

STATE OF CONNECTICUT *CONNECTICUT SITING COUNCIL* Ten Franklin Square, New Britain, CT 06051 Phone: (860) 827-2935 Fax: (860) 827-2950 E-Mail: siting.council@ct.gov Web Site: portal.ct.gov/csc

VIA ELECTRONIC MAIL

April 21, 2023

Paul R. Michaud, Esq. Michaud Law Group LLC 515 Centerpoint Dr., Suite 502 Middletown, CT 06457 pmichaud@michaud.law

RE: **PETITION NO. 1487** – TRITEC Americas, LLC declaratory ruling, pursuant to Connecticut General Statutes §4-176 and §16-50k, for the proposed construction, maintenance and operation of a 1.97 megawatt AC solar photovoltaic electric generating facility located at 254 Putnam Road, Pomfret, Connecticut, and associated electrical interconnection.

Dear Attorney Michaud:

The Connecticut Siting Council (Council) is in receipt of your correspondence dated April 20, 2023, regarding compliance with Condition No. 4 of the Council's Declaratory Ruling issued on June 10, 2022 for the above-referenced facility. The correspondence includes the final structural design of the racking system, stamped by a Professional Engineer duly licensed in the State of Connecticut, in accordance with Condition No. 4.

Therefore, the Council acknowledges that Condition No. 4 has been satisfied. This acknowledgment applies only to the condition satisfied by the April 20, 2023 correspondence.

Please be advised that deviations from the standards established by the Council in the Declaratory Ruling are enforceable under the provisions of Connecticut General Statutes §16-50u.

Thank you for your attention and cooperation.

Sincerely,

Mulinkhart

Melanie A. Bachman Executive Director

MB/RDM/laf



PAUL R. MICHAUD Managing Attorney / Principal 515 Centerpoint Drive, Suite 503 Middletown, CT 06457 Direct Telephone: (860) 338-3728 Email: pmichaud@michaud.law Web: www.michaudlaw.com

April 20, 2023

FILED BY ELECTRONIC MAIL AND US MAIL

Melanie Bachman, Esq. Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

Re: **PETITION NO. 1487** – TRITEC Americas, LLC petition for a declaratory ruling, pursuant to Connecticut General Statutes §4-176 and §16-50k, for the proposed construction, maintenance and operation of a 1.97-megawatt AC solar photovoltaic electric generating facility located at 254 Putnam Road, Pomfret, Connecticut, and associated electrical interconnection.

Dear Attorney Bachman:

On behalf of TRITEC Americas, LLC ("Petitioner"), this letter to the Connecticut Siting Council ("Council") is in response to Council's Petition No. 1487 Decision dated June 10, 2022 ("Decision"), specifically Condition #4.

Enclosed – and in response to Condition #4 of the Decision – please find a copy of the final structural design for the racking system stamped by a Connecticut Professional Engineer. Petitioner believes they have satisfied the Conditions required to construct the solar panel racking system; however, Petitioner will refrain from doing so until they receive Council's acknowledgement.

Consistent with Council requirements, Petitioner submits one electronic version, an original, and fifteen hard copies of all necessary documents.

Please feel free to contact me if you have any questions.

Very truly yours,

Paul R. michan

Paul R. Michaud



Prepared For:

Solv Inc.

Amaral



SFDC ID# 17009 1x35 FTS - Structural Calculations 254 Putnam Road – Pomfret Center, CT 06259



A product of Northern States Metals (NSM)

3207 Innovation Place, Youngstown, Ohio, 44509-4023

Checked By: JS

Prepared By: NZ

* 20323 * 20324 * 20325 * 20325 * 20325 * 20325 * 20325 * 20325 * 20325 * 20355 * 20355 * 20355 * 20355 * 20355 * 20355 * 20355 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 2035555 * 203555 * 203555 * 203555 * 203555 * 203555 * 203555 * 2035555 * 2035555 * 203555 * 203555 * 203555 * 203555 * 2035555 * 2035555 * 203555 * 203555 * 203555 * 203555 * 2035555 * 2035555 * 2035555 * 2035555 * 2035555 * 2035555 * 2035555 * 2035555 * 2035555 * 2035555 * 2035555 * 2035555 * 203555 * 2035555 * 203555 * 203555 * 203555 * 203555 * 2035555 * 2035555 * 2035555 * 203555 * 203555 * 203



Solar FlexRack Engineering Analysis

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Solar FlexRack Engineering Analysis

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Hot Rolled Code Check	33
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Customer: S	Solv Inc.
SFDC ID #: 1	17009
Project/Location: A	Amaral - Pomfret Center, CT 06259
Date/Engineer: 0	01/20/23 - JS

Solar Flexrack Loading Analysis

Configuration Data

Configuration:	1x35 TDP	
Horiz. Length (N-S):	128.95 ft	
Array Surface Area:	1008.56 ft ²	
Number of Posts:	9	

Design Data Summary

Module Length:	7.82	ft
Solar Panel Dead Load:	2.56	psf
Max Stow Wind Speed:	118	mph
Max Operation:	35	mph
Snow Load:	40	psf
Ground Clearance:	42.78	in
Exposure Category:	c	
Building Classification:	1	

Snow Load Parameters

Flat Roof Snow Load, P _f :	Pf=0.7*Ce*Ct*I*Pg
Sloped Roof Snow Load, P _s :	Ps = Pf*Cs
Snow Exposure Category, Ce:	0.9
Snow Thermal Factor, Ct:	1.2
Snow Importance Factor, I:	0.8
P _f :	24.19 psf
Snow Density:	19.2 pcf
Snow Height:	25.00 in

