STATE OF CONNECTICUT

DEPARTMENT OF TRANSPORTATION

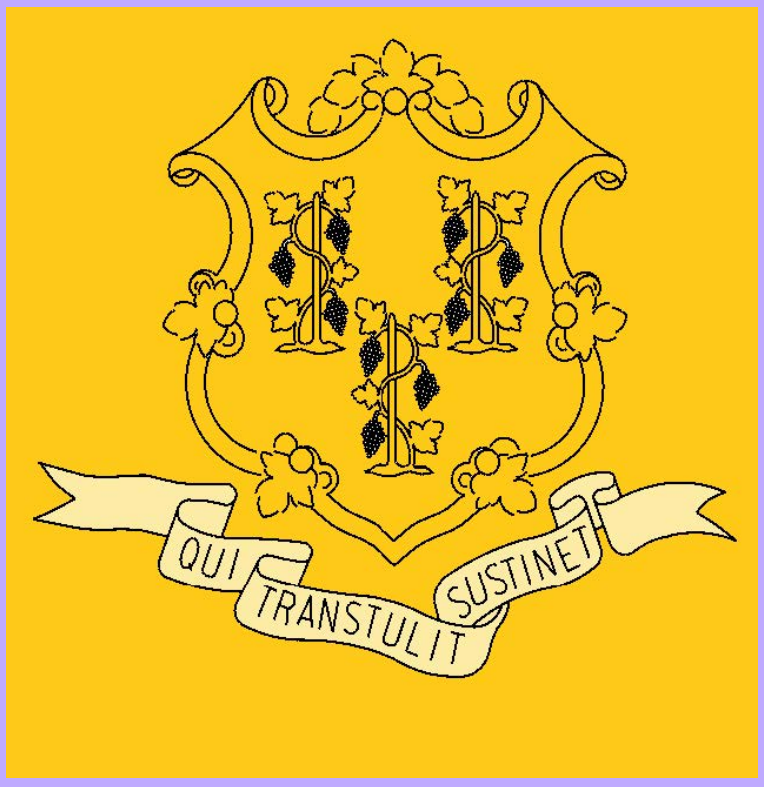
THE STANDARD SPECIFICATIONS

FOR

ROADS, BRIDGES, FACILITIES

AND

INCIDENTAL CONSTRUCTION



FORM 818

2020

CONNECTICUT DEPARTMENT OF TRANSPORTATION

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These Connecticut Department of Transportation *Standard Specifications for Roads, Bridges, Facilities, and Incidental Construction*, Form 818 dated January 2020, are hereby approved for application on highway and related construction contracts as referenced in the Contract plans or Special Provisions, and they shall apply as noted and amended by those documents. These electronic *Standard Specifications* and any Supplements are adopted as the official documents. Printed copies are not considered official and will no longer be provided.

I hereby certify that these *Standard Specifications* were prepared by me or under my responsible charge, compiled from specifications prepared, examined, adopted and implemented by the Connecticut Department of Transportation in accordance with established procedures, and have been approved by the Chief Engineer Mark D. Rolfe and the Federal Highway Administration.

Janet L. Mazeau, P.E.

CTDOT Specifications Engineer

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STANDARD SPECIFICATIONS FOR ROADS, BRIDGES, FACILITIES AND INCIDENTAL CONSTRUCTION FORM 818

These Standard Specifications – Form 818, in a searchable document format, may be accessed on the Department's internet site:
Click on "Doing Business with CTDOT" under "Publications" choose "Specifications for Roads, Bridges, Facilities & Incidental Construction" from the list, then click on "Form 818" at:

https://portal.ct.gov/DOT

NOTE: Adobe Reader is recommended.

ITEM NUMBERING SYSTEM

Item numbers, appearing in the Contract for roadway, bridge, facilities, and incidental construction consist of a 7-digit number which may be followed by a 2-digit decimal extension. The first 4 digits denote the Section number of the Standard Specification, Supplemental Specification, or Special Provision under which the item is to be constructed. The remaining digits are for Departmental coding purposes.

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SECTION 1.01
DEFINITION OF TERMS AND PERMISSIBLE ABBREVIATIONS

1.01.01—Definitions
1.01.02—Abbreviations, Publications and Standards
1.01.03—Abbreviations and Terms

1.01.01—Definitions: In these specifications, unless the context requires otherwise, words of the masculine gender include the feminine and the neuter, and, when the sense so indicates, words of the neuter gender may refer to any gender. Where appropriate, words in the singular form shall be deemed to include the plural, and words in the plural form to include the singular.

ADDENDUM: Contract revisions developed and incorporated into the contract after bid advertisement and before the opening of bid proposals.

AIR OPERATIONS AREA: Any paved or unpaved area of the airport used or intended to be used for the unobstructed movement of aircraft. These movements shall include landings, takeoffs, and surface maneuverings.

AWARD: The Department's acceptance in writing of the proposal of the lowest responsible bidder for the work, subject to the execution and approval by the Department of a contract therefor and the provision by the bidder of performance and payment bonds to secure the performance thereof which are acceptable to the Commissioner, and to such other conditions as may be specified by the Department or required by law.

BID: The submission of a proposal for the work contemplated.

BID ADVERTISEMENT: A public announcement requesting bids for a contract for work to be performed or materials to be furnished.

BIDDER: An individual or legal entity submitting a proposal in response to an advertised request.

BID MANUAL: "The State of Connecticut Department of Transportation Construction Contract Bidding and Award Manual," copies of which are available from the Department's Division of Contracts and at the following link: Construction Contract Bidding and Award Manual

CALENDAR DAY: Every day shown on the calendar, beginning and ending at midnight.

CATALOG CUT (PRODUCT DATA): Document(s) with information such as manufacturer's product specifications, manufacturer's installation instructions, standard color charts, wiring diagrams showing factory-installed wiring, printed performance curves and operational range diagrams. Product data that must be specially prepared because standard printed data is not suitable shall be considered shop drawings.

CERTIFICATE OF COMPLIANCE: The formal document issued at the completion of a project by the State Building Inspector's representative. The document is often referred to informally as a "Certificate of Occupancy," "C.O.C." or "C.O."

CHANNEL: A channel shall be interpreted to mean a natural or artificial watercourse having an average width at the bottom, after excavation, of 4 feet or more.

COMMISSIONER: State of Connecticut Transportation Commissioner acting directly or through a duly-authorized representative.

CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL: This Department of Energy and Environmental Protection (DEEP) Bulletin is intended to provide information to government agencies and the public on soil erosion and sediment control.

CONNECTICUT STORMWATER QUALITY MANUAL: This DEEP publication provides guidance on measures necessary to protect waters of the State from adverse impacts of post-construction stormwater runoff.

CONSTRUCTION ORDER, CHANGE ORDER: A written order signed by the Engineer for a contractor to perform work or provide supplies stipulated therein at the price or upon the basis of payment set forth therein.

CONTRACT: The agreement covering the performance of the work and the furnishing of materials required for the construction of the Project. The Contract shall be deemed to include the "Plans," "Specifications" (*i.e.*, the Department's "Standard Specifications for Roads, Bridges, Facilities and Incidental Construction" which is in effect on the date of the Bid Advertisement), "Construction Orders," and such other provisions as may be incorporated into the agreement, in addition to the contents of the bound contract containing the schedule of prices, signature sheet, addenda, special provisions, required federal and State provisions, supplemental specifications, labor and wage schedules, permits and other such material.

CONTRACTOR: Individual or legal entity contracting with the Department to perform the work. When the word is capitalized, it refers to the party of the second part to the Contract, acting directly or through its agents or employees. When this word is not capitalized, it is to be taken in its more general sense.
CONTRACT PAY ITEM: Specific work unit for which the Contract provides a price.
COUNTY: The largest State administrative division used to designate or identify the location of the proposed work.
CULVERT: A covered channel or a large pipe for carrying a watercourse below ground level, usually under a road or railway.
DEPARTMENT: State of Connecticut Department of Transportation.
DESIGNER: A duly-authorized representative of the Engineer, responsible for the design of the Project.
DRAINAGE DITCH: A paved or unpaved, artificially-constructed open depression having an average width of less than 4 feet at the bottom, after excavation, constructed for the purpose of carrying off surface water.
ENGINEER: The Commissioner or Deputy Transportation Commissioner, acting directly or through a duly-authorized representative.
EQUAL: A material, device, type of equipment, or method other than what is specified in the Contract, which is a recognized equivalent in substance and function for that specified thing, taking into account warranty, performance, weight, size, visual effect, special features and requirements indicated, quality, workmanship, economy of operation, durability, and suitability for purposes intended, provided that the proposed equivalent would not require or constitute a change in Contract work.
EXECUTION OF CONTRACT: The date of execution of the Contract by the Department is the date on which the Department's authorized signatory signs the Contract on behalf of the Department.
HIGHWAY: A general term denoting a public way used for vehicular travel. When referred to in the Contract, it signifies the whole right of way reserved for or secured by the Department for use in constructing or maintaining a roadway and its appurtenances.
INSPECTOR: A duly-authorized representative of the Engineer, assigned to make inspections of the work performed and associated materials.
LABORATORY: Department testing facility or other designated testing laboratory**.**
LIQUIDATED DAMAGES: The amount prescribed in the Contract specifications, to be paid to the State or to be deducted from any payments due or to become due the Contractor, for a specified time unit delay in completing the whole or any specified portion of the work beyond the time allowed in the Contract.
MAJOR ITEM: An individual Contract item, whose value at the time of bidding (either lump sum price or the product of its unit price multiplied by its estimate quantity) is equal to or greater than 10% of the total original Contract bid price shall be considered a Major Item.
MANAGER OF CONTRACTS: The Transportation Manager of Contracts, who is the head of the Department's Division of Contracts, at email address DOTContracts@ct.gov and whose office is located at the headquarters of the Department at 2800 Berlin Turnpike, Newington, CT, 06111.
MATERIAL: Any substance specified in the Contract for use in the construction of the Project, including appurtenances of products that are substantially shaped, cut, worked, mixed, finished, refined or otherwise fabricated, processed, or installed in order to be used for the Project work or become part of the constructed Project.
MILESTONE: An identified point of progress in the timeline of the Project.
MINOR ITEM: An individual Contract item that is not a Major Item.
MUNICIPALITY: City, town or county.
NOTICE TO PROCEED: Written direction to the Contractor stipulating the date for beginning the Work subject to other Contract requirements, including the date from which project time will be charged.
OWNER: Where used herein, it is synonymous with Department or State.
PAVEMENT STRUCTURE: The combination of sub-base, base course and surface course placed on subgrade to support and distribute the traffic load.
PLANS: Contract drawings showing location, type, dimensions, and details of specified work. This includes Standard Sheets.
PRODUCT DATA (CATALOG CUT): Document(s) with information such as manufacturer's product specifications, manufacturer's installation instructions, standard color charts, wiring diagrams showing factory-installed wiring, printed performance curves and operational range diagrams. Product data that must be specially prepared because standard printed data is not suitable shall be considered shop drawings.
PROJECT: All work included under one Department Contract. The Department may use multiple project

numbers for the work included within one Contract.

PROJECT SITE (*or* SITE): The space available to the Contractor, under the Contract, for performing construction activities. The extent of the Project site is as indicated on the plans or elsewhere in the Contract.

QUALIFIED PRODUCTS LIST (QPL): A report that has been developed as a means for determining what products, suppliers, manufacturers, equipment and methodologies may be used on construction projects. This report is located on the CT Department of Transportation Website:
Connecticut DOT Qualified Products List Report

RECLAIMED CONCRETE AGGREGATE: Reclaimed waste consisting of crushed and graded concrete removed from pavements, structures, or buildings. Metal may be acceptable only where it is contained as reinforcement within small fragments of concrete; *e.g.*, metal projecting from concrete fragments would be unacceptable. All such material trucked from beyond the limits of the Project must be accompanied by a Materials Certificate and Certified Test Report indicating that the material is environmentally acceptable and structurally sound in accordance with 1.06.07, unless the source of the material is a Department Project and that source is acceptable to the Engineer.

RECLAIMED MISCELLANEOUS AGGREGATE: Glass-free and clinker-free reclaimed waste, that has been crushed, graded and blended, as specified in the Contract, with natural crushed stone or gravel. Metal may be acceptable only where it is contained as reinforcement within small fragments of concrete; *e.g.*, metal projecting from concrete fragments would be unacceptable. All such material trucked from beyond the limits of the Project must be accompanied by a Materials Certificate and Certified Test Report indicating that the material is environmentally acceptable and structurally sound in accordance with 1.06.07, unless the source of the material is a Department Project and that source is acceptable to the Engineer.

RECLAIMED WASTE: Debris from the demolition of buildings, structures, and pavements; residue from incineration and recycled glass. Acceptable material shall include concrete, bituminous concrete, glass, ceramics, brick, pavement sub-base and base courses, and clinker from resource recovery plants. Metal may be acceptable only when it is contained within large fragments of concrete. Reclaimed waste trucked from beyond the limits of the Project must be accompanied by a Materials Certificate and Certified Test Report indicating that the waste is environmentally acceptable and structurally sound in accordance with 1.06.07, unless the source of the material is a Department Project and that source is acceptable to the Engineer.

RESOURCES: The labor, equipment, and material necessary to perform work on a Contract bid item or other element of work.

RIGHT-OF-WAY: Land, property, or interest therein acquired for or devoted to transportation purposes.

ROADBED: The graded portion of a highway, including portions within the top and side slopes, that has been prepared as a foundation for the pavement structure and shoulders.

ROADWAY: The portion of the highway, including shoulders, which may be used for vehicular travel within the Project limits.

SHOP DRAWINGS: Drawings, including proposed details, diagrams, schedules, procedures and other supporting data, prepared by a Contractor to supplement the Contract documents, showing all information necessary for fabrication of items for which some specific design or detail appears in the Contract.

SHOULDER: The portion of the roadway adjacent to the Traveled Way, that can accommodate stopped vehicles for emergency use, and that provides lateral support of base and surface courses.

SPECIFICATIONS: The Department's written provisions and requirements for the performance of the Contract, contained in or incorporated by the Contract.

A. *Standard Specifications*—A set of specifications approved by the Department for general application and repetitive use, entitled the "Standard Specifications for Roads, Bridges, Facilities and Incidental Construction" found at the following link:
Standard Specifications for Roads, Bridges, Facilities and Incidental Construction
B. *Supplemental Specifications*—Approved additions to and revisions of the Standard Specifications.
C. *Special Provisions*—Other Department specifications applicable to an individual project.

STATE: State of Connecticut.

STANDARD SHEETS: Standardized plans containing details approved by the Department and the FHWA, for construction of a given type on any project, included in contracts on an as-needed basis.

SUBCONTRACTOR: Any individual, firm, partnership or corporation to which the Contractor sublets, with the approval of the Commissioner, any part or parts of the Project covered by the Contract.

SUBSTANTIAL COMPLETION: The date at which the performance of all work on the Project has been completed except minor or incidental items, final cleanup, work required under a warranty and repair of unacceptable work, and provided the Engineer has determined that:

A. The Project is safe and convenient for use by the public, and
B. All traffic lanes including all safety appurtenances are in their final configuration, and
C. Failure to complete the work and repairs excepted above does not result in the deterioration of other completed work, and provided further, that the value of work remaining to be performed, and cleanup is less than one percent (1%) of the estimated final Contract amount, and
D. If applicable a Certificate of Compliance has been issued.

SUBSTITUTE: A replacement for a specified material, device, type of equipment, or method, which is sufficiently different in substance and function, quality, or workmanship to constitute a change in the Contract work.

SUBSTRUCTURE: All of that part of the bridge below the bearings of simple and continuous spans, skewbacks of arches and tops of footings of rigid frames, including backwalls, wingwalls and any protective railings mounted on the wingwalls.

SUB-SUBCONTRACTOR: Any individual, firm, partnership or corporation to which a subcontractor sublets, with the approval of the Commissioner, any part or parts of the Project covered by the Contract.

SUPERSTRUCTURE: The entire bridge except the substructure.

TRAVELED WAY: Portion of the right-of-way designated for vehicle use, excluding shoulders.

UTILITY: Any public service company and the plant of such a company or similar facilities. Such companies include, but are not limited to, companies selling or controlling the sale, distribution or use of water, gas, electricity, communications systems, sewers and railroad lines. Such facilities include, but are not limited to, wires, cables, ducts, pipes, manholes, transformers, poles, towers and tracks.

WATERCOURSE: Rivers, streams, brooks, waterways, lakes, ponds, marshes, swamps, bogs and all other bodies of water, natural or artificial, vernal or intermittent, public or private, which are contained within, flow through or border upon this state.

WORK: The provision of labor, materials or services necessary for or relating to the design and construction of the Project.

WORKING DRAWINGS: Drawings, calculations, procedures and other supporting data prepared by a Contractor, documenting the Contractor's proposed design, details, materials, construction methods and equipment for any construction for which no specific design or detail appears in the Contract.

1.01.02—Abbreviations, Publications and Standards: For publications and standards, the edition governing the Project work will be the edition in effect on the date the Contract was advertised for solicitation of bids. Whenever one of the following abbreviations is used in the Contract, its meaning shall be interpreted as follows:

AA—(The) Aluminum Association, Inc.
AABC—Associated Air Balance Council
AAMA—American Architectural Manufacturers Association
AAN—American Association of Nurserymen
AAPA—American Association of Port Authorities
AASHTO—American Association of State Highway and Transportation Officials

NOTE: Wherever reference is made to an AASHTO Standard Method of Test or Standard Specification, it refers by letter and number to the method or specification published by AASHTO in the "Standard Specifications for Transportation Materials and Methods of Sampling and Testing."

ABMA—American Bearing Manufacturers Association
ACGIH—American Council of Government Industrial Hygienists
ACI—ACI International (American Concrete Institute)
ACOE—Army Corps of Engineers
ADA—Americans with Disabilities Act
ADAAG—Americans with Disabilities Act Accessibility Guidelines for Buildings and Facilities
ADSC—The International Association of Foundation Drilling (formerly Association of Drilled Shaft Contractors)
AF&PA—American Forest & Paper Association
AGA—American Gas Association

AGC—(The) Associated General Contractors of America
AHA—American Hardboard Association
AHAM—Association of Home Appliance Manufacturers
AI—Asphalt Institute
AIA—(The) American Institute of Architects
AISC—American Institute of Steel Construction
AISI—American Iron and Steel Institute
AITC—American Institute of Timber Construction
A.L.I.—Automotive Lift Institute
ALSC—American Lumber Standard Committee, Incorporated
AMCA—Air Movement and Control Association International, Inc.
AMRL—AASHTO Materials Reference Library
ANLA—American Nursery and Landscape Association
ANSI—American National Standards Institute
AOAC—AOAC International
AOSA—Association of Official Seed Analysts
APA—APA-The Engineered Wood Association
API—American Petroleum Institute
AREMA—American Railway Engineering and Maintenance-of-Way Association
ARI—Air-Conditioning & Refrigeration Institute
ARTBA—American Road and Transportation Builders Association
ASA—Acoustical Society of America
ASC—Adhesive and Sealant Council
ASCE—American Society of Civil Engineers
ASHRAE—American Society of Heating, Refrigerating and Air-Conditioning Engineers
ASME—ASME International (The American Society of Mechanical Engineers International)
ASNT—American Society for Non-Destructive Testing
ASSE—American Society of Sanitary Engineering
ASTM—American Society of Testing and Materials (ASTM International)
NOTE: Wherever reference is made to an ASTM specification, test method, or practice, it refers by letter, number, or both to standards published by ASTM International in the "ASTM Standards Source™ Database."
ATSSA—American Traffic Safety Services Association
AWI—Architectural Woodwork Institute
AWPA—American Wood Protection Association (*formerly* American Wood-Preservers' Association)
AWPI—American Wood Preservers Institute
AWS—American Welding Society
NOTE: Wherever reference is made to an AWS materials specification, inspection methods, or welding procedures, it refers by section number to standards of the American Welding Society published in the applicable steel, or aluminum welding code.
AWWA—American Water Works Association
BHMA—Builders Hardware Manufacturers Association
BIA—(The) Brick Industry Association
BOCA—BOCA International, Inc.
CBM—Certified Ballast Manufacturers Association
CCRL—Cement and Concrete Reference Laboratory
CDA—Copper Development Association (The)
CFR—Code of Federal Regulations
CGA—Compressed Gas Association
CGS—Connecticut General Statutes (as revised)
CISCA—Ceilings and Interior Systems Construction Association
CISPI—Cast Iron Soil Pipe Institute
CLFMI—Chain Link Fence Manufacturers Institute
ConnDOT—Connecticut Department of Transportation
CRI—(The) Carpet and Rug Institute
CRSI—Concrete Reinforcing Steel Institute

CSI—(The) Construction Specifications Institute
CSSB—Cedar Shake & Shingle Bureau
CTDOT—Connecticut Department of Transportation
CTI—Cooling Technology Institute
DASMA—Door and Access Systems Manufacturers Association, International
DEEP—Connecticut Department of Energy and Environmental Protection
~~DEP—Connecticut Department of Environmental Protection~~ *see* DEEP
DHI—Door and Hardware Institute
DOD—Department of Defense Military Specifications and Standards
~~DPUC—Department of Public Utility Control~~ *see* PURA
EIA—Electronic Industries Alliance
EPA—Environmental Protection Agency
FAA—Federal Aviation Administration
FCC—Federal Communications Commission
FCICA—Floor Covering Installation Contractors Association
FHWA—Federal Highway Administration
FMG—FM Global
FRA—Federal Railway Administration
FS—Federal Specification

NOTE: Wherever reference is made to FS in the contract, it refers by number, letter, or both, to the latest standard or tentative standard of the Federal Specification Unit, General Services Administration, Federal Supply Service, as to materials, specifications, or methods of testing, whichever the case may be.

FTA—Federal Transit Administration
GA—Gypsum Association
GANA—Glass Association of North America
GSA—General Services Administration
HI—Hydraulics Institute
HPVA—Hardwood Plywood & Veneer Association
IAS—International Accreditation Service
ICC—International Code Council
ICC-ES—ICC Evaluation Service, Inc.
ICEA—Insulated Cable Engineers Association, Inc.
ICRI—International Concrete Repair Institute
IEC—International Electrotechnical Commission
IEEE—(The) Institute of Electrical and Electronics Engineers, Inc.
IES—Illuminating Engineers Society
IESNA—Illuminating Engineering Society of North America
IGCC—Insulating Glass Certification Council
IGMA—Insulating Glass Manufacturers Alliance
IMSA—International Municipal Signal Association
IRI—HSB Industrial Risk Insurers
ISO—International Organization for Standardization
ITE—Institute of Traffic Engineers
ITSNA—Intertek Testing Services, Inc. (*formerly* Edison Testing Laboratories)
IUPAT—International Union of Painters and Allied Trades
KCMA—Kitchen Cabinet Manufacturers Association
LMA—Laminating Materials Association
LPI—Lightning Protection Institute
LWRD—Land and Water Resources Division
MASH—Manual for Assessing Safety Hardware
MBMA—Metal Building Manufacturers Association
MILSPEC—Military Specification and Standards
MMA—Monorail Manufacturers Association
MSHA—Mine Safety and Health Administration
MSS—Manufacturers Standardization Society of The Valve and Fittings Industry, Inc.

MUTCD—Manual on Uniform Traffic Control Devices
NAAMM—National Association of Architectural Metal Manufacturers
NACE—National Association of Corrosion Engineers
NADCA—National Air Duct Cleaners Association
NAIMA—(The) North American Insulation Manufacturers Association
NBFU—National Board of Fire Underwriters
NCHRP—National Cooperative Highway Research Program
NCMA—National Concrete Masonry Association
NCPI—National Clay Pipe Institute
NCSPA—National Corrugated Steel Pipe Association
NEAUPG—NorthEast Asphalt User/Producer Group
NEBB—Natural Environmental Balancing Bureau
NEC—National Electrical Code
NECA—National Electrical Contractors Association
NEMA—National Electrical Manufacturers Association
NEPCOAT—North East Protective Coatings Committee
NESC—National Electrical Safety Code
NETA—InterNational Testing Association
NETTCP—NorthEast Transportation Technician Certification Program
NFPA—National Fire Protection Association
NFRC—National Fenestration Rating Council
NHLA—National Hardwood Lumber Association
NICET—National Institute for Certification in Engineering Technologies
NIOSH—National Institute of Occupational Safety and Health
NIST—National Institute of Standards and Technology
NLGA—National Lumber Grades Authority
NMFS–National Marine Fisheries Service
NOAA—National Oceanic and Atmospheric Administration
NRCA—National Roofing Contractors Association
NRMCA—National Ready-Mixed Concrete Association
NSBA—National Steel Bridge Alliance
NSC—National Safety Council
NSF—NSF International
NTMA—National Terrazzo and Mosaic Association, Inc.
OEO—Office of Equal Opportunity
OSHA—Occupational Safety and Health Administration
PCA—Portland Cement Association
PCI—Precast/Prestressed Concrete Institute
PDI—Plumbing & Drainage Institute
PROWAG—Public Right-of-Way Accessibility Guidelines
PTI—Post-Tensioning Institute
PURA—Public Utilities Regulatory Authority
RCSA—Regulations of Connecticut State Agencies
RMA—Rubber Manufacturers Association
SAE—SAE International (formerly Society of Automotive Engineers)
SDI—Steel Deck Institute *or*
.....—Steel Door Institute
SFPA—Southern Forest Products Association
SHRP—Strategic Highway Research Program
SJI—Steel Joist Institute
SMACNA—Sheet Metal and Air Conditioning Contractors National Association
SPIB—(The) Southern Pine Inspection Bureau
SPRI—Single Ply Roofing Institute
SSPC—The Society for Protective Coatings (*formerly* The Steel Structures Painting Council)

NOTE: Where reference is made to SSPC in the Contract, it refers by number, letter, or both, to the latest standard or tentative standard specification as to materials specifications, methods of testing,

systems, procedures, inspection or other specification pertaining to any or all phases of cleaning or painting, whichever may apply.

SWRI—Sealant, Waterproofing, & Restoration Institute
TCA—Tile Council of America, Inc.
TIA—Telecommunications Industry Association
TIA/EIA—Telecommunications Industry Association/Electronics Industries Alliance
TPI—Truss Plate Institute, Inc.
TRB—Transportation Research Board
UFAS—Uniform Federal Accessibility Standards
UL—Underwriters Laboratories Inc.
USACOE—United States Army Corps of Engineers
USCG—United States Coast Guard
USDA—United States Department of Agriculture
USFWS–United States Fish and Wildlife Service
USGBC—U.S. Green Building Council
USSWG—United States Steel Wire Gauge
WCLIB—West Coast Lumber Inspection Bureau
WCSC—Window Covering Safety Council
WDMA—Window & Door Manufacturers Association
WWPA—Western Wood Products Association

1.01.03—Abbreviations and Terms: Abbreviations and terms used in the Contract are in lieu of and are to be construed in the same way as are the terms or phrases following them in the list below. Those abbreviations and terms include, but are not necessarily limited to:

ABS—acrylonitrile butadiene styrene
AC—alternating current
ACCM Pipe or ACCMP—Asphalt-Coated Corrugated Metal Pipe
ACSR—Aluminum Conductor, Steel Reinforced
A.E.A.—Air Entraining Admixture
AIC—Ampere Interrupting Current
AOEC—Area of Environmental Concern
APA—Aquifer Protection Area
AWG—American Wire Gauge
B & B—balled and burlapped
bbl—barrel
BCPC—Bituminous Concrete Park Curbing
Bit.—bituminous
Bit. Conc.—bituminous concrete
CAPWAP—CAse Pile Wave Analysis Program
CAS—Coating Applicator Specialist
CB—catch basin
CCA—chromated copper arsenate
CCM Pipe or CCMP—coated corrugated metal pipe
CICU—controller interface communications unit
CLLCU—closed loop local coordination unit
CLMU—closed loop master unit
CMS—Changeable Message Sign
Conc.—concrete
CPE Pipe or CPEP—corrugated plastic or polyethylene pipe
CPS—centipoise second
CUF—Commercially Useful Function
CWI— Certified Welding Inspector
cwt.—hundredweight or 100 pounds
DBE—Disadvantaged Business Enterprise
DBH—Diameter at breast height
DC—direct current

dist.—distillation
DMT—Division of Materials Testing
DTI—Direct Tension Indicator
EW—endwall
est.—estimated
exc—excavation
fi—jacking tension
FRC—Fiberglass Reinforced Composite
f' c—specified minimum compressive strength at a specified age
f' ci—required strength at time of transfer
ga—gauge or gage
GFRP—glass fiber reinforced polymer
Gsa—Apparent specific gravity
Gsb—Bulk specific gravity
HASP—Health and Safety Plan
HMA—hot mix asphalt or bituminous concrete
HRB—Rockwell B Scale hardness
HRC—Rockwell C Scale hardness
Hz—hertz
IMC—intermediate metal conduit
IP—internet protocol
I.P.S.—iron pipe size
IWRC—independent wire rope core
JMF—job mix formula
KD—kiln dried
KDAT—kiln dried after treatment
kip—1000 pounds
ksf—kips per square foot
LCD—Liquid Crystal Display
LED—light-emitting diode
LID—Low Impact Development
LRFD—Load & Resistance Factor Design
l.s.—lump sum
mbf—1000-foot board measure
MBR—metal beam rail
Mgal—1000 gallons
MH—manhole
MLSI—Major Lump Sum Item
MS4—Municipal Separate Storm Sewer Systems
MPT—Maintenance and Protection of Traffic
N.C.—National Coarse
NDT—non-destructive testing
Pavt.—pavement
PCBC—precast concrete barrier curb
PCC—Portland Cement Concrete
PE—polyethylene
Perf. ACCM Pipe or Perf. ACCMP—Perforated Asphalt-Coated Corrugated Metal Pipe
Perf. CCM Pipe or Perf. CCMP—Perforated Coated Corrugated Metal Pipe
Perf. CPE Pipe or Perf. CPEP—Perforated Corrugated Plastic or Polyethylene Pipe
pfmd.—preformed
PROM—programmable read only memory
psf—pounds per square foot
psi—pounds per square inch
p/s—prestressed
PTFE—polytetrafluoroethylene
PVC—polyvinyl chloride

Pwa—Percent water absorbed
QCPFA—Quality Control Plan for Fine Aggregates
RAP—reclaimed asphalt pavement
RC—Reinforced Concrete
RCCE—Reinforced Concrete Culvert End
RC Pipe *or* RCP—Reinforced Concrete Pipe
RFC—Request for Change
RFI—Request for Information
RMC—Rigid Metal Conduit
SD—system detector
SDR—Standard Dimension Ratio (ratio of pipe diameter to wall thickness)
sec.—second
SMT—surface-mount technology
S.O.W.—Service cord (Oil resistant jacket, Weather/water resistant)
sol.—soluble
sp. gr.—specific gravity
sp. visc.—specific viscosity
SSA—Sole Source Aquifer
SSP—Subspecies
std.—standard
surf.—surface
TDC—Transportation Division Chief
THHN—Heat resistant thermoplastic, insulated nylon jacket, 90°C, 600 V building wire
THWN—Moisture and heat resistant thermoplastic, insulated nylon jacket, 75°C, 600 V building wire
TL—Test Level
TMA—Truck Mounted Impact Attenuator
TMP—Transportation Management Plan
tsf—tons per square foot
TTC—Temporary Traffic Control
U'drain *or* Udrain—Underdrain
UNC—Unified National Coarse
UNS—Unified Numbering System
USB—Universal Serial bus
UTCS—urban traffic control system
UV—ultra-violet or ultra violet light
VAC—Volts Alternating Current
VDC—Volts Direct Current
VECP—Value Engineering Change Proposal
VMS—Variable Message Sign
VOC—Volatile Organic Compound
VT—vitrified tile
W—watt
WSA—Temporary Waste Stockpile Area

SECTION 1.02
PROPOSAL REQUIREMENTS AND CONDITIONS

1.02.01—Contract Bidding and Award
1.02.02—Interpreting Bid Proposal Quantities
1.02.03—Examination of Contract Documents and Work Site
1.02.04—Knowledge of Applicable Laws

1.02.01—Contract Bidding and Award: All bids for construction contracts must be submitted electronically. It is the responsibility of each bidder and all other interested parties to obtain all bidding related information and documents from the Department of Administrative Services (DAS) State Contracting Portal.

Connecticut Department of Transportation bidding and other information and documents which are obtained from any other source must not be submitted to the Department. Reproduced, reformatted or altered forms of documents are not authorized or acceptable.

For information about the bidding and award of Department construction contracts, consult the "State of Connecticut Department of Transportation Construction Contract Bidding and Award Manual," available from the Division of Contracts. In order to be eligible for award of a Department construction contract, a bidder must follow the requirements of this Bid Manual, and all bidding and award matters regarding Department construction contracts shall be governed by the terms of the Bid Manual, unless treated otherwise in the Contract, including these Specifications.

1.02.02—Interpretation of Estimate: The quantities shown on the proposal form are approximate only and are given as a basis for the pricing upon which the award of the Contract will be made. The Department does not warrant that these quantities shall remain unchanged in the actual construction, and the Contractor may not plead misunderstanding or deception because of any variation between estimated and final quantities. The Engineer reserves the right to increase or decrease any or all of the quantities shown on the proposal form as may be necessary to properly complete the Project.

The Department will pay for the actual quantity of authorized and accepted work done or material furnished under each of the items.

1.02.03—Examination of Contract Documents and Work Site: The bidder is required to examine the site of the Contract work and the proposal form, plans, special provisions, specifications, supplemental specifications, Contract form and other Contract documents for the work contemplated, as well as any permits or permit applications that are likely to affect the Contract work. The bidder must judge for itself and satisfy itself as to the conditions to be encountered; the character, quality and quantities of the work to be performed; the materials to be furnished; and the requirements of the above documents, particularly the requirements under each Contract item, under the general cost of the work, or under other applicable, but more general, provisions, of the Contract.

The subsurface information furnished in the Contract is based on the interpretation, by the Department, of investigations made only at the specific locations indicated; and the Department gives no assurance that the conditions discovered are typical of the conditions at other Project site locations or that those conditions will have remained unchanged since the field data were obtained. The Department also gives no assurance that the presence or absence of subsurface water at the time and locations of these explorations will be representative of actual conditions at the time of construction. Such subsurface information as was obtained by the Department for its use in the design of the Project will be available for inspection by bidders through the Division of Contracts. Also, bidders may arrange through the Division of Contracts an opportunity to examine, in advance of bidding, at a location to be specified by the Department, any available samples of the materials encountered in the Department's subsurface explorations. The Contractor shall be solely responsible for all assumptions, deductions, or conclusions it may make or derive from its examination of any Department subsurface information, document or sample. In furnishing or making available such information, the Department makes no warranty or representation as to the actual conditions that may be encountered or actual quantities or distribution of quantities of work that will be required in the course of the Project.

The Department does not intend or warrant that plan sheets furnished to the State by utility companies whose facilities may be affected by the proposed construction will show all proposed utility work that will be done by utility companies or municipal authorities or both before, during, or after the life of this Contract. In addition to the work indicated on such plan sheets, the utility companies and authorities may make adjustments to or remove certain of their installations other than those indicated on the plans, or may

install facilities not so indicated.

Bidders must inform the Department in writing, at the earliest opportunity, of any and all omissions, errors, and/or discrepancies that the bidder discovers within or among the plans, specifications, and bidding documents. Information and inquiries concerning such matters, and any other information or inquiry concerning the conditions of bidding or award or the interpretation of contract documents, must be transmitted in writing to mailto:DOTContracts@ct.gov. The Department cannot ensure a response to inquiries received later than ten (10) days prior to the scheduled opening of the related bid. When the Department deems it warranted, responses to such inquiries that relate to changes in or interpretations of the Project documents (plans and specifications) will be issued to all bidders in the form of addenda and made a part of the Contract. Bidders are responsible for ensuring that they are aware of all addenda.

1.02.04—Knowledge of Applicable Laws: Bidders shall be deemed to know and understand all federal, state and local laws, ordinances and regulations and municipal bylaws which in any manner apply to projects for which they bid; such legal requirements shall include, but not necessarily be limited to, those which apply to the conduct of the Contract work, the equipment and materials to be used on the Project, or the treatment of individuals or classes of individuals in relationship to their involvement with the Project. A Contractor's ignorance of such requirements shall not, in any internal Department proceeding or in any claims or other legal proceeding, constitute justification for the Contractor's failure to consider such requirements in formulating a bid proposal, or for the Contractor's failure to ensure that such legal requirements are met with regard to any Department project in which that Contractor participates.

The Contractor agrees that if it should be awarded the contract for any project supported at least in part by federal funding, the Contractor will not knowingly enter into any lower-tier transaction on that project with a person (including entities) who, by virtue of federal law or regulation, or by voluntary agreement, is currently ineligible to participate in such a project, unless after disclosure of such ineligibility, such participation is authorized by appropriate federal and State authorities.

The Department expects the Contractor to obey municipal laws and regulations and cooperate with municipal officials. In some instances, however, municipal laws or regulations, or the orders of municipal officials, may conflict with necessary Project activities. In most such cases, the municipality does not have the legal power to enforce its laws and regulations upon the State or upon a State project. This is because the State is protected by its sovereign immunity. If local police or other authorities should attempt to stop the Contractor from carrying out activities that are necessary in order for the Contractor to comply with Contract requirements, the Contractor should politely inform the municipal authorities that they probably do not have jurisdiction over the State’s project, and the Contractor should immediately inform the Engineer of the attempted interference with Project activities. If the municipal authorities continue to insist upon preventing the Contractor from carrying out Project activities, the Contractor should not defy the authorities, but, to the extent possible, should await directions from the Engineer.

SECTION 1.03
AWARD AND EXECUTION OF CONTRACT

1.03.01—Consideration of Bids
1.03.02—Award and Execution of Contract
1.03.03—Return of Proposal Guaranty
1.03.04—Requirements of Performance Contract Bond and Payment Bond
1.03.05—Vacant
1.03.06—Failure to Execute Contract
1.03.07—Insurance
1.03.08—Notice to Proceed and Commencement of Work
1.03.09—Contractor Readiness

1.03.01—Consideration of Bids: See 1.02.01.

1.03.02—Award and Execution of Contract: Except as otherwise authorized by the Commissioner, all contracts will be awarded and executed in accordance with the order of the Commissioner. The award, if made, will be made within 60 days after the opening of the proposals unless otherwise agreed upon by the Commissioner, the successful bidder, and the surety. The successful bidder, upon receipt of notice from the Department that the contract is ready for execution, shall, at the time and place designated in said notice, be present in person or be represented by an official legally authorized to sign the Contract, and shall there and then sign the necessary Project contract with the State. No proposal shall be considered binding upon the State until the proper execution of the Contract by both parties.

1.03.03—Return of Proposal Guaranty: All proposal guaranties will be returned within 3 calendar days following the award of the Contract. Ten (10) calendar days after the opening of the proposals, all guaranties, except those of the 3 lowest bidders, will be returned. Should no award be made within 60 calendar days after the opening of proposals, the Commissioner may reject all proposals and return the proposal guaranties, except that with the approval of the lowest bidder and its surety, the Commissioner may extend the time for the award and may retain the proposal and proposal guaranty of the lowest bidder for said extended time, or for any other period of time agreed upon by the Commissioner, bidder and surety.

1.03.04—Requirements of Performance Contract Bond and Payment Bond: See 1.02.01.

In conformance with 49-41a of the Connecticut General Statutes, as revised, the Contractor:

(1) shall, within 30 days after any given Contract payment to the Contractor by the State, pay any amounts due any subcontractor, whether for labor performed or materials furnished, when charges for such labor or materials have been included in a payment estimate paid by the State; and

(2) shall include in each of its subcontracts a provision requiring each subcontractor to pay any amounts due any of its subcontractors on the Project, whether for labor performed or materials furnished, within 30 days after such subcontractor receives a payment from the Contractor which encompasses labor or materials furnished by such subcontractor for the Project.

If the Contractor believes that it has a valid reason for withholding payment for particular work or materials from a subcontractor or supplier, then the Contractor, within 30 days of receiving payment from the State for that work or materials, shall notify the subcontractor or supplier and the Department of its reasons for withholding payment.

1.03.05—Vacant

1.03.06—Failure to Execute Contract: See 1.02.01.

1.03.07--Insurance: Coverage shall be on a primary basis.

The Contractor shall carry and maintain at all times during the term of the Contract the insurance coverages required by this article and any additional coverages(s) or higher minimum insurance coverage amount(s) required by the Special Provisions of the Contract.

If the Project includes work on or adjacent to railroad property additional insurance may be required as specified by the railroad. Refer to the Contract special provisions for any additional insurance requirements by the railroad.

1. Workers' Compensation Insurance: With respect to all operations the Contractor performs and all those performed for it by subcontractors, the Contractor shall carry, and require each subcontractor to carry, Workers' Compensation insurance as required by the laws of the State of Connecticut.

Employer's Liability insurance shall be provided in amounts not less than $100,000 per accident for bodily injury by accident; $100,000 policy limit by disease and $100,000 per employee for bodily injury by disease. Each Workers' Compensation policy shall contain the U.S. Longshoreman's and Harbor Workers' Act endorsement when work is to be performed over or adjacent to navigable water.

2. Commercial General Liability Insurance: With respect to the operations the Contractor performs and also those performed for it by subcontractors, the Contractor shall carry, and require each subcontractor to carry, Commercial General Liability insurance, including Contractual Liability, Products and Completed Operations, Broad Form Property Damage and Independent Contractors.

Products and completed operations insurance for ongoing and completed operations shall be maintained for a period of 1 year after the acceptance of the Project by the Department in accordance with 1.08.14. See chart below for applicable minimum coverage amounts.

Contract Amount ($)	Minimum Single Occurrence Amount ($)	Minimum Annual Aggregate Amount ($)
0-2,000,000	1,000,000	2,000,000
>2,000,001-10,000,000	2,000,000	4,000,000
>10,000,000	4,000,000	8,000,000

If underground work is to be undertaken, each policy shall have coverage for and exclusions removed for "Explosion, Collapse and Underground" ("XCU").

3. Automobile Liability Insurance: The Contractor shall obtain automobile liability insurance covering the operation of all motor vehicles, including those hired or borrowed, that are used in connection with the Project for all damages arising out of:

(1) bodily injury to or death of all persons and/or

(2) injury to or destruction of property; in any one accident or occurrence.

This policy shall not be subject to an annual aggregate limitation. See chart above for applicable minimum coverage amounts.

4. Owner's and Contractor's Protective Liability Insurance for and in the Name of the State: With respect to the Contractor's Project operations and also those of its subcontractors, the Contractor shall carry, for and on behalf of the State for each accident or occurrence resulting in damages from

(1) bodily injury to or death of persons and/or

(2) injury to or destruction of property.

See chart below for applicable minimum coverage amounts.

Contract Amount ($)	Minimum Single Occurrence Amount ($)	Minimum Annual Aggregate Amount ($)
0 - 20 Million	1,000,000	1,000,000
20 Million - 50 Million	2,000,000	2,000,000
> 50 Million	4,000,000	4,000,000

5. Railroad Protective Liability Insurance: When the Contract involves work within 50 feet of the railroad right-of-way or State-owned rail property, with respect to Project operations and also those of its subcontractors, the Contractor shall carry Railroad Protective Liability Insurance providing coverage of at least $2,000,000 for each accident or occurrence resulting in damages from

(a) bodily injury to or death of all persons and

(b) injury to or destruction of property, and subject to that limit per accident or occurrence, an aggregate coverage of at least $6,000,000 for all damages during the policy period, and with all entities falling within any of the following listed categories named as insured parties:

1. the owner of the railroad right-of-way,
2. the owner of any railcar licensed or permitted to travel within that affected portion of railroad right-of-way, and
3. the operator of any railcar licensed or permitted to travel within that affected portion of the railroad right-of-way, and with the State, if not falling within any of the above-listed categories, also named as an insured party.

6. Blasting: When explosives are to be used in the Project, the Commercial General Liability insurance

policy shall include XCU coverage, in the same limits as the per occurrence policy limits.

7. Protection and Indemnity Insurance for Marine Construction Operations in Navigable Waters: If a vessel of any kind will be involved in Project work, the Contractor shall obtain the following additional insurance coverage:

A. Protection and Indemnity Coverage of at least $300,000 per vessel or equal to at least the value of hull and machinery, whichever is greater.

B. If there is any limitation or exclusion with regard to crew and employees under the protection and indemnity form, the Contractor must obtain and keep in effect throughout the Project a workers' compensation policy, including coverage for operations under admiralty jurisdiction, with a limit of liability of at least $300,000 per accident or a limit equal to at least the value of the hull and machinery, whichever is greater, or for any amount otherwise required by statute.

8. Builder's Risk Insurance: For Facilities construction projects, the Contractor shall maintain comprehensive replacement cost builder's risk (completed value) insurance providing coverage for the entire work at the Project site, including all fixtures, machinery and equipment, any heating, cooling and constituting a permanent part of the building and shall cover portions of work located away from the site, but intended for use at the site. If it is determined that all or a portion of the project is located within an area designated as a Special Flood Hazard Area, the Contractor shall maintain flood insurance (no less than $10,000,000 sublimit). The State of Connecticut shall be named as Loss Payee. Equipment breakdown coverage may be sub limited to 50% of the project cost.

9. Architects and Engineer's Professional Liability Insurance for Structural Engineer: If required, limits will be specified in 1.03.07 of the Special Provisions of the Contract or 1.05.02.

10. Umbrella Liability Insurance: The Contractor may satisfy the minimum limits required for Commercial General Liability and Automobile Liability Insurance using Umbrella Liability Insurance. In the event that the Contractor obtains Umbrella Liability Insurance to meet the minimum coverage requirements for Commercial General Liability or Automobile Liability Insurance coverage, the Umbrella Liability Insurance policy shall have an annual aggregate at a limit not less than twice the single occurrence and must specifically endorse the State of Connecticut as an additional insured. Specifically for Bridge Projects with a low bid equal to or higher than $80,000,000, the Umbrella Liability Insurance policy must have a minimum limit of at least $25,000,000.

11. Certificate of Insurance: Before the Contract is executed, the Contractor must provide to the Department a certificate of insurance acceptable to the Commissioner and executed by an insurance company or companies satisfactory to the State of Connecticut for the insurance coverage(s) required by this Article and the Special Provisions of the Contract. The Contractor shall maintain the required insurance coverage during the entire term of the Contract. The certificate of insurance must clearly include the name of the insured and identify the project for which it is being issued.

12. Copies of Policies: The Contractor shall provide, within 5 business days, a copy or copies of all applicable insurance policies when requested by the State. In providing said policies, the Contractor may redact provisions of the policy that are proprietary. This provision shall survive the expiration or termination of the Contract.

13. Sovereign Immunity: The Contractor may not assert the defense of sovereign immunity in the adjustment of claims or in the defense of any claim or suit brought against the Contractor or the State, unless the State, in writing, requests that the Contractor do so or consents to its doing.

14. Contractor Assumes Costs: The Contractor shall assume and pay all costs and billings for premiums, deductibles, self-insured retentions and audit charges earned and payable under the required insurance.

15. State Named as Additional Insured: The State must be named as an additional insured party for the Commercial General Liability and Automobile Liability insurance policies required by this Article and the Special Provisions to the Contract, and any Umbrella Liability Insurance, as applicable, obtained in accordance with this Article. Each policy shall waive right of recovery (waiver of subrogation) against the State of Connecticut.

16. Termination or Change of Insurance:

a) The Contractor shall notify the Department of any cancelation of insurance carrier or change to the required insurance coverage by submitting a new insurance certificate to the Department immediately following said cancelation or change in required coverage.

b) It is the responsibility of the Contractor to maintain evidence of a current insurance coverage with the Department for the duration of contract. It is the responsibility of the Contractor to file with the

Department all renewals and new certificates of insurance issued due to changes in policy terms or changes in insurance carriers prior to the expiration dates on the forms already on file with the Department.

17. Duration of Coverage. The Contractor shall keep all the required insurance in continuous effect until the date that the Department designates for the termination of the Contractor's responsibility, as defined by 1.08.14.

18. Compensation: There shall be no direct compensation allowed the Contractor on account of any premium or other charge necessary to obtain and keep in effect any insurance or bonds in connection with the Project, but the cost thereof shall be considered included in the general cost of the Project work.

1.03.08—Notice to Proceed and Commencement of Work: The Contractor shall commence and proceed with the Contract work on the date specified in a written Notice to Proceed issued by the Engineer to the Contractor. The date specified will be no later than 45 calendar days after the date of the execution of the Contract by the Department, except that if the expiration of said 45 calendar days occurs during the period between November 30 and April 1 of the following year, the Engineer may specify that the April 1 following said expiration shall be the date for the Contractor to proceed with the work.

If the Engineer does not issue a Notice to Proceed to the Contractor within the said 45 calendar days, the Contractor shall have the option of canceling the Contract and its payment and performance bonds for the Project. Any failure by the Department to issue a notice to proceed, or to issue one on a timely basis, shall not, however, constitute a breach of the Contract. Neither the Contractor nor any other party may use such a failure as a basis for any claim against the Department for damages.

The Contractor shall not begin physical Project construction prior to the date specified for same by the Engineer in the Notice to Proceed, except as may be otherwise authorized by the Engineer in writing.

1.03.09—Contractor Readiness: Immediately upon Contract execution, the Contractor shall prepare submittals and procure materials in order to complete the Project work within the Contract time.

SECTION 1.04
SCOPE OF WORK

1.04.01—Intent of Contract
1.04.02—Changes in Quantities of Pay Items, Including Elimination of Such Items
1.04.03—Changes in Quantities and Significant Changes in the Character of Work
1.04.04—Differing Site Conditions
1.04.05—Extra Work
1.04.06—Removal and Disposal of Structures on the Work Site
1.04.07—Rights in and Use of Materials Found on the Work Site

1.04.01—Intent of Contract: The Contract directs and obliges the Contractor to perform the Project described in strict compliance with the Contract terms, including its specifications, plans, special provisions, and other Contract documents. If the Engineer revises any of those terms in writing during the life of the Contract, the Contractor must comply with said revised terms. Among other things, the Contract obliges the Contractor to perform all Project work in conformity with the lines, grades, typical cross-sections, dimensions, and other data shown on the plans and other Contract documents. The Department will pay the Contractor only for work (including materials necessary for that work, whether or not they are incorporated into that work) that the Contractor has actually performed under a Contract pay item, and only if the Engineer has accepted said work. (See 1.02.03 herein.) (The Contract as it existed when first duly executed by the Engineer is sometimes referred to herein as "the original Contract.")

1.04.02—Changes in Quantities of Pay Items, Including Elimination of Such Items: The quantities given in the original Contract for Contract pay items are only estimates of the quantities of those items that may be required for Project completion. (The quantities for given pay items in the original Contract are sometimes referred to herein as the "estimated quantities" or "original quantities.") A change in the original quantity of a Contract pay item (whether an increase or decrease of the quantity) shall be deemed to have occurred when the Engineer explicitly orders said change of quantity or when the change of quantity has been necessitated by a construction order or other written direction issued by the Engineer to the Contractor.

A Contract pay item shall be deemed a Major Item if the item's lump sum price in the original Contract, or its original quantity multiplied by its unit price in the original Contract, is equal to or greater than 10% of the original Contract's total bid price. All other Contract items shall be deemed Minor Items.

The provisions of 1.04.03 herein shall govern changes in compensation related to a "significant change" in Contract work (as such changes are defined in 1.04.03) necessitated by a written order of the Engineer.

The provisions of 1.04.04 herein shall govern changes in compensation related to any differing site condition encountered by the Contractor that affects its performance of Contract work.

The provisions of 1.04.03 or 1.04.04 shall govern in any case in which they conflict with another provision of the Contract.

If the Engineer and the Contractor together determine that a particular change in compensation to the Contractor should be made due to a change in a Contract pay item quantity (including an item's complete elimination), they may make that change in compensation by a written agreement to do so.

Changes in Quantities to Minor Items:

(a) Quantity Increases of More Than 25% over Original Quantity: If the actual quantity of work authorized and accepted by the Engineer under a Contract pay item exceeds the item's original quantity by 25%, the Department will pay for the quantity in excess of 125% of the original quantity in one of the following three ways. (One-time fixed costs for which the Department has already reimbursed the Contractor in paying for 125% of the original quantity shall not be included in a calculation of the actual cost of the excess units.)

(1) Pay for the aggregate excess units on a cost-plus basis as provided in 1.09.04.
(2) Adjust the unit price by the increase or decrease in the unit price for the excess units, which shall be the difference between the original Contract unit price and the actual unit cost (calculated on a cost-plus basis as provided in 1.09.04) of the excess units, said difference to be calculated as of the time when work under the item was completed.
(3) Pay for the units in any other manner agreed on in writing by the Engineer and the Contractor.

If, however, the aggregate payment for the units in excess of 125% is less than $25,000 (using the original Contract unit price for the calculation) the Engineer will not adjust that unit price.

(b) Quantity Decreases of More Than 25% below Original Quantity: If the actual quantity of a Contract pay item authorized and accepted by the Engineer is less than 75% of the item's original quantity, the Engineer will not adjust the original Contract unit price for said item unless the Contractor makes a written request to the Engineer for such adjustment and the Engineer grants it in writing. If the Engineer grants such a request, the Engineer will adjust the price for each accepted unit of said item performed or provided in one of the following three ways:

(1) Pay for the total item units actually performed or provided in the aggregate units on a cost-plus basis as provided in 1.09.04.

(2) Adjust the unit price by any increase in the unit price for the deficit units, which shall be the difference between the original Contract unit price and the actual unit cost (calculated on a cost-plus basis as provided in 1.09.04) of the total units performed or provided, said difference to be calculated as of the time when work under the item was completed.

(3) Pay for the item units performed or provided in any manner agreed on in writing by the Engineer and the Contractor.

In no instance however, shall the unit price paid for the number of units performed or provided, when their quantity has been decreased by more than 25% of the original quantity, be less than their original unit price; and in no instance shall the aggregate payment for such a decreased quantity of items be more than the Engineer would have paid for the performance or provision of 75% of the original quantity at the original unit price.

Regarding treatment of eliminated Contract items, refer to 1.09.05 herein.

1.04.03—Changes in Quantities and Significant Changes in the Character of Work:

(i) The Engineer reserves the right to make, in writing, at any time during the work, such changes in quantities and such alterations in the work as are necessary to satisfactorily complete the Project. Such changes in quantities and alterations shall not invalidate the Contract nor release the surety, and the Contractor agrees to perform the work as altered.

(ii) If the alterations or changes in quantities significantly change the character of the work under the Contract, whether such alterations or changes are in themselves significant changes to the character of the work or by affecting other work cause such other work to become significantly different in character, an adjustment, excluding anticipated profit, will be made to the Contract. The basis for the adjustment shall be agreed upon prior to the performance of the work. If a basis cannot be agreed upon, then an adjustment will be made either for or against the Contractor in such amount as the Engineer may determine to be fair and equitable.

(iii) If the alterations or changes in quantities do not significantly change the character of the work to be performed under the Contract, the altered work will be paid for as provided elsewhere in the Contract.

(iv) The term "significant change" shall be construed to apply only to the following circumstances:

(A) When the character of the work as altered differs materially in kind or nature from that involved or included in the original proposed construction; or

(B) When a Major Item of work, as defined elsewhere in the Contract, is increased in excess of 125% or decreased below 75% of the original Contract quantity. Any allowance for an increase in quantity shall apply only to that portion in excess of 125% of original Contract item quantity, or in case of a decrease below 75%, to the actual amount of work performed.

1.04.04—Differing Site Conditions:

(i) During the progress of the work, if subsurface or latent physical conditions are encountered at the Site differing materially from those indicated in the Contract or if unknown physical conditions of an unusual nature, differing materially from those ordinarily encountered and generally recognized as inherent in the work provided for in the Contract, are encountered at the Site, the party discovering such conditions shall promptly notify the other party in writing of the specific differing conditions before the Site is disturbed and before the affected work is performed.

(ii) Upon written notification, the Engineer will investigate the conditions, and if it is determined that the conditions materially differ and cause an increase or decrease in the cost or time required for the performance of any work under the Contract, an adjustment, excluding anticipated profits, will be made and the Contract modified in writing accordingly. The Engineer will notify the Contractor of the determination whether or not an adjustment of the Contract is warranted.

(iii) No Contract adjustment which results in a benefit to the Contractor will be allowed unless the

Contractor has provided the required written notice.

(iv) No Contract adjustment will be allowed under this clause for any effects caused on unchanged work.

1.04.05—Extra Work: Unforeseen work made necessary by the Engineer's changes of the Contract plans or specifications, or work that is necessary for completion of the Project, but for which no price is provided in the Contract, shall be done in accordance with the requirements of the specifications and as directed by the Engineer. The Engineer will notify the Contractor of the necessity for such extra work, stipulating its character and extent, and will notify the Contractor as to whether the Engineer wants the Contractor to propose a unit price or, lump sum price, or to perform the extra work on a cost-plus basis in accordance with 1.09.04. The Engineer need not solicit any price for the extra work from the Contractor, but may, in any event, simply order the Contractor to perform the extra work on a cost-plus basis. If the Engineer does solicit from the Contractor a unit or lump sum price for the extra work, the Contractor must propose such a price in writing within 5 days of the Engineer's request for one.

The Contractor's price proposal shall be itemized and reasonably detailed, and shall include all known or anticipated direct and indirect costs of the work, including but not limited to, the costs of all safety and other equipment, small tools, labor, subcontractor quotes, consumables, field office overhead, home office overhead, insurance, bonding, and profit.

The character and extent of the extra work, together with the basis of compensation, shall be communicated to the Contractor by means of a construction order which, when signed by the Engineer, shall become a part of the Contract. If a Contractor objects to any portion of a construction order submitted to it, the Contractor must, within 15 days of its receipt of said order, return the order with a letter to the Department's Assistant District Engineer administering the Contract, describing specifically what portions of the order the Contractor finds objectionable, the nature of its objections, and the bases for its objections. If the Contractor does not do so, it shall be deemed to have accepted the terms of the construction order.

If the Engineer changes the scope of Contract work, the Contractor shall submit a proposed revised schedule and a cost revision proposal, which takes all such changes into account, if the Contractor believes that such revisions are warranted. If the schedule is to be revised, it will be revised in accordance with 1.08.08.

1.04.06—Removal and Disposal of Structures on the Work Site: All structures on the Project site which are not to remain on the Project site after completion of the Project shall be removed from said site and disposed of by the Contractor once it is no longer needed for the Project, and any such structure shall then become the property of the Contractor, except as otherwise required or provided by 1.10.07.

1.04.07—Rights in and Use of Materials Found on the Work Site: Upon written request of the Contractor and with the written approval of the Engineer, subject to limitations which may be set forth within such approval, any stone, gravel, sand, topsoil or any material from existing bridge substructures, buildings, or other structures, found within the limits of the Project may be excavated or removed and used by the Contractor on the Project, provided that said materials meet the requirements of the specification for such materials. Any materials excavated or removed shall not be taken off the Project site unless the Engineer in writing specifically authorizes such action. The following conditions shall govern these matters:

1. Excavation or removal of materials that would necessarily be excavated or removed in making the improvement will be paid for at the applicable Contract unit prices; and, in addition, the item for which this material is used will also be paid for at its Contract unit price. The Contractor will not be charged for such materials. The Contractor shall, without compensation, place in the embankment or elsewhere, as appropriate, sufficient suitable material to fill the space that the excavated materials would have occupied, unless otherwise directed by the Engineer.
2. The excavation or removal of materials that are not required to be excavated or removed in connection with the Contract work will not be paid for; and the Contractor will be charged for such materials at a negotiated unit price. The item for which this material is used will be paid for at its Contract unit price. The Contractor shall, without compensation, backfill with accepted material the space that the excavated materials had occupied, to the satisfaction of the Engineer, unless otherwise directed by the Engineer.

Surplus material shall be removed from the Project only with the Engineer's written permission. The Engineer may determine that such material is not surplus, and may order that it be incorporated into the Project.

SECTION 1.05
CONTROL OF THE WORK

1.05.01—Authority of Engineer
1.05.02—Plans, Working Drawings, Shop Drawings, Product Data, Submittal Preparation and Processing, and Designers Action
1.05.03—Conformity with Plans and Specifications (including Quality Control)
1.05.04—Coordination of Special Provisions, Plans, Supplemental Specifications and Standard Specifications and Other Contract Requirements
1.05.05—Cooperation by Contractor
1.05.06—Cooperation with Utilities (Including Railroads)
1.05.07—Coordination with Work by Other Parties
1.05.08—Schedules and Reports
1.05.09—Authority of Inspectors
1.05.10—Inspection
1.05.11—Removal of Defective or Unauthorized Work
1.05.12—Payrolls
1.05.13—Examining and Copying Contractor's Records
1.05.14—Termination for Convenience
1.05.15—Markings for Underground Facilities
1.05.16—Dimensions and Measurements
1.05.17—Welding
1.05.18—Structural Fabrication Shop Prequalification
1.05.23—Requests for Information (RFIs) and Requests for Change (RFCs)

1.05.01—Authority of Engineer: All work shall be subject to the review of the Engineer. He shall decide all questions as to interpretation of the plans and specifications, and questions of mutual or respective rights of the Contractor and other Department contractors. The Engineer shall decide on an acceptable rate of progress, on the manner of performance, and on what shall be deemed acceptable fulfillment of the Contract. The Engineer shall have the right to determine the points at which the Contractor may begin work and the order in which the work shall be prosecuted in the best interests of the State within the intent of the terms in the Contract.

If a Project-related dispute arises between the Contractor and Department personnel assigned to the Project, and if those parties prove unable to resolve it, the Contractor may submit a detailed written description of the dispute to the Department's Assistant District Engineer administering the Contract.

It must be understood, though, that at no time may the Contractor, because of its disagreement with the Engineer, either disregard the orders of the Engineer or halt Project construction. If the Contractor cannot resolve a Project work or pricing dispute with the Engineer, the Contractor's proper remedy is a claim under CGS 4-61. A Contractor that disregards the orders of the Engineer with regard to the prosecution of Project work, or who refuses to continue Project work because of a disagreement with the Engineer, may be subject to termination of its Contract, to a subsequent finding that it is nonresponsible as an apparent low bidder for a Department contract, to the assessment of liquidated damages, and to other adverse legal or administrative action by the Department.

1.05.02—Plans, Working Drawings, Shop Drawings, Product Data, Submittal Preparation and Processing, and Designers Action:

1. Plans: The plans prepared by the Department show the details necessary to give a comprehensive idea of the construction contemplated under the Contract. The plans will generally show location, character, dimensions, and details necessary to complete the Project. If the plans do not show complete details, they will show the necessary dimensions and details, which when used along with the other Contract documents, will enable the Contractor to prepare working drawings, shop drawings or product data necessary to complete the Project.

2. Working Drawings: When required by the Contract or when ordered to do so by the Engineer, the Contractor shall prepare and submit six printed copies and one electronic copy in a pdf file format of the working drawings, signed, sealed and dated by a qualified Professional Engineer licensed to practice in the State of Connecticut, for review. The drawings shall be submitted to the Assistant District Engineer sufficiently in advance of the work detailed, to allow for their review in accordance with the review periods specified in 1.05.02-5 (including any necessary revisions, resubmittal, and final review).

There will be no direct payment for furnishing any working drawings, procedures or supporting calculations, but the cost thereof shall be considered as included in the general cost of the work.

a. Working Drawings for Permanent Construction: Drawings shall be submitted on 22 inch × 34 inch sheets with a border and title block similar to the Department standard. Calculations, procedures and other supporting data may be submitted in an 8-1/2 inch × 11 inch format.

   The Contractor shall supply to the Assistant District Engineer a certificate of insurance in accordance with 1.03.07 at the time that the working drawings for the Project are submitted.

   The Contractor's designer, who prepares the working drawings, shall secure and maintain at no direct cost to the State a Professional Liability Insurance Policy for errors and omissions in the minimum amount of $2,000,000 per error or omission. The Contractor's designer may elect to obtain a policy containing a maximum $250,000 deductible clause, but if the Contractor's designer should obtain a policy containing such a clause, they shall be liable to the extent of at least the deductible amount. The Contractor's designer shall obtain the appropriate and proper endorsement of its Professional Liability Policy to cover the indemnification clause in this Contract, as the same relates to negligent acts, errors or omissions in the Project work performed by them. The Contractor's designer shall continue this liability insurance coverage for a period of

   (i) 3 years from the date of acceptance of the work by the Engineer, as evidenced by a State of Connecticut, Department of Transportation form entitled "Certificate of Acceptance of Work," issued to the Contractor; or

   (ii) 3 years after the termination of the Contract, whichever is earlier, subject to the continued commercial availability of such insurance.

b. Working Drawings for Temporary Construction: The Contractor shall submit drawings, calculations, procedures and other supporting data in a format acceptable to the Assistant District Engineer.

3. Shop Drawings: When required by the Contract, or when ordered to do so by the Engineer, the Contractor shall prepare and submit 6 printed copies and 1 electronic copy in a pdf file format of the shop drawings to the Designer for review. Review timeframes and submission locations are specified in 1.05.02-5.

Drawings shall be submitted on 22 inch × 34 inch sheets with an appropriate border and with a title block in the lower right-hand corner of each sheet. Procedures and other supporting data may be submitted on 8-1/2 inch × 11 inch sheets.

There will be no direct payment for furnishing any shop drawings, but the cost thereof shall be considered as included in the general cost of the work.

4. Product Data: When required by the Contract, or when ordered to do so by the Engineer, the Contractor shall prepare and submit 6 printed copies and 1 electronic copy in a pdf file format of the product data.

The product data shall be submitted to the Designer for review, sufficiently in advance of the work detailed, to allow for their review in accordance with the review periods specified in 1.05.02-5 (including any necessary revisions, resubmittal, and final review), and acquisition of materials, without causing a delay of the Project.

The Contractor shall submit the product data in a single submittal for each element of construction.

The Contractor shall mark each copy of the product data submittal to show applicable choices and options. Where product data includes information on several products that are not required, copies shall be marked to indicate the applicable information. Product data shall include the following information and confirmation of conformance with the Contract to the extent applicable: manufacturer's printed recommendations, compliance with recognized trade association standards, compliance with recognized testing agency standards, application of testing agency labels and seals, notation of coordination requirements, Contract item number, and any other information required by the individual Contract provisions.

There will be no direct payment for furnishing any product data, but the cost thereof shall be considered as included in the general cost of the work.

5. Submittal Preparation and Processing – Review Timeframes: The Contractor shall allow 30 calendar days for submittal review by the Department, from the date of receipt of printed copies in the appropriate Designer or Engineer's office. For any submittals marked with "Revise and Resubmit" or "Rejected," the Department is allowed an additional 20 calendar days for review of any resubmissions.

An extension of Contract time will not be authorized due to the Contractor's failure to transmit submittals sufficiently in advance of the work to permit processing.

The furnishing of shop drawings, working drawings or product data, or any comments or suggestions by the Designer or Engineer concerning shop drawings, working drawings or product data, shall not relieve the Contractor of any of its responsibility for claims by the State or by third parties, as per 1.07.10.

The furnishing of the shop drawings, working drawings and product data shall not serve to relieve the Contractor of any part of its responsibility for the safety or the successful completion of the Project construction.

Submissions: Unless otherwise defined in the Contract, the Contractor shall transmit the working drawings, shop drawings and product data as follows:

a. Working drawings for permanent construction, shop drawings, and product data shall be submitted to the Designer. A copy of the transmittal or cover letter shall be forwarded to the Assistant District Engineer of the administering Construction District.
b. Working drawings for temporary construction shall be submitted to the Assistant District Engineer of the administering Construction District.
c. If not provided in the Contract, the Contractor shall request a list detailing the delivery location and contact person for each type of submittal, from the administering Construction District.

6. Designers Action: The Designer or Engineer will review each submittal, mark each with a uniform, self-explanatory action stamp, and return the stamped submittal promptly to the Contractor. The Contractor shall not proceed with the part of the Project covered by the submittal until the submittal is marked "No Exceptions Noted" or "Exceptions as Noted" by the Designer or Engineer. The Contractor shall retain sole responsibility for compliance with all Contract requirements. The stamp will be marked as follows to indicate the action taken:

a. If submittals are marked "No Exceptions Noted," the Designer or Engineer has not observed any statement or feature that appears to deviate from the Contract requirements. This disposition is contingent on being able to execute any manufacturer's written warranty in compliance with the Contract provisions. The Contractor may proceed with the work covered in the submittal.
b. If submittals are marked "Exceptions as Noted" the considerations or changes noted by the Designer or Engineer are necessary in order for the submittal to comply with Contract requirements. The Contractor shall review the required changes and inform the Designer or Engineer if they feel the changes violate a provision of the Contract or would lessen the warranty coverage.
c. If submittals are marked "Revise and Resubmit," the Contractor shall revise the submittals to address the deficiencies or provide additional information as noted by the Designer or Engineer. The Contractor shall allow an additional review period as specified in 1.05.02-5.
d. If submittals are marked "Rejected," the Contractor shall prepare and submit a new submittal in accordance with the Designer's or Engineer's notations. The resubmissions require an additional review and determination by the Designer or Engineer. The Contractor shall allow an additional review period as specified in 1.05.02-5.

1.05.03—Conformity with Plans and Specifications (including Quality Control): The Contractor shall perform all work and provide all materials in conformity with the lines, grades, cross-sections, dimensions and material requirements, including tolerances, shown on the plans or indicated in the Contract specifications, or as directed by the Engineer.

Quality Management Plan: The Contractor shall maintain and implement a written Quality Management Plan (QMP). The QMP shall document the overall internal quality control operating procedures for the Contractor to meet or exceed Contract requirements. The details of the QMP must discuss how the Contractor will ensure that:

- Work processes are performed efficiently and as documented
- Work processes out of conformance are quickly identified
- Corrective action is quickly taken to bring such work processes back into conformance

The QMP must include the following components:

- Identification of Contractor staff and their specific duties and responsibilities with regard to execution of the QMP
- Standard operating procedures and frequency of quality control inspection and testing used to measure quality before, during and after those procedures
- Action plan for reporting and reacting to nonconformance and quality control issues

The Contractor shall furnish a copy of the QMP to the Engineer prior to the start of the work. The Contractor must revise the QMP if, as determined by the Engineer, the Contractor's procedures prove to be

inadequate or ineffective in producing work that meets the Contract requirements. Failure of the Contractor to comply with the provisions of this Article may result in a suspension of work in whole or in part. The Department will not grant the Contractor additional Contract time or compensation in connection with such a suspension.

1.05.04—Coordination of Special Provisions, Plans, Supplemental Specifications and Standard Specifications and Other Contract Requirements: All requirements indicated on the plans or in the Standard Specifications, the Supplemental Specifications, Special Provisions or other Contract provisions shall be equally binding on the Contractor, unless there is a conflict between or among any of those requirements. In the case of such a conflict, the order of governance among those requirements, in order of descending authority, shall be as follows:

1. Environmental Permit Special Conditions
2. Special Provisions
3. Plans other than Standard Sheets (enlarged details on plans, used to clarify construction, shall take precedence over smaller details of the same area; and information contained in schedules or tables, titled as such, shall take precedence over other data on plans)
4. Environmental Applications / Registrations / Certifications
5. Supplemental Specifications
6. Standard Specifications
7. Standard Sheets

Numerical designations of dimensions shall take precedence over dimensions calculated by applying a scale to graphic representations. Neither party to the Contract may take advantage of any obvious error or omission in the Contract. Should either party to the Contract discover such an error or omission, that party shall notify the other party of same immediately in writing. The Engineer will make such corrections and interpretations of the Contract as are necessary, in his judgment, to fulfill the purposes of the Contract that are evident from examining the Contract as a whole.

If the Contract includes an item that does not have a corresponding specification for either performance or payment purposes, the Contractor shall notify the Engineer of that fact in writing at least 2 weeks prior to ordering materials for or commencing work on the item. If the Department's documents do not contain such a specification, the Engineer shall, if possible, derive an appropriate specification from applicable AASHTO Specifications or, if necessary, ASTM Specifications. If neither of those sources provides a suitable specification, the Contractor shall seek guidance from the Engineer with regard to the item, and the Engineer will formulate a reasonable specification for the item. When compliance with two or more standards is specified, and the standards may establish different or conflicting requirements for minimum quantities or quality levels, the Contractor shall refer such issues to the Engineer for a decision before proceeding with the pertinent work.

1.05.05—Cooperation by Contractor: The Contractor will be supplied by the Department with copies of the plans, and the Contractor shall have available on the Project site at all times during the prosecution of the Project, a copy of the Contract plans and specifications. The Contractor shall give the Project constant attention to facilitate the progress thereof, shall cooperate with the Department, and shall promptly comply with all orders and directions of the Engineer.

The Contractor shall at all times during Project construction have on the Project site one of its employees who is thoroughly experienced in the type of work being performed, to supervise the work and accept directions from the Engineer. The Contractor shall always notify the Engineer of the identity of said employee representative in advance of the employee's assignment to that position. The Contractor's representative must have full authority to promptly execute and carry out the orders and directions of the Engineer within the terms of the Contract, and to supply such materials, equipment, tools, labor and incidentals as may be required by the Contract or by the Engineer.

Voluntary Partnering: The Connecticut Department of Transportation ("Department") wants to establish a cohesive partnership with the Contractor and its principal subcontractors on the Project, so that the partnership can draw on the strengths of each organization in order to identify and pursue the partners' mutual Project goals. Chief among those will be the effective and efficient completion of the Project, within budget, on schedule, and in accordance with applicable plans, specifications, and other Contract provisions.

If the Contractor believes at any point before or during Project construction that the creation of formal partnering between itself and the Department, with the use of a third-party facilitator, would help the

Contractor and the Department ("Partners") to reach these goals, the Contractor may submit a written request to the District Engineer of the District in which the Project will be constructed for the establishment of formal partnering between the Parties. If the Contractor makes such a request, the Department will engage in that partnering.

Any costs incurred by the Partners jointly in connection with Project partnering activities, to the extent that those costs are recognized as legitimate and appropriate by both Partners, will be shared equally between them. Any other costs incurred because of partnering activities will be borne by the Partner that incurred them.

If the Contractor and the Department decide to pursue a formal partnering initiative, they Contractor and The Department will arrange first to meet in order to select a third-party partnering facilitator and to plan a partnering development and team-building workshop. After they agree upon the services to be performed by the facilitator and the range of compensation for the facilitator that would be acceptable to them, the Contractor will contract accordingly for the services of said facilitator. The Department will reimburse the Contractor for 50% of the payments made under that contract, so long as the activities paid for were appropriate and within the contemplation of the Partners.

At the Partners' initial partnering meeting, the Partners will also determine who should attend the first partnering workshop, what the workshop's agenda will be, how long the workshop should last, and when and where it will be held. Unless the Partners agree otherwise, attendance at the first partnering workshop will be mandatory for the Department's District Engineer for the Project and the Department's other key Project personnel, the Contractor's on-Site Project manager and other key supervisory Project personnel, and, if the Contractor agrees to it, the key supervisory personnel of the Contractor's principal Project subcontractors. The Partners will also request that the Project design engineers and key local government personnel send Regional/District and Corporate/State-level managers to the workshop and direct them to participate in Project partnering activities as and when requested to do so by the Partners.

With the agreement of the Partners, follow-up Project partnering workshops will be held periodically until the Department closes out the Contract.

If the Partners agree on a formal partnering charter for the Project, the establishment of that charter will not change the legal relationship of the Partners to the Contract; it will not alter, supplement, or eliminate any of the Partners' rights or obligations under the Contract.

1.05.06—Cooperation with Utilities (Including Railroads): The Engineer may anticipate that a Project construction activity will require the removal, repair, replacement or relocation of a utility appurtenance. In such an instance, the Engineer, in advance of the commencement of such activity, will notify the affected utilities, either directly or through the local government, of the anticipated nature and timing of said activity. The Engineer will endeavor to have all necessary adjustments of public or private utility fixtures, pipelines, and other appurtenances within or adjacent to the limits of Project construction made as soon as practicable, when such changes are required by the State or local government.

Whenever the Engineer determines that the relocation or adjustment of poles or the overhead plant of public or private utilities or railroad facilities is dependent upon the completion of certain required Contract activities, the Contractor shall complete those activities within a reasonable length of time.

Temporary and permanent changes required by the State or local government in water lines, gas lines, sewer lines, wire lines, service connections, water or gas meter boxes, water or gas valve boxes, light standards, cableways, signals and all other utility (including railroad) appurtenances within the site of the proposed Project construction are to be made by others at no expense to the Contractor, except as otherwise provided for in the Special Provisions or as noted on the plans.

When the Contractor is required by the Engineer to relocate utility appurtenances, such work will be paid for as extra work unless specific bid items for such work appear in the Contract.

If the Contractor, for its convenience or for any other reason, desires a change in the location of a water line, gas line, sewer line, wire line, service connection, water or gas meter box, valve box, light standard, cableway, signal or any other utility (including railroad) appurtenances, the Contractor shall satisfy the Department that the proposed relocation will not interfere with the Contractor's or other contractors' Project operations or their fulfillment of the requirements of the plans, and that said change will not create an obstruction or hazard to traffic. If the requested change of location is acceptable to the Engineer, the Contractor shall make its own request for such relocation work to the utility companies, pipe owners or other parties likely to be affected by said work. Such relocation work shall be done at the Contractor's sole expense.

The Contractor shall schedule its operations in such a manner as to minimize interference with the operations of the utility companies or local governments in effecting the installation of new facilities, as shown on the plans, or the relocation of their existing facilities. The Contractor shall consider in its bid all permanent and temporary utility appurtenances in their present or relocated positions and any installation of new facilities required for the Project. The Department will not make any additional compensation to the Contractor for delays, inconvenience or damage sustained by the Contractor due to

(i) interference with Project construction caused by the location, condition or operation of utility (including railroad) appurtenances or

(ii) the installation, removal, or relocation of such appurtenances; and the Contractor may not make a claim for any such compensation.

1.05.07—Coordination with Work by Other Parties: The Contractor shall make every effort to perform its work so as not to interfere with other work for the State or other parties. In the case of a dispute with another contractor working for the Department regarding their work for the State, or in the case of a conflict between their planned operations or the needs of their projects, the Contractor shall bring that dispute or conflict to the Engineer's attention, and the Engineer shall decide how it shall be resolved. The Engineer's decision shall be binding upon all of the contractors working for the Department who are involved in the matter.

The Contractor shall, as far as possible, schedule and otherwise plan and arrange its work, and place and dispose of its Project materials, so as not to interfere with the operations of other contractors working for the State. The Contractor shall, as necessary to accomplish this goal, endeavor to coordinate and schedule its work in the way which will interfere least with the work of other parties.

If the Contractor's work or activities under the Contract come into conflict with other activities or work for the State, any financial or other liability arising from such conflicts shall be the Contractor's; and the Contractor shall protect and save harmless the State from any and all damages or claims, and the costs of defending same, which may arise because of inconvenience, delay, financial hardship, or injuries caused to the Contractor or to other contractors as a result of such conflicts, unless:

(a) The Contractor notifies the Engineer of such conflicts as soon as the likelihood of such a conflict becomes apparent; or, if such likelihood could not have been foreseen earlier, then as soon as the conflict becomes apparent.

(b) The Contractor waits for direction from the Engineer as to how the conflict should be avoided or resolved, and the Contractor does not proceed with the work involved in the conflict until the Engineer has provided the Contractor with such direction.

(c) The Contractor follows the directions given by the Engineer for avoiding, resolving, or minimizing the conflict.

The Contractor shall be responsible for the completion of its Contract work, regardless of any interference with, or delay of, that work which may be caused by the presence or activities of other contractors working for the State.

1.05.08—Schedules and Reports: When a project coordinator is not required by the Contract the following shall apply:

Baseline Bar Chart Construction Schedule: Within 20 calendar days after contract award the Contractor shall develop a comprehensive bar chart as a baseline schedule for the project. The bar chart schedule shall be submitted to the Engineer for approval and shall be based on the following guidelines:

1. The bar chart schedule shall contain a list of activities that represents the major activities of the project. At a minimum, this list should include a breakdown by individual structure or stage, including major components of each. The bar chart schedule shall contain sufficient detail to describe the progression of the work in a comprehensive manner. As a guide, 10 to 15 bar chart activities should be provided for each $1 million of contract value.

The following list is provided as an example only and is not meant to be all-inclusive or all-applicable:

General Activities Applicable to all projects

Project Constraints

-Winter shutdowns
-Environmental permits/application time of year restrictions
-Milestones
-Third Party approvals
-Long lead time items (procurement and fabrication of major elements)

-Adjacent Projects or work by others
Award
Notice to Proceed
Signing (Construction, temporary, permanent by location)
Mobilization
Permits as required
Field Office
Utility Relocations
Submittals/shop drawings/working drawings/product data
Construction of Waste Stock pile area
Clearing and Grubbing
Earthwork (Borrow, earth ex, rock ex etc.)
Traffic control items (including illumination and signalization)
Pavement markings
Roadway Construction (Breakdown into components)
Drainage (Breakdown into components)
Culverts
Plantings (including turf establishment)
Semi-final inspection
Final Cleanup

As required the following may supplement the activities listed above for the specific project types indicated:

a. For bridges and other structures, include major components such as abutments, wingwalls, piers, decks and retaining walls; further breakdown by footings, wall sections, parapets etc.
   Temporary Earth Retention Systems
   Cofferdam and Dewatering
   Structure Excavation
   Piles/test piles
   Temporary Structures
   Removal of Superstructure
   Bearing Pads
   Structural Steel (Breakdown by fabrication, delivery, installation, painting etc.)
   Bridge Deck
b. Multiple location projects such as traffic signal, incident management, lighting, planting and guiderail projects will be broken down first by location and then by operation. Other major activities of these types of projects should include, but are not limited to:
   Installation of anchors
   Driving posts
   Foundations
   Trenching and Backfilling
   Installation of Span poles/mast arms
   Installation of luminaries
   Installation of cameras
   Installation of VMS
   Hanging signal heads
   Sawcut loops
   Energizing equipment
c. Facility Projects – Facilities construction shall reflect the same breakdown of the project as the schedule of values:
   Division 2 – Existing Conditions
   Division 3 – Concrete
   Division 4 – Masonry
   Division 5 – Metals
   Division 6 – Wood, Plastic, and Composites
   Division 7 – Thermal and Moisture Protection
   Division 8 – Openings

Division 9 – Finishes
Division 10 – Specialties
Division 11 – Equipment
Division 12 - Furnishings
Division 13 – Special Construction
Division 14 – Conveying Equipment
Division 21 – Fire Suppression
Division 22 – Plumbing
Division 23 – Heating, Ventilating, and Air Conditioning
Division 26 – Electrical
Division 27 – Communications
Division 28 – Electronic Safety and Security
Division 31 – Earthwork
Division 32 – Exterior Improvements
Division 33 - Utilities

2. If the Engineer determines that additional detail is necessary, the Contractor shall provide it.
3. Each activity shall have a separate schedule bar. The schedule timeline shall be broken into weekly time periods with a vertical line to identify the first working day of each week.
4. The bar chart schedule shall show relationships among activities. The critical path for the Project shall be clearly defined on the schedule. The schedule shall show milestones for major elements of work, and shall be prepared on a sheet, or series of sheets of sufficient width to show data for the entire construction period**.**
5. If scheduling software is used to create the bar chart schedule, related reports such as a predecessor and successor report, a sort by total float, and a sort by early start shall also be submitted.
6. Project activities shall be scheduled to demonstrate that the construction completion date for the Project will occur prior to expiration of the Contract time. In addition, the schedule shall demonstrate conformance with any other dates stipulated in the Contract.
7. The Contractor is responsible to inform its subcontractor(s) and supplier(s) of the project schedule and any relevant updates.
8. There will be no direct payment for furnishing schedules, the cost thereof shall be considered as included in the general cost of the work.
9. For projects without a Mobilization item, 5% of the contract value will be withheld until such time as the Baseline Schedule is approved.

Monthly Updates: No later than the 10th day of each month, unless directed otherwise by the Engineer, the Contractor shall deliver to the Engineer 3 copies of the schedule to show the work actually accomplished during the preceding month, the actual time spent on each activity, and the estimated time needed to complete any activity which has been started but not completed. Each time bar shall indicate, in 10% increments, the estimated percentage of that activity which remains to be completed. As the Project progresses, the Contractor shall place a contrasting mark in each bar to indicate the actual percentage of the activity that has been completed.

The monthly update shall include revisions of the schedule necessitated by revisions to the Project directed by the Engineer (including, but not limited to extra work), during the month preceding the update. Similarly, any changes of the schedule required due to changes in the Contractor's planning or progress shall also be included. The Engineer reserves the right to reject any such revisions. If the schedule revisions extend the Contract completion date, due to extra or added work or delays beyond the control of the Contractor, the Contractor shall submit a request in writing for an extension of time in accordance with 1.08.08. This request shall be supported by an analysis of the schedules submitted previously.

Any schedule revisions shall be identified and explained in a cover letter accompanying the monthly update. The letter shall also describe in general terms the progress of the Project since the last schedule update and shall identify any items of special interest.

If the Contractor fails to provide monthly schedule updates, the Engineer has the right to hold 10% of the monthly estimated payment, or $5,000, whichever is less, until such time as an update has been provided in accordance with this provision.

Biweekly Schedules: Each week, the Contractor shall submit to the Engineer a two week look-ahead schedule. This short-term schedule may be handwritten but shall clearly indicate all work planned for the following 2 week period.

Recovery Schedules: If the updated schedule indicates that the Project has fallen behind schedule, the Contractor shall either submit a time extension request in accordance with 1.08.08 or immediately institute steps acceptable to the Engineer to improve its progress of the Project. In such a case, the Contractor shall submit a recovery plan, as may be deemed necessary by the Engineer, to demonstrate the manner in which an acceptable rate of progress will be regained.

1.05.09—Authority of Inspectors: Inspectors employed by the Department are authorized to inspect all work done and all materials furnished for Project construction. Such inspection may extend to any part of the Project work and to the preparation or manufacture of the materials to be used for same. In case of any dispute arising between the Contractor and the inspector as to materials furnished or the manner of performing work, the inspector has the authority to reject material or stop the work until the question at issue can be referred to and decided by the Engineer. The inspector is not authorized to revoke, alter, enlarge, relax, or release any requirements of the Contract, nor to approve or accept any portion of the Contract work, nor to issue instructions contrary to the Contract. The inspector shall in no case act as a foreman, or fulfill other duties for the Contractor. Any advice that the inspector may give to the Contractor shall not be construed as binding the Department in any way, nor as releasing the Contractor from its obligation to fulfill the terms of the Contract.

1.05.10—Inspection: All materials and each part or detail of the Project work shall be subject at all times to inspection by the Engineer. Such inspection may take place on the Site or an an offsite location such as a mill, subcontractor fabrication plant or shop, or other type of location. The Engineer shall be allowed access to all parts of the work and shall be furnished with such information and assistance by the Contractor as the Engineer deems necessary to make complete, detailed, and timely inspections. Information collected by the inspector may include written observations, sketches, photographs and other data as deemed appropriate by the inspector. The Contractor shall allow the Engineer to collect such information without restriction on the Site and shall ensure that the Engineer will have the same unrestricted ability to gather such pertinent information regarding Project work or materials at a location controlled by a subcontractor or supplier of the Contractor.

The Contractor shall always notify the Engineer of its intention to perform work on the Project site, including the nature of the particular work it intends to perform, at least 48 hours before the Contractor commences that work. If, after receiving such notice, the Engineer decides that he needs more than 48 hours to arrange for and conduct inspection related to that work, he shall so notify the Contractor, and the Contractor shall refrain from commencing the work until the Engineer has arranged for such inspection. The Contractor may not commence any portion of its work without prior related inspection by the Engineer unless the Engineer agrees otherwise. In the absence of such advance agreement by the Engineer, any work done or material used without inspection by a Department representative may be ordered exposed for examination and testing, and then corrected or restored, all at the Contractor's expense.

The Contractor shall provide the Engineer the name(s), contact information, and location(s) of any subcontractor(s) fabricating materials or components outside the Project limits for permanent incorporation into the Project. The Contractor shall provide such information sufficiently in advance of such fabrication to allow the Engineer to schedule inspections of said fabrication, and the Contractor shall ensure that such work does not commence until it has confirmed that the Engineer has arranged for adequate inspection at the offsite location(s). Any such work done without inspection by a Department representative may be ordered exposed for examination and testing. If the Engineer then judges that the work requires correction or restoration, the Contractor shall perform such remedial work at its own expense.

If, at any time before the Department's acceptance of the Project, the Engineer requests the Contractor to remove or uncover any portion of the Project work for inspection by the Engineer, the Contractor shall do so. After such inspection is completed, the Contractor shall restore such portions of the work to the condition required by the Contract as construed by the Engineer. If the work or material exposed and inspected under this provision proves acceptable to the Engineer, the Department shall pay the Contractor for any removal, uncovering or restoration of its previous Contract work. The Department shall pay the Contractor for such removal, uncovering, and restoration of the prior work as extra work. If the work or material exposed and inspected proves, in the opinion of the Engineer, not to conform with Contract requirements, the Contractor shall be responsible for the costs of the removal, uncovering, correction and restoration of the work and material in accordance with the Contract or as the Engineer requires.

The fact that the Engineer may have conducted or failed to conduct, or conducted insufficiently or inaccurately, any inspection of Project work will not relieve the Contractor of its responsibility to perform

the Project work properly, to monitor its work and the work of its subcontractors, and to institute and maintain quality control procedures appropriate for the proper execution of Project work.

1.05.11—Removal of Defective or Unauthorized Work: Work that does not conform to the requirements of the Contract shall be remedied in a manner acceptable to the Engineer or removed and replaced at the Contractor's expense in a manner acceptable to the Engineer.

No work shall be done without appropriate lines and grades having been established in the field. Work done contrary to the instructions of the Engineer, work done beyond the lines shown on the plans, or extra work done without the Engineer's prior written direction to perform it will be considered as unauthorized and the Department will not pay for it. Work so done may be ordered removed or replaced at the Contractor's expense.

If the Contractor fails to comply with any order of the Engineer made under the provisions of this Article, the Engineer has the authority to cause unacceptable or unauthorized work to be remedied or removed and replaced by a party or parties other than the Contractor, and to deduct the costs of such activities from any monies due or to become due to the Contractor from the Department or any other agency of the State.

1.05.12—Payrolls: For each week of the Project from the first week during which an employee of the Contractor does Project work to which prevailing wage requirements apply, until the last week on which such an employee does such work, the Contractor shall furnish to the Engineer certified copies of payrolls showing

(a) the names of the employees who worked on the Project and whose work is subject to prevailing wage requirements,
(b) the specific days and hours and numbers of hours that each such employee worked on the Project, and
(c) the amount of money paid to each such employee for Project work.

Each such payroll shall include the statement(s) of compliance with prevailing wage laws required by the State of Connecticut and, if applicable, by the Federal government. Said payrolls must contain all information required by CGS 31-53 (as it may be revised). For contracts subject to Federal prevailing wage requirements, each payroll shall also contain the information required by the Davis Bacon and Related Acts (DBR). All of the payroll requirements in this Article shall also apply to the work of any subcontractor or other party that performs work on the Project site, and the Contractor shall be responsible for ensuring that each such party meets said requirements.

Every Contractor or subcontractor performing Project work is required to post the relevant prevailing wage rates as determined by the State Labor Commissioner and, on federal aid projects, those determined by the United States Secretary of Labor. The wage rate determinations shall be posted in prominent and easily accessible places at the work site.

1.05.13—Examining and Copying Contractor's Records: The Contractor shall permit the Department and its duly-authorized representatives to examine and copy all documents and other records of the Contractor that are relevant to charges for extra work, alleged breaches of Contract, or any formal or informal claim for additional compensation or for damages in connection with the Project.

With the exception noted below, the Contractor shall also permit the Department to examine and copy such of its documents and other records pertaining to the Project as the Department may deem necessary in order to determine whether or not the Contractor has complied with all laws, regulations and other governmental mandates, e.g., those relating to labor compliance, affirmative action programs, and equal employment opportunity. Documents and other records relating to the Project, if they were created prior to the opening of bids for the Contract, and if they are sought by the Department only for the purpose of confirming such compliance with legal requirements, shall, however, not be subject to examination by the Department pursuant to this Article without the consent of the Contractor.

The Contractor further agrees that it shall keep all documents and other records relating to the Project at least until the expiration of 3 years after the date of acceptance of the Project by the Department, as designated in a "Certificate of Acceptance of Work and Acceptance of Project," issued by the Department. If any claims are brought by the Department or the Contractor prior to that expiration, however, the Contractor shall keep all such records until the Department has given the Contractor a full and final release from all pending and potential claims regarding the Project. If the Contractor does not so keep any such records, it may not assert any formal or informal claim for compensation or damages that could have been substantiated or disproven with such records.

The Contractor shall ensure that the requirements of this provision are made applicable to its

subcontractors and suppliers, for the State's benefit, by including the operative language of this Article in its Project subcontracts and purchase agreements.

1.05.14—Termination for Convenience: The State may terminate the Contract whenever the Engineer determines that such termination is in the best interests of the State. Any such termination shall be effected by delivery to the Contractor of a written Notice of Termination specifying the extent to which performance of work under the Contract is terminated and the date upon which said termination shall be effective.

In the case of such a termination, the Department will pay the Contractor at the Contract unit prices for the actual number of units or items of Contract work completed prior to the effective date of termination, or as may be agreed by the parties for such items of work partially completed. No claim for loss of overhead or anticipated profits shall be allowed.

When the volume of work completed is too small to compensate the Contractor under Contract unit prices for its related expenses, the Department may consider reimbursing the Contractor for such expenses.

Materials obtained by the Contractor for the Project, if they have been inspected, tested as required, and accepted by the Engineer, but have not been incorporated into the Project construction, shall, if the Engineer and the Contractor so agree, be purchased by the Department from the Contractor at their actual cost as shown by receipted bills. To this cost shall be added all actual costs for delivery at such points of delivery as may be designated by the Engineer, as shown by actual cost records. If the Engineer does not agree to purchase such materials, the Department shall reimburse the Contractor for any reasonable restocking fees and handling costs incurred by the Contractor in returning said materials to the vendor.

Termination of the Contract shall not relieve the Contractor of its responsibilities for the completed Project, nor shall it relieve the Contractor's surety of its obligation concerning any claims arising out of the work performed, until the requirements of 1.08.13 and 1.08.14 have been met.

1.05.15—Markings for Underground Facilities: In conformance with 16-345 through 16-359 of the Regulations of the PURA, the Contractor is responsible for notifying “Call Before You Dig” prior to commencing any excavation, including milling, reclamation or trenching; and the Contractor shall install a warning tape located a minimum of 12 inches above all conduits, wires, cables, utility pipes, drainage pipes, underdrains, or other facility, unless the excavation’s depth, other underground facilities, or other engineering considerations make this minimum separation unfeasible. The warning tape shall be of durable impervious material, designed to withstand extended underground exposure without material deterioration or fading of color. The tape shall be of the color assigned to the type of facility for surface markings and shall be durably imprinted with an appropriate warning message. The tape shall also comply with the specific requirements of the utility that owns the facility.

All tapes, unless otherwise directed by the specific utility, shall be detectable to a depth of at least 3 feet with a commercial radio-type metal locater.

Assigned colors are:

Green—Storm and sanitary sewers and drainage systems, including force mains and other non-hazardous materials

Blue—Water

Orange—Communication lines or cables, including, but not limited to, those used in, or in connection with, telephone, telegraph, fire signals, cable television, civil defense, data systems, electronic controls and other instrumentation

Red—Electrical power lines, electrical power conduits and other electrical power facilities, traffic signals and appurtenances and illumination facilities

Yellow—Gas, oil petroleum products, steam, compressed air, compressed gases and all other hazardous material except water

Brown—Other

Purple—Radioactive materials

Payment for warning tapes shall be included in the bid price for the pay item of the specific facility for which the tape is used.

1.05.16—Dimensions and Measurements: The Contractor or one of its subcontractors shall verify each dimension that is needed in order to ensure that its work complies with the Contract, and must do so before ordering any material or doing any work for which such dimension is needed. Such dimensions include, but are not limited to, dimensions given on the plans, as well as dimensions of structures in place prior to Project construction or installed in the course of construction. The Contractor or any subcontractor that

finds a discrepancy or error in dimensions must report it promptly to the Engineer and may proceed with affected work only after receiving clarification and direction from the Engineer regarding the matter. Any costs for delays, changes, cutting or repairs that are incurred due to the Contractor's failure to observe the above requirements shall be borne by the Contractor.

1.05.17—Welding: The Contractor shall ensure that all welding of materials permanently incorporated into the work, and welding of materials used temporarily during construction of the work is performed in accordance with the following codes:

(a) American Welding Society (AWS) Structural Welding Code – Steel – ANSI/AWS D1.1: Miscellaneous steel items that are statically loaded including but not limited to columns, and floor beams in buildings, railings, sign supports, cofferdams, tubular items, and modifications to existing statically loaded structures.

(b) AWS Structural Welding Code – Aluminum – AWS D1.2/D1.2M: Any aluminum structure or member including but not limited to brackets, light standards, and poles.

(c) AWS Structural Welding Code – Sheet Steel – AWS D1.3/D1.3M: Sheet steel and cold-formed members 0.18 inch or less in thickness used as, but not limited, to decking and stay-in-place forms.

(d) AWS Structural Welding Code – Reinforcing Steel – AWS D1.4/D1.4M: Steel material used in the reinforcement of cast-in-place or pre-cast Portland cement concrete elements including but not limited to bridge decks, catch basin components, walls, beams, deck units, and girders.

(e) AASHTO/AWS – Bridge Welding Code, AASHTO/AWS D1.5/D1.5M: Steel highway bridges and other dynamically loaded steel structures. Also includes sign supports, and any other fracture critical structure.

The edition governing the work shall be in effect on the date the Contract was advertised for solicitation of bids.

The Contractor is responsible to provide a Certified Welding Inspector and Non Destructive Examination personnel in accordance with the above noted codes. The cost for this service is included in the general cost of the work.

Prior to performing any shop welding on Department projects, all shop welders shall be qualified to weld in accordance with the welding code(s) specified on the approved drawings or otherwise stated in the Contract.

Prior to performing field welding on any permanent or temporary structure or component for Department projects, all field welders, field welding operators, and field tackers must possess a valid Welder Certification card issued by the Department's Division of Materials Testing (DMT), as outlined in the Department's Materials Testing Manual, and must be up to date in the Department's online Welder Database. If a welder has not been engaged in welding operations on a Department project or a project acceptable to the Department over the past 6 months, or does not posses an approved welding certificate dated within the past 12 months from a welding agency acceptable to the Engineer, the field welder shall be required to requalify through examination. The Engineer may require requalification of anyone whose quality of work is in question.

Prior to start of fabrication or field welding, all welding procedures shall be submitted to the Engineer and the DMT for review. Reviewed welding procedures shall be available at all times while welding is being performed.

1.05.18—Structural Steel Fabrication Shop Prequalification: Material fabricated for Department projects, other than Facilities Construction, are required to be produced in a facility with the AISC Quality Management System (QMS) Certification category noted on the plans as Certified Bridge Fabricator – Simple (SBR), Certified Bridge Fabricator- Intermediate (IBR), Certified Bridge Fabricator- Advanced (ABR), or Certified Metal Component Manufacturer (CPT).

SBR or CPT certification is required as a minimum for fabrication of secondary, non-primary load carrying elements such as but not limited to diaphragms, cross frames, lateral bracing in straight or skewed bridges less than 30 degrees, camera light supports and structures, sign and signal light support structures, bridge railings, stairs, walkways, grid decks, drains, scuppers, expansion joints, bearings, ballast plates and mechanical movable bridge equipment.

SBR certification is required as a minimum for fabrication of unspliced rolled beams.

IBR certification is required as a minimum for all other bridges other than unspliced rolled beam bridges.

FC Fracture Critical Endorsement is required when noted on the plans for fabrication of fracture critical members.

Material fabricated for Facilities Construction is required to be produced in a facility with either a Certified Building Fabricator (BU) certification by AISC, or an AC172 accreditation by the International Accreditation Service (IAS).

1.05.23—Requests for Information (RFIs) and Requests for Change (RFCs): The Contractor shall send all RFIs and RFCs to the Engineer in PDF format for review. All RFIs will be responded to within 10 calendar days of receipt by the Engineer. All RFCs will be responded to within 21 calendar days of receipt by the Engineer. If additional information is required from the Contractor for the Engineer to respond to the RFI or RFC, the time to respond to such will begin once the Contractor has provided the additional information.

NOTES

SECTION 1.06
CONTROL OF MATERIALS

1.06.01—Source of Supply, Buy America and Material Quality
1.06.02—Samples and Test
1.06.03—Storage
1.06.04—Defective Materials
1.06.05—Shipping Material
1.06.06—Vacant
1.06.07—Certified Test Reports and Materials Certificates
1.06.08—Warranties, Guarantees and Instruction Sheets

1.06.01—Source of Supply, Buy America and Material Quality:

1. Source of Supply: A source of supply is defined as the original manufacturer of material(s) used within a project. A source of supply may fabricate material(s) such as precast concrete structures or hot mix asphalt from components originally manufactured by others. A broker or distributor of material(s) must not be identified as a source of supply.

The Contractor must notify the Engineer of the proposed source of supply for each of the materials listed on the Anticipated Source of Material (CON-083) Form within 30 days after bid opening. Should it become necessary for the Contractor to obtain material from sources other than those indicated in the submitted Anticipated Source of Material (CON-083) Form, the Contractor shall notify the Engineer. The Engineer reserves the right to request additional information regarding such sources.

If, at any time, the Department discovers that materials from a source of supply do not consistently conform to the Contract specifications, the Engineer will notify the Contractor of its nonconformance and that source of supply shall no longer be used for said application.

2. Buy America: All permanently incorporated steel and iron used in the construction of the Project must have been produced and fabricated in the United States. It is the intent of this specification to require that all manufacturing processes for all steel and iron materials and products to be used for the Project, including the coating of steel and iron, occur within the United States, with the following exceptions:

The Contractor may request, in accordance with 635.410(b)(4) of Title 23 CFR, approval to include a minimal amount of foreign steel in the Project construction. This amount is defined as 1/10 of 1% of the total Contract price or $2,500.00, whichever is greater. The cost of the foreign steel or iron is defined as its Contract value when delivered to the Project site.

Additionally, the FHWA has granted a nationwide waiver of the requirements of 23 CFR 635.410, Buy America requirements, for the production of pig iron and processed, pelletized, and reduced iron ore. Items not specifically included in the waiver remain subject to the Buy America requirements. The Contractor may request the Engineer to seek from the FHWA a further waiver of said requirements, but it shall be at the sole discretion of the Engineer whether or not to seek such a waiver.

3. Material Quality: Only materials conforming to the Contract and accepted by the Engineer shall be permanently incorporated into the Project.

Prior to installation, material that is damaged or otherwise changed in a way that it no longer meets Contract specifications shall not be incorporated into the Project.

When one manufacturer's product is specified in the Contract, it shall be understood that this represents the standard required. Unless otherwise stated, a comparable product of another manufacturer may be proposed by the Contractor unless the plans or special provisions indicate that no equal will be allowed. The Contractor shall submit a complete description of the proposed product, together with shop drawings, catalog cuts, product data or other descriptive literature for review in accordance with 1.05.02. Should a product be designated as an equal, this will not relieve the Contractor from any material testing requirements or a related Certified Test Report and/or Materials Certificate that may be required.

1.06.02—Samples and Test: The Contractor shall furnish all required samples without charge, and provide secure facilities for their storage. The Contractor shall provide means for, and shall assist in the verification of, all scales, measures and other devices that it operates or uses in connection with the Project. The Contractor shall provide calibration documentation when requested.

The minimum number and size of material samples per Contract item that are required by the Department are listed in the "Minimum Schedule for Acceptance Testing" contained in the Department's Materials Testing Manual. The acceptance methods used to determine compliance with the Contract are also listed. Where applicable, physical testing will be performed in accordance with the test standards which are in

effect at the time of bidding, unless otherwise indicated in the Contract. Any item or material not listed in the "Minimum Schedule for Acceptance Testing," or Contract shall be sampled and tested and/or certified, as directed by the Engineer.

Certification may be used as the basis for approval of such materials as the Contract documents specify or as the Engineer may require. With regard to such materials, the Contractor shall furnish the Engineer a Certified Test Report (CTR) and/or Materials Certificate (MC), in accordance with 1.06.07 for each type of material, as may be required in the "Minimum Schedule for Acceptance Testing." The Contractor shall bear any costs involved in furnishing the CTR and MC.

If the Contractor has purchased materials for use on a previous Department project and if they meet the requirements of this Contract, then those materials, with the approval of the Engineer, may be used for the Project, provided that the Contractor, acting as the "supplier," submits a related MC meeting the requirements of 1.06.07. This MC shall further identify the project for which the material was originally purchased and shall be accompanied by a copy of the original MC.

Materials will be rejected by the Engineer whenever, in their judgment, they fail to meet Contract requirements. The Engineer may accept material or the combination of materials and thereby waive noncomplying test results, provided that the following conditions are met:

1. The Engineer finds results of prior and subsequent series of tests of the material or materials from the same source or sources to be satisfactory.
2. The incidence and degree of nonconformance with the Contract requirements are, in the Engineer's judgment, within reasonable limits.
3. The Contractor, in the Engineer's judgment, had diligently exercised material controls consistent with good practices.
4. No adverse effect on the value or serviceability of the completed work could result from said degree of nonconformance.

The Engineer may, in their discretion, waive testing of minor quantities of a particular material if said material was obtained from sources that have furnished supplies of the material that have consistently met Department testing standards.

1.06.03—Storage: The Contractor shall store all materials for the Project in a way that ensures that their quality and fitness for the Project will be preserved, and that the Engineer will have easy and prompt access to them for inspection purposes. Materials shall be kept on wooden platforms or on other hard, clean surfaces and not on the ground. When so directed by the Engineer, the Contractor shall store materials in a weatherproof building.

The Contractor shall not store materials in any way that would lead to a violation of these specifications including but not limited to 1.10. Stored materials, even if they have been approved by the Engineer prior to their storage, must be inspected by the Engineer and meet all pertinent Contract requirements immediately prior to use of those materials for the Project.

1.06.04—Defective Materials: Unless otherwise permitted by the Engineer, all materials not meeting Contract requirements shall be considered defective, shall be rejected, and shall be removed immediately from the Project site.

If deemed necessary, the Engineer may require retesting of materials previously tested, accepted and incorporated into the Project. If materials do not meet the Contract requirements after retesting, the Engineer will make a determination whether to allow materials be left in place (with an equitable reduction of payment) or be removed and replaced. No rejected material, the defects of which have been subsequently corrected, shall be used until approval has been given by the Engineer. Should the Contractor fail to comply with these requirements, the Engineer has the authority to order the removal and replacement of defective material and deduct the cost of such removal and replacement from any future payments to the Contractor.

When a material is fabricated, or treated with another material, or when any combination of materials is assembled to form a product, any or all of which are covered by the Contract specifications, the failure of any components of the product to meet the requirements of the specifications may be sufficient cause for the rejection of the whole combination or product.

1.06.05—Shipping Material: Any conveyance used for transporting materials must be clean when used, be in proper working condition, have a strong and substantial body that will prevent the loss of materials during transportation, and be approved by the Engineer.

1.06.06—Vacant

1.06.07—Certified Test Reports and Materials Certificates: The Contractor shall furnish the Engineer with any Certified Test Report and Materials Certificate required by the Contract and the "Minimum Schedule for Acceptance Testing" contained in the Department's Materials Testing Manual.

The Contractor shall forward the Certified Test Report and Materials Certificate to the Engineer, and, in addition, shall deliver a copy of same to the Department's inspector at the Site. Materials for which such documentation is required may be conditionally incorporated into the Project prior to the Engineer's acceptance of a Certified Test Report and a Materials Certificate; however, payment for such incorporated material may not be made prior to acceptance by the Department of a Certified Test Report and Materials Certificate indicating that the material meets the Contract requirements.

1. A Certified Test Report (CTR) is a document containing a list of the dimensional, chemical, metallurgical, electrical and physical results obtained from a physical test of the materials involved, and shall demonstrate that the materials meet the requirements of the Contract. The CTR shall be signed by a duly-authorized and responsible agent of the original manufacturer of the material(s), and the signature must include the date the CTR was signed and notarized.

The CTR shall also include the following information:

a. Description of material(s)
b. Date of manufacture of the material(s)
c. Date of test(s)
d. Name of organization to which the material has been consigned
e. Quantity of material represented
f. Means of identifying the consignment, such as label, marking, lot number, etc.
g. Date and method of shipment
h. Name of organization performing the test(s)

2. A Materials Certificate (MC) is a document certifying that the materials, components and equipment furnished meet all requirements of the Contract. The MC shall be signed by a duly-authorized and responsible agent of the organization assembling or fabricating the materials and the signature must include the date the MC was signed and notarized. Such MC shall also include the following information:

a. Project for which the material has been consigned
b. Name of Contractor to which material is supplied
c. Item number and description of material
d. Quantity of material represented by the MC
e. Means of identifying the consignment, such as label, marking, lot numbers, etc.
f. Identification of all sources of supply of material components
g. Means of verifying Buy America requirements for steel and/or steel components
h. Date and method of shipment

1.06.08—Warranties, Guarantees and Instruction Sheets: Manufacturers' warranties and guaranties furnished for materials used for the Project, as well as instruction sheets and parts lists supplied with Project materials, shall be delivered to the Engineer prior to acceptance of the Project. Each warranty or guaranty so furnished shall indicate its commencement and expiration dates.

SECTION 1.07
LEGAL RELATIONS AND RESPONSIBILITIES

1.07.01—Laws to Be Observed
1.07.02—Permits and Licenses
1.07.03—Proprietary Devices, Materials and Processes
1.07.04—Restoration of Surfaces Opened Pursuant to Permit or Contract
1.07.05—Load Restrictions
1.07.06—Sanitary Provisions
1.07.07—Safety and Public Convenience
1.07.08—Use of Explosives
1.07.09—Protection and Restoration of Property
1.07.10—Contractor's Duty to Indemnify the State against Claims for Injury or Damage
1.07.11—Opening of Section of Project to Traffic or Occupancy
1.07.12—Contractor's Responsibility for Work
1.07.13—Contractor's Responsibility for Adjacent Property, Facilities and Services
1.07.14—Personal Liability of Representatives of the State
1.07.15—No Waiver of Legal Rights
1.07.16—Unauthorized Use of Area(s) within the Project Site
1.07.17—Vacant
1.07.18—Use of State Property

1.07.01—Laws to Be Observed: The Contractor at all times shall observe and comply with all laws, ordinances, government bylaws, permits, regulations, orders and decrees which in any manner affect the conduct of the Contract work. The Contractor shall indemnify and save harmless the State and all of its officers, employees and agents against any claim, fine, or other liability arising from or based on the violation of any such law, bylaw, permit, ordinance, regulation, order or decree, whether by the Contractor, its subcontractors or any of their officers, employees or agents. See the last paragraph in 1.02.04, however, regarding conflicts between municipal law or authorities and the requirements of Project construction.

1.07.02—Permits and Licenses: Except as may be provided otherwise in a specific Contract provision or a written direction from the Engineer, the Contractor shall procure all permits and licenses, pay all charges and fees, and give all notices required by government authorities in connection with the due prosecution of the Project.

Under Connecticut law, a commercial vehicle used by a contractor or vendor in connection with the Project may be subject to Connecticut registration requirements. The CGS require such registration for any vehicle that most often is garaged in this State, or that most often leaves from and returns to one or more points within this State in the normal course of its operation. In addition, a vehicle must be registered in Connecticut if it continuously receives and discharges cargo within this State. Reciprocal registrations as allowed under CGS are acceptable for meeting the registration requirements.

Residence or domicile of the owner, lessor or lessee of the motor vehicle, or the place where the owner, lessor or lessee is incorporated or organized, shall not be a factor in determining whether or not the vehicle must be registered in this State. Failure to register a vehicle, if the law requires it, may result in issuance of a citation for such an infraction, and also may result in administrative action by the Commissioner of Motor Vehicles.

The registration requirement applies not only to the Contractor, but also to its subcontractors, suppliers, and other agents and representatives. It is the Contractor's responsibility to ensure that such entities and individuals comply with this requirement as well. The Contractor shall maintain, on the Project site, records that document compliance with this requirement in connection with all vehicles used for the Project.

1.07.03—Proprietary Devices, Materials and Processes: If the Contractor is required or desires to use any design, device, material or process covered by another party's license, patent, copyright or trademark, the Contractor shall provide for such use by suitable legal agreement with the license, patent, copyright or trademark holder.

The Contractor shall provide a copy of any and all such agreements to the Engineer.

If the Contractor is allowed, but not specifically required by the Engineer, to use any particular proprietor's design, device, material or process covered by license, patent, copyright or trademark, the Contractor and its surety shall indemnify and save harmless the State from any and all claims that may be

brought against the State, and any and all costs, expenses, and damages that the State may be obligated to pay by reason of any infringement or alleged infringement relating to the use of such licensed, patented, copyrighted or trademarked design, device, material or process at any time during the prosecution or after the completion of the Project.

1.07.04—Restoration of Surfaces Opened Pursuant to Permit or Contract: The Contractor shall not make, and shall not allow any person to make, an opening in a highway unless written and duly-authorized permission to do so has been obtained from the Department. If at any time prior to the completion of the Project, the Contractor should make such an opening without such permission, the Contractor shall perform all restoration necessary to close said opening, at its own expense, if the Engineer directs it to do so.

1.07.05—Load Restrictions

(a) Vehicle Weights: This subarticle will apply to travel both on existing pavements and pavements under construction. The Contractor shall comply with all legal load restrictions as to vehicle size, the gross weight of vehicles, and the axle weight of vehicles while hauling materials. Throughout the duration of the Contract, the Contractor shall take precautions to ensure existing and newly installed roadway structures and appurtenances are not damaged by construction vehicles or operations.

Unless otherwise noted in Contract specifications or plans, on and off road equipment of the Contractor, either loaded or unloaded, will not be allowed to travel across any bridge or on any highway when such a vehicle exceeds the statutory limit or posted limit of such bridge or highway. Should such movement of equipment become necessary the Contractor shall apply for a permit from the Department for such travel, as provided in the CGS. The movement of any such vehicles within the Project limits or detour routes shall be submitted to the Engineer for Project record. Such permit or submittal will not excuse the Contractor from liability for damage to the highway caused by its equipment.

The Contractor is subject to fines, assessments and other penalties that may be levied as a result of violations by its employees or agents of the legal restrictions as to vehicle size and weight.

(b) Storage of Construction Materials/Equipment on Structures: Storage is determined to be non-operating equipment or material. The Contractor shall not exceed the statutory limit or posted limit for either an existing or new structure when storing materials and/or construction equipment. When a structure is not posted, then the maximum weight of equipment or materials stored in each 12 foot wide travel lane of any given span shall be limited to 750 pounds per linear foot combined with a 20,000 pound concentrated load located anywhere within the subject lane. If anticipated storage of equipment or material exceeds the above provision, then the Contractor shall submit its proposal of storage supported by calculations stamped by a Professional Engineer registered in the State of Connecticut, to the Engineer for approval 14 days prior to the storage operation. Operations related to structural steel demolition or erection shall follow the guidelines under 6.03. All other submittals shall include a detailed description of the material/equipment to be stored, the quantity of storage if it is stockpiled materials, the storage location, gross weight with supporting calculations if applicable, anticipated duration of storage and any environmental safety, or traffic protection that may be required. Storage location on the structure shall be clearly defined in the field. If structures are in a state of staged construction or demolition, additional structural analysis may be required prior to authorization of storage.

1.07.06—Sanitary Provisions: The Contractor shall provide and maintain in a neat and sanitary condition such accommodations for the use of its employees as may be necessary to comply with the regulations and other requirements of the State Department of Public Health or of other bodies or tribunals having jurisdiction over such matters.

1.07.07—Safety and Public Convenience: The Contractor shall conduct the Project work at all times in such a manner as to ensure the least possible obstruction to traffic. In a manner acceptable to the Engineer, the Contractor shall provide for the convenience and interests of the general public; the traveling public; parties residing along or adjacent to the highway or Project site; and parties owning, occupying or using property adjacent to the Project site, such as commuters, workers, tenants, lessors and operating agencies.

Notwithstanding any other Contract provision, the Contractor shall not close to normal pedestrian or vehicular traffic any section of road, access drive, parking lot, sidewalk, station platform, railroad track, bus stop, runway, taxiway, occupied space within a site, or occupied space within a building, except with the written permission of the Engineer.

All equipment, materials, equipment or material storage areas, and work areas must be placed, located, and used in ways that do not create a hazard to people or property, especially in areas open to public

pedestrian or vehicular traffic. All equipment and materials shall be placed or stored in such a way and in such locations as will not create a hazard to the traveling public. In an area unprotected by barriers or other means, equipment and materials must not be stored within the clear zone of any traveled way. Clear zones are based on design speed and roadway geometry. The following minimum distances, measured from edge of travelway to the temporary hazard, are recommended:

Posted Speed Limit, mph	Distance, feet
55 or more	30
45 to 55	24
under 45	16

The Contractor must always erect barriers and warning signs between any of its work or storage areas and any area open to public, pedestrian, or vehicular traffic. Such barriers and signs must comply with all laws and regulations, including any applicable codes.

The Contractor must arrange for temporary lighting, snow and ice removal, security against vandalism and theft, and protection against excessive precipitation runoff within its Project work and storage areas, and within other areas specifically designated in the Contract.

In addition to meeting the requirements of 9.71, the Contractor shall take all precautions necessary and reasonable for the protection of all persons, including, but not limited to, employees of the Contractor or the Department, and for the protection of property, until the Engineer notifies the Contractor in writing that the Project or the pertinent portion of the Project has been completed to the Engineer's satisfaction. The Contractor shall comply with the safety provisions of applicable laws, including building and construction codes and the latest edition of the CFR. All Contractor personnel working on limited access roadways shall wear ANSI Class 3 (high visibility) protective clothing at all times. The Contractor must make available for reference in its field office, throughout the duration of the Project, a copy of the Safety Plan and the latest edition, including all supplements, of the CFR pertaining to OSHA.

The Contractor shall furnish to the Engineer's representative supervising the Project a report on any accident that occurs on the Project site with regard to which the Contractor is required to report under OSHA or any other legal requirement. The Contractor shall also furnish to the Engineer a report regarding any other accident involving public liability or property damage in connection with the Project. The form and detail of such reports must be acceptable to the Engineer.

The Contractor shall designate a competent representative with authority to act in cooperation with the Department in the enforcement of safety provisions and promotion of safe practices on and related to the Project throughout the duration of the Project.

Before beginning work on the Project, the Contractor shall have a Safety Plan on file with the Department. The Safety Plan shall include the policies and procedures necessary for the Contractor to comply with OSHA and other pertinent regulatory rules, regulations and guidelines. The Safety Plan may be a comprehensive company-wide plan provided it addresses the scope and type of work contemplated by the Contract. The Safety Plan shall address all the requirements of this Section and any applicable State or Federal regulations, and shall be revised and updated as necessary.

The following elements shall be included in the Safety Plan:

1. General introduction describing the scope and applicability of the Safety Plan.
2. Identification of key staff responsible for the implementation and monitoring of the Contractor's Safety Plan, and their roles and responsibilities for safety.
3. Training requirements relative to safety.
4. Safety rules and checklists specific to the types of work generally performed by the Contractor.
5. Record-keeping and reporting requirements.
6. Identification of special hazards related to specific work elements.

The Contractor is responsible for the Safety Plan. Pursuant to 1.07.10, the Contractor shall indemnify, and save harmless the State from any and all liability related to any violation of the Safety Plan.

Under 1.08.06, the Engineer may suspend the work of the Contractor if and when the latter does not take the safety precautions referenced in this article. Nothing herein shall be construed, however, to relieve the Contractor from responsibility for the prosecution of the Project.

1.07.08—Use of Explosives: To the extent possible, the Contractor shall avoid using explosives in proximity to existing structures. When the use of explosives is necessary for the prosecution of the Project, the Contractor shall take the utmost care not to endanger life or property.

The Contractor shall take adequate protective measures when engaging in blasting operations, and shall be responsible for any damage resulting from such operations.

The Contractor shall notify each utility with facilities in proximity to the site of such blasting operations, and any other individuals and entities that may be affected thereby, of the Contractor's intention to use explosives. Such notice shall be given sufficiently in advance of any blasting to enable such affected parties to take steps to prevent such blasting from injuring persons or property. Such notice shall not relieve the Contractor of responsibility for damage resulting from its blasting operations.

1.07.09—Protection and Restoration of Property: The Contractor shall not enter upon private property for any purpose without having obtained written permission to do so from the owner of such property and having provided the Engineer with a copy of same. The Contractor shall use every reasonable precaution to avoid disturbing or damaging public or private property, including, but not limited to, trees and monuments. The Contractor shall use suitable precautions to avoid disturbing or damaging underground or overhead structures or facilities, whether or not they are shown on the plans.

If the Project requires the moving or removal of a land monument or property marker, the Contractor shall not disturb it until a duly-authorized agent of the public or private property's owner has witnessed or recorded the monument or marker's location. The Contractor shall not move or remove such property until and unless directed to do so by the Engineer.

The Contractor shall not remove, cut, injure or destroy trees or shrubs without the Engineer's prior approval.

The Contractor shall be responsible for all damage to property resulting from any act, omission, neglect or misconduct in the Contractor's manner or method of executing its work, or due to its defective work or materials. When or where any direct or indirect damage is done to public or private property by or on account of any act, omission, neglect, or misconduct in the execution of the Project work, the Contractor shall restore, at its own expense, such property to a condition as close as possible to that which existed before such damage was done, by repairing, rebuilding or otherwise restoring the property, as may be directed by the Engineer; or the Contractor shall make good such damage in another manner acceptable to the Engineer. If the Contractor fails to restore such property or make good such damage in a way acceptable to the Engineer, the Engineer may, upon 48 hours' notice, proceed to have such property repaired, rebuilt or restored as they may deem necessary; and the cost thereof will be deducted from any monies due or which may become due the Contractor under the Contract or under any other contract(s) that the Contractor may have with the State.

The Engineer shall mark the locations of underground facilities belonging to the State when given 72 hours' (excluding Saturdays, Sundays, and State holidays) notice by the Contractor that it will be excavating or driving material into the ground near such facilities as a part of necessary Contract work. After the Engineer marks the location of such facilities, it will be the Contractor's responsibility to maintain the location markers until no longer needed. Repairs of State facilities located further than 1 foot from the line delineated by such markers shall be paid for by the State.

1.07.10—Contractor's Duty to Indemnify the State against Claims for Injury or Damage: The Contractor shall indemnify and save harmless the State, the Department and all of its officers, employees and agents from all suits, actions or claims of any character, name or description brought for or on account of any injury or damage caused to any person or property as a result of, in connection with, or pursuant to the performance of the Contract, including all costs incurred by the State in defending itself against such claims or actions, in proportion to the extent that the Contractor is held liable for same by an arbiter of competent jurisdiction. As much of any money that may be due the Contractor under the Contract as the Commissioner considers necessary for the purpose of such indemnification or holding the State harmless may be retained for such use by the State; and the Contractor's surety bonds may be held until such suit or suits, action or actions, claim or claims, as aforesaid, shall have been settled and until the Contractor has furnished to the Commissioner suitable evidence to that effect. Such indemnity shall not be limited by reason of any insurance coverage required under the Contract.

It is further understood and agreed by the parties hereto, that the Contractor shall not use the defense of Sovereign Immunity in the adjustment of claims or in the defense of any suit, including any suit between the State and the Contractor, unless requested to do so by the State.

1.07.11—Opening of Section of Project to Traffic or Occupancy: Whenever, in the opinion of the Engineer, any portion of the Project has been substantially completed, it may be opened to traffic or occupancy as directed by the Engineer. The Engineer's approval of any such opening shall not be held to

be in any way an acceptance of such completed portion of the Project, or as a waiver of any of the provisions of these Specifications, or of any state or federal statutes, applicable building codes, or other Contract provisions. Such approval shall not constitute a basis for claims for damages due to interruptions to, or interference with, the Contractor's operations.

If repair or replacement of any portion of the Project construction becomes necessary because the Engineer has directed that said portion be opened to travel or occupancy prior to completion of the Contract work, the Contractor shall perform that repair or replacement. The Contractor shall perform such work at its own expense, unless the Department or an arbiter of competent jurisdiction shall determine definitely that the damage necessitating the repair or replacement was caused by equipment operated by a State employee while controlling snow or ice, or by routine State maintenance operations. In the latter cases, the State shall reimburse the Contractor for the cost of the repair or replacement. If the damage was caused by a traffic accident involving only a vehicle or vehicles that were not owned by the State and were not operated by an agent of the State, the Contractor may seek recovery from the responsible parties, but not from the State.

1.07.12—Contractor's Responsibility for Work: From the date for commencement of construction given in the "Notice to Proceed" until the date when the Engineer relieves the Contractor of responsibility for the Project, the Project construction and site shall be under the charge and care of the Contractor; and the Contractor shall take every necessary precaution against damage to the same or any part thereof by the action of the elements or from any other cause, including either execution or non-execution of Project work. The Contractor shall rebuild, repair, restore or otherwise make good, at its own expense, all damage to, or impairment of, any portion or purpose of the Project which results from any of the above causes prior to completion of the Project, except as provided in 1.07.11.

1.07.13—Contractor's Responsibility for Adjacent Property, Facilities and Services: The Project work shall not commence until the Contractor has made all arrangements necessary to protect all property and facilities adjacent to the Site, including, but not limited to, those of utilities, from damaging or disruptive effects of Project operations. The Contractor shall cooperate with the owners of any underground or overhead utility lines in their removal and rearrangement operations in order that these operations may progress in a reasonable manner, that duplication of such utilities work may be kept to a minimum, and that services rendered by those parties will not be unnecessarily interrupted.

In the event of interruption of water or utility services as a result of accidental breakage of facilities, or as a result of related facilities being exposed or unsupported, the Contractor shall promptly notify the proper utility and law enforcement authorities and the Engineer of same. The Contractor shall cooperate with said authorities in the restoration of such services as promptly as possible. In no case shall the Contractor leave the Site until the interrupted service has been restored. Fire hydrants shall be kept accessible at all times, and no materials shall be left within 15 feet of any fire hydrant.

1.07.14—Personal Liability of Representatives of the State: In carrying out any of the provisions of these Specifications, or in exercising any power or authority granted by the Contract, or by law or regulation, the Commissioner, Engineer, and their authorized representatives, including consultant engineering firms and their employees, shall be subject to no liability, either personally or as officials of the State, it being understood that in all such matters they act solely as agents and representatives of the State.

1.07.15—No Waiver of Legal Rights: The Commissioner reserves the right, should the Department discover an error in the estimate, or conclusive proof of defective work done or material used by or on behalf of the Contractor, either before or after the acceptance of the Contract, or even after the final payment has been made to the Contractor, to retain and apply monies owed to the Contractor under any State contract, or to claim and recover by process of law such sums, in order to correct any error or make good any defects in the Project work or materials.

1.07.16—Unauthorized Use of Area(s) Within the Project Site: The use of any area within the Project site for any purpose other than the construction of the Project, without prior written authorization to do so from the Commissioner, is prohibited.

Any request by the Contractor for authorization of such special use must include details describing the proposed use. If the proposed special use would involve the Contractor’s making any lease or any profits in connection with the proposed use, the Contractor must enter into an agreement with the State for an equitable sharing of any profits with the State before such use may be authorized.

1.07.17—Vacant

1.07.18—Use of State Property: The Contractor may not use State property for any purpose or activity other than carrying out the construction activities required by the Contract, except with the prior written consent of the Engineer.

Such other activities, which require the Engineer's advance consent, include, but are not limited to, the establishment of staging areas, storage areas, asphalt plants, concrete plants, or gravel/borrow pits; or the conduct of screening, crushing, manufacturing, or mining operations.

Any permitted use of the Project site or other State property for such other purposes or activities must be for the performance of the specific Contract only, and must be at no cost to the State. In addition, the Contractor may not assert or bring any claim or formal proceeding for damages or additional compensation based on either the approval or denial of a request to make such use of the Project site or other State property.

Under no circumstances shall the bulk storage of fuel or lubricants by the Contractor or its agents be permitted on State property. The Contractor shall not conduct work processes or store any construction materials or supplies of such types, quantities or configuration, either individually or in total, on, under or near a structure, that creates an unreasonable risk of substantial damage to State property. The Contractor shall not store any hazardous materials on State property other than those that are integral to the Contractor's performance of the Contract, as allowed by the Contract and in accordance with 1.10, or in writing from the Engineer. The Contractor shall have the responsibility and duty to ensure the proper storage, handling, management and disposal of any such hazardous materials. The Contractor shall be liable to the Department for all remedial or punitive costs, damages or penalties incurred by the Department as a result of the Contractor's failure to fulfill this duty.

The Engineer may require environmental testing of the affected site at the Contractor's expense both prior to and upon completion of the Contractor's permitted use of the site or of other related State property. The Contractor shall be responsible for ensuring that such a site is restored to the condition required by the Engineer and that all contaminants deposited on the site by the Contractor or its agents are removed and properly disposed of. All such restoration and removal activities must be carried out at the Contractor's expense, and must be carried out in accordance with the provisions of the Department's Required Best Management Practices, any applicable environmental permits, and all other applicable State or Federal laws or regulations.

The Contractor must submit any request to use State property for a staging or storage area to the District Engineer at the District Construction Office. The following information, at a minimum, must accompany such written request:

(a) A detailed description of the proposed operation or use of State property.

(b) A site plan detailing the proposed location of any operations, materials, or facilities related to the requested use, including any appropriate sedimentation or erosion controls.

(c) An area plan detailing anticipated ingress to and egress from the site of the proposed activity or the Project site, as appropriate, and indicating the location of and proximity to residential or occupied buildings in the vicinity.

(d) Copies of any related, required or affected environmental permits.

(e) A detailed listing or description of the anticipated dates and hours of the proposed operations or activities.

(f) Photo documentation (a minimum of 12 - 8 inch × 10 inch color photographs) as follows:

(i) the preconstruction condition of each site of the proposed activities and

(ii) adjacent property at the boundaries of those areas.

If the site to be used or affected is State property that lies outside of any Department right-of-way, the Contractor must also obtain from other State agencies all necessary or appropriate authorizations for the proposed use(s) of State property.

Any request by the Contractor relating to a proposed use of State property for activities other than the establishment of a construction staging or storage area must also be submitted to the District Engineer at the District Construction Office, and must include the same information required by (a) through (f) of the preceding paragraph. In addition, in connection with such other requests, the Contractor must submit to the District Engineer

(a) written confirmation from the municipality or municipalities in which each affected site is located that each such municipality has no objection to the proposed use or activity; and

(b) a license agreement with the Department, executed by the Contractor, on terms acceptable to the Department, defining the nature and scope of the proposed use or activity.

Gore areas are not available for disposal of surplus material.

For any request to establish or operate an asphalt batching or continuous mix facility, the Contractor must also provide to the District Engineer at the District Construction Office a map detailing the outermost perimeter of the proposed facilities and operations, showing all related and potentially-affected structures, land uses, watercourses, wetlands, and other areas of environmental concern within 1/3 of a mile of the facility or operation perimeter. No such facility will be permitted on State property where any hospital, nursing home, school, area of environmental concern, watercourse, or residential housing exists within the perimeter of 1/3 mile from the facility or operation (as per Public Act 98-216).

SECTION 1.08
PROSECUTION AND PROGRESS

1.08.01—Transfer of Work or Contract
1.08.02—Establishment of Construction Field Office
1.08.03—Prosecution of Work
1.08.04—Limitation of Operations
1.08.05—Personnel and Equipment
1.08.06—Suspensions of Work Ordered by the Engineer
1.08.07—Determination of Contract Time
1.08.08—Extension of Time
1.08.09—Failure to Complete Work on Time
1.08.10—Termination of Contract for Cause
1.08.11—Final Cleaning Up
1.08.12—Final Inspection
1.08.13—Termination of the Contractor's Responsibility
1.08.14—Acceptance of Project

1.08.01—Transfer of Work or Contract: The Contractor shall perform with its own organization Contract work with a value under the Contract of at least 50% of the original total Contract value. If the Contractor sublets, sells, transfers, or otherwise disposes of any part of the Contract work without the Commissioner's prior written consent, the Contractor will not be relieved of any Contractual or other legal responsibility in connection therewith. Such an unauthorized act by the Contractor shall constitute a material breach of the Contract, and the Commissioner may, in such a case, terminate the Contract without further compensation to the Contractor.

The Contractor shall include the following alternative dispute resolution clause in all of its Project subcontracts:

"*For any dispute arising out of the agreement between the Contractor and a subcontractor, including claims of late payment or non-payment, which cannot be settled within 60 days of the subcontractor submitting a written claim to the Contractor, either party may bring the dispute before an alternative dispute resolution entity for resolution. If the parties do not agree upon a particular dispute resolution entity for that purpose, the dispute shall be resolved under the auspices and construction arbitration rules of the American Arbitration Association, or under the rules of any other alternative dispute resolution entity approved by the Department either generally or for the specific dispute. The Department may not be made a party to formal arbitration regarding such a dispute. These rights and restrictions may not be waived, and if these provisions are not included in the Contractor's subcontracts for the Project, these provisions shall nonetheless be read into them.*"

The Contractor shall not knowingly enter into any lower-tier transaction on a Department project with any person or entity which, under any federal or state law or regulation, or by voluntary agreement, is currently debarred or disqualified from bidding for construction contracts or participating in construction projects in any jurisdiction within the United States, unless after disclosure of such ineligibility, such participation is authorized by appropriate federal and State authorities, including the Commissioner.

The Contractor shall not sublet, sell, transfer, assign, or otherwise dispose of the Contract or any portion thereof, or of the work provided for therein, or of its right, title, or interest therein, to any individual or entity without the written consent of the Commissioner. No payment will be made for such work until written consent is provided by the Commissioner.

The Contractor shall pay the subcontractor for work performed within 30 days after the Contractor receives payment for the work performed by the subcontractor. Withholding retainage by the Contractor, subcontractor or lower tier subcontractors is not allowed.

Payment for work that has been performed by a subcontractor does not eliminate the Contractor’s responsibilities for all the work as defined in 1.07.12, “Contractor’s Responsibility for Work.”

Payment for work that has been performed by a subcontractor also does not release the subcontractor from its responsibility for maintenance and other periods of subcontractor responsibility specified for the subcontractor’s items of work. Failure of a subcontractor to meet its maintenance, warranty or defective work responsibilities may result in administrative action on future Department contracts.

For any dispute regarding prompt payment, the alternate dispute resolution provisions of this article shall apply.

The above requirements are also applicable to all sub-tier subcontractors and the above provisions shall be made a part of all subcontract agreements.

Failure of the Contractor to comply with the provisions of this section may result in a finding that the Contractor is nonresponsible as a bidder for a Department contract.

1.08.02—Establishment of Construction Field Office: Prior to the start of Project construction, and within 10 calendar days after the signing of the Contract by the parties, the Contractor shall propose in writing to the Engineer a field office location. The proposal shall include the office telephone number to be used, the nearest utility pole number, and the distance from that pole to the proposed field office. The office shall be made acceptable to the Engineer and available for use, including all utility hookups, local permits and inspections, within 30 days of the Engineer's order to establish the office. Such order shall not be deemed the "Notice to Proceed."

1.08.03—Prosecution of Work: The Contractor shall commence construction operations with that part of the Project designated for such commencement in the progress schedule which it has submitted to the Department, unless the Engineer directs the Contractor to commence with a different part of the Project. The work shall be conducted in such manner and with sufficient materials, equipment and labor as are necessary to ensure completion of the Project in accordance with the Contract within the time set forth in the Contract. The Contractor shall notify the Engineer of its intention to commence or recommence any Project operation at least 48 hours in advance of doing so. The Contractor shall also give the Engineer such advance notice of any intent to discontinue any Project operation, unless emergency conditions make it impracticable to give such notice so far in advance. The Engineer retains the right to disallow such commencement, recommencement or discontinuance of operations.

1.08.04—Limitation of Operations: The Contractor shall plan and perform the Project work in such a manner and in such sequence as will cause as little interference as is practicable with vehicular, railroad, aircraft, pedestrian or other traffic. The Contractor shall cooperate with any utilities involved in or affected by the Project operations, and shall schedule its operations in accordance with 1.05.06.

The Contractor shall give the Engineer 7 days' advance written notice of any proposed changes in Project activities that will alter vehicular traffic patterns, causing lane shifts, detours, temporary closure of a lane, permanent closure of a lane or lane reductions, or any other alteration of railroad, aircraft, pedestrian or other traffic patterns affecting usage of such a transportation facility by the traveling public. This advance notification will allow the Department to publish news releases and provide public radio announcements to inform the public of revised traffic patterns or possible traffic delays. Failure of the Contractor to provide such timely notice will subject the Contractor to stop work orders until such time as the 7 days of required notice have run from either the Contractor's giving of the relevant notice or the Department's discovery of the pertinent alteration of traffic conditions.

1.08.05—Personnel and Equipment: The Contractor shall assign to the Project only personnel who are careful and competent. The Engineer may demand the removal of any person employed by the Contractor on the Project who is, in the opinion of the Engineer, guilty of misconduct on a Department project or incompetent or negligent in the performance of his duties on a Department project or any portion thereof, or who neglects or refuses to comply with directions given by the Engineer in connection with the Project. Following such a demand for his removal, such person shall not work again on the Project without the prior written consent of the Engineer. Should the Contractor, following such a demand for removal, continue to employ or again employ such person on any Department project without the required consent of the Engineer, the Commissioner may withhold all estimated payments that are or may become due to the Contractor for the Project, or the Engineer may shut down the Project until the Contractor has complied with the Engineer's orders concerning that person. The use of convict labor on projects funded in whole or in part by the federal government is prohibited.

The Contractor shall furnish whatever equipment is necessary for the Project to be performed in a manner and at a rate of progress that is acceptable to the Engineer. Equipment used on any portion of the Project shall not be used in any way that may cause injury to the roadway, adjacent property, or other property on or adjacent to the Project site, unless such damage is allowed by the Engineer for the performance of the Project.

The Contractor may submit to the Engineer a request to use equipment or methods other than those specified in the Contract. If the Engineer so directs, there shall be a trial of such equipment or methods. If the results of the trial are satisfactory to the Engineer, the Contractor may begin using the proposed

equipment or method on the Project. Failure of the equipment or method to meet the specified Contract performance standards in the course of the specified trial, or, in the absence of such standards, a failure to perform to the satisfaction of the Engineer, shall be cause for rejection of any such method or equipment, and any work performed with either. Such rejected equipment or work shall be removed immediately from the Project site.

1.08.06—Suspensions of Work Ordered by the Engineer: The Engineer may suspend the Project wholly or in part, for such period or periods as the Engineer considers to be in the best interests of the State, including, but not limited to, the interests of public necessity, convenience, or safety.

During such periods of suspension, and subject to any related directions from the Engineer, the Contractor shall store all materials and equipment in a way that will keep them from obstructing or impeding the traveling public unnecessarily, and that will keep the materials from being damaged; shall take all reasonable measures to prevent damage to the work performed; shall provide suitable drainage of the roadway and Project site by opening ditches, shoulder drains, etc., as appropriate; and shall erect temporary structures to prevent damage to the Project or to other property, and to protect the public, where and when necessary.

The Contractor shall maintain the Project site and all roadways and buildings thereon in a condition safe for travel or occupancy, and shall maintain all required barricades, signs, and lights during the period of suspension:

(1) If the Engineer orders in writing that performance of all or any portion of the Project shall be suspended, or that it shall be delayed for an unreasonable period of time (not customary, within the scope of possibilities that an experienced contractor should know might occur on a construction project, or inherent in the nature of construction activities), and if the Contractor believes that additional compensation or Contract time is due to it as a result of such suspension or delay, the Contractor shall submit to the Engineer in writing a request for a related Contract adjustment within 7 calendar days of the Contractor's receipt of a direction from the Engineer to resume work. The request shall set forth the specific reasons and support for the requested adjustment.

(2) Upon his receipt of the Contractor's request, the Engineer will evaluate the request. If the Engineer agrees that the expenditures or time required for the Contractor's performance of the Contract have increased as a result of such suspension or delay, and if the suspension or delay was caused by conditions beyond the control of and not the fault of the Contractor, its suppliers, or subcontractors at any approved tier, and was not caused by weather, the Engineer will make an appropriate adjustment (excluding any profit) of the written terms of the Contract. The Engineer will give the Contractor written notice of his determination as to whether or not the requested adjustment of the Contract is warranted and will be made.

(3) No Contract adjustment will be made unless the Contractor has submitted the request for adjustment within the time prescribed.

(4) In addition to the other limits and requirements imposed by this article, no Contract adjustment will be allowed under this article to the extent

  (a) that the Project work would have been suspended or delayed by any cause other than the ones identified in the Contractor's request, or

  (b) that the requested adjustment or type of adjustment is provided for or barred by another provision of the Contract.

(With regard to Items 1-4 above, refer to 23 CFR, 635.109, "Standardized Changed Condition Clauses," Required FHWA Contract Specification.)

1.08.07—Determination of Contract Time: Unless the Contract requires the Project completion by a specified date, the number of calendar days allowed for the completion of the Project will be fixed by the Department, will be stated in the Contract, and will be known (with any subsequent adjustments) as the "Contract time." If at any time the Contractor submits a schedule showing completion of the work more than 30 calendar days in advance of the Contract completion date, the Department will issue a no-cost construction order revising the allowable Contract time to that shown on the Contractor's schedule.

When the Contract time is stated on a calendar-day basis, that time shall be the number of consecutive calendar days contained in the Contract period designated in the Contract, excluding the time period from each December 1 through the following March 31 (the "winter shutdown period"). The Contract time will begin to run on the date designated in the Engineer's "Notice to Proceed" as the date for commencement of the Project, and the time will be computed as herein provided on a consecutive-day basis, including all

Saturdays, Sundays, holidays, and non-work days from April 1 through November 30 of each included year.

Time will not be charged for days in the winter shutdown period, except for time used in working on planting projects and any other projects with regard to which the Standard Specifications or Project bid documents provide otherwise. If the Engineer so approves, the Contractor may work on the Project during the winter shutdown period with no charge being made against the Contract time.

If the time specified for completion of the Project, with time extensions, is due to expire before December 1 of a given year, but the Project has not been completed by that December 1, the time charged to the Contractor will continue through the winter shutdown period.

The total elapsed time in calendar days, computed as described above, from the commencement date specified in the Engineer's "Notice to Proceed" to the "Substantial Completion" date specified in the Engineer's "Notice of Substantial Completion" shall be considered as the time used in the performance of the Contract work.

Suspension involving cessation of work on all items, except minor construction not affected by or connected with the cause of suspension, shall be considered as total suspension. In case of a total suspension of the Project ordered by the Engineer, not due to any fault of the Contractor, the elapsed time during which the Project is suspended will not be charged against the Contract time. Work of an emergency nature ordered by the Engineer for the convenience or safety of the public or the protection of the Project work, if performed during a period of total suspension, will not be charged against Contract time. No such time allowance will be granted in case of partial suspension; provided, however, that the Contractor may request and the Engineer may grant permission to perform specific limited operations during such a partial suspension, in which case Contract time chargeable for those operations shall be negotiated and agreed to in writing before such operations may commence.

1.08.08—Extension of Time: The Contractor may present to the Engineer a request in writing for an extension of Contract time if the time necessary for completion of the Project has been increased due to extra or added work or delays resulting from unforeseeable causes beyond the control and without the fault or negligence of the Contractor, except for weather or seasonal conditions (unless extraordinary and catastrophic). Such causes include, but are not limited to, natural catastrophes, acts of the State in either its sovereign or contractual capacity, acts of another contractor in the performance of a contract with the State, the presence of utility facilities (including railroads), fires, strikes, floods, or delays by suppliers arising from unforeseeable causes beyond the control and without the fault or negligence of either the Contractor or such suppliers.

The Contractor's plea that insufficient Contract time was allowed under the Contract before commencement of the Project is not a valid reason for extending the Contract time. Requests for an extension of time, with adequate substantiation, must be presented within 60 calendar days from the event that is the basis of the request or from the first effect of such an event on the Project. The Contractor will be responsible for providing all the documentation necessary to support the reasonableness of the additional time requested. This shall include a Critical Path Method Schedule Analysis and accompanying narrative that includes the specific dates and number of days for which the extension is sought, the basis or bases for the extension, and the schedule analysis illustrated in a graphic representation of the schedule impacts such as a bar chart or other type of graphical schedule. The critical path is a sequence of activities in a project wherein none of the activities can be delayed without affecting the final project end date.

Such requests will be considered by the Engineer and granted to the extent that the Engineer deems to be fair and reasonable. Requests will not be considered if based on delays caused solely by conditions existing at the time the bids were received and of which the Contractor might reasonably be expected to have had full knowledge at the time, or upon delays caused by failure on the part of the Contractor to anticipate properly the requirements of the Project as to materials, labor, or equipment. For all Project delays or time increases, except as provided below, additional Contract time is the sole remedy that the Contractor may have, and such periods of additional Contract time shall be deemed "Non-Compensable Delays." For delays caused by the State in its Contractual capacity, the Contractor may, in addition to a time extension, request additional compensation to reimburse it for damages sustained as a direct result of such delay, and such periods of extended Contract time may be deemed "Compensable Delays."

The period of compensable delay is limited as follows:

(1) it may not include time more than 60 days prior to the Engineer's receiving written notice from the Contractor, with adequate substantiation, of its intent to claim damages for the delay, and

(2) it may not include periods of delay for which the State was responsible, but during which the Contractor experienced concurrent delays for which the State was not responsible.

Damages for periods of Project delay for which the State had sole responsibility shall be limited to the increased costs incurred by the Contractor (which shall not include lost profits), which the Contractor substantiates and which the Contractor shows were caused by such delays.

If an approved extension of Contract time extends beyond November 30, the number of days of the approved extension remaining on that date will not begin to run again until the following April 1.

The Critical Path Method Schedule Analysis shall include at a minimum:

1. The manner in which the Contractor planned to construct the Project, in terms of activities, logical interrelationships of activities, work sequences, activity durations, and calendars.
2. The actual duration and sequences of the activities, based on what actually occurred on the Project.
3. The variances between the planned and actual performance of the work, listed in a chronological and cumulative manner, summing to the net total delay on the Project at the time of the request.
    a. The causes of the variances between the planned and actual performance of the work, specifically allocating legal responsibility for each to either the Department or the Contractor.
    b. The effects of the variances in work sequences, activity durations and Resources on the incurred costs of the affected party or parties.
4. An identification analysis of the causes of any concurrent delays on the Project.
5. Statements as to whether the time extension days sought are compensable or non-compensable, along with a specific statement of any compensation requested in connection with the time extension. Any request for a time extension that does not include a corresponding request for compensation will be assumed to be a request for a non-compensable time extension.
6. All associated analysis documents, worksheets, schedules and contemporaneous documents supporting the Critical Path Method Schedule Delay Analysis.

1.08.09—Failure to Complete Work on Time: Time is an essential element of the Contract. Since the prosecution of the Project may obstruct traffic, interfere with business, and otherwise inconvenience the public, it is important that the Project be pressed vigorously to completion. The cost to the Department of the administration of the Contract, including engineering, inspection and supervision, will also be increased as the time for Project completion is lengthened. Therefore, for each calendar day that any work shall remain uncompleted after the Contract time has expired, the per diem sum of liquidated damages specified in the Contract shall be deducted from any money due to the Contractor. Liquidated damages are not a penalty, but are a reasonable estimate of the damages caused by such delay.

If the last day of the initial Contract time or the initial Contract date determined for Substantial Completion is before December 1 in the given year, liquidated damages as specified in the Contract shall be assessed against the Contractor per calendar day (including any days during a winter shutdown period) from that day until the date on which the Project is substantially completed.

The Engineer has the right to deduct the amount of the liquidated damages assessed against the Contractor from any estimated payment for work performed under the Contract or under any other State contract, or from any other sums owed by the State to the Contractor; or to claim and recover such sums by process of law.

1.08.10—Termination of Contract for Cause: The Commissioner may give notice in writing to the Contractor and its surety of any delay, neglect, or default of the Contractor which the Commissioner believes has occurred, including one or more of the following:

1. Failure to begin the Project on the date specified in the Notice to Proceed.
2. Failure to perform the Project with sufficient personnel, equipment or materials to ensure timely Project completion.
3. Unsuitable performance of the Project or failure to perform Project work in accordance with the Contract.
4. Failure or refusal to remove or correct work rejected by the Engineer.
5. Discontinuance of suitable prosecution of the Project for a period of 72 hours, excluding Sundays and holidays, without written authorization to do so from the Engineer.
6. Failure to recommence discontinued work within 48 hours (excluding Sundays and holidays) after being ordered to do so by the Engineer.

7. Insolvency, filing for bankruptcy, or any act or occurrence which may render the Contractor financially incapable of completing the Project.
8. Failure to satisfy any final judgment for a period of 30 calendar days.
9. Making of any assignment for the benefit of creditors.
10. Violation of any provisions of the Contract.
11. Any other cause which, in the judgment of the Commissioner, warrants termination, including, but not limited to, violations of the antitrust or criminal laws, and attempts to deceive or defraud the Department in material matters.

If the Contractor or surety within a period of 10 calendar days after such notice does not proceed in conformance with the directions set forth in the notification, or fails to present a remedial plan of operation satisfactory to the Commissioner, then the Commissioner may, at his discretion, order the surety to complete the Project or, without violating the Contract, take the right to control and prosecute the Project out of the hands of said Contractor and surety. No termination of the Contract for such cause will be deemed to have occurred, however, unless the Commissioner himself or herself (and not merely a designated representative of his or hers) expressly declares it in a writing to the Contractor.

The Department may acquire or rent whatever materials or equipment are necessary in order to complete the Project and may seize and use for purposes of the Project (with any appropriate compensation to the Contractor) any material or equipment that the Contractor acquired or purchased expressly for the Project in accordance with a specific Contract requirement.

The Department may also enter into an agreement, either by negotiation or public letting, for the completion of the Contract according to the terms and provisions thereof, or use such other methods or combinations thereof as in the Commissioner's opinion shall be required or desirable for the completion of the Contract in an acceptable manner. All costs and charges incurred by the Department, in connection with completing the Project under the Contract, or as a result of the Contractor's default, shall be deducted from any monies due to or which may become due to the Contractor. In case such expense exceeds the sum which would have been payable under the Contract, then the Contractor and the surety shall be liable for, and shall pay to the State, the amount of the excess.

1.08.11—Final Cleaning Up: The Project will not be considered complete and will not be accepted until the rights of way, borrow pits, and all other ground, both public and private, occupied by the Contractor in connection with the Project has been cleared of all surplus and discarded materials, rubbish and temporary structures. The Contractor must drain all borrow pits where practicable. All property, both public and private, which has been damaged during the prosecution of the Project, shall be restored by the Contractor to an appearance and condition acceptable to the Engineer.

All ditches, waterways, drainage structures and culverts constructed under the Contract shall be cleaned and cleared of obstructions by the Contractor, and shall be left in a condition acceptable to the Engineer. When so directed by the Engineer, the Contractor shall clean all existing ditches, waterways, drainage structures and culverts of obstructions resulting from Project operations.

1.08.12—Final Inspection: If the Engineer determines that the work may be substantially complete, a Semi Final Inspection will be held as soon as practical. After the Semi Final Inspection is held and the Engineer determines that the requirements for Substantial Completion have been satisfied the Engineer will prepare a "Notice of Substantial Completion."

When the Contractor has completed all work listed in the "Notice of Substantial Completion" the Contractor shall prepare a written notice requesting a Final Inspection and a "Certificate of Acceptance of Work." The Engineer will hold an Inspection of the Project as soon as practical after the Engineer determines that the Project may be completed. If the Engineer deems the Project complete, said inspection shall constitute the Final Inspection, and the Engineer will notify the Contractor in writing that the Final Inspection has been performed.

If the inspection discloses any unsatisfactory or incomplete Project work, the Engineer will notify the Contractor of such deficiencies. The Contractor shall immediately correct the deficiencies. Upon such correction, another inspection will be made that shall then constitute the Final Inspection, provided that the work has been satisfactorily completed.

On projects consisting of 2 or more individual, geographically-separated sections of roadways, sites, or physically-separated buildings or portions of buildings, a partial final inspection of individual, fully-completed sections will be performed by the Engineer if the Contractor so requests.

If at any time during the prosecution of the Project the Contractor substantially completes a unit or portion

of the Project, such as, but not limited to, a structure, an interchange, a building, a portion of a site, a branch circuit in highway illumination that has been in satisfactory and continuous operation for a period of at least 30 days, or a section of road or pavement, then, to the extent that said portion's stability and integrity are not dependent upon the completion of other Contract work, the completed unit or portion of the Project may be opened:

(1) to vehicular or pedestrian traffic under the provisions of 1.07.11; or

(2) to railroad, aircraft, pedestrian or other traffic, or to occupancy, at the direction of the Engineer.

A final inspection of the substantially-completed unit or portion of the Project will be arranged by the Engineer. Upon evidence of such completion, including issuance of a Certificate of Compliance for any building, and upon agreement by the Engineer, the Contractor will be relieved of the responsibility for that work which was placed upon it by 1.07.12. The Contractor shall be responsible for all damages to the completed unit or portion of the Project that may be caused by or that may result from the operations of the Contractor or its subcontractors, or their employees; and the Contractor shall take such precautions and shall provide such protection as may be necessary to avoid possible damage to said unit or portion of the Project. The acceptance of the completed unit or portion shall in no way affect any other requirements governing the completion and acceptance of the Project as a whole.

1.08.13—Termination of the Contractor's Responsibility: The Contractor's responsibility for non-administrative Project work will be considered terminated when the final inspection has been held, any required additional work and final cleaning-up have been completed, all final operation and maintenance manuals have been submitted, and all of the Contractor's equipment and construction signs have been removed from the Project site. When these requirements have been met to the satisfaction of the Engineer, the Commissioner will accept the work by certifying in writing to the Contractor that the non-administrative Project work has been completed.

1.08.14—Acceptance of Project: The Project will be accepted by the Commissioner when all Project work has been completed, as defined by the requirements of 1.08.13, and the following have been submitted to the satisfaction of the Engineer:

1. Supporting information necessary to substantiate pay quantities, such as cost-plus backup documentation;
2. Reports and forms required on all Federal Aid Projects;
3. Warranties, guaranties, final operation and maintenance manuals, and documentation to the effect that training and start-up support required by the Contract have been completed;
4. Any other documents required by the Contract.

NOTES

SECTION 1.09
MEASUREMENT AND PAYMENT

1.09.01—Measurement of Quantities
1.09.02—Value Engineering Change Proposal
1.09.03—Increased or Decreased Quantities
1.09.04—Extra and Cost-Plus Work
1.09.05—Eliminated Items
1.09.06—Partial Payments
1.09.07—Final Payment
1.09.08—Vacant
1.09.09—Payment of Costs Owed to the State

1.09.01—Measurement of Quantities: Work completed in compliance with the Contract will be measured by the Engineer according to U.S. Customary standard measures, and quantities of work performed shall be computed based on such measurements made in accordance with the methods of measurement described herein under provisions regarding the applicable Contract item.

Notwithstanding any other provision in the Contract, only work that is within the payment limits prescribed by the Contract or ordered by the Engineer will be measured for payment. No payment will be made for work that is not actually performed.

Structures shall be measured and quantities computed according to the neat lines shown on the plans (as those plans may be revised by authorization of the Engineer), or as may otherwise be required by the Contract.

Quantities of materials measured for payment by net weight shall be measured in tons, while contained in hauling vehicles on scales furnished by and at the expense of the Contractor. The scales shall be satisfactory to the Engineer and shall be sealed. When required by the Engineer, an inspector will be appointed and compensated by the Department to check the weight of all materials to be incorporated into the Project. The tare weight of trucks used to haul materials being paid for by weight shall be taken at such times as the Engineer directs.

1.09.02—Value Engineering Change Proposal: These Value Engineering Change Proposal (VECP) provisions apply as encouragement to the Contractor to initiate, develop, and present to the Department for consideration cost- or time- reduction proposals or a combination of both conceived by the Contractor, involving changes to the drawings, designs, specifications, or other requirements of the Contract. These provisions do not apply unless the proposal submitted is specifically identified by the Contractor as being presented for consideration as a VECP. All such proposals must be made on the Department's VECP form, copies of which are available from the Department. The Department reserves the right to decline to review, or to reject after initial review, any VECP. Before expending considerable funds in development of a formal VECP, the Contractor shall submit a conceptual Proposal to the Department on Department-provided forms.

The proposals which may be considered as VECPs are those which, if implemented,

(a) would require modification of the Contract by construction order;
(b) would produce a savings to the Department by calling for the use of items or methods less costly than those specified in the Contract;
(c) would not alter necessary standardized features of the original Project; and
(d) would not impair essential functions or characteristics of the construction called for by the original Contract, such as service life, reliability, economy of operation, and ease of maintenance.

Material substitution alone will not be considered as a VECP.

A VECP may shorten Contract time, however, acceleration alone will not be considered as a VECP.

Cautions and Conditions:

1. The Contractor is cautioned not to base any bid or bid price on the anticipated approval of a VECP and to recognize that such Proposal may be rejected. The Contractor will be required to perform the Contract in accordance with the existing Contract plans and specifications at the prices bid unless and until the Department formally accepts, in writing, the Contractor's VECP.
2. In order for the Department to consider such a Proposal, the savings likely to be generated by the Proposal must be sufficient, in the sole judgement of the Department, to warrant its review and processing by the Department. All costs resulting from such review or processing will be borne by the Department. Before any VECP will be considered by the Department, the Department must determine,

in its sole judgement, that implementation of the Proposal would result in a total cost savings of more than $100,000.00, reflecting a savings of at least $50,000.00 for the Department. The Department will not consider any VECP that would require an increase in Contract time.

3. All VECPs apply only to the ongoing Contract, and whether approved or not, such Proposals become the property of the Department. Such Proposals shall contain no restrictions imposed by the Contractor on their use or disclosure by the State. The Department will have the right to use, duplicate and disclose in whole or in part any data necessary for the use or implementation of the Proposal. The Department retains the right to use any accepted Proposal or part thereof on any other current or subsequent Department projects without any obligation to the Contractor for such use. This provision is not intended to deny rights provided by law with respect to patented materials or processes.
4. If the Department already has under consideration certain revisions of the Contract or has approved certain changes in specifications or standard drawings for general use which subsequently appear in a VECP, the Department may reject the Contractor's Proposal and may proceed with such revisions without any obligation to the Contractor.
5. The Proposal must be presented and approved in writing prior to the Contractor's undertaking any work on the Contract items involved in the proposal. Savings due to a reduction in quantities or deletion of items which result solely from adjustments to field conditions, and Proposals which would only waive specification or other Contract requirements, are not considered to be VECPs.
6. The Contractor shall have no claim against the Department for any costs or delays due to the Department's review or rejection of a VECP, including, but not limited to, development costs, anticipated profits, or increased material or labor costs resulting from delays in the review or rejection of such Proposal.
7. The Department will be the sole judge of the acceptability of a Proposal and of the estimated net savings in construction costs that would result from adoption of all or any part(s) of such Proposal. In determining such estimated net savings, the Department reserves the right to disregard the Contract bid prices if, in the judgment of the Engineer, such prices do not represent a fair measure of the value of work to be performed or deleted under the Proposal. Errors in the estimated quantities in the bid proposal form for the Contract shall be corrected by the Department prior to calculating the savings that would likely result from adoption of the VECP.
8. The Engineer may reject all or any portion of work performed pursuant to an approved VECP if the Engineer determines that unsatisfactory results are being obtained because of the Proposal's implementation with regard to that work. The Engineer may direct the removal of such rejected work and require the Contractor to proceed in accordance with the original Contract requirements. Where modifications of the VECP have been approved in order to adjust to field or other conditions, payment will be limited to the total amount payable for the work at the Contract bid prices, as if the pertinent work had been constructed in accordance with the original Contract requirements. The Contractor waives the right to use such rejection or limitation of reimbursement as the basis of any claim against the State for delay damages or for any other damages or costs.
9. VECPs must meet the requirements of the specifications or standards of the Department. The standards governing the original design of the Contract will be the minimal standard allowed.
10. If additional information is needed in order for the Department to evaluate Proposals, the Contractor must provide the Department with this information within 14 calendar days of such request or within such other time period as may be approved by the Department. Failure to do so will result in rejection of the Proposal.
11. The Contractor shall provide revised Project plans, specifications and estimates to the Department in construction order format, reflecting such changes as would be required for implementation of the VECP. The Contractor shall be solely responsible for any errors or omissions resulting from such revisions.
12. Savings not directly related to the Contract, such as, but not limited to, reductions in inspection or testing costs or Department overhead, will not be included in the savings calculation for any VECP.

After the Contractor submits a conceptual Proposal, they will be notified in writing of the acceptability or the reason(s) for its rejection. The Department retains the right to reject the formal Proposal even if the conceptual Proposal was determined acceptable.

VECPs will be processed in the same manner as are alterations of the Contract that require a construction order.

VECP Submittal Requirements:

1. A statement that the Proposal is being submitted as a VECP.
2. A description of the difference between the existing Contract requirements and the proposed change(s), and the comparative advantages and disadvantages of each, taking into account considerations of service life, economy of operations, ease of maintenance, desired appearance, safety, and environmental impacts or necessary permit changes. When an item's function or characteristics would be altered by implementation of the Proposal, a justification of the anticipated effects of the alteration on the end item's performance must be included in the Proposal. A life-cycle cost analysis must be included for items involving alteration of functional characteristics. Factors for determining future worth will be provided by the Department.
3. Complete plans, specifications, and computations signed and sealed by a Professional Engineer licensed by the State of Connecticut, showing that the proposed Contract revisions would incorporate the same design criteria and restrictions that applied to the original Contract features and requirements. Said revisions shall be submitted by the Contractor in the Department's construction order format consisting of 1 paper copy of the plans and 1 electronic copy of the plans as a portable document format (PDF) file, indicating
   (a) quantity increases and decreases by item number, with associated cost;
   (b) new items, with their quantities and costs;
   (c) specifications in contract format; and, if needed,
   (d) compliance permit applications and revisions in accordance with 1.10.
4. A complete analysis of the probable cost effects of the proposed changes on Project construction, future operations in connection with the completed Project, maintenance and durability of completed Project construction, and other aspects of the Project, as appropriate.
5. The date by which the Proposal would have to be implemented in order for the Department to obtain the maximum cost reduction from the Proposal's implementation. The period established by the date must allow the Department ample time for review and processing of the Proposal. Should the Department find that it does not have sufficient time for such review and processing, it may reject the Proposal solely on such basis. If the Department fails to respond to the Proposal by said date, the Contractor shall consider the Proposal to be rejected and shall have no claims against the State as a result thereof.
6. A description of the effect that the implementation of the Proposal would likely have on the time required to complete the Project.

Payment for accepted VECPs:

1. The changes resulting from a VECP will be incorporated into the Contract by construction order and shall reflect the changes in existing unit bid item quantities, or any new agreed price items, cost-plus lump sum, or any combination thereof, as appropriate, in accordance with the Specifications and as determined by the Department. Any lump sum submission shall be accompanied by a schedule of payment values.
2. The Contract prices for the revised Project work will be paid directly as accomplished. In addition to such payment, the Department will pay the Contractor, under a separate item or a Value Engineering Incentive item, 50% of the total savings obtained by the State as a result of its implementation of the VECP. An estimate of said savings is to be calculated by the Department within 1 week prior to the Proposal's acceptance, by
   (a) estimating what it will cost the Department to carry out the Project as revised according to the VECP;
   (b) estimating what it would have cost the Department to carry out the Project under the terms of the Contract as modified by any construction orders as of the time that the Department accepted the Proposal; and
   (c) subtracting the sum estimated as per (a) from the sum estimated as per (b).

   When the implementation of the Proposal, including all related construction, has been completed, the Department will calculate the actual savings that resulted from it. The Department will then distribute half of the actual savings to the Contractor.
3. The Contractor's costs for development, design, submission and processing of the VECP are not eligible for reimbursement.
4. The Department will not reimburse the Contractor based on any cost savings not identified in the VECP prior to its acceptance.

5. The cost savings from a VECP that is exclusively time reduction shall be calculated as the number of Contract days reduced multiplied by the amount of liquidated damages for 1 day under the Contract.

1.09.03—Increased or Decreased Quantities: Whenever the quantity of any item as given in both the bid proposal form and Contract is increased or decreased, the Department will pay for such item at the Contract price, on the basis of the actual quantity completed, except as otherwise expressly authorized under the provisions of 1.04.02, 1.04.03 or 1.04.04.

1.09.04—Extra and Cost-Plus Work: Extra work shall be performed only under the conditions and subject to the requirements outlined in 1.04.05. Payment for extra work may be made on any unit price or lump sum price or other basis to which the Engineer and the Contractor agree in writing, or the Engineer may order that the Contractor will be paid for the work on the cost-plus basis described in this article.

The following sets forth the components of the cost-plus basis for making payments:

(a) Labor:

(1) For all labor used by the Contractor for the subject work, the Department will pay the Contractor the wage rate that it actually paid for same, as shown by its certified payroll, which shall be at least the minimum rate established for the Project by the CT Department or the U.S. Department of Labor. For all foremen in direct charge of Project work, the Department will pay the Contractor the actual wage paid to the foremen as shown on the Contractor's certified payroll.

(2) The Department will reimburse the Contractor for the actual costs paid to, or on behalf of, workers by reason of allowances, health and welfare benefits, pension fund benefits and other such benefits in connection with the subject work, when such amounts are required by a collective bargaining agreement or another employment contract generally applicable to the classes of labor employed on the Project. The Contractor shall certify all such costs in writing to the Engineer.

(3) For property damage, liability and workmen's compensation insurance premiums, unemployment insurance contributions and social security taxes on Project cost-plus work, the Department will reimburse the Contractor for its actual Project costs. The Contractor shall provide to the Engineer documentation, satisfactory to the Engineer in form and substance, of all such costs.

(4) The Department will also pay to the Contractor an amount equal to 20% (15% for overhead, 5% for profit) of the total sums described in **(a)** (1) through (3) above.

No part of the salary or expenses of anyone connected with the Contractor's forces above the grade of foreman, who provides general supervision of Project work, will be included in the above payment calculations, except when the Contractor's organization is entirely occupied with cost-plus work, in which case the salary of a superintendent may be included in said labor item when the nature of the pertinent Project work is such that, in the opinion of the Engineer, a superintendent was required for that work. The Engineer and the Contractor may agree in writing to the allowable rate of pay for such superintendent, or the Engineer may make payment based on such rate as the Engineer deems reasonable.

The Engineer reserves the right to determine the number and type of personnel to be employed for the cost-plus Project work.

(b) Specialized Work: When the Engineer directs the Contractor to perform specialized work requiring skills, tools and equipment substantially unlike those ordinarily used by the Contractor or its authorized Project subcontractors, the Department will pay the Contractor for the use of a specialist to perform the specialized work. For such specialized services, including materials incorporated into the Project, the Department will pay the Contractor its actual costs, plus additional compensation in accordance with subparagraph (e) below. If so requested by the Engineer, the Contractor shall obtain and submit to the Engineer, prior to performing such specialized work, a minimum of three price quotes for the work.

(c) Materials: For all materials necessary for cost-plus Project work, the Department will pay the Contractor its actual cost for such materials, including delivery charges as shown by original receipted bills, plus 15 % of the sum of said cost and charges.

In lieu of receipted bills for materials used for the Project, but which were not specifically purchased for the Project, but rather were taken from the Contractor's stock, the Contractor shall provide to the Engineer an affidavit certifying that such materials were not purchased for the Project, that the materials were taken from the Contractor's stock, that the quantity claimed to have been used on the Project was actually so used, and that the price claimed for the materials reflects their fair market value at the time of their use on the Project. The Department will pay for costs of transporting the materials to the Project site, in accordance with subparagraphs (a) and (d) hereof.

The Department will not reimburse the Contractor for any penalty or charge incurred by the Contractor

due to the Contractor's late or delayed payment for the pertinent materials.

(d) Equipment: All equipment used for cost-plus Project work must, in the judgment of the Engineer, be in good working condition and suitable for its Project purpose; and the Engineer reserves the right to determine the size and number of units of equipment to be used for such work. The manufacturer's ratings shall be the basis for all Rental Rate Blue Book classifications used for payment purposes. ("Rental Rate Blue Book" as used in these specifications refers to the current edition of the Rental Rate Equipment Watch Blue Book Services, taking into account all current Rate Adjustment Tables, and amendments thereof.) Trucks will be classified by cubic-yard capacity.

No percentage mark-up will be added for payment purposes to amounts charged by the Contractor based on equipment rental rates.

The Department will not pay rental rates for small tools needed to complete the cost-plus Project work.

For payment purposes, estimated operating costs per hour from the Rental Rate Blue Book will apply only to the actual time during which the equipment is actively being used to perform cost-plus Project work.

For equipment that is also being used for non-cost-plus Project work, the Department will pay the applicable hourly rate only for the actual time that the equipment was assigned to cost-plus Project work. The applicable period of assignment for each piece of equipment shall start when the equipment commences to be used for cost-plus Project work ordered by the Engineer, and shall end at the time designated by the Engineer.

For equipment brought to the Site exclusively for cost-plus work, the Department will reimburse the Contractor for loading and unloading costs and costs of transporting such equipment to and from the Project site; provided, however, that payment for return transportation from the Site shall not exceed the cost of moving the equipment to the Site. If such a piece of equipment is self-propelled, and is driven to the Site under its own power, then the Department will pay only operating costs and labor costs for its transport to and from the Project site. The Department will not, however, pay for any loading, unloading and transportation costs if the equipment is used for any Project work on the Site other than cost-plus work.

(1) Owned Equipment: The Department will pay the Contractor the applicable rental rate as set forth in the Rental Rate Blue Book for any equipment (1) which the Contractor uses, with the Engineer's authorization, to perform cost-plus Project work, and (2) which is owned by the Contractor or a subsidiary, affiliate, or parent company of the Contractor (no matter how far up or down the chain of ownership from the Contractor).

The maximum hourly rate to be used in paying for Contractor-owned equipment assigned to cost-plus work shall be the applicable monthly rate in the Rental Rate Blue Book, divided by 176 (176 being the number of working hours per month).

Should the proper completion of the cost-plus Project work require equipment of a type not covered by the Rental Rate Blue Book, the Engineer will determine, and the Department will make payment to the Contractor at, a reasonable rental rate based on relevant rates prevailing in the area of the Project. If practicable, such rates shall be determined by the Engineer before the affected work is begun. If the Contractor proposes that the Engineer use a particular rate in such an instance, the Contractor must disclose to the Engineer the specific sources of, or support for, said rate.

If a piece of equipment owned by the Contractor is assigned to cost-plus Project work, but remains idle for some portion of the period of the cost-plus work, the Department will pay for that idle time at 50% of the applicable rental rate (exclusive of operating costs) in the Rental Rate Blue Book.

For payment purposes, the period of equipment usage shall be deemed to start when the Contractor begins to use the equipment for cost-plus Project work and shall be deemed to end when the equipment is released by the Engineer from use for such work. Any hours during which the equipment is used for work other than cost-plus Project work will be deducted from the pertinent payment period.

For any piece of Contractor-owned equipment assigned to cost-plus Project work, the Department will reimburse the Contractor for an aggregate minimum of 8 hours (of use time, idle time, or a combination thereof) in each 24-hour day (measured from one midnight to the following midnight) during the assignment period. No such reimbursement will be made, however, for Saturdays, Sundays and legal holidays during which the Contractor does no Project work, or for any other day on which the Engineer orders the Contractor to do no Project work. If the equipment is used to perform cost-plus Project work for more than 8 hours in a day, the Department will pay the Contractor at the applicable hourly rate computed on a monthly basis for the actual time of use; however the Department will not pay the Contractor for more than 8 hours of idle time for a piece of equipment

during a given day.

The Department shall have the right to limit its aggregate Project payments for idle time for a given piece of equipment to the replacement value of that equipment.

(2) Rented Equipment: If the Engineer determines that in order to perform the cost-plus Project work the Contractor must rent certain machinery, trucks or other equipment not owned by the Contractor or a subsidiary, affiliate, or parent company of the Contractor (no matter how far up or down the chain of ownership from the Contractor), the Contractor shall provide to the Engineer in writing, in advance of such rental:

(1) the specific nature of the rental(s),
(2) the reasons for its need for such rental(s),
(3) the anticipated or proposed rental rate(s), and
(4) the estimated duration for the use of each piece of such rented equipment.

Proposed rates for such rented equipment must be based on the following:

—A daily rate per hour when the equipment is to be specifically assigned to Project work by the Engineer for a period of 7 consecutive calendar days or less.

—A weekly rate per hour when such assigned time exceeds 7 consecutive calendar days, but does not exceed 21 consecutive calendar days.

—A monthly rate per hour when such assigned time exceeds 21 consecutive calendar days.

The applicable daily, weekly, or monthly rate will be determined at the expiration of 21 calendar days or upon release of the equipment by the Engineer, whichever occurs first. Interruptions of the rental period, when equipment is used for work other than assigned cost-plus work, will not entitle the Contractor to payment at a rental rate that would be applicable to a shorter period that might arguably have been occasioned by such interruptions.

If so requested by the Engineer, the Contractor shall, prior to renting such equipment, obtain and submit to the Engineer in writing a minimum of three rate quotes for rental of the equipment.

The Department will pay the Contractor for such rental at the rate actually paid by the Contractor, provided that the given use and rental rate are acceptable to the Engineer. In order to obtain such payment, the Contractor must provide the Engineer with a copy of the original receipted bill for the rental expenses incurred.

(e) Administrative Expense: When extra work is performed on a cost-plus basis by a subcontractor acceptable to the Engineer, the Department will pay the Contractor an additional 7.5% for that work; such payment will be in addition to the percentage payments described in (a), (b), (c) and (d) above, as a reimbursement for the Contractor's administrative expense in connection with such work. The Engineer will approve such additional payments only if and when the Contractor provides to the Engineer receipted invoices for all relevant costs.

(f) Miscellaneous: The compensation provided for in (a), (b), (c), (d) and (e) above shall be deemed to be payment in full for the extra work, and shall be deemed as full compensation for same, including costs of superintendence, use of small tools, equipment for which no rental is allowed, safety equipment, consumables, field office overhead, home office overhead, bonding, other insurance, and profit. The Contractor's representative and the Engineer shall compare their respective records related to the extra work done on a cost-plus basis at the end of each day. Copies of these records shall be signed by both the Engineer and the Contractor's representative. The Engineer will then forward a copy of same to the Contractor and to any affected subcontractor in accordance with Department procedures. Upon payment of such costs by the Contractor, the Contractor shall immediately furnish the Engineer with original receipted bills covering the costs, including transportation charges, for all materials used for such work.

1.09.05—Eliminated Items: The Engineer may eliminate from the Contract any pay unit item, or any portion of Project work contained in a lump sum item by giving written notice of said elimination to the Contractor. Such elimination shall in no way invalidate the Contract.

The Engineer will make final payment to the Contractor for materials at the actual cost of the materials and any applicable material markup for eliminated pay unit items, or portions of work contained in a lump sum item only under the following terms and conditions:

1. the materials were ordered by the Contractor prior to the Engineer’s issuance to the Contractor of a written notice of the unit or work’s elimination (as evidenced by a dated invoice from the vendor);
2. the materials conformed to all Contract requirements; and

3. the Contractor could not have cancelled its order within 2 days after the issuance of the elimination notice.

Any materials paid for by the Department on these conditions shall then be the property of the State, and the State will assume, or will reimburse the Contractor for, the actual cost of any further handling necessary to deliver said materials to a location designated by the Engineer.

If the relevant materials purchased by the Contractor are returnable to their vendor and if the Engineer so directs, the Contractor shall return the materials to the vendor, in which case the Department will reimburse the Contractor for any reasonable charges made to the Contractor by the vendor for the return of the materials, and for the actual costs to the Contractor of its handling the materials in returning them to the vendor. Such reimbursements by the Department shall be computed as though the work were being paid for on a cost-plus basis under 1.09.04.

If the Engineer determines that an elimination of a pay unit item, or portion of work contained in a lump sum item constitutes a "significant change" in the character of the Contract work, as defined under 1.04.03 necessitated by a written order of the Engineer, the terms of 1.04.03 shall govern the payment to be made in relation to the eliminated item or work.

1.09.06—Partial Payments:

A. Monthly and Semi-monthly Estimates:

(1) Once each month, the Engineer will make, in writing, current estimates of the value of work performed in accordance with the Contract, calculated at Contract unit prices, including but not limited to the value of materials complete in place and materials not yet incorporated into the Project, but approved by the Engineer for payment (as provided for elsewhere in this article). Retainage will not be held.

Exceptions may be made as follows:

(a) When not in conflict with the interests of the State, the Contractor may request, and the Engineer may make, semi-monthly estimates for payment.

(b) If, in the judgment of the Assistant District Engineer, the Project is not proceeding in accordance with the Contract the Engineer may decline to make a payment estimate.

(c) If the total value of the Project work complete since the last estimate amounts to less than $2,500, the Engineer also may decline to make a payment estimate.

(2) The Engineer may also make payment at Contract unit prices for the number of units that represent the value of the Project work performed to date, if said units are essentially, though not totally, complete.

B. Payment for Stored Materials: Non-perishable materials that are required for Project construction and that the Contractor has produced or purchased specifically for incorporation into the Project, but which have not yet been so incorporated, may be included in a payment estimate if

(i) the materials meet all applicable Contract specifications,

(ii) the materials have been delivered to the Project site or to another location approved by the Engineer, and

(iii) the Contractor has submitted to the Engineer, as evidence of the Contractor's purchase of the materials, either a copy of a receipted bill for same or a Certificate of Title to the materials, in the form approved by the Department, duly-executed by the Contractor and the Vendor.

The Engineer will decide at what fair and appropriate fraction of the applicable Contract price such materials may be included in a payment estimate.

Offsite storage may be approved by the Engineer, provided that the materials proposed for payment are segregated from other materials, clearly labeled as being owned by the Department for use on the identified Project, otherwise handled in compliance with 1.06.03, and stored in accordance with the manufacturer’s recommendations. All such materials must be readily-available for inventory and inspection by the Engineer. Storage outside of the State of Connecticut may be considered only when a representative of the Department is able to verify that the above requirements have been satisfied.

For items requiring extended fabrication, manufacturing or assembly time, the Contractor may propose to the Engineer a schedule of values for the related material costs. If the Engineer approves such a schedule of values, it shall become the Basis of Payment for the stored materials, so long as all other pertinent Contract requirements have been satisfied.

Generic materials having a use on many projects will be considered for payment prior to their incorporation into the Project only if stored in unopened packaging or in large lots. Stock and raw

materials will not be considered for such advance payment without the Engineer's prior written consent thereto.

In no case shall material payments exceed the Contract unit price or lump sum price less the actual value of delivery and installation of the materials; if they do exceed such a price, the Engineer reserves the right to reduce any related payment accordingly. Such reductions in payment shall in no way affect the Department's ownership interest in the stored materials.

1.09.07—Final Payment: When the Commissioner has accepted the Project in accordance with 1.08.14, the Engineer will prepare a final payment estimate.

1.09.08—Vacant

1.09.09—Payment of Costs Owed to the State: The State shall have the right to set off against amounts otherwise due to the Contractor under this Contract or under any other contract or arrangement that the Contractor has with the State

(a) any costs that the State has incurred due to the Contractor's noncompliance with this Contract and

(b) any other amounts that are due and payable from the Contractor to the State.

Any sum taken in setoff from the Contractor shall be deemed to have been paid to the Contractor for purposes of payment obligations under 1.03.04 of these Specifications.

SECTION 1.10
ENVIRONMENTAL COMPLIANCE

1.10.01—General
1.10.02—Compliance with Laws and Regulations
1.10.03—Water Pollution Control
1.10.04—Vacant
1.10.05—Construction Noise Pollution
1.10.06—Protection of Archaeological and Paleontological Remains and Materials
1.10.07—Controlled and Hazardous Materials
1.10.08—Vehicle Emissions

1.10.01—General: During and following Project construction, the Contractor shall exercise precaution and care to prevent or minimize negative effects on the environment, including the State's waters, wetlands, and other natural resources.

The Contractor shall comply with all Project permits and permit applications as though the Contractor were the permittee.

The Contractor must comply with the environmental provisions specified in the Contract, and any Federal, State or municipal laws or regulations. If the Contractor fails to comply with these environmental provisions, the Contractor shall be penalized as specified in this Section and elsewhere in the Contract.

1.10.02—Compliance with Laws and Regulations: The Contractor shall conduct its operations in conformance with the permit requirements established by Federal, State and municipal laws and regulations.

The Department will be responsible for obtaining all environmental permits required for Contract work. If at the time such a permit is issued, its contents differ from those described in the Contract, the permit shall govern. Should the permit be issued after the solicitation of bid proposals, and should the permit requirements significantly change the character of the work as described in the Department's Project bid documents, Contract adjustments will be made in accordance with the applicable articles in 1.04 herein.

The Contractor shall be responsible for, and hold the State harmless from, any penalties or fines assessed by any authority due to the Contractor's failure to comply with any term of an applicable environmental permit.

Any request by the Contractor for the Department's authorization of an activity or use of a method not specifically called for or allowed by the applicable permits issued for the Project must be submitted by the Contractor in writing to the Engineer. Such a request must include a detailed description of the proposed alternate activity or method, and must include justifications for same, along with supporting documentation, showing that the proposed alternate activity or method will not create a risk of damage to the environment, increase the permitted wetland impact footprint, or increase fill within a floodplain. If such request is granted by the Engineer, the Department will forward to the appropriate regulatory agency or agencies any permit modification, permit revision, *de minimis* change or new permit required for the Contractor to carry out the proposed alternate activity or method in question. The Department does not, however, guarantee that it will be able to obtain such approval from the regulatory agency or agencies; and the Department will not be liable for the effects of such inability to do so.

The Contractor will not be entitled to any extension of Contract time as a result of the Engineer's granting of such a request from the Contractor. If changes to the permit are not necessary except to accommodate changes requested by the Contractor, then no claim may be made by the Contractor based on the amount of time taken by the Department to review the Contractor's request or to secure approval of related permit changes from the regulatory agency or agencies. The proposed alternate activity or method shall not commence until and unless the Engineer has approved the Contractor's request.

1.10.03—Water Pollution Control: The Contractor shall, throughout the duration of the Contract, control and abate siltation, sedimentation and pollution of all waters, including but not limited to under-ground water systems, inland wetlands, tidal wetlands, and coastal or navigable waters.

Construction methods proposed by the Contractor must comply with the approved permit requirements and permit applications. The Contractor shall be responsible for all obligations and costs incurred as a result of the Contractor's failure to comply with the terms and conditions of such permits or permit applications.

The following are Required Best Management Practices for prevention and control of water pollution. Provisions of the Required Best Management Practices may be superseded as specified in 1.05.04. The

Contractor shall not make any design change in the Contract work that requires a variance from the requirements of the following items until and unless the Contractor has first submitted a detailed written proposal for such variance to the Engineer for review by the Department and for transmittal to and review by the Federal, State or municipal environmental authority, and has then received written approval from the Department of the proposed variance.

REQUIRED BEST MANAGEMENT PRACTICES

1. Prior to commencing Project Site work, the Contractor shall submit in writing to the Engineer an "Erosion and Sedimentation Control Plan" and a "Dust Control Plan" for all Project construction stages. The Contractor shall install all control measures specified in said Plans prior to commencement of Project construction activities. The Plans shall be consistent with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control, the 2004 Connecticut Stormwater Quality Manual, and all environmental laws and regulations established by Federal, State or municipal agencies, as well as the Department's published environmental policies and standards. If the Contractor elects to work during a winter shut-down period, the Contractor shall submit to the Engineer a separate Winter Erosion and Sedimentation Control Plan, obtain the Engineer's written approval of it, and implement it before the Contractor begins Project work during the winter shut-down period.
2. The Contractor shall inspect erosion and sedimentation controls at least weekly, immediately after each rainfall event of at least 0.1 inch, and daily during periods of prolonged rainfall. The Contractor shall maintain all erosion and sedimentation control devices in a functional condition, in accordance with the Contract plans, relevant permits, Special Provisions, and 2002 Connecticut Guidelines for Soil Erosion and Sediment Control. In the event that the Contractor fails to maintain such devices in accordance with said documents, and the Contractor does not correct such a failure within 24 hours after receipt of written notice of such a failure from the Engineer, the Department may proceed with its own or other forces to remedy such failures. The cost to the Department of curing any such specified failure will be deducted from monies owed to the Contractor under the Contract or under any other State contract.
3. Washout of applicators, containers, vehicles, and equipment that have been used with concrete (including bituminous concrete), paint or other such possible contaminants shall be conducted: (i) at least 50 feet from any stream, wetland or other sensitive resource; and (ii) in an entirely self-contained washout system. Such materials shall be collected and disposed of in accordance with all applicable Federal, State and municipal laws and regulations.
4. No materials resulting from Project construction activities shall be placed in or allowed to contribute to the degradation of a wetland, watercourse or storm drainage system. Good housekeeping of the Site by the Contractor for the purpose of preventing construction-related debris or runoff from entering a regulated area is required. The Contractor shall not leave waste or debris within the travel way or roadside where it might create a safety hazard to the traveling public. The Contractor shall dispose of all construction-related materials in accordance with Federal, State and municipal laws and regulations.
5. In accordance with CGS 22a-38, the Contractor shall not withdraw water from any watercourse system, except as allowed by applicable permits.
6. The Contractor shall not dispose of any material until and unless it has proposed a location for its disposal to the Engineer and the Engineer has approved said location in writing.
   If the proposed disposal location is on private property, the Contractor must include in the disposal location proposal to the Engineer letters from the property owner and the affected municipality, agreeing to the proposed location for disposal.
   The Contractor shall ensure that proposed disposal locations are outside of wetlands or watercourses, floodplains and water or natural resource areas.
7. Before commencing any work in or adjacent to a regulated area shown on the plans, permit(s), or identified by the Engineer, the Contractor must submit in writing to the Engineer a construction-sequencing plan, a water-handling plan, and a flood contingency plan, and obtain from the Engineer written approval of said plans.
8. When dewatering is necessary, the Contractor must not allow pumps used for same to discharge directly into a wetland or watercourse. Prior to any dewatering, the Contractor must submit to the Engineer a written proposal for specific methods and devices to be used for same, and must obtain the Engineer's written approval of such methods and devices, including, but not limited to, the

pumping of water into a temporary sedimentation basin, providing surge protection at the inlet or outlet of pumps, floating the intake of a pump, or any other method for minimizing or retaining the suspended solids. If the Engineer determines that a pumping operation is causing turbidity in a regulated area, the Contractor shall halt said operation until a means of controlling the turbidity is submitted by the Contractor in writing to the Engineer, approved in writing by the Engineer, and implemented by the Contractor.

9. Whenever possible, work within or adjacent to watercourses shall be conducted during periods of low flow. The Engineer shall remain aware of flow conditions during the conduct of such work, and shall order such work stopped if flow conditions threaten to cause excessive erosion, siltation or turbidity. Before predicted major storms (*i.e.*, a storm predicted by NOAA Weather Service, with warnings of flooding, severe thunderstorms, or similarly severe weather conditions or effects), the Contractor shall make every effort to secure the Site to the satisfaction of the Engineer. Unless allowed by a DEEP permit, the Contractor shall store no materials and place no staging areas below the 100-year elevation. The Contractor shall not store below the 500-year flood level any materials which are buoyant, hazardous, flammable, explosive, soluble, expansive, radioactive, or any other materials that could be injurious to human, animal or plant life in the event of a flood.
10. Upon completion of the associated work, the Contractor shall immediately clear all areas of all forms, false work, piling, debris or other obstructions created or caused by construction operations.
11. If the Contractor wants to make a change in construction operations, staging or scheduling that would affect the use of or necessity for any pollution controls, the Contractor must submit to the Engineer a written proposal detailing the proposed change, and must receive the Engineer's approval of such change, before implementing it. Such submission must include a plan showing what erosion and sedimentation controls above and beyond those called for in the Contract would be necessitated by the proposed change.
12. Dumping of oil, fuel, chemicals or other harmful materials on the ground or into a regulated area is forbidden. The Contractor shall provide to the Engineer a written Spill Prevention and Remediation Plan for the Project, outlining the Contractor's intended means of catching, retaining, and properly disposing of drained oil, removed oil filters, fuel, chemicals and other harmful material. Such plan shall also include the information and protocols needed for the remediation of any spill that might occur on the Site, including emergency contact information. No construction activities shall commence until such a plan has been approved in writing by the Engineer.
13. The Contractor shall restore all areas within or outside the State right-of-way that have been disturbed as a result of construction activities, in accordance with 1.08.11.

1.10.04—Vacant

1.10.05—Construction Noise Pollution: The Contractor shall take measures to minimize the noise caused by its construction operations, including but not limited to noise generated by equipment used for drilling, pile-driving, blasting, excavation or hauling.

All methods and devices employed to minimize noise shall be subject to the continuing approval of the Engineer. The maximum allowable level of noise at the residence or occupied building nearest to the Site shall be 90 decibels on the "A"-weighted scale (dBA). The Contractor shall halt any Project operation that violates this standard at any time until the Contractor develops and implements a methodology that enables it to keep the noise from its Project operations below the 90-dBA limit.

1.10.06—Protection of Archaeological and Paleontological Remains and Materials: The Contractor shall be alert to the possibility that Project operations may disturb or uncover significant archaeological or paleontological resources or other such remains which in many cases are protected by Federal laws, State laws or both. Archaeological resources are minimally defined by Federal regulations as materials 50 years of age or older. They typically consist of subsurface concentrations of metal, bone, ceramic, or flaked or other shaped stone artifacts. They might also consist of *features* such as buried building foundations, linear or circular walls made of individual stones rather than concrete or cement, trash-filled pits, patches of burned earth, or distinct patterns of nearly-circular, elliptical, or squared discolorations in newly-exposed soil, accompanied by the types of *artifacts* described above.

Paleontological resources are defined as any fossilized remains, traces, or imprints of organisms, preserved in or on the earth’s crust. These typically include fossilized bones, teeth, shells, eggs, or distinct

impressions made in bedrock.

When archaeological or paleontological materials are inadvertently encountered, the Contractor shall immediately halt operations in the location of same and shall notify the Engineer of said discovery. The Contractor shall make every effort to preserve archaeological or paleontological materials intact in their original positions, in order to preserve the geological context and information content of the remains in relation to one another and to the enclosing soil.

The Engineer shall have the authority to suspend Project work in the area of such discovery for the purpose of preserving or recovering and documenting the archaeological or paleontological materials. The Contractor shall carry out all instructions of the Engineer for the protection of such materials, including steps to protect the site from vandalism, unauthorized investigations, accidental damage, and damage from such causes as heavy rainfall or runoff. The Contractor shall reschedule its work to minimize any loss of the time needed to complete the Project while the State evaluates, records and salvages the archaeological or paleontological materials.

Extra work ordered by the Engineer in this connection will be paid for in accordance with 1.04.05 and 1.09.04. Delays caused by archaeological or paleontological preservation and protection, which the Contractor demonstrates have delayed completion of the Project, will be treated under the provisions for extension of time, 1.08.08.

1.10.07—Controlled and Hazardous Materials: The Department will acquire any "Hazardous Waste Generator Permit(s)" required under the Resource Conservation and Recovery Act, for the management and disposal of hazardous materials on the Site, provided that

1. such material is within the construction limits defined in the Contract, and
2. such material was not generated by the Contractor.

If the Department has designated in the Contract an area of known or suspected contamination within the Project limits, the Contractor shall dispose of such material in accordance with the relevant Special Provisions.

In the event that the Contractor encounters or exposes any material, not previously known or suspected to be contaminated, but exhibiting properties that may indicate the presence of controlled or hazardous material, the Contractor shall cease all operations in the material's vicinity and shall immediately notify the Engineer of the material's discovery. The presence of barrels, discolored earth, metal, wood, visible fumes or smoke, abnormal odors or excessively hot earth may indicate the presence of controlled or hazardous material, and the Contractor shall treat it with extreme caution.

If controlled or hazardous materials, other than those required for Contract operations, are discovered at the Site, the Department may engage a specialty contractor to handle and dispose of the materials.

When the Contractor performs support work incidental to the removal, treatment or disposal of controlled or hazardous material, the Department will pay for same at the applicable Contract unit prices. When the Contract does not include appropriate pay items for such work, the Department will pay for it in accordance with 1.04.05.

The Contractor shall observe all security precautions established pursuant to 29 CFR 1910.120 and 1926.65, including all revisions and amendments thereof, and shall not work in any area known to contain or suspected of containing controlled or hazardous material without prior written approval to do so from the Engineer.

The Contractor shall assume sole responsibility for the proper storage, handling, management, and disposal of all regulated materials and wastes associated with its operations, including, but not limited to, lubricants, antifreeze, engine fluids, paints, and solvents. All costs associated with any failure by the Contractor to properly manage such materials in accordance with Federal, State and municipal regulations, and all remedial and punitive costs incurred by the Department as a result of such failure by the Contractor, shall be borne by the Contractor.

This article does not apply to coatings removed by the Contractor.

1.10.08–Vehicle Emissions: All motor vehicles and construction equipment used for the Project (both on-highway and off-road) shall comply with all Federal, State and municipal regulations concerning exhaust emission controls or safety.

The Contractor shall establish staging zones for vehicles waiting to load or unload at the Site. Such zones shall be located where the emissions from the vehicles will have minimum impact on abutting properties and the general public.

Idling of delivery trucks, dump trucks, and other equipment shall not be permitted for longer than 3

minutes during periods of non-activity, except as allowed by the Regulations of Connecticut State Agencies 22a-174-18(b)(3)(c):

No mobile source engine shall be allowed "*to operate for more than 3 consecutive minutes when the mobile source is not in motion, except as follows:*

1. *When a mobile source is forced to remain motionless because of traffic conditions or mechanical difficulties over which the operator has no control,*
2. *When it is necessary to operate defrosting, heating or cooling equipment to ensure the safety or health of the driver or passengers,*
3. *When it is necessary to operate auxiliary equipment that is located in or on the mobile source to accomplish the intended use of the mobile source,*
4. *To bring the mobile source to the manufacturer's recommended operating temperature,*
5. *When the outdoor temperature is below 20°F*
6. *When the mobile source is undergoing maintenance that requires such mobile source be operated for more than 3 consecutive minutes, or*
7. *When a mobile source is in queue to be inspected by U.S. military personnel prior to gaining access to a U.S. military installation.*"

The Contractor shall conduct all of its Project work in a way that causes no harm to adjacent sensitive receptors. Sensitive receptors include but are not limited to hospitals, schools, daycare facilities, elderly housing, and convalescent facilities. The Contractor shall see to it that any engine exhaust is not directed toward fresh air intakes, air conditioners, or windows.

Before performing extensive work within less than 50 feet of a sensitive receptor, the Contractor must (1) submit to the Engineer a Vehicle Emissions Mitigation plan, proposing detailed means for minimizing vehicle emissions from vehicles and construction equipment in the affected area, including a proposed sequence of construction; (2) obtain the Engineer's written approval of the Plan, making any revisions of same necessary to obtain said permission; and (3) implement the Plan, as it may have been revised.

Any costs associated with this "Vehicle Emissions" Article shall be included in the general cost of the Contract. In addition, there shall be no additional time granted to the Contractor for compliance with this Article. The Contractor's compliance with this Article and any associated laws or regulations shall not be grounds for claims as outlined in 1.11 – "Claims."

SECTION 1.11
CLAIMS

1.11.01—General
1.11.02—Notice of Claim
1.11.03—Record Keeping
1.11.04—Claim Compensation
1.11.05—Required Claim Documentation
1.11.06—Auditing of Claims

1.11.01—General: When the Contractor files against the Department or the State a formal claim (a "formal" claim being one that seeks resolution through binding arbitration or court litigation, rather than through negotiation or mediation) under C.G.S 4-61 as revised ("Section 4-61"), whether as a 4-61 notice of claim, demand for arbitration or as a complaint in the Superior Court, the Contractor must follow the procedures and comply with the requirements set forth in this Section of the Specifications, as well as those set forth in 4-61. If this Section sets forth additional, more specific, or demanding requirements than does 4-61 in any respect, this Section shall govern the matter. While the requirements of this Section may not strictly apply to informal claims ("informal" claims being those which the Contractor seeks to resolve through negotiations with the Department, in or outside of a mediation) for additional compensation or other relief from the Department, the Contractor should understand that the Department may need and may demand (in which case the Contractor must provide), the same kinds of documentation and other substantiation that are required under this Section for formal claims. In addition, any time extension request submitted as part of a claim, must satisfy the requirements of this provision and those of 1.08.08. It is the intent of the Department to compensate the Contractor for actual increased costs caused by or arising from acts or omissions on the part of the Department that violate legal or contractual duties owed to the Contractor by the Department.

1.11.02—Notice of Claim: Whenever the Contractor intends to file a demand for arbitration or a court complaint against the Department under 4-61, the Contractor must first notify the Commissioner of the details of said claim, in writing via certified mail (in strict compliance with 4-61), and such written notice must contain all pertinent information described in 1.11.05 below.

Once a formal notice of a claim under 4-61 has been given to the Commissioner, the claimant may not change the claim in any way, in either concept or monetary amount, except insofar as the claim seeks damages that will continue to accrue after submission of the notice, in ways described and anticipated in that notice.

1.11.03—Record Keeping: The Contractor shall keep daily records identifying:

(1) each aspect of the Project affected by matters related to any claim for additional compensation or relief that the Contractor has filed, intends to file, or has reason to believe that it may file against the Department;
(2) the specific Project locations where Project work has been so affected;
(3) the number of people working on the affected aspects of the Project at the pertinent time(s); and
(4) the types and number of pieces of equipment on the Site at the pertinent time(s).

All events or conditions that have a potential or anticipated effect on the Project's progress or schedule and that may result in a claim by the Contractor shall be documented contemporaneously with the event or discovery of the pertinent condition(s), or immediately thereafter. If this is not done, the Contractor may not file the related claim and may not be awarded relief upon it. Without such information, the Department and the Office of the Attorney General may not be able to adequately determine what claims have merit or to what extent they have merit, or what amounts of compensation may be warranted and supportable. Moreover, State officials involved in the analytic or negotiation process may not be able to properly substantiate and support the recommendations that they must make to their superiors, including the Attorney General, and sometimes the Governor, in the course of a settlement process.

1.11.04—Claim Compensation: If the Contractor proves entitlement for damages, payment shall be made in accordance with the following provisions:

1. **Compensable Items:** The liability of the Department for claims will be limited to the following specifically-identified items of cost, insofar as they have not otherwise been paid for by the Department (for instance, through payment for extra work, which under 1.04.05 includes overhead and profit), and insofar as they were caused solely by the actions or omissions of the Department or

its agents.

The Department will pay for direct labor expenses, direct costs for materials, and direct costs for active equipment use, plus an additional ten percent (10%) of the total amount of such direct costs as payment for home office overhead and profit.

Compensable delay-related costs: The Department will pay for any additional field office overhead and idle equipment costs for each day of Project Critical Path delay or suspension caused solely by action or inaction of the Department.

If the Critical Path delay or suspension period is less than 30 calendar days, the Department will pay an additional ten percent (10%) of the additional field office overhead costs as payment for home office overhead and profit. For delays less than 30 calendar days, idle equipment will be paid at 50% of the Rental Rate Blue Book rate.

For delays equal to or longer than 30 calendar days, the Department will pay a per diem rate, calculated as six percent (6%) of the original total Contract amount divided by the original number of days of Contract time, as payment for home office overhead and profit.

In paying for idle equipment equal to or longer than 30 calendar days, the Department will pay for actual equipment costs. Actual equipment costs shall be based upon records kept in the normal course of business and in accordance with generally-accepted accounting principles. Under no circumstances shall Blue Book or other guide or rental rates be used for this purpose (unless the Contractor had to rent the equipment from an unrelated party, in which case the actual rental charges paid by the Contractor, so long as they are reasonable, shall be reimbursed by the Department).

If the final Contract Value is greater than the original Contract Value, any delay-related costs that are compensable under this Article shall be reduced by eight percent (8%) of the difference between the final Contract Value and the original Contract Value.

Such payments for compensable delay-related costs shall be deemed to be complete and mutually-satisfactory compensation for field and home office overhead related to the period of delay or suspension.

Subcontractor costs of any kind, however, may be paid for by the Department only (a) in the context of a negotiated claims settlement or (b) if the Contractor has itself paid or legally-assumed, present unconditional liability for those subcontractor costs.

2. **Non-Compensable Items:** The Department will have no liability for the following specifically-identified non-compensable items:
   (1) Profit in excess of that provided for herein.
   (2) Loss of anticipated profit.
   (3) Loss of bidding opportunities.
   (4) Reduction of bidding capacity.
   (5) Home office overhead in excess of that provided for herein.
   (6) Attorney's fees, claims preparation expenses, or other costs of claims proceedings or resolution.
   (7) Any other consequential or indirect expenses or costs, such as tort damages, or any other form of expense or damages not provided for in these Specifications or elsewhere in the Contract.

1.11.05—Required Claim Documentation: All claims shall be submitted in writing to the Commissioner, and shall be sufficient in detail to enable the Engineer to ascertain the basis and the amount of each claim, and to investigate and evaluate each claim in detail. When submitting any claim over $50,000, the Contractor shall certify in writing, under oath and in accordance with the formalities required by the Contract, that the following are true:

1. That supporting data is accurate and complete to the Contractor's best knowledge and belief;
2. That the amount of the dispute and the dispute itself accurately reflects what the Contractor in good faith believes to be the Department's liability.

The certification shall be executed by an officer or general partner of the Contractor having overall responsibility for the conduct of the Contractor's affairs.

When submitting a claim to the Commissioner, as a minimum, the Contractor must provide the following information for each and every claim and sub-claim asserted:

(a) A detailed factual statement of the claim, with all dates, locations and items of work pertinent to the claim.
(b) A statement of whether each requested additional amount of compensation or extension of time is based on provisions of the Contract or on an alleged breach of the Contract. Each supporting or

breached Contract provision and a statement of the reasons why each such provision supports the claim must be specifically identified or explained.

(c) Excerpts from manuals or other texts which are standard in the industry, if available, that support the Contractor's claim.

(d) The details of the circumstances that gave rise to the claim.

(e) The date(s) on which any and all events resulting in the claim occurred, and the date(s) on which conditions resulting in the claim first became evident to the Contractor.

(f) Specific identification of any pertinent document, and detailed description of the substance of any material oral communication, relating to the substance of such claim.

(g) The name, function, and pertinent activity of each Contractor's or subcontractor's official, or employee involved in or knowledgeable about events that give rise to, or facts that relate to, the claim.

(h) The amount(s) of additional compensation sought and a break-down of the amount(s) into the categories specified as payable under 1.11.04 above.

(i) The name, function, and pertinent activity of each Department official, employee or agent involved in or knowledgeable about events that give rise to, or facts that relate to, the claim.

1.11.06—Auditing of Claims: All claims filed against the Department shall be subject to audit by the Department or its agents at any time following the filing of notice of such claim. The Contractor and its subcontractors and suppliers shall cooperate fully with the inquiries and document requests of the Department's auditors. Failure of the Contractor**,** its subcontractors, or its suppliers to maintain and retain records that are sufficient to enable the Department or its agents to fully evaluate the claim shall constitute a waiver of any portion of such claim that cannot be verified by specific, adequate, contemporaneous records, and shall bar recovery on any formal claim or any portion of such a claim for which such verification is not produced. Without limiting the foregoing requirements, and as a minimum, the Contractor shall make available to the Department and its agents the following documents in connection with any claim that the Contractor submits:

(1) Daily time sheets and foreman's daily reports.
(2) Union agreements, if any.
(3) Insurance, welfare, and benefits records.
(4) Payroll register.
(5) Earnings records.
(6) Payroll tax returns.
(7) Records of property tax payments.
(8) Material invoices, purchase orders, and all material and supply acquisition contracts.
(9) Materials cost distribution worksheets.
(10) Equipment records (list of company equipment, rates, cost pools, etc.).
(11) Vendor rental agreements.
(12) Subcontractor and vendor subcontracts, purchase orders, and/or agreements including all change orders and modifications.
(13) Subcontractor and vendor invoices to the Contractor, and the Contractor's certificates of payments to subcontractors and vendors.
(14) Subcontractor payment certificates.
(15) Canceled checks (payroll, subcontractors, and vendors).
(16) Job cost reports.
(17) Job payroll ledger.
(18) General ledger, general journal (if used), and all subsidiary ledgers and journals**,** together with all supporting documentation pertinent to entries made in these ledgers and journals.
(19) Cash disbursements journals.
(20) Financial statements for all years reflecting the operations on the Project.
(21) Income tax returns for all years reflecting the operations on the Project.
(22) Depreciation records on all company equipment**,** whether such records are maintained by the company involved, its accountant, or others.
(23) If a source other than depreciation records is used to develop costs for the Contractor's internal purposes in establishing the actual cost of owning and operating equipment, all such other source documents.

