### <u>EXHIBIT D</u>

Stormwater Management Report

### PROJECT NARRATIVE & STORMWATER REPORT

For the Proposed:

### **SOLAR PHOTOVOLTAIC ARRAY**

Located At: Riggs Street Oxford, Connecticut

Prepared On: January 15<sup>th</sup>, 2024

Prepared For:



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#### **INTRODUCTION**

At the request of TRITEC Americas, LLC (Petitioner), Solli Engineering (Solli) has prepared this Stormwater Management Report to provide an analysis of the potential stormwater impacts associated with the proposed  $4.64\pm$  megawatt (MW) alternating current (AC) ground-mounted solar electric generating facility (Project/Facility) located at Riggs Street, Oxford, Connecticut (Site). The proposed stormwater management plan outlined herein has been designed accordance with the following State of Connecticut guidelines as well as other applicable state and federal requirements and regulations:

- General Permit for the Discharge of Stormwater and Dewatering Wastewaters from Construction Activities (Effective Date: December 31, 2020, Modification Date: November 25, 2022)
- Connecticut Stormwater Quality Manual (Publication Date: September 30, 2023, Effective Date: March 30, 2024)
- Connecticut Guidelines for Soil Erosion and Sediment Control (Publication Date: September 30, 2023, Effective Date: March 30, 2024)
- Connecticut Department of Transportation 2000 Drainage Manual
- CT DEEP Appendix I Stormwater Management at Solar Array Construction Projects

#### **EXISTING SITE CONDITIONS**

The Site consists of one (1) parcel totaling  $65.87\pm$  acres located on Riggs Street, Oxford, Connecticut. The Site is bound by residential uses to the west, north, and south, and Riggs Street to the east. The entire parcel is comprised of vacant land, consisting of wooded and wetland areas with two (2) streams, an unnamed stream and Jacks Brook, that flow from north to south through the property and divides the site into three (3) main project areas.

The east Project area's topography has a ridge line that splits the area. Slopes from the ridgeline to the east range from 10%-50% and drains into Riggs Street. Slopes from the ridgeline to the west range from 10%-50% and drains into Jacks Brook. The central Project area's topography slopes between 5%-50% from the north property line of the site to the south, to the east into Jacks Brook and to the west to the unnamed stream. The west Project area's topography slopes between 3%-30% from the north property line to the south. The wetlands that are onsite are associated with the two (2) streams that flow through the site. Some work will be required within the 50' wetland buffer for access to the central project area. All other proposed work will remain outside of the 50' wetland buffer area for the other wetlands.

For more information regarding the Site, refer to the Property & Topographic Survey Map and the Field Data Location Map in Appendix A.

#### PROPOSED SITE CONDITIONS

The total proposed Project area is  $29.95\pm$  acres, within wooded areas within the Site. The total project is broken up into three (3) solar arrays. Access to the eastern and central array will be provided at the southeastern edge of the Site, from Riggs Street, via a new 12' wide,  $1,500'\pm$  long gravel road. The access drive off Riggs Street is proposed to cross over Jacks Brook to access the central array. A 20'-4'' wide x 4'-6'' high x 27' long open bottom aluminum box culvert is proposed to span Jacks Brook. Access to the western array will be provided at the southwestern edge of the site from Seymour Southbury Road (Rte 67), via a new 12' wide,  $695'\pm$  long gravel road. The three (3) arrays will be surrounded by a 7-ft tall chain link fence to provide adequate security measures.



As currently designed, the proposed Facility will consist of 11,970 TrinaSolar TSM-DEG19C20 540W modules. The modules will be installed on a post-driven ground-mounted, single-axis tracking system, with no anticipated changes to the existing grades within the array, therefore the post-development site conditions will mimic the pre-development site conditions to the maximum extent possible. As discussed later in this report, perimeter grassed swales with check dams and stormwater basins are proposed to assist in mitigating peak runoff flows, as well as to treat the Water Quality Volume (WQv) per CT DEEP requirements.

The 20'-4" wide x 4'-6" high x 27' long open bottom aluminum box culvert and the 100-year streams flows for Jacks Brook were inputted into the Federal Highway Administration's HY-8 culvert analysis software. The flows from Jacks Brook were obtained from USGS StreamStats website. The proposed culvert passes the 100-year storm while providing at least 1' of freeboard.

For more information regarding the Project, refer to the Site Layout Plan (Sheet 2.10 – Sheet 2.16) in Appendix A.

### STORMWATER MANAGEMENT

The Project will add approximately 32,500 square feet of impervious/gravel area. Due to some proposed modules being located on slopes greater than 15%, approximately 94,800 square feet of the modules will also be considered impervious area for the calculation of the water quality volume, per CT DEEP Appendix I requirements. The proposed stormwater management design consists of stormwater basins and drainage swales providing adequate storage for the water quality volume (WQv) that will effectively clean and treat the stormwater runoff prior to discharging from the basins.

#### METHODOLOGY

A hydrologic analysis was performed using the HydroCAD stormwater modeling system computer program developed by HydroCAD Software Solutions, LLC. Hydrographs for each watershed were developed using the SCS Synthetic Unit Hydrograph Method with a NRCC 24-hr Type D rainfall distribution.

Rainfall depths for the site were used for calculating the volumes and rates of runoff for this project. The depths were taken from the NOAA Atlas documents (Latitude: 41.4438°, Longitude: -73.1205°) and the rainfall values are listed in Table 1 below.

Table 1. Kamian Data			
<b>Return Period (Storm Event)</b>	24-hr Rainfall Depth (inches)		
2-year	3.62		
25-year	6.90		
50-year	7.83		
100-year	8.85		

Table	1:	Rainfall	Data
Lanc		ixaiiiiaii	Data

The drainage areas used in the calculations are illustrated on the Existing and Proposed Drainage Area Maps (DA-1 & DA-2). These maps and the corresponding Hydrocad output are attached in Appendices B. Utilizing CT DEEP Appendix I, this hydrologic analysis will reflect a reduction of the Hydrologic Soil Group ("HSG") present on-site by a half (1/2) step (e.g., half the difference between the runoff curve number for HSG A versus HSG B). This reduction, as indicated by CT DEEP, is intended to account for the compaction of soils that results from extensive machinery traffic during construction of the array. The



Water Quality Volume ("WQV") for the site will be calculated assuming that the gravel surfaces and concrete equipment pads are effectively impervious cover.

#### **EXISTING CONDITIONS**

Approximately 84.93 acres of Off-Site and On-Site area were analyzed for stormwater management purposes. The areas analyzed contain the contributing areas which directly impact and are impacted by the proposed redevelopment. Based on existing drainage patterns, the 84.93-acre area was considered as four (4) contributing drainage areas, labeled as Existing Drainage Area 1 (EDA-1), Existing Drainage Area 2 (EDA-2), Existing Drainage Area 3 (EDA-3) and Existing Drainage Area 4 (EDA-4) with two (2) Analysis Points (AP-1 and AP-2). AP-1 is the southern property line at an unnamed stream. AP-2 is the southern property line where Jacks Brook and Riggs Street Brook converge.

EDA-1 has a contributing wooded drainage area of approximately 3.57 acres. The majority of the runoff from EDA-1 flows from north to southwest overland and off-site along the southwestern property line.

EDA-2 has a contributing wooded drainage area of approximately 32.95 acres. The majority of the runoff from EDA-2 flows from east and west overland into an unnamed stream which flows from north to south through the drainage area and discharges at the southern property line.

EDA-3 has a contributing wooded drainage area of approximately 32.95 acres. The majority of the runoff from EDA-3 flows from east and west overland into Jacks Brook which flows from north to south through the drainage area and discharges into Riggs Street Brook at the southern property line.

EDA-4 has a contributing wooded drainage area of approximately 4.26 acres. The majority of the runoff from EDA-4 flows from north to southeast overland and offsite along the eastern property line into Riggs Street and eventually into Riggs Street Brook.

Drainage Area Label	Drainage Area	Curve Number	Time of Concentration
Existing Drainage Area 1 (EDA-1)	3.57 AC	60	13.5 Min.
Existing Drainage Area 2 (EDA-2)	32.95 AC	70	16.6 Min.
Existing Drainage Area 3 (EDA-3)	44.15 AC	68	19.6 Min.
Existing Drainage Area 4 (EDA-4)	4.26 AC	74	15.8 Min.

**Table 2: Existing Drainage Areas** 

For more information regarding the existing drainage conditions of the project area refer to the Existing Drainage Area Map (DA-1) in Appendix A and the HydroCAD calculations in Appendix B.

#### **PROPOSED CONDITIONS**

The Project proposes five (5) stormwater basins with forebays that will provide storage for reduction in peak flows and WQv. Based on the proposed drainage patterns, the 84.93-acre area was divided into nine (9) contributing drainage areas, Proposed Drainage Area 1 (PDA-1), Proposed Drainage Area 2A (PDA-2A), Proposed Drainage Area 2B (PDA-2B), Proposed Drainage Area 2C (PDA-2C), Proposed Drainage Area 2D (PDA-2D), Proposed Drainage Area 3A (PDA-3A), Proposed Drainage Area 3B (PDA-3B), Proposed Drainage Area 3C (PDA-3C) and Proposed Drainage Area 4 (PDA-4).



PDA-1 has a contributing drainage area of approximately 2.49 acres. Similar to existing conditions, the majority of the runoff from PDA-1 from north to southwest overland and off-site along the southwestern property line.

PDA-2A has a contributing drainage area of approximately 24.11 acres. Similar to existing conditions, the majority of the runoff from PDA-2A flows from east and west overland into an unnamed stream which flows from north to south through the drainage area and discharges at the southern property line.

PDA-2B has a contributing drainage area of approximately 4.75 acres. The runoff flows south overland and into the proposed stormwater basin P-1. Stormwater eventually exits the basin through a proposed outlet control structure and emergency spillway and then flows off site along the southwestern property line.

PDA-2C has a contributing drainage area of approximately 1.88 acres. The runoff flows east overland and into the proposed stormwater basin P-2. Stormwater eventually exits the basin through a proposed outlet control structure and emergency spillway and then flows overland into an unnamed stream.

PDA-2D has a contributing drainage area of approximately 2.69 acres. The runoff flows south overland and into the proposed stormwater basin P-3. Stormwater eventually exits the basin through a proposed outlet control structure and emergency spillway and then flows overland into an unnamed stream.

PDA-3A has a contributing drainage area of approximately 34.35 acres. Similar to existing conditions, the majority of the runoff from PDA-3A flows from east and west overland into Jacks Brook which flows from north to south through the drainage area and discharges into Riggs Street Brook at the southern property line.

PDA-3B has a contributing drainage area of approximately 4.79 acres. The runoff flows south overland and into the proposed stormwater basin P-4. Stormwater eventually exits the basin through a proposed outlet control structure and emergency spillway and then flows overland into Riggs Street Brook.

PDA-3C has a contributing drainage area of approximately 5.62 acres. The runoff flows south overland and into the proposed stormwater basin P-5. Stormwater eventually exits the basin through a proposed outlet control structure and emergency spillway and then flows overland into Jacks Brook.

PDA-4 has a contributing drainage area of approximately 4.26 acres. Similar to existing conditions, The majority of the runoff from PDA-4 flows from north to southeast overland and discharges into Riggs Street and eventually into Riggs Street Brook.

All proposed areas of disturbance within the solar array will be seeded with a Fuzz & Buzz Mix – ERNMX-147 or approved equal.

Drainage Area Label	Drainage Area	Curve Number	Time of Concentration
Proposed Drainage Area 1 (PDA-1)	2.493 AC	60	13.0 Min.
Proposed Drainage Area 2A (PDA-2A)	24.113 AC	72	16.6 Min.
Proposed Drainage Area 2B (PDA-2B)	4.751 AC	64	14.8 Min.
Proposed Drainage Area 2C (PDA-2C)	1.88 AC	71	7.3 Min.
Proposed Drainage Area 3A (PDA-3A)	34.353 AC	68	19.6 Min.
Proposed Drainage Area 3B (PDA-3B)	4.786 AC	65	16.2 Min.
Proposed Drainage Area 3C (PDA-3C)	5.622 AC	75	14.7 Min.
Proposed Drainage Area 4 (PDA-4)	4.259 AC	75	15.8 Min.

#### **Table 3: Proposed Drainage Areas**



For more information regarding the proposed stormwater management design of the Project area refer to the Proposed Drainage Area Map (DA-2) in Appendix A; and the Hydrocad and Water Quality Volume calculations in Appendix B.

As a result of the proposed stormwater management measures, the peak flows for the 2, 25, 50 and 100year storm events are reduced from existing conditions as shown in the chart below.

AP-1 - Peak Flow (cfs)			
Storm Event	Total Drai	Percent Reduction in	
Storm Event	Existing	Proposed	Peak Flow
2-Year	26.91	22.14	17.7%
25-Year	96.65	88.31	8.6%
50-Year	118.54	108.01	8.9%
100-Year	143.04	133.77	6.5%

AP-2 - Peak Flow (cfs)			
Storm Event —	Total Drai	Percent Reduction in	
	Existing	Proposed	Peak Flow
2-Year	31.60	25.86	18.2%
25-Year	116.36	105.08	9.7%
50-Year	143.11	135.30	5.5%
100-Year	173.09	168.78	2.5%

#### CT DEEP APPENDIX I DESIGN REGULATIONS/COMPLIANCE

The following identifies and details the regulations and proposed compliance measures within CT DEEP Appendix I that pertain specifically to civil, stormwater, and erosion control designs.

- *I. Design and construction requirements:*
- 1. Roadways, gravel surfaces, transformer pads are considered effective impervious cover for the purposes of calculating the WQV. The proposed solar panels in the array that are within existing and post-construction slopes that are greater than 15% are considered impervious for the purposes of calculating the WQV. The remainder of the proposed solar panels that are proposed within existing and post-construction slopes that are less than 15% are not considered impervious cover for the purposes of calculating the WQV because the following have been met:
  - a. Vegetative areas between the rows of solar panels have a width of 9 feet which is greater than the solar panel width of 7.8 feet.
  - b. The post-development stormwater runoff will be less than that of the pre-development stormwater runoff due to the proposed grassed swales and stormwater management basin.
  - c. The Project meets (iv) of this requirement as the plan includes specific engineered phased construction plans and detailed erosion control measures.
  - d. The panels are spaced and provide a minimum height of 3 feet from the ground to provide growth of native vegetation.
- 2. Setback and buffer requirements have been met following the below:



- a. No wetlands or waters are located within 100 feet of the proposed solar facility area. No solar panels are located within the 50-foot setback of any property boundary that is located downgradient of the construction activity.
- b. There is a minimum of 50 feet between the limit of construction activity and downgradient wetlands.
- c. There is a minimum of 10 feet between the construction activity associated with the installation of the access road and interconnection and downgradient wetlands.
- 3. The wetlands and water courses were originally delineated by Soil Science and Environmental Services Inc. around 2014. The location of delineated resources, as well as buffers, are shown on the Site Layout Plans (Sheets 2.11-2.16) in Appendix A.
- *II.* Design requirements for post-construction stormwater management measures:
- 1. Post-construction stormwater control measures have been designed and will be constructed to provide permanent stabilization and non-erosive conveyance of runoff from the site.
- 2. The orientation of the panels follows the existing slopes on the site to the extent practicable.
- 3. The hydrologic analysis has been completed, as described above, with the following details:
  - a. The Project evaluates and controls the 2, 25, 50, and 100-year 24-hour rainfall events in accordance with the CT Stormwater Quality Manual. Maximum sheet flow was kept to 100 feet and shallow concentrated flows were calculated using velocity factors per NRCS Part 630 National Engineering Handbook Chapter 15. The proposed swales have been to convey and control stormwater from a 100-year, 24-hr rainfall event.
  - b. NRCS soil mapping was used for the stormwater design.
  - c. There are no areas within the solar arrays where the grades will change by more than two (2) feet from existing conditions. With the modeled half-drop (1/2) in HSG for the facility area and the change in curve number associated with the ground cover change from existing to proposed conditions, there will be a decrease in post-development runoff in comparison to pre-development runoff.
  - d. Pre-and post-development drainage area maps & computations are provided in Appendices A and B.
  - e. The information above and herein demonstrates that the Project will have no net increase in peak flows, erosive velocities or volumes, or adverse impacts to downstream properties.

### SOIL EROSION & SEDIMENT CONTROL

The proposed plans for soil erosion and sediment control prepared for this project have been developed in accordance with the Connecticut Guidelines for Soil Erosion and Sediment Control, prepared by the Connecticut Council on Soil and Water Conservation in Collaboration with the Connecticut Department of Energy and Environmental Protection.

The soil erosion and sediment control measures that will be proposed as part of this project include geotextile silt fences with wings for areas less than 1 acre, compost filter socks, construction entrance, dust control measures, and a temporary sediment trap. The soil erosion and sediment control measures will be implemented in two (2) phases. Phase I measures are associated with the clearing, grubbing and installation of the sediment trap and diversion swales. Phase II measures are associated with the remain clearing and grubbing, fine grading and installation of the modules, hardscape, and utilities infrastructure.



### **CONCLUSION**

The stormwater management for the proposed site has been designed such that the post-development peak discharges to the waters of the State of Connecticut for the 2-, 25-, 50-, and 100- year storm events are less than the pre-development peak discharges. In addition, the Project adheres to the regulations and guidelines presented by CT DEEP's Appendix I as described above. As a result, the proposed solar array will not result in any adverse conditions to the surrounding areas and properties.



## **APPENDICES**

Appendix A – Figures Appendix B – Stormwater Calculations

### **Appendix A – Figures**

Perimeter Survey Map Prepared for The William L. Ives Revocable Trust, Riggs Street, Oxford, Connecticut J. Edwards Associates, LLC NRCS Soil Survey Map Site Layout Plans (2.10-2.16) Existing Drainage Area Map (DA-1) Proposed Drainage Area Map (DA-2)



FILE NO. 2129-DTL\_200



USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey





### Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
3	Ridgebury, Leicester, and Whitman soils, 0 to 8 percent slopes, extremely stony	D	26.5	12.4%
13	Walpole sandy loam, 0 to 3 percent slopes	B/D	3.1	1.5%
17	Timakwa and Natchaug soils, 0 to 2 percent slopes	B/D	0.5	0.2%
38C	Hinckley loamy sand, 3 to 15 percent slopes	А	2.3	1.1%
47C	Woodbridge fine sandy loam, 3 to 15 percent slopes, extremely stony	C/D	0.9	0.4%
52C	Sutton fine sandy loam, 2 to 15 percent slopes, extremely stony	B/D	1.8	0.8%
60C	Canton and Charlton fine sandy loams, 8 to 15 percent slopes	В	3.1	1.4%
60D	Canton and Charlton soils, 15 to 25 percent slopes	В	2.8	1.3%
62C	Canton and Charlton fine sandy loams, 3 to 15 percent slopes, extremely stony	В	6.8	3.2%
73C	Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky	В	58.5	27.4%
73E	Charlton-Chatfield complex, 15 to 45 percent slopes, very rocky	В	29.4	13.7%
75E	Hollis-Chatfield-Rock outcrop complex, 15 to 45 percent slopes	D	30.9	14.4%
84B	Paxton and Montauk fine sandy loams, 3 to 8 percent slopes	с	4.5	2.1%
84C	Paxton and Montauk fine sandy loams, 8 to 15 percent slopes	С	4.6	2.2%

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
84D	Paxton and Montauk fine sandy loams, 15 to 25 percent slopes	С	16.8	7.9%
86C	Paxton and Montauk fine sandy loams, 3 to 15 percent slopes, extremely stony	С	9.0	4.2%
102	Pootatuck fine sandy loam	В	4.1	1.9%
103	Rippowam fine sandy loam	B/D	0.8	0.4%
229B	Agawam-Urban land complex, 0 to 8 percent slopes	В	0.2	0.1%
306	Udorthents-Urban land complex	В	5.9	2.8%
308	Udorthents, smoothed	С	0.7	0.3%
W	Water		0.6	0.3%
Totals for Area of Inter	est		213.6	100.0%



### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

### **Rating Options**

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher



### SITE PLAN NOTES

- EXISTING BOUNDARY, TOPOGRAPHY AND SITE CONDITIONS INFORMATION TAKEN FROM A PLAN
- ALL CONSTRUCTION SHALL COMPLY WITH TOWN OF OXFORD STANDARDS, CONNECTICUT DEPARTMENT OF TRANSPORTATION STANDARDS, CONNECTICUT DEPARTMENT OF ENERGY AND ENVIRONMENTAL PROTECTION STANDARDS, AND SPECIFICATIONS IN THE ABOVE REFERENCED SHALL APPLY. ALL CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE OSHA, FEDERAL, STATE AND LOCAL REGULATIONS.
- GOVERNMENT AGENCIES PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL OBTAIN ALL COUNTY AND TOWN CONSTRUCTION PERMITS. THE CONTRACTOR SHALL POST ALL BONDS, PAY ALL FEES, PROVIDE PROOF OF INSURANCE AND PROVIDE TRAFFIC CONTROL NECESSARY FOR THIS WORK.
- OF RECORD IF THERE ARE ANY QUESTIONS OR CONFLICTS REGARDING THE CONSTRUCTION DOCUMENTS AND/OR FIELD CONDITIONS SO THAT APPROPRIATE REVISIONS CAN BE MADE PRIOR TO BIDDING. ANY CONFLICT BETWEEN THE DRAWINGS SHALL BE CONFIRMED WITH THE OWNER'S CONSTRUCTION MANAGER PRIOR TO BIDDING.
- UNCOVERED DURING EXCAVATION, CONSULT THE ENGINEER OF RECORD IMMEDIATELY FOR DIRECTIONS BEFORE PROCEEDING FURTHER WITH WORK IN THIS AREA.
- IN WRITING BY THE OWNER AND THE LOCAL MUNICIPALITIES. INTERRUPTIONS SHALL ONLY OCCUR AFTER ACCEPTABLE TEMPORARY SERVICE HAS BEEN PROVIDED.
- SIDEWALKS, LANDSCAPED AREAS OR SIGNAGE DISTURBED DURING CONSTRUCTION TO THEIR ORIGINAL CONDITION OR BETTER, AS APPROVED BY THE ENGINEER OF RECORD.
- DURING CONSTRUCTION. THE ENGINEER OF RECORD HAS NO CONTRACTUAL DUTY TO CONTROL THE SAFEST METHODS OR MEANS OF THE WORK, JOB SITE RESPONSIBILITIES, SUPERVISION OR TO SUPERVISE SAFETY AND DOES NOT VOLUNTARILY ASSUME ANY SUCH DUTY OR RESPONSIBILITY.
- PROTECTION REQUIREMENTS. 1. ALTERNATIVE METHODS AND PRODUCTS OTHER THAN THOSE SPECIFIED MAY BE USED IF REVIEWED
- 2. INFORMATION ON EXISTING UTILITIES AND STORM DRAINAGE SYSTEMS HAS BEEN COMPILED FROM AVAILABLE INFORMATION INCLUDING UTILITY PROVIDER AND MUNICIPAL RECORD MAPS AND/OR SYSTEMS ARE SHOWN TO ALERT THE CONTRACTOR TO THEIR PRESENCE AND THE CONTRACTOR IS SOLELY RESPONSIBLE FOR DETERMINING ACTUAL LOCATIONS AND ELEVATIONS OF ALL UTILITIES AND STORM DRAINAGE SYSTEMS INCLUDING SERVICES. PRIOR TO DEMOLITION OR CONSTRUCTION,

# LEGEND





# SOLAR ARRAY SYSTEM INFORMATION

	TOTAL OUTPUT
SIZE DC	6.41 MW
SIZE AC	4.93 MW
INVERTER LOAD RATIO	1.30
MODULE TYPE	TRACKING TRINASOLAR TSM-540-DEG19C.20 (540W)
MODULE QUANTITY	11,865
INVERTER	SUNGROW SG125HV 125KW
UTILITY	EVERSOURCE



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Approved By:	KMS			
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Checked By:	EEL				
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# GENERAL NOTES

- THE STORMWATER MANAGEMENT PLAN AND DESIGN IS INTENDED TO BE IN COMPLIANCE WITH THE CONNECTICUT DEPARTMENT OF ENERGY AND ENVIRONMENTAL PROTECTION STORMWATER QUALITY MANUAL AND THE TOWN OF SUFFIELD, CONNECTICUT STORMWATER REGULATIONS.
   STORMWATER RUNOFF ANALYSIS WAS CALCULATED USING THE SCS TR-55 METHODOLOGY.

# LEGEND

 PROPERTY LINE
 RIGHT-OF-WAY LINE
 ADJOINING LOT LINE
LIMIT OF DRAINAGE AREA
 FLOW PATH

# EXISTING CONDITIONS PEAK FLOWS

ANALYSIS POINT	2-YEAR (CFS)	25-YEAR (CFS)	50-YEAR (CFS)	100-YEAR (CFS)
AP-1	26.91	96.65	118.54	143.04
AP-2	31.60	116.36	143.11	173.09

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Drawn By:	CSH	
Checked By:	EEL	
Approved By:	KMS	
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# GENERAL NOTES

- 1. THE STORMWATER MANAGEMENT PLAN AND DESIGN IS INTENDED TO BE IN COMPLIANCE WITH THE CONNECTICUT DEPARTMENT OF ENERGY AND ENVIRONMENTAL PROTECTION DEFARTMENT OF ENERGY AND ENVIRONMENTAL PROTECTION STORMWATER QUALITY MANUAL AND THE TOWN OF SUFFIELD, CONNECTICUT STORMWATER REGULATIONS.
  2. STORMWATER RUNOFF ANALYSIS WAS CALCULATED USING THE SCS TR-55 METHODOLOGY.

