

PAUL R. MICHAUD

Managing Attorney / Principal 515 Centerpoint Drive, Suite 503 Middletown, CT 06457

Direct Telephone: (860) 338-3728 Email: pmichaud@michaud.law Web: www.michaud.law

May 7, 2024

DELIVERED BY E-MAIL AND HAND DELIVERY

Melanie Bachman Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

Re: PETITION NO. 1609 – TRITEC Americas, LLC notice of election to waive exclusion from Connecticut Siting Council jurisdiction, pursuant to Connecticut General Statutes §16-50k(e), and petition for a declaratory ruling, pursuant to Connecticut General Statutes §4-176 and §16-50k, for the proposed construction, maintenance and operation of a 0.999-megawatt AC solar photovoltaic electric generating facility located at 250 Carter Street, Manchester, Connecticut, and associated electrical interconnection. Supplemental Filing - Geotechnical Engineering Report.

Dear Attorney Bachman:

On behalf of TRITEC Americas, LLC ("Petitioner"), please see the enclosed Geotechnical Engineering Report as a Supplemental Filing in the above-referenced Petition. The Petitioner submits fifteen hard copies via hand delivery.

Please feel free to contact me if you have any questions.

Very sincerely yours,

Paul R. Michaud

cc: Service List dated April 30, 2024
John F. Sullivan, Attorney for the Town of Manchester
Raymond Welnicki
Rachel and Dana Schnabel
Rosemary Carroll (MARSD)



March 27, 2024

David Trepeck TRITEC Americas, LLC 888 Prospect Street, Suite 200 LaJolla, CA 92037

RE: Geotechnical Engineering Report

250 Carter Street Solar Development

Manchester, Connecticut Solli Project No.: 23100101

Dear Mr. Trepeck:

Solli Engineering, LLC ("SOLLI") is pleased to submit this geotechnical engineering report for the above referenced project located at 250 Carter Street in Manchester, Connecticut. Refer to Figure 1, Locus Plan, in Appendix 1 for the general location of the project.

Our recommendations are based in part on guidance from the 2022 Connecticut State Building Code, which includes the 2021 International Building Code (IBC) and Connecticut Amendment. Design recommendations are based on Allowable Stress Design Methods.

SITE DESCRIPTION AND PROPOSED CONSTRUCTION

The approximately 41-acre site is located south of Carter Street, east of Amanda Drive, and west of Blue Ridge Drive. The site is undeveloped, lightly wooded, and contains areas of wetlands. Site grades generally slope down from the southeast to northwest at about seven to nine percent.

The proposed solar array will have a footprint of about 6.9 acres and will consist of approximately 2,590 TrinaSolar TSM-DEG19C20 540W modules on single axis tracking systems that will be supported by driven piles with no significant changes to the existing grades within the array. Foundation loads have not been provided; however, we anticipate that pile lengths will be less than 15 feet.

Access to the Project will be from Carter Street via a 12-ft wide, 740± ft long gravel road. The road will extend to the south to provide access to the proposed equipment, and will generate minimal traffic, for the primary use of operation and maintenance of the solar array.

SUBSURFACE EXPLORATIONS

SOLLI observed and documented 12 test pits (TP-1 through TP-12) that were excavated by David M. Koch, LLC of Middlebury, Connecticut on February 19 and 20, 2024. The approximate locations of the test pits are depicted on Figure 2, Exploration Location Plan, in Appendix 1. The test pits were located by GPS and the ground surface elevation at each was estimated from available survey data; this information should be considered approximate. Logs of the test pits are included in Appendix 2.

The test pits were performed to explore the subsurface conditions in the area of the proposed solar array. The test pits were excavated to between seven and ten feet below existing grades with a Caterpillar 304E2 mini excavator that has a bucket capacity of approximately 1/2 cubic yards and a reach of about 10 feet.

One infiltration test was performed at Test Pits TP-4 and TP-5 in general accordance with the 2024 Connecticut Stormwater Quality Manual, the results of which are included with the logs of the test pits in Appendix 2.

LABORATORY TESTING

Representative soil samples collected from the test pits were submitted to a certified laboratory for testing to confirm visual classifications and to evaluate corrosion potential. The laboratory testing was performed in accordance with the American Society for Testing and Materials (ASTM) standards as follows:

- Sieve and hydrometer ASTM D6913 and ASTM D7928
- pH ASTM D4972
- Sulfates ASTM D516
- Chlorides ASTM D512
- Electrical Resistivity ASTM G57

The results of the laboratory testing are included in Appendix 3 and are discussed below.

SUBSURFACE CONDITIONS

Regional Geology

The United States Geologic Survey (USGS) map "Surficial Materials Map of Connecticut" by Stone, J.R., Schafer, J.P., London, E.H., and Thompson, W.B. (1992) indicates the surficial material at the site is thin till described as loose to moderately compact, generally sand, commonly stony.

Connecticut Geological and Natural History Survey, Quadrangle Report 6, titled "The Bedrock Geology of the Rockville Quadrangle" by Aitken, J.M. (1955) indicates the bedrock at the site is Glastonbury Gneiss that is described as light gray, medium to coarse grained, massive to well foliated, compositionally banded metamorphic rock.

General Subsurface Profile

The subsurface profile, as inferred from the test pit data, generally consists of topsoil over subsoil over natural granular soils (e.g., silty sand) to the depths explored, which is consistent with published geologic data. The following are more detailed descriptions of the subsurface materials encountered.

Topsoil

Topsoil was observed at the surface of each test pit that is about six inches thick and consists of dark brown, fine to coarse sand, some silt, trace fine gravel, and trace roots. The effort to advance through this material with the excavation equipment is generally easy.

Subsoil

Subsoil was observed below the topsoil in each test pit and is six to 24 inches thick. The subsoil is light brown, fine to coarse sand with some to and silt, trace fine gravel, and trace roots. The effort to advance through this material with the excavation equipment is generally easy.

Natural Granular Soil

Natural granular soil was observed below the topsoil and subsoil in each test pit and is at least five to nine feet thick. The natural granular soil is brown, fine to medium sand with some to and silt, little fine to coarse



gravel, and trace cobbles. The effort to advance through this material with the excavation equipment generally increases with depth from easy to moderate.

