TITLE	DATE*
STRUCTURES GENERAL NOTES	01-09
MISCELLANEOUS CONCRETE DETAILS	01-09
MASONRY DETAILS	01-09
ROOF DIAPHRAGM TO MASONRY WALL DETAILS	01-09

TITLE	DATE*

*REVISED OR ADDED

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STATE OF CONNECTICUT
DEPARTMENT OF TRANSPORTATION

Filename: CTDOT_STRUCTURAL_GD.dgn Model: XX-GD_INDEX

OFFICE OF ENGINEERING

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GUIDE SHEET INDEX

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CODES AND REFERENCES

1. ALL WORK SHALL CONFORM TO THE STATE OF CONNECTICUT BUILDING CODE AND THE CONTRACT DOCUMENTS.

DESIGN LOADS

1. ROOF SNOW LOAD
   a. GROUND SNOW LOAD, Pg = 30 PSF.
   b. FLAT-ROOF SNOW LOAD, Pf = 30 PSF.
   c. EXPOSURE FACTOR, Ce = 1.0
   d. SNOW IMPORTANCE FACTOR, Is = 1.2
2. WIND LOAD
   a. BASIC WIND SPEED, 110 MPH.
   b. WIND IMPORTANCE FACTOR, Iw = 1.15
   c. EXPOSURE B
3. SEISMIC LOAD
   a. PEAK VELOCITY-RELATED ACCELERATION, Av = 0.11
   b. PEAK ACCELERATION, Aa = 0.15
   c. SEISMIC IMPORTANCE FACTOR, Ie = 1.5
   d. SEISMIC USE GROUP III
   e. SITE CLASSIFICATION - C
   f. LATERAL LOAD-RESISTING SYSTEMS:
      STEEL BRACED FRAME
      REINFORCED CMU SHEAR WALLS
   g. RESPONSE MODIFICATION FACTOR, R = 5
      DEFLECTION AMPLIFICATION FACTOR, Cd = 2½
   h. ANALYSIS PROCEDURE UTILIZED - EQUIVALENT LATERAL FORCE PROCEDURE
   i. SEISMIC DESIGN CATEGORY C.
   j. SEISMIC LOAD FACTORS, SDs = 0.2504, SD1 = 0.0963

GENERAL NOTES

1. THE STRUCTURAL DRAWINGS SHALL BE COORDINATED WITH OTHER DISCIPLINE DRAWINGS AND WITH EQUIPMENT MANUFACTURERS TO ENSURE THAT OPENINGS IN THE ROOF AND WALLS ARE PROVIDED WITH THE REQUIRED APPURTENANT FRAMING OR SUPPORT SYSTEM.
2. DIMENSIONS AND DETAILS RELATED TO THE SIZE AND LOCATION OF EQUIPMENT SHALL BE VERIFIED WITH THE EQUIPMENT MANUFACTURER PRIOR TO CONSTRUCTION.
3. SIZES AND LOCATIONS OF EMBEDDED MECHANICAL AND ELECTRICAL FIXTURES SHALL BE VERIFIED PRIOR TO CONSTRUCTION.
4. METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITIES OF THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE FOR IDENTIFYING AND IMPLEMENTING THE NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.
5. TEMPORARY BRACING, SHEETING, SHORING AND OTHER SIMILAR SAFETY PRECAUTIONARY MEASURES DURING CONSTRUCTION ARE THE RESPONSIBILITIES OF THE CONTRACTOR AND SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EMPLOYED BY HIM.
6. THE CONTRACTOR SHALL REPAIR, AT THEIR OWN EXPENSE, ANY DAMAGE TO THE STRUCTURES AND APPURTENANCES DUE TO CONTRACTOR'S CONSTRUCTION OPERATIONS.
7. THE IMPLEMENTATION OF JOB SAFETY SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

