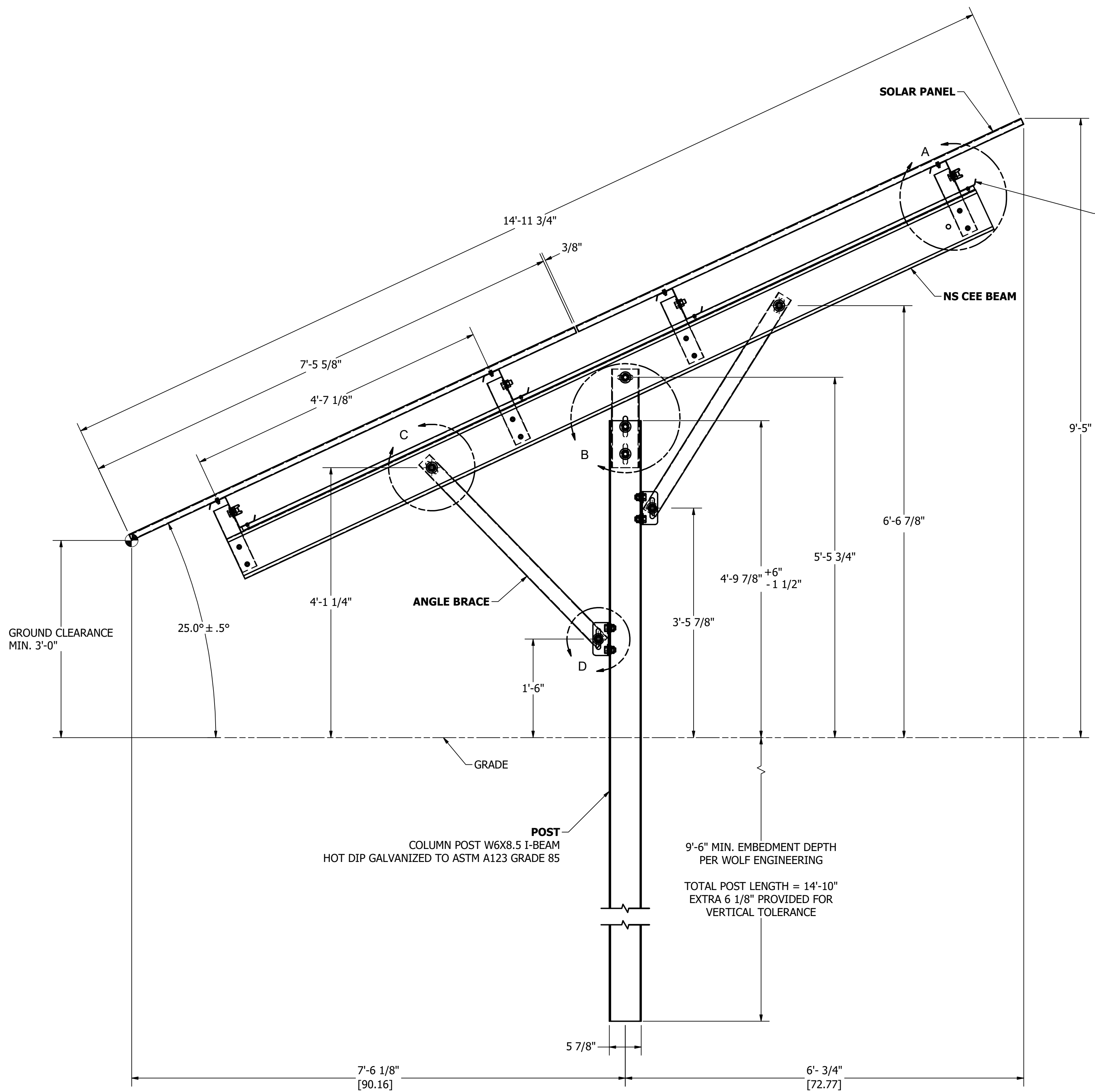


**APPENDIX- A  
STRUCTURAL DETAIL DRAWING**

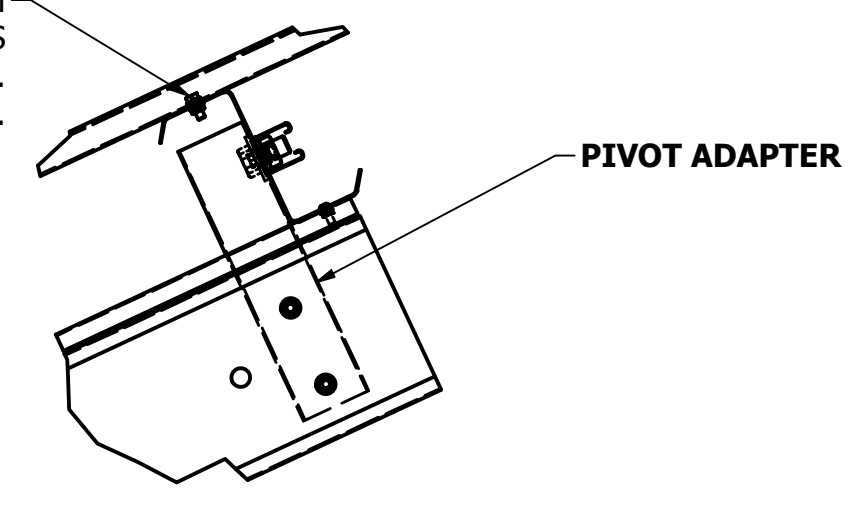
PROJECT INFORMATION	
INSTALLATION ADDRESS: 54 South St, Morris, CT 06763	
Structural General Notes	
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2. If existing conditions make it necessary to revise structural details, consult DCE Solar before proceeding with any change.	
3. These drawings and notes are for this specific project and no other use is authorized.	
4. Structure designed in accordance with the International Building Code, 2021 Edition, ASCE 7-16, AISC 360-16 (14th Edition), and AISI S100-16: ASD	
Snow Loads: -Ground Snow Load $p_g = 35$ psf -Importance Factor $I_s = 0.8$ -Exposure Factor $C_e = 0.9$ -Slope Snow Load $p_s = 17.32$ psf	
Wind Loads: -MRI Factor = 1.00 -Basic Wind Speed $V = 110$ mph - $I_w = 1$ -Exposure = C -Wind Design performed in accordance with the requirements of ASCE - Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.	
Seismic Loads: - $SS = 0.183g$ , $S1 = 0.054g$ -Site Class = D - $SDS = 0.200g$ , $SD1 = 0.090g$ -Seismic Design Category = B -Ordinary Steel Cantilever Column System	
5. Material strengths: -Hot-rolled structural steel ASTM A992 GR50. -Cold Formed Steel Sections comply w/ASTM A1003, structural grade, galvanized to Grade as noted. -Formed Steel Brackets - ASTM A653 Galvanized Grade 50 SS -I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85 -Plate - A36 Steel, Hot Dip Galvanized -Connectors - Stainless Steel unless otherwise noted.	
6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.	
7. Foundation embedment depths are to be calculated and sealed by a CT State Licensed Geotechnical engineer.	
8. For the purposes of this project, all arrays are classified as Exterior Arrays.	
9. Scope of work by Structural Engineer includes member design, connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.	
Engineer of Record	

ALL PANEL MOUNTING HARDWARE CALLED OUT BELOW WILL BE PROVIDED BY DCE SOLAR. ANY CUSTOMIZED PANEL MOUNTING HARDWARE WILL BE PROVIDED BY OTHERS AND MY VOID DCE SOLAR'S UL2703 CERTIFICATION.



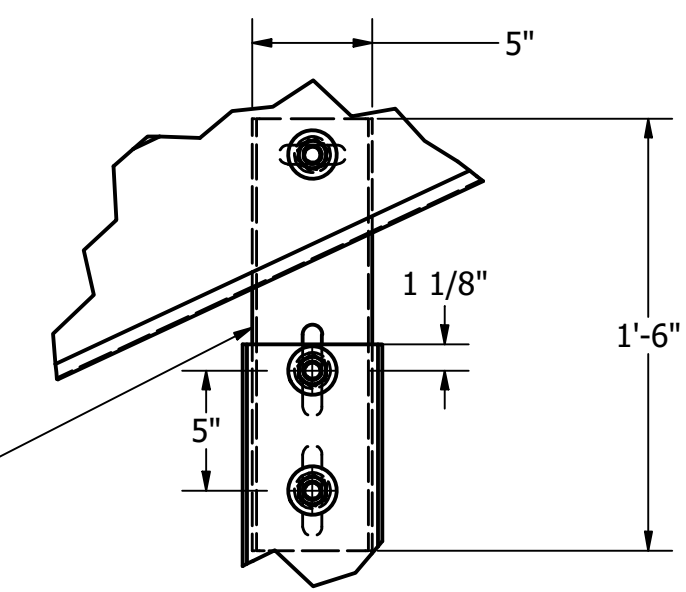
**SIDE VIEW**  
VIEW1  
SCALE 1 : 12

PANEL ATTACHES TO PANEL BEAMS WITH (4) 5/16-18 X 3/4" SERRATED FLANGE CAP SCREWS AND 5/16-18 SERRATED FLANGE NUTS. TORQUE TO 15 FT-LBS.



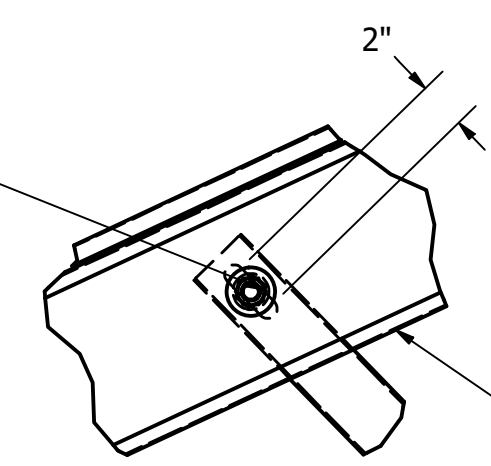
DETAIL A  
SCALE 1 / 8

**TOP BEAM ADAPTER**  
5" X 1.75" X 8G CHANNEL, 18"L  
ASTM A653 GALVANIZED GRADE 50 SS STEEL  
ATTACHES TO NS BEAM AND COLUMN POST WITH (3) 3/4-10 X 1.5" GRADE 5 STEEL HHCS, WASHERS, AND SERRATED FLANGE NUTS. TORQUE TO 250 FT-LB.



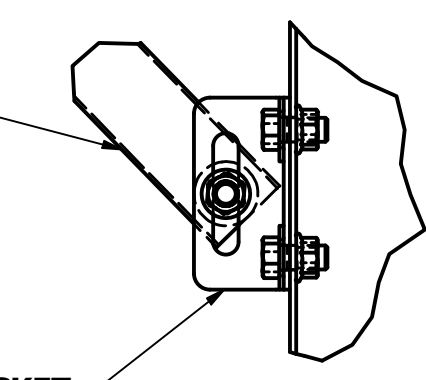
DETAIL B  
SCALE 1 / 8

ANGLE BRACE ATTACHES TO NS CEE BEAM WITH (1) 3/4-10 X 1.5" GRADE 5 STEEL HHCS, WASHER, AND SERRATED FLANGE NUT. TORQUE TO 250 FT-LB.



DETAIL C  
SCALE 1 / 8

**ANGLE BRACE**  
2.75" X 1.75" U-CHANNEL  
14 GAUGE  
ASTM A653 GALVANIZED  
GRADE 50 SS STEEL



DETAIL D  
SCALE 1 / 6

**LOWER MOUNT BRACKET**  
3" X 2.06" X 0.188" X 6"L BENT PLATE  
ASTM A653 GALVANIZED SS GRADE 37.  
ATTACHES TO ANGLE BRACE AND I-BEAM WITH (3) 3/4-10 X 1.5" GRADE 5 STEEL HHCS, WASHERS, AND SERRATED FLANGE NUTS. TORQUE TO 250 FT-LB.

REVISION HISTORY			
REV	DESCRIPTION	DESIGNER	DATE
0	STRUCTURAL DETAIL DRAWING	CPATTERSON	2/11/2026
1	REVISED DRILLED SHAFT ALT. FOUNDATION NOTE	CPATTERSON	2/18/2026
2	ADDED REBAR TO BALLAST BLOCKS	CPATTERSON	3/5/2026
3	REVISED GML ON PAGE 9	CPATTERSON	3/9/2026

**\*\*PROPRIETARY AND CONFIDENTIAL\*\***

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.XX = ± 0.030" (0.76mm)  
.XXX = ± 0.010" (0.25mm)  
ANGLE = ± 5°  
MIN. BREAK = 0.012" (0.3mm)  
SURFACE FINISH = 63 (US)

Material:	2089.091 lbmass		
Description:	<b>CT-LS-DBII, LONGI LR5-72HBD-540M, 2x9, 25 DEG, EVERSOURCE-MORRIS, CT - NORTH SECTION LS-DB, GREENSKIES</b>		
Project:	EVERSOURCE-MORRIS, CT - NORTH SECTION LS-DB		
Drawn:	CPATTERSON	Date:	3/9/2026
Scale:	Sheet:		1 of 9
Format:	D	Part Number:	<b>6777</b>
Rev:	3		



STRUCTURAL DETAIL DRAWING - REAR

PROJECT INFORMATION

INSTALLATION ADDRESS:  
54 South St, Morris, CT 06763

Structural General Notes  
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4. Structure designed in accordance with the International Building Code, 2021 Edition, ASCE 7-16, AISC 360-16 (14th Edition), and AISI S100-16: ASD

Snow Loads:  
-Ground Snow Load  $p_g = 35$  psf  
-Importance Factor  $I_s = 0.8$   
-Exposure Factor  $C_e = 0.9$   
-Slope Snow Load  $p_s = 17.32$  psf

Wind Loads:  
-MRI Factor = 1.00  
-Basic Wind Speed  $V = 110$  mph  
- $I_w = 1$   
-Exposure = C  
-Wind Design performed in accordance with the requirements of ASCE - Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.

