STATE OF CONNECTICUT CONNECTICUT SITING COUNCIL

IN RE:	
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A PETITION OF CELLCO PARTNERSHIP : PETITION NO. ____

D/B/A VERIZON WIRELESS FOR A :

DECLARATORY RULING ON THE NEED TO OBTAIN A SITING COUNCIL CERTIFICATE FOR THE MODIFICATION OF AN EXISTING

TELECOMMUNICATIONS FACILITY AT 194

MOUNT PARNASSUS ROAD, EAST :

HADDAM, CONNECTICUT : FEBRUARY 22, 2024

PETITION FOR A DECLARATORY RULING: INSTALLATION HAVING NO SUBSTANTIAL ADVERSE ENVIRONMENTAL EFFECT

I. <u>Introduction</u>

Pursuant to Sections 16-50j-38 and 16-50j-39 of the Regulations of Connecticut State Agencies ("R.C.S.A."), Cellco Partnership d/b/a Verizon Wireless ("Cellco") (collectively the "Petitioners") hereby petitions the Connecticut Siting Council (the "Council") for a declaratory ruling ("Petition") that no Certificate of Environmental Compatibility and Public Need ("Certificate") is required under Section 16-50k(a) of the Connecticut General Statutes ("C.G.S.") to modify the existing wireless telecommunications facility at 194 Mount Parnassus Road (Route 434) in East Haddam, Connecticut. The proposed facility modifications involve a 40-foot extension of the existing 121-foot lattice tower and the installation of Cellco's antennas and remote radio heads at the 146-foot level of the extended tower. Cellco's radio equipment, a battery cabinet and a diesel-fuel back-up generator would be installed on the ground adjacent to the tower and within the existing fenced compound. If approved, the modified facility would

allow Cellco to provide its customers and emergency service providers with enhanced wireless services in central portions of East Haddam, in areas unserved or underserved today. Cellco has identified this cell site as its "East Haddam 3 Facility".

II. Factual Background

On February 19, 2015, the Siting Council approved Petition No. 1130, a request by the State of Connecticut ("State") Department of Emergency Services and Public Protection ("DESPP") to replace the existing 120-foot monopole tower with a new 120-foot lattice tower at 194 Mount Parnassus Road (Route 434) in East Haddam (the "Property"). (*See* Petition No. 1130 Staff Report included in <u>Attachment 1</u>). The Property is owned by the State and is used for emergency communications purposes. An existing paved access driveway extends from Mount Parnassus Road to the existing communications facility. (*See* Site Aerial Photograph included in <u>Attachment 2</u>).

The existing tower is shared by the DESPP and the Town of East Haddam with antennas located at various levels. Radio equipment associated with the existing antennas is located in an equipment shelter near the base of the tower.

Cellco is licensed to provide wireless telecommunications services in the 700 MHz, 850 MHz, 1900 MHz, 2100 MHz, 3550 MHz, 3600 MHz, and 3700 MHz (5G) frequency ranges in East Haddam and throughout the State of Connecticut. Cellco's wireless service in central portions of East Haddam is currently provided by four (4) existing macro-cell facilities, identified as *Salem CT*, a tower at 399 West Road in Salem, CT; *Chester*, a tower at 49 Wig Hill Road in Chester, CT; *East Haddam* CT, a tower at 135 Honey Hill Road in East Haddam; and *East Haddam 2* CT, a tower at 33 Neptune Avenue in Moodus (East Haddam), CT. Coverage Plots showing the extent of wireless service from these existing cell sites in Cellco's 700 MHz

"base" frequencies alone, and together with service from the proposed East Haddam 3 Facility are included in <u>Attachment 3</u>. As indicated on these plots, Cellco currently provides little or no reliable wireless service along significant portions of Routes 434 (Mount Parnassus Road) and local roads in the surrounding area in its 700 MHz operating frequencies. The proposed East Haddam 3 Facility would allow Cellco to fill these gaps, improve wireless service in the area overall and also provide some capacity relief to its adjacent Salem cell site.

III. Proposed East Haddam 3 Facility Modifications

To accommodate Cellco's need for improved wireless services in the area, Cellco proposes to extend the existing lattice tower by 40-feet. If approved, Cellco will install twelve (12) antennas and remote radio heads ("RRHs") on the extended tower at a centerline height of 146 feet above ground level. Cellco would install an equipment cabinet, a battery cabinet and diesel-fueled back-up generator all on the ground adjacent to the tower, within the existing fenced compound. Project plans and specification for Cellco's proposed antennas, remote radio heads ("RRHs") and backup generator for the modified wireless facility showing all proposed site improvements are included in Attachment 5 is a Structural Analysis Report ("SA") and an Antenna Mount Analysis Report ("MA") confirming that the existing tower, tower foundation, and Cellco's proposed new antenna mounting system can support Cellco's proposed modifications.

IV. <u>Discussion</u>

A. The Proposed Facility Modifications Will Not Have A Substantial Adverse Environmental Effect

The Public Utility Environmental Standards Act (the "Act"), C.G.S. § 16-50g et seq., provides for the orderly and environmentally compatible development of telecommunications towers in the state to avoid "a significant impact on the environment and ecology of the State of

Connecticut." C.G.S. § 16-50g. To achieve these goals, the Act established the Council, and requires a Certificate of Environmental Compatibility and Public Need for the construction of cellular telecommunication towers "that may, as determined by the council, have a substantial adverse environmental effect". C.G.S. § 16-50k(a).

1. Physical Environmental Effects

Cellco respectfully submits that the proposed facility modifications will not involve a significant alteration in the physical and environmental characteristics of the Property or the surrounding area. Cellco will install two equipment cabinets and a back-up generator within the limits of the existing fenced facility compound. No tree clearing or facility expansion is required to accommodate Cellco's ground-based equipment or the proposed tower extension.

2. Visual Effects

As discussed in numerous other Council filings, visual impact of a tower is often the most significant and, in many cases, the only discernible environmental effect associated with such facilities. To assess these conditions, Cellco, through its consultants, All-Points Technologies ("APT") assessed the visual impact of the existing lattice tower and compared it to the visual impact of the proposed 161-foot lattice tower described in the Petition. A copy of APT's Visibility Analysis is included in Attachment 6.

The Visibility Analysis concludes that the upper portion of the modified lattice tower is not highly visible beyond the immediate vicinity of the Property. The open steel weave of the lattice tower, combined with the rolling and wooded terrain within the two-mile radius study area, serve to minimize the visibility of the extended structure. The taller tower may be more prominent in areas where the existing tower is visible, but no new areas of visibility are created by the proposed tower extension. Overall, year-round visibility of the extended tower will

-4-

increase from approximately 12-acres for the existing structure to approximately 23 acres for the extended tower or 0.28% of the two-mile study area. Seasonal visibility of the extended tower would occur over approximately 94 acres or 1.116% of the two-mile radius study area.

3. Compliance with Radio Frequency Emissions Standards

Cumulative radio frequency ("RF") emissions from the proposed replacement tower will not exceed the Maximum Permissible Exposure ("MPE") standards adopted by the Federal Communications Commission ("FCC"). Included in <u>Attachment 7</u> is a Calculated Radio Frequency Emissions Report conforming that the existing State and municipal antennas together with the proposed Cellco antennas will operate well within MPE standards established by the FCC.

4. <u>FAA Summary Report</u>

Included in <u>Attachment 8</u> of this Petition is a Federal Airways and Airspace Summary Report (the "Summary Report"). The Summary Report recommends that the FAA be notified of the existing structure and of the proposed extended structure to determine if obstruction marking and lighting is required. Cellco will make the appropriate FAA notice filing if the Council approves the tower extension. A copy of the FAA notification and the FAA's final determination will be provided to the Council upon receipt.

In sum, the effect of the modified facility on the environment would be minimal and limited, rather than significant. This stands in contrast to typical proposals for new towers that frequently must be located on properties with no other approved towers, or with no development at all. Thus, the proposed tower extension would not present a substantial adverse environmental effect and is not a modification for which the General Assembly intended to require a Certificate under C.G.S. § 16-50k(a).

B. Notice to the Town of East Haddam, Property Owner and Abutting Landowners
On February 22, 2024, a copy of this Petition was sent to East Haddam's First Selectman
Irene M. Haines, Land Use Administrator, James Ventres and the State of Connecticut, the
Property owner. Included in Attachment 9 is a copy of the letters sent to Ms. Haines, Mr.
Ventres, and the State of Connecticut.

Notice of Cellco's intent to file the Petition together with a copy of the Petition was also sent to those owners whose land abuts the Property. A sample abutter's notice letter, and the list of those abutting landowners who were sent notice of the Petition is included in <u>Attachment 10</u>.

C. <u>A Conclusion That the Proposed Facility Modifications Will Not Have a</u>

<u>Substantial Adverse Environmental Effect Would Be Consistent With Siting Council Precedent</u>

The Council has previously determined, under similar circumstances, that the extension of an existing tower would have no substantial adverse environmental effect, does not require a Certificate and, most importantly, is preferable to the construction of a new tower in this area.

V. Conclusion

Based on the information provided above, Cellco respectfully requests that the Council issue a determination in the form of a declaratory ruling that the 40-foot extension of the existing tower at the Property (from 121 feet to 161 feet), and the installation of Cellco antennas and RRHs on a mounting assembly at the 146-foot level and the installation of the radio equipment and a backup generator in the existing facility compound will not have a substantial adverse environmental effect and does not require the issuance of a Certificate of Environmental Compatibility and Public Need pursuant to § 16-50k of the General Statutes.

Respectfully submitted,

CELLCO PARTNERSHIP d/b/a VERIZON WIRELESS

 $By_{\underline{}}$

Kenneth C. Baldwin, Esq. Robinson & Cole LLP 280 Trumbull Street Hartford, CT 06103-3597 (860) 275-8200

Attorney for the Petitioner

ATTACHMENT 1

Petition No. 1130 DESPP East Haddam, Connecticut Staff Report February 19, 2015

On December 23, 2014, the Connecticut Siting Council (Council) received a petition from the Connecticut Department of Emergency Services and Public Protection (DESPP) for a declaratory ruling that no Certificate of Environmental Compatibility and Public Need is required for the replacement of an existing 120-foot tall monopole tower with a self-supporting lattice tower of the same height. The tower site is at 194 Mount Parnassus Road in East Haddam, Connecticut. Council members Phil Ashton and Larry Levesque with Siting Analyst David Martin and Supervising Siting Analyst Christina Walsh visited the site on February 4, 2015 to review the proposal. Brian Benito and Matt Valleau represented DESPP at the field review. Craig Mansfield, the Emergency Management Director of the Town of East Haddam, was also present.

The existing 120-foot monopole was erected prior to the 1960s and currently hosts antennas of the DESPP and the Department of Transportation (DOT). DESPP is now working with DOT and the Town of East Haddam to upgrade their respective public safety telecommunications systems. The three parties are interested in using the Mt. Parnassus Road site, but the existing tower is structurally deficient and not capable of supporting all of the anticipated antenna systems. DESPP seeks to replace the existing monopole with a 120-foot lattice tower, designed to the Revision G tower standard that could accommodate all of the intended antenna systems as well potentially hosting commercial wireless antenna systems in the future. A lattice tower would provide the stability needed by DESPP to maintain microwave links between its adjacent sites. This location would also provide much improved public safety coverage for the Town of East Haddam, which encompasses a large area with topography that is difficult for wireless telecommunications.

In addition to replacing the existing tower, DESPP would replace an existing 80 square foot equipment building with an 882 square foot equipment building that would be used by each of the three parties. The new equipment building would include a 70 kW generator, which would provide back up power for all three tower users. The generator would run on propane, which would be supplied from an 1,800 gallon propane tank to be buried at the site.

The new tower and equipment building would be enclosed by an eight-foot tall chain link fence within a compound area of approximately 6,500 square feet. When asked, the DESPP representatives said they could specify a small mesh size to make climbing the fence more difficult.

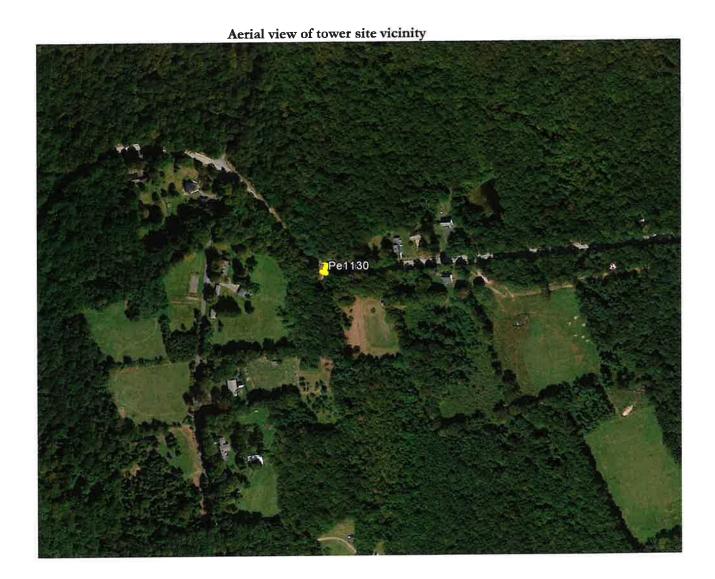
DESPP submitted its plans to the State Historic Preservation Office for its review. SHPO determined that no historic properties would be affected by the proposal. A review of the Natural Diversity Data Base map for East Haddam indicates that no State or Federal Listed Species occur in the vicinity of the site.

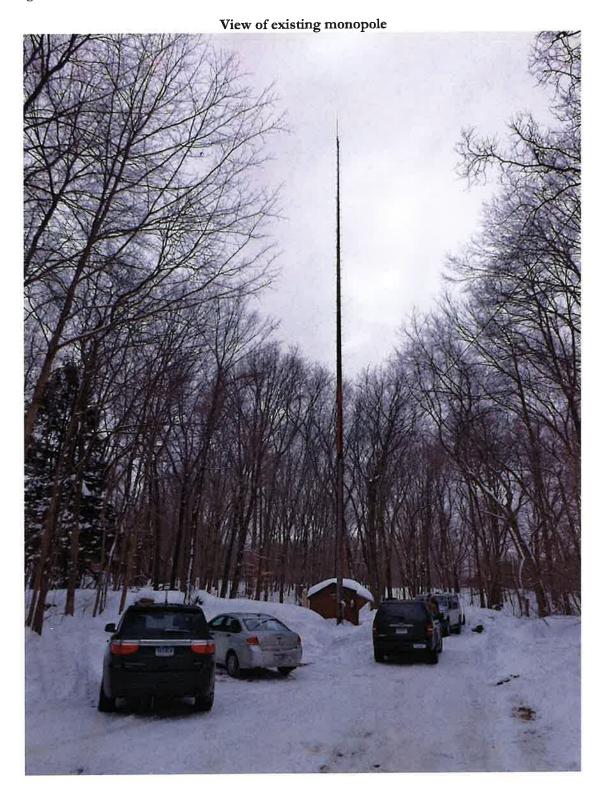
The area around the Mount Parnassus site is heavily wooded with sparse single family residential development. The existing tower is scarcely visible amid the trees, so much so that Council representatives drove past it even though they were looking for it. The replacement tower, although it would present a broader profile, will be set further back from the road and should not be significantly more visible, even to the nearest neighbors.

DESPP calculates that the power density of the antennas to be located on the replacement tower would equal approximately 2.5% of the FCC's Maximum Permissible Emissions limit.

For this petition, DESPP notified the Town and abutting property owners. No comments have been received from any neighbors. The Town fully endorses the proposal.

This proposed tower replacement is not expected to have any substantial adverse environmental effects. Staff recommends approval with the condition that the Council shall be notified in writing when the existing tower is removed and the new self-supporting lattice tower is operational.





ATTACHMENT 2





Proposed Verizon Wireless Equipment

= = = = Proposed Verizon Wireless Conduit

Proposed Verizon Wireless Lease Area

x - x Proposed Verizon Wireless Fence Extension

Subject Property

Approximate Parcel Boundary

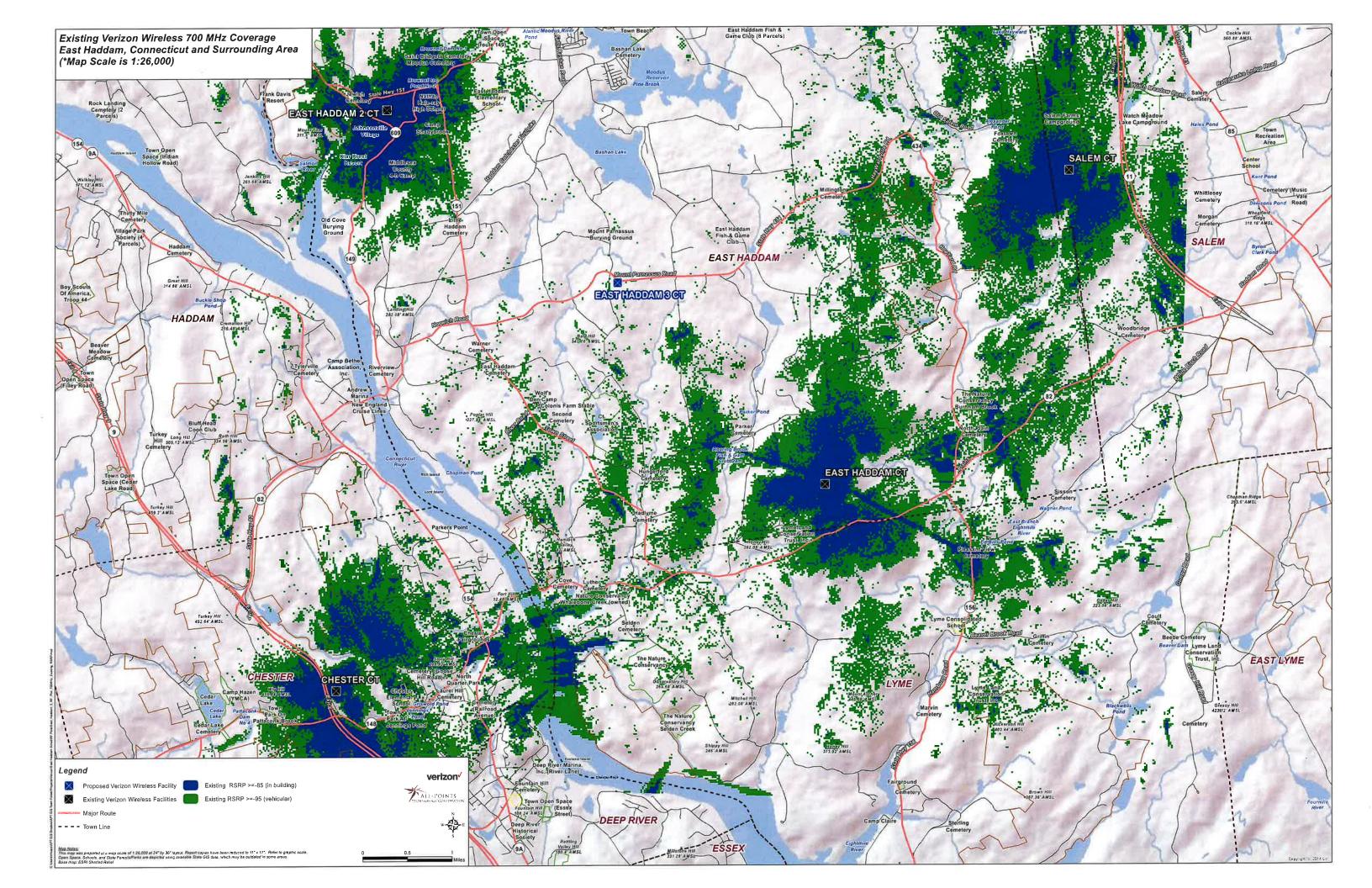
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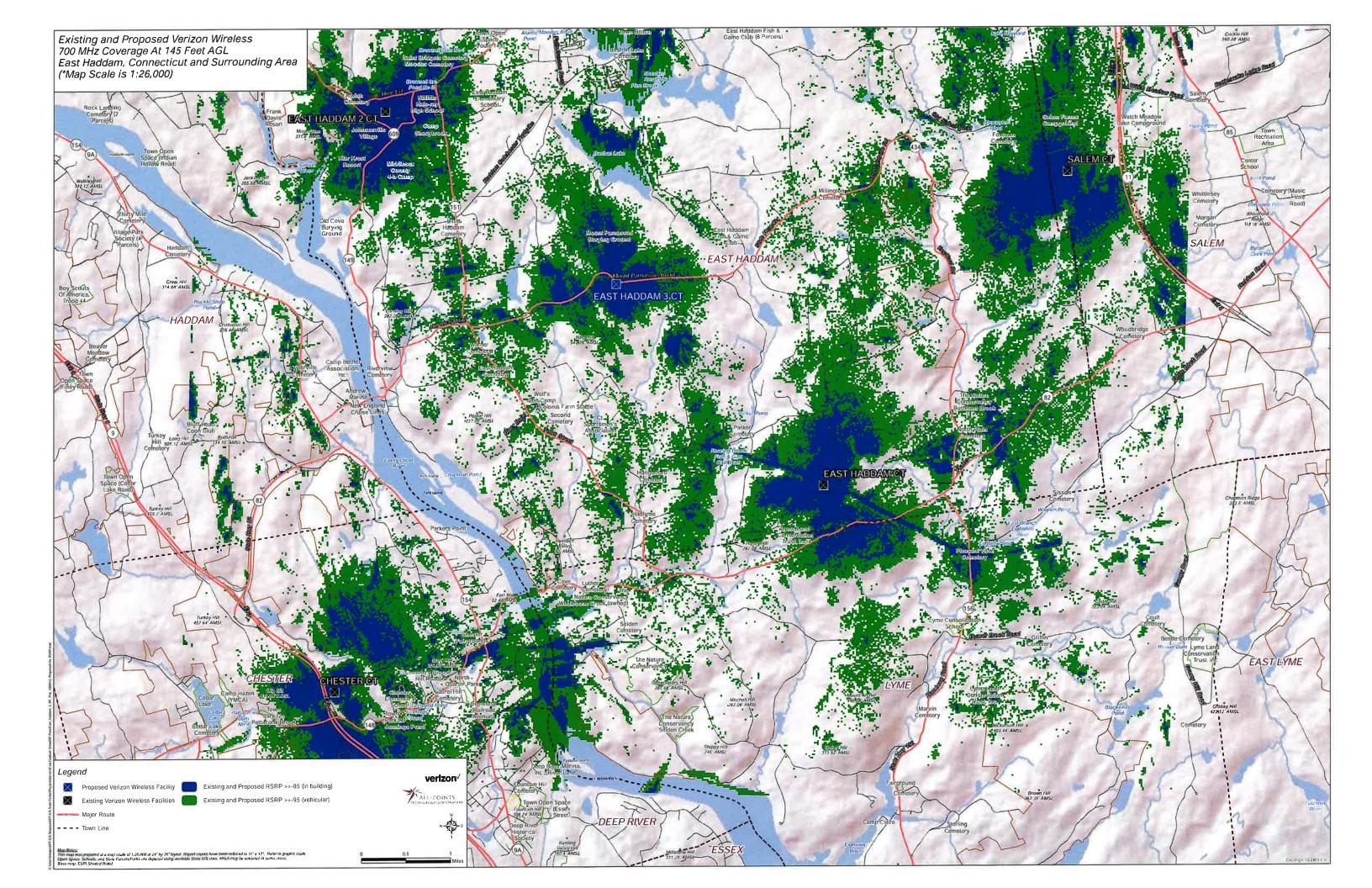
Proposed Wireless Telecommunications Facility East Haddam 3 CT 194 Mt Parnassus Road East Haddam, Connecticut

verizon/



ATTACHMENT 3





ATTACHMENT 4

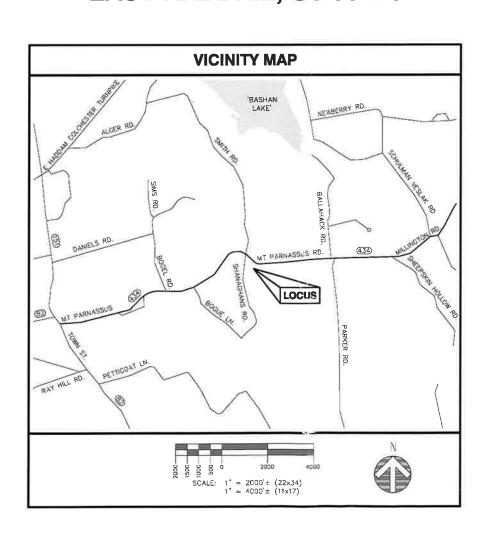


SITE NAME: EAST HADDAM 3 CT LOCATION CODE (PSLC): 300023 PROJECT ID: 17133983 ADDRESS: 194 MT PARNASSUS ROAD EAST HADDAM, CT 06423

	DRAWING INDEX	
SHEET	DESCRIPTION	REVISION
T-1	TITLE SHEET	1
Z-1	OVERALL SITE PLAN	1
Z-2	COMPOUND PLAN AND ELEVATION	1
D-1 TO D-3	DETAILS	1

GENERAL NOTES

- CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER & OWNER REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
- 2. ALL UNDERGROUND UTILITY INFORMATION WAS DETERMINED FROM SURFACE INVESTIGATIONS AND EXISTING PLANS OF RECORD. THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND UTILITIES IN THE FIELD PRIOR TO ANY SITE WORK, CALL DIG-SAFE (888) 344-7233 (OR 811) 72-HOURS PRIOR TO ANY EXCAVATION.
- 3. ALL WORK TO BE PERFORMED IN ACCORDANCE WITH THE LATEST VERIZON WIRELESS CONSTRUCTION GUIDELINES.
- NEW CONSTRUCTION WILL CONFORM TO ALL APPLICABLE CODES AND ORDINANCES:
- BUILDING CODE: 2022 CONNECTICUT STATE BUILDING CODE (IBC 2021)
 AND AMENDMENTS
- ELECTRICAL CODE: 2020 NEC (NFPA-70) WITH CONNECTICUT AMENDMENTS
- ALL WORK SHALL BE COMPLETED IN ACCORDANCE WITH THE PASSING ANTENNA MOUNT ANALYSIS REPORT BY CENTEK ENGINEERING DATED OCTOBER 9, 2023.
- 6. REFER TO PASSING STRUCTURAL ANALYSIS BY CENTEK ENGINEERING DATED OCTOBER 6, 2023.
- THE CONSTRUCTION SHOWN HEREIN MAY REQUIRE SPECIAL INSPECTIONS UNDER THE STATE BUILDING CODE, APPLICANT/CONTRACTOR SHALL VERIFY WITH THE AUTHORITIES HAVING JURISDICTION (AHJ) PRIOR TO CONSTRUCTION AND ENGAGE THE INSPECTOR AND/OR APPROPRIATE 3RD PARTIES AS MAY BE REQUIRED.



PROJECT INFORMATION CO-LOCATION ON EXISTING SELF-SUPPORT TOWER TO BE EXTENDED SITE TYPE: SITE NAME: EAST HADDAM 3 CT LOCATION CODE (PSLC): 300023 FUZE ID: 17133983 RFDS DATE: 08/01/23 REV1 SITE ADDRESS: 194 MT PARNASSUS ROAD EAST HADDAM, CT 06423 COUNTY: PARCEL #: 41° 28' 12.04"± N (41.470012'± N) (RECORD SURVEY 1-A) TOWER LATITUDE: 72° 24' 39.83"± W (72.411064°± W) (RECORD SURVEY 1-A) TOWER LONGITUDE: 595.0'± (AMSL) (RECORD SURVEY 1-A) TOWER GROUND ELEV .: NAD83/NAVD88 DATUM: STRUCTURE HEIGHT: 161'± (TOP OF SELF-SUPPORT TOWER) HIGHEST APPURTENANCE: 165'± (TOP OF LIGHTNING ROD) N/F STATE OF CONNECTIOU" NEWINGTON, CT 06131 STATE OF CONNECTICUT TOWER OWNER: STATE OF CONNECTICUT PUBLIC SAFETY DEPT. 1111 COUNTRY CLUB ROAD MIDDLETOWN, CT 06457 TOWER OWNER ADDRESS: CELLCO PARTNERSHIP, A DELAWARE GENERAL PARTNERSHIP, doo VERIZON WRELESS 20 ALEXANDER DRIVE, SECOND FLOOR APPLICANT: WALLINGFORD, CT 06492 PROTERRA DESIGN GROUP, LLC 4 BAY ROAD BUILDING A, SUITE 200 HADLEY, MA 01035 SITE ENGINEER: (413) 320-4918

PERMITTING





ADDRESS: 194 MT PARASSONS ROLD

EAST HADDAM, CT 06423

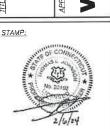
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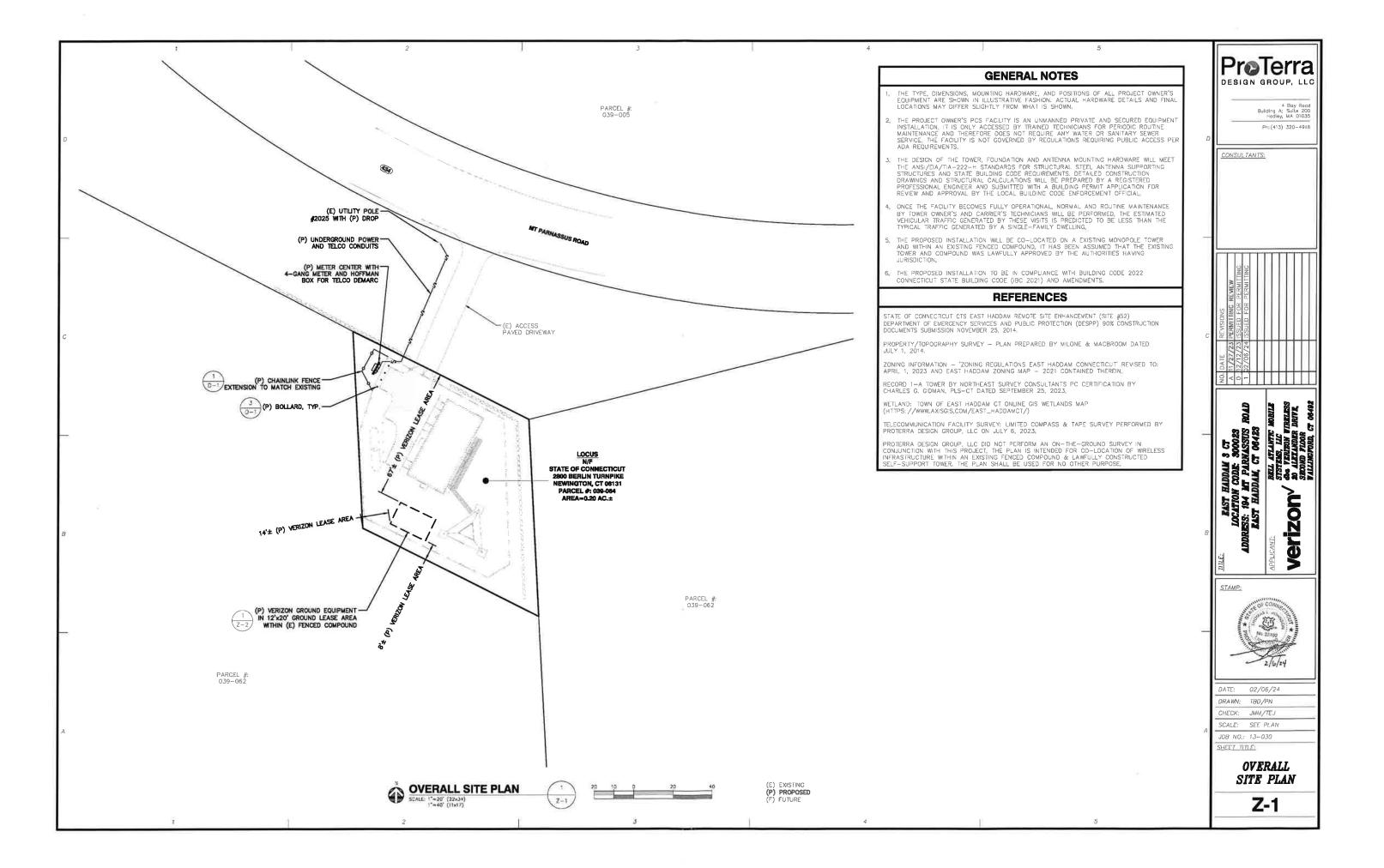
SCALE: SEE PLAN

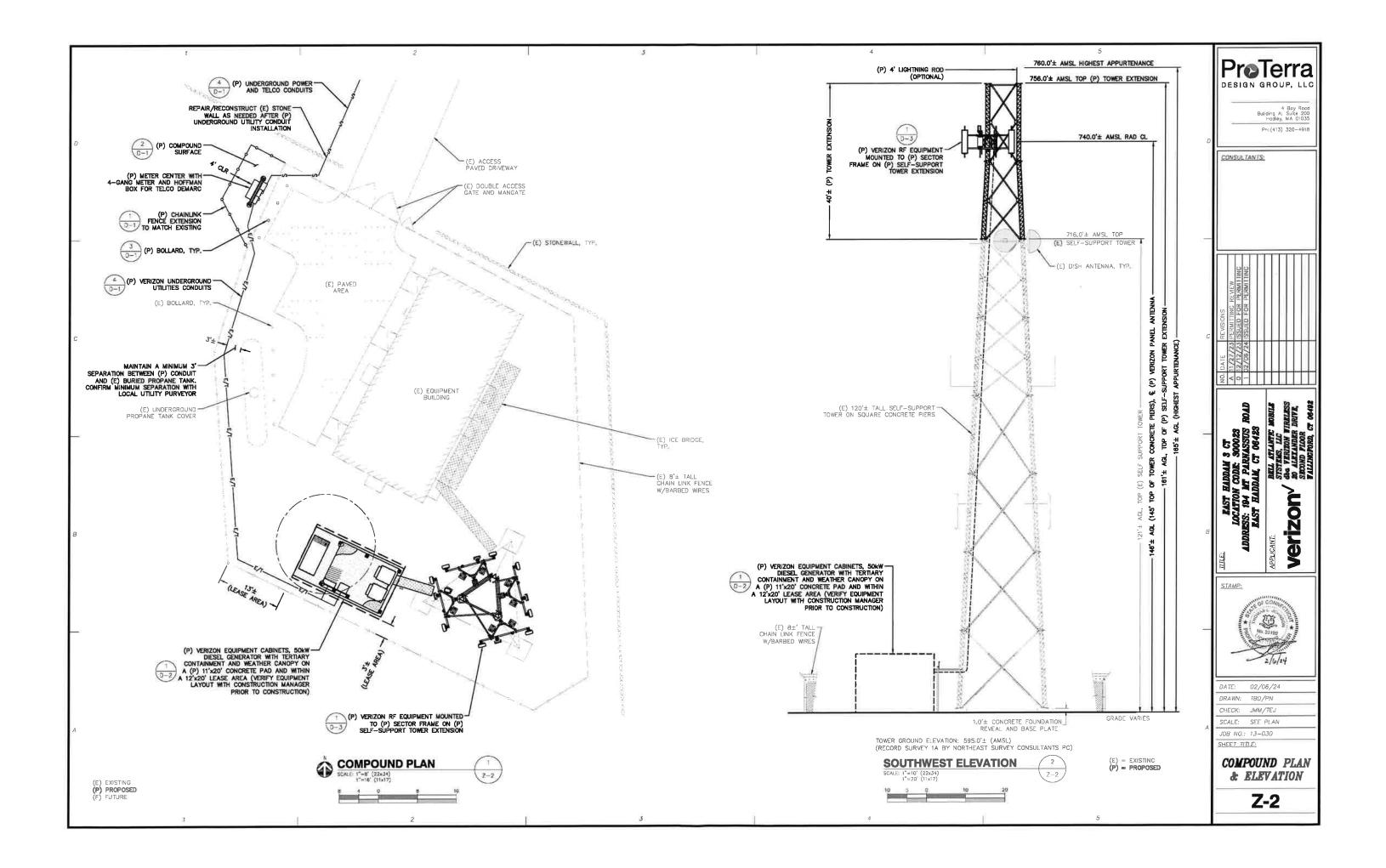
JOB NO: 13-030

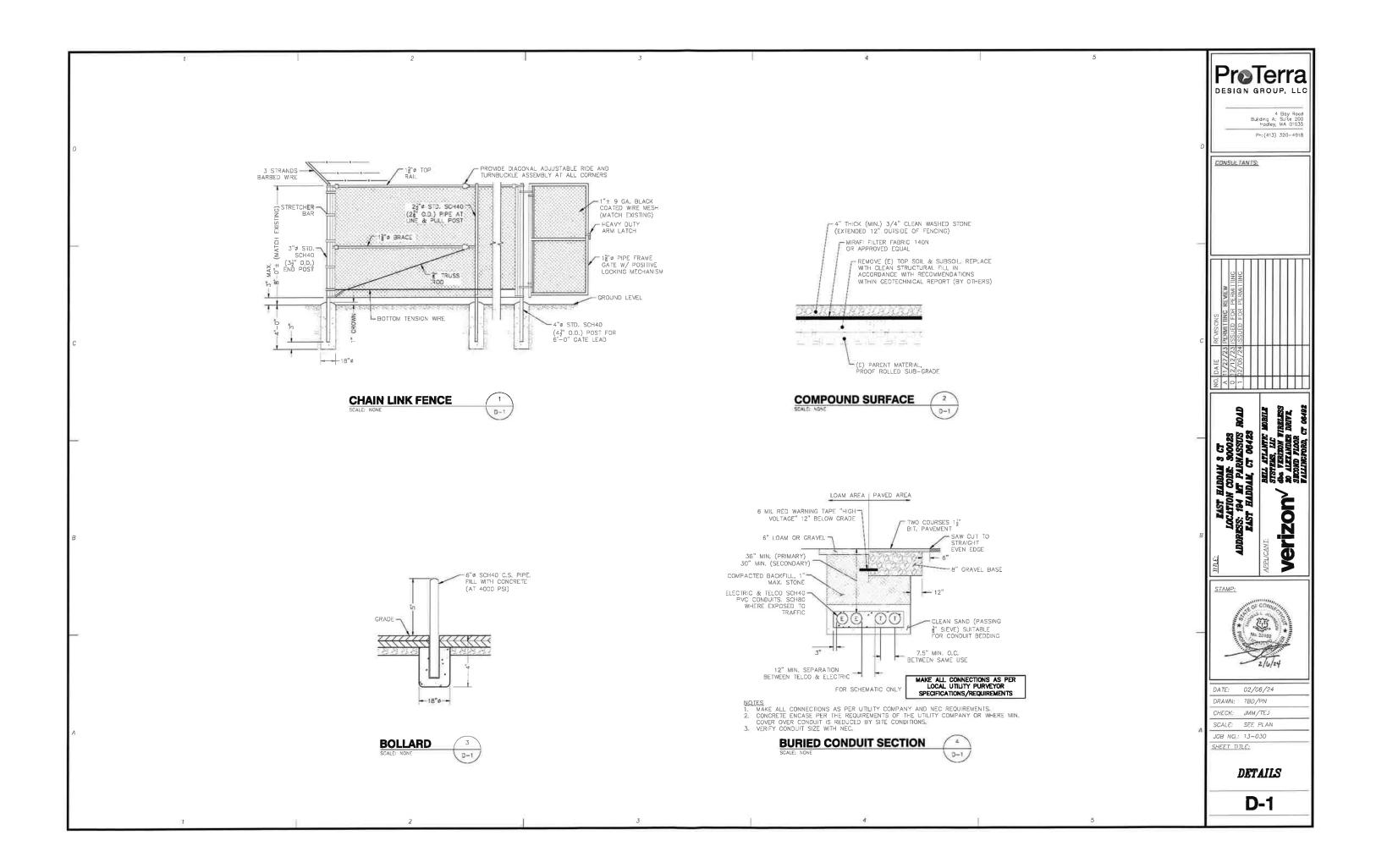
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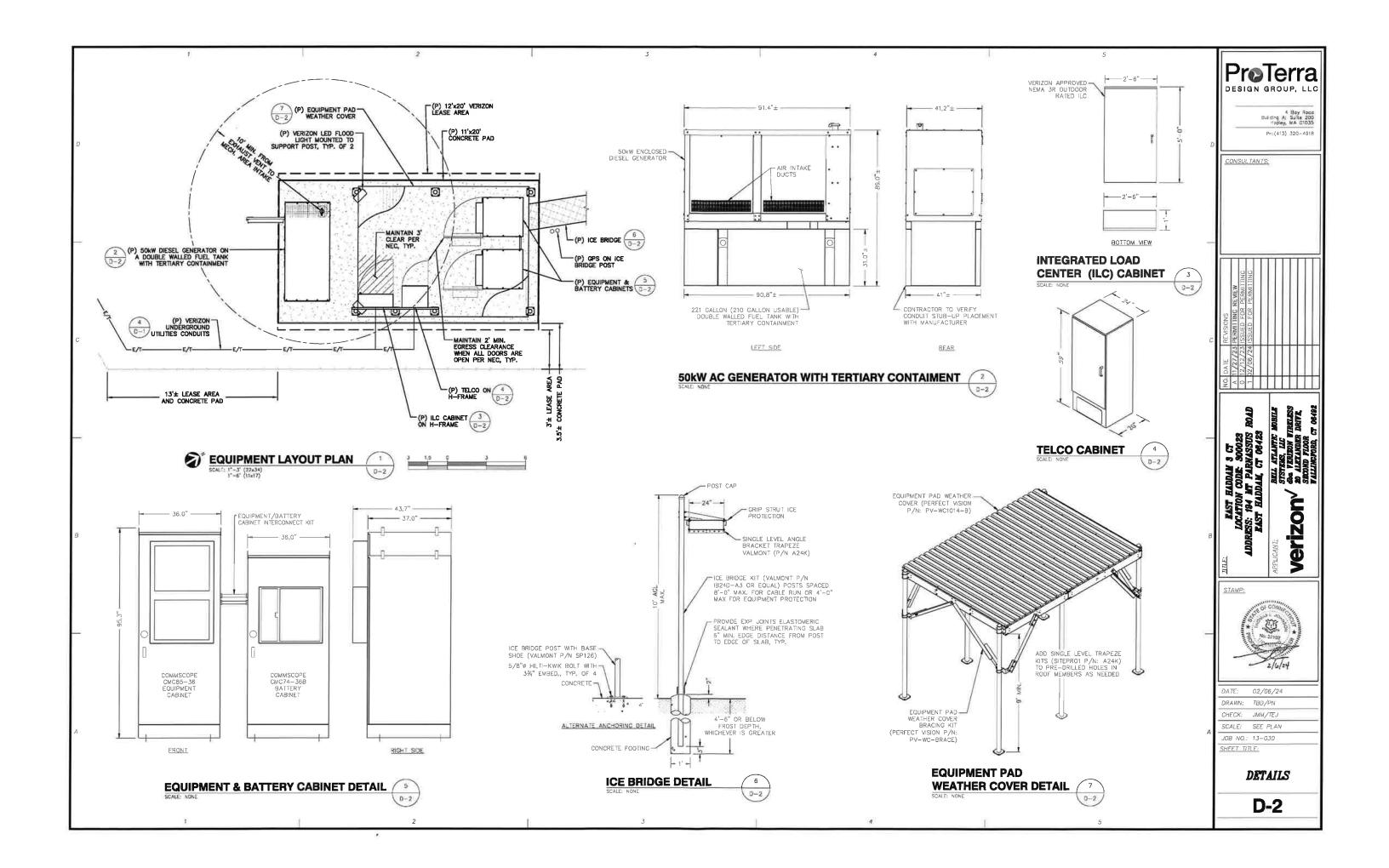
TITLE SHEET

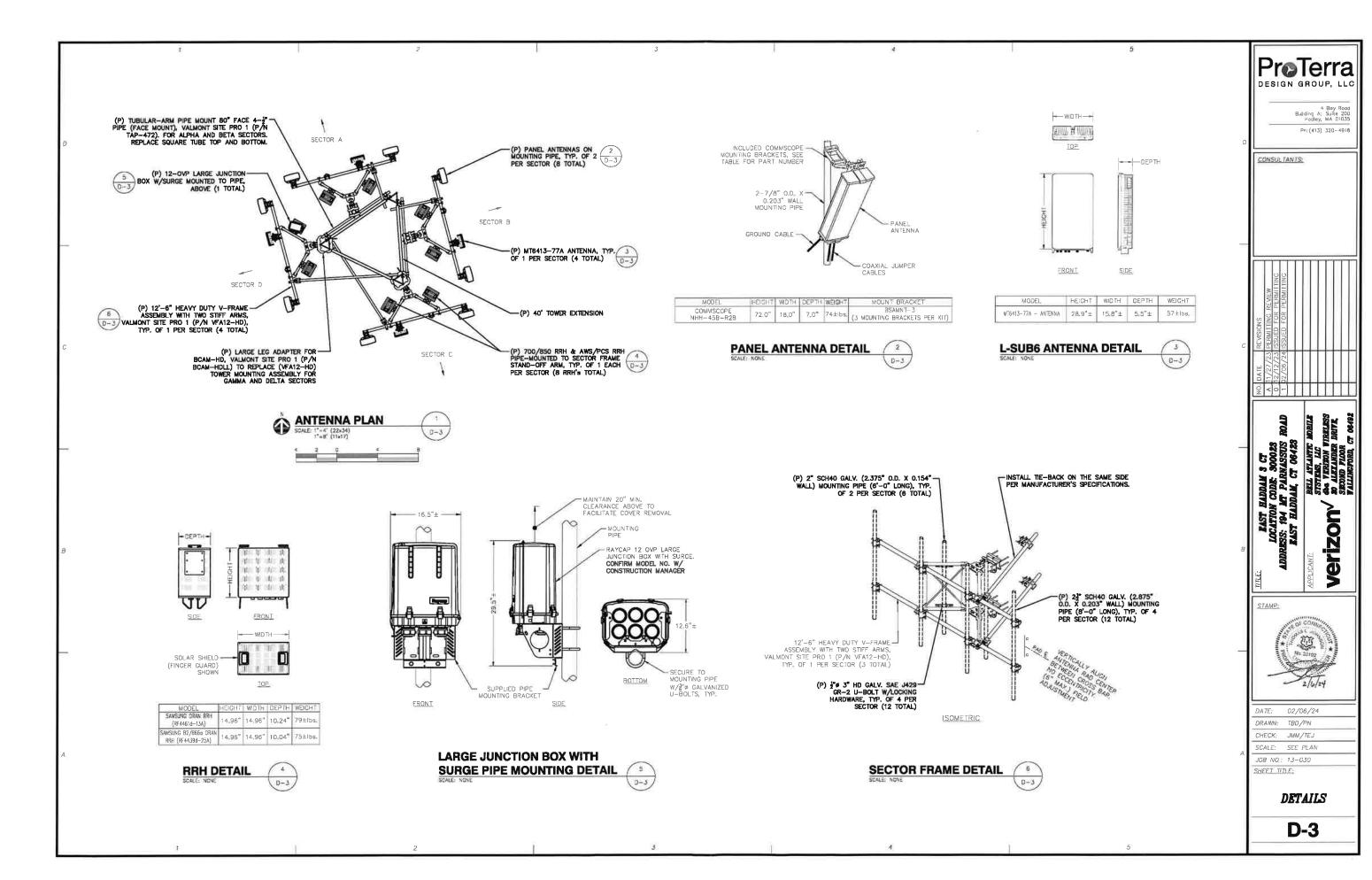
T-1











NHH-45B-R2B



6-port sector antenna, 2x 698–896 and 4x 1695–2360 MHz, 45° HPBW, 2x RETs and 2x SBTs. Both high bands share the same electrical tilt.

- Narrow beamwidth capacity antenna for higher level of densification and enhanced data throughput
- Internal SBT on low and high band allow remote RET control from the radio over the RF jumper cable
- Separate RS-485 RET input/output for low and high band
- One LB RET and one HB RET. Both high bands are controlled by one RET to ensure same tilt level for 4x Rx or 4x MIMO

General Specifications

Antenna Type Sector

Band Multiband

Color Light gray

Grounding TypeRF connector body grounded to reflector and mounting bracket

6

Performance Note

Outdoor usage | Wind loading figures are validated by wind tunnel

measurements described in white paper WP-112534-EN

Radome Material Fiberglass, UV resistant

Radiator Material Aluminum | Low loss circuit board

Reflector Material Aluminum

RF Connector Interface 4.3-10 Female

RF Connector Location Bottom

RF Connector Quantity, high band 4

RF Connector Quantity, low band 2

RF Connector Quantity, total

Remote Electrical Tilt (RET) Information

RET Interface 8-pin DIN Female | 8-pin DIN Male

RET Interface, quantity 2 female | 2 male

Input Voltage 10-30 Vdc

Internal Bias Tee Port 1 | Port 3

Internal RET High band (1) | Low band (1)

COMMSCOPE°

NHH-45B-R2B

Power Consumption, idle state, maximum 1 W

Power Consumption, normal conditions, maximum 10 W

Protocol 3GPP/AISG 2.0 (Single RET)

Dimensions

Width 457 mm | 17.992 in

Depth 178 mm | 7.008 in

Length 1829 mm | 72.008 in

Net Weight, without mounting kit 33.4 kg | 73.634 lb

Array Layout



Array	Freq (MHz)	Conns	RET (SRET)	AISG RET UID
R1	698-896	1-2	1	ANxxxxxxxxxxxxxxxx1
Y1	1695-2360	3-4	2	ANxxxxxxxxxxxxxx
Y2	1695-2360	5-6	2	AIVXXXXXXXXXXXXXXXX

(Siz

(Sizes of colored boxes are not true depictions of array sizes)

Port Configuration

Bottom



Electrical Specifications

Impedance 50 ohm

Operating Frequency Band 1695 – 2360 MHz | 698 – 896 MHz

Polarization ±45°

Total Input Power, maximum 800 W @ 50 °C

Electrical Specifications

Frequency Band, MHz	698-806	806-896	1695-188	80 1850-1990	1920-2200	2300-2360
Gain, dBi	16.8	17.5	19.3	19.9	20.3	20.8
Beamwidth, Horizontal degrees	, 48	43	4 5	43	41	39
Beamwidth, Vertical, d	egrees 12.5	11.4	5.8	5.4	5	4.5
Beam Tilt, degrees	2-14	2-14	0-8	0-8	0-8	0-8
USLS (First Lobe), dB	19	22	18	18	18	17
Front-to-Back Ratio at dB	180°, 34	39	37	38	40	38
Isolation, Cross Polariz dB	zation, 25	25	25	25	25	25
Isolation, Inter-band, d	B 30	30	28	28	28	28
VSWR Return loss, dE	1,5 14,0	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0	1.5 14.0

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NHH-45B-R2B

	- L-J					
PIM, 3rd Order, 2 x 20 W, dBc	-153 300	-153 300	-153 300	-153 300	-153 300	-153 250
maximum, watts						
Electrical Specificati	ons. BAS	ГА				
Frequency Band, MHz	698-806	806-896	1695-1880	1850-1990	1920-2200	2300-2360
Gain by all Beam Tilts, average, dBi	16.5	17.2	19.1	19.8	20.2	20.8
Gain by all Beam Tilts Tolerance, dB	±0.4	±0.4	±0.5	±0.4	±0.4	±0.3
Gain by Beam Tilt, average, dBi	2 * 16.5 8 * 16.6 14 * 16.3	2° 173 8° 174 14° 169	0 ° 19.0 4 ° 19.2 8 ° 19.0	0° 19.7 4° 19.9 8° 19.7	0° 20.0 4° 20.2 8° 20.2	0° 20.6 4° 20.9 8° 20.6
Beamwidth, Horizontal Tolerance, degrees	±1.5	±2.8	±1.8	±1	±2.7	±1.4
Beamwidth, Vertical Tolerance, degrees	±0.7	±0.6	±0.3	±0.2	±0.3	±0.1
USLS, beampeak to 20° above beampeak, dB	19	23	16	17	16	16
Front-to-Back Total Power at 180° ± 30°, dB	24	24	29	31	33	33
CPR at Boresight, dB	25	26	1 9	20	18	17
CPR at Sector, dB	6	4	10	10	8	16
Mechanical Specifica	itions					
Effective Projective Area (EPA),	frontal		1 m ² 10.764 f	t ²		
Effective Projective Area (EPA),	, lateral		0.21 m ² 2.26 f	ft²		
Wind Loading @ Velocity, fronta	al		1,065.0 N @ 150	km/h (239. 4 l bf @	150 km/h)	
Wind Loading @ Velocity, lateral 220.0 N @ 150 km/h (49.5 lbf @ 150 km/h)						
Wind Loading @ Velocity, maximum 1,065.0 N @ 150 km/h (239.4 lbf @ 150 km/h)) 150 km/h)				
Wind Loading @ Velocity, rear			935.0 N @ 150 kr	m/h (210.2 lbf @ 1	150 km/h)	
Wind Speed, maximum			241 km/h 149	9.75 mph		
Packaging and Weig	hts					
Width, packed			608 mm 23.93	37 in		
Depth, packed			346 mm 13.62	22 in		
Length, packed			1970 mm 77.	559 in		
				4		

55.8 kg | 123.018 lb

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Weight, gross

NHH-45B-R2B

Regulatory Compliance/Certifications

Agency

Classification

CHINA-ROHS

Above maximum concentration value

ISO 9001:2015

Designed, manufactured and/or distributed under this quality management system

ROHS

Compliant/Exempted





Included Products

BSAMNT-3

Wide Profile Antenna Downtilt Mounting Kit for 2.4 - 4.5 in (60 - 115 mm) OD round members.
 Kit contains one scissor top bracket set and one bottom bracket set.

BSAMNT-M

 Middle Downtilt Mounting Kit for Long Antennas for 2.4 - 4.5 in (60 - 115 mm) OD round members. Kit contains one scissor bracket set.

* Footnotes

Performance Note

Severe environmental conditions may degrade optimum performance



C-band 64T64R

Gen 2

Gen 2: Higher conducted power radio with reduced size/volume/weight vs Gen 1 and also SOC embedded for flexibility to support new features



₩ Preliminary Design: External appearance and mechanical design can be subject to change

Own 2. 64T64	Gan 2. 64T64R C-band WMU Dimensions
Size (WwHxD)	400 x 734 x 140 mm (15.75 x 28.90 x 5.51 Inch)
Weight	26kg (57.3 lb)

Reserved.
All Rights
Electronics
Samsung

Itam	Gen 2 64T64R (MT6413-77A)
Air Technology	NR n77/TDD
Frequency	3700 – 3980 MHz
Wal	200 MHz
W8C	200 MHz
Carrier Bandwidth	20(HW rearly)/40/60/100 MHz
# of Carriers	2 carriers
Layer	DL:16L, UL:16RX (8L)
RF Chain	64T64R
Antenna Configuration	4V16H with 192 AE
EIRP	80.5 dBm @320W (55 dBm + 25.5 dBi)
Canductive Power	320W
Spectrum Analyzer	TX/RX support
RX Sensitivity	Typical -97.8dBm @(1Rx, 18.36MHz with 30kHz,51RBs)
Modulation	DL 256QAM support, (DL 1024QAM with 1~2dB power back-off)
Function Split	DL/UL option 7-2x
Input Power	-48 VDC (-38 VDC to -57 VDC)
Power Consumption	1,287W (100% load, room temp.)
Size (WHD)	400 x 734 x 140 mm (15.75 x 28.90 x 5.51 lnch)
Volume	41.11
Weight	26kg (57.3 lb)
Operating Temperature	-40°C - 55°C (w/o solar load)
Cooling	Natural convection
	3GPP 38.104
	FCC 47 CFR 27.53 : < -13dBm/MHz
Unwanted Emission	< -40 dBm/MHz @ above 4 GHz <-50 dBm /MHz @ 4,040 ~ 4,050 MHz, <-60 dBm /MHz @ above 4,050 MHz
Optic Interface	15km, 4 ports (25Gbps x 4), 5FP28, single mode, Bi-di (Option: Duplex)
Mounting Options	Pole, wall
NB-1oT	Not support
External Alarm	4RX
Emothani Interface	Mode

SAMSUNG

AWS/PCS MACRO RADIO

DUAL-BAND AND HIGH POWER FOR MACRO COVERAGE

Samsung's future proof dual-band radio is designed to help effectively increase the coverage areas in wireless networks. This AWS/PCS 4T4R dual-band radio has 4Tx/4Rx to 2Tx/2Rx RF chains options and a total output power of 320W, making it ideal for macro sites.

Model Code

RF4439d-25A



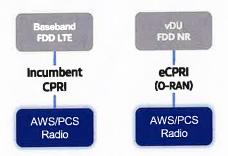




Points of Differentiation

Continuous Migration

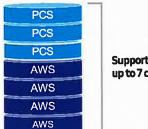
Samsung's AWS/PCS macro radio can support each incumbent CPRI interface as well as advanced eCPRI interfaces. This feature provides installable options for both legacy LTE networks and added NR networks.



Optimum Spectrum Utilization

The number of required carriers varies according to site (region). Supporting many carriers is essential for using all frequencies that the operator has available.

The new AWS/PCS dual-band radio can support up to 3 carriers in the PCS (1.9GHz) band and 4 carriers in the AWS (2.1GHz) band, respectively.



Supports up to 7 carriers

Technical Specifications

Item	Specification
Tech	LTE/NR
Brand	B25(PCS), B66(AWS)
Frequency Band	DL: 1930 – 1995MHz, UL: 1850 – 1915MHz DL: 2110 – 2200MHz, UL: 1710 – 1780MHz
RF Power	(B25) 4 × 40W or 2 × 60W (B66) 4 × 60W or 2 × 80W
IBW/OBW	(B25) 65MHz / 30MHz (B66) DL 90MHz, UL 70MHz / 60MHz
Installation	Pole, Wall
Size/ Weight	14.96 x 14.96 x 10.04inch (36.8L) / 74.7lb

O-RAN Compliant

A standardized O-RAN radio can help in implementing costeffective networks, which are capable of sending more data without compromising additional investments.

Samsung's state-of-the-art O-RAN technology will help accelerate the effort toward constructing a solid O-RAN ecosystem.



Brand New Features in a Compact Size

Samsung's AWS/PCS macro radio offers several features, such as dual connectivity for baseband for both CDU and vDU, O-RAN capability, more carriers and an enlarged PCS spectrum, combined into an incumbent radio volume of 36.8L



Same as an Incumbent radio volume

700/850 4T4R Macro 320W ORU - New Filter (RF4461d-13A)

Specifications



* 5MHz supporting in B13(700MHz) depends on 3GPP std. and UE capability. External filters in interferer and victim sides for Mexican boarder to support 5MHz service need to be considered
** Finger guard is not needed.

Item	Specif	Specification
Air Interface	LTE, NR(HW P	LTE, NR(HW resource ready)
Band	Band13 (700MHz)	Band5 (850MHz)
-	DL: 746~756MHz	DL: 869~894MHz
Frequency	UL: 777~787MHZ	UL: 824~849MHz
IBW	10MHz	25MHz
OBW	10MHz	25MHZ
Carrier Bandwidth	LTE/NR 5*/10MHz	LTE 5/10MHz NR 5/10/15/20MHz
# of carriers	2C*	3C
Total # of carriers	4C + 81	4C + 813 (SDL) 1C
RF Chain	414R/214R 212R+212	4T4R/2T4R/2T2R/1T2R 2T2R+2T2R bi-sector
	Total	Total : 320W
RF Output Power	4 x 40W or 2 x 60W	4 x 40W or 2 x 60W
Spectrum Analyzer	TX/RX	TX/RX Support
RX Sensitivity	Typ104.5d8m @	Typ104.5d8m @1Rx (25R8s 5MHz)
Modulation	256QAM support, (1024QAN	256QAM support, (1024QAM with 1~2dB power back-off)
Input Power	-48VDC (-38V	-48VDC (-38VDC to -57VDC)
Power Consumption	1,165 Watt @ 100% RF	1,165 Watt @ 100% RF load, room temperature
Size (WHD)	380 x 380 x 260 mm (1	380 x 380 x 260 mm (14.96 x 14.96 x 10.23 inch)
Volume	37	37.5 L
Weight (W/o Solar Shield & finger quard)	35.9 kg	35.9 kg (79.1 lb)
Operating Temperature	-40°C (-40°F) - 55°C (1	- 55°C (131°F) (Without solar load)
Cooling	Natural	Natural convection
	3GPP 36.104	3GPP 36.104
Unwanted Emission	FCC 47 CFR 27.53 c), f)	FCC 47 CFR 22.917
	•	-69 dBm/100 kHz per path @ 896 ~901MHz
CPRI Cascade	Not su	Not supported
Optic Interface	20km, 2 ports (9.8Gbps x 2), SFP+,	20km, 2 ports (9.8Gbps x 2), SFP+, single mode, Duplex (Option: Bi-di)
RET & TMA interface	AIS	AISG 3.0
Bias-T	4 ports (2 p	4 ports (2 ports per band)
Mounting Options	Pole	Pole, wall
NB-loT	2G8+2IB or 4IB	2SA+2GB or 2GB+2IB or 4GB
PIM Cancellation	Suj	Support
# of antenna port		4
External Alarm		4
Fronthaul Interface	Opt. 8 CPRI / Opt. 7-2x select	Opt. 8 CPRI / Opt. 7-2x selectable (not simultaneous support)
CPRI compression	Not	Not Support

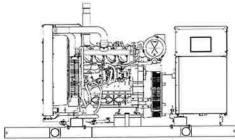


Tier 3 EPA-Certified for Stationary Emergency Applications

Ratings Range

		00 HZ
Standby:	kW	44-52
	kVA	44-65
Prime:	kW	40-47

kVA



40-58

Model with TM Engine shown

Generator Set Ratings

				130°C Rise Standby Rating		105°C Rise Prime Rating		
Alternator	Voltage	Ph	Hz	kW/kVA	Amps	kW/kVA	Amps	
	120/208	3	60	51/63	176	46/57	159	
	127/220	3	60	51/63	167	46/57	150	
	120/240	3	60	49/61	147	44/55	132	
	120/240	1	60	44/44	183	40/40	166	
4P7BX	139/240	3	60	51/63	153	46/57	138	
	220/380	3	60	49/61	93	45/56	85	
	277/480	3	60	51/63	76	46/57	69	
	347/600	3	60	51/63	61	46/57	55	
-	120/208	3	60	52/65	180	47/58	163	
	127/220	3	60	52/65	170	47/58	154	
4P8X	120/240	3	60	50/62	150	45/56	135	
	120/240	1	60	50/50	208	45/45	187	
	139/240	3	60	52/65	156	47/58	141	
	220/380	3	60	52/65	98	47/58	89	
	277/480	3	60	52/65	78	47/58	70	
	347/600	3	60	52/65	62	47/58	56	
	120/208	3	60	52/65	180	47/58	163	
	127/220	3	60	52/65	170	47/58	154	
	120/240	3	60	50/62	150	45/56	135	
	120/240	1	60	50/50	208	45/45	187	
4P10X	139/240	3	60	52/65	156	47/58	141	
	220/380	3	60	52/65	98	47/58	89	
	277/480	3	60	52/65	78	47/58	70	
	347/600	3	60	52/65	62	47/58	56	
4Q7BX	120/240	1	60	48/48	200	43/43	179	
4Q8X	120/240	1	60	50/50	208	45/45	187	
4Q10X	120/240	1	60	50/50	208	45/45	187	

Standard Features

- Kohler Co. provides one-source responsibility for the generating system and accessories.
- The generator set and its components are prototype-tested, factory-built, and production-tested.
- The 60 Hz generator set offers a UL 2200 listing.
- The generator set accepts rated load in one step.
- The 60 Hz generator set meets NFPA 110, Level 1, when equipped with the necessary accessories and installed per NFPA standards.
- The generator set engine is certified to meet the Environmental Protection Agency (EPA) emergency stationary emissions requirements.
- A one-year limited warranty covers all generator set systems and components. Two- and five-year extended limited warranties are also available.
- Alternator features:
 - The unique Fast-Response® X excitation system delivers excellent voltage response and short-circuit capability using a rare-earth, permanent magnet (PM)-excited alternator.
 - The brushless, rotating-field alternator has broadrange reconnectability.
- Other features:
 - Kohler designed controllers for one-source system integration and remote communication. See Controllers on page 3.
 - The low coolant level shutdown prevents overheating (standard on radiator models only).
 - Integral vibration isolation eliminates the need for under-unit vibration spring isolators.
 - The generator set for 49-state applications is equipped with the KDI 3404 TM engine. The generator set that is CARB compliant/California South Coast Air Quality Management District (SCAQMD) pre-certified is equipped with the KDI 3404 TCR engine.

RATINGS: All three-phase units are rated at 0.8 power factor. All single-phase units are rated at 1.0 power factor. Standby Ratings: Standby rating is applicable to varying loads for the duration of a power outage. There is no overload capability for this rating. Prime Power Ratings: At varying load, the number of generator set operating hours is unlimited. A 10% overload capacity is available for one hour in twelve. Ratings are in accordance with ISO-8528-1 and ISO-3046-1. For limited running time and continuous ratings, consult the factory. Obtain the technical information bulletin (TIB-101) for ratings guidelines, complete ratings definitions, and site condition derates. The generator set manufacturer reserves the right to change the design or specifications without notice and without any obligation or liability whatsoever.

Alternator Specifications

Specifications	Alternator
Manufacturer	Kohler
Type	4-Pole, Rotating-Field
Exciter type	Brushless, Rare-Earth Permanent Magnet
Leads: quantity, type	
	12, Reconnectable 4, 110- 120/220- 240 V
Voltage regulator	Solid State, Volts/Hz
Insulation:	NEMA MG1
Material	Class H
Temperature rise	130°C, Standby
Bearing: quantity, type	1, Sealed
Coupling	Flexible Disc
Amortisseur windings	Full
Voltage regulation, no-load to full-load	Controller Dependent
One-step load acceptance	100% of Rating
Unbalanced load capability	100% of Rated Standby Current

- NEMA MG1, IEEE, and ANSI standards compliance for temperature rise and motor starting.
- Sustained short-circuit current of up to 300% of the rated current for up to 10 seconds.
- Sustained short-circuit current enabling downstream circuit breakers to trip without collapsing the alternator field.
- Self-ventilated and dripproof construction.
- Windings are vacuum-impregnated with epoxy varnish for dependability and long life.
- Superior voltage waveform from a two-thirds pitch stator and skewed rotor.

Specifica	tions	Alternator
Peak mot	or starting kVA:	(35% dip for voltages below)
480 V	4P7BX (12 lead)	180
480 V	4P8X (12 lead)	261
480 V	4P10X (12 lead)	275
240 V	4Q7BX (4 lead)	113
240 V	4Q8X (4 lead)	121
240 V	4Q10X (4 lead)	144

Application Data

Engine

Engine		
Engine Specifications	49-State Engine	California SCAQMD
Manufacturer	Kohler	Diesel
	KDI	KDI
Engine model	3404TM	3404TCR
Engine type	4-Cycle, Tu	rbocharged
Cylinder arrangement	4 In	line
Displacement, L (cu. in.)	3.4 (207)
Bore and stroke, mm (in.)	96 x 116 (3	1.28 x 4.57)
Compression ratio	18.5:1	17.0:1
Piston speed, m/min. (ft./min.)	418 (1371)	510 (1673)
Main bearings: quantity, type	5, Replace	able Insert
Rated rpm	18	00
Max. power at rated rpm, kWm (BHP)	64 (86)	70 (94)
Cylinder head material	Cast	Iron
Crankshaft material	Cast	Iron
Valve material:		
Intake	Chromium-	Silicon Steel
Exhaust	Chromit	ım Steel
Governor: type, make/model	Mech. (or Electronic *)	Electronic
	Droop, 5%	
Frequency regulation, no-load to full-load	(or Isochr. *)	Isochronous
Frequency regulation, steady state	±0.5%	±0.28%
Frequency	Fix	æd
Air cleaner type, all models	D	ry
* Requires available electronic governor of	ption	

Exhaust

Exhaust System	49-State Engine	California SCAQMD
Exhaust manifold type	D	ry
Exhaust flow at rated kW, m ³ /min. (cfm)	8.8	(310)
Exhaust temperature at rated kW, dry exhaust, °C (°F)	490 (914)	471 (880)
Minimum/maximum allowable back pressure, kPa (in. Hg)	6 (1.8)/ 9 (2.7)	8 (2.4)/ 13.5 (4.0)
Exhaust outlet size at engine hookup, mm (in.)	63.5	(2.5)

Engine Electrical

Engine Electrical System	49-State Engine	California SCAQMD
Battery charging alternator:		
Ground (negative/positive)	Neg	ative
Volts (DC)	1	12
Ampere rating	9	90
Starter motor rated voltage (DC)	1	2
Battery, recommended cold cranking amps (CCA):		
Quantity, CCA rating	One	, 650
Battery voltage (DC)	1	2

Fuel

401		
Fuel System	49-State Engine	California SCAQMD
Fuel supply line, min. ID, mm (in.)	8.0 (0.31)
Fuel return line, min. ID, mm (in.)	6.0 (0.25)
Max. lift, engine-driven fuel pump, m (ft.)	6.0 (20.0)	3.7 (12.1)
Max. fuel flow, Lph (gph)	46 (12.2)	87.4 (23.1)
Max. return line restriction, kPa (in. Hg)	20 (5.9)	17.7 (5.2)
Fuel filter		
Prefilter	74 M	icrons
Primary/Water Separator	5 Microns @ 98% Efficiency	5 Microns @ 95% Efficiency
Recommended fuel	#2 Ultra Low	Sulfur Diesel

Lubrication

Lubricating System	49-State Engine	California SCAQMD
Туре	Full Pressure	
Oil pan capacity, L (qt.) §	15.3 (16.2)	
Oil pan capacity with filter, L (qt.) §	15.6 (16.5)	
Oil filter: quantity, type §	1, Cartridge	
Oil cooler	Water-Cooled	
\S Kohler recommends the use of Kohler	Genuine oil and	filters.