### LEGEND

 PROPERTY LINE
 RIGHT-OF-WAY LINE
 ADJOINING LOT LINE
LIMIT OF DRAINAGE AREA
 FLOW PATH

# PROPOSED CONDITIONS PEAK FLOWS

ANALYSIS POINT	2-YEAR (CFS)	25-YEAR (CFS)	50-YEAR (CFS)	100-YEAR (CFS)
AP-1	22.14	88.31	108.01	133.77
AP-2	25.86	105.08	135.30	168.78

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Drawn By:	CSH	
Checked By:	EEL	
Approved By:	KMS	
Project #:	22108801	
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		DA-2
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### **Appendix B – Stormwater Calculations**

<u>Hydrology Calculations (2-, 25-, 50-, 100-year storm events)</u> Water Quality Volume Calculations NOAA Atlas Precipitation Data USGS StreamStats Data HY-8 Culvert Crossing Report



Event#	Event Name	Storm Type	Curve	Mode	Duration (bours)	B/B	Depth (inches)	AMC
	Name				(nours)		(incries)	
1	2-yr	NRCC 24-hr	D	Default	24.00	1	3.62	2
2	25-yr	NRCC 24-hr	D	Default	24.00	1	6.90	2
3	50-yr	NRCC 24-hr	D	Default	24.00	1	7.83	2
4	100-yr	NRCC 24-hr	D	Default	24.00	1	8.85	2

### **Rainfall Events Listing**

### Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.220	98	Paved roads w/curbs & sewers, HSG C (EDA-4)
0.018	98	Paved roads w/curbs & sewers, HSG D (EDA-4)
0.515	36	Woods, Fair, HSG A (EDA-3)
38.277	60	Woods, Fair, HSG B (EDA-1, EDA-2, EDA-3)
23.267	73	Woods, Fair, HSG C (EDA-3, EDA-4)
22.633	79	Woods, Fair, HSG D (EDA-2, EDA-3, EDA-4)
84.930	69	TOTAL AREA

### Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.515	HSG A	EDA-3
38.277	HSG B	EDA-1, EDA-2, EDA-3
23.487	HSG C	EDA-3, EDA-4
22.651	HSG D	EDA-2, EDA-3, EDA-4
0.000	Other	
84.930		TOTAL AREA

### Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
 (acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.000	0.220	0.018	0.000	0.238	Paved roads w/curbs & sewers	ED
							A-4
0.515	38.277	23.267	22.633	0.000	84.692	Woods, Fair	ED
							A-1
							,
							ED
							A-2
							,
							ED
							A-3
							,
							ED
							A-4
0.515	38.277	23.487	22.651	0.000	84.930	TOTAL AREA	

Riggs St_Exist.	
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Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Width	Diam/Height	Inside-Fill	Node
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)	Name
1	EDA-4	0.00	0.00	885.0	0.0575	0.012	0.0	15.0	0.0	

Riggs St_Exist.	N
Prepared by Solli Engineering	
HydroCAD® 10 20-3g s/n 10621 © 2023 HydroCAD Softwar	e Solutions LLC

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: EDA-1	Runoff Area=3.573 ac 0.00% Impervious Runoff Depth=0.58" Flow Length=753' Tc=13.5 min CN=60 Runoff=1.26 cfs 0.174 af
SubcatchmentEDA-2: EDA-2	Runoff Area=32.950 ac 0.00% Impervious Runoff Depth=1.08" Flow Length=1,686' Tc=16.6 min CN=70 Runoff=25.66 cfs 2.974 af
SubcatchmentEDA-3: EDA-3	Runoff Area=44.148 ac 0.00% Impervious Runoff Depth=0.97" Flow Length=2,518' Tc=19.6 min CN=68 Runoff=27.61 cfs 3.575 af
SubcatchmentEDA-4: EDA-4	Runoff Area=4.259 ac 5.59% Impervious Runoff Depth=1.32" Flow Length=1,349' Tc=15.8 min CN=74 Runoff=4.28 cfs 0.470 af
Link AP-1: AP-1	Inflow=26.91 cfs 3.148 af Primary=26.91 cfs 3.148 af
Link AP-2: AP-2	Inflow=31.60 cfs 4.045 af Primary=31.60 cfs 4.045 af
Total Duraff Area - 9	4.020  as  Dun off  Volume = 7.402  of  Austerne Dun off Double = 4.0

Total Runoff Area = 84.930 ac Runoff Volume = 7.192 af Average Runoff Depth = 1.02" 99.72% Pervious = 84.692 ac 0.28% Impervious = 0.238 ac

0.174 af, Depth= 0.58"

### Summary for Subcatchment EDA-1: EDA-1

Runoff = 1.26 cfs @ 12.25 hrs, Volume= Routed to Link AP-1 : AP-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

Area	(ac) C	N Desc	cription		
3.	573 6	0 Woo	ds, Fair, H	ISG B	
3.	573	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	100	0.2000	0.21		Sheet Flow, A-B
5.5	653	0.1593	2.00		Woods: Light underbrush n= 0.400 P2= 3.62" <b>Shallow Concentrated Flow, B-C</b> Woodland Kv= 5.0 fps
13.5	753	Total			

### Subcatchment EDA-1: EDA-1

![](_page_35_Figure_7.jpeg)
### Summary for Subcatchment EDA-2: EDA-2

Runoff = 25.66 cfs @ 12.26 hrs, Volume= Routed to Link AP-1 : AP-1 2.974 af, Depth= 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

Area	a (ac)	CN	Desc	cription		
16	6.153	60	Woo	ds, Fair, H	ISG B	
16	6.797	79	Woo	ds, Fair, H	ISG D	
32	2.950	70	Weig	ghted Aver	age	
32	2.950		100.	00% Pervi	ous Area	
Tc	Lengt	h	Slope	Velocity	Capacity	Description
(min)	(fee	:)	(ft/ft)	(ft/sec)	(cfs)	
10.2	10	0 0	.1100	0.16		Sheet Flow, A-B
						Woods: Light underbrush n= 0.400 P2= 3.62"
4.7	63	50	.2000	2.24		Shallow Concentrated Flow, B-C
						Woodland Kv= 5.0 fps
1.7	95	1 0	.0421	9.10	151.99	Channel Flow, C-D
						Area= 16.7 sf Perim= 12.8' r= 1.30'
						n= 0.040 Winding stream, pools & shoals
10.0	4 00	~ -				

16.6 1,686 Total

### Subcatchment EDA-2: EDA-2



#### Summary for Subcatchment EDA-3: EDA-3

Runoff = 27.61 cfs @ 12.31 hrs, Volume= Routed to Link AP-2 : AP-2 3.575 af, Depth= 0.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

Area	(ac) (	CN De	scription		
0.	515	36 Wo	ods, Fair, F	ISG A	
18.	551	60 Wc	ods, Fair, F	ISG B	
19.	257	73 Wo	ods, Fair, F	ISG C	
5.	825	79 Wc	ods, Fair, F	ISG D	
44.	148	68 We	ighted Ave	rage	
44.	148	100	).00% Perv	ious Area	
Tc	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
8.8	100	0.1600	0.19		Sheet Flow, A-B
					Woods: Light underbrush n= 0.400 P2= 3.62"
8.1	735	0.0912	2 1.51		Shallow Concentrated Flow, B-C
					Woodland Kv= 5.0 fps
2.7	1,683	0.0552	10.50	346.60	Channel Flow, C-D
					Area= 33.0 sf Perim= 25.0' r= 1.32'
					n= 0.040 Winding stream, pools & shoals

19.6 2,518 Total

### Subcatchment EDA-3: EDA-3



### Summary for Subcatchment EDA-4: EDA-4

Runoff = 4.28 cfs @ 12.25 hrs, Volume= 0.470 af, Depth= 1.32" Routed to Link AP-2 : AP-2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

Area (	ac) C	N Des	cription				
4.0	4.010 73 Woods, Fair, HSG C						
0.0	)11 7	79 Woo	ods, Fair, F	ISG D			
0.2	220 9	8 Pave	ed roads w	/curbs & se	ewers, HSG C		
0.0	)18 9	8 Pave	ed roads w	/curbs & se	ewers, HSG D		
4.2	259 7	74 Weig	ghted Aver	age			
4.0	)21	94.4	1% Pervio	us Area			
0.2	238	5.59	% Impervi	ous Area			
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
12.2	100	0.0700	0.14		Sheet Flow, A-B		
					Woods: Light underbrush n= 0.400 P2= 3.62"		
2.3	301	0.1827	2.14		Shallow Concentrated Flow, B-C		
					Woodland Kv= 5.0 fps		
0.2	63	0.0635	5.12		Shallow Concentrated Flow, C-D		
					Paved Kv= 20.3 fps		
1.1	885	0.0575	13.67	16.78	Pipe Channel, D-E		
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
					n= 0.012 Corrugated PP, smooth interior		
15.8	1,349	Total					

Hydrograph Runoff 4.28 cfs NRCC 24-hr D 4 2-yr Rainfall=3.62" Runoff Area=4.259 ac Runoff Volume=0.470 af 3-Flow (cfs) Runoff Depth=1.32" Flow Length=1,349' 2-Tc=15.8 min CN=74 1 0 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 5 Time (hours)

### Subcatchment EDA-4: EDA-4

## Summary for Link AP-1: AP-1

Inflow /	Area =	36.523 ac,	0.00% Impervious,	Inflow Depth = 1.0	03" for 2-yr event
Inflow	=	26.91 cfs @	12.26 hrs, Volume	= 3.148 af	
Primar	y =	26.91 cfs @	12.26 hrs, Volume	= 3.148 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs



### Link AP-1: AP-1

## Summary for Link AP-2: AP-2

Inflow /	Area =	48.407 ac,	0.49% Impervious, I	nflow Depth = 1.0	0" for 2-yr event
Inflow	=	31.60 cfs @	12.30 hrs, Volume=	4.045 af	
Primar	y =	31.60 cfs @	12.30 hrs, Volume=	4.045 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs



## Link AP-2: AP-2

Riggs St_Exist.	NRCC
Prepared by Solli Engineering	
HvdroCAD® 10.20-3g s/n 10621 © 2023 HvdroCAD Software Solutions	LLC

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: EDA-1	Runoff Area=3.573 ac 0.00% Impervious Runoff Depth=2.53" Flow Length=753' Tc=13.5 min CN=60 Runoff=7.38 cfs 0.754 af
SubcatchmentEDA-2: EDA-2	Runoff Area=32.950 ac 0.00% Impervious Runoff Depth=3.54" Flow Length=1,686' Tc=16.6 min CN=70 Runoff=89.51 cfs 9.708 af
SubcatchmentEDA-3: EDA-3	Runoff Area=44.148 ac 0.00% Impervious Runoff Depth=3.33" Flow Length=2,518' Tc=19.6 min CN=68 Runoff=103.95 cfs 12.249 af
SubcatchmentEDA-4: EDA-4	Runoff Area=4.259 ac 5.59% Impervious Runoff Depth=3.96" Flow Length=1,349' Tc=15.8 min CN=74 Runoff=13.16 cfs 1.404 af
Link AP-1: AP-1	Inflow=96.65 cfs 10.462 af Primary=96.65 cfs 10.462 af
Link AP-2: AP-2	Inflow=116.36 cfs 13.653 af Primary=116.36 cfs 13.653 af
Total Runoff Area = 8	84.930 ac Runoff Volume = 24.115 af Average Runoff Depth = 3.41"

99.72% Pervious = 84.692 ac 0.28% Impervious = 0.238 ac

### Summary for Subcatchment EDA-1: EDA-1

Runoff = 7.38 cfs @ 12.22 hrs, Volume= Routed to Link AP-1 : AP-1 0.754 af, Depth= 2.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

Area (a	c) C	N Dese	cription		
3.57	73 6	0 Woo	ds, Fair, H	ISG B	
3.573 100.00% Pervious Area					
Tc L (min)	.ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	100	0.2000	0.21		Sheet Flow, A-B
5.5	653	0.1593	2.00		Woods: Light underbrush n= 0.400 P2= 3.62" <b>Shallow Concentrated Flow, B-C</b> Woodland Kv= 5.0 fps
13 5	753	Total			

### Subcatchment EDA-1: EDA-1



#### Summary for Subcatchment EDA-2: EDA-2

Runoff = 89.51 cfs @ 12.25 hrs, Volume= Routed to Link AP-1 : AP-1 9.708 af, Depth= 3.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

Area	a (ac)	CN	Desc	cription		
16	6.153	60	Woo	ds, Fair, H	ISG B	
16	6.797	79	Woo	ds, Fair, H	ISG D	
32	2.950	70	Weig	ghted Aver	age	
32	2.950		100.	00% Pervi	ous Area	
Tc	Lengt	h	Slope	Velocity	Capacity	Description
(min)	(fee	:)	(ft/ft)	(ft/sec)	(cfs)	
10.2	10	0 0	.1100	0.16		Sheet Flow, A-B
						Woods: Light underbrush n= 0.400 P2= 3.62"
4.7	63	50	.2000	2.24		Shallow Concentrated Flow, B-C
						Woodland Kv= 5.0 fps
1.7	95	1 0	.0421	9.10	151.99	Channel Flow, C-D
						Area= 16.7 sf Perim= 12.8' r= 1.30'
						n= 0.040 Winding stream, pools & shoals
10.0	4 00	~ -				

16.6 1,686 Total

### Subcatchment EDA-2: EDA-2



#### Summary for Subcatchment EDA-3: EDA-3

103.95 cfs @ 12.29 hrs, Volume= Runoff Routed to Link AP-2 : AP-2

12.249 af, Depth= 3.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

Area	(ac) (	CN Des	cription				
0.	515	36 Woo	ods, Fair, ⊦	ISG A			
18.	551	60 Woo	ods, Fair, F	ISG B			
19.	257	73 Woo	Woods, Fair, HSG C				
5.	825	79 Woo	ods, Fair, F	ISG D			
44.	148	68 Wei	ghted Aver	age			
44.	148	100	.00% Pervi	ous Area			
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
8.8	100	0.1600	0.19		Sheet Flow, A-B		
					Woods: Light underbrush n= 0.400 P2= 3.62"		
8.1	735	0.0912	1.51		Shallow Concentrated Flow, B-C		
					Woodland Kv= 5.0 fps		
2.7	1,683	0.0552	10.50	346.60	Channel Flow, C-D		
					Area= 33.0 sf Perim= 25.0' r= 1.32'		
					n= 0.040 Winding stream, pools & shoals		

19.6 2,518 Total

### Subcatchment EDA-3: EDA-3



### Summary for Subcatchment EDA-4: EDA-4

Runoff = 13.16 cfs @ 12.24 hrs, Volume= 1 Routed to Link AP-2 : AP-2

1.404 af, Depth= 3.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

Area (	ac) C	N Des	cription				
4.0	4.010 73 Woods, Fair, HSG C						
0.0	0.011 79 Woods, Fair, HSG D						
0.2	220 9	98 Pave	ed roads w	/curbs & se	ewers, HSG C		
0.0	)18 9	98 Pave	ed roads w	/curbs & se	ewers, HSG D		
4.2	259 7	74 Weig	ghted Aver	age			
4.0	)21	94.4	1% Pervio	us Area			
0.2	238	5.59	% Impervi	ous Area			
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
12.2	100	0.0700	0.14		Sheet Flow, A-B		
					Woods: Light underbrush n= 0.400 P2= 3.62"		
2.3	301	0.1827	2.14		Shallow Concentrated Flow, B-C		
					Woodland Kv= 5.0 fps		
0.2	63	0.0635	5.12		Shallow Concentrated Flow, C-D		
					Paved Kv= 20.3 fps		
1.1	885	0.0575	13.67	16.78	Pipe Channel, D-E		
					15.0" Round Area= 1.2 st Perim= 3.9' r= 0.31'		
					n= 0.012 Corrugated PP, smooth interior		
15.8	1,349	Total					



### Subcatchment EDA-4: EDA-4

## Summary for Link AP-1: AP-1

Inflow /	Area =	36.523 ac,	0.00% Impervious,	Inflow Depth = 3.4	44" for 25-yr event
Inflow	=	96.65 cfs @	12.25 hrs, Volume	= 10.462 af	
Primar	y =	96.65 cfs @	12.25 hrs, Volume	= 10.462 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs



## Link AP-1: AP-1

## Summary for Link AP-2: AP-2

Inflow A	Area =	48.407 ac,	0.49% Impervious,	Inflow Depth = 3.3	38" for 25-yr event
Inflow	=	116.36 cfs @	12.29 hrs, Volume	= 13.653 af	
Primary	y =	116.36 cfs @	12.29 hrs, Volume	= 13.653 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs



## Link AP-2: AP-2

Riggs St_Exist.	NRCC 24-h
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Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: EDA-1	Runoff Area=3.573 ac 0.00% Impervious Runoff Depth=3.21" Flow Length=753' Tc=13.5 min CN=60 Runoff=9.46 cfs 0.955 af
SubcatchmentEDA-2: EDA-2	Runoff Area=32.950 ac 0.00% Impervious Runoff Depth=4.32" Flow Length=1,686' Tc=16.6 min CN=70 Runoff=109.39 cfs 11.858 af
SubcatchmentEDA-3: EDA-3	Runoff Area=44.148 ac 0.00% Impervious Runoff Depth=4.09" Flow Length=2,518' Tc=19.6 min CN=68 Runoff=128.15 cfs 15.058 af
SubcatchmentEDA-4: EDA-4	Runoff Area=4.259 ac 5.59% Impervious Runoff Depth=4.77" Flow Length=1,349' Tc=15.8 min CN=74 Runoff=15.84 cfs 1.694 af
Link AP-1: AP-1	Inflow=118.54 cfs 12.813 af Primary=118.54 cfs 12.813 af
Link AP-2: AP-2	Inflow=143.11 cfs 16.752 af Primary=143.11 cfs 16.752 af
Total Runoff Area =	84.930 ac Runoff Volume = 29.565 af Average Runoff Depth = 4.18"

99.72% Pervious = 84.692 ac 0.28% Impervious = 0.238 ac

### Summary for Subcatchment EDA-1: EDA-1

Runoff = 9.46 cfs @ 12.22 hrs, Volume= Routed to Link AP-1 : AP-1 0.955 af, Depth= 3.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

Area	(ac) C	N Dese	cription		
3.	573 6	0 Woo	ds, Fair, H	ISG B	
3.	573	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	100	0.2000	0.21		Sheet Flow, A-B
5.5	653	0.1593	2.00		Woods: Light underbrush n= 0.400 P2= 3.62" <b>Shallow Concentrated Flow, B-C</b> Woodland Kv= 5.0 fps
13.5	753	Total			

### Subcatchment EDA-1: EDA-1



### Summary for Subcatchment EDA-2: EDA-2

Runoff = 109.39 cfs @ 12.25 hrs, Volume= Routed to Link AP-1 : AP-1 11.858 af, Depth= 4.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

Area	(ac) (	CN Des	cription		
16.	.153	60 Wo	ods, Fair, ⊦	ISG B	
16.	.797	79 Wo	ods, Fair, H	ISG D	
32.	.950	70 Wei	ghted Aver	age	
32.	.950	100	.00% Pervi	ous Area	
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.2	100	0.1100	0.16		Sheet Flow, A-B
					Woods: Light underbrush n= 0.400 P2= 3.62"
4.7	635	0.2000	2.24		Shallow Concentrated Flow, B-C
					Woodland Kv= 5.0 fps
1.7	951	0.0421	9.10	151.99	Channel Flow, C-D
					Area= 16.7 sf Perim= 12.8' r= 1.30'
					n= 0.040 Winding stream, pools & shoals
10.0					

16.6 1,686 Total

### Subcatchment EDA-2: EDA-2



#### Summary for Subcatchment EDA-3: EDA-3

Runoff = 128.15 cfs @ 12.29 hrs, Volume= Routed to Link AP-2 : AP-2 15.058 af, Depth= 4.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

Area	(ac) (	CN De	scription		
0.	515	36 Wo	ods, Fair, F	ISG A	
18.	551	60 Wc	ods, Fair, F	ISG B	
19.	257	73 Wc	ods, Fair, F	ISG C	
5.	825	79 Wc	ods, Fair, F	ISG D	
44.	148	68 We	ighted Ave	rage	
44.	148	100	0.00% Perv	ious Area	
Tc	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
8.8	100	0.1600	0.19		Sheet Flow, A-B
					Woods: Light underbrush n= 0.400 P2= 3.62"
8.1	735	0.0912	1.51		Shallow Concentrated Flow, B-C
					Woodland Kv= 5.0 fps
2.7	1,683	0.0552	10.50	346.60	Channel Flow, C-D
					Area= 33.0 sf Perim= 25.0' r= 1.32'
					n= 0.040 Winding stream, pools & shoals

19.6 2,518 Total

### Subcatchment EDA-3: EDA-3



### Summary for Subcatchment EDA-4: EDA-4

Runoff = 15.84 cfs @ 12.24 hrs, Volume= 1 Routed to Link AP-2 : AP-2

1.694 af, Depth= 4.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

Area (	ac) C	N Des	cription		
4.0	4.010 73 Woods, Fair, HSG C				
0.0	0.011 79 Woods, Fair, HSG D				
0.2	220 9	8 Pave	ed roads w	/curbs & se	ewers, HSG C
0.0	)18 9	8 Pave	ed roads w	/curbs & se	ewers, HSG D
4.2	259 7	'4 Weig	ghted Aver	age	
4.0	)21	94.4	1% Pervio	us Area	
0.2	238	5.59	% Impervi	ous Area	
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.2	100	0.0700	0.14		Sheet Flow, A-B
					Woods: Light underbrush n= 0.400 P2= 3.62"
2.3	301	0.1827	2.14		Shallow Concentrated Flow, B-C
					Woodland Kv= 5.0 fps
0.2	63	0.0635	5.12		Shallow Concentrated Flow, C-D
					Paved Kv= 20.3 fps
1.1	885	0.0575	13.67	16.78	Pipe Channel, D-E
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.012 Corrugated PP, smooth interior
15.8	1,349	Total			

### Subcatchment EDA-4: EDA-4



## Summary for Link AP-1: AP-1

Inflow /	Area =	36.523 ac,	0.00% Impervious, I	nflow Depth = 4.2	1" for 50-yr event
Inflow	=	118.54 cfs @	12.25 hrs, Volume=	12.813 af	
Primary	y =	118.54 cfs @	12.25 hrs, Volume=	: 12.813 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs



### Link AP-1: AP-1

## Summary for Link AP-2: AP-2

Inflow /	Area =	=	48.407 ac,	0.49% Impervious,	Inflow Depth = $4.$	15" for 50-yr event
Inflow	=		143.11 cfs @	12.28 hrs, Volume	= 16.752 af	
Primar	y =		143.11 cfs @	12.28 hrs, Volume	= 16.752 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs



## Link AP-2: AP-2

Riggs St_Exist.	NRCC 2
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Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: EDA-1	Runoff Area=3.573 ac 0.00% Impervious Runoff Depth=3.98" Flow Length=753' Tc=13.5 min CN=60 Runoff=11.85 cfs 1.186 af
SubcatchmentEDA-2: EDA-2	Runoff Area=32.950 ac 0.00% Impervious Runoff Depth=5.20" Flow Length=1,686' Tc=16.6 min CN=70 Runoff=131.60 cfs 14.287 af
SubcatchmentEDA-3: EDA-3	Runoff Area=44.148 ac 0.00% Impervious Runoff Depth=4.96" Flow Length=2,518' Tc=19.6 min CN=68 Runoff=155.30 cfs 18.242 af
SubcatchmentEDA-4: EDA-4	Runoff Area=4.259 ac 5.59% Impervious Runoff Depth>5.69" Flow Length=1,349' Tc=15.8 min CN=74 Runoff=18.80 cfs 2.020 af
Link AP-1: AP-1	Inflow=143.04 cfs 15.473 af Primary=143.04 cfs 15.473 af
Link AP-2: AP-2	Inflow=173.09 cfs 20.263 af Primary=173.09 cfs 20.263 af
Total Runoff Area =	84.930 ac Runoff Volume = 35.735 af Average Runoff Depth = 5.05"

99.72% Pervious = 84.692 ac 0.28% Impervious = 0.238 ac

### Summary for Subcatchment EDA-1: EDA-1

Runoff = 11.85 cfs @ 12.22 hrs, Volume= Routed to Link AP-1 : AP-1 1.186 af, Depth= 3.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

Area (a	ac) C	N Dese	cription		
3.5	73 6	0 Woo	ds, Fair, H	ISG B	
3.5	73	100.	00% Pervi	ous Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	100	0.2000	0.21		Sheet Flow, A-B
5.5	653	0.1593	2.00		Woods: Light underbrush n= 0.400 P2= 3.62" <b>Shallow Concentrated Flow, B-C</b> Woodland Kv= 5.0 fps
13 5	753	Total			

### Subcatchment EDA-1: EDA-1



### Summary for Subcatchment EDA-2: EDA-2

Runoff = 131.60 cfs @ 12.25 hrs, Volume= 14.28 Routed to Link AP-1 : AP-1

14.287 af, Depth= 5.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

A	rea	(ac) (	CN I	Desc	cription		
	16.	153	60 V	Woo	ds, Fair, H	ISG B	
	16.	797	79	Woo	ds, Fair, H	ISG D	
	32.	950	70	Weig	ghted Aver	age	
	32.	950		100.	00% Pervi	ous Area	
	Тс	Length	Slo	ope	Velocity	Capacity	Description
(m	in)	(feet)	(f	t/ft)	(ft/sec)	(cfs)	
10	).2	100	0.11	100	0.16		Sheet Flow, A-B
							Woods: Light underbrush n= 0.400 P2= 3.62"
2	1.7	635	0.20	000	2.24		Shallow Concentrated Flow, B-C
							Woodland Kv= 5.0 fps
	1.7	951	0.04	421	9.10	151.99	Channel Flow, C-D
							Area= 16.7 sf Perim= 12.8' r= 1.30'
							n= 0.040 Winding stream, pools & shoals

16.6 1,686 Total

### Subcatchment EDA-2: EDA-2



#### Summary for Subcatchment EDA-3: EDA-3

Runoff = 155.30 cfs @ 12.29 hrs, Volume= Routed to Link AP-2 : AP-2 18.242 af, Depth= 4.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

Area	(ac) (	CN De	scription		
0.	515	36 W	ods, Fair, <del>I</del>	ISG A	
18.	551	60 W	ods, Fair, F	ISG B	
19.:	257	73 W	oods, Fair, H	ISG C	
5.	825	79 W	oods, Fair, H	ISG D	
44.	148	68 W	eighted Ave	rage	
44.	148	10	0.00% Perv	ious Area	
Tc	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/f	) (ft/sec)	(cfs)	
8.8	100	0.160	0.19		Sheet Flow, A-B
					Woods: Light underbrush n= 0.400 P2= 3.62"
8.1	735	0.091	2 1.51		Shallow Concentrated Flow, B-C
					Woodland Kv= 5.0 fps
2.7	1,683	0.055	2 10.50	346.60	Channel Flow, C-D
					Area= 33.0 sf Perim= 25.0' r= 1.32'
					n= 0.040 Winding stream, pools & shoals