(24) All documents which reflect the Contractor's actual profit and overhead during the years that the Project was being performed**,** and for each of the five years prior to the commencement of the Project.
(25) All documents related to the preparation of the Contractor's bid**,** including the final calculations on which the bid was based.
(26) All documents which relate to the claim or to any sub-claim, together with all documents that support the amount of damages as to each claim or sub-claim.
(27) Worksheets used to prepare the claim, which indicate the cost components of each item of the claim, including but not limited to the pertinent costs of labor, benefits and insurance, materials, equipment, and subcontractors' damages, as well as all documents which establish the relevant time periods, individuals involved, and the Project hours and the rates for the individuals.

SECTION 1.20
GENERAL CLAUSES FOR FACILITIES CONSTRUCTION

SECTION 1.20-1.00
FACILITIES CONSTRUCTION - GENERAL

1.20-1.00—Facilities Construction - General

1.20-1.00—Facilities Construction - General: Facilities Construction is defined as the type of construction that requires the issuance of a Certificate of Compliance (C.O.C.) by the State Building Inspector at the completion of a Project, and includes site work considered ancillary to this type of construction.

SECTION 1.20-1.01
DEFINITION OF TERMS AND PERMISSIBLE ABBREVIATIONS FOR FACILITIES CONSTRUCTION

1.20-1.01.00—Facilities Construction - Definitions
1.20-1.01.02—Facilities Construction - Abbreviations, Publications and Standards
1.20-1.01.03—Facilities Construction - Abbreviations and Terms

1.20-1.01.01—Facilities Construction - Definitions: In these specifications, unless the context requires otherwise, words of the masculine gender include the feminine and the neuter, and, when the sense so indicates, words of the neuter gender may refer to any gender. Where appropriate, words in the singular form shall be deemed to include the plural, and words in the plural form to include the singular.

ADDENDUM: Contract revisions developed and incorporated into the contract after bid advertisement and before the opening of bid proposals.

AIR OPERATIONS AREA: Any paved or unpaved area of the airport used or intended to be used for the unobstructed movement of aircraft. These movements shall include landings, takeoffs, and surface maneuverings.

AWARD: The Department's acceptance in writing of the proposal of the lowest responsible bidder for the work, subject to the execution and approval by the Department of a contract therefor and the provision by the bidder of performance and payment bonds to secure the performance thereof which are acceptable to the Commissioner, and to such other conditions as may be specified by the Department or required by law.

BID: The submission of a proposal for the work contemplated.

BID ADVERTISEMENT: A public announcement requesting bids for a contract for work to be performed or materials to be furnished.

BIDDER: An individual or legal entity submitting a proposal in response to an advertised request.

BID MANUAL: "The State of Connecticut Department of Transportation Construction Contract Bidding and Award Manual," copies of which are available from the Department's Division of Contracts and at the following link: Construction Contract Bidding and Award Manual

CALENDAR DAY: Every day shown on the calendar, beginning and ending at midnight.

CATALOG CUT (PRODUCT DATA): Document(s) with information such as manufacturer's product specifications, manufacturer's installation instructions, standard color charts, wiring diagrams showing factory-installed wiring, printed performance curves and operational range diagrams. Product data that must be specially prepared because standard printed data is not suitable shall be considered shop drawings.

CERTIFICATE OF COMPLIANCE: The formal document issued at the completion of a project by the State Building Inspector. The document is often referred to informally as a "Certificate of Occupancy," "C.O.C." or "C.O."

CHANNEL: A channel shall be interpreted to mean a natural or artificial watercourse having an average width at the bottom, after excavation, of 4 feet or more.

COMMISSIONER: State of Connecticut Transportation Commissioner acting directly or through a duly-authorized representative.

CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL: This Department of Energy and Environmental Protection (DEEP) Bulletin is intended to provide information to government agencies and the public on soil erosion and sediment control.

CONNECTICUT STORMWATER QUALITY MANUAL: This DEEP publication provides guidance on measures necessary to protect waters of the State from adverse impacts of post-construction stormwater runoff.

CONSTRUCTION ORDER, CHANGE ORDER: A written order signed by the Engineer for a

contractor to perform work or provide supplies stipulated therein at the price or upon the basis of payment set forth therein.

CONTRACT: The agreement covering the performance of the work and the furnishing of materials required for the construction of the Project. The Contract shall be deemed to include the "Plans," "Specifications" (*i.e.*, the Department's "Standard Specifications for Roads, Bridges, Facilities and Incidental Construction" which is in effect on the date of the Bid Advertisement), "Construction Orders," and such other provisions as may be incorporated into the agreement, in addition to the contents of the bound contract containing the schedule of prices, signature sheet, addenda, special provisions, required federal and state provisions, supplemental specifications, labor and wage schedules, permits and other such material.

CONTRACTOR: Individual or legal entity contracting with the Department to perform the Work. When the word is capitalized, the party of the second part to the Contract, acting directly or through its agents or employees. When this word is not capitalized, it is to be taken in its more general sense.

CONTRACT PAY ITEM: Specific work unit for which the Contract provides a price.

COUNTY: The largest State administrative division used to designate or identify the location of the proposed work.

CULVERT: A covered channel or a large pipe for carrying a watercourse below ground level, usually under a road or railway.

DEPARTMENT: State of Connecticut Department of Transportation.

DESIGNER: A duly-authorized representative of the Engineer, responsible for the design of the Project.

DRAINAGE DITCH: A paved or unpaved, artificially-constructed open depression having an average width of less than 4 feet at the bottom, after excavation, constructed for the purpose of carrying off surface water.

ENGINEER: The Commissioner or Deputy Transportation Commissioner, acting directly or through a duly-authorized representative.

EXECUTION OF CONTRACT: The date of execution of the Contract by the Department is the date on which the Department's authorized signatory signs the Contract on behalf of the Department.

EQUAL: A material, device, type of equipment, or method other than what is specified in the Contract, which is a recognized equivalent in substance and function for that specified thing, taking into account warranty, performance, weight, size, visual effect, specific features and requirements indicated, quality, workmanship, economy of operation, durability, and suitability for purposes intended, provided that the proposed equivalent would not require or constitute a change in Contract work.

HIGHWAY: A general term denoting a public way used for vehicular travel. When referred to in the Contract, it signifies the whole right of way reserved for or secured by the Department for use in constructing or maintaining a roadway and its appurtenances.

INSPECTOR: A duly-authorized representative of the Engineer, assigned to make inspections of the work performed and associated materials.

LABORATORY: Department testing facility or other designated testing laboratory.

LIQUIDATED DAMAGES: The amount prescribed in the Contract specifications, to be paid to the State or to be deducted from any payments due or to become due the Contractor, for a specified time unit delay in completing the whole or any specified portion of the work beyond the time allowed in the Contract.

MAJOR ITEM: An individual Contract item, whose value at the time of bidding (either lump sum price or the product of its unit price multiplied by its estimate quantity) is equal to or greater than 10% of the total Contract bid price shall be considered a Major Item.

MAJOR LUMP SUM ITEM (MLSI): The original Contract item(s) that includes all work depicted on the Contract Plans, described in the Contract Specifications, or is otherwise required for performance and completion of the work, including mobilization and project closeout, but not including any unit price or other lump sum items listed in the Bid Proposal Form.

MANAGER OF CONTRACTS: The Transportation Manager of Contracts, who is the head of the Department's Division of Contracts, at email address DOTContracts@ct.gov and whose office is located at the headquarters of the Department at 2800 Berlin Turnpike, Newington, CT 06111.

MATERIAL: Any substance specified in the Contract for use in the construction of the Project, including appurtenances of products that are substantially shaped, cut, worked, mixed, finished, refined or otherwise fabricated, processed, or installed to form a part of the Work or become part of the constructed Project.

MILESTONE: An identified point of progress in the timeline of the Project.

MINOR ITEM: An individual Contract item that is not a Major Item.

MUNICIPALITY: City, town or county.
NOTICE TO PROCEED: Written direction to the Contractor stipulating the date for beginning the Work subject to other Contract requirements, including the date from which the Project time will be charged.
OWNER: Where used herein, it is synonymous with Department or State.
PAVEMENT STRUCTURE: The combination of sub-base, base course and surface course placed on subgrade to support and distribute the traffic load.
PLANS: Contract drawings showing location, type, dimensions, and details of specified work. This includes Standard Sheets.
PRODUCT DATA (CATALOG CUT): Document(s) with information such as manufacturer's product specifications, manufacturer's installation instructions, standard color charts, wiring diagrams showing factory-installed wiring, printed performance curves and operational range diagrams. Product data that must be specially prepared because standard printed data is not suitable shall be considered shop drawings.
PROJECT: All work included under one Department Contract. The Department may use multiple project numbers for the work included within one Contract.
PROJECT SITE (*or* SITE): The space available to the Contractor, under the Contract, for performing construction activities. The extent of the Project site is as indicated on the plans or elsewhere in the Contract.
QUALIFIED PRODUCTS LIST (QPL): A report that has been developed as a means for determining what products, suppliers, manufacturers, equipment and methodologies may be used on construction projects. This report can be located on the CT Department of Transportation Website:

Connecticut DOT Qualified Products List Report

RECLAIMED CONCRETE AGGREGATE: Reclaimed waste consisting of crushed and graded concrete removed from pavements, structures, or buildings. Metal may be acceptable only where it is contained as reinforcement within small fragments of concrete; *e.g.*, metal projecting from concrete fragments would be unacceptable. All such material trucked from beyond the limits of the Project must be accompanied by a Materials Certificate and Certified Test Report indicating that the material is environmentally acceptable and structurally sound in accordance with 1.20-1.06.07, unless the source of the material is a Department Project and that source is acceptable to the Engineer.
RECLAIMED MISCELLANEOUS AGGREGATE: Glass-free and clinker-free reclaimed waste that has been crushed, graded and blended, as specified in the Contract, with natural crushed stone or gravel. Metal may be acceptable only where it is contained as reinforcement within small fragments of concrete; *e.g.*, metal projecting from concrete fragments would be unacceptable. All such material trucked from beyond the limits of the Project must be accompanied by a Materials Certificate and Certified Test Report indicating that the material is environmentally acceptable and structurally sound in accordance with 1.20-1.06.07, unless the source of the material is a Department Project and that source is acceptable to the Engineer.
RECLAIMED WASTE: Debris from the demolition of buildings, structures, and pavements; residue from incineration and recycled glass. Acceptable material shall include concrete, bituminous concrete, glass, ceramics, brick, pavement sub-base and base courses, and clinker from resource recovery plants. Metal may be acceptable only when it is contained within large fragments of concrete. Reclaimed waste trucked from beyond the limits of the Project must be accompanied by a Materials Certificate and Certified Test Report indicating that the waste is environmentally acceptable and structurally sound in accordance with 1.20-1.06.07, unless the source of the material is a Department Project and that source is acceptable to the Engineer.
RESOURCES: The labor, equipment, and material necessary to perform work on a Contract bid item or other element of work.
RIGHT-OF-WAY: Land, property or interest therein acquired for or devoted to transportation purposes.
ROADBED: The graded portion of a highway, including portions within the top and side slopes, that has been prepared as a foundation for the pavement structure and shoulders.
ROADWAY: The portion of the highway, including shoulders, which may be used for vehicular travel within the Project limits.
SHOP DRAWINGS: Drawings, including proposed details, diagrams, schedules, procedures and other supporting data, prepared by a Contractor to supplement the Contract documents, showing all information necessary for fabrication of items for which some specific design or detail appears in the Contract.
SHOULDER: The portion of the roadway adjacent to the Traveled Way, that can accommodate stopped vehicles for emergency use, and that provides lateral support of base and surface courses.

SPECIFICATIONS: The Department's written provisions and requirements for the performance of the Contract, contained in or incorporated by the Contract.

A. *Standard Specifications*—A set of specifications approved by the Department for general application and repetitive use, available from the Manager of Contracts and entitled the "Standard Specifications for Roads, Bridges, Facilities and Incidental Construction" found at the following link: Specifications for Roads, Bridges, Facilities and Incidental Construction
B. *Supplemental Specifications*—Approved additions to and revisions of the Standard Specifications.
C. *Special Provisions*—Other Department specifications applicable to an individual project.

STATE: State of Connecticut.

STANDARD SHEETS: Standardized plans containing details, approved by the Department and the FHWA, for construction of a given type on any project included in all contracts on an as-needed basis.

SUBCONTRACTOR: Any individual, firm, partnership or corporation to which the Contractor sublets, with the approval of the Commissioner, any part or parts of the Project covered by the Contract.

SUBSTANTIAL COMPLETION: The date at which the performance of all work on the Project has been completed except minor or incidental items, final cleanup, work required under a warranty, and repair of unacceptable work, and provided the Engineer has determined that:

A. The Project is safe and convenient for use by the public, and
B. All traffic lanes including all safety appurtenances are in their final configuration, and
C. Failure to complete the work and repairs excepted above does not result in the deterioration of other completed work, and provided further, that the value of work remaining to be performed, and cleanup is less than one percent (1%) of the estimated final Contract amount, and
D. A Certificate of Compliance has been issued.

SUBSTITUTE: A replacement for a specified material, device, type of equipment, or method, which is sufficiently different in substance and function, quality, or workmanship to constitute a change in the Contract work.

SUBSTRUCTURE: All of that part of the bridge below the bearings of simple and continuous spans, skewbacks of arches and tops of footings of rigid frames, including backwalls, wingwalls and any protective railings mounted on the wingwalls.

SUB-SUBCONTRACTOR: Any individual, firm, partnership or corporation to which a subcontractor sublets, with the approval of the Commissioner, any part or parts of the Project covered by the Contract.

SUPERSTRUCTURE: The entire bridge except the substructure.

TRAVELED WAY: Portion of the right-of-way designated for vehicle use, excluding shoulders.

UTILITY: Any public service company and the plant of such a company or similar facilities. Such companies may consist of, but not be limited to, companies selling or controlling the sale, distribution or use of water, gas, electricity, communications systems, sewers and railroad lines. Such facilities may consist of, but not be limited to, wires, cables, ducts, pipes, manholes, transformers, poles, towers and tracks.

WATERCOURSE: Rivers, streams, brooks, waterways, lakes, ponds, marshes, swamps, bogs and all other bodies of water, natural or artificial, vernal or intermittent, public or private, which are contained within, flow through or border upon this state or any portion thereof.

WORK: The provision of labor, materials or services necessary for or relating to the design and construction of the Project.

WORKING DRAWINGS: Drawings, calculations, procedures and other supporting data prepared by a Contractor, documenting the Contractor's proposed design, details, materials, construction methods and equipment for any construction for which no specific design or detail appears in the Contract.

1.20-1.01.02—Facilities Construction - Abbreviations, Publications and Standards: For publications and standards, the edition governing the work will be the edition in effect on the date the Contract was advertised for solicitation of bids. Whenever one of the following abbreviations is used in the Contract, its meaning shall be interpreted as follows:

AA—(The) Aluminum Association, Inc.
AABC—Associated Air Balance Council
AAMA—American Architectural Manufacturers Association
AAN—American Association of Nurserymen
AAPA—American Association of Port Authorities
AASHTO—American Association of State Highway and Transportation Officials

NOTE: Wherever reference is made to an AASHTO Standard Method of Test or Standard Specification, it refers by letter and number to the method or specification published by AASHTO in the "Standard Specifications for Transportation Materials and Methods of Sampling and Testing."

ABMA—American Bearing Manufacturers Association
ACGIH—American Council of Government Industrial Hygienists
ACI—ACI International (American Concrete Institute)
ACOE—Army Corps of Engineers
ADA—Americans with Disabilities Act
ADAAG—Americans with Disabilities Act Accessibility Guidelines for Buildings and Facilities
ADSC—The International Association of Foundation Drilling (formerly Association of Drilled Shaft Contractors)
AF&PA—American Forest & Paper Association
AGA—American Gas Association
AGC—(The) Associated General Contractors of America
AHA—American Hardboard Association
AHAM—Association of Home Appliance Manufacturers
AI—Asphalt Institute
AIA—(The) American Institute of Architects
AISC—American Institute of Steel Construction
AISI—American Iron and Steel Institute
AITC—American Institute of Timber Construction
A.L.I.—Automotive Lift Institute
ALSC—American Lumber Standard Committee, Incorporated
AMCA—Air Movement and Control Association International, Inc.
AMRL—AASHTO Materials Reference Library
ANLA—American Nursery and Landscape Association
ANSI—American National Standards Institute
AOAC—AOAC International
AOSA—Association of Official Seed Analysts
APA—APA-The Engineered Wood Association
API—American Petroleum Institute
AREMA—American Railway Engineering and Maintenance-of-Way Association
ARI—Air-Conditioning & Refrigeration Institute
ARTBA—American Road and Transportation Builders Association
ASA—Acoustical Society of America
ASC—Adhesive and Sealant Council
ASCE—American Society of Civil Engineers
ASHRAE—American Society of Heating, Refrigerating and Air-Conditioning Engineers
ASME—ASME International (The American Society of Mechanical Engineers International)
ASNT—American Society for Non-Destructive Testing
ASSE—American Society of Sanitary Engineering
ASTM—American Society of Testing and Materials (ASTM International)

NOTE: Wherever reference is made to an ASTM specification, test method, or practice, it refers by letter, number, or both to standards published by ASTM International in the "ASTM Standards Source™ Database."

ATSSA—American Traffic Safety Services Association
AWI—Architectural Woodwork Institute
AWPA—American Wood Protection Association (*formerly* American Wood-Preservers' Association)
AWPI—American Wood Preservers Institute
AWS—American Welding Society

NOTE: Wherever reference is made to an AWS materials specification, inspection methods, or welding procedures, it refers by section number to standards of the American Welding Society published in the applicable steel, or aluminum welding code.

AWWA—American Water Works Association
BHMA—Builders Hardware Manufacturers Association
BIA—(The) Brick Industry Association

BOCA—BOCA International, Inc.
CBM—Certified Ballast Manufacturers Association
CCRL—Cement and Concrete Reference Laboratory
CDA—Copper Development Association (The)
CFR—Code of Federal Regulations
CGA—Compressed Gas Association
CGS—Connecticut General Statutes (as revised)
CISCA—Ceilings and Interior Systems Construction Association
CISPI—Cast Iron Soil Pipe Institute
CLFMI—Chain Link Fence Manufacturers Institute
ConnDOT—Connecticut Department of Transportation
CRI—(The) Carpet and Rug Institute
CRSI—Concrete Reinforcing Steel Institute
CSI—(The) Construction Specifications Institute
CSSB—Cedar Shake & Shingle Bureau
CTDOT—Connecticut Department of Transportation
CTI—Cooling Technology Institute
DASMA—Door and Access Systems Manufacturers Association, International
DEEP—Connecticut Department of Energy and Environmental Protection
DHI—Door and Hardware Institute
DOD—Department of Defense Military Specifications and Standards
EIA—Electronic Industries Alliance
EPA—Environmental Protection Agency
FAA—Federal Aviation Administration
FCC—Federal Communications Commission
FCICA—Floor Covering Installation Contractors Association
FHWA—Federal Highway Administration
FMG—FM Global
FRA—Federal Railway Administration
FS—Federal Specification

NOTE: Wherever reference is made to FS in the contract, it refers by number, letter, or both, to the latest standard or tentative standard of the Federal Specification Unit, General Services Administration, Federal Supply Service, as to materials, specifications, or methods of testing, whichever the case may be.

FTA—Federal Transit Administration
GA—Gypsum Association
GANA—Glass Association of North America
GSA—General Services Administration
HI—Hydraulics Institute
HPVA—Hardwood Plywood & Veneer Association
IAS—International Accreditation Service
ICC—International Code Council
ICC-ES—ICC Evaluation Service, Inc.
ICEA—Insulated Cable Engineers Association, Inc.
ICRI—International Concrete Repair Institute
IEC—International Electrotechnical Commission
IEEE—(The) Institute of Electrical and Electronics Engineers, Inc.
IES—Illuminating Engineers Society
IESNA—Illuminating Engineering Society of North America
IGCC—Insulating Glass Certification Council
IGMA—Insulating Glass Manufacturers Alliance
IMSA—International Municipal Signal Association
IRI—HSB Industrial Risk Insurers
ISO—International Organization for Standardization
ITE—Institute of Traffic Engineers
ITSNA—Intertek Testing Services, Inc. (*formerly* Edison Testing Laboratories)

IUPAT—International Union of Painters and Allied Trades
KCMA—Kitchen Cabinet Manufacturers Association
LMA—Laminating Materials Association
LPI—Lightning Protection Institute
LWRD—Land and Water Resources Division
MASH—Manual for Assessing Safety Hardware
MBMA—Metal Building Manufacturers Association
MILSPEC—Military Specification and Standards
MMA—Monorail Manufacturers Association
MSHA—Mine Safety and Health Administration
MSS—Manufacturers Standardization Society of The Valve and Fittings Industry, Inc.
MUTCD—Manual on Uniform Traffic Control Devices
NAAMM—National Association of Architectural Metal Manufacturers
NACE—National Association of Corrosion Engineers
NADCA—National Air Duct Cleaners Association
NAIMA—(The) North American Insulation Manufacturers Association
NBFU—National Board of Fire Underwriters
NCHRP—National Cooperative Highway Research Program
NCMA—National Concrete Masonry Association
NCPI—National Clay Pipe Institute
NCSPA—National Corrugated Steel Pipe Association
NEAUPG—NorthEast Asphalt User/Producer Group
NEBB—Natural Environmental Balancing Bureau
NEC—National Electrical Code
NECA—National Electrical Contractors Association
NEMA—National Electrical Manufacturers Association
NEPCOAT—North East Protective Coatings Committee
NESC—National Electrical Safety Code
NETA—InterNational Testing Association
NETTCP—NorthEast Transportation Technician Certification Program
NFPA—National Fire Protection Association
NFRC—National Fenestration Rating Council
NHLA—National Hardwood Lumber Association
NICET—National Institute for Certification in Engineering Technologies
NIOSH—National Institute of Occupational Safety and Health
NIST—National Institute of Standards and Technology
NLGA—National Lumber Grades Authority
NMFS—National Marine Fisheries Service
NOAA—National Oceanic and Atmospheric Administration
NRCA—National Roofing Contractors Association
NRMCA—National Ready Mix Concrete Association
NSBA—National Steel Bridge Alliance
NSC—National Safety Council
NSF—NSF International
NTMA—National Terrazzo and Mosaic Association, Inc.
OEO—Office of Equal Opportunity
OSHA—Occupational Safety and Health Administration
PCA—Portland Cement Association
PCI—Precast/Prestressed Concrete Institute
PDI—Plumbing & Drainage Institute
PROWAG—Public Right-of-Way Accessibility Guidelines
PTI—Post-Tensioning Institute
PURA—Public Utilities Regulatory Authority
RCSA—Regulations of Connecticut State Agencies
RFCI—Resilient Floor Covering Institute
RMA—Rubber Manufacturers Association

SAE—SAE International (formerly Society of Automotive Engineers)
SDI—Steel Deck Institute *or* Steel Door Institute
SFPA—Southern Forest Products Association
SHRP—Strategic Highway Research Program
SJI—Steel Joist Institute
SMACNA—Sheet Metal and Air Conditioning Contractors National Association
SPIB—(The) Southern Pine Inspection Bureau
SPRI—Single Ply Roofing Institute
SSPC—The Society for Protective Coatings (*formerly* The Steel Structures Painting Council)
NOTE: Where reference is made to SSPC in the Contract, it refers by number, letter, or both, to the latest standard or tentative standard specification as to materials specifications, methods of testing, systems, procedures, inspection or other specification pertaining to any or all phases of cleaning or painting, whichever may apply.
SWRI—Sealant, Waterproofing, & Restoration Institute
TCA—Tile Council of America, Inc.
TIA—Telecommunications Industry Association
TIA/EIA—Telecommunications Industry Association/Electronics Industries Alliance
TPI—Truss Plate Institute, Inc.
TRB—Transportation Research Board
UFAS—Uniform Federal Accessibility Standards
UL—Underwriters Laboratories Inc.
USACOE—United States Army Corps of Engineers
USCG—United States Coast Guard
USDA—United States Department of Agriculture
USFWS—United States Fisheries and Wildlife Service
USGBC—U.S. Green Building Council
USSWG—United States Steel Wire Gauge
WCLIB—West Coast Lumber Inspection Bureau
WCSC—Window Covering Safety Council
WDMA—Window & Door Manufacturers Association
WWPA—Western Wood Products Association

1.20-1.01.03—Abbreviations and Terms: Abbreviations and terms used in the Contract are in lieu of and are to be construed in the same way as are the terms or phrases following them in the list below. Those abbreviations and terms include, but are not necessarily limited to:
ABS—acrylonitrile butadiene styrene
AC—alternating current
ACCM Pipe or ACCMP—Asphalt-Coated Corrugated Metal Pipe
ACSR—Aluminum Conductor, Steel Reinforced
A.E.A.—Air Entraining Admixture
AIC—Ampere Interrupting Current
AOEC—Area of Environmental Concern
APA—Aquifer Protection Area
AWG—American Wire Gauge
B & B—balled and burlapped
bbl—barrel
BCPC—Bituminous Concrete Park Curbing
Bit.—bituminous
Bit. Conc.—bituminous concrete
CAPWAP—CAse Pile Wave Analysis Program
CAS—Coating Applicator Specialist
CB—catch basin
CCA—chromated copper arsenate
CCM Pipe or CCMP—coated corrugated metal pipe
CICU—controller interface communications unit1
CLLCU—closed loop local coordination unit

CLMU—closed loop master unit
CMS—Changeable Message Sign
Conc.—concrete
CPE Pipe or CPEP—corrugated plastic or polyethylene pipe
CPS—centipoise second
CWI— Certified Welding Inspector
cwt.—hundredweight or 100 pounds
DBE—Disadvantaged Business Enterprise
DBH—Diameter at breast height
DC—direct current
dist.—distillation
DMT—Division of Materials Testing
DTI—Direct Tension Indicator
EW—endwall
est.—estimated
exc—excavation
fi—jacking tension
FRC—Fiberglass Reinforced Composite
f’ c—specified minimum compressive strength at a specified age
f' ci—required strength at time of transfer
ga—gauge or gage
GFRP—glass fiber reinforced polymer
Gsa—Apparent specific gravity
Gsb—Bulk specific gravity
HASP—Health and Safety Plan
HMA—hot mix asphalt or bituminous concrete
HRB—Rockwell B Scale hardness
HRC—Rockwell C Scale hardness
Hz—hertz
IMC—Intermediate Metal Conduit
IP—internet protocol
I.P.S.—Iron Pipe Size
IWRC—Independent Wire Rope Core
JMF—job mix formula
KD—Kiln Dried
KDAT—Kiln Dried After Treatment
kip—1000 pounds
ksf—kips per square foot
LCD—Liquid Crystal Display
LED—light-emitting diode
LID—Low Impact Development
LRFD—Load & Resistance Factor Design
l.s.—lump sum
mbf—1000-foot board measure
MBR—metal beam rail
Mgal—1000 gallons
MH—manhole
MLSI—Major Lump Sum Item
MS4—Municipal Separate Storm Sewer Systems
MSDS—Material Safety Data Sheet(s)
MPT—Maintenance and Protection of Traffic
N.C.—National Coarse
NDT—non-destructive testing
Pavt.—pavement
PCBC—precast concrete barrier curb
PCC—Portland Cement Concrete

PE—polyethylene
Perf. ACCM Pipe or Perf. ACCMP—Perforated Asphalt-Coated Corrugated Metal Pipe
Perf. CCM Pipe or Perf. CCMP—Perforated Coated Corrugated Metal Pipe
Perf. CPE Pipe or Perf. CPEP—Perforated Corrugated Plastic or Polyethylene Pipe
pfmd.—preformed
PROM—programmable read only memory
psf—pounds per square foot
psi—pounds per square inch
p/s—prestressed
PVC—polyvinyl chloride
Pwa—Percent water absorbed
QCPFA—Quality Control Plan for Fine Aggregates
RAP—reclaimed asphalt pavement
RC—Reinforced Concrete
RCCE—Reinforced Concrete Culvert End
RC Pipe *or* RCP—Reinforced Concrete Pipe
RFC—Request for Change
RFI—Request for Information
RMC—Rigid Metal Conduit
SD—system detector
SDR—Standard Dimension Ratio (ratio of pipe diameter to wall thickness)
sec.—second
sol.—soluble
S.O.W.—Service cord (Oil resistant jacket, Weather/water resistant)
sp. gr.—specific gravity
sp. visc.—specific viscosity
SSA—Sole Source Aquifer
SSP—Subspecies
std.—standard
surf.—surface
TDC—Transportation Division Chief
THHN—Heat resistant thermoplastic, insulated nylon jacket, 90 degrees Centigrade, 600 volt building wire
THWN—Moisture and heat resistant thermoplastic, insulated nylon jacket, 75 degrees Centigrade, 600 volt building wire
TL—Test Level
TMA—Truck Mounted Impact Attenuator
TMP—Transportation Management Plan
tsf—tons per square foot
TTC—Temporary Traffic Control
U'drain *or* Udrain—Underdrain
UNC—Unified National Coarse
UNS—Unified Numbering System
USB—Universal Serial Bus
UTCS—urban traffic control system
UV—ultra-violet or ultra violet light
VAC—Volts Alternating Current
VDC—Volts Direct Current
VECP—Value Engineering Change Proposal
VMS—Variable Message Sign
VOC—Volatile Organic Compound
VT—vitrified tile
W—watt
WSA—Temporary Waste Stockpile Area

SECTION 1.20-1.02
PROPOSAL REQUIREMENTS AND CONDITIONS FOR FACILITIES CONSTRUCTION

1.20-1.02.01—Facilities Construction - Contract Bidding and Award
1.20-1.02.02—Facilities Construction - Interpreting Bid Proposal Quantities
1.20-1.02.03—Facilities Construction - Examination of Contract Documents and Work Site
1.20-1.02.04—Facilities Construction - Knowledge of Applicable Laws

1.20-1.02.01—Facilities Construction – Contract Bidding and Award: All bids for construction contracts must be submitted electronically. It is the responsibility of each bidder and all other interested parties to obtain all bidding related information and documents from the Department of Administrative Services (DAS) State Contracting Portal.

Connecticut Department of Transportation bidding and other information and documents which are obtained from any other source must not be submitted to the Department. Reproduced, reformatted or altered forms of documents are not authorized or acceptable.

For information about the bidding and award of Department construction contracts, consult the "State of Connecticut Department of Transportation Construction Contract Bidding and Award Manual," available from the Division of Contracts. In order to be eligible for award of a Department construction contract, a bidder must follow the requirements of this Bid Manual, and all bidding and award matters regarding Department construction contracts shall be governed by the terms of the Bid Manual, unless treated otherwise in the Contract, including these Specifications.

1.20-1.02.02—Facilities Construction – Interpreting Bid Proposal Quantities: The quantities shown on the proposal form are approximate only and are given as a basis for the pricing upon which the award of the Contract will be made. The Department does not warrant that these quantities shall remain unchanged in the actual construction, and the Contractor may not plead misunderstanding or deception because of any variation between estimated and final quantities. The Engineer reserves the right to increase or decrease any or all of the quantities shown on the proposal form as may be necessary to properly complete the Project.

The Department will pay for the actual quantity of authorized and accepted work done or material furnished under each of the items.

1.20-1.02.03—Facilities Construction - Examination of Contract Documents and Work Site: The bidder is required to examine the site of the Contract work and the proposal form, plans, special provisions, specifications, supplemental specifications, Contract form and other Contract documents for the work contemplated, as well as any permits or permit applications that are likely to affect the Contract work. The bidder must judge for itself and satisfy itself as to the conditions to be encountered; the character, quality and quantities of the work to be performed; the materials to be furnished; and the requirements of the above documents, particularly the requirements under each Contract item, under the general cost of the work, or under other applicable, but more general, provisions, of the Contract.

The subsurface information furnished in the Contract is based on the interpretation, by the Department, of investigations made only at the specific locations indicated; and the Department gives no assurance that the conditions discovered are typical of the conditions at other Project site locations or that those conditions will have remained unchanged since the field data were obtained. The Department also gives no assurance that the presence or absence of subsurface water at the time and locations of these explorations will be representative of actual conditions at the time of construction. Such subsurface information as was obtained by the Department for its use in the design of the Project will be available for inspection by bidders through the Division of Contracts. Also, bidders may arrange through the Division of Contracts an opportunity to examine, in advance of bidding, at a location to be specified by the Department, any available samples of the materials encountered in the Department's subsurface explorations. The Contractor shall be solely responsible for all assumptions, deductions, or conclusions it may make or derive from its examination of any Department subsurface information, document or sample. In furnishing or making available such information, the Department makes no warranty or representation as to the actual conditions that may be encountered or actual quantities or distribution of quantities of work that will be required in the course of the Project.

The Department does not intend or warrant that plan sheets furnished to the State by utility companies whose facilities may be affected by the proposed construction will show all proposed utility work that will

be done by utility companies or municipal authorities or both before, during, or after the life of this Contract. In addition to the work indicated on such plan sheets, the utility companies and authorities may make adjustments to or remove certain of their installations other than those indicated on the plans, or may install facilities not so indicated.

Bidders must inform the Department in writing, at the earliest opportunity, of any and all omissions, errors, and/or discrepancies that the bidder discovers within or among the plans, specifications, and bidding documents. Information and inquiries concerning such matters, and any other information or inquiry concerning the conditions of bidding or award or the interpretation of contract documents, must be transmitted in writing to DOTContracts@ct.gov. The Department cannot ensure a response to inquiries received later than ten (10) days prior to the scheduled opening of the related bid. When the Department deems it warranted, responses to such inquiries that relate to changes in or interpretations of the Project documents (plans and specifications) will be issued to all bidders in the form of addenda and made a part of the Contract. Bidders are responsible for ensuring that they are aware of all addenda.

CSI-formatted specifications are organized into Divisions and Sections based on the CSI's "MasterFormat" numbering system. CSI-formatted specifications use certain conventions for the style of language and the intended meaning of certain terms, words, and phrases when used in particular situations. These conventions are as follows:

(a) Language used is abbreviated. Words and meanings shall be interpreted as appropriate. Words implied, but not stated, shall be interpreted as the sense requires. Singular words shall be interpreted as plural and plural words shall be interpreted as singular where applicable as the context indicates.

(b) Imperative mood and streamlined language are generally used. Requirements expressed in the imperative mood are to be performed by the Contractor. Subjective language is used for clarity to describe responsibilities that must be fulfilled indirectly by the Contractor or by others when so noted.

(c) The words "shall," "shall be," or "shall comply with" are implied where a colon (:) is used within a sentence or phrase.

1.20-1.02.04—Facilities Construction - Knowledge of Applicable Laws: Bidders shall be deemed to know and understand all federal, state and local laws, ordinances and regulations and municipal bylaws which in any manner apply to projects for which they bid; such legal requirements shall include, but not necessarily be limited to, those which apply to the conduct of the Contract work, the equipment and materials to be used on the Project, or the treatment of individuals or classes of individuals in relationship to their involvement with the Project. A Contractor's ignorance of such requirements shall not, in any internal Department proceeding or in any claims or other legal proceeding, constitute justification for the Contractor's failure to consider such requirements in formulating a bid proposal, or for the Contractor's failure to ensure that such legal requirements are met with regard to any Department project in which that Contractor participates.

The Contractor agrees that if it should be awarded the contract for any project supported at least in part by federal funding, the Contractor will not knowingly enter into any lower-tier transaction on that project with a person (including entities) who, by virtue of federal law or regulation, or by voluntary agreement, is currently ineligible to participate in such a project, unless after disclosure of such ineligibility, such participation is authorized by appropriate federal and State authorities.

The Department expects the Contractor to obey municipal laws and regulations and cooperate with municipal officials. In some instances, however, municipal laws or regulations, or the orders of municipal officials, may conflict with necessary Project activities. In most such cases, the municipality does not have the legal power to enforce its laws and regulations upon the State or upon a State project. This is because the State is protected by its sovereign immunity. If local police or other authorities should attempt to stop the Contractor from carrying out activities that are necessary in order for the Contractor to comply with Contract requirements, the Contractor should politely inform the municipal authorities that they probably do not have jurisdiction over the State's project, and the Contractor should immediately inform the Engineer of the attempted interference with Project activities. If the municipal authorities continue to insist upon preventing the Contractor from carrying out Project activities, the Contractor should not defy the authorities, but, to the extent possible, should await directions from the Engineer.

All work to be performed by the Contractor shall comply with, as a minimum, the State Building Code as adopted pursuant to CGS 29-252, as amended; the State Fire Prevention Code as adopted pursuant to CGS 29-291a, as amended; and the Fire Safety Code as adopted pursuant to CGS 29-292, as amended.

The State Building Code, including latest Connecticut Supplements and Amendments, includes the

following:

1. The 2015 International Building Code.
2. The 2015 International Plumbing Code.
3. The 2015 International Mechanical Code.
4. The 2015 International Existing Building Code.
5. The 2015 International Energy Conservation Code.
6. The 2017 NFPA 70 National Electrical Code.
7. The 2009 ICC/ANSI A117.1.

The State Fire Safety Code, including latest Connecticut Supplements and Amendments, includes the following:

1. The 2015 International Fire Code.
2. The 2015 NFPA 101 Life Safety Code.

The State Fire Prevention Code, including latest Connecticut Supplements and Amendments, includes the following:

1. The 2015 NFPA 1.

The edition of the code governing the Project shall be the code which is in effect as per the above CGS Sections on the date that the Contract is advertised for solicitation of bids.

All work to be performed by the Contractor shall comply with the 2010 Department of Justice "ADA Standards for Accessible Design."

SECTION 1.20-1.03
AWARD AND EXECUTION OF CONTRACT FOR FACILITIES CONSTRUCTION

1.20-1.03.01—Facilities Construction - Consideration of Bids
1.20-1.03.02—Facilities Construction - Award and Execution of Contract
1.20-1.03.03—Facilities Construction - Return of Proposal Guaranty
1.20-1.03.04—Facilities Construction - Requirements of Performance Contract Bond and Payment Bond
1.20-1.03.05—Facilities Construction - Vacant
1.20-1.03.06—Facilities Construction - Failure to Execute Contract
1.20-1.03.07—Facilities Construction - Insurance
1.20-1.03.08—Facilities Construction - Notice to Proceed and Commencement of Work
1.20-1.03.09—Facilities Construction - Contractor Readiness

1.20-1.03.01—Facilities Construction - Consideration of Bids: See 1.20-1.02.01.

The apparent low bidder shall submit to the Manager of Contracts at email address DOTContracts@ct.gov a Schedule of Values within 7 calendar days after bid opening. Any other Contractor that the Department may subsequently designate as the apparent lowest bidder shall make the aforesaid submission within 7 calendar days from the date on which the Department notifies said Contractor that it has become the apparent lowest bidder. If, however, the Department deems it necessary for such a subsequently designated Contractor to make said submission within a shorter period of time, the Contractor shall make the submission within the time designated by the Department.

The total in the Schedule of Values shall equal the bid dollar amount for the MLSI.

The Schedule of Values shall be divided into "Line Items" listed separately for each CSI Section of the Special Provisions. An additional line item for "Mobilization" may be incorporated into the Schedule of Values; however, this item may not exceed 7.5% of the value of the MLSI. The "Mobilization" line item will also include costs associated with "General Conditions," "Insurance/Bonding," and "Project Superintendent." An additional line item for "Project Closeout" shall be incorporated into the Schedule of Values; however, this item must be at least 2.5% of the value of the MLSI. Where requested by the Department, the Contractor shall breakdown the line items further into more specific line items.

In the event that this Contract is terminated or a portion of this Contract is deleted for any reason or in any way allowable by law under this Contract after the apparent low bidder has been awarded the Contract, the Schedule of Values will not be used for estimating payment due the Contractor for work completed prior to such termination of the Contract or deletion of work thereunder. In the case of Contract termination, payment shall be made in accordance with 1.20-1.05.14.

1.20-1.03.02—Facilities Construction - Award and Execution of Contract: Except as otherwise authorized by the Commissioner, all contracts will be awarded and executed in accordance with the order of the Commissioner. The award, if made, will be made within 60 days after the opening of the proposals unless otherwise agreed upon by the Commissioner, the successful bidder, and the surety. The successful bidder, upon receipt of notice from the Department that the contract is ready for execution, shall, at the time and place designated in said notice, be present in person or be represented by an official legally authorized to sign the Contract, and shall there and then sign the necessary Project contract with the State. No proposal shall be considered binding upon the State until the proper execution of the Contract by both parties.

1.20-1.03.03—Facilities Construction - Return of Proposal Guaranty: All proposal guaranties will be returned within 3 calendar days following the award of the Contract. Ten calendar days after the opening of the proposals, all guaranties, except those of the 3 lowest bidders, will be returned. Should no award be made within 60 calendar days after the opening of proposals, the Commissioner may reject all proposals and return the proposal guaranties, except that with the approval of the lowest bidder and its surety, the Commissioner may extend the time for the award and may retain the proposal and proposal guaranty of the lowest bidder for said extended time, or for any other period of time agreed upon by the Commissioner, bidder and surety.

1.20-1.03.04—Facilities Construction - Requirements of Performance Contract Bond and Payment Bond: See 1.20-1.02.01.

In conformance with Section 49-41a of the Connecticut General Statutes, as revised, the Contractor (1)

shall, within 30 days after any given Contract payment to the Contractor by the State, pay any amounts due any subcontractor, whether for labor performed or materials furnished, when charges for such labor or materials have been included in a payment estimate paid by the State; (2) and shall include in each of its subcontracts a provision requiring each subcontractor to pay any amounts due any of its subcontractors on the Project, whether for labor performed or materials furnished, within 30 days after such subcontractor receives a payment from the Contractor which encompasses labor or materials furnished by such subcontractor for the Project.

If the Contractor believes that it has a valid reason for withholding payment for particular work or materials from a subcontractor or supplier, then the Contractor, within 30 days of receiving payment from the State for that work or materials, shall notify the subcontractor or supplier and the Department of its reasons for withholding payment.

1.20.1.03.05—Facilities Construction - Vacant

1.20-1.03.06—Facilities Construction - Failure to Execute Contract: See 1.20-1.02.01.

1.20-1.03.07—Facilities Construction - Insurance:

Coverage shall be on a primary basis.

The Contractor shall carry and maintain at all times during the term of the Contract the insurance coverages required by this Article and any additional coverages(s) or higher minimum insurance coverage amount(s) required by the Special Provisions of the Contract.

If the Project includes work on or adjacent to railroad property additional insurance may be required as specified by the railroad. Please refer to the Special Provisions for any additional insurance requirements by the railroad.

1. Workers' Compensation Insurance: With respect to all operations the Contractor performs and all those performed for it by subcontractors, the Contractor shall carry, and require each subcontractor to carry, Workers' Compensation insurance as required by the laws of the State of Connecticut.

Employer's Liability insurance shall be provided in amounts not less than $100,000 per accident for bodily injury by accident; $100,000 policy limit by disease and $100,000 per employee for bodily injury by disease. Each Workers' Compensation policy shall contain the U.S. Longshoreman's and Harbor Workers' Act endorsement when work is to be performed over or adjacent to navigable water.

2. Commercial General Liability Insurance: With respect to the operations the Contractor performs and also those performed for it by subcontractors, the Contractor shall carry, and require each subcontractor to carry, Commercial General Liability insurance, including Contractual Liability, Products and Completed Operations, Broad Form Property Damage and Independent Contractors.

Products and completed operations insurance for ongoing and completed operations shall be maintained for a period of one (1) year after the acceptance of the project by the Department in accordance with 1.20-1.08.14. See chart below for applicable minimum coverage amounts.

Contract Amount ($)	Minimum Single Occurrence Amount ($)	Minimum Annual Aggregate Amount ($)
0-2,000,000	1,000,000	2,000,000
>2,000,001-10,000,000	2,000,000	4,000,000
>10,000,000	4,000,000	8,000,000

Each policy shall have coverage for and exclusions removed for "Explosion, Collapse and Underground" ("XCU") if underground work is to be undertaken.

3. Automobile Liability Insurance: The Contractor shall obtain automobile liability insurance covering the operation of all motor vehicles, including those hired or borrowed, that are used in connection with the Project for all damages arising out of: (1) bodily injury to or death of all persons and/or (2) injury to or destruction of property; in any one accident or occurrence. This policy shall not be subject to an annual aggregate limitation. See chart above for applicable minimum coverage amounts.

4. Owner's and Contractor's Protective Liability Insurance for and in the Name of the State: With respect to the Contractor's Project operations and also those of its subcontractors, the Contractor shall carry, for and on behalf of the State for each accident or occurrence resulting in damages from (1) bodily injury to or death of persons and/or (2) injury to or destruction of property. See chart below for applicable minimum coverage amounts.

Contract Amount ($)	Minimum Single Occurrence Amount ($)	Minimum Annual Aggregate Amount ($)
0 - 20 Million	1,000,000	1,000,000
20 Million - 50 Million	2,000,000	2,000,000
> 50 Million	4,000,000	4,000,000

5. Railroad Protective Liability Insurance: When the Contract involves work within fifty (50) feet of the railroad right-of-way or State-owned rail property, with respect to Project operations and also those of its subcontractors, the Contractor shall carry Railroad Protective Liability Insurance providing coverage of at least $2,000,000 for each accident or occurrence resulting in damages from

(1) bodily injury to or death of all persons and

(2) injury to or destruction of property, and subject to that limit per accident or occurrence, an aggregate coverage of at least $6,000,000 for all damages during the policy period, and with all entities falling within any of the following listed categories named as insured parties:

(i) the owner of the railroad right-of-way,

(ii) the owner of any railcar licensed or permitted to travel within that affected portion of railroad right-of-way, and

(iii) the operator of any railcar licensed or permitted to travel within that affected portion of the railroad right-of-way, and with the State, if not falling within any of the above-listed categories, also named as an insured party.

6. Blasting: When explosives are to be used in the Project, the Commercial General Liability insurance policy shall include XCU coverage, in the same limits as the per occurrence policy limits.

7. Protection and Indemnity Insurance for Marine Construction Operations in Navigable Waters: If a vessel of any kind will be involved in Project work, the Contractor shall obtain the following additional insurance.

coverage:

A. Protection and Indemnity Coverage of at least $300,000 per vessel or equal to at least the value of hull and machinery, whichever is greater.

B. If there is any limitation or exclusion with regard to crew and employees under the protection and indemnity form, the Contractor must obtain and keep in effect throughout the Project a workers' compensation policy, including coverage for operations under admiralty jurisdiction, with a limit of liability of at least $300,000 per accident or a limit equal to at least the value of the hull and machinery, whichever is greater, or for any amount otherwise required by statute.

8. Builder's Risk Insurance: The Contractor shall maintain comprehensive replacement cost builder's risk (completed value) insurance providing coverage for the entire work at the Project site, including all fixtures, machinery and equipment, any heating, cooling and constituting a permanent part of the building and shall cover portions of work located away from the site, but intended for use at the site. If it is determined that all or a portion of the project is located within an area designated as a Special Flood Hazard Area, the Contractor shall maintain flood insurance (no less than $10,000,000 sublimit). The State of Connecticut shall be named as Loss Payee. Equipment breakdown coverage may be sub limited to 50% of the project cost.

9. Architects and Engineer's Professional Liability Insurance for Structural Engineer: If required, limits will be specified in 1.20-1.03.07 of the Special Provisions of the Contract or 1.20-1.05.02.

10. Umbrella Liability Insurance: The Contractor may satisfy the minimum limits required for Commercial General Liability and Automobile Liability Insurance using Umbrella Liability Insurance. In the event that the Contractor obtains Umbrella Liability Insurance to meet the minimum coverage requirements for Commercial General Liability or Automobile Liability Insurance coverage, the Umbrella Liability Insurance policy shall have an annual aggregate at a limit not less than twice the single occurrence and must specifically endorse the State of Connecticut as an additional insured. Specifically for Bridge Projects with a low bid equal to or higher than $80,000,000, the Umbrella Liability Insurance policy must have a minimum limit of at least $25,000,000.

11. Certificate of Insurance: Before the Contract is executed, the Contractor must provide to the Department a certificate of insurance acceptable to the Commissioner and executed by an insurance

company or companies satisfactory to the State of Connecticut for the insurance coverage(s) required by this Article and the Special Provisions of the Contract. The Contractor shall maintain the required insurance coverage during the entire term of the Contract. The certificate of insurance must clearly include the name of the insured and identify the project for which it is being issued.

12. Copies of Policies: The Contractor shall provide, within five (5) business days, a copy or copies of all applicable insurance policies when requested by the State. In providing said policies, the Contractor may redact provisions of the policy that are proprietary. This provision shall survive the expiration or termination of the Contract.

13. **Sovereign Immunity:** The Contractor may not assert the defense of sovereign immunity in the adjustment of claims or in the defense of any claim or suit brought against the Contractor or the State, unless the State, in writing, requests that the Contractor do so or consents to its doing.

14. Contractor Assumes Costs: The Contractor shall assume and pay all costs and billings for premiums, deductibles, self-insured retentions and audit charges earned and payable under the required insurance.

15. State Named as Additional Insured: The State must be named as an additional insured party for the Commercial General Liability and Automobile Liability insurance policies required by this Article and the Special Provisions to the Contract, and any Umbrella Liability Insurance, as applicable, obtained in accordance with this Article. Each policy shall waive right of recovery (waiver of subrogation) against the State of Connecticut.

16. Termination or Change of Insurance:

A. The Contractor shall notify the Department of any cancelation of insurance carrier or change to the required insurance coverage by submitting a new insurance certificate to the Department immediately following said cancelation or change in required coverage.

B. It is the responsibility of the Contractor to maintain evidence of a current insurance coverage with the Department for the duration of contract. It is the responsibility of the Contractor to file with the Department all renewals and new certificates of insurance issued due to changes in policy terms or changes in insurance carriers prior to the expiration dates on the forms already on file with the Department.

17. Duration of Coverage. The Contractor shall keep all the required insurance in continuous effect until the date that the Department designates for the termination of the Contractor's responsibility, as defined by 1.20-1.08.14.

18. Compensation: There shall be no direct compensation allowed the Contractor on account of any premium or other charge necessary to obtain and keep in effect any insurance or bonds in connection with the Project, but the cost thereof shall be considered included in the general cost of the Project work.

1.20-1.03.08—Facilities Construction - Notice to Proceed and Commencement of Work: The Contractor shall commence and proceed with the Contract work on the date specified in a written Notice to Proceed issued by the Engineer to the Contractor. The date specified will be no later than 45 calendar days after the date of the execution of the Contract by the Department.

If the Engineer does not issue a Notice to Proceed to the Contractor within the said 45 calendar days, the Contractor shall have the option of canceling the Contract and its payment and performance bonds for the Project. Any failure by the Department to issue a notice to proceed, or to issue one on a timely basis, shall not, however, constitute a breach of the Contract. Neither the Contractor nor any other party may use such a failure as a basis for any claim against the Department for damages.

The Contractor shall not begin physical Project construction prior to the date specified for same by the Engineer in the Notice to Proceed, except as may be otherwise authorized by the Engineer in writing.

1.20-1.03.09—Facilities Construction - Contractor Readiness: Immediately upon Contract execution, the Contractor shall prepare submittals and procure materials in order to complete the Project work within the Contract time.

SECTION 1.20-1.04
SCOPE OF WORK FOR
FACILITIES CONSTRUCTION

1.20-1.04.01—Facilities Construction - Intent of Contract
1.20-1.04.02—Facilities Construction - Changes in Quantities of Pay, Including Elimination of Such Items
1.20-1.04.03—Facilities Construction - Changes in Quantities and Significant Changes in the Character of Work
1.20-1.04.04—Facilities Construction - Differing Site Conditions
1.20-1.04.05—Facilities Construction - Extra Work
1.20-1.04.06—Facilities Construction - Removal and Disposal of Structures on the Work Site
1.20-1.04.07—Facilities Construction - Rights in and Use of Materials Found on the Work Site

1.20-1.04.01—Facilities Construction - Intent of Contract: The Contract directs and obliges the Contractor to perform the Project described in strict compliance with the Contract terms, including its specifications, plans, special provisions, and other Contract documents. If the Engineer revises any of those terms in writing during the life of the Contract, the Contractor must comply with said revised terms. Among other things, the Contract obliges the Contractor to perform all Project work in conformity with the lines, grades, typical cross-sections, dimensions, and other data shown on the plans or other Contract documents. The Department will pay the Contractor only for work (including materials necessary for that work, whether or not they are incorporated into that work) that the Contractor has actually performed under a Contract pay item, and only if the Engineer has accepted said work. (See 1.20-1.02.02 herein.) (The Contract as it existed when first duly executed by the Engineer is sometimes referred to herein as "the original Contract.")

1.20-1.04.02—Facilities Construction - Changes in Quantities of Pay Items, Including Elimination of Such Items: The quantities given in the original Contract for Contract pay items are only estimates of the quantities of those items that may be required for Project completion. (The quantities for given pay items in the original Contract are sometimes referred to herein as the "estimated quantities" or "original quantities.") A change in the original quantity of a Contract pay item (whether an increase or decrease of the quantity) shall be deemed to have occurred when the Engineer explicitly orders said change of quantity or when the change of quantity has been necessitated by a construction order or other written direction issued by the Engineer to the Contractor.

A Contract pay item shall be deemed a Major Item if the item's lump sum price in the original Contract, or its original quantity multiplied by its unit price in the original Contract, is equal to or greater than 10% of the original Contract's total bid price. All other Contract items shall be deemed Minor Items.

The provisions of 1.20-1.04.03 herein shall govern changes in compensation related to a "significant change" in Contract work, (as such changes are defined in 1.20-1.04.03) necessitated by a written order of the Engineer.

The provisions of 1.20-1.04.04 herein shall govern changes in compensation related to any differing site condition encountered by the Contractor that affects its performance of Contract work.

The provisions of 1.20-1.04.03 or 1.20-1.04.04 shall govern in any case in which they conflict with another provision of the Contract.

In any event, if the Engineer and the Contractor together determine that a particular change in compensation to the Contractor should be made due to a change in a Contract pay item quantity (including an item's complete elimination), they may make that change in compensation by a written agreement to do so.

Changes in Quantities to Minor Items:

(a) Quantity Increases of More Than 25% over Original Quantity: If the actual quantity of work authorized and accepted by the Engineer under a Contract pay item exceeds the item's original quantity by 25%, the Department will pay for the quantity in excess of 125% of the original quantity in one of the following three ways. (One-time fixed costs for which the Department has already reimbursed the Contractor in paying for 125% of the original quantity shall not be included in a calculation of the actual cost of the excess units.)

(1) Pay for the aggregate excess units on a cost-plus basis as provided in 1.20-1.09.04.
(2) Adjust the unit price by the increase or decrease in the unit price for the excess units, said difference to be calculated as of the time when work under the item was completed.

(3) Pay for the units in any other manner agreed on in writing by the Engineer and the Contractor.

If, however, the aggregate payment for the units in excess of 125% is less than $25,000 (using the original Contract unit price for the calculation) the Engineer will not adjust that unit price.

(b) Quantity Decreases of More Than 25% below Original Quantity: If the actual quantity of a Contract pay item authorized and accepted by the Engineer is less than 75% of the item's original quantity, the Engineer will not adjust the original Contract unit price for said item unless the Contractor makes a written request to the Engineer for such an adjustment and the Engineer grants it in writing. If the Engineer grants such a request, the Engineer will adjust the price for each accepted unit of said item performed or provided in one of the following three ways:

(1) Pay for the total item units actually performed or provided in the aggregate units on a cost-plus basis as provided in 1.20-1.09.04.

(2) Adjust the unit price by any increase in the unit price for the deficit units, which shall be the difference between the original Contract unit price and the actual unit cost (calculated on a cost-plus basis as provided in 1.20-1.09.04) of the total units performed or provided, said difference to be calculated as of the time when work under the item was completed.

(3) Pay for the item units performed or provided in any manner agreed on in writing by the Engineer and the Contractor.

In no instance however, shall the unit price paid for the number of units performed or provided, when their quantity has been decreased by more than 25% of the original quantity, be less than their original unit price; and in no instance shall the aggregate payment for such a decreased quantity of items be more than the Engineer would have paid for the performance or provision of 75% of the original quantity at the original unit price.

Regarding treatment of eliminated Contract items, refer to 1.20-1.09.05 herein.

1.20-1.04.03—Facilities Construction - Changes in Quantities and Significant Changes in the Character of Work:

(i) The Engineer reserves the right to make, in writing, at any time during the work, such changes in quantities and such alterations in the work as are necessary to satisfactorily complete the project. Such changes in quantities and alterations shall not invalidate the contract nor release the surety, and the Contractor agrees to perform the work as altered.

(ii) If the alterations or changes in quantities significantly change the character of the work under the Contract, whether such alterations or changes are in themselves significant changes to the character of the work or by affecting other work cause such other work to become significantly different in character, an adjustment, excluding loss of anticipated profits, will be made to the Contract. The basis for the adjustment shall be agreed upon prior to the performance of the work. If a basis cannot be agreed upon, then an adjustment will be made either for or against the Contractor in such amount as the Engineer may determine to be fair and equitable.

(iii) If the alterations or changes in quantities do not significantly change the character of the work to be performed under the Contract, the altered work will be paid for as provided elsewhere in the Contract.

(iv) The term "significant change" shall be construed to apply only to the following circumstances:

(A) When the character of the work as altered differs materially in kind or nature from that involved or included in the original proposed construction or

(B) When a Major Item of work, as defined elsewhere in the Contract, is increased in excess of 125% or decreased below 75% of the original Contract quantity. Any allowance for an increase in quantity shall apply only to that portion in excess of 125% of original Contract item quantity, or in case of a decrease below 75%, to the actual amount of work performed

1.20-1.04.04—Facilities Construction - Differing Site Conditions:

(i) During the progress of the work, if subsurface or latent physical conditions are encountered at the Site differing materially from those indicated in the Contract or if unknown physical conditions of an unusual nature, differing materially from those ordinarily encountered and generally recognized as inherent in the work provided for in the Contract, are encountered at the Site, the party discovering such conditions shall promptly notify the other party in writing of the specific differing conditions before the Site is disturbed and before the affected work is performed.

(ii) Upon written notification, the Engineer will investigate the conditions, and if it is determined that the conditions materially differ and cause an increase or decrease in the cost or time required for the

performance of any work under the Contract, an adjustment, excluding loss of anticipated profits, will be made and the Contract modified in writing accordingly. The Engineer will notify the Contractor of his/her determination whether or not an adjustment of the Contract is warranted.

(iii) No Contract adjustment which results in a benefit to the Contractor will be allowed unless the Contractor has provided the required written notice.

(iv) No Contract adjustment will be allowed under this clause for any effects caused on unchanged work.

1.20.1.04.05—Facilities Construction - Extra Work: Unforeseen work made necessary by the Engineer's changes of the Contract plans or specifications, or work that is necessary for completion of the Project, but for which no price is provided in the Contract, shall be done in accordance with the requirements of the specifications and as directed by the Engineer. The Engineer shall notify the Contractor of the necessity for such extra work, stipulating its character and extent, and shall notify the Contractor as to whether the Engineer wants the Contractor to propose a unit price or, lump sum price, or to perform the extra work on a cost-plus basis in accordance with 1.20-1.09.04. The Engineer need not solicit any price for the extra work from the Contract, but may, in any event, simply order the Contractor to perform the extra work on a cost-plus basis. If the Engineer does solicit from the Contractor a unit or lump sum price for the extra work, the Contractor must propose such a price in writing within 5 days of the Engineer's request for one.

The Contractor's price proposal shall be itemized and reasonably detailed, and shall include all known or anticipated direct and indirect costs of the work, including but not limited to, the costs of all safety and other equipment, small tools, labor, subcontractor quotes, consumables, field office overhead, home office overhead, insurance, bonding, and profit.

The character and extent of the extra work, together with the basis of compensation, shall be communicated to the Contractor by means of a construction order which, when signed by the Engineer, shall become a part of the Contract. If a Contractor objects to any portion of a construction order submitted to it, the Contractor must, within 15 days of its receipt of said order, return the order with a letter to the Department's Assistant District Engineer administering the Contract, describing specifically what portions of the order the Contractor finds objectionable, the nature of its objections, and the bases for its objections. If the Contractor does not do so, it shall be deemed to have accepted the terms of the construction order.

If the Engineer changes the scope of Contract work, the Contractor shall submit a proposed revised schedule and a cost revision proposal, which takes all such changes into account, if the Contractor believes that such revisions are warranted. If the schedule is to be revised, it will be revised in accordance with 1.20-1.08.08.

1.20-1.04.06—Facilities Construction - Removal and Disposal of Structures on the Work Site: All structures on the Project site which are not to remain on the Project site after completion of the Project shall be removed from said site and disposed of by the Contractor once it is no longer needed for the Project, and any such structure shall then become the property of the Contractor, except as otherwise required or provided by 1.20-1.10.07.

1.20-1.04.07—Facilities Construction - Rights in and Use of Materials Found on the Work Site: Upon written request of the Contractor and with the written approval of the Engineer, subject to limitations which may be set forth within such approval, any stone, gravel, sand, topsoil or any material from existing bridge substructures, buildings, or other structures, found within the limits of the Project may be excavated or removed and used by the Contractor on the Project, provided that said materials meet the requirements of the specification for such materials. Any materials excavated or removed shall not be taken off the Project site unless the Engineer in writing specifically authorizes such action. The following conditions shall govern these matters:

1. Excavation or removal of materials that would necessarily be excavated or removed in making the improvement will be paid for at the applicable Contract unit prices; and, in addition, the item for which this material is used will also be paid for at its Contract unit price. The Contractor will not be charged for such materials. The Contractor shall, without compensation, place in the embankment or elsewhere, as appropriate, sufficient suitable material to fill the space that the excavated materials would have occupied, unless otherwise directed by the Engineer.
2. The excavation or removal of materials that are not required to be excavated or removed in connection with the Contract work will not be paid for; and the Contractor will be charged for such materials at a negotiated unit price. The item for which this material is used will be paid for at its

Contract unit price. The Contractor shall, without compensation, backfill with accepted material the space that the excavated materials had occupied, to the satisfaction of the Engineer, unless otherwise directed by the Engineer.

Surplus material shall be removed from the Project only with the Engineer's written permission. The Engineer may determine that such material is not surplus, and may order that it be incorporated into the Project.

SECTION 1.20-1.05
CONTROL OF THE WORK FOR FACILITIES CONSTRUCTION

1.20-1.05.01—Facilities Construction - Authority of Engineer
1.20-1.05.02—Facilities Construction - Contractor Submittals
1.20-1.05.03—Facilities Construction - Conformity with Plans and Specifications (including Quality Control)
1.20-1.05.04—Facilities Construction - Coordination of Special Provisions, Plans, Supplemental Specifications and Standard Specifications and Other Contract Requirements
1.20-1.05.05—Facilities Construction - Cooperation by Contractor
1.20-1.05.06—Facilities Construction - Cooperation with Utilities (Including Railroads)
1.20-1.05.07—Facilities Construction - Coordination with Work by Other Parties
1.20-1.05.08—Facilities Construction - Schedules and Reports
1.20-1.05.09—Facilities Construction - Authority of Inspectors
1.20-1.05.10—Facilities Construction - Inspection
1.20-1.05.11—Facilities Construction - Removal of Defective or Unauthorized Work
1.20-1.05.12—Facilities Construction - Payrolls
1.20-1.05.13—Facilities Construction - Examining and Copying Contractor's Records
1.20-1.05.14—Facilities Construction - Termination for Convenience
1.20-1.05.15—Facilities Construction - Markings for Underground Facilities
1.20-1.05.16—Facilities Construction - Dimensions and Measurements
1.20-1.05.17—Facilities Construction - Welding
1.20-1.05.18—Facilities Construction - Structural Steel Fabrication Shop Prequalification
1.20-1.05.19—Facilities Construction - Field Erector Prequalification
1.20-1.05.23—Facilities Construction - Requests for Information (RFIs) and Requests for Change (RFCs)
1.20-1.05.24—Facilities Construction - Project Meetings

1.20-1.05.01—Facilities Construction - Authority of Engineer: All work shall be subject to the review of the Engineer. He shall decide all questions as to interpretation of the plans and specifications, and questions of mutual or respective rights of the Contractor and other Department contractors. The Engineer shall decide on an acceptable rate of progress, on the manner of performance, and on what shall be deemed acceptable fulfillment of the Contract. The Engineer shall have the right to determine the points at which the Contractor may begin work and the order in which the work shall be prosecuted in the best interests of the State within the intent of the terms in the Contract.

If a Project-related dispute arises between the Contractor and Department personnel assigned to the Project, and if those parties prove unable to resolve it, the Contractor may submit a detailed written description of the dispute to the Department's Assistant District Engineer administering the Contract.

It must be understood, though, that at no time may the Contractor, because of its disagreement with the Engineer, either disregard the orders of the Engineer or halt Project construction. If the Contractor cannot resolve a Project work or pricing dispute with the Engineer, the Contractor's proper remedy is a claim under CGS Section 4-61. A Contractor that disregards the orders of the Engineer with regard to the prosecution of Project work, or who refuses to continue Project work because of a disagreement with the Engineer, may be subject to termination of its Contract, to a subsequent finding that it is non-responsible as an apparent low bidder for a Department contract, to the assessment of liquidated damages, and to other adverse legal or administrative action by the Department.

1.20-1.05.02—Facilities Construction -Contractor Submittals:

1. **General:** Vacant
2. **Submittal Preparation and Processing:** Vacant
3. **Transmittal of Submittals**: Vacant
4. **Submittal Schedule:** At the Pre-Construction Meeting, the Contractor shall submit the initial submittal schedule. The initial submittal schedule will include all submittals required during the first 60 calendar days of construction, all submittals required to maintain orderly progress of the Work, and all submittal required early because of long lead time for manufacture or fabrication.

Following the Engineer's response to the initial submittal, the Contractor shall provide copies of the

schedule to the Engineer, Designer, the Contractor's subcontractors, and other parties required to comply with submittal dates indicated.

The Contractor shall submit the complete submittal schedule within 60 calendar days of the Notice to Proceed.

The Contractor shall update its submittal schedule once a month and distribute and post each updated schedule in the manner described above.

The submittal schedule shall be organized in numerical order by special provision number and by CSI-formatted specification section number. The Contractor shall include (1) time required for review, ordering, manufacturing, fabrication, and delivery when establishing dates; and (2) additional time required for making corrections or revisions to submittals noted by Designer or Engineer and additional time for handling and reviewing submittals required by those corrections. The Contractor shall coordinate submittal schedule with its subcontracts, the schedule of values, and their construction schedule.

5. Working Drawings (Delegated Design Submittals): When required by the Contract, or when ordered to do so by the Engineer, the Contractor shall prepare and submit working drawings, signed, sealed and dated by a qualified Professional Engineer licensed to practice in the State of Connecticut, for review.

There will be no direct payment for furnishing any working drawings, procedures or supporting calculations, but the cost thereof shall be considered as included in the general cost of the work.

a. Working Drawings for Permanent Construction: The Contractor shall submit drawings to the Designer on 22 inch × 34 inch sheets with a border and title block similar to the Department standard. Each drawing shall be a separate PDF file. Drawings shall be searchable. The first drawing shall include the Contractor's designer's Professional Engineer's digital signature, meeting the requirements of Adobe's Certified Document Services (CDS), and all other drawings shall include a watermark of the Professional Engineer's stamp in a common area of the border. Calculations, procedures and other supporting data may be submitted in an 8 1/2 inch × 11 inch format and shall be in a single PDF file. The first sheet of calculations shall include the Contractor's designer's Professional Engineer's digital signature, meeting the CDS requirements. Documents shall be named "Drawings," "Calculations," or "Supporting Documentation" as applicable.

   The Contractor's designer, who prepares the working drawings, shall secure and maintain at no direct cost to the State a Professional Liability Insurance Policy for errors and omissions in the minimum amount of $2,000,000 per error or omission. The Contractor's designer may elect to obtain a policy containing a maximum $250,000 deductible clause, but if the Contractor's designer should obtain a policy containing such a clause, they shall be liable to the extent of at least the deductible amount. The Contractor's designer shall obtain the appropriate and proper endorsement of its Professional Liability Policy to cover the indemnification clause in this Contract, as the same relates to negligent acts, errors or omissions in the Project work performed by them. The Contractor's designer shall continue this liability insurance coverage for a period of (1) 3 years from the date of acceptance of the work by the Engineer, as evidenced by a State of Connecticut, Department of Transportation form entitled "Certificate of Acceptance of Work," issued to the Contractor; or (2) 3 years after the termination of the Contract, whichever is earlier, subject to the continued commercial availability of such insurance. The Contractor shall supply to the Assistant District Engineer a certificate of insurance in accordance with 1.20-1.03.07 at the time that the working drawings for the Project are submitted.

b. Working Drawings for Temporary Construction: The Contractor shall submit drawings, calculations, procedures and other supporting data to the Assistant District Engineer in a format acceptable to the Assistant District Engineer.

c. Working Drawings for Permanent Construction: Drawings shall be submitted to the Designer on 22 inch × 34 inch sheets with a border and title block similar to the Department standard. Each drawing shall be a separate PDF file. Drawings shall be searchable. The first drawing shall include the Contractor's designer's Professional Engineer's digital signature, meeting the requirements of Adobe's Certified Document Services (CDS), and all other drawings shall include a watermark of the Professional Engineer's stamp in a common area of the border. Calculations, procedures and other supporting data may be submitted in an 8-1/2 inch × 11 inch format and shall be in a single PDF file. The first sheet of calculations shall include the Contractor's designer's Professional Engineer's digital signature, meeting the CDS requirements. Documents shall be named "Drawings," "Calculations," or "Supporting Documentation" as applicable.

6. Shop Drawings: When required by the Contract, or when ordered to do so by the Engineer, the Contractor shall prepare and submit shop drawings for review. Drawings shall be submitted on 22 inch × 34 inch sheets with an appropriate border and with a title block in the lower right-hand corner of each sheet. Each drawing shall be a separate PDF file. Drawings shall be searchable.

Shop Drawings consist of fabrication and installation drawings, roughing-in and setting drawings, schedules, patterns, templates and similar drawings, and wiring diagrams showing field-installed wiring, including power, signal, and control wiring. Standard information prepared without specific reference to the Project shall not be considered to be a Shop Drawing. Shop Drawings shall be project specific.

Shop drawings shall include the following information: Contract number, Project description, number and title of the drawing, date of drawing, revision number, name of Contractor and subcontractor submitting drawings, dimensions, identification of products, shop work manufacturing instructions, design calculations, statement of compliance with Contractual standards, notation of dimensions established by field measurement, notation of coordination requirements, relationship to adjoining construction clearly indicated, seal and signature of a professional engineer if specified, and any other information required by individual Contract provisions.

There will be no direct payment for furnishing any shop drawings, procedures or supporting calculations, but the cost thereof shall be considered as included in the general cost of the work.

7. Coordination Drawings: When required by the Contract, or when ordered to do so by the Engineer, the Contractor shall prepare and submit coordination drawings for review. Each drawing shall be a separate PDF file. Drawings shall be searchable.

The Contractor shall prepare coordination drawings according to requirements in other Contract provisions, and additionally where installation is not completely shown on Shop Drawings, where limited space availability necessitates coordination, or if coordination is required to facilitate integration of products and materials fabricated or installed by more than one entity.

Coordination Drawings shall include Project-specific information drawn accurately to a scale large enough to indicate and resolve conflicts. Coordination Drawings shall not be based on standard printed data. Coordination Drawings shall include the following information, as applicable: (1) use applicable plans as a basis for preparation of Coordination Drawings and prepare sections, elevations, and details as needed to describe relationship of various systems and components; (2) coordinate the addition of trade-specific information to the coordination drawings by multiple contractors in a sequence that best provides for coordination of the information and resolution of conflicts between installed components before submitting for review; (3) indicate functional and spatial relationships of components of architectural, structural, civil, mechanical, and electrical systems; (4) indicate space requirements for routine maintenance and for anticipated replacement of components during the life of the installation; (5) show location and size of access doors required for access to concealed dampers, valves, and other controls; (6) indicate required installation sequences; (7) indicate dimensions shown on the plans, specifically noting dimensions that appear to be in conflict with submitted equipment and minimum clearance requirements, and (8) provide alternate sketches to the Designer indicating proposed resolution of such conflicts.

The Contractor shall ensure the Coordination Drawings are signed by each installer, indicating their approval prior to submission.

There will be no direct payment for furnishing any coordination drawings, but the cost thereof shall be considered as included in the general cost of the work.

8. Product Data: When required by the Contract, or when ordered to do so by the Engineer, the Contractor shall prepare and submit product data for review in a PDF file.

The Contractor shall provide all product data in a single submittal for each element of construction or system and shall mark each submittal with the Contract item number.

The Contractor shall mark each copy of a product data submittal to show applicable choices and options. Where product data includes information on several products that are not required, copies shall be marked to indicate the applicable information. Product data shall include the following information and confirmations to the extent applicable: manufacturer's printed recommendations, compliance with recognized trade association standards, compliance with recognized testing agency standards, application of testing agency labels and seals, notation of coordination requirements, and any other information required by the individual Contract provisions.

There will be no direct payment for furnishing any product data, but the cost thereof shall be considered as included in the general cost of the work.

9. Product Samples: When required by the Contract, or when ordered to do so by the Engineer, the Contractor shall prepare and submit product samples for review. All samples shall be submitted to the Engineer for testing in accordance with the latest edition of the Materials Testing Manual's "Minimum Schedule for Acceptance Testing."

Product Samples are samples submitted for review and action by the Designer, which are: (1) physically identical to the proposed product or material cured and finished as required by the Contract; or (2) submitted for review of kind, color, pattern, thickness, and texture. Samples shall be used for a final check of these characteristics with other elements, and for a comparison of the characteristics of the approved sample with those of the actual component as delivered and installed.

The following information shall be submitted with product samples to the extent applicable: Contract number; Project description; generic description of the sample (name or trade reference, type or quality or grade, and any further designation necessary to identify the items or materials); sample source; product name; manufacturer's name; confirmation of availability; and anticipated delivery time.

In conjunction with the submission of physical product samples, a digital photograph of the sample shall be uploaded into ProjectWise.

The Designer will retain one set of the samples, transmit one set of same to the Engineer, and transmit any remaining sets of samples to the Contractor. The Engineer will retain the samples at the Project site for quality comparisons throughout the duration of the Project.

There will be no direct payment for furnishing any product samples, but the cost thereof shall be considered as included in the general cost of the work.

10. Quality Assurance Submittals: When required by the Contract, or when ordered to do so by the Engineer, the Contractor shall prepare and submit quality assurance submittals for review in a PDF file. Quality assurance submittals consist of qualification data, design data, certifications, manufacturer's instructions, manufacturer's field reports, test reports, Material Safety Data Sheets (MSDSs), and other quality assurance information required by individual Contract provisions.

Where Contract provisions require certification that a product, material, or installation complies with specified requirements, the Contractor shall submit a notarized certification from the manufacturer certifying said compliance. An officer of the manufacturer or other individual authorized to sign documents on behalf of the company shall sign the certification.

Where Contract provisions require the Contractor shall provide a certification letter on the manufacturer's letterhead to certify that asbestos is not contained in the materials. The manufacturer certification letter shall be formatted in the following manner:

[Addressed to:]	Commissioner of Transportation Department of Transportation P.O. Box 317546 Newington, Connecticut 06131-7546
Project Title and Number	
[We] hereby certify that all materials manufactured by [Insert Manufacturer Name] are asbestos-free.	
[Signature:]	[Name of authorized signatory] [Title]

Submittals associated with these materials will not be reviewed without the required manufacturer certification letter.

There will be no direct payment for furnishing any quality assurance submittals, but the cost thereof shall be considered as included in the general cost of the work.

11. Submittal Reviewer's Action: The Designer or Engineer will review each submittal, mark each with a uniform, self-explanatory action stamp, and return the stamped submittal promptly to the Contractor. The stamp will be marked as follows to indicate the action taken:

(a) If submittals are marked "No Exceptions Noted," the Designer or Engineer has not observed any statement or feature that appears to deviate from the Contract requirements. This disposition is contingent on being able to execute the manufacturer's written warranty in compliance with the Contract provisions.

(b) If submittals are marked "Exceptions as Noted," the considerations or changes noted by the Designer or Engineer are necessary in order for the submittal to comply with Contract requirements. This disposition

is contingent on being able to execute the manufacturer's written warranty in compliance with the Contract provisions.

(c) If submittals are marked "Revise and Resubmit," the Contractor shall revise and resubmit the submittal to address the deficiencies or provide additional information requested by the Designer or Engineer.

(d) If submittals are marked "Rejected," the Contractor shall prepare and submit a new submittal in accordance with the Designer's notations.

(e) If submittals are primarily for information or record purposes, the Designer will return the submittal marked "No Action Required." This disposition is contingent on being able to execute the manufacturer's written warranty in compliance with the Contract provisions.

Upon completion of the review, the submittal reviewer will notify the Contractor by e-mail that the submittal dispositions are available in ProjectWise.

The Contractor shall not proceed with the part of the Project covered by the submittal until the submittal is marked "No Exceptions Noted" or "Exceptions as Noted" by the Designer or the Engineer. The Contractor shall retain sole responsibility for compliance with all Contract requirements.

The Contractor shall print 2 color copies through ProjectWise of each submittal marked "No Exceptions Noted" or "Exceptions as Noted" to the Assistant District Engineer for use by the Engineer within 7 calendar days of the Contractor's receipt of the submittal reviewer's e-mail. The Contractor shall not perform physical work related to the submittal until the 2 color copies are provided to the Assistant District Engineer.

The Contractor shall mark up one set of Working Drawings (including any related calculations), Shop Drawings, and Coordination Drawings and retain them as a "Record Document."

Maintenance manuals and warranties will not be returned unless they are Rejected.

1.20-1.05.03—Facilities Construction - Conformity with Plans and Specifications (including Quality Control): The Contractor shall perform all work and provide all materials in conformity with the lines, grades, cross-sections, dimensions and material requirements, including tolerances, shown on the plans or indicated in the Contract specifications, or as directed by the Engineer.

The minimum quantity or quality level to be provided or performed is shown or specified in the Contract. The actual installation may comply exactly with the minimum quantity or quality specified or it may exceed the minimum within reasonable limits. Indicated numeric values are minimum or maximum, as appropriate for the context of the requirements. The Contractor shall refer uncertainties to the Engineer for a decision before proceeding.

If the Engineer believes that the materials or the finished product in which the materials were used are not in conformity with the plans and specifications, but believes nonetheless that the finished product is acceptable, he will then determine whether or not the work will be accepted and remain in place. If the Engineer believes that the work should be accepted, he will issue a construction order confirming his determination, and may provide therein for any equitable adjustment in the basis of payment which he deems appropriate.

If, in the opinion of the Engineer, any material provided by the Contractor, any finished product in which the materials were used, or any work performed does not conform to the plans and specifications and has resulted in an unacceptable product, the Contractor shall, at its own expense, either cure or remove and replace the unaccepted work and material, as the Engineer directs.

Quality Management Plan: The Contractor shall maintain and implement a written Quality Management Plan (QMP). The QMP shall document the overall internal quality control operating procedures for the Contractor to meet or exceed Contract requirements. The details of the WMP must discuss how the Contractor will ensure that:

- Work processes are performed efficiently and as documented
- Work processes out of conformance are quickly identified
- Corrective action is quickly taken to bring such work processes back into conformance

The QMP must include the following components:

- Identification of Contractor staff and their specific duties and responsibilities with regard to execution of the QMP
- Standard operating procedures and frequency of quality control inspection and testing used to measure quality before, during and after those procedures
- Action plan for reporting and reacting to nonconformance and quality control issues

The Contractor shall furnish a copy of the QMP to the Engineer prior to the start of the work. The Contractor must revise the QMP if, as determined by the Engineer, the Contractor's procedures prove to be inadequate or ineffective in producing work that meets the Contract requirements. Failure of the Contractor to comply with the provisions of this Article may result in a suspension of work in whole or in part. The Department will not grant the Contractor additional Contract time or compensation in connection with such a suspension.

1.20-1.05.04—Facilities Construction - Coordination of Special Provisions, Plans, Supplemental Specifications and Standard Specifications and Other Contract Requirements: All requirements indicated on the plans or in the Standard Specifications, the Supplemental Specifications, Special Provisions or other Contract provisions shall be equally binding on the Contractor, unless there is a conflict between or among any of those requirements. In the case of such a conflict, the order of governance among those requirements, in order of descending authority, shall be as follows:

1. Environmental Permit Special Conditions
2. Special Provisions
3. Plans other than Standard Sheets (enlarged details on plans, used to clarify construction, shall take precedence over smaller details of the same area; and information contained in schedules or tables, titled as such, shall take precedence over other data on plans)
4. Environmental Applications / Registrations / Certifications
5. Supplemental Specifications
6. Standard Specifications
7. Standard Sheets

Numerical designations of dimensions shall take precedence over dimensions calculated by applying a scale to graphic representations. Neither party to the Contract may take advantage of any obvious error or omission in the Contract. Should either party to the Contract discover such an error or omission, that party shall notify the other party of same immediately in writing. The Engineer will make such corrections and interpretations of the Contract as are necessary, in his judgment, to fulfill the purposes of the Contract that are evident from examining the Contract as a whole.

If the Contract includes an item that does not have a corresponding specification for either performance or payment purposes, the Contractor shall notify the Engineer of that fact in writing at least 2 weeks prior to ordering materials for or commencing work on the item. If the Department’s documents do not contain such a specification, the Engineer shall, if possible, derive an appropriate specification from applicable AASHTO Specifications or, if necessary, ASTM Specifications. If neither of those sources provides a suitable specification, the Contractor shall seek guidance from the Engineer with regard to the item, and the Engineer will formulate a reasonable specification for the item. When compliance with 2 or more standards is specified, and the standards may establish different or conflicting requirements for minimum quantities or quality levels, the Contractor shall refer such issues to the Engineer for a decision before proceeding with the pertinent work.

Industry Standards: Each entity engaged in construction of the Contract shall be familiar with industry standards applicable to that entity's construction activities. If printed standards have been established by organizations referenced in 1.20-1.01.02 or in the Contract, the Contractor shall obtain copies of said standards directly from the publication source.

Unless the Special Provisions include more stringent requirements, applicable construction industry standards have the same force and effect as if bound or copied directly into the Special Provisions to the extent referenced. Such standards are made a part of the Contract by reference.

The Contractor shall comply with the standard in effect as of the date of the advertisement for solicitation of bids, unless specifically directed otherwise in writing by the Engineer.

All references in the Contract to industry standards or codes refer to the last editions of same that were in effect at the date for the advertisement for solicitation of bids. Such references include current addenda and errata, if any, and shall be considered a part of the Contract.

1.20-1.05.05—Facilities Construction - Cooperation by Contractor: The Contractor will be supplied by the Department with copies of the plans.

The Contractor shall maintain in good order, in a secure, fire-resistant location at the Project site, 2 copies of all plans, Special Provisions (including CSI-formatted specifications within a particular Special Provision), Addenda, submittals, Construction Orders, and other modifications, schedules and instructions. Both sets shall be available to the Engineer at all times. The Contractor shall keep one set clean of all

markings. The Contractor shall mark one set of these documents to record all changes made during construction. The Contractor shall keep these documents current. The Contractor shall not permanently conceal any work until the required information has been recorded. The Engineer may withhold payments due the Contractor should they fail to keep these documents current.

Record Drawings: The Contractor shall maintain a complete set of Record Drawings by maintaining a clean, undamaged set of Contract drawings (original Contract plans as modified by Addenda and Construction Orders), Working Drawings (including any related calculations), Shop Drawings, and Coordination Drawings. The Contractor shall mark whichever drawings within the set that are most capable of showing conditions fully and accurately where the actual installation varies substantially from the Project work as originally shown. The Contractor shall include hyperlinks on the Contract plans to cross-reference to the related Working Drawings, Shop Drawings, Coordination Drawings, as well as RFIs and RFCs. The Contractor shall give particular attention to concealed elements that would be difficult to measure and record at a later date. The Contractor shall use separate colors to distinguish between variations in separate categories of the Project work.

Record Specifications: The Contractor shall maintain one complete copy of the Record Specifications, including related Addenda, construction orders and modifications issued during construction. The Contractor shall (1) mark these documents to show substantial variations in actual Project work performed in comparison with the text of the Specifications and modifications, (2) take care to show clearly on these documents any selected options and information on concealed construction that would be difficult to view at a later date, (3) note related record drawing information and Product Data.

Record Reports: The Contractor shall maintain one binder of all miscellaneous records such as manufacturer startup reports, test reports, and Building and Fire Code inspection reports required by other Contract Provisions (including CSI-formatted Specifications within a particular Special Provision). The miscellaneous records shall be arranged systematically according to the organization of the Contract provisions.

Record Survey: The Contractor shall submit a Record Survey in accordance with other Contract requirements.

No Asbestos Certification: The Contractor shall complete and sign a certification letter assuring the Department that no asbestos-containing materials have been used in the construction of the Contract. The Department will not issue the Certificate of Compliance without this completed and signed certification form. The Contractor certification letter shall be formatted in the following manner:

CONTRACTOR CERTIFICATION:
RE/ASBESTOS CONTENT OF MATERIALS

State of Connecticut
Department of Transportation
PO Box 317546
Newington, CT 06131-7546

1. Project Number: _____
2. Project Name: _____
3. Contractor Name:_____
4. This is to certify that I fully understand that it is the requirement of the Connecticut Department of Transportation that only materials that do not contain asbestos of any kind or amount are to be utilized in the construction of this Project.

   I therefore certify that, to the best of my knowledge, all materials installed under this Contract are asbestos-free.

   For the one-year warranty period after the issuance of the Certificate of Compliance, I agree to remove any asbestos-containing material identified by the Connecticut Department of Transportation and reinstall an approved, non-asbestos-containing material that is in compliance with the original Contract at no additional cost to the State.

5. Date of Certificate of Compliance: _____
6. Date of the Asbestos Certification: _____
7. Signature of Authorized Party Agreeing to the Terms & Conditions Identified Herein & as Further Stated in the Contract:

   _____ _____
   Signature Title

   _____ _____
   Printed Name Date

The Contractor shall give the Project constant attention to facilitate the progress thereof, shall cooperate with the Department, and shall promptly comply with all orders and directions of the Engineer.

Project Superintendent: The Contractor shall be represented on Site by a Project Superintendent. The Project Superintendent shall be on the Project Site whenever Project work is being performed. The Project Superintendent shall (1) attend all meetings between the Contractor and the Department, the Contractor and its subcontractors, and any other meetings that affect the progress of the Project; (2) be knowledgeable of the status of all parts of the work throughout the duration of the Project; (3) coordinate the activities of the subcontractors; (4) maintain the construction schedule; (5) be the Contractor's quality assurance/quality control representative; (6) prepare Daily Construction Reports in accordance with 1.20-1.05.08; (7) prepare or approve the Biweekly Schedules required to be submitted by the Contractor in 1.20-1.05.08; (8) have full authority to promptly execute and carry out the orders and directions of the Engineer within the terms of the Contract; and (9) to supply such materials, equipment, tools, labor and incidentals as may be required by the Contract or by the Engineer.

The Project Superintendent shall be an administrative employee of the Contractor or a Consultant hired by the Contractor to coordinate and expedite all phases of the work on a full-time daily basis, including associated project trades, on the Project Site.

The Contractor shall submit a written resume of the proposed Project Superintendent within 7 calendar days of the award of the Contract for the Department's acceptance. This resume shall demonstrate their superintendent work experience on a minimum of 2 projects of this type, scale, and complexity of this Project.

At any time during the Project, the Department may ask for any reason that the Project Superintendent be replaced. If the Department directs this action, the Contractor shall submit a written resume for a new Project Superintendent with the intent that this individual be in place on the Project within 30 calendar days of their acceptance by the Department. During this time, the duties of the Project Superintendent shall be

performed by the Project Coordinator. If there is no Project Coordinator on the Project, the Department may allow the original Project Superintendent to remain until the new Project Superintendent begins. The original Project Superintendent may still work on the Project in another capacity at the discretion of the Contractor.

Voluntary Partnering: The Department wants to establish a cohesive partnership with the Contractor and its principal subcontractors on the Project, so that the partnership can draw on the strengths of each organization in order to identify and pursue the partners' mutual Project goals. Chief among those will be the effective and efficient completion of the Project, within budget, on schedule, and in accordance with applicable plans, specifications, and other Contract provisions.

If the Contractor believes at any point before or during Project construction that the creation of formal partnering between itself and the Department, with the use of a third-party facilitator, would help the Contractor and the Department ("Partners") to reach these goals, the Contractor may submit a written request to the District Engineer of the District in which the Project will be constructed for the establishment of formal partnering between the Parties. If the Contractor makes such a request, the Department will engage in that partnering.

Any costs incurred by the Partners jointly in connection with Project partnering activities, to the extent that those costs are recognized as legitimate and appropriate by both Partners, will be shared equally between them. Any other costs incurred because of partnering activities will be borne by the Partner that incurred them.

If the Contractor and the Department decide to pursue a formal partnering initiative, they Contractor and The Department will arrange first to meet in order to select a third-party partnering facilitator and to plan a partnering development and team-building workshop. After they agree upon the services to be performed by the facilitator and the range of compensation for the facilitator that would be acceptable to them, the Contractor will contract accordingly for the services of said facilitator. The Department will reimburse the Contractor for fifty percent (50%) of the payments made under that contract, so long as the activities paid for were appropriate and within the contemplation of the Partners.

At the Partners' initial partnering meeting, the Partners will also determine who should attend the first partnering workshop, what the workshop's agenda will be, how long the workshop should last, and when and where it will be held. Unless the Partners agree otherwise, attendance at the first partnering workshop will be mandatory for the Department's District Engineer for the Project and the Department's other key Project personnel, the Contractor's on-Site Project manager and other key supervisory Project personnel, and, if the Contractor agrees to it, the key supervisory personnel of the Contractor's principal Project subcontractors. The Partners will also request that the Project design engineers and key local government personnel send Regional/District and Corporate/State-level managers to the workshop and direct them to participate in Project partnering activities as and when requested to do so by the Partners.

With the agreement of the Partners, follow-up Project partnering workshops will be held periodically until the Department closes out the Contract.

If the Partners agree on a formal partnering charter for the Project, the establishment of that charter will not change the legal relationship of the Partners to the Contract; it will not alter, supplement, or eliminate any of the Partners' rights or obligations under the Contract.

1.20-1.05.06—Facilities Construction - Cooperation with Utilities (Including Railroads): The Engineer may anticipate that a Project construction activity will require the removal, repair, replacement or relocation of a utility appurtenance. In such an instance, the Engineer, in advance of the commencement of such activity, will notify the affected utilities, either directly or through the local government, of the anticipated nature and timing of said activity. The Engineer will endeavor to have all necessary adjustments of public or private utility fixtures, pipelines, and other appurtenances within or adjacent to the limits of Project construction made as soon as practicable, when such changes are required by the State or local government.

Whenever the Engineer determines that the relocation or adjustment of poles or the overhead plant of public or private utilities or railroad facilities is dependent upon the completion of certain required Contract activities, the Contractor shall complete those activities within a reasonable length of time.

Temporary and permanent changes required by the State or local government in water lines, gas lines, sewer lines, wire lines, service connections, water or gas meter boxes, water or gas valve boxes, light standards, cableways, signals and all other utility (including railroad) appurtenances within the Site of the proposed Project construction are to be made by others at no expense to the Contractor, except as otherwise

provided for in the Special Provisions or as noted on the plans.

When the Contractor is required by the Engineer to relocate utility appurtenances, such work will be paid for as extra work unless specific bid items for such work appear in the Contract.

If the Contractor, for its convenience or for any other reason, desires a change in the location of a water line, gas line, sewer line, wire line, service connection, water or gas meter box, valve box, light standard, cableway, signal or any other utility (including railroad) appurtenances, the Contractor shall satisfy the Department that the proposed relocation will not interfere with the Contractor's or other contractors' Project operations or their fulfillment of the requirements of the plans, and that said change will not create an obstruction or hazard to traffic. If the requested change of location is acceptable to the Engineer, the Contractor shall make its own request for such relocation work to the utility companies, pipe owners or other parties likely to be affected by said work. Such relocation work shall be done at the Contractor's sole expense.

The Contractor shall schedule its operations in such a manner as to minimize interference with the operations of the utility companies or local governments in effecting the installation of new facilities, as shown on the plans, or the relocation of their existing facilities. The Contractor shall consider in its bid all permanent and temporary utility appurtenances in their present or relocated positions and any installation of new facilities required for the Project. The Department will not make any additional compensation to the Contractor for delays, inconvenience or damage sustained by the Contractor due to

(i) interference with Project construction caused by the location, condition or operation of utility (including railroad) appurtenances or

(ii) the installation, removal, or relocation of such appurtenances; and the Contractor may not make a claim for any such compensation.

1.20-1.05.07—Facilities Construction - Coordination with Work by Other Parties: The Contractor shall make every effort to perform its work so as not to interfere with other work for the State or other parties. In the case of a dispute with another contractor working for the Department regarding their work for the State, or in the case of a conflict between their planned operations or the needs of their projects, the Contractor shall bring that dispute or conflict to the Engineer's attention and the Engineer shall decide how it shall be resolved. The Engineer's decision shall be binding upon all of the contractors working for the Department who are involved in the matter.

The Contractor shall, as far as possible, schedule and otherwise plan and arrange its work, and place and dispose of its Project materials, so as not to interfere with the operations of other contractors working for the State. The Contractor shall, as necessary to accomplish this goal, endeavor to coordinate and schedule its work in the way which will interfere least with the work of other parties.

If the Contractor's work or activities under the Contract come into conflict with other activities or work for the State, any financial or other liability arising from such conflicts shall be the Contractor's; and the Contractor shall protect and save harmless the State from any and all damages or claims, and the costs of defending same, which may arise because of inconvenience, delay, financial hardship, or injuries caused to the Contractor or to other contractors as a result of such conflicts, unless:

(a) The Contractor notifies the Engineer of such conflicts as soon as the likelihood of such a conflict becomes apparent; or, if such likelihood could not have been foreseen earlier, then as soon as the conflict becomes apparent.

(b) The Contractor waits for direction from the Engineer as to how the conflict should be avoided or resolved, and the Contractor does not proceed with the work involved in the conflict until the Engineer has provided the Contractor with such direction.

(c) The Contractor follows the directions given by the Engineer for avoiding, resolving, or minimizing the conflict.

The Contractor shall be responsible for the completion of its Contract work, regardless of any interference with, or delay of, that work which may be caused by the presence or activities of other contractors working for the State.

The Engineer and the Owner will occupy the Project Site during the entire construction period. The Engineer and the Owner reserve the right to install equipment prior to Semi-Final Inspection and the issuance of the Certificate of Compliance provided that such installation does not interfere with the Contractor’s completion of their Work. The Owner and any PURA regulated utility installers reserve the right to perform work in the Communications Room, including the Owner changing locks on the doors, on

or about the time the above-ceiling inspection is performed by the Engineer. Such installations shall not constitute acceptance of the total Project.

1.20-1.05.08—Facilities Construction - Schedules and Reports: When a project coordinator is not required by the Contract the following shall apply:

Baseline Bar Chart Construction Schedule: Within 20 calendar days after contract award the Contractor shall develop a comprehensive bar chart as a baseline schedule for the project. The bar chart schedule shall be submitted to the Engineer for approval and shall be based on the following guidelines:

1. The bar chart schedule shall contain a list of activities that represents the major activities of the project. At a minimum, this list should include a breakdown by individual structure or stage, including major components of each. The bar chart schedule shall contain sufficient detail to describe the progression of the work in a comprehensive manner. As a guide, 10 to 15 bar chart activities should be provided for each $1 million of contract value.

The following list is provided as an example only and is not meant to be all-inclusive or all-applicable:

General Activities Applicable to all projects

- Project Constraints
  - -Winter shutdowns
  - -Environmental permits/application time of year restrictions
  - -Milestones
  - -Third Party approvals
  - -Long lead time items (procurement and fabrication of major elements)
  - -Adjacent Projects or work by others
- Award
- Notice to Proceed
- Signing (Construction, temporary, permanent by location)
- Mobilization
- Permits as required
- Field Office
- Utility Relocations
- Submittals/shop drawings/working drawings/product data
- Construction of Waste Stock pile area
- Clearing and Grubbing
- Earthwork (Borrow, earth ex, rock ex etc.)
- Traffic control items (including illumination and signalization)
- Pavement markings
- Roadway Construction (Breakdown into components)
- Drainage (Breakdown into components)
- Culverts
- Plantings (including turf establishment)
- Semi-final inspection
- Final Cleanup

As required the following may supplement the activities listed above for the specific project types indicated:

a. For bridges and other structures, include major components such as abutments, wingwalls, piers, decks and retaining walls; further breakdown by footings, wall sections, parapets etc.
   - Temporary Earth Retention Systems
   - Cofferdam and Dewatering
   - Structure Excavation
   - Piles/test piles
   - Temporary Structures
   - Removal of Superstructure
   - Bearing Pads
   - Structural Steel (Breakdown by fabrication, delivery, installation, painting etc.)
   - Bridge Deck

b. Multiple location projects such as traffic signal, incident management, lighting, planting and guiderail projects will be broken down first by location and then by operation. Other major activities of these types of projects should include, but are not limited to:
   Installation of anchors
   Driving posts
   Foundations
   Trenching and Backfilling
   Installation of Span poles/mast arms
   Installation of luminaries
   Installation of cameras
   Installation of VMS
   Hanging signal heads
   Sawcut loops
   Energizing equipment
c. Facility Projects – Facilities construction shall reflect the same breakdown of the project as the schedule of values:
   Division 2 – Existing Conditions
   Division 3 – Concrete
   Division 4 – Masonry
   Division 5 – Metals
   Division 6 – Wood, Plastic, and Composites
   Division 7 – Thermal and Moisture Protection
   Division 8 – Openings
   Division 9 – Finishes
   Division 10 – Specialties
   Division 11 – Equipment
   Division 12 - Furnishings
   Division 13 – Special Construction
   Division 14 – Conveying Equipment
   Division 21 – Fire Suppression
   Division 22 – Plumbing
   Division 23 – Heating, Ventilating, and Air Conditioning
   Division 26 – Electrical
   Division 27 – Communications
   Division 28 – Electronic Safety and Security
   Division 30 – Site Work
   Division 31 – Earthwork
   Division 32 – Exterior Improvements
   Division 33 - Utilities

2. If the Engineer determines that additional detail is necessary, the Contractor shall provide it.

3. Each activity shall have a separate schedule bar. The schedule timeline shall be broken into weekly time periods with a vertical line to identify the first working day of each week.

4. The bar chart schedule shall show relationships among activities. The critical path for the Project shall be clearly defined on the schedule. The schedule shall show milestones for major elements of work, and shall be prepared on a sheet, or series of sheets of sufficient width to show data for the entire construction period**.**

5. If scheduling software is used to create the bar chart schedule, related reports such as a predecessor and successor report, a sort by total float, and a sort by early start shall also be submitted.

6. Project activities shall be scheduled to demonstrate that the construction completion date for the Project will occur prior to expiration of the Contract time. In addition, the schedule shall demonstrate conformance with any other dates stipulated in the Contract.

7. The Contractor is responsible to inform its subcontractor(s) and supplier(s) of the Project schedule and any relevant updates.

8. There will be no direct payment for furnishing schedules, the cost thereof shall be considered as included in the general cost of the work.

Monthly Updates: No later than the 10th day of each month, unless directed otherwise by the Engineer, the Contractor shall deliver to the Engineer three (3) copies of the schedule to show the work actually accomplished during the preceding month, the actual time spent on each activity, and the estimated time needed to complete any activity which has been started but not completed. Each time bar shall indicate, in 10% increments, the estimated percentage of that activity which remains to be completed. As the Project progresses, the Contractor shall place a contrasting mark in each bar to indicate the actual percentage of the activity that has been completed.

The monthly update shall include revisions of the schedule necessitated by revisions to the Project directed by the Engineer (including, but not limited to extra work), during the month preceding the update. Similarly, any changes of the schedule required due to changes in the Contractor's planning or progress shall also be included. The Engineer reserves the right to reject any such revisions. If the schedule revisions extend the contract completion date, due to extra or added work or delays beyond the control of the Contractor, the Contractor shall submit a request in writing for an extension of time in accordance with 1.20-1.08.08. This request shall be supported by an analysis of the schedules submitted previously.

Any schedule revisions shall be identified and explained in a cover letter accompanying the monthly update. The letter shall also describe in general terms the progress of the Project since the last schedule update and shall identify any items of special interest.

If the Contractor fails to provide monthly schedule updates, the Engineer has the right to hold 10% of the monthly estimated payment, or $5,000, whichever is less, until such time as an update has been provided in accordance with this provision.

Biweekly Schedules: Each week, the Contractor shall submit to the Engineer a two week look-ahead schedule. This short-term schedule may be handwritten but shall clearly indicate all work planned for the following two week period.

Recovery Schedules: If the updated schedule indicates that the Project has fallen behind schedule, the Contractor shall either submit a time extension request in accordance with 1.20-1.08.08 or immediately institute steps acceptable to the Engineer to improve its progress of the Project. In such a case, the Contractor shall submit a recovery plan, as may be deemed necessary by the Engineer, to demonstrate the manner in which an acceptable rate of progress will be regained.

Daily Construction Reports: The Project Superintendent shall assist the Engineer in the preparation of a daily construction report, by ensuring that each of the Contractor's employees and subcontractors working on the Project Site on a given day signs the Engineer's sign-in sheet for that day; and by keeping and providing to the Engineer its own daily list of employees and subcontractors who worked on the Project Site on that day.

1.20-1.05.09—Facilities Construction - Authority of Inspectors: Inspectors employed by the Department are authorized to inspect all work done and all materials furnished for Project construction. Such inspection may extend to any part of the Project work and to the preparation or manufacture of the materials to be used for same. In case of any dispute arising between the Contractor and the inspector as to materials furnished or the manner of performing work, the inspector has the authority to reject material or stop the work until the question at issue can be referred to and decided by the Engineer. The inspector is not authorized to revoke, alter, enlarge, relax, or release any requirements of the Contract, nor to approve or accept any portion of the Contract work, nor to issue instructions contrary to the Contract. The inspector shall in no case act as a foreman, or fulfill other duties for the Contractor. Any advice that the inspector may give to the Contractor shall not be construed as binding the Department in any way, nor as releasing the Contractor from its obligation to fulfill the terms of the Contract.

1.20-1.05.10—Facilities Construction - Inspection: All materials and each part or detail of the Project work shall be subject at all times to inspection by the Engineer. Such inspection may take place on the Site or at an offsite location such as a mill, subcontractor fabrication plant or shop, or other type of location. The Engineer shall be allowed access to all parts of the work and shall be furnished with such information and assistance by the Contractor as the Engineer deems necessary to make complete, detailed, and timely inspections. Information collected by the inspector may include written observations, sketches, photographs and other data as deemed appropriate by the inspector. The Contractor shall allow the Engineer to collect such information without restriction on the Site and shall ensure that the Engineer will have the same unrestricted ability to gather such pertinent information regarding Project work or materials at a location controlled by a subcontractor or supplier of the Contractor.

The Contractor shall always notify the Engineer of its intention to perform work on the Project site,

including the nature of the particular work it intends to perform, at least 48 hours before the Contractor commences that work. If, after receiving such notice, the Engineer decides that more than 48 hours is needed to arrange for and conduct inspection related to that work, the Engineer will so notify the Contractor, and the Contractor shall refrain from commencing the work until the Engineer has arranged for such inspection. The Contractor may not commence any portion of its work without prior related inspection by the Engineer unless the Engineer agrees otherwise. In the absence of such advance agreement by the Engineer, any work done or material used without inspection by a Department representative may be ordered exposed for examination and testing, and then corrected or restored, all at the Contractor's expense.

The Contractor shall provide the Engineer the name(s), contact information, and location(s) of any subcontractor(s) fabricating materials or components outside the Project limits for permanent incorporation into the Project. The Contractor shall provide such information sufficiently in advance of such fabrication to allow the Engineer to schedule inspections of said fabrication, and the Contractor shall ensure that such work does not commence until it has confirmed that the Engineer has arranged for adequate inspection at the offsite location(s). Any such work done without inspection by a Department representative may be ordered exposed for examination and testing. If the Engineer then judges that the work requires correction or restoration, the Contractor shall perform such remedial work at its own expense.

If, at any time before the Department's acceptance of the Project, the Engineer requests the Contractor to remove or uncover any portion of the Project work for inspection by the Engineer, the Contractor shall do so. After such inspection is completed, the Contractor shall restore such portions of the work to the condition required by the Contract as construed by the Engineer. If the work or material exposed and inspected under this provision proves acceptable to the Engineer, the Department shall pay the Contractor for any removal, uncovering or restoration of its previous Contract work. The Department shall pay the Contractor for such removal, uncovering, and restoration of the prior work as extra work. If the work or material exposed and inspected proves, in the opinion of the Engineer, not to conform with Contract requirements, the Contractor shall be responsible for the costs of the removal, uncovering, correction and restoration of the work and material in accordance with the Contract or as the Engineer requires.

The fact that the Engineer may have conducted or failed to conduct, or conducted insufficiently or inaccurately, any inspection of Project work will not relieve the Contractor of its responsibility to perform the Project work properly, to monitor its work and the work of its subcontractors, and to institute and maintain quality control procedures appropriate for the proper execution of Project work.

1.20-1.05.11—Facilities Construction - Removal of Defective or Unauthorized Work: Work that does not conform to the requirements of the Contract shall be remedied in a manner acceptable to the Engineer or removed and replaced at the Contractor's expense in a manner acceptable to the Engineer.

No work shall be done without appropriate lines and grades having been established in the field. Work done contrary to the instructions of the Engineer, work done beyond the lines shown on the plans, or extra work done without the Engineer's prior written direction to perform it will be considered as unauthorized and the Department will not pay for it. Work so done may be ordered removed or replaced at the Contractor's expense.

If the Contractor fails to comply with any order of the Engineer made under the provisions of this Article, the Engineer has the authority to cause unacceptable or unauthorized work to be remedied or removed and replaced by a party or parties other than the Contractor, and to deduct the costs of such activities from any monies due or to become due to the Contractor from the Department or any other agency of the State.

The Contractor shall remove all materials that have a probability of containing asbestos if they failed to provide the manufacturer certification letter required by 1.20-1.05.02 unless a manufacturer certification letter is provided and is acceptable to the Designer. The Contractor shall remove all asbestos containing material that is found to have been installed. The Contractor shall replace the removed material with appropriate material. The Contractor shall pay for any asbestos testing charges the Department incurred in order to prove that the material contains asbestos fibers. This obligation will extend throughout the one year warranty period after the issuance of the Certificate of Compliance.

1.20-1.05.12—Facilities Construction - Payrolls: For each week of the Project from the first week during which an employee of the Contractor does Project work to which prevailing wage requirements apply, until the last week on which such an employee does such work, the Contractor shall furnish to the Engineer certified copies of payrolls showing

(a) the names of the employees who worked on the Project and whose work is subject to prevailing wage requirements,
(b) the specific days and hours and numbers of hours that each such employee worked on the Project, and
(c) the amount of money paid to each such employee for Project work.

Each such payroll shall include the statement(s) of compliance with prevailing wage laws required by the State of Connecticut and, if applicable, by the Federal government. Said payrolls must contain all information required by CGS Section 31-53 (as it may be revised). For contracts subject to Federal prevailing wage requirements, each payroll shall also contain the information required by the Davis Bacon and Related Acts (DBR). All of the payroll requirements in this Article shall also apply to the work of any subcontractor or other party that performs work on the Project site, and the Contractor shall be responsible for ensuring that each such party meets said requirements.

Every Contractor or subcontractor performing Project work is required to post the relevant prevailing wage rates as determined by the State Labor Commissioner and, on federal aid projects, those determined by the United States Secretary of Labor. The wage rate determinations shall be posted in prominent and easily accessible places at the work site.

1.20-1.05.13—Facilities Construction - Examining and Copying Contractor's Records: The Contractor shall permit the Department and its duly-authorized representatives to examine and copy all documents and other records of the Contractor that are relevant to charges for extra work, alleged breaches of Contract, or any formal or informal claim for additional compensation or for damages in connection with the Project.

With the exception noted below, the Contractor shall also permit the Department to examine and copy such of its documents and other records pertaining to the Project as the Department may deem necessary in order to determine whether or not the Contractor has complied with all laws, regulations and other governmental mandates, e.g., those relating to labor compliance, affirmative action programs, and equal employment opportunity. Documents and other records relating to the Project, if they were created prior to the opening of bids for the Contract, and if they are sought by the Department only for the purpose of confirming such compliance with legal requirements, shall, however, not be subject to examination by the Department pursuant to this Article without the consent of the Contractor.

The Contractor further agrees that it shall keep all documents and other records relating to the Project at least until the expiration of 3 years after the date of acceptance of the Project by the Department, as designated in a "Certificate of Acceptance of Work and Acceptance of Project" issued by the Department. If any claims are brought by the Department or the Contractor prior to that expiration, however, the Contractor shall keep all such records until the Department has given the Contractor a full and final release from all pending and potential claims regarding the Project. If the Contractor does not so keep any such records, it may not assert any formal or informal claim for compensation or damages that could have been substantiated or disproven with such records.

The Contractor shall ensure that the requirements of this provision are made applicable to its subcontractors and suppliers, for the State's benefit, by including the operative language of this Article in its Project subcontracts and purchase agreements.

1.20-1.05.14—Facilities Construction - Termination for Convenience: The State may terminate the Contract whenever the Engineer determines that such termination is in the best interests of the State. Any such termination shall be effected by delivery to the Contractor of a written Notice of Termination specifying the extent to which performance of work under the Contract is terminated and the date upon which said termination shall be effective.

In the case of such a termination, the Department will pay the Contractor at the Contract unit prices for the actual number of units or items of Contract work completed prior to the effective date of termination, or as may be agreed by the parties for such items of work partially completed. No claim for loss of overhead or anticipated profits shall be allowed.

When the volume of work completed is too small to compensate the Contractor under Contract unit prices for its related expenses, the Department may consider reimbursing the Contractor for such expenses.

Materials obtained by the Contractor for the Project, if they have been inspected, tested as required, and accepted by the Engineer, but have not been incorporated into the Project construction, shall, if the Engineer and the Contractor so agree, be purchased by the Department from the Contractor at their actual cost as shown by receipted bills. To this cost shall be added all actual costs for delivery at such points of

delivery as may be designated by the Engineer, as shown by actual cost records. If the Engineer does not agree to purchase such materials, the Department shall reimburse the Contractor for any reasonable restocking fees and handling costs incurred by the Contractor in returning said materials to the vendor.

Termination of the Contract shall not relieve the Contractor of its responsibilities for the completed Project, nor shall it relieve the Contractor's surety of its obligation concerning any claims arising out of the work performed, until the requirements of 1.20-1.08.13 and 1.20-1.08.14 have been met.

1.20-1.05.15—Facilities Construction - Markings for Underground Facilities: In conformance with Section 16-345 through 16-359 of the Regulations of the PURA, the Contractor is responsible for notifying "Call Before You Dig" prior to commencing any excavation, including milling, reclamation or trenching; and the Contractor shall install a warning tape located a minimum of 12 inches above all conduits, wires, cables, utility pipes, drainage pipes, underdrains, or other facility, unless the excavation's depth, other underground facilities, or other engineering considerations make this minimum separation unfeasible. The warning tape shall be of durable impervious material, designed to withstand extended underground exposure without material deterioration or fading of color. The tape shall be of the color assigned to the type of facility for surface markings and shall be durably imprinted with an appropriate warning message. The tape shall also comply with the specific requirements of the utility that owns the facility.

All tapes, unless otherwise directed by the specific utility, shall be detectable to a depth of at least 3 feet with a commercial radio-type metal locater.

Assigned colors are:

Green—Storm and sanitary sewers and drainage systems, including force mains and other non-hazardous materials

Blue—Water

Orange—Communication lines or cables, including, but not limited to, those used in, or in connection with, telephone, telegraph, fire signals, cable television, civil defense, data systems, electronic controls and other instrumentation

Red—Electrical power lines, electrical power conduits and other electrical power facilities, traffic signals and appurtenances and illumination facilities

Yellow—Gas, oil petroleum products, steam, compressed air, compressed gases and all other hazardous material except water

Brown—Other

Purple—Radioactive materials

Payment for warning tapes shall be included in the bid price for the pay item of the specific facility for which the tape is used.

1.20-1.05.16—Facilities Construction - Dimensions and Measurements: The Contractor or one of its subcontractors shall verify each dimension that is needed in order to ensure that its work complies with the Contract, and must do so before ordering any material or doing any work for which such dimension is needed. Such dimensions include, but are not limited to, dimensions given on the plans, as well as dimensions of structures in place prior to Project construction or installed in the course of construction. The Contractor or any subcontractor that finds a discrepancy or error in dimensions must report it promptly to the Engineer and may proceed with affected work only after receiving clarification and direction from the Engineer regarding the matter. Any costs for delays, changes, cutting or repairs that are incurred due to the Contractor's failure to observe the above requirements shall be borne by the Contractor.

1.20-1.05.17—Facilities Construction - Welding: The Contractor shall ensure that all welding of materials permanently incorporated into the work, and welding of materials used temporarily during construction of the work is performed in accordance with the following codes:

- AWS Structural Welding Code – Steel – ANSI/AWS D1.1: Miscellaneous steel items that are statically loaded including but not limited to columns, and floor beams in buildings, railings, sign supports, cofferdams, tubular items, and modifications to existing statically loaded structures.
- AWS Structural Welding Code – Aluminum – AWS D1.2/D1.2M: Any aluminum structure or member including but not limited to brackets, light standards, and poles.
- AWS Structural Welding Code – Sheet Steel – AWS D1.3/D1.3M: Sheet steel and cold-formed members 0.18 inch or less in thickness used as, but not limited, to decking and stay-in-place forms.

- AWS Structural Welding Code – Reinforcing Steel – AWS D1.4/D1.4M: Steel material used in the reinforcement of cast-in-place or pre-cast Portland cement concrete elements including but not limited to bridge decks, catch basin components, walls, beams, deck units, and girders.
- AASHTO/AWS – Bridge Welding Code, AASHTO/AWS D1.5/D1.5M: Steel highway bridges and other dynamically loaded steel structures. Also includes sign supports, and any other fracture critical structure.

The edition governing the work shall be in effect on the date the Contract was advertised for solicitation of bids.

The Contractor is responsible to provide a Certified Welding Inspector and Non Destructive Examination personnel in accordance with the above noted codes. The cost for this service is included in the general cost of the work.

Prior to performing any shop welding on Department projects, all shop welders shall be qualified to weld in accordance with the welding code(s) specified on the approved drawings or otherwise stated in the Contract.

Prior to performing field welding on any permanent or temporary structure or component for Department projects, all field welders, field welding operators, and field tackers must possess a valid Welder Certification card issued by the Department's Division of Materials Testing (DMT), as outlined in the Department's Materials Testing Manual, and must be up to date in the Department's online Welder Database. If a welder has not been engaged in welding operations on a Department project or a project acceptable to the Department over the past 6 months, or does not posses an approved welding certificate dated within the past 12 months from a welding agency acceptable to the Engineer, the field welder shall be required to requalify through examination. The Engineer may require requalification of anyone whose quality of work is in question.

Prior to start of fabrication or field welding, all welding procedures shall be submitted to the Engineer and the DMT for review. Reviewed welding procedures shall be available at all times while welding is being performed.

1.20-1.05.18—Facilities Construction - Structural Steel Fabrication Shop Prequalification: Material fabricated for Department projects, other than Facilities Construction, are required to be produced in a facility with the AISC Quality Management System (QMS) Certification category noted on the plans as Certified Bridge Fabricator – Simple (SBR), Certified Bridge Fabricator- Intermediate (IBR), Certified Bridge Fabricator- Advanced (ABR), or Certified Metal Component Manufacturer (CPT).

SBR or CPT certification is required as a minimum for fabrication of secondary, non-primary load carrying elements such as but not limited to diaphragms, cross frames, lateral bracing in straight or skewed bridges less than 30 degrees, camera light supports and structures, sign and signal light support structures, bridge railings, stairs, walkways, grid decks, drains, scuppers, expansion joints, bearings, ballast plates and mechanical movable bridge equipment.

SBR certification is required as a minimum for fabrication of unspliced rolled beams.

IBR certification is required as a minimum for all other bridges other than unspliced rolled beam bridges.

FC Fracture Critical Endorsement is required when noted on the plans for fabrication of fracture critical members.

Material fabricated for Facilities Construction is required to be produced in a facility with either a Certified Building Fabricator (BU) certification by AISC, or an AC172 accreditation by the International Accreditation Service (IAS).

1.20-1.05.19—Facilities Construction - Field Erector Prequalification: Contractors erecting structural steel for Facilities Construction are required to possess an AISC Certified Steel Erector - Advanced (CSEA) CSE certification for Steel-Framed Buildings when erecting steel.

1.20-1.05.23—Facilities Construction -Requests for Information (RFIs) and Requests for Change (RFCs): The Contractor shall forward all RFIs and RFCs to the Engineer by in PDF format for review. The Engineer will forward the RFI or RFC to the Designer for review. Upon receipt of an RFI or RFC, the Designer will attempt to determine if additional information is required from the Contractor to respond to the RFI or RFC, and request said information from the Engineer.

All RFIs will be responded to within 10 calendar days of receipt by the Designer. All RFCs will be responded to within 21 calendar days of receipt by the Designer.

1.20-1.05.24—Facilities Construction -Project Meetings: In order to maximize effective use of time, and to minimize disruption during construction, the Contractor shall work closely with the Engineer to combine required meetings when possible.

1. Pre-Construction Meetings: The Engineer will schedule a pre-construction and organizational meeting at the District Office or other convenient location after the Award of the Contract. At such meeting, the Engineer will review the parties' responsibilities and personnel assignments.

The Engineer, Designer, Owner, the Contractor and its project coordinator, superintendent, major subcontractors, and other concerned parties shall attend the meeting. All participants at the meeting shall be familiar with the Project and authorized to conclude matters relating to the Project.

The Engineer will distribute copies of minutes of the Pre-Construction Meeting to all attendees. The Contractor shall distribute copies to other parties who were not present at the meeting.

2. Pre-Installation Meetings: The Engineer, Designer, the Contractor's project coordinator, superintendent, the Installer, technical and field service engineering representatives of each manufacturer and fabricator involved in or affected by the installation, and other representatives required for coordination or integration of Project work or materials shall attend the scheduled Pre-Installation Meeting. All meeting participants shall be familiar with the Project and authorized to conclude matters relating to the Project.

The meeting participants shall review progress of other construction activities and preparations for the particular activity under consideration, including requirements of Contract documents, related requests for interpretations, related construction orders, purchases, deliveries, submittals, review of mockups, possible conflicts, compatibility problems, time schedules, weather limitations, manufacturer's written recommendations, warranty requirements, compatibility of materials, acceptability of substrates, temporary facilities and controls, space and access limitations, regulations of authorities having jurisdiction, testing and inspecting requirements, installation procedures coordination with other work, required performance results, protection of adjacent work, and protection of construction and personnel.

The Engineer will distribute copies of minutes of the meeting to the Designer and the Contractor. The Contractor shall distribute copies to parties who were or should have been at the meeting.

3. Progress Meetings: The Engineer will conduct progress meetings at the Project site at regularly scheduled intervals, but no less than twice a month.

The Contractor shall provide the Engineer with a detailed agenda for the proposed meeting, specifying what topics will be covered. In addition to representatives of the Engineer, the Contractor's project coordinator and superintendent, each subcontractor, supplier or other entity concerned with current progress or involved in planning, coordination or performance of future activities shall attend these meetings. All participants at the meeting shall be familiar with the Project and authorized to conclude matters relating to the Project.

At each progress meeting, the participants shall (1) review items of significance that could affect progress; (2) discuss topics appropriate to the current status of the Project; (3) review progress since the last meeting; (4) determine whether each activity is on time, ahead of schedule, or behind schedule, in relation to the Contractor's Construction Schedule; (5) determine how to expedite any Project work that may be behind schedule; (6) discuss whether or not schedule revisions are required to ensure that current and subsequent activities will be completed within the Contract time; and (7) review the present and future needs of each entity represented at the meeting, including such items as interface requirements, time, sequences, deliveries, off-site fabrication problems, access, site utilization, temporary facilities and controls, hours of work, hazards and risks, housekeeping, quality and work standards, status of correction of deficient items, field observations, requests for interpretations, status of proposal requests, pending changes, status of construction orders, and documentation of information for payment requests.

The Contractor shall provide the Engineer, for inclusion in the meeting minutes, a brief summary of the Project's progress since the previous meeting.

The Engineer will distribute copies of minutes of the meeting to the Designer and the Contractor. The Contractor shall distribute copies to parties who were or should have been at the meeting.

4. Coordination Meetings: The Engineer will conduct Project coordination meetings as necessary, and shall follow the procedures established for progress meetings.

The Contractor shall request representation at each meeting by every party currently involved in coordination or planning for the construction activities involved.

The Engineer will record meeting results and distribute copies to everyone in attendance, the Designer, and to the Contractor to be distributed to others affected by decisions or actions resulting from each meeting.

5. Project Closeout Meeting: The Engineer will conduct a Project closeout meeting no later than 90 days prior to the anticipated expiration of Contract Time to review the requirements and responsibilities related to Project closeout.

The Engineer, Designer, Owner, the Contractor and its project coordinator, superintendent, major subcontractors, and other concerned parties shall attend the meeting. All participants at the meeting shall be familiar with Project and authorized to conclude matters relating to the Work.

The meeting participants shall discuss items of significance that could affect or delay Project closeout, including the following: (1) preparation of record documents; (2) procedures required prior to inspection for Semi-Final and Substantial Completion for acceptance; (3) submittal of written warranties; (4) requirements for preparing operations and maintenance data; (5) requirements for delivery of spare parts; (6) requirements for demonstration and training; (7) submittal procedures; (8) coordination of separate contracts or work to be performed by others; (9) Owner's partial occupancy requirements; (10) installation of Owner's furniture, fixtures, and equipment; and (11) responsibility for removing temporary facilities and controls.

The Engineer will record meeting results and distribute copies to everyone in attendance, the Designer, and to the Contractor to be distributed to others affected by decisions or actions resulting from the meeting.

NOTES

SECTION 1.20-1.06
CONTROL OF MATERIALS FOR FACILITIES CONSTRUCTION

1.20-1.06.01—Facilities Construction - Source of Supply, Buy America and Material Quality
1.20-1.06.02—Facilities Construction - Samples and Test
1.20-1.06.03—Facilities Construction - Storage
1.20-1.06.04—Facilities Construction - Defective Materials
1.20-1.06.05—Facilities Construction - Shipping Material
1.20-1.06.06—Facilities Construction - Vacant
1.20-1.06.07—Facilities Construction - Certified Test Reports and Materials Certificates
1.20-1.06.08—Facilities Construction -Warranties
1.20-1.06.25—Facilities Construction- Product Selection

1.20-1.06.01—Facilities Construction - Source of Supply, Buy America and Material Quality:

1. Source of Supply: A source of supply is defined as the original manufacturer of material(s) used within a project. A source of supply may fabricate material(s) such as precast concrete structures or hot mix asphalt from components originally manufactured by others. A broker or distributor of material(s) must not be identified as a source of supply.

The Contractor must notify the Engineer of the proposed source of supply for each of the materials specified listed on the Anticipated Source of Material (CON-083) Form within 30 days after bid award. Should it become necessary for the Contractor to obtain material from sources other than those indicated in the submitted Anticipated Source of Material (CON-083) Form, the Contractor shall notify the Engineer. the Engineer reserves the right to request additional information regarding such sources.

If at any time, the Department discovers that materials from a source of supply do not consistently conform to the Contract specifications, the Engineer will notify the Contractor of its nonconformance and that source of supply shall no longer be used for said application.

2. Buy America: All permanently incorporated steel and iron used in the construction of the Project must have been produced and fabricated in the United States. It is the intent of this specification to require that all manufacturing processes for all steel and iron materials and products to be used for the Project, including the coating of steel and iron, occur within the United States, with the following exceptions:

The Contractor may request, in accordance with Section 635.410(b)(4) of Title 23 CFR, approval to include a minimal amount of foreign steel in the Project construction. This amount is defined as 1/10 of 1% of the total Contract price or $2,500.00, whichever is greater. The cost of the foreign steel or iron is defined as its Contract value when delivered to the Project site.

Additionally, the FHWA has granted a nationwide waiver of the requirements of 23 CFR 635.410, Buy America requirements, for the production of pig iron and processed, pelletized, and reduced iron ore. Items not specifically included in the waiver remain subject to the Buy America requirements. The Contractor may request the Engineer to seek from the FHWA a further waiver of said requirements, but it shall be at the sole discretion of the Engineer whether or not to seek such a waiver.

3. Material Quality: Only materials conforming to the Contract and accepted by the Engineer shall be permanently incorporated into the Project.

Prior to installation, material that is damaged or otherwise changed in a way that it no longer meets Contract specifications shall not be incorporated into the Project.

When one manufacturer's product is specified in the Contract, it shall be understood that this represents the standard required. Unless otherwise stated, a comparable product of another manufacturer may be proposed by the Contractor unless the plans or special provisions indicate that no equal shall be allowed. The Contractor shall submit a complete description of the proposed product, together with shop drawings, catalog cuts, product data or other descriptive literature for review in accordance with 1.20-1.05.02. Should a product be designated as an equal, this will not relieve the Contractor from any material testing requirements or a related Certified Test Report and/or Materials Certificate that may be required.

The identification of a manufacturer or fabricator in the Contract does not imply acceptability of products from the named entity. All products must satisfy the Contract criteria for performance, efficiency, materials, and special accessories.

To the fullest extent possible, the Contractor shall provide products of the same kind from a single source. When specified products are available only from sources that do not or cannot produce a quantity adequate to complete Project requirements in a timely manner, the Contractor shall consult with the Engineer to

determine the most important product qualities before proceeding. Such qualities may include attributes such as visual appearance, strength, durability, or compatibility. When the Engineer has made such a determination, the Contractor shall select products in accordance with said determination to the fullest extent possible.

With respect to the Project, all products selected by the Contractor must be compatible with its previously selected products.

The Contractor shall place a permanent nameplate on each item of service-connected or power-operated equipment. In occupied spaces, the nameplate shall be located on an easily-accessible but inconspicuous surface. The nameplate shall contain: name of product and manufacturer, model and serial number, capacity, speed, ratings, and other essential operating data.

Except for required labels and operating data, the Contractor shall not attach or imprint manufacturer's or producer's nameplates or trademarks on exposed surfaces of products that will be exposed to view in occupied spaces or on a structure's exterior. The Contractor shall locate required product labels and stamps on concealed surfaces or, if required for observation after installation, on accessible but inconspicuous surfaces.

1.20-1.06.02—Facilities Construction - Samples and Test: The Contractor shall furnish all required samples without charge, and provide secure facilities for their storage. The Contractor shall provide means for, and shall assist in the verification of, all scales, measures and other devices that it operates or uses in connection with the Project. The Contractor shall provide calibration documentation when requested.

The minimum number and size of material samples per Contract item that are required by the Department are listed in the "Minimum Schedule for Acceptance Testing" contained in the Department's Materials Testing Manual. The acceptance methods used to determine compliance with the Contract are also listed. Where applicable, physical testing will be performed in accordance with the test standards which are in effect at the time of bidding, unless otherwise indicated in the Contract. Any item or material not listed in the "Minimum Schedule for Acceptance Testing" or Contract shall be sampled and tested and/or certified, as directed by the Engineer.

Certification may be used as the basis for approval of such materials as the Contract documents specify or as the Engineer may require. With regard to such materials, the Contractor shall furnish the Engineer a Certified Test Report (CTR) and/or Materials Certificate (MC), in accordance with 1.20-1.06.07 for each type of material, as may be required in the "Minimum Schedule for Acceptance Testing." The Contractor shall bear any costs involved in furnishing the CTR and MC.

If the Contractor has purchased materials for use on a previous Department project, and if they meet the requirements of this Contract, then those materials, with the approval of the Engineer, may be used for the Project provided that the Contractor, acting as the "supplier," submits a related MC meeting the requirements of 1.20-1.06.07. This MC shall further identify the project for which the material was originally purchased and shall be accompanied by a copy of the original MC.

Materials will be rejected by the Engineer whenever, in their judgment, they fail to meet Contract requirements. The Engineer may accept material or the combination of materials and thereby waive noncomplying test results, provided that the following conditions are met:

1. The Engineer finds results of prior and subsequent series of tests of the material or materials from the same source or sources to be satisfactory.
2. The incidence and degree of nonconformance with the Contract requirements are, in the Engineer's judgment, within reasonable limits.
3. The Contractor, in the Engineer's judgment, had diligently exercised material controls consistent with good practices.
4. No adverse effect on the value or serviceability of the completed work could result from said degree of nonconformance.

The Engineer may, in their discretion, waive testing of minor quantities of a particular material if said material was obtained from sources that have furnished supplies of the material that have consistently met Department testing standards.

1.20-1.06.03—Facilities Construction - Storage: The Contractor shall store all materials for the Project in a way that ensures that their quality and fitness for the Project will be preserved, and that the Engineer will have easy and prompt access to them for inspection purposes. Materials shall be kept on wooden platforms or on other hard, clean surfaces and not on the ground. When so directed by the Engineer, the Contractor shall store materials in a weatherproof building.

The Contractor shall not store materials in any way that would lead to a violation of these specifications including but not limited to 1.20-1.10. Stored materials, even if they have been approved by the Engineer prior to their storage, must be inspected by the Engineer and meet all pertinent Contract requirements immediately prior to use of those materials for the Project.

The Contractor shall (1) store products in accordance with the manufacturer's recommendations; (2) store products at the Site in a manner that will facilitate inspection and measurement or counting of units; (3) store heavy materials away from Project structures so as not to endanger the supporting construction; (4) if the products are subject to damage by the elements, store them off the ground, under cover in a weatherproof enclosure, with ventilation adequate to prevent condensation; and (5) maintain temperature and humidity within any range recommended by the manufacturer.

Off-site staging and storage of materials and equipment may be required due to restrictive Project Limits and other operational constraints. Arrangement for off-site staging and storage of materials and equipment shall be the responsibility of the Contractor. Payment for off-site staging and storage of materials and equipment shall be in accordance with 1.20-1.09.06.

1.20-1.06.04—Facilities Construction - Defective Materials: Unless otherwise permitted by the Engineer, all materials not meeting Contract requirements shall be considered defective, shall be rejected, and shall be removed immediately from the Project site.

If deemed necessary, the Engineer may require retesting of materials previously tested, accepted and incorporated into the Project. If materials do not meet the Contract requirements after retesting, the Engineer will make a determination whether to allow materials be left in place (with an equitable reduction of payment) or be removed and replaced. No rejected material, the defects of which have been subsequently corrected, shall be used until approval has been given by the Engineer. Should the Contractor fail to comply with these requirements, the Engineer has the authority to order the removal and replacement of defective material and deduct the cost of such removal and replacement from any future payment to the Contractor.

When a material is fabricated or treated with another material, or when any combination of materials is assembled to form a product, any or all of which are covered by the Contract specifications, the failure of any components of the product to meet the requirements of the specifications may be sufficient cause for the rejection of the whole combination or product.

1.20-1.06.05—Facilities Construction - Shipping Material: Any conveyance used for transporting materials must be clean when used, be in proper working condition, have a strong and substantial body that will prevent the loss of materials during transportation, and be approved by the Engineer.

1.20-1.06.06—Facilities Construction - Vacant

1.20-1.06.07—Facilities Construction - Certified Test Reports and Materials Certificates: The Contractor shall furnish the Engineer with any Certified Test Report and Materials Certificate required by the Contract or the "Minimum Schedule for Acceptance Testing" contained in the Department's Materials Testing Manual.

The Contractor shall forward the Certified Test Report and Materials Certificate to the Engineer, and, in addition, shall deliver a copy of same to the Department's inspector at the Site. Materials for which such documentation is required may be conditionally incorporated into the Project prior to the Engineer's acceptance of a Certified Test Report and a Materials Certificate; however, payment for such incorporated material may not be made prior to acceptance by the Department of a Certified Test Report and Materials Certificate indicating that the material meets the Contract requirements.

1. A Certified Test Report (CTR) is a document containing a list of the dimensional, chemical, metallurgical, electrical and physical results obtained from a physical test of the materials involved, and shall demonstrate that the materials meet the requirements of the Contract. The CTR shall be signed by a duly-authorized and responsible agent of the original manufacturer of the material(s), and the signature must include the date the CTR was signed and notarized.

The CTR shall also include the following information:

a. Description of material(s)
b. Date of manufacture of the material(s)
c. Date of test(s)
d. Name of organization to which the material has been consigned
e. Quantity of material represented

f. Means of identifying the consignment, such as label, marking, lot number, etc.
g. Date and method of shipment
h. Name of organization performing the test(s)

2. A Materials Certificate (MC) is a document certifying that the materials, components and equipment furnished meet all requirements of the Contract. The MC shall be signed by a duly-authorized and responsible agent of the organization assembling or fabricating the material and the signature must include the date the MC was signed and notarized.

The MC shall also include the following information:

a. Project for which the material has been consigned
b. Name of Contractor to which material is supplied
c. Item number and description of material
d. Quantity of material represented by the MC
e. Means of identifying the consignment, such as label, marking, lot numbers, etc.
f. Identification of all sources of supply of material components
g. Means of verifying Buy America requirements for steel and/or steel components
h. Date and method of shipment

1.20-1.06.08—Facilities Construction - Warranties: Warranties shall be delivered to the Designer prior to acceptance of the Project.

Standard warranties are written warranties published by individual manufacturers for particular products, which are specifically endorsed by the manufacturer to the State. Special warranties are written warranties required by the Contract, either to extend time limits provided by standard warranties or to provide greater rights for the State. All required warranties shall be endorsed to, or have named as obligee, the State.

Manufacturer's disclaimers and limitations on product warranties do not relieve the Contractor of the Contractually-required warranty, that incorporates the products, nor does it relieve suppliers, manufacturers, and subcontractors required by the Contract to countersign special warranties with the Contractor.

Unless otherwise directed by the Engineer, the commencement date for warranties shall be the date of the issuance of the Certificate of Compliance. When a designated portion of the Project is completed and used by the Engineer or occupied by the Owner, by separate agreement with the Contractor during the construction period, the Contractor shall coordinate with the Engineer the submission date for properly-executed warranties and commencement date for those affected warranties. When a special warranty is required to be executed by the Contractor, or by the Contractor and a subcontractor, supplier or manufacturer, the Contractor shall prepare a written document that contains appropriate terms and identification, ready for execution by the required parties.

Written warranties made to the Engineer shall be deemed to supplement implied warranties, and shall not limit the duties, obligations, rights or remedies otherwise available under the law, nor shall warranty periods be interpreted as limitations of the time in which the Engineer can enforce such other duties, obligations, rights, or remedies.

The Contractor shall submit draft warranties for approval prior to final execution. The Engineer reserves the right to reject warranties and to limit selections to products with warranties that do not conflict with Contract requirements.

Where the Contract requires a special warranty, or similar commitment regarding the Project or part of the Project, the Engineer reserves the right to refuse to accept the related work until evidence is presented that entities required to countersign such commitments are willing to do so.

Prior to the date for the Substantial Completion Inspection, the Contractor shall compile 3 copies of each required warranty, properly executed by the Contractor or any other required party. The Contractor shall place the warranty documents in an orderly sequence based on the organization of the Contract provisions (including specific CSI-formatted specifications contained within a particular Special Provision).

The Contractor shall:

(a) Bind warranties in heavy-duty, commercial-quality, durable 3-ring vinyl-covered loose-leaf binders, thick enough to accommodate the contents, and sized to receive 8 1/2 inch × 11 inch paper.
(b) Identify the binder's contents on the binder's front and spine with the typed or printed title "WARRANTIES," the Project title or name, and the name of the Contractor.
(c) Provide a heavy paper divider with a tab for each separate warranty.
(d) Mark the tab to identify the related product or installation.

(e) Provide a typed description of the product or installation, including the name of the product, and the name, address and telephone number of the Contractor or pertinent subcontractor.

(f) Furnish to the Department a written warranty for all Project work accompanied by a cover letter with the following contents:

[Addressed to:]

Commissioner of Transportation
Department of Transportation
P.O. Box 317546
Newington, Connecticut 06131-7546

Project Title and Number

[We] hereby warrant all materials and workmanship for all work performed under this Contract for a period of one (1) year from [date of issuance of C.O.C.] against failures of workmanship and materials in accordance with the Contract. Furthermore, as a condition of this warranty, [we] agree to have in place all insurance coverage identified in the Contract for the performance of any warranty work.

[Signature:] [Name of authorized signatory]
[Title]

(g) Submit to the Engineer, upon completion of installation of materials or assemblies that are required to have either a flame-rating or a fire-endurance hourly rating, a detailed letter certifying that the required rating has been attained.

Upon determination by the Engineer that Project work covered by a warranty has failed, the Contractor shall replace or rebuild the work to an acceptable condition complying with Contract requirements. The Contractor is responsible for the cost of replacing or rebuilding defective construction or components and those which may have needed to be damaged or removed in order to cure the defective work including costs of material, equipment, labor, and material disposal, regardless of whether or not the State has benefited from use of the work through a portion of its anticipated useful service life. The Contractor shall respond to the Project Site when Project work covered by a warranty has failed within 3 calendar days, unless in the Engineer's opinion said failure is deemed to be an emergency, in which case the Contractor shall respond to the Project Site as directed by the Engineer.

When Project work covered by a warranty has failed and been corrected by replacement or rebuilding, the Contractor shall reinstate the warranty by written endorsement. The reinstated warranty shall be equal to the time that remains on the original warranty period at the time of the failure.

1.20-1.06.25—Facilities Construction - Product Selection: The Contractor shall provide products that comply with the Contract, that are undamaged and, unless otherwise indicated, unused at the time of installation. The Contractor shall provide products complete with accessories, trim, finish, safety guards and other devices and details needed for a complete installation and the intended use and effect. The Contractor shall provide standard products of types that have been produced and used successfully in similar situations on other projects, when such products are available, unless the Contract requires otherwise. Descriptive, performance, and reference standard requirements in the Contract provisions establish salient characteristics of products.

Contractor's options for selecting products are limited by the Contract and governing regulations, and are NOT controlled by industry traditions or procedures used by the Contractor on previous construction projects. Procedures governing product selection include the following:

(a) The Contractor shall not use product substitutes as defined in 1.20-1.01.01.

(b) Semi-proprietary Specification Requirements: When the Contract lists 3 or more acceptable products or manufacturers unaccompanied by the term "Or Equal," the Contractor shall provide one of the products indicated. In such a case, no "Equal" will be permitted.

(c) Non-Proprietary Specification Requirements: When the Contract lists products or manufacturers whose products are available and may be incorporated into the Project, or when the list is accompanied by the term "Or Equal," then the Contractor is not restricted to use those products, but may propose any available product that complies with Contract requirements.

(d) Descriptive Specification Requirements: When the Contract describes a product or assembly, listing

exact characteristics required, with or without use of a brand or trade name, the Contractor shall provide a product or assembly that provides those characteristics and otherwise complies with the Contract.

(e) Performance Specification Requirements: When the Contract contains performance requirements, the Contractor shall provide products that comply with those requirements, and that are recommended by the manufacturer for the application indicated. Such recommendations may be derived from the manufacturer's published product literature or by the manufacturer's certification of performance.

(f) Visual Matching: When the Contract requires matching an established sample, the Engineer's decision will be final as to whether or not a proposed product matches satisfactorily. When no product available within the specified category matches satisfactorily and complies with other specified requirements, the Contractor shall comply with Contract provisions concerning "Or Equal" submissions for selection of a matching product in another product category.

(g) Visual Selection: When a Contractual product requirement includes the phrase "...as selected by the Designer from manufacturer's full range ..." or a similar phrase, the Contractor shall select a product line that complies with Contract requirements. The Designer will select the color, gloss, pattern, density, or texture from the product line that includes both standard and premium items. Bids will be based on premium items.

(h) Basis-of-Design Product Specification: A specification in which a specific manufacturer's product is named and accompanied by the words "basis-of-design," including make or model number or other designation, to establish the significant qualities related to type, function, dimension, in-service performance, physical properties, appearance, and other characteristics for purposes of evaluating comparable products of additional manufacturers.

SECTION 1.20-1.07
LEGAL RELATIONS AND RESPONSIBILTIES FOR FACILITIES CONSTRUCTION

1.20-1.07.01—Facilities Construction - Laws to Be Observed
1.20-1.07.02—Facilities Construction - Permits and Licenses
1.20-1.07.03—Facilities Construction - Proprietary Devices, Materials and Processes
1.20-1.07.04—Facilities Construction - Restoration of Surfaces Opened Pursuant to Permit or Contract
1.20-1.07.05—Facilities Construction - Load Restrictions
1.20-1.07.06—Facilities Construction - Sanitary Provisions
1.20-1.07.07—Facilities Construction - Safety and Public Convenience
1.20-1.07.08—Facilities Construction - Use of Explosives
1.20-1.07.09—Facilities Construction - Protection and Restoration of Property
1.20-1.07.10—Facilities Construction - Contractor's Duty to Indemnify the State against Claims for Injury or Damage
1.20-1.07.11—Facilities Construction - Opening of Section of Project to Traffic or Occupancy
1.20-1.07.12—Facilities Construction - Contractor's Responsibility for Work
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1.20-1.07.14—Facilities Construction - Personal Liability of Representatives of the State
1.20-1.07.15—Facilities Construction - No Waiver of Legal Rights
1.20-1.07.16—Facilities Construction - Unauthorized Use of Area(s) within the Project Site
1.20-1.07.17—Facilities Construction - Vacant
1.20-1.07.18—Facilities Construction - Use of State Property

1.20-1.07.01—Facilities Construction - Laws to Be Observed: The Contractor at all times shall observe and comply with all laws, ordinances, government bylaws, permits, regulations, orders and decrees which in any manner affect the conduct of the Contract work. The Contractor shall indemnify and save harmless the State and all of its officers, employees and agents against any claim, fine, or other liability arising from or based on the violation of any such law, bylaw, permit, ordinance, regulation, order or decree, whether by the Contractor, its subcontractors or any of their officers, employees or agents. See the third paragraph in 1.20-1.02.04, however, regarding conflicts between municipal law or authorities and the requirements of Project construction.

1.20-1.07.02—Facilities Construction - Permits and Licenses: Except as may be provided otherwise in a specific Contract provision or a written direction from the Engineer, the Contractor shall procure all permits and licenses, pay all charges and fees, and give all notices required by government authorities in connection with the due prosecution of the Project.

The Contractor will not be required to apply for a Building Permit from the local Building Official since the state will issue a Notice of Intent to Construct (NOIC) as the Building Permit.

The Contractor shall apply for and obtain a demolition permit from the municipality for each building to be demolished prior to initiating any demolition activities in accordance with CGS 29-401 through 29-415. Said statutes permit a waiting period from the demolition permit application to approval of not more than 90 days so the Contractor shall schedule its work accordingly.

The Contractor shall apply for and obtain the necessary permits related to the installation of wells and septic systems.

Under Connecticut law, a commercial vehicle used by a contractor or vendor in connection with the Project may be subject to Connecticut registration requirements. The CGS require such registration for any vehicle that most often is garaged in this State, or that most often leaves from and returns to one or more points within this State in the normal course of its operation. In addition, a vehicle must be registered in Connecticut if it continuously receives and discharges cargo within this State. Reciprocal registrations as allowed under CGS are acceptable for meeting the registration requirements.

Residence or domicile of the owner, lessor or lessee of the motor vehicle, or the place where the owner, lessor or lessee is incorporated or organized, shall not be a factor in determining whether or not the vehicle must be registered in this State. Failure to register a vehicle, if the law requires it, may result in issuance of a citation for such an infraction, and also may result in administrative action by the Commissioner of Motor Vehicles.

The registration requirement applies not only to the Contractor, but also to its subcontractors, suppliers, and other agents and representatives. It is the Contractor's responsibility to ensure that such entities and individuals comply with this requirement as well. The Contractor shall maintain, on the Project Site, records that document compliance with this requirement in connection with all vehicles used for the Project.

1.20-1.07.03—Facilities Construction - Proprietary Devices, Materials and Processes: If the Contractor is required or desires to use any design, device, material or process covered by another party's license, patent, copyright or trademark, the Contractor shall provide for such use by suitable legal agreement with the license, patent, copyright or trademark holder.

The Contractor shall provide a copy of any and all such agreements to the Engineer.

If the Contractor is allowed, but not specifically required by the Engineer, to use any particular proprietor's design, device, material or process covered by license, patent, copyright or trademark, the Contractor and its surety shall indemnify and save harmless the State from any and all claims that may be brought against the State, and any and all costs, expenses, and damages that the State may be obligated to pay by reason of any infringement or alleged infringement relating to the use of such licensed, patented, copyrighted or trademarked design, device, material or process at any time during the prosecution or after the completion of the Project.

1.20-1.07.04—Facilities Construction - Restoration of Surfaces Opened Pursuant to Permit or Contract: The Contractor shall not make, and shall not allow any person to make, an opening in a highway unless written and duly-authorized permission to do so has been obtained from the Department. If at any time prior to the completion of the Project, the Contractor should make such an opening without such permission, the Contractor shall perform all restoration necessary to close said opening, at its own expense, if the Engineer directs it to do so.

1.20-1.07.05—Facilities Construction - Load Restrictions

(a) Vehicle Weights: This subarticle will apply to travel both on existing pavements and pavements under construction. The Contractor shall comply with all legal load restrictions as to vehicle size, the gross weight of vehicles, and the axle weight of vehicles while hauling materials. Throughout the duration of the Contract, the Contractor shall take precautions to ensure existing and newly installed roadway structures and appurtenances are not damaged by construction vehicles or operations.

Unless otherwise noted in Contract specifications or plans, on and off road equipment of the Contractor, either loaded or unloaded, will not be allowed to travel across any bridge or on any highway when such a vehicle exceeds the statutory limit or posted limit of such bridge or highway. Should such movement of equipment become necessary the Contractor shall apply for a permit from the Department for such travel, as provided in the CGS. The movement of any such vehicles within the Project limits or detour routes shall be submitted to the Engineer for Project record. Such permit or submittal will not excuse the Contractor from liability for damage to the highway caused by its equipment.

The Contractor is subject to fines, assessments and other penalties that may be levied as a result of violations by its employees or agents of the legal restrictions as to vehicle size and weight.

(b) Storage of Construction Materials/Equipment on Structures: Storage is determined to be non-operating equipment or material. The Contractor shall not exceed the statutory limit or posted limit for either an existing or new structure when storing materials and/or construction equipment. When a structure is not posted, then the maximum weight of equipment or materials stored in each 12 foot wide travel lane of any given span shall be limited to 750 pounds per linear foot combined with a 20,000 pound concentrated load located anywhere within the subject lane. If anticipated storage of equipment or material exceeds the above provision, then the Contractor shall submit its proposal of storage supported by calculations stamped by a Professional Engineer registered in the State of Connecticut, to the Engineer for approval 14 days prior to the storage operation. Operations related to structural steel demolition or erection shall follow the guidelines under 6.03. All other submittals shall include a detailed description of the material/equipment to be stored, the quantity of storage if it is stockpiled materials, the storage location, gross weight with supporting calculations if applicable, anticipated duration of storage and any environmental safety, or traffic protection that may be required. Storage location on the structure shall be clearly defined in the field. If structures are in a state of staged construction or demolition, additional structural analysis may be required prior to authorization of storage.

1.20-1.07.06—Facilities Construction - Sanitary Provisions: The Contractor shall provide and maintain in a neat and sanitary condition such accommodations for the use of its employees as may be necessary to comply with the regulations and other requirements of the State Department of Public Health or of other bodies or tribunals having jurisdiction over such matters.

The Contractor may not use the State's existing toilet facilities.

1.20-1.07.07—Facilities Construction - Safety and Public Convenience: The Contractor shall conduct the Project work at all times in such a manner as to ensure the least possible obstruction to traffic. In a manner acceptable to the Engineer, the Contractor shall provide for the convenience and interests of the general public; the traveling public; parties residing along or adjacent to the highway or Project site; and parties owning, occupying or using property adjacent to the Project site, such as commuters, workers, tenants, lessors and operating agencies.

Notwithstanding any other Contract provision, the Contractor shall not close to normal pedestrian or vehicular traffic any section of road, access drive, parking lot, sidewalk, station platform, railroad track, bus stop, runway, taxiway, occupied space within a site, or occupied space within a building, except with the written permission of the Engineer.

All equipment, materials, equipment or material storage areas, and work areas must be placed, located, and used in ways that do not create a hazard to people or property, especially in areas open to public pedestrian or vehicular traffic. All equipment and materials shall be placed or stored in such a way and in such locations as will not create a hazard to the traveling public. In an area unprotected by barriers or other means, equipment and materials must not be stored within the clear zone of any traveled way. Clear zones are based on design speed and roadway geometry. The following minimum distances, measured from edge of travelway to the temporary hazard, are recommended:

Posted Speed Limit, mph	Distance, feet
55 or more	30
45 to 55	24
under 45	16

The Contractor must always erect barriers and warning signs between any of its work or storage areas and any area open to public, pedestrian, or vehicular traffic. Such barriers and signs must comply with all laws and regulations, including any applicable codes.

The Contractor must arrange for temporary lighting, snow and ice removal, security against vandalism and theft, and protection against excessive precipitation runoff within its Project work and storage areas, and within other areas specifically designated in the Contract.

In addition to meeting the requirements of 9.71, the Contractor shall take all precautions necessary and reasonable for the protection of all persons, including, but not limited to, employees of the Contractor or the Department, and for the protection of property, until the Engineer notifies the Contractor in writing that the Project or the pertinent portion of the Project has been completed to the Engineer's satisfaction. The Contractor shall comply with the safety provisions of applicable laws, including building and construction codes and the latest edition of the CFR. The Contractor must make available for reference in its field office, throughout the duration of the Project, a copy of the Safety Plan and the latest edition, including all supplements, of the CFR pertaining to OSHA.

The Contractor shall furnish to the Engineer's representative supervising the Project a report on any accident that occurs on the Project site with regard to which the Contractor is required to report under OSHA or any other legal requirement. The Contractor shall also furnish to the Engineer a report regarding any other accident involving public liability or property damage in connection with the Project. The form and detail of such reports must be acceptable to the Engineer.

The Contractor shall designate a competent representative with authority to act in cooperation with the Department in the enforcement of safety provisions and promotion of safe practices on and related to the Project throughout the duration of the Project.

Before beginning work on the Project, the Contractor shall have a Safety Plan on file with the Department. The Safety Plan shall include the policies and procedures necessary for the Contractor to comply with OSHA and other pertinent regulatory rules, regulations and guidelines. The Safety Plan may be a comprehensive company-wide plan provided it addresses the scope and type of work contemplated by the Contract. The Safety Plan shall address all the requirements of this Section and any applicable State or Federal regulations, and shall be revised and updated as necessary.

The following elements shall be included in the Safety Plan:

1. General introduction describing the scope and applicability of the Safety Plan.
2. Identification of key staff responsible for the implementation and monitoring of the Contractor's Safety Plan, and their roles and responsibilities for safety.
3. Training requirements relative to safety.
4. Safety rules and checklists specific to the types of work generally performed by the Contractor.
5. Record-keeping and reporting requirements.
6. Identification of special hazards related to specific work elements.

The Contractor is responsible for the Safety Plan. Pursuant to 1.20-1.07.10, the Contractor shall indemnify, and save harmless the State from any and all liability related to any violation of the Safety Plan.

Under 1.20-1.08.06, the Engineer may suspend the work of the Contractor if and when the latter does not take the safety precautions referenced in this article. Nothing herein shall be construed, however, to relieve the Contractor from responsibility for the prosecution of the Project.

1.20-1.07.08—Facilities Construction - Use of Explosives: To the extent possible, the Contractor shall avoid using explosives in proximity to existing structures. When the use of explosives is necessary for the prosecution of the Project, the Contractor shall take the utmost care not to endanger life or property.

The Contractor shall take adequate protective measures when engaging in blasting operations, and shall be responsible for any damage resulting from such operations.

The Contractor shall notify each utility with facilities in proximity to the site of such blasting operations, and any other individuals and entities that may be affected thereby, of the Contractor's intention to use explosives. Such notice shall be given sufficiently in advance of any blasting to enable such affected parties to take steps to prevent such blasting from injuring persons or property. Such notice shall not relieve the Contractor of responsibility for damage resulting from its blasting operations.

1.20-1.07.09—Facilities Construction - Protection and Restoration of Property: The Contractor shall not enter upon public or private property for any purpose without having obtained written permission to do so from the owner of such property and having provided the Engineer with a copy of same. The Department is not, and may not be deemed, a party to any agreement between the Contractor and a property owner unless the Department executes said agreement.

The Contractor shall use every reasonable precaution to avoid disturbing or damaging public or private property, including, but not limited to, trees and monuments. The Contractor shall use suitable precautions to avoid disturbing or damaging underground or overhead structures or facilities, whether or not they are shown on the plans.

If the Project requires the moving or removal of a land monument or property marker, the Contractor shall not disturb it until a duly-authorized agent of the public or private property's owner has witnessed or recorded the monument or marker's location. The Contractor shall not move or remove such property until and unless directed to do so by the Engineer.

The Contractor shall not remove, cut, injure or destroy trees or shrubs without the Engineer's prior approval.

The Contractor shall be responsible for all damage to property resulting from any act, omission, neglect or misconduct in the Contractor's manner or method of executing its work, or due to its defective work or materials. When or where any direct or indirect damage is done to public or private property by or on account of any act, omission, neglect, or misconduct in the execution of the Project work, the Contractor shall restore, at its own expense, such property to a condition as close as possible to that which existed before such damage was done, by repairing, rebuilding or otherwise restoring the property, as may be directed by the Engineer; or the Contractor shall make good such damage in another manner acceptable to the Engineer. If the Contractor fails to restore such property or make good such damage in a way acceptable to the Engineer, the Engineer may, upon 48 hours' notice, proceed to have such property repaired, rebuilt or restored as he may deem necessary; and the cost thereof will be deducted from any monies due or which may become due the Contractor under the Contract or under any other contract(s) that the Contractor may have with the State.

The Engineer shall mark the locations of underground facilities belonging to the State when given 3 calendar days (excluding Saturdays, Sundays, and State holidays) notice by the Contractor that it will be excavating or driving material into the ground near such facilities as a part of necessary Contract work. After the Engineer marks the location of such facilities, it will be the Contractor's responsibility to maintain the location markers until no longer needed. Repairs of State facilities located further than 1 foot from the

line delineated by such markers shall be paid for by the State.

1.20-1.07.10—Facilities Construction - Contractor's Duty to Indemnify the State against Claims for Injury or Damage: The Contractor shall indemnify and save harmless the State, the Department and all of its officers, employees and agents from all suits, actions or claims of any character, name or description brought for or on account of any injury or damage caused to any person or property as a result of, in connection with, or pursuant to the performance of the Contract, including all costs incurred by the State in defending itself against such claims or actions, in proportion to the extent that the Contractor is held liable for same by an arbiter of competent jurisdiction. As much of any money that may be due the Contractor under the Contract as the Commissioner considers necessary for the purpose of such indemnification or holding the State harmless may be retained for such use by the State; and the Contractor's surety bonds may be held until such suit or suits, action or actions, claim or claims, as aforesaid, shall have been settled and until the Contractor has furnished to the Commissioner suitable evidence to that effect. Such indemnity shall not be limited by reason of any insurance coverage required under the Contract.

It is further understood and agreed by the parties hereto, that the Contractor shall not use the defense of Sovereign Immunity in the adjustment of claims or in the defense of any suit, including any suit between the State and the Contractor, unless directed to do so by the State.

1.20-1.07.11—Facilities Construction - Opening of Section of Project to Traffic or Occupancy: Whenever, in the opinion of the Engineer, any portion of the Project has been substantially completed, it may be opened to traffic or occupancy as directed by the Engineer. The Engineer's approval of any such opening shall not be held to be in any way an acceptance of such completed portion of the Project, or as a waiver of any of the provisions of these Specifications, or of any state or federal statutes, applicable building codes, or other Contract provisions. Such approval shall not constitute a basis for claims for damages due to interruptions to, or interference with, the Contractor's operations.

If repair or replacement of any portion of the Project construction becomes necessary because the Engineer has directed that said portion be opened to travel or occupancy prior to completion of the Contract work, the Contractor shall perform that repair or replacement. The Contractor shall perform such work at its own expense, unless the Department or an arbiter of competent jurisdiction shall determine definitely that the damage necessitating the repair or replacement was caused by equipment operated by a State employee while controlling snow or ice, or by routine State maintenance operations. In the latter cases, the State shall reimburse the Contractor for the cost of the repair or replacement. If the damage was caused by a traffic accident involving only a vehicle or vehicles that were not owned by the State and were not operated by an agent of the State, the Contractor may seek recovery from the responsible parties, but not from the State.

1.20-1.07.12—Facilities Construction - Contractor's Responsibility for Work: From the date for commencement of construction given in the "Notice to Proceed" until the date when the Engineer relieves the Contractor of responsibility for the Project, the Project construction and site shall be under the charge and care of the Contractor; and the Contractor shall take every necessary precaution against damage to the same or any part thereof by the action of the elements or from any other cause, including either execution or non-execution of Project work. The Contractor shall rebuild, repair, restore or otherwise make good, at its own expense, all damage to, or impairment of, any portion or purpose of the Project which results from any of the above causes prior to completion of the Project, except as provided in 1.20-1.07.11.

1.20-1.07.13—Facilities Construction - Contractor's Responsibility for Adjacent Property, Facilities and Services: The Project work shall not commence until the Contractor has made all arrangements necessary to protect all property and facilities adjacent to the Project site, including, but not limited to, those of utilities, from damaging or disruptive effects of Project operations. The Contractor shall cooperate with the owners of any underground or overhead utility lines in their removal and rearrangement operations in order that these operations may progress in a reasonable manner, that duplication of such utilities work may be kept to a minimum, and that services rendered by those parties will not be unnecessarily interrupted.

In the event of interruption of water or utility services as a result of accidental breakage of facilities, or as a result of related facilities being exposed or unsupported, the Contractor shall promptly notify the proper utility and law enforcement authorities and the Engineer of same. The Contractor shall cooperate with said authorities in the restoration of such services as promptly as possible. In no case shall the Contractor leave the site until the interrupted service has been restored. Fire hydrants shall be kept accessible at all times,

and no materials shall be left within 15 feet of any fire hydrant.

1.20-1.07.14—Facilities Construction - Personal Liability of Representatives of the State: In carrying out any of the provisions of these Specifications, or in exercising any power or authority granted by the Contract, or by law or regulation, the Commissioner, Engineer, and their authorized representatives, including consultant engineering firms and their employees, shall be subject to no liability, either personally or as officials of the State, it being understood that in all such matters they act solely as agents and representatives of the State.

1.20-1.07.15—Facilities Construction - No Waiver of Legal Rights: The Commissioner reserves the right, should the Department discover an error in the estimate, or conclusive proof of defective work done or material used by or on behalf of the Contractor, either before or after the acceptance of the Contract, or even after the final payment has been made to the Contractor, to retain and apply monies owed to the Contractor under any State contract, or to claim and recover by process of law such sums, in order to correct any error or make good any defects in the Project work or materials.

1.20-1.07.16—Facilities Construction - Unauthorized Use of Area(s) Within the Project Site: The use of any area within the Project site for any purpose other than the construction of the Project, without prior written authorization to do so from the Commissioner, is prohibited.

Any request by the Contractor for authorization of such special use must include details describing the proposed use. If the proposed special use would involve the Contractor's making any lease or any profits in connection with the proposed use, the Contractor must enter into an agreement with the State for an equitable sharing of any profits with the State before such use may be authorized.

1.20-1.07.17—Facilities Construction - Vacant

1.20-1.07.18—Facilities Construction - Use of State Property: The Contractor may not use State property for any purpose or activity other than carrying out the construction activities required by the Contract, except with the prior written consent of the Engineer.

Such other activities, which require the Engineer's advance consent, include, but are not limited to, the establishment of staging areas, storage areas, asphalt plants, concrete plants, or gravel/borrow pits; or the conduct of screening, crushing, manufacturing, or mining operations.

Any permitted use of the Project site or other State property for such other purposes or activities must be for the performance of the specific Contract only, and must be at no cost to the State. In addition, the Contractor may not assert or bring any claim or formal proceeding for damages or additional compensation based on either the approval or denial of a request to make such use of the Project site or other State property.

Under no circumstances shall the bulk storage of fuel or lubricants by the Contractor or its agents be permitted on State property. The Contractor shall not conduct work processes or store any construction materials or supplies of such types, quantities or configuration, either individually or in total, on, under or near a structure, that creates an unreasonable risk of substantial damage to State property. The Contractor shall not store any hazardous materials on State property other than those that are integral to the Contractor's performance of the Contract, as allowed by the Contract and in accordance with 1.20-1.10, or in writing from the Engineer. The Contractor shall have the responsibility and duty to ensure the proper storage, handling, management and disposal of any such hazardous materials. The Contractor shall be liable to the Department for all remedial or punitive costs, damages or penalties incurred by the Department as a result of the Contractor's failure to fulfill this duty.

The Engineer may require environmental testing of the affected site at the Contractor's expense both prior to and upon completion of the Contractor's permitted use of the site or of other related State property. The Contractor shall be responsible for ensuring that such a site is restored to the condition required by the Engineer and that all contaminants deposited on the site by the Contractor or its agents are removed and properly disposed of. All such restoration and removal activities must be carried out at the Contractor's expense, and must be carried out in accordance with the provisions of the Department's Required Best Management Practices, any applicable environmental permits, and all other applicable State or Federal laws or regulations.

The Contractor must submit any request to use State property for a staging or storage area to the District Engineer at the District Construction Office. The following information, at a minimum, must accompany such written request:

(a) A detailed description of the proposed operation or use of State property.

(b) A site plan detailing the proposed location of any operations, materials, or facilities related to the requested use, including any appropriate sedimentation or erosion controls.

(c) An area plan detailing anticipated ingress to and egress from the site of the proposed activity or the Project site, as appropriate, and indicating the location of and proximity to residential or occupied buildings in the vicinity.

(d) Copies of any related, required or affected environmental permits.

(e) A detailed listing or description of the anticipated dates and hours of the proposed operations or activities.

(f) Photo documentation (a minimum of 12 - 8 inch × 10 inch color photographs) as follows:

(i) the preconstruction condition of each site of the proposed activities and

(ii) adjacent property at the boundaries of those areas.

If the site to be used or affected is State property that lies outside of any Department right-of-way, the Contractor must also obtain from other State agencies all necessary or appropriate authorizations for the proposed use(s) of State property.

Any request by the Contractor relating to a proposed use of State property for activities other than the establishment of a construction staging or storage area must also be submitted to the District Engineer at the District Construction Office, and must include the same information required by (a) through (f) of the preceding paragraph. In addition, in connection with such other requests, the Contractor must submit to the District Engineer:

(g) written confirmation from the municipality or municipalities in which each affected site is located that each such municipality has no objection to the proposed use or activity; and

(h) a license agreement with the Department, executed by the Contractor, on terms acceptable to the Department, defining the nature and scope of the proposed use or activity.

Gore areas are not available for disposal of surplus material.

For any request to establish or operate an asphalt batching or continuous mix facility, the Contractor must also provide to the District Engineer at the District Construction Office a map detailing the outermost perimeter of the proposed facilities and operations, showing all related and potentially-affected structures, land uses, watercourses, wetlands, and other areas of environmental concern within 1/3 of a mile of the facility or operation perimeter. No such facility will be permitted on State property where any hospital, nursing home, school, area of environmental concern, watercourse, or residential housing exists within 1/3 of a mile of the perimeter of the facility or operation (as per Public Act 98-216).

NOTES

SECTION 1.20-1.08
PROSECUTION AND PROGRESS FOR FACILITIES CONSTRUCTION

1.20-1.08.01—Facilities Construction - Transfer of Work or Contract
1.20-1.08.02—Facilities Construction - Establishment of Construction Field Office
1.20-1.08.03—Facilities Construction - Prosecution of Work
1.20-1.08.04—Facilities Construction - Limitation of Operations
1.20-1.08.05—Facilities Construction - Personnel and Equipment
1.20-1.08.06—Facilities Construction - Suspensions of Work Ordered by the Engineer
1.20-1.08.07—Facilities Construction - Determination of Contract Time
1.20-1.08.08—Facilities Construction - Extension of Time
1.20-1.08.09—Facilities Construction - Failure to Complete Work on Time
1.20-1.08.10—Facilities Construction - Termination of Contract for Cause
1.20-1.08.11—Facilities Construction - Final Cleaning Up
1.20-1.08.12—Facilities Construction - Semi-Final, Substantial Completion, and Final Completion Inspections
1.20-1.08.13—Facilities Construction - Termination of the Contractor's Responsibility
1.20-1.08.14—Facilities Construction - Acceptance of Project

1.20-1.08.01—Facilities Construction - Transfer of Work or Contract: The Contractor shall perform with its own organization Contract work with a value under the Contract of at least 25% of the original total Contract value. If the Contractor sublets, sells, transfers, or otherwise disposes of any part of the Contract work without the Commissioner's prior written consent, the Contractor will not be relieved of any Contractual or other legal responsibility in connection therewith. Such an unauthorized act by the Contractor shall constitute a material breach of the Contract, and the Commissioner may, in such a case, terminate the Contract without further compensation to the Contractor.

The Contractor shall include the following alternative dispute resolution clause in all of its Project subcontracts:

"For any dispute arising out of the agreement between the Contractor and a subcontractor, including claims of late payment or non-payment, which cannot be settled within 60 days of the subcontractor submitting a written claim to the Contractor, either party may bring the dispute before an alternative dispute resolution entity for resolution. If the parties do not agree upon a particular dispute resolution entity for that purpose, the dispute shall be resolved under the auspices and construction arbitration rules of the American Arbitration Association, or under the rules of any other alternative dispute resolution entity approved by the Department either generally or for the specific dispute. The Department may not be made a party to formal arbitration regarding such a dispute. These rights and restrictions may not be waived, and if these provisions are not included in the Contractor's subcontracts for the Project, these provisions shall nonetheless be read into them."

The Contractor shall not knowingly enter into any lower-tier transaction on a Department project with any person or entity which, under any federal or state law or regulation, or by voluntary agreement, is currently debarred or disqualified from bidding for construction contracts or participating in construction projects in any jurisdiction within the United States, unless after disclosure of such ineligibility, such participation is authorized by appropriate federal and State authorities, including the Commissioner.

The Contractor shall not sublet, sell, transfer, assign, or otherwise dispose of the Contract or any portion thereof, or of the work provided for therein, or of its right, title, or interest therein, to any individual or entity without the written consent of the Commissioner. No payment will be made for such work until written consent is provided by the Commissioner.

The Contractor shall pay the subcontractor for work performed within thirty (30) days after the Contractor receives payment for the work performed by the subcontractor. Withholding retainage by the Contractor, subcontractor or lower tier subcontractors is not allowed.

Payment for work that has been performed by a subcontractor does not eliminate the Contractor’s responsibilities for all the work as defined in 1.20-1.07.12, “Contractor’s Responsibility for Work.”

Payment for work that has been performed by a subcontractor also does not release the subcontractor from its responsibility for maintenance and other periods of subcontractor responsibility specified for the subcontractor’s items of work. Failure of a subcontractor to meet its maintenance, warranty or defective work responsibilities may result in administrative action on future Department contracts.

For any dispute regarding prompt payment, the alternate dispute resolution provisions of this article shall apply.

The above requirements are also applicable to all sub-tier subcontractors and the above provisions shall be made a part of all subcontract agreements.

Failure of the Contractor to comply with the provisions of this section may result in a finding that the Contractor is non-responsible as a bidder for a Department contract.

1.20-1.08.02—Facilities Construction - Establishment of Construction Field Office: Prior to the start of Project construction, and within 10 calendar days after the signing of the Contract by the parties, the Contractor shall propose in writing to the Engineer a field office location. The proposal shall include the office telephone number to be used, the nearest utility pole number, and the distance from that pole to the proposed field office. The office shall be made acceptable to the Engineer and available for use, including all utility hookups, local permits and inspections, within 30 days of the Engineer's order to establish the office. Such order shall not be deemed the "Notice to Proceed."

The Contractor shall purchase one set of all building and fire codes listed in 1.20-1.02.04 for the Engineer's use. If any codes are not readily available, the Contractor shall provide the Engineer with a copy of the code order form along with an anticipated delivery date. At the end of the Project, the codes will remain the property of the Engineer.

1.20-1.08.03—Facilities Construction - Prosecution of Work:

1. General: The Contractor shall commence construction operations with that part of the Project designated for such commencement in the progress schedule which it has submitted to the Department, unless the Engineer directs the Contractor to commence with a different part of the Project. The work shall be conducted in such manner and with sufficient materials, equipment and labor as are necessary to ensure completion of the Project in accordance with the Contract within the time set forth in the Contract. The Contractor shall notify the Engineer of its intention to commence or recommence any Project operation at least 48 hours in advance of doing so. The Contractor shall also give the Engineer such advance notice of any intent to discontinue any Project operation, unless emergency conditions make it impracticable to give such notice so far in advance. The Engineer retains the right to disallow such commencement, recommencement or discontinuance of operations.

2. Permanent Utilities: The Contractor shall place all permanent utility services in its name until the requirements of 1.20-1.08.13-1 are met.

3. Temporary Utilities, Services, and Facilities: All utility usage charges for the Project site for Project construction are the responsibility of the Contractor except as may be provided by the Contract. The Contractor shall place all temporary utility services in its name. Installation or use charges for temporary facilities are not chargeable to the State, and may not be used as a basis for construction orders.

The Contractor shall:

(a) Submit to the Engineer a schedule indicating the Contractor's plan for implementation and termination of each temporary utility within 21 calendar days of the Notice to Proceed.

(b) Obtain required certifications and permits for temporary utilities and submit copies of same to the Engineer as soon as each is obtained.

(c) Arrange for authorities having relevant jurisdiction to inspect and test each temporary utility before use, and after any relocation of same.

(d) Use qualified personnel for installation of temporary facilities, including subsequent relocations.

(e) Install such facilities in locations where they will serve the Project adequately and result in minimum interference with performance of the Project.

(f) Engage the appropriate utility company to install temporary service or connect to existing service. If such company provides only part of the service, the Contractor shall provide the remainder with matching, compatible materials and equipment and shall comply with the company recommendations and arrange with the company and the Engineer for a time when service may be interrupted, if necessary, to make connections for temporary services.

(g) Provide adequate utility capacity at each stage of Project construction.

(h) Prior to temporary utility availability, the Contractor shall provide trucked-in services. The Contractor shall obtain easements to bring temporary utilities to the site, where easements cannot be used for that purpose.

(i) Provide weatherproof, grounded electric power service and distribution system of sufficient size, capacity, and power characteristics during Project construction. The Contractor shall include meters,

transformers, overload-protected disconnects, automatic ground-fault interrupters and main distribution switch-gear and shall install underground electric power service, except where overhead service must be used, or the Engineer directs it.

Whenever the Contractor installs an overhead floor or roof deck, the Contractor shall provide temporary lighting with local switching. The Contractor shall provide temporary lighting that will fulfill security and protection requirements, that will be adequate for construction operations and traffic conditions, and that will render signs on the Project site visible when Project work is being performed for the duration of the Project.

The Contractor shall provide temporary heat required for curing or drying activities, for protection of installed construction from adverse effects of low temperatures or high humidity, or for heating of interior building areas. The Contractor shall use safe equipment that will not have a harmful effect on elements being installed or on completed installations. The Contractor shall coordinate ventilation and temporary heating so as to produce the ambient condition required and to minimize consumption of energy. All temporary heating must comply with OSHA regulations and other applicable codes, statutes, rules and regulations. The Contractor shall bear the costs related to furnishing temporary heat as herein required, including the cost of energy.

Except when use of the permanent heating system is authorized by the Engineer, the Contractor shall provide vented, self-contained LP-gas or fuel oil heaters with individual-space thermostatic control. Use of gasoline-burning space heaters, open flame, or salamander type heating units are prohibited.

The Contractor shall install water service and distribution piping of sizes and pressures adequate for Project construction until permanent water service is in use. The Contractor shall sterilize temporary water piping prior to use.

The Contractor shall collect waste daily from the Project Site. The Contractor shall comply with requirements of NFPA 241 for removal of combustible waste material and debris. The Contractor shall not hold such materials more than 7 calendar days during normal weather or 3 calendar days when the temperature is expected to rise above 80°F. The Contractor shall handle hazardous, dangerous, or unsanitary waste materials separately from other waste by placing them in proper containers. The Contractor shall dispose of material in a lawful manner.

The Contractor shall remove each temporary facility as authorized by the Engineer. Materials and facilities that constitute temporary facilities are the Contractor's property, unless otherwise noted in the Contract.

4. Cutting and Patching:

A. Approval Process: Well in advance of performing any cutting and patching on the Project, the Contractor shall submit to the Engineer a proposal describing the procedures that the Contractor intends to use for same.

The Contractor shall include the following information, as applicable, in the proposal:

(1) Description of the extent of cutting and patching required, how it will be performed, and an indication as to why it cannot be avoided;
(2) Changes in structural elements, operating components, and the building's appearance and other significant visual elements;
(3) List of products to be used and firms or entities that will perform Project work;
(4) Dates when cutting and patching are to be performed;
(5) List of utilities that cutting and patching procedures will affect, list of utilities that will be relocated, and list of utilities that will be temporarily rendered out of service (including duration);
(6) Where cutting and patching involves adding reinforcement to structural elements and is required due to the fault of the Contractor, details and engineering calculations prepared by a Professional Engineer registered in the State of Connecticut to show integration of reinforcement with the original structure; if such is not due to fault of the Contractor, the Designer shall supply details to show integration of reinforcement with the original structure.

Approval by the Engineer to proceed with cutting and patching does not waive the Engineer's right to later require complete removal and replacement of unsatisfactory work.

B. Protection:

(1) Structural Elements: The Contractor shall obtain approval of the cutting and patching proposal before cutting and patching any structural element, including but not limited to structural concrete, structural steel, timber and primary wood framing, and structural decking.

The Contractor shall not cut and patch structural elements in a manner that would reduce their

load-carrying capacity or load-deflection ratio; or cut and patch operating elements or related components in a manner that would reduce their capacity to perform as intended, increase maintenance needs, or decrease operational life or safety.

(2) Operational Elements: The Contractor shall not cut and patch operating elements and related components in a manner that results in their reducing their capacity to perform as intended or that results in increased maintenance or decreased operational life or safety.

(3) Other Construction Elements: The Contractor shall not cut and patch miscellaneous elements or related components in a manner that could change their load-carrying capacity, that results reducing their capacity to perform as intended, or that results in increased maintenance or decreased operational life or safety.

C. Protection of Visual Elements: The Contractor shall not cut and patch construction exposed on the exterior or in occupied spaces in a manner that would, in the Engineer's opinion, reduce the building's aesthetic qualities; or cut and patch construction in a manner that would result in visual evidence of cutting and patching. The Contractor shall remove and replace any such unsatisfactory work.

If possible, the Contractor shall retain the original installer or fabricator to cut and patch exposed work. If it is impossible to engage the original installer or fabricator, the Contractor shall engage another firm acceptable to the Engineer.

D. Warranty: When replacing, patching, or repairing material or surfaces that have been cut or damaged, the Contractor shall use methods and materials in such a manner as not to void any required or existing warranties.

E. Materials: To the extent possible, the Contractor shall use materials identical to existing materials. For exposed surfaces, the Contractor shall use materials that visually match adjacent surfaces to the fullest extent possible. The Contractor shall use materials whose performance will equal or surpass that of existing materials.

F. Coordination: Before proceeding with any cutting and patching, the Contractor shall meet at the Project site with parties that will be involved in that work, including the Engineer and mechanical and electrical subcontractors, to review and resolve areas of potential interference and conflicts.

G. Preparation: The Contractor shall

(1) provide temporary support of work to be cut;

(2) protect existing construction during cutting and patching;

(3) protect such construction from adverse weather conditions where it may be exposed during cutting and patching operations; and

(4) avoid interference with use of adjoining areas or interruption of free passage to adjoining areas.

The Contractor shall avoid cutting existing pipe, conduit, or ductwork which serves the building, but which is scheduled to be removed or relocated, until adequate to bypass, replace, or discontinue those services, as applicable.

H. Performance: The Contractor shall:

(1) employ skilled workers to perform cutting and patching;

(2) proceed with cutting and patching at the earliest feasible time, and complete the work without delay;

(3) cut existing construction to provide for installation of other components or performance of other construction activities and the subsequent fitting and patching required for restoring surfaces to their original condition;

(4) cut existing construction using methods least likely to damage elements retained or adjoining construction; and

(5) review proposed procedures with the original installer and comply with the original installer's recommendations, if possible.

In general, for cutting and patching the Contractor shall:

(1) use hand or small power tools designed for sawing or grinding, not for hammering and chopping;

(2) cut holes and slots neatly to the size required, and with minimum disturbance of adjacent surfaces;

(3) temporarily cover openings when not in use;

(4) cut or drill from the exposed or finished side into concealed surfaces to avoid marring existing finished surfaces;

(5) cut through concrete and masonry using a cutting machine such as a carborundum-saw or diamond-core drill; and

(6) review any Contract provisions applicable to cutting and patching which requires excavating and backfilling.

Where services are required to be removed, relocated or abandoned, the Contractor shall:
(1) by-pass utility services such as pipe or conduit, before cutting;
(2) cut-off pipe or conduit in walls or partitions to be removed; and
(3) cap, valve, or plug and seal the remaining portion of pipe or conduit to prevent entrance of moisture or other foreign matter after bypassing and cutting.

The Contractor shall:
(1) patch with durable seams that are as invisible as possible;
(2) comply with specified tolerances;
(3) inspect patched areas to ensure integrity of the installation where feasible; and
(4) restore exposed finishes of patched areas and extend finish restoration into retained adjoining construction in a manner that will eliminate evidence of patching and refinishing.

Where removal of walls or partitions extends one finished area into another, the Contractor shall:
(1) patch and repair floor and wall surfaces in the new space;
(2) provide an even surface of uniform color and appearance;
(3) remove existing floor and wall coverings and replace with new materials, if necessary to achieve uniform color and appearance; and
(4) patch, repair or re-hang existing ceilings as necessary to provide an even plane surface of uniform appearance.

Where patching occurs in a smooth painted surface, the Contractor shall extend final paint coat over entire unbroken surface containing the patch, after the patched area has received primer and second coat.

I. Cleaning: The Contractor shall:
(1) clean areas and spaces where cutting and patching are performed or used as access;
(2) remove paint, mortar, oils, putty and similar items;
(3) clean piping, conduit, and similar features before applying paint or other finishing materials; and
(4) restore damaged pipe covering to its original condition.

5. Selective Demolition:

A. Definitions:

Remove: The Contractor shall detach materials from existing construction and legally dispose or recycle them off-site, unless indicated to be removed and salvaged or removed and reinstalled. Except for materials indicated to be reused, salvaged, reinstalled, or otherwise indicated to remain Engineer's property, demolished materials shall become Contractor's property and shall be removed from the Project Site.

Remove and Salvage: The Contractor shall detach materials from existing construction and deliver them to Engineer. The Engineer reserves the right to identify other materials for salvage during the course of demolition.

Remove and Reinstall: The Contractor shall detach materials from existing construction, prepare them for reuse, and reinstall them where indicated.

Existing to Remain: Existing materials of construction that are not to be removed and that are not otherwise indicated to be removed, removed and salvaged, or removed and reinstalled.

B. Approval Process:

The Contractor shall submit pre-demolition photographs to the Engineer prior to the commencement of Project work to show existing conditions of adjoining construction and site improvements, including finish surfaces, that might be misconstrued as damage caused by selective demolition operations.

Well in advance of performing any selective demolition on the Project, the Contractor shall submit to the Engineer a proposal describing the procedures that the Contractor intends to use for same.

The Contractor shall include the following information, as applicable, in its proposal:
(1) detailed sequence of selective demolition and removal work with starting and ending dates for each activity while ensuring that the Engineer's on-site operations are not disrupted;
(2) interruption of utility services;
(3) coordination for shutoff, capping, and continuation of utility services;
(4) use of elevators and stairs;
(5) locations of temporary partitions and means of egress;
(6) coordination of Engineer's continuing occupancy of portions of existing building and of Engineer's partial occupancy of completed Project work; and
(7) means of protection for items to remain and items in path of waste removal from building.

The Contractor shall comply with
(1) governing EPA notification regulations before beginning selective demolition;

(2) hauling and disposal regulations of authorities having jurisdiction;
(3) ANSI A10.6; and
(4) NFPA 241.

The Engineer will conduct a Pre-Demolition Meeting at the Project site in accordance with 1.20-1.05.24. Said meeting will review the methods and procedures related to selective demolition including, but not limited to, the following:

(1) an inspection and discussion of the condition of construction to be selectively demolished;
(2) a review of the structural load limitations of the existing structure;
(3) a review and finalization of the selective demolition schedule and a verification of the availability of materials, demolition personnel, equipment, and facilities needed to make progress and avoid delays;
(4) a review of requirements of Project work performed by other trades that rely on substrates exposed by selective demolition operations; and
(5) a review of areas where existing construction is to remain and requires protection.

C. Repair Materials:

The Contractor shall comply with 1.20-1.08.03-4E for repair materials and shall comply with material and installation requirements specified in other Contract provisions.

D. Examination:

The Contractor shall:

(1) verify that utilities have been disconnected and capped;
(2) survey existing conditions and correlate with requirements indicated to determine extent of selective demolition required;
(3) inventory and record the condition of items to be removed and reinstalled and items to be removed and salvaged;
(4) investigate and measure the nature and extent of unanticipated mechanical, electrical, or structural elements that conflict with intended function or design and submit a written report to Engineer; and
(5) perform surveys as the Project work progresses to detect hazards resulting from selective demolition activities.

E. Utility Services:

The Contractor shall:

(1) maintain existing utility services indicated to remain and protect them against damage during selective demolition operations;
(2) not interrupt existing utilities serving occupied or operating facilities unless authorized in writing by the Engineer;
(3) provide temporary services during interruptions to existing utilities, as acceptable to Engineer;
(4) provide at least 3 calendar days' notice to the Engineer if shutdown of service is required during changeover; and
(5) locate, identify, disconnect, and seal or cap off indicated utilities serving areas to be selectively demolished.

The Contractor shall arrange to shut off indicated utilities with utility companies. If utility services are required to be removed, relocated, or abandoned, before proceeding with selective demolition the Contractor shall provide temporary utilities that bypass area of selective demolition and that maintain continuity of service to other parts of building. The Contractor shall cut off pipe or conduit in walls or partitions to be removed and shall cap, valve, or plug and seal remaining portion of pipe or conduit after bypassing.

The Contractor shall refer to other Contract provisions for shutting off, disconnecting, removing, and sealing or capping utilities. The Contractor shall not start selective demolition work until utility disconnecting and sealing have been completed and verified by the Engineer in writing.

F. Preparation:

The Contractor shall conduct selective demolition and debris-removal operations to ensure minimum interference with adjacent occupied and used facilities on the Project site. The Contractor shall not disrupt the Owner's operations without the Engineer's permission. The Contractor shall protect existing site improvements, appurtenances, and landscaping to remain.

The Contractor shall provide temporary barricades and other protection required to prevent injury to people and damage to adjacent buildings and facilities to remain. The Contractor shall provide temporary weather protection, during interval between selective demolition of existing construction on exterior surfaces and new construction, to prevent water leakage and damage to structure and interior areas. The

Contractor shall protect walls, ceilings, floors, and other existing finish work that are to remain or that are exposed during selective demolition operations. The Contractor shall cover and protect furniture, furnishings, and equipment that have not been removed.

The Contractor shall provide temporary enclosures for protection of existing building and construction, in progress and completed, from exposure, foul weather, other construction operations, and similar activities. The Contractor shall provide temporary weathertight enclosure for building exterior. Where heating is needed and permanent enclosure is not complete, the Contractor shall provide insulated temporary enclosures and shall coordinate enclosure with ventilating and material drying or curing requirements to avoid dangerous conditions and effects.

The Contractor shall erect and maintain dustproof partitions and temporary enclosures to limit dust and dirt migration and to separate areas from fumes and noise.

The Contractor shall provide and maintain interior and exterior shoring, bracing, or structural support to preserve stability and prevent movement, settlement, or collapse of construction to remain, and to prevent unexpected or uncontrolled movement or collapse of construction being demolished. The Contractor shall strengthen or add new supports when required during progress of selective demolition.

G. Pollution Controls:

The Contractor shall comply with governing regulations pertaining to environmental protection. The Contractor shall not use water when it may create a hazardous or objectionable condition such as ice, flooding, or pollution.

The Contractor shall remove and transport debris in a manner that will prevent spillage on adjacent surfaces and areas. The Contractor shall remove debris from elevated portions of building by chute, hoist, or other device that will convey debris to grade level in a controlled descent.

The Contractor shall clean adjacent structures and improvements of dust, dirt, and debris caused by selective demolition operations. The Contractor shall return adjacent areas to condition existing before selective demolition operations began.

H. Performance:

The Contractor shall not use explosives for demolition purposes.

The Contractor shall demolish and remove existing construction only to the extent required by new construction and as indicated. The Contractor shall:

(1) proceed with selective demolition systematically;
(2) neatly cut openings and holes plumb, square, and true to dimensions required;
(3) use cutting methods least likely to damage remaining or adjoining construction;
(4) use hand tools or small power tools designed for sawing or grinding, not hammering and chopping, to minimize disturbance of adjacent surfaces;
(5) temporarily cover openings to remain;
(6) cut or drill from the exposed or finished side into concealed surfaces to avoid marring existing finished surfaces;
(7) not use cutting torches until work area is cleared of flammable materials;
(8) verify condition and contents of concealed spaces such as duct and pipe interiors before starting flame-cutting operations;
(9) maintain fire watch and portable fire-suppression devices during flame-cutting operations;
(10) maintain adequate ventilation when using cutting torches;
(11) remove decayed, vermin-infested, or otherwise dangerous or unsuitable materials and promptly dispose of off-site;
(12) remove structural framing members and lower to ground by method suitable to avoid free fall and to prevent ground impact or dust generation;
(13) locate selective demolition equipment and remove debris and materials so as not to impose excessive loads on supporting walls, floors, or framing; and
(14) dispose of demolished items and materials promptly.

The Contractor shall comply with the Engineer's requirements for using and protecting walkways, building entries, and other building facilities during selective demolition operations.

The Contractor shall demolish and remove foundations and other below grade structures completely unless otherwise indicated on the plans. The Contractor shall fill below grade areas and voids resulting from demolition of structures with granular fill materials. Prior to placement of fill materials, the Contractor shall ensure that the areas to be filled are free of standing water, frost, frozen material, trash, and debris. After fill placement and compaction, grade surface to meet adjacent contours and provide flow to

surface drainage structures. Backfilling and grading related to demolition is included in the Major Lump Sum Item (MLSI) for the Project. There will be no separate payment for this backfilling and grading.

The Contractor shall (1) demolish concrete in sections; (2) cut concrete at junctures with construction to remain to the depth shown on the Contract plans and at regular intervals using power-driven saw; and (3) remove concrete between saw cuts.

The Contractor shall:

(1) demolish masonry in small sections;
(2) cut masonry at junctures with construction to remain using power-driven saw; and
(3) remove masonry between saw cuts.

The Contractor shall:

(1) saw-cut perimeter of concrete slabs-on-grade to be demolished as shown on the Contract plans; and
(2) break up and remove concrete slabs-on-grade.

The Contractor shall;

(1) remove floor coverings and adhesive according to recommendations in RFCI-WP and its Addendum; and
(2) remove residual adhesive and prepare substrate for new floor coverings by one of the methods recommended by RFCI.

The Contractor shall:

(1) only remove existing roofing in one day to the extent that it can be covered by new roofing; and
(2) refer to other Contract provisions for new roofing requirements.

The Contractor shall remove air conditioning equipment without releasing refrigerants.

I. Reuse of Building Elements:

The Contractor shall not demolish building elements beyond what is indicated on the plans without the Engineer's approval.

J. Removed and Salvaged Materials:

Unless otherwise directed by the Engineer, the Contractor shall:

(1) store materials in a secure area until delivery to the Owner;
(2) transport materials to the Owner's storage area off-site; and
(3) protect materials from damage during transport and storage.

K. Removed and Reinstalled Materials:

Unless otherwise directed by the Engineer, the Contractor shall:

(1) clean and repair materials to functional condition adequate for intended reuse;
(2) paint equipment to match the color of new equipment;
(3) protect materials from damage during transport and storage; and
(4) reinstall items in locations indicated complying with installation requirements for new materials and equipment and providing connections, supports, and miscellaneous materials necessary to make item functional for use indicated.

L. Existing Materials to Remain:

The Contractor shall protect construction indicated to remain against damage and soiling during selective demolition.

The Contractor shall drain piping and cap or plug piping with the same or a compatible piping material for piping to be abandoned in place.

The Contractor shall cap or plug ducts with the same or a compatible ductwork material for ducts to be abandoned in place.

The Contractor shall cut and remove concealed conduits and wiring to be abandoned in place 2 inches below the surface of the adjacent construction, cap the conduit end, and patch the surface to match the existing finish. The Contractor shall cut existing conduits installed in concrete slabs to be abandoned in place flush with the top of the slab and fill conduit end with a minimum of 4 inches of concrete.

M. Patching and Repairing:

The Contractor shall comply with 1.20-1.08.03-4H for patching and repairing damage to adjacent construction caused by selective demolition operations.

N. Disposal of Demolished Materials:

The Contractor shall:

(1) not allow demolished materials to accumulate or be sold on the Project Site;
(2) not burn demolished materials on the Project Site; and
(3) promptly and legally dispose or recycle demolished materials off the Project Site.

1.20-1.08.04—Facilities Construction - Limitation of Operations: The Contractor shall plan and perform the Project work in such a manner and in such sequence as will cause as little interference as is practicable with vehicular, railroad, aircraft, pedestrian or other traffic. The Contractor shall cooperate with any utilities involved in or affected by the Project operations, and shall schedule its operations in accordance with 1.20-1.05.06.

The Contractor shall give the Engineer 7 days' advance written notice of any proposed changes in Project activities that will alter vehicular traffic patterns, causing lane shifts, detours, temporary closure of a lane, permanent closure of a lane or lane reductions, or any other alteration of railroad, aircraft, pedestrian or other traffic patterns affecting usage of such a transportation facility by the traveling public. This advance notification will allow the Department to publish news releases and provide public radio announcements to inform the public of revised traffic patterns or possible traffic delays. Failure of the Contractor to provide such timely notice will subject the Contractor to stop work orders until such time as the 7 days of required notice have run from either the Contractor's giving of the relevant notice or the Department's discovery of the pertinent alteration of traffic conditions.

1.20-1.08.05—Facilities Construction - Personnel and Equipment: The Contractor shall assign to the Project only personnel who are careful and competent. The Engineer may demand the removal of any person employed by the Contractor on the Project who is, in the opinion of the Engineer, guilty of misconduct on a Department project or incompetent or negligent in the performance of his duties on a Department project or any portion thereof, or who neglects or refuses to comply with directions given by the Engineer in connection with the Project. Following such a demand for his removal, such person shall not work again on the Project without the prior written consent of the Engineer. Should the Contractor, following such a demand for removal, continue to employ or again employ such person on any Department project without the required consent of the Engineer, the Commissioner may withhold all estimated payments that are or may become due to the Contractor for the Project, or the Engineer may shut down the Project until the Contractor has complied with the Engineer's orders concerning that person. The use of convict labor on projects funded in whole or in part by the federal government is prohibited.

The Contractor shall furnish whatever equipment is necessary for the Project to be performed in a manner and at a rate of progress that is acceptable to the Engineer. Equipment used on any portion of the Project shall not be used in any way that may cause injury to the roadway, adjacent property, or other property on or adjacent to the Project Site, unless such damage is allowed by the Engineer for the performance of the Project.

The Contractor may submit to the Engineer a request to use equipment or methods other than those specified in the Contract. If the Engineer so directs, there shall be a trial of such equipment or methods. If the results of the trial are satisfactory to the Engineer, the Contractor may begin using the proposed equipment or method on the Project. Failure of the equipment or method to meet the specified Contract performance standards in the course of the specified trial, or, in the absence of such standards, a failure to perform to the satisfaction of the Engineer, shall be cause for rejection of any such method or equipment, and any work performed with either. Such rejected equipment or work shall be removed immediately from the Project site.

The Contractor shall provide:

(a) Temporary heating units that have been tested and labeled by UL, FMG or another recognized trade association related to the type of fuel being consumed.

(b) Hand-carried, portable, UL-rated, Class ABC, dry-chemical extinguishers or a combination of extinguishers of NFPA recommended classes that comply with NFPA 10 and 241 for classification, extinguishing agent, and size required by location and class of fire exposure.

(c) The Contractor shall install a filter with a minimum MERV rating of 10 at each return air grille in the permanent HVAC system if the Engineer authorizes the use of the permanent HVAC system for temporary use during construction. The Contractor shall remove the filters prior to the Semi-Final Inspection.

1.20-1.08.06—Facilities Construction - Suspensions of Work Ordered by the Engineer: The Engineer may suspend the Project wholly or in part, for such period or periods as he considers to be in the best interests of the State, including, but not limited to, the interests of public necessity, convenience, or safety.

During such periods of suspension, and subject to any related directions from the Engineer, the Contractor shall store all materials and equipment in a way that will keep them from obstructing or impeding the

traveling public unnecessarily, and that will keep the materials from being damaged; shall take all reasonable measures to prevent damage to the work performed; shall provide suitable drainage of the roadway and Project site by opening ditches, shoulder drains, etc., as appropriate; and shall erect temporary structures to prevent damage to the Project or to other property, and to protect the public, where and when necessary.

The Contractor shall maintain the Project site and all roadways and buildings thereon in a condition safe for travel or occupancy, and shall maintain all required barricades, signs, and lights during the period of suspension:

(1) If the Engineer orders in writing that performance of all or any portion of the Project shall be suspended, or that it shall be delayed for an unreasonable period of time (not customary, within the scope of possibilities that an experienced contractor should know might occur on a construction project, or inherent in the nature of construction activities), and if the Contractor believes that additional compensation or Contract time is due to it as a result of such suspension or delay, the Contractor shall submit to the Engineer in writing a request for a related Contract adjustment within 7 calendar days of the Contractor's receipt of a direction from the Engineer to resume work. The request shall set forth the specific reasons and support for the requested adjustment.

(2) Upon receipt of the Contractor's request, the Engineer will evaluate the request. If the Engineer agrees that the expenditures or time required for the Contractor's performance of the Contract have increased as a result of such suspension or delay, and if the suspension or delay was caused by conditions beyond the control of and not the fault of the Contractor, its suppliers, or subcontractors at any approved tier, and was not caused by weather, the Engineer will make an appropriate adjustment (excluding any profit) of the written terms of the Contract. The Engineer will give the Contractor written notice of his determination as to whether or not the requested adjustment of the Contract is warranted and will be made.

(3) No Contract adjustment will be made unless the Contractor has submitted the request for adjustment within the time prescribed.

(4) In addition to the other limits and requirements imposed by this article, no Contract adjustment will be allowed under this article to the extent

  (a) that the Project work would have been suspended or delayed by any cause other than the ones identified in the Contractor's request, or

  (b) that the requested adjustment or type of adjustment is provided for or barred by another provision of the Contract.

(With regard to Items 1-4, refer to 23 CFR, Section 635.109, "Standardized Changed Condition Clauses," Required FHWA Contract Specification.)

1.20-1.08.07—Facilities Construction - Determination of Contract Time: Unless the Contract requires the Project completion by a specified date, the number of calendar days allowed for the completion of the Project will be fixed by the Department, will be stated in the Contract, and will be known (with any subsequent adjustments) as the "Contract time." If at any time the Contractor submits a schedule showing completion of the work more than 30 calendar days in advance of the Contract completion date, the Department will issue a no-cost construction order revising the allowable Contract time to that shown on the Contractor's schedule.

When the Contract time is stated on a calendar-day basis, that time shall be the number of consecutive calendar days contained in the Contract period designated in the Contract, INCLUDING the time period from each December 1 through the following March 31. The Contract time will begin to run on the date designated in the Engineer's "Notice to Proceed" as the date for commencement of the Project, and the time will be computed as herein provided on a consecutive-day basis, including all Saturdays, Sundays, holidays, and non-work days.

The total elapsed time in calendar days, computed as described above, from the commencement date specified in the Engineer's "Notice to Proceed" to the "Substantial Completion" date specified in the Engineer's "Notice of Substantial Completion" shall be considered as the time used in the performance of the Contract work.

Suspension involving cessation of work on all items, except minor construction not affected by or connected with the cause of suspension, shall be considered as total suspension. In case of a total suspension of the Project ordered by the Engineer, not due to any fault of the Contractor, the elapsed time during which the Project is suspended will not be charged against the Contract time. Work of an

emergency nature ordered by the Engineer for the convenience or safety of the public or the protection of the Project work, if performed during a period of total suspension, will not be charged against Contract time. No such time allowance will be granted in case of partial suspension; provided, however, that the Contractor may request and the Engineer may grant permission to perform specific limited operations during such a partial suspension, in which case Contract time chargeable for those operations shall be negotiated and agreed to in writing before such operations may commence.

1.20-1.08.08—Facilities Construction - Extension of Time: The Contractor may present to the Engineer a request in writing for an extension of Contract time if the time necessary for completion of the Project has been increased due to extra or added work or delays resulting from unforeseeable causes beyond the control and without the fault or negligence of the Contractor, except for weather or seasonal conditions (unless extraordinary and catastrophic). Such causes include, but are not limited to, natural catastrophes, acts of the State in either its sovereign or contractual capacity, acts of another contractor in the performance of a contract with the State, the presence of utility facilities (including railroads), fires, strikes, floods, or delays by suppliers arising from unforeseeable causes beyond the control and without the fault or negligence of either the Contractor or such suppliers.

The Contractor's plea that insufficient Contract time was allowed under the Contract before commencement of the Project is not a valid reason for extending the Contract time. Requests for an extension of time, with adequate substantiation, must be presented within 60 calendar days from the event that is the basis of the request or from the first effect of such an event on the Project. The Contractor will be responsible for providing all the documentation necessary to support the reasonableness of the additional time requested. This shall include a Critical Path Method Schedule Analysis and accompanying narrative that includes the specific dates and number of days for which the extension is sought, the basis or bases for the extension, and the schedule analysis illustrated in a graphic representation of the schedule impacts such as a bar chart or other type of graphical schedule. The critical path is a sequence of activities in a project wherein none of the activities can be delayed without affecting the final project end date.

Such requests will be considered by the Engineer and granted to the extent that he deems to be fair and reasonable. Requests will not be considered if based on delays caused solely by conditions existing at the time the bids were received and of which the Contractor might reasonably be expected to have had full knowledge at the time, or upon delays caused by failure on the part of the Contractor to anticipate properly the requirements of the Project as to materials, labor, or equipment. For all Project delays or time increases, except as provided below, additional Contract time is the sole remedy that the Contractor may have, and such periods of additional Contract time shall be deemed "Non-Compensable Delays." For delays caused by the State in its Contractual capacity, the Contractor may, in addition to a time extension, request additional compensation to reimburse it for damages sustained as a direct result of such delay, and such periods of extended Contract time may be deemed "Compensable Delays."

The period of a compensable delay is limited as follows:

(1) It may not include time more than 60 days prior to the Engineer’s receiving written notice from the Contractor with adequate substantiation, of its intent to claim damages for the delay, and

(2) It may not include periods of delay for which the State was responsible, but during which the Contractor experienced concurrent delays for which the State was not responsible.

Damages for periods of Project delay for which the State had sole responsibility shall be limited to the increased costs incurred by the Contractor (which shall not include lost profits), which the Contractor substantiates and which the Contractor shows were caused by such delays.

The Critical Path Method Schedule Analysis shall include at a minimum:

1. The manner in which the Contractor planned to construct the Project, in terms of activities, logical interrelationships of activities, work sequences, activity durations, and calendars.
2. The actual duration and sequences of the activities, based on what actually occurred on the Project.
3. The variances between the planned and actual performance of the work, listed in a chronological and cumulative manner, summing to the net total delay on the Project at the time of the request.
    a. The causes of the variances between the planned and actual performance of the work, specifically allocating legal responsibility for each to either the Department or the Contractor.
    b. The effects of the variances in work sequences, activity durations and Resources on the incurred costs of the affected party or parties.

4. An identification analysis of the causes of any concurrent delays on the Project.
5. Statements as to whether the time extension days sought are compensable or non-compensable, along with a specific statement of any compensation requested in connection with the time extension. Any request for a time extension that does not include a corresponding request for compensation will be assumed to be a request for a non-compensable time extension.
6. All associated analysis documents, worksheets, schedules and contemporaneous documents supporting the Critical Path Method Schedule Delay Analysis.

1.20-1.08.09—Facilities Construction - Failure to Complete Work on Time: Time is an essential element of the Contract. Since the prosecution of the Project may obstruct traffic, interfere with business, and otherwise inconvenience the public, it is important that the Project be pressed vigorously to completion. The cost to the Department of the administration of the Contract, including engineering, inspection and supervision, will also be increased as the time for Project completion is lengthened. Therefore, for each calendar day that any work shall remain uncompleted after the Contract time has expired, the per diem sum of liquidated damages specified in the Contract shall be deducted from any money due to the Contractor. Liquidated damages are not a penalty, but are a reasonable estimate of the damages caused by such delay.

Liquidated damages as specified in the Contract shall be assessed against the Contractor per calendar day from that day until the date on which the Project is substantially completed.

The Engineer has the right to deduct the amount of the liquidated damages assessed against the Contractor from any estimated payment for work performed under the Contract or under any other State contract, or from any other sums owed by the State to the Contractor; or to claim and recover such sums by process of law.

1.20-1.08.10—Facilities Construction - Termination of Contract for Cause: The Commissioner may give notice in writing to the Contractor and its surety of any delay, neglect, or default of the Contractor which the Commissioner believes has occurred, including one or more of the following:

1. Failure to begin the Project on the date specified in the Notice to Proceed.
2. Failure to perform the Project with sufficient personnel, equipment or materials to ensure timely Project completion.
3. Unsuitable performance of the Project or failure to perform Project work in accordance with the Contract.
4. Failure or refusal to remove or correct work rejected by the Engineer.
5. Discontinuance of suitable prosecution of the Project for a period of 72 hours, excluding Sundays and holidays, without written authorization to do so from the Engineer.
6. Failure to recommence discontinued work within 48 hours (excluding Sundays and holidays) after being ordered to do so by the Engineer.
7. Insolvency, filing for bankruptcy, or any act or occurrence which may render the Contractor financially incapable of completing the Project.
8. Failure to satisfy any final judgment for a period of 30 calendar days.
9. Making of any assignment for the benefit of creditors.
10. Violation of any provisions of the Contract.
11. Any other cause which, in the judgment of the Commissioner, warrants termination, including, but not limited to, violations of the antitrust or criminal laws, and attempts to deceive or defraud the Department in material matters.

If the Contractor or surety within a period of 10 calendar days after such notice does not proceed in conformance with the directions set forth in the notification, or fails to present a remedial plan of operation satisfactory to the Commissioner, then the Commissioner may, at his discretion, order the surety to complete the Project or, without violating the Contract, take the right to control and prosecute the Project out of the hands of said Contractor and surety. No termination of the Contract for such cause will be deemed to have occurred, however, unless the Commissioner himself or herself (and not merely a designated representative of his or hers) expressly declares it in a writing to the Contractor.