Application Data

Cooling

Radiator System	49-State Engine	California SCAQMD
Ambient temperature, °C (°F) *	50 (122)
Engine jacket water capacity, L (gal.)	4.5 (1.19)
Radiator system capacity, including engine, L (gal.)	12.3	(3.2)
Engine jacket water flow, Lpm (gpm)	125 (33)	120 (32)
Heat rejected to cooling water at rated kW, dry exhaust, kW (Btu/min.)	37.8 (2207)	41.3 (2352)
Heat rejected to air charge cooler at rated kW, dry exhaust, kW (Btu/min.)	12 (682)	8.4(477)
Water pump type	Centi	rifugal
Fan diameter, including blades, mm (in.)	597 ((23.5)
Fan, kWm (HP)	1.8	(2.3)
Max. restriction of cooling air, intake and discharge side of radiator, kPa (in. $\rm H_2O$)	0.125	5 (0.5)

^{*} Enclosure reduces ambient temperature capability by 5°C (9°F).

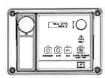
Operation Requirements

Air Requirements	49-State Engine	California SCAQMD
Radiator-cooled cooling air, m³/min. (scfm) †	96.3 ((3400)
Combustion air, m ³ /min. (cfm)	4.8 (170)	4.0 (140)
Heat rejected to ambient air:		
Engine, kW (Btu/min.)	13.2	(750)
Alternator, kW (Btu/min.)	7.6 (435)
Max. air intake restriction, kPa (in. Hg)	5.2 (1.54)	4.2 (1.24)

† Air density = 1.20 kg/m 3 (0.075 lbm/ft 3)

Fuel Consumption	49-State	Engine
Diesel, Lph (gph) at % load	Standby	Rating
100%	17.4	(4.6)
75%	13.2	(3.5)
50%	9.1	(2.4)
25%	5.3	(1.4)
Diesel, Lph (gph) at % load	Prime I	Rating
100%	16.1	(4.2)
75%	12.1	(3.2)
50%	8.3	(2.2)
25%	4.9	(1.3)
Fuel Consumption	Calif. SCAQ	MD Engine
Diesel, Lph (gph) at % load	Standby	Rating
100%	15.2	(4.0)
75%	11.6	(3.1)
50%	8.0	(2.1)
25%	4.6	(1.2)
Diesel, Lph (gph) at % load	Prime I	Rating
100%	12.3	(3.2)
75%	10.6	(2.8)
50%	6.6	(1.7)
3076		

Controllers



APM402 Controller

Provides advanced control, system monitoring, and system diagnostics for optimum performance and compatibility.

- Digital display and menu control provide easy local data access
- Measurements are selectable in metric or English units
- Remote communication thru a PC via network or serial configuration
- Controller supports Modbus® protocol
- Integrated hybrid voltage regulator with ±0.5% regulation
- Built-in alternator thermal overload protection
- NFPA 110 Level 1 capability

Refer to G6-161 for additional controller features and accessories.



(Available with the 49-State generator set only.)

Decision-Maker® 550 Controller

Provides advanced control, system monitoring, and system diagnostics with remote monitoring capabilities.

- Digital display and keypad provide easy local data access
- Measurements are selectable in metric or English units
- Remote communication thru a PC via network or modem configuration
- Controller supports Modbus[®] protocol
- Integrated voltage regulator with ±0.25% regulation
- Built-in alternator thermal overload protection
- NFPA 110 Level 1 capability

Refer to G6-46 for additional controller features and accessories.

Modbus® is a registered trademark of Schneider Electric.



KOHLER CO., Kohler, Wisconsin 53044 USA Phone 920-457-4441, Fax 920-459-1646 For the nearest sales and service outlet in the US and Canada, phone 1-800-544-2444 KOHLERPower.com

Α	Additional Standard Features
•	Air Cleaner, Heavy Duty
•	Alternator Protection
•	Battery Rack and Cables

- Open Crankcase Ventilation
- Oil Drain and Coolant Drain with Hose Barb
- Oil Drain Extension (with narrow skid and enclosure models only)
- Operation and Installation Literature
- Radiator Drain Extension (with enclosure models only)
- Stainless Steel Fasteners on Enclosure (with enclosure models only)

Αv	ailable Options
_	Approvals and Listings CSA Certified IBC Seismic Certification UL2200 Listing
	Enclosed Unit Sound Enclosure (with enclosed critical silencer) Weather Enclosure (with enclosed critical silencer) Stainless Steel Latches and Hinges
	Open Unit Exhaust Silencer, Critical (kit: PA-324470) Flexible Exhaust Connector, Stainless Steel
	Fuel System Flexible Fuel Lines Fuel Pressure Gauge (Available with 49-state engine only) Subbase Fuel Tanks
	Controller 15-Relay Dry Contact (SCAQMD engine with APM402 controller only)
00000	Common Failure Relay (550 controller only) Communication Products and PC Software (550 controller only) Customer Connection (550 controller only) Dry Contact (isolated alarm) (550 controller only) Two Input/Five Output Module (49-state engine with APM402 controller only)
	Key Switch (SCAQMD engine with APM402 controller only)
_	Remote Annunciator Panel Remote Emergency Stop Run Relay
0	Cooling System Block Heater (1000 W, 110-120 V) Required for ambient temperatures below 0°C (32°F). Radiator Duct Flange
000	Electrical System Alternator Strip Heater Battery Battery Charger, Equalize/Float Type Battery Heater Electronic Governor Line Circuit Breaker (NEMA type 1 enclosure)

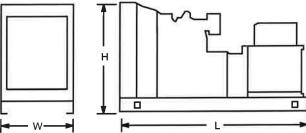
☐ Line Circuit Breaker with Shunt Trip (NEMA type 1 enclosure)

	Air Cleaner Restriction Indicator Engine Fluids Added Rated Power Factor Testing Rodent Guards
0000	Literature General Maintenance NFPA 110 Overhaul Production
	Warranty
	2-Year Basic Limited Warranty 5-Year Basic Limited Warranty 5-Year Comprehensive Limited Warranty

Dimensions and Weights

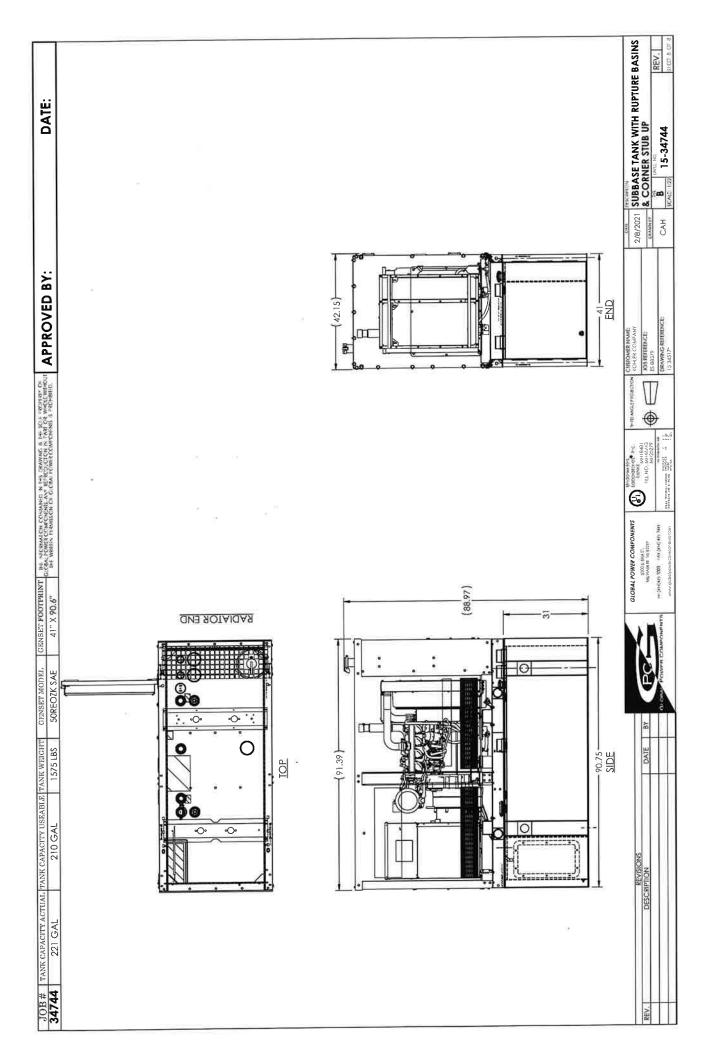
Miscellaneous

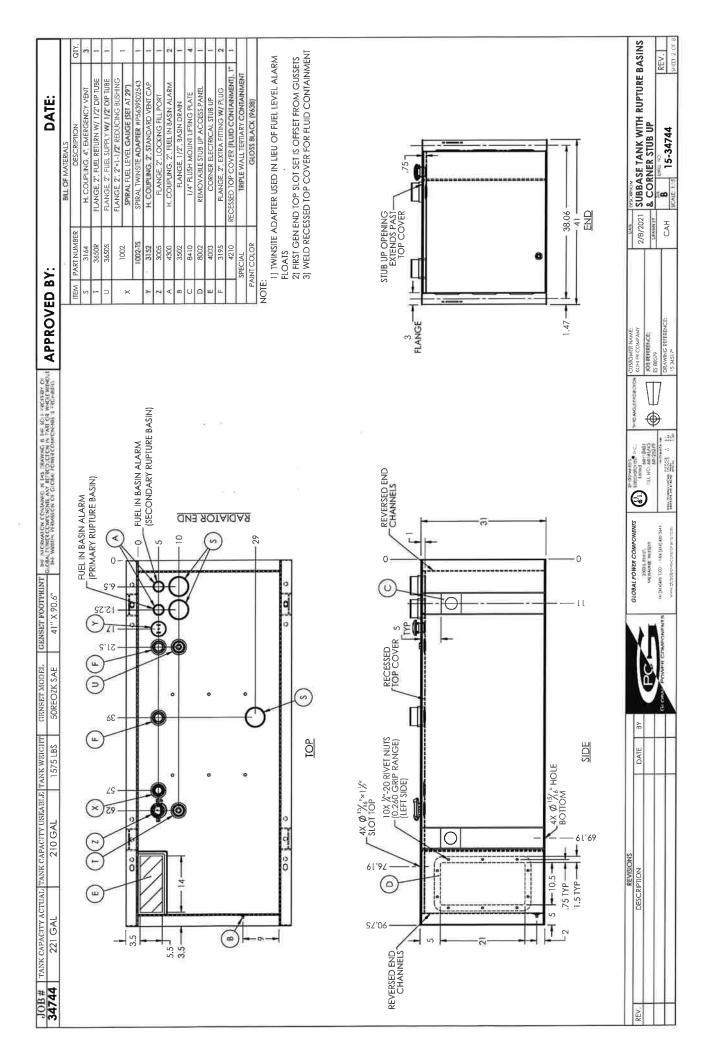
Overall Size, L x W x H, mm (in.): Wide Skid: 2300 x 1040 x 1133 (90.6 x 41.0 x 44.6) 1875 x 780 x 1067 (73.8 x 30.7 x 42.0) g (lb.): 802 (1769) Narrow Skid: Weight (radiator model), wet, kg (lb.):

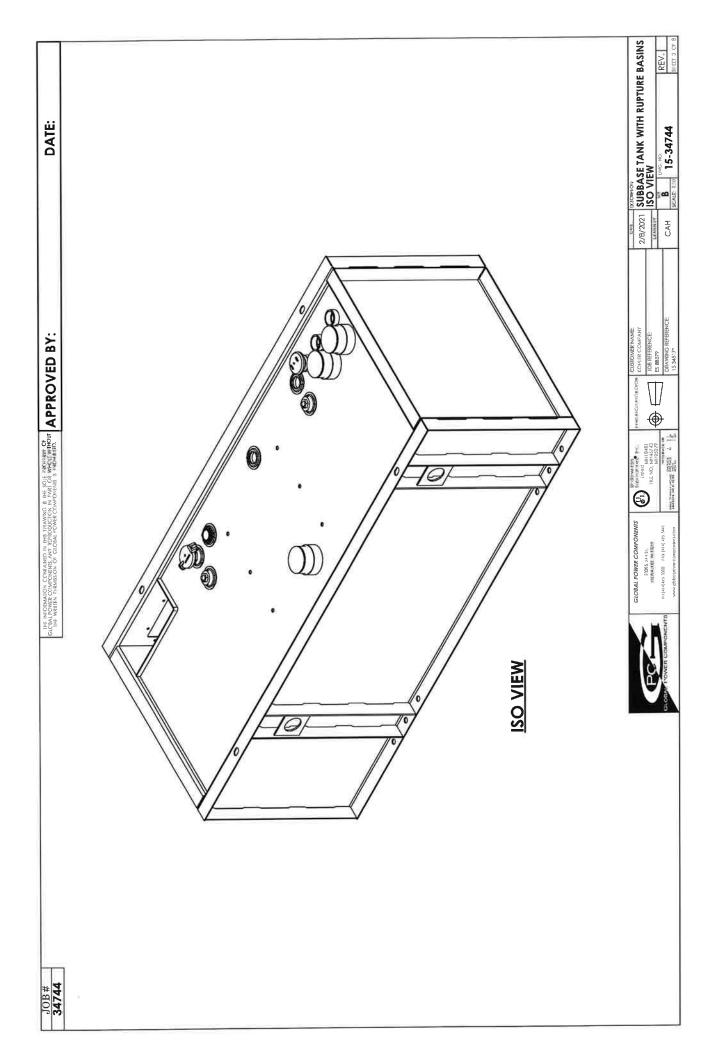


NOTE: This drawing is provided for reference only and should not be used for planning installation. Contact your local distributor for more detailed information.

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ATTACHMENT 5



Centered on Solutions™

Structural Analysis Report

160' Lattice Tower

Verizon Antenna Installation

CSP Tower Ref: #52

194 MT Parnassus Road East Haddam, CT

CENTEK Project No. 23134.00

Date: October 6, 2023

Max Stress Ratio = 61%



Prepared for:

Verizon Wireless 20 Alexander Drive Wallingford, CT 06492

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RFDS

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Introduction

The purpose of this report is to summarize the results of the non-linear, $P-\Delta$ structural analysis of the antenna installation by Verizon on the existing lattice tower located in East Haddam, Connecticut.

The host tower is a 120-ft, three legged, lattice tower originally designed and manufactured by Valmont. File no. 265482 dated June 21, 2016. The tower geometry, structure member sizes and foundation information were taken from the aforementioned design documents.

Antenna and appurtenance inventory was taken from an existing tower inventory provided by the CSP, a field visit conducted by Centek personnel and information provided by Verizon.

The tower consists of six (6) existing and two (2) proposed vertical sections consisting of steel solid round truss legs conforming to ASTM A572-58 and steel angle lateral bracing. The vertical tower sections are connected by bolted flange plates with the diagonal and horizontal bracing to legs consisting of bolted connections. The width of the tower face is 8-ft at the top and 22-ft at the bottom.

Antenna and Appurtenance Summary

The existing and proposed loads considered in the analysis consist of the following:

Antenna Type	Carrier	Mount	Antenna Elevation	Cable
(1) Lightning Rod	Tower (relocated)	Leg Mount	160'	N/A
(4) Samsung MT6413-77A (8) Commscope NHH-45B-R2B (4) Samsung RF4439d-25A (4) Samsung RF4461d-13A (1) OVP-RC2DC-3315-PF-48	Verizon (Proposed)	(4) V-frames	145'	(2) 6x12 Fiber Cables
(1) PA6-65 dish	CSP (existing)	Pipe Mount	120'	(1) WEP65 Cable
(1) PA6-65 dish	CSP (existing)	Pipe Mount	120'	(1) WEP65 Cable
(3) SC479-HF1LDF Omni (1) 432E-83101T TTA	CSP (existing)	10-ft T-Frame	120'	(3) 1-5/8" coax cable (1) 1/2" coax cable
(1) WPA-80080-4CF Panel (2) SC479-HF1LDF Omni (1) ANT450-F6 Omni (1) 432E-83101T TTA	CSP (existing)	10-ft T-Frame	120'	(3) 1-5/8" coax cable (1) 7/8" coax cable (1) 1/2" coax cable
(2) DB-810 Omni	CSP (Future)	Pipe Mount	120'	(2) 1-5/8" coax cable
(1) ANT150F2 Antenna	VS/QV (existing)	6' Side Arm Mount	100'	(1) 7/8" coax cable

Antenna Type	Carrier	Mount	Antenna Elevation	Cable
(1) ANT450F6 Antenna	VS/QV (existing)	6' Side Arm Mount	85'	(1) 7/8" coax cable
(1) SD314 Antenna	VS/QV (existing)	6' Side Arm Mount	85'	(1) 7/8" coax cable
(1) ANT150F2 Antenna	VS/QV (existing)	6' Side Arm Mount	80'	(1) 7/8" coax cable
(1) 531-70 Dipole	DOT (existing)	2' Side Arm Mount	55'	(1) 1/2" coax cable
(1) 531-70 Dipole	East Haddam (existing)	2' Side Arm Mount	55'	(1) 1/2" coax cable
(1) 531-70 Dipole	East Haddam (existing)	2' Side Arm Mount	55'	(1) 1/2" coax cable
(1) ANT150F2 Antenna	East Haddam (existing)	2' Side Arm Mount	25'	(1) 1/2" coax cable
(1) ANT150F2 Antenna (inverted)	VS/QV (existing)	Shared with mount above	20'	(1) 1/2" coax cable

Primary Assumptions Used in the Analysis

- The tower structure's theoretical capacity not including any assessment of the condition of the tower.
- The tower carries the horizontal and vertical loads due to the weight of antennas, ice load and wind.
- Tower is properly installed and maintained.
- Tower is in plumb condition.
- Tower loading for antennas and mounts as listed in this report.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds are fabricated with ER-70S-6 electrodes.
- All members are assumed to be as specified in the original tower design documents.
- All members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
- All member protective coatings are in good condition.
- All tower members were properly designed, detailed, fabricated, installed and have been properly maintained since erection.
- Any deviation from the analyzed antenna loading will require a new analysis for verification of structural adequacy.
- All coax cables should be routed as specified in section 3 of this report.

Analysis

The existing tower was analyzed using a comprehensive computer program entitled tnxTower. The program analyzes the tower, considering the worst case loading condition. The tower is considered as loaded by concentric forces along the tower, and the model assumes that the tower members are subjected to bending, axial, and shear forces.

The existing tower was analyzed for the controlling basic wind speed with no ice and the applicable wind and ice combination to determine stresses in members as per guidelines of TIA-222-H entitled "Structural Standard for Antenna Support Structures, Antennas and Small Wind Turbine Support Structures", the American Institute of Steel Construction (AISC) and the Manual of Steel Construction; Load and Resistance Factor Design (LRFD).

The controlling wind speed is determined by evaluating the local available wind speed data as provided in Appendix P of the CSBC¹ and the wind speed data available in the TIA-222-H Standard.

Tower Loading

Tower loading was determined by the basic wind speed as applied to projected surface areas with modification factors per TIA-222-H, gravity loads of the tower structure and its components, and the application of 1.0" radial ice on the tower structure and its components.

Load Cases:

Load Case 1; 135 mph (Risk Cat IV) wind speed w/ no ice plus gravity load – used in calculation of tower stresses and rotation

Load Case 2; 50 mph wind speed w/ 1.00" radial ice plus gravity load – used in calculation of tower stresses.

Load Case 3; 90 mph wind speed w/ 0.5" radial ice plus gravity load – used in calculation of tower twist and sway.

[Appendix P of the 2022 CT Building Code]

[Annex B of TIA-222-H]

[TIA-222-F used for calculation of tower twist and sway per the requirements of the CSP]

The 2021 International Building Code as amended by the 2022 Connecticut State Building Code (CSBC).

Tower Capacity

Calculated stresses were found to be within allowable limits.

Tower Section	Elevation	Stress Ratio (percentage of capacity)	Result
Leg (T1)	140.0' - 160.0'	60.9%	PASS
Diagonal (T4)	80.0' - 100.0'	52.5%	PASS
Top Girt (T3)	100.0' - 120.0'	12.1%	PASS

The tower combined deflection was found to be within allowable limits.

Deflection Criteria	Proposed (degrees)	Allowable (degrees)	Result
Sway (Tilt)	0.4012	n/a	n/a
Twist	0.1476	n/a	n/a
Combined	0.5488	0.75	PASS

TIA-222-F standard used for calculation of tower twist and sway per the requirements of the CSP.

Foundation and Anchors

The existing foundation consists of a (3) 4.0-ft diameter x 3.25-ft long reinforced concrete piers supported on a 36-ft square x 2.25-ft thick mat. The base of the tower is connected to the foundation by means of (12) 1.25" \varnothing anchor bolts per leg embedded into the concrete foundation structure.

The tower reactions developed from the governing Load Case were used in the verification of the foundation and anchor bolts:

Load Effect	Proposed Tower Reactions
Leg Shear	30 kips
Leg Compression	254 kips
Leg Tension	214 kips
Base Moment	4,528 ft-kips
Base Shear	49 kips

The anchor bolts were found to be within allowable limits.

Tower Section	Component	Stress Ratio (percentage of capacity)	Result
Anchor Bolts	Combined Tension and Shear	18.5%	PASS

The foundation was found to be within allowable limits.

Foundation	Design Limit	(percentage of capacity)	Result
Reinforced Concrete	Overturning	31%	PASS
Pad and Piers	Bearing	22%	PASS

Conclusion

This analysis shows that the subject tower <u>is adequate</u> to support the proposed antenna configuration.

The analysis is based, in part, on the information provided to this office by Verizon and the CSP. If the existing conditions are different than the information in this report, Centek Engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:

Timothy J. Lynn, PE Structural Engineer

REPORT

Standard Conditions for Furnishing of Professional Engineering Services on Existing Structures

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

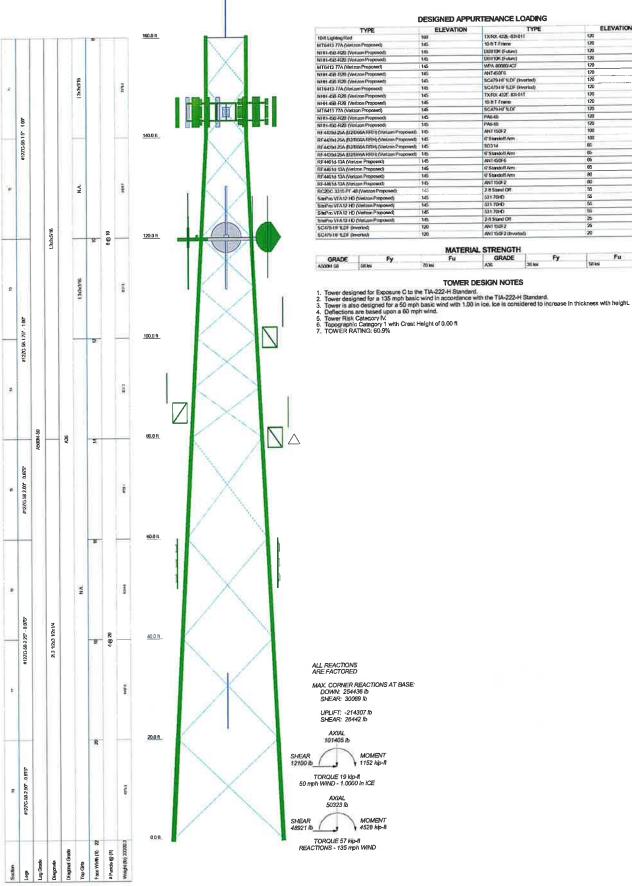
- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of Centek Engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to Centek Engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an uncorroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the "as new" condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222
- All services performed, results obtained, and recommendations made are in accordance
 with generally accepted engineering principles and practices. Centek Engineering, Inc.
 is not responsible for the conclusions, opinions and recommendations made by others
 based on the information we supply.

<u>GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM</u>

tnxTower, is an integrated structural analysis and design software package for Designed specifically for the telecommunications industry, tnxTower, formerly RISA Tower, automates much of the tower analysis and design required by the TIA/EIA 222 Standard.

tnxTower Features:

- tnxTower can analyze and design 3- and 4-sided guyed towers, 3- and 4-sided selfsupporting towers and either round or tapered ground mounted poles with or without guys.
- The program analyzes towers using the TIA-222-H standard or any of the previous TIA/EIA standards back to RS-222 (1959). Steel design is checked using the AISC ASD or the AISC LRFD specifications.
- Linear and non-linear (P-delta) analyses can be used in determining displacements and forces in the structure. Wind pressures and forces are automatically calculated.
- Extensive graphics plots include material take-off, shear-moment, leg compression, displacement, twist, feed line, guy anchor and stress plots.
- tnxTower contains unique features such as True Cable behavior, hog rod take-up, foundation stiffness and much more.



DESIGNED APPURTENANCE LOADING ELEVATION ELEVATION SO IT Frame
DBISSE Grober)
DBISSE Grober)
DBISSE Grober)
WASH BODD GF
SCATPARE SLIF (Invested)
SCATPARE SLIF (Invested)
TXSIS ADDI. SBIDT
TXSIS ADDI. SBIDT 10 ft T. Frene 120 130 120 120 SCI314
6' Standolf Ann
ANT 450F6
6' Standolf Ann
6' Standolf Ann
6' Standolf Ann
ANT 150F2
2 If Stand Off
501 70F0
501 70F0
2 If Stand Off
ANT 150F2

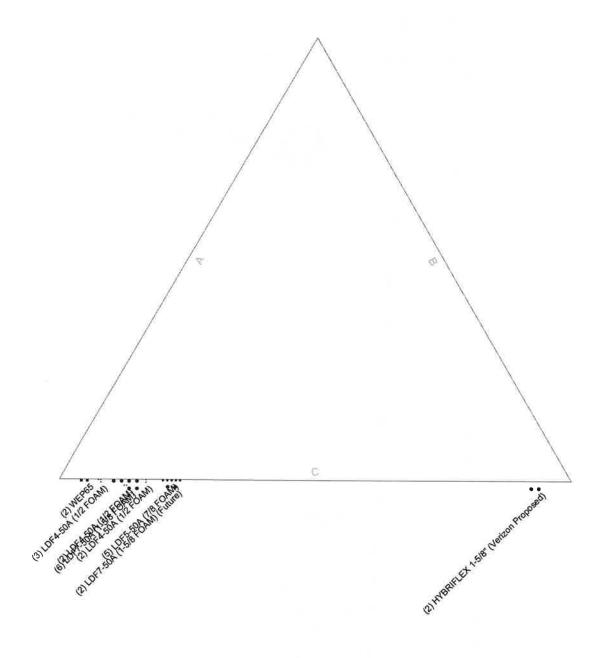
MATERIAL STRENGTH						
GRADE	Fy	Fu	GRADE	Fy	Fa	
A500M 58	deltes	70 tei	AM	36 tol	TAT And	
A500M 58	29194	70 lei	A%	36 KK	1 191 ete	

TOWER DESIGN NOTES

* 23134.00 - East Haddam Centek Engineering Inc. Present 160-ft Lattice Tower (CSP #52)
Cheek Verizon Drawn by: T.J.L. 63-2 North Branford Rd. Branford, CT 06405 Date: 10/09/23 Scale NTS Phone: (203) 488-0580 FAX: (203) 488-8587

Feed Line Plan

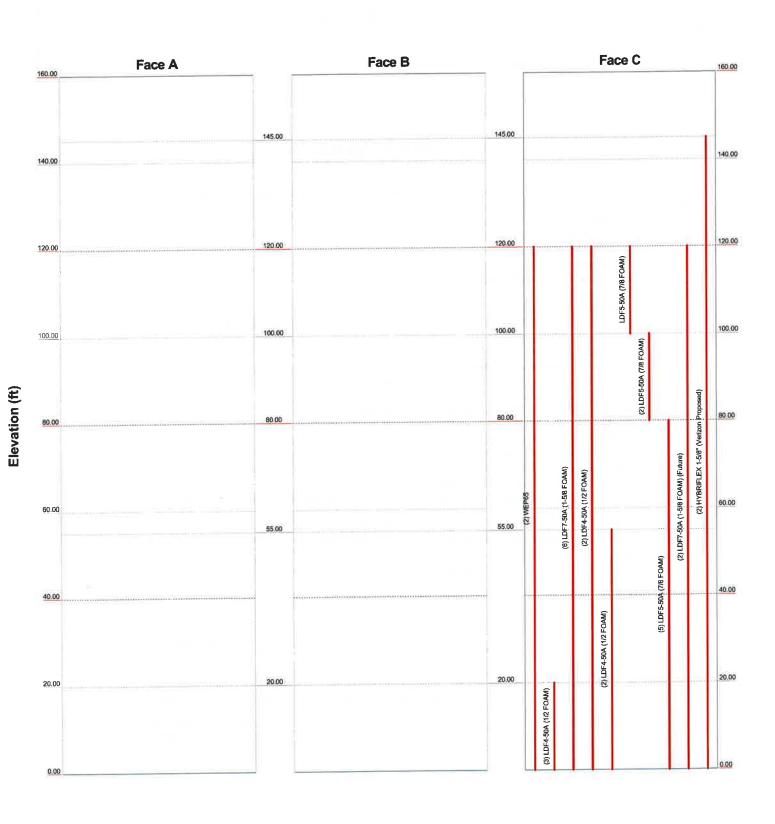
Round Flat App In Face App Out Face Truss-Leg



^{ab:} 23134.00 - East Haddam				
Project: 160-ft Lattice	Tower (CSP #52)			
Client: Verizon	Drawn by: TJL	App'd:		
Code: TIA-222-H	Date: 10/06/23	Scale: NTS		
Path:		Dwg No. E-7		

Feed Line Distribution Chart 0' - 160'

Round _____ Flat ____ App in Face ____ App Out Face ____ Truss Leg



Centek Engineering Inc. [505: 23134.00 - East Haddam					
63-2 North Branford Rd.		Project: 160-ft Lattice Tower (CSP #52)			
Branford, CT 06405	Client: Verizon	Drawn by: TJL	App'd:		
Phone: (203) 488-0580	Code: TIA-222-H	Date: 10/06/23	Scale: NTS		
FAX: (203) 488-8587	Path:		Dwg No. E-7		

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Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 160.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 8.00 ft at the top and 22.00 ft at the base.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower base elevation above sea level: 0.00 ft.

Basic wind speed of 135 mph.

Risk Category IV.

Exposure Category C.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0.00 ft.

Nominal ice thickness of 1,0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- Use Code Stress Ratios
- Use Code Safety Factors Guys Escalate Ice Always Use Max Kz Use Special Wind Profile
- Include Bolts In Member Capacity Leg Bolts Are At Top Of Section
- Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided)
- SR Members Have Cut Ends SR Members Are Concentric

- Distribute Leg Loads As Uniform Assume Legs Pinned
- Assume Rigid Index Plate
- Use Clear Spans For Wind Area
- Use Clear Spans For KL/r Retension Guys To Initial Tension
- Bypass Mast Stability Checks Use Azimuth Dish Coefficients Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination
- Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs

- Use ASCE 10 X-Brace Ly Rules
- Calculate Redundant Bracing Forces Ignore Redundant Members in FEA
- SR Leg Bolts Resist Compression
- All Leg Panels Have Same Allowable Offset Girt At Foundation
- Consider Feed Line Torque
- Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption Poles

Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances

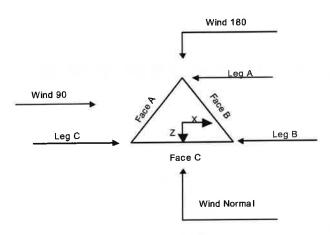
Outside and Inside Corner Radii Are

Known

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Triangular Tower

Tower	Section	Geometry
--------------	---------	----------

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of	Section Length
Section					Sections	
	ft			ft		ft
T1	160.00-140.00			8.00	1	20.00
T2	140.00-120.00			8.00	1	20.00
T3	120.00-100.00			10.00	1	20.00
T4	100.00-80.00			12.00	1	20.00
T5	80.00-60.00			14.00	1	20.00
T6	60.00-40.00			16.00	1	20.00
	40.00-20.00			18.00	1	20.00
T7 T8	20.00-0.00			20.00	1	20.00

Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Gir
Section	Elevation	Spacing	Туре	K Brace	Horizontals	Offset	Offset
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,				End			
	ft	ft		Panels		in	in
T1	160.00-140.00	10.00	X Brace	No	No	0.0000	0.0000
T2	140.00-120.00	10.00	X Brace	No	No	0.0000	0.0000
T3	120.00-100.00	10.00	X Brace	No	No	0.0000	0.0000
T4	100.00-80.00	10.00	X Brace	No	Yes	0.0000	0.0000
T5	80.00-60.00	20.00	X Brace	No	No	0.0000	0.0000
T6	60.00-40.00	20.00	X Brace	No	No	0.0000	0.0000
T7	40.00-20.00	20.00	Х Втасе	No	No	0.0000	0.0000

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Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Gir
Section	Elevation	Spacing	Type	K Brace End	Horizontals	Offset	Offset
	ft	fi		Panels		in	în
T8	20.00-0.00	20.00	X Brace	No	No	0.0000	0.0000

Tower Section (Geometry ((cont'd)
-----------------	------------	----------

Tower	Leg	Leg	Leg	Diagonal	Diagonal	Diagonal
Elevation ft	Туре	Size	Grade	Туре	Size	Grade
Γ1 160.00-140.00	Truss Leg	#12ZG-58-1.5" - 1.00"	A500M-58 (58 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)
Γ2 140.00-120.00	Truss Leg	#12ZG-58-1.5" - 1.00"	A500M-58 (58 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)
ТЗ 120.00-100.00	Truss Leg	#12ZG-58-1.75" - 1.00"	A500M-58 (58 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)
T4 100.00-80.00	Truss Leg	#12ZG-58-1.75" - 1.00"	A500M-58 (58 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)
T5 80.00-60.00	Truss Leg	#12ZG-58-2.00" - 0.875"	A500M-58 (58 ksi)	Double Angle	2L3 1/2x3 1/2x1/4	A36 (36 ksi)
T6 60.00-40.00	Truss Leg	#12ZG-58-2.25" - 0.875"	A500M-58 (58 ksi)	Double Angle	2L3 1/2x3 1/2x1/4	A36 (36 ksi)
T7 40.00-20.00	Truss Leg	#12ZG-58-2.25" - 0.875"	A500M-58 (58 ksi)	Double Angle	2L3 1/2x3 1/2x1/4	A36 (36 ksi)
T8 20.00-0.00	Truss Leg	#12ZG-58-2.50" - 0.875"	A500M-58 (58 ksi)	Double Angle	2L3 1/2x3 1/2x1/4	A36 (36 ksi)

	10.00	
Tower Section	n Geometry	(cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 160.00-140.00	Equal Angle	L3x3x3/16	A36	Solid Round		A36
			(36 ksi)			(36 ksi)
T3 120.00-100.00	Equal Angle	L3x3x3/16	A36	Solid Round		A36
			(36 ksi)			(36 ksi)

Tower	Gusset	Gusset	Gusset Grade	Adjust, Factor	Adjust.	Weight Mult.	Double Angle	Double Angle	Double Angle
Elevation	Area	Thickness		A_f	Factor	Ü	Stitch Bolt	Stitch Bolt	Stitch Bolt
	(per face)				A_r		Spacing	Spacing	Spacing
	27						Diagonals	Horizontals	Redundants
ft	fr	in					in	in	in
T1	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
160.00-140.00			(36 ksi)						
T2	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
140.00-120.00			(36 ksi)						
T3	0.00	0.0000	A36	1	1	1	36.0000	36,0000	36.0000
120.00-100.00			(36 ksi)						

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Tower	Gusset	Gusset	Gusset Grade	Adjust. Factor	Adjust. Factor	Weight Mult.	Double Angle Stitch Bolt	Double Angle Stitch Bolt	Double Angle Stitch Bolt
Elevation	Area	Thickness		A_f	A_r		Spacing	Spacing	Spacing
	(per face)				117		Diagonals	Horizontals	Redundants
fi	fr ²	în					in	in	in
T4	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
100.00-80.00			(36 ksi)						
T5 80.00-60.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
			(36 ksi)		•		24,0000	26,0000	36.0000
T6 60.00-40.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000
			(36 ksi)	92	20	Sign Control	3 (0000	36.0000	36.0000
T7 40.00-20.00	0.00	0.0000	A36	,1	1	1	36.0000	30.0000	30.0000
			(36 ksi)	24	40		26,0000	36.0000	36.0000
T8 20.00-0.00	0.00	0.0000	A36	1	19	1	36.0000	30.0000	30.0000
			(36 ksi)						

Tower Section Geometry (cont'd)

						K Fac	ctors			
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner B r ace
ft	Angles	Rounds		X Y	X Y	X Y	X Y	X Y	X Y	X Y
T1	Yes	Yes	1	1	1	1	1	1	1	1
160.00-140.00				1	1	1	1	1	1	1
T2	Yes	Yes	1	1	1	1	1	1	1	1
140.00-120.00				1	1	1	1	1	1	1
Т3	Yes	Yes	1	1	1	1	1	1	1	1
120.00-100.00				1	1	1	1	1	1	1
T4	Yes	Yes	1	1	1	1	1	1	1	1
100.00-80.00				1	1	1	1	1	1	1
T5	Yes	Yes	1	1	1	1	1	1	1	1
80.00-60.00				1	1	1	1	1	1	1
T6	Yes	Yes	1	1	1	1	1	1	1	1
60.00-40.00	2 20	- 45		1	1	1	1	1	1	1
T7	Yes	Yes	1	1	1	1	1	1	1	1
40.00-20.00	- 50		.0	ī	1	1	1	1	1	1
T8 20.00-0.00	Yes	Yes	1	î	1	1	1	1	1	1
10 20.00-0.00	1 63	1 05		ī	1	1	1	1	1	1

Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

	Truss-Leg K Factors										
-	Trus	s-Legs Used As Leg Me	mbers	Truss-Legs Used As Inner Members							
Tower Elevation ft	Leg Panels	X Brace Diagonals	Z Brace Diagonals	Leg Panels	X Brace Diagonals	Z Brace Diagonals					
T1	1	0.5	0.85	I,	0.5	0.85					
160.00-140.00 T2	1	0.5	0.85	1	0.5	0.85					
140.00-120.00 T3 120.00-100.00	1	0.5	0.85	<u>î</u>	0.5	0.85					

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			Truss-Leg	K Factors				
	Trus	s-Legs Used As Leg Me	mbers	Truss-Legs Used As Inner Members				
Tower Elevation ft	Leg Panels	X Brace Diagonals	Z Brace Diagonals	Leg Panels	X Brace Diagonals	Z Brace Diagonals		
T4 100.00-80.00	1	0.5	0.85	1	0.5	0.85		
T5 80.00-60.00	1	0.5	0.85	1	0.5	0.85		
T6 60.00-40.00	1	0.5	0.85	1	0.5	0.85		
T7 40.00-20.00	1	0.5	0.85	1	0.5	0.85		
T8 20.00-0.00	1	0.5	0.85	1	0.5	0.85		

Tower Elevation ft	Leg Diagonal		nal	Тор G	Top Girt Bottom Girt			Mid Girt		Long Ho	rizontal	Short Ho	rizontal	
	Net Width	U	Net Width	U	Net Width	U	Net	U	Net	\overline{U}	Net	U	Net	U
	Deduct		Deduct		Deduct		Width		Width		Width		Width	
	in		in		in		Deduct		Deduct		Deduct		Deduct	
							in		in		in		in	
T1	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
160.00-140.00														
T2	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
140.00-120.00														
T3	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
120.00-100.00														
T4	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
100.00-80.00														
T5 80.00-60.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 60.00-40.00		1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 40.00-20.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 20.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

Tower	Reduna		Reduna	lant	Reduna	lant	Redun	dant	Redundan	t Vertical	Redundo	ant Hip	Redunda	ınt Hip
Elevation	Horizo	ntal	Diago	nal	Sub-Diagonal		Sub-Hor	izontal					Diagonal	
ft														
	Net Width	U	Net Width	U	Net Width	U	Net	U	Net	U	Net	U	Net	U
	Deduct		Deduct		Deduct		Width		Width		Width		Width	
	in		in		in		Deduct		Deduct		Deduct		Deduct	
							in		in		in		in	
T1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
160.00-140.00			1											
T2	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
140.00-120.00			1											
T3	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
120.00-100.00														
T4	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
100.00-80.00					1									
T5 80.00-60.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 60.00-40.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 40.00-20.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 20.00-0.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

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Tower Section Geometry (cont'd)

Tower Elevation	Leg Connection	Leg		Diagonal		Top G	Top Girt		Bottom Girt		Mid Girt		zontal	Short Hor	izonta
ft	Туре	Bolt Size	No.	Bolt Size	No.	Bolt Size	No	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size in	No.
T1 160.00-140.00	Flange	1.0000 A325N	6	1.0000 A325N	1	1.0000 A325N	I.	0.0000 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T2	Flange	1.0000	6	1.0000 A325N	1	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
140.00-120.00 T3	Flange	A325N 1.2500	6	1.0000	1	1.0000	1	0.6250	0	0.6250	0	0.6250 A325N	0	0.6250 A325N	0
120.00-100.00 T4	Flange	A325N 1.2500	6	A325N 1.0000	1	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0	0.6250	0	0.6250 A325N	0
100.00-80.00 T5 80.00-60.00	Flange	A325N 1.0000	12	A325N 0.8750	1	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0	0.6250	0
T6 60.00-40.00	Flange	A325N 1.0000	12	A325N 0.8750	1	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0
T7 40.00-20.00	J	A325N 1.0000	12	A325N 0.8750	1	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0
	_	A325N 1.2500	12	A325N 0.8750	1	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0	A325N 0.6250	0
T8 20.00-0.00	Flange	F1554-105	12	A325N	1	A325N	U	A325N		A325N		A325N		A325N	

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or	Allow Shield	Exclude From	Component Type	Placement	Face Offset	Lateral Offset	#	# Per		Width or Diameter	Perimeter	Weight
	Leg		Torque Calculation		,ft	in	(Frac FW)		Row	in	in	in	plf ———
WEP65	С	No	No	Ar (CaAa)	120.00 - 0.00	0.0000	0.45	2	2	1.5836	1.5836		0.53
LDF4-50A (1/2 FOAM)	C	No	No	Ar (CaAa)	20.00 - 0.00	0.0000	0.42	3	2	0.6300	0.6300		0.15
LDF7-50A 1-5/8 FOAM)	C	No	No	Ar (CaAa)	120.00 - 0.00	0.0000	0.37	6	4	1.9800	1.9800		0.82
LDF4-50A (1/2 FOAM)	C	No	No	Аг (СаАа)	120.00 - 0.00	3.0000	0.37	2	2	0.6300	0.6300		0.15
LDF4-50A	С	No	No	Ar (CaAa)	55.00 - 0.00	0.0000	0.33	2	1	0.6300	0.6300		0.15
(1/2 FOAM) LDF5-50A	C	No	No	Ar (CaAa)	120.00 - 100.00	0.0000	0.28	1	1	1.0900	1.0900		0.33
(7/8 FOAM) LDF5-50A (7/8 FOAM)	С	No	No	Ar (CaAa)	100.00 - 80.00	0.0000	0.28	2	2	1.0900	1.0900		0.33
LDF5-50A (7/8 FOAM)	С	No	No	Ar (CaAa)	80.00 - 0.00	0.0000	0.28	5	5	1.0900	1.0900		0.33
LDF7-50A 1-5/8 FOAM)	С	No	No	Ar (CaAa)	120.00 - 0.00	3.0000	0.28	2	2	1.9800	1.9800		0.82
(Future) HYBRIFLEX 1-5/8"	С	No	No	Ar (CaAa)	145.00 - 0.00	3.0000	-0.43	2	2	1.9800	1.9800		1.90
(Verizon Proposed)													

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Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation	Face	A_R	A_F	$C_A A_A$ In Face	C _A A _A Out Face	Weight
	ft		ft²	ft²	fr ²	fr	lb
T1	160.00-140.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	1.980	0.000	19.00
T2	140.00-120.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	7.920	0.000	76.00
T3	120.00-100.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	50.634	0.000	241.00
T4	100.00-80.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	52.814	0.000	247.60
T5	80.00-60.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	59.354	0.000	267.40
T6	60.00-40.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	61.244	0.000	271.90
T7	40.00-20.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	61.874	0.000	273.40
T8	20.00-0.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	65.654	0.000	282.40

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	Ü
	ft	Leg	in_	ft ²	ft ²	fr ²	ft ²	lb
T1	160.00-140.00	A	1.454	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	6.318	0.000	81.17
T2	140.00-120.00	Α	1.434	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	25.133	0.000	320.94
Т3	120.00-100.00	Α	1.410	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	141.354	0.000	1807.16
T4	100.00-80.00	Α	1.382	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	150.477	0.000	1842.00
T5	80.00-60.00	Α	1.348	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	164.072	0.000	1995.16
T6	60.00-40.00	Α	1.303	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	172.661	0.000	2039.33
T7	40.00-20.00	Α	1.238	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	173.058	0.000	1985.95
T8	20.00-0.00	Α	1.109	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00

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Tower	Tower	Face	Ice	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness	- 20		In Face	Out Face	
	ft	Leg	in	ft²	fr	fr	fr	lb
		С		0.000	0.000	181.514	0.000	1924.50

Feed Line Center of Pressure

Section	Elevation	CP_X	CP_Z	CP_X Ice	CP _Z Ice
	ft	in	in	in	in
TI	160.00-140.00	0.6699	0.5517	0.9532	0.7526
T2	140.00-120.00	2.8900	2.3325	4.5043	3.4977
T3	120.00-100.00	-8.2568	11.4611	-10.7031	15.4270
T4	100.00-80.00	-10.0405	13.6336	-13.3877	18.6728
T5	80.00-60.00	-12.8138	16.7579	-16.7707	22.5786
Т6	60.00-40.00	-14.3559	18,4617	-19.6391	25.5818
T7	40.00-20.00	-15.7617	20.1512	-21.7180	27.9884
T8	20.00-0.00	-18.1255	22.1717	-25.8003	31.1967

Shielding Factor Ka

Tower	Feed Line	Description	Feed Line	Ka	K_a
Section	Record No.		Segment Elev.	No Ice	Ice
T1	10	HYBRIFLEX 1-5/8"	140.00 -	0.6000	0.4922
			145.00	55000×5000×5	
T2	10	HYBRIFLEX 1-5/8"	120.00 -	0.6000	0.551
			140.00		0.500
T3	1	WEP65	100.00 -	0.6000	0.593
			120.00	0.4000	0.000
T3	3	LDF7-50A (1-5/8 FOAM)	100.00 -	0.6000	0.5930
			120.00	0.0000	0.5930
T3	4	LDF4-50A (1/2 FOAM)	100.00 -	0.6000	0.593
			120.00	0.6000	0.593
T3	6	LDF5-50A (7/8 FOAM)	100.00 -	0.6000	0.595
			120.00	0.6000	0.593
Т3	9	LDF7-50A (1-5/8 FOAM)	100.00 - 120.00	0.0000	0.393
		107001ELEX 1 6/09	100.00 -	0.6000	0.593
Т3	10	HYBRIFLEX 1-5/8"	120.00	0.0000	0.555
	,	WED65	80.00 - 100.00	0.6000	0.600
T4	3	LDF7-50A (1-5/8 FOAM)		0.6000	0.600
T4 T4	3 4	LDF4-50A (1/2 FOAM)	80.00 - 100.00	0.6000	0.600
T4	7	LDF5-50A (7/8 FOAM)	80.00 - 100.00	0.6000	0.600
T4	9	LDF7-50A (1-5/8 FOAM)	80.00 - 100.00	0.6000	0.600
T4	10	HYBRIFLEX 1-5/8"		0.6000	0.600
T5	10	WEP65	60.00 - 80.00	0.6000	0.600
T5	3	LDF7-50A (1-5/8 FOAM)		0.6000	0.600
T5	4	LDF4-50A (1/2 FOAM)	4 1	0.6000	0.600
T5	8	LDF5-50A (7/8 FOAM)	1	0.6000	0.600
T5	9	LDF7-50A (1-5/8 FOAM)		0.6000	0.600
T5	10	HYBRIFLEX 1-5/8"	60.00 - 80.00	0.6000	0.600
T6	1	WEP65	40.00 - 60.00	0.6000	0.600
T6	3	LDF7-50A (1-5/8 FOAM)		0.6000	0.600
T6		LDF4-50A (1/2 FOAM)		0.6000	0.600

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Tower	Feed Line	Description	Feed Line	Ka	K_a
Section	Record No.		Segment Elev.	No Ice	Ice
Т6	5	LDF4-50A (1/2 FOAM)	40.00 - 55.00	0.6000	0.6000
T6	8	LDF5-50A (7/8 FOAM)	40.00 - 60.00	0.6000	0.6000
T6	9	LDF7-50A (1-5/8 FOAM)	40.00 - 60.00	0.6000	0.6000
Т6	10	HYBRIFLEX 1-5/8"	40.00 - 60.00	0.6000	0.6000
T7	1	WEP65	20.00 - 40.00	0.6000	0.6000
T7	3	LDF7-50A (1-5/8 FOAM)	20.00 - 40.00	0.6000	0.6000
T7	4	LDF4-50A (1/2 FOAM)	20.00 - 40.00	0.6000	0.6000
T7	5	LDF4-50A (1/2 FOAM)	20.00 - 40.00	0.6000	0.6000
T7	8	LDF5-50A (7/8 FOAM)	20.00 - 40.00	0.6000	0.6000
T7	9	LDF7-50A (1-5/8 FOAM)	20.00 - 40.00	0.6000	0.6000
T7	10	HYBRIFLEX 1-5/8"	20.00 - 40.00	0.6000	0.6000
T8	1	WEP65	0.00 - 20.00	0.6000	0.6000
T8	2	LDF4-50A (1/2 FOAM)	0.00 - 20.00	0.6000	0.6000
T8	3	LDF7-50A (1-5/8 FOAM)	0.00 - 20.00	0.6000	0.6000
Т8	4	LDF4-50A (1/2 FOAM)	0.00 - 20.00	0.6000	0.6000
T8	5	LDF4-50A (1/2 FOAM)	0.00 - 20.00	0.6000	0.6000
Т8	8	LDF5-50A (7/8 FOAM)	0.00 - 20.00	0.6000	0.6000
Т8	9	LDF7-50A (1-5/8 FOAM)	0.00 - 20.00	0.6000	0.6000
T8	10	HYBRIFLEX 1-5/8"	0.00 - 20.00	0.6000	0.6000

Discrete Tower Loads

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
	Leg		Lateral				2.011	0.120	
			Vert						
			ft	0.	ft		ft²	fr^2	lb
			ft				-	v	
			ft						
SC479-HF1LDF	Α	From Leg	3.00	0.0000	120.00	No Ice	4.37	4.37	34.00
			0.00			1/2" Ice	6.54	6.54	69.82
			3.00			l" Ice	8.04	8.04	114.98
SC479-HF1LDF	Α	From Leg	3.00	0.0000	120.00	No Ice	4.37	4.37	34.00
(Inverted)			0.00			1/2" Ice	6.54	6.54	69.82
			-3.00			1" Ice	8.04	8.04	114.98
SC479-HF1LDF	Α	From Leg	3.00	0.0000	120.00	No Ice	4.37	4.37	34.00
(Inverted)			0.00			1/2" Ice	6.54	6.54	69.82
			-3.00			1" Ice	8.04	8.04	114.98
TX/RX 432E-83I-01T	Α	From Leg	1.00	0.0000	120.00	No Ice	1.20	0.75	25.00
			0.00			1/2" Ice	1.34	0.86	36.55
			0.00			1" Ice	1.48	0.98	50.34
10-ft T-Frame	Α	From Leg	2.00	0.0000	120.00	No Ice	13.60	13.60	378.00
			0.00			1/2" Ice	17.50	17.50	530.00
			0.00			1" Ice	21.40	21.40	682.00
SD314	Α	From Leg	6.00	0.0000	85.00	No Ice	3.17	3.17	21.00
			0.00			1/2" Ice	5.02	5.02	44.00
			5.00			l" Ice	6.87	6.87	67.00
6' Standoff Arm	Α	From Leg	3.00	0.0000	85.00	No Ice	2.40	0.13	50.00
			0.00			1/2" Ice	2.83	0.18	71.81
			0.00			1" Ice	3.26	0.24	99.00
531-70HD	Α	From Leg	2.00	0.0000	55.00	No Ice	6.00	6.00	40.00
			0.00			1/2" Ice	6.90	6.90	50.00
			0.00			1" Ice	7.80	7.80	60.00
2-ft Stand Off	Α	From Leg	1.00	0.0000	55.00	No Ice	1.07	1.07	20.00
		_	0.00			1/2" Ice	1.62	1.62	28.00

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Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_A A_A$ Front	$C_A A_A$ Side	Weig
	Leg	-JF -	Lateral Vert	,					
			ft ft	۰	ft		ft²	ft²	lb
			ft						26.0
			0.00			1" Ice	2.17	2.17	36.0
ANT150F2	Α	From Leg	2.00	0.0000	25.00	No Ice 1/2" Ice	1.30 1.60	1.30 1.60	16.0 20.8
			0.00			1" Ice	1.90	1.90	25.6
13/21/50/20		E-am I on	5.00 2.00	0.0000	20.00	No Ice	1.30	1.30	16.0
ANT150F2	Α	From Leg	0.00	0.0000	20.00	1/2" Ice	1.60	1.60	20.8
(Inverted)			5.00			1" Ice	1.90	1.90	25.6
2-ft Stand Off	Α	From Leg	1.00	0.0000	25.00	No Ice	1.07	1.07	20.0
Z A Dizila O II			0.00			1/2" Ice	1.62	1.62	28.0
			0.00			1" Ice	2.17	2.17	36.0
DB810K	В	From Leg	2.00	0.0000	120.00	No Ice	4.08	4.08	35.0
(Future)			3.00			1/2" Ice	5.73	5.73	65.1 105.
	_	_ *	0.00	0.0000	120.00	1" Ice No Ice	7.41 4.08	7.41 4.08	35.0
DB810K	В	From Leg	2.00	0.0000	120.00	1/2" Ice	5.73	5.73	65.1
(Future)			-3.00 0.00			1" Ice	7.41	7.41	105.
ANIT1 SOE2	В	From Leg	6.00	0.0000	100.00	No Ice	1.30	1.30	16.0
ANT150F2	ь	TIOM Log	0.00	0.0000		1/2" Ice	1.60	1.60	20.8
			5.00			1" Ice	1.90	1.90	25.6
6' Standoff Arm	В	From Leg	3.00	0.0000	100.00	No Ice	2.40	0.13	50.0
• •,			0.00			1/2" Ice	2.83	0.18	71.8
			0.00			1" Ice	3.26	0.24	99.0
ANT150F2	В	From Leg	6.00	0.0000	80.00	No Ice	1.30	1.30	16.0 20.8
			0.00			1/2" Ice 1" Ice	1.60 1.90	1.60 1.90	25.6
		F I	5.00	0.0000	80.00	No Ice	2.40	0.13	50.0
6' Standoff Arm	В	From Leg	3.00 0.00	0.0000	80.00	1/2" Ice	2.83	0.18	71.8
			0.00			1" Ice	3.26	0.24	99.0
531-70HD	В	From Leg	2.00	0.0000	55.00	No Ice	6.00	6.00	40.0
331-70IID		110111 206	0.00			1/2" Ice	6.90	6.90	50.0
			0.00			1" Ice	7.80	7.80	60.0
WPA-80080/4CF	С	From Leg	3.00	0.0000	120.00	No Ice	4.71	2.25	10.0
			0.00			1/2" Ice	5.03	2.55	37.8
			0.00		100.00	1" Ice	5.35	2.85	69.9
ANT450F6	Č	From Leg	3.00	0.0000	120.00	No Ice	1.86 2.67	1.86 2.67	21.0 35.0
			0.00			1/2" Ice 1" Ice	3.30	3.30	54.2
COLEO MENT DE	C	From Log	5.00 3.00	0.0000	120.00	No Ice	4.37	4.37	34.0
SC479-HF1LDF	C	From Leg	0.00	0.0000	120.00	1/2" Ice	6.54	6.54	69.8
(Inverted)			-3.00			1" Ice	8.04	8.04	114.
SC479-HF1LDF	C	From Leg	3.00	0.0000	120.00	No Ice	4.37	4.37	34.0
(Inverted)		č	0.00			1/2" Ice	6.54	6.54	69.8
(-3.00			1" Ice	8.04	8.04	114.
TX/RX 432E-83I-01T	C	From Leg	1.00	0.0000	120.00	No Ice	1.20	0.75	25.0
			0.00			1/2" Ice	1.34	0.86	36.5
			0.00	0.0000	120.00	1" Ice	1.48 13.60	0.98 13.60	50.3 378.
10-ft T-Frame	C	From Leg	2.00	0.0000	120.00	No Ice 1/2" Ice	17.50	17.50	530.
			0.00			1" Ice	21.40	21.40	682.
ANITAGODE	C	From Leg	6.00	0.0000	85.00	No Ice	1.86	1.86	21.0
ANT450F6	C	Lion reg	0.00	0.000	22.00	1/2" Ice	2.67	2.67	35.0
			5.00			1" Ice	3.30	3.30	54.2
6' Standoff Arm	С	From Leg	3.00	0.0000	85.00	No Ice	2.40	0.13	50.0
		J	0.00			1/2" Ice	2.83	0.18	71.8
			0.00			1" Ice	3.26	0.24	99.0
									40.0
531-70HD	С	From Leg	2.00	0.0000	55.00	No Ice	6.00	6.00 6.90	4(5(

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Project	160-ft Lattice Tower (CSP #52)	Date 09:38:07 10/09/23
Client	Verizon	Designed by TJL

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		C _A A _A Front	$C_A A_A$ Side	Weig
	Leg		Lateral Vert					_,	
			ft	0	ft		fr²	ft²	lb
			ft ft		<i>J</i> •		<i>J.</i>	J	ı
			0.00			1" Ice	7.80	7.80	60.0
MT6413-77A	C	From Face	3.00	0.0000	145.00	No Ice	3.79	1.46	60.0
(Verizon Proposed)			-3.00			1/2" Ice	4.04	1.65	84.3
MITT 450 DOD		B B	0.00			I" Ice	4.30	1.85	112.2
NHH-45B-R2B (Verizon Proposed)	C	From Face	3.00	0.0000	145.00	No Ice	11.40	5.28	0.08
(Venzon Froposed)			-6.00			1/2" Ice	11.89	5.74	65.6
NHH-45B-R2B	С	From Face	0.00 3.00	0.0000	145.00	1" Ice	12.38	6.20	137.
(Verizon Proposed)	0	1 John 1 acc	6.00	0.0000	145.00	No Ice 1/2" Ice	11.40 11.89	5.28 5.74	0.08
(· • · · · · · · · · · · · · · · · · ·			0.00			1" Ice	12.38	6.20	65.6 137.1
MT6413-77A	Α	From Face	3.00	0.0000	145.00	No Ice	3.79	1.46	60.0
(Verizon Proposed)			-3.00	0.000	115.00	1/2" Ice	4.04	1.65	84.3
• •			0.00			1" Ice	4.30	1.85	112.2
NHH-45B-R2B	Α	From Face	3.00	0.0000	145.00	No Ice	11.40	5.28	0.08
(Verizon Proposed)			-6.00			1/2" Ice	11.89	5.74	65.6
			0.00			1" Ice	12.38	6.20	137.7
NHH-45B-R2B	Α	From Face	3.00	0.0000	145.00	No Ice	11.40	5.28	0.08
(Verizon Proposed)			6.00			1/2" Ice	11.89	5.74	65.6
			0.00			1" Ice	12.38	6.20	137.7
MT6413-77A	В	From Leg	3.00	0.0000	145.00	No Ice	3.79	1.46	60.0
(Verizon Proposed)			-3.00			1/2" Ice	4.04	1.65	84.3
	_	_	0.00			1" Ice	4.30	1.85	112.2
NHH-45B-R2B	В	From Leg	3.00	0.0000	145.00	No Ice	11.40	5.28	0.08
(Verizon Proposed)			-6.00			1/2" Ice	11.89	5.74	65.6
NHH-45B-R2B	р	E I	0.00	0.0000	145.00	1" Ice	12.38	6.20	137.7
(Verizon Proposed)	В	From Leg	3.00	0.0000	145.00	No Ice	11.40	5.28	0.08
(venzon rroposeu)			6.00 0.00			1/2" Ice	11.89	5.74	65.6
MT6413-77A	C	From Leg	3.00	0.0000	145.00	1" Ice No Ice	12.38 3.79	6.20	137.7
(Verizon Proposed)	•	110III Log	-3.00	0.0000	145.00	1/2" Ice	4.04	1.46 1.65	60.0 84.3
(·····)			0.00			1" Ice	4.30	1.85	112.2
NHH-45B-R2B	C	From Leg	3.00	0.0000	145.00	No Ice	11.40	5.28	0.08
(Verizon Proposed)		0	-6.00		2 10100	1/2" Ice	11.89	5.74	65.6
• •			0.00			1" Ice	12.38	6.20	137.7
NHH-45B-R2B	C	From Leg	3.00	0.0000	145.00	No Ice	11.40	5.28	0.08
(Verizon Proposed)			6.00			1/2" Ice	11.89	5.74	65.6
			0.00			1" Ice	12.38	6.20	137.7
RF4439d-25A (B2/B66A	C	From Face	3.00	0.0000	145.00	No Ice	1.88	1.25	75.0
RRH)			-3.00			1/2" Ice	2.05	1.39	93.34
(Verizon Proposed)			2.00			1" Ice	2.22	1.54	114.4
LF4439d-25A (B2/B66A	Α	From Face	3.00	0.0000	145.00	No Ice	1.88	1.25	75.00
RRH)			-3.00			1/2" Ice	2.05	1.39	93.34
(Verizon Proposed)			2.00			1" Ice	2.22	1.54	114.4
2F4439d-25A (B2/B66A	В	From Leg	3.00	0.0000	145.00	No Ice	1.88	1.25	75.00
RRH) (Verizon Proposed)			-3.00			1/2" Ice	2.05	1.39	93.34
F4439d-25A (B2/B66A	C	Enom I am	2.00	0.0000	145.00	1" Ice	2.22	1.54	114.4
RRH)	C	From Leg	3.00 -3.00	0.0000	145.00	No Ice	1.88	1.25	75.00
(Verizon Proposed)			2.00			1/2" Ice	2.05	1.39	93.34
RF4461d-13A	C	From Face	3.00	0.0000	145.00	1" Ice No Ice	2.22	1.54	114.4
(Verizon Proposed)	~	. 10111 1 100	-3.00	0.0000	175,00	1/2" Ice	1.87 2.03	1.28 1.42	80.00 98.5
,			-2.00			1" Ice	2.03	1.42	119.8
RF4461d-13A	Α	From Face	3.00	0.0000	145.00	No Ice	1.87	1.28	80.00
(Verizon Proposed)			-3.00	0.0000	115.00	1/2" Ice	2.03	1.42	98.51
• ′			-2.00			1" Ice	2.21	1.57	119.8
RF4461d-13A	В	From Leg	3.00	0.0000	145.00	No Ice	1.87	1.28	80.00
(Verizon Proposed)		_	-3.00			1/2" Ice	2.03	1.42	98.51

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	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
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Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weigh
	Leg		Lateral Vert ft ft	٥	ft		ft²	ft²	lb
			ft						
			-2.00			l" Ice	2.21	1.57	119.81
RF4461d-13A	C	From Leg	3.00	0.0000	145.00	No Ice	1.87	1.28	80.00
(Verizon Proposed)			-3.00			1/2" Ice	2.03	1.42	98.51
			-2.00			1" Ice	2.21	1.57	119.8
RC2DC-3315-PF-48	Α	From Leg	1.00	0.0000	145.00	No Ice	3.01	1.96	25.00
(Verizon Proposed)			0.00			1/2" Ice	3.23	2.15	51.21
			0.00			1" Ice	3.46	2.35	80.79
SitePro VFA12-HD	C	From Face	1.00	0.0000	145.00	No Ice	21.00	21.00	750.0
(Verizon Proposed)			0.00			1/2" Ice	25.00	25.00	900.0
•			0.00			1" Ice	29.00	29.00	1050.0
SitePro VFA12-HD	Α	From Face	1.00	0.0000	145.00	No Ice	21.00	21.00	750.0
(Verizon Proposed)			0.00			1/2" Ice	25.00	25.00	900.0
			0.00			1" Ice	29.00	29.00	1050.0
SitePro VFA12-HD	В	From Leg	1.00	0.0000	145.00	No Ice	21.00	21.00	750.0
(Verizon Proposed)			0.00			1/2" Ice	25.00	25.00	900.0
			0.00			1" Ice	29.00	29.00	1050.0
SitePro VFA12-HD	C	From Leg	1.00	0.0000	145.00	No Ice	21.00	21.00	750.0
(Verizon Proposed)			0.00			1/2" Ice	25.00	25.00	900.0
,			0.00			1" Ice	29.00	29.00	1050.0
10-ft Lighting Rod	Α	From Leg	1.00	0.0000	160.00	No Ice	1.00	1.00	40.00
		_	0.00			1/2" Ice	2.02	2.02	49.26
			5.00			1" Ice	3.05	3.05	64.89

	Dishes											
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight	
				Vert ft	٥	0	ft	ft		ft²	lb	
PA6-65	A	Paraboloid w/Radome	From Leg	1.00 0.00 0.00	Worst		120.00	6.00	No Ice 1/2" Ice 1" Ice	28.30 30.00 31.70	744.00 1000.00 1256.00	
PA6-65	В	Paraboloid w/Radome	From Leg	1.00 0.00 0.00	Worst		120.00	6.00	No Ice 1/2" Ice 1" Ice	28.30 30.00 31.70	744.00 1000.00 1256.00	

	Truss-Leg Properties										
Section Designation	Area	Area Ice	Self Weight	Ice Weight	Equiv. Diameter	Equiv. Diameter Ice	Leg Area				
	in ²	in ²	lb	lb	in	in	in ²				
#12ZG-58-1.5" -	2303.0530	6015.7699	552.82	825.57	7.9967	20.8881	5.301				
1.00" #12ZG-58-1.5" - 1.00"	2303.0530	6001.0711	552.82	802.23	7.9967	20.8371	5.301				
#12ZG-58-1.75" -	2421.2670	6056.1760	678.44	735.74	8.4072	21.0284	7.215				

Centek Engineering Inc. 63-2 North Branford Rd.