19.6 2,518 Total

### Subcatchment EDA-3: EDA-3



# Summary for Subcatchment EDA-4: EDA-4

[47] Hint: Peak is 112% of capacity of segment #4

Runoff = 18.80 cfs @ 12.24 hrs, Volume= Routed to Link AP-2 : AP-2

2.020 af, Depth> 5.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

Area (	ac) C	N Des	cription					
4.010 73 Woods, Fair, HSG C								
0.0	011 7	'9 Woo	ods, Fair, ⊢	ISG D				
0.2	220 9	8 Pave	ed roads w	/curbs & se	ewers, HSG C			
0.0	018 9	98 Pave	ed roads w	/curbs & se	ewers, HSG D			
4.2	259 7	4 Weig	ghted Aver	age				
4.(	)21	94.4	1% Pervio	us Area				
0.2	238	5.59	% Impervi	ous Area				
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
12.2	100	0.0700	0.14		Sheet Flow, A-B			
					Woods: Light underbrush n= 0.400 P2= 3.62"			
2.3	301	0.1827	2.14		Shallow Concentrated Flow, B-C			
					Woodland Kv= 5.0 fps			
0.2	63	0.0635	5.12		Shallow Concentrated Flow, C-D			
					Paved Kv= 20.3 fps			
1.1	885	0.0575	13.67	16.78	Pipe Channel, D-E			
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'			
					n= 0.012 Corrugated PP, smooth interior			
15.8	1,349	Total						

21

20

19

18

17 16

15-14

13-12-

11-

9-

7-

6 5 4-3-2 1 0-

6 7

5

9

8

10 11 12 13 14 15

Flow (cfs)

**CN=74** 

20 21 22 23 24 25 26 27 28 29 30

Hydrograph Runoff 18.80 cfs NRCC 24-hr D 100-yr Rainfall=8.85" Runoff Area=4.259 ac Runoff Volume=2.020 af Runoff Depth>5.69" 10-Flow Length=1,349' 8 Tc=15.8 min

16 17 18 19

Time (hours)

### Subcatchment EDA-4: EDA-4

## Summary for Link AP-1: AP-1

Inflow /	Area	=	36.523 ac,	0.00% Impervious,	Inflow Depth = 5.0	08" for 100-yr event
Inflow	=	=	143.04 cfs @	12.25 hrs, Volume	= 15.473 af	
Primar	у =	=	143.04 cfs @	12.25 hrs, Volume	= 15.473 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs



## Link AP-1: AP-1

## Summary for Link AP-2: AP-2

Inflow /	Area :	=	48.407 ac,	0.49% Impervious	, Inflow Depth = 5.	02" for 100-yr event
Inflow	=	=	173.09 cfs @	12.28 hrs, Volum	e= 20.263 af	
Primary	y =	=	173.09 cfs @	12.28 hrs, Volum	e= 20.263 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs



## Link AP-2: AP-2



Event#	Event Name	Storm Type	Curve	Mode	Duration (bours)	B/B	Depth (inches)	AMC
	Name				(nours)		(incries)	
1	2-yr	NRCC 24-hr	D	Default	24.00	1	3.62	2
2	25-yr	NRCC 24-hr	D	Default	24.00	1	6.90	2
3	50-yr	NRCC 24-hr	D	Default	24.00	1	7.83	2
4	100-yr	NRCC 24-hr	D	Default	24.00	1	8.85	2

## **Rainfall Events Listing**

## Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.186	96	Gravel surface, HSG B (PDA-1, PDA-2B, PDA-3B)
0.407	96	Gravel surface, HSG C (PDA-3A, PDA-4)
0.003	96	Gravel surface, HSG D (PDA-4)
3.415	58	Meadow, non-grazed, HSG B (PDA-1, PDA-2A, PDA-2B, PDA-2C, PDA-3A)
12.330	65	Meadow, non-grazed, HSG B/C (PDA-2A, PDA-2B, PDA-2C, PDA-2D, PDA-3A,
		PDA-3B)
1.876	71	Meadow, non-grazed, HSG C (PDA-3A, PDA-3C, PDA-4)
7.866	75	Meadow, non-grazed, HSG C/D (PDA-3A, PDA-3C, PDA-4)
3.980	78	Meadow, non-grazed, HSG D (PDA-2A, PDA-2C, PDA-2D, PDA-3A, PDA-4)
0.220	98	Paved roads w/curbs & sewers, HSG C (PDA-4)
0.018	98	Paved roads w/curbs & sewers, HSG D (PDA-4)
0.034	98	Unconnected pavement, HSG B (PDA-3B)
0.034	98	Unconnected pavement, HSG D (PDA-3A)
0.034	98	Unconnected roofs, HSG B/C (PDA-2B)
0.515	36	Woods, Fair, HSG A (PDA-3A)
22.155	60	Woods, Fair, HSG B (PDA-1, PDA-2A, PDA-2B, PDA-2C, PDA-3A)
13.305	73	Woods, Fair, HSG C (PDA-3A, PDA-3C, PDA-4)
18.567	79	Woods, Fair, HSG D (PDA-2A, PDA-2B, PDA-2C, PDA-3A, PDA-4)
84.945	70	TOTAL AREA

## Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.515	HSG A	PDA-3A
38.154	HSG B	PDA-1, PDA-2A, PDA-2B, PDA-2C, PDA-2D, PDA-3A, PDA-3B
23.674	HSG C	PDA-3A, PDA-3C, PDA-4
22.602	HSG D	PDA-2A, PDA-2B, PDA-2C, PDA-2D, PDA-3A, PDA-4
0.000	Other	
84.945		TOTAL AREA

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A-4

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
 0.000	0.186	0.407	0.003	0.000	0.596	Gravel surface	PD
							A-1
							, PD
							A-2
							В.
							PD
							A-3
							А,
							PD
							A-3
							В,
							PD
							A-4
0.000	15.745	9.742	3.980	0.000	29.467	Meadow, non-grazed	PD
							A-1
							,
							PD
							A-2
							А,
							PD
							A-2
							В,
							PD
							A-2
							C,
							PD
							A-2
							D, DD
							A-3
							А, ПП
							A-J B
							D, PD
							ι υ Δ_3
							<u> </u>
							O, PD
							Δ_4
0 000	0 000	0 220	0.018	0 000	0 228	Paved roads w/curbs & sewers	
0.000	0.000	0.220	0.010	0.000	0.200		- U

## Ground Covers (all nodes)

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.034	0.000	0.034	0.000	0.068	Unconnected pavement	PD
							A-3
							А,
							PD
							A-3
							В
0.000	0.034	0.000	0.000	0.000	0.034	Unconnected roofs	PD
							A-2
							В
0.515	22.155	13.305	18.567	0.000	54.542	Woods, Fair	PD
							A-1
							,
							PD
							A-2
							А,
							PD
							A-2
							В,
							PD
							A-2
							С,
							PD
							A-3
							А,
							PD
							A-3
							С,
							PD
							A-4
0.515	38.154	23.674	22.602	0.000	84.945	TOTAL AREA	

### Ground Covers (all nodes) (continued)
Riggs St_Prop.	
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# Pipe Listing (all nodes)

Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Width	Diam/Height	Inside-Fill	Node
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)	Name
1	PDA-3B	0.00	0.00	55.0	0.0100	0.013	0.0	18.0	0.0	
2	PDA-4	0.00	0.00	885.0	0.0575	0.012	0.0	15.0	0.0	
3	1P	426.00	425.00	65.0	0.0154	0.013	0.0	18.0	0.0	
4	2P	416.00	414.00	40.0	0.0500	0.013	0.0	18.0	0.0	
5	3P	456.00	445.00	60.0	0.1833	0.013	0.0	18.0	0.0	
6	4P	478.00	476.00	50.0	0.0400	0.013	0.0	18.0	0.0	
7	5P	460.00	458.00	50.0	0.0400	0.013	0.0	18.0	0.0	

Riggs St_Prop.	Ν
Prepared by Solli Engineering	
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Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPDA-1: PDA-1	Runoff Area=2.493 ac 0.00% Impervious Runoff Depth=0.58" Flow Length=720' Tc=13.0 min CN=60 Runoff=0.89 cfs 0.121 af
SubcatchmentPDA-2A:PDA-2A	Runoff Area=24.113 ac 0.00% Impervious Runoff Depth=1.20" Flow Length=1,686' Tc=16.6 min CN=72 Runoff=21.26 cfs 2.412 af
SubcatchmentPDA-2B: PDA-2B	Runoff Area=4.751 ac 0.72% Impervious Runoff Depth=0.77" Flow Length=813' Tc=14.8 min CN=64 Runoff=2.47 cfs 0.304 af
SubcatchmentPDA-2C: PDA-2C	Runoff Area=1.880 ac 0.00% Impervious Runoff Depth=1.14" Flow Length=482' Tc=7.3 min CN=71 Runoff=2.15 cfs 0.179 af
SubcatchmentPDA-2D: PDA-2D	Runoff Area=2.688 ac 0.00% Impervious Runoff Depth=1.03" Flow Length=618' Tc=12.3 min CN=69 Runoff=2.23 cfs 0.230 af
SubcatchmentPDA-3A:PDA-3A	Runoff Area=34.353 ac 0.10% Impervious Runoff Depth=0.97" Flow Length=2,518' Tc=19.6 min CN=68 Runoff=21.48 cfs 2.782 af
SubcatchmentPDA-3B:PDA-3B	Runoff Area=4.786 ac 0.71% Impervious Runoff Depth=0.82" Flow Length=774' Tc=16.2 min CN=65 Runoff=2.60 cfs 0.325 af
SubcatchmentPDA-3C:PDA-3C	Runoff Area=5.622 ac 0.00% Impervious Runoff Depth=1.39" Flow Length=812' Tc=14.7 min CN=75 Runoff=6.14 cfs 0.650 af
SubcatchmentPDA-4: PDA-4	Runoff Area=4.259 ac 5.59% Impervious Runoff Depth=1.39" Flow Length=1,349' Tc=15.8 min CN=75 Runoff=4.52 cfs 0.492 af
Pond 1P: P=1	Peak Elev=427.32' Storage=4,593 cf Inflow=2.47 cfs 0.304 af Outflow=0.47 cfs 0.226 af
Pond 2P: P-2	Peak Elev=417.91' Storage=3,903 cf Inflow=2.15 cfs 0.179 af Outflow=0.18 cfs 0.101 af
Pond 3P: P-3	Peak Elev=457.27' Storage=3,953 cf Inflow=2.23 cfs 0.230 af Outflow=0.36 cfs 0.161 af
Pond 4P: P-5	Peak Elev=479.58' Storage=10,551 cf Inflow=6.14 cfs 0.650 af Outflow=1.45 cfs 0.506 af
Pond 5P: P-4	Peak Elev=461.29' Storage=5,767 cf Inflow=2.60 cfs 0.325 af Outflow=0.39 cfs 0.225 af
Link AP-1: AP-1	Inflow=22.14 cfs 3.021 af Primary=22.14 cfs 3.021 af
Link AP-2: AP-2	Inflow=25.86 cfs  4.006 af Primary=25.86 cfs  4.006 af

Total Runoff Area = 84.945 acRunoff Volume = 7.495 afAverage Runoff Depth = 1.06"99.60% Pervious = 84.605 ac0.40% Impervious = 0.340 ac

#### Summary for Subcatchment PDA-1: PDA-1

Runoff = 0.89 cfs @ 12.24 hrs, Volume= Routed to Link AP-1 : AP-1

0.121 af, Depth= 0.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

 Area	(ac) (	CN	Desc	cription		
 1.	713	60	Woo	ds, Fair, H	ISG B	
0.	730	58	Mead	dow, non-g	grazed, HS	GB
 0.	050	96	Grav	el surface	, HSG B	
 2.	493	60	Weig	hted Aver	age	
2.	493		100.0	, 00% Pervi	ous Area	
Тс	Length	S	Slope	Velocity	Capacity	Description
(min)	(feet)		(ft/ft)	(ft/sec)	(cfs)	
8.0	100	0.	2000	0.21		Sheet Flow, A-B
						Woods: Light underbrush n= 0.400 P2= 3.62"
5.0	620	0.	1710	2.07		Shallow Concentrated Flow, B-C
						Woodland Kv= 5.0 fps
 13.0	720	To	otal			

#### Subcatchment PDA-1: PDA-1



# Summary for Subcatchment PDA-2A: PDA-2A

Runoff = 21.26 cfs @ 12.26 hrs, Volume= 2.412 af, Depth= 1.20" Routed to Link AP-1 : AP-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

	Area	(ac) (	CN	Desc	ription		
	6.	774	60	Woo	ds, Fair, H	ISG B	
	13.	166	79	Woo	ds, Fair, H	ISG D	
	1.	006	58	Mead	dow, non-g	grazed, HS	GB
*	1.	425	65	Mea	dow, non-g	grazed, HS	G B/C
_	1.	742	78	Mea	dow, non-g	grazed, HS	G D
	24.	113	72	Weig	hted Aver	age	
	24.	113		100.0	00% Pervi	ous Area	
	Tc	Length	S	lope	Velocity	Capacity	Description
	(min)	(feet)	(*	ft/ft)	(ft/sec)	(cfs)	
	10.2	100	0.1	100	0.16		Sheet Flow, A-B
							Woods: Light underbrush n= 0.400 P2= 3.62"
	4.7	635	0.2	2000	2.24		Shallow Concentrated Flow, B-C
							Woodland Kv= 5.0 fps
	1.7	951	0.0	)421	9.10	151.99	Channel Flow, C-D
							Area= 16.7 sf Perim= 12.8' r= 1.30'
							n= 0.040 Winding stream, pools & shoals
	16.6	1.686	To	tal			

#### Subcatchment PDA-2A: PDA-2A



# Summary for Subcatchment PDA-2B: PDA-2B

Runoff = 2.47 cfs @ 12.25 hrs, Volume= 0.304 af, Depth= 0.77" Routed to Pond 1P : P=1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

	Area	(ac)	CN	Desc	cription		
	0.	959	60	Woo	ds, Fair, H	ISG B	
	0.	012	79	Woo	ds, Fair, H	ISG D	
	0.	395	58	Mea	dow, non-	grazed, HS	G B
*	3.	239	65	Mea	dow, non-	grazed, HS	G B/C
*	0.	034	98	Unco	onnected r	oofs, HSG	B/C
	0.	112	96	Grav	el surface	, HSG B	
	4.	751	64	Weig	hted Aver	age	
	4.	717		99.2	8% Pervio	us Area	
	0.034 0.72% Impervious Area						
	0.034 100.00% Unconnected					nnected	
	Тс	Length	າ ຮ	Slope	Velocity	Capacity	Description
	(min)	(feet	)	(ft/ft)	(ft/sec)	(cfs)	
	10.2	100	) 0.	1100	0.16		Sheet Flow, A-B
							Woods: Light underbrush n= 0.400 P2= 3.62"
	0.2	33	<b>3</b> 0.	3000	2.74		Shallow Concentrated Flow, B-C
							Woodland Kv= 5.0 fps
	4.4	680	) 0.	1367	2.59		Shallow Concentrated Flow, C-D
_							Short Grass Pasture Kv= 7.0 fps
	14.8	813	3 To	otal			

#### Subcatchment PDA-2B: PDA-2B



#### Summary for Subcatchment PDA-2C: PDA-2C

Runoff = 2.15 cfs @ 12.15 hrs, Volume= 0.179 af, Depth= 1.14" Routed to Pond 2P : P-2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

	Area	(ac) (	CN De	scription					
	0.	089	58 Me	adow, non-	grazed, HS	G B			
*	0.	667	65 Me	adow, non-	grazed, HS	G B/C			
	0.	930	78 Me	adow, non-	grazed, HS	G D			
	0.	105	60 Wc	ods, Fair, F	ÍSG B				
	0.	089	79 Wc	ods, Fair, F	ISG D				
	1.	880	71 We	ighted Ave	rage				
	1.880			100.00% Pervious Area					
	Tc	Length	Slope	e Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.1	100	0.2200	0.32		Sheet Flow, A-B			
						Grass: Dense n= 0.240 P2= 3.62"			
	2.0	340	0.1647	2.84		Shallow Concentrated Flow, B-C			
						Short Grass Pasture Kv= 7.0 fps			
	0.2	42	0.0476	3.27		Shallow Concentrated Flow, C-D			
						Grassed Waterway Kv= 15.0 fps			
	7.3	482	Total						

#### Subcatchment PDA-2C: PDA-2C

Hydrograph



#### Summary for Subcatchment PDA-2D: PDA-2D

Runoff = 2.23 cfs @ 12.21 hrs, Volume= 0.230 af, Depth= 1.03" Routed to Pond 3P : P-3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

	Area	(ac)	CN	Desc	ription			
*	1.	831	65	Mead	dow, non-g	grazed, HS	G B/C	
	0.	857	78	Mead	dow, non-g	grazed, HS	G D	
	2.	688	69	Weig	hted Aver	age		
	2.	688		100.0	00% Pervi	ous Area		
	Tc (min)	Lengtł (feet	n Slo ) (f	ope t/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	9.3	100	0.05	500	0.18		Sheet Flow, A-B	
	3.0	518	3 0.16	654	2.85		Grass: Dense n= 0.240 P2= 3.62" <b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps	
	12.3	618	3 Tota	al				

#### Subcatchment PDA-2D: PDA-2D



# Summary for Subcatchment PDA-3A: PDA-3A

Runoff = 21.48 cfs @ 12.31 hrs, Volume= Routed to Link AP-2 : AP-2 2.782 af, Depth= 0.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

	Area	(ac)	CN	Desc	ription					
	0.	515	36	Woo	ds, Fair, H	ISG A				
	12.	604	60	Woo	ds, Fair, H	ISG B				
	10.	470	73	Woo	ds, Fair, H	SG C				
	5.	295	79	Woo	ds, Fair, H	ISG D				
*	2.	024	75	Mea	dow, non-g	grazed, HS	G C/D			
	0.	034	98	Unco	onnected p	avement, l	HSG D			
	0.	397	96	Grav	el surface	, HSG C				
	1.	195	58	Mea	dow, non-g	grazed, HS	GB			
	0.	931	71	Mea	dow, non-g	grazed, HS	GC			
	0.	448	78	Mea	dow, non-ę	grazed, HS	GD			
<u>*</u>	0.	440	65	Mea	dow, non-g	grazed, HS	G B/C			
	34.	353	68	Weig	hted Aver	age				
	34.	319		99.90	0% Pervio	us Area				
	0.	034		0.10	0.10% Impervious Area					
	0.	034		100.0	JU% Unco	nnected				
	То	Longth	-	Slopa	Volocity	Conacity	Description			
	(min)	(foot	) )	(ft/ft)	(ft/sec)	Capacity (cfs)	Description			
	00	100	/	1600	0 10	(013)	Shoot Flow A R			
	0.0	100	5 0	.1000	0.19		Woode: Light underbruch n= 0.400 P2= 3.62"			
	8 1	734	5 0	0012	1 51		Shallow Concentrated Flow B-C			
	0.1	100	5 0	.0012	1.01		Woodland $Ky = 5.0$ fps			
	27	1 683	30	0552	10 50	346 60	Channel Flow C-D			
		1,000			10.00	010100	Area= 33.0 sf Perim= 25.0' r= 1.32'			
							n= 0.040 Winding stream, pools & shoals			
	19.6	2,518	3 Т	otal						

#### Subcatchment PDA-3A: PDA-3A



# Summary for Subcatchment PDA-3B: PDA-3B

Runoff	=	2.60 cfs @	12.27 hrs,	Volume=	(	).325 af,	Depth=	0.82"
Routed	l to Pond	15P:P-4						

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

_	Area	(ac)	CN	Desc	cription					
*	4.	728	65	Mea	dow, non-g	grazed, HS	G B/C			
0.024 96 Gravel surf				Grav	el surface	, HSG B				
	0.	034	98	Unco	Jnconnected pavement, HSG B					
	4.	786	65	Weig	phted Aver	age				
	4.	752		99.2	9% Pervio	us Area				
	0.	034		0.71	% Impervi	ous Area				
	0.	034		100.	00% <sup>.</sup> Unco	nnected				
	Tc	Lengt	n :	Slope	Velocity	Capacity	Description			
_	(min)	(feet	)	(ft/ft)	(ft/sec)	(cfs)				
	10.6	10	0 C	.1000	0.16		Sheet Flow, A-B			
							Woods: Light underbrush n= 0.400 P2= 3.62"			
	5.0	54	70	.1334	1.83		Shallow Concentrated Flow, B-C			
							Woodland Kv= 5.0 fps			
	0.4	73	20	.0444	3.16		Shallow Concentrated Flow, C-D			
							Grassed Waterway Kv= 15.0 fps			
	0.2	5	50	.0100	5.94	10.50	Pipe Channel, D-E			
							18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'			
							n= 0.013 Corrugated PE, smooth interior			
	16.2	774	4 T	otal						

#### Subcatchment PDA-3B: PDA-3B



#### Summary for Subcatchment PDA-3C: PDA-3C

Runoff = 6.14 cfs @ 12.24 hrs, Volume= 0.650 af, Depth= 1.39" Routed to Pond 4P : P-5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

	Area	(ac) (	CN Des	cription					
	1.	062	73 Woo	ods, Fair, F	ISG C				
	0.	126	71 Mea	dow, non-	grazed, HS	GC			
*	4.	434	75 Mea	idow, non-	grazed, HS	G C/D			
	5.622 75 Weighted Average								
	5.	622	100	00% Pervi	ious Area				
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	9.5	100	0.1300	0.17		Sheet Flow, A-B			
						Woods: Light underbrush n= 0.400 P2= 3.62"			
	2.0	249	0.1687	2.05		Shallow Concentrated Flow, B-C			
						Woodland Kv= 5.0 fps			
	3.2	463	0.1210	2.43		Shallow Concentrated Flow, C-D			
						Short Grass Pasture Kv= 7.0 fps			
	14.7	812	Total						

#### Subcatchment PDA-3C: PDA-3C



# Summary for Subcatchment PDA-4: PDA-4

Runoff = 4.52 cfs @ 12.25 hrs, Volume= 0.492 af, Depth= 1.39" Routed to Link AP-2 : AP-2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 2-yr Rainfall=3.62"

	Area	(ac) (	CN	Desc	cription			
_	1.	773	73	Woo	ds, Fair, H	ISG C		
	0.	005	79	Woo	ds, Fair, H	ISG D		
	0.	220	98	Pave	ed roads w	/curbs & se	ewers, HSG C	
	0.	018	98	Pave	ed roads w	/curbs & se	ewers, HSG D	
*	1.	408	75	Mea	dow, non-	grazed, HS	G C/D	
	0.	819	71	Mea	dow, non-	grazed, HS	GC	
	0.	010	96	Grav	el surface	, HSG C		
	0.	.003	96	Grav	el surface	, HSG D		
_	0.	003	78	Mea	dow, non-	grazed, HS	G D	
	4.	259	75	Weig	ghted Aver	age		
4.021 94.41% Pervious Area								
	0.	238		5.59	% Impervi	ous Area		
	<b>–</b>	1	~		17.1.14.1	0	Description	
	IC (minn)	Length		slope		Capacity	Description	
_	(min)	(leet)		$\frac{(II/II)}{2}$	(II/sec)	(CIS)		
	12.2	100	0.0	0700	0.14		Sheet Flow, A-B	
	0.0	004	~	4007	0.44		Woods: Light underbrush n= 0.400 P2= 3.62"	
	2.3	301	0.	1827	2.14		Shallow Concentrated Flow, B-C	
	0.0	60	0	0625	E 10		woodland KV= 5.0 lps	
	0.2 63 0.0		0635	5.1Z		Shallow Concentrated Flow, C-D		
	1 1	885		0575	13 67	16 78	Pipe Channel D E	
	1.1	000	0.0	0375	15.07	10.70	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'	
							n=0.012 Corrugated PP smooth interior	
_	15.8	1 3/0		stal				
	10.0	1,048	, 10	nai				

#### Subcatchment PDA-4: PDA-4



# Summary for Pond 1P: P=1

Inflow Area	a =	4.751 ac,	0.72% Imp	ervious, In	nflow Dept	th =	0.77"	for 2-yr	event
Inflow	=	2.47 cfs @	12.25 hrs,	Volume=	0.	.304 a	af	-	
Outflow	=	0.47 cfs @	13.42 hrs,	Volume=	0.	.226 a	af, Atte	n= 81%,	Lag= 69.9 min
Primary	=	0.47 cfs @	13.42 hrs,	Volume=	0.	.226 a	af		-
Routed	to Link /	AP-1 : AP-1							

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 427.32' @ 13.42 hrs Surf.Area= 3,958 sf Storage= 4,593 cf

Plug-Flow detention time= 244.0 min calculated for 0.226 af (74% of inflow) Center-of-Mass det. time= 135.5 min (1,070.3 - 934.9)

Volume	Invert	Avail.Sto	rage	Storage Description				
#1	426.00'	18,0	67 cf	Custom Stage Data	<b>a (Irregular)</b> Listed	below (Recalc)		
Elevatic (fee	n Su t)	rf.Area F (sq-ft)	erim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
426.0 427.0 428.0 429.0 430.0	)0  0  0  0  0	3,028 3,726 4,480 5,291 6,158	223.1 242.0 260.8 279.7 298.5	0 3,371 4,097 4,880 5,719	0 3,371 7,468 12,348 18,067	3,028 3,765 4,558 5,415 6,327		
Device	Routing	Invert	Outle	et Devices				
#1	Primary	426.00'	<b>18.0</b> L= 6 Inlet n= 0	<b>" Round Culvert</b> 5.0' CMP, projecting / Outlet Invert= 426.0 .013 Corrugated PE.	j, no headwall, Ke 00' / 425.00' S= 0 smooth interior. F	= 0.900 .0154 '/'    Cc= 0.900 Flow Area= 1.77 sf		
#2	Device 1	427.00'	15.0	<b>15.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads				
#3 Device 1 429.00' <b>36.0" x 21.0" Horiz. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads		00						
#4 Primary 429.00' <b>25.0'</b> Head Coef.				<b>' long + 3.0 '/' SideZ</b> d (feet) 0.20 0.40 0. f. (English) 2.68 2.70	2 <b>x 15.0' breadth</b> 60 0.80 1.00 1.2 0 2.70 2.64 2.63	Broad-Crested Rect 0 1.40 1.60 2.64 2.64 2.63	angular Weir	

**Primary OutFlow** Max=0.47 cfs @ 13.42 hrs HW=427.32' (Free Discharge)

**\_1=Culvert** (Passes 0.47 cfs of 5.08 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 0.47 cfs @ 1.92 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 1P: P=1



