

Northeast Site Solutions Denise Sabo 4 Angela's Way, Burlington CT 06013 203-435-3640 denise@northeastsitesolutions.com

September 21, 2021

Members of the Siting Council Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

RE: Tower Share Application 85 Paper Mill Road, Woodbury, CT 06798 Latitude: 41.853475

Longitude: 72.452089 Site# 857528\_Crown\_Dish

#### Dear Ms. Bachman:

This letter and attachments are submitted on behalf of Dish Wireless LLC. Dish Wireless LLC plans to install antennas and related equipment to the tower site located at 85 Paper Mill Road, Woodbury, Connecticut.

Dish Wireless LLC proposes to install three (3) 600/1900 5G MHz antenna and six (6) RRUs, at the 138-foot level of the existing 150-foot monopole tower, one (1) Fiber cables will also be installed. Dish Wireless LLC equipment cabinets will be placed within 7x5 lease area. Included are plans by Infinigy, dated August 31, 2021 Exhibit C. Also included is a structural analysis prepared by Crown Castle, dated May 28, 2021, confirming that the existing tower is structurally capable of supporting the proposed equipment. Attached as Exhibit D. This facility was approved by Connecticut Siting Council, Docket No. 375 on August 27, 2009. Please see attached Exhibit A.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies 16-50aa, of Dish Wireless LLC intent to share a telecommunications facility pursuant to R.C.S.A. 16-50j-88. In accordance with R.C.S.A., a copy of this letter is being sent to Barbara Perkinson, First Selectwoman, for the Town of Woodbury, William Agresta, Town Planner, as well as the tower owner (Crown Castle) and property owner (Jodie Bryan)

The planned modifications of the facility fall squarely within those activities explicitly provided for in R.C.S.A. 16-50j-89.

- 1. The proposed modification will not result in an increase in the height of the existing structure. The top of the tower is 150-feet; Dish Wireless LLC proposed antennas will be located at a center line height of 138-feet.
- 2. The proposed modifications will not result in the increase of the site boundary as depicted on the attached site plan.



- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed local and state criteria. The incremental effect of the proposed changes will be negligent.
- 4. The operation of the proposed antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard. As indicated in the attached power density calculations, the combined site operations will result in a total power density of 5.61% as evidenced by Exhibit F.

Connecticut General Statutes 16-50aa indicates that the Council must approve the shared use of a telecommunications facility provided it finds the shared use is technically, legally, environmentally, and economically feasible and meets public safety concerns. As demonstrated in this letter, Dish Wireless LLC respectfully indicates that the shared use of this facility satisfies these criteria.

- A. Technical Feasibility. The existing monopole has been deemed structurally capable of supporting Dish Wireless LLC proposed loading. The structural analysis is included as Exhibit D.
- B. Legal Feasibility. As referenced above, C.G.S. 16-50aa has been authorized to issue orders approving the shared use of an existing tower such as this support tower in Woodbury. Under the authority granted to the Council, an order of the Council approving the requested shared use would permit Dish Wireless LLC to obtain a building permit for the proposed installation. Further, a Letter of Authorization is included as Exhibit G, authorizing Dish Wireless LLC to file this application for shared use.
- C. Environmental Feasibility. The proposed shared use of this facility would have a minimal environmental impact. The installation of Dish Wireless LLC equipment at the 138-foot level of the existing 150-foot tower would have an insignificant visual impact on the area around the tower. Dish Wireless LLC ground equipment would be installed within the existing facility compound. Dish Wireless LLC shared use would therefore not cause any significant alteration in the physical or environmental characteristics of the existing site. Additionally, as evidenced by Exhibit F, the proposed antennas would not increase radio frequency emissions to a level at or above the Federal Communications Commission safety standard.
- D. Economic Feasibility. Dish Wireless LLC will be entering into an agreement with the owner of this facility to mutually agreeable terms. As previously mentioned, the Letter of Authorization has been provided by the owner to assist Dish Wireless LLC with this tower sharing application.
- E. Public Safety Concerns. As discussed above, the tower is structurally capable of supporting Dish Wireless LLC proposed loading. Dish Wireless LLC is not aware of any public safety concerns relative to the proposed sharing of the existing tower. Dish Wireless LLC intentions of providing new and improved wireless service through the shared use of this facility is expected to enhance the safety and welfare of local residents and individuals traveling through Woodbury.

Sincerely,

### Denise Sabo

Denise Sabo

Mobile: 203-435-3640 Fax: 413-521-0558

Office: 4 Angela's Way, Burlington CT 06013 Email: denise@northeastsitesolutions.com



Attachments cc: Barbara Perkinson, First Selectwoman Town of Woodbury 281 Main Street South Woodbury, CT 06798

William Agresta, Town Planner Town of Woodbury 281 Main Street South Woodbury, CT 06798

Jodie Bryan, Property Owner) 85 Paper Mill Road Woodbury, CT 06798

Crown Castle, Tower Owner

# Exhibit A

**Original Facility Approval** 

DOCKET NO. 375 – New Cingular Wireless PCS, LLC application for a Certificate of Environmental Compatibility and Public Need for the construction, maintenance and operation of a telecommunications facility located at 85 Paper Mill Road, Woodbury, Connecticut.

Siting Council

Council

August 27, 2009

#### **Decision and Order**

Pursuant to the foregoing Findings of Fact and Opinion, the Connecticut Siting Council (Council) finds that the effects associated with the construction, operation, and maintenance of a telecommunications facility, including effects on the natural environment; ecological integrity and balance; public health and safety; scenic, historic, and recreational values; forests and parks; air and water purity; and fish and wildlife are not disproportionate, either alone or cumulatively with other effects, when compared to need, are not in conflict with the policies of the State concerning such effects, and are not sufficient reason to deny the application, and therefore directs that a Certificate of Environmental Compatibility and Public Need, as provided by General Statutes § 16-50k, be issued to New Cingular Wireless PCS, LLC, hereinafter referred to as the Certificate Holder, for a telecommunications facility located at 85 Paper Mill Road, Woodbury, Connecticut.

The facility shall be constructed, operated, and maintained substantially as specified in the Council's record in this matter, and subject to the following conditions:

- 1. The tower shall be constructed as a monopole, no taller than necessary to provide the proposed telecommunications services, sufficient to accommodate the antennas of the Certificate Holder and other entities, both public and private, but such tower shall not exceed a height of 150 feet above ground level.
- 2. The Certificate Holder shall prepare a Development and Management (D&M) Plan for this site in compliance with Sections 16-50j-75 through 16-50j-77 of the Regulations of Connecticut State Agencies. Prior to the submission of the D&M Plan to the Council, the Certificate Holder shall discuss and resolve issues pertaining to the existing driveway that serves the site property with the Town of Woodbury. Once the driveway issues are resolved, the D&M Plan shall be served on the Town of Woodbury for comment, and all parties and intervenors as listed in the service list, and submitted to and approved by the Council prior to the commencement of facility construction and shall include:
  - a) a final site plan(s) of site development to include specifications for the tower, tower foundation, antennas, equipment compound, radio equipment, access road, utility line, and landscaping; and
  - b) construction plans for site clearing, grading, landscaping, water drainage, and erosion and sedimentation controls consistent with the <u>2002 Connecticut Guidelines for Soil Erosion and Sediment Control</u>, as amended.
- 3. The Certificate Holder shall, prior to the commencement of operation, provide the Council worst-case modeling of the electromagnetic radio frequency power density of all proposed entities' antennas at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin No. 65, August 1997. The Certificate Holder shall ensure a recalculated report of the electromagnetic radio frequency power density be submitted to the Council if and when circumstances in operation cause a change in power density above the levels calculated and provided pursuant to this Decision and Order.

- 4. Upon the establishment of any new State or federal radio frequency standards applicable to frequencies of this facility, the facility granted herein shall be brought into compliance with such standards.
- 5. The Certificate Holder shall permit public or private entities to share space on the proposed tower for fair consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing.
- 6. The Certificate Holder shall provide reasonable space on the tower for no compensation for any Town of Woodbury public safety services (police, fire and medical services), provided such use can be accommodated and is compatible with the structural integrity of the tower.
- 7. Unless otherwise approved by the Council, if the facility authorized herein is not fully constructed and providing wireless services within eighteen months from the date of the mailing of the Council's Findings of Fact, Opinion, and Decision and Order (collectively called "Final Decision"), this Decision and Order shall be void, and the Certificate Holder shall dismantle the tower and remove all associated equipment or reapply for any continued or new use to the Council before any such use is made. The time between the filing and resolution of any appeals of the Council's Final Decision shall not be counted in calculating this deadline.
- 8. Any request for extension of the time period referred to in Condition 7 shall be filed with the Council not later than 60 days prior to the expiration date of this Certificate and shall be served on all parties and intervenors, as listed in the service list, and the Town of Woodbury. Any proposed modifications to this Decision and Order shall likewise be so served.
- 9. If the facility ceases to provide wireless services for a period of one year, this Decision and Order shall be void, and the Certificate Holder shall dismantle the tower and remove all associated equipment or reapply for any continued or new use to the Council before any such use is made.
- 10. The Certificate Holder shall remove any nonfunctioning antenna, and associated antenna mounting equipment, within 60 days of the date the antenna ceased to function.
- 11. In accordance with Section 16-50j-77 of the Regulations of Connecticut State Agencies, the Certificate Holder shall provide the Council with written notice two weeks prior to the commencement of site construction activities. In addition, the Certificate Holder shall provide the Council with written notice of the completion of site construction and the commencement of site operation.

Pursuant to General Statutes § 16-50p, the Council hereby directs that a copy of the Findings of Fact, Opinion, and Decision and Order be served on each person listed below, and notice of issuance shall be published in the <u>Republican-American</u> and <u>Voices</u>.

By this Decision and Order, the Council disposes of the legal rights, duties, and privileges of each party named or admitted to the proceeding in accordance with Section 16-50j-17 of the Regulations of Connecticut State Agencies.

Docket No. 375 Decision and Order Page 3

The parties and intervenors to this proceeding are:

### **Applicant**

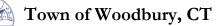
New Cingular Wireless PCS, LLC

### **Its Representative**

Christopher B. Fisher, Esq. Cuddy & Feder LLP 445 Hamilton Avenue, 14<sup>th</sup> Floor White Plains, New York 10601

# Exhibit B

**Property Card** 



Property Listing Report

Map Block Lot

040-032A

Building #

Unique Identifier

240100

### **Property Information**

Property Location	85 PAPER MILL RD		
Matter Address	85 PAPER MILL RD		
Mailing Address	WOODBURY CT 067982006		
Land Use	Residential		
Zoning Code	oning Code OS100		
Neighborhood	20		

Owner	BRYAN JODIE A	
Co-Owner		
Book / Page	0251/0215	
Land Class	Residential	
Census Tract	3621	
Acreage	31.88	

### **Valuation Summary**

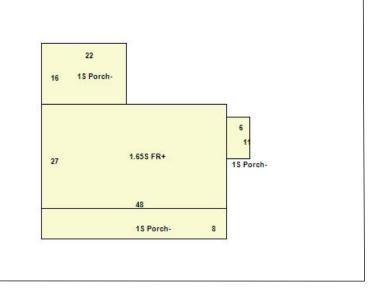
(Assessed value = 70% of Appraised Value)

Item	Appraised	Assessed	
Buildings	214102	149870	
Outbuildings	53768	37640	
Land	335358	113870	
Total	603228	0	

### **Utility Information**

<u> </u>		
Electric	No	
Gas	No	
Sewer	No	
Public Water	No	
Well	Yes	





### **Primary Construction Details**

1992
Residential
Cape
1.65
Vinyl
Drywall
Hardwood
Carpet

Oil
HWBB
Central
2
2
1
0
5
NA
Typical
1

Building Use	Single Family
<b>Building Condition</b>	Average
Frame Type	Wood Frame
Fireplaces	0
Bsmt Gar	0
Fin Bsmt Area	
Fin Bsmt Quality	
Building Grade	0
Roof Style	Gable
Roof Cover	Asphalt
eport Created On	9/24/2021

Report Created On 9/24/20

### Town of Woodbury, CT

**Property Listing Report** 

**BRYAN RALPH D & JODIE A** 

Map Block Lot

040-032A

Building #

Unique Identifier

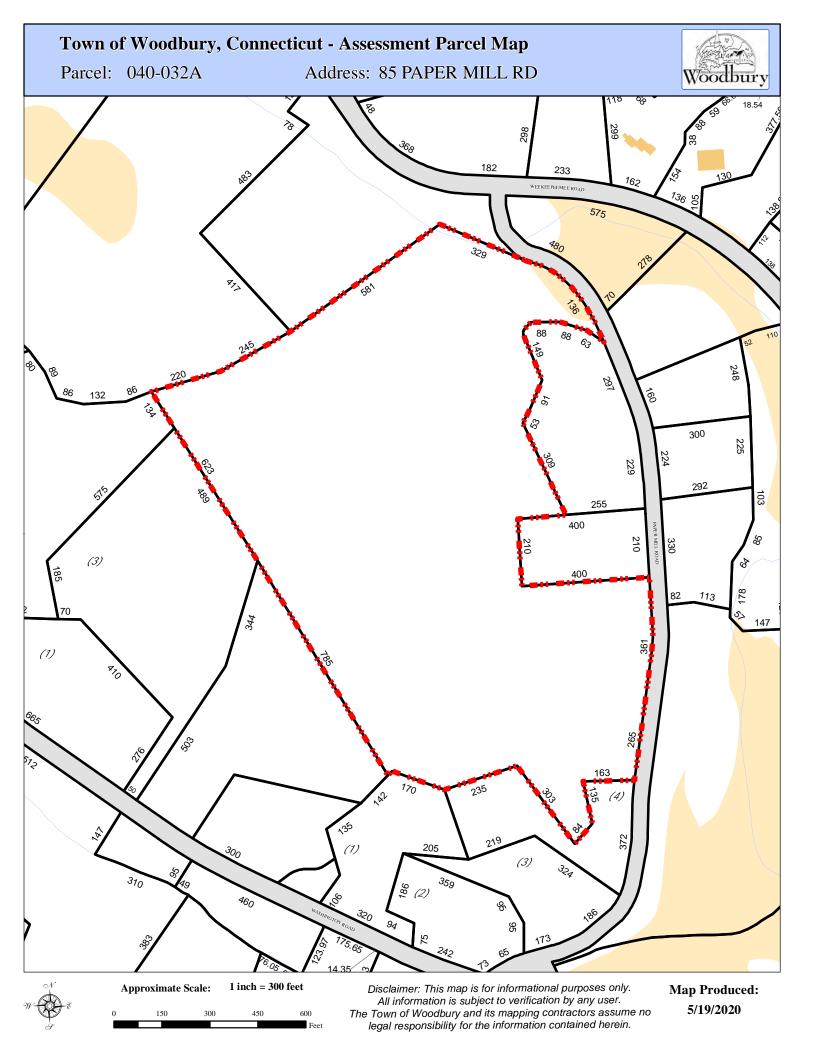
240100

Type	Description	Area (sq ft)	Condition	Year Built
Barn	Frame	2560	Average	1994
Barn	Frame	1200	Average	1994
Generator	Generator	10	Average	2012
Attached Extra Feature	<u>s</u>			
Type	Description	Area (sq ft)	Condition	Year Built
Porch	Open	384	Average	2009
Porch	Open	66	Average	1992
Porch	Open	352	Average	1992
Sales History				
owner of Record		Book/ Page	Sale Date	Sale Price

0222\_0581

6/30/1997

260000



# Exhibit C

**Construction Drawings** 

# wireless.

DISH Wireless L.L.C. SITE ID:

### **BOHVN00018A**

DISH Wireless L.L.C. SITE ADDRESS:

### **85 PAPER MILL ROAD WOODBURY, CT 06798**

### CONNECTICUT CODE COMPLIANCE

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

2018 CT STATE BUILDING CODE/2015 IBC W/ CT AMENDMENTS MECHANICAL ELECTRICAL 2018 CT STATE BUILDING CODE/2015 IMC W/ CT AMENDMENTS 2018 CT STATE BUILDING CODE/2017 NEC W/ CT AMENDMENTS

	SHEET INDEX		
SHEET NO.	SHEET TITLE		
T-1	TITLE SHEET		
A-1	OVERALL AND ENLARGED SITE PLAN		
A-2	ELEVATION, ANTENNA LAYOUT AND SCHEDULE		
A-3	EQUIPMENT PLATFORM AND H-FRAME DETAILS		
A-4	EQUIPMENT DETAILS		
A-5	EQUIPMENT DETAILS		
A-6	EQUIPMENT DETAILS		
E-1	ELECTRICAL/FIBER ROUTE PLAN AND NOTES		
E-2	ELECTRICAL DETAILS		
E-3	ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE		
G-1	GROUNDING PLANS AND NOTES		
G-2	GROUNDING DETAILS		
G-3	GROUNDING DETAILS		
RF-1	RF CABLE COLOR CODE		
GN-1	LEGEND AND ABBREVIATIONS		
GN-2	GENERAL NOTES		
GN-3	GENERAL NOTES		
GN-4	GENERAL NOTES		

### SCOPE OF WORK

THIS IS NOT AN ALL INCLUSIVE LIST. CONTRACTOR SHALL UTILIZE SPECIFIED EQUIPMENT PART OR ENGINEER APPROVED EQUIPMENT. CONTRACTOR SHALL VERIFY ALL NEEDED EQUIPMENT TO PROVIDE A FUNCTIONAL SITE. THE PROJECT GENERALLY CONSISTS OF THE FOLLOWING:

- TOWER SCOPE OF WORK:

  INSTALL (3) PROPOSED PANEL ANTENNAS (1 PER SECTOR)

  INSTALL (1) PROPOSED PLATFORM MOUNT

  INSTALL PROPOSED JUMPERS
- INSTALL (6) PROPOSED RRUS (2 PER SECTOR)
  INSTALL (1) PROPOSED OVER VOLTAGE PROTECTION DEVICE (OVP)
- INSTALL (1) PROPOSED HYBRID CABLE

- GROUND SCOPE OF WORK:
   INSTALL (1) PROPOSED METAL PLATFORM
- INSTALL (1) PROPOSED ICE BRIDGE INSTALL (1) PROPOSED PPC CABINET
- INSTALL (1) PROPOSED EQUIPMENT CABINET
- INSTALL (1) PROPOSED POWER CONDUIT INSTALL (1) PROPOSED TELCO CONDUIT
- PROPOSED TELCO-FIBER BOX
- INSTALL (1) PROPOSED GPS UNIT
- INSTALL (1) PROPOSED SAFETY SWITCH (IF REQUIRED)
- INSTALL (1) PROPOSED FIBER NID (IF REQUIRED)
- EXISTING METER SOCKET IS AVAILABLE AT SOCKET

### SITE PHOTO





**UNDERGROUND SERVICE ALERT CBYD 811** UTILITY NOTIFICATION CENTER OF CONNECTICUT (800) 922-4455 WWW.CBYD.COM

CALL 2 WORKING DAYS UTILITY NOTIFICATION PRIOR TO CONSTRUCTION

NO SCALE

### **GENERAL NOTES**

THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON DRAINAGE. NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.

### 11"x17" PLOT WILL BE HALF SCALE UNLESS OTHERWISE NOTED

CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, AND CONDITIONS ON THE JOB SITE, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCFEDING WITH THE WORK.

### CONSTRUCTION TYPE: **EVERSOURCE**

**DIRECTIONS** 

HEAD EAST ON WIBLING RD TOWARD SUGAR HOLLOW RD, TURN RIGHT ONTO SUGAR HOLLOW RD TURN LEFT ONTO WOOSTER HEIGHTS, TAKE THE RAMP FOR US-7 NORTH AND HEAD TOWARD NEW MILFORD TAKE THE RAMP ON THE RIGHT FOR US-7 NORTH / I-84 EAST AND HEAD TOWARD NEW MILFORD / WATERBURY, TAKE THE RAMP ON THE RIGHT FOR I-84 EAST AND HEAD TOWARD WATERBURY, AT EXIT 15, HEAD RIGHT ON THE RAMP FOR CT-67 / US-6 EAST TOWARD SOUTHBURY, TURN LEFT ONTO US-6 E /

CT-67 / CHURCH HILL RD, MODERATE CONGESTION, TURN LEFT ONTO CT-47 / WASHINGTON RD, TURN RIGHT ONTO PAPER MILL RDARRIVE AT, 85 PAPER MILL ROAD, WOODBURY, CT 06798.

**VICINITY MAP** 

### PROJECT DIRECTORY

BRYAN JODIE A PROPERTY OWNER: DISH Wireless L.L.C. 85 PAPER MILL RD

SITE INFORMATION

WOODBURY, CT 06798

MONOPOLE

857528

LITCHFIELD

41° 34' 23.07" N 41.573083 N

73.227639 W

05100

040-032A

DIRECTIONS FROM DANBURY MUNICIPAL AIRPORT:

SITE LOCATION

TOWN OF WOODBURY

ADDRESS:

COUNTY:

TOWER TYPE:

TOWER CO SITE ID:

LATITUDE (NAD 83):

ZONING JURISDICTION:

ZONING DISTRICT:

PARCEL NUMBER:

OCCUPANCY GROUP:

TELEPHONE COMPANY: TBD

TOWER APP NUMBER: 553361

LONGITUDE (NAD 83): 73° 13' 39.51" W

5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120

TOWER OWNER: CROWN CASTLE

2000 CORPORATE DRIVE CANONSBURG, PA 15317 (877) 486-9377

SITE DESIGNER: INFINIGY

2500 W. HIGGINS RD. SUITE 500 HOFFMAN ESTATES, IL 60169

(847) 648-4068

SITE ACQUISITION: NICHOLAS CURRY

NICHOLAS.CURRY@crowncastle.com

CONSTRUCTION MANAGER: JAVIER SOTO

JAVIER SOTTO@dish.com

SYED ZAIDI RF ENGINEER:

SYED.ZAIDI@dish.com

# the solutions are endless 2500 W. HIGGINS RD. SUITE 500 | HOFFAMA ESTATES, IL 60169 PHONE: 847-648-4068 | FAX: 518-690-0793 WWW.INFINIGY.COM OF CONNEC 34916 9/14/2021

5701 SOUTH SANTA FE DRIVE

LITTLETON, CO 80120

2000 CORPORATE DRIVE

CANONSBURG, PA 15317

**INFINIGY8** 

CROWN

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

DRAWN BY: CHECKED BY: APPROVED BY CJW

RFDS REV #: N/A

### CONSTRUCTION **DOCUMENTS**

	SUBMITTALS		
REV	DATE	DESCRIPTION	
A	07/01/2021	ISSUED FOR REVIEW	
0	08/31/2021	ISSUED FOR CONSTRUCTION	
	A&E PROJECT NUMBER		

6039-Z0001-C

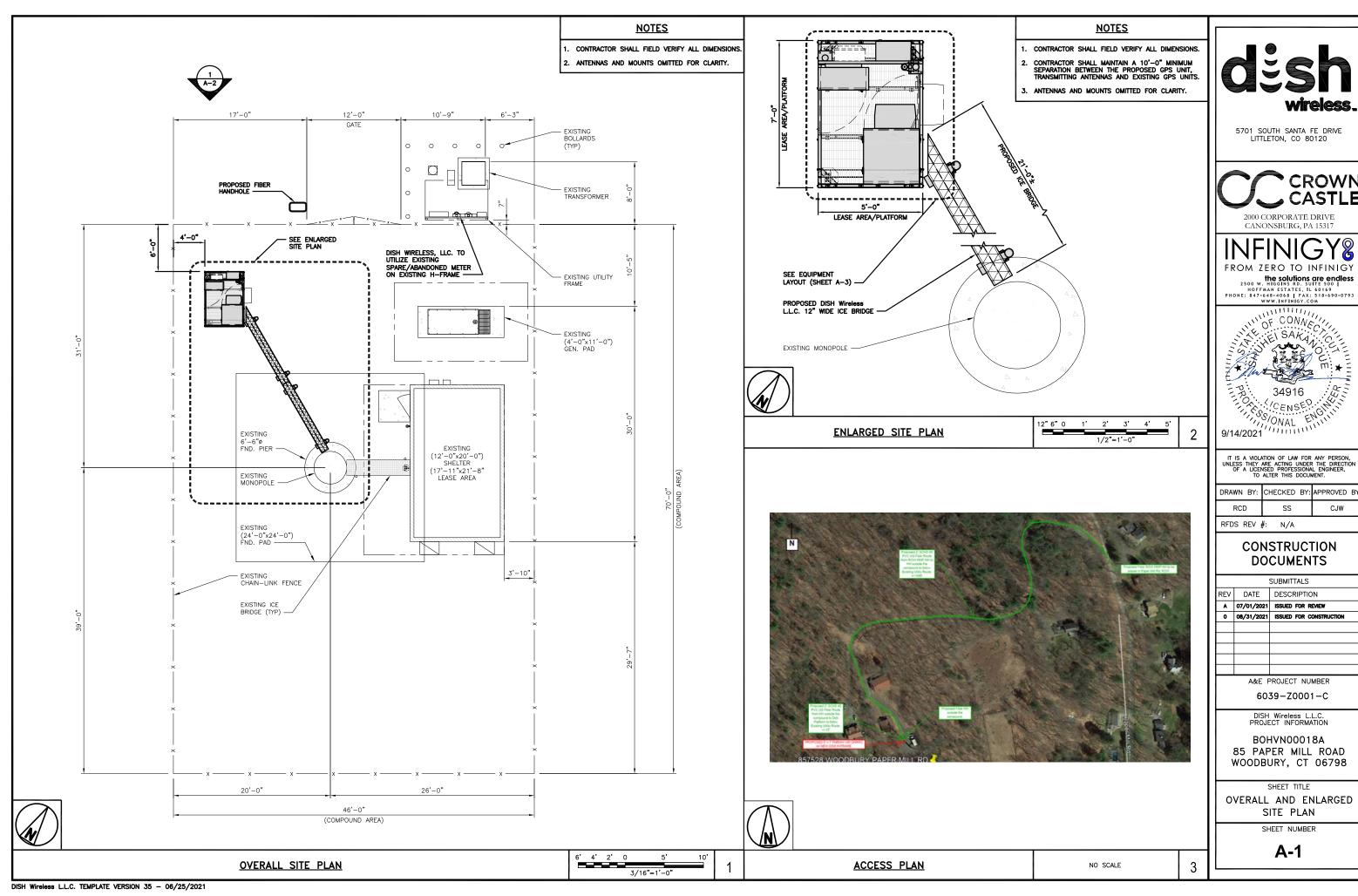
BOHVN00018A 85 PAPER MILL ROAD WOODBURY, CT 06798

> SHEET TITLE TITLE SHEET

SHEET NUMBER

T-1

DISH Wireless L.L.C. TEMPLATE VERSION 35 - 06/25/2021





### INFINIGY8

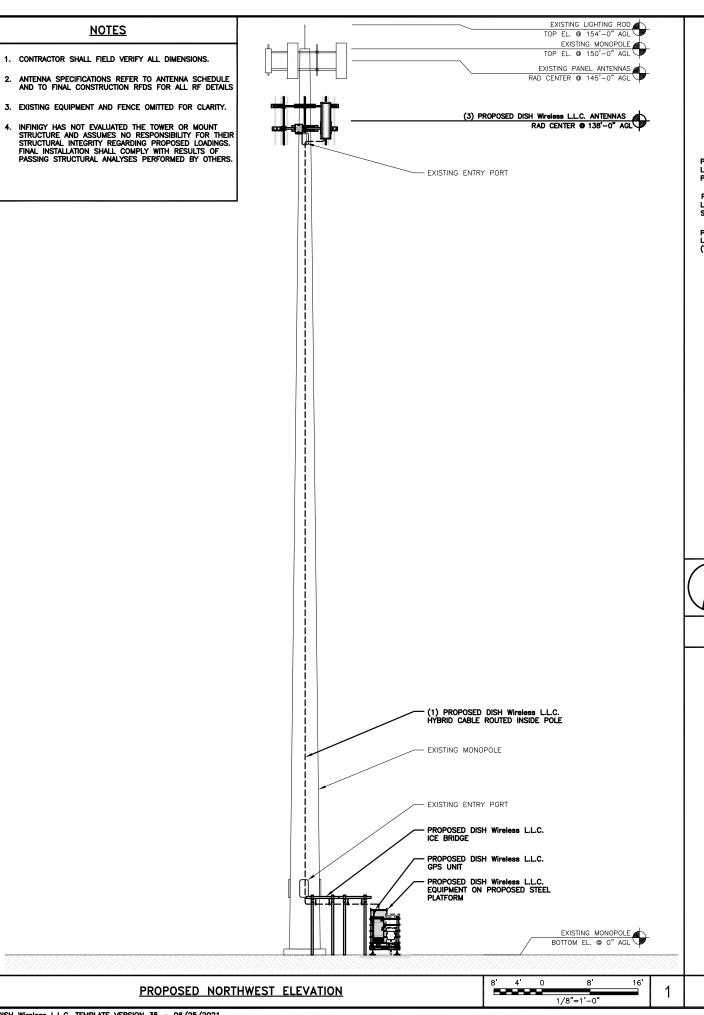


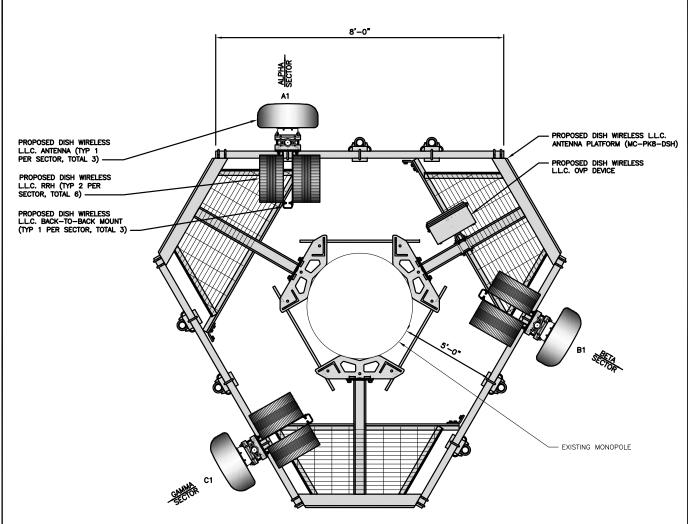
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RCD	SS	CJW
DEDG DEL II		

### CONSTRUCTION

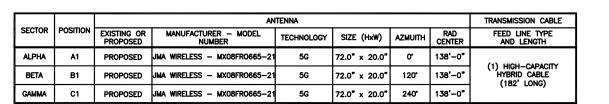
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A&E PROJECT NUMBER			

85 PAPER MILL ROAD





**ANTENNA LAYOUT** 



		RRH		
SECTOR	POSITION	MANUFACTURER — MODEL NUMBER	TECHNOLOGY	
ALPHA	A1	FUJITSU - TA08025-B604	5G	
	A1	FUJITSU - TA08025-B605	5G	
BETA	B1	FUJITSU - TA08025-B604	5G	
	B1	FUJITSU - TA08025-B605	5G	
GAMMA	C1	FUJITSU - TA08025-B604	5G	
GAMMA	C1	FUJITSU - TA08025-B605	5G	

#### NOTES

CONTRACTOR TO REFER TO FINAL CONSTRUCTION RFDS FOR ALL RF DETAILS.

2" 6" 0

3/4"=1'-0'

NO SCALE

ANTENNA AND RRH MODELS MAY CHANGE DUE TO EQUIPMENT AVAILABILITY. ALL EQUIPMENT CHANGES MUST BE APPROVED AND REMAIN IN COMPLIANCE WITH THE PROPOSED DESIGN AND STRUCTURAL ANALYSES.

