## STATE OF CONNECTICUT CONNECTICUT SITING COUNCIL

IN RE:

| A PETITION OF CELLCO PARTNERSHIP | $:$ | PETITION NO. |
| :--- | :---: | :--- |
| D/B/A VERIZON WIRELESS FOR A | $:$ |  |
| DECLARATORY RULING ON THE NEED | $:$ |  |
| TO OBTAIN A SITING COUNCIL | $:$ |  |
| CERTIFICATE FOR MODIFICATIONS TO | $:$ |  |
| AN EXISTING WIRELESS | $:$ |  |
| TELECOMMUNICATIONS FACILITY AT 54 | $:$ |  |
| MEADOW STREET, NEW HAVEN, | $:$ | AUGUST 26, 2020 |
| CONNECTICUT | $:$ |  |

## PETITION FOR A DECLARATORY RULING: <br> INSTALLATION HAVING NO <br> SUBSTANTIAL ADVERSE ENVIRONMENTAL EFFECT

## I. Introduction

Pursuant to Sections 16-50j-38 and 16-50j-39 of the Regulations of Connecticut State Agencies ("R.C.S.A."), Cellco Partnership d/b/a Verizon Wireless ("Cellco") hereby petitions the Connecticut Siting Council (the "Council") for a declaratory ruling ("Petition") that no Certificate of Environmental Compatibility and Public Need ("Certificate") is required under Section 16-50k(a) of the Connecticut General Statutes ("C.G.S.") for modifications to its existing wireless telecommunications facility at 54 Meadow Street in New Haven, Connecticut (the "Property"). See Attachment 1 - Site Vicinity and Site Schematic Maps (Aerial Photograph).

## II. Factual Background

In April of 1991, Cellco received Council approval to establish a wireless telecommunication facility on the roof of the building at the Property (Council Docket No. 140). Cellco currently maintains twelve (12) antennas (three (3) sectors of four (4) antennas) attached to the façade of the rooftop penthouse on the building. Equipment associated with the Cellco antennas is located in a secure equipment room inside the building. The Property is owned by

Gateway Partners LLC (the "Owner"). MCM Holdings LLC manages the rooftop telecommunications facilities at the Property for the Owner.

## III. Proposed Construction Activity

## A. Cellco's Proposed Modifications to the New Haven Facility

The proposed facility modifications will involve the removal of nine (9) existing antennas and the installation of twelve (12) new antennas (fifteen (15) total) and ten (10) remote radio heads ("RRHs") at various locations on the roof of the building. Three (3) antennas will remain attached to the penthouse façade; four (4) new antennas will also be attached to the penthouse façade (Beta and Beta/Gamma Sector antennas); Two (2) existing antennas and four (4) new antennas will be attached to the existing mechanical screen wall in the northwest corner of the building rooftop (Alpha and Delta Sector antennas); and one (1) existing antenna and one (1) new antenna will be attached to the building façade along the southeast corner of the building (Beta and Gama Sector antennas). (See Cellco's Project Plans included in Attachment 2).

Cellco is licensed to provide wireless telecommunications services in the $850 \mathrm{MHz}, 1900$ $\mathrm{MHz}, 2100 \mathrm{MHz}$ and 28 GHz frequency ranges in New Haven and throughout the State of Connecticut. The modified facility will utilize all of Cellco's frequency ranges. Specifications for Cellco's antennas and remote radio heads are included in Attachment 3. A Structural Assessment Letter confirming that the building's structural components and the antenna mounting systems can adequately support Cellco's proposed facility modifications is included in Attachment 4.
IV. Discussion
A. The Proposed Facility Modifications Will Not Have A Substantial Adverse Environmental Effect

The Public Utility Environmental Standards Act (the "Act"), C.G.S. § 16-50g et seq.,
provides for the orderly and environmentally compatible development of telecommunications facilities in the state to avoid "a significant impact on the environment and ecology of the State of Connecticut." C.G.S. § 16-50g. To achieve these goals, the Act established the Council, and requires a Certificate of Environmental Compatibility and Public Need for the construction of cellular telecommunication towers "that may, as determined by the council, have a substantial adverse environmental effect". C.G.S. § 16-50k(a).

## 1. Physical Environmental Effects

Cellco respectfully submits that the modifications to its roof-top facility will not involve a significant alteration in the physical and environmental characteristics of the Property.

## 2. Visual Effects

Portions of the existing building and Cellco's modified facility may be visible from the commercial and industrial properties that surround the Property. The modifications proposed by Cellco will not, however, increase the visibility of the Cellco facility or the building and will not detract from the general characteristics of the building at the Property. A Photo Documentation \& Simulations report ("Visual Assessment") is included in Attachment 5.

## 3. FCC Compliance

Radio frequency ("RF") emissions from Cellco's modified facility will not exceed the maximum permissible exposure limits established by the Federal Communications Commission ("FCC"). Included in Attachment 6 is a general power density table that demonstrates that Cellco's modified facility will operate within the FCC safety standards.

## 4. FAA Notification Not Required

Cellco has run an Obstruction Analysis Report ("FAA Report") for the proposed New Haven Facility modifications. According to the FAA Report, notice of the facility modifications to the FAA is not required. A copy of the FAA Report is included in Attachment 7.

## B. Notice to the City, Property Owner and Abutting Landowners

On August 26, 2020, a copy of this Petition was sent to New Haven's Mayor, Justin Elicker; Aicha Woods, New Haven's Director of City Plan; Gateway Partners LLC, the Owner of the Property, and MCM Holdings LLC, the roof-top manager. Copies of the letters sent to Mayor Elicker, Ms. Woods, Gateway Partners LLC, and MCM Holdings LLC are included in Attachment 8.

A copy of this Petition was also sent to the owners of land that abut the Property. A sample abutter's letter and the list of those abutting landowners to whom notice was sent is included in Attachment 9.

## V. Conclusion

Based on the information provided above, Cellco respectfully requests that the Council issue a determination, in the form of a declaratory ruling, that the replacement and relocation of antennas, RRHs and related equipment, as described above, will not have a substantial adverse environmental effect and does not require the issuance of a Certificate of Environmental Compatibility and Public Need pursuant to § 16-50k of the General Statutes.

Respectfully submitted,

## CELLCO PARTNERSHIP d/b/a VERIZON WIRELESS

Kenneth C. Baldwin, Esq. Robinson \& Cole LLP 280 Trumbull Street Hartford, CT 06103-3597
(860) 275-8200

Its Attorneys

## ATTACHMENT 1



## Legend

Proposed Verizon Wireless Facility
【 Surrounding Verizon Wireless Facilities
Municipal Boundary

## Site Vicinity Map

Proposed Wireless
Telecommunications Facility New Haven CT
54 Meadow Street
New Haven, Connecticut


Legend
$\square$ Proposed/Relocated Verizon Wireless Equipment
= = = = Proposed Verizon Wireless Cable
$\square$ Existing Verizon Wireless Equipment to Remain
Existing Verizon Wireless Equipment Room

## Site Schematic

Proposed Wireless
Telecommunications Facility
New Haven CT
54 Meadow Street
New Haven, Connecticut

## ATTACHMENT 2







## ATTACHMENT 3

## MX10FIT665-xx

## NWA V ${ }^{\text {T }} \boldsymbol{X}$-Pol Ten-Port Antenna

X-Pol Ten-Port 6 ft, $65^{\circ}$ Form in Tighter with Smart Bias Ts, 698-4200 MHz:
2 ports $698-894 \mathrm{MHz}, 4$ ports $1695-2180 \mathrm{MHz}$, and 4 ports $3400-4200 \mathrm{MHz}$

- Excellent passive intermodulation (PIM) performance reduces harmful interference.
- Fully integrated (iRETs) with independent RET control for low band and mid band
- FET configured with internal RET for high band \& ease of future network optimization.
- SON-Ready array spacing supports beamforming capabilities
- Suitable for 3G, 4G, and 5G interface technologies
- Integrated Smart Bias-Ts reduce leasing costs
- Optimized form factor for reduced wind loading

חWWRV

| Electrical specification (minimum/maximum) | Ports 1, 2 |  | Ports 3, 4, 5, 6 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Frequency bands, MHz | 698-798 | 824-894 | 1695-1880 | 1850-1990 | 1920-2180 |
| Polarization | $\pm 45^{\circ}$ |  | $\pm 45^{\circ}$ |  |  |
| Average gain over all tilts, dBi | 14.4 | 14.8 | 17.8 | 18.1 | 18.2 |
| Horizontal beamwidth (HBW), degrees ${ }^{1}$ | 66.0 | 57.0 | 63.0 | 63.0 | 58.0 |
| Front-to-back ratio, co-polar power @ $180^{\circ} \pm \mathbf{3 0}^{\circ}$, dB | >22 | >22.0 | >25.0 | >25.0 | >25.0 |
| X-Pol discrimination (CPR) at boresight, dB | >17.0 | >15.6 | >23 | >18 | >18 |
| Vertical beamwidth (VBW), degrees ${ }^{1}$ | 13.5 | 12.0 | 6.0 | 5.5 | 5.4 |
| Electrical downtilt (EDT) range, degrees | 2-14 |  | 0-9 |  |  |
| First upper side lobe (USLS) suppression, $\mathrm{dB}^{1}$ | $\leq-17.0$ | $\leq-16.0$ | $\leq-17.0$ | $\leq-16.0$ | $\leq-16.0$ |
| Cross-polar isolation, port-to-port, $\mathrm{dB}^{1}$ | 25 | 25 | 25 | 25 | 25 |
| Max VSWR / return loss, dB | 1.5:1 /-14.0 |  | 1.5:1 /-14.0 |  |  |
| Max passive intermodulation (PIM), 2x20W carrier, dBc | -153 |  | -153 |  |  |
| Max input power per any port, watts | 300 |  | 250 |  |  |
| Total composite power all ports (1-10), watts | 1500 |  |  |  |  |

[^0]| Electrical specification (minimum/maximum) | Ports 7, 8, 9, 10 |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Frequency bands, MHz | 3400-3550 | 3550-3700 | 3700-3950 | 3950-4200 |
| Polarization | $\pm 45^{\circ}$ |  |  |  |
| Average gain over all tilts, dBi | 13.6 | 13.8 | 14.0 | 14.2 |
| Horizontal beamwidth (HBW), degrees | 65 | 62 | 60 | 58 |
| Front-to-back ratio, co-polar power @ $180^{\circ} \pm 3^{\circ}{ }^{\circ}$, dB | >23 | >23 | >23 | >22 |
| Vertical beamwidth (VBW), degrees ${ }^{1}$ | 20 | 19.6 | 19.3 | 18.5 |
| Electrical downtilt (EDT) range, degrees | 2-12 orderable in 1 deg increments |  |  |  |
| First upper side lobe (USLS) suppression, $\mathrm{dB}^{1}$ | $\leq-15$ | $\leq-15$ | $\leq-15$ | $\leq-15$ |
| Cross-polar isolation, port-to-port, $\mathrm{dB}^{1}$ | 25 | 25 | 25 | 25 |
| Max VSWR / return loss, dB | 1.5:1 / -14.0 |  |  |  |
| Max input power per any port, watts | 150 |  |  |  |
| Total composite power all ports (1-10), watts | 1500 |  |  |  |

1 Typical value over frequency and tilt

* For ports 7-10, the electrical downtilt is FET configured with internal RET, where the required electrical downtilt is defined at the time of order per the ordering information below.


## Ordering information

| Antenna model | Description |
| :---: | :---: |
|  | $6 F$ X- Pol 10 Port FIT 65 ${ }^{\circ} 2-14 \%$ 0-9 ${ }^{\circ} / 2-12^{\circ}, 4.3-10$ \& SBTs |
| MX10FIT665-xx (xx represents the FET in one degree increments for $3.4-4.2 \mathrm{GHz}$ ) | $\mathrm{xx}=02$ thru 12 for each 1 degree tilt $3.4-4.2 \mathrm{GHz}$ <br> Examples MX10FIT665-02 - 2deg, MX10FIT665-09 - 9deg, MX10FIT665-12- <br> 12deg |
| Optional accessories |  |
| AISG cables | M/F cables for AISG connections |
| PCU-1000 RET controller | Stand-alone controller for RET control and configurations |
| 91900314-02 | Dual Mount Bracket (see 91900314 bracket document for details) |

MX10FIT665-xx
NWA $V^{\text {TM }}$ X-Pol Ten-Port Antenna
Mechanical specifications

| Dimensions height/width/depth, inches (mm) | $70.9 / 12.2 / 7.5(1800 / 309.9 / 190.5)$ |
| :--- | :--- |
| Shipping dimensions length/width/height, inches (mm) | $76 / 20 / 14.5(1930 / 508 / 368)$ |
| No. of RF input ports, connector type, and location | $10 \times 4.3-10$ female, bottom |
| RF connector torque | $96 \mathrm{lbf} \cdot \mathrm{in}(10.85 \mathrm{~N} \cdot \mathrm{~m}$ or $8 \mathrm{lbf} \cdot \mathrm{ft})$ |
| Net antenna weight, $\mathbf{\mathrm { lb }} \mathbf{( k g )}$ | $53.4(24.3)$ |
| Shipping weight, lb (kg) | $97.5(44.3)$ |
| Antenna mounting and downtilt kit included with antenna | 91900318 |
| Net weight of the mounting and downtilt kit, lb (kg) | $20.3(9.2)$ |
| Range of mechanical up/down tilt | $-2^{\circ}$ to 12 |
| Rated wind survival speed, mph (km/h) | $150(241)$ |
| Frontal, lateral, and rear wind loading @ 150 km/h, lbf (N) | $74.1(330), 26.1(116), 69.8(311)$ |
| Equivalent flat plate @ 100 mph and Cd=2, sq ft | 1.49 |

Front view

MX10FIT665-xx
NWA V ${ }^{\text {T }}$ X-Pol Ten-Port Antenna
W|RELESS
Remote electrical tilt (RET 1000) information

| RET location | Integrated into antenna |
| :--- | :--- |
| RET interface connector type | 8-pin AISG connector per IEC 60130-9 or RF port bias-t |
| RET connector torque | Min $0.5 \mathrm{~N} \cdot \mathrm{~m}$ to max $1.0 \mathrm{~N} \cdot \mathrm{~m}$ (hand pressure \& finger tight) |
| RET interface connector quantity | 2 pairs of AISG male/female connectors and 2 RF port bias-ts |
| RET interface connector location | Bottom of the antenna |
| Total no. of internal RETs $\mathbf{6 9 8} \mathbf{- 8 9 4} \mathbf{~ M H z}$ | 1 |
| Total no. of internal RETs $\mathbf{1 6 9 5 - 2 1 8 0} \mathbf{~ M H z}$ | 1 |
| Total no. of internal RETs $\mathbf{3 4 0 0} \mathbf{- 4 2 0 0} \mathbf{~ M H z}$ | 1 |
| RET input operating voltage, vdc | $10-30$ |
| RET max power consumption, idle state, $\mathbf{W}$ | $\leq 2.0$ |
| RET max power consumption, normal operating conditions, $\mathbf{W}$ | $\leq 13.0$ |
| RET communication protocol | AISG 2.0 / 3GPP |

## RET and RF connector topology

Each RET device can be controlled either via the designated external AISG connector or RF smart bias-t port as shown below:


Note: The RET Device for $3400-4200 \mathrm{MHz}$ is connected via the 1695-2180 Port 3 Bias T port or 1695-2180/3400-4200 MHz AISG ports.


