Santa Fuel, Inc.

PETITION OF SANTA FUEL, INC. FOR A DECLARATORY RULING THAT A CERTIFICATE OF ENVIRONMENTAL COMPATIBILITY AND PUBLIC NEED IS NOT REQUIRED FOR THE CONSTRUCTION, OPERATION, AND MAINTENANCE OF A 3.85 MEGA WATT (AC) SOLAR PHOTOVOLTAIC POWER GENERATION FACILITY AT 159 SOUTH ROAD IN SOMERS CONNECTICUT

September 14, 2023

Prepared for:

Santa Fuel, Inc 154 Admiral Street Bridgeport CT 06605

Project No. 2023-001

Prepared by:

J.R. Russo & Associates, LLC Land Surveyors & Professional Engineers P.O. Box 938 East Windsor, CT 06088 (860) 623-0569

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I. INTRODUCTION

Pursuant to Conn. Gen. Stat.§§ 4-176(a) and 16-50k(a) and Conn. Agencies Regs.§ 16- 50j-38 *et seq.*, Santa Fuel, Inc, ("Santa Fuel" or the "Petitioner") requests that the Connecticut Siting Council ("Siting Council") approve by declaratory ruling the location, construction, operation, and maintenance of a solar photovoltaic facility capable of up to 3.85 MW AC, and associated equipment ("Project") consisting of approximately 17.11 acres of solar panels to be constructed within a 20 acre lease area located at 159 South Road in Somers, Connecticut (the "Project Site").

Conn. Gen. Stat. § 16-50k(a) provides:

Notwithstanding the provisions of this chapter or title 16a, the council shall, in the exercise of its jurisdiction over the siting of generating facilities, approve by declaratory ruling... the construction or location of any customerside distributed resources project or facility or grid-side distributed resources project or facility with a capacity of not more thansixty-five megawatts, as long as: (i) Such project meets air and water quality standards of the Department of Energy and Environmental Protection, and (ii) the Council does not find a substantial adverse environmental effect...

As discussed in this Petition, the Petitioner's goal is to design and construct an environmentally compatible project that produces the maximum amount of energy while avoiding and minimizing adverse environmental impacts. Based on the information presented in this Petition, the Project will meet the air and water quality standards established by the Department of Energy and Environmental Protection ("DEEP"), and will not cause any substantial adverse environmental effects to the immediate and surrounding area. Accordingly, the construction, operation, and maintenance of the Project satisfies the criteria of Conn. Gen. Stat. § 16-50k(a).

II. PETITIONER

Santa Fuel will be the owner and lead the development of the project. They are based in Bridgeport, Connecticut and have been providing a variety of energy products to the northeast for over 60 years. To assist in the development, they have retained Louth Callan Renewables, LLC (LCR), a Connecticut-based developer of renewable energy having its principal place of business at 921 Thrall Road in Suffield, Connecticut. LCR is a leading developer and operator of solar energy facilities with over 40 years of experience with solar energy development having developed more than 180 MW of commercial solar projects in North America.

Please address all correspondence and/or communications regarding this Petition to:

Kyzer Gardiola Vice President of EPC Louth Callan Renewables LLC 921 Thrall Road Suffield, CT 06078 kyzer@louthcallanrenewables.com

Sue O'Donovan Chief Financial Officer Santa Fuel, Inc 154 Admiral Street, Bridgeport CT 06605 odonovans@santaenergy.com

A copy of all such correspondence and/or communications to the Petitioner's

Engineering Consultant:

Timothy Coon, P.E. Principal Engineer J.R. Russo & Associates, LLC P.O. Box 938 East Windsor, CT 06088 tcoon@jrrusso.com

III. PROPOSED PROJECT

A. **PROJECT BACKGROUND**

In developing this Project, the Petitioner has taken into account the State's energy policy to develop and utilize renewable energy resources, such as solar and wind energy, to the maximum practicable extent. As a solar development, the proposed Project is considered a Class I renewable energy source under General Statutes § 16- 1(a)(26).

The Project creates a significant benefit for the State and its residents. First, the Project will return currently unused land to productive use. Over the course of a 25-year lease between the Company and the landowner, the Project will produce income in the form of rent for the landowner. When the solar array is removed from the Property upon expiration of the lease, the prior agricultural use of the Property can resume, if so desired. During its lifespan, the Project will help to reduce greenhouse gas emissions and pollutants.

B. SITE SELECTION

The Petitioner based the site selection process for the Project on a detailed evaluation of the following key criteria.

- Site suitability (size, topography, and apparent lack of biological and hydrological conflicts in initial screening);
- Site availability and mutual benefits (ability to come to suitable lease terms with landowner; and
- Proposed cost of interconnecting to and proximity to critical infrastructure (suitable electrical grid access).

After performing an initial site evaluation, the Petitioner began a preliminary design of a site layout that would best minimize negative environmental impacts. The Petitioner conducted further due diligence on the Project Site, and entered into a contract option to lease the land. The Petitioner performed outreach within Town government, and met with the Somer's

Conservation Commission and Zoning Commission. In addition, the Petitioner retained the following consultants to assist in the evaluation and design of the Project:

- J.R. Russo & Associates, LLC Civil Engineering/Surveying/Planning
- Richard Zulick Wetland Delineation Report
- Hartgen Archeological Associates, Inc. Archaeologist

C. PROPERTY DESCRIPTION

The Project Site consists of approximately 22.1 acres of undeveloped land, part of a larger 108.5-acre parcel (the Property) located at 159 South Road in Somers, Connecticut. A Vicinity Map is provided as Exhibit I. The Property is owned by the Nancy B. Edgar Revocable Trust & Dianne Bordeaux Lenti. The property is abutted to the north by Geissler's Supermarket plaza and other vacant land of the Nancy B. Edgar Revocable Trust, to the east by undeveloped woodland owned by the Northern Connecticut Land Trust, to the south by residential properties along Stillmeadow Lane and Mountain View Road, and to the west by residential properties along South Road (Rte. 83). Exhibit II includes an aerial map which depicts the surrounding land uses within one-half mile of the Property.

The northeastern portion of the Property and abutting land of the Nancy B. Edgar Revocable Trust to the north was formerly mined for gravel. The gravel mining operation was initiated in 1998 and terminated in 2009. Upon completion of the mining operations, disturbed areas were restored and are currently maintained as hay field. The southeastern portion of the property is undeveloped woodland. The southwestern portion of the property consists of an old orchard that is no longer maintained. An existing single-family home with a couple of barns and a former fruit stand are located adjacent to South Road on the western portion of the Property. Two dug ponds are located in the northwestern portion of the Property. The area around the ponds has become overgrown with brush.

The Project Site consists of a 17.11-acre fenced area surrounding the array and approximately 5 additional acres outside of the fence that will be disturbed for the purpose of shade management, access, stormwater management, and vegetative screening. The site is currently accessible from South Road from the driveway to the existing house and via an existing dirt farm road on the northern end of the Property. The existing house and out-buildings are anticipated to remain. The Project Site will encompass approximately 13.75 acres of the existing hay field, 3.05 acres of undeveloped woodland, and 5.3 acres of the overgrown orchard.

D. PROJECT DESCRIPTION

The proposed solar array is anticipated to include 8,710 PV tracking modules within a 17.11 acre fenced area. Construction activities will include clearing and grubbing, re-grading, layout and placement of foundation systems, racking, solar PV panels, and string inverters; installation of utility pads and associated electrical equipment; installation of electrical conduit, conduit supports; installation of underground transmission lines; installation of utility poles and overhead 3-phase power lines; installation of a gravel access road; and installation of security fencing. A new 15' wide gravel driveway will be installed south of the existing residence to provide access to the Project Site. The access road will extend from South Road to equipment pads located along the eastern edge of the array. The array will be completely enclosed within a 7-foot chain-link security fence with gated access elevated 8" off the ground to allow for small animal movement into and out of the array areas. Detailed Site Plans are provided in Exhibit III.

The PV panels and inverters will be mounted on a driven post racking system. The rows of panels are oriented along the north-south axis, and the panels will rotate to track the sun from east to west. Inverters will be centrally located near the equipment pads. The minimum and maximum height of the panels above grade will be three feet (3') and thirteen feet (13'), respectively. The aisle width between rows of panels will be 11.23 feet. A specification sheet for the anticipated PV module is included as Exhibit IV. However, the PV module is subject to change as additional optimization and market conditions may dictate.

The panels in the northern portion of the project site will be installed at existing grade without the need to disturb the existing soils or vegetation. However, approximately 5 acres in the southern portion of the array will be re-graded to reduce the existing slopes to 15% or less to facilitate construction. In areas to be re-graded, topsoil will be stripped and set as-side, the subsoils re-graded to the appropriate slope, and then the topsoil re-spread and seeded with a pollinator seed mix. Other excavation and grading will be limited to the construction of the access road, equipment pads, trenching for conduit installation, and construction of two infiltration basins along the downgradient, western edge of the array to collect and infiltrate the majority of the runoff from the site.

As discussed above, the project will involve the clearing of approximately 3.04 acres of trees in the southeast corner of the Project Site. This includes approximately 1.92 acres of clearing outside of the perimeter fence for shade management. In the areas outside of the fence, large trees will be removed, but brush and stumps will remain in order to minimize soil disturbance and retain existing vegetation.

Construction of the project is anticipated to begin in the summer of 2024. The project construction period is estimated at 4-6 months from Notice to Proceed. Once

operational, the Project will have a design life of at a minimum, twenty-five (25) years. The anticipated wattage of the Project is 3,850 kW AC. At the end of the operational life of the Project, the Petitioner will remove all equipment (e.g. racking system, panels, inverters, electrical collection system, etc.) from the Project Site. A Decommissioning Plan is provided as Exhibit V. At that point, the land can be reverted back to an agricultural is if desired

E. INTERCONNECTION

The Petitioner proposes interconnecting the Project to an existing 23kV overhead circuit (27H12) that runs along Mountain View Road and is part of the Eversource distribution system. The interconnection will require the extension of circuit 27H12 from the existing termination point at the intersection of Mountain View Road and South Road approximately 670 feet north to the proposed point of interconnection on a new pole to be installed at the site entrance drive. The extension of the existing circuit to the interconnection point is anticipated to utilize existing Eversource poles located on the west side of South Road. A series of new poles for the necessary reclosers and metering will be installed from the interconnection pole to a new customer riser pole alongside of the access driveway. From there the new power line will be run in new underground MV conduits to the proposed pad mounted transformers and switchgear on the eastern edge of the array.

F. LOCAL INPUT & NOTICE

The Petitioner has actively sought input from the Town of Somers land use boards, and remains committed to providing the Town with as much information regarding the Project as possible. In support of this goal, the Petitioner submitted Site Plans to the Town in July and attended the Somers Conservation Commission meeting on August 2, 2023 to present the site plan and solicit feedback. The project was well received, and the Commission voted for their agent to compose a letter in support of the Project.

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This letter is anticipated to be received shortly and will be forwarded upon receipt. In addition, the Petitioner attended a meeting of the Somers Zoning Commission on September 5, 2023 to present the project.. The Zoning Commission did not appear to have any specific issues with the proposed plan; however, they expressed their desire for the public to have the ability to comment on the project. In response, the Commission was informed that it would be up to the Siting Council to decide whether or not a public hearing will be held. The Commission was also informed that all abutters would be notified of the application at the time that it is submitted to the Siting Council, and they can raise questions and provide comments at that time. A copy of the final Site Plans was provided to the Town of Somers Planning Department at the time of the submission of this Petition to the Siting Council.

Additionally, as required by the Regulations of Connecticut State Agencies § 16-50j-40(a), the Petitioner provided notice of its intent to file this Petition to all adjacent property owners and appropriate municipal and state legislative officials. Attached as Exhibit VI, is a copy of the notice, a list of those notified and a map showing the abutting property owners.

IV. POTENTIAL ENVIRONMENTALIMPACTS

The Petitioner and its consultants have completed a comprehensive environmental and cultural resources assessment of the Site. As part of this process, relevant agencies were consulted and environmental impacts were evaluated and mitigated as appropriate. This included a pre-application meeting with the DEEP to discuss the proposed project on August 16, 2023. For these reasons and those addressed further below, this Project avoids, reduces, and mitigates potential environmental impacts.

A. AIR QUALITY

The Project will have no air emissions during operation and only very minor air emissions of regulated air pollutants and greenhouse gases during construction. The Petitioner will control any temporary emissions at the Project Site by enacting appropriate mitigation measures (e.g., water for dust control; avoid mass early morning vehicle startups and excessive idling times, etc.). Accordingly, any potential air effects produced by the Project's construction activities will be *de minimis*. During operation, the Project will not emit regulated air pollutants or greenhouse gases (e.g., PM, VOCs, GHG or ozone). No air permit will be required for either construction or operation of the Project. Moreover, as discussed above, the Project will provide a benefit to Air Quality by eliminating the discharge of CO₂ and other pollutants generated by displacing other fossil fuel burning energy sources.

B. BIOLOGICAL RESOURCES

A review of the Natural Diversity Database (NDDB) map for Somers, Connecticut dated June 2023 shows that the Project Site is not located within any potential locations of State and Federal Listed Species and Critical Habitats (See Exhibit VII). Based on the results of the preliminary screening, no further investigation into the presence of endangered species was conducted.

C. WETLANDS

The state and federal wetlands in the vicinity of the Project were delineated by Richard Zulick, Certified Soil Scientist with Datum Engineering & Surveying LLC, on December 4, 2022. Wetlands identified in the vicinity of the array consist of three excavated ponds. One small pond was identified just off of the subject property in the field to the north at a location topographically cross-gradient from the proposed array. The other two ponds are located east and downgradient of the array, between the open field and South Road. As stated in Mr. Zulick's, Wetland Delineation report provided as Exhibit VIII,

the ponds appear to be man-made by excavation into the water table. The wetland flags were surveyed and mapped, and the wetland resources are shown on the attached Site Plans (Exhibit III.)

The proposed Project was designed to avoid and minimize impacts to the existing wetland resources in the vicinity of the project site. In accordance with the requirements of DEEP's General Permit for the Discharge of Stormwater and Dewatering Wastewaters from Construction Activities (General Permit), a 100-foot buffer has been maintained between all proposed panels in the array and the downgradient wetlands. In addition, an undisturbed buffer of 50 feet has been maintained between any proposed construction activity and all wetlands.

Silt fence will be installed between the construction activities at the array and the downgradient wetlands in order to provide protection from sedimentation. Where trench excavation and conduit installation occur in vegetated areas, the activity should take no more than a day or two. Disturbed areas will be seeded as soon as feasible to facilitate the stabilization of the area and minimize the potential for erosion and sedimentation. All silt fence will be maintained until vegetation becomes established, and the Project will adhere to all requirements of the General Permit. As a result of the limited activity, distance from the wetland, and erosion and sediment control measures to be implemented, no significant adverse impact to the wetland is anticipated as a result of the Project.

D. STORMWATER MANAGEMENT

As discussed above, the northern portion of the array will be installed at existing grades and maintain existing vegetation. However, much of the southern portion of the array will be re-graded in order to achieve slopes of 15% or less to facilitate the construction of the racking system. Additional soil disturbance will be associated with the stump removal where clearing is required within the fenced array area, as well as for the construction of the two infiltration basins, access drive and equipment pads. In addition to the perimeter silt fence to be installed

downgradient of all construction activities, the material generated from the clearing activities will be chipped/ground to generate a coarse woody debris that will be installed in rows along the contours in the southern portion of the site that has been re-graded to provide additional filtering and maintain sheet flow.

The proposed tracker solar arrays will be installed on elevated racks that provide adequate height above the ground to promote the continued growth of the existing vegetative cover and allow for infiltration. Thus, the areas between and surrounding the panels were analyzed as pervious vegetated cover. Two stormwater management basins will be constructed downgradient of the array in order collect the runoff and provide treatment, groundwater recharge, and retention of the stormwater. These basins have been designed in accordance with the Connecticut Stormwater Quality Manual and the DEEP's General Permit for the Discharge of Stormwater and Dewatering Wastewaters from Construction Activities ("General Permit").

A detailed Drainage Report has been prepared by J.R. Russo & Associates, LLC (Exhibit IX). As detailed in the report, the development of the site is anticipated to result in a reduction of runoff from the site. Other temporary soil erosion and sedimentation control measures will include silt fencing, fiber rolls, anti-tracking pads, outlet protection, woody debris berms, and permanent seeding to stabilize disturbed soils as soon as possible during construction. With these measures, the completed development is not anticipated to have an adverse impact to the surrounding water and wetland resources.

Since the construction will disturb more than 1 acre of land, the Petitioner must register under the DEEP's General Permit at least sixty (60) days prior to commencing construction activities. The Petitioner will prepare a Stormwater Pollution Control Plan, submit it to the DEEP for review, and register under the General Permit in accordance with the requirements and timelines established by the General Permit.

E. FLOODPLAINS

The attached Federal Emergency Management Agency (FEMA) Flood Map (Exhibit X) indicates that the Project is not located within the 100-year flood zone. As a result, the proposed project is not expected to have an impact on the floodplain.

F. DRINKING WATER RESOURCES

The availability of the municipal water supply appears to terminate at the northern end of the subject property. Thus, existing houses at and surrounding the subject property appear to be served by private wells. The proposed activities associated with the Project do not involve the withdrawal of water, nor the storage or use of oil or hazardous materials (other than what is present in the construction equipment). Any water utilized during construction for dust control will be minimal. Thus, the proposed project is not anticipated to have an impact on the water supply and quality in the vicinity of the Site.

A review of the Connecticut Aquifer Protection Area Map prepared by the CT DEEP Aquifer Protection Area Program (Exhibit XI) indicates that the Project is not located within an area identified as an Aquifer Protection Area. The nearest Aquifer Protection Area is located approximately over three (3) miles to the west of the Project Site. Based on the separation distance, the proposed project is not anticipated to have an impact on the Aquifer Protection Area.

G. HISTORIC RESOURCES

On March 7, 2023, a request was submitted to the Connecticut State Historic Preservation Office (SHPO) for review of the Project in relation to historic and archaeological resources. SHPO's response dated March 21, 2023 is provided as Exhibit XII. Based on the environmental characteristics of the Site, SHPO determined that the Project has the potential to contain significant archeological resources and requested a professional archaeological assessment and reconnaissance survey be completed prior to construction. Based on SHPO's request, Hartgen Archeological Associates, Inc. (Hartgen) was retained to conduct a Phase 1 archeological reconnaissance investigation at the Project Site. The assessment was completed in June 2023. The Investigation Report prepared by Hartgen is provided as Exhibit XIII. The report concludes that no artifacts of historical or archeological significance were identified and recommended that no further investigation was required. This report was submitted to SHPO for review and approval. SHPO's response letter dated July 25, 2023 (Exhibit XIV) concurs that no historic property will be affected by the proposed development and no additional archaeological investigations are warranted.

H. SCENIC VALUES & VISUAL IMPACTS

No state or local designated scenic roads or scenic areas are located near the Project Site and therefore none will be physically impacted by the development. Similarly, no state parks or trails are located in the vicinity of the project site.

Other than the existing residential home to remain on the Subject Property, the nearest residence is approximately 80 feet south of the southwest corner of the array along South Road. Residential homes are also located on the opposite side of South Road south of the site entrance. The nearest of these homes to the array is approximately 300 feet. In order to mitigate potential visual impacts of the southern portion of the array to these nearby residences, a row of evergreens is proposed along the southern and western limits of the array. As a result of the existing buffers and proposed evergreen screen, the Project is not anticipated to have any significant adverse visual impacts on nearby sensitive receptors. Furthermore, the use of low-profile Project components that will be no greater than thirteen (13) feet above grade (e.g., racking system, panels, inverters, etc.) also significantly reduces potential visible impact.

I. PUBLIC HEALTH AND SAFETY

Overall, the Project will meet or exceed all health and safety requirements applicable for electric power generation. Each employee working on the Project Site will:

- Receive required general and site-specific health and safety training;
- Comply with all health and safety controls as directed by local and state, requirements;
- Understand and employ the Site health and safety plan;
- Know the location of local emergency care facilities, travel times, ingress and egress routes; and
- Report all unsafe conditions to the construction manager.

During construction, heavy equipment and construction vehicles will be required to access the Project Site during normal working hours (7 a.m. to 7 p.m. Monday through Saturdays; Sundays only as required). After construction is complete and during operation, traffic to the Site will be limited to one to two light-duty vehicles on a quarterly recurring basis for the standard operations and maintenance activities. There will not be a permanent staff present at the Site, and the facility will be monitored remotely.

The Project will not produce significant noise during operation. During the construction of the Project, higher levels of noise are anticipated. However, all work will be conducted during normal working hours and it is not anticipated that the levels of noise will exceed State or local noise standards or limits. Commercial transformers will be located on the eastern side of the array and will be limited to 61 decibels which will not be heard on abutting properties.

Because the solar modules are designed to absorb incoming solar radiation and minimize reflectivity, only a small percentage of incidental light will be reflected off the panels. This incidental light is significantly less reflective than common building materials, such as steel, and the surface of smooth water.

Prior to beginning the Project operation, the Petitioner will meet with Town first responders to provide them information regarding response to emergencies at PV facilities, discuss industry best practices, and provide a tour of the Site. The first responders will also be provided keys to the facility gates so that, in the event of a fire or emergency requiring site access, they will have access to the sites.

J. FEDERAL AVIATION ADMINISTRATION NOTIFICATION

Pursuant to 14 CFR § 77.9 regarding the Federal Aviation Administration (FAA) Notice of Proposed Construction or Alteration, an evaluation was performed using the FAA's on-line Notice Criteria Tool. Based on the proximity to the nearby airports, the Notice Criteria Tool concluded that FAA notification of the project is not required. A copy of the FAA Determination is included as Exhibit XV.

K. CORE FOREST

"Core Forest" is defined as unfragmented forest land that is 300 feet or greater from the boundary between the forest land and non-forest land. As mentioned above, the development will involve the clearing of approximately 3.04 acres of existing woodland in the southeast corner of the Project Site. However, all of the clearing will be within 300 feet of the existing boundary between the forest land and non-forest land. Thus, none of the trees to be removed are within the "Core Forest".

L. FARMLAND CLASSIFICATION

According to mapping prepared by the U.S. Department of Agriculture's Natural Resources Conservation Service available on the Web Soil Survey, the northern and eastern portions of the Project Site, including the existing hayfield and orchard area, are classified as Farmland of Statewide Importance. The remaining wooded southwestern portion of the project site is classified as Not Prime Farmland. As discussed above, the hayfield area has all been previously disturbed by historic gravel mining operations. Regardless, as a result of no soils on the Project Site being classified as Prime Farmland, consultation with the Department of Agriculture and receipt of a "letter of no material affect" is not required. A Farmland Classification Map is provided as Exhibit XVI.

V. CONCLUSION

The Project will provide numerous and significant benefits to the Town. State and its citizens, and will provide a step toward the State's goal of achieving cleaner, less expensive, and more reliable sources of energy. This development of a source of green energy will produce substantial environmental benefits with minimal environmental impacts. Pursuant to CGS § 16-50k(a), the Siting Council shall approve by declaratory ruling the construction or location of a customer-side distributed resources project or facility with a capacity of not more than sixty-five (65) MW, as long as such project meets DEEP air and water quality standards and will not have a substantial adverse environmental effect. As demonstrated within this Petition, the Project meets the criteria.

Accordingly, and for the reasons stated herein, because the proposed Project will meet state air and water quality standards and will not have a substantial adverse effect on the environment, the Petitioner requests that the Siting Council approve the location and construction of the proposed Project by declaratory ruling.

Respectfully submitted, Santa Fuel, Inc.

By: Ano hu

Name: Sue O'Donovan Title: Chief Financial Officer

EXHIBIT I

VICINITY MAP



EXHIBIT II

LAND USE MAP



Somers Louth Callan R	Solar Senewables, LLC
159 Sou Somers, C	uth Road Connecticut
	<u>DATE</u> 9-5-2023
	<u>SCALE</u> 1*=2,000'
	<u>Job Number</u> 2023-001
J.R. RUSSO & ASSOCIATES, LLC 1Shoham Rd East Windsor, CT 08088 • CT 880.623.0569 • MA 4/3785.158 www.jrrusso.com • info@jrrusso.com	sheet EXHIBIT II

<u>SOURCE:</u> MICROSOFT BING © 2023 Microsoft Corportation © 2023 Maxar © CNES (2023) Distribution Airbus DS

EXHIBIT III

SITE PLANS (BOUND SEPARATELY) EXHIBIT IV

PV MODULE SPECIFICATIONS & TCLP TESTING RESULTS



VSUN550-144BMH-DG



VSUN, a BNEF Tier-1 PV module manufacturer invested by Fuji Solar, has been committed to providing greener, cleaner and more intelligent renewable energy solutions. VSUN is dedicated to bringing reliable, customized and high-efficient products into various markets and customers worldwide





Engineered in Japan www.vsun-solar.com

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Electrical Characteristics at Standard Test Conditions(STC)

Module Type	VSUN550-144BMH-DG	VSUN545-144BMH-DG	VSUN540-144BMH-DG	VSUN535-144BMH-DG
Maximum Power - Pmax (W)	550	545	540	535
Open Circuit Voltage - Voc (V)	49.92	49.81	49.65	49.5
Short Circuit Current - Isc (A)	13.99	13.92	13.85	13.78
Maximum Power Voltage - Vmpp (V)	42	41.8	41.65	41.5
Maximum Power Current - Impp (A)	13.1	13.04	12.97	12.9
Module Efficiency	21.52%	21.32%	21.13%	20.93%
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Standard Test Conditions (STC): irradiance 1,000 W/m²; AM 1,5; module temperature 25°C. Pmax Sorting : 0~5W. Measuring Tolerance: ±3%. Remark: Electrical data do not refer to a single module and they are not part of the offer. They only serve for comparison among different module types.