Seismic Loads:  
- $SS = 0.183g$ ,  $S1 = 0.054g$   
-Site Class = D  
- $SDS = 0.200g$ ,  $SD1 = 0.090g$   
-Seismic Design Category = B  
-Ordinary Steel Cantilever Column System

5. Material strengths:  
-Hot-rolled structural steel ASTM A992 GR50.  
-Cold Formed Steel Sections comply w/ASTM A1003, structural grade, galvanized to Grade as noted.  
-Formed Steel Brackets - ASTM A653 Galvanized Grade 50 SS  
-I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85  
-Plate - A36 Steel, Hot Dip Galvanized  
-Connectors - Stainless Steel unless otherwise noted.

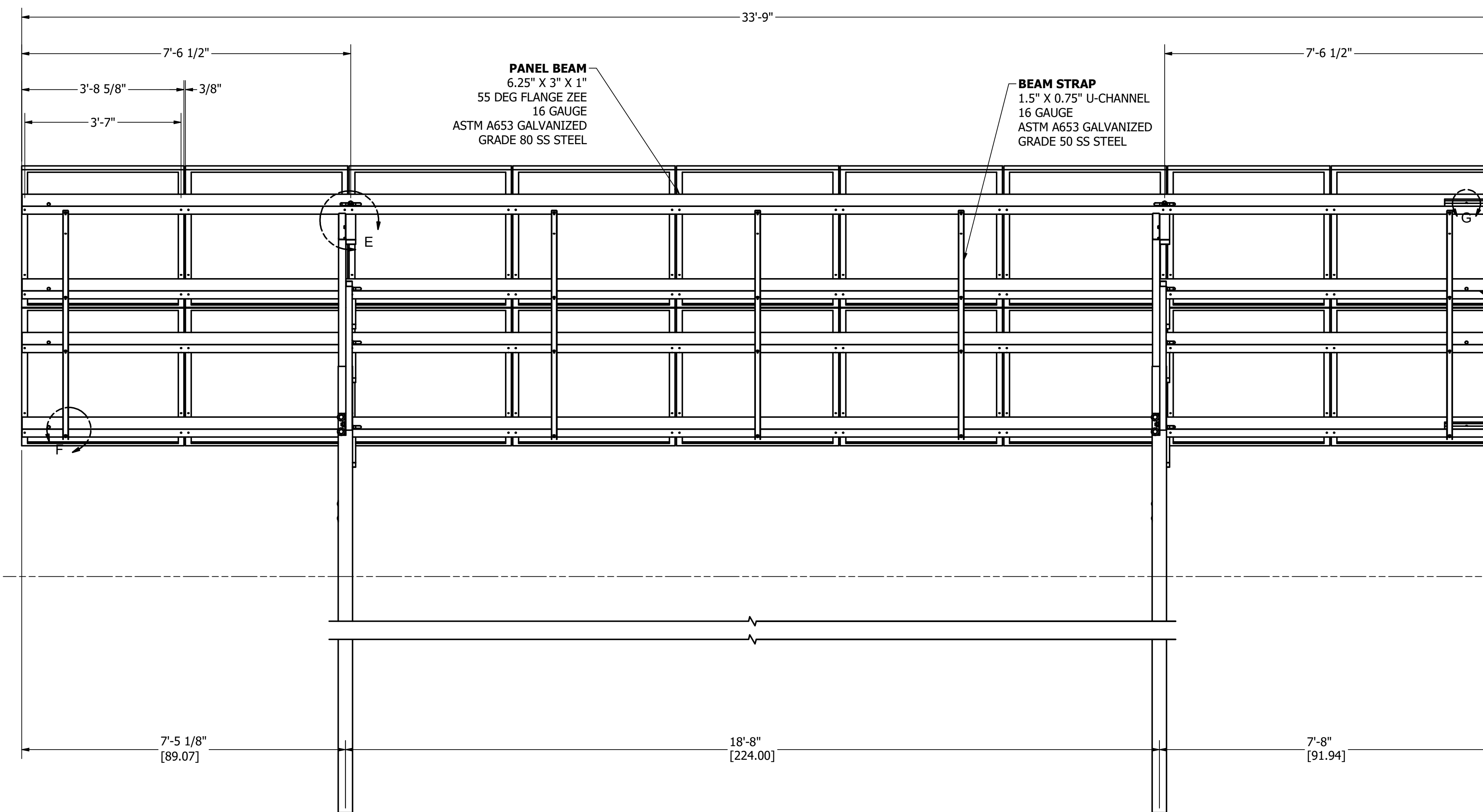
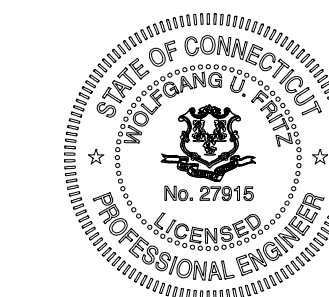
6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.

7. Foundation embedment depths are to be calculated and sealed by a CT State Licensed Geotechnical engineer.

8. For the purposes of this project, all arrays are classified as Exterior Arrays.

9. Scope of work by Structural Engineer includes member design, connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.

Engineer of Record



**PANEL BEAM**  
6.25" X 3" X 1"  
55 DEG FLANGE ZEE  
16 GAUGE  
ASTM A653 GALVANIZED  
GRADE 80 SS STEEL

**BEAM STRAP**  
1.5" X 0.75" U-CHANNEL  
16 GAUGE  
ASTM A653 GALVANIZED  
GRADE 50 SS STEEL

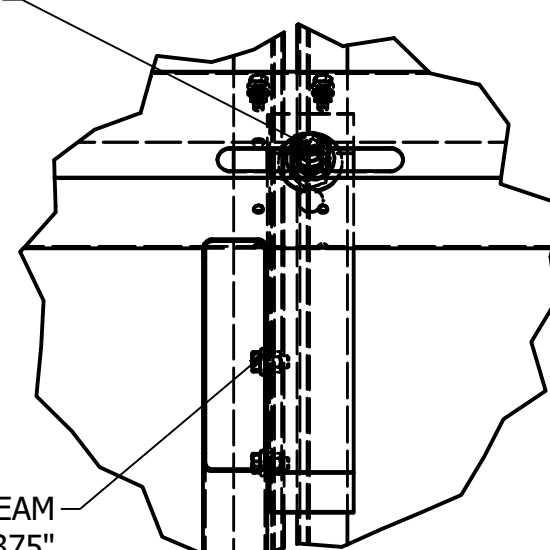
**NOTE:** FOR ALTERNATE ARRAY CONNECTIONS, STRUT CONNECTORS MUST BE PLACED ON 2ND & 3RD EW PANEL BEAMS PER INSTALLATION MANUAL AND GROUND MOUNT LAYOUT

**EW STRUT CONNECTOR**  
1.625" X 1.625" U-CHANNEL  
18 GAUGE  
ASTM A653 GALVANIZED  
GRADE 80 SS STEEL

**REAR VIEW**

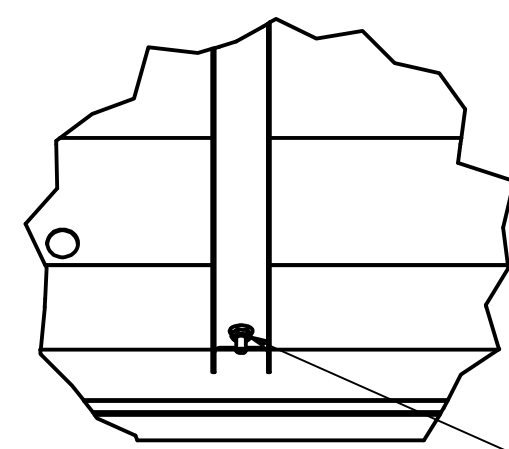
VIEW2  
SCALE 1 / 20

ZEE BEAM ATTACHES TO PIVOT BRACKET USING (1) 3/4-10 GRADE 5 STEEL HHCS, WASHER, AND SERRATED FLANGE NUT. TORQUE TO 250 FT-LBS.



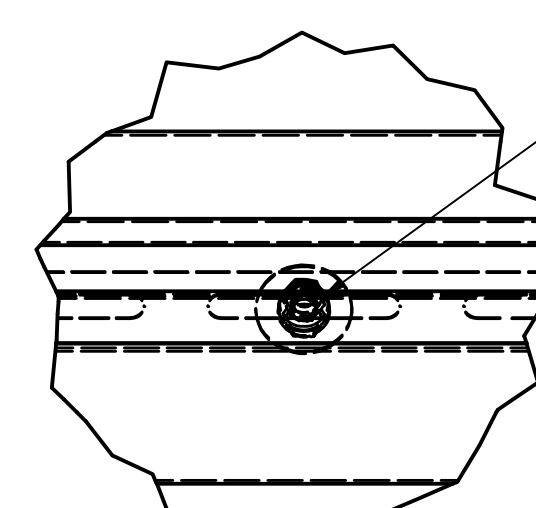
DETAIL E  
SCALE 1 / 6

ZEE BEAM ATTACHES TO CEE BEAM USING PIVOT BRACKET 3" X 2.7" X 12.375" 14G CHANNEL ASTM A653 GRADE 80 SS STEEL ASTM A653 GALVANIZED. BRACKET ATTACHES TO NS BEAM WITH (2) 18-8 SS 3/8-16 SERRATED FLANGE CAP SCREWS AND SERRATED FLANGE NUTS. TORQUE TO 20 FT-LBS.



DETAIL F  
SCALE 1 / 5

BEAM STRAP ATTACHES TO PANEL BEAM WITH (2) 18-8 SS 1/4-20 BUTTON HEAD CAP SCREWS AND SERRATED FLANGE NUTS. TORQUE TO 15 FT-LBS



DETAIL G  
SCALE 1 / 3

NEIGHBORING TABLES BONDED VIA 18G CHANNEL STRUTS. STRUTS CONNECT TO EW PANEL ZEE BEAMS WITH (2) 18-8 SS 3/8-16 SERRATED FLANGE CAP SCREWS, FENDER WASHERS, AND SERRATED FLANGE NUTS. TORQUE TO 20 FT-LBS.

DIMENSIONS ARE IN INCHES UNLESS OTHERWISE NOTED TOLERANCES ARE AS FOLLOWS:  
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.XX = ± 0.030" (0.76mm)  
.XXX = ± 0.010" (0.25mm)  
ANGLE = ± 5°  
MIN. BREAK = 0.012" (0.3mm)  
SURFACE FINISH = 63 (US)

Material:	2089.091 lbmass		
Description:	CT-LS-DBII, LONGI LR5-72HBD-540M, 2x9, 25 DEG, EVERSOURCE-MORRIS, CT - NORTH SECTION LS-DB, GREENSKIES		
Project:	EVERSOURCE-MORRIS, CT - NORTH SECTION LS-DB		
Drawn:	CPATTERSON	Date:	3/9/2026
Scale:	2 of 9		Sheet:

**\*\*PROPRIETARY AND CONFIDENTIAL\*\***

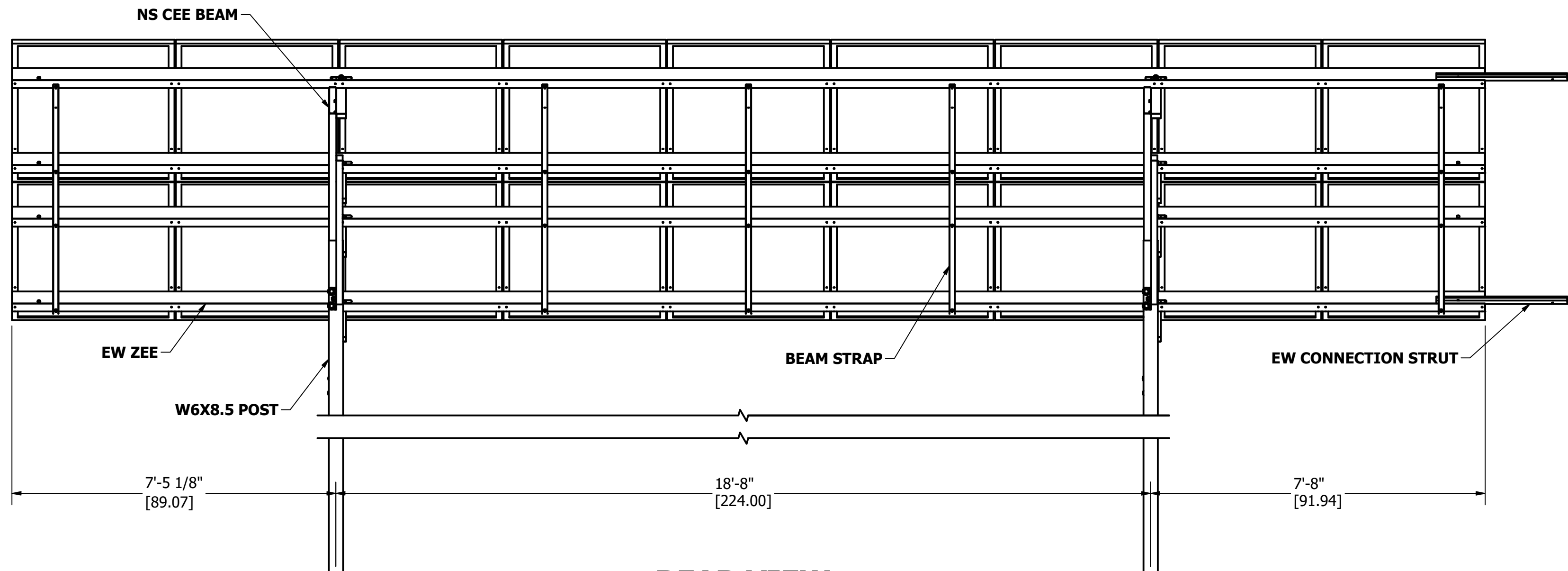
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Cornelius, NC 28031  
www.dcesolar.com  
Phone: 1-704-659-7474

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**REAR VIEW**  
VIEW3  
SCALE 0.04 : 1

PANEL SPECIFICATION			
NAME	DESCRIPTION		
MANUFACTURER	LONGI		
MODEL	LR5-72HBD-540M		
LENGTH (mm)	2278		
WIDTH (mm)	1134		
THICKNESS (mm)	30		

PROJECT INFORMATION	
INSTALLATION ADDRESS:	54 South St, Morris, CT 06763
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MATERIAL DESCRIPTION			
MEMBER	SHAPE	MATERIAL	GAGE
PANEL BEAM	6.25Z3X1X55DEG	A653 SS Gr80	16GA
NS CEE BEAM	8CS2X0.625	A653 SS Gr80	14GA
KICKER BRACE	2.75CU1.75	A653 SS Gr50	12GA
BEAM BRACE	1.5CU0.75	A653 SS Gr50	16GA
POST	W6x8.5	A992	-

2. If existing conditions make it necessary to revise structural details, consult DCE Solar before proceeding with any change.