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

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Section Designation	Area Area Ice		Self Weight	Ice Weight	Equiv. Diameter	Equiv. Diameter	Leg Area
	in²	in ²	lb	lb	în	Ice in	in^2
1.00"							
#12ZG-58-1.75" - 1.00"	2421.2670	6036.2507	678.44	710.75	8.4072	20,9592	7.2158
#12ZG-58-2.00" - 0.875"	2556.3970	6083.8542	998.16	728.08	8.8764	21.1245	9.4248
#12ZG-58-2.25" - 0.875"	2686.5516	6124.1363	1165.70	702.94	9.3283	21.2644	11.9282
#12ZG-58-2,25" - 0.875"	2686.5516	6077.9773	1165.70	650.68	9.3283	21.1041	11.9282
#12ZG-58-2.50" - 0.875"	2826.7749	6058.3446	1354.97	568.88	9.8152	21.0359	14.7262

222-H Verification Constants

Constant	Value
K_d	0.85
Ice Thickness Importance Factor	1.25
Z_{g}	900
α	9.5
K_{zmin}	0.85
K_c	n/a
K_{ι}	1
f	1
K_{e}	1

222-H Section Verification ArRr By Element

Section	Elem.	Size	C	C	F	e	е	A_r	A_r	A_rR_r	A_rR_r
Elevation	Num.			w/Ice	a		w/Ice		w/Ice		w/Ice
_					с				- 1		
-ft					е			ft ²	ft²	ft²	fl ²
T1	1	#12ZG-58-1.5" -	105.621	102.182	С	0.217	0.508	13.328	34.813	7.699	23.996
160.00-140.00		1.00"	·	175.52			10.000 10.000	17000140000	000		
	1	#12ZG-58-1.5" - 1.00"	105.621	102.182	A	0.217	0.508	13.328	34.813	7.699	23.996
	2	#12ZG-58-1.5" - 1.00"	105.621	102.182	С	0.217	0.508	13.328	34.813	7.699	23.996
	2	#12ZG-58-1.5" - 1.00"	105.621	102.182	В	0.217	0.508	13.328	34.813	7.699	23.996
	3	#12ZG-58-1.5" - 1.00"	105.621	102.182	В	0.217	0.508	13.328	34.813	7.699	23.996
	3	#12ZG-58-1.5" - 1.00"	105.621	102.182	Α	0.217	0.508	13.328	34.813	7.699	23.996
					Α		Sum:	26.656	69.627	15.398	47,992
	- 1			1	В			26.656	69.627	15.398	47.992
					С			26.656	69.627	15.398	47.992
T2 140.00-120.00	19	#12ZG-58-1.5" - 1.00"	104.042	100.409	С	0.191	0.448	13.350	34.786	7.649	22.908
555418.00	19	#12ZG-58-1.5" - 1.00"	104.042	100.409	Α	0.191	0.448	13.350	34.786	7.649	22.908
	20	#12ZG-58-1.5" - 1.00"	104.042	100.409	С	0.191	0.448	13.350	34.786	7.649	22.908
	20	#12ZG-58-1.5" - 1.00"	104.042	100.409	В	0.191	0.448	13.350	34.786	7.649	22.908

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Section	Elem.	Size	C	С	F	е	е	A_r	A_r	A_rR_r	A_rR_r
Elevation	Num.			w/Ice	а		w/Ice		w/Ice		w/Ice
ft					c e			∈ft²	ft²	fi²	ft²
	21	#12ZG-58-1.5" -	104.042	100.409	В	0.191	0.448	13.350	34.786	7.649	22.908
	21	1.00" #12ZG-58-1.5" -	104 042	100.409	A	0.191	0.448	13.350	34.786	7.649	22.908
	21	1.00"	104.042	100.402	11	0.121	=3	50050	7 - Vet	1000000	200000000000000000000000000000000000000
					A B		Sum:	26.700 26.700	69.573 69.573	15.298 15.298	45.816 45.816
					C			26.700	69.573	15.298	45.816
Т3	34	#12ZG-58-1.75" -	107.476	99.564	č	0.18	0.407	14.035	35.106	8.019	22.444
120.00-100.00	34	1.00" #12ZG-58-1.75" -	107.476	99.564	A	0.18	0.407	14.035	35,106	8.019	22.444
		1.00"					20 × 12 × 20	14.025	25.106	0.010	22.444
	35	#12ZG-58-1.75" - 1.00"	107.476	99.564	С	0.18	0.407	14.035	35.106	8.019	22.444
	35	#12ZG-58-1.75" -	107.476	99.564	В	0.18	0.407	14.035	35.106	8.019	22.444
	36	1.00" #12ZG-58-1.75" -	107.476	99.564	В	0.18	0.407	14.035	35.106	8.019	22.444
	1000	1.00"					5000.00	Î	29600	0.010	22.444
	36	#12ZG-58-1.75" - 1.00"	107.476	99.564	A	0.18	0.407	14.035	35.106	8.019	22.444
		1.00			Α		Sum:	28.071	70.211	16.038	44.887
					В			28.071	70.211	16.038	44.887
					C	200.2004	WW.0016	28.071	70.211	16.038	44.887
T4 100.00-80.00	52	#12ZG-58-1.75" -	105.23	97.162	С	0.153	0.345	14.035	34.990	7.970	21.485
	52	1.00" #12ZG-58-1.75" -	105.23	97.162	A	0.153	0.345	14.035	34.990	7.970	21.485
	53	1.00" #12ZG-58-1-75" -	105.23	97.162	С	0.153	0.345	14.035	34.990	7.970	21.485
		1.00"	105 22	07.163	D	0.153	0.345	14.035	34.990	7.970	21.485
	53	#12ZG-58-1.75" - 1.00"	105.23	97.162	В	0.133	0.545	14.032	34.550	1/25/2009	61,000
	54	#12ZG-58-1.75" - 1.00"	105.23	97.162	В	0.153	0.345	14.035	34.990	7.970	21.485
	54	#12ZG-58-1.75" -	105.23	97.162	A	0.153	0.345	14.035	34.990	7.970	21.485
		1.00"			A		Sum:	28.071	69.980	15.939	42.971
					В		Julia	28.071	69.980	15.939	42.971
					c			28.071	69.980	15.939	42.971
T5 80.00-60.00	67	#12ZG-58-2.00" -	108.202	95.372	č	0.134	0.289	14.819	35.266	8.389	21,000
	67	0.875" #12ZG-58-2.00" -	108.202	95.372	A	0.134	0.289	14.819	35.266	8.389	21.000
		0.875"				5900.00	1000 mm (200 mm		25.266	0 200	21,000
	68	#12ZG-58-2.00" - 0.875"	108.202	95.372	C	0.134	0.289	14.819	35.266	8.389	21.000
	68	#12ZG-58-2.00" -	108.202	95.372	В	0.134	0.289	14.819	35.266	8.389	21.000
	69	0.875" #12ZG-58-2.00" -	108.202	95.372	В	0.134	0.289	14.819	35.266	8.389	21.000
		0.875"	108.202	05 272	A	0.134	0.289	14.819	35.266	8.389	21.000
	69	#12ZG-58-2.00" - 0.875"	108.202	93.312	_ ^	0.134	0.202				2000
					Α		Sum:	29.637	70.532	16.777	42.000
					В			29.637	70.532	16.777	42.000
		***************************************	100 551	00.550	C	0.135	0.261	29.637	70.532 35.500	16.777 8.806	42.000 20.868
T6 60.00-40.00	76	#12ZG-58-2.25" - 0.875"	109.754	92.663	С	0.125	0.261	15.573	200	James N	
	76	#12ZG-58-2.25" - 0.875"	109.754	92.663	A	0.125	0.261	15.573	35,500	8.806	20.868
	77	#12ZG-58-2.25" -	109.754	92.663	С	0.125	0.261	15.573	35.500	8.806	20.868
	77	0.875" #12 ZG- 58-2.25" -	109.754	92.663	В	0.125	0.261	15.573	35,500	8.806	20.868
		0.875"			1		SANTEDIA.	S-52-00167		TRACE	

Centek Engineering Inc. 63-2 North Branford Rd.

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Section	Elem.	Size	C	C	F	е	е	A_r	A_r	A_rR_r	A_rR_r
Elevation	Num.			w/Ice	а		w/Ice		w/Ice		w/Ice
ft					c			ft²	ft²	fi²	c.2
	78	#12ZG-58-2.25" -	109.754	92 663	B	0.125	0.261	15.573	35.500	8.806	fr²
1	(1	0.875"	107.754	72.003	"	0.123	0.201	15.575	33.300	0.000	20.000
	78	#12ZG-58-2.25" - 0.875"	109.754	92.663	A	0.125	0.261	15.573	35.500	8.806	20.868
					A		Sum:	31.146	70.999	17.613	41.735
					В			31.146	70.999	17.613	41.735
					С			31.146	70.999	17.613	41.735
T7 40.00-20.00	85	#12ZG-58-2.25" - 0.875"	104.008		С	0.115	0.237	15.573	35.232	8.797	20.499
	85	#12ZG-58-2.25" - 0.875"	104.008	87.15	A	0.115	0.237	15.573	35.232	8.797	20.499
	86	#12ZG-58-2.25" - 0.875"	104.008	87.15	С	0.115	0.237	15.573	35.232	8.797	20.499
	86	#12ZG-58-2.25" - 0.875"	104.008	87.15	В	0.115	0.237	15.573	35.232	8.797	20.499
	87	#12ZG-58-2.25" - 0.875"	104.008	87.15	В	0.115	0.237	15.573	35.232	8.797	20.499
	87	#12ZG-58-2.25" - 0.875"	104.008	87.15	A	0.115	0.237	15.573	35.232	8.797	20.499
	- 1	0.0.0			A		Sum:	31.146	70.464	17.595	40.998
	- 1				В			31.146	70.464	17.595	40.998
					С			31.146	70.464	17.595	40.998
T8 20.00-0.00	94	#12ZG-58-2.50" - 0.875"	101.803	80.809	С	0.11	0.216	16.386	35.118	9.253	20.277
	94	#12ZG-58-2.50" - 0.875"	101.803	80.809	A	0.11	0.216	16.386	35.118	9.253	20.277
	95	#12ZG-58-2.50" - 0.875"	101.803	80.809	С	0.11	0.216	16.386	35.118	9.253	20.277
	95	#12ZG-58-2.50" - 0.875"	101.803	80.809	В	0.11	0.216	16.386	35.118	9.253	20.277
	96	#12ZG-58-2.50" - 0.875"	101.803	80.809	В	0.11	0.216	16.386	35.118	9.253	20.277
	96	#12ZG-58-2.50" - 0.875"	101.803	80.809	Α	0.11	0.216	16.386	35.118	9.253	20.277
		0.075			A		Sum:	32.772	70.236	18,506	40.554
					В			32.772	70.236	18.506	40.554
					С			32.772	70.236	18.506	40.554

222-H Section Verification Tables - No Ice

Section Elevation	Z_{wind}	Zice	K ₂	Kh	K_{zt}	t _z	q_z	F	е	A_rR_r
Elevation								а		
	c					. 1		С		-2
Jt	ft	्रft				in	psf	е		fi ⁺
T1 160.00-140.00	150.00		1.378	1	1		55	A	0.217	15.398
								В	0.217	15.398
								C	0.217	15.398
T2 140.00-120.00	130.00		1.337	1	1		53	Α	0.191	15.298
						ľ		В	0.191	15.298
								C	0.191	15.298
T3 120.00-100.00	110.00		1.291	1	1		51	A	0.18	16.038
								В	0.18	16.038
1) /						C	0.18	16.038

Centek Engineering Inc. 63-2 North Branford Rd.

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

Job	23134.00 - East Haddam	Page 16 of 42
Project	160-ft Lattice Tower (CSP #52)	Date 09:38:07 10/09/23
Client	Verizon	Designed by TJL

Section	Z_{wind}	Zice	K _z	K _h	K_{zt}	t_z	q_z	F	е	$A_i R_r$
Elevation	~							а		
								c		G.
ft	ft	ft				in	psf	е		17.000
T4 100.00-80.00	90.00		1.238	1	1		49	A	0.153	15.939
								В	0.153	15.939
								C	0.153	15.939
T5 80,00-60,00	70.00		1.174	1	1		47	A	0.134	16,777
13 80,00 00,00	70.00							В	0.134	16.777
								С	0.134	16,777
T6 60.00-40.00	50.00		1.094	1	1		43	Α	0.125	17,613
10 00.00-10.00	50.00		1107.	-				В	0.125	17.613
								С	0.125	17.613
T7 40.00-20.00	30,00		0.982	1	1		39	Α	0.115	17.595
1 / 40.00-20.00	20,00		0,,,,,,		_			В	0.115	17,595
								C	0.115	17.595
T8 20.00-0.00	10.00		0.85	1	1		34	Α	0.11	18,506
18 20.00-0.00	10.00		0,05	_ ^				В	0.11	18.506
								С	0.11	18.506

222-H Section Verification Tables - Ice

Section	Zwind	Zice	K_z	K_h	K_{zt}	t_z	q_z	F	е	A_rR_r
Elevation								а		
		ft				in	psf	c e		ft²
T1 160 00 140 00	150.00	150.00	1.378	1	1	1.4543	7	A	0.508	56.650
T1 160.00-140.00	150.00	130,00	1,576	1	1	1,7515		В	0.508	56.650
		- 1						Č	0.508	56.650
T2 140.00-120.00	130.00	130.00	1,337	1	1	1,4337	7	Ā	0.448	53.346
12 140.00-120.00	150.00	150,00	1,,557	1	1	1,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		В	0.448	53.346
								С	0.448	53.346
T3 120.00-100.00	110.00	110,00	1.291	1	1	1.4099	7	A	0.407	54.364
13 120,00-100,00	110.00	110,00	1,527	-				В	0.407	54.364
	- 1							С	0.407	54.364
T4 100.00-80.00	90.00	90.00	1.238	1	1	1,3819	7	Α	0.345	51.538
14 100.00 00.00	30.00	2 41						В	0.345	51.538
								С	0.345	51.538
T5 80.00-60.00	70.00	70.00	1:174	1	1	1.3476	6	A	0.289	48.243
15 00.00 00.00	70.00	, , , ,						В	0.289	48.243
1							1	C	0.289	48.243
T6 60.00-40.00	50.00	50.00	1.094	1	1	1,3030	6	A	0.261	48.044
10 00:00 10:00	20,00		72					В	0.261	48.044
1								С	0.261	48.044
T7 40.00-20.00	30.00	30.00	0.982	1	1	1.2381	5	Α	0.237	47.276
17 10:00 20:00		- 1,311						В	0.237	47.276
	- 1							С	0.237	47.276
T8 20.00-0.00	10.00	10.00	0.85	I	1	1.1093	5	Α	0.216	46.452
1320,00 0,00	- 50	112		20				В	0.216	46.452
								C	0.216	46.452

222-H Section Verification Tables - Service

Section Elevation	Zwind	Z_{ice}	K _z	K_h	K_{zt}	t ₂	q_z	F a	е	A_rR_r
Elevation 4	#	ft				in	psf	c e		fi²
T1 160 00-140 00	150.00		1,378	1	1		11	A	0.217	15.398

Centek Engineering Inc. 63-2 North Branford Rd.

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

Job		Page
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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client	Ma :	Designed by
	Verizon	TJL

Section	Zwind	Z _{ice}	K ₂	Kh	Kzt	t _z	q_z	F	е	A_rR_r
Elevation			1 1					а		
£	ft	ft						C		0.7
·		Ji				in	psf	е	0.015	Jr .
								В	0.217	15.398
T2 140.00-120.00	120.00		1 227					C	0.217	15.398
12 140.00-120.00	130.00		1,337	1	1		10	A	0.191	15.298
1	1							В	0.191	15.298
T3 120.00-100.00	110.00		1 , ,,,,					C	0.191	15.298
13 120,00-100,00	110.00		1,291	1	1		10	A	0.18	16.038
								В	0.18	16.038
T4 100.00-80.00	00.00		1 , ,,,,,					C	0.18	16.038
14 100.00-80.00	90.00		1.238	1	1		10	A	0.153	15.939
								В	0.153	15.939
TE 80.00.40.00	70.00							C	0.153	15.939
T5 80.00-60.00	70.00		1.174	1	1		9	Α	0.134	16.777
								В	0.134	16.777
TC (0.00.40.00	50.00							C	0.134	16.777
T6 60.00-40.00	50.00		1.094	1	1		9	Α	0.125	17.613
			1					В	0.125	17.613
mg 40 00 go 00								C	0.125	17.613
T7 40.00-20.00	30,00		0.982	1	1		8	Α	0.115	17.595
			l I					В	0.115	17.595
			1 1					С	0.115	17.595
T8 20.00-0.00	10.00		0.85	1	1		7	A	0.11	18.506
			1 1					В	0.11	18.506
								С	0.11	18.506

Tower Pressures - No Ice

 $G_H = \theta.85\theta$

Section	Z	Kz	q_z	A_G	F	A_F	A_R	A_{leg}	Leg	$C_A A_A$	C_AA_A
Elevation					а			1-25	%	In	Out
					С					Face	Face
्रft	ft		psf	_ft²	е	ft²	ft²	ft ²		ft²	ft ²
T1	150.00	1.378	55	182.500	Α	12.955	26.656	26.656	67.29	0.000	0.000
160.00-140.00					В	12.955	26.656		67.29	0.000	0.000
					C	12.955	26.656		67.29	1.980	0.000
T2	130.00	1.337	53	202.528	Α	11.964	26.700	26.700	69.06	0.000	0.000
140.00-120.00					В	11.964	26.700		69.06	0.000	0.000
					С	11.964	26.700		69.06	7.920	0.000
T3	110.00	1.291	51	242.945	Α	15.770	28.071	28.071	64.03	0.000	0.000
120.00-100.00					В	15.770	28.071		64.03	0.000	0.000
					C	15.770	28.071		64.03	50.634	0.000
T4	90.00	1.238	49	282.945	Α	15.144	28.071	28.071	64.96	0.000	0.000
100.00-80.00					В	15.144	28.071		64.96	0.000	0.000
					C	15.144	28.071		64.96	52.814	0.000
T5 80.00-60.00	70.00	1.174	47	323.362	Α	13.615	29.637	29.637	68.52	0.000	0.000
					В	13.615	29.637		68.52	0.000	0.000
					C	13.615	29.637		68.52	59.354	0.000
T6 60.00-40.00	50.00	1.094	43	363.780	Α	14.415	31.146	31.146	68.36	0.000	0.000
	1				В	14.415	31.146		68.36	0.000	0.000
1					C	14.415	31.146		68.36	61.244	0.000
T7 40.00-20.00	30.00	0.982	39	403.780	Α	15.249	31.146	31.146	67.13	0.000	0.000
					В	15.249	31.146		67.13	0.000	0.000
1					C	15.249	31.146		67.13	61.874	0.000
T8 20.00-0.00	10.00	0.85	34	444.197	Α	16.115	32.772	32.772	67.04	0.000	0.000
1					В	16.115	32.772		67.04	0.000	0.000
					С	16.115	32.772		67.04	65.654	0.000

Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

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Project	160-ft Lattice Tower (CSP #52)	Date 09:38:07 10/09/23
Client	Verizon	Designed by TJL

Tower Pressure - With Ice

 $G_H = 0.850$

Section	z	Kz	q_z	t _Z	A_G	F	A_F	A_R	A_{leg}	Leg	$C_A A_A$	C_AA_A
Elevation			1			а				%	In	Out
	- 1	- 1	- 1			с					Face	Face
ft	ft		psf	in	ft ²	е	ft ²	ft²	ft²		fr ²	ft ²
T1	150.00	1.378	7	1.4543	187.348	Α	12.955	82.188	69.627	73.18	0.000	0.000
160.00-140.00		- 1		- 1		В	12.955	82.188		73.18	0.000	0.000
		- 1				С	12.955	82.188		73.18	6.318	0.000
T2	130.00	1.337	7	1.4337	207.313	Α	11.964	81.008	69.573	74.83	0.000	0.000
140.00-120.00						В	11.964	81.008		74.83	0.000	0.000
110100 12011						С	11.964	81.008		74.83	25.133	0.000
T3	110.00	1.291	7	1.4099	247.651	Α	15.770	85.034	70.211	69.65	0.000	0.000
120.00-100.00	110.00					В	15.770	85.034		69.65	0.000	0.000
120.00 100.00		- 1				C	15.770	85.034		69.65	141.354	0.000
T4 100,00-80.00	90.00	1.238	7	1.3819	287.557	Α	15.144	83.932	69.980	70.63	0.000	0.000
14 100.00 00.00	70.00	1.200				В	15.144	83.932		70.63	0.000	0.000
l 1						С	15.144	83.932		70.63	150.477	0.000
T5 80.00-60.00	70.00	1.174	6	1.3476	327.860	Α	13.615	81.017	70.532	74.53	0.000	0.000
13 80.00-00.00	70.00					В	13.615	81.017		74.53	0.000	0.000
						С	13.615	81.017		74.53	164.072	0.000
т6 60.00-40.00	50.00	1.094	6	1.3030	368.129	A	14.415	81.733	70.999	73.84	0.000	0.000
10 00.00-40.00	50.00	1.074	ĭ	1.5050		В	14.415	81.733		73.84	0.000	0.000
						C	14.415	81.733		73.84	172.661	0.000
T7 40.00-20.00	30.00	0.982	5	1.2381	407.912	Ã	15.249	81.253	70.464	73.02	0.000	0.000
1 / 40.00-20.00	50.00	0.702	Ĭ	1.2501	1071312	В	15.249	81.253		73.02	0.000	0.000
						Č	15.249	81,253		73.02	173.058	0.000
T8 20.00-0.00	10.00	0.85	5	1.1093	447.899	_	16.115	80,452	70.236	72.73	0.000	0.000
1 8 20.00-0.00	10.00	0.05	[ا	1.1075	117.077	В	16.115	80.452		72.73	0.000	0.000
						C	16.115	80.452		72.73	181.514	0.000

Tower Pressure - Service

 $G_H = 0.850$

Section	z	Kz	q_z	A_G	\overline{F}	A_F	A_R	A_{leg}	Leg	$C_A A_A$	$C_A A_A$
Elevation					а				%	In	Out
					С			,	- 1	Face	Face
ft	ft		psf	ft ²	е	fr²	ft ²	ft²		ft	ft
T1	150.00	1.378	11	182.500	Α	12.955	26.656	26.656	67.29	0.000	0.000
160.00-140.00	2.0.2.2.				В	12.955	26.656		67.29	0.000	0.000
100.00 110.00					С	12.955	26.656		67.29	1.980	0.000
Т2	130.00	1.337	10	202.528	Α	11.964	26.700	26.700	69.06	0.000	0.000
140.00-120.00	150.00	1.557			В	11.964	26.700		69.06	0.000	0.000
140,00-120.00					С	11.964	26.700		69.06	7.920	0.000
Т3	110.00	1.291	10	242,945	A	15.770	28.071	28.071	64.03	0.000	0.000
120.00-100.00	110.00	1.271			В	15.770	28.071		64.03	0.000	0.000
120.00-100.00					C	15,770	28.071		64.03	50.634	0.000
Т4	90.00	1.238	10	282,945	Ā	15.144	28.071	28.071	64.96	0.000	0.000
100.00-80.00	70.00	1.250			В	15.144	28.071		64.96	0.000	0.000
100.00-60.00					Č	15,144	28.071		64.96	52.814	0.000
T5 80.00-60.00	70.00	1.174	9	323.362	A	13.615	29.637	29.637	68.52	0.000	0.000
15 80.00-00.00	70.00	1.174		22.502	В	13.615	29.637		68.52	0.000	0.000

Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

Job		Page
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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client	Varian	Designed by
	Verizon	l TJL

Section	Z	Kz	q_z	A_G	F	A_F	A_R	A_{leg}	Leg	$C_A A_A$	$C_A A_A$
Elevation					a				%	In	Out
				_	c					Face	Face
fi	ft		psf	ft ²	e	ft ²	ft ²	ft ²		ft²	ft ²
					С	13.615	29.637		68.52	59.354	0.000
T6 60.00-40.00	50.00	1.094	9	363.780	Α	14.415	31.146	31.146	68.36	0.000	0.000
					В	14.415	31.146		68.36	0.000	0.000
					C	14.415	31.146		68.36	61.244	0.000
T7 40.00-20.00	30.00	0.982	8	403.780	Α	15.249	31.146	31.146	67.13	0.000	0.000
					В	15.249	31.146		67.13	0.000	0.000
					C	15.249	31.146		67.13	61.874	0.000
T8 20.00-0.00	10.00	0.85	7	444.197	Α	16.115	32.772	32.772	67.04	0.000	0.000
)				В	16.115	32.772		67.04	0.000	0.000
					C	16.115	32.772		67.04	65.654	0.000

Tower Forces - No Ice - Wind Normal To Face

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	n.	Ctrl.
Elevation	Weight	Weight	а									Face
			c			psf						
ft	lb	lb	e						ft ²	lb	plf	
T1	19.00	2678.27	Α	0.217	2.54	55	1	1	28.354	3401.56	170.08	С
160.00-140.00			В	0.217	2.54		1	1	28.354			
			С	0.217	2.54		1	1	28.354			
T2	76.00	2639.67	Α	0.191	2.627	53	1	1	27.262	3442.71	172.14	l c
140.00-120.00			В	0.191	2.627		1	1	27.262			
			C	0.191	2.627		1	1	27.262			
T3	241.00	3230.97	Α	0.18	2.663	51	1	1	31.807	5008.72	250.44	С
120.00-100.00			В	0.18	2.663		1	1	31.807			
			С	0.18	2.663		1	1	31.807			
T4	247.60	3231.19	Α	0.153	2.761	49	1	1	31.083	4903.91	245.20	С
100.00-80.00			В	0.153	2.761		1	1	31.083			
			C	0.153	2.761		1	1	31.083			
T5	267.40	4725.12	Α	0.134	2.832	47	1	1	30.393	4816.05	240.80	С
80.00-60.00			В	0.134	2.832		1	1	30.393			
			C	0.134	2.832		1	1	30,393			
T6	271.90	5314.76	Α	0.125	2.865	43	1	1	32.028	4737.65	236.88	С
60.00-40.00			В	0.125	2.865		1	1	32.028			
			C	0.125	2.865		1	1	32.028			
Т7	273.40	5407.03	Α	0.115	2.905	39	1	1	32.843	4388.15	219.41	С
40.00-20.00			В	0.115	2.905		1	1	32.843			
			С	0.115	2.905		1	1	32.843			
T8 20.00-0.00	282.40	6073.32	Α	0.11	2.924	34	1	1	34.620	4029.13	201.46	C
			В	0.11	2.924		1	1	34.620			
			C	0.11	2.924		1	1	34.620			
Sum Weight:	1678.70	33300.33						OTM	2696.04	34727.88		
									kip-ft			

Tower Forces - No Ice - Wind 45 To Face

I	Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	W.	Ctrl.
ı	Elevation	Weight	Weight	а			_		1				Face
ı				С			psf						
ı	ft	lb	lb	е						ft ²	lb	plf	
ı	T1	19.00	2678.27	Α	0.217	2.54	55	0.825	1	26.087	3133.98	156.70	С

Centek Engineering Inc. 63-2 North Branford Rd.

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

Job		Page
Job	23134.00 - East Haddam	20 of 42
Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	w,	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf			, ,			
ft	lb	lb	е						ft ²	lb	plf	
160.00-140.00			В	0.217	2.54		0.825	1	26.087			
			C	0.217	2.54		0.825	1	26.087			_
T2	76.00	2639.67	Α	0.191	2.627	53	0.825	1	25.168	3194.76	159.74	С
140.00-120.00			В	0.191	2.627		0.825	1	25.168			
			C	0.191	2.627		0.825	1	25.168			_
T3	241.00	3230.97	Α	0.18	2.663	51	0.825	1	29.048	4688.88	234.44	С
120.00-100.00			В	0.18	2.663		0.825	1	29.048			
			C	0.18	2.663		0.825	1	29.048			_
T4	247.60	3231.19	Α	0.153	2.761	49	0.825	1	28.433	4598.53	229.93	С
100.00-80.00			В	0.153	2.761		0.825	1	28.433			
			C	0.153	2.761		0.825	1	28.433			
T5	267.40	4725.12	Α	0.134	2.832	47	0.825	1	28.010	4548.98	227.45	С
80.00-60.00			В	0.134	2.832		0.825	1	28.010			
			С	0.134	2.832		0.825	1	28.010			
Т6	271.90	5314.76	Α	0.125	2.865	43	0.825	1	29.505	4471.21	223.56	С
60.00-40.00			В	0.125	2.865		0.825	1	29.505			
			С	0.125	2.865		0.825	1	29.505			
Т7	273.40	5407.03	Α	0.115	2.905	39	0.825	1	30.175	4131.48	206.57	C
40.00-20.00			В	0.115	2.905		0.825	1	30.175			
			C	0.115	2.905		0.825	1	30.175			
T8 20.00-0.00	282.40	6073.32	A	0.11	2.924	34	0.825	1	31.800	3792.87	189.64	C
			В	0.11	2.924		0.825	1	31.800			
1			С	0.11	2.924		0.825	1	31.800			
Sum Weight:	1678.70	33300.33						OTM	2518.92	32560.70		
									kip-ft			

Tower Forces - No Ice - Wind 60 To Face

Section	Add	Self	F	e	C_F	<i>q</i> ₌	D_F	D_R	$A_{\mathcal{E}}$	F	w	Ctrl.
Elevation	Weight	Weight	а)				Face
	ŭ		c			psf						
ft	lb	lb	е						ft^2	lb	plf	
T1	19.00	2678.27	Α	0.217	2.54	55	0.8	1	25.763	3095.76	154.79	C
160.00-140.00			В	0.217	2.54		0.8	1	25.763			
			С	0.217	2.54		0.8	1	25.763			
T2	76.00	2639.67	Α	0.191	2.627	53	0.8	1	24.869	3159.34	157.97	С
140.00-120.00			В	0.191	2.627		0.8	1	24.869			
.,,,,,,			С	0.191	2.627		0.8	1	24.869			
Т3	241.00	3230.97	Α	0.18	2.663	51	0.8	1	28.653	4643.19	232.16	С
120.00-100.00			В	0.18	2.663		0.8	1	28.653			
			С	0.18	2.663		0.8	1	28.653			
T4	247.60	3231.19	Α	0.153	2.761	49	0.8	1	28.054	4554.91	227.75	С
100.00-80.00			В	0.153	2.761		0.8	1	28.054			
			С	0.153	2.761		0.8	1	28.054			
T5	267.40	4725.12	Α	0.134	2.832	47	0.8	1	27.670	4510.82	225.54	С
80.00-60.00			В	0.134	2.832		0.8	1	27.670			
00.00 00.00			С	0.134	2.832		0.8	1	27.670			
Т6	271.90	5314.76	Α	0.125	2.865	43	0.8	1	29.145	4433.15	221.66	С
60.00-40.00			В	0.125	2.865		0.8	1	29.145	0		
00.00 10.00			c	0.125	2.865		0.8	1	29.145			
Т7	273.40	5407.03	A	0.115	2.905	39	0.8	1	29.794	4094.82	204.74	С
40.00-20.00			В	0.115	2.905		0.8	1	29.794			
10.00 20.00			С	0.115	2.905		0.8	1	29.794			
T8 20.00-0.00	282.40	6073.32	A	0.11	2.924	34	0.8	1	31.398	3759.12	187.96	С

Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405

Phone: (203) 488-0580 FAX: (203) 488-8587

Job		Page
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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	W	Ctrl.
Elevation	Weight	Weight	а									Face
			c			psf						
ft	lb	lb	e						ft ²	lb	plf	
			В	0.11	2.924		0.8	1	31.398			
		1	C	0.11	2.924		0.8	1	31.398			
Sum Weight:	1678.70	33300.33						OTM	2493.62	32251.10		
									kip-ft			

Tower Forces - No Ice - Wind 90 To Face

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а			4.5						Face
			c			psf						
ft	lb	lb	e			1 3			ft ²	lb	plf	
T1	19.00	2678.27	Α	0.217	2.54	55	0.85	1	26.410	3172.21	158.61	С
160.00-140.00			В	0.217	2.54		0.85	1	26.410			
1			С	0.217	2.54		0.85	1	26.410			
T2	76.00	2639.67	A	0.191	2.627	53	0.85	1	25.467	3230.18	161.51	С
140.00-120.00			В	0.191	2.627		0.85	1	25.467			
			C	0.191	2.627		0.85	1	25.467			
T3	241.00	3230.97	A	0.18	2.663	51	0.85	1	29.442	4734.57	236.73	C
120.00-100.00			В	0.18	2.663		0.85	1	29.442			- 7
			С	0.18	2.663		0.85	1	29.442			
T4	247.60	3231.19	Α	0.153	2.761	49	0.85	1	28.812	4642.16	232.11	С
100.00-80,00		i i	В	0.153	2.761		0.85	1	28.812			
		1	C	0.153	2.761		0.85	1	28.812			
T5	267.40	4725.12	Α	0.134	2.832	47	0.85	1	28.350	4587.13	229.36	C
80.00-60.00			В	0.134	2.832		0.85	1	28.350			
			C	0.134	2.832		0.85	1	28.350			
Т6	271.90	5314.76	A	0.125	2.865	43	0.85	1	29.865	4509.27	225.46	C
60.00-40.00			В	0.125	2.865		0.85	1	29.865			
			C	0.125	2.865		0.85	1	29.865			
T7	273.40	5407.03	A	0.115	2.905	39	0.85	1	30.556	4168.15	208.41	C
40.00-20.00			В	0.115	2.905		0.85	1	30.556			
			C	0.115	2.905		0.85	1	30.556			
T8 20.00-0.00	282.40	6073.32	Α	0.11	2.924	34	0.85	1	32.203	3826.62	191.33	С
			В	0.11	2.924		0.85	1	32.203			
1 1			C	0.11	2.924		0.85	1	32.203			
Sum Weight:	1678.70	33300.33						OTM	2544.23	32870.29		
									kip-ft			

Tower Forces - With Ice - Wind Normal To Face

Section	Add	Self	F	е	C_F	q_{z}	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			С			psf				3		
ft	lb	lb	е						ft ²	lb	plf	
T1	81.17	6953.50	A	0.508	1.89	7	1	1	69.605	858.31	42.92	С
160.00-140.00			В	0.508	1.89	10	1	1	69.605	2,550,000,000	C. (*0.75-6.0)	
			C	0.508	1.89		1	1	69.605			
T2	320.94	6656.50	A	0.448	1.976	7	1	1	65.310	883.88	44.19	С
140.00-120.00			В	0.448	1.976		1	1	65.310			

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Job		Page
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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
	Ŭ		c			psf			,			
ft	lb	lb	е						fi ²	lb	plf	
			С	0.448	1.976		1	1	65.310			
Т3	1807.16	7471.73	Α	0.407	2.05	7	1	1	70.134	1359.00	67.95	С
120.00-100.00			В	0.407	2.05		1	1	70.134			
			C	0.407	2.05	_	1	1	70.134			_
T4	1842.00	7236.58	Α	0.345	2.184	7	1	1	66.682	1350.46	67.52	C
100.00-80.00			В	0.345	2.184		1	1	66.682			
			C	0.345	2.184		1	1	66.682			
T5	1995.16	8975.12	A	0.289	2.327	6	1	1	61.858	1315.78	65.79	C
80.00-60.00			В	0.289	2.327		1	1	61.858			
			C	0.289	2.327		1	1	61.858			
Т6	2039.33	9509.36	Α	0.261	2.404	6	1	1	62.459	1283.49	64.17	С
60.00-40.00			В	0.261	2.404		1	1	62.459			
			C	0.261	2.404		1	1	62.459			
T7	1985.95	9425.41	Α	0.237	2.478	5	1	1	62.524	1175.43	58.77	С
40.00-20.00			В	0.237	2.478		1	1	62.524			
			C	0.237	2.478		1	1	62.524			_
T8 20.00-0.00	1924.50	9695.52	Α	0.216	2.545	5	1	1	62.566	1053.83	52.69	С
			В	0.216	2.545		1	1	62.566			
			C	0.216	2.545		1	1	62.566			
Sum Weight:	11996.20	65923.71						OTM	716.76	9280.18		
Ŭ									kip-ft			

Tower Forces - With Ice - Wind 45 To Face

Section	Add	Self	F	е	C_F	q=	D_F	D_R	$A_{\mathcal{E}}$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
l			С			psf		l V	a?		1.0	
ft	lь	lb	e					0	fr²	lb	plf	
T1	81.17	6953.50	Α	0.508	1.89	7	0.825	1	67.338	831.00	41.55	С
160.00-140.00			В	0.508	1.89		0.825	1	67.338			
			C	0.508	1.89		0.825	1	67.338			_
T2	320.94	6656.50	Α	0.448	1.976	7	0.825	1	63.217	858.29	42.91	С
140.00-120.00			В	0.448	1.976	1	0.825	1	63.217			
			С	0.448	1.976		0.825	1	63.217			
T3	1807.16	7471.73	Α	0.407	2.05	7	0.825	1	67.374	1325.22	66.26	С
120.00-100.00			В	0.407	2.05		0.825	1	67.374			
l			C	0.407	2.05		0.825	1	67.374			
T4	1842.00	7236.58	Α	0.345	2.184	7	0.825	1	64.032	1317.33	65.87	С
100.00-80.00			В	0.345	2.184		0.825	1	64.032			
l .			C	0.345	2.184		0.825	1	64.032			
T5	1995.16	8975.12	Α	0.289	2.327	6	0.825	1	59.476	1285.68	64.28	С
80.00-60.00			В	0.289	2.327		0.825	1	59.476			
			C	0.289	2.327		0.825	1	59.476			_
Т6	2039.33	9509.36	Α	0.261	2.404	6	0.825	1	59.937	1252.81	62.64	С
60.00-40.00			В	0.261	2.404		0.825	1	59.937			
			C	0.261	2.404		0.825	1	59.937			
Т7	1985.95	9425.41	A	0.237	2.478	5	0.825	1	59.856	1145.39	57.27	С
40.00-20.00			В	0.237	2.478		0.825	1	59.856			
			C	0.237	2.478		0.825	1	59.856			
T8 20.00-0.00	1924.50	9695.52	Α	0.216	2.545	5	0.825	1	59.746	1025.62	51.28	С
			В	0.216	2.545		0.825	1	59.746			
l			С	0.216	2.545		0.825	1	59.746			
Sum Weight:	11996.20	65923.71						OTM	697.82	9041.35		
J									kip-fl			

Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405

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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Tower Forces - With Ice - Wind 60 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf						l
ft	lb	lb	е						ft ²	lb	plf	
T1	81.17	6953.50	Α	0.508	1.89	7	0.8	1	67.014	827.10	41.35	С
160.00-140.00			В	0.508	1.89		0.8	1	67.014			
			C	0.508	1.89		0.8	1	67.014			
T2	320.94	6656.50	Α	0.448	1.976	7	0.8	1	62.918	854.64	42.73	l c
140.00-120.00			В	0.448	1.976		0.8	1	62.918			
			С	0.448	1.976		0.8	1	62.918			
T3	1807.16	7471.73	Α	0.407	2.05	7	0.8	1	66.980	1320.39	66.02	С
120.00-100.00			В	0.407	2.05		0.8	1	66.980			
			С	0.407	2.05		0.8	1	66.980			
T4	1842.00	7236.58	Α	0.345	2.184	7	0.8	1	63.653	1312.60	65.63	С
100.00-80.00			В	0.345	2.184		0.8	1	63.653		- 2	
			C	0.345	2.184		0.8	1	63,653			
T5	1995.16	8975.12	Α	0.289	2.327	6	0.8	1	59.135	1281.38	64.07	С
80.00-60.00			В	0.289	2.327		0.8	1	59.135			
			C	0.289	2.327		0.8	1	59.135			
Т6	2039.33	9509.36	A	0.261	2.404	6	0.8	1	59.576	1248.43	62.42	С
60.00-40.00			В	0.261	2.404		0.8	1	59.576			
			C	0.261	2.404		0.8	1	59.576			
T7	1985.95	9425.41	Α	0.237	2.478	5	0.8	1	59.475	1141.10	57.05	С
40.00-20.00			В	0.237	2.478		0.8	1	59.475		125	
			C	0.237	2.478		0.8	1	59.475			
T8 20.00-0.00	1924.50	9695.52	Α	0.216	2.545	5	0.8	1	59.343	1021.59	51.08	С
			В	0.216	2.545		0.8	1	59.343			_
			С	0.216	2.545		0.8	ī	59.343			
Sum Weight:	11996.20	65923.71					- 1	OTM	695.11	9007.23		
									kip-ft	. 507125		

Tower Forces - With Ice - Wind 90 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а			1-	1	_ ^	-	- 1		Face
			С			psf				1		
ft	lb	lb	е						ft^2	lb	plf	
T1	81.17	6953.50	Α	0.508	1.89	7	0.85	1	67.662	834.90	41.74	С
160.00-140.00			В	0.508	1.89		0.85	1	67.662	24943256363	V-0.700-VV	980
			C	0.508	1.89		0.85	1	67.662			
T2	320.94	6656.50	Α	0.448	1.976	7	0.85	1	63.516	861.95	43.10	С
140.00-120.00			В	0.448	1.976		0.85	1	63.516			
			C	0.448	1.976		0.85	1	63.516			
T3	1807.16	7471.73	Α	0.407	2.05	7	0.85	1	67.768	1330.05	66.50	С
120.00-100.00			В	0.407	2.05		0.85	1	67.768			
			C	0.407	2.05		0.85	1	67.768			
T4	1842.00	7236.58	Α	0.345	2.184	7	0.85	1	64.410	1322.06	66.10	С
100.00-80.00			В	0.345	2.184		0.85	1	64.410			
			C	0.345	2.184		0.85	1	64.410			
T5	1995.16	8975.12	Α	0.289	2.327	6	0.85	1	59.816	1289.98	64.50	C

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Job		Page
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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	W ²	Ctrl.
Elevation	Weight	Weight	а									Face
			c			psf			f_i^2	lb	nH	
ft	lb	lb	e						- 21	10	plf	
80.00-60.00			В	0.289	2.327		0.85	1	59.816			
			С	0.289	2.327		0.85	1	59.816			
Т6	2039.33	9509.36	Α	0.261	2.404	6	0.85	1	60.297	1257.19	62.86	С
60.00-40.00			В	0.261	2.404		0.85	1	60.297			
			С	0.261	2.404		0.85	1	60.297			
T7	1985.95	9425.41	Α	0.237	2.478	5	0.85	1	60.237	1149.68	57.48	С
40.00-20.00			В	0.237	2.478		0.85	1	60.237			
			С	0.237	2.478		0.85	1	60.237			
T8 20.00-0.00	1924.50	9695.52	Α	0.216	2.545	5	0.85	1	60.149	1029.65	51.48	С
			В	0.216	2.545		0.85	1	60.149			
			С	0.216	2.545		0.85	1	60.149			
Sum Weight:	11996.20	65923.71						OTM	700.52	9075.46		
1									kip-ft			

Tower Forces - Service - Wind Normal To Face

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
	- J	Ū	с			psf						
ft	lb	lb	е						ft²	lb	plf	
T1	19.00	2678.27	Α	0.217	2.54	11	1	1	28.354	671.91	33.60	С
160.00-140.00			В	0.217	2.54		1	1	28.354			
			C	0.217	2.54		1	1	28.354			
T2	76.00	2639.67	A	0.191	2.627	10	1	1	27.262	680.04	34.00	С
140.00-120.00			В	0:191	2.627		1	1	27.262			
			С	0.191	2.627		1	1	27.262			
Т3	241.00	3230.97	Α	0.18	2.663	10	1	1	31.807	989.38	49.47	С
120.00-100.00			В	0.18	2.663		1	1	31.807			
120.00 100.00			С	0.18	2.663		1	1	31.807			
T4	247.60	3231.19	Α	0.153	2.761	10	1	1	31.083	968.67	48.43	С
100.00-80.00			В	0.153	2.761		1	1	31.083			
100.00 00.00			С	0.153	2.761		1	1	31.083			
T5	267.40	4725.12	A	0.134	2.832	9	1	1	30.393	951.32	47.57	C
80.00-60.00	20,,,,,		В	0.134	2.832		1	1	30.393			
00.00 00.00			C	0.134	2.832		1	1	30.393			l
Т6	271.90	5314.76	Ā	0.125	2.865	9	1	1	32.028	935.83	46.79	С
60.00-40.00	271.70		В	0.125	2.865		1	1	32.028			l
00.00-40.00			Гc	0.125	2.865		1	1	32.028			l
Т7	273.40	5407.03	Ā	0.115	2.905	8	1	1	32.843	866.79	43.34	С
40.00-20.00	2,5		В	0.115	2.905		1	1	32.843			l
40.00-20.00			Īc	0.115	2.905	1	1	1	32.843			l
T8 20.00-0.00	282.40	6073.32	Ă	0.11	2.924	7	1	1	34.620	795.88	39.79	С
16 20.00-0.00	202.70	00,5.52	В	0.11	2.924		1	1	34.620			
			ľ	0.11	2.924		1	1	34.620			
Sum Weight:	1678.70	33300.33	ľ	"	,,		-	OTM	532.55	6859.83		l
Sum weight.	10/6.70	35500.55							kip-fl			

Tower Forces - Service - Wind 45 To Face

Centek Engineering Inc. 63-2 North Branford Rd.

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

Job		Page
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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client	· · · · · · · · · · · · · · · · · · ·	Designed by
	Verizon	TJL

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a			-						Face
			c			psf						
ft	lb	lb	e						ft ²	lb	plf	
T1	19.00	2678.27	Α	0.217	2.54	11	0.825	- 1	26.087	619.06	30.95	С
160.00-140.00			В	0.217	2.54		0.825	1	26.087			
			C	0.217	2.54		0.825	1	26.087			
Т2	76.00	2639.67	Α	0.191	2.627	10	0.825	1	25.168	631.06	31.55	С
140.00-120.00			В	0.191	2.627		0.825	1	25.168			
			C	0.191	2.627		0.825	1	25.168			
T3	241.00	3230.97	Α	0.18	2.663	10	0.825	1	29.048	926.20	46.31	C
120.00-100.00	1		В	0.18	2.663		0.825	1	29.048			l
			C	0.18	2.663		0.825	1	29.048			l
T4	247.60	3231.19	Α	0.153	2.761	10	0.825	1	28.433	908.35	45,42	l c
100.00-80.00			В	0.153	2.761		0.825	1	28.433			l
			C	0.153	2.761		0.825	1	28.433			l
T5	267.40	4725.12	Α	0.134	2.832	9	0.825	1	28.010	898.56	44.93	l c
80.00-60.00			В	0.134	2.832		0.825	1	28.010			l
			C	0.134	2.832		0.825	1	28.010			l
Т6	271.90	5314.76	Α	0.125	2.865	9	0.825	1	29.505	883.20	44.16	l c
60.00-40.00			В	0.125	2.865		0.825	1	29.505			
			C	0.125	2.865		0.825	1	29.505			l
T7	273.40	5407.03	Α	0.115	2.905	8	0.825	1	30.175	816.10	40.80	l c
40.00-20.00			В	0.115	2.905		0.825	1	30.175			
			C	0.115	2.905		0.825	1	30.175			
T8 20.00-0,00	282.40	6073.32	Α	0.11	2.924	7	0.825	1	31.800	749.21	37.46	l c
			В	0.11	2.924		0.825	1	31.800			
			C	0.11	2.924		0.825	1	31.800			
Sum Weight:	1678.70	33300.33						OTM	497.57	6431.74		
									kip-ft			

Tower Forces - Service - Wind 60 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	N)(Čtrl.
Elevation	Weight	Weight	a			4-	1		2			Face
	_		c			psf						1 410
ft	lb	lb	e			1 3			ft ²	lb	plf	
T1	19.00	2678.27	Α	0.217	2.54	11	0.8	1	25.763	611.51	30.58	С
160.00-140.00			В	0.217	2.54		0.8	1	25.763			
			С	0.217	2.54		0.8	1	25.763			
T2	76.00	2639.67	Α	0.191	2.627	10	0.8	1	24.869	624.07	31.20	С
140.00-120.00			В	0.191	2.627		0.8	1	24.869			
			C	0.191	2.627		0.8	1	24.869			
T3	241.00	3230.97	Α	0.18	2.663	10	0.8	1	28.653	917.17	45.86	С
120.00-100.00			В	0.18	2.663		0.8	1	28.653			
			C	0.18	2.663		0.8	1	28.653			
T4	247.60	3231.19	Α	0.153	2.761	10	0.8	1	28.054	899.73	44.99	С
100.00-80.00			В	0.153	2.761		0.8	1	28.054			
			C	0.153	2.761		0.8	1	28.054			
T5	267.40	4725.12	Α	0.134	2.832	9	0.8	1	27.670	891.03	44.55	С
80.00-60.00			В	0.134	2.832		0.8	1	27.670			
			C	0.134	2.832		0.8	1	27.670			
T6	271.90	5314.76	Α	0.125	2.865	9	0.8	1	29.145	875.68	43.78	С
60.00-40.00			В	0.125	2.865		0.8	1	29.145			
			C	0.125	2.865		0.8	1	29.145			
T7	273.40	5407.03	Α	0.115	2.905	8	0.8	1	29.794	808.85	40.44	С
40.00-20.00			В	0.115	2.905		0.8	1	29.794		- 50	_
	- 1		C	0.115	2.905		0.8	1	29.794			

Centek Engineering Inc. 63-2 North Branford Rd.

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

Job	23134.00 - East Haddam	Page 26 of 42
Project	160-ft Lattice Tower (CSP #52)	Date 09:38:07 10/09/23
Client	Verizon	Designed by TJL

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а			1-						Face
2.074		, o	c			psf			, ,			
ft	lb	lb	е						ft²	lb	plf	
T8 20.00-0.00	282.40	6073.32	Α	0.11	2.924	7	0.8	1	31.398	742.54	37.13	С
10 20100 0100			В	0.11	2.924		0.8	1	31.398			
			C	0.11	2.924		0.8	1	31.398			
Sum Weight:	1678.70	33300.33						OTM	492.57	6370.59		
Batti Weight.	10/0//0	22200111							kip-fl			

Tower Forces - Service - Wind 90 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_{E}	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
	, i		c			psf			,			
ft	lb	lь	e						ft ²	lb	plf	
T1	19.00	2678.27	Α	0.217	2.54	11	0.85	1	26.410	626.61	31.33	С
160.00-140.00			В	0.217	2.54		0.85	1	26.410			
			C	0.217	2.54		0.85	1	26.410			
T2	76.00	2639.67	Α	0.191	2.627	10	0.85	1	25.467	638.06	31.90	С
140.00-120.00			В	0.191	2.627		0.85	1	25.467			
			C	0.191	2.627		0.85	1	25.467			
T3	241.00	3230.97	Α	0.18	2.663	10	0.85	1	29.442	935.22	46.76	С
120.00-100.00			В	0.18	2.663		0.85	1	29.442			
	- 1		C	0.18	2.663		0.85	1	29.442			
T4	247.60	3231.19	Α	0.153	2.761	10	0.85	1	28.812	916.97	45.85	С
100.00-80.00			В	0.153	2.761		0.85	1	28.812			l
			C	0.153	2.761		0.85	1	28.812			l
T5	267.40	4725.12	Α	0.134	2.832	9	0.85	1	28.350	906.10	45.30	С
80.00-60.00			В	0.134	2.832		0.85	1	28.350			l
			C	0.134	2.832		0.85	1	28.350			l
Т6	271.90	5314.76	A	0.125	2.865	9	0.85	1	29.865	890.72	44.54	С
60.00-40.00			В	0.125	2.865		0.85	1	29.865			l
00,00			С	0.125	2.865		0.85	1	29.865			l
T7	273.40	5407.03	Α	0.115	2.905	8	0.85	1	30.556	823.34	41.17	С
40.00-20.00			В	0.115	2.905		0.85	1	30.556			l
10.00 20.00			С	0.115	2.905		0.85	1	30.556			l
T8 20.00-0.00	282.40	6073.32	Α	0.11	2.924	7	0.85	1	32.203	755.88	37.79	С
2021.30 0.00			В	0.11	2.924		0.85	1	32.203			
			С	0.11	2.924		0.85	1	32.203			
Sum Weight:	1678.70	33300.33						OTM	502.56	6492.90		l
Dun Weight.	1070170								kip-fl			

Force Totals

Load	Vertical	Sum of	Sum of	Sum of	Sum of	Sum of Torques
Case	Forces	Forces	Forces	Overturning	Overturning	
Case	1 orces	X	Z	Moments, Mx	Moments, Mz	
	lb	lb	lb	kip-ft	kip-ft	kip-ft
Leg Weight	21474.07					
Bracing Weight	11826.26					
Total Member Self-Weight	33300.33			11.10	3.14	
Total Weight	41935.67			11.10	3.14	ALC: EVIEW

Centek Engineering Inc. 63-2 North Branford Rd.

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

Job		Page
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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client	V.	Designed by
	Verizon	l TJL

Load	Vertical	Sum of	Sum of	Sum of	Sum of	Sum of Torques
Case	Forces	Forces	Forces	Overturning	Overturning	Sum of Torques
	11111	X	Z	Moments, M,	Moments, M.	
	lb	lb	lb	kip-ft	kip-ft	kip-ft
Wind 0 deg - No Ice		0.00	-48921.16	-4496.78	3.14	-34.65
Wind 30 deg - No Ice		23531.79	40758.25	-3761.37	-2174.89	-7.17
Wind 45 deg - No Ice		33060.05	-33060.05	-3051.21	-3059.17	7.80
Wind 60 deg - No Ice	10000	40222.01	-23222.19	-2141.63	-3725.50	22.24
Wind 90 deg - No Ice		47063.57	0.00	11.10	-4352.93	45.69
Wind 120 deg - No Ice		42366.97	24460.58	2265.04	-3900.80	56.89
Wind 135 deg - No Ice	Total Control	33935.73	33935.73	3144.98	-3130.74	56.81
Wind 150 deg - No Ice	- Carl - V	23531.79	40758.25	3783.57	-2174.89	52.85
Wind 180 deg - No Ice		0.00	46444.38	4316.56	3.14	34.65
Wind 210 deg - No Ice	50 %	-23531.79	40758.25	3783.57	2181.17	7.17
Wind 225 deg - No Ice	W 100 100 100 100 100 100 100 100 100 10	-33060.05	33060.05	3073.41	3065.45	-7.80
Wind 240 deg - No Ice	KIND WILL	-42366.97	24460.58	2265.04	3907.08	-22.24
Wind 270 deg - No Ice		-47063.57	0.00	11.10	4359.21	-45.69
Wind 300 deg - No Ice	* y 23	-40222.01	-23222.19	-2141.63	3731.78	-56.89
Wind 315 deg - No Ice	10 100 200	-33060.05	-33060.05	-3051.21	3065.45	-56.81
Wind 330 deg - No Ice	100000	-23531.79	-40758.25	-3761.37	2181.17	-52.85
Member Ice	32623.38		THE PERSON NAMED IN			
Total Weight Ice	93017.72		The same of the sa	64.83	46.20	
Wind 0 deg - Ice	A 1 1 1 14 L	0.00	-12099.82	-1007.49	46.20	-12.11
Wind 30 deg - Ice		5947.55	-10301.47	-849.76	-481.84	-2.93
Wind 45 deg - Ice		8386.99	-8386.99	-680.02	-698.65	2.12
Wind 60 deg - Ice		10242.37	-5913.43	-460.51	-863.71	7.03
Wind 90 deg - Ice	KIT, LES,	11895.11	0.00	64.83	-1009,88	15.10
Wind 120 deg - Ice	THE PERSON NAMED IN	10478.75	6049.91	600.98	-882.46	19.13
Wind 135 deg - Ice	10000	8483.49	8483.49	817.33	-706.30	19.24
Wind 150 deg - Ice		5947.55	10301.47	979.41	-481.84	18.04
Wind 180 deg - Ice	1 2 1500 2 7	0.00	11826.87	1115.49	46.20	12.11
Wind 210 deg - Ice	1000	-5947.55	10301.47	979.41	574.23	2.93
Wind 225 deg - Ice		-8386.99	8386.99	809.67	791.04	-2.12
Wind 240 deg - Ice		-10478.75	6049.91	600.98	974.85	-7.03
Wind 270 deg - Ice	100 100 100 100 100 100 100 100 100 100	-11895.11	0.00	64.83	1102.27	-15.10
Wind 300 deg - Ice		-10242.37	-5913.43	-460.51	956.10	-19.13
Wind 315 deg - Ice	1000	-8386.99	-8386.99	-680.02	791.04	-19.24
Wind 330 deg - Ice	41005.58	-5947.55	-10301.47	-849.76	574.23	-18.04
Total Weight	41935.67	0.00	0.00.00	11.10	3.14	EVEN TO A
Wind 0 deg - Service	100000000000000000000000000000000000000	0.00	-9693.53	-890.96	0.21	-6.94
Wind 30 deg - Service Wind 45 deg - Service	No. of the last of	4663.30	-8077.07	-745.22	-431.79	-1.55
Wind 60 deg - Service	Name of Street, or other Persons	6551.66	-6551.66	-604.38	-607.20	1.40
Wind 90 deg - Service	A PERSON NAMED IN	7971.15 9326.60	-4602.14	-423.97	-739.39	4.26
Wind 120 deg - Service	10 10 10 10 10	8394.84	0.00	3.04	-863.80	8.92
Wind 135 deg - Service	The second second	6724.63	4846. 76 6724.63	450.04	-774.02	11.19
Wind 150 deg - Service	A STATE OF THE PARTY OF THE PAR	4663.30	8077.07	624.59	-621.34	11.21
Wind 180 deg - Service	2000	0.00	9204.29	751.30	-431.79	10.47
Wind 210 deg - Service		-4663.30	8077.07	857.06 751.30	0.21 432.22	6.94
Wind 216 deg - Service	ALC: VI	-6551.66	6551.66	610.46	607.63	1.55
Wind 240 deg - Service		-8394.84	4846.76	450.04	774.44	-1.40
Wind 270 deg - Service		-9326.60	0.00	3.04	864.23	-4.26 -8.92
Wind 300 deg - Service	The same	-7971.15	4602.14	423.97	739.82	570
Wind 315 deg - Service		-6551.66	-6551.66	-604.38	607.63	-11.19
Wind 330 deg - Service		-4663.30	-8077.07	-745.22	432.22	-11.21 -10.47
			-00//.0/	-/43.22	432.22	-10.4/

Load Combinations

Comb	,
Ma	

Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

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	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Comb.		Description	ī	
No.	7 10 1			
1	Dead Only			
2	1.2 Dead+1.0 Wind 0 deg - No Ice			
3	0.9 Dead+1.0 Wind 0 deg - No Ice			
4	1.2 Dead+1.0 Wind 30 deg - No Ice			
5	0.9 Dead+1.0 Wind 30 deg - No Ice			
6	1.2 Dead+1.0 Wind 45 deg - No Ice			
7	0.9 Dead+1.0 Wind 45 deg - No Ice			
8	1.2 Dead+1.0 Wind 60 deg - No Ice			
9	0.9 Dead+1.0 Wind 60 deg - No Ice			
10	1.2 Dead+1.0 Wind 90 deg - No Ice			
11	0.9 Dead+1.0 Wind 90 deg - No Ice			
12	1.2 Dead+1.0 Wind 120 deg - No Ice			
13	0.9 Dead+1.0 Wind 120 deg - No Ice			
14	1.2 Dead+1.0 Wind 135 deg - No Ice			
15	0.9 Dead+1.0 Wind 135 deg - No Ice			
16	1.2 Dead+1.0 Wind 150 deg - No Ice			
17	0.9 Dead+1.0 Wind 150 deg - No Ice			
18	1.2 Dead+1.0 Wind 180 deg - No Ice			
19	0.9 Dead+1.0 Wind 180 deg - No Ice			
20	1.2 Dead+1.0 Wind 210 deg - No Ice			
21	0.9 Dead+1.0 Wind 210 deg - No Ice			
22	1.2 Dead+1.0 Wind 225 deg - No Ice			
23	0.9 Dead+1.0 Wind 225 deg - No Ice			
24	1.2 Dead+1.0 Wind 240 deg - No Ice			
25	0.9 Dead+1.0 Wind 240 deg - No Ice			
26	1.2 Dead+1.0 Wind 270 deg - No Ice			
27	0.9 Dead+1.0 Wind 270 deg - No Ice			
28	1.2 Dead+1.0 Wind 300 deg - No Ice			
29	0.9 Dead+1.0 Wind 300 deg - No Ice			
30	1.2 Dead+1.0 Wind 315 deg - No Ice			
31	0.9 Dead+1.0 Wind 315 deg - No Ice			
32	1.2 Dead+1.0 Wind 330 deg - No Ice			
33	0.9 Dead+1.0 Wind 330 deg - No Ice			
34	1.2 Dead+1.0 Ice+1.0 Temp			
35	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp			
36	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp			
37	1.2 Dead+1.0 Wind 45 deg+1.0 Ice+1.0 Temp			
38	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp			
39	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp			
40	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp			
41	1.2 Dead+1.0 Wind 135 deg+1.0 Ice+1.0 Temp			
42	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp			
43	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp			
44	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp			
45	1.2 Dead+1.0 Wind 225 deg+1.0 Ice+1.0 Temp			
46	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp			
47	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp			
48	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp			
49	1.2 Dead+1.0 Wind 315 deg+1.0 Ice+1.0 Temp			
50	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp			
51	Dead+Wind 0 deg - Service			
52	Dead+Wind 30 deg - Service			
53	Dead+Wind 45 deg - Service			
54	Dead+Wind 60 deg - Service			
55	Dead÷Wind 90 deg - Service			
56	Dead÷Wind 120 deg - Service			
57	Dead+Wind 135 deg - Service			
58	Dead+Wind 150 deg - Service			
59	Dead+Wind 180 deg - Service			
60	Dead+Wind 210 deg - Service			
	Total Itting Tio god Souther			
61	Dead+Wind 225 deg - Service			

Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
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	Verizon	TJL

Comb.		Description
No.		·
63	Dead÷Wind 270 deg - Service	
64	Dead+Wind 300 deg - Service	
65	Dead+Wind 315 deg - Service	
66	Dead+Wind 330 deg - Service	

Maximum Member Forces

Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Туре		Load		Moment	Moment
				Comb.	lb	kip-ft	kip-ft
T1	160 - 140	Leg	Max Tension	9	3407.47	-1.89	-0.42
			Max. Compression	46	-8111.62	0.89	0.08
			Max. Mx	8	392.98	4.55	-0.68
			Max. My	30	-4403.30	0.85	-7.07
			Max. Vy	24	-1547.63	2.82	0.63
			Max. Vx	14	2001.26	-0.51	-2.51
		Diagonal	Max Tension	29	4352.56	0.00	0.00
			Max. Compression	12	-4634.90	0.00	0.00
			Max. Mx	43	676.12	0.06	-0.00
			Max. My	28	-3135.12	0.02	0.01
			Max. Vy	43	-44.48	0.06	-0.00
			Max. Vx	28	-1.63	0.00	0.00
		Top Girt	Max Tension	9	237.95	0.00	0.00
		-	Max. Compression	2	-321.83	0.00	0.00
			Max. Mx	34	-120.19	-0.12	0.00
			Max. My	14	27.33	0.00	0.00
			Max. Vy	34	58.29	0.00	0.00
			Max. Vx	14	-0.00	0.00	0.00
T2	140 - 120	Leg	Max Tension	19	27758.43	-0.36	0.07
		- 6	Max. Compression	24	-34721.03	1.75	0.24
			Max. Mx	24	-20227.61	2.82	0.63
			Max. My	14	623.58	-0.51	-2.51
			Max. Vy	24	459.18	2.82	0.63
			Max. Vx	26	-467.32	-0.26	-2.32
		Diagonal	Max Tension	10	7339.83	0.00	0.00
			Max. Compression	10	-7444.27	0.00	0.00
			Max. Mx	44	801.03	0.08	0.01
			Max. My	26	-7337.16	0.01	0.03
			Max. Vy	44	54.50	0.08	0.01
			Max. Vx	26	-6.19	0.00	0.00
T3	120 - 100	Leg	Max Tension	19	58281.21	-1.60	-0.05
		Ü	Max. Compression	24	-70895.41	1.73	-0.00
			Max. Mx	24	-50298.77	1.75	0.24
			Max. My	16	-6486.09	-0.03	-2.62
			Max. Vy	18	-1957.67	-1.27	-0.01
			Max. Vx	10	1938.58	0.18	-0.70
		Diagonal	Max Tension	10	8616.98	0.00	0.00
		Ü	Max. Compression	10	-8522.99	0.00	0.00
			Max. Mx	43	1511.02	0.10	-0.01
			Max. My	10	-8404.40	0.01	-0.02
			Max. Vy	43	63.84	0.10	-0.01
			Max. Vx	41	4.00	0.00	0.00
		Top Girt	Max Tension	18	517.14	0.00	0.00
		r	Max. Compression	25	-274.47	0.00	0.00
			Max. Mx	34	288.42	-0.18	0.00
			Max. My	41	306.18	0.00	0.01
			Max. Vy	34	70.91	0.00	0.00
			Max. Vx	41	-2.05	0.00	0.00
T4	100 - 80	Leg	Max Tension	19	91081.82	-2.51	-0.04
1 1	100 00	205	MINU I CHOICH	17	71001.02	-Z.JI	-0.04

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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Section	Elevation	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Ax Momen
No_*	ft	Туре		Comb.	lb	kip-ft	kip-ft
			Max. Compression	24	-107745.28	3.54	0.08
			Max. Mx	12	-107745.28	3.54	-0.40
				16	-7985.45	0.00	-3.79
			Max. My	38	382.52	-2.19	-0.06
			Max. Vy		650.34	0.00	-3.79
		·	Max. Vx	16	8894.90	0.00	0.00
		Diagonal	Max Tension	10		0.00	0.00
			Max. Compression	10	-9079.75		-0.02
			Max. Mx	43	1844.12	0.12	0.02
			Max. My	26	-8669.91	0.02	-0.02
			Max. Vy	43	72.72	0.12	
			Max. Vx	48	-4.41	0.00	0.00
T5	80 - 60	Leg	Max Tension	19	112463.09	-3.38	-0.32
			Max. Compression	24	-131866.25	3.03	0.18
			Max. Mx	8	108696.83	-3.60	-0.21
			Max. My	16	-9434.67	-0.40	-6.18
			Max. Vy	3	436.18	3.54	0.31
			Max. Vx	14	639.65	-1.27	-6.10
		Diagonal	Max Tension	10	12975.21	0.00	0.00
		-	Max. Compression	12	-13766.92	0.00	0.00
			Max. Mx	43	1073.40	-0.39	0.06
			Max. My	26	-13330.42	-0.05	-0.09
			Max. Vy	43	-139.46	-0.39	0.06
			Max. Vx	48	11.21	0.00	0.00
T6	60 - 40	Leg	Max Tension	19	144157.91	-3.50	-0.31
10	00-40	LUE	Max. Compression	24	-169577.95	5.28	0.12
			Max. Mx	12	-169350.51	5.29	-0.39
			Max. My	16	-10499.37	-0.40	-6.18
			Max. Vy	8	-526.49	-3.60	-0.21
			Max. Vx	16	-913.61	-0.40	-6.18
		D'I	Max Tension	10	13823.89	0.00	0.00
		Diagonal		10	-14013.69	0.00	0.00
			Max. Compression	43	3857.19	-0.41	-0.07
			Max. Mx	42		-0.41	0.07
			Max. My		3437.06	-0.41	-0.07
			Max. Vy	43	-151.22	0.00	0.00
			Max. Vx	48	-10.73		
T7	40 - 20	Leg	Max Tension	19	173093.68	-4.66	-0.27
			Max. Compression	24	-203992.13	3.15	0.12
			Max. Mx	12	-202236.69	5.29	-0.39
			Max. My	16	-14254.53	-0.59	-8.55
			Max. Vy	38	-442.19	-3.75	-0.05
			Max. Vx	16	822.15	-0.59	-8.55
		Diagonal	Max Tension	10	13629.47	0.00	0.00
28		-	Max. Compression	10	-13995.98	0.00	0.00
			Max. Mx	43	620.25	-0.52	0.08
			Max. My	26	-13659.06	-0.12	-0.10
			Max. Vy	43	-167.13	-0.52	0.08
			Max. Vx	48	11.85	0.00	0.00
T8	20 - 0	Leg	Max Tension	19	200394.89	-3.90	-0.32
10	20 - 0		Max. Compression	24	-237151.30	-0.00	-0.00
			Max. Mx	8	195977.49	-4.05	-0.16
			Max. My	16	-15086.31	-0.59	-8.55
			Max. Vy	8	-559.15	4.05	-0.16
			Max. Vx	16	-855.69	-0.59	-8.55
		DiaI	Max Tension	10	14272.70	0.00	0.00
		Diagonal		10	-14796.09	0.00	0.00
			Max. Compression			-0.48	-0.08
			Max. Mx	44	4408.41	-0.48	0.08
			Max. My	41	3565.53	-0.48	-0.08
			Max. Vy	44	-168.11		
			Max. Vx	41	10.93	0.00	0.00