Electrical Characteristics with different rear side power gain(reference to 545 front)

			<u> </u>			_
Pmax (W)	Voc (V)	Isc (A)	Vmpp (V)	Impp (A)	Pmax gain	
575	49.76	14.69	41.80	13.76	5%	
602	49.76	15.39	41.80	14.41	10%	
656	49.81	16.79	41.75	15.72	20%	
684	49.81	17.49	41.75	16.38	25%	

Temperature Characteristics

_		_	
NOCT	45°C(±2°C)	Maximum System Voltage [V]	1500
Voltage Temperature Coefficient	-0.27%/°C	Series Fuse Rating [A]	30
Current Temperature Coefficient	+0.048%/°C	Bifaciality	70%±10%
Power Temperature Coefficient	-0.32%/°C		

Maximum Ratings

Material Characteristics

Dimensions	2256×1133×35mm (L×W×H)
Weight	32.5kg
Frame	Silver anodized aluminum profile
Front Glass	High transparency, Antireflection coated, Semi-toughened safety glass, 2.0mm
Cell Encapsulation	EVA (Ethylene-Vinyl-Acetate) or POE
Back Glass	Glazed & Semi-toughened safety glass, 2.0mm
Cells	12×12 pieces bifacial monocrystalline solar cells series strings
Junction Box	IP68, 3 diodes
Cable&Connector	Potrait: 500 mm (cable length can be customized) , 1×4 mm 2 , compatible with MC4
Packaging	System Design

Dimensions(L×W×H)	2290×1125×1253mm	Temperature Range	-40 °C to + 85 °C
Container 20'	150	Withstanding Hail	Maximum diameter of 25 mm with
Container 40'	300		impact speed of 23 m/s
Container 40'HC	600	Maximum Surface Load	5,400 Pa
		Application class	class A

Dimensions Note:mm <u>—</u>В A-A Frame Current/A B-B D Frame 1400 ± 1 2256 ± 1 990±1 400 ± 1 30 Voltage/V KUIIIIII 13 25°C -Mounting Hole $1092\!\pm\!1$ 8 place rrent/A >C Mounting Hole 30 Voltage/V 1133±1 FRONT VIEW 1092±1 BACK VIEW

IV-Curves





TEST REPORT

CLIENT DETAILS		LABORATORY DETA	ILS
Contact Client	- VIETNAM SUNERGY JOINT STOCK COMPANY	Manager Laboratory	SGS-CSTC Environment Laboratory
Address	LOT III-DONG VANG AREA,DINH TRAM INDUSTRIAL ZONE,VIET YEN DISTRICT,BAC GIANG PROVINCE 230000	Address	2/F, 3RD BUILDING NO. 889, YISHAN ROAD, XUHUI DISTRICT, SHANGHAI, CHINA
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Order Number Samples Project	- Solid waste(2) -	Report Number SGS Reference Date Reported	SHE23-03234 R0 0000275298 2023/06/30
		Analysis Date	2023/06/13 - 2023/06/30

COMMENTS

1. The results apply to the sample(s) as received.

-SIGNATORIES-

窦卓文

Reported by

三百

Reviewed by

唐黎政

Approved by



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符号表/Legend

- "-" 未测试该参数或不适用/The parameter is not tested or not applicable
- ↑ 提高检出限/Detection limit raised
- ↓ 降低检出限/Detection limit lowered

ND 未检出/Not Detected



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TEST REPORT

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			Sa Si Sam R	mple Number ample Name Fest Object ple Description eceive Date	23-03234.001 VSUN545-144BMH-DG Solid waste SHES2306011243TX 2023/06/13
Parameter	Method	Units	MDL	Limit	Testing Results
Arsenic (As)	USEPA 200.8	mg/L	0.050	≤5	ND
Barium (Ba)	USEPA 200.8	mg/L	0.010	≤100	0.336
Cadmium (Cd)	USEPA 200.8	mg/L	0.001	≤1	0.001
Chromium (Cr)	USEPA 200.8	mg/L	0.010	≤5	ND
Lead (Pb)	USEPA 200.8	mg/L	0.010	≤5	4 98
Selenium (Se)	USEPA 200.8	mg/L	0.050	≤1	ND
Silver (Ag)	USEPA 200.8	mg/L	0.010	≤5	ND
Mercury (Hg)	USEPA 7473	mg/L	0.005	≤0.2	ND
2,4-D*	USEPA 8151A	mg/L	0.0005	≤10	ND
2,4,5-TP (Silvex, Fenopop)	USEPA 8151A	mg/L	0.0005	≤1	ND
Benzene	USEPA 8260D	mg/L	0.0005	≤0.5	ND
Carbon tetrachloride	USEPA 8260D	mg/L	0.0005	≤0.5	ND
Chlorobenzene	USEPA 8260D	mg/L	0.0005	≤100	ND
Chloroform	USEPA 8260D	mg/L	0.0005	≤6	ND
1,4-Dichlorobenzene	USEPA 8260D	mg/L	0.0005	≤7.5	ND
1,2-Dichloroethane	USEPA 8260D	mg/L	0.0005	≤0.5	ND
1,1-Dichloroethene	USEPA 8260D	mg/L	0.0005	≤0.7	ND
2-butanone(MEK)	USEPA 8260D	mg/L	0.020	≤200	ND
Tetrachloroethene	USEPA 8260D	mg/L	0.0005	≤0.7	ND
Trichloroethene	USEPA 8260D	mg/L	0.0005	≤0.5	ND
Vinyl chloride	USEPA 8260D	mg/L	0.0005	≤0.2	ND
Methylphenol 1	USEPA 8270E	mg/L	0.001	≤200	ND
2-Methylphenol	USEPA 8270E	mg/L	0.0005		ND
3&4-Methylphenol	USEPA 8270E	mg/L	0.0005		ND
2,4-Dinitrotoluene	USEPA 8270E	mg/L	0.0005	≤0.13	ND
Hexachlorobenzene	USEPA 8270E	mg/L	0.0005	≤0.13	ND
Hexachlorobutadiene	USEPA 8270E	mg/L	0.0005	≤0.5	ND
Hexachloroethane	USEPA 8270E	mg/L	0.0005	≤3	ND
Nitrobenzene	USEPA 8270E	mg/L	0.0005	≤2	ND
Pentachlorophenol	USEPA 8270E	mg/L	0.0025	≤100	ND
Pyridine	USEPA 8270E	mg/L	0.002	≤5.0	ND
2,4,5-Trichlorophenol	USEPA 8270E	mg/L	0.0005	≤400	ND
2,4,6-Trichlorophenol	USEPA 8270E	mg/L	0.0005	≤2	ND
Chlordane(Total) ²	USEPA 8270E	mg/L	0.001	≤0.03	ND
Endrin	USEPA 8270E	mg/L	0.0005	≤0.02	ND
ү-ВНС	USEPA 8270E	mg/L	0.0005	≤0.4	ND
Toxaphene	USEPA 8270E	mg/L	0.050	≤0.5	ND
γ-Chlordane	USEPA 8270E	mg/L	0.0005	-	ND
a-Chlordane	USEPA 8270E	mg/L	0.0005	-	ND
Methoxychlor	USEPA 8270E	mg/L	0.0005	≤10	ND
Heptachlor	USEPA 8270E	mg/L	0.0005	≤0.008	ND



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TEST REPORT

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			Sar Sa T Samı Re	nple Number Imple Name est Object ole Description eceive Date	23-03234.002 VSUN545-144BMH-DG Solid waste SHES2306011243TX 2023/06/13
Parameter	Method	Units	MDL	Limit	Testing Results
Arsenic (As)	USEPA 200.8	mg/L	0.050	≤5	ND
Barium (Ba)	USEPA 200.8	mg/L	0.010	≤100	0.223
Cadmium (Cd)	USEPA 200.8	mg/L	0.001	≤1	0.001
Chromium (Cr)	USEPA 200.8	mg/L	0.010	≤5	0.013
Lead (Pb)	USEPA 200.8	mg/L	0.010	≤5	4.97
Selenium (Se)	USEPA 200.8	mg/L	0.050	≤1	ND
Silver (Ag)	USEPA 200.8	mg/L	0.010	≤5	ND
Mercury (Hg)	USEPA 7473	mg/L	0.005	≤0.2	ND
2,4-D*	USEPA 8151A	mg/L	0.0005	≤10	ND
2,4,5-TP (Silvex, Fenopop)	USEPA 8151A	mg/L	0.0005	≤1	ND
Benzene	USEPA 8260D	mg/L	0.0005	≤0.5	ND
Carbon tetrachloride	USEPA 8260D	mg/L	0.0005	≤0.5	ND
Chlorobenzene	USEPA 8260D	mg/L	0.0005	≤100	ND
Chloroform	USEPA 8260D	mg/L	0.0005	≤6	ND
1,4-Dichlorobenzene	USEPA 8260D	mg/L	0.0005	≤7.5	ND
1,2-Dichloroethane	USEPA 8260D	mg/L	0.0005	≤0.5	ND
1,1-Dichloroethene	USEPA 8260D	mg/L	0.0005	≤0.7	ND
2-butanone(MEK)	USEPA 8260D	mg/L	0.020	≤200	ND
Tetrachloroethene	USEPA 8260D	mg/L	0.0005	≤0.7	ND
Trichloroethene	USEPA 8260D	mg/L	0.0005	≤0.5	ND
Vinyl chloride	USEPA 8260D	mg/L	0.0005	≤0.2	ND
Methylphenol 1	USEPA 8270E	mg/L	0.001	≤200	ND
2-Methylphenol	USEPA 8270E	mg/L	0.0005	-	ND
3&4-Methylphenol	USEPA 8270E	mg/L	0.0005		ND
2,4-Dinitrotoluene	USEPA 8270E	mg/L	0.0005	≤0.13	ND
Hexachlorobenzene	USEPA 8270E	mg/L	0.0005	≤0.13	ND
Hexachlorobutadiene	USEPA 8270E	mg/L	0.0005	≤0.5	ND
Hexachloroethane	USEPA 8270E	mg/L	0.0005	≤3	ND
Nitrobenzene	USEPA 8270E	mg/L	0.0005	≤2	ND
Pentachlorophenol	USEPA 8270E	mg/L	0.0025	≤100	ND
Pyridine	USEPA 8270E	mg/L	0.002	≤5.0	ND
2,4,5-Trichlorophenol	USEPA 8270E	mg/L	0.0005	≤400	ND
2,4,6-Trichlorophenol	USEPA 8270E	mg/L	0.0005	≤2	ND
Chlordane(Total) ²	USEPA 8270E	mg/L	0.001	≤0.03	ND
Endrin	USEPA 8270E	mg/L	0.0005	≤0.02	ND
A-RHC	USEPA 8270E	mg/L	0.0005	≤0.4	ND
Ioxaphene	USEPA 8270E	mg/L	0.050	≤0.5	ND
γ-Chiordane	USEPA 8270E	mg/L	0.0005	(-).	ND
	USEPA 8270E	mg/L	0.0005	-	ND
Methoxychlor	USEPA 8270E	mg/L	0.0005	≤10	ND
Heptachlor	USEPA 8270E	mg/L	0.0005	≤0.008	ND

Remark:

1.Methylphenol are the sum of 2-Methylphenol and 3&4-Methylphenol.

2.Chlordane(Total) are the sum of α-Chlordane and γ-Chlordane.

3.Preparative method:USEPA1311-1992(Toxicity Characteristic Leaching Procedure)

4. The Limits comes from CFR(code of federal regulations) title 40 part 261.24.

5.SHE23-03234.001sample No.:CLD4F8201230530905793,SHE23-03234.002 sample No.:CLD4F8201230530904413

6.*:Not certificated by CNAS



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Method List

USEPA 200.8-1994 Determination of trace elements in waters and wastes by inductively coupled plasma-mass spectrometry USEPA 7473-2007 Metals-Hg USEPA 8151A-1996 Acid Herbicides in Water by GC-MS USEPA 8260D-2018 VOCs USEPA 8270E-2018 SVOCs



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Method:USEPA 200.8-1994

Equipment Name	Model	Equipment Number	· Serial Number
ICP-MS	Agilent 7900	CHEM-998	JP16311502
Method:USEPA 7473-2007			4
Equipment Name	Model	Equipment Number	Serial Number
Hg analyzer	Milestone DMA-80	CHEM-958	16041979
Method:USEPA 8151A-1996			
Equipment Name	Model	Equipment Number	Serial Number
GC-MS	Agilent6890N/5973i	CHEM-126	US144004/CN10539052/US52411034
Method:USEPA 8260D-2018	······································		
Equipment Name	Model	Equipment Number	Serial Number
PT-GC-MS	Atomx XYZ/7890B/5977A	CHEM-ENV091	CA20247008/CN13313013/US1330M207
lethod:USEPA 8270E-2018		Le	
Equipment Name	Model	Equipment Number	Serial Number
GC-MS	Agilent 7890B/5977A	CHEM-1118	CN18053182/US1805M023
lethod:USEPA 8270E-2018	-		ļ
Equipment Name	Model	Equipment Number	Serial Number
GC-MS	Agilent 7890B/5977A	CHEM-1118	CN18053182/US1805M023



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QC Report

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Denormalia	wiethod Bis	ank(IVIB)			
Parameter	Batch ID	Unit	MDL	MB	Control Rang
Amonio (Ac)	suctively coupled plasma-mas	s spectrometry Me	thod: USEPA 200.8-19	94	
Arsenic (As)	LB2321699	mg/L	0.050	<0.05	<0.050
Barium (Ba)	LB2321699	mg/L	0.010	<0.01	<0.010
Cadmium (Cd)	LB2321699	mg/L	0.001	<0.001	<0.001
Chromium (Cr)	LB2321699	mg/L	0.010	<0.010	<0.010
Lead (Pb)	LB2321699	mg/L	0.010	<0.01	<0.010
Selenium (Se)	LB2321699	mg/L	0.050	<0.050	<0.050
Silver (Ag)	LB2321699	mg/L	0.010	<0.01	< 0.010
Aetals-Hg Method: USEPA 7473-2007				0.5.200.000	
Mercury (Hg)	LB2321802	mg/L	0.005	<0.005	<0.005
cid Herbicides in Water by GC-MS Method: USEPA 8151A-	1996				
2,4-D	LB2321721	mg/L	0.0005	<0.0005	<0.0005
2,4,5-TP (Silvex, Fenopop)	LB2321721	mg/L	0.0005	<0.0005	<0.0005
OCs Method: USEPA 8260D-2018					
Benzene	LB2321822	mg/L	0.0005	<0.0005	<0.0005
Carbon tetrachloride	LB2321822	mg/L	0.0005	<0.0005	<0.0005
Chlorobenzene	LB2321822	mg/L	0.0005	<0.0005	<0.0005
Chloroform	LB2321822	mg/L	0.0005	<0.0005	<0.0005
1,4-Dichlorobenzene	LB2321822	mg/L	0.0005	<0.0005	<0.0005
1,2-Dichloroethane	LB2321822	mg/L	0.0005	<0.0005	<0.0005
1,1-Dichloroethene	LB2321822	mg/L	0.0005	<0.0005	<0.0005
2-butanone(MEK)	LB2321822	mg/L	0.020	<0.020	<0.020
Tetrachloroethene	LB2321822	mg/L	0.0005	<0.0005	<0.0005
Trichloroethene	LB2321822	mg/L	0.0005	<0.0005	<0.0005
Vinyl chloride	LB2321822	mg/L	0.0005	<0.0005	<0.0005
OCs Method: USEPA 8270E-2018		10-			0.0000
2-Methylphenol	LB2321643	mg/L	0.0005	<0.0005	< 0.0005
3&4-Methylphenol	LB2321643	mg/L	0.0005	<0.0005	<0.0005
2,4-Dinitrotoluene	LB2321643	mg/L	0.0005	<0.0005	<0.0005
Hexachlorobenzene	LB2321643	mg/L	0.0005	<0.0005	<0.0005
Hexachlorobutadiene	LB2321643	mg/L	0.0005	<0.0005	<0.0005
Hexachloroethane	LB2321643	mg/L	0.0005	<0.0005	<0.0005
Nitrobenzene	LB2321643	mg/L	0.0005	<0.0005	<0.0005
Pentachlorophenol	I B2321643	ma/l	0.0005	-0.000	<0.0005
. enternorophenor	LDZ321043	mg/L	0.0025	<0.0025	<0.0025



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Method Blank(MB)

Parameter	Batch ID	Unit	MDL	MB	Control Range	
SVOCs Method: USEPA 8270E-2018 (continued)						
Pyridine	LB2321643	mg/L	0.002	<0.002	<0.002	
2,4,5-Trichlorophenol	LB2321643	mg/L	0.0005	<0.0005 <0.00		
2,4,6-Trichlorophenol	LB2321643	mg/L	0.0005	<0.0005	<0.0005	
SVOCs Method: USEPA 8270E-2018						
Endrin	LB2321641	mg/L	0.0005	<0.0005	<0.0005	
ү-ВНС	LB2321641	mg/L	0.0005	<0.0005	<0.0005	
Toxaphene	LB2321641	mg/L	0.050	<0.050	<0.050	
γ-Chlordane	LB2321641	mg/L	0.0005	<0.0005	<0.0005	
α-Chlordane	LB2321641	mg/L	0.0005	<0.0005	<0.0005 <0.0005	
Methoxychlor	LB2321641	mg/L	0.0005	<0.0005	005 <0.0005	
Heptachlor	LB2321641	mg/L	0.0005	<0.0005	<0.0005	

The evaluation of Method Blanks (MB): All results of MB on this batch are lower than method detection limits, which meet the acceptance criteria of lab quality control.

Laboratory Control Sample(LCS)

LCS Recovery%= Result*100/ Reference Value.

Parameter	Batch ID	Unit	MDL	Result	Ref. Value	Recevorv%	Control Range	
							Lower	Upper
Determination of trace elements in waters and wastes by	y inductively coupled plasm	a-mass spe	ctrometry Meth	nod: USEPA 20	0.8-1994			
Arsenic (As)	LB2321699	mg/L	0.050	0.195	0.2	97.4	80%	120%
Barium (Ba)	LB2321699	mg/L	0.010	0.199	0.2	99.6	80%	120%
Cadmium (Cd)	LB2321699	mg/L	0.001	0.198	0.2	98.9	80%	120%
Chromium (Cr)	LB2321699	mg/L	0.010	0.199	0.2	99.5	80%	120%
Lead (Pb)	LB2321699	mg/L	0.010	0.207	0.2	103	80%	120%
Selenium (Se)	LB2321699	mg/L	0.050	0.180	0.2	90.1	80%	120%
Silver (Ag)	LB2321699	mg/L	0.010	0.201	0.2	100	80%	120%
Aetals-Hg Method: USEPA 7473-2007								
Mercury (Hg)	LB2321802	mg/L	0.005	<0.005	0.001	100	80%	120%
cid Herbicides in Water by GC-MS Method: USEPA 81	51A-1996							
2,4-D	LB2321721	mg/L	0.0005	0.0007	0.001	74.0	70%	130%
2,4,5-TP (Silvex, Fenopop)	LB2321721	mg/L	0.0005	0.0008	0.001	78.0	70%	130%
OCs Method: USEPA 8260D-2018	1			L	1			0
Benzene	LB2321822	mg/L	0.0005	0.0163	0.02	81.6	70%	130%



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SHE23-03234 R0

Laboratory Control Sample(LCS)

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LCS Recovery%= Result*10	0/ Reference	Value
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Upper
130%
130%
130%
130%
130%
130%
130%
130%
120%
120%
130%
1449/
144 %
4170
127%
11%
31%
320/
20%
0.0%
40%
10%

The evaluation of recoveries for Laboratory Control Samples (LCS): All recoveries of LCS on this batch are in the controlled range, which meet the acceptance criteria of lab quality control.

Laboratory Duplicate(DUP)

Relative deviation(RD)%=|Sample Result -Duplicate Result|*100/(Sample Result +Duplicate Result).

Parameter	Sample ID	Unit	MDL	Sample Result	Duplicate Result	RD%	RD Control Range%	Sur Control Range
Determination of trace elements in water	rs and wastes by inductively co	upled plas	ma-mass sp	pectrometry Method	: USEPA 200.8-	1994		
Arsenic (As)	SHE23-03413.001	mg/L	0.050	<0.05	<0.05	0.0	≤20	-
Barium (Ba)	SHE23-03413.001	mg/L	0.010	0.081	0.077	2.5	≤20	-



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Laboratory Duplicate(DUP)

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Relative deviation(RD)%= Sample Result -Dup	plicate Result *100/(Samp	le Result	Duplicate Re	esult).				
Parameter	Sample ID	Unit	MDL	Sample Result	Duplicate Result	RD%	RD Control Range%	Sur Control Range
Determination of trace elements in waters and	d wastes by inductively co	pupled plas	ma-mass sp	ectrometry Metho	d: USEPA 200.8-	-1994 (continu	ied)	
Cadmium (Cd)	SHE23-03413.001	mg/L	0.001	0.002	0.001	8.4	≤20	-
Chromium (Cr)	SHE23-03413.001	mg/L	0.010	<0.01	<0.01	0.0	≤20	
Lead (Pb)	SHE23-03413.001	mg/L	0.010	0.014	0.014	1.3	≤20	-
Selenium (Se)	SHE23-03413.001	mg/L	0.050	<0.05	<0.05	0.0	≤20	-
Silver (Ag)	SHE23-03413.001	mg/L	0.010	0.015	0.013	4.5	≤20	-
Metals-Hg Method: USEPA 7473-2007			1					
Mercury (Hg)	SHE23-03413.001	mg/L	0.005	<0.005	<0.005	0.0	≤10	-
VOCs Method: USEPA 8260D-2018								1
Benzene	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	-
Carbon tetrachloride	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	-
Chlorobenzene	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	
Chloroform	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	-
1,4-Dichlorobenzene	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	-
1,2-Dichloroethane	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	-
1,1-Dichloroethene	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	-
2-butanone(MEK)	SHE23-03413.001	mg/L	0.020	<0.02	<0.02	0.0	≤30	-
Tetrachloroethene	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	-
Trichloroethene	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	-
Vinyl chloride	SHE23-03413.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤30	-
SVOCs Method: USEPA 8270E-2018								
2-Methylphenol	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
3&4-Methylphenol	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
2,4-Dinitrotoluene	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
Hexachlorobenzene	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
Hexachlorobutadiene	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
Hexachloroethane	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
Nitrobenzene	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
Pentachlorophenol	SHE23-03234.001	mg/L	0.0025	<0.0025	<0.0025	0.0	≤17.5	-
Pyridine	SHE23-03234.001	mg/L	0.002	<0.002	<0.002	0.0	≤17.5	-
2,4,5-Trichlorophenol	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
2,4,6-Trichlorophenol	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
VOCs Method: USEPA 8270E-2018						I		
Endrin	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-





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Laboratory Duplicate(DUP)

Relative deviation(RD)%=|Sample Result -Duplicate Result|*100/(Sample Result +Duplicate Result).

Parameter	Sample ID	Unit	MDL	Sample Result	Duplicate Result	RD%	RD Control Range%	Sur Control Range
SVOCs Method: USEPA 8270E-2018 (con	ntinued)							
Toxaphene	SHE23-03234.001	mg/L	0.050	<0.05	<0.05	0.0	≤17.5	-
y-Chlordane	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
α-Chlordane	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
Methoxychlor	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-
Heptachlor	SHE23-03234.001	mg/L	0.0005	<0.0005	<0.0005	0.0	≤17.5	-

The evaluation of Relative Deviation (RD) for Duplicates: All RD of duplicates on this batch are in the controlled range, which meet the acceptance criteria of lab quality control.

Matrix Spike(MS)

MS Recovery%= (MS Result-Sample Result) *100/Spike Added (Related factor should be taken into consideration) .