The Department may acquire or rent whatever materials or equipment are necessary in order to complete the Project and may seize and use for purposes of the Project (with any appropriate compensation to the Contractor) any material or equipment that the Contractor acquired or purchased expressly for the Project in accordance with a specific Contract requirement.

The Department may also enter into an agreement, either by negotiation or public letting, for the

completion of the Contract according to the terms and provisions thereof, or use such other methods or combinations thereof as in the Commissioner's opinion shall be required or desirable for the completion of the Contract in an acceptable manner. All costs and charges incurred by the Department, in connection with completing the Project under the Contract, or as a result of the Contractor's default, shall be deducted from any monies due to or which may become due to the Contractor. In case such expense exceeds the sum which would have been payable under the Contract, then the Contractor and the surety shall be liable for, and shall pay to the State, the amount of the excess.

1.20-1.08.11—Facilities Construction - Final Cleaning Up: The Project will not be considered complete and will not be accepted until the rights of way, borrow pits, and all other ground, both public and private, occupied by the Contractor in connection with the Project has been cleared of all surplus and discarded materials, rubbish and temporary structures. The Contractor must drain all borrow pits where practicable. All property, both public and private, which has been damaged during the prosecution of the Project, shall be restored by the Contractor to an appearance and condition acceptable to the Engineer.

All ditches, waterways, drainage structures and culverts constructed under the Contract shall be cleaned and cleared of obstructions by the Contractor, and shall be left in a condition acceptable to the Engineer. When so directed by the Engineer, the Contractor shall clean all existing ditches, waterways, drainage structures and culverts of obstructions resulting from Project operations.

The Contractor shall:

(a) Clean each surface or unit to the satisfaction of the Engineer.
(b) Comply with all applicable manufacturer's recommendations for cleaning products and methods.
(c) Complete the following cleaning operations before requesting Substantial Completion Inspection for issuance of the Certificate of Compliance: remove labels that are not permanent labels; clean transparent materials, including mirrors and glass in doors and windows; remove glazing compound and other substances that are noticeable vision-obscuring materials; replace chipped or broken glass and other damaged transparent materials; clean exposed exterior and interior hard-surfaced finishes to a dust-free condition, free of stains, films and similar foreign substances; restore reflective surfaces to their original reflective condition; leave concrete floors broom-clean; vacuum carpeted surfaces; wipe surfaces of mechanical and electrical equipment; remove excess lubrication and other substances; clean plumbing fixtures to a sanitary condition; clean light fixtures and lamps; clean the site, including landscape development areas, of rubbish, litter and other foreign substances; sweep paved areas broom-clean; remove stains, spills and other foreign deposits; and rake unpaved and unplanted grounds to a smooth even-textured surface.
(d) Engage a licensed exterminator to conduct an inspection and rid the Project of rodents, insects, and other pests, as necessary.
(e) Remove temporary facilities installed for protection of the Project work during construction.
(f) The Contractor shall not burn waste materials, bury debris or excess materials on the State's property or discharge volatile, harmful or dangerous materials into drainage systems. The Contractor shall remove waste materials from the site and dispose of lawfully.
(g) The Contractor shall not leave partial or full containers of materials, such as paints and solvents, other than those specified in the Contract, on the Project site. Such materials shall remain property of the Contractor and be removed from State property at the completion of the Project.

1.20-1.08.12—Facilities Construction - Semi-Final, Substantial Completion, and Final Inspections:

1. Semi-Final Inspection: If the Contractor has installed the permanent electrical service and completed all physical work, a Semi-Final Inspection will be scheduled as soon as practical after the Contractor submits the following:

(1) Record Drawings, Record Specifications, miscellaneous records, and Contractor Asbestos Certification Letter as referenced in 1.20-1.05.05;
(2) final survey;
(3) preliminary test/adjust/balance records including the air and water balance report;
(4) one draft copy of all Operation and Maintenance Manuals as referenced in 1.20-1.08.14;
(5) a list of all required training along with the entities who will provide the training and estimated time frames for each session;
(6) a list of all spare parts and salvage materials to be turned over to the Owner, and
(7) account numbers and copies of latest bills from each utility.

The Engineer, Designer, Building and Fire Code Officials, and the Owner will conduct an inspection to prepare a "Punch List" of unfulfilled, substandard, or incomplete items. During this inspection, the Contractor shall have all technicians necessary to demonstrate the complete operation of all systems on site. Examples of such systems include, but are not limited to, the following: boiler, HVAC, fire alarm, and building automation. Results of the completed inspection will form the basis of requirements for the Substantial Completion Inspection. The Engineer reserves the right to issue the C.O.C. after the Semi-Final Inspection if the requirements of the Substantial Completion Inspection are met and there are no Building Code or Fire Code compliance issues or any major "Punch List" items that would adversely affect the tenants of the facility after moving in. The Engineer will advise the Contractor of the construction that shall be completed before the issuance of the C.O.C.

2. Substantial Completion Inspection: Before requesting a Substantial Completion Inspection for the issuance of the C.O.C, the Contractor shall complete all items listed on the Engineer's Semi-Final Inspection "Punch List." If the Engineer determines that the "Punch List" is complete, a Substantial Completion Inspection will be scheduled as soon as practical after the Contractor submits the following:

(1) final test/adjust/balance records including the air and water balance report;
(2) final copies of Operation and Maintenance Manuals as referenced in 1.20-1.08.14;
(3) executed warranties as referenced in 1.20-1.06.08;
(4) maintenance service agreements;
(5) final construction photographs;
(6) final meter readings for all utilities;

and the Contractor

(1) completes final cleaning requirements and touch up painting;
(2) delivers all spare parts and salvage materials to the Engineer; and
(3) completes all training of the Owner's personnel.

The Engineer and code officials will conduct the inspection. During this inspection, the Contractor shall have all technicians necessary to demonstrate the complete operation of all systems on-site if requested by the Engineer. The Engineer will advise the Contractor of the construction that is required to be completed.

3. Final Completion Inspection: The Engineer will schedule a Final Completion Inspection 1 year after the issuance of the C.O.C. for "Relief of Responsibility," notwithstanding any warranty obligations, only after the Engineer determines that the Contractor has satisfactorily:

(1) completed follow up door hardware adjusting;
(2) completed subsequent season air and water balancing;
(3) resolved warranty issues;
(4) completed miscellaneous follow up testing; and
(5) completed landscaping requirements.

1.20-1.08.13—Facilities Construction - Termination of the Contractor's Responsibility:

1. General: The Contractor's responsibility for non-administrative Project work will be considered terminated when the final inspection has been held, any required additional work and final cleaning-up have been completed, all final operation and maintenance manuals have been submitted, and all of the Contractor's equipment and construction signs have been removed from the Project site. When these requirements have been met to the satisfaction of the Engineer, the Commissioner will accept the work by certifying in writing to the Contractor that the non-administrative Project work has been completed.

2. Utility Services: At the issuance of the Certificate of Compliance or at an earlier date if directed by the Engineer, the Contractor shall request in writing that permanent utility services be placed in the Department's name. The Contractor's written request shall include the following information: account number, meter number, exact street address, and, if applicable, the Certificate of Compliance date. Within 7 calendar days of the receipt of the Contractor's written request, the Department will notify the utility providing the service that it will accept billing. The Department will not accept billing of any utility service until the Certificate of Compliance has been issued, unless the Engineer establishes an earlier date in writing.

3. Spare Parts: The Contractor shall review the Contract and prepare a list of acceptable material to be turned over to the State at the completion of the Project for review and concurrence by the Engineer.

The Contractor shall provide a material safety data sheet with all required items to comply with OSHA requirements. The Engineer will not accept partially used and open items such as paints and solvents.

4. Insurance Coverage: The Contractor shall have in place all insurance coverage identified in

1.20-1.03.07 for the performance of any warranty work.

1.20-1.08.14—Facilities Construction - Acceptance of Project: The Project will be accepted by the Commissioner when all Project work has been completed, as defined by the requirements of 1.20-1.08.13, and the following have been submitted to the satisfaction of the Engineer:

1. **Supporting information necessary to substantiate pay quantities, such as cost-plus backup documentation;**
2. **Reports and forms required on all Federal Aid Projects;**
3. **Record Documents:** The Contractor shall submit all documents required by 1.20-1.05.05 to the Engineer prior to the date of the Semi-Final Inspection.
4. **Operation and Maintenance Manuals:** Prior to the date of the Semi-Final Inspection, the Contractor shall compile operation and maintenance manuals in the form of instructional manuals for use by the Owner. The Contractor shall organize said manuals into suitable sets of manageable size and, where possible, assemble instructions for similar equipment into a single binder.

Where 2 or more binders are necessary to accommodate data of a system, the Contractor shall cross-reference other binders where necessary to provide essential information for proper operation or maintenance of the piece of equipment or system.

For each manual, the Contractor shall:

(a) Provide heavy-duty, commercial-quality, 3-ring, vinyl-covered, loose-leaf binders, thick enough to accommodate contents, sized to receive 8 1/2 inch × 11 inch paper.

(b) Identify the binder's contents on binder's front and spine with the printed title "OPERATION AND MAINTENANCE MANUAL," Project title or name, and subject matter covered, and volume number for multiple volume sets.

(c) Organize each manual into sections, separated by a heavy paper divider with a tab marked to indicate the contents of the section.

(d) Provide a title page as the first sheet of each manual with the following information: subject matter covered by the manual; Contract number and title; date of submittal; name, address, and telephone number of the Contractor; and cross-reference to related systems in other sections.

(e) Provide a written table of contents for each volume, arranged systematically according to the organization of the Contract provisions (including specific CSI-formatted specifications within a particular Special Provision).

(f) Provide a general information section immediately following the table of contents, listing each product included in the manual, identified by product name. The Contractor shall list the name, address, and telephone number of the subcontractor, the maintenance contractor, and the local source for replacement parts and equipment for each product.

(g) Include manufacturer's standard printed data and mark each sheet to identify each part or product included in the Project, identify each product using appropriate references from the Contract, and delete references to information that is not applicable. The use of project record documents as part of operation and maintenance manuals is not permitted.

(h) Prepare supplementary text to provide operation and maintenance information when the manufacturer's standard printed data is not available or printed data is insufficient and the information is necessary for proper operation and maintenance of equipment or systems, organize text in a consistent format under separate headings for each procedure, and provide a logical sequence of instruction for each operation or maintenance procedure.

(i) Provide drawings where necessary in order to supplement manufacturer's printed data to illustrate the relationship of component parts of equipment or systems or to provide control or flow diagrams. Where oversize drawings are necessary, the Contractor shall fold drawings to the same size as text pages and use as a foldout. If the drawings are too large to be used practically as a foldout, the Contractor shall place the folded drawing in a 8 1/2 inch × 11 inch labeled pre-punched envelope or sleeve, and place it with the manufacturer's printed data. The Contractor shall coordinate these drawings with information contained in project record drawings to ensure correct illustration of the completed installation. The use of Project record documents as part of operation and maintenance manuals is not permitted.

(j) Provide estimated life cycle costs to maintain each product included in the manual to reach maximum useful life (i.e. annual, mid-life overhaul, end of life overhaul, or programmed interval replacement).

Product Maintenance Manual: The Contractor shall provide:

(a) Manufacturer's data and instructions on care and maintenance of product material, and finish.
(b) Complete information on architectural products, including the following, as applicable: manufacturer's catalog number, size, material composition, color, texture, and re-ordering information for specially manufactured products.
(c) Information (including cleaning schedule) on care and maintenance, including manufacturer's recommendations for types of cleaning agents and methods of cleaning, and methods of cleaning that could prove detrimental to the product.
(d) Complete manufacturer's data with instructions on inspection, maintenance, and repair of products exposed to the weather or designed for moisture-protection purposes.
(e) Manufacturer's data giving detailed information, including the following, as applicable: identification of relevant industry standards, chemical composition, installation details, inspection procedures, maintenance information, and repair procedures.

Equipment and Systems Maintenance Manual: The Contractor shall provide:

(a) A complete description of each unit and related component parts, including the following: name of manufacturer, model number and serial number, equipment or system function, operating characteristics, limiting conditions, performance curves, and engineering data and test results.
(b) The following for each unit and related component part: assembly drawings and diagrams required for maintenance, complete list of parts and supplies with current unit prices (identify which items are recommended to be stocked as spare parts and identify which items have an anticipated ordering and delivery time greater than 10 days), complete list of distributors and authorized repair facilities, and telephone numbers for technical service.
(c) Information detailing essential maintenance procedures, including the following or information about the following: routine operations; troubleshooting guide; disassembly, repair, and reassembly; alignment, adjusting, and checking; a list of any special tools required.
(d) Information on equipment and system operating procedures, including the following: startup procedures, equipment or system break-in, normal operating instructions, regulation and control procedures, instructions for shutdown and emergencies, summer and winter operating instructions, required sequences for electric or electronic systems, and special operating instructions.
(e) A schedule of routine servicing and lubrication requirements, including a list of required lubricants for equipment.
(f) As-installed control diagrams for systems requiring controls.
(g) Contractor's coordination drawings of as-installed piping and other systems, color-coded as needed for identification.
(h) Charts of valve tag numbers, with the location and function of each valve (clearly mark as such any valve intended for emergency shut-off or similar special use).
(i) Complete circuit directories of panelboards, including designations of the following: electric service, controls, and communication.
(j) Copies of maintenance agreements with service agent name and telephone number.

5. Training: The Contractor shall use experienced instructors thoroughly trained and experienced in operation and maintenance of Project equipment and systems, in order to instruct the Owner's operation and maintenance personnel.

The Contractor shall develop a training schedule for approval by the Engineer, the Owner, and the Commissioning Authority when applicable that is coordinated with the Owner's operations and working hours. This schedule shall be submitted a minimum of 30 calendar days in advance of the start of training.

The Contractor shall;

(1) arrange for each installer of equipment that requires regular maintenance to meet with the Owner in order to provide instruction in the proper operation and maintenance of any equipment that requires regular maintenance,
(2) provide instruction by manufacturer's representatives if installers are not experienced in any relevant procedures,
(3) provide instruction at agreed-upon times, and
(4) provide Engineer with a minimum of 72 hours advance notice of the training sessions.

The Contractor shall provide a syllabus prior to the training to ensure that the appropriate Owner's operation and maintenance personnel are in attendance.

The Contractor shall video record each training session.

6. Any other documents required by the Contract.

SECTION 1.20-1.09
MEASUREMENT AND PAYMENT FOR FACILITIES CONSTRUCTION

1.20-1.09.00—Facilities Construction - Unit Price Items, Lump Sum Items, Major Lump Sum Items
1.20-1.09.01—Facilities Construction - Measurement of Quantities
1.20-1.09.02—Facilities Construction - Value Engineering Change Proposal
1.20-1.09.03—Facilities Construction - Increased or Decreased Quantities
1.20-1.09.04—Facilities Construction - Extra and Cost-Plus Work
1.20-1.09.05—Facilities Construction - Eliminated Items
1.20-1.09.06—Facilities Construction - Partial Payments
1.20-1.09.07—Facilities Construction - Final Payment
1.20-1.09.08—Facilities Construction - Payment of Costs Owed to the State

1.20-1.09.00—Facilities Construction - Unit Price Items, Lump Sum Items, Major Lump Sum Items: Facilities Construction projects are bid with both lump sum and unit price items which are in addition to the Major Lump Sum Item (MLSI) of the Project. These separate items will be measured for payment on a unit price or lump sum basis (whichever is applicable) for which a separate bid price is required, at the quantities indicated in the Bid Proposal Form. Each item to be measured is more specifically described in a corresponding Standard Specification or a special provision.

Standard Items are referenced by their item numbers; refer to the applicable article for the requirements for this item. Special provisions are referenced by their item number followed by an "A" suffix; refer to the special provisions for requirements for this item.

All work depicted on the Contract Plans and described in the Contract Specifications, including mobilization and project closeout, is included in the MLSI of the Project, with the exception of the unit price or other lump sum items listed in the Bid Proposal Form. Any work incidental to an item which is not specifically described or included in the item, but which is required for performance and completion of the work required under the Contract, is included in the MLSI.

1.20.1.09.01—Facilities Construction - Measurement of Quantities: Work completed in compliance with the Contract will be measured by the Engineer according to U.S. Customary standard measures, and quantities of work performed shall be computed based on such measurements made in accordance with the methods of measurement described herein under provisions regarding the applicable Contract item.

Notwithstanding any other provision in the Contract, only work that is within the payment limits prescribed by the Contract or ordered by the Engineer will be measured for payment. No payment will be made for work that is not actually performed.

Structures shall be measured and quantities computed according to the neat lines shown on the plans (as those plans may be revised by authorization of the Engineer), or as may otherwise be required by the Contract.

Quantities of materials measured for payment by net weight shall be measured in tons, while contained in hauling vehicles on scales furnished by and at the expense of the Contractor. The scales shall be satisfactory to the Engineer and shall be sealed. When required by the Engineer, an inspector will be appointed and compensated by the Department to check the weight of all materials to be incorporated into the Project. The tare weight of trucks used to haul materials being paid for by weight shall be taken at such times as the Engineer directs.

1.20-1.09.02—Facilities Construction - Value Engineering Change Proposal: These Value Engineering Change Proposal (VECP) provisions apply as encouragement to the Contractor to initiate, develop, and present to the Department for consideration cost- or time- reduction proposals or a combination of both conceived by the Contractor, involving changes to the drawings, designs, specifications, or other requirements of the Contract. These provisions do not apply unless the proposal submitted is specifically identified by the Contractor as being presented for consideration as a VECP. All such proposals must be made on the Department's VECP form, copies of which are available from the Department. The Department reserves the right to decline to review, or to reject after initial review, any VECP. Before expending considerable funds in development of a formal VECP, the Contractor shall submit a conceptual Proposal to the Department on Department-provided forms.

The proposals which may be considered as VECPs are those which, if implemented, (a) would require modification of the Contract by construction order; (b) would produce a savings to the Department by calling for the use of items or methods less costly than those specified in the Contract; (c) would not alter

necessary standardized features of the original Project; and (d) would not impair essential functions or characteristics of the construction called for by the original Contract, such as service life, reliability, economy of operation, and ease of maintenance.

Material substitution alone will not be considered as a VECP.

A VECP may shorten Contract time, however, acceleration alone will not be considered as a VECP.

Cautions and Conditions:

1. The Contractor is cautioned not to base any bid or bid price on the anticipated approval of a VECP and to recognize that such Proposal may be rejected. The Contractor will be required to perform the Contract in accordance with the existing Contract plans and specifications at the prices bid unless and until the Department formally accepts, in writing, the Contractor's VECP.
2. In order for the Department to consider such a Proposal, the savings likely to be generated by the Proposal must be sufficient, in the sole judgement of the Department, to warrant its review and processing by the Department. All costs resulting from such review or processing will be borne by the Department. Before any VECP will be considered by the Department, the Department must determine, in its sole judgement, that implementation of the Proposal would result in a total cost savings of more than $100,000.00, reflecting a savings of at least $50,000.00 for the Department. The Department will not consider any VECP that would require an increase in Contract time.
3. All VECPs apply only to the ongoing Contract, and whether approved or not, such Proposals become the property of the Department. Such Proposals shall contain no restrictions imposed by the Contractor on their use or disclosure by the State. The Department will have the right to use, duplicate and disclose in whole or in part any data necessary for the use or implementation of the Proposal. The Department retains the right to use any accepted Proposal or part thereof on any other current or subsequent Department projects without any obligation to the Contractor for such use. This provision is not intended to deny rights provided by law with respect to patented materials or processes.
4. If the Department already has under consideration certain revisions of the Contract or has approved certain changes in specifications or standard drawings for general use which subsequently appear in a VECP, the Department may reject the Contractor's Proposal and may proceed with such revisions without any obligation to the Contractor.
5. The Proposal must be presented and approved in writing prior to the Contractor's undertaking any work on the Contract items involved in the proposal. Savings due to a reduction in quantities or deletion of items which result solely from adjustments to field conditions, and Proposals which would only waive specification or other Contract requirements, are not considered to be VECPs.
6. The Contractor shall have no claim against the Department for any costs or delays due to the Department's review or rejection of a VECP, including, but not limited to, development costs, anticipated profits, or increased material or labor costs resulting from delays in the review or rejection of such Proposal.
7. The Department will be the sole judge of the acceptability of a Proposal and of the estimated net savings in construction costs that would result from adoption of all or any part(s) of such Proposal. In determining such estimated net savings, the Department reserves the right to disregard the Contract bid prices if, in the judgment of the Engineer, such prices do not represent a fair measure of the value of work to be performed or deleted under the Proposal. Errors in the estimated quantities in the bid proposal form for the Contract shall be corrected by the Department prior to calculating the savings that would likely result from adoption of the VECP.
8. The Engineer may reject all or any portion of work performed pursuant to an approved VECP if the Engineer determines that unsatisfactory results are being obtained because of the Proposal's implementation with regard to that work. The Engineer may direct the removal of such rejected work and require the Contractor to proceed in accordance with the original Contract requirements. Where modifications of the VECP have been approved in order to adjust to field or other conditions, payment will be limited to the total amount payable for the work at the Contract bid prices, as if the pertinent work had been constructed in accordance with the original Contract requirements. The Contractor waives the right to use such rejection or limitation of reimbursement as the basis of any claim against the State for delay damages or for any other damages or costs.
9. VECPs must meet the requirements of the specifications or standards of the Department. The standards governing the original design of the Contract will be the minimal standard allowed.
10. If additional information is needed in order for the Department to evaluate Proposals, the Contractor must provide the Department with this information within 14 calendar days of such request or within

such other time period as may be approved by the Department. Failure to do so will result in rejection of the Proposal.

11. The Contractor shall provide revised Project plans, specifications and estimates to the Department in construction order format, reflecting such changes as would be required for implementation of the VECP. The Contractor shall be solely responsible for any errors or omissions resulting from such revisions.
12. Savings not directly related to the Contract, such as, but not limited to, reductions in inspection or testing costs or Department overhead, will not be included in the savings calculation for any VECP.

After the Contractor submits a conceptual Proposal, they will be notified in writing of the acceptability or the reason(s) for its rejection. The Department retains the right to reject the formal Proposal even if the conceptual Proposal was determined acceptable.

VECPs will be processed in the same manner as are alterations of the Contract that require a construction order.

VECP Submittal Requirements:

1. A statement that the Proposal is being submitted as a VECP.
2. A description of the difference between the existing Contract requirements and the proposed change(s), and the comparative advantages and disadvantages of each, taking into account considerations of service life, economy of operations, ease of maintenance, desired appearance, safety, and environmental impacts or necessary permit changes. When an item's function or characteristics would be altered by implementation of the Proposal, a justification of the anticipated effects of the alteration on the end item's performance must be included in the Proposal. A life-cycle cost analysis must be included for items involving alteration of functional characteristics. Factors for determining future worth will be provided by the Department.
3. Complete plans, specifications, and computations signed and sealed by a Professional Engineer licensed by the State of Connecticut, showing that the proposed Contract revisions would incorporate the same design criteria and restrictions that applied to the original Contract features and requirements. Said revisions shall be submitted by the Contractor in the Department's construction order format consisting of 1 paper copy of the plans and 1 electronic copy of the plans as a portable document format (PDF) file, indicating
   (a) quantity increases and decreases by item number, with associated cost;
   (b) new items, with their quantities and costs;
   (c) specifications in contract format; and, if needed,
   (d) compliance permit applications and revisions in accordance with 1.20-1.10.01 through 1.20-1.10.08 of these specifications.
4. A complete analysis of the probable cost effects of the proposed changes on Project construction, future operations in connection with the completed Project, maintenance and durability of completed Project construction, and other aspects of the Project, as appropriate.
5. The date by which the Proposal would have to be implemented in order for the Department to obtain the maximum cost reduction from the Proposal's implementation. The period established by the date must allow the Department ample time for review and processing of the Proposal. Should the Department find that it does not have sufficient time for such review and processing, it may reject the Proposal solely on such basis. If the Department fails to respond to the Proposal by said date, the Contractor shall consider the Proposal to be rejected and shall have no claims against the State as a result thereof.
6. A description of the effect that the implementation of the Proposal would likely have on the time required to complete the Project.

Payment for accepted VECPs:

1. The changes resulting from a VECP will be incorporated into the Contract by construction order and shall reflect the changes in existing unit bid item quantities, or any new agreed price items, cost-plus lump sum, or any combination thereof, as appropriate, in accordance with the Specifications and as determined by the Department. Any lump sum submission shall be accompanied by a schedule of payment values.
2. The Contract prices for the revised Project work will be paid directly as accomplished. In addition to such payment, the Department will pay the Contractor, under a separate item or a Value Engineering Incentive item, 50% of the total savings obtained by the State as a result of its implementation of the VECP. An estimate of said savings is to be calculated by the Department within 1 week prior to the

Proposal's acceptance, by

(a) estimating what it will cost the Department to carry out the Project as revised according to the VECP;

(b) estimating what it would have cost the Department to carry out the Project under the terms of the Contract as modified by any construction orders as of the time that the Department accepted the Proposal; and

(c) subtracting the sum estimated as per (a) from the sum estimated as per (b).

When the implementation of the Proposal, including all related construction, has been completed, the Department will calculate the actual savings that resulted from it. The Department will then distribute half of the actual savings to the Contractor.

3. The Contractor's costs for development, design, submission and processing of the VECP are not eligible for reimbursement.
4. The Department will not reimburse the Contractor based on any cost savings not identified in the VECP prior to its acceptance.
5. The cost savings from a VECP that is exclusively time reduction shall be calculated as the number of Contract days reduced multiplied by the amount of liquidated damages for 1 day under the Contract.

1.20-1.09.03—Facilities Construction - Increased or Decreased Quantities: Whenever the quantity of any item as given in both the bid proposal form and Contract is increased or decreased, the Department will pay for such item at the Contract price, on the basis of the actual quantity completed, except as otherwise expressly authorized under the provisions of 1.20-1.04.02, 1.20-1.04.03 or 1.20- 1.04.04.

1.20-1.09.04—Facilities Construction - Extra and Cost-Plus Work: Extra work shall be performed only under the conditions and subject to the requirements outlined in 1.20-1.04.05. Payment for extra work may be made on any unit price or lump sum price or other basis to which the Engineer and the Contractor agreed in writing, or the Engineer may order that the Contractor will be paid for the work on the cost-plus basis described in this Article.

The following sets forth the components of the cost-plus basis for making payments:

(a) Labor:

(1) For all labor used by the Contractor for the subject work, the Department will pay the Contractor the wage rate that it actually paid for same, as shown by its certified payroll, which shall be at least the minimum rate established for the Project by the CT Department or the U.S. Department of Labor. For all foremen in direct charge of Project work, the Department will pay the Contractor the actual wage paid to the foremen as shown on the Contractor's certified payroll.

(2) The Department will reimburse the Contractor for the actual costs paid to, or on behalf of, workers by reason of allowances, health and welfare benefits, pension fund benefits and other such benefits in connection with the subject work, when such amounts are required by a collective bargaining agreement or another employment contract generally applicable to the classes of labor employed on the Project. The Contractor shall certify all such costs in writing to the Engineer.

(3) For property damage, liability and workmen's compensation insurance premiums, unemployment insurance contributions and social security taxes on Project cost-plus work, the Department will reimburse the Contractor for its actual Project costs. The Contractor shall provide to the Engineer documentation, satisfactory to the Engineer in form and substance, of all such costs.

(4) The Department will also pay to the Contractor an amount equal to 20% (15% for overhead, 5% for profit) of the total sums described in **(a)** (1) through (3) above.

No part of the salary or expenses of anyone connected with the Contractor's forces above the grade of project superintendent, who provides general supervision of Project work, will be included in the above payment calculations, except when the Contractor's organization is entirely occupied with cost-plus work, in which case the salary of a superintendent may be included in said labor item when the nature of the pertinent Project work is such that, in the opinion of the Engineer, a superintendent was required for that work. The Engineer and the Contractor may agree in writing to the allowable rate of pay for such superintendent, or the Engineer may make payment based on such rate as he deems reasonable.

The Engineer reserves the right to determine the number and type of personnel to be employed for the cost-plus Project work.

(b) Specialized Work: When the Engineer directs the Contractor to perform specialized work requiring skills, tools and equipment substantially unlike those ordinarily used by the Contractor or its authorized Project subcontractors, the Department will pay the Contractor for the use of a specialist to perform the

specialized work. For such specialized services, including materials incorporated into the Project, the Department will pay the Contractor its actual costs, plus additional compensation in accordance with subparagraph (e) below. If so requested by the Engineer, the Contractor shall obtain and submit to the Engineer, prior to performing such specialized work, a minimum of three price quotes for the work.

(c) Materials: For all materials necessary for cost-plus Project work, the Department will pay the Contractor its actual cost for such materials, including delivery charges as shown by original receipted bills, plus 15 % of the sum of said cost and charges.

In lieu of receipted bills for materials used for the Project, but which were not specifically purchased for the Project, but rather were taken from the Contractor's stock, the Contractor shall provide to the Engineer an affidavit certifying that such materials were not purchased for the Project, that the materials were taken from the Contractor's stock, that the quantity claimed to have been used on the Project was actually so used, and that the price claimed for the materials reflects their fair market value at the time of their use on the Project. The Department will pay for costs of transporting the materials to the Project site, in accordance with subparagraphs (a) and (d) hereof.

The Department will not reimburse the Contractor for any penalty or charge incurred by the Contractor due to the Contractor's late or delayed payment for the pertinent materials.

(d) Equipment: All equipment used for cost-plus Project work must, in the judgment of the Engineer, be in good working condition and suitable for its Project purpose; and the Engineer reserves the right to determine the size and number of units of equipment to be used for such work. The manufacturer's ratings shall be the basis for all Rental Rate Blue Book classifications used for payment purposes. ("Rental Rate Blue Book" as used in these specifications refers to the current edition of the Rental Rate Equipment Watch Blue Book Services, taking into account all current Rate Adjustment Tables, and amendments thereof.) Trucks will be classified by cubic-yard capacity.

No percentage mark-up will be added for payment purposes to amounts charged by the Contractor based on equipment rental rates.

The Department will not pay rental rates for small tools needed to complete the cost-plus Project work.

For payment purposes, estimated operating costs per hour from the Rental Rate Blue Book will apply only to the actual time during which the equipment is actively being used to perform cost-plus Project work.

For equipment that is also being used for non-cost-plus Project work, the Department will pay the applicable hourly rate only for the actual time that the equipment was assigned to cost-plus Project work. The applicable period of assignment for each piece of equipment shall start when the equipment commences to be used for cost-plus Project work ordered by the Engineer, and shall end at the time designated by the Engineer.

For equipment brought to the Site exclusively for cost-plus work, the Department will reimburse the Contractor for loading and unloading costs and costs of transporting such equipment to and from the Project site; provided, however, that payment for return transportation from the Site shall not exceed the cost of moving the equipment to the Site. If such a piece of equipment is self-propelled, and is driven to the Site under its own power, then the Department will pay only operating costs and labor costs for its transport to and from the Project site. The Department will not, however, pay for any loading, unloading and transportation costs if the equipment is used for any Project work on the Site other than cost-plus work.

(1) Owned Equipment: The Department will pay the Contractor the applicable rental rate set forth in the Rental Rate Blue Book for any equipment (1) which the Contractor uses, with the Engineer's authorization, to perform cost-plus Project work, and (2) which is owned by the Contractor or a subsidiary, affiliate, or parent company of the Contractor (no matter how far up or down the chain of ownership from the Contractor).

The maximum hourly rate to be used in paying for Contractor-owned equipment assigned to cost-plus work shall be the applicable monthly rate in the Rental Rate Blue Book, divided by 176 (176 working hours per month).

Should the proper completion of the cost-plus Project work require equipment of a type not covered by the Rental Rate Blue Book, the Engineer will determine, and the Department will make payment to the Contractor at, a reasonable rental rate based on relevant rates prevailing in the area of the Project. If practicable, such rates shall be determined by the Engineer before the affected work is begun. If the Contractor proposes that the Engineer use a particular rate in such an instance, the Contractor must disclose to the Engineer the specific sources of, or support for, said rate.

If a piece of equipment owned by the Contractor is assigned to cost-plus Project work, but remains idle for some portion of the period of the cost-plus work, the Department will pay for that idle time at

50% of the applicable rental rate (exclusive of operating costs) in the Rental Rate Blue Book.

For payment purposes, the period of equipment usage shall be deemed to start when the Contractor begins to use the equipment for cost-plus Project work and shall be deemed to end when the equipment is released by the Engineer from use for such work. Any hours during which the equipment is used for work other than cost-plus Project work will be deducted from the pertinent payment period.

For any piece of Contractor-owned equipment assigned to cost-plus Project work, the Department will reimburse the Contractor for an aggregate minimum of 8 hours (of use time, idle time, or a combination thereof) in each 24-hour day (measured from one midnight to the following midnight) during the assignment period. No such reimbursement will be made, however, for Saturdays, Sundays and legal holidays during which the Contractor does no Project work, or for any other day on which the Engineer orders the Contractor to do no Project work. If the equipment is used to perform cost-plus Project work for more than 8 hours in a day, the Department will pay the Contractor at the applicable hourly rate computed on a monthly basis for the actual time of use; however the Department will not pay the Contractor for more than 8 hours of idle time for a piece of equipment during a given day.

The Department shall have the right to limit its aggregate Project payments for idle time for a given piece of equipment to the replacement value of that equipment.

(2) Rented Equipment: If the Engineer determines that in order to perform the cost-plus Project work the Contractor must rent certain machinery, trucks or other equipment not owned by the Contractor or a subsidiary, affiliate, or parent company of the Contractor (no matter how far up or down the chain of ownership from the Contractor), the Contractor shall provide to the Engineer in writing, in advance of such rental,

1. the specific nature of the rental(s),
2. the reasons for its need for such rental(s),
3. the anticipated or proposed rental rate(s), and
4. the estimated duration for the use of each piece of such rented equipment.

Proposed rates for such rented equipment must be based on the following:

—A daily rate per hour when the equipment is to be specifically assigned to Project work by the Engineer for a period of 7 consecutive calendar days or less.

—A weekly rate per hour when such assigned time exceeds 7 consecutive calendar days, but does not exceed 21 consecutive calendar days.

—A monthly rate per hour when such assigned time exceeds 21 consecutive calendar days.

The applicable daily, weekly, or monthly rate will be determined at the expiration of 21 calendar days or upon release of the equipment by the Engineer, whichever occurs first. Interruptions of the rental period, when equipment is used for work other than assigned cost-plus work, will not entitle the Contractor to payment at a rental rate that would be applicable to a shorter period that might arguably have been occasioned by such interruptions.

If so requested by the Engineer, the Contractor shall, prior to renting such equipment, obtain and submit to the Engineer in writing a minimum of three rate quotes for rental of the equipment.

The Department will pay the Contractor for such rental at the rate actually paid by the Contractor, provided that the given use and rental rate are acceptable to the Engineer. In order to obtain such payment, the Contractor must provide the Engineer with a copy of the original receipted bill for the rental expenses incurred.

(e) Administrative Expense: When extra work is performed on a cost-plus basis by a subcontractor acceptable to the Engineer, the Department will pay the Contractor an additional 7.5% for that work; such payment will be in addition to the percentage payments described in (a), (b), (c) and (d) above, as a reimbursement for the Contractor's administrative expense in connection with such work. The Engineer will approve such additional payments only if and when the Contractor provides to the Engineer receipted invoices for all relevant costs.

(f) Miscellaneous: The compensation provided for in (a), (b), (c), (d) and (e) above shall be deemed to be payment in full for the extra work, and shall be deemed as full compensation for same, including costs of superintendence, use of small tools, equipment for which no rental is allowed, safety equipment, consumables, field office overhead, home office overhead, bonding, other insurance, and profit. The Contractor's representative and the Engineer shall compare their respective records related to the extra work done on a cost-plus basis at the end of each day. Copies of these records shall be signed by both the

Engineer and the Contractor's representative. The Engineer will then forward a copy of same to the Contractor and to any affected subcontractor in accordance with Department procedures. Upon payment of such costs by the Contractor, the Contractor shall immediately furnish the Engineer with original receipted bills covering the costs, including transportation charges, for all materials used for such work.

1.20-1.09.05—Facilities Construction - Eliminated Items: The Engineer may eliminate from the Contract any pay unit item, or any portion of Project work contained in a lump sum item by giving written notice of said elimination to the Contractor. Such elimination shall in no way invalidate the Contract.

The Engineer will make final payment to the Contractor for materials at the actual cost of the materials for eliminated pay unit items or portions of work contained in a lump sum item only under the following terms and conditions:

1. the materials were ordered by the Contractor prior to the Engineer's issuance to the Contractor of a written notice of the unit or work's elimination (as evidenced by a dated invoice from the vendor);
2. the materials conformed to all Contract requirements; and
3. the Contractor could not have cancelled its order within 2 days after the issuance of the elimination notice.

Any materials paid for by the Department on these conditions shall then be property of the State, and the State shall assume, or shall reimburse the Contractor for, the actual cost of any further handling necessary to deliver said materials to a location designated by the Engineer.

If the relevant materials purchased by the Contractor are returnable to their vendor and if the Engineer so directs, the Contractor shall return the materials to the vendor, in which case the Department will reimburse the Contractor for any reasonable changes made to the Contractor by the vendor for the return of the materials, and for the actual costs to the Contractor of its handling the materials in returning them to the vendor. Such reimbursements by the Department shall be computed as though the work were being paid for on a cost-plus basis under 1.20-1.09.04.

If the Engineer determines that an elimination of a pay unit item or portion of work contained in a lump sum item constitutes a "significant change" in the character of the Contract work, as defined under 1.20-1.04.03, necessitated by a written order of the Engineer, the terms of 1.20-1.04.03 shall govern the payment to be made in relation to the eliminated item or work.

1.20-1.09.06—Facilities Construction - Partial Payments:

A. Monthly and Semi-monthly Estimates:

(1) Once each month, the Engineer will make, in writing, current estimates of the value of work performed in accordance with the Contract, calculated at Contract unit prices, including but not limited to the value of materials complete in place and materials not yet incorporated into the Project, but approved by the Engineer for payment (as provided for elsewhere in this article). Retainage will not be held.

Exceptions may be made as follows:

(a) When not in conflict with the interests of the State, the Contractor may request, and the Engineer may make, semi-monthly estimates for payment.

(b) If, in the judgment of the Assistant District Engineer, the Project is not proceeding in accordance with the Contract the Engineer may decline to make a payment estimate.

(c) If the total value of the Project work completed since the last estimate amounts to less than $2,500, the Engineer also may decline to make a payment estimate.

(2) The Engineer may also make payment at Contract unit prices for the number of units that represent the value of the Project work performed to date, if said units are essentially, though not totally, complete.

B. Payment for Stored Materials: Non-perishable materials that are required for Project construction and that the Contractor has produced or purchased specifically for incorporation into the Project, but which have not yet been so incorporated, may be included in a payment estimate if

(i) the materials meet all applicable Contract specifications,

(ii) the materials have been delivered to the Project site or to another location approved by the Engineer, and

(iii) the Contractor has submitted to the Engineer, as evidence of the Contractor's purchase of the materials, either a copy of a receipted bill for same or a Certificate of Title to the materials, in the form approved by the Department, duly-executed by the Contractor and the Vendor.

The Engineer will decide at what fair and appropriate fraction of the applicable Contract price such materials may be included in a payment estimate.

Offsite storage may be approved by the Engineer, provided that the materials proposed for payment are

segregated from other materials, clearly labeled as being owned by the Department for use on the identified Project, otherwise handled in compliance with 1.20-1.06.03, and stored in accordance with the manufacturer's recommendations. All such materials must be readily-available for inventory and inspection by the Engineer. Storage outside of the State of Connecticut may be considered only when a representative of the Department is able to verify that the above requirements have been satisfied.

For items requiring extended fabrication, manufacturing or assembly time, the Contractor may propose to the Engineer a schedule of values for the related material costs. If the Engineer approves such a schedule of values, it shall become the Basis of Payment for the stored materials, so long as all other pertinent Contract requirements have been satisfied.

Generic materials having a use on many projects will be considered for payment prior to their incorporation into the Project only if stored in unopened packaging or in large lots. Stock and raw materials will not be considered for such advance payment without the Engineer's prior written consent thereto.

In no case shall material payments exceed the Contract unit price or lump sum price less the actual value of delivery and installation of the materials; if they do exceed such a price, the Engineer reserves the right to reduce any related payment accordingly. Such reductions in payment shall in no way affect the Department's ownership interest in the stored materials.

1.20-1.09.07—Facilities Construction - Final Payment: When the Commissioner has accepted the Project in accordance with 1.20-1.08.14, the Engineer will prepare a final payment estimate.

1.20-1.09.08—Facilities Construction - Vacant

1.20-1.09.09—Facilities Construction - Payment of Costs Owed to the State: The State shall have the right to set off against amounts otherwise due to the Contractor under this Contract or under any other contract or arrangement that the Contractor has with the State

(a) any costs that the State has incurred due to the Contractor's noncompliance with this Contract and

(b) any other amounts that are due and payable from the Contractor to the State.

Any sum taken in setoff from the Contractor shall be deemed to have been paid to the Contractor for purposes of payment obligations under 1.20-1.03.04 of these Specifications.

SECTION 1.20-1.10
ENVIRONMENTAL COMPLIANCE FOR FACILITIES CONSTRUCTION

1.20-1.10.01—Facilities Construction - General
1.20-1.10.02—Facilities Construction - Compliance with Laws and Regulations
1.20-1.10.03—Facilities Construction - Water Pollution Control
1.20-1.10.04—Facilities Construction - Vacant
1.20-1.10.05—Facilities Construction - Construction Noise Pollution
1.20-1.10.06—Facilities Construction - Protection of Archaeological and Paleontological Remains and Materials
1.20-1.10.07—Facilities Construction - Controlled and Hazardous Materials
1.20-1.10.08—Facilities Construction - Vehicle Emissions
1.20-1.10.09—Facilities Construction - Compliance with Existing Site Permits

1.20-1.10.01—Facilities Construction - General: During and following Project construction, the Contractor shall exercise precaution and care to prevent or minimize negative effects on the environment, including the State's waters, wetlands, and other natural resources.

The Contractor shall comply with all Project permits and permit applications as though the Contractor were the permittee.

The Contractor must comply with the environmental provisions specified in the Contract, and any Federal, State or municipal laws or regulations. If the Contractor fails to comply with these environmental provisions, the Contractor shall be penalized as specified in this Section and elsewhere in the Contract.

1.20-1.10.02—Facilities Construction - Compliance with Laws and Regulations: The Contractor shall conduct its operations in conformance with the permit requirements established by Federal, State and municipal laws and regulations.

The Department will be responsible for obtaining all environmental permits required for Contract work. If at the time such a permit is issued, its contents differ from those described in the Contract, the permit shall govern. Should the permit be issued after the solicitation of bid proposals, and should the permit requirements significantly change the character of the work as described in the Department's Project bid documents, Contract adjustments will be made in accordance with the applicable articles in 1.20-1.04.01 through 1.20-1.04.07 of these specifications.

The Contractor shall be responsible for, and hold the State harmless from, any penalties or fines assessed by any authority due to the Contractor's failure to comply with any term of an applicable environmental permit.

Any request by the Contractor for the Department's authorization of an activity or use of a method not specifically called for or allowed by the applicable permits issued for the Project must be submitted by the Contractor in writing to the Engineer. Such a request must include a detailed description of the proposed alternate activity or method, and must include justifications for same, along with supporting documentation, showing that the proposed alternate activity or method will not create a risk of damage to the environment, increase the permitted wetland impact footprint, or increase fill within a floodplain. If such request is granted by the Engineer, the Department will forward to the appropriate regulatory agency or agencies any permit modification, permit revision, *de minimis* change or new permit required for the Contractor to carry out the proposed alternate activity or method in question. The Department does not, however, guarantee that it will be able to obtain such approval from the regulatory agency or agencies; and the Department will not be liable for the effects of such inability to do so.

The Contractor will not be entitled to any extension of Contract time as a result of the Engineer's granting of such a request from the Contractor. If changes to the permit are not necessary except to accommodate changes requested by the Contractor, then no claim may be made by the Contractor based on the amount of time taken by the Department to review the Contractor's request or to secure approval of related permit changes from the regulatory agency or agencies. The proposed alternate activity or method shall not commence until and unless the Engineer has approved the Contractor's request.

1.20-1.10.03—Facilities Construction - Water Pollution Control: The Contractor shall, throughout the duration of the Contract, control and abate siltation, sedimentation and pollution of all waters, including but not limited to under-ground water systems, inland wetlands, tidal wetlands, and coastal or navigable waters.

Construction methods proposed by the Contractor must comply with the approved permit requirements and permit applications. The Contractor shall be responsible for all obligations and costs incurred as a

result of the Contractor's failure to comply with the terms and conditions of such permits or permit applications.

The following are Required Best Management Practices for prevention and control of water pollution. Provisions of the Required Best Management Practices may be superseded as specified in 1.20-1.05.04. The Contractor shall not make any design change in the Contract work that requires a variance from the requirements of the following items until and unless the Contractor has first submitted a detailed written proposal for such variance to the Engineer for review by the Department and for transmittal to and review by the Federal, State or municipal environmental authority, and has then received written approval from the Department of the proposed variance.

REQUIRED BEST MANAGEMENT PRACTICES

1. Prior to commencing Project Site work, the Contractor shall submit in writing to the Engineer an "Erosion and Sedimentation Control Plan" and a "Dust Control Plan" for all Project construction stages. The Contractor shall install all control measures specified in said Plans prior to commencement of Project construction activities. The Plans shall be consistent with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control, the 2004 Connecticut Stormwater Quality Manual, and all environmental laws and regulations established by Federal, State or municipal agencies, as well as the Department's published environmental policies and standards. If the Contractor elects to work during a winter shut-down period, the Contractor shall submit to the Engineer a separate Winter Erosion and Sedimentation Control Plan, obtain the Engineer's written approval of it, and implement it before the Contractor begins Project work during the winter shut-down period.
2. The Contractor shall inspect erosion and sedimentation controls at least weekly, immediately after each rainfall event of at least 0.1 inch, and daily during periods of prolonged rainfall. The Contractor shall maintain all erosion and sedimentation control devices in a functional condition, in accordance with the Contract plans, relevant permits, Special Provisions, and 2002 Connecticut Guidelines for Soil Erosion and Sediment Control. In the event that the Contractor fails to maintain such devices in accordance with said documents, and the Contractor does not correct such a failure within 24 hours after receipt of written notice of such a failure from the Engineer, the Department may proceed with its own or other forces to remedy such failures. The cost to the Department of curing any such specified failure will be deducted from monies owed to the Contractor under the Contract or under any other State contract.
3. Washout of applicators, containers, vehicles, and equipment that have been used with concrete (including bituminous concrete), paint or other such possible contaminants shall be conducted: (i) at least 50 feet from any stream, wetland or other sensitive resource; and (ii) in an entirely self-contained washout system. Such materials shall be collected and disposed of in accordance with all applicable Federal, State and municipal laws and regulations.
4. No materials resulting from Project construction activities shall be placed in or allowed to contribute to the degradation of a wetland, watercourse or storm drainage system. Good housekeeping of the Site by the Contractor for the purpose of preventing construction-related debris or runoff from entering a regulated area is required. The Contractor shall not leave waste or debris within the travel way or roadside where it might create a safety hazard to the traveling public. The Contractor shall dispose of all construction-related materials in accordance with Federal, State and municipal laws and regulations.
5. In accordance with CGS Section 22a-38, the Contractor shall not withdraw water from any watercourse system, except as allowed by applicable permits.
6. The Contractor shall not dispose of any material until and unless it has proposed a location for its disposal to the Engineer and the Engineer has approved said location in writing.

   If the proposed disposal location is on private property, the Contractor must include in the disposal location proposal to the Engineer letters from the property owner and the affected municipality, agreeing to the proposed location for disposal.

   The Contractor shall ensure that proposed disposal locations are outside of wetlands or watercourses, floodplains and water or natural resource areas.
7. Before commencing any work in or adjacent to a regulated area shown on the plans, permit(s), or identified by the Engineer, the Contractor must submit in writing to the Engineer a construction-sequencing plan, a water-handling plan, and a flood contingency plan, and obtain from the

Engineer written approval of said plans..

8. When dewatering is necessary, the Contractor must not allow pumps used for same to discharge directly into a wetland or watercourse. Prior to any dewatering, the Contractor must submit to the Engineer a written proposal for specific methods and devices to be used for same, and must obtain the Engineer's written approval of such methods and devices, including, but not limited to, the pumping of water into a temporary sedimentation basin, providing surge protection at the inlet or outlet of pumps, floating the intake of a pump, or any other method for minimizing or retaining the suspended solids. If the Engineer determines that a pumping operation is causing turbidity in a regulated area, the Contractor shall halt said operation until a means of controlling the turbidity is submitted by the Contractor in writing to the Engineer, approved in writing by the Engineer, and implemented by the Contractor.
9. Whenever possible, work within or adjacent to watercourses shall be conducted during periods of low flow. The Engineer shall remain aware of flow conditions during the conduct of such work, and shall order such work stopped if flow conditions threaten to cause excessive erosion, siltation or turbidity. Before predicted major storms (*i.e.*, a storm predicted by NOAA Weather Service, with warnings of flooding, severe thunderstorms, or similarly severe weather conditions or effects), the Contractor shall make every effort to secure the Site to the satisfaction of the Engineer. Unless allowed by a DEEP permit, the Contractor shall store no materials and place no staging areas below the 100-year elevation. The Contractor shall not store below the 500-year flood level any materials which are buoyant, hazardous, flammable, explosive, soluble, expansive, radioactive, and any other materials that could be injurious to human, animal or plant life in the event of a flood.
10. Upon completion of the associated work, the Contractor shall immediately clear all areas of all forms, false work, piling, debris or other obstructions created or caused by construction operations.
11. If the Contractor wants to make a change in construction operations, staging or scheduling that would affect the use of or necessity for any pollution controls, the Contractor must submit to the Engineer a written proposal detailing them the proposed change, and must receive the Engineer's approval of such change, before implementing it. Such submission must include a plan showing what erosion and sedimentation controls above and beyond those called for in the Contract would be necessitated by the proposed change.
12. Dumping of oil, fuel, chemicals or other harmful materials on the ground or into a regulated area is forbidden. The Contractor shall provide to the Engineer a written Spill Prevention and Remediation Plan for the Project, outlining the Contractor's intended means of catching, retaining, and properly disposing of drained oil, removed oil filters, fuel, chemicals and other harmful material. Such plan shall also include the information and protocols needed for the remediation of, any spill that might occur on the Site, including emergency contact information. No construction activities shall commence until such a plan has been approved in writing by the Engineer.
13. The Contractor shall restore all areas within or outside the State right-of-way that have been disturbed as a result of construction activities, in accordance with 1.20-1.08.11.

1.20-1.10.04—Facilities Construction - Vacant

1.20-1.10.05—Facilities Construction - Construction Noise Pollution: The Contractor shall take measures to minimize the noise caused by its construction operations, including but not limited to noise generated by equipment used for drilling, pile-driving, blasting, excavation or hauling.

All methods and devices employed to minimize noise shall be subject to the continuing approval of the Engineer. The maximum allowable level of noise at the residence or occupied building nearest to the Site shall be 90 decibels on the "A"-weighted scale (dBA). The Contractor shall halt any Project operation that violates this standard at any time until the Contractor develops and implements a methodology that enables it to keep the noise from its Project operations below the 90-dBA limit.

1.20-1.10.06—Facilities Construction - Protection of Archaeological and Paleontological Remains and Materials: The Contractor shall be alert to the possibility that Project operations may disturb or uncover significant archaeological or paleontological resources or other such remains which in many cases are protected by Federal laws, State laws or both. Archaeological resources are minimally defined by

Federal regulations as materials 50 years of age or older. They typically consist of subsurface concentrations of metal, bone, ceramic, or flaked or other shaped stone artifacts. They might also consist of *features* such as buried building foundations, linear or circular walls made of individual stones rather than concrete or cement, trash-filled pits, patches of burned earth, or distinct patterns of nearly-circular, elliptical, or squared discolorations in newly-exposed soil, accompanied by the types of *artifacts* described above.

Paleontological resources are defined as any fossilized remains, traces, or imprints of organisms, preserved in or on the earth's crust. These typically include fossilized bones, teeth, shells, eggs, or distinct impressions made in bedrock.

When archaeological or paleontological materials are inadvertently encountered, the Contractor shall immediately halt operations in the location of same and shall notify the Engineer of said discovery. The Contractor shall make every effort to preserve archaeological or paleontological materials intact in their original positions, in order to preserve the geological context and information content of the remains in relation to one another and to the enclosing soil.

The Engineer shall have the authority to suspend Project work in the area of such discovery for the purpose of preserving or recovering and documenting the archaeological or paleontological materials. The Contractor shall carry out all instructions of the Engineer for the protection of such materials, including steps to protect the site from vandalism, unauthorized investigations, accidental damage, and damage from such causes as heavy rainfall or runoff. The Contractor shall reschedule its work to minimize any loss of the time needed to complete the Project while the State evaluates, records and salvages the archaeological or paleontological materials.

Extra work ordered by the Engineer in this connection will be paid for in accordance with 1.20-1.04.05 and 1.20-1.09.04. Delays caused by archaeological or paleontological preservation and protection, which the Contractor demonstrates have delayed completion of the Project, will be treated under the provisions for extension of time, 1.20-1.08.08.

1.20-1.10.07—Facilities Construction - Controlled and Hazardous Materials: The Department will acquire any "Hazardous Waste Generator Permit(s)" required under the Resource Conservation and Recovery Act, for the management and disposal of hazardous materials on the Site, provided that

1. such material is within the construction limits defined in the Contract, and
2. such material was not generated by the Contractor.

If the Department has designated in the Contract an area of known or suspected contamination within the Project limits, the Contractor shall dispose of such material in accordance with the relevant Special Provisions.

In the event that the Contractor encounters or exposes any material, not previously known or suspected to be contaminated, but exhibiting properties that may indicate the presence of controlled or hazardous material, the Contractor shall cease all operations in the material's vicinity and shall immediately notify the Engineer of the material's discovery. The presence of barrels, discolored earth, metal, wood, visible fumes or smoke, abnormal odors or excessively hot earth may indicate the presence of controlled or hazardous material, and the Contractor shall treat it with extreme caution.

If controlled or hazardous materials, other than those required for Contract operations, are discovered at the Site, the Department may engage a specialty contractor to handle and dispose of the materials.

When the Contractor performs support work incidental to the removal, treatment or disposal of controlled or hazardous material, the Department will pay for same at the applicable Contract unit prices. When the Contract does not include appropriate pay items for such work, the Department will pay for it in accordance with 1.20-1.04.05.

The Contractor shall observe all security precautions established pursuant to 29 CFR 1910.120 and 1926.65, including all revisions and amendments thereof, and shall not work in any area known to contain or suspected of containing controlled or hazardous material without prior written approval to do so from the Engineer.

The Contractor shall assume sole responsibility for the proper storage, handling, management, and disposal of all regulated materials and wastes associated with its operations, including, but not limited to, lubricants, antifreeze, engine fluids, paints, and solvents. All costs associated with any failure by the Contractor to properly manage such materials in accordance with Federal, State and municipal regulations, and all remedial and punitive costs incurred by the Department as a result of such failure by the Contractor, shall be borne by the Contractor.

This article does not apply to coatings removed by the Contractor.

1.20-1.10.08–Facilities Construction - Vehicle Emissions: All motor vehicles and construction equipment used for the Project (both on-highway and off-road) shall comply with all Federal, State and municipal regulations concerning exhaust emission controls or safety.

The Contractor shall establish staging zones for vehicles waiting to load or unload at the Site. Such zones shall be located where the emissions from the vehicles will have minimum impact on abutting properties and the general public.

Idling of delivery trucks, dump trucks, and other equipment shall not be permitted for longer than 3 minutes during periods of non-activity, except as allowed by the Regulations of Connecticut State Agencies Section 22a-174-18(b)(3)(c):

No mobile source engine shall be allowed "*to operate for more than 3 consecutive minutes when the mobile source is not in motion, except as follows:*

1. *When a mobile source is forced to remain motionless because of traffic conditions or mechanical difficulties over which the operator has no control,*
2. *When it is necessary to operate defrosting, heating or cooling equipment to ensure the safety or health of the driver or passengers,*
3. *When it is necessary to operate auxiliary equipment that is located in or on the mobile source to accomplish the intended use of the mobile source,*
4. *To bring the mobile source to the manufacturer's recommended operating temperature,*
5. *When the outdoor temperature is below 20°F*
6. *When the mobile source is undergoing maintenance that requires such mobile source be operated for more than 3 consecutive minutes, or*
7. *When a mobile source is in queue to be inspected by U.S. military personnel prior to gaining access to a U.S. military installation.*"

The Contractor shall conduct all of its Project work in a way that causes no harm to adjacent sensitive receptors. Sensitive receptors include but are not limited to hospitals, schools, daycare facilities, elderly housing, and convalescent facilities. The Contractor shall see to it that any engine exhaust is not directed toward fresh air intakes, air conditioners, or windows.

Before performing extensive work within less than 50 feet of a sensitive receptor, the Contractor must (1) submit to the Engineer a Vehicle Emissions Mitigation plan, proposing detailed means for minimizing vehicle emissions from vehicles and construction equipment in the affected area, including a proposed sequence of construction; (2) obtain the Engineer's written approval of the Plan, making any revisions of same necessary to obtain said permission; and (3) implement the Plan, as it may have been revised.

Any costs associated with this "Vehicle Emissions" Article shall be included in the general cost of the Contract. In addition, there shall be no additional time granted to the Contractor for compliance with this Article. The Contractor's compliance with this Article and any associated laws or regulations shall not be grounds for claims as outlined in 1.20-1.11.01 through 1.20-1.11.06 of these specifications.

1.20-1.10.09—Facilities Construction - Compliance with Existing Site Permits: The Contractor shall conduct its operations in conformance with the permit requirements established by Federal, State and municipal laws and regulations.

In addition to permits obtained by the Department specifically for the Project, facilities have existing site specific permits and regulatory requirements related to site operational activities. The specific permits and regulatory requirements will be identified in the Contract. The Contractor shall become familiar with these requirements and shall conduct their operations in conformance with these requirements.

The Contractor shall be responsible for, and hold the State harmless from, any penalties or fines assessed by any authority due to the Contractor's failure to comply with any term of an applicable environmental permit.

SECTION 1.20-1.11
CLAIMS FOR
FACILITIES CONSTRUCTION

1.20-1.11.01—Facilities Construction - General
1.20-1.11.02—Facilities Construction - Notice of Claim
1.20-1.11.03—Facilities Construction - Record Keeping
1.20-1.11.04—Facilities Construction - Claim Compensation
1.20-1.11.05—Facilities Construction - Required Claim Documentation
1.20-1.11.06—Facilities Construction - Auditing of Claims

1.20-1.11.01—Facilities Construction - General: When the Contractor files against the Department or the State a formal claim (a "formal" claim being one that seeks resolution through binding arbitration or court litigation, rather than through negotiation or mediation) under CGS Section 4-61 as revised ("Section 4-61"), whether as a Section 4-61 notice of claim, demand for arbitration or as a complaint in the Superior Court, the Contractor must follow the procedures and comply with the requirements set forth in this Section of the Specifications, as well as those set forth in Section 4-61. If this Section sets forth additional, more specific, or demanding requirements than does Section 4-61 in any respect, this Section shall govern the matter. While the requirements of this Section may not strictly apply to informal claims ("informal" claims being those which the Contractor seeks to resolve through negotiations with the Department, in or outside of a mediation) for additional compensation or other relief from the Department, the Contractor should understand that the Department may need and may demand (in which case the Contractor must provide), the same kinds of documentation and other substantiation that are required under this Section for formal claims. In addition, any time extension request submitted as part of a claim, must satisfy the requirements of this specification and those of 1.08.08. It is the intent of the Department to compensate the Contractor for actual increased costs caused by or arising from acts or omissions on the part of the Department that violate legal or contractual duties owed to the Contractor by the Department.

1.20-1.11.02—Facilities Construction - Notice of Claim: Whenever the Contractor intends to file a demand for arbitration or a court complaint against the Department under Section 4-61, the Contractor must first notify the Commissioner of the details of said claim, in writing via certified mail (in strict compliance with Section 4-61), and such written notice must contain all pertinent information described in 1.20-1.11.05 below.

Once a formal notice of a claim under Section 4-61 has been given to the Commissioner, the claimant may not change the claim in any way, in either concept or monetary amount, except insofar as the claim seeks damages that will continue to accrue after submission of the notice, in ways described and anticipated in that notice.

1.20-1.11.03—Facilities Construction - Record Keeping: The Contractor shall keep daily records identifying:

(1) Each aspect of the Project affected by matters related to any claim for additional compensation or relief that the Contractor has filed, intends to file, or has reason to believe that it may file against the Department
(2) The specific Project locations where Project work has been so affected
(3) The number of people working on the affected aspects of the Project at the pertinent time(s)
(4) The types and number of pieces of equipment on the Project site at the pertinent time(s)

All events or conditions that have a potential or anticipated effect on the Project's progress or schedule and that may result in a claim by the Contractor shall be documented contemporaneously with the event or discovery of the pertinent condition(s), or immediately thereafter. If this is not done, the Contractor may not file the related claim and may not be awarded relief upon it. Without such information, the Department and the Office of the Attorney General may not be able to adequately determine what claims have merit or to what extent they have merit, or what amounts of compensation may be warranted and supportable. Moreover, State officials involved in the analytic or negotiation process may not be able to properly substantiate and support the recommendations that they must make to their superiors, including the Attorney General, and sometimes the Governor, in the course of a settlement process.

1.20-1.11.04—Facilities Construction - Claim Compensation: If the Contractor proves entitlement for damages, payment shall be made in accordance with the following provisions:

(a) Compensable Items: The liability of the Department for claims will be limited to the following

specifically-identified items of cost, insofar as they have not otherwise been paid for by the Department (for instance, through payment for extra work, which under 1.20-1.04.05 includes overhead and profit), and insofar as they were caused solely by the actions or omissions of the Department or its agents. The Department will pay for direct labor expenses, direct costs for materials, and direct costs for active equipment use, plus an additional ten percent (10%) of the total amount of such direct costs as payment for home office overhead and profit.

Compensable delay-related costs: The Department will pay for any additional field office overhead and idle equipment costs for each day of Project Critical Path delay or suspension caused solely by action or inaction of the Department.

If the Critical Path delay or suspension period is less than 30 calendar days, the Department will pay an additional ten percent (10%) of the additional field office overhead costs as payment for home office overhead and profit. For delays less than 30 calendar days, idle equipment will be paid at 50% of the Rental Rate Blue Book rate.

For delays equal to or longer than 30 calendar days, the Department will pay a per diem rate, calculated as six percent (6%) of the original total Contract amount divided by the original number of days of Contract time, as payment for home office overhead and profit.

In paying for idle equipment equal to or longer than 30 calendar days, the Department will pay for actual equipment costs. Actual equipment costs shall be based upon records kept in the normal course of business and in accordance with generally-accepted accounting principles. Under no circumstances shall Rental Rate Blue Book or other guide or rental rates be used for this purpose (unless the Contractor had to rent the equipment from an unrelated party, in which case the actual rental charges paid by the Contractor, so long as they are reasonable, shall be reimbursed by the Department).

If the final Contract Value is greater than the original Contract Value, any delay-related costs that are compensable under this Article shall be reduced by eight percent (8%) of the difference between the final Contract Value and the original Contract Value.

Such payments for compensable delay-related costs shall be deemed to be complete and mutually-satisfactory compensation for field and home office overhead related to the period of delay or suspension.

Subcontractor costs of any kind, however, may be paid for by the Department only (a) in the context of a negotiated claims settlement or (b) if the Contractor has itself paid or legally-assumed, present unconditional liability for those subcontractor costs.

(b) Non-Compensable Items: The Department will have no liability for the following specifically-identified non-compensable items:

(1) Profit, in excess of that provided for herein.
(2) Loss of anticipated profit.
(3) Loss of bidding opportunities.
(4) Reduction of bidding capacity.
(5) Home office overhead in excess of that provided for herein.
(6) Attorney's fees, claims preparation expenses, or other costs of claims proceedings or resolution.
(7) Any other consequential or indirect expenses or costs, such as tort damages, or any other form of expense or damages not provided for in these Specifications or elsewhere in the Contract.

1.20-1.11.05—Facilities Construction - Required Claim Documentation: All claims shall be submitted in writing to the Commissioner, and shall be sufficient in detail to enable the Engineer to ascertain the basis and the amount of each claim, and to investigate and evaluate each claim in detail. When submitting any claim over $50,000, the Contractor shall certify in writing, under oath and in accordance with the formalities required by the Contract, that the following are true:

1. That supporting data is accurate and complete to the Contractor's best knowledge and belief;
2. That the amount of the dispute and the dispute itself accurately reflects what the Contractor in good faith believes to be the Department's liability.

The certification shall be executed by an officer or general partner of the Contractor having overall responsibility for the conduct of the Contractors affairs.

When submitting a claim to the Commissioner, as a minimum, the Contractor must provide the following information for each and every claim and sub-claim asserted:

(a) A detailed factual statement of the claim, with all dates, locations and items of work pertinent to the claim.

(b) A statement of whether each requested additional amount of compensation or extension of time is based on provisions of the Contract or on an alleged breach of the Contract. Each supporting or breached Contract provision and a statement of the reasons why each such provision supports the claim, must be specifically identified or explained.
(c) Excerpts from manuals or other texts which are standard in the industry, if available, that support the Contractor's claim.
(d) The details of the circumstances that gave rise to the claim.
(e) The date(s) on which any and all events resulting in the claim occurred, and the date(s) on which conditions resulting in the claim first became evident to the Contractor.
(f) Specific identification of any pertinent document, and detailed description of the substance of any material oral communication, relating to the substance of such claim.
(g) The name, function, and pertinent activity of each Contractor's or subcontractor's official, or employee involved in or knowledgeable about events that give rise to, or facts that relate to, the claim.
(h) The amount(s) of additional compensation sought and a breakdown of the amount(s) into the categories specified as payable under 1.20-1.11.04 above.
(i) The name, function, and pertinent activity of each Department official, employee or agent involved in or knowledgeable about events that give rise to, or facts that relate to, the claim.

1.20-1.11.06—Facilities Construction - Auditing of Claims: All claims filed against the Department shall be subject to audit by the Department or its agents at any time following the filing of notice of such claim. The Contractor and its subcontractors and suppliers shall cooperate fully with the inquiries and document requests of the Department's auditors. Failure of the Contractor**,** its subcontractors, or its suppliers to maintain and retain records that are sufficient to enable the Department or its agents to fully evaluate the claim shall constitute a waiver of any portion of such claim that cannot be verified by specific, adequate, contemporaneous records, and shall bar recovery on any formal claim or any portion of such a claim for which such verification is not produced. Without limiting the foregoing requirements, and as a minimum, the Contractor shall make available to the Department and its agents the following documents in connection with any claim that the Contractor submits:

(1) Daily time sheets and project superintendent's daily reports.
(2) Union agreements, if any.
(3) Insurance, welfare, and benefits records.
(4) Payroll register.
(5) Earnings records.
(6) Payroll tax returns.
(7) Records of property tax payments.
(8) Material invoices, purchase orders, and all material and supply acquisition contracts.
(9) Materials cost distribution worksheets.
(10) Equipment records (list of company equipment, rates, cost pools, etc.).
(11) Vendor rental agreements
(12) Subcontractor and vendor subcontracts, purchase orders, and/or agreements including all change orders and modifications.
(13) Subcontractor and vendor invoices to the Contractor, and the Contractor's certificates of payments to subcontractors and vendors.
(14) Subcontractor payment certificates.
(15) Canceled checks (payroll, subcontractors, and vendors).
(16) Job cost reports.
(17) Job payroll ledger.
(18) General ledger, general journal (if used), and all subsidiary ledgers and journals, together with all supporting documentation pertinent to entries made in these ledgers and journals.
(19) Cash disbursements journals.
(20) Financial statements for all years reflecting the operations on the Project.
(21) Income tax returns for all years reflecting the operations on the Project.
(22) Depreciation records on all company equipment**,** whether such records are maintained by the company involved, its accountant, or others.

(23) If a source other than depreciation records is used to develop costs for the Contractor's internal purposes in establishing the actual cost of owning and operating equipment, all such other source documents.

(24) All documents which reflect the Contractor's actual profit and overhead during the years that the Project was being performed**,** and for each of the five (5) years prior to the commencement of the Project.

(25) All documents related to the preparation of the Contractor's bid, including the final calculations on which the bid was based.

(26) All documents which relate to the claim or to any sub-claim, together with all documents that support the amount of damages as to each claim or sub-claim.

(27) Worksheets used to prepare the claim, which indicate the cost components of each item of the claim, including but not limited to the pertinent costs of labor, benefits and insurance, materials, equipment, and subcontractors' damages, as well as all documents which establish the relevant time periods, individuals involved, and the Project hours and the rates for the individuals.

SECTION 1.20-9.75
MOBILIZATION FOR
FACILITIES CONSTRUCTION

1.20-9.75.01—Facilities Construction - Mobilization Description
1.20-9.75.04—Facilities Construction - Mobilization Method of Measurement

1.20-9.75.01—Facilities Construction - Mobilization Description: This item consists of:

1. all work necessary for moving Project personnel and equipment to the Project Site;
2. all work necessary for the establishment of the Contractors' field offices, buildings and other facilities necessary for Contract performance;
3. the preparation of work plans and other documents that must be submitted by the Contractor to the Department prior to the start of physical Project construction. These initial submittals are identified elsewhere in the Contract and may include Project schedules, Project management plans, staging and storage areas, safety plans, quality control plans, erosion and sedimentation control plans, and other documents addressing general Project sequencing or management;
4. demobilization of plant and equipment;
5. completion of all physical work, and
6. completion of administrative closeout items as required by the Contract.

1.20-9.75.04—Facilities Construction - Mobilization Method of Measurement: Mobilization as defined in 1.20-1.03.01 will be paid in the manner described hereinafter; however, the determination of the total Contract amount earned shall not include the amount of mobilization earned during the period covered by the current monthly estimate – but shall include amounts previously earned and certified for payment:

1. When the first payment estimate is made, 25% of the "Mobilization" line item will be certified for payment.
2. When the Baseline Schedule, as specified under 1.20-1.05.08, is accepted, 50% of the "Mobilization" line item, minus any previous payments, will be certified for payment.
3. When 10% of the total original Contract price is earned and the Baseline Schedule, as specified under 1.20-1.05.08, is accepted, 75% of the "Mobilization" line item, minus any previous payments, will be certified for payment.
4. When 30% of the total original Contract price is earned and the Baseline Schedule, as specified under 1.20-1.05.08, is accepted, 100% of the "Mobilization" line item, minus any previous payments, will be certified for payment.

Project Closeout as defined in 1.20-1.03.01 shall include demobilization of plant and equipment, completion of all physical work, and administrative closeout items necessary to satisfy all Contract requirements. Project Closeout will be paid in the manner described hereinafter:

- When the non-administrative Project completion requirements (as specified under 1.20-1.08.13) and the administrative completion requirements (as specified under 1.20-1.08.14) have been satisfied, 100% of the "Project Closeout" line item will be certified for payment.

DIVISION II
CONSTRUCTION DETAILS

EARTHWORK

SECTION 2.01
CLEARING AND GRUBBING

2.01.01—Description
2.01.02—Materials (Vacant)
2.01.03—Construction Methods
2.01.04—Method of Measurement
2.01.05—Basis of Payment

2.01.01—Description: This work shall consist of clearing the ground of trees, stumps, brush, rubbish and all objectionable material in accordance with these specifications or as directed by the Engineer. This work shall also include the clearing of the ground necessary for the construction and installation of drainage, structures, ditches, channels, fences and other appurtenances. Included in this work shall be the preservation from injury or defacement of vegetation and objects designated to remain.

2.01.03—Construction Methods: The Contractor shall mark all trees, shrubs and plants to be removed in accordance with the clearing limits shown on the plans and these specifications. The Engineer shall have 7 days to field review the markings and make any adjustments prior to the start of the clearing operation. Within the excavation lines all trees shall be cut off and stumps removed to a depth of not less than 12 inches below the graded surface.

Within the fill lines where an embankment is to be made not more than 5 feet deep, trees, stumps, roots, etc., shall be removed. Where the embankments to be made exceed 5 feet deep, trees, stumps, roots, etc., shall be cut off to within 6 inches of the ground surface.

In areas where clearing is necessary for the construction and installation of various appurtenances, all trees and stumps shall be cut flush with the ground; and all dead or uprooted trees, brush, roots or otherwise objectionable material shall be removed as directed unless otherwise indicated on the plans.

Prior to clearing operations, a meeting must be held. Those attending the meeting should include, at a minimum, the Contractor, the Engineer, local tree warden or equivalent, and the District Environmental Coordinator. All clearing issues shall be resolved to the satisfaction of the Engineer before any trees are cut.

All trees scheduled to be removed outside of the proposed gutter or curb lines shall be visibly marked or flagged by the Contractor at least seven days prior to cutting of such trees.

The Engineer will inspect the identified trees within 7 days of the marking of the trees and verify the limits of clearing and grubbing prior to the Contractor proceeding with his cutting operation.

All branches of trees extending within the roadway shall be trimmed as directed to provide a 16-foot minimum vertical clearance including selective trimming of such trees as directed.

The Contractor shall dispose of all such trees, stumps, brush, etc., in a satisfactory manner and shall remove all rubbish and refuse from within the highway limits.

All excavations made below subgrade surface by the removal of trees, stumps, etc., shall be filled with suitable material, which shall be compacted thoroughly in accordance with the provisions governing formation of embankments.

All fences, stonewall fences and ornamental and utilitarian domestic accessories, such as, but not limited to garden pools, arbors, stair railings, fireplaces, sheds and incinerators, within the highway limits shall be removed as directed. However, the removal of materials in stonewalls, that are to be removed and not used in a new stonewall fences, will be paid for according to the provisions of 2.02.

All road signs, mail boxes, etc., shall be removed and reset as directed.

2.01.04—Method of Measurement: When no price for "Clearing and Grubbing" is asked for on the proposal form, the cost of the work as described above shall be included in the cost of the grading items and no direct payment for "Clearing and Grubbing" will be made.

When a price is asked for on the proposal form on a lump sum basis, this shall include all the work as described above, which may be necessary to properly complete the Project, unless the item is included under another Project pay item.

Should the Project be increased in length or the scope of work increased due to construction changes beyond the requirements hereinabove, any additional work required will be paid for as extra work. Should

the Project be decreased in length, a suitable credit, mutually agreed upon and based on the reduction in actual work or scope, will be taken by the State.

The work, material, tools, equipment and labor incidental to the disposal of trees, stumps, etc., will not be measured for payment.

2.01.05—Basis of Payment: Payment for this work will be at the Contract lump sum for "Clearing and Grubbing," except as noted above, and shall include all equipment, tools and labor incidental to the completion of this item.

All costs incidental to the disposal of trees, stumps, etc., will be included in the price of "Clearing and Grubbing."

Pay Item	Pay Unit
Clearing and Grubbing	l.s.

SECTION 2.02
ROADWAY EXCAVATION, FORMATION OF EMBANKMENT AND DISPOSAL OF SURPLUS MATERIAL

2.02.01—Description: Roadway excavation shall consist of the removal and satisfactory disposal, in the manner herein required, of all material taken from within the limits of the work contracted for, the removal of which is necessary for the construction of the roadway, subgrade, shoulders, slopes, entrances, retaining walls, gutters, channels, swales, concrete sidewalks, driveways and other miscellaneous construction to the dimensions and limits shown on the plans or as ordered by the Engineer and shall include the necessary excavation for pervious structure backfill outside of structure excavation limits. It shall also include the formation of embankments, the disposal of surplus or unsuitable material, removal of old foundations, concrete or masonry walls, crib walls, bin walls, stone wall fences or farm wall fences and filling of cellar or other holes, and in the absence of such items in the Contract, the clearing and grubbing and the shaping and cleaning of slopes and of shoulders.

Classification: Roadway excavation shall be classified for the purpose of payment as "Earth," "Rock," "Channel Excavation—Earth," or "Channel Excavation—Rock," in accordance with the following definitions. The classifications applying to any particular project shall be as indicated on the proposal form.

Earth Excavation shall include all materials removed as indicated or directed except water, "Rock," "Channel Excavation—Earth," and "Channel Excavation—Rock."

Rock Excavation shall include rock in definite ledge formation and boulders, or the portion of boulders, 1 cubic yard or more, in volume.

Channel Excavation—Earth shall include all materials other than water or "Channel Excavation—Rock" removed from the existing new or temporary water courses as indicated on the plans or as directed.

Channel Excavation—Rock shall include rock in definite ledge formation and boulders of 1 cubic yard or more in volume removed from existing, new or temporary water courses, as indicated on the plans or as directed.

2.02.03—Construction Methods: Excavation shall be made in conformity with the requirements of the plans and as ordered by the Engineer. The Contractor shall, when necessary in excavation areas, provide and maintain ditches which are adequate to prevent free water from becoming incorporated in material to be used to form embankments, such ditching to be at the sole expense of the Contractor. Where buildings have been removed to clear the way for construction or where old foundations, concrete or masonry walls exist, they shall be removed to 2 feet below the directed or finished grade; and all cellar and other holes shall be filled with suitable material.

1. **Sod and other organic matter** shall be disposed of as directed by the Engineer. In the event the Engineer does not direct the disposal of unsuitable material in conformance with 2.02.03-8, the Contractor has the option of disposing of the unsuitable material as specified within 2.02.03-10.
2. **Frozen material,** otherwise suitable for formation of embankments, shall be placed on embankment slopes or disposed of as directed by the Engineer. The Engineer may direct the removal of any portion of an accepted layer which has become frozen after placement and compaction. This frozen material shall be placed on embankment slopes or disposed of as directed by the Engineer. The removal and placement of frozen material shall be at the sole expense of the Contractor.
3. **Topsoil** shall be excavated within pavement and shoulder limits at grade points and to an elevation 3 feet below finish grade and elsewhere as directed. The material excavated may be used in the construction of embankments, if permitted by the Engineer, and shall be thus used if the Engineer so directs. In all cases, the use of topsoil in constructing embankments shall be restricted to locations where the surface of the underlying material is dry, its distance above the free water surface at the time of filling is more than 3 feet, and its distance below finish pavement grade is more than 5 feet.
4. **Excavation of Rock:** When rock is encountered, it shall be excavated to the slope lines and depths indicated on the plans.

The Contractor shall presplit the rock along the proposed rock slopes to the lines and inclinations shown on the plans except as otherwise provided in the specifications. Presplitting will be required where the backslope is designed at an inclination 1 (vertical) on 1 (horizontal) or steeper or where the cut in rock is 10 feet or more, measured on the inclination of the proposed slope from the bottom of excavation to the natural surface of sound rock. The maximum vertical height of slope face which can be presplit at the same

time shall be 50 feet.

The presplitting holes shall follow the required rock slope lines and inclinations. Hole drilling shall commence only when solid rock is encountered and exposed to the satisfaction of the Engineer. Unless otherwise permitted by the Engineer, presplitting holes shall have a spacing of 3 feet, center-to-center, and a diameter not greater than 3 inches. The holes shall be extended from the top of solid rock surface to the toe of finished rock slope, unless lesser depths are specified on the plans. The proper angle of drilling shall be maintained at all times so all presplit holes lie essentially in the same plane and are paralleled to each other. No holes shall deviate more than 6 inches at any place in the plane of the specified slope line nor in its vertical alignment. If any cut is presplit by vertical stages (lifts), the presplit holes may be offset, for each stage, a distance not more than 24 inches inside the previously presplit face. Presplit holes shall be lightly loaded with a continuous column charge manufactured especially for presplitting. All space in each hole not occupied by the explosive charge shall be filled with clean stone chips less than 3/8 inch in size or approved equal. Charges near the top of hole shall be reduced sufficiently to eliminate overbreak and heaving. The top charge shall not be less than 3 feet below the top of the drill hole. The methods of detonation shall be such that a uniform plane of rupture of the rock occurs from top to bottom and between presplit holes. If necessary, the Contractor shall adjust the methods as outlined above so as to result in a uniform plane of rupture in the rock.

Unless otherwise approved by the Engineer, presplit holes shall be drilled at least 50 feet ahead of, and shall be detonated prior to the drilling and blasting, the general pattern holes within the section of any lift of rock to be excavated. The presplitting shall be performed so as to produce a uniform plane of rupture in the rock such that the resulting rock face will not be affected by subsequent blasting and excavation operations.

In the general pattern, blasting following presplitting operations, no portion of any blast hole shall be drilled closer than 4 feet to the presplit face. No portion of any blast hole larger than 3 inches diameter shall be permitted closer than 12 feet to the presplit face. The spacing of blast holes, distribution and type of explosives, methods of detonation, and the blasting techniques shall be adjusted by the Contractor according to the characteristics and structure of the rock encountered so as not to fracture the rock beyond the presplit face.

Prior to any blasting, the Department will call a blasting conference at which the Contractor shall be represented to determine the methods to be used and the required protection to insure the utmost safety during blasting operations. The Contractor shall be responsible for all damage due either directly or indirectly to such operation.

The Contractor shall schedule his operations so that all rock excavation within a distance of 100 feet of bridges or other large structures, or any portion thereof, is completed to the required slope lines and depths before any structure work is started.

All loose and unstable material, even if located beyond the payment lines, and all breakage and slices shall be removed as directed and as the excavation for each vertical stage (lift) progresses. It shall be, at all times, the responsibility of the Contractor to perform all phases of this work to produce the required rock slope faces to the satisfaction of the Engineer.

Where indicated on the plans or as ordered by the Engineer, rock shall be excavated without the use of explosives. Excavation methods by the use of drilling, splitting, wedging or other approved methods not involving the use of explosives shall be utilized. The method selected by the Contractor shall allow excavation to the slope line(s) and depth(s) as shown on the plans and shall not affect in any way the material or structures outside the excavation line or grade.

5. Placement of Embankment Material: All excavated material and reclaimed waste obtained within the limits of the Project shall be used in the formation of embankments, except as provided elsewhere herein or as ordered by the Engineer.

Overhaul will not be allowed; but excavated material shall be transported where directed, provided the designated point of deposit is not more than 100 yards beyond the limits of the work contracted for, unless stated otherwise in the special provisions or plans.

When embankments are to be constructed on slopes steeper than 1:3, the slope of the existing ground on which the embankment is to be placed shall be plowed deeply or cut into steps before the filling is begun.

Embankments shall be constructed of earth, rock, reclaimed waste or a mixture thereof containing no more than 2% by weight of asphalt cement. The embankment shall be constructed by depositing successive layers of fill for the full width of the embankment, unless a partial width is permitted by the Contract or by the Engineer. If glass or clinker, or both, are included in reclaimed waste, their individual particles shall be no larger than 1 inch. Glass or clinker, or both, shall be thoroughly mixed with other embankment

materials such that their content anywhere in the embankment shall not exceed 25% by weight, with the exception that material placed within 5 feet from the face of the slope shall be free of glass and clinker. No embankment layer shall be deposited on surfaces of snow or ice, nor shall it be placed on frozen or unstable surfaces except under the conditions permitted elsewhere herein. If the Contractor is permitted to continue work, he shall remove, at no cost to the State, any frozen embankment material unless otherwise directed by the Engineer.

The depth of each layer, before compaction, shall not exceed 12 inches except as permitted hereinafter by these specifications, or with the permission of the Engineer.

The embankment shall be crowned or pitched to provide drainage at the close of each day's operations.

Where filling in 12 inch layers is impracticable, as in the case of filling in water or over slopes too steep for the operation of equipment, the embankment may be constructed in a single layer to the minimum elevation at which equipment can be operated, as determined by the Engineer; and above this elevation, the embankment shall be constructed as specified herein.

Embankments to an elevation 3 feet above the free water surface at the time of filling, shall be constructed of rock or free-draining material, or a mixture of both. Free-draining material shall conform to the requirements of M.02.07.

In fills where the top of the proposed pavement will be less than 4 feet above an existing flexible pavement, and the existing pavement is not required to be removed, it shall be scarified as directed by the Engineer.

In fills where the top of the proposed pavement will be less than 3 feet above an existing concrete pavement, including all bituminous resurfacing thereon, the concrete pavement shall be removed.

In fills where the top of the proposed pavement will be between 3 and 4 feet above an existing concrete pavement, the concrete pavement shall be broken in such a manner that complete fractures are obtained. Intact fragments, undamaged after breaking, shall not be larger than 2 s.f.

When present pavement not in cut or fill is removed, as called for on the plans or directed, the area shall be backfilled with a suitable earth material which shall be free from admixture of subsoil, refuse, stumps, roots, rocks, brush, weeds and other material which will prevent the formation of a suitable seed bed.

Wherever portions of existing concrete pavement are to be removed, such removals shall be made to neat lines. The areas in which such concrete surfaces are to be removed will be delineated by the Engineer before such work is done. Where no break or joint exists in the concrete pavement at the line of delineation, a kerf, at least 2 inches, but no more than 3 inches deep, shall be made in the concrete with an approved concrete cutting saw. The concrete shall then be removed from within the delineated area exercising extreme care to avoid "breakbacks" beyond the kerf, break or joint. Concrete pavement remaining in place shall have vertical edges, and that portion below the kerf shall be reasonably smooth.

Wherever portions of existing bituminous concrete pavement are to be removed, they shall be removed to neat lines as shown on the plans or as directed by the Engineer. Where the delineated limits of the areas in which such bituminous surfaces are to be removed are adjacent to existing bituminous concrete pavement that is to remain in place, the line of delineation shall be cut by a method approved by the Engineer.

When the embankment material consists predominantly of rock fragments and/or fragments of reclaimed waste of such size that material cannot be placed in horizontal layers of the thickness specified above without crushing or further breaking down the pieces resulting from the excavation methods, such material may be placed in the embankments in horizontal layers not exceeding 3 feet thick. Large stones or fragments of reclaimed waste shall not be placed in nests but shall be distributed over the area; and the interstices shall be filled with spalls, finer fragments or earth to form a solid, compact mass.

The entire area of each layer shall be leveled off by suitable grading equipment and shall be compacted as hereinafter specified.

In portions of embankments where piles are to be driven, the Contractor shall not place any material which might interfere with pile driving operations. The correction of any condition which interferes with the pile driving operations in embankments constructed under the Contract shall be made by the Contractor at no cost to the State.

Rock fill or reclaimed waste containing fragments with their greatest dimension over 12 inches shall not be placed above an elevation which is 2 feet below the top of the embankment.

Particle with their greatest dimension over 5 inches shall not be placed within 12 inches of the elevation of the top of the prepared subbase unless otherwise specifically authorized.

Prior to the formation of any embankment, the Contractor may submit a plan in a form acceptable to the Engineer for grading operations detailing the location of embankment material sources and points of

deposit for the entire Project in order to qualify for payment for overhaul as hereinafter provided. Failure to submit such a plan will be construed as a waiver of any and all rights to payment for overhaul.

6. Compaction: The entire area of each layer of the embankment and the subgrade in the excavated areas shall be uniformly compacted to at least the required minimum density by use of compaction equipment consisting of rollers, compactors or a combination thereof. Earth-moving and other equipment not specifically manufactured for compaction purposes will not be considered as compaction equipment.

The dry density after compaction shall not be less than 95% of the maximum dry density for that soil when determined by the Contractor in accordance with AASHTO T 180 and measured in-place with ASTM D6938 or other methods approved by the Engineer.

The Contractor shall perform in-place density testing at a sufficient frequency to ensure that the specified results are continuously met. The Contractor shall submit complete field density testing and inspection records to the Engineer within 48 hours (excluding weekends and holidays) of the test in a manner acceptable to the Engineer.

7. Stability: If after full compliance with the requirements of these specifications with regard to excavation, placement and compaction density requirements, a stable embankment or subgrade has not been obtained, the Contractor shall proceed to perform such corrective work as is necessary to produce a stable embankment or subgrade. This work may include, but not necessarily be limited to control of moisture to within limits suitable for obtaining the required stability; blending with rock or granular material from any Project excavation or borrow, or free draining material or any combination thereof; removal and replacement with acceptably compacted material or a combination of these methods approved by the Engineer. Any of the foregoing methods may be supplemented by suspending embankment operations and allowing the material to dry.

When embankments are constructed of material from Project excavation and the Engineer determines that the material properly compacted is not sufficiently stabilized, the cost of corrective measures directed by the Engineer shall be paid for at applicable Contract unit prices, or in the absence thereof, as extra work.

If the corrective work on embankments constructed of Project excavation is necessary, and requires departure from the plan for grading operations to obtain material at locations other than shown on the aforementioned plan which result in increased net costs to the Contractor, such increased costs will be paid as extra work, except that no payment will be made for overhaul for any corrective material for the first 1/2 mile of increased hauling distance.

When embankments are constructed of borrow, they shall be stable. In this case however, the methods and material used to obtain such stability shall be determined by the Contractor and performed at its expense.

8. Surplus Excavated Material: All surplus excavated material shall be used where directed by the Engineer, to uniformly widen embankments, to flatten slopes, to fill low places in the right of way, or for such other purposes as the Engineer may direct, provided the area designated for deposit does not conflict with 2.02.03-5.

Any surplus or unsuitable material not required, nor permitted to be used for such purposes, shall be disposed of in accordance with 2.02.03-10.

9. Fences: The Contractor shall erect either the permanent or temporary fence, to the satisfaction of the Engineer, at all points where the land is used for pasturing and where the existing fences are affected by the grading operations. Permanent or temporary fences shall be erected prior to the removal or destruction of any part of the existing fence, and any temporary fence erected shall be removed when no longer required. There will be no direct payment for any temporary fence erected, but the cost thereof shall be considered as included in the cost of the grading operations; permanent fences will be paid for at the Contract unit price for this item.

10. Disposal of Unsuitable Material: When the Engineer has not directed otherwise, the Contractor has the option to dispose of unsuitable excavated material by either or both of the following methods:

(a) By removing such material from within the limits of the highway and disposing of such material at point or points as the Contractor shall determine provided this does not create any detrimental effects to the Project and the Engineer does not object to the area selected for disposal.

(b) By disposing of such material within the limits of the highway in accordance with the details and requirements shown on the plans and the following requirements:

Not less than 15 days prior to disposing of any such material within Project limits, the Contractor shall submit to the Engineer for his approval a proposal delineating the locations and extent of the areas in which he intends to dispose of such material. The proposal shall describe the nature of the material and the

methods to be employed in placing and covering the material. The proposal will be reviewed for its effects on the completed construction and the environment. The proposal shall be amended as required by the Engineer. No such material shall be disposed of within Project limits until the proposal has been approved by the Engineer.

All work shall be done in accordance with the approved proposal.

Suitable material excavated for the disposal of the unsuitable excavated material shall be placed in the embankment in accordance with the requirements of 2.02 or shall be used to cover the unsuitable excavated material.

The State does not guarantee nor imply that the areas available for disposal of unsuitable excavated material within Project limits will be adequate for the disposal of all unsuitable excavated material to be removed. The Contractor shall dispose of all unsuitable excavated material in excess of that which can be placed within the permitted areas.

11. Slopes: Earth slopes with a degree of slope from 2:1 to 5:1 shall be tracked unless the Engineer directs that they shall not be tracked. Tracking shall consist of traversing the slopes with cleated tracks so that the cleat indentations are horizontal. Where topsoil is to be placed on slopes, the tracking shall be done prior to the installation of the topsoil.

Tracking is not to be construed to be used for slope compaction. Its sole purpose is to provide indentations in the slope to help reduce soil erosion. Other methods of achieving the desired results may be used, with the permission of the Engineer.

12. Concrete sidewalks or driveway ramps: Wherever portions of concrete sidewalks or driveway ramps are to be removed, such removals shall be made to neat lines. Partial removals shall generally be at existing joints except when a location other than a joint is set as the limit by the Engineer due to construction staging limits. At removal limits where a joint is not present, the Contractor shall saw cut the concrete full depth to create a neat line.

2.02.04—Method of Measurement: Payment lines for earth excavation shall coincide with the slope and subgrade lines or the top of the payment lines for ditch excavation, whichever applies, as shown on the plans or as ordered. The amount of excavation will be determined as described below by the average end area method, or by a method approved by the Engineer.

Payment lines for unsuitable material excavation shall be the area designated by the plans, special provisions or the Engineer as unsuitable material below the subgrade in cut sections, below the original ground line in fill sections and beyond the normal payment lines for ditch and channel excavation.

Unsuitable material within the slope and subgrade lines or the top of the normal payment lines for ditch and channel excavation shall be measured as earth excavation, ditch excavation or channel excavation.

Any stockpiling, drying or re-excavation necessary to utilize such material on the Project shall not be measured for payment, but shall be included in the payment for unsuitable material.

Also measured for payment shall be the volume of earth moved in cutting or plowing of steps on steep slopes, as described in 2.02.03, and the removal of existing flexible pavement where shown on the plans or ordered by the Engineer.

The stockpiling, re-excavation and final placement of material will not be measured for payment, unless such has been made a part of the Contract or unless the State has created conditions different from those that existed or could have been foreseen or anticipated when the Contract was bid.

Payment lines for Channel Excavation—Earth shall coincide with the side slopes and bottom of channel as shown on the plans or as directed.

Payment lines for Channel Excavation-Rock shall coincide with the depth shown on the plans or to the depth ordered. Payment lines for slopes will be extended to a limit of 12 inches outside of and parallel to the slope lines shown on the plans, or as ordered, to include rock actually removed within this limit. In case of natural faults or fissures which make the removal of additional rock necessary for reasons of safety, or which produce slides clearly not attributable to the Contractor's method of operation, the slope payment lines will be fixed to coincide with the natural faults or fissures of the rock.

Payment lines for rock excavation, where presplitting bedrock is required by these specifications, will extend to the slope and depth line shown on the plans or as directed, to include only the rock actually removed within this limit.

Payment lines for rock excavation, where presplitting bedrock is not required by these specifications, shall coincide with the depth shown on the plans or to the depth directed; and payment lines for the slopes will be extended to a limit of 1 foot outside of and parallel to the slope lines shown on the plans, or as directed,

to include rock actually removed within this limit. Where removal of rock is necessary for reasons of safety or due to conditions clearly not attributable to the Contractor's method of operation, the payment lines will be fixed to coincide with limits ordered by the Engineer.

Presplitting of bedrock performed in accordance with these specifications will not be measured for payment.

Where removal of rock is necessary for reason of safety or due to conditions clearly not attributable to the Contractor's methods of operation, the payment lines for rock excavation where presplitting is required will be fixed to coincide with limits ordered by the Engineer. Payment lines for Rock Excavation (No Explosives), where mechanical means of removal are required by these specifications, will extend to the slope and depth line(s) shown on the plans or as directed, to include only the rock actually removed within these limits.

Concrete and masonry foundation walls, or portions thereof, to be removed will be measured for payment by the volume in cubic yards, in place, before removal.

Existing concrete pavement and concrete base over 5 s.y., including any bituminous surfacing material immediately thereon, shall be measured in place before removal.

Existing concrete and cement masonry structures over 1 c.y., shall be measured in place before removal.

Existing concrete sidewalks or driveway ramps shall be measured in place before removal by the number of square yards of concrete sidewalk or ramp to be removed.

When rock is encountered, and its removal is to be paid for as "Rock Excavation" or "Channel Excavation—Rock," the Contractor shall strip or expose the rock to such an extent that in the Engineer's opinion the necessary measurements can be taken. The Contractor shall notify the Engineer at least 2 days prior to disturbing any of the rock to allow ample time to obtain the necessary measurements. If the Contractor shall fail to give such notice, or remove any rock prior to the taking of the measurements, the Engineer shall presume that measurements taken at the time the Engineer first sees the material in question will give a true quantity of excavation.

The amount of excavation will be determined by the average end area method, or by a method approved by the Engineer.

The work of scarifying existing pavement will not be measured for payment, but the cost shall be considered as included in the general cost of the Contract.

The work of cutting concrete pavement will be measured for payment by the number of linear feet of saw cut made with an approved concrete saw to the lines delineated by the Engineer on the concrete pavement.

The cutting of bituminous concrete pavement will be measured for payment by the number of linear feet of cut made by an approved method to the lines delineated on the plans or as directed by the Engineer. Cuts made necessary by the Contractor's operation, such as, but not limited to, patching, bituminous concrete samples, continuance of previous runs, faulty work or faulty materials will not be measured for payment. Bituminous parking areas are considered as bituminous concrete pavement.

The work, materials, tools, equipment and labor incidental to the disposal of unsuitable excavated material or breaking concrete pavement will not be measured for payment.

2.02.05—Basis of Payment: Roadway excavation will be paid for at the Contract unit price per cubic yard for "Earth Excavation," "Rock Excavation," "Rock Excavation (No Explosives)," "Channel Excavation—Earth," or "Channel Excavation—Rock" as the case may be, in accordance with the classification given herein and subject to the method of measurement described above. The price shall include all equipment, tools, compaction testing and labor incidental to the completion of the excavation, the formation and compaction of embankments, and the disposal of surplus or unsuitable material in accordance with the provisions of the plans and of these specifications.

The removal of concrete pavement or concrete base will be paid for at the Contract unit price per square yard for "Removal of Concrete Pavement," including any bituminous surfacing material immediately thereon.

The removal of concrete sidewalk or concrete driveway ramp will be paid for at the Contract unit price per square yard for “Removal of Concrete Sidewalk” which price shall also include cutting concrete at neat lines and all disposal costs.

The removal of concrete or cement masonry structures over 1 c.y., other than retaining walls or bridge structures, will be paid for at the Contract unit price per cubic yard for "Rock Excavation" or "Unclassified Excavation," as the case may be.

The removal of drainage structures outside of the limits of Roadway Excavation, Structure Excavation and

proposed drainage installations will be paid for under “Remove Drainage Structure – 0’ – 10’ Deep” or “Remove Drainage Structure – 0’ – 20’ Deep.”

Concrete and masonry foundation walls or portions thereof ordered removed will be paid for at the Contract unit price per cubic yard for "Rock Excavation" or "Unclassified Excavation," as the case may be.

The removal of retaining walls and bridge substructures will be paid for under the item "Removal of Existing Masonry."

The removal of crib walls, bin walls, stone wall fences or farm wall fences will be paid for as Earth Excavation.

The removal of all pavement or pavement bases other than concrete will be paid for at the Contract unit price per cubic yard for "Earth Excavation."

The work of cutting concrete pavement will be paid for at the Contract unit price per linear foot for "Cut Concrete Pavement" including any bituminous surfacing material immediately thereon, which price shall include all materials, equipment, tools and labor incidental thereto.

The work of cutting bituminous concrete pavement will be paid for at the Contract unit price per linear foot for "Cut Bituminous Concrete Pavement" which price shall include all materials, equipment, tools and labor incidental thereto.

Unsuitable material excavation outside of the limits of earth, unclassified, ditch and channel excavation will be paid for at the Contract unit price per cubic yard for "Unsuitable Material Excavation," which price shall include all equipment, tools, labor and material incidental thereto.

All costs incidental to the disposal of unsuitable excavated material will be included in the price for "Earth Excavation."

When no item for "Channel Excavation—Rock" appears in the proposal and rock, conforming to the description given under "Channel Excavation—Rock" in 2.02.01 is encountered in the channel excavation, the rock so encountered and removed will be classified and treated as "Channel Excavation—Rock," and its removal will be paid for at 300% of the Contract unit price per cubic yard for "Channel Excavation—Earth."

All costs incidental to breaking concrete pavement shall be considered as being included in the general cost of the Contract.

Pay Item	Pay Unit
Rock Excavation (No Explosives)	c.y.
Earth Excavation	c.y.
Rock Excavation	c.y.
Channel Excavation—Earth	c.y.
Channel Excavation—Rock	c.y.
Cut Concrete Pavement	l.f.
Cut Bituminous Concrete Pavement	l.f.
Removal of Concrete Pavement	s.y.
Removal of Concrete Sidewalk	s.y.
Unsuitable Material Excavation	c.y.

NOTES

SECTION 2.03
STRUCTURE EXCAVATION

2.03.01—Description
2.03.03—Construction Methods
2.03.04—Method of Measurement
2.03.05—Basis of Payment

2.03.01—Description: With the exceptions noted below, structure excavation shall include the removal of all material of whatever nature, the removal of which is necessary for the construction of foundations of bridges, box culverts, retaining walls outside the earth excavation payment limits, and other structures shown on the plans; the placing of all necessary fill with the exception of pervious structure backfill hereinafter specified; and the wasting of excavated material which is not required for backfilling or embankment, or which is unsuitable for that purpose.

This item shall also include dewatering; the design and construction of all cofferdams and related environmental controls used in dewatering operations required for the execution of the work; the repair, reconstruction and removal of cofferdams and related environmental controls used in dewatering operations; and the removal of all obstructions necessary for the construction of cofferdams. However, dewatering and the construction of a cofferdam will not be required under any structure excavation item for any foundation or structure that has an item for "Cofferdam and Dewatering" shown on the plans and in the proposal estimate for a given location. In such instances, the provisions of 2.04 shall govern for the items described in 2.03.05 (b) below.

"Structure Excavation—Earth" and "Structure Excavation—Rock" are defined as follows:

1. **"Structure Excavation—Earth"** shall include removal of all materials, other than water or "Structure Excavation—Rock."
2. **"Structure Excavation—Rock"** shall include the removal of rock in definite ledge formation; boulders or portions of boulders that have a volume of 1 c.y. or greater; and masonry structures of 1 c.y. or more in volume, except retaining walls and bridge substructures, the removal of which is covered by 9.74.

2.03.03—Construction Methods:

1. **Dimensions and Elevations of Footings:** The elevations of the bottom of footings, as shown on the plans, shall be considered as approximate only; and the Engineer may order, in writing, such changes in dimensions or elevations of footings as may be necessary to secure a satisfactory foundation.
2. **Preparation of Foundations:** All rock or other suitable foundation material shall be cleared of all overlying material, cleaned and cut to a firm surface, either level, stepped or serrated, as directed by the Engineer. All seams shall be cleaned out and filled with concrete, mortar or grout. Any over-breakage in rock more than 6 inches below the plan grade for the bottom of the footing not authorized by the Engineer shall be replaced by the Contractor with Class PCC03340 Concrete at the Contractor's expense.

When the structure is to rest on a material other than rock, special care shall be taken not to disturb the material below the bottom of the excavation, and the final removal of the foundation material to grade shall not be made until just before the forms for concrete or masonry are placed. Any foundation material disturbed below plan grade or revised plan grade shall be dressed and compacted at the Contractor's expense. This shall not apply, however, when a granular fill foundation course is required.

3. **Cofferdams:** If a cofferdam is required under this item, it will be designed and constructed in accordance with the provisions of 2.04.03-1 and 2.04.03-3.
4. **Dewatering:** If a cofferdam is required under this item, dewatering will be performed in accordance with the provisions of 2.04.03-2.
5. **Inspection:** After each excavation is completed, the Contractor shall notify the Engineer and no construction shall be started until the Engineer has approved the depth of the excavation and the character of the foundation material.
6. **Fill Adjacent to Structures:** All spaces excavated and not occupied by the abutments, piers, other permanent work or pervious structure backfill shall be filled to the surface of the surrounding ground with suitable material. Such backfill shall be thoroughly compacted and neatly graded.

Fill placed around arches, rigid frames, box culverts and piers shall be deposited on both sides of the structure to approximately the same elevation at the same time.

Each layer of backfill shall be spread to a thickness not exceeding 6 inches deep after compaction and shall be thoroughly compacted by the use of power rollers or other motorized vehicular equipment, by tamping with mechanical rammers or vibrators, or by pneumatic tampers. Any equipment not principally

manufactured for compaction purposes or which is not in proper working order in all respects shall not be used within the area described above.

Special attention shall be given to compaction in places close to walls where motorized vehicular compaction equipment cannot reach. Within 3 feet of the back face of walls and within a greater distance at angle points of walls, each layer of backfill shall be compacted by mechanical rammers, vibrators or pneumatic tampers.

The dry density of each layer of backfill after compaction shall not be less than 95% of the maximum dry density for that material when determined by the Contractor in accordance with AASHTO T180 and measured in-place with ASTM D6938 or other methods approved by the Engineer.

The Contractor shall perform in-place density testing at a sufficient frequency to ensure that the specified results are continuously met. The Contractor shall submit complete field density testing and inspection records to the Engineer within 48 hours (excluding weekends and holidays) of the test in a manner acceptable to the Engineer.

Adequate provision shall be made for the drainage of all fill in accordance with the provisions of the plans, or as ordered by the Engineer.

No fill shall be placed against any structure until the Engineer has given permission to do so, and in no case until after the permitted time for removal of forms.

2.03.04—Method of Measurement: The Contractor shall notify the Engineer before starting any excavation, so that elevations and the measurements of the excavation area may first be obtained. When ledge rock is encountered, the Contractor shall notify the Engineer and shall strip or expose the rock to such an extent that in the Engineer's opinion the necessary measurements can be taken for "Structure Excavation—Rock." If the Contractor fails to give such notice or notices, or removes any material prior to the taking of measurements, the Engineer may presume that measurements taken at the time the Engineer first saw the material in question indicates the true quantity of excavation.

Vertical payment limits will be measured for payment as follows:

1. Structure Excavation—Earth will be measured in place by taking the difference in elevation between the existing ground surface or the bottom of roadway excavation or channel excavation, whichever is lower, and the surface of the completed structure excavation at plan grade or approved revised plan grade.

Structure excavation in roadway cuts, or embankment areas where the removal of unsuitable material is indicated on the plans, shall include only the portion below the bottom of the unsuitable material or subbase, if any, or the subgrade, shoulder foundation or cut slope lines, as the case may be or as may be more specifically shown on the plans.

2. Structure Excavation—Rock will be measured in place by taking the difference in elevation between the existing ledge rock or bottom of roadway excavation or channel excavation, whichever is lower, and the bottom of the actual completed and accepted structure excavation, except that any excavation to a depth greater than 6 inches below the plan grade or revised plan grade, will not be measured for payment.

Horizontal payment limits for "Structure Excavation—Earth" and "Structure Excavation—Rock" will be measured between plumb lines 2 feet outside of the neat lines of the original foundations only, unless otherwise shown on the plans and unless the size of the footing is increased more than 2 feet in length or width (or both), in which case the area of the excavation that extends beyond the original Contract payment limits, will be used for determining the additional amount of excavation.

2.03.05—Basis of Payment: Payment for this work will be made at the Contract unit price per cubic yard for:

(a) Structure Excavation—Earth (complete) or "Structure Excavation—Rock (complete)," whichever applies, in whole or in part, which price shall include all materials, tools, and equipment; all work related to cofferdams, including their design, construction, dewatering, repair, removal of obstructions, and any required reconstruction; all labor necessary to complete the excavation in accordance with the requirements of the plans or as ordered by the Engineer; the preparation of foundations as described under 2.03.03 including compaction testing; all necessary filling, except as otherwise provided in the Contract; and the removal of all surplus or unsuitable material resulting from the excavations. Any suitable surplus material shall be placed in the embankments, if so ordered by the Engineer, without additional compensation.

(b) "Structure Excavation—Earth (excluding Cofferdam and Dewatering)" or "Structure Excavation—Rock (excluding Cofferdam and Dewatering)," whichever applies in whole or in part, which price shall include all materials, tools, equipment and labor necessary to complete the excavations in accordance with the requirements of the plans or as ordered by the Engineer. It shall also include the preparation of foundations as described under 2.03.03 including compaction testing, the necessary filling, except as otherwise provided in the Contract, and the removal of all surplus or unsuitable material resulting from the excavations. Any suitable surplus material shall be placed in the embankments, if so ordered by the Engineer, without additional compensation.

Should it become necessary to change the dimensions of the footings from those shown on the plans or to excavate below the elevation shown on the plans, payment will be made in accordance with the following provisions:

1. The length or width (or both) may be increased horizontally not more than 2 feet and the depth of excavation increased not more than 2 feet without change in the unit price as specified above.
2. If the depth of the excavation is increased more than 2 feet, but not more than 10 feet below the original plan grade, payment for excavation below an elevation 2 feet below the elevation shown on the plans and within the horizontal payment limits as specified above, will be at the Contract unit price plus 100% thereof.
3. When the size of the footing is increased horizontally more than 2 feet in length or width, or both, excavation actually required outside the horizontal payment limits defined above will be paid for at the Contract unit price plus l00% thereof.
4. In the event the depth of the excavation has to be increased to a depth greater than 10 feet below the original plan elevation, the excavation actually made below the 10 foot limit will be considered extra work and will be paid for in accordance with l.04.05.

Pay Item	Pay Unit
Structure Excavation—Earth (complete)	c.y.
Structure Excavation—Rock (complete)	c.y.
Structure Excavation—Earth (excluding Cofferdam and Dewatering)	c.y.
Structure Excavation—Rock (excluding Cofferdam and Dewatering)	c.y.

SECTION 2.04
COFFERDAM AND DEWATERING
COFFERDAM MATERIAL LEFT IN PLACE

2.04.01—Description: Work under this item shall consist of the design, construction, maintenance and removal of a cofferdam, and necessary dewatering within the cofferdam, as shown on the plans.

If designated on the plans, the installed cofferdam material shall be left in place.

For the purposes of this specification, cofferdam shall be understood to mean a structure, the type of which the Contractor elects to build, to fully enclose and confine an area to be pumped dry to enable construction to be carried out.

2.04.03—Materials: Sheet pile material left in place shall meet the requirements of ASTM A328.

2.04.03—Construction Methods: The Contractor shall submit to the Engineer cofferdam working drawings in accordance with 1.05.02. The Contractor's proposed design must meet all requirements established in regulatory permits for the Project, the requirements of 1.10, and any stage construction configurations.

1. Cofferdams: Construction of the cofferdam shall be carried to the height shown on the plans and to an adequate depth. The cofferdam shall be constructed so that the work within can be safely carried to the bottom of the structure excavation.

The interior dimensions of the cofferdam shall be sufficient for the unobstructed and satisfactory completion of all necessary substructure work, including but not limited to pile driving, form building, inspection and pumping.

The Contractor shall be responsible for maintenance of the cofferdam. If the cofferdam becomes tilted or displaced prior to the completion of all work to be done within, the cofferdam shall be righted, reset, or enlarged as may be necessary to provide the clearance for the unobstructed performance of all necessary work.

The cofferdam shall be completely dewatered as required to complete the work entirely in the dry, except as specified below.

When conditions are encountered that render it impractical to dewater the cofferdam, the Engineer may require the placing of underwater concrete of such dimensions as will be necessary to allow the Contractor to complete the work in the dry. The placement of underwater concrete shall comply with 6.01.03-II-6.

The cofferdam must be constructed to protect uncured masonry and concrete against damage from a sudden rising of the water and prevent damage to structure foundations by erosion. No part of the cofferdam which extends into the substructure may be left in place without written permission from the Engineer.

2. Dewatering: Pumping from the interior of any cofferdam shall be done in such a manner as to preclude the possibility of water moving through uncured masonry or concrete. During the placement of concrete or masonry, and for at least 24 hours thereafter, any pumping shall be done from a suitable sump located outside the horizontal limits and below the elevation of the work being placed or as directed by the Engineer.

The pumped water must be discharged in accordance with the requirements of 1.10. Pumping to dewater a cofferdam shall not start until any underwater concrete has sufficiently set to withstand the hydrostatic pressure created by pumping.

3. Removal of Cofferdam: Unless designated on the plans or directed by the Engineer, the Contractor shall remove all parts of the cofferdam and associated dewatering components after completion of the required work. This shall be done in such a way as not to disturb or otherwise damage any permanent construction.

4. Cofferdam Material Left in Place: Sheet piling used in constructing the cofferdam may be designated on the plans to be left in place. The sheet piling shall be cut off at elevations shown on the plans or approved in advance by the Engineer, and the cut off portions shall be removed by the Contractor from the Site.

2.04.04—Method of Measurement:

1. Cofferdam and dewatering will be measured for payment by the actual quantity installed and accepted, in linear feet along the centerline of the top of the cofferdam.

If the cofferdam becomes tilted or displaced prior to the completion of all work to be done within, the corrections and adjustments of the cofferdam will not be measured for payment.

2. Cofferdam material left in place will be measured for payment by the actual quantity of linear feet of material left in place and accepted by the Engineer.

2.04.05—Basis of Payment:

1. Cofferdam and Dewatering: Payment for this work will be made at the Contract unit price per linear foot for "Cofferdam and Dewatering," measured as described above, which price shall include all costs of design, materials, equipment, labor, work, and any related environmental controls used in dewatering operations, which are required for the construction of the cofferdam shown in the plans; of any repair, correction, adjustment or reconstruction of such cofferdam as required by the plans; removal of obstructions; pumping and dewatering; removal of such cofferdam, and related environmental controls used in dewatering operations.

If the total number of linear feet of the cofferdam as accepted by the Engineer is greater than the quantity as designated on the original Contract plans, the Department will pay the Contractor for the revised number of such linear feet at the Contract unit price, subject to the provisions of 1.04.02 and 1.04.03.

If the Engineer allows the addition or enlargement of a cofferdam for the convenience or other benefit of the Contractor, but does not deem it essential for the performance of the Contract work, no additional payment will be made for the cofferdam or portion of the cofferdam which the Engineer does not deem essential.

2. Cofferdam Material Left in Place: In addition to Cofferdam and Dewatering, that portion of the cofferdam designated on the plans or ordered to be left in place will be paid for at the Contract unit price per linear foot for "Cofferdam Material Left in Place," which price shall include the cost of the sheet piling material left in place, the work to cut the sheet piling and removal of the cut off portions from the Site and all work incidental thereto.

Pay Item	Pay Unit
Cofferdam and Dewatering	l.f.
Cofferdam Material Left in Place	l.f.

SECTION 2.06
DITCH EXCAVATION

2.06.01—Description: Ditch excavation shall consist of the removal and satisfactory disposal of all materials except water, the removal of which is necessary for the construction of drainage ditches, paved ditches, and paved leak-offs, except as noted below. Not included herein is excavation for gutters and channels.

2.06.03—Construction Methods: Ditch excavation shall be made in accordance with the requirements of the plans or as ordered by the Engineer. The excavated material shall be removed from the area adjoining the excavation and the suitable excavated material used in the embankments, unless otherwise directed by the Engineer. Unsuitable materials shall be disposed of as directed. The top of the banks of the ditches shall be graded and left in a neat and acceptable condition.

2.06.04—Method of Measurement: Ditch excavation will be measured for payment according to conditions and between the limits set forth in the following:

1. **Drainage Ditches (unpaved):**
   - (a) Within normal cut slopes, excavation will be measured under 2.02.04
   - (b) On fill slopes, between the surface of the embankment and the bottom of the excavation.
   - (c) In areas other than (a) and (b), above, between the original ground surface and that after excavation is completed.
2. **Paved Leak-offs and Paved Ditches:**
   - (a) Within normal cut slopes, between the limits of the top of paved surface and the bottom of the base course.
   - (b) Within normal fill slopes, between the surface of the embankment and the bottom of the base course.
   - (c) Outside of cut or fill slopes, between the ground surface and that after the excavation is completed.

Before starting any excavation, the Contractor shall notify the Engineer so that elevations and measurements of the work may be obtained. Any work done prior to such notification will not be paid for.

If rock meeting the description given under 2.86.01 is encountered, the Contractor shall strip it of sufficient overlying material to allow for proper measurement. The Contractor shall notify the Engineer at least 2 days prior to disturbing the rock that the rock surface is ready for measurement. If the Contractor shall fail to give such notice, or remove any rock prior to the taking of such measurement, the Engineer shall presume that the measurements taken at the time the Engineer first saw the material in question will give the true quantity of excavation.

2.06.05—Basis of Payment: This work will be paid for at the Contract unit price per cubic yard for "Ditch Excavation" or "Rock in Ditch Excavation," whichever may apply in whole or in part. These prices shall include all equipment, tools and labor necessary to complete the work, dispose of the excavated material and leave the Site in a neat and workmanlike condition.

When no item for "Rock in Ditch Excavation" appears in the Contract and rock meeting the description given under 2.86.01 is encountered in ditch excavation, its removal will be paid for at 500% of the Contract unit price per cubic yard for "Ditch Excavation."

Pay Item	Pay Unit
Ditch Excavation	c.y.
Rock in Ditch Excavation	c.y.

SECTION 2.07
BORROW

2.07.01—Description: When the amount of usable material excavated within the limits of the work contracted for is not sufficient to form the embankments and other features of the work, additional material shall be furnished by the Contractor from borrow pits obtained at its expense and located beyond the limits of the highway. This material shall be known as borrow. Borrow shall include the furnishing, removing and satisfactory placing of the additional material necessary to complete the embankments and other features of the work.

Stockpiled hydraulically dredged and/or reclaimed waste material containing no more than 25% of glass or clinker by weight and no more than 2% by weight of asphalt cement may be used for part or all borrow requirements. Stockpiles shall be placed at locations arranged for and provided by the Contractor at its expense and will not be allowed within the highway limits except at locations and under conditions stipulated by the Engineer. All permits, easements, rights or other requirements related to dredging and stockpiling shall be the obligation and responsibility of the Contractor.

2.07.02—Materials: Borrow, excluding hydraulically dredged borrow, shall meet the requirements of 2.02.03 for performance when incorporated in embankments. For purposes other than embankments, it shall be of a satisfactory quality as determined by the Engineer for the purpose intended. Hydraulically dredged borrow shall contain no more than 20% by dry weight of material passing the No. 200 sieve when placed in the embankment.

2.07.03—Construction Methods: Borrow will be permitted only to the extent necessary to complete the embankments and similar details and only after all usable material from the excavation has been placed. With the approval of the Engineer, the Contractor may be permitted to place borrow before the excavation is completed; but will be held responsible for the proper placing of all suitable excavated material, and no payment will be allowed for any borrow placed in lieu of suitable excavated material. This permission may be revoked by the Engineer at any time if in his opinion satisfactory progress is not maintained on other operations.

The Contractor shall notify the Engineer at least 5 days prior to obtaining material from any borrow pits so that an examination may be made of the fitness of the material and so that the necessary measurements may be taken. The limits of the proposed borrow pit shall be shown to the Engineer; and prior to the taking of measurements, the Contractor shall be required to clear the area of all unsuitable material. No payment will be made for any material removed outside of the area measured or which was not used in the work. No excavation shall be made within the confines of a borrow pit after the Engineer has taken original measurements, other than for material to be used in the formation of embankments or elsewhere as shown on the plans or as directed by the Engineer; except that material may be taken for Contract items other than borrow with the permission of the Engineer under terms and conditions set forth in writing.

If the Contractor elects to obtain borrow from a commercial pit, it shall be necessary to have a section of the pit set off for the Contractor's use solely, so that the amount of material removed may be accurately determined. During the period between the original and the final measurements and release by the Engineer, no material shall be taken from a measured borrow pit except by the Contractor for use in the work under this Contract. Borrow pits shall be excavated to regular lines as staked, so as to permit accurate measurements, and shall be drained and left in a neat condition as directed.

If the Contractor elects to obtain borrow from a stockpile of reclaimed waste, approval shall be obtained from the Engineer prior to removing any material from the stockpile to the job Site. Once the stockpiles have been tested and approved, no additional material shall be added to them.

If the Contractor uses hydraulically dredged material, the Contractor shall, at its expense, employ effective dust control measures so that the public will not be adversely affected by dust from stockpiled material or such material in transit. The Contractor shall also provide and maintain at its expense effective and adequate drainage for all dredging and stockpiling operations at all times, and shall be solely responsible for all damages which result from dredging and stockpiling and for the continuing maintenance and final restoration of all drainage facilities affected by its operations.

Stockpiles of hydraulic material shall be reserved for exclusive use of the State and be placed so as to be readily measured, and no area shall be reused for stockpiling prior to its being re-measured by the Engineer. Dredging to a stockpile and removing material therefrom simultaneously is expressly prohibited.

Borrow shall be placed where directed and in accordance with the provisions for the formation of embankments of 2.02.03.

2.07.04—Method of Measurement: Except as provided under (a), (b), (c), (d) and (e) below, the amount of borrow to be paid for will be determined by using the results of cross-sectional elevations taken before and after the borrow material has been excavated from the pit or stockpile in the average end area method, or by a method approved by the Engineer. Measurements of stockpiles will not be taken until they are firm and can be measured safely and accurately.

(a) If, in the opinion of the Engineer, cross-sections of the stockpiled hydraulically dredged material will not indicate the true volume removed for borrow, supplementary measurements will be taken. This may involve the use of settlement platforms, trenches, test holes or other methods or any combinations thereof. The data so derived will be applied in the adjustment of the computation of the quantity for payment.

(b) At the request of the Engineer or with the written permission of the Engineer, borrow may be measured by the cubic contents of the vehicles used for transportation less a 10% shrinkage factor for earth or less an agreed upon shrinkage factor for rock to be determined by the Engineer.

(c) At the request of the Engineer or with the written permission of the Engineer, borrow may be weighed and such weight (mass) will be converted to cubic yards on the basis of 3200 lb./c.y.

(d) At the request of the Engineer or with the written permission of the Engineer, the volume of borrow used to construct embankment may be measured in place. The shrinkage factor for earth shall be 10%. The swell factor for rock shall be determined by the Engineer.

(e) At the request of the Engineer or with the written permission of the Engineer, the Contractor may remove material from the borrow pit for purposes other than the formation of embankments, in which event the method of adjusting borrow measurements will be stated in the authorization.

2.07.05—Basis of Payment: Payment will be at the Contract unit price per cubic yard for "Borrow" complete in place, which price shall include furnishing and placing the material and all equipment, tools and labor necessary thereto.

No payment will be allowed for "Borrow" until all excavation has been placed in embankments except under the following conditions: If the Contractor has secured the permission of the Engineer to place borrow before the excavation is complete, as provided for in Paragraph 1 of 2.07.03, payment for such borrow material may be made at the discretion of the Engineer, before completion of the excavation. A request for such payment shall be made by the Contractor in writing, certifying therein that it waives payment for any borrow placed in lieu of suitable excavated material.

Deductions will be made from borrow measurements for any materials wasted from the cuts or placed in embankments outside of the cross-sections, except when unsuitable material is ordered to be wasted by the Engineer.

Pay Item	Pay Unit
Borrow	c.y.

SECTION 2.08
FREE-DRAINING MATERIAL

2.08.01—Description: Free-draining material shall consist of material meeting the requirements stated elsewhere herein. This material shall be furnished and placed in accordance with these specifications and as indicated on the plans, special provisions, or as ordered by the Engineer for the stabilization of embankments, or for fill in free water areas, or wherever specified.

2.08.02—Materials: Material for this work shall meet the requirements of M.02.07.

2.08.03—Construction Methods: The Contractor shall notify the Engineer at least 3 days prior to obtaining free-draining material from any source so that an examination may be made of the material and the necessary measurements may be taken.

The limits of the proposed source shall be shown to the Engineer; and prior to the taking of measurements, the Contractor shall be required to clear the area of all unsuitable material. No payment will be made for any material removed outside of the area measured or which was not used in the Contract work. No excavation shall be made within the confines of a source after the Engineer has taken the original measurements, except for material to be used for the purposes hereinbefore indicated, except with the permission of the Engineer.

The Contractor may, with the approval of the Engineer, obtain free-draining material, if available, from within the roadway excavation limits subject to the provisions of 1.04.07.

If the Contractor elects to obtain free-draining material from a commercial pit, it will be necessary to have a section of the pit set off for its use solely, so that the amount of material removed may be accurately determined. During the period between the original and the final release by the Engineer, no material shall be taken from a measured pit except by the Contractor and for use only in the work under the Contract.

If the Contractor elects to use stone from a quarry or reclaimed miscellaneous aggregate, the Contractor shall use only the materials from stockpiles which have been approved for use.

Free-draining material will be permitted only to the extent necessary to construct embankments to 3 feet above free water, or to promote free drainage in areas specified on the plans, in the special provisions, or as directed by the Engineer.

Free-draining material shall be placed in accordance with the provisions for the formation of embankment of 2.02.

2.08.04—Method of Measurement: The amount of free-draining material to be paid for will be determined by using the results of cross-sectional elevations taken before and after the free-draining material has been excavated in the average end area method, or by a method approved by the Engineer.

When material is drawn from a quarry stockpile or stockpile formed from reclaimed miscellaneous aggregate, the material shall be weighed on scales furnished by and at the expense of the Contractor. The scales shall be of a type satisfactory to the Engineer and shall be sealed at the expense of the Contractor as often as the Engineer may require. All weighing shall be done in the presence of a representative of the Department. From the weight so obtained, the volume shall be computed based on a specific gravity of 2.92 for the aggregate and smaller particles. For material having a different specific gravity, an appropriate correction shall be made.

2.08.05—Basis of Payment: Payment will be made at the Contract unit price per cubic yard for "Free-Draining Material," complete in place, which price shall include furnishing and placing the material and all equipment, tools and labor necessary thereto.

Pay Item	Pay Unit
Free-Draining Material	c.y.

SECTION 2.09
SUBGRADE

2.09.01—Description: The area upon which the pavement structure and paved shoulders are placed, including the shoulder base courses and subbase, shall be known as the subgrade. This is the plane coincident with the bottom of the subbase and the edge of pavement, as shown on the plans and cross-sections or as ordered by the Engineer. The work of formation of subgrade shall be performed at this plane.

Where precast concrete barrier curb or curbing is to be permanently installed, the work of formation of subgrade shall be performed on the area under the precast concrete barrier curb or curbing.

Where shoulders are to be reconstructed and the existing subbase is to remain, the work of formation of subgrade shall be performed at the plane coincident with the surface of the existing subbase.

After all grading has been substantially completed and all drains laid, the subgrade shall be brought to the lines, grades and cross-sections shown on the plans.

When no item for "Clearing and Grubbing" and no grading items appear in the proposal, the work of clearing waterways, ditches, drainage structures and culverts, as described in 2.01.03, shall be performed as part of this work.

2.09.03—Construction Methods: All soft and yielding material and other portions of the subgrade which will not compact readily shall be removed and replaced with suitable material.

In cut areas, the surface shall be uniformly compacted by use of equipment specifically manufactured for that purpose. Rollers shall deliver a ground pressure of not less than 300 lb./inch of contact width and weigh not less than 10 tons. Vibratory units shall have a static weight of not less than 4 tons. The amount of compactive effort shall be as directed by the Engineer, but shall be at least 4 complete passes of the compacting equipment being used.

The maximum dry density after compaction shall be as specified in 2.02.03-6.

Where multiple compacting units are to be used, that unit which exerts the larger compactive effort shall make the initial passes. Any portion of the subgrade not accessible to larger compacting units shall be compacted, as directed by the Engineer, to a degree equal to that obtained on the other portions of the subgrade with equipment and by methods appropriate to the size of the inaccessible area.

After compacting, the subgrade shall be true to required line and grade. There will be no direct payment for any materials required to bring the subgrade to the line, grade and cross-section shown on the plans.

The Contractor shall protect the completed subgrade from damage by exercising such precautions as the Engineer deems necessary. The subgrade surface shall be maintained in such condition as to permit proper drainage. It shall be checked and approved by the Engineer prior to placing any pavement structure and shoulders thereon.

2.09.04—Method of Measurement: Payment lines for formation of subgrade shall be coincident with the outside edges of the pavement or where paved shoulders are constructed, with the outside edges of the shoulder. Where precast concrete barrier curb is permanently installed, payment shall include the area under the precast concrete barrier curb. Payment for formation of subgrade will be allowed when such work is done in providing connections to public roads. Payment for formation of subgrade will not be allowed for work at private drives, in areas where traffic bound gravel is constructed or in areas where existing pavement is used as a base for resurfacing with bituminous concrete.

The area computed for payment shall not include the area of any bridge floor where the type of construction is such as to eliminate any necessity for the work described herein.

2.09.05—Basis of Payment: Formation and protection of subgrade, including all work provided for hereinbefore, will be paid for at the Contract unit price per square yard for "Formation of Subgrade," which price shall include all materials, equipment, tools, compaction testing and labor necessary thereto.

There will be no specific payment for the work of scarifying existing stone or gravel roads as described in 2.02.03, but the cost of such work shall be considered included in the payment for the item providing for the formation of subgrade.

When no item for "Formation of Subgrade" appears in the proposal, the cost of this work shall be included in the Contract unit price for the pavement item or items involved.

Pay Item	Pay Unit
Formation of Subgrade	s.y.

SECTION 2.11
ANTI-TRACKING PAD

2.11.01—Description
2.11.02—Materials
2.11.03—Construction Methods
2.11.04—Method of Measurement
2.11.05—Basis of Payment

2.11.01—Description: This work shall consist of furnishing, installing, maintaining and removing a crushed stone anti-tracking pad on geotextile filter fabric. All areas affected by the anti-tracking pad shall be restored to the original or plan contours. If shown on the plans or ordered by the Engineer, the restored areas shall be stabilized with turf establishment.

2.11.02—Materials:

The crushed stone shall meet the gradation requirements of M.01.02 for No. 3 stone.
Geotextile filter fabric shall meet the requirements of 7.55 and M.08.01-19.
Topsoil, if necessary, shall meet the requirements of M.13.01.
Seed, if necessary, shall meet the requirements of M.13.04.
Fertilizer, if necessary, shall meet the requirements of M.13.03.
Mulch, if necessary, shall meet the requirements of M.13.05.

2.11.03—Construction Methods: Clear area of anti-tracking pad of all vegetation and excavate to a maximum depth of 4 inches. Place geotextile filter fabric over the full width and length of excavated area and cover with No. 3 crushed stone to a minimum depth of 6 inches.

The anti-tracking pad shall be uniformly graded to produce the entry and exit path to the Site for all construction equipment. The pad shall be maintained of sufficient grading and stone surface to capture all soils and sediment from equipment tires prior to such exiting from the Site.

Crushed stone shall be replenished or replaced as necessary or as ordered by the Engineer to assure sufficient capture of sediment at the construction Site. Any sediment or crushed stone tracked off the Site shall be immediately cleaned, swept and removed by the Contractor at no cost to the State.

2.11.04—Method of Measurement: This work will be measured for payment by the number of square yards of accepted anti-tracking pad completed as shown on the plans or as ordered by the Engineer.

2.11.05—Basis of Payment: Payment for this work will be made at the Contract unit price per square yard for “Anti-Tracking Pad,” which shall include furnishing and placing all material, including the geotextile; for maintaining the anti-tracking pad during the Project construction period; for removing the anti-tracking pad after completion of the Project; for restoring the Site, including any required turf establishment; and for all labor, equipment, tools, and incidentals required to complete the work as well as the cleaning and sweeping of any sediment or crushed stone tracked off the Site.

Clearing and grubbing required to install the anti-tracking pad will be paid under the item "Clearing and Grubbing."

Pay Item	Pay Unit
Anti-Tracking Pad	s.y.

NOTES

SECTION 2.12
SUBBASE

2.12.01—Description: The subbase shall consist of a clean soil-aggregate mixture of bank or crushed gravel, crusher run stone, reclaimed miscellaneous aggregate containing no more than 2% by weight of asphalt cement or any combinations thereof, placed where shown on the plans or where directed by the Engineer and constructed in accordance with these specifications.

2.12.02—Materials: All materials for this work shall conform to the requirements of M.02.02 and M.02.06.

2.12.03—Construction Methods: The prepared foundation for the subbase shall be carefully shaped to the required cross-section and compacted. Where underdrains and outlets are specified on the plans or ordered by the Engineer, they shall be in place and functioning before any subbase material is placed.

The subbase material shall be spread uniformly upon the required grade, in courses not to exceed 6 inches thick after final compaction. However, if the required thickness of subbase does not exceed 8 inches it may be placed in a single course.

After each course has been placed as specified above, its entire area shall be compacted with equipment specifically manufactured for that purpose. The use of hauling and spreading equipment shall not be considered as a substitute for compacting equipment. Compaction shall be continued until the entire course is uniformly compacted to the required minimum density. The dry density after compaction shall not be less than 95% of the maximum dry density for that subbase material when determined by the Contractor in accordance with AASHTO T 180 and measured in-place with ASTM D6938 or other methods approved by the Engineer. If a subbase course is formed from reclaimed miscellaneous aggregate containing bituminous concrete, the wet density after compaction on this course shall not be less than 95% of the maximum wet density for that subbase when determined by the Contractor in accordance with AASHTO T 180 and measured in-place with ASTM D6938 or other methods approved by the Engineer.

The Contractor shall perform in-place density testing at a sufficient frequency to ensure that the specified results are continuously met. The Contractor shall submit complete field density testing and inspection records to the Engineer within 48 hours (excluding weekends and holidays) of the test in a manner acceptable to the Engineer.

Should the foundation material beneath the subbase become churned up and mixed with subbase material at any time, the Contractor shall, without additional compensation, remove the mixture and replace it with new subbase material to the required thickness shown on the plans or as previously required by the Engineer. Such replaced subbase material shall be compacted to the required minimum density.

2.12.04—Method of Measurement: Subbase will be measured horizontally in place after final grading and compaction. The thickness will be as indicated on the plans, or as ordered by the Engineer, and within the following tolerances:

Less than 24 inches: minus 1 inch to plus 3/4 inch

24 inches and greater: minus 2 inches to plus 1 inch

Measurements to determine the thickness will be made by the Engineer at intervals of 500 feet, or less, along lanes and shall be considered as representative of the lane. For purposes of these measurements, a shoulder will be considered a lane.

If deficient thicknesses are found, the Engineer will make such additional measurements considered necessary to determine the longitudinal limits of the deficiency. Areas not within allowable tolerances shall be corrected, as ordered by the Engineer, without additional compensation to the Contractor.

2.12.05—Basis of Payment: This work will be paid at the Contract unit price per cubic yard for "Subbase," which price shall include all materials, equipment, tools, compaction testing and labor incidental thereto.

Pay Item	Pay Unit
Subbase	c.y.

NOTES

SECTION 2.13
GRANULAR FILL

2.13.01—Description: This material shall be used as a foundation for structures, to replace unstable material in slopes, as a foundation for sidewalks and culverts, in shoulders and elsewhere as indicated on the plans, required by the specifications or ordered by the Engineer. It shall consist of gravel or reclaimed miscellaneous aggregate containing no more than 2% by weight of asphalt cement meeting the requirements of these specifications.

2.13.02—Materials: Granular fill shall meet the requirements of M.02.01

2.13.03—Construction Methods: When granular fill is used for foundation for structures or to replace rock or unsuitable material in trenches, it shall be deposited in layers not over 6 inches deep, with each layer thoroughly compacted before the addition of other layers.

2.13.04—Method of Measurement: Granular fill will be measured in place after compaction within the payment lines shown or specified by the Engineer.

2.13.05—Basis of Payment: This work will be paid for at the Contract unit price per cubic yard for "Granular Fill," complete in place, which price shall include all materials, tools, equipment and labor incidental thereto.

Pay Item	Pay Unit
Granular Fill	c.y.

SECTION 2.14
COMPACTED GRANULAR FILL

2.14.01—Description: Work under this item shall consist of furnishing and placing compacted granular fill as foundation for structures where shown on the plans or directed by the Engineer.

2.14.02—Materials: Gravel or reclaimed miscellaneous aggregate shall meet the requirements of M.02.02. Admixtures and surface protective materials used to prevent the gravel from freezing must meet the approval of the Engineer.

2.14.03—Construction Methods: After all excavation has been completed, gravel or reclaimed miscellaneous aggregate shall be deposited in layers not exceeding 8 inches deep over the areas. In exceptional cases, the Engineer may permit the first layer to be thicker than 8 inches. Each layer shall be leveled off by the use of blade graders or bulldozers with adequate power for the work involved. The entire area of each layer shall be compacted by use of vibratory, pneumatic tire or tread type compaction equipment approved by the Engineer. The compaction shall be continued until the dry density over the entire area of each layer is not less than 95% of the maximum dry density for that material when determined by the Contractor in accordance with AASHTO T 180 and measured in-place with ASTM D6938 or other methods approved by the Engineer.

If a layer is formed from reclaimed miscellaneous aggregate containing bituminous concrete, the wet density after compaction on this layer shall not be less than 95% of the maximum wet density for that compacted granular fill when determined by the Contractor in accordance with AASHTO T 180 and measured in-place with ASTM D6938 or other methods approved by the Engineer.

The Contractor shall perform in-place density testing at a sufficient frequency to ensure that the specified results are continuously met. The Contractor shall submit complete field density testing and inspection records to the Engineer within 48 hours (excluding weekends and holidays) of the test in a manner acceptable to the Engineer.

The embankment adjacent to the compacted granular fill shall be placed simultaneously with the compacted granular fill, and at no time shall there be a difference of more than 2 feet in elevation between the classes of material. The embankment material to be placed simultaneously with the compacted granular fill shall extend at least 20 feet in every direction beyond the limits of the compacted granular fill, except that where a narrower width is shown on the plans, such narrower width of material shall be placed as prescribed above.

2.14.04—Method of Measurement: Compacted granular fill will be measured in place after compaction, by the average end area method, or by a method approved by the Engineer.

2.14.05—Basis of Payment: This work will be paid for at the Contract unit price per cubic yard for "Compacted Granular Fill," complete in place, which price shall include all materials, equipment, tools, compaction testing and labor incidental thereto.

The cost of water and work involved in puddling, admixtures and protective materials shall be included in the Contract unit price per cubic yard for "Compacted Granular Fill."

Pay Item	Pay Unit
Compacted Granular Fill	c.y.

SECTION 2.16
PERVIOUS STRUCTURE BACKFILL

2.16.01—Description: Pervious structure backfill shall include the furnishing, placing, and compaction of pervious material adjacent to structures. This item shall also consist of furnishing and placing crushed stone or gravel in permeable material bags at the inlet ends of weep holes in structures to the dimensions indicated on the plans or as ordered by the Engineer.

2.16.02—Materials: Pervious structure backfill shall meet the requirements of M.02.05.

The materials for bagged stone shall meet the following requirements:

1. The crushed stone or gravel shall meet the gradation requirements specified in Table M.01.02-2 for No. 3 or No. 4 coarse aggregate or a combination of both.
2. The bag shall be of permeable material sized to contain 1 c.f. of loosely packed granular material.

2.16.03—Construction Methods: Pervious structure backfill shall be placed adjacent to abutments, retaining walls, box culverts, and elsewhere as called for. It shall be placed above a plane extending on a 2 to 1 slope from the upper edge of the footing to the top of the embankment, or as shown on the plans. Where the face of undisturbed material is above or beneath this slope plane, the amount of pervious structure backfill shall be decreased or increased accordingly, if ordered by the Engineer.

In filling behind abutments, retaining walls, box culverts, or other structures, the fill is placed against undisturbed material, or against compacted embankments having a length in a direction at right angles to the abutment wall or culvert not less than twice the height of the structure against which the fill is placed. The slope of the embankment on which the pervious structure backfill is to be placed shall be plowed deeply or cut into steps before and during the placing of pervious structure backfill so both types of material will be thoroughly bonded and compacted.

Each layer of pervious structure backfill shall be spread to a thickness not exceeding 6 inches deep after compaction and shall be thoroughly compacted as directed by the Engineer by the use of power rollers or other motorized vehicular equipment, by tamping with mechanical rammers or vibrators, or by pneumatic tampers. Any equipment not principally manufactured for compaction purposes and equipment which is not in proper working order in all respects shall not be used within the area described above.

Special attention shall be given to compaction in places close to walls where motorized vehicular equipment cannot reach. Within 3 feet of the back face of walls and within a greater distance at angle points of walls, each layer of pervious structure backfill shall be compacted by mechanical rammers, vibrators, or pneumatic tampers.

The dry density of each layer of pervious structure backfill formed from broken or crushed stone, broken or crushed gravel or reclaimed miscellaneous aggregate free of bituminous concrete shall have a dry density after compaction that is not less than 98% of the maximum dry density for that material when determined by the Contractor in accordance with AASHTO T 180 and measured in-place with ASTM D6938 or other methods approved by the Engineer.

If a layer formed from reclaimed miscellaneous aggregate containing bituminous concrete is placed as pervious structure backfill, the wet density of this layer after compaction shall not be less than 98% of the maximum wet density for that material when determined by the Contractor in accordance with AASHTO T180 and measured in-place with ASTM D6938 or other methods approved by the Engineer.

The Contractor shall perform in-place density testing at a sufficient frequency to ensure that the specified results are continuously met. The Contractor shall submit complete field density testing and inspection records to the Engineer within 48 hours (excluding weekends and holidays) of the test in a manner acceptable to the Engineer.

Where weep holes are installed, bagged stone shall be placed around the inlet end of each weep hole, to prevent movement of the pervious material into the weep hole. Approximately 1 c.f. of crushed stone or gravel shall be enclosed in each of the permeable material bags. All bags shall then be securely tied at the neck with cord or wire so that the enclosed material is contained loosely. The filled bags shall be stacked at the weep holes to the dimensions shown on the plans or as directed by the Engineer. The bags shall be unbroken at the time pervious material is placed around them, and bags which are broken or burst prior to or during the placing of the pervious material shall be replaced at the Contractor's expense.

2.16.04—Method of Measurement: Payment lines for pervious structure backfill shall coincide with the limits of the compacted pervious structure backfill as actually placed and ordered by the Engineer.

2.16.05—Basis of Payment: Pervious structure backfill will be paid for the Contract unit price per cubic

yard for "Pervious Structure Backfill," complete in place and the price shall include compaction testing.

There will be no direct payment for bagged stone, but the cost thereof shall be included in the cost of the work for “Pervious Structure Backfill.”

Pay Item	Pay Unit
Pervious Structure Backfill	c.y.

SECTION 2.19
SEDIMENTATION CONTROL SYSTEM

2.19.01—Description: This work shall consist of furnishing, placing, maintaining and removing sedimentation control systems as shown on the plans or as directed by the Engineer. Maintaining shall include the clean out and disposal of accumulated sediment.

2.19.02—Materials: The sedimentation control system materials for this work shall meet the following requirements:

Hay bales shall be made of hay with 40 lb. minimum weight and 120 lb. maximum weight held together by twine or wire.

Geotextile shall meet the requirements of 7.55 and M.08.

2.19.03—Construction Methods: Sedimentation Control Systems shall be installed by the Contractor in locations shown on the plans or as directed by the Engineer.

Hay bale systems shall be installed lengthwise along the contour with ends of adjacent bales tightly abutting each other. All hay bales shall be installed so that bindings are oriented around the sides, rather than along the tops and bottoms. Each hay bale shall be entrenched 4 inches deep and backfilled, with the backfilled soil placed toward the potential silt source. They shall be held in place by 2 wooden stakes in each hay bale and each wooden stake shall be driven 18 inches deep into the ground. Gaps shall be filled with hay or straw to prevent water or debris escaping between bales.

Geotextile systems shall be installed along the contour so that the bottom 6 inches of the fabric is buried by either trenching or by laying the 6 inches section horizontally on the ground and burying by ramping the soil up to the control fence. All geotextile fences shall be exposed at least 30 inches high as installed. Spacing between posts shall not exceed 10 feet and all wooden posts shall be driven a minimum of 12 inches deep into the ground. When joints between sections of geotextile sedimentation control systems are necessary, geotextile shall be spliced together only at a support post, with a minimum 6 inch overlap, and securely sealed.

When trench excavation of a hay bale or geotextile fence is obstructed by an occasional stone or tree root, provide a smooth transition between the trench bottom and the obstruction.

Clean out of accumulated sediment shall be accomplished when 1/2 of the original height of the sedimentation control system, as installed, becomes filled with sediment or as directed by the Engineer.

The sedimentation control systems shall be maintained or replaced until they are no longer necessary for the purpose intended or are ordered removed from the Site at the completion of the Project when full stabilization has occurred, unless specifically authorized by the Engineer to be left in place.

2.19.04—Method of Measurement: This work will be measured for payment by the actual number of linear feet of “Sedimentation Control System” or “Sedimentation Control (Type) System” installed and accepted. Measurement shall be made along the top center-line of the system. Replacement systems will not be measured for payment.

2.19.05—Basis of Payment: Payment for this work will be made at the Contract unit price per linear foot for “Sedimentation Control System” or “Sedimentation Control (Type) System” complete in place, which price shall include all materials, equipment, tools and labor incidental to the installation, maintenance, replacement, removal and disposal of the system and surplus material. No additional payment will be made for the clean out of accumulated sediment.

Pay Item	Pay Unit
Sedimentation Control System	l.f.
Sedimentation Control (Type) System	l.f.

SECTION 2.86
DRAINAGE TRENCH EXCAVATION,
ROCK IN DRAINAGE TRENCH EXCAVATION

2.86.01—Description
2.86.03—Construction Methods
2.86.04—Method of Measurement
2.86.05—Basis of Payment

2.86.01—Description: Drainage trench excavation consists of the excavation necessary for the proper installation of drainage structures, pipes, pipe ends and any other incidental drainage items.

It shall include earth and rock excavation, removal of existing pipes, dewatering, backfill, and disposal of materials; to the trench limits described herein, to the dimensions shown on the plans, or as directed by the Engineer.

Classifications:

(1) Drainage Trench Excavation will include only the excavation necessary for the construction of the drainage items and the removals specified above.

(2) Rock in Drainage Trench Excavation, insofar as it applies to drainage trench excavation, shall be defined as **1/2 cubic yard or more** in volume of the following obstructions removed from the limits of the drainage trench:

(a) rock in definite ledge formation
(b) boulders, or portions of boulders
(c) cement masonry structures
(d) concrete or reinforced concrete structures
(e) reinforced concrete pipe
(f) subsurface concrete pavement or concrete base

The removal shall be as indicated or directed from within the limits defined in 2.86.03 for drainage trench excavation.

2.86.03—Construction Methods:

(1) Drainage Trench Excavation Limits:

Horizontal Limits: Trench widths for pipes, pipe ends, pipe-arches, and drainage structures shall be as follows:

(a) 2 feet greater than the nominal inside diameter of circular pipe or nominal inside span of elliptical pipe or pipe-arch for such diameters or spans of less than 30 inches
(b) 3 feet greater than the nominal inside diameter of circular pipe or the nominal inside span of elliptical pipe or pipe-arch for such diameters or spans that are 30 inches or greater
(c) 4 feet greater than the nominal inside diameter or nominal horizontal inside span for pipe-arches fabricated from structural plates
(d) 2 feet beyond the neat lines of all exterior or foundation walls of drainage structures

Vertical Limits: Trench depths shall extend vertically as follows:

(a) From the bottom of the trench to the bottom of the roadway excavation, or in areas away from roadway excavation, to the top of existing ground surface.
(b) Where drainage pipe is to be laid in a fill area, the embankment shall be placed and compacted to a minimum elevation 12 inches above the top of the proposed pipe, whereupon the drainage trench excavation shall be performed and the pipe installed.

(2) Drainage Trench Excavation: Drainage trench excavation shall be made in conformity with the requirements of the plans, or as directed by the Engineer. The Contractor shall furnish and employ such shores, braces, pumps, or ancillary equipment as needed for the proper protection of property, proper completion of the work, as well as safety of the public and employees of both the Contractor and the Department. All bracing and shoring shall be removed when no longer required for the construction or safety of the work. When required, the Contractor shall provide or have on the Site at all times any OSHA certification for equipment to be used, per 1.07.07. For support of trenches greater than 10 feet in depth, working drawings shall be submitted, in accordance with 1.05.02. The Contractor shall control erosion and sedimentation at trench locations and ensure that pumped water from the drainage excavation is discharged in accordance with the requirements of 1.10.

Where a firm foundation is not encountered at the grades established due to unsuitable material,

such as soft, spongy, or unstable soil, the unsuitable material shall be removed and replaced with approved backfill, thoroughly compacted in lifts not to exceed 6 inches, for the full trench width. The Engineer shall be notified prior to removal of the unsuitable material in order to determine the depth of removal necessary.

After the excavation is complete, the Contractor shall notify the Engineer and no drainage structure or material shall be placed in the excavated area until the Engineer has approved the depth of excavation and the character of the foundation material.

(3) Rock in Drainage Trench Excavation:

(a) Rock in Drainage Trench Excavation - Ledge: When rock in definite ledge form is encountered, the Contractor shall excavate a minimum of 12 inches below the bottom of the proposed pipe or drainage structure; and this depth shall be filled with bedding material (as specified in M.08.03-1) below the proposed pipe; or granular fill (as specified in M.02.01) below the proposed drainage structure, which shall be thoroughly compacted in lifts not to exceed 6 inches.

(b) Rock in Drainage Trench Excavation - Boulders: When boulders are encountered, the Contractor shall remove them from the trench and if backfill is required, the void shall be filled with bedding material, surplus excavated material (as specified in 2.02.03-8) or granular fill which shall be thoroughly compacted in lifts not to exceed 6 inches.

(c) Rock in Drainage Trench Excavation –Structures: When cement masonry, concrete or reinforced concrete structures are encountered within the drainage trench limits, the Contractor shall remove the structure in its entirety or as directed by the Engineer, and if backfill is required, the void shall be filled with bedding material, surplus excavated material or granular fill which shall be thoroughly compacted in lifts not to exceed 6 inches.

(4) Backfill: Suitable material excavated from the drainage trench shall be used as backfill material prior to consideration of using any other source of backfill. Backfill material used shall be of a quality satisfactory to the Engineer and shall be free from large or frozen lumps, wood and other extraneous material. Rock fill or stones larger than 5 inches shall not be placed within 1 foot of the drainage structure or pipe. The grading shall be completed to the lines shown on the plans, or as ordered, by refilling to the required elevation with approved material, placed in layers not to exceed 6 inches in depth after compaction, which shall be thoroughly compacted with equipment approved by the Engineer.

All surplus or unsuitable material shall be removed and disposed of as directed. Should additional material be required for backfilling, it may be obtained from the Project surplus excavation in accordance with 2.02.03-8 or from borrow pits, gravel pits, or elsewhere as directed by the Engineer.

2.86.04—Method of Measurement:

Drainage Trench Excavation: Drainage trench excavation will not be measured for payment.

If granular fill or borrow is required to replace unsuitable material it will be measured for payment as directed by the Engineer.

Rock in Drainage Trench Excavation: If any material meeting the definition of Rock in Drainage Trench Excavation is encountered, the Contractor shall strip it of sufficient overlying material to allow for proper measurement and shall then notify the Engineer that the rock surface is ready for measurement. If the Contractor fails to give such notice, the Engineer will presume that the measurements taken at the time the Engineer first saw the material in question will give the true quantity of excavation.

Rock in Drainage Trench Excavation will be measured according to the classification provided in 2.86.01 and within the drainage trench excavation limits provided in 2.86.03.

For the removal of underground obstructions, as classified in 2.86.01-2, the measurement shall be the actual volume of rock removed (1/2 cubic yard or more) as approved by the Engineer.

Rock in Drainage Trench Excavation will not be measured for payment in fills.

Bedding Material or other suitable fill, as specified in 2.86.03(3), used to fill voids after rock is excavated will not be measured for payment.

2.86.05—Basis of Payment:

Drainage Trench Excavation: There will be no direct payment for drainage trench excavation required for the installation of drainage pipes, pipe ends, catch basins, drop inlets, manholes, and other drainage structures, or any other incidental drainage work including materials, tools, equipment and labor necessary to complete the drainage trench excavation in conformity with the plans or as directed by the Engineer.

There will be no direct payment for backfill or disposal of surplus material necessary for the satisfactory completion of this work.

There will be no direct payment made for shoring, bracing, dewatering, or for material or equipment necessary for the satisfactory completion of the work.

Where called for on the plans to install temporary earth retaining systems for the support of existing facilities, pavement, utilities, or for other constraints, payment will be made in accordance with such items in the Contract.

If granular fill or borrow is used to replace unsuitable material, payment will be made at the respective Contract unit prices, or in the absence of such items in the Contract, as Extra Work in accordance with 1.04.05.

Rock in Drainage Trench Excavation: When rock, conforming to the description in 2.86.01 is encountered within the limits of drainage trench excavation, its removal will be classified and paid for at the Contract unit price per cubic yard for "Rock in Drainage Trench Excavation 0' – 10' Deep," or "Rock in Drainage Trench Excavation 0' – 20' Deep," as the case may be.

Those portions of drainage trench excavation classified and paid for as "Rock in Drainage Trench Excavation" of the various depths will be the actual volumes of rock excavated within the limits for drainage trench excavation, at the applicable bottom depth price.

Where no item or items for "Rock in Drainage Trench Excavation" at the applicable depth appear in the proposal and rock is encountered in drainage trench excavation, its removal will be paid for as Extra Work in accordance with 1.04.05.

When excavation is necessary in fill, no such excavation will be paid for as "Rock in Drainage Trench Excavation."

When excavation is necessary for any purpose other than drainage-related items, no such excavation will be paid under this item.

Bedding material or any other suitable material used to fill voids vacated by excavated rock will not be paid for but shall be included in the unit price per cubic yard for "Rock in Drainage Trench Excavation."

Payment for the work of “Cut Concrete Pavement” and “Cut Bituminous Concrete Pavement” will be made in accordance with 2.02.05 except that the cut made in the bituminous concrete overlay 1 foot beyond the neat line for concrete pavement removal is not included under “Cut Concrete Pavement” and will be paid for as “Cut Bituminous Concrete Pavement.”

Pay Item	Pay Unit
Rock in Drainage Trench Excavation 0' - 10' Deep	c.y.
Rock in Drainage Trench Excavation 0' - 20' Deep	c.y.

NOTES

SECTION 3.04
PROCESSED AGGREGATE BASE

3.04.01—Description
3.04.02—Materials
3.04.03—Construction Methods
3.04.04—Method of Measurement
3.04.05—Basis of Payment

3.04.01—Description: The base shall consist of a foundation constructed on the prepared subbase or subgrade in accordance with these specifications and in accordance with the lines, grades, compacted thickness and typical cross-section as shown on the plans.

3.04.02—Materials: All materials for this work shall meet the requirements of M.05.01.

3.04.03—Construction Methods: The same type of coarse aggregate shall be used throughout a Project unless otherwise permitted by the Engineer.

Prior to placing the processed aggregate base, the prepared subbase or subgrade shall be maintained true to line and grade, for a minimum distance of 200 feet in advance of the work. None of the aggregate courses shall be placed more than 500 feet ahead of the compaction and binding operation on that particular course.

The processed aggregate base shall be spread uniformly by a method approved by the Engineer. The thickness of each course shall not be more than 4 inches after compaction, unless otherwise ordered.

After the aggregate is spread, it shall be thoroughly compacted and bound by use of equipment specifically manufactured for that purpose. Rollers shall deliver a ground pressure of not less than 300 lb./in of contact width and shall have a weight not less than 10 tons. Vibratory units shall have a static weight of not less than 4 tons. Water may be used during the compaction and binding operation and shall be applied from an approved watering device. The compacting and binding operation shall begin at the outside edges, overlapping the shoulders for a distance of not less than 6 inches and progress towards the middle, parallel with the centerline of the pavement. The work shall cover the entire surface of the course with uniform overlapping of each preceding track or pass. Areas of super-elevation and special cross slope shall be compacted by beginning at the lowest edge and proceeding towards the higher edge, unless otherwise directed by the Engineer. The compacting and binding operation shall be continued until the voids in the aggregates have been reduced to provide a firm and uniform surface satisfactory to the Engineer. The amount of compactive effort shall in no case be less than 4 complete passes of the compacting and binding operations. All aggregate shall be completely compacted and bound at the end of each day's work or when traffic is to be permitted to operate on the road. The dry density of each layer of processed aggregate base after compaction shall not be less than 95% of the maximum dry density for that material when determined by the Contractor in accordance with AASHTO T 180 and measured in-place with ASTM D6938 or other methods approved by the Engineer.

The Contractor shall perform in-place density testing at a sufficient frequency to ensure that the specified results are continuously met. The Contractor shall submit complete field density testing and inspection records to the Engineer within 48 hours (excluding weekends and holidays) of the test in a manner acceptable to the Engineer.

Should the subbase or subgrade material become churned up or mixed with the processed aggregate base at any time, the Contractor shall, without additional compensation remove the mixture. The Contractor shall add new subbase material, if required, and reshape and recompact the subbase in accordance with the requirements of 2.12.03. New aggregate material shall be added, compacted and bound, as hereinbefore specified, to match the surrounding surface.

Any surface irregularities which develop during, or after work on each course, shall be corrected by loosening material already in place and removing or adding aggregate as required. The entire area, including the surrounding surface, shall be re-compacted and rebound until it is brought to a firm and uniform surface satisfactory to the Engineer.

3.04.04—Method of Measurement: Processed Aggregate Base will be measured horizontally in place after final grading and compaction. Materials placed beyond the horizontal limits indicated on the plans will not be measured for payment.

The total thickness shall be as indicated on the plans, or as ordered by the Engineer and within a tolerance of -3/4 inch to +1/2 inch. Measurements to determine the thickness will be taken by the Engineer at

intervals of 500 feet or less, along lanes, and shall be considered representative of the lane. For the purpose of these measurements, a shoulder will be considered a lane.

If a thickness measurement is taken and found deficient, additional measurements considered necessary by the Engineer will be taken to determine the longitudinal limits of the deficiency. Areas not within allowable tolerances shall be corrected, as ordered by the Engineer, without additional compensation to the Contractor.

3.04.05--Basis of Payment: This work will be paid for at the Contract unit price per cubic yard for "Processed Aggregate Base," complete in place, which price shall include all materials, tools, equipment, compaction testing and work incidental thereto.

Pay Item	Pay Unit
Processed Aggregate Base	c.y.

SECTION 3.05
PROCESSED AGGREGATE

3.05.01—Description: Work under this item shall consist of furnishing, placing, shaping and compacting processed aggregate to be used for back-up to bituminous concrete overlays in areas shown on the plans or where directed by the Engineer.

3.05.02—Materials: The material for this item shall meet the requirements of M.05.01, except that coarse aggregate shall be broken stone, and fine aggregate shall be stone sand, screenings, or a combination thereof.

3.05.03—Construction Methods: The material shall be placed, shaped, and compacted in a single continuous operation to the lines, grades, and cross slopes shown on the plans or as directed by the Engineer. Only the amount of material that can be placed, shaped, and compacted during the work shift shall be placed. No excess loose material shall be left along the edge of road.

Compaction will be by vibratory equipment determined to be acceptable to the Engineer prior to the start of the work. No specific percent of compaction is required; however, no loose material shall be evident after completion of compaction as approved by the Engineer.

During the hauling and placing operations, the Contractor shall immediately remove any material dumped or spilled on the shoulders or pavement.

It shall be the Contractor's responsibility to maintain and restore any eroded areas to the required line, grade, and cross slope with approved material and to keep the areas in acceptable condition until the construction work is considered complete by the Engineer.

3.05.04—Method of Measurement: The quantity of processed aggregate to be included for payment will be determined by the net weight, in tons, measured in the hauling vehicles. Scales shall be of a type satisfactory to the Engineer and shall be sealed by the Department of Consumer Protection at the expense of the Contractor, as often as the Engineer may require. When required, weighing shall be done in the presence of a Department representative.

3.05.05—Basis of Payment: This material will be paid for at the Contract unit price per ton for "Processed Aggregate," complete in place, which price shall include all materials, equipment, tools, and labor incidental thereto.

Pay Item	Pay Unit
Processed Aggregate	ton

SURFACE COURSES OR PAVEMENTS

SECTION 4.01
CONCRETE FOR PAVEMENT

4.01.01—Description
4.01.02—Materials
4.01.03—Construction Methods
4.01.04—Method of Measurement
4.01.05—Basis of Payment

4.01.01—Description: This pavement shall consist of Portland cement concrete, with or without reinforcement, in accordance with the lines, grades, thickness, and typical cross section as shown on the plans.

4.01.02—Materials: The materials for this work shall meet the requirements for concrete pavement under M.03. Reinforcing steel shall be as specified in M.06.01.

4.01.03—Construction Methods:

A. Material Documentation, Transportation and Testing: All material delivered to the Project shall be documented, transported and tested in accordance with 6.01.03-II-3 Transportation and Delivery of Concrete and 6.01.03-II-4 Acceptance Testing and Test Specimens. The plastic properties for concrete pavement shall meet the standard mix properties as indicated in Table 6.01.03-2.

In addition, the air content of the plastic concrete shall be determined in accordance with AASHTO Method T152, Pressure Method. No alternative method is acceptable.

B. Placing Concrete:

All forms, slip form pavers, and other equipment used in the placing, curing, and finishing of concrete shall be of a design acceptable to the Engineer.

1. Forms: Forms shall be set by the Contractor to true line and grade.

The specified grade shall be set and compacted prior to the setting of forms.

Conditioning of Subgrade or Base Course: The subgrade or base course shall be brought to proper cross-section. High areas shall be trimmed to proper elevation. Low areas shall be filled and compacted to a condition similar to that of surrounding grade. The finished grades shall be maintained in a smooth and compacted condition until the pavement is placed.

2. Slip Form Paving: When slip form paving, the subgrade, or the surface of the subbase over which the tracks of the slip form paver will travel shall not vary more than ± 3/16 inch from the grade established by the Engineer.

Except when the Contractor elects to place reinforcement in a two-layer procedure, the slip form paving equipment shall spread, consolidate, screed and float finish the freshly placed concrete in a single pass in a manner such that a minimum of hand floating will be required. The operations of depositing, spreading, consolidating and finishing shall be such that, insofar as possible, continuous operation of the paver will be maintained. Frequent starting and stopping of the paver shall be kept to a minimum.

3. Placement: Where concrete is to be placed adjoining a previously constructed lane of pavement, and hauling units and mechanical equipment will be operated upon that previously constructed lane of pavement, that lane shall have attained the strength specified for opening the pavement to vehicular traffic. If only finishing equipment is carried on the existing lane, paving in adjoining lanes may be permitted after the concrete is tested in accordance with 4.01.03-D and has attained a modulus of rupture of 400 psi.

Concrete shall be thoroughly consolidated against and along the faces of all forms and along the full length and on both sides of all joint assemblies, by means of vibrators inserted in the concrete. Vibrators shall not be permitted to come in contact with a joint assembly, the grade, or a side form. In no case shall the vibrator be operated longer than 5 seconds in any location.

Concrete shall be deposited as close to expansion and contraction joints as possible without disturbing them, but shall not be dumped from the discharge bucket or hopper on to a joint assembly unless the hopper is well centered on the joint assembly.

Should any concrete materials fall on or be worked into the surface of a completed slab, they shall be removed immediately by approved methods.

In order that the concrete may be properly protected from rain before the concrete is sufficiently hardened, the Contractor will be required to have available at all times materials for the protection of the edges and surface of the unhardened concrete.

4. Placing Reinforcement: If mechanical means are used to place pavement reinforcement or pavement fabric, the mechanical placement equipment shall be capable of positioning the reinforcement or fabric within the tolerances specified.

When reinforced concrete pavement is placed in 2 layers, the entire width of the bottom layer shall be struck off and consolidated to such length and depth that the sheet of fabric or bar mat may be laid full length on the concrete in its final position without further manipulation. The reinforcement shall be placed directly upon the concrete, after which the top layer of the concrete shall be placed, struck off and screeded.

Any portion of the bottom layer of concrete which has been in place for more than 45 minutes without being covered with a top layer shall be removed and replaced with freshly mixed concrete at the Contractor's expense. When reinforced concrete is placed in a single layer, the reinforcement may be positioned in advance of concrete placement or it may be placed in plastic concrete after spreading, by mechanical or vibratory means.

Reinforcing steel shall be free from dirt, oil, paint, grease, mill scale, and loose or thick rust, which could impair bond of the steel with the concrete.

5. Finishing:

(a) Sequence: The sequence of operations shall be the strike-off and consolidation floating and removal of laitance, and final surface finish. Work bridges or other devices necessary to provide access to the pavement surface for the purpose of finishing, straight-edging, and making corrections as hereinafter specified, shall be provided by the Contractor.

In general, the addition of superficial water to the surface of the concrete to assist in finishing operations will not be permitted. If the application of water to the surfaces is permitted, it shall be applied as a fog spray by means of approved spray equipment.

(b) Finishing at Joints: The concrete adjacent to joints shall be compacted or firmly placed without voids or segregation against the joint material, under and around all load transfer devices, joint assembly units, and other features designed to extend into the pavement. Concrete adjacent to joints shall be mechanically vibrated.

(c) Hand Finishing: Unless otherwise specified, hand finishing methods will not be permitted except under the following conditions:

i. In the event of breakdown of the mechanical equipment, hand methods may be used to finish the concrete already deposited on the grade when the breakdown occurs.
ii. Narrow widths or areas of irregular dimensions where operations of the mechanical equipment is impractical may be finished by hand methods.

Concrete, as soon as placed, shall be struck off and screeded. An approved portable screed shall be used. A second screed shall be provided for striking off the bottom layer of concrete if reinforcement is used.

(d) Floating: After the concrete has been struck off and consolidated it shall be further smoothed, trued, and consolidated by means of a float.

(e) Final Finish: When all irregularities have been removed, and the edges of the slabs and joints given their initial edging, the pavement shall be subjected to the following 2-stage texturing treatment:

i. **Dragging with Burlap:** The burlap shall be at least 3 feet wide and a length of 2 feet greater than the width of the lane or lanes being placed. The burlap shall not have frayed edges and shall be kept wet and clean of accumulations of dried concrete particles or other foreign materials, which might leave distinctive undesirable marks. The burlap shall be drawn longitudinally along the surface in a slow manner so as to have an even texture. When not in use, the burlap shall not be allowed to rest on the pavement.
ii. **Texturing with Tines:** As soon as possible, after the pavement surface has been dragged with the burlap, mechanical equipment shall be used to texture the surface with deep transverse grooves. The equipment shall consist of a self-propelled device capable of applying a textured finish transverse to the centerline of the pavement. The texturing shall be done with tines 0.03 inch thick, 0.08 inch wide, and 4 to 6 inches long with an average spacing of 1/2 inch on centers.

The transverse grooving shall be performed when the condition of the concrete is optimum. This condition will prevail when the grooves can be formed to a depth of 1/8 to 1/4 inch with relative ease and without the walls of the grooves closing back in on each other. Other adjustments to the tining procedure may be permitted by the Engineer to obtain grooves of the proper width and depth.

The tined grooving shall extend from edge to edge of the pavement, and shall be perpendicular to the centerline of the pavement. The tining mechanism shall be aligned so as to prevent overlapping of grooves in any 2 successive passes. The depth of the grooves formed in the surface by the tines shall be checked

randomly with a tire tread depth-measuring gage furnished by the Contractor to ensure compliance with the required limits of 1/8 to 1/4 inch. The original surface of the concrete shall serve as the datum for the depth measurements.

(f) Edging at Forms and Joints: After the final finish, but before the concrete has taken its initial set, the edges of the pavement along each side of each slab, and on each side of transverse expansion joints, formed joints, transverse construction joints, and emergency construction joints shall be worked with an approved tool and rounded to the radius required by the plans.

A well-defined and continuous radius shall be produced and a smooth, dense mortar finish obtained. The surface of the slab shall not be unduly disturbed by tilting the tool during use.

At all joints, any tool marks appearing on the slab adjacent to the joints shall be eliminated by brooming the surface. In doing this, the rounding of the corner of the slab shall not be disturbed. All concrete on top of the joint filler shall be completely removed. All joints shall be tested with a straight edge before the concrete has set, and correction shall be made if either side of the joint is higher than the other or if they are higher or lower than the adjacent slabs.

6. Joints: Joints shall be constructed of the type and dimensions, and at the locations required by the plans or specifications. All joints shall be protected from the intrusion of injurious foreign material until sealed.

(a) Longitudinal Joint: Deformed steel tie bars of specified length, size, spacing and material shall be placed perpendicular to the longitudinal joints; they shall be placed by approved mechanical equipment or rigidly secured on chairs or other approved supports to prevent displacement. Tie bars shall be coated with epoxy conforming to AASHTO M 284. When shown on the plans and when adjacent lanes of pavement are constructed, steel side forms shall be used which will form a keyway along the construction joint. Tie bars may be bent at right angles against the form of the first lane constructed and straightened into final position before the concrete of the adjacent lane is placed, or in lieu of bent tie bars, approved 2- piece connectors may be used.

Longitudinal formed joints shall consist of a groove or cleft, extending downward from, and normal to, the surface of the pavement. These joints shall be formed by an approved mechanically or manually operated device to the dimensions and line indicated on the plans and while the concrete is in a plastic state. The groove, or cleft, shall be filled with either a pre-molded strip or poured material.

The longitudinal joint, however formed, shall be continuous. There shall be no gaps in either transverse or longitudinal joints at the intersection of the joints.

Longitudinal sawed joints shall be cut by means of approved concrete saws to the depth, width and line shown on the plans. Suitable guidelines or devices shall be used to assure cutting the longitudinal joint on the true line as shown on the plans. The longitudinal joint shall be sawed within 36 hours of placement and before any other equipment or vehicles are allowed on the pavement. The sawed area shall be thoroughly cleaned and the joint shall immediately be filled with sealer.

(b) Transverse Expansion Joints: The expansion joint fillers shall be continuous from edge to edge, shaped to the subbase and to the keyway along the edge. Preformed joint filler shall be furnished in lengths equal to the pavement width or equal to the lane width. Damaged or repaired joint filler shall not be used unless approved by the Engineer.

The expansion joint filler shall be held in a vertical position. An approved installing bar, or other device shall be used if required to secure preformed expansion joint filler at the proper grade and alignment during placing and finishing of the concrete. Finished joints shall not deviate more than 1/4 inch along horizontal alignment from a straight line. If joint fillers are assembled in sections, there shall be no offsets between adjacent units. No plugs of concrete shall be permitted anywhere within the expansion space.

(c) Transverse Contraction Joints: Transverse contraction joints shall consist of planes of weakness created by forming or cutting grooves in the surface of the pavement and, when shown on the plans, shall include transfer assemblies.

i. Formed grooves shall be made by depressing an approved tool or device into the plastic concrete. The tool or device shall remain in place until the concrete has attained its initial set and shall then be removed without disturbing the adjacent concrete, unless the device is designed to remain in the joint.
ii. Sawed contraction joints shall be created by sawing grooves in the surface of the pavement of the dimensions and at the spacing and lines shown on the plans with an approved concrete saw. After each joint is sawed, the saw cut and adjacent concrete surface shall be thoroughly cleaned.

Sawing of the joints shall commence as soon as the concrete has hardened sufficiently to permit

sawing without excessive raveling, usually 4 to 24 hours. All joints shall be sawed before uncontrolled shrinkage cracking takes place. If necessary, the sawing operations shall be carried on both during the day and night, regardless of weather conditions. The sawing of any joint shall be omitted if a crack occurs at or near the joint location prior to the time of sawing. Sawing shall be discontinued when a crack develops ahead of the saw. In general, all joints shall be sawed in sequence. If extreme conditions exist, which make it impractical to prevent erratic cracking by early sawing, the contraction joint groove shall be formed prior to initial set of concrete as provided above. The Engineer will review procedures and methods to correct random cracking.

iii. Transverse formed contraction joints shall comply with the requirements for the longitudinal formed joint.

(d) Transverse Construction Joints: Transverse construction joints shall be constructed when there is an interruption of more than 30 minutes in the concreting operations. No transverse joint shall be constructed within 10 feet of an expansion joint, contraction joint, or plane of weakness. If sufficient concrete has not been mixed at the time of interruption to form a slab at least 10 feet long, the excess concrete back to the last preceding joint shall be removed and disposed of as directed.

(e) Load Transfer Devices: Load transfer devices shall meet the requirements of M.03.08.

(f) Sealing Joints: Joints shall be sealed as soon after completion of the curing period as feasible and before the pavement is opened to traffic, including the Contractor's equipment. Just prior to sealing, each joint shall be thoroughly cleaned of all foreign material, including membrane curing compound and the joint-faces shall be clean and surface dry when the seal is applied.

The sealing material shall be applied to each joint opening to conform to the details shown on the plans or as directed by the Engineer. Material for seal applied hot shall be stirred during heating so that localized overheating does not occur. The pouring shall be done in such a manner that the material will not be spilled on the exposed surfaces of the concrete. The use of sand or similar material as a cover for the seal will not be permitted. Poured joint-sealing material shall not be placed when the air temperature in the shade is less than 50°F, unless approved by the Engineer.

Preformed elastomeric gaskets for sealing joints shall be of the cross sectional dimensions shown on the plans. Seals shall be installed by suitable tools, without elongation, and secured in place with an approved lubricant adhesive which shall cover both sides of the concrete joints. The seals shall be installed in a compressed condition and shall at time of placement be below the level of the pavement surface by approximately 1/4 inch. The seals shall be a single piece for the full width of each transverse joint.

7. Curing: Immediately after the finishing operations have been completed and as soon as marring of the concrete will not occur, the entire surface of the newly placed concrete shall be covered and cured in accordance with any of the methods below. Liquid Membrane - Forming Cure is the preferred method of curing. Moist curing and cover sheet curing must be approved by the Engineer prior to paving operations. In all cases in which curing requires the use of water, the curing shall have prior right to all water supply or supplies. Failure to use, or lack of water to adequately take care of curing and other requirements, shall be cause for immediate suspension of concreting operations. The concrete shall not be left exposed for more than 30 minutes between stages of curing or during the curing period.

When concrete is being placed and the air temperature may be expected to drop below 35°F a sufficient supply of straw, hay, grass, or other suitable blanketing material shall be provided along the work and any time the temperature may be expected to reach the freezing point during the day or night, the material so provided shall be spread over the pavement to a sufficient depth to prevent freezing of the concrete. During the period of time such protection is used, the Contractor shall be responsible for the quality and strength of the concrete placed during cold weather, and any concrete injured by frost action shall be removed and replaced at the Contractor's expense.

(a) Liquid Membrane-Forming Cure: The liquid curing compound shall be as specified in M.03.04-3. When resin-based curing compound is used, it shall be applied following the final finishing immediately after the disappearance of the water sheen and before any surface checking or marked dehydration of the concrete occurs.

When water-soluble, linseed oil-based compound is used, it shall be applied immediately following the final finishing.

The compound shall be applied by an approved self-propelled mechanical pressured sprayer delivering a fine, even spray with uniform coverage. This equipment shall be provided with a suitable device for adequate agitation of the compound to prevent settlement. For the sides of concrete pavement exposed after removal of the forms and for areas where the use of self-propelled equipment is not practical,

approved hand-held spray bars delivering a fine, even spray may be used; and the compound shall be drawn from a tank equipped with an approved device to prevent settlement or by a method which will prevent settlement.

If the compound is applied in two applications, the second application shall follow the first application within 30 minutes. The compound shall be applied in a continuous, uniform film at not less than 1 gal. per 150 s.f.

If rain falls on the newly coated pavement before the film has dried sufficiently to resist damage, or if the film is damaged in any other way, the Contractor shall be required to apply a new coat of material to the affected areas equal in curing value to that specified for the original coat. The treated surface shall be protected by the Contractor from injury for a period of at least 7 days. All traffic, either foot or otherwise, will be considered as injurious to the film of the applied compound.

The Contractor shall have on the Project sufficient approved cover sheet or cotton mats for the protection of the pavement in case of rain or breakdown of the spray equipment. The cotton mats shall be maintained in a wet condition during the period of use, as elsewhere herein specified.

Transverse joints shall be protected from the compound by means or methods approved by the Engineer.

(b) Moist Curing: After the surface of the concrete has been given its final finish, it shall be protected by covering it with moist mats of the size and quality specified in M.03.04-2. These mats shall be laid longitudinally over the surface of the finished pavement by unrolling from the supported roll so as not to bring an excessive weight upon or to mar the new surface of the pavement. After placing, these mats shall be saturated and kept saturated for a period of 7 days, at the end of which time they may be removed, and no further wetting or artificial curing will be required.

(c) Cover Sheet Curing: As soon as practicable after the finishing operations, paper or polyethylene cover sheets as specified in M.03.04-4 shall be placed in such a manner that the surface of the concrete shall not be marred. The adjoining covers shall overlap at least 18 inches, and the lap shall be securely fastened down to form a closed joint. On removal of the forms, the edges shall be covered down to the bottom of the pavement. The cover sheets shall remain in place for a period of 7 days.

In the event that hair-checking develops before the cover can be placed, the procedure set forth shall be modified at the direction of the Engineer. Moist curing mats shall then be used for the initial 24 hours of the curing period, and the cover sheets placed for the remainder of the curing period.

Before reusing paper or polyethylene covers, they shall be checked for holes or tears, and any such perforations shall be repaired. Covers which have become unserviceable will be rejected by the Engineer.

C. Protection of Pavement: The Contractor shall protect the pavement and its appurtenances against public traffic and traffic caused by its own employees and agents until the pavement is opened to traffic.

Any damage to the pavement occurring prior to the opening of the pavement to traffic by the State shall be repaired or the pavement replaced.

D. Riding Surface Tests:

1. Surface Test (Profilograph) and Correction: The finished surface of all mainline pavement shall be tested and corrected to a smoothness as described herein. Mainline pavement is defined as all pavement for traffic lanes and climbing lanes, but excluding concrete base, acceleration and deceleration lanes, and all taper sections, pavement widening, shoulders, and side street returns. Pavement on horizontal curves having a baseline radius of curvature of 6 degrees or greater and superelevation transitions of such curves will also be excluded.

The surface smoothness of pavement not classified as mainline pavement shall be determined by the Surface Test (straightedge) as described hereinafter. The smoothness of the pavement surface will be determined by using a California Type Profilograph over each designated lane to develop a Profile Index. The equipment shall be furnished and maintained by the Contractor, and will be operated by the Engineer in accordance with Department test methods.

The Contractor shall furnish paving equipment and employ methods that produce a riding surface having a Profile Index of 12 inches/mile, or less. The profile will terminate 50 feet from each pressure relief joint or existing pavement which is joined by the new pavement.

Pavement profiles will be taken 3 feet from and parallel to each edge of pavement for pavement placed at a 12 feet width or less. When pavement is placed at a greater width than 12 feet, the profile will be taken 3 feet from and parallel to each edge and at the approximate locations of each planned longitudinal joint. Additional profiles may be taken only to define the limits of an out of tolerance surface variation.

During the initial paving operations, either when starting up or after a long shut down period, the pavement surface will be tested with the profilograph as soon as the concrete has cured sufficiently to allow

testing. Membrane curing damaged during the testing operation shall be repaired by the Contractor as directed by the Engineer. The purpose of this initial testing is to aid the Contractor and the Engineer in evaluating the paving methods and equipment. Once the initial pavement smoothness, paving methods, and paving equipment are acceptable to the Engineer, the Contractor may proceed with the paving operation. Subsequent to the aforementioned initial testing, daily profiles of each day's paving will be run as soon as possible, preferably during the next working day following placement of the pavement.

A daily average Profile Index will be determined for each day's paving. A day's paving is defined as a minimum of 1000 linear feet of full-width pavement placed in a single day. If less than 1000 linear feet is paved, the day's production shall be grouped with the subsequent day's production. If an average Profile Index of 20 inches/mile is exceeded in any daily paving operation, the paving operation will be suspended and will not be allowed to resume until corrective action is taken by the Contractor. In the event that paving operations are suspended as a result of the average Profile Index exceeding 20 inches/mile, subsequent paving operations will be tested in accordance with the initial paving testing procedures.

For the purpose of determining pavement sections where corrective work or pay adjustments will be necessary, the pavement will be evaluated in 0.1 mile sections. Within each 0.1 mile section, all areas represented by high points having deviations in excess of 0.5 inches per 25 feet or less shall be removed by the Contractor with an approved grinding device or a device consisting of multiple saws. The final texture of the concrete pavement shall be such that texture, appearance, and skid resistance are comparable to adjacent sections that do not require corrective work. The use of a bush hammer or other impact device will not be permitted. Deviations in excess of 0.5 inches will be determined from the profilogram in accordance with Department Test Methods.

After removing all individual deviations in excess of 0.5 inches per 25 feet, additional cutting shall be performed if necessary to reduce the Profile Index.

On those 0.1 mile sections where corrections are necessary, second profilograph runs will be performed to verify that corrections have produced an average Profile Index 20 inches/mile or less. If the initial average Profile Index is less than 12 inches/mile, only the areas in excess of 0.5 inch deviations will be re-profiled for correction verification.

After removing all individual deviations in excess of 0.5 inches, as stated above, additional correction shall be performed if necessary to reduce the average Profile Index to 20 inches/mile or less. All correction work shall be completed prior to determinations of pavement thickness.

2. Surface Test (Straightedge) and Corrections: As soon as the concrete has hardened sufficiently, the pavement surface, except as specified herein above for mainline pavement, shall be tested by the Contractor with an approved 10 foot straightedge placed both transversely and longitudinally to the centerline at sufficient intervals to check the surface profile. Areas showing high spots of more than 1/8 inch exclusive of tining corrugations, shall be marked by the Engineer and removed by the Contractor to an elevation where the area or spot will not show surface deviations in excess of 1/8 inch when tested with a 10 foot straightedge. Correction of surface irregularities and resulting surface finish shall meet the requirements specified herein for mainline pavement.

E. Flexural Testing of Concrete: The flexural strength of the concrete pavement shall be monitored by the evaluation of compressive strength cylinders. The compressive strength specimens shall be cast and cured in the field in accordance with ASTM C31. After proper curing, these cylinders shall be transported to the Division of Materials Testing for strength evaluation.

F. Opening to Traffic: Vehicular traffic shall be excluded from the pavement until sufficient curing has taken place. The pavement shall be opened to traffic with the attainment of a compressive strength of 3,500 psi.

Any damage to the pavement from traffic or any other causes, occurring prior to acceptance of this Contract, shall be repaired by the Contractor at its own expense.

In order to expedite the opening of a pavement, or part thereof, to traffic, the Engineer may direct the Contractor to supply a concrete with a higher cement factor, or to substitute TYPE III Portland Cement for the type being used. The mix design for this concrete must be approved by the Engineer. The State will only pay the extra cost per ton for the cement used.

If the Contractor wants to increase the early strength of the pavement in order to facilitate its plan for the paving sequence, substitution of TYPE III Portland Cement, or use of a concrete with a higher cement factor may be allowed. The mix design for these types of concrete must be approved by the Engineer. The extra costs for the cement used shall be borne by the Contractor.

4.01.04—Method of Measurement: It is the intent of these specifications that the pavement shall be constructed strictly in accordance with the thickness and widths shown on the plans. The State will not be liable for payment for any thickness and widths in excess of that required.

A. Pay Adjustment for Surface Smoothness: Payment to the Contractor will be based on the average Profile Index per 0.1 mile section according to the following table:

AVERAGE PROFILE INDEX	CONTRACT UNIT PRICE ADJUSTMENT
Inches per mile per 0.1 mile section	Percentage of pavement unit bid price
0 to 6	106
over 6 to 8	104
over 8 to 10	102
over 10 to 12	100
over 12 to 14	98
over 14 to 16	96
over 16 to 18	94
over 18 to 20	92
over 20	see below

When the average Profile Index exceeds 12 inches/mile per 0.1 mile section, but does not exceed 20 inches/mile per 0.1 mile section, the Contractor may elect to accept a Contract unit price adjustment in lieu of reducing the average Profile Index.

The unit bid price adjustment will be computed using the designed thickness of Portland Cement pavement or as adjusted under 4.01.04. This unit bid price adjustment will apply to the total area of the 0.1 mile long section for the lane width represented by the profile (usually 12 feet wide). No payment will be made for any pavement which has an average profile index in excess of 20 inches/mile until corrective work has been completed by the Contractor and the pavement re-profiled to verify that the average Profile Index has been reduced to 20 inches/mile or less.

B. Pay Adjustment for Pavement Thickness:

1. Determination of Pavement Thickness: The pavement thickness shall be measured after surface correction is completed.

The thickness of fixed form pavement shall be determined by using measurements or elevations, or both, obtained by the Contractor and submitted to the Engineer prior to the placement of concrete. Thickness measurements using cores tested in accordance with AASHTO T 148 shall remain the option of the Engineer.

The thickness of slip form pavement will be determined by average caliper measurements of cores tested in accordance with AASHTO T 148.

For the purpose of establishing an adjusted unit price for pavement, units to be considered separately are defined at 1,000 linear feet of pavement in each traffic lane starting at the end of the pavement bearing the smaller station number. The last unit in each lane shall be 1,000 feet plus the fractional part of 1,000 feet remaining. One (1) core will be taken at random by the Department in each unit. When the measurement of the core from a unit is not deficient more than 0.2 inches from the plan thickness, full payment will be made. When such measurement is deficient more than 0.2 inches and not more than 1.0 inch from the plan thickness, 2 additional cores at intervals not less than 300 feet will be taken and used in the average thickness for that unit. If the average measurement of these 3 cores is not deficient more than 0.2 inches from the plan thickness, full payment will be made. If the average thickness of the 3 cores is deficient more than 0.2 inches, but not more than 1.0 inch from the plan thickness, an adjusted unit price as provided in 4.01.04-B.2 will be paid for the area represented by these cores.

Other areas such as intersections, entrances, crossover ramps, etc., will be considered as a single unit, and the thickness of each unit will be determined separately. Small irregular unit areas may be included as part of another unit. At such points as the Engineer may select in each unit, 1 core will be taken for each 1,000 s.y. of pavement, or fraction thereof, in the unit. If the core so taken is not deficient more than 0.2 inch from the plan thickness, full payment will be made. If the core is deficient in thickness by more than 0.2 inches, but not more than 1.0 inch from the plan thickness, 2 additional cores will be taken from the area represented and the average of the 3 cores determined. If the average thickness of the 3 cores is deficient more than 0.2 inch, but not more than 1.0 inch from the plan thickness, an adjusted unit price as provided in 4.01.04-B.2 will be paid for the area represented by these cores.

In calculating the average thickness of the pavement, measurements that are in excess of the specified thickness by more than 0.2 inch will be considered as the specified thickness plus 0.2 inch, and measurements that are less than the specified thickness by more than 1.0 inch will not be included in the average.

When the measurement of any core is less than the specified thickness by more than 1.0 inch, the actual thickness of the pavement in this area will be determined by taking additional cores at (not less than) 10 foot intervals parallel to the centerline in each direction from the effected location until in each direction a core is found that is not deficient by more than 1.0 inch. Areas found deficient in thickness by more than 1.0 inch shall be evaluated by the Engineer, and if so judged that the deficient areas warrant removal, they shall be removed and replaced with concrete of the thickness shown on the plans. Exploratory cores for deficient thickness will not be used in averages for adjusted unit price. The cost of all thickness measurements made to determine average thickness variation or to isolate areas that are deficient by more than 1.0 inch, including filling holes with concrete, will be deducted from any monies due or that may come due to the Contractor under the Contract at the rate of $50.00 per core.

2. Determination of Adjusted Payments: After the thickness of pavement has been determined on the basis of core measurements as hereinbefore described, the following rules for payment shall be applied.

Where the average thickness of pavement is deficient in thickness by more than 0.2 inch, but not more than 1.0 inch, payment will be made at an adjusted price as specified in the following table.

Concrete Pavement Deficiency Deficiency in Thickness Determined by Cores	Proportional part of Contract Price Allowed
Inches	Percentage
0.00 to 0.20	100
0.21 to 0.30	80
0.31 to 0.40	72
0.41 to 0.50	68
0.51 to 0.75	57
0.76 to 1.00	50

When the thickness of pavement is deficient by more than 1 inch, the Engineer will determine whether the area of such deficiency will be left in place or replaced in accordance with the provisions of 1.05.03. All additional work required and any delay to the Contractor's operation as a result of these specifications will not be cause for additional compensation for an extension of time.

On all sections of pavement which have to be removed, the Contractor shall bear the entire cost of the initial construction and the removal of the pavement including all concrete, reinforcement steel and joint material. Measured for payment hereunder will be only the items of construction entering into the replacement of the concrete pavement to the specified thickness.

C. Measurement for Payment: This work will be measured for payment as follows:

1. Concrete Pavement: The quantity of concrete included under this item shall be the number of cubic yards of accepted concrete pavement contained in the theoretical computed volume of the pavement having the required thickness and widths, but subject to adjusted proportional payment or non-payment as stated hereinbefore.

2. Fabric or Bar Mat Reinforcement: This material will be measured by the number of square yards of completed and accepted reinforced concrete pavement.

3. Transverse Expansion Joint: This material will be measured by the number of linear feet of completed and accepted transverse expansion joint.

4. Contraction Joint: This material will be measured by the number of linear feet of completed and accepted contraction joint.

5. Longitudinal Joint Support Assembly: This material will be measured by the number of joint support assemblies actually incorporated in the completed and accepted concrete pavement. If multilane construction methods are used, 2 longitudinal joint tie bars will be measured as 1 longitudinal joint support assembly.

6. Expansion Joint Filler: This material, when not part of a transverse joint assembly, will be measured by the number of linear feet of expansion joint filler of the type and thickness specified and of the width required, actually installed and accepted.

4.01.05—Basis of Payment: Payment for the concrete pavement will be made as follows:

1. Concrete: This material will be paid for at the Contract unit price per cubic yard for "Concrete for Pavement," complete in place, which price shall include all equipment, tools, labor, work incidental thereto and all materials, except reinforcement and joints.

No direct payment will be made for the concrete testing equipment, the instruction in its use, in-place testing, the concrete in or curing of the required test beams and cylinders as specified hereinbefore, but the cost of this work shall be considered included in the general cost of the work.

The transporting and testing of beams and cylinders will be done by the Department without expense to the Contractor.

2. Reinforcement: The reinforcement for the concrete pavement will be paid for at the Contract unit price per square yard of pavement for "Mat Reinforcement for Concrete Pavement," complete in place, which price shall include all materials, equipment, tools, labor and work incidental thereto.

3. Joints: The expansion joints and the contraction joints will be paid for respectively at the Contract unit prices per linear foot for "Transverse Expansion Joint" and for "Transverse Contraction Joint," complete in place, which price shall include all joint filler where required, all joint seal, load transfer devices, and all other material, equipment, tools, labor and work incidental thereto. Longitudinal joints and dummy joints including seal, will not be paid for directly, but the cost thereof shall be considered as included in the general cost of the work.

4. Longitudinal Joint Support Assembly: The devices required in connection with longitudinal construction joints will be paid for at the Contract unit price each for "Longitudinal Joint Support," complete in place, which price shall include all materials, supports, equipment, tools, labor and work incidental thereto.

5. Expansion Joint Filler: The expansion joint filler, other than that which is a part of the transverse joint assembly, will be paid for at the Contract unit price per linear foot for "Expansion Joint Filler" of the type and thickness specified, complete in place, which price shall include all joint filler, joint seal and all other materials, equipment, tools, labor and work incidental thereto.

6. Type III Portland Cement: When such material is substituted for other Portland Cement at the direction of the Engineer in order to expedite the opening of pavement to traffic, it will be paid for at the actual unit cost per barrel to the Contractor for the quantity actually incorporated in the pavement less the actual unit cost per barrel to the Contractor for an equal quantity of the Portland Cement so replaced, which cost shall include all equipment, labor, storage, transportation and work incidental to its inclusion in the concrete and incorporation in the finished pavement.

Pay Item	Pay Unit
Concrete for Pavement	c.y.
Mat Reinforcement for Concrete Pavement	s.y.
Transverse Expansion Joints	l.f.
Transverse Contraction Joints	l.f.
Longitudinal Joint Support	ea.
Type and Thickness Expansion Joint Filler	l.f.

SECTION 4.06
BITUMINOUS CONCRETE

4.06.01—Description
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4.06.03—Construction Methods
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2. **Transportation of Mixture**
3. **Paving Equipment**
4. **Test Section**
5. **Transitions for Roadway Surface**
6. **Spreading and Finishing of Mixture**
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4.06.04—Method of Measurement
4.06.05—Basis of Payment

4.06.01—Description: Work under this Section shall include the production, delivery, placement and compaction of a uniform textured, non-segregated, smooth bituminous concrete pavement to the grade and cross section shown on the plans.

The following terms as used in this specification are defined as:

Bituminous Concrete: A composite material consisting of prescribed amounts of asphalt binder and aggregates. Asphalt binder may also contain additives engineered to modify specific properties and/or behavior of the composite material. References to bituminous concrete apply to all of its forms, such as those identified as hot-mix asphalt (HMA) or polymer-modified asphalt (PMA).

Bituminous Concrete Plant (Plant): A structure where aggregates and asphalt binder are combined in a controlled fashion into a bituminous concrete mixture suitable for forming pavements and other paved surfaces.

Course: A continuous layer (a lift or multiple lifts) of the same bituminous concrete mixture placed as part of the pavement structure.

Density Lot: The total tonnage of all bituminous concrete placed in a single lift which are:

PWL density lots = When the project total estimated quantity per mixture is larger than 3,500 tons

Simple Average density lots = When the project total estimated quantity per mixture is 3,500 tons or less

Disintegration: Erosion or fragmentation of the pavement surface which can be described as polishing, weathering-oxidizing, scaling, spalling, raveling, or formation of potholes.

Dispute Resolution: A procedure used to resolve conflicts between the Engineer and the Contractor's results that may affect payment.

Hot Mix Asphalt (HMA): A bituminous concrete mixture typically produced at 325°F.

Job Mix Formula (JMF): A recommended aggregate gradation and asphalt binder content to achieve the required mixture properties.

Lift: An application of a bituminous concrete mixture placed and compacted to a specified thickness in a single paver pass.

Percent Within Limits (PWL): The percentage of the lot falling between the Upper Specification Limit (USL) and the Lower Specification Limit (LSL).

Polymer Modified Asphalt (PMA): A bituminous concrete mixture containing a polymer-modified asphalt binder and using a qualified warm mix technology.

Production Lot: The total tonnage of a bituminous concrete mixture from a single source that may receive an adjustment.

Production Sub Lot: Portion of the production lot typically represented by a single sample.

Quality Assurance (QA): All those planned and systematic actions necessary to provide CTDOT the confidence that a Contractor will perform the work as specified in the Contract.
Quality Control (QC): The sum total of activities performed by the vendor (Producer, Manufacturer, and Contractor) to ensure that a product meets contract specification requirements.
Superpave: A bituminous concrete mix design used in mixtures designated as "S*" Where "S" indicates Superpave and * indicates the sieve related to the nominal maximum aggregate size of the mix.
Segregation: A non-uniform distribution of a bituminous concrete mixture in terms of gradation, temperature, or volumetric properties.
Warm Mix Asphalt (WMA) Technology: A qualified additive or technology that may be used to produce a bituminous concrete at reduced temperatures and/or increase workability of the mixture.

4.06.02—Materials: All materials shall meet the requirements of M.04.

1. Materials Supply: The bituminous concrete mixture must be from one source of supply and originate from one Plant unless authorized by the Engineer.

2. Recycled Materials: Reclaimed Asphalt Pavement (RAP), Crushed Recycled Container Glass (CRCG), Recycled Asphalt Shingles (RAS), or crumb rubber (CR) from recycled tires may be incorporated in bituminous concrete mixtures in accordance with Project Specifications.

4.06.03—Construction Methods

1. Material Documentation: All vendors producing bituminous concrete must have Plants with automated vehicle-weighing scales, storage scales, and material feeds capable of producing a delivery ticket containing the information below.

a. State of Connecticut printed on ticket.
b. Name of Producer, identification of Plant, and specific storage silo if used.
c. Date and time.
d. Mixture Designation, mix type and level. Curb mixtures for machine-placed curbing must state "curb mix only."
e. If WMA Technology is used, "-W" must be listed following the mixture designation.
f. Net weight of mixture loaded into the vehicle. (When RAP and/or RAS is used, the moisture content shall be excluded from mixture net weight.)
g. Gross weight (equal to the net weight plus the tare weight or the loaded scale weight).
h. Tare weight of vehicle (daily scale weight of the empty vehicle).
i. Project number, purchase order number, name of Contractor (if Contractor other than Producer).
j. Vehicle number - unique means of identification of vehicle.
k. For Batch Plants: individual aggregate, recycled materials, and virgin asphalt max/target/min weights when silos are not used.
l. For every mixture designation: the running daily and project total delivered and sequential load number.

The net weight of mixture loaded into the vehicle must be equal to the cumulative measured weights of its components.

The Contractor must notify the Engineer immediately if, during production, there is a malfunction of the weight recording system in the automated Plant. Manually written tickets containing all required information will be allowed for no more than 1 hour.

The State reserves the right to have an Inspector present to monitor batching and/or weighing operations.

2. Transportation of Mixture: The mixture shall be transported in vehicles that are clean of all foreign material, excessive coating or cleaning agents, and that have no gaps through which material might spill. Any material spilled during the loading or transportation process shall be quantified by re-weighing the vehicle. The Contractor shall load vehicles uniformly so that segregation is minimized. Loaded vehicles shall be tightly covered with waterproof covers acceptable to the Engineer. Mesh covers are prohibited. The cover must minimize air infiltration. Vehicles found not to be in conformance shall not be loaded

Vehicles with loads of bituminous concrete being delivered to State projects must not exceed the statutory or permitted load limits referred to as gross vehicle weight (GVW). The Contractor shall furnish a list and allowable weights of all vehicles transporting mixture. The State reserves the right to check the gross and tare weight of any vehicle. If the gross or tare weight varies from that shown on the delivery ticket by more than 0.4%, the Engineer will recalculate the net weight. The Contractor shall correct the discrepancy to the satisfaction of the Engineer.

If a vehicle delivers mixture to the Project and the delivery ticket indicates that the vehicle is overweight,

the load may not be rejected but a "Measured Weight Adjustment" will be taken in accordance with 4.06.04.

Vehicle body coating and cleaning agents must not have a deleterious effect on the mixture. The use of solvents or fuel oil, in any concentration, is prohibited for the coating of vehicle bodies.

For each delivery, the Engineer shall be provided a clear, legible copy of the delivery ticket.

3. Paving Equipment: The Contractor shall have the necessary paving and compaction equipment at the Project Site to perform the work. All equipment shall be in good working order and any equipment that is worn, defective, or inadequate for performance of the work shall be repaired or replaced by the Contractor to the satisfaction of the Engineer. During the paving operation, the use of solvents or fuel oil, in any concentration, is strictly prohibited as a release agent or cleaner on any paving equipment (i.e., rollers, pavers, transfer devices, etc.).

Refueling or cleaning of equipment is prohibited in any location on the Project where fuel or solvents might come in contact with paved areas or areas to be paved. Solvents used in cleaning mechanical equipment or hand tools shall be stored clear of areas paved or to be paved. Before any such equipment and tools are cleaned, they shall be moved off of areas paved or to be paved.

Pavers: Each paver shall have a receiving hopper with sufficient capacity to provide for a uniform spreading operation and a distribution system that places the mix uniformly, without segregation. The paver shall be equipped with and use a vibratory screed system with heaters or burners. The screed system shall be capable of producing a finished surface of the required evenness and texture without tearing, shoving, or gouging the mixture. Pavers with extendible screed units as part of the system shall have auger extensions and tunnel extenders as necessary. Automatic screed controls for grade and slope shall be used at all times unless otherwise authorized by the Engineer. The controls shall automatically adjust the screed to compensate for irregularities in the preceding course or existing base. The controls shall maintain the proper transverse slope and be readily adjustable, and shall operate from a fixed or moving reference such as a grade wire or floating beam (minimum length 20 feet).

Rollers: All rollers shall be self-propelled and designed for compaction of bituminous concrete. Roller types shall include steel wheeled, pneumatic, or a combination thereof. Rollers that operate in a dynamic mode shall have drums that use a vibratory or oscillatory system or combination. Vibratory rollers shall be equipped with indicators for amplitude, frequency, and speed settings/readouts to measure the impacts per foot during the compaction process. Oscillatory rollers shall be equipped with frequency indicators. Rollers can operate in the dynamic mode using the oscillatory system on concrete structures such as bridges and catch basins if at the lowest frequency setting.

Pneumatic tire rollers shall be equipped with wide-tread compaction tires capable of exerting an average contact pressure from 60 to 90 psi uniformly over the surface. The Contractor shall furnish documentation to the Engineer regarding tire size, pressure and loading to confirm that the proper contact pressure is being developed and that the loading and contact pressure are uniform for all wheels.

Lighting: For paving operations which will be performed during hours of darkness the paving equipment shall be equipped with lighting fixtures as described below or with an approved equal. Lighting shall minimize glare to passing traffic. The lighting options and minimum number of fixtures are listed in Tables 4.06-1 and 4.06-2.

TABLE 4.06-1: Minimum Paver lighting

Option	Fixture Configuration	Fixture Quantity	Requirement
1	Type A	3	Mount over screed area
	Type B (narrow) or Type C (spot)	2	Aim to auger and guideline
	Type B (wide) or Type C (flood)	2	Aim 25 feet behind paving machine
2	Type D Balloon	2	Mount over screed area

TABLE 4.06-2: Minimum Roller Lighting

Option	Fixture Configuration	Fixture Quantity	Requirement
1	Type B (wide)	2	Aim 50 feet in front of and behind roller
	Type B (narrow)	2	Aim 100 feet in front of and behind roller
2	Type C (flood)	2	Aim 50 feet in front of and behind roller
	Type C (spot)	2	Aim 100 feet in front of and behind roller
3	Type D Balloon	1	Mount above the roller

*All fixtures shall be mounted above the roller.

Type A: Fluorescent fixture shall be heavy duty industrial type. Each fixture shall have a minimum output of 8,000 lumens. The fixtures shall be mounted horizontally and be designed for continuous row installation.

Type B: Each floodlight fixture shall have a minimum output of 18,000 lumens.

Type C: Each fixture shall have a minimum output of 19,000 lumens.

Type D: Balloon light – each balloon light fixture shall have minimum output of 50,000 lumens and emit light equally in all directions.

Material Transfer Vehicle (MTV): A MTV shall be used when placing bituminous concrete surface course (a lift or multiple lifts) as indicated in the Contract except as noted on the plans or as directed by the Engineer. In addition, continuous paving lengths of less than 500 feet may not require the use of a MTV as determined by the Engineer.

The MTV must be a vehicle specifically designed for the purpose of delivering the bituminous concrete mixture from the delivery vehicle to the paver. The MTV must continuously remix the bituminous concrete mixture throughout the placement process.

The use of a MTV will be subject to the requirements stated in 1.07.05 Load Restrictions. The Engineer may limit the use of the vehicle if it is determined that the use of the MTV may damage highway components, utilities, or bridges. The Contractor shall submit to the Engineer at time of pre-construction the following information:

1. The make and model of the MTV.
2. The individual axle weights and axle spacing for each piece of paving equipment (haul vehicle, MTV and paver).
3. A working drawing showing the axle spacing in combination with all pieces of equipment that will comprise the paving echelon.

4. Test Section: The Engineer may require the Contractor to place a test section whenever the requirements of this specification or M.04 are not met.

The Contractor shall submit the quantity of mixture to be placed and the location of the test section for review and approval by the Engineer. The same equipment used in the construction of a passing test section shall be used throughout production.

If a test section fails to meet specifications, the Contractor shall stop production, make necessary adjustments to the job mix formula, Plant operations, or procedures for placement and compaction. The Contractor shall construct test sections, as allowed by the Engineer, until all the required specifications are met. All test sections shall also be subject to removal as set forth in 1.06.04.

5. Transitions for Roadway Surface: Transitions shall be formed at any point on the roadway where the pavement surface deviates, vertically, from the uniform longitudinal profile as specified on the plans. Whether formed by milling or by bituminous concrete mixture, all transition lengths shall meet the criteria below unless otherwise specified.

Permanent Transitions: Defined as any gradual change in pavement elevation that remains as a permanent part of the work.

A transition shall be constructed no closer than 75 feet from either side of a bridge expansion joint or parapet. All permanent transitions, leading and trailing ends shall meet the following length requirements:

Posted Speed Limit	Permanent Transition Length Required
> 35 mph	30 feet per inch of elevation change
35 mph or less	15 feet per inch of elevation change

In areas where it is impractical to use the above-described permanent transition lengths, the use of a shorter permanent transition length may be permitted when approved by the Engineer.

Temporary Transitions: Defined as a transition that does not remain a permanent part of the work. All temporary transitions shall meet the following length requirements:

Posted Speed Limit	Temporary Transition Length Required
> 50 mph	Leading Transition: 15 feet per inch of vertical change (thickness) Trailing Transition: 6 feet per inch of vertical change (thickness)
40, 45 or 50 mph	Leading and Trailing: 4 feet per inch of vertical change (thickness)
35 mph or less	Leading and Trailing: 3 feet per inch of vertical change (thickness)

Note: Any temporary transition to be in place over the winter shutdown period or during extended periods of inactivity (more than 14 calendar days) shall meet the greater than 50 mph requirements shown above.