# Summary for Pond 2P: P-2

Inflow Ar Inflow Outflow Primary Route	rea = 2 = 2 = 0 = 0 ed to Link Al	1.880 ac, 2.15 cfs @ 0.18 cfs @ 0.18 cfs @ P-1 : AP-1	0.00% In 12.15 hr 14.06 hr 14.06 hr	npervious, Inflow D s, Volume= s, Volume= s, Volume=	epth = 1.14" for 0.179 af 0.101 af, Atten= 9 0.101 af	2-yr event 92%, Lag= 114.7 min	I				
Routing Peak Ele	Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 417.91' @ 14.06 hrs Surf.Area= 2,595 sf Storage= 3,903 cf										
Plug-Flo Center-o	Plug-Flow detention time= 349.9 min calculated for 0.101 af (57% of inflow) Center-of-Mass det. time= 198.4 min(1,096.6-898.3)										
Volume	Invert	Avail.S	torage	Storage Description	า						
#1	416.00'	10,	757 cf	Custom Stage Dat	ta (Irregular)Listed	below (Recalc)					
Elevatio	on Si	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area					
(fee	t)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)					
416.0	0	1,538	166.9	0	0	1,538					
417.0	00	2,067	185.7	1,796	1,796	2,094					
418.0	00	2,653	204.6	2,354	4,150	2,713					
419.0	0	3,295	223.4	2,968	7,118	3,388					
420.0	00	3,993	242.3	3,638	10,757	4,126					
Device	Routing	Inver	t Outle	t Devices							
#1	Primary	416.00	)' <b>18.0''</b>	Round Culvert							
			L= 40	0.0' CMP, projectin	ig, no headwall, Ke	= 0.900					
			Inlet /	Outlet Invert= 416	.00'/414.00' S= 0	.0500 '/' Cc= 0.900					
			n= 0.0	013 Corrugated PE	E, smooth interior, F	-low Area= 1.77 sf					
#2	Device 1	417.70	) 12.0"	Vert. Orifice/Grat	e C = 0.600 Limite	ed to weir flow at low	neads				
#3	Device 1	419.00	<sup>7</sup> 36.0"	X 21.0" Horiz. Ori	rice/Grate C= 0.60	0					
<i>₩</i> Λ	Primary	110.00	⊂IIIII€ >' <b>20 0'</b>	1000000000000000000000000000000000000	w neaus 7 x 15 0' broadth l	Broad-Crosted Rect	angular Woir				
<del>#4</del>	Timary	419.00	, <b>20.0</b> Head	(feet) 0 20 0 40 (		0 1 40 1 60					
			Coef	(English) 2.68 27	70 2.70 2.64 2.63	2.64 2.64 2.63					
			0.001	()							

**Primary OutFlow** Max=0.18 cfs @ 14.06 hrs HW=417.91' (Free Discharge)

**2=Orifice/Grate** (Orifice Controls 0.18 cfs @ 1.54 fps)

-3=Orifice/Grate (Controls 0.00 cfs) -4=Broad-Crested Rectangular Weir(Controls 0.00 cfs)

Pond 2P: P-2



# Summary for Pond 3P: P-3

Inflow Area =		2.688 ac,	0.00% Imp	ervious, Inflov	v Depth =	= 1.03'	' for 2	2-yr ev	/ent	
Inflow	=	2.23 cfs @	12.21 hrs,	Volume=	0.23	0 af				
Outflow	=	0.36 cfs @	13.30 hrs,	Volume=	0.16	1 af, A	tten= 84	4%, L	ag= 65.2 min	
Primary	=	0.36 cfs @	13.30 hrs,	Volume=	0.16	1 af			•	
Routed	Routed to Link AP-1 : AP-1									
Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 457.27' @ 13.30 hrs Surf.Area= 3,533 sf Storage= 3,953 cf										
Plug-Flow	detentio	n time= 272.	6 min calcu	lated for 0.161	l af (70%	of inflo	w)			
Center-of-	Mass de	t. time= 149.	3 min ( 1,0	60.1 - 910.9 )	·					
				,						
Volume	Inve	t Avail.S	storage S	torage Descrip	otion					
#1	456.00	)' 16	,375 cf <b>C</b>	ustom Stage	Data (Irre	egular)	_isted b	elow (	Recalc)	
				•	· ·	- /		,		

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
456.00	2,689	209.2	0	0	2,689
457.00	3,345	228.1	3,011	3,011	3,382
458.00	4,057	246.9	3,695	6,706	4,131
459.00	4,825	265.8	4,435	11,142	4,944
460.00	5,652	284.6	5,233	16,375	5,812
Device Routing	Inv	ert Outlet	Devices		

#1	Primary	456.00'	18.0" Round Culvert				
			L= 60.0' CMP, projecting, no headwall, Ke= 0.900				
			Inlet / Outlet Invert= 456.00' / 445.00' S= 0.1833 '/' Cc= 0.900				
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf				
#2	Device 1	457.00'	<b>15.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads				
#3	Device 1	459.00'	36.0" x 21.0" Horiz. Orifice/Grate C= 0.600				
			Limited to weir flow at low heads				
#4	Primary	459.00'	16.0' long + 3.0 '/' SideZ x 15.0' breadth Broad-Crested Rectangular Weir				
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60				
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63				

**Primary OutFlow** Max=0.36 cfs @ 13.30 hrs HW=457.27' (Free Discharge)

**2=Orifice/Grate** (Orifice Controls 0.36 cfs @ 1.78 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: P-3



# Summary for Pond 4P: P-5

Inflow Are	a =	5.622 ac,	0.00% Impervious, Inflow D	epth = 1.39" for 2-yr event
Inflow	=	6.14 cfs @	12.24 hrs, Volume=	0.650 af
Outflow	=	1.45 cfs @	12.81 hrs, Volume=	0.506 af, Atten= 76%, Lag= 34.5 min
Primary	=	1.45 cfs @	12.81 hrs, Volume=	0.506 af
Routed	to Link	AP-2 : AP-2		

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 479.58' @ 12.81 hrs Surf.Area= 7,877 sf Storage= 10,551 cf

Plug-Flow detention time= 233.4 min calculated for 0.505 af (78% of inflow) Center-of-Mass det. time= 136.8 min (1,026.6 - 889.8)

Volume	Inver	t Avail.St	orage	Storage Description	on		
#1	478.00	<b>'</b> 34,	305 cf	Custom Stage D	<b>ata (Irregular)</b> List	ed below (Recalc)	
Elevatio (fee	on S et)	Surf.Area (sɑ-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
478.0 479.0 480.0 481.0 482.0	00 00 00 00 00 00	5,499 6,994 8,546 10,155 11,820	489.1 507.9 526.8 545.6 564.5	0 6,232 7,757 9,339 10,977	0 6,232 13,989 23,328 34,305	5,499 7,072 8,712 10,403 12,163	
Device	Routing	Inver	t Outle	et Devices			
#1	Primary	478.00	' <b>18.0</b> L= 5 Inlet n= 0	" Round Culvert 0.0' CMP, project / Outlet Invert= 47 .013 Corrugated F	Ke= 0.900 = 0.0400 '/'    Cc= 0.900 ;   Flow Area= 1.77 sf		
#2 #3	Device 1 Device 1	479.00 481.00	<ul> <li>479.00' 15.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow a</li> <li>481.00' 36.0" x 21.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads</li> </ul>			nited to weir flow at low hea .600	ads
#4         Primary         481.00'         20.0' long x 15.0' breadth Broad-Crested Rectangula           Head (feet)         0.20         0.40         0.60         0.80         1.00         1.40         1.60           Coef. (English)         2.68         2.70         2.64         2.63         2.64         2.64         2						<b>ed Rectangular Weir</b> 1.20 1.40 1.60 53 2.64 2.64 2.63	

**Primary OutFlow** Max=1.45 cfs @ 12.81 hrs HW=479.58' (Free Discharge)

**-1=Culvert** (Passes 1.45 cfs of 6.12 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 1.45 cfs @ 2.60 fps)

**3=Orifice/Grate** (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 4P: P-5



# Summary for Pond 5P: P-4

Inflow Area	a =	4.786 ac,	0.71% Impervious, I	nflow Depth = 0.82"	for 2-yr event
Inflow	=	2.60 cfs @	12.27 hrs, Volume=	0.325 af	-
Outflow	=	0.39 cfs @	13.87 hrs, Volume=	0.225 af, Att	en= 85%, Lag= 96.3 min
Primary	=	0.39 cfs @	13.87 hrs, Volume=	0.225 af	-
Routed	to Link A	AP-2 : AP-2			

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 461.29' @ 13.87 hrs Surf.Area= 5,108 sf Storage= 5,767 cf

Plug-Flow detention time= 300.4 min calculated for 0.225 af (69% of inflow) Center-of-Mass det. time= 174.1 min (1,105.7 - 931.6)

Volume	Inver	t Avail.S	torage	Storage Description				
#1	460.00	' 23,	498 cf	Custom Stage Data (Irregular)Listed below (Recalc)				
Elevatio	on S	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area		
460.0 461.0 462.0 463.0	20 20 20 20 20	3,869 4,826 5,840 6,910	309.7 328.5 347.4 366.2	0 4,339 5,325 6,368	0 4,339 9,664 16,031	<u> </u>		
464.0 Device	Routing	8,037 Inver	385.1 t Outle	7,466 et Devices	23,498	8,263		
#1	Primary	460.00' <b>18.0</b> L= 5 Inlet n= 0		<b>" Round Culvert</b> 0.0' CMP, projecti / Outlet Invert= 460 .013 Corrugated P	ng, no headwall,  ł ).00' / 458.00'   S= E, smooth interior,	Ke= 0.900 0.0400 '/' Cc= 0.900 Flow Area= 1.77 sf		
#2 #3	Device 1 Device 1	461.00 463.00	)' <b>15.0</b> )' <b>36.0</b> Limit	<b>15.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads <b>36.0" x 21.0" Horiz. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads				
#4	Primary	rimary 463.00' <b>15.0'</b> Head Coef		long x 15.0' breadth Broad-Crested Rectangular Weir (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63				

**Primary OutFlow** Max=0.39 cfs @ 13.87 hrs HW=461.29' (Free Discharge)

**-1=Culvert** (Passes 0.39 cfs of 4.92 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 0.39 cfs @ 1.83 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 5P: P-4



# Summary for Link AP-1: AP-1

Inflow /	Area =	=	35.925 ac,	0.09% Impervious	, Inflow Depth =	1.01"	for 2-yr	event
Inflow	=		22.14 cfs @	12.26 hrs, Volum	e= 3.021 a	af		
Primary	y =		22.14 cfs @	12.26 hrs, Volum	e= 3.021 a	af, Atte	en= 0%, L	_ag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

# Link AP-1: AP-1



# Summary for Link AP-2: AP-2

Inflow /	Area =	49.020 ac,	0.62% Impervious,	Inflow Depth = 0.9	98" for 2-yr event
Inflow	=	25.86 cfs @	12.30 hrs, Volume	= 4.006 af	
Primar	y =	25.86 cfs @	12.30 hrs, Volume	= 4.006 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs



# Link AP-2: AP-2

Riggs St_Prop.	NR
Prepared by Solli Engineering	
HydroCAD® 10 20-3g s/n 10621 © 2023 HydroCAD Software Solutions I	I C

Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPDA-1: PDA-1	Runoff Area=2.493 ac 0.00% Impervious Runoff Depth=2.53" Flow Length=720' Tc=13.0 min CN=60 Runoff=5.24 cfs 0.526 af
SubcatchmentPDA-2A:PDA-2A	Runoff Area=24.113 ac 0.00% Impervious Runoff Depth=3.74" Flow Length=1,686' Tc=16.6 min CN=72 Runoff=69.41 cfs 7.523 af
SubcatchmentPDA-2B:PDA-2B	Runoff Area=4.751 ac 0.72% Impervious Runoff Depth=2.93" Flow Length=813' Tc=14.8 min CN=64 Runoff=11.07 cfs 1.158 af
SubcatchmentPDA-2C:PDA-2C	Runoff Area=1.880 ac 0.00% Impervious Runoff Depth=3.64" Flow Length=482' Tc=7.3 min CN=71 Runoff=7.12 cfs 0.570 af
SubcatchmentPDA-2D: PDA-2D	Runoff Area=2.688 ac 0.00% Impervious Runoff Depth=3.43" Flow Length=618' Tc=12.3 min CN=69 Runoff=8.02 cfs 0.769 af
SubcatchmentPDA-3A:PDA-3A	Runoff Area=34.353 ac 0.10% Impervious Runoff Depth=3.33" Flow Length=2,518' Tc=19.6 min CN=68 Runoff=80.89 cfs 9.531 af
SubcatchmentPDA-3B:PDA-3B	Runoff Area=4.786 ac 0.71% Impervious Runoff Depth=3.03" Flow Length=774' Tc=16.2 min CN=65 Runoff=11.12 cfs 1.207 af
SubcatchmentPDA-3C:PDA-3C	Runoff Area=5.622 ac 0.00% Impervious Runoff Depth=4.06" Flow Length=812' Tc=14.7 min CN=75 Runoff=18.30 cfs 1.903 af
SubcatchmentPDA-4: PDA-4	Runoff Area=4.259 ac 5.59% Impervious Runoff Depth=4.06" Flow Length=1,349' Tc=15.8 min CN=75 Runoff=13.50 cfs 1.441 af
Pond 1P: P=1	Peak Elev=428.77' Storage=11,135 cf Inflow=11.07 cfs 1.158 af Outflow=6.31 cfs 1.081 af
Pond 2P: P-2	Peak Elev=419.03' Storage=7,224 cf Inflow=7.12 cfs 0.570 af Outflow=3.95 cfs 0.492 af
Pond 3P: P-3	Peak Elev=458.32' Storage=8,064 cf Inflow=8.02 cfs 0.769 af Outflow=4.94 cfs 0.700 af
Pond 4P: P-5	Peak Elev=481.10' Storage=24,343 cf Inflow=18.30 cfs 1.903 af Outflow=9.86 cfs 1.758 af
Pond 5P: P-4	Peak Elev=462.60' Storage=13,346 cf Inflow=11.12 cfs 1.207 af Outflow=5.83 cfs 1.107 af
Link AP-1: AP-1	Inflow=88.31 cfs 10.322 af Primary=88.31 cfs 10.322 af
Link AP-2: AP-2	Inflow=105.08 cfs 13.838 af Primary=105.08 cfs 13.838 af

Total Runoff Area = 84.945 ac Runoff Volume = 24.629 af Average Runoff Depth = 3.48" 99.60% Pervious = 84.605 ac 0.40% Impervious = 0.340 ac

#### Summary for Subcatchment PDA-1: PDA-1

Runoff = 5.24 cfs @ 12.21 hrs, Volume= Routed to Link AP-1 : AP-1 0.526 af, Depth= 2.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

A	rea (	(ac)	CN	Desc	cription		
1.713 60 Woods, Fair, HSG B							
	0.7	730	58	Mea	dow, non-g	grazed, HS	GB
	0.0	050	96	Grav	el surface	, HSG B	
	2.4	493	60	Weig	ghted Aver	age	
	2.4	493		100.	00% Pervi	ous Area	
	Тс	Lengtl	٦	Slope	Velocity	Capacity	Description
(n	nin)	(feet	)	(ft/ft)	(ft/sec)	(cfs)	
	8.0	100	) (	0.2000	0.21		Sheet Flow, A-B
							Woods: Light underbrush n= 0.400 P2= 3.62"
	5.0	620	) (	).1710	2.07		Shallow Concentrated Flow, B-C
							Woodland Kv= 5.0 fps
1	30	720	ר ו	[otal			

#### Subcatchment PDA-1: PDA-1



# Summary for Subcatchment PDA-2A: PDA-2A

Runoff = 69.41 cfs @ 12.25 hrs, Volume= 7.523 af, Depth= 3.74" Routed to Link AP-1 : AP-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

Area (ac) CN			CN	Desc	cription					
	6.774 60 Woods, Fair, HSG B									
13.166 79				Woo	Woods, Fair, HSG D					
1.006 58 Meadow, non-grazed, HSG B							GB			
*	1.	425	65	Mea	dow, non-g	grazed, HS	G B/C			
_	1.	742	78	Mea	dow, non-o	grazed, HS	G D			
	24.	113	72	Weig	phted Aver	age				
	24.	113		100.	00% Pervi	ous Area				
	_		_							
	IC	Length	n S	slope	Velocity	Capacity	Description			
_	(min)	(feet		(ft/ft)	(ft/sec)	(cfs)				
	10.2	100	) 0.1	1100	0.16		Sheet Flow, A-B			
							Woods: Light underbrush n= 0.400 P2= 3.62"			
	4.7	635	5 0.2	2000	2.24		Shallow Concentrated Flow, B-C			
							Woodland Kv= 5.0 fps			
	1.7	951	0.0	0421	9.10	151.99	Channel Flow, C-D			
							Area= 16.7 sf Perim= 12.8' r= 1.30'			
_							n= 0.040 Winding stream, pools & shoals			
	16.6	1,686	6 To	otal						

# Subcatchment PDA-2A: PDA-2A



# Summary for Subcatchment PDA-2B: PDA-2B

Runoff = 11.07 cfs @ 12.24 hrs, Volume= 1.158 af, Depth= 2.93" Routed to Pond 1P : P=1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

	Area	(ac)	CN	Desc	cription		
	0.	959	60	Woo	ds, Fair, H	ISG B	
	0.	012	79	Woo	ds, Fair, H	ISG D	
	0.	395	58	Mea	dow, non-g	grazed, HS	G B
*	3.	239	65	Mea	dow, non-	grazed, HS	G B/C
*	0.	034	98	Unco	onnected r	oofs, HSG	B/C
	0.	112	96	Grav	el surface	, HSG B	
	4.	751	64	Weig	hted Aver	age	
	4.	717		99.2	8% Pervio	us Area	
	0.	034		0.72	% Impervi	ous Area	
	0.034 100.00% Unconnected						
	Тс	Length	า 8	Slope	Velocity	Capacity	Description
_	(min)	(feet)	)	(ft/ft)	(ft/sec)	(cfs)	
	10.2	100	) 0.	1100	0.16		Sheet Flow, A-B
							Woods: Light underbrush n= 0.400 P2= 3.62"
	0.2	33	30.	3000	2.74		Shallow Concentrated Flow, B-C
							Woodland Kv= 5.0 fps
	4.4	680	) 0.	1367	2.59		Shallow Concentrated Flow, C-D
_							Short Grass Pasture Kv= 7.0 fps
	14.8	813	3 To	otal			

# Subcatchment PDA-2B: PDA-2B


#### Summary for Subcatchment PDA-2C: PDA-2C

Runoff = 7.12 cfs @ 12.15 hrs, Volume= 0.570 af, Depth= 3.64" Routed to Pond 2P : P-2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

	Area	(ac) (	CN	Desc	ription			
	0.	089	58	Mea	dow, non-g	grazed, HS	GB	
*	0.	667	65	Mea	dow, non-g	grazed, HS	G B/C	
	0.	930	78	Mead	dow, non-g	grazed, HS	GD	
0.105 60 Woods, Fair, HSG B								
_	0.	089	79	Woo	ds, Fair, H	ISG D		
	1.	880	71	Weig	hted Aver	age		
	1.	880		100.0	00% Pervi	ous Area		
	Тс	Length	S	lope	Velocity	Capacity	Description	
	(min)	(feet)	(	(ft/ft)	(ft/sec)	(cfs)		
	5.1	100	0.2	2200	0.32		Sheet Flow, A-B	
							Grass: Dense n= 0.240 P2= 3.62"	
	2.0	340	0.1	1647	2.84		Shallow Concentrated Flow, B-C	
							Short Grass Pasture Kv= 7.0 fps	
	0.2	42	0.0	0476	3.27		Shallow Concentrated Flow, C-D	
							Grassed Waterway Kv= 15.0 fps	
	7.3	482	То	otal				

#### Subcatchment PDA-2C: PDA-2C



#### Summary for Subcatchment PDA-2D: PDA-2D

Runoff = 8.02 cfs @ 12.20 hrs, Volume= 0.769 af, Depth= 3.43" Routed to Pond 3P : P-3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

	Area	(ac) (	CN Des	cription			
*	1.	831	65 Mea	adow, non-	grazed, HS	G B/C	
0.857 78 Meadow, non-grazed, HSG D							
	2.	688	69 We	ghted Aver	age		
	2.	688	100	.00% Pervi	ous Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	9.3	100	0.0500	0.18		Sheet Flow, A-B	
	3.0	518	0.1654	2.85		Grass: Dense n= 0.240 P2= 3.62" <b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps	
	12.3	618	Total				

#### Subcatchment PDA-2D: PDA-2D



### Summary for Subcatchment PDA-3A: PDA-3A

Runoff = 80.89 cfs @ 12.29 hrs, Volume= Routed to Link AP-2 : AP-2 9.531 af, Depth= 3.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

_	Area	(ac)	CN	Desc	cription						
	0.	515	36	Woo	ds, Fair, H	ISG A					
	12.	604	60	Woo	ds, Fair, H	ISG B					
	10.	470	73	Woo	ds, Fair, H	ISG C					
	5.	295	79	Woo	ds, Fair, H	ISG D					
*	2.	024	75	Mea	Meadow, non-grazed, HSG C/D						
	0.	034	98	Unco	onnected p	pavement, l	HSG D				
0.397 96 Gravel surface, HSG C											
	1.	195	58	Mea	dow, non-	grazed, HS	GB				
	0.	931	71	Mea	dow, non-	grazed, HS	GC				
÷	0.	448	78	Mea	dow, non- <u>(</u>	grazed, HS	GD				
-	0.	440	65	Mea	dow, non-	grazed, HS	G B/C				
	34.	353	68	Weig	phted Aver	age					
	34.	319		99.9	0% Pervio	us Area					
	0.	034		0.10	% Impervi	ous Area					
	0.	034		100.0	00% Unco	nnected					
	То	Longth		Slone	Volocity	Conocity	Description				
	(min)	(foot		/ft/ft)		Capacity (cfs)	Description				
		100	)	1000		(015)	Check Flow, A.D.				
	8.8	100	J U.	1600	0.19		Sheet Flow, A-B Weeder Light underbruch n= 0.400 D2= 2.62"				
	0 1	726	5 0	0012	1 5 1		Shallow Concentrated Flow B C				
	0.1	730	<b>)</b> 0.	0912	1.01		Woodland Ky= 5.0 fps				
	27	1 683	2 0	0552	10 50	346 60	Channel Flow C-D				
	2.1	1,000	<i>J</i> 0.	0002	10.50	540.00	$\Delta r_{eq} = 33.0 \text{ sf } Perim = 25.0' r = 1.32'$				
							n=0.040 Winding stream pools & shoals				
_	10.0	0 540	. т.	atal							

19.6 2,518 Total

### Subcatchment PDA-3A: PDA-3A



# Summary for Subcatchment PDA-3B: PDA-3B

[47] Hint: Peak is 106% of capacity of segment #4

Runoff = 11.12 cfs @ 12.25 hrs, Volume= Routed to Pond 5P : P-4 1.207 af, Depth= 3.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

	Area	(ac)	CN	Desc	cription							
*	4.	728	65	Mea	dow, non-	grazed, HS	G B/C					
	0.	024	96	Grav	el surface	, HSG B						
	0.	034	98	Unco	Jnconnected pavement, HSG B							
	4.	786	65	Weig	Weighted Average							
4.752				99.2	9% Pervio	us Area						
	0.	034		0.71	% Impervi	ous Area						
	0.	034		100.	00% Ünco	nnected						
	Tc	Lengtl	า เ	Slope	Velocity	Capacity	Description					
	(min)	(feet	)	(ft/ft)	(ft/sec)	(cfs)	·					
	10.6	100	0 0	.1000	0.16		Sheet Flow, A-B					
							Woods: Light underbrush n= 0.400 P2= 3.62"					
	5.0	547	7 0.	.1334	1.83		Shallow Concentrated Flow, B-C					
							Woodland Kv= 5.0 fps					
	0.4	72	2 0	.0444	3.16		Shallow Concentrated Flow, C-D					
							Grassed Waterway Kv= 15.0 fps					
	0.2	55	5 0	.0100	5.94	10.50	Pipe Channel, D-E					
							18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'					
							n= 0.013 Corrugated PE, smooth interior					
	16.2	774	4 T	otal								

### Subcatchment PDA-3B: PDA-3B



#### Summary for Subcatchment PDA-3C: PDA-3C

Runoff = 18.30 cfs @ 12.23 hrs, Volume= 1.903 af, Depth= 4.06" Routed to Pond 4P : P-5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

	Area	(ac) (	CN Des	cription		
	1.	062	73 Woo	ods, Fair, F	ISG C	
	0.	126	71 Mea	dow, non-	grazed, HS	GC
*	4.	434	75 Mea	dow, non-	grazed, HS	G C/D
	5.	622	75 Wei	ghted Aver	age	
	5.	622	100.	00% Pervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.5	100	0.1300	0.17		Sheet Flow, A-B
						Woods: Light underbrush n= 0.400 P2= 3.62"
	2.0	249	0.1687	2.05		Shallow Concentrated Flow, B-C
						Woodland Kv= 5.0 fps
	3.2	463	0.1210	2.43		Shallow Concentrated Flow, C-D
						Short Grass Pasture Kv= 7.0 fps

#### 14.7 812 Total

#### Subcatchment PDA-3C: PDA-3C



### Summary for Subcatchment PDA-4: PDA-4

Runoff = 13.50 cfs @ 12.24 hrs, Volume= Routed to Link AP-2 : AP-2 1.441 af, Depth= 4.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 25-yr Rainfall=6.90"