Parameter	Sample ID	Unit	MDL	MDL Gample Result		Spike	Docovory%	Control Range	
				P. S.	into result	Added	Necevoly /6	Lower	Upper
Determination of trace elements in waters a	and wastes by inductively cou	pled plasm	a-mass spect	rometry Method: L	JSEPA 200.8-	1994			
Arsenic (As)	SHE23-03413.001	mg/L	0.050	<0.050	0.151	0.2	75.2	70%	130%
Barium (Ba)	SHE23-03413.001	mg/L	0.010	0.079	0.273	0.2	97.2	70%	130%
Cadmium (Cd)	SHE23-03413.001	mg/L	0.001	0.001	0.193	0.2	95.6	70%	130%
Chromium (Cr)	SHE23-03413.001	mg/L	0.010	<0.010	0.205	0.2	99.9	70%	130%
Lead (Pb)	SHE23-03413.001	mg/L	0.010	0.014	0.223	0.2	104	70%	130%
Selenium (Se)	SHE23-03413.001	mg/L	0.050	<0.050	0.170	0.2	78.1	70%	130%
Silver (Ag)	SHE23-03413.001	mg/L	0.010	0.014	0.212	0.2	99.0	70%	130%
OCs Method: USEPA 8260D-2018			•	-	+		+		
Benzene	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0164	0.02	82.2	50%	150%
Carbon tetrachloride	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0107	0.02	53.7	50%	150%
Chlorobenzene	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0196	0.02	97.8	50%	150%
Chloroform	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0151	0.02	75.6	50%	150%
1,4-Dichlorobenzene	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0172	0.02	85.9	50%	150%
1,2-Dichloroethane	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0143	0.02	71.3	50%	150%
1,1-Dichloroethene	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0102	0.02	51.2	50%	150%
2-butanone(MEK)	SHE23-03413.001	mg/L	0.020	<0.020	<0.02	0.02	67.7	50%	150%
Tetrachloroethene	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0217	0.02	108	50%	150%
Trichloroethene	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0211	0.02	106	50%	150%
Vinyl chloride	SHE23-03413.001	mg/L	0.0005	<0.0005	0.0104	0.02	52.2	50%	150%
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The evaluation of recoveries for Matrix Spiked (MS): All recoveries for MS on this batch are in the controlled range, which meet the acceptance criteria of lab quality control.



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Matrix Spike Duplicate(MSD)

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Relative deviation(RD)%= MS Recovery% -MSD Recovery% *100	/(MS Recovery%+MSD Recovery%).
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Parameter	Sample ID	Unit	MDL	MS Recovery%	MSD Recovery%	RD%	RD Control Range%	Sur Control Range
Determination of trace elements in waters and wastes b	y inductively coupled p	lasma-mas	s spectrome	try Method: US	EPA 200.8-1994			
Arsenic (As)	SHE23-03413.001	mg/L	0.050	75.2	73.8	1.0	≤20	-
Barium (Ba)	SHE23-03413.001	mg/L	0.010	97.2	99.4	1.1	≤20	-
Cadmium (Cd)	SHE23-03413.001	mg/L	0.001	95.6	95.2	0.2	≤20	-
Chromium (Cr)	SHE23-03413.001	mg/L	0.010	99.9	98.9	0.5	≤20	-
Lead (Pb)	SHE23-03413.001	mg/L	0.010	104	106	0.9	≤20	-
Selenium (Se)	SHE23-03413.001	mg/L	0.050	78.1	86.0	4.8	≤20	
Silver (Ag)	SHE23-03413.001	mg/L	0.010	99.0	97.5	0.7	≤20	-
VOCs Method: USEPA 8260D-2018								
Benzene	SHE23-03413.001	mg/L	0.0005	82.2	86.0	2.2	≤30	-
Carbon tetrachloride	SHE23-03413.001	mg/L	0.0005	53.7	64.8	9.4	≤30	-
Chlorobenzene	SHE23-03413.001	mg/L	0.0005	97.8	102	1.9	≤30	-
Chloroform	SHE23-03413.001	mg/L	0.0005	75.6	81.1	3.5	≤30	
1,4-Dichlorobenzene	SHE23-03413.001	mg/L	0.0005	85.9	91.8	3.3	≤30	-
1,2-Dichloroethane	SHE23-03413.001	mg/L	0.0005	71.3	75.8	3.1	≤30	-
1,1-Dichloroethene	SHE23-03413.001	mg/L	0.0005	51.2	53.9	2.5	≤30	
2-butanone(MEK)	SHE23-03413.001	mg/L	0.020	67.7	72.5	3.4	≤30	-
Tetrachloroethene	SHE23-03413.001	mg/L	0.0005	108	109	0.3	≤30	-
Trichloroethene	SHE23-03413.001	mg/L	0.0005	106	114	3.6	≤30	-
Vinyl chloride	SHE23-03413.001	mg/L	0.0005	52.2	51.6	0.5	≤30	-

The evaluation of Matrix Spiked Duplicates (MSD): All recoveries for MSD on this batch are in the controlled range, which meet the acceptance criteria of lab quality control. All RD for MS and MSD on this batch are in the controlled range, which meet the acceptance criteria of lab quality control.



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EXHIBIT V

DECOMMISSIONING PLAN

DECOMMISSIONING PLAN

1. Project Description

Santa Fuel Inc. is proposing to develop a 3.85-megawatt ("MW") solar energy farm in Somers, CT. A solar array consists of photovoltaic panels that transform sunlight into usable energy. The facility will have approximately 8,710 individual panels transforming sunlight each day into usable energy that is fed into the regional electric grid. The project will generate enough electricity to power over 500 homes per year. The estimated operational life of the project will be 25 years with an option to extend.

2. Construction

The Project Site consists of approximately 22.1 acres of undeveloped land, part of a larger 108.5-acre parcel (the Property) located at 159 South Road in Somers, Connecticut. The ground-mounted solar panels will be located within a fenced area of approximately 17.11 acres in size. A new access road coming off South Rd will be utilized to access the solar panels, and a new gravel road will be built to access the panels within the parcel. Each solar panel will rest on a galvanized steel and aluminum frame and will be located on a metal pole that will be driven into the ground. Utility trenches will be excavated to install the underground electrical lines leading to each string of solar panels. Once the utilities are installed the utility trench will be filled and seeded to maintain a consistent grassed surface. Concrete slabs will be installed to hold the necessary inverters/transformers required to operate the solar array.

3. Decommissioning Process

This section sets out the details and different steps of decommissioning the solar farm. The installation will be removed no more than one year after operations are discontinued.

a) Deconstruction: DC-Cabling

All inverter systems and electrical components of the PV system will be switched off. In following all plug-in connectors and string cables will be disconnected. To remove the cables which are laid in the ground, all cable trenches will be opened. In the following, all cables will be removed and separated. After the uninstalling of the wiring, the materials will be disposed of in accordance with the disposal regulations for metal waste which applies at the installation site at the time.

b) Deconstruction: PV-Modules

All PV modules will be removed and separated from the mounting system and removed from the site. After removal, the PV modules will be reused or recycled.

c) Deconstruction: Inverters / Transformers / Substation

After the uninstallation of the entire monitoring system (cabling + components) the inverter / transformer stations as well as the substation will be removed from the site. The concrete foundations will be removed, and the holes will be filled with soil. Then the transformer stations will be removed and disposed of in accordance with the disposal regulations for metal and concrete waste which apply at the installation site at the time.

d) Deconstruction: Mounting System

The mounting system will be removed completely. The deconstruction shall proceed as follows: I. module carrier system, II. purlin profiles, III. Posts. The disposal of the materials will be done in accordance with the disposal regulations for metal waste which applies at the installation site at the time.

e) Deconstruction: AC- Cabling / Earthing

All AC-cables and combiner boxes will be disconnected and removed. To remove the cables which are laid in the ground, all cable trenches will be opened. In the following all cables and earth stripes will be removed and separated. The cable trenches will be back filled. After all wiring has been uninstalled, the materials will be disposed of in accordance with the disposal regulations for metal waste which applies at the installation site at the time.

f) Deconstruction: Fence and Alarm System

All parts of the fence as well as the alarm system (if applicable) will be removed. The disposal of the materials follows in accordance with the disposal regulations for metal waste which apply at the installation site at the time.

g) Ground Regulation

When the decommission works are completed, the land will be restored by grading and revegetating disturbed areas.

All equipment and fixtures removed from the solar farm will either be reused, recycled, or disposed of at the time of decommissioning. Upon decommissioning of this solar farm, reuse of the solar panels will be the priority. If reuse is not feasible, the solar panels will be recycled in accordance with the PV CYCLE USA waste management scheme, or similar. Items that are not able to be reused or recycled will be disposed of in accordance with local rules and regulations.

4. Cost

We have researched the current price estimates for the disassembly & disposal of the solar equipment, site restoration, and value of salvageable materials and have made the following assumptions:

- Current labor costs are currently about \$26 per hour for the state of Connecticut, according to the Bureau of Labor Statistics. We have assumed 2% inflation per annum over the lifetime of the project (40 years).
- Salvage values have been estimated using data from http://www.scrapmonster.com.

The value of salvageable parts offsets approximately 35% of decommissioning costs. Salvage value is likely to fluctuate and change over the next 40 years.

	Projected Cost of	
ltem	Tasks	Estimated Labor Cost
	Remove panels from racks/disassemble racking	\$50,857
2	Remove earth screws	\$59,590
3	Remove fencing	\$12,900
4	Remove underground electrical	\$10,476
5	Remove concrete pads	\$2,640
6	Load salvageable parts	\$30,696
7	Grade and seed disturbed areas	\$18,938
8	Supervision	\$9,631
9	116 haul truck loads	\$34,254
	Total Cost	\$235,981
	Salvageable Parts and Materials	
Item	Parts/Materials	Salvageable Cost
1	Steel fence – 9.8 tons at \$111/ton	\$1,086
2	Solar racking components - 458 tons at \$160/ton	\$73,617
3	Copper wiring – 2.4 tons at \$1.43/lb	\$6,864
4	Aluminum wiring – 3.1 tons at \$0.25/lb	\$1,581
5	Solar inverters + transformers (value covers removal)	\$-
	Total Value of Salvageable Materials	\$83,148
	Net Decommissioning Cost	\$152,832

5. Force Majeure

An exception to these requirements will be allowed for a force majeure event, which is defined as any event or circumstance that wholly or partly prevents or delays the performance of any material obligation arising under the Project permits, but only to the extent:

- Such event is not within the reasonable control, directly or indirectly, of Santa Fuel Inc. (including without limitation event such as fire, earthquake, flood, tornado, hurricane, acts of God and natural disasters; war, civil strike or similar violence);
- Santa Fuel Inc has taken all responsible precautions and measures to prevent or avoid such event or mitigate the effect of such event on Santa Fuel Inc's ability to perform its obligations under the Project permits and which, by the exercise of due diligence, it has been unable to overcome; and

• Such event is not the direct or indirect result of the fault of negligence of Santa Fuel Inc.

In the event of force majeure event, which results in the absence of electrical generation by one or more solar panels for 12 months, Santa Fuel Inc will demonstrate to the Municipality by the end of the 12 months of non-operation that the Project, or any single solar panel, will be substantially operational and producing electricity within 24 months of the force majeure event. If such a demonstration is not made to the Municipality's satisfaction, the decommissioning of any single solar panel only (and no other part of the Project that is operational) or if the entire Project is not substantially operational and producing electricity, then decommissioning of the Project will be initiated within 18 months after the force majeure event.

EXHIBIT VI

NOTICE TO TOWN AND STATE OFFICIALS AND ABUTTERS AND ABUTTERS MAP



September 15, 2023

Via Certified Mail

<Name and Address>

Re: Santa Fuel, Inc. – Notice of Intent to File a Petition for Declaratory Ruling for the Construction, Operation and Maintenance of a 3.85 MW(ac) Solar Photovoltaic Electric Generating Facility at the 159 South Road in Somers, Connecticut

Dear <Salutation>:

Pursuant to the provisions of §16-50j-40(a) of the Regulations of Connecticut State Agencies, this letter serves as notice that the Santa Fuel, Inc. intends to file a Petition for Declaratory Ruling (Petition) with the Connecticut Siting Council (Council) on or about September 15, 2023, seeking approval of the construction, operation and maintenance of a 3.85 megawatt (MW)(ac) solar power generating facility, including all associated equipment, related site improvements, and interconnection (the Project).

The Project is located at 159 South Road in Somers, Connecticut. The Project will include an approximate 17.11 acre solar array. The Project shall provide power to the Eversource grid. The Project will consist of the installation of ground-mounted photovoltaic panels, centralized inverters and transformers, electrical lines, electrical transformers, a perimeter fence and stormwater management controls, and evergreens for screening. For details regarding the location and layout of the Project, please see the attached reduced sized copy of the Overall Site Plan.

Pursuant to the provisions of the Connecticut General Statutes §16-50g et seq., the location of certain project features may change as this Petition proceeds through the Council's regulatory review process.

If you have any questions, please feel free to contact me. My contact information is provided below.

Respectfully,

Timothy A. Coon, P.E. J.R. Russo & Associates, LLC

Attachment (Overall Plan)

CERTIFICATION OF SERVICE

I hereby certify that on this 15th day of September 2023 notice of intent to file the Santa Fuel, Inc. Petition for Declaratory Ruling was sent, via certified mail, to the following:

Somers Town Officials:

Tim Keeney, First Selectman Town of Somers 600 Main Street Somers, CT 06071

Joan Formeister, Chairman Somers Conservation Commission 600 Main Street Somers, CT 06071

W. Karl Walton, Chairperson Somers Zoning Commission 600 Main Street Somers, CT 06071

Regional Council of Governments:

Capitol Region Council of Governments (CRCOG) 241 Main Street Hartford, CT 06106-5310

State Officials:

Kurt Vail Representative – 52th District Legislative Office Building 300 Capitol Avenue, Room 4200 Hartford, CT 06106

John Kissel Senator – 7th District Legislative Office Building 300 Capitol Avenue, Room 3400 Hartford, CT 06106-1591

The Honorable William Tong Attorney General Office of the Attorney General 165 Capitol Avenue Hartford, CT 06106 James C. Rovella, Commissioner Department of Emergency Services and Public Protection Emergency Management and Homeland Security Division 1111 Country Club Road Middletown, CT 06457 Katie Dykes, Commissioner Department of Energy & Environmental Protection 79 Elm Street Hartford, CT 06106-5127

Manisha Juthani, M.D, Commissioner Department of Public Health 410 Capitol Avenue Hartford, CT 06134

Paul Aresta, Executive Director Council on Environmental Quality 79 Elm Street P.O. Box 5066 Hartford, CT 06106

Marissa Gillett, Chair Public Utilities Regulatory Authority Ten Franklin Square New Britain, CT 06051

Jeffrey R. Beckham, Acting Secretary Office of Policy and Management 450 Capitol Avenue Hartford, CT 06106

Alexandra Daum, Commissioner Department of Economic and Community Development 450 Columbus Boulevard Hartford, CT 06103

Garrett Eucalitto, Commissioner Department of Transportation P.O. Box 317546 2800 Berlin Turnpike Newington, CT 06131-7546 Jonathan Kinney State Historic Preservation Officer Department of Economic & Community Development 450 Columbus Boulevard, Suite 5 Hartford, CT 06103

Bryan P. Hurlburt, Commissioner Department of Agriculture 450 Columbus Boulevard, Suite 701 Hartford, CT 06103

Michelle Gilman, Commissioner Department of Administrative Services 450 Columbus Boulevard Hartford, CT 06103

(see attached List of Abutters)

Northern Connecticut Land Trust, Inc. P.O. Box 324 Somers, CT 06071

Dianne B. Lenti 41 Haystack Road Manchester, CT 06040

Karen J. Murphy 187 South Road Somers, CT 06071

Paul J. Arcari 71 Mountain View Road Somers, CT 06071

Abutting Property Owners

JVLADD Properties, LLC P.O. Box 8 Somers, CT 06071

Tara J. & Lennon A Cormie 35 Stillmeadow Lane Somers, CT 06071

Robert & Nancy B. Edgar 11 Mountain View Road Somers, CT 06071 Nancy B. Edgar Revocable Trust 11 Mountain View Road Somers, CT 06071

John G. Talcott III Amended & Restated Trust 48 Main Street Talcottville, CT 06066

Karen D. Clark 70 Mountain View Road Somers, CT 06071





Lot#	Name & Address						
05–68	Northern Connecticut Land Trust, Inc. P.O. Box 324 Somers, CT 06071						
05-72-03	JVLADD Properties, LLC P.O. Box 8 Somers, CT 06071						
05-73A	Nancy B. Edgar Revocable Trust 11 Mountain View Road Somers, CT 06071						
05-74A	Dianne B. Lenti 41 Haystick Road Manchester, CT 06040						
06-01E-05	Tara J. & Lennon A. Comrie 35 Stillmeadow Lane Somers, CT 06071						
06-03A	John G. Talcott III Amended & Restated						
Trust	48 Main Street Talcottville, CT 06066						
06-12	Northern Connecticut Land Trust, Inc. P.O. Box 324 Somers, CT 06071						
17–22	Karen J. Murphy 187 South Road Somers, CT 06071						
17–23	Robert & Nancy B. Edgar 11 Mountain View Road Somers, CT 06071						
17–24	Nancy B. Edgar Revocable Trust 11 Mountain View Road Somers, CT 06071						
17–25	Karen D. Clark 70 Mountain View Road Somers, CT 06071						
17–28	Paul J. Arcari 71 Mountain View Road Somers, CT 06071						
A T							
ABUTTERS MAP							
5	'omers Solar						
Louth	Callan Kenewables, LLC						
<u>,</u>	159 South Road Somers, Connecticut						
		-					
SURVEYOR	NOT TO SCALE						
SERVING	2023-001						

sheet EXHIBIT VI EXHIBIT VII

NATURAL DIVERSITY DATABASE MAP



EXHIBIT VIII

WETLAND REPORT

Datum Engineering & Surveying LLC. Richard Zulick Certified Forester / Soil Scientist 400 Nott Highway Ashford, CT 06278

March 7, 2023

Timothy Coon, P.E. JR Russo & Associates,LLC P.O. Box 938 East Windsor, CT 06088

Re: Wetland report Solar Project – South Road, Somers, CT Map 05 – Lot 73

Dear Tim:

Per your request, I have conducted a wetland delineation to identify the Connecticut regulated wetland soils on the above referenced parcel located on the east side of South Road (Route 83) in Somers. This delineation was conducted for the purposes of identifying wetland soils as they relate to a proposed solar project. The wetlands were delineated on December 4, 2022.

The wetlands on this plan have been field delineated in accordance with the standards of the National Cooperative Soil Survey and the definition of wetlands as found in the Connecticut General Statutes, Chapter 440, Section 22A-38.

This delineation is not intended to be used for soil mapping but to identify the wetland soils relative to the development and management of this parcel

Wetland/watercourses on the site consist of two excavated ponds. The wetland boundaries have been marked with pink and blue flagging and labeled as shown on the plan. Flag numbers are W40 to W47 and W 48 to W54 and delineate the easterly extent of the ponds adjacent to the proposed activity.

All soils on the site, including the field and ponds have been altered by extensive gravel extraction. Based on existing conditions, upland soils were restored with topsoil placed over excavated subsoils. The fields have been producing well maintained grasses and have developed a robust topsoil horizon with depths of 8"-16", overlying sandy to gravelly loam subsoil.

The two ponds appear to be man-made excavations with notable berms around the edges. The ponds were cut into the existing groundwater table and appear to maintain a relatively consistent water level. Based on vegetation within the ponds, they are relatively old and have well developed herbaceous and shrub layer. Shallow areas around the pond fringe have also been colonized by small trees.

Since the ponds were excavated into existing subsoil and substratum, they are defined as Aquepts and consist of the original soil and subsoil (identified as Manchester sand and gravel on the Web Soil Survey)

that are saturated by groundwater at or near the soil surface. The wetland soils do not have a lot of horizon development and consist of dark to dark grey organic sand and gravel.

As indicated above, upland soils are mapped as Manchester series. The Manchester series consists of very deep, excessively drained soils formed in sandy and gravelly glacial outwash and stratified drift. They are nearly level to steep soils on outwash plains, terraces, kames, deltas and eskers. Slope ranges from 0 to 45 percent

TAXONOMIC CLASS: Sandy-skeletal, mixed, mesic Typic Udorthents

Since most of the deep sands and gravels were previously excavated, the remaining soils typically consist of restored topsoil overlying compacted sand and gravel. Borings conducted by hand met often met refusal within 16-24 inches.

I have reviewed the overview plan for the purposes of assessing the wetland functions and values and potential impacts to the wetlands associated with the development of the proposed solar array.

Existing Conditions

The proposed development area consists of a gently sloping gravelly sandy loamy upland area located east of Somers Road and is currently in agricultural (hay) production. This entire area is considered disturbed by definition, in that, the area was previously excavated for sand and gravel and was subsequently restored and maintained in hay production.

Wetland Functions and Values

The wetland complex was inspected to determine wetland functions and values utilizing the Army Corps. of Engineers methodology as outlined in "The Highway Methodology Workbook Supplement". These wetlands exhibited the following wetland functions and values with the corresponding rationale:

Ground water recharge and discharge: both ponds are excavated into the existing groundwater table. The ponds drain into a culvert across south road and as such, serve to discharge groundwater. Potential for and public or private wells occur downstream of the wetland. The wetland is underlain by gravel or sandy soils present in or adjacent to the wetland.

Flood flow alteration: the wetland receives stormwater runoff from the drainage area to the east, but capacity for flood storage is limited by the existing ponded area, which (also) appears to be the local groundwater level.

Sediment/toxicant retention: potential sources of sediment are in the watershed above the wetland, opportunity for sediment trapping by slow moving water and shallow water habitat are present in this wetland. The wetland has a high degree of water and vegetation interspersion, and dense vegetation provides opportunity for sediment trapping.

Nutrient removal: Overall potential for sediment trapping exists within the pond/wetland system. Saturated soils exist for most of the season, ponded water is present in the wetland, organic/sediment deposits are present, dense vegetation is present with emergent vegetation and/or dense woody stems dominant, water retention/detention time in this wetland is increased by thick vegetation and other dense

herbaceous and shrub vegetation in wetlands utilize and immobilize excess nutrients transported/deposited by developed areas up-gradient.

Production export: The pond/wetland complex appear to maintain a stable permanent pool and have diverse vegetation. With permanent ponding and moderate cover, waterfowl use is possible, but may be limited by proximity to South Road. With a permanent pool and diverse wetland vegetation, the ponds likely support a diverse amphibian population. The pond an wetland complex exhibits moderate degree of plant community structure/species diversity and the wetland contains flowering plants that are used by nectar-gathering insects.

The wetlands were also examined for wetland values (recreational, educational/scientific, visual/aesthetic, or uniqueness/heritage values) and the following values were noted with their rationale:

None found

Conclusions:

In summary, it is my opinion that the pond and wetland area are a functioning wetland ecosystem which exhibits 5 wetland functions and 0 out of the 5 wetland values. No significant negative impacts will result from this proposed development. The wetland areas will remain undeveloped and wetland functions will not be altered.

If you have any questions concerning the wetland function assessment or this report, please feel free to contact me.

Sincerely,

Richard Zulick Certified Forester and Soil Scientist Member SSSSNE

22-008

EXHIBIT IX

DRAINAGE REPORT

DRAINAGE REPORT Louth Callan Renewables, LLC Somers Solar

159 South Road Somers, CT

September 12, 2023

Prepared for:

Santa Fuel, Inc. 154 Admiral Street Bridgeport, CT 06605

Owner:

Nancy B. Edgar Revocable Trust & Dianne Bordeax Lenti 11 Mountain View Road Somers, CT 06071

Project No. 2023-001

Prepared by:

J.R. Russo & Associates Land Surveyors & Professional Engineers P.O. Box 938 East Windsor, CT 06088 (860) 623-0569

I. INTRODUCTION

A. Project Description

The applicant is proposing to construct a ground mounted solar array at 159 South Road in Somers. The proposed project includes a fenced area of approximately 17.1 acres containing 8,710 solar panel modules. The development will include two stormwater management basins designed to provide groundwater recharge and retention of stormwater to ensure no environmental or flooding impacts downstream. The development and stormwater management system have been designed in accordance with the CT Stormwater Quality Manual and Department of Energy & Environmental Protection's (DEEP's) Stormwater General Permit.