## SAMSUNG

## Dual-Band Radio Unit 700/850MHz (B13/B5) <br> RFV01U-D2A

Samsung's RFV01U-D2A is a compact remote Radio Unit (RU) designed for deployments that require flexibility in installation and rapid onlining, without compromising on coverage, capacity or operational expenses.


Features and Benefits

- Dual-band support for broad frequency coverage
- Minimal footprint reduces site costs
- Rapid, easy installation
- Flexibly deployable in any location
- Remote RF monitoring capability
- Convection cooled, silent operation

The RFV01U-D2A RU targets dual-band support across Band $13(700 \mathrm{MHz})$ and Band $5(850 \mathrm{MHz})$, making it an ideal product for broad coverage footprints across multiple common low-end, long-range frequencies.

The RU handles all Radio Frequency (RF) processing in a single, compact unit, and is designed to interface via CPRI with Samsung's CDU baseband offerings, in both distributedand central-RAN configurations.

In addition to its minimal footprint and ease of installation, the RU is also designed to reduce cost of ownership through its integrated spectrum analyzer, which allows for remote RF monitoring, greatly reducing the need for on-site maintenance visits.

Key Technical Specifications
Duplex Type: FDD
Operating Frequencies:

> B13: DL(746-756MHz)/UL(777-787MHz)

B5: DL( $869-894 \mathrm{MHz}) / \mathrm{UL}(824-849 \mathrm{MHz})$
Instantaneous Bandwidth: $10 \mathrm{MHz}(\mathrm{B} 13)+25 \mathrm{MHz}(\mathrm{B} 5)$
RF Chain: 4T4R/2T4R/2T2R
Output Power: Total 320W
DU-RU Interface: CPRI (10Gbps)
Dimensions: $380 \times 380 \times 207 \mathrm{~mm}$ (29.9L)
Weight: 31.9 kg
Input Power: -48V DC
Operating Temp.: -40-55 (w/o solar load)
Cooling: Natural convection

## SAMSUNG

## Dual-Band Radio Unit <br> AWS/PCS (B66/B2) <br> RFV01U-D1A


#### Abstract

Samsung's RFV01U-D1A is a compact remote Radio Unit (RU) designed for deployments that require flexibility in installation and rapid onlining, without compromising on coverage, capacity or operational expenses.




Features and Benefits

- Dual-band support for broad frequency coverage
- Minimal footprint reduces site costs
- Rapid, easy installation
- Flexibly deployable in any location
- Remote RF monitoring capability
- Convection cooled, silent operation
- Built-in Broadcast Auxiliary Services (BAS) filter ensures compliant AWS operation without impacting footprint

The RFV01U-D1A RU targets dual-band support across Band 66 (AWS) and Band 2 (PCS), making it an ideal product for broad coverage footprints across multiple common mid-range frequencies.

The RU handles all Radio Frequency (RF) processing in a single, compact unit, and is designed to interface via CPRI with Samsung's CDU baseband offerings, in both distributedand central-RAN configurations.

In addition to its minimal footprint and ease of installation, the RU is also designed to reduce cost of ownership through its integrated spectrum analyzer, which allows for remote RF monitoring, greatly reducing the need for on-site maintenance visits.

Key Technical Specifications
Duplex Type: FDD
Operating Frequencies:
B66: DL( $2,110-2,180 \mathrm{MHz}) / \mathrm{UL}(1,710-1,780 \mathrm{MHz})$
B2: DL( $1,930-1,990 \mathrm{MHz}) / \mathrm{UL}(1,850-1,910 \mathrm{MHz})$
Instantaneous Bandwidth: $70 \mathrm{MHz}(\mathrm{B} 66)+60 \mathrm{MHz}(\mathrm{B} 2)$
RF Chain: 4T4R/2T4R/2T2R
Output Power: Total 320W
DU-RU Interface: CPRI (10Gbps)
Dimensions: $380 \times 380 \times 255 \mathrm{~mm}$ (36.8L)
Weight: 38.3 kg
Input Power: -48V DC
Operating Temp.: -40-55 (w/o solar load)
Cooling: Natural convection

## Specifications

The table below outlines the main specifications of the AU:

Table 1. Specifications

| Item |  | AT1K01 |
| :---: | :---: | :---: |
| Technology |  | 5G NR |
| Operating Frequency |  | 27.5 to 28.35 GHz |
| RF Chain |  | 1024 TR/unit |
| Antenna Array | Configuration | 1024 AE (4T4R) |
|  | Element | 256 AE (16H16V)/path, 1024 AE/unit |
|  | Gain | $28 \mathrm{dBi} /$ path |
| IBW/OBW |  | 850/800 MHz |
| Channel Bandwidth/Capacity |  | $100 \mathrm{MHz}$ <br> Max 8CC (50/200/400 MHz will be supported in ES2, SVR19A: 100 MHz ) |
| RF Output Power |  | $26 \mathrm{dBm} /$ path, $32 \mathrm{dBm} / \mathrm{unit}$ |
| Input Voltage |  | -48 V DC ( -36 to -58 V DC) or 100 to 240 V AC |
| Input Current |  | 10.9 A @ -48 V DC <br> 4.3 A @ 100 to 240 V AC |
| LED |  | Total: 1 EA <br> Powered, Operational, Fail (3 Status w/different colors) |
| Operational Temperature |  | $-40 \sim 55^{\circ} \mathrm{C}$ (with solar load) |
| Humidity |  | TBD |
| IP rating |  | IP65 |
| EMC |  | FCC Title 47 CFR Part 15 Subpart B |
| Safety |  | UL 60950 or 62368 |
| Installation |  | Pole/Wall/Tower mounting |
| Dimension (W $\times \mathrm{D} \times \mathrm{H}$ ) |  | - 9.57 in. $(243 \mathrm{~mm}) \times 6.89 \mathrm{in}$. $(175 \mathrm{~mm}) \times 16.81 \mathrm{in}$. ( 427 mm ) •(@without cover) <br> - 9.57 in. $(243 \mathrm{~mm}) \times 6.89 \mathrm{in} .(175 \mathrm{~mm}) \times 19.4 \mathrm{in}$. $(493 \mathrm{~mm})$ (@with cover \& GPS Port) |
| Volume |  | < 18.16 L |
| Weight |  | $<33.07 \mathrm{lb}(15.8 \mathrm{~kg})$ |

## [CBRS RRH] Spec.



| Item | Specification |
| :---: | :---: |
| Band | Band 48 ( 3.5 GHz ) |
| Frequency | $3550 \sim 3700 \mathrm{MHz}$ |
| IBW | 150 MHz |
| OBW | 80 MHz |
| \# of Carriers | $5 / 10 / 15 / 20 \mathrm{MHz} \times 4$ carriers |
| RF Chain | 4TX / 4RX |
| RF Output Power \& EIRP | 4 path $\times 5 \mathrm{~W}$ (Total: $20 \mathrm{~W}=43 \mathrm{dBm}$ ) <br> (EIRP: $47 \mathrm{dBm} / 10 \mathrm{MHz}$ ) |
| RX Sensitivity | Typical : -101.5 dBm @ 1 Rx (3GPP 36.104, Wide Area) |
| Modulation | 256-QAM support (1024-QAM with 1~2dB power back-off) |
| Input Power | -48 VDC (-38 to -57 VDC, 1 SKU), with clip-on AC-DC converter (Option) |
| Power Consumption | About 160 Watt @ 100\% RF load, typical conditions |
| Volume | Under 7L (w/o Antenna), Under 9.6L (with antenna) |
| Weight | Under 8.0 kg ( 18.64 lb ) (w/o Antenna), Under 10.5 Kg (with ant.) |
| Operating Temperature | $-40^{\circ} \mathrm{C}\left(-40^{\circ} \mathrm{F}\right) \sim 55^{\circ} \mathrm{C}\left(131^{\circ} \mathrm{F}\right)$ (W/o solar load) |
| Cooling | Natural convection |
| Unwanted Emission | 3GPP 36.104 Category A |
| Unwanted Emission | [B48] : FCC 47 CFR 96.41 e ) |
| Optic Interface | 20 km , 2 ports (9.8Gbps $\times 2$ ), SFP, single mode, duplex or Bi-Di |
| CPRI Cascade | Not supported |
| \# of Antenna Port | 4 |
| External Alarm (UDA) | 4 |
| RET | AISG 2.2 |
| TMA \& built-in Bias-T I//F and PIM cancellation | Not supported |
| Mounting Options | Pole, wall, tower, back to back, side by side (for external ant), 3 RRH with Clip-on Antenna on the pole |
| Antenna Type | Integrated (Clip-on) antenna (Option), <br> External antenna (Option) |
| NB-IoT | Not Supported (HW Resource reserved for 1 Guard Band NB-IoT per LTE carrier) |
| Spectrum Analyzer | TX/RX Support |
| External Alarm (UDA) | 4 |
| 5G NR | Support with S/W upgrade |
| XRAN | Support with S/W upgrade |

## 5G NR AU (AT1K01) Product Specifications

|  | 28GHz | 39GHz |
| :---: | :---: | :---: |
| Integrated AU |  |  |
| Operating frequency | 26.5 ~ 29.5GHz | $37 \sim 40 \mathrm{GHz}$ |
| IBW/OBW | $850 \mathrm{MHz} / 800 \mathrm{MHz}$ | $1.4 \mathrm{GHz} / 800 \mathrm{MHz}$ |
| EIRP | 60 dBm | 59 dBm |
| Antenna Gain | 25 dBi | 24 dBi |
| Tx/Rx |  |  |
| Antenna Elements |  |  |
| Beam Scan Range |  |  |
| Size/Weight | $9.6 \times 16.8 \times 6.9 \mathrm{in} \mathrm{(18.1)} / 15.0 \mathrm{Kg}$ (33lbs) |  |
| Input Power | -48VDC / 100 ~ 240VAC |  |
| Power Consumption | 362W |  |
| Midhaul (gNB-CU Interface) | 10G Optic x 2 ports |  |
| Installation | Outdoor Pole/Wall Mount |  |
| Clock Synchronization | GPS and IEEE 1588v2 |  |
| Operating Temperature | -40 deg C to +55 deg C with solar load |  |
| Cooling | Natural Convection |  |

## Appearance


9.6(243)


## ATTACHMENT 4

# On Air Engineering, ШС 

88 Foundry Pond Road
Cold Spring, NY 10516
onair@optonline.net
July 13, 2020
Mr. Aleksey Tyurin
Verizon Wireless
20 Alexander Drive
Wallingford, CT 06492
Re: New Haven CT - Structural Assessment Letter - ANTMO 28GHz Carrier Add/Sector Add
MCM Site \# CT-520; 54 Meadow St., New Haven, CT

## Dear Aleksey:

This letter serves as a Structural Assessment for the proposed Cellco Partnership ( $\mathrm{d} / \mathrm{b} / \mathrm{a}$ Verizon Wireless) antenna modification on the above referenced building.

Verizon Wireless is proposing to modify their existing 3-sector antenna configuration with a 4-sector configuration by replacing existing antennas, adding new antennas and relocating several antennas on the structure. The proposed antenna locations are detailed in Zoning Drawings prepared by our office dated July 13,2020 . Verizon's equipment room is located on the 11 th floor inside the building.

Verizon's existing (12) panel antennas are all flush mounted to the central penthouse façade, extending approx. 18" above the parapet wall. The proposed re-configuration will maintain (7) antenna positions on the penthouse, some of which are being relocated to new mounts on the eastern penthouse facade. Verizon's other (5) antennas will be replaced and relocated to the existing mechanical HVAC screen wall framing in the northwest corner of the roof and supplemented with a sixth antenna, all flush mounted to that structure. Lastly, two additional antennas are proposed in southeast corner of the building, mounted to the inside parapet wall and extending over the top to achieve a "flush mount" appearance. Verizon's existing RRH's and OVP's will also be replaced, relocated and supplemented with new equipment as part of the modification, which will yield a total of (15) panel antennas upon completion.

The building structural components have been evaluated for this proposed modification, including the new antenna mounts and we have determined that they are capable of supporting the proposed loading, as per the attached structural calculations.