3. These drawings and notes are for this specific project and no other use is authorized.

4. Structure designed in accordance with the International Building Code, 2021 Edition, ASCE 7-16, AISC 360-16 (14th Edition), and AISI S100-16: ASD

TEST LOADS	
LOAD TYPE	UNFACTORED LOAD (LB)
UPLIFT	800
ADJUSTED UPLIFT*	3800
COMPRESSIVE LOAD	5400
LATERAL LOAD	700

Snow Loads:  
-Ground Snow Load pg = 35 psf  
-Importance Factor Is = 0.8  
-Exposure Factor Ce = 0.9  
-Slope Snow Load ps = 17.32 psf

NOTES  
\*ADJUSTED UPLIFT IS ASSUMED AS 70% OF THE DOWNWARD LOAD. IT'S RECOMMENDED TO USE THIS LOAD FOR PULL TEST IN CASE PUSH TEST CANNOT BE PERFORMED.  
1: USE ADJUSTED UPLIFT IF NO REFUSAL IS ENCOUNTERED.  
2: USE UPLIFT FORCE IN CASE OF REFUSAL.  
3: FOR UPLIFT AND LATERAL FORCES USE SAFETY FACTOR OF 1.5 AND 2, RESPECTIVELY.

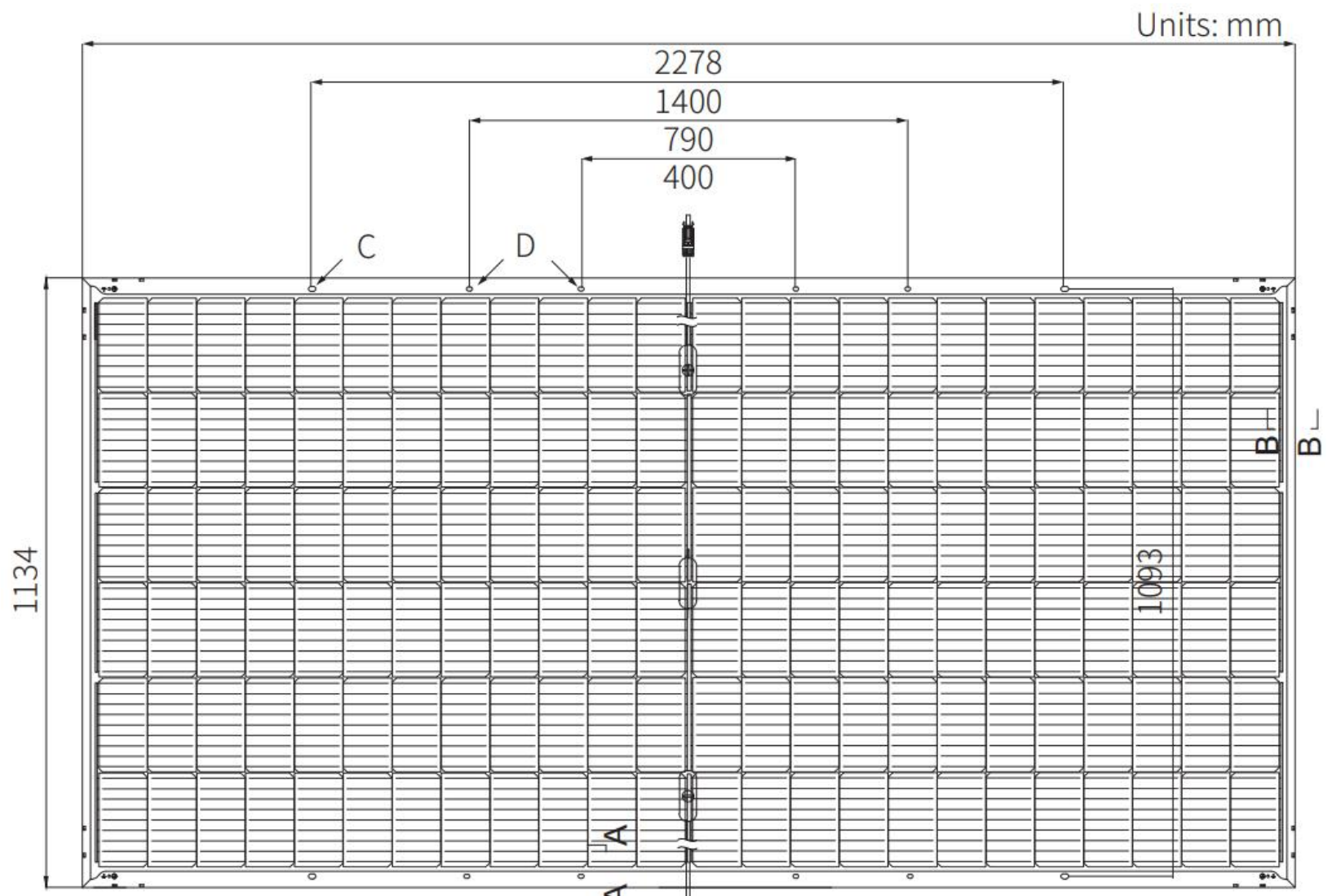
Wind Loads:  
-MRI Factor = 1.00  
-Basic Wind Speed V = 110 mph  
-Iw = 1  
-Exposure = C  
-Wind Design performed in accordance with the requirements of ASCE - Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.

ALTERNATE FOUNDATION DESIGN	
NAME	DESCRIPTION
POST TYPE	W6x8.5
MIN. EMBEDMENT DEPTH (FT) IN CASE OF REFUSAL	6'
ALTERNATE FOUNDATION DESIGN - A	6'-6" MIN. DEPTH, 1'-6" DIAMETER DRILLED SHAFT DESIGN
ALTERNATE FOUNDATION DESIGN - B	8'-6" LONG X 2' WIDE X 2' THICK SPREAD FOOTING DESIGN

Seismic Loads:  
-SS = 0.183g, S1 = 0.054g  
-Site Class = D  
-SDS = 0.200g, SD1 = 0.090g  
-Seismic Design Category = B  
-Ordinary Steel Cantilever Column System

IN-FIELD PILE REMEDIATION  
ANY IN-FIELD REMEDIATION REQUIRING THE CUTTING OR DRILLING OF GALVANIZED MATERIAL SHOULD FOLLOW ONE OF THESE TWO GUIDELINES TO COAT AND TREAT METALS THAT ARE EXPOSED TO GALVANIZATION DAMAGE:  
1. USE PAINTS CONTAINING ZINC DUST (IN ACCORDANCE WITH "ASTM A 780-01" SECTION A2)  
2. USE ZINC SPRAY (IN ACCORDANCE WITH "ASTM A 780-01" SECTION A3) ONE OF THE ABOVE GUIDELINES MUST BE FOLLOWED TO MAINTAIN THE DCE WARRANTY REQUIREMENTS.

5. Material strengths:  
-Hot-rolled structural steel ASTM A992 GR50.  
-Cold Formed Steel Sections comply w/ASTM A1003, structural grade, galvanized to Grade as noted.  
-Formed Steel Brackets - ASTM A653 Galvanized Grade 50 SS  
-I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85  
-Plate - A36 Steel, Hot Dip Galvanized  
-Connectors - Stainless Steel unless otherwise noted.



6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.

7. Foundation embedment depths are to be calculated and sealed by a CT State Licensed Geotechnical engineer.

8. For the purposes of this project, all arrays are classified as Exterior Arrays.

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Engineer of Record



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MIN. BREAK = 0.012" (0.3mm)  
SURFACE FINISH = 63 (US)

Material:	2089.091 lbmass
Weight:	2089.091 lbmass
Description:	CT-LS-DBII, LONGI LR5-72HBD-540M, 2x9, 25 DEG, EVERSOURCE-MORRIS, CT - NORTH SECTION LS-DB, GREENSKIES
Project:	EVSOURCE-MORRIS, CT - NORTH SECTION LS-DB
Date:	3/9/2026
Drawn:	CPATTERSON
Scale:	3 of 9

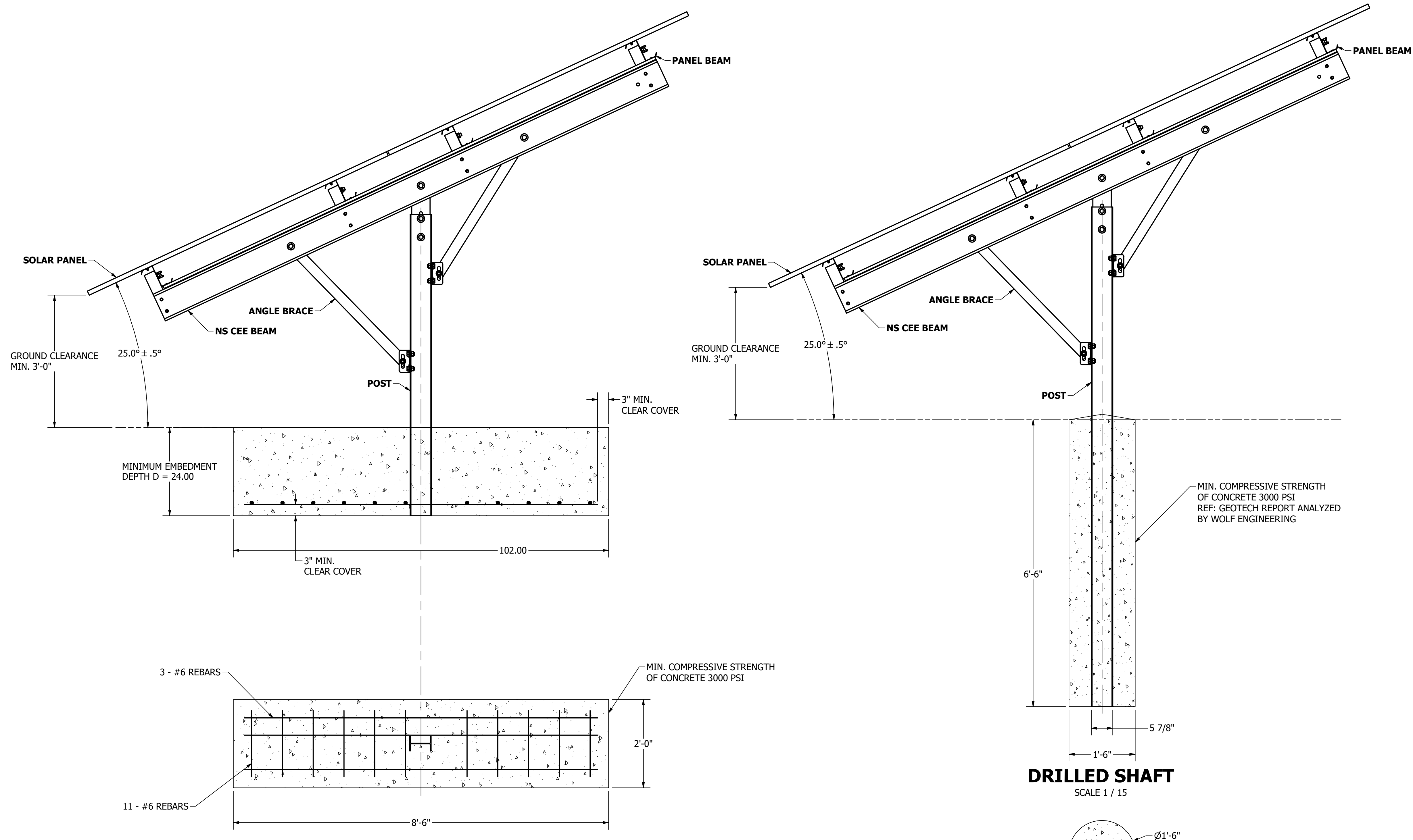
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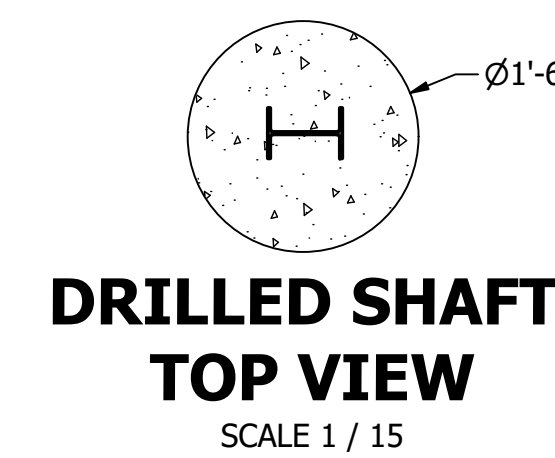
STRUCTURAL DETAIL DRAWING - ALTERNATE FOUNDATIONS