B. Existing Conditions

The project site consists of approximately 22.1 acres of undeveloped land, part of a larger 108.5-acre parcel (the property) located at 159 South Road in Somers, Connecticut. The property is located on the east side of South Road and the north side of Mountain View Road. The northeastern portion of the property was formerly mined for gravel. The gravel mining operation was initiated in 1998 and terminated in 2009. Upon completion of the mining operations, disturbed areas were restored and are currently maintained as hay field. The southeastern portion of the property is undeveloped woodland. The southwestern portion of the property is undeveloped woodland. The southwestern portion of the property consists of an old orchard that is no longer maintained. An existing single-family home with a couple of barns and a former fruit stand are located adjacent to South Road on the western portion of the Property. Two dug ponds are located in the northwestern portion of the Property. The area around the ponds has become overgrown with brush.

The project site slopes downwards from east to the west. Runoff from the northern portion of the project site flows into the two on-site dug ponds. Runoff from the southern portion of the project site flows into an existing depressed area along Somers Road that conveys water to an existing culvert that crosses under Somers Road to the west.

Based on a review of the USDA Soil Survey, the soils in the drainage area of the proposed development are classified as Manchester gravelly sand, Charlton-Chatfield complex, or Cheshire fine sandy loam (See Soils Map in Appendix 1). The USDA Soil Survey defines groups of soils into Hydrologic Soil Groups (HSG) according to their runoff-producing characteristics. Soils are assigned to four groups (A, B, C, and D Groups). In group A, are soils having a high infiltration rate when thoroughly wet and having a low runoff potential. They typically are deep, well drained, and sandy or gravelly. In group D, at the other extreme, are soils having a very slow infiltration rate and thus a high runoff potential. They have a hardpan or clay layer at or near the surface, have a permanent high-water table, or

are shallow over nearly impervious bedrock or other nearly impervious material. Charlton-Chatfield complex and Cheshire fine sandy loam are classified as HSG B. The HSG classification of Manchester gravelly sand is HSG A.

On April 7, 2023, a series of five test pits were performed in the area of the proposed stormwater management basins to confirm the existing soil conditions. Two additional test pits were performed on June 13, 2023 in the area of the second proposed stormwater management basin. Test pits 1-3 were located in the former gravel pit area in the vicinity of the northern basin (#1) while test pits 4-7 were in the orchard in the vicinity of the southern basin (#2). Test Pits 1-3 were excavated to a depth of 108 inches. Soils encountered included 12-16 inches of topsoil over fine to coarse sand with gravel. Soil mottling indicative of the seasonal high water table was encountered at 48 inches below the ground surface in TP1, 80 inches in TP2, and 60 inches in TP3. Test pits 4 & 5 are farther up the hill to the east of the southern basin. Soils encountered included 10-11 inches of topsoil over light brown sandy loam subsoils to a depth of 24 inches, overlying coarse sand and gravel. No soil mottling was encountered in either test pit, but TP5 hit refusal at 66 inches. TP4 was excavated to a depth of 108 inches. Test pits 6 & 7 are down the hill near the southern basin. They were excavated to depths of 144 and 120 inches. Soils encountered included 8-12 inches of topsoil with some brown loamy sand subsoils in TP6 to a depth of 20 inches, overlying fine to coarse sand with gravel. No soil mottling was encountered in either test pit. Test pit logs are provided on the Site Plans.

Soil samples were subsequently collected in the vicinity of test pits 1, 2, 6 and 7 at depths of 18-24 inches with a post hole digger. These samples were submitted to New England Materials Testing Lab, LLC for permeability testing by ASTM D2434. The permeability calculated for the four samples were 5.7 in/hr, 0.75 in/hr, 1.784 in/hr, and 0.49 in/hr, respectively. Permeability test results are also provided in Appendix 1.

II. STORMWATER RUNOFF ANALYSIS

A. Methodology

Peak runoff flow rates were determined for pre- and post-development conditions using Applied Microcomputer System's HydroCAD[™] Stormwater Modeling System. This computer software employs the SCS Technical Release 55 and 20 (TR-55 & TR-20) methodology. The potential stormwater impacts downstream were evaluated for the 2-yr, 25-yr, 50-yr, and 100-yr; 24-hour storm events. The rainfall for these storm events was taken from NOAA Atlas 14 provided in Appendix 2.

Based on the present drainage patterns, two design points were selected for the analysis. Design point #1 (DP1) is the wetland at the edge of the dug ponds located that receives runoff from the northern portion of the development. Design Point #2 (DP2) is the roadside

swale that collects runoff from the southern portion of the project site and conveys it to the culvert south of the site that crosses under Somers Road.

B. Pre-Development Hydrology

The pre-development site was divided into four subcatchments as shown on the predevelopment drainage area map in Appendix 3. Subcatchment E1 includes the off-site area that flows through the site to DP1. Subcatchment E2 includes the on-site area that flows to DP1. Subcatchment E3 includes the off-site area that flows through the site to DP2. Subcatchment E4 includes the on-site area that flows to DP2. The pre-development runoff characteristics of the contributing areas are provided on the HydroCAD data sheets in Appendix 4. The pre-development discharge rates from the site during the design storms are summarized in Tables 1-2.

C. Post-Development Hydrology

The Project Site consists of a 17.11-acre fenced area surrounding the array. The proposed solar array will be mostly be installed at existing grade. Some grading will occur in the area of the existing orchard where slopes are in excess of 15%. These slopes will be reduced to less than 15%. Overall, the drainage patterns will be maintained. Soil disturbance will be limited to the construction of the stormwater management basins and driveway; the stump removal of the southeastern portion of the array; and the re-grading of slopes in excess of 15%. The existing vegetation within the undisturbed portion of the array will be maintained throughout the project to provide stabilization of the underlying soils and prevent erosion and sedimentation. The proposed tracker panel solar arrays will be installed on elevated racks that provide adequate height above the ground to allow for infiltration, and promote the revegetation of the southwest portion and the continued growth of the existing vegetative cover. As a result, post construction, the areas containing the solar arrays can be considered pervious vegetated groundcover.

In accordance with Appendix I of the DEEP's General Permit, the hydrologic analysis is required to account for the compaction of soils that result from extensive machinery traffic over the course of the construction of the array. To account for this, the runoff curve number must be increased by one full HSG category where grading within the array exceeds a 2-foot difference between existing and proposed grades and one half the difference between the on-site HSG and the next higher HSG for the remainder of the array. As discussed above, majority of the proposed array at our site will utilize existing grades. Of the 17.1 acres within the array, only 0.7 acres exceed a 2-foot difference between existing and proposed grades. Thus, to meet this requirement, the post construction runoff curve number was increased by one full HSG category for the 0.7 acres and by one half the difference for the remainder of the array.

The project will include the construction of two stormwater management basins to mitigate the increase in runoff from the development. The northern stormwater management basin (Basin 1) will be equipped with a 30" flared end as a primary outlet and a 20-feet wide earthen berm spillway. The southern stormwater management basin (Basin 2) will be equipped with a 12" pipe as a primary outlet and a 20-feet wide earthen berm spillway. Basin 1's outlet pipe will discharge onto a Type 1 preformed scour hole upgradient of the wetland associated with the northern pond. Basin 2's outlet pipe will discharge onto a Type A riprap apron into the roadside swale. Outlet protection for the basins' spillways will consist of 12" thick modified riprap slopes extended 5 feet beyond the toe of the slope. A stone trench will also be installed in the bottom of each basin to facilitate infiltration, especially in the winter months when the ground could freeze.

As discussed above, two soil samples were collected from the soils at the base of each of the proposed stormwater management basins and analyzed for permeability. Samples PH1 and PH2 were collected from the vicinity of test pits TP1 and TP2 in the area of Basin 1. Samples PH6 and PH7 were collected from the vicinity of test pits TP6 and TP7 in the area of Basin 2. The resulting permeabilities were 5.7 in/hr (PH1), 0.75 in/hr (PH2), 1.784 in/hr (PH6) and 0.49 in/hr (PH7). As a conservative measure, the slowest permeability rate in each the basins (0.75 in/hr for Basin 1 and 0.49 in/hr for Basin 2) was used as the basis for the design infiltration rate.

The post-development site was divided into 6 subcatchments as shown on the postdevelopment drainage area map in Appendix 3. Subcatchment S1 includes the off-site area that flows through the site into the northern basin (Basin 1) that will discharge towards the DP1. Subcatchment S2 includes the on-site area that flows into Basin 1. Subcatchment S3 includes the off-site area that flows through the site into the southern basin (Basin 2) that will discharge towards DP2. Subcatchment S4 includes the on-site area that flows directly to DP2. Subcatchment S6 includes the area that bypasses Basin 2 and sheet flows directly to DP1. The post development subcatchment characteristics are summarized in the attached HydroCAD data sheets in Appendix 5. The post-development discharge rates from the site during the design storms are summarized in Tables 1-2.

Using the characteristics described above, the Post Development peak flow rates for the site were calculated for the 2, 25, 50, and 100-year 24-hour rainfall design storms. Refer to Appendices 4 and 5 for pre-development and post-development HydroCAD data sheets. Tables 1-2 compares the pre-development peak flows with the post-development peak flows at the design point. As shown, the resulting post-development peak flows are less than the pre-development peak flows.

TABLE 1 – COMPARISON OF PRE- & POST-DEVELOPMENTDISCHARGE RATES (CFS) TO DESIGN POINT 1 (WETLAND)

	2-year	25-year	50-year	100-year
Pre-Development	1.9	24.3	33.6	46.2
Post Development	0.4	16.5	24.5	33.8

TABLE 2 – COMPARISON OF PRE- & POST-DEVELOPMENTDISCHARGE RATES (CFS) TO DESIGN POINT 2 (STREET CULVERT)

	2-year	25-year	50-year	100-year
Pre-Development	0.3	4.8	7.0	9.7
Post Development	0.1	4.1	5.1	8.9

D. Stormwater Treatment

Appendix I of the DEEP Stormwater General Permit requires that all solar panels in the array be considered effective impervious cover for the purposes of calculating Water Quality Volume if the proposed post-construction slopes at a site are 15% or more or if slopes less than 15% do not meet the four listed conditions:

- a) The vegetated area receiving runoff between rows of solar panels is equal to or greater than the average width of the row of solar panels draining to the vegetated area;
- b) Overall site conditions and solar panel configuration within the array are designed so stormwater runoff remains as sheet flows across the entire site towards the intended stormwater management controls;
- c) The following conditions are satisfied regarding the design of the post-construction slope of the site:
 - i. Slopes less than or equal to 5%:

Appropriate vegetation shall be established that will ensure sheet flow conditions and that will provide sufficient ground cover throughout the site.

ii. Slopes between 5% and 10%:

Practices such as level spreaders, terraces, or berms shall be used to ensure long term sheet flow conditions.

iii. Slopes greater than or equal to 10% and less than 15%:

The plan must include specific engineered stormwater control measures with detailed specifications that are designed to provide permanent stabilization and non-erosive conveyance of runoff downgradient from the site.

iv. Slopes greater than or equal to 8%:

Erosion control blankets, stump grindings, erosion control mix mulch, or hydroseed with tackifier shall be applied within 72 hours of final grading, or when a rainfall of 0.5 inches or greater is predicted within 24 hours of final grading, whichever time period is less.

d) The solar panels shall be designed as to allow the growth of native vegetation beneath and between the panels.

Proposed grading of the existing steep slopes at the site will reduce slopes to less than 15%. Therefore, conditions (a)-(d) are required to be met in order to avoid treating the panels as impervious area. To satisfy condition (a), the proposed row spacing of 11.23' will exceed the 7.40' width of the panels' horizontal projection. To satisfy condition (b), the existing grades and vegetation will be maintained in the northern portion of the array. Where regrading occurs in the southern portion of the array, berms of coarse woody debris generated from clearing activities will be installed and maintained along the contours at regular intervals throughout the portion of the array that has been cleared to capture and redistribute runoff as sheet flow. For condition (c), where the existing vegetation will be maintained throughout construction, the need for additional erosion control measures to provide stabilization of the slopes are not necessary. Where tree clearing and re-grading woody berms will be installed along the contours at regular intervals to provide additional slope to satisfy condition c. In addition, all disturbed areas will be seeded with a pollinator seed mix and mulched immediately to establish a vegetated cover. Finally, to satisfy condition (d), the proposed fixed panel solar arrays will be installed on elevated racks that provide adequate height above the ground to promote the continued growth of the existing vegetative cover and allow for infiltration.

As a result of satisfying the conditions above, the panels need not be considered as impervious coverage for the calculation of the WQV. Thus, the only proposed surfaces required to be included in the calculation of the WQV are the equipment pads and gravel access drive. However, these areas are small in relation to the overall site and not directly connected to the stormwater collection system. Thus, runoff from these areas will sheet flow over significant distances through the established, dense vegetation which will provide adequate filtering to treat and remove any pollutants that may be generated in these areas.

E. Summary of Results

The proposed design and analysis indicates that the proposed development will not result in negative flooding impacts downstream. In addition, the maintenance of existing grades, vegetation and sheet flow drainage patterns during and after construction will prevent any negative impacts downstream resulting from erosion or sedimentation. Appendix 1: SOILS INFORMATION



	MAP L	EGEND)	MAP INFORMATION
Area of In	terest (AOI)	rest (AOI) 🗧 Spoil A		The soil surveys that comprise your AOI were mapped at
	Area of Interest (AOI)	۵	Stony Spot	1.12,000.
Soils	Soil Man Linit Polygons	0	Very Stony Spot	Warning: Soil Map may not be valid at this scale.
	Soil Map Unit Lines	Ŷ	Wet Spot	
~	Soil Map Unit Points	\triangle	Other	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil
Special		·**	Special Line Features	line placement. The maps do not show the small areas of
(0)	Blowout	Water Fea	atures	scale.
N N N	Borrow Pit	\sim	Streams and Canals	
×	Clay Spot	Transport	Rails	Please rely on the bar scale on each map sheet for map measurements
0	Closed Depression		Interstate Highways	
X	Gravel Pit	~	US Routes	Source of Map: Natural Resources Conservation Service
0 0 0	Gravelly Spot	~	Major Roads	Coordinate System: Web Mercator (EPSG:3857)
0	Landfill	~	Local Roads	Maps from the Web Soil Survey are based on the Web Mercator
Λ.	Lava Flow	Backgrou	Ind	projection, which preserves direction and shape but distorts
علاج	Marsh or swamp	No.	Aerial Photography	distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more
Ŕ	Mine or Quarry			accurate calculations of distance or area are required.
0	Miscellaneous Water			This product is generated from the USDA-NRCS certified data as
0	Perennial Water			of the version date(s) listed below.
\vee	Rock Outcrop			Soil Survey Area: State of Connecticut
+	Saline Spot			Survey Area Data: Version 22, Sep 12, 2022
°.*°	Sandy Spot			Soil map units are labeled (as space allows) for map scales
-	Severely Eroded Spot			1:50,000 or larger.
0	Sinkhole			Date(s) aerial images were photographed: Jun 14, 2022—Oct 6,
≽	Slide or Slip			2022
ø	Sodic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
37C	Manchester gravelly sandy loam, 3 to 15 percent slopes	43.8	44.1%
62C	Canton and Charlton fine sandy loams, 3 to 15 percent slopes, extremely stony	0.7	0.7%
64B	Cheshire fine sandy loam, 3 to 8 percent slopes, very stony	5.8	5.9%
73C	Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky	45.3	45.6%
73E	Charlton-Chatfield complex, 15 to 45 percent slopes, very rocky	3.6	3.7%
Totals for Area of Interest	·	99.3	100.0%

Map Unit Legend

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not
mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

State of Connecticut

37C—Manchester gravelly sandy loam, 3 to 15 percent slopes

Map Unit Setting

National map unit symbol: 9In6 Elevation: 0 to 1,200 feet Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 140 to 185 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Manchester and similar soils: 80 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Manchester

Setting

Landform: Terraces, outwash plains, kames, eskers Down-slope shape: Convex Across-slope shape: Convex Parent material: Sandy and gravelly glaciofluvial deposits derived from sandstone and shale and/or basalt

Typical profile

Ap - 0 to 9 inches: gravelly sandy loam

Bw - 9 to 18 inches: gravelly loamy sand

C - 18 to 65 inches: stratified extremely gravelly coarse sand to very gravelly loamy sand

Properties and qualities

Slope: 3 to 15 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 2.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4e Hydrologic Soil Group: A Ecological site: F145XY008MA - Dry Outwash Hydric soil rating: No

Minor Components

Hartford

Percent of map unit: 5 percent Landform: Terraces, outwash plains *Down-slope shape:* Linear *Across-slope shape:* Linear *Hydric soil rating:* No

Penwood

Percent of map unit: 5 percent Landform: Terraces, outwash plains Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Branford

Percent of map unit: 3 percent Landform: Terraces, outwash plains Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Ellington

Percent of map unit: 3 percent Landform: Terraces, outwash plains Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Unnamed, gravelly loamy sand surface Percent of map unit: 2 percent Hydric soil rating: No

Unnamed, nongravelly surface

Percent of map unit: 2 percent Hydric soil rating: No

62C—Canton and Charlton fine sandy loams, 3 to 15 percent slopes, extremely stony

Map Unit Setting

National map unit symbol: 2wks7 Elevation: 0 to 1,310 feet Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F Frost-free period: 140 to 240 days Farmland classification: Not prime farmland

Map Unit Composition

Canton, extremely stony, and similar soils: 50 percent Charlton, extremely stony, and similar soils: 35 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Canton, Extremely Stony

Setting

Landform: Moraines, hills, ridges

Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Nose slope, side slope, crest Down-slope shape: Convex, linear

Across-slope shape: Convex

Parent material: Coarse-loamy over sandy melt-out till derived from gneiss, granite, and/or schist

Typical profile

Oi - 0 to 2 inches: slightly decomposed plant material

A - 2 to 5 inches: fine sandy loam

Bw1 - 5 to 16 inches: fine sandy loam

Bw2 - 16 to 22 inches: gravelly fine sandy loam

2C - 22 to 67 inches: gravelly loamy sand

Properties and qualities

Slope: 3 to 15 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 19 to 39 inches to strongly contrasting textural stratification
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.14 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: B Ecological site: F144AY034CT - Well Drained Till Uplands Hydric soil rating: No

Description of Charlton, Extremely Stony

Setting

Landform: Ridges, ground moraines, hills Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Side slope, crest Down-slope shape: Convex, linear Across-slope shape: Convex Parent material: Coarse-loamy melt-out till derived from granite, gneiss, and/or schist

Typical profile

Oe - 0 to 2 inches: moderately decomposed plant material *A - 2 to 4 inches:* fine sandy loam *Bw - 4 to 27 inches:* gravelly fine sandy loam *C - 27 to 65 inches:* gravelly fine sandy loam

Properties and qualities

Slope: 3 to 15 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.14 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 8.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: B Ecological site: F144AY034CT - Well Drained Till Uplands Hydric soil rating: No

Minor Components

Chatfield, extremely stony

Percent of map unit: 5 percent Landform: Ridges, hills Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Nose slope, side slope, crest Down-slope shape: Convex Across-slope shape: Linear, convex Hydric soil rating: No

Leicester, extremely stony

Percent of map unit: 5 percent Landform: Hills, drainageways, depressions, ground moraines Landform position (two-dimensional): Footslope, toeslope Landform position (three-dimensional): Base slope Down-slope shape: Concave, linear Across-slope shape: Concave Hydric soil rating: Yes

Sutton, extremely stony

Percent of map unit: 5 percent Landform: Ground moraines, hills Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

64B—Cheshire fine sandy loam, 3 to 8 percent slopes, very stony

Map Unit Setting

National map unit symbol: 9lpz Elevation: 0 to 1,200 feet Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 140 to 185 days Farmland classification: Not prime farmland

Map Unit Composition

Cheshire and similar soils: 80 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Cheshire

Setting

Landform: Till plains, hills Down-slope shape: Linear Across-slope shape: Linear Parent material: Coarse-loamy melt-out till derived from basalt and/or sandstone and shale

Typical profile

Ap - 0 to 8 inches: fine sandy loam Bw1 - 8 to 16 inches: fine sandy loam Bw2 - 16 to 26 inches: fine sandy loam C - 26 to 65 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 8 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Moderate (about 7.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: B Ecological site: F145XY013CT - Well Drained Till Uplands Hydric soil rating: No

Minor Components

Wilbraham

Percent of map unit: 5 percent Landform: Drainageways, depressions Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Yalesville

Percent of map unit: 5 percent Landform: Ridges, hills Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Wethersfield

Percent of map unit: 5 percent Landform: Hills, drumlins Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: No

Watchaug

Percent of map unit: 3 percent Landform: Till plains, hills Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: No

Menlo

Percent of map unit: 2 percent Landform: Drainageways, depressions Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

73C—Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky

Map Unit Setting

National map unit symbol: 2w698 Elevation: 0 to 1,550 feet Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F Frost-free period: 140 to 240 days Farmland classification: Not prime farmland

Map Unit Composition

Charlton, very stony, and similar soils: 50 percent *Chatfield, very stony, and similar soils:* 30 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Charlton, Very Stony

Setting

Landform: Ridges, hills Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Side slope, crest, nose slope Down-slope shape: Convex, linear Across-slope shape: Convex Parent material: Coarse-loamy melt-out till derived from granite, gneiss, and/or schist

Typical profile

Oe - 0 to 2 inches: moderately decomposed plant material

A - 2 to 4 inches: fine sandy loam

Bw - 4 to 27 inches: gravelly fine sandy loam

C - 27 to 65 inches: gravelly fine sandy loam

Properties and qualities

Slope: 3 to 15 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.14 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 8.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: B Ecological site: F144AY034CT - Well Drained Till Uplands Hydric soil rating: No

Description of Chatfield, Very Stony

Setting

Landform: Hills, ridges Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Nose slope, side slope, crest Down-slope shape: Convex Across-slope shape: Linear, convex Parent material: Coarse-loamy melt-out till derived from granite, gneiss, and/or schist

Typical profile

Oi - 0 to 1 inches: slightly decomposed plant material

A - 1 to 2 inches: fine sandy loam

Bw - 2 to 30 inches: gravelly fine sandy loam

2R - 30 to 40 inches: bedrock

Properties and qualities

Slope: 3 to 15 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: 20 to 41 inches to lithic bedrock
Drainage class: Well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 to 0.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: B Ecological site: F144AY034CT - Well Drained Till Uplands Hydric soil rating: No

Minor Components

Rock outcrop

Percent of map unit: 5 percent Hydric soil rating: No

Hollis, very stony

Percent of map unit: 5 percent Landform: Hills, ridges Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Nose slope, side slope, crest Down-slope shape: Convex Across-slope shape: Linear, convex Hydric soil rating: No

Leicester, very stony

Percent of map unit: 5 percent Landform: Drainageways, depressions Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

Sutton, very stony

Percent of map unit: 5 percent Landform: Ground moraines, hills Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

73E—Charlton-Chatfield complex, 15 to 45 percent slopes, very rocky

Map Unit Setting

National map unit symbol: 9lql Elevation: 0 to 1,200 feet Mean annual precipitation: 43 to 56 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 140 to 185 days Farmland classification: Not prime farmland

Map Unit Composition

Charlton and similar soils: 45 percent *Chatfield and similar soils:* 30 percent *Minor components:* 25 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Charlton

Setting

Landform: Hills Down-slope shape: Linear Across-slope shape: Linear Parent material: Coarse-loamy melt-out till derived from granite and/or schist and/or gneiss

Typical profile

Ap - 0 to 4 inches: fine sandy loam Bw1 - 4 to 7 inches: fine sandy loam Bw2 - 7 to 19 inches: fine sandy loam Bw3 - 19 to 27 inches: gravelly fine sandy loam C - 27 to 65 inches: gravelly fine sandy loam

Properties and qualities

Slope: 15 to 45 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 5.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: B *Ecological site:* F144AY034CT - Well Drained Till Uplands *Hydric soil rating:* No

Description of Chatfield

Setting

Landform: Ridges, hills Down-slope shape: Convex Across-slope shape: Linear Parent material: Coarse-loamy melt-out till derived from granite and/or schist and/or gneiss

Typical profile

Oa - 0 to 1 inches: highly decomposed plant material *A - 1 to 6 inches:* gravelly fine sandy loam *Bw1 - 6 to 15 inches:* gravelly fine sandy loam *Bw2 - 15 to 29 inches:* gravelly fine sandy loam *2R - 29 to 80 inches:* unweathered bedrock

Properties and qualities

Slope: 15 to 45 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Drainage class: Well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Low to high (0.01 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 3.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: B Ecological site: F144AY034CT - Well Drained Till Uplands Hydric soil rating: No

Minor Components

Rock outcrop

Percent of map unit: 10 percent Hydric soil rating: No

Sutton

Percent of map unit: 5 percent Landform: Drainageways, depressions Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Leicester

Percent of map unit: 5 percent Landform: Drainageways, depressions Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

Hollis

Percent of map unit: 3 percent Landform: Ridges, hills Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

Unnamed, sandy subsoil

Percent of map unit: 1 percent Hydric soil rating: No

Unnamed, red parent material

Percent of map unit: 1 percent Hydric soil rating: No



72 Bissell Street Manchester, CT 06040 • Tel: 860-783-5830 • Fax: 860-783-5832

Client: JR Russo Surveyors & Engineers P. O Box 938 East Windsor, CT. 06088

Project: Somers Solar 59 South Rd. Somers, CT.