5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



2000 CORPORATE DRIVE CANONSBURG, PA 15317

### INFINIGY8 FROM ZERO TO INFINIGY

the solutions are endless
2500 w. HIGGINS RD. SUITE 500 |
HOFFANN ESTATES, IL 60169
PHONE: 847-648-4086 | FAX: 518-690-0793
WWW.INFINIGY.COM



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ı	DRAWN BY:	CHECKED BY:	APPROVED BY:	
ı	RCD	SS	CJW	

RFDS REV #: N/A

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6039-Z0001-C

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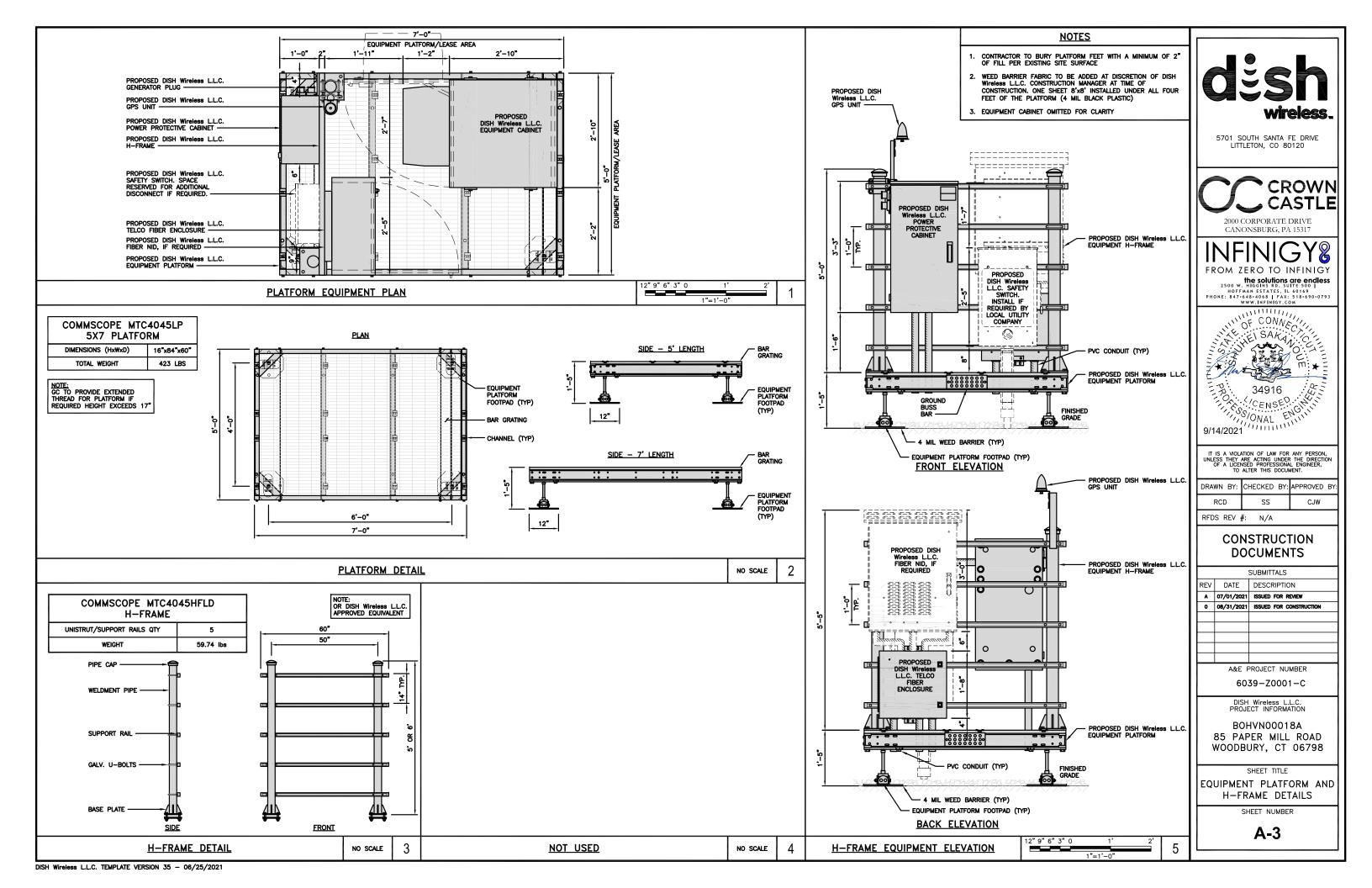
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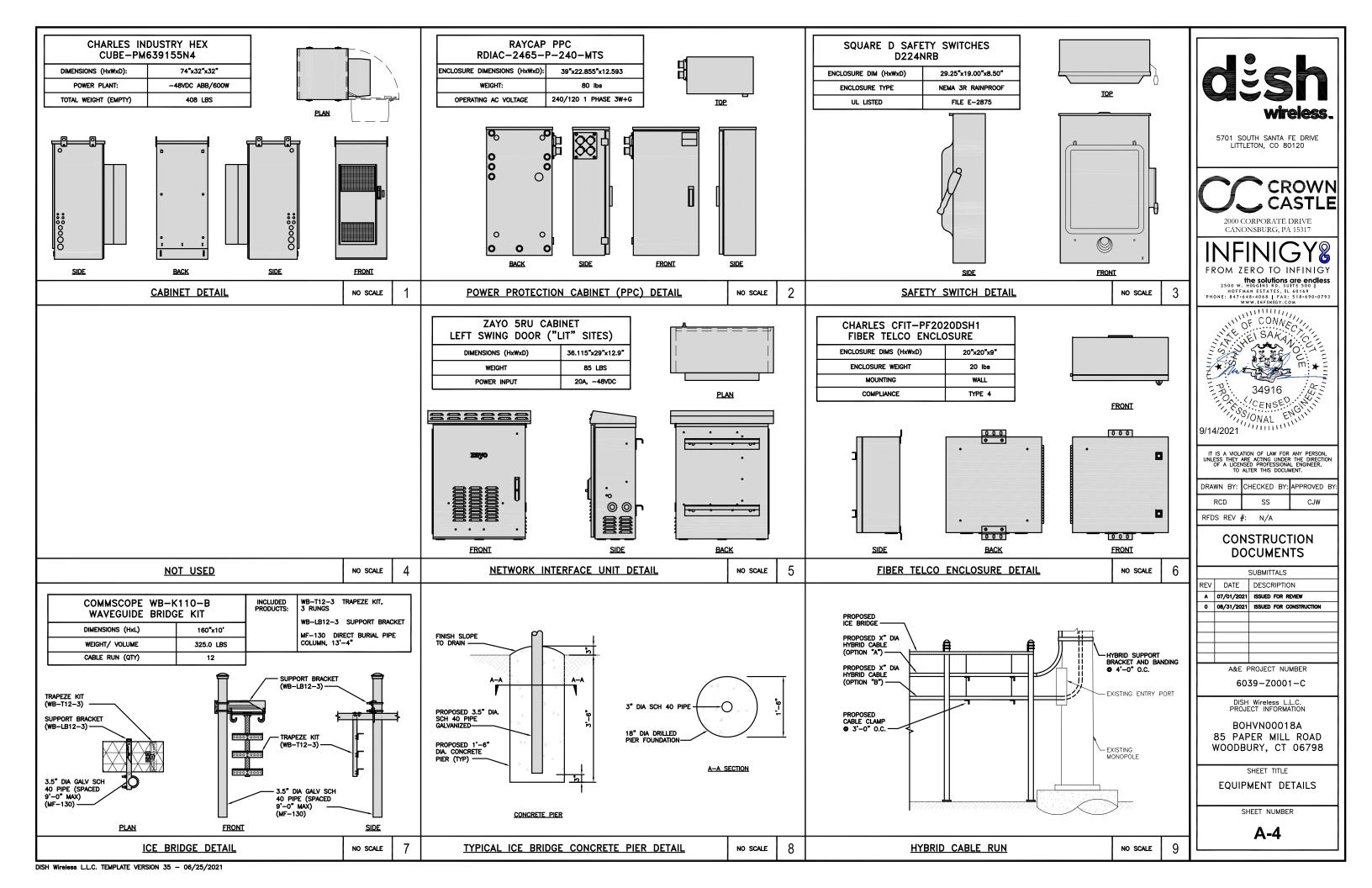
ELEVATION, ANTENNA LAYOUT AND SCHEDULE

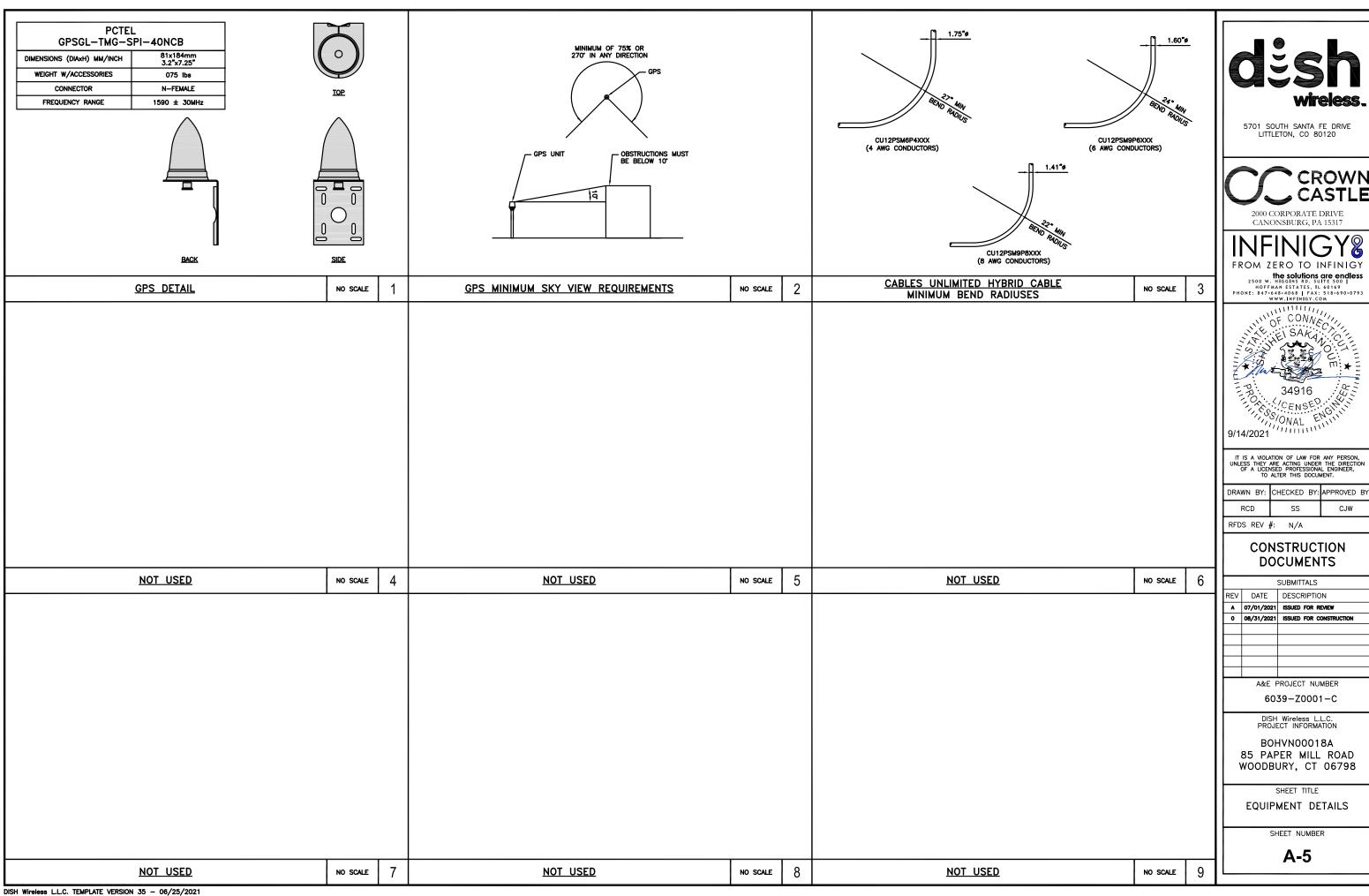
SHEET NUMBER

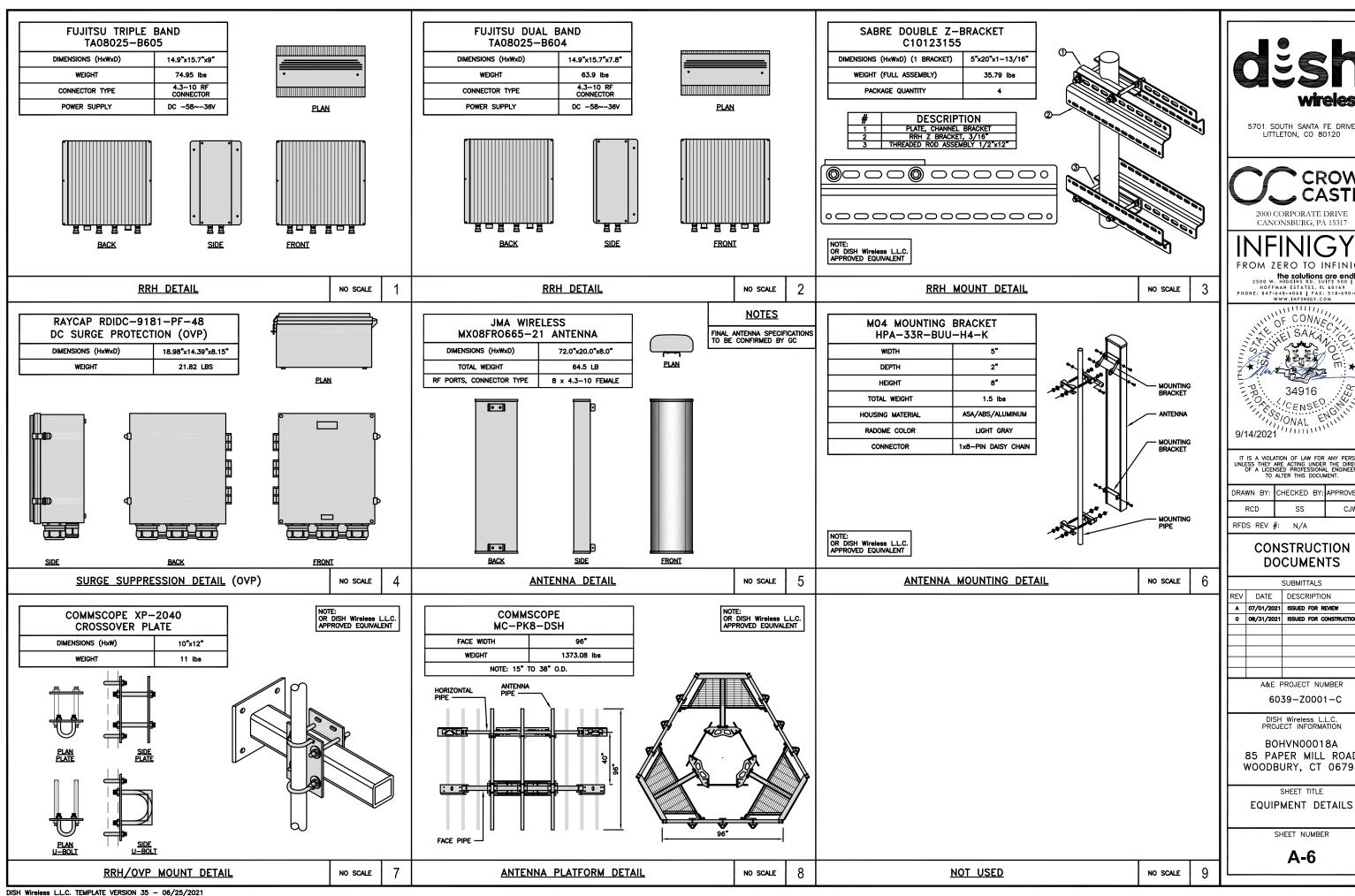
**A-2** 

**ANTENNA SCHEDULE** 









5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



2000 CORPORATE DRIVE CANONSBURG, PA 15317

the solutions are endless
2500 W. HIGGINS RD. SUITE 500 |
HOFFMAN ESTATES, 1L 60169
PHONE: 847-648-4088 | FAX: 518-690-0793
WWW.INFINIGY.COM



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	CHECKED DI.	APPROVED BY
RCD	SS	CJM

RFDS REV #: N/A

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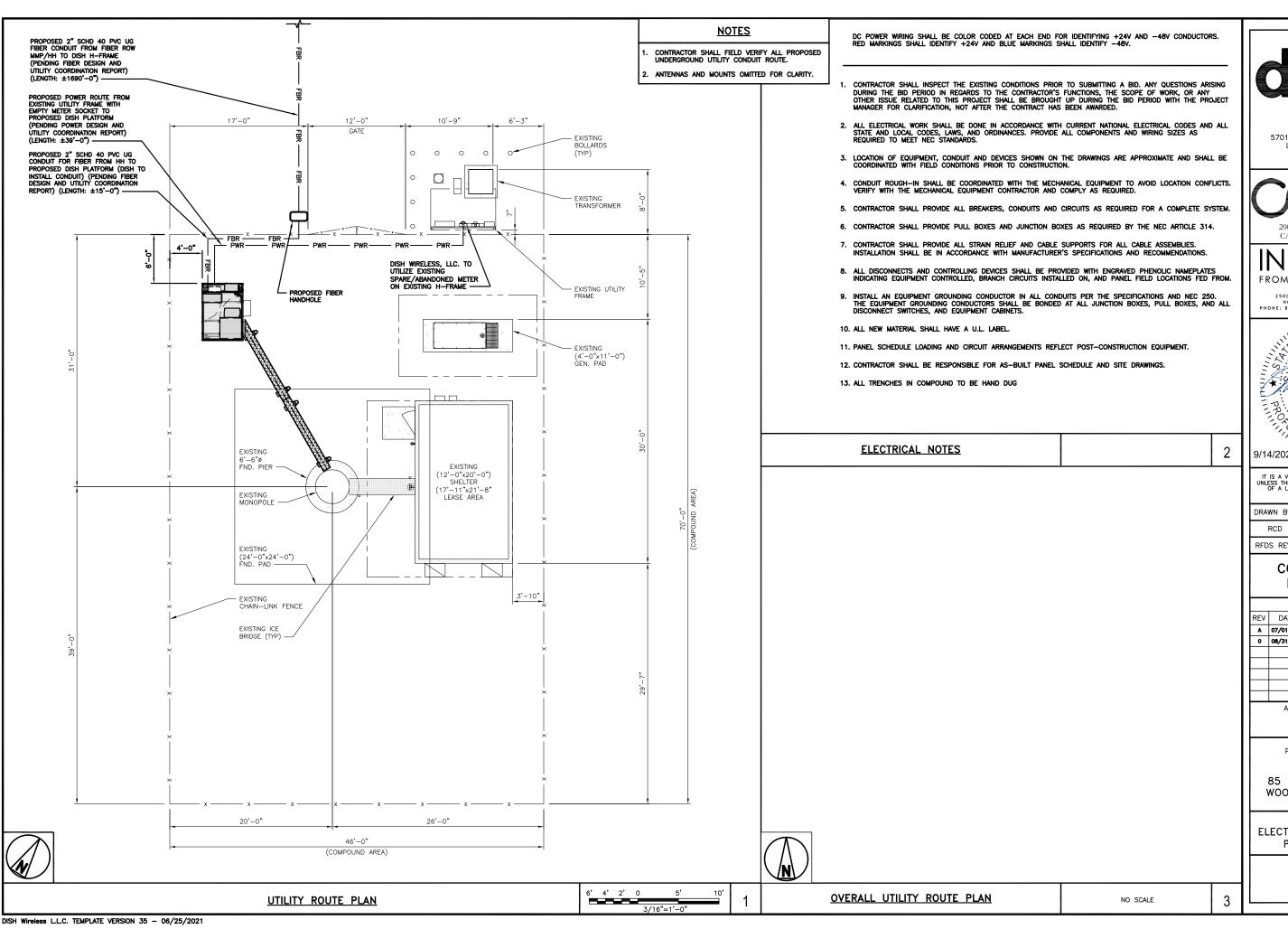
6039-Z0001-C

BOHVN00018A 85 PAPER MILL ROAD WOODBURY, CT 06798

SHEET TITLE

SHEET NUMBER

**A-6** 



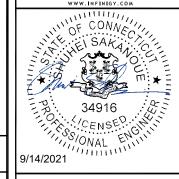
5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



2000 CORPORATE DRIVE CANONSBURG, PA 15317

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the solutions are endless
2500 w. Higgins RD. Suite 500 |
HOFFMAN ESTATES, IL 60169
PHONE: 847-648-4068 | FAX: 518-690-0793
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DOD		
RCD	SS	CJW

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0	08/31/2021	ISSUED FOR CONSTRUCTION			
	A&E F	PROJECT NUMBER			

6039-Z0001-C

DISH Wireless L.L.C. PROJECT INFORMATION

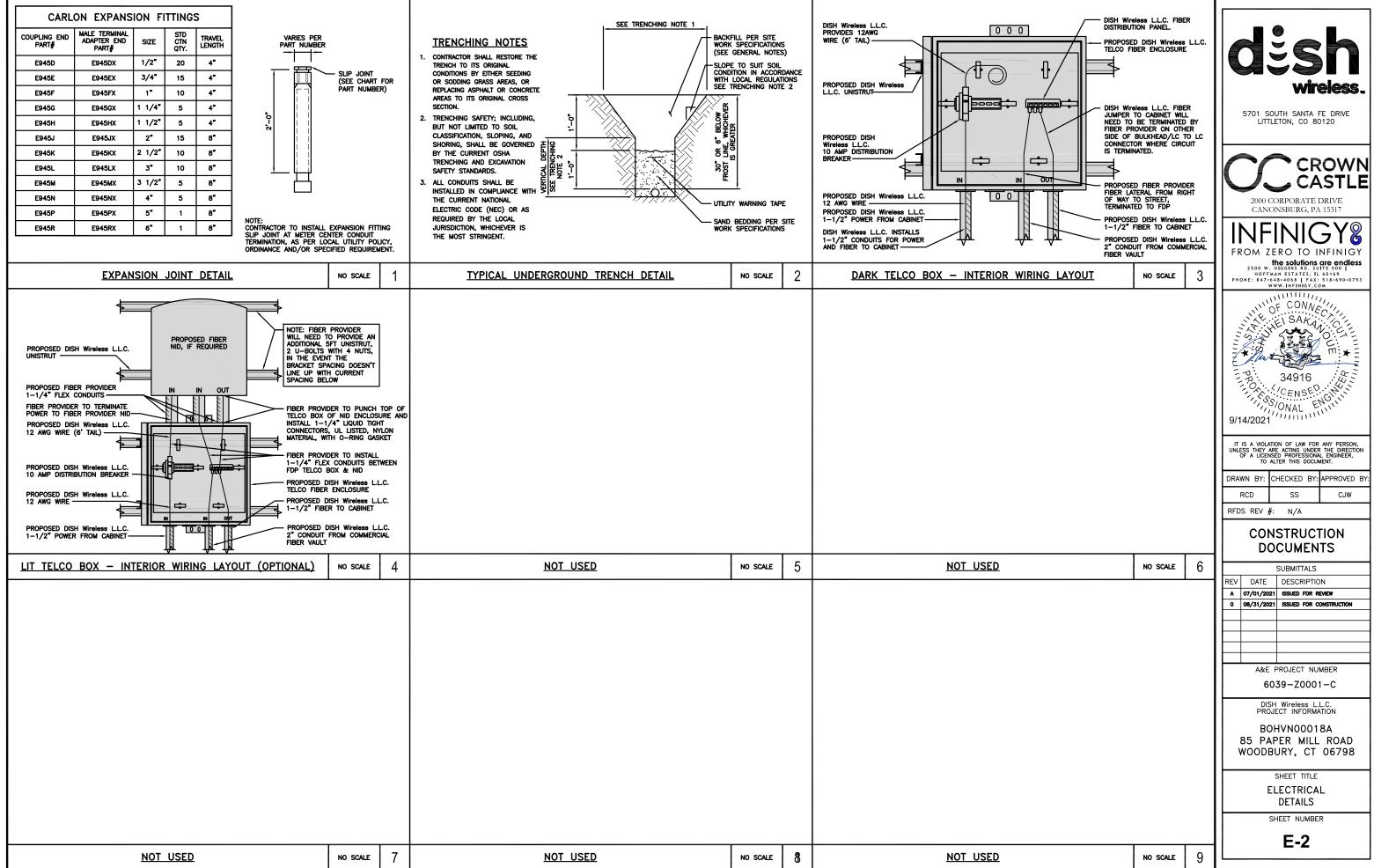
BOHVN00018A 85 PAPER MILL ROAD WOODBURY, CT 06798

SHEET TITLE

ELECTRICAL/FIBER ROUTE PLAN AND NOTES

SHEET NUMBER

E-1



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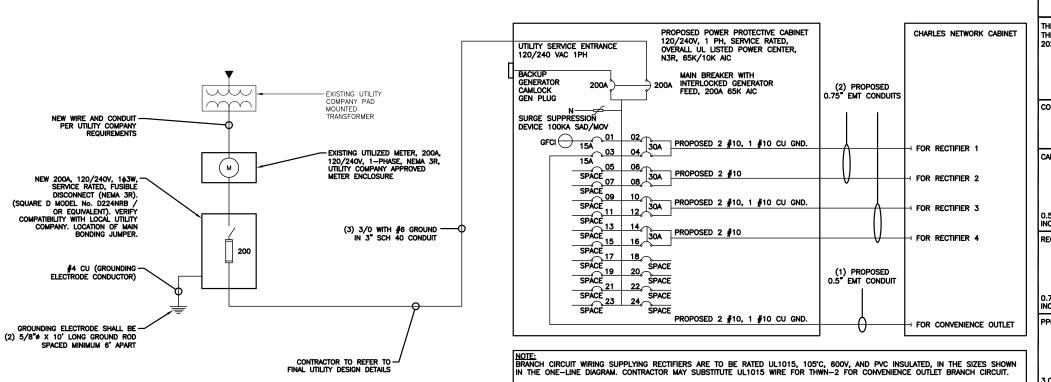
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### CONSTRUCTION

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BOHVN00018A 85 PAPER MILL ROAD



<u>NOTES</u>

THE (2) CONDUITS WITH (4) CURRENT CARRYING CONDUCTORS EACH, SHALL APPLY THE ADJUSTMENT FACTOR OF 80% PER 2014/17 NEC TABLE 310.15(B)(3)(a) OR 2020 NEC TABLE 310.15(C)(1) FOR UL1015 WIRE.

#12 FOR 15A-20A/1P BREAKER: 0.8 x 30A = 24.0A #10 FOR 25A-30A/2P BREAKER: 0.8 x 40A = 32.0A #8 FOR 35A-40A/2P BREAKER: 0.8 x 55A = 44.0A #6 FOR 45A-60A/2P BREAKER: 0.8 x 75A = 60.0A

CONDUIT SIZING: AT 40% FILL PER NEC CHAPTER 9, TABLE 4, ARTICLE 358. 0.5" CONDUIT — 0.122 SQ. IN AREA

0.75" CONDUIT - 0.213 SQ. IN AREA 2.0" CONDUIT - 1.316 SQ. IN AREA 3.0" CONDUIT - 2.907 SQ. IN AREA

CABINET CONVENIENCE OUTLET CONDUCTORS (1 CONDUIT): USING THWN-2, CU.

#10 - 0.0211 SQ. IN X 2 = 0.0422 SQ. IN #10 - 0.0211 SQ. IN X 1 = 0.0211 SQ. IN <GROUND TOTAL = 0.0633 SQ. IN

0.5" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (3) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

RECTIFIER CONDUCTORS (2 CONDUITS): USING UL1015, CU.

#10 - 0.0266 SQ. IN X 4 = 0.1064 SQ. IN #10 - 0.0082 SQ. IN X 1 = 0.0082 SQ. IN <BARE GROUND

= 0.1146 SQ. IN

0.75" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (5) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

PPC FEED CONDUCTORS (1 CONDUIT): USING THWN, CU.

3/0 - 0.2679 SQ. IN X 3 = 0.8037 SQ. IN #6 - 0.0507 SQ. IN X 1 = 0.0507 SQ. IN <GROUND

TOTAL = 0.8544 SQ. IN

3.0" SCH 40 PVC CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (4) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

PPC ONE-LINE DIAGRAM

BREAKERS REQUIRED: (4) 30A, 2P BREAKER - SQUARE D P/N:QO230 (1) 15A, 1P BREAKER - SQUARE D P/N:QO115

NO SCALE 1

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PROJECT INFORMATION

BOHVN00018A 85 PAPER MILL ROAD WOODBURY, CT 06798

SHEET TITLE

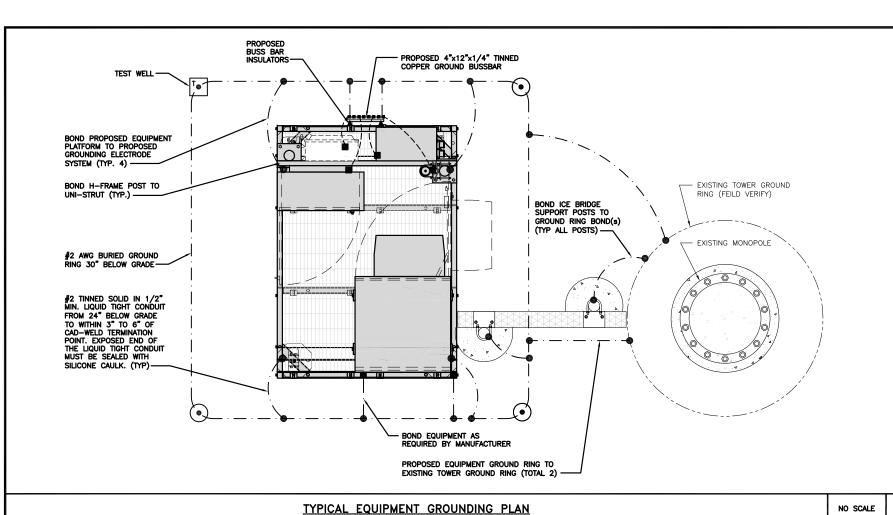
ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE

SHEET NUMBER

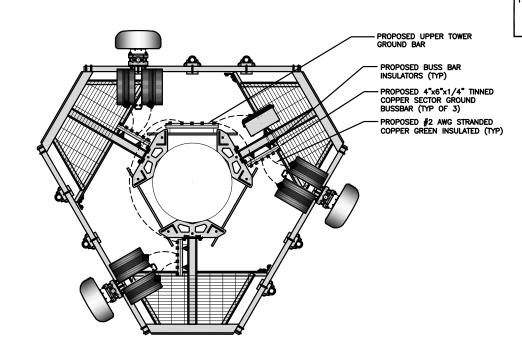
E-3

PANEL SCHEDULE

2



**NOTES** 



EXOTHERMIC CONNECTION MECHANICAL CONNECTION

🖶 GROUND BUS BAR

GROUND ROD

 $(\bullet)$ 

NO SCALE

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DISH Wireless L.L.C. PROJECT INFORMATION

BOHVN00018A 85 PAPER MILL ROAD WOODBURY, CT 06798

SHEET TITLE

GROUNDING PLANS AND NOTES

SHEET NUMBER

G-1

ANTENNAS AND OVP SHOWN ARE GENERIC AND NOT REFERENCING TO A SPECIFIC MANUFACTURER. THIS LAYOUT IS FOR REFERENCE

### **GROUNDING LEGEND**

TEST GROUND ROD WITH INSPECTION SLEEVE

---- #6 AWG STRANDED & INSULATED

— · — · — #2 AWG SOLID COPPER TINNED

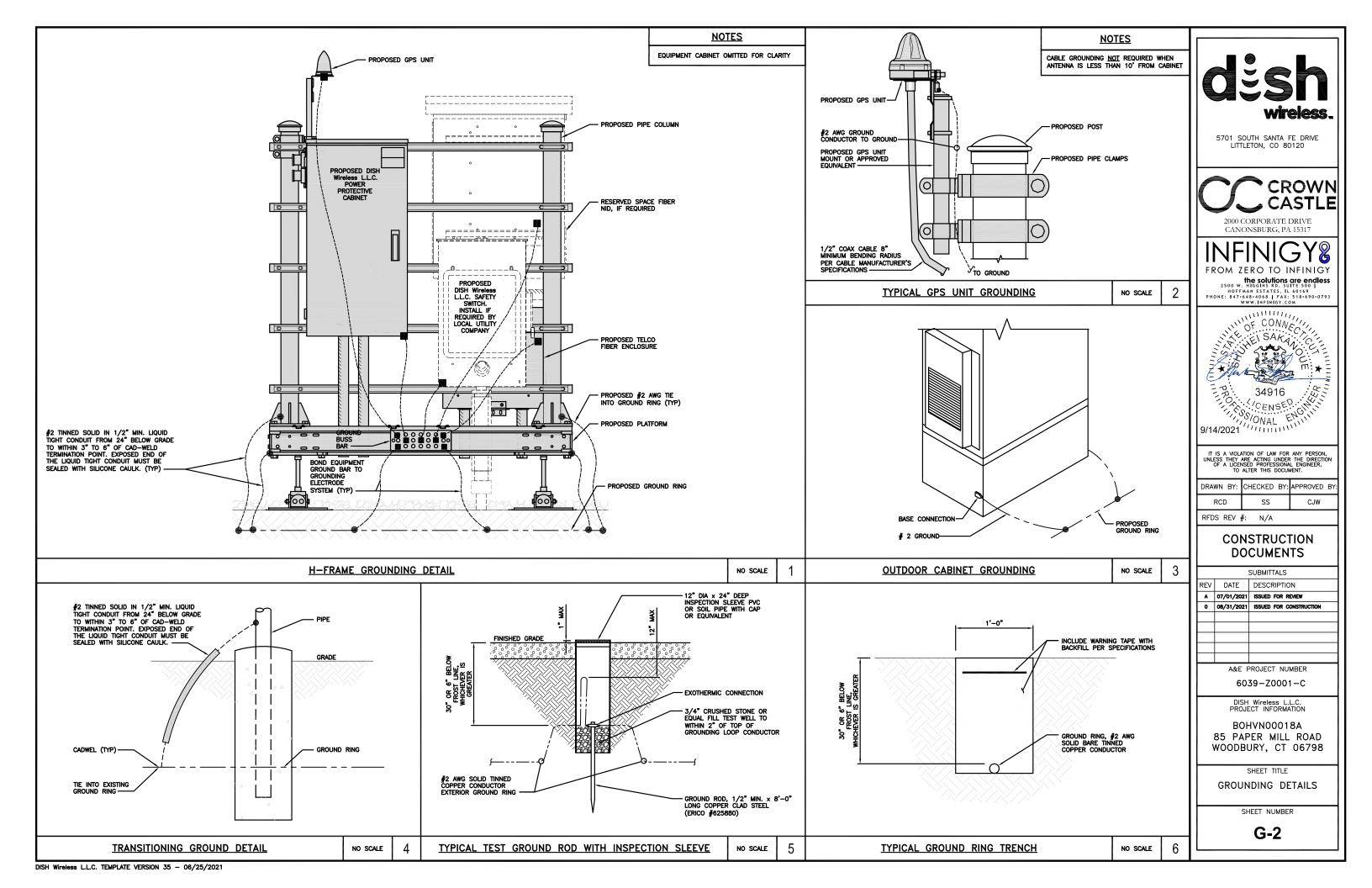
▲ BUSS BAR INSULATOR

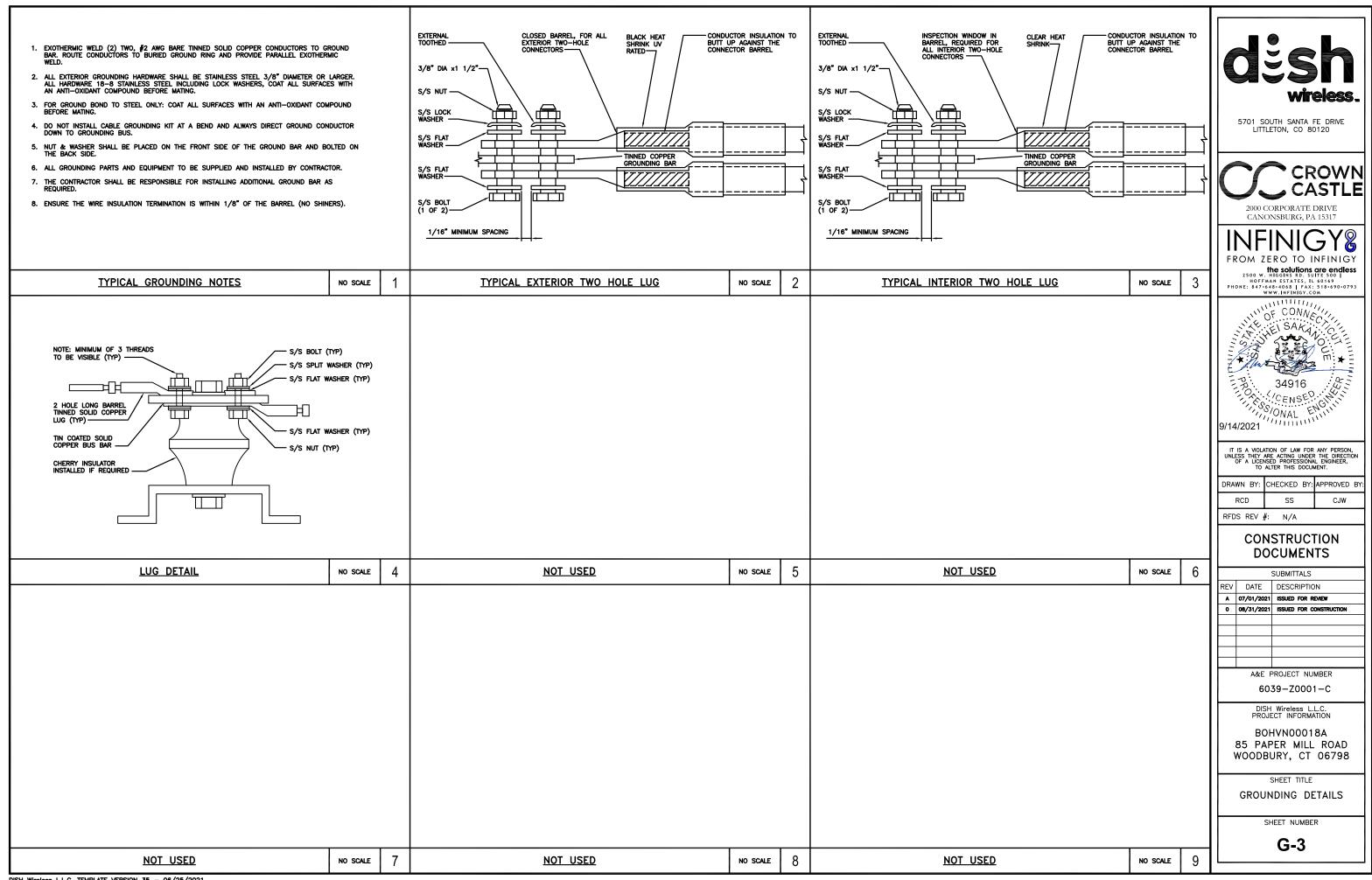
- 1. GROUNDING IS SHOWN DIAGRAMMATICALLY ONLY.
- CONTRACTOR SHALL GROUND ALL EQUIPMENT AS A COMPLETE SYSTEM. GROUNDING SHALL BE IN COMPLIANCE WITH NEC SECTION 250 AND DISH Wireless L.L.C. GROUNDING AND BONDING REQUIREMENTS AND MANUFACTURER'S SPECIFICATIONS.
- 3. ALL GROUND CONDUCTORS SHALL BE COPPER; NO ALUMINUM CONDUCTORS SHALL BE USED.