In conclusion, the proposed Verizon Wireless modification will not negatively impact the structural integrity of the host building and will be installed in accordance with the 2018 Connecticut State Building Code, adopted model codes (as amended) and all referenced standards, including TIA-222-G. Our findings are based on the assumption that the hosting structure and all structural members and appurtenances were properly designed, detailed, fabricated, installed and have been properly maintained since erection. Should you have any questions, please do not hesitate to contact our office.

enclosure

## STRUCTURAL CALCULATIONS

FOR

VERIZON SITE NAME: NEW HAVEN CT 54 MEADOW ST NEW HAVEN, CT

DAVID A. WEINPAHL, P.E.<br>ON AIR ENGINEERING, LLC<br>88 FOUNDRY POND ROAD COLD SPRING, NY 10516

PBA ENGINEERING, P.C.
12 KULICK ROAD
FAIRFIELD, NEW JERSEY 07004-3363
PHONE: (973) 276-1700
FAX: (973) 276-9766

STRUCTURAL ENGINEERS
PROJECT NO. N-545
DATE: 6/15/2020
TOTAL NO. PAGES ATTACHED: 17

|  | PPEN | DIX N） | UNI | Wind Design Parameters |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| $\frac{\underset{Z}{2}}{\bar{\sigma}}$ |  | MCESpectralAccelerations$(\% \mathrm{~g})$ |  | Ultimate Design Wind Speeds，$V_{\text {ult }}$ （mph） |  |  | Nominal Design Wind Speeds，$V_{\text {asd }}$ （mph） |  |  | Wind－Borne Debris Regions ${ }^{1}$ |  |  |
|  |  | Ss | $\mathrm{S}_{1}$ | Risk Cat．I | Risk Cat．II | $\begin{aligned} & \text { Risk } \\ & \text { Cat } \end{aligned}$ IIIIIV | Risk Cat．I | Risk Cat． II | Risk Cat． <br> III－IV |  |  |  |
| New Haven | 30 | 0.186 | 0.062 | 115 | 125 | 135 | 89 | 97 | 105 |  | Type C | Yes |
| Newington | 30 | 0.182 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  |  |
| New London | 30 | 0.161 | 0.058 | 125 | 135 | 145 | 97 | 105 | 112 | Type B | Type A | Yes |
| New Milford | 35 | 0.198 | 0.066 | 105 | 115 | 125 | 81 | 89 | 97 |  |  |  |
| Newtown | 30 | 0.208 | 0.066 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Norfolk | 40 | 0.175 | 0.065 | 105 | 115 | 125 | 81 | 89 | 97 |  |  |  |
| North Branford | 30 | 0.179 | 0.061 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| North Canaan | 40 | 0.173 | 0.065 | 105 | 115 | 120 | 81 | 89 | 93 |  |  |  |
| North Haven | 30 | 0.184 | 0.062 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| North Stonington | 30 | 0.163 | 0.059 | 125 | 135 | 145 | 97 | 105 | 112 |  | Type A | Yes |
| Norwalk | 30 | 0.232 | 0.067 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Norwich | 30 | 0.168 | 0.060 | 125 | 135 | 145 | 97 | 105 | 112 |  | Type A | Yes |
| Old Lyme | 30 | 0.164 | 0.059 | 125 | 135 | 145 | 97 | 105 | 112 | Type B | Type A | Yes |
| Old Saybrook | 30 | 0.164 | 0.059 | 125 | 135 | 145 | 97 | 105 | 112 | Type B | Type A | Yes |
| Orange | 30 | 0.192 | 0.063 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Oxford | 30 | 0.196 | 0.064 | 110 | 125 | 130 | 85 | 97 | 101 |  |  | Yes |
| Plainfield | 35 | 0.170 | 0.061 | 125 | 135 | 145 | 97 | 105 | 112 |  | Type A | Yes |
| Plainville | 35 | 0.184 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Plymouth | 35 | 0.186 | 0.064 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Pomfret | 40 | 0.172 | 0.063 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Portland | 30 | 0.180 | 0.063 | 115 | 130 | 135 | 89 | 101 | 105 |  |  | Yes |
| Preston | 30 | 0.167 | 0.060 | 125 | 135 | 145 | 97 | 105 | 112 |  | Type A | Yes |
| Prospect | 30 | 0.188 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Putnam | 40 | 0.172 | 0.063 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Redding | 30 | 0.220 | 0.067 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Ridgefield | 30 | 0.230 | 0.068 | 110 | 120 | 125 | 85 | 93 | 97 |  |  | Yes |
| Rocky Hill | 30 | 0.181 | 0.063 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Roxbury | 35 | 0.197 | 0.065 | 110 | 120 | 125 | 85 | 93 | 97 |  |  | Yes |
| Salem | 30 | 0.170 | 0.060 | 120 | 135 | 140 | 93 | 105 | 108 |  | Type A | Yes |
| Salisbury | 40 | 0.173 | 0.065 | 105 | 115 | 120 | 81 | 89 | 93 |  |  |  |
| Scotland | 30 | 0.172 | 0.061 | 120 | 130 | 140 | 93 | 101 | 108 |  |  | Yes |
| Seymour | 30 | 0.194 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Sharon | 40 | 0.179 | 0.065 | 105 | 115 | 120 | 81 | 89 | 93 |  |  |  |
| Shelton | 30 | 0.199 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Sherman | 35 | 0.202 | 0.066 | 105 | 115 | 120 | 81 | 89 | 93 |  |  |  |
| Simsbury | 35 | 0.179 | 0.064 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Somers | 35 | 0.174 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| Southbury | 35 | 0.198 | 0.065 | 110 | 120 | 130 | 85 | 93 | 101 |  |  | Yes |
| Southington | 30 | 0.185 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |
| South Windsor | 30 | 0.178 | 0.064 | 115 | 125 | 135 | 89 | 97 | 105 |  |  | Yes |

```
                                    MecaWind v2333
Software Developer: Meca Enterprises Inc., www.meca.biz, Copyright © 2018
Calculations Prepared by:
Date: Jun 12, 2020
FileLocation : U:\Mike\N-Jobs\N-545\N-545.wnd
```


## Basic Wind Parameters

| Wind Load Standard | $=$ ASCE $7-16$ | Exposure Category | $=$ B |
| :--- | :--- | :--- | :--- |
| Wind Design Speed | $=125.0$ mph | Risk Category | II |
| Structure Type | $=$ Other | Other Structure Type | Solid Sign |

## General Wind Settings

| Incl_LF | $=$ Include ASD Load Factor of 0.6 in Pressures | = False |
| :---: | :---: | :---: |
| DynType | $=$ Dynamic Type of Structure | = Rigid |
| NF | = Natural Frequency of Structure (Mode 1) | $=1.000 \mathrm{~Hz}$ |
| Zg | = Altitude (Ground Elevation) above Sea Level | $=0.000 \mathrm{ft}$ |
| Bdist | $=$ Base Elevation of Structure | $=0.000 \mathrm{ft}$ |
| Reacs | $=$ Show the Base Reactions in the output | = False |
| MWFRSType | $=$ MWFRS Method Selected | $=$ Ch 27 Pt 1 |
| Topographic Factor per Fig 26.8-1 |  |  |
| Topo | = Topographic Feature | $=$ None |
| Kzt | = Topographic Factor | $=1.000$ |

## Solid Sign Inputs

h : Height to Top of Sign $=141.500 \mathrm{ft} \mathrm{B}$ : Horizontal Width of Sign $=3.000 \mathrm{ft}$
Lr : Dimension of return corner= 1.000 ft s : Vertical Height of Sign $=8.000 \mathrm{ft}$
e : Solidity Ratio $=1.000$ Att: Attached to Wall = False

Exposure Constants per Table 26.11-1:
Alpha: Const from Table 26.11-1=7.000 Zg: Const from Table 26.11-1=1200.000 ft
At: Const from Table 26.11-1= 0.143
Am: Const from Table 26.11-1= 0.250
Bt: Const from Table 26.11-1= 0.840
$\mathrm{Bm}: \quad$ Const from Table 26.11-1= 0.450
Eps: Const from Table 26.11-1=0.333

## Gust Factor Calculation:

Gust Factor Category I Rigid Structures - Simplified Method
G1 $=$ For Rigid Structures (Nat. Freq.>1 Hz) use $0.85=0.85$
Gust Factor Category II Rigid Structures - Complete Analysis
$\mathrm{Zm}=0.6 * \mathrm{Ht}=84.900 \mathrm{ft}$
$\operatorname{Izm} \quad=\mathrm{Cc} *(33 / \mathrm{Zm}) \wedge 0.167$
$=0.256$
Lzm $=\mathrm{L} *(\mathrm{Zm} / \mathrm{33)}$ ^ Epsilon $=438.478$
$\mathrm{Q} \quad=\left(1 /\left(1+0.63 *((\mathrm{~B}+\mathrm{Ht}) / \mathrm{Lzm})^{\wedge} 0.63\right)\right)^{\wedge} 0.5 \quad=0.873$
G2 $=0.925 *((1+1.7 * 1 z m * 3.4 * Q) /(1+1.7 * 3.4 * 1 \mathrm{zm}))=0.855$
Gust Factor Used in Analysis
G Lessor Of G1 Or G2 $=0.850$

| Main Wind | Force Resisting System (MWFRS) Calculations for Solid Sign p | 29: |
| :---: | :---: | :---: |
| LF | $=$ Load Factor based upon STRENGTH Design | $=1.00$ |
| hs | = Overall height of structure | $=141.500 \mathrm{ft}$ |
| h | $=$ Mean Roof Height above grade | $=141.500 \mathrm{ft}$ |
| Kh |  | $=1.091$ |
| Kzt | $=$ Topographic Factor is 1 since no Topographic feature specifi | $=1.000$ |
| Kd | = Wind Directionality Factor per Table 26.6-1 | $=0.85$ |
| qh | $=\left(0.00256 * \mathrm{Kh} * \mathrm{Kzt} * \mathrm{Kd} * \mathrm{Ke} * \mathrm{~V}^{\wedge} 2\right) * \mathrm{LF}$ | $=37.10 \mathrm{psf}$ |
| MWFRS Pre | sures on Solid Sign per Fig 29.3-1: |  |
| R | $=$ Reduction factor to account for openings: (1-(1-e)^1.5) | $=1.000$ |
| Rc | $=$ Reduction factor for Case C not applicable since $\mathrm{s} / \mathrm{h}<=0.8$ | $=1.000$ |
| As | $=$ Gross Area of Sign: $\mathrm{B} * \mathrm{~s}$ | $=24.00 \mathrm{sq} \mathrm{ft}$ |
| $\mathrm{B} / \mathrm{s}$ | = Aspect Ratio: B / s | $=0.375$ |
| $s / h$ | = Clearance Ratio: s / h | $=0.057$ |
| Cf | $=$ Net Force Coefficient for Case A and B per Fig 29.3-1 | $=1.850$ |

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Page 2 of 2

```
Case A: Resultant force acts normal to face through geometric center
F = Design Wind force: qh * G * Cf * As * R = 1400 lb
Case B: Resultant force acts normal to face at a distance from the geometric
    center toward the windward edge equal to 0.2 times the average width
Dx = Force Offset from Center toward windward edge: 0.2 * B = 0.600 ft
F = Design Wind force: qh * G * Cf * As * R = 1400 lb
Case C: Since B/s < 2 then Case C need not be considered
```

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Project Title:
Engineer:
Project ID:
Project Descr:

## CODE REFERENCES

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

## Material Properties

| Analysis Method: Allowable Strength Design | Fy: Steel Yield: | 35.0 ksi |
| :--- | :--- | ---: |
| Beam Bracing: | Completely Unbraced | E: Modulus : |
| Bending Axis: | Major Axis Bending |  |



## Applied Loads

Service loads entered. Load Factors will be applied for calculations.
Beam self weight NOT internally calculated and added Load(s) for Span Number 1

Point Load: W=0.2790 k @ 2.50 ft

Load(s) for Span Number 2
Point Load: W = $0.2790 \mathrm{k} @ 3.0 \mathrm{ft}$

| DESIGN SUMMARY |  |  | Design OK |
| :---: | :---: | :---: | :---: |
| Maximum Bending Stress Ratio = | 0.210:1 M | ximum Shear Stress Ratio = | 0.018 : 1 |
| Section used for this span | Pipe2-1/2STD | Section used for this span | Pipe2-1/2STD |
| Ma: Applied | $0.502 \mathrm{k}-\mathrm{ft}$ | Va : Applied | 0.1841 k |
| Mn / Omega : Allowable | 2.393 k-ft | Vn/Omega : Allowable | 10.123 k |
| Load Combination | +D+0.60W+H | Load Combination | +D+0.60W+H |
| Location of maximum on span | 5.000 ft | Location of maximum on span | 2.520 ft |
| Span \# where maximum occurs | Span \# 1 | Span \# where maximum occurs | Span \# 1 |
| Maximum Deflection |  |  |  |
| Max Downward Transient Deflection | 0.221 in Ratio $=$ | $325>=180$. |  |
| Max Upward Transient Deflection | -0.028 in Ratio $=$ | $2,161>=180$. |  |
| Max Downward Total Deflection | 0.133 in Ratio $=$ | $543>=180$. |  |
| Max Upward Total Deflection | -0.017 in Ratio $=$ | $3602>=180$. |  |

## Maximum Forces \& Stresses for Load Combinations

| Load Combination Segment Length | Span \# | Max Stress Ratios |  | Summary of Moment Values |  |  |  |  |  |  | Summary of Shear Values |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | M | V | Mmax + | Mmax - | Ma Max | Mnx | Mnx/Omega | Cb | Rm | Va Max | Vnx | Vnx/Omega |
| +D+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+L+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+Lr+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+S+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+0.750Lr+0.750L+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+0.750L+0.750S+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |

Mike Plescia
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Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam
File: N-545.ec6
Lic. \# : KW-06000304
Software copyright ENERCALC, INC. 1983-2020, Build:12.20.3.25
PBA ENGINEERING, P.C.
DESCRIPTION: New Antenna Pipe Mast

| Load Combination |  | Max Stre | atios |  |  | mmary of | nt Valu |  |  |  | Sum | of She | ear Values |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment Length | Span \# | M | V | Mmax + | Mmax - | Ma Max | Mnx | Mnx/Omega | Cb | Rm | Va Max | Vnx | Vnx/Omega |
| Dsgn. L $=5.00 \mathrm{ft}$ | 1 | 0.210 | 0.018 |  | -0.50 | 0.50 | 4.00 | 2.39 | 2.72 | 1.00 | 0.18 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 | 0.210 | 0.017 |  | -0.50 | 0.50 | 4.00 | 2.39 | 1.00 | 1.00 | 0.17 | 16.91 | 10.12 |
| +D+0.750Lr+0.750L+0. |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 | 0.157 | 0.014 |  | -0.38 | 0.38 | 4.00 | 2.39 | 2.72 | 1.00 | 0.14 | 16.91 | 10.12 |
| Dsgn. L $=3.00 \mathrm{ft}$ | 2 | 0.157 | 0.012 |  | -0.38 | 0.38 | 4.00 | 2.39 | 1.00 | 1.00 | 0.13 | 16.91 | 10.12 |
| +D+0.750L+0.750S+0. |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=5.00 \mathrm{ft}$ | 1 | 0.157 | 0.014 |  | -0.38 | 0.38 | 4.00 | 2.39 | 2.72 | 1.00 | 0.14 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 | 0.157 | 0.012 |  | -0.38 | 0.38 | 4.00 | 2.39 | 1.00 | 1.00 | 0.13 | 16.91 | 10.12 |
| +0.60D $+0.60 \mathrm{~W}+0.60 \mathrm{H}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 | 0.210 | 0.018 |  | -0.50 | 0.50 | 4.00 | 2.39 | 2.72 | 1.00 | 0.18 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 | 0.210 | 0.017 |  | -0.50 | 0.50 | 4.00 | 2.39 | 1.00 | 1.00 | 0.17 | 16.91 | 10.12 |
| $+\mathrm{D}+0.70 \mathrm{E}+0.60 \mathrm{H}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+0.750L+0.750S+0. |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 5.00 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +0.60D+0.70E+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=5.00 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 3.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |

Overall Maximum Deflections

| Load Combination | Span | Max. "--" Defl | Location in Span | Load Combination | Max. "+" Defl | Location in Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 0.0000 | 0.000 | W Only | -0.0278 | 3.380 |
| W Only | 2 | 0.2211 | 3.000 |  | 0.0000 | 3.380 |
| Vertical Reactions |  |  | Support notation: Far left is \#1 |  | Values in KIPS |  |
| Load Combination | Support 1 | Support 2 | Support 3 |  |  |  |
| Overall MAXimum | -0.028 | 0.586 |  |  |  |  |
| Overall MINimum | -0.013 | 0.264 |  |  |  |  |
| +D+0.60W+H | -0.017 | 0.352 |  |  |  |  |
| +D+0.750Lr+0.750L+0.450W+H | -0.013 | 0.264 |  |  |  |  |
| +D+0.750L+0.750S+0.450W+H | -0.013 | 0.264 |  |  |  |  |
| $+0.60 \mathrm{D}+0.60 \mathrm{~W}+0.60 \mathrm{H}$ | -0.017 | 0.352 |  |  |  |  |
| W Only | -0.028 | 0.586 |  |  |  |  |
| H Only |  |  |  |  |  |  |

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Project Title:
Engineer:
Project ID:
Project Descr:

## CODE REFERENCES

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

## Material Properties

| Analysis Method: Allowable Strength Design | Fy: Steel Yield : | 46.0 ksi |
| :--- | :--- | ---: |
| Beam Bracing: | Completely Unbraced | E: Modulus : |
| Bending Axis : $\quad$ Major Axis Bending |  | $29,000.0 \mathrm{ksi}$ |



Applied Loads Service loads entered. Load Factors will be applied for calculations.
Beam self weight NOT internally calculated and added
Loads on all spans...
Uniform Load on ALL spans : W $=0.01855$ ksf, Tributary Width $=6.0 \mathrm{ft}$

Load(s) for Span Number 2
Point Load: W $=-0.0280 \mathrm{k} @ 0.250 \mathrm{ft}$

Point Load : W $=0.5860 \mathrm{k} @ 5.0 \mathrm{ft}$

| DESIGN SUMMARY |  |  | Design OK |
| :---: | :---: | :---: | :---: |
| Maximum Bending Stress Ratio = | 0.361: 1 Max | ximum Shear Stress Ratio = | 0.117:1 |
| Section used for this span | HSS4x4x1/4 | Section used for this span | HSS $4 \times 4 \times 1 / 4$ |
| Ma : Applied | 3.891 k-ft | Va: Applied | 2.970 k |
| Mn / Omega : Allowable | 10.765 k-ft | Vn/Omega : Allowable | 25.423 k |
| Load Combination | +D+0.60W+H | Load Combination | +D+0.60W+H |
| Location of maximum on span | 1.330 ft | Location of maximum on span | 1.330 ft |
| Span \# where maximum occurs | Span \# 1 | Span \# where maximum occurs | Span \# 1 |
| Maximum Deflection |  |  |  |
| Max Downward Transient Deflection | 0.963 in Ratio $=$ | $199>=180$. |  |
| Max Upward Transient Deflection | -0.006 in Ratio $=$ | $2,841>=180$. |  |
| Max Downward Total Deflection | 0.578 in Ratio $=$ | $332>=180$. |  |
| Max Upward Total Deflection | -0.003 in Ratio $=$ | $4736>=180$. |  |

Maximum Forces \& Stresses for Load Combinations

| Load Combination Segment Length | Span \# | Max Stress Ratios |  | Summary of Moment Values |  |  |  |  |  |  | Summary of Shear Values |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | M | V | Mmax + | Mmax - | Ma Max | Mnx | Mnx/Omega | Cb | Rm | Va Max | Vnx | Vnx/Omega |
| +D+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| Dsgn. L = 8.00 ft | 2 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| +D+L+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| Dsgn. L = 8.00 ft | 2 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| +D+Lr+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| Dsgn. L = 8.00 ft | 2 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| +D+S+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| Dsgn. L $=8.00 \mathrm{ft}$ | 2 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| +D+0.750Lr+0.750L+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| Dsgn. $\mathrm{L}=8.00 \mathrm{ft}$ | 2 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| +D+0.750L+0.750S+H |  |  |  |  |  |  |  |  |  |  |  |  |  |

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Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam
File: N-545.ec6
Lic. \# : KW-06000304
Software copyright ENERCALC, INC. 1983-2020, Build:12.20.3.25
PBA ENGINEERING, P.C.
DESCRIPTION: Existing Screen Verticals

| Load Combination |  | Max Stres | atios |  |  | mmary of | ent Valu |  |  |  | Sum | y of She | ear Values |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment Length | Span \# | M | V | Mmax + | Mmax - | Ma Max | Mnx | Mnx/Omega | Cb | Rm | Va Max | Vnx | Vnx/Omega |
| Dsgn. L $=1.33 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| Dsgn. L = 8.00 ft | 2 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| +D+0.60W+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 | 0.361 | 0.117 |  | -3.89 | 3.89 | 17.98 | 10.77 | 1.67 | 1.00 | 2.97 | 42.46 | 25.42 |
| Dsgn. L = 8.00 ft | 2 | 0.361 | 0.034 |  | -3.89 | 3.89 | 17.98 | 10.77 | 1.00 | 1.00 | 0.87 | 42.46 | 25.42 |
| +D+0.750Lr+0.750L+0. |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 | 0.271 | 0.088 |  | -2.92 | 2.92 | 17.98 | 10.77 | 1.67 | 1.00 | 2.23 | 42.46 | 25.42 |
| Dsgn. L = 8.00 ft | 2 | 0.271 | 0.026 |  | -2.92 | 2.92 | 17.98 | 10.77 | 1.00 | 1.00 | 0.65 | 42.46 | 25.42 |
| +D+0.750L+0.750S+0 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 | 0.271 | 0.088 |  | -2.92 | 2.92 | 17.98 | 10.77 | 1.67 | 1.00 | 2.23 | 42.46 | 25.42 |
| Dsgn. $\mathrm{L}=8.00 \mathrm{ft}$ | 2 | 0.271 | 0.026 |  | -2.92 | 2.92 | 17.98 | 10.77 | 1.00 | 1.00 | 0.65 | 42.46 | 25.42 |
| $+0.60 \mathrm{D}+0.60 \mathrm{~W}+0.60 \mathrm{H}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 | 0.361 | 0.117 |  | -3.89 | 3.89 | 17.98 | 10.77 | 1.67 | 1.00 | 2.97 | 42.46 | 25.42 |
| Dsgn. L $=8.00 \mathrm{ft}$ | 2 | 0.361 | 0.034 |  | -3.89 | 3.89 | 17.98 | 10.77 | 1.00 | 1.00 | 0.87 | 42.46 | 25.42 |
| +D+0.70E+0.60H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 1.33 ft | 1 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| Dsgn. L = 8.00 ft | 2 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| +D+0.750L+0.750S+0 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=1.33 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| Dsgn. L = 8.00 ft | 2 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| +0.60D+0.70E+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=1.33 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |
| Dsgn. L = 8.00 ft | 2 |  | 0.000 |  |  |  | 17.98 | 10.77 | 1.00 | 1.00 | -0.00 | 42.46 | 25.42 |

## Overall Maximum Deflections

| Load Combination | Span | Max. "-- Defl | Location in Span | Load Combination | Max. "+" Defl | Location in Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 0.0000 | 0.000 | W Only | -0.0056 | 0.771 |
| W Only | 2 | 0.9626 | 8.000 |  | 0.0000 | 0.771 |
| Vertical Reactions |  |  | Support notation: Far left is \#1 |  | Values in KIPS |  |
| Load Combination | Support 1 | Support 2 | Support 3 |  |  |  |
| Overall MAXimum | -4.802 | 6.398 |  |  |  |  |
| Overall MINimum | -2.161 | 2.879 |  |  |  |  |
| +D+0.60W+H | -2.881 | 3.839 |  |  |  |  |
| +D+0.750Lr+0.750L+0.450W+H | -2.161 | 2.879 |  |  |  |  |
| +D+0.750L+0.750S+0.450W+H | -2.161 | 2.879 |  |  |  |  |
| $+0.60 \mathrm{D}+0.60 \mathrm{~W}+0.60 \mathrm{H}$ | -2.881 | 3.839 |  |  |  |  |
| W Only | -4.802 | 6.398 |  |  |  |  |
| H Only |  |  |  |  |  |  |

Mike Plescia
PBA Engineering, P.C.
12 Kulick Road

Project Title:
Engineer:
Project ID:
Project Descr:

DESCRIPTION: Facade Mount Vertical

## CODE REFERENCES

Calculations per AISC 360-16, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

## Material Properties

| Analysis Method: Allowable Strength Design | Fy : Steel Yield : | 35.0 ksi |
| :--- | :--- | ---: |
| Beam Bracing: | Completely Unbraced | E: Modulus : |
| Bending Axis : | Major Axis Bending |  |



## Applied Loads

Service loads entered. Load Factors will be applied for calculations.
Beam self weight NOT internally calculated and added Load(s) for Span Number 2

Point Load: W=0.10 k @ 2.0 ft

| DESIGN SUMMARY |  |  | Design OK |
| :---: | :---: | :---: | :---: |
| Maximum Bending Stress Ratio | 0.050 : 1 | Maximum Shear Stress Ratio = | 0.047 : 1 |
| Section used for this span | Pipe2-1/2STD | Section used for this span | Pipe2-1/2STD |
| Ma : Applied | $0.120 \mathrm{k}-\mathrm{ft}$ | Va : Applied | 0.480 k |
| Mn / Omega : Allowable | 2.393 k-ft | Vn/Omega : Allowable | 10.123 k |
| Load Combination | +D $+0.60 \mathrm{~W}+\mathrm{H}$ | Load Combination | +D+0.60W+H |
| Location of maximum on span | 0.250ft | Location of maximum on span | 0.000 ft |
| Span \# where maximum occurs | Span \# 1 | Span \# where maximum occurs | Span \# 1 |
| Maximum Deflection |  |  |  |
| Max Downward Transient Deflection | 0.012 in | = $3,3,903>=180$. |  |
| Max Upward Transient Deflection | 0.000 in | $0=0<180.0$ |  |
| Max Downward Total Deflection | 0.007 in | $0=6507>=180$. |  |
| Max Upward Total Deflection | -0.000 in | = $150950>=180$. |  |

## Maximum Forces \& Stresses for Load Combinations

| Load Combination Segment Length | Span \# | Max Stress Ratios |  | Summary of Moment Values |  |  |  |  |  |  | Summary of Shear Values |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | M | V | Mmax + | Mmax - | Ma Max | Mnx | Mnx/Omega | Cb | Rm | Va Max | Vnx | Vnx/Omega |
| +D+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 0.25 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 2.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+L+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 0.25 ft | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 2.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+Lr+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=0.25 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 2.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+S+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=0.25 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. $\mathrm{L}=2.00 \mathrm{ft}$ | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+0.750Lr+0.750L+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=0.25 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. $\mathrm{L}=2.00 \mathrm{ft}$ | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+0.750L+0.750S+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=0.25 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 2.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+0.60W+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=0.25 \mathrm{ft}$ | 1 | 0.050 | 0.047 |  | -0.12 | 0.12 | 4.00 | 2.39 | 1.67 | 1.00 | 0.48 | 16.91 | 10.12 |
| Dsgn. L $=2.00 \mathrm{ft}$ | 2 | 0.050 | 0.006 |  | -0.12 | 0.12 | 4.00 | 2.39 | 1.00 | 1.00 | 0.06 | 16.91 | 10.12 |
| +D+0.750Lr+0.750L+0.450W+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L = 0.25 ft | 1 | 0.038 | 0.036 |  | -0.09 | 0.09 | 4.00 | 2.39 | 1.67 | 1.00 | 0.36 | 16.91 | 10.12 |

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Project Title:
Engineer:
Project ID:
Project Descr:

Steel Beam
File: N-545.ec6
Lic. \# : KW-06000304
Software copyright ENERCALC, INC. 1983-2020, Build:12.20.3.25
PBA ENGINEERING, P.C.
DESCRIPTION: Facade Mount Vertical

| Load Combination |  | Max Stre | atios |  |  | mmary of | nt Valu |  |  |  | Summ | y of She | ear Values |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment Length | Span \# | M | V | Mmax + | Mmax - | Ma Max | Mnx | Mnx/Omega | Cb | Rm | Va Max | Vnx | Vnx/Omega |
| Dsgn. L = 2.00 ft | 2 | 0.038 | 0.004 |  | -0.09 | 0.09 | 4.00 | 2.39 | 1.00 | 1.00 | 0.05 | 16.91 | 10.12 |
| +D+0.750L+0.750S+0. |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. $\mathrm{L}=0.25 \mathrm{ft}$ | 1 | 0.038 | 0.036 |  | -0.09 | 0.09 | 4.00 | 2.39 | 1.67 | 1.00 | 0.36 | 16.91 | 10.12 |
| Dsgn. L = 2.00 ft | 2 | 0.038 | 0.004 |  | -0.09 | 0.09 | 4.00 | 2.39 | 1.00 | 1.00 | 0.05 | 16.91 | 10.12 |
| $+0.60 \mathrm{D}+0.60 \mathrm{~W}+0.60 \mathrm{H}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=0.25 \mathrm{ft}$ | 1 | 0.050 | 0.047 |  | -0.12 | 0.12 | 4.00 | 2.39 | 1.67 | 1.00 | 0.48 | 16.91 | 10.12 |
| Dsgn. L = 2.00 ft | 2 | 0.050 | 0.006 |  | -0.12 | 0.12 | 4.00 | 2.39 | 1.00 | 1.00 | 0.06 | 16.91 | 10.12 |
| $+\mathrm{D}+0.70 \mathrm{E}+0.60 \mathrm{H}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=0.25 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 2.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +D+0.750L+0.750S+0. |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=0.25 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 2.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| +0.60D+0.70E+H |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Dsgn. L $=0.25 \mathrm{ft}$ | 1 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |
| Dsgn. L = 2.00 ft | 2 |  | 0.000 |  |  |  | 4.00 | 2.39 | 1.00 | 1.00 | -0.00 | 16.91 | 10.12 |