Technician: Z. A

Report #: 001-23

Lab ID: 210-23

Client ID: PH-1

Date: 09/05/2023

LAB PERMEABILITY TEST

Sample description: 1 1/2" minus reddish br. sand.

Location: Onsite (Somers Solar, 159 South Rd. Somers, CT).

Sample depth: 18" to 24"

Method: Permeability by ASTM D2434 (Constant Head Method)

k = QL/ath

Where k = coefficient of permeability,

Q = quantity of water discharged,	Q =	1000 cm^3
L = length of sample in centimeters	L =	15.24 cm
A = cross sectional area of specimen,	A =	43.10 cm ²
t = total time for discharge, in seconds	t =	1440 sec
h = difference in head manometers,	h =	61.5 cm

k = 0.003992731 cm/sec.

k = 5.7 inch/hour

Reported To: JR Russo Surveyors & Engineers

Submitted By: New England Materials Testing Lab, LLC.

Reviewed By: Laboratory Manager

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Client: JR Russo Surveyors & Engineers P. O Box 938 East Windsor, CT. 06088

Project: Somers Solar 59 South Rd. Somers, CT. **Report #:** 002-23

Lab ID: 211-23

Client ID: PH-2

Technician: Z. A

Date: 09/05/2023

LAB PERMEABILITY TEST

Sample description: 1 1/2" minus reddish br. sand, little fines.

Location: Onsite (Somers Solar, 159 South Rd. Somers, CT).

Sample depth: 18" to 24"

Method: Permeability by ASTM D2434 (Constant Head Method)

k = QL/ath

Where k = coefficient of permeability,

Q = quantity of water discharged,	Q =	500 cm^3
L = length of sample in centimeters	$\Gamma =$	15.24 cm
A = cross sectional area of specimen,	A =	43.10 cm^2
t = total time for discharge, in seconds	t =	5400 sec
h = difference in head manometers,	h =	61.5 cm

k = 0.000532364 cm/sec.

k = 0.75 inch/hour

Reported To: JR Russo Surveyors & Engineers

Submitted By: New England Materials Testing Lab, LLC.

Reviewed By: Laboratory Manager

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72 Bissell Street Manchester, CT 06040 • Tel: 860-783-5830 • Fax: 860-783-5832

Client: JR Russo Surveyors & Engineers P. O Box 938 East Windsor, CT. 06088

Project: Somers Solar 59 South Rd. Somers, CT.

Report #: 003-23

Lab ID: 212-23

Client ID: PH-6

Technician: Z. A

Date: 09/05/2023

LAB PERMEABILITY TEST

Sample description: 1 ¹/₂" minus reddish br. sand, some fines.

Location: Onsite (Somers Solar, 159 South Rd. Somers, CT).

Sample depth: 18" to 24"

Method: Permeability by ASTM D2434 (Constant Head Method)

k = QL/ath

Where k = coefficient of permeability,

Q = quantity of water discharged,	Q =	1000 cm^3
L = length of sample in centimeters	L =	15.24 cm
A = cross sectional area of specimen,	A =	43.10 cm ²
t = total time for discharge, in seconds	t =	4560 sec
h = difference in head manometers,	h =	61.6 cm

k = 0.0012588157 cm/sec.

k = 1.784 inch/hour

Reported To: JR Russo Surveyors & Engineers

Submitted By: New England Materials Testing Lab, LLC.

Reviewed By: Laboratory Manager

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72 Bissell Street Manchester, CT 06040 • Tel: 860-783-5830 • Fax: 860-783-5832

Client: JR Russo Surveyors & Engineers P. O Box 938 East Windsor, CT. 06088

Project: Somers Solar 59 South Rd. Somers, CT.

Technician: Z. A

Report #: 004-23

Lab ID: 213-23

Client ID: PH-7

Date: 09/05/2023

LAB PERMEABILITY TEST

Sample description: 1 1/2" minus reddish br. silty clayed sand.

Location: Onsite (Somers Solar, 159 South Rd. Somers, CT).

Sample depth: 18" to 24"

Method: Permeability by ASTM D2434 (Constant Head Method)

k = QL/ath

Where k = coefficient of permeability,

Q = quantity of water discharged,	Q =	400 cm^3
L = length of sample in centimeters	L =	15.24 cm
A = cross sectional area of specimen,	A =	43.10 cm^2
t = total time for discharge, in seconds	t =	6600 sec
h = difference in head manometers,	h =	61.5 cm

k = 0.000348457 cm/sec.

k = 0.49 inch/hour

Reported To: JR Russo Surveyors & Engineers

Submitted By: New England Materials Testing Lab, LLC.

Reviewed By: Laboratory Manager

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Appendix 2: RAINFALL DATA

Precipitation Frequency Data Server



Location name: Town of Somers, Connecticut, USA* Latitude: 41.977°, Longitude: -72.4422° Elevation: m/ft** * source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

NOAA Atlas 14, Volume 10, Version 3

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.334 (0.257-0.434)	0.402 (0.309-0.523)	0.513 (0.393-0.670)	0.606 (0.462-0.796)	0.733 (0.542-1.01)	0.829 (0.602-1.16)	0.929 (0.655-1.35)	1.04 (0.699-1.55)	1.20 (0.775-1.85)	1.33 (0.839-2.09)
10-min	0.473 (0.364-0.614)	0.569 (0.438-0.741)	0.727 (0.557-0.949)	0.858 (0.654-1.13)	1.04 (0.768-1.43)	1.17 (0.852-1.65)	1.32 (0.929-1.92)	1.47 (0.989-2.20)	1.70 (1.10-2.63)	1.88 (1.19-2.97)
15-min	0.556 (0.428-0.723)	0.670 (0.515-0.871)	0.856 (0.655-1.12)	1.01 (0.769-1.33)	1.22 (0.903-1.68)	1.38 (1.00-1.94)	1.55 (1.09-2.26)	1.73 (1.16-2.59)	2.00 (1.29-3.09)	2.21 (1.40-3.49)
30-min	0.753 (0.580-0.979)	0.908 (0.698-1.18)	1.16 (0.890-1.52)	1.37 (1.05-1.80)	1.66 (1.23-2.28)	1.88 (1.36-2.64)	2.11 (1.49-3.07)	2.36 (1.58-3.52)	2.72 (1.76-4.20)	3.01 (1.90-4.75)
60-min	0.950 (0.731-1.24)	1.15 (0.881-1.49)	1.47 (1.12-1.92)	1.73 (1.32-2.28)	2.10 (1.55-2.88)	2.37 (1.72-3.33)	2.66 (1.88-3.88)	2.98 (2.00-4.45)	3.44 (2.22-5.32)	3.80 (2.41-6.01)
2-hr	1.21 (0.938-1.57)	1.46 (1.12-1.88)	1.85 (1.43-2.41)	2.18 (1.67-2.85)	2.63 (1.96-3.61)	2.97 (2.17-4.17)	3.33 (2.38-4.87)	3.76 (2.53-5.58)	4.39 (2.85-6.75)	4.92 (3.12-7.73)
3-hr	1.39 (1.08-1.79)	1.67 (1.30-2.16)	2.13 (1.64-2.76)	2.51 (1.93-3.27)	3.03 (2.27-4.14)	3.42 (2.51-4.79)	3.83 (2.75-5.60)	4.34 (2.93-6.43)	5.11 (3.32-7.84)	5.77 (3.67-9.03)
6-hr	1.75 (1.37-2.25)	2.12 (1.65-2.72)	2.72 (2.12-3.51)	3.22 (2.49-4.17)	3.91 (2.94-5.33)	4.41 (3.27-6.17)	4.97 (3.60-7.27)	5.66 (3.83-8.34)	6.74 (4.40-10.3)	7.69 (4.90-12.0)
12-hr	2.17 (1.71-2.77)	2.67 (2.09-3.41)	3.47 (2.71-4.45)	4.14 (3.22-5.33)	5.06 (3.83-6.88)	5.73 (4.27-7.99)	6.48 (4.72-9.45)	7.42 (5.04-10.9)	8.91 (5.83-13.5)	10.2 (6.54-15.8)
24-hr	2.57 (2.03-3.26)	3.20 (2.52-4.06)	4.22 (3.31-5.37)	5.06 (3.95-6.49)	6.23 (4.74-8.43)	7.08 (5.30-9.84)	8.03 (5.89-11.7)	9.24 (6.29-13.5)	11.2 (7.34-16.9)	12.9 (8.28-19.8)
2-day	2.91 (2.31-3.67)	3.65 (2.89-4.61)	4.86 (3.83-6.15)	5.86 (4.60-7.46)	7.24 (5.54-9.76)	8.24 (6.21-11.4)	9.36 (6.92-13.6)	10.8 (7.40-15.7)	13.2 (8.69-19.8)	15.3 (9.86-23.4)
3-day	3.17 (2.52-3.98)	3.97 (3.16-5.00)	5.29 (4.19-6.68)	6.38 (5.02-8.10)	7.88 (6.05-10.6)	8.97 (6.78-12.4)	10.2 (7.56-14.8)	11.8 (8.07-17.0)	14.4 (9.49-21.5)	16.7 (10.8-25.5)
4-day	3.41 (2.71-4.27)	4.26 (3.39-5.35)	5.66 (4.49-7.13)	6.82 (5.38-8.63)	8.41 (6.47-11.3)	9.57 (7.24-13.2)	10.9 (8.07-15.7)	12.6 (8.62-18.1)	15.3 (10.1-22.9)	17.8 (11.5-27.0)
7-day	4.06 (3.25-5.07)	5.02 (4.01-6.27)	6.59 (5.25-8.27)	7.90 (6.26-9.96)	9.69 (7.48-12.9)	11.0 (8.35-15.0)	12.5 (9.26-17.9)	14.3 (9.87-20.6)	17.4 (11.5-25.8)	20.1 (13.0-30.3)
10-day	4.71 (3.78-5.87)	5.73 (4.60-7.15)	7.40 (5.92-9.26)	8.79 (6.98-11.1)	10.7 (8.26-14.2)	12.1 (9.18-16.4)	13.6 (10.1-19.4)	15.6 (10.8-22.3)	18.7 (12.4-27.6)	21.4 (13.9-32.2)
20-day	6.77 (5.46-8.38)	7.86 (6.34-9.74)	9.65 (7.75-12.0)	11.1 (8.89-13.9)	13.2 (10.2-17.2)	14.7 (11.1-19.6)	16.3 (12.0-22.7)	18.2 (12.6-25.8)	21.0 (14.0-30.8)	23.4 (15.2-35.0)
30-day	8.51 (6.89-10.5)	9.63 (7.79-11.9)	11.5 (9.23-14.2)	13.0 (10.4-16.2)	15.1 (11.7-19.5)	16.7 (12.6-22.0)	18.3 (13.4-25.1)	20.1 (14.0-28.3)	22.5 (15.1-32.9)	24.5 (16.0-36.6)
45-day	10.7 (8.67-13.1)	11.8 (9.59-14.5)	13.7 (11.1-16.9)	15.3 (12.3-19.0)	17.4 (13.5-22.4)	19.1 (14.4-25.0)	20.7 (15.1-28.0)	22.4 (15.6-31.4)	24.5 (16.4-35.6)	26.1 (17.0-38.8)
60-day	12.5 (10.2-15.3)	13.7 (11.1-16.8)	15.6 (12.6-19.2)	17.2 (13.8-21.3)	19.4 (15.1-24.8)	21.1 (16.0-27.5)	22.8 (16.6-30.6)	24.3 (17.1-34.0)	26.2 (17.6-38.0)	27.6 (18.0-40.9)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

Appendix 3: DRAINAGE AREA MAPS





Appendix 4: PRE-DEVELOPMENT ANALYSIS



2023-001 Louth Callan - South Rd, Somers

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Area Listing (selected nodes)

CN	Description	
	(subcatchment-numbers)	
39	>75% Grass cover, Good, HSG A (E4)	
30	Meadow, non-grazed, HSG A (E2)	
98	Paved parking, HSG A (E4)	
30	Woods, Good, HSG A (E2)	
55	Woods, Good, HSG B (E1, E2, E3, E4)	
32	Woods/grass comb., Good, HSG A (E2, E4)	
58	Woods/grass comb., Good, HSG B (E2, E4)	
	CN 39 30 98 30 55 32 58	

			Pre-Development
2023-001 Louth Callan - South	Rd, Somers	Type III 24	-hr 100-year Rainfall=8.03"
Prepared by {enter your company na	ame here}		Printed 9/14/2023
HydroCAD® 10.00-25 s/n 02386 © 2019	HydroCAD Software So	olutions LLC	Page 3
			-
Time span=	1.00-72.00 hrs, dt=0.	01 hrs, 7101 poir	nts
Runoff by SC	S TR-20 method, UH	=SCS, Weighted	-CN
Reach routing by Sim-I	Route method - Pone	d routing by Sim-	Route method
SubcatchmontE1: WOODS TO WET	Pupoff Area-1 23	8551 cf 0.00% In	panyious Punoff Depth-2.80"
Subcatchinenter. WOODS TO WET	Flow Length= $2.707'$	$C_{c}=40.5 \text{ min}$ CN=	55 Runoff=43 79 cfs 6 645 af
SubcatchmentE2: SITE TO WET	Runoff Area=86	4,358 sf 0.00% In	npervious Runoff Depth=0.50"
	Flow Length=1,037'	Tc=25.1 min CN	=31 Runoff=2.51 cfs 0.820 af
SubcatchmentE3: WOODS TO STRE	ET Runoff Area=17	2,758 sf 0.00% In	npervious Runoff Depth=2.80"
	Flow Length=1,759'	Tc=29.9 min CN	=55 Runoff=7.07 cfs 0.927 af
SubcatchmontE4: SITE TO STREET	Rupoff Area-1/	0.211 cf 1.01% In	penvious Rupoff Depth-1.45"
Subcalchinente4. Site to Street	Flow Length=930'	$T_c=28.6 \text{ min}$ CN	=42 Runoff=2.64 cfs 0.415 af
	Tiow Longar 500		
Pond P1: WET			Inflow=46.21 cfs 7.465 af
			Primary=46.21 cfs 7.465 af
Pond P2: STREET			Inflow=9.69 cfs 1.342 af
			Primary=9.69 cfs 1.342 af

Summary for Subcatchment E1: WOODS TO WET

Runoff = 43.79 cfs @ 12.60 hrs, Volume= 6.645 af, Depth= 2.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"

Α	rea (sf)	CN [Description		
1,2	38,554	55 \	Noods, Go	od, HSG B	
1,2	38,554		100.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3	100	0.1600	0.18		Sheet Flow, W
14.6	1,617	0.1369	1.85		Woods: Light underbrush n= 0.400 P2= 3.20" Shallow Concentrated Flow, W Woodland Ky= 5.0 fps
2.7	357	0.0964	2.17		Shallow Concentrated Flow, FARM
9.5	401	0.0102	0.71		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow, FARM Short Grass Pasture Kv= 7.0 fps
4.4	232	0.0310	0.88		Shallow Concentrated Flow, W Woodland Kv= 5.0 fps

40.5 2,707 Total

Summary for Subcatchment E2: SITE TO WET

Runoff = 2.51 cfs @ 12.67 hrs, Volume= 0.820 af, Depth= 0.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"

Area (sf)	CN	Description
21,189	55	Woods, Good, HSG B
11,505	58	Woods/grass comb., Good, HSG B
56,742	30	Woods, Good, HSG A
68,684	32	Woods/grass comb., Good, HSG A
706,238	30	Meadow, non-grazed, HSG A
864,358	31	Weighted Average
864,358		100.00% Pervious Area

2023-001 Louth Callan - South Rd, Somers

Pre-Development Type III 24-hr 100-year Rainfall=8.03" Printed 9/14/2023

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
2.8	20	0.1283	0.12		Sheet Flow, W
					Woods: Light underbrush n= 0.400 P2= 3.20"
5.7	80	0.1263	0.23		Sheet Flow, FARM
					Grass: Dense n= 0.240 P2= 3.20"
2.3	291	0.0919	2.12		Shallow Concentrated Flow, FARM
					Short Grass Pasture Kv= 7.0 fps
9.9	414	0.0099	0.70		Shallow Concentrated Flow, FARM
					Short Grass Pasture Kv= 7.0 fps
4.4	232	0.0310	0.88		Shallow Concentrated Flow, W
					Woodland Kv= 5.0 fps

25.1 1,037 Total

Summary for Subcatchment E3: WOODS TO STREET

Runoff = 7.07 cfs @ 12.46 hrs, Volume= 0.927 af, Depth= 2.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"

_	A	rea (sf)	CN E	Description		
_	1	72,758	55 V	Voods, Go	od, HSG B	
	1	72,758	1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	10.1	100	0.1333	0.17		Sheet Flow, W
						Woods: Light underbrush n= 0.400 P2= 3.20"
	7.6	891	0.1539	1.96		Shallow Concentrated Flow, W
	1.9	202	0.1317	1.81		Woodland Kv= 5.0 fps Shallow Concentrated Flow, ORCHARD Woodland Kv= 5.0 fps
	2.5	113	0.0228	0.75		Shallow Concentrated Flow, ORCHARD
						Woodland Kv= 5.0 fps
	1.8	200	0.1440	1.90		Shallow Concentrated Flow, ORCHARD
						Woodland Kv= 5.0 fps
	6.0	253	0.0100	0.70		Shallow Concentrated Flow, GRASS
_						Short Grass Pasture Kv= 7.0 fps

29.9 1,759 Total

Summary for Subcatchment E4: SITE TO STREET

Runoff = 2.64 cfs @ 12.49 hrs, Volume= 0.415 af, Depth= 1.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"

Pre-Development Type III 24-hr 100-year Rainfall=8.03"

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A	rea (sf)	CN	Description					
	25,220	55	Woods, Good, HSG B					
	25,450	58	Woods/gras	ss comb., G	Good, HSG B			
	85,754	32	Woods/gras	ss comb., G	Good, HSG A			
	9,895	39	>75% Gras	s cover, Go	ood, HSG A			
	2,895	98	Paved park	ing, HSG A	۱			
1	49,214	42	Weighted A	verage				
1	46,319		98.06% Pei	rvious Area				
	2,895		1.94% Impe	ervious Area	а			
Tc	Length	Slope	 Velocity 	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
12.6	92	0.0641	0.12		Sheet Flow, W			
					Woods: Light underbrush n= 0.400 P2= 3.20"			
1.4	8	0.1258	0.10		Sheet Flow, ORCHARD			
					Woods: Light underbrush n= 0.400 P2= 3.20"			
1.9	201	0.1258	i 1.77		Shallow Concentrated Flow, ORCHARD			
					Woodland Kv= 5.0 fps			
5.1	191	0.0157	0.63		Shallow Concentrated Flow, ORCHARD			
	405	0 4 5 0 0			Woodland Kv= 5.0 fps			
1.6	185	0.1530	1.96		Shallow Concentrated Flow, ORCHARD			
0.0	050	0.0400	0.70		Woodland KV= 5.0 fps			
6.0	253	0.0100	0.70		Shart Cross Desture - Kuz 7.0 fre			
					Short Grass Pasture KV= 7.0 lps			

28.6 930 Total

Summary for Pond P1: WET

Inflow A	Area	=	48.276 ac,	0.00% Imperviou	s, Inflow Depth =	= 1.8	36" for 10	0-year event
Inflow	=	=	46.21 cfs @	12.60 hrs, Volu	ne= 7.46	5 af		-
Primary	/ =	=	46.21 cfs @	12.61 hrs, Volu	ne= 7.46	5 af,	Atten= 0%	,Lag= 0.6 min

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Summary for Pond P2: STREET

Inflow Are	a =	7.391 ac,	0.90% Impervious,	Inflow Depth = 2.1	18" for 100-year event
Inflow	=	9.69 cfs @	12.46 hrs, Volume	= 1.342 af	
Primary	=	9.69 cfs @	12.47 hrs, Volume	= 1.342 af,	Atten= 0%, Lag= 0.6 min

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

2023-001 Louth Callan - South Ro Prepared by {enter your company nam HydroCAD® 10.00-25 s/n 02386 © 2019 Hy	I, Somers ne here} rdroCAD Software Solutions	Type III 24-hr LLC	Pre-Development 2-year Rainfall=3.20" Printed 9/14/2023 Page 6				
Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Sim-Route method - Pond routing by Sim-Route method							
SubcatchmentE1: WOODS TO WET	Runoff Area=1,238,554 sf	0.00% Impervio	us Runoff Depth=0.25"				
	Flow Length=2,707' Tc=40.	5 min CN=55 R	Runoff=1.88 cfs 0.594 af				
SubcatchmentE2: SITE TO WET	Runoff Area=864,358 sf	0.00% Impervio	us Runoff Depth=0.00"				
	Flow Length=1,037' Tc=25.	1 min CN=31 R	Runoff=0.00 cfs 0.000 af				
SubcatchmentE3: WOODS TO STREET	Runoff Area=172,758 sf	0.00% Impervio	us Runoff Depth=0.25"				
	Flow Length=1,759' Tc=29.	9 min CN=55 R	Runoff=0.30 cfs 0.083 af				
SubcatchmentE4: SITE TO STREET	Runoff Area=149,214 sf	1.94% Impervio	us Runoff Depth=0.01"				
	Flow Length=930' Tc=28.	6 min CN=42 R	Runoff=0.01 cfs 0.004 af				
Pond P1: WET		Pi	Inflow=1.88 cfs 0.594 af rimary=1.88 cfs 0.594 af				
Pond P2: STREET		Pi	Inflow=0.30 cfs 0.087 af rimary=0.30 cfs 0.087 af				

2023-001 Louth Callan - South Prepared by {enter your company na HydroCAD® 10.00-25 s/n 02386 © 2019	Type III 24-hr	Pre-Development 25-year Rainfall=6.23 Printed 9/14/2023 Page 7				
Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Sim-Route method - Pond routing by Sim-Route method						
SubcatchmentE1: WOODS TO WET	Runoff Area=1,238,5	554 sf 0.00% Imperv	vious Runoff Depth=1.65"			
	Flow Length=2,707' Tc	=40.5 min CN=55	Runoff=24.28 cfs 3.914 af			
SubcatchmentE2: SITE TO WET	Runoff Area=864,3	358 sf 0.00% Imperv	vious Runoff Depth=0.13"			
	Flow Length=1,037' T	c=25.1 min CN=31	Runoff=0.35 cfs 0.218 af			
SubcatchmentE3: WOODS TO STRE	ET Runoff Area=172,7	758 sf 0.00% Imperv	vious Runoff Depth=1.65"			
	Flow Length=1,759' T	c=29.9 min CN=55	Runoff=3.92 cfs 0.546 af			
SubcatchmentE4: SITE TO STREET	Runoff Area=149,2	214 sf 1.94% Imperv	vious Runoff Depth=0.70"			
	Flow Length=930' T	c=28.6 min CN=42	Runoff=0.96 cfs 0.199 af			
Pond P1: WET		F	Inflow=24.28 cfs 4.131 af Primary=24.28 cfs 4.131 af			
Pond P2: STREET			Inflow=4.81 cfs 0.745 af			

Inflow=4.81 cfs 0.745 af Primary=4.81 cfs 0.745 af

			Pre-Development
2023-001 Louth Callan - South	Rd, Somers	Type III 24-I	hr 50-year Rainfall=7.08"
Prepared by {enter your company na	ame here}		Printed 9/14/2023
HydroCAD® 10.00-25 s/n 02386 © 2019	HydroCAD Software So	lutions LLC	Page 8
Time span= Runoff by SC Reach routing by Sim-l	=1.00-72.00 hrs, dt=0.0 S TR-20 method, UH= Route method - Pond	01 hrs, 7101 points =SCS, Weighted-Cl I routing by Sim-Ro	N ute method
SubcatchmentE1: WOODS TO WET	Runoff Area=1,238 Flow Length=2,707' T	8,554 sf 0.00% Impe c=40.5 min CN=55	ervious Runoff Depth=2.17" Runoff=33.12 cfs 5.153 af
SubcatchmentE2: SITE TO WET	Runoff Area=864 Flow Length=1,037'	l,358 sf 0.00% Impe Tc=25.1 min CN=3	ervious Runoff Depth=0.28" 1 Runoff=0.80 cfs 0.459 af
SubcatchmentE3: WOODS TO STRE	ET Runoff Area=172 Flow Length=1,759'	2,758 sf 0.00% Impe Tc=29.9 min CN=5	ervious Runoff Depth=2.17" 5 Runoff=5.36 cfs 0.719 af
SubcatchmentE4: SITE TO STREET	Runoff Area=149 Flow Length=930'	9,214 sf 1.94% Impe Tc=28.6 min CN=4	ervious Runoff Depth=1.03" 2 Runoff=1.66 cfs 0.294 af
Pond P1: WET			Inflow=33.60 cfs 5.612 af Primary=33.60 cfs 5.612 af
Pond P2: STREET			Inflow=6.96 cfs 1.012 af