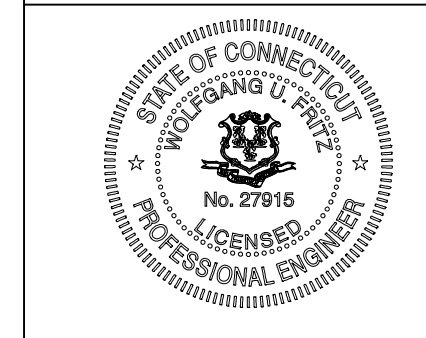
**ALTERNATE PILE SECTION FOR USE WHEN REFUSAL CONDITION ENCOUNTERED AT EMBEDMENT DEPTHS LESS THAN 6'**

SPREAD FOOTING  
SCALE 1 / 15

**DRILLED SHAFT**  
SCALE 1 / 15



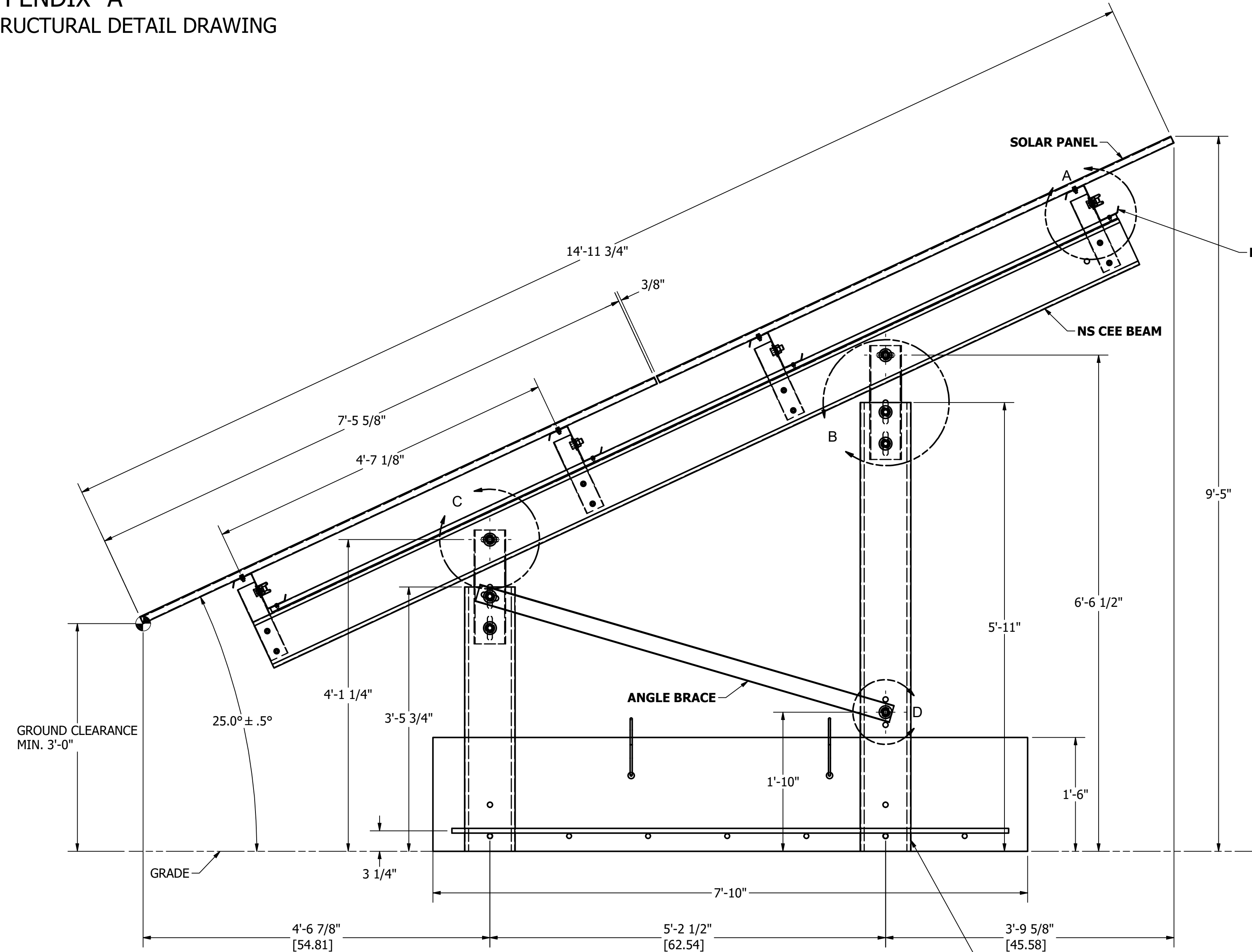
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-Exposure Factor Ce = 0.9	
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-Iw = 1	
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Seismic Loads:	
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-I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85	
-Plate - A36 Steel, Hot Dip Galvanized	
-Connectors - Stainless Steel unless otherwise noted.	
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Engineer of Record	



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ANGLE = ± 5° MIN. BREAK = 0.012" (0.3mm) SURFACE FINISH = 63 (US)		Weight: 2089.091 lbmass
**PROPRIETARY AND CONFIDENTIAL** THIS DRAWING AND ALL INFORMATION THEREIN IS THE PROPERTY OF DCE SOLAR AND IS CONFIDENTIAL AND MUST NOT BE MADE PUBLIC OR COPIED UNLESS AUTHORIZED BY DCE SOLAR AND IS SUBJECT TO RETURN UPON REQUEST.		<b>CT-LS-DBII, LONGI LR5-72HBD-540M, 2x9, 25 DEG, EVERSOURCE-MORRIS, CT - NORTH SECTION LS-DB, GREENSKIES</b>
DCE SOLAR 19410 Jetton Rd, Ste 220 Cornelius, NC 28031 www.dcesolar.com Phone: 1-704-659-7474		Project: EVERSOURCE-MORRIS, CT - NORTH SECTION LS-DB
Date: 3/9/2026		Drawn: CPATTERSON
Scale: 4 of 9		Sheet: 4 of 9
Format: D	Part Number: 6777	Rev: 3

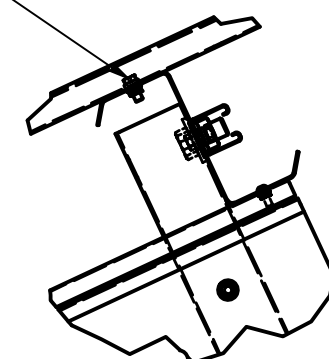
**APPENDIX- A  
STRUCTURAL DETAIL DRAWING**

PROJECT INFORMATION	
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Wind Loads: -MRI Factor = 1.00 -Basic Wind Speed V = 110 mph -Iw = 1 -Exposure = C -Wind Design performed in accordance with the requirements of ASCE - Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.	
Seismic Loads: -SS = 0.183g, S1 = 0.054g -Site Class = D -SDS = 0.200g, SD1 = 0.090g -Seismic Design Category = B -Ordinary Steel Cantilever Column System	
5. Material strengths: -Hot-rolled structural steel ASTM A992 GR50. -Cold Formed Steel Sections comply w/ASTM A1003, structural grade, galvanized to Grade as noted. -Formed Steel Brackets - ASTM A653 Galvanized Grade 50 SS -I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85 -Plate - A36 Steel, Hot Dip Galvanized -Connectors - Stainless Steel unless otherwise noted.	
6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.	
7. For the purposes of this project, all arrays are classified as Exterior Arrays.	
8. Scope of work by Structural Engineer includes member design, connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.	
Engineer of Record	



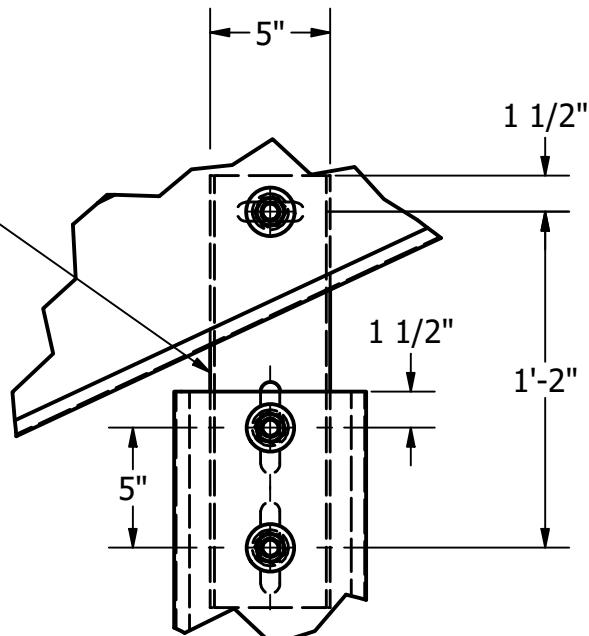
ALL PANEL MOUNTING HARDWARE CALLED OUT BELOW WILL BE PROVIDED BY DCE SOLAR. ANY CUSTOMIZED PANEL MOUNTING HARDWARE PROVIDED BY OTHERS MAY VOID DCE SOLAR'S UL2703 CERTIFICATION.

PANEL ATTACHES TO PANEL BEAMS WITH (4) 5/16-18 X 3/4" SERRATED FLANGE CAP SCREWS AND 5/16-18 SERRATED FLANGE NUTS. TORQUE TO 15 FT-LBS.



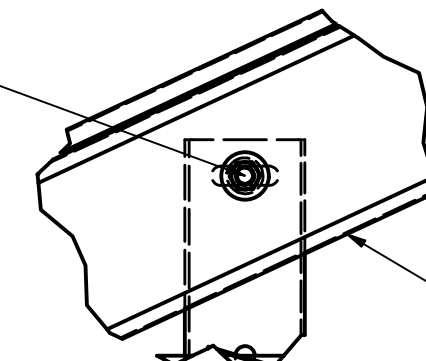
DETAIL A  
SCALE 1 / 8

**TOP BEAM ADAPTER**  
5" X 1.75" X 18" X 8G CHANNEL  
ASTM A653 GALVANIZED GRADE 50 SS STEEL  
ATTACHES TO NS BEAM AND COLUMN POST WITH (3) 3/4-10 X 1.5" SERRATED FLANGE CAP SCREW, WASHER, AND SERRATED FLANGE NUT. TORQUE TO 250 FT-LB.



DETAIL B  
SCALE 1 / 8

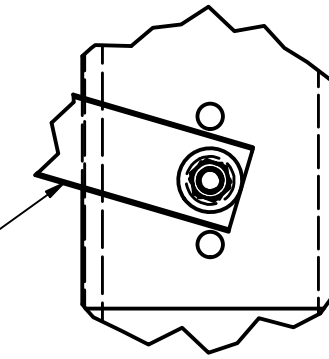
ANGLE BRACE ATTACHES TO NS BEAM WITH (1) 3/4-10 X 1.5" GRADE 5 STEEL HHCS, WASHER, AND SERRATED FLANGE NUT. TORQUE TO 250 FT-LBS.



DETAIL C  
SCALE 1 / 8

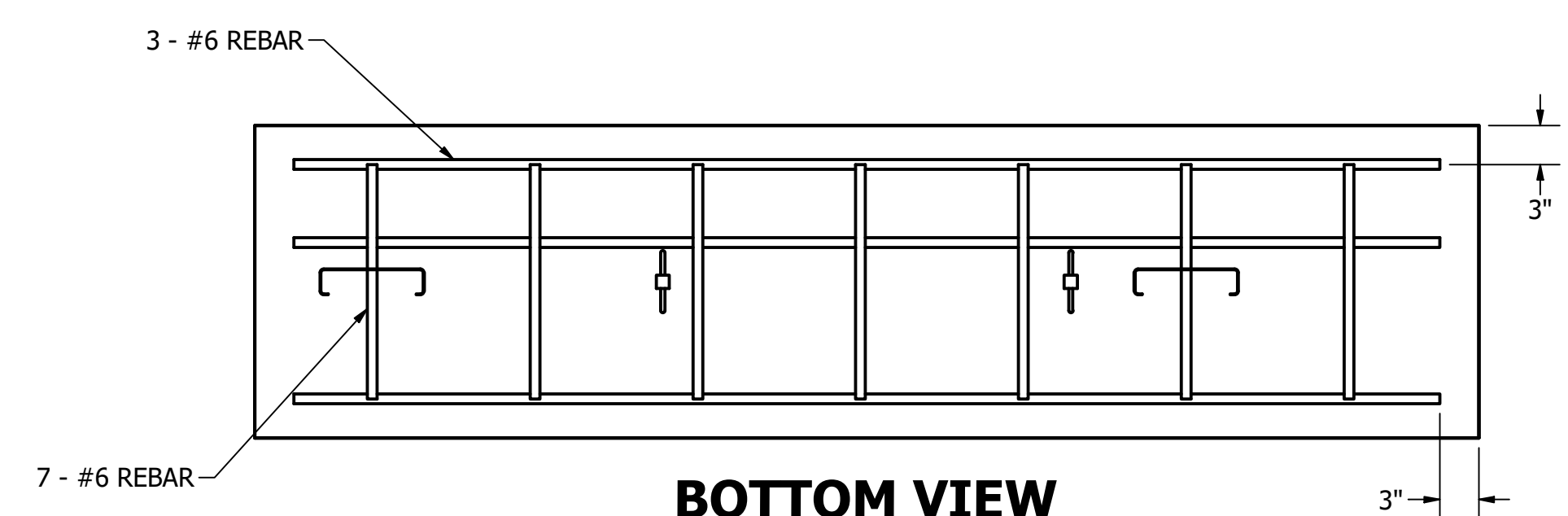
**NS BEAM**  
8" X 2" X 0.625"  
14 GAUGE  
ASTM A653 GALVANIZED  
GRADE 80 SS STEEL

**ANGLE BRACE**  
2.75" X 1.75" U-CHANNEL,  
14 GAUGE  
ASTM A653 GALVANIZED  
GRADE 50 SS STEEL



DETAIL D  
SCALE 1 / 6

**SIDE VIEW**  
VIEW1  
SCALE 1 : 12



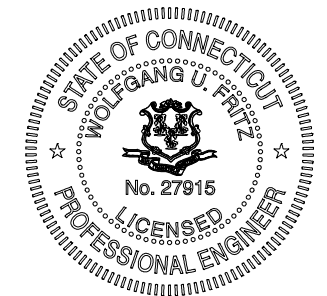
**BOTTOM VIEW**  
BALLAST ASSEMBLY  
VIEW27  
SCALE 1 : 12