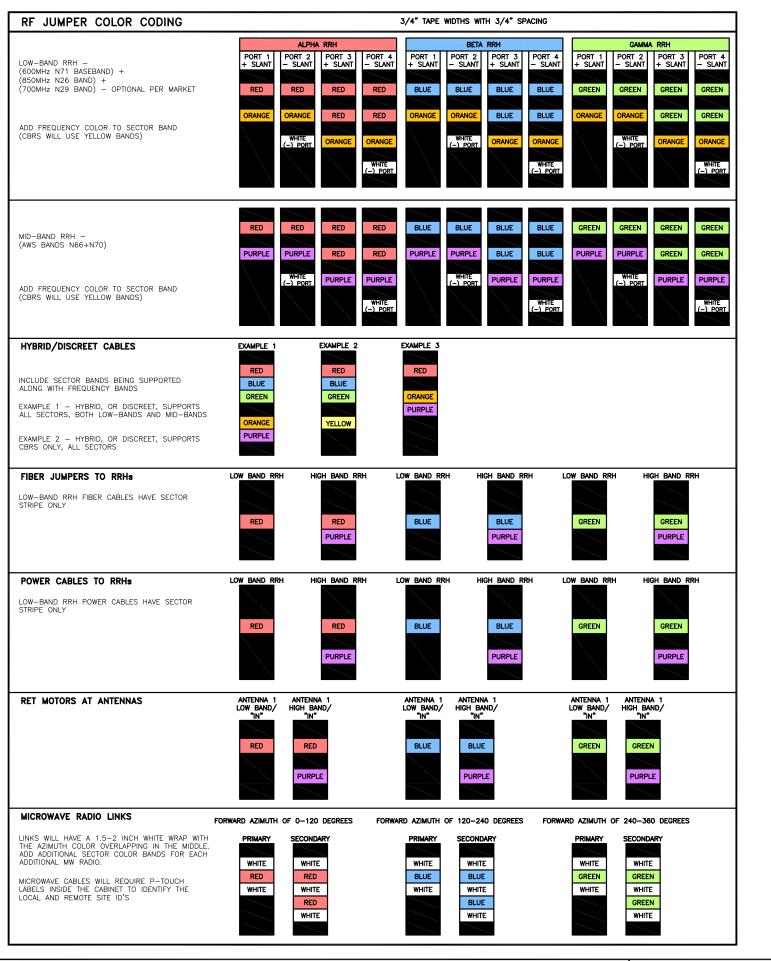
#### **GROUNDING KEY NOTES**

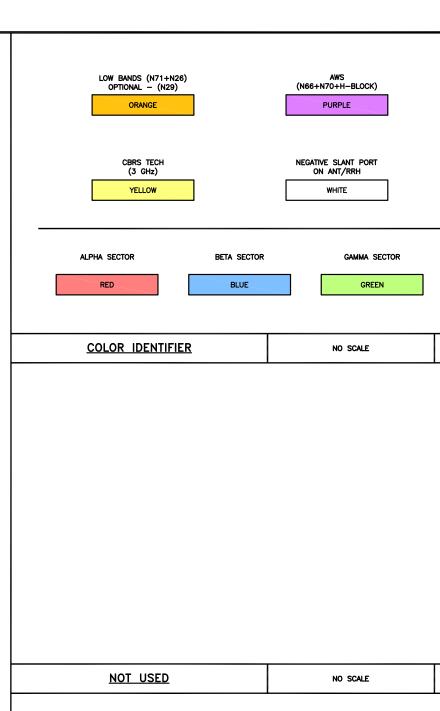
- (A) EXTERIOR GROUND RING: #2 AWG SOLID COPPER, BURIED AT A DEPTH OF AT LEAST 30 INCHES BELOW GRADE, OR 6 INCHES BELOW THE FROST LINE AND APPROXIMATELY 24 INCHES FROM THE EXTERIOR WALL OR FOOTING.
- B TOWER GROUND RING: THE GROUND RING SYSTEM SHALL BE INSTALLED AROUND AN ANTENNA TOWER'S LEGS, AND/OR GUY ANCHORS. WHERE SEPARATE SYSTEMS HAVE BEEN BROWNER FOR THE FOUNDATION OF THE FOUNDATION AND/OR GUY ANCHORS. WHERE SEPARATE SYSTEMS HAVE BEEN PROVIDED FOR THE TOWER AND THE BUILDING, AT LEAST TWO BONDS SHALL BE MADE BETWEEN THE TOWER RING GROUND SYSTEM AND THE BUILDING RING GROUND SYSTEM USING MINIMUM #2 AWG SOLID COPPER CONDUCTORS.
- C Interior ground Ring: #2 awg stranded green insulated copper conductor extended around the perimeter of the equipment area. All non-telecommunications related metallic objects found within a site shall be grounded to the interior ground ring with #6 awg stranded green
- D BOND TO INTERIOR GROUND RING: #2 AWG SOLID TINNED COPPER WIRE PRIMARY BONDS SHALL BE PROVIDED AT LEAST AT FOUR POINTS ON THE INTERIOR GROUND RING, LOCATED AT THE CORNERS OF THE
- (E) GROUND ROD: UL LISTED COPPER CLAD STEEL. MINIMUM 1/2" DIAMETER BY EIGHT FEET LONG. GROUND RODS SHALL BE INSTALLED WITH INSPECTION SLEEVES. GROUND RODS SHALL BE DRIVEN TO THE DEPTH OF GROUND RING CONDUCTOR.
- F CELL REFERENCE GROUND BAR: POINT OF GROUND REFERENCE FOR ALL COMMUNICATIONS EQUIPMENT FRAMES. ALL BONDS ARE MADE WITH #2 AWG UNLESS NOTED OTHERWISE STRANDED GREEN INSULATED COPPER CONDUCTORS. BOND TO GROUND RING WITH (2) #2 SOLID TINNED COPPER CONDUCTORS.
- G HATCH PLATE GROUND BAR: BOND TO THE INTERIOR GROUND RING WITH TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS. WHEN A HATCH-PLATE AND A CELL REFERENCE GROUND BAR ARE BOTH PRESENT, THE CRGB MUST BE CONNECTED TO THE HATCH-PLATE AND TO THE INTERIOR GROUND RING USING (2) TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS EACH.
- (H) EXTERIOR CABLE ENTRY PORT GROUND BARS; LOCATED AT THE ENTRANCE TO THE CELL SITE BUILDING, BOND TO GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTORS WITH AN EXOTHERMIC WELD AND INSPECTION SLEEVE.
- 1) TELCO GROUND BAR: BOND TO BOTH CELL REFERENCE GROUND BAR OR EXTERIOR GROUND RING.
- J FRAME BONDING: THE BONDING POINT FOR TELECOM EQUIPMENT FRAMES SHALL BE THE GROUND BUS THAT IS NOT ISOLATED FROM THE EQUIPMENTS METAL FRAMEWORK.
- ) Interior unit bonds: Metal frames, cabinets and individual metallic units located with the area of the interior ground ring require a #6 awg stranded green insulated copper bond to the
- L FENCE AND GATE GROUNDING: METAL FENCES WITHIN 7 FEET OF THE EXTERIOR GROUND RING OR OBJECTS BONDED TO THE EXTERIOR GROUND RING SHALL BE BONDED TO THE GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTOR AT AN INTERVAL NOT EXCEEDING 25 FEET. BONDS SHALL BE MADE AT EACH CAST DEPARTS AND ACCOUNTS.
- M <u>Exterior unit bonds:</u> Metallic objects, external to or mounted to the building, shall be bonded to the exterior ground ring. Using #2 tinned solid copper wire
- N ICE BRIDGE SUPPORTS: EACH ICE BRIDGE LEG SHALL BE BONDED TO THE GROUND RING WITH #2 AWG BARE TINNED COPPER CONDUCTOR. PROVIDE EXOTHERMIC WELDS AT BOTH THE ICE BRIDGE LEG AND BURIED
- DURING ALL DC POWER SYSTEM CHANGES INCLUDING DC SYSTEM CHANGE OUTS, RECTIFIER REPLACEMENTS OR ADDITIONS, BREAKER DISTRIBUTION CHANGES, BATTERY ADDITIONS, BATTERY REPLACEMENTS AND INSTALLATIONS OR CHANGES TO DC CONVERTER SYSTEMS IT SHALL BE REQUIRED THAT SERVICE CONTRACTORS VERIFY ALL DC POWER SYSTEMS ARE EQUIPPED WITH A MASTER DC SYSTEM RETURN GROUND CONDUCTOR FROM THE DC POWER SYSTEM COMMON RETURN BUS DIRECTLY CONNECTED TO THE CELL SITE REFERENCE (COLUMN) BAR
- (P) TOWER TOP COLLECTOR BUSS BAR IS TO BE MECHANICALLY BONDED TO PROPOSED ANTENNA MOUNT COLLAR.

REFER TO DISH Wireless L.L.C. GROUNDING NOTES.











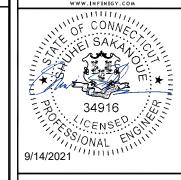
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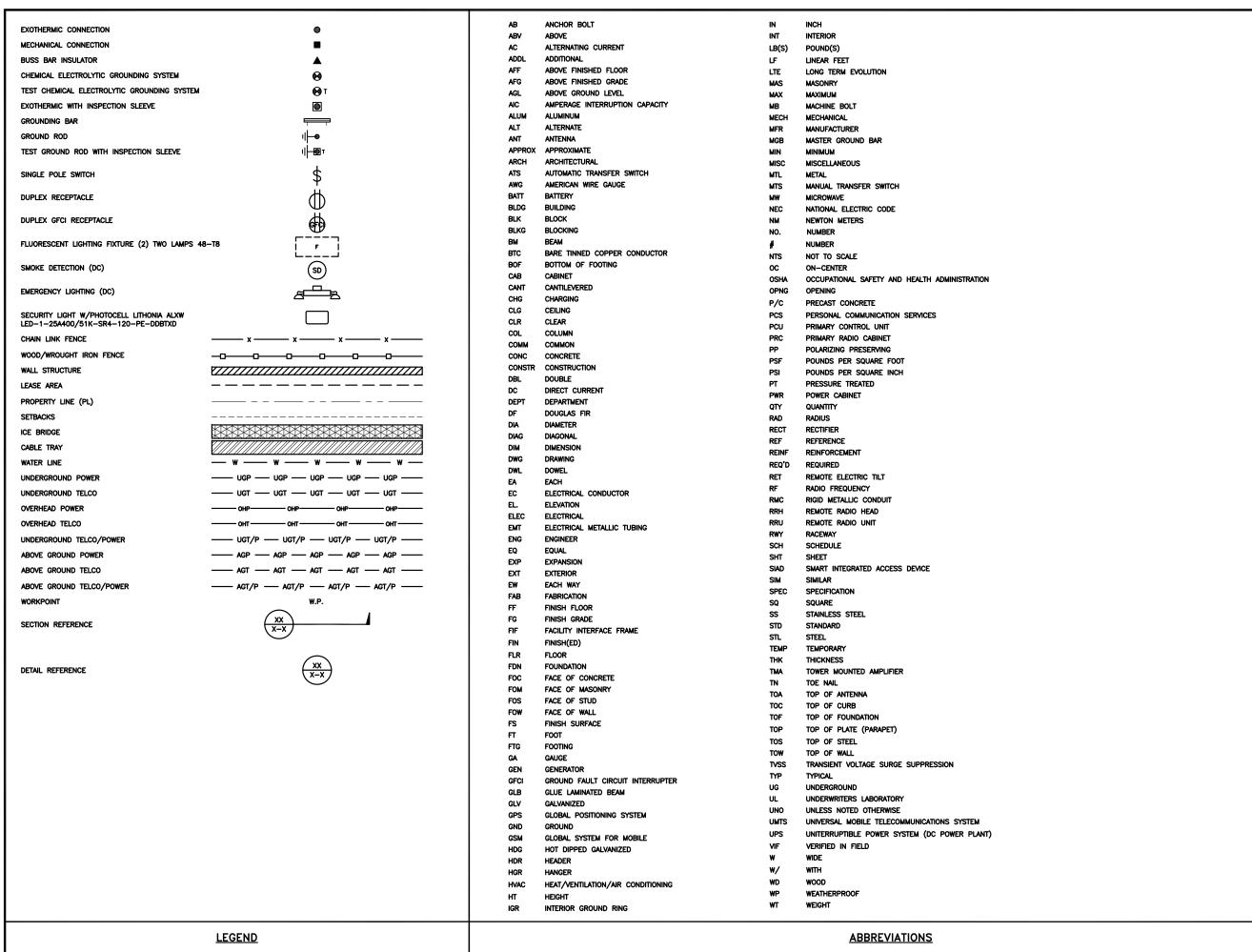
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SHEET TITLE CABLE COLOR CODES

SHEET NUMBER

RF-1

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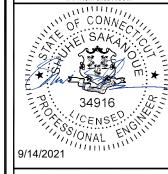
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6039-Z0001-C

PROJECT INFORMATIO

BOHVN00018A 85 PAPER MILL ROAD WOODBURY, CT 06798

SHEET TITLE

LEGEND AND ABBREVIATIONS

SHEET NUMBER

#### SITE ACTIVITY REQUIREMENTS:

- 1. NOTICE TO PROCEED NO WORK SHALL COMMENCE PRIOR TO CONTRACTOR RECEIVING A WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE DISH Wireless L.L.C. AND TOWER OWNER NOC & THE DISH Wireless L.L.C. AND TOWER CONSTRUCTION MANAGER.
- 2. "LOOK UP" DISH Wireless L.L.C. AND TOWER OWNER SAFETY CLIMB REQUIREMENT:

THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR DISH Wireless L.L.C. AND DISH Wireless L.L.C. AND TOWER OWNER POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.

- 3. PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.
- 4. ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILTY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND DISH WIRELESS L.L.C. AND TOWER OWNER STANDARDS, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA—322 (LATEST EDITION).
- 5. ALL SITE WORK TO COMPLY WITH DISH Wireless L.L.C. AND TOWER OWNER INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON DISH Wireless L.L.C. AND TOWER OWNER TOWER SITE AND LATEST VERSION OF ANSI/TIA-1019-A-2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS."
- 6. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY DISH Wireless L.L.C. AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 9. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES INCLUDING PRIVATE LOCATES SERVICES PRIOR TO THE START OF CONSTRUCTION.
- 10. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFETY PROCEDURES.
- 11. ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND DISH PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.
- 12. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- 13. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF DISH Wireless L.L.C. AND TOWER OWNER, AND/OR LOCAL UTILITIES.
- 14. THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.
- 15. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER'S EQUIPMENT AND TOWER AREAS.
- 16. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
- 17. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
- 18. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
- 19. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
- 20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS AND RADIOS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 21. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.
- 22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

#### **GENERAL NOTES:**

1.FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR:GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION

CARRIER:DISH Wireless L.L.C.

TOWER OWNER:TOWER OWNER

- 2. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
- 3. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY.
- 4. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.
- 5. SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
- 6. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CARRIER POC AND TOWER OWNER.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- 9. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 10. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION
- 11. CONTRACTOR IS TO PERFORM A SITE INVESTIGATION, BEFORE SUBMITTING BIDS, TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.
- 12. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF DISH Wireless L.L.C. AND TOWER OWNER
- 13. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 14. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.



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	RCD	SS	CJW	
	RFDS REV	#: N/A		

### CONSTRUCTION DOCUMENTS

6039-Z0001-C

BOHVN00018A 85 PAPER MILL ROAD WOODBURY, CT 06798

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

#### CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST—IN—PLACE CONCRETE.
- 2. UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED TO BE 1000 psf.
- 3. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) OF 3000 psi AT 28 DAYS, UNLESS NOTED OTHERWISE. NO MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90'F AT TIME OF PLACEMENT.
- 4. CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL CONTAIN AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER-TO-CEMENT RATIO (W/C) OF 0.45.
- 5. ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE STANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS:

#4 BARS AND SMALLER 40 ksi

#5 BARS AND LARGER 60 ksi

- 6. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
- · CONCRETE EXPOSED TO EARTH OR WEATHER:
- #6 BARS AND LARGER 2"
- #5 BARS AND SMALLER 1-1/2"
- CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
- SLAB AND WALLS 3/4"
- BEAMS AND COLUMNS 1-1/2\*
- 7. A TOOLED EDGE OR A 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

#### **ELECTRICAL INSTALLATION NOTES:**

- 1. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
- 2. CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED AND TRIP HAZARDS ARE ELIMINATED.
- 3. WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.
- 4. ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.
- 4.1. ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.
- 4.2. ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE PRE THE GOVERNING JURISDICTION.
- 5. EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR—CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.
- 6. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT ID'S).
- 7. PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
- 8. TIE WRAPS ARE NOT ALLOWED.
- 9. ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 10. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 11. POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS OTHERWISE SPECIFIED.
- 12. POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 13. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75° C (90° C IF AVAILABLE).
- 14. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.
- 15. ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

- . ELECTRICAL METALLIC TUBING (EMT) OR METAL-CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- 17. SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE GRADE PVC CONDUIT.
- 18. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
- 19. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION—TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
- 20. CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND THE NEC.
- 21. WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS (WIREMOLD SPECMATE WIREWAY).
- 22. SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).
- 23. CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE TEMPORARILY CAPPED FLUSH TO FINISH GRADE TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE.
- 24. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY—COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3 (OR BETTER) FOR EXTERIOR LOCATIONS.
- 25. METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY—COATED OR NON—CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 26. NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 27. THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR DISH Wireless L.L.C. AND TOWER OWNER BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
- 28. THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.
- 29. INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "DISH Wireless L.L.C.".
- 30. ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.



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### CONSTRUCTION DOCUMENTS

	SUBMITTALS							
REV	DATE	DESCRIPTION						
A 07/01/2021		ISSUED FOR REVIEW						
0	08/31/2021	ISSUED FOR CONSTRUCTION						
	A&E F	PROJECT NUMBER						
1								

DISH Wireless L.L.C.

BOHVNOO018A 85 PAPER MILL ROAD WOODBURY, CT 06798

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

#### **GROUNDING NOTES:**

- 1. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- 2. THE CONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.
- 4. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 5. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- 6. EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS.
- 7. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
- 8. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.
- 9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- 10. USE OF  $90^{\circ}$  BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN  $45^{\circ}$  BENDS CAN BE ADEQUATELY SUPPORTED.
- 11. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- 12. ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.
- COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.
- 14. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.
- 15. APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- 16. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
- 17. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- 18. BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.
- 19. GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.
- 20. ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON-METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD-WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).
- 21. BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/O COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY). DO NOT ATTACH GROUNDING TO FIRE SPRINKLER SYSTEM PIPES.



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2522 254 #4:					

RFDS REV #: N/A

### CONSTRUCTION DOCUMENTS

SUBMITTALS							
REV	DATE	DESCRIPTION					
A	07/01/2021	ISSUED FOR REVIEW					
0	08/31/2021	ISSUED FOR CONSTRUCTION					
	∧ 9aE E	DECT NUMBER					

A&E PROJECT NUMBER

6039-Z0001-C

DISH Wireless L.L.C PROJECT INFORMATION

BOHVNOOO18A 85 PAPER MILL ROAD WOODBURY, CT 06798

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

# Exhibit D

**Structural Analysis Report** 

Date: May 28, 2021



Crown Castle 2000 Corporate Drive Canonsburg, PA 15317 (724) 416-2000

Subject: Structural Analysis Report

Carrier Designation: DISH Network Co-Locate

Site Number: BOHVN00018A Site Name: CT-CCI-T-857528

Crown Castle Designation: BU Number: 857528

Site Name: WOODBURY PAPER MILL RD

 JDE Job Number:
 645155

 Work Order Number:
 1966221

 Order Number:
 553361 Rev. 1

Engineering Firm Designation: Crown Castle Project Number: 1966221

Site Data: 85 PAPER MILL ROAD, WOODBURY, LITCHFIELD County, CT

Latitude 41° 34′ 23.07″, Longitude -73° 13′ 39.51″

150 Foot - Monopole Tower

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration

**Sufficient Capacity - 52.6%** 

This analysis utilizes an ultimate 3-second gust wind speed of 120 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - "Analysis Criteria".

Structural analysis prepared by: Michael Lopienski

Respectfully submitted by:

Bradley E. Byrom, P.E., S.E. Senior Project Engineer



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tnxTower Output

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**Additional Calculations** 

### 1) INTRODUCTION

This tower is a 150 ft Monopole tower designed by Ehresmann Engineering 1995.

### 2) ANALYSIS CRITERIA

TIA-222 Revision: TIA-222-H

Risk Category:

Wind Speed: 120 mph

Exposure Category:BTopographic Factor:1Ice Thickness:1.5 inWind Speed with Ice:50 mphService Wind Speed:60 mph

**Table 1 - Proposed Equipment Configuration** 

Mounting Level (ft)	Elevation	Number of Antennas	Antenna Manufacturer	Antenna Model		Feed Line Size (in)
		3	fujitsu	TA08025-B604		
		3	fujitsu	TA08025-B605		
138.0	138.0	3	jma wireless	MX08FRO665-21 w/ Mount Pipe	1	1-1/2
		1	raycap	RDIDC-9181-PF-48		
		1	tower mounts	Commscope MC-PK8-DSH		

**Table 2 - Other Considered Equipment** 

Mounting Level (ft)	- ANTENNA MICHAEL		Number of Feed Lines	Feed Line Size (in)		
		1	cci antennas	DMP65R-BU4D w/ Mount Pipe		
		2	cci antennas	DMP65R-BU6D w/ Mount Pipe		
		1	cci antennas	OPA65R-BU4D w/ Mount Pipe		1-5/8 3/8 5/8 3/4
		2	cci antennas	OPA65R-BU6D w/ Mount Pipe		
		3	ericsson	RRUS 4449 B5/B12		
	148.0	3	ericsson	RRUS 4478 B14	6	
147.0		3	ericsson	RRUS 8843 B2/B66A	2	
		3	powerwave technologies	P90-14-XLH-RR w/ Mount Pipe	2 3	
		powerwave technologies	TT19-08BP111-001			
		2	raycap	DC6-48-60-18-8F		
	147.0	1	tower mounts	Miscellaneous [NA 507-1]		
	147.0	1	tower mounts	Platform Mount [LP 601-1]		

### 3) ANALYSIS PROCEDURE

**Table 3 - Documents Provided** 

Document	Reference	Source
4-GEOTECHNICAL REPORTS	4570959	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	4724414	CCISITES
4-TOWER MANUFACTURER DRAWINGS	4724415	CCISITES

### 3.1) Analysis Method

tnxTower (version 8.0.9.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

### 3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

#### 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary)** 

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	150 - 104.5	Pole	TP28.1875x18x0.1875	1	-8.95	988.77	48.5	Pass
L2	104.5 - 68.75	Pole	TP35.75x26.8609x0.25	2	-13.42	1673.43	48.8	Pass
L3	68.75 - 34	Pole	TP43x34.0833x0.3125	3	-19.95	2519.29	43.7	Pass
L4	34 - 0	Pole	TP50x41.0375x0.3125	4	-29.27	3027.25	50.2	Pass
						Summary		
						Pole (L4)	50.2	Pass
						Rating =	50.2	Pass

Table 5 - Tower Component Stresses vs. Capacity - LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	21.8	Pass
1	Base Plate	0	31.7	Pass
1	Base Foundation (Structure)	0	25.4	Pass
1	Base Foundation (Soil Interaction)	0	52.6	Pass

Structure Rating (max from all components) =	52.6%

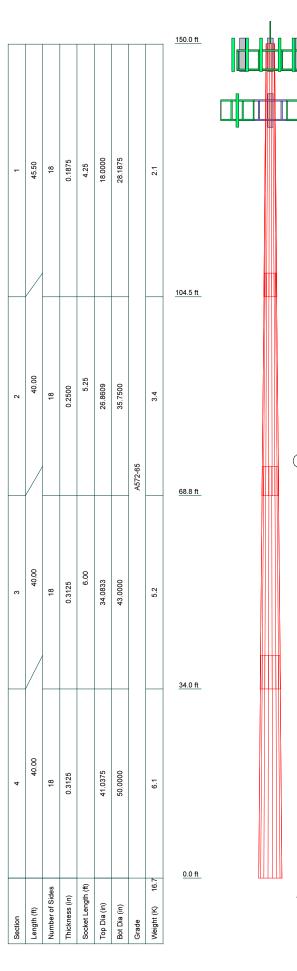
Notes:

<sup>1)</sup> See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

# APPENDIX A TNXTOWER OUTPUT

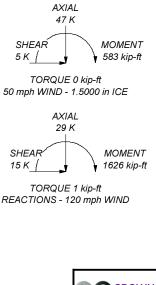


#### **MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

#### **TOWER DESIGN NOTES**

- 1. Tower designed for Exposure B to the TIA-222-H Standard.
- 2. Tower designed for a 120 mph basic wind in accordance with the TIA-222-H Standard.
- 3. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
- Deflections are based upon a 60 mph wind.
   Tower Risk Category II.
- 6. Topographic Category 1 with Crest Height of 0.00 ft 7. TOWER RATING: 50.2%



15 K

ALL REACTIONS ARE FACTORED



### **Tower Input Data**

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- Tower base elevation above sea level: 528.00 ft.
- Basic wind speed of 120 mph.
- Risk Category II.
- Exposure Category B.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0.00 ft.
- Nominal ice thickness of 1.5000 in.
- Ice thickness is considered to increase with height.
- Ice density of 56 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 60 mph.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.
- Tower analysis based on target reliabilities in accordance with Annex S.
- Load Modification Factors used: Kes(Fw) = 0.95, Kes(ti) = 0.85.
- Maximum demand-capacity ratio is: 1.05.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

### **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

√ Use Code Stress Ratios

✓ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile

Include Bolts In Member Capacity

Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.

Autocalc Torque Arm Areas

Add IBC .6D+W Combination

Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs

Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

 ✓ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption

Poles

- ✓ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
- √ Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known

## **Tapered Pole Section Geometry**

Section	Elevation	Section	Splice	Number	Тор	Bottom	Wall	Bend	Pole Grade
		Length	Length	of	Diameter	Diameter	Thickness	Radius	
	ft	ft	ft	Sides	in	in	in	in	

Section	Elevation	Section	Splice	Number	Тор	Bottom	Wall	Bend	Pole Grade
	ft	Length ft	Length ft	of Sides	Diameter in	Diameter in	Thickness in	Radius in	
L1	150.00-104.50	45.50	4.25	18	18.0000	28.1875	0.1875	0.7500	A572-65 (65 ksi)
L2	104.50-68.75	40.00	5.25	18	26.8609	35.7500	0.2500	1.0000	À572-65 (65 ksi)
L3	68.75-34.00	40.00	6.00	18	34.0833	43.0000	0.3125	1.2500	À572-65 (65 ksi)
L4	34.00-0.00	40.00		18	41.0375	50.0000	0.3125	1.2500	À572-65 (65 ksi)

Section	Tip Dia.	Area	1	r	С	I/C	J	It/Q	W	w/t
	in	in²	in⁴	in	in	in³	in⁴	in²	in	
L1	18.2488	10.6007	424.9328	6.3234	9.1440	46.4712	850.4248	5.3013	2.8380	15.136
	28.5934	16.6635	1650.5160	9.9400	14.3193	115.2655	3303.2038	8.3333	4.6310	24.699
L2	28.1958	21.1158	1889.1397	9.4469	13.6453	138.4457	3780.7651	10.5599	4.2875	17.15
	36.2629	28.1692	4485.0722	12.6025	18.1610	246.9617	8976.0460	14.0873	5.8520	23.408
L3	35.7493	33.4964	4826.3494	11.9886	17.3143	278.7490	9659.0495	16.7514	5.4487	17.436
	43.6151	42.3407	9747.5744	15.1541	21.8440	446.2358	19507.974 9	21.1744	7.0180	22.458
L4	42.9875	40.3941	8464.0370	14.4574	20.8470	406.0065	16939.211 2	20.2009	6.6726	21.352
	50.7231	49.2838	15372.193 1	17.6391	25.4000	605.2045	30764.613 4	24.6466	8.2500	26.4

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade Adjust. Factor A <sub>i</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in				in	in	in
L1 150.00-			1	1	1			
104.50								
L2 104.50-			1	1	1			
68.75								
L3 68.75-			1	1	1			
34.00								
L4 34.00-0.00			1	1	1			

# Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face	Allow	Exclude	Componen	Placement	Total	Number	Clear	Width or	Perimete	Weight
	or	Shield	From	t		Number	Per Row	Spacing	Diamete	r	
	Leg		Torque	Type	ft			in	r		plf
	•		Calculation						in	in	
*											

## Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Exclude From	Componen	Placement	Total Number		$C_A A_A$	Weight
	Leg	Siliela	Torque	Туре	ft	Number		ft²/ft	plf
			Calculation	)					
*** 147 ***									
LDF7-50A(1-5/8)	В	No	No	Inside Pole	147.00 - 0.00	12	No Ice	0.00	0.82
							1/2" Ice	0.00	0.82
							1" Ice	0.00	0.82

Description	Face or	Allow Shield	Exclude From	Componen t	Placement	Total Number		$C_A A_A$	Weight
	Leg		Torque Calculation	Type	ft			ftº/ft	plf
							2" Ice	0.00	0.82
FB-L98B-034-	В	No	No	Inside Pole	147.00 - 0.00	1	No Ice	0.00	0.06
XXX(3/8)							1/2" Ice	0.00	0.06
							1" Ice	0.00	0.06
							2" Ice	0.00	0.06
WR-VG82ST-	В	No	No	Inside Pole	147.00 - 0.00	2	No Ice	0.00	0.31
BRDA(5/8)							1/2" Ice	0.00	0.31
							1" Ice	0.00	0.31
							2" Ice	0.00	0.31
WR-VG86ST-	С	No	No	Inside Pole	147.00 - 0.00	2	No Ice	0.00	0.58
BRD(3/4)							1/2" Ice	0.00	0.58
							1" Ice	0.00	0.58
							2" Ice	0.00	0.58
FB-L98B-034-	С	No	No	Inside Pole	147.00 - 0.00	1	No Ice	0.00	0.06
XXX(3/8)							1/2" Ice	0.00	0.06
, ,							1" Ice	0.00	0.06
							2" Ice	0.00	0.06
***									
****									
Safety Line 3/8	С	No	No	CaAa (Out	150.00 - 0.00	1	No Ice	0.04	0.22
•				Of Face)			1/2" Ice	0.14	0.75
							1" Ice	0.24	1.28
							2" Ice	0.44	2.34
5/8 rod/step	С	No	No	CaAa (Out	150.00 - 0.00	1	No Ice	0.02	0.27
·				Of Face)			1/2" Ice	0.12	0.70
							1" Ice	0.22	1.74
							2" Ice	0.42	5.65
****									
CU12PSM9P6XXX	С	No	No	Inside Pole	138.00 - 0.00	1	No Ice	0.00	2.35
(1-1/2)							1/2" Ice	0.00	2.35
` ,							1" Ice	0.00	2.35
							2" Ice	0.00	2.35
*									
*									

# Feed Line/Linear Appurtenances Section Areas

Tower Sectio	Tower	Face	$A_R$	$A_F$	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face	Weight
	Elevation		-0	•0			
n	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
L1	150.00-104.50	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.45
		С	0.000	0.000	0.000	2.616	0.15
L2	104.50-68.75	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.38
		С	0.000	0.000	0.000	2.056	0.15
L3	68.75-34.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.37
		С	0.000	0.000	0.000	1.998	0.14
L4	34.00-0.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.36
		С	0.000	0.000	0.000	1.955	0.14

# Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Sectio	Elevation	or	Thickness			In Face	Out Face	_
n	ft	Leg	in	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
L1	150.00-104.50	Α	1.458	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.45

