



November 4, 2014

Melanie A. Bachman Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: Sprint PCS-Exempt Modification – Crown Site BU: 876380

Sprint PCS Site ID: CT33XC520

Located at: Great Hollow Road, Woodbury, CT 06798

Dear Ms. Bachman:

This letter and exhibits are submitted on behalf of Sprint PCS (Sprint). Sprint is making modifications to certain existing sites in its Connecticut system in order to implement their 2.5GHz LTE technology. Please accept this letter and exhibits as notification, pursuant to § 16-50j-73 of the Regulations of Connecticut State Agencies ("R.C.S.A."), of construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Mr. William J. Butterly, Jr., First Selectman for the Town of Woodbury, and O & G Industries, Inc., Property Owner.

Sprint plans to modify the existing wireless communications facility owned by Crown Castle and located at **Great Hollow Road**, **Woodbury**, **CT 06798**. Attached are a compound plan and elevation depicting the planned changes (Exhibit-1), and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration (Exhibit-2). Also included is a power density table report reflecting the modification to Sprint's operations at the site (Exhibit-3).

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes ("C.G.S.") § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in the R.C.S.A. § 16-50j-72(b)(2).

- 1. The proposed modifications will not result in an increase in the height of the existing tower. Sprint's additional antennas will be located at the same elevation on the existing tower.
- 2. There will be no proposed modifications to the ground and no extension of boundaries.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more.

- 4. A Structural Modification Report confirming that the tower and foundation can support Sprint's proposed modifications is included as Exhibit-2.
- 5. The operation of the additional antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table report for Sprint's modified facility is included as Exhibit-3.

For the foregoing reasons, Sprint respectfully submits the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Donna Neal.

Sincerely,

Susan Vale

Real Estate Specialist

Enclosures

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Mr. William J. Butterly, Jr., First Selectman

281 Main Street South Woodbury, CT 06798

cc: O & G Industries, Inc.

112 Wall Street

Torrington, CT 06790



SITE NUMBER:

CT33XC520

SITE NAME:

O&G WOODBURY

SITE ADDRESS:

GREAT HOLLOW ROAD WOODBURY, CT 06798

876380

CROWN SITE NAME: O&G WOODBURY

CROWN ID#:

SHEET INFORMATION SHEET INDEX VICINITY MAP (NOT TO SCALE) SITE NUMBER: CT33XC520 LANDLORD: CROWN CASTLE USA SHEET DESCRIPTION SHT. NO. 2000 CORPORATE DRIVE CANONSBURG, PA O&G WOODBURY SITE NAME: T-1 TITLE SHEET LOCAL POWER CONNECTICUT LIGHT AND POWER CONTACT CUSTOMER SERVICE SP-1 GENERAL NOTES SITE ADDRESS: GREAT HOLLOW ROAD (800) 286-2000 SP-2 GENERAL NOTES A-1SITE PLAN APPLICANT: SPRINT 6580 SPRINT PARKWAY COUNTY: LITCHFIELD A-2 ELEVATION OVERLAND PARK, COORDINATES: 41° 31' 19.2"N KANSAS 66251 ENLARGED EQUIPMENT LAYOUT PLANS (NAD 83) 73° 13' 15.6"W ENGINEER: JAMES QUICKSELL ANTENNA LAYOUT PLANS A-4 (845) 567-6656 EXT. 2835 JOnicksell@tector GROUND ELEV: 597'± AMSL SITE RAN WIRING DIAGRAM SPRINT CM: GARY WOOD A-6 CABLE DETAILS (860) 940-9168 STRUCTURE TYPE: MONOPOLE gary.wood@sprint.com EQUIPMENT DETAILS STRUCTURE HEIGHT: 139'-0"± AGL JASON D'AMICO S-2EQUIPMENT SCHEMATIC DETAILS (860) 209-0104 STRUCTURE RAD CENTER: ELECTRICAL & GROUNDING PLANS jason.d'amico@crowncastle.com E-1 108'-0"± AGL GROUNDING DETAILS & NOTES AAV: AT&T ZONING CLASSIFICATION: PARCEL ID: 034-015 REVIEWED BY 10/31/14 GENERAL NOTES AERIAL VIEW (NOT TO SCALE) **APPROVALS** THIS IS AN UNMANNED TELECOMMUNICATION FACILITY AND NOT FOR HUMAN HABITATION: THE FOLLOWING PARTIES HEREBY APPROVE AND ACCEPT THESE DOCUMENTS AND HANDICAP ACCESS REQUIREMENTS ARE NOT REQUIRED.

FACILITY HAS NO PLUMBING OR REFRIGERANTS.

THIS FACILITY SHALL MEET OR EXCEED ALL FAA AND FCC REGULATOR REQUIREMENTS. AUTHORIZE THE CONTRACTOR TO PROCEED WITH THE CONSTRUCTION DESCRIBED HEREIN.
ALL DOCUMENTS ARE SUBJECT TO REVIEW BY THE LOCAL BUILDING DEPARTMENT AND
MAY IMPOSE CHANGES OR MODIFICATIONS. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE PROJECT OWNER'S REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME. CONSTRUCTION: DEVELOPMENT AND USE OF THIS SITE WILL CONFORM TO ALL APPLICABLE CODES BUILDING CODE OF CONNECTICUT, LATEST EDITION. LEASING/ SITE ACQUISITION: ANSI/TIA/EIA-222-F-1996.
NATIONAL ELECTRICAL CODE, LATEST EDITION. SITE NAME: I ANDLORD PROJECT DESCRIPTION SITE SITE ADDRESS: 1. (1) NEW ALU 9929 EXPANSION CABINET. ____ DATE: ____ R.F. ENGINEER: 2. (3) NEW RFS APXVTM14-C-120 ANTENNAS. SHEET TITLE: 3. (3) NEW TD-RRH8x20-25 RRH. 4. (1) NEW 5/8" FIBER CABLE. SHEET NO: FOR CONNECTICUT T-1





TECTONIC

TECTONIC Engineering & Surveying Consultants P.C.

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	SL	JBMITTALS	
PRO	JECT NO	: 7225.CT33XC520	
NO	DATE	DESCRIPTION	BY
0	6/19/14	FOR COMMENT	DC
1	10/31/14	FOR CONSTRUCTION	DC
	20		
		æ	