CONCRETE

1. FOUNDATIONS SHALL BE PLACED ON 12 INCHES MINIMUM OF COMPACTED GRANULAR FILL IN ACCORDANCE WITH FORM 816, SECTION 2.13. COMPACTION SHALL BE DONE STATICALLY. EXTEND THE LIMITS OF GRANULAR FILL BY 2 FEET HORIZONTALLY OUTSIDE THE ENTIRE FOOTPRINT OF THE BUILDING. THE AREA OUTSIDE OF THE LIMITS OF GRANULAR FILL IS TO HAVE THE FOUNDATION PREPARATION IN ACCORDANCE WITH FORM 816 SECTION 2.03.03-2.
2. SLAB-ON-GRADE SHALL BE PLACED ON 8 INCHES THICK OF PROCESSED AGGREGATE BASE IN ACCORDANCE WITH FORM 816, SECTION 2.13. COMPACTION SHALL BE DONE STATICALLY.
3. PROVIDE RIGID INSULATION AT EXTERIOR WALLS (TYP.) AND VAPOR BARRIER UNDER SLAB-ON-GRADE (TYP.), AS SHOWN ON THE STRUCTURAL PLANS.
4. THE CONTRACTOR SHALL COORDINATE THE PLACEMENT OF FOUNDATION EXPANSION JOINTS AND MASONRY CONTROL JOINTS. CONTROL JOINTS FOR MASONRY SHALL BE PLACED AT LOCATIONS WHERE EXPANSION JOINTS OCCUR IN THE FOUNDATION.
5. CONSTRUCTION JOINTS IN THE FOUNDATION WALLS SHALL BE PROVIDED AT A MAXIMUM SPACING OF 50 FT.
6. REINFORCED CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
7. ALL CAST-IN-PLACE CONCRETE SHALL BE NORMAL WEIGHT AND SHALL BE CLASS 'F' IN ACCORDANCE WITH FORM 816, SECTION M.03. CONCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 4,000 PSI.
8. REINFORCING BARS SHALL BE DEFORMED BILLET STEEL CONFORMING TO ASTM A615, GRADE 60.
9. COORDINATE PLANS WITH THE CIVIL, MECHANICAL AND ELECTRICAL DRAWINGS FOR MISCELLANEOUS CONCRETE WORK.
10. WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185.
11. REINFORCEMENT DETAILS SHALL CONFORM TO ACI 318. ALL CONCRETE REINFORCEMENT SPLICES SHALL BE ACI 318 CLASS 'B' SPLICES, UNLESS OTHERWISE NOTED.
12. ADDITIONAL REINFORCEMENT AT PENETRATIONS THROUGH REINFORCED CONCRETE WALLS AND SLABS SHALL BE PROVIDED AS SHOWN ON THE STRUCTURAL PLANS.
13. EXPOSED CORNERS OF CONCRETE SHALL HAVE A ¾" X ¾" CHAMFER, UNLESS OTHERWISE NOTED.
14. NON-SHRINK GROUT UNDER BASE PLATES SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 6,000 PSI.
15. REINFORCING CLEAR COVER, UNLESS OTHERWISE NOTED:
    FOOTINGS - 3"
    FOUNDATION WALLS AND PIERS - 2"
    SLAB ON GRADE - 2"
16. CONCRETE MIX DESIGN WITH ADMIXTURES SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL.

STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO THE LATEST EDITION OF AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
2. STRUCTURAL SHAPES AND PLATES SHALL CONFORM TO ASTM A992 50 KSI, STEEL, UNLESS OTHERWISE NOTED.
3. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS OF AWS D1.1, USING E7018 ELECTRODES.
4. WHERE WELD SIZES ARE NOT SPECIFIED, A MINIMUM WELD SIZE SHALL BE USED IN ACCORDANCE WITH THE REQUIREMENTS OF AWS D1.1
5. WELDS SHALL BE VISUALLY INSPECTED, UNLESS OTHERWISE NOTED.
6. SHOP AND FIELD WELDS SHALL BE INSPECTED BY AN AWS-CERTIFIED WELDING INSPECTOR HIRED BY THE CONTRACTOR.
7. BOLTS SHALL BE ¾" DIAMETER ASTM A325-X WITH $^{13}/_{16}$" DIAMETER HOLES, UNLESS OTHERWISE NOTED. ALL BOLT CONNECTIONS SHALL BE SLIP-CRITICAL UNLESS OTHERWISE NOTED.
8. ALL A325 BOLTS SHALL BE PRETENSIONED, UNLESS OTHERWISE NOTED. TENSION-CONTROL (TC) BOLTS ARE ALSO ACCEPTABLE.
9. BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST EDITION OF THE SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS.
10. ALL STRUCTURAL STEEL SHALL BE PRIMED AND PAINTED IN CONFORMANCE WITH THE CONTRACT SPECIFICATIONS.
11. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, UNLESS OTHERWISE NOTED.
12. COLUMN BASE PLATES SHALL BE LEVELED USING DOUBLE NUTS AND GROUT.
13. THE CONTRACTOR SHALL COMPLY WITH OSHA SAFETY STANDARD FOR STEEL ERECTION, EFFECTIVE JANUARY 18, 2002.