Tilt Angle 0 - 15 C_s 0.91

P_s psf

Snow Load Design

Wind Load Parameters

Exposure Coefficient, Kz:	0.85	Wind Load: qh = 0.00256*Kz*Kzt*Kd*V ²	n:	2.9 Hz
Topographic Factor, Kzt:	1.00	qh _{vuit} 25.72 psf	Dampening Ratio:	2.50 %
Wind Directionality Factor, Kd:	0.85	qh ₃₅ 3.77 psf	nL/V _{ult} :	0.169
			nL/V ₃₅ :	0.442

Wind Load Design

	Perimeter Loading																
	Stow Position																
				Sta	tic								Inertial				
Tilt Angle	A Distribution	B Distribution	GCp Up	GCp Dn	A*qz*GCp Up	B*qz*GCp Up	A*qz*GCp Dn	B*qz*GCp Dn	A Distribution	B Distribution	Mod. Factor	GCp Up	GCp Dn	A*qz*GCp Up	B*qz*GCp Up	A*qz*GCp Dn	B*qz*GCp Dn
0	2.00	0.00	-0.32	0.22	-16.59	0.00	11.32	0.00	1.17	0.83	0.57	-0.89	0.79	-26.82	-19.10	23.74	16.91
								Tilted Position	(35 mph max)								
				Sta	tic								Inertial				
Tilt Angle	A Distribution	B Distribution	GCp Up	GCp Dn	A*qz*GCp Up	B*qz*GCp Up	A*qz*GCp Dn	B*qz*GCp Dn	A Distribution	B Distribution	Mod. Factor	GCp Up	GCp Dn	A*qz*GCp Up	B*qz*GCp Up	A*qz*GCp Dn	B*qz*GCp Dn
5	1.95	0.05	-0.48	0.44	-3.54	-0.10	3.22	0.09	1.85	0.15	0.30	-0.79	0.74	-5.48	-0.44	5.18	0.42
10	1.89	0.11	-0.65	0.55	-4.61	-0.27	3.93	0.23	1.85	0.15	0.30	-0.95	0.86	-6.63	-0.53	5.96	0.48
15	1.84	0.17	-0.60	0.72	-4.13	-0.37	4.99	0.45	2.00	0.00	0.06	-0.66	0.78	-4.96	0.00	5.90	0.00
20	1.78	0.22	-0.66	0.73	-4.41	-0.54	4.88	0.60	2.00	0.00	0.06	-0.72	0.79	-5.41	0.00	5.95	0.00
25	1.73	0.28	-0.66	0.67	-4.27	-0.68	4.37	0.70	2.00	0.00	0.06	-0.72	0.73	-5.41	0.00	5.52	0.00
30	1.67	0.33	-0.66	0.62	-4.15	-0.82	3.89	0.77	2.00	0.00	0.06	-0.72	0.68	-5.43	0.00	5.12	0.00
35	1.62	0.39	-0.66	0.73	-4.03	-0.96	4.46	1.06	2.00	0.00	0.06	-0.72	0.79	-5.45	0.00	5.99	0.00
40	1.56	0.44	-0.72	0.68	-4.23	-1.19	3.99	1.13	2.00	0.00	0.06	-0.78	0.74	-5.88	0.00	5.58	0.00
45	1.51	0.50	-0.72	0.71	-4.10	-1.35	4.01	1.32	2.00	0.00	0.06	-0.78	0.77	-5.91	0.00	5.79	0.00

Seismic Load Parameters / Design

S _S :	0.172	S _{DS:} S _{DS} = (2/3) x F _a x S _S	Site Class: D	
S ₁ :	0.063	S _{DS:} 0.183	Seismic Design Category: B	This Base Shear Value represents the seismic effect of the panel weight on the rack. This Base Shear
Site Coefficient, Fa:	1.60	S _{D1:} S _{D1} = (2/3) x F _V x S _S		includes 20% of the design snow load when the flat roof snow load exceeds 30 psf per AISC. A
Site Coefficient, F _{V:}	2.400	S _{D1:} 0.101	Seismic Response Coefficient, CS: 0.092	separate term in the Risa load combination accounts for the remaining Dead Load caused by member
Response Modification Coefficent, R:	2	Cu = 1.70	Panel Seismic Load, V = C _S x (Panel DL)	self-weight.
Importance Factor, Ie:	1	TL = 6	V = 0.235 psf	

Note: GCp values and A-B distributions determined from RWDI Wind Tunnel Testing. GCp values for 20* and 45* were not given in results from RWDI and are assumed to be the higher value of the two adjacent values.

Loading analysis and design in accordance with wind and snow load information obtained from ASCE 7-10 Minimum Design Loads for Building and Other Structures

Disclaimer: The use of the topographic factor, Kzt, requires project engineer to evaluate based on specific site topographic conditions.







Model Settings	
Solution	
Members	
Number of Ponerted Sections	20
Number of Internal Sections	20
Member Area Load Meab Size (in ²)	1
Consider Shear Deformation	l Vee
	Yes
	Tes
Wall Papele	
Approximate Mesh Size (in)	12
Transfor Earcos Botwoon Intersecting Wood Walls	12 Voc
Increase Wood Wall Nailing Canacity for Wind Loads	Vec
Include P. Dette for Walls	Vec
Ontimize Meeonry and Weed Wells	Vec
Maximum Number of Iterationa	
	3
Processor Core Utilization	
Single	No
Multiple (Optimum)	Yes
Maximum	No
Maximum	
Auto	
Axis Martinal Clahal Avia	
Venical Global Axis	V
Global Axis corresponding to vertical direction	Yee
Convert Existing Data	res
Default Member Orientation	
Default Global Plane for z-axis	XZ
Plate Axis	
Plate Local Axis Orientation	Nodal
Codes	
Hot Rolled Steel	AISC 14th (360-10): ASD
Stiffness Adjustment	Yes (Iterative)
Notional Annex	None
Connections	AISC 14th (360-10): ASD
Cold Formed Steel	AISI S100-12: ASD
Stiffness Adjustment	Yes (Iterative)
Wood	None
Temperature	< 100F
Concrete	None
Masonry	None
Aluminum	None
Structure Type	Building
Stiffness Adjustment	Yes (Iterative)
Stainless	None
Stiffness Adjustment	Yes (Iterative)