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Sectio	Elevation	or	Thickness			In Face	Out Face	
n	ft	Leg	in	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
		С		0.000	0.000	0.000	29.145	0.37
L2	104.50-68.75	Α	1.403	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.38
		С		0.000	0.000	0.000	22.900	0.32
L3	68.75-34.00	Α	1.332	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.37
		С		0.000	0.000	0.000	21.505	0.30
L4	34.00-0.00	Α	1.190	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.36
		С		0.000	0.000	0.000	20.074	0.28

## **Feed Line Center of Pressure**

Section	Elevation	CP <sub>X</sub>	CPz	CP <sub>X</sub>	CPz
				lce	Ice
	ft	in	in	in	in
L1	150.00-104.50	-0.4467	0.2579	-2.0969	1.2106
L2	104.50-68.75	-0.4522	0.2611	-2.2881	1.3210
L3	68.75-34.00	-0.4551	0.2627	-2.3296	1.3450
L4	34.00-0.00	-0.4568	0.2638	-2.3139	1.3359

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

	Discrete Tower Loads											
Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight			
			ft ft ft	0	ft		ft²	ft²	К			
Lighting Rod 5/8" x 5'	С	None		0.0000	152.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.31 0.83 1.32 1.96	0.31 0.83 1.32 1.96	0.03 0.03 0.04 0.07			
*** 147 *** P90-14-XLH-RR w/ Mount Pipe	Α	From Leg	4.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	5.30 5.69 6.09 6.90	4.05 4.67 5.29 6.57	0.05 0.09 0.15 0.27			
P90-14-XLH-RR w/ Mount Pipe	В	From Leg	4.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	5.30 5.69 6.09 6.90	4.05 4.67 5.29 6.57	0.05 0.09 0.15 0.27			
P90-14-XLH-RR w/ Mount Pipe	С	From Leg	4.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	5.30 5.69 6.09 6.90	4.05 4.67 5.29 6.57	0.05 0.09 0.15 0.27			
TT19-08BP111-001	Α	From Leg	4.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.55 0.64 0.74 0.97	0.44 0.53 0.63 0.84	0.02 0.02 0.03 0.05			
TT19-08BP111-001	Α	From Leg	4.00	0.0000	147.00	No Ice	0.55	0.44	0.02			

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	٥	ft		ft <sup>2</sup>	ft²	K
			0.00 1.00			1/2" Ice 1" Ice	0.64 0.74 0.97	0.53 0.63 0.84	0.02 0.03 0.05
TT19-08BP111-001	С	From Leg	4.00 0.00 1.00	0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	0.55 0.64 0.74	0.44 0.53 0.63	0.02 0.02 0.03
DC6-48-60-18-8F	В	From Leg	4.00 0.00	0.0000	147.00	2" Ice No Ice 1/2"	0.97 1.21 1.89	0.84 1.21 1.89	0.05 0.02 0.04
			1.00			lce 1" lce 2" lce	2.11 2.57	2.11 2.57	0.04 0.07 0.13
6' x 2" Mount Pipe	В	From Leg	4.00 0.00 0.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice	1.43 1.92 2.29 3.06	1.43 1.92 2.29 3.06	0.02 0.03 0.05 0.09
Platform Mount [LP 601-1]	С	None		0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	28.50 31.69 34.87 41.23	28.50 31.69 34.87 41.23	1.12 1.68 2.28 3.65
Miscellaneous [NA 507-1]	С	None		0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	4.56 6.39 8.18 11.66	4.56 6.39 8.18 11.66	0.25 0.31 0.40 0.66
DMP65R-BU6D w/ Mount Pipe	Α	From Leg	4.00 0.00 1.00	0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	11.96 12.70 13.46 15.02	5.97 6.63 7.30 8.69	0.11 0.20 0.30 0.53
DMP65R-BU6D w/ Mount Pipe	В	From Leg	4.00 0.00 1.00	0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	11.96 12.70 13.46 15.02	5.97 6.63 7.30 8.69	0.11 0.20 0.30 0.53
DMP65R-BU4D w/ Mount Pipe	С	From Leg	4.00 0.00 1.00	0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	7.53 8.04 8.57 9.68	3.79 4.23 4.68 5.63	0.09 0.16 0.22 0.39
OPA65R-BU6D w/ Mount Pipe	Α	From Leg	4.00 0.00 1.00	0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	12.25 13.00 13.76 15.34	6.05 6.71 7.39 8.79	0.09 0.18 0.27 0.51
OPA65R-BU6D w/ Mount Pipe	В	From Leg	4.00 0.00 1.00	0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	12.25 13.00 13.76 15.34	6.05 6.71 7.39 8.79	0.09 0.18 0.27 0.51
OPA65R-BU4D w/ Mount Pipe	С	From Leg	4.00 0.00 1.00	0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	8.10 8.65 9.21 10.39	4.03 4.50 4.98 5.98	0.08 0.14 0.21 0.38
RRUS 4449 B5/B12	Α	From Leg	4.00 0.00 1.00	0.0000	147.00	2" Ice No Ice 1/2" Ice 1" Ice	1.97 2.14 2.33 2.72	1.41 1.56 1.73 2.07	0.07 0.09 0.11
RRUS 4449 B5/B12	В	From Leg	4.00	0.0000	147.00	2" Ice No Ice	1.97	1.41	0.16 0.07

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	٥	ft		ft²	ft²	K
			0.00			1/2"	2.14	1.56	0.09
			1.00			Ice 1" Ice 2" Ice	2.33 2.72	1.73 2.07	0.11 0.16
RRUS 4449 B5/B12	С	From Leg	4.00	0.0000	147.00	No Ice	1.97	1.41	0.07
			0.00			1/2"	2.14	1.56	0.09
			1.00			Ice	2.33	1.73	0.11
RRUS 4478 B14	А	Erom Log	4.00	0.0000	147.00	1" Ice 2" Ice No Ice	2.72 1.84	2.07 1.06	0.16 0.06
KKUS 4476 B14	А	From Leg	0.00	0.0000	147.00	1/2"	2.01	1.00	0.08
			1.00			Ice	2.19	1.34	0.09
						1" Ice	2.57	1.66	0.14
						2" Ice			
RRUS 4478 B14	В	From Leg	4.00	0.0000	147.00	No Ice	1.84	1.06	0.06
			0.00			1/2"	2.01	1.20	0.08
			1.00			Ice	2.19	1.34	0.09
						1" Ice 2" Ice	2.57	1.66	0.14
RRUS 4478 B14	С	From Leg	4.00	0.0000	147.00	No Ice	1.84	1.06	0.06
	•		0.00	0.000		1/2"	2.01	1.20	0.08
			1.00			Ice	2.19	1.34	0.09
						1" Ice 2" Ice	2.57	1.66	0.14
RRUS 8843 B2/B66A	Α	From Leg	4.00	0.0000	147.00	No Ice	1.64	1.35	0.07
			0.00			1/2"	1.80	1.50	0.09
			1.00			lce 1" lce	1.97 2.32	1.65 1.99	0.11 0.16
						2" Ice	2.02	1.55	0.10
RRUS 8843 B2/B66A	В	From Leg	4.00	0.0000	147.00	No Ice	1.64	1.35	0.07
			0.00			1/2"	1.80	1.50	0.09
			1.00			Ice	1.97	1.65	0.11
						1" Ice 2" Ice	2.32	1.99	0.16
RRUS 8843 B2/B66A	С	From Leg	4.00	0.0000	147.00	No Ice	1.64	1.35	0.07
11100 0040 <i>D2/B</i> 00/1	Ü	1 Tom Log	0.00	0.0000	147.00	1/2"	1.80	1.50	0.09
			1.00			Ice	1.97	1.65	0.11
						1" Ice	2.32	1.99	0.16
500 40 00 40 05						2" Ice			
DC6-48-60-18-8F	Α	From Leg	4.00	0.0000	147.00	No Ice	1.21	1.21	0.02
			0.00 1.00			1/2" Ice	1.89 2.11	1.89 2.11	0.04 0.07
			1.00			1" Ice	2.57	2.57	0.07
						2" Ice			
***									
****	^	Г., I	4.00	0.0000	400.00	No les	0.04	4.00	0.44
MX08FRO665-21 w/ Mount Pipe	Α	From Leg	4.00 0.00	0.0000	138.00	No Ice 1/2"	8.01 8.52	4.23 4.69	0.11 0.19
wount i ipe			0.00			Ice	9.04	5.16	0.19
			0.00			1" Ice	10.11	6.12	0.52
						2" Ice			
MX08FRO665-21 w/	В	From Leg	4.00	0.0000	138.00	No Ice	8.01	4.23	0.11
Mount Pipe			0.00			1/2"	8.52	4.69	0.19
			0.00			Ice	9.04	5.16	0.29
						1" Ice 2" Ice	10.11	6.12	0.52
MX08FRO665-21 w/	С	From Leg	4.00	0.0000	138.00	No Ice	8.01	4.23	0.11
Mount Pipe		3	0.00		/-	1/2"	8.52	4.69	0.19
			0.00			Ice	9.04	5.16	0.29
						1" Ice	10.11	6.12	0.52
DDIDC 0404 DE 40	^	From Las	4.00	0.0000	120.00	2" Ice	0.04	4.00	0.00
RDIDC-9181-PF-48	Α	From Leg	4.00 0.00	0.0000	138.00	No Ice 1/2"	2.31 2.50	1.29 1.45	0.02 0.04
			0.00			Ice	2.70	1.43	0.04
			3.00			1" Ice	3.12	1.96	0.12

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		$C_A A_A$ Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	o	ft		ft <sup>2</sup>	ft²	K
						2" Ice			
TA08025-B604	Α	From Leg	4.00	0.0000	138.00	No Ice	1.96	0.98	0.06
			0.00			1/2"	2.14	1.11	0.08
			0.00			Ice	2.32	1.25	0.10
						1" Ice 2" Ice	2.71	1.55	0.15
TA08025-B604	В	From Leg	4.00	0.0000	138.00	No Ice	1.96	0.98	0.06
17400023 0004	Ь	r rom Log	0.00	0.0000	100.00	1/2"	2.14	1.11	0.08
			0.00			Ice	2.32	1.25	0.10
			0.00			1" Ice	2.71	1.55	0.15
						2" Ice	2.7 1	1.00	0.10
TA08025-B604	С	From Leg	4.00	0.0000	138.00	No Ice	1.96	0.98	0.06
.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	•		0.00	0.0000	.00.00	1/2"	2.14	1.11	0.08
			0.00			lce	2.32	1.25	0.10
						1" Ice	2.71	1.55	0.15
						2" Ice			
TA08025-B605	Α	From Leg	4.00	0.0000	138.00	No Ice	1.96	1.13	0.08
		· ·	0.00			1/2"	2.14	1.27	0.09
			0.00			Ice	2.32	1.41	0.11
						1" Ice	2.71	1.72	0.16
						2" Ice			
TA08025-B605	В	From Leg	4.00	0.0000	138.00	No Ice	1.96	1.13	0.08
			0.00			1/2"	2.14	1.27	0.09
			0.00			Ice	2.32	1.41	0.11
						1" Ice	2.71	1.72	0.16
						2" Ice			
TA08025-B605	С	From Leg	4.00	0.0000	138.00	No Ice	1.96	1.13	0.08
			0.00			1/2"	2.14	1.27	0.09
			0.00			Ice	2.32	1.41	0.11
						1" Ice	2.71	1.72	0.16
(0) 01 011 14 ( D)			4.00	0.0000	400.00	2" Ice	4.00	4.00	0.00
(2) 8' x 2" Mount Pipe	Α	From Leg	4.00	0.0000	138.00	No Ice	1.90	1.90	0.03
			0.00			1/2"	2.73	2.73	0.04
			0.00			Ice	3.40	3.40	0.06
						1" Ice	4.40	4.40	0.12
(2) 8' x 2" Mount Pipe	В	From Leg	4.00	0.0000	138.00	2" Ice No Ice	1.90	1.90	0.03
(2) 6 X 2 Would Pipe	ь	Fioni Leg	0.00	0.0000	130.00	1/2"	2.73	2.73	0.03
			0.00			Ice	3.40	3.40	0.04
			0.00			1" Ice	4.40	4.40	0.00
						2" Ice	4.40	4.40	0.12
(2) 8' x 2" Mount Pipe	С	From Leg	4.00	0.0000	138.00	No Ice	1.90	1.90	0.03
(2) 0 X 2 Would in ipo	Ū	r rom Log	0.00	0.0000	100.00	1/2"	2.73	2.73	0.04
			0.00			lce	3.40	3.40	0.06
						1" Ice	4.40	4.40	0.12
						2" Ice	• •	• •	
Commscope MC-PK8-DSH	С	None		0.0000	138.00	No Ice	34.24	34.24	1.75
,						1/2"	62.95	62.95	2.10
						Ice	91.66	91.66	2.45
						1" Ice	149.08	149.08	3.15
						2" Ice			
**									

## **Load Combinations**

Comb. Description No.

Comb.	Description
<u>No.</u>	422 4420 142
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 lce+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 lce+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 lce+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 lce+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 lce+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

## **Maximum Member Forces**

Sectio n No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	150 - 104.5	Pole	Max Tension	1	0.00	0.00	0.00
	100 104.0	1 010	Max. Compression	26	-19.28	-1.32	1.34
			Max. Mx	8	-8.97	-285.90	-2.11
			Max. My	2	-8.95	2.27	290.14
			Max. Vý	8	9.04	-285.90	-2.11
			Max. Vx	2	-9.15	2.27	290.14
			Max. Torque	25			-0.99
L2	104.5 - 68.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-25.94	-1.14	1.25
			Max. Mx	8	-13.43	-634.11	-4.29
			Max. My	2	-13.42	4.46	642.29

Sectio	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
n	ft	Туре		Load		Moment	Moment
No.				Comb.	K	kip-ft	kip-ft
			Max. Vy	8	11.01	-634.11	-4.29
			Max. Vx	2	-11.12	4.46	642.29
			Max. Torque	25			-0.91
L3	68.75 - 34	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-35.02	-0.90	1.11
			Max. Mx	8	-19.96	-1042.85	-6.41
			Max. My	2	-19.95	6.58	1054.84
			Max. Vy	8	12.99	-1042.85	-6.41
			Max. Vx	2	-13.10	6.58	1054.84
			Max. Torque	25			-0.82
L4	34 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-47.42	-0.59	0.93
			Max. Mx	8	-29.27	-1605.47	-8.84
			Max. My	2	-29.27	9.03	1621.83
			Max. Vý	8	15.12	-1605.47	-8.84
			Max. Vx	2	-15.22	9.03	1621.83
			Max. Torque	25			-0.72

## **Maximum Reactions**

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, 2
		Load	K	K	K
		Comb.			
Pole	Max. Vert	27	47.42	0.01	5.33
	Max. H <sub>x</sub>	20	29.27	15.10	0.06
	Max. H <sub>z</sub>	2	29.27	0.06	15.21
	Max. M <sub>x</sub>	2	1621.83	0.06	15.21
	$Max. M_z$	8	1605.47	-15.10	-0.06
	Max. Torsion	13	0.63	-7.60	-13.20
	Min. Vert	7	21.96	-13.05	7.55
	Min. H <sub>x</sub>	8	29.27	-15.10	-0.06
	Min. H <sub>z</sub>	15	21.96	-0.06	-15.21
	Min. M <sub>x</sub>	14	-1621.17	-0.06	-15.21
	Min. M <sub>z</sub>	20	-1605.17	15.10	0.06
	Min. Torsion	25	-0.63	7.60	13.20

# **Tower Mast Reaction Summary**

Load Combination	Vertical	Shear <sub>x</sub>	Shearz	Overturning Moment, M <sub>x</sub>	Overturning Moment, M <sub>z</sub>	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	24.40	0.00	0.00	-0.26	-0.11	-0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	29.27	-0.06	-15.21	-1621.83	9.03	0.50
0.9 Dead+1.0 Wind 0 deg - No Ice	21.96	-0.06	-15.21	-1602.75	8.95	0.50
1.2 Dead+1.0 Wind 30 deg - No Ice	29.27	7.50	-13.14	-1400.04	-794.86	0.24
0.9 Dead+1.0 Wind 30 deg - No Ice	21.96	7.50	-13.14	-1383.56	-785.54	0.24
1.2 Dead+1.0 Wind 60 deg - No Ice	29.27	13.05	-7.55	-803.17	-1385.83	-0.09
0.9 Dead+1.0 Wind 60 deg - No Ice	21.96	13.05	-7.55	-793.69	-1369.59	-0.09
1.2 Dead+1.0 Wind 90 deg - No Ice	29.27	15.10	0.06	8.84	-1605.47	-0.39
0.9 Dead+1.0 Wind 90 deg - No Ice	21.96	15.10	0.06	8.80	-1586.66	-0.40
1.2 Dead+1.0 Wind 120 deg - No Ice	29.27	13.11	7.65	818.37	-1394.96	-0.59
0.9 Dead+1.0 Wind 120 deg	21.96	13.11	7.65	808.85	-1378.60	-0.59

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub>	Overturning Moment, M <sub>z</sub>	Torque kip-ft
- No Ice		^		kip-ft	kip-ft	кір-п
1.2 Dead+1.0 Wind 150 deg	29.27	7.60	13.20	1408.51	-810.72	-0.63
- No Ice	04.00	7.00	40.00	1000.00	004.40	0.00
0.9 Dead+1.0 Wind 150 deg - No Ice	21.96	7.60	13.20	1392.09	-801.19	-0.63
1.2 Dead+1.0 Wind 180 deg	29.27	0.06	15.21	1621.17	-9.31	-0.50
- No Ice						
0.9 Dead+1.0 Wind 180 deg - No Ice	21.96	0.06	15.21	1602.26	-9.15	-0.50
1.2 Dead+1.0 Wind 210 deg	29.27	-7.50	13.14	1399.36	794.57	-0.23
- No Ice						
0.9 Dead+1.0 Wind 210 deg - No Ice	21.96	-7.50	13.14	1383.07	785.33	-0.23
1.2 Dead+1.0 Wind 240 deg	29.27	-13.05	7.55	802.50	1385.53	0.09
- No Ice						
0.9 Dead+1.0 Wind 240 deg - No Ice	21.96	-13.05	7.55	793.20	1369.38	0.09
1.2 Dead+1.0 Wind 270 deg	29.27	-15.10	-0.06	-9.51	1605.17	0.39
- No Ice						
0.9 Dead+1.0 Wind 270 deg - No Ice	21.96	-15.10	-0.06	-9.29	1586.45	0.39
1.2 Dead+1.0 Wind 300 deg	29.27	-13.11	-7.65	-819.03	1394.67	0.59
- No Ice						
0.9 Dead+1.0 Wind 300 deg - No Ice	21.96	-13.11	-7.65	-809.34	1378.39	0.59
1.2 Dead+1.0 Wind 330 deg	29.27	-7.60	-13.20	-1409.17	810.44	0.63
- No Ice			40.00			
0.9 Dead+1.0 Wind 330 deg - No Ice	21.96	-7.60	-13.20	-1392.57	800.99	0.63
1.2 Dead+1.0 Ice+1.0 Temp	47.42	0.00	-0.00	-0.93	-0.59	-0.00
1.2 Dead+1.0 Wind 0	47.42	-0.01	-5.33	-582.44	1.04	-0.41
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 30	47.42	2.65	-4.61	-503.69	-288.35	-0.24
deg+1.0 Ice+1.0 Temp	47.42	2.05	-4.01	-303.09	-200.55	-0.24
1.2 Dead+1.0 Wind 60	47.42	4.60	-2.66	-290.26	-500.65	-0.01
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 90	47.42	5.32	0.01	0.69	-578.97	0.23
deg+1.0 lce+1.0 Temp	47.42	5.52	0.01	0.09	-510.91	0.23
1.2 Dead+1.0 Wind 120	47.42	4.61	2.68	291.18	-502.34	0.40
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 150	47.42	2.67	4.63	503.38	-291.27	0.47
deg+1.0 Ice+1.0 Temp	77.72	2.07	4.00	303.30	201.21	0.47
1.2 Dead+1.0 Wind 180	47.42	0.01	5.33	580.43	-2.34	0.41
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 210	47.42	-2.65	4.61	501.69	287.05	0.24
deg+1.0 lce+1.0 Temp	47.42	-2.05	4.01	501.69	207.05	0.24
1.2 Dead+1.0 Wind 240	47.42	-4.60	2.66	288.25	499.36	0.01
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 270	47.42	-5.32	-0.01	-2.70	E77 60	-0.23
deg+1.0 Ice+1.0 Temp	47.42	-5.52	-0.01	-2.70	577.68	-0.23
1.2 Dead+1.0 Wind 300	47.42	-4.61	-2.68	-293.19	501.05	-0.40
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 330	47.42	-2.67	-4.63	-505.39	289.98	0.47
deg+1.0 Ice+1.0 Temp	47.42	-2.07	-4.03	-505.59	209.90	-0.47
Dead+Wind 0 deg - Service	24.40	-0.01	-3.58	-379.91	2.02	0.13
Dead+Wind 30 deg - Service	24.40	1.77	-3.10	-327.98	-186.19	0.06
Dead+Wind 60 deg - Service	24.40	3.08	-1.78	-188.24	-324.54	-0.02
Dead+Wind 90 deg - Service	24.40	3.56	0.01	1.86	-375.96	-0.10
Dead+Wind 120 deg - Service	24.40	3.09	1.80	191.39	-326.68	-0.15
Dead+Wind 150 deg -	24.40	1.79	3.11	329.56	-189.90	-0.16
Service	24.40	0.01	2.50	270.25	2.26	0.12
Dead+Wind 180 deg - Service	24.40	0.01	3.58	379.35	-2.26	-0.12
Dead+Wind 210 deg -	24.40	-1.77	3.10	327.42	185.94	-0.06
Service Dead+Wind 240 deg -	24.40	-3.08	1.78	187.68	324.29	0.02
Service	24.40	-3.06	1.70	107.00	324.29	0.02
Dead+Wind 270 deg -	24.40	-3.56	-0.01	-2.42	375.72	0.10

Load Combination	Vertical	Shear <sub>x</sub>	Shearz	Overturning Moment, M <sub>x</sub>	Overturning Moment, Mz	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Service						· ·
Dead+Wind 300 deg - Service	24.40	-3.09	-1.80	-191.95	326.44	0.15
Dead+Wind 330 deg - Service	24.40	-1.79	-3.11	-330.12	189.65	0.16

# **Solution Summary**

	Sun	n of Applied Force	es		Sum of Reaction		
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0.00	-24.40	0.00	0.00	24.40	0.00	0.000%
2	-0.06	-29.27	-15.21	0.06	29.27	15.21	0.000%
3	-0.06	-21.96	-15.21	0.06	21.96	15.21	0.000%
4	7.50	-29.27	-13.14	-7.50	29.27	13.14	0.000%
5	7.50	-21.96	-13.14	-7.50	21.96	13.14	0.000%
6	13.05	-29.27	-7.55	-13.05	29.27	7.55	0.000%
7	13.05	-21.96	-7.55	-13.05	21.96	7.55	0.000%
8	15.10	-29.27	0.06	-15.10	29.27	-0.06	0.000%
9	15.10	-21.96	0.06	-15.10	21.96	-0.06	0.000%
10	13.11	-29.27	7.65	-13.11	29.27	-7.65	0.000%
11	13.11	-21.96	7.65	-13.11	21.96	-7.65	0.000%
12	7.60	-29.27	13.20	-7.60	29.27	-13.20	0.000%
13	7.60	-21.96	13.20	-7.60	21.96	-13.20	0.000%
14	0.06	-29.27	15.21	-0.06	29.27	-15.21	0.000%
15	0.06	-21.96	15.21	-0.06	21.96	-15.21	0.000%
16	-7.50	-29.27	13.14	7.50	29.27	-13.14	0.000%
17	-7.50	-21.96	13.14	7.50	21.96	-13.14	0.000%
18	-13.05	-29.27	7.55	13.05	29.27	-7.55	0.000%
19	-13.05	-21.96	7.55	13.05	21.96	-7.55	0.000%
20	-15.10	-29.27	-0.06	15.10	29.27	0.06	0.000%
21	-15.10	-21.96	-0.06	15.10	21.96	0.06	0.000%
22	-13.11	-29.27	-7.65	13.11	29.27	7.65	0.000%
23	-13.11	-21.96	-7.65	13.11	21.96	7.65	0.000%
24	-7.60	-29.27	-13.20	7.60	29.27	13.20	0.000%
25	-7.60	-21.96	-13.20	7.60	21.96	13.20	0.000%
26	0.00	-47.42	0.00	-0.00	47.42	0.00	0.000%
27	-0.01	-47.42	-5.33	0.01	47.42	5.33	0.000%
28	2.65	-47.42	-4.61	-2.65	47.42	4.61	0.000%
29	4.60	-47.42	-2.66	-4.60	47.42	2.66	0.000%
30	5.32	-47.42	0.01	-5.32	47.42	-0.01	0.000%
31	4.61	-47.42	2.68	-4.61	47.42	-2.68	0.000%
32	2.67	-47.42	4.62	-2.67	47.42	-4.63	0.000%
33	0.01	-47.42	5.33	-0.01	47.42	-5.33	0.000%
34	-2.65	-47.42	4.61	2.65	47.42	-4.61	0.000%
35	-4.60	-47.42	2.66	4.60	47.42	-2.66	0.000%
36	-5.32	-47.42	-0.01	5.32	47.42	0.01	0.000%
37	-4.61	-47.42	-2.68	4.61	47.42	2.68	0.000%
38	-2.67	-47.42	-4.62	2.67	47.42	4.63	0.000%
39	-0.01	-24.40	-3.58	0.01	24.40	3.58	0.000%
40	1.77	-24.40	-3.10	-1.77	24.40	3.10	0.000%
41	3.08	-24.40	-1.78	-3.08	24.40	1.78	0.000%
42	3.56	-24.40	0.01	-3.56	24.40	-0.01	0.000%
43	3.09	-24.40	1.80	-3.09	24.40	-1.80	0.000%
44	1.79	-24.40	3.11	-1.79	24.40	-3.11	0.000%
45	0.01	-24.40	3.58	-0.01	24.40	-3.58	0.000%
46	-1.77	-24.40	3.10	1.77	24.40	-3.10	0.000%
47	-3.08	-24.40	1.78	3.08	24.40	-1.78	0.000%
48	-3.56	-24.40	-0.01	3.56	24.40	0.01	0.000%
49	-3.09	-24.40	-1.80	3.09	24.40	1.80	0.000%
50	-1.79	-24.40	-3.11	1.79	24.40	3.11	0.000%

## **Non-Linear Convergence Results**

Load	Converged?	Number	Displacement	Force
Combination		of Cycles	Tolerance	Tolerance
1	Yes	4	0.0000001	0.00000001
2	Yes	4	0.0000001	0.00077993
3	Yes	4	0.0000001	0.00043461
4	Yes	5	0.0000001	0.00090305
5	Yes	5	0.0000001	0.00042554
6	Yes	5	0.0000001	0.00088979
7	Yes	5	0.0000001	0.00041933
8	Yes	4	0.0000001	0.00063493
9	Yes	4	0.0000001	0.00031224
10	Yes	5	0.0000001	0.00089108
11	Yes	5	0.0000001	0.00041773
12	Yes	5	0.0000001	0.00095135
13	Yes	5	0.0000001	0.00044795
14	Yes	5	0.0000001	0.00006259
15	Yes	4	0.0000001	0.00071759
16	Yes	5	0.0000001	0.00087578
17	Yes	5	0.0000001	0.00041219
18	Yes	5	0.0000001	0.00088091
19	Yes	5	0.0000001	0.00041526
20	Yes	4	0.0000001	0.00096805
21	Yes	4	0.0000001	0.00057753
22	Yes	5	0.0000001	0.00094446
23	Yes	5	0.0000001	0.00044484
24	Yes	5	0.0000001	0.00089245
25	Yes	5	0.0000001	0.00041779
26	Yes	4	0.0000001	0.00004036
27	Yes	5	0.0000001	0.00066783
28	Yes	5	0.0000001	0.00083857
29	Yes	5	0.0000001	0.00083799
30	Yes	5	0.0000001	0.00066312
31	Yes	5	0.0000001	0.00083904
32	Yes	5	0.0000001	0.00083662
33	Yes	5	0.0000001	0.00065991
34	Yes	5	0.0000001	0.00082235
35	Yes	5	0.0000001	0.00081901
36	Yes	5	0.0000001	0.00065577
37	Yes	5	0.0000001	0.00083582
38	Yes	5	0.0000001	0.00084222
39	Yes	4	0.0000001	0.00006757
40	Yes	4	0.0000001	0.00025103
41	Yes	4	0.0000001	0.00024080
42	Yes	4	0.0000001	0.00005868
43	Yes	4	0.00000001	0.00022808
44	Yes	4	0.00000001	0.00027788
45	Yes	4	0.00000001	0.00007163
46	Yes	4	0.0000001	0.00022678
47	Yes	4	0.00000001	0.00023253
48	Yes	4	0.00000001	0.00006210
49	Yes	4	0.0000001	0.00027353
50	Yes	4	0.00000001	0.00022806

## **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	150 - 104.5	19.108	50	1.2173	0.0053
L2	108.75 - 68.75	9.547	50	0.8971	0.0015
L3	74 - 34	4.226	50	0.5467	0.0005
L4	40 - 0	1.233	50	0.2813	0.0002

## **Critical Deflections and Radius of Curvature - Service Wind**

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
152.00	Lighting Rod 5/8" x 5'	50	19.108	1.2173	0.0053	36383
147.00	P90-14-XLH-RR w/ Mount Pipe	50	18.357	1.1965	0.0050	36383
138.00	MX08FRO665-21 w/ Mount Pipe	50	16.121	1.1333	0.0040	15159

## **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	150 - 104.5	81.603	24	5.2027	0.0218
L2	108.75 - 68.75	40.798	24	3.8375	0.0059
L3	74 - 34	18.060	24	2.3377	0.0022
L4	40 - 0	5.268	24	1.2022	0.0008

## Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
152.00	Lighting Rod 5/8" x 5'	24	81.603	5.2027	0.0218	8649
147.00	P90-14-XLH-RR w/ Mount Pipe	24	78.398	5.1142	0.0204	8649
138.00	MX08FRO665-21 w/ Mount Pipe	24	68.858	4.8451	0.0164	3602

## **Compression Checks**

## **Pole Design Data**

Section No.	Elevation	Size	L	Lu	KI/r	Α	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in²	K	K	$\phi P_n$
L1	150 - 104.5 (1)	TP28.1875x18x0.1875	45.50	0.00	0.0	16.097 2	-8.95	941.69	0.010
L2	104.5 - 68.75 (2)	TP35.75x26.8609x0.25	40.00	0.00	0.0	27.243 5	-13.42	1593.74	0.008
L3	68.75 - 34 (3)	TP43x34.0833x0.3125	40.00	0.00	0.0	41.014 0	-19.95	2399.32	0.008
L4	34 - 0 (4)	TP50x41.0375x0.3125	40.00	0.00	0.0	49.283 8	-29.27	2883.10	0.010

## **Pole Bending Design Data**

Section	Elevation	Size	M <sub>ux</sub>	$\phi M_{nx}$	Ratio	Muy	$\phi M_{ny}$	Ratio
No.					$M_{ux}$			$M_{uy}$
	ft		kip-ft	kip-ft	$\phi M_{nx}$	kip-ft	kip-ft	$\phi M_{ny}$
L1	150 - 104.5 (1)	TP28.1875x18x0.1875	291.09	583.66	0.499	0.00	583.66	0.000

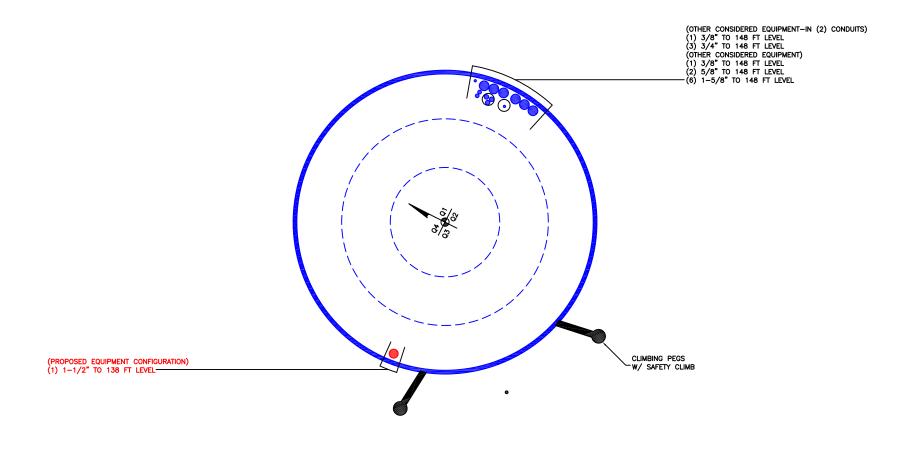
Section No.	Elevation	Size	$M_{ux}$	$\phi M_{nx}$	Ratio M <sub>ux</sub>	$M_{uy}$	$\phi M_{ny}$	Ratio M <sub>uy</sub>
740.	ft		kip-ft	kip-ft	$\frac{M_{ux}}{\phi M_{nx}}$	kip-ft	kip-ft	$\frac{M_{ny}}{\phi M_{ny}}$
L2	104.5 - 68.75 (2)	TP35.75x26.8609x0.25	644.15	1278.26	0.504	0.00	1278.26	0.000
L3	68.75 - 34 (3)	TP43x34.0833x0.3125	1057.59	2349.59	0.450	0.00	2349.59	0.000
L4	34 - 0 (4)	TP50x41.0375x0.3125	1625.61	3146.22	0.517	0.00	3146.22	0.000

		Po	le Shea	r Desig	n Data			
Section No.	Elevation	Size	Actual V <sub>u</sub>	φV <sub>n</sub>	Ratio V <sub>u</sub>	Actual T <sub>u</sub>	φ <i>T</i> <sub>n</sub>	Ratio T <sub>u</sub>
	ft		K	K	$\overline{\phi V_n}$	kip-ft	kip-ft	$\phi T_n$
L1	150 - 104.5 (1)	TP28.1875x18x0.1875	9.18	282.51	0.032	0.93	669.19	0.001
L2	104.5 - 68.75 (2)	TP35.75x26.8609x0.25	11.15	478.12	0.023	0.84	1437.59	0.001
L3	68.75 - 34 (3)	TP43x34.0833x0.3125	13.13	719.80	0.018	0.74	2606.54	0.000
L4	34 - 0 (4)	TP50x41.0375x0.3125	15.25	864.93	0.018	0.63	3763.64	0.000