MASONRY

1. MASONRY DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 530, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES".
2. CONCRETE BLOCK UNITS SHALL BE TYPE I, LIGHTWEIGHT, LOAD BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C90 FOR HOLLOW BLOKS AND ASTM C145 FOR SOLID BLOCKS.
3. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH (fm) OF 1,900 PSI.
4. ALL REINFORCED CELLS SHALL BE CONTINUOUSLY GROUTED. GROUT IS TO CONFORM TO ASTM C476 AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.
5. MORTAR FOR CONCRETE BLOCK SHALL BE TYPE S, CONFORMING TO ASTM C270, AS SPECIFIED, UNLESS OTHERWISE NOTED. THE FIRST COURSE OF BLOCK SHALL BE PLACED USING A FULL MORTAR BED. FOR EXTERIOR BRICK VENEER MORTAR SHALL BE TYPE N, CONFORMING TO ASTM C270.
6. MASONRY WALLS WITH A HEIGHT GREATER THAN 12 FT. SHALL BE LATERALLY SUPPORTED AT THE TOP AS SPECIFIED ON THE STRUCTURAL DRAWINGS USING ANGLES ATTACHED TO STRUCTURAL STEEL OR ROOF DECKING, UNLESS OTHERWISE NOTED. THE ATTACHMENT SHALL ALLOW VERTICAL MOVEMENT (DEFLECTION) OF THE DECK AND JOIST, UNLESS OTHERWISE NOTED. ALL MASONRY WALLS SHALL BE APPROPRIATELY SHORED DURING CONSTRUCTION TO RESIST ALL DESIGN WIND LOADING.
7. BOND BEAMS WITH TWO (2) #5 BARS SHALL BE PROVIDED AT THE TOP OF ALL MASONRY WALLS, UNLESS OTHERWISE NOTED.
8. LINTELS SHALL BE PROVIDED AT ALL MASONRY OPENINGS AS SHOWN ON THE STRUCTURAL PLANS OR AS REQUIRED DUE TO ELECTRICAL OR MECHANICAL PENETRATIONS, UNLESS OTHERWISE NOTED.
9. PROVIDE ONE (1) #5 REBAR EACH SIDE OF ALL OPENINGS WITHIN MASONRY, OR CELLS ADJACENT TO STRUCTURAL STEEL OR CONTROL JOINTS. REBAR SHALL EXTEND 2 FEET BEYOND OPENING, OR BE HOOKED IF APPROVED BY THE DESIGNER.
10. ALL VISIBLE JOINTS SHALL BE SEALED. PROVIDE BACKER ROD.
11. PROVIDE CONTROL JOINTS IN MASONRY AS SHOWN ON THE PLANS AND AS CALLED FOR WITHIN THE CONTRACT SPECIFICATIONS.
12. WHERE MASONRY IS ADJACENT TO STRUCTURAL STEEL, FLEXIBLE ANCHORS, COMPRESSIBLE JOINT FILLER MATERIAL AND JOINT SEALANT SHALL BE USED.
13. MASONRY REINFORCEMENT SPLICES SHALL CONFORM TO ACI 530, UNLESS OTHERWISE SHOWN.
14. UNFINISHED WALLS AT THE END OF A WORKDAY SHALL BE COVERED TO PREVENT INFILTRATION OF WATER.

STEEL JOIST

1. STEEL JOISTS SHALL BE INSTALLED IN ACCORDANCE THE THE SJI STANDARD SPECIFICATIONS FOR STEEL JOIST CONSTRUCTION, UNLESS OTHERWISE NOTED. WELDERS MUST BE AWS-CERTIFIED.
2. WHERE THE BOTTOM OF A JOIST IS TO BE ATTACHED TO A BEAM, COLUMN OR WALL, THE ATTACHMENT SHALL BE MADE AFTER THE ROOF SYSTEM HAS BEEN INSTALLED AND ATTAINED FULL DEAD LOAD DEFLECTION.
3. MISCELLANEOUS SUPPORTS ATTACHED TO JOISTS SHALL BE LOCATED AT THE PANEL POINTS (NODES) AND SHALL NOT IMPOSE LOADS EXCEEDING 200 POUNDS.
4. ALL JOIST EXTENSIONS SHALL BE PROVIDED AS INDICATED ON THE PLANS OR AS REQUIRED FOR COMPLETE INSTALLATION.
5. ALL JOISTS SHALL BE ANCHORED DOWN AT BEARING POINTS AS SHOWN ON THE STRUCTURAL PLANS.
6. STABILIZER PLATES SHALL BE INSTALLED AT THE TWO END JOISTS AND AT EVERY THIRD INTERIOR JOIST SUCH THAT THE MAXIMUM DISTANCE BETWEEN STABILIZED JOISTS IS 16'-6".
7. JOISTS SHALL BE DESIGNED FOR 15 LBS. PER SQUARE FOOT OF NET UPLIFT FORCE.
8. THE JOIST MANUFACTURER SHALL VERIFY THE WEIGHTS OF EQUIPMENTS (UNIT HEATERS, ETC.) AND POINTS OF SUPPORTS FOR THE PURPOSE OF SPECIAL JOIST MANUFACTURE, WHERE APPLICABLE.