JMG



SITE NUMBER CT33XC520

O&G WOODBURY

GREAT HOLLOW ROAD WOODBURY, CT 6798

TITLE SHEET

DIVISION 01000-GENERAL NOTES

- 1. THE CONTRACTOR SHALL GIVE ALL NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY, MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS, AND LOCAL AND STATE JURISDICTIONAL CODES BEARING ON THE PERFORMANCE OF THE WORK. THE WORK PERFORMED ON THE PROJECT AND THE MATERIALS INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES.
- 2. THE ARCHITECT/ENGINEER HAVE MADE EVERY EFFORT TO SET FORTH IN THE CONSTRUCTION AND CONTRACT DOCUMENTS THE COMPLETE SCOPE OF WORK. THE CONTRACTOR BIDDING THE JOB IS NEVERTHELESS CAUTIONED THAT MINOR OMISSIONS OR ERRORS IN THE DRAWINGS AND OR SPECIFICATIONS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING HE PROJECT AND IMPROVEMENTS IN ACCORDANCE WITH THE INTENT OF
- 3. THE CONTRACTOR OR BIDDER SHALL BEAR THE RESPONSIBILITY OF NOTIFYING (IN WRITING) THE PROJECT OWNER'S REPRESENTATIVE OF ANY CONFLICTS, ERRORS, OR OMISSIONS PRIOR TO THE SUBMISSION OF CONTRACTOR'S PROPOSAL OR PERFORMANCE OF WORK
- 4. THE SCOPE OF WORK SHALL INCLUDE FURNISHING ALL MATERIALS, EQUIPMENT, LABOR AND ALL OTHER MATERIALS AND LABOR DEEMED NECESSARY TO COMPLETE THE WORK/PROJECT AS DESCRIBED HEREIN.
- 5. THE CONTRACTOR SHALL VISIT THE JOB SITE PRIOR TO THE SUBMISSION OF BIDS OR PERFORMING WORK TO FAMILIARIZE HIMSELF WITH THE FIELD CONDITIONS AND TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- 6. ONCE THE CONTRACTOR HAS RECEIVED AND ACCEPTED THE NOTICE TO PROCEED, CONTRACTOR WILL CONTACT THE CROWN CASTLE CONSTRUCTION MANAGER OF RECORD (NOTED ON THE FIRST PAGE ON THIS CONSTRUCTION DRAWING) A MINIMUM OF 48 HOURS PRIOR TO WORK
 START. UPON ARRIVAL TO THE JOB SITE, CONTRACTOR CREW IS REQUIRED
 TO CALL 1–800–788–7011 TO NOTIFY THE CROWN CASTLE NOC WORK
- 7. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS ACCORDING TO THE MANUFACTURER'S/VENDOR'S SPECIFICATIONS UNLESS NOTED OTHERWISE OR WHERE LOCAL CODES OR ORDINANCES TAKE PRECEDENCE.
- 8. THE CONTRACTOR SHALL PROVIDE A FULL SET OF CONSTRUCTION DOCUMENTS AT THE SITE UPDATED WITH THE LATEST REVISIONS AND ADDENDUMS OR CLARIFICATIONS AVAILABLE FOR THE USE BY ALL PERSONNEL INVOLVED WITH THE PROJECT.
- 9. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS. METHODS. TECHNIQUES. SEQUENCES AND
- 10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS WHICH MAY BE REQUIRED FOR THE WORK BY THE ARCHITECT/ENGINEER, THE STATE, COUNTY OR LOCAL GOVERNMENT
- 11. THE CONTRACTOR SHALL MAKE NECESSARY PROVISIONS TO PROTECT EXISTING IMPROVEMENTS, EASEMENTS, PAVING, CURBING, ETC. DURING CONSTRUCTION. UPON COMPLETION OF WORK, THE CONTRACTOR SHALL REPAIR ANY DAMAGE THAT MAY HAVE OCCURRED DUE TO CONSTRUCTION ON OR ABOUT THE PROPERTY.
- 12. THE CONTRACTOR SHALL KEEP THE GENERAL WORK AREA CLEAN AND HAZARD FREE DURING CONSTRUCTION AND DISPOSE OF ALL DIRT, DEBRIS, RUBBISH AND REMOVE EQUIPMENT NOT SPECIFIED AS REMAINING ON THE PROPERTY, PREMISES SHALL BE LEFT IN CLEAN CONDITION AND FREE FROM PAINT SPOTS, DUST, OR SMUDGES OF ANY NATURE
- 13. THE CONTRACTOR SHALL COMPLY WITH ALL PERTINENT SECTIONS OF THE THE CONTRACTOR SHALL COMPLY WITH ALL PERTINENT SECTIONS OF THE BASIC STATE BUILDING CODE, LATEST EDITION, AND ALL OSHA REQUIREMENTS AS THEY APPLY TO THIS PROJECT. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK SHALL BE RELOCATED AS DIRECTED BY THE ARCHITECT/ENGINEER. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR PIER DRILLING AROUND OR NEAR UTILITIES. THE CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT LIMITED TO A) FALL PROTECTION, B) CONFINED SPACE, C) ELECTRICAL SAFETY, D) TRENCHING AND EXCAVATION OF ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHICH INTERFERE WITH THE EXECUTION OF THE WORK SHALL BE REMOVED AND OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT THE POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK SUBJECT TO THE APPROVAL OF THE ARCHITECT/ENGINEER.
- 14. THE CONTRACTOR SHALL NOTIFY THE PROJECT OWNER'S REPRESENTATIVE IN WRITING WHERE A CONFLICT OCCURS ON ANY OF THE CONTRACT
 DOCUMENTS. THE CONTRACTOR IS NOT TO ORDER MATERIAL OR
 CONSTRUCT ANY PORTION OF THE WORK THAT IS IN CONFLICT UNTIL CONFLICT IS RESOLVED BY THE LESSEE/LICENSEE REPRESENTATIVE
- 15. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, PROPERTY
- 16. THE CONTRACTOR SHALL NOTIFY THE THE RF ENGINEER FOR ANTENNA AZIMUTH VERIFICATION (DURING ANTENNA INSTALLATION) PRIOR TO
- 17. THE CONTRACTOR SHALL SUBMIT AT THE END OF THE PROJECT A COMPLETE SET OF AS-BUILT DRAWINGS TO THE CLIENT REPRESENTATIVE.
- 18. REFER TO: CONSTRUCTION STANDARDS-SPRINT DOCUMENT EXHIBIT -STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES REV.
- 19. REFER TO: WEATHER PROOFING SPECS: EXCERPT EXH A-WIHRPRF-STD NSTR SPECS._157201110421855492.DOCM
- 20. REFER TO: COLOR CODING-SPRINT NEXTEL ANT AND LINE COLOR CODING (DRAFT) V3 09-08-11.PDF
- 21. REFER TO LATEST DOCUMENTATION REVISION.

DIVISION 03000-CONCRETE

- 1.03 APPLICABLE STANDARDS (USE LATEST EDITIONS)
- AC1-301 SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS.
- ACI-347 GUIDE TO FORM WORK FOR CONCRETE.

 ASTM C33- CONCRETE AGGREGATE

 ASTM C94 READY MIXED CONCRETE e. ASTM C150 PORTLAND CEMENT.
- ASTM C260 AIR—ENTRAINING ADMIXTURES FOR CONCRETE ASTM C309— LIQUID MEMBRANE FORMING COMPOUNDS FOR CURING CONCRETE.
- ASTM C494 CHEMICAL ADMIXTURES FOR CONCRETE ASTM A615— DEFORMED AND PLAIN BILLET—STEEL BARS FOR CONCRETE REINFORCEMENT J. ASTM A185- STEEL WELDED WIRE FABRIC (PLAIN) FOR CONCRETE REINFORCEMENT

1.04 QUALITY ASSURANCE

CONCRETE MATERIALS AND OPERATIONS SHALL BE TESTED AND INSPECTED BY THE ARCHITECT/ENGINEER AS DIRECTED BY THE CLIENT'S REPRESENTATIVE.

A. SURFACES AGAINST WHICH BACKFILL OR CONCRETE SHALL BE PLACED REQUIRE NO TREATMENT EXCEPT REPAIR OF DEFECTIVE

B. SURFACES THAT WILL BE PERMANENTLY EXPOSED SHALL PRESENT A UNIFORM FINISH PROVIDED BY THE REMOVAL OF FINS AND THE FILLING HOLES AND OTHER IRREGULARITIES WITH DRY PACK GROUT, OR BY SACKING WITH UTILITY OR ORDINARY GROUT.

- C. SURFACES THAT WOULD NORMALLY BE LEVEL AND WHICH WILL BE PERMANENTLY EXPOSED TO THE WEATHER SHALL BE SLOPED FOR DRAINAGE. UNLESS ENGINEER'S DESIGN DRAWING SPECIFIES A HORIZONTAL SURFACE OR SURFACES SUCH AS STAIR TREADS, WALLS, CURBS, AND PARAPETS SHALL BE SLOPED APPROXIMATELY 1/4" PER FOOT.
- D. SURFACES THAT WILL BE COVERED BY BACKFILL OR CONCRETE
- EXPOSED SLAB SURFACES SHALL BE CONSOLIDATED, SCREENED, FLOATED, AND STEEL TROWELED. HAND OR POWER-DRIVEN EQUIPMENT MAY BE USED FOR FLOATING. FLOATING SHALL BE STARTED AS SOON AS THE SCREENED SURFACE HAS ATTAINED A STIFFNESS TO PERMIT FINISHING OPERATIONS. OPERATIONS. ALL EDGES MUST HAVE A 3/4" CHAMFER.
- 1.04 QUALITY ASSURANCE CONCRETE MATERIALS AND OPERATIONS SHALL BE TESTED AND INSPECTED BY THE ENGINEER.

3.05 PATCHING

THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY UPON REMOVAL OF THE FORMS TO OBSERVE CONCRETE SURFACE CONDITIONS.
IMPERFECTIONS SHALL BE PATCHED ACCORDING TO THE ENGINEER'S

3.06 DEFECTIVE CONCRETE

THE CONTRACTOR SHALL NOTIFY OR REPLACE CONCRETE NOT CONFORMING TO REQUIRED LEVELS AND LINES, DETAILS, AND ELEVATIONS AS SPECIFIED IN ACI 301.