	Area	(ac) (	CN	Desc	ription		
	1.	773	73	Woo	ds, Fair, H	ISG C	
	0.	005	79	Woo	ds, Fair, H	ISG D	
	0.	220	98	Pave	ed roads w	/curbs & se	ewers, HSG C
	0.	018	98	Pave	ed roads w	/curbs & se	ewers, HSG D
*	1.	408	75	Mead	dow, non-	grazed, HS	G C/D
	0.	819	71	Mead	dow, non-g	grazed, HS	GC
	0.	010	96	Grav	el surface	, HSG C	
	0.	003	96	Grav	el surface	, HSG D	~ -
	0.	003	78	Mead	dow, non-g	grazed, HS	G D
	4.	259	75	Weig	hted Aver	age	
	4.	021		94.4 <sup>-</sup>	1% Pervio	us Area	
	0.	238		5.59	% Impervi	ous Area	
	-		~			0	
		Length	S	lope	Velocity	Capacity	Description
	(min)	(feet)	(	π/π)	(tt/sec)	(CIS)	
	12.2	100	0.0	)700	0.14		Sheet Flow, A-B
							Woods: Light underbrush n= 0.400 P2= 3.62"
	2.3	301	0.1	827	2.14		Shallow Concentrated Flow, B-C
		00	~ ~		5.40		Woodland Kv= 5.0 fps
	0.2	63	0.0	635	5.12		Shallow Concentrated Flow, C-D
		005	0.0		10.07	46 70	Paved KV= 20.3 fps
	1.1	880	0.0	00/0	13.07	10.78	Pipe Channel, D-E
							n= 0.012 Corrugated PD smooth interior
	45.0	4 0 4 0	т.,	4 - 1			II- 0.012 Collugated FF, Shooth Interior

15.8 1,349 Total

#### Subcatchment PDA-4: PDA-4



## Summary for Pond 1P: P=1

Inflow Are	a =	4.751 ac,	0.72% Impervious	, Inflow Depth =	2.93" for	25-yr event
Inflow	=	11.07 cfs @	12.24 hrs, Volum	e= 1.158	af	-
Outflow	=	6.31 cfs @	12.42 hrs, Volum	e= 1.081	af, Atten=	43%, Lag= 11.1 min
Primary	=	6.31 cfs @	12.42 hrs, Volum	e= 1.081	af	-
Routed	to Link	AP-1 : AP-1				

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 428.77' @ 12.42 hrs Surf.Area= 5,095 sf Storage= 11,135 cf

Plug-Flow detention time= 81.9 min calculated for 1.080 af (93% of inflow) Center-of-Mass det. time= 47.4 min (928.9 - 881.5)

Volume	Invert	Avail.Sto	orage	Storage Description			
#1	426.00'	18,0	67 cf	Custom Stage Data	<b>a (Irregular)</b> Listed	below (Recalc)	
Elevatio (fee	on Su t)	rf.Area F (sg-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
426.0 427.0 428.0 429.0 430.0	)0 )0 )0 )0 )0 )0	3,028 3,726 4,480 5,291 6,158	223.1 242.0 260.8 279.7 298.5	0 3,371 4,097 4,880 5,719	0 3,371 7,468 12,348 18,067	3,028 3,765 4,558 5,415 6,327	
Device	Routing	Invert	Outle	et Devices			
#1	Primary	426.00'	<b>18.0</b> L= 6 Inlet n= 0	" Round Culvert 5.0' CMP, projecting / Outlet Invert= 426.0 .013 Corrugated PE	g, no headwall, Ke 00' / 425.00' S= 0 . smooth interior. F	= 0.900 .0154 '/'    Cc= 0.900 Flow Area= 1.77 sf	
#2 #3	Device 1 Device 1	427.00' 429.00'	15.0 36.0	" Vert. Orifice/Grate " x 21.0" Horiz. Orif	C= 0.600 Limite ice/Grate C= 0.60	ed to weir flow at low 00	heads
#4	Primary	429.00'	<b>25.0</b> Head Coef	' long + 3.0 '/' Sidez d (feet) 0.20 0.40 0 f. (English) 2.68 2.7	<b>Z x 15.0' breadth</b> .60 0.80 1.00 1.2 0 2.70 2.64 2.63	Broad-Crested Rect 0 1.40 1.60 2.64 2.64 2.63	angular Weir

**Primary OutFlow** Max=6.30 cfs @ 12.42 hrs HW=428.76' (Free Discharge)

**-1=Culvert** (Passes 6.30 cfs of 9.52 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 6.30 cfs @ 5.13 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 1P: P=1



## Summary for Pond 2P: P-2

Inflow Area	a =	1.880 ac,	0.00% Impe	ervious,	Inflow Depth =	3.64	I" for 2	25-yr event	
Inflow	=	7.12 cfs @	12.15 hrs,	Volume	= 0.570	) af		-	
Outflow	=	3.95 cfs @	12.26 hrs,	Volume	= 0.492	2 af, A	Atten= 44	4%, Lag=6	6.6 min
Primary	=	3.95 cfs @	12.26 hrs,	Volume	= 0.492	2 af		-	
Routed	to Link A	AP-1 : AP-1							

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 419.03' @ 12.26 hrs Surf.Area= 3,316 sf Storage= 7,224 cf

Plug-Flow detention time= 131.9 min calculated for 0.492 af (86% of inflow) Center-of-Mass det. time= 64.1 min (918.6 - 854.5 )

Volume	Invert	Avail.Sto	rage	Storage Description	า		
#1	416.00'	10,7	57 cf	Custom Stage Dat	t <b>a (Irregular)</b> Listed	below (Recalc)	
Elevatio (fee	on Su et)	ırf.Area F (sɑ-ft)	erim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
416.0 417.0 418.0 419.0 420.0	00 00 00 00 00	1,538 2,067 2,653 3,295 3,993	166.9 185.7 204.6 223.4 242.3	0 1,796 2,354 2,968 3,638	0 1,796 4,150 7,118 10,757	1,538 2,094 2,713 3,388 4,126	
Device	Routing	Invert	Outle	et Devices			
#1	Primary	416.00'	<b>18.0</b> L= 4 Inlet n= 0	<b>" Round Culvert</b> 0.0' CMP, projectin / Outlet Invert= 416. .013 Corrugated PE	g, no headwall, Ke 00' / 414.00' S= 0 . smooth interior. F	= 0.900 .0500 '/'    Cc= 0.900 Flow Area= 1.77 sf	
#2	Device 1	417.70'	12.0	" Vert. Orifice/Grate	e C= 0.600 Limite	ed to weir flow at low hea	ıds
#3	Device 1	419.00'	<b>36.0</b> Limit	" x 21.0" Horiz. Ori	fice/Grate C= 0.60 w heads	00	
#4	Primary	419.00'	<b>20.0</b> Head Coef	<b>' long + 3.0 '/' Side</b> d (feet) 0.20 0.40 0 f. (English) 2.68 2.7	<b>Z x 15.0' breadth</b> 0.60 0.80 1.00 1.2 70 2.70 2.64 2.63	Broad-Crested Rectang 0 1.40 1.60 2.64 2.64 2.63	jular Weir

**Primary OutFlow** Max=3.82 cfs @ 12.26 hrs HW=419.03' (Free Discharge)

**-1=Culvert** (Passes 3.58 cfs of 10.14 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 3.44 cfs @ 4.38 fps)

-3=Orifice/Grate (Weir Controls 0.14 cfs @ 0.54 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 0.24 cfs @ 0.44 fps)

Pond 2P: P-2



## Summary for Pond 3P: P-3

Inflow Area	a =	2.688 ac,	0.00% Impervious,	Inflow Depth =	3.43"	for 25-yr	<sup>-</sup> event
Inflow	=	8.02 cfs @	12.20 hrs, Volume	e 0.769	af	•	
Outflow	=	4.94 cfs @	12.35 hrs, Volume	e 0.700	af, Atter	า= 38%,	Lag= 8.5 min
Primary	=	4.94 cfs @	12.35 hrs, Volume	e 0.700	af		-
Routed	to Link /	AP-1 : AP-1					

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 458.32' @ 12.35 hrs Surf.Area= 4,299 sf Storage= 8,064 cf

Plug-Flow detention time= 102.9 min calculated for 0.700 af (91% of inflow) Center-of-Mass det. time= 55.6 min ( 920.4 - 864.8 )

Volume	Invert	Avail.Sto	orage	Storage Description	า		
#1	456.00'	16,3	75 cf	Custom Stage Dat	<b>a (Irregular)</b> Listed	below (Recalc)	
Elevatic (fee	n Su t)	rf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
456.0 457.0 458.0 459.0 460.0	00 00 00 00 00	2,689 3,345 4,057 4,825 5,652	209.2 228.1 246.9 265.8 284.6	0 3,011 3,695 4,435 5,233	0 3,011 6,706 11,142 16,375	2,689 3,382 4,131 4,944 5,812	
Device	Routing	Invert	Outle	et Devices			
#1	Primary	456.00'	<b>18.0</b> L= 6 Inlet n= 0	<b>" Round Culvert</b> 0.0' CMP, projecting / Outlet Invert= 456. .013 Corrugated PE	g, no headwall, Ke 00' / 445.00' S= 0 . smooth interior. F	= 0.900 .1833 '/'   Cc= 0.900 Flow Area= 1.77 sf	
#2	Device 1	457.00'	15.0	" Vert. Orifice/Grate	• C= 0.600 Limite	ed to weir flow at low h	eads
#3	Device 1	459.00'	<b>36.0</b> Limit	" x 21.0" Horiz. Orif ted to weir flow at lov	<b>fice/Grate</b> C= 0.60 v heads	00	
#4	Primary	459.00'	<b>16.0</b> Head Coet	<b>' long + 3.0 '/' Side</b> d (feet) 0.20 0.40 0 f. (English) 2.68 2.7	<b>Z x 15.0' breadth</b> 0.60 0.80 1.00 1.2 0 2.70 2.64 2.63	Broad-Crested Recta 0 1.40 1.60 2.64 2.64 2.63	ngular Weir

**Primary OutFlow** Max=4.94 cfs @ 12.35 hrs HW=458.32' (Free Discharge)

**-1=Culvert** (Passes 4.94 cfs of 8.42 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 4.94 cfs @ 4.02 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: P-3



### Summary for Pond 4P: P-5

Inflow Are	ea =	5.622 ac,	0.00% Impervious, Inflow	Depth = 4.06" for 25-yr event	
Inflow	=	18.30 cfs @	12.23 hrs, Volume=	1.903 af	
Outflow	=	9.86 cfs @	12.43 hrs, Volume=	1.758 af, Atten= 46%, Lag= 11.9 mi	in
Primary	=	9.86 cfs @	12.43 hrs, Volume=	1.758 af	
Routed	d to Lin	k AP-2 : AP-2			

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 481.10' @ 12.43 hrs Surf.Area= 10,315 sf Storage= 24,343 cf

Plug-Flow detention time= 113.0 min calculated for 1.756 af (92% of inflow) Center-of-Mass det. time= 73.2 min ( 923.0 - 849.7 )

Volume	Inver	: Avail.S	torage	Storage Description	on			
#1	478.00	<b>'</b> 34,	305 cf	Custom Stage Data (Irregular)Listed below (Recalc)				
Elevatio	on S	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area		
478.0 479.0 480.0 481.0 482.0	)0 )0 )0 )0 )0	5,499 6,994 8,546 10,155 11,820	489.1 507.9 526.8 545.6 564.5	0 6,232 7,757 9,339 10,977	0 6,232 13,989 23,328 34,305	5,499 7,072 8,712 10,403 12,163		
Device	Routing	Invei	rt Outle	et Devices				
#1	Primary	478.00	)' <b>18.0</b> L= 5 Inlet n= 0	<b>18.0" Round Culvert</b> L= $50.0'$ CMP, projecting, no headwall, Ke= $0.900$ Inlet / Outlet Invert= $478.00' / 476.00'$ S= $0.0400 '/$ Cc= $0.900$ n= $0.013$ Corrugated PE smooth interior. Flow Area= $1.77$		Ke= 0.900 = 0.0400 '/'    Cc= 0.900 r,  Flow Area= 1.77 sf		
#2	Device 1	479.00	)' 15.0	<b>15.0" Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads				
#3	Device 1	481.00	)' <b>36.0</b> Limit	" x 21.0" Horiz. O ted to weir flow at l	rifice/Grate C= ( ow heads	0.600		
#4	Primary	481.00' <b>20</b> He Ce		<b>).0' long x 15.0' breadth Broad-Crested Rectangular Weir</b> ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 oef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63				

**Primary OutFlow** Max=9.71 cfs @ 12.43 hrs HW=481.10' (Free Discharge)

**-1=Culvert** (Passes 8.10 cfs of 10.29 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 7.17 cfs @ 5.84 fps)

-3=Orifice/Grate (Weir Controls 0.93 cfs @ 1.02 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 1.61 cfs @ 0.83 fps)

### Pond 4P: P-5



## Summary for Pond 5P: P-4

Inflow Area =		4.786 ac,	0.71% Impervious, Infl	ow Depth = 3.03"	for 25-yr event
Inflow	=	11.12 cfs @	12.25 hrs, Volume=	1.207 af	-
Outflow	=	5.83 cfs @	12.48 hrs, Volume=	1.107 af, Atte	en= 48%, Lag= 13.4 min
Primary	=	5.83 cfs @	12.48 hrs, Volume=	1.107 af	-
Route	ed to Link	k AP-2 : AP-2			

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 462.60' @ 12.48 hrs Surf.Area= 6,470 sf Storage= 13,346 cf

Plug-Flow detention time= 102.3 min calculated for 1.105 af (92% of inflow) Center-of-Mass det. time= 60.2 min ( 940.1 - 879.9 )

Volume	Inver	t Avail.St	torage	Storage Description	on			
#1	460.00	' 23,	498 cf	Custom Stage D	<b>ata (Irregular)</b> List	ed below (Recalc)		
Elevatio	on S	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area		
(lee	el)	(sq-it)	(leet)	(Jeer-Siduo)	(cubic-leet)	(\$q-11)		
460.0	00	3,869	309.7	0	0	3,869		
461.0	00	4,826	328.5	4,339	4,339	4,876		
462.0	00	5,840	347.4	5,325	9,664	5,947		
463.0	00	6,910	366.2	6,368	16,031	7,073		
464.0	00	8,037	385.1	7,466	23,498	8,263		
Device	Routing	Inver	t Outle	et Devices				
#1	Primary	460.00	b' <b>18.0</b> L= 5 Inlet n= 0	" Round Culvert 0.0' CMP, project / Outlet Invert= 46 .013 Corrugated F	ing, no headwall, 0.00' / 458.00' S 'E, smooth interioi	Ke= 0.900 = 0.0400 '/'    Cc= 0.900 r,  Flow Area= 1.77 sf		
#2	Device 1	461.00	15.0	" Vert. Orifice/Gra	te C= 0.600 Lir	nited to weir flow at low heads		
#3	Device 1	463.00	9' <b>36.0</b> Limit	<b>36.0" x 21.0" Horiz. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads				
#4	Primary	463.00	" <b>15.0</b> Head Coet	<b>' long x 15.0' brea</b> d (feet) 0.20 0.40 f. (English) 2.68 2	adth Broad-Crest 0.60 0.80 1.00 .70 2.70 2.64 2.	ted Rectangular Weir 1.20 1.40 1.60 63 2.64 2.64 2.63		

**Primary OutFlow** Max=5.82 cfs @ 12.48 hrs HW=462.60' (Free Discharge)

**-1=Culvert** (Passes 5.82 cfs of 9.13 cfs potential flow)

**1**-2=Orifice/Grate (Orifice Controls 5.82 cfs @ 4.74 fps)

**3=Orifice/Grate** (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 5P: P-4



## Summary for Link AP-1: AP-1

Inflow A	Area	=	35.925 ac,	0.09% Impervious,	Inflow Depth = 3.4	45" for 25-yr event
Inflow	:	=	88.31 cfs @	12.26 hrs, Volume	= 10.322 af	
Primar	y :	=	88.31 cfs @	12.26 hrs, Volume	= 10.322 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs



#### Link AP-1: AP-1

## Summary for Link AP-2: AP-2

Inflow /	Area =	=	49.020 ac,	0.62% Impervious,	Inflow Depth = 3.3	39" for 25-yr event
Inflow	=		105.08 cfs @	12.29 hrs, Volume	= 13.838 af	
Primar	y =		105.08 cfs @	12.29 hrs, Volume	= 13.838 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs



### Link AP-2: AP-2

Riggs St_Prop.	NF
Prepared by Solli Engineering	
HydroCAD® 10 20-3g s/n 10621 © 2023 HydroCAD Software Solutions	SII C

Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPDA-1: PDA-1	Runoff Area=2.493 ac 0.00% Impervious Runoff Depth=3.21" Flow Length=720' Tc=13.0 min CN=60 Runoff=6.72 cfs 0.666 af
SubcatchmentPDA-2A:PDA-2A	Runoff Area=24.113 ac 0.00% Impervious Runoff Depth=4.55" Flow Length=1,686' Tc=16.6 min CN=72 Runoff=84.17 cfs 9.134 af
SubcatchmentPDA-2B: PDA-2B	Runoff Area=4.751 ac 0.72% Impervious Runoff Depth=3.65" Flow Length=813' Tc=14.8 min CN=64 Runoff=13.88 cfs 1.444 af
SubcatchmentPDA-2C: PDA-2C	Runoff Area=1.880 ac 0.00% Impervious Runoff Depth=4.43" Flow Length=482' Tc=7.3 min CN=71 Runoff=8.64 cfs 0.694 af
SubcatchmentPDA-2D: PDA-2D	Runoff Area=2.688 ac 0.00% Impervious Runoff Depth=4.21" Flow Length=618' Tc=12.3 min CN=69 Runoff=9.83 cfs 0.942 af
SubcatchmentPDA-3A:PDA-3A	Runoff Area=34.353 ac 0.10% Impervious Runoff Depth=4.09" Flow Length=2,518' Tc=19.6 min CN=68 Runoff=99.72 cfs 11.717 af
SubcatchmentPDA-3B: PDA-3B	Runoff Area=4.786 ac 0.71% Impervious Runoff Depth=3.76" Flow Length=774' Tc=16.2 min CN=65 Runoff=13.89 cfs 1.499 af
SubcatchmentPDA-3C: PDA-3C	Runoff Area=5.622 ac 0.00% Impervious Runoff Depth=4.89" Flow Length=812' Tc=14.7 min CN=75 Runoff=21.95 cfs 2.290 af
SubcatchmentPDA-4: PDA-4	Runoff Area=4.259 ac 5.59% Impervious Runoff Depth=4.89" Flow Length=1,349' Tc=15.8 min CN=75 Runoff=16.19 cfs 1.735 af
Pond 1P: P=1	Peak Elev=429.10' Storage=12,881 cf Inflow=13.88 cfs 1.444 af Outflow=10.22 cfs 1.365 af
Pond 2P: P-2	Peak Elev=419.12' Storage=7,523 cf Inflow=8.64 cfs 0.694 af Outflow=7.23 cfs 0.616 af
Pond 3P: P-3	Peak Elev=458.61' Storage=9,314 cf Inflow=9.83 cfs 0.942 af Outflow=5.86 cfs 0.873 af
Pond 4P: P-5	Peak Elev=481.23' Storage=25,679 cf Inflow=21.95 cfs 2.290 af Outflow=16.31 cfs 2.146 af
Pond 5P: P-4	Peak Elev=463.03' Storage=16,204 cf Inflow=13.89 cfs 1.499 af Outflow=7.32 cfs 1.399 af
Link AP-1: AP-1	Inflow=108.01 cfs 12.654 af Primary=108.01 cfs 12.654 af
Link AP-2: AP-2	Inflow=135.30 cfs 16.996 af Primary=135.30 cfs 16.996 af

Total Runoff Area = 84.945 ac Runoff Volume = 30.121 af Average Runoff Depth = 4.26" 99.60% Pervious = 84.605 ac 0.40% Impervious = 0.340 ac

#### Summary for Subcatchment PDA-1: PDA-1

Runoff = 6.72 cfs @ 12.21 hrs, Volume= 0.666 af, Depth= 3.21" Routed to Link AP-1 : AP-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

 Area (	(ac)	CN	Desc	cription		
 1.	713	60	Woo	ds, Fair, H	ISG B	
0.	730	58	Mea	dow, non-g	grazed, HS	GB
 0.	050	96	Grav	el surface	, HSG B	
 2.4	493	60	Weig	phted Aver	age	
2.4	493		100.0	, 00% Pervi	ous Area	
Тс	Length	ר ו	Slope	Velocity	Capacity	Description
 (min)	(feet	)	(ft/ft)	(ft/sec)	(cfs)	
8.0	100	0 (	.2000	0.21		Sheet Flow, A-B
						Woods: Light underbrush n= 0.400 P2= 3.62"
5.0	620	0 (	.1710	2.07		Shallow Concentrated Flow, B-C
						Woodland Kv= 5.0 fps
13 0	720	) T	otal			

#### Subcatchment PDA-1: PDA-1



### Summary for Subcatchment PDA-2A: PDA-2A

Runoff = 84.17 cfs @ 12.25 hrs, Volume= 9.134 af, Depth= 4.55" Routed to Link AP-1 : AP-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

	Area	(ac) (	N Des	cription		
_	6.	774	60 Wo	ods, Fair, H	ISG B	
	13.	166	79 Wo	ods, Fair, F	ISG D	
	1.	006	58 Mea	adow, non-	grazed, HS	GB
*	1.	425	65 Mea	adow, non-	grazed, HS	G B/C
_	1.	742	78 Mea	adow, non-	grazed, HS	G D
	24.	113	72 Wei	ghted Avei	rage	
	24.	113	100	.00% Perv	ious Area	
	_					
	TC	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cts)	
	10.2	100	0.1100	0.16		Sheet Flow, A-B
						Woods: Light underbrush n= 0.400 P2= 3.62"
	4.7	635	0.2000	2.24		Shallow Concentrated Flow, B-C
						Woodland Kv= 5.0 fps
	1.7	951	0.0421	9.10	151.99	Channel Flow, C-D
						Area= 16.7 sf Perim= 12.8' r= 1.30'
_						n= 0.040 Winding stream, pools & shoals
	16.6	1.686	Total			

### Subcatchment PDA-2A: PDA-2A



### Summary for Subcatchment PDA-2B: PDA-2B

Runoff = 13.88 cfs @ 12.23 hrs, Volume= 1.444 af, Depth= 3.65" Routed to Pond 1P : P=1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

	Area	(ac)	CN	Desc	cription		
	0.	959	60	Woo	ds, Fair, H	ISG B	
	0.	012	79	Woo	ds, Fair, H	ISG D	
	0.	395	58	Mea	dow, non-g	grazed, HS	G B
*	3.	239	65	Mea	dow, non-	grazed, HS	G B/C
*	0.	034	98	Unco	onnected r	oofs, HSG	B/C
	0.	112	96	Grav	el surface	, HSG B	
	4.	751	64	Weig	ghted Aver	age	
	4.	717		99.2	8% Pervio	us Area	
	0.	034		0.72	% Impervi	ous Area	
	0.	034		100.0	00% Únco	nnected	
	Тс	Length	າ ຮ	Slope	Velocity	Capacity	Description
_	(min)	(feet)	)	(ft/ft)	(ft/sec)	(cfs)	
	10.2	100	0.	1100	0.16		Sheet Flow, A-B
							Woods: Light underbrush n= 0.400 P2= 3.62"
	0.2	33	B 0.	3000	2.74		Shallow Concentrated Flow, B-C
							Woodland Kv= 5.0 fps
	4.4	680	) 0.	1367	2.59		Shallow Concentrated Flow, C-D
_							Short Grass Pasture Kv= 7.0 fps
	14.8	813	3 To	otal			

### Subcatchment PDA-2B: PDA-2B



#### Summary for Subcatchment PDA-2C: PDA-2C

Runoff = 8.64 cfs @ 12.14 hrs, Volume= 0.694 af, Depth= 4.43" Routed to Pond 2P : P-2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

	Area	(ac) (	CN De	scription			
	0.	089	58 Me	adow, non-	grazed, HS	G B	
*	0.	667	65 Me	adow, non-	grazed, HS	G B/C	
	0.	930	78 Me	adow, non-	grazed, HS	G D	
	0.	105	60 Wc	ods, Fair, F	ÍSG B		
	0.	089	79 Wc	ods, Fair, F	ISG D		
	1.	880	71 We	ighted Ave	rage		
	1.	880	100	).00% Perv	ious Area		
	Тс	Length	Slope	e Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	5.1	100	0.2200	0.32		Sheet Flow, A-B	
						Grass: Dense n= 0.240 P2= 3.62"	
	2.0	340	0.1647	2.84		Shallow Concentrated Flow, B-C	
						Short Grass Pasture Kv= 7.0 fps	
	0.2	42	0.0476	3.27		Shallow Concentrated Flow, C-D	
						Grassed Waterway Kv= 15.0 fps	
	7.3	482	Total				

#### Subcatchment PDA-2C: PDA-2C

Hydrograph



#### Summary for Subcatchment PDA-2D: PDA-2D

Runoff = 9.83 cfs @ 12.20 hrs, Volume= 0.942 af, Depth= 4.21" Routed to Pond 3P : P-3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

	Area	(ac)	CN	Desc	ription			
* 1.831 65		65	Meadow, non-grazed, HSG B/C					
0.857 78 Meadow, non-grazed, HS						grazed, HS	G D	
	2.688		69	Weig	hted Aver	age		
	2.688			100.0	00% Pervi	ous Area		
	Tc (min)	Lengtł (feet	n Slo ) (f	ope t/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	9.3	100	0.05	500	0.18		Sheet Flow, A-B	
	3.0	518	3 0.16	654	2.85		Grass: Dense n= 0.240 P2= 3.62" <b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps	
	12.3	618	3 Tota	al				

#### Subcatchment PDA-2D: PDA-2D



### Summary for Subcatchment PDA-3A: PDA-3A

Runoff = 99.72 cfs @ 12.29 hrs, Volume= Routed to Link AP-2 : AP-2

11.717 af, Depth= 4.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

	Area	(ac)	CN	Desc	cription				
	0.	515	36	Woo	ds, Fair, H	ISG A			
	12.	604	60	Woo	ds, Fair, H	ISG B			
	10.	470	73	Woo	ds, Fair, H	ISG C			
	5.	295	79	Woo	ds, Fair, H	ISG D			
*	2.	024	75	Mea	dow, non-g	grazed, HS	G C/D		
	0.	034	98	Unco	onnected p	pavement, l	HSG D		
	0.	397	96	Grav	el surface	, HSG C			
	1.	195	58	Mea	dow, non-o	grazed, HS	GB		
	0.	931	71	Mea	dow, non-(	grazed, HS	GC		
÷	0.	448	/8	Mea	dow, non-(	grazed, HS	GD		
_	0.	440	65	Mea	dow, non-g	grazed, HS	G B/C		
	34.	353	68	Weig	hted Aver	age			
	34.	319		99.9	0% Pervio	us Area			
	0.034		0.10	100 00% Upgenpacted					
	0.	034		100.0		nneclea			
	Тс	l enath	, ¢	Slone	Velocity	Canacity	Description		
	(min)	(feet)	)	(ft/ft)	(ft/sec)	(cfs)	Decemption		
	8.8	100	) ()	1600	0 19	()	Sheet Flow, A-B		
	0.0		. 0.		0.10		Woods: Light underbrush n= 0.400 P2= 3.62"		
	8.1	735	50.	0912	1.51		Shallow Concentrated Flow, B-C		
							Woodland Kv= 5.0 fps		
	2.7	1,683	<b>3</b> 0.	0552	10.50	346.60	Channel Flow, C-D		
							Area= 33.0 sf Perim= 25.0' r= 1.32'		
							n= 0.040 Winding stream, pools & shoals		
	19.6	2,518	3 To	otal					