Concrete	
Compression Stress Block	Rectangular Stress Block
Analyze using Cracked Sections	Yes
Leave room for horizontal rebar splices (2*d bar spacing)	No



Model Settings (Continued)

List forces which were ignored for design in the Detail Report	Yes
Electronoco which were ignored for debigh in the Botal Roport	100

Rebar	
Column Min Steel	1
Column Max Steel	8
Rebar Material Spec	ASTM A615
Warn if beam-column framing arrangement is not understood	No

Shear Reinforcement	
Number of Shear Regions	4
Region 2 & 3 Spacing Increase Increment (in)	4

Seismic

RISA-3D Seismic Load Options				
Code	ASCE 7-10			
Risk Category	l or ll			
Drift Cat	Other			
Base Elevation (ft)				
Include the weight of the structure in base shear calcs	Yes			

Site Parameters	
S ₁ (g)	0.063
SD ₁ (g)	0.101
SD₅ (g)	0.183
T _L (sec)	6

Structure Characteristics

T Z (sec)	
T X (sec)	
C _t X	0.02
C _t Exp. Z	0.75
C _t Exp. X	0.75
RZ	2
RX	2
$\Omega_0 Z$	2
$\Omega_0 X$	2
C _d Z	2
C₄X	2
ρΖ	1
ρΧ	1



Load Combinations

	Description	Solve	P-Delta	BLC	Factor	BLC	Factor	BLC	Factor	BLC	Factor
1		Yes	Y	DL	1						
2	IBC 16-10	Yes	Y	DL	1	SL	1				
3	IBC 16-12 (A)	Yes	Y	DL	1	WL+X	0.6	OL3	0.6		
4	IBC 16-12 (B)	Yes	Y	DL	1	WL-X	0.6	OL3	0.6		
5	IBC 16-13 (A) (static wind)	Yes	Y	DL	1	OL1	0.45	SL	0.75	OL3	0.45
6	IBC 16-13 (B) (static wind)	Yes	Y	DL	1	OL2	0.45	SL	0.75	OL3	0.45
7	Total WL + 0.25 SL	Yes	Y	DL	1	WL+X	0.45	SL	0.25		
8	Total WL + 0.25 SL	Yes	Y	DL	1	WL-X	0.45	SL	0.25		
9	IBC 16-15 (A)	Yes	Y	DL	0.6	WL+X	0.6	OL3	0.6		
10	IBC 16-15 (B)	Yes	Y	DL	0.6	WL-X	0.6	OL3	0.6		
11	Seismic										
12	IBC 16-12 C (A)	Yes	Y	DL	1	ELX	0.7			Sds*DL	0.14
13	IBC 16-12 C (B)	Yes	Y	DL	1	ELX	-0.7			Sps*DL	0.14
14	IBC 16-12 (D) (A)	Yes	Y	DL	1	ELZ	0.7			Sds*DL	0.14
15	IBC 16-12 (D) (B)	Yes	Y	DL	1	ELZ	-0.7			Sds*DL	0.14
16	IBC 16-14 (A) (A)	Yes	Y	DL	1	ELX	0.525	SL	0.75	Sds*DL	0.105
17	IBC 16-14 (A) (B)	Yes	Y	DL	1	ELX	-0.525	SL	0.75	Sds*DL	0.105
18	IBC 16-14 (B) (A)	Yes	Y	DL	1	ELZ	0.525	SL	0.75	Sds*DL	0.105
19	IBC 16-14 (B) (B)	Yes	Y	DL	1	ELZ	-0.525	SL	0.75	Sds*DL	0.105
20	IBC 16-16 (A) (A)	Yes	Y	DL	0.6	ELX	0.7			Sds*DL	-0.14
21	IBC 16-16 (A) (B)	Yes	Y	DL	0.6	ELX	-0.7			Sps*DL	-0.14
22	IBC 16-16 (B) (A)	Yes	Y	DL	0.6	ELZ	0.7			Sds*DL	-0.14
23	IBC 16-16 (B) (B)	Yes	Y	DL	0.6	ELZ	-0.7			Sds*DL	-0.14

Solv Inc.	Amaral	SK-1
JRD		4 47 0000
		1 Apr 1 / 2023
		1 Apr 1 / 2023
JRD	Amarai	SK-1

-2.56 psf	.LL.	
Loads: BLC 2, Solar Panels		
Solv Inc. JRD	Amaral	SK-2 Apr 17, 2023
17009		23-0417 - 1x35 TDP 1.0 0°- Trin

-24.19 psf		
	Amaral	SK-3
JRD	Amarai	Apr 17. 2023
17009		23-0417 - 1x35 TDP 1.0 0°- Trin

0.235 psi		
Loads: BLC 4, Seismic X Solv Inc.	Amaral	SK-4
JRD		Apr 17, 2023
17009		23-0417 - 1x35 TDP 1.0 0°- Trin

Y KAN AND AND AND AND AND AND AND AND AND A	A second se	
	A	
Solv Inc.	Amaral	SK-5
17009		23-0417 - 1x35 TDP 1 0 0°- Trin
17009		23-0417 - 1X35 IDP 1.0 0°- 1rin

Y z → x		
-16.59 psf		
Loads: BLC 6, Wind Uplift - Static		
Solv Inc.	Amaral	SK-6
JRD	1	Apr 17, 2023
17009]	23-0417 - 1x35 TDP 1.0 0°- Trin

11.32 psf	1 1	
Loads: BLC 7, Wind Downforce -	Static	
Solv Inc.	Amaral	SK-7
17009	4	23-0417 - 1x35 TDP 1.0 0°- Trin