Groundwater

Groundwater was observed in each exploration except Test Pit TP-12. Where encountered, the groundwater is between three and nine feet below existing grades. Please note that groundwater levels may vary depending on factors such as temperature, season, precipitation, and other conditions, which may be different from those at the time of these observations.

GEOTECHNICAL DESIGN RECOMMENDATIONS:

Pile Foundations

Foundation loads have not been provided; however, we understand that HP8x36 piles are typically used for these applications. We recommend adopting the following soil properties for the geotechnical design of the pile foundations:

Strata	Total Unit Weight	Effective Unit Weight	Friction Angle
Topsoil/Subsoil	-	-	-
Natural Granular Soil	125 lb/ft ³	63 lb/ft ³	33 degrees

We recommend neglecting the topsoil and subsoil from contributing to the load carrying capacity of the piles. For axial loading, we recommend the piles be designed in accordance with Section 5.3.2, Bearing Capacity of a Single Pile, of the Naval Facilities Engineering Command Manual DM7.2 (NAVFAC). For lateral loading, we recommend the piles be designed in accordance with Section 5.7, Lateral Load Capacity, of NAVFAC.

Site soils are considered frost susceptible based on their higher fines content and as such the designer should consider uplift forces from adfreeze above the frost depth of the locale, which is 42 inches below final grade. We recommend adopting an average adfreeze bond stress of two kips per square foot (2 ksf) for frost susceptible soils frozen to steel as reported in the Canadian Foundation Engineering Manual, 4th Edition.

Soil Corrosivity

The corrosion potential of site soils was evaluated, and we anticipate they have a low potential as they are predominantly natural; however, if desired, a nominal allowance can be included in the structural design of the piles as a precaution.

Laboratory Test	Laboratory Result	Reference Standard	Corrosion Potential
Electrical Resistivity	38,206 ohm-cm	EPRI Report 1021854	Low
pН	5.5	CALTRANS 2015	Low
Chloride	22 ppm	CALTRANS 2015	Low
Sulfate	<10 ppm	CALTRANS 2015	Low

Seismic Site Class and Liquefaction Potential

We recommend adopting a Site Class "D" (Stiff Soil Profile) per the IBC. The Connecticut State Building Code defines S_S and S_1 for Manchester, Connecticut as 0.192g and 0.055g respectively and we compute S_{MS} as 0.306g, S_{M1} as 0.132g, S_{DS} as 0.204g, S_{D1} as 0.088g. Based on the anticipated groundwater level and material types encountered, and expected peak ground acceleration at this locale, it is our opinion that site soils are not prone to liquefaction.



Equipment Pads

The equipment pads should bear on a six-inch-thick layer of compacted granular fill (CGF) or crushed stone over firm, undisturbed, natural granular soils. The CGF or crushed stone should be placed at least one foot beyond the edges of the equipment pads and down and away at a 1H:1V slope to the top of the natural granular soils. Based on Table 1806.2 of the Connecticut State Building Code, we recommend a maximum allowable bearing pressure of one and one-half tons per square foot (1.5 TSF) for equipment pads bearing on the subgrade described above.

Access Roads

We understand that access roads at the site will be gravel surface roads. We recommend the access roads consist of at least 12-inches of crushed stone over a stabilizing, separation, roadway fabric over firm and stable natural granular soil. Drainage ditches should be provided along the edges of the access roads to direct surface water and runoff away from the road.

Infiltration Rate

We anticipate the bottom of the proposed stormwater basin will be in the natural granular soils below the topsoil and subsoil. We evaluated the USDA soil texture based on our visual classifications and performed infiltration tests in general accordance with 2024 Connecticut Stormwater Quality Manual. The soil texture is Silt Loam, and the infiltration tests indicate an average rate ranging between 0.01 and 0.03 inches per minute.

MATERIALS AND COMPACTION REQUIREMENTS

Compacted Granular Fill (CGF) shall consist of inorganic soil free of clay, loam, ice and snow, tree stumps, roots, and other organic matter and should conform to the requirements of the State of Connecticut Department of Transportation, Standard Specifications for Roads, Bridges, Facilities, and Incidental Construction (CTDOT Form 818) Section M.02.05, Pervious Structure Backfill. We recommend this material be placed within two percent of its optimum moisture content and to a minimum in-place dry density of 95-percent as per ASTM D1557.

Crushed Stone should consist of sound, tough, durable, rock that conforms to the requirements of CTDOT Form 818 Section M.01.02, Coarse Aggregates, for No. 67 stone.

Geotextile road fabric should consist of US200 as manufactured by US Farics or equal.

CONSTRUCTION CONSIDERATIONS:

Pile Foundation Installations

Site soils were observed to be generally easy to moderate to excavate and contain only occasional cobbles with no boulders observed. As such, we anticipate lower effort will be required to install piles and misalignments due to obstructions should be minimal. However, to manage these occasional situations, the contractor should be prepared to pull and reinstall piles that are misaligned and/or cannot reach design tip elevations during conventional installation. Other remedial measure may be available depending on the Contractor's capabilities, which should be reviewed prior to implementation.



Subgrade Preparation

All topsoil and subsoil must be removed from beneath proposed equipment pads and access roads to the top of the natural granular soils prior to placement of new fill or construction. Final subgrades should be proof rolled until firm and stable with a ride-on roller. Areas disturbed during preparation should be undercut to undisturbed material below and backfilled with CGF or crushed stone. Final subgrades should be free of water, ice, frozen soil, and loose soil. Construction on final subgrades should occur as soon as possible after excavation so that disturbance of bearing materials does not occur.

Temporary Excavations

The on-site material is classified as OSHA Class "C" soil and can be cut at a maximum one vertical to one and half horizontal (1V:1.5H) and sloped up to a maximum excavation depth of 20 feet. This maximum slope and excavation depth assumes no surcharge load (i.e. stockpiles, construction equipment, etc.) at the top of the excavations or seepage (e.g. cuts below the groundwater table).

Dewatering

Groundwater is not expected to be encountered during construction. We expect that temporary groundwater or storm water control can be accomplished by means of shallow trenches, sumps, and grading the excavation to low points.