Pole Interaction Design Data									
Section No.	Elevation	Ratio P <sub>u</sub>	Ratio M <sub>ux</sub>	Ratio M <sub>uy</sub>	Ratio V <sub>u</sub>	Ratio T <sub>u</sub>	Comb. Stress	Allow. Stress	Criteria
	ft	$\phi P_n$	$\phi M_{nx}$	$\phi M_{nv}$	$\overline{\phi V_n}$	$\phi T_n$	Ratio	Ratio	
L1	150 - 104.5 (1)	0.010	0.499	0.000	0.032	0.001	0.509	1.050	4.8.2
L2	104.5 - 68.75 (2)	0.008	0.504	0.000	0.023	0.001	0.513	1.050	4.8.2
L3	68.75 - 34 (3)	0.008	0.450	0.000	0.018	0.000	0.459	1.050	4.8.2
L4	34 - 0 (4)	0.010	0.517	0.000	0.018	0.000	0.527	1.050	4.8.2

			Section Capac	ity Tab	le			
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	øP <sub>allow</sub> K	% Capacity	Pass Fail
L1	150 - 104.5	Pole	TP28.1875x18x0.1875	1	-8.95	988.77	48.5	Pass
L2	104.5 - 68.75	Pole	TP35.75x26.8609x0.25	2	-13.42	1673.43	48.8	Pass
L3	68.75 - 34	Pole	TP43x34.0833x0.3125	3	-19.95	2519.29	43.7	Pass
L4	34 - 0	Pole	TP50x41.0375x0.3125	4	-29.27	3027.25	50.2	Pass
							Summary	
						Pole (L4)	50.2	Pass
						RATING =	50.2	Pass

# APPENDIX B BASE LEVEL DRAWING



# APPENDIX C ADDITIONAL CALCULATIONS

# **Monopole Base Plate Connection**

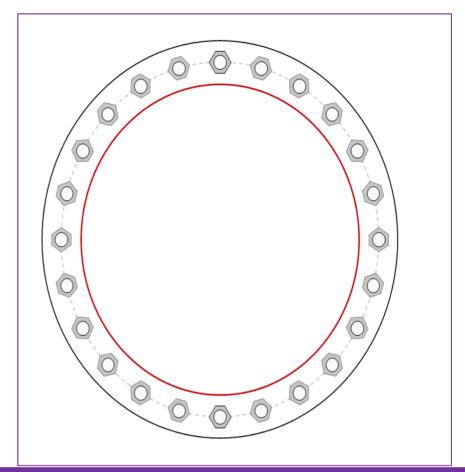


Site Info		
	BU#	857528
	Site Name	ODBURY PAPER MILL
	Order #	553361 Rev. 1

<b>Analysis Considerations</b>	
TIA-222 Revision	H
Grout Considered:	No
I <sub>ar</sub> (in)	2.25

Applied Loads	
Moment (kip-ft)	1625.60
Axial Force (kips)	29.27
Shear Force (kips)	15.25

<sup>\*</sup>TIA-222-H Section 15.5 Applied



Stress Rating:

## **Connection Properties**

### **Anchor Rod Data**

(24) 2-1/4" ø bolts (A615-75 N; Fy=75 ksi, Fu=100 ksi) on 57" BC

### **Base Plate Data**

64" OD x 2.25" Plate (A572-50; Fy=50 ksi, Fu=65 ksi)

## Stiffener Data

N/A

### **Pole Data**

50" x 0.3125" 18-sided pole (A572-65; Fy=65 ksi, Fu=80 ksi)

## **Analysis Results**

Anchor Rod Summary	(L	ınits of kips, kip-in)
Pu_t = 55.78	φPn_t = 243.75	Stress Rating
Vu = 0.64	φVn = 149.1	21.8%
Mu = n/a	φMn = n/a	Pass
Base Plate Summary		
Max Stress (ksi):	14.99	(Flexural)
Allowable Stress (ksi):	45	

31.7%

**Pass** 

CCIplate - Version 4.1.1 Analysis Date: 5/28/2021

# **Pier and Pad Foundation**

BU #: 857528
Site Name: WOODBURY PAP
App. Number: 553361 Rev. 1



TIA-222 Revision: H
Tower Type: Monopole

Top & Bot. Pad Rein. Different?:	
Block Foundation?:	
Rectangular Pad?:	

Superstructure Analysis Reactions		
Compression, P <sub>comp</sub> :	29.27	kips
Base Shear, Vu_comp:	15.23	kips
Moment, $\mathbf{M}_{\mathbf{u}}$ :	1625.6	ft-kips
Tower Height, <b>H</b> :	150	ft
BP Dist. Above Fdn, <b>bp</b> <sub>dist</sub> :	4.25	in

Pier Properties		
Pier Shape:	Circular	
Pier Diameter, <b>dpier</b> :	6.5	ft
Ext. Above Grade, E:	0.5	ft
Pier Rebar Size, <b>Sc</b> :	10	
Pier Rebar Quantity, mc:	34	
Pier Tie/Spiral Size, <b>St</b> :	4	
Pier Tie/Spiral Quantity, <b>mt</b> :	5	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, <b>cc</b> <sub>pier</sub> :	3	in

Pad Properties		
Depth, D:	4	ft
Pad Width, <b>W</b> <sub>1</sub> :	24	ft
Pad Thickness, <b>T</b> :	2.5	ft
Pad Rebar Size (Bottom dir. 2), Sp <sub>2</sub> :	10	
Pad Rebar Quantity (Bottom dir. 2), mp <sub>2</sub> :	31	
Pad Clear Cover, <b>cc</b> <sub>pad</sub> :	3	in

Material Properties			
Rebar Grade, Fy:	60	ksi	
Concrete Compressive Strength, F'c:	4	ksi	
Dry Concrete Density, δ <b>c</b> :	150	pcf	

Soil Properties			
Total Soil Unit Weight, $\gamma$ :	90	pcf	
Ultimate Gross Bearing, Qult:	12.000	ksf	
Cohesion, <b>Cu</b> :	0.000	ksf	
Friction Angle, $oldsymbol{arphi}$ :	0	degrees	
SPT Blow Count, N <sub>blows</sub> :	79		
Base Friction, $\mu$ :	0.3		
Neglected Depth, N:	4.17	ft	
Foundation Bearing on Rock?	Yes		
Groundwater Depth, gw:	n/a	ft	

Foundation Analysis Checks				
	Capacity	Demand	Rating*	Check
Lateral (Sliding) (kips)	65.53	15.23	22.1%	Pass
Bearing Pressure (ksf)	9.00	1.76	19.6%	Pass
Overturning (kip*ft)	3233.33	1699.53	52.6%	Pass
Pier Flexure (Comp.) (kip*ft)	6219.92	1656.06	25.4%	Pass
Pier Compression (kip)	21120.36	41.22	0.2%	Pass
Pad Flexure (kip*ft)	4232.26	593.88	13.4%	Pass
Pad Shear - 1-way (kips)	685.65	98.62	13.7%	Pass
Pad Shear - 2-way (Comp) (ksi)	0.190	0.026	13.1%	Pass
Flexural 2-way (Comp) (kip*ft)	4903.88	993.64	19.3%	Pass

\*Rating per TIA-222-H Section 15.5

Structural Rating*:	25.4%
Soil Rating*:	52.6%

<--Toggle between Gross and Net



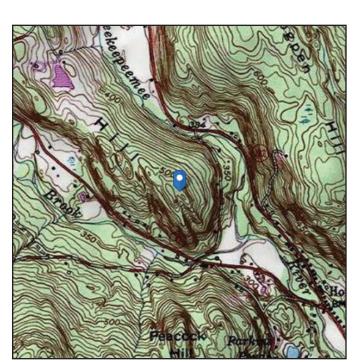
#### Address:

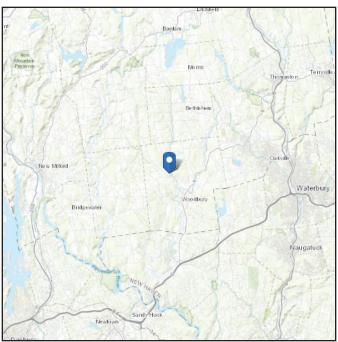
No Address at This Location

# ASCE 7 Hazards Report

Standard: ASCE/SEI 7-10 Elevation: 528.06 ft (NAVD 88)

Risk Category: || Latitude: 41.573075 Soil Class: D - Stiff Soil Longitude: -73.227642





## Wind

#### Results:

Wind Speed: 117 Vmph ← 120 mph Required by Jurisdiction

 10-year MRI
 76 Vmph

 25-year MRI
 85 Vmph

 50-year MRI
 90 Vmph

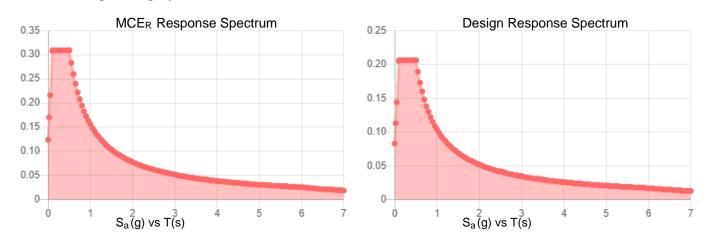
 100-year MRI
 96 Vmph



## **Seismic**

Site Soil Class: Results:	D - Stiff Soil			
S <sub>s</sub> :	0.194	$S_{ extsf{DS}}$ :	0.206	
$S_1$ :	0.065	S <sub>D1</sub> :	0.104	
F <sub>a</sub> :	1.6	T <sub>L</sub> :	6	
F <sub>v</sub> :	2.4	PGA:	0.101	
S <sub>MS</sub> :	0.309	PGA <sub>M</sub> :	0.161	
S <sub>M1</sub> :	0.156	F <sub>PGA</sub> :	1.598	
		l. ·	1	

#### Seismic Design Category B



Data Accessed: Wed May 26 2021

Date Source: USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating

Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with

ASCE/SEI 7-10 Ch. 21 are available from USGS.



#### **Ice**

#### Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 15 F

Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Wed May 26 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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# Exhibit E

**Mount Analysis** 

Date: July 30, 2021

Darcy Tarr Crown Castle 3530 Toringdon Way, Suite 300 Charlotte, NC, 28277 704-405-6619



Trylon 1825 W. Walnut Hill Lane, Suite 302 Irving, TX 75038 214-930-1730

Subject: Mount Replacement Analysis Report

Carrier Designation: Dish Wireless Dish 5G

Carrier Site Number:BOHVN00018ACarrier Site Name:CT-CCI-T-857528

Crown Castle Designation: Crown Castle BU Number: 857528

Crown Castle Site Name: Woodbury Paper Mill RD

Crown Castle JDE Job Number: 645155 Crown Castle Order Number: 553361 Rev. 1

Engineering Firm Designation: Trylon Report Designation: 189034

Site Data: 85 Paper Mill Road, Woodbury, Litchfield County, CT, 06798

Latitude 41°34'23.07" Longitude -73°13'39.51"

Structure Information: Tower Height & Type: 150.0 ft Monopole

Mount Elevation: 138.0 ft
Mount Type: 8.0 ft Platform

Dear Darcy Tarr,

Trylon is pleased to submit this "Mount Replacement Analysis Report" to determine the structural integrity of Dish Wireless's antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

Platform Sufficient\*
\*Sufficient upon completion of the changes listed in the 'Recommendations' section of this report.

This analysis utilizes an ultimate 3-second gust wind speed of 120 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Teodor Nitescu

Respectfully Submitted by: Cliff Abernathy, P.E.



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Supplemental Drawings

#### 1) INTRODUCTION

This is an proposed 3 sector 8.0 ft Platform, designed by COMMSCOPE.

#### 2) ANALYSIS CRITERIA

**Building Code:** 2015 IBC TIA-222 Revision: TIA-222-H

Risk Category:

Ultimate Wind Speed: 120 mph

**Exposure Category:** Topographic Factor at Base: 1.00 Topographic Factor at Mount: 1.00 Ice Thickness: 1.50 in Wind Speed with Ice: 50 mph Seismic S<sub>s</sub>: 0.194 Seismic S<sub>1</sub>: 0.065 Live Loading Wind Speed: 30 mph Man Live Load at Mid/End-Points: 250 lb Man Live Load at Mount Pipes: 500 lb

**Table 1 - Proposed Equipment Configuration** 

	Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details	
ĺ	138.0	139.0	3	JMA WIRELESS	MX08FRO665-21	0.0 % Dlatfama	
			3	FUJITSU	TA08025-B604	8.0 ft Platform	
		136.0	138.0	3	FUJITSU	TA08025-B605	[COMMSCOPE and MC-PK8-C1
				1	RAYCAP	RDIDC-9181-PF-48	MC-PRO-CJ

#### 3) ANALYSIS PROCEDURE

**Table 2 - Documents Provided** 

Document	Remarks	Reference	Source
Crown Application	Dish Network Application	553361 Rev. 1	CCI Sites
Mount Manufacturer Drawings	COMMSCOPE	MC-PK8-C	Trylon
Exposure Category Determination	Crown Castle	5963760	CCI Sites

#### 3.1) Analysis Method

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed, using Microsoft Excel, by Trylon was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision B).

#### 3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 5) Prior structural modifications to the tower mounting system are assumed to be installed as shown per available data.
- 6) Steel grades have been assumed as follows, unless noted otherwise:

Channel, Solid Round, Angle, Plate

HSS (Rectangular)

Pipe

ASTM A36 (GR 36)

ASTM A500 (GR B-46)

ASTM A335

Connection Bolts ASTM A325

This analysis may be affected if any assumptions are not valid or have been made in error. Trylon should be notified to determine the effect on the structural integrity of the antenna mounting system.

#### 4) ANALYSIS RESULTS

Table - Mount Component Stresses vs. Capacity (Platform, All Sectors)

industry (industry (industry))					
Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
1, 2	Mount Pipe(s)	M48	138.0	26.8	Pass
	Horizontal(s)	M18		10.0	Pass
	Standoff(s)	M7		47.2	Pass
	Bracing(s)	M6		36.0	Pass
	Handrail(s)	M21		11.0	Pass
	Plates(s)	M10		18.9	Pass
	Mount Connection(s)	-		19.0	Pass

Structure Rating (max from all components) =	47.2%
--	-------

Notes:

See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed.

<sup>2)</sup> Rating per TIA-222-H, Section 15.5.

#### 4.1) Recommendations

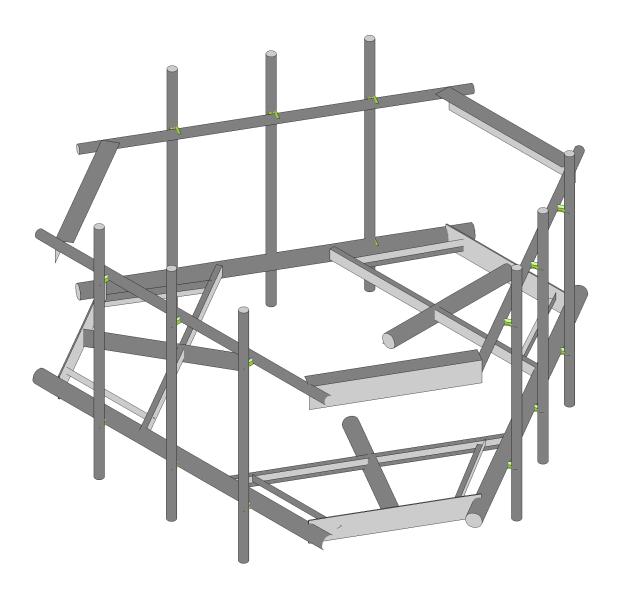
The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the proposed mount listed below must be installed.

1. COMMSCOPE, MC-PK8-C.

No structural modifications are required at this time, provided that the above-listed changes are implemented.

# APPENDIX A WIRE FRAME AND RENDERED MODELS

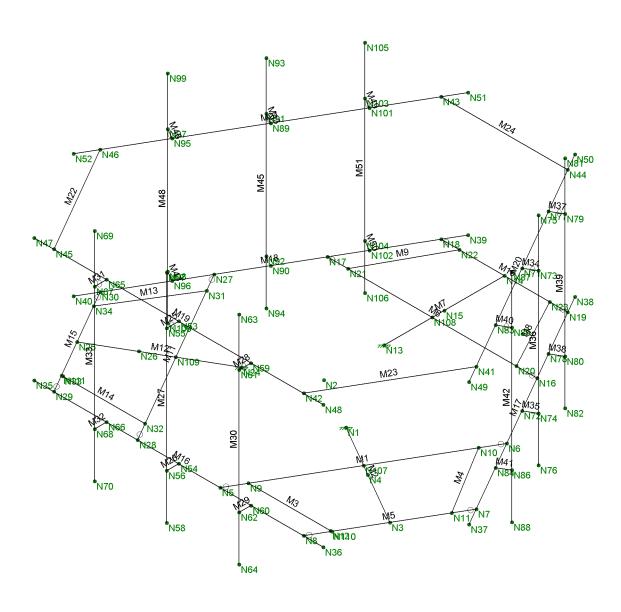




#### Envelope Only Solution

Trylon		SK - 1
TN	857528	July 30, 2021 at 7:02 PM
189034		857528.r3d





#### **Envelope Only Solution**

Trylon		SK - 2
TN	857528	July 30, 2021 at 7:03 PM
189034		857528.r3d

# APPENDIX B SOFTWARE INPUT CALCULATIONS



#### Address:

No Address at This Location

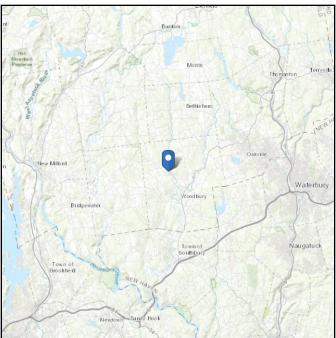
# **ASCE 7 Hazards Report**

Standard: ASCE/SEI 7-10 Elevation: 528.06 ft (NAVD 88)

Risk Category: || Latitude: 41.573075

Soil Class: D - Stiff Soil Longitude: -73.227642





### **Ice**

#### Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 15 F

Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Fri Jul 30 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.



The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE 7 Hazard Tool.



#### **TIA LOAD CALCULATOR 2.0**

PROJECT DATA	
Job Code:	189034
Carrier Site ID:	BOHVN00018A
Carrier Site Name:	CT-CCI-T-857528

CODES AND STANDARDS	
Building Code:	2015 IBC
Local Building Code:	2018 CSBS
Design Standard:	TIA-222-H

STRUCTURE DETAILS		
Mount Type:	Platform	
Mount Elevation:	138.0	ft.
Number of Sectors:	3	
Structure Type:	Monopole	
Structure Height:	150.0	ft.

ANALYSIS CRITERIA		
Structure Risk Category:	II	
Exposure Category:	В	
Site Class:	D - Default	
Ground Elevation:	528.06	ft.

TOPOGRAPHIC DATA		
Topographic Category:	1.00	
Topographic Feature:	N/A	
Crest Point Elevation:	0.00	ft.
Base Point Elevation:	0.00	ft.
Crest to Mid-Height (L/2):	0.00	ft.
Distance from Crest (x):	0.00	ft.
Base Topo Factor (K <sub>zt</sub> ):	1.00	
Mount Topo Factor (K <sub>zt</sub> ):	1.00	

WIND PARAM	IETERS	
Design Wind Speed:	120	mph
Wind Escalation Factor (K <sub>s</sub> ):	1.00	
Velocity Coefficient (K <sub>z</sub> ):	1.08	
Directionality Factor (K <sub>d</sub> ):	0.95	
Gust Effect Factor (Gh):	1.00	
Shielding Factor (K <sub>a</sub> ):	0.90	
Velocity Pressure (q <sub>z</sub> ):	37.23	psf

ICE PARAMETERS		
Design Ice Wind Speed:	50	mph
Design Ice Thickness (t <sub>i</sub> ):	1.50	in
Importance Factor (I <sub>i</sub> ):	1.00	
Ice Velocity Pressure (qzi):	37.23	psf
Mount Ice Thickness (tiz):	1.73	in

WIND STRUCTURE C	ALCULATIONS	
Flat Member Pressure:	67.01	psf
Round Member Pressure:	40.20	psf
Ice Wind Pressure:	7.43	psf

SEISMIC PARAMETERS		
Importance Factor (I <sub>e</sub> ):	1.00	
Short Period Accel .(S <sub>s</sub> ):	0.194	g
1 Second Accel (S <sub>1</sub> ):	0.065	g
Short Period Des. (S <sub>DS</sub> ):	0.21	g
1 Second Des. (S <sub>D1</sub> ):	0.10	g
Short Period Coeff. (F <sub>a</sub> ):	1.60	
1 Second Coeff. (F <sub>v</sub> ):	2.40	
Response Coefficient (Cs):	0.10	
Amplification Factor (A <sub>S</sub> ):	1.20	

## **LOAD COMBINATIONS [LRFD]**

#	Description
1	1.4DL
2	1.2DL + 1WL 0 AZI
3	1.2DL + 1WL 30 AZI
4	1.2DL + 1WL 45 AZI
5	1.2DL + 1WL 60 AZI
6	1.2DL + 1WL 90 AZI
7	1.2DL + 1WL 120 AZI
8	1.2DL + 1WL 135 AZI
9	1.2DL + 1WL 150 AZI
10	1.2DL + 1WL 180 AZI
11	1.2DL + 1WL 210 AZI
12	1.2DL + 1WL 225 AZI
13	1.2DL + 1WL 240 AZI
14	1.2DL + 1WL 270 AZI
15	1.2DL + 1WL 300 AZI
16	1.2DL + 1WL 315 AZI
17	1.2DL + 1WL 330 AZI
18	0.9DL + 1WL 0 AZI
19	0.9DL + 1WL 30 AZI
20	0.9DL + 1WL 45 AZI
21	0.9DL + 1WL 60 AZI
22	0.9DL + 1WL 90 AZI
23	0.9DL + 1WL 120 AZI
24	0.9DL + 1WL 135 AZI
25	0.9DL + 1WL 150 AZI
26	0.9DL + 1WL 180 AZI
27	0.9DL + 1WL 210 AZI
28	0.9DL + 1WL 225 AZI
29	0.9DL + 1WL 240 AZI
30	0.9DL + 1WL 270 AZI
31	0.9DL + 1WL 300 AZI
32	0.9DL + 1WL 315 AZI
33	0.9DL + 1WL 330 AZI
34	1.2DL + 1DLi + 1WLi 0 AZI
35	1.2DL + 1DLi + 1WLi 30 AZI
36	1.2DL + 1DLi + 1WLi 45 AZI
37	1.2DL + 1DLi + 1WLi 60 AZI
38	1.2DL + 1DLi + 1WLi 90 AZI
39	1.2DL + 1DLi + 1WLi 120 AZI
40	1.2DL + 1DLi + 1WLi 135 AZI
41	1.2DL + 1DLi + 1WLi 150 AZI

#	Description
42	1.2DL + 1DLi + 1WLi 180 AZI
43	1.2DL + 1DLi + 1WLi 210 AZI
44	1.2DL + 1DLi + 1WLi 225 AZI
45	1.2DL + 1DLi + 1WLi 240 AZI
46	1.2DL + 1DLi + 1WLi 270 AZI
47	1.2DL + 1DLi + 1WLi 300 AZI
48	1.2DL + 1DLi + 1WLi 315 AZI
49	1.2DL + 1DLi + 1WLi 330 AZI
50	(1.2+0.2Sds) + 1.0E 0 AZI
51	(1.2+0.2Sds) + 1.0E 30 AZI
52	(1.2+0.2Sds) + 1.0E 45 AZI
53	(1.2+0.2Sds) + 1.0E 60 AZI
54	(1.2+0.2Sds) + 1.0E 90 AZI
55	(1.2+0.2Sds) + 1.0E 120 AZI
56	(1.2+0.2Sds) + 1.0E 135 AZI
57	(1.2+0.2Sds) + 1.0E 150 AZI
58	(1.2+0.2Sds) + 1.0E 180 AZI
59	(1.2+0.2Sds) + 1.0E 210 AZI
60	(1.2+0.2Sds) + 1.0E 225 AZI
61	(1.2+0.2Sds) + 1.0E 240 AZI
62	(1.2+0.2Sds) + 1.0E 270 AZI
63	(1.2+0.2Sds) + 1.0E 300 AZI
64	(1.2+0.2Sds) + 1.0E 315 AZI
65	(1.2+0.2Sds) + 1.0E 330 AZI
66	(0.9-0.2Sds) + 1.0E 0 AZI
67	(0.9-0.2Sds) + 1.0E 30 AZI
68	(0.9-0.2Sds) + 1.0E 45 AZI
69	(0.9-0.2Sds) + 1.0E 60 AZI
70	(0.9-0.2Sds) + 1.0E 90 AZI
71	(0.9-0.2Sds) + 1.0E 120 AZI
72	(0.9-0.2Sds) + 1.0E 135 AZI
73	(0.9-0.2Sds) + 1.0E 150 AZI
74	(0.9-0.2Sds) + 1.0E 180 AZI
75	(0.9-0.2Sds) + 1.0E 210 AZI
76	(0.9-0.2Sds) + 1.0E 225 AZI
77	(0.9-0.2Sds) + 1.0E 240 AZI
78	(0.9-0.2Sds) + 1.0E 270 AZI
79	(0.9-0.2Sds) + 1.0E 300 AZI
80	
81	
	`
	(0.9-0.2Sds) + 1.0E 315 AZI (0.9-0.2Sds) + 1.0E 330 AZI 1.2D + 1.5 Lv1

#	Description
89	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP1
90	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP1
91	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP1
92	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP1
93	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP1
94	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP1
95	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP1
96	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP1
97	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP1
98	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP1
99	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP1
100	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP1
101	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP1
102	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP1
103	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP1
104	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP1
105	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP2
106	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP2
107	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP2
108	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP2
109	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP2
110	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP2
111	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP2
112	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP2
113	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP2
114	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP2
115	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP2
116	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP2
117	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP2
118	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP2
119	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP2
120	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP2

#	Description
121	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP3
122	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP3
123	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP3
124	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP3
125	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP3
126	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP3
127	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP3
128	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP3
129	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP3
130	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP3
131	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP3
132	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP3
133	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP3
134	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP3
135	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP3
136	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP3
137	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP4
138	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP4
139	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP4
140	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP4
141	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP4
142	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP4
143	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP4
144	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP4
145	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP4
146	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP4
147	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP4
148	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP4
149	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP4
150	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP4
151	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP4
152	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP4

<sup>\*</sup>This page shows an example of maintenance loads for (4) pipes, the number of mount pipe LCs may vary per site

## **EQUIPMENT LOADING**

Appurtenance Name/Location	Qty.	Elevation [ft]		EPA <sub>N</sub> (ft2)	EPA <sub>T</sub> (ft2)	Weight (lbs)
MX08FRO665-21	3	138	No Ice	8.01	3.21	82.50
M27/M45/M36, 0/120/240			w/ Ice	9.62	4.62	284.27
TA08025-B604	3	138	No Ice	1.96	0.98	63.90
M27/M45/M36, 0/120/240			w/ Ice	2.39	1.31	69.87
TA08025-B605	3	138	No Ice	1.96	1.13	75.00
M27/M45/M36, 0/120/240			w/ Ice	2.39	1.47	74.43
RDIDC-9181-PF-48	1	138	No Ice	2.01	1.17	21.85
M/M45, 120			w/ Ice	2.44	1.53	73.35
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
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## **EQUIPMENT LOADING [CONT.]**

Appurtenance Name/Location	Qty.	Elevation [ft]		EPA <sub>N</sub> (ft2)	EPA <sub>T</sub> (ft2)	Weight (lbs)
			No Ice			
			w/ Ice			
			No Ice			
-			w/ Ice			
			No Ice			
-			w/ Ice			
			No Ice			
			w/ Ice			
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## **EQUIPMENT WIND CALCULATIONS**

Appurtenance Name	Qty.	Elevation [ft]	<b>K</b> <sub>zt</sub>	Kz	<b>K</b> <sub>d</sub>	t <sub>d</sub>	<b>q</b> z [psf]	<b>q</b> <sub>zi</sub> [psf]
MX08FRO665-21	3	138	1.00	1.08	0.95	1.73	37.23	6.46
TA08025-B604	3	138	1.00	1.08	0.95	1.73	37.23	6.46
TA08025-B605	3	138	1.00	1.08	0.95	1.73	37.23	6.46
RDIDC-9181-PF-48	1	138	1.00	1.08	0.95	1.73	37.23	6.46

## **EQUIPMENT LATERAL WIND FORCE CALCULATIONS**

Appurtenance Name	Qty.		0° 180°	30° 210°	60° 240°	90° 270°	120° 300°	150° 330°
MX08FRO665-21	3	No Ice	268.36	147.75	228.16	107.55	228.16	147.75
M27/M45/M36, 0/120/240		w/ Ice	55.96	34.16	48.69	26.89	48.69	34.16
TA08025-B604	3	No Ice	65.78	41.10	57.56	32.87	57.56	41.10
M27/M45/M36, 0/120/240		w/ Ice	13.89	9.19	12.32	7.62	12.32	9.19
TA08025-B605	3	No Ice	65.78	44.83	58.80	37.84	58.80	44.83
M27/M45/M36, 0/120/240		w/ Ice	13.89	9.90	12.56	8.57	12.56	9.90
RDIDC-9181-PF-48	1	No Ice	67.40	46.20	60.34	39.14	60.34	46.20
M/M45, 120		w/ Ice	14.21	10.21	12.87	8.87	12.87	10.21
		No Ice						
		w/ Ice						
		No Ice						
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### **EQUIPMENT LATERAL WIND FORCE CALCULATIONS [CONT.]**

Appurtenance Name	Qty.		0° 180°	30° 210°	60° 240°	90° 270°	120° 300°	150° 330°
		No Ice						
		w/ Ice						
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		No Ice						
		w/ Ice						

## **EQUIPMENT SEISMIC FORCE CALCULATIONS**

Appurtenance Name	Qty.	Elevation [ft]	Weight [lbs]	<b>F</b> p [lbs]
MX08FRO665-21	3	138	82.5	10.24
TA08025-B604	3	138	63.9	7.93
TA08025-B605	3	138	75	9.31
RDIDC-9181-PF-48	1	138	21.85	2.71

# APPENDIX C SOFTWARE ANALYSIS OUTPUT

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P[ œÜ[   ^ å ÁÛ œ^^  ÁÔ[ å^	OBOÙÔÁFÍc@OHÎ∈ËFÎDMÁŠÜØÖ
Œabi • œÛcã-} ^••Ñ	Ÿ^• <b>@</b> \¦æãc^D
ÜOÙOĐÔ[}}^&cal}ÁÔ[å^	OEQÙÔÁFÍc@QH΀ËFÎDMÁŠÜØÖ
Ô[  åÁØ[   { ^åÁÛc^  ÁÔ[ å^	OED)GÚJF€€ÉFGKÁŠÜØÖ
Y [ [ å ÁÔ[ å^	CEY ÔÁÞÖÙËTÍ KÁQEÙÖ
Y [ [ åÁv^{ ] ^  æc`   ^	Ł <i>Á</i> F€€Ø
Ô[ } & ^ e^ ÁÔ[ å^	OĐÔ(ÁHFÌ ËTI
Tæ[}¦^ÆÔ[å^	OEÔOÁIH€ÉFHKÁÙd^}*c@
OE { a { AÔ   a^	OEDÆÓEÖT FËF€KSÖÜØÖÆÖÓ° åååå *
Ùæ    ^•• ÂÛ¢^^   ÁÔ[ å^	OEDÙÔÁFI c@HÎ €ËF€DAÉSÜØÖ Ó
Œabio OAÛcã-}^••Ñ	Ÿ^• <b>@</b> \¦æãç^D
Þˇ{ à^¦Á; ÂÛ@ æÁÜ^* ã;} •	1
Ü^* ā[} ÁÛ] æ&ā] * ÁQ &¦^{ ^} cÁG]; D	
Óãæ¢ãæ∮Ô[ `{} ÁT^c@[å	Ò¢æ\$cÁQ;c^*¦æaã[}
Úæl{ ^ÁÓ^œÁØæ&d ÁQÚÔŒD	ÈÍ
Ô[}&\^&\AÛd^•• ÁÓ [&\	Ü^&ca) *
W•^ÁÔ¦æ&\^åÁÙ^&æĨ}•Ñ	ΫΛ•
W•^ÁÔ¦æ&\^åÁÙ^&cã }•ÁÛ æàÑ	Ÿ∧•
ÓæåÁØlæ{ā,*Á/æ}}ã,*•Ñ	Þ
W, * • ^å ÁØ[ ¦ & ^Á/ æ } ā * • Ñ	Ϋ́Λ•
TāļÁFÁÓækÁÖææ ÞÁÚJæ&ā;*Ñ	Þ
Ô[}&\^&\AÜ^àæ\AÛ^c	ÜÒӌܴÙÒV´ŒÙVTŒÎFÍ
T ā ÁĀ ÁÙC^^ Á[¦ÁÔ[ ˇ{ }	F
T æ¢Áà ÁÙ¢^^ Á[¦ÁÔ[ ˇ{}	ì
[. 24 k.f., of   f.]	<u> </u>

#### fļ`cVUŁAcXY`GYhhjb[gž7cbhjbi YX

Ù^ã{ 884Ô[ å^	ŒÙÔÒÁ ËFÎ
Ù^ã{ ã&ÁÓæ• ^ÁÒ ^çæaã  }ÁQ; D	Þ[ 🐠 & '^å
O∄åÁÓæ•^Á⁄^ðã®Ñ	ΫΛ•
ÔæÝ	È€G
ÔÁZ	E€G
V <i>Ř</i> Y <b>ÁĢ</b> ^&D	Þ[ 🐠 🖒 e^ ¦^ å
<i>√Æ</i> <b>Á</b> §^&D	Þ[ 🐠 e^\^å
ÜŔ	Н
ÜÆ	H
ÔớÔ¢] ÉÝ	ĚÍ
ÔæÔ¢] ÉZ	Ěi
ÙÖF	F
ÙÖÙ	F
ÙĘ	F
VŠÁÇ^&D	İ
Üã\ÂÔæ	QA LÁQQ
Öl ão/Ôæc	U@\
U{ Æ	F
U{ #	F
	F
Ôå <b>Á</b> Ý	F
ÜQÆ	F
Ü <b>@</b> K	F