6. Spreading and Finishing of Mixture: Prior to the placement of the mixture, the underlying base course shall be brought to the plan grade and cross section within the allowable tolerance.

Immediately before placing a bituminous concrete lift, a uniform coating of tack coat shall be applied to all existing underlying pavement surfaces and on the exposed surface of a wedge joint. Such surfaces shall be clean and dry. Sweeping or other means acceptable to the Engineer shall be used.

The mixture shall not be placed whenever the surface is wet or frozen.

Tack Coat Application: The tack coat shall be applied by a pressurized spray system that results in uniform overlapping coverage at an application rate of 0.03 to 0.05 gal./s.y. for a non-milled surface and an application rate of 0.05 to 0.07 gal./s.y. for a milled surface. For areas where both milled and un-milled surfaces occur, the tack coat shall be an application rate of 0.03 to 0.05 gal /s.y. The Engineer must approve the equipment and the method of measurement prior to use. The material for tack coat shall be heated to 160°F ± 10°F and shall not be further diluted.

Tack coat shall be allowed sufficient time to break prior to any paving equipment or haul vehicles driving on it.

The Contractor may request to omit the tack coat application between bituminous concrete layers that have not been exposed to traffic and are placed during the same work shift. Requests to omit tack coat application on the upper and lower surfaces of a wedge joint will not be considered.

Placement: The mixture shall be placed and compacted to provide a smooth, dense surface with a uniform texture and no segregation at the specified thickness and dimensions indicated in the plans and specifications.

When unforeseen weather conditions prevent further placement of the mixture, the Engineer is not obligated to accept or place the bituminous concrete mixture that is in transit from the Plant.

In advance of paving, traffic control requirements shall be set up, maintained throughout placement, and shall not be removed until all associated work including density testing is completed.

The mixture temperature will be verified by means of a probe or infrared type of thermometer. The placement temperature range shall be listed in the Quality Control Plan (QCP) for placement and shall meet the requirements of Table M.04.03-4. Any HMA material that that falls outside the specified temperature range as measured by a probe thermometer may be rejected.

The Contractor shall inspect the newly placed pavement for defects in mixture or placement before rolling

is started. Any deviation from standard crown or section shall be immediately remedied by placing additional mixture or removing surplus mixture. Such defects shall be corrected to the satisfaction of the Engineer.

Where it is impracticable due to physical limitations to operate the paving equipment, the Engineer may permit the use of other methods or equipment. Where hand spreading is permitted, the mixture shall be placed by means of suitable shovels and other tools, and in a uniformly loose layer at a thickness that will result in a completed pavement meeting the designed grade and elevation.

Placement Tolerances: Each lift of bituminous concrete placed at a specified thickness shall meet the following requirements for thickness and area. Any pavement exceeding these limits shall be subject to an adjustment or removal. Lift tolerances will not relieve the Contractor from meeting the final designed grade. Lifts of specified non-uniform thickness, i.e. wedge or shim course, shall not be subject to thickness and area adjustments.

a) Thickness: Where the average thickness of the lift exceeds that shown on the plans beyond the tolerances shown in Table 4.06-3, the Engineer will calculate the thickness adjustment in accordance with 4.06.04.

TABLE 4.06-3: Thickness Tolerances

Mixture Designation	Lift Tolerance
S1	+/- 3/8 inch
S0.25, S0.375, S0.5	+/- 1/4 inch

Where the thickness of the lift of mixture is less than that shown on the plans beyond the tolerances shown in Table 4.06-3, the Contractor, with the approval of the Engineer, shall take corrective action in accordance with this Section.

b) Area: Where the width of the lift exceeds that shown on the plans by more than the specified thickness, the Engineer will calculate the area adjustment in 4.06.04.

c) Delivered Weight of Mixture: When the delivery ticket shows that the truck exceeds the allowable gross weight for the vehicle type, the Engineer will calculate the weight adjustment in accordance with 4.06.04.

Transverse Joints: All transverse joints shall be formed by saw-cutting to expose the full thickness of the lift. Tack coat shall be applied to the sawn face immediately prior to additional mixture being placed.

Compaction: The Contractor shall compact the mixture to meet the density requirements as stated in 4.06.04 and eliminate all roller marks without displacement, shoving cracking, or aggregate breakage.

When placing a lift with a specified thickness less than 1 1/2 inches, or a wedge course, the Contractor shall provide a minimum rolling pattern as determined by the development of a compaction curve. The procedure to be used shall be documented in the Contractor's QCP for placement and demonstrated on the first day of placement.

The use of the vibratory system on concrete structures is prohibited. When approved by the Engineer, the Contractor may operate a roller using an oscillatory system at the lowest frequency setting.

If the Engineer determines that the use of compaction equipment in the dynamic mode may damage highway components, utilities or adjacent property, the Contractor shall provide alternate compaction equipment.

Rollers operating in the dynamic mode shall be shut off when changing directions.

These allowances will not relieve the Contractor from meeting pavement compaction requirements.

Surface Requirements:

Each lift of the surface course shall not vary more than 1/4 inch from a Contractor-supplied 10 foot straightedge. For all other lifts of bituminous concrete, the tolerance shall be 3/8 inch. Such tolerance will apply to all paved areas.

Any surface that exceeds these tolerances shall be corrected by the Contractor at its own expense.

7. Longitudinal Joint Construction Methods: The Contractor shall use Method I - Notched Wedge Joint (see Figure 4.06-1) when constructing longitudinal joints where lift thicknesses are 1 1/2 inches to 3 inches. S1.0 mixtures shall be excluded from using Method I. Method II - Butt Joint (see Figure 4.06-2) shall be used for lifts less than 1 1/2 inches or greater than 3 inches. Each longitudinal joint shall maintain a consistent offset from the centerline of the roadway along its entire length. The difference in elevation between the two faces of any completed longitudinal joint shall not exceed 1/4 inch at any location.

Method I - Notched Wedge Joint:

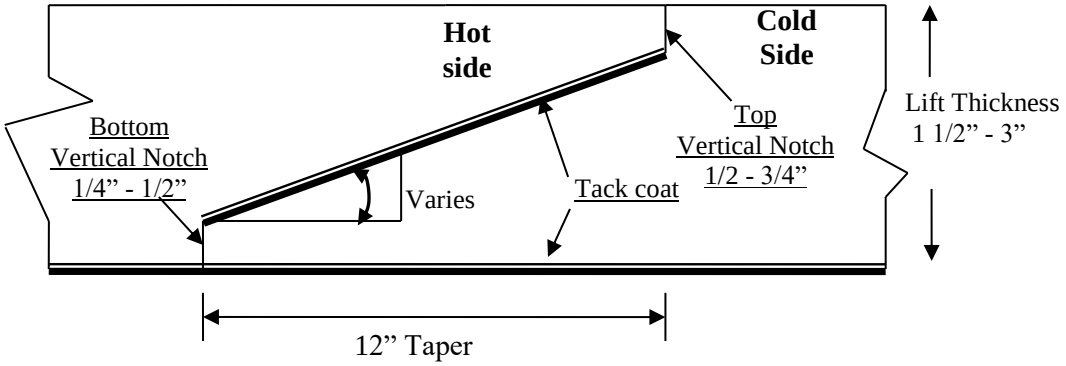
A notched wedge joint shall be constructed as shown in Figure 4.06-1 using a device that is attached to the paver screed and is capable of independently adjusting the top and bottom vertical notches. The device shall have an integrated vibratory system. The top vertical notch must be located at the centerline or lane line in the final lift. The requirement for paving full width "curb to curb" as described in Method II may be waived if addressed in the QC plan and approved by the Engineer.

The taper portion of the wedge joint shall be evenly compacted using equipment other than the paver or notch wedge joint device. The compaction device shall be the same width as the taper and not reduce the angle of the wedge or ravel the top notch of the joint during compaction.

When placed on paved surfaces, the area below the sloped section of the joint shall be treated with tack coat. The top surface of the sloped section of the joint shall be treated with tack coat prior to placing the completing pass.

The taper portion of the wedge joint shall not be exposed to traffic for more than 5 calendar days.

Figure 4.06-1: Notched Wedge Joint (Not to Scale)



Any exposed wedge joint must be located to allow for the free draining of water from the road surface.

The Engineer reserves the right to define the paving limits when using a wedge joint that will be exposed to traffic.

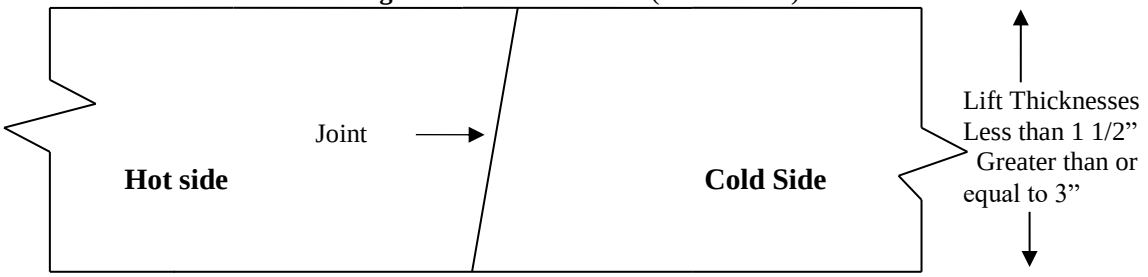
If Method I cannot be used on those lifts which are 1 1/2 inches to 3 inches, Method III may be substituted according to the requirements below for "Method III - Butt Joint with Hot Poured Rubberized Asphalt Treatment."

Method II - Butt Joint:

When adjoining passes are placed, the Contractor shall use the end gate to create a near vertical edge (refer to Figure 4.06-2). The completing pass (hot side) shall have sufficient mixture so that the compacted thickness is not less than the previous pass (cold side). During placement of multiple lifts, the longitudinal joint shall be constructed in such a manner that it is located at least 6 inch from the joint in the lift immediately below. The joint in the final lift shall be at the centerline or at lane lines. The end gate on the paver should be set so there is an overlap onto the cold side of the joint.

The Contractor shall not allow any butt joint to be incomplete at the end of a work shift unless otherwise allowed by the Engineer. When using this method, the Contractor is not allowed to leave a vertical edge exposed at the end of a work shift and must complete paving of the roadway full width "curb to curb."

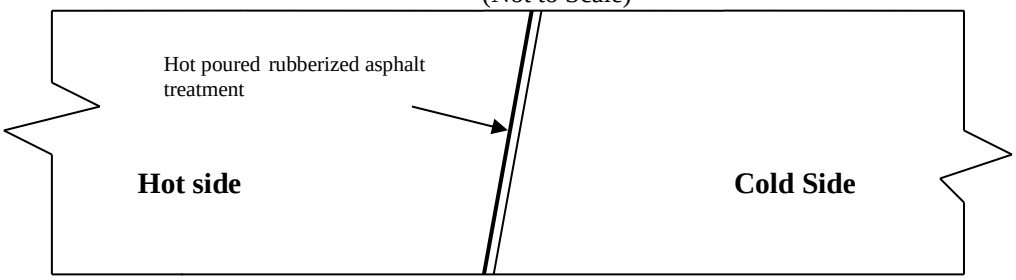
Figure 4.06-2: Butt Joint (Not to Scale)



Method III - Butt Joint with Hot Poured Rubberized Asphalt Treatment:

If Method I cannot be used due to physical constraints in certain limited locations, the Contractor may submit a request in writing for approval by the Engineer to use Method III as a substitution in those locations. There shall be no additional measurement or payment made when Method III is substituted for Method I. When required by the Contract or approved by the Engineer, Method III (see Figure 4.06-3) shall be used.

Figure 4.06-3: Butt Joint with Hot Poured Rubberized Asphalt Treatment
(Not to Scale)



All of the requirements of Method II must be met with Method III. In addition, the longitudinal vertical edge must be treated with a rubberized joint seal material meeting the requirements of ASTM D6690, Type 2. The joint sealant shall be placed on the face of the "cold side" of the butt joint as shown above prior to placing the "hot side" of the butt joint. The joint seal material shall be applied in accordance with the manufacturer's recommendation so as to provide a uniform coverage and avoid excess bleeding onto the newly placed pavement.

8. Contractor Quality Control (QC) Requirements: The Contractor shall be responsible for maintaining adequate quality control procedures throughout the production and placement operations. Therefore, the Contractor must ensure that the materials, mixture, and work provided by Subcontractors, Suppliers, and Producers also meet Contract specification requirements.

This effort must be documented in Quality Control Plans (QCP) and must address the actions, inspection, or sampling and testing necessary to keep the production and placement operations in control, to determine when an operation has gone out of control and to respond to correct the situation in a timely fashion.

The Standard QCP for production shall consist of the quality control program specific to the production facility.

There are 3 components to the QCP for placement: a Standard QCP, a Project Summary Sheet that details Project-specific information, and, if applicable, a separate Extended Season Paving Plan as required in 4.06.03-9 "Temperature and Seasonal Requirements."

The Standard QCP for both production and placement shall be submitted to the Department for approval each calendar year and at a minimum of 30 days prior to production or placement.

Production or placement shall not occur until all QCP components have been approved by the Engineer.

Each QCP shall include the name and qualifications of a Quality Control Manager (QCM). The QCM shall be responsible for the administration of the QCP, and any modifications that may become necessary.

The QCM shall have the ability to direct all Contractor personnel on the Project during paving operations.

The QCPs shall also include the name and qualifications of any outside testing laboratory performing any

QC functions on behalf of the Contractor. The QC Technician performing in-place density testing shall be NETTCP certified as a paving inspector.

Approval of the QCP does not relieve the Contractor of its responsibility to comply with the Project specifications. The Contractor may modify the QCPs as work progresses and must document the changes in writing prior to resuming operations. These changes include but are not limited to changes in quality control procedures or personnel. The Department reserves the right to deny significant changes to the QCPs.

QCP for Production: Refer to M.04.03-1.

QCP for Placement: The Standard QCP, Project Summary Sheet, and Extended Season Paving Plan shall conform to the format provided by the Engineer. The format is available at https://portal.ct.gov/DOT/Office-of-Construction/Advisory-Team

The Contractor shall perform all quality control sampling and testing, provide inspection, and exercise management control to ensure that bituminous concrete placement conforms to the requirements as outlined in its QCP during all phases of the work. The Contractor shall document these activities for each day of placement.

The Contractor shall submit complete field density testing and inspection records to the Engineer within 48 hours in a manner acceptable to the Engineer.

The Contractor may obtain one mat core and one joint core per day for process control, provided this process is detailed in the QCP. The results of these process control cores shall not be used to dispute the Department's determinations from the acceptance cores. The Contractor shall submit the location of each process control core to the Engineer for approval prior to taking the core. The core holes shall be filled to the same requirements described in 4.06.03-10.

9. Temperature and Seasonal Requirements: Paving, including placement of temporary pavements, shall be divided into 2 seasons, "In-Season" and "Extended-Season." In-Season paving occurs from May 1 to October 14, and Extended Season paving occurs from October 15 to April 30. The following requirements shall apply unless otherwise authorized or directed by the Engineer:

- Mixtures shall not be placed when the air or subbase temperature is less than 40°F regardless of the season.
- Should paving operations be scheduled during the Extended Season, the Contractor must submit an Extended Season Paving Plan for the Project that addresses minimum delivered mix temperature considering WMA, PMA, or other additives; maximum paver speed; enhanced rolling patterns; and the method to balance mixture delivery and placement operations. Paving during Extended Season shall not commence until the Engineer has approved the plan.

10. Field Density: The Contractor shall obtain cores for the determination of mat and longitudinal joint density of bituminous concrete pavements. Within five calendar days of placement, mat and joint cores shall be extracted on each lift with a specified thickness of 1 1/2 inches or more. Joint cores shall not be extracted on HMA S1.0 lifts.

The Contractor shall extract cores from random locations determined by the Engineer in accordance with ASTM D3665. 4 or 6 inch diameter cores shall be extracted for S0.25, S0.375 and S0.5 mixtures; 6 inch diameter cores shall be required for S1.0 mixtures. The Contractor shall coordinate with the Engineer to witness the extraction, labeling of cores, and filling of the core holes.

Each lift will be separated into lots as follows:

a. Simple Average Density Lots: For total estimated quantities below 2,000 tons, the lift will be evaluated in one lot which will include the total paved tonnage of the lift and all longitudinal joints between the curb lines.
   For total estimated quantities between 2,000 and 3,500 tons, the lift will be evaluated in two lots in which each lot will include approximately half of the total tonnage placed for the full paving width of a lift including all longitudinal joints between the curb lines.
b. PWL Density Lots: Mat density lots will include each 3,500 tons of mixture placed within 30 calendar days. Joint density lots will include 14,000 linear feet of constructed joints. Bridge density lots will always be analyzed using simple average lot methodology.
c. Partial Density Lot (For PWL only): A mat density lot with less than 3,500 tons or a joint density lot with less than 14,000 linear feet due to:
   - completion of the course; or
   - a lot spanning 30 calendar days.

Prior to paving, the type and number of lot(s) will be determined by the Engineer. Noncontiguous areas

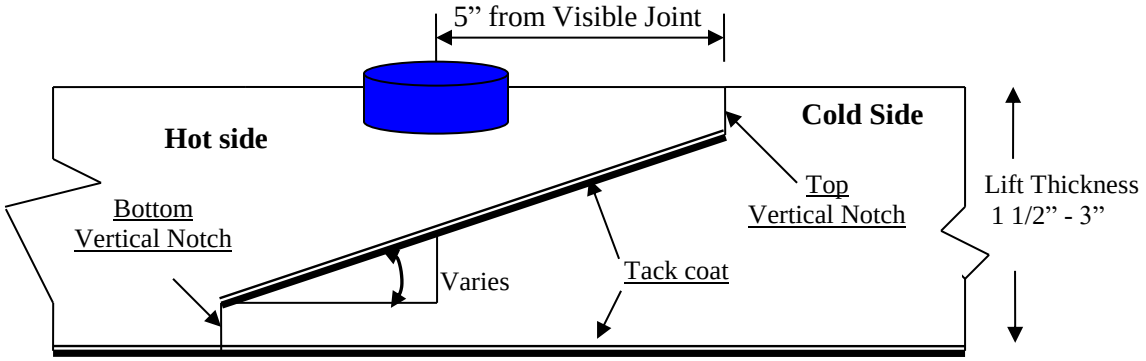
such as highway ramps may be combined to create one lot.

After the lift has been compacted and cooled, the Contractor shall cut cores to a depth equal to or greater than the lift thickness and shall remove them without damaging the lift(s) to be tested. Any core that is damaged or obviously defective while being obtained will be replaced with a new core from a location within 2 feet measured in a longitudinal direction.

A mat core shall not be located any closer than 1 foot from the edge of a paver pass. If a random number locates a core less than 1 foot from any edge, the location will be adjusted by the Engineer so that the outer edge of the core is 1 foot from the edge of the paver pass.

Method I, Notched Wedge Joint cores shall be taken so that the center of the core is 5 inches from the visible joint on the hot mat side (Figure 4.06-4).

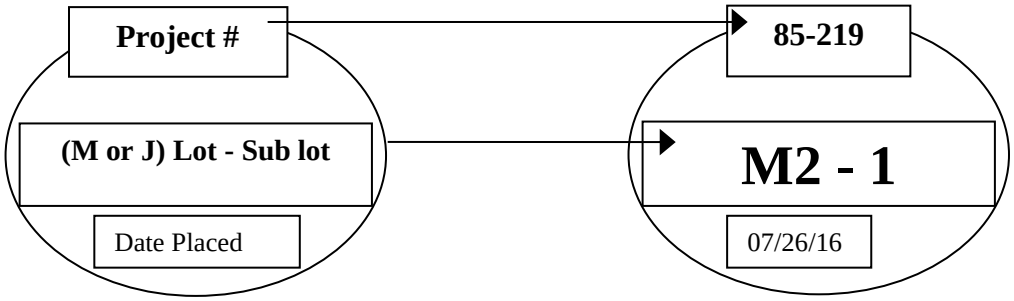
Figure 4.06-4: Notched Wedge Joint Cores (Not to Scale)



When Method II or Method III Butt Joint is used, cores shall be taken from the hot side so the edge of the core is within 1 inch of the longitudinal joint.

The cores shall be labeled by the Contractor with the Project number, date placed, lot number, and sub-lot number. The core's label shall include "M" for a mat core and "J" for a joint core. For example, a mat core from the first lot and the first sub-lot shall be labeled with "M1 – 1." A mat core from the second lot and first sub-lot shall be labeled "M2-1" (see Figure 4.06-5). The Engineer will fill out a MAT-109 to accompany the cores. The Contractor shall deliver the cores and MAT-109 to the Department's Central Lab. The Contractor shall use a container approved by the Engineer. The container shall have a lid capable of being locked shut and tamper proof. The Contractor shall use foam, bubble wrap, or another suitable material to prevent the cores from being damaged during handling and transportation. Once the cores and MAT-109 are in the container the Engineer will secure the lid using security seals at the removable hinges(s) and at the lid opening(s). The security seals' identification number must be documented on the MAT-109. All sealed containers shall be delivered to the Department's Central Lab within two working days from time of extraction. Central Lab personnel will break the security seal and take possession of the cores.

Figure 4.06-5: Labeling of Cores



Each core hole shall be filled within 4 hours upon core extraction. Prior to being filled, the hole shall be prepared by removing any free water and applying tack coat using a brush or other means to uniformly cover the cut surface. The core hole shall be filled using a bituminous concrete mixture at a minimum temperature of 240°F containing the same or smaller nominal maximum aggregate size and compacted with a hand compactor or other mechanical means to the maximum compaction possible. The bituminous concrete shall be compacted to 1/8 inch above the finished pavement.

Simple Average Density Lots:

A standard simple average density lot is the quantity of material placed within the defined area excluding any bridge decks.

A combo simple average density lot is the quantity of material placed within the defined area including bridge decks less than or equal to 500 feet long.

A bridge simple average density lot is the quantity of material placed on a bridge deck longer than 500 feet.

The number of cores per lot shall be determined in accordance with Table 4.06-4. If a randomly selected mat or joint core location is on a bridge deck, the core is to be obtained on the bridge deck in addition to the core(s) required on the bridge deck.

The number of cores per lot shall be determined in accordance with Table 4.06-5. Multiple bridge decks can be combined into one lot if the paving and underlying conditions are comparable. If multiple bridge decks are combined into a single bridge lot, at least one mat and joint core shall be obtained on each bridge.

The longitudinal locations of mat cores within a standard, combo, or bridge lot containing multiple paving passes will be determined using the combined length of the paving passes within the lot.

TABLE 4.06-4: Number of Cores per Lot (Simple Average)

Lot Type	No. of Mat Cores		No. of Joint Cores	
Standard Lot < 500 Tons	3		3	
Standard Lot ≥ 500 Tons	4		4	
Combo Lot < 500 Tons	2 plus	1 per bridge (≤ 300')	2 plus	1 per bridge (≤ 300)
Combo Lot ≥ 500 Tons[1]	4 plus	2 per bridge (301' – 500')	4 plus	2 per bridge (301' – 500')

TABLE 4.06-5: Number of Core per Bridge Density Lot (Simple Average)

Length of Bridge(s) (Feet)	Minimum No. of Mat Cores	Minimum No. of Joint Cores
< 500	2	2
501 – 1,500	3	3
1,501 – 2,500	4	4
2,501 and greater	5	5

PWL Density Lots:

A PWL mat density lot is 3,500 tons of material placed within the defined area excluding any bridges. One mat core will be obtained per every 500 tons placed.

A PWL joint density lot is 14,000 linear feet of longitudinal joint excluding any joints on bridge decks. One joint core will be obtained per every 2,000 linear feet of joint.

Bridge density lots will always be analyzed as using the simple average lot methodology. The number of cores per lot shall be determined in accordance with Table 4.06-5. Multiple bridge decks can be combined into one lot if the paving and underlying conditions are comparable. If multiple bridge decks are combined into a single bridge lot, at least one mat and joint core shall be obtained on each bridge.

11. Acceptance Sampling and Testing: Sampling shall be performed in accordance with ASTM D3665 or a statistically-based procedure of stratified random sampling approved by the Engineer.

Plant Material Acceptance: The Contractor shall provide the required sampling and testing during all phases of the work in accordance with M.04. The Department will verify the Contractor's acceptance test

results. Should any test results exceed the specified tolerances in the Department's current QA Program for Materials, the Contractor's test results for a subject lot or sub lot may be replaced with the Department's results for the purpose of calculating adjustments. The verification procedure is included in the Department's current QA Program for Materials.

Density Acceptance: The Engineer will perform all acceptance testing in accordance with AASHTO T 331. The density of each core will be determined using the daily production's average maximum theoretical specific gravity (Gmm) established during the testing of the parent material at the Plant. When there was no testing of the parent material or any Gmm exceeds the specified tolerances in the Department's current QA Program for Materials, the Engineer will determine the maximum theoretical density value to be used for density calculations.

12. Density Dispute Resolution Process: The Contractor and Engineer will work in partnership to avoid potential conflicts and to resolve any differences that may arise during quality control or acceptance testing for density. Both parties will review their sampling and testing procedures and results and share their findings. If the Contractor disputes the Engineer's test results, the Contractor must submit in writing a request to initiate the Dispute Resolution Process within five calendar days of the notification of the test results. No request for dispute resolution will be allowed unless the Contractor provides quality control results from samples taken prior to and after finish rolling, and within the timeframe described in 4.06.03-8 supporting its position. No request for dispute resolution will be allowed for a density lot in which any core was not taken within the required 5 calendar days of placement. Should the dispute not be resolved through evaluation of existing testing data or procedures, the Engineer may authorize the Contractor to obtain a new core or set of core samples per disputed lot. The core samples must be extracted no later than 7 calendar days from the date of the Engineer's authorization. All such core samples shall be extracted and the core hole filled using the procedure outlined in 4.06.03-10.

(a) Simple Average Lots: The Contractor may only dispute any simple average lot that is adjusted at or below 95 percent payment. The number and location (mat, joint, or structure) of the cores taken for dispute resolution must reflect the number and location of the original cores. The location of each core shall be randomly located within the respective original sub lot. The dispute resolution results shall be combined with the original results and averaged for determining the final in-place density value.

(b) PWL Lots: The Contractor may dispute any PWL sublot when the PWL falls below 50% calculated in accordance with 4.06.04-2b. An additional random core in the sublot may be taken to validate the accuracy of the core in question. The Department will verify the additional core test result and may average the original test result with the additional core result for purpose of calculating adjustments.

13. Corrective Work Procedure:

If pavement placed by the Contractor does not meet the specifications, and the Engineer requires its replacement or correction, the Contractor shall:

(a) Propose a corrective procedure to the Engineer for review and approval prior to any corrective work commencing. The proposal shall include:
- Limits of pavement to be replaced or corrected, indicating stationing or other landmarks that are readily distinguishable.
- Proposed work schedule.
- Construction method and sequence of operations.
- Methods of maintenance and protection of traffic.
- Material sources.
- Names and telephone numbers of supervising personnel.

(b) Any corrective courses placed as the final wearing surface shall match the specified lift thickness after completion.

14. Protection of the Work: The Contractor shall protect all sections of the newly finished pavement from damage that may occur as a result of the Contractor's operations for the duration of the Project.

15. Cut Bituminous Concrete Pavement: Work under this item shall consist of making a straight-line cut in the bituminous concrete pavement to the lines delineated on the plans or as directed by the Engineer. The cut shall provide a straight, clean, vertical face with no cracking, tearing or breakage along the cut edge.

4.06.04—Method of Measurement:

1. HMA S* or PMA S*: Bituminous concrete will be measured for payment as the amount of material in tons placed as determined by the net weight on the delivered tickets and adjusted by area, thickness and

weight as follows:

Quantity Adjustments**:** Adjustments may be applied to the placed bituminous concrete quantities that will be measured for payment using the following formulas:

Yield Factor for Adjustment Calculation = 0.0575 tons/SY/inch

Actual Area (SY) = [(Measured Length (ft)) × (Avg. of width measurements (ft))]÷9 s.f./SY

Actual Thickness (t) = Total tons delivered / [Actual Area (SY) × 0.0575 tons/SY/inch]

(a) Area: If the average width exceeds the allowable tolerance, an adjustment will be made using the following formula. The tolerance for width is equal to the specified thickness (inch) of the lift being placed.

Quantity Adjusted for Area (T_A) = [(L × W_{adj})/9] × (t) × 0.0575 Tons/SY/inch = (-) tons

Where: L = Length (ft)
(t) = Actual thickness (inches)
W_{adj} = (Designed width (ft) + tolerance /12) - Measured Width)

(b) Thickness: If the actual average thickness is less than the allowable tolerance, the Contractor shall submit a repair procedure to the Engineer for approval. If the actual thickness exceeds the allowable tolerance, an adjustment will be made using the following formula:

Quantity Adjusted for Thickness (T_T) = A × t_{adj} × 0.0575 = (-) tons

Where: A = Area = {[L × (Design width + tolerance (lift thickness)/12)] / 9}
t_{adj} = Adjusted thickness = [(Dt + tolerance) - Actual thickness]
Dt = Designed thickness (inches)

(c) Weight: If the quantity of bituminous concrete representing the mixture delivered to the Project is in excess of the allowable gross vehicle weight (GVW) for each vehicle, an adjustment will be made using the following formula:

Quantity Adjusted for Weight (T_W) = GVW – DGW= (-) tons

Where: DGW = Delivered gross weight as shown on the delivery ticket or measured on a certified scale

2. Bituminous Concrete Adjustment Cost:

(a) Production Lot Adjustment: An adjustment may be applied to each production lot as follows:

i. Non-PWL Production Lot (less than 3,500 tons):
The adjustment values in Tables 4.06-6 and 4.06-7 will be calculated for each sub lot based on the Air Void (AV) and Asphalt Binder Content (PB) test results for that sub lot. The total adjustment for each day's production (lot) will be computed as follows:

Tons Adjusted for Superpave Design (T_{SD}) = [($AdjAV_t$ + $AdjPB_t$) / 100] × Tons

Where: $AdjAV_t$: Percent adjustment for air voids
$AdjPB_t$: Percent adjustment for asphalt binder
Tons: Weight of material (tons) in the lot adjusted by 4.06.04-1

Percent Adjustment for Air Voids = $AdjAV_t$ = [$AdjAV_1$ + $AdjAV_2$ + $AdjAV_i$ + … + $AdjAV_n$)] /n

Where: $AdjAV_t$ = Total percent air void adjustment value for the lot
$AdjAV_i$ = Adjustment value from Table 4.06-6 resulting from each sub lot or the average of the adjustment values resulting from multiple tests within a sub lot, as approved by the Engineer.
n = number of sub lots based on Table M.04.03-2

TABLE 4.06-6: Adjustment Values for Air Voids

Adjustment Value ($AdjAV_i$) (%)	S0.25, S0.375, S0.5, S1 Air Voids (AV)
+2.5	3.8 - 4.2
+3.125*(AV-3)	3.0 - 3.7
-3.125*(AV-5)	4.3 – 5.0
20*(AV-3)	2.3 – 2.9
-20*(AV-5)	5.1 – 5.7
-20.0	≤ 2.2 or ≥ 5.8

Percent Adjustment for Asphalt Binder = $AdjPB_t = [(AdjPB_1 + AdjPB_2 + AdjPB_i + \ldots + AdjPB_n)] /n$

Where: $AdjPB_t$= Total percent liquid binder adjustment value for the lot
$AdjPB_i$ = Adjustment value from Table 4.06-7 resulting from each sub lot
n = number of binder tests in a production lot

TABLE 4.06-7: Adjustment Values for Binder Content

Adjustment Value ($AdjAV_i$) (%)	S0.25, S0.375, S0.5, S1 Pb
0.0	JMF Pb ± 0.3
- 10.0	≤ JMF Pb - 0.4 or ≥ JMF Pb + 0.4

ii. PWL Production Lot (3500 tons or more):

For each lot, the adjustment values will be calculated using PWL methodology based on AV, VMA, and PB test results. The results will be considered as being normally distributed and all applicable equations in AASHTO R 9 and AASHTO R 42 Appendix X4 will apply.

Only one test result will be considered for each sub lot. The specification limits are listed in M.04.

For AV, PB, and voids in mineral aggregate (VMA), the individual material quantity characteristic adjustment (Adj) will be calculated as follows:

For PWL between 50 and 90%: $Adj(AV_t \text{ or } PB_t \text{ or } VMA_t) = (55 + 0.5\ PWL) - 100$

For PWL at and above 90%: $Adj(AV_t \text{ or } PB_t \text{ or } VMA_t) = (77.5 + 0.25\ PWL) - 100$

Where: $AdjAV_t$ = Total percent AV adjustment value for the lot
$AdjPB_t$= Total percent PB adjustment value for the lot
$AdjVMA_t$= Total percent VMA adjustment value for the lot

A lot with PWL less than 50% in any of the 3 individual material quality characteristics will be evaluated under 1.06.04.

The total adjustment for each production lot will be computed using the following formula:

Tons Adjusted for Superpave Design (T_{SD}) = $[(0.5AdjAV_t + 0.25AdjPB_t + 0.25\ AdjVMA_t) / 100] \times Tons$

Where Tons: Weight of material (tons) in the lot adjusted by 4.06.04-1

iii. Partial Lots:

Lots with less than 4 sub lots will be combined with the prior lot. If there is no prior lot with equivalent material or if the last test result of the prior lot is over 30 calendar days old, the adjustment will be calculated as indicated in 4.06.04-2(a)i.

Lots with 4 or more sub lots will be calculated as indicated in 4.06.04-2(a)ii.

Production Lot Adjustment: T_{SD} × Unit Price = Est. (Pi)

Where: Unit Price = Contract unit price per ton per type of mixture
Est. (Pi) = Pay Unit in dollars representing incentive or disincentive per lot

(b) Density Lot Adjustment: An adjustment may be applied to each density lot as follows:

i. Simple Average Density Lot (less than 3500 tons) and Bridge Lots:

The final lot quantity shall be the difference between the total payable tons for the Project and the sum of the previous lots. If either the Mat or Joint adjustment value is "remove and replace," the density lot shall be removed and replaced (curb to curb).

No positive adjustment will be applied to a density lot in which any core was not taken within the required 5 calendar days of placement.

Tons Adjusted for Density (T_D) = [{(PA_M × 0.50) + (PA_J × 0.50)} / 100] × Tons

Where: T_D = Total tons adjusted for density for each lot
PA_M = Mat density percent adjustment from Table 4.06-8
PA_J = Joint density percent adjustment from Table 4.06-9
Tons: Weight of material (tons) in the lot adjusted by 4.06.04-1

TABLE 4.06-8: Adjustment Values for Pavement Mat density

Average Core Result **Percent Mat Density**	**Percent Adjustment (Bridge and Non-Bridge)** [(1)(2)]
97.1 - 100	-1.667*(ACRPD-98.5)
94.5 – 97.0	+2.5
93.5 – 94.4	+2.5*(ACRPD-93.5)
92.0 – 93.4	0
90.0 – 91.9	-5*(92-ACRPD)
88.0 – 89.9	-10*(91-ACRPD)
87.0 – 87.9	-30
86.9 or less	Remove and Replace (curb to curb)

Notes:
(1) ACRPD = Average Core Result Percent Density
(2) All Percent Adjustments to be rounded to the second decimal place; for example round 1.667 to 1.67.

TABLE 4.06-9: Adjustment Values for Pavement Joint Density

Average Core Result **Percent Joint Density**	**Percent Adjustment (Bridge and Non-Bridge)** [(1)(2)]
97.1 – 100	-1.667*(ACRPD-98.5)
93.5 – 97.0	+2.5
92.0 – 93.4	+1.667*(ACRPD-92)
91.0 – 91.9	0
89.0 – 90.9	-7.5*(91-ACRPD)
88.0 – 88.9	-15*(90-ACRPD)
87.0 – 87.9	-30
86.9 or less	Remove and Replace (curb to curb)

Notes:
(1) ACRPD = Average Core Result Percent Density
(2) All Percent Adjustments to be rounded to the second decimal place; for example round 1.667 to 1.67

Additionally, any sublot with a density result below 87% will be evaluated under 1.06.04.

ii. PWL Density Lot (3,500 tons or more):
For each lot, the adjustment values will be calculated using PWL methodology based on mat and joint density test results. Only one result will be included for each sublot. The results will be considered as being normally distributed and all applicable equations in AASHTO R 9 and AASHTO R 42 Appendix X4 will apply.

The specification limits for the PWL determination are as follows:

Mat Density: 91.5-98%

Joint Density: 90-98%

For mat and joint density, the individual percent adjustment (PA) will be calculated as follows:

For PWL between 50 and 90%: PA ($_M$ or $_J$)= 0.25 * PWL – 22.50

For PWL at and above 90%: PA ($_M$ or $_J$)= 0.125 * PWL – 11.25

Where: PA_M = Total percent mat density adjustment value for the PWL mat density lot

PA_J= Total percent joint density adjustment value for the PWL joint density lot

No positive adjustment will be applied to a density lot in which any core was not taken within the required 5 calendar days of placement.

A lot with PWL less than 50% will be evaluated under 1.06.04.

The total adjustment for each PWL mat density lot will be computed as follows:

Tons Adjusted for Mat Density (T_{MD}) = (PA_M / 100) × Tons

Where: Tons= Weight of material (tons) in the lot adjusted by 4.06.04-1.

The total adjustment for each PWL joint density lot will be computed as follows:

Tons Adjusted for Joint Density (T_{JD}) = (PA_J / 100) × J_Tons

Tons Adjusted for Joint Density will be calculated at the end of each project or project phase.

Where: $\text{J_Tons} = \text{Tons in project or phase adjusted by } 4.06.4 - 1 \times \frac{\text{Lot joint length}}{\text{Joint length in project or phase}}$

All bridge density lot adjustments will be evaluated in accordance with 4.06.04-2(b)i.

Additionally, any sublot with a density result below 87% will be evaluated under 1.06.04.

iii. Partial Lots:

Lots with less than 4 sub lots will be combined with the prior lot. If there is no prior lot with equivalent material and placement conditions or if the last test result of the prior lot is over 30 calendar days old, the mat and joint individual adjustments will be calculated in accordance to Tables 4.06-8 and 4.06-9. T_{MD} and T_{JD} will be calculated as indicated in 4.06.04-2(b)i.

Lots with 4 or more sub lots will be calculated as indicated in 4.06.04-2(b)ii.

Density Lot Adjustment (Simple Average Lots): T_D × Unit Price = Est. (Di)

Density Lot Adjustment (PWL Lots): (T_{MD} or T_{JD}) × Unit Price = Est. (DMi or DJi)

Where: Unit Price = Contract unit price per ton per type of mixture

Est. (Di)= Pay Unit in dollars representing incentive or disincentive per simple average density lot

Est. (DMi)= Pay Unit in dollars representing incentive or disincentive per PWL mat lot

Est. (DJi)= Pay Unit in dollars representing incentive or disincentive per PWL joint lot

Additionally, any sublot with a density result below 87% will be evaluated under 1.06.04.

3. Transitions for Roadway Surface: The installation of permanent transitions will be measured under the appropriate item used in the formation of the transition.

The quantity of material used for the installation of temporary transitions will be measured for payment under the appropriate item used in the formation of the transition. The installation and removal of a bond breaker and the removal and disposal of any temporary transition formed by milling or with bituminous concrete pavement is not measured for payment.

4. Cut Bituminous Concrete Pavement: The quantity of bituminous concrete pavement cut will be measured in accordance with 2.02.04.

5. Material for Tack Coat: The quantity of tack coat will be measured for payment by the number of gallons furnished and applied on the Project and approved by the Engineer. No tack coat material shall be included that is placed in excess of the tolerance described in 4.06.03.

a. Container Method – Material furnished in a container will be measured to the nearest 1/2 gallon. The volume will be determined by either measuring the volume in the original container by a method approved by the Engineer or using a separate graduated container capable of measuring the volume to the nearest 1/2 gallon. The container in which the material is furnished must include the description of material, including lot number or batch number and manufacturer or product source.

b. Vehicle Method

   i. Measured by Weight: The number of gallons furnished will be determined by weighing the material on calibrated scales furnished by the Contractor. To convert weight to gallons, one of the following formulas will be used:
   Tack Coat (gallons at 60°F) = Measured Weight (pounds) / Weight per gallon at 60°F
   Tack Coat (gallons at 60°F) = 0.996 × Measured Weight (pounds) / Weight per gallon at 77°F

   ii. Measured by automated metering system on the delivery vehicle:
   Tack Coat (gallons at 60°F) = 0.976 × Measured Volume (gallons).

6. Material Transfer Vehicle (MTV): The furnishing and use of a MTV will be measured separately for payment based on the actual number of surface course tons delivered to a paver using the MTV.

4.06.05—Basis of Payment:

1. HMA S* or PMA S*: The furnishing and placing of bituminous concrete will be paid for at the Contract unit price per ton for " HMA S*" or " PMA S*."

All costs associated with providing illumination of the work area are included in the general cost of the work.

All costs associated with cleaning the surface to be paved, including mechanical sweeping, are included in the general cost of the work. All costs associated with constructing longitudinal joints are included in the general cost of the work.

All costs associated with obtaining cores for acceptance testing and dispute resolution are included in the general cost of the work.

2. Bituminous Concrete Adjustment Costs: This adjustment will be calculated using the formulas shown below if all of the measured adjustments in 4.06.04-2 are not equal to zero. A positive or negative adjustment will be applied to monies due the Contractor.

Production Lot: Σ Est (Pi) = Est. (P)
Density Lot (Simple Average Lots): Σ Est (Di) = Est. (D)
Density Lot (PWL): Σ Est (DMi) + Σ (DJi) = Est. (D)
Bituminous Concrete Adjustment Cost= Est. (P) + Est. (D)

Where: Est. ( )= Pay Unit in dollars representing incentive or disincentive in each production or density lot calculated in 4.06.04-2

The Bituminous Concrete Adjustment Cost item, if included in the bid proposal or estimate, is not to be altered in any manner by the Bidder. If the Bidder should alter the amount shown, the altered figure will be disregarded and the original estimated cost will be used for the Contract.

3. Transitions for Roadway Surface: The installation of permanent transitions will be paid under the appropriate item used in the formation of the transition. The quantity of material used for the installation of temporary transitions will be paid under the appropriate pay item used in the formation of the transition. The installation and removal of a bond breaker, and the removal and disposal of any temporary transition formed by milling or with bituminous concrete pavement is included in the general cost of the work.

4. The cutting of bituminous concrete pavement will be paid in accordance with 2.02.05.

5. Material for tack coat will be paid for at the Contract unit price per gallon at 60°F for "Material for Tack Coat."

6. The Material Transfer Vehicle (MTV) will be paid at the Contract unit price per ton for "Material Transfer Vehicle."

Pay Item	Pay Unit
HMA S*	ton
PMA S*	ton
Bituminous Concrete Adjustment Cost	est.
Material for Tack Coat	gal.
Material Transfer Vehicle	ton

SECTION 4.09
MILLING,
REMOVAL OF EXISTING WEARING SURFACE

4.09.01—Description: This work shall consist of the milling, removal, and disposal of existing bituminous concrete pavement. It shall also include the complete removal and disposal of the existing bituminous concrete wearing surface, membrane waterproofing and bond breaker covering the reinforced concrete bridge deck(s) as shown on the plans or as ordered by the Engineer. The types of milling shall include the following:

1. Standard Milling shall be used for the removal of bituminous concrete in excess of 4 inch depth.
2. Fine Milling shall be used to remove bituminous concrete from 0 to 4 inches. It may also be used to remove bituminous concrete greater than 4 inches in limited areas or where required.
3. Micro-Milling shall be used to remove bituminous concrete from 0 to 3 inches. It may also be used to remove bituminous concrete greater than 3 inches in limited areas or where required.
4. Removal of Existing Wearing Surface shall be used where shown on the plans.

4.09.03—Construction Methods:

A. Milling:

1. General: The Contractor shall remove the bituminous concrete material using the milling type specified on the Plans. The pavement surface shall be removed to the line, grade, and existing or typical cross-section shown on the plans or as directed by the Engineer.

The bituminous concrete material shall be disposed of offsite by the Contractor at an approved disposal facility unless otherwise stated in the Contract.

Any milled surface, or portion thereof, that is exposed to traffic shall be paved within 14 calendar days unless otherwise stated in the Contract.

2. Equipment: The equipment for milling the pavement surface shall be designed and built for milling bituminous concrete pavements. It shall be self-propelled with sufficient power, traction, and stability to maintain depth and slope and shall be capable of removing the existing bituminous concrete pavement.

The milling machine shall be equipped with a built-in automatic grade averaging control system that can control the longitudinal profile and the transverse cross-slope to produce the specified results. The longitudinal controls shall be capable of operating from any longitudinal grade reference, including string line, contact ski (30 feet minimum), non-contact ski (20 feet minimum), or mobile string line (30 feet minimum). The transverse controls shall have an automatic system for controlling cross-slope at a given rate. The Engineer may waive the requirement for automatic grade or slope controls where the situation warrants such action.

The machine shall be equipped with an integral pickup and conveying device to immediately remove material being milled from the surface of the roadway and discharge the millings into a truck, all in one operation. The machine shall also be equipped with a means of effectively limiting the amount of dust escaping from the milling and removal operation.

When milling smaller areas or areas where it is impractical to use the above described equipment, the use of a lesser equipped milling machine may be permitted when approved by the Engineer.

The rotary drum of the milling machine shall have carbide or diamond-tipped teeth with the following maximum spacing and minimum milling depth:

Milling Type	Maximum Tooth Spacing*	Minimum Depth Capability (single pass)
Standard Milling	15 mm	4 – 8 inches
		Greater than 8 inches
Fine Milling	8 mm	0 – 4 inches
Micro-Milling	5 mm	0 – 3 inches

* Industry standard

The forward speed of any milling machine shall be limited to no more than 45 feet/minute.

The teeth on the revolving cutting drum must be continually maintained and shall be replaced as warranted to provide a uniform pavement texture, as outlined in 4.09.03-C, Surface Tolerance.

3. Protection: Protection shall be provided around existing catch basin inlets, manholes, utility valve boxes, and any similar structures. Any damage to such structures as a result of the milling operation is the Contractor's responsibility and shall be repaired at the Contractor's expense.

To prevent the infiltration of milled material into the storm drainage system, the Contractor shall take special care to prevent the milled material from falling into the inlet openings or inlet grates. Any milled material that has fallen into inlet openings or inlet grates shall be removed at the Contractor's expense.

B. Removal of Existing Wearing Surface: The bituminous concrete wearing surface, membrane waterproofing and bond breaker shall be removed from the structure(s) using means acceptable to the Engineer to completely expose the concrete bridge deck(s).

Prior to removal of bituminous concrete wearing surface, the Contractor shall field verify the depth of the existing bituminous concrete by obtaining depth measurements (maximum 4 inch diameter holes) at intervals no greater than 25 feet apart in each lane. Depth verification holes shall be filled with bituminous material if the removal of wearing surface operation will not be completed within 5 days.

The existing bituminous concrete wearing surface and membrane waterproofing shall be removed in their entireties to the limits shown on the plans. The removal operations shall not begin until the Contractor is prepared to perform the permanent patching or repair to the underlying concrete within 5 working days. If this is in conflict with "Prosecution and Progress," "Maintenance and Protection of Traffic," or other Contract requirements, the more stringent specification shall apply.

Methods for removal of existing wearing surfaces may include micro-milling or fine-milling and shall include as many passes or amount of effort required to completely expose the concrete deck(s). Any membrane not completely removed by the milling process shall be removed by scarifying or other means as approved by the Engineer.

Alternate methods for the removal of a bituminous concrete surface may be submitted to the Engineer for review. Demonstration of the alternate removal methods may be required prior to consideration.

The existing bituminous concrete wearing surface, membrane waterproofing, bond breaker, and any other products being removed shall be disposed of offsite by the Contractor unless otherwise noted in the Contract or as directed by the Engineer.

If membrane waterproofing, as specified elsewhere in the Contract, is to be re-installed on the existing deck(s), the surface profile following removal shall be suitable for such reinstallation. The profile of the cleaned concrete surface shall meet the membrane waterproofing manufacturer's recommendations, and have no gouges greater than 1/2 inch in depth. Any deficiencies that could, in the Engineer's opinion, cause failure of, or puncture the new membrane shall be removed as part of this work.

C. Surface Tolerance:

1. General: The surface shall be free from gouges, longitudinal grooves and ridges, oil film, and other imperfections, that are a result of defective equipment, improper use of equipment, poor workmanship, or inadequate field verification. Any unsatisfactory surfaces caused by the removal operations are the Contractor's responsibility and shall be corrected at the Contractor's expense and to the satisfaction of the Engineer prior to opening the surface to traffic.

Any raised structures shall be delineated with traffic control devices, as directed by the Engineer.

2. Tolerances: All milling types shall provide a satisfactory riding surface with a uniform textured appearance. The Contractor shall perform random spot-checks at a minimum of 5 locations per working shift with a Contractor-supplied 10 foot straight edge to verify the surface tolerances listed below. Random spot-checks (minimum of 5 checks per shift) shall occur at a maximum of 250 feet per pass of the milling machine and shall be performed with the Engineer present. The following tolerances shall apply:

(a) Standard Milling: The variation of the top of two ridges from the testing edge of the straightedge, between any two ridge contact points, shall not exceed 3/8 inch. The variation of the top of any ridge to the bottom of the groove adjacent to that ridge shall not exceed 3/8 inch.

(b) Fine-Milling: The variation of the top of two ridges from the testing edge of the straightedge, between any two ridge contact points, shall not exceed 1/4 inch. The variation of the top of any ridge to the bottom of the groove adjacent to that ridge shall not exceed 1/4 inch.

(c) Micro-Milling: The variation of the top of two ridges from the testing edge of the straightedge, between any two ridge contact points, shall not exceed 1/16 inch. The variation of the top of any ridge to the bottom of the groove adjacent to that ridge shall not exceed 1/16 inch.

Where a surface delamination between bituminous concrete layers or a surface delamination of bituminous concrete on Portland cement concrete causes a non-uniform texture to occur, the depth of milling shall be adjusted in small increments to a maximum of +/- 1/2 inch to eliminate the condition.

When removing bituminous concrete pavement entirely from an underlying Portland cement concrete pavement, all bituminous concrete pavement shall be removed leaving a uniform surface of Portland cement concrete, unless otherwise directed by the Engineer.

Any unsatisfactory surfaces produced by the milling operation are the Contractor's responsibility and shall be corrected at the Contractor's expense and to the satisfaction of the Engineer.

D. Transitions:

1. Construction Joints: No transverse vertical face shall be left exposed to traffic. No longitudinal vertical face greater than 1 inch shall be left exposed to traffic. Any other vertical face created by milling shall have a bituminous concrete taper constructed to the temporary transition requirements as described below.

2. Roadway Structures: Roadway structures shall not have a vertical face of greater than 1 inch exposed to traffic as a result of milling. All roadway structure edges and bituminous concrete tapers shall be clearly marked with fluorescent paint. The paint shall be maintained throughout the exposure to traffic.

All structures within the roadway that are exposed to traffic and greater than 1 inch above the milled surface shall receive a transition meeting the following requirements:

(a) For roadways with a posted speed limit of 35 mph or less:

(i) Round structures with an exposed vertical face between 1 inch and 2.5 inches shall be transitioned with a hard rubber tapered protection ring designed for that purpose of the appropriate inside diameter designed specifically to protect roadway structures. Bituminous concrete tapers at a minimum 12 to 1 (12:1) taper in all directions may be substituted for the protection rings if approved by the Engineer.

(ii) Round structures with an exposed vertical face greater than 2.5 inches shall receive a transition of bituminous concrete formed at a minimum 12 to 1 (12:1) taper in all directions.

(iii) All rectangular structures shall receive a transition of bituminous concrete formed at a minimum 12 to 1 (12:1) taper in all directions.

(b) For roadways with a posted speed limit of 40, 45 or 50 mph: All structures shall receive a transition of bituminous concrete formed at a minimum 24 to 1 (24:1) taper in all directions of travel. Direction of travel shall include both the leading and trailing sides of a structure. The minimum taper shall be 12 to 1 (12:1) in all other directions.

(c) For roadways with a posted speed limit of greater than 50 mph: All structures shall receive a transition of bituminous concrete formed at a minimum 36 to 1 (36:1) taper in the direction of travel. Direction of travel shall include both the leading and trailing sides of a structure. The minimum taper shall be 12 to 1 (12:1) in all other directions.

3. Temporary Transitions: If any vertical face is formed in an area exposed to traffic, a temporary paved transition shall be established according to the requirements shown on the plans or in accordance with 4.06.03-5. If a milling machine is used to form a temporary transition, the length of the temporary transition shall meet the requirements of 4.06.03-5 "Transitions for Roadway Surface," the requirements shown on the plans, or as directed by the Engineer. A clean vertical face shall be established by saw cutting at all final termini limits of the Project.

4. Milling for Permanent Pavement Transitions: When called for on the plans, milling a tapered "keyway" to transition the top course of a bituminous concrete overlay to an existing pavement shall be performed as specified elsewhere in the Contract.

E. Sweeping: Prior to opening an area which has been milled to traffic, the pavement shall be thoroughly swept with a sweeper truck. The sweeper truck shall be equipped with a water tank and be capable of removing the millings and loose debris from the surface. The sweeper truck shall operate at a speed that allows for the maximum pickup of millings from the roadway surface. Other sweeping equipment may be provided in lieu of the sweeper where acceptable by the Engineer.

Any milled area that will not be exposed to live traffic for a minimum of 48 hours prior to paving shall require a vacuum sweeper truck in addition to, or in lieu of, mechanical sweeping. The vacuum sweeper truck shall have sufficient power and capacity to completely remove all millings from the roadway surface including any fine particles within the texture of the milled surface. Vacuum sweeper truck hose attachments shall be used to clean around pavement structures or areas that cannot be reached effectively by the main vacuum. Compressed air may be used in lieu of vacuum attachments if approved by the Engineer.

4.09.04—Method of Measurement:

Milling of bituminous concrete will be measured for payment by the number of square yards of area from which the particular type of milling has been completed and the work accepted. Deductions will not be made for minor unmilled areas such as catch basin inlets, manholes, utility boxes and any similar structures.

The removal of wearing surface will be measured for payment by the number of square yards of bituminous concrete wearing surface removed to expose the underlying concrete deck(s). No area deductions will be made for scuppers, joints, and any similar areas.

There will be no measurement for marking roadway structures, transitions for roadway structures and sweeping of any surface that has been milled.

4.09.05—Basis of Payment: Milling work will be paid for at the Contract unit price per square yard for "Fine Milling of Bituminous Concrete (0" to 4")," "Standard Milling of Bituminous Concrete (Greater Than 4" up to 8")," "Standard Milling of Bituminous Concrete (Greater Than 8")," and "Micro Milling of Bituminous Concrete (0" to 3")." This price shall include all equipment, tools, labor, and materials incidental thereto.

Work for the removal of wearing surface will be paid for at the Contract unit price per square yard for "Removal of Existing Wearing Surface," complete and accepted, which price shall include the field verification, removal of wearing surface, removal of membrane waterproofing and bond breaker, saw cutting, and all equipment, tools and labor. No additional payments will be made for multiple passes with the milling machine(s) to remove the wearing surface.

No separate payments will be made for cleaning the pavement prior to paving; providing protection and doing handwork to remove bituminous concrete around catch basin inlets, bridge scuppers, manholes, utility valve boxes, median barriers, parapets, joints and any similar structures; repairing surface defects as a result of Contractor negligence; providing protection to underground utilities from the vibration of the milling operation; removal of any temporary milled transition; removal and disposal of millings; sweeping and all associated work.

Milling for Pavement Transitions, where identified on the plans, will be paid under a separate item specified elsewhere.

Installation of traffic control devices shall be included under the costs for "Maintenance and Protection of Traffic," payment for the devices will be under the applicable items.

Pay Item	Pay Unit
Fine Milling of Bituminous Concrete (0" to 4")	s.y.
Micro-Milling of Bituminous Concrete (0" to 3")	s.y.
Standard Milling of Bituminous Concrete (Greater Than 4" up to 8")	s.y.
Standard Milling of Bituminous Concrete (Greater Than 8")	s.y.
Removal of Existing Wearing Surface	s.y.

SECTION 4.15
PRESSURE RELIEF JOINT

4.15.01—Description: Work under this item shall consist of constructing, in the new or existing reinforced concrete pavement, a pressure relief joint composed of a bituminous concrete pavement course on a bituminous concrete base course. The pressure relief joint shall be constructed on the prepared subbase in accordance with these specifications and in accordance with the lines, grades, compacted thickness, pavement class, and cross-sections shown on the plans.

4.15.02—Materials: Materials shall meet the requirements of M.04.

4.15.03—Construction Methods: Construction methods of pressure relief joints shall meet the applicable requirements of 4.06, 2.02, and 2.12. Prior to the installation of the pressure relief joint, the subbase shall be adjusted to the correct grade, with additional material, if required, and compacted.

4.15.04—Method of Measurement: This work will be measured by the actual number of square yards of completed and accepted pressure relief joint.

4.15.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Pressure Relief Joint," completed and accepted in place, which price shall include all materials, equipment, tools and labor and work incidental thereto.

There will be no payment for cutting concrete pavement and bituminous concrete pavement, removal of concrete pavement and bituminous concrete pavement, additional subbase, compaction of subbase and compaction testing, but the cost thereof shall be included in the cost of the pressure relief joint.

Pay Item	Pay Unit
Pressure Relief Joint	s.y.

SECTION 5.03
REMOVAL OF SUPERSTRUCTURE

5.03.01—Description
5.03.02—Vacant
5.03.03—Construction Methods
5.03.04—Method of Measurement
5.03.05—Basis of Payment

5.03.01—Description: This work shall include the full or partial removal and disposal of superstructure(s), as indicated on the plans.

5.03.02—Vacant

5.03.03—Construction Methods:

1. Submittals:

The Contractor shall prepare and submit written procedures and working drawings, in accordance with 1.05.02. The submittals shall address the following:

- proposed equipment and removal method(s)
- operating and storage location(s) of equipment and materials
- containment and disposal of debris, including lead paint where required
- installation and removal of
    a. debris shields
    b. working platforms
    c. falsework
- temporary support(s) for maintenance of traffic
- modification to and restoration of the structure to remain in place

2. Removal: Superstructures which are to be fully or partially removed by the Contractor shall be removed to the limits shown on the plans or as directed by the Engineer. Where components to be removed are designated on the plans for salvage, the Contractor shall carefully remove, preserve, deliver to and unload the components at the location specified in the Contract.

General removal may be performed by excavator-mounted demolition equipment or other methods except where prohibited on the plans or as directed by the Engineer.

If partial removal of concrete is required, it shall be sawcut to the neat lines as indicated on the plans. Near reinforcing steel that is to remain, the Contractor must use limited methods for removal, such as fifteen (15) pound hammers or other methods accepted by the Engineer.

Reinforcing steel shall be cut and removed where shown on the plans. Reinforcing steel to remain shall be cleaned of all concrete and corrosion products by oil-free abrasive blasting, high-pressure water blasting or other methods accepted by the Engineer. The reinforcing steel and concrete surfaces shall be free from dirt, oil, cement fines (slurry), or any material that may interfere with the bond of the proposed concrete. Tightly-bonded light rust on the reinforcing surface is acceptable.

Where stage construction requires concrete to be removed adjacent to the existing superstructure that will continue to support live load, the Contractor shall cut the concrete in accordance with the accepted working drawings at the demolition limit shown on the plans to minimize disturbance to the section that is to remain in place.

When the existing structure is to carry traffic during the staged construction of the new work, the Contractor shall alter the structure as required by the plans. The structure and approaches shall be kept in a safe and satisfactory condition for the use of traffic at all times until the new structure is completed and open to traffic. The Contractor shall take all precautions and do such work as may be necessary to prevent damage to the structure or approaches due to the construction operations. When no longer required for traffic, the temporary alteration to the structure shall be removed in accordance with the requirements of the plans or as directed by the Engineer.

3. Disposal of Debris: The Contractor shall properly dispose of all construction debris either off-Site, or on-Site in accordance with 2.02.03-5.

4. Damage Mitigation: When removing the superstructure or a portion thereof, the Contractor shall take necessary precautions to prevent debris from dropping to areas below the superstructure, onto adjacent traffic lanes or onto adjacent property. Any damage to adjoining areas, including but not limited to new construction, public utility installations, abutting property and to the portions of the structure that will remain shall be repaired by the Contractor in accordance with 1.05.11.

5.03.04—Method of Measurement: This work, being paid on a lump sum basis, will not be measured.

5.03.05—Basis of Payment: Prior to beginning work, the Contractor shall submit a proposed schedule of values for review and concurrence by the Engineer.

This work will be paid for at the Contract lump sum price for “Removal of Superstructure,” at the location designated, which price shall include all equipment, tools and labor incidental to the full or partial removal of the superstructure (including saw cutting and the erection and removal of temporary falsework or supports of any kind) and shall include the proper disposal thereof.

Payment for the full or partial removal of bridge substructure(s) will be made at the Contract unit price per cubic yard for "Removal of Existing Masonry," in accordance with 9.74.05.

Pay Item	Pay Unit
Removal of Superstructure	l.s.

SECTION 5.04
RAILROAD PROTECTION

5.04.01—Description: This item shall consist of securing protective services of workers such as flagmen, electric traction linemen, inspectors, track foremen, signalmen, or other such protective services deemed necessary by a railroad engaged in or affected by the Project operations of the Contractor on, over, under or adjacent to the railroad's right-of-way. This item shall also include any material or equipment incidental to or required for the provision of such required protective services. The Contractor shall secure such services as are required by the railroad, and if said services are obtained from the railroad, the Contractor shall reimburse the railroad for them, in accordance with relevant Contract terms or with the railroad's customary terms for such transactions. The Contractor must understand that the railroad may require advance payment of all or a portion of the estimated costs for the services, in which case the Contractor shall make such advance payment.

5.04.02—Vacant

5.04.03—Vacant

5.04.04—Method of Measurement: Only Project-related protective services billed by the railroad and approved by the Engineer will be measured for payment. Protective services which the Engineer did not approve or deem necessary for the proper completion of the Project will not be measured for payment.

5.04.05—Basis of Payment: The sum of money for this item shown in the bid Estimate and in the itemized bid proposal as "Estimated Cost" for this work will be considered and treated as the bid price for it, even though payment for it will be made as described below. The estimated cost figure is not to be altered in any manner by the bidder. Should the bidder alter the amount shown, the altered figures will be disregarded and the original bid price will be used as the total amount for the Contract item. The Department will pay the Contractor for "Railroad Protection" at the actual hourly rate charged to the Contractor for railroad protection services approved by the Engineer (shown in the monthly statement or receipted bills to the Contractor from the entity that provided the actual services), plus a 5% markup. This price shall include all labor, material and equipment provided by a railroad for protective services required for Project operations.

Protective services used solely for the convenience or benefit of the Contractor shall be the legal and financial responsibility of the Contractor and will not be included in this item.

Final acceptance of the Project and resolution of financial Project obligations by the Department will be contingent upon the Contractor's providing the Department with proof that each railroad involved in the Project has been reimbursed for all necessary protective services provided by the railroad or that the Contractor has made some other arrangements satisfactory to said railroad(s) for such reimbursement.

Pay Item	Pay Unit
Railroad Protection	est.

SECTION 5.06
RETAINING WALLS, ENDWALLS AND STEPS

5.06.01—Description: Work under this heading shall include all concrete and masonry retaining walls, endwalls, steps and other miscellaneous structures.

All such structures shall be built in the locations indicated or directed, to the lines, grades, dimensions and details shown on the plans and in accordance with the provisions of these specifications for the various Contract items which constitute the completed structure.

5.06.02—Materials: The materials furnished and used in the work shall be those prescribed herein for the several Contract items which constitute the completed structure.

Unless otherwise noted, concrete shall be Class PCC03340 Concrete, except the concrete for steps and copings shall be Class PCC03360 Concrete, meeting the applicable requirements of M.03.

5.06.03—Construction Methods: All construction methods for endwalls, retaining walls, steps and other miscellaneous structures shall be in accordance with the detailed requirements prescribed for the construction of the several Contract items entering into the completed structure. All requirements relative to concrete structures that are pertinent shall apply equally to work covered by this Section.

1. **Foundations** shall be excavated to the depth shown on the plans, unless the character of the material encountered is such as to require changes, in which case the depth shall be as ordered by the Engineer. Should suitable material not be encountered at depths to which it is practicable to excavate, piles shall be driven or other special treatment employed as the Engineer may direct. Piles shall be driven in accordance with the requirements of the plans and shall be either vertical or battered, as the Engineer may require. Special treatment of foundations shall meet the requirements of the plans and special provisions; or if not specified therein, shall be as directed by the Engineer and will be classed as extra work or additional work. No construction material shall be placed in any foundation until the Engineer has examined the material encountered and approved its character and the depth of the excavation.
2. **Endwalls** shall be built in the location and to the dimensions and details shown on the plans; or as ordered, and they shall be neatly and accurately finished, true to the lines and grades given. Pipes shall be of sufficient length to extend to the exposed face of the endwall, and the end shall be finished to provide neat, watertight joints. The ends of pipe culverts which enter endwalls on a skew shall be cut to the angle of the skew; or the short side of the pipe may be built out with adequate forms, if this method is approved by the Engineer.
3. **Retaining Walls** shall be built in the location and to the dimensions and details shown on the plans or as ordered. Drainage of retained material shall be provided as specified in 2.16 and as shown on the plans.
4. **Steps** shall be built where indicated or directed to the dimensions and details shown on the plans or as ordered and in accordance with the requirements of these specifications.

5.06.04—Method of Measurement: The quantities of the various Contract items entering into the completed work shall be determined in accordance with the provisions of the plans and specifications for the several items involved. Only accepted work shall be included, and the dimensions used shall be those shown on the plans, except as modified by the written orders of the Engineer.

5.06.05—Basis of Payment: Payment will be made for the actual quantities of accepted work or materials, measured as provided above, at the Contract prices for the several items involved, which prices shall be full compensation for all materials, tools, equipment and labor necessary to the completion of the work.

Concrete for steps and for copings on masonry endwalls or retaining walls will be paid for at the Contract unit prices per cubic yard for "Concrete for Steps and Copings" complete in place including all incidental expense. Reinforcement, if used, will be paid for in accordance with the provisions of these specifications for the kind of material involved.

Excavation necessary to the construction of steps will not be measured or paid for directly, but the cost thereof shall be included in the cost of the concrete item.

Excavation necessary to the construction of endwalls or retaining walls will be paid for in accordance with 2.02 with respect to the classification applying to the balance of the Project, unless otherwise specified.

There will be no direct payment for the cost of forming keys or construction joints, but the cost thereof shall be included in the cost of the concrete items. Where steel dowels are used, this material will be paid for under the reinforcement item.

There will be no direct payment for forming weep holes through the walls or for the pipe necessary for the

purpose, but the cost thereof shall be considered as included in the general cost of the work.

Expansion joints will not be paid for as such, but the various materials entering into their construction shall be paid for at the Contract prices for the materials in question, complete in place, including all incidental expense.

Pay Item	Pay Unit
Concrete for Steps and Copings	c.y.

SECTION 5.08
SHEAR CONNECTORS

5.08.01—Description: This item shall consist of furnishing, placing and welding stud shear connectors to the top of structural steel supporting members to induce composite action with the concrete slab. The shear connectors shall be to the details, size and quantity shown on the plans for the designated locations.

5.08.02—Materials: Stud shear connectors shall meet the requirements of M.06.02-4 Welded Stud Shear Connectors.

5.08.03—Construction Methods: The surface on which the shear connectors are to be welded shall be thoroughly cleaned of mill scale, rust, dirt, paint, grease or other materials which would reduce the quality of the weld. The stud shear connectors shall be free from rust scale, rust pits and oil at the time of welding.

Before welding any stud shear connectors, the Contractor shall submit for the Engineer's approval a notarized laboratory qualification from the manufacturer certifying that each shipment of material meets the minimum requirements. Each separate container shall be clearly tagged or marked by the manufacturer for identification with the certified laboratory qualification.

Stud shear connectors shall be end welded with automatically timed stud welding equipment connected to a recommended power source. If 2 or more stud welding guns are to be operated from the same power source, they shall be interlocked so that only 1 gun can operate at a time and so the power source has fully recovered from making each weld before another weld is started. Stud shear connectors may be stacked to meet heights greater than the 8-inch maximum for individual studs.

Welding, either in the shop or field, will not be permitted when the air temperature is below 35° F, except with the special permission of the Engineer, and in no case will welding be permitted when air temperature is below 25° F, or when surfaces to be welded are wet from condensation, rain, snow or ice, when rain or snow is falling on the surfaces to be welded, or during periods of high wind, unless the welding operation and the work are properly protected. The minimum requirement for preheat tabulated elsewhere herein and in the current "ANSI/AASHTO/AWS D1.5 Bridge Welding Code" shall apply, except where the requirements noted above shall take precedence. While in operation, the welding gun shall be held in position without movement until the weld metal has solidified. After cooling, the ferrules shall be removed from each stud.

After 2 successive studs on each beam or girder are welded and have withstood the test of being bent to 45 degrees by striking with a hammer (without weld or stud failure) further stud welding shall proceed. The Engineer shall be promptly informed of any changes in the welding procedure at any time during construction. If the reduction in the height of studs as they are welded becomes less than normal, welding shall be stopped immediately and not resumed until the cause has been determined and corrected.

When so directed by the Engineer, the Contractor shall test additional studs by bending. Studs exhibiting no signs of failure after bending shall be left in the bent position, if allowed by the Engineer.

Before welding a new stud where a defective stud has been removed, the area shall be ground smooth and flush; or in the case of a pullout of metal, the pocket shall be filled with weld metal using the shielded metal-arc process with low-hydrogen welding electrodes and then ground flush. Where a stud is defective or does not have a full 360 degree weld, a new stud may be welded adjacent to the defective stud. However, this may be done only in areas of compression or where permitted by the Engineer.

The Engineer may at any time require the Contractor to supply sample studs for a requalification test without additional compensation.

5.08.04—Method of Measurement: Installed and accepted shear connectors will be measured as units.

For stacked studs, the Department will measure for payment any stack higher than 8 inches as 2 studs.

5.08.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Shear Connectors," which price shall include all materials, tools, equipment and labor incidental thereto for all work under this item on the Project.

Pay Item	Pay Unit
Shear Connectors	ea.

SECTION 5.09
WELDED STUDS

5.09.01—Description: Under this item the Contractor shall furnish and install studs, exclusive of studs used as shear connectors, of the type, detail, size and quantity shown on the plans.

5.09.02—Materials: Steel for the studs shall meet the requirements of M.06.02-4 Welded Stud Shear Connectors.

5.09.03—Construction Methods: The installation of welded studs shall be in accordance with the requirements of 5.08.03.

5.09.04—Method of Measurement: Studs will be measured as units, in place, and acceptable to the Engineer.

5.09.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Welded Studs," complete in place, which price shall include all materials, equipment, tools and labor incidental thereto. Welded studs used as shear connectors shall be paid for in accordance with the provisions of 5.08.

Pay Item	Pay Unit
Welded Studs	ea.

SECTION 5.13
POLYVINYL CHLORIDE PLASTIC PIPE

5.13.01—Description: This item shall consist of furnishing and installing polyvinyl chloride pipe for use as drains through the concrete bridge slab where shown on the plans or as ordered by the Engineer.

5.13.02—Materials: The pipe shall meet the requirements of M.08.01-20 PVC Pipe or M.08.01-21 PVC Gravity Pipe and shall be Schedule 40.

5.13.03—Construction Methods: The pipe drain shall be placed through the concrete slab at locations shown and as indicated on the plans. Pipe shall be secured at the proper locations before placing concrete. Where required, couplings and elbows may be used. After placing of the concrete slab, the pipe end shall be trimmed flush with the top surface of the concrete slab.

5.13.04—Method of Measurement: This work will be measured for payment by the number of linear feet of pipe completed and accepted in place and measured through all fittings.

5.13.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Polyvinyl Chloride Plastic Pipe," of the type specified, complete in place, which price shall include all materials, fittings, equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
(Size) Polyvinyl Chloride Plastic Pipe	l.f.

SECTION 5.14
PRESTRESSED CONCRETE MEMBERS

5.14.01—Description
5.14.02—Materials
5.14.03—Construction Methods
5.14.04—Method of Measurement
5.14.05—Basis of Payment

5.14.01—Description: Work under this item shall consist of furnishing, erecting, and joining prestressed members, such as beams and deck units of the type and size shown on the plans, including concrete, strands, reinforcing, pipe sleeves, threaded inserts, erection and tie plates, transverse tie strands, tie wires, lifting inserts or devices, mortar, void forms, grout, anchoring hardware, and all other necessary materials and equipment to complete the work.

5.14.02—Materials: The materials for prestressed beams and deck units shall meet the requirements of M.14.01.

Prestressing and reinforcing steel shall be free of corrosion that imparts etching, pitting, or scaling of the surface. Light surface rusting shall be removed as ordered by the Engineer.

Prestressing and reinforcing steel shall be stored under shelter and kept free of deleterious material, such as grease, oil, wax, dirt, paint, loose rust, and other similar contaminants that reduce the bond between steel and concrete. It shall not be stored on surfaces which contribute to galvanic or battery action, nor shall it be used as a ground for electrical welding.

When called for on the plans, the deformed reinforcing steel bars shall be epoxy coated in accordance with the applicable sections of ASTM D3963.

A representative 8-ounce sample of the coating material shall be retained by the manufacturer from each batch. The sample shall be packaged in an airtight container and identified by batch number. When required, infrared spectroscopy, gas chromatography, or any other method which is deemed necessary shall be completed to ensure that the sample is of the same composition as the initial samples.

Epoxy material for touch up and repair work shall be subject to approval by the Engineer. It shall be furnished by the epoxy manufacturer, shall be compatible with the coating material, inert in concrete and shall be suitable for use by the fabricator installing the coated bars.

All wires, clips, chair and bar supports, and other metallic materials used for the installation of the epoxy coated reinforcing bars shall be coated with the powdered epoxy resin; or coated with an acceptable plastic material; or made of rustproof or coated material that has been approved in writing by the Engineer.

All systems for handling coated bars shall have padded contact areas for the bars wherever possible. All bundling bands shall be padded, and all bundles shall be lifted with a strong back, multiple supports, or a platform bridge to prevent bar-to-bar abrasion from sags in the bar bundle. Special attention shall be given to loading and unloading procedures and to equipment so that damage to the coating will not occur.

The epoxy coated steel bars shall be carefully unloaded and stored to avoid damage or contamination. The bars shall be installed in accordance with the plans, and care shall be taken to protect and preserve the epoxy coating.

During and after the installation of the bars, the fabricator shall repair all significant cuts, nicks, and abraded places in the coating on the bars with the epoxy repair material supplied by the manufacturer of the powdered epoxy resin. Any damaged metallic accessories shall also be repaired with a suitable material. All touchup material shall be fully cured prior to the pouring of concrete.

Damage caused during shipment of epoxy bars, or by installation, or by both, need not be repaired when the damaged area is 1/8 square inches or smaller and the sum of all damaged areas in each 1 foot length of bar does not exceed 2% of the bar surface area. All damaged areas larger than 1/8 square inches shall be repaired and all bars with total damage greater than 2% of bar surface shall be rejected and removed. The total bar surface area covered by patching material shall not exceed 3%.

The fabricator shall exercise care to ensure that the coated bars incorporated into the work are reasonably free from dirt, paint, oil, grease, or other foreign substance; and when deemed necessary, the bars shall be cleaned to the satisfaction of the Engineer. The placing of the concrete shall be performed by using methods and equipment which will not damage the coated material.

Since the epoxy coating is flammable, the coated bars shall not be exposed to any fire or flame. Cutting coated bars by burning will not be permitted.

The coating applicator shall be responsible for performing quality control, tests and repairs of coated

reinforcing steel bars in accordance with the requirements listed in ASTM D3963.

The Department shall have free access to the plant of the epoxy coating applicator, and shall be permitted to have any or all the work specified performed in his presence. The inspector shall be furnished with check samples of the coated bars on a random basis for testing by the Department. If access to this facility is denied, the material shall be rejected.

Prior to fabrication of the prestressed item, the Contractor shall provide a Certified Test Report in accordance with 1.06.07 for epoxy coated bars to verify they meet the specifications.

The Contractor shall also provide a Materials Certificate in accordance with 1.06.07 for the epoxy powder to verify it is in accordance with these specifications.

5.14.03—Construction Methods: The members shall be manufactured in a concrete products plant with approved facilities and equipment for pretensioning.

1. Shop Drawings: Before fabrication, the Contractor shall submit shop drawings to the Engineer for approval in accordance with 1.05.02-3. The shop drawings shall include complete details of the proposed methods, materials and equipment to be used. Such details shall outline the method and sequence of stressing and give complete material designations and details of the prestressing steel and anchorage devices, and other data pertaining to the prestressing operations.

2. Prestressing: In all methods of tensioning, stress induced in the strands shall be determined by monitoring applied force and independently by measurement of elongation. Applied force may be monitored by direct measurement using a pressure gauge piped into the hydraulic pump and jack system, dynamometer or load cell. The two control measurements shall agree with their computed theoretical values, within a tolerance of ± 5%. If discrepancies are in excess of 5% between the two calculated forces, determined by elongation measurement and gauge reading, the tensioning operation shall be suspended and the source of error determined and evaluated by qualified personnel before proceeding. Additionally, the control measurements of force and elongation shall algebraically agree with each other within a 5% tolerance. If the measurements do not agree within 5%, a load cell shall be added at the dead end and if force measurements agree within 5% between the gauge at a live end and the load cell at the dead end, the elongation agreement may be waived with permission from the Engineer.

After an initial force has been applied to the tendon, reference points for measuring elongation due to additional tensioning forces shall be established.

Calculations for elongation and gauge readings must include appropriate allowances for friction in the jacking system, strand seating, movement of abutments, bed shortening if under load, thermal corrections, and any other compensation for the setup. Copies of all calculations shall be made available to the Engineer.

Hydraulic gauges, load cells, or other devices for measuring the stressing load shall be graduated so they can be read within a tolerance of ±2%. Gauges, jacks, and pumps shall be calibrated as a system in the same manner as they are used in tensioning operations. Calibrations shall be performed by an approved testing laboratory, calibration service, or under the supervision of a licensed professional engineer, and a certified calibration curve shall accompany each tensioning system. Pressure readings can be used directly if the calibration determines a reading is within a ±2% tolerance of actual load. Calibrations shall be performed at any time a tensioning system indicates erratic results and, in any case, at intervals not greater than 6 months.

Those producers using multiple strand tensioning systems may use a "Master Gauge" monitoring system for ongoing calibration of their systems after an initial calibration as a total system. The producer shall have a master gauge calibrated every 6 months by an approved testing laboratory, calibration service or under the supervision of a licensed professional engineer. Production gauges shall be checked by plumbing the master gauge adjacent to the production gauge and recording pressures shown on both gauges throughout the normal operating range. Should the difference in gauge pressures exceed 2%, the production gauge shall be taken out of service. This check shall be performed at any time a tensioning system indicates erratic results and, in any case, at intervals not greater than 6 months.

Pressure gauges or other measuring devices, such as digital readout, shall have a full range of measurement of 1 1/2 to 2 times their normal working pressure, whether for initial or final load.

Tensioning methods employing hydraulic gauges shall have appropriate bypass valve snubbers and fittings so that the gauge pointer will not fluctuate but will remain steady until the jacking load is released.

In all methods of pretensioning, the load shall be applied in two increments. An initial load is applied to the individual strands to straighten them, eliminate slack, and provide a starting or reference point for

measuring elongation. The final load is then applied for which elongation of strands is computed and measured. This method of operation shall be mandatory except as noted below in multiple strand tensioning.

Initial tensioning shall not exceed 15% of the specified tensioning force, or 3,000 lb., whichever is greater. In single-strand tensioning, the initial and final loads may be applied in immediate succession on each strand.

At the completion of initial tension, reference marks shall be established from which elongation by final tensioning forces can be measured. Elongations shall then be accurately measured from these reference points. Elongations shall be measured as outlined hereinafter.

Elongation measurement shall take into account all operational losses and compensations in the tensioning system.

A. Final Stressing of Straight Strands:

(1) Single-strand tensioning: After application of the initial load and establishment of reference marks for measuring elongation, the full strand load shall be applied. Loads indicated by gauging systems shall control the tensioning, with elongation checked on every strand. An exception is the case of a completely open bed with no headers or other possible sources of friction. In such instances, strand elongation shall be checked on only the first and last strands and 10% of all others.

(2) Multiple-strand tensioning: Following application of initial stress and seating of each strand on the anchorage header, reference marks shall be established for measuring elongation and slippage.

Reference marks for slippage shall be made by marking a straight line across the strands in each row along the face of the anchorage. For uniform application of load to strands, the face of anchorage at final load must be in a plane parallel to its position under initial load. Parallel movement shall be verified by measurement of movement on opposite sides of the anchorage and a check of its plumb position before and after application of the final load.

B. Final Stressing of Draped Strands: Draped pretensioning strands may be finally stressed by any of the following methods:

(1) Partial stressing and subsequent strains: In this method, the strands shall be tensioned in a straight position or one a partially draped trajectory to a predetermined, intermediate stress value between initial and final stress. The final stress shall be induced by strains resulting from lifting or depressing strands at all other points of change in strand alignment. Final position and stress shall be offset symmetrically about the center of the setup to distribute friction evenly. Stress and elongation shall be measured as specified for the intermediate stress value. Suitable stress measurements at each anchorage at each end of the bed shall verify calculated strand stresses within 5%.

(2) Final stressing in draped position: In this method the strands are stressed to final value in their draped position for the full length of the bed. The strands shall pass over devices which effectively minimize friction at all deflection points. Stress and elongation shall be measured as specified.

When final stressing is done by jacking strands from one end of the bed, even when that tensioning is within tolerance, the force shall be measured on at least two strands at the far end. This force shall not be below the theoretical values by more than 5%. If the theoretical elongation has not been attained at one end of the bed when the force, as indicated by pressure gauge or load cell, is exceeded by 5%, the strand shall be jacked from the other end of the bed to the theoretical elongation. If this requires an overstress as indicated by the gauge in excess of 5% overload, the number of deflection points on the bed shall be reduced until the elongation can be attained with not more than 5% overload or the hardware shall be improved to reduce friction. Remaining deflection points shall then be achieved as outlined.

If elongation is not obtained within 5% tolerance when theoretical force has been applied, the strand may be temporarily overstressed to overcome friction. Overstress shall not exceed 80% of the specified tensile strength of the strand. Strands shall not be seated in this overstress condition. Provision shall be made to reduce the force on the strand before anchorage. Anchorage by seating of strand into chucks shall be done within 5% tolerance of theoretical force.

Stress shall not be transferred to pretensioned members until concrete strength, as indicated by test cylinders or any other properly calibrated nondestructive test technique, is in accordance with specified transfer strength.

If concrete has been heat-cured, detensioning shall be performed immediately following the curing period while the concrete is still warm and moist.

In all detensioning operations, the prestressing forces shall be kept nearly symmetrical about the vertical axis of the member and shall be applied in a manner that will minimize sudden shock or loading.

Maximum eccentricity about vertical axis of the member shall be limited to 10% of the strand group. Limitation of vertical axis eccentricity shall be at the initial cutting of strands at the ends of the bed and as strands are cut between members in the setup. For unusual shapes and heavily stressed shapes, production drawings shall show detensioning procedures.

Forms, ties, inserts, or other devices that would restrict longitudinal movement of the members along the bed shall be removed or adequately loosened. Hold-downs shall be removed at the appropriate time for the product and setup. Alternately, detensioning shall be performed in such a manner sequence that longitudinal movement is precluded.

In a single strand detensioning, both ends of the bed shall be released simultaneously and symmetrically to minimize sliding of members.

The sequence used for detensioning strands shall be according to an approved pattern and schedule that keeps the stresses nearly symmetrical about the vertical axis of the members as specified above.

In multiple strand detensioning, strands shall be released simultaneously by hydraulic dejacking. The total force shall be taken from the header by the jack, then released gradually.

The overstress required to loosen lock nuts or other anchoring devices at the header shall not exceed the force in the strand by 5%.

The maximum permissible time for holding tensioned strands in the bed before starting concrete placement is 36 hours.

Prestressing details that have been approved by the Engineer shall not be deviated from unless details of such deviations are approved in advance of use. The approval by the Engineer of any proposed method, materials, or equipment shall not relieve the Contractor of full responsibility for successfully completing the prestressing operations in accordance with the requirements of these specifications.

3. Forms: The forms for the beams and deck units shall be of substantial construction. A firm compressible material as indicated on the approved shop drawings, shall form the bottom of the casting bed near the ends of the beams in order that cracking and chipping of the beams will be minimized at the time of load transfer. A minimum concrete cover of 1 1/2 inches must be maintained for prestressing strands by the use of approved spreaders or by bundling in areas adjacent to openings, cavities, or inserts. Stirrups and ties shall have a minimum cover of 1 inch at these locations.

Side forms carrying no load may be removed after 24 hours with the permission of the Engineer or after the concrete has reached the strength required at time of transfer (f'ci).

4. Placing Concrete: Concrete shall not be deposited in the forms until the Engineer has inspected the placing of the prestressing elements and reinforcing steel, including all other cast-in-place components, and has given his approval thereof.

Concrete shall not be deposited into the forms when the ambient temperature is below 40°F or above 100° F, unless adequate heating or cooling procedures have been previously approved by the Engineer. The concrete temperature shall be 60°F to 90°F at the time of placement. At no time will truck-mixed or transit-mixed concrete be allowed.

Production during the winter season, from November 15 to March 15 inclusive, will be permitted only on beds located in a completely enclosed structure of suitable size and dimension that provides a controlled atmosphere for the protection of the casting operation and the product.

Outside concreting operations will not be permitted during rainfall unless the operation is completely under cover.

Void forms shall be held in place against uplift or lateral displacement during the pouring and vibrating of the concrete by substantial wire ties or other satisfactory means as approved by the Engineer.

The concrete shall be vibrated internally, or externally, or both, as ordered by the Engineer. The vibrating shall be done with care in such a manner as to avoid displacement of reinforcing steel, prestressing elements, voids, forms, or other components. There shall be no interruption in the pouring of any of the members. Concrete shall be carefully placed in the forms and vibrated sufficiently to produce a surface free from imperfections such as honeycombing, segregation, cracking, or checking. Any deficiencies noted in the members may be cause for rejection.

5. Finishing: Prestressing elements shall be recessed 1/8 inch to 1/4 inch into the member. The recess shall be patched with material approved by the Engineer. When the patch material has cured, it shall be coated with a waterproof material approved by the Engineer. Special care shall be given to the finishing of exposed surface areas. The surface area of all shear keys shall be blast cleaned. Additional finishing of members shall be as shown on the plans or as otherwise directed by the Engineer.

Formed surfaces shall not be finished in any manner unless permitted by the Engineer. Top surfaces shall

be finished as follows:

Beams: The laitance shall be removed from the top of the beams. After the concrete has set sufficiently to prevent tearing loose of coarse aggregate, the top surface shall be transversely raked.

Deck Units: Deck units in structures that will have a bituminous concrete wearing surface shall be given a float finish on the top surface as specified in 6.01.03-II-10. Where the deck units are used with a concrete deck, the top surface of the units shall be finished in accordance with the above requirements for beams.

6. Test Cylinders: During the casting of each prestressed member, the Contractor shall make test cylinders under the supervision of a representative of the Department. The dimensions, type of cylinder mold and number of cylinders shall be specified by the Engineer. At least 4 test cylinders shall be cured by the same methods employed for the curing of the member and shall be used to verify the (f'ci) required for transfer of the prestressing load. The remaining test cylinders shall be used to determine when the required 28-day strength (f'c) has been achieved.

Failure of any of the 28 day test cylinders to meet 90% of the minimum compressive strength or failure of the average to meet the full minimum compressive strength requirement may be cause for rejection.

7. Curing: Unless otherwise shown on the plans or indicated in the special provisions, prestressed concrete members shall be cured in accordance with the latest edition (including Interim Specifications) of "AASHTO LRFD Bridge Construction Specifications."

8. Patching: No patching of the completed members will be allowed unless permitted by the Engineer. The Contractor's proposal for methods and materials to be used in the patching operation shall be submitted to the Engineer for approval. Failure to do so may be cause for rejection.

9. Joining Deck Units: The transverse tie strands shall be installed and tensioned to the total force shown on the plans. When the required length of the transverse tie strand is greater than 13 feet, the strands shall be checked after a period of 36 hours and retensioned if necessary to the required force. After the check and required retensioning, the longitudinal shear keys between the adjacent deck units and the recesses at the ends of the transverse tie strands shall be filled with non-shrink grout meeting the requirements of M.03.05.

Before the non-shrink grout is placed, the key spaces and recesses shall be thoroughly cleaned. If Portland cement non-shrink grout is used, the key areas shall be thoroughly wetted prior to placement of the grout. The Portland cement grout shall be mixed and placed as directed by the Engineer. If premixed non-shrink grout is used, the key areas shall be prepared and the grout mixed and placed as recommended by the grout manufacturer's directions. Where the top surfaces of the adjacent deck units do not match, the grout shall be sloped to form a smooth transition.

No superimposed dead or live loads shall be applied to the adjacent deck units until the non-shrink grout in the longitudinal shear keys and recesses has cured.

The curing time for Portland cement non-shrink grout shall be 7 days. The curing time for premixed non-shrink grout shall be considered the time when the grout has attained a compressive strength of 4,500 psi and as recommended by the grout manufacturer.

10. Quality Control: Quality control shall be the responsibility of the fabricator. Quality assurance shall be the prerogative of the State. At no expense to the State, there shall be provided a separate office building or room having an area of at least 80 s.f., with the least dimension to be 6 feet. This office shall be equipped with a suitable heating system capable of maintaining a minimum temperature of 65°F. It shall be clean and free of extraneous material and equipment. Sufficient light and ventilation shall be provided. During the summer months, the office temperature shall not exceed the ambient temperature. A desk and chair shall be provided for the inspector, and a telephone shall be within audible range of the office.

The quality of fabrication and construction and the dimensional tolerance of the prestressed members shall conform to the limits specified in the "Manual for Quality Control for Plans and the Production of Precast Prestressed Concrete Products (MNL-116-Latest Edition)" published by PCI, unless otherwise specified herein, except that:

1. The tolerance for horizontal alignment shall be measured from a straight line intersecting the ends of beam at either or both faces of top and bottom flanges.
2. The tolerance for beam seat bearing area shall apply to the entire width of the member.

Deviations in excess of the permissible tolerances will be cause for rejection.

11. Marking: Beams and deck units shall be identified as to Project, structure, casting date, and position in the structure by means of a non-corrosive metal tag embedded permanently in the bottom of the members at the mark end.

12. Inspection: The provisions of 6.03.03-4 (Shop Fabrication), (a) Notification, shall apply to the steel

items, and the pertinent requirements contained herein shall apply equally to the work performed in the plant producing the precast concrete products.

13. Lifting Hooks: The number and location of lifting hooks shall be as shown on plans.

14. Special Considerations: Before erection, the Contractor shall recess, clean, patch and paint ends of strands in a neat and workmanlike manner. Projecting fins and surface imperfections shall be removed.

The bearing area at the bottom ends of the beams and deck units must lie in the same plane as the grade of the beam or deck units in their final erected position after application of full dead load. Prior to erection, any deviations of the bearing area from a true plane shall be remedied by grinding, by applying an approved epoxy mortar, or by a combination of both as approved by the Engineer.

15. Handling and Storage: Care shall be taken during storage, transporting, hoisting, and handling of all prestressed members to prevent cracking or damage. Members damaged by improper storing, transporting, or handling shall be replaced by the Contractor at his expense. All storage and handling operations shall be as directed by the Engineer.

After erection, any marking, chipping, or spalling sustained by the member shall be removed or repaired in a neat and workmanlike manner as directed by the Engineer.

16. Methods and Equipment: The Contractor shall be solely responsible for the adequacy of the erection scheme and for all details of plant, falsework, and other equipment and material necessary to carry it out. The Contractor's responsibility includes the investigation of erection stresses. The results of this investigation, including computations, shall be submitted to the Engineer.

5.14.04—Method of Measurement: This work will be measured for payment by the actual number of linear feet of prestressed beams or deck units installed and accepted. Measurement will be made along the center line of each member.

5.14.05—Basis of Payment: Payment for this work will be made at the Contract unit price per linear foot for “Prestressed Beams—Pretensioned, Type ( )” and “Prestressed Deck Units (size)” as shown on the plans, completed and accepted, including all materials, equipment, tools, and labor incidental thereto, and without differentiation as to interior or exterior members.

Pay Item	Pay Unit
Prestressed Beams—Pretensioned, Type ( )	l.f.
Prestressed Deck Units (size)	l.f.

SECTION 5.21
ELASTOMERIC BEARING PADS

5.21.01—Description: Work under this item shall consist of furnishing and installing elastomeric bearing pads and all other necessary materials and equipment to complete the work as shown on the plans.

5.21.02—Materials: Elastomeric bearing pads and adhesive for bonding the pads to steel shall meet the requirements of M.17.01.

5.21.03—Construction Method:

1. Shop Drawings: Before fabricating any materials, the Contractor shall submit drawings to the Engineer for approval in accordance with 1.05.02-3. These drawings shall include but not be limited to the following information:

1. name of manufacturer
2. complete details of the pads
3. pertinent material designations

2. Setting the bearing pads: The Contractor shall set the elastomeric bearing pads in accordance with the following requirements:

(a) Concrete bearing areas: The bearing areas of the masonry upon which the elastomeric bearing pads are to rest shall be carefully finished, by grinding if necessary, to a smooth, even level surface of the required elevation, and shall show no variations from a true plane greater than 1/16 inch over the entire area upon which the elastomeric bearing pads are to rest.

(b) Metal bearing areas: The elastomeric bearing pads shall be bonded to steel masonry plates or to steel sole plates or girders where indicated on plans or when directed by the Engineer.

3. There shall be uniform bearing between the bottom of the beam or steel sole plate and the elastomeric bearing pad and between the elastomeric bearing pad and the concrete seat or steel base plate, after application of full dead load. Also after application of full dead load, there shall be uniform deflection of the elastomeric bearing pad.

4. The elastomeric bearing pads shall be bonded to the steel plates or beams as shown on Contract plans or where directed. All contact surfaces to which the adhesive is to be applied shall be free of oil, paint, lacquer, galvanizing, mill scale, and rust. Primer, if required, and adhesive shall be applied in strict accordance with the manufacturer's printed instructions. The prefabricated pads shall be pressed to the steel plate under sufficient pressure to provide full intimate contact until the adhesive has set.

5. The adhesive shall be mixed in strict accordance with the manufacturer's printed directions.

5.21.04—Method of Measurement: This work will be measured for payment by the number of cubic inches of elastomeric bearing pads, installed and accepted.

5.21.05—Basis of Payment: This work will be paid for at the Contract unit price per cubic inch of "Elastomeric Bearing Pads," complete in place, which price shall include all materials, equipment, tools and labor incidental thereto, including all the cost of furnishing test pads.

Pay Item	Pay Unit
Elastomeric Bearing Pads	c.i.

SECTION 5.22
ELASTOMERIC COMPRESSION SEAL

5.22.01—Description
5.22.02—Materials
5.22.03—Construction Methods
5.22.04—Method of Measurement
5.22.05—Basis of Payment

5.22.01—Description: Under this item the Contractor shall furnish and install elastomeric compression seal with a lubricant-adhesive in joints as shown, dimensioned and detailed on the plans, or as ordered by the Engineer.

5.22.02—Materials: The materials for this work shall meet the requirements of M.17.02.

5.22.03—Construction Methods: The Contractor shall arrange to have a representative of the manufacturer knowledgeable in the methods of installation present at the time of the first installation. The Contractor shall also arrange to have the representative of the manufacturer present at such other times as the Engineer may require.

Shop Drawings: Before fabricating any materials, the Contractor shall submit shop drawings to the Engineer in accordance with 1.05.02-3. These drawings shall include but not be limited to the following information:

1. complete details of the method, materials and equipment proposed to be used
2. name of the manufacturer
3. complete details of the compression seal and method of notching at bends where necessary, showing length and depth of notch

Joint Preparation: Where the concrete has spalled, cracked or chipped adjacent to the joint, the Contractor shall make the necessary repairs with an approved material to the satisfaction of the Engineer and at no additional cost to the State. Immediately before placing the compression seal, the entire joint shall be thoroughly cleaned of all scale, loose concrete, dirt, dust and foreign matter to the satisfaction of the Engineer. The portion of the joint outside of the traveled way, including the mall and all curb areas shall be masked sufficiently on each side of the joint to prevent the lubricant-adhesive from coming in contact with the exposed surfaces.

Installation: The compression seal shall be furnished and installed continuous in length for the full length of the transverse joint as shown on the plans. Cutting shall be performed in accordance with the methods and procedures as indicated on the shop drawings.

The compression seal shall be installed by special hand or machine tools in the joint with the lubricant-adhesive after the concrete in curb areas adjacent to the joint has cured. The lubricant-adhesive shall be applied to the sides of the joint or to the sides of the compression seal or to both such that the full area in contact between the compression seal and the joint shall be covered with the lubricant-adhesive.

Immediately prior to installation, the compression seal shall be wiped with a suitable solvent to remove any dirt, dust, protective coating or any foreign matter. The compression seal shall be installed in a compressed condition and shall at all times be recessed to the depth indicated on the plans. The sides of the compression seal shall be fully in contact with the sides of the joint and the "V" section of the top of the compression seal shall be folded so that it projects inwards towards the inner web sections. Any lubricant-adhesive on the top of the compression seal shall be removed immediately.

Any compression seal that is punctured or ruptured shall be removed in its entirety and replaced by the Contractor at no additional cost to the State.

5.22.04—Method of Measurement: This work will be measured for payment by the actual number of linear feet of elastomeric compression seal, of the various widths, installed and accepted.

5.22.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Elastomeric Compression Seal" of the various widths, complete in place, which price shall include all material including lubricant-adhesive, splicing, equipment, tools, and labor incidental thereto.

Pay Item	Pay Unit
(Size) Elastomeric Compression Seal	l.f.

SECTION 5.86
CATCH BASINS, MANHOLES AND DROP INLETS

5.86.01—Description
5.86.02—Materials
5.86.03—Construction Methods
5.86.04—Method of Measurement
5.86.05—Basis of Payment

5.86.01—Description: The work under this Section shall consist of furnishing, preparing, and installing catch basins, manholes and drop inlets (and also the removal, abandonment, alteration, reconstruction, or conversion of such existing structures) in conformity with the lines, grades, dimensions and details shown on the plans.

This Section shall also include resetting or replacing catch basin tops, as well as manhole frames and covers.

5.86.02—Materials: The materials for this work shall meet the following requirements:

Drainage structures shall meet the requirements of M.08.02 and shall utilize concrete with a 28-day minimum compressive strength of 4000 psi.

Galvanizing shall meet the requirements of M.06.03.

Mortar shall meet the requirements of M.11.04.

Butyl rubber joint seal shall meet the requirements of ASTM C990.

Granular fill, if necessary, shall meet the requirements of M.02.01.

Protective compound material shall be a type listed on the Department's Qualified Products List and be acceptable to the Engineer, as specified in M.03.09.

5.86.03—Construction Methods: Drainage trench excavation, including rock in drainage trench excavation and backfilling, shall be performed in accordance with 2.86.03 and the requirements of the plans.

Where a drainage structure is to be installed below the surface, a drainage trench shall be excavated to the required depth, the bottom of which shall be graded to the elevation of the bottom of the proposed drainage structure or to ensure a uniform foundation for the structure.

Where a firm foundation is not encountered at the grades established due to unsuitable material, such as soft, spongy, or unstable soil, the unsuitable material shall be removed and replaced with approved granular fill, thoroughly compacted in lifts not to exceed 6 inches. The Engineer shall be notified prior to removal of the unsuitable material in order to determine the depth of removal necessary.

When rock, as defined in 2.86.01-2, is encountered, work shall be performed in accordance with 2.86.03 and the requirements of the plans.

When a drainage structure outside of proposed drainage trench limits is to be removed, it shall be completely removed and all pipes shall be removed or plugged with cement masonry.

When a drainage structure is to be abandoned, the structure shall be removed to a depth 2 feet below the subgrade or as directed by the Engineer. The floor of the structure shall be broken and all pipes shall be plugged with cement masonry.

Drainage structures shall be constructed in accordance with the plans and the requirements contained herein for the character of the work involved. The provisions of 6.02.03 pertaining to bar reinforcement shall apply except that shop drawings need not be submitted for approval unless called for in the plans, Contract or directed by the Engineer. Welding shall be performed in accordance with the applicable sections of the AWS Structural Welding Code, D1.1.

When it becomes necessary to increase the horizontal dimensions of manholes, catch basins and drop inlets to sizes greater than those shown on the plans in order to provide for multiple pipe installations, large pipes or for other reasons, the Contractor shall construct such manholes, catch basins and drop inlets to modified dimensions as directed by the Engineer.

The surfaces of the tops of all catch basins, and drop inlets shall be given a coat of protective compound material, at the manufacturer's recommended application rate, immediately upon completion of the concrete curing period.

All masonry units shall be laid in full mortar beds.

Metal fittings for catch basins, manholes or drop inlets shall be set in full mortar beds or otherwise secured as shown on the plans.

All inlet and outlet pipes shall be set flush with the inside face of the wall of the drainage structure as shown on the plans. The pipes shall extend through the walls for a sufficient distance beyond the outside surface to allow for satisfactory connections, and the concrete or masonry shall be constructed around them neatly to prevent leakage along their outer surfaces.

When constructing a new drainage structure within a run of existing pipe, the section of existing pipe disturbed by the construction shall be replaced with new pipe of identical type and size extending from the drainage structure to the nearest joint of the existing pipe in accordance with 6.86.03 or as directed by the Engineer.

Backfilling shall be performed in accordance with 2.86.03.

Frames, covers and tops which are to be reset shall be removed from their present beds, the walls or sides shall be rebuilt to conform to the requirements of the new construction and the frames, covers and tops shall be reset as shown on the plans or as directed by the Engineer.

5.86.04—Method of Measurement:

Drainage Trench Excavation: In accordance with 2.86.04, excavation for drainage trench will not be measured for payment but shall be included in the Contract unit price for the type of structure being installed.

Rock in Drainage Trench Excavation: The volume in cubic yards of Rock in Drainage Trench Excavation will be measured in accordance with the drainage trench excavation limits described in 2.86.03.

Manholes, Catch Basins and Drop Inlets will be measured as separate units.

Resetting of Manholes, Catch Basins and Drop Inlets will be measured as separate units.

Replacement of frames, covers, and tops will be measured as a unit for catch basin top or manhole frame and cover.

Conversion of drainage structures as specified on the plans, or as directed by the Engineer, including structure reconstruction will be measured for payment as a unit.

Removal or abandonment of drainage structures outside of drainage trench excavation limits, as defined in 2.86.03, will be measured as separate units.

There will be no measurement or direct payment for the application of the protective compound material, the cost of this work shall be considered as included in the general cost of the work.

Measurement for payment for work and materials involved with installing pipes to connect new drainage structures into a run of existing pipe will be as provided for under the applicable Contract items in accordance with 6.86.04.

There will be no measurement or direct payment for plugging existing pipes with cement masonry, the cost of this work will be considered as included in the general cost of the work.

5.86.05—Basis of Payment:

Drainage Trench Excavation for the installation of proposed structures described herein will be included in the unit price paid for the respective drainage Contract item(s) for which the excavation is being performed, in accordance with the provisions of 2.86.05.

Rock in Drainage Trench Excavation will be paid for in accordance with the provisions of 2.86.05.

Manholes and Catch Basins will be paid for at the Contract unit price for each "Manhole," or "Catch Basin," of the type and size specified, at "0' to 10' Deep" or "0' to 20' Deep," complete in place, which price shall include all excavation, backfill, materials, equipment, tools and labor incidental thereto.

Drop Inlets will be paid for at the Contract unit price for each "Drop Inlet," of the type specified, complete in place, which price shall include all excavation, backfill, materials, equipment, tools and labor incidental thereto.

Manholes, Catch Basins and Drop Inlets constructed to modified dimensions as directed by the Engineer, will be paid for as follows:

Where the interior floor area has to be increased to accommodate existing field conditions, as measured horizontally at the top of the base of the completed structure, and does not exceed 125% of the interior floor area as shown on the plans for that structure, then the structure shall be paid for at the Contract unit price for each "Manhole," "Catch Basin," or "Drop Inlet" of the type specified. Where the floor area is greater than 125%, the increase in the unit price for the individual structure shall be in direct proportion to the increase of the completed structure interior floor area as compared to the interior floor area as shown on the plans for that structure. Such increased unit price shall include all excavation, materials, equipment, tools, and labor incidental to the completion of the structure.

Reset Units will be paid for at the Contract unit price each for "Reset Manhole (Type)," "Reset (Type) Catch Basin," or "Reset (Type) Drop Inlet," respectively, complete in place, which price shall include excavation, cutting of pavement, removal and replacement of pavement structure, and all materials, equipment, tools and labor incidental thereto, except when the work requires reconstruction greater than 3 feet, measured vertically, then the entire cost of resetting the unit will be paid for as Extra Work in accordance with the provisions of 1.04.05.

Frames, Covers, and Tops, when required in connection with reset units, will be paid for at the Contract unit price each for such "Manhole Frame and Cover" or "(Type) Catch Basin Top," complete in place, including all incidental expense; or when no price exists, the furnishing and placing of such material will be paid for as Extra Work in accordance with the provisions of 1.04.05.

When the catch basin top has a stone or granite curb in its design, the curb or inlet shall be included in the cost of the "(Type) Catch Basin Top."

Conversion of drainage structures will be paid for at the Contract unit price each for "Convert Catch Basin to (Type) Catch Basin," "Convert Catch Basin to (Type) Manhole," or "Convert Manhole to (Type) Catch Basin," complete in place, which price shall include excavation, cutting of pavement, removal and replacement of pavement, backfill, all alterations to existing structure, all materials including catch basin frame and grate of the type specified, or manhole frame and cover, all equipment, tools and labor incidental thereto.

The maximum change in elevation of frame under these items shall not exceed 3 feet. Greater depth changes, if required, shall be paid for as Extra Work, in accordance with 1.04.05.

Removal or abandonment of drainage structures outside of drainage trench excavation limits as defined in 2.86.03 will be paid for at the Contract unit price each for "Remove Drainage Structure – 0' to 10' Deep," "Remove Drainage Structure – 0' to 20' Deep," or “Abandon Drainage Structure,” which price shall include excavation, cutting of pavement, removal and replacement of pavement, backfill, and all equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
(Type) Catch Basin – 0' to 10' Deep	ea.
(Type) Catch Basin – 0' to 20' Deep	ea.
Manhole (Size) – 0' to 10' Deep	ea.
Manhole (Size) – 0' to 20' Deep	ea.
(Type) Drop Inlet	ea.
Reset (Type) Catch Basin	ea.
Reset Manhole (Type)	ea.
Reset (Type) Drop Inlet	ea.
Convert Catch Basin to (Type) Catch Basin	ea.
Convert Catch Basin to (Type) Manhole	ea.
Convert Manhole to (Type) Catch Basin	ea.
Manhole Frame and Cover	ea.
(Type) Catch Basin Top	ea.
Remove Drainage Structure – 0' to 10' Deep	ea.
Remove Drainage Structure – 0' to 20' Deep	ea.
Abandon Drainage Structure	ea.

NOTES

SECTION 6.01
CONCRETE FOR STRUCTURES

6.01.01—Description
6.01.02—Materials
6.01.03—Construction Methods
6.01.04—Method of Measurement
6.01.05—Basis of Payment

6.01.01—Description: This item shall include concrete for use in new construction, surface repair or structural repair of bridges and culverts, walls, catch basins, drop inlets and other incidental construction. The concrete shall be composed of Portland cement, pozzolans, fine and coarse aggregate, admixtures and water, prepared and constructed in accordance with these specifications, at the locations and of the form dimensions and class shown on the plans, or as directed by the Engineer.

The use of concrete from dry batch or central mixed plants is permitted for all concrete mixtures.

6.01.02—Materials: The materials for this work shall meet the requirements of M.03. Surface or structural repair concrete shall be documented on the delivery ticket, as required in 6.01.03-II-3(a), as having the plastic properties necessary for confined placement to ensure appropriate workability for consolidation within the forms.

6.01.03—Construction Methods:

I. Concrete Quality Control (QC) Requirements: For all bridge deck and bridge parapet construction, the Contractor must demonstrate to the Engineer that the materials and work that will be provided by their field staff, subcontractors, and suppliers meets Contract specification requirements.

This effort shall be documented with a **Concrete Quality Control Plan (CQCP)** and shall address the communication with all parties, on-site inspection, sampling and testing frequency necessary to keep the production, placement and finishing operations in control, to determine when an operation has gone out of control and anticipated procedure to correct the situation in a timely manner.

1. General – provide an overview of the means and methods anticipated to perform the work including any anticipated conditions that may need additional attention (such as seasonal conditions requiring heating or cooling of concrete)

2. Contractor Organization – address authority levels/duties by position and name of persons holding those positions; include those who have decision making authority with regard to quality control, materials, sampling and testing who can be contacted by the Engineer

3. Concrete Mix Design – identify concrete supplier(s); provide copies of all applicable mix designs to field staff; and address submittal timeframe

4. Transportation and Delivery of Concrete – identify the supplier's plant capacity and ability to ensure continuous delivery to the Project to meet the requirements of the mix design and a corrective procedure if it does not meet Project requirements; include a provision for the addition of admixtures and follow up testing

5. Placement and Finishing of Concrete – identify and describe:

- **(a)** placement equipment
- **(b)** placement method(s) to be used (chute, pump, hopper or other)
- **(c)** starting point and direction of placement (logistical sequencing)
- **(d)** slip forming, formwork, stay-in-place forms or other forming method(s)
- **(e)** joint construction method(s)
- **(f)** process and documentation that the elevations, base, forms, reinforcement (including support chairs and ties), utility inserts or any other appurtenance installations have been inspected by the Contractor prior to concrete placement
- **(g)** equipment and method(s) to be used for vibrating and consolidating concrete
- **(h)** procedure for verifying adequate consolidation and how segregation will be addressed
- **(i)** schedule and method(s) to be used for finishing all exposed surfaces

6. Curing of Concrete – describe schedule and method(s) for curing of concrete and how the method(s) will be monitored and maintained

7. Contractor QC testing – identify person(s) or firms responsible for Contractor QC testing and provide copies of their certification(s) (see 6.01.03-II-5), and testing facility location(s). In addition, describe the process used for communication between the QC testing personnel and the Contractor project staff;

describe what measures will be taken when test results are out of compliance; this shall include what increased frequency of testing is to be performed to verify that concrete properties are in compliance; the threshold at which time placement ceases; describe what protective measures will be used in case of unforeseen weather

8. The CQCP shall include the name and qualifications of a Quality Control Manager (QCM) provided by the Contractor. The QCM shall be responsible for the administration of the CQCP, and any modifications that may become necessary. The QCM shall have the ability to direct all Contractor personnel on the Project during concreting operations and must communicate directly with the concrete supplier. The QCM shall be certified as either a **Concrete Transportation Construction Inspector by the American Concrete Institute (ACI)** or a **NETTCP Concrete Inspector.**

9. The CQCP must include a provision for pre-placement meeting(s) to be held with representatives of the Engineer, the concrete supplier, the QCM and the Contractor's field staff supervising the work.

- **(a)** Timing and number of the meeting(s) will be determined by the complexity of the mix design or placement.
- **(b)** Non-Standard mix designs that require trial placements will be discussed at the Preconstruction Meeting to remind the Contractor of the time needed for testing. Additional meeting(s) should be scheduled at least 90 days prior to first use of non-standard mix designs, to allow suppliers to perform trial batches and testing.
- **(c)** Discussions shall include the configuration and specific application that the concrete will be used for, plastic properties and workability, any mix design challenges, trial placement procedures and subsequent trial results, timing and quantities. Refer to 6.01.03-II-6(e) for additional requirements.

10. The CQCP shall be submitted to the Engineer and concrete supplier for review and comment a minimum of 30 days prior to production or placement. Production and placement shall not occur until all comments of the Engineer and supplier have been addressed by the Contractor. Changes to the CQCP based on data not available at time of submittal may be added via addendum.

11. The Contractor shall provide the Engineer QC test results within 48 hours after testing or inspection in a format acceptable to the Engineer. The Contractor shall also maintain complete records of all QC tests.

Review of the CQCP does not relieve the Contractor of its responsibility to comply with the Project specifications. The Contractor may modify the CQCP as work progresses and must document the changes in writing prior to resuming operations. These changes include but are not limited to changes in quality control procedures or personnel.

II. New Construction:

1. Falsework and Forms: Falsework is considered to be any temporary structure which supports structural elements of concrete, steel, masonry or other material during the construction or erection. Forms are to be considered to be the enclosures or panels which contain the fluid concrete and withstand the forces due to its placement and consolidation. Forms may in turn be supported on falsework.

This work shall consist of the construction and removal of falsework and forms that are designed by the Contractor in the execution of the work, and whose failure to perform properly could adversely affect the character of the Contract work or endanger the safety of adjacent facilities, property, or the public. Forms shall be mortar tight. Forms and falsework shall be of sufficient rigidity and strength to safely support all loads imposed and to produce in the finished structure the lines and grades indicated in the Contract documents. Forms shall also impart the required surface texture and rustication and shall not detract from the uniformity of color of the formed surfaces. Forms shall be made of wood, steel or other material approved by the Engineer.

- **(a) Design:** The design of falsework and formwork shall conform to the *AASHTO Guide Design Specifications for Bridge Temporary Works*, or to other established and generally accepted design codes such as ACI Standard *ACI 347-Recommended Practice for Concrete Formwork* or specific form or falsework manufacturer specifications. When other than new or undamaged materials are used, appropriate reductions in allowable stresses, and decreases in resistance factors or imposed loads shall be used for design.
- **(b) Loads:** The design of the falsework and forms shall be based on load factors specified in the *AASHTO LRFD Bridge Design Specifications* and all applicable load combinations shall be investigated. The design load for falsework shall consist of the sum of appropriate dead and live vertical loads and any horizontal loads.

As a minimum, dead loads shall include the weight of the falsework and all construction material to be supported. The combined unit weight of concrete, reinforcing and pre-stressing steel, and forms that is supported shall be assumed to be not less than:

1. Normal-weight concrete: 0.16 kip/ft^3
2. Lightweight concrete: 0.13 kip/ft^3

Live loads shall consist of the actual weight of any equipment to be supported, applied as concentrated loads at the points of contact and a uniform load of not less than 0.02 kip/ft^2 applied over the area supported, plus 0.075 kip/ft applied at the outside edge of deck overhangs.

The horizontal load used for the design of the falsework bracing system shall be the sum of the horizontal loads due to equipment; construction sequence including unbalanced hydrostatic forces from fluid concrete and traffic control devices; stream flow, when applicable; and an allowance for wind. However, in no case shall the horizontal load to be resisted in any direction be less than 2% of the total dead load.

For post-tensioned structures, the falsework shall also be designed to support any increase in or redistribution of loads caused by tensioning of the structure. Loads imposed by falsework onto existing, new, or partially completed structures shall not exceed those permitted in 6.01.03-II-12, Application of Loads.

(c) Working Drawings: The working drawings for falsework and formwork shall be prepared in accordance with 1.05.02 whenever the falsework or formwork exceeds 14.0 feet high or whenever vehicular, marine, or pedestrian traffic may travel under or adjacent to the falsework or formwork. Working drawings shall include the sequence, method and rate of placement of the concrete.

Manufacturer catalog cuts or written installation procedures shall be provided for any clips, braces, hangers or other manufactured parts used with the formwork or falsework.

(d) Construction: Forms and falsework shall be built true to lines and grades shall be strong, stable, firm, mortar-tight and adequately braced or tied, or both. They shall be designed and constructed to withstand all loads and pressures including those imposed by plastic concrete, taking full account of the stresses due to the rate of placement, effect of vibration and conditions brought about by construction methods. Forms and falsework shall be constructed to compensate for variations in camber of supporting members and allow for deflections.

Falsework and formwork shall be chamfered at all sharp corners, unless otherwise ordered or permitted, and shall be given a slight bevel or draft in the case of projections to ensure satisfactory removal. Materials for falsework and formwork and their supports, ties and bracing, shall be of the type, quality and strength to achieve the structural requirements. Form material in contact with concrete shall provide the finished concrete surface smoothness as specified in 6.01.03-II-10, Finishing Concrete Surfaces, and shall have a uniform appearance.

Falsework and formwork shall be treated with form oil or other release agent approved by the Engineer before the reinforcing steel is placed or self-releasing forms approved by the Engineer may be used. Release agents which will adhere to or discolor the concrete shall not be used.

Falsework and formwork for concrete surfaces exposed to view shall produce a smooth surface of uniform texture, free of voids, indentations, protrusions and bulges. Panels lining falsework and formwork shall be arranged so that the joint lines form a symmetrical pattern conforming to the general lines of the structure. The same type of form-lining material shall be used throughout each element of a structure. Falsework and formwork shall be sufficiently rigid so that the undulation of the concrete surface shall not exceed 1/4 inch when checked with a 4 foot straightedge or template.

For non-exposed surfaces the falsework and formwork shall be sufficiently rigid so that the undulation of the concrete surface shall not exceed 1/2 inch when checked with a 4 foot straightedge or template.

Metal ties and anchors to hold the falsework and formwork in alignment and location shall be so constructed that the metal work can be removed to a depth of at least 2 inches from the concrete surface without damage to the concrete. All cavities resulting from the removal of metal ties shall be filled after removal of forms with cement mortar of the same proportions used in the body of the work or other materials approved by the Engineer, and the surface finished smooth and even, and if exposed in the finished work, shall be similar in texture and color of adjacent surfaces. With permission of the Engineer, the Contractor need not remove from the underneath side of bridge decks portions of metal devices used to support reinforcing steel providing such devices are of material, or are adequately coated with material, that will not rust or corrode. When coated

reinforcing steel is required, all metal ties, anchorages, or spreaders that remain in the concrete shall be of corrosion-resistant material or coated with a dielectric material.

Forms shall be clean and clear of all debris. For narrow walls and columns where the bottom of the form is inaccessible, an access opening will be allowed in the form and falsework for cleaning out extraneous material.

(e) Vacant

(f) Bridge Decks: After erection of beams and prior to placing falsework and forms, the Contractor shall take elevations along the top of the beam at the points shown on the plans or as directed by the Engineer. The Contractor shall calculate the haunch depths and provide them to the Engineer a minimum of 7 days prior to installing the falsework and forms. The Contractor shall also provide calculations for the setting of the overhang brackets based on the final beam deflection. These calculations shall be based on the final proposed deck grade and parapet elevations.

Falsework or formwork for deck forms on girder bridges shall be supported directly on the girders so that there will be no appreciable differential settlement during placing of the concrete. Girders shall be either braced and tied to resist any forces that would cause rotation or torsion in the girders caused by the placing of concrete for diaphragms or decks, or shown to be adequate for those effects. Unless specifically permitted, welding of falsework support brackets or braces to structural steel members or reinforcing steel shall not be allowed.

(g) Stay-In-Place Metal Forms for Bridge Decks: These forms may be used if shown in the Contract documents or approved by the Engineer. Prior to the use of such forms and before fabricating any material, the Contractor shall submit working drawings to the Engineer for review in accordance with 1.05.02. These drawings shall include the proposed method of form construction, erection plans including placement plans, attachment details, weld procedure(s), material lists, material designation, gage of all materials, and the details of corrugation. Also, copies of the form design computations shall be submitted with the working drawings. Any changes necessary to accommodate stay-in-place forms, if approved, shall be at no cost to the Department.

The metal forms shall be designed on the basis of the dead load of the form, reinforcement and the plastic concrete, including the additional weight of concrete [considered to be equivalent to the weight imposed by an additional concrete thickness equal to 3% of the proposed deck thickness, but not to exceed 0.3 inch] due to the deflection of the metal forms, plus 50 psf for construction loads. The allowable stress in the corrugated form and the accessories shall not be greater than 0.725 times the yield strength of the furnished material and the allowable stress shall not exceed 36,000 psi. The span for design and deflection shall be the clear distance between edges of the beams or girders less 2 inches and shall be measured parallel to the form flutes. The maximum deflection under the weight of plastic concrete, reinforcement, and forms shall not exceed 1/180 of the form span or 0.5 inches, whichever is less. In no case shall the loading used to estimate this deflection be less than 120 psf. The permissible form camber shall be based on the actual dead load condition. Camber shall not be used to compensate for deflection in excess of the foregoing limits. The form support angles shall be designed as a cantilever and the horizontal leg of the form support angle shall not be greater than 3 inches.

No stay-in-place metal forms shall be placed over or be directly supported by the top flanges of beams or girders. The form supporting steel angles may be supported by or attached to the top flanges.

Stay-in-place metal forms shall not be used in bays where longitudinal slab construction joints are located, under cantilevered slabs such as the overhang outside of fascia members, and bridges where the clearance over a salt-laden body of water is less than 15 feet above mean high water level.

Welding to the top flanges of steel beams and girders is not permitted in the areas where the top flanges are in tension, or as indicated on the plans. Alternate installation procedures shall be submitted addressing this condition.

Drilling of holes in pre-stressed concrete beams or the use of power-actuated tools on the prestressed concrete beams for fastening of the form supports to the pre-stressed concrete beams will not be permitted. Welding of the reinforcing steel to the pre-stressed units is not permitted.

All edges of openings cut for drains, pipes, and similar appurtenances shall be independently supported around the entire periphery of the opening. All fabricated stay-in-place metal forms shall be unloaded, stored at the Project Site at least 4 inches above the ground on platforms, skids or other suitable supports and shall be protected against corrosion and damage and handled in such a manner

as to preclude damage to the forms. Damaged material shall be replaced at no additional cost to the State.

Any exposed form or form support metal where the galvanized coating has been damaged, shall be thoroughly cleaned, wire brushed, then coated with 2 coats of Zinc Dust – Zinc Oxide primer, FS No. TT-P-641d, Type II or another product acceptable to the Engineer.

The forms shall be installed from the topside in accordance with the manufacturer's recommended installation procedures. The form supports shall ensure that the forms retain their correct dimensions and positions during use at all times. Form supports shall provide vertical adjustment to maintain design slab thickness at the crest of corrugation, to compensate for variations in camber of beams and girders and to allow for deflections. Stay-in-place metal forms shall have a minimum depth of the form valley equal to 2 inches. The forms shall have closed tapered ends. Lightweight filler material shall be used in the form valleys.

All field cutting shall be done with a steel cutting saw or shears including the cutting of supports, closures and cutouts Flame cutting of forms is not permitted.

All welding shall be performed by Department-certified welders in accordance with the Welding subarticle in 6.03. Welding of forms to supports is not permitted.

The steel form supports shall be placed in direct contact with the flange of stringer or floor beam flanges and attached by bolts, clips, welding where permitted, or other approved means. Form sheets shall not be permitted to rest directly on the top of the stringer or floor beam flanges. The forms shall be securely fastened to form supports with self-drilling fasteners and shall have a minimum bearing length of 1 inch at each end. In the areas where the form sheets lap, the form sheets shall be securely fastened to one another by fasteners at a maximum spacing of 18 inches. The ends of the form sheets shall be securely attached to the support angles with fasteners at a maximum spacing of 18 inches or 2 corrugation widths, whichever is less.

The depth of the concrete slab shall be as shown on the plans and the corrugated forms shall be placed so that the top of the corrugation will coincide with the bottom of the deck slab. No part of the forms or their supports shall protrude into the slab. All reinforcement in the bottom reinforcement mat shall have a minimum concrete cover of 1 inch unless noted otherwise on the plans.

The completed stay-in-place metal form system shall be sufficiently tight to prevent leakage of mortar. Where forms or their installation are unsatisfactory in the opinion of the Engineer, either before or during placement of the concrete, the Contractor shall correct the defects before proceeding with the work.

(h) Construction Joints: Construction joints other than those shown on the plans will not be permitted without prior approval of the Engineer. In joining fresh concrete to concrete that has already set, the work already in place shall have all loose and foreign material removed, and the surface roughened and thoroughly drenched with water.

All reinforcing steel shall extend continuously through joints. Where unplanned construction joints may be needed, they shall be constructed as directed by the Engineer.

(i) Expansion and Contraction Joints: Expansion and contraction joints shall be constructed at the locations and in accordance with the details specified in the Contract. The forming of joint openings shall be dimensioned in accordance with the joint manufacturer’s design requirements. Joints include open joints, filled joints, joints sealed with sealants, joints reinforced with steel armor plates or shapes, paraffin coated joints, and joints with combinations of these features.

Open joints shall be placed at locations designated on the plans and shall be formed by the insertion and subsequent removal of templates of wood, metal or other suitable material. The templates shall be so constructed that their removal may be readily accomplished without damage to the work.

Filled joints shall be made with joint filler, the materials for which shall meet the requirements of the plans and of these specifications.

For mechanical joint systems, the concrete shall be placed in such a manner that does not interfere with the movement of the joint.

(j) Pipes, Conduits and Utility Installations: The Contractor shall coordinate the installation of pipes, conduits and utilities as shown on the plans and in accordance with the Contract or as directed by the Engineer. The openings accommodating such pipe, conduit and utility installations shall be incorporated into the formwork by the Contractor.

(k) Anchorages: Anchor bolts and systems shall be set to the requirements of the plans and Contract. Anchor bolts and systems shall be clean and free of dirt, moisture or other foreign materials at the time of installation. The anchor bolts and systems shall be installed prior to placing concrete.

With the Engineer's approval, the Contractor may install anchorages after placement and setting of the concrete or in formed holes. The anchorages shall be installed into drilled or formed holes having a diameter and a depth suitable to receive the bolts in accordance with the grout manufacturer's requirements. Such holes shall be located to avoid damage to the existing reinforcement. All holes shall be perpendicular to the plane surface. The Contractor shall take every precaution necessary to prevent damage to the concrete due to freezing of water or grout in anchor bolt holes.

(l) Ornament or Reverse Moulds: Ornamental work, when so noted on the plans, shall be formed by the use of reverse moulds. These moulds shall be produced by a qualified manufacturer approved by the Engineer. They shall be built in accordance with the general dimensions and appearance shown on the plans. The Contractor shall submit all detailed drawings, models, or carvings for review by the Engineer before the moulds are made.

The Contractor shall be responsible for their condition at all times, and shall be required to remove and replace any damaged or defective moulds at no additional cost to the State.

The surfaces of the moulds shall be given a coating of form release agent to prevent the adherence of concrete. Any material which will adhere to or discolor the concrete shall not be used.

Form Liners, if required, shall be installed as specified elsewhere.

(m) Removal of Falsework and Forms: The Contractor shall consider the location and character of the structure, the weather, the materials used in the mix, and other conditions influencing the early strength of the concrete when removing forms and falsework. Methods of removal likely to cause damage to the concrete surface shall not be used. Supports shall be removed in such a manner as to permit the structure to uniformly and gradually take the stresses due to its own weight. For structures of 2 or more spans, the sequence of falsework release shall be as specified in the Contract or approved by the Engineer.

Removal shall be controlled by field-cured cylinder tests. The removal shall not begin until the concrete has achieved 75% of the design compressive strength. To facilitate finishing, side forms carrying no load may be removed after 24 hours with the permission of the Engineer, but the curing process must be continued for 7 days.

When the results of field-cured cylinder tests are unavailable, the time periods listed in Table 6.01.03-1, exclusive of days when the temperature drops below 40°F, may govern the removal of forms.

Table 6.01.03-1 Time Restrictions for Removal of Formwork

Structure Element	Minimum Time Period
Arch Centers, centering under beams, pier caps, and unsupported elements	14 days
Slabs on grade, Abutments and Walls	24 hours
Columns	2 days
Bridge Decks	28 days

The Contractor may submit for review and approval by the Engineer, alternate methods to determine the in-place strength of the concrete for removal of forms and falsework.

2. Protection from Environmental Conditions: The concrete shall be protected from damage due to weather or other environmental conditions during placing and curing periods. In-place concrete that has been damaged by weather conditions shall be either repaired to an acceptable condition or removed and replaced as determined by the Engineer.

(a) Rain Protection: The placement of concrete shall not commence or continue unless adequate protection satisfactory to the Engineer is provided by the Contractor.

(b) Hot Weather Protection: When the ambient air temperature is above 90°F, the forms, which will come in contact with the mix shall be cooled to below 90°F for a minimum of 1 hour prior to and 1 hour after completion of the concrete placement by means of a water spray or other methods satisfactory to the Engineer.

(c) Cold Weather Protection: When there is a probability of ambient air temperature below 40°F

during placement and curing, a Cold-Weather Concreting Plan shall be submitted to the Engineer for review and comment. The Plan shall detail the methods and equipment, including temperature measuring devices that will be used to ensure that the required concrete and air temperatures are maintained.

1. Placement: The forms, reinforcing steel, steel beam flanges, and other surfaces which will come in contact with the mix shall be heated to a minimum of 40°F, by methods satisfactory to the Engineer, for a minimum of 1 hour prior to, and maintained throughout, concrete placement.
2. Curing: For the first 6 days, considered the initial cure period, the concrete shall be maintained at a temperature of not less than 45°F and the air temperature surrounding the structure shall be maintained at a temperature of not less than 60°F. When the concrete mix includes pozzolans or slag, the initial cure period shall be increased to 10 days. After the initial cure period, the air surrounding the structure shall be maintained above 40°F for an additional 8 days. If external heating is employed, the heat shall be applied and withdrawn gradually and uniformly so that no part of the concrete surface is heated to more than 90°F or caused to change temperature by more than 20°F in 8 hours. The Engineer may reduce or increase the amount of time that the structure must be protected or heated based on an indication of in-place concrete strength acceptable to the Engineer.

(d) Additional Requirements for Bridge Decks: Prior to the application of curing materials, all the concrete placed on bridge decks shall be protected from damage due to rapid evaporation by methods acceptable to the Engineer. During periods of low humidity (less than 60% relative humidity), sustained winds of 25 mph or more, or ambient air temperatures greater than 80ºF the Contractor shall provide written details of additional measures to be taken during placement and curing.

Protection may include increasing the humidity of the surrounding air with fog sprayers and employing wind-breaks or sun-shades. Additional actions may include reduction of the temperature of the concrete prior to placement, scheduling placement during the cooler times of days or nights, or any combination of these actions.

(e) Concrete Exposed to Salt Water: No Construction joints shall be formed between the levels of extreme low water and extreme high water or the upper limit of wave action as determined by the Engineer.

3. Transportation and Delivery of Concrete: All material delivered to the Project shall be supplied by a producer qualified in accordance with M.03. The producer shall have sufficient plant capacity and trucks to ensure continuous delivery at the rate required to prevent the formation of cold joints.

(a) Material Documentation: All vendors producing concrete must have their weigh scales and mixing plant automated to provide a detailed ticket. Delivery tickets must include the following information:

1. State of Connecticut printed on ticket
2. Name of producer, identification of plant
3. Date and time of day
4. Type of material
5. Cubic yards of material loaded into truck
6. Project number, purchase order number, name of Contractor (if Contractor other than producer)
7. Truck number for specific identification of truck
8. Individual aggregate, cement, water weights and any admixtures shall be printed on plant tickets
9. Water/cement ratio, and
10. Additional water allowance in gallons based on water/cement ratio for mix

A State inspector may be present to monitor batching or weighing operations.

The Contractor shall notify the Engineer immediately if, during the production day, there is a malfunction of the recording system in the automated plant or weigh scales.

Manually written tickets containing all required information may be allowed for up to 1 hour after malfunction provided they are signed by an authorized representative of the producer.

(b) Transportation of Mixture: Trucks delivering concrete shall be qualified in accordance with M.03.

If the concrete mix arrives at the Project with a slump lower than allowed by specification, water may be considered as a means to temper concrete to bring the slump back to within specification. This tempering may only be done prior to discharge with the permission of the Engineer. The

quantity of water in gallons added to the concrete cannot exceed the allowance shown on the delivery ticket.

The concrete shall be completely discharged into the forms within 1-1/2 hours from the batch time stamped on the delivery ticket. This time may be extended if the measured temperature of the concrete is below 90ºF. This time may also be reduced if the temperature of the concrete is over 90º F. Rejected concrete shall be disposed of by the Contractor at no cost to the State.

The addition of chemical admixtures or air entrainment admixtures at the Project Site, to increase the workability or to alter the time of set, will only be permitted if prior approval has been granted by the Engineer. The addition of air entrainment admixtures at the Project Site will only be permitted by the producer's quality control staff. The Contractor is responsible for follow-up quality control testing to verify compliance with the Specifications.

4. Acceptance Testing and Test Specimens: The Contractor shall furnish the facilities and concrete required for sampling, transport to the testing location in the field, performing field testing and for casting sample cylinders for compressive-strength determinations. The Department will furnish personnel for sampling and casting Acceptance specimens and the number of specimens required will be determined by the Engineer. The equipment for the Department's testing is provided for elsewhere in the Contract.

(a) Temperature, Air Content and Slump: Field testing in accordance with AASHTO T-23, "Making and Curing Concrete Test Specimens in the Field" will be performed at the point of placement and at a frequency determined by the Engineer.

(b) Acceptance Testing and Compressive Strength Specimens: Concrete samples are to be taken at the point of placement into the forms or molds. Representatives of the Engineer will sample the mix.

Table 6.01.03-2 Plastic Properties of Portland Cement Concrete

Standard Mix Class	Air Content	Slump[3]	Concrete Temperature
PCC0334Z[1] (3300 psi)	6.0 +/- 1.5%	As submitted	60º-90º F
PCC0336Z[1] (3300 psi)			
PCC0446Z[1] (4400 psi)			
PCCXXX8Z[1]	7.5 +/- 1.5%	As submitted	
Modified Standards[2]	6.0 +/- 1.5% [2]	As submitted	
Special Provision Mix[4]	As specified	As submitted	
[1] "Z" denotes the Exposure Factor 0, 1 or 2 as described in Table M.03.02-1a			
[2] Modifications to Standard Mixes, including mixes placed by pumping, shall be reviewed by the Engineer prior to use. These include but are not limited to the use of chemical admixtures such as high range water reducing (HRWR) admixtures and the use of coarse aggregate sizes for that class not specified in M.03.			
[3] If the only modification is the addition of HRWR, the maximum allowable slump shall be 7 inches.			
[4] All concrete mixes with a mix design strength not shown in the table must be approved by the Engineer on a case-by-case basis. Limits on the plastic properties and strength requirements of these mixes are listed in the Specifications.			

The Contractor shall provide and maintain facilities on the Project Site, acceptable to the Engineer, for sampling, transporting the initial sample, casting, safe storage and initial curing of the concrete test specimens as required by AASHTO T-23. This shall include but not be limited to a sampling receptacle, a means of transport of the initial concrete sample from the location of the concrete placement to the testing location, a level and protected area of adequate size to perform testing, and a specimen storage container capable of maintaining the temperature and moisture requirements for initial curing of Acceptance specimens. The distance from the location of concrete placement to the location of testing and initial curing shall be 100 feet or less, unless otherwise approved by the Engineer.

The specimen storage container described in this section is in addition to the concrete cylinder curing box provided for elsewhere in the Contract.

After initial curing, the test specimens will be transported by Department personnel and stored in the concrete cylinder curing box until they can be transported to the Division of Materials Testing for strength evaluation.

(c) Sampling Procedure for Pumping: It is the responsibility of the Contractor to provide concrete that meets specification at the point of placement.

Samples of concrete shall be taken at the discharge end of the pump at the point of placement with the exception of underwater concrete. The Contractor may submit an alternate location to provide a sample from the discharge end of the pump with verification showing that the characteristics of the mix will not be altered from that of which would have been attained at the point of placement. The Engineer will review the documentation and other extenuating circumstances when evaluating the request.

In the case of underwater concrete the Contractor shall submit the proposed sampling location with the submittals required in 6.01.03-II-6(f).

(d) Additional field testing: Additional field testing such as density and yield measurements may be required at the time of placement as determined by the Engineer.

5. Progression Cylinders and Compressive Strength Specimens: Progression Cylinders outlined in this section are field cured compressive strength specimens taken for information related to when a structure or segment of a structure can be loaded or put into service, adequacy of curing and protection of concrete in the structure, or when formwork or shoring may be removed from the structure. The information produced from strength results of Progression Cylinders will not be considered for acceptance of the concrete.

The personnel, equipment, and molds for sampling, casting, curing and testing of Progression Cylinders shall be furnished by the Contractor at no expense to the Department.

Sampling, casting, and field curing of the specimens shall be performed in accordance with AASHTO T23 by an ACI Concrete Field Testing Technician Grade 1 or higher and will be witnessed by a representative of the Department.

The sample shall be taken at the point of placement into the forms or molds from 1 or more of the same truck loads that an Acceptance sample is taken from.

A minimum of 2 of cylinder results will be used to determine in-place strength.

Compression testing shall be performed in accordance with AASHTO T 22 by personnel approved by the Engineer.

A Certified Test Report in accordance with 1.06.07 shall be provided to the Engineer reporting the Progression Cylinder test results. A copy of the results of the compressive strength testing shall be provided to the Engineer at least 24 hours prior to any Project activity that the results may control.

6. Handling and Placing Concrete: Concrete shall be handled, placed, and consolidated by methods acceptable to the Engineer that will not segregate the mix and shall result in a dense homogeneous concrete. The methods used shall not cause displacement of reinforcing steel or other materials to be embedded in the concrete. Concrete shall not be placed until the forms and all materials have been inspected by the Engineer. All mortar from previous placements, debris, and foreign material shall be removed from the forms and steel prior to commencing placement. The forms and subgrade shall be thoroughly moistened with water immediately before concrete is placed. All water that has ponded within the forms shall also be removed. Temporary form spreader devices shall not be left in place.

All laitance or unsound material shall be removed before placing substructure concrete onto the surface of any concrete placed underwater.

Placement of concrete for each section of the structure shall be performed continuously between construction or expansion joints as shown on the plans. The delivery rate, placing sequence and methods shall be such that fresh concrete is always placed and consolidated against previously placed concrete before initial set has occurred. The temperature of the concrete mixture during placement shall be maintained between 60ºF and 90ºF. During and after placement of concrete, care shall be taken not to damage the concrete or break the bond with reinforcing steel. Platforms for workers and equipment shall not be supported directly on any reinforcing steel. Forces that may damage the concrete shall not be applied to the forms or reinforcing steel.

(a) Sequence of Placement: The sequence of placement shall be in accordance with the Contract or as permitted by the Engineer.

Concrete for integral horizontal members, such as caps, slabs, or footings shall not be placed until the concrete for the columns, substructure, culvert walls and similar vertical members has achieved sufficient strength as stated in 6.01.03-II-1(m).

The concrete in arches shall be placed in such a manner as to load the formwork uniformly and symmetrically.

The base slab or footings of cast-in-place box culverts shall reach sufficient strength before the remainder of the culvert is constructed.

(b) Placement Methods: The Contractor shall notify the Engineer at least 24 hours in advance of intention to place concrete.

Vibrators shall not be used to shift the fresh concrete horizontally. Vibrators shall be adequate to consolidate the concrete and integrate it with the previous lift.

The rate of concrete placement must not produce loadings that exceed those considered in the design of the forms.

The use of chutes and pipes for conveying concrete into the forms must be reviewed by the Engineer. Chutes shall be clean, lined with smooth watertight material and, when steep slopes are involved, shall be equipped with baffles or reverses. When the discharge must be intermittent, a hopper or other device for regulating the discharge shall be provided.

Aluminum shall not be permanently incorporated into the concrete unless otherwise specified.

When placing operations involve dropping the concrete more than 5 feet, the Contractor shall take action to prevent segregation of the mix and spattering of mortar on steel and forms above the elevation of the lift being placed. This restriction shall not apply to cast-in-place pilings.

When using stay-in-place forms, concrete shall not be dropped more than 3 feet above the top of the forms, and the concrete shall be discharged directly over the beams or girders.

(c) Pumping: The Contractor shall use equipment specifically manufactured to pump concrete mixes and that meets the needs of the specific concrete placement.

(d) Consolidation: Unless otherwise specified, all concrete, except concrete placed under water, shall be sufficiently consolidated by mechanical vibration immediately after placement.

The Contractor shall provide a sufficient number of commercially available mechanical immersion type vibrators to properly consolidate the concrete immediately after it is placed in the forms unless external form vibrators are used. The Contractor shall have an adequate number of operable vibrators available in case of breakdown.

External form vibrators may be used if submitted prior to concrete placement and reviewed by the Engineer.

Vibration shall not be applied directly to the reinforcement or hardened concrete. Special care shall be taken in placing and consolidating concrete around ornamental moulds, form liners and other embedded items. The vibrator shall not touch these items at any time.

(e) Additional Requirements for Bridge Decks: At least 15 days before the erection of the screed rails, the Contractor shall submit screed erection plans, grades and sequence of concrete placement and proposed rate of placing concrete for review by the Engineer. These plans shall include details of equipment to be used in the placement and finishing of the concrete, including the number and type of personnel who will be engaged in placing the concrete. The screed equipment shall be a commercially available vibratory system. The use of wooden screeds is prohibited.

When setting screed rails for mechanical finishing, the Contractor shall take into consideration and make proper allowances for the deflection of the bridge superstructure due to all operations.

Screed and runway supports shall not be located on any stay-in-place metal form sheets, form supports or reinforcing steel. The Contractor shall operate the mechanical screed at least 24 hours prior to actual placement of the concrete to verify deck survey and equipment operations to the satisfaction of the Engineer.

A Pre-Placement Meeting shall be held on the project site with Contractor, Engineer and concrete supplier 48 hours before the concrete deck pour. The Pre-Placement Meeting will document and include discussion on the following topics:

1. **Schedule:**
   - **(a)** Deck pour sequence
   - **(b)** Daily start and finish times for concrete delivery

(c) Anticipated completion time

2. **Key Personnel**:
   - **(a)** Concrete placement foreman
   - **(b)** Total number of personnel involved in deck pour and their roles during the pour
   - **(c)** Concrete supplier
   - **(d)** Concrete pump truck operator/service
   - **(e)** Discuss QC/QA
3. **Placement:**
   - **(a)** List of approved delivery trucks per pour
   - **(b)** Pre-wetting forms prior to placement
   - **(c)** Placement sequence
   - **(d)** Rate of concrete placement and vibrator process
   - **(e)** Monitor concrete temperature during placement
   - **(f)** Transverse joint bulkheads
   - **(g)** Approved concrete low-permeability mix design
4. **Curing:**
   - **(a)** Curing materials (burlap, quilted blankets, etc.)
   - **(b)** Means for pre-soaking curing materials.
   - **(c)** Foggers
   - **(d)** Soaker hoses
   - **(e)** White Plastic Sheeting
   - **(f)** Water source and supply tanks

Concrete shall be deposited in a uniform manner across the entire width being placed, and only 2 passes of the transverse screed will be permitted over a given deck area, unless otherwise allowed by the Engineer.

If the Contractor proposes to place concrete outside of daylight hours, an adequate lighting system must be provided.

Concrete shall be deposited in accordance with the placement sequence as noted on the plans. If no sequence is indicated, the Contractor shall provide a placement sequence to the Engineer for review. The placement sequence shall proceed in such a manner that the total deflection or settlement of supporting members, and the final finishing of the surface will occur before the initial set of the concrete takes place.

At construction joints, concrete shall not be placed against the previously placed concrete for at least 12 hours unless otherwise allowed by the Engineer.

(f) Underwater Placement: Concrete may only be placed under water within a cofferdam unless otherwise specified in the Contract or allowed by the Engineer. Placement shall begin following inspection and acceptance of the depth and character of the foundation material by the Engineer.

Underwater concrete mixes are considered non-standard designs and shall be submitted to the Engineer for approval. Typically a minimum of 10% additional cement than comparable non-underwater mixes will be required.

Underwater concrete shall be placed continuously with the surface of the concrete kept as horizontal as practical. To ensure thorough bonding, each succeeding layer shall be placed before the preceding layer has taken initial set. For large concrete placements, more than 1 tremie or pump shall be used to ensure compliance with this requirement.

Mass concrete placement requirements, outlined in 6.01.03-II-6(g), do not apply to underwater concrete.

To prevent segregation, underwater concrete shall be placed in a compact mass, in its final position, by means of a tremie, concrete pump, or other approved method and shall not be disturbed. Still water shall be maintained at the point of deposit. Cofferdams shall be vented during the placement and curing of the concrete to equalize the hydrostatic pressure and thus prevent flow of water through the concrete.

If a tremie is used, the method of depositing the concrete shall be detailed in a submission to the Engineer as a working drawing for review. The tube shall have watertight couplings and shall permit the free movement of the discharge end over the area of the work.

(g) Mass concrete placement: Mass concrete placement shall be defined as any placement, excluding underwater concrete placement, in which the concrete being cast has dimensions of 5 feet or greater

in each of 3 different directions. For placements with a circular cross-section, a mass concrete placement shall be defined as any placement that has a diameter of 6 feet or greater and a height of 5 feet or greater. For all mass concrete placements, the mix temperature shall not exceed 85°F as measured at point of discharge into the forms.

Any special concrete mix design proposed by the Contractor to meet the above temperature requirements shall be submitted to the Engineer for review.

7. Finishing Plastic Concrete: Unless otherwise specified in the Contract, after concrete has been consolidated and prior to final curing, all surfaces of concrete that are not placed against forms shall be struck-off to the planned elevation or slope. The surface shall be finished by floating with an acceptable tool. While the concrete is still in a workable state, all construction and expansion joints shall be tooled with an edger. Joint filler shall be left exposed. For requirements on float finish, refer to 6.01.03-II-10, Finishing Concrete Surfaces.

After completion of the placing and finishing operation and for at least 12 hours after the concrete has set, the Contractor shall not operate any equipment in the immediate vicinity of the freshly placed concrete if, in the opinion of the Engineer, it could cause excessive vibration, movement or deflection of the forms.

The addition of water to the surface of the concrete to assist in finishing operations will not be permitted.

(a) Bridge Decks: After the concrete has been consolidated and brought to the proper elevation by the screed machine, it shall be finished by use of a suitable float. The Contractor shall not disturb the fresh concrete after it has been finished. All finishing work, including the application of the fog spray and placement of the curing mats, shall be performed from work bridges supported above the deck surface. A work bridge shall be made available to the Engineer for inspection of the concrete work.

Surfaces that are to be covered with a waterproofing membrane shall be finished to a smooth surface, free of mortar ridges and other projections and in accordance with the membrane manufacturer's recommendations.

Unless otherwise noted in the Contract, the concrete wearing surfaces shall be given a skid-resistant texture by dragging, brooming, tining, or by a combination of these methods These methods shall be done after floating and at such time and in such manner that the desired texture will be achieved while minimizing displacement of the larger aggregate particles.

1. Dragging: The surface shall be finished by dragging a seamless strip of damp burlap over the surface. The burlap to be dragged shall consist of sufficient layers and have sufficient length in contact with the concrete to slightly groove the surface. The burlap shall be drawn longitudinally along the surface in a slow manner so as to leave an even texture. The burlap shall be kept damp, clean, and free of particles of hardened concrete. The Contractor may propose an alternate material for the Engineer's consideration.
2. Tining: Tining shall be in a transverse direction using a wire broom, comb, or float having a single row of tines or fins. The tining grooves shall be between 1/16 inch and 3/16 inch wide and between 1/8 inch and 3/16 inch deep, spaced 1/2 inch to 3/4 inch on centers. Tining shall be discontinued 12 inches from the curb line on bridge decks. The area adjacent to the curbs shall be given a light broom finish longitudinally. As an alternative, tining may be achieved using a machine designed specifically for tining or grooving concrete pavements.

   The transverse grooving shall be performed when the grooves can be formed to a maximum depth of 3/16 inch with relative ease and without the walls of the grooves closing in on each other. The tining shall be aligned so as to prevent overlapping of grooves in any 2 successive transverse passes. The Contractor shall measure the depth of the grooves in the presence of the Engineer with an appropriate device to ensure compliance.

(b) Surface Testing and Correction: The completed surface shall be constructed in accordance with grades and cross slopes shown on the plans. The entire surface shall be checked by the Contractor in the presence of the Engineer, with an acceptable 10 foot straightedge.

1. The surface shall not vary more than +/- 1/8 inch over 10 feet for decks which will not be covered with an overlay.
2. The surface shall not vary more than +/- 1/4 inch over 10 feet for decks which will be covered with an overlay.

Variances greater than these, which, in the opinion of the Engineer, may adversely affect the riding qualities of the surface shall be corrected, and this shall be done at the expense of the Contractor. The Contractor shall submit a corrective procedure to the Engineer for review and approval. The

procedure shall correct such irregularities by methods such as, but not limited to, concrete planing or grooving.

8. Bearing Surfaces: Concrete surfaces under metallic masonry plates and elastomeric bearings shall have a float finish. After the concrete has set, the area which will be in contact with the masonry plate shall be ground as necessary to provide full and even bearing. The finished surface shall not vary from a straightedge laid on the surface in any direction within the limits of the masonry plate by more than 0.0625 inch. Surfaces which fail to conform shall be ground or filled until acceptable to the Engineer.

9. Curing Concrete: All newly placed concrete shall be cured so as to prevent loss of water by use of the methods specified. The Engineer may request that the Contractor furnish a curing plan.

The duration of the initial and final curing period in total shall continue uninterrupted for a minimum of 7 days.

(a) Curing Methods:

1. Forms-In-Place Method: Formed surfaces of concrete may be cured by retaining the forms in place without loosening. During periods of hot weather, water shall be applied to the forms until the Engineer determines that it is no longer required.
2. Water Method: Exposed concrete surfaces shall be kept continuously wet by ponding, spraying, or covering with materials that are kept continuously and thoroughly wet. Such materials may consist of cotton mats, multiple layers of burlap, or other approved materials that do not discolor or otherwise damage the concrete.
3. Waterproof Cover Method: This method shall consist of covering exposed surfaces with a waterproof sheet material to prevent moisture loss from the concrete. The concrete shall be wet at the time the cover is installed. The sheets shall be of the widest practicable width and adjacent sheets shall overlap a minimum of 6.0 inches to form a waterproof cover of the entire concrete surface and shall be adequately secured. Broken or damaged sheets shall be immediately repaired and the concrete shall be remoistened.

(b) Additional Requirements for Bridge Decks:

Curing Plan: The Contractor shall submit to the Engineer, at least 14 days prior to the placement of concrete for the bridge deck, a detailed curing plan that describes the following:

A. the initial and final curing durations,
B. equipment and materials to be used for curing concrete and monitoring concrete temperature,
C. and proposed primary and secondary water and heat sources

1. Initial Curing Period: A water fog spray shall be used by the Contractor from the time of initial placement until the final curing period begins. The amount of fog spray shall be strictly controlled so that accumulations of standing or flowing water on the surface of the concrete shall not occur.

   Should atmospheric conditions render the use of fog spray impractical, the Contractor shall request approval from the Engineer to use a curing compound that meets the requirements of M.03 in lieu of a fog spray. The application shall be in accordance with the manufacturer's recommendation and be compatible with the membrane waterproofing.
2. Final Curing: After completion of finishing and as soon as any bleed water has dissipated and the concrete reaches sufficient strength to avoid marring, the Final curing period shall begin and the entire concrete surface shall be covered with water-retaining materials such as cotton mats, multiple layers of burlap, or other materials approved by the Engineer. Materials used shall be kept saturated by means of an acceptable sprinkler or wetting system.

   The Contractor may cover the wet water-retaining material with a suitable polyethylene film to minimize evaporation during the curing period. The use of the polyethylene film does not relieve the Contractor from maintaining saturation of the curing materials.
3. Temperature Monitoring: The internal temperature of the concrete shall be monitored with a calibrated continuous recording thermometer for a minimum of 7 days. The air temperature at the concrete surface or the air temperature between the concrete surface and its protective covering shall be monitored with a minimum of 1 recording thermometer.

   The number and placement of the thermometers will be determined by the Engineer. A minimum of 2 thermometers per concrete placement shall be provided by the Contractor.

   The following types of thermometers shall be used to monitor curing temperatures:

   i) Continuously Recording Thermometer: The thermometer shall be capable of continuously recording temperatures within a range of -4ºF to 122ºF for a minimum of 24 hours.

ii) Maximum–Minimum Recording Thermometer: For all placements, the thermometer shall be capable of recording maximum and minimum temperatures in a range of -4ºF to 122ºF.

10. Finishing Concrete Surfaces: Any minor repairs due to fins, bulges, offsets and irregular projections shall be performed immediately following the removal of forms. For areas of newly placed concrete that are honeycombed or segregated the Contractor shall provide a written corrective procedure for review by the Engineer prior to the work being performed. Construction and expansion joints in the completed work shall be left carefully tooled and free of mortar and concrete. The joint filler shall be left exposed for its full length with clean and true edges.

The cavities produced by form ties and all other holes, broken corners or edges, and other defects shall be cleaned, saturated with water, pointed and trued with a mortar conforming to M.11.04. Cement similar in color to the exposed surface being repaired shall be added to the mortar. Mortar used in pointing shall be used within 1 hour of mixing. The concrete shall be finished as defined below if required and the cure continued as previously specified in 6.01.03-II-9, Curing Concrete.

Finishing work shall not interrupt the curing period unless permitted by the Engineer. The curing period may be extended to provide the minimum total number of days required.

Concrete surface finishes shall be classified as follows:

(a) Float Finish: This finish shall be achieved by placing an excess of material in the form and removing or striking off of such excess forcing the coarse aggregate below the mortar surface. Concave surfaces in which water will be retained will not be allowed. After the concrete has been struck off, the surface shall be thoroughly worked and floated. Before this last finish has set, the surface shall be lightly stripped with a fine brush to remove the surface cement film, leaving a fine-grained, smooth, but sanded texture. Curing, as specified elsewhere, shall follow. Any surfaces that will support appurtenances such as light standards, railing, or fences shall be finished in accordance with 6.01.03-II-8, Bearing Surfaces.

(b) Rubbed Finish: The initial rubbing shall only be allowed within 3 days after placement. The entire surface shall be thoroughly wet with a brush and rubbed with a No. 16 Carborundum Stone or an abrasive of equal quality, bringing the surface to a paste. The rubbing shall be continued sufficiently to remove all form marks and projections, producing a smooth, dense surface without pits or irregularities. The paste formed by the rubbing may be finished by stripping with a clean brush, or it may be spread uniformly over the surface and allowed to re-set. If all or portions of the rubbed surface are unacceptable to the Engineer or a rubbed finish is not provided within 3 days after removal of forms, the Contractor will be directed to provide a grout clean down finish.

(c) Grout Clean-Down Finish: As soon as all cavities have been filled as required elsewhere and the cement mortar has set sufficiently, grout clean-down shall be performed. All burrs, unevenness, laitance, including that in air holes, and any other material which will adversely affect the bond of the grout to the concrete, shall be removed by acceptable methods. This cleaning shall be done from the top or uppermost part of the surface to be finished to the bottom.

A mixture of a fine aggregate and Portland cement shall be thoroughly blended while dry. The proportions shall be such that when mixed with the proper amount of water, the color will match that of the concrete to be finished. Water shall be added to this mixture in an amount which will bring the grout to a workable thick paint-like consistency.

The surface to be treated shall be thoroughly wetted with a sufficient amount of water to prevent the absorption of water from the grout. Grout shall then be applied to the wetted surface before setting of the grout occurs. Grout which has set shall not be re-tempered and shall be disposed of by the Contractor at no cost to the State.

The grout shall be uniformly applied over the entire surface, completely filling all air bubbles and holes. Immediately after applying the grout, the surface shall be floated with a suitable float, scouring the surface vigorously. While the grout is still plastic, all excess grout shall be removed.

After the final rubbing is completed and the surface has dried, it shall be rubbed to remove loose powder and shall be left free from all unsound patches, paste, powder, and objectionable marks. Wetting, application and removal of excess grout shall be completed in 1 work shift.

All finished surfaces shall be cured for a minimum of 24 hours. Horizontal surfaces shall have a float finish and vertical exposed surfaces shall have a rubbed finish. A grout clean down finish may be substituted for a rubbed finish as noted in this section or as directed by the Engineer.

11. Mortar, Grout, Epoxy and Joint Seal:

(a) Mortar and Grout: This work consists of the making and placing of mortar and grout. At least 48 hours prior to the planned use, a copy of the installation instructions and MSDS sheets shall be provided to the Engineer for review and concurrence of their applicability and for verification of proper hole sizes in concrete structures. Such uses include mortar for filling under masonry plates, mortar used to fill voids and repair surface defects, grout used to fill sleeves for anchor bolts, and mortar and grout for other such uses where required or approved.

Concrete areas to be in contact with the mortar or grout shall be cleaned of all loose or foreign material that would in any way prevent bond, and the concrete surfaces shall be flushed with water and allowed to dry until no free-standing water is present.

The mortar or grout shall completely fill and shall be tightly packed into recesses and holes, on surfaces, under structural members, and at other locations specified. After placing, all surfaces of mortar or grout shall be cured as previously specified in 6.01.03-II-9(a)-2, for a period of not less than 3 days.

(b) Epoxy: The epoxy shall be prepared and placed in accordance with the manufacturer's directions and with the equipment prescribed by the manufacturer. Instructions furnished by the supplier for the safe storage, mixing, handling and application of the epoxy shall be followed. Contents of damaged or previously opened containers shall not be used.

(c) Joint Seal: This work consists of sealing joints where shown on the plans or as otherwise directed by the Engineer.

Before placement of the sealing material, the joints shall be thoroughly cleaned of all scale, loose concrete, dirt, dust or other foreign matter. Projections of concrete into the joint space shall be removed. The joint shall be clean and dry before the sealing compound is applied.

The joint sealant shall be prepared and placed in accordance with the manufacturer's directions and with the equipment prescribed by the manufacturer. The sealing compound shall be flush with, or not more than 1/8 inch above the adjacent surface of concrete, cutting off all excess compounds after the application. The joints shall be sealed in a neat and workmanlike manner and when the work is completed, the joints shall effectively seal against infiltration of moisture and water.

The Contractor shall arrange for, and have present at the commencement of the joint-sealing operation, a technically competent manufacturer's representative knowledgeable in the methods of installation of the sealant. The Contractor shall also arrange to have the representative present at such other times as the Engineer may request.

(d) Closed Cell Elastomer: The closed cell elastomer shall be of the thickness specified and installed as shown on the plans and shall be in accordance with M.03.08-6.

12. Application of Loads: Loads shall not be applied to concrete structures until the concrete has attained sufficient strength and, when applicable, sufficient pre-stressing and post tensioning has been completed, so that damage will not occur. The means to determine when the concrete has attained sufficient strength shall be the use of Progression cylinders as defined elsewhere in this specification, or other means approved in advance by the Engineer.

(a) Earth Loads: The placement of backfill shall not begin until the concrete is cured and has reached at least 80% of its specified strength unless otherwise permitted by the Engineer. The sequence of placing backfill around structures shall minimize overturning or sliding forces and flexural stresses in the concrete.

(b) Construction Loads: Light materials and equipment may be hand carried onto bridge decks only after the concrete has been in place at least 24 hours providing curing is not interfered with and the surface texture is not damaged.

Prior to the concrete achieving its specified compressive strength, any other live or dead loads imposed on existing, new, or partially completed portions of structures, shall not exceed the reduced load carrying capacity of the structure, or portion of structure. The Contractor may be required to submit calculations to the Engineer that verify these requirements are being met. The compressive strength of concrete (f'_c) to be used in computing the load-carrying capacity shall be the smaller of the actual field compressive strength at the time of loading or the specified design strength of the concrete. The means to determine the actual field compressive strength shall be approved by the Engineer.

For post-tensioned structures, no live or dead loads shall be allowed on any span until the steel for that span has been tensioned.

(c) Precast concrete or steel girders shall not be placed on substructure elements until the substructure concrete has attained 85% of its specified strength.

No load shall be allowed on mortar or grout that has been in place less than 72 hours.

(d) Traffic Loads: The concrete deck will not be opened to traffic until at least 14 days after the last placement of deck concrete and until such concrete has attained its specified strength.

13. Dispute Resolution: The basis of any dispute resolution is side-by-side and quality control testing by the Contractor or the Contractor's representative. The Contractor and Engineer should perform independent testing on the material to reasonably establish the true characteristics of the material at the time of delivery. Absent of Contractor QC testing, the Engineer's test results will apply to the quantity of concrete represented by the sample, not to exceed 75 c.y.

Air Content: Contractor QC Testing must be performed by personnel qualified by The American Concrete Institute as an ACI Concrete Field Testing Technician Grade 1 or higher and performed in accordance with AASHTO T-23. If the Contractor's test results vary from those of the Engineer, the Contractor shall immediately notify the Engineer of the difference and work cooperatively to determine the reasonable cause and recognize the valid test. Should there be agreement, the result of the valid test will be used for acceptance and adjustment purposes for that lot of material. Should there not be an agreement as to the valid test, an additional set of tests should be performed. Results of all valid tests on the same lot may be averaged and used for acceptance and adjustment purposes. Should the Contractor wish to perform additional QC testing on subsequent material, the lot sizes may be adjusted to the amount of material included in that specific delivery. Any such QC testing must be witnessed and agreed to by the Engineer.

Compressive Strength: Contractor QC testing for compressive strength must be performed in accordance with AASHTO T-22 by personnel approved by the Engineer. Samples used to dispute the Engineer's test results must be made simultaneously and from the same batch of concrete. Should the Contractor wish to pursue a dispute resolution with regard to compressive strength, the Contractor shall submit in writing to the Engineer all test results, control charts, or other documentation that may be useful in determining if the specific lot(s) of material met the Contract specifications. The Engineer will consider the submittal and may average specific test results on the disputed lot(s) for acceptance and adjustment purposes. Destructive testing of any kind on the placed concrete structure will not be allowed.

III. Additional Requirements for Surface Repairs and Structural Repairs

1. Work Area Access and Shielding: Prior to removal of existing concrete, the Contractor shall provide access to the anticipated work areas so that the inspector and the Contractor may together determine and delineate the exact limits and locations of the work.

The Contractor shall design, furnish, install and remove a shield(s) to prevent debris from entering areas adjacent or beneath the work. The Contractor shall submit working drawings to the Engineer in accordance with 1.05.02. The shield(s) shall be maintained by the Contractor and remain in place during all phases of the repair work.

2. Concrete Removal: The perimeter of each area to be repaired shall be saw cut as shown on the plans. All concrete within that area shall be removed to at least 1 inch beneath any visible reinforcing steel and to sound concrete. The reinforcing steel shall not be damaged or its bond in the surrounding concrete. The Contractor must use fifteen (15) pound hammers or other methods accepted by the Engineer.

In addition to removal of concrete to a depth of 1 inch below reinforcing steel, localized areas of removal may be required if embedded galvanic anodes are specified in the Contract, to allow a minimum of 2 inches of concrete cover over the anodes.

Any steel reinforcing scheduled to be left in place that is damaged during the concrete removal process shall be replaced in accordance with 6.02 to the satisfaction of the Engineer and at the expense of the Contractor.

Corroded, missing, or broken reinforcing steel shall be replaced in accordance with 6.02 and as shown on the plans or as directed by the Engineer.

The Contractor shall perform the work in a manner that prevents debris from entering roadway lanes or areas below the structure. All debris shall be removed from the Site and disposed of by the Contractor.

3. Surface Preparation: All newly exposed surfaces of concrete shall be sandblasted and be visibly free from oil, solvent, grease, loose particles, or any other foreign matter. Exposed reinforcing steel shall be sandblasted in accordance with SSPC-SP-6, Commercial Blast Cleaning, to remove all contaminants, rust and rust scale.

4. Installation of Embedded Galvanic Anodes: After sandblasting reinforcing steel, galvanic anodes shall be embedded where shown on the plans and in accordance with the Contract.

5. Welded Wire Fabric in Vertical and Overhead Surface Repairs: Prior to installing formwork, steel welded wire fabric meeting the requirements of M.06.01-3 shall be installed at the proper depth in those areas as shown on the plans or directed by the Engineer. The fabric shall be tied to exposed reinforcing steel or anchored to sound concrete using means approved by the Engineer.

6. Formwork: Forms and support systems shall be designed in accordance with 6.01.03-II-1. Forms shall be so designed so that access is from the top of the formwork. If access is not possible from the top of the formwork, the Contractor shall submit a method of concrete placement for review by the Engineer.

7. Concrete Placement and Curing: Bonding compounds shall not be used before or during the placement of the concrete. Exposed surfaces shall be wetted with water immediately prior to placement. There shall be no excessive water on the surface or in the formwork. Light rust on sandblasted reinforcing steel can be anticipated and is acceptable.

The temperature of the air and surface to be repaired at the time of placement and curing shall be a minimum of 45°F. Concrete shall be placed and consolidated immediately with appropriate vibratory equipment.

Forms shall be kept moist and shall be left in place for a minimum of 7 days or as shown on the plans.

8. Form Removal and Sequence of Repair: Form removal shall be in accordance with 6.01.03-II-1(m) unless otherwise noted on the plans. The Contractor shall follow the sequence of repairs shown on the plans.

9. Finishing: Immediately following curing and form stripping, the exposed faces shall be finished in accordance with Subarticle 6.01.03-II-10(c) Grout Clean-Down Finish.

10. Sounding of Completed Repairs: Cured and finished areas may be sounded by the Engineer to detect the presence of subsurface voids or delamination. Such areas shall be removed and replaced by the Contractor at its expense until an acceptable repair is in place as determined by the Engineer.

11. Sealing Concrete Surfaces: After all repairs have been accepted, penetrating sealer shall be applied in accordance with the Contract to the repaired areas as well as all contiguous areas to the repair or as directed by the Engineer.

6.01.04—Method of Measurement: This work will be measured for payment as follows:

1. Concrete used for new construction: The quantity of concrete used for new construction will be the actual volume in cubic yards of the specified class, with the exception of underwater concrete, completed and accepted within the neat lines as shown on the plans or as ordered by the Engineer. Parapets will be measured for payment by the number of linear feet of parapet, completed and accepted. The length of parapet will be measured along the centerline of the top of the parapet.

When concrete is placed against bedrock, a maximum of 6 additional inches beyond the neat lines can be measured for payment.

No deduction will be made for panels, form liners, reinforcing bars, structural steel shapes or for pile heads. There will be no deduction made for the volume occupied by culvert and drainage pipes, scuppers, weep holes, public utility structures or any other opening, unless the surface area of any such single opening is 9 s.f. or more.

In the case of culverts or drainage pipes, the computation of the surface area will be based on the nominal diameter of the pipe, disregarding the thickness of the shell.

Miscellaneous materials necessary for completion of the work such as felt, mortar, grout, epoxy and joint seal will not be measured for payment.

Incidental work such as forming for anchor bolts, utilities, keyways, and sampling and testing will not be measured for payment.

The work to produce and administer the Concrete Quality Control Plan (CQCP) will not be measured for payment.

2. Underwater Concrete: When underwater concrete is used, it will be measured by the volume in cubic yards within the actual horizontal limits of the cofferdam and between the elevations established by the Engineer.

3. Concrete used for Surface or Structural Repairs: The quantity of concrete used for surface repairs or structural repairs will be the actual volume completed and accepted. Welded wire fabric used in repair areas will not be measured for payment.

4. Joint Filler: This material will be measured by the area in square feet of the joint filler, of the type and thickness specified, installed and accepted.

5. Closed Cell Elastomer: This material will be measured by the volume in cubic inches of elastomer, of the thickness specified, installed and accepted.

6.01.05—Basis of Payment: Payment for this work will be made as follows:

1. Concrete: Progress payments may be allowed for completed major labor elements of work such as forming, placing and curing. Prior to placement, the Contractor shall submit a proposed schedule of values for review and approval by the Engineer.

Payment for any lot of concrete allowed to remain in place will be adjusted when the field and laboratory testing of the material is completed. The quantity of concrete in each lot for new construction will be a maximum of 75 c.y. Payment for each lot of concrete will be adjusted based on the results of the acceptance testing performed by the Engineer.

The pay factors listed in Table 6.01.05-1 apply for Standard and Modified Standard Mix classes with regard to entrained air content.

Table 6.01.05-1 Entrained Air Content Pay Factors

Specified Entrained air (%)*				**Pay factor (%)**
6.0 +/- 1.5%		7.5 +/- 1.5%		1.00 (100)
4.3 and 4.4	7.6 and 7.7	5.8 and 5.9	9.1 and 9.2	0.98 (98)
4.1 and 4.2	7.8 and 7.9	5.6 and 5.7	9.3 and 9.4	0.96 (96)
3.9 and 4.0	8.0and 8.1	5.4 and 5.5	9.5 and 9.6	0.94 (94)
3.7 and 3.8	8.2 and 8.3	5.2 and 5.3	9.7 and 9.8	0.92 (92)
3.5 and 3.6	8.4 and 8.5	5.0 and 5.1	9.9 and 10.0	0.90 (90)
Concrete lots with less than 3.5% or greater than 8.5% entrained air will be rejected.		Concrete lots with less than 5.0% or greater than 10% entrained air will be rejected.		
***Air content measured at time and point of placement**				

The pay factors listed in Table 6.01.05-2a apply for Standard and Modified Standard Mix classes with regard to compressive strength.

Table 6.01.05-2a Compressive Strength Pay Factors

Compressive Strength (%)	**Pay factor (%)**
95 or greater	1.00 (100)
90 to 94.9	0.95 (95)
85 to 89.9	0.90 (90)
*Measured at 28 days	
Concrete lots with less than 85% specified strength will be rejected.	

The pay factors listed in Table 6.01.05-2b apply for Standard and Modified Standard Mix classes with regard to surface resistivity when specified in accordance with AASHTO T 358 using 4 inch × 8-inch cylinders.

Table 6.01.05-2b Permeability Pay Factors

Surface Resistivity (kΩ-cm)*	**Pay factor (%)**
37 or greater	1.1 (110)
29 to 36.9	1 (100)
25 to 28.9	0.85 (85)
21 to 24.9	0.75 (75)
***Measured at 56 days**	
Concrete lots with resistivity values less than 21 will be rejected.	

The payment adjustment value for entrained air, 28-day strength, and permeability if applicable, for any lot of concrete for new construction that is allowed to remain in-place is determined using the formulas listed in Table 6.01.05-3a. An Index Price of $400.00 per c.y. will be used to calculate each adjustment, except for Parapet Concrete, for which an Index Price of $100 per l.f. will be used. The sum of the

individual adjustment values will be deducted from the cubic yard or linear foot payment for the appropriate item.

Table 6.01.05-3a Payment Adjustment Formulas for New Construction

Adj (air) = (1 - air pay factor) × Index Price × lot size (c.y. or l.f.)
Adj (strength) = (1 - strength pay factor) × Index Price × lot size (c.y. or l.f.)
Adj (permeability) = (1 – permeability pay factor) × Index Price × lot size (c.y. or l.f.)
Total Adjustment = Adj (air) + Adj (strength) + Adj (permeability)

The payment adjustment value for entrained air and 28-day strength for any lot of repair concrete that is allowed to remain in-place is determined using the formulas listed in Table 6.01.05-3b. An index price of $200.00 per c.f. shall be used to calculate each adjustment. The total adjustment value will be the sum of each individual adjustment value and will be deducted from the cubic foot payment for the appropriate item.

Table 6.01.05-3b Payment Adjustment Formulas for Repair Concrete

Adj (air) = (1 - air pay factor) × $200/c.f. × lot size (c.f.)
Adj (strength) = (1 - strength pay factor) × $200/c.f. × lot size (c.f.)
Total Adj = Adj (air) + Adj (strength)

The Contractor shall request permission from the Engineer to remove and replace a lot(s) of concrete to avoid a negative payment adjustment. Any replacement material will be sampled, tested and evaluated in accordance with this specification.

No direct payment will be made for any labor, equipment or materials used during the sampling and testing of the concrete for Progression or Acceptance. The cost shall be considered as included in the general cost of the work or as stated elsewhere in the Contract. The work of transporting the concrete test specimens, after initial curing, for Acceptance testing will be performed by the Department without expense to the Contractor.

This material used for new construction will be paid for at the Contract unit price per cubic yard or linear foot less any adjustments, for the specified class, complete in place, which price shall include all materials, equipment, tools, labor and work incidental thereto, including Concrete Quality Control Plan, heating, all admixtures, joint sealer, roofing felt, and any miscellaneous materials such as metal flashing and metal used in expansion joints and bearings.

2. Underwater Concrete: When this class of concrete is used, it will be paid for at the Contract unit price per cubic yard for "Underwater Concrete," complete in place, which price shall include all materials, equipment, tools, labor and work incidental thereto.

3. Concrete Used For Structural Repairs or Surface Repairs: The material used for structural repairs or surface repairs will be paid for at the Contract unit price per cubic foot less any adjustments, complete in place, which price shall include saw cutting, removing concrete, sandblasting, cleaning, forming, placing, curing, stripping, and finishing new surfaces, and all materials, equipment, tools, labor and clean-up incidental thereto.

4. Joint Filler: Expansion joint filler will be paid for at the Contract unit price per square foot for "Joint Filler for Bridges" of the type and thickness specified, complete in place, which price shall include all materials, equipment, tools, labor and work incidental thereto.

5. Closed Cell Elastomer: Closed cell elastomer will be paid for at the Contract unit price per cubic inch for “Closed Cell Elastomer” of the thickness specified, complete in place, which price shall include all materials, equipment, tools, labor and work incidental thereto.

Embedded galvanic anodes, deformed steel bars, and penetrating sealer, will be paid for separately.

Pay Item	Pay Unit
Footing Concrete	c.y.
Footing Concrete (Mass)	c.y.
Abutment and Wall Concrete	c.y.

Abutment and Wall Concrete (Mass)	c.y.
Column and Cap Concrete	c.y.
Column and Cap Concrete (Mass)	c.y.
Bridge Deck Concrete	c.y.
Bridge Deck Concrete (SIP Forms)	c.y.
Parapet Concrete	l.f.
Bridge Sidewalk Concrete	c.y.
Approach Slab Concrete	c.y.
Barrier Wall Concrete	c.y.
Underwater Concrete	c.y.
Surface Repair Concrete	c.f.
Structural Repair Concrete	c.f.
Class PCCXXXYZ	c.y.
(Thickness and Type) Joint Filler for Bridges	s.f.
(Thickness) Closed Cell Elastomer	c.i.

SECTION 6.02
REINFORCING STEEL

6.02.01—Description: Work under this item shall consist of furnishing and placing reinforcing steel and splicing materials, of the type and size designated, as shown on the plans, as directed by the Engineer and in accordance with these specifications.

6.02.02—Materials: The materials for this work shall meet the requirements of M.06.01.

6.02.03—Construction Methods:

1. Shop Drawings: Prior to fabricating any materials, the Contractor shall submit shop drawings of the reinforcing steel and splicing materials, with material lists, material designations, placement diagrams, bending diagrams and manufacturer's literature for mechanical connections, for review and approval, in accordance with 1.05.02. Any expenses incidental to the revision of materials furnished in accordance with shop drawing and order lists to make them comply with the requirements of the plans, specifications or special provisions shall be borne by the Contractor.

2. Fabrication:

(a) Cutting and Bending: Bar reinforcement shall be cut and bent to the shapes shown on the plans. Fabrication tolerances shall be in accordance with the requirements of ACI 315. All bars shall be bent cold, unless otherwise permitted.

Coated bars shall not be field cut, unless permitted by the Engineer. If allowed, field cutting of coated bars should be performed using hydraulic-powered cutters or friction cutting tools to minimize coating damage and field touch-up. Flame cutting of coated bars will not be permitted. Field cut coated bars shall be repaired immediately.

Bars partially embedded in concrete shall not be field bent, except as shown on the plans or permitted by the Engineer.

(b) Hooks and Bend Dimensions: The dimensions of hooks and the diameters of bends measured on the inside of the bar shall be as shown on the plans. When the dimensions of hooks or the diameter of bends are not shown, they shall be in accordance with the ACI 318, "Building Code Requirements for Reinforced Concrete" as amended by ASTM A767 for galvanized bars.

(c) Identification: Bar reinforcement shall be shipped in standard bundles, tagged and marked in accordance with the CRSI "Manual of Standard Practice."

3. Handling, Storing and Surface Condition of Reinforcement: Steel reinforcement shall be stored above the surface of the ground on platforms, skids, or other supports and shall be protected as far as practical from mechanical injury and surface deterioration caused by exposure to conditions producing rust.

Epoxy-coated and galvanized reinforcing steel shall be handled and stored by methods that will not damage the coating. All systems for handling coated reinforcement shall have adequately padded contact areas wherever possible. All bundling bands shall be padded and all bundles shall be lifted with a strong back, multiple supports, or platform bridge so as to prevent bar-to-bar abrasion from sags in the bar bundle. Bars or bundles shall not be dropped or dragged. Coated reinforcing steel shall be transported and stored on wooden or padded supports. Epoxy-coated reinforcing steel, stored at the job site, shall be protected by covering with opaque polyethylene or other suitable protective material. Provisions shall be made for adequate ventilation to prevent condensation under the covering. Since the epoxy coating is flammable, the epoxy coated reinforcing shall not be exposed to any fire or flame.

Prior to placement of concrete, all reinforcement shall be free from dirt, loose rust or scale, mortar, paint, grease, oil, or other materials that would reduce bond. Reinforcement shall be free from injurious defects such as cracks and laminations. Bonded rust, surface seams, surface irregularities, or mill scale will not be cause for rejection, provided the minimum dimensions, cross section area, and tensile properties of a hand wire brushed specimen meet the physical requirements for the size and grade of steel specified.

4. Placing and Fastening

(a) General: Steel reinforcement shall be accurately placed as shown on the plans and firmly held in position during the placing and setting of concrete. Bars shall be tied at all intersections except where the spacing is less than 12 inches between intersections shall be tied. Bars shall be tied at all intersections around the perimeter of each mat.

Bundled bars shall be tied together at not more than 6-foot centers. Lap splices shall have a minimum of 2 ties or be tied 12 inches apart for the length of the splice, whichever requires the greater number of ties. For epoxy-coated reinforcement, tie wires and metal clips shall be epoxy, plastic or nylon coated. For galvanized reinforcement, tie wires and metal clips shall be plastic coated or galvanized.

With the exception of tie down bars, welding (tack welding) will not be permitted for assembly of reinforcement, unless shown on the plans. Tie down bars shall be placed as shown on the plans and a top longitudinal reinforcing bar tied to these bars. When welding coated bars an appropriate protective mask must be worn, safety equipment used and suitable ventilation provided.

If wire fabric reinforcement is shipped in rolls, it shall be straightened into flat sheets before being placed.

(b) Support Systems: Reinforcing steel shall be supported in its proper position by use of precast mortar blocks, wire bar supports, supplementary bars (tie-down bars), side form spacers or other approved devices. Such devices shall be sufficiently strong and properly placed at frequent intervals so as to maintain the cover between the reinforcing and the surface of the concrete.

The reinforcing steel cover shall be no less than that shown on the plans and no greater than that shown plus 1/4 inch.

Platforms for the support of workers and equipment during concrete placement shall be supported directly on the forms and not on the reinforcing steel.

(c) Precast Mortar Blocks: Precast mortar blocks shall have a compressive strength not less than that of the concrete in which they are to be embedded. The face of the blocks in contact with forms for exposed surfaces shall not exceed 2 inches × 2 inches size and shall have a color and texture that will match the concrete surface. Precast mortar blocks shall not be used on exposed surfaces of precast concrete members. When used on vertical or sloping surfaces, such blocks shall have an embedded wire for securing the block to the reinforcing. When used in slabs, either such a tie wire or, when the weight of the reinforcing is sufficient to firmly hold the blocks in place, a groove in the top of the block may be used. For epoxy-coated bars, such tie wires shall be epoxy, plastic or nylon coated. For galvanized bars, such tie wires shall be plastic coated or galvanized.

(d) Wire Supports: Wire bar supports, such as ferrous metal chairs and bolsters, shall conform to industry practice as described in the CRSI "Manual of Standard Practice of the Concrete Reinforcing Steel Institute." All bolsters or chairs which bear against the forms for exposed surfaces shall be equipped with snug fitting, high density, polyethylene tips which provide 1/2-inch minimum clearance between the metal and any exposed surface. For epoxy-coated reinforcement, all wire bar supports and bar clips shall be epoxy or plastic coated. For galvanized reinforcement, chair and bar supports shall be hot-dip galvanized, after fabrication, in accordance with ASTM A123.

The maximum spacing of slab bolster rows and high chair rows for concrete deck slabs shall be 4 feet unless otherwise directed by the Engineer.

(e) Repair of Coated Reinforcing Steel:

Epoxy-coated Reinforcing Steel - In addition to the requirements of ASTM D3963, all damage (i.e., scratches, nicks, cracks) to the epoxy coating of the bar reinforcement, visible to the unaided eye with corrective vision, caused during shipment, storage or placement shall be repaired by the Contractor at the Project Site with approved patching material. Ends of bars that have been sheared, saw cut or cut by other means shall be coated with approved patching material. The areas on the bars and tie down bars damaged by welding shall be repaired with approved patching material.

Patching of damaged areas shall be performed in accordance with the patching material manufacturer's recommendations. Any singular damaged surface area (prior to repair with approved patching material), shall not exceed 2% of the total surface area of the bar. The total bar surface area covered by patching material shall not exceed 5% of the total surface area of the bar. Should either of these limits be exceeded the bar shall be removed from the work and replaced with an acceptable bar. All patching material shall be fully cured prior to placing concrete.

The patching material shall be compatible with the epoxy coating, inert in concrete, and suitable for repairs in the field. The patching material shall be prequalified, as required for the coating material, and shall be either identified on the container as meeting the requirements of Annex A1 of ASTM D3963 or shall be accompanied by a Materials Certificate certifying that the material meets the requirements of said Annex A1.

Galvanized Reinforcing Steel - All damage (i.e. scratches, nicks, cracks) to the galvanized coating on bar reinforcement, visible to the unaided eye with corrective vision, caused during shipment, storage or placement shall be repaired by the Contractor at the Project Site in accordance with ASTM A780, Annex A2 - "Repair using Zinc-Rich Paints." Ends of bars that have been sheared, saw cut or cut by other means shall be coated with zinc-rich paint. The area on the bars and tie down bars damaged by welding shall be repaired with zinc-rich paint.

Field coating of damaged areas shall be performed in accordance with the zinc-rich paint manufacturer's

recommendations. The zinc-rich paint shall conform to FS TT-P-641, Type 1 and shall be brush applied to achieve a dry film thickness from 3 - 6 mils. All touchup paint shall be fully cured prior to placing concrete.

5. Splicing of Bars:

(a) General: All reinforcement shall be furnished in the full lengths indicated on the plans unless otherwise permitted. Except for splices shown on the plans, splicing of bars will not be permitted without written approval of the Engineer. Splices shall be staggered as far as possible.

(b) Lapped Splices: Lapped splices shall be of the lengths shown on the plans.

In contact lap splices, the bars shall be placed in contact and tied together in such a manner as to maintain the minimum distance to the surface of the concrete shown on the plans.

In non-contact lap splices, the bars shall be placed as shown on the plans and tied to adjacent bars in such a manner as to maintain the minimum distance to the surface of the concrete shown on the plans.

(c) Welded Splices: Welded splices shall be used at the locations shown on the plans. Welding shall conform to AWS publication "Structural Welding Code, Reinforcing Steel, AWS D1.4" and applicable special provisions.

Welded splices shall not be used on epoxy-coated or galvanized bars. No welding shall be performed close enough to epoxy-coated or galvanized bars to cause any heating of the coating.

(d) Splices made with Dowel Bar Mechanical Connections: Splices made with dowel bar mechanical connections shall be used at the locations shown on the plans. The minimum size of the bars and the length of the lap splices for the dowel bar mechanical connections shall be as shown on the plans.

The mechanical connections shall be installed in accordance with the manufacturer's recommendations. All tools and equipment required to install and field inspect the connections shall be provided by the Contractor. The Contractor shall take all measures necessary to prevent concrete from adhering to the threaded portions of the mechanical connections.

After installing the coated mechanical connectors, all damaged areas on the coated connectors shall be repaired in accordance with 6.02.03-4(e).

6. Splicing of Welded Wire Fabric: Welded wire fabric reinforcement shall be lap spliced as shown on the plans.

7. Substitutions: Substitution of different size bars will be permitted only when authorized by the Engineer. The substituted bars shall have an area equivalent to or larger than the area shown on the plans.

8. Inspection: Reinforcement in any member or component shall be placed, inspected and approved by the Engineer before placing of concrete begins. Concrete placed prior to approval of the reinforcement may be rejected and its removal required.

6.02.04—Method of Measurement:

1. General: No measurement will be made for payment for any clips, wire, separators, wire chairs, precast mortar blocks and other material used for fastening and supporting the reinforcement in the correct position.

2. Bar Reinforcement: Uncoated, epoxy coated, galvanized and weldable bar reinforcement will be classified as "Deformed Steel Bars," "Deformed Steel Bars - Epoxy Coated," "Deformed Steel Bars - Galvanized" and "Deformed Steel Bars - Weldable" respectively.

This work will be measured for payment by the number of pounds of bar reinforcement installed and accepted.

The weight of reinforcing steel will be computed using the values tabulated in M.06.01-2. No allowance will be made for the weight of the epoxy or galvanized coatings.

Tie down bars will not be measured for payment.

In case short bars are used when full length bars might reasonably be required, only the amount which would be obtained if full length bars were used will be measured for payment. No allowance will be made for lap splices not contemplated by the plans unless approved by the Engineer.

If bars are substituted upon the Contractor's request and as a result more reinforcing steel is used than specified, only the amount specified will be included.

3. Welded Wire Fabric: This work will be measured for payment by the number of pounds of welded wire fabric installed and accepted.

The weight of welded wire fabric will be computed from the values published in the CRSI "Manual of Standard Practice."

4. Dowel Bar Mechanical Connections: Uncoated, epoxy coated and galvanized dowel bar

mechanical connections will be classified as "Dowel Bar Splicer System," "Dowel Bar Splicer System - Epoxy Coated" and "Dowel Bar Splicer System - Galvanized" respectively.

This work will be measured for payment by the number of dowel bar mechanical connections installed and accepted.

6.02.05—Basis of Payment: Payment for this work will be made as follows:

1. **Bar Reinforcement**: This work will be paid for at the Contract unit price per pound for "Deformed Steel Bars," "Deformed Steel Bars - Epoxy Coated" or "Deformed Steel Bars - Galvanized" and "Deformed Steel Bars - Weldable" complete in place and accepted, including shop drawings, furnishing, fabricating and placing reinforcing steel, welding splices and all materials, equipment, tools, labor and work incidental thereto.
2. **Welded Wire Fabric**: This work will be paid for at the Contract unit price per pound for "Welded Wire Fabric," complete in place and accepted, including shop drawings, furnishing, fabricating and placing welded wire fabric and all materials, equipment, tools, labor and work incidental thereto.
3. **Dowel Bar Mechanical Connections**: This work will be paid for at the Contract unit price each for "Dowel Bar Splicer System," "Dowel Bar Splicer System - Epoxy Coated" and "Dowel Bar Splicer System - Galvanized" complete in place and accepted, including shop drawings, furnishing, fabricating and placing dowel bar mechanical connections and all materials, equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
Deformed Steel Bars	lb.
Deformed Steel Bars - Epoxy Coated	lb.
Deformed Steel Bars – Galvanized	lb.
Deformed Steel Bars - Weldable	lb.
Welded Wire Fabric	lb.
Dowel Bar Splicer System	ea.
Dowel Bar Splicer System - Epoxy Coated	ea.
Dowel Bar Splicer System - Galvanized	ea.

SECTION 6.03
STRUCTURAL STEEL

6.03.01—Description: Work under this item shall consist of furnishing, fabricating, transporting, storing, handling and erecting of both permanent and temporary structural steel of the type and size designated, as shown on the plans or working drawings, as directed by the Engineer and in accordance with these specifications.

All work, except as stated in the following paragraph, shall meet the requirements of the AASHTO LRFD Bridge Construction Specifications and the ANSI/AASHTO/AWS D1.5 – Bridge Welding Code.

All work subject to railroad loading shall meet the requirements of AREMA and the ANSI/AASHTO/AWS D1.5 – Bridge Welding Code.

6.03.02—Materials: The materials for this work shall meet the requirements of M.06.

Material for this work shall be stored off the ground before, during, and after fabrication. It shall be kept free from dirt, grease and other contaminants and shall be reasonably protected from corrosion. In addition, weathering steel shall be stored as to allow free drainage and promote the development of the oxide coating and a uniform appearance.

6.03.03—Construction Methods:

1. Shop Prequalification:

(a) Fabricators producing material for Department projects shall be qualified in accordance with 1.05.18.

(b) **Shop Welders:** Prior to performing any welding on Department projects under this specification, all welders shall be qualified to weld in accordance with the welding code(s) specified on the approved drawings or otherwise stated in the Contract.

2. Field Prequalification - Field Welders: Field welders shall be certified in accordance with 1.05.17.

3. Submittals:

(a) **Shop Drawings:** Prior to any fabrication, the Contractor shall submit shop drawings in accordance with 1.05.02 to the Engineer for review. Shop drawings shall include a cambering procedure and diagram. In the case of trusses, the Contractor is responsible for calculation of the camber (lengthening and shortening) of all truss members.

(b) **Shop Schedule:** The Contractor shall submit a detailed shop fabrication schedule to the Engineer and the DMT for review within 30 days of the Notice to Proceed unless otherwise agreed to by the Engineer. At a minimum the schedule shall include the start date, milestone dates, and completion date. Any significant changes shall be brought to the attention of the Engineer immediately.

(c) **Welding Procedures:** Prior to start of fabrication or field welding, all welding procedures shall be submitted for review and approval in accordance with 1.05.17.

(d) **Working Drawings for Falsework and Erection of Structural Steel:** Prior to erecting any steel fabricated under this specification, the Contractor shall submit drawings and supporting calculations, including erection stresses, in accordance with 1.05.02, to the Engineer. The design of temporary supports and falsework shall be as specified in the *AASHTO Specifications,* the *AASHTO Guide Design Specifications for Bridge Temporary Works* or any other standard acceptable to the Engineer. Falsework shall be of sufficient rigidity and strength to safely support all loads imposed and to produce in the finished structure the lines and grades indicated in the Contract.

The working drawings submittal shall include at a minimum:

1. Title block with Contract number, Project identification number (PIN), town, and structure number and name.
2. Plan of the work area showing support structures, roads, railroad tracks, Federal and State regulated areas as depicted on the plans, utilities or any other information relative to erection.
3. A detailed narrative describing the erection sequence for main members and secondary members (cross frames, diaphragms, lateral bracing, portals, etc.), noting use of holding cranes or temporary supports, falsework, or bents.
4. Delivery location of each girder.
5. Location of each crane for each pick.
6. Capacity chart for each crane and boom length used in the work.
7. The capacity of the crane and of all lifting and connecting devices shall be adequate for the total pick load including spreaders and other materials. In the area of railroads and navigable waterways, the capacity shall be as required by Amtrak, Metro North, U.S. Coast Guard or other

regulatory authorities. No picks shall be allowed over vehicular or pedestrian traffic unless otherwise noted on the plans or permitted by the Engineer.

8. Pick point location(s) on each member.
9. Lifting weight of each member including clamps, spreader beams, etc.
10. Lift and setting radius for each pick (or maximum lift radius).
11. Description of lifting devices or other connecting equipment.
12. Girder tie-down details or other method of stabilizing erected girders.
13. Bolting requirements, including the minimum number of bolts and erection pins required to stabilize members during the erection sequence.
14. Blocking details for stabilizing members supported on expansion bearings and on bearings that do not limit movement in the transverse direction.
15. The method and location for temporary supports for field spliced or curved girders, including shoring, false work, holding cranes, guys, etc. The Engineer will review, but not approve details of temporary supports. The design, erection, and stability of these supports shall be the sole responsibility of the Contractor.
16. Offsets necessary to adjust expansion bearings during erection to provide for temperature variance and dead load rotation.

The following notes shall be placed on the Erection Drawings:

1. Cranes shall be operated in accordance with the Connecticut Department of Public Safety regulations.
2. The Contractor shall be responsible for verifying the weight of each lift and for insuring the stability of each member during all phases of erection.
3. Members shall be subject to only light drifting to align holes. Any drifting that results in distortion of the member or damage to the holes will be cause for rejection of the member.
4. Field reaming of holes shall not be performed unless required by the Contract drawings or approved by the Engineer.

The Contractor shall submit these documents to the Engineer at least 60 calendar days in advance of their proposed use. If the proposed method of erection requires additional members or modifications to the existing members of the structure, such additions and modifications shall be made by the Contractor at no expense to the State.

4. Shop Fabrication: Unless otherwise shown on the plans or indicated in the Special Provisions, Structural Steel shall be fabricated in accordance with the AASHTO LRFD Bridge Construction Specifications, amended as follows:

(a) Notification: The Contractor shall submit written notification to both the Engineer and the Division Chief (OOC) not less than 30 calendar days prior to start of fabrication. No material shall be manufactured or worked in the shop before the Engineer has been so notified. The notification shall include the name and location of the fabrication shop where the work will be done so that arrangements can be made for an audit of the facility and the assignment of a Department Quality Assurance inspector.

(b) Camber: All members shall be cambered prior to heat curving and painting. Rolled beams shall be heat cambered by methods approved by the Engineer. Plate girders shall be cambered by cutting the web to the prescribed shape with allowances for shrinkage due to cutting, welding, and heat curving. The fabricator is responsible to determine what allowances should be made. Rolled, plate-rolled, or fabricated sections shall be cambered to the total amount shown on the plans and within the camber deviation tolerances permitted for welded beams and girders, as indicated in the ANSI/AASHTO/AWS D1.5 Bridge Welding Code. The Contractor must submit to the Engineer for approval, a plan for corrective action if the actual camber is not within tolerance.

(c) Welding: Unless otherwise indicated in the Contract, all work shall be performed in accordance with ANSI/AASHTO/AWS D1.5 – Bridge Welding Code.

(d) Preassembly of Field Connections: Field connections of main members of continuous beams, plate girders, bents, towers, rigid frames, trusses and arches shall be preassembled prior to erection as necessary to verify the geometry of the completed structure or unit and to verify or prepare field splices. The Contractor shall propose an appropriate method of preassembly for review and comment by the Engineer. The method and details of preassembly shall be consistent with the erection procedures shown on the working drawings and camber diagrams. As a minimum, the preassembly procedure shall consist of assembling three contiguous panels accurately adjusted for

line and camber. Successive assemblies shall consist of at least 1 section or panel of the previous assembly plus two or more sections or panels added at the advancing end. In the case of structures longer than 150 feet, each assembly shall not be less than 150 feet long regardless of the length of individual continuous panels or section. All falsework, tools, machinery and appliances, including drift pins and bolts necessary for the expeditious handling of the work shall be provided by the Contractor at no cost to the State.

(e) Inspection: The Contractor shall furnish facilities for the inspection of material and workmanship in the shop by the Engineer. The Engineer and any representative of the Engineer shall be allowed free access to the necessary parts of the premises.

The Engineer will provide Quality Assurance (QA) inspection at the fabrication shop to assure that all applicable Quality Control plans and inspections are adequately adhered to and maintained by the Contractor during all phases of the fabrication. A thorough inspection of a random selection of elements at the fabrication shop may serve as the basis of this assurance.

Prior to shipment to the Project, each individual piece of structural steel shall be stamped or marked in a clear and permanent fashion by a representative of the fabricators' Quality Control (QC) Department to indicate complete final inspection by the fabricator and conformance to the Project specifications for that piece. The stamp or mark must be dated. A Materials Certificate in accordance with 1.06.07 may be used in lieu of individual stamps or markings, for all material in a single shipment. The Materials Certificate must list each piece within the shipment and accompany the shipment to the Project Site.

Following the final inspection by the fabricator's QC personnel, the Engineer may select pieces of structural steel for re-inspection by the Department's QA inspector. Should non-conforming pieces be identified, all similar pieces must be re-inspected by the fabricator and repair procedure(s) submitted to the Engineer for approval. Repairs will be made at the Contractor's expense.

The pieces selected for re-inspection and found to be in conformance, or adequately repaired pieces, may be stamped or marked by the QA inspector. Such markings indicate the Engineer takes no exception to the pieces being sent to the Project Site. Such marking does not indicate acceptance or approval of the material by the Engineer.

Following delivery to the Project Site, the Engineer will perform a visual inspection of all material to verify shipping documents, fabricator markings, and that there was no damage to the material or coatings during transportation and handling.

The Engineer is not responsible for approving or accepting any fabricated materials prior to final erection and assembly at the Project Site.

(f) Nondestructive Testing: All nondestructive testing of structural steel and welding shall be performed as designated in the plans and specifications. Such testing shall be performed by personnel approved by the Engineer.

Personnel performing radiographic, ultrasonic or magnetic particle testing shall be certified as a NDT Level II technician in accordance with the American Society for Non Destructive Testing (ASNT), Recommended Practice SNT-TC-1A.

Nondestructive testing shall be performed in accordance with the procedures and standards set forth in the AASHTO/AWS D1.5, Bridge Welding Code. The Department reserves the right to perform additional testing as determined by the Engineer.

All nondestructive testing shall be witnessed by an authorized representative of the Department. Certified reports of all tests shall be submitted to the Division of Materials Testing for examination. Each certified report shall identify the structure, member, and location of weld or welds tested. Each report shall also list the length and location of any defective welds and include information on the corrective action taken and results of all retests of repaired welds.

Should the Engineer require nondestructive testing on welds not designated in the Contract, the cost of such inspection shall be borne by the Contractor if the testing indicates that any weld is defective. If the testing indicates the weld to be satisfactory, the actual cost of such inspection will be paid by the Department.

(g) Marking: Each member shall be identified with an erection mark corresponding with the member identification mark on the approved shop drawings. Identification marks shall be applied on the member with a method and in a location in accordance with standard industry practice.

(h) Shipping, Handling, Storage and Receiving: The Contractor shall make all arrangements necessary to properly load, transport, unload, handle and store all material. The Contractor shall

furnish to the Engineer copies of all shipping statements. The weight of the individual members shall be shown on the statements. Members having a weight of more than 3 tons shall have the weight marked thereon. All material shall be unloaded promptly upon delivery. The Contractor shall be responsible for any demurrage charges. Damage to any material during transportation, improper storage, faulty erection, or undocumented fabrication errors may be cause for rejection of said material at the Project Site. Top lateral bracing shall be installed in tub girders prior to shipping and erection of the field pieces. All costs associated with any corrective action will be borne by the Contractor.

5. Field Erection: A meeting shall be held on Site prior to any erection of structural steel. The Contractor shall name the person responsible for the steel erection work and provide copies of all crane operator licenses. Proposed equipment, rigging, timetable and methods shall be proposed at this meeting.

(a) Falsework: Any temporary work shall be constructed in accordance with the working drawings. The Contractor shall verify that the quality of materials and work employed are consistent with their design.

All girders shall be stabilized with falsework, temporary braces, or holding cranes until a sufficient number of adjacent girders are erected with all diaphragms and cross frames connected to provide necessary lateral support as shown in the erecting diagrams.

Adjustment shall be provided in the falsework and other temporary supports so that the temporary elevation of the structural steel provided by the falsework is consistent with the deflections that will occur as the structure is completed. The elevation of falsework shall be such as to support the girders at the cambered no-load elevation. Unloading of temporary supports shall be performed such that all temporary supports at each cross section are unloaded uniformly. Unless specifically permitted by the Engineer, welding of falsework support brackets to structural steel is not allowed.

Unless erected by the cantilever method, truss spans shall be erected on blocking. The blocking shall be left in place until the tension chord splices are fully bolted and all other truss connections pinned and bolted and the proper geometric shape is achieved.

(b) Anchorages: Anchor bolts and similar materials which are to be placed during the erection of the structural steel shall be carefully and accurately set to the requirements of 6.01.03.

(c) Bearings: Bearing plates shall have a full and uniform bearing upon the substructure masonry. Bearing plates shall be placed upon bearing areas which are finished according to the requirements of 6.01.03.

Prefabricated pads conforming to the requirements of M.12.01 shall be installed unless specifically noted otherwise in the Contract plans.

Each piece shall be the same size as the bearing plate it is to support and the holes to accommodate the anchor bolts shall be clearly and accurately punched before setting the pad in place.

In placing expansion bearings, due consideration shall be given to the temperature at the time of erection and stage construction requirements. The nuts of anchor bolts at expansion bearings shall be adjusted to permit the free movement of the span.

(d) Field Assembly: Members and components shall be accurately assembled as shown on the plans and any match marks shall be followed. The material shall be carefully handled so that no components will be bent, broken or otherwise damaged.

Hammering which will injure or distort the members is not permitted. Bearing surfaces and surfaces to be in permanent contact shall be cleaned before the members are assembled. Cylindrical erection pins shall be 1/32 inch larger than the nominal diameter of the holes.

Splices and field connections of main stress carrying members shall be made with a minimum of 50% of the holes filled and tightened with high strength bolts before the lifting system is released. The bolts shall be installed uniformly throughout the connection. Lateral stability must be maintained until the deck is placed.

The Contractor shall ensure that girders are stable throughout the erection process. The stage of completeness of the bolted connections shall be considered when evaluating the strength and stability of the steel during erection. For Closed Box and Tub Girders the Contractor shall ensure that the cross-section shape of each box is maintained during erection. Top lateral bracing shall be installed in tub girders prior to shipping and erection of the field pieces.

(e) Welded Connections: Unless otherwise shown on the plans or indicated by the special provisions, welding of structural steel shall be done in accordance with ANSI/AASHTO/AWS D1.5 Bridge Welding Code.

The Contractor's welding and inspection procedures for each type of field weld and field tacking must be submitted to the Engineer on the form designated by the Department. All procedures must be approved by the Division of Materials Testing prior to any work and must be adhered to at all times.

Quality control is the responsibility of the Contractor. The Contractor must provide an AWS Certified Welding Inspector (CWI) in accordance with AWS D1.5. The CWI must be qualified and certified in accordance with the provisions of AWS QC1, *Standard for Qualification and Certification of Welding Inspectors.* The CWI shall make visual inspection of all welds. The Contractor will perform magnetic particle inspection, ultrasonic testing inspection, or radiographic testing inspection of field welds when required on the plans or special provisions. Each test may be witnessed by an authorized representative of the Engineer.

Welds or sections of welds containing imperfections determined to be unacceptable by either the CWI or the Engineer shall be removed and re-welded by the Contractor at their expense. Welds so removed and replaced shall be re-inspected by the CWI. All costs for re-inspection or testing of such welds shall be borne by the Contractor.

(f) High Strength Bolted Connections: The assembly of structural connections using high-strength bolts shall be installed so as to develop the minimum required bolt tension specified in Table A. The Manufacturer's Certified Test Report, including the rotational capacity test results must accompany the fastener assemblies. Fastener Assemblies delivered without the certified reports will be rejected.

Table A: Minimum Bolt Tension in kips*

Bolt Size (Inches)	ASTM F3125 Grade A325	ASTM F3125 Grade A490
5/8	19	24
3/4	28	35
7/8	39	49
1	51	64
1 1/8	64	80
1 1/4	81	102
1 3/8	97	121
1 1/2	118	148

*Equal to 70% of specified minimum tensile strength of bolts (as specified in ASTM Specifications for tests of full-size F3125 Grade A325 and F3125 Grade A490 bolts with UNC threads, loaded in axial tension) rounded to the nearest kip.

Bolts, nuts and washers from each rotational-capacity lot shall be shipped in the same container. If there is a single production lot number for each size of nut and washer, the nuts and washers may be shipped in separate containers. Each container shall be permanently marked with the rotational-capacity lot number such that identification will be possible at any stage prior to installation. Assemblies of bolts, nuts and washers shall be installed from the same rotational-capacity lot. Pins, small parts and packages of bolts, washers, and nuts shall be shipped in boxes, crates, kegs, or barrels. A list and description of the contained materials shall be plainly marked on the outside of each shipping container.

Bolted Parts: All material within the grip of the bolt shall be steel; there shall be no compressible material, such as gaskets or insulation, within the grip. Bolted steel shall fit solidly together after the bolts are tensioned. The length of the bolts shall be such that the end of the bolt will be flush with or outside of the face of the nut when properly installed.