REVISION HISTORY			
REV	DESCRIPTION	DESIGNER	DATE
0	STRUCTURAL DETAIL DRAWING	CPATTERSON	2/11/2026
1	REVISED DRILLED SHAFT ALT. FOUNDATION NOTE	CPATTERSON	2/18/2026
2	ADDED REBAR TO BALLAST BLOCKS	CPATTERSON	3/5/2026
3	REVISED GML ON PAGE 9	CPATTERSON	3/9/2026

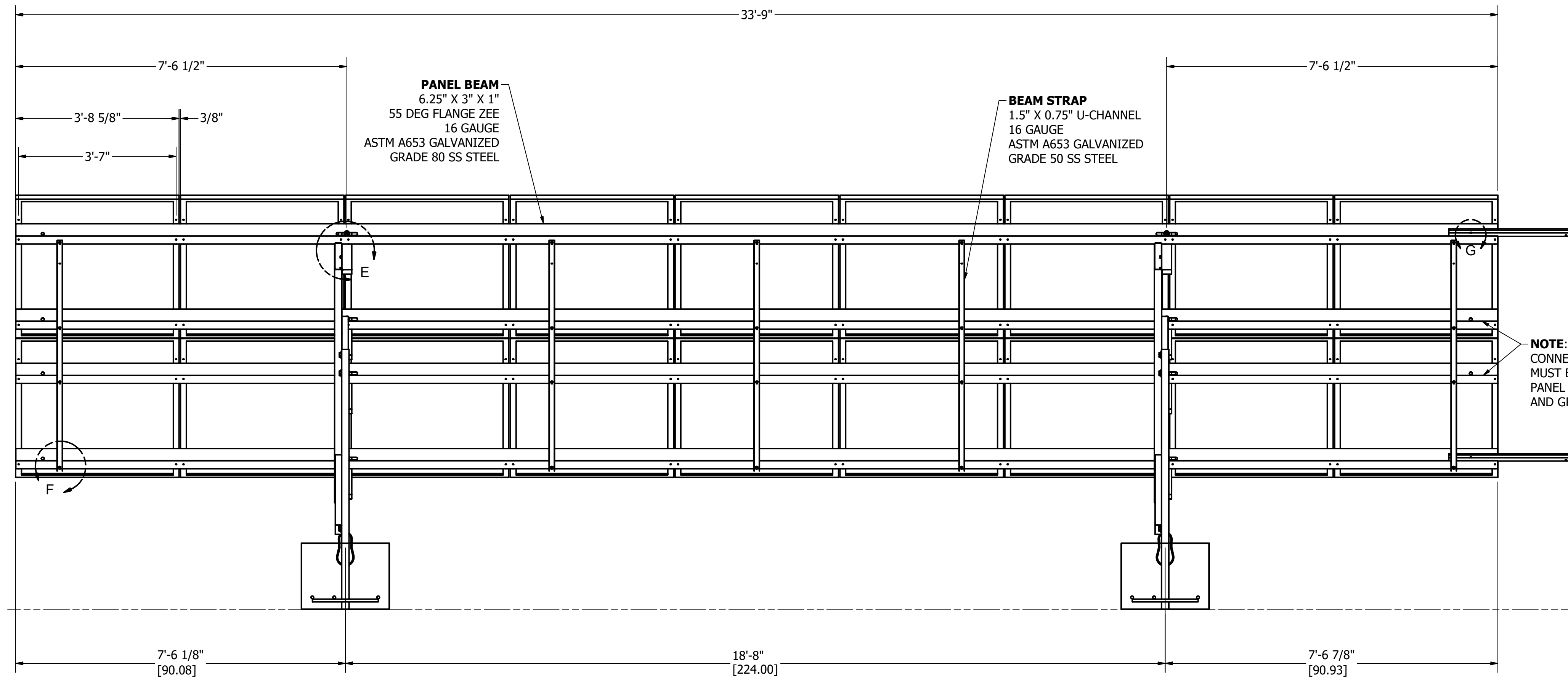
**\*\*PROPRIETARY AND CONFIDENTIAL\*\***  
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DIMENSIONS ARE IN INCHES UNLESS OTHERWISE NOTED  
TOLERANCES ARE AS FOLLOWS:  
.X = ± 0.100" (2.54mm)  
.XX = ± 0.030" (0.76mm)  
.XXX = ± 0.010" (0.25mm)  
ANGLE = ± 5°  
MIN. BREAK = 0.012" (0.3mm)  
SURFACE FINISH = 63 (US)

Material:	9066.549 lbmass		
Weight:	9066.549 lbmass		
Description:	CT-LS-DC-B, LONGI LR5-72HBD-540M, 2x9, 25 DEG, EVERSOURCE-MORRIS, CT - SOUTH SECTION LS-B, GREENSKIES		
Project:	EVERSOURCE-MORRIS, CT - SOUTH SECTION LS-B		
Drawn:	CPATTERSON	Date:	3/9/2026
Scale:		Sheet:	5 of 9
Format:	D	Part Number:	6778
Rev:			3



STRUCTURAL DETAIL DRAWING - REAR



**REAR VIEW**

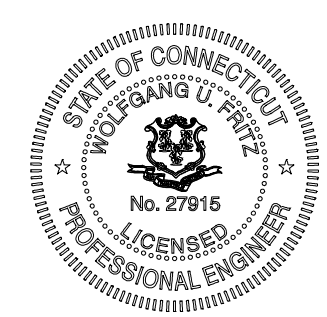
VIEW3  
SCALE 1 / 20

**PANEL BEAM**  
6.25" X 3" X 1"  
55 DEG FLANGE ZEE  
16 GAUGE  
ASTM A653 GALVANIZED  
GRADE 80 SS STEEL

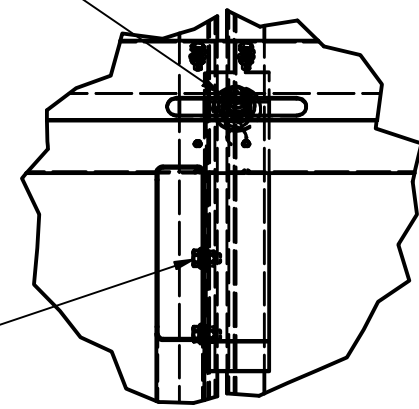
**BEAM STRAP**  
1.5" X 0.75" U-CHANNEL  
16 GAUGE  
ASTM A653 GALVANIZED  
GRADE 50 SS STEEL

**NOTE:** FOR ALTERNATE ARRAY CONNECTIONS, STRUT CONNECTORS MUST BE PLACED ON 2ND & 3RD EW PANEL BEAMS PER INSTALLATION MANUAL AND GROUND MOUNT LAYOUT

PROJECT INFORMATION	
INSTALLATION ADDRESS: 54 South St, Morris, CT 06763	
Structural General Notes	
1. The contractor will be solely responsible for all construction means, methods, techniques, sequences and procedures and shall at all times take reasonable precautions for the safety of its employees on the project, and shall comply with all applicable provisions of federal, state, and municipal safety laws and building construction codes.	
2. If existing conditions make it necessary to revise structural details, consult DCE Solar before proceeding with any change.	
3. These drawings and notes are for this specific project and no other use is authorized.	
4. Structure designed in accordance with the International Building Code, 2021 Edition. ASCE 7-16, AISC 360-16 (14th Edition), and AISI S100-16: ASD	
Snow Loads: -Ground Snow Load $p_g = 35$ psf -Importance Factor $I_s = 0.8$ -Exposure Factor $C_e = 0.9$ -Slope Snow Load $p_s = 17.32$ psf	
Wind Loads: -MRI Factor = 1.00 -Basic Wind Speed $V = 110$ mph - $I_w = 1$ -Exposure = C -Wind Design performed in accordance with the requirements of ASCE - Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.	
Seismic Loads: - $SS = 0.183g$ , $S1 = 0.054g$ -Site Class = D - $SDS = 0.200g$ , $SD1 = 0.090g$ -Seismic Design Category = B -Ordinary Steel Cantilever Column System	
5. Material strengths: -Hot-rolled structural steel ASTM A992 GR50. -Cold Formed Steel Sections comply w/ASTM A1003, structural grade, galvanized to Grade as noted. -Formed Steel Brackets - ASTM A653 Galvanized Grade 50 SS -I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85 -Plate - A36 Steel, Hot Dip Galvanized -Connectors - Stainless Steel unless otherwise noted.	
6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.	
7. For the purposes of this project, all arrays are classified as Exterior Arrays.	
8. Scope of work by Structural Engineer includes member design, connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.	
Engineer of Record	

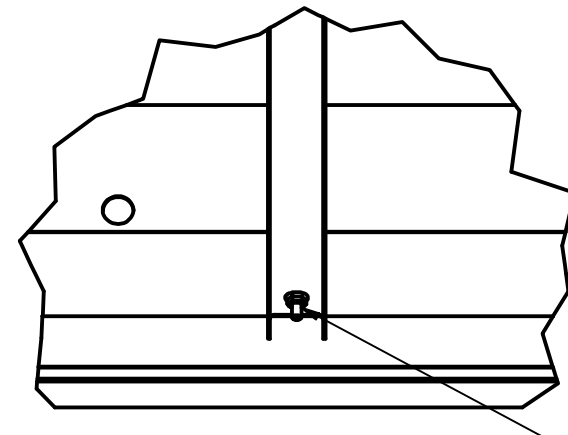


ZEE BEAM ATTACHES TO PIVOT BRACKET USING (1) 3/4-10 GRADE 5 COATED STEEL HHCS, WASHER, AND SERRATED FLANGE NUT. TORQUE TO 250 FT-LBS.



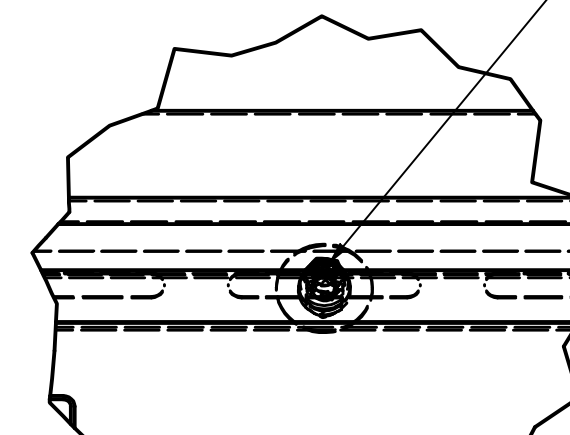
DETAIL E  
SCALE 1 / 8

ZEE BEAM ATTACHES TO CEE BEAM USING PIVOT BRACKET 3" X 2.7" X 12.375" 14G CHANNEL ASTM A653 GRADE 80 SS STEEL G115 GALVANIZED. BRACKET ATTACHES TO NS BEAM WITH (2) 18-8 SS 3/8-16 SERRATED FLANGE CAP SCREWS AND SERRATED FLANGE NUTS. TORQUE TO 20 FT-LBS.



DETAIL F  
SCALE 1 / 5

BEAM STRAP ATTACHES TO PANEL BEAM WITH (2) 18-8 SS 1/4-20 FLANGED BUTTON HEAD CAP SCREWS AND SERRATED FLANGE NUTS. TORQUE TO 15 FT-LBS



DETAIL G  
SCALE 1 / 3

NEIGHBORING TABLES BONDED VIA 18G CHANNEL STRUTS. STRUTS CONNECT TO EW PANEL ZEE BEAMS WITH (2) 18-8 SS 3/8-16 SERRATED FLANGE CAP SCREWS, FENDER WASHERS, AND SERRATED FLANGE NUTS. TORQUE TO 20 FT-LBS.

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	DIMENSIONS ARE IN INCHES UNLESS OTHERWISE NOTED TOLERANCES ARE AS FOLLOWS:		Material:	
	.X = ± 0.100" (2.54mm)	.XX = ± 0.030" (0.76mm)	.XXX = ± 0.010" (0.25mm)	Weight: 9066.549 lbmass
ANGLE = ± 5° MIN. BREAK = 0.012" (0.3mm)	SURFACE FINISH = 63 (US)		Description: <b>CT-LS-DC-B, LONGI LR5-72HBD-540M, 2x9, 25 DEG, EVERSOURCE-MORRIS, CT - SOUTH SECTION LS-B, GREENSKIES</b>	
Project: EVERSOURCE-MORRIS, CT - SOUTH SECTION LS-B		Drawn: CPATTERSON	Date: 3/9/2026	Scale: 6 of 9
Format: D		Part Number: <b>6778</b>	Rev: 3	

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Cornelius, NC, 28031  
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Phone: 1-704-659-7474

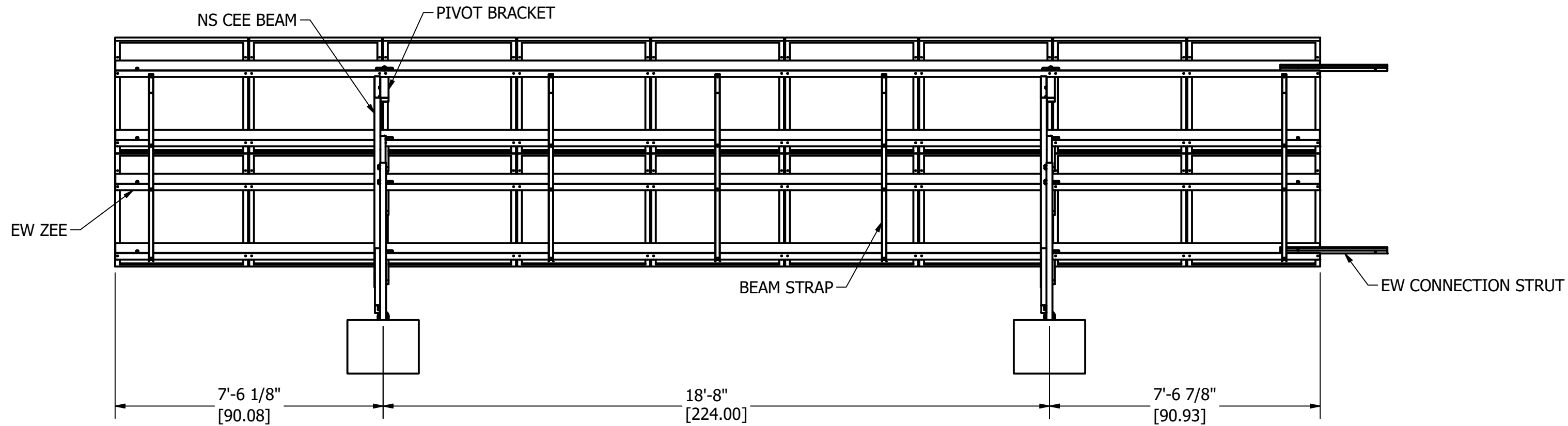
PANEL SPECIFICATION			PROJECT INFORMATION
NAME	DESCRIPTION		INSTALLATION ADDRESS:
MANUFACTURER	LONGI		54 South St, Morris, CT 06763
MODEL	LR5-72HBD-540M		Structural General Notes
LENGTH (mm)	2278		1. The contractor will be solely responsible for all construction means, methods, techniques, sequences and procedures and shall at all times take reasonable precautions for the safety of its employees on the project, and shall comply with all applicable provisions of federal, state, and municipal safety laws and building construction codes.
WIDTH (mm)	1134		2. If existing conditions make it necessary to revise structural details, consult DCE Solar before proceeding with any change.
THICKNESS (mm)	30		3. These drawings and notes are for this specific project and no other use is authorized.