Overall Maximum Deflections

| Load Combination | Span | Max. "-" Defl | Location in Span | Load Combination | Max. "+" Defl | Location in Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 0.0000 | 0.000 | W Only | -0.0000 | 0.145 |
| W Only | 2 | 0.0123 | 2.000 |  | 0.0000 | 0.145 |
| Vertical Reactions |  |  | Support notation : Far left is \#1 |  | Values in KIPS |  |
| Load Combination | Support 1 | Support 2 | Support 3 |  |  |  |
| Overall MAXimum | -0.800 | 0.900 |  |  |  |  |
| Overall MINimum | -0.360 | 0.405 |  |  |  |  |
| +D+0.60W+H | -0.480 | 0.540 |  |  |  |  |
| +D+0.750Lr+0.750L+0.450W+H | -0.360 | 0.405 |  |  |  |  |
| +D+0.750L+0.750S+0.450W+H | -0.360 | 0.405 |  |  |  |  |
| $+0.60 \mathrm{D}+0.60 \mathrm{~W}+0.60 \mathrm{H}$ | -0.480 | 0.540 |  |  |  |  |
| W Only | -0.800 | 0.900 |  |  |  |  |
| H Only |  |  |  |  |  |  |

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| Company: |  | Page: | 1 |
| :--- | :--- | :--- | :--- |
| Address: | Specifier: |  |  |
| Phone I Fax: | I | E-Mail: |  |
| Design: | Date: | $6 / 12 / 2020$ |  |
| Fastening point |  |  |  |

Specifier's comments:

## 1 Input data

## Anchor type and diameter:

Item number:

Effective embedment depth:
Material:
Evaluation Service Report:
Issued I Valid:
Proof:
Stand-off installation:
Anchor plate ${ }^{R}$ :
Profile:
Base material:

Installation:
Seismic loads

HY 270 + threaded rod 5.8 1/2, HIT-SC 18x50
385422 HAS 5.8 1/2"x3-1/8" (element) / 2194247 HIT-HY 270 (adhesive) / 360485 HIT-SC 18x50 (sieve sleeve)
$h_{\text {ef }}=2.000 \mathrm{in}$.
5.8

ESR-4143
1/1/2020 | 1/1/2021
Design Method ASD Masonry
$e_{b}=0.000$ in. (no stand-off); $t=0.250$ in.
$\mathrm{I}_{\mathrm{x}} \times \mathrm{I}_{\mathrm{y}} \times \mathrm{t}=4.000 \mathrm{in} . \times 8.000 \mathrm{in}$. $\times 0.250 \mathrm{in}$.; (Recommended plate thickness: not calculated)
no profile
Hollow CMU, L x W x H: 16.000 in. x 8.000 in. x 8.000 in.;
Joints: vertical: 0.375 in.; horizontal: 0.375 in.
Base material temperature: $68{ }^{\circ} \mathrm{F}$
Face installation
no
${ }^{R}$ - The anchor calculation is based on a rigid anchor plate assumption.

## Geometry [in.]



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| :--- | :--- | :--- | :--- |
| Address: | Specifier: | 2 |  |
| Phone I Fax: | M | E-Mail: |  |
| Design: | Date: | $6 / 12 / 2020$ |  |
| Fastening point: | Masonry - Jun 12, 2020 |  |  |

## Geometry [in.] \& Loading [lb, in.lb]


1.1 Design results

| Case | Description | Forces $[\mathrm{lb}] /$ Moments [in.lb] | Seismic | Max. Util. Anchor [\%] |
| :---: | :--- | :---: | :---: | :---: |
| 1 | Combination 1 | $N=300 ; V_{x}=0 ; V_{y}=0 ;$ | no | 46 |
|  |  | $M_{x}=0 ; M_{y}=0 ; M_{z}=0 ;$ |  |  |

## 2 Load case/Resulting anchor forces

Load case: Service loads

## Anchor reactions [lb]

Tension force: (+Tension, -Compression)

| Anchor | Tension force | Shear force | Shear force x | Shear force y |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 150 | 0 | 0 | 0 |
| 2 | 150 | 0 | 0 | 0 |

max. compressive strain:
max. compressive stress:
resulting tension force in $(x / y)=(0.000 / 0.000)$ :

- [psi]

resulting compression force in $(x / y)=(0.000 / 0.000): 0[\mathrm{lb}]$

Anchor forces are calculated based on the assumption of a rigid anchor plate.


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| Company: |  | Page: | 3 |
| :--- | :--- | :--- | :--- |
| Address: | Specifier: |  |  |
| Phone I Fax: | I | E-Mail: |  |
| Design: | Masonry - Jun 12, 2020 |  | $6 / 12 / 2020$ |
| Fastening point: |  |  |  |

## 3 Tension load (Most utilized anchor 1)

|  | Load $\mathbf{P}_{\mathbf{s}}[\mathrm{lb}]$ | Capacity $\mathbf{P}_{\mathbf{t}}[\mathrm{lb}]$ | Utilization $\boldsymbol{\beta}_{\mathbf{P}}=\mathbf{P}_{\mathbf{s}} / \mathbf{P}_{\mathbf{t}}[\%]$ | Status |
| :--- | :---: | :---: | :---: | :---: |
| Steel strength | 150 | 4,700 | 4 |  |
| Bond strength | 150 | 330 | 46 |  |

### 3.1 Steel strength

$P_{t, s}=E S R$ Value refer to ICC-ES ESR-4143
$P_{t, \mathrm{~s}} \geq P_{\mathrm{s}}$

Results

| $\mathrm{P}_{\mathrm{t}, \mathrm{s}}[\mathrm{lb}]$ | $\mathrm{P}_{\mathrm{s}}[\mathrm{lb}]$ |
| :---: | :---: |
| 4,700 | 150 |

3.2 Bond strength
$P_{t, b, B a s e}=E S R$ Value refer to ICC-ES ESR-4143
$P_{t, b}=P_{t, b, B a s e} \cdot f_{\text {red }, \mathrm{E}} \cdot f_{\text {red }, \mathrm{s}} \cdot f_{\text {red, Temp }}$
$P_{t, b} \quad \geq P_{s}$

Variables

| $\mathrm{c}_{\min }[\mathrm{in}]$. | $\mathrm{c}_{\mathrm{cr}}$ [in.] | $\mathrm{s}_{\min }[\mathrm{in}]$. | $\mathrm{s}_{\mathrm{cr}}$ [in.] | Temperature [ $\left.{ }^{\circ} \mathrm{F}\right]$ |
| :---: | :---: | :---: | :---: | :---: |
| 4.000 | - | 4.000 | - | 68 |

Results

| $\mathrm{P}_{\mathrm{t}, \mathrm{b}}[\mathrm{lb}]$ | $\mathrm{P}_{\mathrm{t}, \mathrm{b}, \text { Base }}[\mathrm{lb}]$ | $\mathrm{P}_{\mathrm{s}}[\mathrm{lb}]$ | $\mathrm{f}_{\text {red, } \mathrm{E}}$ | $\mathrm{f}_{\text {red,S }}$ | $\mathrm{f}_{\text {red,Temp }}$ | $\mathrm{f}_{\text {red,TwolnOne }}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 330 | 330 | 150 | 1.000 | 1.000 | 1.000 |  |

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| Company: |  | Page: | 4 |
| :--- | :--- | :--- | :--- |
| Address: | Specifier: | 4 |  |
| Phone I Fax: | I | E-Mail: |  |
| Design: | Dasonry - Jun 12, 2020 | Date: | $6 / 12 / 2020$ |

Fastening point:

## 4 Shear load (Most utilized anchor 1)

|  | Load $V_{s}[\mathrm{lb}]$ | Capacity $V_{t}[\mathrm{lb}]$ | Utilization $\beta_{v}=V_{s} / V_{t}[\%]$ | Status |
| :--- | :---: | :---: | :---: | :---: |
| Overall strength | $\mathrm{N} / \mathrm{A}$ | $\mathrm{N} / \mathrm{A}$ | $\mathrm{N} / \mathrm{A}$ | $\mathrm{N} / \mathrm{A}$ |

## 5 Warnings

- The anchor design methods in PROFIS Engineering require rigid anchor plates per current regulations (AS 5216:2018, ETAG 001/Annex C, EOTA TR029 etc.). This means load re-distribution on the anchors due to elastic deformations of the anchor plate are not considered - the anchor plate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the design loading. PROFIS Engineering calculates the minimum required anchor plate thickness with CBFEM to limit the stress of the anchor plate based on the assumptions explained above. The proof if the rigid anchor plate assumption is valid is not carried out by PROFIS Engineering. Input data and results must be checked for agreement with the existing conditions and for plausibility!
- Refer to the manufacturer's product literature for cleaning and installation instructions.
- For additional information about ACl 318 strength design provisions, please go to https://submittals.us.hilti.com/PROFISAnchorDesignGuide/
- The min. sizes of the bricks, the masonry compressive strength, the type / strength of the mortar and the grout (in case of fully grouted CMU walls) has to fulfill the requirements given in the relevant ESR-approval or in the PTG.
- Only the local load transfer from the anchor(s) to the wall is considered, a further load transfer in the wall is not covered by PROFIS!
- Wall is assumed as being perfectly aligned vertically - checking required(!): Noncompliance can lead to significantly different distribution of forces and higher tension loads than those calculated by PROFIS. Masonry wall must not have any damages (neither visible nor not visible)! While installation, the positioning of the anchors needs to be maintained as in the design phase i.e. either relative to the brick or relative to the mortar joints.
- The effect of the joints on the compressive stress distribution on the plate / bricks was not taken into consideration.
- If no significant resistance is felt over the entire depth of the hole when drilling (e.g. in unfilled butt joints), the anchor should not be set at this position or the area should be assessed and reinforced. Hilti recommends the anchoring in masonry always with sieve sleeve. Anchors can only be installed without sieve sleeves in solid bricks when it is guaranteed that it has not any hole or void.
- The accessories and installation remarks listed on this report are for the information of the user only. In any case, the instructions for use provided with the product have to be followed to ensure a proper installation.
- The compliance with current standards (e.g. 2015, 2012, 2009 and 2006 IBC) is the responsibility of the user.
- Drilling method (hammer, rotary) to be in accordance with the approval!
- Masonry needs to be built in a regular way in accordance with state-of the art guidelines!
- Warnings/Notes - OST in Masonry HNA!


## Fastening meets the design criteria!

Hilti PROFIS Engineering 3.0.61
www.hilti.com

| Company: |  | Page: | 5 |
| :--- | :--- | :--- | :--- |
| Address: | Specifier: |  |  |
| Phone I Fax: | I | E-Mail: |  |
| Design: | Dasonry - Jun 12, 2020 |  | $6 / 12 / 2020$ |
| Fastening point: |  |  |  |

## 6 Installation data

Profile: no profile

Hole diameter in the fixture: $\mathrm{d}_{\mathrm{f}}=0.563$ in
Plate thickness (input): 0.250 in.

Drilling method: Drilled in rotary mode
Anchor type and diameter: HY 270 + threaded rod 5.8 1/2, HIT-SC 18x50
Item number: 385422 HAS 5.8 1/2"x3-1/8" (element) / 2194247 HIT-HY 270 (adhesive) / 360485 HIT-SC 18x50 (sieve sleeve)

Installation torque: 54 in.lb
Hole diameter in the base material: 0.688 in.
Hole depth in the base material: 2.375 in.
Minimum thickness of the base material: 7.625 in

Hilti HIT-V threaded rod with HIT-HY 270 injection mortar and 1 HIT-SC $18 x 50$ sieve sleeve(s) with 2 in embedment h_ef, $1 / 2$, Steel galvanized, Rotary drilled installation per ESR-4143


## Coordinates Anchor [in.]

| Anchor | $\mathbf{x}$ | $\mathbf{y}$ | $\mathbf{c}_{-\mathbf{x}}$ | $\mathbf{c}_{+\mathbf{x}}$ | $\mathbf{c}_{-\mathbf{y}}$ | $\mathbf{c}_{+\mathbf{y}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 0.000 | -2.000 | 50.000 | 10.000 | 48.000 | 8.000 |
| 2 | 0.000 | 2.000 | 50.000 | 10.000 | 52.000 | 4.000 |

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| Company: |  | Page: | 6 |
| :--- | :--- | :--- | :--- |
| Address: | Specifier: |  |  |
| Phone I Fax: | I | E-Mail: |  |
| Design: | Dasonry - Jun 12, 2020 |  | $6 / 12 / 2020$ |
| Fastening point: |  |  |  |

## 7 Remarks; Your Cooperation Duties

- Any and all information and data contained in the Software concern solely the use of Hilti products and are based on the principles, formulas and security regulations in accordance with Hilti's technical directions and operating, mounting and assembly instructions, etc., that must be strictly complied with by the user. All figures contained therein are average figures, and therefore use-specific tests are to be conducted prior to using the relevant Hilti product. The results of the calculations carried out by means of the Software are based essentially on the data you put in. Therefore, you bear the sole responsibility for the absence of errors, the completeness and the relevance of the data to be put in by you. Moreover, you bear sole responsibility for having the results of the calculation checked and cleared by an expert, particularly with regard to compliance with applicable norms and permits, prior to using them for your specific facility. The Software serves only as an aid to interpret norms and permits without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application.
- You must take all necessary and reasonable steps to prevent or limit damage caused by the Software. In particular, you must arrange for the regular backup of programs and data and, if applicable, carry out the updates of the Software offered by Hilti on a regular basis. If you do not use the AutoUpdate function of the Software, you must ensure that you are using the current and thus up-to-date version of the Software in each case by carrying out manual updates via the Hilti Website. Hilti will not be liable for consequences, such as the recovery of lost or damaged data or programs, arising from a culpable breach of duty by you.


## ATTACHMENT 5

## Photo Documentation \& Simulations



## VISUAL ASSESSMENT \& PHOTO-SIMULATIONS

Cellco Partnership, d/b/a Verizon Wireless is seeking approval for the modification of an existing wireless communications facility (the "Facility") at 54 Meadow Street in New Haven, Connecticut. At the request of Verizon Wireless, All-Points Technology Corporation, P.C. ("APT") completed this assessment and prepared computer-generated photo-simulations depicting the Facility.

## Project Undertaking

The proposed modifications to the Facility would take place on the rooftop of an existing multistory building (the "Host Building"). Currently, Verizon Wireless maintains twelve (12) antennas and various appurtenances on the building's penthouse. Verizon Wireless plans to replace nine (9) antennas, leave three (3) existing antennas as is, and install three (3) new antennas for a total of fifteen (15) antennas. Of the nine (9) antennas being replaced, seven (7) will be relocated to positions on the building façade, penthouse façade, and an existing mechanical screen wall on the rooftop.

The building rooftop extends to a height of approximately $129^{\prime} 2$ " above ground level ("AGL"). A parapet wall extends to a height of approximately $131^{\prime} 6^{\prime \prime}$ AGL. The penthouse rooftop extends to a height of approximately $155^{\prime}$ AGL with a parapet extending to approximately $157^{\prime} 2^{\prime \prime}$ AGL. Please refer to the Site Drawings prepared by On Air Engineering, LLC, Rev. 2 dated July 13, 2020, and provided under separate cover, for details regarding the proposed installation.