METAL DECKING

1. ALL STEEL DECK SHALL BE MANUFACTURED AND INSTALLED ACCORDING TO THE SDI "SPECIFICATIONS FOR COMPOSITE DECKS, FORM DECKS, ROOF DECKS AND CELLULAR METAL FLOOR DECK WITH ELECTRICAL DISTRIBUTION," AND SHALL BE INSTALLED ACCORDING TO THE PROVISIONS AS OUTLINED WITHIN THE CONTRACT SPECIFICATIONS.
2. ALL STEEL DECK SHALL COMPLY WITH ASTM A446, AND SHALL BE GALVANIZED PER ASTM A525, COATING DESIGNATION G90.
3. METAL DECKING SHALL BE ATTACHED AS SPECIFIED IN SPECIFICATION SECTION 053100 OF THE CONTRACT SPECIFICATIONS.
4. ROOF DECK SHALL BE 1½" WIDE RIB, 18 GAUGE STEEL TYPE B, WITH MINIMUM YIELD STRENGTH OF 33 KSI.
   -SUPPORT FASTENERS: ⅝" PUDDLE WELDS,
   -SIDELAP FASTENERS: 2-#10 TEK SCREWS
5. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH THE REQUIRMENTS OF AWS D1.3.
6. ALL PUDDLE WELD BLOWOUTS MUST BE PATCHED.

ABBREVIATIONS

BRG	BEARING
BFE	BOTTOM OF FOOTING ELEVATION
BOS	BOTTOM OF STEEL
℄	CENTER LINE
CJ	CONTROL (CONTRACTION) JOINT
CLR	CLEAR
COL	COLUMN
CONC	CONCRETE
CONC BLK	CONCRETE BLOCK
CONT	CONTINUOUS
DET	DETAIL
DIA; Ø	DIAMETER
DL	DOUBLE ANGLE
EA	EACH
EL	ELEVATION
EQ	EQUAL
EJ	EXPANSION (ISOLATION) JOINT
EW	EACH WAY
FH	FLIPPED HORIZONTALLY
FV	FLIPPED VERTICALLY
FF	FINISH FLOOR
FT	FOOT
GRT	GIRT
HP	HIGH POINT
HT	HEIGHT
JT	JOINT
LP	LOW POINT
MAINT	MAINTENANCE
MATL	MATERIAL
MET	METAL
MFR	MANUFACTURER
MIN	MINIMUM
M.O.	MASONRY OPENING
NTS	NOT TO SCALE
O.C.	ON CENTER
OH	OVERHEAD
PL	PLATE
REINF	REINFORCED
REQ	REQUIRED
R.O.	ROUGH OPENING
SECT	SECTION
SIM	SIMILAR
SP	SPECIAL
SQ	SQUARE
STRUCT	STRUCTURAL
T & B	TOP AND BOTTOM
TOC	TOP OF CONCRETE
TOCh	TOP OF CHANNEL
TOB	TOP OF BEAM
TOG	TOP OF GRATE
TOJ	TOP OF JOIST
TOKW	TOP OF KNEEWALL
TOS	TOP OF STEEL
TS	STRUCTURAL STEEL TUBING
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
UOS	UNLESS OTHERWISE SHOWN
W/	WITH
WWR	WELDED WIRE REINFORCEMENT

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DRAWING TITLE:
STRUCTURES GENERAL NOTES & ABBREVIATIONS

PROJECT NO. -

DRAWING NO. -

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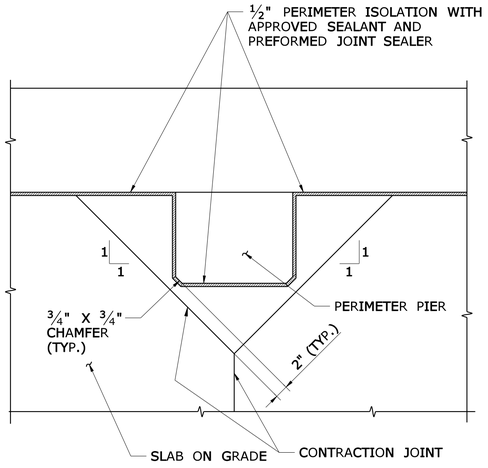