MATERIAL DESCRIPTION			
MEMBER	SHAPE	MATERIAL	GAGE
PANEL BEAM	6.25Z3X1X55DEG	A653 SS Gr80	16GA
NS CEE BEAM	8CS2X0.625	A653 SS Gr80	14GA
KICKER BRACE	2.75CU1.75	A653 SS Gr50	14GA
BEAM BRACE	1.5CU0.75	A653 SS Gr50	16GA
POST	CEE POST 8 X 2 X .625	A653 SS Gr80	14GA

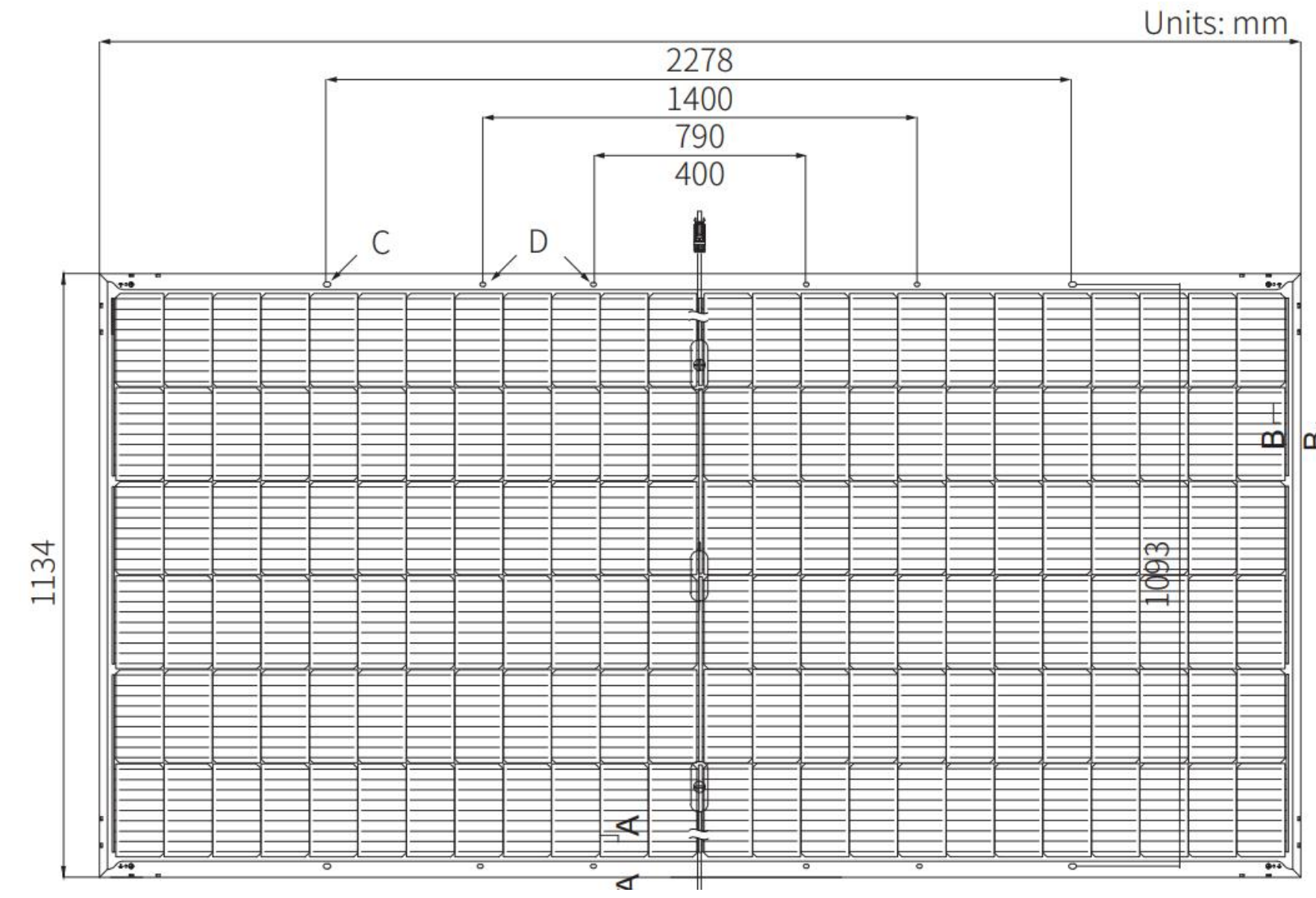
**NOTES**  
 \*ADJUSTED UPLIFT IS ASSUMED AS 70% OF THE DOWNWARD LOAD. IT'S RECOMMENDED TO USE THIS LOAD FOR PULL TEST IN CASE PUSH TEST CANNOT BE PERFORMED.  
 1: USE ADJUSTED UPLIFT IF NO REFUSAL IS ENCOUNTERED.  
 2: USE UPLIFT FORCE IN CASE OF REFUSAL.  
 3: FOR UPLIFT AND LATERAL FORCES USE SAFETY FACTOR OF 1.5 AND 2, RESPECTIVELY.

**IN-FIELD PILE REMEDIATION**  
 ANY IN-FIELD REMEDIATION REQUIRING THE CUTTING OR DRILLING OF GALVANIZED MATERIAL SHOULD FOLLOW ONE OF THESE TWO GUIDELINES TO COAT AND TREAT METALS THAT ARE EXPOSED TO GALVANIZATION DAMAGE:  
 1. USE PAINTS CONTAINING ZINC DUST (IN ACCORDANCE WITH "ASTM A 780-01" SECTION A2)  
 2. USE ZINC SPRAY (IN ACCORDANCE WITH "ASTM A 780-01" SECTION A3) ONE OF THE ABOVE GUIDELINES MUST BE FOLLOWED TO MAINTAIN THE DCE WARRANTY REQUIREMENTS.

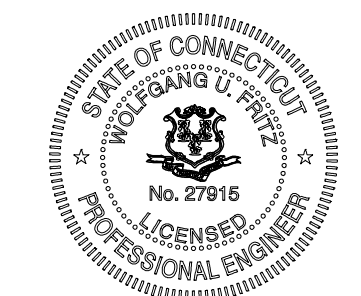
**PROJECT INFORMATION**  
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 3. These drawings and notes are for this specific project and no other use is authorized.  
 4. Structure designed in accordance with the International Building Code, 2021 Edition. ASCE 7-16, AISC 360-16 (14th Edition), and AISI S100-16: ASD  
**Snow Loads:**  
 -Ground Snow Load  $p_g = 35$  psf  
 -Importance Factor  $I_s = 0.8$   
 -Exposure Factor  $C_e = 0.9$   
 -Slope Snow Load  $p_s = 17.32$  psf  
**Wind Loads:**  
 -MRI Factor = 1.00  
 -Basic Wind Speed  $V = 110$  mph  
 - $I_w = 1$   
 -Exposure = C  
 -Wind Design performed in accordance with the requirements of ASCE - Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.  
**Seismic Loads:**  
 - $SS = 0.183g$ ,  $S1 = 0.054g$   
 -Site Class = D  
 - $SDS = 0.200g$ ,  $SD1 = 0.090g$   
 -Seismic Design Category = B  
 -Ordinary Steel Cantilever Column System  
 5. Material strengths:  
 -Hot-rolled structural steel ASTM A992 GR50.  
 -Cold Formed Steel Sections comply w/ASTM A1003, structural grade, galvanized to Grade as noted.  
 -Formed Steel Brackets - ASTM A653 Galvanized Grade 50 SS  
 -I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85  
 -Plate - A36 Steel, Hot Dip Galvanized  
 -Connectors - Stainless Steel unless otherwise noted.  
 6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.  
 7. For the purposes of this project, all arrays are classified as Exterior Arrays.  
 8. Scope of work by Structural Engineer includes member design, connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.



**REAR VIEW**  
 2X9 ARRAY  
 VIEW13  
 SCALE 0.03 : 1



Engineer of Record



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 .X = ± 0.100" (2.54mm)  
 .XX = ± 0.030" (0.76mm)  
 .XXX = ± 0.010" (0.25mm)  
 ANGLE = ± 5°  
 MIN. BREAK = 0.012" (0.3mm)  
 SURFACE FINISH = 63 (US)