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Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Maximum Reactions

Location	Condition	Gov	Vertical	Horizontal, X	Horizontal,
		Load	lb	lb	lb
		Comb.			
Leg C	Max. Vert	24	254435.74	26327.43	-14525.32
*	Max. H _x	24	254435.74	26327.43	-14525.32
	Max. H ₂	7	-206962.20	-21851.75	13460.34
	Min. Vert	9	-213390.31	-23162.33	12702.64
	Min. H _x	9	-213390.31	-23162.33	12702.64
	Min. H _z	22	237355.85	23765.65	-14576.41
Leg B	Max. Vert	12	254091.95	-26773.99	-13740.18
	Max. H _x	29	-213647.97	23623.38	11914.24
	$Max. H_z$	31	-207220.88	22499.79	12342.03
	Min. Vert	29	-213647.97	23623.38	11914.24
	Min. H _x	12	254091.95	-26773.99	-13740.18
	Min. Hz	14	242153.85	-24985.42	-13839.65
Leg A	Max. Vert	2	253211.46	-903.12	30041.81
	Max. H _x	25	-106449.00	2703.10	-13497.20
	$Max. H_z$	2	253211.46	-903.12	30041,81
	Min. Vert	19	-214307.23	913.32	-26426.13
	$Min. H_x$	8	129316.44	-2565.00	14904.95
	Min. H _z	19	-214307.23	913.32	-26426.13

Tower Mast Reaction Summary

Load Combination	Vertical	Shearx	Shearz	Overturning Moment, M.	Overturning Moment, M.	Torque
	1b	lb	lb	kip-ft	kip-fi	kip-ft
Dead Only	41935.67	0.00	-0.00	11,10	3.14	-0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	50322.80	-0.01	-48920.82	-4504.73	3.83	-34.66
0.9 Dead+1.0 Wind 0 deg - No Ice	37742.10	-0.01	48920.93	-4505.52	2.89	-34.66
1.2 Dead+1.0 Wind 30 deg - No Ice	50322.80	23531.48	-40757.92	-3767.66	-2179.19	-7.15
0.9 Dead+1.0 Wind 30 deg - No Ice	37742.10	23531.53	-40758.02	-3768.87	-2178.90	-7.14
1.2 Dead+1.0 Wind 45 deg - No Ice	50322.80	33059.64	-33059.74	-3055.89	-3065.50	7.84
0.9 Dead+1.0 Wind 45 deg - No Ice	37742.10	33059.72	-33059.82	-3057.49	-3064.70	7.83
1.2 Dead+1.0 Wind 60 deg - No Ice	50322.80	40221.54	-23221.91	-2144.22	-3733.36	22.28
0.9 Dead+1.0 Wind 60 deg - No Ice	37742.10	40221.64	-23221.97	-2146.34	-3732.17	22.27
1.2 Dead+1.0 Wind 90 deg - No Ice	50322.80	47063.13	0.12	13.45	-4362.19	45.74
0.9 Dead+1.0 Wind 90 deg - No Ice	37742.10	47063.24	0.12	10.10	-4360.65	45.72
1.2 Dead+1.0 Wind 120 deg - No Ice	50322.80	42366.67	24460.42	2272.49	-3908.97	56.95
0.9 Dead+1.0 Wind 120 deg - No Ice	37742.10	42366.76	24460.48	2267.87	-3907.70	56.93
1.2 Dead+1.0 Wind 135 deg - No Ice	50322.80	33935.51	33935.42	3154.40	-3137.16	56.86

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Project	160-ft Lattice Tower (CSP #52)	Date 09:38:07 10/09/23
Client	Verizon	Designed by TJL

Load Combination	Vertical	Shear _x	Shear <u>:</u>	Overturning Moment, M_x	Overturning Moment, M <u>.</u>	Torque
	lb	lb	lb	kip-ft	kip-ft	kip-ft
0.9 Dead+1.0 Wind 135 deg -	37742.10	33935.59	33935.50	3149.28	-3136.32	56.85
No Ice 1.2 Dead+1.0 Wind 150 deg -	50322.80	23531.65	40757.82	3794.43	-2179.15	52.90
No Ice	27742 10	23531.71	40757.92	3788.93	-2178.85	52.89
0.9 Dead+1.0 Wind 150 deg -	37742.10	23331.71	40/3/.92	3700.23	21,0.03	52.07
No Ice 1.2 Dead+1.0 Wind 180 deg -	50322.80	-0.00	46443.83	4328.60	3.82	34.66
No Ice 0.9 Dead+1.0 Wind 180 deg -	37742.10	-0.00	46443.95	4322.80	2.88	34.66
No Ice 1.2 Dead+1.0 Wind 210 deg -	50322.80	-23531.66	40757.81	3794.38	2186.76	7.15
No Ice 0.9 Dead+1.0 Wind 210 deg -	37742.10	-23531.71	40757.91	3788.89	2184.58	7.14
No Ice 1.2 Dead+1.0 Wind 225 deg -	50322.80	-33059.85	33059.75	3082.64	3073.03	-7.83
No Ice 0.9 Dead+1.0 Wind 225 deg -	37742.10	-33059.93	33059.82	3077.55	3070.35	-7.83
No Ice 1.2 Dead+1.0 Wind 240 deg -	50322.80	-42366.67	24460.41	2272.45	3916.55	-22.28
No Ice 0.9 Dead+1.0 Wind 240 deg -	37742.10	-42366.77	24460.47	2267.82	3913.39	-22.27
No Ice 1.2 Dead+1.0 Wind 270 deg -	50322.80	-47063.13	0.12	13.45	4369.74	-45.74
No Ice 0.9 Dead+1.0 Wind 270 deg -	37742.10	-47063.24	0.12	10.10	4366.32	-45.72
No Ice 1.2 Dead+1.0 Wind 300 deg -	50322.80	-40221.55	-23221.91	-2144.18	3740.94	-56.95
No Ice 0.9 Dead+1.0 Wind 300 deg -	37742.10	-40221.64	-23221.97	-2146.30	3737.87	-56.93
No Ice 1.2 Dead+1.0 Wind 315 deg -	50322.80	-33059.64	-33059.74	-3055.84	3073.12	-56.86
No Ice 0.9 Dead+1.0 Wind 315 deg -	37742.10	-33059.72	-33059.82	-3057.44	3070.42	-56.85
No Ice 1.2 Dead+1.0 Wind 330 deg -	50322.80	-23531.48	-40757.92	-3767.63	2186.83	-52.90
No Ice 0.9 Dead+1.0 Wind 330 deg -	37742.10	-23531.54	-40758.02	-3768.83	2184.64	-52.89
No Ice 1.2 Dead+1.0 Ice+1.0 Temp	101404.86	0.01	-0.01	67.30	47.02	0.00
1.2 Dead+1.0 Wind 0 deg+1.0	101404.86	0.01	-12099.61	-1009.72	47.03	-12.13
Ice+1.0 Temp 1.2 Dead+1.0 Wind 30 deg+1.0	101404.86	5947.44	-10301.29	-851.29	-483.38	-2.93
Ice+1.0 Temp 1.2 Dead+1.0 Wind 45 deg+1.0	101404.86	8386.83	-8386.84	-680.78	-701.16	2.13
Ice+1.0 Temp 1.2 Dead+1.0 Wind 60 deg+1.0	101404.86	10242.18	-5913.34	-460.29	-866.95	7.05
Ice+1.0 Temp 1.2 Dead+1.0 Wind 90 deg+1.0	101404.86	11894.89	-0.01	67.40	-1013.79	15.15
Ice+1.0 Temp 1.2 Dead+1.0 Wind 120	101404.86	10478.57	6049.78	605.95	-885.79	19.19
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 135	101404.86	8483.34	8483.31	823.26	-708.85	19.29
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 150	101404.86	5947.45	10301.24	986.07	-483.38	18.08
deg+1.0 lce+1.0 Temp 1.2 Dead+1.0 Wind 180	101404.86	0.01	11826.61	1122.75	47.02	12.13
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 210	101404.86	-5947.43	10301.24	986.06	577.42	2.93
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 225 deg+1.0 Ice+1.0 Temp	101404.86	-8386.82	8386.80	815.56	795.20	-2.13

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Client		Designed by
	Verizon	TJL

Load	Vertical	Shear _x	Shear=	Overturning	Overturning	Torque
Combination				Moment, Mx	Moment, M.	-
	<u>lb</u>	lb	lb	kip-ft	kip-ft	kip-ft
1.2 Dead+1.0 Wind 240	101404.86	-10478.55	6049.78	606.04	979.97	-7.05
deg÷1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 270	101404.86	-11894.87	-0.01	67.39	1107.83	-15.15
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 300	101404.86	-10242.15	-5913.33	-460.29	961.01	-19.19
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 315	101404.86	-8386.81	-8386.84	-680.78	795.21	-19.29
deg+1.0 lce+1.0 Temp						
1.2 Dead+1.0 Wind 330	101404.86	-5947.42	-10301.29	-851.29	577.44	-18.08
deg+1.0 Ice+1.0 Temp						
Dead+Wind 0 deg - Service	41935.67	0.00	-9693.46	-884.58	3.15	-6.94
Dead+Wind 30 deg - Service	41935.67	4663.26	-8077.01	-738.54	-429.67	-1.54
Dead+Wind 45 deg - Service	41935.67	6551.60	-6551.61	-597.43	-605.38	1.41
Dead+Wind 60 deg - Service	41935.67	7971.08	-4602.11	-416.69	-737.86	4.26
Dead+Wind 90 deg - Service	41935.67	9326.52	0.00	11.14	-862.50	8.93
Dead+Wind 120 deg - Service	41935.67	8394.78	4846.73	459.00	-772.56	11.20
Dead+Wind 135 deg - Service	41935.67	6724.58	6724.58	633.89	-619.59	11.22
Dead+Wind 150 deg - Service	41935.67	4663.27	8077.00	760.84	-429.68	10.48
Dead+Wind 180 deg - Service	41935.67	-0.00	9204.21	866.77	3.15	6.94
Dead+Wind 210 deg - Service	41935.67	-4663.27	8077.00	760.79	435.97	1.54
Dead+Wind 225 deg - Service	41935.67	-6551.61	6551.60	619.73	611.73	-1.41
Dead+Wind 240 deg - Service	41935.67	-8394.78	4846.73	458.99	778.86	-4.26
Dead+Wind 270 deg - Service	41935.67	-9326.52	0.00	11.14	868.79	-8.93
Dead+Wind 300 deg - Service	41935.67	-7971.08	-4602.11	-416.69	744.16	-11.20
Dead+Wind 315 deg - Service	41935.67	-6551.60	-6551.61	-597.42	611.68	-11.22
Dead+Wind 330 deg - Service	41935.67	-4663.26	-8077.01	-738.56	436.00	-10.48

Solution Summary

Comb. Ib			m of Applied Force	S		Sum of Reaction	15	
1 -0.00 -41935.67 0.00 -0.00 41935.67 0.00 0.0009 2 0.00 -50322.80 -48921.16 0.01 50322.80 48920.82 0.0009 3 0.00 -37742.10 -48921.16 0.01 37742.10 48920.93 0.0009 4 23531.79 -50322.80 -40758.25 -23531.48 50322.80 40757.92 0.0019 6 33060.05 -50322.80 -33060.05 -33059.64 50322.80 33059.74 0.0019 7 33060.05 -37742.10 -33060.05 -33059.62 37742.10 33059.82 0.0019 8 40222.01 -50322.80 -23222.19 -40221.54 50322.80 23221.91 0.0019 9 40222.01 -37742.10 -23222.19 -40221.64 37742.10 23221.97 0.0019 10 47063.57 -50322.80 -0.00 47063.13 50322.80 -0.12 0.0019 11 47063.57 -50322.80 42	Load	PX	PY	PZ	PX	PY	PZ	% Error
2 0.00 -50322.80	Comb.	lb	lb	lb	lb	lb	lb	
3 0.00 37742.10 48921.16 0.01 37742.10 48920.93 0.0009 4 23531.79 -50322.80 -40758.25 -23531.48 50322.80 40757.92 0.0019 5 23531.79 -37742.10 -40758.25 -23531.53 37742.10 40758.02 0.0019 6 33060.05 -50322.80 -33060.05 -33059.64 50322.80 33059.74 0.0019 7 33060.05 -37742.10 -33060.05 -33059.72 37742.10 33059.82 0.0019 8 40222.01 -50322.80 -23222.19 -40221.54 50322.80 23221.91 0.0019 9 40222.01 -37742.10 -23222.19 -40221.54 50322.80 23221.97 0.0019 10 47063.57 -50322.80 -0.00 -47063.13 50322.80 -0.12 0.0019 11 47063.57 -37742.10 -0.00 -47063.24 37742.10 -0.12 0.0019 12 42366.97 -50322.80 24460.58 -42366.67 50322.80 -24460.42 0.0009 13 42366.97 -37742.10 24460.58 -42366.76 37742.10 -24460.48 0.0009 14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.42 0.0019 15 33935.73 -37742.10 33935.73 -33935.55 50322.80 -33935.52 0.0009 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 -40757.82 0.0019 17 23531.79 -37742.10 40758.25 -23531.65 50322.80 -40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 -40757.92 0.0019 19 0.00 -37742.10 40758.25 -23531.71 37742.10 -40757.92 0.0019 20 -23531.79 -50322.80 40758.25 -23531.71 37742.10 -40757.92 0.0019 21 -23531.79 -37742.10 40758.25 -23531.71 37742.10 -40757.92 0.0019 22 -33060.05 -50322.80 40758.25 23531.66 50322.80 -40757.81 0.0019 23 -33060.05 -37742.10 40758.25 23531.71 37742.10 -40757.91 0.0019 24 -22531.79 -37742.10 40758.25 23531.71 37742.10 -40757.91 0.0019 24 -23531.79 -37742.10 40758.25 23531.71 37742.10 -40757.91 0.0019 24 -23531.79 -37742.10 40758.25 23531.66 50322.80 -33059.75 0.0019 24 -2366.97 -37742.10 33060.05 33059.85 50322.80 -33059.75 0.0019 24 -2366.97 -37742.10 33060.05 33059.85 50322.80 -33059.75 0.0019 25 -42366.97 -50322.80 33060.05 33059.83 50322.80 -34060.41 0.0009 25 -42366.97 -37742.10 33060.05 33059.83 50322.80 -34060.41 0.0009 25 -42366.97 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 25 -42366.97 -37742.10 33060.05 33059.93 37742.10 -34460.47 0.0009	1	-0.00	-41935.67	0.00	-0.00	41935.67	0.00	0.000%
4 23531.79 -50322.80	2	0.00	-50322.80	-48921.16	0.01	50322.80	48920.82	0.000%
5 23531.79 -37742.10 -40758.25 -23531.53 37742.10 40758.02 0.0019 6 33060.05 -50322.80 -33060.05 -33059.64 50322.80 33059.74 0.0019 7 33060.05 -37742.10 -33060.05 -33059.72 37742.10 33059.82 0.0019 8 40222.01 -50322.80 -23222.19 -40221.54 50322.80 23221.97 0.0019 9 40222.01 -37742.10 -23222.19 -40221.64 37742.10 23221.97 0.0019 10 47063.57 -50322.80 -0.00 -47063.13 50322.80 -0.12 0.0019 11 47063.57 -37742.10 -0.00 -47063.24 37742.10 -0.12 0.0019 12 42366.97 -50322.80 24460.58 -42366.67 50322.80 -24460.42 0.0009 13 42366.97 -37742.10 24460.58 -42366.76 37742.10 -24460.48 0.0009 15 33935.73 <t< td=""><td>3</td><td>0.00</td><td>-37742.10</td><td>-48921.16</td><td>0.01</td><td>37742.10</td><td>48920.93</td><td>0.000%</td></t<>	3	0.00	-37742.10	-48921.16	0.01	37742.10	48920.93	0.000%
6 33060.05 -50322.80 -33060.05 -33059.74 0.0019 7 33060.05 -37742.10 -33060.05 -33059.72 37742.10 33059.82 0.0019 8 40222.01 -50322.80 -23222.19 -40221.54 50322.80 23221.91 0.0019 9 40222.01 -37742.10 -23222.19 -40221.64 37742.10 23221.97 0.0019 10 47063.57 -50322.80 -0.00 -47063.13 50322.80 -0.12 0.0019 11 47063.57 -37742.10 -0.00 -47063.24 37742.10 -0.12 0.0019 12 42366.97 -50322.80 24460.58 -42366.67 50322.80 -24460.42 0.0009 13 42366.97 -37742.10 24460.58 -42366.67 50322.80 -24460.42 0.0009 14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.42 0.0019 15 33935.73 -37742.10 33935.73 -33935.59 37742.10 -33935.50 0.0009 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019 17 23531.79 -37742.10 40758.25 -23531.71 37742.10 -40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 46443.83 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 46443.83 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 40757.81 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 40757.81 0.0019 20 -23531.79 -50322.80 40758.25 23531.71 37742.10 46443.95 0.0019 21 -23531.79 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.0009	4	23531.79	-50322.80	-40758.25	-23531.48	50322.80	40757.92	0.001%
7 33060.05 -37742.10 -33060.05 -33059.72 37742.10 33059.82 0.0019 8 40222.01 -50322.80 -23222.19 -40221.54 50322.80 23221.91 0.0019 9 40222.01 -37742.10 -23222.19 -40221.64 37742.10 23221.97 0.0019 10 47063.57 -50322.80 -0.00 -47063.13 50322.80 -0.12 0.0019 11 47063.57 -37742.10 -0.00 -47063.24 37742.10 -0.12 0.0019 12 42366.97 -50322.80 24460.58 -42366.67 50322.80 -24460.42 0.0009 13 42366.97 -37742.10 24460.58 -42366.76 37742.10 -24460.48 0.0009 14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.42 0.0019 15 33935.73 -37742.10 33935.73 -33935.51 50322.80 -33935.42 0.0019 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019 17 23531.79 -37742.10 40758.25 -23531.65 50322.80 40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 46443.83 0.0019 19 0.00 -37742.10 46444.38 0.00 50322.80 46443.95 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 46443.95 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -57322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.67 50322.80 -24460.47 0.0009	5	23531.79	-37742.10	-40758.25	-23531.53	37742.10	40758.02	0.001%
7 33060.05 -37742.10 -33060.05 -33059.72 37742.10 33059.82 0.0019 8 40222.01 -50322.80 -23222.19 -40221.54 50322.80 23221.91 0.0019 9 40222.01 -37742.10 -23222.19 -40221.64 37742.10 23221.97 0.0019 10 47063.57 -50322.80 -0.00 -47063.13 50322.80 -0.12 0.0019 11 47063.57 -37742.10 -0.00 -47063.24 37742.10 -0.12 0.0019 12 42366.97 -50322.80 24460.58 -42366.67 50322.80 -24460.42 0.0009 13 42366.97 -37742.10 24460.58 -42366.76 37742.10 -24460.48 0.0009 14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.50 0.0019 15 33935.73 -37742.10 33935.73 -33935.59 37742.10 -33935.50 0.0019 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019	6	33060.05	-50322.80	-33060.05	-33059.64	50322.80	33059.74	0.001%
8 40222.01 -50322.80 -23222.19 -40221.54 50322.80 23221.91 0.0019 9 40222.01 -37742.10 -23222.19 -40221.64 37742.10 23221.97 0.0019 10 47063.57 -50322.80 -0.00 -47063.13 50322.80 -0.12 0.0019 11 47063.57 -37742.10 -0.00 -47063.24 37742.10 -0.12 0.0019 12 42366.97 -50322.80 24460.58 -42366.76 50322.80 -24460.42 0.0009 13 42366.97 -37742.10 24460.58 -42366.76 37742.10 -24460.48 0.0009 14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.50 0.0019 15 33935.73 -37742.10 33935.73 -33935.59 37742.10 -33935.50 0.0019 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 46443.83 0.0019	7	33060.05	-37742.10	-33060.05	-33059.72	37742.10	33059.82	0.001%
9	8	40222.01	-50322.80	-23222.19	-40221.54	50322.80	23221.91	0.001%
10 47063.57 -50322.80 -0.00 -47063.13 50322.80 -0.12 0.0019 11 47063.57 -37742.10 -0.00 -47063.24 37742.10 -0.12 0.0019 12 42366.97 -50322.80 24460.58 -42366.67 50322.80 -24460.42 0.0009 13 42366.97 -37742.10 24460.58 -42366.76 37742.10 -24460.48 0.0009 14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.42 0.0011 15 33935.73 -37742.10 33935.73 -33935.59 37742.10 -33935.50 0.0009 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019 17 23531.79 -37742.10 40758.25 -23531.71 37742.10 40757.92 0.0019 18 0.00 -37742.10 46444.38 0.00 50322.80 46443.83 0.0019 20 -23531.79 -503		40222.01	-37742.10	-23222.19	-40221.64	37742.10	23221.97	0.001%
11 47063.57 -37742.10 -0.00 -47063.24 37742.10 -0.12 0.0019 12 42366.97 -50322.80 24460.58 -42366.67 50322.80 -24460.42 0.0009 13 42366.97 -37742.10 24460.58 -42366.76 37742.10 -24460.48 0.0009 14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.42 0.0019 15 33935.73 -37742.10 33935.73 -33935.59 37742.10 -33935.50 0.0009 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019 17 23531.79 -37742.10 40758.25 -23531.71 37742.10 40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 46443.83 0.0019 19 0.00 -37742.10 46444.38 0.00 37742.10 46443.95 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 40757.81 0.0019	10	47063.57	-50322.80	-0.00	-47063.13	50322.80		0.001%
13 42366.97 -37742.10 24460.58 -42366.76 37742.10 -24460.48 0.0009 14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.42 0.0019 15 33935.73 -37742.10 33935.73 -33935.59 37742.10 -33935.50 0.0009 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019 17 23531.79 -37742.10 40758.25 -23531.71 37742.10 40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 46443.83 0.0019 19 0.00 -37742.10 46444.38 0.00 37742.10 46443.83 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 40757.91 0.0019 22 -33060.05 -5		47063.57	-37742.10	-0.00	-47063.24	37742.10	-0.12	0.001%
13 42366.97 -37742.10 24460.58 -42366.76 37742.10 -24460.48 0.0009 14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.42 0.0019 15 33935.73 -37742.10 33935.73 -33935.59 37742.10 -33935.50 0.0009 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019 17 23531.79 -37742.10 40758.25 -23531.71 37742.10 40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 46443.83 0.0019 19 0.00 -37742.10 46444.38 0.00 37742.10 46443.95 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 40757.91 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 <td>12</td> <td>42366.97</td> <td>-50322.80</td> <td>24460.58</td> <td>-42366.67</td> <td>50322.80</td> <td>-24460.42</td> <td>0.000%</td>	12	42366.97	-50322.80	24460.58	-42366.67	50322.80	-24460.42	0.000%
14 33935.73 -50322.80 33935.73 -33935.51 50322.80 -33935.42 0.0019 15 33935.73 -37742.10 33935.73 -33935.59 37742.10 -33935.50 0.0009 16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019 17 23531.79 -37742.10 40758.25 -23531.71 37742.10 40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 46443.83 0.0019 19 0.00 -37742.10 46444.38 0.00 37742.10 46443.95 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 40757.91 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25	13	42366.97	-37742.10	24460.58	-42366.76	37742.10	-24460.48	0.000%
16 23531.79 -50322.80 40758.25 -23531.65 50322.80 40757.82 0.0019 17 23531.79 -37742.10 40758.25 -23531.71 37742.10 40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 46443.83 0.0019 19 0.00 -37742.10 46444.38 0.00 37742.10 46443.95 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 40757.91 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -3		33935.73	-50322.80	33935.73	-33935.51	50322.80	-33935.42	0.001%
17 23531.79 -37742.10 40758.25 -23531.71 37742.10 40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 -46443.83 0.0019 19 0.00 -37742.10 46444.38 0.00 37742.10 -46443.95 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 40757.91 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.0009	15	33935.73	-37742.10	33935.73	-33935.59	37742.10	-33935.50	0.000%
17 23531.79 -37742.10 40758.25 -23531.71 37742.10 -40757.92 0.0019 18 0.00 -50322.80 46444.38 0.00 50322.80 -46443.83 0.0019 19 0.00 -37742.10 46444.38 0.00 37742.10 -46443.95 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 -40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 -40757.91 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.0009		23531.79	-50322.80	40758.25	-23531.65	50322.80	-40757.82	0.001%
18 0.00 -50322.80 46444.38 0.00 50322.80 -46443.83 0.0019 19 0.00 -37742.10 46444.38 0.00 37742.10 -46443.95 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 -40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 -40757.91 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.0009	17	23531.79	-37742.10	40758.25	-23531.71	37742.10	-40757.92	0.001%
19 0.00 -37742.10 46444.38 0.00 37742.10 -46443.95 0.0019 20 -23531.79 -50322.80 40758.25 23531.66 50322.80 -40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 -40757.91 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.0009		0.00	-50322.80	46444.38	0.00	50322.80	-46443.83	0.001%
20 -23531.79 -50322.80 40758.25 23531.66 50322.80 -40757.81 0.0019 21 -23531.79 -37742.10 40758.25 23531.71 37742.10 -40757.91 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.0009	19	0.00	-37742.10	46444.38	0.00	37742.10	-46443.95	0.001%
21 -23531.79 -37742.10 40758.25 23531.71 37742.10 -40757.91 0.0019 22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.0009	20	-23531.79	-50322.80	40758.25	23531.66	50322.80	-40757.81	0.001%
22 -33060.05 -50322.80 33060.05 33059.85 50322.80 -33059.75 0.0019 23 -33060.05 -37742.10 33060.05 33059.93 37742.10 -33059.82 0.0009 24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.0009 25 -42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.0009		-23531.79	-37742.10	40758.25	23531.71	37742.10	-40757.91	0.001%
24		-33060.05	-50322.80	33060.05	33059.85	50322.80		0.001%
24 -42366.97 -50322.80 24460.58 42366.67 50322.80 -24460.41 0.000% 25 -42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.000%		-33060.05	-37742.10	33060.05	33059.93	37742.10		0.000%
25 42366.97 -37742.10 24460.58 42366.77 37742.10 -24460.47 0.0009		-42366.97	-50322.80	24460.58	42366.67			0.000%
		-42366.97	-37742.10	24460.58	42366.77	37742.10	-24460.47	0.000%
1700110 0012 0.0017	26	-47063.57	-50322.80	-0.00	47063.13	50322.80	-0.12	0.001%

Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

Job		Page
	23134.00 - East Haddam	34 of 42
Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client	•	Designed by
	Verizon	TJL

	Sur	n of Applied Forces			Sum of Reaction		
Load	PX	PY	PZ	PX	PY	PZ	o Erro
Comb.	lb	lb	lb	lb	lb	lb	
27	-47063.57	-37742.10	-0.00	47063.24	37742.10	-0.12	0.001%
28	-40222.01	-50322.80	-23222.19	40221.55	50322.80	23221.91	0.001%
29	-40222.01	-37742.10	-23222.19	40221.64	37742.10	23221.97	0.001%
30	-33060.05	-50322.80	-33060.05	33059.64	50322.80	33059.74	0.001%
31	-33060.05	-37742.10	-33060.05	33059.72	37742.10	33059.82	0.001%
32	-23531.79	-50322.80	40758.25	23531.48	50322.80	40757.92	0.001%
33	-23531.79	-37742.10	-40758.25	23531.54	37742.10	40758.02	0.001%
34	0.00	-101404.86	0.00	-0.01	101404.86	0.01	0.000%
35	0.00	-101404.86	-12099.82	-0.01	101404.86	12099.61	0.000%
36	5947.55	-101404.86	-10301.47	-5947.44	101404.86	10301.29	0.000%
37	8386.99	-101404.86	-8386.99	-8386.83	101404.86	8386.84	0.000%
38	10242.37	-101404.86	-5913.43	-10242.18	101404.86	5913.34	0.000%
39	11895.11	-101404.86	-0.00	-11894.89	101404.86	0.01	0.000%
40	10478.75	-101404.86	6049.91	-10478.57	101404.86	-6049.78	0.000%
41	8483.49	-101404.86	8483.49	-8483.34	101404.86	-8483.31	0.000%
42	5947.55	-101404.86	10301.47	-5947.45	101404.86	-10301.24	0.000%
43	0.00	-101404.86	11826.87	-0.01	101404.86	-11826.61	0.000%
44	-5947.55	-101404.86	10301.47	5947.43	101404.86	-10301.24	0.000%
45	-8386.99	-101404.86	8386.99	8386.82	101404.86	-8386.80	0.000%
46	-10478.75	-101404.86	6049.91	10478.55	101404.86	-6049.78	0.000%
47	-11895.11	-101404.86	-0.00	11894.87	101404.86	0.01	0.000%
48	-10242.37	-101404.86	-5913.43	10242.15	101404.86	5913.33	0.000%
49	-8386.99	-101404.86	-8386.99	8386.81	101404.86	8386.84	0.000%
50	-5947.55	-101404.86	-10301.47	5947.42	101404.86	10301.29	0.000%
51	0.00	-41935.67	-9693.53	-0.00	41935.67	9693.46	0.000%
52	4663.30	-41935.67	-8077.07	-4663.26	41935.67	8077.01	0.000%
53	6551.66	-41935.67	-6551.66	-6551.60	41935.67	6551.61	0.000%
54	7971.15	-41935.67	-4602.14	-7971.08	41935.67	4602.11	0.000%
55	9326.60	-41935.67	0.00	-9326.52	41935.67	-0.00	0.000%
56	8394.84	-41935.67	4846.76	-8394.78	41935.67	-4846.73	0.000%
57	6724.63	-41935.67	6724.63	-6724.58	41935.67	-6724.58	0.000%
58	4663.30	-41935.67	8077.07	-4663.27	41935.67	-8077.00	0.000%
59	-0.00	-41935.67	9204.29	0.00	41935.67	-9204.21	0.000%
60	-4663.30	-41935.67	8077.07	4663.27	41935.67	-8077.00	0.000%
61	-6551.66	-41935.67	6551.66	6551.61	41935.67	-6551.60	0.000%
62	-8394.84	-41935.67	4846.76	8394.78	41935.67	-4846.73	0.000%
63	-9326.60	-41935.67	0.00	9326.52	41935.67	-0.00	0.000%
64	-7971.15	-41935.67	-4602.14	7971.08	41935.67	4602.11	0.000%
65	-6551.66	-41935.67	-6551.66	6551.60	41935.67	6551.61	0.000%
66	-4663.30	-41935.67	-8077.07	4663.26	41935.67	8077.01	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00005378
3	Yes	4	0.00000001	0.00003553
4	Yes	4	0.00000001	0.00007540
5	Yes	4	0.0000001	0.00005755
6	Yes	4	0.00000001	0.00008712
7	Yes	4	0.00000001	0.00006864
8	Yes	4	0.00000001	0.00009151
9	Yes	4	0.00000001	0.00007276
10	Yes	4	0.00000001	0.00007572
11	Yes	4	0.00000001	0.00005804

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12	Yes	4	0.00000001	0.00005380
13	Yes	4	0.00000001	0.00003558
14	Yes	4	0.0000001	0.00006095
15	Yes	4	0.00000001	0.00004300
16	Yes	4	0.00000001	0.00007525
17	Yes	4	0.00000001	0.00005725
18	Yes	4	0.00000001	0.00009163
19	Yes	4	0.00000001	0.00007283
20	Yes	4	0.00000001	0.00007548
21	Yes	4	0.00000001	0.00005760
22	Yes	4	0.00000001	0.00006129
23	Yes	4	0.00000001	0.00004342
24	Yes	4	0.00000001	0.00005377
25	Yes	4	0.00000001	0.00003552
26	Yes	4	0.00000001	0.00007574
27	Yes	4	0.00000001	0.00005805
28	Yes	4	0.00000001	0.00009157
29	Yes	4	0.00000001	0.00007280
30	Yes	4	0.00000001	0.00008706
31	Yes	4	100000001	0.00006851
32	Yes	4	0.00000001	0.00007522
33	Yes	4	0.00000001	0.00005725
34	Yes	4	0.00000001	0.00001225
35	Yes	4	0.00000001	0.00010994
36	Yes	4	0.00000001	0.00011142
37	Yes	4	0.00000001	0.00011394
38	Yes	4	0.00000001	0.00011536
39	Yes	4	0.00000001	0.00011454
40	Yes	4	0.00000001	0.00011545
41	Yes	4	0.00000001	0.00011791
42	Yes	4	0.00000001	0.00012127
43	Yes	4	0.00000001	0.00012543
44	Yes	4	0.00000001	0.00012255
45	Yes	4	0.00000001	0.00011964
46	Yes	4	0.00000001	0.00011928
47	Yes	4	0.00000001	0.00011958
48	Yes	4	0.00000001	0.00012074
49	Yes	4	0.0000001	0.00011891
50	Yes	4	0.00000001	0.00011544
51	Yes	4	0.00000001	0.00005232
52	Yes	4	0.00000001	0.00005561
53	Yes	4	0.00000001	0.00005788
54	Yes	4	0.00000001	0.00005885
55	Yes	4	0.00000001	0.00005574
56	Yes	4	0.00000001	0.00005275
57	Yes	4	0.00000001	0.00005395
58	Yes	4	0.0000001	0.00005644
59	Yes	4	0.0000001	0.00005940
60	Yes	4	0.0000001	0.00005619
61	Yes	4	0.0000001	0.00005381
62	Yes	4	0.00000001	0.00005274
63	Yes	4	0.0000001	0.00005581
64	Yes	4	0.00000001	0.00005897
65	Yes	4	0.0000001	0.00005801
66	Yes	4	0.0000001	0.00005594

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Elevation	Horz.	Gov.	Tilt	Twist
	Deflection	Load		
ft	in	Comb.	0	.0
160 - 140	1.484	62	0.0727	0.0200
140 - 120	1.177	62	0.0718	0.0185
120 - 100	0.880	62	0.0646	0.0124
100 - 80	0.612	62	0.0556	0.0096
	0.389	62	0.0427	0.0069
	0.221	62	0.0310	0.0050
	0.104	62	0.0207	0.0032
20 - 0	0.026	62	0.0094	0.0015
	fi 160 - 140 140 - 120 120 - 100 100 - 80 80 - 60 60 - 40 40 - 20	ft Deflection 160 - 140 1.484 140 - 120 1.177 120 - 100 0.880 100 - 80 0.612 80 - 60 0.389 60 - 40 0.221 40 - 20 0.104	Deflection Load Comb. 160 - 140 1.484 62 140 - 120 1.177 62 120 - 100 0.880 62 100 - 80 0.612 62 80 - 60 0.389 62 60 - 40 0.221 62 40 - 20 0.104 62	Deflection Load Comb. Comb.

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
ft		Load Comb.	in	0	0	Curvature ft
160.00	10-ft Lighting Rod	62	1.484	0.0727	0.0200	Inf
145.00	MT6413-77A	62	1.253	0.0726	0.0193	701761
120.00	PA6-65	62	0.880	0.0646	0.0124	158382
100.00	ANT150F2	62	0.612	0.0556	0.0096	107200
85.00	SD314	62	0.440	0.0460	0.0075	87862
80.00	ANT150F2	62	0.389	0.0427	0.0069	83912
55.00	531-70HD	62	0.188	0.0285	0.0045	100254
25.00	ANT150F2	62	0.041	0.0121	0.0019	80473
20.00	ANT150F2	62	0.026	0.0094	0.0015	73481

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	٥	6)
T1	160 - 140	7.388	24	0.3579	0.1021
T2	140 - 120	5.876	24	0.3544	0.0941
T3	120 - 100	4.401	24	0.3223	0.0636
T4	100 - 80	3.064	24	0.2774	0.0490
T5	80 - 60	1.949	24	0.2135	0.0351
T6	60 - 40	1.108	24	0.1552	0.0252
T7	40 - 20	0.520	24	0.1036	0.0161
T8	20 - 0	0.132	24	0.0470	0.0078

Critical Deflections and Radius of Curvature - Design Wind

Annurtenance	Gov	Deflection	Tilt	Twist	Radius of
	Load	,			Curvature
	Comb.	in	۰	0	ft
10-ft Lighting Rod	24	7.388	0.3579	0.1021	910222
	24	6.253	0.3576	0.0985	303406
	24	4.401	0.3223	0.0636	33009
	24	3.064	0.2774	0.0490	21552
	24	2.203	0.2300	0.0385	17594
ANT150F2	24	1.949	0.2135	0.0351	16795
	Appurtenance 10-ft Lighting Rod MT6413-77A PA6-65 ANT150F2 SD314 ANT150F2	Load Comb. 10-ft Lighting Rod 24 MT6413-77A 24 PA6-65 24 ANT150F2 24 SD314 24	Load Comb. in 10-ft Lighting Rod 24 7.388 MT6413-77A 24 6.253 PA6-65 24 4.401 ANT150F2 24 3.064 SD314 24 2.203	Load Comb. in 0	Load Comb. in 0 0 0 0 0 0 0 0 0

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	Verizon	TJL

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	٥	0	ft
55.00	531-70HD	24	0.941	0.1423	0.0230	20134
25.00	ANT150F2	24	0.204	0.0608	0.0098	16132
20.00	ANT150F2	24	0.132	0.0470	0.0078	14753

Bolt Design Data	n Dat	ar	es	D	lt	la	Е
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Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of	Maximum Load	Allowable Load	Ratio Load	Allowable Ratio	Criteria
	ft			in	Bolts	per Bolt lb	per Bolt lb	Allowable		
T1	160	Leg	A325N	1.0000	6	567.91	54517.00	0.010	1	Bolt Tension
		Diagonal	A325N	1.0000	1	4352.56	16939.50	0.257	1	Member Block Shear
		Top Girt	A325N	1.0000	1	237.95	10163.70	0.023	1	Member Block Shear
T2	140	Leg	A325N	1.0000	6	4626.41	54517.00	0.085	1	Bolt Tension
		Diagonal	A325N	1.0000	1	7339.83	16939.50	0.433	Î	Member Block Shear
T3	120	Leg	A325N	1.2500	6	9713.53	87219.80	0.111	1	Bolt Tension
		Diagonal	A325N	1.0000	1	8616.98	16939.50	0.509	1	Member Block Shear
		Top Girt	A325N	1.0000	1	1229.48	10163.70	0.121	1	Member Block Shear
T 4	100	Leg	A325N	1.2500	6	15180.30	87219.80	0.174	1	Bolt Tension
		Diagonal	A325N	1.0000	1	8894.90	16939.50	0.525	1	Member Block Shear
T5	80	Leg	A325N	1.0000	12	9371.92	54517.00	0.172	1	Bolt Tension
		Diagonal	A325N	0.8750	1	12975.20	29580.00	0.439	1	Member Bearing
T6	60	Leg	A325N	1.0000	12	12013.20	54517.00	0.220	1	Bolt Tension
		Diagonal	A325N	0.8750	1	13823.90	29580.00	0.467	1	Member Bearing
T7	40	Leg	A325N	1.0000	12	14424.50	54517.00	0.265	1	Bolt Tension
		Diagonal	A325N	0.8750	1	13629.50	29580.00	0.461	1	Member Bearing
T8	20	Leg	F1554-10 5	1.2500	12	16699.60	90854.00	0.184	1	Bolt Tension
		Diagonal	A325N	0.8750	1	14272.70	29580.00	0.483	1	Member Bearing

Compression Checks

Leg Design Data (Compression)

Section	Elevation	Size	L	L_{u}	Kl/r	A	P_u	ϕP_n	Ratio
No.	ft		ft	ft		in ²	lb	lb	$\frac{P_u}{\Phi P}$

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Section	Elevation	Size	L	L_u	Kl/r	A	P_u	ϕP_n	Ratio P _u
No.	ft		fi	ft		in ²	lb	lb	ϕP_n
Tl	160 - 140	#12ZG-58-1.5" - 1.00"	20.00	10.00	37.5 K=1.00	5.3014	-8111.62	245621.00	0.033
T2	140 - 120	#12ZG-58-1.5" - 1.00"	20.03	10.02	37.5 K=1.00	5.3014	-34721.00	245621.00	0.141
Т3	120 - 100	#12ZG-58-1.75" - 1.00"	20.03	10.02	31.9 K=1.00	7.2158	-70895.40	345601.00	0.205 1
T4	100 - 80	#12ZG-58-1.75" - 1.00"	20.03	10.02	31.9 K=1.00	7.2158	-107745.00	345601.00	0.312 1
Т5	80 - 60	#12ZG-58-2.00" - 0.875"	20.03	20.03	48.8 K=1.00	9.4248	-131866.00	401936.00	0.328
Т6	60 - 40	#12ZG-58-2.25" - 0.875"	20.03	20.03	48.8 K=1.00	11.9282	-169578.00	508981.00	0.333
Т7	40 - 20	#12ZG-58-2.25" - 0.875"	20.03	20.03	48.8 K=1.00	11.9282	-203992.00	508981.00	0.401
Т8	20 - 0	#12ZG-58-2.50" - 0.875"	20.03	20.03	48.7 K=1.00	14.7262	-237151.00	628758.00	0.377

 $^{^{1}}$ P_{u} / ϕP_{n} controls

Truss-Leg	Diagonal	Data
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Section No.	Elevation	Diagonal Size	L_d ft	Kl/r	φ <i>P</i> ,,	A in ²	V., lb	φV" lb	Stress Ratio
T1	160 - 140	0.5	1.46	119.3	276735.00	0.1963	2057.68	3381.46	0.609
T2	140 - 120	0.5	1.46	119.3	276735.00	0.1963	475.18	3381.46	0.141
Т3	120 - 100	0.5	1.44	117.6	376667.00	0.1963	2010.15	3471.55	0.580
T4	100 - 80	0.5	1.44	117.6	376667.00	0.1963	650.85	3471.55	0.189
T5	80 - 60	0.5	1.39	113.2	491973.00	0.1963	657.72	3759.23	0.177
Т6	60 - 40	0.5	1.38	112.2	622654.00	0.1963	929.88	3804.69	0.246
Т7	40 - 20	0.5	1.38	112.2	622654.00	0.1963	832.11	3804.69	0.220
Т8	20 - 0	0.5	1.36	111.2	768708.00	0.1963	878.16	3849.52	0.230

Diagonal Design Data (Compression)

Section	Elevation	Size	L	L_{μ}	Kl/r	Λ	P_u	ϕP_n	Ratio
No.	Dictation								P_{μ}
140	ft		ft	ft		in²	lb	lЬ	ϕP_n
701	1.00 1.10	12.2-5/16	12.81	5,44	113.1	1.7800	-4634.90	38321.80	0.121
TT	160 - 140	L3x3x5/16	12.01	2.44		1.7000		•	
					K=1.02				

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Section No.	Elevation	Size	L	L_u	Kl/r	A	P_u	фР"	Ratio P _u
	ft		ft	ft		in ²	lb	lb	ϕP_n
T2	140 - 120	L3x3x5/16	13.80	6.37	129.8 K=1.00	1.7800	-6819.41	30261.30	0.225
Т3	120 - 100	L3x3x5/16	15.24	7.12	145.1 K=1.00	1.7800	-8522.99	24188.40	0.352 1
T4	100 - 80	L3x3x5/16	16.80	7.92	161.4 K=1.00	1.7800	-8807.84	19549.20	0.451 1
T5	80 - 60	2L3 1/2x3 1/2x1/4	25.01	12.35	136.0 K=1.00	3.3800	-13766.90	52307.80	0.263 1
Т6	60 - 40	2L3 1/2x3 1/2x1/4	26.26	12.98	142.9 K=1.00	3.3800	-14013.70	47397.70	0.296 1
T7	40 - 20	2L3 1/2x3 1/2x1/4	27.59	13.65	150.2 K=1.00	3.3800	-13996.00	42867.70	0.326 1
T8	20 - 0	2L3 1/2x3 1/2x1/4	29.01	14.35	158.0 K=1.00	3.3800	-14796.10	38750.20	0.382 1

¹ P_u / ϕP_n controls

Top Girt	Design	Data ((Com	pression)	١
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Section No	Elevation	Size	L	L_u	Kl/r	A	P_{u}	ϕP_n	Ratio P _u
	ft		ft	ft		in ²	lb	lb	ΦP_{ν}
T1	160 - 140	L3x3x3/16	8.00	6.67	134.2 K=1.00	1.0900	-321.83	17315.60	0.019
Т3	120 - 100	L3x3x3/16	10.00	8,67	174.5 K=1.00	1.0900	-1229.48	10245.90	0.120 1

¹ P_u / ϕP_n controls

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation	Size	L	L_{u}	Kl/r	A	P_{u}	ϕP_n	Ratio P _u
	.ft		ft	ft		in ²	lb	lb	φP,,
T1	160 - 140	#12ZG-58-1.5" - 1.00"	20.00	10.00	37.5	5.3014	3407.47	276735.00	0.012
T2	140 - 120	#12ZG-58-1.5" - 1.00"	20.03	10.02	37.5	5.3014	27758.40	276735.00	0.100 1
T3	120 - 100	#12ZG-58-1.75" - 1.00"	20.03	10.02	31.9	7.2158	58281.20	376667.00	0.155 1
T 4	100 - 80	#12ZG-58-1.75" - 1.00"	20.03	10.02	31.9	7.2158	91081.80	376667.00	0,242 1
T5	80 - 60	#12ZG-58-2.00" - 0.875"	20.03	20.03	48.8	9.4248	112463.00	491973.00	0.229

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Section	Elevation	Size	L	L_{u}	Kl/r	A	P_{u}	ϕP_n	Ratio P _u
No.	fi		ft	ft		in ²	lb	lb	ϕP_n
Т6	60 - 40	#12ZG-58-2.25" - 0.875"	20.03	20.03	48.8	11.9282	144158.00	622654.00	0.232
Т7	40 - 20	#12ZG-58-2.25" - 0.875"	20.03	20.03	48.8	11.9282	173094.00	622654.00	0.278
Т8	20 - 0	#12ZG-58-2.50" - 0.875"	20.03	20.03	48.7	14.7262	200395.00	768708.00	0.261

 $^{^{1}}$ P_{u} / ϕP_{n} controls

T8

			Truss-	Leg [Diagona	I Data	a		
Section No.	Elevation ft	Diagonal Size	L_d ft	Kl/r	ϕP_n lb	A in²	V _u lb	ϕV_n lb	Stress Ratio
T1	160 - 140	0.5	1.46	119.3	276735.00	0.1963	2057.68	3381.46	0.609
T2	140 - 120	0.5	1.46	119.3	276735.00	0.1963	475.18	3381.46	0.141
Т3	120 - 100	0.5	1.44	117.6	376667.00	0.1963	2010.15	3471.55	0.580
Т4	100 - 80	0.5	1.44	117.6	376667.00	0.1963	650.85	3471.55	0.189
T5	80 - 60	0.5	1.39	113.2	491973.00	0.1963	657.72	3759.23	0.177
Т6	60 - 40	0.5	1.38	112.2	622654.00	0.1963	929.88	3804.69	0.246
Т7	40 - 20	0.5	1.38	112.2	622654.00	0.1963	832.11	3804.69	0.220
Т8	20 - 0	0.5	1.36	111.2	768708.00	0.1963	878.16	3849.52	0.230

	Diagonal Design Data (Tension)												
Section No.	Elevation	Size	L	L_u	Kl/r	A	P_u	ϕP_n	Ratio P _u				
110.	ft		ft	ft		in ²	lb	lb	ΦP_n				
T1	160 - 140	L3x3x5/16	12.81	5.44	72.9	1.0713	4352.56	46602.80	0.093				
T2	140 - 120	L3x3x5/16	13.13	6.06	81.0	1.0713	7339.83	46602.80	0,157 1				
Т3	120 - 100	L3x3x5/16	15.24	7.12	94.9	1.0713	8616.98	46602.80	0.185 1				
T4	100 - 80	L3x3x5/16	16.80	7.92	105.3	1.0713	8894.90	46602.80	0.191 1				
T5	80 - 60	2L3 1/2x3 1/2x1/4	25.01	12.35	137.7	2.1600	12975.20	93960.00	0.138 1				
T6	60 - 40	2L3 1/2x3 1/2x1/4	26.26	12.98	144.5	2.1600	13823.90	93960.00	0.147 1				

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Section No.	Elevation	Size	L	L_{u}	Kl/r	A	P_u	ϕP_n	Ratio P _u
	fi		ft	ft		in ²	lb	lb	ΦP_n
Т7	40 - 20	2L3 1/2x3 1/2x1/4	27.59	13.65	151.9	2.1600	13629.50	93960.00	0.145
Т8	20 - 0	2L3 1/2x3 1/2x1/4	29.01	14.35	159.7	2,1600	14272.70	93960.00	0.152 1

 $^{^{1}}$ P_{u} / ϕP_{n} controls

Top Girt Design Data (Tension

Section No.	Elevation	Size	L	L_{μ}	Kl/r	Α	P_{u}	ϕP_n	Ratio P
	ft		ft	ft		in^2	lb	lb	ΦP_n
T1	160 - 140	L3x3x3/16	8.00	6.67	89.5	0.6593	237.95	28679.40	0.008
T3	120 - 100	L3x3x3/16	10.00	8.67	115.0	0.6593	1229.48	28679.40	0.043

 $^{^{1}}$ P_{u} / ϕP_{n} controls

Section Capacity Table

Section	Elevation	Component	Size	Critical	P	$ \rho P_{allow} $	%	Pass
No:	ft	Type		Element	lb	lb	Capacity	Fail
T1	160 - 140	Leg	#12ZG-58-1.5" - 1.00"	1	-8111.62	245621.00	60.9	Pass
T2	140 - 120	Leg	#12ZG-58-1.5" - 1.00"	19	-34721.00	245621.00	14.1	Pass
T3	120 - 100	Leg	#12ZG-58-1.75" - 1.00"	36	-50370.10	345601.00	58.0	Pass
T4	100 - 80	Leg	#12ZG-58-1.75" - 1.00"	52	-107745.00	345601.00	31.2	Pass
T5	80 - 60	Leg	#12ZG-58-2.00" - 0.875"	67	-131866.00	401936.00	32.8	Pass
T6	60 - 40	Leg	#12ZG-58-2.25" - 0.875"	76	-169578.00	508981.00	33.3	Pass
T7	40 - 20	Leg	#12ZG-58-2.25" - 0.875"	85	-203992.00	508981.00	40.1	Pass
T8	20 - 0	Leg	#12ZG-58-2.50" - 0.875"	94	-237151.00	628758.00	37.7	Pass
T1	160 - 140	Diagonal	L3x3x5/16	8	-4634.90	38321.80	12.1	Pass
							25.7 (b)	
T2	140 - 120	Diagonal	L3x3x5/16	23	-6819.41	30261.30	22.5	Pass
							43.3 (b)	
T3	120 - 100	Diagonal	L3x3x5/16	41	-8522.99	24188.40	35.2	Pass
							50.9 (b)	
T4	100 - 80	Diagonal	L3x3x5/16	56	-8807.84	19549.20	45.Ì	Pass
							52.5 (b)	
T5	80 - 60	Diagonal	2L3 1/2x3 1/2x1/4	71	-13766.90	52307.80	26.3	Pass
							43.9 (b)	
T6	60 - 40	Diagonal	2L3 1/2x3 1/2x1/4	80	-14013.70	47397.70	29.6	Pass
							46.7 (b)	
T7	40 - 20	Diagonal	2L3 1/2x3 1/2x1/4	89	-13996.00	42867.70	32.6	Pass
							46.1 (b)	
T8	20 - 0	Diagonal	2L3 1/2x3 1/2x1/4	98	-1 4796.10	38750.20	38.2	Pass
							48.3 (b)	
T1	160 - 140	Top Girt	L3x3x3/16	4	-321.83	17315.60	1.9	Pass
							2.3 (b)	

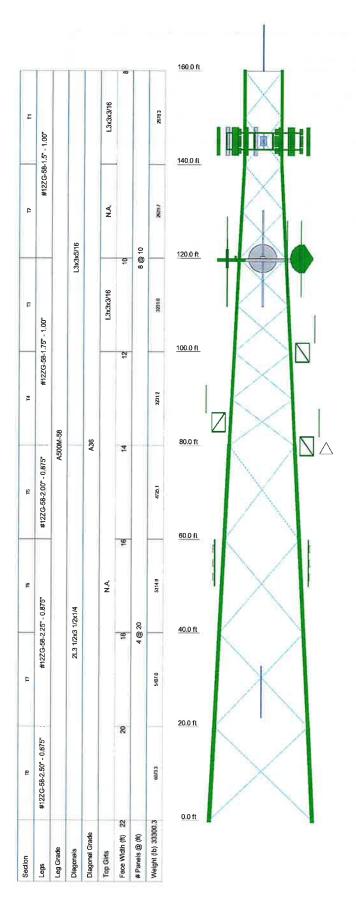
Centek Engineering Inc. 63-2 North Branford Rd.

Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

Job		Page
	23134.00 - East Haddam	42 of 42
Project		Date
	160-ft Lattice Tower (CSP #52)	09:38:07 10/09/23
Client		Designed by
	Verizon	TJL

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	øP _{allow} lb	% Capacity	Pass Fail
T3	120 - 100	Top Girt	L3x3x3/16	39	-1229.48	10245.90	12.0 12.1 (b) Summary	Pass
						Leg (T1)	60.9	Pass
						Diagonal (T4)	52.5	Pass
						Top Girt (T3)	12.1	Pass
						Bolt Checks	52.5	Pass
						RATING =	60.9	Pass

 $Program\ Version\ 8.1.1.0\ -\ 6/3/2021\ File: J:/Jobs/2313400. WI/05_Structural/Structural\ Analysis/Backup\ Documentation/Tnxtower/Tower\ \#52\ -\ East\ Haddam.erington and the program of the program$



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION	
10-ft Lighting Rod	160	SC479-HF1LDF (Inverted)	120	
MT6413-77A (Verizon Proposed)	145	SC479-HF1LDF (Inverted)	120	
NHH-45B-R2B (Verizon Proposed)	145	TX/RX 432E-83I-01T	120	
NHH-45B-R2B (Verizon Proposed)	145	10-ft T-Frame	120	
MT6413-77A (Verizon Proposed)	145	DB810K (Future)	120	
NHH-45B-R2B (Verizon Proposed)	145	DB810K (Future)	120	
NHH-45B-R2B (Verizon Proposed)	145	WPA-80080/4CF	120	
MT6413-77A (Verizon Proposed)	145	ANT450F6	120	
NHH-45B-R2B (Verizon Proposed)	145	SC479-HF1LDF (Inverted)	120	
NHH-45B-R2B (Verizon Proposed)	145	SC479-HF1LDF (Inverted)	120	
MT6413-77A (Verlzon Proposed)	145	TX/RX 432E-83I-01T	120	
NHH-45B-R2B (Verizon Proposed)	145	10-ft T-Frame	120	
NHH-45B-R2B (Verizon Proposed)	145	SC479-HF1LDF	120	
RF4439d-25A (B2/B66A RRH) (Verlzon	145	PA6-65	120	
Proposed)		PA6-65	120	
RF4439d-25A (B2/B66A RRH) (Verizon	145	ANT150F2	100	
Proposed)		6' Standoff Arm	100	
RF4439d-25A (B2/B66A RRH) (Verizon Proposed)	145	SD314	85	
RF4439d-25A (B2/B66A RRH) (Verizon	145	6' Standoff Arm	85	
Proposed)	145	ANT450F6	85	
RF4461d-13A (Verizon Proposed)	145	6' Standoff Arm	85	
RF4461d-13A (Verizon Proposed)	145	6" Standoff Arm	80	
RF4461d-13A (Verizon Proposed)	145	ANT150F2	80	
RF4461d-13A (Verizon Proposed)	145	2-ft Stand Off	55	
RC2DC-3315-PF-48 (Verizon	145	531-70HD	55	
Proposed)		531-70HD	55	
SitePro VFA12-HD (Verizon Proposed)	145	531-70HD	55	
SitePro VFA12-HD (Verizon Proposed)	145	2-ft Stand Off	25	
SitePro VFA12-HD (Verizon Proposed)	145	ANT150F2	25	
SitePro VFA12-HD (Verizon Proposed)	145	ANT150F2 (Inverted)	20	

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A500M-58	58 ksi	70 ksl	A36	36 ksl	58 ksi

TOWER DESIGN NOTES

- Tower designed for a 90 mph basic wind in accordance with the TIA/EIA-222-F Standard.
 Tower is also designed for a 90 mph basic wind with 0.50 in ice.
 Deflections are based upon a 90 mph wind,

Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587

23134.00 - East	Haddam	
Project: 160-ft Lattice Tow	er (CSP #52)	
Sient: Verizon	Drawn by: TJL	App'd:
ode: TIA/EIA-222-F	Date: 10/09/23	Scale: NTS
Path:		Dwg No. E-1

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Job	23134.00 - East Haddam	Page 1 of 3
	23 134.00 - East Haudaill	
Project	160-ft Lattice Tower (CSP #52)	Date 09:41:56 10/09/23
Client	Verizon	Designed by TJL

Load Combinations

Carrit		Description
Comb. No.		Desc. phon
1	Dead Only	
2	Dead+Wind 0 deg - No Ice	
3	Dead+Wind 30 deg - No Ice	
4	Dead+Wind 45 deg - No Ice	
5	Dead+Wind 60 deg - No Ice	
6	Dead+Wind 90 deg - No Ice	
7	Dead+Wind 120 deg - No Ice	
8	Dead+Wind 125 deg - No Ice	
9	Dead+Wind 155 deg - No Ice	
10	Dead+Wind 180 deg - No Ice	
11	Dead+Wind 180 deg - No Ice	
12	Dead+Wind 225 deg - No Ice	
13	Dead+Wind 240 deg - No Ice	
14	Dead+Wind 270 deg - No Ice	
15	Dead+Wind 300 deg - No Ice	
16	Dead+Wind 315 deg - No Ice	
17	Dead+Wind 313 deg - No Ice	
18	Dead+Ice+Temp	
19	Dead+Wind 0 deg+Ice+Temp	
20	Dead+Wind 30 deg+Ice+Temp	
21	Dead+Wind 45 deg+Ice+Temp	
22	Dead+Wind 60 deg+Ice+Temp	
23	Dead+Wind 90 deg+Ice+Temp	
24	Dead+Wind 120 deg+Ice+Temp	
25	Dead+Wind 135 deg+Ice+Temp	
26	Dead+Wind 155 deg+Ice+Temp	
27	Dead+Wind 180 deg+Ice+Temp	
28	Dead+Wind 210 deg+Ice+Temp	
29	Dead+Wind 225 deg+Ice+Temp	
30	Dead+Wind 240 deg+Ice+Temp	
31	Dead+Wind 270 deg+Ice+Temp	
32	Dead+Wind 300 deg+Ice+Temp	
33	Dead+Wind 315 deg+Ice+Temp	
34	Dead+Wind 330 deg+Ice+Temp	
35	Dead+Wind 0 deg - Service	
36	Dead+Wind 30 deg - Service	
37	Dead+Wind 45 deg - Service	
38	Dead+Wind 60 deg - Service	
39	Dead+Wind 90 deg - Service	
40	Dead+Wind 120 deg - Service	
41	Dead+Wind 135 deg - Service	
42	Dead+Wind 150 deg - Service	
43	Dead+Wind 180 deg - Service	
44	Dead+Wind 210 deg - Service	
45	Dead+Wind 225 deg - Service	
46	Dead+Wind 240 deg - Service	
47	Dead+Wind 270 deg - Service	
	Dead+Wind 300 deg - Service	
48 49	Dead+Wind 300 deg - Service Dead+Wind 315 deg - Service	

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FAX: (203) 488-8587

Job		Page
	23134.00 - East Haddam	2 of 3
Project		Date
	160-ft Lattice Tower (CSP #52)	09:41:56 10/09/23
Client		Designed by
	Verizon	TJL

Section	Elevation	Horz.	Gov.	Tilt	Twist	
No.		Deflection	Load			
	ft	in	Comb.	0	۰	
T1	160 - 140	6.698	46	0.3266	0.1213	
T2	140 - 120	5.318	46	0.3236	0.1127	
T3	120 - 100	3.970	46	0.2932	0.0799	
T4	100 - 80	2.757	46	0.2513	0.0619	
T5	80 - 60	1.749	46	0.1927	0.0445	
T6	60 - 40	0.993	46	0.1397	0.0320	
T7	40 - 20	0.464	46	0.0931	0.0205	
Т8	20 - 0	0.118	46	0.0422	0.0100	

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	٥	ft
160.00	10-ft Lighting Rod	46	6.698	0.3266	0.1213	Inf
145.00	MT6413-77A	46	5.662	0.3266	0.1174	578923
120.00	PA6-65	46	3.970	0.2932	0.0799	32773
100.00	ANT150F2	46	2,757	0.2513	0.0619	23511
85.00	SD314	46	1.979	0.2078	0.0487	19271
80.00	ANT150F2	46	1.749	0.1927	0.0445	18406
55.00	531-70HD	46	0.842	0.1281	0.0291	22205
25.00	ANT150F2	46	0.182	0.0545	0.0125	18019
20.00	ANT150F2	46	0.118	0.0422	0.0100	16526

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No_*		Deflection	Load		
	ft	in	Comb.	· ·	0
T1	160 - 140	8.301	30	0.4012	0.1476
T2	140 - 120	6.607	30	0.3973	0.1387
T3	120 - 100	4.953	30	0.3611	0.1028
T4	100 - 80	3.458	30	0.3111	0.0820
T5	80 - 60	2.207	30	0.2402	0.0605
T6	60 - 40	1.260	30	0.1751	0.0442
T7	40 - 20	0.593	30	0.1173	0.0286
T8	20 - 0	0.153	30	0.0533	0.0141

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	o	0	ft
160.00	10-ft Lighting Rod	30	8.301	0.4012	0.1476	Inf
145.00	MT6413-77A	30	7.030	0.4009	0.1437	346582
120.00	PA6-65	30	4.953	0.3611	0.1028	28300
100.00	ANT150F2	30	3.458	0.3111	0.0820	19812
85.00	SD314	30	2.493	0.2585	0.0657	15905
80.00	ANT150F2	30	2.207	0.2402	0.0605	15119

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Job	23134.00 - East Haddam	Page 3 of 3
Project	160-ft Lattice Tower (CSP #52)	Date 09:41:56 10/09/23
Client	Verizon	Designed by TJL

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	•	٥	fi
55.00	531-70HD	30	1.071	0.1607	0.0404	18084
25.00	ANT150F2	30	0.235	0.0689	0.0177	14381
20.00	ANT150F2	30	0.153	0.0533	0.0141	13196

Section Capacity Table

	Elevation	Component	Size	Critical	P	SF*Pallow	%	Pass
Section	Elevation ft	Туре	5/20	Element	lb	lb	Capacity	Fail
No. T1	160 - 140	Leg	#12ZG-58-1.5" - 1.00"	1	-8351.40	211311.15	79.2	Pass
T2	140 - 120	Leg	#12ZG-58-1.5" - 1.00"	20	-22542.50	211311.15	18.6	Pass
T3	120 - 100	Leg	#12ZG-58-1.75" - 1.00"	36	-56256.70	296839.09	67.2	Pass
T4	100 - 80	Leg	#12ZG-58-1.75" - 1.00"	52	-120710.00	296839.09	40.7	Pass
T5	80 - 60	Leg	#12ZG-58-2.00" - 0.875"	67	-146222.00	348940.73	41.9	Pass
15 T6	60 - 40	Leg	#12ZG-58-2.25" - 0.875"	76	-192537.00	441840.16	43.6	Pass
16 T7	40 - 20	Leg	#12ZG-58-2.25" - 0.875"	85	-228786.00	441840.16	51.8	Pass
	20 - 0	Leg	#12ZG-58-2.50" - 0.875"	94	-271572.00	545771.50	49.8	Pass
T8		Diagonal	L3x3x5/16	8	-5241.32	26705.85	19.6	Pass
T1	160 - 140	Diagonai	L3K3K3/10	ŭ			32.9 (b)	
Т2	140 - 120	Diagonal	L3x3x5/16	23	-7808.46	21046.07	37.1	Pass
		•					56.2 (b)	_
Т3	120 - 100	Diagonal	L3x3x5/16	41	-9716.22	16822.59	57.8	Pass
							66.1 (b)	
T4	100 - 80	Diagonal	L3x3x5/16	56	-9966.91	13596.07	73.3	Pass
T5	80 - 60	Diagonal	2L3 1/2x3 1/2x1/4	71	-17527.70	36378.90	48.2	Pass
							58.6 (b)	
Т6	60 - 40	Diagonal	2L3 1/2x3 1/2x1/4	80	-16153.60	32964.16	49.0	Pass
10	00 10	8					71.1 (b)	
T7	40 - 20	Diagonal	2L3 1/2x3 1/2x1/4	89	-18757.30	29813.61	62.9	Pass
T8	20 - 0	Diagonal	2L3 1/2x3 1/2x1/4	98	-17155.80	26949.93	63.7	Pass
10	20 0	2.26					77.4 (b)	
T1	160 - 140	Top Girt	L3x3x3/16	4	-381.52	12042.60	3.2	Pass
T3	120 - 100	Top Girt	L3x3x3/16	39	-1371.63	7125.80	19.2	Pass
13	120 - 100	Top Gar					Summary	
						Leg (T1)	79.2	Pass
						Diagonal	77.4	Pass
						(T8)		
						Top Girt	19.2	Pass
						(T3)		
						Bolt Checks	77.4	Pass
						RATING =	79.2	Pass

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NT=K engineering

Centered on Solutions* 63-2 North Branford Road Branford, CT 06405

F: (203) 488-8587

Subject:

Location:

Rev. 0: 10/6/23

Anchor Bolt Analysis

160-ft Lattice Tower East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C. Job No. 23134.00

Anchor Bolt Analysis:

Input Data:

Tower Reactions:

Tension Force = Tension := 214-kips

(Input From tnxTower)

Compression Force =

Compression := 254-kips

(Input From tnxTower)

Shear Force =

Shear := 30-kips

(Input From tnxTower)

Anchor Bolt Data:

ASTM F1554-105

Per Valmont Drawing 266040T dated 7/21/2016

Number of Anc hor Bolts =

N:= 12

(User input)

Bolt Ultimate Strength =

(User Input)

Bolt Yield Strength =

 $F_{\text{LL}} := 125 \cdot \text{ksi}$ $F_V := 105 \cdot ksi$

(User input)

Bolt Modulus=

E := 29000-ksi

(User Input)

Diameter of Anchor Bolts =

D:= 1.25·in

(User Input)

Threads per Inch =

n := 7

(User Input)

Length from Top of Pier to Bottom of Leveling Nut= $L_{ar} := 0 \cdot in$

(User Input)



63-2 North Branford Road Branford, CT 06405

Subject:

Location:

Anchor Bolt Analysis

160-ft Lattice Tower East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C. Job No. 23134.00

Rev. 0: 10/6/23

Anchor Bolt Analysis:

Calculated Anchor Bolt Properties:

GrossArea of Bol t=
$$A_g := \frac{\pi}{4} \cdot D^2 = 1.227 \cdot \text{in}^2$$

NetArea of Bdt =
$$A_n := \frac{\pi}{4} \cdot \left(D - \frac{0.9743 \cdot in}{n}\right)^2 = 0.969 \cdot in^2$$

Net Diameter =
$$D_n := \frac{2 \cdot \sqrt{A_n}}{\sqrt{\pi}} = 1.111 \cdot \text{in}$$

$$\mbox{Radius of Gyration of Bolt} = \mbox{$r:=$} \frac{\mbox{D_n}}{\mbox{4}} = 0.278 \cdot \mbox{in}$$

Elastic Section Modulus of Bolt =
$$S_x := \frac{\pi \cdot D_n^3}{32} = 0.135 \cdot \text{in}^3$$

Plastic Section Modulus of Bolt =
$$Z_{\chi} := \frac{D_{\eta}^{3}}{6} = 0.228 \cdot \text{in}^{3}$$

Anchor Bolt Design Strength:

Resistance Factor for Flexure =
$$\phi_f := 0.9$$

Resistance Factor for Compression =
$$\phi_c := 0.9$$

$$\text{Resistance Factor for Tension} = \qquad \qquad \varphi_t \coloneqq 0.75$$

$$\mbox{Resistance Factor for Shear} = \qquad \qquad \varphi_{\mbox{V}} := 0.75$$

Design Tensile Strength =
$$\Phi R_{nt} := \Phi_t \cdot F_u \cdot A_n = 90.9 \cdot k$$