## Field Reconnaissance

APT completed field reconnaissance in the vicinity of the Facility to record existing conditions, inventory visible and non-visible locations, and provide photographic documentation from publicly accessible areas. The field reconnaissance was completed on July 8, 2020. Weather conditions were favorable for the in-field activity with partly cloudy skies.

## Photographic Documentation and Simulations

During the field reconnaissance, APT obtained photo-documentation of representative locations where the Host Building was visible. At each photo location, the geographic coordinates of the camera's position were logged using global positioning system ("GPS") technology. Photographs were taken with a Canon EOS 6D digital camera body ${ }^{1}$ and Canon EF 24 to 105 millimeter ("mm") zoom lens. APT typically uses a standard focal length of 50 mm to present a consistent

[^1]field of view. On occasion, photos are taken at lower focal lengths to provide a greater depth of field and to provide context to the scene by including surrounding features within the photograph. During this evaluation, one (1) photograph was taken at a 24 mm focal length as noted in the table (Table 1 - Photo Locations) on the following pages.

Photographic simulations were generated to portray scaled renderings of the proposed Facility from all eight (8) locations presented herein where the Facility may be recognizable. Using field data, Site plan information and 3-dimensional (3D) modeling software, spatially referenced models of the Facility were generated and merged. The geographic coordinates obtained in the field for the photograph locations were incorporated into the model to produce virtual camera positions within the spatial 3D model. Photo-simulations were then created using a combination of renderings generated in the 3D model and photo-rendering software programs, which were ultimately composited and merged with the existing conditions photographs (using Photoshop image editing software). The scale of the subjects in the photograph (the Host Building) and the corresponding simulation (the Facility) is proportional to their surroundings.

For presentation purposes in this report, the photographs were produced in an approximate 7 inch by 10.5 -inch format. When reproducing the images in this format size, we believe it is important to present the largest view while providing key contextual landscape elements (existing developments, street signs, utility poles, etc.) so that the viewer can determine the proportionate scale of each object within the scene. Photographs presented in the attachment at the end of this report include documentation of existing conditions, identification of antennas proposed for removal/relocation, identification of relocated/new antennas, and photosimulations of the modified Facility. The photo-simulations are intended to provide the reader with a general understanding of the different view characteristics associated with the Facility from various locations. Photographs were taken from publicly-accessible areas and unobstructed view lines were chosen wherever possible.

The table on the following page summarizes the photographs and simulations presented in the attachment to this report, and includes a description of each location, view orientation, and distance from where the photo was taken relative to the proposed Facility. The photo locations are depicted on the photolog provided as an attachment to this report.

Table 1 - Photo Locations

| Photo | Location | Orientation | Distance <br> to Site |
| :---: | :---: | :---: | :---: |
| 1 | Orange Street at George Street | South | $\pm 0.23$ Mile |
| 2 | Church Street South | Southeast | $\pm 0.18$ Mile |
| 3 | Cedar Street | Northeast | $\pm 0.32$ Mile |
| 4 | Union Avenue | Northeast | $\pm 0.22$ Mile |
| 5 | Church Street | Northwest | $\pm 0.44$ Mile |
| 6 | Food Terminal Plaza | Northwest | $\pm 0.41$ Mile |
| 7 | Brewery Street | Northwest | $\pm 0.32$ Mile |
| 8 | Union Avenue | West | $\pm 309$ Feet |
|  |  |  |  |

## Conclusions

As presented on the attached photo-simulations, views of the Facility would change slightly with the proposed modifications. The Host Building currently has multiple antennas and equipment mounted on the penthouse façade. The modifications as proposed by Verizon Wireless do not increase visibility of the Facility, nor do they detract from the general characteristics of the Host Building, as is.

## Limitations

This analysis may not account for all visible locations, as it is based on the combination of computer modeling, incorporating aerial photographs, and in-field observations from publiclyaccessible locations. No access to private properties was provided to APT personnel. This analysis does not claim to depict the only areas, or all locations, where visibility may occur; it is intended to provide a representation of those areas where the Facility is likely to be seen.

The photo-simulations provide a representation of the Facility under similar settings as those encountered during the field review and reconnaissance. Views of the Facility can change throughout the seasons and the time of day, and are dependent on weather and other atmospheric conditions (e.g., haze, fog, clouds); the location, angle and intensity of the sun; and the specific viewer location. Weather conditions on the day of the field review included variable winds and sunny skies.

## ATTACHMENTS



Legend
$\bullet$ Site
Photographic Location











TECHNOLOGY CORPORATIO
verizon


TECHNOLOGY CORPORATIO
verizon


TECHNOLOGY CORPORATIO
verizon







verizon ${ }^{\checkmark}$


verizon ${ }^{\checkmark}$

verizon ${ }^{\checkmark}$

verizon








## ATTACHMENT 6

Site Name: New Haven CT
Cumulative Power Density

| Operator | Operating <br> Frequency | Number <br> of Trans. | ERP Per <br> Trans. | Total <br> ERP | Distance <br> to Target | Calculated <br> Power <br> Density | Maximum <br> Permissible <br> Exposure | Fraction <br> of MPE |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $(\mathbf{M H z})$ |  | (watts) | (watts) | (feet) | $(\mathbf{m W / c m} \mathbf{\wedge})$ | $(\mathbf{m W / c m} \mathbf{2})$ | $(\%)$ |
| VZW 700 | 746 | 4 | 556 | 2,224 | 136 | 0.0432 | 0.497333333 | $8.69 \%$ |
| VZW Cellular | 869 | 2 | 354 | 708 | 155 | 0.0106 | 0.579333333 | $1.83 \%$ |
| VZW Cellular | 880 | 4 | 556 | 2,224 | 136 | 0.0432 | 0.586666667 | $7.37 \%$ |
| VZW PCS | 1,970 | 4 | 1,303 | 5,213 | 136 | 0.1013 | 1.0 | $10.13 \%$ |
| VZW AWS | 2,145 | 4 | 1,396 | 5,585 | 136 | 0.1086 | 1.0 | $10.86 \%$ |
| VZW CBRS | 3,550 | 4 | 51 | 204 | 136 | 0.0040 | 1.0 | $0.40 \%$ |
| VZW 28GHz | 27,600 | 1 | 610 | 610 | 136 | 0.0119 | 1.0 | $1.19 \%$ |

Total Percentage of Maximum Permissible Exposure
40.47\%
*Guidelines adopted by the FCC on August 1, 1996, 47 CFR Section 1.13101 based on NCRP Report 86, 1986 and generally on ANSI/IEEE C95.1-1992
$\mathrm{MHz}=\mathrm{Megahertz}$
$\mathrm{mW} / \mathrm{cm}^{\wedge} 2$ = milliwatts per square centimeter
ERP = Effective Radiated Power

Absolute worst case maximum values used, including the following assumptions:

1. closest accessible point is distance from antenna to base of pole;
2. continuous transmission from all available channels at full power for indefinite time period; and,
3. all RF energy is assumed to be directed solely to the base of the pole.

## ATTACHMENT 7

## AIRSPACE®

## Federal Aviation Regulations Part 77 Sub-Part C Obstruction Analysis Report

| Verizon Wireless Ziad Cheiban |  |  |
| :---: | :---: | :---: |
|  |  |  |
| 20 Alexander Drive |  |  |
| Wallingford, CT 06492 |  |  |
| E-mail: ziad.cheiban@verizonwireless.com Phone: 8604717860 Fax: |  |  |
| Site Identification: MCM CT-520 <br> Nearest City: New Haven, CT |  |  |
|  |  |  |
| Site Information (Coordinate Datum - NAD83) |  |  |
| Latitude: | 410-17' - 59.54" | Decimal Degrees: $41.2998722222222^{\circ}$ |
| Longitude: | 72 ${ }^{\circ}-55^{\prime}-35.57{ }^{\prime \prime}$ | Decimal Degrees: $72.9265472222222^{\circ}$ |
| Ground Elevatir | ation: 11 | et AMSL |
| Structure He | eight: 160 | t AGL |
| Overall Heig |  | et AMSL |

FAA Number: Null
Airspace Study \#: 2020-APS-3708-OE
Analyzed on: 5/22/2020. Using Airspace® 20.5.463. Airspace® Data Date: 5/15/2020
This Airspace Analysis was completed under all obstacle evaluation rules specified in Federal Aviation Regulations (FAR) Part 77 sub-Part C.

Approved,

Bella B Harris, Airspace Technician


#### Abstract

AIRSPACE®

Site ID Number: MCM_CT-520

\section*{AERONAUTICAL RECOMMENDATIONS}

Notice to the FAA is not required at the analyzed height and location. TERPS® analysis has been completed for the proposed site. The maximum allowable height identified is 363 feet AMSL based upon HVN VFR Traffic Pattern Airspace.


The proposed structure does not penetrate obstruction standards. An aeronautical analysis by the Federal Aviation Administration would likely find no adverse aeronautical impact. An extended study will not be required. The maximum not to exceed height to avoid an extended study by the FAA is 213 feet AMSL based upon HVN FAR 77.17(a)(2) VFR Transitional Surface.

Marking and Lighting are not normally required for structures 200 feet or less. However, it may become a requirement based upon the outcome of the aeronautical study conducted by the FAA. It will then become part of the determination and a requirement of the determination.

No adverse impact to low altitude federal airways are identified.
No impact to VFR Traffic Pattern Airspace.
No Potential FCC Licensed AM Broadcast Station interference identified.
No impact to an Air Navigation Facility has been identified.

## AIRSPACE®

## Site ID Number: <br> MCM_CT-520

## LANDING FACILITY INFORMATION

The nearest public use landing facility to the proposed location is:
TWEED-NEW HAVE (Ident: HVN)
The distance to the nearest runway of this landing facility is 14983 feet or 2.8 statute miles. The true bearing is $140.12^{\circ}$ to this landing facility.

Private landing facilities are exempt from review by the FAA under FAR Part 77. However, locating near a private landing facility may affect aircraft operations during take-off and landing.

The nearest private landing facility is: 1CT2: YALE
The proposed structure is located 3220 feet or .6 statute miles.
The true bearing to this landing facility is 301 degrees.
The proposed structure is within 3 nautical miles ( 3.45 statute miles) of a private landing facility. This landing facility and supporters are likely to resist this proposal during the local zoning board hearing.

## FAA NOTICE REQUIREMENTS

Notice to the FAA is not required because the proposed structure

1) is less than 200 feet above ground level [FAR Part 77.9(a)].
2) does not exceed runway slope criteria [FAR Part 77.9(b)].

3 ) is not a traverse way (road) [FAR 77.9(c)].
4) is not within a protected instrument procedure area [FAR 77.9 IFR].
5) is not on airport property [FAR 77.9(d)].

6 ) is not near an air navigation facility [FAR 77.9 IFR].

## AIRSPACE®

## Site ID Number: MCM_CT-520

## AERONAUTICAL IMPACT

## FAR Part 77 Subpart-C Obstruction Standards

The proposed structure would not violate or exceed obstruction standards as defined by FAR Part 77.17(a)(1), 77.17(a)(2) and 77.19.

## Terminal Instrument Procedure Standards - FAR Part 77.17(a)(3)

No adverse impact with a US Terminal Approach or Departure Procedure has been identified.

## Minimum Obstacle Clearance Altitude (MOCA) - FAR Part 77.17(a)(4)

The proposed structure is not located within a low altitude airway area or will not impact aircraft using any airway.

## VFR Traffic Pattern Airspace

The proposed structure is not located within a VFR Traffic Pattern Airspace or is below the allowable height. It will not impact aircraft circling to land.

## FCC Licensed AM Broadcast Station Proof-of-Performance

The proposed structure is not located within the specified range of an FCC Licensed AM radio and will not require Proof-of-Performance analysis.

[^2]```
*****************************************************
* Summary Report: Alteration Of Existing Structure *
* Non-Antenna Structure *
******************************************************
Airspace User:
File: 2020-APS-3708-OE
Location: New Haven, CT
Latitude: 410-17'-59.54" Longitude: 72o.55'-35.57"
SITE ELEVATION AMSL...... 11 ft.
STRUCTURE HEIGHT......... 160 ft.
OVERALL HEIGHT AMSL...... 171 ft .
NOTICE CRITERIA
FAR 77.9(a): NNR (DNE 200 ft AGL)
FAR 77.9(b): NR (Exceeds Notice Slope, Maximum: 162 ft.)
NNR See below regarding Notice Criteria Exemption under 77.9(e) (4).
FAR 77.9(c): NNR (Not a Traverse Way)
FAR 77.9: NNR FAR 77.9 IFR Straight-In Notice Criteria for HVN
FAR 77.9: NNR FAR 77.9 IFR Straight-In Notice Criteria for BDR
FAR 77.9(d): NNR (Off Airport Construction)
NR = Notice Required
NNR \(=\) Notice Not Required
PNR = Possible Notice Required (depends upon actual IFR procedure) For new construction review Air Navigation Facilities at bottom of this report.
If the proposed construction is an alteration to an existing structure, notice requirements may be superceded by the item exemptions listed below.
The location and analysis were based upon an existing structure. However, no existing aeronautical study number was identified. If the 'existing' structure penetrates an obstruction surface defined by CFR 77.17, 77.19, 77.21 or 77.23 (see below) it is strongly recommended the FAA be notified of the 'existing' structure to determine obstruction marking or lighting requirements. It is not uncommon for the FAA to issue a Determination of No Hazard (DNH) for an existing structure and modify the airspace to accommodate the structure, should that be required. If the FAA issues a DNH enter the aeronautical study number (ASN) in the space provided on the Airspace Analysis Window Form and re-run Airspace.
No frequencies were identified in this alteration are included in the FAA's Co-Location Policy published in the Federal Register November 15, 2007. Therefore, application of the Co-Location Policy notice exemption rule can not be applied.
Title 14 CFR Part 77.9(e), Notice Criteria Exception:
The location and analysis were based upon an existing structure with the alteration limited to the addition of an antenna with a height no greater than 20 feet. Title 14 CFR Part 77.9(e)(4) exempts the requirement for notice to the FAA; "Any antenna structure of 20 feet or less in height except one that would increase the height of another antenna structure." If the addition of an antenna of 20 feet or less to an existing structure increase the height of the structure to exceed 200 feet AGL or penetrate an obstruction surface defined by Title 14 CFR \(77.17,77.19,77.21\) or 77.23 notice is recommended. This will allow the FAA to determine the level of obstruction lighting required and any aeronautical impacts, if any, to aircraft operations. Notice of an existing structure almost always receives a No Hazard Determination. Please see Summary Report below plus the Airport and Part 77 Reports for application of the above listed CFRs.
```



| CCC | VOR/DME | R | 117.2 | 165.42 | 139419 | +86 | NY | CALVERTON |  | 04 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| KOKX | RADAR WXL | I |  | 173.79 | 159182 | -24 | NY | NEW YORK |  | . 01 |
| HFD | VOR/DME | R | 114.9 | 39.76 | 162031 | -678 | CT | HARTFORD |  | . 24 |
| QVH | RADAR ARSR | I | 1326.9 | 156.82 | 167033 | -180 | NY | RIVERHEAD |  | . 06 |
| CMK | VOR/DME | R | 116.6 | 267.49 | 180089 | -523 | NY | CARMEL |  | . 17 |
| ISP | RADAR | I | 2735 | 194.47 | 185780 | -11 | NY | LONG ISLAND | MacAR | 0.00 |
| FOK | TACAN | R | 111.0 | 154.33 | 186959 | +121 | NY | SUFFOLK CO |  | . 04 |
| DPK | VOR/DME | R | 117.7 | 209.21 | 212362 | +48 | NY | DEER PARK |  | 01 |
| HPN | RADAR | I | 2735. | 248.88 | 232364 | -339 | NY | WESTCHESTER | COUNT | -. 08 |
| BDL | RADAR | I |  | 15.92 | 242105 | -65 | CT | BRADLEY INTL |  | -. 02 |

CFR Title 47, §1.30000-§1.30004
AM STUDY NOT REQUIRED: Structure is near a licensed AM radio station. However, Movement Method Proof is not required because only antenna structures fall within the jurisdiction of the FCC. Please review AM Station Report for details.