A. IMMEDIATELY AFTER PLACEMENT. THE CONTRACTOR SHALL PROTECT THE CONCRETE FROM PREMATURE DRYING, EXCESSIVELY HOT OR COLD TEMPERATURES, AND MECHANICAL INJURY. FINISHED WORK

- B. CONCRETE SHALL BE MAINTAINED WITH MINIMAL MOISTURE LOSS AT RELATIVELY CONSTANT TEMPERATURE FOR PERIOD NECESSARY FOR HYDRATION OF CEMENT AND HARDENING OF CONCRETE
- C. ALL CONCRETE SHALL BE WATER CURED PER ACCEPTABLE PRACTICES SPECIFIED BY ACI CODE (LATEST EDITION)

DIVISION 05000 - METALS

PART 1 - GENERAL

1.01 WORK INCLUDED

- THE WORK CONSISTS OF THE FABRICATION AND INSTALLATION OF ALL MATERIALS TO BE FURNISHED. AND WITHOUT LIMITING THE GENERALITY THEREOF, INCLUDING ALL EQUIPMENT, LABOR AND SERVICES REQUIRED FOR ALL STRUCTURAL STEEL WORK AND ALL ITEMS INCIDENTAL AS SPECIFIED AND AS SHOWN ON THE DRAWINGS:
- STEEL FRAMING INCLUDING BEAMS, ANGLES, CHANNELS AND PLATES. WELDING AND BOLTING OF ATTACHMENTS.

1.02 REFERENCE STANDARDS

- A. THE WORK SHALL CONFORM TO THE CODES AND STANDARDS OF THE FOLLOWING AGENCIES AS FURTHER CITED HEREIN:
- ASTM: AMERICAN SOCIETY FOR TESTING AND MATERIALS AS PUBLISHED ASIM: AMERICAN SOCIETY FOR TESTING AND MATERIALS AS IN "COMPILATION OF ASTM STANDARDS IN BUILDING CODES" OR LATEST EDITION.

 AWS: AMERICAN WELDING SOCIETY CODE OR LATEST EDITION.
- AISC: AMERICAN INSTITUTE OF STEEL CONSTRUCTION,
 "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" (LATEST EDITION).

PART 2 - PRODUCTS 2.01 MATERIALS

A. STRUCTURAL STEEL: SHALL COMPLY WITH THE REQUIREMENTS OF ASTM A36 AND A992 FOR STRUCTURAL STEEL.

ALL PROPOSED STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH AISC CODE AND ASTM SPECIFICATIONS (LATEST EDITION) ALL NEW STEEL SHALL CONFORM TO THE FOLLOWING.

 STRUCTURAL WIDE FLANGE: ASTM A992 Fy=50KSI. 2. MISCELLANEOUS STEEL (PLATES), CHANNELS, ANGLES, ETC): ASTM A36 (Fy=36KSI).

3.STRUCTURAL TUBING: ASTM A500 Gr. B (Fy=46KSI). 4. STEEL PIPE: ASTM A53 Gr B (Fy=35KSI).

2.02 WELDING

- ALL WELDING SHALL BE DONE BY CERTIFIED WELDERS. CERTIFICATION DOCUMENTS SHALL BE MADE AVAILABLE FOR ENGINEER'S AND/OR OWNER'S REVIEW IF REQUESTED.
- WELDING ELECTRODES FOR MANUAL SHIELDED METAL ARC WELDING SHALL CONFORM TO ASTM 1-233, E70 SERIES. BARE ELECTRODES AND GRANULAR FLUX USED IN THE SUBMERGED ARC PROCESS SHALL CONFORM TO AISC SPECIFICATIONS.
- FIELD WELDING SHALL BE DONE AS PER AWS D1.1 REQUIREMENTS VISUAL
- STUD WELDING SHALL BE ACCOMPLISHED BY CAPACITOR DISCHARGE (CD) WELDING TECHNIQUE USING CAPACITOR DISCHARGE STUD WELDER
- PROVIDE STUD FASTENERS OF MATERIALS AND SIZES SHOWN ON DRAWINGS OR AS RECOMMENDED BY THE MANUFACTURER FOR STRUCTURAL LOADINGS REQUIRED.
- F. FOLLOW MANUFACTURERS SPECIFICATIONS AND INSTRUCTIONS TO PROPERLY SELECT AND INSTALL STUD WELDS.

2.03 BOLTING

- BOLTS SHALL BE CONFORMING TO ASTM A35 HIGH STRENGTH HOT DIP GALVANIZED WITH ASTM A153 HEAVY HEX TYPE NUTS.
- BOLTS SHALL BE $3/4^{\prime\prime}$ (MINIMUM) CONFORMING TO ASTM A325, HOT DIP GALVANIZED, ASTM A153 NUTS SHALL BE HEAVY HEX TYPE.
- ALL CONNECTIONS SHALL BE 2 BOLTS MINIMUM.
- EXCEPT WHERE SHOWN, ALL BEAM TO BEAM AND BEAM TO COLUMN CONNECTIONS TO BE DOUBLE ANGLED CONNECTIONS WITH STRENGTH BOLTS (THREADS EXCLUDED FROM SHEAR PLANE) AND HARDENED WASHERS.
- STANDARD, OVERSIZED OR HORIZONTAL SHORT SLOTTED HOLES.
- SNUG-TIGHT STRENGTH BEARING BOLTS MAY BE USED IN STANDARD HOLES CONFORMING TO ACIS. USING THE TURN OF THE NUT METHOD
- FULLY-TENSIONED HIGH STRENGTH (SLIP CRITICAL) SHALL BE USED IN OVERSIZED SLOT HOLES (RESPECTIVE OF SLOT ORIENTATION)
- ALL BRACED CONNECTION, MOMENT CONNECTION AND CONNECTIONS NOTED AS "SLIP CRITICAL" SHALL BE BE SLIP CRITICAL JOINTS WITH CLASS A SURFACE CONDITIONS, UNLESS OTHERWISE NOTED.
- EPOXY ANCHOR ASSEMBLIES SHALL BE AS MANUFACTURED BY HILTI OR ENGINEER APPROVED EQUAL, AS FOLLOWS:

BASE MATERIAL

ANCHOR SYSTEM

CONCRETE HOLLOW & GROUTED CMU OR BRICK

HILTI HIT-HY 70

2.04 FABRICATION

A. FABRICATION OF STEEL SHALL CONFORM TO THE AISC AND AWS

2.05 FINISH

A. STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123. (LATEST EDITION) UNLESS OTHERWISE NOTED.

2.06 PROTECTION

A. UPON COMPLETION OF ERECTION, INSPECT ALL GALVANIZED STEEL AND PAINT ANY FIELD CUTS, WELDS OR GALVANIZED BREAKS WITH (2) COATS OF ZINC-RICH COLD GALVANIZING PAINT.

- A. PROVIDE ALL ERECTION, EQUIPMENT, BRACING, PLANKING, FIELD BOLTS, NUTS, WASHERS, DRIFT PINS, AND SIMILAR MATERIALS WHICH DO NOT FORM A PART OF THE COMPLETED
 CONSTRUCTION, BUT ARE NECESSARY FOR ITS
- B. ERECT AND ANCHOR ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC REFERENCE STANDARDS.
 ALL WORK SHALL BE ACCURATELY SET TO
 ESTABLISHED SUITABLE ATTACHMENTS TO THE CONSTRUCTION OF THE BUILDING
- C. TEMPORARY BRACING, GUYING, AND SUPPORT SHALL BE PROVIDED TO KEEP THE STRUCTURE
 SET AND ALIGNED AT ALL TIMES DURING
 CONSTRUCTION, AND TO PREVENT DANGER TO PERSONS AND PROPERTY. CHECK ALL
 TEMPORARY LOADS AND STAY WITHIN SAFE
 CAPACITY OF ALL BUILDING COMPONENTS.