ISOLATION AND CONTRACTION JOINTS AT PERIMETER PIERS

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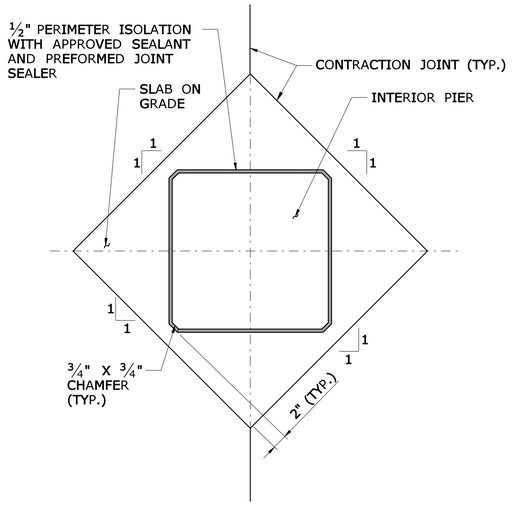

ISOLATION AND CONTRACTION JOINTS AT INTERIOR PIERS

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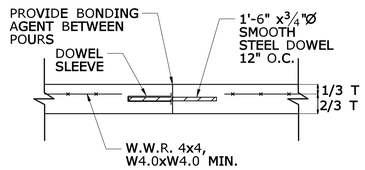

NOTES:
1) CONSTRUCTION JOINT SHALL BE FORMED FULL DEPTH OF SLAB.
2) DOWELS OR DOWEL SLEEVES SHALL BE INSTALLED AND SECURED AGAINST' DISPLACEMENT PRIOR TO POUR. DOWELS SHALL NOT BE INSTALLED AFTER POUR.
4) DOWEL SLEEVES SHALL BE USED ON ONE SIDE OF JOINT WITH OTHER SIDE EXPOSED TO CONCRETE.

SLAB CONSTRUCTION JOINT

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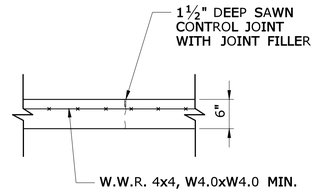

SLAB CONTROL JOINT

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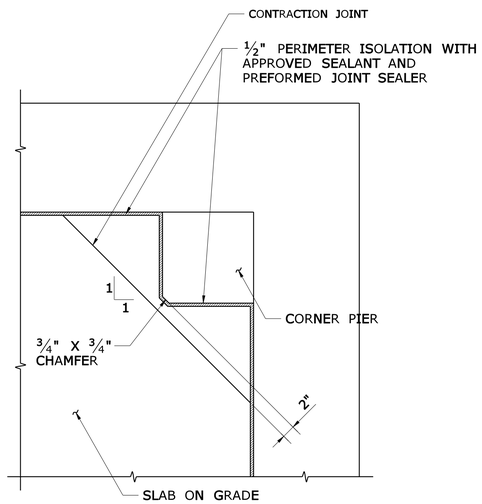
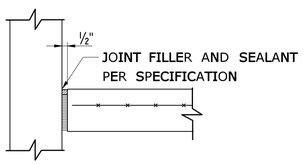


ISOLATION JOINT AT CORNER PIERS



TYP. SLAB ON GRADE ISOLATION JOINT

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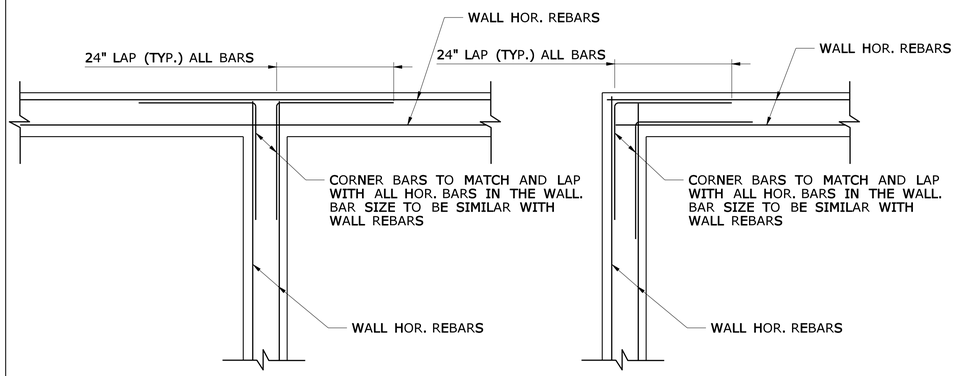

TYP. CONCRETE CORNER DETAIL

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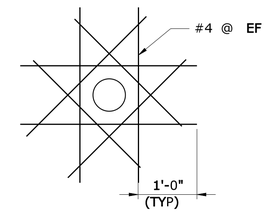

ADDITIONAL REINFORCEMENT AT WALL PENETRATION (TYP.)