CONSTRUCTION DOCUMENTS AND QUALITY CONTROL:

Project drawings should be provided to SOLLI to review for conformance with the geotechnical recommendations contained herein. If changes are made to the locations or type of structure the recommendations in this report will need to be reviewed. We recommend that SOLLI make field observations of foundation installations to monitor compliance with our recommendations and project specifications.

LIMITATIONS

This report is subject to the limitations included in Appendix 4.

Thank you for the opportunity to be of service. Please feel free to call if you have questions.

Sincerely,

Joseph W. Kidd, P.E. Senior Project Manager

Attachments:

Appendix 1 – Figures

Appendix 2 – Test Pit Logs

Appendix 3 – Laboratory Test Results

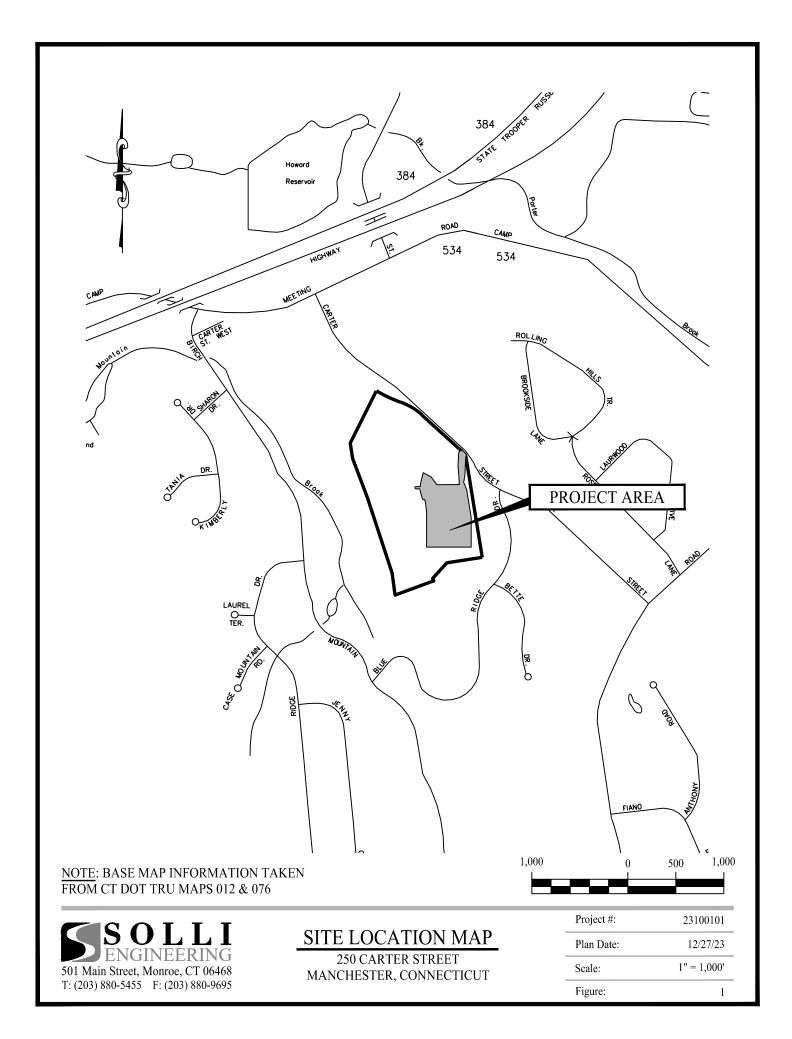
Appendix 4 – Limitations

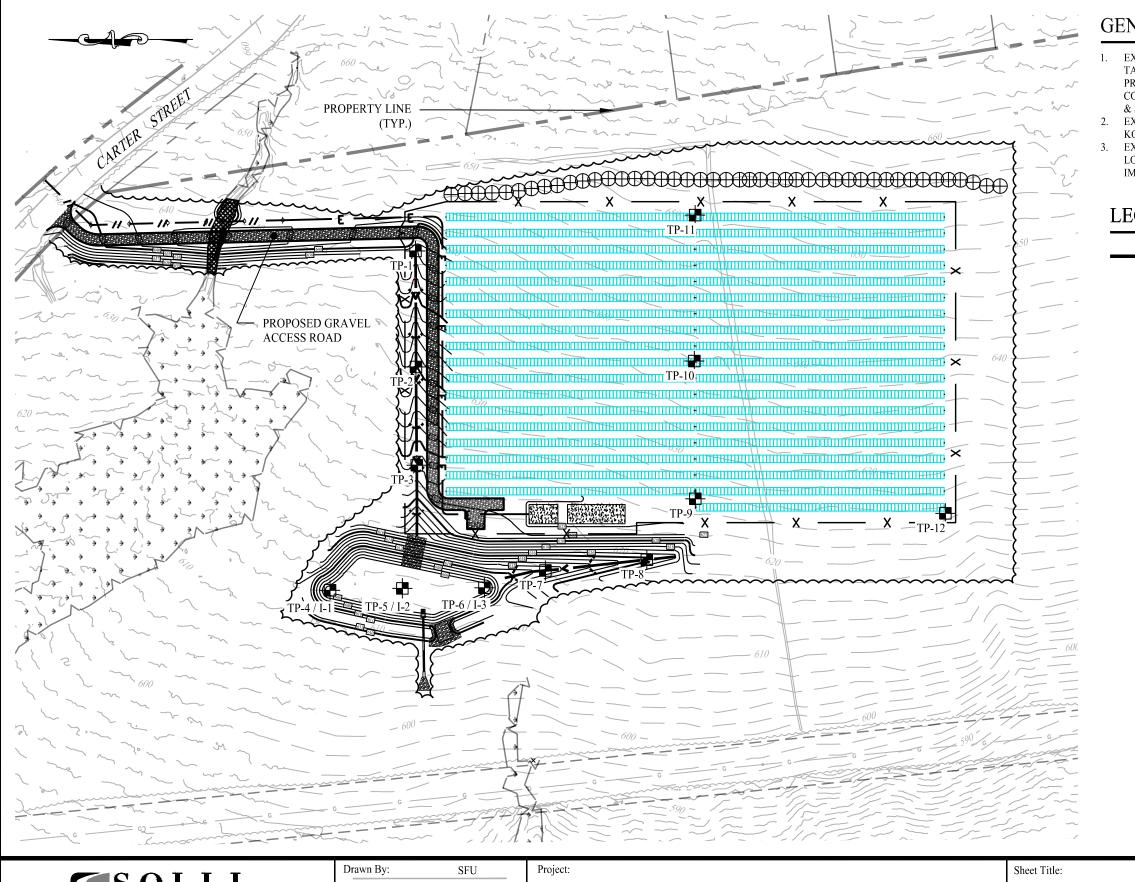


Appendix 1

Figures







GENERAL NOTES

- EXISTING BOUNDARY, TOPOGRAPHY, AND SITE CONDITION INFORMATION TAKEN FROM A PLAN TITLED "PROPERTY & TOPOGRAPHIC SURVEY MAP PREPARED FOR SOLLI ENGINEERING, 250 CARTER STREET, MANCHESTER, CONNECTICUT," DATED OCTOBER 20, 2023, SCALE: 1"=100', BY HARRY E. COLE & SONS
- EXPLORATIONS WERE PERFORMED ON FEBRUARY 19 & 20, 2024 BY DAVID M. KOCH LANDSCAPING LLC
- EXPLORATIONS WERE LOCATED USING A HAND-HELD GPS SYSTEM. THESE LOCATIONS SHOULD BE CONSIDERED ACCURATE ONLY TO THE DEGREE IMPLIED BY THE METHOD USED.

LEGEND