OVERLAND PARK, KANSAS 66251



TECTONIC

TECTONIC Engineering & Surveying

1279 Route 300 Newburgh, NY 12550 Phone: (845) 567-6656 Fax: (845) 567-8703

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	SU	JBMITTALS	
PRO	DJECT NO	: 7225.CT33XC520	
NO	DATE	DESCRIPTION	BY
0	6/19/14	FOR COMMENT	DC
Ì	10/31/14	FOR CONSTRUCTION	DC



SITE NUMBER: CT33XC520

SITE NAME: O&G WOODBURY

SITE ADDRESS

GREAT HOLLOW ROAD WOODBURY, CT 6798

SHEET TITLE:

GENERAL NOTES

SHEET NO:

SP-1

DIVISION 13000-SPECIAL CONSTRUCTION ANTENNA INSTALLATION

PART 1 - GENERAL

1.01 WORK INCLUDED

- ANTENNAS AND HYBRIFLEX CABLES ARE FURNISHED BY CLIENT'S A. ANTIBINAS AND HIBRIES AGES AND PONISHED TO CLEEN S REPRESENTATIVE UNDER SEPARATE CONTRACT. THE CONTRACTOR SHALL ASSIST ANTENNA INSTALLATION CONTRACTOR IN TERMS OF COORDINATION AND SITE ACCESS. ERECTION SUBCONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPERTY.
- B. INSTALL ANTENNAS AS INDICATED ON DRAWINGS AND CLIENT'S REPRESENTATIVE SPECIFICATIONS.
- INSTALL GALVANIZED STEEL ANTENNA MOUNTS AS INDICATED ON DRAWINGS.
- D. INSTALL FURNISHED GALVANIZED STEEL OR ALUMINUM WAVEGUIDE AND PROVIDE PRINTOUT OF THAT RESULT
- INSTALL HYBRIFLEX CABLES AND TERMINATIONS BETWEEN ANTENNAS AND EQUIPMENT PER MANUFACTURER'S RECOMMENDATIONS. WEATHERPROOF ALL CONNECTORS BETWEEN THE ANTENNA AND EQUIPMENT PER MANUFACTURER'S REQUIREMENTS.
- G. ANTENNA AND HYBRIFLEX CABLE GROUNDING:
- ALL EXTERIOR #6 GREEN GROUND WIRE DAISY CHAIN CONNECTIONS ARE TO BE WEATHER SEALED WITH ANDREWS CONNECTOR/SPLICE WEATHERPROOFING KIT TYPE 3221213 OR
- ALL HYBRIFLEX CABLE GROUNDING KITS ARE TO BE INSTALLED ON STRAIGHT RUNS OF HYBRIFLEX CABLE (NOT WITHIN BENDS).

 1.02 RELATED WORK FURNISH THE FOLLOWING WORK AS SPECIFIED UNDER CONSTRUCTION DOCUMENTS, BUT COORDINATE WITH QOTHER TRADES PRIOR TO BID:
- FLASHING OF OPENING INTO OUTSIDE WALLS
- SEALING AND CAULKING ALL OPENINGS.
 PAINTING. 2. SEALING AND CAUCHING A
 3. PAINTING.
 4. CUTTING AND PATCHING.
- 1.03 REQUIREMENTS OF REGULATOR AGENCIES
- A. FURNISH U.L. LISTED EQUIPMENT WHERE SUCH LABEL IS
- FURNISH U.L. ISTIELD EQUIPMENT WHERE SUCH LABEL IS AVAILABLE. INSTALL IN CONFORMANCE WITH U.L. STANDARDS WHERE APPLICABLE. INSTALL ANTENNA, ANTENNA CABLES, GROUNDING SYSTEM IN ACCORDANCE WITH DRAWINGS AND SPECIFICATIONS IN EFFECT AT PROJECT LOCATION AND RECOMMENDATIONS OF STATE AND LOCAL BUILDING CODES HAVING JURISDICTION OVER SPECIFIC PORTIONS OF WORK. THIS WORK INCLUDES, BUT IS NOT LIMITED TO THE
- EIA ELECTRONIC INDUSTRIES ASSOCIATION RS-22 STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES.
- 2. FAA FEDERAL AVIATION ADMINISTRATION ADVISORY CIRCULAR AC 70/7480-IH. CONSTRUCTION MARKING AND LIGHTING.
- FCC FEDERAL COMMUNICATION COMMISSION RULES AND REGULATIONS FORM 715, OBSTRUCTION MARKING AND LIGHTING SPECIFICATION FOR ANTENNA STRUCTURES
- AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION FOR STRUCTURAL JOINTS USING ASTM 1325 OR A490 BOLTS.
- 5. NEC NATIONAL ELECTRIC CODE ON TOWER LIGHTING KITS.
- UL UNDERWRITER'S LABORATORIES APPROVED ELECTRICAL
- IN ALL CASES, PART 77 OF THE FAA RULES AND PARTS 17 AND 22 OF THE FCC RULES ARE APPLICABLE AND IN THE EVENT OF CONFLICT, SUPERSEDE ANY OTHER STANDARDS OR
- B. LIFE SAFETY CODE NFPA, LATEST EDITION.

DIVISION 13000-EARTHWORK

PART 1 GENERAL

- 1.01 WORK INCLUDED: REFER TO SURVEY AND SITE PLAN FOR WORK INCLUDED.
- CONSTRUCTION OF EQUIPMENT FOUNDATIONS
- INSTALLATION OF ANTENNA SYSTEM

PART 2 PRODUCTS

2.01 MATERIALS

- ROAD AND SITE MATERIALS; FILL MATERIAL SHALL BE ACCEPTABLE, SELECT FILL SHALL BE IN ACCORDANCE WITH LOCAL DEPARTMENT OF HIGHWAY AND PUBLIC TRANSPORTATION
- SOIL STERILIZER SHALL BE EPA REGISTERED OF LIQUID COMPOSITION AND OF PRE-EMERGENCE DESIGN.
- SOIL STABILIZER FABRIC SHALL BE MIRAFI OR EQUAL $-\ 600\mbox{X}$ AT ACCESS ROAD AND COMPOUND.
- GRAVEL FILL; WELL GRADED, HARD, DURABLE, NATURAL SAND AND GRAVEL, FREE FROM ICE AND SNOW, ROOTS, SOD RUBBISH, AND OTHER DELETERIOUS OR ORGANIC MATTER.

MATERIAL SHALL CONFORM TO THE FOLLOWING GRADATION

GRAVEL FILL TO BE PLACED IN LIFTS OF 9" MAXIMUM THICKNESS AND 90 % DENSITY. COMPACTED TO 95

E. NO FILL OR EMBANKMENT MATERIALS SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OF EMBANKMENT

2.02 EQUIPMENT

- COMPACTION SHALL BE ACCOMPLISHED BY MECHANICAL MEANS. LARGER AREAS SHALL BE COMPACTED BY SHEEPS FOOT, VIBRATORY OR RUBBER TIED ROLLERS WEIGHING AT LEAST FIVE TONS. SMALLER AREAS SHALL BE COMPACTED BY POWER-DRIVER, HAND HELD TAMPERS.
- PRIOR TO OTHER EXCAVATION AND CONSTRUCTION EFFORTS GRUB ORGANIC MATERIAL TO A MINIMUM OF 6" BELOW ORIGINAL GROUND
- UNLESS OTHERWISE INSTRUCTED BY CLIENT'S REPRESENTATIVE. REMOVE TREES, BRUSH AND DEBRIS FROM THE PROPERTY TO AN AUTHORIZED DISPOSAL LOCATION.
- PRIOR TO PLACEMENT OF FILL OR BASE MATERIALS, ROLL THE SOIL.
- WHERE UNSTABLE SOIL CONDITIONS ARE ENCOUNTERED, LINE THE GRUBBED AREAS WITH STABILIZER MAT PRIOR TO PLACEMENT OF FILL OR BASE MATERIAL.

- THE SITE AND TURNAROUND AREAS SHALL BE AT THE SUB-BASE COURSE ELEVATION PRIOR TO FORMING FOUNDATIONS. GRADE OR FILL THE SITE AND ACCESS ROAD AS REQUIRED TO PRODUCE EVEN DISTRIBUTION OF SPOILS RESULTING FROM FOUNDATION EXCAVATIONS. THE RESULTING GRADE SHALL CORRESPOND WITH SAID SUB-BASE COURSE, ELEVATIONS ARE TO BE CALCULATED FORM FINISHED GRADES OR SLOPES INDICATED.
- THE ACCESS ROAD SHALL BE BROUGHT TO BASE COURSE FLEVATION PRIOR TO FOUNDATION CONSTRUCTION.
- DO NOT CREATE DEPRESSIONS WHERE WATER MAY POND.
- THE CONTRACT INCLUDES ALL NECESSARY GRADING, BANKING, DITCHING AND COMPLETE SURFACE COURSE FOR ACCESS ROAD.
 ALL ROADS OR ROUTES UTILIZED FOR ACCESS TO PUBLIC
 THOROUGHFARE IS INCLUDED IN SCOPE OF WORK UNLESS
 OTHERWISE INDICATED.
- WHEN IMPROVING AN EXISTING ACCESS ROAD, GRADE THE EXISTING ROAD TO REMOVE ANY ORGANIC MATTER AND SMOOTH THE SURFACE BEFORE PLACING FILL OR STONE.
- F. PLACE FILL OR STONE IN 3" MAXIMUM LIFTS AND COMPACT
- THE FINISH GRADE, INCLUDING TOP SURFACE COURSE, SHALL EXTEND A MINIMUM OF 12" BEYOND THE SITE FENCE AND SHALL
- RIPRAP SHALL BE APPLIED TO THE SIDE SLOPES OF ALL FENCED AREAS, PARKING AREAS AND TO ALL OTHER SLOPES GREATER THAN 2:1.
- RIPRAP SHALL BE APPLIED TO THE SIDES OF DITCHES OR DRAINAGE SWALES AS INDICATED ON PLANS.
- RIPRAP ENTIRE DITCH FOR 6'-0" IN ALL DIRECTIONS AT CULVERT