-26.82 psf		
Loads: BLC 8, Wind Uplift - Dynan	nic	SK-8
JRD	Amarai	Apr 17, 2023
17000	4	74117,2023
11/008		23-0417 - 1X30 IDP 1.00 - IMM

23.74 psf	16.91 psf	
Loads: BLC 9, Wind Downforce -	Dynamic	1
Solv Inc.	Amaral	SK-9
17009		23-0417 - 1x35 TDP 1.0 0°- Trin

Company : Solv Inc. Designer : JRD Job Number : 17009 Model Name : Amaral	
A NEWERSCHEK COMPANY	

4/17/2023 4:03:21 PM Checked By : NZ

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Envelope AISC 14th (360-10): ASD Steel Code Checks

Eqn	H3-6	H3-6	H1-1b	H1-1b*	H1-1b*	H1-1b*	H1-1b*	H1-1b*	H1-1b*	H1-1b*	H1-1b*
ср С	1	٢	1	1	-	1	-	٢	٢	-	-
Mnzz/om [k-in]	92.515	92.515	421.115	94.678	94.678	94.678	94.678	94.678	94.678	94.678	94.678
Mnyy/om [k-in]	92.515	92.515	201.198	35.079	35.079	35.079	35.079	35.079	35.079	35.079	35.079
Pnt/om [k]	70.15	70.15	175.749	59.947	59.947	59.947	59.947	59.947	59.947	59.947	59.947
Pnc/om [k]	14.136	14.136	135.323	26.494	26.494	26.494	26.494	26.494	26.494	26.494	26.494
ပ	5	5	5	12	12	12	12	5	5	13	13
<u>D</u>	٨	٨	٨	٨	٨	٨	٨	٨	٨	٨	λ
Loc[in]	0	0	90	0	0	0	0	96.885	96.885	0	0
Shear Check	0.709	0.708	0.014	0.002	0.002	0.002	0.002	0.003	0.003	0.002	0.002
с С	5	5	5	2	2	2	2	2	2	2	2
Loc[in]	0	0	33.632	0	0	0	0	0	0	0	0
Code Check	0.864	0.864	0.253	0.132	0.132	0.124	0.124	0.123	0.123	0.095	0.095
Shape	4X4X0 126	4X4X0.126	W6X20	W6X7	W6X7	W6X7	W6X7	W6X7	W6X7	W6X7	W6X7
Member	Tube 1	Tube 2	DRIVE POST	IDLER POST 2	IDLER POST 7	IDLER POST 3	IDLER POST 6	IDLER POST 5	IDLER POST 4	DIDLER POST 8	1 IDLER POST 1
	~	2	З	4	2	9	7	ω	တ	12	÷

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Detail Report: Tube 1		Unity Check: 0.864 (axia	al/bending)	Load Combination: LC 5: IBC 16-13 (A) (static wind		
		put Data: Shape: Member Type: Length (in): Material Type: Design Rule: Number of Internal Sections:	4X4X0.126 Beam 764.43 Hot Rolled Steel Typical 191	l Node: J Node: I Release: J Release: I Offset (in): J Offset (in):	GA VX1C Fixed Fixed N/A N/A	
Material Properties:						
Material:	A500 Gr. 60	Therm. Coeff. (1e⁵°F⁻¹):	0.65	R.,:	1.5	
E (ksi):	29000	Density (k/ft ³):	0.49	F ₁₁ (ksi):	70	
G (ksi):	11154	F _v (ksi):	60	R _t :	1.2	
Nu:	0.3	, ,				
Shape Properties:						
d (in):	4	I _{vv} (in⁴):	4.889	Area (in²):	1.952	
b _f (in):	4	I_{77}^{y} (in ⁴):	4.889	J (in⁴):	7.326	
t (in):	0.126					
Design Properties:						
L _{b v-v} (in):	228	К _{v-v} :	1	Max Defl Ratio:	L/387	
L _{b z-z} (in):	228	K _{z-z} :	1	Max Defl Location:	627.637	
L _{comp top} (in) :	Lbyy	y sway:	No	Span:	4	
L _{comp bot} (in) :	228	z sway:	No			
L _{torque} (in):	N/A	Function:	Latera			
C _b :	1	Seismic DR:	None			
		Tube 1				
GA					• VX1C	



15





Limit State	Required	Available	Unity Check	रesult
Applied Loading - Bending/Axial				
Applied Loading - Shear + Torsion	-	-	-	-
Axial Tension Analysis	0.000 k	70.15 k	-	-
Axial Compression Analysis	0.000 k	14.136 k	-	-
Flexural Analysis (Strong Axis)	29.02 k-in	92.515 k-in	-	-
Flexural Analysis (Weak Axis)	4.087 k-in	92.515 k-in	-	-
Shear Analysis (Major Axis y)	13.942 k	19.676 k	0.709	Pass
Shear Analysis (Minor Axis z)	12.946 k	19.676 k	0.658	Pass
Bending & Axial Interaction Check (UC Bending Max)	-	-	0.864	Pass
Torsional Analysis	53.492 k-in	81.361 k-in	0.657	Pass