Inflow=6.96 cfs 1.012 af Primary=6.96 cfs 1.012 af Appendix 5: POST-DEVELOPMENT ANALYSIS



2023-001 Louth Callan - South I	Rd. Somers 7	/pe 24-hr 100-	Post Development vear Rainfall=8.03"
Prepared by {enter your company na	ame here}		Printed 9/14/2023
HydroCAD® 10.00-25 s/n 02386 © 2019	HydroCAD Software Solutions	LLC	Page 2
Time span=	1.00-72.00 hrs, dt=0.01 hrs,	7101 points	ethod
Runoff by SC	S TR-20 method, UH=SCS,	Weighted-CN	
Reach routing by Sim-F	Route method - Pond routing	g by Sim-Route me	
SubcatchmentS1: WOODS TO BASIN	1 Runoff Area=1,229,454 sf	0.00% Impervious	Runoff Depth=2.80"
	Flow Length=2,404' Tc=34.4	min CN=55 Runo	ff=47.22 cfs 6.597 af
SubcatchmentS2: SITE TO BASIN1	Runoff Area=806,951 sf	1.86% Impervious	Runoff Depth=1.45"
	Flow Length=733' Tc=19.0	min CN=42 Runo	ff=16.62 cfs 2.246 af
SubcatchmentS3: WOODS TO BASIN	12 Runoff Area=181,857 sf	0.00% Impervious	Runoff Depth=2.80"
	Flow Length=1,450' Tc=20.	6 min CN=55 Run	off=8.73 cfs 0.976 af
SubcatchmentS4: SITE TO BASIN2	Runoff Area=191,130 sf	0.00% Impervious	Runoff Depth=2.27"
	Flow Length=606' Tc=15.	2 min CN=50 Run	off=7.97 cfs 0.829 af
SubcatchmentS5: SITE TO STREET	Runoff Area=23,747 sf	12.19% Impervious	Runoff Depth=1.08"
	Flow Length=367' Tc=16.3	3 min CN=38 Run	off=0.31 cfs 0.049 af
SubcatchmentS6: SITE TO WET	Runoff Area=8,207 sf	24.20% Impervious	Runoff Depth=2.37"
	Tc=5.	0 min CN=51 Run	off=0.51 cfs 0.037 af
Pond BAS1: BASIN1	Peak Elev=271.10' Stora	age=85,726 cf Inflov	w=61.48 cfs 8.842 af
Discarded=1.97	cfs 1.105 af Primary=33.70 c	cfs 7.744 af Outflov	w=35.67 cfs 8.849 af
Pond BAS2: BASIN2	Peak Elev=291.15' Stora	age=19,903 cf Inflov	w=16.34 cfs 1.805 af
Discarded=0.	13 cfs 0.108 af Primary=8.68	cfs 1.698 af Outflo	ow=8.80 cfs 1.806 af
Pond DP1: WET		Inflo Prima	w=33.77 cfs 7.781 af ry=33.77 cfs 7.781 af
Pond DP2: STREET		Infl Prima	ow=8.90 cfs 1.747 af ary=8.90 cfs 1.747 af

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Runoff = 47.22	cfs @ 12.50 hrs,	Volume=	6.597 af,	Depth= 2.80"	1
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"

A	rea (sf)	CN D	escription		
1,201,860		55 V	Voods, Go	od, HSG B	
	27,594	48 B	rush, Goo	d, HSG B	
1,2	29,454	55 V	Veighted A	verage	
1,2	29,454	1	00.00% Pe	ervious Are	a
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.3	100	0.1600	0.18		Sheet Flow, W
					Woods: Light underbrush n= 0.400 P2= 3.20"
14.6	1,617	0.1369	1.85		Shallow Concentrated Flow, W
					Woodland Kv= 5.0 fps
2.7	357	0.0964	2.17		Shallow Concentrated Flow, ARRAY
					Short Grass Pasture Kv= 7.0 fps
7.8	330	0.0102	0.71		Shallow Concentrated Flow, ARRAY
					Short Grass Pasture Kv= 7.0 fps

2,404 Total 34.4

Summary for Subcatchment S2: SITE TO BASIN1

Runoff 16.62 cfs @ 12.33 hrs, Volume= 2.246 af, Depth= 1.45" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"

	Area (sf)	CN	Description
	498	48	Brush, Good, HSG B
	1,265	58	Meadow, non-grazed, HSG B
*	10,045	65	Meadow, non-grazed, HSG B/C
	9,578	71	Meadow, non-grazed, HSG C
	39,631	30	Woods, Good, HSG A
	16,454	30	Brush, Good, HSG A
	171,745	30	Meadow, non-grazed, HSG A
*	541,433	44	Meadow, non-grazed, HSG A/B
	1,307	58	Meadow, non-grazed, HSG B
*	14,836	98	Paved parking, HSG A/B
	159	98	Roofs, HSG A
	806,951	42	Weighted Average
	791,956		98.14% Pervious Area
	14,995		1.86% Impervious Area

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Post Development Type III 24-hr 100-year Rainfall=8.03" Printed 9/14/2023

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
2.8	20	0.1283	0.12		Sheet Flow, W
					Woods: Light underbrush n= 0.400 P2= 3.20"
5.7	80	0.1263	0.23		Sheet Flow, ARRAY
					Grass: Dense n= 0.240 P2= 3.20"
2.3	291	0.0919	2.12		Shallow Concentrated Flow, ARRAY
					Short Grass Pasture Kv= 7.0 fps
8.2	342	0.0099	0.70		Shallow Concentrated Flow, ARRAY
					Short Grass Pasture Kv= 7.0 fps
19.0	733	Total			

Summary for Subcatchment S3: WOODS TO BASIN2

Runoff	=	8.73 cfs @	12.30 hrs.	Volume=	0.976 af. Depth= 2.80"
1 tunion		0.70 013 @	12.00 113,	volume=	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"

_	A	rea (sf)	CN	Description		
	1	68,909 12 948	55 48	Woods, Goo	od, HSG B d HSG B	
-	1 1	81,857 81,857 81,857	55	Weighted A 100.00% Pe	verage ervious Are	а
	Tc (min)	Length (feet)	Slope (ft/ft	e Velocity) (ft/sec)	Capacity (cfs)	Description
	8.7	100	0.1900	0.19		Sheet Flow, W
	5.3	632	0.155 ⁻	1 1.97		Woods: Light underbrush n= 0.400 P2= 3.20" Shallow Concentrated Flow, W Woodland Ky= 5.0 fps
	0.9	99	0.131	1 1.81		Shallow Concentrated Flow, W Woodland Ky= 5.0 fps
	2.6	388	0.131	1 2.53		Shallow Concentrated Flow, ARRAY Short Grass Pasture Ky= 7.0 fps
	2.5	131	0.0153	3 0.87		Shallow Concentrated Flow, ARRAY Short Grass Pasture Ky= 7.0 fps
	0.6	100	0.1500) 2.71		Shallow Concentrated Flow, ARRAY Short Grass Pasture Kv= 7.0 fps
_						÷

20.6 1,450 Total

Summary for Subcatchment S4: SITE TO BASIN2

Runoff = 7.97 cfs @ 12.23 hrs, Volume= 0.829 af, Depth= 2.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"
Post Development Type III 24-hr 100-year Rainfall=8.03"

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_	A	rea (sf)	CN [Description		
		1,146	48 E	Brush, Goo	d, HSG B	
		2,932	58 I	Meadow, no	on-grazed,	HSG B
*		55,619	65 I	Meadow, no	on-grazed,	HSG B/C
		2,281	71 I	Meadow, no	on-grazed,	HSG C
		23,326	30 I	Meadow, no	on-grazed,	HSG A
*		87,095	44 I	Meadow, no	on-grazed,	HSG A/B
_		18,731	58 I	Meadow, no	on-grazed,	HSG B
	1	91,130	50 \	Neighted A	verage	
	1	91,130		100.00% P	ervious Are	a
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.9	100	0.0660	0.19		Sheet Flow, ARRAY
						Grass: Dense n= 0.240 P2= 3.20"
	1.5	223	0.1197	2.42		Shallow Concentrated Flow, ARRAY
						Short Grass Pasture Kv= 7.0 fps
	4.2	183	0.0109	0.73		Shallow Concentrated Flow, ARRAY
						Short Grass Pasture Kv= 7.0 fps
	0.6	100	0.1500	2.71		Shallow Concentrated Flow, ARRAY
_						Short Grass Pasture Kv= 7.0 fps
	15.2	606	Total			

Summary for Subcatchment S5: SITE TO STREET

0.049 af, Depth= 1.08"

0.31 cfs @	12.33 hrs.	Volume=	
	12.00110,	VOIGINO	
	0.31 cfs @	0.31 cfs @ 12.33 hrs,	0.31 cfs @_ 12.33 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"

A	rea (sf)	CN	Description					
	20,852 30 Meadow, non-grazed, HSG A							
	2,895 98 Paved parking, HSG A							
	23,747	38	Weighted A	verage				
	20,852		87.81% Pe	rvious Area				
	2,895		12.19% Im	pervious Ar	ea			
Tc (min)	Length (feet)	Slope (ft/ft)	e Velocity) (ft/sec)	Capacity (cfs)	Description			
6.4	100	0.1470	0.26		Sheet Flow, ARRAY			
9.9	267	0.0041	0.45		Grass: Dense n= 0.240 P2= 3.20" Shallow Concentrated Flow, ARRAY Short Grass Pasture Kv= 7.0 fps			
16.3	367	Total						

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Runoff 0.51 cfs @ 12.08 hrs, Volume= 0.037 af, Depth= 2.37" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100-year Rainfall=8.03"

	Area (sf)	CN	Description				
	998	30	Brush, Goo	d, HSG A			
	2,494	30	Meadow, no	on-grazed,	HSG A		
*	2,729	44	Meadow, no	on-grazed,	HSG A/B		
	414	98	Paved park	ing, HSG A	L Contraction of the second seco		
*	1,572	98	Paved park	ing, HSG A	/B		
	8,207	51	Weighted Average				
	6,221		75.80% Pei	vious Area			
	1,986		24.20% Imp	pervious Are	ea		
Т	c Length	Slop	e Velocity	Capacity	Description		
(mir	ı) (feet)	(ft/f	t) (ft/sec)	(cfs)			
5.	0				Direct Entry,		

Summary for Pond BAS1: BASIN1

[86] Warning: Oscillations may require smaller dt (severity=28)

Inflow Area	a =	46.749 ac,	0.74% Impervious, In	flow Depth = 2.27" for 100-	year event
Inflow	=	61.48 cfs @	12.49 hrs, Volume=	8.842 af	-
Outflow	=	35.67 cfs @	12.88 hrs, Volume=	8.849 af, Atten= 42%,	Lag= 23.6 min
Discarded	=	1.97 cfs @	12.88 hrs, Volume=	1.105 af	•
Primary	=	33.70 cfs @	12.88 hrs, Volume=	7.744 af	

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 271.10' @ 12.88 hrs Surf.Area= 36,250 sf Storage= 85,726 cf Flood Elev= 272.00' Surf.Area= 63,880 sf Storage= 130,814 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 52.8 min (943.2 - 890.4)

Volume	Invert	Avail.S	Storage S	Storage	e Description	
#1	268.00'	130	,814 cf	Custor	n Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation (feet)	Surf (.Area (sq-ft)	Inc.S (cubic-	Store feet)	Cum.Store (cubic-feet)	
268.00 270.00 271.00 272.00	2 2 3 6	1,706 9,312 3,200 3,880	51 31 48	0 ,018 ,256 3,540	0 51,018 82,274 130,814	

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Device	Routing	Invert	Outlet Devices
#1	Discarded	268.00'	0.750 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 267.00'
#2	Primary	268.00'	30.0" Round Culvert L= 156.0' Ke= 0.500
	-		Inlet / Outlet Invert= 268.00' / 267.00' S= 0.0064 '/' Cc= 0.900
			n= 0.012, Flow Area= 4.91 sf
#3	#3 Primary 271.00' 20.0' long x 10.0		20.0' long x 10.0' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Discarded OutFlow Max=1.97 cfs @ 12.88 hrs HW=271.10' (Free Discharge) **1=Exfiltration** (Controls 1.97 cfs)

Primary OutFlow Max=33.70 cfs @ 12.88 hrs HW=271.10' TW=0.00' (Dynamic Tailwater) -2=Culvert (Inlet Controls 32.14 cfs @ 6.55 fps) -3=Broad-Crested Rectangular Weir (Weir Controls 1.56 cfs @ 0.78 fps)

Summary for Pond BAS2: BASIN2

Inflow Area	a =	8.563 ac,	0.00% Impervious, Inflo	w Depth = 2.53" for 100-year event
Inflow	=	16.34 cfs @	12.27 hrs, Volume=	1.805 af
Outflow	=	8.80 cfs @	12.62 hrs, Volume=	1.806 af, Atten= 46%, Lag= 21.1 min
Discarded	=	0.13 cfs @	12.62 hrs, Volume=	0.108 af
Primary	=	8.68 cfs @	12.62 hrs, Volume=	1.698 af

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 291.15' @ 12.62 hrs Surf.Area= 9,528 sf Storage= 19,903 cf Flood Elev= 292.00' Surf.Area= 11,327 sf Storage= 28,771 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 54.5 min (929.2 - 874.7)

Volume	Invert	Avail.Sto	rage Storage	Description			
#1	288.00	28,77	1 cf Custom	n Stage Data (P	rismatic)Listed below (Recalc)		
Elevatio	on S t)	urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
288.0	00	3,250	0	0			
290.0	00	7,097	10,347	10,347			
292.0	00	11,327	18,424	28,771			
Device	Routing	Invert	Outlet Device	S			
#1	Discarded	288.00'	0.490 in/hr E Conductivity t	xfiltration over	Surface area Elevation = 276.00'		
#2	Primary	291.00'	20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64				
#3 Primary 288.3			12.0" Round Culvert L= 58.0' Ke= 0.500 Inlet / Outlet Invert= 288.30' / 278.00' S= 0.1776 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf				

Discarded OutFlow Max=0.13 cfs @ 12.62 hrs HW=291.15' (Free Discharge) **1=Exfiltration** (Controls 0.13 cfs)

Primary OutFlow Max=8.67 cfs @ 12.62 hrs HW=291.15' TW=0.00' (Dynamic Tailwater) -2=Broad-Crested Rectangular Weir (Weir Controls 2.88 cfs @ 0.96 fps) -3=Culvert (Inlet Controls 5.80 cfs @ 7.38 fps)

Summary for Pond DP1: WET

[40] Hint: Not Described (Outflow=Inflow)

Inflow Ar	rea =	46.938 ac,	0.83% Impervious,	Inflow Depth = 1.9	99" for 100-year event
Inflow	=	33.77 cfs @	12.88 hrs, Volume	= 7.781 af	-
Primary	=	33.77 cfs @	12.89 hrs, Volume	= 7.781 af,	Atten= 0%, Lag= 0.6 min

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Summary for Pond DP2: STREET

[40] Hint: Not Described (Outflow=Inflow)

Inflow /	Area	=	9.108 ac,	0.73% Impervious,	Inflow Depth = 2.3	30" for 100-year event
Inflow		=	8.90 cfs @	12.62 hrs, Volume	e 1.747 af	-
Primar	у	=	8.90 cfs @	12.63 hrs, Volume	e 1.747 af,	Atten= 0%, Lag= 0.6 min

Routing by Sim-Route method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

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l Reach rou	Time span=1.0 Runoff by SCS T Iting by Sim-Rou	0-72.00 hrs, dt=0.01 R-20 method, UH=S ite method - Pond re	hrs, 7101 points CS, Weighted-CN outing by Sim-Rou	te method
SubcatchmentS1: WOOI	DS TO BASIN1 F	Runoff Area=1,229,4 low Length=2,404' To	54 sf 0.00% Imper =34.4 min CN=55	vious Runoff Depth=0.25" Runoff=2.00 cfs 0.590 af
SubcatchmentS2: SITE 1	TO BASIN1	Runoff Area=806,9 Flow Length=733' To	51 sf 1.86% Imper =19.0 min CN=42	vious Runoff Depth=0.01" Runoff=0.03 cfs 0.021 af
SubcatchmentS3: WOOI	DS TO BASIN2 F	Runoff Area=181,8 low Length=1,450' To	57 sf 0.00% Imper =20.6 min CN=55	vious Runoff Depth=0.25" Runoff=0.36 cfs 0.087 af
SubcatchmentS4: SITE 1	TO BASIN2	Runoff Area=191,1 Flow Length=606' To	30 sf 0.00% Imper =15.2 min CN=50	vious Runoff Depth=0.13" Runoff=0.09 cfs 0.047 af
SubcatchmentS5: SITE 1	TO STREET	Runoff Area=23,74 Flow Length=367' To	7 sf 12.19% Imper =16.3 min CN=38	vious Runoff Depth=0.00" Runoff=0.00 cfs 0.000 af
SubcatchmentS6: SITE 1	TO WET	Runoff Area=8,20	7 sf 24.20% Imper Гc=5.0 min CN=51	vious Runoff Depth=0.15" Runoff=0.01 cfs 0.002 af
Pond BAS1: BASIN1	Discarded=0.48	Peak Elev=268.2 cfs 0.476 af Primary	4' Storage=5,256 c =0.35 cfs 0.152 af	f Inflow=2.00 cfs 0.611 af Outflow=0.83 cfs 0.628 af
Pond BAS2: BASIN2	Discarded=0.05	Peak Elev=288.4 cfs 0.077 af Primary	7' Storage=1,746 c =0.13 cfs 0.058 af	f Inflow=0.45 cfs 0.134 af Outflow=0.18 cfs 0.134 af
Pond DP1: WET				Inflow=0.35 cfs 0.154 af Primary=0.35 cfs 0.154 af
Pond DP2: STREET				Inflow=0.13 cfs 0.058 af Primary=0.13 cfs 0.058 af

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Time span=1.	00-72.00 hrs, dt=0.01 hrs, 7	7101 points	method
Runoff by SCS	TR-20 method, UH=SCS, V	Veighted-CN	
Reach routing by Sim-Ro	ute method - Pond routing	by Sim-Route	
SubcatchmentS1: WOODS TO BASIN1	Runoff Area=1,229,454 sf	0.00% Impervio	us Runoff Depth=1.65"
	ow Length=2,404' Tc=34.4 r	nin CN=55 Ru	noff=26.14 cfs 3.885 af
SubcatchmentS2: SITE TO BASIN1	Runoff Area=806,951 sf	1.86% Impervio	us Runoff Depth=0.70"
	Flow Length=733' Tc=19.0	min CN=42 R	unoff=5.88 cfs 1.075 af
SubcatchmentS3: WOODS TO BASIN2	Runoff Area=181,857 sf	0.00% Impervio	us Runoff Depth=1.65"
	Flow Length=1,450' Tc=20.6	min CN=55 R	unoff=4.81 cfs 0.575 af
SubcatchmentS4: SITE TO BASIN2	Runoff Area=191,130 sf	0.00% Impervio	us Runoff Depth=1.26"
	Flow Length=606' Tc=15.2	min CN=50 R	unoff=3.90 cfs 0.460 af
SubcatchmentS5: SITE TO STREET	Runoff Area=23,747 sf 1	12.19% Impervio	us Runoff Depth=0.46"
	Flow Length=367' Tc=16.3	min CN=38 R	unoff=0.09 cfs 0.021 af
SubcatchmentS6: SITE TO WET	Runoff Area=8,207 sf 2	24.20% Impervio	us Runoff Depth=1.33"
	Tc=5.0	min CN=51 R	unoff=0.26 cfs 0.021 af
Pond BAS1: BASIN1	Peak Elev=269.83' Stora	ge=46,066 cf In	flow=31.80 cfs 4.960 af
Discarded=1.26 cf	s 0.889 af Primary=16.44 cf	s 4.077 af Out	flow=17.70 cfs 4.966 af
Pond BAS2: BASIN2	Peak Elev=289.93' Sto	orage=9,873 cf _I	nflow=8.57 cfs 1.034 af
Discarded=0.09	cfs 0.095 af Primary=4.02 (cfs_0.940 af _Oเ	tflow=4.11 cfs 1.035 af
Pond DP1: WET		Ir Prir	flow=16.48 cfs 4.098 af nary=16.48 cfs 4.098 af
Pond DP2: STREET		Pr	Inflow=4.09 cfs 0.960 af imary=4.09 cfs 0.960 af

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2023-001 Louin Canan - Souin Ru, Son	
HydroCAD® 10 00-25 s/p 02386 @ 2019 HydroCAD	Contrare Solutions LLC Page 11
	P Software Solutions LEG Fage 11
Time span=1.00-72.0 Runoff by SCS TR-20	00 hrs, dt=0.01 hrs, 7101 points method, UH=SCS, Weighted-CN
Reach routing by Sim-Route me	thod - Pond routing by Sim-Route method
SubcatchmentS1: WOODS TO BASIN1 Runc Flow Len	off Area=1,229,454 sf 0.00% Impervious Runoff Depth=2.17" igth=2,404' Tc=34.4 min CN=55 Runoff=35.66 cfs 5.115 af
SubcatchmentS2: SITE TO BASIN1 Ru Flow Le	noff Area=806,951 sf 1.86% Impervious Runoff Depth=1.03" ength=733' Tc=19.0 min CN=42 Runoff=10.32 cfs 1.588 af
SubcatchmentS3: WOODS TO BASIN2 Ru Flow Le	noff Area=181,857 sf 0.00% Impervious Runoff Depth=2.17" ength=1,450' Tc=20.6 min CN=55 Runoff=6.59 cfs 0.757 af
SubcatchmentS4: SITE TO BASIN2 Ru Flow I	noff Area=191,130 sf 0.00% Impervious Runoff Depth=1.71" Length=606' Tc=15.2 min CN=50 Runoff=5.73 cfs 0.626 af
SubcatchmentS5: SITE TO STREET Ru Flow I	noff Area=23,747 sf 12.19% Impervious Runoff Depth=0.72" Length=367' Tc=16.3 min CN=38 Runoff=0.18 cfs 0.033 af
SubcatchmentS6: SITE TO WET	unoff Area=8,207 sf 24.20% Impervious Runoff Depth=1.80" Tc=5.0 min CN=51 Runoff=0.37 cfs 0.028 af
Pond BAS1: BASIN1 Pea Discarded=1.55 cfs 0.988	ak Elev=270.40' Storage=63,013 cf Inflow=45.16 cfs 6.703 af 8 af Primary=24.42 cfs 5.721 af Outflow=25.97 cfs 6.710 af
Pond BAS2: BASIN2 Pea Discarded=0.11 cfs 0.7	ak Elev=290.57' Storage=14,699 cf Inflow=12.07 cfs 1.382 af 101 af Primary=5.02 cfs 1.282 af Outflow=5.13 cfs 1.383 af
Pond DP1: WET	Inflow=24.47 cfs 5.750 af Primary=24.47 cfs 5.750 af
Pond DP2: STREET	Inflow=5.14 cfs 1.315 af Primary=5.14 cfs 1.315 af

EXHIBIT X

FEMA FLOOD MAP



EXHIBIT XI

AQUIFER PROTECTION MAP



EXHIBIT XII

SHPO INITIAL DETERMINATION

Connecticut

March 21, 2023

Mr. Timothy Coon J.R. Russo & Associates, LLC P.O. Box 938 East Windsor, CT 06088 (sent via email only to tcoon@jrrusso.com)

> Subject: Somers Solar 159 South Road Somers, Connecticut

Dear Mr. Coon:

The State Historic Preservation Office (SHPO) has reviewed your request for information concerning the potential effects to historic properties associated with the referenced project. SHPO understands that the proposed project entails the construction of a 4.95 Megawatt AC ground mounted solar photovoltaic electric facility with associated site improvements. The proposed activities are under the jurisdiction of the Connecticut Siting Council and will require a Stormwater Discharge permit issued by the Department of Energy and Environmental Protection through the authority of the Environmental Protection Agency. As a result, the project is subject to review by this office pursuant to the Connecticut Environmental Policy Act and Section 106 of the National Historic Preservation Act.