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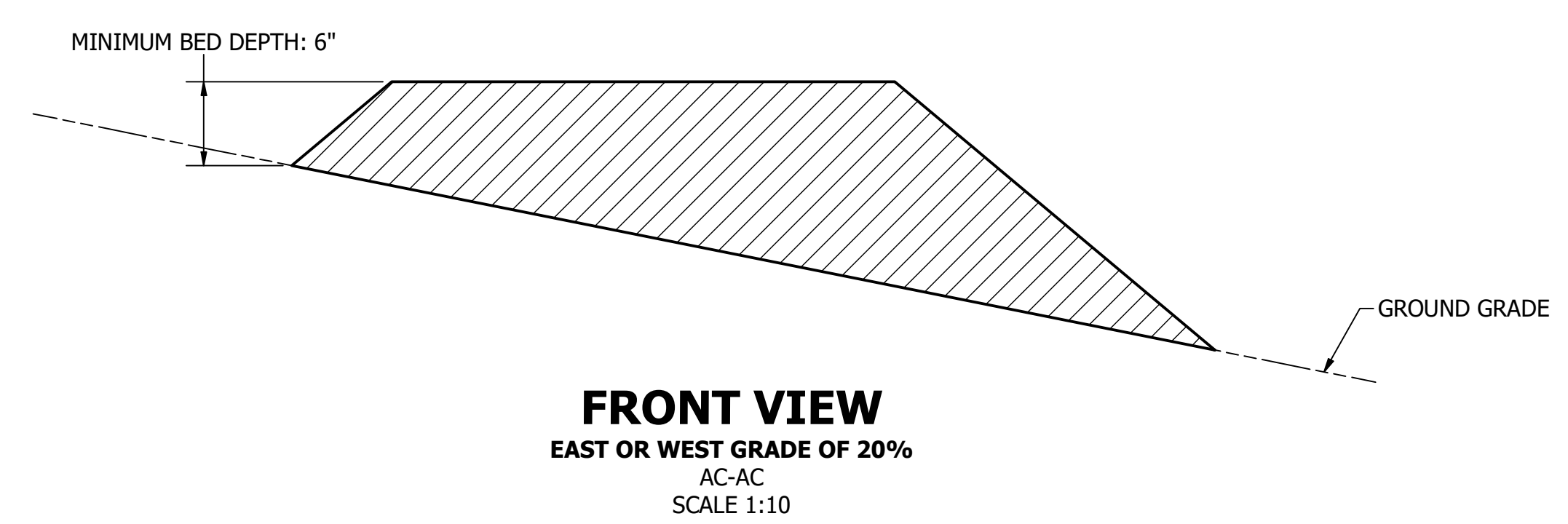
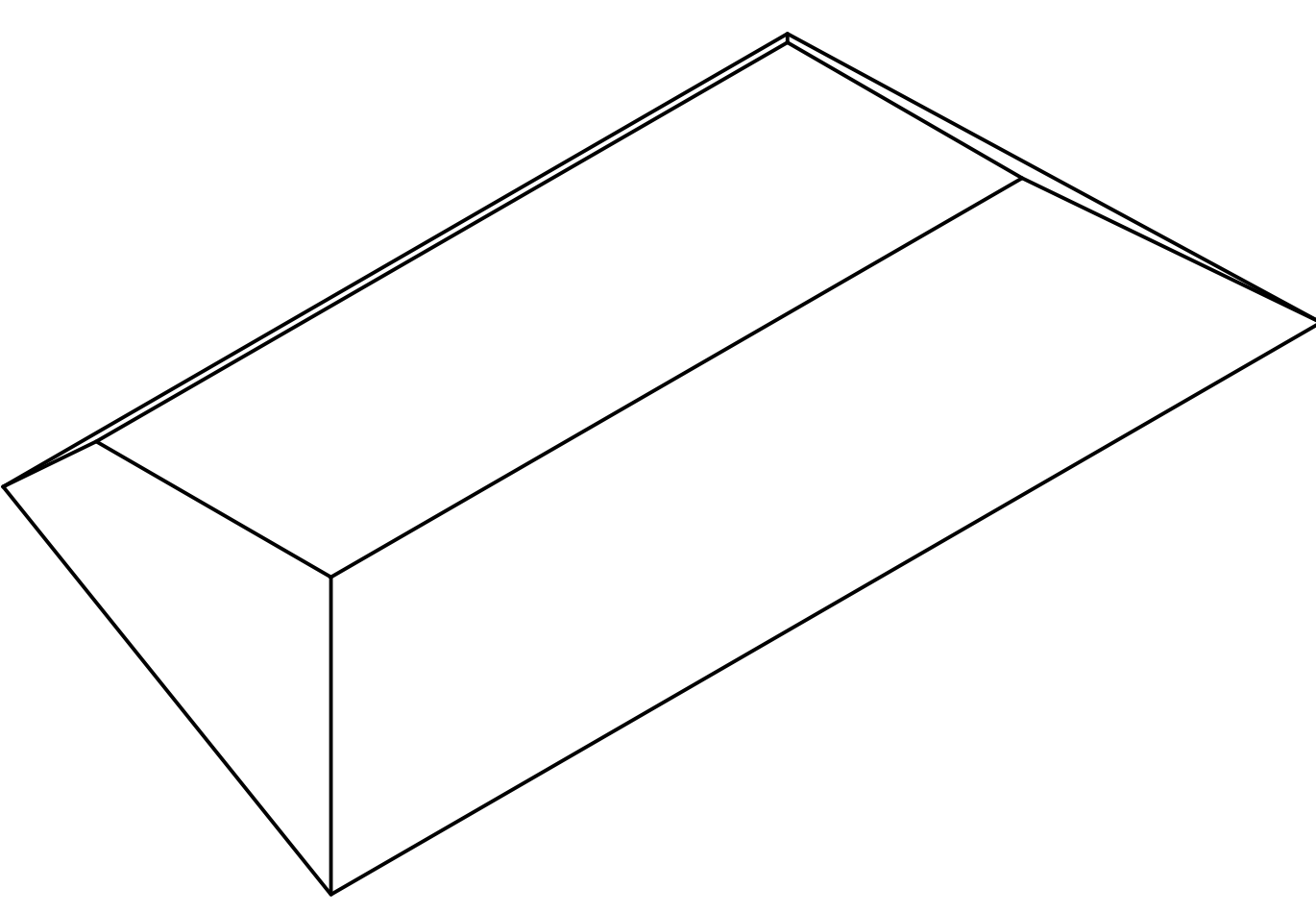
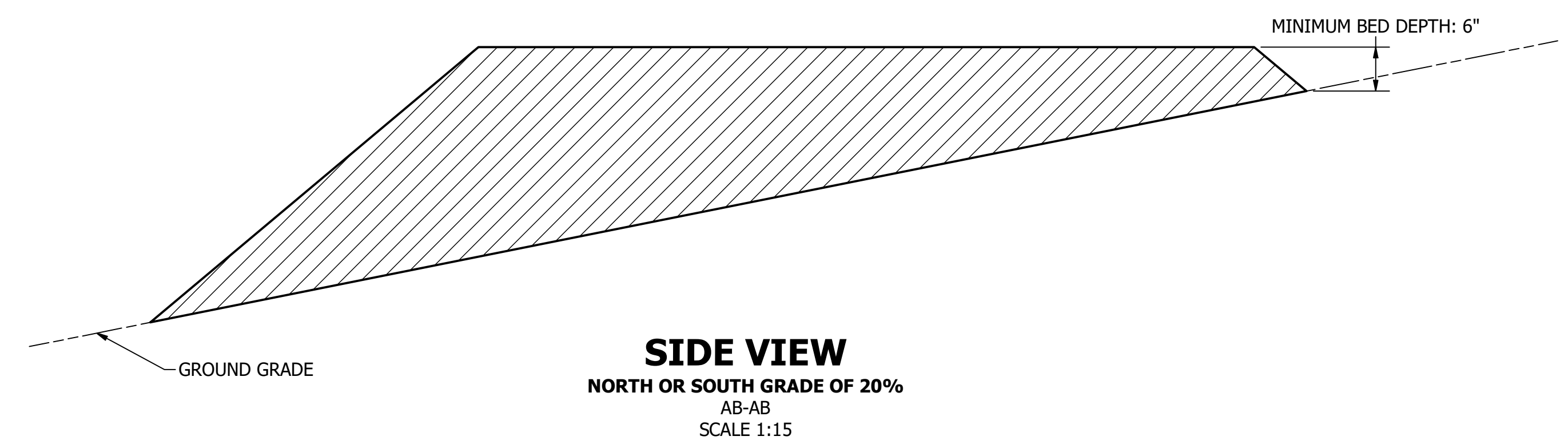
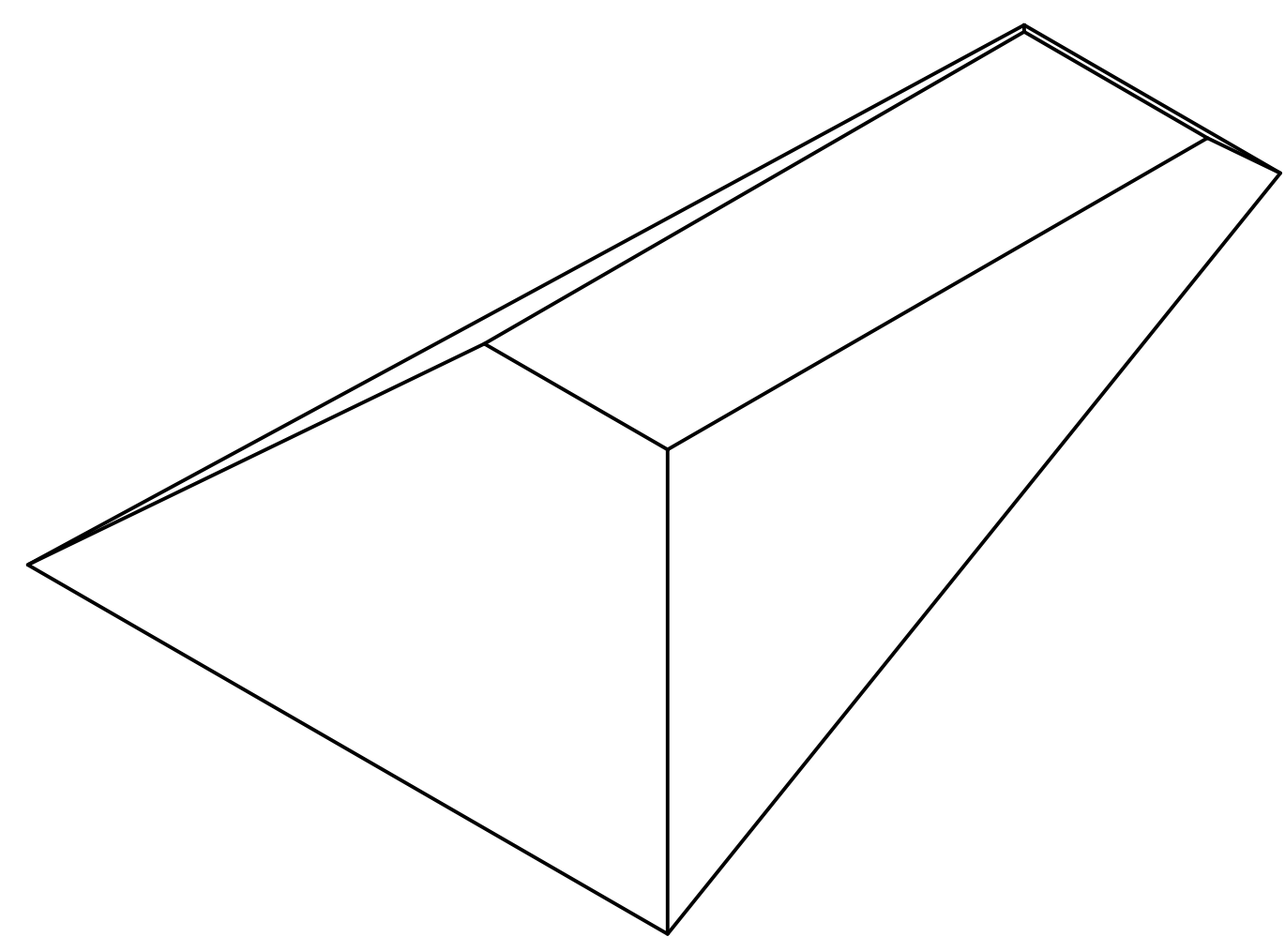
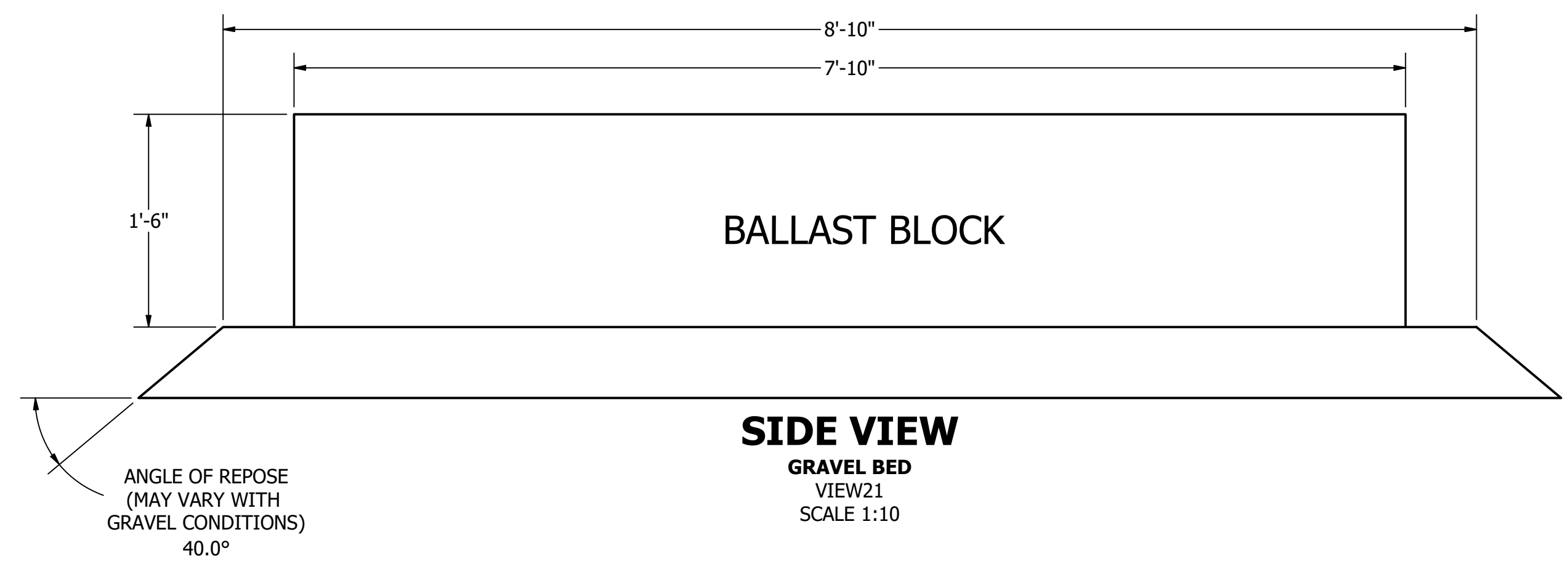
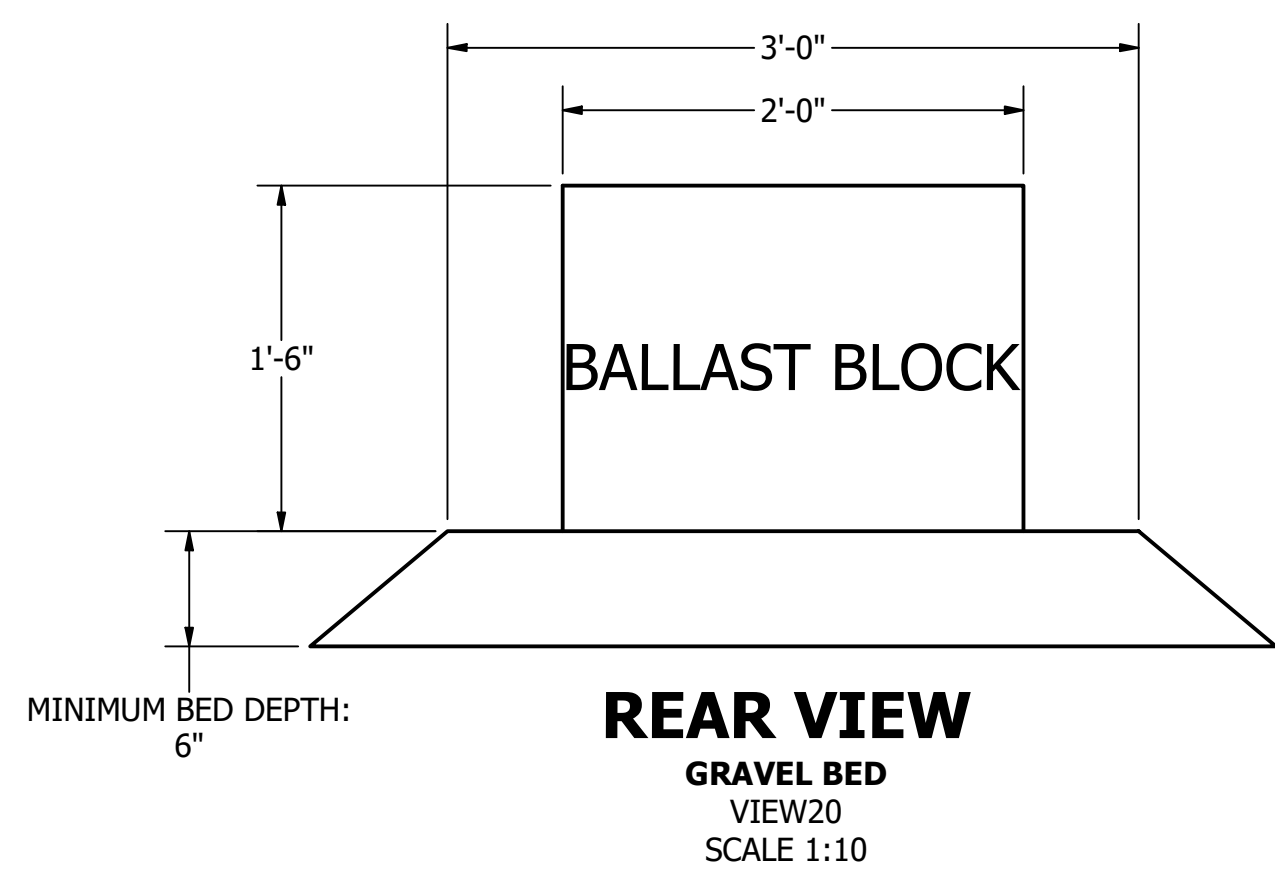