```
Airspace }\mp@subsup{}{}{\circledR}\mathrm{ Summary Version 20.5.565
AIRSPACE }\mp@subsup{}{}{\circledR}\mathrm{ and TERPS }\mp@subsup{}{}{\circledR}\mathrm{ are registered }\mp@subsup{}{}{\circledR}\mathrm{ trademarks of Federal Airways & Airspace }\mp@subsup{}{}{\circledR
Copyright © 1989 - 2020
```

05-22-2020
14:12:20

```
*********************************************
* F.A.R. 77 OBSTRUCTION ANALYSIS *
*********************************************
```

FILE: 2020-APS-3708-OE

SITE ELEVATION AMSL...... 11 ft.
STRUCTURE HEIGHT......... 160 ft.
OVERALL HEIGHT AMSL...... 171 ft.
77.17(a)(1) A height more than 499 ft. Above Ground Level (AGL).
************* DOES NOT EXCEED *************
THE MAXIMUM ALLOWABLE HEIGHT IS:...... 510 ft. AMSL
THE GROUND ELEVATION AT THE SITE IS:... 11 ft. AMSL
THE OVERALL CASE ELEVATION IS:......... 171 ft. AMSL
THE CASE IS BELOW THE ALLOWABLE BY:.... 339 ft.
********************************************
BEGIN AIRPORT ANALYSIS FOR HVN
$\star \star \star \star \star \star * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *$
77.17(a) (2) A height AGL or airport elevation, whichever is higher.

```
************* DOES NOT EXCEED **************
```

BECAUSE: Proposed height does not exceed 200 feet Above Ground Level.
THE REFERENCE AIRPORT IDENT IS:........ HVN
THE AIRPORT ELEVATION IS:............. 13 ft. AMSL
THE DISTANCE FROM THE CASE TO ARP IS:.. 2.8126 NAUTICAL MILES
THE BEARING AIRPORT TO CASE IS:....... 320.115 DEGREES
THE CASE HEIGHT AGL IS:................ 160 ft.
ALLOWABLE HEIGHT....................... 213 ft. AMSL
77.19(a) A height exceeding a horizontal surface 150 ft . above airport elevation within a radius of >> HVN <<.
************* DOES NOT EXCEED **************
NOT WITHIN SPECIFIED HORIZONTAL SURFACE AREA
77.19(b) A height exceeding a conical surface (a slope outward 4000 ft. from the horizontal surface at 20/1 ratio).
************* DOES NOT EXCEED **************
NOT WITHIN SPECIFIED CONICAL SURFACE AREA

```
*************************
* BEGIN RUNWAY ANALYSIS *
****************************
```

RUNWAY 02/20

## EXISTING RUNWAY 02/20

77.19(c) A height exceeding runway primary surface.

DOES NOT EXCEED **************
NOT WITHIN SPECIFIED RUNWAY PRIMARY SURFACE
77.19(e) A height exceeding a transitional surface abeam runway.
************* DOES NOT EXCEED **************
NOT WITHIN SPECIFIED RUNWAY ABEAM TRANSITIONAL SURFACE
77.19 (d) A height exceeding an approach surface of RUNWAY 20.

THE BEARING TO THE CASE FROM THE THRESHOLD IS...... 313.249 degrees THE NORMAL BEARING TO THE CENTERLINE IS............. 92.91 degrees THE CENTERLINE OUTBOUND TRUE BEARING IS............. 2.91 degrees THE ABEAM DISTANCE TO CENTERLINE FROM CASE IS....... 11321.31 ft. THE RUNWAY THRESHOLD ELEVATION IS.................... 12.6 ft. AMSL THE DISTANCE FROM THRESHOLD + 200' TO THE CASE IS... 14853.313 ft. THE DISTANCE FROM THRESHOLD + 200' TO NB IS........ 9609.94 ft. THE CRITICAL WIDTH OF HALF THE APPROACH IS......... 1701.237 ft.

```
    DOES NOT EXCEED **************
```

BEYOND DEFINED APPROACH \& TRANSITIONAL AREAS.
RUNWAY CENTERLINE OFFSET IS......................... 11321.31 ft.
DISTANCE FROM THE THRESHOLD TO OFFSET IS........... 9809.9 ft.
THE SLOPE OF RUNWAY 20 IS: 34 TO 1.
The FAA has defined this runway as a non-utility runway. It has a non-precision approach. The obstacle surface extends 10,000 feet (34:1 Slope) symmetrically centered along the runway centerline extended. Please review the US Terminal Procedures volume associated with this airport. If a procedure for this airport and/or runway exist use Terps ${ }^{\circledR}$ Professional software to determine the height limits (if any) the procedure will have on the proposed structure. Non-precision instrument procedures can extend 10 NM from the runway and a circling approach to the airport or runway can extend out up to 4.5 NM from every runway end.

$$
\begin{aligned}
& * * * * * * * * * * * * * * * * * * * * * * * * * ~ \\
& \text { * BEGIN RUNWAY ANALYSIS } \\
& \text { *************************************)}
\end{aligned}
$$

RUNWAY 14/32
EXISTING RUNWAY 14/32
77.19(c) A height exceeding runway primary surface.

NOT WITHIN SPECIFIED RUNWAY PRIMARY SURFACE
77.19(e) A height exceeding a transitional surface abeam runway.

DOES NOT EXCEED **************
NOT WITHIN SPECIFIED RUNWAY ABEAM TRANSITIONAL SURFACE
$77.19(d)$ A height exceeding an approach surface of RUNWAY 14.


BEGIN AIRPORT ANALYSIS FOR BDR
*********************************************
77.17(a)(2) A height AGL or airport elevation, whichever is higher.
************* DOES NOT EXCEED **************
BECAUSE: Location studied is further than 5.99 nm. from ARP.
77.19(a) A height exceeding a horizontal surface 150 ft. above airport elevation within a radius of $\gg B D R \ll$.
************* DOES NOT EXCEED **************
NOT WITHIN SPECIFIED HORIZONTAL SURFACE AREA
77.19(b) A height exceeding a conical surface (a slope outward 4000 ft . from the horizontal surface at 20/1 ratio).
************* DOES NOT EXCEED **************

NOT WITHIN SPECIFIED CONICAL SURFACE AREA

```
*************************
* BEGIN RUNWAY ANALYSIS *
****************************
        RUNWAY 06/24
    EXISTING RUNWAY 06/24
```

77.19(c) A height exceeding runway primary surface.
************* DOES NOT EXCEED **************
NOT WITHIN SPECIFIED RUNWAY PRIMARY SURFACE
77.19(e) A height exceeding a transitional surface abeam runway.
************* DOES NOT EXCEED **************
NOT WITHIN SPECIFIED RUNWAY ABEAM TRANSITIONAL SURFACE
$77.19(d)$ A height exceeding an approach surface of RUNWAY 24.
THE BEARING TO THE CASE FROM THE THRESHOLD IS....... 47.451 degrees
THE NORMAL BEARING TO THE CENTERLINE IS............. 135 degrees
THE CENTERLINE OUTBOUND TRUE BEARING IS............. 45 degrees
THE ABEAM DISTANCE TO CENTERLINE FROM CASE IS....... 3070.97 ft.

```
************** DOES NOT EXCEED **************
CASE MEETS ANGULAR CRITERIA BUT IS LOCATED
GREATER THAN 50,000 ft. FROM THE START OF ANY APPROACH TYPE, OUT BY 21872.7 feet
```

```
*************************
* BEGIN RUNWAY ANALYSIS *
***************************
```

RUNWAY 11/29
EXISTING RUNWAY 11/29
77.19(c) A height exceeding runway primary surface.
************* DOES NOT EXCEED **************
NOT WITHIN SPECIFIED RUNWAY PRIMARY SURFACE
77.19(e) A height exceeding a transitional surface abeam runway.
************* DOES NOT EXCEED **************
NOT WITHIN SPECIFIED RUNWAY ABEAM TRANSITIONAL SURFACE
$77.19(d)$ A height exceeding an approach surface of RUNWAY 29.
THE BEARING TO THE CASE FROM THE THRESHOLD IS....... 46.656 degrees
THE NORMAL BEARING TO THE CENTERLINE IS............. 187.83 degrees

THE CENTERLINE OUTBOUND TRUE BEARING IS............. 97.83 degrees
THE ABEAM DISTANCE TO CENTERLINE FROM CASE IS....... 56098.88 ft.
THE RUNWAY THRESHOLD ELEVATION IS................... 6.5 ft. AMSL
THE DISTANCE FROM THRESHOLD + 200' TO THE CASE IS... 72006.221 ft.
THE DISTANCE FROM THRESHOLD + 200' TO NB IS........ 45265.32 ft.
THE CRITICAL WIDTH OF HALF THE APPROACH IS......... 7039.792 ft.
************* DOES NOT EXCEED **************
BEYOND DEFINED APPROACH \& TRANSITIONAL AREAS.
RUNWAY CENTERLINE OFFSET IS......................... 56098.88 ft.
DISTANCE FROM THE THRESHOLD TO OFFSET IS........... 45465.28 ft.
THE SLOPE OF RUNWAY 29 IS: 34 TO 1.
The FAA has defined this runway as a non-utility runway. It has a non-precision approach. The obstacle surface extends 10,000 feet (34:1 Slope) symmetrically centered along the runway centerline extended. Please review the US Terminal Procedures volume associated with this airport. If a procedure for this airport and/or runway exist use Terps ${ }^{\circledR}$ Professional software to determine the height limits (if any) the procedure will have on the proposed structure. Non-precision instrument procedures can extend 10 NM from the runway and a circling approach to the airport or runway can extend out up to 4.5 NM from every runway end.

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12:40:38

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********************************************
PUBLIC AIRPORTS IN PROXIMITY OF CASE
```

File: 2020-APS-3708-OE
OVERALL ELEVATION (AMSL) : 171
LATITUDE: 410-17'-59.54"
LONGITUDE: 720-55'-35.57"

| FACIL |  |  | BEARING | DISTANCE | DELTA ARP | FAR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| IDENT | TYP | NAME | TO FACIL | IN N.M. | ELEVATION | P77 |
| HVN | AIR | TWEED-NEW HAVEN | 140.12 | 2.812 | +158.4 | YES |

This facility has at least one runway over 3,200 feet in length.
Your structure DNE FAR 77.9(a) but EXCEEDS FAR 77.9(b) Notice Criteria for this airport. You must notify the Federal Aviation Administration using a FAA Form 7460-1 a minimum of 45 days prior to your construction start date. As a minimum, please review reports for FAR Part 77 Obstruction Surfaces, Air Navigation and Communication facilities.

EXCEEDS FAR 77.9(b)(2) Notice Criteria by: 9 feet.
You are 14983 feet from the nearest runway threshold and the threshold elevation is 13 feet. Please review runway analysis for remaining airport surfaces.

This airport has both Circling and Straight-In Instrument Procedures. Please review published US Terminal (TERPS ${ }^{\circledR}$ ) Approach Procedures for this landing facility.

DNE FAR 77.9 IFR Notice Criteria for HVN
Category 'D' Circling Approach Area extends 3.78 NM from each runway.

| FACIL |  |  |  | BEARING | DISTANCE | DELTA ARP | FAR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| IDENT | TYP | NAME |  | To FACIL | IN N.M. | ELEVATION | P77 |
| BDR | AIR | IGOR | SIKORSKY MEMORIAL | 227.75 | 12.193 | +162.5 | YES |

This facility has at least one runway over 3,200 feet in length.
Your structure DNE FAR 77.9(a) or 77.9(b) Notice Criteria for this airport.
However, you may EXCEED other Notice Standards. As a minimum, please review
reports for FAR Part 77 Obstruction Surfaces, Air Navigation and Communication facilities.

You are 72131 feet from the nearest runway threshold and the threshold elevation is 7 feet. Please review runway analysis for remaining airport surfaces.

This airport has both Circling and Straight-In Instrument Procedures. Please review published US Terminal (TERPS ${ }^{\circledR}$ ) Approach Procedures for this landing facility.

DNE 77.9 IFR Notice Criteria BDR
Category 'D' Circling Approach Area extends 3.78 NM from each runway.

THE NEAREST AIRPORT TO CASE COORDINATES IS: HVN
TWEED-NEW HAVEN is an Airport type landing facility and is associated with the city of NEW HAVEN, CT. The facility is eligible for Study
under FAR Part 77 sub-Part C.
Its Reference Point (ARP) elevation is: 12.6 feet AMSL and you are locating 17089 feet from this landing facility.