N.T.S.

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DRAWING TITLE: MISCELLANEOUS CONCRETE DETAILS

PROJECT NO. -

DRAWING NO. -

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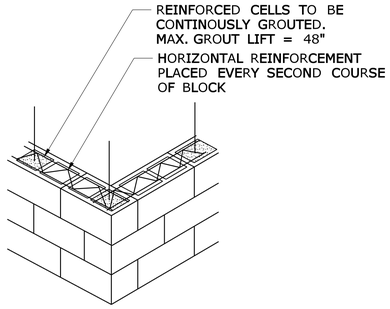
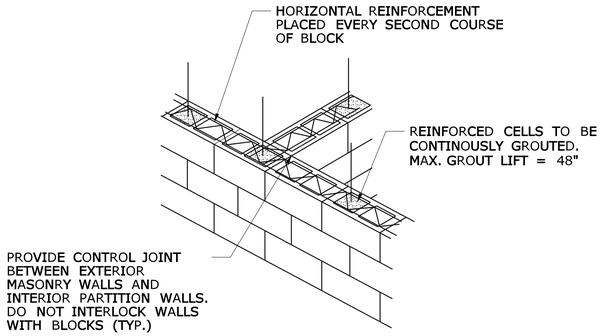


CMU WALL TYP.
CORNER REINFORCEMENT
N.T. SCALE



CMU WALL TYP.
INTERSECTION REINFORCEMENT
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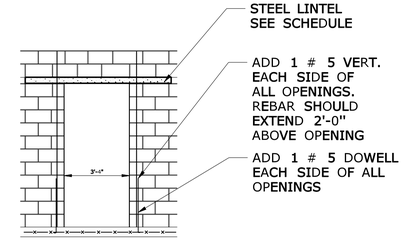
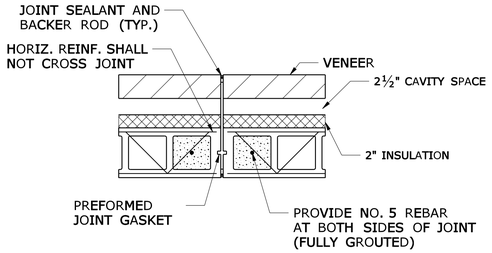
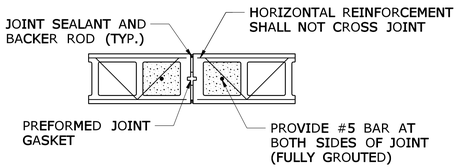


INTERIOR MASONRY
WALL OPENING
SCALE 1"=1'-0"



NOTE: CONTROL JOINTS SHALL COINCIDE WITH THE CENTER LINE OF STEEL COLUMNS.

TYP. EXTERIOR CONTROL JOINT
SCALE 1"=1'-0"



TYP. INTERIOR CONTROL JOINT
SCALE 1"=1'-0"

LOOSE LINTEL SCHEDULE
FOR NON-LOAD BEARING MASONRY WALLS

MASONRY OPENING	ANGLE SIZE	BEARING EACH END
3'-0" OR LESS	$3\frac{1}{2}$" x $3\frac{1}{2}$" x $\frac{5}{16}$"	8"
OVER 3'-0" TO 6'-0"	5" x $3\frac{1}{2}$" x $\frac{5}{16}$"	8"
OVER 6'-0" TO 8'-0"	6" x $3\frac{1}{2}$" x $\frac{5}{16}$"	8"