#### <chFc``YX'GhYY`DfcdYff]Yg

	Šæà^	ÒÆŽ•ãã	ÕÆŽ•ãã	Þř	V@N¦{ÁÇEFFÒÍÁØD	Ö^}•ãcÎŽÐcâHá	Ÿã∿∣åŽj∙ãã	Ü^	ØĭŽj•ãã	Üc
F	ŒJG	GJ€€€	FFFÍ I	È	Ēί	ÈJ	Í€€€€	FÈ	îÍ€€€	FÈ
G	OEHÎ ÁÕ¦ÈHÎ	GJ€€€	FFFÍ I	ÈH	Ēί	ÈJ	H΀€€	FĚ	ÍÌ€€€	FÈG
Н	OÉÏGÁŐ¦ÉE	GJ€€€	FFFÍ I	ÈH	ĚÍ	ÈJ	Í€€€€	FÈF	ÎÍ€€€	FÈ
1	OÉ €€ÁÕ¦ÈÓÁÜÞÖ	GJ€€€	FFFÍ I	ÈH	Ēί	ĚĞ	IŒ€€	FÈ	ÍÌ€€€	FÈH
ĺ	OÉ €€ÁÕ¦ÈÓÁÜ^&c	GJ€€€	FFFÍ I	ÈH	Ēί	ĚĞ	lÎ <b>€€€</b>	FÈ	í쀀€	FÈH
Î	OÉ HÁÕ¦ ÈÓ	GJ€€€	FFFÍ I	ÈH	Ēί	ÈJ	HÍ €€€	F₿	€€	FÈG
Ϊ	OEF€ÌÍ	GJ€€€	FFFÍ I	ÈH	Ēί	ÈJ	Í€€€€	FÈ	î퀀€	FÈH

#### 7c'X': cfa YX'GhYY'DfcdYff]Yg

	Šæà^	ÒÃŽ•ãã	ÕÆX•ãã	Þř	V@N¦{ ÁQEEFÒÍÁAØD	Ö^}•ãcÎŽÐcâHá	ŸãN∣åŽj∙ãã	Ø"Žj•ãã
F	OÊÍHÁÙÙÁÕ¦HH	GJÍ €€	FFHI Î	È	Ēί	ÈJ	HH€€€	IÍ€€€
G	OÉÍHÁÙÙÁզ̀EF	GJÍ €€	FFHI Î	È	Ēί	ÈΙ	Í <del>€€€€</del>	îÍ€€€

#### <chFc``YX'GhYY'GYWJcb'GYlg</pre>

	Šæà^	Ù@ <b>≱</b> ^	V^]^	Ö^∙ã}Šãc	Tæe^∖lãæ∳	Ö^•ãi}ÁEÈ OEÃA;G	áQ^ÁŽajlá	Q:ÆŽjlá	RÁŽ <b>á</b> lá
F	Ú æe^•	ÎËÄ¢€ÈHÏÄÚ æc^	Ó^æŧ	ÜÒÔV	OEHÎÁÕ¦ÈHÎ	V^]ã&æ; GÈT€Í	È€GÏ	ìÈîì	ÈEÎ
G	Õ¦æ@}*ÁÓ¦æ&&}*	ŠŒ¢ŒH	Ó^æŧ	Ùā;*  ^ÁOE;*  ^	OEHÎÁÕ¦ÈHÎ	V^]	i BÖİF	ÈÄF	È€J
Н	Ùœ); å[⊶•	ÚQÚÒ´HĒ	Ó^æŧ	Úą^	OÉ HÁÕ¦ ÈÓ	V^]ã&æ <b>;</b> GHĚ	ΙĚG	ΙĚĞ	JÈ€I
I	Ùœa) å[~~ÁÓ¦æ&&a]*	ÔHÝÍ	Ó^æ{	Ô@#}}^	OEHÎ ÁÕ¦ÈHÎ	V^]ã&æ; FÈÏ	ÈGIF	FÈÍ	ÈEIH
ĺ	Pæ}妿ãj∙	ڌҴŒÈE	Ó^æŧ	Úą^	OÉ HÁÕ¦ ÈÓ	V^]ã&æ; FÈ€G	ĒĞ	ĖĠ	FÈGÍ
Î	Pæ}妿ãjÁÔ[¦}^¦•	ÎÈ¢IÈ΢€ÈGÍ	Ó^æ{	Ùā,*  ^ÁOĘ* ^	OEHÎÁÕ¦ÈHÎ	V^]ã&æ; GĒE	ΙΙĖĖ́Ι	FŒÌÏH	È€ÍÍ
Ϊ	P[¦ã[}æ••	ÚŒÓ HĚ	Ó^æŧ	Úą^	OÉ HÁÕ¦ ÈÓ	V^]ã&æ <b>;</b> GÉ	ΙĚG	ΙĚG	JÈ€I

<chFc``YX`GhYY`GYWIjcb`GYlg`fI'cbhjbi YXŁ</pre>

	Šæà^	Ù@ <b>≱</b> ^	V^]^	Ö^∙ã*}ÁŠãic	Tæe^∖ãæ⇔	Ö^•ã*}ÁEÈ 0EÃÃ)Gá	Q^ÃÃjlá	Q:Æğilá F	RÁŽ[lá
ì	T [ * ] OÁÚā] ^•	ÚŒÓ′ŒĒ	Ó^æ	Úą^	OÉ HÁÕ¦ ÈÓ	V^]ã&æ; FÈ€G	ĒĞ	ĒĞ	FÉGÍ

7c'X': cfa YX'GhYY'GYWIjcb'GYhg

	Šæà^	Ù@ <b>4</b> ^	V^]^				OEÁŽajGá	Q^Ããa,lá	Q:Æãjlá	RÁŽájlá
F	ÔØFŒ	ÌÔWFÈGÍÝ€ÍÏ	Ó^æ	þ[}^ Œ	ÎÍHÁÙÙÁÈÈ	V^1 a8æ	ĚÌF	ÈÉÍÏ	IÈF	È€€ĜH

>c]bhi6 ci bXUfm7 cbX]h]cbg

	R[ã]oÁŠæà∧	ÝÁŽÐajá	ŸÁŽÐajá	ZÁŽEAjá	ÝÁÜ[dŽŽËdĐæåá	ŸÁÜ[dĚŽËdĐæåá	ZÁÜ[dÈŽË-6Dæåá
F	ÞG	Ü^æ <b>\$</b> æ <b>[</b> }	Ü^æ <b>\$a</b> [}	Ü^æ <b>&amp;</b> æ <b>[</b> }	Ü^æ <b>\$a</b> { }	Ü^æ <b>&amp;a</b> {}}	Ü^æ <b>\$</b> æ <b>[</b> }
G	ÞF	Ü^æ <b>\$</b> æ <b>[</b> }	Ü^æ <b>\$</b> æ <b>[</b> }	Ü^æ <b>&amp;</b> æ <b>[</b> ]	Ü^æ <b>\$a</b> {i}}	Ü^æ <b>&amp;aaa</b> i}	Ü^æ <b>\$</b> æ <b>[</b> }
Н	ÞFH	Ü^æ <b>\$</b> æ <b>[</b> }	Ü^æ <b>\$</b> æ <b>[</b> }	Ü^æ <b>&amp;</b> æ <b>[</b> }	Ü^æ <b>\$a</b> {i}}	Ü^æ <b>&amp;a</b> {}}	Ü^æ <b>\$</b> æ <b>[</b> }

6 Ug]W@:UX'7 UgYg

	ÓŠÔÁÖ^∙&¦∄;₫}	Ôæe^*[¦^	Ý ÁÕ¦æçãÈ	HŸÁŐ¦æçáHÌŻÁ	Õ¦æçãcî	Rją̃c	Ú[ã]c	Öãrd ãa⊞	OE^æQT^{àl	 } } } } }  }  }  }  }  }  }  }  }  }  }
F	Ù^ -ÁY ^ a @c	ÖŠ			Ë		FH		Н	
G	Ùd šc ¦^Ár a åÁr	Y ŠÝ						HH		
Н	Ùd šc ¦^Ár a åÄr	Y ŠŸ						HH		
	YājåÁŠjæåÁ€ÁOEZQ	Y ŠÝ					FH			
ĺ	Y ajåÁŠjæåÁH€ÁOZQ	Þ[}^					Ĝ			
Î	Y aj å ÁŠį æ å ÁlÍ ÁOEZQ	Þ[}^					GÎ			
Ϊ	Y ajåÁŠ[æåÁÌ€ÁOEZQ	Þ[}^					Ĝ			
Ì	Y ajáÁŠ[æåÁJ€ÁOZQ	Y ŠŸ					FH			
J	YajåÁŠ[æåÁFG€ÁOEZQ	Þ[}^					Ĝ			
F€	YājāÁŠ[æáÁFHÍÁOEZQ	Þ[}^					GÎ			
FF	YājåÁŠ[æåÁFÍ€ÁOEZQ	Þ[}^					Ĝ			
FG	<b>(3</b> (^Á <b>/</b> ^ā* ( <b>0</b> c)	UŠF					FH	HH	Н	
FH	Ùd`&c`¦^Á@a∧ÁYājåÁÝ	UŠG						HH		
FI	Ùd * & o` ¦ ^ Á 🟖 ^ ÁY 🧿 å ÁŸ	UŠH						HH		
FÍ	O&^ÁY ajáÁS[æáÁ€ÁOEZQ	UŠG					FH			
FÎ	O&^ÁYa}åÆŠ[æåÁH€ÁOEZQ	Þ[}^					GÎ			
FΪ	O&∧ÁYa}åÁŠ[æåÁnÍÁOEZQ	Þ[}^					Ĝ			
FÌ	Qa^ÁYa}åÁŠ[æåÁn€ÁOEQ	Þ[}^					GÎ			
FJ	Qa^ÁYa}åÁŠ[æåÁJ€ÁOZQ	UŠH					FH			
G€	O&^ÁY ∄,åÁŠ[æåÁFG€ÁOZQ	Þ[}^					Ĝ			
Œ	O&∧ÁY ∄åÁŠ[æåÁFHÍÁOEZQ	Þ[}^					Ĝ			
GG	Qs∧ÁYa}åÁŠ[æåÁFÍ€ÁOEZQ	Þ[}^					Ĝ			
GH	Ù^ãr{ã&ÃS[æåÁÝ	ÒŠÝ	⊞G				FH			
G	Ù^ãr{ã&ÃS[æåÄŸ	ÒŚŸ ŠŠ ŠŠ ŠŠ ŠŠ ŠŠ		⊞G			FH			
GÍ	Šãç^ÁŠ[æåÁFÁÇŠçD	ŠŠ					F			
GÎ	Šãç^ÁŠ[æåÁGÁÇŠçD	ŠŠ					F			
ĞÏ	Šãç^ÁŠ[æåÁHÁÇŠçD	ŠŠ					F			
GÌ	Šãç^ÁŠ[æåÁkÁÇŠçD	ŠŠ					F			
GJ	Šãç^ÁŠ[æåÁÁÁŠçD	ŠŠ					F			
H€	Šãç^ÁŠ[æåÁÌÁÇŠçD	ŠŠ					F			
HF	Tænfor}æn}&^ÁŠ[ænáÆnÁÇŠ(D	Þ[}^					F			
HG	Tænije^}ænj&^ÁðjæniáÁÓÁÇŠ; D	Þ[}^					F			
HH	Tænā (°) æ) & ^ ÁŠ[æná ÁHÁÇŠ(D	Þ[}^					F			

6 Ug]W@ UX'7 UgYg'f7 cbhjbi YXŁ

	ÓŠÔÁÖ^∙&¦ą[caa[}	Ôæz^*[¦^	ÝÁÕ¦æçãEEÖ ÁÕ¦æç	adde do lace accompanda de la companda de la compa	R[ã]c	Ú[ặc	Öãdãd	EŒ^æÇT^{àÈ	<u>```</u> `⊹æ&^ <u>```</u>
H	Tænag (^) æng & ^ Á Ší ænn Á Á Á Ší D	Þ[}^				F			
HÍ	Tæālo^}æ}&^ÁŠTæåÁÍÁČŠ/D	Þ[}^				F			
HÎ	Tædic^}æ)&^ÁŠjæåÁÎÁQŠ(D	Þ[}^				F			
ΗÏ	Tænāje^}ænj&^ÁŠjænjÁjÁČŠjD	Þ[}^				F			
HÌ	Tæilig (^)æ)&^ÁŠ[æåÁÌÁÇŠ(D	Þ[}^				F			
HJ	Tæilig (^)æ)&^ÁŠ[æåÁJÁÇŠ(D	Þ[}^				F			
I€	Tæaje^}æ)&^ÁŠjæåÁÏÁÇŠ{D	Þ[}^							
1 F	Tænāje^}ænj&^ÁŠ[ænnåÁiÁÇŠ(D	Þ[}^							
IG	Tæilig (^)æ)&^ÁŠ[æåÁJÁÇŠ(D	Þ[}^							
ΙH	ÓŠÔÁFÁ/¦æ}•ã} œÁŒ^æÁŠ[æå•	Þ[}^					J		
11	ÓŠÔÁFGÁV¦æ)•ã^}oÁŒ^æÆS[æå•	Þ[}^					J		

@cUX'7ca V]bUh]cbg

	A rea vjeorjeog																					
	Ö^• &¦āj cāj}	Ù[  ç^	ÚÖÈÈÙÈ	<u>ĚÓŠÔ</u>	Øæ	ÓЩ̈	ØæŧÌÌÌ	ÓЩ̈	Øæ	ÓЩ̈	Øæ	É	Øæ&d[	ÓÜ	Øæŧ	Ó∰Œ	all ÓI	<del>ìì</del> Øælì	ÉÓĤ	Zea <del>lli</del> tí	)∰Øal	莊
F	FÈ ÖŠ	Ϋ́Λ∙	Ÿ	ÖŠ																		
G	FÈCÖŠÆÆFYŠÆÆOZQ		Ϋ́		FÈG					1	F											
Н	FÉGÖŠÁÉÁFY ŠÁHEÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	FÈG	G	Èîî	Τ	Ě	_	F											
- 1	FÉGÖŠÁÉÁFY ŠÁLÍÁOEZQ	ΫΛ∙	Ÿ	ÖŠ	FÈG					Î	F											
ĺ	FÉCÖŠÁÉÁFYŠÁÑ. €ÁOEZQ	ΫΛ∙	Ÿ	ÖŠ	FÈG	G	Ě	Н	ÈÎÎ	Ϊ	F											
Î	FÈCÖŠÆÆFYŠÁJ€ÁOZQ	Ϋ́Λ∙	Ÿ		FÈG			Τ	F		F											
Ï	FÉGÖŠÆÆFYŠÆFŒÆOZQ	Ϋ́Λ∙	Ÿ		FÈG				ÈÎÎ	ے	F											
Ì	FÉGÖSÁÉÁFY ŠÁFH ÁOZQ	Ϋ́Λ∙	Ÿ		FÈG					F€	F											
J	FÈGÖŠÆÆFYŠÆFÍ€ÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	FÈG	G	ÉÈÈ	Ï	Ě	Ŧ	F											
F€	FÈGÖŠÆÆFYŠÆFÌ€ÁOZQ	Ϋ́Λ∙	Ÿ		FÈG			Τ		$\overline{}$	Ħ											
FF	FÉGÖŠÆÁFYŠÆGF€ÁOZQ	Ϋ́Λ∙	Ϋ	ÖŠ	FÈG	G	ШÜШ	Ħ	ËΪ	ĺ	Ë											
FG	FÉGÖŠÁÉÁFY ŠÁGGÍÁOZQ	Ϋ́Λ∙	Ϋ	ÖŠ	FÈG	G	田田	Ħ	田田	ΞÎ	Ë											
FH	FÉGÖŠÆÁFYŠÁGI€ÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	FÈG	G	Η̈́	Н	ÉÈÈ	ΞÏ	Ë											
FI	FÉGÖŠÆÁFYŠÆGÏ€ÁOZQ	Ϋ́Λ∙	Ϋ	ÖŠ	FÈG	G		Н		Ì	Ë											
FÍ	FÉGÖŠÆÆÆY ŠÆHE€ÆOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	FÈG	G	Ě	Н	曲曲	J	Ë											
FÎ	FÉGÖSÁÉÁFY ŠÁHFÍ ÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	FÈG	G	Ë€Ï	Τ	田田	ਜੋ	Ħ											
FΪ	FÉGÖSÁÉÁFY ŠÁHHEÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	FÈG	G	Èîî	Н	ËΪ	FF	Ë											
FÌ	€ÐÖŠÆÆFY ŠÆÆDZQ	Ϋ́Λ∙	Ÿ	ÖŠ	È	G	F	Τ		_	F											
FJ	€ÈIÖŠÆÆFYŠÆH€ÆOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	È	G	Èîî	Н	Ě	ĺ	F											
G€	€ÈIÖŠÁÉÁFYŠÁNÍÁOEZQ	Ϋ́Λ∙	Ϋ	ÖŠ	È	G	Ë€Ï	Н	Ë€Ï	Î	F											
GF	€ÈIÖŠÆÆFYŠÂR€ÆOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	È	G	Ě	Н	Èîî	Ϊ	F											
Œ	€ÈIÖŠÆÆFYŠÁJ€ÁOZQ	Ϋ́Λ∙	Ϋ	ÖŠ	È	G		Н	F	Ì	F											
GH	€ÈÖŠÆÆFYŠÆFŒÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	È	G	Η̈́	Τ	Èîî	ے	F											
G	€ÐÖŠÆÆFYŠÆFHÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ			田田			F€	F											
GÍ	€ÈÖŠÆÆFYŠÆFÍ€ÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ		G	ÉH	Ï	Ě	Ŧ	F											
GÎ	€ÈÖŠÆÆÆYŠÆFÌ€ÆOZQ	Ϋ́Λ∙	Ϋ	ÖŠ	È		Ë			$\overline{}$	Ħ											
GÏ	€ÈÖŠÆÆFYŠÆGF€ÆOZQ	Ϋ́Λ∙	Ϋ	ÖŠ	È	G	ШÜШ	Ħ	ËΪ	ĺ	Ë											
GÌ	€ÈÖŠÆÆFYŠÆGGÆQ	Ϋ́Λ∙	Ϋ	ÖŠ	È	G	田田	Ħ	田田	ΞÎ	Ë											
GJ	€ÈÖŠÁÉÁFYŠÁGI€ÁOZQ	Ϋ́Λ∙	Ϋ	ÖŠ	È	G	Η̈́	Н	ÉÈ	ΞÏ	Ë											
H€	€ÈÖŠÆÆFYŠÆGÏ€ÆOZQ	Ϋ́Λ∙	Ϋ	ÖŠ	È	G		Н		Ì	Ë											
HF	€ÈÖŠÆÆFYŠÆHEEÁOZQ	Ϋ́Λ∙	Ϋ	ÖŠ	È	G	Ě	Н	ÉÈ	J	Ë											
HG	€ÐÖŠÆÁFYŠÁHFÍÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	È	G	Ë€Ï	Н	Ш̈́Н	F€	Ë											
HH	€ÈÖŠÆÁFYŠÁHH€ÁOZQ	Ϋ́Λ∙	Ÿ	ÖŠ	È	G	Èîî	Н	ËΪ	FF	Ë											7
Н	FÉGÖŠÁÉÁFÖŠÁÉÁFY ŠÁÆÉ	•	Ϋ	ÖŠ	FÈG	UΉ	ÈF	FΗ	F	FI		FÍ	F									
HÍ	FÉGÖŠÁÉÁFÖŠÁÉÁFY ŠÁHHÉ	ŸΛ•	Ÿ	ÖŠ	FÈG	UΉ	ÈF	FΗ	Èîî	FI	Ě	FÎ	F									
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FÌÌ FÈSÖÆÆÆËŠ ÆÆÆEY		Ϋ́ÖŠ		ÈÉÎH	
FÌJ FÉGÖÆÆÆÉŠ ÆÆÆÆEY ÉÉ				ÈÉÍI	
FJ€ FÉSÖÆÆÆĚŠ(ÆÆÆEY ÉÉ		Ϋ́ÖŠ		ÈEII	
FJF FEGÖÆÆÆË Š, ÆÆÆEY EE				ÈEHF	
FJG FÉSÖÆÆÆĚŠ ÆÆÆÈEY ÉÉ	Ÿ^•	Ϋ́ÖŠ	FEGHI FÉ I ÉGHG ÉEÉH		

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GEÎ FÎESÖÁÉÁFIĚ ŠĮ ÁÉÁFIÈEY ÉÉ	Ÿ۸∙		Š FEGHI FE F€E H G EEEHH EEI	
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GG FESÖÆÆFE Š ÆÆFEY EE	Ÿ^•		Š FĒGHJ FĒL Ï ĒĒH G ĒĒĒH ĒĒĒI	
GG FEGÖÆÆFE Š, ÆÆFEY EE	ΫΛ∙		Š FĒGHJ FĒL Ì EĒ H G H EĒĒ H H	
GGJ FÉGÖÁÉÁFÉ Š ÁÉÁFÉEY ÉÉ	ΫΛ∙		Š FĒGHJ FĒLJ ĒĒHG ĒHFH ĒĒĒI	
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J	TIÍ	ڌӴGȀ	ΕĠĤ	ÍÏ F€ ÈEHÍ ÍÏ	F€	ŒÌÎÎË⊞	HŒH€	Fì Ï FË GÍ FÌ Ï FË GÍ F P FË à
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GÍ	TFH	ŠG¢G¢H	ÈF€G	€ H€ ÈEGJ € :	ΙH	FÌ€ÌIÈG€€G	GHH)ŒÌ	
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GÌ	TH	ŠG¢G¢H	<b>E</b> ìì	€ HÈEGÏ € :	ΙÌ	FÌ€ÌIÈG€€G	GHN ŒÌ	
GJ	ΤÌ	ŠG¢G¢H	ÈÌ∣	€ GÈEGÌ € :	HÌ	FÌ€ÌIÈG€€G	GHN ŒÌ	ÍÍÏËFÎÎFGHUÊGHÊGÊGÊÊPŒË
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НН	TGH	ÎÈ΢IÈ΢€ÈĞÍ	ÈEI€	IGHFÈEH€ € ^	J	ÍFFÏ <b>⊕</b> ÌIJG	ÌÏÍÎF	GÎIÈ€ËFÉÈË F PŒË

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# APPENDIX D ADDITIONAL CALCUATIONS

Analysis date: 7/30/2021

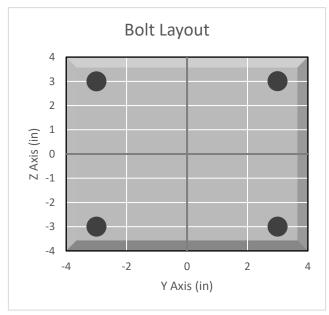


#### **BOLT TOOL 1.5.2**

Project Data					
Job Code:	189034				
Carrier Site ID:	BOHVN00018A				
Carrier Site Name:	CT-CCI-T-857528				

Code				
Design Standard:	TIA-222-H			
Slip Check:	No			
Pretension Standard:	AISC			

Bolt Properties					
Connection Type:	Bolt				
Diameter:	0.625	in			
Grade:	A325				
Yield Strength (Fy):	Strength (Fy): 92				
Ultimate Strength (Fu):	120	ksi			
Number of Bolts:	4				
Threads Included:	Yes				
Double Shear:	No				
Connection Pipe Size:	1	in			



Connection Description	
Mount to Collar	

Bolt Check*						
Tensile Capacity $(\phi T_n)$ :		lbs				
Shear Capacity $(\phi V_n)$ :		lbs				
Tension Force (T <sub>u</sub> ):	4054.1	lbs				
Shear Force (V <sub>u</sub> ):	478.7	lbs				
Tension Usage:	19.0%					
Shear Usage:	3.3%					
Interaction:	19.0%	Pass				
Controlling Member:	M12					
Controlling LC:	42					

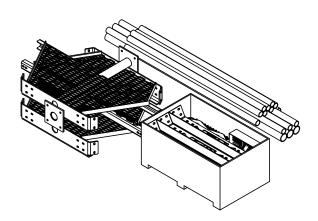
<sup>\*</sup>Rating per TIA-222-H Section 15.5

## APPENDIX E SUPPLEMENTAL DRAWINGS

ITEM	PART NO.	DESCRIPTION	QTY.	WEIGHT	NOTE NO.
1 MTC3006SB	STEEL BUNDLE FOR SNUB NOSE PLATFORM	1 402.64 LBS			
2 MCPK8CSB	PIPE STEEL BUNDLE FOR MC-PK8-C	1 464.27 LBS			
3 MCPK8CHWK	HARDWARE KIT FOR MC-PK8-C	1 543.22 LBS			

		REVISIONS		
REV.	ECN	DESCRIPTION	BY	DATE
Α		Initial release	DRR	12/27/11
В	8000005979	CHANGE NOSE CORNER BRKT, ADD GUB-4240	MSM	11/25/14
С	8000007579	NEW RINGMOUNT WELDMENT DESIGN	RJC	04/07/15

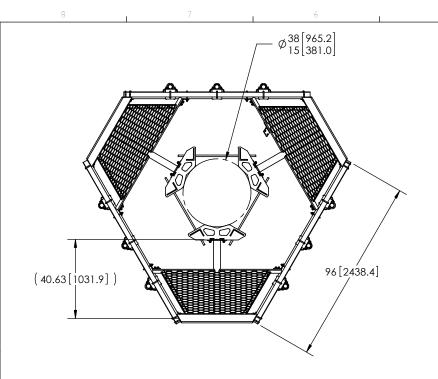
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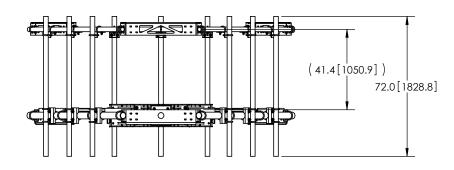


NOTES:

1. CUSTOMER ASSEMBLY SHEETS 2-3.

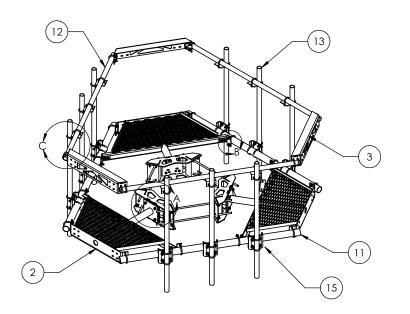
property of ANDREW CORPORATION and may be only for the specific purpose authorized in wr Andrew Corporation.	be used iriting by	MSM	1 of 3	MC-PK8-C
Andrew Corporation.  ALL DIMENSIONS ARE IN INCHES U.O.S.  TOLERANCES UNLESS OTHERWISE SPECIFIED:		онохо ву: ТР	NTS	LOW PROFILE PLATFORM KIT 8' FACE
.X = ± .12 ANGLES .XX = ± .06 FRACTIONS	±2° ±1/32	10/18/11	A36, A500	ASSEMBLY DRAWING
.XXX= ± .03  REMOVE BURRS AND BREAK EDGES .005	11/02	REVISION:	GALV A123	WESTCHESTER, IL, 60154
DO NOT SCALE THIS PRIN	NT	C	1410.14 LBS	ANDREW @ U.S.A.





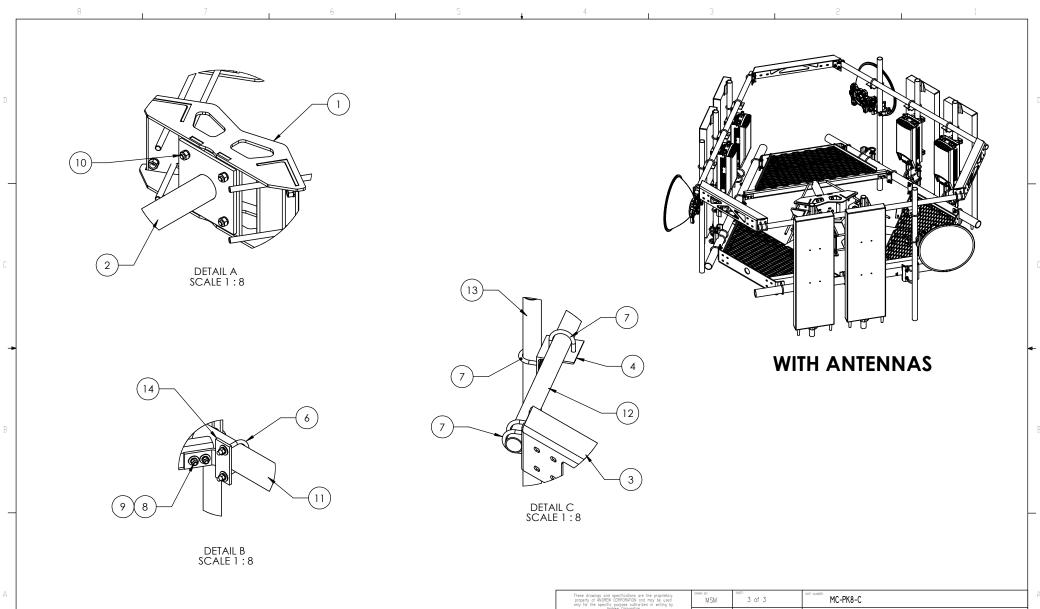
#### NOTES:

- 1. ALL METRIC DIMENSIONS ARE IN BRACKETS.
  2. WILL FIT MONOPOLES 15"-38" OD.



	ITEM	PART NO.	DESCRIPTION	QTY.	WEIGHT
>	1	MC-RM1550-3	12" - 50" OD RINGMOUNT	1	230.42 LBS
	2	MTC300601	Low Profile Co-Location Platform Snub Nose	3	134.21 LBS
	3	MT195801	Corner Weldment Snub Nose Handrail	3	27.10 LBS
	4	XA2020.01	CROSS OVER ANGLE	9	2.65 LBS
	5	GUB-4356	1/2" X 3-5/8" X 6" GALV U-BOLT	18	0.82 LBS
	6	GUB-4355	1/2" X 3-5/8" X 5" GALV U-BOLT	12	0.71 LBS
	7	GUB-4240	1/2" X 2-1/2" X 4" GALV U-BOLT	48	0.56 LBS
	8	GB-04145	1/2" X 1-1/2" GALV BOLT KIT	12	0.13 LBS
	9	GWF-04	1/2" GALV FLAT WASHER	24	0.03 LBS
	10	GB-0520A	5/8" X 2" GALV BOLT KIT (A325)	12	0.27 LBS
	11	MT54796	3.50" OD X 96" GALV PIPE	3	60.28 LBS
	12	MT-651-96	Ø 2.375" OD X 96" PIPE	3	29.07 LBS
Ī	13	MT-651	2.375" OD x 72" PIPE	9	21.80 LBS
Ī	14	MT19617	MT196 Pipe Mount Plate	6	2.49 LBS
	15	MT21701	PIPE MOUNT PLATE	9	7.93 LBS

These drawings and specifications are the proprietary property of ANDREW CORPORATION and may be used only for the specific purpose authorized in writing by Andrew Corporation.	MSM	2 of 3	MC-PK8-C
LL DIMENSIONS ARE IN INCHES U.O.S.	онахиах вт: ТР	NTS	25" OD Snub Nose MT-196
OLERANCES UNLESS OTHERWISE SPECIFIED:  .X = $\pm$ .12 ANGLES $\pm$ 2'  .XX = $\pm$ .06 FRACTIONS $\pm$ 1/32	10/18/11	A36, A53	BRANG TYSE ASSEMBLY DRAWING
.XXX= ± .03  REMOVE BURRS AND BREAK EDGES .005	REVISION:	GALV A123	WESTCHESTER, IL, 60154
DO NOT SCALE THIS PRINT	C	1361.27 LBS	ANDREW & U.S.A.



NTS

A36, A53 FNSH GALV A123

1361.27 LBS

10/18/11

С

DO NOT SCALE THIS PRINT

25" OD Snub Nose MT-196

WESTCHESTER, IL. 60154

ASSEMBLY DRAWING

NOTES:

1. ALL METRIC DIMENSIONS ARE IN BRACKETS.

## Exhibit F

**Power Density/RF Emissions Report** 



### RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

Dish Wireless Existing Facility

Site ID: BOHVN00018A

857528 85 Paper Mill Road Woodbury, Connecticut 06798

August 31, 2021

EBI Project Number: 6221004794

Site Compliance Summary			
Compliance Status:	COMPLIANT		
Site total MPE% of FCC general population allowable limit:	5.61%		



August 31, 2021

Dish Wireless

Emissions Analysis for Site: BOHVN00018A - 857528

EBI Consulting was directed to analyze the proposed Dish Wireless facility located at **85 Paper Mill**Road in Woodbury, Connecticut for the purpose of determining whether the emissions from the Proposed Dish Wireless Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm²). The number of  $\mu$ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm²). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately 400  $\mu$ W/cm² and 467  $\mu$ W/cm², respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is 1000  $\mu$ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure.



Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

#### **CALCULATIONS**

Calculations were done for the proposed Dish Wireless Wireless antenna facility located at 85 Paper Mill Road in Woodbury, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since Dish Wireless is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 4 n71 channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 4 n70 channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 3) 4 n66 channels (AWS Band 2190 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 5) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative



estimate as gain reductions for these particular antennas are typically much higher in this direction.

- 6) The antennas used in this modeling are the JMA MX08FRO665-20 for the 600 MHz / 1900 MHz / 2190 MHz channel(s) in Sector A, the JMA MX08FRO665-20 for the 600 MHz / 1900 MHz / 2190 MHz channel(s) in Sector B, the JMA MX08FRO665-20 for the 600 MHz / 1900 MHz / 2190 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antenna mounting height centerline of the proposed antennas is 138 feet above ground level (AGL).
- 8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 9) All calculations were done with respect to uncontrolled / general population threshold limits.