Surface Conditions: At the time of assembly, all connection surfaces, including surfaces adjacent to the bolt head and nut, shall be free of scale, except tight mill scale, and shall be free of dirt or other foreign material. Burrs that would prevent solid seating of the connected parts in the snug tight condition shall be removed.

Paint is permitted on the faying surface, including slip critical connections, only when shown on the plans. The faying surfaces of slip-critical connections shall meet the requirements of the following paragraphs, as applicable:

1. Connections specified to have uncoated faying surfaces: any paint, including any inadvertent over spray, shall be excluded from areas closer than one bolt diameter, but not less than 1.0 inch, from

the edge of any hole and all areas within the bolt pattern.

2. Connections specified to have painted faying surfaces shall be blast cleaned and coated in accordance with the Contract, and shall not be assembled until the coating system has been properly cured.
3. Connections specified to have galvanized faying surfaces shall be hot-dip galvanized in accordance with ASTM A123, and shall subsequently be roughened by means of hand wire brushing. Power wire brushing is not permitted.

Installation: At the pre-erection meeting, the Contractor shall inform the Engineer of its planned method of tensioning high strength bolts. Acceptable methods are:

1. Turn-of-Nut
2. Calibrated Wrench

Fastener Assemblies: A "fastener assembly" is defined as a bolt, a nut, and a washer. Only complete fastener assemblies of appropriately assigned lot numbers shall be installed.

Fastener assemblies shall be stored in a sheltered area protected from dirt and moisture. Only as many fastener assemblies as are anticipated to be installed and tensioned during a work shift shall be taken from protected storage. Fastener assemblies not used shall be returned to protected storage at the end of the shift. Prior to installation, fastener assemblies shall not be cleaned of lubricant. Fastener assemblies which accumulate rust or dirt resulting from site conditions shall be cleaned, relubricated and tested for rotational-capacity prior to installation. All galvanized nuts shall be lubricated with a lubricant containing a visible dye. Plain bolts must be oily to the touch when delivered and installed. Lubricant shall be removed prior to painting.

All bolts shall have a hardened washer under the turned element (nut or bolt head). All hardened washers shall conform to the requirements of ASTM F436.

Where necessary, washers may be clipped on 1 side to a point not closer than 7/8 of the bolt diameter from the center of the washer.

Bolt Tension Measuring Device: The Contractor shall provide a calibrated bolt tension measuring device (a Skidmore-Wilhelm calibrator (Skidmore) or other acceptable bolt tension indicating device) at all times when, and at all locations where high-strength fasteners are being installed and tensioned. The tension measuring device (Skidmore) shall be calibrated by an approved testing agency at least annually. The Skidmore shall be used to perform the rotational-capacity test of the fastener assemblies. The Skidmore will also be used to substantiate

1. the suitability of the fastener assembly to satisfy the requirements of Table A, including lubrication as required,
2. calibration of the installation wrenches, if applicable, and
3. the understanding and proper use by the Contractor of the selected method of tensioning to be used.

Complete fastener assemblies shall be installed in properly aligned holes and then tensioned by the Turn-of-Nut or Calibrated Wrench method to the minimum tension specified in Table A. Tensioning may be done by turning the bolt while the nut is prevented from rotating when it is impractical to turn the nut. Impact wrenches, if used, shall be of adequate capacity and sufficiently supplied with air to perform the required tensioning of each bolt in approximately 10 seconds.

Bolts shall be installed in all holes of the connection and the connection brought to a snug condition. Snug is defined as having all the plies of the connection in firm contact. Snugging shall progress systematically from the most rigid part of the connection to the free edges. The bolts of the connection shall then be tightened in a similar manner as necessary until the connection is properly tensioned.

Nuts shall be located, whenever practical, on the side of the connection which will not be visible from the traveled way.

Unless otherwise approved by the Engineer fastener assemblies shall be brought to full tension immediately following snugging.

Fully tensioned fastener assemblies shall not be reused. Retightening previously tensioned bolts which may have been loosened by the tensioning of adjacent bolts shall not be considered as reuse.

Rotational-Capacity Tests: In addition to the Certified Test Reports, on site Rotational-capacity tests may be required by the Engineer. This test shall be performed by the Contractor at the location where the fasteners are installed and tensioned. When performed in the field, the procedure shall meet the requirements of ASTM F3125 Annex A2.

Turn-of-Nut Installation Method: At the start of the work, the Contractor shall demonstrate that the procedure used by the bolting crew to develop a snug condition and to control the turns from a snug

condition develops the tension required in Table A. To verify their procedure, the Contractor shall test a representative sample of not less than 3 complete fastener assemblies of each diameter, length and grade to be used in the work. This shall be performed at the start of work using a Skidmore. Periodic retesting shall be performed when ordered by the Engineer.

After snugging the connection, the applicable amount of rotation specified in Table B shall be achieved.

During the tensioning operation there shall be no rotation of the part not turned by the wrench. Tensioning shall progress systematically from the most rigid part of the connection to its free edges.

TABLE B: Nut Rotation from the Snug Condition
Geometry[a,b,c] of Outer Faces of Bolted Parts

Bolt Length (measured from underside of head to end of bolt)	Both Faces Normal to Bolt Axis	One Face Normal to Bolt Axis and Other Face Sloped Not More Than 1:20, Bevel Washer Not Used	Both Faces Sloped Not More Than 1:20 From Normal to Bolt Axis, Bevel Washer Not Used
Up to and including 4 diameters	1/3 turn	1/2 turn	2/3 turn
Over 4 diameters but not exceeding 8 diameters	1/2 turn	2/3 turn	5/6 turn
Over 8 diameters but not exceeding 12 diameters	2/3 turn	5/6 turn	1 turn

(a) Nut rotation, as used in Table B, shall be taken as relative to the bolt, regardless of the element (nut or bolt) being turned. For bolts installed by 1/2 turn and less, the tolerance should be plus or minus 30 degrees; for bolts installed by 2/3 turn and more, the tolerance should be plus or minus 45 degrees.
To determine the nut rotation for installation and inspection of the fasteners, the nut and the end of the bolt or the head of the bolt and the adjacent steel shall be match marked.

(b) The values, given in Table B, shall be applicable only to connections in which all material within grip of the bolt is steel.

(c) No research work has been performed by the Research Council on Riveted and Bolted Structural Joints to establish the turn-of-nut procedure when bolt lengths exceed 12 diameters. For situations in which the bolt length, measured from the underside of the head to the end of the bolt, exceeds 12 diameters, the required rotation shall be determined by actual tests in a suitable tension device simulating the actual conditions.

Calibrated Wrench Installation Method: Calibrated wrench method may be used only when the installation wrenches are properly calibrated daily, or as determined by the Engineer. Standard torques determined from tables or from formulas which are assumed to relate torque to tension shall not be acceptable.

The Contractor shall demonstrate to the Engineer periodically that all equipment and wrenches are providing a torque which has been calibrated to produce the minimum tension specified in Table A. The installation procedures shall be verified periodically, as determined by the Engineer, for each bolt diameter, length and grade using the fastener assemblies that are being installed in the work. This verification testing shall be accomplished in a Skidmore by tensioning 3 complete fastener assemblies of each diameter, length and grade from those being installed with a hardened washer under the element turned.

When significant difference is noted in the surface condition of the bolts, threads, nuts or washers, as determined by the Engineer, wrenches shall be recalibrated. The Contractor shall verify during the installation of the assembled steel work that the wrench adjustment selected by the calibration does not produce a nut or bolt head rotation from snug greater than that permitted in Table B. If manual torque wrenches are used, nuts shall be turned in the tensioning direction when torque is measured.

When calibrated wrenches are used to install and tension bolts in a connection, bolts shall be installed with hardened washers under the element turned to tension the bolts. Once the connection has been snugged, the bolts shall be tensioned using the calibrated wrench. Tensioning shall progress systematically from the most rigid part of the connection to its free edges. A calibrated torque wrench

shall be used to "touch up" previously tensioned bolts which may have been relaxed as a result of the subsequent tensioning of adjacent bolts until all bolts are tensioned to the prescribed amount.

Inspection: The Contractor shall provide all the material, equipment, tools and labor necessary for the inspection of the bolted connections. Access to the bolted parts and fastener assemblies, both before and after the fasteners are installed and tensioned, shall be provided.

The Contractor is responsible for Quality Control (QC). The Contractor shall review this specification with its Project personnel prior to performing the work. The Contractor shall verify the proper markings, surface conditions and storage of fastener assemblies. The Contractor shall inspect the faying surfaces of connections for compliance with the plans and specifications. The Contractor shall provide to the Engineer a copy of their written QC report for each shift of the calibration or verification testing specified. This report shall confirm that the selected procedure is properly used and that the fastener assemblies installed meet the tensions specified in Table A. The Contractor shall monitor the installation of fasteners in the work to assure that the selected procedure, as demonstrated in the initial testing to provide the specified tension, is routinely and properly applied.

The Contractor, in the presence of the Engineer, shall inspect the tensioned bolts using an inspection torque wrench, as defined below.

Inspection tests shall be performed within 24 hours of bolt tensioning to prevent possible loss of lubrication or corrosion influence on tensioning torque.

The inspection torque wrench shall be calibrated as follows: Three (3) bolts of the same grade, size, and condition as those under inspection shall be placed individually in a device calibrated to measure bolt tension. This calibration operation shall be done at least once each inspection day. There shall be a washer under the part turned in torqueing each bolt. In the calibrated device, each bolt shall be tightened by any convenient means to the specified tension. The inspection wrench shall then be applied to the tensioned bolt to determine the torque required to turn the nut or head 5 degrees in the tightening direction. The average of the torque required for all 3 bolts shall be defined as the job-inspection torque.

Twenty-five percent (25%), but a minimum of 2, of the tensioned bolts shall be selected by the Engineer for inspection in each connection. (The Engineer may reduce the number of bolts tested at a connection to 10% based on the Contractor's past performance and splice location.) The job-inspection torque shall then be applied to each selected assembly with the inspection torque wrench turned in the tightening direction. If all inspected bolt heads or nuts do not turn, the bolts in the connection shall be considered to be properly tensioned. If the torque turns 1 or more bolt heads or nuts, the job-inspection torque shall then be applied to all bolts in the connection or to the satisfaction of the Engineer. Any bolt whose head or nut turns shall be re-tensioned and re-inspected. The Contractor may, however, re-tension all the bolts in the connection with the inspection torque wrench and resubmit it for inspection, so long as the bolts are not over-tensioned or damaged by this action.

(g) Field Corrections and Misfits: Reaming of bolt holes during erection shall be permitted only with approval of the Engineer. No excessive forces shall be applied to any member to provide for proper alignment of the bolt holes.

The correction of minor misfits involving minor amounts of reaming, cutting, grinding and chipping shall be considered a legitimate part of the erection. However, any error in the shop fabrication or deformation resulting from handling and transportation may be cause for rejection. The Contractor shall be responsible for all misfits, errors and damage and shall make the necessary corrections and replacements.

6.03.04—Method of Measurement: Payment under this item will be at the Contract lump sum price per each complete bridge structure or shall be based on the net weight of metal in the fabricated structure, whichever method appears on the bid proposal form.

When payment is on a lump sum basis, the work, including anchor bolts, steel bearings and plates will not be measured for payment. Bearing plates welded to the girder are included in the price of the structural steel and bearing plates bonded to the bearings are included in the price of the bearing.

When payment is based on the net weight of metal in the fabricated structure, it shall be computed as described below.

The weight of the metal works to be paid for under the item of structural steel shall be computed on the basis of the net finished dimensions of the parts as shown on the shop drawings, deducting for copes, cuts, clips and all open holes, except bolt holes, and on the following basis:

1. The weights of rolled shapes shall be computed on the basis of their nominal weights per foot, as

shown in the shop drawings or listed in handbooks.

The weight of plates shall be computed on the basis of the nominal weight for their width and thickness as shown on the shop drawings.

2. The weight of temporary erection bolts, shop and field paint, galvanization, boxes, crates and other containers used for shipping, and materials used for supporting members during transportation and erection, will not be included.
3. The weight of all high strength bolts, nuts, and washers shall be included on the basis of the following weights:

Weight per 100 pieces (in pounds)	
Nominal diameter of H.S. bolt (inch)	**Bolthead, nut, 1 washer and stickthrough (lbs)**
1/2	22
5/8	33
3/4	55
7/8	84
1	120
1 1/8	169
1 1/4	216

4. The weight of weld metal shall be computed on the basis of the theoretical volume from plan dimensions of the welds.

Size of fillet in Inches	Weight of Weld in pounds per foot
3/16	0.08
1/4	0.14
5/16	0.22
3/8	0.30
1/2	0.55
5/8	0.80
3/4	1.10
7/8	1.50
1	2.00

5. The weight of steel shims, filler plates and anchor bolts shall be measured for payment.

6.03.05—Basis of Payment: The structural steel, incorporated in the completed and accepted structure, will be paid for either at the Contract lump sum price for "Structural Steel (Site No.)" or at the Contract unit price per hundredweight for "Structural Steel," whichever is indicated in the Contract.

Payment for either method shall be for structural steel, complete in place, which price shall include quality control, furnishing, fabricating, transporting, storing, erecting, welding, surface preparation and all materials including fastener assemblies, steel bearing assemblies and anchor bolts, equipment, tools and labor incidental thereto.

The cost of the raw material is included in the lump sum payment for "Structural Steel (Site No.)." All remaining work including, but not limited to, preparation of shop drawings, fabricating, transporting, storage and handling, erecting, surface preparation and all materials, equipment, tools and labor incidental thereto, will be paid for under "Structural Steel (Site No.)."

No direct payment will be made for setting anchor bolts, preparing bearing areas, furnishing and placing materials under bearings. No direct payment will be made for non destructive testing as shown on the plans.

Pay Item	Pay Unit
Structural Steel (Site No.)	l.s.
Structural Steel	cwt.

SECTION 6.05
MASONRY FACING

6.05.01—Description
6.05.02—Materials
6.05.03—Construction Methods
6.05.04—Method of Measurement
6.05.05—Basis of Payment

6.05.01—Description: This item shall consist of a surface course or facing of masonry of the hereinafter specified quality, applied to the exposed surfaces of the concrete structures as a protection or ornamentation. It shall be constructed to the dimensions indicated on the plans or as ordered and in accordance with these specifications.

6.05.02—Materials: The materials for this work shall meet the requirements of M.11.01 for masonry facing stone, M.11.04 for mortar and M.06.01 for metal dowels and ties.

6.05.03—Construction Methods: Each grade of masonry facing stone shall be constructed in the location and to the dimensions shown on the plans or as ordered and shall be built in conjunction with concrete backing. The backing shall not be placed until the masonry facing and any required dowels or ties have been properly placed. The work shall be carried on in successive layers or lifts as permitted by the Engineer. Care must be taken to prevent any movement of the masonry already in place while placing and compacting the concrete. When required by the Engineer, the facing stone shall be supported by such bracing and form work as may be necessary to prevent movement.

All stone shall be set by competent and experienced masons.

1. Dressing Stone: All stones shall be dressed to the required size and shape before being laid and, except in the case of granite, shall be cut to lie on their natural beds. The bottom bed shall be the full size of the stone, and no stone shall have an overhanging top. In rock-faced work the exposed surface of any stone shall not present an excessively undercut contour adjacent to its bottom arris giving a top heavy, unstable appearance when laid.

Beds and joints of dimensioned masonry stone, except for rock-faced surface finish stone, shall be fine finished a full 2 inches from the pitch line of the face from which point the bottom bed may fall under the square 1 inch per 12 inches; the top bed 1 1/2 inch per 12 inches; and the joints 2 inches per 12 inches; but all beds must have a reasonable area of bearing spots back of the face. They shall be free from large depressions which might impair the stability of the work. On rock-faced stone, the beds shall be sawn or dressed full for at least the specified minimum width of the stone; beyond that the beds may fall under the square not in excess of 3 inches per 12 inches. The joints shall be cut full and square for at least 2 inches back of the pitch lines from which point they may fall away not over 3 inches per 12 inches. Headers, if called for, shall have a back dimension not less than 2/3 of the corresponding face dimension unless otherwise shown on the plans.

Beds and joints of ashlar stone shall be rough finished for a depth of not less than 2 inches from the arris lines, and the balance of the stone shall not fall off more than 1/4 of the minimum dimension of the piece.

Holes for dowels or ties or for handling stone shall not be permitted to show in exposed surfaces.

2. Mixing Mortar: The mortar shall be hand or machine mixed as may be directed by the Engineer. In the preparation of hand mixed mortar, the fine aggregate, cement and lime, if used, shall be thoroughly mixed together in a clean, tight mortar box until the mixture is of uniform color, after which water shall be added in such quantity as to form a stiff paste. Machine mixed mortar shall be prepared in an approved mixer and shall be mixed not less than 1 1/2 minutes.

Mortar shall be used before obtaining initial set. Re-tempering of mortar will not be permitted.

3. Laying Stone: Stone shall not be laid when the air temperature in the shade and away from artificial heat is 40° F or below and falling, except with the permission of the Engineer and subject to such conditions as the Engineer may impose.

Dimensioned stone masonry shall be carefully and accurately constructed in accordance with the provisions of the approved plans.

Ashlar stone masonry shall be so constructed as to produce the general effect shown on the plans. As an aid in determining the required appearance, the Contractor shall lay up a sample section of wall, under the direction of the Engineer, which when approved, shall be accepted as representing the appearance to be obtained in the construction.

Each stone shall be cleaned and thoroughly saturated with water before being set. Stone shall not be dropped on or slid over the wall, but shall be carefully set without jarring stone already laid and shall be handled with a lewis or other appliance which shall not cause disfigurement. All stones shall be well bedded in freshly made mortar and settled in place with a suitable wooden maul before the setting of the mortar. Whenever possible the face joints shall be properly pointed before the mortar becomes set. Joints which cannot be so pointed shall be prepared for pointing by raking them out to a depth of about 2 inches before mortar has set. The face surfaces of stone shall not be smeared with the mortar forced out of joints or that used in pointing. No hammering, rolling or turning of stones will be allowed on the wall. Precautions shall be taken to prevent seepage of moisture, through or from the beds and joints, which may cause discoloration of the exposed surfaces.

For rock-faced stone, the course heights may vary from 2 feet to 3 feet 2 inches with the higher courses at the bottom. Face bond shall be not less than 12 inches.

Headers shall be placed in the masonry only if so specified. They shall then be placed in each course and shall have a face length of not less than the rise of the stone. They shall bond with the core or backing not less than 12 inches. A minimum face bond of not less than 12 inches shall be maintained. In no case shall a joint occur over or under a header. Approximately 1/5 of the area of the wall shall consist of headers.

Beds and joints in dimensioned stone masonry, unless otherwise specified, shall be not less than 3/8 inch nor more than 3/4 inch thick and shall present a uniform appearance. Abrupt changes in the thickness of beds and joints will not be permitted.

Beds and joints in ashlar masonry shall have an average thickness of not more than 1 inch.

4. Dowels and Ties: Metal dowels and ties of the type called for on the plans or as ordered shall be installed during the placing of the stone and before the backing is placed.

5. Resetting: In case any stone is moved or the joint broken, the stone shall be taken up, the mortar thoroughly cleaned from bed and joints, and the stone reset in fresh mortar.

6. Concrete Backing: Before concrete backing is poured, the backs of all beds and joints shall be pargeted with not less than 1/2 inch of setting mortar. No concrete shall be placed until the pargeting mortar has set. In general, horizontal construction joints produced by successive placements of the backing concrete shall be located not less than 6 inches below the top or above the bottom bed of any course of masonry.

7. Joints: Joints shall be pointed or raked as called for in the Contract.

8. Pointing: Pointing shall not be done in freezing weather or when the stone contains frost.

Joints not pointed at the time the stone is laid shall be thoroughly cleaned out, wet with water and filled with mortar. The mortar shall be well driven into the joints and finished with an approved pointing tool to produce the type of joint shown on the plans or required by the Engineer. The wall shall be kept wet while pointing is being done; and in hot or dry weather, the pointed masonry shall be protected from the sun and kept wet for a period of at least three days after completion.

After the pointing is completed and the mortar set, the wall shall be thoroughly cleaned and left in a neat and workmanlike condition. The use of acid for cleaning will not be permitted.

6.05.04—Method of Measurement: This work will be measured for payment as follows:

1. Masonry Facing: The quantity of masonry facing shall be the actual number of square yards of the face area of accepted masonry facing, completed within the neat lines as shown on the plans, or as ordered by the Engineer.

When the masonry facing is backed with concrete, the quantity to be paid under the concrete item shall be determined by assuming that the facing has the average thickness stated on the plans.

2. Metal Dowels or Ties: Metal dowels or ties will be measured for payment by the number of pounds of metal actually incorporated in the completed and accepted work in accordance with the provisions of 6.02.04.

3. Pointing: Unless otherwise specified, all pointing shall be included in the cost of the masonry facing. When the item of "Pointing" appears as a Contract item, measurement will be made according to the gross area of the surface to receive this treatment.

6.05.05—Basis of Payment: This work will be paid for as follows:

1. Masonry Facing: Masonry facing will be paid for at the Contract unit price per square yard for "Dimension Stone Masonry" or "Ashlar Stone Masonry," as the case may be, complete in place, which price shall include all equipment, tools and labor incidental thereto and all materials except metal dowels or ties. The cost of drilling holes for dowels or ties shall be considered as included in the general cost of the

work.

2. Metal Dowels or Ties: Metal dowels or ties will be included with and paid for at the Contract unit price per pound for "Deformed Steel Bars" in accordance with the provisions of 6.02.05.

3. Pointing: Where the item of pointing appears, pointing will be paid for at the Contract unit price per square yard for "Pointing" which shall include all materials and all other expense incidental thereto.

Pay Item	Pay Unit
Dimension Stone Masonry	s.y.
Ashlar Stone Masonry	s.y.
Pointing (Kind)	s.y.

SECTION 6.06
CEMENT RUBBLE MASONRY

6.06.01—Description
6.06.02—Materials
6.06.03—Construction Methods
6.06.04—Method of Measurement
6.06.05—Basis of Payment

6.06.01—Description: This item shall consist of approved stone, laid in full mortar beds, constructed in such shapes and at such places as indicated on the plans or where directed, and in accordance with these specifications.

6.06.02—Materials: The materials for this work shall meet the requirements of M.11.02 for rubble masonry stone and M.11.04 for mortar.

6.06.03—Construction Methods: The masonry shall be constructed in the location and to the dimensions shown on the plans or as ordered.

1. Dressing Stone: All necessary dressing or shaping shall be done before the stone is laid in the wall. No dressing or hammering which will loosen the stone will be permitted after it is placed. Stone at angles or at ends of walls shall be roughly squared and dressed to the required lines.

2. Laying Stone: Stone shall not be laid when the air temperature in the shade and away from artificial heat is 40°F or below and falling, except with the approval of the Engineer and subject to such conditions as the Engineer may impose.

The masonry shall be laid to line and in courses roughly leveled up. The bottom or foundation courses shall be composed of large, selected stones; and all courses shall be laid with bearing beds parallel to the natural bed of the material.

Each stone shall be cleaned and thoroughly saturated with water before being set. Stone shall not be dropped or slid over the wall but shall be carefully set without jarring stone already laid. All stone shall be well bedded in freshly made mortar. The mortar joints shall be full and the stones carefully settled in place before the mortar has set. No spalls will be permitted in the beds. Joints and beds shall have an average thickness of not more than 1 1/2 inches.

Headers shall be placed in each course and shall have a width of not less than 1 1/2 times their thickness. They shall bond with the core or backing not less than 12 inches; and in walls 2 feet or less in thickness, they shall extend entirely through the wall. Headers shall occupy not less than 1/5 of the face area of the wall and shall be evenly distributed.

Whenever possible the face joints shall be properly pointed before the mortar becomes set. Joints which cannot be so pointed shall be prepared for pointing by raking them out to a depth of about 2 inches before the mortar has set. The face surfaces of stones shall not be smeared with the mortar forced out of the joints or that used in pointing.

The vertical joints in each course shall break joints with those in adjoining courses at least 6 inches. In no case shall vertical joints be so located as to occur directly above or below a header.

In case any stone is moved or the joint broken, the stone shall be taken up, the mortar thoroughly cleaned from bed and joints, and the stone reset in fresh mortar.

3. Pointing: Pointing shall not be done in freezing weather or when the stones contain frost.

Joints not pointed at the time the stones are laid shall be thoroughly wet with water and filled with mortar. The mortar shall be well driven into the joints and finished with an approved pointing tool. The wall shall be kept wet while pointing is being done; and in hot or dry weather, the pointed masonry shall be protected from the sun and kept wet for a period of at least 3 days after completion.

After the pointing is completed and the mortar set, the wall shall be thoroughly cleaned and left in a neat and workman-like condition.

Cofferdams shall be so maintained that the masonry is not exposed to the action of running water until such time as the mortar has attained a strength sufficient, in the opinion of the Engineer, to prevent injury to the work from such exposure.

6.06.04—Method of Measurement: This work will be measured for payment as follows:

1. Cement Rubble Masonry: The quantity of cement rubble masonry shall be the actual number of cubic yards, completed and accepted, within the neat lines as shown on the plans or as ordered by the Engineer. No deduction will be made for the volume occupied by culvert or drainage pipes having an

exposed surface area of less than 9 s.f. for each opening. The computation of the surface area shall be based on the nominal diameter of the pipe, disregarding the thickness of the shell.

6.06.05—Basis of Payment: This work will be paid for as follows:

1. Cement Rubble Masonry: Cement rubble masonry will be paid for at the Contract unit price per cubic yard for "Cement Rubble Masonry" complete in place which price shall include all materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Cement Rubble Masonry	c.y.

SECTION 6.07
DRY RUBBLE MASONRY

6.07.01—Description
6.07.02—Materials
6.07.03—Construction Methods
6.07.04—Method of Measurement
6.07.05—Basis of Payment

6.07.01—Description: This item shall consist of approved stone, laid without the aid of mortar, so as to fit neatly and firmly, constructed in such shapes and at such places as indicated on the plans or where directed, and in accordance with these specifications.

6.07.02—Materials: The materials for this work shall meet the requirements of M.11.02 for rubble masonry stone.

6.07.03—Construction Methods: The masonry shall be constructed in the location and to the dimensions shown on the plans or as ordered.

1. **Dressing Stone:** All necessary dressing or shaping of stone shall be done before the stone is laid in the wall. No dressing or hammering which will loosen the stone will be permitted after it is placed. Stones at angles or at ends of walls shall be roughly squared and dressed to the required lines.
2. **Laying Stones:** The masonry shall be laid to line in courses, roughly leveled up. The bottom or foundation courses shall be composed of large, selected stones, and all courses shall be laid with bearing beds parallel to the natural bed of the material. Face joints shall have a width of not more than 1 1/2 inches.

In laying rubble masonry, care shall be taken that each stone takes a firm bearing at not less than 3 separate points upon the underlying course. Open joints, both front and rear, shall be chinked with spalls, fitted to take firm bearing upon their top and bottom surfaces, for the purpose of securing firm bearing throughout the length of the stone.

6.07.04—Method of Measurement: The quantity of dry rubble masonry shall be the actual number of cubic yards, completed and accepted, within the neat lines of the structure as shown on the plans or as ordered by the Engineer. No deduction will be made for the volume occupied by culvert or drainage pipes having an exposed surface area of less than 9 s.f. for each opening. The computation of the surface area shall be based on the nominal diameter of the pipe, disregarding the thickness of the shell.

6.07.05—Basis of Payment: This work will be paid for at the Contract unit price per cubic yard for "Dry Rubble Masonry," complete in place, which price shall include all materials, equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
Dry Rubble Masonry	c.y.

SECTION 6.09
REPOINTED MASONRY

6.09.01—Description
6.09.02—Materials
6.09.03—Construction Methods
6.09.04—Method of Measurement
6.09.05—Basis of Payment

6.09.01—Description: This item shall consist of repointing existing masonry walls where indicated on the plans or as directed, in accordance with these specifications.

6.09.02—Materials: The mortar for this work shall meet the requirements of M.11.04.

6.09.03—Construction Methods: Pointing shall not be done in freezing weather or when the stone contains frost. The joints in the existing masonry shall be thoroughly raked out and cleaned of all loose mortar, dirt, or other foreign material to a depth of about 2 inches or as ordered. The joints shall be thoroughly wet with water and filled with mortar. The mortar shall be well driven into the joints and finished with an approved pointing tool. The wall shall be kept wet while pointing is being done; and in hot or dry weather, the pointed masonry shall be protected from the sun and kept wet for a period of at least 3 days after completion.

After the pointing is completed and the mortar set, the wall shall be thoroughly cleaned and left in a neat and workmanlike condition.

6.09.04—Method of Measurement: The quantity of repointed masonry shall be the actual number of square yards completed and accepted within the neat lines as shown on the plans or as ordered.

6.09.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Repointed Masonry" which price shall include all materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Repointed Masonry	s.y.

SECTION 6.11
SHOTCRETE

6.11.01—Description: This item shall consist of a surface coating of Portland cement mortar applied to the reinforcement by means of pneumatic equipment, built in the locations and to the dimensions and details shown on the plans.

6.11.02—Materials: The materials for mortar for this work shall meet the requirements of M.11.04. Reinforcement shall be of the type called for on the plans and shall meet the requirements of M.06.01.

6.11.03—Construction Methods: The preparation and application of shotcrete shall be done under the supervision of experienced foremen, using only experienced personnel as machine and nozzle operators.

1. Preparation of Bonding Surface: The bonding surface to receive the shotcrete shall be rough and clean. Loose particles, dust and dirt shall be removed by the use of wire brushes or other approved means. Steel members to be protected with shotcrete shall be cleaned thoroughly of all loose rust, scale, grease, dirt, paint and any foreign material that will prevent or lessen the bond between the shotcrete and the steel.

The bonding surface shall be maintained constantly wet for a minimum of 1 hour prior to the application of the shotcrete. After wetting, a sufficient time shall elapse or special means shall be used, to remove all surplus surface water and to produce a damp surface that is slightly absorptive. The shotcrete shall be applied when this condition is attained. In no case shall shotcrete be applied to a dry surface.

2. Placing Reinforcement: Reinforcement shall be placed in accordance with the requirements of the plans; and where dowels or anchor bolts are specified, the reinforcement shall be securely fastened to them. When wire fabric reinforcement is used, adjoining sections shall be lapped not less than 4 inches. Fabric around the top flanges of beams and girders carrying concrete slabs shall be placed before the slab is poured and shall extend at least 5 inches below the bottom of the slab and be so located as to properly lap the web reinforcement. Exposed reinforcement in existing structures that are to be repaired or altered shall be thoroughly cleaned. No reinforcement shall be located closer than 3/4 inches from any finished surface.

3. Composition: Shotcrete shall be mixed in the proportions of 1 part Portland cement to not more than 3 1/2 parts of fine aggregate, by dry, loose volume. Due consideration shall be given to the bulking effect of any moisture contained in the fine aggregate.

4. Placing: Shotcrete shall be placed by pneumatic pressure through a suitable machine, with the proper amount of water for hydration applied in the mixing nozzle. No shotcrete shall be placed when the air temperature is below 40° F or against surfaces containing frost.

Portland cement and fine aggregate shall be premixed dry in a batch mixer and screened through a 1/4-inch screen before being placed in the hopper of the machine. The sand so used shall have only a normal moisture content of 3% to 8%.

Water pressure in the delivery pipe shall be maintained at a pressure of 10 to 20 psi above the air pressure in the machine.

A uniform pressure of not less than 35 psi shall be maintained in the placing machine. Where the length of the hose between the placing machine and the point of application exceeds 100 feet horizontally or 25 feet vertically, the pressure shall be increased so as to secure a satisfactory velocity at the nozzle.

Shooting strips shall be employed to insure square corners, straight lines and the required thickness of shotcrete. These shooting strips shall be so placed that they will not trap rebound.

When "shooting" any surface, the stream of material flowing from the nozzle shall impinge as nearly as possible at right angles to the surface being covered. All deposits of loose sand shall be removed prior to "shooting." Should any such deposit be covered with shotcrete, it shall be cut out and removed by the Contractor at its own expense.

Shotcrete shall be applied in 2 or more coats, the next to the last coat being screeded to remove irregularities after which the final or "flash" coat shall be applied. This coat shall not be troweled or screeded but shall be left without further finish except that outside exposed surfaces shall be given a "float" finish as specified in 6.01.03.

5. Curing: Shotcrete shall be cured and protected in accordance with the requirements of 6.01.03.

6.11.04—Method of Measurement: This work will be measured for payment as follows:

1. Shotcrete: When shotcrete is applied to new or repair work, the measurement for work under this item will be based on the actual number of 94-pound net weight bags of cement used to cover the exposed surface area to the full depth required within the neat lines as shown on the plans or as ordered and accepted by the Engineer.

2. Reinforcement: Reinforcement will be measured in accordance with the provisions of 6.02.04.

6.11.05—Basis of Payment: Payment for this work will be made as follows:

1. Shotcrete: This work will be paid for at the Contract unit price per bag for "Shotcrete," complete in place, which price shall include all equipment, tools, labor and all materials except reinforcement.

2. Reinforcement: This material will be paid for at the Contract unit price per pound for "Deformed Steel Bars" and per square yard for "Welded Wire Fabric" complete in place, which price shall include all materials, equipment and labor incidental thereto.

Pay Item	Pay Unit
Shotcrete	bag

SECTION 6.12
CONCRETE CYLINDER CURING BOX

6.12.01—Description: This item shall consist of furnishing a box for curing concrete test cylinders. The box shall be commercially available and manufactured specifically for curing concrete test cylinders. The box will remain the property of the Contractor at the conclusion of the Project. The box shall be delivered to a location on the Project as directed by the Engineer.

6.12.02—Materials: A catalog cut listing detailed specifications of the box and operating instructions from the manufacturer must be submitted to the Engineer. The box and its components shall be constructed of non-corroding materials and shall be capable of storing a minimum of 18 test cylinders, 6 inches × 12 inches stored vertically with the lid closed. The lid must be watertight when closed and hinged in the back with security latches on the front that can be padlocked. The box must be capable of holding water to a maximum level of 1 inch above test cylinders placed in the box vertically. A drain hole must be provided in a wall of the box to allow manual drainage of the water that exceeds this level. A drain hole must also be provided at the bottom of the box so that it can be manually emptied. The temperature of the water must be controlled by heating and cooling device capable of maintaining the temperature of the water within a range of 60 to 80ºF, +/- 2ºF within an outside ambient air temperature range of -10 to 120ºF. The heating and cooling device must be positioned to allow free circulation of air and water around the cylinders and be rated at 120 volts and 15 amps. A rack must be provided within the box to support the cylinders above the pool of temperature controlled water. The device must be thermostatically controlled with a digital readout that is capable of displaying the high/low water temperature within the box since the last reading was taken.

6.12.03—Construction Methods: The Contractor shall maintain the curing box in working order and shall provide all necessary electrical service and water so that the curing box can be used properly during the entire course of the Project. Any curing box that is not operating properly, as determined by the Engineer, shall be replaced within 24 hours by the Contractor at no expense to the State. The Engineer reserves the right to prohibit placement of fresh concrete on the Project until a curing box acceptable to the Engineer is operational on the Project Site.

6.12.04—Method of Measurement: The furnishing of the concrete test cylinder curing box will be measured for payment by the number of boxes delivered by the Contractor and accepted by the Engineer.

6.12.05—Basis of Payment: This item will be paid for at the Contract unit price each for "Concrete Cylinder Curing Box" ordered and accepted on the Project, which price shall include all submittals, material, tools, equipment, and labor incidental thereto. The price shall also include all maintenance and operating costs related to the curing box for the duration of the Project.

Pay Item	Pay Unit
Concrete Cylinder Curing Box	ea.

SECTION 6.53
CLEAN EXISTING DRAINAGE SYSTEM

6.53.01—Description: The work included in this item shall consist of furnishing all equipment, tools, labor, and materials and performing all work necessary for cleaning, removing and disposing of all sludge, dirt, sand, gravel, roots, grease, and other debris from the existing drainage system which includes: culverts, 12 inch to 42 inch diameter; culverts, greater than 42 inch diameter; manholes; catch basins; and drop inlets, throughout the Project limits, as directed by the Engineer.

6.53.02—Materials: Vacant

6.53.03—Construction Methods: Selection of the equipment used shall be based on the condition of the lines at the time the cleaning operations commence and shall be approved by the Engineer.

The sequence of the Contractor's work shall allow for the proper and adequate maintenance of all functional drainage systems.

Precautions shall be taken to protect the drainage systems at all times. All workmen shall be experienced and skilled in the use of the equipment used. The Engineer reserves the right to prohibit use of any equipment or method deemed inappropriate for the intended work.

Any and all debris resulting from the cleaning operations shall be removed from the Site and disposed of by the Contractor. The Contractor shall make every effort to remove all sludge, dirt, sand, gravel, roots, grease, and other debris from the existing drainage systems including discharge points. Washing sludge, dirt, sand, gravel, roots, grease, and other debris downstream will not be permitted.

6.53.04—Method of Measurement: Catch basins, manholes and drop inlets will be measured for payment by the actual number of units cleaned. Culverts cleaned under this item will be measured for payment by the actual number of linear feet of culvert cleaned for those 12 inch to 42 inch diameter, and those greater than 42 inch diameter.

6.53.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Clean Existing Catch Basin," "Clean Existing Manhole," or "Clean Existing Drop Inlet." Cleaning of culverts will be paid for at the Contract unit price per linear foot for, "Clean Existing Culvert - 12 inch to 42 inch Diameter," "Clean Existing Culvert - Greater than 42 inch Diameter," which price shall include all equipment, tools, and labor incidental to the completion of these items. All costs incidental to the disposal of sludge, dirt, sand, gravel, roots, grease, and other debris will be included in the price above.

Pay Item	Pay Unit
Clean Existing Catch Basin	ea.
Clean Existing Manhole	ea.
Clean Existing Drop Inlet	ea.
Clean Existing Culvert - 12” to 42” Diameter	l.f.
Clean Existing Culvert - Greater than 42” Diameter	l.f.

SECTION 6.86
DRAINAGE PIPES, DRAINAGE PIPE ENDS

6.86.01—Description
6.86.02—Materials
6.86.03—Construction Methods
6.86.04—Method of Measurement
6.86.05—Basis of Payment

6.86.01—Description: This work shall consist of furnishing, preparing and installing drainage pipes of the size and type specified, bedding material, joint sealant, rubber gaskets, clamps, collars, grout, grout collars, drainage trench excavation, backfilling or satisfactory disposal of all materials, the removal of which is necessary for the proper completion of the work, connecting proposed drainage systems to existing systems, plugging or abandoning existing pipes and removal of existing pipe within trench limits, as shown on the plans or as directed by the Engineer.

This Section shall also include removal of drainage pipes outside of drainage trench excavation limits, as defined in 2.86.03-1.

6.86.02—Materials: The materials for this work shall meet the following requirements:

Drainage Pipe, Drainage Pipe Ends, Sealers, Gaskets and connection hardware shall meet the requirements of M.08.01.

Bedding Material shall meet the requirements of M.08.03-1.

Granular Fill, if necessary, shall meet the requirements of M.02.01.

Brick Masonry shall meet the requirements of M.11.03 and Mortar shall meet the requirements of M.11.04.

Concrete used for Concrete Pipe Connections shall be Class PCC04460 Concrete meeting the requirements of M.03.

6.86.03—Construction Methods:

(1) Drainage Trench Excavation: Drainage trench excavation and backfilling shall be performed in accordance with 2.86.03 and the requirements of the plans.

Where drainage pipe is to be laid below the surface, a drainage trench shall be excavated to the required depth, the bottom of which shall be graded to the elevation of the bottom of the bedding material.

Where drainage pipe is to be laid in a fill area, the embankment shall be placed and compacted to a minimum elevation 12 inches above the top of the proposed pipe, whereupon the drainage trench excavation shall be performed and the pipe installed.

(2) Rock in Drainage Trench Excavation: When rock, as defined in 2.86.01-2, is encountered, work shall be performed in accordance with 2.86.03 and the requirements of the plans.

(3) Drainage Pipe Installation: New or re-laid drainage pipes shall be installed on 4 inches of bedding material (12 inches if over rock in ledge formation), the details as shown on the plans, or as directed by the Engineer. Prior to placement of the drainage pipe, in accordance with the plans, bedding material shall be pre-shaped to 10% of the total height of the pipe in order to keep the pipe in the center of the trench. Following placement of the drainage pipe, bedding material backfill shall be placed in accordance with the following table:

Internal Pipe Diameter	**Required Bedding Material Backfill**
< 48 inches*	25% of total height of the pipe
≥ 48 inches*	12 inches above the top of the pipe
*Includes pipe arch of equivalent internal horizontal span See Standard Drawing	

The placement of the drainage pipe shall start at the downstream end and progress upstream or as shown on the plans, or as directed by the Engineer. All drainage pipes shall be carefully laid in the center of the drainage trench, true to the lines and grades given. Bell ends shall face upgrade and all joints shall be tight.

Joints in concrete pipe shall be sealed with cold-applied bituminous sealer, preformed plastic gaskets or flexible, watertight, rubber-type gaskets. Portland cement mortar shall not be used for sealing pipe joints except with permission of the Engineer.

When cold-applied bituminous sealer is used, the bell and spigot ends shall be wiped clean and dry before applying the bituminous sealer to the pipe ends. Before the drainage pipes are placed in contact with each other, the spigot or tongue end shall be completely covered with bituminous sealer; then the pipe shall be laid to line and grade so the inside surface of all abutting pipes are flush. Additional bituminous sealer shall be applied to the joint after the connection has been made to ensure a water tight connection.

Where the end of an existing drainage pipe is not compatible with the end of a proposed concrete pipe, the Contractor shall align the inner diameters of the pipes being connected, butt the pipe ends together, and construct a cast-in-place concrete pipe connection, as shown in the plans. Incompatible bell/spigot or tongue/groove ends shall be cut off as required to ensure the interior drainage pipe walls are aligned to provide a smooth transition between the pipes.

Metal pipe and pipe arches shall be carefully joined and firmly clamped together by approved connecting bands, which shall be properly bolted in place before any backfill is placed.

Newly installed drainage pipe which is not in true alignment, or which shows any settlement or distortion, shall be reinstalled in accordance with 1.05.03.

When newly installed drainage pipe is to be temporarily plugged due to stage construction, it shall be as approved by the Engineer. When the plug is removed, the pipe shall be cleaned and free of debris.

When drainage pipe outside of proposed drainage trench limits is to be removed, it shall be removed to the limits shown on the plans and all remaining pipes shall be plugged with cement masonry.

Where shown on the plans or directed by the Engineer, the Contractor shall plug abandoned existing pipes with cement masonry.

All inlet and outlet pipes shall be set flush with the inside face of the wall as shown on the plans. The pipes shall extend through the exterior walls for a sufficient distance beyond the exterior surface to allow for satisfactory connections, and the concrete or masonry shall be constructed around them neatly to prevent leakage along their exterior surfaces.

(4) Drainage Pipe End Installation: Reinforced concrete drainage pipe ends shall be placed on a prepared bed of the existing ground and accurately aligned as shown on the plans. The joints shall be sealed as specified in 6.86.03-3 and backfill shall be placed around both sides of the unit simultaneously to the elevation shown on the plans.

Metal drainage pipe ends shall be placed on a prepared bed of the existing ground and accurately aligned as shown on the plans. After the attachment of the drainage pipe end, backfill shall be placed around both sides of the unit up to the elevation shown on the plans, exercising caution to avoid displacement or deformation of the unit.

6.86.04—Method of Measurement: This work will be measured as follows:

Drainage Trench Excavation, in accordance with 2.86.04, will not be measured for payment.

Rock in Drainage Trench Excavation will be measured in accordance with 2.86.04.

Bedding Material will not be measured for payment.

New and Re-laid Pipes and Pipe Arches will be measured for payment by the actual number of linear feet of pipe or pipe arch of the various sizes and types, completed and accepted and measured in place along the invert. Coupling bands and fittings for pipes and pipe arches will not be measured for payment.

Reinforced Concrete Drainage Pipe Ends and Metal Drainage Pipe Ends will be measured for payment as separate units.

Corrugated Metal Pipe Elbows (of the Size and Type specified) will be measured for payment by the actual number of linear feet of pipe elbows completed and accepted, based on 6 linear feet per elbow, as shown on the plans. Coupling bands for elbows will not be measured for payment.

Concrete Pipe Connection will be measured for payment by the number of each concrete pipe connection constructed at locations where proposed concrete pipes tie into an existing pipe with an incompatible end, completed and accepted by the Engineer.

Removal of drainage pipe outside of drainage trench excavation limits, as defined in 2.86.03, will be measured for payment by the actual number of linear feet of drainage pipe removed.

There will be no measurement for plugging existing pipes with cement masonry.

6.86.05—Basis of Payment:

Drainage Trench Excavation for the installation of drainage pipes will not be paid separately but shall be included in the Contract unit price for the respective drainage pipe or pipe end item(s), in accordance with the provisions of 2.86.05.

Rock in Drainage Trench Excavation will be paid for in accordance with the provisions of 2.86.05.

Bedding Material necessary for the installation of drainage items described herein will be included in the Contract unit price for the respective drainage pipe or pipe end item(s). Bedding material required to fill voids when rock in drainage trench is encountered will not be measured for payment but shall be included in the Contract unit price for "Rock in Drainage Trench Excavation," in accordance with 2.86.05.

New Pipes and Pipe Arches will be paid for at the Contract unit price per linear foot for "(Size and Type) Pipe (Thickness) – 0' to 10' Deep," "(Size and Type) Pipe (Thickness) – 0' to 20' Deep," "(Size) Pipe Arch (Thickness) – 0' to 10' Deep" or "(Size) Pipe Arch (Thickness) – 0' to 20' Deep" complete in place, including materials, drainage trench excavation, bedding material, equipment, tools, and labor incidental thereto. The work required for connecting new pipe into a drainage structure shall be included in the cost of the pipe.

Relaid Pipes and Pipe Arches will be paid for at the Contract unit price per linear foot for "Relaid Pipe (Size and Type) – 0' to 10' Deep," "Re-laid Pipe (Size and Type) – 0' to 20' Deep," "Relaid Pipe Arch (Size and Type) – 0' to 10' Deep," or "Relaid Pipe Arch (Size and Type) – 0' to 20' Deep," complete in place, including all materials, drainage trench excavation, bedding material, equipment, tools, and labor incidental thereto.

Reinforced Concrete Drainage Pipe Ends and Metal Drainage Pipe Ends will be paid for at the Contract unit price for each drainage pipe end of the Size and Type specified, complete in place, including all excavation, materials, attachment systems, equipment, tools and labor incidental thereto.

Corrugated Metal Pipe Elbows will be paid for at the Contract unit price per linear foot for "(Size and Type) Corrugated Metal Pipe Elbow" including all materials, drainage trench excavation, bedding material, equipment, tools, and labor incidental thereto.

Concrete Pipe Connection will be paid for at the Contract unit price each for "Concrete Pipe Connection" complete in place, including all materials, equipment, tools and labor incidental thereto.

Removal of drainage pipes of all types and sizes, outside of drainage trench excavation limits, as defined in 2.86.03-1, will be paid for at the Contract unit price per linear foot for "Remove Existing Pipe – 0' to 10' Deep," or "Remove Existing Pipe – 0' to 20' Deep," which price shall include excavation, temporary trench protection, backfill, and all equipment, tools and labor incidental thereto.

There will be no direct payment for the plugging of drainage pipes.

Pay Item	Pay Unit
(Size and Type) Pipe (Thickness) – 0' to 10' Deep	l.f.
(Size and Type) Pipe (Thickness) – 0' to 20' Deep	l.f.
(Size and Type) Pipe Arch (Thickness) – 0' to 10' Deep	l.f.
(Size and Type) Pipe Arch (Thickness) – 0' to 20' Deep	l.f.
Relaid (Size and Type) Pipe– 0' to 10' Deep	l.f.
Relaid (Size and Type) Pipe– 0' to 20' Deep	l.f.
(Size and Type) Relaid Pipe Arch – 0' to 10' Deep	l.f.
(Size and Type) Relaid Pipe Arch – 0' to 20' Deep	l.f.
(Size) Reinforced Concrete Drainage Pipe End	ea.
(Size) Metal Drainage Pipe End	ea.
(Size and Type) Corrugated Metal Pipe Elbow	l.f.
Concrete Pipe Connection	ea.
Remove Existing Pipe – 0' to 10' Deep	l.f.
Remove Existing Pipe – 0' to 20' Deep	l.f.

NOTES

SECTION 7.01
DRILLED SHAFTS

7.01.01—Description
7.01.02—Materials
7.01.03—Construction Methods
7.01.04—Method of Measurement
7.01.05—Basis of Payment

7.01.01—Description: This work shall consist of all labor, materials, equipment and services necessary to complete the Drilled Shaft installation in accordance with the Contract. Drilled shafts shall be made primarily of reinforced or unreinforced concrete.

7.01.02—Materials: Drilled Shafts shall be made of the following materials:

1. **Portland Cement Concrete:** Concrete used in the construction of the shaft shall be as shown on the plans, M.03, and shall meet the following requirements:
   - **(a)** The concrete shall have a minimum initial slump of 8 inches.
   - **(b)** The concrete mix shall maintain a slump of no less than 4 inches for a minimum of 3 hours beyond the expected time for placement of concrete and removal of temporary casing (if used), as indicated by trial mixes and physical tests of slump loss. The trial mix and physical tests (slump loss tests) shall be conducted using concrete mix and ambient air temperatures anticipated during concrete placement.
   - **(c)** All admixtures, if approved for use, shall be adjusted for the conditions encountered on the job so as to conform to the slump loss requirements within this specification and must not adversely affect the timing of, taking of or interpretation of any Nondestructive Testing that may be called for in the Contract.
   - **(d)** Coarse aggregate shall meet the gradation requirements specified in Table M.01.02-2 for No. 8 coarse aggregate.
2. **Reinforcing Steel:** Reinforcing steel used in construction of the shaft shall be as specified in M.06.01.
3. **Access Tubes:** Access tubes for cross-hole acoustic logging shall be made of Schedule 40 steel pipe meeting the requirements of ASTM A53, Grade A or B, Type E, F, or S. The tubes' inside diameter shall be at least 1.5 inches. All access tubes, including all pipe joints, shall have a round, regular inside surface free of defects and obstructions in order to permit the free, unobstructed passage of probes to the bottoms of the tubes. The access tubes shall be watertight, free from corrosion and free of deleterious material on the outside that could prevent bonding with the concrete. All access tubes shall be fitted with watertight caps on the bottom and top.
4. **Grout:** Grout used for filling Access Tubes shall meet the requirements of M.03.05. The grout shall have strength properties equivalent to or better than those of the drilled shaft concrete.
5. **Permanent Casing:** Steel casing shall meet the requirements of ASTM A36 or ASTM A252 Grade 2 unless otherwise specified on the plans. Casings shall be smooth, clean, watertight, and of ample strength to withstand handling, installation, and the pressure from surrounding concrete and earth materials. The outside diameter of any casing shall not be less than the specified diameter of the shaft.

7.01.03 - Construction Methods:

1. **Qualifications of Drilled Shaft Contractor and Submittals:** The Contractor performing the work described in this specification shall have been installing drilled shafts of both diameter and length similar to those shown on the plans for a minimum of 3 years prior to the bid date for this Project. The Contractor shall submit a list of projects that it has performed in said 3 years that met these criteria. The list of projects shall contain names and phone numbers of owner's representatives who can verify the Contractors' participation on those projects, and that they meet said criteria.

   As early as possible, and no later than 30 days prior to constructing drilled shafts, the Contractor shall submit to the Engineer an Installation Plan for the shafts.

   This Plan shall provide the following information:

   - **(a)** A list identifying the intended on-site supervisor(s) and drill operator(s), for approval by the Engineer. The on-site supervisor(s) shall have a minimum of 2 years' experience supervising the construction of drilled shafts of a diameter and length similar to those shown on the plans. The drill operator(s) shall have a minimum of 1 years' experience drilling for the construction of drilled shafts of a diameter and length similar to those shown on the plans. The list shall contain a summary of each individual's experience.

     Should the Contractor elect to change any of these intended personnel during construction of the shafts, this same approval process will need to be completed for the new personnel prior to their working on the

Project. The Contractor shall not be compensated for any delays resulting from such changing of personnel.

(b) List of proposed equipment to be used, including cranes, drills, augers, bailing buckets, final cleaning equipment, desanding equipment, slurry pumps, core sampling equipment, tremies or concrete pumps, casing and any other equipment required for construction of the shafts.

(c) Details of overall construction operation sequence and the sequence of shaft construction in bents or groups.

(d) Details of the Contractor's intended shaft excavation methods.

(e) When the use of slurry is anticipated, details of the mix design and its suitability for the subsurface conditions at the Site, mixing and storage methods, maintenance methods, and disposal procedures.

(f) Details of methods for cleaning the shaft excavation.

(g) Details of reinforcement placement, including support and centralization methods.

(h) Details of concrete mix design and test results of both a trial mix and a slump loss test. The tests shall be conducted by an approved testing laboratory, using approved methods to demonstrate that the concrete meets slump loss requirements.

(i) Details of concrete placement, including proposed operational procedures for free fall, tremie or pumping methods, proposed concreting log form and computations for time duration of shaft pour estimates.

(j) Details of casing installation and removal methods. If welding of casing is proposed, submit the welding procedure. All welding shall be done in accordance with the current AWS Structural Welding Code.

(k) Details of methods for removal of obstructions. Obstructions for which the Contractor shall provide details of methods for removal include, but are not necessarily limited to, removal of boulders, concrete, riprap, steel, timber or miscellaneous debris.

(l) Details for any monitoring plan as called for in the Contract.

The Engineer will evaluate the drilled shaft Installation Plan for conformance with the Contract and will then notify the Contractor of any additional information required or changes necessary in order to meet Contract requirements. All procedural approvals given by the Engineer shall be subject to trial in the field and shall not relieve the Contractor of the responsibility to satisfactorily complete Project work as detailed in the plans and specifications. The Contractor shall not commence construction of drilled shafts until the Engineer has approved the Installation Plan.

If integrity or load testing of the drilled shafts is called for by the Contract or the Site conditions, this submittal shall be developed in coordination with and submitted concurrently with working drawing submittals, as required in the testing specifications.

All submittals shall comply with the working drawing submittal requirements outlined in 1.05.02.

2. **Trial Drilled Shaft Installation and Load Testing:** If called for in the Contract, the Contractor shall demonstrate the adequacy of the proposed methods, techniques and equipment by successfully constructing a trial drilled shaft in accordance with these specifications. This trial drilled shaft shall be positioned away from production shafts, in the location shown on the plans or as directed by the Engineer. The trial shaft shall be drilled to the maximum depth shown in the plans. Failure by the Contractor to demonstrate to the Engineer the adequacy of methods and equipment shall be reason for the Engineer to require alterations of the Contractor's equipment or methods in order to prevent results unacceptable under the Contract or to the Engineer. Any additional trial drilled shaft required to demonstrate the adequacy of altered methods or construction equipment shall be at the Contractor's expense. Once the Engineer approves construction of production shafts, no changes will be permitted in the personnel, materials, methods or equipment used by the Contractor in constructing the satisfactory trial drilled shaft, unless the Contractor obtains the Engineer's written approval to do so.

   Unless otherwise shown in the Contract, the trial drilled shaft shall have reinforcing bars, access tubes and concrete placed using the same materials and methods of construction to be used during construction of the production drilled shafts. The trial drilled shaft shall be cut off 2 feet below finished grade and left in place. The disturbed area(s) at the site(s) of the trial drilled shaft(s) shall be restored as nearly as practical to original conditions.

   Should the plans call for load testing of the trial drilled shaft, all necessary loading apparatus, instrumentation and other equipment required for performing the load test will be specified and paid for under a separate item.

   All trial drilled shaft(s) and load test(s) must be completed and accepted by the Engineer prior to construction of any production drilled shafts. In the event that there is more than one trial drilled shaft and load test, the Contractor may begin construction of some of the production drilled shafts, in whatever way that the Engineer requires or approves.

3. **Protection of Existing Structures:** The Contractor shall control drilled shaft operations in a way that will

prevent damage to existing structures or utilities, in accordance with 1.07.09 and 1.07.13. Preventive measures shall include, but are not limited to: selecting construction methods and procedures to prevent caving of the shaft excavation; and that will include monitoring and controlling the vibrations from construction activities such as the driving of casing or sheeting, drilling of the shaft, or from any blasting that the Contract or the Engineer may have permitted.

If monitoring is called for in the Contract, a preconstruction survey of existing facilities shall be performed to establish baseline data, including ambient vibration levels and existing structural defects. In general, monumented survey points shall be established on structures which are located within a distance of either 10 shaft diameters or the estimated shaft depth, whichever is greater. These points shall be monitored by the Contractor for vertical and lateral movement in an approved manner to the accuracy required by the Engineer.

When deformations exceed the predetermined amount included in the plans, the Contractor shall immediately stop work and, if directed by the Engineer to do so, backfill the excavated hole. The Contractor shall be responsible for selecting and using equipment and procedures that keep deformations of existing structures within levels specified by the Contract or Engineer.

When vibrations are to be monitored, the Contractor must engage the services of a professional vibrations consultant to monitor and record vibration levels during drilled shaft construction. Unless the Engineer states otherwise, vibration monitoring equipment must be capable of detecting velocities of 0.1 inch/second or less. When vibration levels exceed tolerable levels established by the Contract or the Engineer, the Contractor shall immediately stop the work causing the vibrations and take whatever measures are necessary to reduce vibration levels to below tolerable levels. All costs related to vibration monitoring required in the Contract shall be included in the bid price for the Drilled Shaft item.

4. **Construction Sequence:** Excavation to footing elevation shall be completed before shaft construction begins unless otherwise noted in the Contract or approved by the Engineer. Any disturbance at or below the footing area caused by shaft installation shall be repaired by the Contractor prior to the footing construction.

   When drilled shafts are to be installed in conjunction with embankment placement, the Contractor shall construct drilled shafts after the placement of fills, unless shown otherwise in the Contract or approved by the Engineer.

   Drilled shafts, constructed prior to the completion of the fills, shall not be capped until the fills have been placed as near to final grade as possible, leaving only the workroom necessary for construction of the caps.

5. **Exploration Test Borings:** As soon as possible, the Contractor shall take soil samples or rock cores, where shown on the plans or as directed by the Engineer, in order to determine the character of the material directly below the completed shaft excavation. The soil samples shall be extracted with a split spoon sampler or undisturbed sample tube. The rock cores shall be cut with an approved triple tube core barrel to a minimum of 10 feet below the bottom of the drilled shaft excavation before the excavation is made. The Engineer may require the depth of coring be extended up to a total depth of 20 feet. Rock core and standard penetration test samples shall be measured, visually identified and described in the Contractor's log. The samples shall be placed in suitable containers, identified by shaft location, elevation, and Project number and shall be delivered with the Contractor's field log to the Engineer within 24 hours after each boring exploration is completed. The Engineer will inspect the samples and log in order to determine the final depth of required excavation based on evaluation of the material’s suitability. The Contractor shall not start shaft drilling or construction of the shafts until the Engineer has determined the final depth of required excavation. Two (2) copies of the Contractor’s final typed log shall be furnished to the Engineer within 7 calendar days after completion of the borings. The logs shall contain specific information about the drilling equipment and tools used and the rate of hole advancement, as well as descriptions of soil, rock, obstructions, and water encountered. The Contractor shall supply a suitable, secure location on the Site for storage of all soil and rock core samples. At no time shall the soil or rock core samples be taken off the Site without the Engineer’s permission to do so.

6. **General Methods and Equipment:** The Contractor shall perform the excavations required for shafts through whatever materials are encountered, to the dimensions and elevations shown in the plans or otherwise required by the Contract. The Contractor's methods and equipment shall be suitable for the intended purpose and materials encountered. The permanent casing method shall be used only at locations shown on the plans or authorized by the Engineer in writing. Blasting shall be permitted only if specifically authorized on the plans or in writing by the Engineer.

7. **Uncased Construction Method:** This method consists of using water or slurry (mineral or polymer) to maintain stability of the borehole perimeter while advancing the excavation to final depth, placing the reinforcing cage, and concreting the shaft. Where drilled shafts are located in open water areas, exterior casings shall be extended from above the anticipated high water elevation into the ground in order to protect the shaft

concrete from water action during placement and curing of the concrete. The exterior casing shall be installed in a manner that will produce a positive seal at the bottom of the casing, so that no piping of water or other materials occurs into or from the shaft excavation.

8. **Casing Construction Method:** The casing method may be used either where shown on the plans or at sites where uncased construction methods are inadequate to prevent hole caving or excessive deformation of the hole. In using this method, the casing may either be placed in a predrilled hole or advanced through the ground by twisting, driving or vibration before being cleaned out.
9. **Excavation and Drilling Equipment:** The Contractor's excavation and drilling equipment shall have adequate capacity, including power, torque and down-thrust to make it possible to excavate a hole of the maximum diameter and to a depth of 20% beyond the depths shown on the plans.

   The excavation and overreaming tools shall be of adequate design, size and strength to perform the work shown in the plans or described herein. When the material encountered cannot be drilled using conventional earth augers with soil or rock teeth, drill buckets, grooving tools, or underreaming tools, the Contractor shall provide special drilling equipment, including but not limited to: rock core barrels, rock tools, air tools, blasting materials, or other equipment as necessary to enable construction of the shaft excavation to the size and depth required. Excavation by blasting may not be performed without the prior written approval of same by the Engineer.
10. **Excavation:** Shaft excavations shall be made at locations and to the top of shaft elevations, estimated bottom of shaft elevations, shaft geometry and dimensions shown in the Contract. If material encountered during excavation is unsuitable for these purposes or differs from that anticipated in the design of the drilled shaft, the Contractor shall extend drilled shaft tip (base) elevations as and when the Engineer directs it to do so.

    The Contractor shall maintain a construction method log during shaft excavation. The log shall contain information such as the description and approximate top and bottom elevation of each soil or rock material encountered, of seepage or ground water, and any other relevant information or observations, including a description of the tools and drill rigs used and any changes necessitated by changing ground conditions.

    The Contractor shall dispose of any excavated materials removed from shaft excavations in accordance with the applicable Contract requirements for disposal of excavated materials, including those in 1.10.

    The Contractor shall not permit workers to enter the shaft excavation for any reason unless:

    (1) a suitable casing has been installed and the water level has been lowered and stabilized below the level to be occupied, and

    (2) adequate safety equipment and procedures have been provided to workers entering the excavation.

    Any placement of workers within the shaft excavation shall be in accordance with OSHA regulations and industry standards.
11. **Drilled Shaft Earth Excavation:** Drilled shaft earth excavation is:

    (1) excavation accomplished with conventional tools such as augers and drilling buckets attached to drilling equipment of the size, power, torque, and down-thrust (crowd) proposed by the Contractor in the Installation Plan approved by the Engineer or

    (2) successful construction of a trial drilled shaft.

    Earth excavation may include, but will not necessarily be limited to, excavation of clay, silt, sand, gravel, cobbles, boulders, weathered rock, and miscellaneous fill.
12. **Drilled Shaft Rock Excavation:** Drilled shaft rock excavation is:

    (1) excavation of competent rock, accomplished with conventional rock drilling tools, such as core barrels attached to drilling equipment of the size, power, torque, and down-thrust (crowd) as proposed by the Contractor in the approved Installation Plan, or

    (2) successful construction of a trial drilled shaft.

    Top of competent rock is as shown on the Contract drawings.
13. **Obstructions:** When obstructions are encountered, the Contractor shall notify the Engineer of them immediately. Obstructions are defined as impenetrable objects that:

    (a) cannot be removed or excavated using conventional augers fitted with soil or rock teeth, underreaming tools, or drilling buckets; or

    (b) cause a significant decrease in the rate of excavation advancement, relative to the rate of advancement for the rest of the shaft excavation within the particular strata where the obstruction is located that had been achieved using the same techniques and equipment that had previously been used successfully to excavate the shaft.

    The Engineer will be the sole judge of the significance of any reduced rate of shaft advancement and of the classification of obstruction excavation. The Engineer will be present at the site of the obstruction in order to

evaluate obstructions, to authorize measures for dealing with them, and to approve the designation of each obstruction. Sloping bedrock or bedrock that is higher than anticipated by the plans shall not be considered as requiring obstruction excavation. Shallow obstructions are obstructions located within 5 feet of the top level of the shaft. Shallow obstructions at shaft locations shall be removed at the Contractor's expense.

The Contractor shall remove all subsurface obstructions at drilled shaft locations. Such obstructions may include man-made materials, such as concrete foundations, and natural materials, such as boulders. Subsurface obstruction removal special procedures/tools may include, but are not limited to, chisels, boulder breakers, core barrels, down-the-hole hammers, air tools, hand excavation, temporary casing, and increases of the hole diameter. Blasting shall not be permitted unless approved in advance in writing by the Engineer.

14. **Lost Tools:** Drilling tools lost in the excavation shall not be considered obstructions and shall be promptly removed by the Contractor without compensation. All costs due to lost tool removal shall be borne by the Contractor including, but not limited to, costs associated with the repair of hole degradation due to removal operations or due to the hole's remaining open for an excessively long time.
15. **Casing:** Casings shall be steel, smooth, clean, watertight, and of ample strength to withstand both handling and installation and the pressure of both concrete and the surrounding earth materials. The outside diameter of casings shall not be less than the specified diameter of the shaft, and the outside diameter of any excavation made below the casing shall not be less than the specified diameter of the shaft. No extra compensation will be paid for concrete required to fill an oversized casing or oversized excavation. All casings, except permanent casings, shall be removed from shaft excavations. Any length of permanent casing installed below the shaft cutoff elevation shall remain in place.

    When the shaft extends above ground or through a body of water, the portion exposed above ground or through the water may be formed with removable casing, except when permanent casing is specified. Removable casing shall be stripped from the shaft in a manner that will not damage the concrete. Casings may be removed when the concrete has attained sufficient strength, provided: curing of the concrete is continued for a 72 hour period; the shaft concrete is not exposed to salt water or moving water for 7 days; and the concrete reaches a compressive strength of at least 2500 psi as determined from concrete cylinder breaks.
16. **Temporary Casing:** All subsurface casing shall be considered temporary unless specifically shown as permanent casing in the Contract. The Contractor shall be required to remove temporary casing before or immediately after completion of concreting the drilled shaft. Casing shall never be pulled after the concrete begins to set, due to probable entrapment of drilling fluid in the shaft concrete and probable separation of the concrete within the shaft.

    If the Contractor elects to remove a casing and substitute a longer or larger diameter casing through caving soils, the excavation shall either be stabilized with slurry or backfilled before the new casing is installed. Other methods, approved by the Engineer, may be used to control the stability of the excavation and protect the integrity of foundation materials.

    Before the casing is withdrawn, the level of fresh concrete in the casing shall be a minimum of 5 feet above either the hydrostatic water level in the formation or the level of drilling fluid in the annular space behind the casing, whichever is higher. As the casing is withdrawn, care shall be exercised to maintain an adequate level of concrete within the casing, so that fluid trapped behind the casing is displaced upward and discharged at the ground surface without contaminating or displacing the shaft concrete.

    Temporary casings that become bound or fouled during shaft construction and cannot practicably be removed shall constitute a defect in the drilled shaft. The Contractor shall be responsible to improve such defective shafts to the satisfaction of the Engineer. Improvement may consist of, but not be limited to, removing the shaft concrete and extending the shaft deeper in order to compensate for loss of frictional capacity in the cased zone; providing straddle shafts to compensate for capacity loss; grouting around the exterior of the shaft; or providing a replacement shaft. All corrective measures, including redesign of footings caused by defective shafts, shall be done to the satisfaction of the Engineer by the Contractor without either compensation or an extension of Contract time of the Project. In addition, no compensation will be paid for casing remaining in place.
17. **Permanent Casing:** Permanent casing shall be used where specified by the Contract. The casing shall be continuous between top and bottom elevations as shown in the plans. After installation is complete, the permanent casing shall be cut off at the prescribed elevation.

    In cases in which special temporary casings are shown on the plans or authorized in writing by the Engineer to be used in conjunction with permanent casing, the Contractor shall maintain both alignment of the temporary casing with the permanent casing and a positive, watertight seal between the two casings during excavation and concreting operations.

    Permanent casing shall maintain close contact with the surrounding earth after installation. Use of an

oversized hole or temporary casing outside the permanent casing beneath the ground surface will not be allowed without the advance written permission of the Engineer to do so. Should an oversized hole or temporary casing outside the permanent casing beneath the ground surface be allowed by the Engineer, grouting of the exterior annular space shall be provided by the Contractor in order to create close contact between the casing and the surrounding ground. The grouting shall extend from the bottom of the annular space to an elevation determined by the Engineer. No compensation will be paid to the Contractor for grouting of the exterior annular space.

18. **Slurry:** Mineral or polymer slurries shall be employed when slurry is used in the drilling process, unless other drilling fluids are approved in writing by the Engineer. Mineral slurry shall have both a mineral grain size that will remain in suspension and sufficient viscosity and gel characteristics to allow it to transport excavated material to a suitable screening system. The percentage and specific gravity of the material used to make the mineral suspension shall be sufficient to maintain the stability of the excavation and to allow proper concrete placement.

During construction, the level of the slurry shall be maintained at a height sufficient to prevent caving of the hole. The slurry head shall remain above the piezometric head of the groundwater. This includes initial drilling of the borehole down to the piezometric level. Slurry shall be introduced when the depth of the borehole is still above the piezometric level, not after the inflow of water can be detected and sloughing has begun. In the event of a sudden significant loss of slurry to the hole, the construction of that foundation shall be stopped until either a method to stop slurry loss or an alternate construction procedure has been approved by the Engineer.

Mineral slurry shall be premixed thoroughly with clean fresh water and adequate time (as prescribed by the mineral manufacturer) shall be allotted for hydration prior to introduction into the shaft excavation. Slurry tanks of adequate capacity shall be required for slurry circulation, storage, and treatment. No excavated slurry pits will be allowed in lieu of slurry tanks without the written consent of the Engineer to the substitution. Desanding equipment shall be provided by the Contractor as necessary to keep slurry sand content at less than 4% by volume at any point in the borehole at the time the slurry is introduced, including situations in which temporary casing will be used. The Contractor shall take all steps necessary to prevent the slurry from "setting up" in the shaft. Such methods may include, but are not limited to: agitation, circulation and adjusting the properties of the slurry. The Contractor shall dispose of all slurry in suitable off-Site areas. Disposal of the slurry shall also comply with 1.10.

Control tests using suitable apparatus shall be carried out on the mineral slurry by the Contractor in order to determine density, viscosity and pH. An acceptable range of values for mineral slurry physical properties is shown in Table 7.01-1:

TABLE 7.01-1, MINERAL SLURRY PROPERTIES

(Sodium Bentonite or Attapulgite in Fresh Water)

	Acceptable Range of Values		
Property	**At Time of Slurry Introduction**	**In Hole at Time of Concreting**	**Test Method**
Density - pcf	64.3* - 69.1*	64.3* - 75.0*	Density Balance
Viscosity - sec./quart	28 – 45	28 – 45	Marsh Funnel
pH	8 - 11	8 - 11	pH paper, pH meter

* Increase by 2 pcf in salt water

Notes:

(a) Tests shall be performed when the slurry temperature is above 40°F.

(b) If desanding is required, sand content shall not exceed 4% (by volume) at any point in the borehole, as determined by the American Petroleum Institute sand content test when the slurry is introduced.

Tests to determine density, viscosity and pH value shall be performed during the shaft excavation to establish a consistent working pattern. A minimum of 4 sets of tests shall be made during the first 8 hours of slurry use. When the tests show consistent results, the testing frequency may be decreased to 1 set every 4 hours of slurry use.

If the Contractor proposes to use polymer slurry, either natural or synthetic, use of the product must be approved in advance by the Engineer. Slurry properties at the time of mixing and at the time of concreting must comply with the manufacturer's written recommendations. Whatever product is used, the sand content at the base of the drilled shaft excavation shall not exceed 1% when measured by Method API 13B-1, Section 5,

immediately prior to concreting.

If the Contractor proposes to use blended mineral-polymer slurry, the Contractor shall submit to the Engineer a detailed report specific to the Project, prepared and signed by a qualified slurry consultant, describing the proposed slurry materials, the mix proportions, mixing methods and quality control methods.

If polymer slurry, or blended mineral-polymer slurry, is proposed, the Contractor's slurry management plan shall include detailed provisions for controlling the quality of the slurry, including tests to be performed, the frequency of those tests, the test methods, and any maximum or minimum property requirements that must be met in order to ensure that the slurry meets its intended functions in the subsurface conditions at the Project site and with the construction methods to be used. The slurry management plan shall include a set of the slurry manufacturer's written recommendations and shall include the following tests, as a minimum: Density test (API 13B-1, Section 1), viscosity test (Marsh funnel and cup, API 13B-1, Section 2.2, or approved viscometer), pH test (pH meter, pH paper), and sand content test (API sand content kit, API 13B-1, Section 5).

If such a proposal is approved by the Engineer, the Contractor may use water as a drilling fluid. In that case, all of the provisions in Table 7.01-1 for mineral slurries must be met, except that the maximum density shall not exceed 70 pcf.

The Contractor shall ensure that a heavily-contaminated slurry suspension, which could impair the free flow of concrete, has not accumulated in the bottom of the shaft. Prior to placing concrete in any shaft excavation, the Contractor shall take slurry samples using a sampling tool approved by the Engineer. Slurry samples shall be extracted from the base of the shaft and at intervals not exceeding 10 feet up the slurry column in the shaft, until 2 consecutive samples produce acceptable values for density, viscosity, and pH.

When any slurry samples are found to be unacceptable, the Contractor shall take whatever action is necessary to bring the slurry within specification requirements. Concrete shall not be placed until the slurry in the hole is re-sampled and test results produce acceptable values.

Reports of all tests specified above, signed by an authorized representative of the Contractor, shall be furnished to the Engineer on completion of each drilled shaft.

During construction, the level of mineral or blended mineral-polymer slurry in the shaft excavation shall be maintained at a level not less than 4 feet above the highest expected piezometric pressure head along the depth of the shaft, and the level of polymer slurry shall be maintained at a level not less than 6 feet above the highest expected piezometric pressure head along the shaft. If at any time, in the opinion of the Engineer, the slurry construction method fails to produce the desired final results, the Contractor shall discontinue this method and propose an alternate method for approval by the Engineer.

Drilling tools shall contain vents to stabilize hydrostatic pressure above and below the tool during insertion and extraction. The rate of tool extraction shall not cause any noticeable turbulence in the slurry column in the borehole.

The Contractor shall retain the slurry manufacturer's technical representative to be present at the Site during Project startup or throughout the entire Project if continual difficulty is expected, in order to ensure that the slurry is mixed and managed properly.

19. **Excavation Inspection:** The Contractor shall check the dimensions and alignment of each shaft excavation. Final shaft depths shall be measured with a suitable weighted tape or other approved method after final cleaning. The Contractor shall provide equipment and access to the Engineer for confirming dimension, alignment, and bottom cleanliness. Acceptable shaft cleanliness will be determined by the Engineer.
20. **Construction Tolerances:** The following construction tolerances apply to drilled shafts, unless otherwise stated in the Contract:
    - **(a)** The center of the drilled shaft shall be within 3 inches of plan position in the horizontal plane at the plan elevation for the top of the shaft.
    - **(b)** The vertical alignment of a vertical shaft excavation shall not vary from the plan alignment by more than 1/4 inch/foot of depth.
    - **(c)** After the concrete is placed, the top of the reinforcing steel cage shall be no more than 6 inches above and no more than 3 inches below plan position.
    - **(d)** All casing diameters shown on the plans refer to outside diameter ("OD") dimensions. The dimensions of casings are subject to American Petroleum Institute tolerances applicable to regular steel pipe. The Contractor may elect to provide a casing larger in diameter than shown in the plans, if the Engineer approves to do so.
    - **(e)** The top elevation of the shaft shall have a tolerance of plus 1 inch or minus 3 inches from the plan top-of-shaft elevation.
    - **(f)** Excavation equipment and methods shall be designed so that the completed shaft excavation will have a

planar bottom. The cutting edges of excavation equipment shall be normal to the vertical axis of the equipment within a tolerance of +/- 3/8 inch/foot of diameter.

Drilled shaft excavations and completed shafts not constructed within the required tolerances are unacceptable. The Contractor shall be responsible for correcting all unacceptable shaft excavations and completed shafts to the satisfaction of the Engineer. Materials and work necessary, including engineering analysis and redesign, in order to complete corrections for out-of-tolerance drilled shaft excavations, shall be furnished without cost to the State or extension of Contract time.

21. **Reinforcing Steel Cage Construction and Placement:** The reinforcing steel cage, consisting of longitudinal bars, ties, cage stiffener bars, spacers, centralizers, and other necessary appurtenances, shall be completely assembled and placed as a unit immediately after the shaft excavation is inspected and accepted, and prior to concrete placement. Internal stiffeners shall be removed as the cage is placed in the borehole, so as not to interfere with the placement of concrete.

    The reinforcing steel in the shaft shall be tied and supported so that the reinforcing steel will remain within allowable tolerances. Concrete spacers or other approved noncorrosive spacing devices shall be used at sufficient intervals near the bottom and at intervals not exceeding 10 feet up the shaft, in order to ensure concentric spacing for the entire cage length. Spacers shall be constructed of approved material, equal in quality and durability to the concrete specified for the shaft. The spacers shall be of adequate dimension to ensure a minimum 3 inch annular space between the outside of the reinforcing cage and the side of the excavated hole. Approved cylindrical concrete feet (bottom supports) shall be provided to ensure that the bottom of the cage is maintained the proper distance above the base.

    The elevation of the top of the steel cage shall be checked before and after the concrete is placed. If the upward displacement of the rebar cage exceeds 2 inches or if the downward displacement exceeds 6 inches per 20 feet of shaft length, the drilled shaft will be considered defective. In such a case, corrections shall be made by the Contractor to the satisfaction of the Engineer. No additional shafts shall be constructed until the Contractor has modified the rebar cage support in a manner satisfactory to the Engineer.

22. **Concrete Placement:** Concrete placement shall be performed in accordance with the applicable portions of 6.01 and the requirements herein dealing with concrete materials.

    Concrete shall be placed as soon as possible after reinforcing steel placement and after the Engineer has accepted the cleanliness of the shaft. The Engineer may re-inspect the shaft for cleanliness should there be any delays between initial acceptance of shaft cleanliness and commencement of the concrete placement. If during such a delay the Engineer has determined that shaft cleanliness has deteriorated, the Engineer may require the Contractor to re-clean the shaft. The Contractor may be required to remove the rebar cage should it be necessary in order to achieve the required shaft cleanliness. The Contractor will not be compensated for any cost or loss of time due to the need to re-clean the shaft.

    Concrete placement shall be continuous from the bottom to the top elevation of the shaft. Concrete placement shall continue after the shaft excavation is filled and good quality concrete is evident at the top of shaft. Concrete shall be placed by free fall, or through a tremie or concrete pump. Free fall placement will be permitted only in dry holes. Concrete placed by free fall shall fall directly to the base without contacting the rebar cage or hole sidewall. Drop chutes may be used to direct concrete to the base during free fall placement.

    The Contractor shall maintain Concreting Logs during all concrete placement. The log shall include, but not be limited to, concreting curves plotting Depth-to-Top of Concrete vs. Volume of Concrete Placed (for both theoretical and actual volumes of concrete placed). The Contractor shall provide a copy of each log to the Engineer upon completion of each drilled shaft concrete placement. A sample of the proposed log to be used by the Contractor shall be submitted as part of the Installation Plan working drawing submittal.

23. **Tremies:** Tremies may be used for concrete placement in either wet or dry holes. Tremies used to place concrete shall consist of a tube of sufficient length, weight, and diameter to discharge concrete at the shaft base elevation. The tremie shall not contain aluminum parts that may come in contact with the concrete. The tremie's inside diameter shall be at least 6 times the maximum size of aggregate used in the concrete mix, but shall not be less than 10 inches. The inside and outside surfaces of the tremie shall be clean and smooth in order to permit flow of concrete and unimpeded withdrawal during concreting. The wall thickness of the tremie shall be adequate to prevent crimping or sharp bends, which would restrict concrete placement.

    The tremie used for wet excavation concrete placement shall be watertight. Underwater or under-slurry placement shall not begin until the tremie is placed to the shaft base elevation, and the concrete shall be kept completely separated from the water or slurry prior to the time that it is discharged. Valves, bottom plates or plugs may be used for this purpose only if concrete discharge can begin within 1 tremie diameter of the base of the drilled shaft. Plugs shall either be removed from the excavation or be of a material, approved by the

Engineer that will not cause a defect in the shaft if not removed. The discharge end of the tremie shall be constructed to permit the free radial flow of concrete during placement operations. The tremie discharge end shall be immersed at least 5 feet in concrete at all times after starting the flow of concrete. The flow of the concrete shall be continuous. The level of the concrete in the tremie shall be maintained above the level of slurry or water in the borehole at all times, in order to prevent water or slurry intrusion into the shaft concrete.

If at any time during the concrete placement, the tremie line orifice is removed from the fluid concrete column and it discharges concrete above the rising concrete level, the shaft will be considered defective. All costs of repair or replacement of defective shafts shall be the responsibility of the Contractor.

24. **Pumped Concrete:** Concrete pumps and lines may be used for concrete placement in either wet or dry excavations. All pump lines shall have a minimum 4 inch diameter and be constructed with watertight joints. Concrete placement shall not begin until the pump line discharge orifice is at the shaft base elevation.

    For wet excavations, a plug or similar device shall be used to separate the concrete from the fluid in the hole until pumping begins. The plug shall either be removed from the excavation or be of a material approved by the Engineer that will not cause a defect in the shaft if not removed.

    The discharge orifice shall remain at least 5 feet below the surface of the fluid concrete. When lifting the pump line during concreting, the Contractor shall temporarily reduce the line pressure until the orifice has been repositioned at a higher level in the excavation.

    If at any time during the concrete placement the pump line orifice is removed from the fluid concrete column and it discharges concrete above the rising concrete level, the shaft will be considered defective. All costs of repair or replacement of defective shafts shall be the responsibility of the Contractor.

25. **Drop Chutes:** Drop chutes may be used to direct placement of free-fall concrete in excavations where the maximum depth of water does not exceed 3 inches. Free fall placement is not permitted in wet excavations. Drop chutes shall consist of a smooth tube of either one-piece construction or sections that can be added and removed. A drop chute can also be a hopper with a short tube to direct the flow of concrete. Concrete may be placed through either the hopper at the top of the tube or side openings as the drop chute is retrieved during concrete placement. If concrete placement causes the shaft excavation to cave or slough, or if the concrete strikes the rebar cage or sidewall, the Contractor shall reduce the height of free fall or reduce the rate of concrete flow into the excavation, or both. If caving or sloughing of the borehole walls occurs during free-fall placement of concrete, the shaft will be considered defective. All costs of repair or replacement of defective shafts shall be the responsibility of the Contractor. If concrete placement cannot be satisfactorily accomplished by free fall, in the opinion of the Engineer, the Contractor shall use either tremie or pumping techniques to accomplish the concrete placement.

26. **Access Tubes for Crosshole Acoustic Logging:** Access tubes for crosshole acoustic logging shall be placed on each reinforcing cage designated in the Contract in the position and at the frequency shown on the plans. Access tubes must be firmly secured to the cage. Normally, the tubes shall extend from 6 inches above the bottom of the shaft to at least 3 feet above the top of the shaft, or 2 feet above the ground surface if the shaft is cut off below the ground surface. If cross-hole acoustic tests are to be performed, the access tubes shall be filled with clean water no later than 4 hours after placement of the concrete and the tubes capped during concrete placement to keep out concrete and debris. In all cases, the access tubes shall be as nearly parallel as possible and be placed as far from the longitudinal steel bars as possible.

    Prior to the beginning of downhole logging, the Contractor shall assure that the Cross-Hole Acoustic Logging test probes can pass through every tube to the bottom. If a tube is obstructed, the entire length of the obstructed access tube will not be measured for payment. The Engineer may also require the Contractor to core a hole within the drilled shaft near and to the full depth of the obstructed tube. The cored hole shall be large enough to accommodate the test probe for the full length of the hole. The coring equipment, coring procedure and location of the core hole shall be approved by the Engineer before the coring process may begin. The coring method shall provide for complete core recovery and shall minimize abrasion and erosion of the core. The core hole shall be placed at a position in the shaft that will not produce damage to the reinforcing steel in the shaft. The core hole shall be logged, voids or defects indicated on the log, and the log submitted to the Engineer. Cores shall be preserved and made available for inspection by the Engineer. The core hole will be treated as an access tube for downhole testing. Core holes that are drilled to substitute for a blocked access tube shall be measured and paid for at the Contract unit price for Access Tubes.

    Upon completion of all tests involving access tubes and after acceptance of the drilled shaft, the access tubes and core holes shall be filled with grout.

27. **Evaluation and Acceptance or Rejection of Drilled Shafts:** Upon completion and integrity testing (if called for) of a drilled shaft, the Engineer will review all available drilling logs, drilled shaft construction logs,

concreting logs, inspection reports, load test results, and integrity test results in order to determine the acceptability of the drilled shaft. If the Engineer determines that available data is inconclusive, the Engineer may call for additional integrity testing, coring, or other appropriate actions necessary for evaluating the drilled shaft. Should the additional integrity testing or coring confirm the presence of anomalies, the Contractor will not be compensated for the cost of the additional integrity testing or coring (even if the anomalies are determined to be non-critical and the shaft is found to be acceptable). Should additional integrity testing or coring demonstrate that anomalies are not present (prior to any remedial work), the additional integrity testing or coring will be paid for by the Department. The Contractor may continue to construct drilled shafts before receipt of notice of acceptance of the tested shaft or shafts by the Engineer. If the Engineer finds previously-constructed shaft(s) to be unacceptable, the Contractor shall be required to repair, its expense, the unacceptable shaft(s) to the satisfaction of the Engineer. The Contractor shall prove to the satisfaction of the Engineer, at no expense to the State, the acceptability of all shafts constructed since the time that the unacceptable shaft was constructed and to prove the acceptability of the procedure to be used in constructing future shafts. If the Engineer deems the construction procedure to be unacceptable, the Contractor shall cease all drilled shaft construction until submittal of a new construction procedure to the Engineer and the Engineer has accepted it.

The Contractor shall submit repair procedures to the Engineer for review and approval. If these plans involve change of or impact on the structural design of the shafts or shaft caps, or the geometry of the shafts, any proposed redesign of the Contractor's plan shall be performed at the Contractor's expense by a qualified Professional Engineer registered in the State of Connecticut.

The Engineer may require that additional shafts be tested. If the testing of the additional shaft(s) indicates the presence of a defect in any additional shaft, the testing cost for that shaft shall be borne by the Contractor, and the Contractor shall repair the shaft at its own expense, as above. Any additional testing required by the Engineer on repaired drilled shafts shall be considered part of the Contractors remediation plan, to be paid for by the Contractor.

7.01.04—Method of Measurement:

1. **Furnishing Drilled Shaft Drilling Equipment:** There will be no measurement of the work performed under this Lump Sum item.
2. **Drilled Shaft** will be measured for payment by the length in linear feet of the completed and accepted concrete drilled shaft, of the diameter and containing the reinforcement shown on the plans. The length will be determined as the difference between the plan top of shaft elevation and the final bottom of shaft elevation.
3. **Drilled Shaft Earth Excavation** will be measured for payment by the length in linear feet of completed earth excavation of the diameter shown on the plans (measured along the centerline of the shaft), either from the top of existing grade elevation prior to drilling or from the bottom of the drilled shaft cap elevation (whichever is lower), to either the top of competent rock elevation (if the drilled shaft extends onto or into competent rock) or to the bottom of the shaft elevation (if the shaft does not extend onto or into competent rock).
4. **Drilled Shaft Rock Excavation** will be measured for payment by the length in linear feet of completed rock excavation of the diameter shown on the plans, measured along the centerline of the shaft from the top of competent rock elevation to the bottom of the shaft elevation.
5. **Obstructions** will be measured for payment, after designation as an obstruction by the Engineer, by the number of hours of work, or fraction thereof per obstruction, required to remove the obstruction.
6. **Trial Drilled Shaft** will be measured for payment by the authorized linear feet of trial shaft holes drilled to the diameter shown on the plans, completed (including backfill and restoration of area, when required) and accepted. The length of trial shaft holes will be determined as the difference between the existing ground surface elevation at the center of the trial shaft hole prior to drilling and the authorized bottom elevation of the hole.
7. **Exploration Test Borings** will be measured for payment by the length in linear feet, measured from the existing grade elevation to the bottom elevation of the exploration hole, for each authorized exploration boring drilled.
8. **Permanent Casing** will be measured for payment by the length in linear feet of each diameter casing installed and accepted. The length to be paid will be measured along the casing from the top of the shaft elevation or the top of the casing, whichever is lower, to the bottom of the casing at each shaft location where permanent casing is used.
9. **Access Tubes** will be measured for payment by the length in linear feet of unobstructed access tube, installed and accepted in the drilled shafts, to the depths shown on the plans.

7.01.05—Basis of Payment:

1. **Furnishing Drilled Shaft Drilling Equipment:** Payment for this item will be at the Contract lump sum price for "Furnishing Drilled Shaft Drilling Equipment" which will be considered full and complete payment for furnishing and moving the drilling equipment to the Site, setting up the equipment at the required locations, and removing the equipment from the Site.

   Payment of 60% of the lump sum amount bid for this item will be made when all drilling equipment is on the Site, assembled and ready to drill foundation shafts. Payment of the remaining 40% of the lump sum amount will be made when all shafts have been drilled, all shaft concrete has been placed to the top of the shaft, all defects are repaired, and all drilled shafts have been accepted by the State.
2. **Drilled Shaft:** Drilled shafts will be paid for at the Contract unit price per linear foot for "Drilled Shaft (Diameter)" complete and accepted in place, including submittals, concrete and reinforcing steel, all labor, materials, equipment, temporary casings, slurry, slurry technical representative, blasting (if allowed), protection of existing facilities or utilities, vibration monitoring and incidentals necessary to complete the drilled shaft.
3. **Drilled Shaft Earth Excavation:** This work will be paid for at the Contract unit price per linear foot for "Drilled Shaft Earth Excavation (Diameter)" complete, including all labor, equipment, materials, water control, and disposal of excavated material necessary.
4. **Drilled Shaft Rock Excavation:** Drilled shaft rock excavation will be paid for at the Contract unit price per linear foot for "Drilled Shaft Rock Excavation (Diameter)" complete, including all labor, equipment, materials, water control, and disposal of excavated material necessary. No payment will be made for additional rock excavation or placement of additional shaft concrete resulting from blasting overbreak (if blasting is allowed).
5. **Obstructions:** Removal of obstructions will be paid for at the Contract unit price per hour for "Obstructions" complete, including all labor, equipment, materials, excavation of obstructions, water control, and disposal of excavated material necessary. If the Contractor chooses to use a larger shaft diameter for obstruction excavation, no additional compensation will be paid for performing such oversized obstruction excavation.
6. **Trial Drilled Shaft:** Trial drilled shafts will be paid for at the Contract unit price per linear foot for "Trial Drilled Shaft (Diameter)" complete and accepted, including all labor, equipment, materials, excavation of the trial drilled shaft through whatever materials are encountered, to the bottom of shaft elevation shown on the plans or as authorized by the Engineer (using slurry approved by the Engineer as necessary), providing inspection facilities, backfilling the hole, restoring the Site as required, and all other expenses to complete the trial.
7. **Exploration Test Borings:** Soil samples, rock cores or both, of the diameter and length required and authorized by the Engineer will be paid for at the Contract unit price per linear foot for "Exploration Test Boring" complete, including drilling, extracting, packaging and classifying samples or cores, delivery of same to the Engineer, furnishing concrete or grout to fill the core hole, providing a written log of the hole, and all other expenses necessary.
8. **Permanent Casing:** Permanent casings will be paid for at the Contract price per linear foot for "Permanent Casing (Diameter)" complete, including furnishing and placing the permanent casing in the shaft excavation.
9. **Access Tubes:** Access tubes will be paid for at the Contract unit price per linear foot of unobstructed "Access Tubes" complete and accepted, installed in the drilled shafts to the depths shown on the plans, including the post-test grouting of the access tubes.

Pay Item	Pay Unit
Furnishing Drilled Shaft Drilling Equipment	l.s.
Drilled Shaft (Diameter)	l.f.
Drilled Shaft Earth Excavation (Diameter)	l.f.
Drilled Shaft Rock Excavation (Diameter)	l.f.
Obstructions	hr.
Trial Drilled Shaft (Diameter)	l.f.
Exploration Test Boring	l.f.
Permanent Casing (Diameter)	l.f.
Access Tubes	l.f.

NOTES

SECTION 7.02
PILES

7.02.01—Description
7.02.02—Materials
7.02.03—Construction Methods
7.02.04—Method of Measurement
7.02.05—Basis of Payment

7.02.01—Description: This item shall consist of furnishing and driving foundation piles of the type and dimensions designated. Piles shall be installed at the location, and to the elevation, penetration and capacity shown on the plans, or as directed by the Engineer. If specified in the plans or directed by the Engineer, piles shall be tipped, shaped, reinforced or otherwise pointed and strengthened.

Test piles shall be piles of the type specified, driven in advance of placing orders for the piles, for the purpose of determining length or bearing capacity of piles. The Contractor shall furnish the piles in accordance with an itemized order list which will be furnished by the Engineer, showing the number and length of all piles. When test piles are specified, the pile lengths shown on the plans are for estimating purposes only. The actual lengths to be furnished for production piles will be determined by the Engineer after the test piles have been driven.

7.02.02—Materials: Piles of the type indicated on the plans shall meet the requirements of M.09.02 and M.14.01.

7.02.03—Construction Methods:

1. Pile Types:

(a) Timber Piles: The method of storing and handling timber piles shall be in accordance with AWPA M4 Standards. Special care shall be taken to avoid damaging the surface of treated piles. Cant dogs, hooks, or pike-poles shall not be used. Cuts or breaks in the surface of treated piling shall be addressed by the Contractor.

(**b) Steel Piles:** The methods of storing and handling steel piles shall be such as to prevent damage to the piles and to protect them from corrosion.

(c) Cast-In-Place Concrete Piles: Cast-in-place concrete piles shall be constructed by driving steel shells and filling them with concrete. Shells shall be continuously or incrementally tapered, or cylindrical, or a combination of continuously or incrementally tapered lower sections, which are extended with cylindrical upper sections, unless otherwise provided in the plans or special provisions. The tapered portion of piles shall have a minimum tip diameter of 8 inches and shall change in diameter not less than 1 inch every 12 feet. Cylindrical piles and the cylindrical extension portions of tapered piles shall have a minimum diameter of 12 inches. Shells for cast-in-place concrete piles shall be formed by joining sections of the same manufacture, unless otherwise permitted by the Engineer. Composite shell piles, which are piles composed of different thicknesses or of different manufacture, shall not be used unless shown on the plans or approved by the Engineer. Prefabricated driving points or other type tip enclosures shall be subject to the approval of the Engineer.

The Contractor shall furnish shells of a type and gage which can be driven without distortion. Shells which fail, fracture or otherwise distort during driving or after driving shall be withdrawn or replaced at the Contractor's expense. The metal of shells which are to be driven without a mandrel shall be of sufficient thickness to withstand the driving without failure, fracture or distortion, but in no case shall the thickness be less than No. 7 gage. Shells driven with a mandrel shall have a thickness not less than No. 18 gage. Piles having a shell thickness less than No. 9 gage shall be reinforced as shown on the plans.

Composite shell piles formed by extending lower sections of No. 7 or heavier gage, with upper sections of lighter than No. 7 gage, shall be driven with an internal mandrel in such a manner so as to insure shell alignment and maximum hammer energy transmission throughout the pile shell length. All details concerning compatibility of shell and mandrel construction shall be subject to the approval of the Engineer.

After driving has been completed, the shell shall be inspected and approved before any concrete is placed. The Contractor shall provide suitable lights and other equipment necessary to inspect each shell throughout its length.

All seams, joints and splices in shells shall develop the full strength of the shell and shall be watertight. Damaged shells that are unacceptable to the Engineer shall be filled with sand and a replacement shell or shells shall be driven adjacent thereto.

Reinforcement shall be placed in accordance with the requirements of the plans or special provisions.

No concrete shall be placed in a pile until all driving within a radius of 15 feet from the pile has been completed, or until all the shells for any one bent have been completely driven. If this is not practical, all driving within the above limits shall be discontinued until the concrete in the last pile cast has set at least 7 days.

Concrete shall be placed continuously in each pile, care being used to fill every part of the shell, and to work concrete around the reinforcement without displacing it. No concrete shall be placed in shells containing an accumulation of water or any foreign material.

Extensions, or "build-ups" on concrete piles, shall be avoided; but when necessary, they shall be made as specified in 7.02.03-7.

(d) Prestressed Concrete Piles (Pretensioned): The piles shall be manufactured in accordance with the provisions of 5.14.03, except as follows:

(1) Forms: The forms for the piles shall be of substantial construction and shall produce a uniformly smooth surface on all formed sides. A minimum concrete cover of 2 inches shall be maintained for prestressing elements by the use of spreaders or by bundling in areas adjacent to openings or inserts. Ties shall also have a minimum cover of 2 inches at these locations. Side forms carrying no load may be removed after 24 hours with the permission of the Engineer or after the concrete has reached the minimum transfer strength as required by M.09.02-6.

(2) Finishing: The topside surface of the piles shall be given a uniformly smooth steel trowel finish to match the surface of the formed sides. The prestressing elements shall be cut flush or recessed 1/8 inch to the top of the pile. Projecting fins and surface imperfections shall be removed in a workmanlike manner. Exposed jet pipe connections, inserts or other devices shall be removed or recessed to a depth as directed, and the hole or opening patched with non-shrink grout in a workmanlike manner. The patching material shall have a degree of finish comparable to the adjacent surfaces. Additional finishing of piles, if required, shall be as shown on the plans or as otherwise directed.

(3) Handling and Storage: Care shall be taken during storage, transporting, hoisting and handling of the prestressed piles to prevent cracking or damage. Damaged piles shall be replaced by the Contractor at his expense. Lifting and support points shall be marked on the piles as required.

(4) Pile Extensions: Pile extensions shall normally be fabricated for this purpose in accordance with the specifications. However, sound sections of pile cutoffs or sound portions of rejected piles may be used, subject to the approval of the Engineer. Short pile extensions may, with the permission of the Engineer, be cast-in-place monolithically with the footing or cap.

2. Pile Driving Equipment:

(a) Hammers: Piles shall be driven with approved air, steam, diesel, or hydraulic hammers or a combination of acceptable hammer and water jet. The plant and equipment furnished for air/steam hammers shall have sufficient capacity to maintain at the hammer, under working conditions, the volume and pressure specified by the manufacturer. The plant and equipment shall be equipped with accurate pressure gauges which are easily accessible to the Engineer. The valve mechanism and other parts of the hammer shall be properly maintained so that the length of stroke for a single-acting hammer and the number of blows per minute for a double-acting hammer will be obtained. The power plant for hydraulic hammers shall have sufficient capacity to maintain at the hammer, under working conditions, the volume and pressure specified by the manufacturer. The power plant and equipment shall be equipped with accurate pressure gauges which are easily accessible to the Engineer.

The size of hammer shall be adapted to the type and size of piles and the driving conditions. Unless otherwise specified, the minimum rated striking energy per blow for hammers used shall be 7000 ft-lb. for driving timber piles; 15,000 ft-lb. for driving steel piles and for driving shells for cast-in-place concrete piles; and 19,000 ft-lb. for driving precast concrete piles and for driving prestressed concrete piles. The hammer model used for the driving of test piles shall be used for the driving of service or production piles, unless a change is authorized by the Engineer in writing. Hammers delivering an energy which the Engineer considers detrimental to the piles shall not be used.

Non-impact hammers, such as vibratory hammers, or driving aids such as jets, followers, pre-augered and prebored holes shall not be used unless either specifically permitted in writing by the Engineer or stated in the Contract.

(b) Pile Hammer Approval: All pile driving equipment furnished by the Contractor shall be subject to the approval of the Engineer. All pile driving equipment shall be sized in such a way that the piles can be driven with reasonable effort to the ordered lengths without damage. Approval of pile driving equipment by the Engineer will be based on wave equation analysis or other judgments. In no case shall the driving

equipment be used without written approval of the Engineer. Prerequisite to such approval, the Contractor shall submit to the Engineer the necessary pile driving equipment information and wave equation analysis at least 30 days prior to driving piles. The wave equation analysis shall be signed, sealed and dated by a Connecticut licensed Professional Engineer.

The criteria that the Engineer will use to evaluate the driving equipment consists of both the required number of hammer blows per foot as well as the pile stresses at the required ultimate pile capacity. The required number of hammer blows indicated by the wave equation at the ultimate pile capacity shall be between 36 and 180 blows per foot for the driving equipment to be acceptable. In addition, for the driving equipment to be acceptable the pile stresses which are indicated by the wave equation to be generated by the driving equipment shall not exceed the maximum driving stresses allowed by the governing design code stated in the Contract.

During pile driving operations, the Contractor shall use the approved system. No variations in the driving system will be permitted without the Engineer's written approval. Any change in the driving system will only be considered after the Contractor has submitted the necessary information for a revised wave equation analysis.

If the Engineer determines the Contractor's hammer is unable to transfer sufficient energy to the pile, the hammer shall be removed from service until repaired to the satisfaction of the Engineer.

(c) Drive System Components and Accessories:

(1) Hammer Cushion: Impact pile driving equipment designed to be used with a hammer cushion shall be equipped with a suitable thickness of hammer cushion material to prevent damage to the hammer or pile and to insure uniform driving behavior. Hammer cushions shall be made of durable manufactured materials, provided in accordance with the hammer manufacturer's guidelines. Wood, wire rope, and asbestos hammer cushions are specifically disallowed and shall not be used. A striker plate as recommended by the hammer manufacturer shall be placed on the hammer cushion to insure uniform compression of the cushion material. The hammer cushion shall be removed from the helmet and inspected prior to beginning pile driving at each structure or after each 100 hours of pile driving, whichever is less. The Contractor shall replace any hammer cushion whose thickness is less than 75% of the original thickness.

(2) Helmet: Piles driven with impact hammers require an adequate helmet or drive head to distribute the hammer blow to the pile head. The helmet shall be axially aligned with the hammer and the pile. The helmet shall be guided by the leads and not be free-swinging. The helmet shall fit around the pile head in such a manner as to prevent transfer of torsional forces during driving, while maintaining proper alignment of hammer and pile. For steel and timber piling, the pile heads shall be cut squarely and a helmet, as recommended by the hammer manufacturer, shall be provided to hold the axis of the pile in line with the axis of the hammer. For precast concrete and prestressed concrete piles, the pile head shall be plane and perpendicular to the longitudinal axis of the pile to prevent eccentric impacts from the helmet. For special types of piles, appropriate helmets, mandrels or other devices shall be provided in accordance with the manufacturer's recommendations so that the piles may be driven without damage.

(3) Pile Cushion: The heads of concrete piles shall be protected by a pile cushion. Pile cushions shall be made of plywood, hardwood, or composite plywood and hardwood materials. The minimum pile cushion thickness placed on the pile head prior to driving shall be at least 4 inches. A new pile cushion shall be provided for each pile. In addition the pile cushion shall be replaced if, during the driving of any pile, the cushion is compressed more than one-half the original thickness or it begins to burn. The pile cushion dimensions shall match the cross sectional area of the pile top. The use of manufactured pile cushion materials in lieu of a wood pile cushion shall be evaluated on a case by case basis.

(4) Leads: Piles shall be supported in line and position with leads while being driven. Pile driver leads shall be constructed in a manner that affords freedom of movement of the hammer while maintaining alignment of the hammer and the pile to insure concentric impact for each blow. Leads may be either fixed or swinging type. Swinging leads, when used, shall be fitted with a pile gate at the bottom of the leads and, in the case of batter piles, a horizontal brace may be required between the crane and the leads. The pile section being driven shall not extend above the leads. The leads shall be adequately embedded in the ground or the pile constrained in a structural frame such as a template to maintain alignment. The leads shall be of sufficient length to make the use of a follower unnecessary, and shall be so designed as to permit proper alignment of batter piles.

(5) Followers: Followers shall only be used when approved in writing by the Engineer, or when specifically stated in the contract documents. In cases where a follower is permitted, the first pile in each

bent and every tenth pile driven thereafter shall be driven full length without a follower, to determine that adequate pile penetration is being attained to develop the ultimate pile capacity. The follower and pile shall be held and maintained in equal and proper alignment during driving. The follower shall be of such material and dimensions to permit the piles to be driven to the penetration depth determined necessary from the driving of the full length piles. The final position and alignment of the first 2 piles installed with followers in each substructure unit shall be verified to be within the required location tolerances before additional piles are installed.

(6) Jets: Jetting shall only be permitted if approved in writing by the Engineer or when specifically stated in the contract documents. When jetting is not required in the contract documents, but approved after the Contractor's request, the Contractor shall determine the number of jets and the volume and pressure of water at the jet nozzles necessary to freely erode the material adjacent to the pile without affecting the lateral stability of the in place pile. When jetting is specifically required in the Contract, the plant shall have sufficient capacity to deliver at all times at least 100 psi pressure at two 3/4 inch jet nozzles. In either case, unless otherwise indicated by the Engineer, jet pipes shall be removed when the pile toe is a minimum of 5 feet above prescribed toe elevation and the pile shall be driven to the required ultimate pile capacity with an impact hammer. Also, the Contractor shall control, treat if necessary, and dispose of all jet water in a manner satisfactory to the Engineer and in accordance with the provisions of 1.10.

(7) Pre-Augering: When stated in the Contract, the Contractor shall pre-auger holes at pile locations to the depths shown on the plans. Pre-augered holes shall be of a size smaller than the diameter or diagonal of the pile cross section; however, large enough to allow penetration of the pile to the specified depth. If subsurface obstructions, such as boulders or rock layers, are encountered, the hole diameter may be increased to the least dimension which is adequate for pile installation. Any void space remaining around the pile after completion of driving shall be filled with sand or other approved material. The use of spuds shall not be permitted in lieu of pre-augering. Augering, wet-rotary drilling, or other methods of pre-augering shall be used only when approved by the Engineer. When permitted, such procedures shall be carried out in a manner which will not impair the capacity of the piles already in place or the safety of existing adjacent structures. If the Engineer determines that pre-augering has disturbed the capacities of previously installed piles, those piles that have been disturbed shall be restored to conditions meeting the requirements of this specification by redriving or by other methods acceptable to the Engineer. Redriving or other remedial measures shall be instituted after the pre-augering operations in the area have been completed.

3. Pile Capacity

(a) Ultimate Pile Capacity: Piles shall be driven by the Contractor to the penetration depth shown on the plans or to a greater depth if necessary to obtain the ultimate pile capacity. The ultimate pile capacity shall be as defined in the contract documents.

Jetting or other methods shall not be used to facilitate pile penetration unless specifically permitted in the contract documents or in writing by the Engineer. The ultimate pile capacity of jetted piles shall be based on driving resistances recorded during impact driving after the jet pipes have been removed. Jetted piles not attaining the ultimate pile capacity at the ordered length shall be spliced, as required, at the Contractor's cost, and driven with an impact hammer until the ultimate pile capacity is achieved.

The ultimate pile capacity of piles driven with followers shall only be considered acceptable when the follower driven piles attain the same pile toe elevation or top of bedrock elevation as required for the full length piles driven without followers that attained the required ultimate pile capacity.

(b) Wave Equation: The ultimate pile capacity shall be determined by the Engineer. Piles shall be driven with the approved driving equipment to the ordered length or other lengths necessary to obtain the required ultimate pile capacity. Jetting or other methods to facilitate pile penetration shall not be used unless specifically permitted either in the Contract or approved by the Engineer after a revised driving resistance is established from the wave equation analysis. Adequate pile penetration shall be considered to be obtained when the specified wave equation resistance criteria is achieved within 5 feet of the pile toe elevation, based on ordered length. Piles not achieving the specified resistance within these limits shall be driven to penetrations established by the Engineer.

(c) Static Load Tests: Compression load tests shall be performed by procedures set forth in ASTM D1143 using the quick load test method, except that the test shall be taken to plunging failure or the capacity of the loading system. Testing equipment and measuring systems shall conform to ASTM D1143, except that the loading system shall be capable of applying 150% of the ultimate pile capacity as stated in

the Contract, and that a load cell and spherical bearing plate shall be used. The apparatus shall be constructed to allow the various increments of the load to be placed gradually, without causing vibration to the test pile. The Contractor shall submit to the Engineer for approval working drawings of the loading apparatus in accordance with 1.05.02. When the approved method requires the use of tension (reaction) piles, the tension piles, when feasible, shall be of the same type and diameter as the production piles, and shall be driven in the location of permanent piles except that timber or tapered piles installed in permanent locations shall not be used as tension piles.

The top elevation of the test pile shall be determined immediately after driving and again just before load testing to check for heave. Any pile which heaves more than 1/4 inch shall be redriven or jacked to the original elevation prior to testing. Unless otherwise specified in the Contract, a minimum 3-day waiting period shall be observed between the driving of any anchor piles or the load test pile and the commencement of the load test.

On completion of the load testing, any test or anchor piling not a part of the finished structure shall be removed or cut off at least 1 foot below either the bottom of footing or the finished ground elevation, if not located within the footing area.

(d) Dynamic Pile Driving Analysis (PDA) Test: Dynamic measurements following procedures set forth in ASTM D4945 will be taken during the driving of piles designated as dynamic monitoring test piles. The Contractor shall employ a qualified specialty Consultant, which has successfully completed no less than 10 dynamic pile driving tests, to perform the testing and report preparation for all Dynamic Pile Driving Analysis (PDA) Tests to be performed.

At least 30 days prior to driving the test piles the Contractor shall submit to the Engineer for review and approval the qualified specialty consultant, as well as the complete installation, and testing procedures. The submittal shall include all necessary pile driving equipment and support facilities to drive the piles to capacities and depths shown on the plans within allowable stress limits. As part of the submittal the Contractor's Consultant shall perform a wave equation analyses, and a summary report confirming that the pile driving system proposed by the Contractor can meet the capacity, driving resistance and allowable stress limits.

All equipment necessary for the dynamic monitoring of the piles such as gages, cables, etc., shall be furnished by the Contractor's Consultant. The equipment shall meet the requirements of ASTM D4945, Standard Test Method for High Strain Dynamic Testing of Piles, and be capable of testing the pile to 1.5 times the ultimate pile capacity. An experienced engineer, who has successfully completed no less than 10 dynamic pile driving tests, shall operate the Pile Driving Analyzer in the field. The Contractor shall furnish a shelter within 100 feet of test location(s) to protect the dynamic test equipment from the elements. The shelter shall be a minimum floor size of 400 s.f., with a minimum ceiling height of 7 feet, and an inside temperature maintained between 50° and 85° F.

The Contractor shall provide power to the test pile locations for the duration of the dynamic testing. The power supply shall consist of a power source providing 115-Volt alternating current with a frequency of 60 Hz and a minimum of 2 kilowatts. If field generators are used as the power source, provide functioning meters to monitor power voltage and frequency. Direct current welders or non-constant power sources are unacceptable.

Prior to lifting the pile to be dynamically tested, the Contractor shall provide as a minimum 3 feet of clear access to 180 degree opposite faces of the pile for pile preparation. The Contractor or its Consultant shall then drill and prepare holes in the pile for gage attachment.

The Contractor or its Consultant shall attach the gages to the pile before driving the piles. Pile driving shall be performed using routine pile installation procedures. When the level of the gages is within 1 foot of the ground surface, or obstruction, driving shall be halted to remove the gages from the pile. If additional driving is required, the pile shall be spliced and gages shall be reattached to the head of the next pile segment.

With the dynamic testing equipment attached, the Contractor shall drive the pile to the design penetration depth or to a depth determined by the Engineer. The Engineer will use the ultimate pile capacity estimates at the time of driving and/or restriking from dynamic test methods to determine the required pile penetration depth for the ultimate pile capacity. The stresses in the piles will be monitored during driving with the dynamic test equipment to ensure that the actual driving stresses do not exceed the maximum allowed values. If necessary, the Contractor shall reduce the driving energy transmitted to the pile by using additional cushions or reducing the energy output of the hammer in order to maintain driving stresses below the maximum values. If non-axial driving is indicated by dynamic test equipment measurements, the

Contractor shall immediately realign the driving system.

After the initial drive of the pile, the Contractor shall wait 24 hours, or the time specified in the Contract, and restrike the dynamic monitoring test pile with the dynamic testing instruments attached. A cold hammer shall not be used for the restrike. The hammer shall be warmed up before restrike begins by applying at least 20 blows to another pile. The maximum amount of penetration required during restrike shall be 6 inches, or 50 hammer blows, whichever occurs first.

The Contractor's Consultant shall provide preliminary estimates of pile capacity of the test pile to the Engineer within 24 hours of the restrike of each tested pile. The Contractor's Consultant shall also prepare and submit a written report within 5 calendar days of the completion of the testing. This report shall contain a discussion of the pile capacity obtained from the dynamic testing. Case Pile Wave Analysis Program (CAPWAP) analyses of the dynamic testing data shall be performed on data obtained at the end of initial driving and the beginning of restrike. The Engineer may request additional analyses at selected pile penetration depths. The report shall also discuss hammer and driving system performance, driving stress levels, and pile integrity. The report is to be prepared, signed, sealed and dated by a Connecticut licensed Professional Engineer. No production piles can be driven until the report has been submitted and approved by the Engineer.

4. Test Piles and Order Lists: Test piles shall be driven at the locations shown on the plans and to the penetration depths specified by the Engineer. Test piles shall be driven to a driving resistance established by the Engineer at the estimated pile toe elevation. The Contractor shall excavate the ground at each test pile to the elevation of the bottom of the footing before the pile is driven. All test piles shall be driven with impact hammers unless specifically stated otherwise in the plans. In general, the specified length of test piles will be greater than the estimated length of production piles in order to provide for variation in soil conditions. The driving equipment used for driving test piles shall be identical to the equipment proposed for driving the production piling. Approval of driving equipment shall conform to the requirements of these Specifications.

Test piles that do not attain the specified driving resistance at a depth of 6 inches above the estimated pile tip elevation, or are specified as a dynamic monitoring pile, shall be redriven after being allowed to set up. The minimum time period before restrike shall be 24 hours, or as specified in the contract documents. A cold hammer shall not be used for the restrike. The hammer used shall be warmed up by applying at least 20 blows to another pile.

Unless otherwise specified in the contract documents, the Contractor shall not order piling to be used in the permanent structure until test pile data has been reviewed and pile order lengths are authorized by the Engineer. The Engineer will provide the pile order list after completion of the test pile(s) and dynamic pile driving analysis (PDA) tests and/or pile loading tests specified in the Contract.

When no test piles are specified for a substructure, the estimated pile lengths in the Contract are taken as the pile order length.

The lengths given in the order list will be based on the lengths which are assumed after cutoff to remain in the completed structure. The Contractor shall, without added compensation, increase the lengths to provide for fresh heading and for such additional length as may be necessary to suit the Contractor's method of operation.

5. Pile Preparation and Driving: The heads of all piles shall be plane and perpendicular to the longitudinal axis of the pile before the helmet is attached. Approval of a pile hammer relative to driving stress damage shall not relieve the Contractor of responsibility for piles damaged because of misalignment of the leads, failure of cushion materials, failure of splices, malfunctioning of the pile hammer, or other improper construction methods. Piles damaged for such reasons shall be rejected and replaced at the Contractor's expense when the Engineer determines that the damage impairs the strength of the pile.

If it becomes necessary and is authorized by the Engineer to resort to jetting, spudding or pre-holing - and further, if no Contract bid price is asked for in the proposal for jetting, spudding, or pre-holing - such work will be paid for as "extra work" in accordance with 1.04.05 and 1.09.04.

The use of a hammer with a greater mass, or the use of piles manufactured or designed with pile tips of a nature to provide for better penetration such as but not limited to composite shells, tapered sections or H-pile sections, shall not be considered as extra work. Authorized point reinforcement for piles shall be a separate item.

Piles for exposed pile bents shall be driven with pile driver leads and templates. They shall be of rigid design and construction and shall maintain the required position and alignment of the piles within the tolerances hereinafter specified. Templates shall be anchored or spudded into position, shall be capable of

guiding all piles required for the bent and shall remain in place until all the piles in the bent are driven.

(a) Location and Alignment Tolerance: Piles shall be driven with a variation of not more than 1/4 in/ft from the vertical or from the batter line indicated, except that piles for trestle bents shall be so driven that the cap may be placed in its proper location without inducing excessive stresses in the piles. Upon completion of driving and released from leads, exposed piles such as in bents shall not have a variation of more than 2 inches at the cut-off elevation from the position shown on the plans. Unless otherwise permitted in writing by the Engineer, failure to meet this tolerance shall be cause for rejection. Other foundation piles shall not be out of the position shown on the plans more than 6 inches after driving. The Engineer may require that driving be stopped in order to check the pile alignment. Pulling laterally on piles to correct misalignment, or splicing a properly aligned section on a misaligned section shall not be permitted.

If the location and/or alignment tolerances specified are exceeded, the extent of overloading shall be evaluated by the Engineer. If in the judgment of the Engineer, corrective measures are necessary, suitable measures shall be designed and constructed by the Contractor.

(b) Heaved Piles: Level readings to measure pile heave after driving shall be made by the Contractor at the start of pile driving operations and shall continue until the Engineer determines that such checking is no longer required. Level readings shall be taken immediately after the pile has been driven and again after piles within a radius of 15 feet have been driven. If pile heave is observed, the Contractor shall take accurate level readings referenced to a fixed datum on all piles immediately after installation and periodically thereafter as adjacent piles are driven to determine the pile heave range. All piles that have been heaved more than 1/4 inch shall be redriven at the Contractor's cost, to the required resistance or penetration. Concrete shall not be placed in pile casings until pile driving has progressed beyond a radius of 15 feet from the pile to be concreted. If pile heave is detected for pipe or shell piles which have been filled with concrete, the piles shall be redriven to original position after the concrete has obtained sufficient strength and a proper hammer-pile cushion system, satisfactory to the Engineer, is used.

(c) Installation Sequence: The order of placing individual piles in pile groups shall be either starting from the center of the group and proceeding outwards in both directions or starting at the outside row and proceeding progressively across the group.

6. Unsatisfactory Piles: The method used in driving piles shall not subject the piles to excessive or undue abuse producing crushing and spalling of concrete, injurious splitting, splintering, and brooming of the wood, or deformation of the steel. Misaligned piles shall not be forced into proper position. Any pile damaged during driving by reason of internal defects, or by improper driving, or driven out of its proper location, or driven below the designated cutoff elevation, shall be corrected by the Contractor by a method approved by the Engineer.

Piles which have been bent during installation shall be considered unsatisfactory unless the ultimate capacity is proven by load tests performed at the Contractor's expense. If such tests indicate inadequate capacity, corrective measures as determined by the Engineer shall be taken, such as use of bent piles at reduced capacity, installation of additional piles, strengthening of bent piles, or replacement of bent piles.

A concrete pile will be considered defective if a visible crack, or cracks, appears around the entire periphery of the pile, or if any defect is observed which, as determined by the Engineer, affects the strength or life of the pile.

7. Splicing Piles and Extensions: Full length piles shall be used when practicable; but if splices cannot be avoided, piles or shells for cast-in-place piles may be spliced in accordance with the requirements of the plans. Piles shall not be spliced except with the approval of the Engineer. Splices in excess of 2 per pile for timber, steel and cast-in-place concrete piles will not be permitted except with special permission of the Engineer. Only 1 splice per pile will be permitted in precast concrete or prestressed concrete piles. In the absence of splice details in the plans, piles or shells for cast-in-place concrete piles shall be spliced in accordance with the pile or shell manufacturer's recommendations, subject to the approval of the Engineer. Working Drawings for prefabricated splicing devices and their method of installation shall be submitted to the Engineer for review. All seams, joints and splices shall develop the full strength of the pile.

8. Point Reinforcement: When directed by the Engineer, the Contractor shall point-reinforce piles. Such point-reinforcement shall be in accordance with the plans or as directed.

9. Cutoff Lengths: The pile head of all permanent piles and pile casings shall be cutoff at the elevation shown on the plans or as ordered by the Engineer. All cutoff lengths shall become the property of the Contractor, and shall be removed by the Contractor from the Site of the work.

10. Painting Steel Piles and Steel Pile Shells: When steel piles or steel pile shells extend above the

ground surface or water surface, they shall be painted as specified elsewhere in the Contract or as ordered by the Engineer. This protection shall extend from an elevation 2 feet below the ground or water surface to the top of the exposed steel.

11. Welding on Piles: When required or permitted, all welding on piles shall be done in accordance with the requirements of the current AWS Structural Welding Code.

7.02.04—Method of Measurement

1. Steel Piles-Timber Piles-Precast Concrete Piles: The length of (type) piles which will be the basis for the pay computation to be included under the item of furnishing (type) piles, shall be number of linear feet of (type) piles authorized by the Engineer or actually furnished by the Contractor, whichever is the lesser amount.

Length of pile cutoffs previously paid for under authorized lengths of piles and subsequently incorporated into the work will not be measured for payment.

The work, materials, tools, equipment and labor incidental to the disposal of pile cutoffs will not be measured for payment.

The amounts to be included under the item for driving (type) piles will be the number of linear feet of piles actually driven and accepted in the completed structure.

2. Cast-in-Place Concrete Piles: The amount to be included under the item of cast-in-place concrete piles shall be the number of linear feet of piles actually driven and accepted in place in the completed structure.

Cut-off materials from shells shall remain the property of the Contractor. They will be paid for in accordance with the unit cost applying in the Contractor's bill or bills for such shells, except that no payment will be made of material cut off from shells furnished by the Contractor in excess of the ordered length. The unit of measurement will be the unit applying in the Contractor's bill or bills for such shells. Material cut off from shells furnished by the Contractor in lengths in excess of those ordered by the Engineer will not be measured for payment hereunder. The work, materials, tools, equipment and labor incidental to the disposal of cutoffs will not be measured for payment.

Reinforcement, if required in cast-in-place concrete piles, will not be measured for payment.

3. Prestressed Concrete Piles (Pretensioned): The length of the prestressed concrete piles, which will be the basis for the pay computation, shall be the number of linear feet of piles authorized by the Engineer or actually furnished by the Contractor, whichever is the lesser amount. The length of any specified pile tip protruding from the concrete will be included in the length measured for payment.

Also included in the length measured for payment will be the length of precast pile extensions ordered by the Engineer. Not to be included, however, is the length of pile extension furnished in excess of the ordered length. The length of projection dowels shall not be included in the length measured for payment.

Extensions to prestressed concrete piles which are poured monolithically with the footing or pier cap will be paid for at the Contract unit prices for the several items involved, which prices shall be full compensation for all materials, tools, equipment and labor necessary to the completion of the work.

Cut-offs shall not be used for pile extension. The work, material, tools equipment and labor incidental to the disposal of cutoffs will not be measured for payment.

The amounts to be included under the item for driving prestressed concrete piles shall be the number of linear feet of piles actually driven and accepted in the completed structure.

4. Test Piles: The amounts to be included under the respective items for test piles, of the type and length specified, shall be the number of test piles actually driven and accepted. Lengths of test piles ordered by the Engineer in excess of the length or lengths specified in the contract will be measured for payment by the actual number of linear feet ordered, furnished and accepted by the Engineer. Driving of such pile extensions will be measured for payment by the actual length driven and left in place.

Authorized splices performed on test piles will be measured for payment by the number of authorized splices actually completed and accepted. Splicing of test piles shall not be considered as authorized splices when such splicing is done to complete piles to the test pile length specified in the Contract.

5. Static Load Tests: The amount to be included under the item of static loading tests shall be the actual number of static load tests completed and accepted.

6. Dynamic Pile Driving Analysis (PDA) Test: The amount to be included under this item shall be the actual number of piles which are driven and restruck with dynamic monitoring equipment attached, completed and accepted.

7. Splices: The amount to be included under the items for splicing timber, steel, cast-in-place concrete, precast concrete and prestressed concrete piles (pretensioned) shall be the number of authorized pile splices actually completed and accepted.

The splicing of timber and steel piles, steel shells for cast-in-place concrete piles, precast concrete piles and prestressed concrete piles (pretensioned) shall not be considered as authorized splices when such splicing is performed to complete piles to the order lengths, as defined in 7.02.03-7, or when the furnished lengths of such piles are less than the order lengths approved by the Engineer.

8. Point Reinforcement for Piles: The amount to be included under the item of "Point Reinforcement for Piles" for the type of piles specified shall be the number of authorized reinforced points actually completed and accepted.

9. Pre-Augering of Piles: The amount to be included under the item “Pre-Augering of Piles” shall be the number of linear feet of pre-augering completed and accepted by the Engineer.

7.02.05--Basis of Payment: This work will be paid for as follows:

1. Steel Piles: Payment for furnishing steel piles of the lengths authorized will be at the Contract unit price per pound for "Furnishing Steel Piles," which price shall include furnishing, delivery, storage and handling, and all materials, equipment, tools and labor incidental thereto. The weight of steel pile caps will be included with and paid for under this item.

Payment for driving steel piles will be at the Contract unit price per linear foot for "Driving Steel Piles," complete in place, which price shall include all materials, equipment, tools and labor incidental thereto.

2. Timber Piles: Payment for furnishing timber piles or treated timber piles, up to a length 10 feet greater than that specified on the plans or in the proposal form, will be at the Contract unit price per foot for "Furnishing Timber Piles (Length)" and "Furnishing Treated Timber Piles (Length)," respectively, which price shall include furnishing, delivery, peeling, storage and handling, and all materials, equipment, tools and labor incidental thereto.

In case the length of any piles finally ordered is more than 10 feet, but less than 20 feet, greater than the length specified on the plans or proposal form, payment for furnishing such piles shall be at a price per linear foot equal to the original Contract price, plus 20% thereof.

In case the length of any piles finally ordered is 20 feet or more greater than the length specified on the plans or proposal form, payment for furnishing such piles shall be at a price per linear foot equal to the original Contract price plus 40% thereof.

Payment for driving timber piles or treated timber piles will be at the Contract unit price per linear foot for "Driving Timber Piles" and "Driving Treated Timber Piles," respectively, complete in place and regardless of length, which price shall include all materials, equipment, tools and labor incidental thereto.

3. Cast-in-Place Concrete Piles: Payment for cast-in-place concrete piles will be at the Contract unit price per linear foot for "Cast-in-Place Concrete Piles," complete in place, including all materials, equipment, tools and labor incidental thereto.

Cut-off materials from shells shall remain the property of the Contractor. They will be paid for in accordance with the unit cost applying in the Contractor's bill or bills for such shells, except that no payment will be made for material cut off from shells furnished by the Contractor in excess of the ordered length.

4. Prestressed Concrete Piles: Payment for furnishing prestressed concrete piles, of the lengths required, will be at the Contract unit price per linear foot for "Furnishing Prestressed Concrete Piles" of the type and size as shown on the plans, which price shall include furnishing, delivery, storage and handling, and all materials, equipment, tools and labor incidental thereto.

Payment for driving prestressed concrete piles will be at the contract unit price per linear foot for "Driving Prestressed Concrete Piles," complete in place, which price shall include all material, equipment, tools and labor incidental thereto. Also included shall be all work involved in cutting piles to the direct cut-off elevation.

5. Test Piles: Test piles will be paid for at the Contract unit price each for "Test Pile," of the type and length specified, which price shall constitute the complete compensation for furnishing and driving test piles and shall include all materials, equipment, tools and labor incidental thereto. Authorized splices to test piles will be paid for at 200% of the Contract unit price bid for Splicing Timber Piles, Splicing Steel Piles, Splicing Cast-in-Place Piles or Splicing Prestressed Concrete Piles, whichever type of test pile the splice has been performed on; and such payment shall be for all costs including materials, equipment, tools and labor incidental thereto.

Extension to test piles in excess of the specified length will be paid for on the following basis, which shall include all equipment, tools, splices, labor and work incidental thereto:

(a) Timber Test Piles: Extensions will be paid for at 125% of the Contract unit price per linear foot for "Furnishing Timber Piles," of the shortest length specified in the proposal, and at 125% of the Contract unit price per linear foot for "Driving Timber Piles."

(b) Steel Test Piles: Extensions will be paid for at 125% of the Contract unit price per pound for "Furnishing Steel Piles" and at 125% of the Contract unit price per linear foot for "Driving Steel Piles."

(c) Cast-in-Place Concrete Test Piles: Extensions will be paid for at 125% of the Contract unit price per linear foot for "Cast-in-Place Concrete Piles." Cut-off materials from shells will be paid for as provided in 7.02.05-3.

(d) Prestressed Concrete Test Piles: Extensions will be paid for at 125% of the Contract unit price per linear foot for "Furnishing Prestressed Concrete Piles," and at 125% of the Contract unit price per linear foot for "Driving Prestressed Concrete Piles."

6. Static Load Tests: Loading tests will be paid for at the Contract unit price each for "Pile Loading Test," which price shall include all expenses incidental to loading the pile or group of piles and removing the load, platform, etc., upon completion of the test.

7. Dynamic Pile Driving Analysis (PDA) Test: Dynamic monitoring will be paid for at the Contract unit price each for “Dynamic Pile Driving Analysis (PDA) Test” which price shall include complete compensation for each pile tested using a pile driving analyzer during driving and restrike, including all materials, equipment, tools and labor incidental thereto, as well as providing preliminary and summary report(s).

8. Splices: Authorized splices in timber, steel, cast-in-place piles, precast concrete and prestressed concrete piles will be paid for at the Contract unit price each for "Splicing Timber Piles," "Splicing Steel Piles," "Splicing Cast-in-Place Concrete Piles," "Splicing Precast Concrete Piles," "Splicing Prestressed Concrete Piles," respectively, which price shall include all materials, except as otherwise noted, and all equipment, tools and labor incidental thereto. In the absence of such prices, authorized splices will be paid for as extra work.

9. Trimming and Cutting: There shall be no direct compensation for cutting off timber, steel, precast concrete or prestressed concrete piles and shells for cast-in-place concrete piles as ordered; but the cost thereof shall be considered as included in the cost of the pile items.

10. Point Reinforcement for Piles: Authorized points for pointing and reinforcing piles will be paid for at the Contract unit price each for "Point Reinforcement for Timber Piles," or "Point Reinforcement for Steel Piles," respectively, whichever applies, which price shall include all materials, equipment, tools and labor incidental thereto. In the absence of such prices, authorized points will be paid for as extra work.

11. Pre-Augering of Piles: Payment for “Pre-Augering of Piles” will be at the Contract unit price per linear foot for “Pre-Augering of Piles,” which price shall include which price shall include all materials, and all equipment, tools and labor incidental thereto.

12. Underground Obstructions: If the required pile penetration is not reached due to the presence of underground obstructions which are not the result of the Contractor's operations but are due to the presence of earlier construction at the Site, then the cost of removing these obstructions and back-filling the area will be paid for as extra work unless otherwise specified in the Contract.

13. Painting: There will be no additional payment for painting steel piles and steel pile shells, but the cost thereof shall be considered as included in the cost of furnishing and driving the piles.

14. Disposal of Pile Cutoffs: All costs incidental to the disposal of cutoff material will be included in the price of furnishing of the type of pile specified.

Pay Item	Pay Unit
Furnishing Steel Piles	lb.
Furnishing (Type) Piles (Length)	l.f.
Driving (Type) Piles	l.f.
Test Pile (Type-Length)	ea.
Splicing (Type) Piles	ea.
Point Reinforcement for (Type) Piles	ea.
Pile Loading Test	ea.
Dynamic Pile Driving Analysis (PDA) Test	ea.
Pre-Augering of Piles	l.f.

SECTION 7.03
RIPRAP

7.03.01—Description: Riprap shall consist of angular shaped stones used to protect foundations of piers, abutments, walls, slopes of embankments and waterways from water damage.

7.03.02—Materials:

1. Stone: The stone for this work shall be the type called for on the plans and shall meet the requirements of M.12.02.

2. Bedding: The bedding material for riprap shall conform to the specifications of the material indicated on the plans.

7.03.03—Construction Methods: The area to be protected by riprap shall be accurately shaped prior to placing of any bedding material or riprap. Where bedding material is called for, it shall be placed on the prepared area and compacted to the depth, lines and grades indicated on the plans.

The riprap shall be placed to its full course thickness in one operation in such a manner as to produce a reasonably well-graded mass of rock without causing displacement of the underlying material. The finished surface shall be free from pockets of small stones and clusters of larger stones. Placing this material by methods likely to cause segregation of the various sizes of stone will not be permitted. Rearranging of individual stones by mechanical or hand methods will be required to the extent necessary to obtain a reasonably well-graded distribution of the specified stone sizes. The completed course shall be of the specified thickness and to the lines and grades as shown on the plans or as ordered by the Engineer.

7.03.04—Method of Measurement: The quantity of riprap measured for payment shall be the number of cubic yards whose length and width is measured in place as accepted and the thickness as shown on the plans.

7.03.05—Basis of Payment: This work will be paid for at the Contract unit price per cubic yard for the type of riprap indicated, complete in place, including all materials, equipment, tools and labor incidental thereto.

Excavation and bedding material will be measured and each paid for under its particular pay item.

Pay Item	Pay Unit
(Type) Riprap	c.y.

SECTION 7.04
GABIONS

7.04.01—Description: This work shall consist of furnishing, assembling, and tying open wire mesh baskets, and filling them with aggregate, in accordance with the line, grade and dimensions shown on the plans.

7.04.02—Materials: The materials for this work shall conform to the following requirements:

1. Wire Mesh: All wire shall be coated in accordance with ASTM A641, Finish 5, Class 3. The tensile strength of lacing wire and wire used for manufacturing the baskets shall be at least 60,000 psi, and have a minimum elongation of 12%, when tested in accordance with ASTM A641.

(a) Galvanized gabions: The wire mesh used for galvanized gabions shall be made of galvanized steel wire with a minimum diameter of 0.118 inch. The maximum dimension of the mesh opening shall be 4.5 inches and the area of the mesh opening shall not exceed 8 square inches. A selvedge wire running through the edges of the basket shall be made of galvanized steel wire with a minimum diameter of 0.148 inch. The lacing wire necessary for assembling and lacing the basket units and the connecting wires shall be made of galvanized steel wire with a minimum diameter of 0.087 inch.

(b) Polyvinyl chloride coated gabions: The wire mesh used for polyvinyl chloride (PVC) coated gabions shall have a galvanized steel wire core with a minimum diameter of 0.104 inch. The maximum dimension of the mesh opening shall be 4.5 inches and the area of the mesh opening shall not exceed 8 square inches. The selvedge wire running through the edges of the basket shall be made of galvanized steel wire with a minimum diameter of 0.131 inch. The lacing wire shall have a galvanized wire core with a minimum diameter of 0.085 inch. The minimum PVC coating thickness shall be 0.015 inch. The color of the polyvinyl chloride shall be black, unless otherwise noted on the plans.

2. Aggregate: The aggregate used shall meet the requirements of M.12.02 except that each side of the aggregate shall be at least 4 inches and not more than 13 inches or 7/10 of the basket's smallest dimension, whichever is less. It shall be reasonably well graded between the limiting sizes.

7.04.03—Construction Methods:

(a) Dimensions: The baskets shall be constructed to meet the details shown on the plans, or as directed by the Engineer. All baskets shall be of uniform width. All basket units shall be subdivided into equal compartments and separated by diaphragms of the same mesh and wire diameter as the basket body. Each compartment's length shall not exceed its width. The overall height of the gabion shall not exceed its overall width.

(b) Fabrication: The baskets shall be fabricated so that the sides, ends, lids and diaphragms can be assembled at the construction Site into rectangular baskets of the specified sizes. The wire mesh shall be cut so as to be non-raveling. The front, base, back and lid shall be woven into a single unit. The ends shall be factory-connected to the base section of the basket so that strength and flexibility at the point of connection are at least equal to that of the mesh. All perimeter edges of the mesh shall be securely fastened with selvedge wire so that the joints have at least the same strength as the wire mesh. Lacing wire shall be supplied in a quantity sufficient for securely fastening all edges of the baskets and diaphragms and to provide for the necessary internal connecting wires in each compartment.

There shall be 4 cross connecting wires in each compartment whose height is either 1/3 or 1/2 the width of the gabion and 8 cross connecting wires in each compartment whose height equals the width of the gabion.

Each basket unit shall be assembled by binding together all vertical edges with lacing wire at intervals of no more than 5 inch spacing or by a continuous piece of lacing wire looped around the vertical edges with at least one tie every 5 inches. Empty baskets shall be set to line and grade as shown on the plans. All adjoining empty basket units must be laced along the perimeter of their contact surfaces in the manner described previously for assembling. The empty upper baskets that form the upper tier shall also be laced to the top of the lower ones. A standard fence-stretcher or other approved device shall be used to remove any kinks from the mesh and hold alignment of the units. The aggregate shall be carefully placed to ensure alignment, avoid bulges and create a minimum of voids. After the aggregate is placed to the level at which the connecting wires are to be installed, 2 connecting wires are to be placed perpendicular to each other, then looped and tied around 2 meshes of each gabion wall. Filling is then resumed until the level of the next connecting wires or the top of the gabion is reached. After a gabion has been filled, the lid shall be stretched tightly over the filling until the lid meets the perimeter edges of the front and end panels. The lid shall then be tightly laced along all edges, ends and diaphragms in the manner described for assembling. The Contractor shall furnish the Engineer working drawings detailing the layout of the gabions to the

Department at least 2 weeks prior to their installation.

7.04.04—Method of Measurement: This work will be measured for payment by the number of cubic yards of “Gabions” or “Polyvinyl Chloride Coated Gabions,” measured in place within neat lines as shown on the plans or as directed by the Engineer. Excavation will be measured for payment under 2.02.

7.04.05—Basis of Payment: This work will be paid for at the Contract unit price per cubic yard for "Gabions" or "Polyvinyl Chloride Coated Gabions," complete in place, including all materials, equipment, tools and labor incidental thereto.

Excavation will be paid for at the Contract unit price for the appropriate excavation item.

Pay Item	Pay Unit
Gabions	c.y
Polyvinyl-Chloride-Coated Gabions	c.y.

SECTION 7.05
SLOPE PAVING

7.05.01—Description: Slope paving shall be composed of approved stone, cast-in-place or precast concrete slabs. The stones or slabs shall be individually laid or cast in place on slopes where indicated on the plans or directed, and in accordance with these specifications. Unless otherwise specified, the choice of material shall be optional with the Contractor; provided, however, only one type of material shall be used in any one location.

7.05.02—Materials: The stone for this item shall meet the requirements of M.12.03. Concrete shall meet the requirements of M.03.

7.05.03—Construction Methods: Slopes where paving is to be used shall be no steeper than the angle of repose of the material unless otherwise indicated or directed. At the bottom of the embankment 2 rows of stone or slabs shall be laid 1 above the other and securely embedded at least 2 feet into the slope. Above these, the paving stone or slabs shall be embedded, against each other, with the sides in contact. The spaces between the larger stone shall be filled with spalls of suitable size, and all spalls shall be rammed thoroughly into place. The finish of the surface of the slope paving shall present an even, tight surface true to the lines, grades and sections given.

Cast-in-place slabs shall be constructed to the lines, grades, and dimensions shown or as ordered by the Engineer; the vertical joints in each course shall break joints with those in adjoining courses at least 6 inches. Placing and mixing shall meet the requirements of 6.01.03 insofar as it may apply. The concrete shall contain not less than 4%, nor more than 6%, entrained air at the time the concrete is deposited in the forms.

7.05.04—Method of Measurement: This work will be measured for payment by the number of square yards in the face area of accepted slope paving completed within the neat lines as shown on the plans or as ordered by the Engineer.

7.05.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Slope Paving" complete in place, which price shall include all materials, equipment, tools, and labor incidental thereto; also all necessary excavations below the surface of the final slope and the disposal of surplus material.

Pay Item	Pay Unit
Slope Paving	s.y.

SECTION 7.06
MICROPILES

7.06.01—Description
7.06.02—Materials
7.06.03—Construction Methods
7.06.04—Method of Measurement
7.06.05—Basis of Payment

7.06.01—Description: This work shall consist of constructing micropiles in accordance with the Contract. The Contractor is responsible for furnishing all design, materials, products, accessories, tools, equipment, services, transportation, labor and supervision required for design, installation and testing of micropiles and micropile top attachments for this Project.

The Contractor shall select the micropile type, size, pile-top attachment, installation means and methods, and shall estimate the grout-to-ground bond value(s) and determine the required grout-to-ground bond length and final micropile diameter.

The Contractor shall design and install micropiles that will develop the load capacities indicated on the plans. The micropile load capacities shall be confirmed by verification and proof-load testing as required and must meet the test acceptance criteria specified herein. The Contractor's micropile design shall conform to requirements set forth in this specification and to micropile design minimums/maximums shown on the Contract drawings.

7.06.02—Materials: Furnish new materials without defects. Materials for micropiles shall comply with the following:

1. **Admixtures for Grout:** Admixtures shall comply with M.03.01. Accelerators are not permitted. Expansive admixtures and admixtures containing chlorides are not permitted.
2. **Cement:** Cement shall meet the requirements of ASTM C150/AASHTO M85, Types II, III or V.
3. **Centralizers and Spacers:** Centralizers and spacers shall be fabricated from Schedule 40 PVC pipe.
4. **Grout:** Grout shall consist of neat cement or fine aggregate and cement mixture meeting the 3- and 28- day required compressive strengths specified in the Contract. The grout shall conform to AASHTO T 106/ASTM C109 and to any minimum and maximum properties shown on the plans or in M.03.05.
5. **Permanent Casing Pipe:** Permanent steel casing or steel pipe shall meet the required minimum and maximum properties shown on the plans. The steel casing or steel pipe shall meet the requirements of the following specifications ASTM A252 or A106, or API N-80.
6. **Reinforcing Bars:** Reinforcing steel shall be deformed bars in accordance with ASTM A615/AASHTO M 31. Continuous spiral deformations (*i.e.*, continuous thread bars) shall be used for same. Bar tendon couplers, if required, shall develop the ultimate tensile strength of the bars without evidence of any failure.
7. **Encapsulation:** Encapsulation (double corrosion protection) shall be shop-fabricated using high-density, corrugated polyethylene tubing complying with the requirements of ASTM D3350/AASHTO M 252 with a nominal wall thickness of 0.03 inch. The inside annulus between the reinforcing bar(s) and the encapsulation tube shall measure a minimum 0.2 inch and be fully grouted with non-shrink grout as specified in M.03.

7.06.03—Construction Methods:

1. **Contractor's Experience Requirements**: The micropile Contractor shall be experienced in the construction and load testing of micropiles, having successfully constructed at least 5 projects in the last 5 years involving construction totaling at least 100 micropiles of equal or greater capacity than is required in these plans and Specifications.

   The Contractor shall have previous micropile drilling and grouting experience in soil/rock conditions similar to those on this Project. The Contractor shall submit construction details, structural details and load test results for at least 3 previous successful micropile load tests from different projects similar in scope to this Project.

   The Contractor shall assign or hire a professional engineer, licensed in the State of Connecticut, to supervise the micropile work. That engineer shall have experience on at least 10 projects of similar scope to this Project, completed over the past 5 years. The Contractor shall not use manufacturers' representatives to satisfy the supervising engineer requirements of this Section. The Contractor may use a single independent consultant for this purpose, provided that the consultant has specific experience as described above and operates specifically for the purpose of transferring technology and skills in micropiling to contractors. The on-Site foremen and drill rig operators shall also have experience on at least 10 projects over the past 5 years installing micropiles of equal or greater capacity than is required in these plans and Specifications.

   The Contractor shall assign or hire a Professional Engineer, licensed in the State of Connecticut, to design the micropiles. This engineer shall have experience in the design of at least 3 successfully-completed micropile projects over the past 5 years, with micropiles of capacity similar to that required in these plans and

Specifications. This engineer shall also be responsible for design, supervision and reporting of the verification and proof test(s).

At least 45 calendar days before the planned start of micropile construction, the Contractor shall submit 5 copies of the completed Project reference list and a personnel list. The Project reference list shall include a brief Project description with the owner's name and current phone number and load test reports. The personnel list shall identify the supervising Project Engineer, drill rig operators, and on-Site foremen to be assigned to this Project by the Contractor. The personnel list shall contain a summary of each individual's experience and be complete enough for the Engineer to determine whether each individual has the required qualifications.

Work shall not start, nor materials be ordered, until the Engineer gives written approval of the Contractor's experience qualifications. The Engineer may suspend work if the Contractor uses non-approved personnel on the Project. If work is suspended for that reason, the Contractor shall be fully liable for all resulting costs, and the Department will not make any Contract time adjustments because of the suspension.

2. Micropile Design Requirements and Submittals: The micropiles shall be designed to meet the specific loading conditions, as shown on the plans and approved working drawings. The micropile design shall meet all required minimum and maximum properties shown on the plans, the "American Association of State Highway and Transportation Officials (AASHTO) LRFD Bridge Design Specifications" (including the latest Interims), and the "Connecticut Department of Transportation Bridge Design Manual."

Where called for on the plans, the Contractor shall provide corrosion protection of the internal steel reinforcing bars, consisting of encapsulation, epoxy coating or grout. Where the permanent casing is used for a portion of the micropile, the corrosion protection shall extend at least 5 feet into the casing. Steel pipe used for micropile permanent casing shall incorporate an additional 1/16 inch thickness of sacrificial steel for corrosion protection.

The Contractor shall submit working drawings in accordance with 1.05 to the Engineer. The working drawings shall include all information required for the design, plans, construction and quality control of the micropile installation. The information shall include, but not necessarily be limited to, the following:

(a) Design Computations

I. A written summary report describing the overall micropile design.
II. A statement of applicable code requirements and design references.
III. Micropile structure critical design cross-section(s) geometry, including soil/rock strata and piezometric levels and location, magnitude and direction of applied loadings, including slope or external surcharge loads.
IV. A description of design criteria to be applied to the work, including, soil/rock shear strengths (friction angle and cohesion), unit weights, and grout-to-ground bond value(s) and micropile drill-hole diameter assumptions for each soil/rock stratum.
V. A statement of Resistance/Load factors used in the design of the grout-to-ground bond value(s), surcharges, soil/rock and material unit weights, steel, grout and concrete materials.
VI. Design calculation sheets with the Project number, micropile structure location, designation, date of preparation, initials of designer and checker, and page number at the top of each page. Provide an index page for the design calculations.
VII. Design notes including an explanation of any symbols and computer program used in the design.
VIII. Pile-to-footing connection calculations.

(b) Plans

I. A plan view of the micropile structures providing:
   i. A reference baseline and elevation datum.
   ii. The offset from the construction centerline or baseline to the face of the micropile structure at all changes in horizontal alignment.
   iii. Beginning and end of micropile structure stations.
   iv. Right-of-way and permanent or temporary construction easement limits, location of all known active and abandoned utilities, adjacent structures or other potential interference; and the centerline of any drainage structure or drainage pipe behind, passing through or passing under the micropile structure.
   v. Subsurface exploration locations shown on the plan view of the proposed micropile structure alignment with appropriate reference baselines to fix the locations of the exploration relative to the micropile structure.
II. An elevation view of the micropile structure(s) providing:
   i. Elevation view showing micropile locations and elevations; vertical and horizontal spacing; batter and alignment and the location of drainage elements (if applicable).
   ii. Existing and finished grade profiles both behind and in front of the micropile structure.

III. Design parameters and applicable codes.
IV. General notes for constructing the micropile structure, including construction sequencing or other special construction requirements.
V. Horizontal and vertical curve data affecting the micropile structure and micropile structure control points. Match lines or other details to relate micropile structure stationing to centerline stationing.
VI. A listing of the summary of quantities on the elevation drawing of each micropile structure, showing pay item estimated quantities.
VII. Micropile typical sections, including micropile spacing and inclination; minimum drill-hole diameter; pipe casing and reinforcing bar size and details; splice type and locations; centralizers and spacers; grout bond zone and casing plunge length (if used); corrosion protection details; and connection details to the substructure footing, anchorage and plates.
VIII. A typical detail of verification and production proof test micropiles defining the micropile length, minimum drill-hole diameter, inclination, and load test bonded and unbonded test lengths.
IX. Details, dimensions and schedules for all micropiles, casing and reinforcing steel, including reinforcing bar bending details.
X. Details for constructing micropile structures around drainage facilities (if applicable).

(c) Construction Procedures

I. Detailed step-by-step description of the proposed micropile construction procedure, including personnel, testing and equipment to ensure quality control. This step-by-step procedure shall be shown in sufficient detail to allow the Engineer to monitor the construction and quality of the micropiles.
II. Proposed start date, time schedule and micropile installation schedule providing the following:
    i. Micropile number.
    ii. Micropile design load.
    iii. Type and size of rebar.
    iv. Minimum total bond length.
    v. Total micropile length.
    vi. Micropile top footing attachment.
III. If welding of casing is proposed, submit the welding procedure. All welding shall be done in accordance with the current AWS Structural Welding Code.
IV. Information on space requirements for installation equipment that verify the proposed equipment can perform at the Site.
V. Proposed Plan describing how surface water, drill flush, and excess waste grout will be controlled and disposed. This will include computations showing that the proposed equipment used for flushing the micropile during installation (*i.e.*, pumps for water flushing and compressors for air flushing) will maintain up-hole (flushing) velocities necessary to ensure that all of the flush and drill cuttings are returned up through the annulus between the drill rod and casing.
VI. Certified mill test reports for the reinforcing steel and for permanent casing. The ultimate strength, yield strength, elongation, and material properties composition shall be included. For API N-80 pipe casing, coupon test results may be submitted in lieu of mill certification.
VII. Proposed Grouting Plan. The grouting plan shall include complete descriptions, and details for the following:
    a. Grout mix design and type of materials to be used in the grout, including certified test data and trial batch reports. The Contractor shall also provide specific gravity of the wet mix design.
    b. Methods and equipment for accurately monitoring and recording the grout depth and grout volume as the grout is being placed.
    c. Estimated curing time for grout to achieve specified strength. Previous test results for the proposed grout mix completed within 1 year of the start of grouting may be submitted for initial verification and acceptance and start of production work. During production, grout shall be tested in accordance with M.03.05.
    d. Procedure and equipment for Contractor monitoring of grout quality. At a minimum, the Contractor shall be required to use a Baroid Mud Balance (per API RP-13B-1) to check the specific gravity of the mixed grout prior to placement into each drilled micropile.

(d) Detailed plans for the proposed micropile load testing method. This shall include all drawings, details, and structural design calculations necessary to clearly describe the proposed test method, reaction load system capacity and equipment setup, types and accuracy of apparatus to be used for applying and measuring the test loads and pile top movements in accordance with this Specification.

(e) Calibration reports and data for each test jack, pressure gauge and master pressure gauge and electronic

load cell to be used. The calibration tests shall have been performed by an independent testing laboratory within 90 calendar days of the date submitted. Testing shall not commence until the Engineer has reviewed and accepted the jack, pressure gauge, master pressure gauge and electronic load cell calibration data.

Work shall not begin until the construction submittals have been received, reviewed, and accepted in writing by the Engineer. Any submittals found to be unacceptable by the Engineer shall be revised, resubmitted and accepted prior to commencing work.

3. **Pre-construction Meeting:** A pre-construction meeting will be scheduled by the Engineer and held prior to the start of micropile construction. The Engineer, prime Contractor, micropile specialty Contractor and micropile design engineer shall attend the meeting. Attendance is mandatory. The pre-construction meeting will be conducted in order to clarify the construction requirements for the work, to coordinate the construction schedule and activities, and to identify contractual relationships and delineation of responsibilities among the prime Contractor and the various subcontractors – specifically those pertaining to excavation for micropile structures, installation of temporary sheeting, anticipated subsurface conditions, micropile installation and testing, micropile structure survey control and Site drainage control.
4. **Site Drainage Control:** The Contractor shall control and properly dispose of drill flush and construction related waste, including excess grout, in accordance with 1.10, any related Special Provisions in the Contract, and all applicable codes and regulations. Drill flush shall be conveyed by pipe, hose or conduit away from the location where the micropile is being drilled and away from any adjacent structure or facility. The Engineer shall determine the acceptable distance required to convey the drill flush away from the micropile location. The Contractor shall provide positive control and discharge of all surface water that will affect construction of the micropile installation; maintain all pipes or conduits used to control surface water during construction; and repair any damage caused by surface water at no additional cost to the Department. Upon substantial completion of the work, the Contractor shall remove surface water control pipes or conduits from the Site. Alternatively, with the approval of the Engineer, the Contractor may leave pipes or conduits in place if fully grouted.

   The Contractor shall immediately contact the Engineer if unanticipated existing subsurface drainage structures or other utilities are discovered during excavation or drilling; and shall suspend work in such areas until remedial measures meeting the Engineer's approval are implemented.
5. **Micropile Allowable Construction Tolerances:**
   (a) Centerline of piling shall not be more than 3 inches from indicated plan location. Centerline of reinforcing steel shall not be more than 0.5 inch from the centerline of the pile.
   (b) Pile shall be plumb or battered within 2% of total-length plan alignment.
   (c) Top elevation of pile shall be plus 1 inch or minus 1 inch maximum from vertical elevation indicated.
6. **Micropile Installation:** The micropile Contractor shall select the drilling method, the grouting procedure and the grouting pressure used for installation of the micropiles. The micropile Contractor shall also determine the micropile casing size, final drill-hole diameter and bond length, and central tendon reinforcement steel size necessary to develop the specified load capacities and load testing requirements. All micropile material properties and dimensions shall conform to minimum/maximum properties and dimensions as shown in the Contract drawings. The micropile Contractor is also responsible for estimating the grout take. The Department will make no extra payment for grout overruns.

   Should the plans require uncased drilling of the micropile into bedrock, the permanent or temporary casing shall be drilled a minimum 6 inches into ledge or to a depth within the ledge so as to prevent subsidence of overburden into the uncased and bonded zone portion of the drill-hole (*i.e.*, the rock socket). The plans show estimated permanent casing lengths for each substructure unit. Any difference in the required length of permanent casing accepted by the Engineer from the estimated lengths shown on the plans shall be measured for payment and credit. The Department will make no payment for differences in required length of temporary casing.

   The drilling equipment and methods shall be suitable for drilling through the conditions to be encountered, without causing damage to the overburden, any overlying or adjacent structures, buried structures, utilities or services. If called for in the drilling method description, or by the nature of the stratum to be drilled through, the micropile Contractor shall furnish an overburden casing of the type and thickness that can be installed without distortion. Casings that fail, fracture, or otherwise distort during drilling or after drilling shall, unless otherwise directed, be withdrawn or replaced at the micropile Contractor's expense. The drill-hole must be open along its full length to at least the design minimum drill-hole diameter prior to placing grout and reinforcement.

   Temporary casing or other approved method of pile drill-hole support will be required in caving or unstable

ground in order to permit the pile shaft to form a drill hole of the minimum design diameter. The Contractor's proposed method(s) to provide drill-hole support and to prevent detrimental ground movements must be reviewed by the Engineer in advance of its use. Detrimental ground movement is defined as movement that requires remedial repair measures, in order to maintain Site conditions as determined by the Engineer.

Drilling and flushing methods shall be selected by the Contractor. Use of drilling fluid containing bentonite or any other non-reverting drilling fluid, however, is not allowed. The drilling and flushing system chosen by the Contractor shall be capable of providing the necessary up-hole velocity so as to ensure that all the flush and drill cuttings are returned up through the annulus between the drill rod and casing. The flush must not be allowed to escape in an uncontrollable fashion into the soil and rock formations outside the casing. The return flush must never be blocked or suppressed within the casing on its way back to the surface. The Contractor shall monitor and modify, as needed, the flush velocity and other elements of its drilling methods that could contribute to return of flush outside the casing. When return of flush is substantially lost during drilling, the Contractor shall halt drilling operations and immediately notify the Engineer of the situation.

During construction, the Contractor shall observe the ground conditions in the vicinity of the micropile construction site on a daily basis for signs of ground heave or subsidence, and must immediately notify the Engineer if signs of movements are observed. The micropile Contractor shall immediately suspend or modify drilling or grouting operations if ground heave or subsidence is observed, if the micropile structure is adversely affected, or if adjacent structures are damaged because of the drilling or grouting. If the Engineer determines that the movements require corrective action, the micropile Contractor shall take corrective actions necessary to stop the movement or perform repairs.

Reinforcement may be placed prior to grouting the drill-hole. Reinforcement surface shall be free of deleterious substances such as soil, mud, grease or oil that might contaminate the grout or coat the reinforcement and impair bond. Pile reinforcement groups, if used, shall be sufficiently strong to withstand the installation and grouting process without damage or disturbance.

The micropile Contractor shall check pile-top elevations and adjust all installed micropiles to the planned elevations.

Centralizers and spacers shall be provided at 10 feet on center maximum spacing. The uppermost and lowest centralizers shall be located a maximum of 3 feet from the top and bottom of the micropile. Centralizers and spacers shall be securely attached to the reinforcement, sized to position the reinforcement within 1/2 inch of plan location from center of pile, sized to allow grout tremie pipe insertion to the bottom of the drill-hole, and must be of sufficient size to allow grout to flow freely up the drill-hole, up the casing, and between adjacent reinforcing bars. The reinforcing steel shall be inserted into the drill-hole to the desired depth without difficulty. Partially inserted reinforcing bars shall not be driven or forced into the hole. The micropile Contractor shall re-drill and reinsert reinforcing steel when necessary in order to facilitate insertion.

Lengths of casing and reinforcing bars to be spliced shall be secured in proper alignment and in a manner that prevents eccentricity or an angle between the axes of the lengths to be spliced. Splices and threaded joints shall meet the requirements of the rebar material. Threaded pipe casing joints shall be located at least 2 casing diameters (OD) from a splice in any reinforcing bar. When multiple bars are used, bar splices shall be staggered at least 1 foot.

Micropiles shall be grouted on the same day that the load transfer bond length is drilled. The grouting equipment used shall be a colloidal grout plant and shall produce a grout free of lumps and undispersed cement. Paddle type mixers are not acceptable. The micropile Contractor shall have means and methods of measuring the grout quantity and pumping pressures during the grouting operations. The grout pump shall be equipped with a pressure gauge to monitor grout pressure. A second pressure gauge shall be placed at the point of injection into the pile top. The pressure gauge shall be capable of measuring pressures of at least 145 psi or 2 times the actual grout pressure used, whichever is greater. The grout shall be kept in agitation prior to mixing. Grout shall be placed within 1 hour of mixing. The grouting equipment shall be sized to enable each pile to be grouted in one continuous operation. The grout shall be injected from the lowest point of the drill-hole, and injection shall continue until uncontaminated grout flows from the top of the pile. The grout may be pumped through grout tubes, casing, hollow stem augers or drill rods. Temporary casing, if used, shall be extracted in stages so as to ensure that, after each length of casing is removed, the grout level is brought back up to the ground level before the next length is removed. The tremie pipe or casing shall always extend below the level of the existing grout in the drill-hole. The grout takes shall be controlled to prevent excessive heave or fracturing of rock or soil formations. Upon completion of grouting, the grout tube may remain in the hole, but must be filled with grout.

If the Contractor elects to use a post-grouting system, working drawings and details shall be submitted to the Engineer for review in accordance with 1.05.02.

Grout within the micropile verification and proof test piles shall attain the minimum required 3-day compressive strength prior to load testing. During production, micropile grout shall be tested by the Contractor for compressive strength in accordance with AASHTO T 106/ASTM C109 at a frequency of no less than 1 set of 3 each 2 inch grout cubes, or 3 inch cylinders, from each grout plant each day of operation, or per every 10 micropiles, whichever occurs more frequently. The compressive strength shall be the average of the 3 cubes or cylinders tested.

Grout consistency as measured by grout density shall be determined by the micropile Contractor per API RP-13B-1 at a frequency of at least 1 test per pile, conducted just prior to start of pile grouting. The Baroid Mud Balance used in accordance with API RP-13B-1 is an approved device for determining the grout density of neat cement grout.

Provide grout cube or cylinder compressive strength and grout density test results to the Engineer within 24 hours of testing.

7. **Micropile Installation Records:** The micropile Contractor shall prepare and submit to the Engineer full-length installation records for each micropile installed. The records shall be submitted within 1 work shift after that pile installation is completed. The data shall be recorded on a micropile installation log. A separate log shall be provided for each micropile.
8. **Verification and Proof Tests:** The Contractor shall perform verification and proof testing of piles at the locations specified on the plans, and perform compression load testing in accord with ASTM D1143 and tension load testing in accord with ASTM D3689, except as modified herein. If the Contractor designs micropiles using tip resistance, ASTM D1143 shall be used for verification and proof tests thereof.

   The Contractor shall perform pre-production verification pile load test(s) to verify the design of the pile system and the construction methods proposed prior to installing any production piles. Sacrificial verification test pile(s) shall be constructed by the Contractor in accordance with the approved working drawings, and shall install verification test pile(s) at the location(s) shown on the plans or at location(s) approved by the Engineer.

   Verification load test(s) shall be performed in order to verify that the micropiles installed by the Contractor will meet the compression and tensile load capacities and load test acceptance criteria, and to verify that the length of the micropile load transfer bond zone is adequate. The micropile verification load test results must verify the Contractor's design and installation methods.

   The drilling method, grouting method, permanent casing length, micropile diameter (cased and uncased) and bond-zone length for the verification test pile shall be identical to those specified for the production piles at the given locations. The verification test micropile structural steel sections shall be sized to safely resist the maximum test load.

   The maximum verification and proof test loads applied to the micropile shall not exceed 80% of the structural capacity of the micropile structural elements, including steel yield in tension, steel yield or buckling in compression, or grout crushing in compression. Any required increase in strength of the verification and proof test pile elements above the strength required for the production piles shall be provided for in the Contractor's bid price.

   Testing equipment used in connection with the micropiles shall include dial gauges, dial gauge independent reference frame, jack and pressure gauge, electronic load cell (with readout device), and a reaction frame. The load cell is required only for the creep test portion of the verification test. The Contractor shall provide a description of test setup and jack, pressure gauge and load cell calibration curves as outlined in the Submittals Section.

   The Contractor shall design the testing reaction frame to be sufficiently rigid and of adequate dimensions to ensure that excessive deformation of the testing equipment does not occur; and must align the jack, bearing plates, and stressing anchorage so that unloading and repositioning of the equipment will not be required during the test.

   The Contractor shall also apply and measure the test load with a hydraulic jack and pressure gauge. The pressure gauge shall be graduated in 100 psi increments or less. The jack and pressure gauge shall have a pressure range not exceeding twice the anticipated maximum test pressure. Jack ram travel shall be sufficient to allow the test to be done without resetting the equipment. The Contractor shall monitor the creep-test-load-hold during verification tests with both the pressure gauge and the electronic load cell; and shall use the load cell in order to accurately maintain a constant load hold during the creep-test-load-hold increment of the verification test.

   The Contractor shall measure the pile top movement with a dial gauge capable of measuring to 0.001 inch. The dial gauge shall have a travel sufficient to allow the test to be done without having to reset the gauge; and the Contractor shall visually align the gauge to be parallel with the axis of the micropile and support the gauge independently from the jack, pile or reaction frame. The Contractor shall also use a minimum of 2 dial gauges

when the test setup requires reaction against the ground or single reaction piles on each side of the test pile.

The Contractor shall test verification piles to the following loads: Alignment Load ("AL"), Maximum Service Limit Pile Load ("SVL") and the Ultimate Pile Capacity ("UPC"). The SVL and UPC loads are provided on the Contract drawings. The AL is the minimum load applied to the micropile during testing needed to keep the testing equipment correctly positioned. The AL shall not exceed 5% of the SVL. The verification pile load tests shall be made by incrementally loading the micropile in accordance with the cyclic load schedule shown in Table 7.06-1, for both compression and tension loading (test the compression prior to tension).

Table 7.06-1, Cyclic Load Schedule for Verification Pile Load Test

Step	Loading	Applied Load	Hold Time (minutes)
1	Apply AL	AL	2.5
2	Cycle 1	0.15 SVL	2.5
		0.30 SVL	2.5
		0.45 SVL	2.5
		0.60 SVL	2.5
		0.75 SVL	2.5
		0.90 SVL	2.5
		1.00 SVL	10 to 60
		0.60 SVL	2.5
		0.30 SVL	2.5
		AL	
3	Cycle 2	0.075 UPC	2.5
		0.150 UPC	2.5
		0.225 UPC	2.5
		0.300 UPC	2.5
		0.375 UPC	2.5
		0.450 UPC	2.5
		0.525 UPC	2.5
		0.600 UPC	2.5
		0.675 UPC	2.5
		0.750 UPC	2.5
		0.825 UPC	2.5
		0.900 UPC	2.5
		1.000 UPC	10 to 60
		0.750 UPC	2.5
		0.525 UPC	2.5
		0.225 UPC	2.5
		AL	

Pile-top movement shall be measured at each load increment. The load-hold period shall start as soon as each test load increment is applied. Pile movement during the 1.00 SVL and 1.000 UPC loads shall be measured and recorded at 1, 2, 3, 4, 5, 6, 10, 20, 30, 50, and 60 minutes. The alignment load shall not exceed 5% of the SVL. Dial gauges shall be reset to zero after the initial AL is applied.

The acceptance criteria for micropile verification load test are:

(a) The Engineer shall determine the criteria for tolerable movement during the load test at the top of the micropile.

(b) At the end of the maximum test load increment for each cycle, test piles shall have a creep rate not exceeding 0.05 inch/log cycle time (1 to 10 minutes) or 0.1 inch/log cycle time (6 to 60 minutes or the last log cycle if held longer). The creep rate shall be linear or decreasing throughout the hold period.

(c) Failure does not occur at any load increment up to and including the maximum test load for each cycle. Failure is defined as load at which attempts to further increase the test load simply result in continued pile movement.

Upon completion of the test, the Contractor shall prepare and submit a report of the test results, stamped by a professional engineer, for review and acceptance by the Engineer prior to beginning installation of production micropiles. This report shall include written confirmation of the verification micropile's capacity.

If a verification tested micropile fails to meet the acceptance criteria, the Contractor shall modify the design, the construction procedure, or both. These modifications may include modifying the installation methods,

increasing the bond length, or changing the micropile type. Any modification that necessitates changes of the structure shall be submitted as a revision to the working drawings and require the Engineer's review and acceptance. Any modifications of design or construction procedures or cost of additional verification test piles and load testing shall be at the Contractor's expense. At the completion of verification testing, the Contractor shall remove test piles down to the elevation specified by the Engineer.

The Contractor shall perform proof load tests at the micropile locations as shown on the plans, and shall perform proof-load tests on the first set of production piles installed at each designated substructure unit prior to the installation of the remaining production piles in that unit. The initial proof-test piles shall be installed at the locations shown on the plans. Upon completion of each test, the Contractor shall prepare and submit a report of the test results, stamped by a professional engineer, for review and acceptance by the Engineer

The Contractor shall test proof test piles to a maximum test load of 1.00 times the Maximum Strength Limit Pile Load (STL). The STL load is provided on the Contract drawings. Proof tests shall be made by incrementally loading the micropile as shown in Table 7.06-2, to be used for both compression and tension loading:

Table 7.06-2, Incremental Loading for Proof Test Piles

Step	Loading	Applied Load	Hold Time (minutes)
1	Apply AL		2.5
2	Cycle 1	0.15 STL	2.5
		0.30 STL	2.5
		0.45 STL	2.5
		0.60 STL	2.5
		0.75 STL	2.5
		0.90 STL	2.5
		1.00 STL	10 to 60 minutes
		0.60 STL	2.5
		0.30 STL	2.5
		AL	

Depending on performance, either a 10-minute or 60-minute creep test shall be performed at the 1.00 STL test load. Where the pile top movement between 1 and 10 minutes exceeds 0.039 inch, the 1.00 STL load shall be maintained an additional 50 minutes. Movements shall be recorded at 1, 2, 3, 5, 6, 10, 20, 30, 50 and 60 minutes. The alignment load shall not exceed 5% of STL. Dial gauges shall be reset to zero after the initial AL is applied.

The acceptance criteria for micropile proof load tests are:

(a) The Engineer shall determine the criteria for tolerable movement during the load test at the top of the micropile.

(b) At the end of the 1.00 STL test load increment, test piles shall have a creep rate not exceeding 0.05 inch/log cycle time (1 to 10 minutes) or 0.1 inch/log cycle time (6 to 60 minutes). The creep rate shall be linear or decreasing throughout the creep-load hold period.

(c) Failure does not occur at the 1.00 STL maximum test load. Failure is defined as the load at which attempts to further increase the test load simply result in continued pile movement.

If a proof-tested micropile fails to meet the acceptance criteria, the Contractor shall immediately proof test another micropile within that footing. For failed piles and further construction of other piles, the Contractor shall modify the design, the construction procedure, or both. These modifications may include installing replacement micropiles, incorporating piles at not more than 50% of the maximum load attained, post-grouting the tested pile and re-proof testing the pile, modifying installation methods, increasing the bond length, or changing the micropile type. Any modification that necessitates changes of the structure design shall require the Engineer's prior review and acceptance. Any modifications of design or construction procedures, or cost of additional verification test piles and verification or proof load testing, or replacement production micropiles, shall be at the Contractor's expense.

7.06.04—Method of Measurement:

1. **Micropiles** will be measured for payment by the number of micropiles installed and accepted. There will be no separate measurement or payment for furnishing the design of the micropiles or developing installation methods to meet these Specifications.
2. **Verification Test for Micropiles** will be measured for payment by the number of verification tests performed on sacrificial micropiles.

3. **Proof Test for Micropiles** will be measured for payment by the number of proof tests performed on production micropiles.
4. **Micropile Length Adjustment** will be measured for payment by the length in linear feet of the difference between the estimated length of permanent casing, as shown on the plans, and the actual length of permanent casing installed and accepted by the Engineer. (Note that the permanent casing length is measured from the bottom of the pile cap to the permanent casing tip, including the required embedment of casing into rock. Embedment into the pile cap will not be measured for payment because it is considered incidental to micropile construction. Any increase in casing length will be measured for payment to the Contractor, and any decrease in casing length will be measured for credit to the State.)

There will be no separate measurement or payment for mobilization and demobilization associated with this item.

7.06.05—Basis of Payment:

1. **Micropiles** will be paid for at the Contract unit price each for "Micropiles" complete and accepted in place, including all design, development of installation methods, materials, equipment, tools, proper disposal of drilling spoil and labor incidental thereto.
2. **Verification Test for Micropiles** will be paid for at the Contract unit price each for "Verification Test for Micropiles" completed on sacrificial micropiles, including all materials, testing equipment, tools, test reports, removal of test piles and labor incidental thereto.
3. **Proof Test for Micropiles** will be paid for at the Contract unit price each for "Proof Test for Micropiles" completed on production micropiles, including all materials, testing equipment, tools, test reports and labor incidental thereto.
4. **Micropile Length Adjustment** will be paid for at the Contract unit price per linear foot for "Micropile Length Adjustment" complete and accepted, including all materials, equipment, tools, and labor incidental thereto.

Pay Item	Pay Unit
Micropiles	ea.
Verification Test for Micropiles	ea.
Proof Test for Micropiles	ea.
Micropile Length Adjustment	l.f.

SECTION 7.07
MEMBRANE WATERPROOFING
(WOVEN GLASS FABRIC)

7.07.01—Description
7.07.02—Materials
7.07.03—Construction Methods
7.07.04—Method of Measurement
7.07.05—Basis of Payment

7.07.01—Description: This item shall consist of furnishing and applying a membrane system of waterproofing to the surface of concrete structures where a bituminous concrete wearing surface is to be constructed, at construction joints and at other locations shown on the plans or ordered.

7.07.02—Materials: The materials for this work shall meet the requirements of M.12.04 for primer, woven glass fabric saturated with asphalt, seal coat, and for asphalt flashing cement.

7.07.03—Construction Methods: Concrete surfaces which are to be waterproofed shall be smooth, free from projections or holes, which might cause puncturing of the membrane fabric. The surface shall be thoroughly cleaned of loose or foreign substances.

No priming or waterproofing shall be done in wet weather nor when the temperature of the air or concrete surface is below 35°F, without special authorization from the Engineer.

Unless otherwise directed, construction joints above those at the footings of abutments, wing walls and retaining walls shall be made impervious to water by the application of membrane waterproofing on the unexposed surface for a distance of 18 inches on each side of the joint.

1. Primer: Beginning at the low point of the surface to be waterproofed, the primer shall be applied in one coat at a rate of 0.1 gal./s.y. The cure time of the primer shall be as recommended by the manufacturer.

2. Membrane Fabric: The waterproofing fabric and the seal coat shall be applied as follows:

The asphalt shall be heated to a temperature of about 300°F (not over 350°F) with frequent stirring to avoid local overheating. The heating kettles shall be equipped with approved thermometers.

Waterproofing shall begin at the low point of the surface so that water will run over and not against or along the laps. A section about 20 inches wide and the full length of the surface shall be mopped with hot asphalt, and a strip of fabric of half width shall be rolled into it, pressed into place to eliminate all air bubbles and to obtain close conformity with the surface. This strip and an adjacent section of the surface of a width equal to slightly more than half the width of the fabric being used, shall then be mopped with hot asphalt, and a full width of fabric rolled into it, completely covering the first strip, and pressed into place as before. This second strip and adjacent section of the concrete surface shall then be mopped with hot asphalt and the next succeeding strips of fabric "shingled" on so that there will be 2 layers of fabric at all points with laps not less than 2 inches wide. All end laps shall be at least 12 inches.

This process shall be continued until the entire surface to be waterproofed has been covered. The fabric shall then be given a final mopping of hot asphalt. The mopping on concrete shall cover the surface so that no gray spots appear, and on fabric it shall be sufficiently heavy to completely conceal the weave. Special care shall be taken that all laps are thoroughly sealed down.

To insure a watertight joint between the bridge deck and the curb, asphalt flashing cement shall be applied to the joint for a distance of approximately 2 inches up on the curb and 2 inches out on the deck after the primer has set up and before the membrane is started. After the final mopping of the membrane, an additional coat of asphalt flashing cement shall be applied to the top edge of the membrane waterproofing to completely cover the fabric and to seal the joint made with the curb.

Care shall be taken to prevent injury to the finished membrane by the passage over it of men and equipment. Any damage which may occur shall be repaired by patching. Patches shall extend at least 12 inches beyond the outermost damaged portion, and the second ply shall extend at least 3 inches beyond the first.

Vehicular traffic shall not be allowed to pass over the finished membrane waterproofing. Upon completion of the membrane waterproofing, at least 1 course of the bituminous concrete wearing surface shall be placed as soon as practicable in order to prevent damage to the membrane waterproofing.

7.07.04—Method of Measurement: The quantity to be paid for under this item will be the number of square yards of waterproofed surface in the completed and accepted work.

7.07.05—Basis of Payment: This item will be paid for at the Contract unit price per square yard for "Membrane Waterproofing (Woven Glass Fabric)" which price shall be full compensation for furnishing all material, equipment, labor and incidentals necessary to complete the item.

Pay Item	Pay Unit
Membrane Waterproofing (Woven Glass Fabric)	s.y.

SECTION 7.08
DAMPPROOFING

7.08.01—Description: Dampproofing of concrete or masonry work shall consist of a coating of asphalt as indicated on the plans and in accordance with these specifications.

7.08.02—Materials: The materials for this work shall meet the requirements of M.12.05.

7.08.03—Construction Methods: The surface to which the dampproofing coating is to be applied shall be cleaned of all loose and foreign material and dirt and shall be dry. Where necessary, the Engineer may require the surface to be scrubbed with water and a stiff brush, after which the surface shall be allowed to dry before the application of the primer.

Concrete, brick or other surfaces which are to be protected by dampproofing shall be thoroughly clean and dry before the primer is applied. Apply one coat of primer and one coat of sealer using methods, application rates and temperature constraints as recommended by the manufacturer of each product.

Care shall be taken to confine all applied material to the areas to be dampproofed and to prevent disfigurement of any other parts of the structure by dripping or spreading.

7.08.04—Method of Measurement: This work will be measured for payment by the number of square yards of dampproofing, consisting of primer and seal coat, completed and accepted within the neat lines shown on the plans or as ordered by the Engineer.

7.08.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Dampproofing," complete in place, including all material, equipment, tools, labor and incidental expense.

Pay Item	Pay Unit
Dampproofing	s.y.

SECTION 7.13
PERMANENT STEEL SHEET PILING

7.13.01—Description: This specification covers only steel sheet piling shown on the plans to be left in place so that it becomes a part of the finished structure. This steel sheet piling shall meet the sheeting design indicated on the plans with respect to type, size, and waler spacing, or it shall consist of any type of adequately braced steel sheet pile wall meeting these specifications, which the Contractor elects to build to satisfy, and which does satisfy, the condition that existing facilities be properly retained during excavation for and placement of substructures.

7.13.02—Materials: Materials for steel sheet piling shall meet the requirements of ASTM A328.

7.13.03—Construction Methods: The sheet piling shall be safely designed and braced as necessary for proper performance of the work. Construction shall be such as to permit excavation as required. Interior dimensions shall be such as to give sufficient clearance for construction of forms and their inspection and for batter pile clearance when necessary. Movements of steel sheet piling or bracing which prevent the proper completion of the substructure shall be corrected at the sole expense of the Contractor. No part of the steel sheet piling or bracing shall be allowed to extend into the substructure without written permission of the Engineer.

The Contractor shall submit to the Engineer for approval plans showing the proposed method of construction prior to the start of such construction. The furnishing of such plans shall not serve to relieve the Contractor of any part of his responsibility for the safety of the work or for the successful completion of the Project.

The steel sheet piling shall be cut off at the elevation shown on the plans or as ordered by the Engineer. All material cut off shall remain the property of the Contractor and shall be disposed of by the Contractor.

7.13.04—Method of Measurement: Permanent steel sheet piling will be measured for payment by the square foot. This area will be measured or computed from the horizontal and vertical payment limits shown on the plans or as ordered.

7.13.05—Basis of Payment: This work will be paid for at the Contract unit price per square foot for "Permanent Steel Sheet Piling," which price shall include the cost of designing, furnishing, driving, cutting off, and all incidental expenses including all materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Permanent Steel Sheet Piling	s.f.

SECTION 7.14
TEMPORARY SHEET PILING

7.14.01—Description: Temporary sheet piling shall be constructed where shown on the plans. This sheet piling shall be removed upon completion of the permanent work, except that some sections may be left in place when so ordered by the Engineer. For purposes of this specification, temporary sheet piling shall be any type of adequately braced sheet pile wall which the Contractor elects to build to satisfy, and which does satisfy, the condition that existing facilities be properly retained during excavation for the placement of substructure or other facilities.

7.14.02—Materials: Materials of steel sheet piling shall conform to the requirement of ASTM A328. Timber sheet piling shall conform to the requirements of M.09.01-1. Materials other than steel or timber, or a combination of these may be used provided they are properly designed for the purpose intended.

7.14.03—Construction Methods: Temporary sheet piling shall be safely designed and shall be carried to adequate depths and braced as necessary for proper performance of the work. Construction shall be such as to permit excavation as required. Interior dimensions shall be such as to give sufficient clearance for construction of forms and their inspection and for batter pile clearance when necessary. Movements of sheet piling or bracing which prevent the proper completion of the substructure shall be corrected at the sole expense of the Contractor. No part of the temporary sheet piling or bracing shall be allowed to extend into the substructure without written permission of the Engineer.

Working drawings and design calculations for temporary sheeting shall be submitted in accordance with the requirements of 1.05.02-2. The working drawings and design calculations shall be prepared, sealed, and signed by a Professional Engineer, licensed in the state of Connecticut. The furnishing of such plans shall not serve to relieve the Contractor of any part of the responsibility for the safety of the work or for the successful completion of the Project.

Unless otherwise ordered by the Engineer, all parts of the temporary sheet piling shall be removed upon completion of the work for which it was provided. The excavation shall be backfilled and properly compacted, prior to removal of piling unless otherwise permitted by the Engineer. Sheet piling may be left in place at the option of the Contractor if so permitted by the Engineer, provided that it is cut off at an elevation directed by the Engineer and the cutoffs removed from the Site.

7.14.04—Method of Measurement: Temporary sheet piling will be measured for payment by the number of square feet of temporary sheet piling completed and accepted, computed from the horizontal and vertical payment lines shown on the plans or as ordered. If no payment limits are shown on the plans, the limits used for payment will be the actual horizontal limit of temporary sheet piling installed and accepted, and the vertical limit as measured from the bottom of the exposed face of the sheeting to the top of the retained earth behind the sheeting. The measurement for temporary sheet piling which is used as a common wall for staged construction will be the horizontal payment limit shown on the plans and the greater vertical dimension of the common wall face.

No measurement will be made of end extensions or returns necessary for the safety of the retained facility. Sheeting ordered left in place by the Engineer shall be measured in accordance with 7.15.04.

Sheet piling left in place solely at the Contractor's option, with the Engineer's permission, will not be measured for payment.

7.14.05—Basis of Payment: Payment for this work will be made at the Contract unit price per square foot for "Temporary Sheet Piling," measured as described above, which price shall include all design, materials, equipment and labor incidental to the construction and removal of the temporary sheet piling required at the locations specified on the plans; including removal of obstructions, repair and correction, adjustments or reconstruction required by the plans. Any common sheet piling wall required for staged construction will be measured for payment only once. For sheet piling ordered left in place, the Department will, however, make an additional payment at the Contract unit price per square foot for "Sheet Piling Material Left in Place."

Pay Item	Pay Unit
Temporary Sheet Piling	s.f.

NOTES

SECTION 7.15
SHEET PILING MATERIAL LEFT IN PLACE

7.15.01 and 7.15.02—Description and Materials: This specification covers only that portion of the temporary sheet piling that may be ordered left in place by the Engineer.

7.15.03—Construction Methods: The Contractor shall submit to the Engineer for approval, plans showing the proposed method of construction prior to the start of such construction. If no limits are shown on the plans for the sheet piling that may be ordered left in place, then the plans for the entire temporary installation shall be submitted for approval.

7.15.04—Method of Measurement: Sheet piling material left in place will be measured for payment by the square foot. This area will be measured or computed from the horizontal and vertical payment limits shown on the plans or as ordered.

7.15.05—Basis of Payment: Payment for this work will be made as follows:

That portion of the temporary sheet piling ordered left in place will be paid for at the Contract unit price per square foot for "Sheet Piling Material Left in Place," applying to 1 or more structures or portions of structures, which price shall include only the cost of material left in place. All other expenses shall be paid for under the item for "Temporary Sheet Piling."

Pay Item	Pay Unit
Sheet Piling Material Left in Place	s.f.

NOTES

SECTION 7.16
TEMPORARY EARTH RETAINING SYSTEM

7.16.01—Description
7.16.02—Materials
7.16.03—Construction Methods
7.16.04—Method of Measurement
7.16.05—Basis of Payment

7.16.01—Description: The temporary earth retaining system shall be any type of adequately braced temporary retaining wall which the Contractor elects to build to satisfy the condition that existing or planned facilities be properly retained during excavation or placement of fill. The temporary earth retaining system shall be designed by the Contractor and constructed where shown on the plans. The Contractor shall remove this system upon completion of the permanent work, unless the Engineer allows some sections to remain in place.

7.16.02—Materials: Steel sheet piling shall meet the requirements of ASTM A328. Timber sheet piling shall meet the requirements of M.09.01-1. Materials other than steel or timber, or a combination of these may be used provided they are properly designed for the purpose intended. Systems utilizing other material(s) shall meet the manufacturer's specifications and Project specifications. The parts list shall be furnished for the proprietary system and the Contractor shall provide Materials Certificates for the parts.

7.16.03—Construction Methods: The temporary earth retaining system shall be safely designed and shall be carried to adequate depths and braced as necessary for proper performance of the work. Construction shall be such as to permit excavation or fill as required. Interior dimensions shall be such as to give sufficient clearance for construction of forms and their inspection, and for battered pile clearance when necessary. Movements of the system or bracing which prevent the proper completion of the substructure shall be corrected at the sole expense of the Contractor. No part of the temporary earth retaining system or bracing shall be allowed to extend into the substructure without written permission of the Engineer.

Working drawings and design calculations for the temporary earth retaining system shall be submitted in accordance with the requirements of 1.05.02-2. The working drawings and design calculations shall be prepared, sealed, and signed by a Professional Engineer, licensed in the State of Connecticut. The furnishing of such plans shall not serve to relieve the Contractor of any part of its responsibility for the safety of the work or for the successful completion of the Project.

Unless otherwise ordered by the Engineer, all parts of the temporary earth retaining system shall be removed upon completion of the work for which it was provided. The excavation shall be backfilled and properly compacted, prior to removal of the system, unless otherwise permitted by the Engineer. The temporary earth retaining system may be left in place at the option of the Contractor if so permitted by the Engineer, provided that it is cut off at an elevation as directed by the Engineer and the cutoffs removed from the Site.

7.16.04—Method of Measurement: Temporary earth retaining system will be measured for payment by the number of square feet of temporary earth retaining system completed and accepted, as computed from the horizontal and vertical payment limits shown on the plans or as ordered. If no payment limits are shown on the plans, the limits used for payment will be the actual horizontal limit of temporary earth retaining system installed and accepted, and the vertical limit as measured from the bottom of the exposed face of the wall system to the top of the retained earth behind the system. The measurement for temporary earth retaining system which is used as a common wall for staged construction will be the horizontal payment limit shown on the plans and the greater vertical dimension of the common wall face.

No measurement will be made of end extensions or returns necessary for the safety of the retained facility. Earth retaining system ordered left in place by the Engineer shall be measured in accordance with "Earth Retaining System Left in Place."

Earth retaining systems left in place solely at the Contractor's option, and with the Engineer's permission, will not be measured.

7.16.05—Basis of Payment: Payment for this work will be made at the Contract unit price per square foot for "Temporary Earth Retaining System" which price shall include all design, materials, equipment and labor incidental to the construction and removal of the temporary earth retaining system required at the

locations specified on the plans; including removal of obstructions, repair and correction, adjustments or reconstruction required by the plans.

Any common earth retaining system required for staged construction will be measured for payment only once.

Pay Item	Pay Unit
Temporary Earth Retaining System	s.f.

SECTION 7.17
EARTH RETAINING SYSTEM LEFT IN PLACE

7.17.01—Description: This specification covers only that portion of the temporary earth retaining system that may be ordered left in place by the Engineer or designated in the plans to be left in place.

7.17.02—Materials: The materials shall be as specified in 7.16.02.

7.17.03—Construction Methods: The Contractor shall submit plans showing the proposed method of construction prior to the start of such construction to the Engineer for approval.

7.17.04—Method of Measurement: Earth retaining system material left in place will be measured for payment by the square foot. This area will be measured or computed from the horizontal and vertical payment limits shown on the plans or as ordered. If no payment limits are shown on the plans, the limits used for payment will be the actual horizontal limit of temporary earth retaining system ordered or designated in the plans to be left in place, and the vertical limit will correspond to the method of measurement of the temporary earth retaining system.

Temporary earth retaining system left in place solely at the Contractor's option, and with the Engineer's permission, will not be measured for payment.

7.17.05—Basis of Payment: Payment for this work will be made as follows:

That portion of the temporary earth retaining system ordered or designated in the plans to be left in place will be paid for at the Contract unit price per square foot for "Earth Retaining System Left in Place" applying to 1 or more structures or portions of structures, which price shall include only the cost of material left in place. All other expenses shall be paid for under the item "Temporary Earth Retaining System."

Pay Item	Pay Unit
Earth Retaining System Left in Place	s.f.

NOTES

SECTION 7.28
CRUSHED STONE FOR SLOPE PROTECTION

7.28.01—Description: This item shall consist of crushed stone or gravel placed to a uniform depth on that portion of the embankment at the abutments of designated structures, to the limits and for the depth shown on the plans, or as directed by the Engineer.

7.28.02—Materials: The crushed stone shall meet the requirements of Table M.01.02-2 for No. 3 coarse aggregate.

7.28.03—Construction Methods: The area on which the crushed stone is to be placed shall be shaped to a reasonably true surface prior to placing the crushed stone. The stone shall be spread by any suitable means which will not crush the stone and shall be shaped to a smooth uniform finished grade.

7.28.04—Method of Measurement: Crushed stone or gravel, except when shipped by rail, shall be weighed on scales furnished by the Contractor. The scales shall be of a type satisfactory to the Engineer and shall be sealed, at the expense of the Contractor, as often as the Engineer may require. All weighing shall be done in the presence of a Department representative. If broken stone is shipped by rail, the car weights may be accepted, but scales shall be used as provided above if the Engineer so directs.

7.28.05—Basis of Payment: This work will be paid for at the Contract unit price per ton for "Crushed Stone for Slope Protection," complete in place, which price shall include all materials, tools, equipment and labor incidental thereto.

Pay Item	Pay Unit
Crushed Stone for Slope Protection	ton

SECTION 7.32
CONCRETE BLOCK SLOPE PROTECTION

7.32.01—Description: Work under this item shall consist of furnishing and placing concrete block slope protection on an approved base to the limits and dimensions as shown on the plans or as directed by the Engineer.

7.32.02—Materials: Concrete blocks for this work shall meet the requirements of M.12.12.
Granular Base: This material shall meet the requirements of M.02.03 and Grading "C" of M.02.06.
Grout: Grout for this item shall meet the requirements of M.11.04.

7.32.03—Construction Methods: The surface on which the granular base is to be placed shall be shaped to an even surface and to the desired grade. The granular base shall be placed to the thickness shown on the plans and after thorough compaction shall be dressed smooth and to the required grade. The blocks shall be set in straight rows as shown on the plans with staggered joints normal to the rows. Joints between concrete blocks shall be not greater than 1/2 inch wide. The blocks shall be carefully laid and solidly rammed into position and shall be clean when placed. Cutting of blocks to meet pattern requirements will be permitted, subject to approval of the Engineer. The paved surface shall be tested with a straight edge not less than 10 feet long and the blocks found to be above or below the grade by more than 1/4 inch shall be taken up and reset. Each section of paving must be acceptable to the Engineer before the joints in that section are filled.

The blocks shall be thoroughly wetted with water immediately prior to the placing of the grout. Grout shall be spread over the blocks and worked into the joints beginning at the lowest elevation and progressing upward, so as to maintain the upper surface of the grout at approximately even rows, until all joints are completely filled. The surface shall then be swept and all excess grout removed from the surface of the blocks before initial set has developed. At such time after initial set has occurred, as will be determined by the Engineer, the surface of the blocks shall be cleaned by brushing, while simultaneously flushing with water, in such a manner as to completely remove the remaining grout from the surface of the blocks without dislodging any of the grout in the joints.

7.32.04—Method of Measurement: This work will be measured for payment by the number of square yards in the face area of accepted “Concrete Block Slope Protection” completed within the neat lines as shown on the plans or as ordered by the Engineer.

7.32.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Concrete Block Slope Protection," complete in place, which price shall include all materials, equipment, tools, and labor incidental thereto, also necessary excavation and grading below the surface of the roadway excavation and outside the limits of structure excavation and placing the granular base.

Pay Item	Pay Unit
Concrete Block Slope Protection	s.y.

SECTION 7.51
UNDERDRAIN AND OUTLETS

7.51.01—Description: Underdrains shall consist of pipe pervious to water, laid in a trench refilled with pervious material. They shall be of the dimensions and details as indicated on the plans. They shall be classed as "Underdrains," "Foundation Underdrains," "Slope Underdrains" or "Structure Underdrains."

Outlets for underdrains shall consist of pipe laid in a trench and refilled with earth. The size and type of outlet pipe shall be the same as that of the underdrain to which it is connected, except that it shall not be pervious to water.

7.51.02—Materials:

(1) Pipe: The pipe of underdrains and outlets shall meet the requirements of M.08.01. The kind of pipe to be installed shall be at the option of the Contractor unless a specific type of pipe is indicated on the plans or in the special provisions.

(2) Aggregates: The aggregates specified for filling the trench shall meet the requirements of M.08.03.

(3) Joint Sealants: The materials for sealing and coupling of joints shall meet the requirements of M.08.01.

(4) Geotextile: Geotextile shall be as specified in M.08.01-19 Geotextiles.

7.51.03—Construction Methods: The trench for the underdrain shall be excavated in accordance with the requirements of 2.86.03. The dimensions of the trench shall be as indicated on the plans or as ordered. Where the bottom of the trench is unstable or in rock, the trench shall be excavated 6 inches deeper and an additional 6 inches layer of granular fill or aggregate similar to that used to fill the trench shall be placed and compacted in the trench.

Where the perforations are to be at the bottom of the pipe, the aggregate for filling the trench shall then be placed to a depth of 3 inches and tamped true to grade. The pipe shall be placed and firmly bedded on the aggregate. This aggregate shall be placed whether the pipe is encased with geotextile or not.

When the pipe used has a bell, the pipe shall be installed with the bell end upgrade with the spigot end entered fully into the adjacent bell.

When clay or concrete pipe is used, the joints will not have to be filled with a joint sealant or fitted with a gasket.

When metal, bituminized fiber, plastic, polyethylene or asbestos cement pipe is used, the pipe shall be carefully butted together and held by bands or other approved means so as to prevent any displacement of the joint.

After the pipe has been installed as described above, the aggregate shall be placed carefully around and over the pipe to a height of 12 inches above the top of the pipe. The remainder of the trench shall be filled with aggregate and tamped in layers as shown on the plans. When the underdrain pipe is used with the holes in an upward position, and in all cases where sand is used instead of the aggregate described hereinbefore, a protective 3-inch minimum layer of 3/8-inch aggregate shall be placed over the pipe and around all of the holes. Geotextile may be substituted for the 3-inch layer of aggregate. When geotextile is used, the entire length of each drain pipe shall be wrapped with the fabric and the seams lapped and welded or bonded. Where the seams of the geotextile are not welded or bonded, they shall be lapped to a minimum width equal to the diameter of the pipe for 6-inch pipe and larger and a minimum of 6 inches for smaller pipe.

In all cases where subbase material or gravel is to be placed over the underdrains, a layer of at least 6 inches of subbase material or gravel shall be placed over the underdrain immediately after its completion.

For outlets, the trench shall be excavated and the pipe installed in accordance with the requirements of 2.86.03.

Where shown on the plans or directed by the Engineer, the Contractor shall connect underdrains or outlets to existing or proposed drainage systems or structures.

This work shall be performed in a workmanlike manner satisfactory to the Engineer by installation of tees or wyes branches or by providing a hole in the main line underdrain.

Where the upgrade end of the underdrain does not enter a structure, it shall be capped or plugged as directed.

7.51.04—Method of Measurement: This work will be measured for payment by the actual number of linear feet of underdrains, foundation underdrains, slope underdrains, structure underdrains and outlets for underdrains, completed, accepted and measured in place. Trench excavation will not be measured for payment. Rock in drainage trench will be measured for payment in accordance with 2.86.04.

7.51.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Underdrain," "Foundation Underdrain," "Slope Underdrain," "Structure Underdrain" and "Outlets for Underdrain" complete in place, which price shall include pipe of the size specified, elbows, tees, wyes, couplings, fitting, trench excavation, geotextile, aggregate, sand, tools, material and labor incidental thereto.

There will be no direct payment made for capping, plugging or connecting underdrains or outlets to existing or proposed drainage systems or structures, but the cost thereof shall be included in the cost of the underdrain items involved.

Rock in drainage trench will be paid for in accordance with 2.86.05 at the Contract unit price per cubic yard for "Rock in Drainage Trench Excavation" of the applying depth.

Pay Item	Pay Unit
(Size) Underdrain	l.f.
(Size) Foundation Underdrain	l.f.
(Size) Slope Underdrain	l.f.
(Size) Structure Underdrain	l.f.
(Size) Outlets for Underdrain	l.f.
(Size-Type) Underdrain	l.f.
(Size-Type) Foundation Underdrain	l.f.
(Size-Type) Slope Underdrain	l.f.
(Size-Type) Structure Underdrain	l.f.
(Size-Type) Outlets for Underdrain	l.f.

SECTION 7.55
GEOTEXTILE

7.55.01—Description: This item shall consist of furnishing and installing geotextile in the locations and to the dimensions shown on the plans as directed or as approved by the Engineer.

7.55.02—Materials: Geotextile shall meet the requirements of M.08.01-19 Geotextiles. Materials incidental to and necessary for the installation of the geotextile, such as, but not limited to sewing thread, staples, pins, etc., shall meet the requirements of the manufacturer of the geotextile.

7.55.03—Construction Method: The geotextile shall be installed at the locations and to the dimensions shown on the plans or as directed by the Engineer. Geotextile shall be installed as recommended by the manufacturer for the specific use or purpose intended, or as otherwise approved by the Engineer.

7.55.04—Method of Measurement: This work will be measured for payment by the actual number of square yards of the type indicated on the plans or authorized by the Engineer. Geotextile specifically included in the payment of another item will not be measured for payment under this item. No additional measurement will be made for necessary lap material.

7.55.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard of "Geotextile" or "Geotextile (Type)," complete in place, which price shall include all materials, labor, tools, and equipment incidental and necessary for each type of installation and removal where necessary.

Pay Item	Pay Unit
Geotextile	s.y.
Geotextile (Type)	s.y.

SECTION 8.03
PAVED DITCHES, PAVED APRONS AND PAVED CHANNELS

8.03.01—Description
8.03.02—Materials
8.03.03—Construction Methods
8.03.04—Method of Measurement
8.03.05—Basis of Payment

8.03.01—Description: The work under this item includes placing and compacting of a bituminous concrete course on a pre-excavated foundation forming paved ditches, aprons or channels in accordance with the line, grade, compacted final thickness and typical cross-section shown on the plans.

8.03.02—Materials: The materials for this work shall meet the following requirements:
Bituminous Concrete Curb Mix shall meet the requirements of 4.06 and M.04.01.
Processed Aggregate Base shall meet the requirements of M.05.01.

8.03.03—Construction Methods: The processed aggregate base course shall be placed in a single course, 4 inches compacted thickness, in accordance with 3.04.03. The surface shall be a 2 inch course of bituminous concrete curb mix. The bituminous concrete shall be placed and thoroughly compacted with compaction equipment suitable for small areas.

8.03.04—Method of Measurement: The quantity to be measured for these items will be the surface area in square yards of paved ditch, paved apron or paved channel constructed and accepted.
Formation of Subgrade and Processed Aggregate Base will not be measured for payment.

8.03.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Paved Ditch," "Paved Apron" or "Paved Channel." The price shall include all materials, tools, equipment and work incidental thereto.
Excavation will be paid for in accordance with 2.06.
Bituminous Concrete Lip Curbing for Paved Channels will be paid for in accordance with 8.15.

Pay Item	Pay Unit
Paved Ditch	s.y.
Paved Apron	s.y.
Paved Channel	s.y.

SECTION 8.11
CONCRETE CURBING

8.11.01—Description: This item shall consist of concrete curbing, furnished in accordance with the dimensions and details of the plans, and installed to the lines and grades shown on the plans.

8.11.02—Materials: All concrete curbing shall be constructed with Class PCC04460 Concrete as defined in M.03.02.

Precast curbing shall meet the requirements of M.08.02-4.

Joint filler shall meet the requirements of M.03.08-2.

Base material, if required, shall meet the requirements of M.02.01, M.02.02 or M.05.01.

8.11.03—Construction Methods: Construction methods shall meet the requirements of 6.01.03, as supplemented by the following:

1. Excavation: Excavation shall be made to the required depth, and the base upon which the curbing is to be set shall be compacted to a firm, even surface.

2. Section Lengths and Joints: All straight curbing sections shall be uniform length and a minimum of 8 feet.

Curved curb section lengths may vary with radii of curves.

When a gap of less than 8 feet is required for closure, the length of curbing may be varied, but no section less than 2 feet will be permitted.

For both precast and cast-in-place concrete curbing, a 1/2 inch joint shall be filled with joint filler at intervals of approximately 50 feet, and contraction joints shall be placed at intervals of approximately 15 feet.

3. Cast-In-Place Curbing: Forms shall be clean and founded on a moist, firm, unfrozen base and the curbing shall be constructed so that the exposed faces may be accessed before the concrete has taken final set to allow finishing. Cast-in-place curbing shall be finished in accordance with 6.01.03-II-10(b).

4. Precast Concrete Curbing: The Contractor shall stabilize the precast concrete curbing during installation until backfilling is complete.

Precast curbing set on a radius of 50 feet or less shall be fabricated to the required radius within the manufacturer’s tolerance..

5. Backfilling: The backfill shall consist of approved material placed in 6 inch layers and each layer shall be thoroughly compacted. The final elevation of the backfill shall match the lines shown on the plans or as ordered by the Engineer.

8.11.04—Method of Measurement: This work will be measured for payment along the top of the curb and will be the actual number of linear feet of concrete curbing completed and accepted.

8.11.05—Basis of Payment: Payment for this work will be made at the Contract unit price per linear foot for "Concrete Curbing" of the type specified, complete and accepted in place, which price shall include all excavation, materials, equipment, tools, backfilling, disposal of surplus material, and labor incidental thereto.

There will be no direct payment for furnishing, placing and compacting base material, but the cost of this work shall be considered as included in the general cost of the work.

Pay Item	Pay Unit
Concrete Curbing (Type)	l.f.

SECTION 8.13
STONE CURBING

8.13.01—Description: This item shall consist of stone curbing, furnished in accordance with the dimensions and details of the plans, and installed to the lines and grades shown on the plans.

8.13.02—Materials: The stone curbing shall meet the requirements of M.12.06.

The mound of concrete required at all stone curbing joints shall meet the requirements of any mix design type listed in Table M.03.02-1.

Mortar shall meet the requirements of M.11.04.

Base material, if required, shall meet the requirements of M.02.01, M.02.02 or M.05.01.

8.13.03—Construction Methods:

1. **Excavation:** Excavation shall be made to the required depth and the base upon which the curbing is to be set shall be compacted to a firm, even surface.
2. **Section Lengths and Joints:** For straight curbing, approximately 80% of the stones shall be furnished in lengths of not less than 6 feet and the remaining 20% in lengths of not less than 4 feet, interspersed at random in order to allow for closures.

Curved curb section lengths may vary with radii of curves, but no section less than 2 feet will be permitted.

The space between each section of curbing shall be 1/2 inch and shall be pointed with mortar for the full depth of the curbing. At uniform intervals of approximately 50 feet, one joint shall be left unfilled

A mound of concrete, as shown on the plans, shall be placed at each joint prior to placing sections adjacent to the joint.

Break back of stone curbing shall be as shown on the plans.

The ends of the curbing at driveways and intersections shall be cut at a bevel or rounded, as directed by the Engineer.

3. **Curved Stone Curbing:** This shall be defined as curbing set on a radius of 100 feet or less and shall be fabricated to the required radius within the manufacturer's tolerance.
4. **Backfilling:** The backfill shall consist of approved material placed in 6 inch layers and each layer shall be thoroughly compacted. The final elevation of the backfill shall match the lines shown on the plans, or as ordered by the Engineer.

8.13.04—Method of Measurement: This work will be measured for payment along the top of the curb and will be the actual number of linear feet of stone curbing or curved stone curbing completed and accepted.

8.13.05—Basis of Payment: Payment for this work will be made at the Contract unit price per linear foot for "Stone Curbing" or "Curved Stone Curbing," of the type and size specified, complete and accepted in place, which price shall include all excavation, materials, equipment, tools, backfilling, disposal of surplus material and labor incidental thereto.

There will be no direct payment for furnishing, placing and compacting base material, beveling or rounding the ends of the curbing and pointing the joints with mortar, but the cost of this work shall be considered as included in the general cost of the work.

Pay Item	Pay Unit
Stone Curbing (Type-Size)	l.f.
Curved Stone Curbing	l.f.

SECTION 8.14
RESET STONE CURBING

8.14.01—Description: This work shall consist of the removing and resetting or adjusting of existing stone curbing to the lines and grades given, in accordance with the dimensions and details of the plans or as ordered and in conformity with these specifications.

8.14.02—Materials: All existing curbing which is acceptable shall be used. The reset stone curbing shall be in lengths of not less than 4 feet long, except where necessary for closures, where no piece shall be less than 3 feet long. Mortar for this work shall be as specified in M.11.04.