PROPERTY LINE

TP-X / I-X

TEST PIT / INFILTRATION TEST

100 0 50 100

SOLLI ENGINEERING501 Main Street, Monroe, CT 06468
T: (203) 880-5455 | F: (203) 880-9695

 Drawn By:
 SFU

 Checked By:
 JWK

 Project #:
 23100101

 Plan Date:
 02/27/24

 Scale:
 1" = 100'

PROPOSED SOLAR ARRAY

250 CARTER STREET MANCHESTER, CONNECTICUT **EXPLORATION LOCATION PLAN**

FIG. 2

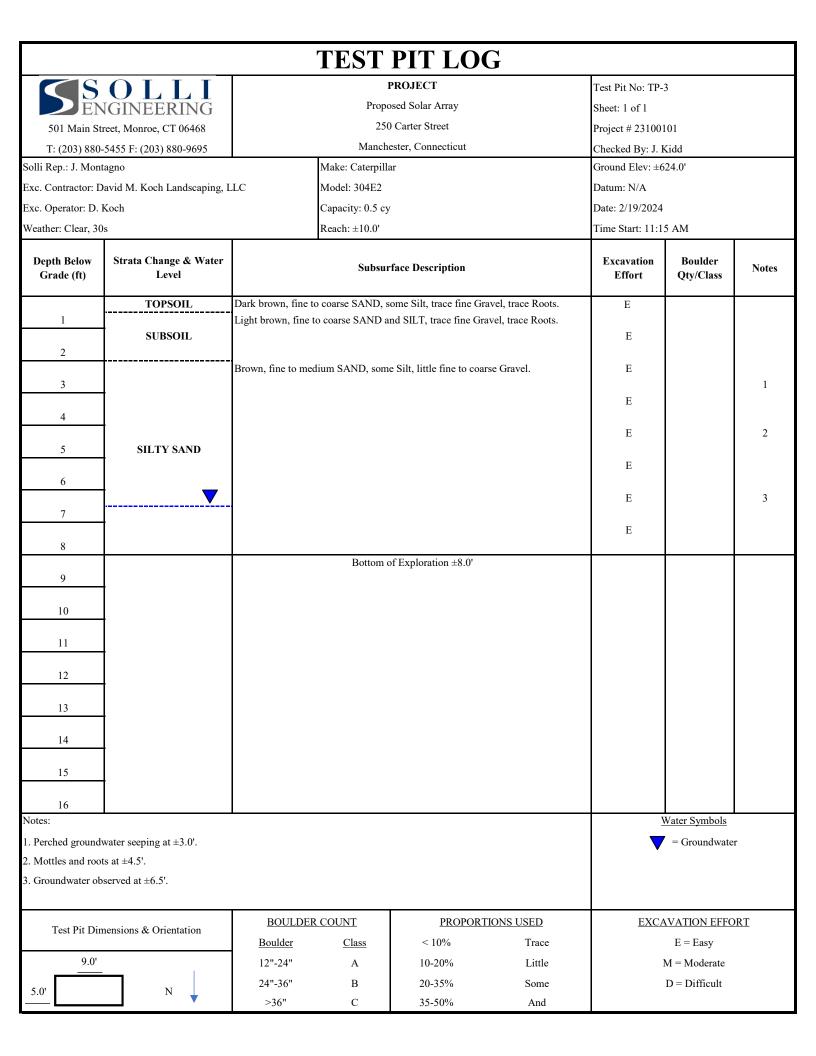
Appendix 2

Test Pit Logs



TEST PIT LOG PROJECT Test Pit No: TP-1 Proposed Solar Array Sheet: 1 of 1 250 Carter Street 501 Main Street, Monroe, CT 06468 Project # 23100101 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Solli Rep.: J. Montagno Ground Elev: ±639.0' Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/19/2024 Time Start: 11:45 AM Weather: Clear, 30s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND and SILT, trace fine Gravel, trace Roots. **SUBSOIL** Е 2 Brown, fine to medium SAND and SILT, little fine to coarse Gravel, Е 3 trace Cobbles. 1, 2 Е 4 Е SILTY SAND 5 Е 6 3 Е 7 Bottom of Exploration ±7.0' 8 9 10 11 12 13 14 15 16 Notes: Water Symbols 1. Mottles and perched groundwater seeping at ± 3.0 '. = Groundwater 2. Roots to ± 3.0 '. 3. Groundwater observed at ± 6.0 '. **BOULDER COUNT** PROPORTIONS USED **EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% E = EasyBoulder Class Trace 10.0' 12"-24" Little M = ModerateΑ 10-20% D = Difficult 24"-36" 20-35% Some В 6.0' N >36" \mathbf{C} 35-50% And