- SEED, FERTILIZER AND STRAW COVER SHALL BE APPLIED TO ALL OTHER DISTURBED AREAS AND DITCHES, DRAINAGE, SWALES, NOT OTHERWISE RIP-RAPPED.
- UNDER NO CIRCUMSTANCES SHALL DITCHES, SWALES OR CULVERTS BE PLACED SO THEY DIRECT WATER TOWARDS, OR PERMIT STANDING WATER IMMEDIATELY ADJACENT TO SITE. IF OWNER DESIGNS OR IF DESIGN ELEVATIONS CONFLICT WITH THIS GUIDANCE ADVISE THE OWNER IMMEDIATELY.
- IF A DITCH LIES WITH SLOPE GREATER THAN TEN PERCENT, MOUND DIVERSIONARY HEADWALL IN THE DITCH AT CULVERT ENTRANCES. RIP—RAP THE UPSTREAM SIDE OF THE HEADWALL AS WELL AS THE DITCH FOR 6'-0" ABOVE THE CULVERT.
- IF A DITCH LIES WITH SLOPES GREATER THAN TEN PERCENT. MOUND DIVERSIONARY HEADWALLS IN THE DITCH FOR 6'-0'' ABOVE THE CULVERT ENTRANCE.
- SEED AND FERTILIZER SHALL BE APPLIED TO SURFACE CONDITIONS WHICH WILL ENCOURAGE ROOTING. RAKE AREAS TO BE SEEDED TO EVEN THE SURFACE AND TO LOOSEN THE SOIL.
- SOW SEED IN TWO DIRECTIONS IN TWICE THE QUANTITY RECOMMENDED BY THE SEED PRODUCER.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE GROWTH OF SEEDED AND LANDSCAPED AREAS BY WATERING UP TO THE POINT OF RELEASE FROM THE CONTRACT. CONTINUE TO REWORK BARE AREAS UNTIL COMPLETE COVERAGE IS OBTAINED.

3.04 FIELD QUALITY CONTROL

- A. COMPACTION SHALL BE D-1557 FOR SITE WORK AND 95 % MAXIMUM DENSITY UNDER SLAB AREAS. AREAS OF SETTLEMENT WILL BE EXCAVATED AND REFILLED AT CONTRACTOR'S EXPENSE.
 REQUIRED. USE OF EROSION CONTROL MESH OR MULCH NET
 SHALL BE AN ACCEPTABLE ALTERNATIVE.
- B. THE COMPACTION TEST RESULTS SHALL BE AVAILABLE PRIOR TO THE CONCRETE POUR.

3.05 PROTECTION

- PROTECT SEEDED AREAS FORM EROSION BY SPREADING STRAW TO A UNIFORM LOOSE DEPTH OF 1"-2". STAKE AND TIE DOWN AS REQUIRED. USE OF EROSION CONTROL MESH OR MULCH NET SHALL BE AN ACCEPTABLE ALTERNATIVE.
- ALL TREES PLACED IN CONJUNCTION WITH A LANDSCAPE CONTRACT SHALL BE WRAPPED. TIED WITH HOSE PROTECTED WIRE AND SECURED TO STAKES EXTENDING 2'-0" INTO THE GROUND ON FOUR SIDES OF THE TREE.
- C. ALL EXPOSED AREAS SHALL BE PROTECTED AGAINST WASHOUTS AND SOIL EROSION. STRAW BALES SHALL BE PLACED AT THE INLET APPROACH TO ALL NEW OR EXISTING CULVERTS. REFER TO DETAILS ON DRAWINGS

SYMBOLS	ABBREVIATIONS
	GROUND WIRE
———E———E—	ELECTRIC
	TELEPHONE
CHE CHE CHE CHE CHE	OVERHEAD WIRE
	PROPERTY LINE
xxx	CHAIN LINK FENCE
A-1	ANTENNA MARK
(E)	EXISTING
(P)	PROPOSED DETAIL
DET #)	REFERENCE
\(\rightarrow \)	SURFACE ELEVATION



OVERLAND PARK, KANSAS 66251



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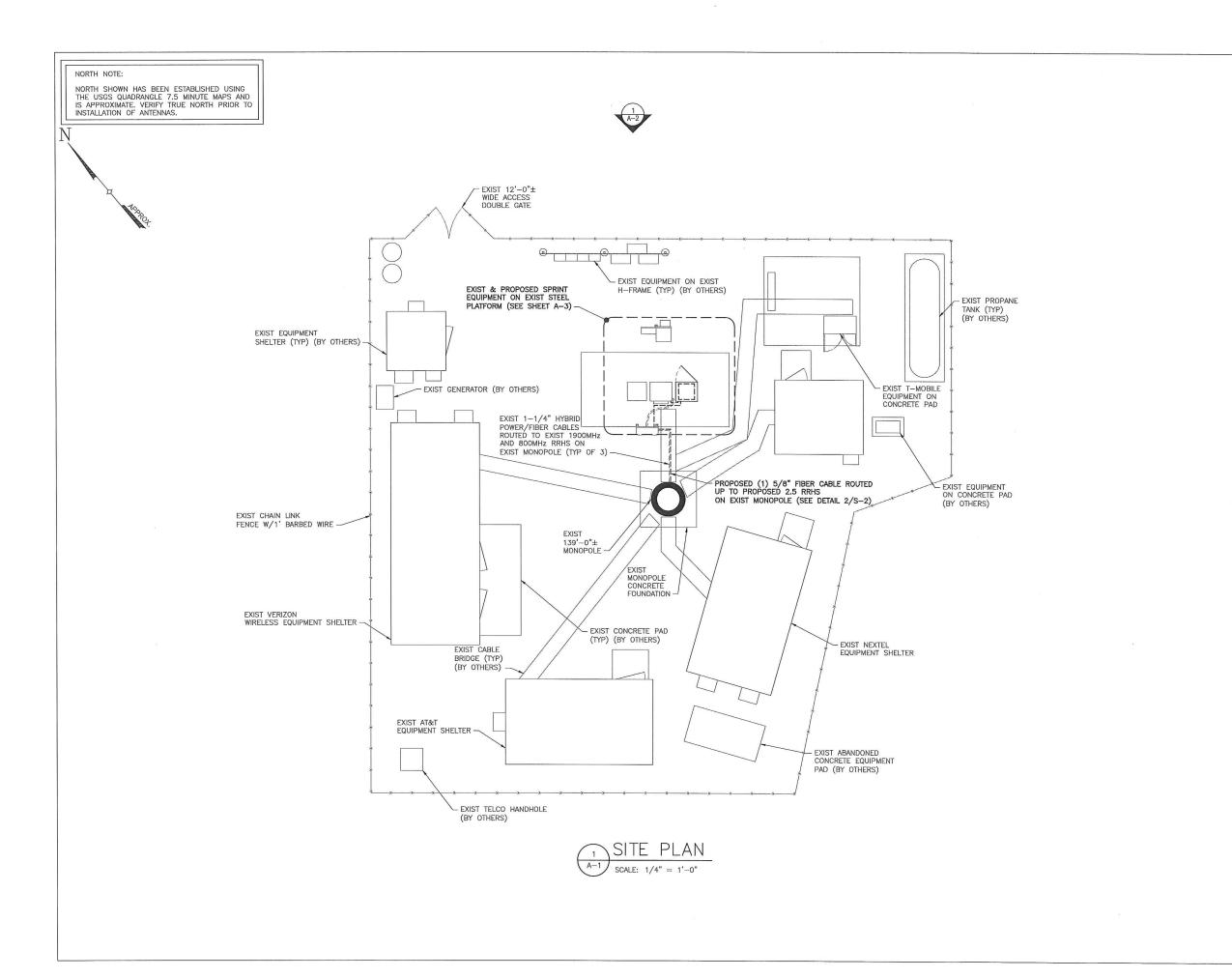
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SHEET TITLE:

GENERAL NOTES

SHEET NO:

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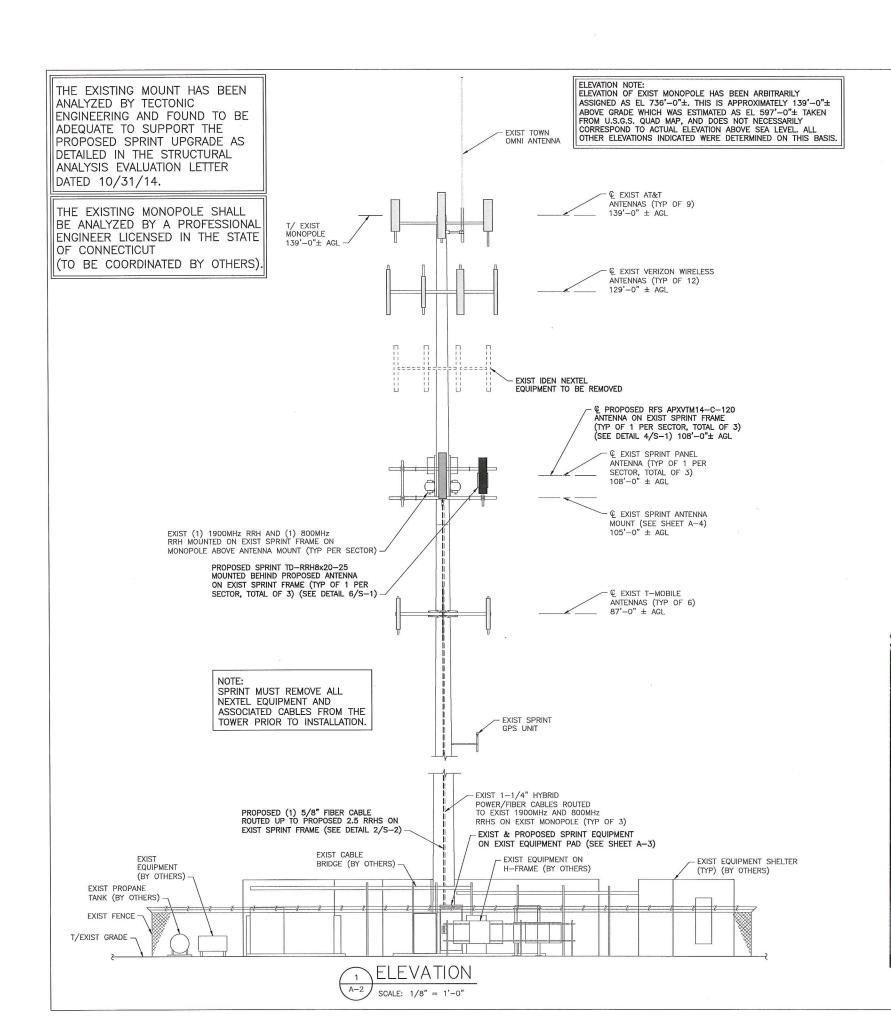
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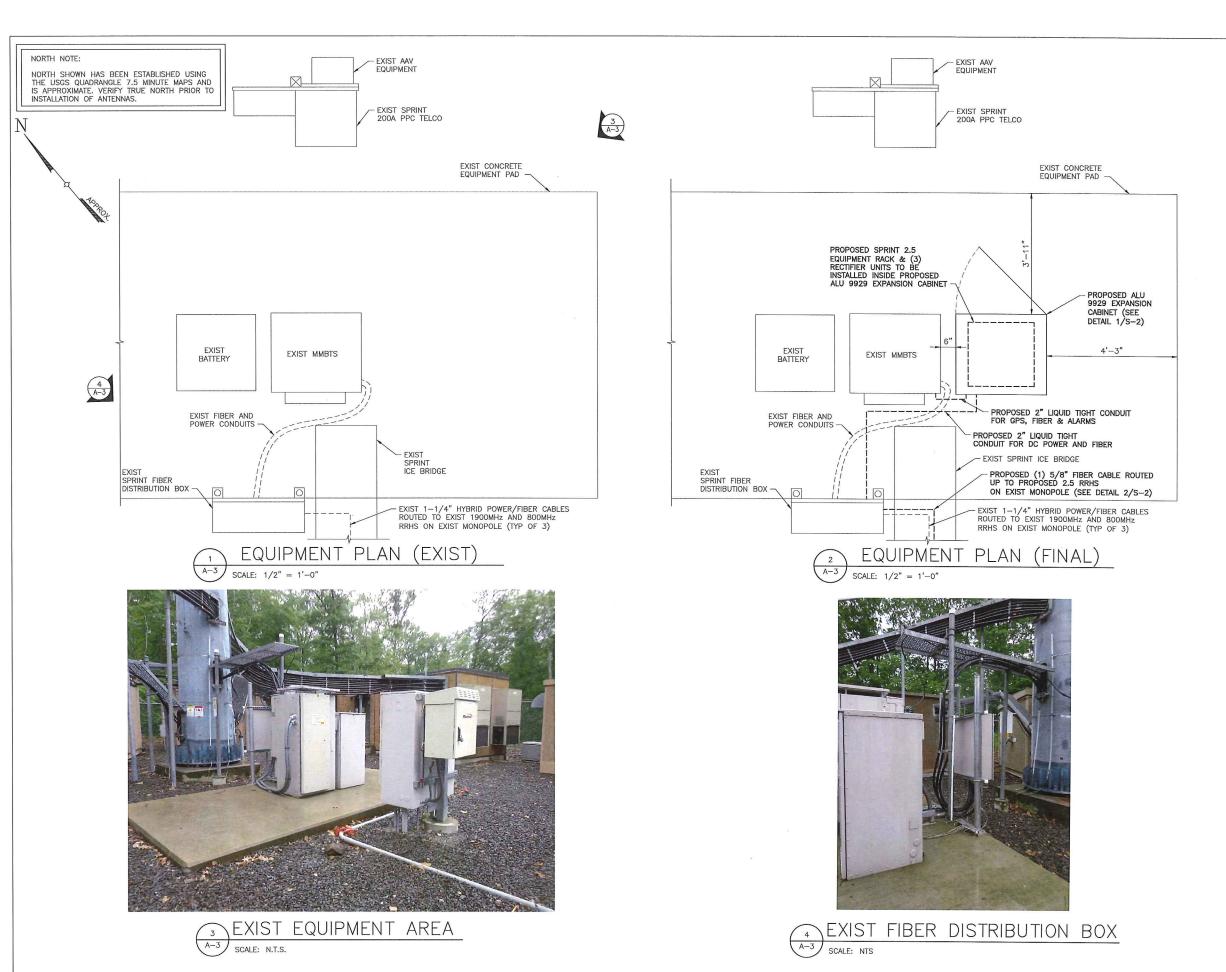
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2.5 EQUIPMENT DEPLOYMENT 6580 SPRINT PARKWAY OVERLAND PARK, KANSAS 66251



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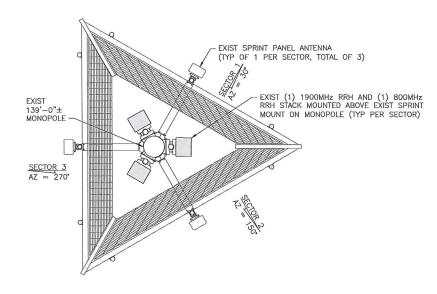
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SHEET TITLE:

ENLARGED EQUIPMENT LAYOUT PLANS

SHEET NO:



EXIST SPRINT PANEL ANTENNA (TYP OF 1 PER SECTOR, TOTAL OF 3) EXIST (1) 1900MHz RRH AND (1) 800MHz RRH STACK MOUNTED ON EXIST SPRINT FRAME ON MONOPOLE (TYP PER SECTOR)

THE EXISTING MOUNT HAS BEEN ANALYZED BY TECTONIC ENGINEERING AND FOUND TO BE ADEQUATE TO SUPPORT THE PROPOSED SPRINT UPGRADE AS DETAILED IN THE STRUCTURAL ANALYSIS EVALUATION LETTER DATED 10/31/14.