#### Subcatchment PDA-3A: PDA-3A



# Summary for Subcatchment PDA-3B: PDA-3B

[47] Hint: Peak is 132% of capacity of segment #4

Runoff = 13.89 cfs @ 12.25 hrs, Volume= Routed to Pond 5P : P-4 1.499 af, Depth= 3.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

_	Area	(ac)	CN	Desc	cription						
*	4.	728	65	Mea	dow, non-	grazed, HS	G B/C				
	0.	024	96	Grav	el surface	, HSG B					
	0.	034	98	Unco	onnected p	pavement, l	HSG B				
4.786 65			Weig	Weighted Average							
	4.	752		99.29% Pervious Area							
	0.	034		0.71	% Impervi	ous Area					
	0.	034		100.	00% Ünco	nnected					
	Тс	Lengtl	n	Slope	Velocity	Capacity	Description				
_	(min)	(feet	)	(ft/ft)	(ft/sec)	(cfs)					
	10.6	100	0 C	.1000	0.16		Sheet Flow, A-B				
							Woods: Light underbrush n= 0.400 P2= 3.62"				
	5.0	54	70	.1334	1.83		Shallow Concentrated Flow, B-C				
							Woodland Kv= 5.0 fps				
	0.4	72	20	.0444	3.16		Shallow Concentrated Flow, C-D				
							Grassed Waterway Kv= 15.0 fps				
	0.2	5	50	.0100	5.94	10.50	Pipe Channel, D-E				
							18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'				
							n= 0.013 Corrugated PE, smooth interior				
	16.2	774	4 T	otal							

### Subcatchment PDA-3B: PDA-3B



#### Summary for Subcatchment PDA-3C: PDA-3C

Runoff = 21.95 cfs @ 12.23 hrs, Volume= 2.290 af, Depth= 4.89" Routed to Pond 4P : P-5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

	Area	(ac) (	CN Des	cription		
	1.	062	73 Woo	ods, Fair, F	ISG C	
	0.	126	71 Mea	dow, non-	grazed, HS	GC
*	4.	434	75 Mea	idow, non-	grazed, HS	G C/D
	5.622 75		75 Wei	ghted Aver	age	
	5.	622	100	00% Pervi	ious Area	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.5	100	0.1300	0.17		Sheet Flow, A-B
						Woods: Light underbrush n= 0.400 P2= 3.62"
	2.0	249	0.1687	2.05		Shallow Concentrated Flow, B-C
						Woodland Kv= 5.0 fps
	3.2	463	0.1210	2.43		Shallow Concentrated Flow, C-D
						Short Grass Pasture Kv= 7.0 fps
	14.7	812	Total			

#### Subcatchment PDA-3C: PDA-3C



### Summary for Subcatchment PDA-4: PDA-4

Runoff = 16.19 cfs @ 12.24 hrs, Volume= Routed to Link AP-2 : AP-2 1.735 af, Depth= 4.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 50-yr Rainfall=7.83"

	Area	(ac) (	CN	Desc	cription				
	1.	773	73	Woo	ds, Fair, H	ISG C			
	0.	005	79	Woo	ds, Fair, H	ISG D			
	0.	220	98	Pave	ed roads w	/curbs & se	ewers, HSG C		
	0.	018	98	Pave	ed roads w	/curbs & se	ewers, HSG D		
*	1.	408	75	Mea	dow, non-g	grazed, HS	G C/D		
	0.	819	71	Mea	dow, non-	grazed, HS	GC		
	0.	010	96	Grav	el surface	, HSG C			
	0.	003	96	Grav	el surface	, HSG D			
	0.	003	/8	Mea	dow, non-	grazed, HS	G D		
	4.	259	75	Weig	ghted Aver	age			
	4.	021		94.4	1% Pervio	us Area			
	0.	238		5.59% Impervious Area					
	Тс	Length	S	lone	Velocity	Canacity	Description		
	(min)	(feet)		(ft/ft)	(ft/sec)	(cfs)	Description		
	12.2	100	0 (	1700	0 14	(010)	Sheet Flow A-B		
	12.2	100	0.0	51.00	0.11		Woods: Light underbrush n= 0.400 P2= 3.62"		
	2.3	301	0.1	1827	2.14		Shallow Concentrated Flow, B-C		
							Woodland Kv= 5.0 fps		
	0.2	63	0.0	0635	5.12		Shallow Concentrated Flow, C-D		
							Paved Kv= 20.3 fps		
	1.1	885	0.0	)575	13.67	16.78	Pipe Channel, D-E		
							15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
							n= 0.012 Corrugated PP, smooth interior		
	15.8	1,349	То	tal					
## Subcatchment PDA-4: PDA-4



## Summary for Pond 1P: P=1

Inflow Area	a =	4.751 ac,	0.72% Impervious,	Inflow Depth = 3	3.65" fo	or 50-yr	event
Inflow	=	13.88 cfs @	12.23 hrs, Volume	= 1.444 a	af	-	
Outflow	=	10.22 cfs @	12.36 hrs, Volume	= 1.365 a	af, Atten=	= 26%, l	_ag= 7.8 min
Primary	=	10.22 cfs @	12.36 hrs, Volume	= 1.365 a	af		-
Routed	to Link	AP-1 : AP-1					

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 429.10' @ 12.37 hrs Surf.Area= 5,375 sf Storage= 12,881 cf

Plug-Flow detention time= 72.8 min calculated for 1.365 af (95% of inflow) Center-of-Mass det. time= 43.1 min (916.3 - 873.3)

Volume	Invert	Avail.Sto	rage	Storage Description			
#1	426.00'	18,0	67 cf	Custom Stage Data	(Irregular)Listed	below (Recalc)	
Elevatio (fee	n Su t)	rf.Area P (sq-ft)	erim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
426.0 427.0 428.0 429.0 430.0	00 00 00 00 00	3,028 3,726 4,480 5,291 6,158	223.1 242.0 260.8 279.7 298.5	0 3,371 4,097 4,880 5,719	0 3,371 7,468 12,348 18,067	3,028 3,765 4,558 5,415 6,327	
Device	Routing	Invert	Outle	et Devices			
#1	Primary	426.00'	<b>18.0'</b> L= 68 Inlet n= 0.	' Round Culvert 5.0' CMP, projecting / Outlet Invert= 426.0 013 Corrugated PE.	, no headwall, Ke 0' / 425.00' S= 0. smooth interior. F	= 0.900 0154 '/'    Cc= 0.900 Flow Area= 1.77 sf	
#2 #3	Device 1 Device 1	427.00' 429.00'	15.0' 36.0' Limit	' Vert. Orifice/Grate ' x 21.0" Horiz. Orified to weir flow at low	C= 0.600 Limite ce/Grate C= 0.60 heads	ed to weir flow at low h 0	neads
#4	Primary	429.00'	<b>25.0'</b> Heac Coef	long + 3.0 '/ SideZ (feet) 0.20 0.40 0.1 . (English) 2.68 2.70	<b>x 15.0' breadth</b>   60 0.80 1.00 1.2 0 2.70 2.64 2.63	Broad-Crested Recta 0 1.40 1.60 2.64 2.64 2.63	angular Weir

**Primary OutFlow** Max=9.81 cfs @ 12.36 hrs HW=429.09' (Free Discharge)

**-1=Culvert** (Passes 7.99 cfs of 10.28 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 7.15 cfs @ 5.83 fps)

-3=Orifice/Grate (Weir Controls 0.84 cfs @ 0.98 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 1.82 cfs @ 0.80 fps)

Pond 1P: P=1



## Summary for Pond 2P: P-2

Inflow Area	ı =	1.880 ac,	0.00% Impervious	, Inflow Depth =	4.43" f	or 50-yı	<sup>-</sup> event
Inflow	=	8.64 cfs @	12.14 hrs, Volum	e= 0.694	af	-	
Outflow	=	7.23 cfs @	12.21 hrs, Volum	e= 0.616	af, Atten	= 16%,	Lag= 3.6 min
Primary	=	7.23 cfs @	12.21 hrs, Volum	e= 0.616	af		-
Routed	to Link A	AP-1 : AP-1					

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 419.12' @ 12.21 hrs Surf.Area= 3,376 sf Storage= 7,523 cf

Plug-Flow detention time= 115.7 min calculated for 0.616 af (89% of inflow) Center-of-Mass det. time= 57.4 min (904.5 - 847.2)

Volume	Invert	Avail.Sto	orage	Storage Description	1		
#1	416.00'	10,7	57 cf	Custom Stage Dat	<b>a (Irregular)</b> Listed	below (Recalc)	
Elevatic (fee	on Su :t)	ırf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
416.0 417.0 418.0 419.0 420.0	)0 )0 )0 )0 )0	1,538 2,067 2,653 3,295 3,993	166.9 185.7 204.6 223.4 242.3	0 1,796 2,354 2,968 3,638	0 1,796 4,150 7,118 10,757	1,538 2,094 2,713 3,388 4,126	
Device	Routing	Invert	Outle	et Devices			
#1	Primary	416.00'	<b>18.0'</b> L= 4( Inlet n= 0.	<b>' Round Culvert</b> 0.0' CMP, projecting / Outlet Invert= 416. .013 Corrugated PE	g, no headwall, Ke 00' / 414.00' S= 0 . smooth interior. F	= 0.900 .0500 '/'    Cc= 0.900 Flow Area= 1.77 sf	
#2 #3	Device 1 Device 1	417.70' 419.00'	12.0' 36.0'	Vert. Orifice/Grate x 21.0" Horiz. Orif ed to weir flow at low	C= 0.600 Limite ice/Grate C= 0.60 v heads	ed to weir flow at low 00	heads
#4	Primary	419.00'	<b>20.0'</b> Head Coef	long + 3.0 '/' Side (feet) 0.20 0.40 0 . (English) 2.68 2.7	<b>Z x 15.0' breadth</b> .60 0.80 1.00 1.2 0 2.70 2.64 2.63	Broad-Crested Rec 0 1.40 1.60 2.64 2.64 2.63	tangular Weir

**Primary OutFlow** Max=7.02 cfs @ 12.21 hrs HW=419.12' (Free Discharge)

**-1=Culvert** (Passes 4.86 cfs of 10.33 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 3.62 cfs @ 4.61 fps)

-3=Orifice/Grate (Weir Controls 1.24 cfs @ 1.12 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 2.17 cfs @ 0.91 fps)

# Pond 2P: P-2



## Summary for Pond 3P: P-3

Inflow Area	ı =	2.688 ac,	0.00% Impervious	Inflow Depth =	4.21" for	50-yr event
Inflow	=	9.83 cfs @	12.20 hrs, Volum	e= 0.942 a	af	-
Outflow	=	5.86 cfs @	12.35 hrs, Volum	e= 0.873 a	af, Atten= 4	40%, Lag= 8.9 min
Primary	=	5.86 cfs @	12.35 hrs, Volum	e= 0.873 a	af	-
Routed	to Link A	AP-1 : AP-1				

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 458.61' @ 12.35 hrs Surf.Area= 4,517 sf Storage= 9,314 cf

Plug-Flow detention time= 89.1 min calculated for 0.872 af (93% of inflow) Center-of-Mass det. time= 50.5 min (907.8 - 857.3)

Volume	Invert	Avail.Sto	orage	Storage Description	า		
#1	456.00'	16,3	75 cf	Custom Stage Dat	<b>ta (Irregular)</b> Listed	below (Recalc)	
Elevatio (fee	on Su	rf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sg-ft)	
456.0 457.0 458.0 459.0 460.0	00 00 00 00 00	2,689 3,345 4,057 4,825 5,652	209.2 228.1 246.9 265.8 284.6	0 3,011 3,695 4,435 5,233	0 3,011 6,706 11,142 16,375	2,689 3,382 4,131 4,944 5,812	
Device	Routing	Invert	Outle	et Devices			
#1	Primary	456.00'	<b>18.0</b> L= 6 Inlet n= 0	" Round Culvert 0.0' CMP, projectin / Outlet Invert= 456 .013 Corrugated PE	ig, no headwall, Ke .00' / 445.00' S= 0 5. smooth interior. 1	= 0.900 .1833 '/'    Cc= 0.900 Flow Area= 1.77 sf	1
#2	Device 1	457.00'	15.0	" Vert. Orifice/Grat	e C= 0.600 Limite	ed to weir flow at low	v heads
#3	Device 1	459.00'	<b>36.0</b> Limit	" x 21.0" Horiz. Ori ed to weir flow at lov	fice/Grate C= 0.60 w heads	00	
#4	Primary	459.00'	<b>16.0</b> Head Coef	<b>' long + 3.0 '/' Side</b> d (feet) 0.20 0.40 ( f. (English) 2.68 2.7	<b>Z x 15.0' breadth</b> 0.60 0.80 1.00 1.2 70 2.70 2.64 2.63	Broad-Crested Red 20 1.40 1.60 2.64 2.64 2.63	tangular Weir:

**Primary OutFlow** Max=5.86 cfs @ 12.35 hrs HW=458.61' (Free Discharge)

**-1=Culvert** (Passes 5.86 cfs of 9.16 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 5.86 cfs @ 4.77 fps)

**3=Orifice/Grate** (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: P-3



## Summary for Pond 4P: P-5

Inflow Area	a =	5.622 ac,	0.00% Impervious,	Inflow Depth = $4$	4.89" for	50-yr event
Inflow	=	21.95 cfs @	12.23 hrs, Volume	;= 2.290 a	af	-
Outflow	=	16.31 cfs @	12.35 hrs, Volume	;= 2.146 a	af, Atten= 2	26%, Lag= 7.6 min
Primary	=	16.31 cfs @	12.35 hrs, Volume	;= 2.146 a	af	-
Routed	to Link	AP-2 : AP-2				

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 481.23' @ 12.35 hrs Surf.Area= 10,523 sf Storage= 25,679 cf

Plug-Flow detention time= 100.8 min calculated for 2.143 af (94% of inflow) Center-of-Mass det. time= 67.1 min ( 909.9 - 842.9 )

Volume	Inver	t Avail.	Storage	Storage Description	on	
#1	478.00	' 34	4,305 cf	Custom Stage Da	ata (Irregular)Liste	ed below (Recalc)
Elevatio	on S	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(fee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
478.0	00	5,499	489.1	0	0	5,499
479.0	00	6,994	507.9	6,232	6,232	7,072
480.0	00	8,546	526.8	7,757	13,989	8,712
481.0	00	10,155	545.6	9,339	23,328	10,403
482.0	00	11,820	564.5	10,977	34,305	12,163
Device	Routing	Inve	ert Outle	et Devices		
#1	Primary	478.0	00' <b>18.0</b> '	" Round Culvert		
			L= 5	0.0' CMP, project	ing, no headwall,	Ke= 0.900
			Inlet	/ Outlet Invert= 47	8.00'/476.00'S=	= 0.0400 '/'     Cc= 0.900
			n= 0	.013 Corrugated P	E, smooth interior	,Flow Area= 1.77 sf
#2	Device 1	479.0	00' <b>15.0</b> '	" Vert. Orifice/Gra	te C= 0.600 Lin	nited to weir flow at low heads
#3	Device 1	481.0	00' <b>36.0</b> '	" x 21.0" Horiz. O	rifice/Grate C= 0	.600
			Limit	ed to weir flow at lo	ow heads	
#4	Primary	481.0	00' <b>20.0</b> '	' long x 15.0' brea	adth Broad-Crest	ed Rectangular Weir
			Head	d (feet) 0.20 0.40	0.60 0.80 1.00	1.20 1.40 1.60
			Coef	. (English) 2.68 2	.70 2.70 2.64 2.6	63 2.64 2.64 2.63

**Primary OutFlow** Max=16.29 cfs @ 12.35 hrs HW=481.22' (Free Discharge)

-1=Culvert (Inlet Controls 10.57 cfs @ 5.98 fps)

**2=Orifice/Grate** (Passes < 7.47 cfs potential flow)

**3=Orifice/Grate** (Passes < 3.31 cfs potential flow)

-4=Broad-Crested Rectangular Weir (Weir Controls 5.72 cfs @ 1.27 fps)

## Pond 4P: P-5



## Summary for Pond 5P: P-4

Inflow Are	a =	4.786 ac,	0.71% Impervious, Inf	low Depth = 3.76"	for 50-yr event
Inflow	=	13.89 cfs @	12.25 hrs, Volume=	1.499 af	-
Outflow	=	7.32 cfs @	12.47 hrs, Volume=	1.399 af, Atte	n= 47%, Lag= 13.3 min
Primary	=	7.32 cfs @	12.47 hrs, Volume=	1.399 af	-
Routed	to Link	AP-2 : AP-2			

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 463.03' @ 12.47 hrs Surf.Area= 6,937 sf Storage= 16,204 cf

Plug-Flow detention time= 91.0 min calculated for 1.399 af (93% of inflow) Center-of-Mass det. time= 55.0 min (926.8 - 871.8)

Volume	Inver	t Avail.S	torage	Storage Description	on	
#1	460.00	o' 23,	,498 cf	Custom Stage Da	ata (Irregular)Liste	ed below (Recalc)
Elevatio	on S	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
460.0 461.0 462.0 463.0 464.0	50 00 00 00 00 00	3,869 4,826 5,840 6,910 8,037	309.7 328.5 347.4 366.2 385.1	0 4,339 5,325 6,368 7,466	0 4,339 9,664 16,031 23,498	3,869 4,876 5,947 7,073 8,263
Device	Routing	Inve	rt Outle	et Devices		
#1	Primary	460.00	0' <b>18.0</b> L= 5 Inlet n= 0	<b>" Round Culvert</b> 0.0' CMP, projecti / Outlet Invert= 460 .013 Corrugated P	ng, no headwall,  I ).00' / 458.00'   S= E, smooth interior,	Ke= 0.900 0.0400 '/' Cc= 0.900 Flow Area= 1.77 sf
#2 #3	Device 1 Device 1	461.00 463.00	0' <b>15.0</b> 0' <b>36.0</b> Limit	" Vert. Orifice/Gra " x 21.0" Horiz. Or red to weir flow at lo	te C= 0.600 Lim fifice/Grate C= 0.	ited to weir flow at low heads 600
#4	Primary	463.00	)' <b>15.0</b> Head Coef	' <b>long x 15.0' brea</b> d (feet) 0.20 0.40 f. (English) 2.68 2	adth Broad-Crest 0.60 0.80 1.00 1 .70 2.70 2.64 2.6	ed Rectangular Weir .20 1.40 1.60 3 2.64 2.64 2.63

**Primary OutFlow** Max=7.18 cfs @ 12.47 hrs HW=463.02' (Free Discharge)

-1=Culvert (Passes 7.07 cfs of 10.12 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 6.98 cfs @ 5.69 fps)

-3=Orifice/Grate (Weir Controls 0.09 cfs @ 0.46 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 0.11 cfs @ 0.38 fps)

Pond 5P: P-4



## Summary for Link AP-1: AP-1

Inflow A	Area =	35.925 ac,	0.09% Impervious,	Inflow Depth = 4.	23" for 50-yr event
Inflow	=	108.01 cfs @	12.25 hrs, Volume	= 12.654 af	
Primary	y =	108.01 cfs @	12.25 hrs, Volume	= 12.654 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs



## Link AP-1: AP-1

## Summary for Link AP-2: AP-2

Inflow /	Area :	=	49.020 ac,	0.62% Impervious,	Inflow Depth = $4.7$	16" for 50-yr event
Inflow	=	=	135.30 cfs @	12.31 hrs, Volume	= 16.996 af	
Primar	y =	=	135.30 cfs @	12.31 hrs, Volume	= 16.996 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs



#### Link AP-2: AP-2

Riggs St_Prop.	NRC
Prepared by Solli Engineering	
HvdroCAD® 10.20-3g_s/n 10621 © 2023 HvdroCAD Software	Solutions LLC

Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPDA-1: PDA-1	Runoff Area=2.493 ac 0.00% Impervious Runoff Depth=3.98" Flow Length=720' Tc=13.0 min CN=60 Runoff=8.41 cfs 0.828 af
SubcatchmentPDA-2A:PDA-2A	Runoff Area=24.113 ac 0.00% Impervious Runoff Depth=5.45" Flow Length=1,686' Tc=16.6 min CN=72 Runoff=100.59 cfs 10.947 af
SubcatchmentPDA-2B: PDA-2B	Runoff Area=4.751 ac 0.72% Impervious Runoff Depth=4.47" Flow Length=813' Tc=14.8 min CN=64 Runoff=17.07 cfs 1.770 af
SubcatchmentPDA-2C: PDA-2C	Runoff Area=1.880 ac 0.00% Impervious Runoff Depth=5.33" Flow Length=482' Tc=7.3 min CN=71 Runoff=10.34 cfs 0.834 af
SubcatchmentPDA-2D: PDA-2D	Runoff Area=2.688 ac 0.00% Impervious Runoff Depth=5.08" Flow Length=618' Tc=12.3 min CN=69 Runoff=11.86 cfs 1.138 af
SubcatchmentPDA-3A:PDA-3A	Runoff Area=34.353 ac 0.10% Impervious Runoff Depth=4.96" Flow Length=2,518' Tc=19.6 min CN=68 Runoff=120.84 cfs 14.195 af
SubcatchmentPDA-3B: PDA-3B	Runoff Area=4.786 ac 0.71% Impervious Runoff Depth=4.59" Flow Length=774' Tc=16.2 min CN=65 Runoff=17.02 cfs 1.831 af
SubcatchmentPDA-3C:PDA-3C	Runoff Area=5.622 ac 0.00% Impervious Runoff Depth>5.81" Flow Length=812' Tc=14.7 min CN=75 Runoff=25.99 cfs 2.724 af
SubcatchmentPDA-4: PDA-4	Runoff Area=4.259 ac 5.59% Impervious Runoff Depth>5.81" Flow Length=1,349' Tc=15.8 min CN=75 Runoff=19.17 cfs 2.064 af
Pond 1P: P=1	Peak Elev=429.19' Storage=13,357 cf Inflow=17.07 cfs 1.770 af Outflow=15.49 cfs 1.694 af
Pond 2P: P-2	Peak Elev=419.18' Storage=7,717 cf Inflow=10.34 cfs 0.834 af Outflow=9.96 cfs 0.755 af
Pond 3P: P-3	Peak Elev=458.92' Storage=10,761 cf Inflow=11.86 cfs 1.138 af Outflow=6.73 cfs 1.069 af
Pond 4P: P-5	Peak Elev=481.35' Storage=26,934 cf Inflow=25.99 cfs 2.724 af Outflow=21.76 cfs 2.580 af
Pond 5P: P-4	Peak Elev=463.19' Storage=17,393 cf Inflow=17.02 cfs 1.831 af Outflow=13.44 cfs 1.732 af
Link AP-1: AP-1	Inflow=133.77 cfs 15.293 af Primary=133.77 cfs 15.293 af
Link AP-2: AP-2	Inflow=168.78 cfs 20.570 af Primary=168.78 cfs 20.570 af

Total Runoff Area = 84.945 acRunoff Volume = 36.331 afAverage Runoff Depth = 5.13"99.60% Pervious = 84.605 ac0.40% Impervious = 0.340 ac

#### Summary for Subcatchment PDA-1: PDA-1

Runoff	=	8.41 cfs @	12.21 hrs,	Volume=	0.828 af,	Depth=	3.98"
Routed	l to Li	ink AP-1 : AP-1					

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

_	Area	(ac)	CN	Desc	cription			
	1.713 60 Woods, Fair, HSG B							
	0.	730	58	Mea	dow, non-	grazed, HS	GB	
	0.	050	96	Grav	el surface	, HSG B		
	2.493 60 Weighted Average							
	2.	493		100.0	, 00% Pervi	ous Area		
	Тс	Length	า :	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	)	(ft/ft)	(ft/sec)	(cfs)		
	8.0	100	0	.2000	0.21		Sheet Flow, A-B	
							Woods: Light underbrush n= 0.400 P2= 3.62"	
	5.0	620	0	.1710	2.07		Shallow Concentrated Flow, B-C	
							Woodland Kv= 5.0 fps	
_	13.0	720	) Т	otal				

## Subcatchment PDA-1: PDA-1



## Summary for Subcatchment PDA-2A: PDA-2A

Runoff = 100.59 cfs @ 12.25 hrs, Volume= 10.947 af, Depth= 5.45" Routed to Link AP-1 : AP-1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

	Area	(ac) (	N Des	cription						
_	6.	774	60 Wo	ods, Fair, H	ISG B					
	13.	166	79 Wo	Woods, Fair, HSG D						
	1.	006	58 Mea	adow, non-	grazed, HS	GB				
*	1.	425	65 Mea	adow, non-	grazed, HS	G B/C				
_	1.	742	78 Mea	adow, non-	grazed, HS	G D				
	24.	113	72 Wei	ghted Avei	rage					
	24.	113	100	.00% Perv	ious Area					
	_									
	TC	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cts)					
	10.2	100	0.1100	0.16		Sheet Flow, A-B				
						Woods: Light underbrush n= 0.400 P2= 3.62"				
	4.7	635	0.2000	2.24		Shallow Concentrated Flow, B-C				
						Woodland Kv= 5.0 fps				
	1.7	951	0.0421	9.10	151.99	Channel Flow, C-D				
						Area= 16.7 sf Perim= 12.8' r= 1.30'				
_						n= 0.040 Winding stream, pools & shoals				
	16.6	1.686	Total							

## Subcatchment PDA-2A: PDA-2A



## Summary for Subcatchment PDA-2B: PDA-2B

Runoff = 17.07 cfs @ 12.23 hrs, Volume= 1.770 af, Depth= 4.47" Routed to Pond 1P : P=1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