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etali Report. VP 34					
× × ×	× >z	Input Data: Shape: Member Type: Length (in): Material Type: Design Rule: Number of Internal Sections:	V-HU-2.25X0.055X1.25 Beam 55.118 Cold Formed Steel Typical 191	l Node: J Node: I Release: J Release: I Offset (in): J Offset (in):	V34 V34 Fixe Fixe N/ N/
Material Properties:					
Material:	A653 Grade 50	Nu:	0.3	F _y (ksi):	
E (ksi):	29500	Therm. Coeff. (1e ⁵ °F ^{−1}):	0.65	F _u (ksi):	ī
G (ksi):	11346	Density (k/ft³):	0.49		
hape Properties:					
D (in):	2.25	J (in ⁴):	0.000432	r _y (in):	N/
B (in):	1.25	C _w (in ⁶):	0.21	x ₀ (in):	-1.44
t (in):	0.055	r _o (in):	1.875	S _{e,z} (in³):	N,
R (in):	0.112	X _c (in):	1.274	S _{f,z} (in³):	N,
d (in):	1.25	m (in):	0.172	S _{c,z} (in³):	N,
I _{yy} (in ⁴):	0.308	j (in):	1.589	S _{e,y} (in³):	N
I _{zz} (in⁴):	0.303	r _z (in):	N/A	S _{f,y} (in³):	N,
Area (in ²):	0.428				
esign Properties:					
L _{b y-y} (in):	N/A	К _{у-у} :	1	Max Defl Ratio:	L/100
L _{b z-z} (in):	N/A	K _{z-z} :	1	Max Defl Location:	N
L _{comp top} (in):	Lbyy	R:	N/A	Span:	IN,
L _{comp bot} (in):	N/A	y sway:	No		
C _b :	1	z sway:	No		
C _{m y-y} : C _{m z-z} :	N/A N/A	a (in):	N/A		
		VP	34		
● V34B					• V34C
Diagrams:					
				-0.385 at 29.88 in	
		-0.046 at 0 in			
			-0.046 at 54.828 in	-0.592 at 0 in	
		y Deflection	(in)	z Deflection (in)
				0.368 at 29.88 in	
-8	3.063e-07 at 25.528 in			-0.368 a	t 25.238 in
Axial Forc	e (kips)	y Shear Force	(kips)	z Shear Force (ki	ps)





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Member Check - AISI S100-12, US, ASD

Material	Type: A653 SS	Grade 50/	3, Fy=50 ks	i	
Design Pa	arameters:				
Lx	4.593 ft	Ly	4.593 ft	: Lt	4.593 ft
Kx	1.0000	Ку	1.0000	Kt	1.0000
Cbx	1.0000	Cby	1.0000	ex	0.0000 in
Cmx	1.0000	Cmy	1.0000	ey	0.0000 in
Braced FI	Lange: None	kφ	0 k		
Red. Fact	tor, R: 0	Lm	20.000 ft	:	
Entered m	noments incluc	le P-δ effe	cts		
Loads:	Р	Мх	Vy	Му	Vx
	(k)	(k-in)	(k)	(k-in)	(k)
Entered	0.0000	0.0000	0.0000	-8.1730	-0.3680
Applied	0.0000	0.0000	0.0000	-8.1730	-0.3680
Strength	3.5214	5.4412	0.9446	8.3624	3.9518

Interaction Equations Eq. C5.2.1-1 (P, Mx, My) 0.000 + 0.000 + 0.977 = 0.977 <= 1.0 Eq. C5.2.1-2 (P, Mx, My) 0.000 + 0.000 + 0.977 = 0.977 <= 1.0 Eq. C3.3.1-1 (Mx, Vy) Sqrt(0.000 + 0.000)= 0.000 <= 1.0 Eq. C3.3.1-1 (My, Vx) Sqrt(0.955 + 0.009)= 0.982 <= 1.0



D2

Detail Report: DRIVE POST		Unity Check: 0.253 (axia	al/bending)	Load Combination: LC 5: IBC 16-13 (A) (static wind		
		put Data: Shape: Member Type: Length (in): Material Type: Design Rule: Number of Internal Sections:	W6X20 Column 90 Hot Rolled Steel Typical 191	l Node: J Node: I Release: J Release: I Offset (in): J Offset (in):	D1 D2 Fixed AllPIN N/A N/A	
Material Properties:						
Material:	A992	Therm. Coeff. (1e ⁵ °F ⁻¹):	0.65	R _v :	1.1	
E (ksi):	29000	Density (k/ft³):	0.49	F (ksi):	65	
G (ksi):	11154	F _v (ksi):	50	R _t :	1.1	
Nu:	0.3	,		•		
Shape Properties:						
d (in):	6.2	Area (in²):	5.87	S _w (in⁴):	4.82	
b _f (in):	6.02	Z_{yy} (in ³):	6.72	r _τ (in):	1.64	
t _f (in):	0.365	Z ₇₇ (in ³):	14.9	J (in⁴):	0.24	
t _w (in):	0.26	C _w (in ⁶):	113	k _{det} (in):	0.875	
I_{yy} (in ⁴):	13.3	$W_{no}(in^2)$:	8.78	k _{des} (in):	0.615	
I _{zz} (in ⁴):	41.4					
Design Properties:						
L _{b vav} (in):	N/A	К _{и-и} :	1	Max Defl Ratio:	L/308	
$L_{h, z-z}$ (in):	N/A	K ₇₋₇ :	1	Max Defl Location:	0	
L _{comp top} (in):	Lbyy	y sway:	No	Span:	N/A	
L _{comp bot} (in):	N/A	z sway:	No			
L _{toraue} (in):	N/A	Function:	Latera			
C _b :	1	Seismic DR:	None			
		DRIVE	POST			
					₩ -	

D1







Limit State	Required	Available	Unity Check	Result
Applied Loading - Bending/Axial				
Applied Loading - Shear + Torsion	-	-	-	-
Axial Tension Analysis	0.000 k	175.749 k	-	-
Axial Compression Analysis	2.158 k	135.323 k	-	-
Flexural Analysis (Strong Axis)	103.254 k-in	421.115 k-in	-	-
Flexural Analysis (Weak Axis)	0.000 k-in	201.198 k-in	-	-
Shear Analysis (Major Axis y)	0.452 k	32.24 k	0.014	Pass
Shear Analysis (Minor Axis z)	0.000 k	78.945 k	0.000	Pass
Bending & Axial Interaction Check (UC Bending Max)	-	-	0.253	Pass