TEST PIT LOG PROJECT Test Pit No: TP-2 Proposed Solar Array Sheet: 1 of 1 250 Carter Street 501 Main Street, Monroe, CT 06468 Project # 23100101 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Solli Rep.: J. Montagno Ground Elev: ±631.0' Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/19/2024 Time Start: 11:30 AM Weather: Clear, 30s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND and SILT, trace fine Gravel, trace Roots. Е **SUBSOIL** 2 Е 3 Brown, fine to medium SAND and SILT, little fine to coarse Gravel. 1, 2 Е 4 Е 5 Е 6 SILTY SAND Е 7 Е 3 8 Е 9 Bottom of Exploration ±9.0' 10 11 12 13 14 15 16 Notes: Water Symbols 1. Mottles and perched groundwater seeping at ± 3.0 '. = Groundwater 2. Roots to ± 3.0 '. 3. Groundwater observed at ± 7.5 '. **BOULDER COUNT PROPORTIONS USED EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% E = EasyBoulder Class Trace 10.0' 12"-24" Little M = ModerateΑ 10-20% D = Difficult 24"-36" 20-35% Some В 4.0' >36" \mathbf{C} 35-50% And



TEST PIT LOG PROJECT Test Pit No: TP-4 Proposed Solar Array Sheet: 1 of 1 250 Carter Street 501 Main Street, Monroe, CT 06468 Project # 23100101 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Ground Elev: ±612.0' Solli Rep.: J. Montagno Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/19/2024 Time Start: 8:30 AM Weather: Clear, 30s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е **SUBSOIL** 2 Е 1 Brown, fine to medium SAND, some Silt, little fine to coarse Gravel. 3 Е 4 Е 5 Е 6 SILTY SAND Е 3 7 Е 8 Е 9 Е 10 Bottom of Exploration ±9.5' 11 12 13 14 15 16 Notes: Water Symbols 1. Perched groundwater seeping at ± 2.5 '. = Groundwater 2. Roots to ± 3.0 '. 3. Test pit started caving. Groundwater level unable to be measured. **BOULDER COUNT** PROPORTIONS USED **EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% Boulder Class Trace E = Easy9.0' 12"-24" Little M = ModerateΑ 10-20% D = Difficult 24"-36" 20-35% Some В 5.0' >36" \mathbf{C} 35-50% And

TEST PIT LOG PROJECT Test Pit No: TP-5 Proposed Solar Array Sheet: 1 of 1 250 Carter Street 501 Main Street, Monroe, CT 06468 Project # 23100101 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Ground Elev: ±613.0' Solli Rep.: J. Montagno Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/19/2024 Time Start: 9:00 AM Weather: Clear, 30s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е **SUBSOIL** 2 Е Brown, fine to medium SAND and SILT, little fine to coarse Gravel, 3 1 trace Cobbles. Е 4 Е 5 Е 2 6 SILTY SAND Е 7 Е 8 Е 9 Е 10 Bottom of Exploration ±9.5' 11 12 13 14 15 16 Notes: Water Symbols 1. Mottles and perched groundwater seeping at ± 3.0 '. = Groundwater 2. Groundwater observed at 6.0' after 15 minutes, 5.5' after 45 minutes. **BOULDER COUNT** PROPORTIONS USED **EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% E = EasyBoulder Class Trace 9.0' 12"-24" Little M = ModerateΑ 10-20% D = Difficult 24"-36" 20-35% Some В 5.0' >36" \mathbf{C} 35-50% And

TEST PIT LOG PROJECT Test Pit No: TP-6 Proposed Solar Array Sheet: 1 of 1 250 Carter Street 501 Main Street, Monroe, CT 06468 Project # 23100101 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Solli Rep.: J. Montagno Ground Elev: ±614.0' Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/19/2024 Time Start: 9:30 AM Weather: Clear, 30s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е **SUBSOIL** 2 Е 1 3 Brown, fine to medium SAND, some Silt, little fine to coarse Gravel, trace Cobbles. Е 4 Е 5 Е SILTY SAND 6 Е 7 3 Е 8 Е 9 Bottom of Exploration ±9.0' 10 11 12 13 14 15 16 Notes: Water Symbols 1. Roots and perched groundwater seeping at ± 2.5 '. = Groundwater 2. Mottles at ± 3.5 '. 3. Groundwater observed at ± 7.0 '. **BOULDER COUNT PROPORTIONS USED EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% E = EasyBoulder Class Trace 9.0' 12"-24" Little M = ModerateΑ 10-20% D = Difficult 24"-36" 20-35% Some В 5.0' >36" \mathbf{C} 35-50% And

TEST PIT LOG PROJECT Test Pit No: TP-7 Proposed Solar Array Sheet: 1 of 1 250 Carter Street Project # 23100101 501 Main Street, Monroe, CT 06468 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Solli Rep.: J. Montagno Ground Elev: ±616.0' Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/19/2024 Time Start: 10:30 AM Weather: Clear, 30s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. **SUBSOIL** Е 2 Brown, fine to medium SAND, some Silt, little fine to coarse Gravel, Е 1 3 with Cobbles. Ε 4 Brown, fine SAND and SILT, little fine to coarse Gravel. Е 5 SILTY SAND Е 6 Е 7 3 Е 8 Bottom of Exploration ±8.0' 9 10 11 12 13 14 15 16 Notes: Water Symbols 1. Roots and perched groundwater seeping at ± 2.5 '. = Groundwater 2. Mottles at ± 3.5 '. 3. Groundwater observed at ± 7.0 '. **BOULDER COUNT** PROPORTIONS USED **EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% Boulder Class Trace E = Easy9.0' 12"-24" Little M = ModerateΑ 10-20% D = Difficult 24"-36" 20-35% Some В 5.0' >36" \mathbf{C} 35-50% And