	Area	(ac)	CN	Desc	cription		
	0.	959	60	Woo	ds, Fair, H	ISG B	
	0.	012	79	Woo	ds, Fair, H	ISG D	
	0.	395	58	Mea	dow, non-g	grazed, HS	G B
*	3.	239	65	Mea	dow, non-	grazed, HS	G B/C
*	0.	034	98	Unco	onnected r	oofs, HSG	B/C
	0.	112	96	Grav	el surface	, HSG B	
	4.	751	64	Weig	ghted Aver	age	
	4.	717		99.2	8% Pervio	us Area	
	0.	034		0.72	% Impervi	ous Area	
	0.	034		100.0	00% Únco	nnected	
	Tc	Length	າ ຮ	Slope	Velocity	Capacity	Description
_	(min)	(feet)	)	(ft/ft)	(ft/sec)	(cfs)	
	10.2	100	0.	1100	0.16		Sheet Flow, A-B
							Woods: Light underbrush n= 0.400 P2= 3.62"
	0.2	33	B 0.	3000	2.74		Shallow Concentrated Flow, B-C
							Woodland Kv= 5.0 fps
	4.4	680	) 0.	1367	2.59		Shallow Concentrated Flow, C-D
_							Short Grass Pasture Kv= 7.0 fps
	14.8	813	3 To	otal			

## Subcatchment PDA-2B: PDA-2B



#### Summary for Subcatchment PDA-2C: PDA-2C

10.34 cfs @ 12.14 hrs, Volume= Runoff 0.834 af, Depth= 5.33" = Routed to Pond 2P : P-2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

	Area	(ac)	CN	Desc	cription					
	0.089 58 Meadow, non-grazed, HSG						GB			
*	0.	667	65	Mea	dow, non-	grazed, HS	G B/C			
	0.	930	78	Mea	Meadow, non-grazed, HSG D					
	0.	105	60	Woo	Woods, Fair, HSG B					
	0.	089	79	Woo	ds, Fair, H	ISG D				
	1.	880	71	Weig	hted Aver	age				
	1.880			100.00% Pervious Area						
	Тс	Length	1 8	Slope	Velocity	Capacity	Description			
	(min)	(feet)		(ft/ft)	(ft/sec)	(cfs)				
	5.1	100	0.	2200	0.32		Sheet Flow, A-B			
							Grass: Dense n= 0.240 P2= 3.62"			
	2.0	340	0.	1647	2.84		Shallow Concentrated Flow, B-C			
							Short Grass Pasture Kv= 7.0 fps			
	0.2	42	0.	0476	3.27		Shallow Concentrated Flow, C-D			
							Grassed Waterway Kv= 15.0 fps			
	7.3	482	2 To	otal						

#### Subcatchment PDA-2C: PDA-2C

Hydrograph



#### Summary for Subcatchment PDA-2D: PDA-2D

Runoff = 11.86 cfs @ 12.20 hrs, Volume= 1.138 af, Depth= 5.08" Routed to Pond 3P : P-3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

	Area	(ac) (	CN De	scription			
*	1.	831	65 Me	adow, non-	grazed, HS	G B/C	
	0.	857	78 Me	adow, non-	grazed, HS	G D	
2.688 69 Weighted Average							
	2.	688	100	0.00% Perv	ious Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	9.3	100	0.0500	0.18		Sheet Flow, A-B	
	3.0	518	0.1654	2.85		Grass: Dense n= 0.240 P2= 3.62" <b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps	
	12.3	618	Total				

#### Subcatchment PDA-2D: PDA-2D



## Summary for Subcatchment PDA-3A: PDA-3A

Runoff = 120.84 cfs @ 12.29 hrs, Volume= Routed to Link AP-2 : AP-2 14.195 af, Depth= 4.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

	Area	(ac)	CN	Desc	cription					
	0.	515	36	Woo	ds, Fair, H	ISG A				
	12.	604	60	Woo	ds, Fair, H	ISG B				
	10.	470	73	Woo	ds, Fair, H	ISG C				
	5.	295	79	Woo	ds, Fair, H	ISG D				
*	2.	024	75	Mea	Meadow, non-grazed, HSG C/D					
	0.	034	98	Unco	Unconnected pavement, HSG D					
	0.	397	96	Grav	el surface	, HSG C				
	1.	195	58	Mea	dow, non-	grazed, HS	GB			
	0.	931	71	Mea	dow, non-	grazed, HS	GC			
÷	0.	448	78	Mea	dow, non- <u>(</u>	grazed, HS	GD			
_	0.	440	65	Mea	dow, non-	grazed, HS	G B/C			
	34.353 68 Weighted Average									
	34.	319		99.9	0% Pervio	us Area				
	0.034			0.10	% Impervi	ous Area				
	0.	034		100.0	00% Unco	nnected				
	То	Longth	- (	Slope	Volocity	Conocity	Description			
	(min)	(foot	۱ ، ۱			Capacity (cfs)	Description			
_		100	)	(1011)		(015)	Check Flow, A.D.			
	8.8	100	J U	. 1600	0.19		Sheet Flow, A-B Weeder Light underbruch n= 0.400 D2= 2.62"			
	0 1	721	- 0	0012	1 5 1		Shallow Concentrated Flow B C			
	0.1	730	5 0	.0912	1.01		Woodland Ky= 5.0 fps			
	27	1 683	a 0	0552	10 50	346 60	Channel Flow C-D			
	2.1	1,000	5 0	.0002	10.50	540.00	$\Delta r_{eq} = 33.0 \text{ sf } Perim = 25.0' r = 1.32'$			
							n=0.040 Winding stream pools & shoals			
_	10.0	0.540	<u> </u>	atal						

19.6 2,518 Total

## Subcatchment PDA-3A: PDA-3A



## Summary for Subcatchment PDA-3B: PDA-3B

[47] Hint: Peak is 162% of capacity of segment #4

Runoff = 17.02 cfs @ 12.25 hrs, Volume= Routed to Pond 5P : P-4 1.831 af, Depth= 4.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

	Area	(ac)	CN	Desc	cription						
*	4.	728	65	Mea	dow, non-g	grazed, HS	G B/C				
	0.	024	96	Grav	el surface	, HSG B					
	0.	034	98	Unco	onnected p	pavement, ł	HSG B				
	4.	786	65	Weig	Weighted Average						
	4.	752		99.2	9% Pervio	us Area					
	0.	034		0.71	% Impervi	ous Area					
	0.	034		100.	00% Ünco	nnected					
	Тс	Lengt	h	Slope	Velocity	Capacity	Description				
_	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)					
	10.6	10	0 0	0.1000	0.16		Sheet Flow, A-B				
							Woods: Light underbrush n= 0.400 P2= 3.62"				
	5.0	54	7 C	).1334	1.83		Shallow Concentrated Flow, B-C				
							Woodland Kv= 5.0 fps				
	0.4	73	2 0	0.0444	3.16		Shallow Concentrated Flow, C-D				
							Grassed Waterway Kv= 15.0 fps				
	0.2	5	50	0.0100	5.94	10.50	Pipe Channel, D-E				
							18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'				
							n= 0.013 Corrugated PE, smooth interior				
	16.2	77	4 T	otal							

## Subcatchment PDA-3B: PDA-3B



#### Summary for Subcatchment PDA-3C: PDA-3C

Runoff = 25.99 cfs @ 12.23 hrs, Volume= 2.724 af, Depth> 5.81" Routed to Pond 4P : P-5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

	Area	(ac) (	CN Des	cription		
	1.	062	73 Wo	ods, Fair, F	ISG C	
	0.	126	71 Mea	adow, non-	grazed, HS	GC
*	4.	434	75 Mea	adow, non-	grazed, HS	G C/D
	5.	622	75 Wei	ghted Aver	age	
	5.	622	100	.00% Pervi	ious Area	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.5	100	0.1300	0.17		Sheet Flow, A-B
						Woods: Light underbrush n= 0.400 P2= 3.62"
	2.0	249	0.1687	2.05		Shallow Concentrated Flow, B-C
						Woodland Kv= 5.0 fps
	3.2	463	0.1210	2.43		Shallow Concentrated Flow, C-D
_						Short Grass Pasture Kv= 7.0 fps
	14.7	812	Total			

#### Subcatchment PDA-3C: PDA-3C



# Summary for Subcatchment PDA-4: PDA-4

[47] Hint: Peak is 114% of capacity of segment #4

Runoff = 19.17 cfs @ 12.24 hrs, Volume= Routed to Link AP-2 : AP-2

2.064 af, Depth> 5.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs NRCC 24-hr D 100-yr Rainfall=8.85"

_	Area	(ac) (	CN	Desc	ription		
	1.	773	73	Woo	ds, Fair, H	ISG C	
	0.	005	79	Woo	ds, Fair, H	ISG D	
	0.	220	98	Pave	ed roads w	/curbs & se	ewers, HSG C
	0.	018	98	Pave	ed roads w	/curbs & se	ewers, HSG D
*	1.	408	75	Mea	dow, non-g	grazed, HS	G C/D
	0.	819	71	Mea	dow, non-g	grazed, HS	GC
	0.	010	96	Grav	el surface	, HSG C	
	0.	003	96	Grav	el surface	, HSG D	
	0.	003	78	Mea	dow, non-g	grazed, HS	G D
	4.	259	75	Weig	hted Aver	age	
	4.	021		94.4	1% Pervio	us Area	
	0.238 5.59% Impervious Area						
	_		_				<b>—</b> • • •
	IC	Length		slope	Velocity	Capacity	Description
_	(min)	(feet		(ft/ft)	(ft/sec)	(cfs)	
	12.2	100	0.0	0700	0.14		Sheet Flow, A-B
							Woods: Light underbrush n= 0.400 P2= 3.62"
	2.3	301	0.	1827	2.14		Shallow Concentrated Flow, B-C
				0005	5.40		Woodland Kv= 5.0 fps
	0.2	63	0.0	0635	5.12		Shallow Concentrated Flow, C-D
		0.05		0575	40.07	40.70	Paved Kv= 20.3 fps
	1.1	885	0.0	0575	13.67	16.78	Pipe Channel, D-E
							15.0° Round Area= 1.2 st Perim= 3.9° r= 0.31°
_	45.0						n= 0.012 Corrugated PP, smooth Interior
	15.8	1,349	) Ic	otal			

## Subcatchment PDA-4: PDA-4



## Summary for Pond 1P: P=1

Inflow Area	a =	4.751 ac,	0.72% Impervious,	Inflow Depth =	4.47" for 1	00-yr event
Inflow	=	17.07 cfs @	12.23 hrs, Volume	= 1.770 a	af	-
Outflow	=	15.49 cfs @	12.31 hrs, Volume	= 1.694 a	af, Atten= 9%	%, Lag= 4.5 min
Primary	=	15.49 cfs @	12.31 hrs, Volume	= 1.694 a	af	
Routed	to Link	AP-1 : AP-1				

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 429.19' @ 12.31 hrs Surf.Area= 5,449 sf Storage= 13,357 cf

Plug-Flow detention time= 61.5 min calculated for 1.692 af (96% of inflow) Center-of-Mass det. time= 38.7 min (904.4 - 865.7)

Volume	Invert	Avail.Sto	orage	Storage Description	1		
#1	426.00'	18,0	67 cf	Custom Stage Dat	<b>a (Irregular)</b> Listed	below (Recalc)	
Elevatic (fee	on Su et)	rf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
426.0 427.0 428.0 429.0 430.0	)0 )0 )0 )0 )0	3,028 3,726 4,480 5,291 6,158	223.1 242.0 260.8 279.7 298.5	0 3,371 4,097 4,880 5,719	0 3,371 7,468 12,348 18,067	3,028 3,765 4,558 5,415 6,327	
Device	Routing	Invert	Outle	et Devices			
#1	Primary	426.00'	<b>18.0</b> L= 6 Inlet n= 0	" Round Culvert 5.0' CMP, projecting / Outlet Invert= 426. .013 Corrugated PE	g, no headwall, Ke 00' / 425.00' S= 0 . smooth interior. F	= 0.900 .0154 '/'    Cc= 0.900 Flow Area= 1.77 sf	
#2	Device 1	427.00'	15.0	" Vert. Orifice/Grate	C= 0.600 Limite	ed to weir flow at low	heads
#3	Device 1	429.00'	<b>36.0</b> Limit	" x 21.0" Horiz. Orif ted to weir flow at lov	<b>ice/Grate</b> C= 0.60 v heads	00	
#4	Primary	429.00'	<b>25.0</b> Head Coet	<b>' long + 3.0 '/' Sidea</b> d (feet) 0.20 0.40 0 f. (English) 2.68 2.7	<b>Z x 15.0' breadth</b> .60 0.80 1.00 1.2 0 2.70 2.64 2.63	Broad-Crested Rec 0 1.40 1.60 2.64 2.64 2.63	tangular Weir

**Primary OutFlow** Max=15.15 cfs @ 12.31 hrs HW=429.18' (Free Discharge)

**-1=Culvert** (Passes 9.81 cfs of 10.48 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 7.38 cfs @ 6.01 fps)

-3=Orifice/Grate (Weir Controls 2.43 cfs @ 1.40 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 5.34 cfs @ 1.14 fps)

## Pond 1P: P=1



## Summary for Pond 2P: P-2

Inflow Area	a =	1.880 ac,	0.00% Impervious, Infl	ow Depth = $5.33$ "	for 100-yr event
Inflow	=	10.34 cfs @	12.14 hrs, Volume=	0.834 af	-
Outflow	=	9.96 cfs @	12.17 hrs, Volume=	0.755 af, Att	en= 4%, Lag= 1.6 min
Primary	=	9.96 cfs @	12.17 hrs, Volume=	0.755 af	
Routed	to Link	AP-1 : AP-1			

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs / 2 Peak Elev= 419.18' @ 12.17 hrs Surf.Area= 3,415 sf Storage= 7,717 cf

Plug-Flow detention time= 101.5 min calculated for 0.754 af (90% of inflow) Center-of-Mass det. time= 52.2 min ( 892.6 - 840.4 )

Volume	Invert	Avail.Sto	orage	Storage Description			
#1	416.00'	10,7	57 cf	Custom Stage Data	a (Irregular)Listed	below (Recalc)	
Elevatic (fee	on Su t)	ırf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
416.0 417.0 418.0 419.0 420.0	)0 )0 )0 )0 )0	1,538 2,067 2,653 3,295 3,993	166.9 185.7 204.6 223.4 242.3	0 1,796 2,354 2,968 3,638	0 1,796 4,150 7,118 10,757	1,538 2,094 2,713 3,388 4,126	
Device	Routing	Invert	Outle	et Devices			
#1	Primary	416.00'	<b>18.0</b> L= 4 Inlet n= 0	" Round Culvert 0.0' CMP, projecting / Outlet Invert= 416.0 .013 Corrugated PE.	, no headwall, Ke )0' / 414.00' S= 0 smooth interior. F	= 0.900 .0500 '/'    Cc= 0.900 Flow Area= 1.77 sf	
#2 #3	Device 1 Device 1	417.70' 419.00'	12.0 36.0	" Vert. Orifice/Grate " x 21.0" Horiz. Orifi	C= 0.600 Limite ce/Grate C= 0.60	ed to weir flow at low h 0	eads
#4	Primary	419.00'	LIMIT 20.0 Head Coef	ed to weir flow at low ' <b>long + 3.0 '/' SideZ</b> d (feet) 0.20 0.40 0. f. (English) 2.68 2.70	<b>x 15.0' breadth</b> 60 0.80 1.00 1.2 2.70 2.64 2.63	Broad-Crested Recta 0 1.40 1.60 2.64 2.64 2.63	ngular Weir

**Primary OutFlow** Max=9.19 cfs @ 12.17 hrs HW=419.16' (Free Discharge)

**-1=Culvert** (Passes 5.69 cfs of 10.43 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 3.71 cfs @ 4.72 fps)

-3=Orifice/Grate (Weir Controls 1.99 cfs @ 1.31 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 3.50 cfs @ 1.07 fps)

# Pond 2P: P-2



## Summary for Pond 3P: P-3

Inflow Are	a =	2.688 ac,	0.00% Impervious,	Inflow Depth = 5.	.08" for 100-yr event
Inflow	=	11.86 cfs @	12.20 hrs, Volume	= 1.138 af	-
Outflow	=	6.73 cfs @	12.36 hrs, Volume	= 1.069 af,	Atten= 43%, Lag= 9.5 min
Primary	=	6.73 cfs @	12.36 hrs, Volume	= 1.069 af	-
Routed	to Link	AP-1 : AP-1			

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 458.92' @ 12.36 hrs Surf.Area= 4,762 sf Storage= 10,761 cf

Plug-Flow detention time= 79.5 min calculated for 1.068 af (94% of inflow) Center-of-Mass det. time= 46.9 min (897.1 - 850.3)

Volume	Invert	Avail.Sto	rage	Storage Description			
#1	456.00'	16,3	75 cf	Custom Stage Data	<b>a (Irregular)</b> Listed	below (Recalc)	
Elevatio (fee	n Su t)	rf.Area F (sq-ft)	erim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
456.0 457.0 458.0 459.0 460.0	)0  0  0  0  0	2,689 3,345 4,057 4,825 5,652	209.2 228.1 246.9 265.8 284.6	0 3,011 3,695 4,435 5,233	0 3,011 6,706 11,142 16,375	2,689 3,382 4,131 4,944 5,812	
Device	Routing	Invert	Outle	et Devices			
#1	Primary	456.00'	<b>18.0</b> L= 6 Inlet n= 0	" Round Culvert 0.0' CMP, projecting / Outlet Invert= 456.0 .013 Corrugated PE.	, no headwall, Ke )0' / 445.00' S= 0 smooth interior. F	= 0.900 .1833 '/'    Cc= 0.900 Flow Area= 1.77 sf	
#2	Device 1	457.00'	15.0	" Vert. Orifice/Grate	C= 0.600 Limite	ed to weir flow at low	heads
#3	Device 1	459.00'	<b>36.0</b> Limit	" x 21.0" Horiz. Orifi ed to weir flow at low	ce/Grate C= 0.60 heads	00	
#4	Primary	459.00'	<b>16.0</b> Head Coef	<b>' long + 3.0 '/' SideZ</b> d (feet) 0.20 0.40 0. f. (English) 2.68 2.70	<b>x 15.0' breadth</b> 60 0.80 1.00 1.2 2.70 2.64 2.63	Broad-Crested Rect 0 1.40 1.60 2.64 2.64 2.63	angular Weir

**Primary OutFlow** Max=6.71 cfs @ 12.36 hrs HW=458.92' (Free Discharge)

**-1=Culvert** (Passes 6.71 cfs of 9.89 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 6.71 cfs @ 5.47 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: P-3



## Summary for Pond 4P: P-5

Inflow Area	a =	5.622 ac,	0.00% Impervious	, Inflow Depth >	5.81" fo	or 100-yr event
Inflow	=	25.99 cfs @	12.23 hrs, Volum	e= 2.724	af	•
Outflow	=	21.76 cfs @	12.32 hrs, Volum	e= 2.580	af, Atten=	= 16%, Lag= 5.5 min
Primary	=	21.76 cfs @	12.32 hrs, Volum	e= 2.580	af	-
Routed	to I ink	AP-2 · AP-2				

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 481.35' @ 12.32 hrs Surf.Area= 10,716 sf Storage= 26,934 cf

Plug-Flow detention time= 91.0 min calculated for 2.577 af (95% of inflow) Center-of-Mass det. time= 61.9 min (898.4 - 836.5)

Volume	Inve	rt Avai	I.Storage	Storage Descript	ion	
#1	478.00	כי 'כ	34,305 cf	Custom Stage D	<b>)ata (Irregular)</b> Lisi	ted below (Recalc)
Elevatio	on s	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
(tee	et)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	<u>(sq-ft)</u>
478.0	00	5,499	489.1	0	0	5,499
479.0	00	6,994	507.9	6,232	6,232	7,072
480.0	00	8,546	526.8	7,757	13,989	8,712
481.0	00	10,155	545.6	9,339	23,328	10,403
482.0	00	11,820	564.5	10,977	34,305	12,163
Device	Routing	In	vert Outle	et Devices		
#1	Primary	478	.00' <b>18.0</b> L= 5 Inlet n= 0	" Round Culvert 0.0' CMP, projec / Outlet Invert= 47 .013 Corrugated I	ting, no headwall, 78.00' / 476.00' S PE, smooth interio	Ke= 0.900 = 0.0400 '/'    Cc= 0.900 r,  Flow Area= 1.77 sf
#2	Device 1	479	.00' <b>15.0</b>	" Vert. Orifice/Gr	ate C= 0.600 Li	mited to weir flow at low heads
#3	Device 1	481	.00' <b>36.0</b> Limit	" x 21.0" Horiz. C ted to weir flow at	<b>Prifice/Grate</b> C= ( low heads	0.600
#4	Primary	481	.00' <b>20.0</b> Head Coet	<b>20.0' long x 15.0' breadth Broad-Cre</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.68 2.70 2.70 2.64		ted Rectangular Weir 1.20 1.40 1.60 63 2.64 2.64 2.63

**Primary OutFlow** Max=21.29 cfs @ 12.32 hrs HW=481.34' (Free Discharge)

-1=Culvert (Inlet Controls 10.80 cfs @ 6.11 fps)

**2=Orifice/Grate** (Passes < 7.73 cfs potential flow)

-3=Orifice/Grate (Passes < 6.05 cfs potential flow)

-4=Broad-Crested Rectangular Weir (Weir Controls 10.48 cfs @ 1.56 fps)
#### Pond 4P: P-5



## Summary for Pond 5P: P-4

Inflow Area	a =	4.786 ac,	0.71% Impervious,	Inflow Depth =	4.59" fo	or 100-yr event
Inflow	=	17.02 cfs @	12.25 hrs, Volume	= 1.831 a	af	•
Outflow	=	13.44 cfs @	12.37 hrs, Volume	= 1.732 a	af, Atten=	= 21%, Lag= 7.3 min
Primary	=	13.44 cfs @	12.37 hrs, Volume	= 1.732 a	af	-
Routed	to Link	AP-2 : AP-2				

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 463.19' @ 12.37 hrs Surf.Area= 7,122 sf Storage= 17,393 cf

Plug-Flow detention time= 78.8 min calculated for 1.730 af (94% of inflow) Center-of-Mass det. time= 49.9 min (914.3 - 864.4)

Volume	Inver	t Avail.S	torage	Storage Description	on	
#1	460.00	' 23,	498 cf	Custom Stage Da	<b>ata (Irregular)</b> Liste	d below (Recalc)
Elevatio	on S	urf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area
460.0 461.0 462.0 463.0	20 20 20 20 20	3,869 4,826 5,840 6,910	309.7 328.5 347.4 366.2	0 4,339 5,325 6,368	0 4,339 9,664 16,031	<u> </u>
464.0 Device	Routing	8,037 Inver	385.1 t Outle	7,466 et Devices	23,498	8,263
#1	Primary	460.00	0' <b>18.0</b> L= 5 Inlet n= 0	<b>" Round Culvert</b> 0.0' CMP, projecti / Outlet Invert= 460 .013 Corrugated P	ng, no headwall,  k ).00' / 458.00'   S= E, smooth interior,	Ke= 0.900 0.0400 '/' Cc= 0.900 Flow Area= 1.77 sf
#2 #3	Device 1 Device 1	461.00 463.00	)' <b>15.0</b> )' <b>36.0</b> Limit	" Vert. Orifice/Gra " x 21.0" Horiz. Or ted to weir flow at lo	te C= 0.600 Lim ifice/Grate C= 0. ow heads	ited to weir flow at low heads 600
#4	Primary	463.00	)' <b>15.0</b> Head Coet	<b>' long x 15.0' brea</b> d (feet) 0.20 0.40 f. (English) 2.68 2.	Oth Broad-Crester           0.60         0.80         1.00         1           70         2.70         2.64         2.6	ed Rectangular Weir .20 1.40 1.60 3 2.64 2.64 2.63

**Primary OutFlow** Max=12.89 cfs @ 12.37 hrs HW=463.18' (Free Discharge)

**-1=Culvert** (Passes 9.78 cfs of 10.48 cfs potential flow)

**2=Orifice/Grate** (Orifice Controls 7.37 cfs @ 6.01 fps)

-3=Orifice/Grate (Weir Controls 2.41 cfs @ 1.39 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 3.11 cfs @ 1.14 fps)

## Pond 5P: P-4



## Summary for Link AP-1: AP-1

Inflow /	Area	=	35.925 ac,	0.09% Impervious,	Inflow Depth = $5.^{\circ}$	11" for 100-yr event
Inflow	=	=	133.77 cfs @	12.26 hrs, Volume	= 15.293 af	
Primary	y =	=	133.77 cfs @	12.26 hrs, Volume	= 15.293 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs



#### Link AP-1: AP-1

## Summary for Link AP-2: AP-2

Inflow /	Area	=	49.020 ac,	0.62% Impe	ervious,	Inflow	Depth >	5.0	4" for	100-	yr ever	nt
Inflow		=	168.78 cfs @	12.30 hrs,	Volume	=	20.570	af				
Primar	у	=	168.78 cfs @	12.30 hrs,	Volume	=	20.570	af,	Atten= 0	%, L	_ag= 0.	0 min

Primary outflow = Inflow, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs



## Link AP-2: AP-2

		WATER QUALITY VOLU
Project:	Proposed Solar Photovoltaic A	urray
Location:	0 Riggs Street, Oxford, CT	
Date:	12/01/23	
Water Qual	ity Volume Calculations:	
WQV =	(1.3")(R)(A)	Where:
	12	WQV = water quality volume (ac-ft)
		R = volumentric runoff coefficient = 0.05+0.009(I)
		I = percent impervious cover (see below)
		A = site area in acres
	<b>A</b>	Where:
=	ATOT X 100	l = percent impervious cover
	. 101	$A_{mp}$ = area of impervious cover
		$A_{\text{Tot}} = \text{total area of watershed}$
Watershed [	Description:	PDA
Area of impe	ervious coverage, A <sub>IMP</sub>	0.50 Acres
Total area o	f watershed, A <sub>TOT</sub>	4.75 Acres
Percent imp	verious cover, I	10.53 %
Volumentric	runoff coefficient, R	0.14
Water Quali	ty Volume, WQV	0.074 ac-ft 3,244 cf