### Dish Wireless Site Inventory and Power Data

Sector:	Α	Sector:	В	Sector:	С
Antenna #:	I	Antenna #:	I	Antenna #:	I
Make / Model:	JMA MX08FRO665- 20	Make / Model:	JMA MX08FRO665- 20	Make / Model:	JMA MX08FRO665- 20
Frequency Bands:	600 MHz / 1900 MHz / 2190 MHz	Frequency Bands:	600 MHz / 1900 MHz / 2190 MHz	Frequency Bands:	600 MHz / 1900 MHz / 2190 MHz
Gain:	17.45 dBd / 22.65 dBd / 22.65 dBd	Gain:	17.45 dBd / 22.65 dBd / 22.65 dBd	Gain:	17.45 dBd / 22.65 dBd / 22.65 dBd
Height (AGL):	138 feet	Height (AGL):	138 feet	Height (AGL):	I 38 feet
Channel Count:	12	Channel Count:	12	Channel Count:	12
Total TX Power (W):	440 Watts	Total TX Power (W):	440 Watts	Total TX Power (W):	440 Watts
ERP (W):	5,236.31	ERP (W):	5,236.31	ERP (W):	5,236.31
Antenna A1 MPE %:	1.36%	Antenna B1 MPE %:	1.36%	Antenna C1 MPE %:	1.36%

#### environmental | engineering | due diligence

Site Composite MPE %				
Carrier	MPE %			
Dish Wireless (Max at Sector A):	1.36%			
AT&T	4.25%			
Site Total MPE % :	5.61%			

Dish Wireless MPE % Per Sector					
Dish Wireless Sector A Total:	1.36%				
Dish Wireless Sector B Total:	1.36%				
Dish Wireless Sector C Total:	1.36%				
Site Total MPE % :	5.61%				

Dish Wireless Maximum MPE Power Values (Sector A)							
Dish Wireless Frequency Band / Technology (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (μW/cm²)	Calculated % MPE
Dish Wireless 600 MHz n71	4	223.68	138.0	1.85	600 MHz n71	400	0.46%
Dish Wireless 1900 MHz n70	4	542.70	138.0	4.48	1900 MHz n70	1000	0.45%
Dish Wireless 2190 MHz n66	4	542.70	138.0	4.48	2190 MHz n66	1000	0.45%
NOTE T						Total:	1.36%

<sup>•</sup> NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.



#### **Summary**

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the Dish Wireless facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

Dish Wireless Sector	Power Density Value (%)
Sector A:	1.36%
Sector B:	1.36%
Sector C:	1.36%
Dish Wireless	
Maximum MPE %	1.36%
(Sector A):	
Site Total:	5.61%
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **5.61**% of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

## Exhibit G

## **Letter of Authorization**



4545 E River Rd, Suite 320 West Henrietta, NY 14586

Phone: (585) 445-5896 Fax: (724) 416-4461 www.crowncastle.com

#### **Crown Castle Letter of Authorization**

#### **CT - CONNECTICUT SITING COUNCIL**

Melanie A. Bachman Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

**Re:** Tower Share Application

**Crown Castle telecommunications site at:** 

85 PAPER MILL ROAD, WOODBURY, CT 06798

CCATT LLC ("Crown Castle") hereby authorizes DISH Wireless LLC, including their Agent, to act as our Agent in the processing of all zoning applications, building permits and approvals through the CT - CONNECTICUT SITING COUNCIL for the existing wireless communications site described below:

 $Crown\ Site\ ID/Name:\ \ 857528/WOODBURY\ PAPER\ MILL\ RD$ 

Customer Site ID: BOHVN00018A/CT-CCI-T-857528

Site Address: 85 PAPER MILL ROAD, WOODBURY, CT 06798

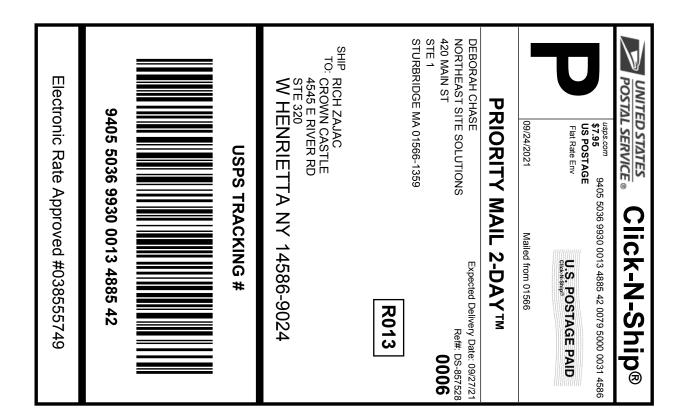
By:

Richard Zajac

Site Acquisition Specialist

# Exhibit H

**Recipient Mailings** 





#### Instructions

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- 3. Adhere your label to the package. A self-adhesive label is recommended. If tape or glue is used, DO NOT TAPE OVER BARCODE. Be sure all edges are secure.
- 4. To mail your package with PC Postage®, you may schedule a Package Pickup online, hand to your letter carrier, take to a Post Office™, or drop in a USPS collection box.
- 5. Mail your package on the "Ship Date" you selected when creating this label.

### Click-N-Ship® Label Record

### **USPS TRACKING #:** 9405 5036 9930 0013 4885 42

544385820 09/24/2021 Trans. #: Print Date: Ship Date: 09/24/2021 09/27/2021 Delivery Date:

Priority Mail® Postage: Total:

\$7.95 \$7.95

Ref#: DS-857528

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

STURBRIDGE MA 01566-1359

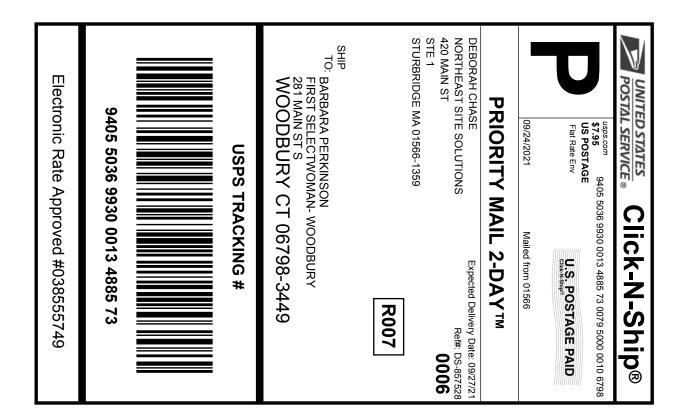
**RICH ZAJAC** 

**CROWN CASTLE** 4545 E RIVER RD

**STE 320** 

W HENRIETTA NY 14586-9024

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### **USPS TRACKING #:** 9405 5036 9930 0013 4885 73

544385820 09/24/2021 Trans. #: Print Date: Ship Date: 09/24/2021 09/27/2021 Delivery Date:

Priority Mail® Postage: Total:

\$7.95 \$7.95

Ref#: DS-857528

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

**STURBRIDGE MA 01566-1359** 

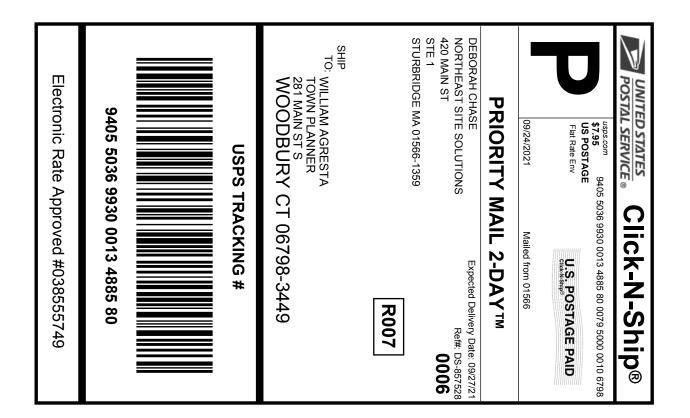
BARBARA PERKINSON

FIRST SELECTWOMAN- WOODBURY

281 MAIN ST S

WOODBURY CT 06798-3449

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- 4. To mail your package with PC Postage®, you may schedule a Package Pickup online, hand to your letter carrier, take to a Post Office™, or drop in a USPS collection box.
- 5. Mail your package on the "Ship Date" you selected when creating this label.

### Click-N-Ship® Label Record

#### **USPS TRACKING #:** 9405 5036 9930 0013 4885 80

544385820 09/24/2021 Trans. #: Print Date: Ship Date: 09/24/2021 09/27/2021 Delivery Date:

Priority Mail® Postage: Total:

\$7.95 \$7.95

Ref#: DS-857528

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

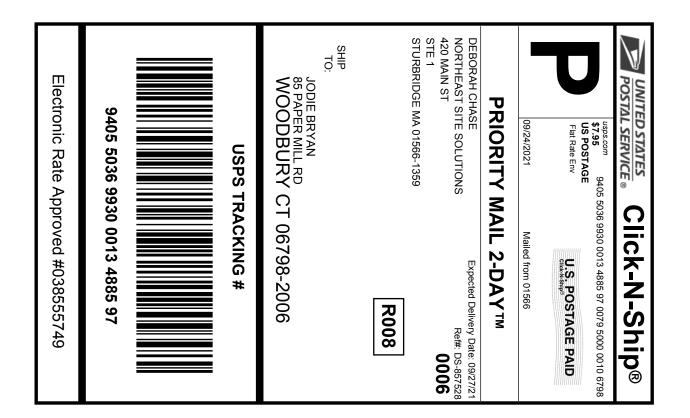
**STURBRIDGE MA 01566-1359** 

WILLIAM AGRESTA

TOWN PLANNER 281 MAIN ST S

WOODBURY CT 06798-3449

Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.





#### Instructions

- 1. Each Click-N-Ship® label is unique. Labels are to be used as printed and used only once. DO NOT PHOTO **COPY OR ALTER LABEL.**
- 2. Place your label so it does not wrap around the edge of the package.
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- 5. Mail your package on the "Ship Date" you selected when creating this label.

### Click-N-Ship® Label Record

### **USPS TRACKING #:** 9405 5036 9930 0013 4885 97

544385820 09/24/2021 Trans. #: Print Date: Ship Date: 09/24/2021 09/27/2021 Delivery Date:

Priority Mail® Postage: Total:

\$7.95 \$7.95

Ref#: DS-857528

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

STURBRIDGE MA 01566-1359

JODIE BRYAN

85 PAPER MILL RD

WOODBURY CT 06798-2006

Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.

# 857528-Woodbury



## FISKDALE

	458 MAIN ALE, MA 015 (800)275-8	18-9990 777	11:17 AM
09/24/2021	for two core and talk their som who have their and the		the same state and state town
Product	Qty	Unit Price	Price
Prepaid Mail Woodbury, Weight: 1 Acceptance Fri 09	1 CT 06798 lb 3.30 oz Date: 0/24/2021		\$0.00
Prepaid Mail Woodbury, Weight: 1 Acceptand Fri	1 CT 06798 Nb 3,30 oce Date: 09/24/2021	Z	\$0.00
Weight: Acceptar Fri	1 1b 3.30 nce Date: 09/24/2021		
Weight: Accepta Fri	nrietta, Mi 0  b 2.00 ance Date:   09/24/2021	14586 oz	\$0.00°
and the part that the first and the section		more than their part, that have still to	\$0.00

Grand Total:

Date: May 28, 2021



Crown Castle 2000 Corporate Drive Canonsburg, PA 15317 (724) 416-2000

Subject: Structural Analysis Report

Carrier Designation: DISH Network Co-Locate

Site Number: BOHVN00018A Site Name: CT-CCI-T-857528

Crown Castle Designation: BU Number: 857528

Site Name: WOODBURY PAPER MILL RD

 JDE Job Number:
 645155

 Work Order Number:
 1966221

 Order Number:
 553361 Rev. 1

Engineering Firm Designation: Crown Castle Project Number: 1966221

Site Data: 85 PAPER MILL ROAD, WOODBURY, LITCHFIELD County, CT

Latitude 41° 34′ 23.07″, Longitude -73° 13′ 39.51″

150 Foot - Monopole Tower

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration

**Sufficient Capacity - 52.6%** 

This analysis utilizes an ultimate 3-second gust wind speed of 120 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - "Analysis Criteria".

Structural analysis prepared by: Michael Lopienski

Respectfully submitted by:

Bradley E. Byrom, P.E., S.E. Senior Project Engineer



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#### 1) INTRODUCTION

#### 2) ANALYSIS CRITERIA

Table 1 - Proposed Equipment Configuration

Table 2 - Other Considered Equipment

#### 3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

- 3.1) Analysis Method
- 3.2) Assumptions

#### 4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Table 5 - Tower Component Stresses vs. Capacity - LC5

4.1) Recommendations

#### 5) APPENDIX A

tnxTower Output

#### 6) APPENDIX B

**Base Level Drawing** 

#### 7) APPENDIX C

**Additional Calculations** 

#### 1) INTRODUCTION

This tower is a 150 ft Monopole tower designed by Ehresmann Engineering 1995.

#### 2) ANALYSIS CRITERIA

TIA-222 Revision: TIA-222-H

Risk Category:

Wind Speed: 120 mph

Exposure Category:BTopographic Factor:1Ice Thickness:1.5 inWind Speed with Ice:50 mphService Wind Speed:60 mph

**Table 1 - Proposed Equipment Configuration** 

Mounting Level (ft)	Elevation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
		3	fujitsu	TA08025-B604		
		3	fujitsu	TA08025-B605		
138.0	138.0	3	jma wireless	MX08FRO665-21 w/ Mount Pipe	1	1-1/2
		1	raycap	RDIDC-9181-PF-48		
		1	tower mounts	Commscope MC-PK8-DSH		

**Table 2 - Other Considered Equipment** 

Mounting Line Slevation (ft) Center Number of Antennas		Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	
		1	cci antennas	DMP65R-BU4D w/ Mount Pipe		
		2	cci antennas	DMP65R-BU6D w/ Mount Pipe		
		1	cci antennas	OPA65R-BU4D w/ Mount Pipe		1-5/8 3/8
		2	cci antennas	OPA65R-BU6D w/ Mount Pipe		
	148.0	3	ericsson	RRUS 4449 B5/B12		
		3	ericsson	RRUS 4478 B14	6	
147.0		3	ericsson	RRUS 8843 B2/B66A	2	
		powerwave technologies P90-14-XLH-RR w/ Mount Pipe		2 3	5/8 3/4	
		3	powerwave technologies	TT19-08BP111-001		
		2	raycap	DC6-48-60-18-8F		
147.0		tower mounts	Miscellaneous [NA 507-1]			
	147.0	1	tower mounts	Platform Mount [LP 601-1]		

#### 3) ANALYSIS PROCEDURE

**Table 3 - Documents Provided** 

Document	Reference	Source
4-GEOTECHNICAL REPORTS	4570959	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	4724414	CCISITES
4-TOWER MANUFACTURER DRAWINGS	4724415	CCISITES

#### 3.1) Analysis Method

tnxTower (version 8.0.9.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

#### 3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

#### 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary)** 

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	150 - 104.5	Pole	TP28.1875x18x0.1875	1	-8.95	988.77	48.5	Pass
L2	104.5 - 68.75	Pole	TP35.75x26.8609x0.25	2	-13.42	1673.43	48.8	Pass
L3	68.75 - 34	Pole	TP43x34.0833x0.3125	3	-19.95	2519.29	43.7	Pass
L4	34 - 0	Pole	TP50x41.0375x0.3125	4	-29.27	3027.25	50.2	Pass
							Summary	
						Pole (L4)	50.2	Pass
						Rating =	50.2	Pass

Table 5 - Tower Component Stresses vs. Capacity - LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	21.8	Pass
1	Base Plate	0	31.7	Pass
1	Base Foundation (Structure)	0	25.4	Pass
1	Base Foundation (Soil Interaction)	0	52.6	Pass

Structure Rating (max from all components) =	52.6%

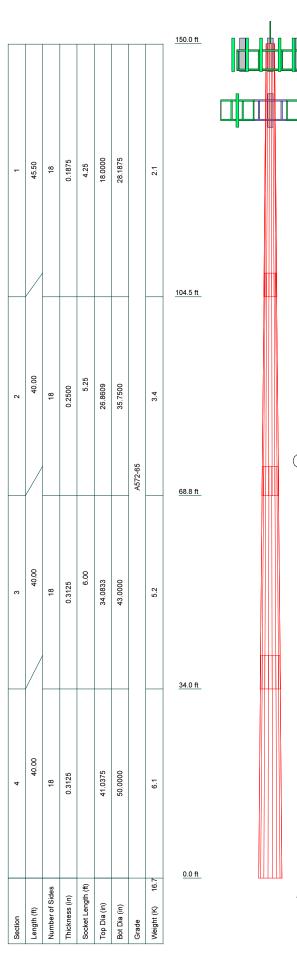
Notes:

<sup>1)</sup> See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

#### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

## APPENDIX A TNXTOWER OUTPUT

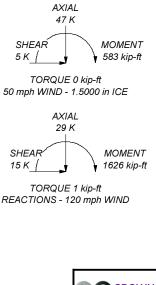


#### **MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

#### **TOWER DESIGN NOTES**

- 1. Tower designed for Exposure B to the TIA-222-H Standard.
- 2. Tower designed for a 120 mph basic wind in accordance with the TIA-222-H Standard.
- 3. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
- Deflections are based upon a 60 mph wind.
   Tower Risk Category II.
- 6. Topographic Category 1 with Crest Height of 0.00 ft 7. TOWER RATING: 50.2%



15 K

ALL REACTIONS ARE FACTORED



### **Tower Input Data**

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- Tower base elevation above sea level: 528.00 ft.
- Basic wind speed of 120 mph.
- Risk Category II.
- Exposure Category B.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0.00 ft.
- Nominal ice thickness of 1.5000 in.
- Ice thickness is considered to increase with height.
- Ice density of 56 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 60 mph.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.
- Tower analysis based on target reliabilities in accordance with Annex S.
- Load Modification Factors used: Kes(Fw) = 0.95, Kes(ti) = 0.85.
- Maximum demand-capacity ratio is: 1.05.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

### **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

√ Use Code Stress Ratios

√ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile

Include Bolts In Member Capacity

Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.

Autocalc Torque Arm Areas

Add IBC .6D+W Combination

✓ Sort Capacity Reports By Component
Triangulate Diamond Inner Bracing
Treat Feed Line Bundles As Cylinder
Ignore KL/ry For 60 Deg. Angle Legs

Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

 ✓ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption

Poles

- √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
- √ Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known

### **Tapered Pole Section Geometry**

Section	Elevation	Section	Splice	Number	Тор	Bottom	Wall	Bend	Pole Grade
		Length	Length	of	Diameter	Diameter	Thickness	Radius	
	ft	ft	ft	Sides	in	in	in	in	

Section	Elevation	Section	Splice	Number	Тор	Bottom	Wall	Bend	Pole Grade
	ft	Length ft	Length ft	of Sides	Diameter in	Diameter in	Thickness in	Radius in	
L1	150.00-104.50	45.50	4.25	18	18.0000	28.1875	0.1875	0.7500	A572-65 (65 ksi)
L2	104.50-68.75	40.00	5.25	18	26.8609	35.7500	0.2500	1.0000	À572-65 (65 ksi)
L3	68.75-34.00	40.00	6.00	18	34.0833	43.0000	0.3125	1.2500	À572-65 (65 ksi)
L4	34.00-0.00	40.00		18	41.0375	50.0000	0.3125	1.2500	À572-65 (65 ksi)

Section	Tip Dia.	Area	1	r	С	I/C	J	It/Q	W	w/t
	in	in²	in⁴	in	in	in³	in⁴	in²	in	
L1	18.2488	10.6007	424.9328	6.3234	9.1440	46.4712	850.4248	5.3013	2.8380	15.136
	28.5934	16.6635	1650.5160	9.9400	14.3193	115.2655	3303.2038	8.3333	4.6310	24.699
L2	28.1958	21.1158	1889.1397	9.4469	13.6453	138.4457	3780.7651	10.5599	4.2875	17.15
	36.2629	28.1692	4485.0722	12.6025	18.1610	246.9617	8976.0460	14.0873	5.8520	23.408
L3	35.7493	33.4964	4826.3494	11.9886	17.3143	278.7490	9659.0495	16.7514	5.4487	17.436
	43.6151	42.3407	9747.5744	15.1541	21.8440	446.2358	19507.974 9	21.1744	7.0180	22.458
L4	42.9875	40.3941	8464.0370	14.4574	20.8470	406.0065	16939.211 2	20.2009	6.6726	21.352
	50.7231	49.2838	15372.193 1	17.6391	25.4000	605.2045	30764.613 4	24.6466	8.2500	26.4

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade Adjust. Factor A <sub>i</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in				in	in	in
L1 150.00-			1	1	1			
104.50								
L2 104.50-			1	1	1			
68.75								
L3 68.75-			1	1	1			
34.00								
L4 34.00-0.00			1	1	1			

### Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face	Allow	Exclude	Componen	Placement	Total	Number	Clear	Width or	Perimete	Weight
	or	Shield	From	t		Number	Per Row	Spacing	Diamete	r	
	Leg		Torque	Type	ft			in	r		plf
	•		Calculation						in	in	
*											

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Exclude From	Componen	Placement	Total Number		$C_A A_A$	Weight
	Leg	Siliela	Torque	Туре	ft	Number		ft²/ft	plf
			Calculation	)					
*** 147 ***									
LDF7-50A(1-5/8)	В	No	No	Inside Pole	147.00 - 0.00	12	No Ice	0.00	0.82
							1/2" Ice	0.00	0.82
							1" Ice	0.00	0.82

Description	Face or	Allow Shield	Exclude From	Componen t	Placement	Total Number		$C_A A_A$	Weight
	Leg		Torque Calculation	Type	ft			f <del>l'</del> /ft	plf
							2" Ice	0.00	0.82
FB-L98B-034-	В	No	No	Inside Pole	147.00 - 0.00	1	No Ice	0.00	0.06
XXX(3/8)							1/2" Ice	0.00	0.06
							1" Ice	0.00	0.06
							2" Ice	0.00	0.06
WR-VG82ST-	В	No	No	Inside Pole	147.00 - 0.00	2	No Ice	0.00	0.31
BRDA(5/8)							1/2" Ice	0.00	0.31
							1" Ice	0.00	0.31
							2" Ice	0.00	0.31
WR-VG86ST-	С	No	No	Inside Pole	147.00 - 0.00	2	No Ice	0.00	0.58
BRD(3/4)							1/2" Ice	0.00	0.58
							1" Ice	0.00	0.58
							2" Ice	0.00	0.58
FB-L98B-034-	С	No	No	Inside Pole	147.00 - 0.00	1	No Ice	0.00	0.06
XXX(3/8)							1/2" Ice	0.00	0.06
							1" Ice	0.00	0.06
							2" Ice	0.00	0.06
****									
****									
Safety Line 3/8	С	No	No	CaAa (Out	150.00 - 0.00	1	No Ice	0.04	0.22
				Of Face)			1/2" Ice	0.14	0.75
							1" Ice	0.24	1.28
							2" Ice	0.44	2.34
5/8 rod/step	С	No	No	CaAa (Out	150.00 - 0.00	1	No Ice	0.02	0.27
				Of Face)			1/2" Ice	0.12	0.70
							1" Ice	0.22	1.74
							2" Ice	0.42	5.65
****									
CU12PSM9P6XXX	С	No	No	Inside Pole	138.00 - 0.00	1	No Ice	0.00	2.35
(1-1/2)							1/2" Ice	0.00	2.35
							1" Ice	0.00	2.35
							2" Ice	0.00	2.35
*									
*									

### Feed Line/Linear Appurtenances Section Areas

Tower Sectio	Tower	Face	$A_R$	$A_F$	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face	Weight
	Elevation		-0	•0			
n	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
L1	150.00-104.50	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.45
		С	0.000	0.000	0.000	2.616	0.15
L2	104.50-68.75	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.38
		С	0.000	0.000	0.000	2.056	0.15
L3	68.75-34.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.37
		С	0.000	0.000	0.000	1.998	0.14
L4	34.00-0.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.36
		С	0.000	0.000	0.000	1.955	0.14

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Sectio	Elevation	or	Thickness			In Face	Out Face	_
n	ft	Leg	in	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
L1	150.00-104.50	Α	1.458	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.45

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Sectio	Elevation	or	Thickness			In Face	Out Face	
n	ft	Leg	in	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
		С		0.000	0.000	0.000	29.145	0.37
L2	104.50-68.75	Α	1.403	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.38
		С		0.000	0.000	0.000	22.900	0.32
L3	68.75-34.00	Α	1.332	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.37
		С		0.000	0.000	0.000	21.505	0.30
L4	34.00-0.00	Α	1.190	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.36
		С		0.000	0.000	0.000	20.074	0.28

### **Feed Line Center of Pressure**

Section	Elevation	CP <sub>X</sub>	CPz	CP <sub>X</sub>	CPz
				lce	Ice
	ft	in	in	in	in
L1	150.00-104.50	-0.4467	0.2579	-2.0969	1.2106
L2	104.50-68.75	-0.4522	0.2611	-2.2881	1.3210
L3	68.75-34.00	-0.4551	0.2627	-2.3296	1.3450
L4	34.00-0.00	-0.4568	0.2638	-2.3139	1.3359

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Discrete Tower Loads										
Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight	
			ft ft ft	0	ft		ft²	ft²	К	
Lighting Rod 5/8" x 5'	С	None		0.0000	152.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.31 0.83 1.32 1.96	0.31 0.83 1.32 1.96	0.03 0.03 0.04 0.07	
*** 147 *** P90-14-XLH-RR w/ Mount Pipe	Α	From Leg	4.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	5.30 5.69 6.09 6.90	4.05 4.67 5.29 6.57	0.05 0.09 0.15 0.27	
P90-14-XLH-RR w/ Mount Pipe	В	From Leg	4.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	5.30 5.69 6.09 6.90	4.05 4.67 5.29 6.57	0.05 0.09 0.15 0.27	
P90-14-XLH-RR w/ Mount Pipe	С	From Leg	4.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	5.30 5.69 6.09 6.90	4.05 4.67 5.29 6.57	0.05 0.09 0.15 0.27	
TT19-08BP111-001	Α	From Leg	4.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.55 0.64 0.74 0.97	0.44 0.53 0.63 0.84	0.02 0.02 0.03 0.05	
TT19-08BP111-001	Α	From Leg	4.00	0.0000	147.00	No Ice	0.55	0.44	0.02	

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	0	ft		ft <sup>2</sup>	ft²	K
			0.00 1.00			1/2" Ice 1" Ice	0.64 0.74 0.97	0.53 0.63 0.84	0.02 0.03 0.05
TT19-08BP111-001	С	From Leg	4.00 0.00	0.0000	147.00	2" Ice No Ice 1/2"	0.55 0.64	0.44 0.53	0.02 0.02
			1.00			Ice 1" Ice 2" Ice	0.74 0.97	0.63 0.84	0.03 0.05
DC6-48-60-18-8F	В	From Leg	4.00	0.0000	147.00	No Ice	1.21	1.21	0.02
			0.00 1.00			1/2" Ice	1.89 2.11	1.89 2.11	0.04 0.07
						1" Ice 2" Ice	2.11	2.11	0.13
6' x 2" Mount Pipe	В	From Leg	4.00	0.0000	147.00	No Ice	1.43	1.43	0.02
			0.00 0.00			1/2" Ice	1.92 2.29	1.92 2.29	0.03 0.05
			0.00			1" Ice 2" Ice	3.06	3.06	0.09
Platform Mount [LP 601-1]	С	None		0.0000	147.00	No Ice	28.50	28.50	1.12
						1/2" Ice	31.69 34.87	31.69 34.87	1.68 2.28
						1" Ice 2" Ice	41.23	41.23	3.65
Miscellaneous [NA 507-1]	С	None		0.0000	147.00	No Ice	4.56	4.56	0.25
						1/2" Ice	6.39 8.18	6.39 8.18	0.31 0.40
						1" Ice 2" Ice	11.66	11.66	0.66
DMP65R-BU6D w/ Mount	Α	From Leg	4.00	0.0000	147.00	No Ice	11.96	5.97	0.11
Pipe			0.00 1.00			1/2" Ice	12.70 13.46	6.63 7.30	0.20 0.30
			1.00			1" Ice 2" Ice	15.02	8.69	0.53
DMP65R-BU6D w/ Mount	В	From Leg	4.00	0.0000	147.00	No Ice	11.96	5.97	0.11
Pipe			0.00			1/2"	12.70	6.63	0.20
			1.00			Ice 1" Ice 2" Ice	13.46 15.02	7.30 8.69	0.30 0.53
DMP65R-BU4D w/ Mount	С	From Leg	4.00	0.0000	147.00	No Ice	7.53	3.79	0.09
Pipe			0.00			1/2"	8.04	4.23	0.16
			1.00			Ice 1" Ice 2" Ice	8.57 9.68	4.68 5.63	0.22 0.39
OPA65R-BU6D w/ Mount	Α	From Leg	4.00	0.0000	147.00	No Ice	12.25	6.05	0.09
Pipe			0.00 1.00			1/2" Ice	13.00 13.76	6.71 7.39	0.18 0.27
			1.00			1" Ice 2" Ice	15.34	8.79	0.51
OPA65R-BU6D w/ Mount	В	From Leg	4.00	0.0000	147.00	No Ice	12.25	6.05	0.09
Pipe			0.00 1.00			1/2"	13.00 13.76	6.71 7.39	0.18 0.27
			1.00			Ice 1" Ice 2" Ice	15.34	8.79	0.51
OPA65R-BU4D w/ Mount	С	From Leg	4.00	0.0000	147.00	No Ice	8.10	4.03	0.08
Pipe			0.00			1/2"	8.65	4.50	0.14
			1.00			Ice 1" Ice 2" Ice	9.21 10.39	4.98 5.98	0.21 0.38
RRUS 4449 B5/B12	Α	From Leg	4.00	0.0000	147.00	No Ice	1.97	1.41	0.07
		3	0.00			1/2"	2.14	1.56	0.09
			1.00			Ice 1" Ice	2.33 2.72	1.73 2.07	0.11 0.16
RRUS 4449 B5/B12	В	From Leg	4.00	0.0000	147.00	2" Ice No Ice	1.97	1.41	0.07

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	٥	ft		ft²	ft²	K
			0.00			1/2"	2.14	1.56	0.09
			1.00			Ice 1" Ice 2" Ice	2.33 2.72	1.73 2.07	0.11 0.16
RRUS 4449 B5/B12	С	From Leg	4.00	0.0000	147.00	No Ice	1.97	1.41	0.07
			0.00			1/2"	2.14	1.56	0.09
			1.00			Ice	2.33	1.73	0.11
RRUS 4478 B14	А	Erom Log	4.00	0.0000	147.00	1" Ice 2" Ice No Ice	2.72 1.84	2.07 1.06	0.16 0.06
KKUS 4476 B14	А	From Leg	0.00	0.0000	147.00	1/2"	2.01	1.00	0.08
			1.00			Ice	2.19	1.34	0.09
						1" Ice	2.57	1.66	0.14
						2" Ice			
RRUS 4478 B14	В	From Leg	4.00	0.0000	147.00	No Ice	1.84	1.06	0.06
			0.00			1/2"	2.01	1.20	0.08
			1.00			Ice	2.19	1.34	0.09
						1" Ice 2" Ice	2.57	1.66	0.14
RRUS 4478 B14	С	From Leg	4.00	0.0000	147.00	No Ice	1.84	1.06	0.06
	•		0.00	0.000		1/2"	2.01	1.20	0.08
			1.00			Ice	2.19	1.34	0.09
						1" Ice 2" Ice	2.57	1.66	0.14
RRUS 8843 B2/B66A	Α	From Leg	4.00	0.0000	147.00	No Ice	1.64	1.35	0.07
			0.00			1/2"	1.80	1.50	0.09
			1.00			lce 1" lce	1.97 2.32	1.65 1.99	0.11 0.16
						2" Ice	2.02	1.55	0.10
RRUS 8843 B2/B66A	В	From Leg	4.00	0.0000	147.00	No Ice	1.64	1.35	0.07
			0.00			1/2"	1.80	1.50	0.09
			1.00			Ice	1.97	1.65	0.11
						1" Ice 2" Ice	2.32	1.99	0.16
RRUS 8843 B2/B66A	С	From Leg	4.00	0.0000	147.00	No Ice	1.64	1.35	0.07
11100 0040 <i>D2/B</i> 00/1	Ü	1 Tom Log	0.00	0.0000	147.00	1/2"	1.80	1.50	0.09
			1.00			Ice	1.97	1.65	0.11
						1" Ice	2.32	1.99	0.16
500 40 00 40 05						2" Ice			
DC6-48-60-18-8F	Α	From Leg	4.00	0.0000	147.00	No Ice	1.21	1.21	0.02
			0.00 1.00			1/2" Ice	1.89 2.11	1.89 2.11	0.04 0.07
			1.00			1" Ice	2.11	2.57	0.07
						2" Ice			
***									
****	^	Г., I	4.00	0.0000	400.00	No les	0.04	4.00	0.44
MX08FRO665-21 w/ Mount Pipe	Α	From Leg	4.00 0.00	0.0000	138.00	No Ice 1/2"	8.01 8.52	4.23 4.69	0.11 0.19
wount i ipe			0.00			Ice	9.04	5.16	0.19
			0.00			1" Ice	10.11	6.12	0.52
						2" Ice			
MX08FRO665-21 w/	В	From Leg	4.00	0.0000	138.00	No Ice	8.01	4.23	0.11
Mount Pipe			0.00			1/2"	8.52	4.69	0.19
			0.00			Ice	9.04	5.16	0.29
						1" Ice 2" Ice	10.11	6.12	0.52
MX08FRO665-21 w/	С	From Leg	4.00	0.0000	138.00	No Ice	8.01	4.23	0.11
Mount Pipe		3	0.00		/-	1/2"	8.52	4.69	0.19
			0.00			Ice	9.04	5.16	0.29
						1" Ice	10.11	6.12	0.52
DDIDC 0404 DE 40	^	From Las	4.00	0.0000	120.00	2" Ice	0.04	4.00	0.00
RDIDC-9181-PF-48	Α	From Leg	4.00 0.00	0.0000	138.00	No Ice 1/2"	2.31 2.50	1.29 1.45	0.02 0.04
			0.00			Ice	2.70	1.43	0.04
			3.00			1" Ice	3.12	1.96	0.12

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		$C_A A_A$ Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	o	ft		ft <sup>2</sup>	ft²	K
						2" Ice			
TA08025-B604	Α	From Leg	4.00	0.0000	138.00	No Ice	1.96	0.98	0.06
			0.00			1/2"	2.14	1.11	0.08
			0.00			Ice	2.32	1.25	0.10
						1" Ice 2" Ice	2.71	1.55	0.15
TA08025-B604	В	From Leg	4.00	0.0000	138.00	No Ice	1.96	0.98	0.06
17400023 0004	Ь	r rom Log	0.00	0.0000	100.00	1/2"	2.14	1.11	0.08
			0.00			Ice	2.32	1.25	0.10
			0.00			1" Ice	2.71	1.55	0.15
						2" Ice	2.7 1	1.00	0.10
TA08025-B604	С	From Leg	4.00	0.0000	138.00	No Ice	1.96	0.98	0.06
.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	•		0.00	0.0000	.00.00	1/2"	2.14	1.11	0.08
			0.00			lce	2.32	1.25	0.10
						1" Ice	2.71	1.55	0.15
						2" Ice			
TA08025-B605	Α	From Leg	4.00	0.0000	138.00	No Ice	1.96	1.13	0.08
		· ·	0.00			1/2"	2.14	1.27	0.09
			0.00			Ice	2.32	1.41	0.11
						1" Ice	2.71	1.72	0.16
						2" Ice			
TA08025-B605	В	From Leg	4.00	0.0000	138.00	No Ice	1.96	1.13	0.08
			0.00			1/2"	2.14	1.27	0.09
			0.00			Ice	2.32	1.41	0.11
						1" Ice	2.71	1.72	0.16
						2" Ice			
TA08025-B605	С	From Leg	4.00	0.0000	138.00	No Ice	1.96	1.13	0.08
			0.00			1/2"	2.14	1.27	0.09
			0.00			Ice	2.32	1.41	0.11
						1" Ice	2.71	1.72	0.16
(0) 01 011 14 ( D)			4.00	0.0000	400.00	2" Ice	4.00	4.00	0.00
(2) 8' x 2" Mount Pipe	Α	From Leg	4.00	0.0000	138.00	No Ice	1.90	1.90	0.03
			0.00			1/2"	2.73	2.73	0.04
			0.00			Ice	3.40	3.40	0.06
						1" Ice	4.40	4.40	0.12
(2) 8' x 2" Mount Pipe	В	From Leg	4.00	0.0000	138.00	2" Ice No Ice	1.90	1.90	0.03
(2) 6 X 2 Would Pipe	ь	Fioni Leg	0.00	0.0000	130.00	1/2"	2.73	2.73	0.03
			0.00			Ice	3.40	3.40	0.04
			0.00			1" Ice	4.40	4.40	0.00
						2" Ice	4.40	4.40	0.12
(2) 8' x 2" Mount Pipe	С	From Leg	4.00	0.0000	138.00	No Ice	1.90	1.90	0.03
(2) 0 X 2 Would in ipo	Ū	r rom Log	0.00	0.0000	100.00	1/2"	2.73	2.73	0.04
			0.00			Ice	3.40	3.40	0.06
						1" Ice	4.40	4.40	0.12
						2" Ice	• •	• •	
Commscope MC-PK8-DSH	С	None		0.0000	138.00	No Ice	34.24	34.24	1.75
,						1/2"	62.95	62.95	2.10
						Ice	91.66	91.66	2.45
						1" Ice	149.08	149.08	3.15
						2" Ice			
**									

### **Load Combinations**

Comb. Description No.