TEST PIT LOG PROJECT Test Pit No: TP-8 Proposed Solar Array Sheet: 1 of 1 250 Carter Street 501 Main Street, Monroe, CT 06468 Project # 23100101 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Solli Rep.: J. Montagno Ground Elev: ±619.0' Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/19/2024 Time Start: 11:00 AM Weather: Clear, 30s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. **SUBSOIL** Е 2 Brown, fine SAND and SILT, little fine to coarse Gravel. Е 1 3 Е 4 Е 5 SILTY SAND Е 6 Е 7 Е 8 Е 9 3 Е 10 Bottom of Exploration ±9.5' 11 12 13 14 15 16 Notes: Water Symbols 1. Roots to ± 2.5 '. = Groundwater 2. Mottles and perched groundwater seeping at ± 3.5 '. 3. Groundwater observed at ± 9.0 '. **BOULDER COUNT PROPORTIONS USED EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% E = EasyBoulder Class Trace 9.0' 12"-24" Little M = ModerateΑ 10-20% D = Difficult 24"-36" 20-35% Some В 5.0' >36" \mathbf{C} 35-50% And

TEST PIT LOG PROJECT Test Pit No: TP-9 Proposed Solar Array Sheet: 1 of 1 250 Carter Street 501 Main Street, Monroe, CT 06468 Project # 23100101 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Ground Elev: ±626.0' Solli Rep.: J. Kidd Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/20/2024 Time Start: 10:40 AM Weather: Clear, 20s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. **SUBSOIL** Е 2A 2 Brown, fine to coarse SAND, some Silt, little fine to coarse Gravel, trace 1 Cobbles. Е 3 Е 4 M 2 5 M SILTY SAND 6 D 7 1C D 8 D 1B 9 D 10 Bottom of Exploration ±10.0' 11 12 13 14 15 16 Notes: Water Symbols 1. Roots to ± 1.5 '. = Groundwater 2. Perched groundwater seeping at ± 4.0 '. **BOULDER COUNT PROPORTIONS USED EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% E = EasyBoulder Class Trace 10.0' 12"-24" M = ModerateΑ 10-20% Little D = Difficult 24"-36" 20-35% В Some 4.0' >36" \mathbf{C} 35-50% And

TEST PIT LOG PROJECT Test Pit No: TP-10 Proposed Solar Array Sheet: 1 of 1 250 Carter Street 501 Main Street, Monroe, CT 06468 Project # 23100101 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Ground Elev: ±638.0' Solli Rep.: J. Kidd Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/20/2024 Time Start: 8:55 AM Weather: Clear, 20s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. 1C **SUBSOIL** Е 1 2 Brown, fine to coarse SAND, some Silt, little fine to coarse Gravel, trace Cobbles. Е 3 M 4 M 5 M 6 SILTY SAND M 7 M 8 M 9 M 2A 10 Bottom of Exploration ±10.0' 11 12 13 14 15 16 Notes: Water Symbols 1. Roots to ± 1.0 '. = Groundwater 2. Perched groundwater seeping at ± 3.0 '. **BOULDER COUNT PROPORTIONS USED EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% E = EasyBoulder Class Trace 10.0' 12"-24" Little M = ModerateΑ 10-20% D = Difficult 24"-36" 20-35% Some В 3.5' >36" \mathbf{C} 35-50% And

TEST PIT LOG PROJECT Test Pit No: TP-11 Proposed Solar Array Sheet: 1 of 1 250 Carter Street Project # 23100101 501 Main Street, Monroe, CT 06468 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Ground Elev: ±639.0' Solli Rep.: J. Kidd Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/20/2024 Time Start: 9:50 AM Weather: Clear, 20s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е Light brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. **SUBSOIL** Е 1 2 Brown, fine to coarse SAND, some Silt, little fine to coarse Gravel, trace Cobbles. Е 3 Е 4 Е 5 M 6 SILTY SAND M 7 1A M 8 Brown, fine to coarse SAND, some Silt, little fine to coarse Gravel, little M 9 Cobbles. D 10 Bottom of Exploration ±10.0' 11 12 13 14 15 16 Notes: Water Symbols 1. Roots to ± 1.0 '. = Groundwater 2. Perched groundwater seeping at ± 3.0 '. **BOULDER COUNT** PROPORTIONS USED **EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% E = EasyBoulder Class Trace 10.0' 12"-24" M = ModerateΑ 10-20% Little D = Difficult 24"-36" 20-35% В Some 3.5' >36" \mathbf{C} 35-50% And

TEST PIT LOG PROJECT Test Pit No: TP-12 Proposed Solar Array Sheet: 1 of 1 250 Carter Street 501 Main Street, Monroe, CT 06468 Project # 23100101 Manchester, Connecticut T: (203) 880-5455 F: (203) 880-9695 Checked By: J. Kidd Ground Elev: ±625.0' Solli Rep.: J. Kidd Make: Caterpillar Exc. Contractor: David M. Koch Landscaping, LLC Model: 304E2 Datum: N/A Exc. Operator: D. Koch Capacity: 0.5 cy Date: 2/20/2024 Time Start: 9:20 AM Weather: Clear, 20s Reach: ±10.0' **Depth Below** Strata Change & Water **Excavation** Boulder **Subsurface Description** Notes Grade (ft) Level Effort Qty/Class TOPSOIL Dark brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Е SUBSOIL Light brown, fine to coarse SAND, some Silt, trace fine Gravel, trace Roots. Brown, fine to coarse SAND, some Silt, little fine to coarse Gravel, trace Е 1 2 Cobbles. Е 3 Е 4 M 5 1A, 1B M 6 SILTY SAND M 7 1B M 8 Brown, fine to coarse SAND, some Silt, little fine to coarse Gravel, little M 9 Cobbles. 3A D 10 Bottom of Exploration ±9.5' 11 12 13 14 15 16 Notes: Water Symbols 1. Roots to ± 1.0 '. = Groundwater 2. Groundwater not observed. **BOULDER COUNT** PROPORTIONS USED **EXCAVATION EFFORT** Test Pit Dimensions & Orientation < 10% E = EasyBoulder Class Trace 12.0' 12"-24" Little M = ModerateΑ 10-20% D = Difficult 24"-36" 20-35% Some В 4.0' >36" \mathbf{C} 35-50% And



PROPOSED SOLAR ARRAY

250 Carter Street, Manchester, Connecticut

Project # 23100101

Date: February 22, 2024

J. Montagno

Solli Rep.:

Test No.: I-1					Location: TP-4	
Temperature	30°F	Test Type: 4" PVC Casing (solid)			Ground Surface El.	±612.0'
Weather	Clear	USDA Textural Class:			Subgrade El.	$\pm 609.0'$
					Test El.	±607.0'
Time	Elapsed Tim (min.)	Water Depth (in.)	Infiltration Rate (in./min.)		Remarks	
10:15 AM	0	4.0	-	1		
10:20 AM	5	4.1	0.03	2		
	Average Infiltration Rate (in./min.): 0.03					

REMARKS:

- 1. First soak test
- 2. No measurable change in water depth after 5 minutes.