1. CONTRACTOR SHALL SUPPLY LOOSE ANGLE LINTELS OVER ALL MASONRY OPENINGS AND RECESSES U.N.O. LINTELS NOT SCHEDULED ON DRAWINGS SHALL CONSIST OF SINGLE ANGLE WITH $3\frac{1}{2}$INCH LEG HORIZONTAL FOR EACH 4 INCH OF WALL THICKNESS, ANGLES SHALL BE AS FOLLOWS:
2. EXTERIOR LINTELS SHALL BE GALVANIZED.
3. ALL DOUBLE ANGLES OVER 5'-0" LONG ARE TO BE BOLTED @ 3'-0" O.C.
4. ALL ANGLES LONG LEG VERTICAL, UON.
5. PROVIDE MIN. 6" BEARING ON BRICK OR SOLID CONCRETE BLOCK.
6. PROVIDE MIN.8" x WALL THICKNESS x 8" HIGH GROUTED CMU OR BRICK BEARING PAD UNDER ALL LINTELS, UON. GROUT JAMBS OF MASONRY OPENINGS 6'-0' OR LARGER FULL HEIGHT FOR 8" MIN. WIDTH.
7. AT OPENINGS FOR 6" CMU USE PLT. 5" X 1" WITH BEARING OF MIN. OF 8" AT EACH END

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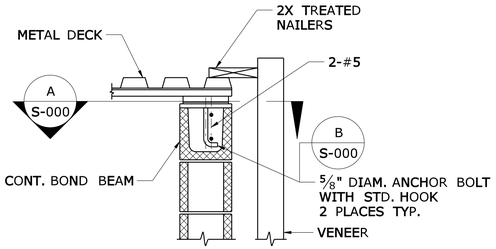
TOWN: -

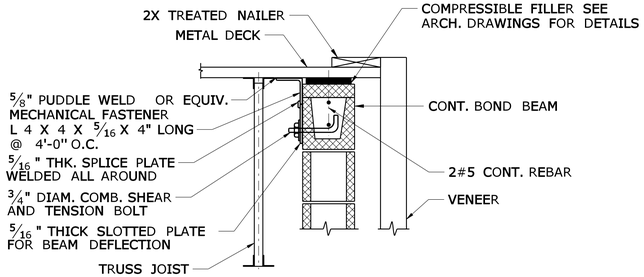
DRAWING TITLE: **MASONRY DETAILS**

PROJECT NO. -
DRAWING NO. -
SHEET NO. 4



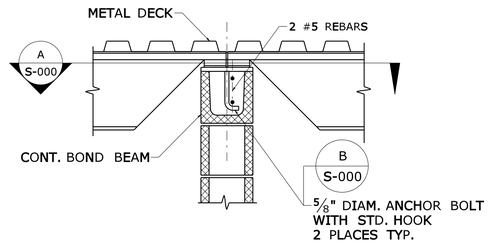
ROOF TRUSS JOIST PERPENDICULAR TO C.M.U. PERIMETER WALL (TYP.)

SCALE 1"=1'-0"



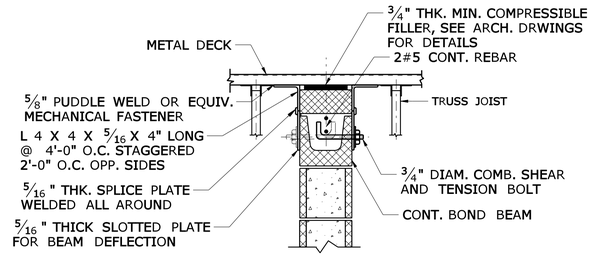
ROOF TRUSS JOIST PARALLEL TO C.M.U. PERIMETER WALL (TYP.)

SCALE 1"=1'-0"



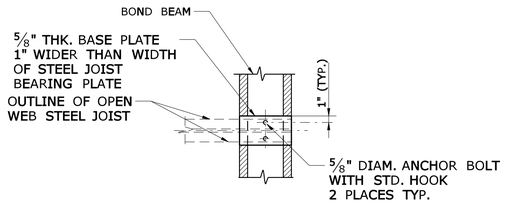
ROOF TRUSS JOIST PERPENDICULAR TO C.M.U. INTERIOR WALL (TYP.)

SCALE 1"=1'-0"



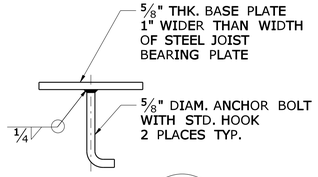
ROOF TRUSS JOIST PARALLEL TO C.M.U. INTERIOR WALL (TYP.)