#### WATER QUALITY VOLUME (WQV) COMPUTATIONS FOR PDA-2B

		WATER QUALITY VOLU
Project:	Proposed Solar Photovoltaic A	rray
Location:	0 Riggs Street, Oxford, CT	
Date:	12/01/23	
Water Quali	ty Volume Calculations:	
WQV =	(1.3")(R)(A)	Where:
	12	WQV = water quality volume (ac-ft)
		R = volumentric runoff coefficient = 0.05+0.009(I)
		I = percent impervious cover (see below)
		A = site area in acres
	٨	10/1
=	AIMP X 100	
	ATOT	I = percent impervious cover
		A <sub>IMP</sub> – area of impervious cover
		A <sub>TOT</sub> - total area of watershed
Watershed [	Description:	PDA
Area of impe	ervious coverage, A <sub>IMP</sub>	0.63 Acres
Total area of	f watershed, A <sub>TOT</sub>	1.88 Acres
Percent imp	verious cover. I	33.51 %
Volumentric	runoff coefficient, R	0.35
Water Qualit	y Volume, WQV	0.072 ac-ft 3,119 cf

#### ITY VOLUME (WQV) COMPUTATIONS FOR PDA-2C

				WATER QUALI	TY VOLU
Project:	Proposed Solar Photovoltaic A	rray			
Location:	0 Riggs Street, Oxford, CT				
Date:	12/01/23				
Water Quali	ty Volume Calculations:				
WOV =	(1.3")(R)(A)	Where:			
	12	WQV = water	quality volu	ıme (ac-ft)	
		R = volumentr	ic runoff co	efficient = 0.05+	0.009(I)
		I = percent imp	pervious co	ver (see below)	
		A = site area i	n acres		
	٨	Whore:			
=	X 100	Villere.		vor	
	ATOT	A = area of	impervious co		
			on of water	cover	
		A101 - total all	ea or water	Siled	
Watershed D	Description:		PDA		
Area of impe	ervious coverage, A <sub>IMP</sub>	0.23	Acres		
Total area of	watershed, A <sub>TOT</sub>	2.69	Acres		
		·			
Percent imp	verious cover, I	8.55	%		
Volumentric	runoff coefficient, R	0.13			
Water Qualit	y Volume, WQV	0.037	ac-ft	1,612	cf

#### WATER QUALITY VOLUME (WQV) COMPUTATIONS FOR PDA-2E

			1	NATER QUALI	LA AOTI
Project:	Proposed Solar Photovoltaic A	rray			
Location:	0 Riggs Street, Oxford, CT				
Date:	12/01/23				
Water Quali	ty Volume Calculations:				
WQV =	(1.3")(R)(A)	Where:			
	12	WQV = water	quality volu	me (ac-ft)	
		R = volumentr	ic runoff co	efficient = 0.05+	0.009(I)
		I = percent imp	pervious co	ver (see below)	
		A = site area ii	n acres		
	٨	10/1			
=	X 100	Villele.			
	ATOT	A = area of	imponious co	ver	
		A <sub>IMP</sub> – alea or	an of wotor	cover	
		ATOT - IOIAI AI	ea ur water	sneu	
Watershed E	Description:		PDA		
Area of impe	ervious coverage, A <sub>IMP</sub>	0.06	Acres		
Total area of	watershed Anna	1 79	Acres		
	Waterbried, 7101	<del>4</del> .75	AGIES		
Percent imp	verious cover, I	1.21	%		
Volumentric	runoff coefficient, R	0.06			
Water Qualit	y Volume, WQV	0.032	ac-ft	1,376	cf

#### WATER QUALITY VOLUME (WQV) COMPUTATIONS FOR PDA-3B

			v	VATER QUALI	TY VOLU
Project:	Proposed Solar Photovoltaic A	rray			
Location:	0 Riggs Street, Oxford, CT				
Date:	12/01/23				
Water Quali	ty Volume Calculations:				
WQV =	(1.3")(R)(A)	Where:			
	12	WQV = water qu	uality volur	me (ac-ft)	
		R = volumentric	runoff coe	efficient = 0.05+	0.009(I)
		I = percent impe	ervious cov	ver (see below)	
		A = site area in a	acres		
	٨	\A/h			
=	X 100	Villele.		107	
	ATOT	A = area of in	nonvious cov		
		A <sub>IMP</sub> – area or in		bod	
		Atot – total alea	a or waters	sileu	
Watershed E	Description:		PDA		
Area of impe	ervious coverage, A <sub>IMP</sub>	0.97	Acres		
Total area of	watershed, A <sub>TOT</sub>	5.41	Acres		
Percent imp	verious cover, I	17.91	%		
Volumentric	runoff coefficient, R	0.21			
Water Qualit	y Volume, WQV	0.124 a	ac-ft	5,392	cf

#### LITY VOLUME (WQV) COMPUTATIONS FOR PDA-3B

Precipitation Frequency Data Server



NOAA Atlas 14, Volume 10, Version 3 Location name: Oxford, Connecticut, USA\* Latitude: 41.4438°, Longitude: -73.1205° Elevation: 536 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF\_tabular | PF\_graphical | Maps\_&\_aerials

#### **PF** tabular

PDS-k	based poi	nt precipit	tation free	luency es	timates v	vith 90%	confiden	ce interv	als (in in	ches) <sup>1</sup>
Duration				Average	recurrence	interval (y	ears)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.360</b> (0.277-0.463)	<b>0.427</b> (0.328-0.549)	<b>0.536</b> (0.410-0.691)	<b>0.627</b> (0.477-0.813)	<b>0.751</b> (0.554-1.01)	<b>0.845</b> (0.612-1.16)	<b>0.943</b> (0.664-1.34)	<b>1.05</b> (0.705-1.53)	<b>1.21</b> (0.781-1.81)	<b>1.33</b> (0.843-2.03)
10-min	<b>0.511</b> (0.392-0.656)	<b>0.605</b> (0.464-0.778)	<b>0.759</b> (0.581-0.979)	<b>0.888</b> (0.675-1.15)	<b>1.06</b> (0.785-1.43)	<b>1.20</b> (0.867-1.64)	<b>1.34</b> (0.941-1.90)	<b>1.49</b> (0.999-2.16)	<b>1.71</b> (1.10-2.56)	<b>1.89</b> (1.19-2.88)
15-min	<b>0.601</b> (0.461-0.771)	<b>0.712</b> (0.546-0.915)	<b>0.893</b> (0.683-1.15)	<b>1.04</b> (0.794-1.35)	<b>1.25</b> (0.924-1.69)	<b>1.41</b> (1.02-1.93)	<b>1.57</b> (1.11-2.23)	<b>1.75</b> (1.18-2.54)	<b>2.01</b> (1.30-3.01)	<b>2.22</b> (1.40-3.39)
30-min	<b>0.829</b> (0.637-1.06)	<b>0.982</b> (0.753-1.26)	<b>1.23</b> (0.942-1.59)	<b>1.44</b> (1.09-1.86)	<b>1.72</b> (1.27-2.32)	<b>1.94</b> (1.40-2.66)	<b>2.16</b> (1.52-3.06)	<b>2.41</b> (1.61-3.49)	<b>2.75</b> (1.78-4.12)	<b>3.02</b> (1.91-4.61)
60-min	<b>1.06</b> (0.813-1.36)	<b>1.25</b> (0.961-1.61)	<b>1.57</b> (1.20-2.02)	<b>1.83</b> (1.39-2.38)	<b>2.20</b> (1.62-2.96)	<b>2.47</b> (1.79-3.39)	<b>2.75</b> (1.94-3.90)	<b>3.06</b> (2.05-4.44)	<b>3.48</b> (2.26-5.22)	<b>3.82</b> (2.42-5.84)
2-hr	<b>1.39</b> (1.07-1.77)	<b>1.63</b> (1.26-2.08)	<b>2.03</b> (1.56-2.60)	<b>2.36</b> (1.81-3.05)	<b>2.82</b> (2.10-3.78)	<b>3.17</b> (2.31-4.33)	<b>3.53</b> (2.50-4.99)	<b>3.94</b> (2.65-5.68)	<b>4.52</b> (2.94-6.73)	<b>5.00</b> (3.17-7.58)
3-hr	<b>1.61</b> (1.25-2.04)	<b>1.89</b> (1.47-2.41)	<b>2.36</b> (1.82-3.01)	<b>2.75</b> (2.11-3.53)	<b>3.28</b> (2.45-4.39)	<b>3.68</b> (2.70-5.02)	<b>4.11</b> (2.93-5.80)	<b>4.59</b> (3.10-6.60)	<b>5.30</b> (3.45-7.86)	<b>5.89</b> (3.74-8.90)
6-hr	<b>2.03</b> (1.59-2.56)	<b>2.41</b> (1.88-3.05)	<b>3.04</b> (2.36-3.85)	<b>3.56</b> (2.75-4.53)	<b>4.27</b> (3.21-5.68)	<b>4.80</b> (3.54-6.52)	<b>5.37</b> (3.86-7.57)	<b>6.04</b> (4.09-8.63)	<b>7.03</b> (4.59-10.4)	<b>7.87</b> (5.02-11.8)
12-hr	<b>2.50</b> (1.96-3.13)	<b>3.02</b> (2.37-3.79)	<b>3.86</b> (3.02-4.87)	<b>4.57</b> (3.56-5.78)	<b>5.54</b> (4.18-7.32)	<b>6.25</b> (4.64-8.45)	<b>7.03</b> (5.08-9.86)	<b>7.95</b> (5.40-11.3)	<b>9.33</b> (6.11-13.7)	<b>10.5</b> (6.72-15.7)
24-hr	<b>2.94</b> (2.33-3.66)	<b>3.62</b> (2.86-4.51)	<b>4.72</b> (3.72-5.91)	<b>5.64</b> (4.42-7.09)	6.90 (5.25-9.09)	<b>7.83</b> (5.86-10.6)	8.85 (6.46-12.4)	<b>10.1</b> (6.88-14.2)	<b>12.0</b> (7.88-17.5)	<b>13.6</b> (8.76-20.2)
2-day	<b>3.34</b> (2.66-4.13)	<b>4.17</b> (3.32-5.16)	<b>5.53</b> (4.39-6.87)	<b>6.66</b> (5.25-8.32)	<b>8.22</b> (6.31-10.8)	<b>9.36</b> (7.06-12.6)	<b>10.6</b> (7.83-14.9)	<b>12.2</b> (8.36-17.1)	<b>14.8</b> (9.71-21.3)	<b>17.0</b> (10.9-25.0)
3-day	<b>3.63</b> (2.90-4.48)	<b>4.55</b> (3.64-5.61)	<b>6.05</b> (4.82-7.49)	<b>7.30</b> (5.78-9.08)	<b>9.01</b> (6.94-11.8)	<b>10.3</b> (7.77-13.8)	<b>11.7</b> (8.64-16.3)	<b>13.4</b> (9.21-18.8)	<b>16.3</b> (10.7-23.5)	<b>18.8</b> (12.1-27.5)
4-day	<b>3.90</b> (3.13-4.80)	<b>4.88</b> (3.91-6.00)	<b>6.47</b> (5.17-7.99)	<b>7.80</b> (6.19-9.67)	<b>9.62</b> (7.42-12.5)	<b>10.9</b> (8.30-14.6)	<b>12.4</b> (9.22-17.3)	<b>14.3</b> (9.83-19.9)	<b>17.3</b> (11.4-24.9)	<b>19.9</b> (12.9-29.2)
7-day	<b>4.67</b> (3.77-5.71)	<b>5.75</b> (4.63-7.03)	<b>7.51</b> (6.03-9.22)	<b>8.98</b> (7.16-11.1)	<b>11.0</b> (8.51-14.2)	<b>12.5</b> (9.48-16.5)	<b>14.1</b> (10.5-19.5)	<b>16.1</b> (11.1-22.3)	<b>19.3</b> (12.8-27.5)	<b>22.0</b> (14.2-32.0)
10-day	<b>5.43</b> (4.39-6.61)	<b>6.56</b> (5.31-8.00)	<b>8.42</b> (6.79-10.3)	<b>9.96</b> (7.98-12.2)	<b>12.1</b> (9.38-15.5)	<b>13.7</b> (10.4-17.9)	<b>15.4</b> (11.4-21.0)	<b>17.4</b> (12.0-24.0)	<b>20.5</b> (13.6-29.2)	<b>23.2</b> (15.1-33.7)
20-day	<b>7.76</b> (6.32-9.39)	<b>8.98</b> (7.31-10.9)	<b>11.0</b> (8.91-13.3)	<b>12.6</b> (10.2-15.4)	<b>14.9</b> (11.6-18.9)	<b>16.6</b> (12.7-21.5)	<b>18.4</b> (13.6-24.7)	<b>20.4</b> (14.2-28.0)	<b>23.3</b> (15.6-32.9)	<b>25.7</b> (16.7-36.9)
30-day	<b>9.69</b> (7.93-11.7)	<b>11.0</b> (8.96-13.2)	<b>13.0</b> (10.6-15.8)	<b>14.7</b> (11.9-17.9)	<b>17.1</b> (13.3-21.6)	<b>18.9</b> (14.4-24.3)	<b>20.8</b> (15.2-27.5)	<b>22.7</b> (15.9-30.9)	<b>25.4</b> (17.0-35.7)	<b>27.5</b> (17.9-39.4)
45-day	<b>12.1</b> (9.92-14.5)	<b>13.4</b> (11.0-16.1)	<b>15.5</b> (12.7-18.7)	<b>17.3</b> (14.1-21.0)	<b>19.8</b> (15.5-24.7)	<b>21.7</b> (16.5-27.6)	<b>23.6</b> (17.3-30.9)	<b>25.4</b> (17.8-34.5)	<b>27.9</b> (18.8-39.1)	<b>29.8</b> (19.5-42.6)
60-day	<b>14.0</b> (11.6-16.8)	<b>15.4</b> (12.7-18.4)	<b>17.6</b> (14.4-21.1)	<b>19.4</b> (15.8-23.5)	<b>22.0</b> (17.2-27.4)	<b>23.9</b> (18.3-30.3)	<b>25.9</b> (19.0-33.7)	<b>27.7</b> (19.5-37.4)	<b>30.2</b> (20.3-42.0)	<b>31.9</b> (20.9-45.4)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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#### **PF graphical**







Duration						
5-min	2-day					
10-min	— 3-day					
- 15-min	— 4-day					
- 30-min	- 7-day					
- 60-min	— 10-day					
- 2-hr	— 20-day					
— 3-hr	— 30-day					
— 6-hr	— 45-day					
- 12-hr	- 60-day					
24-hr						

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Maps & aerials

Small scale terrain

Precipitation Frequency Data Server



Large scale terrain





Large scale aerial

Precipitation Frequency Data Server



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# Jacks Brook StreamStats Report



Collapse All

# Basin Characteristics

Parameter Code	Parameter Description	Value	Unit
DRNAREA	Area that drains to a point on a stream	1.75	square miles
I24H100Y	Maximum 24-hour precipitation that occurs on average once in 100 years	8.87	inches
I24H10Y	Maximum 24-hour precipitation that occurs on average once in 10 years	5.57	inches
I24H200Y	Maximum 24-hour precipitation that occurs on average once in 200 years	10.34	inches

Parameter Code	Parameter Description	Value	Unit
I24H25Y	Maximum 24-hour precipitation that occurs on average once in 25 years	6.89	inches
I24H2Y	Maximum 24-hour precipitation that occurs on average once in 2 years - Equivalent to precipitation intensity index	3.26	inches
I24H500Y	Maximum 24-hour precipitation that occurs on average once in 500 years	12.28	inches
I24H50Y	Maximum 24-hour precipitation that occurs on average once in 50 years	7.88	inches
I24H5Y	Maximum 24-hour precipitation that occurs on average once in 5 years	4.58	inches
SSURGOCCDD	Percentage of area with hydrologic soil types C, D, or C/D from SSURGO	0.4821	percent

# > Peak-Flow Statistics

## Peak-Flow Statistics Parameters [Statewide DA only SIR 2020 5054]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	1.75	square miles	0.69	325

Peak-Flow Statistics Parameters [Statewide Multiparameter SIR 2020 5054]

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
DRNAREA	Drainage Area	1.75	square miles	0.69	325
124H2Y	24 Hour 2 Year Precipitation	3.26	inches	2.77	3.32
SSURGOCCDD	Percent soil type C or D from SSURGO	0.4821	percent	0.118	0.945
I24H5Y	24 Hour 5 Year Precipitation	4.58	inches	4	4.7
I24H10Y	24 Hour 10 Year Precipitation	5.57	inches	4.86	5.79
I24H25Y	24 Hour 25 Year Precipitation	6.89	inches	5.99	7.22
124H50Y	24 Hour 50 Year Precipitation	7.88	inches	6.81	8.3

Parameter Code	Parameter Name	Value	Units	Min Limit	Max Limit
I24H100Y	24 Hour 100 Year Precipitation	8.87	inches	7.62	9.38
I24H200Y	24 Hour 200 YearPrecipitation	10.34	inches	8.7	11.22
I24H500Y	24 Hour 500 Year Precipitation	12.28	inches	10.1	13.64

## Peak-Flow Statistics Flow Report [Statewide DA only SIR 2020 5054]

PII: Prediction Interval-Lower, PIu: Prediction Interval-Upper, ASEp: Average Standard Error of Prediction, SE: Standard Error (other -- see report)

Statistic	Value	Unit	ASEp
Drainage Area Only 50-percent AEP flood	99.1	ft^3/s	35
Drainage Area Only 20-percent AEP flood	174	ft^3/s	35
Drainage Area Only 10-percent AEP flood	236	ft^3/s	36.3
Drainage Area Only 4-percent AEP flood	328	ft^3/s	37.8
Drainage Area Only 2-percent AEP flood	406	ft^3/s	39.8
Drainage Area Only 1-percent AEP flood	493	ft^3/s	42.4
Drainage Area Only 0.5-percent AEP flood	591	ft^3/s	44.4
Drainage Area Only 0.2-percent AEP flood	736	ft^3/s	48

## Peak-Flow Statistics Flow Report [Statewide Multiparameter SIR 2020 5054]

PII: Prediction Interval-Lower, PIu: Prediction Interval-Upper, ASEp: Average Standard Error of Prediction, SE: Standard Error (other -- see report)

Statistic	Value	Unit	PII	Plu	ASEp
50-percent AEP flood	117	ft^3/s	28.9	474	26.5
20-percent AEP flood	228	ft^3/s	51.2	1020	26.3
10-percent AEP flood	307	ft^3/s	63.5	1480	28.4
4-percent AEP flood	430	ft^3/s	80.3	2300	31.5
2-percent AEP flood	534	ft^3/s	90.7	3150	34.3
1-percent AEP flood	647	ft^3/s	99.8	4200	37.1
0.5-percent AEP flood	737	ft^3/s	128	4230	40.6
0.2-percent AEP flood	896	ft^3/s	166	4830	45

## Peak-Flow Statistics Flow Report [Area-Averaged]

PII: Prediction Interval-Lower, PIu: Prediction Interval-Upper, ASEp: Average Standard Error of Prediction, SE: Standard Error (other -- see report)

Statistic	Value	Unit	ASEp		
Drainage Area Only 50-percent AEP flood	99.1	ft^3/s	35	-	
Drainage Area Only 20-percent AEP flood	174	ft^3/s	35		
Drainage Area Only 10-percent AEP flood	236	ft^3/s	36.3		
Drainage Area Only 4-percent AEP flood	328	ft^3/s	37.8		
Drainage Area Only 2-percent AEP flood	406	ft^3/s	39.8		
Drainage Area Only 1-percent AEP flood	493	ft^3/s	42.4		
Drainage Area Only 0.5-percent AEP flood	591	ft^3/s	44.4		
Drainage Area Only 0.2-percent AEP flood	736	ft^3/s	48		
50-percent AEP flood	117	ft^3/s	28.9	474	26.5
20-percent AEP flood	228	ft^3/s	51.2	1020	26.3
10-percent AEP flood	307	ft^3/s	63.5	1480	28.4
4-percent AEP flood	430	ft^3/s	80.3	2300	31.5
2-percent AEP flood	534	ft^3/s	90.7	3150	34.3
1-percent AEP flood	647	ft^3/s	99.8	4200	37.1
0.5-percent AEP flood	737	ft^3/s	128	4230	40.6
0.2-percent AEP flood	896	ft^3/s	166	4830	45

Peak-Flow Statistics Citations

## Ahearn, E.A., and Hodgkins, G.A.,2020, Estimating flood magnitude and frequency on streams and rivers in Connecticut, based on data through water year 2015: U.S. Geological Survey Scientific Investigations Report 2020–5054, 42 p. (https://doi.org/10.3133/sir20205054)

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Application Version: 4.17.0 StreamStats Services Version: 1.2.22 NSS Services Version: 2.2.1

# HY-8 Culvert Analysis Report

#### **Crossing Discharge Data**

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 93.00 cfs

Design Flow: 493.00 cfs

Maximum Flow: 493.00 cfs

#### Table 1 - Summary of Culvert Flows at Crossing: Jacks Brook

Headwater Elevation (ft)	Total Discharge (cfs)	Contech ALBC Aluminum Box Culvert 20'-4" span by 4'6" rise Discharge (cfs)	Roadway Discharge (cfs)	Iterations
430.37	93.00	93.00	0.00	1
430.74	133.00	133.00	0.00	1
431.08	173.00	173.00	0.00	1
431.41	213.00	213.00	0.00	1
431.74	253.00	253.00	0.00	1
432.06	293.00	293.00	0.00	1
432.34	333.00	333.00	0.00	1
432.62	373.00	373.00	0.00	1
432.90	413.00	413.00	0.00	1
433.26	453.00	453.00	0.00	1
433.63	493.00	493.00	0.00	1
436.00	725.61	725.61	0.00	Overtopping

## Culvert Data: Contech ALBC Aluminum Box Culvert 20'-4" span by 4'6" rise

Total Disch arge (cfs)	Culve rt Disch arge (cfs)	Head water Elevat ion (ft)	Inle t Cont rol Dep th (ft)	Outl et Cont rol Dep th (ft)	Fl ow Ty pe	Nor mal Dep th (ft)	Criti cal Dep th (ft)	Out let De pth (ft)	Tailw ater Dept h (ft)	Outl et Velo city (ft/s )	Tailw ater Veloc ity (ft/s)
93.00 cfs	93.00 cfs	430.37	1.37	0.0*	1- S2 n	0.59	0.87	0.5 9	0.59	7.84	10.14
133.0 0 cfs	133.0 0 cfs	430.74	1.74	0.0*	1- S2	0.74	1.11	0.7 4	0.73	8.98	11.59

 Table 1 - Culvert Summary Table: Contech ALBC Aluminum Box Culvert 20'-4" span by 4'6" rise

					n						
173.0 0 cfs	173.0 0 cfs	431.08	2.08	0.0*	1- S2 n	0.87	1.32	0.8 9	0.85	9.75	12.77
213.0 0 cfs	213.0 0 cfs	431.41	2.41	0.0*	1- S2 n	0.99	1.51	0.9 9	0.97	10.7 2	13.78
253.0 0 cfs	253.0 0 cfs	431.74	2.74	0.14 3	1- S2 n	1.11	1.70	1.1 4	1.07	11.1 1	14.66
293.0 0 cfs	293.0 0 cfs	432.06	3.06	0.47 1	1- S2 n	1.22	1.87	1.2 6	1.17	11.6 7	15.46
333.0 0 cfs	333.0 0 cfs	432.34	3.34	0.81 0	1- S2 n	1.32	2.04	1.3 8	1.27	12.1 8	16.17
373.0 0 cfs	373.0 0 cfs	432.62	3.62	1.16 3	1- S2 n	1.43	2.19	1.4 9	1.35	12.6 5	16.83
413.0 0 cfs	413.0 0 cfs	432.90	3.90	1.53 3	1- S2 n	1.53	2.34	1.6 0	1.44	13.0 5	17.44
453.0 0 cfs	453.0 0 cfs	433.26	4.26	1.92 1	1- S2 n	1.62	2.49	1.7 2	1.52	13.4 1	18.01
493.0 0 cfs	493.0 0 cfs	433.63	4.63	2.31 4	5- S2 n	1.72	2.62	1.8 2	1.60	13.7 9	18.55

\* Full Flow Headwater elevation is below inlet invert.

#### **Culvert Barrel Data**

Culvert Barrel Type Straight Culvert

Inlet Elevation (invert): 429.00 ft,

Outlet Elevation (invert): 427.00 ft

Culvert Length: 27.07 ft,

Culvert Slope: 0.0741

# Water Surface Profile Plot for Culvert: Contech ALBC Aluminum Box Culvert 20'-4" span by 4'6" rise



Site Data - Contech ALBC Aluminum Box Culvert 20'-4" span by 4'6" rise Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 429.00 ft

Outlet Station: 27.00 ft

Outlet Elevation: 427.00 ft

Number of Barrels: 1

Culvert Data Summary - Contech ALBC Aluminum Box Culvert 20'-4" span by

## 4'6" rise

Barrel Shape: User Defined

Barrel Span: 20.23 ft

Barrel Rise: 4.26 ft

Barrel Material: Corrugated Metal Riveted or Welded

Embedment: 0.00 in

Barrel Manning's n: 0.0350 (top and sides)

Manning's n: 0.0350 (bottom)

Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall (Ke=0.5)

Inlet Depression: None

## **Tailwater Data for Crossing: Jacks Brook**

Flow (cfs)	Water Surface Elev (ft)	Velocity (ft/s)	Depth (ft)	Shear (psf)	Froude Number
93.00	427.59	0.59	10.14	1.84	2.37
133.00	427.73	0.73	11.59	2.28	2.45
173.00	427.85	0.85	12.77	2.67	2.50
213.00	427.97	0.97	13.78	3.02	2.54
253.00	428.07	1.07	14.66	3.35	2.58
293.00	428.17	1.17	15.46	3.66	2.61
333.00	428.27	1.27	16.17	3.95	2.63
373.00	428.35	1.35	16.83	4.23	2.65
413.00	428.44	1.44	17.44	4.49	2.67
453.00	428.52	1.52	18.01	4.75	2.69
493.00	428.60	1.60	18.55	5.00	2.71

#### Table 2 - Downstream Channel Rating Curve (Crossing: Jacks Brook)

#### **Tailwater Channel Data - Jacks Brook**

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 15.00 ft

Side Slope (H:V): 1.00 (\_:1)

Channel Slope: 0.0500

Channel Manning's n: 0.0220

Channel Invert Elevation: 427.00 ft

#### **Roadway Data for Crossing: Jacks Brook**

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 50.00 ft

Crest Elevation: 436.00 ft

Roadway Surface: Gravel

Roadway Top Width: 16.00 ft