THE EXISTING MONOPOLE SHALL BE ANALYZED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF CONNECTICUT (TO BE COORDINATED BY OTHERS)





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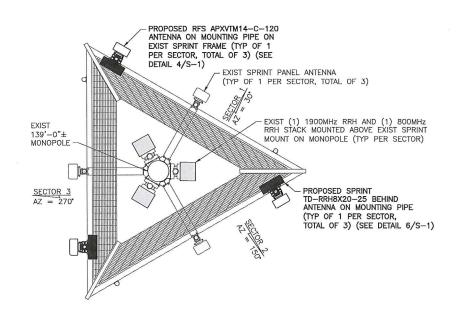
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ANTENNA LAYOUT PLANS

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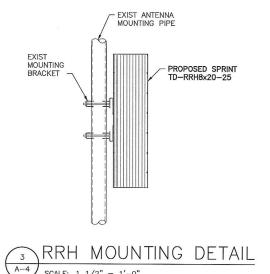
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ANTENNA LAYOUT PLAN (EXIST) SCALE: 3/8" = 1'-0"



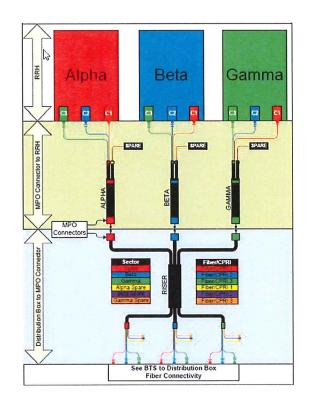
ANTENNA LAYOUT PLAN (FINAL)

SCALE: 3/8" = 1'-0"

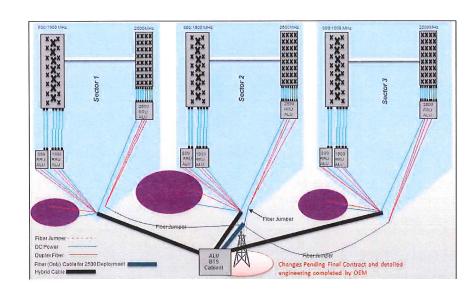


ANTENNA DATA

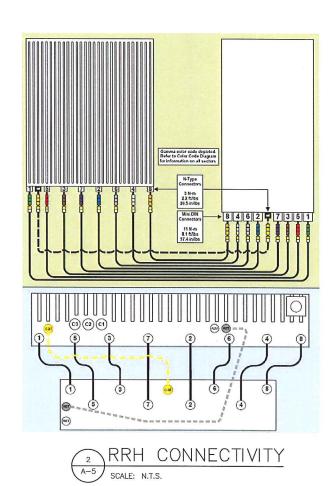
Status	Exist	Proposed
Antenna Manufacturer	RFS-CELWAVE	RFS-CELWAVE
Antenna Model Number	APXVSPP18-C-A20	APXV9TM14-ALV-120
Number of Antennas	3	3
Antenna RAD Center	108'	108'
Antenna Azimuth	30/150/270	30/150/270
Antenna RRH Model Number	800MHz/1900MHz	TD-RRH8x20-25
Number of RRH	6	3

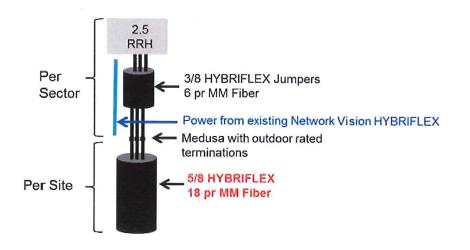


2.5 CABLE COLOR CODING SCALE: N.T.S.















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O&G WOODBURY

SITE ADDRESS

GREAT HOLLOW ROAD WOODBURY, CT 6798

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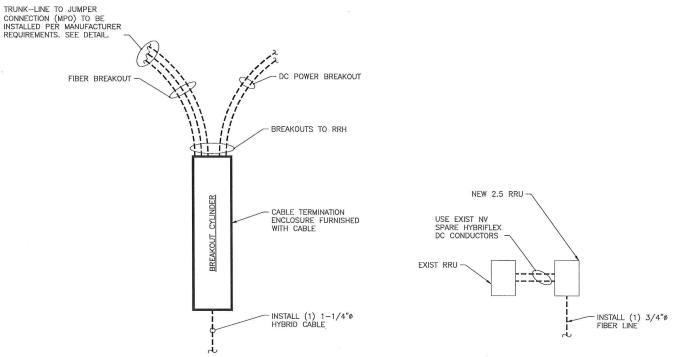
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IMPORTANTII LINE UP WHITE MARKINGS ON JUMPER AND RISER IP—MPO CONNECTOR. PUSH THE WHITE MARK ON THE JUMPER CONNECTOR FLUSH AGAINST THE RED SEAL ON THE RISER CONNECTION



IMPORTANTII ROTATE THE BAYONET HOUSING CLOCKWISE UNTIL A CLICK SOUND IS HEARD TO ENSURE A GOOD CONNECTION





2.5 HYBRID CABLE W/FIBER & DC FEEDERS

FIBER ONLY TRUNK LINES

HYBRIFLEX RISER/JUMPER CONNECTION DETAILS

SCALE: N.T.S.



SPECIAL NOTES: CABLE MARKINGS AT RAD CENTER AND ALL WALL/BLDG. PENETRATIONS

- ALL COLOR CODE TAPE SHALL BE 3M-35 AND SHALL BE INSTALLED USING A MINIMUM OF (3) WRAPS OF TAPE.
- . ALL COLOR BANDS INSTALLED AT THE TOWER TOP SHALL BE A MINIMUM OF 3" WIDE AND SHALL HAVE A MINIMUM OF 3/4" OF SPACING BETWEEN EACH COLOR.
- · ALL COLOR BANDS INSTALLED AT OR NEAR THE GROUND MAY BE ONLY 3/4" WIDE. EACH TOP-JUMPER SHALL BE COLOR CORDED WITH (1) SET OF 3" WIDE BANDS.
- EACH MAIN COAX SHALL BE COLOR CODED WITH (1) SET OF 3" BANDS NEAR THE TOP-JUMPER CONNECTION AND WITH 3/4" COLOR BANDS JUST PRIOR TO ENTERING THE BTS OR TRANSMITTER BUILDING.
- ALL BOTTOM JUMPERS SHALL BE COLOR CODED WITH (1) SET OF 3/4" BANDS ON EACH END OF THE BOTTOM JUMPER.
- ALL COLOR CODES SHALL BE INSTALLED SO AS TO ALIGN NEATLY WITH ONE ANOTHER
- EACH COLOR BAND SHALL HAVE A MINIMUM OF (3) WRAPS AND SHALL BE NEATLY TRIMMED AND SMOOTHED OUT AS TO AVOID UNRAVELING.
- X-POLE ANTENNAS SHOULD USE "XX-1" FOR THE "+45" PORT, "XX-2" FOR THE "-45"
- COLOR BAND #4 REFERS TO THE FREQUENCY BAND: ORANGE=850, VIOLET=1900. USED ON JUMPERS ONLY. • RF FEEDLINE SHALL BE IDENTIFIED WITH A METAL TAG (STAINLESS OR BRASS) AND
- STAMPED WITH THE SECTOR, ANTENNA POSITION, AND CABLE NUMBER. • ANTENNAS MUST BE IDENTIFIED, USING THE SECTOR LETTER AND ANTENNA NUMBER, WITH A BLACK MARKER PRIOR TO INSTALLATION.





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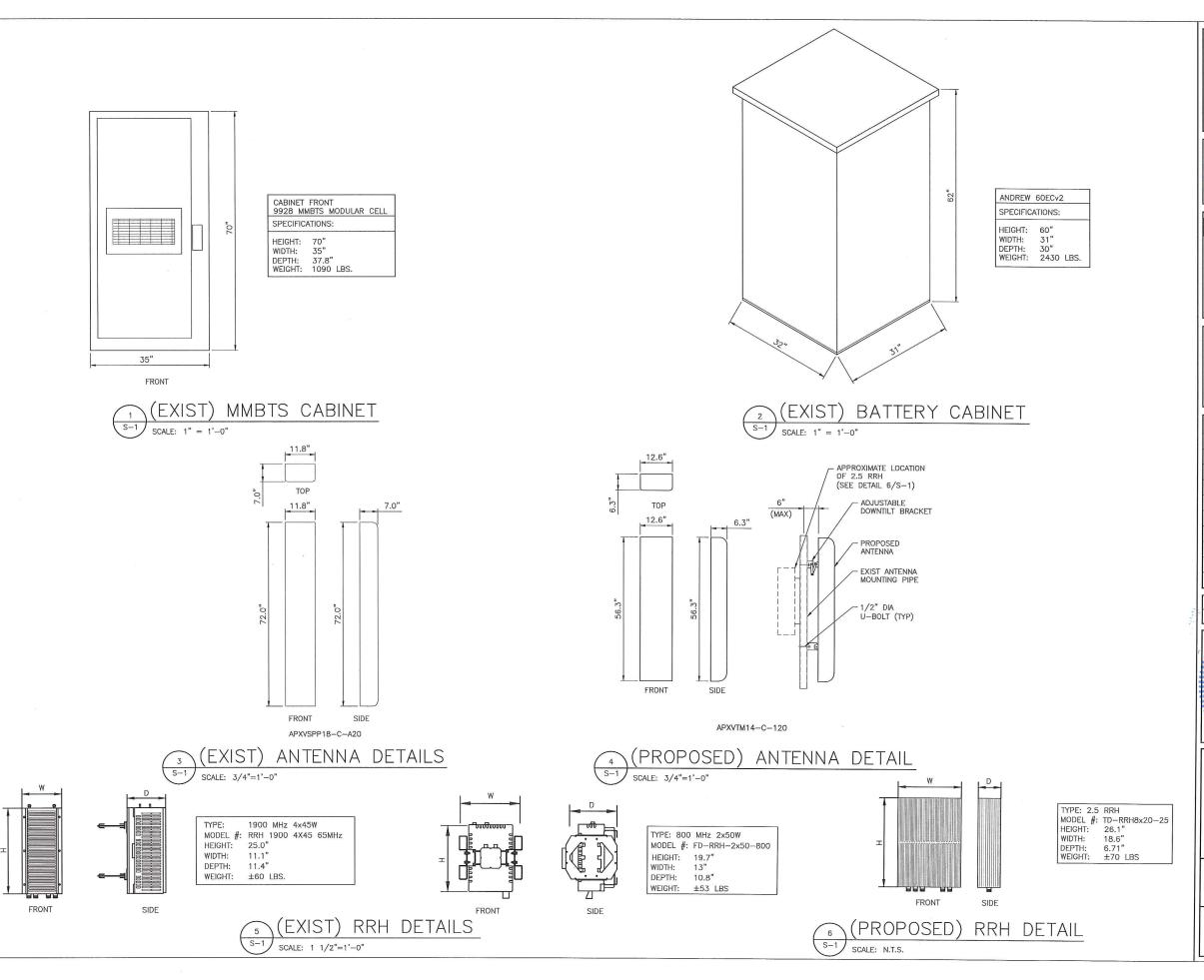
SITE NAME: O&G WOODBURY