Test No.: I-2	Test No.: I-2 Location: TP-5					
Temperature	30°F	Test Type:	4" PVC Casing (solid)		Ground Surface El.	±613.0'
Weather	Clear	USDA Textural Class:			Subgrade El.	$\pm 609.0'$
					Test El.	±607.0'
Time	Elapsed Tim (min.)	Water Depth (in.)	Infiltration Rate (in./min.)		Remarks	
10:16 AM	0	2.3	-	1		
10:21 AM	5	2.3	0.00			
10:26 AM	10	2.4	0.03			
10:36 AM	20	2.5	0.01	2		
	Average Infiltration Rate (in./min.): 0.01					

REMARKS:

- 1. First soak test
- 2. No measurable change in water depth after 20 minutes.

Appendix 3 Laboratory Test Results





Client: Solli Engineering, LLC

Project: Manchester PV Location: Manchester, CT

Boring ID: --- Sample Type: Bag Tested By: ckg Sample ID: 23100101- TP10 Test Date: 03/01/24 Checked By: ank

Project No:

GTX-318673

Depth: --- Test Id: 760613

Test Comment: ---

Visual Description: Moist, brown silty sand

Sample Comment: ---

pH of Soil by ASTM D4972

Boring ID	Sample ID	Depth	Visual Description	pH of Soil in Distilled Water	pH of Soil in Calcium Chloride
	23100101- TP10		Moist, brown silty sand	5.5	4.3

Notes: Sample Preparation: screened through #10 sieve

Method A, pH meter used



Client: Solli Engineering, LLC Project: Manchester PV Location: Manchester, CT GTX#: 318673 03/01/24 Test Date: Due Date: 03/06/24 Tested By: NMK Checked By: ank

Laboratory Measurement of Soil Resistivity Using the Wenner Four-Electrode Method by ASTM G57 (Laboratory Measurement)

Boring ID	Sample ID	Depth, ft.	Sample Description	Electrical Resistivity, ohm-cm	Electrical Conductivity, (ohm-cm) ⁻¹
	23100101-TP10		Moist, brown silty sand	38,206	2.6174E-05

Notes: Test Equipment: Nilsson Model 400 Soil Resistance Meter, MC Miller Soil Box

Water added to sample to create a thick slurry prior to testing (saturated condition). Electrical Conductivity is calculated as inverse of Electrical Resistivity (per ASTM G57)

Test conducted in standard laboratory atmosphere: 68-73 F



Client: Solli Engineering, LLC Project: Manchester PV

Location: Manchester, CT Project No:

Boring ID: ---Sample Type: Bag Tested By: ckg Test Date: Sample ID: 23100101- TP10 03/02/24 Checked By: ank

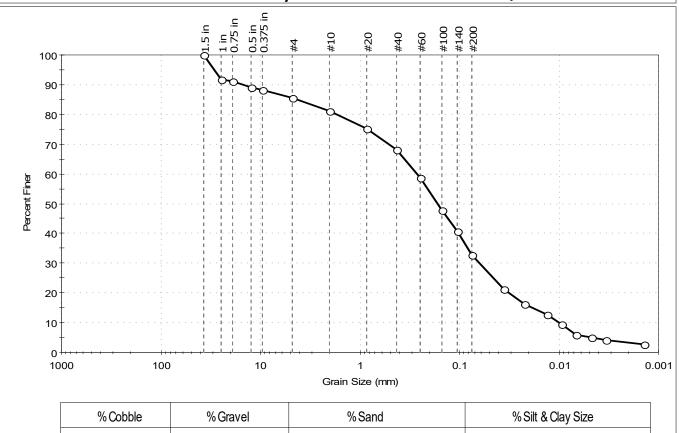
Test Id: 760565 Depth:

Test Comment:

Visual Description: Moist, brown silty sand

Sample Comment:

Particle Size Analysis - ASTM D6913/D7928



Name	Sieve Size, mm Percen	t Finer Spec. Percent (Complies	Coefficients
	_	14.5	52.6	32.9
	% Cobble	% Gravel	% Sand	% Silt & Clay Size

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
1.5 in	37.50	100		
1 in	25.00	92		
0.75 in	19.00	91		
0.5 in	12.50	89		
0.375 in	9.50	88		
#4	4.75	86		
#10	2.00	81		
#20	0.85	75		
#40	0.42	68		
#60	0.25	59		
#100	0.15	48		
#140	0.11	41		
#200	0.075	33		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
	0.0359	21		
	0.0225	16		
	0.0132	13		
	0.0094	9		
	0.0067	6		
	0.0048	5		
	0.0034	4		
	0.0014	3		

<u> </u>	IIICIEIICS
D ₈₅ =4.2819 mm	$D_{30} = 0.0625 \text{ mm}$
D ₆₀ = 0.2680 mm	$D_{15} = 0.0186 \text{ mm}$
D ₅₀ = 0.1655 mm	$D_{10} = 0.0100 \text{ mm}$
$C_u = 26.800$	$C_c = 1.458$

GTX-318673

Classification N/A <u>ASTM</u>

AASHTO Silty Gravel and Sand (A-2-4 (0))

<u>Sample/Test Description</u> Sand/Gravel Particle Shape: ANGULAR

Sand/Gravel Hardness: HARD

Dispersion Device : Apparatus A - Mech Mixer

Dispersion Period: 1 minute Est. Specific Gravity: 2.65

Separation of Sample: #200 Sieve



Client: Solli Engineering, LLC

Project: Manchester PV

Location: Manchester, CT

Sample Type: Bag Boring ID: ---Tested By: ckg Test Date: Sample ID: 23100101- TP3 03/01/24 Checked By: ank