Detail Report: IDLER POST 2		Unity Check: 0.1	Load Combi	Load Combination: LC 2: IBC 16-7		
	× × × ×	nput Data: Shape: Member Type: Length (in): Material Type: Design Rule: Number of Internal Sectio	W6X7 Column 96.885 Hot Rolled Steel Typical ons: 191	I Node: J Node: I Release: J Release: I Offset (in): J Offset (in):	N345 N346 Fixed Custom N/A N/A	
Material Properties:						
Material:	A992	Therm. Coeff. (1e ³ °F ⁻¹):	0.65	R _y :	1.1	
E (ksi):	29000	Density (k/ft³):	0.49	F _u (ksi):	6	
G (ksi):	11154	F _y (ksi):	50	R _t :	1.1	
Nu:	0.3					
Shape Properties:						
d (in):	5.772	Area (in ²):	2.002	S _w (in ⁴):	0.89	
b _f (in):	3.94	Z_{yy} (in ³):	1.303	r _τ (in):	1.04	
t₂(in):	0.165	$Z_{}(in^3)$:	4.6	J (in⁴):	0.01	
t (in):	0.129	$C_{(in^6)}$:	13 227	k (in):	0.6	
(in^4)	1 683	W (in ²):	5.522	k. (in):	0.0	
I _{zz} (in ⁴):	11.955	w _{no} (iii).	5.565	Maes (III) -		
Design Properties:						
L _{b v-v} (in):	N/A	К _{у-у} :	1	Max Defl Ratio:	L/1000	
$L_{h,\tau,\tau}$ (in):	N/A	К, .:	1	Max Defl Location:		
L (in):	Lbvv	v swav:	No	Span:	N/J	
	N/A	z sway:	No			
Comp bot (III)	N/A	Eunction:	Latera			
L _{torque} (III).	1 N/A	Colorado DP	Nene			
C _b :		Seismic DR:	none			
			IDLER POST 2			
• N345					● N346	
Diagrame			6.326e-07 at 96.375 in			
Diagrams.						
		y Deflec	ction (in)	z Deflection	(in)	
3.496 at 0 in						
3.441	1 at 96.885 in					
Axial Force (kips)		y Shear Fe	orce (kips)	z Shear Force	(kips)	





Limit State	Required	Available	Unity Check	Result
Applied Loading - Bending/Axial				
Applied Loading - Shear + Torsion	-	-	-	-
Axial Tension Analysis	0.000 k	59.947 k	-	-
Axial Compression Analysis	3.496 k	26.494 k	-	-
Flexural Analysis (Strong Axis)	0.000 k-in	94.678 k-in	-	-
Flexural Analysis (Weak Axis)	0.000 k-in	35.079 k-in	-	-
Shear Analysis (Major Axis y)	0.000 k	14.892 k	0.000	Pass
Shear Analysis (Minor Axis z)	0.000 k	23.357 k	0.000	Pass
Bending & Axial Interaction Check (UC Bending Max)	-	-	0.132	Pass

z k x HHH	HHHHHHH	- HHHH
Solv Inc. JRD	Amaral	SK-10 Apr 18, 2023
17009		23-0417 - 1x35 TDP 1.0 55°- Trina
17003	l	







olv Inc. Amaral SK-14 RD Apr 18, 2023	z k x	HAR AND	
Amarai SK-14 RD Apr 18, 2023		A maral	SK 14
Δμ110, 2020		Amarai	Sr-14 Apr 18, 2023
7009 23-0417 - 1x35 TDP 1 0 55°- Trina	17009		23-0417 - 1x35 TDP 1.0 55°- Trina







Company : Solv Inc. Designer : JRD Job Number : 17009 Model Name : Amaral	
A NEWERSCHEK COMPANY	

4/18/2023 8:32:15 AM Checked By : NZ

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Envelope AISC 14th (360-10): ASD Steel Code Checks

Eqn	H3-6	H3-6	H1-1b	H1-1b	H1-1b	H1-1b	H1-1b	H1-1b	H1-1b	H1-1b	H1-1b
g	1	1	1	1	1	1	1	1	1	1	1
Mnzz/om [k-in]	92.515	92.515	94.678	94.678	421.115	94.678	94.678	94.678	94.678	94.678	94.678
Mnyy/om [k-in]	92.515	92.515	35.079	35.079	201.198	35.079	35.079	35.079	35.079	35.079	35.079
Pnt/om [k]	70.15	70.15	59.947	59.947	175.749	59.947	59.947	59.947	59.947	59.947	59.947
Pnc/om [k]	14.136	14.136	26.494	26.494	135.323	26.494	26.494	26 494	26.494	26.494	26.494
ပ	5	5	З	ი	4	З	ო	З	З	ო	ო
Ē	z	z	٨	٨	٨	٨	٨	٨	γ	>	>
Loc[in]	0	0	96.885	96.885	90	96.885	96.885	96.885	96.885	96.885	96.885
Shear Check	0.323	0.323	0.01	0.01	0.005	0.01	0.01	0.01	0.01	0.008	0 <u>.</u> 008
ပ	2	2	4	4	18	4	4	3	3	4	4
Loc[in]	0	0	0	0	0	0	0	0	0	0	0
Code Check	0.466	0.466	0.166	0.166	0.164	0.162	0.162	0.158	0.158	0.123	0.123
Shape	4X4X0.126	4X4X0.126	W6X7	W6X7	W6X20	W6X7	W6X7	W6X7	. W6X7	W6X7	W6X7
Member	Tube 1	Tube 2	IDLER POST 2	IDLER POST 7	DRIVE POST	IDLER POST 3	IDLER POST 6	IDLER POST 5	IDLER POST 4	DIDLER POST 1	IDLER POST 8
	~	2	3	4	2	9	7	ω	9	10	-