Design Compression Strength =
$$\Phi R_{nc} := \varphi_c \cdot F_y \cdot A_g = 116 \cdot k$$

Design Shear Strength (Tension) =
$$\Phi R_{nv} := \phi_v \cdot 0.5 F_u \cdot A_q = 57.5 \cdot k$$

$$\text{Design Shear Strength (Compression)} = \\ \qquad \qquad \Phi R_{\text{NVC}} := \ \varphi_{\text{C}} \cdot 0.6 F_{\text{y}} \cdot A_{\text{g}} \cdot 0.75 = 52.2 \cdot \text{k}$$

T=K engineering

Subject:

Anchor Bolt Analysis

Centered on Solutions * ***
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Presided CT 06405 P: (203) 488-8587

Location:

Rev. 0: 10/6/23

160-ft Lattice Tower East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

Job No. 23134.00

Check Anc hor Bolt Tension Force:

$$P_{ut} := \frac{Tension}{N} = 17.8 \text{ kips}$$

$$P_{UC} := \frac{Compression}{N} = 21.2 \cdot kips$$

$$V_u := \frac{Shear}{N} = 2.5 \cdot kips$$

$$Condition1 := it \left[\left(\frac{P_{ut}}{\Phi R_{nt}} \right)^2 + \left(\frac{V_u}{\Phi R_{nv}} \right)^2 \right] \leq 1.00, "OK", "Overstressed" \right]$$

Condition1 = "OK"

Condition2 := if
$$\left[\left(\frac{P_{uc}}{\Phi R_{nc}} \right) + \left(\frac{V_u}{\Phi R_{nvc}} \right)^2 \right] \le 1.00, \text{"OK"}, \text{"Overstressed"}$$

Condition2 = "OK"

$$\text{max}\!\!\left[\!\!\left(\frac{P_{ut}}{\Phi R_{nt}}\!\right)^{\!2} + \!\left(\frac{V_{u}}{\Phi R_{nv}}\!\right)^{\!2}, \!\!\left(\frac{P_{uc}}{\Phi R_{nc}}\!\right) + \!\left(\frac{V_{u}}{\Phi R_{nvc}}\!\right)^{\!2}\!\!\right] = 18.5 \cdot \%$$



Centered on Solutions* ***
63-2 North Branford Road Pr. (203) 488-6580 F: (203) 488-8587

FOUNDATION ANALYSIS Subject:

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C. Job No. 23134.00

(User Input)

Pier and Mat Foundation Analysis:

Input Data:

Location:

Rev. 0: 10/3/23

Tower Data		
Overturning Moment =	OM := 4528·ft·kips	(User Input from tnxTower)
Shear Force =	$S_t := 48.9 \cdot \text{kip}$	(User Input from tnxTower)
Axial Force =	$WT_t := 50.3 \cdot kip$	(User Input from tnxTower)
Max Compression Force =	C _t := 254 kip	(User Input from InxTower)
Max Uplift Force =	U _t := 214⋅kip	(User Input from tnxTower)
Tower Height=	H _t := 160⋅ft	(User Input)
Tower Width =	$W_t := 22 \cdot ft$	(User Input)
Tower Position on Foundation (1=offset, 2=centered) =	Pos _t := 1	(User Input)
Fooling Data:		
Overall Depth of Footing =	D _f := 5·ft	(User Input)
Length of Pier =	L _p := 3.25·ft	(User Input)
Extension of Pier Above Grade =	L _{pag} := 0.5⋅ft	(User Input)
Diameter of Pier =	d _p := 4.0⋅ft	(User Input)
Thickness of Footing =	$T_f := 2.25 \cdot ft$	(User Input)
Width of Footing =	$W_f := 36 \cdot ft$	(User Input)
Material Properties:		
Concrete Compressive Strength =	f _c := 4500⋅psi	(User Input)
Steel Reinforcment Yield Strength =	f _y := 60000⋅psi	(User Input)
Internal Friction Angle of Soil =	$\Phi_{S} \coloneqq 30 \text{-} deg$	(User Input)
Ultimate Soil Bearing Capacity =	$q_S := 8000 \cdot psf$	(User Input)
Unit Weight of Soil =	$\gamma_{Soil} \coloneqq 120 \cdot pcf$	(User Input)
Unit Weight of Concrete =	$\gamma_{conc} := 150 \cdot pcf$	(User Input)
Foundation Bouyancy =	Bouyancy := 0	(User Input) (Yes=1 / No=0)
Depth to Neglect =	n:= 0·ft	(User Input)
Cohesion of Clay Type Soil =	c:= 0-ksf	(Use 0 for Sandy Soil)
Seismic Zone Factor =	Z:= 2	(User Input) (UBC-1997 Fig 23-2)

 $\mu := 0.45$

Coefficient of Friction Between Concrete =



Centered on Solutions 63-2 North Branford Road Pt (203) 488-0580 Ft (203) 488-8587

Subject:

Location:

FOUNDATION ANALYSIS

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C. Job No. 23134.00

Rev. 0: 10/3/23

Pier Reinforcement:

Bar Size =	BS _{pier} := 8	(User Input)	
Bar Diameter =	d _{bpier} := 1.00·in	(User Input)	
Number of Bars =	NB _{pier} := 23	(User Input)	
Clear Cover of Reinforcement =	Cvr _{pier} := 3-in	(User Input)	
Reinforcement Location Factor =	$\alpha_{\text{pier}} = 1.0$	(User Input)	(ACI-2008 12.2.4)
Coating Factor =	β _{pier} := 1.0	(User Input)	(ACI-2008 12.2.4)
Concrete Strength Factor =	$\lambda_{pier} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Reinforcement Size Factor =	$\gamma_{\text{pier}} = 1.0$	(User Input)	(ACI-2008 12.2.4)
Diameter of Tie =	$d_{Tie} = 0.5 \cdot in$	(User Input)	
Pad Reinforcement:			
Bar Size =	BS _{top} := 8	(User Input)	(Top of Pad)
Bar Size ≃ Bar Diameter =	$BS_{top} := 8$ $d_{btop} := 1.00 \cdot in$	(User Input) (User Input)	(Top of Pad) (Top of Pad)
Bar Diameter =	d _{btop} ≔ 1.00·in	(User Input)	(Top of Pad)
Bar Diameter = Number of Bars =	d _{btop} := 1.00·in NB _{top} := 76	(User Input) (User Input)	(Top of Pad) (Top of Pad)
Bar Diameter = Number of Bars = Bar Size =	$d_{btop} := 1.00 \cdot in$ $NB_{top} := 76$ $BS_{bot} := 8$	(User Input) (User Input) (User Input)	(Top of Pad) (Top of Pad) (Bottom of Pad)
Bar Diameter = Number of Bars = Bar Size = Bar Diameter =	$d_{btop} := 1.00 \cdot in$ $NB_{top} := 76$ $BS_{bol} := 8$ $d_{bbot} := 1.00 \cdot in$	(User Input) (User Input) (User Input) (User Input)	(Top of Pad) (Top of Pad) (Bottom of Pad) (Bottom of Pad)
Bar Diameter = Number of Bars = Bar Size = Bar Diameter = Number of Bars =	$d_{btop} := 1.00 \cdot in$ $NB_{top} := 76$ $BS_{bot} := 8$ $d_{bbot} := 1.00 \cdot in$ $NB_{bot} := 76$	(User Input) (User Input) (User Input) (User Input) (User Input)	(Top of Pad) (Top of Pad) (Bottom of Pad) (Bottom of Pad)
Bar Diameter = Number of Bars = Bar Size = Bar Diameter = Number of Bars = Clear Cover of Reinforcement =	$d_{btop} := 1.00 \cdot in$ $NB_{top} := 76$ $BS_{bot} := 8$ $d_{bbot} := 1.00 \cdot in$ $NB_{bot} := 76$ $Cvr_{pad} := 3.0 \cdot in$	(User Input) (User Input) (User Input) (User Input) (User Input) (User Input)	(Top of Pad) (Top of Pad) (Bottom of Pad) (Bottom of Pad) (Bottom of Pad)
Bar Diameter = Number of Bars = Bar Size = Bar Diameter = Number of Bars = Clear Cover of Reinforcement = Reinforcement Location Factor =	$d_{btop} := 1.00 \cdot in$ $NB_{top} := 76$ $BS_{bot} := 8$ $d_{bbot} := 1.00 \cdot in$ $NB_{bot} := 76$ $Cvr_{pad} := 3.0 \cdot in$ $\alpha_{pad} := 1.0$	(User Input)	(Top of Pad) (Top of Pad) (Bottom of Pad) (Bottom of Pad) (Bottom of Pad) (Bottom of Pad)

 $\gamma_{pad} = 1.0$

(User Input)

(ACI-2008 12.2.4)

Calculated Factors:

Reinforcement Size Factor =

 $A_{bpier} := \frac{\pi \cdot d_{bpier}^2}{4} = 0.785 \cdot in^2$ $A_{btop} := \frac{\pi \cdot d_{btop}^2}{4} = 0.785 \cdot in^2$ $A_{bbot} := \frac{\pi \cdot d_{bbot}^2}{4} = 0.785 \cdot in^2$ Pier Reinforcement Bar Area = Pad Top Reinforcement Bar Area = Pad Bottom Reinforcement Bar Area = Coefficient of Lateral Soil Pressure =

> Load Factor = LF := 1

Subject:

FOUNDATION ANALYSIS

Centered on Solutions 63-2 North Branford Road Branford, CT 06405

Location:

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

Rev. 0: 10/3/23

Job No. 23134.00

Stability of Footing:

$$\gamma_{\rm C} := if(\text{Bouyancy} = 1, \gamma_{\rm conc} - 62.4 \text{pcf}, \gamma_{\rm conc}) = 150 \cdot \text{pcf}$$

$$\gamma_s := if(Bouyancy = 1, \gamma_{soil} - 62.4pcf, \gamma_{soil}) = 120 \cdot pcf$$

$$P_{pn} := K_p \cdot \gamma_s \cdot n + c \cdot 2 \cdot \sqrt{K_p} = 0 \text{ ksf}$$

$$\textbf{P}_{pt} \coloneqq \textbf{K}_{p} \cdot \gamma_{s} \cdot \left(\textbf{D}_{f} - \textbf{T}_{f} \right) + c \cdot 2 \cdot \sqrt{\textbf{K}_{p}} = 0.99 \cdot \text{ksf}$$

$$P_{top} := if \left[n < \left(D_f - T_f \right), P_{pt}, P_{pn} \right] = 0.99 \cdot ksf$$

$$P_{bot} := K_{p'} \gamma_{s'} D_f + c \cdot 2 \cdot \sqrt{K_p} = 1.8 \cdot ksf$$

$$P_{ave} := \frac{P_{top} + P_{bot}}{2} = 1.395 \cdot ksf$$

$$T_{D} \coloneqq \text{if} \left\lceil n < \left(D_{f} - T_{f}\right), T_{f}, \left(D_{f} - n\right) \right\rceil = 2.25 \cdot \text{ft}$$

$$A_D := W_f T_D = 81 \cdot ft^2$$

Ultimate Shear =

$$S_u := P_{ave} \cdot A_p = 112.995 \cdot kip$$

$$WT_c := \left[\left(W_f^2 \cdot T_f \right) + (4) \cdot \left(\frac{d_p^2 \cdot \pi}{4} L_p \right) \right] \cdot \gamma_c = 461.904 \cdot kip$$

Weight of Soil Above Footing =

$$WT_{s1} := \left[\left[W_f^2 - (4) \cdot \left(\frac{d_p^2 \cdot \pi}{4} \right) \right] \cdot \left(\left| L_p - L_{pag} - n \right| \right) \right] \cdot \gamma_s = 411.09 \cdot \text{kip}$$

Weight of Soil Wedge at Back Face =

$$WT_{s2} := \left[\frac{\left(D_f - n\right)^2 \cdot tan\left(\Phi_s\right)}{2} \cdot W_f \right] \cdot \gamma_s = 31.177 \cdot kip$$

Tower Offset =

$$X_{t1} \coloneqq \left[\frac{W_f}{2} - \frac{\left(W_{t} \cdot \cos(30 \cdot \deg)\right)}{2} \right] \qquad X_{t2} \coloneqq \frac{W_f}{2} - \frac{\left(W_{t} \cdot \cos(30 \cdot \deg)\right)}{3}$$

$$X_{t2} := \frac{W_f}{2} - \frac{(W_t \cdot \cos(30 \cdot \deg))}{2}$$

$$X_t := if(Pos_t = 1, X_{t1}, X_{t2}) = 8.474$$

$$X_{\text{off1}} := \frac{W_f}{2} - \left[\frac{\left(W_t \cdot \cos(30 \cdot \text{deg})\right)}{3} + X_t \right] = 3.175$$

$$X_{\text{off}} := if(Pos_t = 1, X_{\text{off1}}, X_{\text{off2}})$$

Total Weight =

$$WT_{tot} := 0.9WT_{c} + 0.75WT_{s1} + WT_{t} = 774.3 \cdot kip$$

Resisting Moment =

$$\mathsf{M}_{f} \coloneqq \left(\mathsf{WT}_{tot}\right) \cdot \frac{\mathsf{W}_{f}}{2} + 0.9 \mathsf{WT}_{t} \cdot \left(\frac{\mathsf{W}_{f}}{2} - \mathsf{X}_{off}\right) + 0.75 \left(\mathsf{S}_{u'} \cdot \frac{\mathsf{T}_{p}}{3}\right) + 0.75 \mathsf{WT}_{s2} \cdot \left[\mathsf{W}_{f} + \frac{\left(\mathsf{D}_{f} - n\right) \cdot \mathsf{tan}\left(\Phi_{s}\right)}{3}\right] = 15537 \cdot \mathsf{kip} \cdot \mathsf{ft}$$

Overturning Moment =

$$M_{ot} := OM + S_t \cdot (L_p + T_f) = 4797 \cdot kip \cdot ft$$

Factor of SafetyActual =

$$FS := \frac{M_r}{M_{of}} = 3.24$$

Factor of Safety Required =

OverTurning_Moment_Check = "Okay"

Branford, CT 06405

FOUNDATION ANALYSIS

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C. Job No. 23134.00

Rev. 0: 10/3/23

Shear Capacity in Pier:

$$S_p := \frac{P_{ave} \cdot A_p + \mu \cdot WT_{lot}}{FS_{req}} = 461.445 \cdot kips$$

$$Shear_Check := if \Big(S_p > S_t, "Okay", "No Good"\Big)$$

Bearing Pressure Caused by Footing:

$$\mathsf{Load}_{tot} := \mathsf{WT}_c + \mathsf{WT}_{s1} + \mathsf{WT}_t = 923 \cdot \mathsf{kip}$$

$$A_{mat} := W_f^2 = 1.296 \times 10^3$$

$$S := \frac{W_f^3}{6} = 7776 \cdot ft^3$$

$$P_{\text{max}} := \frac{\text{Load}_{\text{tot}}}{A_{\text{mat}}} + \frac{M_{\text{ot}}}{S} = 1.329 \cdot \text{ksf}$$

$$\label{eq:max_pressure_check} \text{Max_Pressure_Check} := \text{if} \Big(\text{P}_{\text{max}} < 0.75 q_{\text{s}}, \text{"Okay"}, \text{"No Good"} \Big)$$

$$P_{min} := \frac{Load_{tot}}{A_{mat}} - \frac{M_{ot}}{s} = 0.096 \cdot ksf$$

$$\label{eq:min_pressure_check} \begin{aligned} &\text{Min_Pressure_Check} := \text{if} \Big[\Big(P_{min} \geq 0 \Big) \cdot \Big(P_{min} < 0.75 q_{s} \Big), \text{"Okay"} \text{, "No Good"} \Big] \end{aligned}$$

$$X_p := \frac{P_{max}}{\frac{P_{max} - P_{min}}{W_f}} \cdot \frac{1}{3} = 12.929$$

$$X_k := \frac{W_f}{6} = 6$$

Since Resultant Force is Not in Kern, Area to which Pressure is Applied Must be Reduced.

$$e := \frac{M_{ot}}{L_{oad}_{tot}} = 5.195$$

$$P_a := \frac{2 \cdot Load_{tot}}{3 \cdot W_f \left(\frac{W_f}{2} - e\right)} = 1.335 \cdot ksf$$

$$q_{adj} := if(P_{min} < 0, P_a, P_{max}) = 1.329 \cdot ksf$$



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Subject:

Location:

FOUNDATION ANALYSIS

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

Job No. 23134.00

Rev. 0: 10/3/23

Concrete Bearing Capacity:

Strength Reduction Factor =

 $\Phi_{\rm C} \coloneqq 0.65$

(ACI-2008 9,3.2.2)

Bearing Strength Between Pier and Pad =

$$P_b := \Phi_c \cdot 0.85 \cdot f_c \cdot \frac{\pi \cdot d_p^2}{4} = 4.499 \times 10^3 \cdot \text{kips}$$

Bearing_Check := $if(P_b > LF \cdot C_t, "Okay", "No Good")$

Bearing_Check = "Okay"

Shear Strength of Concrete:

Beam Shear:

(Critical section located at a distance d from the face of Pier)

(ACI 11.3.1.1)

$$\phi_{c} := 0.85$$

(ACI 9.3.2.5)

$$d := T_f - Cvr_{pad} - d_{bbot} = 23 \cdot in$$

$$FL := LF \cdot \frac{C_1}{W_e^2} = 0.196 \cdot ksf$$

$$V_{req} := FL \cdot (X_t - .5 \cdot d_p - d) \cdot W_f = 32.153 \cdot kips$$

$$V_{Avail} := \phi_c \cdot 2 \cdot \sqrt{f_c \cdot psi} \cdot W_f \cdot d = 1133 \cdot kip$$

(ACI-2008 11.2.1.1)

$$Beam_Shear_Check := if \Big(V_{req} < V_{Avail}, "Okay", "No Good" \Big)$$

Beam_Shear_Check = "Okay"

Punching Shear:

(Critical Section Located at a distance of d/2 from the face of pier)

(ACI 11.11.1.2)

Critical Perimeter of Punching Shear =

$$b_0 := (d_0 + d) \cdot \pi = 18.6$$

Area included Inside Perimeter =

$$A_{bo} := \frac{\pi \cdot (d_p + d)^2}{4} = 27.5$$

Required Shear Strength =

$$V_{req} := FL \cdot \left(W_f^2 - A_{bo}\right) = 249 \cdot kips$$

Available Shear Strength =

$$V_{Avail} := \phi_c \cdot 4 \cdot \sqrt{f_c \cdot psi} \cdot b_o \cdot d = 1170.1 \cdot kip$$

(ACI-2008 11.11.2.1)

Punching_Shear_Check := if(V_{req} < V_{Avail}, "Okay", "No Good")

Punching_Shear_Check = "Okay"

CENTEK engineering

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FOUNDATION ANALYSIS

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C. Job No. 23134.00

Rev. 0: 10/3/23 Job No.

Steel Reinforcement in Pad:

Required Reinforcement for Bending:

Strength Reduction Factor =

$$\varphi_{\boldsymbol{m}} \coloneqq .90$$

(ACI-2008 9.3.2.1)

Maximum Momentin Pad =

(User Input)

Design Moment =

Subject:

Location:

$$M_{n} := \frac{LF \cdot M_{max}}{\phi_{m}} = 638.889 \cdot \text{kips-ft}$$

$$\beta := \begin{bmatrix} 0.85 & \text{if } 2500 \cdot psi \leq f_C \leq 4000 \cdot psi \\ 0.65 & \text{if } f_C > 8000 \cdot psi \end{bmatrix} = 0.6 \\ 0.85 - \begin{bmatrix} \frac{f_C}{psi} - 4000 \\ 1000 \end{bmatrix} \cdot 0.5 \end{bmatrix} \text{ otherwise }$$

$$b_{eff} := W_t \cdot cos(30 \cdot deg) + d_p = 276.631 \cdot in$$

$$A_s \coloneqq \frac{M_n}{\left(f_V \cdot d\right)} = 5.556 \cdot in^2$$

$$a := \frac{A_s \cdot f_y}{\beta \cdot f_c \cdot b_{eff}} = 0.446 \cdot in$$

$$A_S := \frac{M_n}{f_y \cdot \left(d - \frac{a}{2}\right)} = 5.61 \cdot in^2$$

$$\rho \coloneqq \frac{A_S}{b_{eff}d} = 0.01058 \cdot in$$

Subject:

FOUNDATION ANALYSIS

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Location:

Rev. 0: 10/3/23

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

Job No. 23134.00

Required Reinforcement for Temperature and Shrinkage:

$$\rho_{\text{sh}} := \begin{bmatrix} .0018 & \text{if} & f_y \geq 60000 \cdot \text{psi} \\ .0020 & \text{otherwise} \end{bmatrix}$$
 (ACI -2008 7.12.2.1)

Check Bottom Bars:

$$\text{As} := \text{if} \left(\rho \geq \rho_{\text{Sh}}, A_{\text{S}}, \rho_{\text{Sh}} \cdot \frac{b_{\text{eff}}}{2} \cdot d \right) = 5.7 \cdot \text{in}^2$$

$$As_{prov} := A_{bbot} \cdot NB_{bot} = 59.7 \cdot in^2$$

Pad_Reinforcement_Bot := if(As_{prov} > As, "Okay", "No Good")

Pad_Reinforcement_Bot = "Okay"

Check top Bars:

$$As := \text{if} \left(\rho \geq \rho_{sh}, A_s, \rho_{sh} \cdot \frac{b_{eff}}{2} \cdot d \right) = 5.7 \cdot in^2$$

$$As_{prov} := A_{btop} \cdot NB_{top} = 59.7 \cdot in^2$$

Pad_Reinforcement_Top := if(As_{prov} > As, "Okay", "No Good")

Pad_Reinforcement_Top = "Okay"

Developement Length Pad Reinforcement:

$$\mathsf{B}_{\mathsf{sPad}} \coloneqq \frac{\mathsf{W}_{\mathsf{f}} - 2 \cdot \mathsf{Cvr}_{\mathsf{pad}} - \mathsf{NB}_{\mathsf{bot}} \cdot \mathsf{d}_{\mathsf{bbot}}}{\mathsf{NB}_{\mathsf{bot}} - 1} = 4.67 \cdot \mathsf{in}$$

$$c := if \left(\text{Cvr}_{pad} < \frac{B_{s} p_{ad}}{2}, \text{Cvr}_{pad}, \frac{B_{s} p_{ad}}{2} \right) = 2.333 \cdot \text{in}$$

Transverse Reinforcement Index =

$$k_{tr} = 0$$

(ACI-2008 12.2.3)

$$L_{dbt} \coloneqq \frac{3 f_y \alpha_{pad} \cdot \beta_{pad} \cdot \gamma_{pad} \cdot \lambda_{pad}}{40 \cdot \sqrt{f_c \cdot psi} \cdot \frac{c + k_{tr}}{d_{bbot}}} \cdot d_{bbot} = 28.7 \cdot in$$

Minimum Development Length =

$$L_{dbmin} := 12 \cdot in$$

(ACI-2008 12.2.1)

Available Length in Pad =

$$L_{dbt} Check \coloneqq \textit{if} \Big(L_{dbt} \geq L_{dbmin}, \text{"Use L.dbt"}, \text{"Use L.dbmin"} \Big) = \text{"Use L.dbt"}$$

$$L_{Pad} := \frac{W_f}{2} - \frac{W_t}{2} - Cvr_{pad} = 81 \cdot in$$

Lpad_Check:= if(Lpad > Ldbt, "Okay", "No Good")

Subject:

FOUNDATION ANALYSIS

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Location:

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C. Job No. 23134.00

Rev. 0: 10/3/23

Steel Reinforcement in Pier:

Area of Pier =

$$A_p := \frac{\pi \cdot d_p^2}{4} = 1809.56 \cdot in^2$$

$$A_{smin} := 0.01 \cdot 0.5 \cdot A_p = 9.05 \cdot in^2$$

(ACI-2008 10.8.4 & 10.9.1)

$$A_{sprov} := NB_{pier} \cdot A_{bpier} = 18.06 \cdot in^2$$

Steel_Area_Check = "Okay"

Bar Spacing In Pier =

$$B_{\text{SPier}} \coloneqq \frac{d_{p} \cdot \pi}{\text{NB}_{pier}} - d_{\text{bpier}} = 5.556 \cdot \text{in}$$

Diameter of Reinforcement Cage =

$$Diam_{cage} := d_p - 2 \cdot Cvr_{pier} = 42 \cdot in$$

Maximum Moment in Pier =

$$M_p := S_t \cdot (L_p) \cdot LF = 1907.1 \cdot in \cdot kips$$

Pier Check evaluated from outside program and results are listed below;

$$(D \ N \ n \ P_{u} \ M_{XU}) = (48 \ 23 \ 8 \ 338.582 \ 1.907 \times 10^{3})$$

$$\left(\Phi P_n \Phi M_{xn} f_{sp} \rho \right) := (0 \ 0 \ 0)$$

$$\begin{pmatrix} \Phi P_n & \Phi M_{XN} & f_{SP} & \rho \end{pmatrix} := \Phi P'_n \begin{pmatrix} D, N, n, P_u, M_{XU} \end{pmatrix}^T$$

$$(\Phi_{n} \Phi_{Xn} f_{SD} \rho) = (2.889 \times 10^{3} 1.627 \times 10^{4} -21.001 0.01)$$

$$Axial_Load_Check := if(\phi P_n \ge P_{\mu}, "Okay", "No Good")$$



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Subject:

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Rev. 0: 10/3/23

FOUNDATION ANALYSIS

East Haddam, CT

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Job No. 23134.00

Development Length Pier Reinforcement:

Available Length in Foundation:

$$L_{pier} := L_p - Cvr_{pier} = 36 \cdot in$$

$$L_{pad} := T_f - Cvr_{pad} = 24 \cdot in$$

Tension:

(ACI-2008 12.2.3)

Spacing or Cover Dimension =

$$c := if \left(Cvr_{pier} < \frac{B_s Pier}{2}, Cvr_{pier}, \frac{B_s Pier}{2} \right) = 2.778 \cdot in$$

Transverse Reinforcement=

(ACI-2008 12.2.3)

$$L_{dbt} \coloneqq \frac{3 \, f_y \alpha_{pier} \cdot \beta_{pier} \cdot \gamma_{pier} \cdot \lambda_{pier}}{40 \cdot \sqrt{f_c \cdot psi} \left(\frac{c + k_{tr}}{d_{bpier}}\right)} \cdot d_{bpier} = 24.15 \cdot in$$

Minimum Development Length =

Pier reinforcement bars are standard 90 degree hooks and therefore developement in the pad is computed asfollows:

$$L_{dh} := \frac{1200 \cdot d_{bpier}}{\sqrt{\frac{f_{c}}{psi}}} \cdot .7 = 12.522 \cdot in$$
 (ACI 12.2.1)

$$L_{db} := max(L_{dbt}, L_{dbmin}) = 24.146 \cdot in$$

$$\textit{L}_{tension_Check} \coloneqq \textit{if} \Big(\textit{L}_{pier} + \textit{L}_{pad} > \textit{L}_{dbt}, \text{"Okay"}, \text{"No Good"} \Big)$$

Compression:

(ACI-2008 12.3.2)

$$L_{dbc1} := \frac{.02 \cdot d_{bpier} \cdot f_y}{\sqrt{f_c \cdot psi}} = 17.889 \cdot in$$

$$L_{dbmin} := 0.0003 \cdot \frac{in^2}{lb} \cdot \left(d_{bpier} f_y \right) = 18 \cdot in$$

$$L_{dbc} := if(L_{dbc1} \ge L_{dbmin}, L_{dbc1}, L_{dbmin}) = 18 \cdot in$$



NORTHEAST > North East > New England > Wallingford-1 > EAST HADDAM 3 CT - A Cheiban, Ziad - ziad.cheiban@verizonwireless.com - 20230801_101321

Project Details		Location Information	
Carrier Aggregation N		Site Id	Site Id 2609730
Ecip N		Search Ring#	
Project Name EA	Project Name EAST HADDAM 3 CT - NEW BUILD	E-NodeB ID# null	llnul
Project Alt Name EAST HADDAM 3 CT -	ST HADDAM 3 CT - NEW BUILD	#SFC#	PSLC# 300023
Project Id 17133983	133983	Switch Name	Switch Name Wallingford-1
Designed Sector Carrier 4G 16		Tower Type	
Designed Sector Carrier 5G 4		Site Type MACRO	MACRO
Additional Sector Carrier 4G 0		Street Address	Street Address 194 Mt Parnassus Road
Additional Sector Carrier 5G 0		City	City East Haddam
Suffix Rev	Suffix Rev1_2023-08-01	State CT	ст
FP Solution Type & Tech Type MC	FP Solution Type & Tech Type MCR;4G_700;5G_850;4G_850;4G_AWS;5G _L-Sub6;4G_PCS	Zip Code 06423	06423
		County	County Middlesex
		Latitude	Latitude 41.47003/ 41° 28' 12.108"
		Longitude	Longitude -72 41120/72° 24' 40 320"

Project Scope	New Build Macro Extension on Existing Tower

Rev1_2023-08-01: Added 4th sector. Adjusted ACL to 145'

					Ar	Antenna Summary					
Added A	ntenna										
200	850	1900	AWS	r-Sub6	Make	Model	Centerl Tip ine Height	Típ Height	Azimuth	Install Type	Quantity
				56	Samsung	MT6413-77A	145	146.2	345(A),75(B),16 5(C),255(D)	PHYSICAL	4
LTE	LTE	LIE	LTE		CommScope	NHH-45B-R2B	145	148	345(A),75(B),16 5(C),255(D)	PHYSICAL	80

	Quantity
	Install Type
	Azimuth
	Tip Height
	Centerl
	Model
	Wake
	F-Sub6
	AWS
	1900
Antenna	850
Removed	700

	Quantity
	Install Type
	Azimuth
2	Tip Height
	Centeri
	Model
	Make
	L-Sub6
	AWS
	1900
Antenna	850
Retained,	200

: 12 Retained: 0

					Ì	Non Antenna Summary			
dded Non A	ntenna								
Equipment	Location	700	850	1900	AWS	Маке	Model	Install	Quantity
RRU	Tower			LTE	LTE	Samsung	B2/B66A RRH ORAN (RF4439d-25A)	PHYSICAL	4
RRU	Tower	LTE	LTE			Samsung	RF4461d-13A	PHYSICAL	4

lent Location 700 850 1900 AWS Make Model Location Cuanti	ed Non.	Antenna								
	nent	Location	200	850	1900	AWS	Make	Model	Install	Quantity

700 88	700 850 1900	AWS Make Model	Install	Quantity
700	700	1900	50 1900 AWS Make Model	50 1900 AWS Make Model Install

Retained: 0
Removed: 0
Added: 8

Services				
1900 LTE		00	0002	
Sector	01	002	03	04
Azimuth	345	75	165	255
Cell/Enodeb-id	064182	064182	064182	064182
Antenna Model	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B
Antenna Make	CommScope	CommScope	CommScope	CommScope
Centerline	145	145	145	145
DLEARFON	1050	1050	1050	1050
Mech Down-tilt	0	0	0	0
Elect Down-tilt	0	0	0	0
Tip Height	148	148	148	148
Regulatory Power	407.15 (W/MHz) EIRP	407.15 (W/MHz) EIRP	407.15 (W/MHz) EIRP	407.15 (W/MHz) EIRP
Cell Max Power	46.0 dBm	46.0 dBm	46.0 dBm	46.0 dBm
TMA Make				
TMA Model				
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)
Number of Tx,Rx	4, 4	4,4	4,4	4,4
Position	1,5	1,5	1,5	1,5
Transmitter Id	18843902	18843906	18843910	18843914
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	10	10	10	10
Ant. Dimensions H × W × D(inch)	72.0 × 18.0 × 7.0	72.0 x 18.0 x 7.0	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0
Weight(lb)	73.6	73.6	73.6	73.6

Services				
700 LTE		00	0002	
Sector	01	02	03	04
Azimuth	345	75	165	255
Cell/Enodeb-ld	064182	064182	064182	064182
Antenna Model	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B
Antenna Make	CommScope	CommScope	CommScope	CommScope
Centerline	145	145	145	145
DLEARFON	5230	5230	5230	5230
Mech Down-tilt	0	0	0	0
Elect Down-till	2	2	2	2
Tip Height	148	148	148	148
Regulatory Power	109.59 (W/MHz) ERP	109.59 (W/MHz) ERP	109.59 (W/MHz) ERP	109.59 (W/MHz) ERP
Cell Max Power	46.0 dBm	46.0 dBm	46.0 dBm	46.0 dBm
TMA Make				
TMA Model				
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	RF4461d-13A	RF4461d-13A	RF4461d-13A	RF4461d-13A
Number of Tx,Rx	4, 4	4 , 4	4 , 4	4,4
Position	1,5	1,5	1,5	1,5
Transmitter Id	18843901	18843905	18843909	18843913
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	10	10	10	10
Ant. Dimensions H x W x D(inch)	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0	72.0 x 18.0 x 7.0
Weight(Ib)	73.6	73.6	73.6	73.6

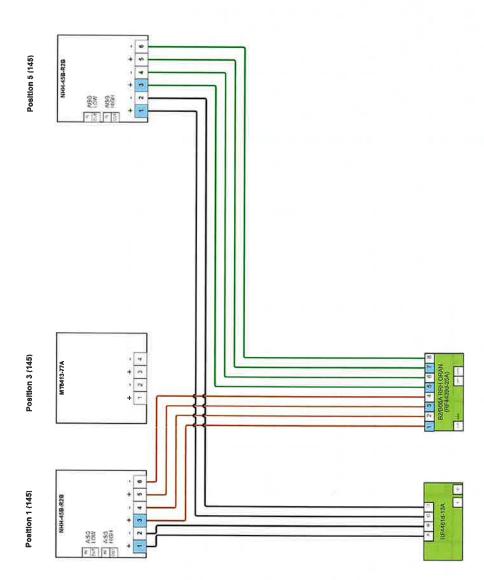
Services				
850 LTE		0002	02	
Sector	01	02	03	04
Azimuth	345	75	165	255
Cell/Enodeb-ld	064182	064182	064182	064182
Antenna Model	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B
Antenna Make	CommScope	CommScope	CommScope	CommScope
Centerline	145	145	145	145
DLEARFCN	2450	2450	2450	2450
Mech Down-tilt	0	0	0	0
Elect Down-tilt	2	2	2	2
Tip Height	148	148	148	148
Regulatory Power	395.20 (W/MHz) ERPSD	395.20 (W/MHz) ERPSD	395.20 (W/MHz) ERPSD	395.20 (W/MHz) ERPSD
Cell Max Power	44.5 dBm	44.5 dBm	44.5 dBm	44.5 dBm
TMA Make				
TMA Model				
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	RF4461d-13A	RF4461d-13A	RF4461d-13A	RF4461d-13A
Number of Tx,Rx	4,4	4 , 4	4,4	4,4
Position	1,5	1,5	1,5	1,5
Transmitter Id	18843904	18843908	18843912	18843916
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	10	10	10	10
Ant. Dimensions H x W x D(inch)	72.0 × 18.0 × 7.0			
Weight(lb)	73.6	73.6	73.6	73.6

Services				
AWS LTE		0	0002	
Sector	01	02	03	04
Azimuth	345	75	165	255
Cell/Enodeb-Id	064182	064182	064182	064182
Antenna Model	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B
Antenna Make	CommScope	CommScope	CommScope	CommScope
Centerline	145	145	145	145
DLEARFON	2050	2050	2050	2050
Mech Down-tilt	0	0	0	0
Elect Down-tilt	0	0	0	0
Tip Height	148	148	148	148
Regulatory Power	228.42 (W/MHz) EIRP	228.42 (W/MHz) EIRP	228.42 (W/MHz) EIRP	228.42 (W/MHz) EIRP
Cell Max Power	46.0 dBm	46.0 dBm	46.0 dBm	46.0 dBm
TMA Make				
TMA Model	P		7	
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)
Number of Tx,Rx	4,4	4 , 4	4 , 4	4 , 4
Position	1,5	1,5	1,5	1,5
Transmitter Id	18843903	18843907	18843911	18843915
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	20	20	20	20
Ant. Dimensions $H \times W \times D(inch)$	72.0 x 18.0 x 7.0	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0
Weight(lb)	73.6	73.6	73.6	73.6

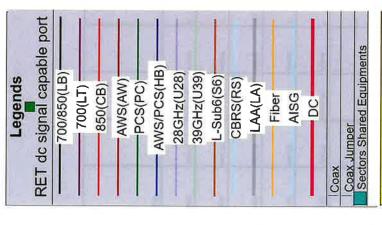
Services				
CBAND NR		00	0002	
Sector	0001	0002	0003	0004
Azimuth	345	75	165	255
Cell/Enodeb-ld	640175	640175	640175	640175
Antenna Model	MT6413-77A	MT6413-77A	MT6413-77A	MT6413-77A
Antenna Make	Samsung	Samsung	Samsung	Samsung
Centerline	145	145	145	145
DLEARFCN	650006	650006	90009	650006
Mech Down-tilt	0	0	0	0
Elect Down-tilt	7	2	2	2
Tip Height	146.2	146.2	146.2	146.2
Regulatory Power	748.75 (W/MHz) EIRP	748.75 (W/MHz) EIRP	748.75 (W/MHz) EIRP	748.75 (W/MHz) EIRP
Cell Max Power	47.0 dBm	47.0 dBm	47.0 dBm	47.0 dBm
TMA Make				
TMA Model				
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	MT6413-77A	MT6413-77A	MT6413-77A	MT6413-77A
Number of Tx,Rx	4,4	4,4	4,4	4,4
Position	3	ဧ	3	3
Transmitter Id	18843917	18843918	18843919	18843920
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	100	100	100	100
Ant. Dimensions H x W x D(inch)	29.53 x 15.75 x 5.51			
Weight(lb)	55.1	55.1	55.1	55.1

Sector	Make	Model	Ant CL Height AG	Ant Tip Height	Azimuth	Elect Down-tilt	Mech Down-tilt	Geln	Bandwidth	Regulator y Power	200	950	1900	2100	28 GHz	31 GHz	39 GHz	LSub-6	CBRS
05	CommScape	NHH-45B-R2	145	148	75	0	0	17.9	41	228.42				WQGA906,WQ GB276					
02			145	148	75	2	0	14.07	49	109.59	WQJQ689								ļ
	e e	NHH-45B-R2	145	148	165	2	0	14.07	49	109.59	WGJQ689								L
0003	Samsung	MT6413-77A 145	145	146.2	165	2	o	23.35	105	748.75								WRNE581,WF NE582,WRNE 583,WRNE58 4,WRNE585	U
	CommScope	NHH-45B-R2	145	148	345	0	0	17.6	43	407.15			KNLH251,WP OJ730						1_
	CommScope	NHH-45B-R2	145	148	165	2	0	15.08	43	395.2		KNKA404							L
			145	148	345	2	0	15,08	43	395.2		KNKA404					L		Ļ
	CommScope	NHH-45B-R2	145	148	255	0	0	17,9	41	228 42				WQGA906,WQ GB276					_
	CommScope	NHH-45B-R2	145	148	255	0	0	17,6	43	407.15			KNLH251,WP 0J730						_
	CommScape	NHH-45B-R2	145	148	75	2	0	15.08	43	395,2		KNKA404							L
	CommScope	NHH-45B-R2	145	148	345	0	0	17.9	41	228 42				WQGA906,WQ GB276					
	CommScope	NHH-45B-R2	145	148	165	0	0	6,71	41	228.42				WGGA906,WG GB276					
	CommScape	NHH-45B-R2	145	148	345	2	0	14.07	49	109.59	WQJQ689								L
	be		145	148	255	2	0	14.07	49	109 59	WQJQ689								L
0004	Samsung	MT6413-77A	145	146,2	255	22	0	23.35	105	748.75								WRNE581,WF NE582,WRNE 583,WRNE58 4,WRNE585	
	CommScope	NHH-45B-R2	145	148	165	0	0	17.6	43	407 15			KNLH251,WP 0J730						
0002	Samsung	MT6413-77A	145	146.2	75	.52	0	23.35	105	748 75								WRNE581,WF NE582,WRNE 583,WRNE58 4,WRNE585	
0001	Samsung	MT6413-77A	145	146.2	345	2	0	23,35	105	748 75								WRNE581,WF NE582,WRNE 583,WRNE58 4,WRNE585	
	CommScope	NHH-45B-R2	145	148	75	0	0	17,6	43	407,15			KNLH251,WP 0J730						
	CommScope NHH-45B-R2 145	NHH-458-R2	145	148	255	2	0	15.0A	43	305.2		KNIKAAOA							1

			_								
Approve for Insvc	-	**	_		-		-	-	0	0	-
Action	added	added	added	added	added	added	added	added	added	added	added
Status	proposed	proposed	proposed	proposed	proposed	proposed	proposed	proposed	proposed	proposed	proposed
POPs/Sq.	444.75	444 75	444,75	444,75	444.75	444.75	444.75	444.75	444.75	444.75	444.75
Threshold (W)	1000	400	1640	1640	1640	1640	1640	1640	1640	1640	1640
Regulator y Power	109,59	395.2	407.15	407 15	228 42	748.75	748.75	748.75	748,75	748.75	228 42
Freq Range 4	776 000 - 787 000/ 0 00 - 000	869 000 - 880 000/89 0 000 - 891 500	1975 000 - 1980 000/ 000 - 000	1970 000 - 1975 000/ 000 - 000	2110.000 - 2120.000/. 000 - 000	000 - 000	- 000, 000,000 000	000 - 000/ 000 - 000	000 - 000 -000	000 - 000	2120,000 - 2130,000, 000 - 000
Freq Range 3	746 000 - 757 000/ 0 00 - 000	824 000 - 835 000/84 5.000 - 846.500	1895,000 - 1900,000/ 000 - ,000	1890,000 - 1895,000/, 000 - ,000	1710.000 - 1720.000/ 000000	3700.000 - 3720.000/ 000 - 000	3720.000 3740.000/ 000 - 000	3740 000 - 3760 000/ 000 - 000	3760 000 - 3780 000/ 000 - 000	3780 000 - 3800 000/ 000 - 000	1720,000 - 1730,000, 000 - 000
Freq Range 2	776 000 - 787 000/ 0 00 - 000	869,000 - 880,000/89 0,000 - 891,500	1975,000 - 1980,000/ 000 - 000	1970.000 - 1975.000/ 000 - 000	2110.000 - 2120.000/, 000 - 000	000,000	000,000	000,000	000 000	000 - 000	2120,000 - 2130,000/ 000 - ,000
Freq Range 1	746.000 - 757.000/.0	824,000 - 835,000/84 5,000 - 846,500	1895.000 - 1900.000/ 000000	1890,000 - 1895,000/ 000 - ,000	1710.000 - 1720.000/ 000000	3700,000 - 3720,000/ 000 - ,000	3720.000 - 3740.000/. 000000	3740,000 - 3760,000/ 000 - 000	3760.000 - 3780.000/ 000 - 000	3780 000 - 3800 000/ 000 - 000	1720,000 - 1730,000/ 000 - 000
Total MHZ	22,000	25,000	10.000	10.000	20,000	20 000	20,000	20.000	20 000	20.000	20.000
Wholly Owner	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
License V	Cellco	Cellco	Cellco Partnershi	Cellco Partnershi	Cellco Partnershi	Cellco Partnershi	Cellco Partnershi	Cellco Partnershi	Cellco Partnershi	Cellco Partnershi	Cellco Partnershi
County	0007	7008	0000	2006	2005	2006	9007	2006	2006	2006	9007
State	CT	CT	CT	CT	СТ	СТ	сī	ст	CT	ст	СТ
Block						14	A2	A3	A4	A5	
Market # B	REA001 C	CMA032 A	BTA164 C	BTA164 F	CMA032 A	PEA001	PEA001	PEA001	PEA001	PEA001	BEA010
Radio M Code	WU		CW B	CW	AW	Wa	MA	M	P.W	Mq	AW
Market R.	Northeast	Hartford-N CL ew Britain-Br istol, CT	Hartford, CC	Hartford, C	Hartford-N Aew Britain-Br Istol, CT	New York, NY	New York, NY	New York, P	New York, F	New York, F	New York-No. New JerLong Island, NY-NJ-CT-P
Callsign M	WQJQ689	KNKA404 H	WPOJ730 H	KNLH251 H	WQGB276 H	WRNE581	WRNE582	WRNE583	WRNE584	WRNE585	WQGA906



Alpha (Proposed)



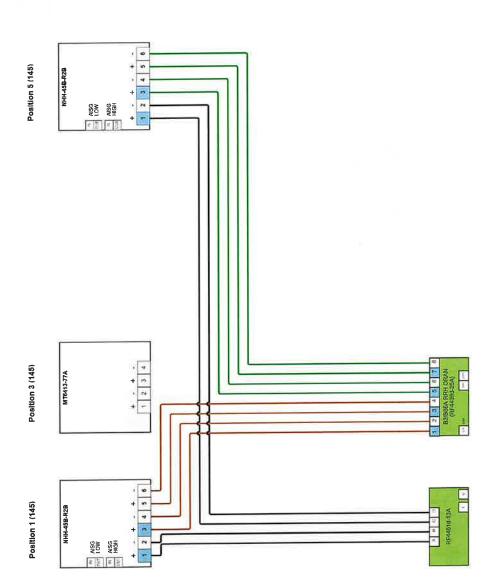
Notes:

-Antenna view is from the back of the antennas -Colors of connections are just for clarification -Size of objects in drawing doesn't reflect equipment true dimensions

Sector design

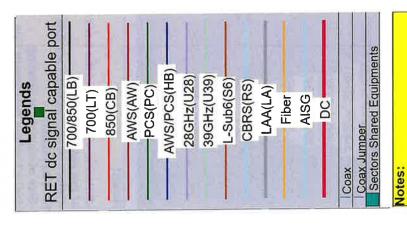
Shelter

Proprietary and Confidential. Not for disclosure outside of Verizon.



Beta

(Proposed)



-Antenna view is from the back of the antennas
-Colors of connections are just for clarification
-Size of objects in drawing doesn't reflect equipment true dimensions

Sector design

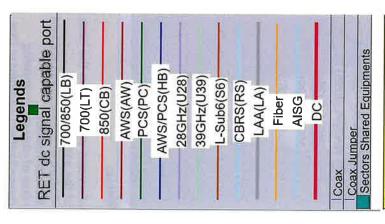
Shelter

Proprietary and Confidential. Not for disclosure outside of Vertzon.

Position 5 (145) NHH-45B-R28 SW SWE + 5 + 6 Position 3 (145) MT5413-77A 1 1 2 8 4 5 6 Position 1 (145) NHH-45B-R28 RF44618-13A

Gamma

(Proposed)



Notes:

Antenna view is from the back of the antennas
-Colors of connections are just for clarification
-Size of objects in drawing doesn't reflect equipment true dimensions

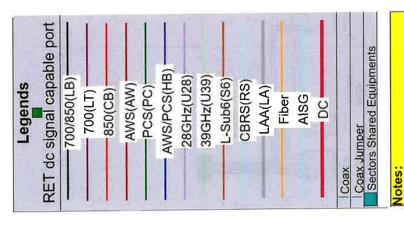
Sector design

Shelter

Proprietary and Confidential. Not for disclosure outside of Verizon.

Position 5 (145) NHH-45B-R2B AISG I OW AISG HIGH Position 3 (145) Ē MT6413-77A 9. Position 1 (145) RE44614-12A AISG

Delta (Proposed)

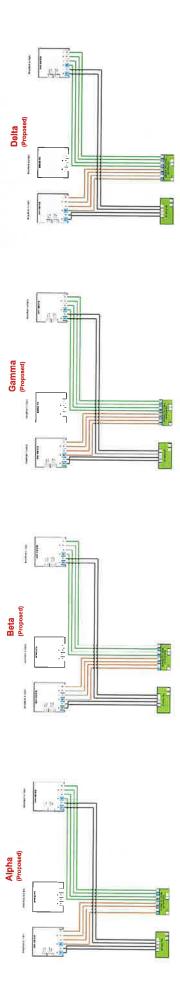


-Antenna view is from the back of the antennas
-Colors of connections are just for clarification
-Size of objects in drawing doesn't reflect equipment true dimensions

Shelter

Sector design

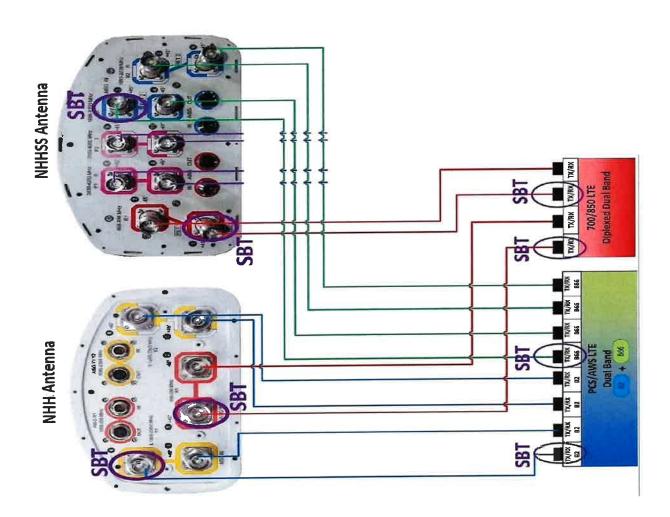
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Centered on Solutions™

Antenna Mount Analysis Report

Site Ref: East Haddam 3

194 MT Parnassus Road East Haddam, CT

Centek Project No. 23134.00

Date: October 9, 2023

Max Stress Ratio = 87%

Prepared for:

Verizon Wireless 20 Alexander Drive Wallingford, CT 06492



CENTEK Engineering, Inc.
Mount Analysis
Verizon Site Ref. ~ East Haddam 3
East Haddam, CT
October 9, 2023

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- ANTENNA AND APPURTENANCE SUMMARY
- STRUCTURE LOADING
- CONCLUSION

SECTION 2 - CALCULATIONS

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- RISA3D OUTPUT REPORT
- CONNECTION

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RF DATA SHEET

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Centered on Solutions

October 9, 2023

Mr. Peter Nute ProTerra Design Group, LLC 4 Bay Road Hadley, MA

Structural Letter ~ Antenna Mount Re. Verizon – Site Ref: East Haddam 3 194 MT Parnassus Road East Haddam, CT

Centek Project No. 23134.00

Dear Mr. Nute,

Centek Engineering, Inc. has reviewed the Verizon antenna installation at the above referenced site. The purpose of the review is to determine the structural adequacy of the proposed mount, consisting of four (4) V-frame sector mounts (SitePro P/N: VFA12-HD) to support the proposed equipment configuration. The review considered the effects of wind load, dead load and ice load in accordance with the 2021 International Building Code as modified by the 2022 Connecticut State Building Code (CTBC) including ASCE 7-16 and ANSI/TIA-222-H Structural Standard for Antenna Supporting Structures, Antennas and Small Wind Turbine Support Structures".

The loads considered in this analysis consist of the following:

Verizon:

V-Frames: Eight (8) Commscope NHH-45B-R2B panel antennas, four (4) Samsung MT6413-77A panel antennas, four (4) Samsung RF4439d-25A (B2/B66A) RRHs, four (4) Samsung RF4461d-13A RRHs and one (1) OVP Box mounted on four (4) V-Frames with a RAD center elevation of 145 ft +/- AGL.

The antenna mount was analyzed per the requirements of the 2021 International Building Code as modified by the 2022 Connecticut State Building Code considering a Ultimate design wind speed of 135 mph for East Haddam as required in Appendix P of the 2022 Connecticut State Building Code.

Based on our review of the installation, it is our opinion that the subject antenna mount has sufficient capacity to support the aforementioned antenna configuration.

If there are any questions regarding this matter, please feel free to call.

Respectfully Submitted by:

PROPERTY OF THE PROPERTY OF TH Structural Engineer

CENTEK Engineering, Inc.
Mount Analysis
Verizon Site Ref. ~ East Haddam 3
East Haddam, CT
October 9, 2023

Section 2 - Calculations



Branford, CT 06405

F: (203) 488-8587

Subject:

TIA-222-H Loads

East Haddam, CT Location:

Prepared by: T.J.L. Checked by: C.F.C.

Job No. 23134.00

Development of Design Heights, Exposure Coefficients, and Velocity Pressures Per TIA-222-H

Wind Speeds

(User Input - CSBC 2022 Appendix P) Basic Wind Speed V := 135 mph (User Input - TIA-222-H Annex B) Basic Wind Speed with Ice $V_i := 50$ mph (User Input - TIA-222-H Section 16.3) Basic Wind Speed (Mount) $V_m := 30$ mph

Input

Rev. 0: 10/9/23

(User Input) Structure Type = Structure_Type := Flexible Structure Category = SC := IV (User Input) (User Input)

Exp := CExposure Category = Structure Height = h:= 160 (User Input)

(User Input) Height to Center of Antennas= z_{ant} := 145

(User Input per Annex B of TIA-222-H) Radial loe Thickness= $t_i := 1.0$

(User Input) Radial ice Density= ld := 56.00(User Input) Topograpic Factor = $K_{2t} := 1$ (User Input)

 $K_a := 1.0$ Shielding Factor for Appurtenances = (User Input) Rooftop Wind Speed-up Factor = $K_{s} := 1.0$

Ground Elevation Factor = $K_{p} = 0.996$ (User Input) (User Input)

Gust Response Factor = $G_H = 1.35$

(Per Table 2-2 of $K_d := 0.95$ Wind Direction Probability Factor =

TIA-222-H) $I_{ice} := \begin{bmatrix} 0 & \text{if } SC = 1 \\ 1.00 & \text{if } SC = 2 \\ 1.15 & \text{if } SC = 3 \end{bmatrix} = 1.25$ (Per Table 2-3 of Importance Factors = TIA-222-H)

$$K_{iz} := \left(\frac{z_{ant}}{33}\right)^{0.1} = 1.16$$
 $t_{iz} := t_{i'} I_{ice'} K_{iz'} K_{zt}^{0.35} = 1.449$

 $t_{iz} := t_{i'} l_{ice'} K_{iz'} K_{zt}^{0.35} = 1.449$ $Kz_{ant} := 2.01 \left(\left(\frac{z_{ant}}{z_g} \right) \right)^{\frac{2}{\alpha}} = 1.369$ Velocity Pressure CoefficientAnternas=

 $qz_{ant} := 0.00256 \cdot K_{zt} \cdot K_{s} \cdot K_{e} \cdot K_{d} \cdot Kz_{ant} \cdot V^{2} = 60.393$ Velocity Pressure w/o Ice Antennas =

 $qz_{ice,ant} := 0.00256 \cdot K_{zt} \cdot K_s \cdot K_e \cdot K_{d} \cdot Kz_{ant} \cdot V_i^2 = 8.284$ Velocity Pressure with Ice Antennas =

 $\mathsf{qz}_{m} \coloneqq 0.00256 \cdot \mathsf{K}_{zt} \cdot \mathsf{K}_{s} \cdot \mathsf{K}_{e} \cdot \mathsf{K}_{d} \cdot \mathsf{Kz}_{ant} \cdot \mathsf{V}_{m}^{2} = 2.982$ Velocity Pressure with Ice Antennas =

Subject:

TIA-222-H Loads

Centered on Solutions movembers com
P: (203) 488-0580 Branford, CT 06405

Location:

Rev. 0: 10/9/23

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

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lbs

lbs

Job No. 23134.00

Development of Wind & Ice Load on Appurtenances

Appurtenance Data:

Appurtenance Model =

Appurtenance Shape =

Appurtenance Height=

Appurtenance Width =

Appurtenance Thickness =

Appurtenance Weight =

Number of Appurtenances=

Appurtenance Aspect Ratio =

Appurtenance Force Coefficient =

Wind Load (without ice)

Surface Area for One Appurtenance (Front) =

Total Appurte rance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurterance Wind Force =

Wind Load (with ice)

SurfaceArea for One Appurtenance w/ Ice (Front)=

Total Appurtenance Wind Force w/ be=

Surface Area for One Appurtenance w/ Ice (Side) =

Total Appurtenance Wind Force w/ be=

Wind Load (Mount)

Surface Area for One Appurlenance (Front) =

Total Appurterance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurte rence Wind Force =

Gravity Loads (ice only)

Volume of Each Appurtenance =

Volume of Ice on Each Appurtenance =

Weight of Ice on Each Appurtenance =

Weight of Ice on All Appurte rances =

Commscope NHH-45B-R2B

Flat

(User Input)

L_{app} := 72

(User Input)

 $W_{app} = 18$

(User Input)

 $T_{app} := 7$

(User Input)

 $WT_{app} := 80$

(User Input)

 $N_{app} := 1$

(User Input)

$$Ar_{app} := \frac{L_{app}}{W_{app}} = 4.0$$

$$Ca_{app} = 1.27$$

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 9$$

$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 3.5$$

$$SA_{ICEappF} := \frac{\left(L_{app} + 2 \cdot t_{iz}\right) \cdot \left(W_{app} + 2 \cdot t_{iz}\right)}{144} = 10.9$$
 sf

$$SA_{ICEappS} := \frac{\left(L_{app} + 2 \cdot l_{iz}\right) \cdot \left(T_{app} + 2 \cdot l_{iz}\right)}{144} = 5.1$$

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 9$$

$$F_{app} := qz_m \cdot G_H \cdot Ca_{app} \cdot K_a \cdot SA_{appF} = 46$$

$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 3.5$$

$$F_{app} := qz_{m} \cdot G_{H} \cdot Ca_{app} \cdot K_{a} \cdot SA_{appS} = 18$$

$$V_{app} := L_{app} \cdot W_{app} \cdot T_{app} = 9072$$
 cu in

$$V_{ice} := \left(L_{app} + 2 \cdot t_{iz} \right) \left(W_{app} + 2 \cdot t_{iz} \right) \cdot \left(T_{app} + 2 \cdot t_{iz} \right) - V_{app} = 6423 \qquad \text{cu in}$$

$$W_{ICEapp} := \frac{V_{ice}}{1728} \cdot Id = 208$$
 lbs

$$W_{|CEapp} \cdot N_{app} = 208$$
 lbs

Centered on Solutions - markentekens.com 63-2 North Branford Road Branford, CT 06405

Subject:

TIA-222-H Loads

Location:

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

Rev. 0: 10/9/23

Job No. 23134.00

Development of Wind & Ice Load on Appurtenances

Appurtenance Data:

Appurtenance Model =

Appurtenance Shape =

Appurtenance Height=

Appurtenance Width =

Appurtenance Thickness =

Appurtenance Weight =

Number of Appurtenances=

Appurtenance Aspect Ratio =

Appurtenance Force Coefficient =

Wind Load (without ice)

Surface Area for One Appurtenance (Front) =

Total Appurtenance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurtenance Wind Force =

Wind Load (with ice)

SurfaceArea for One Appurtenance w/ Ice (Front)=

Total Appurtenance Wind Force w/ be=

Surface Area for One Appurtenance w/loe (Side) =

Total Appurtenance Wind Force w/ be=

Wind Load (Mount)

Surface Area for One Appurtenance (Front) =

Total Appurte rance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurtenance Wind Force =

Gravity Loads (ice only)

Volume of Each Appurtenance =

Volume of Ice on Each Appurtenance =

Weight of Ice on Each Appurtenance =

Weight of Ice on All Appurte rances =

Samsung MT6413-77A

(User Input)

 $L_{app} := 28.9$

(User Input)

W_{app} := 15.75

(User Input)

 $T_{app} := 5.51$

(User Input)

 $WT_{app} = 60$

(User Input)

N_{app} := 1

(User Input)

sf

lbs

lbs

sf

lbs

lhs

$$Ar_{app} := \frac{L_{app}}{W_{app}} = 1.8$$

$$Ca_{app} = 1.2$$

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 3.2$$

in

$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 1.1$$

$$SA_{ICEappF} := \frac{\left(L_{app} + 2 \cdot t_{iz}\right) \cdot \left(W_{app} + 2 \cdot t_{iz}\right)}{144} = 4.1$$
 sf

$$SA_{ICEappS} := \frac{\left(L_{app} + 2 \cdot t_{iz}\right) \cdot \left(T_{app} + 2 \cdot t_{iz}\right)}{144} = 1.9$$

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 3.2$$

$$F_{app} := qz_{m} \cdot G_{H} \cdot Ca_{app} \cdot K_{a} \cdot SA_{appF} = 15$$
 lbs

$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 1.1$$
 sf

$$F_{app} := qz_m \cdot G_H \cdot Ca_{app} \cdot K_a \cdot SA_{appS} = 5$$

$$V_{app} := L_{app} \cdot W_{app} \cdot T_{app} = 2508$$
 cuin

$$V_{ice} := (L_{app} + 2 \cdot t_{iz})(W_{app} + 2 \cdot t_{iz}) \cdot (T_{app} + 2 \cdot t_{iz}) - V_{app} = 2479$$
 cu in

$$W_{ICEapp} := \frac{V_{ice}}{1728} \cdot Id = 80$$
 lbs

Subject:

TIA-222-H Loads

Centered on Solutions P. (203) 488-0580 Branford, CT 06405

Location:

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

lbs

lbs

sf

lbs

lbs

lbs

cu in

Rev. 0: 10/9/23 Job No. 23134.00

Development of Wind & Ice Load on Appurtenances

Appurtenance Data:

Appurtenance Model =

Appurtenance Shape =

Appurtenance Height=

Appurtenance Width =

Appurtenance Thickness =

Appurtenance Weight =

Number of Appurtenances=

Appurtenance Aspect Ratio =

Appurtenance Force Coefficient =

Wind Load (without ice)

Surface Area for One Appurtenance (Front) =

Total Appurtenance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurte nance Wind Force =

Wind Load (with ice)

Surface Area for One Appurtenance w/ Ice (Front)=

Total Appurtenance Wind Force w/ be=

SurfaceArea for One Appurtenance w/ Ice (Side) =

Total Appurtenance Wind Force w/ be=

Wind Load (Mount)

Surface Area for One Appurtenance (Front) =

Total Appurte nance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurtenance Wind Force =

Gravity Loads (ice only)

Volume of Each Appurtenance =

Volume of Ice on Each Appurtenance =

Weight of Ice on Each Appurtenance =

Weight of Ice on All Appurterances =

Samsung RF4439-25A(B2/B66A)RRH

Flat

(User Input)

 $L_{add} = 15$

(User Input)

W_{app} := 15

(User Input)

 $T_{app} := 10$

(User Input)

 $WT_{app} := 75$

(User Input)

(User Input)

$$Ar_{app} := \frac{L_{app}}{W_{app}} = 1.0$$

$$Ca_{app} = 1.2$$

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 1.6$$

$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 1$$

$$SA_{ICEappF} := \frac{(L_{app} + 2 \cdot t_{iz}) \cdot (W_{app} + 2 \cdot t_{iz})}{144} = 2.2$$
 sf

$$Fi_{app} := qz_{ice.ant} \cdot G_H \cdot Ca_{app} \cdot K_a \cdot SA_{ICEappF} = 30$$

$$SA_{ICEappS} = \frac{\left(L_{app} + 2 \cdot t_{iz}\right) \cdot \left(T_{app} + 2 \cdot t_{iz}\right)}{144} = 1.6$$

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 1.6$$

$$F_{app} := qz_m \cdot G_H \cdot Ca_{app} \cdot K_a \cdot SA_{appF} = 8$$

$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 1$$
 sf

$$F_{app} := qz_{m} \cdot G_{H} \cdot Ca_{app} \cdot K_{a} \cdot SA_{appS} = 5$$

$$V_{app} := L_{app} \cdot W_{app} \cdot T_{app} = 2250$$
 cu in
$$V_{ice} := (L_{app} + 2 \cdot t_{iz})(W_{app} + 2 \cdot t_{iz}) \cdot (T_{app} + 2 \cdot t_{iz}) - V_{app} = 1882$$

$$W_{ICEapp} := \frac{V_{ice}}{1728} \cdot Id = 61$$
 lbs

Centered on Solutions 63-2 North Branford Road Branfold, CT 06405

Subject:

TIA-222-H Loads

Location:

Rev. 0: 10/9/23

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

Job No. 23134.00

(User Input)

Development of Wind & Ice Load on Appurtenances

Appurtenance Data:

Samsung RF4461d-13ARRH Appurtenance Model =

Appurtenance Shape = Flat

(User Input) Appurtenance Height= L_{app} := 15

W_{app} := 15 (User Input) Appurtenance Width =

 $T_{app} := 10.23$ (User Input) Appurtenance Thickness =

Appurtenance Weight = $WT_{app} := 80$ lbs (User Input)

 $N_{app} := 1$ (User Input) Number of Appurtenances=

Appurtenance Aspect Ratio =

Appurtenance Force Coefficient =

Wind Load (without ice)

Surface Area for One Appurtenance (Front) =

Total Appurte nance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurte rance Wind Force =

Wind Load (with ice)

Surface Area for One Appurtenance w/ Ice (Front)=

Total Appurtenance Wind Force w/ Ice=

Surface Area for One Appurtenance w/ Ice (Side) =

Total Appurtenance Wind Force w/ Ice=

Wind Load (Mount)

Surface Area for One Appurtenance (Front) =

Total Appurte nance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurterance Wind Force =

Gravity Loads (ice only)

Volume of Each Appurtenance =

Volume of Ice on Each Appurtenance =

Weight of Ice on Each Appurtenance =

Weight of Ice on All Appurterances =

$$Ar_{app} := \frac{L_{app}}{W_{app}} = 1.0$$

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 1.6$$
 sf

$$F_{app} := qz_{ant} \cdot G_{H} \cdot Ca_{app} \cdot K_{a} \cdot SA_{appF} = 153$$
 lbs

$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 1.1$$
 sf

$$F_{app} := qz_{ant} \cdot G_H \cdot Ca_{app} \cdot K_a \cdot SA_{appS} = 104$$
 lbs

$$SA_{ICEappF} := \frac{\left(L_{app} + 2 \cdot l_{iz}\right) \cdot \left(W_{app} + 2 \cdot l_{iz}\right)}{144} = 2.2$$
 sf

$$Fi_{app} := qz_{ice.ant} \cdot G_H \cdot Ca_{app} \cdot K_a \cdot SA_{|CEappF} = 30$$
 lbs

sf

lbs

$$SA_{ICEappS} := \frac{\left(L_{app} + 2 \cdot t_{iz}\right) \cdot \left(T_{app} + 2 \cdot t_{iz}\right)}{144} = 1.6$$

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 1.6$$
 sf

$$F_{app} := qz_m \cdot G_{H'} \cdot Ca_{app} \cdot K_a \cdot SA_{appF} = 8$$
 lbs

$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 1.1$$
 sf

$$F_{app} := qz_m \cdot G_H \cdot Ca_{app} \cdot K_a \cdot SA_{appS} = 5$$

$$\begin{split} & V_{app} \coloneqq L_{app} \cdot W_{app} \cdot T_{app} = 2302 & \text{cu in} \\ & V_{ice} \coloneqq \left(L_{app} + 2 \cdot t_{iz}\right) \left(W_{app} + 2 \cdot t_{iz}\right) \cdot \left(T_{app} + 2 \cdot t_{iz}\right) - V_{app} = 1904 & \text{cu in} \\ & W_{ICEapp} \coloneqq \frac{V_{ice}}{1728} \cdot \text{Id} = 62 & \text{lbs} \\ & W_{ICEapp} \cdot N_{app} = 62 & \text{lbs} \end{split}$$



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Subject:

TIA-222-H Loads

Location:

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

Job No. 23134.00

Development of Wind & Ice Load on Appurtenances

Appurtenance Data:

Appurtenance Model = OVP Box

Rev. 0: 10/9/23

Appurtenance Shape = Flat (User Input)

Appurtenance Height= Lapp := 29.5 (User Input)

Appurtenance Width = W_{app} := 16.5 (User Input)

Appurtenance Thickness = T_{app} := 12.6 (User Input)

Appurtenance Weight = $WT_{app} := 32$ (User Input)

Number of Appurtenances= $N_{app} := 1$ (User Input)

 $Ar_{app} := \frac{L_{app}}{W_{app}} = 1.8$ Appurtenance Aspect Ratio =

Appurtenance Force Coefficient = $Ca_{app} = 1.2$

Wind Load (without ice)

Surface Area for One Appurtenance (Front) =

Total Appurtenance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurte rance Wind Force =

Wind Load (with ice)

Surface Area for One Appurtenance w/ Ice (Front)=

Total Appurtenance Wind Force w/ Ice=

Surface Area for One Appurtenance w/lce (Side) =

Total Appurte rance Wind Force w/ be=

Wind Load (Mount)