There are no properties listed on the National Register of Historic within or immediately adjacent to the Area of Potential Effect (APE) associated with the project. In addition, two previously identified archaeological sites (Site 129-1 and 129-3) have been recorded within a mile of the proposed project. SHPO understands that much of the project area has been previously disturbed by a gravel operation in operation between 1990 and 2010. However, readily available historic aerial imagery indicates that approximately four acres at the southern end of the APE remain intact and retain the potential to contain intact archaeological deposits. Further, the potentially intact portion of the APE retains environmental characteristics frequently associated with archaeological sites. Therefore, SHPO requests that a professional archaeological assessment and reconnaissance survey be completed prior to construction. Areas that will not be developed do not need to be tested. Subsurface testing should evaluate all areas of anticipated ground disturbance that retain a moderate/high archaeological sensitivity unless sufficient research or fieldwork documents that this level of effort is unwarranted. All work should be in compliance with our Environmental Review Primer for Connecticut's Archaeological Resources and no construction or other project-related ground disturbance should be initiated until SHPO has had an opportunity to review and comment upon the requested survey. A list of qualified consultants is attached for your convenience.

SHPO appreciates the opportunity to comment upon this project and we look forward to continuing consultation. Do not hesitate to contact Cory Atkinson, Staff Archaeologist and Environmental Reviewer, for additional information at (860) 500-2458 or cory.atkinson@ct.gov.

Sincerely,

lonathan heaves

Jonathan Kinney State Historic Preservation Officer

EXHIBIT XIII

PHASE 1 ARCHEOLOGICAL INVESTIGATION



PHASE I ARCHEOLOGICAL INVESTIGATION Somers CT Solar

Town of Somers Tolland County, Connecticut

HAA 5987-PH1

Submitted to: Santa Fuel Inc. 154 Admiral Street Bridgeport, CT 06605

Prepared by: Hartgen Archeological Associates, Inc.

1744 Washington Avenue Ext. Rensselaer, New York 12144 p +1 518 283 0534 f +1 518 283 6276 e hartgen@hartgen.com

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June 2023

MANAGEMENT SUMMARY

Involved Agencies:	United States Department of Energy and Environmental Protection and United States Environmental Protection Agency (US EPA)
Phase of survey:	Phase I Archeological Investigation
LOCATION INFORMATION	

Municipality:	Town of Somers
County:	Tolland County

ARCHEOLOGICAL SURVEY OVERVIEW

Area of Potential Effect (APE):	20 acres
Survey Area:	5.5 acres
Shovel Tests:	78 tests at 15-meter intervals

RESULTS OF ARCHEOLOGICAL SURVEY

Precontact sites identified:	None
Historic sites identified:	None

RECOMMENDATIONS

Based on the results of the Phase I archeological investigation, the APE will not affect any significant archeological sites or deposits. No national Register-eligible sites or properties will be impacted by the Project. No further work is recommended for the Somers Connecticut Solar APE if the current project plans are not altered or changed to include further ground disturbance.

Report Authors:	Jaclyn Galdun MA RPA, and Matthew J. Kirk MA RPA
Date of Report:	June 27, 2023

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Photo 4. Archeologist excavating Shovel Test 63 in the wooded eastern portion of the APE, view to north.

Photo 5. Sloped southern portion of APE of proposed access road, view to north.

Photo 6. State Registered Historic Property found at 139/159 South Road, view from northeast.

PHASE I ARCHEOLOGICAL INVESTIGATION

1 Introduction

Hartgen Archeological Associates, Inc. (Hartgen) conducted a Phase I archeological investigation for the proposed Somers Solar (Project) located in the Town of Somers, Tolland County, Connecticut. The Project requires permits issued by the Connecticut Department of Energy and Environmental Protection through the United States Environmental Protection Agency (US EPA).

This investigation was conducted to comply with Section 106 of the National Historical Preservation Act and will be reviewed by the Connecticut State Historic Preservation Office. The investigation and report was conducted according to the Connecticut Historical Commission/State Historic Preservation Office's Environmental Review Primer for Connecticut's Archaeological Resources (1987).

1.1 Project Location

The Project is located in the southern portion of the town of Somers. The proposed Project is located in an agricultural field and wooded area south of the intersection of South Road and Field Road and north of Mountain View Road (Map 1).

1.2 Description of the Project

The Project entails the construction of ground mounted solar arrays on drilled or driven posts, associated access roads, electrical lines, and equipment pads (Appendix 1). According to plans available during fieldwork, it is unclear whether trees located in proposed locations of solar arrays will be simply cut to accommodate the arrays or will be cut then stumped and grubbed.

1.3 Description of the Area of Potential Effects (APE)

The area of potential effects (APE) includes all portions of the property that will be directly altered by the proposed undertaking. The APE encompasses 20 acres (Map 2). However, according to historical aerial imagery, a letter from the Connecticut State Historic Preservation Office, and speaking to the landowner, the northern portion of the Project has been severely disturbed by a gravel operation from 1990 to 2010. Excluding the area affected by the gravel operation, the Phase IB study area includes 5.5 acres (Map 2).

2 Environmental Background

The environment of an area is significant for determining the sensitivity of the APE for archeological resources. Precontact and historic groups often favored level, well-drained areas near wetlands and waterways. Therefore, topography, proximity to wetlands, and soils are examined to determine if there are landforms in the APE that are more likely to contain archeological resources. In addition, bedrock formations may contain chert or other resources that may have been quarried by precontact groups. Soil conditions can provide a clue to past climatic conditions, as well as changes in local hydrography.

2.1 Soils

Soil surveys provide a general characterization of the types and depth of soils that are found in an area. This information is an important factor in determining the appropriate methodology if and when a field study is recommended. The source of this data is the Soil Survey Geographic (SSURGO) Database, maintained by the Natural Resources Conservation Service, United States Department of Agriculture (2018). A total of two soils are mapped within the APE and are shown on Map 3 and listed in Table 1. There are no alluvial, colluvial, aeolian, or fill soils present. Therefore, any archeological deposits present are likely to be located at shallow depths. Shovel testing is an appropriate survey methodology.

Symbol	Name	Depth	Textures	Slope	Drainage	Landform
37C	Manchester gravelly sandy loam	0-22 cm (0-9in)	Gravelly sandy loam	3-15%	Excessively drained	Terraces, outwash plains, kames, eskers
		22- 45 cm (9-18in)	Gravelly sandy loam			
		45 -165 cm (18-65 in)	Stratified extremely gravelly coarse sand to gravelly loamy sand			
64B	Cheshire fine sandy loam	0-22 cm (0-8 in)	Fine sandy loam	3-8% Well drained	Till plains, hills	
		22-40 cm (8-16 in)	Fine sandy loam			
		40-66 cm (16-26 in)	Fine sandy loam			
		66-165 cm (26-65 in)	Gravelly sandy loam			

Table	1. Soils	in the	APE.
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2.2 Bedrock Geology

According to the Geologic Map of Connecticut, the bedrock within the APE is both the Portland Arkose formation and silicified rock and mylonite along Mesozoic faults (Rodgers 1985). The Portland Arkose formation was formed during the Lower Jurassic and is comprised of sandstone, siltstone, and shale and is not chert bearing. The Portland Arkose formation is part of the Central Lowlands of Connecticut and part of the Newark supergroup that was formed in the Upper Triassic and Lower Jurassic (Rodgers 1985). The silicified rock and mylonite along Mesozoic faults were formed during the Jurassic and made up of mylonite. This bedrock formation is part of the Eastern Uplands and is fault related rocks (Rodgers 1985). There are no known outcrops within the APE.

2.3 Topography and Hydrography

The Project consists of relatively flat terrain that slopes upwards to the east and south. The northern portion of the APE consists of open fields/agricultural fields that were previously disturbed by a gravel operation. The southern portion of the APE is an overgrown orchard with some paths as well as a wooded area. There are no streams or bodies of water located within the APE, and the APE is drained by intermittent streams. However, three ponds found are directly to the west of the APE with the Shady Brook to the north flowing southwest (Map 2).

3 Documentary Research

Hartgen conducted research using the Connecticut Cultural Resource Information System (ConnCRIS), which is maintained by the Connecticut State Historic Preservation Office (CT SHPO) and the Department of Economic and Community Development. ConnCRIS contains a comprehensive inventory of State and National Register (NR) properties and properties determined eligible for the NR (NRE). Files detailing inventoried archeological sites and previous cultural resource surveys were obtained with the assistance of Catherine Labadia of CT SHPO. The National Park Service (NPS) online database of NR listed properties was also consulted.

3.1 Archeological Sites

An examination of files from CT SHPO identified two reported archeological sites within one mile (1.6 km) of the APE (Table 2). Previously reported archeological sites provide an overview of both the types of sites that may be present in the APE and the relation of sites throughout the surrounding region. The presence of few reported sites, however, may result from a lack of previous systematic survey and does not necessarily indicate a decreased archeological sensitivity within the APE.

The two sites located within one mile of the APE are both precontact sites. The first which is the Gulf Stream Site, identified by the landowner while surface collecting is an assemblage of ten precontact ceramic sherds that

date to the Middle Woodland. The second precontact site, which is closest to the APE, is composed of five dime-sized quartz projectile points with an unknown cultural affiliation.

Table 2. Al cheological sites within one time (1.0 km) of the Al L.					
Site No.	Site Identifier	Description	Proximity to APE		
129-001	Gulf Stream Site	Middle Woodland site consisting of ceramic sherds; several sherds appear decorated.	4,800 feet northeast		
129-003	Stevenson House	Possible precontact site consisting of five small (dime- sized) quartz projectile points recovered from a disturbed context.	4,200 feet northeast		

Table 2. Archeological sites within one mile (1.6 km) of the APE.

3.2 Historic Properties

An examination of ConnCRIS and the NPS database identified seven inventoried properties within the vicinity of the APE. Of these, none of the properties are listed on the NR, and none have been determined eligible for the National Register. The seven properties are located within the Bordeaux Farmstead, a 108.4-acre farm located directly west of the APE boundary at 159 South Road (CT-83). The Bordeaux Farmstead is State Register (SR) listed; four of the seven inventoried properties are contributing resources to the SR listing for the farmstead (Table 3).

Table 3.	Inventoried	properties	near the	APE.
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Property Name	Status	Description	Proximity to APE
House	SR listed	The 1 ½-story Bordeaux farmhouse was constructed in the late- 18 th century in vernacular style with a gable-end roof. Construction materials include mortared fieldstone foundation, painted clapboard siding, and asphalt shingle roof. Contributing resource to Bordeaux Farmstead.	140 feet west
Barn	SR listed	Extended English barn in vernacular style located north of the Bordeaux House; constructed c. 1790, with later additions. Construction materials include fieldstone and poured concrete foundations, vertical wood siding, and asphalt shingle roof. Contributing resource to Bordeaux Farmstead.	180 feet west
Workshop	SR listed	Located southeast of the house, the workshop has dry-laid stone foundation, wooden siding, and asphalt shingle roof. Contributing resource to Bordeaux Farmstead.	100 feet west
Milk House	SR listed	Located east of the Bordeaux House, the milk house was constructed in the late-19 th century. Contributing resource to Bordeaux Farmstead.	140 feet west
Farmstand	Not eligible	Located just east of South Road, the farmstand was constructed in the mid-20 th century and is noncontributing to the Bordeaux Farmstead.	150 feet west
Wagon Shed	Not eligible	Located east of the house, the wagon shed was constructed in the mid-20 th century and is noncontributing to the Bordeaux Farmstead.	30 feet west
Соор	Not eligible	Located east of the house, the coop was constructed in the mid- 20 th century and is noncontributing to the Bordeaux Farmstead.	35 feet west

3.3 Previous Surveys

A review of files from CT SHPO identified no previous surveys within the immediate vicinity of the Project.

4 Historical Map Review

Maps depicting the APE between 1796 and the present were examined. Selected maps of Eaton 1857, Gray 1869, USGS 1892 and 1952 are reproduced in Map 4a. There was no map documented or existing structures within the APE.

The earliest map of the APE is the Sotzman 1796 *Atlas Von Nordamerika* which shows county boundaries, waterways and bodies of water, roadways, and major topography such as mountains and hills. However, information of home/parcel ownership and presence of structures is not rendered.

The first detailed map of the APE and surrounding area is the Eaton 1857 *Map of Tolland County, Connecticut: from actual surveys.* Similar to the Sotzman 1796 map, the Eaton 1857 map depicts major roads, major topography, and water ways, but also includes smaller roads and structures and the owner's names. For instance, the "P. Fuller" which is now known as the Bordeaux Farmstead (SR # 24073) and is listed on the State Register of Historic Places is directly west of the APE. The farmstead was constructed ca. 1790 in the Extended English Barn, vernacular style with various outbuildings which include a barn, shop, farmstand and chicken coop.

The developing town of Somers is pictured to the north, and referred to as Somers Street, with multiple structures surrounding the modern-day intersections of South Road, Springfield Road, and Main Street. Just south of the street intersections is Gulf Stream immediately north of the APE. The stream supported at least one mill as shown on the Eaton 1857 map. The road configurations on the Eaton 1857 map for the most part, follow the current roads.

Gray's 1869 *Atlas of Windham and Tolland Counties, with a Map of Connecticut, from Actual Surveys* map provides a more detailed picture of the APE and the immediate surrounding area. The "P. Fuller" structure depicted on the Eaton 1857 map is now labeled as "H.L. Fuller" with a "P. Fuller" structure right across the street. There are no structures within the APE, and no additional structures adjacent to the APE. The road configurations are similar to the modern road configurations. The developing town north of the APE which is known as Somers today is still referred to as Somers Street on this map.

The 1886 Beers *Map of the State of Connecticut* shows the APE generally with very little detail. The map details roads, county boundaries, and major waterways such as the Scantic River which is found northwest of the commercial/downtown center of Somers. The town of Somers is shown as a series of dense buildings and structures but no information concerning ownership or type of structure is available. The road configurations shown on the map resemble modern road configurations. Similarly, the 1893 *Town and City Atlas of the State of Connecticut* renders similar features such as the road configurations and waterways. However, the 1893 map does offer specific information about certain structures and their purpose. For instance, in the commercial portion of Somers, there is a hotel, two churches, and a schoolhouse located on Main Street. In addition, topographical features such as mountains and hills are also depicted.

Lastly, a series of USGS topographic maps were reviewed from the years 1892, 1921, 1952, 1967, and 1984. All of the topographic maps reviewed include the same features. The road alignments shown in all the topographic maps match the current configuration of roads in the area. All the maps show water ways, structures, and topographic features of mountains and hills, as well as road names. The topographic maps through time show development of rural areas radiating around Somers, and even the expansion of residential development west of the APE across South Road.

5 Archeological Sensitivity Assessment

Archeologically sensitive areas exhibit one or more variables that may contain evidence of past human activities. Sensitive areas can include location near known sites that share the same ecological setting or similar landform (e.g., terrace above a river). They may also include areas where historic maps or photographs show that a building once stood.

······································				
Precontact Historic		Historic		
Water sources: wetlands, ponds, streams, lakes, bays	\boxtimes	Water sources: wetlands, ponds, streams, lakes, bays	\boxtimes	
and ocean		and ocean		
Nearby chert sources		Nearby natural resources (iron, limestone, building stone, etc.)		
Well-drained soils for habitation	\boxtimes	Well-drained soils for habitation	\boxtimes	

Table 4. Factors influencing precontact and historic archeological sensitivity of the APE.

Precontact		Historic	
Favorable landforms (level, good solar exposure, leeward facing)	\boxtimes	Proximity to transportation systems (road, canals, rivers, railroads, etc.)	
Known archeological sites in the vicinity	\boxtimes	Known archeological sites in the vicinity	
Other documentary sources		Map-documented structures	\boxtimes
Abundance of nearby stone tool ores		Other documentary evidence	
Overall assessment: Moderate sensitivity		Overall assessment: Moderate sensitivity	

6 Archeological Potential

Archeological potential is the likelihood of locating intact archeological remains within an area. The consideration of archeological potential takes into account subsequent uses of an area and the impact those uses would likely have on archeological remains.

A large portion of the APE has been documented to have been disturbed previously from a gravel operation operating in the northern portion of the APE. From the years 1990 to 2010, aerial photography of the APE shows significant ground disturbance and earth moving within the APE. Based on the aerial photography available the disturbance began in the northwest corner in 2004, where an access road connected to South Road (Map 5b). By 2009, the ground moving expanded to the eastern and central portion of the APE (Map 5b). A letter concerning this Project from the Connecticut State Historic Preservation Office (SHPO) stated that the entire northern extent of the APE, besides the 5.5 acres in the southern extent of the APE, has been disturbed previously (See Appendix 3).

Table 5. Factors influencing archeological potential within the APE.

Precontact		Historic	
Undisturbed soils	\boxtimes	Lack of modern development	\boxtimes
No erosion or cutting of sediments		Limited historical re-use of landscape	\boxtimes
Alluvial deposits (cap and preserve deposits)		Alluvial deposits (cap and preserve deposits)	
Relatively deep soils (features)		Historic fill (cap and preserve deposits)	
		Relatively deep soils (features)	
Overall assessment: Low potential		Overall assessment: Moderate potential	

7 Survey Methodology

Archeological sensitivity for precontact and historic resources within the APE is considered moderate, the potential for the archeological resources is considered low for precontact resources and moderate for historic resources due to the previous disturbance of the gravel operation in the area. The soils mapped within the APE are not alluvium or eolian and are relatively shallow, so hand excavated shovel tests are an appropriate survey method for the Project.

7.1 Shovel Testing

Shovel tests were excavated at a standard interval of 15 meters (50 ft). Each shovel test was 40 centimeters in diameter. All excavated soil was passed through 0.25-inch hardware mesh and examined for both precontact (Native American) and historic artifacts. The stratigraphy of each test was recorded including the depth, Munsell color, soil description, and artifact content (Munsell Color 2000). The location of each shovel test was plotted on the project map.

7.2 Artifacts and Laboratory

As general procedure, all precontact (Native American) cultural material identified during the fieldwork are collected. Significant historic artifacts such as glass, ceramics, food remains, hardware, and miscellaneous items are collected. Coal, ash, cinder, brick, and modern materials are noted. Any artifacts collected are placed in paper or plastic bags labeled by provenience and inventoried in a bag list. Bags are numbered in the field and

transported to the Hartgen laboratory in the Town of North Greenbush, Rensselaer County, New York, for processing. Shovel test records and other provenience information were entered into a Microsoft *Access* database (Appendix 1).

8 Survey Results

The Phase IB archeological field reconnaissance was conducted on June 19 to June 20th. The field crew consisted of Catherine Gagnon, Jacquelene Poveromo, and Erin Sheades under the direction of Jaclyn Galdun. Matthew J. Kirk, MA, was the Principal Investigator. The APE is partly agricultural hay fields (Photo 1) and an overgrown orchard with mowed paths through portions of the southern APE (Photo 2 and 3). A wooded area is found along the eastern and southern portions of the APE that is sloped at the very southern portion (Photo 4 and 5). The structures associated with 139/159 South Road are found to the west outside of the APE boundary (Photo 6). The weather was sunny and warm with a slight breeze.

A total of 78 shovel tests were excavated across the southern portion of the APE, where it was determined to not be disturbed by previous ground disturbance associated with the gravel operation to the north.

The soil mapped throughout the site was topsoil/A-horizon over subsoil/B-horizon. The A-horizon is a dark brown to dark yellowish brown silty loam with gravel and cobbles and ranged in thickness from 11 centimeters in the wooded portion to 33 centimeters in the overgrown orchard closer to South Road. The B-horizon is brown with gravel and cobbles. Shovel tests were excavated to an average depth of 43 centimeters. No artifacts were recovered from shovel test excavations in the southern undisturbed portion of the APE.

9 Recommendations

Based on the results of the Phase I archeological investigation, the APE will not affect any significant archeological sites or deposits. No National Register-eligible sites or properties will be impacted by the Project. No further work is recommended for the Somers Connecticut Solar APE, if the current project plans are not altered or changed to include further ground disturbance.

10 Bibliography

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Maps



Meters

Phase I Study Area



Connecticut Environmental Conditions Online, Orthoimagery, 2019

Map 2

Somers Connecticut Solar, Town of Somers, Tolland County, Connecticut Phase I Archeological Investigation







Photographs



Photo 1. Northern view of the northern portion of APE previously disturbed from previous gravel operation.



Photo 2. Archeologist excavating Shovel Test 20 in overgrown orchard with mowed paths, view to the south.



Photo 3. Sloped area located in southeastern portion of APE, view to southeast.



Photo 4. Archeologist excavating Shovel Test 63 in the wooded eastern portion of the APE, view to north.



Photo 5. Sloped southern portion of APE of proposed access road, view to north.



Photo 6. State Registered Historic Property found at 139/159 South Road, view from northeast.

Appendix 1: Shovel Test Records
<u>Test</u>	<u>Ending</u> Depth (cm)	<u>Level</u>	Muns	<u>ell Color</u>	<u>Soil Type</u>	Soil Inclusions	<u>Termination</u> <u>Reason</u>	Not Collected
1	20	1	10yr 3/3	dark brown	sand loam	gravel, cobbles		Brick (13 x 5 x 3.5cm)
	40	2	7.5yr 4/4 brown		sand loam	cobbles, glacial till	subsoil	
2	24	1	10yr 3/4	dark yellowish brown	silt	gravel, cobbles		
	44	2	7.5yr 4/4	brown	silt	gravel, cobbles	subsoil	
3	33	1	7.5yr 3/4	dark brown	silt loam	gravel, cobbles, roots		
	58	2	7.5yr 5/6	strong brown	sand	gravel, cobbles	subsoil	
4	24	1	10yr 3/4	dark yellowish brown	silt	gravel, cobbles		
	46	2	7.5yr 4/4	brown	silt	gravel, cobbles	subsoil	
5	34	1	10yr 3/2	very dark grayish brown	sand loam	gravel, cobbles		
	52	2	7.5yr 4/4	brown	sand loam	gravel, cobbles	subsoil	
6	23	1	7.5yr 4/4	brown	loam	gravel, cobbles, roots		
	46	2	2.5yr 5/6	red	sand	gravel, cobbles	subsoil	
7	20	1	10yr 3/2	very dark grayish brown	sand loam	gravel, cobbles		
	40	2	7.5yr 4/4	brown	sand loam	gravel, cobbles, glacial till	subsoil	
8	22	1	10yr 3/4	dark yellowish brown	silt	gravel, cobbles		
	30	2	7.5yr 4/4	brown	silt	gravel, cobbles		
	55	3	10yr 5/4	yellowish brown	sand	gravel	subsoil	
9	24	1	7.5yr 4/3	brown	silt loam	gravel, cobbles, roots		
	46	2	7.5yr 5/6	strong brown	sand loam	gravel, cobbles	subsoil	
10	16	1	10yr 3/4	dark yellowish brown	silt loam	gravel, cobbles, roots		Modern glass frag
	40	2	10yr 5/8	yellowish brown	sand loam	cobbles	subsoil	
11	23	1	10yr 3/2	very dark grayish brown	sand loam			
	48	2	7.5yr 4/4	brown	sand loam	cobbles	subsoil	
12	28	1	10yr 3/4	dark yellowish brown	silt	gravel, roots		
	48	2	7.5yr 4/4	brown	silt sand	gravel, cobbles	subsoil	

Shovel Test Records

Shove	I Test Rec	ords						
<u>Test</u>	<u>Ending</u> Depth (cm)	Level	Muns	ell Color	<u>Soil Type</u>	Soil Inclusions	<u>Termination</u> <u>Reason</u>	Not Collected
13	23	1	10yr 3/6	dark yellowish brown	silt loam	gravel, cobbles		
	49	2	7.5yr 4/4	brown	silt loam	gravel, cobbles	subsoil	
14	22	1	5yr 3/4	dark reddish brown	sand loam	gravel, cobbles, roots		
	43	2	5yr 4/6	yellowish red	sand	cobbles	subsoil	
15	33	1	10yr 3/2	very dark grayish brown	silt sand	gravel		
	55	2	7.5yr 4/4	brown	silt sand	gravel	subsoil	
16	23	1	10yr 3/3	dark brown	silt loam	gravel, cobbles		
	52	2	10yr 5/4	yellowish brown	silt loam	gravel, cobbles	subsoil	
17	30	1	10yr 3/2	very dark grayish brown	sand loam	gravel, cobbles		
	45	2	7.5yr 4/4	brown	sand loam	gravel, cobbles	subsoil	
18	8 20 1 5yr 3/3 dark reddi		dark reddish brown	silt loam	gravel, cobbles			
	41	2	2.5yr 4/6	dark red	sand loam	gravel, cobbles	subsoil	
19	18	1	5yr 4/3	reddish brown	silt loam	gravel, cobbles		
	39	2	5yr 5/6	yellowish red	sand loam	cobbles	subsoil	
20	30	1	10yr 3/4	dark yellowish brown	silt sand	gravel, cobbles, roots		
	50	2	7.5yr 4/4	brown	silt sand	gravel	subsoil	
21	26	1	10yr 3/3	dark brown	silt loam	gravel, cobbles		
	56	2	10yr 5/4	yellowish brown	silt loam	gravel, cobbles	subsoil	
22	25	1	10yr 3/2	very dark grayish brown	sand loam	gravel, cobbles		
	45	2	7.5yr 4/4	brown	sand loam	gravel, cobbles	subsoil	
23	13	1	5yr 3/2	dark reddish brown	silt loam	gravel, cobbles		
	47	2	5yr 4/6	yellowish red	silt clay		subsoil	
24	26	1	10yr 3/4	dark yellowish brown	silt sand	gravel		
	50	2	7.5yr 4/4	brown	silt sand	gravel, cobbles	subsoil	
25	32	1	10yr 3/3	dark brown	sand loam	roots		
	50	2	7.5yr 4/4	brown	sand loam	cobbles, roots	subsoil	