Detail Report: Tube 1		Unity Check: 0.466 (axia	Load Combine	ation: LC 2: IBC 16-10	
		nput Data: Shape: Member Type: Length (in): Material Type: Design Rule: Number of Internal Sections:	4X4X0.126 Beam 764.43 Hot Rolled Steel Typical 191	l Node: J Node: I Release: J Release: I Offset (in): J Offset (in):	GA VX1C Fixed N/A N/A
Material Properties:					
Material:	A500 Gr. 60	Therm. Coeff. (1e⁵°F ⁻¹):	0.65	R,,:	1.5
E (ksi):	29000	Density (k/ft³):	0.49	F, (ksi):	70
G (ksi):	11154	F _v (ksi):	60	R _t :	1.2
Nu:	0.3	,			
Shape Properties:					
d (in):	4	I,,,, (in⁴):	4.889	Area (in²):	1.952
b _f (in):	4	$I_{\pi}(in^4)$:	4.889	J (in ⁴):	7.326
t (in):	0.126	H (1)			
Design Properties:					
L _{b v-v} (in):	228	К _{v-v} :	1	Max Defl Ratio:	L/670
L _{b z-z} (in):	228	К _{z-z} :	1	Max Defl Location:	627.637
L _{comp top} (in):	Lbyy	y sway:	No	Span:	4
L _{comp bot} (in):	228	z sway:	No		
L _{torque} (in):	N/A	Function:	Lateral		
C ·	1	Seismic DR	None		

Limit State	Required	Available	Unity Check	Result
Applied Loading - Bending/Axial				
Applied Loading - Shear + Torsion	-	-	-	-
Axial Tension Analysis	0.000 k	70.15 k	-	-
Axial Compression Analysis	0.000 k	14.136 k	-	-
Flexural Analysis (Strong Axis)	17.809 k-in	92.515 k-in	-	-
Flexural Analysis (Weak Axis)	15.739 k-in	92.515 k-in	-	-
Shear Analysis (Major Axis y)	5.743 k	19.676 k	0.292	Pass
Shear Analysis (Minor Axis z)	5.725 k	19.676 k	0.291	Pass
Bending & Axial Interaction Check (UC Bending Max)	-	-	0.466	Pass
Torsional Analysis	21.284 k-in	81.361 k-in	0.262	Pass

Detail Report: VP 18		Unity Check: 0.464	(axia l /bending)	Load Comb	ination: LC 2: IBC 16-10
		nput Data: Shape: Member Type: Length (in): Material Type: Design Rule: Number of Internal Sections:	V-HU-2.25X0.055X1.25 Beam 55.118 Cold Formed Steel Typical 191	l Node: J Node: I Release: J Release: I Offset (in): J Offset (in):	V18B V18C Fixed Fixed N/A N/A
Material Properties:					
Material:	A653 Grade 50	Nu:	0.3	F _v (ksi):	50
E (ksi):	29500	Therm. Coeff. (1e ⁵ °F ⁻¹):	0.65	F _u (ksi):	70
G (ksi):	11346	Density (k/ft³):	0.49		
Shape Properties:					
D (in):	2.25	J (in ⁴):	0.000432	r _y (in):	N/A
B (in):	1.25	C _w (in ⁶):	0.21	x ₀ (in):	- 1.446
t(in):	0.055	r _o (in):	1.875	S _{e,z} (in³):	N/A
R(in):	0.112	X _c (in):	1.274	S _{fz} (in ³):	N/A
d (in):	1.25	m (in):	0.172	$S_{c,z}$ (in ³):	N/A
I _{yy} (in⁴):	0.308	j (in):	1.589	S _{e,y} (in³):	N/A
I_{zz} (in ⁴):	0.303	r _z (in):	N/A	S _{fy} (in ³):	N/A
Area (in ²):	0.428				
Design Properties:					
L _{b v-v} (in) :	N/A	К _{v-v} :	1	Max Defl Ratio:	L/10000
L _{b z-z} (in):	N/A	К _{z-z} :	1	Max Defl Location:	0
L _{comp top} (in):	Lbyy	R:	N/A	Span:	N/A
L _{comp bot} (in) :	N/A	y sway:	No		
C _b :	1	z sway:	No		
С _{ту-у} : С	N/A N/A	a (in):	N/A		
- m <i>z-2</i> :		VP	18		
• V18B					• V18C
Diagrams:			0.039 at 55.118 in	0.162 at 0 in	
					-0 19 at 55 118 in
		y Deflection	n (in)	z Deflectior	(in)
0.1	34 at 25 238 in	2.588e-04 at 29.88 in		0.133 at 29.88 in	
-0.134 at 29.88 in		-2.615e-04 at 0 in		-0.1	34 at 25.238 in
Axial Force	e (kips)	y Shear Force	(kips)	z Shear Force	(kips)

AISI S100-12: ASD Code Check

Max Bending Loc: Equation: Gov Φ Equation: R (D6.1.1) Max Shear Loc: Max Defl Ratio: Location: Span:	25.238 in C5.2.1-3 C3.1.1 Not Used 25.238 in L/10000 0 in N/A	Cm (y-y): Cm (z-z): Cb: KL/r (y-y): KL/r (z-z): L Comp Flange: L Torque:	0.85 0.6 1 64.983 55.118 in 55.118 in	A A Iy Sy Sy Iz Sz Sz	e (Fy): e (Fn): eff: v eff (L): v eff (R): eff: : eff (T): : eff (B):	0.373 in ² 0.428 in ² 0.308 in ⁴ 0.237 in ³ 0.325 in ³ 0.247 in ⁴ 0.142 in ³		
Limit State				Required	Available	Unity Check	Result	
Axial Tension Analy	/sis			-	12.829 k	-	-	
Axial Compression	Analysis			-	2.992 k	-	-	
Flexural Analysis (S	Strong Axis)			-	3.903 k-in	-	-	
Flexural Analysis (V	Veak Axis)			-	7.094 k-in	-	-	
Shear Analysis (Ma	jor Axis y)		-	0.945 k	-	-		
Shear Analysis (Mi	nor Axis z)		-	3.952 k	0.034	Pass		
Bending & Axial Int	Bending & Axial Interaction Check (UC Bending Max) 0.464 Pass							