Surface Area for One Appurtenance (Front) =

Total Appurte nance Wind Force =

Surface Area for One Appurtenance (Side) =

Total Appurtenance Wind Force =

Gravity Loads (ice only)

Volume of Each Appurtenance =

Volume of Ice on Each Appurtenance =

Weight of Ice on Each Appurtenance =

Weight of Ice on All Appurtenances =

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 3.4$$
 sf

$$F_{app} := qz_{ant}G_{H}Ca_{app}K_{a}SA_{app} = 331$$
 lbs

$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 2.6$$
 sf

$$SA_{ICEappF} := \frac{\left(L_{app} + 2 \cdot t_{iz}\right) \cdot \left(W_{app} + 2 \cdot t_{iz}\right)}{144} = 4.4$$
 sf

$$Fi_{app} := qz_{ice.ant} \cdot G_H \cdot Ca_{app} \cdot K_a \cdot SA_{ICEappF} = 59$$
 lbs

$$SA_{ICEappS} := \frac{\left(L_{app} + 2 \cdot t_{iz}\right) \cdot \left(T_{app} + 2 \cdot t_{iz}\right)}{144} = 3.5$$
 sf

lbs

$$SA_{appF} := \frac{L_{app} \cdot W_{app}}{144} = 3.4$$
 sf

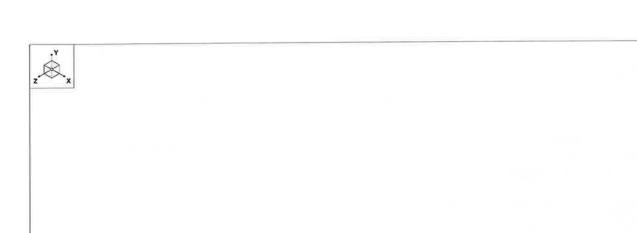
$$F_{app} := qz_m \cdot G_H \cdot Ca_{app} \cdot K_a \cdot SA_{appF} = 16$$

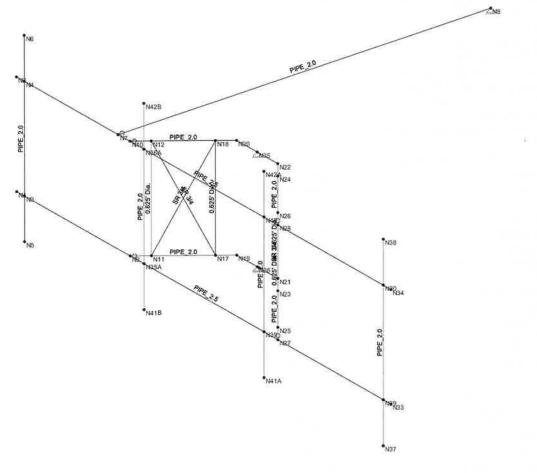
$$SA_{appS} := \frac{L_{app} \cdot T_{app}}{144} = 2.6$$
Ibs

$$F_{app} := qz_{m} \cdot G_{H} \cdot Ca_{app} \cdot K_{a} \cdot SA_{appS} = 12$$
 lbs

$$\begin{split} &V_{app} \coloneqq L_{app} \cdot W_{app} \cdot T_{app} = 6133 & \text{cu in} \\ &V_{ice} \coloneqq \left(L_{app} + 2 \cdot t_{iz}\right) \! \left(W_{app} + 2 \cdot t_{iz}\right) \! \cdot \! \left(T_{app} + 2 \cdot t_{iz}\right) - V_{app} = 3608 & \text{cu in} \end{split}$$

$$W_{\text{ICEapp}} := \frac{V_{\text{ice}}}{1728} \cdot |d = 117$$
 lbs





Envelope Only Solution

Centek Engineering		
TJL	East Haddam 3	Oct 9, 2023 at 10:47 AM
23134.00	Member Framing	Mount.R3D



Company : Centek Engineering Designer : TJL Job Number : 23134.00 Model Name : East Haddam 3

Oct 9, 2023 10:46 AM Checked By:__

(Global) Model Settings

(Global) Model Settings	
Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (ft/sec^2)	32.2
Wall Mesh Size (in)	12
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver
Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AIGI S100 10: ASD

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16); LRFD
Cold Formed Steel Code	AISI S100-10: ASD
Wood Code	AWC NDS-12: ASD
Wood Temperature	< 100F
Concrete Code	ACI 318-11
Masonry Code	ACI 530-11: ASD
Aluminum Code	AA ADM1-10: ASD - Building
Stainless Steel Code	AISC 14th(360-10): ASD
Adjust Stiffness?	Yes(Iterative)

Number of Shear Regions	4	
Region Spacing Increment (in)	4	
Biaxial Column Method	Exact Integration	
Parme Beta Factor (PCA)	.65	
Concrete Stress Block	Rectangular	
Use Cracked Sections?	Yes	
Use Cracked Sections Slab?	No	
Bad Framing Warnings?	No	
Unused Force Warnings?	Yes	
Min 1 Bar Diam. Spacing?	No	
Concrete Rebar Set	REBAR SET ASTMA615	
Min % Steel for Column	1	
Max % Steel for Column	8	



Company Designer : Centek Engineering : TJL

Job Number Model Name

: 23134.00 : East Haddam 3 Oct 9, 2023 10:46 AM Checked By:_____

(Global) Model Settings, Continued

Seismic Code	ASCE 7-10
Seismic Base Elevation (ft)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
RX	3
RZ	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	
Om X	1
Cd Z	1
Cd X	1
Rho Z	
Rho X	1
Footing Overturning Safety Factor	1
Optimize for OTM/Sliding	No
Check Concrete Bearing	No
Footing Concrete Weight (k/ft^3)	150.001
Footing Concrete f'c (ksi)	4
Footing Concrete Ec (ksi)	3644
Lambda	1
Footing Steel fy (ksi)	60
Minimum Steel	0.0018
Maximum Steel	0.0075
Footing Top Bar	#3
Footing Top Bar Cover (in)	2
Footing Bottom Bar	#3
Footing Bottom Bar Cover (in)	3.5
Pedestal Bar	#3
Pedestal Bar Cover (in)	1.5
Pedestal Ties	#3

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (\ D	Density[k/ft^3]	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
2	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	58	1.2
3	A992	29000	11154	.3	.65	.49	50	1.1	58	1.2
4	A500 Gr.42	29000	11154	.3	.65	.49	42	1.3	58	1.1
5	A500 Gr.46	29000	11154	.3	.65	.49	46	1.2	58	1.1
6	A53 Grade B	29000	11154	.3	.65	.49	35	1.5	58	1.2



Company Designer Job Number : Centek Engineering : TJL

23134.00 Model Name East Haddam 3 Oct 9, 2023 10:46 AM Checked By:

Hot Rolled Steel Section Sets

_	Label	Shape	Type	Design List	Material	Design	A [in2]	lyy [in4]	Izz [in4]	J [in4]
1	Antenna Mast_2.0	PIPE 2.0	Column	Pipe	A53 Grade B			.627	.627	1.25
2	Horizontal_2.5 ST	PIPE 2.5	Beam	Pipe	A53 Grade B	Typical	1.61	1.45	1.45	2.89
3	Outrigger_2.0 ST	PIPE 2.0	Beam	Pipe	A53 Grade B	Typical	1.02	.627	.627	1.25
4	Stabilizer_2.0 ST	PIPE 2.0	Beam	Pipe	A53 Grade B	Typical	1.02	.627	.627	1.25
5	0.625" Dia. Bar	0.625' Dia.	Column	BAR	A36 Gr.36	Typical	.307	.007	.007	.015
6	0.75"Dia. Bar	SR 3/4	Column	BAR	A36 Gr.36	Typical	.442	.016	.016	.031

Hot Rolled Steel Design Parameters

	Label	Shape	Length[ft]	Lbyy[ft]	Lbzz[ft]	Lcomp top[.Lcomp bot[.	.L-torq	Kvv	Kzz	Cb	Functi
1	M1	Horizontal_2.5 STD	12.5	Segment		Lbyy						Lateral
2	M2	Horizontal_2.5 STD	12.5	Segment		Lbyy						Lateral
3	М3	Stabilizer_2.0 STD	10.18			Lbyy						Lateral
4	M4	Outrigger_2.0 STD	2.521	Segment	Segment	Lbyy						Lateral
5	M5	Outrigger_2.0 STD	2.521	Segment	Segment							Lateral
6	M6	Outrigger_2.0 STD	2.521	Segment	Segment							Lateral
7	M7	Outrigger_2.0 STD	2.521	Segment								Lateral
8	M8	0.625" Dia. Bar	3.333									Lateral
9	M9	0.625" Dia. Bar	3.333									Lateral
10	M10	0.75"Dia. Bar	3.659	1.83	1.83	Lbyy						Lateral
11	M11	0.625" Dia. Bar	3.333									Lateral
12	M12	0.75"Dia. Bar	3.659	1.83	1.83	Lbyy						Lateral
13	M13	0.625" Dia. Bar	3.333									Lateral
14	M14	0.75"Dia. Bar	3.659	1.83	1.83	Lbyy			June 1	100		Lateral
15	M15	0.75"Dia. Bar	3.659	1.83	1.83	Lbyy						Lateral
16	PS.2	Antenna Mast 2.0	6			Lbyy						Lateral
17	PS.1	Antenna Mast_2.0	6			Lbvv						Lateral
18	M19	Antenna Mast_2.0	6			Lbyy			P U			Lateral
19	M21A	Antenna Mast_2.0	6			Lbyy						Lateral

Member Primary Data

	Label	1 Joint	J Joint	K Joint	Rotate(. Section/Shape	Type	Design List	Material	Design
1	M1	N2	N34			Horizontal_2.5 STD Pipe	Beam	Pipe	A53 Grade B	Typical
2	M2	N1	N33			Horizontal_2.5 STD Pipe	Beam	Pipe	A53 Grade B	Typical
3	M3	N7	N8			Stabilizer_2.0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
4	M4	N10	N20			Outrigger_2.0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
5	M5	N9	N19			Outrigger_2,0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
6	M6	N28	N22			Outrigger_2.0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
7	M7	N27	N21			Outrigger_2.0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
8	M8	N12	N11			0.625" Dia. Bar	Column	BAR	A36 Gr.36	Typical
9	M9	N18	N17			0.625" Dia. Bar	Column	BAR	A36 Gr.36	Typical
10	M10	N12	N17			0.75"Dia. Bar	Column	BAR	A36 Gr.36	Typical
11	M11	N26	N25			0.625" Dia. Bar	Column	BAR	A36 Gr.36	Typical
12	M12	N18	N11			0.75"Dia. Bar	Column	BAR	A36 Gr.36	Typical
13	M13	N24	N23			0.625" Dia. Bar	Column	BAR	A36 Gr.36	Typical
14	M14	N26	N23			0.75"Dia. Bar	Column	BAR	A36 Gr.36	Typical
15	M15	N24	N25			0.75"Dia. Bar	Column	BAR	A36 Gr.36	Typical
16	PS.2	N5	N6			Antenna Mast_2.0 STD Pi	Column	Pipe	A53 Grade B	Typical
17	PS.1	N37	N38			Antenna Mast_2.0 STD Pi	Column	Pipe	A53 Grade B	Typical



Company Designer Job Number

Centek Engineering

23134.00 East Haddam 3 Model Name

Oct 9, 2023 10:46 AM Checked By:_

Member Primary Data (Continued)

	Label	1 Joint	J Joint	K Joint Rotate(Section/Shape	Type	Design List	Material	Design
18	M19	N41A			Antenna Mast_2.0 STD Pi	Column	Pipe	A53 Grade B	Typical
19	M20	N19	N21		RIGID	None	None	RIGID	Typical
20	M21	N20	N22		RIGID	None	None	RIGID	Typical
21	M21A	N41B			Antenna Mast_2.0 STD Pi	Column	Pipe	A53 Grade B	Typical

Joint Coordinates and Temperatures

JOIN C	oorumates and		27.761	7 (6)	Terre (C)	Detach From Dia
	Label	X [ft]	Y [ft]	Z [ft]	Temp [F]	Detach From Dia
1	N1	0	0.	-0.	0	
2	N2	0	3.333334	-0.	0	
3	N3	.25	0.	-0.	0	
4	N4	.25	3.333334	-0.	0	
5	N5	.25	-1.333333	-0.	0	
6	N6	.25	4.666667	-0.	0	
7	N7	3.390625	3.333334	-0.	0	
8	N8	6.025403	3.333334	-9.833125	0	
9	N9	3.78125	0.	-0.	0	
10	N10	3.78125	3.333334	-0.	0	
11	N11	4.138628	0.	-0.357378	0	
12	N12	4.138628	3.333334	-0.357378	0	
13	N17	5.206335	0.	-1.425085	0	
14	N18	5.206335	3.333334	-1.425085	0	
15	N19	5.563713	0.	-1.782463	0	
16	N20	5.563713	3.333334	-1.782463	0	
17	N21	6.936287	0.	-1.782463	0	
18	N22	6.936287	3.333334	-1.782463	0	
19	N23	7.293665	0.	-1.425085	0	
20	N24	7.293665	3.333334	-1.425085	0	
21	N25	8.361372	0.	-0.357378	0	
22	N26	8.361372	3.333334	-0.357378	0	
23	N27	8.71875	0.	-0.	0	
24	N28	8.71875	3.333334	-0.	0	
25	N29	12.25	0.	-0.	0	
26	N30	12.25	3.333334	-0.	0	
27	N33	12.5	0.	-0.	0	
28	N34	12.5	3.333334	-0.	0	
29	N35	6.25	3.333334	-1.782463	0	
30	N36	6.25	0.	-1.782463	0	
31	N35A	4.25	0.	-0.	0	
32	N36A	4.25	3.333334	-0.	0	
33	N37	12.25	-1.333333	0	0	
34	N38	12.25	4.666667	0	0	
35	N39	8.25	0.	-0.	0	
36	N40	8.25	3.333334	-0.	0	Post record
	N41A	8.25	-1.333333	-0.	0	
37 38	N41A N42A	8.25	4.666667	- 0.	Ö	
		4.25	-1.333333	-0.	0	
39	N41B	4.25	4.666667	-0. -0.	0	
40	N42B	4.20	4.000007	-0.	-	



Company Designer Job Number Model Name

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Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	N8	Reaction	Reaction	Reaction			
2	N19						
3	N20						
4	N17						
5	N18						
6	N21				AND DESCRIPTION	AT MANUFACTURE	Charles To be but
7	N22						
8	N23					- 50	
9	N24						
10	N35	Reaction	Reaction	Reaction			
11	N36	Reaction	Reaction	Reaction			

Member Point Loads (BLC 2 : Dead Load)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	Y	04	.5
2	PS.1	Y	04	.5
3	PS.2	Y	04	5.5
4	PS.1	Y	04	5.5
5	M21A	Y	03	.5
6	M21A	Y	03	5.5
7	PS.2	Y	075	%50
8	PS.1	Y	08	%50
9	M19	Y	032	%50

Member Point Loads (BLC 3 : Ice Load)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]			
1	PS.2	Y	104	.5			
2	PS.1	Υ	104	.5			
3	PS.2	Υ	104	5.5			
4	PS.1	Y	104	5.5			
5	M21A	Y	04	.5			
6	M21A	Y	04	5.5			
7	PS.2	Υ	061	%50			
8	PS.1	Υ	062	%50			
9	M19	Y	117	%50			

Member Point Loads (BLC 4 : Lm Maintenance Load (500lb))

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.1	Υ	5	%50

Member Point Loads (BLC 5 : Lv Maintenance Load (250lb))

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	M1	Υ	- 25	12

Member Point Loads (BLC 6 : Wind with Ice X)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	X	.037	.5



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Member Point Loads (BLC 6: Wind with Ice X) (Continued)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
2	PS.1	X	.037	.5
3	PS.2	X	.037	5.5
4	PS.1	X	.037	5.5
5	M21A	X	.013	.5
6	M21A	X	.013	5.5
7	PS.2	X	.022	%50
8	PS.1	X	.022	%50
9	M19	X	.047	%50

Member Point Loads (BLC 7: Wind X)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	X	.181	.5
2	PS.1	X	.181	.5
3	PS.2	X	.181	5.5
4	PS.1	X	.181	5.5
5	M21A	X	.054	.5
6	M21A	X	.054	5.5
7	PS.2	X	.102	%50
8	PS.1	X	.104	%50
9	M19	X	.253	%50

Member Point Loads (BLC 8 : Wm Wind X)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	X	.009	.5
2	PS.1	X	.009	.5
3	PS.2	X	.009	5.5
4	PS.1	X	.009	5.5
5	M21A	X	.003	.5
6	M21A	X	.003	5.5
7	PS.2	X	.005	%50
8	PS.1	X	.005	%50
9	M19	X	.012	%50

Member Point Loads (BLC 9 : Wind with Ice Z)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	Z	.077	.5
2	PS.1	Z	.077	.5
3	PS.2	Z	.077	5.5
4	PS.1	Z	.077	5.5
5	M21A	Z	.028	.5
6	M21A	Z	.028	5.5
7	M19	Z	.059	%50

Member Point Loads (BLC 10 : Wind Z)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	P\$.2	Z	.465	.5
2	PS.1	Z	.465	.5
3	PS.2	Z	.465	5.5
4	PS.1	Z	.465	5.5
5	M21A	Z	.155	.5



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Member Point Loads (BLC 10: Wind Z) (Continued)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
6	M21A	Z	.155	5.5
7	M19	7	.331	%50

Member Point Loads (BLC 11 : Wm Wind Z)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	Z	.023	.5
2	PS.1	Z	.023	.5
3	PS.2	Z	.023	5.5
4	PS.1	Z	.023	5.5
5	M21A	Z	.008	.5
6	M21A	Z	.008	5.5
7	M19	Z	.016	%50

Member Distributed Loads (BLC 6 : Wind with Ice X)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/	.Start Location[ft,%]	End Location[ft,%]
1	M3	X	.003	.003	0	0
2	M4	X	.003	.003	0	0
3	M5	X	.003	.003	0	0
4	M6	X	.003	.003	0	0
5	M7	X	.003	.003	0	0
6	M8	X	.003	.003	0	0
7	M9	X	.003	.003	0	0
8	M10	X	.003	.003	0	0
9	M11	X	.003	.003	0	0
10	M12	X	.003	.003	0	0
11	M13	X	.003	.003	0	0
12	M14	X	.003	.003	0	0
13	M15	X	.003	.003	0	0
14	PS.2	X	.003	.003	0	0
15	PS.1	X	.003	.003	0	0
16	M19	X	.003	.003	0	0
17	M20	X	.003	.003	0	0
18	M21	X	.003	.003	0	0
19	M21A	X	.003	.003	0	0

Member Distributed Loads (BLC 7 : Wind X)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/.	Start Location[ft,%]	End Location[ft,%]
1	M3	X	.018	.018	0	0
2	M4	X	.018	.018	0	0
3	M5	X	.018	.018	0	0
4	M6	X	.018	.018	0	0
5	M7	X	.018	.018	0	0
6	M8	X	.018	.018	0	0
7	M9	X	.018	.018	0	0
8	M10	X	.018	.018	0	0
9	M11	X	.018	.018	0	0
10	M12	X	.018	.018	0	0
11	M13	X	.018	.018	0	0
12	M14	X	.018	.018	0	0



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Member Distributed Loads (BLC 7: Wind X) (Continued)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/	Start Location[ft,%] End Location[ft,%]
13	M15	X	.018	.018	0	0
14	PS.2	X	.018	.018	0	0
15	PS.1	X	.018	.018	0	0
16	M19	X	.018	.018	0	0
17	M20	X	.018	.018	0	0
18	M21	X	.018	.018	0	0
19	M21A	X	.018	.018	0	0

Member Distributed Loads (BLC 8 : Wm Wind X)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/.	Start Location[ft,%]	End Location[ft,%]
1	M3	X	.003	.003	0	0
2	M4	X	.003	.003	0	0
3	M5	X	.003	.003	0	0
4	M6	X	.003	.003	0	0
5	M7	X	.003	.003	0	0
6	M8	X	.003	.003	0	0
7	M9	X	.003	.003	0	0
8	M10	X	.003	.003	0	0
9	M11	X	.003	.003	0	0
10	M12	X	.003	.003	0	0
11	M13	X	.003	.003	0	0
12	M14	X	.003	.003	0	0
13	M15	X	.003	.003	0	0
14	PS.2	X	.003	.003	0	0
15	PS.1	X	.003	.003	0	0
16	M19	X	.003	.003	0	0
17	M20	X	.003	.003	0	0
18	M21	X	.003	.003	0	0
19	M21A	X	.003	.003	0	0

Member Distributed Loads (BLC 9: Wind with Ice Z)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/	.Start Location[ft,%]	End Location[ft,%]
1	M1	Z	.003	.003	0	0
2	M2	Z	.003	.003	0	0
3	M3	Z	.003	.003	0	0
4	M4	Z	.003	.003	0	0
5	M5	Z	.003	.003	0	0
6	M6	Z	.003	.003	0	0
7	M7	Z	.003	.003	0	0
8	M8	Z	.003	.003	0	0
9	M9	Z	.003	.003	0	0
10	M10	Z	.003	.003	0	0
11	M11	Z	.003	.003	0	0
12	M12	Z	.003	.003	0	0
13	M13	Z	.003	.003	0	0
14	M14	Z	.003	.003	0	0
15	M15	Z	.003	.003	0	00
16	PS.2	Z	.003	.003	0	0
17	M19	Z	.003	.003	0	0
18	M20	Z	.003	.003	0	0
19	M21	Z	.003	.003	0	0



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Member Distributed Loads (BLC 9 : Wind with Ice Z) (Continued)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/	Start Location[ft,%	End Location[ft,%]
20	M21A	Z	.003	.003	0	0

Member Distributed Loads (BLC 10: Wind Z)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/	.Start Location[ft,%	End Location[ft,%]
1	M1	Z	.018	.018	0	0
2	M2	Z	.018	.018	0	0
3	M3	Z	.018	.018	0	0
4	M4	Z	.018	.018	0	0
5	M5	Z	.018	.018	0	0
6	M6	Z	.018	.018	0	0
7	M7	Z	.018	.018	0	0
8	M8	Z	.018	.018	0	0
9	M9	Z	.018	.018	0	0
10	M10	Z	.018	.018	0	0
11	M11	Z	.018	.018	0	0
12	M12	Z	.018	.018	0	0
13	M13	Z	.018	.018	0	0
14	M14	Z	.018	.018	0	0
15	M15	Z	.018	.018	0	0
16	PS.2	Z	.018	.018	0	0
17	M19	Z	.018	.018	0	0
18	M20	Z	.018	.018	0	0
19	M21	Z	.018	.018	0	0
20	M21A	Z	.018	.018	0	0

Member Distributed Loads (BLC 11: Wm Wind Z)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude(k/	.Start Location[ft,%]	End Location[ft.%]
1	M1	Z	.003	.003	0	0
2	M2	Z	.003	.003	0	0
3	M3	Z	.003	.003	0	0
4	M4	Z	.003	.003	0	0
5	M5	Z	.003	.003	0	0
6	M6	Z	.003	.003	0	0
7	M7	Z	.003	.003	0	0
8	M8	Z	.003	.003	0	0
9	M9	Z	.003	.003	0	0
10	M10	Z	.003	.003	0	0
11	M11	Z	.003	.003	0	0
12	M12	Z	.003	.003	0	0
13	M13	Z	.003	.003	0	0
14	M14	Z	.003	.003	0	0
15	M15	Z	.003	.003	0	0
16	PS.2	Z	.003	.003	0	0
17	M19	Z	.003	.003	0	0
18	M20	Z	.003	.003	0	0
19	M21	Z	.003	.003	0	0
20	M21A	Z	.003	.003	0	0



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Basic Load Cases

	BLC Description	Category	X Gra	Y Gra.	Z Gra	Joint	Point	Distrib.	.Area(Surfa
1	Self Weight	None		-1						
2	Dead Load	None			LIES.		9			100
3	Ice Load	None					9			
4	Lm Maintenance Load (500lb)	None				TITLE	1	m	VOx	
5	Lv Maintenance Load (250lb)	None					1			
6	Wind with Ice X	None	100		1,315		9	19	124	1
7	Wind X	None					9	19		
8	Wm Wind X	None	THE P		4	1	9	19		
9	Wind with Ice Z	None					7	20		
10	Wind Z	None					7	20	ET pl	
11	Wm Wind Z	None					7	20		

Load Combinations

	Description	So	Р	s	BLC	Fac	BLC	Fac.	BLC	Fac	BLC	Fac.	BLC	Fac										
1	1.4D	Yes	Υ		1	1.4	2	1.4					_								_			
2	1.2D +1.5Lv	Yes	Υ		1	1.2	2	1.2	5	1.5														
3	1.2D + 1.0W (X-directi	Yes	Y		1	1.2	2	1.2	7	1														
4	1.2D + 1.0Di + 1.0Wi (Yes	Y		1	1.2	2	1.2	3	1	6	1												
	1.2D +1.5Lm+ 1.0Wm				1	1.2	2	1.2	4	1.5	8	1												
6	1.2D + 1.0W (Z-directi	Yes	Y		1	1.2	2	1.2	10	1														
7	1.2D + 1.0Di + 1.0Wi (1	1.2	2	1.2	3	1	9	1											_	
8	1.2D +1.5Lm+ 1.0Wm	Yes	Y		1	1.2	2	1.2	4	1.5	11	1							1					

Envelope Joint Reactions

	Joint		X [k]	LC	Y [k]	LC	Z [k]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	.34	3	.025	1	0	2	0	8	0	8	0	8
2		min	0	2	.021	5	-1.615	3	0	1	0	1	0	1
3	N35	max	.032	1	.828	8	1.178	3	0	8	0	8	0	8
4	1100	min	-1.724	3	.411	3	-2.16	6	0	1	0	1	0	1
5	N36	max	1.322	8	.8	5	.838	5	0	8	0	8	0	8
6	1100	min	-1.257	3	.43	6	-1.569	6	0	1	0	1	0	1
7	Totals:	max	0	8	1.63	8	0	3						
8	Totalo.	min	-2.641	3	.88	3	-4.193	6						

Envelope Joint Displacements

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation [rad]	LC	Y Rotation [rad]	LC	Z Rotation [rad]	LC
1	N1	max	.106	3	.058	8	1.084	6	1.511e-03	8	2.992e-02	6	2.819e-03	3
2	Egm	min	057	6	174	4	079	5	-4.861e-03	6	-7.935e-04	5	-6.04e-04	8
3	N2	max	.04	3	.058	8	1.055	6	3.696e-03	6	2.988e-02	6	2.558e-03	4
4	112	min	069	6	174	4	016	5	-3.713e-03	3	-6.653e-04	5	-6.299e-04	8
5	N3	max	.106	3	.056	8	.995	6	1.511e-03	8	2.992e-02	6	2.819e-03	3
6	140	min	057	6	166	4	077	5	-4.861e-03	6	-7.935e-04	5	-6.04e-04	8
7	N4	max	.04	3	.056	8	.965	6	3.696e-03	6	2.988e-02	6	2.558e-03	4
8		min	069	6	166	4	014	5	-3.713e-03	3	-6.653e-04	5	-6.3e-04	8
9	N5	max	.16	3	.056	8	1.094	6	1.479e-03	5	2.992e-02	6	3.511e-03	3
10	140	min	063	8	166	4	101	5	-6.528e-03	6	-7.935e-04	5	-6.039e-04	8



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Envelope Joint Displacements (Continued)

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation [rad]	LC	Y Rotation [rad]	ıc	Z Rotation [rad]	LC
11	N6	max	.033	5	.056	8	1.046	6	5.366e-03	6	2.988e-02	6	2.42e-03	4
12		min	089	6	166	4	059	3	-3.713e-03	3	-6.653e-04	5	-6.301e-04	8
13	N7	max	.04	3	.055	8	.02	3	2.29e-03	6	1.812e-02	6	2.938e-03	4
14		min	068	6	027	4	016	6	-1.974e-03	3	-1.636e-03	3	-2.934e-04	8
15	N8	max	0	8	0	8	0	8	1.803e-03	1	7.927e-03	3	3.287e-03	4
16		min	0	1	0	1	0	1	1.228e-03	4	-7.299e-06	1	-2.824e-04	8
17	N9	max	.105	3	.053	5	.129	3	1.201e-03	8	1.576e-02	6	2.16e-03	4
18		min	057	6	015	4	078	6	-2.42e-03	6	-1.146e-03	5	-6.554e-04	8
19	N10	max	.04	3	.053	5	.028	3	2.115e-03	6	1.504e-02	6	2.206e-03	4
20		min	068	6	015	4	094	6	-1.758e-03	3	-1.547e-03	3	-6.125e-04	8
21	N11	max	.081	3	.057	8	.105	3	1.637e-03	8	5.557e-03	3	1.456e-03	6
22		min	045	6	01	3	066	6	-1.478e-03	3	-3.593e-03	8	-8.994e-04	5
23	N12	max	.027	3	.057	8	.016	3	1.67e-03	8	2.826e-03	3	1.387e-03	3
24		min	054	6	01	3	081	6	-1.694e-03	3	-3.177e-03	6	-9.027e-04	8
25	N17	max	.016	3	.052	8	.041	3	1.222e-03	8	4.146e-03	3	9.478e-04	3
26		min	011	6	018	3	034	6	-3.075e-04	3	-2.57e-03	6	-3.383e-03	8
27	N18	max	0	1	.052	8	0	1	1.191e-03	8	6.319e-04	3	1.175e-03	3
28		min	014	6	018	3	041	6	-4.898e-04	3	-3.144e-03	6	-3.369e-03	8
29	N19	max	0	8	.039	8	.025	3	7.979e-04	4	3.039e-03	3	1.442e-03	3
30		min	0	1	012	3	023	6	3.864e-04	6	-2.764e-03	6	-4.707e-03	8
31	N20	max	0	8	.039	8	0	1	8.179e-04	7	2.96e-05	1	1.429e-03	3
32		min	0	1	012	3	028	6	4.389e-04	3	-3.384e-03	6	-4.707e-03	8
33	N21	max	0	8	.012	3	.023	6	7.979e-04	4	3.039e-03	3	1.442e-03	3
34		min	0	1	039	8	025	3	3.864e-04	6	-2.764e-03	6	-4.707e-03	8
35	N22	max	0	8	.012	3	.028	6	8.179e-04	7	2.96e-05	1	1.429e-03	3
36		min	0	1	039	8	0	1	4.389e-04	3	-3.384e-03	6	-4.707e-03	8
37	N23	max	.016	3	.014	3	.035	6	7.449e-04	3	4.145e-03	3	7.879e-04	3
38		min	012	6	059	8	041	3	-5.568e-04	8	-2.891e-03	6	-3.773e-03	8
39	N24	max	0	1	.014	3	.043	6	9.893e-04	3	7.118e-04	3	1.039e-03	3
40		min	014	6	059	8	0	1	-5.951e-04	8	-3.45e-03	6	-3.798e-03	8
41	N25	max	.081	3	.003	3	.072	6	1.405e-03	3	5.567e-03	3	-3.696e-04	3
42		min	048	6	07	8	105	3	-2.084e-03	8	-3.564e-03	8	-3.713e-03	5
43	N26	max	.028	3	.003	3	.086	6	1.567e-03	3	3.01e-03	3	3.438e-04	3
44	10	min	056	6	07	8	02	3	-2.095e-03	8	-3.139e-03	6	-3.712e-03	8
45	N27	max	.105	3	005	6	.083	6	1.672e-03	3	4.624e-03	3	-6.72e-04	3
46		min	059	6	081	5	129	3	-7.395e-04	6	-2.077e-02	6	-5.886e-03	8
47	N28	max	.041	3	002	3	1	6	1.791e-03	3	2.922e-03	3	-1.358e-04	3
48		min	069	6	081	8	033	3	-4.867e-04	8	-2.101e-02	6	-5.903e-03	8
49	N29	max	.105	3	032	3	1.342	6	2.606e-03	3	3.717e-03	3	6.894e-04	3
50		min	059	6	439	8	299	3	-3.256e-03	6	-3.401e-02	6	-6.479e-03	8
51	N30	max	.041	3	032	3	1.361	6	4.179e-03	6	3.774e-03	3	-2.814e-04	3
52		min	069	6	439	8	181	3	-1.116e-03	8	-3.399e-02	6	-6.472e-03	8
53	N33	max	.105	3	03	3	1.444	6	2.606e-03	3	3.717e-03	3	6.894e-04	3
54		min	059	6	459	8	311	3	-3.256e-03	6	-3.401e-02	6	-6.479e-03	8
55	N34	max	.041	3	032	3	1.463	6	4.179e-03	6	3.774e-03	3	-2.815e-04	3
56	Tibor.	min	069	6	459	8	192	3	-1.116e-03	8	-3.399e-02	6	-6.472e-03	8
57	N35	max	0	8	0	8	0	8	8.179e-04	7	2.96e-05	1	1.429e-03	3
58	1 0	min	0	1	0	1	0	1	4.389e-04	3	-3.384e-03	6	-4.707e-03	8
59	N36	max	0	8	0	8	0	8	7.979e-04	4	3.039e-03	3	1.442e-03	3
60	Tarrest	min	0	1	0	1	0	1	3.864e-04	6	-2.764e-03	6	-4.707e-03	8
61	N35A	max	.105	3	.049	5	.113	3	1.026e-03	8	1.204e-02	6	1.37e-03	4
62		min	057	6	006	1	156	6	-2.276e-03	6	-1.214e-03	5	-9.843e-04	8



: Centek Engineering

23134.00 : East Haddam 3 Model Name

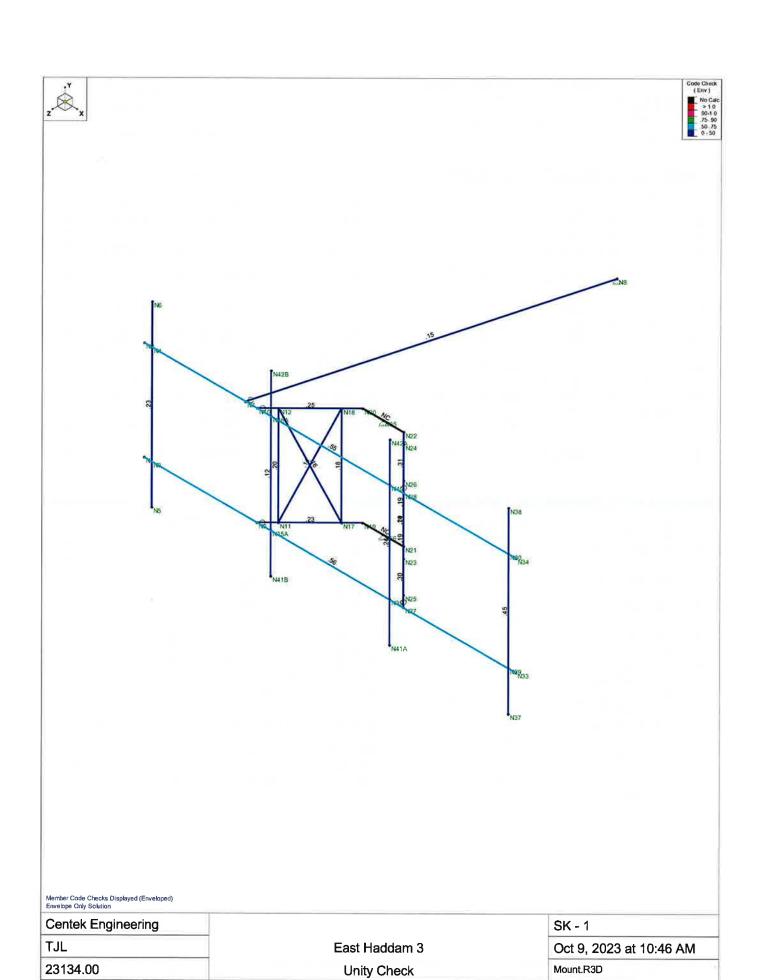
Oct 9, 2023 10:46 AM Checked By:_

Envelope Joint Displacements (Continued)

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation [rad]	LC	Y Rotation [rad]	LC	Z Rotation [rad]	LC
63	N36A	max	.04	3	.048	5	.035	3	1.972e-03	6	1.136e-02	6	1.366e-03	4
64		min	069	6	006	1	168	6	-1.555e-03	3	-8.244e-04	3	-9.544e-04	8
65	N37	max	.125	3	032	3	1.415	6	2.605e-03	3	3.717e-03	3	1.382e-03	3
66		min	159	8	439	8	341	3	-4.853e-03	6	-3.401e-02	6	-6.478e-03	8
67	N38	max	.128	5	032	3	1.449	6	5.779e-03	6	3.774e-03	3	-8.53e-04	6
68		min	056	6	439	8	138	3	-1.104e-03	5	-3.399e-02	6	-6.473e-03	8
69	N39	max	.105	3	002	6	.039	8	1.541e-03	3	4.791e-03	3	-3.497e-04	3
70		min	058	6	053	5	103	3	-5.75e-04	8	-1.729e-02	6	-4.218e-03	8
71	N40	max	.041	3	002	6	0	1	1.623e-03	3	2.71e-03	3	2.554e-04	3
72		min	069	6	053	5	017	3	-5.2e-04	8	-1.759e-02	6	-4.231e-03	8
73	N41A	max	.1	3	002	6	.048	8	1.541e-03	3	4.791e-03	3	-2.793e-04	3
74		min	122	8	053	5	127	3	-6.067e-04	6	-1.729e-02	6	-4.218e-03	8
75	N42A	max	.092	5	002	6	.012	6	1.623e-03	3	2.71e-03	3	1.851e-04	3
76		min	061	6	053	5	015	5	-5.083e-04	8	-1.759e-02	6	-4.232e-03	8
77	N41B	max	.127	3	.048	5	.136	3	1.023e-03	5	1.204e-02	6	1.426e-03	4
78		min	07	8	006	1	111	6	-2.878e-03	6	-1.214e-03	5	-9.841e-04	8
79	N42B	max	.039	5	.048	5	.026	5	2.575e-03	6	1.136e-02	6	1.31e-03	4
80		min	081	6	006	1	129	6	-1.556e-03	3	-8.244e-04	3	-9.546e-04	8

Envelope AISC 15th(360-16): LRFD Steel Code Checks

	Mem	Shape	Code Check	L	LC	ShLoc[ft]	Dir .	phi*P	phi*P	phi*Mn y-y [k-ft]	phi*Cb Eqn
1	M2	PIPE 2.5	.560	3	6	.088 3.776	6	14.559	50.715	3.596	3.5 1 H1
2	M1	PIPE 2.5	.545	3	6	141 3.776	3	14.559	50.715	3.596	3.5 1 H1
3	PS.1	PIPE 2.0	.446	4	5	.056 4.625			32.13	1.872	1.8 1H1
4	M6	PIPE 2.0	.309	2	8	.119 .499	5	32.032	32.13	1.872	1.81H1
5	M7	PIPE 2.0	.304	2	8	.121 .499	8	32.032	32.13	1.872	1.8 1 H1
6	M4	PIPE 2.0	.250	2	5	.093 2.521	5	32.032	32.13	1.872	1.8 1 H1
7	M19	PIPE 2.0	.243	4	5	071 4.625			32.13	1.872	1.8 1H1
8	M5	PIPE 2.0	.233	2	8	.092 .499	6	32.032	32.13	1.872	1.81H1
9	PS.2	PIPE 2.0	.225	1	4	051 4.688	e	20.867	32.13	1.872	1.8 1 H1
10	M14		.197	0	5	.014 0	5	6.954	14.314	.179	.179 2H1
11	M8	0.625' Dia.	.197	0	6	.026 0	8	1.058	9.94	.104	.104 2H1
12		0.625' Dia.	.193	3	3	.022 3.333	3	1.058	9.94	.104	.104 2H1
13	_	0.625' Dia.	.190	0	3	026 3.333	8	1.058	9.94	.104	.104 2H1
14	M9	0.625' Dia.	.180	0	6	023 3.333	3	1.058	9.94	.104	.104 2H1
15	M10		.159	3	3	019 0	8	6.954	14.314	.179	.179 1 H1
16	M3	PIPE 2.0	.150	5	3	.009 10.18	3	9.492	32.13	1.872	1.8 1 H1
17	M15		.144	3	8	.028 0	3	6.954	14.314	.179	.179 2H1
18	M12		.137	0	3	026 3.659	3	6.954	14.314	.179	.179 2H1
19	_	PIPE 2.0	.121	4	7	077 4.625		20.867	32.13	1.872	1.81H1



≡Kengineering

Subject:

Connection to Host Structure

Centered on Solutions* ***
63-2 North Branford Road Pr. (203) 488-0580
Branford, CT 06405 F: (203) 488-8587

Location:

East Haddam, CT

Prepared by: T.J.L. Checked by: C.F.C.

Job No. 23134.00

Antenna Mount Connection:

Anchor Data:

Rev. 0: 10/9/23

A307 Threaded Rod =

Number of Anchor Bolts =

(User Input) N := 4

Diameter of Bolts= D := 0.625in(User Input)

T_{design} := 10.4-kips Design Tension = (User Input)

V_{design} := 6.23·kips (User Input) Design Shear =

Design Reactions:

Fx= F_x:= 1.8·kips (User Input)

Fy= $F_{V} := 0.8 \cdot \text{kips}$ (User Input)

F₇ := 2.2 kips (User Input) Fz=

Anchor Check:

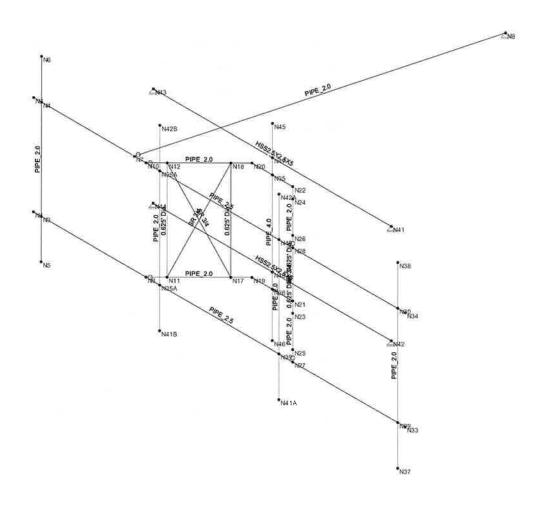
 $T_{\text{Max}} := \frac{F_z}{N} = 550 \, \text{lb}$ Max Tension Force =

 $V_{Max} := \frac{F_y}{N} + \frac{F_x}{N} = 650 lb$ Max Shear Force =

 $Condition1 := if \left(\frac{T_{Max}}{T_{design}} + \frac{V_{Max}}{V_{design}} \le 1.0, "OK", "NG" \right) = "OK"$ Condition 1 =

T_{design} % of Capacity=





Envelope Only Solution

Centek Engineering		
TJL	East Haddam 3	Oct 9, 2023 at 10:57 AM
23134.00	Member Framing	Mount - Face.R3D



: Centek Engineering

: TJL : 23134.00 : East Haddam 3 Oct 9, 2023 10:56 AM Checked By:_____

(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (ft/sec^2)	32.2
Wall Mesh Size (in)	12
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-10: ASD
Wood Code	AWC NDS-12: ASD
Wood Temperature	< 100F
Concrete Code	ACI 318-11
Masonry Code	ACI 530-11: ASD
Aluminum Code	AA ADM1-10: ASD - Building
Stainless Steel Code	AISC 14th(360-10): ASD
Adjust Stiffness?	Yes(Iterative)

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parme Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR_SET_ASTMA615
Min % Steel for Column	1 3
Max % Steel for Column	8



Centek Engineering
TJL

: TJL : 23134.00 : East Haddam 3 Oct 9, 2023 10:56 AM Checked By:____

(Global) Model Settings, Continued

Seismic Base Elevation (ft)	Seismic Code	ASCE 7-10
Ct X .02 Ct Z .02 T X (sec) Not Entered T Z (sec) Not Entered R X 3 R Z 3 Ct Exp. X .75 Ct Exp. Z .75 SD1 1 SDS 1 S1 1 TL (sec) 5 Risk Cat I or II Drift Cat Other Om Z 1 Om X 1 Cd Z 1 Cd X 1 Rho Z 1 Rho X 1 Rho Z 1 Rho X 1 Pooting Overturning Safety Factor 1 Optimize for OTM/Sliding No Check Concrete Bearing No Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete Ec (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0075	Seismic Base Elevation (ft)	Not Entered
Ct Z .02 T X (sec) Not Entered R X 3 R Z 3 Ct Exp. X .75 Ct Exp. Z .75 SD1 1 SDS 1 S1 1 TL (sec) 5 Risk Cat I or II Drift Cat Other Om Z 1 Om X 1 Cd Z 1 Cd X 1 Rho Z 1 Rho X 1 Footing Overturning Safety Factor 1 Optimize for OTM/Sliding No Check Concrete Bearing No Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete Fc (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0018 Maximum Steel 0.0075 Footing Top Bar #3 Footing Bottom Bar Cover (in) 2 Footing Bottom Bar C	Add Base Weight?	Yes
T X (sec) Not Entered T Z (sec) Not Entered R X 3 R Z 3 Cit Exp. X .75 Cit Exp. Z .75 SD1 1 SDS 1 ST 1 S	Ct X	.02
T Z (sec)	Ct Z	.02
T Z (sec)	T X (sec)	Not Entered
R X	T Z (sec)	
Ct Exp. X .75 Ct Exp. Z .75 SD1 1 SDS 1 ST 1 TL (sec) 5 Risk Cat I or II Drift Cat Other Om Z 1 Om X 1 Cd Z 1 Cd X 1 Rho Z 1 Rho X 1 Footing Overturning Safety Factor 1 Optimize for OTM/Sliding No Check Concrete Bearing No Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete Ec (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0018 Maximum Steel 0.0075 Footing Top Bar #3 Footing Bottom Bar Cover (in) 2 Footing Bottom Bar Cover (in) 3.5 Pedestal Bar #3 Pedestal Bar Cover (in) 1.5		3
Ct Exp. Z .75 SD1 1 SDS 1 S1 1 TL (sec) 5 Risk Cat I or II Drift Cat Other Om Z 1 Om X 1 Cd Z 1 Cd X 1 Rho Z 1 Rho X 1 Footing Overturning Safety Factor 1 Optimize for OTM/Sliding No Check Concrete Bearing No Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete Ec (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0018 Maximum Steel 0.0075 Footing Top Bar #3 Footing Bottom Bar #3 Footing Bottom Bar Cover (in) 3.5 Pedestal Bar #3 Pedestal Bar Cover (in) 1.5	RZ	3
SDS	Ct Exp. X	.75
SDS 1 S1 1 TL (sec) 5 Risk Cat I or II Drift Cat Other Om Z 1 Om X 1 Cd Z 1 Cd X 1 Rho Z 1 Rho X 1 Footing Overturning Safety Factor 1 Optimize for OTM/Sliding No Check Concrete Bearing No Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete Fc (ksi) 4 Footing Concrete Ec (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0018 Maximum Steel 0.0075 Footing Top Bar #3 Footing Bottom Bar #3 Footing Bottom Bar Cover (in) 3.5 Pedestal Bar #3 Pedestal Bar Cover (in) 1.5	Ct Exp. Z	.75
S1	SD1	1
TL (sec) 5 Risk Cat I or II Drift Cat Other Om Z 1 Om X 1 Cd Z 1 Cd X 1 Rho Z 1 Rho X 1 Footing Overturning Safety Factor 1 Optimize for OTM/Sliding No Check Concrete Bearing No Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete Fic (ksi) 4 Footing Concrete Ec (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0018 Maximum Steel 0.0075 Footing Top Bar #3 Footing Bottom Bar Cover (in) 2 Footing Bottom Bar Cover (in) 3.5 Pedestal Bar #3 Pedestal Bar Cover (in) 1.5	SDS	1.
Risk Cat	S1	
Drift Cat Other Om Z 1 Om X 1 Cd Z 1 Cd X 1 Rho Z 1 Rho X 1 Footing Overturning Safety Factor 1 Optimize for OTM/Sliding No Check Concrete Bearing No Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete F'c (ksi) 4 Footing Concrete Ec (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0018 Maximum Steel 0.0075 Footing Top Bar #3 Footing Bottom Bar #3 Footing Bottom Bar Cover (in) 3.5 Pedestal Bar #3 Pedestal Bar Cover (in) 1.5	TL (sec)	
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Om X 1 Cd Z 1 Cd X 1 Rho Z 1 Rho X 1 Footing Overturning Safety Factor 1 Optimize for OTM/Sliding No Check Concrete Bearing No Footing Concrete Weight (k/ft^3) 150.001 Footing Concrete F'c (ksi) 4 Footing Concrete Ec (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0018 Maximum Steel 0.0075 Footing Top Bar #3 Footing Top Bar Cover (in) 2 Footing Bottom Bar #3 Footing Bottom Bar Cover (in) 3.5 Pedestal Bar #3 Pedestal Bar Cover (in) 1.5	Drift Cat	Other
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Footing Concrete f'c (ksi) 4 Footing Concrete Ec (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0018 Maximum Steel 0.0075 Footing Top Bar #3 Footing Top Bar Cover (in) 2 Footing Bottom Bar #3 Footing Bottom Bar Cover (in) 3.5 Pedestal Bar #3 Pedestal Bar Cover (in) 1.5		- 174
Footing Concrete Ec (ksi) 3644 Lambda 1 Footing Steel fy (ksi) 60 Minimum Steel 0.0018 Maximum Steel 0.0075 Footing Top Bar #3 Footing Top Bar Cover (in) 2 Footing Bottom Bar #3 Footing Bottom Bar Cover (in) 3.5 Pedestal Bar #3 Pedestal Bar Cover (in) 1.5		
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Pedestal Bar #3 Pedestal Bar Cover (in) 1.5		The state of the s
Pedestal Bar Cover (in) 1.5		
	Pedestal Ties	#3

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (\ [Density[k/ft^3]	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
2	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	58	1.2
3	A992	29000	11154	.3	.65	.49	50	1.1	58	1.2
4	A500 Gr.42	29000	11154	.3	.65	.49	42	1.3	58	1.1
5	A500 Gr.46	29000	11154	.3	.65	.49	46	1.2	58	1.1
6	A53 Grade B	29000	11154	.3	.65	.49	35	1.5	58	1.2



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Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design	A [in2]	lyy [in4]	Izz [in4]	J [in4]
1	Antenna Mast 2.0	PIPE 2.0	Column	Pipe	A53 Grade B	Typical	1.02	.627	.627	1.25
2	Horizontal 2.5 ST	PIPE 2.5	Beam	Pipe	A53 Grade B	Typical	1.61	1.45	1.45	2.89
3	Outrigger_2.0 ST	PIPE 2.0	Beam	Pipe	A53 Grade B	Typical	1.02	.627	.627	1.25
4	Stabilizer 2.0 ST	PIPE 2.0	Beam	Pipe	A53 Grade B	Typical	1.02	.627	.627	1.25
5	0.625" Dia. Bar	0.625' Dia.	Column	BAR	A36 Gr.36	Typical	.307	.007	.007	.015
6	0.75"Dia. Bar	SR 3/4	Column	BAR	A36 Gr.36	Typical	.442	.016	.016	.031
7	Vert	PIPE 4.0	Column	Pipe	A53 Grade B	Typical	2.96	6.82	6.82	13.6
8	Face Tube	HSS2.5X2.5X5	Beam	Tube	A500 Gr.46	Typical	2.35	1.82	1.82	3.2

Hot Rolled Steel Design Parameters

	Label	Shape	Length[ft]	Lbyy[ft]	Lbzz[ft]	Lcomp top[.	Lcomp bot[.L-torg	Kyy	Kzz	Cb	Functi
1	M1	Horizontal_2.5 STD		Segment		Lbyy						Lateral
2	M2	Horizontal_2.5 STD	12.5	Segment		Lbyy		III III				Lateral
3	M3	Stabilizer_2.0 STD	10.18			Lbyy						Lateral
4	M4	Outrigger_2.0 STD	2.521	Segment	Segment	Lbyy						Lateral
5	M5	Outrigger_2.0 STD	2.521	Segment	Segment	Lbyy						Lateral
6	M6	Outrigger 2.0 STD	2.521	Segment			QLI .			ALC: Y		Lateral
7	M7	Outrigger_2.0 STD	2.521	Segment		Lbyy						Lateral
8	M8	0.625" Dia. Bar	3.333				370					Lateral
9	M9	0.625" Dia. Bar	3.333									Lateral
10	M10	0.75"Dia, Bar	3.659	1.83	1.83	Lbyy	- L					Lateral
11	M11	0.625" Dia. Bar	3.333									Lateral
12	M12	0.75"Dia. Bar	3.659	1.83	1.83	Lbyy	12-22-1			Eu.		Lateral
13	M13	0.625" Dia. Bar	3.333									Lateral
14	M14	0.75"Dia. Bar	3.659	1.83	1.83	Lbyy	A THEFT OF					Lateral
15	M15	0.75"Dia. Bar	3.659	1.83	1.83	Lbyy						Lateral
16	PS.2	Antenna Mast 2.0	6			Lbyy	He LITT V T	H-				Lateral
17	PS.1	Antenna Mast 2.0	6			Lbyy						Lateral
18	M19	Antenna Mast 2.0	6			Lbyy	PASSELL II					Lateral
19	M21A	Antenna Mast 2.0	6			Lbyy						Lateral
20	M22	Vert	6.333									Lateral
21	M23	Face Tube	8			Lbyy						Lateral
22	M24	Face Tube	8			Lbyy						Lateral

Member Primary Data

	Label	I Joint	J Joint	K Joint	Rotate(. Section/Shape	Type	Design List	Material	Design
1	M1	N2	N34			Horizontal_2.5 STD Pipe	Beam	Pipe	A53 Grade B	Typical
2	M2	N1	N33			Horizontal_2.5 STD Pipe	Beam	Pipe	A53 Grade B	Typical
3	M3	N7	N8			Stabilizer_2.0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
4	M4	N10	N20			Outrigger_2.0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
5	M5	N9	N19			Outrigger_2.0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
6	M6	N28	N22		TAX :	Outrigger_2.0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
7	M7	N27	N21			Outrigger_2.0 STD Pipe	Beam	Pipe	A53 Grade B	Typical
8	M8	N12	N11			0.625" Dia. Bar	Column	BAR	A36 Gr.36	Typical
9	M9	N18	N17			0.625" Dia. Bar	Column	BAR	A36 Gr.36	Typical
10	M10	N12	N17			0.75"Dia. Bar	Column	BAR	A36 Gr.36	Typical
11	M11	N26	N25			0.625" Dia. Bar	Column	BAR	A36 Gr.36	Typical
12	M12	N18	N11			0.75"Dia. Bar	Column	BAR	A36 Gr.36	Typical



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Member Primary Data (Continued)

	Label	I Joint	J Joint	K Joint F	Rotate(.	Section/Shape	Type	Design List	Material	Design
_13	M13	N24	N23			0.625" Dia. Bar	Column	BAR	A36 Gr.36	Typical
14	M14	N26	N23			0.75"Dia. Bar	Column	BAR	A36 Gr.36	Typical
15	M15	N24	N25			0.75"Dia. Bar	Column	BAR	A36 Gr.36	Typical
16	PS.2	N5	N6			Antenna Mast_2.0 STD Pi	Column	Pipe	A53 Grade B	Typical
_17	PS.1	N37	N38			Antenna Mast_2.0 STD Pi	Column	Pipe	A53 Grade B	Typical
18	M19	N41A	N42A		CHAR	Antenna Mast_2.0 STD Pi	Column	Pipe	A53 Grade B	Typical
19	M20	N19	N21			RIGID	None	None	RIGID	Typical
20	M21	N20	N22		1200	RIGID	None	None	RIGID	Typical
21	M21A	N41B	N42B			Antenna Mast_2.0 STD Pi	Column	Pipe	A53 Grade B	Typical
22	M22	N45	N46			Vert	Column	Pipe	A53 Grade B	Typical
23	M23	N43	N41			Face Tube	Beam	Tube	A500 Gr.46	Typical
24	M24	N44	N42			Face Tube	Beam	Tube	A500 Gr.46	Typical

Joint Coordinates and Temperatures

	Label	X [ft]	Y [ft]	Z [ft]	Temp [F]	Detach From Dia.
_1	N1	0	0.	-0.	0	
2	N2	0	3.333334	-0.	0	
3	N3	.25	0.	-0.	0	
4	N4	.25	3.333334	-0.	0	- 188
5	N5	.25	-1.333333	-0.	0	
6	N6	.25	4.666667	-0.	0	
7	N7	3.390625	3.333334	-0.	0	
8	N8	6.025403	3.333334	-9.833125	0	
9	N9	3.78125	0.	-0.	0	
10	N10	3.78125	3.333334	-0.	0	THE PERSON NAMED IN
11	N11	4.138628	0.	-0.357378	0	
12	N12	4.138628	3.333334	-0.357378	0	15 1815
13	N17	5.206335	0.	-1.425085	0	
14	N18	5.206335	3.333334	-1.425085	0	ACTUAL TO
15	N19	5.563713	0.	-1.782463	0	
16	N20	5.563713	3.333334	-1.782463	0	A RESIDE OF
17	N21	6.936287	0.	-1.782463	0	
18	N22	6.936287	3.333334	-1.782463	0	SSM IS
19	N23	7.293665	0.	-1.425085	0	
20	N24	7.293665	3.333334	-1.425085	0	
21	N25	8.361372	0.	-0.357378	0	
22	N26	8.361372	3.333334	-0.357378	0	and the Property of the Land
23	N27	8.71875	0.	-0.	0	
24	N28	8.71875	3.333334	-0.	0	
25	N29	12.25	0.	-0.	0	
26	N30	12.25	3.333334	-0.	Ō	
27	N33	12.5	0.	-0.	0	
28	N34	12.5	3.333334	-0.	Ö	
29	N35	6.25	3.333334	-1.782463	0	
30	N36	6.25	0.	-1.782463	Ö	
31	N35A	4.25	0.	-0.	0	
32	N36A	4.25	3.333334	-0.	ő	
33	N37	12.25	-1.333333	0	0	
34	N38	12.25	4.666667	0	Ö	
35	N39	8.25	0.	-0.	0	



Company Designer

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Joint Coordinates and Temperatures (Continued)

	Label	X [ft]	Y [ft]	Z [ft]	Temp [F]	Detach From Dia.
36	N40	8.25	3.333334	-0.	0	
37	N41A	8.25	-1.333333	-0,	0	
38	N42A	8.25	4.666667	-0.	0	
39	N41B	4.25	-1.333333	-0.	0	
40	N42B	4.25	4.666667	-0.	0	1 1 2
41	N41	10.25	3.833334	-1.782463	0	
42	N42	10.25	0.5	-1.782463	0	
43	N43	2.25	3.833334	-1.782463	0	
44	N44	2.25	0.5	-1.782463	0	
45	N45	6.25	4.833334	-1.782463	0	
46	N46	6.25	-1.5	-1.782463	0	Life Calphanill
47	N47	6.25	3.833334	-1.782463	0	
48	N48	6.25	0.5	-1.782463	0	

Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	N8	Reaction	Reaction	Reaction			
2	N19						
3	N20						
4	N17		1, 3, 20			And agreed	
5	N18						
6	N21					0.00	
7	N22						
8	N23		61				
9	N24						
10	N35		0				
11	N36						
12	N41	Reaction	Reaction	Reaction			
13	N42	Reaction	Reaction	Reaction			
14	N43	Reaction	Reaction	Reaction			
15	N44	Reaction	Reaction	Reaction			
16	N45						
17	N46						
18	N47						
19	N48						

Member Point Loads (BLC 2 : Dead Load)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	Y	04	.5
2	PS.1	Y	04	.5
3	PS.2	Υ	04	5.5
4	PS.1	Y	04	5.5
5	M21A	Y	03	.5
6	M21A	Y	03	5.5
7	PS.2	Υ	075	%50
8	PS.1	Y	08	%50
9	M19	Υ	032	%50



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Member Point Loads (BLC 3 : Ice Load)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	Y	104	.5
2	PS.1	Y	104	.5
3	PS.2	Y	104	5.5
4	PS.1	Y	104	5.5
5	M21A	Y	04	.5
6	M21A	Y	04	5.5
7	PS.2	Y	061	%50
8	PS.1	Y	062	%50
9	M19	Y	117	%50

Member Point Loads (BLC 4 : Lm Maintenance Load (500lb))

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.1	Y	5	%50

Member Point Loads (BLC 5 : Lv Maintenance Load (250lb))

.,	Member Label	Direction	Magnitude[k,k-ft]	Location[ft.%]
1	M1	Υ	25	12

Member Point Loads (BLC 6 : Wind with Ice X)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	X	.037	.5
2	PS.1	X	.037	.5
3	PS.2	X	.037	5.5
4	PS.1	X	.037	5.5
5	M21A	X	.013	.5
6	M21A	X	.013	5.5
7	PS.2	X	.022	%50
8	PS.1	X	.022	%50
9	M19	X	.047	%50

Member Point Loads (BLC 7: Wind X)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	X	.181	.5
2	PS.1	X	.181	.5
3	PS.2	X	.181	5.5
4	PS.1	X	.181	5.5
5	M21A	X	.054	-5
6	M21A	X	.054	5.5
7	PS.2	X	.102	%50
8	PS.1	X	.104	%50
9	M19	X	.253	%50

Member Point Loads (BLC 8: Wm Wind X)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	X	.009	.5
2	PS.1	X	.009	.5
3	PS.2	X	.009	5.5
4	PS.1	X	.009	5.5
5	M21A	X	.003	.5



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Member Point Loads (BLC 8 : Wm Wind X) (Continued)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
6	M21A	X	.003	5.5
7	PS.2	X	.005	%50
8	PS.1	X	.005	%50
Q	M19	X	.012	%50

Member Point Loads (BLC 9: Wind with Ice Z)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	Z	.077	.5
2	PS.1	Z	.077	.5
3	PS.2	Z	.077	5.5
4	PS.1	Z	.077	5.5
5	M21A	7	.028	.5
6	M21A	Z	.028	5.5
7	M19	Z	.059	%50

Member Point Loads (BLC 10 : Wind Z)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	Z	.465	.5
2	PS.1	Z	.465	.5
3	PS.2	Z	.465	5.5
4	PS.1	Z	.465	5.5
5	M21A	Z	.155	.5
6	M21A	Z	.155	5.5
7	M19	Z	.331	%50

Member Point Loads (BLC 11 : Wm Wind Z)

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	PS.2	Z	.023	.5
2	PS.1	Z	.023	.5
3	PS.2	Z	.023	5.5
4	PS.1	Z	.023	5.5
5	M21A	Z	.008	.5
6	M21A	Z	.008	5.5
7	M19	Z	.016	%50

Member Distributed Loads (BLC 6: Wind with Ice X)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/.	Start Location[ft,%] End Location[ft,%]
1	M3	X	.003	.003	0	0
2	M4	X	.003	.003	0	0
3	M5	X	.003	.003	0	0
4	M6	X	.003	.003	0	0
5	M7	X	.003	.003	0	0
6	M8	X	.003	.003	0	0
7	M9	X	.003	.003	0	0
8	M10	X	.003	.003	0	0
9	M11	X	.003	.003	0	0
10	M12	X	.003	.003	0	0
11	M13	X	.003	.003	0	0
12	M14	X	.003	.003	0	0



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Member Distributed Loads (BLC 6: Wind with Ice X) (Continued)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/	Start LocationIft %	1 End Location(ft %)
13	M15	X	.003	.003	0	0
14	PS.2	X	.003	.003	0	0
15	PS.1	X	.003	.003	0	0
16	M19	X	.003	.003	0	0 1
17	M20	X	.003	.003	0	0
18	M21	X	.003	.003	0	0
19	M21A	X	.003	.003	0	0

Member Distributed Loads (BLC 7 : Wind X)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude(k/	.Start Location[ft.%]	End Location[ft,%]
1	M3	X	.018	.Õ18	0	0
2	M4	X	.018	.018	0	0
3	M5	X	.018	.018	0	0
4	M6	X	.018	.018	0	0
5	M7	X	.018	.018	0	0
6	M8	X	.018	.018	0	0
7	M9	X	.018	.018	0	0
8	M10	X	.018	.018	0	0
9	M11	X	.018	.018	0	0
10	M12	X	.018	.018	0	Ō
11	M13	X	.018	.018	0	0
12	M14	X	.018	.018	0	0
13	M15	X	.018	.018	0	0
14	PS.2	X	.018	.018	0	0
15	PS.1	X	.018	.018	0	0
16	M19	X	.018	.018	0	0
17	M20	X	.018	.018	0	0
18	M21	X	.018	.018	0	0
19	M21A	X	.018	.018	0	0

Member Distributed Loads (BLC 8: Wm Wind X)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/	.Start Location[ft.%]	End Location[ft,%]
1	M3	X	.003	.003	0	0
2	M4	X	.003	.003	0	0
3	M5	X	.003	.003	0	0
4	M6	X	.003	.003	0	0
5	M7	X	.003	.003	0	0
6	M8	X	.003	.003	0	0
7	M9	X	.003	.003	0	0
8	M10	X	.003	.003	0	0
9	M11	X	.003	.003	0	0
10	M12	X	.003	.003	0	0
11	M13	X	.003	.003	0	0
12	M14	X	.003	.003	0	0
13	M15	X	.003	.003	0	0
14	PS.2	X	.003	.003	0	0
15	PS.1	X	.003	.003	0	0
16	M19	X	.003	.003	0	0
17	M20	X	.003	.003	0	0
18	M21	X	.003	.003	0	0
19	M21A	X	.003	.003	0	0



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Member Distributed Loads (BLC 9: Wind with Ice Z)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/.	Start Location[ft,%]	End Location[ft,%]
1	M1	Z	.003	.003	0	0
2	M2	Z	.003	.003	0	0
3	M3	Z	.003	.003	0	0
4	M4	Z	.003	.003	0	0
5	M5	Z	.003	.003	0	0
6	M6	Z	.003	.003	0	0
7	M7	Z	.003	.003	0	0
8	M8	Z	.003	.003	0	0
9	M9	Z	.003	.003	0	0
10	M10	Z	.003	.003	0	0
11	M11	Z	.003	.003	0	0
12	M12	Z	.003	.003	0	0
13	M13	Z	.003	.003	0	0
14	M14	Z	.003	.003	0	0
15	M15	Z	.003	.003	0	0
16	PS.2	Z	.003	.003	0	0
17	M19	Z	.003	.003	0	0
18	M20	Z	.003	.003	0	0
19	M21	Z	.003	.003	0	0
20	M21A	Z	.003	.003	0	0

Member Distributed Loads (BLC 10 : Wind Z)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/	.Start Location[ft,%]	End Location[ft,%]
1	M1	Z	.018	.018	0	0
2	M2	Z	.018	.018	0	0
3	M3	Z	.018	.018	0	0
4	M4	Z	.018	.018	0	0
5	M5	Z	.018	.018	0	0
6	M6	Z	.018	.018	0	0
7	M7	Z	.018	.018	0	0
8	M8	Z	.018	.018	0	0
9	M9	Z	.018	.018	0	0
10	M10	Z	.018	.018	0	0
11	M11	Z	.018	.018	0	0
12	M12	Z	.018	.018	0	0
13	M13	Z	.018	.018	0	0
14	M14	Z	.018	.018	0	0
15	M15	Z	.018	.018	0	0
16	PS.2	Z	.018	.018	0	0
17	M19	Z	.018	.018	0	0
18	M20	Z	.018	.018	0	0
19	M21	Z	.018	.018	0	0
20	M21A	Z	.018	.018	0	0

Member Distributed Loads (BLC 11: Wm Wind Z)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/.	Start Location[ft,%	End Location[ft,%]
1	M1	Z	.003	.003	0	0
2	M2	Z	.003	.003	0	0
3	M3	Z	.003	.003	0	0
4	M4	Z	.003	.003	0	0
5	M5	Z	.003	.003	0	0



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Member Distributed Loads (BLC 11 : Wm Wind Z) (Continued)

	Member Label	Direction	Start Magnitude[k/ft,F,ksf]	End Magnitude[k/.	Start Location[ft,%]	End Location[ft,%]
6	M6	Z	.003	.003	0	0
7	M7	Z	.003	.003	0	0
8	M8	Z	.003	.003	0	0
9	M9	Z	.003	.003	0	0
10	M10	Z	.003	.003	0	0
11	M11	Z	.003	.003	0	0
12	M12	Z	.003	.003	0	0
13	M13	Z	.003	.003	0	0
14	M14	Z	.003	.003	0	0
15	M15	Z	.003	.003	0	0
16	PS.2	Z	.003	.003	0	0
17	M19	Z	.003	.003	0	0
18	M20	Z	.003	.003	0	0
19	M21	Z	.003	.003	0	0
20	M21A	Z	.003	.003	0	0

Basic Load Cases

	BLC Description	Category	X Gra	Y Gra	.Z Gra	Joint	Point	Distrib.	.Area(Surfa
1	Self Weight	None		-1						
2	Dead Load	None			4		9			
3	Ice Load	None					9			
4	Lm Maintenance Load (500lb)	None	100		- 10	0.00	1	De Loon		
5	Lv Maintenance Load (250lb)	None					1			
6	Wind with Ice X	None					9	19		
7	Wind X	None					9	19		
8	Wm Wind X	None	-104				9	19		TITLE
9	Wind with Ice Z	None					7	20		
10	Wind Z	None	3111/				7	20		
11	Wm Wind Z	None					7	20		

Load Combinations

	Description	So	P	S	BLC	Fac.	BLC	Fac	BLC	Fac.	BLC	Fac.	BLC	Fac.	.BLC	Fac.	.BLC	Fac.	.BLC	Fac.	.BLC	Fac.	BLC	Fac
1	1.4D	Yes	Υ		1	1.4	2	1.4																
2	1.2D +1.5Lv	Yes	Y		1	1.2	2	1.2	5	1.5								an z						
3	1.2D + 1.0W (X-directi	Yes	Υ		1	1.2	2	1.2	7	1														
4	1.2D + 1.0Di + 1.0Wi (Yes	Υ		1	1.2	2	1.2	3	1	6	1										1		
5	1.2D +1.5Lm+ 1.0W m	Yes	Υ		1	1.2	2	1.2	4	1.5	8	1												
6	1.2D + 1.0W (Z-directi	Yes	Υ		1	1.2	2	1.2	10	1														
7	1.2D + 1.0Di + 1.0Wi (Yes	Υ		1	1.2	2	1.2	3	1	9	1												
8	1.2D +1.5Lm+ 1.0W m	Yes	Υ		1	1.2	2	1.2	4	1.5	11	1												

Envelope Joint Reactions

	Joint		X [k]	LC	Y [k]	LC	Z [k]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	.536	6	.026	1	305	1	0	8	0	8	0	8
2		min	.082	1	.019	6	-2.076	6	0	1	0	1	0	1
3	N41	max	021	1	.453	8	.235	3	0	8	0	8	0	8
4		min	761	5	.252	3	377	6	0	1	.0	1	0	1



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Envelope Joint Reactions (Continued)

	Joint		X [k]	LC	Y [k]	LC	Z [k]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
5	N42	max	.627	8	.48	8	.42	4	0	8	0	8	0	8
6	Latin de la	min	805	3	.259	3	-1.161	6	0	1	0	1	0	1
7	N43	max	021	1	.466	5	.128	6	0	8	0	8	0	8
8	- Tool 1	min	761	5	.283	6	108	4	0	1	0	1	0	1
9	N44	max	.627	8	.458	4	.509	5	0	8	0	8	0	8
10		min	805	3	.274	6	707	6	0	1	0	1	0	1
11	Totals:	max	0	7	1.86	8	0	1						
12		min	-2.641	3	1.11	3	-4.193	6			Har	OH.	No.	