8.14.03—Construction Methods: The construction methods for this work shall meet the requirements of 8.13.03 for stone curbing, as supplemented by the following requirements:

The curbing to be reset shall be removed with care to avoid damage and, if ordered, shall be transported to a point or points on the project which will allow all the reset curbing to be installed in continuous lines. Curbing removed and not reset shall remain the property of the State unless otherwise ordered by the Engineer and shall be transported to such points, adjacent to the work, as the Engineer may designate. Where adjustments only have to be made to curbing, such adjustments shall be carried out as specified by the Engineer.

8.14.04—Method of Measurement: This work will be measured for payment by the actual number of linear feet of the class of curbing, reset and accepted. Measurement shall be made along the top arris line of face of curb.

8.14.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Reset Curbing," complete in place, which price shall include all materials, equipment, tools and labor incidental thereto and all excavation, backfilling, disposal of surplus material and all drainage openings.

There will be no direct payment for furnishing, placing and compacting granular base, beveling or rounding the ends of the curbing, sealing the joints with mortar, removing the curbing and hauling it to any location on or adjacent to the project as directed by the Engineer; but the cost of this work shall be considered as included in the general cost of the work.

Pay Item	Pay Unit
Reset Curbing (Type-Size)	l.f.

SECTION 8.15
BITUMINOUS CONCRETE CURBING

8.15.01—Description: Bituminous concrete curbing shall consist of machine laid bituminous concrete, constructed on the pavement to the dimensions and details shown on the plans, or as ordered, and in accordance with the specifications.

8.15.02—Materials: Materials, including tack coat, for this work shall meet the requirements of M.04, curb mix.

8.15.03—Construction Methods: The provisions of 4.06 shall govern except that the requirements pertaining to density will not apply. In addition, the curbing shall be constructed in accordance with the following requirements:

Prior to the arrival of the mixture on the Project Site, the surface of the pavement where the curbing is to be constructed shall be cleaned of all loose and foreign material. The surface, which shall be perfectly dry and clean at the time the mix is placed, shall be coated with an approved tack coat just prior to placing the mixture.

On arrival at the Site, the mixture shall be transferred from the truck to the hopper of the curbing machine; and the mixture shall be kept clean and free from dirt and foreign materials at all times.

The surface of the curbing shall be tested with a 10 foot straightedge, and any variation from a true line exceeding 1/4 inch shall be satisfactorily corrected. The only compaction required shall be that obtained by the approved mechanical curbing machine.

Where machine work is impractical, the Engineer may permit hand-laid curbing to be constructed.

If the design of the curbing machine is such that the outside wheels operate outside of the curb, the Contractor will be required to obtain a smooth surface by grading and consolidating the area on which the outside wheel of the machine rides, and this work shall be done at the Contractor's expense.

After the completion of curbing, traffic shall be kept at a safe distance for a period of not less than 24 hours and until the curbing has set sufficiently to prevent injury or damage to the work.

8.15.04—Method of Measurement: This work will be measured for payment along the top of the curb and will be the actual number of linear feet of bituminous concrete curbing completed and accepted.

8.15.05—Basis for Payment: This work will be paid for at the Contract unit price per linear foot for "Bituminous Concrete Curbing" of the type specified, complete in place, which price shall include all materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Bituminous Concrete (Type) Curbing	l.f.

SECTION 8.18
PROTECTIVE COMPOUND FOR BRIDGES

8.18.01—Description: Under this item, the Contractor shall furnish and apply uniformly a protective compound to the exposed concrete surfaces as indicated on the plans or as ordered by the Engineer.

8.18.02—Materials: The material for this work shall meet the requirements of M.03.09.

8.18.03—Construction Methods: Surface preparation, minimum number of days between casting of concrete and application of protective compound, number of coats, rate of coverage, method of application, and other pertinent criteria shall be in strict accordance with the instructions supplied by the manufacturer and as directed by the Engineer.

All surfaces to be coated shall be thoroughly cleaned of any dust, dirt, oil, or other objectionable materials prior to application of the protective compound.

8.18.04—Method of Measurement: This work will be measured for payment by the actual number of square yards of concrete coated and accepted within the designated limits.

8.18.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Protective Compound for Bridges," complete, which price shall include all equipment, tools, labor and materials, incidental thereto, including the preparation of the concrete surfaces.

Pay Item	Pay Unit
Protective Compound for Bridges	s.y.

SECTION 8.21
PRECAST CONCRETE BARRIER CURB

8.21.01—Description: Under this item, the Contractor shall furnish and install precast concrete barrier curb in the locations shown on the plans, or as directed by the Engineer.

8.21.02—Materials:

1. Concrete shall meet the requirements of M.14.01 amended as follows:
   a) Concrete shall have a minimum 28 days strength (f'c) of 4,000 psi.
   b) Coarse Aggregate shall meet the requirements of M.03.01-1, and the grading requirements for Class PCC04460.
   c) Fine Aggregate shall be light in color and shall conform in color and type to the samples on file at the Division of Materials Testing of the Department of Transportation located in Rocky Hill, Connecticut.
   d) Cement for light concrete shall be Type III or Type IIIA Portland Cement or light colored cement approved by the Engineer.
   e) The entrained air content shall be not less than 5%, or more than 7%.
   f) The manufacturer identification and casting date shall be permanently marked on the barrier curb by means of a non-corrosive metal or plastic tag as approved by the Engineer and in the location as shown on the plan.

2. Reinforcing steel shall be galvanized and shall meet the requirements of M.06.01.

3. Lifting hooks, keys, threaded inserts, bolts, devices and attachments shall be of the size indicated on the plans or of a design satisfactory for the purpose intended as approved by the Engineer.

4. Dowels shall be galvanized and shall meet the requirements of ASTM A36.

5. Galvanizing shall meet the requirements of ASTM A123.

6. Penetrating Sealer Protective Compound shall meet the requirements of M.03.09.

7. Precast concrete barrier curb shall be accepted on the basis of manufacturer's certification, as defined in M.08.02.

8.21.03—Construction Methods:

1. Precast Units: Concrete barrier units shall be precast in an approved plant in accordance with the applicable requirements of 5.14.03-4, 6, 7, 8 and 15 supplemented as follows:

Forms for precast concrete barrier units shall be of substantial construction, lined so as to produce a smooth dense surface with a uniform appearance. Form oil shall be a non-staining type. Air holes on exposed surfaces shall be filled immediately after removal of the forms to the satisfaction of the Engineer.

2. The precast units shall be placed on a prepared subbase and shall be installed to the lines and grades given and in accordance with the dimensions and details shown on the plans so as to produce a smooth continuous barrier curb.

Special transition units, as required, may be cast in place in accordance with 6.01.03.

3. Backfilling: The backfilling shall be completed to the lines shown on the plans, or as ordered, by filling to the required elevation with approved material which shall be placed in layers of not over 6 inches deep with each layer compacted until firm and solid.

4. Whenever concrete barrier is being constructed adjacent to areas open to traffic, the Contractor shall complete the installation to and including the designed terminal treatment at the close of each day's work so as to prevent any hazard that would be caused by leaving an exposed end.

On long runs or other locations where it is not practical to complete the installation to and including the designed terminal treatment, the Contractor shall install and maintain temporary protection by methods such as impact attenuating devices for terminating the barrier so as to minimize any hazard.

The Contractor shall submit to the Engineer for approval details of proposed methods for the temporary terminal treatment of the end section.

5. Precast units shall meet the 28 day strength requirements before shipping. Precast units that have been damaged shall be removed from the Project at the Contractor's expense. Each line of barrier shall be uniform in color.

6. Precast units shall be coated with a penetrating sealer protective compound that meets the requirements of M.03.09.

7. Where called for, a cast-in-place concrete cap shall be installed above granular fill placed between a double row of concrete barrier curb, as shown on the plans.

8.21.04—Method of Measurement: This work will be measured for payment along the centerline of the top of the concrete barrier and will be the actual number of linear feet of the shape and size of concrete barrier installed and accepted.

8.21.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Precast Concrete Barrier Curb," of the shape and size specified, complete in place, which price shall include all backfill, sand backfill, joint seal, materials, reinforcing steel, dowels, penetrating sealer protective compound, transportation, equipment, tools and labor incidental thereto.

When shown on the plans, backfill placed between a double row of concrete barrier curb shall be paid for under the item "Granular Fill." The concrete cap, welded wire fabric, 1/2 inch preformed joint filler and joint seal shall be paid for under the item “Class PCC03340.”

Pay Item	Pay Unit
(Shape) Precast Concrete Barrier Curb (Size)	l.f.

SECTION 8.22
TEMPORARY PRECAST CONCRETE BARRIER CURB

8.22.01—Description: Work under this item shall consist of furnishing, installing, relocating and removing temporary precast concrete barrier curb used to separate traffic from opposing traffic or the work area.

8.22.02—Materials: The materials for this work shall meet the requirements of 8.21.02.

When used barrier curb is furnished, the Contractor shall provide documentation stating from where the material came, what project it will be used on, the casting dates, and certification that the barrier meets all State requirements.

The delineator shall be fabricated of aluminum, steel, and plastic or of a material approved by the Engineer. The retroreflective sheeting shall be encapsulated lens sheeting as specified in M.18.09. Delineator fastening hardware or adhesive shall be suitable for the purpose intended.

The threaded steel connection rod shall be manufactured in accordance with AASHTO M 314, Grade 55. Threads shall be Unified National Coarse Series as specified in ANSI B1.1 and shall have Class 2A threaded tolerances before galvanizing.

Plain steel washers shall be manufactured in accordance with ANSI B18.22.

Heavy hex nuts shall be Grade A, manufactured in conformance with AASHTO M 291 and tapped oversize for galvanizing.

The threaded rod, washers and nuts shall be hot-dip galvanized in conformance with AASHTO M 232, Class C.

Connection loop bars shall be bent from smooth bars that conform to ASTM A36.

8.22.03—Construction Methods:

1. Precast Unit: Temporary concrete barrier units shall be precast in accordance with the pertinent requirements of 8.21.03, except the penetrating sealer protective compound need not be applied to the precast unit.

2. Installation: Temporary precast concrete barrier units shall be placed as shown on the plans or as directed by the Engineer, on a firm even surface so as to produce a smooth continuous barrier curb.

The Contractor shall maintain the temporary concrete barrier during all stages of construction. Any damaged material shall be removed and replaced by the Contractor at his expense.

The Contractor shall relocate the concrete barrier and its appurtenances to locations within the Project limits as shown on the plans or as ordered by the Engineer. When the temporary barrier is no longer required, it shall be removed completely from the Project and shall remain the property of the Contractor.

3. Delineator: The delineator shall be installed in the center on top of the barrier at the locations designated on the plans. They may be fastened by adhesive or hardware and must be maintained in good condition at all times.

DE-7 delineators shall be used when the barriers are on the right side of traffic or dividing traffic in the same direction.

DE-7 A delineators shall be used when the barriers are on the left side of traffic.

DE-7B delineators shall be used when the barriers divide opposing traffic lanes.

DE-7C delineators shall be used with the yellow side on the left side of traffic when traffic is alternated.

4. Connection Rod: Nuts at the connection rod shall be turned until the washer is drawn up against the connection loop. The connection loops must not be bent in the tightening process. For ease in removing the nuts, the threads may be waxed.

8.22.04—Method of Measurement: This work will be measured for payment along the centerline of the top of the concrete barrier and will be the actual number of linear feet of temporary concrete barrier furnished, installed and accepted.

Relocated temporary concrete barrier will be measured along the centerline of the top of the concrete barrier each time the barrier has been satisfactorily relocated as directed by the Engineer, including to and from the storage area. Storage of concrete barrier curb will not be measured for payment. Relocation of Temporary Precast Concrete Barrier Curb for access to the work area or for the convenience of the Contractor shall be considered incidental to Maintenance and Protection of Traffic and will not be measured for payment.

Delineators will be measured in accordance with 12.05.04.

8.22.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Temporary Precast Concrete Barrier Curb" complete in place, which price shall include all furnishing, transportation, initial installation, final removal, storage, materials, reinforcing steel, connecting rods, equipment, tools and labor incidental thereto. Each temporary precast concrete barrier curb will be paid for once regardless of the number of times it is used on the Project. Any temporary precast concrete barrier curbs that become lost, damaged or defaced shall be replaced by the Contractor at no cost to the State.

The relocation of the temporary precast concrete barrier curb will be paid for at the Contract unit price per linear foot for "Relocated Temporary Precast Concrete Barrier Curb," which price shall include all transportation, materials, equipment, tools and labor incidental thereto.

Delineators will be paid for in accordance with 12.05.05.

Pay Item	Pay Unit
Temporary Precast Concrete Barrier Curb	l.f.
Relocated Temporary Precast Concrete Barrier Curb	l.f.

SECTION 9.04
METAL BRIDGE RAIL

9.04.01—Description
9.04.02—Materials
9.04.03—Construction Methods
9.04.04—Method of Measurement
9.04.05—Basis of Payment

9.04.01—Description: This railing shall be selected from the different types of railing shown on the bridge plans and shall consist of metal pipe and fittings, structural steel shapes, castings or other material, anchor bolts, hardware fasteners, anchorages and terminal elements for connection to bridge parapets and fabricated in accordance with the dimensions and details given, and erected where shown on the plans or as ordered, in accordance with these specifications.

9.04.02—Materials: The material for the type of metal bridge rail selected for this work shall meet the requirements of 6.03.02 for metal bridge rail (structural steel). Malleable castings shall meet the requirements of the specifications for malleable iron castings, ASTM A47, Grade No. 32510. Ductile iron castings shall meet the Specifications for Ductile Iron Castings, ASTM A536, Grade 60-40-18 unless otherwise specified. In addition to the specified test coupons, test specimens from parts integral with the castings, such as risers, shall be tested for castings having a weight of more than 1000 lb. to determine that the required quality is obtained in the castings in the finished condition.

9.04.03—Construction Methods: The railing shall be accurately fabricated in accordance with the plans or as ordered.

Fabrication and erection shall be done in accordance with the requirements for steel bridges and structural steel as given hereinbefore. In the case of welded railing, after welding, all exposed joints shall be finished by grinding or filing to have a neat appearance.

Metal railings shall be carefully adjusted prior to fixing in place to insure proper matching at abutting joints and correct alignment and cambers throughout their length. Holes for field connections to be drilled in the field shall be drilled with the railing in place in the structure and at proper grade and alignment. Welding may be substituted for rivets or bolts in field connections, with the approval of the Engineer.

Aluminum welding shall be in accordance with the AWS "Structural Welding Code, Aluminum," AWS D1.2, latest issue.

Where aluminum alloys come in contact with other materials, the contacting surfaces shall be thoroughly coated with an aluminum impregnated caulking compound conforming to FS-TT-C598 plaster type caulking compound, aluminum pigmented.

Aluminum Alloys shall not be placed in contact with copper, copper base alloys, lead or nickel.

Metal railing shall be painted in accordance with the painting requirements for structural steel, except that aluminum railing and aluminum posts shall not be painted.

9.04.04—Method of Measurement: This work will be measured for payment by the number of linear feet of metal bridge rail, of the type selected, completed and accepted.

9.04.05—Basis of Payment: This work will be paid for at the contract unit price per linear foot for "Metal Bridge Rail" complete in place, which price shall include all material, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Metal Bridge Rail (Type)	l.f.

SECTION 9.05
STONE WALL FENCE

9.05.01—Description: This item shall consist of approved rubble stones laid without the use of mortar to form a standard stone wall fence of the dimensions and details shown on the plans, constructed where indicated on the plans or as ordered and in accordance with these specifications. The foundation courses shall be of large stones, and the thinnest courses shall be placed toward the top of the wall. Vertical joints shall be broken and the wall bound together with each fourth to sixth stone a header. Headers shall be evenly distributed throughout the wall. The exposed sides and top of the wall shall be roughly faced within the minimum and maximum dimensions called for, and all exposed stones shall fit neatly.

9.05.02—Materials: Stones for this work shall be rubble stones of suitable size and of a quality satisfactory to the Engineer. The stones shall be roughly rectangular in shape with at least one fairly even face.

9.05.03—Construction Methods: The earth along the lines of the proposed wall shall be excavated to a depth of not less than 6 inches and leveled off and stepped as necessary in a satisfactory manner so that the first or base course of stones may be stably bedded. All stones shall be laid on their natural beds. Sufficient chip stones or spalls shall be used so that each stone is bedded securely and all interstices shall be firmly-chinked. Spalls will not be allowed in the heart of the wall. The top of the wall shall be capped with suitable stones, conforming to the minimum and maximum dimensions called for on the detailed plans.

9.05.04—Method of Measurement: This work will be measured for payment by the number of linear feet of completed and accepted stone wall fence.

9.05.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Stone Wall Fence," complete in place, which price shall include materials, equipment, tools, labor and work incidental thereto; also all necessary excavation, refilling and disposal of surplus material.

Removal of material in stone walls that are to be removed and not used in new stone wall fences, will be paid for according to the provision of 2.02.

Pay Item	Pay Unit
Stone Wall Fence	l.f.

SECTION 9.06
WIRE FENCE

9.06.01—Description: Work under this item shall consist of furnishing and installing wire fence with wood or metal posts as shown on the plans.

9.06.02—Materials: Materials for this work shall meet the requirements of M.10.04. Concrete or grout, with the permission of the Engineer, may be hand mixed.

9.06.03—Construction Methods: Wire fence posts shall be set plumb and to the lines and grades shown on the plans. Posts shall be set in holes, and the area adjacent to the posts shall be backfilled with suitable material and thoroughly compacted. Any surplus or unsuitable material remaining after the completed installation shall be removed and disposed of by the Contractor. The Contractor is cautioned that underground utilities, which may be energized, may be present within the Project limits.

If an obstruction is encountered while driving or placing the metal or wood post, the Contractor shall notify the Engineer who will determine whether the obstruction shall be removed, the post or posts relocated, or the metal post in rock detail shall apply. Backfill shall be thoroughly tamped after the posts have been set plumb.

All end, corner and brace posts shall be set in concrete footings as shown on the plans. Braces shall be fastened to the post as shown on the plans.

1. Wire Fence with Wood Posts: For wire fence with wood posts the posts shall be set butt end down. The wire fencing shall be placed on the side of the posts facing the roadway. It shall be attached to the wood posts by wire staples. The fencing shall be stretched taut and fastened to the posts at each horizontal wire. Wood braces shall be fastened to notches in the posts to hold them firmly when the wire is pulled tight.

When exposed ledge rock is encountered, the fence shall be attached to metal posts which shall be set in a 6 inch minimum diameter hole drilled into the rock at least 12 inches deep and shall be grouted in correct position.

2. Wire Fence with Metal Posts: For wire fence with metal posts the line post shall be set with anchor plate at the bottom. On metal line posts set in rock, the anchor plate shall be omitted.

Steel posts shall be driven using suitable driving caps to prevent damage to the posts. When exposed ledge rock is encountered, the metal posts shall be set in a 6 inch minimum diameter hole drilled into the rock at least 12 inches deep and shall be grouted in correct position.

Adjustable brace bands shall be fastened to the metal corner, end and brace posts as shown on the plans.

9.06.04—Method of Measurement: This work will be measured for payment by the number of linear feet of completed and accepted wire fence measured from outside to outside of end posts.

9.06.05—Basis of Payment: This work will be paid for as follows:

1. Wire Fence with Wood Posts: This work will be paid for at the Contract unit price per linear foot for "Wire Fence with Wood Posts," complete in place, which price shall include all excavation, backfilling, materials, equipment, tools, labor, disposal of any surplus material and work incidental thereto.

No additional compensation will be made for furnishing and installing metal fence posts where required or for the drilling and grouting necessary to place them.

2. Wire Fence with Metal Posts: This work will be paid for at the Contract unit price per linear foot for "Wire Fence with Metal Posts," complete in place, which price shall include all excavation, backfilling, materials, equipment, tools, labor, disposal of any surplus material and work incidental thereto.

Pay Item	Pay Unit
Wire Fence with Wood Posts	l.f.
Wire Fence with Metal Posts	l.f.

SECTION 9.10
METAL BEAM RAIL

9.10.01—Description
9.10.02—Materials
9.10.03—Construction Methods
9.10.04—Method of Measurement
9.10.05—Basis of Payment

9.10.01—Description: Work under this item shall consist of the installation of or conversion to a single or double line of steel rail elements and terminal sections fastened to wood or steel posts with or without rubrail, and the appropriate treatment at bridge parapets, barriers, or other fixed objects as shown on the plans. This item shall include metal beam rail types: w-beam, thrie-beam and box-beam. It shall be installed or converted in the locations indicated and fabricated in accordance with the lines, designations, dimensions, and details on the plans or as ordered by the Engineer.

9.10.02—Materials: The material for metal beam rail shall meet the requirements of M.10.02 and the following:

1. Chemical anchoring material shall meet the requirements of M.03.07.

2. Metal beam rail delineators shall meet the requirements of M.18.09 and M.18.13.

3. When converting rail, the Contractor shall reuse any undamaged existing rail elements, appropriate posts, delineators, and lap bolts within the Project limits as approved by the Engineer to construct the converted rail. The Contractor shall use new materials when any components of the existing railing are damaged or missing and cannot be obtained from other rail systems being removed or converted within the Project limits.

9.10.03—Construction Methods: Steel posts shall be driven. The Contractor shall use suitable driving caps and equipment to prevent damage to the posts during driving. Where rock, boulders or debris are encountered while driving the posts, the obstruction shall be removed to make each hole large enough to permit driving of the posts. Each hole shall then be backfilled with suitable material and thoroughly compacted before driving the posts. Any surplus or unsuitable material remaining after the completed installation shall be removed and disposed of by the Contractor.

Wood posts shall be set in holes, and the area adjacent to the posts shall be backfilled with suitable material and thoroughly compacted. Any surplus or unsuitable material remaining after the completed installation shall be removed and disposed of by the Contractor.

The Contractor is cautioned that underground utilities, which may be energized, may be present within the Project limits.

The posts shall be located as shown on the plans, set plumb and in alignment with the rail or rail treatments. Where required, the blockouts, brackets, rubrails, back-up rails and rail elements shall then be erected to produce a smooth continuous rail as shown on the plans. The terminal connectors, rubrails, and rail elements shall be lapped in the direction of traffic.

Whenever metal beam rail or rail treatments are being constructed adjacent to areas open to traffic, the Contractor shall complete the installation up to and including the designated terminal treatment at the close of each day's work.

On long runs or other locations when it is not practical to complete the installation up to and including the designated terminal treatment by the end of the workday, the Contractor shall use temporary methods to terminate the metal beam rail.

Prior to any rail installations, the Contractor shall submit to the Engineer for review its proposed methods for temporarily terminating the end section.

The Contractor shall furnish posts of sufficient length where field conditions warrant to obtain the depth in the ground shown on the plans.

When existing metal beam rail is being converted, the Contractor may punch or drill a hole in the flange of the existing post to facilitate attachment of the blockout and rail element to the post. No other methods shall be used to create this hole.

End anchorages not needed for the converted rail shall be removed in their entirety.

In the welding of steel plates to the steel posts, the welds shall be of the size and type shown on the plans and shall meet the applicable requirements of the AWS.

Before final erection, all galvanized elements which have been cut or worked so as to damage the zinc coating and cause the base metal to be exposed shall have the exposed base metal thoroughly cleaned and

brush coated with 2 coats of zinc-rich touch-up material.

9.10.04—Method of Measurement:

1. Metal Beam Rail (Type): The length of metal beam rail measured for payment will be the number of linear feet of accepted rail of the type or designation installed, including radius rail other than Curved Guide Rail Treatment, measured along the top of rail between centers of end posts in each continuous section.

2. Metal Beam Rail Span Section (Type II or III): Metal Beam Rail Span Section (Type II or III) measured for payment will be the actual number of each type accepted and installed in accordance with the "Pay Limit" shown on the plans.

3. (Type) Attachment: The number of rail attachments to bridge parapets, barriers or other fixed objects measured for payment will be the actual number of accepted attachments of each type or designation installed in accordance with the "Pay Limit for Attachment" shown on the plans.

4. Convert Metal Beam Rail (Type) to (Type): The conversion of existing metal beam rail (Type) to the (Type) specified will be measured for payment by the number of linear feet of rail installed measured along the top of rail between centers of end posts in each continuous section. If a new end anchorage for the converted rail is needed, it shall be measured for payment in accordance with 9.11.

5. (Type) Curved Guide Rail Treatment: The (Type) Curved Guide Rail treatment measured for payment will be the actual number of each type installed and accepted in accordance with the "Pay Limit Curved Guide Rail Treatment" shown on the plans.

6. R-B Terminal Section: R-B Terminal Section will be measured for payment by the number of each R-B Terminal Section installed and accepted in accordance with the "Pay Limit" shown on the plans.

9.10.05—Basis of Payment:

1. Metal Beam Rail (Type): This work will be paid for at the Contract unit price per linear foot for the type or designation indicated on the plans or ordered by the Engineer, complete in place. Prices shall include all materials, posts of all lengths, equipment, tools, removal and disposal of surplus material, and labor incidental to the installation of the rail.

2. Metal Beam Rail Span Section (Type II or III): This work will be paid for at the Contract unit price each for the types specified on the plans complete in place. Prices shall include all materials, equipment, tools, removal and disposal of surplus material, backfilling, and labor incidental to the installation of the rail.

3. (Type) Attachment: This work will be paid for at the Contract unit price each for the type of attachment complete in place. The price shall include all materials, drilling & grouting including anchor bolts, removal of existing rail system, removal and disposal of surplus material, equipment, tools, and labor incidental to the installation of the attachment.

4. Convert Metal Beam Rail (Type) to (Type): The conversion of existing metal beam rail will be paid for at the Contract unit price per linear foot for the type shown on the plans complete in place. The price shall include all materials (excluding new parts for damaged or missing parts), backfilling, punching or drilling of holes in existing posts, removal and resetting of existing railing, removal of the end anchorages where indicated on the plans, removal and disposal of surplus material, equipment, tools and labor incidental to the conversion of the existing rail. Surplus material not needed for the conversion, unless specified otherwise in the Contract, shall become the property of the Contractor.

Payment for new parts approved by the Engineer, which replace damaged or missing parts will be paid for at the applicable Contract unit prices, or in their absence, in accordance with 1.04.05.

5. (Type) Curved Guide Rail Treatment: This work will be paid for at the Contract unit price for each type indicated or as ordered by the Engineer, complete in place. The price shall include all materials, excavation, backfilling, removal and disposal of surplus material, equipment, tools and labor incidental to the installation of the rail treatment.

6. R-B Terminal Section: This work will be paid for at the Contract unit price for each "R-B Terminal Section" complete in place, including all materials, equipment, tools and labor incidental thereto.

General: Drilling in or removal of rock or boulders and backfilling with suitable material when required for the installation of posts will be paid for in accordance with 1.04.05, unless an item for the removal of rock appears in the Contract.

Payment for temporary terminations for metal beam rail and galvanized coating touch-up will be included in the general cost of the work.

Pay Item	Pay Unit
Metal Beam Rail (Type)	l.f.
Metal Beam Rail Span Section (Type II or III)	ea.
(Type) Attachment	ea.
Convert Metal Beam Rail (Type) to (Type)	l.f.
(Type) Curved Guide Rail Treatment	ea.
R-B Terminal Section	ea.

SECTION 9.11
METAL BEAM RAIL ANCHORAGES

9.11.01—Description: This work shall consist of installing metal beam rail end anchorages of the type shown on the plans or as ordered by the Engineer.

9.11.02—Materials: The materials for this work shall meet the requirements of M.10.02-7.

Non-shrink grout associated with rail anchored to rock shall meet the requirements of M.03.07.

9.11.03—Construction Methods: Anchorages, channels, rails, w-beam terminal elements, and fittings shall be installed as indicated on the plans. The excavated area for the anchorages shall be backfilled with suitable material and compacted in 6 inch layers. Any surplus material remaining after the completed installation shall be removed by the Contractor.

When the rail is anchored to rock, preparation of the rock including rock removal and the drilling of holes shall be as shown on the plans. The diameter of the holes shall be sufficient to permit the placement of the bolts and the non-shrink grout, but shall not exceed twice the diameter of the bolts. The bolt holes shall be blown clean with an air jet prior to installing the bolts and non-shrink grout. Spalled areas behind the steel plate shall be filled with non-shrink grout.

The rail elements shall be lapped in the direction of traffic.

Before final erection, all galvanized elements which have been cut or worked so as to damage the zinc coating and cause the base metal to be exposed shall have the exposed base metal thoroughly cleaned and brush coated with zinc-rich touch-up material in accordance with M.10.02-8.

9.11.04—Method of Measurement: The number of end anchorages measured for payment shall be the actual number of end anchorages of each type installed and accepted in accordance with the "Pay Limit Anchorage" shown on the plans.

9.11.05—Basis of Payment: End anchorages will be paid for at the Contract unit price each for the type of end anchorage, complete in place, which price shall include materials, excavation, backfilling, drilling and grouting, removal and disposal of surplus material, equipment, tools, and labor incidental to complete the installation.

Payment for rock removal required for “Anchor in a Rock Cut Slope,” shall be included in the cost of the item. Removal of rock or boulders encountered during excavation for other end anchorages and backfilling with suitable material will be paid for in accordance with 1.04.05, unless an item for the removal of rock appears in the Contract.

Pay Item	Pay Unit
(Type) End Anchorage Type I	ea.
(Type) End Anchorage Type II	ea.
Anchor in (Type) Slope	ea.

NOTES

SECTION 9.12
REMOVE AND RESET POSTS, RAIL AND RAIL ANCHORAGES

9.12.01—Description: This work shall consist of removing or resetting existing single posts, anchorages, cable guide rail and a single or double line of metal beam rail in the locations indicated on the plans or as ordered by the Engineer.

9.12.02—Materials: When resetting rail, the Contractor shall reuse any undamaged existing rail elements, appropriate posts, delineators, and lap bolts within the Project limits, as approved by the Engineer to construct the reset rail. The Contractor shall use new materials meeting the requirements of M.10 to replace any parts of the existing rail system that are damaged or missing and cannot be obtained from other rail systems being removed or reset within the Project limits.

9.12.03—Construction Methods: The Construction Methods for 9.10, 9.11, and 9.18 when applicable, shall apply to the resetting of existing rail systems.

Prior to commencement of work, the Contractor and Engineer shall inventory the existing rail systems within the Project limits to determine which materials are suitable for reuse.

If resetting or removing railing, the Contractor must complete that work, including any required grading and any replacement of materials, by the end of each day's work. When it is not practical to complete such rail work by the end of the day's work, the Engineer may allow the Contractor to temporarily attach the existing rail to the new rail, concrete barrier curb or temporary terminal treatment.

9.12.04—Method of Measurement:

1. **Reset Cable Guide Rail and Reset Metal Beam Rail:** The length of reset rail measured for payment shall be the number of linear feet of completed rail of each type specified on the plans, measured along the top of rail between centers of posts in each continuous section of reset rail.
2. **Reset Single Posts and Reset Rail Anchorages:** The number of reset single posts or rail anchorages measured for payment shall be the actual number of each single post or rail anchorage reset as shown on the plans.
3. **New Materials:** The amounts of new material measured for payment will be the numbers of new rail components authorized by the Engineer.
4. **Removal:** Removal of cable guide rail or metal beam rail shall be measured for payment by the number of linear feet of such rail including posts removed between the centers of anchors, including the anchors, and any rail system attachments.
5. **Remove Single Post:** Removal of single posts not attached to any rail system will be measured for payment by counting each such post.

9.12.05—Basis of Payment:

1. **Reset Cable Guide Rail and Reset Metal Beam Rail:** This work will be paid for at the Contract unit price per linear foot for "Reset (Type) Cable Guide Rail (Type-Post)" and "Reset Metal Beam Rail (Type) (System)," complete in place. This price shall include the complete removal, storage and resetting of existing rail, including excavation, backfilling and disposal of surplus or unsuitable material, and all equipment, tools and labor incidental thereto.
2. **Reset Single Posts and Reset Rail Anchorages:** This work will be paid for at the Contract unit price each for "Reset Single Post," and "Reset Rail Anchorage" complete in place. This price shall include the complete removal, storage and resetting of existing posts and anchorages, including excavation, backfilling and disposal of surplus or unsuitable material, and all equipment, tools, and labor incidental thereto.
3. **New Materials:** Authorized new materials will be paid for at the applicable Contract unit prices, or in their absence, in accordance with 1.04.05.
4. **Remove Cable Guiderail and Remove Metal Beam Rail:** This work will be paid for at the Contract unit price per linear foot for "Remove Cable Guide Rail" and "Remove Metal Beam Rail." This price shall include removal of each post, rail and end anchorage, backfilling, equipment, tools and labor incidental thereto. Surplus rail, cable, posts and hardware not needed for the resetting of railing, unless specified otherwise in the Contract, shall become the property of the Contractor.
5. **Remove Single Post:** This work will be paid for at the Contract unit price each for "Remove Single Post." This price shall include removal and disposal of each post, backfilling, equipment, tools and labor incidental thereto.
6. **Temporary Treatment:** There will be no payment for any temporary treatments or attachments.

Pay Item	Pay Unit
Reset (Type) Cable Guide Rail (Type Post)	l.f.
Reset Metal Beam Rail (Type) (System)	l.f.
Reset Single Post	ea.
Reset Rail Anchorage	ea.
Remove Single Post	ea.
Remove Cable Guide Rail	l.f.
Remove Metal Beam Rail	l.f.

SECTION 9.13
CHAIN LINK FENCE

9.13.01—Description
9.13.02—Materials
9.13.03—Construction Methods
9.13.04—Method of Measurement
9.13.05—Basis of Payment

9.13.01—Description: Work under this item shall consist of furnishing and installing or removing chain link fence and gates of the type and height specified on the plans.

9.13.02—Materials: The fence materials for this work shall meet the requirements of M.10.05.

Concrete footings shall be Portland cement concrete, minimum compressive strength of 3,300 psi, as defined in M.03.02. Where posts are to be grouted into rock, the grout shall meet the requirements of M.03.05.

9.13.03—Construction Methods: Chain link fence posts shall be spaced in line of fence not further than 10 feet on centers. Intermediate or line posts, may be driven by mechanical means. A suitable driving cap shall be used to ensure that no damage is caused to the post, galvanization or polyvinyl chloride coating. Posts not driven, and all other type posts shall be set in Portland cement concrete acceptable to the Engineer.

Concrete footings shall extend a minimum of 4 inches below the bottom of the post. The footing diameter shall be 12 inches for terminal, corner, pull or brace posts. All tops of concrete footings shall be crowned to shed water.

When ledge rock is encountered, the posts shall be set in 6 inch minimum diameter holes drilled into rock to a minimum 12 inches deep and shall be grouted in correct position.

All fence end posts shall be braced. Braces shall be installed at 100 foot intervals to maintain tension. Corner posts shall be braced at each change in direction.

Brace posts with 2 braces shall be provided for all heights where changes in horizontal or vertical alignment of 10 degrees or more occur.

Where additional braces are required, they shall be spaced as indicated on the plans.

Where a top rail is used, it shall pass through the line post loop cap and form a continuous brace from end to end of fence. The rail shall be provided with sleeve connectors approximately every 20 feet. The sleeve connectors shall be at least 7 inches long.

Fabric shall be fastened to line posts with tie wires spaced approximately 12 inches apart. The fabric shall be fastened to the top rail with tie wires spaced approximately 18 inches apart.

If a top rail is not specified, a top tension wire shall be provided. The tension wire shall be a continuous length between pull posts. Sufficient tension shall be applied to provide a wire without a visible sag between posts. Tension wires shall be tied or otherwise fastened to end, gate, corner or pull posts by a method acceptable to the Engineer. Hog rings shall be provided for attaching the tension wire to the fabric at intervals not exceeding 18 inches.

Where it is not practicable to conform the fence to the general contour of the ground, as at ditches, channels, etc., the opening beneath the fence shall be enclosed with chain link fabric and sufficiently braced to preclude access, but not to restrict the flow of water.

Fabric shall be fastened to the end of the gate frames by tension bars and tension bands, and to the top and bottom of the gate frames by tie wires in the same manner as specified for the chain link fence fabric.

The drop bar locking device for the gate shall be provided with a footing of Portland cement concrete crowned at the top to shed water and provided with a hole to receive the locking bar. A heavy-duty padlock with 2 keys shall be furnished by the Contractor for each gate. The size of the footing and depth of penetration of the locking bar into the footing shall be as shown on the plans.

Where indicated, the removal of existing chain link fence and gate shall be to the nearest post as shown on the plans. If any existing fence is to remain, the new terminal post shall be modified to ensure proper bracing as directed by the Engineer. Chain link mesh shall be disconnected and secured to the terminal post using appropriate hardware.

If the fence post being removed is located in pavement or concrete, the Contractor shall fill all holes with non-shrink grout. Concrete footings shall be removed and backfilled unless directed otherwise by the Engineer.

9.13.04—Method of Measurement: Chain link fence will be measured for payment by the number of linear feet of completed and accepted chain link fence or polyvinyl chloride chain link fence of the height specified, measured from outside to outside of terminal posts.

Gates will be measured for payment by the number of gates installed, of the type and size specified, completed and accepted.

Removal of chain link fence will be measured for payment by the number of linear feet of chain link fence and gate removed, including posts and bracing, measured from outside to outside of terminal posts.

9.13.05—Basis of Payment: Chain link fence work will be paid for at the Contract unit price per linear foot for "Chain Link Fence" or "Polyvinyl Chloride Chain Link Fence" of the height specified, complete in place, which price shall include all materials, equipment, tools, excavation, backfill, concrete, grout, disposal of surplus material and labor incidental thereto.

Gate work will be paid for at the Contract unit price each for "Chain Link Gate" or "Polyvinyl Chloride Chain Link Gate" of the type and size specified, complete in place, which price shall include gate frame, gate posts, chain link fabric, lock, concrete, excavation, backfill, fabrication, installation, disposal of surplus material, and all materials, equipment, tools, labor and any work incidental thereto.

Removal of chain link fence work will be paid for at the Contract unit price per linear foot for “Remove Chain Link Fence.” The price shall include removal and disposal of posts, footings, fence fabric, gates, excavation, backfill, equipment, tools, labor, and any work incidental thereto.

Pay Item	Pay Unit
(Height) Chain Link Fence	l.f.
(Height) Polyvinyl Chloride Chain Link Fence	l.f.
(Type) (Size) Chain Link Gate	ea.
(Type) (Size) Polyvinyl Chloride Chain Link Gate	ea.
Remove Chain Link Fence	l.f.

SECTION 9.14
METAL HANDRAIL

9.14.01—Description: This item shall consist of furnishing and installing a 1 1/2-inch Welded and Seamless Steel pipe hand-railing on the proposed concrete steps, as shown on the plans, or as directed by the Engineer.

9.14.02—Materials: All pipe for railing and posts shall be standard weight 1 1/2-inch Welded and Seamless Steel pipe meeting the requirements of ASTM A53, Type E or S, Grade A, Schedule 40 Black Finish.

Caps shall be malleable iron meeting the requirements of the specifications for malleable iron castings, ASTM A47, Grade No. 32510. Ductile iron castings shall be as specified in the specifications for Ductile Iron Castings, ASTM A536, Grade 60-40-18 unless otherwise specified. In addition to the specified test coupons, test specimens from parts integral with the castings, such as risers, shall be tested for castings having a weight of more than 1000 lb. to determine that the required quality is obtained in the castings in the finished condition. Paint shall meet the requirements of M.07 and the Contract.

9.14.03—Construction Methods: The rail elements shall be erected to produce a smooth, continuous rail as shown on the plans.

Pipe posts shall be set in sleeves and fastened by means of setscrews. The sleeves for the post shall be firmly embedded in concrete to the depth shown on the plans. The Contractor shall seal annular space between the pipe and the sleeve with a bituminous compound satisfactory to the Engineer to prevent water from entering.

The top rail shall consist of a single, continuous pipe and shall be welded to posts.

The lower rail shall be welded to the posts.

All joints shall be finished smooth. No rough or sharp projections will be permitted.

The entire railing and exposed portion of the sleeve into which the railing is set shall be thoroughly cleaned before painting and shall be painted with a field coat of zinc rich primer, ready-mixed paint and 2 field coats of paint to be selected by the Engineer and meeting the pertinent requirements of 6.03.

9.14.04—Method of Measurement: This work will be measured for payment by the number of linear feet of railing measured along the top of the rail from end to end of the rail.

9.14.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Metal Handrail," complete in place, which price shall include all materials, including sleeves and fastening devices in which the posts are set, and all equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Metal Handrail	l.f.

SECTION 9.15
TREE ROOT PROTECTION

9.15.01—Description: This item shall consist of the protection of selected trees, shrubs, or other woody plants by placing pervious material to protect the root system, as shown on the plans, and in accordance with these specifications.

9.15.02—Materials: The materials for root protection shall consist of approved materials ranging from 1 to 6 inches in size, such as stone or screened gravel.

9.15.03—Construction Methods: Where tree root protection is required, the area to be protected shall be thoroughly cleared of all vegetation. The pervious material shall then be uniformly spread to the line and grade specified.

Care shall be taken to assure that the trees, shrubs, or other woody plants which are to be preserved in place are not scarred or damaged by the operations under this item. The root area to be protected shall be the area of ground surface lying within the drip line of the outermost branches of the tree concerned.

9.15.04—Method of Measurement: This work will be measured for payment by the number of cubic yards of tree root protection complete in place.

9.15.05—Basis of Payment: This work will be paid for at the Contract unit price per cubic yard for "Tree Root Protection," which price shall be full compensation for clearing the ground surface, for placing the pervious material and all materials, equipment, tools, and labor incidental thereto.

Pay Item	Pay Unit
Tree Root Protection	c.y.

SECTION 9.18
THREE-CABLE GUIDE RAILING
(I-BEAM POSTS) AND ANCHORAGES

9.18.01—Description: This railing shall consist of 3 wire cables attached to American Standard Beam Posts, as indicated on the plan, installed in the locations indicated and in accordance with the dimensions and details shown on the plans or as ordered.

Anchorages shall consist of the necessary concrete anchors or "deadmen," end posts with base plates, anchor assemblies, turnbuckle cable-end assemblies and spring cable-end assemblies (compensating device), singly or in combination, appurtenance hardware and the necessary excavation and backfill, all installed in the locations indicated or ordered in accordance with the dimensions and details shown on the plans or as ordered and in accordance with these specifications.

9.18.02—Materials: The materials for this work shall meet the requirements of M.10.08 for wire rope, steel posts and plate anchors, fittings, and anchorages. The materials for retroreflective delineators shall be as specified in M.18.09.

9.18.03—Construction Methods: The Contractor is cautioned that within the limits of any project, buried cable for illumination or utilities, which may be energized, may be present.

In the welding of steel plates to the steel posts, the welds shall be of the size and shape as shown on the plans and shall meet the applicable requirements of the current AWS Structural Welding Code.

Line posts are to be driven into the ground as shown on the plans, unless otherwise ordered by the Engineer. Posts shall be driven by approved methods and equipment that will provide the posts in their final position free of any distortion and burring or any other damage.

The Contractor shall be required to furnish extra length posts at transition areas, and these posts shall be driven so as to provide the 2 inch nominal cover over the plate as required by the plans.

End posts are to be set in holes dug in thoroughly compact soil. The bottoms of dug holes shall be thoroughly rammed so that the posts have a stable foundation. Backfill for all dug holes shall be approved material and is to be machine-tamped in 4 inch layers in such a way as not to shift the posts from the correct alignment.

Should rock or boulders be encountered, a hole of sufficient size to set the post to the normal depth called for on the plans shall be drilled and back-filled with tamped sand.

Anchorages, plate anchors, cables, turnbuckle cable-end assemblies and spring cable-end assemblies (compensating device), singly or in combination, and all fittings shall be placed as indicated on the plans and in a workmanlike manner. The cable shall be drawn taut and fastened securely at both ends as shown on the plans. No punching, drilling, cutting or welding of posts, post plates or any hardware will be permitted after galvanizing.

After erection, galvanized cable, fittings, steel posts and plate anchors that have been abraded so that the base metal is exposed, threaded portions of all fittings and fasteners and the cut ends of all bolts shall be painted with 2 coats of zinc dust primer conforming to FS TTP-641B, Type II, or MILSPEC P 21035.

After the rail has been erected, the Contractor shall install a reflectorized delineator on every other line post as shown on the plans and wherever the railing is parallel to, but not greater than 12 feet from, the edge of the shoulder. Reflectorized delineators shall not be installed on railing installations or single posts where such installations are greater than 12 feet from the shoulder. An object marker shall be installed at the end of the anchorage, Type II.

9.18.04—Method of Measurement: This work will be measured for payment as follows:

1. **Three-Cable Guide Railing (I-Beam Posts):** The length of cable railing to be included for payment shall be the number of linear feet from centerline to centerline of the most remote end post from the concrete anchor in End Anchorage, Type I, and from centerline to centerline of the end post to which the anchor rod is attached in End Anchorage, Type II. No deduction will be made for the overlap at intermediate anchorages.
2. **Anchorages:** The number of anchorages to be included for payment shall be the actual number of anchorages installed and accepted.

9.18.05—Basis of Payment: Payment for this work will be made as follows:

1. **Three-Cable Guide Railing:** This railing will be paid for at the Contract unit price per linear foot for "Three-Cable Guide Railing (I-Beam Posts)," complete in place, which price shall include all materials, including reflectorized delineators and fittings, equipment, tools and labor necessary for complete

installation, also all excavation, refilling and disposal of surplus material.

2. Anchorages: This work will be paid for at the Contract unit price each for "End Anchorage" of the type specified, complete in place, which price shall include concrete anchors, end posts with base plates, all cable and fastenings from end post to concrete anchor, anchor assemblies (compensating device), singly or in combination, all materials, fittings, equipment, tools and labor necessary for complete installation, also all excavation, refilling and disposal of surplus materials.

There will be no separate payment for the object marker at the end of the End Anchorage, Type II, but the cost shall be included in the Contract bid price for "End Anchorage-Type II."

Drilling in rock or boulders and backfilling with tamped sand, where required for the installation of posts, will be paid for as "Extra Work" according to the provisions of 1.04.05 and 1.09.04, as amended elsewhere herein.

Pay Item	Pay Unit
Three-Cable Guide Railing (I-Beam Posts)	l.f.
End Anchorage (Type)	ea.

SECTION 9.21
CONCRETE SIDEWALKS AND RAMPS

9.21.01—Description
9.21.02—Materials
9.21.03—Construction Methods
9.21.04—Method of Measurement
9.21.05—Basis of Payment

9.21.01—Description: This item shall consist of concrete sidewalks and ramps constructed on a granular fill or reclaimed miscellaneous aggregate base course in the locations and to the dimensions and details shown on the plans or as ordered.

9.21.02—Materials: Materials for this work shall meet the requirements of M.03. Concrete shall have a minimum compressive strength of 4,400 psi. Liquid membrane-forming curing compound shall be as specified in M.03.04-3.

Granular fill or reclaimed miscellaneous aggregate for base shall be as specified in M.02.01.

Detectable warning strips shall be prefabricated detectable warning tile chosen from the Department's Qualified Products List for retrofit or cast in place applications.

9.21.03—Construction Methods:

1. Excavation: Excavation, including removal of any existing sidewalk (bituminous or concrete) and curbing, shall be made to the required depths below the finished grade, as shown on the plans or as directed. All soft and yielding material shall be removed and replaced with suitable material.

When connecting new concrete sidewalk to a section of existing concrete sidewalk, the connection point shall be at the nearest joint in the existing sidewalk.

The Contractor shall establish the limits required to achieve grades for each ramp prior to removal of existing sidewalk and ramps. The Contractor shall document and notify the Engineer of any control points that may conflict with the design grades or configuration of ramps shown on the plans. Control points can be but are not limited to ROW, utility poles, drainage structures, buildings, fences, walls or other features found near the proposed ramp. When control points are encountered within the limits of the ramp, the Engineer will determine if an alternative ramp type is required or the ramp is to be constructed as shown on the plans.

2. Granular Fill or Reclaimed Miscellaneous Aggregate Base: The granular fill or reclaimed miscellaneous aggregate base shall be placed in layers not to exceed 6 inches deep and to such a depth that after compaction it shall be at the specified depth below the finished grade of the walk. The base shall be wetted and rolled or tamped after the spreading of each layer.

3. Forms: Forms shall be of metal or wood, straight, free from warp and of sufficient strength to resist springing from the pressure of the concrete. If made of wood, they shall be of 2 inch surfaced plank except that at sharp curves thinner material may be used. If made of metal, they shall be of approved section and shall have a flat surface on the top. Forms shall be of a depth equal to the depth of the sidewalk. Forms shall be securely staked, braced and held firmly to the required line and grade and shall be sufficiently tight to prevent leakage of mortar. All forms shall be cleaned and oiled or wetted before concrete is placed against them. Sheet metal templates 1/8 inch thick, of the full depth and width of the walk, shall be spaced at intervals of 12 feet or as directed. If the concrete is placed in alternate sections, these templates shall remain in place until concrete has been placed on both sides of the template. As soon as the concrete has obtained its initial set, the templates shall be removed.

4. Concrete: The concrete shall be proportioned, mixed, placed, etc., in accordance with the provisions of 6.01.03, except as modified herein.

5. Finishing: The surface of the concrete shall be finished with a wood float or by other approved means. The outside edges of the slab and all joints shall be edged with a 1/4 inch radius edging tool. Each slab shall be divided into 2 or more sections by forming dummy joints with a jointing tool as directed.

6. Curing: Liquid membrane-forming compound shall be applied following finishing as recommended by the manufacturer.

The Contractor shall have on the Project sufficient approved cover sheet of cotton mats for the protection of the sidewalk in case of rain or breakdown of the spray equipment. The cotton mats shall be maintained in a wet condition during the period of use, as specified in 6.01.03-II-9.

7. Backfilling and Removal of Surplus Material: The sides of the sidewalk shall be backfilled with suitable material thoroughly compacted and finished flush with the top of the sidewalk. All surplus

material shall be removed and the Site left in a neat and presentable condition to the satisfaction of the Engineer.

8. Detectable Warning Strip: The detectable warning strip for new construction shall be set directly in plastic concrete and each tile shall be weighed down to prevent the tile from floating after placement in wet concrete in accordance with curing procedures. Install detectable warning strip according to the plans and the manufacturer's specifications, or as directed by the Engineer.

The detectable warning strip for retrofit construction shall be installed according to the plans in the direction of pedestrian route and contained wholly within painted crosswalk when present. Its installation shall meet all manufacturer's requirements.

9.21.04—Method of Measurement: This work will be measured for payment as follows:

1. Concrete Sidewalk or Ramp: This work will be measured by the actual number of square feet of completed and accepted concrete sidewalk or ramp. Liquid membrane-forming curing compound will not be measured for payment.

2. Excavation: Excavation below the finished grade of the sidewalk or ramp, backfilling, and disposal of surplus material will not be measured for payment, but the cost shall be included in the price bid for the sidewalk or ramp. Excavation above the finished grade of the sidewalk or ramp will be measured and paid for in accordance with 2.02.

3. Granular Fill or Reclaimed Miscellaneous Aggregate Base: This work will not be measured for payment, but the cost shall be considered as included in the price bid for the sidewalk or ramp.

4. Detectable Warning Strip: For new construction (cast in place), the detectable warning strip will be measured for payment by the actual number of each ramp where detectable warning strip has been installed and accepted regardless of the number of tiles installed.

5. Retrofit Detectable Warning Strip: For retrofit construction (surface applied), the detectable warning strip will be measured for payment by the actual number of each ramp where a detectable warning strip has been installed and accepted regardless of the number of tiles installed.

6. Construction Surveying: The establishment of control points and limits of grading will be measured in accordance with the item "Construction Surveying."

9.21.05—Basis of Payment: Construction of a concrete sidewalk or ramp will be paid for at the Contract unit price per square foot for "Concrete Sidewalk" or "Concrete Sidewalk Ramp" complete and accepted in place, which price shall include all excavation as specified above, backfill, disposal of surplus material, curb removal and any monolithic or separately cast sidewalk curb when required for the sidewalk ramp as shown on the plans, granular fill or reclaimed miscellaneous aggregate base, curing compound, equipment, tools, materials and labor incidental thereto.

A new detectable warning strip will be paid for at the Contract unit price for "Detectable Warning Strip" at each ramp where detectable warning strip has been installed complete in place. This price shall include all tiles, materials, equipment, tools and labor incidental thereto.

Retrofitting the existing concrete sidewalk with a detectable warning strip will be paid for at the Contract unit price for "Retrofit Detectable Warning Strip" at each ramp where the retrofit detectable warning strip has been installed complete in place. This price will include all tiles, saw cutting concrete, adhesive, drilling holes for fasteners, materials, equipment, tools and labor incidental thereto.

The establishment of control points and limits of grading will be paid for in accordance with the item "Construction Staking."

Pay Item	Pay Unit
Concrete Sidewalk	s.f.
Concrete Sidewalk Ramp	s.f.
Detectable Warning Strip	ea.
Retrofit Detectable Warning Strip	ea.

SECTION 9.22
BITUMINOUS CONCRETE SIDEWALK
BITUMINOUS CONCRETE DRIVEWAY

9.22.01—Description: This item shall consist of bituminous concrete surfaced sidewalk or driveway constructed on a gravel or reclaimed miscellaneous aggregate base course in the locations and to the dimensions and details shown on the plans or as directed by the Engineer.

9.22.02—Materials: Materials for this work shall meet the following requirements:

1. **Gravel or Reclaimed Miscellaneous Aggregate:** Gravel or reclaimed miscellaneous aggregate for the base course shall meet the requirements of M.02.01 for granular fill.
2. **Bituminous Concrete Surface:** Materials for this surface shall meet the requirements of M.04, HMA S0.375.

9.22.03—Construction Methods:

1. **Excavation:** Excavation, including saw cutting, removal of any existing sidewalk, or driveway, shall be made to the required depth below the finished grade, as shown on the plans or as directed by the Engineer. All soft and yielding material shall be removed and replaced with suitable material.
2. **Forms:** When the bituminous concrete is spread by hand, forms shall be used. Forms shall be of metal or wood, straight, free from warp and of sufficient strength to resist springing from the impact of the roller. If made of wood, they shall be of 2 inch surfaced plank except that at sharp curves thinner material may be used; if made of metal, they shall be of an approved section. All forms shall be of a depth equal to the depth of the sidewalks or driveways and shall be securely staked, braced, and held firmly to the required line and grade. All forms shall be cleaned and oiled each time they are used.
3. **Base Course:** Gravel or reclaimed miscellaneous aggregate for the base course shall be uniformly spread to the required depth and thoroughly compacted with a roller with a weight of at least 500 pounds.
4. **Bituminous Concrete Surface:** This surface shall be constructed in accordance with the requirements of 4.06, except that the material may be spread by hand and thoroughly compacted by multiple passes of a roller weighing at least 500 pounds.
5. **Backfilling and Removal of Surplus Material:** The sides of the sidewalk or driveway shall be backfilled with suitable material thoroughly compacted and finished flush with the top of the sidewalk or driveway. All surplus material shall be removed and the Site left in a neat and presentable condition to the satisfaction of the Engineer. In sections inaccessible to the roller, the base course, surface course and backfill shall be hand-tamped with tampers weighing not less than 12 pounds, the face of which shall not exceed 50 square inches.

9.22.04—Method of Measurement: This work will be measured for payment as follows:

1. **Bituminous Concrete Driveway—Bituminous Concrete Sidewalk:** This work will be measured by the actual number of square yards of completed and accepted sidewalk or driveway.
2. **Excavation:** Excavation below the finished grade of the sidewalk or driveway, backfilling, and disposal of surplus material will not be measured for payment; but the cost shall be included in the price bid for the sidewalk or driveway. Excavation above the finished grade of the sidewalk or driveway will be classified and paid for in accordance with 2.02.
3. **Gravel or Reclaimed Miscellaneous Aggregate Base:** This work will not be measured for payment but the cost thereof shall be included in the price bid for the sidewalk or driveway.

9.22.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Bituminous Concrete Sidewalk," "Bituminous Concrete Driveway" or "Bituminous Concrete Driveway (Commercial)" as the case may be, complete in place, which price shall include all saw cutting, excavation as specified above, backfill, disposal of surplus material, gravel or reclaimed miscellaneous aggregate base, and all equipment, tools, labor and materials incidental thereto.

Pay Item	Pay Unit
Bituminous Concrete Sidewalk	s.y.
Bituminous Concrete Driveway	s.y.
Bituminous Concrete Driveway (Commercial)	s.y.

SECTION 9.23
BITUMINOUS CONCRETE FOR PATCHING

9.23.01—Description: The work under this item shall consist of the temporary patching of any pavement surfaces or pedestrian passways which have become rutted, broken, damaged or otherwise unserviceable, and at such other locations as the Engineer may designate.

9.23.02—Materials: The materials furnished and used in this work shall meet the requirements of 4.06 and M.04. The specific material to be used for patching shall be as directed by the Engineer.

9.23.03—Construction Methods: Patching shall be done only at the locations and at such time as is deemed necessary by the Engineer. Prior to placing the patching material, the areas to be patched shall be cleaned of dirt and other debris and shall be reasonably dry. Compaction of the patching material shall be attained by methods approved by the Engineer.

9.23.04—Method of Measurement: The quantity of material to be included for payment will be determined by the net weight, in tons, measured in the hauling vehicles furnished by and at the expense of the Contractor. The scales shall be a type satisfactory to the Engineer and shall be sealed. An inspector, to be appointed and compensated by the Department, shall check the weight measurements of all material entering into construction. The total weight will be the summation of the weigh slips of bituminous concrete actually incorporated in the work included under this item.

9.23.05—Basis of Payment: This work will be paid for at the Contract unit price per ton for "Bituminous Concrete for Patching," complete in place, which price shall include furnishing all materials, equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
Bituminous Concrete for Patching	ton

SECTION 9.24
CONCRETE DRIVEWAY RAMP

9.24.01—Description: This item shall consist of concrete driveway ramps constructed on a granular fill base in accordance with the Contract.

9.24.02—Materials: Materials for this work shall meet the following requirements:

1. **Portland Cement:** Concrete shall meet the requirements of M.03 for Class PCC04460 Concrete.
2. **Granular Fill Base:** Granular fill shall meet the requirements of M.02.01.
3. **Reinforcement:** Shall meet the requirements of M.06.01.

9.24.03—Construction Methods: Construction methods shall meet the requirements of 9.21.03. The surface shall be finished and marked off as directed by the Engineer.

9.24.04—Method of Measurement: This work will be measured for payment as follows:

1. **Concrete Driveway Ramp:** This work will be measured for payment by the actual number of cubic yards of completed and accepted concrete driveway ramps.
2. **Excavation:** Excavation below the finished grade of each ramp, backfilling and disposal of surplus material will not be measured for payment; but the cost shall be included in the Contract price for Concrete Driveway Ramp.

   Excavation above the finished grade of each ramp will be classified and paid for in accordance with 2.02.
3. **Granular Fill Base:** This work will not be measured for payment, but the cost shall be included in the Contract price for Concrete Driveway Ramp.
4. **Reinforcement:** This material will not be measured for payment, but the cost shall be included in the Contract price for Concrete Driveway Ramp.

9.24.05—Basis of Payment: This work will be paid for at the Contract unit price per cubic yard for "Concrete Driveway Ramp," complete in place, which price shall include all excavation as specified above, backfill, disposal of surplus materials, and all materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Concrete Driveway Ramp	c.y.

SECTION 9.30
OBJECT MARKER

9.30.01—Description: This item shall consist of furnishing and installing an object marker at locations shown on the plans or as ordered and in accordance with these specifications.

9.30.02—Materials: Aluminum sign blanks shall meet the requirements of M.18.13.

Retroreflective sheeting shall meet the requirements of M.18.09. Silk screening shall meet the requirements specified by the retroreflective sheeting manufacturer.

Steel posts shall meet the requirements of M.18.14.

Sign mounting bolts and nuts shall meet the requirements of M.18.15.

9.30.03—Construction Methods: Object marker posts shall be erected to the elevation and at locations shown on the plans. They shall be set vertically, facing the direction noted on the plans or as directed by the Engineer.

Posts shall be driven by approved methods and equipment that will provide the posts in their final position, free from any distortion and burring or any other damage. The panel shall be attached after the posts are driven.

The black legend shall be applied by either the silk screen process or by the application of pressure sensitive opaque film.

9.30.04—Method of Measurement: The number of object markers to be included for payment shall be the number of each type of object markers installed and accepted.

9.30.05—Basis of Payment: This work will be paid for at the Contract unit price for each type of "Object Marker," complete in place, which price shall include all panels, posts, hardware, materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Object Marker (Type)	ea.

NOTES

SECTION 9.39
SWEEPING FOR DUST CONTROL

9.39.01—Description: This item shall consist of furnishing a pickup sweeper and accessory equipment and utilizing it for the removal of earth and other dust producing materials from paved surfaces for the purpose of allaying dust conditions.

9.39.03—Construction Methods: The Contractor shall have available and maintain in an operable condition equipment capable of efficiently sweeping up earth and other materials from paved surfaces. This equipment shall include suitable provisions for the application of water ahead of the sweeping brooms to prevent dusting, for the pickup, internal storage and removal of sweepings, and for the cleaning of areas of heavy accumulation beyond the capacity of the sweeper.

The sweeping operations shall be under the control of the Engineer at all times. Sweeping shall take place at locations and times directed by the Engineer. The disposal of all sweepings shall meet with the approval of the Engineer.

9.39.04—Method of Measurement: Sweeping will be measured for payment by the number of hours of actual sweeping activity by the pickup sweeper or other mechanized equipment necessary for the removal of earth or dust producing materials from paved surfaces as directed by the Engineer or in the Contract.

9.39.05—Basis of Payment: Sweeping will be paid for at the Contract unit price per hour for "Sweeping For Dust Control," which price shall include the furnishing of all equipment, water, tools, labor and work incidental thereto. This price shall also include the maintenance of the pickup sweeper for the life of the Contract.

Pay Item	Pay Unit
Sweeping for Dust Control	hr.

SECTION 9.42
CALCIUM CHLORIDE FOR DUST CONTROL

9.42.01—Description: This item shall consist of furnishing calcium chloride and spreading it on the subgrade or in other areas of a Project under construction, for the purpose of allaying dust conditions.

9.42.02—Materials: Calcium chloride shall meet the requirements of AASHTO M 144, except that the pellet form and the flake form shall be equally acceptable.

9.42.03—Construction Methods: Calcium chloride shall be applied only at the locations, at such times and in the amount as may be directed by the Engineer. It shall be spread in such a manner and by such devices that uniform distribution is attained over the entire area on which it is ordered placed.

9.42.04—Method of Measurement: Weights as marked on the shipping containers shall be used; or if directed by the Engineer, scales shall be furnished by and at the expense of the Contractor, and the calcium chloride weighed in a manner satisfactory to the Engineer.

9.42.05—Basis of Payment: Payment for this work will be made at the Contract unit price per ton for "Calcium Chloride for Dust Control," which price shall include all material, equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
Calcium Chloride for Dust Control	ton

SECTION 9.43
WATER FOR DUST CONTROL

9.43.01—Description: This item shall consist of furnishing water equipment, water, and applying it for the purpose of allaying dust conditions.

9.43.03—Construction Methods: The application of water shall be under the control of the Engineer at all times. It shall be applied only at locations at such times, and in the amount as may be directed by the Engineer. Quantities of water wasted or applied without authorization will not be paid for.

The Contractor shall have available and maintain in an operable condition at all times, sufficient equipment for the purpose of applying water for dust control. This equipment shall consist of pipelines, tanks, tank-trucks, pumps, meters, hose, distributors or other devices approved by the Engineer. A suitable device for a positive shutoff and for regulating the flow of water shall be located so as to permit positive operator control.

9.43.04—Method of Measurement: This work will be measured for payment by the number of m. gallons. The water will be measured in tanks or tank-trucks of predetermined capacity, or by means of satisfactorily installed meters. Any and all measuring devices shall be furnished by the Contractor.

9.43.05—Basis of Payment: This work will be paid for at the Contract unit price per m. gallon for "Water for Dust Control," which price shall include all water, labor, and equipment including devices to measure and apply to surfaces designated by the Engineer and at the times specified.

This price shall also include all work necessary to erect, relocate, re-erect, and dismantle the entire water equipment system.

Pay Item	Pay Unit
Water for Dust Control	m. gal.

SECTION 9.44
TOPSOIL

9.44.01—Description: This work shall consist of furnishing, placing and shaping topsoil in areas shown on the plans or where directed by the Engineer. The topsoil shall be placed to a depth of 4 inches unless stated otherwise in the Contract.

9.44.02—Material: The material shall meet the requirements of M.13.

9.44.03—Construction Methods: Any material delivered to the Project, which does not meet the proper pH requirements for that soil must be amended on Site prior to final acceptance.

The areas on which topsoil is to be placed shall be graded to a reasonably true surface. Topsoil shall then be spread and shaped to the lines and grades shown on the plans, or as directed by the Engineer. The required depth to which the topsoil is to be placed is to be the depth after settlement of the material has taken place. All stones, roots, debris, sod, weeds and other undesirable material shall be removed. After shaping and grading, all trucks and other equipment shall be excluded from the finished areas to prevent excessive compaction. The Contractor shall perform such work as required to provide a friable surface for seed germination and plant growth prior to seeding.

During hauling and spreading operations, the Contractor shall immediately remove any material dumped or spilled on the shoulders or pavement.

It shall be the Contractor's responsibility to restore to the line, grade and surface all eroded areas with approved material and to keep the finished areas in acceptable condition until the completion of the construction work.

9.44.04—Method of Measurement: This work will be measured for payment by the number of square yards of area on which the placing of topsoil has been completed and the work accepted.

9.44.05—Basis of Payment: Payment for this work will made at the Contract unit price per square yard for "Furnishing and Placing Topsoil" which price shall include all materials, application of lime if necessary, equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
Furnishing and Placing Topsoil	s.y.

SECTION 9.46
LIMING

9.46.01—Description: The work included under this item consists of furnishing and placing agricultural ground dolomitic limestone on those areas designated by the Engineer.

9.46.02—Material: Agricultural ground dolomitic limestone shall meet the requirements of M.13.02.

9.46.03—Construction Methods: On the basis of the subsequent tests, the Contractor shall apply limestone to such areas and in such amounts as the Engineer may direct. The limestone may be applied by any approved method.

9.46.04—Method of Measurement: This work will be measured for payment by the actual number of tons of limestone applied and accepted within the limits designated by the Engineer.

9.46.05—Basis of Payment: This work will be paid for at the Contract unit price per ton for "Liming," which price shall include all materials, equipment, tools labor and work incidental thereto.

Pay Item	Pay Unit
Liming	ton

SECTION 9.49
FURNISHING, PLANTING and MULCHING
TREES, SHRUBS, VINES and GROUND COVER PLANTS

9.49.01—Description
9.49.02—Materials
9.49.03—Construction Methods
9.49.04—Method of Measurement
9.49.05—Basis of Payment

9.49.01—Description: The work under these items shall consist of furnishing trees, shrubs, vines and ground cover, preparation of planting areas, plant layout, planting, staking and guying, fertilizing, watering and mulching, as indicated on the plans or in the Contract. It shall also include all incidental procedures, such as the care of the living plants, securing a DOT Encroachment Permit and Permit Bond as specified in 9.49.03-15, and the replacement of dead and unsatisfactory plants or unsatisfactory materials before final acceptance of the Contract.

9.49.02—Materials: The materials for these items shall meet the requirements of M.13.

9.49.03—Construction Methods: Construction methods shall be performed in accordance with the details shown on the landscape plans.

At the discretion of the Engineer, a pre-planting meeting may be held to discuss the source of supply, location of plantings, preparation of soil, time frame of delivery, temporary storage location, Contract specifics and any other incidental procedures relating to this item.

The Contractor is cautioned that within the limits of any project, buried cable for illumination or utilities, which may be energized may be present on Site. The requirements of 1.05.15 shall apply.

1. Planting Season: The planting seasons shall be those indicated below, as specified in the Contract or directed by the Engineer. Planting shall not be done if the ground is frozen, covered in snow, or if the soil is in an unsatisfactory condition as determined by the Engineer.

Deciduous Material

Spring: March 1st to May 31st (inclusive), except for balled and burlapped material. Balled and burlapped material may be planted any time from March 1st to June 15th (inclusive).

Fall: From October 15th until the ground freezes.

Evergreen Material

Spring: March 1st to May 31st (inclusive).

Fall: August 15th to October 31st (inclusive).

2. Delivery and Storage of Plants: The Contractor shall ensure that plants arrive to the Project location undamaged. The following care shall be taken during transport from the nursery through final planting location:

a. Plants shall have mulch and water as necessary to keep moist and fresh at all times.
b. Plants shall be protected against overexposure to sun, wind and freezing temperatures at all times.
c. Bare-root plants, if not planted immediately upon receipt, shall be separated upon delivery and stored in an area where their roots are kept covered to keep air away until they are ready for planting.
d. Balled and burlapped plants shall be stored with their earth balls covered by soil, wood chips, cloth, straw or other suitable material and kept moist until planting.
e. Unless specified, all plants shall be stored in a shady location until planted.

3. Field Coordination: The Contractor shall submit a Source of Supply per M.13.07-4 to initiate the inspection and approval of all material. The Contractor shall review Site conditions and inform the Engineer of any conflicts. The Contractor shall coordinate planting layout with the Engineer for approval. The Contractor must notify the Engineer no less than 48 hours in advance, excluding weekends and holidays, of the completion of layout for approval. The planting layout must be approved by the Engineer prior to the commencement of work. The installation of plant material shall occur only after the completion of paving, the installation of footings or other operations which could damage the plants or alter the finished grades.

4. Planting Layout: Plant material locations and bed outlines shall be staked in the presence of the Engineer before any plant pits or beds are excavated. Labor, equipment and new, smooth stakes of approved quality are to be furnished by the Contractor for this purpose.

5. Preparation of Planting Areas: Planting areas shall be prepared by use of approved tools. All undesirable vegetation, roots or other obstructions shall be removed from the planting areas. Any

unsuitable material shall be removed from the Site and disposed of by the Contractor in a manner satisfactory to the Engineer.

If backfill is required, as determined by the Engineer, it shall meet the planting soil requirements of M.13.01-2.

In planting areas, but not less than 14 day before the installation of plant material, the remaining turf grasses and unwanted vegetation may be sprayed at the Contractor's expense, unless otherwise directed by the Engineer, with Glyphosate or approved equal at the manufacturer's recommended rate.

6. Pit Excavation: Planting pits may be excavated or hand dug at the discretion of the Engineer. Suitable excavated soil may be set aside to be incorporated into the planting mix. The planting pit shall be excavated so that the horizontal dimension of the hole is twice the diameter of the root ball, container, or bare root spread, as shown on the plans. The depth of the plant pit excavation shall be 2 inches less than the distance between the bottom of the root ball, container, or bare root mass, and the location of the root flare or top of the root structure. It may be required to remove the burlap and some soil from the top of the root ball to expose the root structure. Care must be taken so that soil will not loosen from the roots inside the ball.

Any rock or underground obstruction shall be removed to the depth necessary for planting as specified, unless other locations for the planting are approved by the Engineer. If removal of obstructions results in a deeper hole than needed for planting, or if the pit is overexcavated, backfill shall be added, and must be thoroughly compacted to the proper depth prior to setting plants. If backfill is required, it shall meet the planting soil requirements of M.13.01-2.

7. Setting Plants: The Contractor shall move the plants from storage to the planting location, retying any untied burlap to prevent shifting while placing the plant into the planting pit. Carefully place the plant into the center of the pit. Ensure that the root flare or the top of the root system is 2 inches above finished grade. Correct pit depth if the plant is less than 2 inches, or more than 4 inches above finished grade. All plants shall be set plumb. Backfill with planting soil to 1/2 the depth of the planting pit and thoroughly tamp around the ball. Fill the remaining area of the pit with water. Once water has completely drained, fill the remainder of the pit with planting soil. Water the planting area, re-tamp, and add additional planting soil to correct any low spots. Saucers shall be formed outside of individual plants (exclusive of plant beds) by placing ridges of planting soil around each, or as directed by the Engineer. In addition, the following shall be completed for each respective type of plant:

a. **Balled and Burlapped Plants (B&B):** If wire baskets are used, the Contractor shall cut all of the horizontal wires in the top 2/3 of the rootball and bend down or remove the top 1/3 of the wire basket. Remove excess soil from the top of the root ball to expose the root structure, and cut away any small feeder or girdling roots. Roots that have been wrapped around the ball within the burlap shall be straightened.
b. **Container Grown Plants (CG)**: Carefully remove the plant from the container over the prepared pits. Gently loosen the soil and straighten all roots as naturally as possible. It may be required to cut and remove excessive amounts of root mass if roots are tightly wrapped or bound.
c. **Bare-roots Plants (BR):** Carefully spread roots as naturally as possible and place into the bottom of the pit. All broken or frayed roots shall be cleanly cut off**.**

8. Fertilizing: All plants shall be fertilized at the rate of 3 lb. per 100 s.f. of surface area (broadcast). The fertilizer shall be uniformly applied to the surface of the beds and worked into the upper 2 inches of soil. Individual trees shall be fertilized at the rate of 2 lb. per inch of trunk diameter, and the fertilizer shall be mixed into the upper 2 inches of soil.

A second application of fertilizer shall be applied to all plant items at the same specified rates over the wood-chip mulch at the end of the period of establishment.

9. Watering: All plants shall be watered upon setting and as many times thereafter as conditions warrant. The following is a guide for minimum requirements per application:

Trees: 2 1/2 inch Caliper and less – 15 gal. each.
3 inch to 5 inch Caliper – 20 gal. each.
5 1/2 inch Caliper and above – 25 gal. each.
Shrubs: 24 inches and less – 6 gal. each.
More than 24 inches - 10 gal. each.
Vines, Perennials, and Ornamental Grasses – 3 gal. each.
Groundcovers and Bulbs – 2 gal. per s.f.

Water shall be applied at a controlled rate and in such a manner to ensure that the water reaches the root zone of each plant and does not run off to adjacent areas. Watering shall be applied in a manner that does not dislodge plants, erode soil or mulch, or cause damage to saucer.

The Contractor may use slow-release, drip irrigation bags for watering at the Contractor's expense in accordance with manufacturer's instructions.

Overhead hydro-seeder spray nozzles shall not be used as watering devices.

10. Guying and Staking: Immediately after planting, trees shall be guyed or staked as shown on the plans. Guy wires, hose and tree support stakes shall be removed after the initial establishment period.

11. Pruning: As directed by the Engineer, plants shall be pruned before or immediately after planting. No leader shall be cut unless directed by the Engineer. Broken, or badly bruised branches, sucker growth, etc., shall be removed with clean cuts.

12. Spraying: Spraying with antidesiccant shall be at the Contractor's discretion and as approved by the Engineer, at the Contractor's expense.

13. Mulching: After installation of the plantings, the type of mulch specified in the Contract shall be hand placed and spread to a depth of 3 inches and raked to an even surface over all saucer areas for individual trees and shrubs and over the entire area of shrub beds and elsewhere as directed.

14. Repair: Repair of existing grass areas damaged by the Contractor in the progress of the work shall be the responsibility of the Contractor, who shall restore the disturbed areas to their original condition at the Contractor's expense.

15. One-Year Establishment Period: All plant material shall be subject to a One-Year Establishment Period. During this time, the Contractor shall use currently accepted horticultural practices to keep all plant material installed in a healthy, vigorous growing condition at the date of final acceptance. The date of final acceptance shall be 1 full calendar year following the satisfactory completion of the planting activities as confirmed by the Engineer. The Contractor shall secure a Permit Bond in the amount of $10,000 or 20% of the sum of all plant items, whichever is greater, along with an Encroachment Permit from the Department in order to guarantee the One-Year Establishment Period.

The Permit Bond shall be provided to the Department at the completion and acceptance of all planting. A final inspection will be held 1 year from the date of installation, with the Contractor, Engineer, and Landscape Designer in attendance, to determine the acceptability of the plant establishment. An inventory of losses and rejected materials will be made by the Department and corrective clean up measures will be determined. After the Contractor completes all corrective actions, the DOT Encroachment Permit and Permit Bond will be released by the Department.

9.49.04—Method of Measurement:

1. Planting: The quantity for which payment will be made will be the number of each size and kind of plant counted in place, planted and accepted.

2. Mulching: This work will be measured for payment by the number of square yards surface measurement of the specified thickness for the area on which the type of mulch specified in the plans has been completed and accepted.

9.49.05—Basis of Payment:

1. Planting: Payment for this work will be made at the Contract unit price each for the kind and size of plant and method of planting, as the case may be, completed and accepted in place. The cost of the Performance Bond and Encroachment permit will be included

2. Mulching: This work will be paid for at the Contract unit price per square yard for mulch complete in place.

3. The unit prices shall include all materials, equipment, tools, labor, transportation, operations and all work incidental thereto, including the removal of guy wires, hose and tree support stakes after the initial establishment period, except that payment for excavation of solid ledge rock, concrete pavement and boulders 1/2 cubic yard in volume or greater will be made under 9.51, "Rock Excavation for Planting."

Pay Item	Pay Unit
(Plant Name) (Caliper)	ea.
(Plant Name) (Height)	ea.
(Plant Name) (Size)	ea.
(Vine Name) (Size)	ea.
(Ground Cover Name) (Size)	ea.
Wood Chip Mulch	s.y.

9.49.05

Gravel Mulch s.y.

SECTION 9.50
TURF ESTABLISHMENT
EROSION CONTROL MATTING

9.50.01—Description: The work included in this item shall consist of providing an accepted uniform stand of established perennial turf grasses by furnishing and placing fertilizer, seed, and mulch on all areas to be treated as shown on the plans or where designated by the Engineer. Sowing shall be by traditional installation or hydroseed methods.

The work shall also include the installation of erosion control matting, as shown on the plans or where designated by the Engineer, consisting of mulch and netting woven together as a unit.

9.50.02—Materials:

Seed shall meet the requirements of M.13.04.

Fertilizer shall meet the requirements of M.13.03.

Mulch shall be either wood fiber, hay or straw and shall meet the requirements of M.13.05.

Erosion control matting, if required, shall be from the Department's Qualified Products List and shall meet the requirements of M.13.09.

9.50.03—Construction Methods: Construction Methods shall be those established as agronomically acceptable and feasible and which are approved by the Engineer.

1. Surface Preparation:

a) Level areas, medians, interchanges and lawns: These areas shall be made friable and receptive for seeding by disking or by other approved methods to the satisfaction of the Engineer. All disturbed soil areas at final grade shall be seeded within 7 days, or as directed by the Engineer, in accordance with these specifications. In all cases, the final prepared and seeded soil surface shall meet the lines and grades for such surface as shown in the plans, or as directed by the Engineer.

b) Slope and Embankment Areas: These areas shall be made friable and receptive to seeding by disking or by other approved methods which will not disrupt the line and grade of the slope surface. In no event will seeding be permitted on hard or crusted soil surface.

c) Seeding shall not be permitted until all weed growth is removed.

2. Seeding Season: The optimal calendar dates for seeding are:

Spring—March 15 to June 30
Fall—August 15 to October 31

All disturbed soil areas at final grade shall be seeded within 7 days or as directed by the Engineer, in accordance with these specifications.

Any seeding outside the optimal dates shall be performed in the same manner. Since acceptable turf establishment is less likely, the Contractor shall be responsible for reseeding until the turf stand conforms to 9.50.03-5.

3. Sowing Methods: The Contractor shall sow the grass seed mixture using traditional methods or by hydroseeding.

a) Sowing by Traditional Methods:

The rate of application shall be no less than 175 lb./acre.

Fertilizer shall be initially applied at a rate of 320 lb./acre during or preceding seeding.

When wood fiber mulch is used, it shall be applied in water slurry at a rate of 2,000 lb./acre with or immediately after the application of seed, fertilizer and limestone (if limestone is required).

Tackifier may be used with straw mulch as proposed by the Contractor.

When the grass seeding growth has attained a height of 6 inches, the specified grass areas (mowed and un-mowed) shall receive a uniform application of fertilizer hydraulically placed at the rate of 320 lb./acre

b) Sowing by Hydroseeding:

If hydroseeding is proposed to be used, the Contactor shall furnish a Hydroseeding Plan for the Engineer's acceptance two weeks prior to the start of this work.

The Hydroseeding Plan shall include the following:

i. Proposed Manufacturer and copy of the Manufacturer's recommended application rates for various grades and hose angles of application, for the Site's soil type(s) and expected weather conditions.
ii. Number of square feet (s.f.) of seeding that can be covered with the quantity of solution per hydro-seeder.
iii. Time between mixing of slurry and seed in hydroseeding tank and application.

iv. Type of hydroseed machine including nozzle type, including automation information if applicable.

If the Hydroseeding Plan is accepted for use, deviation from 9.50.03-1 (Surface Preparation) will not be allowed. Hydroseeding shall not be used if the extended weather patterns are hot and dry and the soil surface is dry and dusty, unless the Contractor's submission addresses application of straw or hay mulch and addresses follow up maintenance (i.e. additional watering) for "drought conditions."

The hydroseed tank and hose(s) shall be completely flushed and cleaned each day before seeding is to be started, and shall also be thoroughly flushed of all residue after application to every 10 acres.

4. Disturbance: The Contractor shall keep all equipment and vehicular and pedestrian traffic off areas that have been seeded to prevent excessive compaction and damage to young plants. Where any disturbance has occurred, the Contractor shall rework the soil to make a suitable seedbed, then re-seed and mulch such areas with the full amounts of the specified materials, at no additional cost to the State.

5. Stand of Perennial Turf Grasses: The Contractor shall provide and maintain a uniform stand of established turf grass species having attained a height of 6 inches consisting of no less than 60% coverage per square foot throughout the seeded areas until the entire Project has been accepted. Reseeding required to achieve and maintain a uniform stand of established turf grass species shall be at no additional cost to the State.

6. Establishment: The Contractor shall keep all seeded areas free from weeds and debris, such as stones, cables, baling wire, and shall mow at its own expense all slopes 4:1 or less (flatter) and level turf established (seeded) areas to a height of 3 inches when the grass growth attains a height of 6 inches. Mowing shall be done at least once, but for multiple-year projects mowing shall be done at least twice per year.

Clean-up shall include, but not be limited to, the removal of all debris from the turf establishment operations on the shoulders, pavement, or elsewhere on adjacent properties publicly and privately owned.

7. Erosion Control Matting: Erosion control matting shall be installed following seeding where called for on the plans or as directed by the Engineer. Staples shall be installed as per manufacturer's recommendations. Where 2 lengths of matting are joined, the end of the up-grade strip shall overlap the down-grade strip per the manufacturer's recommendations.

The Contractor shall maintain and protect the areas with erosion control matting until such time as the turf grass is established. The Contractor shall replace or repair at its own expense any and all erosion control matting areas damaged by fire, water or other causes including the operation of construction equipment. No mowing will be required in the locations where erosion control matting is installed.

9.50.04—Method of Measurement: This work will be measured for payment by the number of square yards of surface area of accepted established perennial turf grass.

Erosion control matting will be measured by the number of square yards of surface area of erosion control matting installed and accepted.

9.50.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Turf Establishment" which price shall include all materials, mowing, maintenance, equipment, tools, labor, and work incidental thereto. Partial payment of up to 60% may be made for work completed, but not accepted.

Erosion control matting will be paid for at the Contract unit price per square yard for "Erosion Control Matting (Type)" complete in place and accepted, which price shall include the mulch, netting, staples, maintenance, equipment, tools, labor, and work incidental thereto.

Pay Item	Pay Unit
Turf Establishment	s.y.
Erosion Control Matting (Type)	s.y.

SECTION 9.51
ROCK EXCAVATION FOR PLANTING

9.51.01—Description: When solid ledge rock, concrete pavement, or boulders 1/2 c.y. in size or greater, are encountered in the planting area, the Contractor shall notify the Engineer who will determine whether the rock or concrete pavement shall be removed or the plant or plants relocated. Rock or concrete pavement as defined herein shall not be removed without the permission of the Engineer.

9.51.02—Materials: Vacant

9.51.03—Construction Methods: When rock or concrete pavement removal is approved by the Engineer, the plant pit diameter shall be 3 feet greater than the diameter of the ball of earth or root spread of the plant; and the depth of the pit shall be the same as when the excavation is made in earth.

All unsuitable material resulting from rock or concrete pavement excavation, even if beyond the payment lines, and all breakage and slides shall be removed and disposed of as directed.

When blasting is necessary, the work shall meet the requirements of 1.07.08

9.51.04—Method of Measurement: The measurement for payment of the quantity of ledge rock, concrete pavement, or boulders 1/2 c.y. in size or greater, removed upon orders of the Engineer, will be the actual number of cubic yards of such material ordered removed within the designated limits.

9.51.05—Basis of Payment: Payment for the removal of ledge rock, concrete pavement, or boulders 1/2 c.y. in size or greater, will be made at the Contract unit price per cubic yard for "Rock Excavation for Planting," which price shall include all materials, equipment, tools, labor and work incidental thereto. It shall also include disposal of surplus or unsuitable material.

Pay Item	Pay Unit
Rock Excavation for Planting	c.y.

SECTION 9.52
SELECTIVE CLEARING AND THINNING

9.52.01—Description: This work shall consist of cutting, trimming and removal of trees, stumps, brush, rubbish and objectionable material as shown on the plans or as directed by the Engineer.

9.52.03—Construction Methods: All work pertaining to the cutting, trimming and removal of trees, stumps, brush, rubbish and other objectionable material shall be done in a neat and orderly manner so as not to cause damage to adjacent vegetation. Trees, stumps and brush to be removed shall be cut flush with the ground surface. Branches of trees shall be trimmed as directed.

Prior to clearing operations, a meeting must be held. Those attending the meeting should include the Contractor, the Engineer, the designer, local tree warden or equivalent, and the District Environmental Coordinator. All clearing issues shall be resolved to the satisfaction of the Engineer before any trees are cut.

All trees scheduled to be removed shall be visibly marked or flagged by the Contractor at least 7 days prior to the cutting of such trees.

The Engineer will inspect the identified trees and verify the limits of clearing and thinning prior to the Contractor proceeding with its cutting operation.

The Contractor shall dispose of all such trees, branches, stumps, brush, etc., in a satisfactory manner and shall remove all rubbish and refuse from within the highway limits.

9.52.04—Method of Measurement: Selective clearing and thinning, being paid for on a lump sum basis, will not be measured for payment.

9.52.05—Basis of Payment: This work will be paid for at the Contract lump sum price for "Selective Clearing and Thinning" which price shall include all material, tools, equipment and labor incidental thereto.

Pay Item	Pay Unit
Selective Clearing and Thinning	l.s.

SECTION 9.53
SODDING

9.53.01—Description: Work under this item shall consist of furnishing and placing of live sod and also the furnishing and preparing a sod bed composed of 4 inches, after tamping, of approved topsoil in the areas designated on the plans or where directed by the Engineer.

9.53.02—Materials: The material for this work shall meet the requirements of M.13.01 for Topsoil, M.13.02 for Agricultural Ground Dolomitic Limestone, M.13.03 for Fertilizer, and M.13.08 for Sod. Stakes for pegging sod shall be of wood, approximately 1 inch × 2 inches and of sufficient length to penetrate the sod, the topsoil and to a minimum depth of 2 inches of subsoil.

9.53.03—Construction Methods:

1. Season: The work may be performed at any season of the year unless otherwise specified. The Contractor shall notify the Engineer at least 48 hours in advance of the time proposed to begin sodding and shall not proceed with such work until permission to do so has been granted. No frozen sod shall be placed. No sodding shall be done on frozen earth. When sodding is to be accomplished on the same general area where seeding is to be done under the same contract, the sodding work shall be carried out before the seed is sown.

2. Procuring Sod: Care shall be exercised at all times to retain the native soil on the roots of the sod during the process of transplanting. Dumping from vehicles will not be permitted. The sod shall be planted within 24 hours from the time it is harvested unless it is tightly rolled or stored roots-to-roots in a satisfactory manner. All sod in stacks shall be kept moist and shall be protected from exposure to the sun and from freezing. No storage longer than 5 days will be permitted. Sod which becomes dried out or does not meet the specifications will be rejected.

3. Ground Preparation: There shall be a minimum of 4 inches, after tamping, of topsoil under all sod unless otherwise specified. Excavations or trenches shall be made to a sufficient depth below the finished grade of the sod to accommodate the depth of topsoil as specified and the thickness of sod as specified. Fertilizer shall be applied at a rate to provide 100 lb./ac of nitrogen unless fertilizer has been applied under another item in this Contract to the topsoil in the sod bed. Lime, although not required in sod bed preparation, will be permitted in the sod bed if applied in a previous operation. Fertilizer applied under this item shall be incorporated with the topsoil to a depth of at least 2 inches before the sod is laid, unless otherwise specified or approved. Incorporation shall be accomplished by disking, harrowing, drilling, raking or other approved means.

4. Planting Sod: The soil on which the sod is laid shall be reasonably moist and shall be watered, if so directed. The sod shall be laid smoothly, edge to edge, and all openings shall be plugged with sod. In drainage-ways and where continuous or solid sodding is called for on the plans, the sod shall be laid with their longest dimensions parallel to the contours. Such sodding shall be begun at the base of slopes or grades and the sodding progress in continuous parallel rows working upward. Vertical joints between such sod shall be staggered. All sod shall be laid to the grades specified and the grades formed with special care at the junction of drainage-ways. Immediately after the sod is laid, it shall be pressed firmly into contact with the sod bed by tamping, rolling, or by other approved methods so as to eliminate all air pockets, provide true and even surfaces, insure knitting and protect all exposed sod edges but without displacement of the sod or deformation of the surface of the sodded areas and watered at the rate of 5 gal./s.y. of sodded area unless otherwise directed.

5. Pegging: Sod shall be held in place by stakes in all drainage-ways, on all slopes steeper than 1:4 and elsewhere where specified or as directed. Pegging shall be done immediately after tamping. At least one stake shall be driven through each sod to be staked, and the stakes shall be not more than 2 feet apart. Stakes shall have their flat sides against the slope and be driven flush.

6. Finishing: Excess sod or excess soil resulting from excavations or trenching shall be disposed of as approved. Excess soil shall not be left to form a ridge adjacent to the sodded area or sodded strips. No payment will be made for rejected or excess sod which is not laid.

7. Care During Construction: The Contractor shall care for the sodded areas until all work on the entire Contract has been completed and accepted. Such care shall consist of providing protection against traffic by approved warning signs or barricades, and the mowing of grass to the height of 2 inches when the growth attains a maximum height of 4 inches.

Sod shall also be watered if so required by the Engineer. When watered, sufficient water shall be applied to wet the sod at least 2 inches deep in the sod bed. Watering shall be done in a manner which will not

cause erosion nor other damage to the finished surfaces. Any surfaces which become gullied or otherwise damaged shall be repaired to re-establish the grade and conditions of the soil prior to sodding and shall then be re-fertilized and re-sodded as specified under this item.

9.53.04—Method of Measurement: This work will be measured for payment by the number of square yards of surface area acceptably sodded.

9.53.05—Basis of Payment: This work will be paid for at the Contract unit price per square yard for "Sodding," which payment shall constitute full compensation for furnishing all labor, materials, including topsoil placed under the sod, equipment and incidentals necessary to complete and care for the work as specified, including water and its application.

Pay Item	Pay Unit
Sodding	s.y.

SECTION 9.70
TRAFFICPERSON

9.70.01—Description: Under this item the Contractor shall provide the services of Trafficpersons of the type and number, and for such periods, as the Engineer approves for the control and direction of vehicular and pedestrian traffic in areas affected by Project operations. Trafficpersons' services which have not been requested or approved by the Engineer, but which have been obtained by the Contractor solely to meet its operational plans or needs, will not be approved for payment.

9.70.03—Construction Methods: Prior to the start of operations which require the use of Trafficpersons, a meeting will be held with the Contractor, the Engineer, any relevant Trafficperson agency or firm, and, if applicable, State Police, to review Trafficperson operations, lines of responsibility, and operating guidelines for the Project. The Contractor shall provide a copy of each pertinent municipality's billing rates for Municipal Police Officers and their vehicles, as applicable, to the Engineer prior to the start of Project construction.

On a weekly basis, the Contractor shall inform the Engineer of its scheduled operations for the following week and the number of Trafficpersons it proposes to use for those operations. The Engineer will review the proposal and, if it is acceptable to them, approve the type and number of Trafficpersons to be used. In the event of an unplanned, emergency, or short-term operation, the Engineer may approve the temporary use of properly-clothed persons for traffic control until such time as an authorized Trafficperson may be obtained for that work. In no case shall such temporary use exceed 8 hours for any particular operation.

If the Contractor changes or cancels any scheduled operation without any prior notice of same as may be required by the agency providing the Trafficpersons, the Contractor shall be responsible for payment, at no cost to the Department, of any show-up cost charged by said agency for affected Trafficpersons who would have been used if not for the pertinent change or cancellation. Exceptions, as approved by the Engineer, may be granted for adverse weather conditions and unforeseeable causes beyond the control, and not involving the fault or negligence, of the Contractor.

Trafficpersons assigned to a work site shall take direction only from the Engineer.

Trafficpersons shall wear a high-visibility safety garment compliant with OSHA, MUTCD, and ASTM Standards; and the safety garment shall have the words "Traffic Control" printed and clearly visible on the front and rear panels (minimum letter size 2 inches) of the garment. Worn or faded safety garments that are no longer highly visible shall not be used. At the direction of the Engineer, the Contractor must replace any such garments at no cost to the State.

A Trafficperson shall assist in implementing the traffic control as specified in the item "Maintenance and Protection of Traffic" contained elsewhere in the Contract, or as otherwise directed by the Engineer. Any use of a Trafficperson for Project operations in a manner that conflicts with requirements of the Maintenance and Protection of Traffic specification must have been authorized in writing by the Engineer.

Trafficpersons shall consist of the following types:

1. **Municipal Police Officers:** Uniformed Municipal Police Officers shall be sworn Municipal Police Officers or Uniformed Constables employed by the Municipality in which the Project is located, who perform criminal law enforcement duties for the Municipality. Law enforcement personnel shall wear the high-visibility safety garment provided by their law enforcement agency. If no high-visibility safety garment is provided by said agency, the Contractor shall provide the law enforcement personnel with a garment meeting the requirements stated below for a Uniformed Flagger's garment.

   Law Enforcement Personnel may also be used for conducting motor vehicle enforcement operations in and around work areas as directed or approved by the Engineer.

   Their services will also include their use of an official Municipal Police vehicle when so requested by the Engineer. Uniformed Municipal Police Officers must not be used on limited access highways. If Uniformed Municipal Police Officers are unavailable for a given task, other Trafficpersons may be used if so authorized in writing by the Engineer. Uniformed Municipal Police Officers and requested Municipal Police vehicles will be used at such locations and for such periods as the Engineer deems necessary for the control of traffic operations and for the safety of motorists passing through sites affected by Project operations.

2. **Uniformed Flagger**: Uniformed Flaggers shall be persons who have successfully completed flagger training by the American Traffic Safety Services Association (ATSSA), National Safety Council (NSC), or other such training approved by the Engineer. A copy of the Flagger's training certificate shall be provided to the Engineer before the Flagger performs any Project work. The credentials and conduct of Uniformed

Flaggers shall comply with the requirements of Chapter 6E, Flagger Control in the Manual of Uniformed Traffic Control Devices (MUTCD). Uniformed Flaggers shall wear high-visibility safety apparel and use a STOP/SLOW paddle that is at least 18 inches wide with letters at least 6 inches high. The paddle shall be mounted on a pole of sufficient length to be 6 feet above the ground as measured from the bottom of the sign.

Uniformed Flaggers shall be used only on non-limited access highways for the purpose of controlling traffic operations and only when authorized to do so in writing by the Engineer.

9.70.04—Method of Measurement: Services of Trafficpersons will be measured for payment by the actual number of hours for each person rendering services approved by the Engineer. These services shall include only such Trafficpersons as are employed within the limits of construction, right of way of the Project, or along detours authorized by the Engineer in order to assist public travel through areas affected by Project construction. Trafficperson services employed due to use of a detour or bypass beyond the limitations approved by the Engineer, or in connection with movement of construction vehicles or equipment, or at locations where traffic has been unnecessarily restricted by the Contractor's method of operation, will not be measured for payment.

Trafficpersons shall not work more than 12 hours in any one 24-hour period. If such services are essential for more than 12 hours in such period, for a use approved by the Engineer, additional Trafficpersons engaged by the Contractor to meet that circumstance shall be measured for payment. If a Trafficperson used with the Engineer's authorization is an employee on the Contractor's payroll, payment under the item "Trafficperson (Uniformed Flagger)" will be made only for those hours when said employee is performing Trafficperson services.

No travel time will be measured for payment for Uniformed Municipal Police Officers or Uniformed Flaggers.

Mileage fees associated with Trafficperson services will not be measured for payment.

Safety garments and STOP/SLOW paddles will not be measured for payment.

9.70.05—Basis of Payment: Trafficpersons will be paid in accordance with the schedule described herein.

There will be no direct payment for safety garments or STOP/SLOW paddles. All costs associated with furnishing safety garments and STOP/SLOW paddles shall be considered included in the general cost of the item.

1. Uniformed Law Enforcement Personnel: The sum of money for this item shown on the Estimate and in the itemized proposal as "Estimated Cost" for this work will be considered the bid price, even though payment will be made as described below. The estimated cost figure is not to be altered in any manner by the bidder. Should the bidder alter the amount shown, the altered figures will be disregarded and the original price will be used in determining the total amount for the Contract.

The Department will pay the Contractor its actual costs for "Trafficperson (Municipal Police Officer)" plus an additional 5% as reimbursement for the Contractor's administrative expense in connection with the services provided. The 5% markup will be paid when the Engineer receives from the Contractor cancelled check(s) or receipted invoice(s) as proof of its pertinent payments.

The invoice must include a breakdown of each officer's actual hours of work and actual rate applied. Mileage fees associated with Trafficperson services are not reimbursable expenses and are not to be included in the billing invoice. The use of a Municipal police vehicle authorized by the Engineer will be paid at the actual rate charged by the Municipality. Upon receipt of the invoice from the Municipality, the Contractor shall forward a copy of it to the Engineer. No payment on such an invoice will be made until and unless the Engineer has reviewed the invoice and approved the payment. The rate charged by the municipality for use of a Uniformed Municipal Police Officer or a Municipal police vehicle shall not be greater than the rate that the Municipality normally charges others for similar services.

2. Uniformed Flagger: Uniformed Flaggers will be paid for at the Contract unit price per hour for "Trafficperson (Uniformed Flagger)," which price shall include all compensation, insurance benefits and any other cost or liability incidental to the furnishing of the Trafficperson services authorized under the Contract or approved by the Engineer.

Pay Item	Pay Unit
Trafficperson (Municipal Police Officer)	est.
Trafficperson (Uniformed Flagger)	hr.

SECTION 9.71
MAINTENANCE AND PROTECTION OF TRAFFIC

9.71.01—Description: Unless other provisions are made on the plans or in the special provisions of the Contract, the Contractor shall keep the roadway under construction open to traffic for the full length of the Project and shall provide a sufficient number of travel lanes and pedestrian passways to move that traffic ordinarily using the roadway. The travel lanes and pedestrian passways shall be drained and kept reasonably smooth and in suitable condition at all times in order to provide minimum interference to traffic consistent with the proper prosecution of the work.

Suitable ingress and egress shall be provided at all times where required, for all intersecting roads and for all abutting properties having legal access.

When a scheme for maintenance of traffic, which may include detours, is shown on the plans or described in the special provisions of the Contract, this shall govern unless an alternate scheme acceptable to the Engineer is offered by the Contractor at no additional cost. If no scheme is shown on the plans or described in the special provisions of the Contract, and the Contractor wishes to deviate from the provisions of maintaining traffic as described in this Section, the Contractor may submit and the Engineer may approve a schedule showing a proposed sequence of operations and a compatible method of maintaining traffic.

9.71.03—Construction Method: The Contractor shall furnish and erect signs legally closing the highway to traffic, as shown on the plans or directed by the Engineer, prior to commencing any work on the Project.

The Contractor shall furnish a sufficient number of signs, barricades, drums, traffic cones and delineators to forewarn traffic of the construction as shown on the traffic control plans contained within or as directed by the Engineer.

The Contractor shall also provide such safety measures, pavement markings, warning devices and signs as deemed necessary to safeguard and guide the traveling public through detours ordered by the Engineer, included in the approved scheme for maintenance of traffic, or as shown on the plans. The Contractor shall erect, maintain, move, adjust, clean, relocate and store these signs, barricades, drums, traffic cones and delineators when, where and as directed by the Engineer, and in accordance with the ATSSA guidelines contained in "Quality Standards for Work Zone Traffic Control Devices."

The use of unauthorized or unapproved signs, barricades, drums, traffic cones or delineators will not be permitted.

All signs in any one signing pattern shall be mounted the same height above the traveled surface. The Contractor shall keep all signs in proper position, clean and legible at all times. Care shall be taken so that weeds, shrubbery, construction materials or equipment, and soil, are not allowed to obscure any sign, light, or barricade. Signs that do not apply to existing conditions shall be removed or adjusted so that the legend is not visible to approaching traffic.

The Contractor, when ordered by the Engineer, shall remove snow and take care of icy conditions on temporary, new and existing sidewalks on any part of the right-of-way within the limits of the Project. Payment for the cost thereof, will be made as extra work.

Snow removal and correction of icy conditions, other than those resulting from the Contractor's operations, on uncompleted contracts under traffic, will remain an obligation of the State or others.

Should the Contractor fail to perform any of the work required under this section, the State may perform or arrange for others to perform such work. In such cases, the State will deduct from money due or to become due the Contractor all expenses connected there with which are found to be greater than the cost to the State had the Contractor performed the specified work.

9.71.05—Basis of Payment: When the item of "Maintenance and Protection of Traffic" appears in the Contract, this work will be paid for at the Contract lump sum price for "Maintenance and Protection of Traffic." This price shall include all costs for labor, equipment and services involved in the erection, maintenance, moving, adjusting, cleaning, relocating and storing of signs, barricades, drums, traffic cones and delineators furnished by the Contractor as well as all costs of labor and equipment involved in the maintenance of traffic lanes and detours, except for pavement markings, ordered or included in the approved scheme for maintenance of traffic. "Maintenance and Protection of Traffic" does not include the cost of signs, barricades, drums, traffic cones, delineators, or the furnishing and placing of materials such as borrow, gravel, crushed stone, bituminous concrete for patching and pipe. These items will be paid for at Contract unit prices, or in the absence of applicable Contract unit prices, as extra work. If the Engineer requires the Contractor to provide facilities in excess of the requirements of the adopted scheme for

maintenance and protection of traffic, the Contractor shall perform the required work; and payment for the cost thereof will be made at applicable Contract unit prices, or in the absence of applicable Contract unit prices, as extra work.

Pay Item	Pay Unit
Maintenance and Protection of Traffic	l.s.

SECTION 9.74
REMOVAL OF EXISTING MASONRY

9.74.01—Description
9.74.02—Vacant
9.74.03—Construction Methods
9.74.04—Method of Measurement
9.74.05—Basis of Payment

9.74.01—Description: This work shall include the full or partial removal and disposal of substructures, walls, approach slabs and other masonry components, as indicated on the plans. These structures may be constructed of dry masonry, cement rubble masonry, concrete or reinforced concrete.

9.74.03—Construction Methods:

1. Submittals:

The Contractor shall prepare and submit written procedures for removal. Working drawings, in accordance with 1.05.02, shall also be prepared and submitted as warranted by the complexity and safety considerations of the work. The submittals shall address the following:

- proposed equipment and removal method(s)
- operating and storage location(s) of equipment
- containment and disposal of debris
- installation and removal of:
    a. debris shields
    b. working platforms
    c. falsework
- temporary support(s) for maintenance of traffic
- modification to and restoration of the structure to remain in place

2. Removal: Structures and bridge components shall be removed to the limits shown on the plans or as directed by the Engineer.

General removal may be performed by excavator-mounted demolition equipment or other methods except where prohibited on the plans or as directed by the Engineer.

If partial removal of concrete is required, it shall be sawcut to the neat lines as indicated on the plans. Near reinforcing steel that is to remain, the Contractor must use limited methods for removal, such as 15 pound hammers or other methods accepted by the Engineer.

Reinforcing steel shall be cut and removed where shown on the plans. Reinforcing steel to remain shall be cleaned of all concrete and corrosion products by oil-free abrasive blasting, high-pressure water blasting or other methods accepted by the Engineer. The reinforcing steel and concrete surfaces shall be free from dirt, oil, cement fines (slurry), or any material that may interfere with the bond of the proposed concrete. Tightly-bonded light rust on the reinforcing surface is acceptable.

Where stage construction requires concrete to be removed adjacent to the existing structure that will continue to support live load, the Contractor shall cut the concrete in accordance with the accepted working drawings at the demolition limit shown on the plans to minimize disturbance to the section that is to remain in place.

3. Disposal of Debris: The Contractor shall properly dispose of all construction debris either off-Site, or on-Site in accordance with 2.02.03-5.

4. Damage Mitigation: When removing the structures or a portion thereof, the Contractor shall take necessary precautions to prevent debris from dropping to areas below the structure, onto adjacent traffic lanes or onto adjacent property. Any damage to adjoining areas, including but not limited to new construction, public utility installations, abutting property and to the portions of the structure that will remain shall be repaired by the Contractor in accordance with 1.05.11.

9.74.04—Method of Measurement: This work will be measured for payment by the volume in cubic yards in place prior to removal, to the limits shown on the plans or as directed by the Engineer.

9.74.05—Basis of Payment:

Payment for "Removal of Existing Masonry" will be made at the Contract unit price per cubic yard, which price shall include all equipment, tools and labor incidental to the removal and shall include the proper disposal thereof.

The cost of furnishing, installing and removing protective debris shielding, falsework and working platforms is included in the cost of this item.

Payment for the full or partial removal of bridge superstructure(s) will be made at the Contract lump sum price for "Removal of Superstructure," in accordance with 5.03.05.

Pay Item	Pay Unit
Removal of Existing Masonry	c.y.

SECTION 9.75
MOBILIZATION AND PROJECT CLOSEOUT

9.75.01—Description: This item consists of

1. all work necessary for moving Project personnel and equipment to the Project Site;
2. all work necessary for the establishment of the Contractors' field offices, buildings and other facilities necessary for Contract performance;
3. the preparation of work plans and other documents that must be submitted by the Contractor to the Department prior to the start of physical Project construction. These initial submittals are identified elsewhere in the Contract and may include Project schedules, Project management plans, staging and storage areas, safety plans, quality control plans, erosion and sedimentation control plans, and other documents addressing general Project sequencing or management;
4. demobilization of plant and equipment;
5. completion of all physical work, and
6. completion of administrative closeout items as required by the Contract.

The work entailed in this item shall not be subcontracted in whole or part.

9.75.04—Method of Measurement: This work will be measured for payment in the manner described hereinafter; however, the total Contract amount earned will not include payments for mobilization that were earned during the period covered by the current monthly estimate, but will include those payments for mobilization that were previously earned and certified for payment.

1. When the first Project payment estimate is reviewed by the Engineer, 25% of the lump sum bid price for this item or 2.5% of the total original Contract price, whichever is less, will be certified for payment as a part of that estimate.
2. When the Contractor's initial Project submittals are accepted by the Engineer, 50% of the lump sum bid price for this item or 5% of the total original Contract price, whichever is less, minus any previous Project payments made to the Contractor for this item, will be certified for payment.
3. When the Contractor's initial Project submittals are accepted by the Engineer, and 15% of the total original Contract price has been earned by the Contractor, 70% of the lump sum price of this item or 7% of the total original Contract price, whichever is less, minus any previous Project payments made to the Contractor for this item, will be certified for payment.
4. When 30% of the total original Contract price has been earned by the Contractor, 85% of the lump sum price of this item or 8.5% of the total original Contract price, whichever is less, minus any previous payments made to the Contractor for this item, will be certified for payment.
5. When the requirements of 1.08.13 have been satisfied by the Contractor, 95% of the lump sum price of this item, minus any previous payments made to the Contractor for this item, will be certified for payment.
6. When the requirements of 1.08.14 have been satisfied by the Contractor, 100% of the lump sum price of this item, minus any previous payments made to the Contractor for this item, will be certified for payment. When this payment is made, the Contractor should have received full Contract payment for this item.

Nothing herein shall be construed to limit or preclude the Department from making partial payments to the Contractor that are provided for elsewhere in this Contract.

9.75.05—Basis of Payment: The work under this item will be paid for at the Contract lump sum price for "Mobilization and Project Closeout," which price shall include materials, equipment, tools, transportation, labor and all work incidental thereto.

Payment for this item shall be made only once; *i.e.*, for only one instance of mobilization as described in 9.75.01 above. If the Contractor mobilizes equipment or facilities more than one time during the course of the Project, due to reasons solely the responsibility of the Department, the additional work entailed therein will be paid for as Extra Work under 1.04.05 hereof.

Pay Item	Pay Unit
Mobilization and Project Closeout	l.s.

SECTION 9.76
BARRICADE WARNING LIGHTS

9.76.01—Description: This item shall include furnishing and maintaining designated type barricade warning lights on signs and barricades and elsewhere as ordered by the Engineer.

9.76.02—Materials: Barricade warning lights are portable, lens-directed, enclosed lights. The color of the light emitted shall be yellow. They may be used in either a steady-burn or flashing mode. Barricade warning lights shall be in accordance with the requirements of the ITE Standard for Flashing and Steady-Burn Barricade Warning Lights and the following table:

	Type A Low Intensity	**Type B High Intensity**	**Type C Steady Burn**
Lens Directional Faces	1 or 2	1	1 or 2
Flash Rate per minute	55 to 75	55 to 75	Constant
Flash Duration [1]	10%	8%	Constant
Minimum Effective Intensity[2]	4.0 Candelas	35 Candelas	— — —
Minimum Beam Candelas	— — —	— — —	2 Candelas
Hours of Operation	Dusk to dawn	24 hrs. per day	Dusk to dawn

[1]Length of time that instantaneous intensity is equal to or greater than effective intensity.
[2]These values must be maintained within a solid angle 9 degrees on each side of the vertical axis and 5 degrees above and 5 degrees below the horizontal axis.

9.76.03—Construction Methods: Barricade warning lights shall be used as follows:

Type A low-intensity flashing warning lights shall be mounted on separate portable supports, on Class II barricades or on vertical channelizing devices.

Type B high-intensity flashing warning lights shall be mounted on the advance warning signs or on independent supports. Extremely hazardous site conditions within the construction area may require that the lights be mounted on Class II barricades, signs or other supports.

Type C steady-burn lights shall be used to delineate the edge of the traveled way on detour curves, on lane changes, on lane closures and on other similar conditions.

The Engineer shall determine the type of barricade warning lights to be used. Portable supports for barricade warning lights shall provide a minimum mounting height of 36 inches to the bottom of the lens.

The Contractor shall furnish and securely fasten the units to signs, barricades and other objects in such numbers and for such lengths of time as the Engineer may order.

The Contractor shall maintain and relocate the units and, upon final removal, dispose of them.

Any barricade warning lights that are missing, damaged, defaced, or improperly functioning so that they are not effective, as determined by the Engineer and in accordance with the ATSSA guidelines contained in "Quality Standards for Work Zone Traffic Control Devices," shall be replaced by the Contractor at no cost to the State.

9.76.04—Method of Measurement: This work will be measured for payment by the number of calendar days that each individual unit is in place and in operation as herein before described, measured to the nearest day.

9.76.05—Basis of Payment: Barricade warning lights will be paid for at the Contract unit price per day for the type barricade warning lights used, which price shall include all materials, equipment, tools, labor and work incidental to furnishing, maintaining, changing location, removing and disposing of the units.

Pay Item	Pay Unit
Barricade Warning Lights (Type)	day

SECTION 9.77
TRAFFIC CONE

9.77.01—Description: Under this item the Contractor shall furnish all reflectorized orange traffic cones required on the Project to meet the requirements as stated in the item "Maintenance and Protection of Traffic," as shown on the plans and as directed by the Engineer.

The Contractor shall have, available on the Project, a sufficient number of traffic cones to fulfill all the requirements as specified in the Contract and to replace those traffic cones which have become damaged.

9.77.02—Materials: Traffic cones shall be constructed of materials to a thickness to withstand impact without damage to cones or to vehicles. The traffic cones shall be of sufficient mass or have bases to which ballast may be added to assure that they will not be blown over or displaced by wind from passing vehicles. Traffic cones used at night shall be reflectorized by utilizing Retroreflective Sheeting, in accordance with M.18.09.

Prior to using traffic cones on the Project, the Contractor shall submit to the Engineer a copy of the manufacturer’s self-certification that the traffic cones comply with the requirements of the NCHRP Report 350 or the AASHTO Manual for Assessing Safety Hardware (MASH) for Category 1 Devices.

9.77.04—Method of Measurement: This item will be measured for payment by the number of traffic cones used on the Project.

9.77.05—Basis of Payment: This item will be paid for at the Contract unit price each for "Traffic Cone" used on the Project. Each cone will be paid for once, regardless of the number of times it is used on the Project.

Any traffic cones that are missing, damaged or defaced so that they are not effective, as determined by the Engineer in accordance with the ATSSA guidelines contained in "Quality Standards for Work Zone Traffic Control Devices," shall be replaced by the Contractor at no cost to the State.

When the traffic cones are no longer required on the Project they shall remain the property of the Contractor.

Pay Item	Pay Unit
Traffic Cone	ea.

SECTION 9.78
TRAFFIC DRUM

9.78.01—Description
9.78.02—Materials
9.78.03—Construction Methods
9.78.04—Method of Measurement
9.78.05—Basis of Payment

9.78.01—Description: Under this item the Contractor shall furnish all traffic drums required on the Project to correspond to the traffic patterns, as indicated in the "Maintenance and Protection of Traffic," as shown on the plans and as directed by the Engineer.

9.78.02—Materials: Traffic Drums shall be manufactured plastic or rubber devices designed in accordance with the latest edition of the MUTCD. The design of the device will allow for the installation of barricade warning lights. The device shall be stabilized with the use of sandbags or other approved means.

Retroreflective Sheeting, in accordance with M.18.09, shall be used on traffic drums. Only 1 type sheeting shall be used on a drum and all drums furnished on a construction project shall be manufactured with the same type retroreflective sheeting.

Prior to using traffic drums on the Project, the Contractor shall submit to the Engineer a copy of the manufacturer's self-certification that the traffic drums comply with the requirements of the NCHRP Report 350 or the AASHTO MASH for Category 1 Devices.

9.78.03—Construction Methods:

The Contractor shall have, available on the Project, a sufficient number of traffic drums to fulfill all the requirements, as specified in the Contract, to provide adequate traffic control during periods of unforeseen circumstances or emergencies.

Traffic drums shall be designed and installed in accordance with the plans, the MUTCD latest edition, and as directed by the Engineer.

Any traffic drum that is missing, damaged or defaced so that it is not effective, as determined by the Engineer and in accordance with the ATSSA guidelines contained in "Quality Standards for Work Zone Traffic Control Devices," shall be replaced by the Contractor.

When the traffic drums are no longer required on the Project, they shall remain the property of the Contractor.

9.78.04—Method of Measurement: This work will be measured for payment by the number of traffic drums used on the Project.

9.78.05—Basis of Payment: This item will be paid for at the Contract unit price each for "Traffic Drum" used on the Project. Each drum will be paid for once, regardless of the number of times it is used on the Project.

Pay Item	Pay Unit
Traffic Drum	ea.

SECTION 9.79
CONSTRUCTION BARRICADES

9.79.01—Description: Under this item the Contractor shall furnish all construction barricades of the specified type required on the Project to comply with the requirements of NCHRP Report 350 (TL-3), or the AASHTO MASH, and the requirements stated in the item "Maintenance and Protection of Traffic," as shown on the plans and as directed by the Engineer.

9.79.02—Materials: Construction barricades shall consist of the following materials:

The frame shall be of polyvinyl chloride pipe meeting the requirements of ASTM D2241 for PVC 1120 or 1220, SDR 21 (pressure rating 200 psi), ASTM D3034, SDR 35 or an approved equal. All straight members shall be the color white.

Wyes, tees and elbows for joint connections shall be polyvinyl chloride of suitable size and strength for the purpose intended.

Joints shall not be glued and a 3/16 inch nylon rope (or equivalent) shall be threaded loosely through the pipe to keep sections from flying if hit by a vehicle.

Face panels used as horizontal members shall be constructed of a suitable plastic material, 0.060-inch high impact styrene, anodized aluminum of no less than 0.025 inch thickness or a comparable substitute approved by the Engineer.

All hardware shall be in accordance with standard commercial specifications and shall be approved by the Engineer.

Alternate stripes of white and orange retroreflective sheeting shall be applied to the horizontal members as shown on the plans. Only 1 type sheeting shall be used on a barricade and all barricades on a construction project shall be constructed with the same type of retroreflective sheeting. Retroreflective sheeting shall meet the requirements of M.18.09.

Construction barricades shall be designed and fabricated so as to prevent them from being blown over or displaced by wind. Construction barricades shall be approved by the Engineer before they are placed into service.

Materials Certificates shall be required confirming compliance with the requirements set forth in the plans and specifications for these barricades.

Prior to using barricades on the Project, the Contractor shall submit to the Engineer a copy of the Letter of Acceptance issued by the FHWA to the manufacturer documenting that the barricades comply with the requirements of NCHRP Report 350 (TL-3) or the AASHTO MASH for Category 2 Devices.

9.79.03—Construction Methods: The Contractor shall furnish a sufficient number of construction barricades required for the traffic patterns for all operations which are being undertaken concurrently. The barricades shall be constructed in a neat and workmanlike manner to the satisfaction of the Engineer.

Ineffective barricades, as determined by the Engineer and in accordance with the ATSSA guidelines contained in "Quality Standards for Work Zone Traffic Control Devices," shall be replaced by the Contractor at no cost to the State.

Barricades that are no longer required shall be removed from the Project and shall remain the property of the Contractor.

9.79.04—Method of Measurement: This work will be measured for payment by the number of construction barricades used on the Project.

9.79.05—Basis of Payment: This item will be paid for at the Contract unit price each for "Construction Barricade" of the type specified and used on the Project. Each barricade will be paid for once, regardless of the number of times it is used on the Project.

Pay Item	Pay Unit
Construction Barricade (Type)	ea.

SECTION 9.80
CONSTRUCTION SURVEYING

9.80.01—Description
9.80.02—Materials
9.80.03—Construction Methods
9.80.04—Method of Measurement
9.80.05—Basis of Payment

9.80.01—Description: Work under this item shall consist of furnishing labor, equipment, tools and materials to perform surveying, staking, verification, recording of data and calculations as necessary to construct the Project, from existing layout to acceptance of the work according to the plans. Work under this item shall conform to Section 20-300b-1 to 20-300b-20 inclusive of the Department of Consumer Protection, Regulations of CT State Agencies and as supplemented herein.

9.80.02—Materials: Stakes used for control staking shall be a minimum of 1 inch × 1 inch wide and a minimum length of 36 inches. Stakes shall be legibly marked and shall be visible from the edge of the travelway. The stakes shall be durable enough to last for the duration of the Contract. In areas where traditional staking cannot be established, the Contractor may use other materials or methods to mark critical locations, as approved or directed by the Engineer.

9.80.03—Construction Methods:

I Submittals:

1. The Contractor shall provide technically qualified survey crews experienced in construction surveying. All Project surveying and staking shall be performed by or under the supervision of either a Connecticut Licensed Land Surveyor or a Level III Survey Technician certified by the National Society of Professional Surveyors.

The name, authority, relevant experience, and qualifications of the person with overall responsibility for construction surveying and staking shall be submitted to the Engineer ten (10) days prior to any physical work.

2. If using Automated Machine Guidance (AMG) methods, the following information shall also be submitted to the Engineer ten (10) days prior to any physical work:

A. A written technology statement that includes:
   i. The manufacturer, model, and software version of the AMG equipment.
   ii. Verification that the final 3D data which is provided in the Plans is compatible with the AMG equipment.

B. Personnel qualifications:
   i. The name, authority, relevant experience, and qualifications of the person with overall responsibility for the AMG system.
   ii. The name, authority, and relevant experience of personnel directly responsible for operating the AMG equipment.

C. A Quality Control Plan for mechanical calibration and maintenance of both surveying and AMG controlled construction equipment. Include the frequency and types of checks performed.

II Equipment Requirements:

1. The Contractor's survey instruments and supporting equipment shall be capable of achieving the specified tolerances in Table 9.80-1.

2. All instrumentation used on the Project shall have been serviced and calibrated within six (6) months prior to use on the Project, and then every year thereafter.

3. The Contractor shall obtain the Engineer's concurrence prior to using construction equipment equipped with Global Navigation Satellite System (GNSS) or Robotic Total Station (RTS) controlled by an AMG system in the construction of subgrade, subbase and base course aggregate courses, or other construction operations.

4. Tools and supplies shall be of the type and quality suitable for survey work.

5. Stakes and hubs shall be of a sufficient length to provide a solid set in the ground, with sufficient surface area above ground for necessary legible and durable markings.

III General Requirements:

1. The Contractor's Construction Schedule shall include dates and sequences of major surveying activities in accordance with 1.05.08.

2. The Department will furnish the initial horizontal control points, vertical control points and data for

use in establishing control for completion of the work. The Contractor shall recover and preserve the initial reference and control points and shall notify the Engineer of missing control points.

3. The Department will furnish data relating to horizontal and vertical alignments, theoretical slope staking catch points, and other design data. The Contractor is responsible for reformatting and any additional calculations that may be required for the convenient use of the State-furnished data. The Contractor shall provide immediate notification of apparent errors or omissions in the initial staking or in the State-furnished data.

4. The Contractor shall provide survey data and measurements in the format(s) acceptable to the Engineer and submit on a schedule determined by the Engineer. Field data and supporting documentation will become the property of the Department upon completion of the work.

5. Prior to major surveying activities, a survey coordination meeting shall be held, and the following agenda items shall be discussed and coordinated with the Engineer:
   A. Surveying and staking methods;
   B. Stake marking;
   C. Grade control for courses of material;
   D. Referencing;
   E. Structure control;
   F. Field staking data;
   G. Localization of the GNSS systems to the Department-established control points;
   H. Protection of existing survey markers; and
   I. Other procedures and controls necessary for the work.

6. The Contractor shall not start the physical work until the required survey or three-dimensional (3D) verification data for the affected work has been reviewed by the Engineer. Review of the construction survey does not relieve the Contractor of responsibility for correcting errors and omissions discovered during the work and for bearing additional costs associated with the error or omission.

7. The Contractor shall maintain legibility of survey markings for the duration of the Project or until notified by the Engineer.

8. Upon completion of the Project, the Contractor shall remove and dispose of all staking material used on the Project.

9. Should the establishment or re-establishment of property acquisition lines, highway lines, or non-access lines be required, the Contractor shall notify the Engineer at least two (2) weeks in advance of need.

10. The Contractor shall provide and maintain safe facilities for convenient access by Department forces to all survey stakes, control points, batter boards, and references.

IV Specific Requirements:

1. Control points: The Contractor shall
   A. Relocate initial horizontal and vertical control points in conflict with construction to areas that will not be disturbed by construction operations.
   B. Furnish the coordinates, elevations, and support documentation for the relocated points before the initial points are disturbed.
   C. Set durable markers for survey control that uniquely identifies the points.
   D. Furnish the GNSS localization results at least seven (7) days before beginning construction layout survey work. If necessary, the GNSS localization calibration and associated 3D model shall be broken into two or more zones to maintain the localized relationship between control points and original ground.

2. Centerline establishment: The Contractor shall establish or reestablish centerline at roadway design cross-section locations as necessary to construct the work.

3. Original ground topographic verification: In areas where the plan existing ground elevation and the actual ground elevation are not within a tolerance of ±0.25 feet, the Contractor shall immediately notify the Engineer.

4. Horizontal Slope Limits and Reference Stakes: The Contractor shall
   A. At a minimum, set stakes on both sides of centerline at the horizontal slope limit at cross-section intervals.
   B. When the slope is designed with a roll at the top and toe, two stakes shall be set on each side of the roadway, one to mark the intersection of the normal cut or fill with existing ground and the other to determine the limit of the roll.

5. **Clearing and Grubbing Limits:** The Contractor shall set clearing and grubbing limits on both sides of centerline.
6. **Finish-grade stakes:** The Contractor shall
   A. Set finish-grade stakes for grade elevations and horizontal alignment, on centerline and on each shoulder at design roadway cross-section intervals.
   B. Reset finish-grade stakes as many times as necessary for construction of the roadway.
   C. When the centerline curve radius is less than or equal to 250 feet, use a maximum spacing between stakes of 25 feet.
   D. When the centerline curve radius is greater than 250 feet, use a maximum spacing between stakes of 50 feet.
7. **Structures:** The Contractor shall provide survey and staking data in accordance with the above requirements for Structures as follows:
   A. **Culverts:** Verify and set culvert locations at the inlet, outlet, and inlet basin points according to the plans. If the proposed culvert design does not fit field conditions, notify the Engineer and provide the following:
      i. Surveyed ground profile along the culvert centerline;
      ii. Slope catch points at the inlet and outlet.
   B. **Bridges:** Set adequate horizontal, vertical, reference and Working Points for bridge substructure and superstructure components. Field verify the girders, bridge chord, bridge tangent, or control lines are as specified on the bridge plans. Also establish and reference the centerline of each pier, bent, and abutment.

      The Contractor shall establish the center line of bearings for all bridge abutments and piers, by setting offset hubs or reference points, so located and protected to ensure they remain undisturbed until such time as they are no longer needed. The Contractor shall mark the location of anchor bolts to be installed, establish the elevation of bearing surfaces and check bearing plates to ensure installation at their proper elevation. Before the erection of structural steel or concrete beams the Contractor shall verify the locations, both vertically and horizontally, of all bearings and the distances between associated bearings.

      The Contractor shall be responsible for conducting all surveys to verify the structural steel profile and alignment are as specified. The Contractor must submit survey and verification in a form acceptable to the Engineer a minimum of 7 days prior to installing the falsework and forms.
   C. **Retaining walls and Reinforced Soil Slopes:** The Contractor shall set adequate horizontal, vertical, reference and Working Points to perform the work.
8. **Borrow and Waste sites:** The Contractor shall
   A. Perform field work necessary for initial layout and measurement of borrow or waste sites.
   B. Establish site limits and clearing limits.
   C. Measure both original and final ground conditions and submit cross-sections as directed by the Engineer.
9. **Utility Relocations:** The Contractor shall provide additional reference stakes to assist the Engineer and public utility personnel to accurately identify the proposed locations for utility facilities to be relocated. At least 2 weeks prior to the scheduled relocation of public utilities, the Contractor shall stake out the following features throughout the limits of utility relocations at a maximum spacing of 25 feet, unless directed otherwise by the Engineer:
   A. Edge of road on the side adjacent to the proposed utility relocations.
   B. Both edges of sidewalks, where shown on the plans.
   C. Proposed drainage location(s) and invert elevation(s) at proposed utility locations.
   D. Finished grade where existing utility facilities will be reset or relocated.
10. **Regulated Areas:** The Contractor shall install and maintain reference stakes at 25 foot spacing, or as directed by the Engineer, along the permitted permanent or temporary regulated impacted areas depicted in the permit applications. Each stake shall be legibly marked identifying the baseline station and offset, and the feature it represents.
11. **Pavement Markings:** Prior to any resurfacing or obliteration of existing pavement markings, the Contractor and a representative of the Engineer shall establish and document pavement marking control points from the existing markings within the limits of the proposed pavement markings or pavement marking grooves. These control points shall be used to reestablish the positions of the lanes, the beginnings and endings of tapers, channelization lines for on- and off-ramps, lane-use arrows, stop

bars, driveways, private drives, road entrances, and any lane transitions in the Project area, including all line striping grooving. The Contractor shall use these control points to provide appropriate pre-marking prior to the installation of final markings, including grooves.

The Contractor shall provide and maintain reference stakes or markings immediately off the edge of pavement, at 100 foot intervals and at any point where there is a change in pavement markings. If the Contractor proposes an alternative method to establish and document pavement marking control points, it must be approved by the Engineer.

For roadways where the existing pavement markings need to be reestablished or pavement marking grooves are to be installed on non-limited access roadways, the markings shall be adjusted as directed by the Engineer. These adjustments are to provide wider shoulders to accommodate pedestrian and bicycle traffic while maintaining through travel lane widths of no less than 11 feet.

Unless otherwise noted in the Project documents, lane and shoulder widths for commonly encountered half sections shall be established as shown in the table below:

Centerline to curb or edge of road	**Lane width**	**Shoulder width**
12 to 16 feet	11 feet	Remaining Pavement
17 to 20 feet	12 feet	Remaining Pavement

For Projects that only consist of removal and replacement of pavement markings, the requirement for a licensed land surveyor to supervise the staking is waived.

12. **Miscellaneous survey and staking:** The Contractor shall survey and stake other work such as guiderail, curb and gutter, turf establishment, regulated areas, watercourses and excavation limits for structures. When staking increments are not specified, the Contractor shall propose increments for the Engineer's review. The Contractor shall maintain or replace these stakes until the Engineer approves their removal.

Table 9.80-1
Construction Survey Staking Tolerances[1]

Staking Phase	**Horizontal**	**Vertical**
Control points set from existing control points.[2]	±0.03 feet	±0.01 feet × $\sqrt{N}$
Centerline points including all points of curvature and references.	±0.06 feet	±0.03 feet
Slope-stake and slope-stake references.[3]	±0.25 feet	±0.25 feet
Culverts, ditches, and minor drainage structures stakes.	±0.25 feet	±0.06 feet
Retaining walls stakes.	±0.06 feet	±0.03 feet
Bridge substructures and superstructure stakes.[4]	±0.03 feet	±0.03 feet
Pavement markings stakes. [5]	±0.50 feet	N/A
Curb and gutter stakes.	±0.06 feet	±0.03 feet
Working Points. [4]	±0.03 feet	N/A
Clearing and grubbing limit stakes.	±1.00 feet	N/A
Roadway subgrade finish stakes.	±0.16 feet	±0.03 feet
Roadway finish grade stakes.	±0.16 feet	±0.03 feet

1 At statistical 95% confidence level. Tolerances are relative to existing control points.

2 N is the number of instrument setups.

3 Take the cross-sections normal to the centerline ±1 degree.

4 Bridge control is established as a local network and the tolerances are relative to that network.

5 This tolerance also applies to alternative methods of establishing and documenting pavement marking control points from the existing markings, such as GPS recording.

9.80.04—Method of Measurement: Construction Surveying, being paid on a lump sum basis, will not be measured for payment. Prior to beginning the work, the Contractor shall submit a proposed schedule of values for review and concurrence by the Engineer.

9.80.05—Basis of Payment: Construction Surveying will be paid for at the Contract lump sum price for "Construction Surveying," based on completed portions of the work. This price shall include all labor, submittals, maintenance, materials, tools, equipment, removal of materials and all work incidental thereto.

Pay Item	Pay Unit
Construction Surveying	l.s.

SECTION 9.81
42 INCH TRAFFIC CONE

9.81.01—Description: This item shall consist of furnishing 42-inch retroreflective traffic cones required on the Project to meet the requirements of the traffic control plans, as stated in the item "Maintenance and Protection of Traffic," as shown on the plans or as directed by the Engineer.

The Contractor shall have available on the Project a sufficient number of traffic cones to fulfill all the requirements as specified in the Contract and to replace those which have become damaged.

9.81.02—Materials: The traffic cone shall be manufactured of 2-piece construction - cone and stabilizer base. The cone shall be constructed of impact-resistant orange plastic or rubber of a thickness able to withstand impact without damage to cones or vehicles. The bottom of the cone shall be 8 1/2 inch conical diameter tapering to the top of the cone which shall be 3 1/2 inch conical diameter. The design of the device will allow for the installation of a weighted stabilizer base. The stabilizer base shall not be round in shape. It shall have a hole in the middle to allow for quick placement over the cone. The base shall be constructed of impact-resistant black plastic or rubber ballasted to 18 pounds.

Retroreflective stripes shall be fabricated from retroreflective sheeting. All stripes shall be of one type of sheeting. Retroreflective sheeting shall be as specified in M.18.09.

Prior to using traffic cones on the Project, the Contractor shall submit to the Engineer a copy of the manufacturer's self-certification that the traffic cones comply with the requirements of NCHRP Report 350 or the AASHTO MASH for Category 1 Devices.

9.81.03—Construction Methods: The stabilizer base shall be attached to the traffic cone in accordance with the manufacturer's instructions. The Contractor shall ensure that the devices are kept clean and bright.

9.81.04—Method of Measurement: This item will be measured for payment by the number of traffic cones used on the Project.

9.81.05—Basis of Payment: This item will be paid for at the Contract unit price for "42-inch Traffic Cone" used on the Project. Each cone will be paid for once, regardless of the number of times it is used on the Project.

Any traffic cones that are missing, damaged or defaced so that they are not effective, as determined by the Engineer, and in accordance with the ATSSA guidelines contained in "Quality Standards for Work Zone Traffic Control Devices," shall be replaced by the Contractor at no cost to the State.

When the traffic cones are no longer required on the Project, they shall remain the property of the Contractor.

Pay Item	Pay Unit
42-Inch Traffic Cone	ea.

SECTION 9.99
DISPOSAL OF BUILDINGS

9.99.01—Description: Under this item, the Contractor shall, if ordered by the Engineer, dispose of any or all buildings within the right of way or slope limits, or both, as shown on the plans and on the supplemental sheet entitled "Disposal of Buildings," which is made a part of the proposal. Upon written order of the Engineer to dispose of any building, said building shall become the property of the Contractor; and it shall, at its option, demolish, remove or otherwise dispose of said building. Upon receipt of said notice, the Contractor shall accept title and ownership of such buildings as listed thereon, also all risk of loss and any and all liability in connection therewith, and shall not rent or otherwise use the buildings without written permission of the Engineer.

The Commissioner reserves the right to delete from the Contract the removal of any of the buildings listed in the Contract.

The Contractor's attention is called to the fact that buildings shown on the plans within the construction limits, and not included under this item, have been or will be removed by others at or about the time of beginning of construction work.

The State does not engage to protect any buildings against damage, in any form including loss of fixtures or equipment, or vandalism in the period between the bidder's inspection of such building and the time such building is formally released to him as described herein. The Contractor shall take this into account in placing their bid.

9.99.03—Construction Methods: The Contractor shall dispose of the buildings and, unless otherwise ordered by the Engineer, remove from the Site all material resulting therefrom. The Contractor shall remove the structure to the top of the foundation elevation and shall break up any paved floors.

Upon receipt of the aforementioned written notice or notices, the Contractor shall as part of this item indemnify and save the State harmless for damages which might occur before, during or after the removal of the buildings and shall provide such shoring, bracing, barricades, fencing and other devices as may be necessary to protect adjacent properties and the public.

The Contractor shall arrange for the discontinuance or removal or both of all utility services at its own expense, unless otherwise provided.

Should the Contractor intend to move a building or have it moved, it shall advise the Engineer in writing at least 10 days in advance of the move of the proposed method of operation and proposed location of the building.

Should the Contractor's progress be delayed for any reason, due to its decision to relocate or otherwise dispose of any structures, no claims for additional compensation or adjustment of time will be allowed by the Commissioner.

9.99.04—Method of Measurement: The Contractor shall submit a lump sum bid price for the disposal of all buildings on the Project as shown on the supplemental sheet entitled "Item 999001A—Disposal of Buildings," attached to the Proposal and in accordance with the instructions shown on the sheet "Bidding Procedure" included with the Proposal. This lump sum shall be the total of the amounts shown by the Contractor on the above-mentioned list for each individual unit. This lump sum total shall be the difference between the "Plus (+)" and "Minus (-)" columns shown on the Supplemental Sheet entitled "Item #999001A—Disposal of Buildings." Should any unit so listed to be disposed of be deleted for any reason, the lump sum bid will be adjusted by the amount bid for the unit as shown on the supplemental sheet attached to the proposal.

9.99.05—Basis of Payment: This work will be paid for by the State or paid for by the Contractor at the Contract lump sum for the "Disposal of Buildings" as adjusted in accordance with the provisions of the above paragraph, which price shall include all materials, equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
Disposal of Buildings	l.s.

TRAFFIC CONTROL

SECTION 10.00
GENERAL CLAUSES FOR HIGHWAY ILLUMINATION AND TRAFFIC SIGNAL PROJECTS

10.00.01—General
10.00.02—Inspection of Work and Relief of Maintenance Responsibility for Highway Illumination Work
10.00.03—Plans
10.00.04—Materials and Equipment
10.00.05—Regulations, Permits, Fees
10.00.06—Protection of Equipment and Work
10.00.07—Maintenance of Completed and Existing Construction
10.00.08—Prosecution of Work
10.00.09—Removal of Rubbish and Cleaning Up
10.00.10—Tests: Preliminary and Final
10.00.11—Excavation and Miscellaneous Work
10.00.12—Negotiations with Utility Company
10.00.13—Service Installations
10.00.14—Maintenance of Illumination During Construction

10.00.01—General: It is the intent of the plans, specifications, and special provisions to provide for a complete lighting system or traffic control signal installation, as the case may be, operating as designed and specified in the Contract.

Where not specifically covered on plans, in specifications or special provisions, equipment shall be installed according to manufacturer's published recommendations.

10.00.02—Inspection of Work and Relief of Maintenance Responsibility for Highway Illumination Work: Upon completion of all work, the lighting system will be energized and a semi-final inspection held. The Engineer will submit an inventory of all unacceptable work. A 30-day test period will begin the day of the inspection. The lighting will be monitored, and any malfunctions or outages shall be attended to and corrected by the Contractor within 48 hours and to the satisfaction of the Engineer. Failure to comply or properly attend to reported problems will result in the test period being restarted. At the discretion of the Engineer, repetitive or recurring malfunctions may cause the test period to restart. After the successful completion of the 30-day test period, and if all corrective and clean-up work has been completed, a final inspection will be held in accordance with 1.08.12. The Contractor may then request relief of maintenance responsibility for the lighting system. Partial acceptance may be made in accordance with 1.08.12.

10.00.03—Plans: The Contractor shall advise the Engineer of any change of measurement or layout from the plans. The Contractor will be furnished 2 black and white prints of the plans on which to record all changes. These are to be delivered to the Engineer upon completion of the Project. Also upon completion of the Project, the Contractor shall deliver to the Engineer the following:

1. Four (4) paper prints of schematics and wiring diagrams of all cabinets, controllers and auxiliary equipment in accordance with the following requirements:

- **a)** Each separate wiring diagram shall list the Town and Location information in the lower right hand corner.
- **b)** Each and every cabinet wiring diagram copy must show the as built field wiring. No cross-outs, or separate lists will be accepted.
- **c)** Field hookups (signals, detector and coordination, etc.) must be shown on either the cabinet wiring diagram (face # G,Y,R, etc.) or separate hookup sheet.
- **d)** Auxiliary equipment drawings must be completely labeled (connection to connection). Auxiliary relays, coordination units and time switches, etc. must be accompanied with all necessary information such as manufacturer, voltage ratings and type of mounting base, etc.
- **e)** Special information such as controller start-up, controller overlap programming, controller sequence, PROM numbers, and conflict monitor programming, etc. must be plainly labeled on the cabinet wiring sheet.

2. A comprehensive service manual for each type controller, conflict monitor, detector amplifier,

coordination equipment, pre-emption equipment and all other auxiliary equipment furnished.

3. A comprehensive parts list detailing all replaceable components as to manufacturer's part number and commercially available part number and manufacturer's net price each. This list may be referenced from the drawings supplied with the equipment.

The Contractor shall study plans and details and use them as a guide in determining location of illumination and traffic signal equipment. Any discrepancies shall be referred to the Engineer for settlement.

10.00.04—Materials and Equipment: All electrical equipment shall meet the requirements of the plans and specifications and shall be in accordance with 1.06 Control of Materials.

The warranties that the Contractor receives from each manufacturer of materials and equipment pertinent to the complete and satisfactory operation of the proposed system shall be turned over to the Engineer at the time of acceptance of the Project at no cost to the State. Each warranty so furnished shall indicate its expiration date.

10.00.05—Regulations, Permits, Fees: All work shall be done in strict accordance with the latest edition of the NEC, rules and regulations of the State authorities having jurisdiction over such work, and regulations of the utility companies in force where work is being installed. On traffic signal projects, The MUTCD for Streets and Highways shall also govern. All permits shall be obtained and fees paid for by the Contractor.

10.00.06—Protection of Equipment and Work: The Contractor shall adequately protect all equipment, material, tools and work on the premises against theft, destruction by weather or other causes, and shall assume full responsibility for such protection until final acceptance of work by the Commissioner. All material shall be placed or stored in such location so that it will not be a hazard to the traveling public.

The Contractor shall provide adequate protection for its own forces and that of the public in the various locations and phases of the work. Barricades or covers shall be used where deemed necessary to promote safety.

10.00.07—Vacant

10.00.08—Prosecution of Work: Prior to starting work, an "on-Site" meeting shall be held between the Engineer, Contractor and other concerned parties. The purpose of this meeting shall be to coordinate work, review available facilities for work and storage areas, and prepare a preliminary schedule of the work.

Upon receipt of Notice to Proceed, the Contractor shall inform the Department at the District Office of the time at which he will begin actual Contract work; and he shall do no work thereafter without knowledge of the Department.

All work shall be done by qualified and experienced mechanics of each labor class, as determined by the Engineer. All work shall be inspected and approved by the Engineer before concealment.

The Contractor will not be allowed to install traffic signals or pedestrian heads until the controllers are on hand and ready for installation. Once installation of this equipment commences, the Contractor shall complete this work in a most expeditious manner. At locations where existing steel span poles are utilized in the design, the Contractor shall remove the old signals before installing the new signals or install temporary anchors on the poles in order to prevent overloading.

10.00.09—Removal of Rubbish and Cleaning Up: The Contractor shall at all times keep the Site free from accumulation of waste materials or rubbish caused by his employees or work; and at the completion of work shall dispose of all such rubbish and non-usable fill or rock from the site; and remove all his tools, equipment, and surplus materials. The Contractor shall leave the Site and the work in a clean and orderly condition. Upon completion of the work, the Contractor shall clean all equipment of grease, dirt, etc.

10.00.10—Tests: Preliminary and Final

1. Highway Illumination Projects: The Contractor shall perform all operational tests, in the presence of the Engineer. The Contractor at the Contractor’s expense shall supply operational tests and testing equipment.

Upon request, the Contractor shall demonstrate that all ground rods shall have a resistance to earth of not more than 25 ohms. Also, upon request, the Contractor shall perform insulation testing which shall be a minimum of 1000 ohms per volt.

2. Traffic Signal Projects: The Contractor shall arrange for and provide all the necessary field tests, as directed by the Engineer, to demonstrate that the installation is in proper working order and in accordance

with the plans and specifications.

All tests and test equipment shall be supplied at the Contractor's expense.

All acceptance testing of Contractor furnished and installed hardware and software shall be conducted as described below. Tests that apply to the operation of the computer will not be required at intersections that are not under computer control.

Prior to acceptance of the work, the Contractor in the presence of the Engineer shall make the following tests.

a) Detector Acceptance Test: Detector acceptance tests shall be conducted for all system and local detectors. The Contractor shall perform the tests and document the test results on a detector test chart that will provide the following information:

1. **Megger Test:** A 600-volt megger test shall be performed between each circuit and ground for each loop lead-in cable circuit. The cable shall maintain a resistance to ground of not less than 10 mega-ohms. Actual measured resistance shall be recorded.
2. **Loop Circuit Test:** Each loop and lead-in circuit shall be tested for continuity, resistance, and inductance. Resistance shall not exceed four ohms. Total inductance of segmented loops shall be between 70 and 400 micro-Henries. Actual measured resistance and inductance shall be recorded. The detector acceptance tests for all local actuation detectors installed at the intersection shall have been successfully completed prior to initiating the intersection acceptance test.
3. **Power Interruption Test:** Each detector shall be tested for power interruptions to assure that the sensor unit automatically re-tunes each channel when power is restored. The results of this test shall be recorded.

The calculated inductance is the sum of the loop (or segmented arrangement) and the lead-in cable.

Resistance to ground and loop circuit resistance shall be measured at the controller cabinet.

Loop circuit inductance shall be measured at the handhole where the loop or series spliced segmented arrangement is to be spliced to the lead in cable.

When the tests are completed, whether successful or not, the test results documentation shall be furnished to the Engineer.

b) Intersection Acceptance Test: An intersection acceptance test shall be conducted and successfully completed prior to acceptance of each intersection.

The test is designed to demonstrate that the field equipment installed at each intersection is installed properly and that all functions are in accordance with the plans and specifications. The Department reserves the right to make adjustments to the timing of the controllers during and after test periods. These timing adjustments shall not relieve the Contractor of any responsibility otherwise set forth in the Contract.

1. **Visual Final Inspection:** All intersection-related construction such as controller, local actuation detectors, all cabinet accessories, and all cabinet wiring shall be complete and in-place. This shall include space for, and the wiring harness for the Closed Loop Master Unit (CLMU) and the Closed Loop Local Coordination Unit (CLLCU). It is not required that the CLMU and the CLLCU be installed or that the closed loop system be operational for the test.

   All signal display hardware including, but not limited to, support structures, signal heads, pedestrian push-button, conduit, junction boxes, etc., shall be complete and in-place. All intersection restoration work including items such as sidewalks, streets, curbs, gutters, and grassed areas, shall be completed, unless the Engineer grants an exception. All Signs and Markings specified shall be installed prior to placing the traffic signal in operation unless the Engineer grants an exception. Only when the visual intersection inspection is acceptable to the Engineer shall the second part of the test commence.
2. **Preliminary Functional Test:** At locations where an existing traffic signal installation is to be revised or replaced, a preliminary functional test shall be conducted to allow the Contractor to transfer control of the intersection from the existing traffic control equipment to the new equipment. The Office of Traffic and the District Electrical Maintenance Office must be contacted when a preliminary functional test is scheduled. An engineer from the Office of Traffic does not have to be present during the preliminary test unless requested by the Engineer. However, a qualified representative of the traffic controller manufacturer or distributor, must be present to correct any technical malfunction that may occur. At that time the existing signal may be taken out of operation and removed. At no time shall the signal be

left unattended in automatic operation unless authorized by an engineer from the Office of Traffic.

If the new signal is not operating properly, the old controller shall continue to operate the signal and the Contractor, at his own expense, shall make all necessary repairs, adjustments, changes or replacements promptly and to the satisfaction of the Engineer.

When all work is completed and the signal installation is operating properly, the new signal may be left on automatic operation and the existing signal may be taken out of operation and removed.

At locations where there is no existing traffic signal, the above preliminary functional test does not apply. The Engineer shall verify that the new traffic signal equipment is operating according to plan prior to scheduling the functional inspection.

3. **Functional Inspection:** Upon completion of the installation and the above tests, the Engineer shall notify the Office of Traffic and the District Electrical Maintenance Office that the installation is complete and shall arrange a time for an engineer from the Office of Traffic and a representative from the District Electrical Maintenance Office to conduct the Functional Inspection and complete the Intersection Acceptance Test of the traffic signal. A qualified representative of either the traffic controller manufacturer or the distributor must also be present during the Functional Inspection. A punch list of traffic signal items shall be provided to the Contractor as a result of the Functional Inspection.

   A 30-day test shall start at the successful completion of the Functional Inspection. All electronic and electromechanical equipment, including but not limited to the controller, conflict monitor, detector amplifiers, load switches and flasher, shall be tested for proper operation for 30 consecutive days. During this testing period, all equipment shall operate without failure of any type. The test shall begin anew each time a failure is identified. The 30-day working test period shall not start until an engineer from the Office of Traffic and a representative from the District Electrical Maintenance Office has inspected the installation. The Contractor shall be responsible for all equipment installed until the 30-day test is terminated and all punch-list items identified on the Visual Final Inspection and the Functional Inspection have been addressed and resolved. The 30-day working test period shall not be considered complete until final sets of cabinet wiring diagrams have been received by the Municipality or the Department of Transportation, Office of Maintenance, for each intersection under test. The 30-day working test period will not apply to any equipment furnished by the State and installed by the Contractor.

   The Engineer may adjust any timing during this period to fully test the functional operation of the equipment installed. If any failures are identified, the Contractor shall replace or repair the defective equipment within 24 hours of notification by the Engineer.

   All necessary corrections and adjustments shall be made promptly by the Contractor so as to make the installation satisfactory to the Engineer and at no additional cost to the State.

   When both parts of the Intersection Acceptance Test are successfully completed and the 30-day test period has passed, the intersection shall be accepted by the Engineer. It may be possible that all of the traffic signal controllers in the system have completed their 30-day functional test period before the traffic signal interconnect system has been completed.

   The Engineer shall issue an acceptance letter to the Contractor, or permitter if the traffic signal has been installed by permit, stating the 30-day test start and completion dates and relieving the Contractor from maintenance responsibility of the traffic controller. All traffic signal punch list items, identified at the Functional Inspection, shall be resolved prior to issuance of the acceptance letter. The party who assumes ownership shall also receive a copy of the acceptance letter. Completion of the 30-day test period relieves the Contractor of normal maintenance responsibility including accidental damage or vandalism. The Contractor shall repair or replace any equipment found to be defective or damaged due to poor workmanship or the Contractor's operations.

c) CLOSED MASTER UNIT (CLMU) AND CLOSED LOOP LOCAL COORDINATION UNIT (CLLCU) ACCEPTANCE TEST: The CLMU and CLLCU shall be tested before the communications test and prior to performing the final acceptance test. The test shall be performed by installing the CLMU and the CLLCU in the controller cabinet and inhibiting all outputs to the controller. Units failing to successfully complete the test will be rejected. The Contractor shall

submit a test plan of procedures required to test all major functions of the coordination equipment. All tests shall be performed and documented by the Contractor and copies of the test results furnished to the Engineer.

d) **Final Acceptance Test:** The final acceptance test shall consist of two sequential periods: **Intersection Pickup** and **Evaluation and Analysis.** When the requirements of each of the two periods have been met, the system shall be accepted. The requirements for each of the two periods are described below.

The Contractor shall make, and fully document, all modifications made to correct operations that are not in accordance with the special provisions.

The Engineer reserves the right to halt the test at any time, if in his sole judgment, errors or inconsistencies in the performance of any part of the system would lead to improper, inefficient, or unsafe operation of the traffic signal.

1. **Intersection Pickup:** Intersection pickup will consist of performing actual on-line functions between the local coordination equipment and CTDOT's Highway Operations Center in Newington. After the Intersection Acceptance Test has been performed at each location, the Engineer shall notify the Computer Systems Unit to schedule an Intersection Pickup test of the closed loop or UTCS system. The Contractor, in the presence of the Highway Operations Systems Engineer, shall perform and document the actual pickup testing of all functions associated with each intersection, in accordance with the timing plans.

   It shall be the responsibility of the Contractor to correct any equipment malfunctions that caused a failure. The intersection pickup period shall continue until all intersections in the system have been completed.

2. **Evaluation and Analysis Period:** The evaluation and analysis period shall commence when all intersections have been picked up. There shall be at least 7 days between the successful pickup of the final intersection and the beginning of the evaluation and analysis period. The evaluation and analysis period shall be 30 days in length.

   During the evaluation and analysis period, the Engineer will be provided full access to the field equipment in order to evaluate the system for accordance with the Special Provisions. During this period of time, the Contractor will be required to continue maintenance of the signals and may perform any final clean-up, adjustments of the signals, etc., that are necessary and shall cooperate with the Engineer such that evaluation and analysis of the system may continue unimpeded. During this period of evaluation and analysis, the Contractor shall demonstrate any system function as requested.

The Engineer shall advise the Contractor in writing of any portions of the system that do not meet the requirements of the Contract. The Contractor shall promptly make modifications that may be required to bring the system in accordance with the Project requirements. If changes are required to any hardware units that require a retrofit, the 30-day evaluation and analysis period shall begin anew after the hardware changes are completed on all units.

Failure conditions occurring during the evaluation and analysis period that require replacement of a major system element installed by the Contractor, shall cause the count of time for the evaluation and analysis to be discontinued until such time that the corrections have been made. The evaluation and analysis time will commence upon correction of the condition and will continue for the duration of the remaining time of the evaluation and analysis period.

Final acceptance will occur at the successful completion of the 30-day evaluation and analysis period.

10.00.11—Excavation and Miscellaneous Work: All cuts in pavement, sidewalk surfaces, and concrete aprons shall be done in a neat and workmanlike manner, so as to cause the least possible damage. Any property damage caused by excavation shall be repaired as directed by and to the satisfaction of the Engineer at no additional compensation. Excavating shall not be performed until immediately before installation of conduit and other appurtenances. The material from the excavation shall be placed where directed by the Engineer at a location where the least damage and obstruction to vehicular and pedestrian traffic will occur and the least interference with surface drainage.

All excavated material shall be removed and disposed of by the Contractor in accordance with the provisions of 2.02. Excavations, after backfilling, shall be kept well filled and maintained in a smooth and well-drained condition until permanent repairs are made.

At the end of each day's work and at all other times when construction operations are suspended, all

equipment and other obstructions shall be removed from that portion of the roadway open for use by public traffic.

All excavation shall be closed, and sidewalks, pavements and landscaping restored at each intersection prior to opening any other intersection, unless the Engineer otherwise approves it.

Excavations shall be performed in such a manner that not more than one traffic lane is restricted in either direction at any time unless otherwise directed by the Engineer.

Where excavations occur in sidewalks or other pedestrian ways, provisions shall be made to provide a safe orderly pedestrian passage around the excavation area, the use of which shall not subject the pedestrian to hazard from traffic or construction operations or to walk upon unsuitable and hazardous surfaces.

All cutting and patching shall be held to a minimum. Necessary cutting and patching shall be neatly done and in all patching, special care shall be given to water sealing and bonding. On exposed work, patching shall match adjacent areas. In sidewalk areas where foundations, handholes or conduit is required to be installed, the concrete sidewalk shall be sawcut to neat lines no larger than necessary for the installation of the foundation, handhole or conduit. The complete square of sidewalk shall be removed only if shown on the plans or directed by the Engineer.

Upon completion of excavation, the Contractor shall final grade, seed and fertilize in accordance with 9.50 all landscape grass areas disturbed by his construction.

Before the Contractor installs any cable, all conduit and junction boxes shall be cleaned and swabbed to allow cables to be freely installed. Pressure-sensitive vehicle detectors shall be cleaned and checked to see if in working condition. Foundations shall be checked for bolt circle dimensions where required.

All poles and pedestals shall be bonded to the conduit ends by means of a jumper running from bushings on the conduit ends to a grounding lug on the pole or pedestal. At the controller locations the grounding circuit shall be carried from the ground bushing on the conduit up into the controller cabinet. All conduit within junction boxes shall be bonded together.

All dimensions shall be verified on Site, actual always taking precedence over scale dimensions, with every part of the plans fitted to actual conditions at the Site.

Warning Markings for Underground Facilities shall be installed in accordance with 1.05.15.

10.00.12—Negotiations with Utility Company: The Contractor shall be responsible for all negotiations with and between the utility company.

The Contractor shall assume all charges and make all necessary arrangements with the power company for the required electrical services necessary for the energizing of the highway illumination and traffic signal installations, for the installation of wood poles, for signal messenger or span wire which runs between or to utility company poles and for anchors and anchor guys necessary on wood poles. The Contractor shall comply with the utility company regulations. The utility company will connect and disconnect the power as required. The Contractor shall pay any charges incurred.

When an entry into a service manhole or attachment to any utility company pole is required, the Contractor shall notify the utility company involved sufficiently in advance; and under no condition shall the Contractor enter any manhole or place an attachment to a utility company pole without a utility company representative present.

10.00.13—Service Installations: Service installations shall be in accordance with the NEC and with the requirements of the local utility company involved.

Service points shown on the plans are approximate only. The Contractor shall determine exact locations from the serving utility.

The Contractor shall arrange with the serving utility to complete service connections.

The Contractor shall arrange for furnishing electrical energy. Energy used prior to completion of the contract will be charged to the Contractor except that the cost of energy used for public benefit, when the Engineer directs such operation, will be borne by the State.

10.00.14—Maintenance of Illumination During Construction: The Contractor shall organize the Project work so that any portion of roadway which has existing roadway illumination and is open for use remains lighted. The Contractor shall also provide illumination on all temporary crossovers, ramps and roadways that are constructed as part of staged construction and that are open for use. Highway illumination may consist of: existing lighting, new lighting, temporary lighting, or any combination thereof. It is the Contractor's responsibility to stage the installation or relocation of service cabinets, poles, lights, and circuitry so that all roadways of the kind described above remain lighted. If it is necessary to install temporary poles, lights, or circuitry to maintain the integrity of the highway illumination system,

such work shall be submitted to the Engineer for approval prior to installation, and will be paid for at the Contract bid unit price for the relevant items. Temporary illumination work not specifically covered by the Contract specifications and pay items will be paid for as extra work at the discretion of the Engineer.

If the Contract includes temporary illumination plans, those plans shall serve as a framework for providing roadway illumination during construction. Temporary illumination plans may not represent the full extent of the temporary illumination work required, or the exact quantity of temporary lights required to maintain proper roadway illumination.

Prior to the start of any work that will interfere with the existing lighting system, the Contractor and CTDOT District Electrical Maintenance personnel shall inspect the system for lighting outages, pole knockdowns, and circuit malfunctions. Deficiencies will be noted and repaired by Department forces prior to the start of work by the Contractor.

Once the Contractor's work interferes with or impacts the existing roadway lighting system, maintenance of that system within the Project limits becomes the Contractor's responsibility. The repair of lighting system malfunctions occurring outside of the project limits, caused by the Contractor's work, shall also be the Contractor's responsibility. District Construction personnel will note the start and end date of the Contractor's responsibility for maintenance of any existing lighting system.

The Contractor shall maintain the illumination throughout the duration of the Project, until the Project is accepted by the State. The Contractor shall supply to the Project Engineer and to the CTDOT District Electrical Maintenance Supervisor, the names and phone numbers of a primary and back-up representative, to be contacted should a problem with the lighting system occur.

Whoever discovers a lighting outage or pole damage/knockdown within the Project limits shall immediately notify CTDOT Highway Operations of same as follows:

1. For projects in Districts 1, 2, and 4, call (860) 594-3447.
2. For projects in District 3 and along the Interstate 95 corridor within District 2, call (203) 696-2690.

The following procedures will be followed for lighting outages:

1. Once notified of a lighting outage, CTDOT Electrical Maintenance personnel will assess the situation, and in the case of a pole knockdown, may clear the pole from the roadway and make safe any exposed wires.
2. The Project Inspector and the Contractor's designated representative shall be notified after the lighting outage has been assessed by CTDOT Electrical Maintenance, transferring responsibility for further repairs to the Contractor.
3. Upon notification, the Contractor shall be responsible to repair the lighting system before the normal nighttime turn-on of the lights. If this cannot be achieved, the Contractor shall make the lighting operational prior to the next normal nighttime turn-on of the lights, up to a maximum of 24 hours from the time the Contractor was notified of the problem. The Contractor shall contact the Project Inspector to discuss the situation, the steps to be taken to bring the lighting back on line, and the time frame for doing so.
4. For isolated individual luminaire outages (not a continuous circuit), the Contractor shall repair such luminaires within 48 hours from the time that the Contractor became aware of the outage.

The Contractor shall follow standard "lock-out," "tag-out," and "Call Before You Dig" procedures when working on the lighting circuit. Both the Contractor and CTDOT Electrical Maintenance shall have mutual access to active lighting control cabinets.

The Contractor will be reimbursed for any costs associated with the maintenance of the existing lighting system that are beyond the Contractor's control. Reimbursements will be for damage caused by the general public and normal system age related component failures (such as lamp burn-out, ballast/starter failure or cable splice failure). However, the Contractor shall be responsible for repair of damage to the existing lighting system incurred as the result of their operations including damage caused by improper wiring methods. All repairs or replacements due to the Contractor's operations shall be made by the Contractor at their expense.

The Project Inspector will maintain a log book of any lighting repair work performed, which will include a description of the repairs, and the date the work was performed. The log book will be made accessible to CTDOT Electrical Maintenance personnel.

Temporary illumination circuitry shall consist of pre-assembled aerial cable of the type and size as indicated in the Contract documents or as directed by the Engineer.

The Contractor shall notify the Engineer when aerial cable cannot be installed due to construction activities and shall suggest another method for installation of the cable.

Alternate options may include installing cable in duct underground, or installing surface-mounted cable in duct or PVC conduit with cable along the backside of a bridge parapet or temporary concrete barrier curbing. Temporary cable in duct/conduit or aerial cable lying directly on the ground will not be allowed. The option of surface-mounting duct or conduit to the backside of a parapet or barrier will be allowed only when construction activities make it necessary, and where the surface-mounted conduit will not expose workers to a high voltage hazard. The Contractor must obtain the Engineer's approval to do so prior to installing temporary circuitry not installed overhead, unless otherwise indicated on the plans.

When temporary circuitry is installed in trench, standard warning tape procedures shall be followed as set forth in 1.05.15. When temporary circuitry is surface mounted to the backside of a parapet or barrier wall, the Contractor shall install warning placards which read: "Live Electricity." Warning placards shall be installed at the beginning, end, and at intermittent points 100 feet (30 meters) apart along the exposed length of the duct/conduit. All temporary lighting circuits shall include a continuous No. 8 bare copper grounding conductor connected to all light standards and effectively grounded as per the NEC.

SECTION 10.01
TRENCHING AND BACKFILLING

10.01.01—Description
10.01.02—Materials
10.01.03—Construction Methods
10.01.04—Method of Measurement
10.01.05—Basis of Payment

10.01.01—Description: Work under this item shall consist of trenching, backfilling and satisfactory clean-up and disposal of all surplus excavated material, the removal of which is necessary for the proper installation of conduit or cable at the locations shown on the plans or as directed by the Engineer.

Rock, insofar as it applies to trenching and backfilling, shall be defined as rock in definite ledge formation, boulders, or portions of boulders, cement masonry structures, concrete structures, reinforced concrete pipe, Portland cement concrete pavement or base, of 1/2 c.y. or more in volume, removed as indicated or directed from within the payment lines for trench excavation.

It shall also include the encasement of conduit or cable in sand, grading and placement of topsoil, fertilizing, seeding, and mulching, and the removal and reconstruction of bituminous, concrete and granite curbing, riprap, crushed stone, processed aggregate base, granular base, suitable backfill material, pavement or bituminous concrete sidewalk structure of all disturbed areas within the maximum trenching limits shown on the plans.

10.01.02—Materials: The materials for this work for the encasement of conduit or cable, shall be bedding material, all of which passes a 3/8-in sieve, and not more than 10% passes a No. 200 sieve.

Topsoil, fertilizer, seed and mulch shall be as specified in M.13. Pavement and sidewalk shall be as specified in M.02, M.03 and M.04. Bituminous Concrete – HMA S0.375 shall be as specified in M.04.01. Processed Aggregate base shall be as specified in M.05.01

10.01.03—Construction Methods: Trenches shall be of the depth and cross-section shown on the plans. The sand for encasement shall be placed as shown on the plans.

After the excavation is completed, the Contractor shall notify the Engineer; and no conduit or cable shall be placed in the excavated area until the Engineer has approved the depth and cross-section of the excavation.

Suitable material removed in making the excavation shall be used for backfill. No stones or coarse material shall be placed adjacent to the conduit or cable. All surplus or unsuitable material shall be removed and disposed of as directed. Should additional material be required for backfilling, it shall be obtained from sources approved by the Engineer.

All backfill shall be placed in layers of not more than 6 inches deep after compaction and shall be thoroughly compacted by means of vibrators or by pneumatic tampers. Hand tampers shall be used only with permission of the Engineer. The backfill shall be brought to the surface of the surrounding ground and neatly graded, except that where excavation is required in existing lawn or grass areas, the backfill shall be brought to within 4 inches of the top of the trench; and the remainder shall be filled with topsoil to 3/4 inch above adjacent areas as directed by the Engineer. Marking tape shall be installed in the trench at the depth and to the requirements set forth in 1.05.15.

Where trenching occurs in riprap or crushed stone areas, the surface material shall be replaced in kind. Where trenching in bituminous concrete sidewalk or paved areas, the trench shall be sawcut and backfilled to within the depth from the surface required to replace the removed sidewalk or pavement structure, which shall then be replaced. Suitable backfill material and granular fill shall be used for sidewalks, and processed aggregate base shall be used as backfill material for pavement or bituminous concrete surfaces. The edges of all trenches in paved surfaces shall be sawcut to neat lines prior to paving. All trenches in existing paved surfaces, which parallel the curb, shall be no more than 1 1/2 feet from the curb, or when no curb is present, the apparent edge of road. The exception shall be to avoid existing appurtenances such as catch basins, water gates, manholes etc.

Where a trench is placed through a concrete sidewalk, the entire section of sidewalk between joints shall be replaced in accordance with 9.21, unless otherwise directed by the Engineer.

10.01.04—Method of Measurement: This work will be measured for payment by the number of linear feet of trenching and backfilling, complete and accepted. If rock meets the description given under 10.01.01, is encountered, the Contractor shall strip it of sufficient overlying material to allow for proper

measurement, and shall notify the Engineer that the rock surface is ready for measurement. If the Contractor fails to give such notice, the Engineer will presume the measurements taken at the time the Engineer first saw the material in question will give the true quantity of rock excavation.

For measurement where rock is encountered in trenching, payment lines shall be vertical and 2 feet in width.

10.01.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Trenching and Backfilling," which price shall include all materials, tools, equipment and labor necessary to complete the excavation in accordance with the plans or as ordered.

It shall also include all sand encasement, suitable backfill material, processed aggregate base, granular fill, backfilling, grading, seeding, fertilizing, mulching, clean-up and disposal of surplus material, sawcutting sidewalks and paved areas, as well as furnishing and installing curbing, riprap, crushed stone, topsoil, sidewalk, pavement or structure, as the case may be.

When rock that meets the description given under 10.01.01 is encountered within the limits of trenching, its removal will be classified; and the accepted quantities of rock in trench excavation will be paid for at the Contract unit price per cubic yard for "Rock in Trench Excavation." In the absence of a “Rock in Trench Excavation” item, the work will be compensated as extra work.

Material required to replace or supplement backfill as determined by the Engineer will be paid for at the Contract unit price for the material being used or as extra work if no unit price has been established.

Any pavement or bituminous concrete sidewalk replaced beyond the maximum trench limits as shown on the plans and approved by the Engineer, will be paid for at the Contract unit price for the material being used, or as extra work, if no unit price has been established.

All concrete sidewalk replaced due to trench excavation, including that within the trench limits, shall be paid for at the Contract unit price for “Concrete Sidewalk” or as extra work, if no unit price has been established.

Pay Item	Pay Unit
Trenching and Backfilling	l.f.
Rock in Trench Excavation (Deep)	c.y.

SECTION 10.02
LIGHT STANDARD AND TRAFFIC CONTROL FOUNDATIONS

10.02.01—Description: This item shall consist of furnishing and installing a light standard or traffic control foundation of the type called for at the location and to the dimensions and details shown on the plans or as directed by the Engineer. Rock, in so far as it applies to "Rock in Foundation Excavation," shall be defined as rock in definite ledge formation, boulders, or portions of boulders, cement masonry structures, concrete structures or Portland cement concrete pavement which has a cross-sectional area that exceeds 50% of the cross-sectional area of the designed foundation hole.

10.02.02—Materials: The materials for the work shall be as specified in the following:

M.02.01 for Granular Fill
M.03 for Class PCC03340 Concrete
M.04.01 for HMA S0.375 Bituminous Concrete
M.06.01 for Bar Reinforcement
M.08.02-4 for Precast Concrete
M.13.01 for Topsoil
M.13.03 for Fertilizer
M.13.04 for Seed Mixture
M.13.05 for Mulch Materials
M.15.02 for Anchor Bolts
M.15.03 for Rock Anchors
M.15.09 for Electrical Conduit
M.15.15-7 for Ground Rods

10.02.03—Construction Methods: The Light Standard and Traffic Control Foundations shall be constructed in accordance with the pertinent provisions of 6.01.03. Only the Type I Light Standard or Type I Traffic Control Foundation may be precast. The Engineer shall decide whether rock anchors shall be used or the rock shall be excavated. The finished elevation of the top of the foundation shall be as shown on the plans. The final elevation of the top of the light standard foundation shall be 1 1/2 inches above finished grade or as directed by the Engineer. The top of the foundation shall be level in all respects. Concrete for foundation shall be placed monolithically against undisturbed soil for poured in place foundations. Necessary electrical conduit, anchor bolts, ground rod sleeves and ground rods shall be placed in proper position and shall be held in place by means of a template. The concrete may be placed against the sides of the excavation, however, the exposed portion of the foundations shall be formed to the neat lines as shown on the plans. When in the judgment of the Engineer, unusual soil conditions prevent excavation to neat lines as shown on the plans, the complete foundation shall be formed.

After the forms have been completely removed, the entire excavation shall be backfilled in accordance with 2.14. All conduits shall be capped with standard pipe caps before placing the concrete and shall remain capped until the cable is installed. Electrical conduit of the size indicated shall extend 2 feet outside the foundation. All portions of the foundations which will remain exposed to view shall be finished to the satisfaction of the Engineer. Forms shall not be removed until after the concrete has hardened properly and not less than 24 hours after the concrete has been placed. The Contractor shall allow sufficient time for the foundation to cure before placing any strain on the foundation. Steel poles shall not be installed until a minimum of seven days after the concrete has been placed and a minimum of ten days before making span wire attachments thereto. Mast arm assemblies shall not be installed until 10 days after the concrete has been placed.

Where a foundation is placed within or adjacent to a concrete sidewalk, the entire section of sidewalk between joints shall be replaced in accordance with 9.21, unless otherwise directed by the Engineer.

10.02.04—Method of Measurement: Light Standard and Traffic Control Foundations of the type specified shall be measured for payment by the number of units installed and accepted. This measurement shall include the electrical conduit sweeps which shall extend 2 feet outside of the foundation.

The rock-in-foundation excavation will be measured from the top of rock to the bottom of rock excavation. Gravel will be measured in accordance with 2.13.04.

10.02.05—Basis of Payment: This work will be paid for at the Contract unit price each for “Light Standard Foundation” or “Traffic Control Foundation” of the type called for, which price shall include all

materials, equipment, forms, excavation, disposal of surplus material, Class PCC03340 concrete, electrical conduit sweeps, conduit caps, ground rod, sleeves, bonding bushings, anchor bolts, backfill, topsoil, grading, seeding, fertilizing, mulching, riprap, restoration of bituminous concrete sidewalk and pavement surfaces. All concrete sidewalk replaced due to foundation installation shall be paid for at the Contract unit price for “Concrete Sidewalk.”

When rock is encountered within the limits of excavation, its removal will be paid for at the Contract unit price per vertical foot for "Rock-in-Foundation Excavation," which price shall include any additional excavation to remove the rock and any additional concrete required to fill the excavation beyond the designed foundation hole dimensions.

Pay Item	Pay Unit
Light Standard Foundation (Type)	ea.
Traffic Control Foundation (Type)	ea.
Rock in Foundation Excavation	v.f.

SECTION 10.03
LIGHT STANDARDS

10.03.01—Description: This item shall consist of furnishing and installing light standards of the type specified with removable transformer bases, brackets, dampers, grounding wire and connections, complete in place, at the locations and to the dimensions and details shown on the plans or as directed by the Engineer. It shall also include furnishing and installing light standards of the type specified with anchor bases, brackets, elastomeric leveling pads, handholes and covers, complete in place, at bridge or structure locations, and to the dimensions and details shown on the plans or as directed by the Engineer.

10.03.02—Materials: Light standard (aluminum), shall meet the requirements of M.15.04.

10.03.03—Construction Methods: Light standards shall be fastened securely to the anchor bolts in the concrete light standard foundation, bridge, or structure. Light standards mounted on any bridge or structure shall have a vibration isolation pad installed between the pole base and the structure with four reinforced elastomeric washers installed on the top side of the base, between 2 standard galvanized flat washers. The completely assembled light standard shall be erected plumb with the aid of aluminum shims, if necessary. The mounting height shall be as called for on the plans, measured from the center of the light source to the pavement directly below. Brackets shall be erected with the bracket placed perpendicular to the centerline of the roadway.

The top of the shaft shall be drilled to accept three self-tapping screws. Pole caps shall be attached securely with three stainless steel set screws.

Each light standard shall be effectively grounded with #8 AWG ground wire attached to the light standard by an approved aluminum to copper lug and a stainless steel bolt, run to the ground rod, and connected with a square head bolt clamp.

Light standards shall be numbered with strip tags. These tags shall be reflective and not less than 1 3/8 inches x 1 7/8 inches. The tags shall be located 8 feet above the grade on the road side of the light standard. In locations where light standards are mounted in the median, tags shall be located on both road sides. The town number will be the top number and the pole number will be the bottom number.

10.03.04—Method of Measurement: This work will be measured for payment by the number of light standards of the type specified, complete and accepted in place.

10.03.05—Basis of Payment: This work will be paid or at the Contract unit price each for "Light Standard" of the type specified, complete in place, which price shall include all materials, including dampers, shaft, brackets, transformer bases, washers, nuts, bolts, bolt covers, ground wire, strip tags, connections, elastomeric leveling pads, and all equipment, tools, labor incidental thereto, including pole certification and field-checking bolt circle diameters.

Pay Item	Pay Unit
Light Standard (Type-Size-Kind)	ea.

SECTION 10.04
ROADWAY LUMINAIRE

10.04.01—Description: This item shall consist of furnishing and installing a luminaire of the type and size called for on the plans with integral ballast, lamp, fuses, fuse holders, and conductors from the luminaire to the distribution circuit, in accordance with these specifications.

10.04.02—Materials: The materials for this work shall meet the requirements of M.15.05.

10.04.03—Construction Methods: The luminaire, with integral ballast, shall be installed at the end of the bracket and shall be leveled to the road surface, securely fastened, lamped, connected, cleaned and ready for operation. Socket shall be in position to provide the IESNA light distribution type called for on the plans.

Conductors shall be run from the ballast primary terminal in the luminaire to the breakaway fuse holders in the pole base or junction box. Each ungrounded leg shall have a fuse holder containing a 10-amp fuse. Each neutral conductor shall have a fuse holder with a white connecting nut containing a brass slug in place of a fuse.

10.04.04—Method of Measurement: This work will be measured for payment by the number of luminaires of the type and size specified, installed and accepted.

10.04.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Roadway Luminaire" of the type and size specified completed and accepted in place, which price shall include all materials, including luminaire, ballast, conductors, connectors, lamp, brass slug, fuses, fuse holders, identification sticker, photoelectric control where required, and all labor, tools, equipment and work incidental thereto.

Pay Item	Pay Unit
Roadway Luminaire (Type-Size)	ea.

SECTION 10.06
UNDERBRIDGE LUMINAIRE

10.06.01—Description: This item shall consist of furnishing and installing a luminaire of the kind, type and size called for on the plans with necessary mountings, conduit, conductors, fuses, fuse holders, lamp and integral ballast as shown on the plans and details or as directed by the Engineer and in accordance with these specifications.

10.06.02—Materials: The materials for this work shall meet the requirements of M.15.07.

10.06.03—Construction Methods: The underbridge luminaire shall be installed at the location and to the dimensions shown on the plans and details.

The wall or pendent mounted luminaire shall be installed as detailed on the plans. The pendent mounted luminaire shall provide the IESNA light distribution type called for on the plans.

Conduit shall be installed in accordance with 10.08.03.

Conduit shall be installed from the junction box or handhole to the luminaire on the surface of the bridge structure or along the top of the concrete piers or between the bridge girders.

The conductors shall be installed in the conduit from the luminaire to the junction box or handhole and connected to the lighting circuit with fuses and fuse holders.

10.06.04—Method of Measurement: This work will be measured for payment by the number of underbridge luminaires, complete and accepted in place.

10.06.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Underbridge Luminaire," complete in place, which price shall include all materials, including fuses, fuse holders, conduit, luminaire, ballast, reducers, lamp, mounting plate, conductors, all the hardware, fittings, expansion fittings, flexible conduit, straps, back spacers, conduit clamps, equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
Underbridge Luminaire (Kind-Type-Size)	ea.

SECTION 10.08
ELECTRICAL CONDUIT

10.08.01—Description: This item shall consist of furnishing and installing conduit of the size and type specified with necessary fittings, where called for, at locations shown on the plans or as directed by the Engineer and in accordance with these specifications.

10.08.02—Materials: Materials for this work shall meet the requirements of M.15.09.

10.08.03—Construction Methods: The conduit shall be installed in the locations and to the dimensions shown on the plans or as directed by the Engineer. All conduit runs shall be installed in a neat and workmanlike manner in accordance with recognized trade practices. Trenching and backfilling of conduit shall be as specified in 10.01. All conduit shall be installed in strict accordance with the current NEC. Where conduit is to be capped, a commercial pipe or conduit cap shall be used. An expansion fitting shall be used wherever required by an expansion joint in the structure. Upon completion of the work, all conduits shall be cleaned, swabbed and free from obstructions and burrs. For buried conduit, marking tape shall be installed in the trench at the depth and to the requirements as set forth in the 1.05.15. After all cable has been installed, U.L. approved duct sealing compound shall be installed in the ends of all conduit which terminates in foundations, handholes, junction boxes and manholes.

1. **Conduit Surface:** Beam clamps or conduit strips with back spacers shall be provided at intervals in accordance with the NEC. Expansion fittings shall be installed at all expansion joints. All surface mounted conduit on wood poles shall be bonded to a driven ground rod. Stand-offs shall be installed in accordance with serving utility company regulations.
2. **Conduit in Trench:** Trenches shall be of the depth and cross section shown on the plans or as directed by the Engineer. All conduit shall have a minimum covering of 2 feet.
3. **Conduit Under Roadway:** Installations shall be such as to avoid pockets in runs. Conduit shall have a minimum cover of 2 feet. Each end of conduit runs shall terminate with a cap in a concrete handhole as shown on the plans. The Contractor shall coordinate the placement of the conduit prior to the placement of the pavement.

   Where conduit is to be installed under an existing roadway a trench shall be opened and conduit installed as shown on plans, or as directed by the Engineer. The trench shall be backfilled with suitable material and the surface shall be restored to original condition.
4. **Conduit in Structure:** It shall be the Contractor's responsibility to coordinate the setting of all conduit in structure prior to pouring concrete. Expansion fittings shall be installed at all expansion joints. Where shown on the plans, outlet boxes with the conduits properly connected and conduit hanger inserts with proper sized nuts installed, shall be accurately and securely placed in the forms for concrete. Care shall be taken during the placing of the concrete around these boxes and inserts to consolidate the concrete thoroughly, preventing voids and honeycomb and to prevent any material displacement of the boxes or inserts. Sealed bonding bushing shall be provided at each conduit outlet in all boxes.
5. **Conduit Under Slope Protection:** Conduit shall be installed in trench under bridge slope protection or between the slope protection and the edge of pavement at the locations and to the dimensions shown on the plans or as directed by the Engineer. The conduit shall have a minimum cover of 2 feet under finish grade and each end of the conduit shall extend 10 feet beyond the limits of the slope protection. All conduit shall terminate with a standard pipe cap and the trench shall be backfilled with suitable material. The Contractor shall coordinate the placement of the conduit prior to the placement of slope protection.

10.08.04—Method of Measurement: Conduit will be measured for payment by the actual number of linear feet of the type and size installed and accepted. The measured length shall be from end to end along the centerline through all fittings.

10.08.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "(Size) (Kind) Conduit-(Type)."

This price shall include all materials required including expansion fittings, conduit fittings, locknuts, bonding bushings, bonding wire, hangers, clamps, duct seal, caps, inserts, equipment, tools, labor and work incidental thereto.

Trenching and backfilling shall be paid under 10.01.

No payment for trenching and backfilling will be made for conduit installed during construction under new pavement or in the fill area of new median barrier curb installations.

Pay Item	Pay Unit
(Size) (Kind) Conduit (Type)	l.f.

SECTION 10.09
CAST IRON JUNCTION BOX

10.09.01—Description: This item shall consist of furnishing and installing cast iron junction boxes at locations shown on the plans or as directed by the Engineer and in accordance with these specifications.

10.09.02—Materials: Materials for this work shall meet the requirements of M.15.10.

10.09.03—Construction Methods: The junction boxes shall be mounted as shown on the plans or as directed. All hardware used in conjunction with mounting of these boxes shall be rust and corrosion resistant.

Conduit knockouts shall be made in the junction box by an approved method. Diameter of the knockouts shall be no larger than the minimum diameter required to accept the size conduit specified on the plans. All conduits are to be secured to the junction box using the washers, locknuts, and bushings as detailed on the plans. An RMC drain pipe with outlet to free air shall be installed in the cast iron junction box as detailed on the plans, except where the junction box is located over a fill area such as in a wingwall.

All excess concrete shall be removed from the junction box cover, recessed bolt holes, and bolt heads.

10.09.04—Method of Measurement: The work for this item will be measured for payment by the number of cast iron junction boxes of the size specified, complete and accepted in place.

10.09.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Cast Iron Junction Box" of the size specified, complete in place, which price shall include all materials, box, cover, gasket, drain pipe, cover bolts, necessary fittings and hardware, including that necessary for mounting, removal of excess concrete, all equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Cast Iron Junction Box (Size)	ea.

SECTION 10.10
CONCRETE HANDHOLE

10.10.01—Description: This item shall consist of furnishing and installing a concrete handhole of the type called for at the location and to the dimensions and details shown on the plans, or as directed by the Engineer. The concrete handholes may be precast or cast in place.

10.10.02—Materials: The materials for this work shall meet the required specifications in the following articles:

Bituminous Concrete	M.04
Concrete Sidewalk	9.21.02
Class PCC03360 Concrete	M.03
No. 6 Crushed Aggregate	M.01.02
Topsoil	M.13.01
Precast Concrete	M.08.02-4
Fertilizing	M.13.03
Seeding	M.13.04
Mulching	M.13.05-2
Galvanizing	M.06.03
Ground Wire	M.15.13

Handhole cover - The handhole cover shall be galvanized, non-skid floor plate steel which will have a minimum tensile strength of 50,000 psi.

Cast Iron Handhole Cover - The frame and cover shall be designed for H-20 loading. The cast iron shall meet the requirements of AASHTO M 105 Class 25. The cover shall be lettered "TRAFFIC."

10.10.03—Construction Methods: Cast-in-place or precast handholes shall be constructed or installed as shown on the plans. Handholes that are cast-in-place shall be constructed in accordance with the pertinent provisions of 6.01.03.

The areas disturbed by the excavation for the handholes shall be neatly graded to conform to the adjacent contours. Where topsoil has been removed, it shall be replaced to its original depth (except that in no case shall this depth be less than 4 inches) and the area shall be fertilized, seeded and mulched. The final elevation of the top of the concrete handhole shall be 1 1/2 inches above finished grade or as directed by the Engineer.

Where handholes are shown in bituminous concrete sidewalk or paved areas, the concrete handhole shall be set flush and the surrounding area shall be replaced with the same type of material as removed.

Where a handhole is placed within or adjacent to a concrete sidewalk, the entire section of sidewalk between joints shall be replaced in accordance with 9.21, unless otherwise directed by the Engineer.

Unless otherwise ordered by the Engineer, a handhole cover plate (galvanized steel) of the size specified, shall be installed on each handhole. The cover for concrete handhole shall be overlap type with the following exceptions: where the handhole is installed in a sidewalk, concrete median barrier or where concrete handhole type II is used, the cover shall be the recessed type. The handhole cover plate shall be bonded to ground wire and to all metal conduit within the handhole with No. 8 ground wire.

The Type II bank adapter shall be installed to the same contour of the embankment in which it is placed.

The frame for the cast iron cover shall be held in place on the concrete handhole with cementitious grout. The cast iron handhole cover shall be flush with the surrounding surface.

The concrete handhole shall be set on crushed stone in accordance with the details, and a drain pipe shall be installed meeting the requirements of 10.11.

10.10.04—Method of Measurement: This work will be measured for payment by the number of handholes, Type II bank adapters, and cast iron handhole covers of the type specified, complete and accepted in place.

10.10.05—Basis of Payment: This work will be paid for at the Contract price each for "Concrete Handhole" of the type called for, “Concrete Handhole Type II Bank Adapter” or “Cast Iron Handhole Cover,” complete in place, which price shall include all materials, concrete, steel cover, grounding tab, crushed stone, excavating, backfilling and replacement of bituminous sidewalk and pavement, including grading and placing topsoil, seeding, fertilizing, mulching and all equipment, tools, labor and work incidental thereto. All concrete sidewalk replaced due to handhole installation shall be paid for at the Contract unit price for “Concrete Sidewalk.” The ground wire (bonding wire) is included in the Contract unit price under 10.08 - Electrical Conduit.

Pay Item	Pay Unit
Concrete Handhole	ea.
Concrete Handhole Type II	ea.
Concrete Handhole Type II Bank Adapter	ea.
Cast Iron Handhole Cover	ea.
Concrete Handhole (Type)	ea.

SECTION 10.11
4 INCH DRAIN PIPE

10.11.01—Description: Under this item the Contractor shall furnish and install 4 inch drain pipe (non-perforated) and fittings for the drainage of concrete handholes as shown on the plans or as directed by the Engineer.

10.11.02—Materials: Plastic drain pipe shall meet the requirements of M.08.01-20 or M.08.01-21.

10.11.03—Construction Methods: The drain pipe shall be installed in accordance with 6.86.03. Both ends of the drain pipe shall be covered with a 1/4 inch galvanized hardware fabric which shall be secured in place with an adjustable stainless steel band clamp.

Trenching and backfilling shall be as specified in 10.01.03. Only one type of 4 inch drain pipe will be used in each installation.

10.11.04—Method of Measurement: The 4 inch drain pipe shall be measured for payment by the number of linear feet installed, complete and accepted, measured in place, along the centerline of the pipe, including all fittings and wire screen.

Trenching and backfilling shall be measured for payment as specified in 10.01.04.

10.11.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "4 Inch Drain Pipe," complete in place, which price shall include all materials, elbows, fittings, wire screens, and all equipment, tools and labor incidental thereto. Trenching and backfilling shall be paid for under 10.01.05.

Pay Item	Pay Unit
4 Inch Drain Pipe	l.f.

SECTION 10.12
SINGLE CONDUCTOR

10.12.01—Description: This item shall consist of furnishing and installing conductors of the size and type specified, complete in place, as shown on the plans or as directed by the Engineer and in accordance with these specifications.

10.12.02—Materials: The materials for this work shall meet the requirements of M.15.11.

10.12.03—Construction Methods: Conductors shall be installed as shown on the plans or as directed by the Engineer.

Installed conductors shall be free from any damage. Contractor shall install conductors using standard trade practices, tools and equipment. Sufficient slack for splices shall be provided in cast iron junction boxes, concrete handholes, and light standard bases in accordance with the details.

Splices shall be made only at approved locations and in accordance with NEC and insulated to maintain the voltage rating of the insulation of the conductor. All splices shall be made as shown on plans and details using approved waterproof devices and materials.

10.12.04—Method of Measurement: The quantity of single conductor, of the size and type specified, will be measured for payment by the actual number of linear feet of conductor installed and accepted.

10.12.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Single Conductor" of the size and type specified, complete in place, which price shall include all materials, tools, connections, equipment, labor and work incidental thereto.

Pay Item	Pay Unit
Single Conductor (Size)	l.f.

SECTION 10.14
CABLE IN DUCT

10.14.01—Description: This item shall consist of furnishing and installing pre-assembled cable in duct of the size, number, and type of conductors specified at locations shown on the plans or as directed by the Engineer, and in accordance with these specifications.

10.14.02—Materials: The materials for this work shall meet the requirements of M.15.12.

10.14.03—Construction Methods: The cable in duct shall be installed in trench encased in sand, or in conduit as shown on the plans or directed by the Engineer, in a straight continuous piece with no couplings, between light standard bases, between light standard base and handhole, or between handholes. Marking tape shall be installed in the trench at the depth and to the requirements as set forth in 1.05.15. The cable in duct shall be cut by an approved cutting tool in a manner that will not damage the conductors. The cable shall be installed in a neat and workmanlike manner, free from any surface damage.

Sufficient slack for splices shall be provided in cast iron junction boxes, concrete handholes, and light standard bases in accordance with the details. Splices shall be made only at approved locations and in accordance with NEC and insulated to maintain the voltage rating of the insulation of the conductor. All splices shall be made as shown on plans and details using approved waterproof devices and materials.

10.14.04—Method of Measurement: The length of cable in duct to be included for payment shall be the amount accepted and measured from centerline to centerline of light standards or concrete hand holes, as the case may be, plus 8 feet.

10.14.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Cable In Duct" of the size, type, and number of conductors specified, complete in place, which price shall include all materials such as terminators, sealing compound, connections, grounding, disposal of surplus material, and all equipment, tools and labor incidental thereto.

Necessary excavation, including sand encasement will be paid for as "Trenching and Backfilling" in accordance with the provisions of 10.01.05.

Pay Item	Pay Unit
Cable In Duct (No.—Size)	l.f.

SECTION 10.15
GROUNDING CONDUCTOR

10.15.01—Description: This item shall consist of furnishing and installing grounding conductor of the size specified in locations shown on the plans or as directed by the Engineer and in accordance with these specifications.

10.15.02—Materials: The materials for this work shall meet the requirements of M.15.13.

10.15.03—Construction Methods: All grounding and bonding shall be in accordance with NEC. Grounding conductor shall be installed in trench with the cable in duct, in conduit, in light standard bases, in junction boxes, and in handholes as shown on the plans.

All conducting material and equipment shall be bonded.

10.15.04—Method of Measurement: The quantity of grounding conductor of the size specified will be measured for payment by the actual number of linear feet installed and accepted between the centerline of light standards or centerline of concrete handholes, as the case may be, with an extra allowance of 8 feet. The grounding conductor required for the light standard items will not be measured for payment under this item.

10.15.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Bare Copper Grounding Conductor" of the size specified, complete in place, which price shall include all materials, tools, connections, equipment, labor and work incidental thereto.

Pay Item	Pay Unit
Bare Copper Grounding Conductor (Size)	l.f.

SECTION 10.17
SERVICE ENTRANCE AND CABINET

10.17.01—Description: This item shall consist of furnishing and installing a service entrance and cabinets of the type specified as detailed on the plans at the location shown on the plans or as directed by the Engineer and in accordance with these specifications. Included in this work, where called for, are service entrance cabinet, lighting control cabinet, electrical components, service entrance conductors and conduit, foundations, transformer pad, fence, metering provisions and utility work.

10.17.02—Materials: The materials for this work shall meet the requirements of M.15.15.

10.17.03—Construction Methods: All work shall be constructed as shown on the plans or as required by the utility company and directed by the Engineer. The Contractor shall give adequate notice to the utility company for scheduling of work by utility.

The installation of the service conduit of the size and type specified, and all the conduit required in the concrete pad or concrete foundation, shall meet the requirements of 10.08.03 for the type of installation required. Where service entrance conductors are provided by the utility company, the Contractor shall provide pull wire.

Trenching and backfilling for conduit shall meet the provisions of 10.01.03.

The concrete pad and concrete foundations shall be constructed in conformance with 10.02.03. Conduit, inserts, sleeves and fittings shall be placed in proper positions and shall be so held until the concrete sets. Forms shall not be removed until the concrete has hardened properly and not less than 24 hours after the concrete has been placed. All portions of the pad and foundation that will remain exposed to view shall be neatly finished in accordance with the requirements for "Float Finish" as outlined in 6.01.03-I-10.

The transformer pad may be precast per utility company specifications. Pedestal foundations may be precast.

The electrical components shall be mounted to the plywood back panel in each cabinet, and wired as shown on the plans or as directed.

The Contractor shall label all circuits in each cabinet.

The photoelectric control shall be located as indicated on the plans. The cell shall be positioned according to the manufacturer's recommendations. No. 12 conductors shall be run from the cell to the service cabinet and the necessary connections made for proper operation.

10.17.04—Method of Measurement: This work will be measured for payment as a unit by the number of services installed and accepted.

10.17.05—Basis of Payment: This work will be paid for at the Contract unit price for "Service Entrance and Cabinet," of the type called for, complete and accepted in place, which price shall include all materials including all electrical components, concrete pad, service entrance cabinet, lighting control cabinet, main disconnect safety switch, concrete foundation, gravel fill, conduits, conductors, control transformer, circuit breakers, meter socket, photoelectric control, transformer pad, receptacle, contactor, convenience light, pedestal, fence, pull wire, manual switch, fuses, fuseholders, ground rod, grounding conductor, trenching and backfilling, fittings, standoffs, and all equipment, tools, labor and work incidental thereto including excavation, clearing or removal of brush, backfill, grading, topsoil, sodding or turf establishment, and all utility company charges.

Where rock is encountered in excavation, it will be paid for as "Rock in Trench Excavation" in accordance with the provisions of 10.01.05.

Pay Item	Pay Unit
Service Entrance and Cabinets (Type)	ea.

SECTION 10.18
NAVIGATION LIGHT

10.18.01—Description: This work shall consist of furnishing and installing a permanent navigation light with all necessary components as shown on the plans and details or as directed by the Engineer.

10.18.02—Materials: The materials for this work shall meet the requirements of M.15.16.

10.18.03—Construction Methods: The navigation light shall be installed as shown on the plans and details, and shall be easily accessible for maintenance purposes. The circuit for the navigation light shall be as shown on the plans and details.

The Contractor shall submit a wiring diagram, installation drawing and list of equipment to be furnished for approval prior to starting his electrical work. All electrical work shall be strictly in accordance with the requirements of the NEC. Installation shall be strictly in accordance with the requirements of the United States Coast Guard.

10.18.04—Method of Measurement: This work will be measured for payment by the number of navigation lights furnished, installed and accepted in place.

10.18.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Navigation Light," complete in place, which price shall include all materials, equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
Navigation Light	ea.

SECTION 11.01
POLE ANCHOR

11.01.01—Description
11.01.02—Materials
11.01.03—Construction Methods
11.01.04—Method of Measurement
11.01.05—Basis of Payment

11.01.01--Description: This work shall consist of furnishing and installing a pole anchor of the type specified on a new or an existing pole at the location shown on the plans or as directed by the Engineer. The quantity of pole anchors shown on the plans are approximate only. The actual number and location of anchors shall be such as to provide adequate support for those poles to which the span is being attached as determined in the field.

11.01.02--Materials: The materials for this work shall meet the pertinent requirements of M.16.04.

11.01.03--Construction Methods: A lead of approximately 20 feet shall be maintained from the pole when placing the anchor. The hole for the anchor shall be mechanically dug, and the diameter will be the nominal size of the unexpanded diameter of the anchor. A slot shall be cut in the side of the hole to permit the anchor rod to line up with the guy strand. The anchor rod shall extend above the ground not less than 6 inches nor more than 12 inches. Prior to installation, the anchor shall be locked in its expanded position. This shall be done by marking the expanding bar end away from the anchor, with the anchor in the expanded position. The Contractor shall collapse the anchor tie plates and install in the hole with the expanding bar, and expand into undisturbed earth with several heavy blows on the expanding bar. The anchor will be fully expanded when the mark on the expanding bar is opposite the top of the anchor rod. All backfill is to be thoroughly tamped as it is being placed. When required on wood poles, a wet porcelain strain insulator shall be installed on the guy strand as shown on the installation detail sheet. Care shall be exercised in placing anchors in the vicinity of private property. Where necessary state forces will establish property lines.

11.01.04--Method of Measurement: This work will be measured for payment by the number of anchors completed and accepted in place.

11.01.05--Basis of Payment: This work will be paid for at the Contract unit price each for "Pole Anchor," complete in place, which price shall include all materials, anchor, anchor rod, anchor guy strand wire, anchor guy protector, excavation and backfilling, and all necessary fittings, hardware, tools, labor and work incidental thereto. Only those anchors installed on existing poles will be paid for under this item. When new wood poles are installed the required anchor is paid for in the price of the pole.

Pay Item	Pay Unit
Pole Anchor	ea.

SECTION 11.02
PEDESTALS

11.02.01—Description: This item shall consist of furnishing and installing a pedestal of the type and size specified on a prepared foundation at locations and to the details shown on the plans, or as ordered, and in accordance with these specifications.

11.02.02—Materials: The materials for this work shall meet the requirements of M.16.03.

11.02.03—Construction Methods: The complete unit shall be erected vertically, mounted plumb, and shall be securely bolted to the foundation. All pedestals shall be effectively grounded using a 5/8 inch × 10 foot copper ground rod with #8 AWG solid, bare, tinned copper wire attached to the pedestal by a stainless steel bolt and to the ground rod by a square head bolt clamp.

11.02.04—Method of Measurement: This work will be measured for payment by the number and type of pedestals specified, completed and accepted in place.

11.02.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Pedestal" of the type and size specified, which price shall include all materials, pedestal base, shaft, ground wire, paint, painting, labor, tools, equipment, miscellaneous fittings and work incidental thereto.

Pay Item	Pay Unit
Pedestal (Type and Size)	ea.

SECTION 11.03
SPAN POLE

11.03.01—Description: This item shall consist of furnishing and installing a steel span pole, as indicated on the plans or as directed by the Engineer.

This item shall also include furnishing a wood span pole and installing it at the location designated on the plans, or as indicated by the Engineer.

11.03.02—Materials: The materials for this work shall meet the requirements of M.16.04.

11.03.03—Construction Methods:

1. Steel Pole: Each pole shall be securely bolted to its base, electrically grounded, and installed per the manufacturers specifications. The pole shall be oriented so the handhole is away from traffic. The steel pole shall be raked by means of a double nut construction. The opening between the steel pole base and the concrete foundation shall be grouted and neatly finished with a non-shrink, non-stainable grout that meets the requirements of M.03.05. The mast arm anchor bolts are specified in M.15.02.3.

Installation Procedure:

A. Install leveling nuts onto anchor bolts with the approximate pole "rake."
B. Install steel pole base plate onto leveling nuts and tighten the anchoring nuts loosely.
C. The unloaded pole shall be "raked" back according to manufacturer's specifications by adjusting the nuts as necessary. Upon completion the "rake" should give the pole a plumb look under load.
D. Tighten up the anchoring nuts to a "snug-tight" condition, while insuring that the leveling nuts are always in firm contact with the base plate. A "snug-tight" condition is defined as the tightness attained by the full effort of a person using a spud wrench.
E. Check all nuts for looseness after the traffic lights and other loads are in place, paying special attention to the leveling nuts on the tension bolts. Retighten any loose nuts to a "snug-tight" condition. Upon completion each pole shall be securely bolted to its base and have a plumb appearance.
F. Grout underneath base plate.
G. Install handhole cover, anchor bolt covers, and pole cap.

2. Wood Pole: The hole into which the pole is to be placed shall be mechanically dug, and its diameter shall be only large enough to accept the pole and the tamper and shall be 6 feet deep. Each pole shall be securely placed into the hole, electrically grounded, and installed as specified in the plans.

A lead of approximately 20 feet should be maintained from the pole when placing the anchor. The hole for the anchor shall be mechanically dug, and the diameter will be the nominal size of the unexpanded diameter of the anchor. A slot shall be cut in the side of the hole to permit the anchor rod to line up with the guy strand. The anchor rod shall extend above the ground not less than 6 inches nor more than 12 inches.

Prior to installation, the anchor shall be locked in its expanded position. This shall be done by marking the expanding bar end away from the anchor, with the anchor in the expanded position. The Contractor shall collapse the anchor tie plates and install in hole with expanding bar, and then expand into undisturbed earth with several heavy blows on the expanding bar. The anchor will be fully expanded when the mark on the expanding bar is opposite the top of the anchor rod. All backfill is to be thoroughly tamped as it is being placed. A wet porcelain strain insulator shall be installed on the guy strand at a minimum height of 8 feet. The installation of the guy strand will be as shown on the installation detail sheet.

3. Combination Steel Pole: Specifications for steel poles in 11.03.03-1 apply to combination steel poles including the following specifications.

When illumination service is underground, a nylon pull rope shall be installed from the end of the luminaire bracket to the pole base. At combination poles where service is overhead, a separate rigid service clevis, 2-inch entrance fitting and weatherhead shall be installed. A nylon pull rope shall be installed from the end of the bracket to the clevis. The luminaire bracket shall be attached to the pole shaft perpendicular to the centerline of the roadway as shown on the plans. Clamp-type brackets are permissible for ease and adjustment. The mounting height shall be a nominal 30 feet measured from the center of the light source to the pavement directly below. It shall be the responsibility of the Contractor to arrange with the utility company for installation of the luminaire on the bracket and secondary service to the luminaire.

11.03.04—Method of Measurement: This work will be measured for payment by the number of poles of the type specified, completed and accepted in place.

11.03.05—Basis of Payment: This work will be paid for as follows:

1. Steel Span Pole: This work shall be paid for at the Contract unit price each for "Steel Span Pole" or "Combination Steel Span Pole" of the type specified, which price shall include all materials, pole, pole cap, handhole, handhole cover, ground wire, grounding nut, anchor bolt covers, entrance fittings, service clevis, span clamps, paint, painting, grouting, labor, tools, luminaire bracket, miscellaneous hardware, and work incidental thereto.

2. Wood Span Pole: This work will be paid for at the Contract unit price each for "Wood Span Pole," complete in place, which price shall include all materials, pole, ground rod and wire, anchor, anchor rod, anchor guy strand wire, anchor guy protector, neutral bracket, excavating and backfilling, and all necessary fittings, hardware, tools, labor and work incidental thereto.

Pay Item	Pay Unit
(Type) Steel Span Pole	ea.
(Type) Steel Combination Span Pole (Bracket Type)	ea.
Wood Span Pole	ea.

SECTION 11.05
TRAFFIC SIGNALS

11.05.01—Description: This item shall consist of furnishing and installing a traffic control signal, of the type specified, at the location shown on the plans or indicated by the Engineer.

11.05.02—Materials: The materials for this work shall meet the requirements of M.16.06.

11.05.03—Construction Methods: Each completely assembled traffic signal unit shall normally be installed as follows:

Each traffic signal head shall consist of the appropriate number of sections and faces, terminal blocks and mounting equipment, as indicated. Signal heads shall be erected vertically in accordance with the specific type of mountings shown on the installation plan.

No signal head shall be erected until the controller for that signal is installed.

All traffic signals shall be mounted in the manner as shown on the plans, using fittings and attachments standard to the trade.

When traffic signals are span-mounted, they shall be installed so the lowest part of the signal head is not less than 16 feet or more than 17 feet above the pavement grade beneath the signal head.

If couplings are used in "nippling down" a traffic signal or in the construction of signal heads, they shall be drilled, tapped, and furnished with a set screw, or spot welded to eliminate twisting. Entrance fittings and locking nuts shall also be furnished with set screws to eliminate twisting. Signal ways consisting of more than three sections shall be assembled with appropriate numbers of one-way, one-section units.

When adding sections to existing or new signal heads the contractor shall provide all necessary mounting equipment, including new brackets if required, to adapt to the existing heads.

11.05.04—Method of Measurement: The quantity to be paid for under this item will be the number of completed, installed and accepted traffic signal units of the type specified.

11.05.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Traffic Signal" of the type specified which price shall include all materials, housing, brackets, conduit, visors, tunnel visors, louvers, back plates, optical unit, lamp socket, lens, mountings, paint, painting and necessary equipment, tools, labor and work incidental thereto.

No direct payment will be made for furnishing and installing visors or lenses required on existing traffic signals, but the cost thereof shall be included in the costs of the various traffic signal items.

Pay Item	Pay Unit
Traffic Signal (Type)	ea.

SECTION 11.06
PEDESTRIAN SIGNAL

11.06.01—Description: This item shall consist of furnishing and installing a pedestrian signal of the type specified at locations shown on the plans or as indicated by the Engineer in conformity with these specifications.

11.06.02—Materials: The materials for this work shall meet the requirements of M.16.07.

11.06.03—Construction Methods: Each completely assembled pedestrian signal unit shall be installed as follows:

Each pedestrian signal shall be erected vertically in accordance with the specific type of mountings shown on the plans. Each signal face shall be pointed in the direction of the approaching pedestrian traffic it is intended to control.

During construction, and until the signals are placed in actual operation, the signal housing shall be hooded. Every part of the signal head shall be at least 12 inches behind outer edge of the shoulder of the road.

The audible pedestrian signal shall be attached to a traffic signal assembly, using standard hardware as shown on the plan. The volume shall be adjusted to a minimum sound level that can be heard throughout the intersection, or as directed by the Engineer. The 120 VAC signal output shall be connected to a conflict monitor input and shall conflict against all opposing movements.