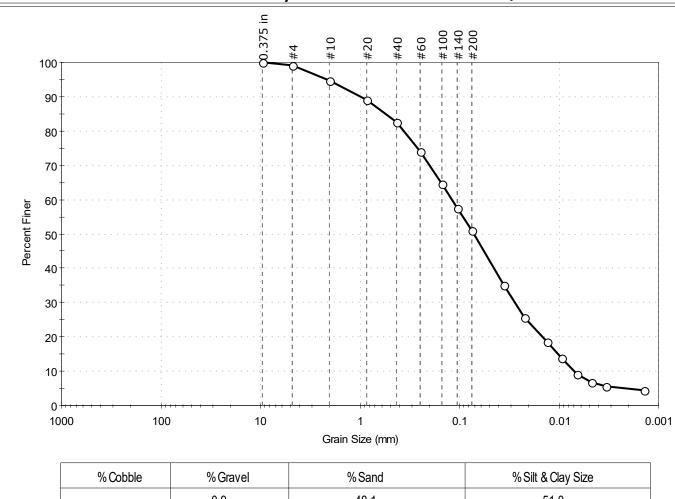
Test Id: 759858 Depth:

Test Comment:

Visual Description: Moist, yellowish brown sandy silt

Sample Comment:

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
_	0.9	48.1	51.0

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	99		
#10	2.00	95		
#20	0.85	89		
#40	0.42	82		
#60	0.25	74		
#100	0.15	65		
#140	0.11	58		
#200	0.075	51		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
	0.0356	35		
	0.0220	26		
	0.0130	19		
	0.0093	14		
	0.0066	9		
	0.0047	7		
	0.0033	6		
	0.0014	5		

<u>Coefficients</u>					
D ₈₅ = 0.5517 mm	$D_{30} = 0.0274 \text{ mm}$				
D ₆₀ = 0.1196 mm	$D_{15} = 0.0100 \text{ mm}$				
D ₅₀ = 0.0717 mm	$D_{10} = 0.0070 \text{ mm}$				
Cu =17.086	$C_c = 0.897$				

Project No:

GTX-318673

Classification <u>ASTM</u> N/A AASHTO Silty Soils (A-4 (0))

<u>Sample/Test Description</u> Sand/Gravel Particle Shape : ---

Sand/Gravel Hardness: ---

Dispersion Device : Apparatus A - Mech Mixer

Dispersion Period: 1 minute Est. Specific Gravity: 2.65

Separation of Sample: #200 Sieve





PO Box 572455 / Salt Lake City UT 84157-2455 / USA TEL +1 801 262 2448 · FAX +1 801 262 9870 · www.TEi-TS.com

Analysis No. TS-A2411753

Report Date 05 March 2024

Date Sampled 29 February 2024

Date Received 04 March 2024

Where Sampled Acton, MA USA

Client

Sampled By

This is to attest that we have examined: Soil: Project: Manchester PV, Andover; Site Location: Manchester, CT; Job Number: GTX-318673

When examined to the applicable requirements of:

ASTM D 512-12* "Standard Test Methods for Chloride Ion in Water" Method B

ASTM D 516-16 "Standard Test Method for Sulfate Ion in Water"

Results:

ASTM D 512 - Chloride Method B

Sample		Results		Minimum
		ppm (mg/kg)	% ¹	Detection Limit
23100101-TP10		22	0.0022	10
		22.	0.0022	10.

NOTE: 1Percent by weight after drying and prepared as per the Standard. *Withdrawn 2021 without Replacement

ASTM D 516 - Sulfates (Soluble)

Sample		Results		Minimum
		ppm (mg/kg)	% ¹	Detection Limit
23100101-TP10		. 10	< 0.0010	10
		< 10.	< 0.0010	10.

NOTE: ¹Percent by weight after drying and prepared as per the Standard.

END OF ANALYSIS

USEPA Laboratory ID UT00930

Merrill Gee P.E. – Engineer in Charge

© 2024 by Testing Engineers International, Inc. CAVEAT: This certificate may not be reproduced except in full, without the expressed written consent of TEi-Testing Services, LLC. Note: The values in this certificate are the values obtained under standard test conditions as reported in the appropriate Report of Test and thus may be used for purposes of demonstrating compliance or for comparison with other units tested under the same standard. The results do not indicate the function of the sample(s) under nonstandard or field conditions. Statement of Risk: Client understands and agrees that declarations of conformity are made by directly comparing the measurement results against the test limits given in the standard without consideration to factors that may contribute to measurement uncertainty and accepts the shared risk that arises from this approach. This certificate gives the characteristics of the sample(s) submitted for testing only. It does not and may not be used to certify the characteristics of the product, nor to imply that the product in general meets the requirements of any standard, nor its acceptability in the marketplace. Tei stylized lettering and logo are registered trademarks and use is by contract and/or written permission only. USEPA Laboratory ID UT00930 TEi-Testing Services is a wholly owned LLC of Testing Engineers International, Inc.

Appendix 4

Limitations



GEOTECHNICAL LIMITATIONS

Explorations

- 1. The analysis and recommendations submitted in this report are based in part upon the data obtained from widely spaced subsurface explorations. The nature and extent of variations between these explorations may not become evident until construction. If variations then appear evident, it will be necessary to reevaluate the recommendations of this report.
- 2. The generalized soil profile described in the text is intended to convey trends in subsurface conditions. The boundaries between strata are approximate and idealized and have been developed by interpretations of widely spaced explorations and samples; actual soil transitions are probably more erratic. For specific information, refer to the test pit logs.
- 3. Water level readings have been made at the times and under the conditions stated on the test pit logs. This data has been reviewed and interpretations made in the text of this report. However, it must be noted that fluctuations in the level of the groundwater may occur due to variations in rainfall, temperature and other factors occurring since the time measurements were made.

Review

4. In the event that any changes in the nature, design or locations of the proposed structures are planned, the conclusions and recommendations contained in this report shall not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing by Solli Engineering, LLC. It is recommended that this firm be provided with the opportunity for a general review of final design and specifications in order that earthwork and geotechnical recommendations may be properly interpreted and implemented in the design and specifications.

Use of Report

5. This report has been prepared for the exclusive use of TRITEC Americas, LLC and their design team for specific application to the Proposed Solar Array that will be located at 250 Carter Street in Manchester, Connecticut in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made.

