.00256 \cdot K_z \cdot K_{zt} \cdot K_d \cdot V^2 \cdot I \cdot psf$



	Sprint	Computed By:	CL
me:	CT33XC576	Date:	5/24/2018
No.	17924006A	Verified By:	PET
	Antenna Mount Analysis	Page:	7

LOAD EQUATIONS

Title:

WIND LOAD

Area (Normal):
Area (Side):
Force Coefficient (Normal):
Force Coefficient (Side):
Pipe Area (Normal):
Pipe Area (Side):
Force Coefficient (Normal):
Normal Effective Projected Area:
Side Effective Projected Area:
Effective Projected Area:
Wind Force:

ICE DEAD LOAD

Largest Out-to-Out Dimension: Cross Sectional Area of Ice: Total Ice Dead Load:

ICE WIND LOAD

Dimensions:

Area (Normal): Area (Side): Force Coefficient (Normal): Force Coefficient (Side): Pipe Area (Normal): Pipe Area (Side): Force Coefficient (Normal): Normal Effective Projected Area: Side Effective Projected Area: Effective Projected Area: Wind Force: $\begin{aligned} AN_{area} &= H_{ant} \cdot Want \\ AT_{area} &= H_{ant} \cdot Dant \\ C_{fn} &= C_{fsquare}(H_{ant}, Want) \\ C_{fs} &= C_{fsquare}(H_{ant}, Dant) \\ AN_p &= \max[(L_p - H_{ant}) * Dp, 0] \\ AT_p &= L_p \cdot Dp \\ C_{fp} &= C_{fround}(Lp, Dp) \\ E_{pan} &= (C_{fn} \cdot ANarea) + (Cfp \cdot ANp) \\ E_{pat} &= (C_{fs} \cdot ATarea) + (Cfp \cdot ATp) \\ EPA &= \max(E_{pan}, Epat) \\ F_{ant} &= q_z \cdot Gh \cdot EPA \end{aligned}$

 $D_{ant} = \sqrt{D_{ant}^{2} + W_{ant}^{2}}$ $A_{ice_{ant}} = \pi \cdot tiz \cdot (Dant + tiz)$ $DL_{ice_{ant}} = \mathbf{\rho}_{i} \cdot (Aice_{ant} \cdot Hant)$

$$\begin{split} H_{i_{ant}} &= H_{ant} + 2tiz \\ W_{i_{ant}} &= W_{ant} + 2tiz \\ D_{i_{ant}} &= D_{ant} + 2tiz \\ AIN_{area} &= H_{i_{ant}} \cdot W_{i_{ant}} \\ AIT_{area} &= H_{i_{ant}} \cdot D_{i_{ant}} \\ Ci_{fn} &= C_{fsquare}(H_{i_{ant}}, W_{i_{ant}}) \\ Ci_{fs} &= C_{fsquare}(H_{i_{ant}}, D_{i_{ant}}) \\ AN_{p} &= \max[(L_{ip} - H_{i_{ant}}) * D_{ip}, 0] \\ AT_{p} &= L_{ip} \cdot Dip \\ C_{fp} &= C_{fround}(L_{ip}, D_{ip}) \\ E_{pain} &= (Ci_{fn} \cdot ANarea) + (Cfp \cdot ANp) \\ E_{pait} &= max(E_{pain}, Epait) \\ F_{i_{ant}} &= q_{z} \cdot Gh \cdot EPAi \end{split}$$



Sprint	Computed By:	CL
CT33XC576	Date:	5/24/2018
17924006A	Verified By:	PET
Antenna Mount Analysis	Page:	8

III. ATTACHMENTS



Sprint	Computed By:	CL
CT33XC576	Date:	5/24/2018
17924006A	Verified By:	PET
Antenna Mount Analysis	Page:	9

RISA MODEL



MASER CONSULTING	Client:	Sprint	Computed By:	CL
	; Site Name:	CT33XC576	Date:	5/24/2018
-CONNECTICUT	Project No.	17924006A	Verified By:	PET
	Title:	Antenna Mount Analysis	Page:	10

RISA WORST CASE LOADING





MASER CONSULTING — CONNECTICUT—	Client:	Sprint	Computed By:	CL
	Site Name:	CT33XC576	Date:	5/24/2018
	Project No.	17924006A	Verified By:	PET
	Title:	Antenna Mount Analysis	Page:	11

RISA CODE CHECK