11.06.04—Method of Measurement: The quantity to be paid for under these items will be the number of completed, installed and accepted pedestrian signals of the type specified.

11.06.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Pedestrian Signal," of the type specified which price shall include all materials, housing, optical unit, visor, miscellaneous fittings and parts, equipment, paint and painting, labor and work incidental thereto.

Pay Item	Pay Unit
Pedestrian Signal (Type)	ea.

SECTION 11.07
PEDESTRIAN PUSH BUTTON

11.07.01—Description
11.07.02—Materials
11.07.03—Construction Methods
11.07.04—Method of Measurement
11.07.05—Basis of Payment

11.07.01—Description: This item shall consist of furnishing and installing pedestrian push buttons of the type specified at the location shown on the plans or as indicated by the Engineer.

11.07.02—Materials: The materials for this work shall meet the requirements of M.16.08.

11.07.03—Construction Methods:

A push button advisory sign 9 inches × 12 inches shall be attached to the saddle as indicated in the case of a pole or pedestal mounted push button or directly to the controller cabinet in the case of a surface mounted push button installed on the side of a controller cabinet. The push button and sign shall be installed on the crosswalk side of a pole or pedestal perpendicular to the direction of walking. On a controller cabinet, the push button and sign shall be surface mounted on the side of the controller cabinet nearest to the crosswalk. A 1/8 inch aluminum backing plate measuring 9 inches × 12 inches shall be used with all pole or pedestal-mounted advisory signs that have a push-button casting which is smaller in dimension than the 9 inch × 12 inch sign that is being attached. The backing plate shall conform exactly to the shape of the sign.

When the push button casting is as large as the 9 inch × 12 inch sign being attached, the aluminum backing plate will not be required.

When surface mounted on a controller cabinet, the sign shall be attached by drilling and tapping four holes, corresponding to the holes in the sign, for 10-32 machine screws, and mounting the sign with 4 each, 1/2 inch, 10-32 brass or stainless steel machine screws. Care is to be taken in drilling and tapping not to damage any wiring or equipment in the cabinet, including the terminal blocks. The push button shall be similarly mounted, using machine screws of proper size and length to meet the requirements of the push button. A suitable hole, properly reamed or smoothed to eliminate rough edges, shall be drilled in the cabinet to match the comparable opening in the back of the push button base through which the lead wires are run.

All buttons shall be furnished with enough No. 16 AWG THWN or THHN stranded wire to make a connection in the walk fixture or in the base of the 4 foot 4 inch aluminum pedestal whichever applies.

11.07.04—Method of Measurement: The quantity to be paid for under this item will be the number of pedestrian push buttons, of the type specified installed, operating and accepted.

11.07.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Pedestrian Push Button and Sign, Pole-Mounted," "Pedestrian Push Button and Sign, Pedestal Mounted," or "Pedestrian Push Button and Sign, Surface-Mounted," as the case may be, which price shall include push button, advisory sign, saddle, wire, all material, equipment, paint, painting, tools, labor and work incidental thereto.

Pay Item	Pay Unit
Pedestrian Push Button and Sign (Type)	ea.

SECTION 11.08
CONTROLLERS

11.08.01—Description: This item shall consist of furnishing and installing a pre-timed traffic controller, of the type specified for controlling the operation of the traffic signals as indicated on the sequence and timing chart on the plans, and all necessary auxiliary equipment required for satisfactory operation of the signal, at the location indicated on the plans or indicated by the Engineer.

This item shall also consist of furnishing and installing an Actuated Controller, which shall be a completely digital solid state unit, for controlling the operation of the traffic signals, as indicated on the sequence and timing chart on the plans, and all necessary auxiliary equipment required. The control equipment shall be installed at the location shown on the plans or as directed by the Engineer, and in accordance with these specifications.

11.08.02—Materials: The materials for this work shall meet the requirements of M.16.09.

11.08.03—Construction Methods: The controller, mounted in controller cabinet, shall be installed at the locations shown on the plans in the following manner:

The pole-mounted controller cabinet shall be attached to the face of the pole with the use of saddle measuring 3 inches × 16 inches × 7/8 inch. The pole shall be drilled and tapped to accept two 3/8 inch × 1 1/2 inch round head, cadmium-plated, machine screws to fasten the controller cabinet to the pole. The saddle shall have two 1/2 inch diameter guide holes for 3/8-inch screws for fastening the controller cabinet. A hole shall be drilled in the pole, using a hole saw, to accept a 2 1/2 inch coupling, which is to be welded to the pole. A 90 degrees pull box shall be attached to the coupling with a standard pipe nipple, the bottom of the cabinet shall have an opening for cable entrance and four holes to fasten the flange of the 90 degrees pull box.

The cabinet, when pedestal-mounted, shall be mounted with the use of a standard 4 inch pedestal adapter as shown on the installation detail sheet.

The base mounted cabinet shall have the base casting attached to the foundation and leveled before the cabinet proper is bolted to the base. A mastic type compound meeting the requirements of ASTM C647 shall be used between the cabinet proper and the base casting to make the two units rain-tight.

11.08.04—Method of Measurement: This work will be measured for payment by the number of each type controller completed, operating, and accepted in place.

11.08.05—Basis of Payment: This work will be paid for as follows:

1. **Pre-timed controller** will be paid for at the Contract unit price each for "Pre-timed Controller" of the type specified, which price shall include cabinet, timer, timing dial, unit motor, signal-switching unit, panel board, flasher, jack-mounted relays, auxiliary equipment, paint and painting, and all materials, equipment, tools and labor incidental thereto.
2. **Actuated controller** will be paid for at the Contract unit price each for "Actuated Controller" of the type specified, which price shall include controller, controller cabinet, wiring, flasher unit, radio line filter, necessary fittings for mounting, miscellaneous fittings, auxiliary equipment, paint and painting, and all materials, equipment, tools, and labor incidental thereto.

Pay Item	Pay Unit
Pre-timed Controller (Type)	ea.
Actuated Controller (Type)	ea.

SECTION 11.11
LOOP VEHICLE DETECTOR AND SAWCUT

11.11.01—Description: These items shall consist of:

1. **Furnishing and installing a loop vehicle detector** in accordance with these specifications.
2. **Furnishing and installing Sawcut** to be used with a loop vehicle detector as shown on the plans and in accordance with these specifications.

11.11.02—Materials: The materials for this work shall meet the requirements of M.16.12.

11.11.03—Construction Methods:

1. The loop vehicle detector shall be mounted in the controller cabinet. Wiring shall be done in a neat manner, and each wire shall be fitted with a spade lug and attached to the proper terminal.

Attach a loop identification tag to each loop amplifier harness. The tag shall be pre-printed by the manufacturer so that the Contractor can record the pertinent information on the tag. Record detector information on the tag with indelible ink. See example below:

Loop No.: ***D4***
Phase Call: ***Phase 4***
Field Location: ***Rt.411 (West St.)***
Eastbound, Left Lane
Detector No.: ***4***
Cabinet Terminals: ***234, 235***

Field location shall include the route number or street name, the direction (such as Northbound), and the lane (left, center, right).

Each system detector (S.D.) shall be connected to the corresponding system sensor input of the local computer communications unit (the CLLCU).

Example: S.D. 1 connected to System Sensor 1
S.D. 2 connected to System Sensor 2

2. Loop detectors installed in new or resurfaced pavements shall have the slots saw cut in the pavement base course and the loop wires placed and sealed before the placing of the pavement wearing course.

No loop detector saw cut shall be placed over a patched trench or existing pavement without the approval of the Engineer. The Contractor shall remove the existing pavement and replace it with new pavement for installation of the loop detectors, at the direction of and within the limits prescribed by the Engineer, where the condition of the existing pavement is not suitable for installation. The work shall be performed under the appropriate pavement removal and replacement items. The existing pavement shall be removed a minimum of 3 inches deep. The replaced pavement shall be overlaid as directed. The loop detector shall be installed in the new pavement, under the overlay.

The size of the loop shall be as shown on the plans and shall be made using a power saw having an abrasive or diamond blade 3/8 inch wide. The depth of the slot shall be 1 3/4 to 2 inches and shall extend from the loop to the location shown on the plan. The cuts shall overlap at the corners of the loop and at any angles in the saw cut to the controller to ensure full depth. The corners are to be rounded off by drilling a 1 1/4 inch diameter hole to eliminate kinking of the wire. When the cutting has been completed, the slot shall be cleaned of all cutting dust and grit with oil-free compressed air. The slot shall be completely dry before inserting the wire. The entire loop and lead-in shall consist of one continuous #14 stranded wire encased in tubing, unless otherwise shown on the plans. The lead-in wires shall be twisted together with at least 5 turns per foot and taped at 2 foot intervals, beginning at a point where the wire leaves the saw cut and enters the plastic conduit to the terminals in the controller cabinet, or when spliced to a 2-conductor lead-in cable. The twisted wire shall remain together and shall not be coiled at any point. Splices will not be permitted at any point of the loop or lead-in unless otherwise indicated on the plans or as directed by the Engineer.

At the time of installing the loop wire, the ends of the tubing shall be sealed to prevent any entrance of moisture into the tubing. When splices are required in handholes or junction boxes, they shall be made in accordance with the installation details. The saw cuts on all lead-ins shall be as shown on the plans or as directed by the Engineer. The number of turns of wire for each loop shall be as shown on the plans. After installation, the wire shall be checked for slack or raised portions in the saw cut slot. A paint mix stick or similar blunt instrument shall be used to push the wire in the slot. The wire shall be held in the slot with wooden pegs or by inserting wedges formed from 1 inch sections of the plastic tubing, folded before insertion. The loop should be tested prior to sealing the saw cut. An unacceptable loop installation would

consist of leakage to ground below 10 megohms or an open circuit. The saw cut shall then be filled with plastic compound to a level of approximately 1/16 inch below the pavement surface. In no case shall the plastic compound overflow the saw cut and all excess material shall be removed. The plastic compound shall be applied in accordance with the manufacturer's recommendations. The plastic compound shall adhere to the sides of the saw cut and not show a separation when pressure is applied by a blunt instrument.

Each pair of lead-in wires in the cabinet shall be tagged and identified to determine phase and geographical location of each loop, Example: "Phase 4—East End Bank St.—Right Lane."

When a one-part urethane elastomeric compound is used to seal the sawcut the manufacturer's installation procedure shall be followed.

The sawcut, complete and tested with wire installed and retained with plastic wedges, shall be filled with the elastomeric compound from the bottom up. The sawcut shall be filled to 1/8 inch from the surface of the pavement. The elastomeric compound shall not be applied if the pavement surface temperature is less than 40°F or more than 100°F.

11.11.04—Method of Measurement: This work will be measured for payment as follows:

1. Loop vehicle detectors will be measured for payment by the number of loop vehicle detectors of the type specified, completed, operating and accepted in place.

2. Loop Detector Saw Cut: The quantity to be paid for under this item shall be the actual number of linear feet of saw cut, measured along the center of the cut, with #14 Stranded Wire encased in tubing completely installed, tested and accepted in place. #14 Stranded Wire encased in tubing shall not be measured for payment separately but shall be included in the measurement for saw cut.

11.11.05—Basis of Payment:

1. Loop vehicle detector will be paid for at the Contract unit price each for "Loop Vehicle Detector" of the type specified, which price shall include connecting cable, sensor unit, all materials, labor and work incidental thereto.

2. Loop detector saw cut will be paid for at the Contract unit price per linear foot of "Loop Detector Saw Cut," which price shall include; saw cut, #14TW stranded wire (including that to handholes, traffic control foundations or controller cabinets), necessary fittings, flexible plastic tubing, plastic compound, splicing and connecting, equipment, labor, materials and work incidental thereto.

Pay Item	Pay Unit
Loop Vehicle Detector (Type)	ea.
Loop Detector Saw Cut	l.f.

SECTION 11.12
MAGNETIC VEHICLE DETECTOR

11.12.01—Description: This item shall consist of furnishing and installing a magnetic vehicle detector of the type specified at the location shown on the plans, or as indicated by the Engineer and in accordance with these specifications.

11.12.02—Materials: The materials for this work shall meet the requirements of M.16.13.

11.12.03—Construction Methods: The magnetic detector shall be installed in a 4 inch polyvinyl chloride conduit. A joint shall be made to the control cable in the concrete handhole. Joints shall be made in accordance with 11.13.03.

The single lane magnetic detector shall be placed in a roadway excavation made only large enough to permit the pavement box and conduit to be lowered into place. Reinforcing rods in concrete need only to be removed within 5 inches or 6 inches of the detector housing.

Remove the detector coil unit and other parts from the pavement box, replace the cover and install the conduit using pipe joint compound to prevent the entrance of moisture.

The pavement box and conduit assembly will be suspended in the excavation using 4 bolts and 2 inch × 4 inch wood pieces as temporary supports. The cover of the box shall be 3 inches below the finished surface of the roadway. Plug up the unused conduit entrances with pipe plugs. Spread a handful of sand over the cover to prevent concrete from getting into the cracks. Place concrete in the excavation up to the level of the top surface of the box. After the concrete has hardened, the cover shall be removed and the coil unit placed in the box with the "arrow" pointing toward the intersection.

After the coil connections have been made, a silicone compound shall be applied to the terminals. The conduit entrances shall be sealed with oakum or equivalent and fill the entire pavement box with melted petrolatum (Vaseline) or equivalent.

The cover shall then be secured in place and the road resurfaced to its original level.

11.12.04—Method of Measurement: This work will be measured for payment by the number of magnetic vehicle detectors of the type specified, completed, operating and accepted.

11.12.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Magnetic Vehicle Detector," of the type specified, which price shall include detector, detector-amplifier, all materials, labor, tools and work incidental thereto.

Pay Item	Pay Unit
Magnetic Vehicle Detector (Type)	ea.

SECTION 11.13
CONTROL CABLE

11.13.01—Description: This item shall consist of furnishing and installing electrical cable and appurtenances of the type specified at the locations shown on the plans or as indicated by the Engineer, in accordance with these specifications.

11.13.02—Materials: The materials for this work shall meet the requirements of M.16.14.

11.13.03—Construction Methods: All wiring shall meet the appropriate requirements for the NEC and shall be insulated for 600 volts. Wire splicing will not be permitted in conduit or outside of junction boxes, hand holes, poles or pedestals, unless otherwise indicated on the plans.

All cables shall contain the required number of conductors as shown on the plans. The substitution of several cables to attain the required number of conductors will not be permitted. Cable to be installed in conduit shall be pulled with a minimum of dragging on the ground or pavement. This shall be accomplished by means of reels mounted on jacks or approved devices conveniently located for unreeling cable directly into the conduit. Powdered soapstone, talc, or other approved lubricants shall be used when inserting cable into the conduit. Cable shall be pulled through conduit by means of a cable or cables. Wiring within junction boxes and cabinets shall be neatly arranged.

Conductors entering the controller cabinet shall be neatly dressed and laced along the base and back of the cabinet to the terminal board mounted in the cabinet. When three or more conductors are attached to the same terminal in the controller cabinet, they shall not be twisted together, but shall be attached with a set-screw type lug of the correct size. When less than three conductors are attached to the same terminal, in the controller cabinet, a separate spade type connector shall be attached to each conductor. Spare conductors shall be tied together with their ends taped. All splices are to be of an approved type and made in such a manner that the connection will be moisture-proof. All aerial splices shall be accomplished as shown on the plans using a cable closure. At least 2 feet of slack shall be left for each conductor at each standard and at least 1 foot of slack at each pull box.

All splicing in junction boxes, hand holes, poles and pedestals can be accomplished by one of the following methods:

1. **Splices** can be made by soldering with the pouring or dipping method.
2. **Non-insulated butt-type pressure connectors** shall be seamless and be UL approved for aluminum or copper wire. The butt-type pressure connector shall be connected to the wire with an installing tool that is designed for butt-type pressure connectors. After crimping, all detector cable splices shall also be soldered. Each butt-type connector shall be insulated separately by applying layers of thermoplastic electrical insulating tape. The thermoplastic tape shall extend 1/2 inch beyond the conductor insulation and be lapped at 1/4 inch intervals. The entire completed splice shall have an application of layers of 30 mil high voltage rubber self-fusing tape well lapped over and equal to the original insulation. The splice shall then be covered with layers of thermoplastic electrical insulating tape not over 0.007 inch thick meeting the requirements of MILSPEC-7798 and a coating of flexible insulating paint approved by the Engineer.

At least 1 foot of slack shall be left for each conductor at each splice. An approved type of water-tight splicing box may be used in lieu of the aforementioned, providing the box has sufficient number of terminals to accommodate the number of conductors to be spliced in the box.

When conductors and cables are pulled into conduits, all ends of conductors and cables shall be taped to exclude moisture, and shall be so kept until the splices are made or terminal appliances attached.

3. **Cable closures** shall be installed as shown on the plans or as directed by the Engineer. The cable closure shall be attached to the span or messenger with suspension brackets. After the cables have been installed, each opening shall be sealed with sealing tape. When using cable less than 0.6 inch diameter, a cone-shaped collar shall be formed with sealing tape, to seal off the space between the cable and closure nipple. Two (2) turns of sealing tape will then be placed over the closure nipple and the cable. The cable shall be attached to the terminal blocks as shown on the typical installation details.

The terminals shall then be securely wrapped with electrical tape. Connectors shall not extend beyond the end of the terminal post. The terminals and connectors shall not be in contact with the closure cover at any point.

11.13.04—Method of Measurement: This work will be measured as follows:

1. **The quantity of control cable** to be paid for under these items shall be the actual number of linear feet measured along the center of the cable specified, installed, tested and accepted in place with the necessary connections.

2. Cable closures will be measured for payment by the number of cable closures of the type specified, completed and accepted in place.

11.13.05—Basis of Payment:

1. Control cable shall be paid for at the Contract unit price per linear foot for "(No.) Conductor #( ) (Type)" which price shall include installation, splicing, connecting, necessary fittings, equipment, labor, all materials and work incidental thereto.

2. Cable closure will be paid for at the Contract unit price for "Cable Closure" complete, in place, which price shall include all materials, sealing tape, terminal blocks and all equipment, tools, labor and work incidental thereto.

Pay Item	Pay Unit
(No.) Conductor #( )(Type)	l.f.
Cable Closure	ea.

SECTION 11.14
MESSENGER AND SPAN WIRE

11.14.01—Description: Work under this item shall consist of furnishing and installing span wire or messenger, whichever is applicable, at the locations indicated on the plans or as directed by the Engineer.

11.14.02—Materials: The materials for this work shall meet the requirements of M.16.15.

11.14.03—Construction Methods:

1. Span Wire used for the suspension of traffic signals shall be attached to the supporting poles at such a level as to make the lowest part of the signal head not less than 16 feet 0 inches, nor more than 17 feet 0 inches above the pavement grade beneath the signal head.

A normal sag of not more than 5% of the total span shall be permitted. Wire secured to the pole bands shall be protected from sharp bends by a thimble clevis. Thimble eyebolts, nuts and washers shall be used instead of eyebolts. Ends of cable shall be fastened to the standing part by 6-inch clamps and shall be wrapped with galvanized wire. A wet porcelain strain insulator shall be used at locations where the messenger or span wire is attached between steel and wood poles. Traffic signal control cable shall be suspended from the span wire by means of 2 inch or 3 inch rings as required by the number and size of the cables installed, spaced not more than 18 inches apart. The traffic signal cable may be suspended from the messenger with the use of 2 inch rings, or by a spiral wrapping of 3/32 inch galvanized steel wire, applied by a machine in a uniform spiral with a lay or pitch of no more than 18 inches. All overhead conductors shall have an 8 inch drip loop at the junction with a pole or signal head. At all electrical cable turns and drops, plastic tie wraps shall be used to insure against any change in physical relationship between the electrical cable and the span wire.

Service wires between a power supply and controller cabinet on a pole shall be connected to the pole by an approved cable grip and enter the poles by means of a wire inlet with a bushing. An 8 inch drip loop shall be formed in the wires at the entrance of the wire inlet. The installation of span wire shall be as shown on the plans.

2. Messenger for carrying signal cable between poles shall be installed as shown on the plans.

The suspension of cable shall conform to either the 2 inch or 3 inch ring or spiral wrapping methods, which are covered elsewhere in this specification.

11.14.04—Method of Measurement: Work under these items will be measured for payment by the actual number of linear feet of steel wire cable, of the type specified, installed and accepted in place with necessary appurtenances.

11.14.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Messenger" or "Span Wire," whichever applies, which price shall include messenger or span wire, wet porcelain insulators, pole bands, thimble eyebolts, nuts, washers, cable rings, all materials, tools, equipment, labor and work incidental thereto.

Pay Item	Pay Unit
Messenger Wire	l.f.
Span Wire (Type)	l.f.

SECTION 11.16
ILLUMINATED SIGNS

11.16.01—Description: This item shall consist of furnishing and installing an illuminated sign, of the type specified, at the location shown on the plans or as directed by the Engineer, and in accordance with these specifications.

11.16.02—Materials: The materials for this work shall be as specified in M.16.18.

11.16.03—Construction Methods: Erection of the sign shall be accomplished in a manner so as not to cause twisting, bending, deforming, or scratching of the sign or sign face. Signs shall be level, correctly aligned as indicated on the plans and shall be properly fastened to the structure or supports with the necessary hardware as indicated on the plans. Any sign panel damaged shall be repaired at the Contractor's expense.

11.16.04—Method of Measurement: This work will be measured for payment by the number of illuminated signs of the type and size specified, completed in place, accepted and operating.

11.16.05—Basis of Payment: This work shall be paid for at the Contract unit price each for “Illuminated Sign” complete in place, which price shall include mounting brackets, hardware, fittings, snap switch, all materials, equipment, paint, tools, labor, and work incidental thereto. Cable to the sign shall be paid under 11.13.

Pay Item	Pay Unit
Overhead Illuminated "Stop Ahead" Sign	ea.
Internally Illuminated Sign	ea.

SECTION 11.17
ALTERNATE FLASHING SIGNALS FOR WARNING SIGNS

11.17.01—Description: Work under this item shall consist of furnishing and installing alternate flashing signals for warning signs at the location shown on the plans or as directed by the Engineer.

11.17.02—Materials: The materials for the work shall be as specified in the following:

M.15.09 for Rigid Metal Conduit
M.16.06 for Traffic Signals
M.16.09 for Painting
M.16.10 for Cabinet
M.16.14 for Control Cable

11.17.03—Construction Methods: All internal wiring shall be #14 AWG stranded drawn copper. All wiring shall be neat and firm. All flasher inputs and outputs shall be wired to a terminal block. The signal indications and flasher cabinet shall be mounted on the sign supports in accordance with the dimensions and details shown on the plans.

11.17.04—Method of Measurement: This work will be measured for payment by the number of alternate flashing signals for warning signs completed and accepted in place. The conduit and cable installed on the line side of the cabinet will not be included for payment under this item.

11.17.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Alternate Flashing Signals for Warning Signs," which price shall include cable, signals, conduit, fittings, hardware, flasher cabinet complete with all necessary equipment, ground rod, armored ground cable, paint and painting, and all materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Alternate Flashing Signals for Warning Signs	ea.

SECTION 11.18
REMOVAL AND/OR RELOCATION OF TRAFFIC SIGNAL EQUIPMENT

11.18.01—Description: Work under this item shall consist of removal and, where required, relocation of existing traffic signal equipment at the location shown on the plans or as directed by the Engineer.

11.18.02—Materials: All materials required for this work shall meet the requirements of these specifications or the special provisions for the material in question; or if not so specified, they shall be of a quality satisfactory to the Engineer.

11.18.03—Construction Methods: Existing traffic signal equipment shall be carefully removed together with all fittings and attachments in such a manner as to safeguard all parts from damage or loss. The Contractor shall replace, at its own expense, all equipment which becomes damaged due to its operations.

Removal of existing signal and control equipment shall be coordinated with the installation of new signal and control equipment so as to maintain uninterrupted traffic signal operation at each intersection. The Contractor shall furnish and install signals, supports or other equipment necessary to provide for uninterrupted signal operation as directed by the Engineer.

Traffic signal equipment, fittings, wires, cable, conduit and all other materials shall be reinstalled and extended where necessary to maintain satisfactory operations.

Where attachment points for span wire or other signal equipment are on a utility pole, the Contractor shall secure from the owning utility company the permission to remove or install such attachments in accordance with their requirements.

Equipment shall be removed in such a manner as to cause no hazard to pedestrians, traffic or property. If necessary, flaggers shall be used to halt traffic briefly while work is in progress. When plans call for relocation of existing equipment, the work shall be accomplished with a minimum of down time.

All removed equipment, including wire and cable that is not reused at the location, is to be returned to the State at a location designated by the Engineer. Material, such as concrete foundations, etc. shall be suitably removed and disposed of by the Contractor. When poles, foundations, etc. are removed, the hole shall be backfilled with clean fill material, which shall be compacted and the ground restored to a grade and condition compatible with the surrounding area. Topsoil to a minimum depth of 4 inches shall be applied and seeded with perennial grass seed. If in a sidewalk area, the surfacing shall be with material compatible with the rest of the sidewalk. Where pressure sensitive vehicle detectors are to be removed, the entire detector frame shall be removed, the hole shall be backfilled and the pavement restored, in accordance with the appropriate sections of the Standard Specifications.

11.18.04—Method of Measurement: This work shall be at the Contract lump sum price for the completion of all work specified.

11.18.05—Basis of Payment: This work will be paid for at the Contract lump sum price for the "Removal and/or Relocation of Traffic Signal Equipment" which price shall include all removals, all relocations, all material, equipment and labor incidental to completion of this item.

Pay Item	Pay Unit
Removal and/or Relocation of Traffic Signal Equipment	l.s.

SECTION 11.30
ARROW BOARD

11.30.01—Description: Work under this item shall include furnishing and maintaining a trailer-mounted or truck-mounted arrow board, at the locations indicated on the plans or as directed by the Engineer.

11.30.02—Materials: A Materials Certificate for the arrow board shall be submitted to the Engineer. The arrow board shall meet the requirements of Type C Arrow Board in MUTCD Chapter 6F, and the following:

1. **Physical Characteristics of the Arrow Board**
    a. Arrow Board Display Dimensions - width 8 feet, height 4 feet
    b. Height above Roadway - Minimum 7 feet from the roadway to the bottom of the display, except on truck-mounted arrow boards, which shall be as high as practical
    c. Power Source - Battery or solar power, including backup
    d. Secure Controller - arrow boards shall be equipped with a lockable cabinet for controller storage
2. **Visual Characteristics of the Arrow Board**
    a. Matrix - Minimum of 15 illuminated elements
    b. Display Modes - flashing arrow, flashing double arrow, flashing caution and flashing alternating diamond caution
    c. Color - Non-reflective black background with yellow or amber elements
    d. Flash Rate - 25 to 40 flashes per minute
    e. Dimming - arrow board shall be equipped with a photocell for automatic sign dimming, with at least 50% from full brilliance, based on lighting conditions
    f. Legibility - arrow board brightness must provide for legibility within 1 mile

11.30.03—Construction Methods: The Contractor shall furnish, place, operate, and relocate the arrow board as required on the plans or as directed by the Engineer, in accordance with Chapter 6F of the MUTCD.

The Contractor shall maintain the arrow board in accordance with the ATSSA "Quality Standards for Temporary Traffic Control Devices and Features." Any arrow board that does not meet these guidelines shall be removed and replaced.

When the arrow board is no longer required, it shall be removed from the Site.

11.30.04—Method of Measurement: This work will be measured for payment by the number of calendar days that the arrow board is in place and in operation. When an arrow board is in operation for less than a day, such a period of time shall be considered to be a full day regardless of actual time in operation.

11.30.05—Basis of Payment: This work will be paid for at the Contract unit price per day for "Arrow Board," which price shall include furnishing, maintaining, relocating, removing the arrow board and its appurtenances, and all material, labor, tools and equipment incidental thereto.

Pay Item	Pay Unit
Arrow Board	day

SECTION 11.31
REMOTE CONTROL CHANGEABLE MESSAGE SIGN

11.31.01—Description: Work under this item shall include furnishing and maintaining a trailer-mounted, changeable message sign (CMS) at the locations indicated on the plans or as directed by the Engineer.

11.31.02—Materials: A Materials Certificate for the CMS shall be submitted to the Engineer. The CMS shall meet the requirements of MUTCD Chapters 2L and 6F and the following:

1. **Physical Characteristics of the CMS**
   a. Sign Display Dimensions - Minimum width 10 feet, minimum height 5.5 feet
   b. Height off Roadway - Minimum 7 feet from the roadway to the bottom of the display
   c. Sign Rotation - Mounting post shall be capable of 180 degree rotation
   d. Power Source - Battery or solar power, including backup
   e. Protection - CMS shall have a secure, lockable controller cabinet and remote, and panel controller password protection
2. **Visual Characteristics of the CMS Display**
   a. Sign Type - CMS shall have LED display
   b. Display - CMS shall be character, line or full matrix
   c. Color - CMS shall have black background with orange, yellow or amber legend
   d. Line - CMS shall be capable of displaying 3 lines and 8 upper-case characters per line
   e. Characters - Letter height shall be 18 inches and letter width shall be 12 inches; Single stroke
   f. Font - 5 × 7 pixels per character
   g. Dimming - CMS shall be equipped with a photocell for automatic sign dimming based on lighting conditions
   h. Memory - CMS shall be capable of storing a minimum of 50 messages
   i. Communication - CMS shall be capable of remote offsite programming
   j. Visibility and Legibility - CMS brightness must provide for visibility at 1/2 mile and be legible within 900 feet

11.31.03—Construction Methods: The Contractor shall furnish, place, operate, maintain and relocate the sign as required on the plans or as directed by the Engineer, in accordance with Chapter 6F of the MUTCD.

The message content and timing shall be in accordance with the CTDOT Portable Variable Message Signs Operations Guide. The Engineer must approve the message and location prior to sign operation.

When the sign is not displaying a message, it shall be relocated beyond the clear zone or shielded behind a traffic barrier and turned away from traffic.

The Contractor shall maintain the changeable message sign in accordance with the ATSSA "Quality Guidelines for Temporary Traffic Control Devices and Features." Any sign that does not meet these guidelines shall be removed and replaced.

When the sign is no longer required, it shall be removed from the Site.

11.31.04—Method of Measurement: This work will be measured for payment by the number of calendar days that the changeable message sign is in place and in operation. When a sign is in operation for less than a day, such a period of time shall be considered to be a full day regardless of actual time in operation.

11.31.05—Basis of Payment: This work will be paid for at the Contract unit price per day for "Remote Control Changeable Message Sign," which price shall include furnishing, maintaining, relocating, removing the sign and appurtenances, the remote controller, and all material, labor, tools and equipment incidental thereto.

Pay Item	Pay Unit
Remote Control Changeable Message Sign	day

SECTION 12.00
GENERAL CLAUSES FOR HIGHWAY SIGNING

12.00.01—General
12.00.02—Shop Drawings
12.00.03—Buried Cable
12.00.04—Negotiations with Utility Company
12.00.05—Sequence of Operations

12.00.01—General: It is the intent of the plans, specifications, and special provisions to provide for a complete signing system. The work shall include the furnishing of all materials incidental to the fabrication and erection of all signs and sign supports.

12.00.02—Shop Drawings: Before fabricating any portion of the Overhead and/or Cantilever Sign Supports, the Contractor shall submit shop drawings drawn to scale for approval in accordance with 1.05.02-3.

These drawings shall include but not be limited to the following information.

1. All pertinent dimensions.
2. Complete details of the Overhead and/or Cantilever Sign Supports including camber and material designations.

12.00.03—Buried Cable: The Contractor is hereby cautioned that within the limits of any Project an illumination contract may be in force or have been completed, in which case buried cable may be present at or near the proposed locations for overhead sign support foundations and the cable may be energized. Any damage to existing buried cable shall be repaired in a satisfactory manner at no additional cost to the State.

12.00.04—Negotiations with Utility Company: 10.00.12 is supplemented by the following:

The Contractor shall make all necessary arrangements with the District Electrical Maintenance Supervisor for locking and unlocking of the circuits on which any work is to be done, through the Engineer.

12.00.05—Sequence of Operations: Whenever a new or relocated sign, of any kind, is replacing an existing sign, the replaced sign shall be removed immediately after the new installation.

SECTION 12.01
OVERHEAD SIGN SUPPORT

12.01.01—Description
12.01.02—Materials
12.01.03—Construction Methods
12.01.04—Method of Measurement
12.01.05—Basis of Payment

12.01.01—Description: Work under this item shall consist of furnishing, fabricating, transporting and erecting galvanized overhead sign supports of the type specified, at the locations indicated, in accordance with the requirements of the plans or as ordered.

12.01.02—Materials: Materials for this work shall meet the requirements of M.03.05, M.06.02, M.18.02 and the Contract.

12.01.03—Construction Methods: Prior to the start of fabrication of the overhead sign supports, the Contractor shall, in the field, verify the location of the foundations and establish and verify all elevations, dimensions and longitudinal grades.

All welding shall meet the requirements of the current AWS Structural Welding Code.

All connections using high-strength bolts shall meet the requirements of 6.03.03-5(f) High Strength Bolted Connections.

Camber shall be interpreted as the curvature induced into the overhead sign support truss to offset the anticipated deflections when the truss is in its erected position and to provide for the permanent camber as indicated on the plans. Cambers shall be measured in this erected position and failure to meet the required amount of camber may be cause for rejection.

Prior to assembly, the top and bottom surface of the base plate shall be planed, or else the plate shall be hot straightened. The post shall be faced at the bottom end.

The top leveling nuts and the grout-leveling template shall be removed prior to placing the base plate and post assembly upon the foundation.

The base plate and post assembly shall be installed upon the foundation pedestal in accordance with the requirements of 6.03.03-5(c) Bearings.

After placing the base plate and post assembly upon the foundation, the anchoring nuts shall be installed "snug tight" and then tightened additionally in accordance with 6.03.03-5(f) High Strength Bolted Connections Turn-of-Nut Installation Method, modified as follows: Nut rotation from the snug tight condition shall be 1/4 turn.

The posts shall have a removable top cap, complete with set screws for proper attachment. The cap shall be provided with a weatherproof ventilation passageway to the interior of the post.

The galvanized Overhead Sign Support shall have damaged galvanized areas repaired in accordance with ASTM A780.

12.01.04—Method of Measurement: This work will be measured for payment by the number of units of "Overhead Truss Sign Supports" or "Overhead Cantilever Sign Supports" of the type specified, installed and accepted.

12.01.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Overhead Truss Sign Support" or "Overhead Cantilever Sign Support" of the type specified, complete in place, which price shall include all materials, equipment, labor, tools, and work incidental thereto.

Pay Item	Pay Unit
Overhead Truss Sign Support (Type)	ea.
Overhead Cantilever Sign Support (Type)	ea.

NOTES

SECTION 12.02
OVERHEAD SIGN SUPPORT FOUNDATION

12.02.01—Description: Work under this item shall consist of the construction of foundations for overhead sign supports including the removal and satisfactory disposal of all materials required for excavation for the foundations, and backfilling with suitable material, in conformity with the requirements of the plans or as ordered, and in conformance with these specifications.

12.02.02—Materials: Class PCC03340 Concrete shall meet the pertinent requirements of 6.01 and M.03.
Non-shrink, non-staining grout shall meet the requirements of M.03.05.
Reinforcing steel shall meet the requirements of 6.02 and M.06.01.
Rigid metal conduits, ground rod sleeves and related hardware shall be as shown on plans.
Anchor bolts shall meet the requirements of ASTM A449.
Leveling nuts and nuts for anchor bolt assemblies shall meet the requirements of M.18.02.
Pedestal Grout Leveling Templates shall meet the requirements of M.18.02.

12.02.03—Construction Methods: Prior to start of construction of the foundations the Contractor shall, in the field, verify the location of the foundations and establish and verify all elevations and dimensions.
Excavation for the footings shall conform to the neat lines of the base as shown on the plans. All steel reinforcement shall be accurately placed in the position shown on the plans and firmly held during placing and setting of concrete.
The footing of the concrete foundation shall be placed without forms except where unusual soil conditions prevent excavation to neat lines as shown on the plans. Where unusual soil conditions make the use of forms necessary, in the opinion of the Engineer, the cost thereof shall be considered included in the cost of the foundations.
Where rock ledge or large boulders are exposed before excavation has attained the required depth, the Engineer may order rock anchors installed.
Where rock anchors are required, they shall be paid for in conformance with 1.04.05.
After the concrete for the footing has sufficiently set, in the opinion of the Engineer, the portion of the concrete foundation, hereinafter designated as the stem, shall be formed to conform to the details shown on the plans. The forms shall include a template to hold the required anchor bolt assemblies, ground rod sleeve and conduit in their correct position.
Anchor bolt assemblies shall conform to the requirements shown on the plans and shall be embedded in the concrete which shall be placed to within the minimum distance of the finished surface of the stem as shown on the plans forming a construction joint for the non-shrink grout. Each bolt of the anchor bolt assembly shall be fitted with 2 leveling nuts.
Construction joints other than those shown on the plans will not be permitted.
The top and bottom surfaces of the grout-leveling template shall be planed, or else the plate shall be hot straightened.
The grout-leveling template shall be clamped in position by 2 leveling nuts at each anchor bolt. These leveling nuts shall be adjusted to assure a truly level finished foundation surface at the proper elevation.
The space between the grout-leveling template and pedestal concrete shall be grouted with non-shrink, non-staining grout. The grout shall be forced by rodding or by other suitable means from one side of the template until it flows freely out at the opposite side. Care shall be taken to eliminate voids underneath the template.
Before grouting, the concrete areas that will be in contact with the grout shall be cleaned of all loose or foreign material that would in any way prevent bond between the grout and concrete surfaces. These concrete surfaces shall be kept thoroughly moistened until the surface is completely saturated prior to placing the grout. The grout shall be moisture cured for 7 days. No load shall be allowed on the grout that has been in place for less than 7 days unless otherwise approved by the Engineer.
The grout-leveling template shall remain in place for a minimum of 2 days after placement of the grout.
All conduits shall be capped with insulated bushings before placing the grout. Where the signs are not to be illuminated, the caps shall be made fully watertight. The number of conduits in the foundation will be determined by the Engineer.
The concrete shall be finished in conformance with the pertinent requirements of 6.01.03-II-10.
Backfill shall be placed and thoroughly tamped to the elevation shown on the plans.

12.02.04—Method of Measurement: This work will be measured for payment by the number of foundation units, of the type specified, completely installed and accepted. Each overhead cantilever sign

support shall have 1 foundation unit and each overhead truss sign support shall have 2 foundation units.

12.02.05—Basis of Payment: The work will be paid for at the Contract unit price each for "Overhead Sign Support Foundation," of the type specified, complete in place, which price shall include all materials, excavation, backfill, equipment, labor, tools and work incidental thereto including rigid metal conduits and ground rod sleeve with 5/8 inch × 12 foot ground rod driven in position as directed by the Engineer.

Pay Item	Pay Unit
Overhead Truss Sign Support Foundation (Type)	ea.
Overhead Cantilever Sign Support Foundation (Type)	ea.

SECTION 12.03
SIDE MOUNTED SIGN FOUNDATION

12.03.01—Description: Work under this item shall consist of all necessary excavation including rock excavation, backfill, when necessary, and the construction of side mounted sign foundations conforming to the details shown on the plans.

12.03.02—Materials: Concrete shall be Class PCC03340 Concrete meeting the pertinent requirements of 6.01 and M.03.

Reinforcing steel shall meet the pertinent requirements of M.06.01.

Stub post assemblies shall be steel meeting ASTM A36 and shall be galvanized after fabrication in accordance with ASTM A123.

All other material shall meet the requirements shown on the plans.

12.03.03—Construction Methods: Excavation for the foundation shall be to the neat lines as shown on the plans.

The concrete foundation shall be placed without forms except where unusual soil conditions prevent excavation to neat lines as shown on the plans. The forms shall be fastened in place before the placing of any concrete so that the resulting foundation will be monolithic.

The anchor bolts or stub post assembly and reinforcing steel shall be fabricated and preset as shown on the plans. After pre-setting the stub post, concrete shall be carefully placed and consolidated around the post.

12.03.04—Methods of Measurement: This work will be measured for payment by the number of units completely installed and accepted.

12.03.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Side-Mounted Sign Foundation," which price shall include all material, excavation including rock excavation, stub post assembly, breakaway coupling system, forms when necessary, labor and work incidental thereto.

Pay Item	Pay Unit
Side-Mounted Sign Foundation	ea.

SECTION 12.04
SIGN-PANEL OVERLAY

12.04.01—Description: Work under this item shall consist of furnishing and installing a plywood overlay of the type specified to cover an existing sign where shown on the plans or where directed by the Engineer.

12.04.02—Materials: Plywood shall have a minimum thickness of 1/4 inch and shall be exterior grade A-C as designated by APA.

The wood preservative shall be of a type that will have no adverse effect on paint adhesion and will not cause future paint discoloration.

Primer shall meet the requirements of A-A-2336.

The enamel paint to be used for the finish coat shall be as specified in M.18.08.

Copy shall meet the requirements contained in M.18.09 or M.18.10 of the Contract.

12.04.03—Construction Methods: The plywood overlay shall completely cover the existing sign, including the exit crown panel. The plywood sheets shall be joined together to form a single overlay by means of 1 inch x 4 inches construction grade fir wood battens securely fastened to adjoining panels with 1 inch galvanized wood screws. The battens shall be fastened to the Grade C back face of the overlay.

Before assembly and before painting, all wood shall be treated with a coat of wood preservative on all surfaces.

The entire overlay surface shall be painted with 1 coat of primer and 1 coat of enamel.

The plywood shall remain in place for the duration of the Project.

All work fabricating and clamping the plywood sign-panel overlay shall be done to ensure that no damage occurs to the existing sign.

12.04.04—Method of Measurement: Sign-panel overlay of the type specified will be measured for payment by the actual number of square feet installed and accepted.

12.04.05—Basis of Payment: This work will be paid for at the Contract unit price per square foot for "Sign Panel Overlay," of the type specified complete in place, which price shall include all materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Sign Panel Overlay—Plain	s.f.
Sign Panel Overlay—with Copy	s.f.

SECTION 12.05
DELINEATORS

12.05.01—Description: This item shall consist of furnishing and installing delineators of retroreflective material and delineator posts or bridge rail mountings in accordance with the requirements of the plans or as ordered.

12.05.02—Materials: The materials for delineators shall be as specified in M.18.07 and M.18.13. Retroreflective sheeting shall be as specified in M.18.09.

12.05.03—Construction Methods: Delineators shall be mounted on the posts or bridge rail mountings with fastening devices as shown in the details on the plans. The spacing and color of the delineators shall be in accordance with the plans or as directed by the Engineer.

The delineator post shall be driven or the hole augered and the backfill thoroughly tamped. The posts shall be plumb above ground, and any post bent or otherwise damaged to the extent that it is, in the opinion of the Engineer, unfit for use, shall be removed from the Site and replaced by the Contractor at its own expense. The delineator retroreflective units shall be positioned to be clearly visible for a distance of 1,000 feet during the hours of darkness, under normal weather and atmospheric conditions, when illuminated by the upper beams of standard automobile headlights when the vehicle is located in the adjacent traffic lane approaching the delineator.

12.05.04—Method of Measurement: This work will be measured for payment by the number of delineators of the type specified, installed and accepted.

12.05.05—Basis of Payment: This work will be paid for at the Contract unit price each for "Delineator," of the type specified, complete in place, which price shall include the delineator, delineator post or bridge rail mounting and materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Type DE-( ) Delineator	ea.

SECTION 12.06
REMOVAL OF EXISTING SIGNING
REMOVAL OF EXISTING OVERHEAD SIGNING

12.06.01—Description: This work shall consist of removing designated side-mounted signs, supports and foundations; plywood and/or sheet aluminum regulatory, warning and guide signs and metal sign posts; delineators and delineator posts; overhead signs, supports and foundations; and structure mounted sign and supports where indicated on the plans or as directed by the Engineer.

12.06.02—Vacant

12.06.03—Construction Methods: Signs constructed of plywood and the hardware used for attachment to the supports shall be removed and disposed of by the Contractor.

Signs constructed of aluminum and the hardware used for attachment to the supports shall be removed and disposed of by the Contractor, unless otherwise specified in the Special Provisions.

Delineators and delineator posts shall be removed and disposed of by the Contractor.

Sign supports and sign illumination appurtenances shall be removed and disposed of by the Contractor unless otherwise specified in the Special Provisions.

When signs and supports are to be delivered to a salvage location specified in the Special Provisions, the Contractor shall transport and carefully unload the signs and supports where directed by the storekeeper.

Overhead sign support foundations shall be removed to a depth of 6 inches below finished grade. Anchor bolts shall be cut approximately flush with the top of the remaining portion of foundation. The concrete and portions of the anchor bolts removed from the foundation shall be disposed of by the Contractor.

For-side-mounted sign foundations, the stub post or anchor bolts and concrete foundations shall be removed to a depth of 6 inches below finished grade. The portion of stub posts or anchor bolts and the concrete removed shall be disposed of by the Contractor. Where sign support foundations exist without stub posts or anchor bolts, the Contractor will have the option of removing the concrete foundation in its entirety or remove the concrete and post to a depth of 6 inches below finished grade. Whichever option the Contractor chooses, the Contractor shall remove the excavated material from the Site.

After removal of all concrete, stub post portions and anchor bolt portions, top soil shall be placed to 1 inch above finished grade, then fertilized and seeded. Fertilizing and seeding shall be in accordance with pertinent provisions of 9.50. Where necessary, backfill shall be placed to 6 inches below grade before topsoil is placed.

Supports located in bituminous concrete or Portland cement concrete areas shall be removed flush with the surrounding area and disposed of by the Contractor. Posts attached to bridge parapets or railings shall be unbolted and the anchor bolts removed flush with the bridge parapet and all material shall be disposed of by the Contractor.

12.06.04—Method of Measurement: This work will be at the Contract lump sum price, which shall include all signs, sign supports and foundations, if applicable, and delineators designated for removal in the Contract.

12.06.05—Basis of Payment: This work will be paid for at the Contract lump sum price for "Removal of Existing Signing" or "Removal of Existing Overhead Signing" complete including the disposal of material unless otherwise specified, and all equipment, material, tools and labor incidental thereto. The price shall also include the transporting and careful unloading of salvaged material at the location designated in the Special Provisions and all equipment, material, tools and labor incidental thereto.

Pay Item	Pay Unit
Removal of Existing Signing	l.s.
Removal of Existing Overhead Signing	l.s.

SECTION 12.07
SIGN FACE - EXTRUDED ALUMINUM

12.07.01—Description: Work under this item shall consist of furnishing and installing sign face extruded aluminum with the Type IV retroreflective sheet facing at locations indicated on the plans or as directed by the Engineer.

12.07.02—Materials: Materials for this work shall meet the requirements of M.18.09, M.18.10, M.18.11 and M.18.12.

Structural steel shall meet the requirements of ASTM A36. All members shall be galvanized after fabrication in accordance with the requirements of ASTM A123.

Zinc paint shall meet the requirements of ASTM A780.

Data Labels shall meet the requirements of M.18.16.

12.07.03—Construction Methods: Erection of completed sign panels shall be accomplished in a manner so as not to cause twisting, bending or deforming of sign panels or scratching of the sign face. Any sign panel damaged shall be repaired or replaced at the Contractor's expense. Sign panels shall be level, correctly aligned as indicated on the plans, and shall be properly fastened to the structure or supports with the necessary hardware as indicated on the plans.

Retroreflective sheeting shall be applied in such a manner that the finished sign will be wrinkle and bubble free. No splices of the retroreflective sheeting will be permitted on any sign face less than 30 square feet in area with 1 dimension of 4 feet or less and no more than 1 splice will be permitted on any sign without the approval of the Engineer.

Extruded aluminum signs shall be attached to existing supports with new post clip assemblies consisting of post clips, bolt, nut and washer as shown on the plans. All other hardware used for attachment of the overhead signs to the supports may remain in use and be relocated on the structure as required. Check and tighten all bolts and nuts for attachment hardware which is to remain in use.

New extruded aluminum signs shall be vertically centered on existing supports. Overhead sign support brackets (Z bar, I bar, channel shape or angle iron) that project beyond the top or bottom of a new sign shall be cut to fit even with the edge of the new sign, preferably at the bottom. If necessary, cuts may be made at both ends.

Where overhead signs having a vertical dimension exceeding the length of the existing sign support brackets, the existing brackets shall be removed and replaced with new vertical brackets having the length equal to the sign height.

All galvanized areas damaged by field cuts or welds shall receive zinc paint that is brush applied to achieve a dry film thickness from 3 to 6 mils.

All overhead sign erections shall be made immediately upon removal of the sign being replaced.

All overhead sign foundations shall be field staked. The locations of the stakes shall be accepted by an Engineer from the Division of Traffic Engineering, a minimum of 7 days prior to installation.

For all side mounted signs, the edge of the sign closest to the roadway and the sign foundation shall be field staked and accepted by an Engineer from the Division of Traffic Engineering, a minimum of 7 days prior to installation.

For side-mounted signs, the offset to the near edge of the sign face shall exceed the maximum deflection of the guide rail, unless otherwise shown on the plans or directed by the Engineer.

The Contractor shall affix data labels to the back of each State-owned and maintained sign in the vicinity of the lower left hand corner or quadrant. The Contractor shall punch the month and year of sign fabrication and installation on each data label prior to affixing to the back of the sign.

12.07.04—Method of Measurement: This work will be measured for payment by the number of square feet of sign face-extruded aluminum of the type specified installed and accepted.

12.07.05—Basis of Payment: This work will be paid for at the Contract unit price per square foot for "Sign Face-Extruded Aluminum" of the type specified complete in place, which price shall include all data labels, materials, equipment, labor and work incidental thereto. Also included shall be any additional vertical sign support brackets required to attach new signs to existing supports.

Pay Item	Pay Unit
Sign Face - Extruded Aluminum (Type)	s.f.

SECTION 12.08
SIGN FACE - SHEET ALUMINUM

12.08.01—Description: Work under this item shall consist of furnishing and installing sign face-sheet aluminum signs of the type specified, metal sign posts, span-mounted sign brackets and mast arm-mounted sign brackets at locations indicated on the plans or as directed by the Engineer.

12.08.02—Materials: Retroreflective sheeting shall meet the requirements of M.18.09, Type IV or IX. Sheet aluminum sign blanks shall meet the requirements of M.18.13. Silk screening of Type IV or IX retroreflective sheeting shall meet the requirements specified by the retroreflective sheeting manufacturer.

Metal sign posts shall meet the requirements of M.18.14. Sign mounting bolts shall meet the requirements of M.18.15. Data Labels shall meet the requirements of M.18.16.

12.08.03—Construction Methods: Placement and dimensions of copy, border and mounting holes shall be as shown in details of the Department of Transportation for Regulatory Warning and Guide signs which are available for inspection at the Department of Transportation office. Non-reflective copy, border and background shall be applied by the silk-screen process in a manner specified by the retroreflective sheeting manufacturer. The silk screening of all copy, border and background on Type IV or IX retroreflective sheeting shall be accomplished prior to the application of the retroreflective sheeting to the finished aluminum sign blank. Type IV or IX retroreflective sheeting shall be of the heat activated adhesive type and shall be applied in a manner specified by the retroreflective sheeting manufacturer.

Retroreflective sheeting shall be applied in such a manner that the finished sign will be wrinkle and bubble free. No splices of the retroreflective sheeting will be permitted on any sign face less than 30 square feet in area with 1 dimension of 4 feet or less and no more than 1 splice will be permitted on any sign without the approval of the Engineer.

Direct application of cutout Type IV or IX retroreflective sheeting copy and border shall meet the requirements specified by the retroreflective sheeting manufacturer. Cutout copy and border shall be applied directly to clean, dust free retroreflective sheeting background panels. Borders shall be cut neatly and butt-joined at corners and panel joints. Type IV or IX retroreflective sheeting used for direct applied cutout copy and border shall be uniform in brightness and color.

The fabrication of aluminum sign blanks including cutting to size and shape and the punching of mounting holes shall be completed prior to metal degreasing and the application of reflective sheeting. Aluminum sign blanks shall be free of buckles, warp, dents, cockles, burrs and defects resulting from fabrication. Span-mounted sign brackets and mast arm-mounted sign brackets shall be installed as shown on the plans.

After complete fabrication of the sign as indicated on the plans and in compliance with the requirements contained in the Specifications, the sign shall be mounted on the type of support designated on the plans after the support has been satisfactorily installed at its proper location. The reinforcing plate shall be installed as shown on the plans.

Metal sign posts shall be driven or the holes augered and the backfill thoroughly tamped after the posts have been set level and plumb.

The Contractor shall affix data labels to the back of each State-owned and maintained sign in the vicinity of the lower left hand corner or quadrant. The Contractor shall punch the month and year of sign fabrication and installation on each data label prior to affixing to the back of the sign.

12.08.04—Method of Measurement: This work will be measured for payment by the number of square feet of sign face-sheet aluminum of the type specified, installed and accepted.

12.08.05—Basis of Payment: This work will be paid for at the Contract unit price per square foot for "Sign Face-Sheet Aluminum" of the type specified complete in place, which price shall include the completed sign, metal sign post(s), span-mounted sign brackets and mast arm-mounted brackets, mounting hardware, including reinforcing plates, data labels, and all materials, equipment, labor and work incidental thereto.

Pay Item	Pay Unit
Sign Face - Sheet Aluminum (Type)	s.f.

SECTION 12.08
SIGN FACE - SHEET ALUMINUM

12.08.01—Description: Work under this item shall consist of furnishing and installing sign face-sheet aluminum signs of the type specified, metal sign posts, span-mounted sign brackets and mast arm-mounted sign brackets at locations indicated on the plans or as directed by the Engineer.

12.08.02—Materials: Retroreflective sheeting shall meet the requirements of M.18.09. Sheet aluminum sign blanks shall meet the requirements of M.18.13. Silk screening of retroreflective sheeting shall meet the requirements specified by the retroreflective sheeting manufacturer.

Metal sign posts shall meet the requirements of M.18.14. Sign mounting bolts shall meet the requirements of M.18.15. Data Labels shall meet the requirements of M.18.16.

12.08.03—Construction Methods: Placement and dimensions of copy, border and mounting holes shall be as shown in details of the Department of Transportation for Regulatory Warning and Guide signs which are available for inspection at the Department of Transportation office. Non-reflective copy, border and background shall be applied by the silk-screen process in a manner specified by the retroreflective sheeting manufacturer. The silk screening of all copy, border and background on retroreflective sheeting shall be accomplished prior to the application of the retroreflective sheeting to the finished aluminum sign blank. Retroreflective sheeting shall be of the heat activated adhesive type and shall be applied in a manner specified by the retroreflective sheeting manufacturer.

Retroreflective sheeting shall be applied in such a manner that the finished sign will be wrinkle and bubble free. No splices of the retroreflective sheeting will be permitted on any sign face less than 30 square feet in area with one dimension of 4 feet or less, and no more than one splice will be permitted on any sign without the approval of the Engineer.

Direct application of cutout retroreflective sheeting copy and border shall meet the requirements specified by the retroreflective sheeting manufacturer. Cutout copy and border shall be applied directly to clean, dust free retroreflective sheeting background panels. Borders shall be cut neatly and butt-joined at corners and panel joints. Retroreflective sheeting used for direct applied cutout copy and border shall be uniform in brightness and color.

The fabrication of aluminum sign blanks including cutting to size and shape and the punching of mounting holes shall be completed prior to metal degreasing and the application of reflective sheeting. Aluminum sign blanks shall be free of buckles, warp, dents, cockles, burrs and defects resulting from fabrication. Span-mounted sign brackets and mast arm-mounted sign brackets shall be installed as shown on the plans.

After complete fabrication of the sign as indicated on the plans and in compliance with the requirements contained in the Specifications, the sign shall be mounted on the type of support designated on the plans after the support has been satisfactorily installed at its proper location. The reinforcing plate shall be installed as shown on the plans.

Metal sign posts shall be driven or the holes augered and the backfill thoroughly tamped after the posts have been set level and plumb.

The Contractor shall affix data labels to the back of each State-owned and maintained sign in the vicinity of the lower left hand corner or quadrant. The Contractor shall punch the month and year of sign fabrication and installation on each data label prior to affixing to the back of the sign.

12.08.04—Method of Measurement: This work will be measured for payment by the number of square feet of sign face-sheet aluminum of the type specified, installed and accepted.

12.08.05—Basis of Payment: This work will be paid for at the Contract unit price per square foot for "Sign Face-Sheet Aluminum" of the type specified complete in place, which price shall include the completed sign, metal sign post(s), span-mounted sign brackets and mast arm-mounted brackets, mounting hardware, including reinforcing plates, data labels, and all materials, equipment, labor and work incidental thereto.

Pay Item	Pay Unit
Sign Face - Sheet Aluminum (Type)	s.f.

SECTION 12.09
PAINTED PAVEMENT MARKINGS

12.09.01—Description: This item shall consist of furnishing and installing painted pavement markings, hot applied painted pavement markings and painted legend, arrows and markings, of the type and color specified at the locations indicated on the plans and in conformity with the plans, these specifications, and as directed by the Engineer.

Painted legend, arrows, and markings includes paint installed with a hand striping machine such as: stop bars, crosswalks, parking stalls, lane arrows, legends, markings within gore areas, and painting of paved islands or medians.

Painted pavement markings and hot applied painted pavement markings include paint installed with a truck-mounted painting machine such as center lines, lane lines and shoulder lines.

The exact location for passing zones will be determined by the Engineer prior to the application of the pavement markings. The Engineer shall be notified a minimum of 2 weeks before the anticipated date of painting, to allow time for the determination of the passing zone locations.

12.09.02—Materials: Materials for this work shall meet the requirements of M.07.20 for waterborne pavement marking paint, M.07.21 for hot-applied waterborne pavement marking paint and M.07.30 for glass beads, Type 1 (smaller beads).

12.09.03—Construction Methods: Pavement areas to be painted shall be dry and sufficiently cleaned of sand and road debris so as to provide an acceptable bond between the paint and the pavement.

Paint shall be applied at a rate of 100 to 115 s.f./gal., with glass beads Type 1 (smaller beads) applied at a rate of 6 lb./gal. of paint for painted pavement markings and painted legend, arrows, and markings, and 8 lb./gal. of paint for hot-applied painted pavement markings.

Hot-applied paint shall be applied at a temperature of 130°F to 145°F at the spray gun.

All painting shall be performed in a neat and workmanlike manner. The lines shall be sharp and clear with no feathered edging or fogging, and precautions shall be taken to prevent tracking by tires of the striping equipment. Paint shall be applied parallel to the centerline or as shown on the plans with no unsightly deviations.

After application, the paint shall be protected from crossing vehicles for a time at least equivalent to the drying time of the paint.

12.09.04—Method of Measurement: Painted pavement markings and hot-applied pavement markings will be measured for payment by the number of linear feet of paint applied by a truck-mounted painting machine on the pavement and accepted. Painted legend, arrows and markings will be measured for payment by the number of square feet of paint installed with a hand striping machine on the pavement and accepted.

12.09.05—Basis of Payment: This work will be paid for at the Contract unit price per square foot for "Painted Legend, Arrows and Markings" and per linear foot of paint for "Painted Pavement Markings" and "Hot-Applied Painted Pavement Markings," of the width and color specified, installed on the pavement and accepted. This price shall include all pre-marking layout, cleaning of pavement, paint, glass beads, application of paint and glass beads, protection during drying and all materials, equipment, tools and labor incidental thereto.

Pay Item	Pay Unit
Painted Legend, Arrows and Markings	s.f.
Painted Pavement Markings (Width) (Color)	l.f.
Hot-Applied Painted Pavement Markings (Width) (Color)	l.f.

SECTION 12.10
EPOXY RESIN PAVEMENT MARKINGS

12.10.01—Description
12.10.02—Materials
12.10.03—Construction Methods
12.10.04—Method of Measurement
12.10.05—Basis of Payment

12.10.01—Description: This item shall consist of furnishing and installing retroreflective white and yellow epoxy resin pavement markings of the width and color specified and epoxy resin pavement markings, symbols and legends at the locations indicated on the plans, in conformity with the plans and as directed by the Engineer.

Epoxy resin pavement markings include epoxy resin installed with a truck-mounted machine, such as center lines, lane lines, and shoulder lines.

Epoxy resin pavement markings, symbols and legends installed with a hand striping machine include stop bars, crosswalks, parking stalls, lane arrows, legends, and markings within areas such as paved islands, gore areas and paved medians.

The exact location for passing zones will be determined by the Engineer prior to the application of the pavement markings. The Contractor shall notify the Engineer of the anticipated date of installation at least 2 weeks prior to that date, to allow time for the determination of the passing zone locations.

12.10.02—Materials: Materials for this work shall meet the requirements of M.07.22.

12.10.03—Construction Methods:

1. Equipment: Equipment furnished shall include an applicator truck of adequate size and power, together with

(a) remote application equipment designed to apply an epoxy resin material in a continuous pattern and
(b) portable glass bead applicators, 1 for each size bead, designed to provide uniform and complete coverage of the epoxy binder by a controlled free-fall method. Pressurized glass bead application shall not be used.

Before epoxy color is changed, equipment shall be cleaned out sufficiently to ensure that the color of material applied will be correct.

When working on a highway with more than 1 lane in either direction, the applicator truck (striper) shall have a permanently mounted direction variable illuminated arrow board, fully operational and visible to approaching traffic. There will be no additional payment for the arrow board, but the cost shall be included in the bid price for this item.

For markings applied on pavements over 1 year old, equipment furnished shall also include a power washing machine capable of cleaning the pavement with a pressure of 2,400 to 2,800 psi with water heated to between 180 and 195°F. No chemicals shall be added to the water used in the process. The power washer shall be equipped with a turbo blast tip with an oscillating head and shall be capable of supplying a minimum of 5 gal./minute gun.

All guns on the spray carriages shall be in full view of the operator(s) during operation.

Each vehicle furnished shall include at least 1 experienced operator, who shall be fully knowledgeable about all equipment operations and application techniques.

The Contractor shall also furnish 1 technical expert, who shall be fully knowledgeable about all equipment operations and application techniques, to oversee the Project operation.

2. Procedures: Pavement markings shall be applied in accordance with the details shown on the plans and the control points established by the Contractor and approved by the Engineer.

The road surface shall be cleaned at the direction of the Engineer just prior to application. Pavement cleaning shall consist of power washing using clean water heated to between 180 and 195°F at a pressure of 2,400 to 2,800 psi. The areas to be power washed shall include all areas where epoxy marking symbols and legends (including stop bars and crosswalks) are to be applied and at least 1 inch beyond the area to be marked. The surface shall be cleaned to the satisfaction of the Engineer. For other pavement areas, cleaning shall consist of brushing with rotary broom (non-metallic), and any additional work as recommended by the material manufacturer and acceptable to the Engineer. New Portland cement concrete surfaces shall be cleaned by abrasive blasting to remove any surface treatments or laitance. New bituminous concrete surfaces are not to be power washed.

All surfaces that are power washed shall be allowed to dry sufficiently prior to the application of the

epoxy markings. The areas to be marked shall be broom cleaned immediately prior to the application of the epoxy markings. Glass beads shall be applied immediately after application of the epoxy resin marking to provide an immediate no-track system.

The Contractor will place necessary "spotting" at appropriate points to provide horizontal control for striping and to determine necessary starting and cutoff points. Broken line intervals will not be marked. Longitudinal joints, pavement edges and existing markings shall serve as horizontal control when so directed.

A tolerance of 0.25 inch under or 0.25 inch over the specified width shall be allowed for striping provided the variation is gradual and does not detract from the general appearance. Alignment deviations from the control guide shall not exceed 2 inches provided the variation is gradual and does not detract from the general appearance. Material shall not be applied over a longitudinal joint. Establishment of application tolerances shall not relieve the Contractor of the responsibility to comply as closely as practicable with the planned dimensions.

Operations shall be conducted only when the road surface temperature is at least 40°F or as allowed by the Engineer. They shall be discontinued during periods of rain, and shall not continue until the Engineer determines that the pavement surface is dry enough to achieve adhesion.

The epoxy shall be uniformly applied to the surface to be marked to ensure a wet film thickness of the applied epoxy, without glass beads, of 20 mils +/- 1 mil.

Glass beads meeting the requirements of Type 4 (larger beads) shall be applied and the rate shall be 12 lb./gal. of epoxy pavement marking material, immediately followed by a second drop of glass beads meeting the requirements of Type 1(smaller beads) applied and the rate shall be 13 lb./gal. of epoxy pavement marking material. For installation of crosswalk pavement markings, only glass beads meeting the requirements of Type 1 (smaller beads) shall be applied and the rate shall be 25 lb./gal. of epoxy pavement marking material. Traffic cones or other acceptable method shall be used to protect the pavement markings until cured.

Time to No-Track: The material shall be in "no-tracking" condition within 15 minutes, or as allowed by the Engineer. The no-tracking time shall be determined by passing over the line with a passenger car or pickup truck in the simulated passing maneuver. A marking showing no visual deposition of the material to the pavement surface when viewed from a distance of 50 feet shall be considered as showing "no-tracking" and meeting this requirement for time to no-track.

When stencils are used during the application of epoxy markings, care must be used when removing the stencils so that the epoxy resin does not drip on the road, sidewalk, grass, or other surfaces, and so that the applied markings have edges which are clean, straight and neat.

Epoxy resin pavement markings may be applied over existing painted markings provided they are sufficiently worn to allow adequate adhesion. If required by the Engineer, existing plastic, thermoplastic, epoxy or freshly painted markings shall be removed prior to the application of epoxy markings. Payment for removal will be made under the item "Removal of Pavement Markings."

3. Initial Performance: The retroreflectivity of the markings applied must be measured by the Contractor using the procedures and equipment detailed below 3 to 14 days after installation. A Certified Test Report (CTR), in accordance with 1.06.07, must be submitted to the Engineer no later than 10 days after the measurements are taken.

Test Lots - The following test lots will be randomly selected by the Engineer to represent the line markings applied:

Table 12.10.03-3.1: Line Test Lots

Length of line	Number of Lots	Length of Test Lot
< 1000 feet	1	Length of Line
< 1.0 mile	1	1000 feet
≥ 1.0 mile	1 per 1.0 mile	1000 feet

Measurement Equipment and Procedure

Portable Retroreflectometer

1. Skip line measurements shall be obtained for every other stripe, taking no more than 2 readings per stripe with readings no closer than 20 inches from either end of the marking.
2. Solid line test lots shall be divided into 10 sub-lots of 100 foot length and measurements obtained at 1 randomly selected location in each sublot.

3. For symbols and legends, 10% of each type shall be measured by obtaining 5 measurements at random locations on the symbol or legend.
4. The Apparatus and Measurements shall be made in accordance with ASTM E1710 (Standard Test Method for Measurement of Retroreflective Pavement Marking Materials with CEN-Prescribed Geometry Using a Portable Retroreflectometer) and evaluated in accordance with ASTM D7585 (Standard Practice for Evaluating Retroreflective Pavement Markings Using Portable Hand-Operated Instruments).

Mobile Retroreflectometer

1. Calibration of the instruments shall be in accordance with the manufacturer's instructions.
2. Retroreflectivity shall be measured in a manner proposed by the Contractor and approved by the Engineer. The basis of approval of the test method will be conformance to a recognized standard test method or provisional standard test method.

The measurements shall be obtained when the pavement surface is clean and dry and shall be reported in millicandelas per square foot per foot candle - mcd/ft^2/fc. Measurements shall be obtained sequentially in the direction of traffic flow.

Additional Contents of Certified Test Report

The CTR shall also list:

1. Project and Route number
2. Geographical location of the test site(s), including distance from the nearest reference point
3. Manufacturer and model of retroreflectometer used
4. Most recent calibration date for equipment used
5. Grand Average and standard deviation of the retroreflectivity readings for each line, symbol or legend

Minimum Initial Performance:

In order to be accepted, all epoxy resin pavement markings must meet the following minimum retroreflectivity reading requirement:

White Epoxy (except Crosswalks): minimum retroreflectivity reading of 400 mcd/ft^2/fc
Yellow Epoxy: minimum retroreflectivity reading of 325 mcd/ft^2/fc
Crosswalks: minimum retroreflectivity reading of 250 mcd/ft^2/fc

At the discretion of the Engineer, the Contractor shall replace, at its expense, such amount of lines, symbols and legends that the grand average reading falls below the minimum value for retroreflectivity. The Engineer will determine the areas and lines to be replaced. The cost of replacement shall include all materials, equipment, labor and work incidental thereto.

12.10.04—Method of Measurement: Epoxy resin pavement markings shall be measured for payment by the actual number of linear feet of epoxy resin pavement markings installed on the pavement and accepted by the Engineer. Epoxy resin pavement markings, symbols and legends will be measured for payment by the actual number of square feet of epoxy resin pavement markings, symbols and legends installed on the pavement and accepted by the Engineer.

12.10.05—Basis of Payment: This work shall be paid for at the Contract unit price per linear foot for "Epoxy Resin Pavement Markings" of the width and color specified, or the Contract unit price per square foot for "Epoxy Resin Pavement Markings, Symbols and Legends" installed on the pavement and accepted. These prices shall be for all the work required by this Section and all materials, equipment, tools and labor incidental thereto. Payment will not be made for pavement markings affected by Contractor error and ordered removed.

Pay Item	Pay Unit
(Width) (Color) Epoxy Resin Pavement Markings	l.f.
Epoxy Resin Pavement Markings, Symbols and Legends	s.f.

NOTES

SECTION 12.11
REMOVAL OF PAVEMENT MARKINGS

12.11.01—Description: This item shall consist of removal of pavement markings, of the type specified, as called for on the plans or as directed by the Engineer.

12.11.03—Construction Methods: Pavement markings shall be removed from the pavement by any method that does not materially damage the surface or texture of the pavement. Any damage to the pavement surface caused by pavement marking removal shall be repaired by the Contractor at its expense by methods acceptable to the Engineer. Sand or other material deposited on the pavement as a result of removing pavement markings shall be removed as the work progresses. Accumulations of sand or other material which might interfere with drainage or might constitute a hazard to traffic will not be permitted. Protection of the work area shall be as indicated in the Specification for "Maintenance and Protection of Traffic."

12.11.04—Method of Measurement: Work under this item will be measured for payment by the actual number of square feet of pavement markings removed.

12.11.05—Basis of Payment: This work shall be paid for at the Contract unit price per square foot for "Removal of Pavement Markings" of the type specified, which price shall include all materials, equipment, tools and labor incidental thereto. Payment will not be made for pavement markings installed by Contractor error and ordered removed by the Engineer.

Pay Item	Pay Unit
Removal of Pavement Markings (Type)	s.f.

SECTION 12.12
TEMPORARY PLASTIC PAVEMENT MARKING TAPE

12.12.01—Description: This item shall consist of furnishing, installing and removing temporary plastic pavement marking tape of the color and width specified at the locations shown on the plans or as directed by the Engineer.

12.12.02—Materials: Materials for this work shall be commercially available pavement marking tape designed and suitable for the purpose intended and readily removable, when required. The tape shall be retroreflective with the use of glass beads throughout the pigments.

The marking tape shall be readily visible during daylight and when viewed with vehicular headlights at night. The marking tape shall be durable, flexible, formable and following application shall remain in contact with the pavement surface. The tape shall be reinforced by a non-metallic medium and pre-coated with a pressure sensitive adhesive.

12.12.03—Construction Methods: Installation of the pavement markings shall be performed in a neat and workman-like manner. The markings shall be applied parallel to the centerline or as shown on the plans or where directed by the Engineer with no unsightly deviations. The Contractor shall pre-mark the pavement prior to application of the marking and use a string line if required to produce a neat appearing line.

Removal shall be accomplished without the use of heat, solvents, grinding or sandblasting and in such a manner that no damage to the pavement results.

12.12.04—Method of Measurement: This work will be measured for payment by the actual number of linear feet of temporary plastic pavement marking tape acceptably installed on and removed from the pavement when it is no longer applicable or when its removal is directed by the Engineer.

12.12.05—Basis of Payment: This work will be paid for at the Contract unit price per linear foot for "Temporary Plastic Pavement Marking Tape" of the width and color specified, acceptably installed on and removed from the pavement. This price shall include all pre-marking layout, cleaning of pavement, application, removal and all materials, equipment, tools and labor incidental thereto. Payment will not be made for pavement markings installed by Contractor error and ordered removed by the Engineer.

Any lines which are no longer effective, in the opinion of the Engineer and in accordance with the ATSSA guidelines contained in "Quality Standards for Work Zone Traffic Control Devices," shall be replaced by the Contractor at no cost to the State.

Removed markings shall become the property of the Contractor and shall be removed from the Project.

Pay Item	Pay Unit
Temporary Plastic Pavement Marking Tape (Width) (Color)	l.f.

SECTION 12.14
PREFORMED BLACK LINE MASK PAVEMENT MARKING TAPE

12.14.01—Description: This item shall consist of furnishing, installing, and removing preformed, patterned, black line mask pavement marking tape of the width specified to temporarily cover existing pavement markings in accordance with this section and in accordance with the plans and as directed by the Engineer.

The preformed, patterned, black line mask pavement marking tape shall be a highly durable, skid resistant, non-reflective, pliant polymer tape designed for the temporary covering of existing pavement markings. The black line mask pavement marking tape shall be removed when no longer needed, unless directed otherwise by the Engineer.

The black line mask pavement marking tape, when applied according to the recommendations of the manufacturer, shall provide a neat, durable masking that will not flow or distort. The black line mask pavement marking tape shall be weather resistant and, through normal traffic wear, shall show no lifting or shrinkage which will significantly impair the intended usage of the tape throughout its useful life and show no significant tearing or other signs of poor adhesion.

12.14.02—Materials: Materials for this work shall meet the requirements of M.07.24.

12.14.03—Construction Methods: The patterned, black line mask pavement marking tape shall be applied in accordance with the manufacturer's recommendations, and shall mask the existing markings being covered.

12.14.04—Method of Measurement: Preformed black line mask pavement marking tape shall be measured for payment by the actual number of linear feet furnished, installed and removed.

12.14.05—Basis of Payment: This work shall be paid for at the Contract unit price per linear foot for "Preformed Black Line Mask Pavement Marking Tape" of the width specified. This price shall be for all the work required by this Section including the cleaning and preparing of the pavement surface, installation and removal, and all materials, equipment, tools and labor incidental thereto.

Any masking tape which is no longer effective, in the opinion of the Engineer, shall be replaced by the Contractor, at its own expense.

Removed masking tape shall become the property of the Contractor and shall be removed from the Project. Any damage to the underlying markings caused by the Contractor's operations shall be repaired by the Contractor, at its own expense.

Pay Item	Pay Unit
(Width) Preformed Black Line Mask Pavement Marking Tape	l.f.

SECTION 12.20
CONSTRUCTION SIGNS

12.20.01—Description: Under this item the Contractor shall furnish, install and remove construction signs with retroreflective sheeting and their required portable supports or metal sign posts that meet the requirements of NCHRP Report 350 (TL-3) or MASH for Category 2 Devices. The construction signs and their required portable supports or metal sign posts shall comply with the signing requirements stated in the item "Maintenance and Protection of Traffic," as shown on the plans and/or as directed by the Engineer. The Contractor shall furnish a sufficient number of signs to provide the signing patterns for all operations which are being undertaken concurrently.

12.20.02—Materials: Prior to using the construction signs and their portable supports, the Contractor shall submit to the Engineer a copy of the Letter of Acceptance issued by the FHWA to the manufacturer documenting that the devices (both sign and portable support tested together) comply with the requirements of NCHRP Report 350 (TL-3) or MASH for Category 2 Devices.

All sign faces shall be rigid and reflectorized and shall meet the requirements of M.18.09. If used as rigid substrate, sheet aluminum sign blanks shall comply with the requirements of M.18.13. Metal sign posts shall comply with the requirements of M.18.14. Application of retroreflective sheeting, legends, symbols, and borders shall comply with the requirements specified by the retroreflective sheeting manufacturer. Attachments shall be provided so that the signs can be firmly attached to the portable sign supports or metal posts without causing damage to the signs.

12.20.03—Construction Methods: The signs and their portable supports or metal posts shall comply with the requirements as shown on the plans and the latest edition of the "Manual on Uniformed Traffic Control Devices." Drawings of the signs, showing placement and dimensions of legend and border, are available for inspection at the Division of Traffic, Connecticut Department of Transportation.

Various types of portable sign supports may be used. These portable supports shall be fabricated in such a manner as to minimize the possibility of the signs being blown over or displaced by the wind from passing vehicles and are to be of a yielding type to withstand impact with minimal damage to the signs, supports, or vehicles. Portable sign supports shall be approved by the Engineer before they are utilized on the Project. Mounting height of signs on portable sign supports shall be a minimum of 1 foot and a maximum of 2 feet, measured from the pavement to the bottom of the sign.

Signs in other than good condition shall be replaced with acceptable signs as determined by the Engineer.

Suitable attachments shall be provided so that the signs can be firmly attached to the portable sign supports or metal posts without causing damage to the signs.

The following types of construction signs shall not be used: mesh, non-rigid, roll-up, corrugated or waffle board types substrates, foam core and composite aluminum sign substrates.

Field Performance: Retroreflective sheeting processed and applied to sign blank materials in accordance with the sheeting manufacturer’s recommendations, shall perform effectively for a minimum of 3 years. The retroreflective sheeting will be considered unsatisfactory if it has deteriorated due to natural causes to the extent that:

1) the sign is ineffective for its intended purpose when viewed from a moving vehicle under normal day and night driving conditions; or
2) the coefficient of retroreflection is less than 100 when measured at 0.2 degree observation angle and -4 degree entrance angle.

All measurements shall be made after sign cleaning according to the sheeting manufacturer’s recommendations.

Ineffective signs, as determined by the Engineer and in accordance with the ATSSA guidelines contained in "Quality Standards for Work Zone Traffic Control Devices,” shall be replaced by the Contractor at no cost to the State.

Signs and their portable sign supports or metal posts that are no longer required shall be removed from the Project and shall remain the property of the Contractor.

12.20.04—Method of Measurement: The work to furnish, install and remove construction signs will be measured for payment by the number of square feet of sign face delivered and used on the Project. Sign supports will not be measured for payment.

12.20.05—Basis of Payment: This item will be paid for at the Contract unit price per square foot for "Construction Signs," delivered and used on the Project, which price shall include the signs, portable sign

supports, metal sign posts and all hardware required to attach the sign to the support or posts. Each sign and support or posts furnished will be paid for once, regardless of the number of times used on the Project.

Pay Item	Pay Unit
Construction Signs	s.f.

SECTIONS 13.00, 14.00, 15.00, 16.00
UTILITIES

Utilities paid under Sections 13.00, 14.00, 15.00 and 16.00 without a Special Provision will meet the requirements of the pertinent sections of the Standard Specifications for the specified item.

NOTES

SECTION 18.02
SAND BARRELS

18.02.01—Description: Work under this item shall consist of furnishing, installing and maintaining permanent sand barrels at the locations shown on the plans. This item shall also include furnishing, installing, maintaining, relocating and removing temporary sand barrels at the locations shown on the plans.

18.02.02—Materials: The sand barrels shall be listed on the Department's Qualified Products List.

The sand shall meet the requirements of M.05.02-2, with a maximum moisture content of 3% and shall be thoroughly mixed with 20% sodium chloride by weight. Sodium chloride shall meet the requirements of AASHTO M 143, Type 1, and Grade 1.

The reflector shall meet the requirements of M.18.09.

18.02.03—Construction Methods: The sand barrels shall be placed at the location on the plans or as directed by the Engineer.

The sand barrels shall be maintained during all stages of construction. Any damaged sand barrels shall be replaced within 24 hours of notification from the Engineer. The Contractor shall be responsible for the removal and proper disposal of all damaged material and debris.

Sand barrels shall be relocated to locations shown on the plans or as directed.

All temporary sand barrels no longer required for the Project shall be removed immediately.

18.02.04—Method of Measurement: Permanent sand barrels will be measured for payment by the number of each sand barrel, of specified weight, installed and accepted by the Engineer.

Temporary sand barrels will be measured for payment by the number of each sand barrel of specified weight installed and removed, and accepted by the Engineer.

Relocation of temporary sand barrels will be measured for payment by the number of each sand barrel of specified weight relocated and accepted by the Engineer. There will be no measurement for relocating sand barrels to a storage location.

Adjustment or realignment of sand barrels will not be measured for payment.

18.02.05—Basis of Payment: Sand barrels designated for permanent installation on the Project will be paid for at the Contract unit price for each "Permanent Sand Barrel (Weight)" furnished, installed and maintained, which price shall include the reflector, all materials, equipment, tools and labor incidental thereto.

Sand barrels designated for temporary installation on the Project, will be paid for at the Contract unit price for each "Temporary Sand Barrel (Weight)" furnished, installed, maintained and removed, which price shall include the reflector, all materials, equipment, tools and labor incidental thereto.

Relocation of temporary sand barrels will be paid for at the Contract unit price for each "Relocation of Temporary Sand Barrel (Weight)." This price shall include transportation, equipment, tools and labor incidental to relocating the sand barrels.

Replacement of damaged barrels will be paid for at the Contract unit price for each barrel, for the type and weight specified.

Pay Item	Pay Unit
Permanent Sand Barrel (Weight)	ea.
Temporary Sand Barrel (Weight)	ea.
Relocation of Temporary Sand Barrel (Weight)	ea.

SECTION 18.03
IMPACT ATTENUATION SYSTEM
TEMPORARY IMPACT ATTENUATION SYSTEM

18.03.01—Description: Work under this item shall consist of furnishing, installing and maintaining an impact attenuation system of the type specified at the location shown on the plans. Work under this item shall also include repair of the impact attenuation system.

18.03.02—Materials: The impact attenuation system shall be listed on the Department's Qualified Products List for the compatible barrier type.

The reflector shall meet the requirements of M.18.09.

18.03.03—Construction Methods: The impact attenuation system shall be installed or repaired according to the manufacturer's recommendations at the location shown on the plans.

Any damaged impact attenuation system shall be repaired within 24 hours of notification from the Engineer. The Contractor shall be responsible for the removal and the proper disposal of all damaged material and debris.

18.03.04—Method of Measurement: The impact attenuation system will be measured for payment by the number of each system installed and accepted by the Engineer.

The sum of money shown on the estimate and in the itemized proposal as "Estimated Cost" for repair of impact attenuation system will be considered the price bid even though payment will be made only for actual work performed. The estimated cost figure is not to be altered in any manner by the bidder. Should the bidder alter the amount shown, the altered figures will be disregarded and the original price will be used to determine the total amount bid for the Contract.

18.03.05—Basis of Payment: Impact attenuation system will be paid at the Contract unit price for each "Impact Attenuation System (Type)" furnished and installed, which price shall include the reflector and all materials, transportation, equipment, tools and labor incidental thereto. Temporary impact attenuation system will be paid at the Contract unit price for each "Temporary Impact Attenuation System (Type)" furnished, installed and removed, which price shall include the reflector and all materials, transportation, equipment, tools and labor incidental thereto.

"Repair of Impact Attenuation System" will be paid for in accordance with 1.09.04 as required to restore the system to its full working condition in accordance with the manufacturer's recommendations. Maintenance and protection of traffic will only be paid for when, in the opinion of the Engineer, it is solely required for repair of the system.

Pay Item	Pay Unit
Impact Attenuation System (Type)	ea.
Temporary Impact Attenuation System (Type)	ea.
Repair of Impact Attenuation System (Type)	est.

SECTION 18.06
TRUCK-MOUNTED OR TRAILER-MOUNTED IMPACT ATTENUATOR

18.06.01—Description: This item shall consist of furnishing, operating, maintaining, and relocating a Truck-Mounted or Trailer-Mounted Impact Attenuator (TMA).

18.06.02—Materials: Prior to using a TMA, the Contractor shall submit to the Engineer a Materials Certificate in accordance with 1.06.07 for each attenuator supplied and a copy of the Federal-aid eligibility letter issued to the manufacturer documenting that the device complies with the requirements of the NCHRP Report 350 (TL-3) or the AASHTO MASH (TL-3).

The impact attenuator shall be attached to a truck in accordance with the manufacturer's recommendations.

The impact attenuator shall be equipped with a minimum of 2 yellow flashing lights. An attenuator reflector consisting of Type IV retroreflective sheeting that displays an inverted "V" pattern, with alternating black and retroreflective yellow stripes, shall be placed on the back of the unit. The attenuator reflector shall fully cover the rear face of the impact attenuator.

The truck shall be equipped with a high mounted internally illuminated flashing arrow which meets the requirements of 11.30.

The truck shall be equipped with a minimum of 2 amber strobe type flashers mounted above the internally illuminated flashing arrow.

18.06.03—Construction Methods: The Contractor shall furnish, operate and relocate each TMA according to the Traffic Control Pattern described in the special provisions for Maintenance and Protection of Traffic and Prosecution and Progress.

The Contractor shall maintain each TMA in a fully operable condition at all times. Any TMA that is not fully functional will not be permitted at the Project Site.

18.06.04—Method of Measurement: This item will be measured for payment by the actual number of hours that the Truck-Mounted or Trailer-Mounted Impact Attenuator is used.

Subject to the approval of the Engineer, the TMA may be used as a High Mounted Internally Illuminated Flashing Arrow. If the TMA is used as a Flashing Arrow, then it will be measured for payment per day under the item "High Mounted Internally Illuminated Flashing Arrow."

18.06.05—Basis of Payment: The TMA will be paid for at the Contract unit price per hour for "Truck-Mounted or Trailer-Mounted Impact Attenuator," which shall include the furnishing and use of the specified vehicle and a driver, attenuator reflector, flashing lights, illuminated flashing arrow sign, and all equipment, materials, tools, labor, disposal of damaged TMA components and work incidental thereto.

Pay Item	Pay Unit
Truck-Mounted or Trailer-Mounted Impact Attenuator	hr.

SECTION M.01
AGGREGATES

M.01.01—General
M.01.02—Coarse Aggregates
M.01.03—Fine Aggregates
M.01.04—Portland Cement Concrete (PCC) Aggregates
M.01.05—Bituminous Concrete Aggregates

M.01.01—General:

Each source of aggregate must be qualified for use by the Engineer as indicated in 1.06.01.

Material from a qualified source is still subject to Project-level testing and may be subject to rejection as indicated in 1.06.04.

Aggregates must not have expansive or reactive properties. Aggregates reclaimed from pavements or structures may only be used where specifically allowed in the specifications.

Aggregate stockpiles must be located on smooth, hard, sloped/well-drained areas. Each source and gradation of aggregate must have an individual stockpile or bin. Stockpiles must be managed to minimize segregation and contamination with foreign materials.

M.01.02—Coarse Aggregates:

Coarse aggregate must be uniform in consistency and only contain clean, hard, tough, durable fragments meeting the criteria in Table M.01.02-1.

TABLE M.01.02-1: Coarse Aggregate Criteria by Pit/Quarry Source

Item	Title	AASHTO Test Methods	Criteria
1	Material Passing No. 200 Sieve	T 11	1% maximum.
2	Loss on Abrasion	T 96	40% maximum
3	Soundness by Magnesium Sulfate	T 104	10% maximum @ 5 cycles

Standard sizes of coarse aggregate for applications other than bituminous concrete must meet the gradation requirements listed in Table M.01.02-2 as determined by AASHTO T 27.

TABLE M.01.02-2: Gradation of Standard Sizes of Coarse Aggregate

Square Mesh Sieves	Percent Passing by Weight					
	No. 3	No. 4	No. 6	No. 67	No. 8	No. 9
2 1/2 inches	100					
2 inches	90-100	100				
1 1/2 inches	35-70	90-100				
1 inch	0-15	20-55	100	100		
3/4 inch		0-15	90-100	90-100		
1/2 inch	0-5		20-55		100	
3/8 inch		0-5	0-15	20-55	85-100	100
No. 4			0-5	0-10	10-30	85-100
No. 8				0-5	0-10	10-40
No. 16					0-5	0-10
No. 50						0-5

M.01.03—Fine Aggregates:

Fine aggregate must consist of clean, hard, durable, tough, uncoated particles free from lumps, meeting the requirements listed in Table M.01.03-1.

TABLE M.01.03-1: Fine Aggregate Requirements

Item	Property	AASHTO Test	Criteria
1	**Grading**		
	Portland Cement Concrete	T 11	3% maximum passing No. 200 sieve
		T 27	Table M.01.04-1
	Bituminous Concrete	T 27	100% Passing 3/8 inch, 95% passing the No. 4 min.
2	**Absorption**	T 84	3% maximum
3	**Plasticity limits**	T 90	0 or not detectable
4	**L.A. Abrasion**	T 96	50% maximum (fine agg. particle size ≥ No. 8)
5	**Soundness by Magnesium Sulfate**	T 104	15% maximum@ 5 cycles for PC Concrete 20% maximum@ 5 cycles for Bituminous Concrete
6	**Clay Lumps and Friable Particles**	T 112	3% maximum
7	**Deleterious Material** - organic or inorganic calcite, hematite, pyrrohtite, shale, clay, coal-lignite, shells, loam, mica, clinkers, or other organic matter (wood, etc.).	As determined by the Engineer	Must not contain more than 3% by mass of any individual listed constituent and not more than 5% by mass in total of all listed constituents.

Screenings and Dust must meet the requirements of Table M.01.03-2 as determined by AASHTO T 27.

TABLE M.01.03-2: Screenings and Dust Gradation

Square Mesh Sieves	Percent Passing by weight	
	Screenings	Dust
3/8 inch	100	
No. 4		100
No. 8	60-100	40-100

M.01.04—Portland Cement Concrete (PCC) Aggregates:

In addition to the requirements in M.01.01 through M.01.03, the aggregates used in Portland Cement Concrete must meet the following:

All Aggregates: Coarse and Fine aggregates must originate from the aggregate producers and locations included on the Department's Qualified Materials List (QML). The list is available on the Department website. The criteria for inclusion in the QML are stated within the list.

Coarse Aggregate: Coarse aggregate of a size retained on a 1 inch square opening sieve must not contain more than 8% of flat and elongated pieces when tested in accordance to ASTM D4791 at a 1:5 ratio.

Reclaimed concrete aggregates must consist of clean, durable fragments of uniform quality. Materials must be from crushing or otherwise processing of concrete structures or portions thereof. Prior to demolition or removal, concrete structures must not exhibit signs of material degradation and be inspected by the Engineer. Reclaimed aggregate must be tested separately to confirm compliance with all requirements prior to blending with virgin aggregate.

Reclaimed coarse aggregate must not contain chlorides in excess of 0.5 lb./c.y. Chloride content must be determined in accordance with AASHTO T 260, Procedure A. Regardless of chloride content, reclaimed aggregates must not be used in concrete for pre-stressed concrete members.

Fine Aggregate: Manufactured sand must be produced from washed stone screenings; stone screenings or gravel; or combinations thereof, after mechanical screening or with a process approved by the Engineer.

The fineness modulus of fine aggregate from a source must not vary more than 0.20 from the base fineness modulus of that source.

The fine aggregate must not produce a color darker than Gardner Color Standard No. 11 in accordance with AASHTO T 21.

Fine aggregates that fail to meet soundness requirements as specified in Table M.01.03-1, but meet all other requirements, may be used with the approval of the Engineer on a case-by-case basis. Typically concrete composing any surface subject to polishing or abrasion (i.e., wheel traffic or running water) will not be allowed to contain such material.

Gradation of each size aggregate must be within the ranges listed in Table M.01.04-1 as determined by AASHTO T 27.

Table M.01.04-1: Fine Aggregate Gradations

Sieve Size	3/8 inch	No. 4	No. 8	No. 16	No. 30	No. 50	No. 100
% passing	100	95-100	80-100	50-85	25-60	10-30	2-10

M.01.05—Bituminous Concrete Aggregates

In addition to the requirements in M.01.01 through M.01.03, the source of aggregates used in Bituminous Concrete must have a Quality Control Plan for Fine Aggregates (QCPFA) on file with the Engineer. The QCPFA must describe the locations and manufacturing processing methods used at the source. The QCPFA must describe how conformance to Items 1 through 7 in Table M.01.03-1 is monitored and what actions will be taken if nonconformance is observed. The QCPFA must be revised and resubmitted to the Engineer whenever the process, location, or manner of how the fine aggregate is produced or monitored changes. A source of fine aggregate may be suspended by the Engineer due to demonstrated noncompliance with the QCPFA or if consistent production of material does not meet Project specifications as determined by the Engineer.

SECTION M.02
GRANULAR FILL
SUBBASE
GRANULAR BASE AND SURFACES
STONE BASE
PERVIOUS STRUCTURE BACKFILL
FREE-DRAINING MATERIAL
CRUSHER-RUN STONE

M.02.01—Granular Fill
M.02.02—Subbase
M.02.03—Granular Base, Rolled Bank Gravel Surface and Traffic Bound Gravel Surface
M.02.04—Gravel Shoulders
M.02.05—Pervious Structure Backfill
M.02.06—Gradation, Plasticity, Resistance to Abrasion and Soundness Requirements
M.02.07—Free-Draining Materials

M.02.01—Granular Fill: For this purpose, the material shall consist of broken or crushed stone, gravel, reclaimed miscellaneous aggregate or a mixture thereof.

1. Broken or crushed stone shall be the product resulting from the artificial crushing of rocks, boulders or large cobblestones, substantially all faces of which have resulted from the crushing operation. Broken or crushed stone shall consist of sound, tough, durable stone, reasonably free from soft, thin, elongated, laminated, friable, micaceous or disintegrated pieces, mud, dirt or other deleterious material and shall be sized to meet the requirements of grading "A," M.02.06.

2. Bank or crushed gravel shall consist of sound, tough, durable particles of crushed or uncrushed gravel, free from soft, thin, elongated or laminated pieces and vegetable or other deleterious substances. It shall meet Grading "A" and the requirements for plasticity and resistance to abrasion indicated in M.02.06. Crushed gravel shall be the manufactured product resulting from the deliberate mechanical crushing of gravel with at least 50% of the gravel retained on the No. 4 sieve having at least 1 fractured face.

3. Reclaimed Miscellaneous Aggregate material shall consist of sound, tough, durable particles of crushed reclaimed waste. It shall be free of soft disintegrated pieces, mud, dirt, glass or other injurious materials and contain no more than 2% by weight of asphalt cement.

This reclaimed miscellaneous material shall meet Grading "A" and the requirements for plasticity and resistance to abrasion, which are set forth in M.02.06.

M.02.02—Subbase: Materials for this work shall conform to the following requirements:

1. Bank or crushed gravel shall consist of sound, tough, durable particles of crushed or uncrushed gravel, free from soft, thin, elongated or laminated pieces and vegetable or other deleterious substances. It shall be hard and durable enough to resist weathering, traffic abrasion and crushing. It shall meet Grading "B" and the requirements for plasticity and resistance to abrasion indicated in M.02.06.

2. Crusher-Run Stone shall consist of sound, tough, durable broken stone. It shall be reasonably free from soft, thin, elongated, laminated, friable, micaceous or disintegrated pieces, mud, dirt or other deleterious material.

(a) Loss on Abrasion: The crusher-run stone shall show a loss on abrasion of not more than 50% using AASHTO Method T 96.

(b) Grading: The crusher-run stone shall meet Grading "A" and the requirements for plasticity indicated in M.02.06.

3. Reclaimed Miscellaneous Aggregate shall consist of sound, tough, durable particles of crushed reclaimed waste. It shall be free from soft, disintegrated pieces, mud, dirt, glass or other injurious material, and contain no more than 2% by weight of asphalt cement.

This reclaimed miscellaneous material shall meet Grading "B" and the requirements for plasticity and resistance to abrasion, which are set forth in M.02.06.

M.02.03—Granular Base, Rolled Bank Gravel Surface and Traffic Bound Gravel Surface: The materials for the "Rolled Granular Base" shall consist of sound, tough, durable particles of bank or crushed gravel, or reclaimed miscellaneous aggregate, or mixtures thereof with the resultant uniform blend containing no more than 2% by weight of asphalt cement. The materials for the Rolled Bank Gravel Surface and Traffic-Bound Gravel Surface shall consist of sound, tough, durable particles of bank or crushed gravel. All materials shall be free from thin or elongated pieces, lumps of clay, loam, or vegetable

matter. Binder may be added and incorporated by approved methods as specified elsewhere. It shall meet Grading "A" except that the top course of the rolled bank gravel surface shall conform to Grading "C."

M.02.04—Gravel Shoulders: The materials for this work shall consist of sound, tough, durable particles of crushed or uncrushed gravel free from soft, thin, elongated or laminated pieces, vegetable or other deleterious substances. Gravel shall meet Grading "A" except that the upper 3 inches shall conform to Grading "C."

M.02.05—Pervious Structure Backfill: Pervious structure backfill shall consist of broken or crushed stone, broken or crushed gravel, or reclaimed miscellaneous aggregate containing no more than 2% by weight of asphalt cement or mixtures thereof.

Materials for this work shall conform to the following requirements:

1. **Broken or crushed stone** shall consist of sound, tough, durable stone, reasonably free from soft, thin, elongated, friable, laminated, micaceous or disintegrated pieces, mud, dirt or other deleterious material and shall be sized to meet the requirements of Grading "B," M.02.06. It shall meet the requirements of loss on abrasion indicated in M.02.02-2(a).
2. **Bank or crushed gravel** shall consist of sound, tough, durable particles of crushed or uncrushed gravel free from soft, thin, elongated or laminated pieces and vegetable or other deleterious substances. It shall meet Grading "B."
3. **Reclaimed Miscellaneous Aggregate** shall consist of sound, tough, durable particles of crushed reclaimed waste. It shall be free of soft disintegrated pieces, mud, dirt, glass or other injurious material, and contain no more than 2% by weight of asphalt cement. It shall meet Grading "B."

M.02.06—Gradation, Plasticity, Resistance to Abrasion and Soundness Requirements:

1. Gradation:

	Grading		
	A	**B**	**C**
Square Mesh Sieves	Percent passing by weight		
Pass 5 inch		100	
Pass 3 1/2 inch	100	90-100	
Pass 1 1/2 inch	55-100	55-95	100
Pass 3/4 inch			45-80
Pass 1/4 inch	25-60	25-60	25-60
Pass No. 10	15-45	15-45	15-45
Pass No. 40	5-25	5-25	5-25
Pass No. 100	0-10	0-10	0-10
Pass No. 200	0-5	0-5	0-5

The grading percentages specified in the above table shall apply to the material after it has been delivered to the construction site as well as when tested at the pit or other source of supply.

When the fraction of the dry sample passing the No. 100 mesh sieve is greater than 8% by weight, the sample will be washed as indicated. The amount obtained from washing shall be added to that obtained by dry sieving; and the total amount passing each sieve shall meet the above gradation.

2. Plasticity:

(a) When the fraction of the dry sample passing the No. 100 mesh sieve is 4% or less by weight, no plastic limit test will be made.

(b) When the fraction of the dry sample passing the No. 100 mesh sieve is greater than 4% and not greater than 8% by weight, that fraction shall not have sufficient plasticity to permit the performing of the plastic limit test using AASHTO Method T 90.

(c) When the fraction of the dry sample passing the No. 100 mesh sieve is greater than 8% by weight, the sample will be washed; and the additional material passing the No. 100 mesh sieve shall be determined

by AASHTO Method T 146, except that the No. 100 mesh sieve will be substituted for the No. 40 mesh sieve where the latter is specified in AASHTO Method T 146. The combined materials that passed the No. 100 mesh sieve shall not have sufficient plasticity to permit the performing of the plastic limit test using AASHTO Method T 90.

3. Test for Resistance to Abrasion. Gravel materials shall show a loss on abrasion of not more than 50% using AASHTO Method T 96.

4. Soundness: When tested with magnesium sulfate solution for soundness using AASHTO Method T 104, coarse aggregate shall not have a loss of more than 15% at the end of 5 cycles.

M.02.07—Free-Draining Materials: Free-draining material shall consist of sand, gravel, rock fragments, quarry run stone, broken stone, reclaimed miscellaneous aggregate containing no more than 2% by weight of asphalt cement or mixtures thereof. This material, or the material from any one source of a mixture, shall not have more than 70%, by weight, passing the No. 40 mesh sieve and not more than 10%, by weight, passing the No. 200 mesh sieve.

SECTION M.03
PORTLAND CEMENT CONCRETE

M.03.01—Component Materials
M.03.02—Mix Design Requirements
M.03.03—Producer Equipment and Production Requirements
M.03.04—Curing Materials
M.03.05—Non Shrink, Non Staining Grout
M.03.06—Expansive Cement for Anchoring
M.03.07—Chemical Anchors
M.03.08—Joint Materials
M.03.09—Protective Compound/Sealers
M.03.10—Formwork

M.03.01—Component Materials

1. **Coarse Aggregate:** Coarse aggregate shall meet the requirements of M.01.
2. **Fine Aggregate:** Fine aggregate shall meet the requirements of M.01.
3. **Cement:**

(a) Portland: Types I, II, and III Portland cement shall meet the requirements of AASHTO M 85. Type I and Type III Portland cement shall be used only when required or expressly permitted by the Project specification or the Engineer. The use of Type I or III will require that these mixtures be submitted as Non-standard Mix Designs. All cement shall be provided by a mill participating in the Departments' Cement Certification program. The requirements of the Certification Program are detailed in the Departments' Quality Assurance Program for Materials.

(b) Pre-Blended Cements: Binary or Ternary cements consisting of Portland Cement and supplemental cementitious materials may be used provided that all the requirements of M.03.01- 3(a) and -3(c) are met.

(c) Replacement Materials: Unless already approved as a Standard Mix Design, any Contractor proposed Mix Designs with partial replacement of Portland Cement (PC) with fly ash or ground granulated blast furnace slag (GGBFS), shall be submitted in writing to the Engineer for approval prior to the start of work, on a project-by-project basis. The type of material, source, and the percentage of the PC replaced shall be clearly indicated. Upon request, a Certified Test Report for the cement replacement material shall be provided to the Engineer for use during the Mix Design review.

1. Fly Ash: Fly ash to be used as a partial replacement for Portland cement shall meet the requirements of AASHTO M 295, either Class C or Class F, including the uniformity requirements of Table 2A. Loss on Ignition for either class of fly ash shall not exceed 4.0%. Fly ash may be used to replace up to a maximum of 20% of the required Portland cement for mixes without permeability requirements. For mixes with permeability requirements, the maximum of 20% may be exceeded. The fly ash shall be substituted on a weight basis, with a minimum of 1 lb. of fly ash for 1 lb. of Portland cement. Different classes of fly ash or the same class from different sources shall not be permitted on any single project without the written approval of the Engineer.
2. Ground Granulated Blast Furnace Slag (GGBFS): GGBFS used as a partial replacement for Portland cement shall meet the requirements of AASHTO M 302/ASTM C989, Grade 100 or 120. As determined by the Engineer, GGBFS may be used to replace a maximum of 30% of the required Portland cement for mixes without permeability requirements. For mixes with permeability requirements, the maximum of 30% may be exceeded. The Engineer may restrict or prohibit the use of GGBFS if ambient temperatures anticipated during the placement and initial curing of the concrete are low. The GGBFS shall be substituted on a weight basis, with a minimum of 1 lb. of slag for 1 lb. of Portland cement. Different sources of GGBFS shall not be permitted on any single project without the written approval of the Engineer.

4. **Water:** All water used in the mixing of concrete shall be odorless and clear in appearance. Surface water may be used if not taken from shallow or muddy sources; classified as Class C or Class D on the Department of Energy and Environmental Protection (DEEP) Water Quality Classification mapping; and accommodations have been made to prevent contaminants from entering the supply to the satisfaction of the Engineer. The Engineer may request that water from any surface or ground source be tested in

accordance with AASHTO T 26 and AASHTO D 512 if the appearance or scent of the water is suspect. To be acceptable, the pH of the water must not be less than 6.0 or greater than 8.0 and Chloride Ion Concentration of the water must not exceed 250ppm. Potable water taken directly from a municipal or regional water supply may be used for mixing concrete without testing. Heating or cooling of water may be required to meet mix temperature requirements at time of placement.

5. Admixtures: All admixtures shall perform their function without injurious effects upon the concrete. If requested by the TDC, the Contractor shall present a certified statement from a recognized laboratory attesting to this requirement. A "recognized" laboratory is any cement and concrete laboratory approved and inspected regularly by the Cement and Concrete Reference Laboratory (CCRL). The statement shall contain results of compression tests of cylinder specimens made with concrete utilizing the admixture(s) in proportions equal to those proposed by the Contractor. The results of at least 5 standard 6 inch × 12 inch cylinders of each mix design shall be listed with the results of at least 5 like-sized cylinders not utilizing the admixture(s). Specimens must be made and cured in the laboratory in accordance with AASHTO T 126 and will be tested in accordance with AASHTO T 22.

(a) Air-Entraining Admixtures: In the event that air entrained concrete is required, an admixture meeting the requirements of AASHTO M 154 may be used. Tests for 7 and 28 day compressive and flexural strengths and resistance to freezing and thawing are required whereas tests for bleeding, bond strength and volume change will not be required.

(b) Other Chemical Admixtures: In the event that concrete properties are specified that require the use of additional admixtures, or the Contractor proposes the use of additional admixtures to facilitate placement, the admixtures shall meet the requirements of AASHTO M194M/M, including the 1 year performance data.

M.03.02—Mix Design Requirements

1. Standard ConnDOT Mix Designs: Standard Mix Designs shall be designed in accordance with applicable sections of ACI 211 and ACI 318. The mixtures shall consist of Portland cement, fine aggregate, coarse aggregate, admixtures, and water proportioned in accordance with Table M.03.02-1. The mixtures shall also be designed to obtain the plastic properties of Portland cement concrete as specified in Table 6.01.03-2.

Table M.03.02-1 Standard Portland Cement Concrete Mixes

Class[1]	Max. Water/Cement[2] ratio	Min. Cement[2] Content - lb./c.y.	Air Content %	Electrical Resistivity (Permeability) kΩ-cm AASHTO T 358
PCC0223Z	0.69	455	6 +/- 1.5	NA
PCC0334Z	0.48	615		NA
PCC0336Z	0.50	564		NA
PCC0354Z	0.49	615		NA
PCC0446Z	0.44	658		NA
PCC04462	0.42			29 minimum
PCC0556Z	0.40			NA
PCC05562	0.40			29 minimum
PCCXXX81[3]	0.46		7.5 +/- 1.5	15 maximum
PCCXXX82	0.40			29 minimum

[1] PCCXXYZ where:
PCC = Portland Cement Concrete
XXX = 28-day minimum compressive strength (psi/100)
Y = Nominal Maximum Aggregate Size (U.S. Sieve No. Designation)
Z = Exposure Factor (See Table M.03.02-1a)

[2] Portland Cement may be partially replaced within a Standard Mix Design by other approved cementitious material meeting the requirements of M.03.01-3(c) if permitted by the Engineer.
[3] When this class is paid for in a surface or structural repair concrete item, the plastic properties necessary for confined placement to ensure appropriate workability for consolidation within the forms shall be noted on the delivery ticket by the concrete supplier.

Table M.03.02-1a Exposure Factor per Application

Exposure		Application
0	Benign	Elements not exposed to weather (buried, enclosed)
1	Moderate	Elements not in contact with salt water or deicing chemicals
2	Severe	Elements in contact with salt water, deicing chemicals, flowing/standing water

Mix designs shall indicate the dosage of admixtures anticipated to provide plastic properties required in the Project specification. Plastic properties of standard mix classes of concrete in the plastic state are listed in Table 6.01.03-2.

Standard Mix Designs are required to be designed and submitted by the concrete producers, and are approved by the Department on a standing basis. Submittal or re-approval of these Standard Mix Designs on an annual basis is not required. Previously approved producer-designed Standard Mixes that have a record of satisfactory performance may be utilized on Department projects unless there is a change in the

gravimetric properties or the sources of any materials. Revisions to the Standard Mix Designs, which include changes in component sources, can be submitted at any time to the TDC, but must be approved prior to use on Department projects.

2. Non-Standard CTDOT Mix Designs: Any proposed Mix Designs that do not comply with Table M.03.02-1 are required to be submitted 15 days prior to use on a project-by-project basis and be approved by the TDC prior to use. The use of an approved admixture with an otherwise approved Standard Mix Design is not considered non-standard.

All Non-standard Mix Designs used for load-bearing structures shall contain a minimum of 658 lb./c.y. of cementitious materials.

Concrete used in applications such as flowable fill or controlled low-strength material may be designed with less than 658 lb./c.y. of cementitious materials.

M.03.03—Producer Equipment and Production Requirements

1. General Requirements: The source of the concrete must be approved by the Engineer prior to use on Department projects. Specifically the location and capacity of the central mix or dry batch plant, and complement of truck mixers/haulers, shall be adequate for continuous placement of concrete on a typical Department project. Approval may be revoked at any time in accordance with 1.06.01.

(a) Inspection: The production facility supplying hydraulic cement concrete shall have a current Certification of Ready Mixed Concrete Production Facilities from the National Ready Mixed Concrete Association (NRMCA), or equivalent certification approved by the Engineer.

(b) In addition to the requirements of approved third party certification, the facility shall produce batch tickets that meet the requirements of 6.01.03-II-3(a).

(c) Quality Control: The Contractor is responsible for all aspects of Quality Control (QC). As determined by the Engineer, should material delivered to a project not meet specification, the Contractor may be required to submit to the Engineer a corrective procedure for approval within 3 calendar days. The procedure shall address any minor adjustments or corrections made to the equipment or procedures at the facility.

(d) Suspension: As determined by the Engineer, repeated or frequent delivery of deficient material to a Department project may be grounds for suspension of that source of material. A detailed QC plan that describes all QC policies and procedures for that facility may be required to formally address quality issues. This plan must be approved by the Engineer and fully implemented, prior to reinstatement of that facility.

2. Hand Mixed Concrete: Hand mixing shall be permitted only with the permission of the Engineer. Hand mixed batches shall not exceed 1/2 c.y. in volume. Hand mixing will not be permitted for concrete to be placed under water.

M.03.04—Curing Materials

1. Water: Any water source deemed acceptable by the Engineer for mixing concrete may be used to provide water for curing purposes. Surface water may be used if classified as Class C or Class D on the Department of Energy and Environmental Protection (DEEP) Water Quality Classification mapping and accommodations have been made to prevent contaminants from entering the supply to the satisfaction of the Engineer. In general, water shall not be taken from shallow or muddy sources. In cases where sources of supply are relatively shallow, the intake pipe shall be enclosed to exclude silt, mud, grass, etc.; and the water in the enclosure shall be maintained at a depth of not less than 2 feet under the intake pipe.

2. Mats: Mats for curing concrete shall be capable of maintaining moisture uniformly on the surface of the concrete. The mats shall not contain any materials such as dyes, sugar, etc., that may be injurious to the concrete.

The length or width of the mats shall be sufficient to cover all concrete surfaces being cured. Should more than one mat be required, sufficient overlap shall be provided by the Contractor as determined by the Engineer.

3. Liquid Membrane-Forming Compound: Liquid membrane-forming compound shall meet the requirements of AASHTO M 148 Type 2, Class B, or shall be a water-soluble linseed oil-based compound meeting the requirements of AASHTO M 148, Type 2.

4. White Polyethylene Sheeting (Film): White polyethylene sheeting (film) shall meet the requirements of AASHTO M 171.

M.03.05—Non Shrink, Non Staining Grout

1. Bagged (pre-mixed): Bagged (pre-mixed) formulations of non-shrink grout shall meet the requirements of ASTM C1107. The grout shall be mixed with potable water for use. The grout shall be mixed to a flowable consistency as determined by ASTM C230. All bagged material shall be clearly marked with the manufacturer's name, date of production, batch number, and written instructions for proper mixing, placement and curing of the product.

2. Bulk: The Contractor may formulate and design a grout mix for use on the Project in lieu of using a pre-bagged product. The Contractor shall obtain prior written approval of the Engineer for any such proposed Mix Design. Any such Mix Design shall include the proportions of hydraulic cement, potable water, fine aggregates, expansive agent, and any other necessary additive or admixture. This material shall meet all of the same chemical and physical requirements as shall the pre-bagged grout, in accordance with ASTM C1107.

M.03.06—Expansive Cement for Anchoring

The premixed anchoring cement shall be non-metallic, concrete gray in color and prepackaged. The mix shall consist of hydraulic cement, fine aggregate, expansive admixtures and water meeting the following requirements:

1. The anchoring cement shall have a minimum 24 hour compressive strength of 2,600 psi when tested in accordance with ASTM C109.

2. The water content of the anchoring cement shall be as recommended by the manufacturer. Water shall meet the requirements of M.03.01-4.

The Contractor shall provide a Certified Test Report and Materials Certificate for the premixed anchoring cement in accordance with 1.06.07. The Contractor shall also provide, when requested by the Engineer, samples of the premixed anchoring cement for testing and approval.

M.03.07—Chemical Anchors

Chemical anchor material must be listed on the Departments' Qualified Products List and approved by the Engineer for the specified use.

The chemical anchor material shall be epoxy or polyester polymer resin. It shall not contain any metals or other products that promote corrosion of steel. The Contractor shall supply the Engineer with a Certified Test Report and Materials Certificate for the chemical anchor material in accordance with 1.06.07. When requested by the Engineer, the Contractor shall also provide samples of the chemical anchor material.

M.03.08—Joint Materials

1. Transverse Joints for Concrete Pavement: Transverse joints shall consist of corrosion resistant load transfer devices, poured joint seal and in addition, in the case of expansion joints, expansion joint filler all meeting the following requirements:

(a) The corrosion resistant load transfer device shall be coated steel or sleeved steel or be made of corrosion resistant material. The dimensions of any devices used shall be as shown on the plans, exclusive of any coating or sleeving. Core material of coated or sleeved metallic devices shall be steel meeting the requirements of AASHTO M 255M/M 255 Grade 520, or steel having equal or better properties and approved by the Engineer. Nonmetallic devices shall meet the various strength requirements applicable to metallic devices as well as all other requirements stated herein.

(b) All coated load transfer devices shall meet the requirements of AASHTO M 254. Uncoated or sleeved load transfer devices shall meet the applicable physical requirements of AASHTO M 254. The use of field applied bond breakers will not be permitted.

(c) The basis of acceptance for corrosion resistant load transfer devices shall be the submission by the Contractor of a minimum of 2 samples accompanied by Certified Test Reports meeting the requirements of 1.06.07 demonstrating that the load transfer device meets the requirements of AASHTO M 254 for the type of device supplied. The Engineer reserves the right to reject any load transfer device deemed unsatisfactory for use.

2. Joint Filler for Concrete Curbing: Expansion joint filler shall be either preformed expansion joint filler or wood joint filler as indicated on the plans and shall meet the following requirements:

(a) Preformed expansion joint filler shall be the bituminous cellular type and shall meet the requirements of AASHTO M 213.

(b) Boards for wood joint filler shall have 2 planed sides and shall be redwood, cypress or white pine. Redwood and cypress boards shall be of sound heartwood. White pine boards shall be of sound sapwood. Occasional small, sound knots and medium surface checks will be permitted provided the board is free of any defects that will impair its usefulness for the purpose intended. The joint filler may be composed of more than one length of board in the length of the joint, but no board of a length less than 6 feet shall be used; and the separate boards shall be held securely to form a straight joint. Boards composed of pieces that are jointed and glued shall be considered as one board.

(c) Dimensions shall be as specified or shown on the plans; and tolerances of plus 1/16 inch thickness, plus 1/8 inch depth and plus 1/4 inch length will be permitted.

(d) All wood joint filler boards shall be given a preservative treatment by brushing with creosote oil meeting the requirements of AASHTO M 133. After treatment, the boards shall be stacked in piles, each layer separated from the next by spacers at least 1/4 inch thick; and the boards shall not be used until 24 hours after treatment. Prior to concreting, all exposed surfaces of the wood filler shall be given a light brush coating of form oil.

(e) Testing of board expansion joint filler shall be in accordance with pertinent sections of AASHTO T 42.

3. Longitudinal Joint Devices: The metal used in the fabrication of longitudinal joint devices shall meet ASTM requirements for each type of metal used. The dimensions shall be as shown on the plans.

4. Expansion Joint Fillers for Bridges and Bridge Bearings:

(a) Preformed expansion joint filler for bridges shall meet the requirements of AASHTO M 153, Type I or
Type II.

(b) Pre-molded expansion joint filler for bridge bearings shall meet the requirements of AASHTO M 33.

5. Joint Sealants:

(a) Joint Sealer for Pavement: The joint sealer for pavement shall be a rubber compound of the hot-poured type and shall meet the requirements of AASHTO M 324 Type II unless otherwise noted on the plans or in the special provisions.

(b) Joint Sealer for Structures: Structure joint sealers shall be one of the following type sealants:

1. Where "Joint Seal" is specified on the plans, it shall meet the requirements of the Federal Specifications SS-S-200-E (Self-leveling type), TT-S-0227E (COM-NBS) Type II-Class A (Non-sag type), or 1 component polyurethane-base elastomeric sealants conforming to FS TT-S-00230C Type II-Class A or an approved equal.
   A Certified Test Report will be required in accordance with 1.06.07, certifying that the sealant meets the requirements set forth in the Federal Specification. Should the consignee noted on a Certified Test Report be other than the Prime Contractor, a Materials Certificate shall be required to identify the shipment.
2. Where "Silicone Joint Sealant" is specified on the plans, it shall be one of the following or an approved equal:
   i. Sealant, manufactured by the Dow Corning Corporation, Midland, Michigan 48686-0994
   ii. Dow Corning 888 Silicone Joint Sealant or
   iii. Dow Corning 888-SL Self-Leveling Silicone Joint 48686-0994

6. Closed Cell Elastomer: The closed cell elastomer shall meet the requirements of ASTM D1056, Grade RE-41 B2. The elastomer shall have a pressure-sensitive adhesive backing on one side.

The Contractor shall deliver the closed cell elastomer to the job site a minimum of 30 days prior to installation. Prior to the delivery of the closed cell elastomer, the Contractor shall notify the Engineer of the date of shipment and the expected date of delivery. Upon delivery of the closed cell elastomer to the job site, the Contractor shall immediately notify the Engineer.

Each separate length, roll or container shall be clearly tagged or marked with the manufacturer's name, trademark and lot number. A lot is defined as that amount of closed cell elastomer manufactured at 1 time from 1 batch of elastomer. A batch is defined as that amount of elastomer prepared and compounded at 1 time. The Contractor shall furnish a Certified Test Report in accordance with 1.06.07, confirming that the closed cell elastomer meets the requirements set forth in these specifications. Should the co-signee noted on a Certified Test Report be other than the Prime Contractor, a Materials Certificate shall be required to identify shipment.

The Contractor shall furnish a 1 foot length of closed cell elastomer in each lot for purposes of inspection and testing by the Engineer. The Engineer will cut a 1 foot sample from each lot and inspect the sample for conformance to size, and perform physical tests on the sample as deemed necessary.

The Engineer shall reject any lot or portion of a lot that does not meet the requirements stated herein. A rejected lot or portion of a lot may be resubmitted provided the Contractor has removed or corrected, in a manner acceptable to the Engineer, all non-conforming material.

M.03.09—Protective Compound/Sealers

The brand and type of material must be listed on the Department's Qualified Products List and approved by the Engineer for the specified use.

M.03.10—Formwork

1. Stay-in-place Forms: Material for stay-in-place metal forms shall be made of zinc-coated (galvanized) steel sheet meeting ASTM A653 (Structural Steel (SS) Grade 33 through 80). The minimum thickness shall be 20 gauge. Coating weight shall meet the requirements of ASTM A924, Class G235, and shall otherwise meet all requirements relevant to steel stay-in-place metal forms and the placing of concrete as specified herein and as noted in the Contract.

Form supports shall either be fabricated and meet the same material requirements as the forms, or be fabricated from structural steel meeting the requirements of ASTM A36 and shall be hot-dip galvanized in accordance with ASTM A123.

Lightweight filler material for forms shall be as recommended by the form manufacturer.

2. Temporary Forms and Falsework: Forms and Falsework shall be of wood, steel or other material approved by the Engineer. This approval does not relieve the Contractor from employing adequately sized materials of sufficient rigidity to prevent objectionable distortion of the formed concrete surfaces caused by pressure of the plastic concrete and other loads incidental to the construction operations.

SECTION M.04
BITUMINOUS CONCRETE MATERIALS

M.04.01—Bituminous Concrete Materials and Facilities
M.04.02—Mix Design and Job Mix Formula (JMF)
M.04.03—Production Requirements

M.04.01—Bituminous Concrete Materials and Facilities: Each source of material, Plant, and laboratory used to produce and test bituminous concrete must be qualified on an annual basis by the Engineer. AASHTO or ASTM Standards noted with an (M) have been modified and are detailed in Table M.04.03-5.

Aggregates from multiple sources of supply must not be blended or stored in the same stockpile.

1. **Coarse Aggregate:** All coarse aggregate shall meet the requirements listed in M.01.
2. **Fine Aggregate:** All fine aggregate shall meet the requirements listed in M.01.
3. **Mineral Filler:** Mineral filler shall conform to the requirements of AASHTO M 17.
4. **Performance Graded (PG) Asphalt Binder:**

(a) General:

i. PG asphalt binder shall be uniformly mixed and blended and be free of contaminants such as fuel oils and other solvents. Binder shall be properly heated and stored to prevent damage or separation.

ii. The binder shall meet the requirements of AASHTO M 332 and shall be graded or verified in accordance with AASHTO R 29. The Contractor shall submit a Certified Test Report and bill of lading representing each delivery in accordance with AASHTO R 26(M). The Certified Test Report must also indicate the binder specific gravity at 77°F; rotational viscosity at 275°F and 329°F; and the mixing and compaction viscosity-temperature chart for each shipment.

iii. The Contractor shall submit the name(s) of personnel responsible for receipt, inspection, and record keeping of PG binder. Contractor Plant personnel shall document specific storage tank(s) where binder will be transferred and stored until used and provide binder samples to the Engineer upon request. The person(s) shall assure that each shipment is accompanied by a statement certifying that the transport vehicle was inspected before loading was found acceptable for the material shipped and that the binder is free of contamination from any residual material, along with 2 copies of the bill of lading.

iv. The blending or combining of PG binders in 1 storage tank at the Plant from different suppliers, grades, or additive percentages is prohibited.

(b) Basis of Approval: The request for approval of the source of supply shall list the location where the material will be manufactured, and the handling and storage methods, along with necessary certification in accordance with AASHTO R 26(M). Only suppliers/refineries that have an approved "Quality Control Plan for Performance Graded Binders" formatted in accordance with AASHTO R 26(M) may supply PG binders to Department projects.

(c) Standard Performance Grade (PG) Binder:

i. Standard PG binder shall be defined as "Neat." Neat PG binders shall be free from modification with: fillers, extenders, reinforcing agents, adhesion promoters, thermoplastic polymers, acid modification and other additives such as re-refined motor oil, and shall indicate such information on each bill of lading and Certified Test Report.

ii. The standard asphalt binder shall be PG 64S-22.

(d) Modified Performance Grade (PG) Binder: The modified asphalt binder shall be Performance Grade PG 64E-22 asphalt modified solely with a Styrene-Butadiene-Styrene (SBS) polymer. The polymer modifier shall be added at either the refinery or terminal and delivered to the bituminous concrete production facility as homogenous blend. The stability of the modified binder shall be verified in accordance with ASTM D7173 using the Dynamic Shear Rheometer (DSR). The DSR $G^*/\sin(\delta)$ results from the top and bottom sections of the ASTM D7173 test shall not differ by more than 10%. The results of ASTM D7173 shall be included on the Certified Test Report. The binder shall meet the requirements of AASHTO M 332 (including Appendix X1) and AASHTO R 29.

(e) Warm Mix Additive or Technology:

i. The warm mix additive or technology must be listed on the North East Asphalt User Producer Group (NEAUPG) Qualified Warm Mix Asphalt (WMA) Technologies List at the time of bid,

which may be accessed online at http://www.neaupg.uconn.edu.

ii. The warm mix additive shall be blended with the asphalt binder in accordance with the manufacturer's recommendations.

iii. The blended binder shall meet the requirements of AASHTO M 332 and shall be graded or verified in accordance with AASHTO R 29 for the specified binder grade. The Contractor shall submit a Certified Test Report showing the results of the testing demonstrating the binder grade. In addition, it must include the grade of the virgin binder, the brand name of the warm mix additive, the manufacturer's suggested rate for the WMA additive, the water injection rate (when applicable), and the WMA Technology manufacturer's recommended mixing and compaction temperature ranges.

5. Emulsified Asphalts:

(a) General:

i. The emulsified asphalt shall meet the requirements of AASHTO M 140(M) or AASHTO M 208 as applicable.

ii. The emulsified asphalts shall be free of contaminants such as fuel oils and other solvents.

iii. The blending at mixing Plants of emulsified asphalts from different suppliers is prohibited.

(b) Basis of Approval:

i. The request for approval of the source of supply shall list the location where the material is manufactured, the handling and storage methods, and certifications in accordance with AASHTO R 77. Only suppliers that have an approved "Quality Control Plan for Emulsified Asphalt" formatted in accordance with AASHTO R 77 and that submit monthly split samples per grade to the Engineer may supply emulsified asphalt to Department projects.

ii. Each shipment of emulsified asphalt delivered to the Project site shall be accompanied with the corresponding Certified Test Report listing Saybolt viscosity, residue by evaporation, penetration of residue, and weight per gallon at 77°F and Material Certificate.

iii. Anionic emulsified asphalts shall meet the requirements of AASHTO M 140. Materials used for tack coat shall not be diluted and meet grade RS-1 or RS-1h. When ambient temperatures are 80°F and rising, grade SS-1 or SS-1h may be substituted if permitted by the Engineer.

iv. Cationic emulsified asphalt shall meet the requirements of AASHTO M 208. Materials used for tack coat shall not be diluted and meet grade CRS-1. The settlement and demulsibility test will not be performed unless deemed necessary by the Engineer. When ambient temperatures are 80°F and rising, grade CSS-1 or CSS-1h may be substituted if permitted by the Engineer.

6. Reclaimed Asphalt Pavement (RAP):

(a) General: RAP is a material obtained from the cold milling or removal and processing of bituminous concrete pavement. RAP material shall be crushed to 100% passing the 1/2 inch sieve and free from contaminants such as joint compound, wood, plastic, and metals.

(b**)** Basis of Approval: The RAP material will be accepted on the basis of one of the following criteria:

i. When the source of all RAP material is from pavements previously constructed on Department projects, the Contractor shall provide a Materials Certificate listing the detailed locations and lengths of those pavements and that the RAP is only from those locations listed.

ii. When the RAP material source or quality is not known, the Contractor shall request approval from the Engineer at least 30 calendar days prior to the start of the paving operation. The request shall include a Material Certificate and applicable test results stating that the RAP consists of aggregates that meet the specification requirements of M.04.01-1 through M.04.01-3 and that the binder in the RAP is substantially free of solvents, tars and other contaminants. The Contractor is prohibited from using unapproved material on Department projects and shall take necessary action to prevent contamination of approved RAP stockpiles. Stockpiles of unapproved material shall remain separate from all other RAP materials at all times. The request for approval shall include the following:

1. A 50-lb. sample of the RAP to be incorporated into the recycled mixture.
2. A 25-lb. sample of the extracted aggregate from the RAP.

7. Crushed Recycled Container Glass (CRCG):

(a) Requirements: The Contractor may propose to use clean and environmentally-acceptable CRCG in an amount not greater than 5% by weight of total aggregate.

(b) Basis of Approval: The Contractor shall submit to the Engineer a request to use CRCG. The

request shall state that the CRCG contains no more than 1% by weight of contaminants such as paper, plastic, and metal and conforms to the following gradation:

CRCG Grading Requirements	
Sieve Size	Percent Passing
3/8 inch	100
No. 4	35-100
No. 200	0.0-10.0

The Contractor shall submit a Material Certificate to the Engineer stating that the CRCG complies with all the applicable requirements in this Section.

8. Joint Seal Material: Joint seal material must meet the requirements of ASTM D6690 - Type 2. The Contractor shall submit a Material Certificate in accordance with 1.06.07 certifying that the joint seal material meets the requirements of this Section.

9. Recycled Asphalt Shingles (RAS): RAS shall consist of processed asphalt roofing shingles from post-consumer asphalt shingles or from manufactured shingle waste. The RAS material under consideration for use in bituminous concrete mixtures must be certified as being asbestos-free and shall be entirely free of whole, intact nails. The RAS material shall meet the requirements of AASHTO MP 23.

The Producer shall test the RAS material to determine the asphalt content and the gradation of the RAS material. The Producer shall take necessary action to prevent contamination of RAS stockpiles.

The Contractor shall submit a Material Certificate to the Engineer stating that the RAS complies with all the applicable requirements in this Section.

10. Plant Requirements:

(a) General: The Plant producing bituminous concrete shall comply with AASHTO M 156.

(b) Storage Silos: The Contractor may use silos for short-term storage with the approval of the Engineer. A storage silo must have heated cones and an unheated silo cylinder if it does not contain a separate internal heating system. When multiple silos are filled, the Contractor shall discharge 1 silo at a time. Simultaneous discharge of multiple silos for the same Project is not permitted.

Type of silo cylinder	Maximum storage time for all classes (hr)	
	HMA	WMA/PMA
Open Surge	4	Mfg Recommendations*
Unheated - Non-insulated	8	Mfg Recommendations*
Unheated - Insulated	18	Mfg Recommendations*
Heated - No inert gas	TBD by the Engineer	TBD by the Engineer

*Not to exceed HMA limits

(c) Documentation System: The mixing Plant documentation system shall include equipment for accurately proportioning the components of the mixture by weight and in the proper order, controlling the cycle sequence, and timing the mixing operations. Recording equipment shall monitor the batching sequence of each component of the mixture and produce a printed record of these operations on each Plant ticket, as specified herein.

If recycled materials are used, the Plant tickets shall include their dry weight, percentage, and daily moisture content.

If a WMA Technology is added at the Plant, the Plant tickets shall include the actual dosage rate.

For drum Plants, the Plant ticket shall be produced at 5 minute intervals and maintained by the vendor for a period of 3 years after the completion of the Project.

For batch Plants, the Plant ticket shall be produced for each bath and maintained by the vendor for a period of 3 years after the completion of the Project. In addition, an asterisk (*) shall be automatically printed next to any individual batch weight(s) exceeding the following tolerances:

Each Aggregate Component	±1.5% of individual or cumulative target weight for each bin
Mineral Filler	±0.5% of the total batch
Bituminous Material	±0.1% of the total batch
Zero Return (Aggregate)	±0.5% of the total batch
Zero Return (Bituminous Material)	±0.1% of the total batch

The entire batching and mixing interlock cut-off circuits shall interrupt and stop the automatic batching operations when an error exceeding the acceptable tolerance occurs in proportioning.

The scales shall not be manually adjusted during the printing process. In addition, the system shall be interlocked to allow printing only when the scale has come to a complete rest. A unique printed character (m) shall automatically be printed on the truck and batch plant printout when the automatic batching sequence is interrupted or switched to auto-manual or full manual during proportioning.

(d) Aggregates: Aggregate stockpiles shall be managed to prevent segregation and cross contamination. For drum Plants only, the percent moisture content, at a minimum prior to production and half way through production, shall be determined.

(e) Mixture: The dry and wet mix times shall be sufficient to provide a uniform mixture and a minimum particle coating of 95% as determined by AASHTO T 195(M).

Bituminous concrete mixtures shall contain no more than 0.5% moisture when tested in accordance with AASHTO T 329.

(f) RAP: RAP moisture content shall be determined a minimum of twice daily (prior to production and halfway through production).

(g) Asphalt Binder: A binder log shall be submitted to the Department's Central Lab on a monthly basis.

(h) Warm mix additive: For mechanically foamed WMA, the water injection rate shall be monitored during production and not exceed 2.0% by total weight of binder. For additive added at the Plant, the dosage rate shall be monitored during production.

(i) Testing Laboratory: The Contractor shall maintain a laboratory to test bituminous concrete mixtures during production. The laboratory shall have a minimum of 300 s.f., have a potable water source and drainage in accordance with the CT Department of Public Health Drinking Water Division, and be equipped with all necessary testing equipment as well as with a PC, printer, and telephone with a dedicated hard-wired phone line. In addition, the PC shall have a high speed internet connection and a functioning web browser with unrestricted access to https://ctmail.ct.gov . This equipment shall be maintained in working order at all times and be made available for use by the Engineer.

The laboratory shall be equipped with a heating system capable of maintaining a minimum temperature of 65°F. It shall be clean and free of all materials and equipment not associated with the laboratory. Sufficient light and ventilation must be provided. During summer months adequate cooling or ventilation must be provided so the indoor air temperature shall not exceed the ambient outdoor temperature.

The laboratory testing apparatus, supplies, and safety equipment shall be capable of performing all the applicable tests in their entirety that are referenced in AASHTO R 35 and AASHTO M 323. The Contractor shall ensure that the Laboratory is adequately supplied at all times during the course of the Project with all necessary testing materials and equipment.

The Contractor shall maintain a list of laboratory equipment used in the acceptance testing processes including, but not limited to, balances, scales, manometer/vacuum gauge, thermometers, and gyratory compactor, clearly showing calibration and/or inspection dates, in accordance with AASHTO R 18. The Contractor shall notify the Engineer if any modifications are made to the equipment within the laboratory. The Contractor shall take immediate action to replace, repair, or recalibrate any piece of equipment that is out of calibration, malfunctioning, or not in operation.

M.04.02—Mix design and Job Mix Formula (JMF)

1. Curb Mix:

(a) Requirements: The Contractor shall use bituminous concrete that meets the requirements of Table M.04.02-1. RAP may be used in 5% increments by weight up to 30%.

(b) Basis of Approval: Annually, an approved JMF based on a mix design for curb mix must be on file with the Engineer prior to use.

The Contractor shall test the mixture for compliance with the submitted JMF and Table M.04.02-1. The maximum theoretical density (Gmm) will be determined by AASHTO T 209. If the mixture does not meet the requirements, the JMF shall be adjusted within the ranges shown in Table M.04.02-1 until an acceptable mixture is produced.

An accepted JMF from the previous operating season may be acceptable to the Engineer provided that there are no changes in the sources of supply for the coarse aggregate, fine aggregate, recycled material (if applicable) and the Plant operation had been consistently producing acceptable mixture.

Any change in component source of supply or consensus properties must be approved by the Engineer. A revised JMF shall be submitted prior to use.

TABLE M.04.02-1:
Control Points for Curb Mix Mixtures

Mix	**Curb Mix**	**Production Tolerances from JMF Target**
Grade of PG Binder content %	**PG 64S-22 6.5 - 9.0**	**0.4**
Sieve Size		
No. 200	3.0 - 8.0 (b)	2.0
No. 50	10 - 30	4
No. 30	20 - 40	5
No. 8	40 - 70	6
No. 4	65 - 87	7
1/4 inch		
3/8 inch	95 - 100	8
1/2 inch	100	8
3/4 inch		8
1 inch		
2 inch		
Additionally, the fraction of material retained between any 2 consecutive sieves shall not be less than 4%.		
Mixture Temperature		
Binder	325°F maximum	
Aggregate	280-350°F	
Mixtures	265-325°F	
Mixture Properties		
Air Voids (VA) %	0 – 4.0 (a)	

Notes: (a) Compaction Parameter 50 gyrations (N_{des})
(b) The percent passing the No. 200 sieve shall not exceed the percentage of bituminous asphalt binder.

2. Superpave Design Method – S0.25, S0.375, S0.5, and S1:

(a) Requirements: All designated mixes shall be designed using the Superpave mix design method in accordance with AASHTO R 35. A JMF based on the mix design shall meet the requirements of Tables M.04.02-2 to M.04.02-5. Each JMF and component samples must be submitted no less than 7 days prior to production and must be approved by the Engineer prior to use. All JMFs expire at the end of the calendar year.

All aggregate component consensus properties and tensile strength ratio (TSR) specimens shall be tested at an AASHTO Materials Reference Laboratory (AMRL) by NETTCP Certified Technicians.

All bituminous concrete mixes shall be tested for stripping susceptibility by performing the TSR test procedure in accordance with AASHTO T 283(M) at a minimum every 36 months. The compacted specimens may be fabricated at the Plant and then tested at an AMRL accredited facility. A minimum of 45000 grams of laboratory or plant blended mixture and the corresponding complete Form MAT-412s shall be submitted to the Division of Material Testing (DMT) for design TSR testing verification. The mixture submitted shall be representative of the corresponding mix design as determined by the Engineer.

i. Superpave Mixtures with RAP: RAP may be used with the following conditions:
   - RAP amounts up to 15% may be used with no binder grade modification.
   - RAP amounts up to 20% may be used provided a new JMF is approved by the Engineer. The JMF submittal shall include the grade of virgin binder added. The JMF shall be accompanied by a blending chart and supporting test results in accordance with AASHTO M 323 Appendix X1, or by testing that shows the combined binder (recovered binder from the RAP, virgin binder at the mix design proportions, warm mix asphalt additive and any other modifier if used) meets the requirements of the specified binder grade.
   - Two (2) representative samples of RAP shall be obtained. Each sample shall be split, and 1 split sample shall be tested for binder content in accordance with AASHTO T 164 and the other in accordance with AASHTO T 308.
   - RAP material shall not be used with any other recycling option.

ii. Superpave Mixtures with RAS: RAS may be used solely in HMA S1 mixtures with the following conditions:
   - RAS amounts up to 3% may be used.
   - RAS total binder replacement up to 15% may be used with no binder grade modification.
   - RAS total binder replacement up to 20% may be used provided a new JMF is approved by the Engineer. The JMF submittal shall include the grade of virgin binder added. The JMF shall be accompanied by a blending chart and supporting test results in accordance with AASHTO M 323 Appendix X1, or by testing that shows the combined binder (recovered binder from the RAP, virgin binder at the mix design proportions, warm mix asphalt additive and any other modifier if used) meets the requirements of the specified binder grade.
   - Superpave Mixtures with RAS shall meet AASHTO PP 78 design considerations.

iii. Superpave Mixtures with CRCG: CRCG may be used solely in HMA S1 mixtures. One percent (1%) of hydrated lime, or other accepted non-stripping agent, shall be added to all mixtures containing CRCG. CRCG material shall not be used with any other recycling option.

(b) Basis of Approval: The following information must be included in the JMF submittal:

i. Gradation, consensus properties and specific gravities of the aggregate, RAP or RAS.
ii. Average asphalt content of the RAP or RAS by AASHTO T 164.
iii. Source of RAP or RAS and percentage to be used.
iv. Warm mix Technology, manufacturer's recommended additive rate and tolerances, and manufacturer recommended mixing and compaction temperatures.
v. TSR test report and anti-strip manufacturer and recommended dosage rate if applicable.
vi. Mixing and compaction temperature ranges for the mix with and without the warm-mix technology incorporated.
vii. JMF ignition oven correction factor by AASHTO T 308.

With each JMF submittal, the following samples shall be submitted to the Division of Materials Testing:

- 4 - one (1) quart cans of PG binder, with corresponding Safety Data Sheet (SDS)

- 1 - 50 lbs. bag of RAP
- 2 - 50 lbs. bags of Plant-blended virgin aggregate

A JMF may not be approved if any of the properties of the aggregate components or mix do not meet the verification tolerances as described in the Department's current QA Program for Materials, Acceptance and Assurance Testing Policies and Procedures.

Any material based on a JMF, once approved, shall only be acceptable for use when it is produced by the designated Plant, it utilizes the same components, and the production of material continues to meet all criteria as specified in Tables M.04.02-2, M.04.02-3 and M.04.02-4. A new JMF must be submitted to the Engineer for approval whenever a new component source is proposed.

Only 1 mix with 1 JMF will be approved for production at a time. Switching between approved JMF mixes with different component percentages or sources of supply is prohibited.

TABLE M.04.02-2: Superpave Master Range for Bituminous Concrete Mixture Design Criteria

	S0.25		**S0.375**		**S0.5**		**S1**	
Sieve	Control Points		Control Points		Control Points		Control Points	
inches	Min (%)	Max (%)	Min (%)	Max (%)	Min (%)	Max (%)	Min (%)	Max (%)
2.0	-	-	-	-	-	-	-	-
1.5	-	-	-	-	-	-	100	-
1.0	-	-	-	-	-	-	90	100
3/4	-	-	-	-	100	-	-	90
1/2	100	-	100	-	90	100	-	-
3/8	97	100	90	100	-	90	-	-
No. 4	72	90	-	72	-	-	-	-
No. 8	32	67	32	67	28	58	19	45
No. 16	-	-	-	-	-	-	-	-
No. 30	-	-	-	-	-	-	-	-
No. 50	-	-	-	-	-	-	-	-
No. 100	-	-	-	-	-	-	-	-
No. 200	2.0	10.0	2.0	10.0	2.0	10.0	1.0	7.0
VMA (%)	16.5 ± 1		16.0 ± 1		15.0 ± 1		13.0 ± 1	
VA (%)	4.0 ± 1		4.0 ± 1		4.0 ± 1		4.0 ± 1	
Gse	JMF value		JMF value		JMF value		JMF value	
Gmm	JMF ± 0.030		JMF ± 0.030		JMF ± 0.030		JMF ± 0.030	
Dust / effective binder	0.6 - 1.2		0.6 - 1.2		0.6 - 1.2		0.6 - 1.2	
TSR	≥ 80%		≥ 80%		≥ 80%		≥ 80%	
T-283 Stripping	Minimal as determined by the Engineer							

(c) Mix Status: Each facility will have each type of bituminous concrete mixture rated based on the results of the previous year of production. Mix status will be provided to each bituminous concrete Producer prior to the beginning of the paving season.

The rating criteria are based on compliance with Air Voids and Voids in Mineral Aggregate (VMA) as indicated in Table M.04.03-4 and are calculated as follows:

Criteria A: Percentage of acceptance test results with compliant air voids.

Criteria B: The average of the percentage of acceptance results with compliant VMA and the percentage of acceptance results with compliant air voids.

The final rating assigned will be the lower of the rating obtained with Criteria A or Criteria B.

Mix status is defined as:

"A" – Approved: Assigned to each mixture type from a production facility with a current rating of 70% or greater, or to each mixture type completing a successful PPT.

"PPT" – Pre-Production Trial: Temporarily assigned to each mixture type from a production facility when:

1. there are no compliant acceptance production test results submitted to the Department from the previous year;
2. there is a source change in one or more aggregate components;
3. there is a component percentage change of more than 5% by weight;
4. there is a change in RAP percentage;
5. the mixture has a rating of less than 70% from the previous season;
6. it is a new JMF not previously submitted; or
7. the average of 10 consecutive acceptance results for VFA, Density to N_{ini} or dust to effective binder ratio does not meet the criteria in tables M.04.02-2 and M.04.02-4.

Bituminous concrete mixtures rated with a "PPT" status cannot be used on Department projects. Testing shall be performed by the Producer with NETTCP certified personnel on material under this status. Test results must confirm that specification requirements in Tables M.04.02-2 through M.04.02-4 are met and the binder content (Pb) meets the requirements in Table M.04.03-2 before material can be used. One of the following methods must be used to verify the test results:

Option A: Schedule a day when a Department Inspector can be at the facility to witness testing

Option B: When the Contractor or their representative performs testing without being witnessed by an Inspector, the Contractor shall submit the test results and a split sample including 2 gyratory molds, 5,000 grams of boxed bituminous concrete, and 5,000 grams of cooled loose bituminous concrete for verification testing and approval

Option C: When the Contractor or their representative performs testing without being witnessed by a Department Inspector, the Engineer may verify the mix in the Contractor's laboratory

Witnessing or verifying by the Department of compliant test results will change the mix's status to "A"

The differences between the Department's test results and the Contractor's must be within the "C" tolerances included in the Department's QA Program for Materials, Acceptance and Assurance Testing Policies and Procedures in order to be verified.

"U" – Not Approved: Status assigned to a type of mixture that does not have an approved JMF. Bituminous concrete mixtures with a "U" status cannot be used on Department projects.

TABLE M.04.02-3:
Superpave Consensus Properties Requirements for Combined Aggregate

Traffic Level	Design ESALs (80kN) Millions	Coarse Aggregate Angularity[1] ASTM D5821, Minimum %	Fine Aggregate Angularity AASHTO T 304, Method A Minimum %	Flat and Elongated Particles[2] ASTM D4791, Maximum %	Sand Equivalent AASHTO T 176, Minimum %
1	< 0.3	55/- -	40	10	40
2	0.3 to < 3.0	75/- -	40	10	40
3	≥ 3.0	95/90	45	10	45

Notes:
[1] 95/90 denotes that a minimum of 95% of the coarse aggregate, by mass, shall have one fractured face and that a minimum of 90% shall have two fractured faces.
[2] Criteria presented as maximum Percent by mass of flat and elongated particles of materials retained on the No. 4 sieve, determined at 5:1 ratio.

TABLE M.04.02-4: Superpave Traffic Levels and Design Volumetric Properties

Traffic Level	Design ESALs	Number of Gyrations by Superpave Gyratory Compactor			Percent Density of Gmm from HMA/ WMA Specimen			Voids Filled with Asphalt (VFA) Based on Nominal Mix Size - Inch			
	(million)	N_{ini}	N_{des}	N_{max}	N_{ini}	N_{des}	N_{max}	0.25	0.375	0.5	1
1	<0.3	6	50	75	≤91.5	96.0	≤98.0	70-80	70-80	70-80	67-80
2	0.3 to <3.0	7	75	115	≤90.5	96.0	≤98.0	65-78	65-78	65-78	65-78
3	≥3.0	7	75	115	≤90.0	96.0	≤98.0	65-77	65-76	65-75	65-75

TABLE M.04.02-5:
Superpave Minimum Binder Content by Mix Type and Level

Mix Type	Level	Binder Content Minimum
S0.25	1	5.80
S0.25	2	5.70
S0.25	3	5.70
S0.375	1	5.70
S0.375	2	5.60
S0.375	3	5.60
S0.5	1	5.10
S0.5	2	5.00
S0.5	3	5.00
S1	1	4.60
S1	2	4.50
S1	3	4.50

M.04.03—Production Requirements:

1. Standard Quality Control Plan (QCP) for Production: The QCP for production shall describe the organization and procedures, which the Contractor shall use to administer quality control. The QCP shall include the procedures used to control the production process, to determine when immediate changes to the processes are needed, and to implement the required changes. The QCP must detail the inspection, sampling and testing protocols to be used, and the frequency for each.

Control Chart(s) shall be developed and maintained for critical aspect(s) of the production process as determined by the Contractor. The control chart(s) shall identify the material property, applicable upper and lower control limits, and be updated with current test data. As a minimum, the following quality characteristics shall be included in the control charts:

- percent passing No. 4 sieve
- percent passing No. 200 sieve
- binder content
- air voids
- Gmm
- Gse
- VMA

The control chart(s) shall be used as part of the quality control system to document variability of the bituminous concrete production process. The control chart(s) shall be submitted to the Engineer the first day of each month.

The QCP shall also include the name and qualifications of a Quality Control Manager. The Quality Control Manager shall be responsible for the administration of the QCP, including compliance with the plan and any plan modifications.

The Contractor shall submit complete production testing records to the Engineer within 24 hours in a manner acceptable to the Engineer.

The QCP shall also include the name and qualifications of any outside testing laboratory performing any QC functions on behalf of the Contractor. The QCP must also include a list of sampling and testing methods and frequencies used during production, and the names of all Quality Control personnel and their

duties.

Approval of the QCP does not imply any warranty by the Engineer that adherence to the plan will result in production of bituminous concrete that complies with these specifications. The Contractor shall submit any changes to the QCP as work progresses.

2. Acceptance Requirements:

(a) General:

For those mixes with a total estimated project tonnage over 500 tons, a NETTCP HMA Paving Inspector certified Contractor representative shall obtain a field sample of the material placed at the project site in accordance with AASHTO T 168 using the procedure indicated in Section 5.2.3 or an alternate procedure approved by the Engineer. Sampling from the truck at the Plant in accordance with AASHTO T 168 using the procedure indicated in Section 5.2.2 will be allowed for those mixes with a total estimated project tonnage equal to or less than 500 tons. Regardless of sampling location, the sample shall be quartered by the Contractor in accordance with AASHTO R 47 and placed in an approved container. The container shall be sealed with a security tape provided by the Department and labelled to include the project number, date of paving, mix type, lot and sublot numbers and daily tonnage. The minimum weight of each quartered sample shall be 14000 grams. The Contractor shall transport one of the containers to the Departments Central Laboratory in Rocky Hill, retain one of the sealed containers for potential use in dispute resolution and test the remaining samples for acceptance in accordance with past practice.

The Contractor shall submit all acceptance tests results to the Engineer within 24 hours or prior to the next day's production. All acceptance test specimens and supporting documentation must be retained by the Contractor and may be disposed of with the approval of the Engineer. All quality control specimens shall be clearly labeled and separated from the acceptance specimens.

Contractor personnel performing QC and acceptance testing must be present at the facility prior to, during, and until completion of production, and be certified as a NETTCP HMA Plant Technician or Interim HMA Plant Technician and be in good standing. Production of material for use on State projects must be suspended by the Contractor if such personnel are not present. Technicians found by the Engineer to be non-compliant with NETTCP policies and procedures or Department policies may be removed by the Engineer from participating in the acceptance testing process for Department projects until their actions can be reviewed.

Verification and dispute resolution testing will be performed by the Engineer in accordance with the Department's QA Program for Materials.

Should the Department be unable to validate the Contractor's acceptance test result(s) for a lot of material, the Engineer will use results from verification testing and re-calculate the pay adjustment for that lot. The Contractor may request to initiate the dispute resolution process in writing within 24 hours of receiving the adjustment and must include supporting documentation or test results to justify the request.

(b) Curb Mix Acceptance Sampling and Testing Procedures: Curb Mixes shall be tested by the Contractor at a frequency of 1 test per every 250 tons of cumulative production, regardless of the day of production.

When these mix designs are specified, the following acceptance procedures and AASHTO test methods shall be used:

TABLE M.04.03-1: Curb Mix Acceptance Test Procedures

Protocol	Reference	Description
1	**AASHTO T 30(M)**	Mechanical Analysis of Extracted Aggregate
2	**AASHTO T 168**	Sampling of Bituminous Concrete
3	**AASHTO T 308**	Binder Content by Ignition Oven Method (adjusted for aggregate correction factor)
4	**AASHTO T 209(M)[(2)]**	Theoretical Maximum Specific Gravity and Density of Bituminous Paving Mixtures
5	**AASHTO T 312[(2)]**	[(1)]Superpave Gyratory Molds Compacted to N_{des}
6	**AASHTO T 329**	Moisture Content of Hot-Mix Asphalt (HMA) by Oven Method

Notes: [(1)] One (1) set equals 2 each of 6 inch molds. Molds to be compacted to 50 gyrations.
[(2)] Once per year or when requested by the Engineer.

i. Determination of Off-Test Status:
   1. Curb Mix is considered "off test" when the test results indicate that any single value for bitumen content or gradation are not within the tolerances shown in Table M.04.02-1 for that mixture. If the mix is "off test," the Contractor must take immediate actions to correct the deficiency and a new acceptance sample shall be tested on the same day or the following day of production.
   2. When multiple silos are located at 1 site, mixture supplied to 1 project is considered as coming from 1 source for the purpose of applying the "off test" status.
   3. The Engineer may cease supply from the Plant when test results from 3 consecutive samples are not within the JMF tolerances or the test results from 2 consecutive samples not within the control points indicated in Table M.04.02-1 regardless of production date.

ii. JMF Revisions
   1. If a test indicates that the bitumen content or gradation are outside the tolerances, the Contractor may make a single JMF revision as allowed by the Engineer prior to any additional testing. Consecutive test results outside the requirements of Table M.04.02-1 JMF tolerances may result in rejection of the mixture.
   2. Any modification to the JMF shall not exceed 50% of the JMF tolerances indicated in Table M.04.02-1 for any given component of the mixture without approval of the Engineer. When such an adjustment is made to the bitumen, the corresponding production percentage of bitumen shall be revised accordingly.

(c) Superpave Mix Acceptance:

i. Sampling and Testing Procedures

Production Lot: The lot will be defined as one of the following types:

- Non-PWL Production Lot for total estimated Project quantities per mixture less than 3500 tons: All mixture placed during a single continuous paving operation.
- PWL Production Lot for total estimated Project quantities per mixture of 3500 tons or more: Each 3500 tons of mixture produced within 30 calendar days.

Production Sub Lot:

- For Non-PWL: As defined in Table M.04.03-2
- For PWL: 500 tons (The last sub lot may be less than 500 tons.)

Partial Production Lots (For PWL only): A Lot with less than 3500 tons due to:

- completion of the course;
- a Job Mix Formula revision due to changes in:
  - cold feed percentages over 5%,
  - target combined gradation over 5%,
  - target binder over 0.15%,
  - any component specific gravity; or
- a lot spanning 30 calendar days.

The acceptance sample(s) location(s) shall be selected using stratified - random sampling in accordance with ASTM D3665 based on:

- the total daily estimated tons of production for non-PWL lots, or
- the total size for PWL lots.

One (1) acceptance sample shall be obtained and tested per sub lot with quantities over 125 tons. The Engineer may direct that additional acceptance samples be obtained. For non-PWL lots, one (1) acceptance test shall always be performed in the last sub lot based on actual tons of material produced.

For non-PWL lots, quantities of the same mixture per Plant may be combined daily for multiple State projects to determine the number of sub lots.

The payment adjustment will be calculated as described in 4.06.

TABLE M.04.03-2:
Superpave Acceptance Testing Frequency per Type/Level/Plant for Non-PWL Lots

Daily Quantity Produced in Tons (Lot)	Number of Sub Lots/Tests
0 to 125	0, Unless requested by the Engineer
126 to 500	1
501 to 1,000	2
1,001 to 1,500	3
1,500 or greater	1 per 500 tons or portions thereof

The following test procedures shall be used for acceptance:

TABLE M.04.03-3: Superpave Acceptance Testing Procedures

Protocol	Procedure	Description
1	AASHTO T 168	Sampling of bituminous concrete
2	AASHTO R 47	Reducing samples to testing size
3	AASHTO T 308	Binder content by ignition oven method (adjusted for aggregate correction factor)
4	AASHTO T 30(M)	Gradation of extracted aggregate for bituminous concrete mixture
5	AASHTO T 312	[(1)]Superpave gyratory molds compacted to N_{des}
6	AASHTO T 166	[(2)]Bulk specific gravity of bituminous concrete
7	AASHTO R 35	[(2)]Air voids, VMA
8	AASHTO T 209(M)	Maximum specific gravity of bituminous concrete (average of 2 tests)
9	AASHTO T 329	Moisture content of bituminous concrete

Notes: (1) One (1) set equals 2 each of 6 inch molds. Molds to be compacted to Nmax for PPTs and to Ndes for production testing. The first sub lot of the year shall be compacted to N_{max}.

(2) Average value of 1 set of 6 inch molds.

If the average ignition oven corrected binder content differs by 0.3% or more from the average of the Plant ticket binder content in 5 consecutive tests regardless of the production date (moving average), the Contractor shall immediately investigate, determine an assignable cause, and correct the issue. When 2 consecutive moving average differences are 0.3% or more and no assignable cause has been established, the Engineer may require a new ignition oven aggregate correction factor to be performed or to adjust the current factor by the average of the differences between the corrected binder content and production Plant ticket for the last 5 acceptance results.

The Contractor shall perform TSR testing within 30 days after the start of production for all design levels of HMA- and PMA- S0.5 Plant-produced mixtures, in accordance with AASHTO T 283(M). The TSR test shall be performed at an AMRL certified laboratory by NETTCP certified technicians. The compacted specimens may be fabricated at the Plant and then tested at an AMRL accredited facility. A

minimum of 45000 grams of plant blended mixture and the corresponding complete Form MAT-412s shall be submitted to the DMT for production TSR testing verification. The mixture submitted shall be representative of the corresponding mix design as determined by the Engineer. Additionally, the TSR test report and tested specimens shall be submitted to the Engineer for review. Superpave mixtures that require anti-strip additives (either liquid or mineral) shall continue to meet all requirements specified herein for binder and bituminous concrete. The Contractor shall submit the name, manufacturer, percent used, technical datasheet and SDS for the anti-strip additive (if applicable) to the Engineer.

i. Determination of Off-Test Status:
   1. Superpave mixes shall be considered *"off test"* when any control point sieve, binder content, VA, VMA, and Gmm value is outside of the limits specified in Table M.04.03-4 or the target binder content at the Plant is below the minimum binder content stated in Table M.04.02-5. Note that further testing of samples or portions of samples not initially tested for this purpose cannot be used to change the status.
   2. Any time the bituminous concrete mixture is considered off-test:
      A. The Contractor shall notify the Engineer when the Plant is *"off test"* for any mix design that is delivered to the Project in any production day. When multiple silos are located at 1 site, mixture supplied to 1 project is considered as coming from 1 source for the purpose of applying the *"off test"* determination.
      B. The Contractor must take immediate actions to correct the deficiency, minimize *"off test"* production to the Project, and obtain an additional Process Control (PC) test after any corrective action to verify production is in conformance with the specifications. A PC test will not be used for acceptance and is solely for the use of the Contractor in its quality control process.

ii. Cessation of Supply for Superpave Mixtures in Non-PWL Lots:

A mixture **shall not be used** on Department projects when it is "off test" for:

1. four (4) consecutive tests in any combination of VA, VMA or Gmm, regardless of date of production, or
2. two (2) consecutive tests in the control point sieves in 1 production shift.

As a result of cessation of supply, the mix status will be changed to PPT

iii. JMF revisions:

JMF revisions are only permitted prior to or after a production shift. A JMF revision is effective from the time it was submitted and is not retroactive to the previous test(s).

JMF revisions shall be justified by a documented trend of test results.

Revisions to aggregate or RAP specific gravities are only permitted when testing is performed at an AMRL certified laboratory by NETTCP certified technicians.

A JMF revision is required when the Plant target RAP or bin percentage deviates by more than 5% or the Plant target binder content deviates by more than 0.15% from the active JMF.

TABLE M.04.03-4: Superpave Mixture Production Requirements

	S0.25		**S0.375**		**S0.5**		**S1**		Tolerances
Sieve	Control Points		Control Points		Control Points		Control Points		From JMF Targets[2]
inches	Min (%)	Max (%)	Min (%)	Max (%)	Min (%)	Max (%)	Min (%)	Max (%)	+/- Tolerance
1.5	-	-	-	-	-	-	100	-	
1.0	-	-	-	-	-	-	90	100	
3/4	-	-	-	-	100	-	-	90	
1/2	100	-	100	-	90	100	-	-	
3/8	97	100	90	100	-	90	-	-	
No. 4	72	90	-	72	-	-	-	-	
No. 8	32	67	32	67	28	58	19	45	
No. 16	-	-	-	-	-	-	-	-	
No. 200	2.0	10.0	2.0	10.0	2.0	10.0	1.0	7.0	
Pb	JMF value		JMF value		JMF value		JMF value		0.3[3]
VMA (%)	16.5		16.0		15.0		13.0		1.0[4]
VA (%)	4.0		4.0		4.0		4.0		1.0[5]
Gmm	JMF value		JMF value		JMF value		JMF value		0.030
Mix Temp. – HMA[6]	265-325°F [1]		265-325°F [1]		265-325°F [1]		265-325°F [1]		
Mix Temp. – PMA[6]	285-335°F [1]		285-335°F [1]		285-335°F [1]		285-335°F [1]		
Prod. TSR	N/A		N/A		≥80%		N/A		
T 283 Stripping	N/A		N/A		Minimal TBD by the Engineer		N/A		

Notes: [1] 300°F minimum after October 15.

[2] JMF tolerances shall be defined as the limits for production compliance.

[3] 0.4 for PWL lots

[4] 1.3 for all PWL lots except S/P 0.25 mixes. 1.1 for S/P 0.25 Non-PWL lots. 1.4 for S/P 0.25 PWL lots

[5] 1.2 for PWL lots

[6] Also applies to placement

Table M.04.03-5:
Modifications to Standard AASHTO and ASTM Test Specifications and Procedures

AASHTO Standard Method of Test	
Reference	**Modification**
T 30	Section 7.2 through 7.4 Samples are not routinely washed for production testing
T 209	Section 7.2 The average of 2 bowls is used proportionally in order to satisfy minimum mass requirements. 8.3 Omit Pycnometer method.
T 283	When foaming technology is used, the material used for the fabrication of the specimens shall be cooled to room temperature, and then reheated to the manufacturer's recommended compaction temperature prior to fabrication of the specimens.
AASHTO Standard Recommended Practices	
Reference	**Modification**
R 26	All laboratory technician(s) responsible for testing PG binders shall be certified or Interim Qualified by NETTCP as a PG Asphalt Binder Lab Technician. All laboratories testing binders for the Department are required to be accredited by the AMRL. Sources interested in being approved to supply PG binders to the Department by use of an "in-line blending system" must record properties of blended material and additives used. Each source of supply of PG binder must indicate that the binders contain no additives used to modify or enhance their performance properties. Binders that are manufactured using additives, modifiers, extenders, etc., shall disclose the type of additive, percentage and any handling specifications or limitations required. All AASHTO M 320 references shall be replaced with AASHTO M 332. Once a month, 1 split sample and test results for each asphalt binder grade and each lot shall be submitted by the PG binder supplier to the Department's Central Lab. Material remaining in a certified lot shall be re-certified no later than 30 days after initial certification. Each April and September, the PG binder supplier shall submit test results for 2 BBR tests at 2 different temperatures in accordance with AASHTO R 29.

SECTION M.05
PROCESSED AGGREGATE BASE AND PAVEMENT SURFACE TREATMENT

M.05.01—Processed Aggregate Base and Pavement
M.05.02—Surface Treatment

M.05.01—Processed Aggregate Base and Pavement: The materials for this work shall meet the following requirements:

1. Gradation: Coarse and fine aggregates shall be combined and mixed by approved methods so that the resulting material shall meet the following gradation requirements:

Table M.05.01-1

Square Mesh Sieves	Percent Passing by Weight
Pass 2 1/2 inches	100
Pass 2 inches	95-100
Pass 3/4 inch	50-75
Pass 1/4 inch	25-45
Pass No. 40	5-20
Pass No. 100	2-12

2. Coarse Aggregate: Coarse aggregate shall be either gravel, broken stone or reclaimed miscellaneous aggregate containing no more than 2% by weight of asphalt cement, at the option of the Contractor. When tested by means of the Los Angeles Machine, using AASHTO Method T 96, the coarse aggregate shall not have a loss of more than 50%.

(a) If gravel is used for the coarse aggregate, it shall consist of sound, tough, durable particles of crushed or uncrushed gravel or a mixture thereof, free from soft, thin, elongated or laminated pieces, lumps of clay, loam and vegetable or other deleterious substances.

(b) If broken stone is used for the coarse aggregate, it shall consist of sound, tough, durable fragments of rock of uniform quality throughout. It shall be free from soft disintegrated pieces, mud, dirt, organic or other injurious material.

(c) If the reclaimed miscellaneous aggregate is used for the coarse aggregate, it shall consist of sound, tough, durable fragments of uniform quality throughout. It shall be free from soft disintegrated pieces, mud, dirt, glass, organic or other injurious material.

(d) Soundness for Gravel, Broken Stone and Reclaimed Miscellaneous Aggregate: When tested by magnesium sulfate solution for soundness using AASHTO Method T 104, the coarse aggregate shall show a loss of not more than 15% at the end of 5 cycles.

3. Fine Aggregate: The fine aggregate shall be natural sand, stone sand, screenings or any combination thereof. The fine aggregate shall be limited to material 95% of which passes a No. 4 sieve having square openings and not more than 8% of which passes a No. 200 sieve. The material shall be free from clay, loam and deleterious materials.

(a) Plasticity: When natural sand is used, the fine aggregate shall conform to the requirements of M.02.06-2.

(b) Plasticity: When screenings or any combination of screenings and natural sand or any combination of stone sand and natural sand are used, the following requirements shall apply:

1. When the fraction of the dry sample passing the No. 100 mesh sieve is 6% or less by weight, no plastic limit test will be made.
2. When the fraction of the dry sample passing the No. 100 mesh sieve is greater than 6% and not greater than 10% by mass, that fraction shall not have sufficient plasticity to permit the performing of the plastic limit test, using AASHTO Method T 90.
3. When the fraction of the dry sample passing the No. 100 mesh sieve is greater than 10% by weight, the sample shall be washed; and additional material passing the No. 100 mesh sieve shall be determined by AASHTO Method T 146, except that the No. 100 mesh sieve shall be substituted for the No. 40 mesh sieve where the latter is specified in AASHTO Method T 146. The combined materials that have passed the No. 100 mesh sieve shall not have sufficient plasticity to permit the performing of the plastic limit test using AASHTO Method T 90.

M.05.02—Surface Treatment: Materials for this work shall meet the following requirements:

1. Bituminous Material: The Bituminous materials meet the requirements of M.04. The Asphalt

Emulsion grade shall be as specified in the Contract or as directed by the Engineer.

The type of bituminous material to be used, as well as its viscosity or grade, will depend upon the character and condition of the surface to be treated, the season of the year at which the work is to be done, and will be determined by the Engineer. The Contractor shall not order any material for this work until it has obtained definite instructions from the Engineer as to the asphalt emulsion that is required and as to the type of the bituminous material selected.

2. Sand Cover: Sand shall contain no more than 3% inorganic silt and clay by actual dry weight, using AASHTO Method T 11 and shall meet the following gradation requirements:

Table M.05.02-1

Square Mesh Sieves	Percent Passing by Weight
Pass 1/2 inch	100
Pass 3/8 inch	95-100
Pass No. 4	80-100
Pass No. 50	10-30
Pass No. 100	0-10

SECTION M.06
METALS

M.06.01—Reinforcing Steel
M.06.02—Structural Steel and Other Structural Materials
M.06.03—Galvanizing
M.06.04—Filler Metal for Welding

M.06.01—Reinforcing Steel: The materials for this work shall meet the following requirements:

1. Bar Reinforcement: Bar reinforcement shall be deformed and conform to the following:

Uncoated bar reinforcement shall meet the requirements of ASTM A615, Grade 60.

Epoxy coated bar reinforcement shall meet the requirements of ASTM A615, Grade 60 and shall be epoxy coated to the requirements of ASTM A775. All field repairs of the epoxy coating shall meet the requirements of ASTM D3963.

Galvanized bar reinforcement shall meet the requirements of ASTM A615, Grade 60 and be galvanized, after fabrication, to the requirements of ASTM A767, Class 1, including supplemental requirements. Dowels and tie bars for masonry facing and for granite curbing shall be galvanized, after fabrication, in accordance with ASTM A767, Class 1.

Weldable bar reinforcement shall meet the requirements of ASTM A706.

2. Unit Weights: Listed below are the bar sizes with approximate weights, diameters, areas and perimeters.*

Bar Designation No.**	Nominal Weight lb./ft	Diameter Inches	Cross Sectional Area s.i.	Perimeter inches
3	0.376	0.375	0.11	1.178
4	0.668	0.500	0.20	1.571
5	1.043	0.625	0.31	1.963
6	1.502	0.750	0.44	2.356
7	2.044	0.875	0.60	2.749
8	2.670	1.000	0.79	3.142
9	3.400	1.128	1.00	3.544
10	4.303	1.270	1.27	3.990
11	5.313	1.410	1.56	4.430
14	7.65	1.693	2.25	5.32
18	13.60	2.257	4.00	7.09

* Nominal dimensions of deformed bars are equivalent to those of plain round bars having the same weight (pounds per foot) as deformed bars.

** Bar numbers are based on the number of eighths of an inch included in the nominal diameter of the bars.

3. Wire and Welded Steel Wire Fabric: Wire shall be cold-drawn steel wire meeting the requirements of ASTM A1064 (AASHTO M 32).

Welded steel wire fabric, when used as reinforcement in concrete, shall meet the requirements of ASTM A1064 (AASHTO M 55). The type of welded steel wire fabric shall be approved by the Engineer.

4. Bar Mat Reinforcement: Bar mat, reinforcement shall conform to the requirements of ASTM A184 (AASHTO M 54).

5. Dowel Bar Mechanical Connections: Dowel bar mechanical connections shall develop in tension and compression at least 125% of the specified yield strength of the bar reinforcement being spliced.

Epoxy coated mechanical connectors shall be epoxy coated in accordance with the requirements of ASTM D3963.

Galvanized mechanical connectors shall be galvanized, after fabrication, in accordance with the requirements of ASTM A767, Class 1, including supplemental requirements.

Prior to incorporation into the work, samples of the uncoated, epoxy coated and galvanized dowel bar mechanical connections shall be submitted to the Engineer for destructive testing in accordance with the latest edition of the Materials Testing Manual's "Minimum Schedule for Acceptance Testing."

6. Deformed Steel Wire and Welded Deformed Steel Wire Fabric: Deformed steel wire shall be cold-worked, deformed steel wire meeting the requirements of AASHTO M 225 (ASTM A1064). Welded

deformed steel wire fabric, when used as reinforcement in concrete, shall meet the requirements of AASHTO M 221 (ASTM A1064). The type of welded deformed steel wire fabric shall be approved by the Engineer.

7. Reinforcing Steel for Pavement: Reinforcing steel for pavement shall be in accordance with the applicable standard plans.

8. Reports and Certification: Mill test reports and materials certification shall be submitted for all types of reinforcing steel and dowel bar mechanical connections confirming they meet the requirements of the applicable specifications.

Materials Certificates shall be submitted in accordance with 1.06.07 for all types of reinforcing steel and dowel bar mechanical connections.

M.06.02—Structural Steel: Certified Test Reports and Materials Certificates for structural steel shall be submitted in accordance with 1.06.07.

The materials for this work shall meet the following requirements:

1. Structural Steel: Structural steel shall correspond to the designation shown on the plans.

Unless otherwise indicated in the plans or specifications, structural steel for non-bridge related members or components shall meet the requirements of ASTM A709, Grade 36.

All surfaces of steel plates and shapes used in fabrication shall be blast cleaned and visually inspected by the Contractor prior to any fabrication or preparation for fabrication. Blast cleaning shall meet the requirements of SSPC-SP-10-Near White Blast Cleaning.

All steel plates and shapes used in fabrication shall be substantially free from pitting and gouges, regardless of the cause. Substantially free is defined as:

(a) The measured surface area of all pits and gouges regardless of depth represent less than 1% of the surface area of the plate or shape.

(b) No pit or gouge greater than 1/32 inch deep.

(c) No pit or gouge closer than 6 inches from another.

Any repair of plates or shapes shall be performed in accordance with ASTM A6.

2. Anchor Bolts: Unless otherwise designated on the plans, anchor bolts, including suitable nuts and washers, shall meet the following requirements:

(a) Anchor bolt assemblies shall conform to the requirements of ASTM F1554, and the grade shall be as specified on the plans. All components of the bolt assembly shall be galvanized in accordance with ASTM F2329.

(b) Certified Test Reports and Material Samples: The Contractor shall submit notarized copies of Certified Test Reports in accordance with 1.06.07. Prior to incorporation into the work, the Contractor shall submit samples of the anchor bolt assemblies to the Engineer for testing in accordance with the latest edition of the Materials Testing Manual's "Minimum Schedule for Acceptance Testing." One (1) sample shall be submitted for each diameter, material designation, grade or coating of anchor bolt assembly.

3. High Strength Bolts: High strength bolts, including suitable nuts and hardened washers, shall meet the following requirements:

(a) High strength bolts shall meet the requirements of ASTM F3125 Grade A325 or ASTM F3125 Grade A490 as shown on the plans. High-strength bolts used with coated steel shall be mechanically galvanized, unless otherwise specified. High-strength bolts used with uncoated weathering grades of steel shall be Type 3.

Nuts for ASTM F3125 Grade A325 bolts shall meet the requirements of ASTM A563, Grades DH, DH3, C, C3 and D. Where galvanized high-strength bolts are used, the nuts shall be galvanized, heat-treated Grade DH. Where Type 3 high-strength bolts are used, the nuts shall be Grade C3 or DH3.

Nuts for ASTM F3125 Grade A490 bolts shall meet the requirements of ASTM A563, Grade DH. Where Type 3 high-strength bolts are used, the nuts shall be Grade DH3.

All galvanized nuts shall be lubricated with a lubricant containing a visible dye of any color that contrasts with the color of the galvanizing. Black bolts must be oily to the touch when delivered and installed.

Circular flat and square or rectangular beveled, hardened steel washers shall meet the requirements of ASTM F436. Unless otherwise specified, galvanized washers meeting the requirements of ASTM B695, Class 55 shall be furnished when galvanized high-strength bolts are specified. Where Type 3 high-strength bolts are used, the washers shall be galvanized in

accordance with ASTM B695, Class 55 and coated with epoxy.

(b) Identifying Marks: ASTM F3125 Grade A325 for bolts and the specifications referenced therein for nuts require that bolts and nuts manufactured to the specification be identified by specific markings on the top of the bolt head and on one face of the nut. Markings shall be raised or depressed at the manufacturer's option and shall be visible after coating if coating is required. Head markings must identify the grade by the symbol "A325," the manufacturer and the type, if Type 3. Nut markings must identify the grade, the manufacturer and if Type 3, the type. Other washer markings must identify the manufacturer and if Type 3, the type.

ASTM F3125 Grade A490 for bolts and the specifications referenced therein for nuts require that bolts and nuts manufactured to the specifications be identified by specific markings on the top of the bolt head and on one face of the nut. Markings shall be raised or depressed at the manufacturer's option and shall be visible after coating if coating is required. Head markings must identify the grade by the symbol "A490" the manufacturer and the type, if Type 3. Nut markings must identify the grade, the manufacturer and if Type 3, the type. Other washer markings must identify the manufacturer and if Type 3, the type.

ASTM F3125 Grade A325 and ASTM F3125 Grade A490 bolt lengths up to 4 times the diameter which are fully threaded but which are not required to be fully threaded by the relevant ASME standard shall be marked with a "T" immediately after the grade designation, for example "A325T." Bolts with any other non-standard dimensions, including thread length, shall be marked with an "S" immediately after the grade designation, for example "A325S." All other markings, if used, such as a private label distributor's mark shall also be separate and distinct.

(c) Dimensions: Bolt and nut dimensions shall conform to the requirements for Heavy Hexagon Structural Bolts and for Heavy Semi-Finished Hexagon Nuts given in ASME Standard B18.2.6.

(d) Galvanized Bolts: Galvanized bolts shall meet the requirements of ASTM F3125 Grade A325, Type 1. The bolts shall be hot-dip galvanized in accordance with ASTM F2329, to a thickness of 50 µm or mechanically galvanized in accordance with ASTM B695, Class 55. Bolts, nuts, and washers of any assembly shall be galvanized by the same process. The nuts shall be overtapped to the minimum amount required for the fastener assembly, and shall be lubricated with a lubricant containing a visible dye so a visual check can be made for the lubricant at the time of field installation. Galvanized bolts shall be tension tested after galvanizing. ASTM F3125 Grade A490 bolts shall be uncoated or shall be coated in accordance with either ASTM F1136 Grade 3 or ASTM F2833 Grade 1.

(e) Test Requirements: The maximum hardness of ASTM F3125 Grade A325 bolts shall be 34 HRC. The maximum hardness of ASTM F3125 Grade A490 bolts shall be 38 HRC.

Plain, ungalvanized nuts shall have a minimum hardness of 89 HRB.

Proof load tests, in accordance with the requirements of ASTM F606 Method 1, shall be required for the bolts. Wedge tests of full-size bolts are required in accordance with Section 10.1 of ASTM F3125. Galvanized bolts shall be wedge tested after galvanizing. Proof load tests of ASTM A563 are required for nuts. Proof load tests for nuts used with galvanized bolts shall be performed after galvanizing, overtapping and lubricating.

Rotational-capacity tests are required and shall be performed on all plain or galvanized (after galvanizing) bolt, nut and washer assemblies by the manufacturer or distributor prior to shipping and by the Contractor at the Site.

The thickness of galvanizing on bolts, nuts and washers shall be measured. On bolts, it shall be measured on the wrench flats or on top of the bolt head, and on nuts it shall be measured on the wrench flats.

(f) Certified Test Reports and Materials Certificates: The Contractor shall submit notarized copies of Certified Test Reports and Materials Certificates in accordance with 1.06.07 for fastener assemblies. In addition the Certified Test Reports and Materials Certificates shall include the following:

1. Mill test reports shall indicate the place where the material was melted and manufactured.
2. Test reports for proof load tests, wedge tests, and rotational-capacity tests shall indicate where the tests were performed, date of tests, location of where the components were manufactured and lot numbers.
3. The test report for galvanized components shall indicate the thickness of the galvanizing.

(g) Material Samples: Prior to incorporation into the work, the Contractor shall submit samples of the bolt assemblies to the Engineer for testing in accordance with the latest edition of the Materials Testing Manual's "Minimum Schedule for Acceptance Testing." Samples shall be submitted for each diameter, length, material designation, grade, coating and manufacturer of bolt assembly.

4. Welded Stud Shear Connectors:

(a) Materials: Stud shear connectors shall conform to the requirements of ASTM A108, cold-drawn bar, Grades 1015, 1018 or 1020, either semi- or fully-killed. If flux-retaining caps are used, the steel for the caps shall be of a low carbon grade suitable for welding and shall comply with ASTM A109.

Stud shear connectors shall be of a design suitable for electrically end-welding to steel with automatically timed stud welding equipment. The studs shall be of the sizes and dimensions noted on the plans. Flux for welding shall be furnished with each stud, either attached to the end of the stud or combined with the arc shield for automatic application in the welding operation. Each stud shall be furnished with a disposable ferrule of sufficient strength to remain intact during the welding operation and not crumble or break; it shall not be detrimental to the weld or create excessive slag.

Tensile properties, as determined by tests of bar stock after drawing or of finished studs, shall conform to the following requirements in which the yield strength is as determined by the 0.2% offset method:

Tensile strength (min.)	60,000 psi
Yield strength (min.)	50,000 psi
Elongation (min.)	20% in 2 inches
Reduction of area (min.)	50%

(b) Test Methods: Tensile properties shall be determined in accordance with the applicable sections of ASTM A370. Tensile tests of finished studs shall be made on studs welded to test plates using a test fixture similar to that shown in Figure 7.2 of the current AASHTO/AWS D1.5 – Bridge Welding Code. If fracture occurs outside of the middle half of the gauge length, the test shall be repeated.

(c) Finish: Finished studs shall be of uniform quality and condition, free from injurious laps, fins, seams, cracks, twists, bends or other injurious defects. Finish shall be as produced by cold-drawing, cold-rolling or machining.

(d) Certified Test Reports and Materials Certificates: The Contractor shall submit a certified copy of the in-plant quality control test report in conformance with 1.06.07. The Contractor shall submit a Materials Certificate in conformance with 1.06.07 for the welded studs.

(e) Sample Materials for Testing: Prior to incorporation into the work, the Contractor shall submit samples of the stud shear connectors to the Engineer for testing in accordance with Chapter 8 in the latest edition of the Material Testing Manual's "Minimum Schedule for Acceptance Testing." One (1) sample shall be submitted for each diameter and length of welded stud.

M.06.03—Galvanizing: Unless otherwise specified on the plans or in the special provisions, the zinc coating on all iron and steel materials, other than wire, shall meet the requirements of ASTM A123, A153 or F2329, whichever shall apply.

When mechanical galvanizing is used it shall meet the requirements of ASTM B695 Class 55.

M.06.04—Filler Metal for Welding: Unless otherwise shown on the plans or as indicated in the special provisions, fill metal for welding shall conform to the requirements of AWS.

The electrode classification number and other identification references for the proposed electrodes and flux shall be noted on the shop or working drawings.

SECTION M.07
PAINT

M.07.01—General for All Paints and Enamels
M.07.02—Coating Systems for Structural Steel
M.07.03—Vacant
M.07.04—Vacant
M.07.05—Vacant
M.07.06—Vacant
M.07.07—Vacant
M.07.08—Vacant
M.07.09—Vacant
M.07.10—Vacant
M.07.11—Vacant
M.07.12—Vacant
M.07.13—Vacant
M.07.14—Vacant
M.07.15—Vacant
M.07.16—Vacant
M.07.17—Vacant
M.07.18—Vacant
M.07.19—Vacant
M.07.20—Waterborne Pavement Marking Paint
M.07.21—Hot-Applied Waterborne Pavement Marking Paint
M.07.22—Epoxy Resin Pavement Markings
M.07.23—Vacant
M.07.24—Preformed Black Line Mask Pavement Marking Tape
M.07.25—Vacant
M.07.30—Glass Beads

M.07.01—General for All Paints and Enamels:

1. **Paints and enamels** shall consist of pigments of the required fineness and composition, ground in the required vehicle by a suitable grinding machine to the required fineness. All pigments, resins, oils, thinners and driers shall be free from adulterants.
2. **Proportions:** All proportions in formulas are by weight unless otherwise specified.
3. **Fineness:** All pigments, except aluminum, unless otherwise specified, shall be finely ground with 100% passing the No. 200 sieve; with no less than 97% passing the No. 325 sieve.
4. **Curdling, Livering, Leveling:** The paint or enamel shall not liver or curdle. The pigment shall remain in suspension in a satisfactory manner through the expected shelf life specified on the label. The enamel type paints shall level properly and not show brush marks.
5. **Colors:** All paints and enamels shall be matched to the Department's standard shades.
6. **Time of Drying:** All paints or enamels, unless otherwise specified, shall dry to full gloss in not more than 18 hours.
7. **Weight per Gallon:** The weight per gallon of all paints and enamels shall be determined at 77°F.
8. **Shipping:** All paints and enamels shall be shipped in containers plainly marked with the name, net weight and volume of paint or enamel content. The manufacturer's name, address, date and lot number shall be marked on every package.
9. **Samples, Sampling, and Testing:** The manufacturer shall supply a Certified Test Report per lot for any pigment, oil, resin, thinner, drier or paint. When a portion of the lot is delivered, a Material Certificate is required. Upon request by the Engineer, the manufacturer shall submit a sample in accordance with the latest edition of the Materials Testing Manual's "Minimum Schedule for Acceptance Testing."

Sampling and testing shall be performed in accordance with ASTM, Federal Standards, or by methods established by the Department.

M.07.02—Coating Systems for Structural Steel: The coating system used shall be specified in the Contract and shall be selected from the Northeast Protective Coating Committee's (NEPCOAT's) Specification Criteria for Protective Coatings qualified products list.

Color: The color of the topcoat material shall be as noted on the plans (FS 595 Color Number).

Packaging and Labeling of Coating Material: The container shall be designed to store the specific coating material. Each container of coating material shall bear a label that identifies the name of the coating manufacturer, the name of the product, the lot and batch numbers, the date of manufacture and the shelf life expiration date. The label shall also include complete specific instructions for opening the container and for mixing, thinning, and applying the coating material contained therein. If the coating material cannot be positively identified from the label on the container, it shall not be used.

Delivery: Coating material shall be furnished in the manufacturer's original sealed and undamaged container.

Control of Materials: For each coating material, a Materials Certificate shall be submitted in conformance with 1.06.07. The Material Certificate shall indicate compliance with NEPCOAT Acceptance Criteria for Protective Coatings, List A or B.

M.07.03—Vacant

M.07.04—Vacant

M.07.05—Vacant

M.07.06—Vacant

M.07.07—Vacant

M.07.08—Vacant

M.07.09—Vacant

M.07.10—Vacant

M.07.11—Vacant

M.07.12—Vacant

M.07.13—Vacant

M.07.14—Vacant

M.07.15—Vacant

M.07.16—Vacant

M.07.17—Vacant

M.07.18—Vacant

M.07.19—Vacant

M.07.20—Waterborne Pavement-Marking Paint: Pavement-marking paint shall be waterborne paint and shall be white or yellow, depending on its use, for application on bituminous concrete and Portland cement concrete pavement. This paint shall be compatible with the stripe-painting equipment to be used on the Project. All requirements shall be as specified in M.07.21, except as follows:

1. Total nonvolatile compounds shall not be less than 70% by weight.
2. Pigment shall be 50 to 60% by weight.
3. Drying time for no-pick-up shall be 15 minutes or less when tested in accordance with ASTM D711.
4. The Contractor shall provide a Materials Certificate in accordance with 1.06.07 for each portion of a batch or lot delivered to the Project site.

M.07.21—Hot-Applied Waterborne Pavement-Marking Paint: Fast-drying waterborne pavement-marking paint to be applied on bituminous concrete and Portland cement concrete pavements shall be the color specified on the plans. This paint shall be capable of being applied with stripe-painting equipment at an application temperature of 130 to 145°F and shall have good spraying characteristics. The Contractor shall provide a Materials Certificate in accordance with 1.06.07 for each portion of a batch or lot delivered to the Project site.

General: Specifications and publications that apply are as follows:

- FS TT-P-1952 - Paint, Traffic and Air Field Marking, Waterborne
- Federal Test Method Standard (FTMS) No.141 - Paint, Varnish, Lacquer and Related Materials, Methods of Inspection, Sampling and Testing
- FS No. 595 – Colors

ASTM Standards:

- D211 - Specifications for Chrome Yellow and Chrome Orange Pigments
- D476 - Classification for Dry Pigmentary for Titanium Dioxide Pigments

Detailed Requirements, Formulation and Manufacture: The paint shall be formulated and manufactured from first-grade raw materials and shall be free from defects and imperfections. The materials shall not exhibit settling or jellying after storage in the sealed containers upon receipt. The paint shall provide the proper anchorage, refraction and reflection for the finished glass spheres when applied as specified.

Composition: The composition of the paint material shall meet the requirements of any applicable Federal, State or Local regulation for products of this type and shall meet the following requirements:

1. Paint shall not contain more than 0.06% lead when tested in accordance with ASTM D3335
2 Total nonvolatile organic compounds shall be a minimum of 76% by weight
3. Pigment shall be 58 to 63% by weight when tested in accordance with ASTM D3723
4. Resin solids shall be composed of 100% acrylic emulsion polymer
5. Volatile organic compounds shall not exceed 1.25 lb./gal. excluding water when tested in accordance with ASTM D2369
6. Flash Point: Closed-cup flash point shall not be less than 145°F
7. Density: Weight per gallon shall not be less than 12.5 lb./gal. when tested in accordance with ASTM D1475

Viscosity: The consistency of the paint shall not be less than 80, nor more than 90 Krebs units when tested in accordance with ASTM D562.

Flexibility: The paint shall not show cracking or flaking when tested in accordance with ASTM D522.The panels shall be lightly buffed with steel wool and thoroughly cleaned with solvent before being used for tests.

Dry Opacity: Both white and yellow paints shall have a minimum contrast ratio of 0.96 when tested in accordance with ASTM D2805. Contrast ratio shall be determined by applying a wet film thickness of 0.005 inch to a standard hiding-power chart. After drying, the black- and- white-reflectance values shall be determined using a suitable reflectometer and the contrast ratio determined.

Bleeding: The paints shall have a minimum bleeding ratio of 0.97 when tested in accordance with FS TT-P-1952.

Abrasion Resistance: No less than 210 liters of sand shall be required to remove paint film when tested in accordance with TT-P-1952.

Color: The paint shall not discolor in sunlight and shall maintain colorfastness throughout its life. Color determination shall be made without beads, after a minimum of 24 hours. Color for yellow paint shall be a visual match for FS 595-13538. If not a visual match, the diffuse day color of the paint when tested in accordance with ASTM E1347 shall conform to the CIE Chromaticity coordinate limits as follows:

	x y	x y	x y	x y	Brightness
White	0.305 0.295	0.360 0.360	0.388 0.377	0.280 0.3l0	84.0 min
Yellow	0.485 0.455	0.506 0.452	0.484 0.428	0.477 0.438	50.0 min

Glass Bead Adhesion: The paint with glass beads conforming to M.07.30, applied at the rate of 6.0 lb./gal. of paint, shall require not less than 150 liters of sand to remove paint film and glass beads.

Scrub Resistance: The paint shall pass 300 cycles minimum when tested in accordance with ASTM D2486.

Drying Time: Drying time to no pick-up shall be 3 minutes or less when tested in accordance with ASTM D711.

M.07.22—Epoxy Resin Pavement Markings:

General Requirements:

Identification: Each container must be labeled with the following information: Name and address of manufacturer, production batch number, date of manufacture, grade name and/or identification number,

type of material, number of gallons, Contract number, directions for mixing and application.

Certification*:* The Contractor shall provide a Material Certificate in accordance with 1.06.07 for each portion of a batch or lot delivered to the Site.

Detailed Requirements:

(a) Epoxy Resin Material: The material shall be composed of epoxy resins and pigments only. The white and the yellow epoxy resin materials shall be composed of approved materials and be lead- and chromium-free.

(b) Composition:

WHITE (percent by weight)	YELLOW (percent by weight)
20% ± 2% Titanium Dioxide (ASTM D476 Type III)	
80% ± 2% Epoxy Resins	75% ± 2% Epoxy Resins

(c) Color: The white material shall be the color of chip 17778 of FS No. 595 of the latest issue, when the material is placed in a type EH weatherometer for a period of 500 hours and weathered according to ASTM G152. The yellow material shall be the color of chip 13538 of the FS No. 595 of the latest issue.

(d) Adhesion Capabilities: When the adhesion of the material to Portland cement concrete is tested in accordance with AASHTO T 237, the failure of the system must take place in the concrete.

(e) Abrasion Resistance: When the abrasion resistance of the material is tested according to ASTM D4060 with a CS-17 wheel under a load of 1000 grams for 1000 cycles, the wear index shall be no greater than 82.

(f) Hardness: The Type D durometer hardness of the material shall be not less than 75 nor more than 90 when tested in accordance with ASTM D2240 after the material has cured for 72 hours at 73°F ± 3.5°F.

(g) Tensile Strength: The tensile strength of the material, when tested in accordance with ASTM D638, shall not be less than 6,000 psi after 72 hours cure at 73°F ± 3.5°F.

(h) Compressive Strength: The compressive strength of the material, when tested in accordance with ASTM D695, shall not be less than 12,000 psi after 72 hours cure at 73°F ± 3.5°F.

(i) Shelf Life: The individual components shall not require mixing prior to use when stored for a period of 12 months.

(j) Glass Beads: The glass beads shall meet the requirements of M.07.30.

M.07.23—Vacant

M.07.24—Preformed Black-Line Mask Pavement-Marking Tape:

General Requirements*:* The preformed, patterned black-line mask pavement-marking tape shall consist of a matte black, non-reflective tape in widths or sizes sufficiently large to mask the existing markings which are to be temporarily covered.

The patterned masking tape shall be pre-coated with a pressure sensitive adhesive and shall be capable of being adhered to existing markings, on bituminous concrete pavement or Portland cement concrete in accordance with the manufacturer's instructions without the use of heat, solvents or other additional adhesives, and shall be immediately ready for traffic use after application. The Contractor shall identify equipment necessary for proper application and removal, and make recommendations for application that will assure effective product performance.

The preformed, patterned black-line masking pavement-marking tape shall be suitable for use for 1 year after the date of receipt when stored in accordance with the manufacturer's recommendations.

Detailed Requirements:

(a) Composition: The non-reflective, patterned black-line mask pavement-marking tape shall not contain metallic foil and shall consist of a mixture of high quality polymeric materials, pigments and inorganic fillers distributed throughout its base cross-sectional area, with a matte black non-reflective top layer. The patterned surface shall have a minimum of 20% of the surface area raised and coated with non-skid particles. The channels between the raised areas shall be substantially free of particles. The film shall be pre-coated with a pressure sensitive adhesive. A non-metallic medium shall be incorporated to facilitate removal.

(b) Skid Resistance: The surface of the patterned, non-reflective black-line mask pavement-marking tape shall provide an initial average skid resistance value of 60 British Pendulum Number when tested in accordance with ASTM E303.

(c) Thickness: The patterned material, without adhesive, shall have a minimum thickness of 0.065 inch

at the thickest portion of the patterned cross-section and a minimum thickness of 0.02 inch at the thinnest portion of the cross-section.

(d) Adhesion: The black-line mask pavement-marking tape shall adhere to the pavement and existing pavement markings under climatic and traffic conditions normally encountered in the construction work zone.

(e) Removability: The black-line mask pavement-marking tape shall be capable of being removed after its intended use without the use of heat, solvents, grinding, sand or water blasting.

M.07.25—Vacant

M.07.30—Glass Beads: The glass beads shall meet the requirements of AASHTO M 247, Type 1 or 4, depending on application.

SECTION M.08
DRAINAGE

M.08.01—Pipe
General
Iron/Steel
1. **Cast Iron Pipe**
2. **Coated Corrugated Metal Pipe and Coated Corrugated Metal Pipe Elbows**
3. **Perforated or Plain Coated Metal Pipe for Underdrains and Outlets**
4. **Coated Corrugated Metal Pipe Arches**
5. **Corrugated Structural Plates and Bolts**
6. **Metal Culvert Ends**

Concrete
7. **Reinforced Concrete Pipe**
8. **Reinforced Concrete Elliptical Pipe**
9. **Perforated Reinforced Concrete Pipe for Underdrains and Outlets**
10. **Vacant**
11. **Reinforced Concrete Culvert Ends**

Aluminum
12. **Corrugated Aluminum Pipe**
13. **Corrugated Aluminum Pipe for Underdrains and Outlets**
14. **Corrugated Aluminum Pipe Arches**

Sealers/Gaskets
15. **Cold-Applied Bituminous Sealer**
16. **Preformed Plastic Gaskets**
17. **Flexible, Watertight, Rubber-Type Gaskets**

Plastic
18. **Corrugated Polyethylene Pipe**
19. **Geotextiles**
20. **Polyvinyl Chloride Plastic Pipe**
21. **Polyvinyl Chloride Gravity Pipe**

M.08.02—Catch Basins, Manholes and Drop Inlets
M.08.03—Aggregates
1. **Bedding Material**
2. **Aggregates for Underdrains**

M.08.01—Pipe:

General

The Contractor shall submit manufacturer's material certifications for all metal and plastic pipes other than PVC, metal pipe-arches, metal fittings and metal coupling bands in accordance with 1.06.07.

IRON/STEEL

1. Cast Iron Pipe:

This material shall meet the requirements of AASHTO M 64 for Extra-Heavy Cast Iron Culvert Pipe.

2. Coated Corrugated Metal Pipe and Coated Corrugated Metal Pipe Elbows:

This material shall meet the following requirements:

Pipe fabricated from zinc-coated steel sheet and aluminum-coated (Type 2) steel sheet must meet AASHTO M 36, Type 1 or IR.

Pipe fabricated from metallic-coated and polymer-precoated steel sheet must meet AASHTO M 245, Type 1.

Unless otherwise indicated on the plans, the corrugation size and sheet thickness shall meet the following requirements:

TABLE M.08.01-1

Nominal Inside Diameter (inches)	Corrugations	Minimum Specified Sheet Thickness (inches)	
6	1 1/2" × 1/4"	.052	
8, 10	1 1/2" × 1/4"	.064	
12, 15, 18 & 21	2 2/3" × 1/2"	.064	
24, 30 , 36	2 2/3" × 1/2"	.079	
42, 48	2 2/3" × 1/2"	.109	
54, 60	3" × 1" or 5" × 1"	.064	
66, 72	3" × 1" or 5" ×1"	.079	
78, 84, 90, & 96	3" × 1" or 5" × 1"	.109	
		Steel	Aluminum
18, 24, 30	Helical Rib ¾" × ¾ "× 7 ½"	.064	.060
36	Helical Rib ¾" × ¾ "× 7 ½"	.064	.075
42, 48 & 54	Helical Rib ¾" × ¾ "× 7 ½"	.079	.105
60, 66, 72, 78, 84	Helical Rib ¾" × ¾ " × 7 ½"	.109	.135

Aluminum pipe sheet thickness may be 0.004 inch less than specified in Table M.08.01-1 for 1 1/2 inch × 1/4 inch, 2 2/3 inch × 1/2 inch, and 3 inch × 1 inch or 5 inch × 1 inch corrugations. Helical Rib shall be as specified in Table M.08.01-1.

Zinc coated steel pipe, fittings, and coupling bands shall be coated with bituminous material as specified in AASHTO M 190 Type C. Pipe, fittings and coupling bands fabricated from aluminum coated steel sheet (Type 2) do not require coating of bituminous material or paved invert.

Metallic-coated and polymer-precoated steel pipe, fittings, and coupling bands shall be coated as specified in AASHTO M 246, Type B. The thicker polymeric coating shall be on the inside of the pipe.

Only 1 type of coating will be allowed for any continuously connected run of pipe.

If elongation of the pipe is required, it shall be done by the manufacturer.

3. Perforated or Plain Coated Metal Pipe for Underdrains or Outlets:

This material shall meet the requirements of AASHTO M 36, Type III or AASHTO M 245, Type III.

(a) Perforations: The minimum diameter of perforations after asphalt coating shall be 1/4 inch.

(b) Coating: All requirements of M.08.01-2 shall apply except that the minimum thickness of the bituminous coating on zinc coated steel pipe, fittings, and coupling bands pipe shall be 0.03 inch, not 0.05 inch.

4. Coated Corrugated Metal Pipe-Arches:

This material shall conform to the requirements of AASHTO M 36, Type II, Type IIR or AASHTO M 245, Type II. All coating requirements of M.08.01-2 shall apply.

Unless otherwise indicated on the plans, the corrugation size and sheet thickness shall meet the dimensional requirements in Table M.08.01-2.

5. Corrugated Structural Plates and Bolts:

These plates and bolts are for use in the construction of metal pipe of the large diameter and for metal plate arches or pipe arches to be assembled in the field, and they shall meet the requirements of AASHTO M 167 for corrugated metal pipe.

The dimensions of plates and details of fabrication shall conform to the requirements of the manufacturer. Where the plans call for a heavier gage for the bottom of the pipe than for the remainder of the pipe circumference, the lower fourth of the circumference shall be the minimum width of the heavier gage material.

TABLE M.08.01-2

Pipe-Arch Equivalent Diameter (Inches)	Corrugations	Minimum Sheet Thickness (Inches)
15, 18, 21	2 2/3" × 1/2"	.064
24, 30	2 2/3" × 1/2"	.079
36, 42, 48	2 2/3" × 1/2"	.109
54, 60	2 2/3" × 1/2"	.138
60, 66, 72	3" × 1" or 5" × 1"	.079
78, 84, 90, 96	3" × 1" or 5" × 1"	.109
18, 21, 24	Helical Rib ¾" × ¾ " × 7 ½"	.064
30, 36	Helical Rib ¾" × ¾ " × 7 ½"	.079
42, 48, 54, 60	Helical Rib ¾" × ¾ " × 7 ½"	.109

The coating shall meet the requirements of AASHTO M 243.

6. Metal Culvert End:

The materials used in this work shall meet the pertinent requirements of M.08.01-2 and M.08.01-4.

Bolts and fittings shall meet the requirements of ASTM A307 and shall be galvanized to meet the requirements of ASTM F2329.

The units shall be coated as specified in M.08.01-2, M.08.01-4 or M.08.01-5.

Fabrication: These units shall be formed from a rectangular sheet of metal by cutting and bending to form the desired shape. Two (2) or more sheets may be fastened together by riveting or bolting so as to form a rectangular sheet of the required width. Skirt extensions and a top plate, as needed to complete the unit, shall be separately formed. Skirt extensions shall be riveted or bolted to the skirt.

All edges which will be exposed above the surface of the ground shall be reinforced before forming the unit by either of the following means:

(1) The edge shall be bent to form a semicircular roll with an exterior diameter of 1 inch, as shown in the detail drawing on the plans.

(2) A split tube of 1 inch outside diameter and not lighter than 14 gage, shall be slipped over a row of rivets spaced not more than 6 inches apart, as shown in the detail drawing on the plans.

One (1) corrugation, matching the corrugations of the pipe or pipe-arch to which the unit is to be attached, shall be formed in the unit to ensure secure and accurate alignment.

Attachment: The unit may be shop-riveted to a length of the appropriate pipe or pipe-arch, or may be field attached to the pipe or pipe arch by either of the other attachment systems shown on the plans, or by other means acceptable to the Engineer. If the unit is shop-riveted to a length of pipe or pipe-arch, this length shall be sufficient to permit proper use of standard coupling bands.

CONCRETE

7. Reinforced Concrete Pipe:

Unless otherwise specified, this pipe shall meet the requirements of AASHTO M 170, Class IV, as supplemented and modified by the following:

(a) Reinforcement: In circular pipe, only circular reinforcement will be allowed.

(b) Laps and Welds: The reinforcement shall be lapped not less than 2 inches and welded with an electric welding machine.

(**c**) **Quality Assurance Testing**: Circular and elliptical reinforced concrete pipe shall be tested by the three-edge bearing method prescribed in AASHTO T 280, except as follows:

(1) Modified or special design pipe shall be tested to the 0.01 inch load and the ultimate load requirements as per AASHTO M 170 and M 207.

(2) At the discretion of the Engineer, pipe of standard design, as specified in AASHTO M 170 and M 207, may be tested to the 0.01 inch requirement plus 10% additional load in lieu of ultimate load testing. Test pipe attaining a 0.01 inch crack will not be acceptable for use on Department projects.

(3) Cores for absorption and determination of steel reinforcement shall be taken on a random basis as determined by the Engineer. The cores shall be at least 6 inches diameter.

(d) Inspection: The pipe plant, materials, processes of manufacture and the finished pipe shall be subject to inspection and approval by the Department. The pipe manufacturer's records related to component materials, production and shipment of pipe for Department use shall be made available to the Department on request. The equipment and labor necessary for inspection, sampling and testing as required by the Department shall be furnished by the pipe manufacturer. Test equipment shall be calibrated at least once each 12 months, or as directed by the Engineer. The plant cement and aggregate scales shall be inspected and sealed by the approved agency at least once every 12 months.

(e) Preliminary Tests and Tests for Extended Deliveries: As directed by the Engineer, the Department shall select for test from the stock of any manufacturer proposing to supply pipe to the Department, 2 of each size pipe up through 30 inch diameter and 1 of each size greater than 30 inch diameter. These sample pipes shall be tested under Department supervision by the three-edge bearing method. For pipe that fails, it shall be necessary for the manufacturer to either physically isolate the rejected pipe at his plant or to provide some means to clearly indicate the unacceptability of the pipe. Either method shall be performed to the satisfaction of the Engineer. When production is resumed on any size, wall thickness or class previously rejected, preliminary tests shall be required. If 95% of all pipe tested at a particular plant from the first of the calendar year to September 30 meet specifications, including both preliminary and extended tests, it will not be necessary to perform the Fall three-edge bearing tests at this plant.

Use of compression tests on representative cylinders or cores to determine the compressive strength of the concrete incorporated into the pipe products will be at the discretion of the Engineer.

(f) Shipping: Pipe shall not be shipped until it is at least 7 days old unless earlier shipment is authorized by the Engineer on the basis of tests.

(g) Certification: Pipe will be accepted by the Department on the basis of manufacturer's certification. The manufacturer shall certify each shipment of pipe on Department Form MAT-314 (PC-1), "Certification of Precast Concrete Products." Two (2) copies of this certification shall be furnished with the shipment to the Engineer at the Project Site.

8. Reinforced Concrete Elliptical and Arch Pipe:

Reinforced concrete elliptical pipe shall be in accordance with AASHTO M 207, Class HE IV. Reinforced concrete arch pipe shall be in accordance with AASHTO M 206, Class A IV. Manufacturing and testing for both pipes shall meet M.08.01-7.

9. Perforated Reinforced Concrete Pipe for Underdrain and Outlets:

This material shall meet the requirements of M.08.01-7 and shall be slotted in accordance with AASHTO M 175, Type 2 or as shown on the plans. Pipe for outlets shall not be perforated.

10. Vacant

11. Reinforced Concrete Culvert End:

The barrel shall meet the requirements of AASHTO M 170, Class II, except that the three-edge bearing tests will not be required. The flare shall be of the same thickness and materials as the barrel, and shall have steel reinforcement equaling or exceeding the amount shown on the table for the pertinent size.

Tongues and grooves shall be compatible with tongues and grooves of pipe meeting AASHTO M 170, Class IV.

Air entrainment shall be added to these units so as to maintain 5 to 8% entrained air.

ALUMINUM

12. Corrugated Aluminum Pipe:

This pipe shall meet the requirements of AASHTO M 196 Type I or Type IR. Sheet thickness shall be as specified in M.08.01-2.

13. Corrugated Aluminum Pipe for Underdrains and Outlets:
This material shall meet the requirements of AASHTO M 196 Type III or Type IIIR. Sheet thickness shall conform to the requirements of M.08.01-2. Pipe for outlets shall not be perforated.