Detail Report: DRIVE POST		Unity Check: 0.16	64 (axial/bending)	Load Combination: LC 18: IBC 16-14 (B) (A		
	, z	nput Data: Shape: Member Type: Length (in): Material Type: Design Rule: Number of Internal Sectio	W6X20 Column 90 Hot Rolled Steel Typical ons: 191	l Node: J Node: I Release: J Release: I Offset (in): J Offset (in):	D1 D2 Fixed AllPIN N/A N/A	
Material Properties:						
Material: E (ksi): G (ksi): Nu:	A992 29000 11154 0.3	Therm. Coeff. (1e ^s °F ⁻¹): Density (k/ft³): F _y (ksi):	0.65 0.49 50	R _y : F _u (ksi): R _t :	1.1 65 1.1	
Shape Properties:						
d (in): b _f (in): t _f (in): t _w (in): I _{yy} (in ⁴): I _{zz} (in ⁴):	6.2 6.02 0.365 0.26 13.3 41.4	Area (in ²): Z _{yy} (in ³): Z _{zz} (in ³): C _w (in ⁶): W _{no} (in ²):	5.87 6.72 14.9 113 8.78	S _w (in ⁴): r _T (in): J (in ⁴): k _{det} (in): k _{des} (in):	4.82 1.64 0.24 0.875 0.615	
Design Properties:						
L _{b y-y} (in) : L _{b z-z} (in) : L _{comp top} (in) : L _{comp bot} (in) : L _{torque} (in) : C _b :	N/A N/A Lbyy N/A N/A 1	K _{y-y} : K _{z-z} : y sway: z sway: Function: Seismic DR:	1 1 No No Lateral None	Max Defl Ratio: Max Defl Location: Span:	L/618 0 N/A	
			DRIVE POST			
D1					• D2	
Diagrams:					0.146 at 90 in	

Limit State	Required	Available	Unity Check	Result
Applied Loading - Bending/Axial				
Applied Loading - Shear + Torsion	-	-	-	-
Axial Tension Analysis	0.000 k	175.749 k	-	-
Axial Compression Analysis	1.53 k	135.323 k	-	-
Flexural Analysis (Strong Axis)	31.848 k-in	421.115 k-in	-	-
Flexural Analysis (Weak Axis)	16.608 k-in	201.198 k-in	-	-
Shear Analysis (Major Axis y)	0.018 k	32.24 k	0.001	Pass
Shear Analysis (Minor Axis z)	0.187 k	78.945 k	0.002	Pass
Bending & Axial Interaction Check (UC Bending Max)	-	-	0.164	Pass

Detail Report: IDLER POST 2		Unity Check: 0.166 (axial/bending)	Load Combina	tion: LC 4: IBC 16-12 (B)
1× 1×	lr	nput Data:	W6X7	l Node:	N345
	> ^z	Member Type: Length (in): Material Type: Design Rule: Number of Internal Sections:	Column 96.885 Hot Rolled Steel Typical 191	J Node: J Release: J Release: I Offset (in): J Offset (in):	N343 N346 Fixed Custom N/A N/A
Material Properties:					
Material:	A992	Therm. Coeff. (1e⁵°F⁻¹):	0.65	R _v :	1.1
E (ksi):	29000	Density (k/ft³):	0.49	F _u (ksi):	65
G (ksi):	11154	F _v (ksi):	50	R _t :	1.1
Nu:	0.3	,		-	
Shape Properties:					
d (in):	5.772	Area (in²):	2.002	S _w (in⁴):	0.898
b _f (in):	3.94	Z _{yy} (in ³):	1.303	r _T (in):	1.047
t _f (in):	0.165	$Z_{zz}^{(in^3)}$:	4.6	J (in⁴):	0.016
t _w (in):	0.129	C _w (in ⁶):	13.227	k _{det} (in):	0.69
I_{yy} (in ⁴):	1.683	$W_{no}(in^2)$:	5.523	k _{des} (in):	0.46
I _{zz} (in ⁴):	11.955				
Design Properties:					
L _{b y-y} (in) :	N/A	К _{у-у} :	1	Max Defl Ratio:	L/583
L _{bz-z} (in):	N/A	K _{z-z} :	1	Max Defl Location:	0
L _{comp top} (in):	Lbyy	y sway:	No	Span:	N/A
L _{comp bot} (in):	N/A	z sway:	No		
L _{torque} (in):	N/A	Function:	Latera		
C _b :	1	Seismic DR:	None		
		IDL	ER POST 2		
• N345					• N346
Diagrama					
Diagranis.					
			-0.166 at 96 885 in		
		y Deflection	(in)	z Deflection	i (in)
0.657 at 0 in		0.15 at 0 in			
0.60	2 at 96.885 in				

y Shear Force(kips)

Axial Force (kips)

z Shear Force (kips)

Limit State	Required	Available	Unity Check	Result
Applied Loading - Bending/Axial				
Applied Loading - Shear + Torsion	-	-	-	-
Axial Tension Analysis	0.000 k	59.947 k	-	-
Axial Compression Analysis	0.657 k	26.494 k	-	-
Flexural Analysis (Strong Axis)	14.552 k-in	94.678 k-in	-	-
Flexural Analysis (Weak Axis)	0.000 k-in	35.079 k-in	-	-
Shear Analysis (Major Axis y)	0.15 k	14.892 k	0.01	Pass
Shear Analysis (Minor Axis z)	0.000 k	23.357 k	0.000	Pass
Bending & Axial Interaction Check (UC Bending Max)	-	-	0.166	Pass