Envelope Joint Displacements

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation [rad]	LC	Y Rotation [rad]	LC	Z Rotation [rad]	LC
1	N1	max	.178	3	.036	6	1.514	6	1.422e-02	5	2.163e-02	6	2.944e-03	7
2		min	188	6	856	4	707	5	-2.531e-02	6	-3.152e-03	5	6.542e-04	5
3	N2	max	.151	3	.036	6	.645	6	1.415e-02	5	2.123e-02	6	2.897e-03	7
4	717	min	254	6	856	4	142	5	-1.673e-02	6	-3.225e-03	5	5.785e-04	5
5	N3	max	.178	3	.043	6	1.449	6	1.422e-02	5	2.163e-02	6	2.944e-03	7
6		min	188	6	848	4	697	5	-2.531e-02	6	-3.152e-03	5	6.541e-04	5
7	N4	max	.151	3	.043	6	.581	6	1.415e-02	5	2.123e-02	6	2.897e-03	7
8		min	254	6	848	4	133	5	-1.673e-02	6	-3.225e-03	5	5.784e-04	5
9	N5	max	.22	3	.043	6	1.876	6	1.421e-02	5	2.163e-02	6	3.035e-03	4
10		min	148	6	848	4	925	5	-2.697e-02	6	-3.152e-03	5	6.966e-04	5
11	N6	max	.14	3	.043	6	.335	6	1.416e-02	5	2.123e-02	6	2.899e-03	7
12	The state of	min	292	6	848	4	.039	1	-1.507e-02	6	-3.225e-03	5	5.358e-04	5
13	N7	max	.151	3	.14	6	.045	3	1.428e-02	5	8.124e-03	6	3.082e-03	7
14		min	253	6	704	4	057	6	-1.681e-02	6	-3.558e-03	8	1.281e-03	5
15	N8	max	0	8	0	8	0	8	7.844e-03	5	8.858e-03	3	6.207e-03	6
16		min	0	1	0	1	0	1	-9.75e-04	6	-6.093e-04	7	-4.436e-04	5
17	N9	max	.178	3	.149	6	.704	6	1.444e-02	5	9.139e-03	6	2.266e-03	7
18		min	188	6	691	4	567	5	-2.143e-02	6	-3.066e-03	8	9.328e-04	5
19	N10	max	.151	3	.149	6	.026	3	1.43e-02	5	5.238e-03	6	2.353e-03	7
20		min	253	6	691	4	087	6	-1.682e-02	6	-3.613e-03	8	1.007e-03	5
21	N11	max	.133	3	.077	6	.75	6	1.388e-02	5	1.027e-02	3	2.309e-03	6
22		min	141	6	624	4	549	5	-1.941e-02	6	-1.06e-02	6	8.493e-05	5
23	N12	max	.112	3	.077	6	.019	8	1.375e-02	5	8.737e-03	3	1.711e-03	4
24		min	189	6	624	4	022	6	-1.779e-02	6	-1.478e-02	6	3.384e-04	8
25	N17	max	.022	3	152	6	.865	6	1.426e-02	5	5.892e-03	3	6.857e-04	6
26		min	025	6	448	4	507	5	-1.847e-02	6	-6.817e-03	6	-2.639e-04	5
27	N18	max	.019	3	152	6	.137	6	1.372e-02	5	4.848e-03	3	4.912e-04	3
28		min	03	6	447	4	104	3	-1.799e-02	6	-8.895e-03	6	-9.992e-05	8
29	N19	max	.003	3	228	6	.89	6	1.433e-02	5	2.97e-03	3	2.963e-04	3
30		min	003	8	387	4	5	5	-1.834e-02	6	-4.771e-03	6	-4.297e-04	8
31	N20	max	.003	3	228	3	.169	6	1.38e-02	5	2.229e-03	3	2.414e-04	3
32		min	0	1	389	5	12	3	-1.765e-02	6	-5.583e-03	6	-1.554e-04	8
33	N21	max	.003	3	223	3	.969	6	1.433e-02	5	2.97e-03	3	2.963e-04	3
34		min	003	8	393	8	484	5	-1.834e-02	6	-4.771e-03	6	-4.297e-04	8
35	N22	max	.003	3	224	3	.261	6	1.38e-02	5	2.229e-03	3	2.414e-04	3
36		min	0	1	391	5	157	3	-1.765e-02	6	-5.583e-03	6	-1.554e-04	8
37	N23	max	.022	3	148	6	.995	6	1.377e-02	5	5.889e-03	3	-3.367e-06	3
38	N 11-5	min	025	6	456	5	477	4	-1.793e-02	6	-7.147e-03	6	-6.473e-04	5
39	N24	max	.019	3	149	6	.293	6	1.385e-02	5	4.895e-03	3	2.355e-04	6



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Envelope Joint Displacements (Continued)

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation [rad]	LC	Y Rotation [rad]	LC	Z Rotation [rad]	LC
40		min	031	6	455	5	172	3	-1.762e-02	6	-9.124e-03	6	-9.139e-04	8
41	N25	max	.133	3	.077	6	1.115	6	1.379e-02	4	1.028e-02	3	-4.753e-04	6
42	- 17	min	144	6	644	5	464	4	-1.767e-02	6	-1.058e-02	6	-2.959e-03	5
43	N26	max	.113	3	.077	6	.454	6	1.374e-02	4	8.825e-03	3	3.766e-04	6
44		min	19	6	644	5	266	3	-1.692e-02	6	-1.455e-02	6	-2.938e-03	5
45	N27	max	.178	3	.151	6	1.161	6	1.426e-02	4	6.911e-03	3	-4.841e-04	6
46		min	19	6	719	5	466	3	-1.793e-02	6	-2.525e-02	6	-4.362e-03	5
47	N28	max	.152	3	.15	6	.517	6	1.421e-02	4	6.407e-03	3	-5.042e-04	3
48		min	254	6	719	5	305	3	-1.552e-02	6	-2.622e-02	6	-4.363e-03	8
49	N29	max	.178	3	.119	6	2.623	6	1.435e-02	4	6.611e-03	3	1.413e-04	6
50		min	19	6	-1.019	5	75	3	-1.989e-02	6	-3.896e-02	6	-5.25e-03	5
51	N30	max	.152	3	.119	6	1.985	6	1.435e-02	4	6.605e-03	3	1.865e-04	6
52	1100	min	253	6	-1.019	5	582	3	-1.242e-02	6	-3.872e-02	6	-5.283e-03	5
53	N33	max	.178	3	.12	6	2.74	6	1.435e-02	4	6.611e-03	3	1.412e-04	6
54	1100	min	19	6	-1.035	5	77	3	-1.989e-02	6	-3.896e-02	6	-5.25e-03	5
55	N34	max	.152	3	.12	6	2.101	6	1.435e-02	4	6.605e-03	3	1.865e-04	6
56	1404	min	253	6	-1.035	5	602	3	-1.242e-02	6	-3.872e-02	6	-5.283e-03	5
57	N35	max	.003	3	226	3	.215	6	1.38e-02	_	2.229e-03	_		
58	NOO	min	0	1	39	5	138	3	-1.765e-02	5		3	2.414e-04	3
59	N36	max	.003	3	226	3	.929	_		6	-5.583e-03	6	-1.554e-04	8
60	1430	min	003	8	39	5	492	5	1.433e-02	5	2.97e-03	3	2.963e-04	3
61	N35A	max	.178	3	.156	6	.662	-	-1.834e-02	6	-4.771e-03	6	-4.297e-04	8
62	NOOA	min	188	6	681	4	551	6	1.441e-02	5	6.117e-03	3	1.45e-03	7
63	N36A	max	.151	_		_		5	-2.113e-02	6	-3.237e-03	8	5.681e-04	5
64	NOON	min		3	.156	6	.025	5	1.429e-02	5	4.607e-03	3	1.512e-03	7
	N37		253	6	681	4	108	6	-1.682e-02	6	-3.445e-03	8	6.149e-04	5
65 66	INOT	max	.186	3	.119	6	2.962	6	1.434e-02	4	6.611e-03	3	5.935e-04	3
67	N38		188 .178	6	-1.019	5	815	3	-2.149e-02	6	-3.896e-02	6	-5.207e-03	8
68	INOO	max		3	.119	6	1.807	6	1.436e-02	4	6.605e-03	3	1.866e-04	6
	NICO	min	256	6	-1.019	5	516	3	-1.083e-02	6	-3.872e-02	6	-5.327e-03	5
69	N39	max	.178	3	.152	6	1.029	6	1.417e-02	4	6.963e-03	3	-1.177e-04	6
70	NIAO	min	19	6	699	5	464	4	-1.772e-02	6	-2.169e-02	6	-2.71e-03	5
71	N40	max	.152	3	.152	6	.379	6	1.413e-02	4	6.349e-03	3	-7.876e-05	3
72	NIAA A	min	253	6	699	5	269	3	-1.585e-02	6	-2.295e-02	6	-2.711e-03	5
73	N41A	max	.168	3	.152	6	1.314	6	1.417e-02	4	6.963e-03	3	-1.177e-04	6
74	NI 4OA	min	191	6	699	5	69	4	-1.779e-02	6	-2.169e-02	6	-2.698e-03	5
75	N42A	max	.154	3	.152	6	.391	7	1.413e-02	4	6.349e-03	3	-1.492e-04	3
76	NIAAD	min	251	6	699	5	208	3	-1.578e-02	6	-2.295e-02	6	-2.723e-03	5
77	N41B	max	.195	3	.156	6	1.008	6	1.44e-02	5	6.117e-03	3	1.49e-03	4
78	NIAOD	min	172	6	681	4	781	5	-2.173e-02	6	-3.237e-03	8	5.901e-04	5
79	N42B	max	.142	3	.156	6	.253	5	1.43e-02	5	4.607e-03	3	1.513e-03	7
80		min	271	6	681	4	37	6	-1.622e-02	6	-3.445e-03	8	5.245e-04	3
81	N41	max	0	8	0	8	0	8	1.376e-02	5	6.61e-03	7	1.217e-02	8
82	and the last	min	0	1	0	1	0	1	-1.767e-02	6	-4.598e-03	3	6.91e-03	3
83	N42	max	0	8	0	8	0	8	1.426e-02	5	2.754e-02	6	1.231e-02	8
84	Addition to the	min	0	1	0	1	0	1	-1.824e-02	6	-1.22e-02	4	6.984e-03	3
85	N43	max	0	8	0	8	0	8	1.376e-02	5	2.775e-03	3	-7.176e-03	6
86	19-7-7	min	0	1	0	1	0	1	-1.767e-02	6	-4.594e-03	7	-1.218e-02	5
87	N44	max	0	8	0	8	0	8	1.426e-02	5	1.308e-02	5	-7.107e-03	6
88		min	0	1	0	1	0	1	-1.824e-02	6	-2.343e-02	6	-1.209e-02	4
89	N45	max	0	8	226	3	.316	5	1.376e-02	5	1.845e-03	3	3.114e-04	3
90	Williams	min	003	3	39	5	103	6	-1.767e-02	6	-4.622e-03	6	7.126e-06	8
91	N46	max	.008	3	226	3	1.26	6	1.433e-02	5	2.97e-03	3	2.963e-04	3



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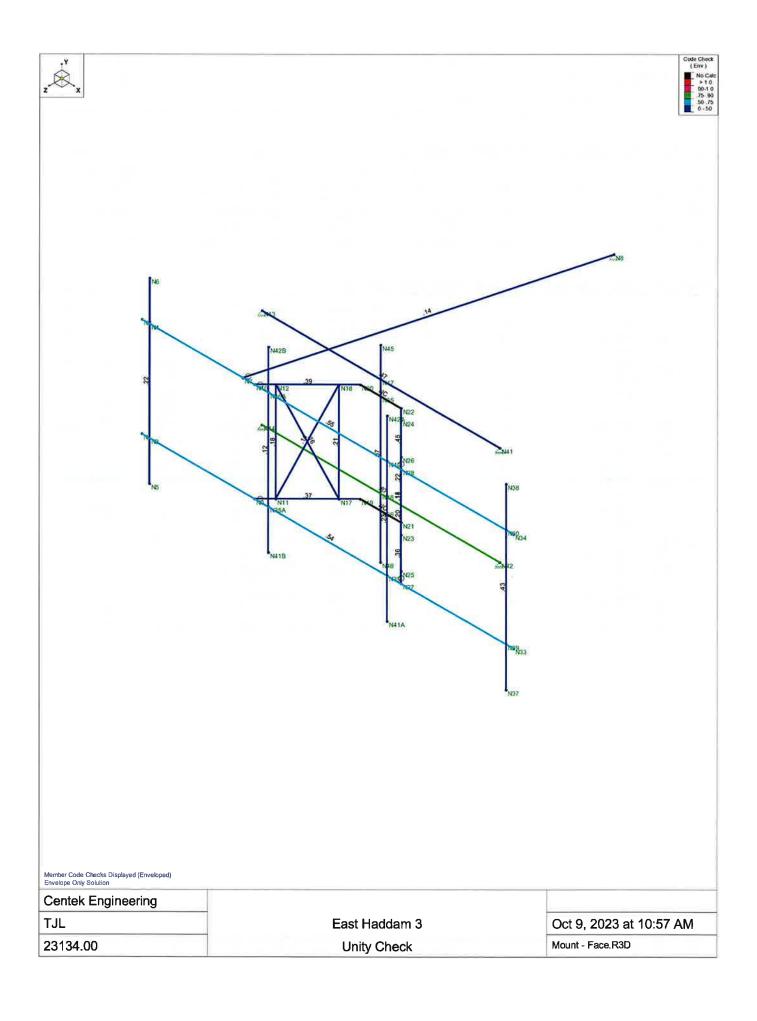
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Envelope Joint Displacements (Continued)

	Joint		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation [rad]	LC	Y Rotation [rad]	LC	Z Rotation [rad]	LC
92		min	011	8	39	5	75	5	-1.834e-02	6	-4.771e-03	6	-4.297e-04	8
93	N47	max	0	5	226	3	.18	7	1.376e-02	5	1.845e-03	3	3.114e-04	3
94		min	0	1	39	5	118	3	-1.767e-02	6	-4.622e-03	6	7.126e-06	8
95	N48	max	0	3	226	3	.819	6	1.426e-02	5	2.43e-03	3	1.634e-04	3
96		min	0	8	39	5	406	5	-1.824e-02	6	-4.158e-03	6	-2.869e-04	8

Envelope AISC 15th(360-16): LRFD Steel Code Checks

	Mem	Shape	Code Check	L	LC	Sh	Loc[ft]	Dir	.phi*P	phi*P	phi*Mn y-y [k-ft]	phi*Cb Eqn
1	M1	PIPE 2.5	.550	3	6	096	8.724	6		50.715	3.596	3.5 1 H1
2	M2	PIPE 2.5	.536	8	6		3.776		14.559	50.715	3.596	3.5 1H1
3	МЗ	PIPE 2.0	.137	5	3	.009	0	3	9.492	32.13	1.872	1.8 1H1
4	M4	PIPE 2.0	.392	2	6	.095	2.521	4	32.032	32.13	1.872	1.8 1 H1
5	M5	PIPE 2.0	.368	2	3	123	0	6	32.032	32.13	1.872	1.8 1 H1
6	M6	PIPE 2.0	.446	2	6	168	2.521	8	32.032	32.13	1.872	1.8 1 H1
7	M7	PIPE 2.0	.361	2	3	144	.499	8	32.032	32.13	1.872	1.8 1H1
8	M8	0.625' Dia.	.181	0	6	.026	0	6	1.058	9.94	.104	.104 1H1
9		0.625' Dia.	.214	3	6	016	3.333	6	1.058	9.94	.104	.104 2H1
10	M10	SR 3/4	.158	0	6	.042	0	6	6.954	14.314	.179	.179 2H1
11		0.625' Dia.	.197	3	6	.025	3.333	6	1.058	9.94	.104	.104 2H1
12	M12	SR 3/4	.136	3	3	.029	3.659	3	6.954	14.314	.179	.179 2H1
13		0.625' Dia.	.218	3	6	016	3.333	6	1.058	9.94	.104	.104 1H1
14	M14	SR 3/4	.144	3	6	.036	3.659	6	6.954	14.314	.179	.179 2H1
15	M15	SR 3/4	.132	0	3	.030	0	3	6.954	14.314	.179	.179 1 H1
16	PS.2		.223	1	4	.051	4.688	6	20.867	32.13	1.872	1.8 1 H1
17	PS.1	PIPE 2.0	.435	1	8	.055	4.625	5	20.867	32.13	1.872	1.8 1H1
18	M19	PIPE 2.0	.226	1	6	.054	1.375	6	20.867	32.13	1.872	1.8 1H1
19	M21A		.118	1	5	.068	4.625	6	20.867	32.13	1.872	1.8 1 H1
20	M22	PIPE 4.0	.070	1	5	231	1.056	6	82.012	93.24	10.631	102H1
21		HSS2.5X2	.467	4	7	.020	0	y 4	43.695	97.29	6.486	6.4 1 H1
22		HSS2.5X2	.874	4	6	.049	4	z 6	43.695	97.29	6.486	6.4 1 H1







NORTHEAST > North East > New England > Wallingford-1 > EAST HADDAM 3 CT - A

Cheiban, Ziad - ziad.cheiban@verizonwireless.com - 20230801_101321

Project Details		Location Information	
Carrier Aggregation N	N	Site Id	Site (d 2609730
Ecip N	Z	Search Ring#	
Project Name	Project Name EAST HADDAM 3 CT - NEW BUILD	E-NodeB ID# null	null
Project Alt Name	Project Alt Name EAST HADDAM 3 CT - NEW BUILD	PSLC# 300023	300023
Project Id	Project Id 17133983	Switch Name Wallingford-1	Wallingford-1
Designed Sector Carrier 4G 16	16	Tower Type	
Designed Sector Carrier 5G	4	Site Type MACRO	MACRO
Additional Sector Carrier 4G 0	0	Street Address	Street Address 194 Mt Pamassus Road
Additional Sector Carrier 5G 0	0	City	City East Haddam
Suffix	Suffix Rev1_2023-08-01	State CT	ст
FP Solution Type & Tech Type	FP Solution Type & Tech Type MCR;4G_700;5G_850;4G_850;4G_AWS;5G _L-Sub6;4G_PCS	Zip Code 06423	06423
		County	County Middlesex
The state of the s		Latitude	Latitude 41.47003/ 41° 28' 12.108"
		Longitude	Longitude -72 41120/ 72° 24' 40 320"

Project Scope				
New Build Macro Extension on Existing Tower				
Rev1_2023-08-01: Added 4th sector. Adjusted ACL to 145'				

					A	Antenna Summary					
4dded Antenna	ntenna										
002	850	1900	AWS	L-Sub6 Make	Маке	Medel	Centeri	Tip Height	Azimuth	Install Type	Quantity
				5G	Samsung	MT6413-77A	145	146.2	345(A),75(B),16 5(C),255(D)	PHYSICAL	4
LTE	LTE	LTE	LTE	I	CommScope	NHH-45B-R2B	145	148	345(A),75(B),16 5(C),255(D)	PHYSICAL	80

	20
	Quantif
	Install
	clmuth
	4
	Tip Height
	Senteri ne
	0.2
	100
	Nodel
	Make
	L-Sub6
	AWS
	1900
oved Antenna	850
moved A	
Rei	700

	Quantity
	Install Type
	Azimuth
	Tip Helght
	Centerl
	Model
	Make
	L-Sub6 Make
	AWS
	1900
Antenna	850
Retained.	200

Added: 12	Removed: 0	Retained: 0
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						Non Antenna Summary			
Added Non Antenna	ntenna								
Equipment Type	Location 700	700	850	1900	AWS	Make	Model	Install Type	Quantity
RRU	Tower			LTE	TTE	Samsung	B2/B66A RRH ORAN (RF4439d-25A)	PHYSICAL	4
RRU	Tower	LTE	<u> </u>			Samsund	RF4461d-13A	PHYSICAL	4

Equipment Location 700 850 1900 AWS Make Model Install Type	Removed Non	Antenna									
	Equipment Type	Location	002	850	1800	AWS	Make	Model	Install Type	I Quantit	fy

	stall Quantity pe
	Model
	Make
	AWS
	1900
	850
	200
Intenna	Location
Retained Non A	Equipment Type

Retained: 0

Services				
1900 LTE		00	0002	
Sector	01	02	03	04
Azimuth	345	75	165	255
Cell/Enodeb-ld	064182	064182	064182	064182
Antenna Model	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B
Antenna Make	CommScope	CommScope	CommScope	CommScope
Centerline	145	145	145	145
DLEARFCN	1050	1050	1050	1050
Mech Down-tilt	0	0	0	0
Elect Down-tilt	0	0	0	0
Tip Height	148	148	148	148
Regulatory Power	407.15 (W/MHz) EIRP	407.15 (W/MHz) EIRP	407.15 (W/MHz) EIRP	407.15 (W/MHz) EIRP
Cell Max Power	46.0 dBm	46.0 dBm	46.0 dBm	46.0 dBm
TMA Make				
TMA Model				
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)
Number of Tx,Rx	4,4	4,4	4,4	4 , 4
Position	1,5	1,5	1,5	1,5
Transmitter Id	18843902	18843906	18843910	18843914
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	10	10	10	10
Ant. Dimensions H x W x D(inch)	72.0 × 18.0 × 7.0	72.0 x 18.0 x 7.0	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0
Weight(Ib)	73.6	73.6	73.6	73.6

Services				
700 LTE		00	0002	
Sector	01	02	03	04
Azimuth	345	75	165	255
Cell/Enodeb-ld	064182	064182	064182	064182
Antenna Model	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B
Antenna Make	CommScope	CommScope	CommScope	CommScope
Centerline	145	145	145	145
DLEARFCN	5230	5230	5230	5230
Mech Down-tilt	0	0	0	0
Elect Down-tilt	2	2	2	2
Tip Height	148	148	148	148
Regulatory Power	109.59 (W/MHz) ERP	109.59 (W/MHz) ERP	109.59 (W/MHz) ERP	109.59 (W/MHz) ERP
Cell Max Power	46.0 dBm	46.0 dBm	46.0 dBm	46.0 dBm
TMA Make				
TMA Model				
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	RF4461d-13A	RF4461d-13A	RF4461d-13A	RF4461d-13A
Number of Tx,Rx	4,4	4,4	4,4	4 , 4
Position	1,5	1,5	1,5	1,5
Transmitter Id	18843901	18843905	18843909	18843913
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	10	10	10	10
Ant. Dimensions H x W x D(inch)	72.0 × 18.0 × 7.0			
Weight(lb)	73.6	73.6	73.6	73.6

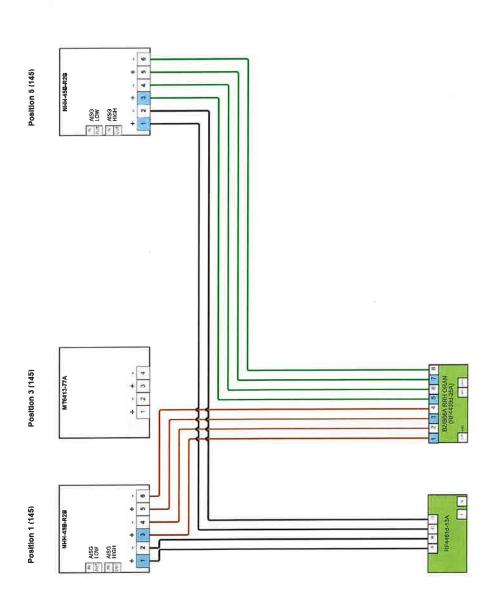
Services				
850 LTE		0	0002	
Sector	10	02	03	04
Azimuth	345	75	165	255
Cell/Enodeb-Id	064182	064182	064182	064182
Antenna Model	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B
Antenna Make	CommScope	CommScope	CommScope	CommScope
Centerline	145	145	145	145
DLEARFCN	2450	2450	2450	2450
Mech Down-tilt	0	0	0	0
Elect Down-tilt	2	2	2	2
Tip Height	148	148	148	148
Regulatory Power	395.20 (W/MHz) ERPSD	395.20 (W/MHz) ERPSD	395.20 (W/MHz) ERPSD	395.20 (W/MHz) ERPSD
Cell Max Power	44.5 dBm	44.5 dBm	44.5 dBm	44.5 dBm
TMA Make				
TMA Model				
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	RF4461d-13A	RF4461d-13A	RF4461d-13A	RF4461d-13A
Number of Tx,Rx	4,4	4,4	4 . 4	4,4
Position	1,5	1,5	1,5	1,5
Transmitter Id	18843904	18843908	18843912	18843916
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	10	10	10	10
Ant. Dimensions H x W x D(inch)	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0
Weight(lb)	73.6	73.6	73.6	73.6

Services				
AWS LTE)0	0002	
Sector	01	02	03	04
Azimuth	345	75	165	255
Cell/Enodeb-Id	064182	064182	064182	064182
Antenna Model	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B	NHH-45B-R2B
Antenna Make	CommScope	CommScope	CommScope	CommScope
Centerline	145	145	145	145
DLEARFCN	2050	2050	2050	2050
Mech Down-tilt	0	0	0	0
Elect Down-tilt	0	0	0	0
Tip Height	148	148	148	148
Regulatory Power	228.42 (W/MHz) EIRP	228.42 (W/MHz) EIRP	228.42 (W/MHz) EIRP	228.42 (W/MHz) EIRP
Cell Max Power	46.0 dBm	46.0 dBm	46.0 dBm	46.0 dBm
TMA Make				
TMA Model				
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)	B2/B66A RRH ORAN (RF4439d-25A)
Number of Tx,Rx	4,4	4,4	4 , 4	4,4
Position	1,5	1,5	1,5	1,5
Transmitter Id	18843903	18843907	18843911	18843915
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	20	20	20	20
Ant. Dimensions H x W x D(inch)	72.0 x 18.0 x 7.0	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0	72.0 × 18.0 × 7.0
Weight(ib)	73.6	73.6	73.6	73.6

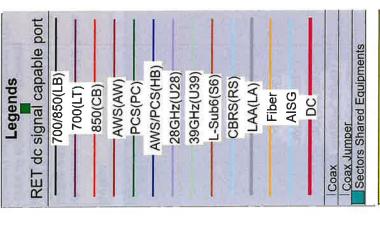
Services				
CBAND NR		0	0002	
Sector	10001	0002	0003	0004
Azimuth	345	75	165	255
Cell/Enodeb-ld	640175	640175	640175	640175
Antenna Model	MT6413-77A	MT6413-77A	MT6413-77A	MT6413-77A
Antenna Make	Samsung	Samsung	Samsung	Samsung
Centerline	145	145	145	145
DLEARFON	650006	920009	650006	650006
Mech Down-tilt	0	0	0	0
Elect Down-tilt	7	2	2	2
Tip Height	146.2	146.2	146.2	146.2
Regulatory Power	748.75 (W/MHz) EIRP	748.75 (W/MHz) EIRP	748.75 (W/MHz) EIRP	748.75 (W/MHz) EIRP
Cell Max Power	47.0 dBm	47.0 dBm	47.0 dBm	47.0 dBm
TMA Make				
TMA Model				
RRU Make	Samsung	Samsung	Samsung	Samsung
RRU Model	MT6413-77A	MT6413-77A	MT6413-77A	MT6413-77A
Number of Tx,Rx	4,4	4,4	4,4	4,4
Position	6	3	3	က
Transmitter Id	18843917	18843918	18843919	18843920
Source	VZNPP	VZNPP	VZNPP	VZNPP
Bandwidth	100	100	100	100
Ant. Dimensions H x W x D(Inch)	29.53 x 15.75 x 5.51	29.53 × 15.75 × 5.51	29.53 x 15.75 x 5.51	29.53 x 15.75 x 5.51
Weight(lb)	55.1	55.1	55.1	55.1

Calisigns Per Antenna	Amenina																		
Sector	Make	Model	Ant CL Height AG	Ant Tip Height	Azimuth	Elect N Down-tilt D	Mech Down-tilk	Gain	Bandwidth y	Regulator 700 y Power		920	1900	2100	28 GHz	31 GHz 3	38 GHz	LSub-8	CBRS
05	CommScope	NHH-45B-R2	145	148	75	0	0	17.9	41 2	228 42				WQGA906,WQ GB276					
02	CommScope	NHH-45B-R2	145	148	75	2 0	0	14,07	49	109 59 WC	WQJQ689								
-	e	NHH-45B-R2 145	145			2 0	0	14.07			WQJQ689								
£000		MT6413-77A 145	145	146.2	165	8	0	23,35	105	748.75								WRNE581,WR NE582,WRNE 583,WRNE58 4,WRNE585	
10	CommScope	NHH-45B-R2	145	148	345	0	0	17.6	43	407,15			KNLH251,WP 0J730						
03	CommScope	NHH-45B-R2	145	148	165	2 0	0	15.08	43	395.2	×	KNKA404							
0.1		NHH-45B-R2	145	148		2 0	0 1			395.2	×	KNKA404							
04	CommScope	NHH-45B-R2	145	148	255	0	0	17.9	41	228 42				WGGA906,WG GB276					
04	CommScope	NHH-45B-R2	145	148	255	0	0	17.6	43	407.15			KNLH251,WP OJ730						
02	CommScope	NHH-45B-R2	145	148	75	2	0	15.08	43	395.2	×	KNKA404							
10	СоттЅсоре	NHH-45B-R2	145	148	345	0	1 0	17.9	41	228 42				WGGA906,WG GB276					
03	CommScope	NHH-45B-R2	145	148	165	0	0	17.9	41	228 42				WQGA906,WQ GB276					
101	CommScope	CommScope NHH-45B-R2 145	145	148	345	2		14.07	49	109.59 W	WQJQ689								
04	CommScope	CommScope NHH-45B-R2	145	148	255	2	0	14.07		109.59 W	WQJQ689								
0004	Samsung	MT6413-77A	145	146.2	255	2	0	23,35	105	748,75								WRNE581,WR NE582,WRNE 583,WRNE58 4,WRNE585	
03	CommScope	NHH-45B-R2	145	148	165	0	0	17.6	43	407.15			KNLH251,WP OJ730						
0005	Samsung	MT6413-77A	145	146.2	75	2	0	23.35	105	748.75								WRNE581,WF NE582,WRNE 583,WRNE56 4,WRNE585	
0001	Samsung	MT6413-77A	145	146,2	345	2	0	23,35	105	748.75								WRNE581,WR NE582,WRNE 583,WRNE58 4,WRNE585	
05	CommScope	NHH-45B-R2	145	148	75	0	0	17.6	43	407,15			KNLH251,WP OJ730						
04	CommScope	CommScope NHH-45B-R2 145	145	148	255	2	0	15,08	43	395.2	-	KNKA404							

Callsigns																			
Celleign	Market	Radio	Market *	Block	State	County	License Name	Wholly Owner	Total MHZ	Freq Range 1	Freq Range 2	Freq Range 3	Freq Range 4	Regulator y Power	Threshold (W)	POPs/Sq.	Status	Action	Approve for Insvc
WQJQ689	Northeast	wu	REA001	υ	- CT	9007	Cellco Partnershi	Yes	22 000	746.000 - 757.000/.0 00000	776 000 - 77 787 000 00 00 00 00 00 00 00 00 00 00 00 0	746 000 - 757 000/ 0 00 - 000	776,000 - 000,000,000 000 - 000	109,59	1000	444.75	proposed	added	
KNKA404	Hartford-N ew Britain-Br Istol, CT	ರ	CMA032	ď	CT.	2002	Cellco Partnershi	Yes	25,000	824 000 - 835 000/84 5 000 - 846 500	869.000 - 880.000/89 8 0 000 - 6 891.500	824 000 - 835 000/84 5 000 - 846 500	869 000 - 880 000/89 0 000 - 891 500	395,2	400	444.75	proposed	added	
WPOJ730	Harfford, CT	cw	BTA184	o	ст	2006	Cellco Partnershi	Yes	10.000	1895,000 - 1900,000/ 000 - 000	1975.000 - 1980.000/ 1	1895,000 1900,000/ 000 - ,000	1975.000 - 1980.000/ 000000	407.15	1640	444.75	proposed	added	
KNLH251	Hartford, CT	cw	BTA184	ti.	ст	2006	Cellco Partnershi	Yes	10,000	1890,000 - 1895,000/ 000 - 000	1970 000 - 1975 000/ 000 - 000	1890,000 - 1895,000/ 000 - ,000	1970 000 - 1975 000/ 000 - 000	407.15	1640	444.75	proposed	added	
WQGB276	Hartford-N ew Britain-Br Istot, CT	AW	CMA032	⋖	CT	9007	Cellco Partnershi	Yes	20,000	1710,000 - 1720,000/ 000 - 000	2110 000 - 1 2120 000/ 1 000 - 000 0	1710.000 - 1720.000/ 000000	2110.000 - 2120.000/ 000000	228 42	1640	444,75	proposed	added	
WRNE581	New York, NY	PM	PEA001	A1	ст	9007	Cellco Partnershi	Yes	20 000 3	3700,000 - 3720,000, 000 - 000	000 000 000 000 000 000 000 000 000	3700 000 - 3720 000/ 000 - 000	000 - 000	748.75	1640	444,75	proposed	added	
WRNE582	New York, NY	PM	PEA001	A2	CT	2006	Celico Partnershi	Yes	20.000 3	3720,000 - 3740,000/, 000 - 000	000 000	3720,000 - 3740,000/ 000 - 000	000 - 000 - 000 -	748,75	1640	444.75	proposed	added	
WRNE583	New York, NY	PM	PEA001	Аз	ст	5008	Cellco Partnershi	Yes	20.000 3	3740,000 - 3760,000/, 000 - 000	000 - 000	3740 000 - 3760 000/ 000 - 000	000 -	748.75	1640	444.75	proposed	added	
WRNE584	New York, NY	Md	PEA001	A4	ст	2006	Cellco Partnershi	Yes	20,000 3	3760,000 - 3780,000/, 000 - 000	000 000 3	3760,000 - 3780,000/ 000 - 000	000 - 000 - 000	748,75	1640	444,75	proposed	0 pappe	
WRNE585	New York, NY	М	PEA001	A5	ст	2008	Cellco Partnershi	Yes	20.000	3780,000 - 3800,000/ 000 - 000	000 - 000	3780,000 - 3800,000/ 000 - 000	000 -	748.75	1640	444.75	proposed	o pappe	
WQGA906	New York-No. New JerLong Island, NY-NJ-CT-P A-MA-	AW.	BEA010		OT.	2005	Partnershi	Yes	20.000	1720,000 - 1730,000, 2 000 - 000 000 - 000	2120.000- 2130.000/. 1 000000	1720,000 - 1730,000 - 000 - 000 - 000 000 - 000	2120,000 - 2130,000/, 000000	228,42	1640	444.75	proposed	added	



Alpha (Proposed)



Notes:

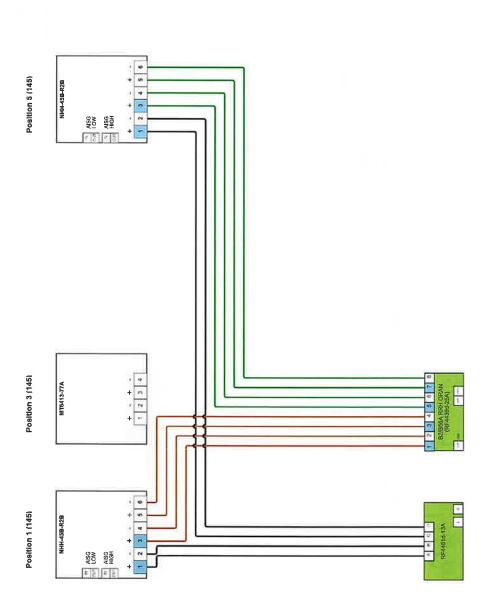
-Antenna view is from the back of the antennas
-Colors of connections are just for clarification
-Size of objects in drawing doesn't reflect equipment true dimensions

Sector design

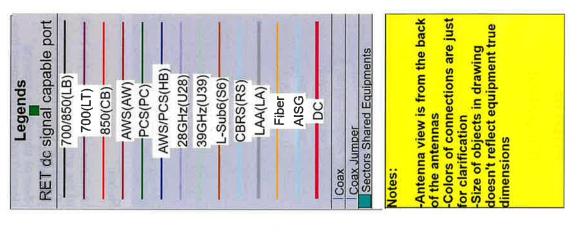
Shelter

Sector design

Shelter



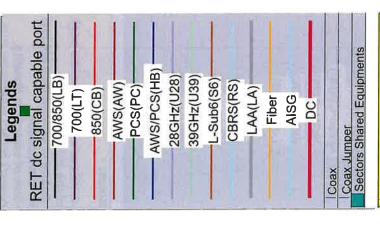
Beta (Proposed)



Position 5 (145) NHH-45B-R2B AISG AISG Position 3 (145) MT6413-77A 1 2 3 4 5 6 Position 1 (145) NHH-45B-R2B AISG AISG AISG HIGH

Gamma

(Proposed)



Notes:

-Antenna view is from the back of the antennas
-Colors of connections are just for clarification
-Size of objects in drawing doesn't reflect equipment true dimensions

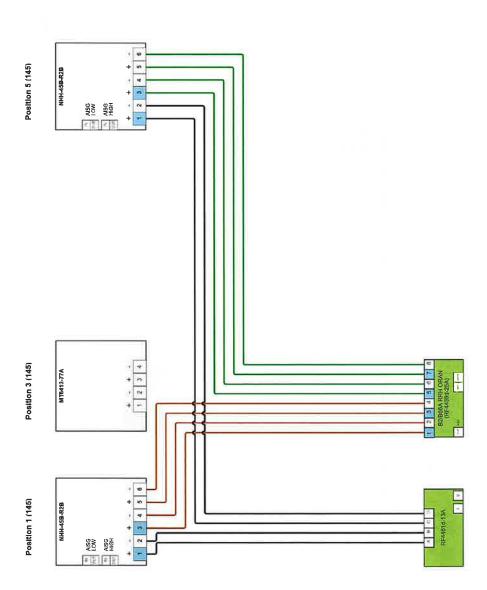
Sector design

Shelter

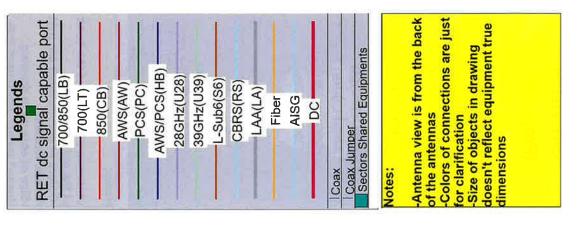
Proprietary and Confidential. Not for disclosure outside of Verizon.

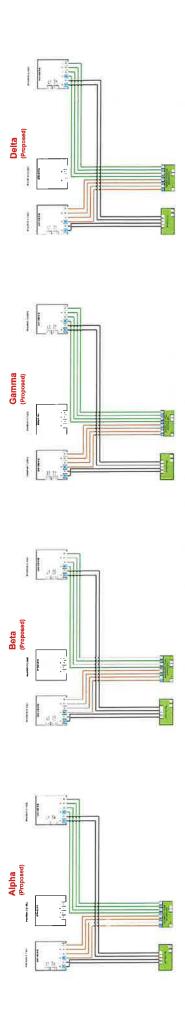
Shelter

Sector design



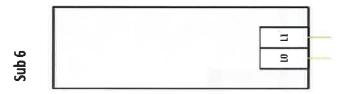
Delta (Proposed)

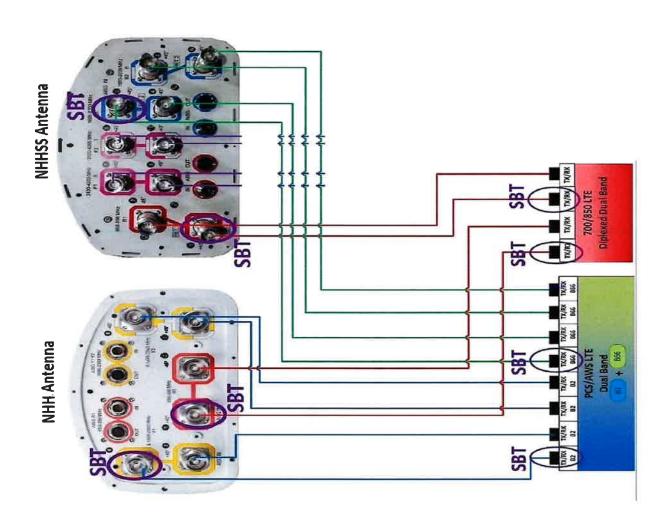




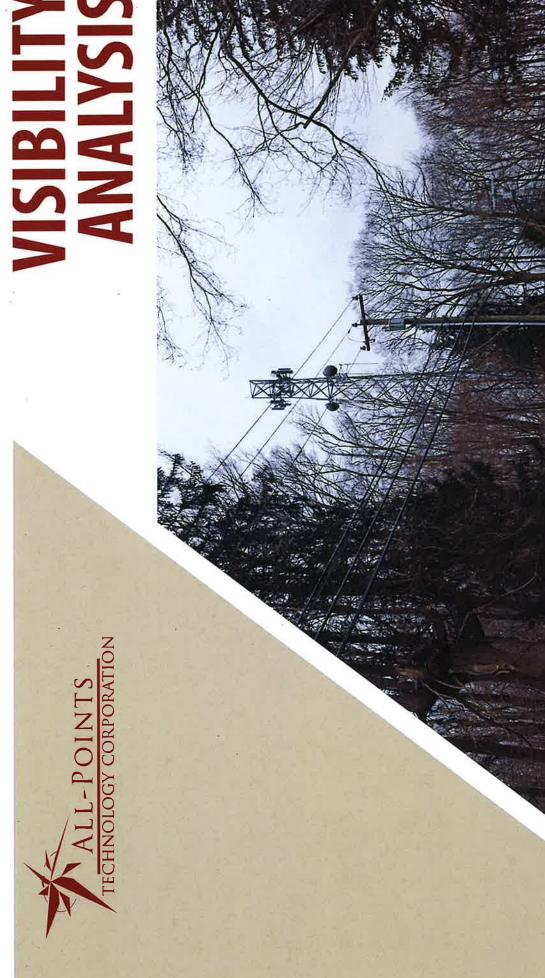
1

Proprietary and Confidential. Not for disclosure outside of Verizon.





ATTACHMENT 6



194 MOUNT PARNASSUS ROAD EAST HADDAM 3 CT EAST HADDAM, CT

PREPARED FOR:

Verizon

PREPARED BY:

567 Vauxhall Street Extension – Suite 311 Waterford, CT 06385 All-Points Technology Corporation, P.C.

VISUAL ASSESSMENT & PHOTO-SIMULATIONS

Cellco Partnership, d/b/a Verizon Wireless ("Verizon") is seeking approval for the extension of an existing wireless communications facility (the "Facility") at 194 Mount Parnassus Road in East Haddam, Connecticut (the "Host Property"). At the request of Verizon, All-Points Technology Corporation, P.C. ("APT") completed this assessment to evaluate the potential visual effects of the proposed Facility extension from within a 2-mile radius (the "Study Area").

Project Setting

The existing Facility is located on a ± 0.20 -acre property owned by the State of Connecticut on the south side of Mount Parnassus Road (the "Site"). The immediate vicinity surrounding the Host Property, and the Study Area in general, consists primarily of low-density residential development, agricultural fields, and forest. The topography within the Study Area is relatively hilly. Ground elevations range from 207 feet above mean seal level ("AMSL") at the southwestern extent of the Study Area to approximately 621 feet AMSL ± 300 feet southeast of the Site. Tree cover within the Study Area (consisting primarily of mixed deciduous hardwoods with interspersed stands of conifers) occupies approximately 6,187 acres (or $\pm 76.9\%$) of the 8,042-acre Study Area.

Project Undertaking

The existing Facility consists of a $\pm 121'$ tall self-support tower, occupied by multiple state and municipal antennas and microwave dishes, within a fenced compound located at a ground elevation of approximately 595 feet AMSL. Verizon intends to collocate antennas on the proposed 40-foot tower extension, which would increase the height of the structure to $\pm 161'$ above ground level ("AGL"). Verizon would install 12 panel antennas, eight (8) remote radio heads, and one (1) OVP junction box on four (4) sector frame mounts at an approximate centerline height of 146' AGL. Related ground equipment, including a 50-kW diesel-fueled generator with tertiary containment would be placed within the existing compound on a new 11'0" by 20'0" concrete pad.

Please refer to the Site Drawings prepared by Proterra Design Group, LLC, Rev. 0, dated February 6, 2024, and provided under separate cover, for details regarding the proposed installation.

Methodology

APT used the combination of predictive computer modeling, in-field reconnaissance, and a review of various data sources to evaluate the proposed Facility's visibility on both a

quantitative and qualitative basis. The predictive model provides a measurable assessment of visibility throughout the Study Area, including private properties and other areas inaccessible for direct observations. The in-field analysis consisted of a field reconnaissance throughout the Study Area to observe existing conditions, verify results of the model, inventory seasonal and year-round view locations, and provide photographic documentation from publicly accessible areas. A description of the procedures used in the analysis is provided below.

Preliminary Computer Modeling

To conduct this assessment, a predictive computer model was developed specifically for this project using ESRI's ArcMap GIS¹ software and available GIS data. The predictive model incorporates Project and Study Area-specific data, including the Site location, its ground elevation and both the existing Facility height and the proposed extension height, as well as the surrounding topography, existing vegetation, and structures (the primary features that can block direct lines of sight).

A digital surface model ("DSM"), capturing both the natural and built features on the Earth's surface, was generated for the extent of the Study Area utilizing State of Connecticut 2016 LiDAR² LAS³ data points. LiDAR is a remote-sensing technology that develops elevation data by measuring the time it takes for laser light to return from the surface to the instrument's sensors. The varying reflectivity of objects also means that the "returns" can be classified based on the characteristics of the reflected light, normally into categories such as "bare earth," "vegetation," "road," "surface water" or "building". Derived from the 2016 LiDAR data, the LAS datasets contain the corresponding elevation point data and return classification values. The Study Area DSM incorporates the first return LAS dataset values that are associated with the highest feature in the landscape, typically a treetop, top of a building, and/or the highest point of other tall structures.

Once the DSM was generated, ESRI's Viewshed Tool was utilized to identify locations within the Study Area where the proposed Facility extension may be visible. ESRI's Viewshed Tool predicts visibility by identifying those cells⁴ within the DSM that can be seen from an observer location. Cells where visibility was indicated were extracted and converted from a raster dataset to a polygon feature which was then overlaid onto aerial photograph and topographic base maps. Since the DSM includes the highest relative feature in the landscape, isolated "visible" cells are often indicated within heavily forested areas (e.g., from the top of the highest tree) or on building rooftops during the initial processing. It is recognized that these areas do not represent

¹ ArcMap is a Geographic Information System desktop application developed by the Environmental Systems Research Institute for creating maps, performing spatial analysis, and managing geographic data.

² Light Detection and Ranging

³ An LAS file is an industry-standard binary format for storing airborne LiDAR data.

⁴ Each DSM cell size is 1 square meter.

typical viewer locations and overstate visibility. As such, the resulting polygon feature is further refined by extracting those areas. The viewshed results are also cross-checked against the most current aerial photographs to assess whether significant changes (a new housing development, for example) have occurred since the time the LiDAR-based LAS datasets were captured.

The results of the preliminary analysis are intended to provide a comparative representation of those areas where portions of the existing and extended Facility may potentially be visible to the human eye without the aid of magnification, based on a viewer eye-height of five (5) feet above the ground and the combination of intervening topography, trees and other vegetation, and structures. However, the Facility may not necessarily be visible from all locations within those areas identified by the predictive model, which has limitations. For instance, the computer model cannot account for mass density, tree diameters and branching variability of trees, or the degradation of views that occur with distance. As a result, some areas depicted on the Viewshed Analysis maps as theoretically offering potential visibility of the extended Facility may be over-predictive because the quality of those views is not sufficient for the human eye to recognize the Facility or discriminate it from other surrounding or intervening objects.

Seasonal Visibility

Visibility also varies seasonally with increased, albeit obstructed, views occurring during "leaf-off" conditions. Beyond the variabilities associated with density of woodland stands found within any given Study Area, each individual tree also has its own unique trunk, pole timber and branching patterns that provide varying degrees of screening in leafless conditions which, as introduced above, cannot be precisely modeled. Seasonal visibility is therefore estimated based on a combination of factors including the type, size, and density of trees within a given area; topographic constraints; and other visual obstructions that may be present. Considering these variables, areas depicting seasonal visibility on the Viewshed Analysis maps are intended to represent locations from where there is a potential for views through intervening trees, as opposed to indicating that leaf-off views will exist from within an entire seasonally-shaded area.

Crane Test and Field Reconnaissance

To supplement and fine tune the results of the computer modeling efforts, APT completed infield verification activities on January 30, 2024 consisting of a crane test, vehicular and pedestrian reconnaissance, and photo-documentation. A crane was positioned at the Facility location with its boom arm extended with a brightly-colored (red) flag over the approximate centerline of the proposed extension at a height of 161' AGL⁵. APT conducted a Study Area reconnaissance by driving publicly accessible roads to inventory where the crane/flag could, and could not, be seen. Visual observations from the reconnaissance were used to evaluate the

⁵ The top of the flag represented the top of the proposed extended Facility.

results of the viewshed mapping and obtain photo-documentation from representative locations within the Study Area.

Photographic Documentation and Simulations

Photographs were taken with a Canon EOS 6D digital camera body⁶ and Canon EF 24 to 105 millimeter ("mm") zoom lens. The coordinates of the existing tower location were entered as a "waypoint" into a handheld global positioning system ("GPS") device, with the "find" tool on the GPS unit then used to provide the distance and orientation to the flag position. The geographic coordinates of each photo location were recorded as meta data using GPS technology internal to the camera.

APT typically uses a standard focal length of 50 mm to present a consistent field of view. On occasion, photos are taken at lower focal lengths to provide a greater depth of field and to provide context to the scene by including surrounding features within the photograph. Four (4) photographs presented in the attached photo-documentation were taken at a 35 mm focal length, as noted in the attached <u>Table 1 – Photo Locations</u>.

Photographic simulations were generated to portray scaled renderings of the proposed Facility from 15 locations presented herein where the Facility may be recognizable above or, seasonally, through the trees. Using field data, Site plan information and 3-dimensional (3D) modeling software, spatially referenced models of the Site and Facility were generated and merged. The geographic coordinates obtained in the field for the photograph locations were incorporated into the model to produce virtual camera positions within the spatial 3D model. Photo-simulations were then created using a combination of renderings generated in the 3D model and photorendering software programs, which were ultimately composited and merged with the existing conditions photographs (using Adobe Photoshop image editing software). The scale of the subjects in the photograph (the boom/flag) and the corresponding simulation (the extended Facility) is proportional to their surroundings.

Photo-documentation of the field reconnaissance and photo-simulations of the proposed extended Facility are presented in the attachment at the end of this report. The field reconnaissance photos that include the boom/flag in the view provide visual reference points for the approximate height and location of the proposed extended Facility relative to the scene. The corresponding photo-simulations depict the proposed tower extension and Verizon appurtenances and provide the reader with representation of the Facility when viewed from various locations. Photographs were taken from publicly accessible areas and unobstructed view lines were chosen wherever possible. For presentation purposes in this report, the photographs were produced in an approximate 7-inch by 10.5-inch format. When reproducing the images in

⁶ The Canon EOS 6D is a full-framed camera which includes a lens receptor of the same size as the film used in 35 mm cameras. As such, the images produced are comparable to those taken with a conventional 35 mm camera.

this format size, we believe it is important to present the largest view while providing key contextual landscape elements (existing developments, street signs, utility poles, etc.) so that the viewer can determine the proportionate scale of each object within the scene.

<u>Table 1 – Photo Locations</u> summarizes the photographs and simulations presented in the attachment to this report, and includes a description of each location, view orientation, distance from where the photo was taken relative to the Site, and the general characteristics of the view. The photo locations are depicted on the photolog and Viewshed Analysis maps provided as attachments to this report.

Final Visibility Mapping

Information obtained during the field reconnaissance was incorporated into the mapping data layers, including observations of the field reconnaissance, the photograph locations, areas that experienced recent land use changes and those places where the initial model was found to over or under-predict visibility. Once the additional data was integrated into the model, APT recalculated the visibility of the proposed extended Facility within the Study Area.

Conclusions

The existing Facility is not highly visible today beyond the immediate vicinity of the Site (0.25 mile or less). The tower's open steel weave design combined with rolling terrain and substantial tree cover throughout the Study Area serve to minimize its visibility. Extending the tower by 40 feet will make the Facility more prominent in some locations where it is currently visible, as it will rise farther above the tree line. However, no new areas of visibility are created by the proposed extension, with the exception of elevated agricultural fields near the western edge of the Study Area at distances approaching two miles away.

As presented on the attached comparative Viewshed Analysis maps, predicted year-round visibility is estimated to increase from ± 12 acres to ± 23 acres and would account for $\pm 0.28\%$ of the Study Area. Seasonal visibility of the proposed extended Facility is estimated to be approximately 94 acres, or $\pm 1.16\%$ of the Study Area.

Overall, the proposed Facility extension would not result in a substantial increase of visibility nor significantly alter the current characteristics of the area.

Proximity to Schools And Commercial Child Day Care Centers

No schools or commercial child day care centers are located within 250 feet of the proposed Facility. Franklin Academy is located approximately 2.28 miles to the southwest of the Site at 140 River Road in East Haddam. The nearest commercial child care center, A First Start

Preschool, is located at 499 Town Street in East Haddam, approximately 1.98 miles to the northwest of the Site. There is no predicted visibility of the proposed extended Facility from either location.

Limitations

The photo-simulations provide a representation of the Facility under similar settings as those encountered during the field review and reconnaissance. Views can change throughout the seasons and the time of day, and are dependent on weather and other atmospheric conditions (e.g., haze, fog, clouds); the location, angle and intensity of the sun; and the specific viewer location. Weather conditions on the day of the field review included overcast skies.

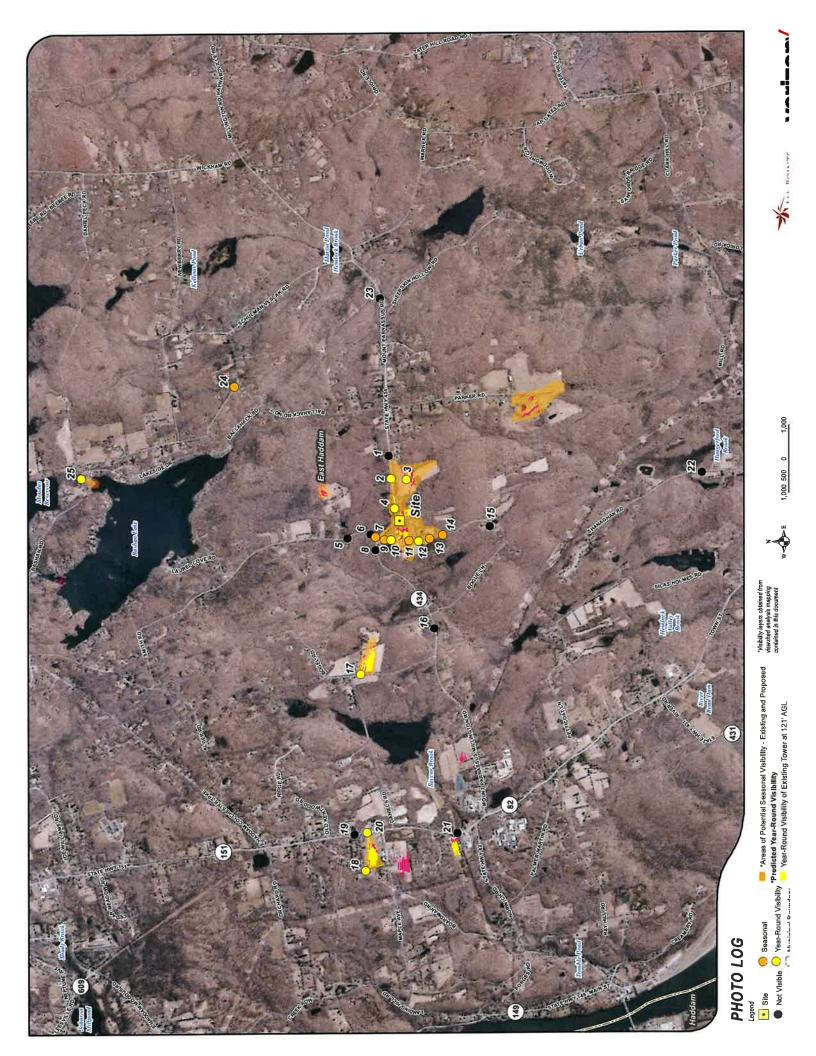
ATTACHMENTS

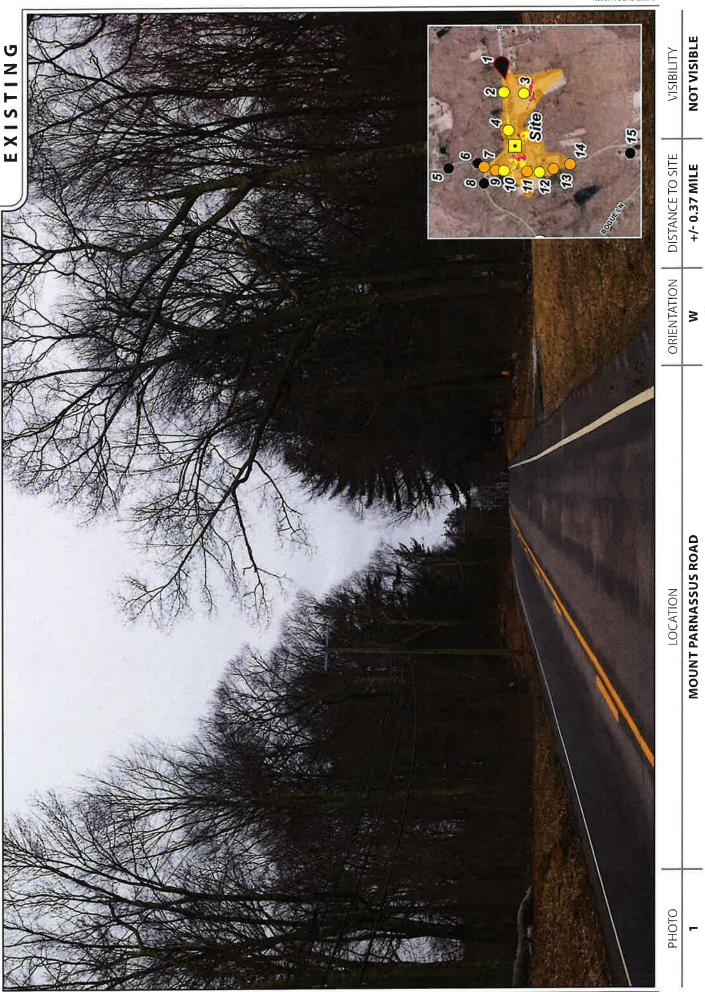
Table 1 - Photo Locations

Photo	Location	Orientation	Distance	Visibility
7-1	MOUNT PARNASSUS ROAD	W	+/- 0.37 MILE	NOT VISIBLE
7	MOUNT PARNASSUS ROAD	8	+/- 0.24 MILE	VISIBLE
٣	CAPTAIN GEORGE COMER MEMORIAL STATE PARK	*	+/- 0.23 MILE	VISIBLE
4	MOUNT PARNASSUS ROAD*	WSW	+/- 415 FEET	VISIBLE
Ŋ	SMITH ROAD*	SSE	+/- 0.31 MILE	NOT VISIBLE
9	SMITH ROAD*	SSE	+/- 0.18 MILE	NOT VISIBLE
7	SMITH ROAD	SSE	+/- 0.16 MILE	SEASONAL
œ	MOUNT PARNASSUS ROAD*	SE	+/- 0.21 MILE	NOT VISIBLE
6	SHANAGHAN ROAD	SE	+/- 0.13 MILE	SEASONAL
10	SHANAGHAN ROAD	ESE	+/- 0.12 MILE	VISIBLE
11	SHANAGHAN ROAD	ENE	+/- 0.12 MILE	SEASONAL
12	SHANAGHAN ROAD	뮏	+/- 0.15 MILE	VISIBLE
13	SHANAGHAN ROAD	NNE	+/- 0.19 MILE	SEASONAL
14	SHANAGHAN ROAD	NNE	+/- 0.25 MILE	SEASONAL
15	SHANAGHAN ROAD	z	+/- 0.50 MILE	NOT VISIBLE
16	MOUNT PARNASSUS ROAD	ENE	+/- 0.62 MILE	NOT VISIBLE
17	BOGEL ROAD	ESE	+/- 0.88 MILE	VISIBLE
18	BOARDMAN ROAD	Ш	+/- 1.95 MILES	VISIBLE
19	TOWN STREET	Ш	+/- 1.76 MILES	NOT VISIBLE
20	TOWN STREET	Ш	+/- 1.74 MILES	VISIBLE
21	TOWN STREET	ш	+/- 1.77 MILES	NOT VISIBLE
22	SHANAGHAN ROAD	z	+/- 1.70 MILES	NOT VISIBLE
23	MOUNT PARNASSUS ROAD	Α	+/- 1.24 MILES	NOT VISIBLE
*Photogr	*Photograph was taken at 35 mm focal length.			