Material:	9066.549 lbmass		
Weight:	9066.549 lbmass		
Description:	CT-LS-DC-B, LONGI LR5-72HBD-540M, 2x9, 25 DEG, EVERSOURCE-MORRIS, CT - SOUTH SECTION LS-B, GREENSKIES		
Project:	EVERSOURCE-MORRIS, CT - SOUTH SECTION LS-B		
Drawn:	CPATTERSON	Date:	3/9/2026
Scale:	7 of 9		

Format:	D	Part Number:	6778	Rev:	3
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 Phone: 1-704-659-7474

8 7 6 5 4 3 2 1



**PROJECT INFORMATION**

INSTALLATION ADDRESS:  
54 South St, Morris, CT 06763

**Structural General Notes**

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**Snow Loads:**

- Ground Snow Load  $p_g = 35$  psf
- Importance Factor  $I_s = 0.8$
- Exposure Factor  $C_e = 0.9$
- Slope Snow Load  $p_s = 17.32$  psf

**Wind Loads:**

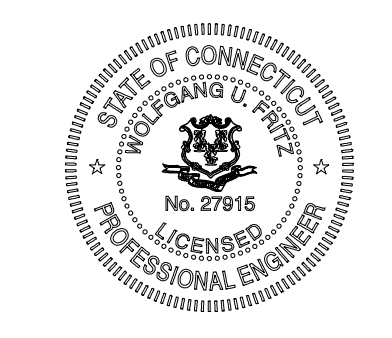
- MRI Factor = 1.00
- Basic Wind Speed  $V = 110$  mph
- $I_w = 1$
- Exposure = C
- Wind Design performed in accordance with the requirements of ASCE - Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.

**Seismic Loads:**

- $SS = 0.183g$ ,  $S1 = 0.054g$
- Site Class = D
- $SDS = 0.200g$ ,  $SD1 = 0.090g$
- Seismic Design Category = B
- Ordinary Steel Cantilever Column System

- Material strengths:
  - Hot-rolled structural steel ASTM A992 GR50.
  - Cold Formed Steel Sections comply w/ASTM A1003, structural grade, galvanized to Grade as noted.
  - Formed Steel Brackets - ASTM A653 Galvanized Grade 50 SS
  - I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85
  - Plate - A36 Steel, Hot Dip Galvanized
  - Connectors - Stainless Steel unless otherwise noted.
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- Scope of work by Structural Engineer includes member design, connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.

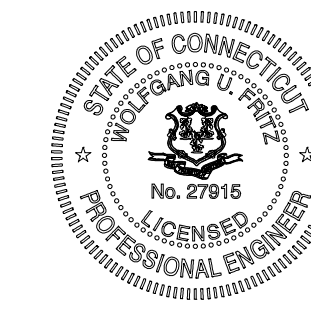
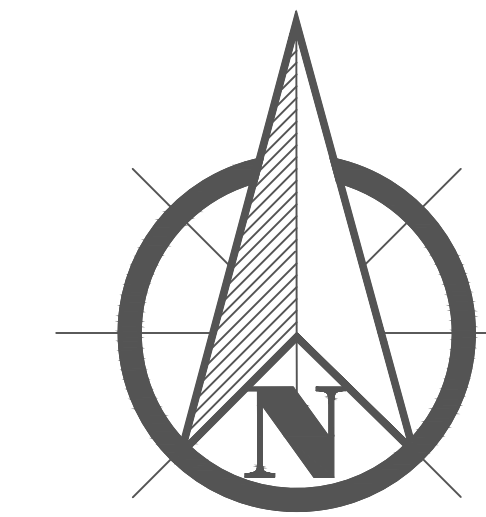
Engineer of Record



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	<b>CT-LS-DC-B, LONGI LR5-72HBD-540M, 2x9, 25 DEG, EVERSOURCE-MORRIS, CT - SOUTH SECTION LS-B, GREENSKIES</b>	
Project: EVERSOURCE-MORRIS, CT - SOUTH SECTION LS-B	Date: 3/9/2026	Sheet: 8 of 9
Drawn: CPATTERSON	Scale:	Part Number: <b>GRAVEL BED WITH BLOCK</b>
Format: D	Rev: 3	

8 7 6 5 4 3 2 1

# Contour™



PROJECT INFORMATION	
PROJECT NAME	EVERSOURCE-MORRIS, CT
INSTALLATION ADDRESS	54 SOUTH ST, MORRIS, CT 06763
CLIENT	GREENSKIES

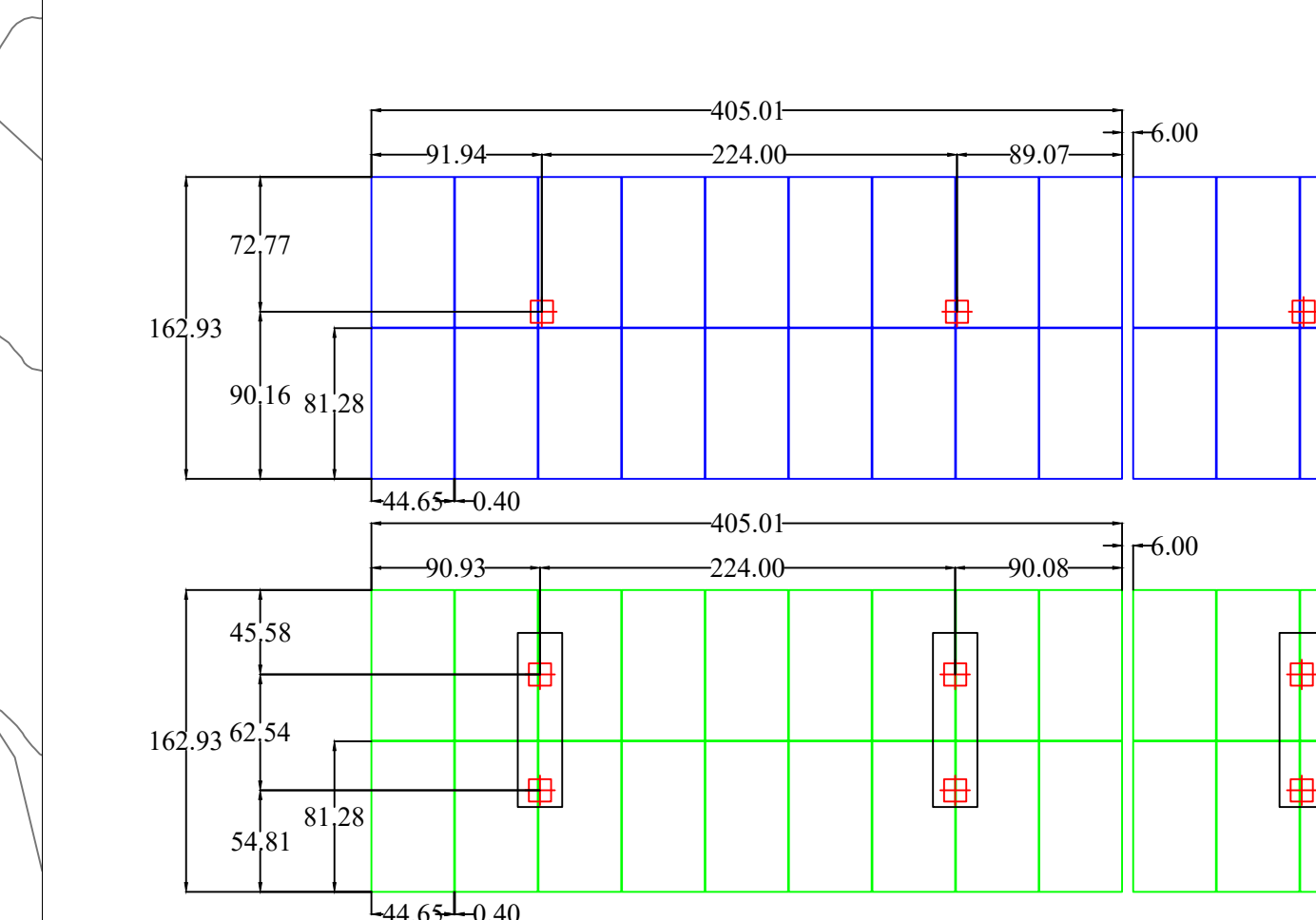
SITE SPECIFICATION		PANEL SPECSHEET
WIND SPEED (MPH)	106 ASCE 7-16	
SNOW LOAD (PSF)	35 ASCE 7-16	
EXPOSURE CATEGORY	C ASCE 7-16	
RISK CATEGORY	1 ASCE 7-16	

PANEL SPECIFICATION	
MODEL	LONGI LR5-72HBD-540M
LENGTH (mm)	2278
WIDTH (mm)	1134
WEIGHT (lb)	70.1
PANEL WATTAGE (W)	540
PROJECT PANEL COUNT	10,944

SYSTEM INFORMATION	
ARRAY CONFIGURATION	2X9
SYSTEM SIZE (W)	5,909,760
ARRAY TILT (°)	25
GROUND CLEARANCE (in)	36

ARRAY DETAILS	
ITEM	QUANTITY
2X9 DRIVEN BEAM TABLES	260
2X9 BALLASTED TABLES	348
POSTS	520
BALLAST BLOCKS	696
ALTERNATE ARRAY CONNECTIONS	110

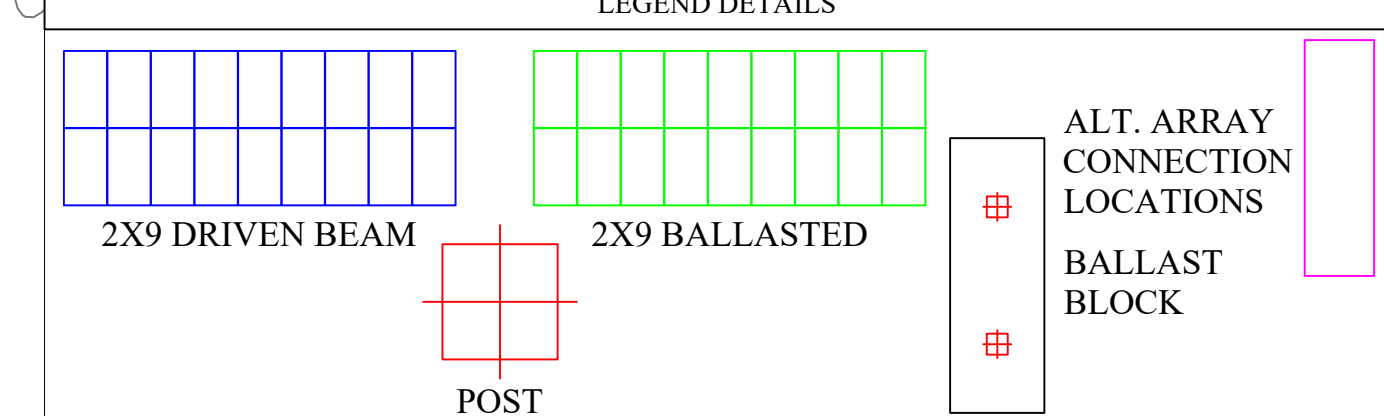
CAD BLOCK



GENERAL NOTES

1. \*\*PROPRIETARY AND CONFIDENTIAL\*\*  
THIS DRAWING AND ALL INFORMATION THERE IN IS THE PROPERTY OF DCE SOLAR AND IS CONFIDENTIAL AND MUST NOT BE MADE PUBLIC OR COPIED UNLESS AUTHORIZED BY DCE SOLAR AND IS SUBJECT TO RETURN UPON REQUEST.
2. THE CONTRACTOR WILL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND SHALL AT ALL TIMES TAKE REASONABLE PRECAUTIONS FOR THE SAFETY OF ITS EMPLOYEES ON THE PROJECT AND SHALL COMPLY WITH ALL APPLICABLE PROVISIONS OF FEDERAL, STATE, AND MUNICIPAL SAFETY LAWS AND BUILDING CONSTRUCTION CODES.
3. CUSTOMER PROVIDED SITE LAYOUTS WERE USED TO GENERATE THE LAYOUT AS SHOWN.
4. ANY CHANGES TO THE LAYOUT SHOWN THAT MAY CAUSE ERRORS DURING INSTALLATION ARE NOT THE RESPONSIBILITY OF DCE SOLAR.

LEGEND DETAILS



REVISION NOTES

REV	DESCRIPTION	PREPARED BY	DATE
0	GROUND MOUNT LAYOUT	CPATTERSON	2/11/2026
1	REVISED ALT. FOUNDATION NOTE ON SDD	CPATTERSON	2/19/2026
2	ADDED REBAR TO BALLAST BLOCKS ON SDD	CPATTERSON	3/5/2026
3	REVISED LAYOUT	CPATTERSON	3/9/2026
4			