Shove	el Test Reco	ords								
<u>Test</u>	Ending Depth (cm) Level		Ending t Depth (cm) Level		Muns	ell Color	<u>Soil Type</u>	Soil Inclusions	<u>Termination</u> <u>Reason</u>	Not Collected
26	22	1	7.5yr 3/3	dark brown	silt loam	gravel, cobbles				
	38	2	7.5yr 5/6	strong brown	silt loam	gravel	subsoil			
27	26	1	7.5yr 3/3	dark brown	silt loam	gravel, cobbles, roots				
	41	2	7.5yr 5/6	strong brown	silt loam	gravel	subsoil			
28	30	1	10yr 3/2	very dark grayish brown	sand loam	roots				
	45	2	7.5yr 4/4	brown	sand loam	gravel, cobbles	subsoil			
29	23	1	10yr 3/2	very dark grayish brown	silt loam	gravel, roots				
	50	2	7.5yr 4/4	brown	loam	gravel, cobbles, roots	subsoil			
30	21	1	7.5yr 3/3	dark brown	silt loam	gravel, cobbles, roots				
	45	2	7.5yr 5/8	strong brown	silt loam	cobbles	subsoil			
31	31	1	10yr 3/3	dark brown	silt sand	gravel, roots				
	41	2	7.5yr 4/4	brown	loam	gravel	impasse (compact soil)			
32	25	1	10yr 3/2	very dark grayish brown	loam	gravel, roots				
	40	2	10yr 5/4	yellowish brown	sand loam	gravel, cobbles, glacial till	subsoil			
33	14	1	7.5yr 3/3	dark brown	silt loam	gravel, cobbles, roots				
	40	2	7.5yr 5/8	strong brown	silt loam	cobbles	subsoil			
34	20	1	10yr 3/2	very dark grayish brown	sand loam	gravel, cobbles, roots				
	35	2	10yr 5/4	yellowish brown	sand loam	gravel, cobbles	subsoil			
35	26	1	10yr 3/4	dark yellowish brown	sand loam					
	46	2	7.5yr 4/4	brown	sand loam		subsoil			
36	23	1	7.5yr 3/3	dark brown	silt loam	gravel, cobbles, roots				
	48	2	7.5yr 5/8	strong brown	silt loam	cobbles	subsoil			
37	21	1	10yr 3/4	dark yellowish brown	silt sand	gravel, cobbles				
	40	2	7.5yr 4/4	brown	loam	gravel, cobbles	subsoil			

Shove	el Test Re	cords						
<u>Test</u>	Ending Depth (cm	<u>) Level</u>	Munse	ell Color	<u>Soil Type</u>	Soil Inclusions	<u>Termination</u> <u>Reason</u>	Not Collected
38	30	1	7.5yr 3/3	dark brown	silt loam	gravel, cobbles, roots		
	52	2	7.5yr 5/8	strong brown	silt loam	cobbles	subsoil	
39	25	1	10yr 3/3	dark brown	silt loam	gravel, cobbles		
	56	2	10yr 5/4	yellowish brown	silt loam	gravel, cobbles	subsoil	
40	25	1	7.5yr 3/2	dark brown	silt loam	gravel, cobbles, roots		
	50	2	7.5yr 5/6	strong brown	silt loam	gravel, cobbles	subsoil	
41	28	1	10yr 3/3	dark brown	silt loam	gravel, cobbles		
	57	2	10yr 5/4	yellowish brown	silt loam	gravel, cobbles	subsoil	
42	26	1	10yr 3/3	dark brown	silt loam	gravel, cobbles		
	33	2	10yr 5/4	yellowish brown	silt loam	gravel, cobbles	impasse (roots)	
43	30	1	10yr 3/2	very dark grayish brown	sand loam	roots		
	50	2	10yr 5/4	yellowish brown	sand loam	gravel, cobbles	subsoil	
44	27	1	10yr 3/2	very dark grayish brown	sand loam	roots		
	45	2	10yr 5/4	yellowish brown	sand loam	gravel, cobbles	subsoil	
45	30	1	10yr 3/3	dark brown	loam	gravel, cobbles, roots	impasse (rocks)	
46	25	1	10yr 3/3	dark brown	silt loam	gravel, cobbles		
	55	2	10yr 5/4	yellowish brown	silt loam	gravel, cobbles	subsoil	
47	23	1	10yr 3/3	dark brown	silt loam	gravel, cobbles		
	54	2	7.5yr 4/4	brown	silt loam	gravel, cobbles	subsoil	
48	30	1	7.5yr 3/2	dark brown	silt loam	gravel, cobbles, roots		
	39	2	7.5yr 4/3	brown	sand loam	gravel, cobbles		
	62	3	7.5yr 5/6	strong brown	sand clay	gravel	subsoil	
49	22	1	10yr 3/2	very dark grayish brown	silt sand	gravel, cobbles, roots		
	29	2	10yr 2/2	very dark brown	sand clay			
	50	3	7.5yr 4/4	brown	loam	gravel, cobbles	subsoil	

Shovel Test Records

<u>Test</u>	<u>Ending</u> Depth (cm)	Level	Muns	ell Color	<u>Soil Type</u>	Soil Inclusions	<u>Termination</u> <u>Reason</u>	Not Collected
50	24	1	10yr 2/2	very dark brown	sand loam	gravel, cobbles, roots		
	45	2	10yr 3/4	dark yellowish brown	sand loam	gravel, cobbles	subsoil	
51	21	1	10yr 3/2	10yr 3/2 very dark grayish brown		gravel, cobbles		
	38	2	10yr 3/4	dark yellowish brown	sand loam	gravel, cobbles	subsoil	
52	22	1	10yr 3/3	dark brown	silt loam	gravel, cobbles, roots	impasse (rocks)	
53	20	1	10yr 3/2	very dark grayish brown	sand loam	gravel, cobbles		Barbed wire
	38	2	10yr 4/4	dark yellowish brown	sand loam	gravel, cobbles	subsoil	
54	27	1	7.5yr 4/3	brown	silt loam	gravel, cobbles		
	56	2	7.5yr 5/8	strong brown	silt loam	gravel, cobbles	subsoil	
55	24	1	10yr 3/3	dark brown	silt loam	gravel, cobbles, roots	impasse (roots, rocks)	
56	35	1	10yr 3/2	very dark grayish brown	sand loam	gravel, cobbles, roots	impasse (roots, rocks)	
57	20	1	10yr 3/3	dark brown	silt loam	gravel, cobbles, roots		
	43	2	10yr 5/6	yellowish brown	sand clay	gravel, cobbles	subsoil	
58	26	1	10yr 3/3	dark brown	silt loam	gravel, cobbles, roots		
	51	2	10yr 5/4	yellowish brown	sand clay	gravel, cobbles	subsoil	
59	10	1	10yr 3/3	dark brown	loam	roots	impasse (roots)	
60	13	1	10yr 3/3	dark brown	silt sand	gravel, cobbles, roots		
	32	2	7.5yr 3/4	dark brown	loam	gravel, cobbles, roots	subsoil	
61	19	1	7.5yr 4/3	brown	silt loam	gravel, cobbles, roots		
	43	2	7.5yr 5/8	strong brown	silt loam	gravel, roots	subsoil	
62	20	1	10yr 3/2	very dark grayish brown	sand loam	gravel, cobbles, roots		
	39	2	10yr 4/4	dark yellowish brown	sand loam	gravel, cobbles	subsoil	

Shove	el Test Re	cords						
<u>Test</u>	Ending Depth (cm	Ending Depth (cm) Level Munsell Color		ell Color	<u>Soil Type</u>	Soil Inclusions	<u>Termination</u> <u>Reason</u>	Not Collected
63	21	1	10yr 3/2	very dark grayish brown	silt sand	gravel, cobbles, roots		
	42	2	10yr 4/4	dark yellowish brown	silt loam	gravel, cobbles	subsoil	
64	14	1	7.5yr 4/2	brown	silt loam	gravel, roots		
	29	2	7.5yr 5/6	strong brown	silt loam	gravel, roots	subsoil	
65	22	1	10yr 3/2	very dark grayish brown	silt loam	gravel, cobbles, roots		
	53	2	10yr 4/4	dark yellowish brown	silt loam	gravel, cobbles, roots	subsoil	
66	15	1	10yr 3/2	very dark grayish brown	silt loam	cobbles, roots		
	43	2	10yr 4/4	dark yellowish brown	silt loam	gravel, cobbles, roots	subsoil	
67	67 20 1 10yr 3/2 very dark gra brown		very dark grayish brown	sand loam	gravel, cobbles, exfoliating bedrock, roots			
	35	35 2 10yr 4/4 dark yellowish brow		dark yellowish brown	sand loam	gravel, cobbles, exfoliating bedrock	subsoil	
68	3 11 1 10yr 3/2 very dark grayis brown		very dark grayish brown	silt loam	gravel, cobbles, roots			
	39	2	7.5yr 5/4	brown	silt loam	gravel, cobbles, roots	subsoil	
69	24	1	10yr 4/2	dark grayish brown	silt loam	gravel, roots		
	51	2	10yr 5/4	yellowish brown	loam	gravel, roots	subsoil	
70	16	1	10yr 3/3	dark brown	silt loam	gravel, cobbles, roots		
	32	2	7.5yr 4/4	brown	loam	gravel, cobbles	impasse (rocks)	
71	18	1	10yr 4/2	dark grayish brown	silt loam	gravel, roots		
	44	2	10yr 5/4	yellowish brown	silt clay	gravel, roots	subsoil	
72	20	1	10yr 3/2	very dark grayish brown	sand loam	cobbles, roots		
	38	2	10yr 4/4	dark yellowish brown	sand loam	cobbles, exfoliating bedrock	subsoil	
73	20	1	10yr 3/3	dark brown	silt loam	gravel, cobbles, roots		
	40	2	7.5yr 4/4	brown	loam clay	gravel, cobbles	subsoil	

Shovel	Test Rec	ords						
Tost	Ending		Mune		Soil Type	Soil Inclusions	Termination Reason	Not Collected
1051	<u>Deptil (cili)</u>	<u>Levei</u>	Munse		<u>Soli Type</u>	Soli Inclusions	Reason	Not Conected
74	18	1	10yr 3/2	very dark grayish brown	sand loam	gravel, cobbles, roots		
	33	2	10yr 4/4	dark yellowish brown	sand loam	gravel, cobbles, exfoliating bedrock, glacial till	subsoil	
75	21	1	10yr 2/2	very dark brown	silt loam	gravel, roots	impasse (rocks)	
76	18	1	10yr 3/3	dark brown	silt loam	gravel, cobbles, roots		
	40	2	7.5yr 4/4	brown	loam	gravel, cobbles	subsoil	
77	28	1	10yr 3/3	dark brown	silt loam	gravel, cobbles, roots		
	53	2	10yr 6/8	brownish yellow	silt loam	gravel, roots	subsoil	
78	27	1	10yr 3/2	very dark grayish brown	silt loam	gravel, cobbles, roots	impasse (roots)	

Somers Connecticut Solar, Town of Somers, Tolland County, Connecticut Phase I Archeological Investigation

Appendix 2: Site Plan





SYSTEM INFORMATION							
(11,388) VSUN, VSUN550-144BMH-DG (550W)							
(18) CPS SCH275KTL-DO/US-800-36							
(6,263.40kW DC / 4,950.00kW AC)							
SINGLE AXIS TRACKER							
±60°							
180°							
ALSOENERGY							

	921 THRALL AVENUE SUFFIELD, CT 06078
	PUNGO ENGINEERING SERVICES, PLLC. 424 EAST MAIN STREET WASHINGTON, NORTH CAROLINA 27889 INFO@PUNGOENGINEERING.COM (252) 945-8652
_	
D	
_	
	PROJECT OWNER:
С	
	AR kR RS , CT 06071
В	SOMERS SOL SANTA SOLA 159 SOUTH RD., SOMEF DRAFT ONLY NOT FOR CONSTRUCTION
	DRAWING ISSUE REVISION DATE A 03/31/2023 INTERCONNECTION SET LH Interconnection set Interconnection set
A	Image: Constraint of the second state of the seco
	SITE PLAN

A-101

250'-0" 500'-0"

ORIGINAL DRAWING SIZE: ARCH D (24"x36")

SCALE 1" = 250'-0"

0

1000'-0"





	7								
SYS	SYSTEM INFORMATION								
MODULE / QTY:	(8,710) VSUN, VSUN550-144BMH-DG (550W)								
INVERTERS:	(14) CPS SCH275KTL-DO/US-800-36								
SYSTEM SIZE (DC / AC):	(4,790.50kW DC / 3,850.00kW AC)								
MOUNTING SYSTEM:	SINGLE AXIS TRACKER								
TILT ANGLE:	±60°								
ARRAY AZIMUTH:	180°								
MONITORING SYSTEM:	ALSOENERGY								



A-101C

N

100'-0" 200'-0"

ORIGINAL DRAWING SIZE: ARCH D (24"x36")

SCALE 1" = 100'-0"

Appendix 3: Connecticut State Historic Preservation Office (SHPO) Letter

Connecticut

March 21, 2023

Mr. Timothy Coon J.R. Russo & Associates, LLC P.O. Box 938 East Windsor, CT 06088 (sent via email only to tcoon@jrrusso.com)

> Subject: Somers Solar 159 South Road Somers, Connecticut

Dear Mr. Coon:

The State Historic Preservation Office (SHPO) has reviewed your request for information concerning the potential effects to historic properties associated with the referenced project. SHPO understands that the proposed project entails the construction of a 4.95 Megawatt AC ground mounted solar photovoltaic electric facility with associated site improvements. The proposed activities are under the jurisdiction of the Connecticut Siting Council and will require a Stormwater Discharge permit issued by the Department of Energy and Environmental Protection through the authority of the Environmental Protection Agency. As a result, the project is subject to review by this office pursuant to the Connecticut Environmental Policy Act and Section 106 of the National Historic Preservation Act.

There are no properties listed on the National Register of Historic within or immediately adjacent to the Area of Potential Effect (APE) associated with the project. In addition, two previously identified archaeological sites (Site 129-1 and 129-3) have been recorded within a mile of the proposed project. SHPO understands that much of the project area has been previously disturbed by a gravel operation in operation between 1990 and 2010. However, readily available historic aerial imagery indicates that approximately four acres at the southern end of the APE remain intact and retain the potential to contain intact archaeological deposits. Further, the potentially intact portion of the APE retains environmental characteristics frequently associated with archaeological sites. Therefore, SHPO requests that a professional archaeological assessment and reconnaissance survey be completed prior to construction. Areas that will not be developed do not need to be tested. Subsurface testing should evaluate all areas of anticipated ground disturbance that retain a moderate/high archaeological sensitivity unless sufficient research or fieldwork documents that this level of effort is unwarranted. All work should be in compliance with our Environmental Review Primer for Connecticut's Archaeological Resources and no construction or other project-related ground disturbance should be initiated until SHPO has had an opportunity to review and comment upon the requested survey. A list of qualified consultants is attached for your convenience.

SHPO appreciates the opportunity to comment upon this project and we look forward to continuing consultation. Do not hesitate to contact Cory Atkinson, Staff Archaeologist and Environmental Reviewer, for additional information at (860) 500-2458 or cory.atkinson@ct.gov.

Sincerely,

lonathan heaves

Jonathan Kinney State Historic Preservation Officer

EXHIBIT XIV

SHPO FINAL DETERMINATION

Connecticut

July 25, 2023

Mr. Timothy Coon J.R. Russo & Associates, LLC P.O. Box 938 East Windsor, CT 06088 (sent via email only to tcoon@jrrusso.com)

Subject:

Somers Solar Phase I Archaeological Reconnaissance Survey 159 South Road Somers, Connecticut

Dear Mr. Coon:

The State Historic Preservation Office (SHPO) has reviewed the technical report titled *Phase I Archaeological Investigation: Somers CT Solar* prepared by Hartgen Archaeological Associates, Inc. (Hartgen), dated June 2023. The fieldwork was completed at the request of this office in a letter dated March 21, 2023, in support of Stormwater Discharge permitting issued by the Connecticut Department of Energy and Environmental Protection through the authority of the Environmental Protection Agency. The completed investigation submitted to our office for review does not meet the standards set forth in the *Environmental Review Primer for Connecticut's Archaeological Resources*. However, SHPO is willing to accept the report in its present form and will work with Hartgen to ensure that future fieldwork meets the standards promulgated by this office.

The investigation identified two previously recorded archaeological sites (Sites 129-1 and 129-3) and a single property listed on the State Register of Historic Places within proximity to the project area. The report concluded that the project will not impact previously identified cultural resources. In addition, the report noted that an approximately 5.5-acre area at the southern end of project parcel retained environmental characteristics associated with intact archaeological deposits and recommended archaeological reconnaissance survey prior to construction. The remaining northern portion of the project area was characterized by previous disturbance related to a former gravel mining operation. A subsequent archaeological reconnaissance survey included the excavation of 78 round shovel tests measuring 40 cm in diameter. The shovel tests were excavated at 15 m intervals along transects placed 15 m apart throughout the single moderate archaeological sensitivity area. The field effort failed to identify evidence of cultural material and no further investigation was recommended. Based on the information submitted to this office, it is the opinion of SHPO that <u>no historic properties will be affected</u> by the proposed development and no additional archaeological investigation is warranted.

Connecticut

This office appreciates the opportunity to review and comment upon this project. Do not hesitate to contact Cory Atkinson, Staff Archaeologist and Environmental Reviewer, for additional information at (860) 500-2458 or cory.atkinson@ct.gov.

Sincerely,

lonathan perse

Jonathan Kinney State Historic Preservation Officer

cc (via email): Venables, Hartgen Kirk, Hartgen Mija, Louth Callan Renewables Gardiola, Louth Callan Renewables EXHIBIT XV

FAA DETERMINATION



Notice Criteria Tool

Notice Criteria Tool - Desk Reference Guide V_2018.2.0

The requirements for filing with the Federal Aviation Administration for proposed structures vary based on a number of factors: height, proximity to an airport, location, and frequencies emitted from the structure, etc. For more details, please reference CFR Title 14 Part 77.9.

You must file with the FAA at least 45 days prior to construction if:

- your structure will exceed 200ft above ground level
- your structure will be in proximity to an airport and will exceed the slope ratio
- your structure involves construction of a traverseway (i.e. highway, railroad, waterway etc...) and once adjusted upward with the appropriate vertical distance would exceed a standard of 77.9(a) or (b) your structure will emit frequencies, and does not meet the conditions of the FAA Co-location Policy
- your structure will be in an instrument approach area and might exceed part 77 Subpart C
- your proposed structure will be in proximity to a navigation facility and may impact the assurance of navigation signal reception
- your structure will be on an airport or heliport
- filing has been requested by the FAA

If you require additional information regarding the filing requirements for your structure, please identify and contact the appropriate FAA representative using the Air Traffic Areas of Responsibility map for Off Airport construction, or contact the FAA Airports Region / District Office for On Airport construction.

The tool below will assist in applying Part 77 Notice Criteria.

* Structure Type:	SOLAR Solar Panel Please select structure type and complete location point information.
Latitude:	41 Deg 58 M 34 S N 🗸
Longitude:	72 Deg 26 M 30 S W 🗸
Horizontal Datum:	NAD83 🗸
Site Elevation (SE):	338 (nearest foot)
Structure Height :	12 (nearest foot)
Is structure on airport:	No Yes

Results

You do not exceed Notice Criteria.

EXHIBIT XVI

FARMLAND CLASSIFICATION MAP





Farmland Classification—State of Connecticut (Somers Solar)

- Prime farmland if 1 A subsoiled, completely removing the root inhibiting soil layer
- Prime farmland if irrigated and the product of I (soil erodibility) x C (climate factor) does not exceed 60
- Prime farmland if irrigated and reclaimed of excess salts and sodium
- Farmland of statewide importance
- Farmland of statewide importance, if drained
- Farmland of statewide importance, if protected from flooding or not frequently flooded during the growing season
- Farmland of statewide importance, if irrigated

- Farmland of statewide importance, if drained and either protected from flooding or not frequently flooded during the
- arowing season Farmland of statewide importance, if irrigated and drained

100

- Farmland of statewide 100 importance, if irrigated and either protected from flooding or not frequently flooded during the growing season Farmland of statewide a 🖬 importance, if subsoiled.
- completely removing the root inhibiting soil layer Farmland of statewide 100 importance, if irrigated

and the product of I (soil erodibility) x C (climate factor) does not exceed 60

- Farmland of statewide الجريدا الم importance, if irrigated and reclaimed of excess salts and sodium
- Farmland of statewide importance, if drained or either protected from flooding or not frequently flooded during the growing season
- Farmland of statewide importance, if warm enough, and either drained or either protected from flooding or not frequently flooded during the growing season
- Farmland of statewide importance, if warm enough
- Farmland of statewide 1990 B importance, if thawed
- Farmland of local importance
- Farmland of local importance, if irrigated

- Farmland of unique importance Not rated or not available an ai
- Soil Rating Points Not prime farmland

- All areas are prime farmland
- Prime farmland if drained
- Prime farmland if protected from flooding or not frequently flooded during the growing season
- Prime farmland if irrigated
- Prime farmland if drained and either protected from flooding or not frequently flooded during the growing season
- Prime farmland if irrigated and drained
- Prime farmland if irrigated and either protected from flooding or not frequently flooded during the growing season

- Prime farmland if subsoiled, completely removing the root inhibiting soil layer
- Prime farmland if irrigated and the product of I (soil erodibility) x C (climate factor) does not exceed 60
- Prime farmland if irrigated and reclaimed of excess salts and sodium
- Farmland of statewide importance
- Farmland of statewide importance, if drained
- Farmland of statewide importance, if protected from flooding or not frequently flooded during the growing season
- Farmland of statewide importance, if irrigated



Farmland Classification—State of Connecticut (Somers Solar)

	Farmland of statewide importance, if drained and either protected from		Farmland of statewide importance, if irrigated and reclaimed of excess		Farmland of unique importance	The soil surveys that comprise your AOI were mapped at 1:12,000.
	flooding or not frequently flooded during the		salts and sodium Farmland of statewide	U Water Feat	tures	Warning: Soil Map may not be valid at this scale.
	growing season Farmland of statewide importance, if irrigated	_	importance, if drained or either protected from flooding or not frequently	~~ Transporta	Streams and Canals	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of
	Farmland of statewide importance, if irrigated and either protected from flooding or not frequently		growing season Farmland of statewide importance, if warm	~	Rails Interstate Highways US Routes	Scale.
_	flooded during the growing season		drained or either protected from flooding or not frequently flooded	~	Major Roads	measurements. Source of Map: Natural Resources Conservation Service
	importance, if subsoiled, completely removing the	_	during the growing season Farmland of statewide	Backgrour	Local Roads	Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
	root inhibiting soil layer Farmland of statewide importance, if irrigated and the product of I (soil	Iland of statewide in rtance, if irrigated e he product of I (soil F	importance, if warm enough Farmland of statewide			projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more
	erodibility) x C (climate factor) does not exceed 60		Importance, if thawed Farmland of local Importance			accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data
			Farmland of local importance, if irrigated			Soil Survey Area: State of Connecticut Survey Area Data: Version 22, Sep 12, 2022
						Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.
						Date(s) aerial images were photographed: Jun 14, 2022—Oct 6, 2022
						The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



Farmland Classification

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
37C	Manchester gravelly sandy loam, 3 to 15 percent slopes	Farmland of statewide importance	21.8	90.6%
64B	Cheshire fine sandy loam, 3 to 8 percent slopes, very stony	Not prime farmland	2.3	9.4%
Totals for Area of Interest			24.1	100.0%

Description

Farmland classification identifies map units as prime farmland, farmland of statewide importance, farmland of local importance, or unique farmland. It identifies the location and extent of the soils that are best suited to food, feed, fiber, forage, and oilseed crops. NRCS policy and procedures on prime and unique farmlands are published in the "Federal Register," Vol. 43, No. 21, January 31, 1978.

Rating Options

Aggregation Method: No Aggregation Necessary

Tie-break Rule: Lower