Table 1 - Photo Locations Continued

32	Location	Orientation	DISTAILCE	VISIDIILLY
24 BALLA	HACK ROAD NUMBER 2	SW	+/- 1.18 MILES	SEASONAL
25	BASHAN ROAD	ഗ	+/- 1.79 MILES	VISIBLE







EXISTING



PHOTO















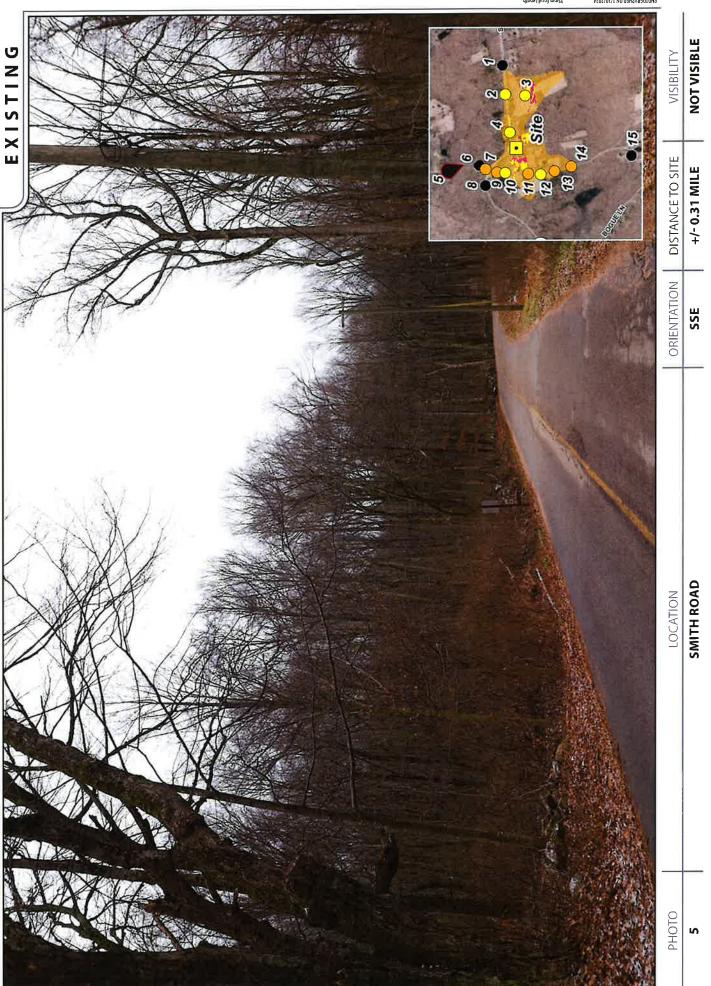




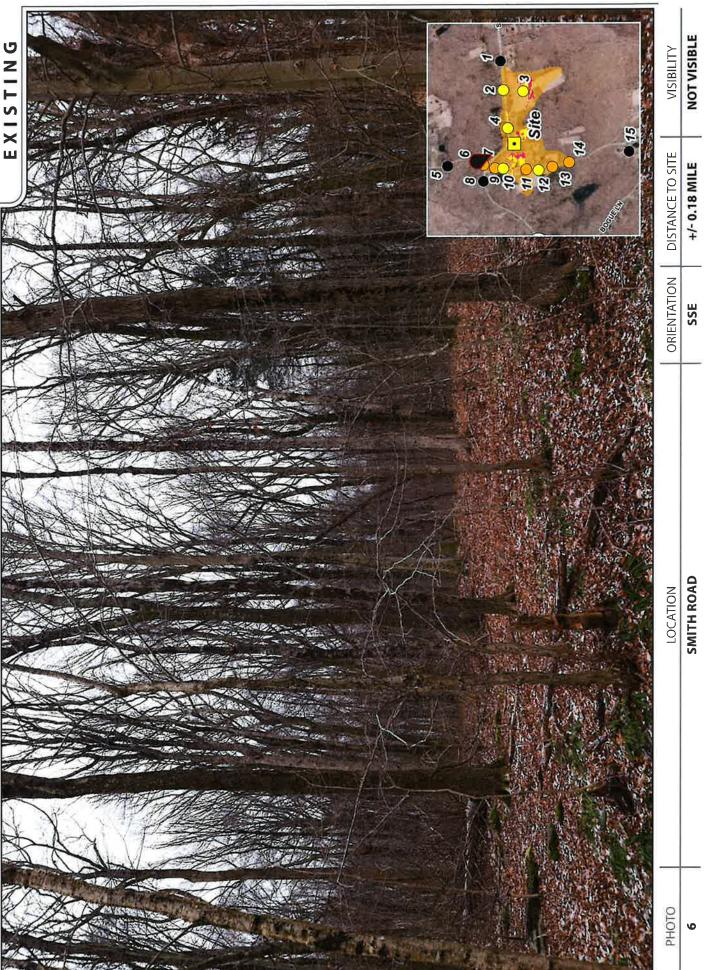








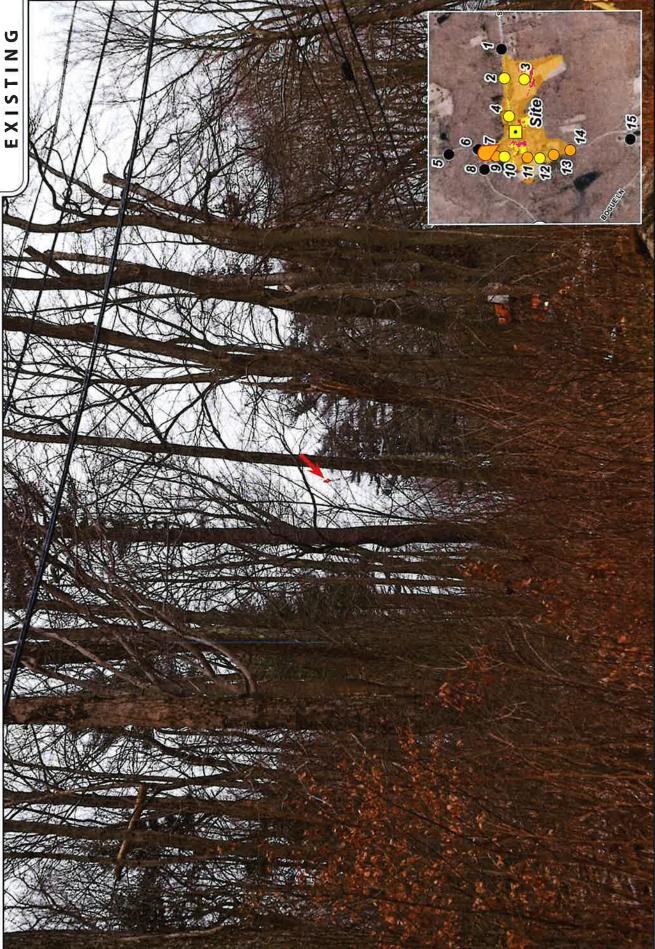




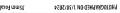


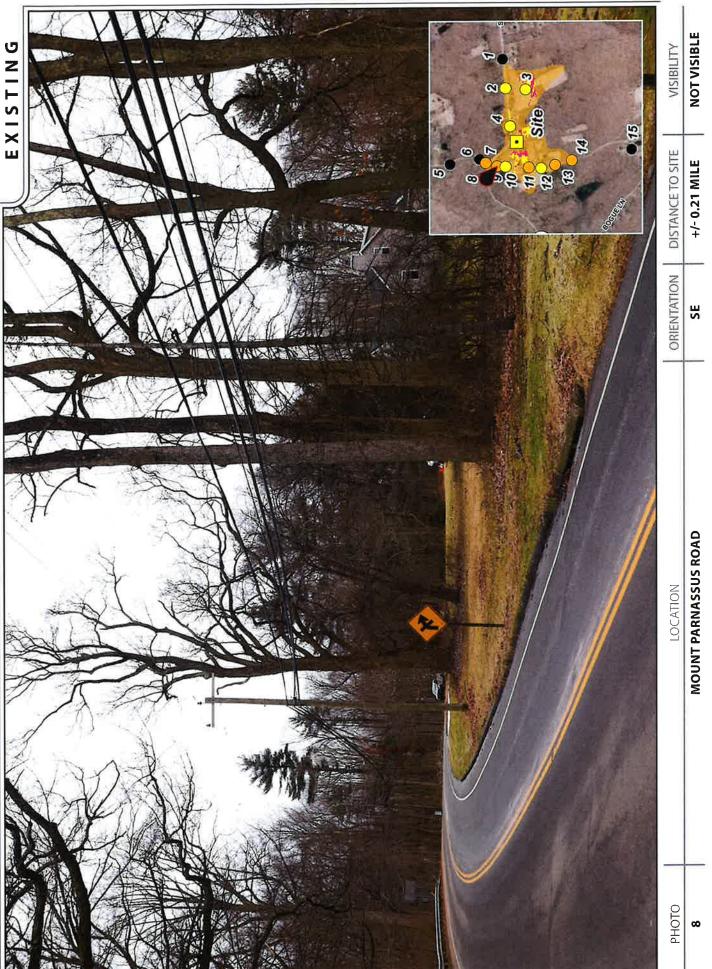
LOCATION

PHOTO































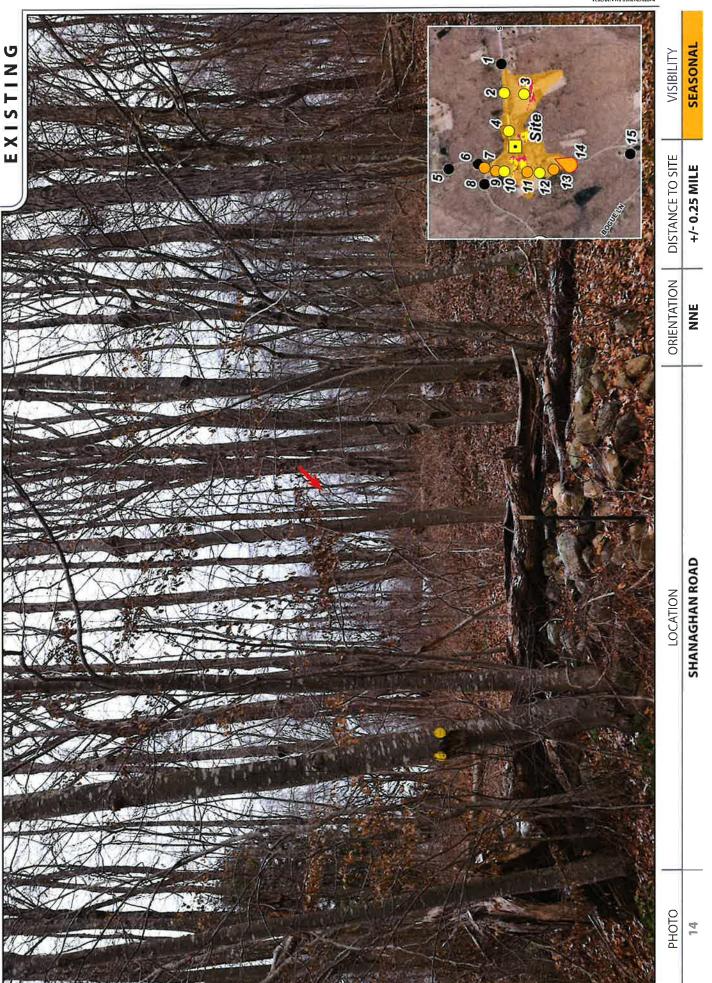






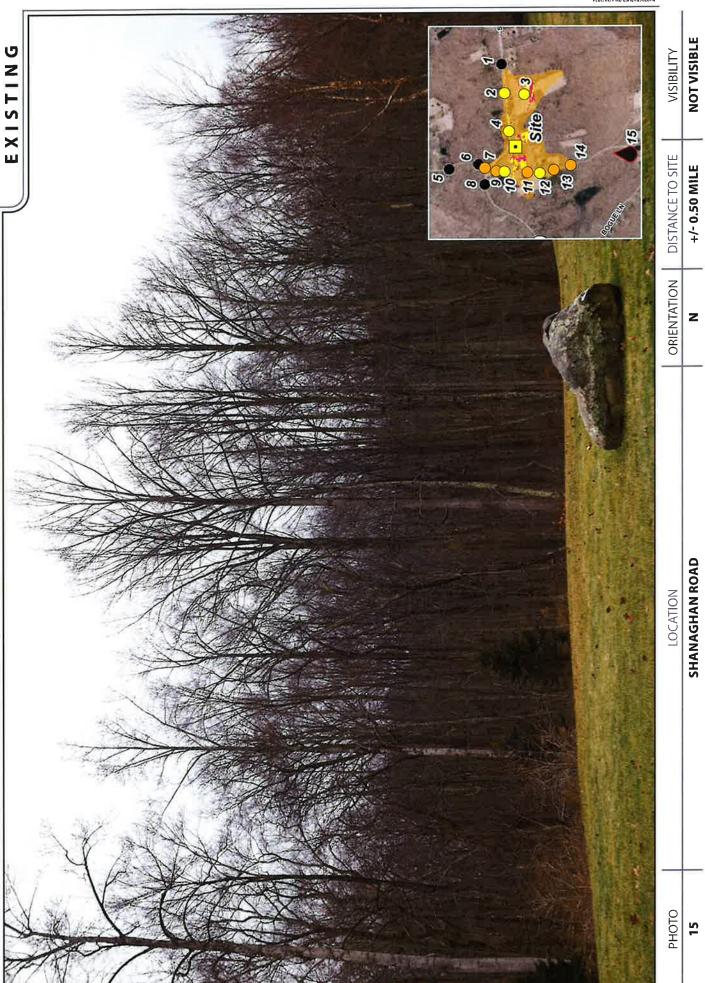






















EXISTING



VISIBILITY

+/- 1.95 MILES

DISTANCE TO SITE

ORIENTATION



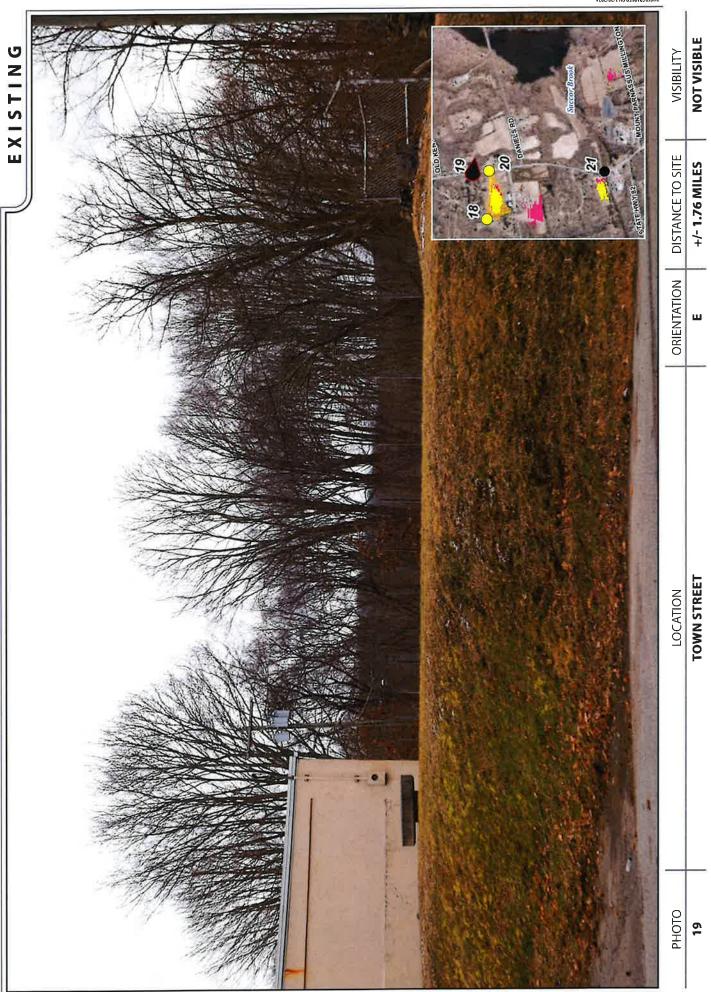
BOARDMAN ROAD LOCATION

PHOTO







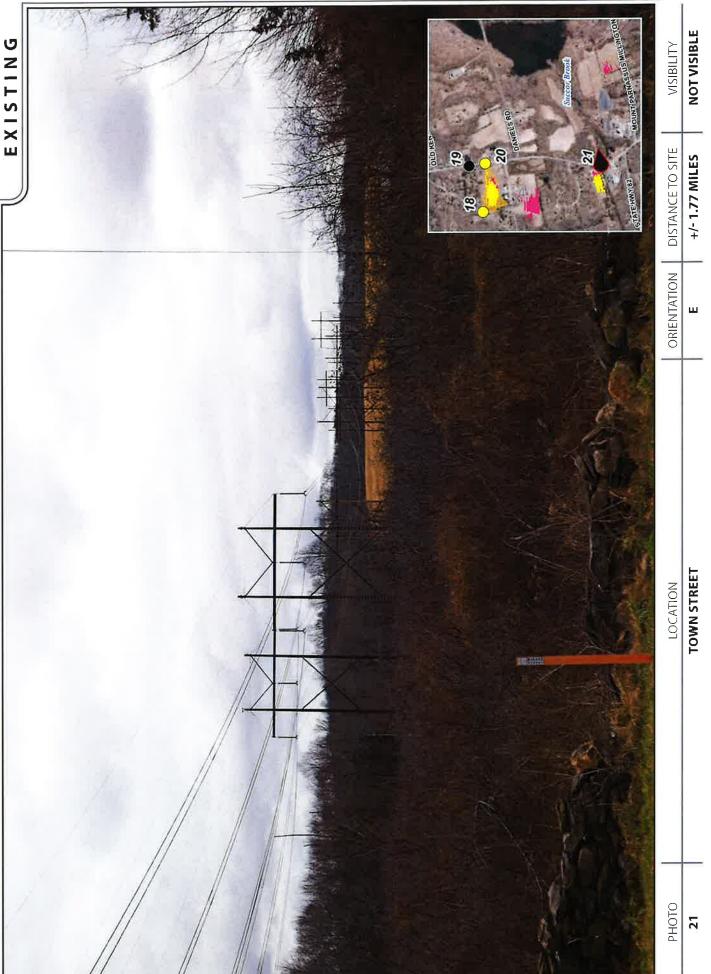




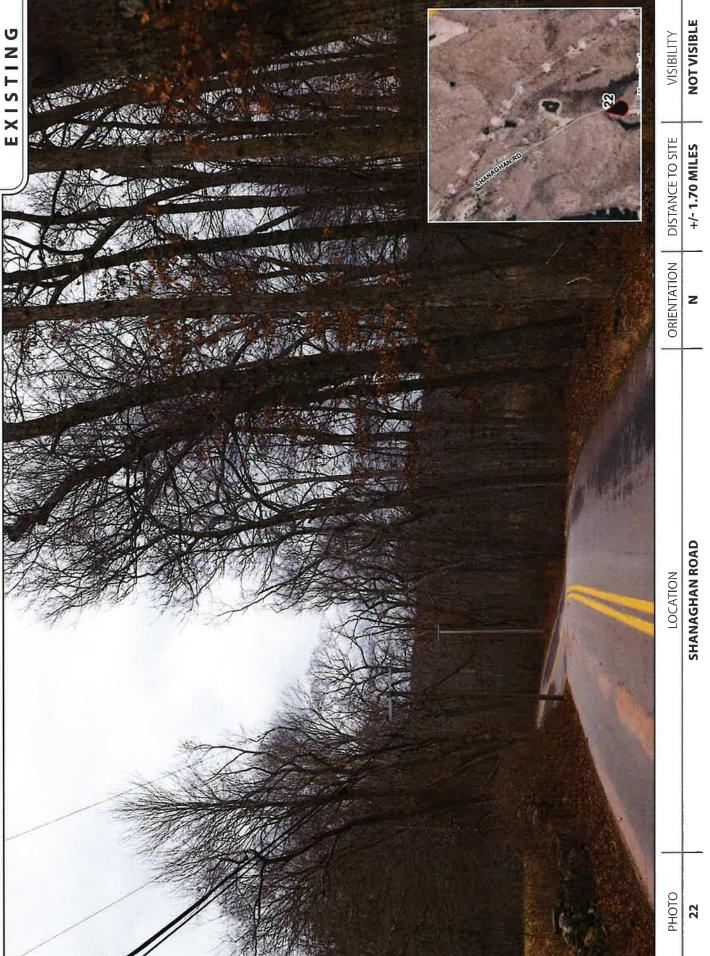




















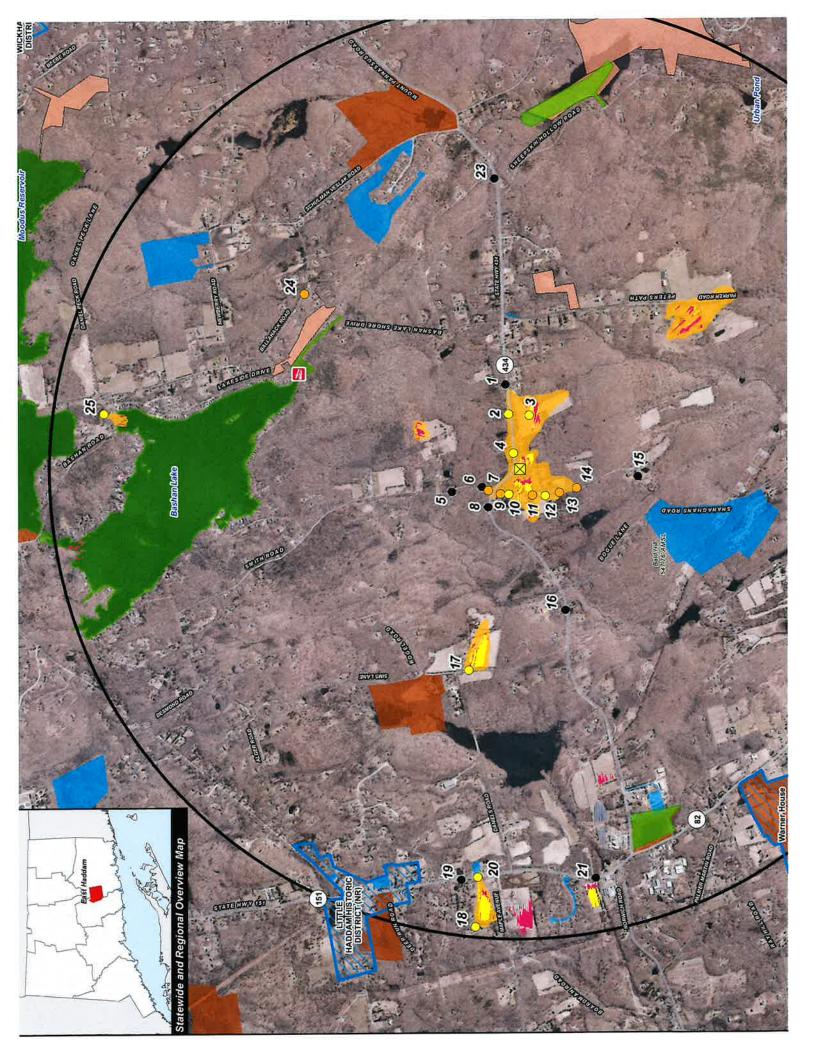


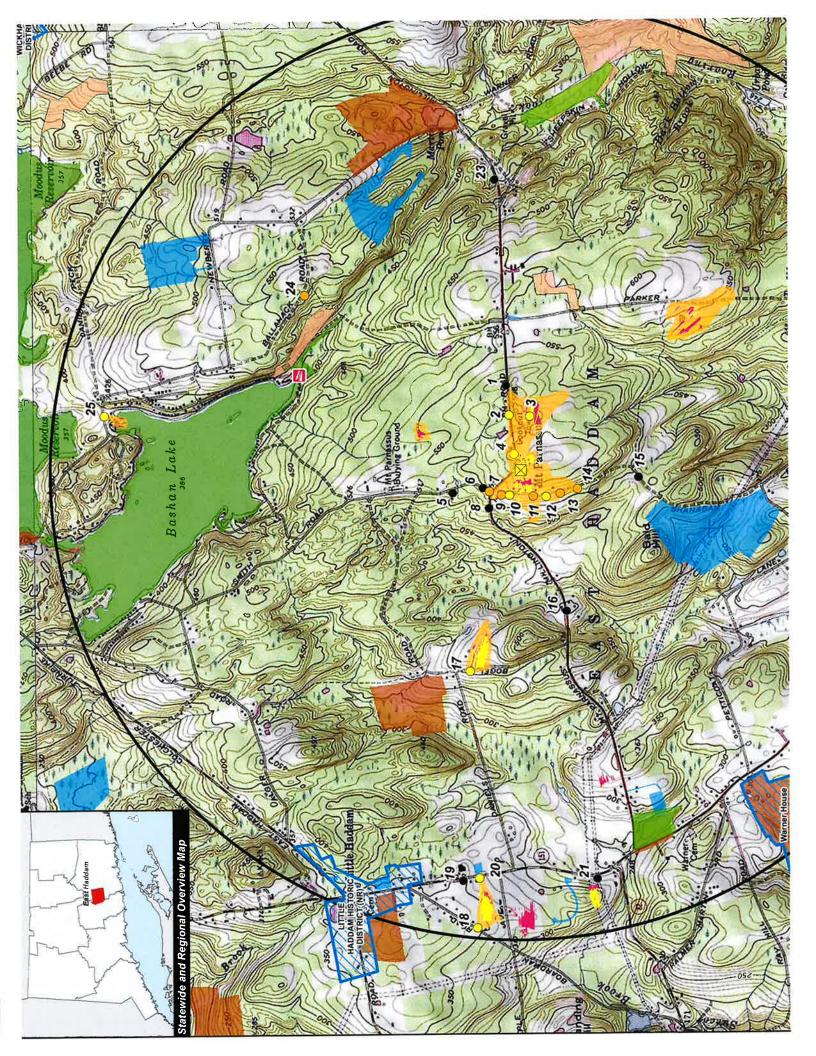


EXISTING









ATTACHMENT 7



C Squared Systems, LLC
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Auburn, NH 03032
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Calculated Radio Frequency Emissions Report



East Haddam 3 CT 194 Mt Parnassus Road, East Haddam, CT 06423

December 21, 2023

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1. Introduction

The purpose of this report is to investigate compliance with applicable FCC regulations for the proposed installation of Verizon's antenna arrays to be mounted at 145' AGL on an existing self-support tower located at 194 Mt Parnassus Road in East Haddam, CT. The coordinates of the self-support tower are 41° 28' 12.108" N, 72° 24' 40.320" W.

Verizon is proposing the following:

1) Install 12 twelve (12) multi-band antennas, four (4) per sector to support its commercial LTE network.

This report considers the proposed antenna configuration for Verizon¹ as well as existing antenna configuration² for CSP, DOT, TOE Haddam, and VS/QV antennas to derive the resulting % MPE of its proposed modification.

2. FCC Guidelines for Evaluating RF Radiation Exposure Limits

In 1985, the FCC established rules to regulate radio frequency (RF) exposure from FCC licensed antenna facilities. In 1996, the FCC updated these rules, which were further amended in August 1997 by OET Bulletin 65 Edition 97-01. These new rules include Maximum Permissible Exposure (MPE) limits for transmitters operating between 300 kHz and 100 GHz. The FCC MPE limits are based upon those recommended by the National Council on Radiation Protection and Measurements (NCRP), developed by the Institute of Electrical and Electronics Engineers, Inc., (IEEE) and adopted by the American National Standards Institute (ANSI).

The FCC general population/uncontrolled limits set the maximum exposure to which most people may be subjected. General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure.

Public exposure to radio frequencies is regulated and enforced in units of milliwatts per square centimeter (mW/cm²). The general population exposure limits for the various frequency ranges are defined in the attached "FCC Limits for Maximum Permissible Exposure (MPE)" in Attachment C of this report.

Higher exposure limits are permitted under the occupational/controlled exposure category, but only for persons who are exposed as a consequence of their employment and who have been made fully aware of the potential for exposure, and they must be able to exercise control over their exposure. General population/uncontrolled limits are five times more stringent than the levels that are acceptable for occupational, or radio frequency trained individuals. Attachment C contains excerpts from OET Bulletin 65 and defines the Maximum Exposure Limit.

Finally, it should be noted that the MPE limits adopted by the FCC for both general population/uncontrolled exposure and for occupational/controlled exposure incorporate a substantial margin of safety and have been established to be well below levels generally accepted as having the potential to cause adverse health effects.

East Haddam 3 CT 1 December 21, 2023

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¹ As referenced to Verizon's Radio Frequency Design Sheet updated 08/01/2023.

² As referenced to equipment list provided by Smartlink – 52 Mt. Parnassus Working Copy 7-19-2022 Equipment List



3. RF Exposure Prediction Methods

The emission field calculation results displayed in the following figures were generated using the following formula as outlined in FCC bulletin OET 65:

Power Density =
$$\left(\frac{GRF^2 \times 1.64 \times ERP}{4\pi \times R^2}\right)$$
 X Off Beam Loss

Where:

EIRP = Effective Isotropic Radiated Power

R = Radial Distance = $\sqrt{(H^2 + V^2)}$

H = Horizontal Distance from antenna in meters

V = Vertical Distance from radiation center of antenna in meters

Off Beam Loss is determined by the selected antenna patterns

Ground reflection factor (GRF) of 1.6

These calculations assume that the antennas are operating at 100 percent capacity, that all antenna channels are transmitting simultaneously, and that the radio transmitters are operating at full power. Obstructions (trees, buildings, etc.) that would normally attenuate the signal are not taken into account. The calculations assume even terrain in the area of study and do not take into account actual terrain elevations which could attenuate the signal. As a result, the predicted signal levels reported below are much higher than the actual signal levels will be from the final installations.



4. Antenna Inventory

Table 1 below outlines Verizon's proposed antenna configuration for the site. The associated data sheets and antenna patterns for these specific antenna models are included in Attachments C.

Operator	Sector / Call Sign	TX Freq (MHz)	Power at Antenna (Watts)	Ant Gain (dBi)	Power EIRP (Watts)	Antenna Model	Beam Width (degree)	Mech. Tilt	Length (ft)	Antenna Centerline Height (ft)
		700	160	16.8	7658	NHH-45B-R2B	48	0	6	145
		850	160	17.5	8997		43			
	Alpha / 345°	1900	160	19.9	15636		43			
	343	2100	240	20.3	25716		41			
		3700	320	26.5	142939	MT6413-77A		0	*	145
		700	160	16.8	7658	NHH-45B-R2B	48	0	6	145
	Beta / 75°	850	160	17.5	8997		43			
		1900	160	19.9	15636		43			
		2100	240	20.3	25716		41			
		3700	320	26.5	142939	MT6413-77A	•	0	6	145
Verizon	Gamma / 165°	700	160	16.8	7658	NHH-45B-R2B	48	0	6	145
		850	160	17.5	8997		43			
		1900	160	19.9	15636		43			
		2100	240	20.3	25716		41			
		3700	320	26.5	142939	MT6413-77A	•	0	6	145
	Delta / 255°	700	160	16.8	7658	NHH-45B-R2B	48	0	6	145
		850	160	17.5	8997		43			
		1900	160	19.9	15636		43			
		2100	240	20.3	25716		41			
		3700	320	26.5	142939	MT6413-77A	¥	0	6	145

Table 1: Proposed Antenna Inventory³⁴

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³ Antenna heights are in reference to Verizon's Radio Frequency Design Sheet updated 08/01/2023.

⁴ Transmit power assumes 0 dB of cable loss.



5. Calculation Results

The calculated power density results are shown in Figure 1 below. For completeness, the calculations for this analysis range from 0 feet horizontal distance (directly below the antennas) to a value of 3,000 feet horizontal distance from the site. In addition to the other worst-case scenario considerations that were previously mentioned, the power density calculations to each horizontal distance point away from the antennas was completed using a local maximum off beam antenna gain (within \pm 5 degrees of the true mathematical angle) to incorporate a realistic worst-case scenario.

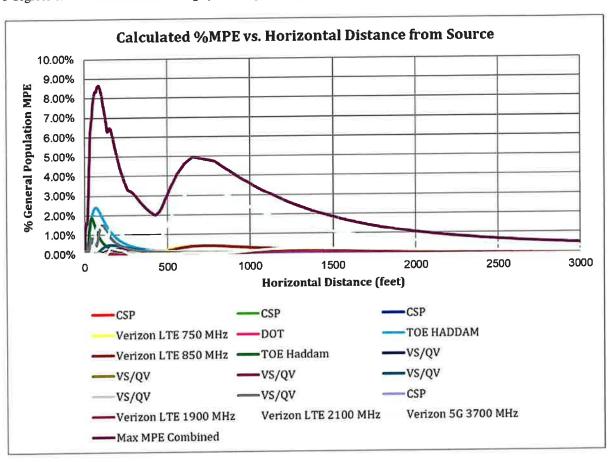


Figure 1: Graph of General Population % MPE vs. Distance

The highest percent of MPE (8.66% of the General Population limit) is calculated to occur at a horizontal distance of 88 feet from antennas. Please note that the percent of MPE calculations close to the site take into account off beam loss, which is determined from the vertical pattern of the antennas used. Therefore, RF power density levels may increase as the distance from the site increases. At distances of approximately 1500 feet and beyond, one would now be in the main beam of the antenna pattern and off beam loss is no longer considered. Beyond this point, RF levels become calculated solely on distance from the site and the percent of MPE decreases significantly as distance from the site increases.



Table 2 below lists percent of MPE values as well as the associated parameters that were included in the calculations. The highest percent of MPE value was calculated to occur at a horizontal distance of 88 feet from the site (reference Figure 1).

As stated in Section 3, all calculations assume that the antennas are operating at 100 percent capacity, that all antenna channels are transmitting simultaneously, and that the radio transmitters are operating at full power. Obstructions (trees, buildings etc.) that would normally attenuate the signal are not taken into account. In addition, a six foot height offset was considered in this analysis to account for average human height. As a result, the predicted signal levels are significantly higher than the actual signal levels will be from the final configuration. The results presented in Figure 1 and Table 2 assume level ground elevation from the base of the tower out to the horizontal distances calculated.

Carrier	Number of Transmitters	Power out of Base Station Per Transmitter (Watts)	Antenn a Height (Feet)	Distance to the Base of Antennas (Feet)	Power Density (mW/cm²	Limit (mW/cm²	% MPE
CSP	1	25.0	120.0	88	0.000021	0.516	0.00%
CSP	4	25.0	120.0	88	0.000086	0.536	0.02%
CSP	1	25.0	120.0	88	0.000051	0.516	0.01%
CSP	2	1.0	120.0	88	0.000000	1.000	0.00%
DOT	1	100.0	55.0	88	0.004445	0.200	2.22%
TOE HADDAM	2	50.0	55.0	88	0.004445	0.200	2.22%
TOE Haddam	1	25.0	25.0	88	0.001799	0.200	0.90%
Verizon 5G 3700 MHz	1	320.0	145.0	88	0.014919	1.000	1.49%
Verizon LTE 1900 MHz	1	160.0	145.0	88	0.000028	1.000	0.00%
Verizon LTE 2100 MHz	1	240.0	145.0	88	0.000209	1.000	0.02%
Verizon LTE 750 MHz	1	160.0	145.0	88	0.000402	0.500	0.08%
Verizon LTE 850 MHz	1	160.0	145.0	88	0.000532	0.567	0.09%
VS/QV	1	100.0	120.0	88	0.000162	0.307	0.05%
VS/QV	1	100.0	85.0	88	0.000277	0.307	0.09%
VS/QV	1	100.0	100.0	88	0.000044	0.200	0.02%
VS/QV	1	100.0	80.0	88	0.000346	0.200	0.17%
VS/QV	1	100.0	85.0	88	0.000277	0.300	0.09%
VS/QV	1	35.0	20.0	88	0.005417	0.467	1.16%
	1,					Total	8.66%

Table 2: Maximum Percent of General Population Exposure Values⁵

⁵ In the case where pattern data was unavailable from the manufacturer, vertical patterns with similar specifications were used



6. Conclusion

The above analysis verifies that RF exposure levels from the site with Verizon's proposed antenna configuration will be well below the maximum permissible levels as outlined by the FCC in the OET Bulletin 65 Ed. 97-01. Using the conservative calculation methods and parameters detailed above, the maximum cumulative percent of MPE in consideration of all transmitters is calculated to be 8.66% of the FCC limit (General Population/Uncontrolled). This maximum cumulative percent of MPE value is calculated to occur 88 feet away from the site.

7. Statement of Certification

I certify to the best of my knowledge that the statements in this report are true and accurate. The calculations follow guidelines set forth in ANSI/IEEE Std. C95.3, ANSI/IEEE Std. C95.1 and FCC OET Bulletin 65 Edition 97-01.

Report Prepared By:

Ram Acharya

RF Engineer 1 C Squared Systems, LLC

Main & Fam

December 19, 2023

Date

Reviewed/Approved By:

Martin Lavin

Senior RF Engineer C Squared Systems, LLC December 21, 2023 Date



Attachment A: References

OET Bulletin 65 - Edition 97-01 - August 1997 Federal Communications Commission Office of Engineering & Technology

IEEE C95.1-2005, IEEE Standard Safety Levels With Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz IEEE-SA Standards Board

IEEE C95.3-2002 (R2008), IEEE Recommended Practice for Measurements and Computations of Radio Frequency Electromagnetic Fields With Respect to Human Exposure to Such Fields, 100 kHz-300 GHz IEEE-SA Standards Board



Attachment B: FCC Limits for Maximum Permissible Exposure (MPE)

(A) Limits for Occupational/Controlled Exposure⁶

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm ²)	Averaging Time E ² , H ² or S (minutes)
0,3-3.0	614	1.63	(100)*	6
3.0-30	1842/f	4.89/f	$(900/f^2)*$	6
30-300	61.4	0.163	1.0	6
300-1500	: - :	-	f/300	6
1500-100,000	-	:+:	5	6

(B) Limits for General Population/Uncontrolled Exposure⁷

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm ²)	Averaging Time $ E ^2$, $ H ^2$ or S (minutes)
0.3-1.34	614	1.63	(100)*	30
1.34-30	824/f	2.19/f	$(180/f^2)*$	30
30-300	27.5	0.073	0.2	30
300-1500		•	f/1500	30
1500-100,000	9=3	:=:	1.0	30

f = frequency in MHz * Plane-wave equivalent power density

Table 3: FCC Limits for Maximum Permissible Exposure

East Haddam 3 CT 8 December 21, 2023

⁶ Occupational/controlled limits apply in situations in which persons are exposed as a consequence of their employment provided those persons are fully aware of the potential for exposure and can exercise control over their exposure. Limits for occupational/controlled exposure also apply in situations when an individual is transient through a location where occupational/controlled limits apply provided he or she is made aware of the potential for exposure.

⁷ General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure.



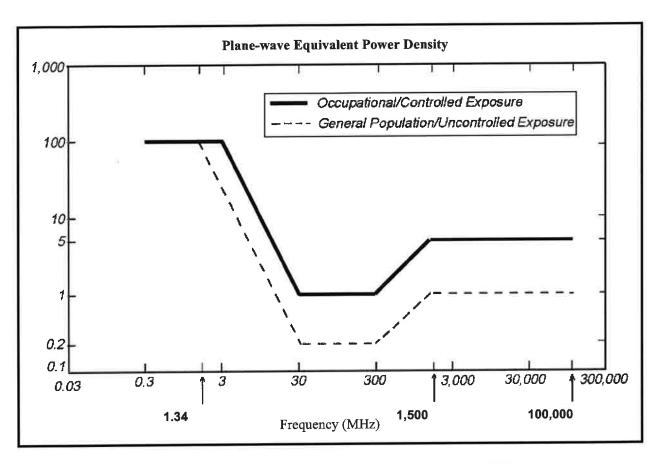


Figure 2: Graph of FCC Limits for Maximum Permissible Exposure (MPE)



Attachment C: Verizon Antenna Model Data Sheets and Electrical Patterns

739 MHz

Manufacturer: COMMSCOPE

Model #: NHH-45B-2RB

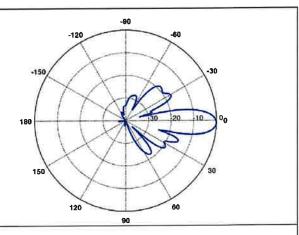
Frequency Band: 698-806 MHz

Gain: 16.8 dBi

Vertical Beamwidth: 12.5° Horizontal Beamwidth: 48°

Polarization: ±45°

Size L x W x D: 72.0" x 17.9" x 7.0"



885 MHz

Manufacturer: COMMSCOPE

Model #: NHH-45B-2RB

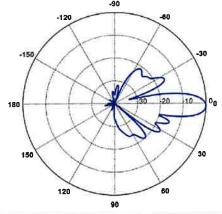
Frequency Band: 806 - 896 MHz

Gain: 17.5 dBi

Vertical Beamwidth: 11.4° Horizontal Beamwidth: 43°

Polarization: ±45°

Size L x W x D: 72.0" x 17.9" x 7.0"





1900 MHz

Manufacturer: COMMSCOPE

Model #: NHH-45B-2RB

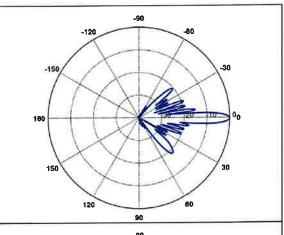
Frequency Band: 1850-1990 MHz

Gain: 19.9 dBi

Vertical Beamwidth: 5.4° Horizontal Beamwidth: 43°

Polarization: ±45°

Size L x W x D: 72.0" x 17.9" x 7.0"



2100 MHz

Manufacturer: COMMSCOPE

Model #: NHH-45B-2RB

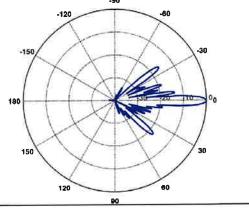
Frequency Band: 1920-2200 MHz

Gain: 20.3 dBi

Vertical Beamwidth: 5° Horizontal Beamwidth: 41°

Polarization: ±45°

Size L x W x D: 72.0" x 17.9" x 7.0"



ATTACHMENT 8

Airspace User: Not Identified

File: EHADDAM3

Location: East Haddam, CT

Latitude: 41°-28'-12.04" Longitude: 72°-24'-39.83"

SITE ELEVATION AMSL.....595 ft. STRUCTURE HEIGHT......165 ft. OVERALL HEIGHT AMSL.....760 ft.

NOTICE CRITERIA

FAR 77.9(a): NNR (DNE 200 ft AGL)

FAR 77.9(b): NR (Exceeds Notice Slope, Maximum: 158 ft.)

FAR 77.9(c): NNR (Not a Traverse Way)

FAR 77.9: NNR FAR 77.9 IFR Straight-In Notice Criteria for 42B

FAR 77.9: NNR FAR 77.9 IFR Notice for SNC FAR 77.9(d): NNR (Off Airport Construction)

NR = Notice Required

NNR = Notice Not Required

PNR = Possible Notice Required (depends upon actual IFR procedure)
For new construction review Air Navigation Facilities at bottom
of this report.

If the proposed construction is an alteration to an existing structure, notice requirements may be superceded by the item exemptions listed below.

The location and analysis were based upon an existing structure. However, no existing aeronautical study number was identified. If the 'existing' structure penetrates an obstruction surface defined by CFR 77.17, 77.19, 77.21 or 77.23 (see below) it is strongly recommended the FAA be notified of the 'existing' structure to determine obstruction marking or lighting requirements. It is not uncommon for the FAA to issue a Determination of No Hazard (DNH) for an existing structure and modify the airspace to accommodate the structure, should that be required. If the FAA issues a DNH enter the aeronautical study number (ASN) in the space provided on the Airspace Analysis Window Form and re-run Airspace.

No frequencies were identified in this alteration are included in the FAA's Co-Location Policy published in the Federal Register November 15, 2007. Therefore, application of the Co-Location Policy notice exemption rule can not be applied.

Notice Criteria found in Title 14 CFR 77.9 applies to the alteration of existing structures.

```
OBSTRUCTION STANDARDS
  FAR 77.17(a)(1): DNE 499 ft AGL
  FAR 77.17(a)(2): DNE - Airport Surface
  FAR 77.19(a):
                   DNE - Horizontal Surface
 FAR 77.19(b):

FAR 77.19(c):

FAR 77.19(d):

FAR 77.19(e):
                   DNE - Conical Surface
                   DNE - Primary Surface
                   DNE - Approach Surface
                   DNE - Approach Transitional Surface
  FAR 77.19(e):
                   DNE - Abeam Transitional Surface
VFR TRAFFIC PATTERN AIRSPACE FOR: 42B: GOODSPEED
          RD: 15078.84 RE: 8.3
Type: A
  FAR 77.17(a)(1):
                            DNE
                            DNE - Height No Greater Than 200 feet AGL.
  FAR 77.17(a)(2):
  VFR Horizontal Surface:
                            DNE
                            DNE
  VFR Conical Surface:
 VFR Primary Surface:
                            DNE
                            DNE
  VFR Approach Surface:
  VFR Transitional Surface: DNE
  The structure is within VFR - Traffic Pattern Airspace Runway Side Area.
  Structures that exceed horizontal, conical, and/or 500' AGL will receive
  a hazard determination from the FAA.
VFR TRAFFIC PATTERN AIRSPACE FOR: SNC: CHESTER
Type: A RD: 40098.11 RE: 395
                            DNE
  FAR 77.17(a)(1):
                            Does Not Apply.
  FAR 77.17(a)(2):
  VFR Horizontal Surface:
                            DNE
  VFR Conical Surface:
                            DNE
                            DNE
  VFR Primary Surface:
  VFR Approach Surface:
                            DNE
  VFR Transitional Surface: DNE
TERPS DEPARTURE PROCEDURE (FAA Order 8260.3, Volume 4)
  FAR 77.17(a)(3) Departure Surface Criteria (40:1)
  The Maximum Height Permitted is 385 ft AMSL
MINIMUM OBSTACLE CLEARANCE ALTITUDE (MOCA)
  FAR 77.17(a)(4) MOCA Altitude Enroute Criteria
  The Maximum Height Permitted is 1600 ft AMSL
PRIVATE LANDING FACILITIES
                                                      RANGE DELTA ARP FAA
                                           BEARING
  FACIL
                                           To FACIL
                                                      IN NM ELEVATION IFR
  IDENT TYP NAME
```

CT86 HEL SANFORD 160.32 2.3 +460

No Impact to Private Landing Facility

Structure is beyond notice limit by 8975 feet.

CT11 AIR DEVILS HOPYARD FLD 115.72 3.61 +635

Possible Impact to Private Landing Facility

Exceeds 261 ft VFR Transitional Surface (N/A Private Airport).

AIR NAVIGATION ELECTRONIC FACILITIES

	FAC		ST			DIST	DELTA			GRND
APCH	IDNT	TYPE	ΑТ	FREQ	VECTOR	(ft)	ELEVA	ST	LOCATION	ANGLE
BEAR										
	HED	VOR/DME	r R	114.9	329.14	72653	-89	СТ	HARTFORD	07
	ט ווו	VOR/ DAL	IX.	114.5	J2J.17	72055	0,5	•		
	MAD	VOR/DME	I	110.4	233.5	95854	+544	СТ	MADISON	.33
	GON	VOR/DME	R	110.8	117.43	110881	+751	СТ	GROTON	.39
	ORW	VOR/DME	I	110.0	74.27	117078	+450	СТ	NORWICH	.22
	BDL	RADAR ASR	I		336.62	186125	+524	СТ	BRADLEY INTL	.16
	нто	VORTAC	R	113.6	172.66	202447	+738	NY	HAMPTON	.21
	QVH	RADAR ARSR	Υ	1326.9	199.35	228472	+409	NY	RIVERHEAD	.10
	PVD	RADAR ASR	I	2735.	67.38	240350	+194	RI	THEODORE FRANCIS	.05

5G AIRPORT SAFETY AREA

No Identified 5G conflict.

CFR Title 47, §1.30000-§1.30004

AM STUDY NOT REQUIRED: Structure is not near a FCC licensed AM station. Movement Method Proof as specified in §73.151(c) is not required. Please review 'AM Station Report' for details.

Nearest AM Station: WLIS @ 15969 meters.

Airspace® Summary Version 23.11.692

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ATTACHMENT 9

KENNETH C. BALDWIN

280 Trumbull Street Hartford, CT 06103-3597 Main (860) 275-8200 Fax (860) 275-8299 kbaldwin@rc.com Direct (860) 275-8345

Also admitted in Massachusetts and New York

February 22, 2024

Via Certificate of Mailing

Irene M. Haines, First Selectman Town of East Haddam 1 Plains Road Moodus, CT 06469

Re: Cellco Partnership d/b/a Verizon Wireless – Petition for Declaratory Ruling Filed with the Connecticut Siting Council for the Modification of an Existing Telecommunications Facility at 194 Mount Parnassus Road, East Haddam, Connecticut

Dear First Selectman Haines:

This firm represents Cellco Partnership d/b/a Verizon Wireless ("Cellco"). Today, Cellco filed a Petition for Declaratory Ruling ("Petition") with the Connecticut Siting Council ("Council") seeking approval to modify the existing wireless telecommunications facility at 194 Mount Parnassus Road in East Haddam, Connecticut (the "Property"). The modifications involve the extension of the existing 121-foot lattice tower by 40 feet, to an overall height of 161 feet. The extended tower would continue to support the State and municipal emergency service antennas and new antennas installed by Cellco. Equipment associated with Cellco's antennas would be located on the ground near the base of the tower within the existing facility compound.

A copy of the full Petition is attached for your review. If you have any questions regarding this Petition, please contact me or the Siting Council directly at (860) 827-2935.

Sincerely,

Kenneth C. Baldwin

Kunie gmu

Attachment

28513044-v1

KENNETH C. BALDWIN

280 Trumbull Street Hartford, CT 06103-3597 Main (860) 275-8200 Fax (860) 275-8299 kbaldwin@rc.com Direct (860) 275-8345

Also admitted in Massachusetts and New York

February 22, 2024

Via Certificate of Mailing

James Ventres, Land Use Administrator Town of East Haddam 1 Plains Road Moodus, CT 06469

Re: Cellco Partnership d/b/a Verizon Wireless – Petition for Declaratory Ruling Filed with the Connecticut Siting Council for the Modification of an Existing Telecommunications Facility at 194 Mount Parnassus Road, East Haddam, Connecticut

Dear Mr. Ventres:

This firm represents Cellco Partnership d/b/a Verizon Wireless ("Cellco"). Today, Cellco filed a Petition for Declaratory Ruling ("Petition") with the Connecticut Siting Council ("Council") seeking approval to modify the existing wireless telecommunications facility at 194 Mount Parnassus Road in East Haddam, Connecticut (the "Property"). The modifications involve the extension of the existing 121-foot lattice tower by 40 feet, to an overall height of 161 feet. The extended tower would continue to support the State and municipal emergency service antennas and new antennas installed by Cellco. Equipment associated with Cellco's antennas would be located on the ground near the base of the tower within the existing facility compound.

A copy of the full Petition is attached for your review. If you have any questions regarding this Petition, please contact me or the Siting Council directly at (860) 827-2935.

Sincerely,

Kenneth C. Baldwin

Kunig gmu

Attachment

28513192-v1

KENNETH C. BALDWIN

280 Trumbull Street IIartford, CT 06103-3597 Main (860) 275-8200 Fax (860) 275-8299 kbaldwin@rc.com Direct (860) 275-8345

Also admitted in Massachusetts and New York

February 22, 2024

Via Certificate of Mailing

State of Connecticut Department of Transportation 2800 Berlin Turnpike Newington, CT 06131

Re: Cellco Partnership d/b/a Verizon Wireless – Petition for Declaratory Ruling Filed with the Connecticut Siting Council for the Modification of an Existing Telecommunications Facility at 194 Mount Parnassus Road, East Haddam, Connecticut

Dear Sir or Madam:

This firm represents Cellco Partnership d/b/a Verizon Wireless ("Cellco"). Today, Cellco filed a Petition for Declaratory Ruling ("Petition") with the Connecticut Siting Council ("Council") seeking approval to modify the existing wireless telecommunications facility at 194 Mount Parnassus Road in East Haddam, Connecticut (the "Property"). The modifications involve the extension of the existing 121-foot lattice tower by 40 feet, to an overall height of 161 feet. The extended tower would continue to support the State and municipal emergency service antennas and new antennas installed by Cellco. Equipment associated with Cellco's antennas would be located on the ground near the base of the tower within the existing facility compound.

A copy of the full Petition is attached for your review. If you have any questions regarding this Petition, please contact me or the Siting Council directly at (860) 827-2935.

Sincerely,

Kenneth C. Baldwin

Kunig gmu-

Attachment

28513233-v1

ATTACHMENT 10

KENNETH C. BALDWIN

280 Trumbull Street Hartford, CT 06103-3597 Main (860) 275-8200 Fax (860) 275-8299 kbaldwin@rc.com Direct (860) 275-8345

Also admitted in Massachusetts and New York

February 22, 2024

Via Certificate of Mailing

«Name and Address»

Re: Petition for Declaratory Ruling Filed with the Connecticut Siting Council for the Modification of an Existing Telecommunications Facility at 194 Mount Parnassus Road, East Haddam, Connecticut

Dear «Salutation»:

This firm represents Cellco Partnership d/b/a Verizon Wireless ("Cellco"). Today, Cellco filed a Petition for Declaratory Ruling ("Petition") with the Connecticut Siting Council ("Council") seeking approval to modify the existing wireless telecommunications facility at 194 Mount Parnassus Road in East Haddam, Connecticut (the "Property"). The modifications involve the extension of the existing 121-foot lattice tower by 40 feet, to an overall height of 161 feet. The extended tower would continue to support the State and municipal emergency service antennas and new antennas installed by Cellco. Equipment associated with Cellco's antennas would be located on the ground near the base of the tower within the existing facility compound.

This notice is being sent to you because you are identified on the Town Assessor's records as an owner of land that abuts the Property. If you have any questions regarding the Petition, the Council's process for reviewing the Petition or the details of the filing itself, please feel free to contact me at the number listed above. You may also contact the Council directly at 860-827-2935.

Sincerely,

Kenneth C. Baldwin

Kunie gmu

Attachment

4 Bay Road Building A. Suile 300 Hodley, MA 01035 CONSULTANTS:

PRESENCE OF STATES OF STAT

AVITINGLOUD CL OCIUS
SECOND ELOGO
SO VIERVNOSE DINAE
POP AREIZON BINSETESS
SIZIEMS' TIC
BETT VIIVALIC NOBIUS

VELIZON

(2) T STAMP:

DATE: 02/06/24

DRAW: TBD/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO: 13-030
SHEET IDLE:

THE CONSTRUCTON SHOWN HEREN MAY REQUIRE SPECIAL INSPECTIONS UNDER THE STATE BURDNE CODE, ADMICAN/TOXATROOR SHALL WERFY WHIT HE AUTHORITIES HAWNG, JURSIDICINO (AHJ) PRIOR TO CONSTRUCTON AND ELOGAGE THE INSPECTIOR AND/OR APPROPRIATE 3ND PARTIES AS MAY BE REQUIRED.

REFER TO PASSING STRUCTURAL ANALYSIS BY CENTEK ENGINEERING DATED OCTOBER 6, 2023,

ALL WORK SHALL BE COMPLETED IN ACCORDANCE WITH THE PASSING ANTENNA MOUNT ANALYSIS REPORT BY CENTEK ENGINEERING DATED OCTOBER 9, 2023,

TITLE SHEET

Ξ

PERMITTING

Pro Terra

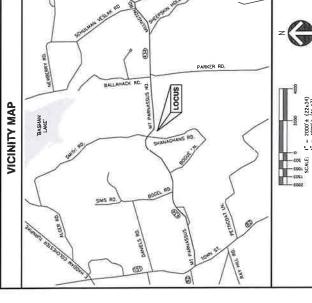
AD

REVISION

DRAWING INDEX

DESCRIPTION TITLE SHEET

SHEET ī Z-1 Z-2



CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE 40S THE TAB SHALL MEMBERIETY NOTIFY THE ENGINEER & OWNER REPRESENTATIVE IN WITHING OS DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

GENERAL NOTES

COMPOUND PLAN AND ELEVATION

DETAILS

D-1 TO D-3

OVERALL SITE PLAN

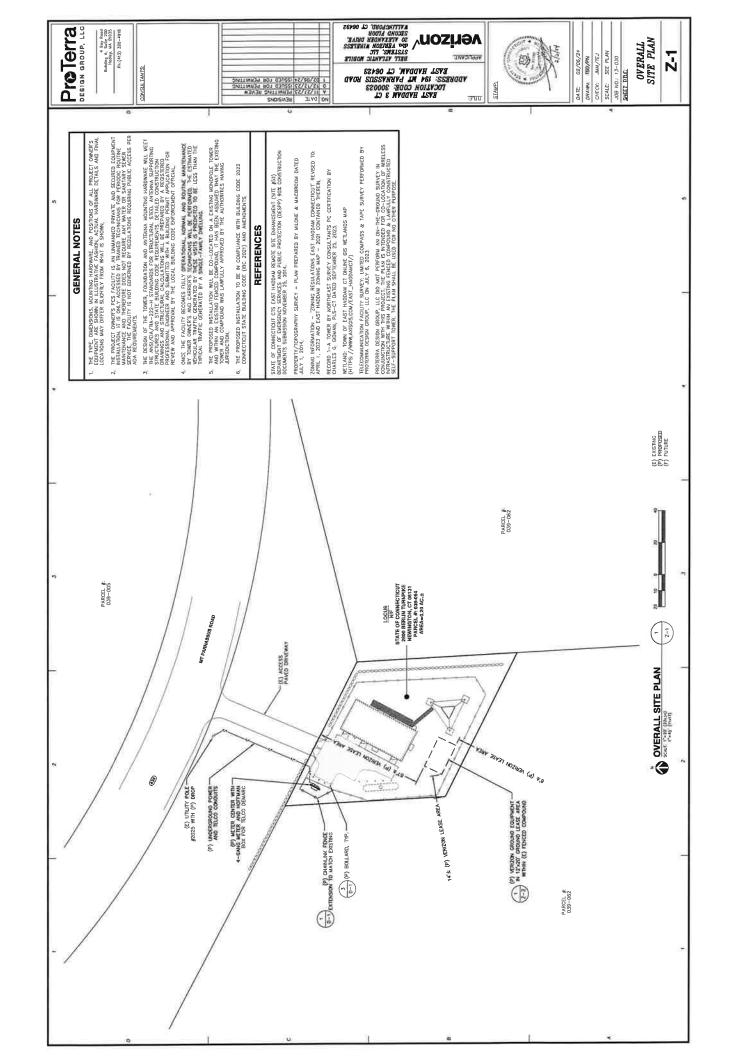
ALL UNDERGROUND UTILITY INFORMATION WAS DETERMINED FROM SURFACE INVESTIGNATIONS AND ENGENORD. THE RECORD, THE CONTRACTORS SHALL LUCKERGROUND UTILITIES IN THE FELD PRIOR TO ANY SITE. WIGH, CALL IDG-SAFE (889) 344–723.3 (OR BIT) 72-HOURS PRIOR TO

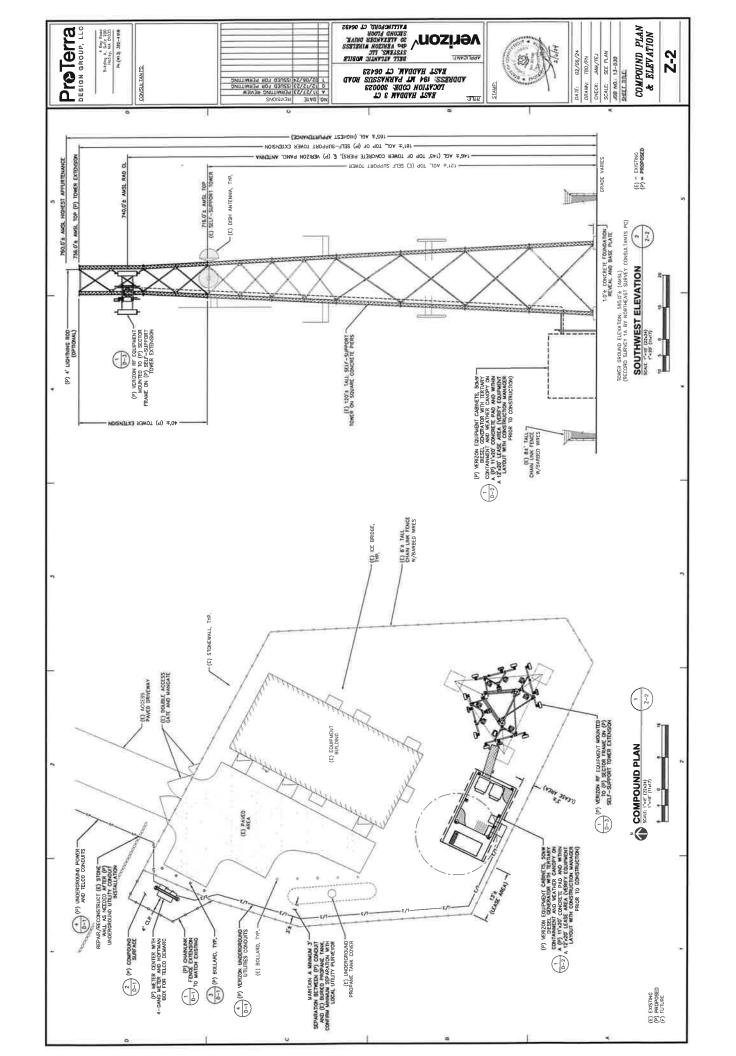
ALL WORK TO BE PERFORMED IN ACCORDANCE WITH THE LATEST VERIZON WIRELESS CONSTRUCTION GLIDELINES.

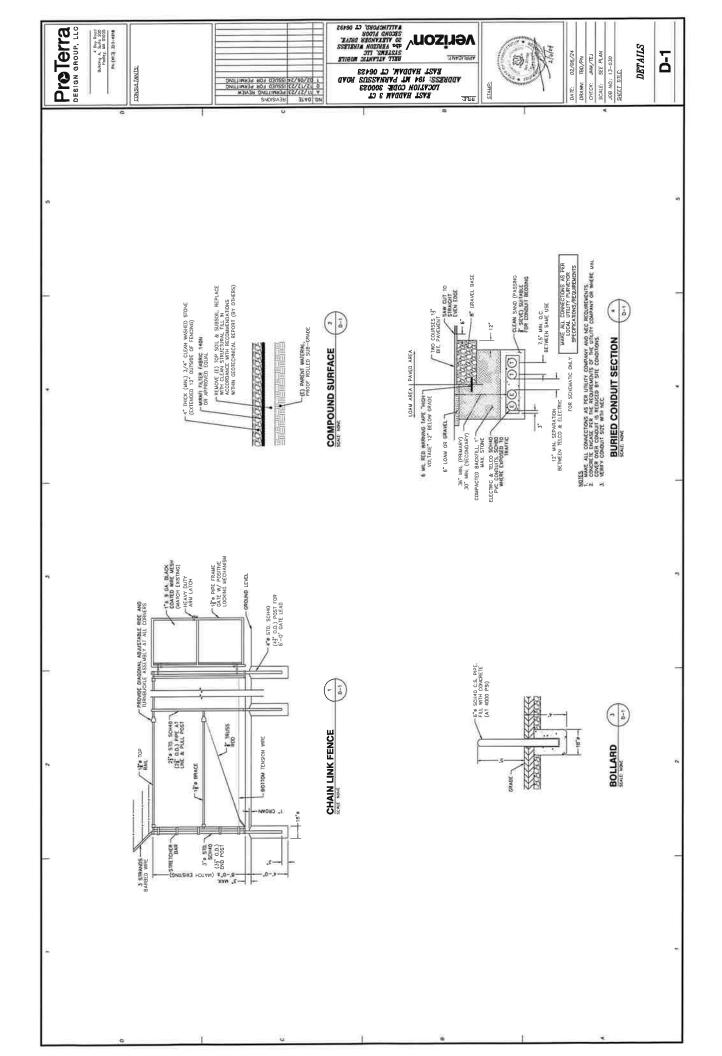
NEW CONSTRUCTION WILL CONFORM TO ALL APPLICABLE CODES AND ORDINANCES. 2022 CONNECTION STATE BUILDING CODE (IRC 2021)
RED MARGINACHIOS. 2020 NEC (AFPA-70) WITH CONNECTION AMENDMENTS

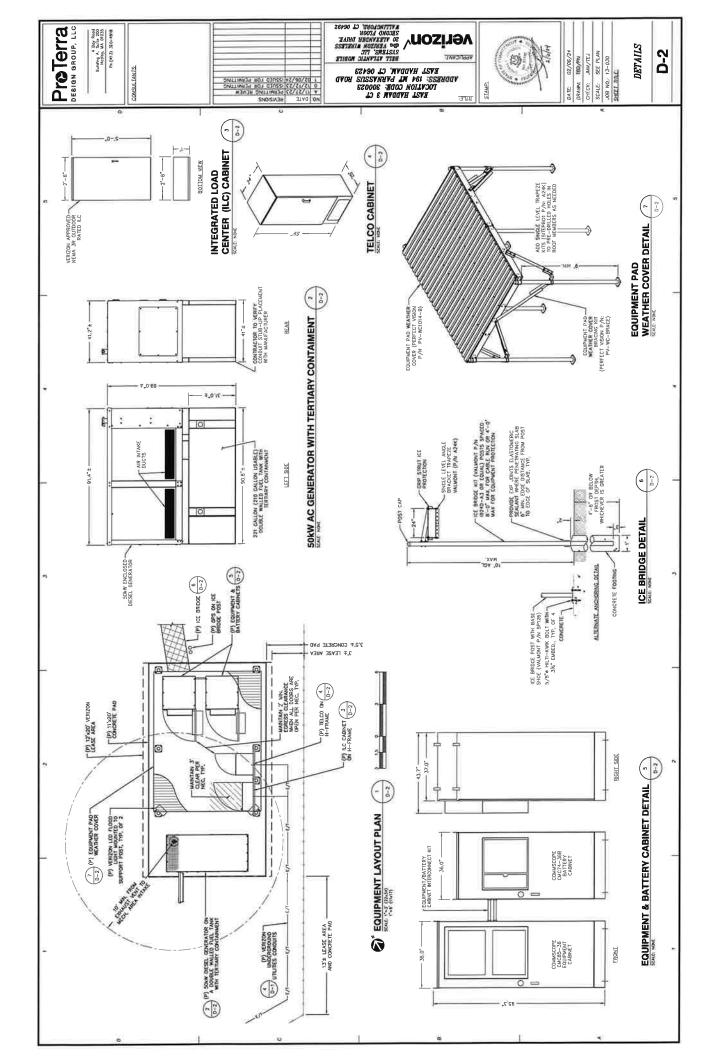
PROJI	PROJECT INFORMATION
SITE TYPE:	CO-LOCATION ON EXISTING SELF-SUPPORT TOWER TO BE EXTENDED
SITE NAME:	EAST HADDAM 3 CT
LOCATION CODE (PSLC):	300023
FUZE 1D:	17133983
RFDS DATE:	08/01/23 REV1
SITE ADDRESS:	194 MT PARNASSUS ROAD EAST HADDAM, CT 06423
COUNTY:	MIDDLESEX
PARCEL #:	039-064
TOWER LATITUDE:	41' 28' 12,04"± N (41,470012'± N) (RECORD SURVEY 1-A)
TOWER LONGITUDE:	72' 24' 39.83"± W (72,411064'± W) (RECORD SURVEY 1-A)
TOWER GROUND ELEV.:	595,0'± (AMSL) (RECORD SURVEY 1-A)
DATUM:	NADB3/NAVD88
STRUCTURE HEIGHT: HIGHEST APPURTENANCE:	161'± (TOP OF SELF-SUPPORT TOWER) 165'± (TOP OF LIGHTNING ROD)
PROPERTY OWNER:	N/F STATE OF CONNECTICUT 2800 BERLIN TURNPIKE NEWINGTON, CT 06131
TOWER OWNER:	STATE OF CONNECTICUT
TOWER OWNER ADDRESS:	STATE OF CONNECTICUT PUBLIC SAFETY DEPT, 1111 COUNTRY CLUB ROAD MIDDLETOWN, CT 06457
APPLICANT:	CCLCO PARTNERSHIP, A DELAWARE GENERAL PARTNERSHIP, GBO "VERZON WRELESS 20 ALEXANDER DENVE, SECOND TION WALLINGFORD, CT 06492
SITE ENGINEER:	PROTERRA DESIGN GROUP, LLC 4 BAY ROAD BULLIDING A, SUITE 200 HADLEY, MA 01035 (413) 320-4918

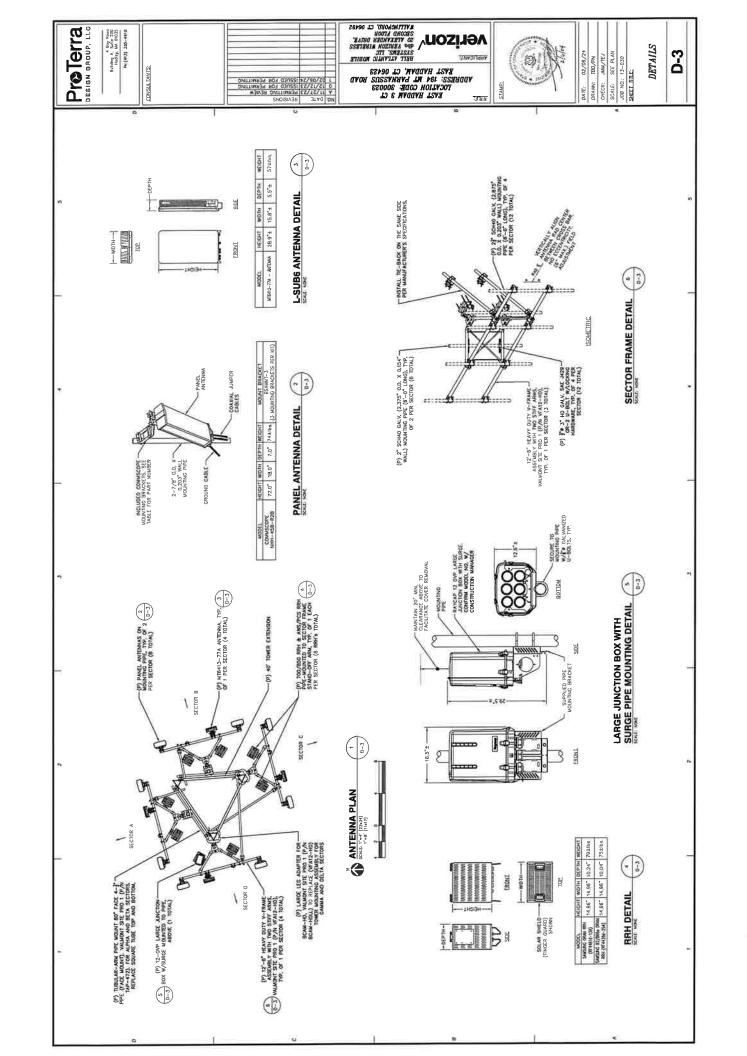
SITE NAME: EAST HADDAM 3 CT LOCATION CODE (PSLC): 300023 PROJECT ID: 17133983 DDRESS: 194 MT PARNASSUS ROAI EAST HADDAM, CT 06423 VICINITY MAP VICINITY MA	SCALE: I' = 2000's (23.34)
--	----------------------------











CELLCO PARTNERSHIP D/B/A VERIZON WIRELESS

ABUTTING PROPERTY OWNERS

194 MOUNT PARNASSUS ROAD EAST HADDAM, CONNECTICUT

	Property Address	Owner's and Mailing Address
1.	122 Shanaghan Road	John Shanaghan and Karen Clifford 176 Mt. Parnassus Road East Haddam, CT 06423
2.	200 Mt. Parnassus Road	Chester and Carol Slabinski 200 Mt. Parnassus Road East Haddam, CT 06423
3.	Mt. Parnassus Road	Richard Clifford and Gale Cummings 29 Laurel Cove Beach Road East Haddam, CT 06423
4.	203 Mt. Parnassus Road	Thomas Comer 203 Mt. Parnassus Road East Haddam, CT 06423