Comb.	Description
No	422 4420 142
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 lce+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 lce+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 lce+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 lce+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 lce+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

### **Maximum Member Forces**

Sectio n No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	150 - 104.5	Pole	Max Tension	1	0.00	0.00	0.00
	100 104.0	1 010	Max. Compression	26	-19.28	-1.32	1.34
			Max. Mx	8	-8.97	-285.90	-2.11
			Max. My	2	-8.95	2.27	290.14
			Max. Vý	8	9.04	-285.90	-2.11
			Max. Vx	2	-9.15	2.27	290.14
			Max. Torque	25			-0.99
L2	104.5 - 68.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-25.94	-1.14	1.25
			Max. Mx	8	-13.43	-634.11	-4.29
			Max. My	2	-13.42	4.46	642.29

Sectio	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
n	ft	Type		Load		Moment	Moment
No.				Comb.	K	kip-ft	kip-ft
			Max. Vy	8	11.01	-634.11	-4.29
			Max. Vx	2	-11.12	4.46	642.29
			Max. Torque	25			-0.91
L3	68.75 - 34	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-35.02	-0.90	1.11
			Max. Mx	8	-19.96	-1042.85	-6.41
			Max. My	2	-19.95	6.58	1054.84
			Max. Vy	8	12.99	-1042.85	-6.41
			Max. Vx	2	-13.10	6.58	1054.84
			Max. Torque	25			-0.82
L4	34 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-47.42	-0.59	0.93
			Max. Mx	8	-29.27	-1605.47	-8.84
			Max. My	2	-29.27	9.03	1621.83
			Max. Vy	8	15.12	-1605.47	-8.84
			Max. Vx	2	-15.22	9.03	1621.83
			Max. Torque	25			-0.72

### **Maximum Reactions**

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, 2
		Load	K	K	K
		Comb.			
Pole	Max. Vert	27	47.42	0.01	5.33
	Max. H <sub>x</sub>	20	29.27	15.10	0.06
	Max. H <sub>z</sub>	2	29.27	0.06	15.21
	Max. M <sub>x</sub>	2	1621.83	0.06	15.21
	$Max. M_z$	8	1605.47	-15.10	-0.06
	Max. Torsion	13	0.63	-7.60	-13.20
	Min. Vert	7	21.96	-13.05	7.55
	Min. H <sub>x</sub>	8	29.27	-15.10	-0.06
	Min. H <sub>z</sub>	15	21.96	-0.06	-15.21
	Min. M <sub>x</sub>	14	-1621.17	-0.06	-15.21
	Min. M <sub>z</sub>	20	-1605.17	15.10	0.06
	Min. Torsion	25	-0.63	7.60	13.20

### **Tower Mast Reaction Summary**

Load Combination	Vertical	Shear <sub>x</sub>	Shearz	Overturning Moment, M <sub>x</sub>	Overturning Moment, M <sub>z</sub>	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	24.40	0.00	0.00	-0.26	-0.11	-0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	29.27	-0.06	-15.21	-1621.83	9.03	0.50
0.9 Dead+1.0 Wind 0 deg - No Ice	21.96	-0.06	-15.21	-1602.75	8.95	0.50
1.2 Dead+1.0 Wind 30 deg - No Ice	29.27	7.50	-13.14	-1400.04	-794.86	0.24
0.9 Dead+1.0 Wind 30 deg - No Ice	21.96	7.50	-13.14	-1383.56	-785.54	0.24
1.2 Dead+1.0 Wind 60 deg - No Ice	29.27	13.05	-7.55	-803.17	-1385.83	-0.09
0.9 Dead+1.0 Wind 60 deg - No Ice	21.96	13.05	-7.55	-793.69	-1369.59	-0.09
1.2 Dead+1.0 Wind 90 deg - No Ice	29.27	15.10	0.06	8.84	-1605.47	-0.39
0.9 Dead+1.0 Wind 90 deg - No Ice	21.96	15.10	0.06	8.80	-1586.66	-0.40
1.2 Dead+1.0 Wind 120 deg - No Ice	29.27	13.11	7.65	818.37	-1394.96	-0.59
0.9 Dead+1.0 Wind 120 deg	21.96	13.11	7.65	808.85	-1378.60	-0.59

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub>	Overturning Moment, M <sub>z</sub>	Torque kip-ft
- No Ice		^		kip-ft	kip-ft	кір-п
1.2 Dead+1.0 Wind 150 deg	29.27	7.60	13.20	1408.51	-810.72	-0.63
- No Ice	04.00	7.00	40.00	1000.00	004.40	0.00
0.9 Dead+1.0 Wind 150 deg - No Ice	21.96	7.60	13.20	1392.09	-801.19	-0.63
1.2 Dead+1.0 Wind 180 deg	29.27	0.06	15.21	1621.17	-9.31	-0.50
- No Ice						
0.9 Dead+1.0 Wind 180 deg - No Ice	21.96	0.06	15.21	1602.26	-9.15	-0.50
1.2 Dead+1.0 Wind 210 deg	29.27	-7.50	13.14	1399.36	794.57	-0.23
- No Ice						
0.9 Dead+1.0 Wind 210 deg - No Ice	21.96	-7.50	13.14	1383.07	785.33	-0.23
1.2 Dead+1.0 Wind 240 deg	29.27	-13.05	7.55	802.50	1385.53	0.09
- No Ice						
0.9 Dead+1.0 Wind 240 deg - No Ice	21.96	-13.05	7.55	793.20	1369.38	0.09
1.2 Dead+1.0 Wind 270 deg	29.27	-15.10	-0.06	-9.51	1605.17	0.39
- No Ice						
0.9 Dead+1.0 Wind 270 deg - No Ice	21.96	-15.10	-0.06	-9.29	1586.45	0.39
1.2 Dead+1.0 Wind 300 deg	29.27	-13.11	-7.65	-819.03	1394.67	0.59
- No Ice						
0.9 Dead+1.0 Wind 300 deg - No Ice	21.96	-13.11	-7.65	-809.34	1378.39	0.59
1.2 Dead+1.0 Wind 330 deg	29.27	-7.60	-13.20	-1409.17	810.44	0.63
- No Ice			40.00			
0.9 Dead+1.0 Wind 330 deg - No Ice	21.96	-7.60	-13.20	-1392.57	800.99	0.63
1.2 Dead+1.0 Ice+1.0 Temp	47.42	0.00	-0.00	-0.93	-0.59	-0.00
1.2 Dead+1.0 Wind 0	47.42	-0.01	-5.33	-582.44	1.04	-0.41
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 30	47.42	2.65	-4.61	-503.69	-288.35	-0.24
deg+1.0 Ice+1.0 Temp	47.42	2.05	-4.01	-303.09	-200.55	-0.24
1.2 Dead+1.0 Wind 60	47.42	4.60	-2.66	-290.26	-500.65	-0.01
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 90	47.42	5.32	0.01	0.69	-578.97	0.23
deg+1.0 lce+1.0 Temp	47.42	5.52	0.01	0.09	-510.91	0.23
1.2 Dead+1.0 Wind 120	47.42	4.61	2.68	291.18	-502.34	0.40
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 150	47.42	2.67	4.63	503.38	-291.27	0.47
deg+1.0 Ice+1.0 Temp	77.72	2.07	4.00	303.30	201.21	0.47
1.2 Dead+1.0 Wind 180	47.42	0.01	5.33	580.43	-2.34	0.41
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 210	47.42	-2.65	4.61	501.69	287.05	0.24
deg+1.0 lce+1.0 Temp	47.42	-2.05	4.01	501.69	207.05	0.24
1.2 Dead+1.0 Wind 240	47.42	-4.60	2.66	288.25	499.36	0.01
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 270	47.42	-5.32	-0.01	-2.70	E77 60	-0.23
deg+1.0 Ice+1.0 Temp	47.42	-5.52	-0.01	-2.70	577.68	-0.23
1.2 Dead+1.0 Wind 300	47.42	-4.61	-2.68	-293.19	501.05	-0.40
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 330	47.42	-2.67	-4.63	-505.39	289.98	0.47
deg+1.0 Ice+1.0 Temp	47.42	-2.07	-4.03	-505.59	209.90	-0.47
Dead+Wind 0 deg - Service	24.40	-0.01	-3.58	-379.91	2.02	0.13
Dead+Wind 30 deg - Service	24.40	1.77	-3.10	-327.98	-186.19	0.06
Dead+Wind 60 deg - Service	24.40	3.08	-1.78	-188.24	-324.54	-0.02
Dead+Wind 90 deg - Service	24.40	3.56	0.01	1.86	-375.96	-0.10
Dead+Wind 120 deg - Service	24.40	3.09	1.80	191.39	-326.68	-0.15
Dead+Wind 150 deg -	24.40	1.79	3.11	329.56	-189.90	-0.16
Service	24.40	0.01	2.50	270.25	2.26	0.12
Dead+Wind 180 deg - Service	24.40	0.01	3.58	379.35	-2.26	-0.12
Dead+Wind 210 deg -	24.40	-1.77	3.10	327.42	185.94	-0.06
Service Dead+Wind 240 deg -	24.40	-3.08	1.78	187.68	324.29	0.02
Service	24.40	-3.06	1.70	107.00	324.29	0.02
Dead+Wind 270 deg -	24.40	-3.56	-0.01	-2.42	375.72	0.10

Load Combination	Vertical	Shear <sub>x</sub>	Shearz	Overturning Moment, M <sub>x</sub>	Overturning Moment, Mz	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Service						· ·
Dead+Wind 300 deg - Service	24.40	-3.09	-1.80	-191.95	326.44	0.15
Dead+Wind 330 deg - Service	24.40	-1.79	-3.11	-330.12	189.65	0.16

### **Solution Summary**

	Sun	n of Applied Force	es		Sum of Reaction		
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0.00	-24.40	0.00	0.00	24.40	0.00	0.000%
2	-0.06	-29.27	-15.21	0.06	29.27	15.21	0.000%
3	-0.06	-21.96	-15.21	0.06	21.96	15.21	0.000%
4	7.50	-29.27	-13.14	-7.50	29.27	13.14	0.000%
5	7.50	-21.96	-13.14	-7.50	21.96	13.14	0.000%
6	13.05	-29.27	-7.55	-13.05	29.27	7.55	0.000%
7	13.05	-21.96	-7.55	-13.05	21.96	7.55	0.000%
8	15.10	-29.27	0.06	-15.10	29.27	-0.06	0.000%
9	15.10	-21.96	0.06	-15.10	21.96	-0.06	0.000%
10	13.11	-29.27	7.65	-13.11	29.27	-7.65	0.000%
11	13.11	-21.96	7.65	-13.11	21.96	-7.65	0.000%
12	7.60	-29.27	13.20	-7.60	29.27	-13.20	0.000%
13	7.60	-21.96	13.20	-7.60	21.96	-13.20	0.000%
14	0.06	-29.27	15.21	-0.06	29.27	-15.21	0.000%
15	0.06	-21.96	15.21	-0.06	21.96	-15.21	0.000%
16	-7.50	-29.27	13.14	7.50	29.27	-13.14	0.000%
17	-7.50	-21.96	13.14	7.50	21.96	-13.14	0.000%
18	-13.05	-29.27	7.55	13.05	29.27	-7.55	0.000%
19	-13.05	-21.96	7.55	13.05	21.96	-7.55	0.000%
20	-15.10	-29.27	-0.06	15.10	29.27	0.06	0.000%
21	-15.10	-21.96	-0.06	15.10	21.96	0.06	0.000%
22	-13.11	-29.27	-7.65	13.11	29.27	7.65	0.000%
23	-13.11	-21.96	-7.65	13.11	21.96	7.65	0.000%
24	-7.60	-29.27	-13.20	7.60	29.27	13.20	0.000%
25	-7.60	-21.96	-13.20	7.60	21.96	13.20	0.000%
26	0.00	-47.42	0.00	-0.00	47.42	0.00	0.000%
27	-0.01	-47.42	-5.33	0.01	47.42	5.33	0.000%
28	2.65	-47.42	-4.61	-2.65	47.42	4.61	0.000%
29	4.60	-47.42	-2.66	-4.60	47.42	2.66	0.000%
30	5.32	-47.42	0.01	-5.32	47.42	-0.01	0.000%
31	4.61	-47.42	2.68	-4.61	47.42	-2.68	0.000%
32	2.67	-47.42	4.62	-2.67	47.42	-4.63	0.000%
33	0.01	-47.42	5.33	-0.01	47.42	-5.33	0.000%
34	-2.65	-47.42	4.61	2.65	47.42	-4.61	0.000%
35	-4.60	-47.42	2.66	4.60	47.42	-2.66	0.000%
36	-5.32	-47.42	-0.01	5.32	47.42	0.01	0.000%
37	-4.61	-47.42	-2.68	4.61	47.42	2.68	0.000%
38	-2.67	-47.42	-4.62	2.67	47.42	4.63	0.000%
39	-0.01	-24.40	-3.58	0.01	24.40	3.58	0.000%
40	1.77	-24.40	-3.10	-1.77	24.40	3.10	0.000%
41	3.08	-24.40	-1.78	-3.08	24.40	1.78	0.000%
42	3.56	-24.40	0.01	-3.56	24.40	-0.01	0.000%
43	3.09	-24.40	1.80	-3.09	24.40	-1.80	0.000%
44	1.79	-24.40	3.11	-1.79	24.40	-3.11	0.000%
45	0.01	-24.40	3.58	-0.01	24.40	-3.58	0.000%
46	-1.77	-24.40	3.10	1.77	24.40	-3.10	0.000%
47	-3.08	-24.40	1.78	3.08	24.40	-1.78	0.000%
48	-3.56	-24.40	-0.01	3.56	24.40	0.01	0.000%
49	-3.09	-24.40	-1.80	3.09	24.40	1.80	0.000%
50	-1.79	-24.40	-3.11	1.79	24.40	3.11	0.000%

### **Non-Linear Convergence Results**

Load	Converged?	Number	Displacement	Force
Combination		of Cycles	Tolerance	Tolerance
1	Yes	4	0.0000001	0.00000001
2	Yes	4	0.0000001	0.00077993
3	Yes	4	0.0000001	0.00043461
4	Yes	5	0.0000001	0.00090305
5	Yes	5	0.0000001	0.00042554
6	Yes	5	0.0000001	0.00088979
7	Yes	5	0.0000001	0.00041933
8	Yes	4	0.0000001	0.00063493
9	Yes	4	0.0000001	0.00031224
10	Yes	5	0.0000001	0.00089108
11	Yes	5	0.0000001	0.00041773
12	Yes	5	0.0000001	0.00095135
13	Yes	5	0.0000001	0.00044795
14	Yes	5	0.0000001	0.00006259
15	Yes	4	0.0000001	0.00071759
16	Yes	5	0.0000001	0.00087578
17	Yes	5	0.0000001	0.00041219
18	Yes	5	0.0000001	0.00088091
19	Yes	5	0.0000001	0.00041526
20	Yes	4	0.0000001	0.00096805
21	Yes	4	0.0000001	0.00057753
22	Yes	5	0.0000001	0.00094446
23	Yes	5	0.0000001	0.00044484
24	Yes	5	0.0000001	0.00089245
25	Yes	5	0.0000001	0.00041779
26	Yes	4	0.0000001	0.00004036
27	Yes	5	0.0000001	0.00066783
28	Yes	5	0.0000001	0.00083857
29	Yes	5	0.0000001	0.00083799
30	Yes	5	0.0000001	0.00066312
31	Yes	5	0.0000001	0.00083904
32	Yes	5	0.0000001	0.00083662
33	Yes	5	0.0000001	0.00065991
34	Yes	5	0.0000001	0.00082235
35	Yes	5	0.0000001	0.00081901
36	Yes	5	0.0000001	0.00065577
37	Yes	5	0.0000001	0.00083582
38	Yes	5	0.0000001	0.00084222
39	Yes	4	0.0000001	0.00006757
40	Yes	4	0.0000001	0.00025103
41	Yes	4	0.0000001	0.00024080
42	Yes	4	0.0000001	0.00005868
43	Yes	4	0.00000001	0.00022808
44	Yes	4	0.00000001	0.00027788
45	Yes	4	0.00000001	0.00007163
46	Yes	4	0.0000001	0.00022678
47	Yes	4	0.00000001	0.00023253
48	Yes	4	0.00000001	0.00006210
49	Yes	4	0.0000001	0.00027353
50	Yes	4	0.00000001	0.00022806

### **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	150 - 104.5	19.108	50	1.2173	0.0053
L2	108.75 - 68.75	9.547	50	0.8971	0.0015
L3	74 - 34	4.226	50	0.5467	0.0005
L4	40 - 0	1.233	50	0.2813	0.0002

### **Critical Deflections and Radius of Curvature - Service Wind**

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
152.00	Lighting Rod 5/8" x 5'	50	19.108	1.2173	0.0053	36383
147.00	P90-14-XLH-RR w/ Mount Pipe	50	18.357	1.1965	0.0050	36383
138.00	MX08FRO665-21 w/ Mount Pipe	50	16.121	1.1333	0.0040	15159

### **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	150 - 104.5	81.603	24	5.2027	0.0218
L2	108.75 - 68.75	40.798	24	3.8375	0.0059
L3	74 - 34	18.060	24	2.3377	0.0022
L4	40 - 0	5.268	24	1.2022	0.0008

### Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
152.00	Lighting Rod 5/8" x 5'	24	81.603	5.2027	0.0218	8649
147.00	P90-14-XLH-RR w/ Mount Pipe	24	78.398	5.1142	0.0204	8649
138.00	MX08FRO665-21 w/ Mount Pipe	24	68.858	4.8451	0.0164	3602

### **Compression Checks**

### **Pole Design Data**

Section No.	Elevation	Size	L	Lu	KI/r	Α	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in²	K	K	$\phi P_n$
L1	150 - 104.5 (1)	TP28.1875x18x0.1875	45.50	0.00	0.0	16.097 2	-8.95	941.69	0.010
L2	104.5 - 68.75 (2)	TP35.75x26.8609x0.25	40.00	0.00	0.0	27.243 5	-13.42	1593.74	0.008
L3	68.75 - 34 (3)	TP43x34.0833x0.3125	40.00	0.00	0.0	41.014 0	-19.95	2399.32	0.008
L4	34 - 0 (4)	TP50x41.0375x0.3125	40.00	0.00	0.0	49.283 8	-29.27	2883.10	0.010

### **Pole Bending Design Data**

Section	Elevation	Size	M <sub>ux</sub>	$\phi M_{nx}$	Ratio	M <sub>uy</sub>	$\phi M_{ny}$	Ratio
No.					$M_{ux}$			$M_{uy}$
	ft		kip-ft	kip-ft	$\phi M_{nx}$	kip-ft	kip-ft	$\phi M_{ny}$
L1	150 - 104.5 (1)	TP28.1875x18x0.1875	291.09	583.66	0.499	0.00	583.66	0.000

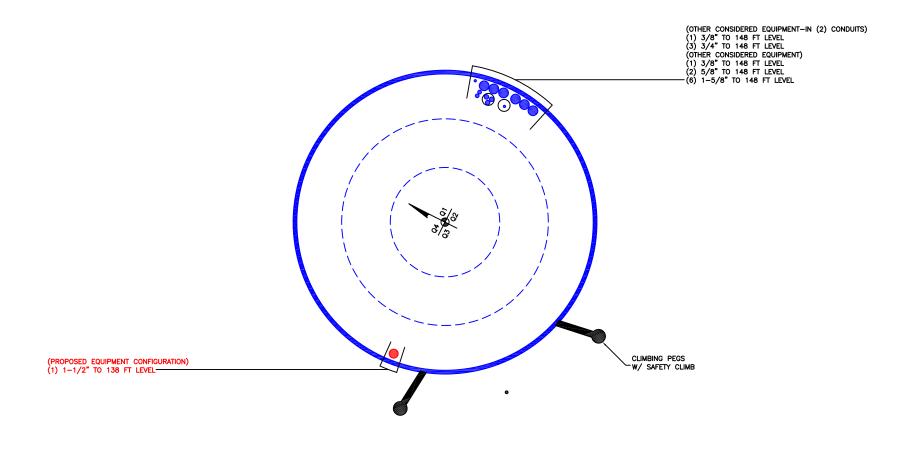
Section No.	Elevation	Size	$M_{ux}$	$\phi M_{nx}$	Ratio M <sub>ux</sub>	$M_{uy}$	$\phi M_{ny}$	Ratio M <sub>uy</sub>
740.	ft		kip-ft	kip-ft	$\frac{M_{ux}}{\phi M_{nx}}$	kip-ft	kip-ft	$\frac{M_{ny}}{\phi M_{ny}}$
L2	104.5 - 68.75 (2)	TP35.75x26.8609x0.25	644.15	1278.26	0.504	0.00	1278.26	0.000
L3	68.75 - 34 (3)	TP43x34.0833x0.3125	1057.59	2349.59	0.450	0.00	2349.59	0.000
L4	34 - 0 (4)	TP50x41.0375x0.3125	1625.61	3146.22	0.517	0.00	3146.22	0.000

Pole Shear Design Data								
Section No.	Elevation	Size	Actual V <sub>u</sub>	φV <sub>n</sub>	Ratio V <sub>u</sub>	Actual T <sub>u</sub>	φ <i>T</i> <sub>n</sub>	Ratio T <sub>u</sub>
	ft		K	K	$\overline{\phi V_n}$	kip-ft	kip-ft	$\phi T_n$
L1	150 - 104.5 (1)	TP28.1875x18x0.1875	9.18	282.51	0.032	0.93	669.19	0.001
L2	104.5 - 68.75 (2)	TP35.75x26.8609x0.25	11.15	478.12	0.023	0.84	1437.59	0.001
L3	68.75 - 34 (3)	TP43x34.0833x0.3125	13.13	719.80	0.018	0.74	2606.54	0.000
L4	34 - 0 (4)	TP50x41.0375x0.3125	15.25	864.93	0.018	0.63	3763.64	0.000

Pole Interaction Design Data									
Section No.	Elevation	Ratio P <sub>u</sub>	Ratio M <sub>ux</sub>	Ratio M <sub>uy</sub>	Ratio V <sub>u</sub>	Ratio T <sub>u</sub>	Comb. Stress	Allow. Stress	Criteria
	ft	$\phi P_n$	$\phi M_{nx}$	$\phi M_{nv}$	$\overline{\phi V_n}$	$\phi T_n$	Ratio	Ratio	
L1	150 - 104.5 (1)	0.010	0.499	0.000	0.032	0.001	0.509	1.050	4.8.2
L2	104.5 - 68.75 (2)	0.008	0.504	0.000	0.023	0.001	0.513	1.050	4.8.2
L3	68.75 - 34 (3)	0.008	0.450	0.000	0.018	0.000	0.459	1.050	4.8.2
L4	34 - 0 (4)	0.010	0.517	0.000	0.018	0.000	0.527	1.050	4.8.2

Section Capacity Table								
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	øP <sub>allow</sub> K	% Capacity	Pass Fail
L1	150 - 104.5	Pole	TP28.1875x18x0.1875	1	-8.95	988.77	48.5	Pass
L2	104.5 - 68.75	Pole	TP35.75x26.8609x0.25	2	-13.42	1673.43	48.8	Pass
L3	68.75 - 34	Pole	TP43x34.0833x0.3125	3	-19.95	2519.29	43.7	Pass
L4	34 - 0	Pole	TP50x41.0375x0.3125	4	-29.27	3027.25	50.2	Pass
							Summary	
						Pole (L4)	50.2	Pass
						RATING =	50.2	Pass

## APPENDIX B BASE LEVEL DRAWING



## APPENDIX C ADDITIONAL CALCULATIONS

### **Monopole Base Plate Connection**

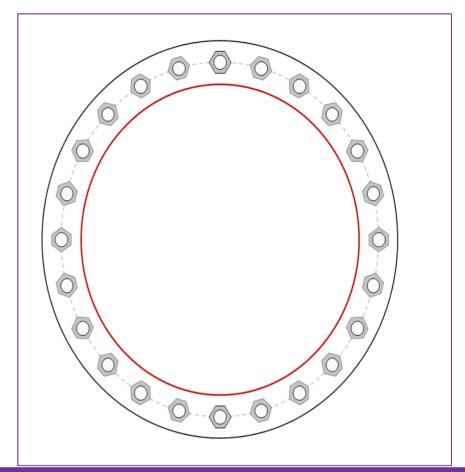


Site Info		
	BU#	857528
	Site Name	ODBURY PAPER MILL
	Order #	553361 Rev. 1

<b>Analysis Considerations</b>	
TIA-222 Revision	H
Grout Considered:	No
I <sub>ar</sub> (in)	2.25

Applied Loads	
Moment (kip-ft)	1625.60
Axial Force (kips)	29.27
Shear Force (kips)	15.25

<sup>\*</sup>TIA-222-H Section 15.5 Applied



Stress Rating:

### **Connection Properties**

### **Anchor Rod Data**

(24) 2-1/4" ø bolts (A615-75 N; Fy=75 ksi, Fu=100 ksi) on 57" BC

### **Base Plate Data**

64" OD x 2.25" Plate (A572-50; Fy=50 ksi, Fu=65 ksi)

### Stiffener Data

N/A

### **Pole Data**

50" x 0.3125" 18-sided pole (A572-65; Fy=65 ksi, Fu=80 ksi)

### **Analysis Results**

Anchor Rod Summary	(L	ınits of kips, kip-in)
Pu_t = 55.78	φPn_t = 243.75	Stress Rating
Vu = 0.64	φVn = 149.1	21.8%
Mu = n/a	φMn = n/a	Pass
Base Plate Summary		
Max Stress (ksi):	14.99	(Flexural)
Allowable Stress (ksi):	45	

31.7%

**Pass** 

CCIplate - Version 4.1.1 Analysis Date: 5/28/2021

### **Pier and Pad Foundation**

BU #: 857528
Site Name: WOODBURY PAP
App. Number: 553361 Rev. 1



TIA-222 Revision: H
Tower Type: Monopole

Top & Bot. Pad Rein. Different?:	
Block Foundation?:	
Rectangular Pad?:	

Superstructure Analysis Reactions				
Compression, P <sub>comp</sub> :	29.27	kips		
Base Shear, Vu_comp:	15.23	kips		
Moment, $\mathbf{M}_{\mathbf{u}}$ :	1625.6	ft-kips		
Tower Height, <b>H</b> :	150	ft		
BP Dist. Above Fdn, <b>bp</b> <sub>dist</sub> :	4.25	in		

Pier Properties				
Pier Shape:	Circular			
Pier Diameter, <b>dpier</b> :	6.5	ft		
Ext. Above Grade, E:	0.5	ft		
Pier Rebar Size, <b>Sc</b> :	10			
Pier Rebar Quantity, mc:	34			
Pier Tie/Spiral Size, <b>St</b> :	4			
Pier Tie/Spiral Quantity, <b>mt</b> :	5			
Pier Reinforcement Type:	Tie			
Pier Clear Cover, <b>cc</b> <sub>pier</sub> :	3	in		

Pad Properties				
Depth, D:	4	ft		
Pad Width, <b>W</b> <sub>1</sub> :	24	ft		
Pad Thickness, <b>T</b> :	2.5	ft		
Pad Rebar Size (Bottom dir. 2), Sp <sub>2</sub> :	10			
Pad Rebar Quantity (Bottom dir. 2), mp <sub>2</sub> :	31			
Pad Clear Cover, <b>cc</b> <sub>pad</sub> :	3	in		

Material Properties				
Rebar Grade, Fy:	60	ksi		
Concrete Compressive Strength, F'c:	4	ksi		
Dry Concrete Density, δ <b>c</b> :	150	pcf		

Soil Properties				
Total Soil Unit Weight, $\gamma$ :	90	pcf		
Ultimate Gross Bearing, Qult:	12.000	ksf		
Cohesion, <b>Cu</b> :	0.000	ksf		
Friction Angle, $oldsymbol{arphi}$ :	0	degrees		
SPT Blow Count, N <sub>blows</sub> :	79			
Base Friction, $\mu$ :	0.3			
Neglected Depth, N:	4.17	ft		
Foundation Bearing on Rock?	Yes			
Groundwater Depth, gw:	n/a	ft		

Foundation Analysis Checks				
	Capacity	Demand	Rating*	Check
Lateral (Sliding) (kips)	65.53	15.23	22.1%	Pass
Bearing Pressure (ksf)	9.00	1.76	19.6%	Pass
Overturning (kip*ft)	3233.33	1699.53	52.6%	Pass
Pier Flexure (Comp.) (kip*ft)	6219.92	1656.06	25.4%	Pass
Pier Compression (kip)	21120.36	41.22	0.2%	Pass
Pad Flexure (kip*ft)	4232.26	593.88	13.4%	Pass
Pad Shear - 1-way (kips)	685.65	98.62	13.7%	Pass
Pad Shear - 2-way (Comp) (ksi)	0.190	0.026	13.1%	Pass
Flexural 2-way (Comp) (kip*ft)	4903.88	993.64	19.3%	Pass

\*Rating per TIA-222-H Section 15.5

Structural Rating*:	25.4%
Soil Rating*:	52.6%

<--Toggle between Gross and Net



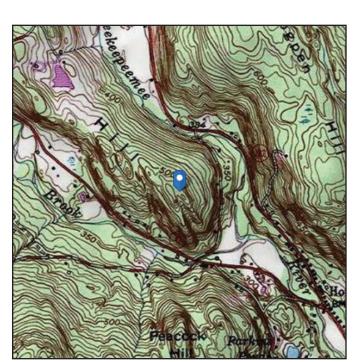
#### Address:

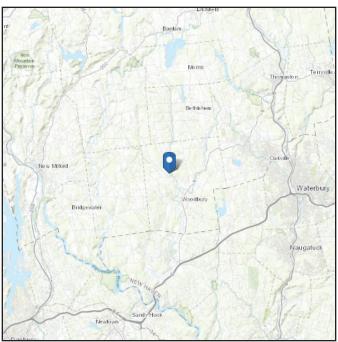
No Address at This Location

### ASCE 7 Hazards Report

Standard: ASCE/SEI 7-10 Elevation: 528.06 ft (NAVD 88)

Risk Category: || Latitude: 41.573075 Soil Class: D - Stiff Soil Longitude: -73.227642





### Wind

#### Results:

Wind Speed: 117 Vmph ← 120 mph Required by Jurisdiction

 10-year MRI
 76 Vmph

 25-year MRI
 85 Vmph

 50-year MRI
 90 Vmph

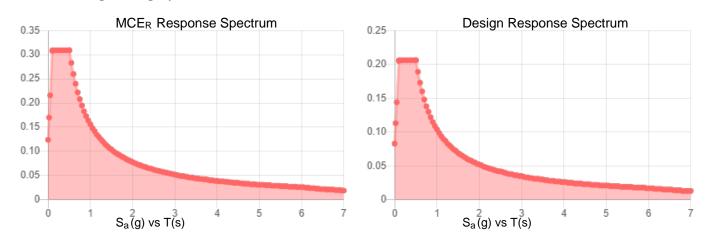
 100-year MRI
 96 Vmph



### **Seismic**

Site Soil Class: Results:	D - Stiff Soil			
S <sub>s</sub> :	0.194	$S_{ extsf{DS}}$ :	0.206	
$S_1$ :	0.065	S <sub>D1</sub> :	0.104	
F <sub>a</sub> :	1.6	T <sub>L</sub> :	6	
F <sub>v</sub> :	2.4	PGA:	0.101	
S <sub>MS</sub> :	0.309	PGA <sub>M</sub> :	0.161	
S <sub>M1</sub> :	0.156	F <sub>PGA</sub> :	1.598	
		l. ·	1	

### Seismic Design Category B



Data Accessed: Wed May 26 2021

Date Source: USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating

Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with

ASCE/SEI 7-10 Ch. 21 are available from USGS.



#### **Ice**

#### Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 15 F

Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Wed May 26 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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