SCALE 1"=1'-0"



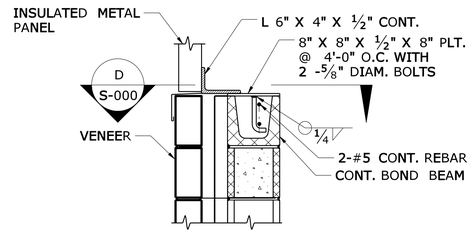
SECTION (TYP.) **A** / **S-000**

SCALE 1"=1'-0"



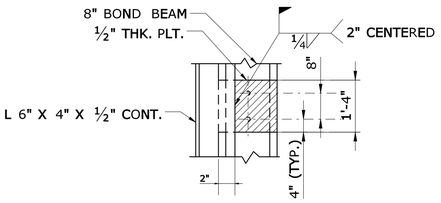
DETAIL **B** / **S-000**

SCALE 2"=1'-0"



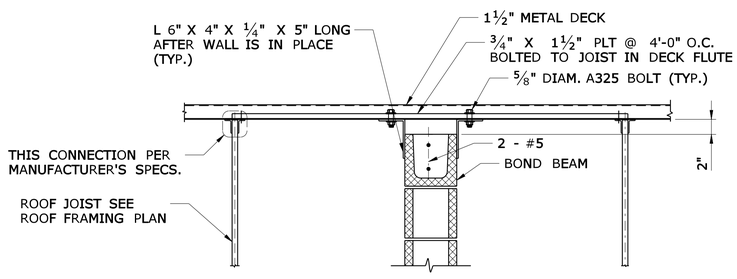
SECTION-METAL PANEL CONNECTION TO C.M.U. WALL (TYP.) **XX** / **S-000**

SCALE 1"=1'-0"



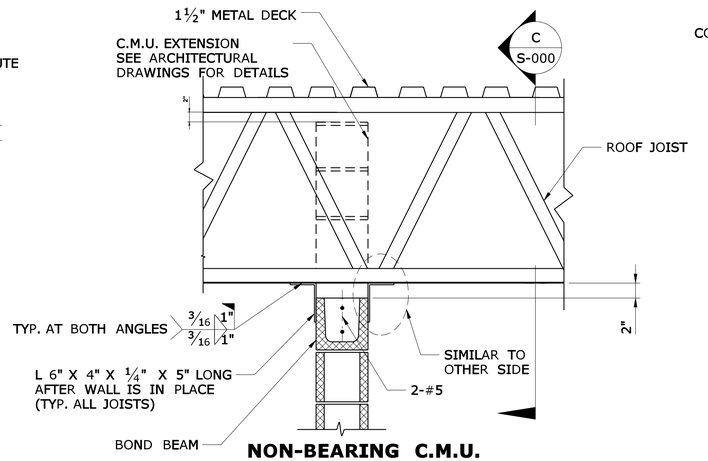
PLAN **D** / **S-000**

SCALE 1/2"=1'-0"



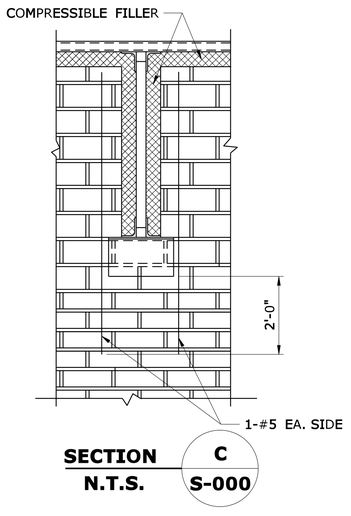
NON-BEARING C.M.U. BRACING PARALLEL TO JOIST

N.T.S.



NON-BEARING C.M.U. BRACING PERPENDICULAR TO TRUSS JOIST

N.T.S.



SECTION **C** / **S-000**

N.T.S.

REV.	DATE	REVISION DESCRIPTION	SHEET NO.
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-
-	-	-	-

THE INFORMATION, INCLUDING ESTIMATED QUANTITIES OF WORK, SHOWN ON THESE SHEETS IS BASED ON LIMITED INVESTIGATIONS BY THE STATE AND IS IN NO WAY WARRANTED TO INDICATE THE CONDITIONS OF ACTUAL QUANTITIES OF WORK WHICH WILL BE REQUIRED.

Plotted Date: 9/11/2009

DESIGNER/DRAFTER: -

CHECKED BY: -

SCALE AS NOTED

STATE OF CONNECTICUT
DEPARTMENT OF TRANSPORTATION

Filename: ...\CTDOT_STRUCTURAL_GD.dgn

SIGNATURE/ BLOCK:
OFFICE OF ENGINEERING

APPROVED BY: DATE:

PROJECT TITLE: -

TOWN: -

DRAWING TITLE: ROOF DIAPHRAGM TO MASONRY WALL DETAILS

PROJECT NO. -

DRAWING NO. -

SHEET NO. 5