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**********************************************************
* AIRWAY ANALYSIS *
* FAR 77.17(a)(4) (EN ROUTE CRITERIA) *
* MINIMUM OBSTACLE CLEARANCE ALTITUDE (MOCA) *
* MINIMUM ENROUTE ALTITUDE (MEA) *
**********************************************************
```

FILE: 2020-APS-3708-OE
LATITUDE: 41 - 17 - 59.54 LONGITUDE: 72 - 55-35.57
SITE ELEVATION AMSL...... 11 ft.
STRUCTURE HEIGHT......... 160 ft.
OVERALL HEIGHT AMSL...... 171 ft.

FAR 77.17(a)(4) - EN ROUTE CRITERIA
MINIMUM OBSTACLE CLEARANCE ALTITUDE (MOCA)

LOW ALTITUDE AIRWAY

| AIRWAY | EQUENCE | LATITUDE | LONGITUDE | MEA | LENGTH (NM) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| V188 | 140 | 41-19-35.11N | 073-16-59.58W | 3000 | 20.09 |
| V188 | 150 | 41-23-33.74N | 072-50-50.56W | 3000 |  |

Minimum Obstacle Clearance Altitude (MOCA) is: 3000 AMSL.
Proposed structure is between the above points along Airway V188. The Abeam distance from the course centerline is 4.74 NM. The proposed structure is within the width of the secondary area of this airway. The width of the primary area is 8 NM and the width of the secondary is 2 NM .

The maximum allowable height permitted by the secondary area MOCA of this airway at this location is 2684 feet AMSL.

LOW ALTITUDE AIRWAY


LOW ALTITUDE AIRWAY

width of the primary area is 8 NM and the width of the secondary is 2 NM .
The maximum allowable height permitted by the secondary area MOCA of this airway at this location is 2474 feet AMSL.

LOW ALTITUDE AIRWAY


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12:40:42
The mathematical algorithms used by this program are derived directly from Federal Aviation Regulations Part 77, sub-part C.


# Circling Approach Area Analysis 

# *** 2020-APS-3708-OE *** <br> TWEED-NEW HAVEN <br> Date: 05-22-2020 Time: 13:17:25 

## STUDY OBJECT DATA

Study Latitude: 410 17' 59.54" N
Study Longitude: 720 55' 35.57" W
Ground Elevation: 11' AMSL ft.
AGL Height: 160' AGL ft.
Overall Elevation: 171' AMSL ft.

INSTRUMENT APPROACH PROCEDURE (IAP) ANALYSIS

Distance: 14984 ft.
Aircraft Category: C
Circling MDA: 780
Vkias: 140 knots
Vktas: 145.7757 knots
Bank Angle: $20^{\circ}$
Straight Segment: 0.5 NM
Expanded CAA: True
Turn Radius: 2.8 NM

Maximum AMSL: 480

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The mathematical algorithms used by this program are derived directly from Federal Aviation Administration (FAA) Orders on Instrument Flight Procedures.


## Departure Runway 32

*** 2020-APS-3708-OE ***

## TWEED-NEW HAVEN - Runway: 32

Date: 05-27-2020 Time: 09:54:32

## STUDY OBJECT DATA

Study Latitude: $41^{\circ} 17{ }^{\circ} 59.54 " \mathrm{~N}$<br>Study Longitude: $72^{\circ}$ 55' 35.57" W<br>Ground Elevation: 11' AMSL ft.<br>AGL Height: 160 ' AGL ft.<br>Overall Elevation: 171' AMSL ft.<br>INSTRUMENT DEPARTURE ANALYSIS<br>Initial Climb Area (ICA) : DNE ICA<br>Diverse Departure A Inside Diverse A - Max Hgt: 899 ft<br>Diverse Departure B Not in Diverse B - DNE Diverse B

The above analysis is in accordance with FAA Order 8260.3B Change 26. This analysis used a 465 ft/NM climb gradient (CG) and an Obstacle Clearance Surface (OCS) that provides 111 feet of obstacle clearance at 1 NM from the Departure End of Runway (DER). Some runways have published climb gradients greater than 200 ft/NM. A specified climb gradient greater than standard (200 ft/NM) is sometimes necessary to allow acceptable obstacle clearance. Should your location exceed the value indicated above you may need to determine if there is a published CG and conduct additional calculations to determine if the CG will provide proper clearance for your proposed structure.

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```

The mathematical algorithms used by this program are derived directly from Federal Aviation Administration (FAA) Orders on Instrument Flight Procedures.

```
STUDY OBJECT DATA
Study Latitude: 410 17' 59.54'
Study Longitude: 72` 55' 35.57"
Ground Elevation: 11' AMSL
AGL Height: 160' AGL
Overall Elevation: 171' AMSL
```


## AIRSPACE/TERPS LIMIT: 363' AMSL --- VFR TRAFFIC PATTERN AIRSPACE

480363```
HVN IAP RWY 02 ILS19
```

HVN IAP RWY 02 ILS19
Amdt 18 10000
Amdt 18 10000
HVN IAP RWY 02 LOC19
HVN IAP RWY 02 LOC19
Amdt 18
Amdt 18
1 0 0 0 0
1 0 0 0 0
HVN IAP RWY 02 LPV
HVN IAP RWY 02 LPV
Amdt 1 10000
Amdt 1 10000
HVN IAP RWY 02 VNAV
HVN IAP RWY 02 VNAV
Amdt 1 10000
Amdt 1 10000
HVN IAP RWY 02 LNAV
HVN IAP RWY 02 LNAV
Amdt 1 10000
Amdt 1 10000
HVN IAP RWY 20X LOC19
HVN IAP RWY 20X LOC19
Orig-1DR
Orig-1DR
10000
10000
HVN IAP RWY 20X LNAV
HVN IAP RWY 20X LNAV
Orig -1DR
Orig -1DR
10000
10000
HVN MRP LOW ALTITUDE AIRWAY
HVN MRP LOW ALTITUDE AIRWAY
V475
V475
5 0 0
5 0 0
HVN CIR CATEGORY C
HVN CIR CATEGORY C
EXPANDED
EXPANDED
HVN DEP RUNWAY 02
HVN DEP RUNWAY 02
DIVERSE A
DIVERSE A
HVN DEP RUNWAY 32
HVN DEP RUNWAY 32
DIVERSE A
DIVERSE A
899
899
HVN VFR TRAFFIC PATTERN AIRSPACE
HVN VFR TRAFFIC PATTERN AIRSPACE
CATEGORY D CLIMB/DESCEND AREA
CATEGORY D CLIMB/DESCEND AREA
HVN VFR TRANSITIONAL SURFACE
HVN VFR TRANSITIONAL SURFACE
77.17(A)(2)
77.17(A)(2)
2 1 3

```
2 1 3
```

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## POINT ELEVATION DATA

# SRTM GROUND ELEVATION DATA <br> North American Datum 1983 <br> North American Vertical Datum 1988 - NAVD88 

## 2020-APS-3708-OE MCM_CT-520

Latitude: $\quad 41^{\circ}-17$ - $59.54 " N \quad$ Decimal Degrees: 41.2998722222222
Longitude: $72^{\circ}-55^{\prime}-35.57{ }^{\prime \prime}$ W Decimal Degrees: 72.9265472222222

## Ground Elevation: 8.18 Feet AMSL

This certifies the Digital Elevation Model (DEM) value for the specified latitude/longitude point was obtained from the SRTM Endeavour radar mission of February 2000. NASA has released the finished version edited by the National Intelligence Agency. The elevation value meets vertical accuracy criteria as specified by FAA Order 8260.19C, Appendix 2, Obstacle Accuracy Standards, Codes And Sources, paragraph 101 for Code 'C'. The elevation value for the specified latitude/longitude is accurate to within $\pm 20$ feet vertically.

## ATTACHMENT 8

Kenneth C. BALDWIN

280 Trumbull Street
Hartford, CT 06103-3597
Main (860) 275-8200
Fax (860) 275-8299
kbaldwin@rc.com
Direct (860) 275-8345

Also admitted in Massachusetts and New York

August 26, 2020

## Via Certificate of Mailing

Justin Elicker, Mayor
City of New Haven
165 Church Street, $2^{\text {nd }}$ Floor
New Haven, CT 06510

## Re: Petition for Declaratory Ruling Filed with the Connecticut Siting Council for Modifications to its Existing Wireless Telecommunications Facility at 54 Meadow Street, New Haven, Connecticut

Dear Mayor Elicker:
This firm represents Cellco Partnership d/b/a Verizon Wireless ("Cellco"). Today, Cellco filed a Petition for Declaratory Ruling ("Petition") with the Connecticut Siting Council ("Council") seeking approval to make certain modifications to its existing telecommunications facility on the roof of the building at 54 Meadow Street in New Haven (the "Property").

The modifications will consist of removing certain antennas and installing newer model antennas and remote radio heads at various locations on the roof. The existing facility is under the exclusive jurisdiction of the Connecticut Siting Council by virtue of its April 1, 1991 approval of Docket No. 140.

A copy of the full Petition is attached for your review. Landowners whose parcels are considered to abut the Property were also sent notice of this filing along with a copy of the Petition.

Please contact me if you have any questions regarding this proposal.

## Sincerely,



Kenneth C. Baldwin
Attachment

Kenneth C. BALDWIN

280 Trumbull Street
Hartford, CT 06103-3597
Main (860) 275-8200
Fax (860) 275-8299
kbaldwin@rc.com
Direct (860) 275-8345

Also admitted in Massachusetts and New York

August 26, 2020

## Via Certificate of Mailing

Aicha Woods, Director of City Plan
City of New Haven
165 Church Street, $5^{\text {th }}$ Floor
New Haven, CT 06510
Re: Petition for Declaratory Ruling Filed with the Connecticut Siting Council for Modifications to its Existing Wireless Telecommunications Facility at 54 Meadow Street, New Haven, Connecticut

Dear Ms. Woods:
This firm represents Cellco Partnership d/b/a Verizon Wireless ("Cellco"). Today, Cellco filed a Petition for Declaratory Ruling ("Petition") with the Connecticut Siting Council ("Council") seeking approval to make certain modifications to its existing telecommunications facility on the roof of the building at 54 Meadow Street in New Haven (the "Property").

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A copy of the full Petition is attached for your review. Landowners whose parcels are considered to abut the Property were also sent notice of this filing along with a copy of the Petition.

Please contact me if you have any questions regarding this proposal.
Sincerely,


Attachment

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Hartford, CT 06103-3597
Main (860) 275-8200
Fax (860) 275-8299
kbaldwin@rc.com
Direct (860) 275-8345

Also admitted in Massachusetts and New York

August 26, 2020

## Via Certificate of Mailing

Gateway Partners LLC
c/o Lexington Property Management
30 Lewis Street
Hartford, CT 06103

## Re: Petition for Declaratory Ruling Filed with the Connecticut Siting Council for Modifications to its Existing Wireless Telecommunications Facility at 54 Meadow Street, New Haven, Connecticut

Dear Sir or Madam:
This firm represents Cellco Partnership d/b/a Verizon Wireless ("Cellco"). Today, Cellco filed a Petition for Declaratory Ruling ("Petition") with the Connecticut Siting Council ("Council") seeking approval to make certain modifications to its existing telecommunications facility on the roof of the building at 54 Meadow Street in New Haven (the "Property").

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A copy of the full Petition is attached for your review. Landowners whose parcels are considered to abut the Property were also sent notice of this filing along with a copy of the Petition.

Please contact me if you have any questions regarding this proposal.
Sincerely,


Attachment

Kenneth C. BALDWIN

280 Trumbull Street
Hartford, CT 06103-3597
Main (860) 275-8200
Fax (860) 275-8299
kbaldwin@rc.com
Direct (860) 275-8345

Also admitted in Massachusetts and New York

August 26, 2020

## Via Certificate of Mailing

MCM Holdings LLC
40 Woodland Street
Hartford, CT 06105

## Re: Petition for Declaratory Ruling Filed with the Connecticut Siting Council for Modifications to its Existing Wireless Telecommunications Facility at 54 Meadow Street, New Haven, Connecticut

Dear Sir or Madam:
This firm represents Cellco Partnership d/b/a Verizon Wireless ("Cellco"). Today, Cellco filed a Petition for Declaratory Ruling ("Petition") with the Connecticut Siting Council ("Council") seeking approval to make certain modifications to its existing telecommunications facility on the roof of the building at 54 Meadow Street in New Haven (the "Property").

The modifications will consist of removing certain antennas and installing newer model antennas and remote radio heads at various locations on the roof. The existing facility is under the exclusive jurisdiction of the Connecticut Siting Council by virtue of its April 1, 1991 approval of Docket No. 140.

A copy of the full Petition is attached for your review. Landowners whose parcels are considered to abut the Property were also sent notice of this filing along with a copy of the Petition.

Please contact me if you have any questions regarding this proposal.
Sincerely,

Kenneth C. Baldwin
Attachment

## ATTACHMENT 9

## Via Certificate of Mailing

«Owners_and_Mailing_Address»

## Re: Petition for Declaratory Ruling Filed with the Connecticut Siting Council for Modifications to an Existing Wireless Telecommunications Facility at 54 Meadow Street, New Haven, Connecticut

Dear «Salutation»:
This firm represents Cellco Partnership d/b/a Verizon Wireless ("Cellco"). Today, Cellco filed a Petition for Declaratory Ruling ("Petition") with the Connecticut Siting Council ("Council") seeking approval to make certain modifications to its existing telecommunications facility on the roof of the building at 54 Meadow Street in New Haven (the "Property").

The modifications will consist of removing certain antennas and installing newer model antennas and remote radio heads at various locations on the roof. The existing facility is under the exclusive jurisdiction of the Connecticut Siting Council by virtue of its April 1, 1991 approval of Docket No. 140. A copy of the full Petition is attached for your review.

This notice is being sent to you because you are listed on the City Assessor's records as an owner of land that abuts the Property. If you have any questions regarding the Petition, the Council's process for reviewing the Petition or the details of the filing itself, please feel free to contact me at the number listed above. You may also contact the Council directly at 860-8272935.

Sincerely,


Kenneth C. Baldwin

Attachment

## CELLCO PARTNERSHIP D/B/A VERIZON WIRELESS

## ABUTTING PROPERTY OWNERS

54 MEADOWN STREET NEW HAVEN, CONNECTICUT

|  | Property Address | Owner's and Mailing Address |
| :---: | :---: | :---: |
| 1. | 78 Meadow Street | Knights of Columbus <br> 1 Columbus Plaza <br> New Haven, CT 06510 |
| 2. | 1 Union Avenue | City of New Haven |
| 1 Union Avenue |  |  |
| New Haven, CT 06519 |  |  |


[^0]:    1 Typical value over frequency and tilt

[^1]:    ${ }^{1}$ The Canon EOS 6D is a full-framed camera which includes a lens receptor of the same size as the film used in 35 mm cameras. As such, the images produced are comparable to those taken with a conventional 35 mm camera.

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