14. Corrugated Aluminum Pipe Arches:
These pipe arches shall meet the requirements of AASHTO M 196 Type II or Type IIR. Sheet thickness shall be as specified in M.08.01-4.

SEALERS/GASKETS

15. Cold-Applied Bituminous Sealer:
This material, for use in sealing of joints in concrete pipes, shall be free of asbestos and shall meet the following requirements:

It shall be of such consistency that it may be spread on the joints with a trowel when the temperature of the air is between -20°F and 100°F. The bituminous material shall adhere to the concrete pipe so as to make a watertight seal, and shall not flow, crack or become brittle when exposed to the atmosphere.

Unless otherwise specified, sampling shall be done in accordance with AASHTO T 40.

The bituminous sealer shall be delivered to the Project in suitable containers for handling and shall be sealed or otherwise protected from contamination. The container shall show the brand name, net mass or volume, and the requirements for application.

16. Preformed Plastic Gaskets:
This material for use in sealing of joints in concrete pipe shall meet the requirements of ASTM C1478.

17. Flexible, Watertight, Rubber Gaskets:
This material, for use in sealing of joints in concrete drainage structures, shall meet the requirements of ASTM C443.

PLASTIC

18. Corrugated Polyethylene Pipe: Corrugated polyethylene pipe shall meet the requirements of AASHTO M 252 for diameters 3 to 10 inches and M 294 for diameters 12 inches and over.

19. Geotextiles:
The geotextile shall be non-rotting, acid and alkali resistant and have sufficient strength and permeability for the purpose intended, including handling and backfilling operations. Fibers shall be low water absorbent. The fiber network must be dimensionally stable and resistant to delamination. The geotextile shall be free of any chemical treatment or coating that will reduce its permeability. The geotextile shall also be free of any flaws or defects which will alter its physical properties. Torn or punctured geotextiles shall not be used. For each specific use, only geotextiles that are already on the Connecticut Department of Transportation's Qualified Products List for the geotextile type will be used. The Engineer reserves the right to reject any geotextile deemed unsatisfactory for a specific use. The brand name shall be labeled on the geotextile or the geotextile container. Geotextiles which are susceptible to damage from sunlight or heat shall be so identified by suitable warning information on the packaging material.

Geotextiles susceptible to sunlight damage shall not be used in any installations where exposure to light will exceed 30 days, unless specifically authorized in writing by the Engineer.

20. Polyvinyl Chloride Plastic Pipe:
The pipe shall conform to the requirements of ASTM D1785. Couplings and elbows shall conform to the requirements of ASTM D2466 or D2467.

21. Polyvinyl Chloride Gravity Pipe:
This pipe shall conform to one of the following specifications: ASTM F789, ASTM F679 or ASTM F794.

M.08.02—Catch Basins, Manholes and Drop Inlets: The materials to be used in the construction shall conform to the following:

1. Brick for Catch Basins, Manholes or Drop Inlets: Brick for catch basins, manholes or drop inlets shall conform to the requirements of ASTM C32 except that the depth shall be 2 1/4 inches, the width 3 5/8 inches and the length 8 inches, and except that the maximum water-absorption by 5 hour boiling shall not exceed the following limits:

Average of 5 bricks	15%
Individual brick	18%

2. Concrete Building Brick for Catch Basins, Manholes or Drop Inlets: Concrete building brick for catch basins, manholes or drop inlets shall conform to the requirements of ASTM C55, Grade S II.

3. Masonry Concrete Units for Catch Basins, Manholes or Drop Inlets: Masonry concrete units for

catch basins, manholes or drop inlets shall conform to the requirements of ASTM C139.

4. Precast Units for Drainage Structures: Precast units for drainage structures may be used except where particular conditions require building or casting in place.

Fabrication plants shall have a quality control plan approved by the Division Chief of Materials Testing that is demonstrated to the satisfaction of the Engineer. The facility, the quality of materials, the process of fabrication and the finished precast units shall be subject to inspection by the Engineer.

Precast manholes shall conform to the requirements of AASHTO M 199 (ASTM C478).

Circular precast catch basins and drop inlets shall conform to AASHTO M 199 (ASTM C478) as supplemented below. Rectangular precast catch basins and drop inlets shall conform to ASTM C913 as supplemented below:

All materials used for concrete shall conform to the requirements of M.03.

The pertinent provisions of 6.01.03 shall apply except that the concrete shall contain 5.0% - 8.0% entrained air. Water-absorption of individual cores taken from precast units shall be not more than 7%.

Reinforcement shall conform to the requirements of M.06.01.

Suitable provision shall be made in casting the units for convenient handling of the completed casting, and additional reinforcement steel shall be provided to allow for such handling in the casting yard and during transportation and placement. Each completed unit shall be identified with the name of manufacturer and date of the concrete pour from which it was cast, either by casting this information into an exposed face of the unit or by suitable stencil. For each day's production of precast units, the fabricator shall mold, cure and test standard cylinders, or cylinders compacted in a similar manner to the parent precast units, for the purpose of determining the compressive strength of the concrete incorporated into the precast units. Concrete used in molding the cylinders shall be representative of the concrete incorporated into the precast units during the production period. Cylinders shall be molded in accordance with AASHTO T 23, cured by the same method as the units they represent, and tested as prescribed in AASHTO T 22.

The fabricator shall determine the air content of the concrete used in the day's production of precast units by performing tests as prescribed in AASHTO T 152.

The equipment and personnel necessary to perform the required testing shall be furnished by the fabricator and approved by the Engineer. All testing equipment shall be calibrated at least once each 12 months or as directed by the Engineer. The fabricator shall maintain records relative to the production, testing and shipment of precast units supplied to the Department. Said records shall be available to a representative of the Department upon request.

The Department may accept precast concrete units on the basis of fabricator’s certification. The fabricator shall certify each shipment of precast concrete units on Department Form MAT 314 (PC-1), "Certification of Precast Concrete Products." Two (2) copies of this certification shall be furnished with the shipment to the Engineer at the Project Site.

Precast units that are cracked, show evidence of honeycomb, or have over 10% of their surface area patched may be subject to rejection, even though meeting other requirements.

5. Metal for Drainage Structures: Metal for catch basins, drop inlet and manhole frames, extensions, covers, and gratings shall be cast iron, structural steel or malleable iron meeting the requirements of the plans. Covers and gratings shall bear uniformly on their supports.

Extension risers shall be designed so that the existing manhole cover or catch basin grate, when set in place, will have substantially the same bearing, fit and load carrying capacity as in the existing frame. The extension shall be designed to fit into the original frame, resting specifically on the flange and rim area. The extension shall accept the existing cover or grate so that the cover or grate is seated firmly without movement.

Steps (ladder rungs) for manholes shall conform to AASHTO M 199 (ASTM C478).

Cast iron shall meet the requirements of AASHTO M 306 "Standard Specification for Drainage, Sewer, Utility and Related Castings," and must be certified to the loading standard of H-25 or HS-25 as detailed in AASHTO M306, Section 6 "Proof-Load Testing." Cast iron material shall be Class 35B Gray Iron or Ductile Iron, minimum 50 ksi yield strength.

Gray Iron Castings shall meet the requirements of ASTM A48 and AASHTO M 105, Class 35B, and must meet all minimum requirements of AASHTO M 306. All covers, grates and frames must be rated H-25 in accordance with AASHTO M 306, Section 6 "Proof-Load Testing."

Ductile Iron Castings shall meet the requirements of ASTM A536, 80-55-06 or 70-50-05, and must meet all minimum requirements of AASHTO M 306. All covers, grates and frames must be rated H-25 in accordance with AASHTO M 306, Section 6 "Proof-Load Testing."

Structural Steel shall meet the requirements of ASTM A36, or A283, Grade B or better, as to quality and details of fabrication, except that in the chemical composition of the steel, the 2/10 of 1% of copper may be omitted.

Malleable iron shall meet the requirements of ASTM A47, Grade 22010.

The materials and method of manufacture for drop inlets shall conform to the requirements as stated on the plans or as ordered.

M.08.03—Aggregates

1. Bedding Material: Material for pipe bedding shall be sand or sandy soil, all of which passes a 3/8 inch sieve, and not more than 10% passes a No. 200 sieve.

When ground water is encountered, the Engineer may allow No. 6 stone conforming to M.01.02 to be used instead of sand or sandy soil.

2. Aggregates for Underdrains: Materials for filling the trench shall consist of well-graded, clean, non-plastic sands or well-graded, clean, durable broken stone or screened gravel. Unless otherwise noted, the type of material to be used shall be sand.

Sand: This material shall meet the requirements of M.03.01-2.

Broken Stone or Screened Gravel: The crushed stone shall meet the gradation requirements of Table M.01.02-2 for Size No. 8 coarse aggregate.

SECTION M.09
SHEET PILING AND PILES

M.09.01—Sheet Piling
M.09.02—Piles

M.09.01—Sheet Piling: The materials for this work shall meet the following requirements:

1. Timber Sheet Piling: The timber, unless otherwise definitely noted on the plans or in the special provisions, may consist of any species which will satisfactorily stand driving. It shall be sawn or hewn with square corners and shall be free from worm holes, loose knots, wind shakes, decayed or unsound portions, or other defects which might impair its strength or tightness.

The piles shall be of the dimensions shown on the plans or as directed, either cut from the solid material or made by building up the piles of 3 planks securely fastened together. The piles shall be drift sharpened at their lower ends so as to wedge the adjacent piles tightly together.

2. Steel Sheet Piling: Steel sheet piles shall conform to the requirements of ASTM A328.

M.09.02—Piles: The materials for this work shall meet the following requirements:

1. Timber Piles: Timber piles shall meet the requirements of AASHTO M 168, as supplemented by the following:

All timber piles shall be cut from sound, live trees. Piles shall contain no unsound knots or other defects which may impair their strength and durability. All knots shall be trimmed close to the body of the piles. All measurements for piles shall be taken under the bark.

Piles shall have the following minimum dimensions:

Pile Length	Tip Dia. Inches	Butt Dia. Inches
20 feet and under	8	11
21 to 40 feet	8	12
41 to 60 feet	7	13
Over 60 feet	6	15

2. Treatment of Timber Piles: Treated timber piles shall be made of douglas fir, western larch, southern yellow pine, Norway pine or red oak. They shall be peeled of outer bark and inner skin soon after cutting so that the piles are smooth and clean. Piles shall contain as much sapwood as possible, and care shall be taken to minimize damage to the sapwood when peeling and cleaning the piles. After preparation, the minimum thickness of the sapwood ring at the butt end shall be 1 inch, except for southern yellow pine which shall be 1 1/2 inches.

Conditioning, treatment and wood preservative shall meet the requirements of AASHTO M 133 and AWPA Standards U1 and T1, UC5A.

The minimum retention of preservative shall be in accordance with AWPA Standards.

Prior to furnishing the treated timber piles, the Contractor shall submit a Materials Certificate, in accordance with 1.06.07, certifying the species, grade or class of pile material, the grade of wood preservative used and the preservative retention.

3. Steel Piles: Piles shall be rolled steel sections of the mass and shape called for on the plans or specified by the Engineer. Piles, splice plates and points shall be new material meeting the requirements of ASTM A36. Stock Steel conforming to these requirements and free from surface imperfections will be accepted upon presentation of certified mill test reports as to the quality of the stock material and receipt of affidavit that the stock material furnished is a product of the open-hearth method manufacture. Immediately prior to driving, steel piles when placed in leads shall not exceed the camber and sweep permitted by allowable mill tolerance. Piles bent or otherwise injured shall be rejected.

4. Precast Concrete Piles: Precast concrete piles shall be in strict accordance with the details shown on the plans or in the special provisions. The materials shall conform to the following requirements:

(a) Concrete: The concrete for the piles shall be air-entrained concrete composed of Portland cement, fine and coarse aggregates, admixtures and water. The air-entraining feature may be obtained by the use of either air-entraining Portland cement or an approved air-entraining admixture. The entrained air content shall be not less than 4% or more than 6%.

The Contractor shall design and submit for the approval of the Engineer a concrete mix which shall attain

a minimum 28-day compressive strength of 5000 psi, or as shown on the plans. The Contractor shall further provide a certified statement that the mix submitted shall meet the requirements.

Prior to any handling or pickup, after casting of piles the compressive strength shall be 3500 psi minimum.

(b) Coarse Aggregate shall consist of broken stone, having a maximum size of 3/4 inch, meeting the requirements of M.03.01.

(c) Fine Aggregate, water, air-entraining admixture and retarder admixture shall meet the requirements of M.03.01.

(d) Portland Cement for the units shall meet the requirements of M.03.01, except that Type III or Type IIIA Portland cement may be used at no additional cost to the State.

(e) Water Reducing Admixture: The Contractor may submit, for approval of the Engineer, a water-reducing admixture for the purpose of increasing the workability and reducing the water requirement for the concrete.

(f) Calcium Chloride: The addition to the mix of calcium chloride, or admixtures containing calcium chloride, will not be permitted.

(g) Steel Reinforcement: Steel Reinforcement shall meet the requirements of M.06.01.

5. Cast-in-Place Concrete Piles: Cast-in-place concrete piles shall be in strict accordance with the details shown on the plans or in the special provisions. Unless otherwise indicated, the concrete shall be Class PCC03360 as specified in M.03, and no air-entraining shall be provided. The steel reinforcement shall meet the requirements of M.06.01

Shells shall be manufactured specifically for use as piles. The use of other type metal products such as piping, tubing or circular forms shall not be permitted without prior written approval of the Engineer. Any such approval will be contingent upon the actual driving performance of the shell in place.

6. Prestressed Concrete Piles (Pretensioned): Materials for pretensioned piles shall conform to the requirements of M.14.01, except as follows:

Concrete: The concrete for the piles shall be air-entrained concrete composed of Portland cement, fine and coarse aggregates, admixtures and water. The air-entraining feature may be obtained by the use of either air-entraining Portland cement or, if permitted by the Engineer, an approved air-entraining admixture. The entrained air content shall be not less than 4% or more than 6%.

The Contractor shall design and submit for the approval of the Engineer a concrete mix, which shall attain a minimum strength of 5000 psi. The mix shall have a minimum cement content of 6.0 bags per cubic yard. The compressive cylinder strength at the time of transfer of prestressing load shall be a minimum of 3500 psi. No piles shall be removed from the casting plant, nor be transported to the work site, nor be driven until the compressive strength is 5000 psi minimum.

The Contractor shall further provide a certified statement that the mix submitted shall meet the requirements. Its ultimate strength at 28 days shall have a minimum value of 5000 psi.

Fine Aggregate: The fine aggregate shall meet the requirements of M.03.01-2 except as amended by the addition of the following:

Fine aggregate for use in prestressed piles shall not contain any materials that are deleteriously reactive with the alkalies in the cement in an amount sufficient to cause excessive expansion of mortar, grout or concrete; except that if such materials are present in injurious amounts, the Portland cement shall be modified in accordance with the provisions listed under the requirements for Portland cement.

Portland Cement: Portland cement for the piles shall conform to the requirements of M.03.01, except as supplemented and amended hereinafter.

Type III or Type IIIA Portland cement may be used at the Contractor's option. The tricalcium aluminate ($3Ca\ Al_2O_3$) content of the Portland cement shall be limited to a maximum of 8%. Where high sulfate resistance is required, as specified on the plans or as directed, the tricalcium aluminate content shall be limited to a maximum of 5%. This criteria is attained when the water-soluble sulfate (as SO_4) in soil exceeds 0.20% or the sulfate (as SO_4) in ground or surface water exceeds 1,000 parts per million.

Where the fine aggregate contains materials that are deleteriously reactive with the alkalies in the cement, as noted in the requirements of fine aggregate, the Portland cement shall contain less than 0.6% alkalies calculated as sodium oxide or shall have a material added that has been shown to prevent harmful expansion due to the alkali-aggregate reaction.

Splicing cement shall meet the requirements of the special provisions.

Pile Tip: Steel for the pile tip shall conform to the requirements of M.06.02. Anchorages for the pile tip shall conform to the requirements of M.06.01 or M.06.02, whichever shall apply.

Jet Pipe: The jet pipe and fittings shall be a 2 inch galvanized steel pipe provided with a top connection and bottom jetting outlets as shown on the plans or as directed. With the approval of the Engineer, other material may be substituted for the galvanized pipe, connections, fittings and outlets.

Pile Extension: Materials for pile extensions shall meet the requirements listed above or as noted on the plans.

SECTION M.10
RAILING AND FENCE

M.10.01—Cable Guide Railing and Anchorages
M.10.02—Metal Beam-Type Rail and Anchorages
M.10.03—Vacant
M.10.04—Wire Fence
M.10.05—Chain Link Fence
M.10.06—Vacant
M.10.07—Vacant
M.10.08—Three-Cable Guide Railing (I-Beam Posts) and Anchorages

M.10.01—Cable Guide Railing and Anchorages: The materials for this work shall meet the following requirements:

1. Wire Rope: Wire rope shall be Class A, 3/4 inch diameter, meeting the requirements of AASHTO M 30.

2. Fittings: All fittings shall meet the details as shown on the plans. Fittings subject to the direct action of the wire rope shall be sufficiently strong to develop the full-specified tensile strength of the rope. Fittings used in the attachment of 2 ropes shall be sufficiently strong to develop the full-specified tensile strength of both ropes. Other fittings shall be in accordance with standard commercial specifications, and shall be free from flaws or defects that would tend to impair their use or durability.

All metal fittings shall be galvanized, after fabrication, to meet the requirements of ASTM A153.

3. Steel Posts: All steel posts shall meet the requirements of ASTM A36.

Steel posts shall meet the details shown on the plans as to size, shape and weight; and they shall be punched or drilled as indicated on the plans. After fabrication, all posts shall be galvanized to meet the requirements of ASTM A123.

4. Anchorages: Anchorages shall be as shown on the plans.

5. Wood Posts: Wood posts shall meet the requirements of M.10.04-2 except that the diameter shall be as shown on the plans

6. Steel Eyebolt and Standard Turnbuckle: The steel eyebolt and standard turnbuckle shall meet the requirements of ASTM A237 and shall be galvanized to meet the requirements of ASTM F2329.

7. Connector Plate Bolts: The connector plate bolts shall meet the requirements of ASTM F3125 Grade A325 and shall be galvanized to meet the requirements of ASTM F2329.

8. Cast Steel Connector Plate: The cast steel connector plate shall meet the requirements of ASTM A27, Grade 65-35 and shall be galvanized to meet the requirements of ASTM A123.

9. Malleable Iron Connector Plate: The malleable iron connector plate shall meet the requirements of ASTM A47, Grade No. 32510 and shall be galvanized to meet the requirements of ASTM A123.

M.10.02—Metal Beam-Type Rail and Anchorages: The materials for this work shall comply with the plans as to size, shape and weight.

1. Steel Posts, Welded-Soil Plates, Brackets, Back-Up Rails and Channel Rubrails:

(a) Steel posts, welded-soil plates, brackets, back-up rails and channel rubrails shall meet the requirements of ASTM A36. After fabrication, all steel posts, welded-soil plates, brackets, back-up rails and channel rubrails shall be galvanized to meet the requirements of ASTM A123.

(b) All welding shall meet ANSI/AASHTO/AWS D1.5.

2. Wood Posts: Wood posts shall be commercial lumber Grade No. 1 or better and shall meet AASHTO M 168. The posts shall be either rough sawn (non-planed) or S4S (surface four sides) southern yellow pine or douglas fir, or western larch with nominal dimensions as indicated on the plans. Actual dimensions of the posts shall not vary by more than 1/4 inch from the dimensions shown on the plans.

After all end cuts are made and all holes are drilled the wood shall be treated in accordance with AASHTO M 133 and the AWPA Standards.

3. Rail Elements (W-Beam, Thrie-Beam) and Terminal Sections: Rail elements and terminal sections shall meet the requirements of AASHTO M 180 and the following:

(a) Class A (12 gauge): Base metal nominal thickness, 0.105 inch

(b) Class B (10 gauge): Base metal nominal thickness, 0.135 inch

(c) Galvanizing shall be Type II.

(d) Acceptance shall be based on AASHTO M 180 Article 5.3 "Acceptance by Brand Registration and

Guarantee."

(e) Rail elements with radii less than or equal to 150 feet, as shown on the plans, shall be shop fabricated and then permanently stamped or embossed with the designated radius (R = _____) on the element near the brand registration stamp.

4. Box Beam Rail Elements: Elements shall be either structural tubing 8 inches × 6 inches × 1/4 inch or structural tubing 6 inches × 6 inches × 3/16 inch manufactured from either ASTM A500 Grade B cold-rolled tubing, ASTM A501 hot-rolled tubing or Automatic Rollover Protective Steel. When ASTM A500 Grade B steel is used, the DROP-Weight-Tear Test in conformance with ASTM E436 shall be performed. All plates shall meet ASTM A36. All material for box beam rail elements and splices shall be galvanized after fabrication in accordance with ASTM A123.

5. Steel Plates, Steel Washer Plates and Square Steel Washers: These components shall meet the requirements of ASTM A36 and shall be galvanized to meet the requirements of ASTM A123 unless otherwise noted on the plans.

6. Bolts, Rods, Washers, and Nuts: Anchor bolts and rods for attachment to barriers and parapets shall meet ASTM A449. The nuts for anchor bolts and rods shall meet ASTM A563, Grade B. The washers for anchor bolts or rods shall meet the requirements of ASTM F436. All other bolts and nuts, unless otherwise noted on the plans, shall meet the requirements of ASTM A307.

Bolts, nuts and washers, unless otherwise noted on the plans, shall be galvanized after fabrication to meet the requirements of ASTM F2329.

7. End Anchorages: The bar reinforcement shall meet the requirements of M.06.01-1. The Class PCC03340 Concrete shall meet the requirements of M.03. Anchor bolts and rods for end anchorages shall meet the requirements of AASHTO M 314.

8. Galvanized Coating Touch-up: The zinc dust-zinc oxide paint for galvanized coating touchup shall meet the requirements of AASHTO M 180.

9. Plastic Blockouts: Plastic blockouts shall be made with a minimum of 50% recyclable polyethylene plastic comprised of low-density and high-density polyethylene with a specific gravity less than or equal to 1.0 in accordance with ASTM D792 and be recyclable. They shall also have a minimum compressive stress of 450 psi in accordance with ASTM D695, meet the dimensions indicated on the plans, and be a shade of gray or black. Blockouts must have been crash-tested and have approval in writing by the FHWA in compliance with MASH, Test Level 3 requirements. Each blockout shall be stamped at the factory with the manufacture's identification and lot number. The Contractor shall furnish to the Engineer prior to construction a Certified Test Report and a Materials Certificate for the blockouts in conformance with 1.06.07.

M.10.03—Vacant

M.10.04—Wire Fence:

1. Wire Fence: The wire fence shall be 9 gauge woven wire fence and shall be hot-dip galvanized in accordance with ASTM A116.

Staples shall be No. 9 galvanized wire staples 1 1/2 inches long. Nails shall be 16d galvanized.

Vertical stays, securely welded to the horizontal wires, shall be provided at equal intervals and shall run from top to bottom of the fence.

2. Treated Wood Posts: Wood posts and wood braces for wire fence shall meet the requirements of AASHTO M 168 and shall be cut from one of the following species: Norway (red) pine, southern yellow pine, scotch pine, pitch pine, oak, red maple, black birch or yellow birch.

All posts shall be straight of the size and length shown on the plans. The wood bracing shall be 4 inches × 4 inches as shown on the plans.

Conditioning, treatment and wood preservative shall meet the requirements of AASHTO M 133 and AWPA Standards U1 and T1 for UC4B Commodity Specification B (Posts). All posts and braces shall be treated for their full length.

3. Metal Post: Metal posts shall be of the length shown on the plans, straight and true to section, and shall be of a standard commercial type. Hot-dip galvanizing shall be in accordance with ASTM A116, Class 2.

All posts, braces, anchors, plates, hardware and other devices shall be galvanized on all inner and outer surfaces by an approved method.

All end posts shall have 1 brace; all corner and intermediate braces or pull posts shall have 2 braces.

(a) Line posts shall be of the following types and shall meet the minimum requirements stated for each:

- Quadruple Ribbed Tee Post with minimum weight of 1.32 lb./l.f.
- Channel or U Post with minimum weight or 1.12 lb./l.f.
- Standard Tee Post with minimum weight of 1.22 lb./l.f.
- Tubular Post with minimum outside diameter 1 3/4 inches minimum gauge No. 15 (U.S. Standard)
- Angle Post with minimum section 2 inches × 2 inches × 1/4 inch

(b) End posts, corner posts, brace posts and braces shall be of the following types and shall meet the minimum requirements stated for each:

- Tubular Section Post and Brace with minimum outside diameter 2 1/2 inches, minimum gauge No. 8 (U.S. Standard)
- Standard Pipe Section Post and Brace with minimum weight of 3.65 lb./l.f.
- Angle Post and Brace with minimum section of post 2 1/2 inches × 2 1/2 inches 1/4 inch, minimum section of brace 2 inches × 2 inches × 1/4 inch

The minimum weights stated for the several types of posts do not include anchors, plates or other devices. Intermediate or line posts shall be provided with a plate or anchor or other satisfactory means to hold the posts in proper alignment and plumb. Plates or anchors shall be securely fastened to the post by welding or by a minimum of 2 rivets per plate.

All posts having a tubular or pipe section shall be provided with a suitable cap at the top.

M.10.05—Chain Link Fence: All gauge measurements of finished wire shall be United States Steel Wire Gauge or equivalent. Tolerance for wire sizes shall be as specified in AASHTO M 181. When aluminized (aluminum-coated) steel fabric is used, the posts and hardware shall be galvanized. When aluminum fabric is used, the posts and hardware shall be aluminum. When polyvinyl chloride-coated steel fabric is used, the posts and hardware shall be polyvinyl chloride-coated.

Materials for this work shall meet the following requirements:

1. Fabric: Wire Fencing shall be composed of chain link woven wire. It shall be the height specified on the plans and shall be constructed of 9 gauge wire. The wire shall be woven to form a continuous fabric having 2 inch mesh. The chain link fabric shall have a knuckled finish on both top and bottom edges.

(a) Aluminized Steel Fabric: The base metal of the fabric shall be of steel wire having a minimum tensile strength of 80,000 psi, coated with aluminum alloy applied at the rate of not less than 0.40 ounces/square foot of uncoated wire surface.

(b) Polyvinyl chloride-coated steel fabric shall meet the requirements of ASTM D1785, and shall be the color black or as noted on the plans.

(c) Aluminum Alloy Fabric shall meet the requirements of ASTM B211, Alloy 6061 wire having a minimum tensile strength of 50,000 psi.

2. Metal Posts and Rails: Metal posts shall be straight, true to section and of sufficient length to enable the post to be encased for a depth of 3 feet deep into a concrete footing which shall have a depth 3 feet 6 inches below ground.

The Contractor shall provide a Materials Certificate in accordance with 1.06.07 for all posts, rails, braces, anchors, plates and other devices with coating of the following types, which shall meet the minimum requirements for each:

(a) Galvanized material shall be made of steel of a standard commercial type meeting the requirements of ASTM F1083 High Strength Grade Schedule 40 and ASTM F1043 Group 1A. It shall be hot-dip galvanized with a zinc coating weighing not less than 2.0 ounces/square foot when tested in accordance with AASHTO T 65 and shall be in accordance with AASHTO M 181, Grade 2.

(b) Aluminized steel posts and rails shall meet ASTM F1043, minimum yield strength 50,000 psi, for industrial chain link fence. The posts and rails shall be manufactured by roll forming aluminum coated steel strip and electric resistance welding into tubular form. The outside of the weld area shall be metabolized with commercially pure aluminum to a thickness sufficient to provide resistance to corrosion equal to that of the remainder of the outside of the tube. The aluminum coating weight on the outer and inner surfaces shall be a minimum of 0.75 ounces/square foot, Triple spot test, 0.70 ounces/square foot, single spot test, as measured in accordance with ASTM A428 and shall meet the requirements of ASTM F1043 Group 1C.

(c) Polyvinyl chloride-coated material shall be made of steel of a standard commercial type coated inside and outside with the same polyvinyl chloride coating as the chain link mesh or shall have all

surfaces galvanized with the outside galvanized surface coated with the same polyvinyl chloride coating as the chain link mesh.

(d) Aluminum alloy shall meet the requirements of ASTM B211.

3. Fittings: These shall be malleable iron, pressed steel, or aluminum alloy. The fittings shall be either hot-dip galvanized, polyvinyl chloride-coated, or aluminum alloy.

(a) Hot-dip galvanizing shall meet the requirements of ASTM A153.

(b) Polyvinyl chloride-coated material shall have the same polyvinyl chloride coating as the chain link mesh.

(c) Aluminum alloy shall meet the requirements of ASTM B211.

4. Tension Wire, Tie Wire and Hog Rings:

(a) Tension wire for steel fence shall be coil spring steel and shall be 7 gauge. The base material shall have a minimum tensile strength of 80,000 psi with an aluminum coating applied at a rate of 0.40 ounces/square foot of surface area.

Aluminum tension wire shall be 7 gauge ATSM B211 Alloy 6061-T6 or 5052-H38 Aluminum Alloy.

(**b)** Tie wire for fastening fabric to line posts shall be 6 gauge. Tie wires or hog rings used to fasten the fabric to the top rail or tension wire shall be 9 gauge aluminum for aluminized or aluminum fabric fence, and 9 gauge polyvinyl chloride coated steel wire for polyvinyl chloride fence.

(c) Aluminized steel hog rings for aluminized fabric shall be 9 gauge.

5. Gates: Gates shall be of the same type of materials used for the chain link fence.

M.10.06—Vacant

M.10.07—Vacant

M.10.08—Three-Cable Guide Railing (I-Beam Posts) and Anchorages:

1. Wire Rope: Wire rope shall be Class A coating, 3/4 inch diameter, Type 1 construction conforming to AASHTO M 30.

2. Fittings:

(a) Material indicated on the plan as "Cast Steel" shall meet the requirements of ASTM A27 Grade 70-40 Class 1.

(b) Material indicated on the plan as "Malleable Iron" shall meet the requirements of ASTM A47, Grade 32510.

(c) The cable wedge for splices and cable fittings shall be malleable iron casting conforming to ASTM A47 Grade 32510. The cable wedge shall be uncoated (black).

(d) The cable splice shall meet the requirements of ASTM A536 Ductile Iron Grade 65-45-12. All cable ends and splices shall meet the details on the plans and have the properties necessary to develop the full tensile strength (25,000 lb.) of 3/4 inch wire rope.

(e) The spring cable end assembly (compensating device) casting shall meet the requirements of ASTM A47 Grade 32510 and must have a spring rate of 450-500 lb./inch and a total available throw of 6 inches minimum.

(f) Hook bolts, as installed, shall develop an ultimate pull-open strength from 500 lb. to 1,000 lb. applied in a direction normal to the longitudinal axis of the post.

(g) Steel turnbuckle cable end assembly shall be pearlitic malleable iron casting conforming to ASTM A220 Grade 50005.

(h) Standard hex nuts and lock nuts shall meet the requirements of ASTM A563 Grade B.

(i) After fabrication, all metal fittings and forged cast fittings required by the plans to be galvanized shall be galvanized to meet the requirements of ASTM A153.

3. Steel Posts and Welded-Soil Plates: All steel posts and welded-soil plates shall meet the requirements of ASTM A36. All required holes shall be punched or drilled. After fabrication, all posts and welded soil-plates shall be galvanized to meet the requirements of ASTM A123. All welding shall meet ANSI/AASHTO/AWS D1.5.

4. Anchorages: Class PCC03340 Concrete shall meet the requirements of M.03. Bar reinforcement shall meet the requirements of M.06.01-1.

Breakaway anchor angles, washers and anchorage plates shall meet the requirements of ASTM A36. Anchor bolts and rods for end anchorages shall meet the requirements of AASHTO M 314. Hex nuts shall meet the requirements of ASTM A563 Grade B. Breakaway anchor-angle tiepin shall be 3/16 inch diameter meeting the requirements of ANSI CR1018 and galvanized after fabrication to meet the requirements of ASTM A123. The angles shall be galvanized after fabrication to meet the requirements of

ASTM A123. Rods, top nuts and washers shall be galvanized in accordance with the requirements of ASTM F2329.

SECTION M.11
MASONRY FACING
CEMENT AND DRY RUBBLE MASONRY
BRICK
MORTAR

M.11.01—Masonry Facing
M.11.02—Cement Rubble Masonry and Dry Rubble Masonry
M.11.03—Brick Masonry
M.11.04—Mortar

M.11.01—Masonry Facing

1. Masonry Facing Stone: This stone shall be of the kind specified in the proposal or on the plans and shall be of a size, quality and color acceptable to the Engineer. The stone shall be hard and durable, resistant to weathering action, reasonably fine grained, and free from structural defects that would impair its strength or durability. Stone shall be of such character that it may be truly cut to such lines and surfaces, either plain or curved, as may be required. Any stone having defects which have been repaired with cement or other materials will be rejected. Samples of stone shall be submitted when required. Masonry facing stone shall be of the following grades: dimensioned masonry stone and ashlar masonry stone.

(a) Dimensioned Masonry Stone: Dimensioned masonry stone shall be dressed to true size and shape, as shown on the plans or as ordered. General details and controlling dimensions will be shown on the plans. The Contractor shall prepare such additional detail drawings as may be required for guidance, and all such drawings shall be approved by the Engineer before construction is started.

(b) Ashlar Masonry Stone: Ashlar masonry stone shall be of such sizes and shapes as to produce the general effect shown on the plans.

Surface Finish: For the purpose of these specifications, the finishes of exposed surfaces of masonry facing stone are defined as follows:

- **Sawed Face:** Exposed surfaces shall be true planes with a tolerance of 3/16 inch from a straightedge placed on the surface in any direction. All saw-faced stone shall be sandblasted to remove rust stains. Where impractical to saw, the surface shall be six-cut. Face arrises of all exposed surfaces shall be true and out of wind.
- **Six-Cut:** Exposed surfaces shall be true planes with a tolerance of 3/16 inch from a straightedge placed on the exposed surface in any direction. The exposed surface shall be finished with a tool having 6 blades to the inch.
- **Four-Cut:** Same surface tolerance as for 6-cut; exposed surfaces to be finished with a tool having 4 blades to the inch.
- **Fine-Pointed:** Projections on fine-pointed finished surfaces shall not exceed 1/2 inch. Exposed edges shall be pitched to true lines.
- **Rough-Pointed:** Projections on rough-pointed finished surface may vary from 1/2 to 1 inch. Exposed edges shall be pitched to true lines.
- **Split-Face:** Exposed surfaces shall have face edges pitched to line and shall have no projection of more than 1 1/2 inches above the plane of the edges.
- **Rock-Face or Quarry Face:** Exposed surfaces shall be freshly split granite; they shall have no projection of more than 3 inches. Hollow faces will not be permitted. They shall be pitched to straight and true lines and shall have a chiseled draft on all edges if so indicated on the plans.

Exposed surfaces of face stone shall be given the surface finish indicated on the plans.

M.11.02—Cement Rubble Masonry and Dry Rubble Masonry:

1. Masonry Stone: This stone shall be of approved quality, sound, durable and free from structural defects or imperfections tending to destroy its resistance to the weather. The individual pieces shall be roughly rectangular in shape, with at least 1 fairly even face, and shall have a volume of not less than 4 c.f., except where smaller pieces are required for closure or where the character of the construction makes the use of smaller pieces necessary.

M.11.03—Brick Masonry:

Brick: The brick for use other than the construction of catch basins, manholes and drop inlets shall conform to the requirements of AASHTO M 114, Grading SW.

The brick shall have a fine-grained, uniform, and dense structure, free from lumps of lime, laminations,

cracks, checks, soluble salts, or other defects which may in any way impair their strength, durability, appearance, or usefulness for the purpose intended. Bricks shall emit a clear, metallic ring when struck with a hammer.

M.11.04—Mortar: Mortar shall be either Pre-blended or Pre-packaged material meeting the following requirements:

- ASTM C1714 - Standard Specification for Pre-blended Dry Mortar Mix for Unit Masonry;
- ASTM C387 - Standard Specification for Packaged, Dry Combined Materials for Concrete and High Strength Mortar; or,
- be composed of 1 part Portland cement and 2 parts, by volume, of surface dry fine aggregate blended on Site. Hydrated lime, in an amount not to exceed 4 lb. of lime to each bag of cement, may be added when the material is blended on Site at the option of the Engineer. Cement and hydrated lime shall meet the following requirements:

(a) Portland cement, Types I, II or IS, and water shall meet the requirements of M.03.

(b) Hydrated lime shall meet the requirements of ASTM C6.

When mortar is mixed on the Project site, fine aggregate shall conform to Grading A or B as indicated in the table below, and to the requirements of M.03. For laying stone, precast units, or for shotcrete, fine aggregate shall conform to Grading A. For pointing stone or the precast units and for laying brick or sealing pipe joints, the fine aggregate shall conform to Grading B.

Table of Gradation, Fine Aggregate for Mortar

Square Mesh Sieves	**Grading**	
	A	B
	Percentage Passing by weight	
Pass 3/8 in.	100	
Pass No. 4	95-100	
Pass No. 8	80-100	100
Pass No. 16	50-85	
Pass No. 30	25-60	
Pass No. 50	10-30	10-40
Pass No. 100	2-10	0-10

SECTION M.12
BEARING AREAS
RIPRAP
SLOPE PAVING & SLOPE PROTECTION
WATERPROOFING AND DAMPPROOFING
STONE AND GRANITE SLOPE CURBING
CALCIUM CHLORIDE FOR DUST CONTROL
WOOD

M.12.01—Bearing Areas
M.12.02—Riprap
M.12.03—Slope Paving
M.12.04—Waterproofing
M.12.05—Dampproofing
M.12.06—Stone Curbing
M.12.07—Granite Slope Curbing
M.12.08—Granite Stone Curbing for Bridges
M.12.09—Vacant
M.12.10—Calcium Chloride for Dust Control
M.12.11—Vacant
M.12.12—Concrete Block for Slope Protection
M.12.13—Wood

M.12.01—Bearing Areas:

Prefabricated Pads: Prefabricated pads shall consist of cotton duck impregnated with rubber and shall be a single sheet of 1/8 inch minimum thickness with a tolerance of plus 15% or minus 5%, composed of 8 ounce duck and high quality natural rubber constructed in 5 or more plies. The breakdown stress for compression perpendicular to the plane of lamination shall be not less than 11,000 psi.

M.12.02—Riprap: Materials for this item shall consist of sound, tough, durable and angular rock, free from decomposed stones or other defects impairing its durability. The size of a stone as hereinafter specified shall be its least dimension. Broken concrete or rounded stones are not acceptable. The type of material to be used shall be as noted on the plans, in the special provisions or as may be ordered by the Engineer.

1. **Standard Riprap:** This material shall conform to the following requirements:
   (a) Not more than 15% of the riprap shall be scattered spalls and stones less than 6 inches on any side.
   (b) No stone shall be larger than 30 inches on any side, and at least 75% of the weight shall be stones at least 15 inches.
2. **Intermediate Riprap:** This material shall conform to the following gradation:

Stone Size	% of the weight
18 inches	0
10 to 18 inches	30-50
6 to 10 inches	30-50
4 to 6 inches.	20-30
2 to 4 inches	10-20
less than 2 inches	0-10

3. **Modified Riprap:** This material shall conform to the following gradation:

Stone Size	% of the weight
10 inches	0
6 to 10 inches	20-50
4 to 6 inches	30-60
2 to 4 inches	30-40
1 to 2 inches	10-20
less than 1 inch	0-10

4. **Special Riprap:** The crushed stone shall meet the gradation requirements of Table M.01.02-2 for No. 3 coarse aggregate.

M.12.03—Slope Paving: The stone for this work shall consist of sound, tough, durable rock, free from decomposed stone or other defects impairing its durability. Each piece shall have an area on its exposed surface of not less than 2 s.f. and a thickness not less than 9 inches, except that stone for the 2 bottom rows shall be of such size that they can be embedded at least 2 feet into the ground; and they shall have a thickness of not less than 12 inches.

Concrete slabs shall conform to the dimensions given above for stone, except that the maximum surface dimensions shall not exceed 10 l.f. in any direction.

The concrete materials shall meet the requirements of M.03.01 and M.03.02 for Slope Paving Concrete.

M.12.04—Waterproofing: The materials for this work shall meet the following requirements:

1. Waterproofing Asphalt: For woven glass fabric, the seal coat material shall be an asphalt conforming to ASTM D449, Type III.

Primer for use with asphalt in waterproofing shall meet the requirements of ASTM D41.

2. Fabric: Woven glass fabric saturated with asphalt shall conform to the requirements of ASTM D1668.

Resin-treated woven glass fabric shall conform to the requirements of ASTM D1668 and shall be compatible for use with asphalt.

3. Mortar: Mortar shall conform to the requirements of M.11.04.

4. Reinforcement: Reinforcement shall meet the requirements of M.06.01.

5. Metal Flashing: Metal flashing shall be of the type and dimensions called for on the plans, and the quality shall be acceptable to the Engineer.

6. Joint Filler: Filler for use in horizontal joints shall be a straight refined petroleum asphalt conforming to the following requirements:

	Min.	Max.
Penetration at 77° F, 100 grams, 5 seconds	50	60
Flashpoint, open cup method, in °F	450	
Softening point, in °F	120	130
Loss on heating, at 325° F, 50 grams, 5 hours, %		0.5
Ductility, at 77° F, 5 cm per minute	85	
Total bitumen (Solution in carbon disulfide) %	99.5	

Filler for use in vertical joints shall be an asphalt conforming to above specified requirements, to which has been added 20% by weight of fiber. The incorporation of the fiber with the asphalt shall be done at the factory of the manufacturer to ensure a uniform distribution of the fiber throughout the mix.

M.12.05—Dampproofing: The materials for this work shall meet the following requirements:

Asphalt for Primer: Asphalt for primer shall conform to ASTM D41.

Asphalt for Seal Coat: The asphalt for seal coat shall meet one of the following:

1. Hot-applied asphalt seal coat—ASTM D449, Type 1
2. Cold-applied asphalt seal coat—ASTM D4479, Type 1 (Asbestos Free)
3. Cold-applied emulsified asphalt seal coat—ASTM D1227, Type III or IV

M.12.06—Stone Curbing: The materials for this work shall meet the following requirements:

1. Granite Curbing: Stone for this work shall be hard and durable granite, fundamentally of light color, of general uniform texture, of smooth splitting appearance, free from seams or imperfections that would impair its structural reliability and containing only such color variations as in the opinion of the Engineer would reasonably be characteristic of the material source. The Contractor shall submit for approval, the name of the quarry and the type of curb which the Contractor proposes to use. No stone from any other quarry shall be used unless it has been properly approved.

The finish and surface dimensions for the curb shall conform to the following requirements:

The curbstone shall have a top surface free from wind; it shall be pointed, peen-hammered or sawed to an approximately true plane, and shall have no projections or depressions greater than 1/8 inch. The front and back arris lines shall be pitched straight and true.

On the back surface of the curbstone there shall be no projection for 3 inches down from the top which

would fall outside of a plane having a batter of 4 inches per 12 inches from the back arris line.

The front face shall be at right angles to the plane of the top and shall be smooth quarry-split, free from drill holes in the exposed face. There shall be no projections greater than 3/4 inch, or depressions greater than 1/2 inch, measured from the vertical plane of the face through the top arris line for a distance of 8 inches down from the top. For the remaining distance, there shall be no projections or depressions greater than 1 inch measured in the same manner. The arris lines at the ends shall be pitched with no variation from the plane of the face greater than 1/8 inch.

The ends of all stones shall be square with the planes of the top and face and so finished that, when the stones are placed end to end as closely as possible, no space more than 1/2 inch shall show in the joint for the full width of the top or down on the face for 8 inches. On curbstones having a length of 6 feet or more, the remainder of the end may break back not over 9 inches; whereas, on shorter curbstones, they shall not break back more than 6 inches.

If sawed, the curbstones shall be thoroughly cleaned of any iron rust or iron particles.

Curbstones to be set on a radius of 100 feet or less shall be cut to the curve required, and their ends shall be cut on radial lines. Requirements for length of individual stones in curved curbing vary with radii of curves.

2. Bluestone Curbing: Stone for this work shall be of a good grade, free from structural defects, and shall be approved by the Engineer.

It shall meet the requirements contained hereinbefore for granite curbing, except that the top surface and the top 8 inches of the front face shall be "fine-pointed" in conformity with the requirements of M.11.01 for masonry facing stone.

M.12.07—Granite Slope Curbing: The materials for this work shall meet the following requirements.

1. Granite Slope Curbing: Stone for this work shall be hard and durable granite, fundamentally of light color, of general uniform texture, of smooth-splitting appearance, free from seams or imperfections that would impair its structural reliability and containing only such color variations as, in the opinion of the Engineer, would reasonably be characteristic of the material source. The exposed face of all curbing shall be smooth, quarry-split to an approximate true plane, and shall have no projections or depressions which will cause over 1 inch to show between a 2 foot straightedge and the face when the straightedge is placed as closely as possible on any part of the face. If projections on the face are more than that specified, they shall be dressed off. The top arris line at the face shall be pitched to a line which shall not show over 1 inch in any direction between the stone and a straightedge the full length of the stone. The bottom arris line at the face shall be pitched so that not over 1 inch shall show between the stone and a straightedge, the full length of the stone, when viewed at right angles to the plane of the face. The ends shall be square to the plane of the face and so finished that when the stones are placed end to end as closely as possible, no space more than 1 1/2 inches shall show in the joint for the full width of the face. The arris lines at the ends shall be pitched with no variation from the plane of the face more than 1/4 inch. Drill holes not more than 3 1/2 inches long, not more than 1/2 inch deep, will be permitted. The sides shall not be under the square more than 4 inches or over the square at the back more than 1 inch.

The straight slope curbing shall be in lengths of not less than 2 feet. The curved slope curbing shall be in lengths of at least 6 inches. The curbing shall have a minimum thickness of 3 inches and a maximum thickness of 6 inches.

When the slope curbing is set adjacent to concrete pavement or gutters, the width of the face of the curbing shall be 12 inches, with a tolerance of plus or minus 1/2 inch. When set adjacent to surfaces other than concrete, the curbing finished shall have a face width of not more than 13 inches and not less than 11 inches.

2. Mortar: The mortar for this work shall conform to M.11.04.

3. Gravel Base: The gravel base under the slope curbing shall be granular fill conforming to M.02.03.

M.12.08—Granite Stone Curbing for Bridges: The materials for this work shall conform to the following requirements:

1. Granite Curbing: Stone for this work shall be hard and durable granite, fundamentally of light color, of general uniform texture, of smooth-splitting appearance, free from seams or imperfections that would impair its structural reliability, and containing only such color variations as in the opinion of the Engineer would be reasonably characteristic of the material source. When so directed by the Engineer, the Contractor shall submit samples of the proposed type of curb.

The finish and surface dimensions for the curb shall conform to the requirements shown on the plans.

The ends of all stone shall be jointed square with the planes of the top and face and finished smooth except that, if so noted on plans, the extreme end face of the curbing shall be finished to a radius as shown on the plans.

The minimum length of a stone shall be 4 feet, except that stones of lengths less than 4 feet will be so noted on the plans.

The maximum length of stone to be used on horizontal and vertical curves shall be such as to produce the effect of a smooth, continuous curve. Curbs to be set on a radius of 160 feet or less shall be cut to the curve required, and their ends shall be cut on radial lines.

Detailed cutting plans or schedule shall be submitted for approval of the Engineer prior to cutting stones.

2. Mortar: The mortar for this work shall conform to M.11.04.

3. Metal Anchors: Metal anchors shall be steel conforming to the requirements of M.06.01-1, bar reinforcement, and shall be thoroughly galvanized by the hot-dip process after fabrication.

M.12.09—Vacant

M.12.10—Calcium Chloride for Dust Control: Calcium chloride shall conform to AASHTO M 144, except that the pellet form and the flake form shall be equally acceptable.

M.12.11—Vacant

M.12.12—Concrete Block for Slope Protection: Concrete blocks shall be solid, precast, rectangular blocks 16 inches long, 8 inches wide, and 4 inches thick. No dimensions shall differ from the theoretical block size specified by more than 1/2 inch. The blocks' compressive strength shall be 3,000 psi at 28 days. The concrete mix shall use 3/8 inch (No. 8) or larger coarse aggregate.

M.12.13—Wood: All wood materials for this work shall be manufactured in accordance with AASHTO M 168.

The Contractor shall submit a Materials Certificate in accordance with 1.06.07.

Treatment for wood shall be with a preservative suitable for the conditions of exposure in accordance with AASHTO M 133 and the AWPA Standards U1 and T1. Preservatives shall not be interchanged. End results of treatment, post treatment handling and quality control shall be in accordance with AWPA Standards U1 and T1 for commodities UC4B, UC4C and UC5A, for the type and use specified.

1. Wood in contact with or immersed in water, such as piers, docks, ferry slips, boardwalks, wharves, bridges, etc. shall be one of the following: bongossi, ekki, or azobe, bonalim or greenheart.

Dolphin piles, bulkheads or lead-in jetties shall be one of the following: basralocus greenheart or bongossi, ekki or azobe in order of preference.

2. Wood in contact with the ground such as piles, noise-walls, bulkheads, etc. shall be one of the following: bongossi, ekki or azobe, bonalim, greenheart, pressure treated southern yellow pine, douglas fir or western larch.

3. Wood in above ground use such as decking, railings, bridges, noise walls and platforms shall be one of the following: bongossi, ekki or azobe, bonalim or greenheart.

Pressure treated wood, where specified shall be No. 1 KD or better southern yellow pine, douglas fir or western larch. Pressure treated wood shall be stained or painted as specified in the plans or special provisions.

SECTION M.13
ROADSIDE DEVELOPMENT

M.13.01—Topsoil and Planting Soil
M.13.02—Agricultural Ground Dolomitic Limestone
M.13.03—Fertilizer
M.13.04—Seed Mixtures
M.13.05—Mulch Materials
M.13.06—Compost
M.13.07—Plant Materials
M.13.08—Sod
M.13.09—Erosion Control Matting

M.13.01—Topsoil and Planting Soil:

1. Topsoil: The term topsoil used herein shall mean a soil meeting the soil textural classes established by the USDA Classification System based upon the proportion of sand, silt, and clay size particles after passing a No. 10 sieve and subjected to a particle size analysis. The topsoil shall contain 5% to 20% organic matter as determined by loss on ignition of oven-dried samples dried at 221°F. The pH range of the topsoil shall be 5.5 to 7.0.

The following textural classes shall be acceptable:

1. Loamy sand, including coarse, loamy fine, and loamy very fine sand, with not more than 80% sand
2. Sandy loam, including coarse, fine and very fine sandy loam
3. Loam
4. Clay loam, with not more than 30% clay
5. Silt loam, with not more than 60% silt
6. Sandy clay loam, with not more than 30% clay

All textural classes of topsoil with greater than 80% sand content will be rejected.

The topsoil furnished by the Contractor shall be a natural, workable soil that is screened and free of subsoil, refuse, stumps, roots, brush, weeds, rocks and stones over 1 1/4 inches diameter, and any other foreign matter that would be detrimental to the proper development of plant growth.

The Contractor shall notify the Engineer of the location of the topsoil at least 15 calendar days prior to delivery. The topsoil and its source shall be inspected and approved by the Engineer before the material is delivered to the Project. Material delivered to the Project which does not meet specifications or which has become mixed with undue amounts of subsoil during any operation at the source or during placing and spreading, will be rejected and shall be replaced by the Contractor with acceptable material.

When topsoil is not furnished by the Contractor, it shall be material taken from the Site in accordance with 2.02 or will be furnished by the State.

2. Planting Soil: Soil Material to be used for plant backfill shall be one of the following textural classes:

1. Loamy sand, with not more than 80% sand
2. Sandy loam
3. Loam
4. Clay loam, with not more than 30% clay
5. Silt loam, with not more than 60% silt
6. Sandy clay loam, with not more than 30% clay

Planting soil shall be premixed, consisting of approximately 15% compost, 10% peat, with topsoil and/or native soil. Planting soil shall be loose, friable, and free from refuse, stumps, roots, brush, weeds, rocks and stones 2 inches diameter. In addition, the material shall be free from any material that will prevent proper development and plant growth.

(a) For ericaceous plants and broad-leaved evergreens requiring an acid soil, planting soil shall have a true pH of 4.5 to 5.5. If it has not, it shall be amended by the Contractor at its expense to the proper pH range by mixing with sulphur.

(b) Planting soil for general planting of nonacid-loving plants shall have a true pH value of 5.6 to 6.5. If it has not, it shall be amended by the Contractor at its expense to the proper pH range by mixing with dolomitic limestone.

The Engineer reserves the right to draw such samples and to perform such tests as deemed necessary to ensure that these specifications are met.

The amount of sulphur or limestone required to adjust the planting soil to the proper pH range appropriate for its use (above) shall be determined by the Contractor based on the physical testing of a representative sample of the material. Testing must be documented in accordance with the Department's "Minimum Schedule for Acceptance Testing," found in Chapter 8 in the Department's Materials Testing Manual. Limestone shall meet the requirements of M.13.02. Sulphur shall be intended for agricultural use and packaged in containers with the manufacturer's name, chemical analysis and net weight clearly shown on the container. The Contractor shall follow the manufacturer's recommended procedures for application of the sulphur to the soil.

M.13.02—Agricultural Ground Dolomitic Limestone: Agricultural ground dolomitic limestone shall conform to the standards of the Association of Official Agricultural Chemists (AOAC), and must comply with all existing State and Federal regulations.

The material must comply with the following gradation:

Square Mesh Sieves	Percent Passing By Weight
Pass No. 10	100
Pass No. 20	95
Pass No. 100	50
The minimum calcium carbonate equivalent shall be	90

The Engineer reserves the right to draw such samples and perform such tests as he deems necessary to assure that these specifications are met.

M.13.03—Fertilizer: Fertilizer shall be slow release and commercial grade granular 10-10-10 fertilizer. At least 40% of the nitrogen content shall be slow release, phosphorus shall be available phosphoric acid, and potassium shall be water soluble potash. The fertilizer shall be delivered to the Project in new, clean, sealed containers which bear a label fully describing the contents, the chemical analysis of each nutrient, the fertilizer grade, the net bulk, the brand, and the name and address of the manufacturer. The fertilizer and labels shall conform to all existing State and Federal regulations, and shall meet the standards of the AOAC.

The delivery of each shipment of fertilizer to the Project shall be accompanied by a properly executed and acceptable affidavit of the form shown herein. The affidavit shall be submitted to the Engineer. The Engineer reserves the right to draw such samples and perform such tests as may be deemed necessary to ensure compliance with these specifications.

Form for Affidavit - Fertilizers (Official Stationery of Supplier)

Date _____

To Whom It May Concern:

I hereby certify that I have sold and delivered _____ tons
of commercial fertilizer of _____ grade. This material is designated as our
batch number(s) _____
and was delivered to _____
for _____
(Contractor's Name)

Connecticut Department of Transportation Project Number(s):_____
at _____, Connecticut. The material was delivered
on _____. The labels and contents meet all State and Federal
regulations. The mixture consists of:

(List analyses of each major plant nutrient as percent by weight)

Signature _____
(Company Official)

Signature and Seal _____
Notary Public

Should the material fail to meet these specifications, the Contractor shall supply additional acceptable material and perform such work necessary to rectify the deficiencies without cost to the State.

M.13.04—Seed Mixtures:

(a) The grass seed mixture shall conform to the following:

Species	**Proportion By Weight Pounds**	**Minimum Purity (Percent)**	**Minimum Germination (Percent)**
VELVET BENTGRASS, (AGROSTIS CANINA) CERTIFIED VARIETY: OR EQUAL CERTIFIED VARIETY;	25	96	85
RED FESCUE (FESTUCA RUBRA L. SSP. RUBRA) CERTIFIED VARIETY: OR EQUAL CERTIFIED VARIETY	35	97	80
PARTRIDGE PEA (CHAMAECRISTA FASCICULATA) CERTIFIED VARIETY:	10	95	90
INDIAN GRASS (SORGHASTRUM NUTANS) CERTIFIED VARIETY:	15	95	90
CANADA WILDRYE (ELYMUS CANADENSIS) CERTIFIED VARIETY:	5	95	90
KENTUCKY BLUE GRASS (POA PRATENSIS) CERTIFIED VARIETY:	10	95	90

Under no circumstances shall annual Ryegrass, Italian Rye, or any other seed be added to the seed mixture.

(b) The "temporary" grass seed shall be perennial ryegrass (Lolium perenne) or an improved variety thereof, such as Manhattan, having a minimum purity of 98% and a minimum germination of 90%.

The seed mixture shall be delivered in new, clean, sealed containers. Labels and contents shall conform to all State and Federal regulations. Seed shall be subject to the testing provisions of the Association of Official Seed Analysts.

The seed shall be delivered to the Project accompanied by a properly executed affidavit for each type and shipment of seed. The affidavit shall be of the form shown herein.

The Engineer reserves the right to take such samples and to make such tests as they deem necessary to ensure compliance with these specifications. The Contractor shall supply such additional acceptable material and perform such work as required to rectify any deficiencies without cost to the State.

Form for Affidavit - Seed (Official Stationery of Supplier)

Date _____

To Whom It May Concern:

I hereby certify that _____ pounds of seed mixture, lot

of commercial fertilizer of _____ grade. This material is designated as our

number _____, (Label attached) has been sold and

delivered to _____

for _____
(Contractor's Name)

Connecticut Department of Transportation Project Number(s):

_____

at _____, Connecticut. The material was delivered

on _____. The labels and contents meet all State and Federal regulations. The mixture consists of:

(List component parts, proportions, minimum purity, minimum germination)

Signature _____
(Company Official)

Signature and Seal _____
Notary Public

M.13.05—Mulch Materials:

1. **Wood Chips:** Wood chip mulch shall be sound, green wood, and shall be 1/8 inch nominal thickness with not less than 50% of the chips having an area of not less than 1 square inch, nor more than 6 square inches. The material shall be free from rot, leaves, twigs, shavings, debris, and any material injurious to plant growth.
2. **Hay and Straw:**
   a. Hay shall be from properly cured grass or legume mowings, free from weeds, reeds, twigs, debris or other objectionable material. It shall be free from rot or mold, and shall have a moisture content of not more than 15% when delivered to the Project. No salt hay shall be used.
   b. Straw shall be derived from threshed stalks of oat, wheat, rye or barley and shall be free of rot, seeds, noxious weeds and other foreign material.
3. **Wood Fiber Mulch:** Wood fiber mulch or wood cellulose fiber mulch shall be material manufactured for mulching seeded areas. The material shall be produced from clean wood, uniform in texture and free of shavings, rot and mold. Wood fiber mulch shall be commercially pre-packaged bearing the brand, name and address of the manufacturer.
4. **Shredded Bark Mulch:** This shall consist of the outer bark of pine or hardwood trees. The material shall be aged for a minimum of 6 months and be dark brown in color, free of chunks and pieces of wood thicker than 1/4 inch, and shall not contain, in the judgement of the Engineer, an excess of fine particles. Mulch must be free of long stringy material and dyed wood chips.

M.13.06—Compost: Compost shall be a stable, humus-like organic material produced by the aerobic, biological and biochemical decomposition of source-separated organic waste, that may include leaves and yard trimmings, food scraps, food processing residuals, manure and/or other agricultural residuals, forest residues and bark. Compost may be either commercially packaged or from a bulk source. Compost shall not be altered by the addition of materials such as sand, soil and glass. Compost shall not contain substances toxic to plants and shall not contain less than 0.1% by dry weight of man-made foreign matter. Compost shall pose no objectionable odor and shall not closely resemble the raw material from which it

was derived. Compost shall be suitable for use as a soil amendment or mulch and shall support the growth of nursery stock or seeding. All compost material must be accompanied by a Materials Certificate and Certified Test Report in accordance with Section 1.06.07.

Compost shall have the following properties:

1. A minimum organic content of 50% dry weight basis as determined by loss on ignition in accordance with ASTM D2974.
2. Carbon:Nitrogen ratio range of 11:1 to 25:1.
3. Carbon:Phosphorus ratio of 120:1 to 240:1.
4. A moisture content of 35 to 60% in accordance with ASTM D2974.
5. Particle size less than 1/2 inch for Planting Backfill, and 1 inch for Erosion Control in accordance with AASHTO T27.
6. The pH of compost shall be in the range of 6 to 7.8.
7. The soluble salt content of compost shall not exceed 4.0 mmhos/cm (dS/m) as determined by using a dilution of 1 part compost to 1 part distilled water.
8. The maturity or stability of the compost shall be Stable or Very Stable, meeting either of the following criteria:
    (a) > 6 using the Solvita Compost Maturity Test, or
    (b) < 10°C above ambient temperature (Dewar self-heating test)
9. Maximum foreign matter 1%.

M.13.07—Plant Materials: The materials for this work shall conform to the following requirements:

1. General: For the most part, the latest revised version of "Standardized Plant Names," prepared by the Editorial Committee of the American Joint Committee on Horticultural Nomenclature, shall be the authority for all botanical plant names.

All plants shall be first-class representatives of their normal species or varieties in accordance with the ANSI American Standards for Nursery Stock and as specified on the plans. They shall have well-furnished branch systems together with vigorous fibrous root systems.

Plants shall be free from all insect pests, plant diseases, disfiguring knots, stubs, sun-scalds, abrasions of the bark or any other form of injury or objectionable disfigurements. All plant material shall comply with the State and Federal laws with respect to inspection for plant diseases and insect infestations.

Plants shall not be pruned before delivery and no plants shall be cut back from larger sizes to meet the sizes specified.

Plants shall be nursery grown unless otherwise specified and bear evidence of proper nursery care, including adequate transplanting and root pruning.

No plant will be considered to be nursery grown unless it has been growing in a nursery for at least 2 years and unless it has been root pruned or transplanted no more than 5 years prior to digging.

2. Balled & Burlapped (B & B) Material: Nursery-grown trees shall meet the requirements as specified in the current edition of "U.S. American Standards for Nursery Stock," or as further specified in the plans. Nursery-grown trees shall have no cuts which are not healing, no cuts over 3/4 inch diameter which have not completely calloused over and no abrasions of the bark. They must have good fibrous root systems characteristic of the kind.

Trees shall have straight trunks, well-balanced tops and a single leader or as may be characteristic of the species.

Trees in which the leader or branches have been cut back or otherwise topped or de-horned will not be accepted. The caliper of shade trees up to and including 4 inches diameter shall be measured above the root collar (or swelling at the ground) 6 inches above ground level. Caliper shall be the determining measurement in grading. Height measurements shall be given in single feet in sizes up to and including 6 feet.

Small deciduous trees shall be completely natural. Tree "clumps" shall have 3 or more main stems starting from the ground. Bush from trees shall be those with branches which start from the main trunk close to the ground.

3. Container Grown (CG): Container grown shrubs shall possess the minimum number of stems and root mass for the height or container size specified.

Vines and groundcover plants shall be well-furnished with vigorous root systems. They shall be field-grown unless otherwise specified. Plants grown in pots or bands shall have sufficient roots to retain the soil in which they are growing when such plants are removed from their containers. Such plants shall not be

root-bound.

4. Inspections: All plants shall be subject to inspection by the Engineer. The Contractor shall designate its wholesale plant material source(s) of supply to the Engineer in writing at least 1 month in advance of each planting season to facilitate an orderly and timely inspection of the items to be installed. Based on the Project schedule, material procured in the spring for fall installation must be approved before digging occurs. The Contractor shall be represented during such inspection. Inspection may be made at the nursery, on Site or via photos at the discretion of the Engineer.

All tagged samples shall be delivered to the Project for which they were sampled. All deliveries to the planting site shall be accompanied by both the vendor's invoice (designating kind, size, quantity and sources of supply) and Certificates of Inspection issued by Federal or State authorities or both. Such certificates shall attest to the freedom of the plant material from diseases and insect infestations. The State reserves the right to inspect all plant materials at the growing sites. Further inspections will be made when the materials are delivered to the Project site or storage area.

5. Substitutions: No change in size, kind or quality of plants from those specified will be permitted without written approval of the Engineer. The Contractor shall submit a written request for permission to make a substitution along with documentation from 3 nursery vendor source suppliers proving that the proposed plants are unavailable. Upon receipt of such request, the Engineer will suggest plants meeting the requirements of the Contract as to function, size and type and indicate the reduced cost to the State as the result of said substitution. In no case shall the price for substitutions exceed the bid price of those replaced.

6. Digging Plants: Plants shall be dug immediately before shipment unless otherwise approved. Special precaution shall be taken to avoid any unnecessary injury to or removal of fibrous roots. Damaged roots shall be cut off clean.

(a) After deciduous bare-root plants are dug, their roots shall be protected from exposure to sun, wind and freezing temperatures. All bare roots of trees, shrubs and vines, unless otherwise directed, shall be puddled in a wet clay mixture which will cover and adhere to the entire root system. Bare roots shall be further protected by wrapping them in wet straw, moss, burlap or other suitable material, or by heeling them in and watering them in order to keep them fresh and viable.

(b) B & B plants shall be lifted so as to retain as many fibrous roots as possible. Excess soil and feeder roots shall be removed prior to digging. All B & B plants must come from soil which will hold a firm ball. The State reserves the right to reject plants grown in excessively sandy or clayey soil if the plant is to be installed in a dissimilar soil type. The plants shall be wrapped with burlap, or similar approved material, and tightly laced with bio-degradable twine in such a manner as to hold the balls firm and intact. All B & B material arriving with broken or loose balls, or with manufactured balls, will be rejected.

7. Transportation and Labeling: Plants transported by open vehicles shall be covered by tarpaulins or other suitable covers securely tied to the body of the vehicle. Closed vehicles shall be adequately ventilated to prevent overheating of the plants. The heads of trees shall be tied in carefully to prevent breakage of the leaders and the branches. Trunks and branches shall be adequately supported on padding to prevent their being scraped or bruised.

Legible labels shall be attached to all separate plants, boxes, bundles, bales or other plant containers, indicating the name, size, and quantity of units in each container and other information necessary for inspection.

8. Delivery: Notice of delivery of plants shall be given to the Engineer by the Contractor at least 48 hours in advance of the anticipated delivery date, unless otherwise authorized. The Engineer shall be furnished a legible copy of the invoice for each shipment showing kind, sizes and quantities of materials.

All plant materials which are delivered in such a stage as to reasonably endanger their survival will not be accepted.

All plant materials shall be produced in a latitude north of Washington, D.C. and in a longitude east of the Mississippi River.

(a) Spring Dug: All deciduous plants shall be received with buds unopened and intact; evergreen plants with the new growth retarded.

(b). Fall Dug: Deciduous plants shall not be dug before the plants have hardened off.

9. Water: Water shall be free from oil, acid, alkalis, salts and any other substances harmful to plants. Water from streams shall not be used unless authorized by the Engineer.

10. Peat: Peat shall be commercially packaged peat from sedge, sphagnum or reed sources. Material shall be in such physical condition that it may be rubbed through a 1/2 inch mesh screen, and may be

readily mixed with soil material. It shall be free from sticks, roots, stones and other objectionable material. It shall be delivered to the Project in clean, new, sealed containers bearing the brand, net bulk, and name and address of the packer. The material shall have an acidity that falls in the pH range of 3.0 to 7.0. It shall have a minimum organic content of 90% and a minimum water-absorbing capacity of 1000%.

14. Miscellaneous:

(a) Anchor stakes for guying trees shall be of sound hardwood with a minimum length of 2 feet and minimum diameter of 2 inches at the smaller end. Stakes made from lumber shall measure no less than 2 inches × 2 inches throughout their lengths. Trees over 3 1/2 inch caliper shall require either stakes or dead-men for support as approved by the Engineer. The type of stake used shall be uniform throughout the Site.

(b) Tree support posts shall be sawed posts cut to a uniform square cross-section of 2 inches × 2 inches throughout their lengths. They shall be cut from sound, hard, clean, straight wood free from crooks, 8 feet long for major trees and 4 - 5 feet long for minor trees or as approved by the Engineer.

(c) Hose for protecting the bark of major and minor trees from guy wires shall be of good quality rubber or plastic hose acceptable to the Engineer, with a minimum inside diameter of 3/8 inch and a maximum inside diameter of 3/4 inch.

(d) Wire shall be pliable, new, annealed, galvanized, 12-gage, for staking support and 10-gage for guying to trees. Alternate staking and guying systems shall be submitted to the Engineer for approval.

(e) Flags shall be white cotton cloth or white plastic ribbon, 2 inches wide and 18 inches long. Gauze is not acceptable.

(f) Anti-desiccant shall be an emulsion such as will provide a film over plant surfaces, permeable enough to permit transpiration. Anti-desiccant shall be delivered in containers of the manufacturer and shall be mixed according to the manufacturer's instructions.

M.13.08—Sod: Sod shall be living sod procured from areas where the soil is reasonably fertile and from areas similar in the degree of moisture to the area to be planted. It shall be cut or stripped, by approved methods, from turf areas relatively free of large stones, roots or other materials which might be detrimental to the sodding operation or to future maintenance. The sod shall contain a sufficient proportion of pasture grasses to ensure a good mat of roots and a reasonably dense turf unless Type No. 1, which is a superior quality, is specified on the plans.

Any growth more than 3 inches high shall be mowed to a height of 3 inches not more than 5 days before the sod is lifted.

Sources of sod shall be made known to the Engineer at least 5 days before cutting and shall be approved before mowing. The sod shall be cut into squares or rectangular portions which shall be 12 inches wide and may vary in length, but must be of a size which will permit them to be lifted without breaking. The sod shall be sufficiently moist so the soil will adhere firmly to the roots when it is handled and may require watering before lifting. Field grown sod shall be cut to a minimum depth of 1 1/2 to 2 inches. Where Type No. 1 Sod is specified, it shall be cut to a minimum depth of 1 to 1 1/2 inches.

Type No. 1 Sod shall be obtained from inspected and approved commercial sod farm sources of supply and shall be free from noxious weeds, insect infestations, and fungus and bacterial diseases.

M.13.09—Erosion Control Matting: Erosion control matting shall be from the Department's Qualified Products List. Staples shall meet the Manufacturer's requirements. Material which shows signs of degradation shall not be used and shall be removed from the Project.

SECTION M.14
PRESTRESSED CONCRETE MEMBERS

M.14.01—Materials

M.14.01—Materials: Materials for pretensioned members shall meet the following requirements:

1. Concrete: The concrete for the members shall be air-entrained concrete composed of Portland cement, fine and coarse aggregates, admixtures and water. The air-entraining feature may be obtained by the use of either air-entraining Portland cement or an approved air-entraining admixture. The entrained air content shall be not less than 4% or more than 6%.

The Contractor shall design and submit to the Engineer a concrete mix which shall attain a minimum 28 day strength (f'c) as shown on the plans. The compressive strength (f'ci) at the time of transfer of prestressing load shall be as shown on the plans. The Contractor shall further provide a certificate stating that the mix submitted shall meet the requirements. Its ultimate strength at 28 days (f'c) shall have the minimum value shown on the plans.

a) Coarse Aggregate shall consist of broken stone, having a maximum size of 3/4 inch, meeting the requirements of M.03.01.

b) Fine Aggregate, Water, Air-Entraining Admixture and Retarder Admixture shall meet the requirements of M.03.01.

c) Portland Cement for the members shall meet the requirements of M.03.01, except that Type III or Type IIIA Portland cement may be used at no additional cost to the State.

d) Water-Reducing Admixture: The Contractor may submit, for the approval of the Engineer, water-reducing admixture for the purpose of increasing workability and reducing the water requirement for the concrete.

When the Engineer has previously approved the use of a high range water reducer in the concrete mix, the entrained air content shall be not less than 5% nor more than 8%.

e) Calcium Chloride: The addition to the mix of calcium chloride or admixture containing calcium chloride will not be permitted.

2. Prestressing Steel: Prestressing elements shall be uncoated, high tensile strength, seven-wire strand conforming to the requirements of AASHTO M 203.

Before incorporating the elements into the work, a minimum of 1 sample, 7 feet in length and 1 sample, 1 foot in length from each reel shall be furnished to the Engineer for testing. When reel packs are identified with the same heat number, only 1 reel pack need be tested for every 5 reel packs.

3. Reinforcing Steel and Tie Wire: All deformed bars, stirrups, dowels, threaded dowels and tie wire shall meet the requirements of M.06.01-1.

4. Paint for the main prestressing strand ends shall conform to the requirements of M.08.01-5 for coating material.

5. Strapping for the beams shall be 3/4 inch × 0.035 inch, uncoated or galvanized, heavy-duty steel strapping having a minimum ultimate tensile strength of 110,000 psi. Strapping shall be obtained from a manufacturer approved by the Engineer.

Before incorporating the strapping in the work, samples of the strapping at least 3 feet long shall be furnished from each coil for testing. Approval of the material shall be obtained before the material is incorporated in the work.

6. Lifting Hooks, Pipe Sleeves, Base Protective Plates, Threaded Inserts, devices and attachments shall be of the size indicated on the plans or of a design satisfactory for the purpose intended.

7. Transverse Tie Strands shall be galvanized 7-wire strand, of the size noted on the plans, specially manufactured for prestressing, and shall conform to the pertinent requirements of M.14.01-2. The tie strands shall be coated with an approved high quality corrosion-resistant mastic and inserted into a black polyethylene tube having a minimum wall thickness of 0.032 inch. The strands shall be clamped at each end by an aluminum strand vise coated with clean vinyl paint or an approved equal.

8. Nonshrink Grout for Longitudinal Shear Keys between Deck Units and for Recesses at Ends of Transverse Tie Strands and Deck Units. The grout shall meet the requirements of M.03.05. At exposed locations, the grout used for patching shall match the color of the adjacent surface.

9. Void Forms: Internal voids may be formed by the use of heavy paper or fiber forms, specially made for this purpose, or with an alternate acceptable to the Engineer. These forms must be of substantial construction and adequately waterproofed in order to maintain their shape during the entire construction cycle. The end caps shall also be of similar construction. Before incorporation of the forms in the work, a

sample 4 feet in length with end caps shall be furnished the Engineer for testing. Approval of the material shall be obtained before the material is incorporated in the work.

SECTION M.15
HIGHWAY ILLUMINATION

M.15.01—General Requirements
M.15.02—Anchor Bolts
M.15.03—Rock Anchors
M.15.04—Light Standards
M.15.05—Roadway Luminaire
M.15.06—Vacant
M.15.07—Underbridge Luminaire
M.15.08—Vacant
M.15.09—Electrical Conduit
M.15.10—Cast Iron Junction Box
M.15.11—Single Conductor
M.15.12—Cable in Duct
M.15.13—Bare Copper Grounding Conductor
M.15.14—Vacant
M.15.15—Service Entrance Cabinets
M.15.16—Navigation Light

M.15.01—General Requirements: The Contractor may use material and products of any manufacturer provided they meet the design standards and are approved by the department. Materials requiring approval are as follows:

1. Light standards
2. Luminaires
3. Service Items
4. Junction Box
5. Single Conductor
6. Cable in Duct
7. Navigation Lights

M.15.02—Anchor Bolts:

1. Light Standard Base: Anchor bolts shall be high strength steel having a minimum yield point of 50,000 psi. Each anchor bolt shall have a thread 6 inches long. The dimensions shall be as shown on the plans. A hexagon nut, hold down washer, and lock washer, shall be furnished with each bolt. The threads and hexagon nuts shall be hot-dip galvanized as per ASTM F2329.

2. Pedestal and Controller Foundation: The anchor bolts shall be ASTM A36 steel rods having a minimum yield of 36,000 psi. The dimensions shall be as shown on the plans. Each anchor bolt shall have a 90 degree bend at 1 end and shall be threaded at the other end for a sufficient length to properly mount the pedestal or controller. Threads, nuts, flat washers and lock washers shall be galvanized as per ASTM F2329.

3. Span Pole and Mast Arm Foundations: Anchor bolts shall meet the requirements of AASHTO Specification M 314. In addition, only Grade 55 or Grade 105 material may be used. The threaded end of Grade 55 material must be painted yellow. The threaded end of Grade 105 material must be painted red. Steel conforming to ASTM Specifications A675 Grade 90, A449 or A687 is also acceptable and, if used, shall be clearly designated as such on the submitted shop drawings, and properly annotated on all certifications.

The chemical requirements for all categories are a maximum Phosphorous content of 0.04% and a maximum Sulfur content of 0.05%.

Anchor bolts shall have a diameter of at least 1 1/2 inches. The dimensions shall be as shown on the plans or as recommended by the manufacturer. Each anchor bolt shall be furnished with 2 nuts, 2 flat washers, and 1 lock washer. The threads, nuts and washers shall be hot-dip galvanized in accordance with the requirements of ASTM F2329. The nuts shall meet the requirements for nuts within AASHTO M 314.

Field welding and field bending of anchor bolts is prohibited. If anchor bolts do not fit with the base plate, the Contractor shall replace the foundation or use a remedy recommended by the pole manufacturer and approved in writing by the Engineer.

The Contractor shall ensure plumbness of the hooked anchor bolts in the foundation. A minimum anchor bolt embedment of at least 4 feet is required. The amount of threading shall be as shown on the typical

drawings.

Bolt Sampling Requirements:

1. Submit samples marked by manufacturer or fabricator.
2. Submit 1 sample of each size and heat number.
3. Submit a proper Materials Certificate for each sample.
4. Submit a proper Certified Test Report for each sample.

M.15.03—Rock Anchors: Rock anchors shall be malleable iron dipped in corrosion-resistant asphalt paint.

M.15.04—Light Standards: (anchor base and transformer base), (aluminum).

(a) General: Each light standard with appurtenances attached thereto shall be fabricated of aluminum alloy, designed and constructed in accordance with the plans and current requirements of AASHTO "Standard Specification for Structural Support for Highway Signs, Luminaires and Traffic Signals." Light standards with brackets and luminaires shall be designed to withstand a wind speed as recommended in AASHTO "Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals."

(b) Base: Light standard with transformer base shall meet the breakaway requirements of the current AASHTO "Standard Specifications for Structural Support for Highway Signs, Luminaires and Traffic Signals" and shall be identified with visible markings. The transformer base shall be approximately 17 inches high with a door having an approximate opening of 9 inches x 12 inches.

A bonding lug shall be provided in each transformer base and each anchor base shaft shall have the handhole frame or anchor base tapped for bonding. All castings shall be clean and smooth with all details well-defined and true to pattern. It shall be the Contractor's responsibility to verify existing bolt circle diameters by field checking that the bolt circle of the light standard base will match the anchor base on the foundation or structure.

(c) Shaft: Shaft shall be fabricated of aluminum alloy 6063-T6 or 6005-T5 as specified under AASHTO, current edition. Each shaft shall be seamless, tapered, circular in cross section. The shaft shall be 1 piece, however shafts over 40 feet may be fabricated from 2 pieces and factory joined, welded, grounded to appear as 1 piece and have a smooth continuous finish.

Each shaft shall be designed and fabricated in such a manner that each shaft-bracket connection will accommodate single member type brackets 2 to 8 feet long, or truss type brackets 10 feet to 15 feet long. Each shaft designed to 2 foot to 8 foot single member bracket shall have the wall thickness of that required for the 8 foot bracket, and each shaft for the 10 foot to 15 foot truss type bracket shall have the wall thickness of that required for the 15 foot bracket.

Each shaft shall be provided with an aluminum pole cap with self-tapping screws and an internal vibration damper. The internal vibration damper shall be factory installed, location and size as determined by the pole manufacturer.

The shaft shall be finished with a uniform surface having the natural color of aluminum. The shaft shall have a smooth, uniform finish, free from disfiguring scratches, dents and similar markings.

(d) Bracket Arms: The bracket arms shall have an upsweep design and shall accommodate a 2 inch slip fitter type luminaire, fabricated of tubular elliptical aluminum which after fabrication shall have a mechanical strength not less than that of T-6 temper for 6063 alloy and T-5 temper for 6005 alloy. Truss type brackets shall have an aluminum end cap on the bottom member and a 1/8 inch weephole located in the vicinity of the lowest point of the bottom member. The bracket arms shall be a truss-type for lengths 10 feet and longer and shall consist of a single member for shorter lengths. The bracket arm shall be furnished with a finish similar to that of the shaft to insure uniformity of appearance. The maximum upsweep for brackets up to 15 feet long shall not exceed 3 feet 6 inches. Bracket arm lengths shall be as indicated on the plans.

(e) Handhole: Each anchor base light standard shall have a handhole with a reinforced frame and cover approximately 4 inches x 6 inches located approximately 12 inches up from the base of the pole and placed 90 degrees from the bracket arm on the far side of the traffic flow.

(f) Isolation Pad: Each light standard mounted on any bridge structure shall have 1/2 inch thick elastomeric pad of the same surface area as pole base, installed between the pole base and the structure, with four 11/32 inch thick by 2 inch O.D. minimum elastomeric washers installed on the top side of the base, between two standard galvanized flat washers. Elastomeric pads and washers shall conform to M.17.01.

(g) Hardware: All screws, nuts, bolts, washers and miscellaneous hardware used to assemble the light standard, base and brackets shall be stainless steel with the exception of washers and nuts for anchor bolts. Stainless steel bolts and washers shall conform to the requirements of ASTM A193, Grade B8. Stainless steel nuts shall conform to the requirements of ASTM A194, Grade 8.

(h) Design Requirements: The pole manufacturer shall certify on the catalog cuts or shop drawings that the light standard, with appurtenances, conform to all pertinent requirements of AASHTO "Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals" including the latest AASHTO breakaway requirements and in accordance with definition given to the latest edition of AWS, as specified under AASHTO, Section 5, Article 1.5.5., Fabrication of Welded Aluminum Structure.

M.15.05—Roadway Luminaire: The luminaire shall be of the "cobra-head" type for roadway lighting with gray finish. The housing shall be die-cast aluminum with refractor door hinged on one end and latched on the other. It shall have a 2 inch slip fitter for end mounting and factory-installed bird guard. Reflector is to be aluminum, refractor to be glass. The optical assembly shall have a captive, non-wicking gasket to filter air. There shall be no photoelectric control receptacle unless specifically required for the application, and there shall be no open holes in the housing. The socket shall be porcelain enclosed for mogul base lamp and adjustable to provide IESNA Light distribution type as shown on the plans.

The integral ballast shall be the regulator type with high power factor (above 90%) and in full compliance with current ANSI lamp/ballast specifications. Starting current shall be less than operating current. The ballast shall be capable of starting and operating a specified lamp at -20° F. The unit shall be designed to operate under ±10% line voltage variation. The ballast shall operate at the specified circuit voltage. The entire ballast and starter unit shall be pre-wired and quick-disconnect type for ease of maintenance.

Conductors shall be #10 AWG in accordance with M.15.11. Insulation shall be THHN.

The breakaway-type fuse holders shall be of rubber or molded plastic housing which is watertight, as detailed on the plans. They shall be rated at 600 volts. Fuses shall be UL listed, fast-acting, current-limiting and rated at 10 amp, 600 volt and 100,000 AIC.

The luminaire shall have an identification sticker indicating the lamp type and wattage located on the underside of the luminaire, in accordance with current ANSI Standards.

The lamp shall be all position of the source and wattage called for on the plans, and in accordance with the current ANSI specification. The lamp shall be new and unused at the time of installation.

M.15.06—Vacant

M.15.07—Underbridge Luminaire: The luminaire shall be wall or pendent mounted with distribution type, wattage and voltage as specified on the plans.

The wall-mounted luminaire shall be complete with an optical assembly consisting of an anodized aluminum reflector and a single piece prismatic refractor made of thermal shock-resistant borosilicate glass protected by a clear polycarbonate shield. If specified, a steel wire guard shall be placed over the polycarbonate shield or used in lieu of the polycarbonate shield. The door shall be die-cast aluminum, self-hinged and secured by 2 captive stainless steel threaded fasteners. The housing shall be die-cast aluminum with a 3/4 inch threaded top entry and a captive non-wicking filtering gasket against which the door shall seat when closed. The luminaire shall provide a maximum vertical candle-power under 75 degrees and shall be UL listed as suitable for wet locations.

The pendant mounted luminaire shall be complete with a prismatic polycarbonate refractor and UL listed as suitable for wet locations. The housing shall be die-cast aluminum with a 1 1/4 inch threaded top entry and a die-cast aluminum refractor holder hinged to the luminaire housing in a manner that allows easy removal of the assembly. The refractor shall be securely held by means of a stainless steel hinge and a positive acting spring loaded latch which will permit single action release and closing with a snap action.

The lamp socket shall have a porcelain enclosed mogul base. The socket for a pendent mounted luminaire shall be adjustable to provide the IESNA light distribution type called for on the plans.

The integral ballast shall be the regulator type with high power factor (above 90%) and in full compliance with current ANSI lamp/ballast specifications. The ballast shall be capable of starting and operating a specified lamp at -20° F. Starting current shall be less than operating current. The unit shall be designed to operate under ± 10% line voltage variation. The ballast shall operate at the circuit voltage specified on the plans. The entire ballast and starter unit shall be pre-wired.

Conductors shall be #10 AWG, 600V, in accordance with M.15.11. Insulation shall be THHN.

Fuse holders shall be of rubber or plastic molded housing which is watertight and shall be used in each

branch circuit supplying a luminaire or luminaires. They shall be rated at 600 volts. Fuses shall be UL listed, fast-acting, current-limiting and rated at 10 amp, 600 volt and 100,000 AIC.

Mounting materials and hardware shall be as indicated on the plans or as directed by the Engineer.

Conduit shall be in accordance with M.15.09.

The luminaire shall have an identification sticker indicating the lamp type and wattage in accordance with current ANSI Standards.

The lamp shall be all position of the source and wattage called for on the plans, and in accordance with the current ANSI Standard. The lamp shall be new and unused at the time of installation.

M.15.08—Vacant

M.15.09—Electrical Conduit:

1. Rigid Metal Conduit: Rigid Metal Conduit (RMC) and fittings shall be galvanized steel. Each section of conduit shall be labeled as UL listed, and shall meet the requirements of the latest UL and ANSI standards for Rigid Metal Conduit. Set-screw or compression fittings shall not be used.

2. Intermediate Metal Conduit: Intermediate Metal Conduit (IMC) and fittings shall be galvanized steel. Each section of conduit shall be labeled as UL listed, and shall meet the requirements of the latest UL and ANSI standards for Intermediate Metal Conduit. Set-screws or compression fittings shall not be used.

3. Polyvinyl Chloride Conduit: Polyvinyl Chloride Conduit (PVC) and fittings shall be UL listed, and shall meet the requirements of the latest UL and ANSI standards for Polyvinyl Chloride Conduit. Schedule 40 grade shall be used where underground, Schedule 80 grade shall be used on surface.

4. Liquid Tight Flexible Metal Conduit: Liquid tight flexible metal conduit shall consist of a continuous strip of flexible galvanized metal, coated with an extruded jacket of polyvinyl chloride. Liquid tight flexible metal conduit shall be suitable for use in wet locations and shall meet the requirements of the latest UL and ANSI standards for liquid tight flexible metal conduit.

M.15.10—Cast Iron Junction Box: Junction Boxes shall be cast iron constructed to NEMA Type 4 requirements. The junction box cover shall be attached with approved stainless steel bolts and sealing washers, and shall be equipped with a neoprene cover gasket. For an 18 inch × 12 inch × 8 inch cast iron junction box, cover bolts shall be 1/4 inch × 5/8 inch; threads shall be 20 N.C.; and the head shall be hex type and shall accept a 7/16 inch socket.

The covers for cast iron junction boxes shall be 1/4 inch thick steel meeting the requirements of ASTM A36 and shall be galvanized in accordance with ASTM A123. Holes for the cover bolts shall be recessed in the cover plate to accept a 7/16 inch socket wrench.

All junction boxes recessed in concrete shall have an outside flush flange with a recessed cover and the conduit entrance holes shall be slip fit.

Junction boxes which are for surface mounting shall have threaded hubs or threaded bosses with a minimum of 5 full threads. All boxes and covers shall be hot-dip galvanized conforming to the requirements of ASTM A153.

Rigid metal conduit for drain pipe shall conform to the requirements of M.15.09-1.

M.15.11—Single Conductor:

1. Insulation: Insulation type, size, and voltage rating shall be as shown on the plans. Insulation shall be permanently color identified throughout its entire length. Color shall be as called for on the plans. A green, white or natural gray color shall not be used unless it is a grounded conductor. The rating, type, size, and manufacturer shall be durably marked in accordance with the NEC.

2. Conductor: Conductors shall be single conductor, stranded, copper, meeting the latest ICEA, UL, NEMA, and FS for the type specified.

M.15.12—Cable in Duct: The materials for this item shall consist of single conductors in a flexible non-metallic conduit, the whole unit shall be pre-assembled for direct burial. The size, number and type of single conductors shall be as called for on the plans and shall meet the requirements of M.15.11. The conductors shall be laid parallel, not twisted, so they may be individually removed.

The duct size shall be governed by the number and size of the single conductors contained as listed below. Nominal duct size shall be as follows unless otherwise noted on the plans:

- 1 1/4 inches with minimum bending radius of 18 inches for up to four No. 2 conductors
- 2 inches with minimum bending radius of 24 inches for more than 4 No. 2 conductors and through 4 No. 2/0.

The duct material shall be polyethylene and meet the requirements of Schedule 40. The duct shall meet all applicable standards of the latest NEMA Standard Publications no. TC-2 and TC-7.

M.15.13—Bare Copper Grounding Conductor: The conductor shall be 7-strand, soft-drawn, bare copper wire of the size specified on the plans.

M.15.14—Vacant

M.15.15—Service Entrance Cabinets:

1. **Cabinets:** The service entrance cabinet and the lighting control cabinet shall be sheet aluminum and conform to NEMA type 3R enclosure requirements. Type and dimensions are as called for on the plans. The doors shall be hinged, weatherproof gasketed, with handle and Conn-1 lock for the lighting control cabinet, and a stainless steel door handle with hasp for utility company padlock for the service entrance cabinet. The cabinet shall have a full-size backboard of 3/4 inch smooth marine grade plywood, painted black.
2. **Instrument Transformer Cabinet:** The instrument transformer cabinet shall meet the requirements for the service cabinet.
3. **Meter Socket and Enclosure:** The meter socket and enclosure shall meet utility company specifications.
4. **Pedestal:** For pedestal mounted cabinets, the pedestal shall be as specified in M16.03-2.
5. **Foundations:** Concrete shall meet the pertinent requirements of M.03, Class PCC03340. Anchor bolts shall meet the requirements of M.15.02-2. Pedestal foundation may be precast.
6. **Transformer Pad:** Concrete shall meet the pertinent requirements of M.03, Class PCC03340. The pad may be pre-cast. This item shall meet utility company requirements.
7. **Ground Rod:** The ground rod shall be dimensioned as shown on the plans, copper clad steel. The clamp shall be a square head bolt type.
8. **Conduit:** Conduit shall be of the type called for in the plans and as specified in M.15.09.
9. **Conductors:** Conductors shall be as specified in M.15.11.
10. **Photoelectric Control:** The photoelectric control shall have an adjustable turn on setting set at approximately 2 foot-candles with a time delay of at least 3 to 5 seconds. The control relay shall have single pole double throw contacts rated for 1000 watts. The coil shall operate at 120 Volts AC. It shall be applicable to the electrically operated, mechanically held contactor. Conductors shall be #12 AWG meeting the requirements of M.15.11. The manual switch shall be a three-way toggle switch mounted in a single gang utility box.
11. **Contactor:** The control contactor shall be of the size, rating and number of poles, as shown on the plans. It shall be electrically operated (120 volt), and mechanically held.
12. **Control Transformer:** The transformer shall be dry type, single-phase, two winding for wall mounting, of the size rating and voltages as called for on the plans.
13. **Circuit Breakers:** Circuit breakers shall be thermal magnetic type, non-enclosed, front-connecting, for mounting to the backboard. The number of poles, voltage rating and current rating shall be as shown in the plans.
14. **A utility light** shall be mounted in the cabinet with porcelain or plastic base, pull chain and incandescent lamp.
15. **Ground Fault-Circuit Interrupter Receptacle:** The ground fault-circuit interrupter receptacle shall be 20 amp, 125 volt, 60 HZ Class A, and shall be UL listed.
16. **Main Disconnect Safety Switch:** The main disconnect safety switch shall be a heavy-duty, fused, three pole, single throw switch, with a manual lever, appropriate service grounding kit, rated for 480 volts, with a current rating as shown on the plans, NEMA Type 1 housing, UL-listed, and suitable for use as service equipment.

M.15.16—Navigation Light: The navigation light shall be of a type approved by the United States Coast Guard and shall be in accordance with their regulations for lighting bridges.

The navigation light shall be made with a bronze casting and shall be entirely weatherproof. It shall be ventilated to prevent sweating of the lens, and the vent shall be screened to prevent entry of bugs and insects. All parts of the lamp shall be interchangeable so that each lamp can be assembled with a 180 degree or 360 degree 8 inch Fresnel lens.

Each navigation light shall be equipped with 2 100-watt, 120-volt lamps and a lamp failure relay. The green navigation lights shall have a 360 degree green Fresnel lens. The red navigation lights on the

structure or pier shall have 180 degree red Fresnel lens.

Flexible cords shall be type S.O.W. 3 conductor #10 AWG rated at 600 volts.

SECTION M.16
TRAFFIC CONTROL SIGNALS

M.16.01—General Requirements
M.16.02—Vacant
M.16.03—Pedestals
M.16.04—Poles
M.16.05—Mast Arm Assembly
M.16.06—Traffic Signals
M.16.07—Pedestrian Signal
M.16.08—Pedestrian Push Button
M.16.09—Controllers
M.16.10—Flasher Cabinet
M.16.11—Vacant
M.16.12—Loop Vehicle Detector and Sawcut
M.16.13—Magnetic Vehicle Detector
M.16.14—Control Cable
M.16.15—Messenger and Span Wire
M.16.16—Vacant
M.16.17—Illuminated Signs

M.16.01—General Requirements: Where reference is made to certain manufacturers' material or products, it is not the intent to preclude the use of others, but rather to establish minimum acceptable design standards. The Contractor may substitute material and products of other manufacturers, provided they meet the minimum design standards and are approved by the Engineer.

M.16.02—Vacant

M.16.03—Pedestals: The materials for this work shall meet the following requirements:

1. Vacant

2. Aluminum Pedestals:

(a) Shaft: The shaft shall be made of ASTM B210 Grade 6063-T6 or ASTM B221 Grade 6005-T5 aluminum alloy and shall be either a seamless tapered tube with a 6 inch outside diameter at the base and a 4 1/2 inch outside diameter at the top, or a seamless straight tube with a 4 1/2 inch outside diameter. The height of the pedestal shall be as required by the item. The shaft shall have a minimum wall thickness of 0.125 inch and shall be devoid of welding except at the base. The shaft shall be satin brush finished and each shaft shall be individually protected from scratches, dents and abrasions during handling and shipping.

(b) Base: The base shall be a permanent mold casting of 356 aluminum alloy conforming to ASTM B108 and shall be of the dimensions shown on the plans. The tapered shaft shall be secured to the base by means of a circumferential fillet weld on the inside of the base top, and the straight shaft shall be secured by means of threads and machine screws, which when set, shall be flush with the base top. The base shall be provided with a cast aluminum door with dimensions shown on the plans, which shall be held in place by an approved locking device. Each base shall be provided with a grounding stud. All hardware used on the pedestal shall be 304 stainless steel meeting the requirements of ASTM A276.

M.16.04—Poles: The materials for this work shall meet the following requirements:

1. Steel Poles:

(a) Round Continuously Tapered Shaft: The continuously tapered shaft must have a maximum of 2 longitudinal continuous arc welds. No intermediate horizontal joints or welds will be allowed. Poles shall have a uniform wall thickness that tapers uniformly. All cross sectional shapes will be allowed, but multisided poles are required to be equilateral with a minimum of 8 sides. The shaft diameter must decrease from the butt end at the rate of not less than 0.07 inch and not more than 0.14 inch per foot of length. Laminated pole designs (e.g. 2-ply pole shafts) will not be permitted.

The shaft shall be placed under sufficient pressure to flatten the weld and increase the physical characteristics to a minimum guaranteed yield strength of 48,000 psi.

(b) Round Step Tapered Shaft: Round step tapered shafts shall be fabricated from round pipe sections with a maximum of one longitudinal seam. Horizontal seams must be joined by a hot-swaged shrink fit and continuously seal welded to prevent the entrance of water.

The shaft diameter must decrease from the butt end at the rate of not less than 0.07 inch and not more than

0.14 inch per foot of length. This is accomplished through the use of decreasing diameter round pipe sections.

(c) Base: The base shall telescope the shaft and shall have 2 continuous transverse arc welds, 1 being on the inside of the base at the end of the shaft and the other weld on the outside at the top of the base. The welded construction shall develop the full strength of the adjacent section resistant to bending action.

The anchor base shall have 4 holes to receive the anchor bolts and four holes directly behind each anchor bolt hole for ventilation. The anchor bolt holes shall be 0.375 inch larger than the diameter of the anchor bolt.

(d) Pole Size: The traffic signal span pole shall have the yield strength, bolt circle, and length indicated on the plans.

(e) Span Clamp: The span clamp shall be made of steel capable of supporting a minimum load of 12,000 pounds without sustaining permanent distortion. There shall be 1 span clamp per span wire attachment. Additional span clamps shall be provided as required.

(f) "J" Hook: The "J" Hook shall be affixed inside the top of the pole for handling and cable support.

(g) Luminaire Bracket: The type and spread of the luminaire bracket shall be as indicated in the details on the plans. The bracket shall be the single-arm type for brackets 8 feet and under in length and the truss type for brackets 10 feet and longer in length unless otherwise specified.

The truss type shall consist of an upper and lower galvanized steel member securely joined by means of a vertical strut steel pipe without ornamental steel scroll.

The brackets shall be attached to the pole by means of a bolted clamp so that the angle for the arm may be adjusted in the field.

(h) Handhole: A steel handhole reinforcing frame a minimum size 4 inches × 6.5 inches, complete with removable cover, shall be welded to the shaft approximately 15 inches above the base. The cover shall be fabricated from galvanized steel.

(i) Wire Entrance Fitting: All steel span poles shall include wire entrance fittings. The number and size of the wire entrance fittings shall be as shown on the plans. A neutral bracket and 1 inch entrance fitting shall be provided for service cables. A separate fitting will also be required for any communications cable. These couplings will be continuously welded to the pole at a 45-degree angle to the pole shaft.

(j) Anchor Bolt Covers: Four (4) removable galvanized ferrous or aluminum anchor bolt covers that attach to the pole base shall be provided. The anchor bolt covers shall be attached by a tapped hole and screw to either the pole base or pole shaft. The covers shall fit snugly and have a neat appearance.

(k) Electrical Ground: Each span pole shall be electrically grounded to a ground rod. The span pole will be connected to the ground rod by a #8 AWG copper wire. The wire shall be connected to the pole by a stainless steel bolt and to the ground rod by a square head bolt clamp, conforming with the NEC.

(l) Identification: Span pole information (as described below) shall be included on the poles in the form of a stamp on the top of the base plate, or an aluminum tag riveted to the base of the pole or to the pole shaft below the handhole.

(a) Minimum load at yield
(b) Name of manufacturer
(c) Year of manufacture
(d) Height of the pole

(m) Fabrication: The steel pole shall be fabricated in conformance with the current AASHTO "Standard Specification for Structural Support for Highway Signs, Luminaires and Traffic Signals." The pole manufacturer shall include a statement on the Materials Certificates stating that the welding design and fabrication is in accordance with the AWS Structural Welding Code D1.1-Steel.

(n) Finish: The steel span pole (shaft and base and luminaire bracket) shall be hot-dip galvanized, meeting the requirements of ASTM A123. Pole cap, bolt covers, handhole covers and span clamps shall be galvanized as per ASTM A153. Bolts, screws, washers, nuts and wire entrance fittings shall be galvanized as per ASTM F2329.

2. Wood Poles:

(a) Pole: Wood poles shall conform to the requirements of ANSI 05.1 and AWPA C-1 and AWPA C-4. The pole shall be class 3 and of sufficient length to obtain the necessary height of the signals above the pavement or the mounting height of luminaires above the pavement.

The poles shall be trimmed by machine without materially changing their size or taper. Poles shall be branded on the face, have a flat roof, and shall be aluminum tagged on the butt in accordance with ANSI 05.1. The arrangement and order of the code letters and figures shall be as the following example indicates.

PTC	Supplier's code or trademark
F-78	Plant location and year of treatment
SPC	Species and preservative code
3-35	Class and size

The dimensions for the poles required shall not be more than 3 inches shorter or 6 inches longer than the nominal length. The length shall be measured between the extreme ends of the pole. The minimum bottom circumference shall be measured at a point 6 feet from the butt and the minimum dimensions for the poles shall be as follows:

Length	Bottom Circumference	Top Circumference
30 feet	32 inches	23 inches
35 feet	34 inches	23 inches
40 feet	36 inches	23 inches
45 feet	37.5 inches	23 inches
50 feet	39 inches	23 inches

The preservative treatment of the pole shall be done in accordance with the Standard Specification for Preservatives and Pressure Treatment Process for Timber, AASHTO M 133. The following named preservatives are acceptable. The properties shall be those set forth in the referenced AWPA standards:

1. Creosote	AWPA	P-1
2. Pentachlorophenol	AWPA	P-8
3. Chromated Copper Arsenate - Type C	AWPA	P-5

(b) Anchor: The anchor to be used for guying the pole shall be a four-way sliding plate as indicated on the plans.

(c) Anchor Rod: The anchor rod to be used for guying the pole shall be of drop steel and shall be hot-dip galvanized. It shall be as indicated on the plans. It shall be a type as listed in the special provisions.

(d) Anchor Guy Strand Wire: The anchor guy strand wire shall be 3/8 inch 7-wire strand, supplied with a zinc coating by the hot-dip galvanized process in accordance with ASTM A475 for zinc-coated steel wire strand, and shall have a minimum breaking strength of strand of 11,200 lb. for utilities grade.

(e) Guy Wire Protector: The guy wire protector to be used shall be the half-round type either fabricated from a minimum of 16-gauge hot-dip galvanized steel or from high impact 13454A Polyvinyl Chloride ASTM D1784 compound standard. It shall have hot-dip galvanized steel clamps. The steel clamps shall conform to ASTM requirements A36 for steel and A123 for galvanization. There will be 3 clamps which will accept any strand size 1/4 inch diameter and larger, and any anchor rod up to a 1 inch diameter.

M.16.05—Mast Arm Assembly: The materials for this work shall meet the following requirements:

1. Aluminum:

(a) Shaft: The shaft shall be 6005-T5 Aluminum Alloy as specified by ASTM B221 or approved equal, and shall be a seamless tube tapered by cold working. The shaft wall thickness, length and tube diameters shall be as indicated on the plans. The shaft shall have a satin brush finish and shall be suitably protected during handling and shipping.

(b) Arm: The arm shall be of tapered elliptical truss-type design. The arm shall consist of an upper and lower member joined by vertical braces and clamps of 356 Cast Aluminum Alloy conforming to ASTM B108 for attachment to the Shaft. All necessary fittings and adjustable signal mounting assemblies required for attachment of the signal heads shall be included with the arm. The arms and struts shall be fabricated of 6005-T5 wrought aluminum alloy as specified by ASTM B221 or approved equal. The arm shall be formed to dimensions as shown on the plans. The wall thickness shall be as shown on the plans. All vertical braces shall be 2 3/8 inches outside diameter having a 0.140 inch wall thickness and elliptically shaped to a cross-section of 3 inches × 1 3/4 inches. The clamp castings shall withstand the design load of the arm assembly as shown on the shop drawings.

(c) Shoe Base: The shoe base shall be 356 cast aluminum, conforming to ASTM B108 of adequate shape and size to develop the full strength of the adjacent shaft section. The base shall be secured to the

lower end of the shaft by 2 continuous welds. The base shall telescope the shaft, and 1 weld shall be on the inside of the base at the end of the shaft and the other weld shall be on the outside at the top of the base. The base shall be provided with four holes for mounting on the 16 inch bolt circle and 4 cast aluminum bolt covers which shall be attached to the base by means of stainless steel screws.

(d) Transformer Base: The transformer base shall be a permanent mold casting of 356-T5 aluminum alloy conforming to ASTM B108 and shall be of sufficient size to withstand the full design load of the shaft. The transformer base shall have a door of dimensions shown on plans attached to the base by means of cast lugs at the bottom and a stainless steel socket head cap screw with a wrought aluminum latch at the top. The bottom of the base shall have provision for anchorage at the corners to accept four anchor bolts.

(e) Welding: Welding of aluminum shall conform to the AASHTO "Specifications for the Design and Construction of Structural Supports for Highway Luminaires," Section 10—Fabrication of Welded Aluminum Structures.

2. Steel:

(a) Shaft: The shaft shall be made from 1 length of sheet steel not less than the gauge specified. There shall be a maximum of 2 longitudinal continuous arc welds. There shall be no intermediate horizontal joint or weld. The length of sheet steel shall be formed into a continuously tapered shaft, having a taper of approximately 0.14 inch/foot. The arm sizes shall be as shown on the plans.

After the welding and cold-rolling, the shaft shall be placed under sufficient pressure to flatten the weld and increase the physical characteristics of the shaft so the metal will have a minimum yield of 48,000 psi. Cross-sectional shapes other than round will be allowed. A steel handhole reinforcing frame a minimum size 4 inches × 6.5 inches, complete with removable cover, shall be welded to the shaft approximately 15 inches above the base. The cover shall be galvanized steel. There shall be a 0.5 inch tapped hole near the base of the shaft for the grounding bolt.

A round step tapered shaft will also be acceptable. The shaft shall be fabricated from round pipe sections with not more than 1 longitudinal seam, joined by a hot-swaged shrink fit, continuously seal-welded to prevent the entrance of water. Stepped round shafts must decrease in diameter from the butt end at the rate of not less than 0.07 inch and not more than 0.14 inch per foot of length by use of decreasing diameter round pipe sections. The minimum yield strength of the material shall be 48,000 psi.

A flange plate, a minimum of 1 1/4 inches thick, shall be welded to the pole near the top supported by side plates tangent to the pole and gusset plates, both top and bottom. The flange plate shall have a 2 1/2 inch diameter wiring hole and 4 tapped holes for 1 1/4 inch high-tensile bolts.

A device shall be provided near the top, inside of the pole, to relieve the strain on the cable feeding the signals. The pole cap shall be secured by means of set screws.

Where shown on the plan, there shall be provided a steel tapered luminaire bracket. The bracket shall be the single arm type for brackets 8 feet and under in length and truss type for brackets 10 feet and longer. The truss type shall consist of an upper and lower member securely joined by means of a vertical strut. The upper and lower members shall be 2 inch Iron Pipe Size (I.P.S.) steel pipe without ornamental steel scroll. The length of the bracket shall be as shown on the plans.

(b) Anchor Base: A 1-piece steel anchor base of the adequate strength, shape and size shall be secured to the lower end of the shaft by 2 continuous electric arc welds. The base shall telescope the shaft, and 1 weld shall be on the inside of the base at the end of the shaft, while the other weld shall be on the outside at the top of the base. The 2 welds shall be approximately 2 inches apart, and the design shall be such that the welded connection shall develop the full strength of the adjacent shaft section to resist bending action.

The base shall be provided with four holes to receive the anchor bolts, and four tapped holes for attaching the anchor bolt covers.

(c) Mast Arm: The mast arm may be made of 1 length or 2 lengths of steel. There shall be a maximum of 2 longitudinal continuous arc welds. The length of sheet steel shall be formed into a continuous taper shaft having a taper of approximately 0.14 inch/foot. After the welding and cold-rolling, the mast arm shall be placed under sufficient pressure to flatten the weld and increase its physical characteristics so that the metal will have a guaranteed minimum yield strength of 48,000 psi.

A flange plate, 1 1/4 inches thick, shall telescope the large end of the mast and be welded by 2 continuous electric arc welds. One (1) weld shall on the outside of the plate, adjacent to the shaft, and the other weld on the inside at the end of the tubular cross-section. The flange plate shall have 4 holes for the 1 1/4 inch high-tensile bolts, which shall match the 4 tapped holes in the mounting plate on the pole.

Mast arms for step tapered shafts may be 2-piece construction with a telescoping joint secured by a thru-bolt and lock nut. Arms less than 35 feet long need not be tapered. The arm shall be not less than the

gauge specified. The minimum yield strength of the material shall be 48,000 psi.

Each mast arm shall be drilled in the field on the bottom surface for wire outlet holes and signals positioned at the location indicated on the signal plans. Rubber grommets shall be provided and installed by the Contractor at each field drilled hole to protect the signal cables from chafing.

A steel removable cap shall be attached to the end of the arm by a set screw. For each required signal, a clamp and clevis with 3/4 inch diameter hole shall be furnished.

(d) Finish: The steel mast arm assembly and luminaire bracket shall be hot-dip galvanized, meeting the requirements of ASTM A123 unless otherwise specified. Pole Cap, bolt covers and handhole covers shall be galvanized to meet the requirements of ASTM A153. Bolts, washers, nuts and screws shall be galvanized, meeting the requirements of ASTM F2329.

(e) Fabrication: The steel Mast Arm Assembly shall be fabricated in conformance with the current AASHTO "Standard Specification for Structural Support for Highway Signs, Luminaires and Traffic Signals." The manufacturer shall include a statement on the Materials Certificates stating that the welding design and fabrication is in accordance with the AWS Structural Welding Code D1. 1 Steel.

3. Identification: Mast Arm identification (as described below) shall be included in the form of a stamp on the top of the base plate, or an aluminum tag riveted to the base of the shaft below the handhole.

(a) Maximum equivalent end load (weight and area)
(b) Name of manufacturer
(c) Year of manufacture
(d) Arm length

M.16.06—Traffic Signals: The materials for this work shall meet the following requirements:

1. General: The components of a traffic control signal head assembly or unit shall conform to the ITE requirements for Adjustable Face Traffic Control Signal Head Standards Technical Report of the latest issue.

2. Traffic Control Signals: Each signal head shall be of the adjustable, vertical type with the number and type of sections and lamps as shown on the plans, shall provide a light indication in 1 direction only; and shall be adjustable through 360 degrees without the removal from the mounting bracket; and shall be mounted at the locations, and in the manner as shown on the plans. Unless otherwise indicated on the plans, all signal heads shall be standard and shall contain three lights arranged as follows: Red-top; Amber-center; Green-bottom. All signal heads in this contract shall be of the same make and type.

3. Housing: The signal head housing shall be made of a die-cast aluminum alloy per ASTM B85. The signal head housing shall consist of an assembly of three or more separate sections, expansible type, for vertical mounting, substantially secured together in a watertight and rigid manner to form a unit of pleasing appearance.

Each section shall house an individual optical unit. The signal head shall be so designed that sections may be added or removed from the head assembly by the use of simple tools.

The housing shall be a die casting, rigidly constructed with a smooth outer surface. There shall be a round opening in the top and bottom of each head to receive a 1 1/2 inch supporting pipe frame. The portion of the housing section around the opening shall be reinforced and serrated so that serrated fittings may be used to secure the housing. The top and bottom of the housing shall include such other openings as are necessary to accommodate fastening devices to hold sections together, and such openings shall not permit entrance of foreign particles. Each section shall be positively indexed with respect to an adjacent section to prevent misalignment. Each housing section shall be equipped with a door with an opening and fittings for the optical unit and visor. The door shall be arranged for easy access to the optical unit components and wiring. It shall be hinged on its left side so that the door cannot be removed without the use of tools at 2 points and shall be provided with a simple, positive-acting door locking device made from stainless steel to assure tight closure. The door or housing shall be grooved to receive a neoprene gasket to provide a resilient seal between the door and housing. The body and door of each housing section shall be of die-cast aluminum alloy and shall be clean, smooth, and free from flaw, crack, blow holes, or other imperfections. The door locking device and visor fastening screws shall be made of stainless steel. All other screws, nuts, washers, hinge pins, latch parts, clips and parts used in the assembly of the signal housing shall be made of either naval brass, Everdur, phosphor bronze, stainless steel or approved equal.

4. Brackets: The signal heads, except for post-top mounting, shall be supported by mounting brackets consisting of assemblies of 1 1/2 inch standard, iron pipe and malleable iron fittings. All members shall be plumb and level, symmetrically arranged, and securely made up. Construction shall be such that all

conductors are concealed within poles and assembly.

Each section of the housing shall be provided with a removable cap visor of not less than #16 Brown and Sharpe (B & S) Gauge Aluminum. The cap visor shall be painted a flat black on the inside to prevent reflection, and must also eliminate sun-phantom. The visor shall fit snugly against the door and shall not permit any perceptible filtration of light between the door and the visor.

A terminal block shall be mounted inside of the back of the housing in the top section. The terminal blocks shall have sufficient studs with separate screws to terminate all field wires and lamp wires independently to the block.

When indicated on the plans, a backplate of dimensions, as shown on the plans, constructed of 3003H 14 aluminum alloy sheet 0.051 inch minimum thickness conforming to ASTM B209 shall be attached to the signal head housing. The backplate shall be painted a flat black on both sides.

5. Optical Unit: The optical unit shall consist of a lens, reflector, reflector frame, and socket and traffic signal lamp of the size shown on the plans. Other necessary equipment shall be designed to give clearly visible signal indications within an angle of at least 45 degrees to the traffic to be controlled and from 10 feet to 300 feet, under all light and atmospheric conditions except dense fog; and shall be of such design as to eliminate sun-phantom effects. The optical unit shall be readily accessible for maintenance.

Lenses shall be of the color indicated, circular in shape, with a visible diameter of 8 inches, unless otherwise noted on the plans, except that all arrow lenses shall have a visible diameter of 12 inches. Except where indicated on the plans, the use of adapters to achieve 12 inch indications will not be allowed. Lenses shall be of such design to give an outward and downward distribution of light with a minimum above the horizontal. Arrow lenses shall conform in design to ITE standards and shall be of a green light diffusing nature, giving uniform distribution of illumination on all areas of the lens. The field for arrow lenses shall consist of a black ceramic compound, baked or fired into the glass.

Each lens shall be polycarbonate capable of withstanding continuous illumination of a standard traffic signal lamp without thermal distortion [100-Watt - 8 inch lens] [150-Watt - 12 inch lens]. The lens shall be 3/16 to 5/16 inch thick, smooth on the outside surface, annealed to relieve internal stresses and of high illumination transmission. An air-cured neoprene lens gasket shall be provided between the lens and the signal door and between the lens and the reflector ring. The 2 ends of the lens gasket shall be joined together by vulcanizing or other approved method. Each lens shall have the word "Top" marked on its flange to indicate proper positioning of the lens in the door, together with the trademark of the lens manufacturer and label indicating conformance with the specifications set forth in the ITE standards. The lens and its gasket shall provide water-tight and dust-tight construction. The gasket shall be molded, heat-resistant neoprene.

The reflector shall be made of specular Alzak aluminum, the thickness of the anodic coating to a minimum of 0.0001 inch, or its equivalent, spun or punched from metal not less than 0.025 inch thick, equipped with a bead or flange on the outer edge to stiffen the reflector and insure its being held true to shape. The reflecting surface shall be totally free of flaws, scratches, defacements or mechanical distortion.

An opening in the back of the reflector for the lamp holder shall be so constructed that there will be no dark spots cast on the lens. The reflector shall be of the pressure type, free floating and must insure full seal against the lens gasket. The reflector frame shall be of die-cast aluminum constructed completely around the reflector, and arranged to assure maintaining proper positioning of the reflector with respect to the lens under normal operating conditions. Proper position shall be maintained after opening and closing the unit for lamp replacement and other maintenance functions. A dust-tight gasket shall be provided between the reflector and the reflector-holding ring. If this gasket is of neoprene, it shall be made continuous by vulcanizing ends together or by another process approved by the Engineer. Additional dust-tight gasketing shall be provided between the lamp holder and the reflector.

6. Lamp Socket: The lamp socket shall be so designed that the tip or center contact shall retain its spring action and shall not twist or turn when lamps are installed. The ring or outer contact shall be securely fastened to the receptacle base so that it will not become loose, break out, or turn when replacing lamps. The lamp socket shall be mounted so that when a lamp is properly inserted and adjusted it shall retain that position. The lamp socket shall be made of heat-resistant material designed to hold a traffic signal lamp of the wattage indicated on the plans, and shall be of the vibration-proof type, weatherproof molded construction, immune to the operating temperatures of the unit and substantially supported, independent of the reflector. Each lamp socket shall be the adjustable type and shall be wired with 2 leads using a minimum size of #18 AWG, 600 volt, 115 degree C stranded wire not less than 3 feet long. This wire shall be marked in accordance with Article 310-11 of the latest edition of the NEC. The sockets shall

be so wired that a white lead will be connected to the shell of the socket and a black lead to the bottom or end terminal of the socket.

All lamps shall have indelibly etched thereon the name of the manufacturer, the ratings in watts for which the lamp was designed and the operating voltage of the lamp. Each lamp shall have the beam candela specifications of the ITE.

The lamps shall operate at 67 or 69 watts as the case may be at plus or minus 4% at rated voltage. The lamps shall have a voltage rating of 125 volts and the initial lumen rating of the 67-Watt lamp shall not be less than 618 and the 69-Watt lamp not less than 630.

The bulb designations shall be AT-21 or A-21 for the 67-Watt and A-21 for the 69-Watt. The 67-Watt lamps shall have a maximum over all length of 4.625 inches and a light center of 2.44 inches. The 69-Watt lamps shall have a maximum over all length of 4.44 inches and a light center of 3 inches. All lamps shall have a fuse wire within their structure to prevent damage to lamp circuitry or receptacle and they shall also have an aluminum reflector disc incorporated within the bulbs to assist light control.

The base of the lamps shall be medium, made of brass, and shall have the base stamped or printed with "Scratch Out" letters and numerals which indicate 12 months of the year and at least 4 indications for years. This is for the purpose of maintaining control for re-lamping.

The burning position shall be base down or horizontal and the lamps shall have a rated life of 8,000 hours of use guaranteed by the manufacturer.

The 67-Watt lamps shall be installed in traffic signals with 8 inch lenses. The 69-Watt lamps shall be installed in the traffic signals with 12 inch lenses.

7. Mountings: All signal heads shall be equipped with positive locking devices and fittings designed to prevent the heads from turning due to external forces. Number 2 Permatex or approved equal shall be used at all points where lock nuts and washers are used in assembling heads.

8. Dual Color Fiber Optic Section: When called for on the plans, signals shall be provided which display alternate legends, consisting of either a green or amber directional arrow. The arrows shall be illuminated by the use of fiber-optics and shall be clearly legible and visible, at full intensity, anywhere within a 50-degree cone, centered about the optical axis.

Any combination of colors shall be available by changing color filters installed in the unit, i.e., a signal supplied to display both a green and amber arrow could be field modified to display red and green arrows without the necessity of removing the signal from the case.

One (1) lamp, type ENL, shall be used for each message. These lamps shall be operated between 10.5 and 10.8 volts. Average lamp life shall be not less than 8,000 hours. Field data, laboratory data and manufacturing specifications shall be supplied to substantiate lamp life.

Non-lensed, fully randomized bundles shall be used to provide a 50-degree, total-viewing angle. Nineteen (19) individual enlarged-diameter bundles shall be used to form the arrow legend. The same bundles shall be used to display either message.

In order to help balance the intensity between colors, provisions shall be made in the construction of the signal to supply approximately 50% more light to the green arrow indication than supplied to the amber arrow indication.

Transformers shall be used to reduce the incoming 120 volts AC to 10.8 volts AC. The transformer shall have Class A insulation and shall be rated at 48.5-volt amps.

Power consumption of the dual color arrow shall be less than 50 Watts.

A separate transformer and bulb shall be used for each color to allow connection with the controller wiring and conflict monitors. Identical lamp types shall be used for each message.

Fiber optics shall be glass fiber bundles assembled on flat black matrix panels with mechanical protection for the assembly. The glass fibers at the input and output ends shall be ground smooth and optically polished for maximum light transmission. Matrix panel to have flat black non-reflective finish.

To eliminate long term condensation effects of thermal cycling, individual fiber-optic bundles shall not be jacketed or encased. Lamps shall be mounted horizontally to prevent their collecting water from condensation or possible gasket leaks.

The front panel shall be rotatable to indicate the desired direction when installed. No moving parts shall be permitted. Fiber-optics shall be protected by a black ABS plastic vacuum formed sculptured cover to eliminate the possibility of damage when installing or re-lamping.

The front panel of the signal section shall be colored black to minimize legibility of the arrow when not illuminated. No color at all shall appear when not illuminated regardless of sunlight intensity.

All fiber-optic, transformers and lamps shall be mounted on the door of the signal section. All screws,

washers, nuts and bolts shall be corrosion-resistant stainless steel. All components shall be readily accessible when the door is opened. The only tool required for maintenance shall be a standard screwdriver.

9. Painting: All surfaces of the signal housing, door, visors, inside and out, shall be cleaned and coated with a Primer conforming to FS TT-P-1757. The surfaces shall then be finished with 3 coats of infrared oven baked paint, before assembly.

First Coat: The primer shall be iron oxide backing primer and shall meet or exceed the requirements of FS TT-P-664.

Second Coat: Shall be light gray exterior baking enamel and shall comply with FS TT-E-489, either No. 16251, No. 16314, or No. 16376 Gray.

Third Coat: Dark Green Enamel: Shall be Dark Green exterior baked enamel and shall comply with FS A-A 2962. The color shall be No. 14056, FS No. 595. The inside of the visors shall be phthalic anhydride black synthetic baking enamel, with zero (0) gloss reflectance and shall meet the performance requirements of MILSPEC-E-5557 Enamel Heating Resisting Glyceryl Phathalate Type 4, Instrument black. The housing door and the outside of the visor shall be painted Flat Black unless otherwise specified.

M.16.07—Pedestrian Signal: The materials for this work shall conform to the following requirements:

A. General: The pedestrian signal shall be 1 section, rectangular in shape, and shall be in accordance with the MUTCD. The over-all size of the pedestrian signal, including visor, shall be as shown on the Typical Installation details. The pedestrian signal shall be the LED type unless indicated otherwise. The display shall be the international symbols for "Walk" and "Don't Walk." The symbols shall be clearly legible when illuminated. The symbols shall have the dimensions as shown on the plans. The "Walk" symbol shall be Lunar White. The "Don't Walk" symbol shall be Portland orange.

B. Housing: The housing shall be 1-piece, corrosion-resistant, aluminum alloy casting. The top and bottom of the housing shall have an opening to accommodate standard 1 1/2 inch pipe brackets. Each housing shall have a terminal block with a sufficient number of terminals to accept field wires. Two (2) integrally-cast hinge-lugs shall be cast on 1 side of each housing, and 2 integrally-cast latch-screw pads shall be cast on the other side of the housing.

The housing door shall be a 1-piece, corrosion-resistant, aluminum alloy casting. Two (2) hinge-lugs shall be cast on the other side of each door. The door shall be attached to the housing by means of 2 stainless steel hinge pins. Two (2) stainless steel hex-head, captive-latch screws shall be permanently attached to the door bosses by means of keeper washers. The door shall have a neoprene gasket which, when the door is closed, shall cause a seal against the housing, making a weatherproof and dust-tight seal.

Each signal shall be provided with an aluminum visor approximately 7 inches long, which shall encompass the top and sides of the signal face. The top of the visor shall have a downward tilt of approximately 3 1/2 degrees. The visor shall be blanked and formed, shall be not less than 0.05 inch thick, and shall meet the minimum requirements of ASTM B209.

C. Optical Unit:

1. Incandescent: The incandescent optical unit shall consist of a lens, reflector frame, lamp socket, and traffic signal lamp.

The lens shall be of rigid type polyester resin having an especially high heat distortion point and excellent resistance to degradation at high temperatures. Lens thickness shall be a minimum of 3/16 inch. The lens gasket shall be molded neoprene rubber channeled to cover the entire perimeter of the edge. The gasket shall press against the frame when the door is closed to form separate dust-tight optical compartments.

Reflectors shall be of 1-piece parabolic construction, designed to direct light from the lamp outward with maximum efficiency. The reflector shall have a flange that will form a flush fit with the gasket when the door is closed.

Sockets shall be 1-piece molded construction and shall be fastened to the reflector. The socket shall have a "lamp grip" to prevent the lamp from loosening. Color-coded leads of 18-gauge wire shall be attached to the terminal block in the signal. Lamps shall be 67-watt standard traffic signal lamps.

2. LED: The optical unit shall consist of multiple LED light sources and a regulated power supply assembled as a sealed unit. The diodes shall be arranged to display a full-hand symbol side by side with a full pedestrian symbol. The optical unit shall fit into a standard pedestrian signal housing so that it may be installed into an existing incandescent pedestrian signal. The LED optical unit shall be capable of maintaining message symbol integrity despite any partial loss of LEDs. The beam color shall match that of

the incandescent message: walking symbol - lunar white, hand - Portland orange. The beam pattern and intensity shall meet ITE specifications. The intensity may not degrade by more than 10% per annum. The optical unit shall be warranteed by the manufacturer for a period of 5 years.

Electrical Requirements:

- Input Voltage: 89 VAC to 135 VAC
- Wattage: 15 Watts
- Input Impedance at 60 Hz must satisfy all conflict monitor requirements.
- A regulated power supply shall be engineered to protect the LEDs from electrical surges and transient voltages.

D. Pedestrian Signal (Audible):

Voltage: 120VAC 50/60 Hz, ± 15 % of nominal line voltage
Operating current: 0.33 amps
Standby current: 0.027 amps
Decibel at 10': 88dB
Operating temperature: -31°F to 161°F
Net weight: 1.7 lb.
Rated life: 400 hours at 50% duty cycle

Housing:

Cast Aluminum
Weatherproof with neoprene gasket
4 7/8 inch square box
Depth 2 inches.
Tapped for 1/2 inch conduit
UL listed

Properties:

Single audible device with varying-tone module
Internal gain control - output adjustable from 64 dB to a maximum of 88 dB at 10 f
Active to coincide with "Walk" indication and pulse to coincide with "Don't Walk" indication

Tone Module:

470 Hz continuous
Sturdy plastic housing
Edge connector termination's to allow rapid installation or removal of module
Varying tones

E. Hardware: All exposed screws and fasteners shall be stainless steel. All internal screws, fasteners and metal parts shall be stainless steel, non-corrosible materials; or cadmium-plated ferrous materials.

F. Painting: All surfaces of the signal housing, door, and visors, inside and out, shall be finished with 3 coats of infrared-oven- baked paint before assembly. All brackets and hardware shall be painted dark green by the manufacturer. The color shall be No. 14056, Federal Standard No. 595.

First Coat: The primer shall be iron oxide baking primer and shall meet or exceed the requirements of FS TT-P-645.

Second Coat: Shall be light gray exterior baking enamel and shall meet the requirements of FS TT-E-489, No. 16251, No. 16314 or No. 16376 gray.

Third Coat: Shall be DARK GREEN exterior-baking enamel and shall meet the requirements of FS A-A2962. The color shall be No. 14056, Federal Standard No. 595. The inside of the visors shall be according to FS TT-P-527. The color shall be lusterless black Color No. 37038 to comply with Federal Standard No. 595.

M.16.08—Pedestrian Push Button: Pedestrian push buttons shall be of substantial tamper-proof construction and shall consist of a direct push type button with a single momentary contact switch in die-cast aluminum alloy housing to meet the minimum requirements of ASTM B85, on which shall be attached the push button advisory sign as shown on the plans.

The switch shall have snap action contacts, connected by a 3-bladed beryllium copper spring, and shall be rated at 10 amperes, 125 volts. Pedestrian push buttons shall not have any levers, handles, or toggle switches externally or internally, and shall operate on 12 volts AC. The pedestrian push button shall conform in size and the required force to activate the pedestrian signals as specified in the ADA, 14.2.6 Crossing Controls.

The assembly shall be weatherproof and so constructed that it will be impossible to receive any electrical shock under any weather conditions. Installations within the housing shall be so designed that they can be removed as units from the housing for the purpose of inspection and maintenance. Push-button contacts shall be entirely insulated from the housing and operating buttons. The contacts shall be normally open, and shall be closed only when the push buttons are operated by pressure, restoring immediately to the normal open position when the pressure is released. The back of the housing shall be flat type to provide a rigid installation. Cable entry shall be through the back so that all wiring may be concealed with no external conduit required, except when indicated on the plans.

Painting: All surfaces of the unit shall be finished with 3 coats of infrared oven-baked paint, before assembly.

First Coat: Primer, shall be epon oxide baking primer and shall meet and exceed1 performance specification of FS TT-P-664.

Second Coat: Gray Enamel, shall be lusterless and shall comply with FS TT-E-527.

Third Coat: Dark Green Enamel, shall be DARK GREEN exterior-baking enamel and shall meet the requirements of FS A-A 2962. The color shall be No. 14056, Federal Standard No. 595.

M.16.09—Controllers: The materials for this work shall meet the following requirements:

1. Pretimed Controller: It is the purpose of this section to set forth minimum design and operating requirements for the 3-dial pre-timed expansible type controller.

Constancy of Intervals: The length of any interval portion shall not change by more than 5% when the voltage of the power supply varies from 95 volts to 130 volts and the ambient temperature within the cabinet housing the controller varies from -30°F to 160°F. This performance shall be obtained without the use of any heater elements.

Power: The controller and all associated equipment shall be designed for use with 120-volt 60-cycle single-phase, alternating current.

Components: All components of the controller shall be designed for heavy duty. All motors, operating coils, bearings, contacts, relays, flashers, etc. shall be sufficiently large, rugged, and accessible to insure reliability and minimum maintenance. The dial contacts shall be of the leaf-spring type mounted on a molded bakelite block and equipped with terminal screws. The contact size shall be at least 1/4 inch diameter, 1/16-inch thick, and contain at least 90% pure silver. The contacts that handle any signal light load shall not require fine adjustment and readjustment for satisfactory and continuous operation. All such mechanisms shall be neatly and systematically arranged to make possible thorough inspection while the controller is operating in accordance with its normal functions.

Cabinet: Each controller assembly shall be completely wired and housed in a rigid metal cabinet with a suitable finish. The cabinet shall be of clean-cut design and appearance and shall be substantially constructed of aluminum. The pedestal mounted and pole mounted cabinets shall be approximately 15 inch × 19 1/4 inch × 35 inch size or the equivalent in volume, and the base mounted cabinet shall be approximately 17 inch 34 inch × 48 inch size or the equivalent volume. The cabinet shall have a main door which shall be equipped with a special CONN-1 lock and key and an auxiliary door which shall be equipped with a lock and a police key. Door hinge pins shall be of stainless steel material. Furnish 2 keys for each lock. When closed, both doors shall fit tightly to neoprene gasketing material. The cabinet shall contain a suitably designed vent for the purpose of releasing any explosive gases which may enter the cabinet. The "T" vent (1 1/2-inch diameter) shall be mounted on the top of the cabinet, with a screen vent at the bottom of the cabinet.

A panel behind the auxiliary door shall contain the following:

1. A switch to control the change from automatic control to flashing operation and vice-versa. In the flashing position, the control unit shall not be de-energized.
2. A switch to control the change from automatic control to manual operation and vice-versa. A manual cord not less than 6 feet long equipped with a rubber-covered hand switch, and permanently installed.
3. A "Main Switch" which will be used for signal shutdown. During signal shutdown, power to the controller shall be unaffected. The function of this switch can be incorporated into the flash switch by providing a three-position switch (NORMAL-OFF-FLASH).

The cabinet shall contain a means of mounting the controller, which will permit easy access to controller while in operation and accessibility to the components and terminals for maintenance and repair purposes. The cabinet shall be provided with necessary openings for mounting and connections and be wired to include a cable for time-clock installations with an octal 5-pin plug.

All panel wiring shall be neat and firm and the panel, which shall be mounted in the rear of the cabinet, shall mount the following:

1. Neutral Bus Bar, for neutral side of power supply line. This Bus Bar shall be rated for 30 amperes.
2. Terminals for conductors of signal light cable - 1 for each signal circuit and 1 or more terminals for the common conductors. This terminal bus shall be rated for 30 amperes. No pressure type terminal blocks will be accepted.
3. A 1-piece 120-volt convenience outlet and lamp receptacle. A heavy-duty 30-ampere screw-type fuse and socket shall be mounted on the left side of the controller cabinet. The controller equipment and terminals shall be so arranged within the cabinet that they will not prevent the entrance, training and connection of the incoming conductors. All field terminals shall be suitably identified.

The cabinet shall have a hole for cable entrance into the bottom of the cabinet of at least 3 inch diameter. There shall be an adapter for mounting the cabinet on a 4 inch pipe, provided with each cabinet.

The outgoing traffic control signal circuits shall be of the same polarity as the line side of the power supply. The grounded side of the power supply shall be grounded to the controller cabinet.

All conductors shall be provided with lugs for attachment to binding posts. Combining of conductors to 1 lug will not be allowed. Non-hygroscopic materials, having good insulating qualities, shall be used for all insulating purposes. All internal wiring to lamp circuits shall be #14 AWG stranded drawn copper. Wiring shall be cabled. The cable connection from the panel board to the controller shall be made by an "MS" type positive plug. A minimum of 19 signal circuits shall be provided each rated at a minimum of 15 amperes.

Leakage resistance between electrical connections or to ground shall not be less than 1 megohm. A minimum spacing of 1/2 inch shall be maintained between any terminal connection and any other similar connection or ground. The electrical connection from the controller to the outgoing and incoming circuits shall be made in such a manner that the controller may be replaced with a similar unit without the necessity of disconnecting and reconnecting the individual wires leading therefrom. Connector plugs shall be used that will contain sufficient contacts for proper functional operation of the controller. The controller to be supplied shall be wired complete for three 3-dial operation. All relay jack panels shall be of the female type.

Timer: The timer of the controller shall consist of a signal switching unit with a combination interval and offset timing dial unit securely mounted and of such construction that the entire timer assembly can be swung out from the cabinet for inspection or maintenance without breaking any electrical connections or interrupting the normal operation of the controller. The timer shall be connected electrically to the panel of the controller by means of an electrical jack or plug disconnect that will permit quick removal of the complete timer assembly as a single unit when necessary.

Timer Housing: The timer shall be enclosed in a rigid metal housing with a transparent window so that all timing settings, interval, offset and total cycle length control shall be visible but not accessible when the control box door is open.

Color Sequence: It shall be possible by effecting a change in the shape of the cams operating the signal circuit contacts to obtain any signal color sequence desired. A minimum of 19 signal circuits and 16 signal color intervals shall be provided on a single cam bar.

Period Timing: All signal color interval adjustments shall be quickly made, without tools, from the front of the timer on a dial clearly marked and calibrated in percent from 0% to 99%. Any signal color interval shall be adjustable to a minimum of 2% in steps of 1%. The timing dial shall give a visual indication of the relative length of each color period at all times. The duration of each signal color interval in percent of the total time cycle shall be as indicated on the percentage dial calibration. Each timing key shall be self-locking in the timing dial.

Timing Dial Units: The controller shall be furnished with 3 dial units each consisting of a synchronous motor-driven timing dial together with its dial contacts and other necessary parts and shall be provided with adequate mechanical mounting and electrical jacks so that it can be removed and/or replaced quickly without the use of tools in a single operation wherein both mounting and electrical connections are made simultaneously.

Motor: A drive coil shall drive the synchronous motor at the cycle length called for by the timing gear. There shall be provided a means of positively stopping the dial motor while the local controller is re-synchronizing with the master controller.

Each motor shall be of the self-starting synchronous type, such as Telechron heavy duty or disk type and shall have sufficient torque to permit its dial to operate synchronous at all outside temperatures between -30° F and 160° F. The transfer to an unused dial shall be possible within this same temperature range. The

motor torque shall be sufficient to operate the timer at synchronous speed when used with a 30-second cycle and shall be sufficient to overcome any tendency to slow up due to shocks or vibration, thereby providing synchronous operation between the limits of 95-130 volts.

Timing Dial: The timing dial shall support the timing keys over their entire length and shall contain 100 precision die-cast slots for this purpose. All timing keys for a given function shall be identical and each key having specialized function shall be appropriately colored to provide a visual indication of such function. All contacts operated by the various timing keys shall have helical springs controlling both movement and contact pressure so that flexible arm adjustments are eliminated.

It shall be possible to keep the timing dials, not controlling the sequence at any given time, running continuously. This will allow an offset relationship to be maintained between on interconnected (hard-wire) intersections.

Cycle Change: Each controller shall be furnished with a cycle gear as specified on the plans, installed in the timing dial unit and gears for 60-70-80-90-100 seconds shall be furnished and stored in the controller cabinet. The range of possible gear change shall be from 30 to 120 seconds in 5-second steps with gears other than those normally supplied with the controller. Gears within the 30- to 120-second range shall be available from the controller manufacturer. It shall be possible to change the total cycle on the street easily and quickly without the use of tools. The action of changing gears shall automatically indicate the total cycle value in use on a scale visible from the front of the timer. The mesh between the motor pinion and any time cycle gear shall be indexed automatically so as to result in proper mesh for free-running gearing and to avoid any possibility of binding which might affect synchronous operation of the driving motor.

Manual Offset: Due to the synchronous motor drive of the timing dial, it shall be possible to obtain coordination between respective dials at adjacent but non-interconnected intersections by means of a simple motor synchronizing switch mounted on the front of the dial unit where it will be tamper-proof by virtue of the timer housing herein before specified.

Automatic Offset: The timing dial unit shall be equipped with means by which it may be automatically resynchronized by remote control when connected to a master controller. The dial unit shall be furnished complete for triple offset. The controller shall be so wired that a choice of 3 different offsets on the dial may be remotely selected over a total of not more then 3 interconnecting offset conductors.

All offset adjustments shall be made without tools on the same timing dial as the signal color period adjustment but calibrated separately in steps of 1% from 0% to 99% in the reverse direction from the color period timing. Each offset key shall be clearly identifiable. It shall only be necessary to move 1 offset key to change an offset setting and each key shall be self-locking in the timing dial.

Signal Switching Unit: The signal switching unit shall provide wiring for a minimum of 19 signal light circuits. Each of the signal contacts shall open and close independently of each other so that the flexibility of their operation shall not be limited. The movable signal circuit contact arms shall be of hardened steel or other suitable rigid material so that they shall be wear-resistant and shall not flex, bend or take a permanent set in operation.

All make and break contact tips, whether movable or fixed, shall give contact faces not less than 5/16 inch diameter, 1/8 inch thick and shall be made of at least 90% pure silver and 10% cadmium oxide, by weight, or superior alternate material. The contacts shall be capable of breaking a current of 15 amperes, 120-volt, 60-cycle alternating current, one million (1,000,000) times without excessive pitting or burning when operated 120 times an hour. All signal contacts shall be readily accessible and easily inspected. The operation of moving or replacing movable contacts shall not change the contact pressure adjustment and shall be accomplished easily and quickly with the use of simple tools. The stationary contacts shall be replaced by the use of simple tools.

The ratchet motor shall be capable of effectively changing the position of the cams with equal efficiency up to and including the capacity of the camshaft.

Panel Board: A molded or pressed panel shall be provided on which all signal terminals (plus and common), relays, flashing mechanism and switches, flash combination terminals, terminals for field addition of manual switch accessory and terminals to allow all dial motors to run under non-interconnected operation shall be mounted.

Flasher: A Jack-mounted, unit type motor-driven or solid state flasher complete with radio interference suppressor shall be furnished to permit flashing operation of the signals when the timer is disconnected and removed from the controller. This shall be a two-circuit flasher rated at approximately 20 amperes.

Manual and Remote Switches in the Auxiliary Door: Manual switching flexibility shall be provided to permit the following operations:

Flashing Operation: The manual and remote flash switch or relays shall provide flashing indications for 4 independent circuits so that the controller may be used to control 2-phase vehicle movements and 4-phase vehicle movements (in doing so, power to dial motors shall be unaffected). In addition flash combination terminals shall allow the selection of flashing either yellow or red on the cross streets.

Manual Operation: The Auto-Hand switch shall provide means whereby manual timing of the signals may be obtained with a separate momentary contact hand switch. Operation of the timer by manual control shall provide the same color sequence as on automatic operation and no momentary undesirable indications shall show. Manual control shall be possible with the main door of the cabinet door closed.

Dial Selector Switch: The manual dial selector switch in the main cabinet shall provide for the selection of dials by:

1. Remote Control
2. Dial 1
3. Dial 2
4. Dial 3
5. Clock Position

Relay Wiring Provisions:

Jack-mounted relays shall be provided for:

1. Flashing 4-signal circuits
2. 2-dial transfer relays

Wires and Insulation: Except where soldered, all wires shall be provided with lugs or other approved terminal fittings for attachment to binding posts.

All wiring between various parts of the control box shall be a minimum of #18 AWG type TFF with 2/64—105 C. thermoplastic insulation or approved equivalent. Where cable wire must be clamped to the walls of the control box to prevent undue wear or flexing of the wires, such clamps shall be of suitable non-conducting material, such as rubber insulated metal straps.

The wiring from the timer jack connection to the terminal board shall be covered with a braided stocking or other method approved by the Engineer. The individual signal circuit wiring shall have a current carrying capacity of not less than 15 amperes.

Actuated Pedestrian Phase: The actuated pedestrian phase shall be obtained by effecting changes on the signal circuit cams and the dial drum in conjunction with auxiliary equipment which shall be external to the controller. Other than these changes, the controller shall be standard in all respects.

When an actuated pedestrian phase is called for, the pedestrian timing shall be taken out of 2 phases with no change in the cycle length.

The pedestrian clearance interval shall be flashing "DON'T WALK."

Painting: All outside surfaces of the cabinet and door shall be finished with 3 coats of infrared oven-baked paint before assembly.

First Coat: The primer shall be epon oxide baking primer and shall meet or exceed FS TT-P-636.

Second and Third Coats: The second and third coats will be aluminum paint meeting the requirements of FS TT-P-320, and Federal Test Method Standard 141. The color shall be No. 17178, Federal Standard No. 595.

Manuals: The contractor shall secure from the manufacturer a comprehensive service controller. Six (6) paper prints of the cabinet wiring diagram the entire field connection chart shall be left blank.

A comprehensive parts list, detailing all replaceable components as to manufacturer part number, and commercially available part number, and manufacturer's net price each, shall be provided. This list may be referenced from the drawings supplied with the equipment.

Guarantee: The Contractor shall secure from the manufacturer a guarantee for the equipment for a period of 12 months, which time shall commence from the date of delivery. If a unit is found to be defective during this 12-month period, it will be the responsibility of the contractor to assume the cost of shipping the controller to and from the factory, supplying parts and making repairs at no cost to the State of Connecticut.

M.16.10—Flasher Cabinet:

1. Cabinet: All equipment shall be housed within an aluminum or aluminum alloy weatherproof cabinet. If not cast aluminum, the seams shall be at least 0.125 inch thick and the seams shall be continuously welded. The cabinet shall be clean cut in design with a door, which shall encompass substantially the full area of the front of the cabinet. When closed, the door shall fit tightly to neoprene

gasket material. The door shall be equipped with a CONN-1 lock and shall have stainless steel hinge pins. Provide two keys with each cabinet. The cabinet shall have a screened "T" vent on top and a screened cut out of the bottom for ventilation. The size of the cabinet shall be the following dimensions:

Depth	Width	Height
12 - 14 inches	12 - 16 inches	14 - 25 inches

A panel shall be mounted in the back of the cabinet on which shall be mounted the following items:

(a) Circuit Breaker - 15 Amp
(b) Duplex Convenience Outlet
(c) Lamp Receptacle
(d) Terminal Block – Heavy-duty, screw type, to accommodate lugs for wire size #16 to #14 AWG
(e) Neutral Buss Bar - Rated for 30 Amps
(f) Radio interference filter - Rated for 30 Amps, 120 VAC, 60 HZ, and shall meet the standards of the UL and the Radio and Television Manufacturer's, Association
(g) Solid-State flasher mounting rack and socket

2. Flasher: The flasher shall be as specified in the latest NEMA Publication, part 8, with the following additions:

The flasher output shall be from optically isolated solid-state relays.
The solid-state relay shall switch off with an input between 3 and 32 volts D.C.
The flasher shall have an L.E.D. indicator connected to each output for visual reference.

M.16.11—Vacant

M.16.12—Loop Vehicle Detector and Sawcut: The materials for this work shall meet the following requirements:

1. Loop Vehicle Detector:

A. Functional Requirements: The loop detector shall be an electronic device, capable of detecting the presence of a moving or parked vehicle; and the detection shall be accomplished by the presence of a parked or moving vehicle over a wire loop embedded in the pavement. The detector unit shall be of the "self-tuning type." The detector shall be capable of tuning to an inductance range of 50-1000 micro Henries. It shall be possible to select a minimum of the following modes of operation, which shall function as follows.

(1) Mode 1 (Short Detection): The detector sensing unit shall detect a vehicle as slow as 1/10 mph entering the loop. If a vehicle stops over a portion of the loop such as waiting for a left turn, the remaining portion of the loop shall detect additional vehicles passing over the unoccupied portion of the loop. The time for the remaining portion of the loop to become capable of detecting additional vehicles shall be no longer than the minimum time it takes for the next vehicle to pass over the loop.

(2) Mode 2 (Long Detection): The detector sensing unit shall detect a vehicle as slow as 1/10 mph entering the loop. When a vehicle remains over the loop or a portion thereof, the detector sensing unit shall cause detection to persist up to at least 10 minutes. After this period any vehicle passing over the unoccupied portion of the loop shall be detected.

(3) Delayed Output Detection: The detector sensing unit shall provide digital type timing, with capabilities of 0-30 seconds delay. The sensing unit shall provide the delay except when the controller is in the green interval of the associated phase. During this green, the delay set into the detector will be omitted.

The sensing unit may also be used as a standard vehicle detector by leaving the selector switch in "off" position. A standard detector wire harness with a type "MS" plug is to be used.

B. Electrical Requirements: The detector sensing unit shall operate on 115 volts, 60 cycles A.C. and shall draw not more than 15 watts.

The unit shall contain an integral regulated power supply which will operate independent of line voltage variations between 100 and 135. The power supply shall be regulated by Zener reference and series regulation and shall be fused.

The detector shall operate properly at all temperatures between -30°F and +150°F. An automatic frequency control feature and automatic equalization feature shall be included in the detector to compensate for long-term drift due to environmental changes.

All transistors and integrated circuits shall be plug-in type for serviceability. The detector relay driver shall employ a silicon transistor.

C. Mechanical Requirements: The detector shall be housed in a durable finished fabricated sheet aluminum case. No special tools shall be required for removal of the cover. Removal of the cover shall provide access to the entire circuit and all components while the unit is connected and operating. The electrical connections of both the incoming and outgoing circuits shall be made by means of suitable multi-pin plug. The entire unit shall be replaced with a similar unit without the necessity of disconnecting the reconnecting individual wires leading therefrom. The plug receptacle shall be attached to one end of a connecting cable at least 48 inches long. The cable shall be color coded and each wire shall be fitted with a spade type lug for easy attachment to the controller terminal block.

A switch mounted on the front of the detector unit shall be provided for selecting the mode of operation to be in effect. Also mounted on the front of the detector will be an indicator light, which will register vehicle actuations.

When 2-channel loop amplifiers are used, each channel shall have a connecting plug with all wires in each of the harnesses connected so that it may be replaced with two single channel amplifiers.

The loop identification tag shall be 3 inches x 5 inches, 10 mil thick plastic.

2. Sawcut:

A. #14 AWG type THWN, or THHN stranded wire rated at 600 volts shall conform to the appropriate articles of the NEC. The wire shall be encased in flexible vinyl plastic or polyethylene tubing having a 3/16 inch ID, a minimum of 1/32 inch wall and a 1/4 inch OD. The tubing shall be capable of resisting deterioration from oils and solvents, have a smooth bore and be highly abrasion resistant. The loop detector wire shall conform in all respects to IMSA Specification.

B. The plastic compound shall be liquid with a synthetic resin base with a specific gravity of approximately 1.7. It shall have a minimum viscosity of 3500 centipoise second at 77°F and a maximum viscosity of 65,000 centipoise second at 25°F. The plastic compound shall be polymerized (cured) by the addition of a specific reaction initiator (hardener). Sufficient hardener (usually a liquid organic peroxide) shall be used to cure the plastic compound in approximately 30 minutes at 70°F. It shall be possible to cure the compound at temperatures below freezing. The plastic compound, after curing shall have sufficient strength and resiliency to withstand stresses set up by vibration and expansion and contraction due to temperature changes. The compound shall also be resistant to most chemicals and solvents, including most salts, acids and hydrocarbons.

The plastic compound shall meet the following testing requirements: ASTM D149, ASTM D495, ASTM D638, ASTM D1167, and ASTM D192.

A 1-part urethane elastomeric compound may be substituted for the plastic compound.

The elastomeric compound shall not require curing by the addition of a reaction initiator. The uncured elastomeric compound shall have a viscosity of 5,000 centipoise second to 35,000 centipoise second. The elastomeric compound shall be viscous enough not to flow out of a sawcut in a sloped road, but still fully encapsulate all wires in the sawcut. The elastomeric compound shall form a surface skin within 60 minutes at 75°F. The elastomeric compound shall be packaged in a standard liter caulking cartridge with the nozzle designed for insertion in a sawcut 0.25 to 0.125 inch wide and 1.5 to 2.5 inches deep. The compound shall also be available in 5-gal. pails to be used with a pneumatic pump application system. After hardening, the elastomeric compound shall remain flexible and exhibit no visible shrinkage. The elastomeric compound shall have sufficient strength and resiliency to withstand normal stresses such as: vibration, expansion and contraction; and to resist most chemicals and vehicular fluids such as: motor oil, gasoline, brake fluids, and antifreeze solution.

The elastomeric compound shall conform to the following testing requirements: ASTM D1875, ASTM D2834, ASTM D1640, ASTM D2340, and ASTM D412.

C. The flexible plastic conduit shall conform to the requirements of ASTM D2737.

M.16.13—Magnetic Vehicle Detector: The materials for this work shall conform to the following requirements:

1. The magnetic elements shall be housed in a nonferrous case, and they shall be waterproofed by the vacuum impregnation process. The detector shall have an internal resistance not to exceed 3500 ohms and shall be designed for operation with a magnetic detector solid state amplifier. One end of the housing shall be threaded to accommodate a conduit. The assembled detector shall not be rendered inoperative or continuously operated by parked cars or other fixed iron objects such as road reinforcement and water or gas pipes which may be within its zone of influence.

The single lane magnetic detector shall consist of two main assemblies, the detector coil unit and the

pavement box. The coil unit shall be assembled with 2 iron-covered coils connected in series and molded in a solid block of insulating material. The pavement box shall be a heavy duty aluminum casting with dimensions of approximately 9 inches x 8 inches x 6 inches. Flanges threaded for 1 1/4 inch conduit will be provided on 2 sides and the bottom. The box shall have a removable cover, fastened with stainless steel bolts.

The concrete that is used in the installation of the pavement box shall be a fast-setting high-strength type. The concrete shall be hard enough in 1 hour to allow traffic to pass over it. It shall be a type acceptable to the Engineer.

The proper operation of the magnetic detector shall not be affected by extreme temperature or humidity.

2. The amplifier unit shall be fully transistorized utilizing open type printed wiring panels and high-grade silicon transistors. The printed circuits shall be of extra heavy (1 ounce or better) copper.

All circuits shall be designed to provide stable operation within an ambient temperature range of -30°F to +180°F.

The magnetic detector amplifier shall be designed for operation on a nominal 120-volt 60-cycle, single-phase A.C. supply. Nominal power consumption shall not exceed 2 watts.

The unit shall be housed in a durable finished fabricated sheet aluminum case. Removal of the unit from its case shall require the use of simple tools. When the unit is removed from the case, it shall be possible to gain access to the printed circuit panel and components.

Electrical connections of both the incoming and outgoing circuits shall be made by means of a suitable plug.

The unit shall be replaceable with a similar unit without the necessity of disconnecting or reconnecting individual wires leading therefrom.

The plug shall be of protected male construction and rigidly fixed to the front of the unit.

The mating plug receptacle shall be attached to one end of a connecting cable at least 24 inches long. The other end of the connecting cable shall have color-coded leads, each of which shall be fitted with a spade lug for easy attachment to terminal blocks.

The sensitivity of the unit shall be dial adjustable, and continuous over the full range. On the front of the unit adjacent to the adjustment dial and connector plug there shall be mounted 2 jacks for the purpose of checking the proper adjustment of the sensitivity knob.

The registration of overlapping pulses from two detector amplifier units shall be ensured by the use of normally open and normally closed contacts provided on the output relay. Any contact that opens or closes in response to vehicle actuations shall be capable of making, breaking, and carrying 3 amperes at 120 volts A.C. The front panel of the detector amplifier unit shall contain a "Detection Indicator Lamp" that will pulse as each actuation is registered.

M.16.14—Control Cable: The materials for this work shall meet the following requirements:

1. General: All cable and wiring to be used on this Project shall conform to the appropriate articles of the NEC.

2. Conductors: Traffic control conductors shall be designed for 600 volts, AC and shall be of solid copper, having conductivity in accordance with IMSA and NEC standards.

The size of the conductors shall be as indicated on the plans.

3. Cable: Traffic control cable shall be multi-conductor and polyvinyl chloride or polyethylene sheathed. Insulation shall be polyethylene. Sheathed insulation and color coding shall conform in all respect to IMSA Specification No. 19-1 or No. 20-1, whichever is applicable. The Contractor shall furnish a manufacturer's warranty that the cable furnished is resistant to damage and deterioration by sustained contact with greases or oil.

Magnetic and loop detector lead-in cable shall be two conductor cable using stranded tinned copper conductors, color-coded polyethylene insulation, an aluminum backed mylar shield with a stranded tinned copper drain wire over the twisted pair, a black polyethylene outer jacket, and is listed under UL Style 2106. The loop lead-in cable shall conform to IMSA specification 50.2.

4. Number of Conductors: Sufficient signal light conductors shall be provided to perform the functional operation of the signal system. The number of conductors utilized shall be as indicated on the plans or as directed by the Engineer. Power supply cable between the sources of power and the controller shall consist of 600-volt, 2-conductor stranded wire of the size and type called for on the plans.

The number of conductors set forth in the plans is the minimum number that the Contractor shall supply. The Contractor may supply more than the specified number of conductors at its own expense.

5. Cable Closure: The cable closure shall be made of Neoprene construction. Internal brackets shall be aluminum and external parts shall be stainless steel or aluminum. The cable closure shall be provided with 4 entrances, each of which will accommodate cable with a diameter of 0.6 inch to 1 inch. The size of the closure shall be as shown on the typical plan sheet. The cable closure shall be provided with the minimum number of terminal blocks shown on the plans. Each terminal block shall be provided with 12 brass studs, nuts, and 24 washers. The block shall be a solid injection molded ABS (acrylonitrite butadiene styrene) that has a low moisture absorption rate. The terminal blocks shall be approximately 3 1/4 inches wide × 1 1/2 inches high × 1/2 inch thick and be provided with mounting studs.

M.16.15—Messenger and Span Wire: The materials for this work shall meet the following requirements:

1. Messenger wire shall be made of double-galvanized 7-strand utilities-grade steel wire cable, not less than 3/16 inch diameter, with at least a 2,400-lb. breaking strength.
2. Span wire:
   (a) "Span wire" shall be made of double-galvanized 7-strand utilities-grade steel wire cable, not less than 3/8 inch diameter, with at least an 11,200-lb. breaking strength.
   (b) "Span wire (high strength)" shall be made of double-galvanized 7-strand extra-high strength grade steel wire cable, not less than 7/16 inch diameter, with at least a 20,800-lb. breaking strength.
3. All hardware accessories shown on the plans to be used in span wire or messenger mounting shall be made of high-strength, double-galvanized, first-quality materials.

M.16.16—Vacant

M.16.17—Illuminated Signs: The sign shall be a blank-out type which shall display the type of message specified, through the use of light-transmitting fiber optic bundles.

The legend displayed shall be in accordance with the plans. Shop drawings of the front panel showing the lettering, and the type of visor shall be submitted for approval. The message shall be clearly legible under any lighting condition without the need for a visor or hood and attract attention at full intensity, anywhere within a 20-degree cone centered about the optical axis. The message shall be blank when not illuminated.

The non-lensed bundles shall be fully bifurcated for burnout protection with 8 lamps. The diameter of the fiber bundles shall be as shown on the plans. There shall be a separate fiber bundle, from each of the light sources, displayed in a row along the bottom of the sign to indicate lamp outage. All fiber optics, transformers and lamps shall be mounted on the hinged door of the unit. The blank-out sign assembly shall display the message in "Lunar White." Type "ENL" quartz-halogen lamps shall be used to illuminate the message. These lamps shall operate between 10.5 and 10.8 volts via the supplied step-down transformers. Average lamp life shall be not less than 8,000 hours. The lamps shall be mounted horizontally. "Overhead Illuminated 'Stop Ahead' Signs" shall have an 8-lamp minimum.

The electrical connection shall be provided by an internal barrier-type strip for connecting the electrical service wires. There shall be a 15 amp AC snap switch mounted in a utility box on the inside of the sign, enabling the sign to be shut off during maintenance. Power consumption for the message shall be less than 500 Watts.

A weatherproof housing of the dimensions specified on the plans shall be provided to enclose the fiber optic module assembly with bifurcated output fiber bundles, color filters, light sources and transformers. The sign housing frame shall be manufactured from extruded aluminum, 6061-T6, ASTM B221. This assembly shall be provided with a hinged access door. The hinge shall be stainless steel piano type hinge mounted on the left side of the door. All external hardware shall be stainless steel, internal hardware shall be corrosion resistant. The housing shall have a minimum of four (4) 1 inch diameter drainage holes. The entire front face of the sign shall be protected by a 1/8 inch thick sheet of clear polycarbonate mounted in the door frame. The housing shall be Federal Yellow according to Federal Standard 595 Color No. 13538 and the aluminum front panel shall be flat black according to Federal Standard 595 Color No. 37031 unless otherwise specified on the plans. The complete sign assembly shall not weigh more than 150 lb.

The legend displayed for an "Overhead Illuminated 'Stop Ahead' Sign" shall consist of letters 12 inches high and approximately 9 inches wide formed by fiber optic bundles spaced approximately 1.5 inches apart. The sign shall be supplied completely assembled and ready to be checked out.

SECTION M.17
ELASTOMERIC MATERIALS

M.17.01—Elastomeric Bearing Pads
M.17.02—Elastomeric Compression Seal

M.17.01—Elastomeric Bearing Pads: Elastomeric bearing pads shall be of the compound known as neoprene. All laminated bearings, and unlaminated bearings more than 1 inch thick, shall be individually cast in molds under pressure and heat and shall be of the size and grade indicated on the plans. Unlaminated bearings of 1 inch or less in thickness may be cut from sheet stock. All elastomeric bearing pads shall meet the requirements listed herein. Test specimens shall be in accordance with ASTM Method D15, Part B.

1. Elastomer:

(a) Physical Properties

Grade (Durometer)	50	60	70
Original Physical Properties Hardness ASTM D2240	50±5	60±5	70±5
Tensile strength, minimum psi ASTM D412	2,500	2,500	2,500
Elongation at break, minimum %	400	350	300
Accelerated Tests to Determine Long-Term Aging Characteristics Oven Aged-70 hrs./212°F ASTM D573			
Hardness, points change, maximum	+15	+15	+15
Tensile strength, % change, maximum	-15	-15	-15
Elongation at break, % change, maximum	-40	-40	-40
Ozone - 1PPM in Air by Volume - 20% Strain - 100± 2°F ASTM D1149* 100 Hours	No cracks	No cracks	No cracks
Compression Set-22 hrs./212°F ASTM D395 - Method B, % maximum ASTM D746 - Procedure B	35	35	35
Brittleness at -40°F	No failure	No failure	No failure

(b) Oil Swell Test: The volume of elastomer, when submerged in ASTM Oil No. 3 for 70 hours at 212°F shall not increase by more than 120% as tested by the procedure contained in ASTM D471.

2. Laminae:

(a) Laminae where so noted on the plans, shall be of the thickness, material and number noted. Steel laminae shall be blast cleaned before being cast integrally with the elastomer, and shall be ASTM A36 steel or an approved steel equivalent. Aluminum laminae shall be pickled before being cast integrally with the elastomer and shall meet the requirements of AA 6061-T6. Cut edges or perforations of the laminae shall be at least as smooth as ANSI 250 finish.

(b) The length and width of the laminae shall be 3/8 inch ± 1/16 inch less than the pad dimension. The thickness of the laminae shall be as shown on the plans +0.012 inch. The vertical dimensions for the position of the laminae shall be as shown on the plans ± 1/16 inch. The vertical dimension between the

bottom of the pad and any point on a particular laminae shall not vary by more than 1/16 inch. The laminae shall have a minimum 1/16 inch side cover after the pad is manufactured.

3. Fabricated Pads:

(a) The elastomeric bearing pads shall meet the following requirements:

1. The finish, flash and bond of laminated bearings shall meet the following requirements as noted in RMA, Rubber Handbook, latest edition.

Symbol	Requirements & Reference
F3	Finish—Table V
T.063	Flash—Table VI

B2 Grade 2, Method B Rubber to Metal Bonding, Tables VII and VIII

2. The dimensional tolerances of plain and laminated bearings shall be fabricated to the dimensions and configurations required by the plans and these specifications with the following permissible variations.

1. Overall Vertical Dimensions	
Average Total Thickness 1 1/4 inch or less	– 0, +1/8 inch
Average Total Thickness over 1 1/4 inch	– 0, +1/4 inch
2. Overall Horizontal Dimension	
36 inches and less	– 0, +1/4 inch
Over 36 inches	– 0, +1/2 inch
3. Thickness of Individual Layers of Elastomer (Laminated Bearings Only)	±1/8 inch
4. Variation from a Plane Parallel to the Theoretical Surface (as determined by measurements at the edges of the bearings)	
Top	1/8 inch
Sides	1/4 inch
Individual Non-Elastic Laminates	1/8 inch
5. Position of Exposed Connection Members	1/8 inch
6. Edge Cover of Embedded Laminates or Connection Members	– 0, +1/8 inch
7. Size of Holes, Slots or Inserts	± 1/8 inch
8. Position of Holes, Slots or Inserts	± 1/8 inch

Buffing, cutting, or any other attempt to alter the size of the pads, for the purpose of meeting the tolerances stated herein will not be permitted.

(b) The following values shall be met under laboratory testing conditions of full size bearings:

1. Compressive strain of any layer of an elastomeric bearing shall not exceed 7% at 800 psi average unit pressure, or at the design dead load plus live load pressure if so indicated on the plans.
2. The shear resistance of the bearing shall not exceed 50 psi for 50 durometer, 75 psi for 60 durometer or 110 psi for 70 durometer at 25% strain of the total effective rubber thickness after an extended 4-day ambient temperature of -20°F.

(c) A minimum of 30 days prior to the installation of the elastomeric bearing pads, the Contractor shall deliver to the Site the required number of pads for installation plus the required number of test pads. Pads shall be packed in containers holding not more than 10 pads. The pads in each container shall be of the same type, size and shall be for 1 structure only. The containers shall be plainly marked with the type and number of pads, the project number, the bridge number, the name of the manufacturer and the lot number. A lot of elastomeric bearing pads is defined as the number of pads manufactured from 1 batch of elastomer. A batch of elastomer is defined as the amount of elastomer prepared and compounded at 1 time. The Contractor shall furnish Certified Test Reports and Materials Certificates for each lot in accordance with the requirements of 1.06.07.

(d) For structures requiring less than 50 pads, 1 test pad shall be furnished. For structures requiring more than 50 pads, 1 extra test pad shall be furnished for each additional 50 pads or part thereof. If there are 2 or more types of pads in 1 structure, and only 1 test pad is required, the test pad will be furnished for the type of which there are the greater number. All test pads shall be furnished without charge.

(e) All of the pads on a structure shall be made by the same manufacturer.

(f) Pad Identification: Each pad shall have embossed on it the following: the word "Conn," the project number, the manufacturer's identification code, the pad type number, and the month and year of manufacture. The pad shall also have stenciled on it, with indelible ink the bridge number, the lot number and the pad number. The location of the identification shall be as shown on the plans.

4. Adhesive for Bonding:

(a) The adhesive for bonding the elastomeric bearing pads to steel or to the concrete surfaces where indicated on plans shall be an approved type and shall be a controlled setting adhesive made of such materials that it is compatible with the elastomeric pads, steel and concrete.

(b) The adhesive shall be a 2-component, contact, long-lasting, high bonding strength material and shall be an air curing adhesive meeting the following requirements:

Tensile Strength, min., psi	1800	ASTM D412
Elongation before breaking, min., %	750	ASTM D412
Brookfield Viscosity @ 77°F, #2 Spindle @ 10 rpm, pascal-seconds	2500 – 3500	ASTM D1084
Average Mass per gallon, pounds	7.60 ±5%	

The 90 degree peel-pull test is described in MILSPEC-R-15058G (ships) shall exceed 35 psi. If the adhesive which is applied to the bonded surfaces requires a primer, the primer shall be supplied by the manufacturer of the adhesive.

(c) The Contractor shall deliver the adhesive and primer (if primer is required) to the Site a minimum of 30 days prior to installation. Each separate container of adhesive and primer shall be clearly tagged or marked with the manufacturer's name, trade-mark and batch number. A batch is defined as that amount of adhesive that can be prepared and compounded at 1 time. The Contractor shall furnish Certified Test Reports and Materials Certificates for each batch in accordance with the requirements of 1.06.07.

M.17.02—Elastomeric Compression Seal: The material shall be a polychloroprene elastomer. It shall be resistant to heat, oil, and ozone, be resilient, and shall be compatible with concrete and steel. The elastomeric compression seal shall conform to the requirements of ASTM D3542.

The compression seal in each lot shall be plainly marked with the manufacturer's name, trademark and lot number. A lot shall be defined as that amount of compression seal extruded from one batch of elastomeric compound for each width indicated on the plans. A batch shall be defined as that amount of elastomeric compound prepared and compounded at 1 time. The Contractor shall furnish Certified Test Reports and Materials Certificates for each lot in accordance with the requirements of 1.06.07.

The Contractor shall deliver the compression seal to the Site a minimum of 15 days prior to installation. The compression seal shall be in a separate length or lengths for each joint as shown on the plans and each separate length shall be clearly marked for identification with the manufacturer's name, trade mark and lot number.

Prior to delivery of the compression seal the Contractor shall notify the Engineer of the date of shipment and the expected date of delivery. Upon delivery of the compression seal to the Site, the Contractor shall immediately notify the Engineer.

The Contractor shall furnish a 2-foot extra length of compression seal, identified by lot number, in each lot for purposes of inspection and testing by the Engineer. The Engineer may also inspect each separate piece of compression seal at each end to determine the conformance to the requirements stated herein.

The Engineer may reject any lot or portion of a lot that does not meet the requirements stated herein. A rejected lot or portion of a lot may be resubmitted provided the Contractor has removed or corrected, in a manner acceptable to the Engineer, all nonconforming material.

Lubricant-Adhesive: The lubricant-adhesive shall be a controlled setting adhesive conforming to 1 of the following systems:

- **System A:** The lubricant-adhesive shall be a 1-component polychloroprene compound containing only soluble phenolic resins blended together with anti-oxidants and acid acceptors in an aromatic hydrocarbon solvent mixture and shall have the following physical properties:
  1. Average net weight per gallon: 7.84 pounds ± 5%
  2. Solids Content: 24% min. by weight
  3. Brookfield Viscosity (77°F, #2 spindle at 10 RPM) or approved equal: 6500 - 7500 centipoise second
  4. The adhesive shall remain fluid from: 5°F to 120°F
  5. Film Strength (ASTM D412) 750% min. elongation before breaking: 2300 psi min. tensile strength
- **System B:** The lubricant-adhesive shall be a 1-part moisture curing polyurethane and hydrocarbon solvent mixture and shall have the following physical properties:
  1. Average net weight per gallon: 8.00 pounds ± 10%
  2. Solids Content: 72-74% by weight
  3. Adhesive to remain fluid from: 5°F to 120°F
  4. Film Strength (ASTM D412): 1200 psi
  5. Elongation: 350%

Only 1 system shall be used in a project.

The adhesive shall be stored at a temperature of 50°F to 80°F.

Any adhesive not used within 270 days of its manufacture shall be unacceptable.

Each batch of the lubricant-adhesive shall be delivered in containers plainly marked with the manufacturer's name or trade mark and batch number. A batch shall be defined as that amount of lubricant-adhesive compounded at1e time. The Contractor shall furnish Certified Test Reports and Materials Certificates for each batch in accordance with the requirements of 1.06.07.

SECTION M.18
SIGNING

M.18.01—Vacant
M.18.02—Anchor Bolts
M.18.03—Vacant
M.18.04—Vacant
M.18.05—Vacant
M.18.06—Vacant
M.18.07—Delineators
M.18.08—Paint for Sign Panel Overlay
M.18.09—Retroreflective Sheeting
M.18.10—Demountable Copy
M.18.11—Sign Panels-Extruded Aluminum
M.18.12—Panel Bolt Assemblies and Post Clip Assemblies
M.18.13—Sign Face—Sheet Aluminum
M.18.14—Metal Sign Posts
M.18.15—Sign Mounting Bolts
M.18.16—Data Labels

M.18.01—Vacant

M.18.02—Anchor Bolts: Anchor bolts shall meet the requirements of ASTM A449.

Leveling nuts and nuts for anchor bolt assemblies shall meet the requirements of ASTM A563, Grade DH. Leveling nuts and anchor bolt assemblies shall be hot-dip galvanized in accordance with the requirements of ASTM F2329. Leveling nuts shall be tapped oversize, after galvanizing, in accordance with ASTM A563, Section 7.5.1, and shall be provided with a lubricant in accordance with the requirements of ASTM F3125.

The Pedestal grout leveling template shall meet the requirements of ASTM A36 and shall be a minimum of 1/2 inch thick.

M.18.03—Vacant

M.18.04—Vacant

M.18.05—Vacant

M.18.06—Vacant

M.18.07—Delineators

1. Reflectors: Reflective sheeting shall meet the requirements of M.18.09 and be the type, color and shape as indicated on the plans. Backplate or sign blank material shall be an aluminum alloy of the type, shape and thickness as indicated on the plans.

2. Metal Delineator Posts:

The "Standard Metal Delineator Posts" shall be made of ASTM A36 structural steel. The posts shall be fabricated to the dimensions and weight shown on the plans. After delineator mounting holes have been established, the posts shall be galvanized in accordance with ASTM A123.

3. Bridge Rail Mounting Brackets:

The bracket shall be made of 0.125 inch Aluminum Alloy 6061-T6 fabricated to the dimensions shown on the plans and shall be fastened to the metal bridge rail with 2 each 3/8 inch diameter × 5/8 inch long cadmium plated steel box head self-tapping screws. Fasteners shall meet the requirements indicated on the plans.

M.18.08—Paint for Sign Panel Overlay: The paint to be used for the finished coat shall be an extremely durable, highest quality, semi-gloss green enamel for use on plywood and metal signs and shall be resistant to air, sun and water.

It shall consist of pigments of the required fineness and composition ground in the required vehicle by a suitable grinding machine to the required fineness. All pigments, resins, oils, thinners and driers used shall be of the best quality, free from adulterants of any kind, and shall comply with the following requirements:

Enamel Composition	Min.	Max.
Pigment, %	40	-
Vehicle, %	-	60
Volatile matter in vehicle, % by weight	-	55
Coarse particles and skins retained on No. 325 screen, based on pigment, %	-	0.5
Viscosity, Krebs units at 77°F	65	75
Weight per gallon, pounds	10.5	-
Fineness of grind (North Standard)	5	-

Pigment Composition	Min.	Max.
Chrome green, %	57	-
Extender pigment, %	-	43

The chrome green shall be Imperial A 4464 Velvet Green or approved equal.

The extender pigments shall consist of any of the following or combination thereof: magnesium silicate, barium sulfate, or diatomaceous silica. A ratio of 50% magnesium silicate and 50% diatomaceous silica has been found to produce the desired semi-gloss.

Vehicle—The vehicle shall contain not less than 45% solids by weight and shall be composed of a long oil soya modified alkyd resin solution or solutions, petroleum solvent thinners and driers. Rosin or rosin derivatives shall not be present. The alkyd resin solution or solutions shall conform to FS TT-R-266, Type I, Class A of latest issue.

Specular Gloss—The enamel shall be flowed on a tin panel and allowed to dry for 24 hours before measuring. The specular gloss at 60 degree angle of incident, ASTM D523 shall be between 35 and 45.

Setting and Drying Time—This enamel shall set to touch in less than 5 hours. It shall dry hard and tough in not more than 24 hours.

Flash Point—Not below 86°F as determined by ASTM D93.

Water Resistance—The enamel shall be flowed on a tin panel and allowed to dry for 48 hours. After being immersed for 18 hours in distilled water, it shall show no blistering or wrinkles upon removal and shall show no dulling or change in color after 2 hours recovery.

Skinning—This enamel shall not skin over within 48 hours in a 3/4 filled, closed container. Small amounts of anti-skinning agents, wetting agents, suspension agents, and anti-drier absorption agents may be added at the discretion of the manufacturer.

Working Properties—The enamel shall be well ground, shall not settle in the container, and shall be capable of being broken up with a paddle to a smooth uniform enamel of good brushing consistency, and shall have good flowing, covering and leveling properties.

M.18.09—Retroreflective Sheeting: The manufacturer and type of retroreflective sheeting materials shall be listed on the Department's Qualified Product List for the application intended.

M.18.10—Demountable Copy: The materials for this work shall meet the following:

1. **Vacant**
2. **Retroreflective Sheeting:**

Demountable cutout letters, digits, border, corner radii and copy accessories shall consist of adhesive coated retroreflective sheeting permanently adhered to flat aluminum backing. The retroreflective sheeting shall conform to M.18.09. The design of letters and accessories shall conform to FHWA Standards for use on “National System of Interstate and Defense” highways.

Aluminum backing shall be a minimum of 0.040 inch thick aluminum sheet of 3003-H14 alloy. Aluminum sheeting shall be properly treated according to sheeting manufacturer's specifications.

The demountable copy shall be fastened to the sign panel with aluminum rivets. Rivets shall be of the

pull through type and of the size and number designated by the demountable copy manufacturer.

3. Non-Reflective Plastic Sheeting:

Description: Demountable cutout letters, digits, border, corner radii and copy accessories shall consist of adhesive-coated, non-reflective plastic sheeting permanently adhered to flat aluminum backing.

The material shall consist of a flexible, pigmented, plastic film completely pre-coated with a solvent or heat-activated, tack-free adhesive. The adhesive shall be protected by a treated paper liner, which shall be removable without soaking in water or other solvents. The non-reflective plastic sheeting shall conform to the following:

Property Requirements:

A. Thickness: The thickness of the plastic film with adhesive shall be a minimum of 0.003 inch and a maximum of 0.004 inch.

B. Film: The unapplied or applied film shall be readily processed with, and insure adequate adhesion of, process inks recommended by the manufacturer.

(1) Flexibility: The material shall be sufficiently flexible to permit application over and conform to moderately contoured surfaces.

(2) Gloss: The film shall have an initial 60-degree gloss value of 35 (minimum), when tested in accordance with ASTM D523, measuring at least 3 portions of the film to obtain uniformity.

C. Adhesive: The pre-coated adhesive shall form a durable bond to smooth, clean, corrosion and weather-resistant surfaces, shall be of uniform thickness, non-corrosive to applied surfaces and shall have no staining affect on the film.

D. Adhesion: The material, applied according to Paragraph J "Preparation of Test Panels" shall have sufficient bond to prevent removal from the panel in 1 piece without the aid of a physical tool.

E. Exterior Exposure: The material shall withstand 3 years' vertical, south-facing exterior exposure at a site acceptable to the Engineer, showing no appreciable discoloration, cracking, crazing, blistering, delamination, or loss of adhesion. A slight amount of chalking is permissible. The film shall not support fungus growth.

F. Dimensional Stability: The material shall show no more than 0.02 inch shrinkage in any direction from edge of the panel when prepared in accordance with Paragraph J after being subjected to a temperature of 149°F for 48 hours.

G. Heat Resistance: The material, applied according to Paragraph J, shall be heat-resistant enough to retain adhesion after 1 week at 149°F.

H. Solvent and Chemical Resistance: The material, when prepared in accordance with Paragraph J, shall withstand immersion in the following liquids at 70-90°F, showing no appreciable decrease in adhesion, color or general appearance:

Liquids	Time/Hours
Reference Fuel (MIL-F-8799A) (15 parts xylol – 85 parts mineral spirits by weight)	1
Distilled Water	24
SAE #20 Motor Oil	24

I. Opacity: When applied, the material shall be sufficiently opaque to hide a contrasting black printed legend and white surface.

J. Preparation of Test Panels: Test panels shall be prepared using a 6.5 inch × 6.5 inch piece of the plastic film, applied to a clean 6.0 inch × 6.0 inch aluminum panel, premasked or as recommended by the manufacturer, trimmed evenly at the edge of the panel, and aged for 48 hours at 70 - 90°F.

K. Shelf-Life Storage: The material shall withstand 1 year's shelf life when stored in a clean area free from exposure to excessive heat, moisture and direct sunlight.

L. General Characteristics and Packaging: The plastic film shall be furnished in rolls, cut sheets or characters, as may be specified. The film, as supplied, shall be free from ragged edges, streaks, blisters, foreign matter or other surface imperfections which would make it unsuitable for the intended usage, and shall be readily cut with scissors, knife, blade, shears or other production tools. Complete and detailed instructions for mounting the plastic film shall be supplied with each package of material.

M. Quality Assurance: For the non-reflective plastic sheeting a Certified Test Report in accordance with 1.06.07 shall be submitted.

M.18.11—Sign Panels-Extruded Aluminum: Sign panels (extruded aluminum) shall be of the butt type, alloy 6063-T6 ASTM B221. Several extruded sections shall be joined with panel nuts, bolts, and washers to achieve the desired sign size. The extruded aluminum panels shall be of 6 inch and 12 inch heights to achieve sign panel vertical dimensions in increments of 6 inches; however, no more than one 6 inch panel shall be used on any sign. The weight and section properties of the 6 inch and 12 inch extruded panels shall be as indicated on the plans.

On the vertical axis (the 6 inch or 12 inch dimension), the panel face shall be in the same plane within 0.015 inch in any 6 inches. Extruded sections shall be mounted horizontally, and the panel faces shall be flush after the erection of the sign is complete.

Cleaning: Extruded aluminum sign panels shall be thoroughly cleaned and degreased by total immersion in an alkaline solution which is controlled and titrated to the solution manufacturer's recommendations. Immersion time shall be sufficient to completely remove all grease, dirt or other contaminants. After cleaning, the panels shall be thoroughly rinsed with clear running water.

Pretreatment: Sign panels shall be treated with a light, tightly adherent chromate conversion coating, free of any powdery residue, ranging in color from a silvery iridescent to a pale yellow, conforming with ASTM B449, Class 2, 10-35 mg/s.f., with 25 mg/s.f. as the optimum coating weight.

M.18.12—Panel Bolt Assemblies and Post Clip Assemblies:

Panel Bolt Assembly: Aluminum hex head bolt, hex nut and washer shall be as shown on the plans and shall be used to unite several panels sections to conform to the designed sign size. Nuts shall be drawn tight. Bolt holes may be drilled or blanked to finished size.

Thread fit for bolts shall conform to class 2-A fit of American Standard Association.

Post Clip Assembly: Aluminum post clips square head bolt, lock nut and washer shall be as shown on the plans.

The shank of the post clip bolts shall fit tightly against the sign support flange after nuts have been tightened. The clip bolts shall be torqued to 20 ft-lb. when using dry, clean, unlubricated threads.

M.18.13—Sign Face—Sheet Aluminum: Sheet aluminum sign blanks shall be constructed of sheet aluminum, alloy 6061 T6 or alloy 5052 H38. Sheet aluminum sign blanks shall meet the requirements of ASTM B209. They shall be degreased and etched in accordance with the recommendations of the sheeting manufacturer or treated with a light, tightly adherent chromate conversion coating, free of any powdery residue, ranging in color from silvery iridescent to a pale yellow, meeting the requirements of ASTM B449, Class 2 10-35 mg/s.f. with 25 mg/s.f. as the optimum coating. The thickness shall be as specified on the plans.

M.18.14—Metal Sign Posts: Metal sign posts, square tubular supports and parapet-mounted sign supports shall conform to the requirements as noted on the plans. The size, shape and mass of posts and supports shall be as specified in the plans.

After fabrication of the posts and supports, including hole punching or drilling, they shall be galvanized in accordance with ASTM A123 unless otherwise noted on the plans.

M.18.15—Sign-Mounting Bolts: Bolts used for sign-mounting shall be stainless steel and meet the requirements of ASTM F593, Group 1 or 2 (Alloy Types 304 or 316). Locking nuts shall be stainless steel and shall meet the requirements of ASTM F594, Group 1 or 2 (Alloy Types 304 or 316). Washers shall also be stainless steel and shall meet the requirements of ASTM A240 (Alloy Types 304 or 316).

M.18.16—Data Labels: Data Labels shall be 2 separate 5 inch × 3 inch, non-reflective weatherproof films with black legend on a yellow background having a pressure-sensitive adhesive backing.

A "Fabrication" data label shall include information about the sign fabricator, date of fabrication and the sheeting manufacture type. An "Installation" data label shall include the State Project Number or Maintenance Permit Number that installed the sign, and date of installation.

All legend ink must be durable and not fade, discolor, or smudge. All variable legends shall be included at label fabrication. Only one installed by permit number or project number shall be provided.

If the sign was fabricated or installed by state forces, insert "State."

The pre-coated pressure-sensitive adhesive, covered by a removable liner, must be removable at application without soaking in water or other solvents.

The adhesive shall form a durable bond to surfaces that are smooth, clean, corrosion free and weather resistant.

FABRICATION DATA LABEL

CONN DOT
SIGN FACE DATA LABEL

Fabricator: (Insert NAME or State)
Sheeting Manufacturer - Type
(Insert NAME - TYPE)

Date Fabricated - Month / Year

J	F	M	A	M	J	J	A	S	O	N	D
12	13	14	15	16	17	18	19	20	21	22	23

INSTALLATION DATA LABEL

CONN DOT
SIGN FACE DATA LABEL

Installed By:
Project No.: (Insert 000-000 or State)
Permit No.: (Insert D_-000000)

Date Installed - Month / Year

J	F	M	A	M	J	J	A	S	O	N	D
12	13	14	15	16	17	18	19	20	21	22	23

Completed Data Labels shall not discolor, crack, craze, blister, delaminate, peel, chalk, or lose adhesion when subjected to temperatures from -30° to 200°F.

NOTES

LIST OF STANDARD PAY ITEMS
FORM 818

This list has been prepared for the convenience of those using these Standard Specifications, for the sole express purpose of locating quickly the information contained herein; and no claims shall arise due to omissions, additions, deletions, etc.

Section Reference	Pay Item	Pay Unit
2.01	Clearing and Grubbing	l.s.
2.02	Rock Excavation (No Explosives)	c.y.
2.02	Earth Excavation	c.y.
2.02	Rock Excavation	c.y.
2.02	Channel Excavation — Earth	c.y.
2.02	Channel Excavation — Rock	c.y.
2.02	Cut Concrete Pavement	l.f.
2.02	Cut Bituminous Concrete Pavement	l.f.
2.02	Removal of Concrete Pavement	s.y.
2.02	Removal of Concrete Sidewalk	s.y.
2.02	Unsuitable Material Excavation	c.y.
2.03	Structure Excavation— Earth (Complete)	c.y.
2.03	Structure Excavation— Rock (Complete)	c.y.
2.03	Structure Excavation— Earth (Excluding Cofferdam and Dewatering)	c.y.
2.03	Structure Excavation—Rock (Excluding Cofferdam and Dewatering)	c.y.
2.04	Cofferdam and Dewatering	l.f.
2.04	Cofferdam Material Left in Place	l.f.
2.06	Ditch Excavation	c.y.
2.06	Rock in Ditch Excavation	c.y.
2.07	Borrow	c.y.
2.08	Free-Draining Material	c.y.
2.09	Formation of Subgrade	s.y.
2.11	Anti-Tracking Pad	s.y.
2.12	Subbase	c.y.

Section Reference	Pay Item	Pay Unit
2.13	Granular Fill	c.y.
2.14	Compacted Granular Fill	c.y.
2.16	Pervious Structure Backfill	c.y.
2.19	Sedimentation Control System	l.f.
2.19	Sedimentation Control (Type) System	l.f.
2.86	Rock in Drainage Trench Excavation ( ) Deep	c.y.
3.04	Processed Aggregate Base	c.y.
3.05	Processed Aggregate	ton
4.01	Concrete for Pavement	c.y.
4.01	Mat Reinforcement for Concrete Pavement	s.y.
4.01	Transverse Expansion Joints	l.f.
4.01	Transverse Contraction Joints	l.f.
4.01	Longitudinal Joint Support	ea.
4.01	(Type and Thickness) Expansion Joint Filler	l.f.
4.06	HMA S*	ton
4.06	PMA S*	ton
4.06	Bituminous Concrete Adjustment Cost	est.
4.06	Material for Tack Coat	gal.
4.06	Material Transfer Vehicle	ton
4.09	Fine Milling of Bituminous Concrete (0” to 4”)	s.y.
4.09	Micro-Milling of Bituminous Concrete (0” to 3”)	s.y.
4.09	Standard Milling of Bituminous Concrete (Greater Than 4” up to 8”)	s.y.
4.09	Standard Milling of Bituminous Concrete (Greater Than 8”)	s.y.
4.09	Removal of Existing Wearing Surface	s.y.
4.15	Pressure Relief Joint	s.y.
5.03	Removal of Superstructure	l.s.
5.04	Railroad Protection	est.

Section Reference	Pay Item	Pay Unit
5.06	Concrete for Steps and Copings	c.y.
5.08	Shear Connectors	ea.
5.09	Welded Studs	ea.
5.13	(Size) Polyvinyl Chloride Plastic Pipe	l.f.
5.14	Prestressed Beams — Pretensioned, Type ( )	l.f.
5.14	Prestressed Deck Units (Size)	l.f.
5.21	Elastomeric Bearing Pads	c.i.
5.22	(Size) Elastomeric Compression Seal	l.f.
5.86	(Type) Catch Basin - ( ) Deep	ea.
5.86	Manhole (Size) - ( ) Deep	ea.
5.86	(Type) Drop Inlet	ea.
5.86	Reset (Type) Catch Basin	ea.
5.86	Reset Manhole (Type)	ea.
5.86	Reset (Type) Drop Inlet	ea.
5.86	Convert Catch Basin to (Type) Catch Basin	ea.
5.86	Convert Catch Basin to (Type) Manhole	ea.
5.86	Convert Manhole to (Type) Catch Basin	ea.
5.86	Manhole Frame and Cover	ea.
5.86	(Type) Catch Basin Top	ea.
5.86	Remove Drainage Structure - ( ) Deep	ea.
5.86	Abandon Drainage Structure	ea.
6.01	Footing Concrete	c.y.
6.01	Footing Concrete (Mass)	c.y.
6.01	Abutment and Wall Concrete	c.y.
6.01	Abutment and Wall Concrete (Mass)	c.y.
6.01	Column and Cap Concrete	c.y.
6.01	Column and Cap Concrete (Mass)	c.y.

Section Reference	Pay Item	Pay Unit
6.01	Bridge Deck Concrete	c.y.
6.01	Bridge Deck Concrete (SIP Forms)	c.y.
6.01	Parapet Concrete	l.f.
6.01	Bridge Sidewalk Concrete	c.y.
6.01	Approach Slab Concrete	c.y.
6.01	Barrier Wall Concrete	c.y.
6.01	Underwater Concrete	c.y.
6.01	Surface Repair Concrete	c.f.
6.01	Structural Repair Concrete	c.f.
6.01	Class PCCXXXYZ	c.y.
6.01	(Thickness and Type) Joint Filler for Bridges	s.f.
6.01	(Thickness) Closed Cell Elastomer	c.i.
6.02	Deformed Steel Bars	lb.
6.02	Deformed Steel Bars - Epoxy Coated	lb.
6.02	Deformed Steel Bars - Galvanized	lb.
6.02	Deformed Steel Bars - Weldable	lb.
6.02	Welded Wire Fabric	lb.
6.02	Dowel Bar Splicer System	ea.
6.02	Dowel Bar Splicer System - Epoxy Coated	ea.
6.02	Dowel Bar Splicer System - Galvanized	ea.
6.03	Structural Steel (Site No.)	l.s.
6.03	Structural Steel	cwt.
6.05	Dimension Stone Masonry	s.y.
6.05	Ashlar Stone Masonry	s.y.
6.05	Pointing (Kind)	s.y.
6.06	Cement Rubble Masonry	c.y.
6.07	Dry Rubble Masonry	c.y.

Section Reference	Pay Item	Pay Unit
6.09	Repointed Masonry	s.y.
6.11	Shotcrete	bag
6.12	Concrete Cylinder Curing Box	ea.
6.53	Clean Existing Catch Basin	ea.
6.53	Clean Existing Manhole	ea.
6.53	Clean Existing Drop Inlet	ea.
6.53	Clean Existing Culvert — 12” to 42” Diameter	l.f.
6.53	Clean Existing Culvert — Greater than 42” Diameter	l.f.
6.86	(Size and Type) Pipe (Thickness) - ( ) Deep	l.f.
6.86	(Size and Type) Pipe-Arch (Thickness) - ( ) Deep	l.f.
6.86	Relaid (Size and Type) Pipe - ( ) Deep	l.f.
6.86	(Size and Type) Relaid Pipe-Arch - ( ) Deep	l.f.
6.86	(Size) Reinforced Concrete Drainage Pipe End	ea.
6.86	(Size) Metal Drainage Pipe End	ea.
6.86	(Size and Type) Corrugated Metal Pipe Elbow	l.f.
6.86	Concrete Pipe Connection	ea.
6.86	Remove Existing Pipe - ( ) Deep	l.f.
7.01	Furnishing Drilled Shaft Drilling Equipment	l.s.
7.01	Drilled Shaft (Diameter)	l.f.
7.01	Drilled Shaft Earth Excavation (Diameter)	l.f.
7.01	Drilled Shaft Rock Excavation (Diameter)	l.f.
7.01	Obstructions	hr.
7.01	Trial Drilled Shaft (Diameter)	l.f.
7.01	Exploration Test Boring	l.f.
7.01	Permanent Casing (Diameter)	l.f.
7.01	Access Tubes	l.f.
7.02	Furnishing Steel Piles	lb.

Section Reference	Pay Item	Pay Unit
7.02	Furnishing (Type) Piles (Length)	l.f.
7.02	Driving (Type) Piles	l.f.
7.02	Test Pile (Type-Length)	ea.
7.02	Splicing (Type) Piles	ea.
7.02	Point Reinforcement for (Type) Piles	ea.
7.02	Pile Loading Test	ea.
7.02	Dynamic Pile Driving Analysis (PDA) Test	ea.
7.02	Pre-Augering of Piles	l.f.
7.03	(Type) Riprap	c.y.
7.04	Gabions	c.y.
7.04	Polyvinyl Chloride Coated Gabions	c.y.
7.05	Slope Paving	s.y.
7.06	Micropiles	ea.
7.06	Verification Test for Micropiles	ea.
7.06	Proof Test for Micropiles	ea.
7.06	Micropile Length Adjustment	l.f.
7.07	Membrane Waterproofing (Woven Glass Fabric)	s.y.
7.08	Dampproofing	s.y.
7.13	Permanent Steel Sheet Piling	s.f.
7.14	Temporary Sheet Piling	s.f.
7.15	Sheet Piling Material Left in Place	s.f.
7.16	Temporary Earth Retaining System	s.f.
7.17	Earth Retaining System Left in Place	s.f.
7.28	Crushed Stone for Slope Protection	ton
7.32	Concrete Block Slope Protection	s.y.
7.51	(Size) Underdrain	l.f.
7.51	(Size) Foundation Underdrain	l.f.

Section Reference	Pay Item	Pay Unit
7.51	(Size) Slope Underdrain	l.f.
7.51	(Size) Structure Underdrain	l.f.
7.51	(Size) Outlet for Underdrain	l.f.
7.51	(Size-Type) Underdrain	l.f.
7.51	(Size-Type) Foundation Underdrain	l.f.
7.51	(Size-Type) Slope Underdrain	l.f.
7.51	(Size-Type) Structure Underdrain	l.f.
7.51	(Size-Type) Outlet for Underdrain	l.f.
7.55	Geotextile	s.y.
7.55	Geotextile (Type)	s.y.
8.03	Paved Ditch	s.y.
8.03	Paved Apron	s.y.
8.03	Paved Channel	s.y.
8.11	Concrete Curbing (Type)	l.f.
8.13	Stone Curbing (Type-Size)	l.f.
8.13	Curved Stone Curbing	l.f.
8.14	Reset Curbing (Type-Size)	l.f.
8.15	Bituminous Concrete (Type) Curbing	l.f.
8.18	Protective Compound for Bridges	s.y.
8.21	(Shape) Precast Concrete Barrier Curb (Size)	l.f.
8.22	Temporary Precast Concrete Barrier Curb	l.f.
8.22	Relocated Temporary Precast Concrete Barrier Curb	l.f.
9.04	Metal Bridge Rail (Type)	l.f.
9.05	Stone Wall Fence	l.f.
9.06	Wire Fence with Wood Posts	l.f.
9.06	Wire Fence with Metal Posts	l.f.
9.10	Metal Beam Rail (Type)	l.f.

Section Reference	Pay Item	Pay Unit
9.10	Metal Beam Rail Span Section (Type II or III)	ea.
9.10	(Type) Attachment	ea.
9.10	Convert Metal Beam Rail (Type) to (Type)	l.f.
9.10	(Type) Curved Guide Rail Treatment	ea.
9.10	R-B Terminal Section	ea.
9.11	(Type) End Anchorage Type I	ea.
9.11	(Type) End Anchorage Type II	ea.
9.11	Anchor in (Type) Slope	ea.
9.12	Reset (Type) Cable Guide Rail (Type Post)	l.f.
9.12	Reset Metal Beam Rail (Type) (System)	l.f.
9.12	Reset Single Post	ea.
9.12	Reset Rail Anchorage	ea.
9.12	Remove Single Post	ea.
9.12	Remove Cable Guide Rail	l.f.
9.12	Remove Metal Beam Rail	l.f.
9.13	(Height) Chain Link Fence	l.f.
9.13	(Height) Polyvinyl Chloride Chain Link Fence	l.f.
9.13	(Type) (Size) Chain Link Gate	ea.
9.13	(Type) (Size) Polyvinyl Chloride Chain Link Gate	ea.
9.13	Remove Chain Link Fence	l.f.
9.14	Metal Handrail	l.f.
9.15	Tree Root Protection	c.y.
9.18	Three-Cable Guide Railing (I-Beam Posts)	l.f.
9.18	End Anchorage (Type)	ea.
9.21	Concrete Sidewalk	s.f.
9.21	Concrete Sidewalk Ramp	s.f.
9.21	Detectable Warning Strip	ea.

Section Reference	Pay Item	Pay Unit
9.21	Retrofit Detectable Warning Strip	ea.
9.22	Bituminous Concrete Sidewalk	s.y.
9.22	Bituminous Concrete Driveway	s.y.
9.22	Bituminous Concrete Driveway (Commercial)	s.y.
9.23	Bituminous Concrete for Patching	ton
9.24	Concrete Driveway Ramp	c.y.
9.30	Object Marker (Type)	ea.
9.39	Sweeping for Dust Control	hr.
9.42	Calcium Chloride for Dust Control	ton
9.43	Water for Dust Control	m. gal.
9.44	Furnishing and Placing Topsoil	s.y.
9.46	Liming	ton
9.49	(Plant Name) (Caliper)	ea.
9.49	(Plant Name) (Height)	ea.
9.49	(Plant Name) (Size)	ea.
9.49	(Vine Name) (Size)	ea.
9.49	Vines (Name)	ea.
9.49	Groundcover Name (Size)	ea.
9.49	Wood Chip Mulch	s.y.
9.49	Gravel Mulch	s.y.
9.50	Turf Establishment	s.y.
9.50	Erosion Control Matting (Type)	s.y.
9.51	Rock Excavation for Planting	c.y.
9.52	Selective Clearing and Thinning	l.s.
9.53	Sodding	s.y.
9.70	Trafficperson (Municipal Police Officer)	est.
9.70	Trafficperson (Uniformed Flagger)	hr.

Section Reference	Pay Item	Pay Unit
9.71	Maintenance and Protection of Traffic	l.s.
9.74	Removal of Existing Masonry	c.y.
9.75	Mobilization and Project Closeout	l.s.
9.76	Barricade Warning Lights (Type)	day
9.77	Traffic Cone	ea.
9.78	Traffic Drum	ea.
9.79	Construction Barricade (Type)	ea.
9.80	Construction Surveying	l.s.
9.81	42 Inch Traffic Cone	ea.
9.99	Disposal of Buildings	l.s.
10.01	Trenching and Backfilling	l.f.
10.01	Rock in Trench Excavation (…Deep)	c.y.
10.02	Light Standard Foundation (Type)	ea.
10.02	Traffic Control Foundation (Type)	ea.
10.02	Rock in Foundation Excavation	v.f.
10.03	Light Standard (Type-Size-Kind)	ea.
10.04	Roadway Luminaire (Type-Size)	ea.
10.06	Underbridge Luminaire (Kind-Type-Size)	ea.
10.08	(Size) (Kind) Conduit (Type)	l.f.
10.09	Cast Iron Junction Box (Size)	ea.
10.10	Concrete Handhole	ea.
10.10	Concrete Handhole Type II	ea.
10.10	Concrete Handhole Type II Bank Adapter	ea.
10.10	Cast Iron Handhole Cover	ea.
10.10	Concrete Handhole (Type)	ea.
10.11	4 Inch Drain Pipe	l.f.
10.12	Single Conductor (Size)	l.f.

Section Reference	Pay Item	Pay Unit
10.14	Cable in Duct (No. — Size)	l.f.
10.15	Bare Copper Grounding Conductor (Size)	l.f.
10.17	Service Entrance and Cabinet (Type)	ea.
10.18	Navigation Light	ea.
11.01	Pole Anchor	ea.
11.02	Pedestal (Type and Size)	ea.
11.03	(Type) Steel Span Pole	ea.
11.03	(Type) Steel Combination Span Pole (Bracket Type)	ea.
11.03	Wood Span Pole	ea.
11.05	Traffic Signal (Type)	ea.
11.06	Pedestrian Signal (Type)	ea.
11.07	Pedestrian Push Button and Sign (Type)	ea.
11.08	Pre-timed Controller (Type)	ea.
11.08	Actuated Controller (Type)	ea.
11.11	Loop Vehicle Detector (Type)	ea.
11.11	Loop Detector Saw Cut	l.f.
11.12	Magnetic Vehicle Detector (Type)	ea.
11.13	(No.) Conductor # ( ) (Type)	l.f.
11.13	Cable Closure	ea.
11.14	Messenger Wire	l.f.
11.14	Span Wire (Type)	l.f.
11.16	Overhead Illuminated "Stop Ahead" Sign	ea.
11.16	Internally Illuminated Sign	ea.
11.17	Alternate Flashing Signals for Warning Signs	ea.
11.18	Removal and/or Relocation of Traffic Signal Equipment	l.s.
11.30	Arrow Board	day
11.31	Remote Control Changeable Message Sign	day

Section Reference	Pay Item	Pay Unit
12.01	Overhead Truss Sign Support (Type)	ea.
12.01	Overhead Cantilever Sign Support (Type)	ea.
12.02	Overhead Truss Sign Support Foundation (Type)	ea.
Section Reference	**Pay Item**	**Pay Unit**
12.02	Overhead Cantilever Sign Support Foundation (Type)	ea.
12.03	Side Mounted Sign Foundation	ea.
12.04	Sign Panel Overlay—Plain	s.f.
12.04	Sign Panel Overlay—with Copy	s.f.
12.05	Type DE-( ) Delineator	ea.
12.06	Removal of Existing Signing	l.s.
12.06	Removal of Existing Overhead Signing	l.s.
12.07	Sign Face—Extruded Aluminum (Type)	s.f.
12.08	Sign Face—Sheet Aluminum (Type)	s.f.
12.09	Painted Legend, Arrows and Markings	s.f.
12.09	Painted Pavement Markings (Width) (Color)	l.f.
12.09	Hot-Applied Painted Pavement Markings (Width) (Color)	l.f.
12.10	(Width) (Color) Epoxy Resin Pavement Markings	l.f.
12.10	Epoxy Resin Pavement Markings, Symbols and Legends	s.f.
12.11	Removal of Pavement Markings (Type)	s.f.
12.12	Temporary Plastic Pavement Marking Tape (Width) (Color)	l.f.
12.14	(Width) Preformed Black Line Mask Pavement Marking Tape	l.f.
12.20	Construction Signs	s.f.
18.02	Permanent Sand Barrel (Weight)	ea.
18.02	Temporary Sand Barrel (Weight)	ea.
18.02	Relocation of Temporary Sand Barrel (Weight)	ea.
18.03	Impact Attenuation System (Type)	ea.
18.03	Temporary Impact Attenuation System (Type)	ea.
18.03	Repair of Impact Attenuation System (Type)	est.
18.06	Truck-Mounted or Trailer-Mounted Impact-Attenuator	hr.

METRIC TO ENGLISH
APPROXIMATE CONVERSION FACTORS

SYMBOL	WHEN YOU KNOW	MULTIPLY BY	TO FIND	SYMBOL
		LENGTH		
mm	millimeters	0.039	inches	in
m	meters	3.28	feet	ft
m	meters	1.09	yards	yd
km	kilometers	0.621	miles	mi
		AREA		
mm^2	square millimeters	0.0016	square inches	in^2
m^2	square meters	10.764	square feet	ft^2
m^2	square meters	1.195	square yard	yd^2
km^2	square kilometers	0.386	square miles	mi^2
ha	hectares (10,000 m^2)	2.47	acres	ac
		MASS		
g	grams	0.035	ounces	oz
kg	kilograms	2.202	pounds	lb
Mg (t)	megagrams (1000 kg) (metric ton)	1.103	short tons (2000 lb)	T
		VOLUME		
ML	milliliters	0.034	fluid ounces	fl oz
L	liters	0.264	gallons	gal
m^3	cubic meters	35.314	cubic feet	ft^3
m^3	cubic meters	1.307	cubic yards	yd^3
		TEMPERATURE (exact)		
ºC	Celsius temperature	9/5 (then add 32)	Fahrenheit temperature	ºF
		ILLUMINATION		
lx	lux	0.0929	foot-candles	fc
cd/m^2	candela/m^2	0.2919	foot-Lamberts	fl
		FORCE and PRESSURE or STRESS		
N	newtons	0.225	pound force	lbf
kPa	kilopascal	0.145	pound force per square inch	lbf/in^2

ENGLISH TO METRIC
APPROXIMATE CONVERSION FACTORS

SYMBOL	WHEN YOU KNOW	MULTIPLY BY	TO FIND	SYMBOL
		LENGTH		
in	inches	25.4	millimeters	mm
ft	feet	0.305	meters	m
yd	yards	0.914	meters	m
mi	miles	1.61	kilometers	km
		AREA		
in^2	square inches	645.2	square millimeters	mm^2
ft^2	square feet	0.093	square meters	m^2
yd^2	square yards	0.836	square meters	m^2
mi^2	square miles	2.59	square kilometers	km^2
ac	acres	0.405	hectares	ha
		MASS		
oz	ounces	28.35	grams	g
lb	pounds	0.454	kilograms	kg
T	short tons (2000 lb.)	0.907	megagrams (metric ton)	Mg (t)
		VOLUME		
fl oz	fluid ounces	29.57	milliliters	mL
gal	gallons	3.785	liters	L
ft^3	cubic feet	0.028	cubic meters	m^3
yd^3	cubic yards	0.765	cubic meters	m^3
		TEMPERATURE (exact)		
ºF	Fahrenheit temperature	5/9 (after subtracting 32)	Celsius temperature	ºC
		ILLUMINATION		
fc	foot-candles	10.76	lux	lx
fl	foot-Lamberts	3.426	candela/m^2	cd/m^2
		FORCE and PRESSURE or STRESS		
lbf	pound force	4.45	newtons	N
lbf/in^2	pound force per square inch	6.8	kilopascals	kPa