GREAT HOLLOW ROAD WOODBURY, CT 6798

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CABLE DETAILS

SHEET NO:





2.5 EQUIPMENT DEPLOYMENT 6580 SPRINT PARKWAY OVERLAND PARK, KANSAS 66251



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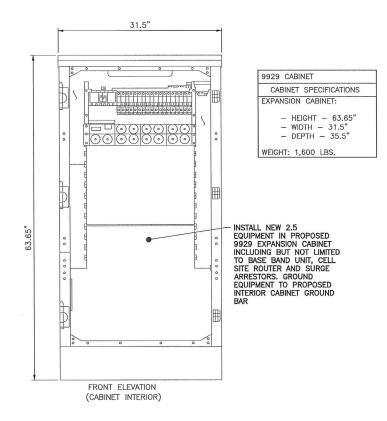
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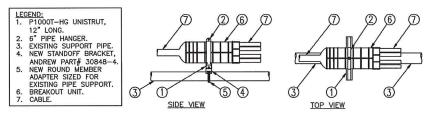
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9929 INTERIOR DETAIL SCALE: N.T.S.





RFS HYBRIFLEX RISER CABLES SCHEDULE

Fiber Only (Existing DC Power)	Hybrid cable MN: HB058-M12-050F 12x multi-mode fiber pairs, Top: Outdoor protected connectors, Bottom:LC Connectors. 5/8 cable. 50ft	50 ft
	MN: HB058-M12-075F	75 ft
	MN: HB058-M12-100F	100 ft
	MN:HB058-M12-125F	125 ft
	MN:HB058-M12-150F	150 ft
	MN:HB058-M12-175F	175 ft
	MN:HB058-M12-200F	200 ft

ver	Hybrid cable MN: H8114-08U3M12-050F 3x 8 AWG power pairs, 12x multi-mode fiber pairs, Outdoor rated connectors & LC Connectors. 11/4 cable. 50ft	50 ft
8 AWG Power	MN: HB114-08U3M12-075F	75 ft
٧	MN: HB114-08U3M12-100F	100 ft
ĕ	MN: HB114-08U3M12-125F	125 ft
w	MN: HB114-08U3M12-150F	150 ft
	MN: HB114-08U3M12-175F	175 ft
	MN: HB114-08U3M12-200F	200 ft

6 AWG Power	Hybrid cable MN: HB114-13U3M12-225F 3x 6 AWG power pairs, 12x multi-mode fiber pairs, Outdoor rated connectors & LC Connectors, 1 1/4 cable, 225ft	225 ft
6 AW	MN: HB114-13U3M12-250F	250 ft
	MN: HB114-13U3M12-275F	275 ft
	MN: HB114-13U3M12-300F	300 ft

NG Power	Hybrid cable MN: HB114-21U3M12-225F 3x 6 AWG power pairs, 12x multi-mode fiber pairs, Outdoor rated connectors & LC Connectors, 11/4 cable, 225ft	325 ft
A A	MN: HB114-21U3M12-350F	350 ft
1	MN: HB114-21U3M12-375F	375 ft

RFS HYBRIFLEX JUMPER CABLE SCHEDULE

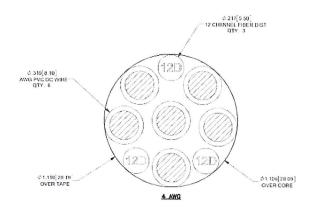
	Hybrid Jumper cable	
	MN: HBF012-M3-5F1	5 ft
_ <u>~</u>	5 ft, 3x multi-mode fiber pairs, Outdoor & LC connectors, 1/2 cable	
Fiber Only	MN: HBF012-M3-10F1	10 ft
þer	MN: HBF012-M3-15F1	15 ft
证	MN: HBF012-M3-20F1	20 ft
	MN: HBF012-M3-25F1	25 ft
	MN: HBF012-M3-30F1	30 ft

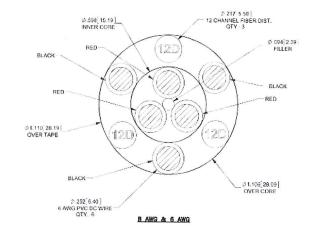
8 AWG Power	Hybrid Jumper cable MN: HBF058-08UJM3-5F1 5ft; 1x 8 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 5/8 cable	5 ft
9	MN: HBF058-08U1M3-10F1	10 ft
≩	MN: HBF058-08U1M3-15F1	15 ft
8	MN: HBF058-08U1M3-20F1	20 ft
	MN: HBF058-08U1M3-25F1	25 ft
	MN: HBF058-08U1M3-30F1	30 ft

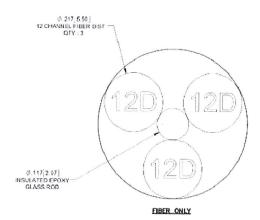
Power	Hybrid Jumper cable MN: HBF058-13U1M3-5F1 5 ft, 1x 6 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 5/8 cable	5 ft
6	MN: HBF058-13U1M3-10F1	10 ft
6 AWG	MN: HBF058-13U1M3-15F1	15 ft
	MN: HBF058-13U1M3-20F1	20 ft
	MN: HBF058-13U1M3-25F1	25 ft
	MN: HBF058-13U1M3-30F1	30 ft

Power	Hybrid Jumper cable MN: HBF078-21UJIM3-5F1 5ft, 1x 4 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 7/8 cable	5 ft
	MN: HBF078-21U1M3-10F1	10 ft
4 AWG	MN: HBF078-21U1M3-15F1	15 ft
	MN: HBF078-21U1M3-20F1	20 ft
	MN: HBF078-21U1M3-25F1	25 ft
	MN: HBF078-21U1M3-30F1	30 ft

HYBRID CABLE DC CONDUCTOR SIZE GUIDELINE					
MANUF:	RFS				
CABLE	<u>LENGTH</u>	DC CONDUCTOR	CABLE DIAMETER		
FIBER ONLY	VARIES	USE NV HYBRIFLEX	7/8"		
HYBRIFLEX	<200'	8 AWG	1-1/4"		
HYBRIFLEX	225-300'	6 AWG	1-1/4"		
HYBRIFLEX	325-375'	4 AWG	1-1/4"		











OVERLAND PARK, KANSAS 66251



TECTONIC

TECTONIC Engineering & Surveying Consultants P.C.

1279 Route 300 Newburgh, NY 12550 Phone: (845) 567-6656 Fax: (845) 567-8703 www.tectonicengineering.com

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NO	DATE	DESCRIPTION
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1	10/31/14	FOR CONSTRUCTION

REVIEWED BY 10 31 14 JMQ



SITE NUMBER: CT33XC520

SITE NAME: O&G WOODBURY

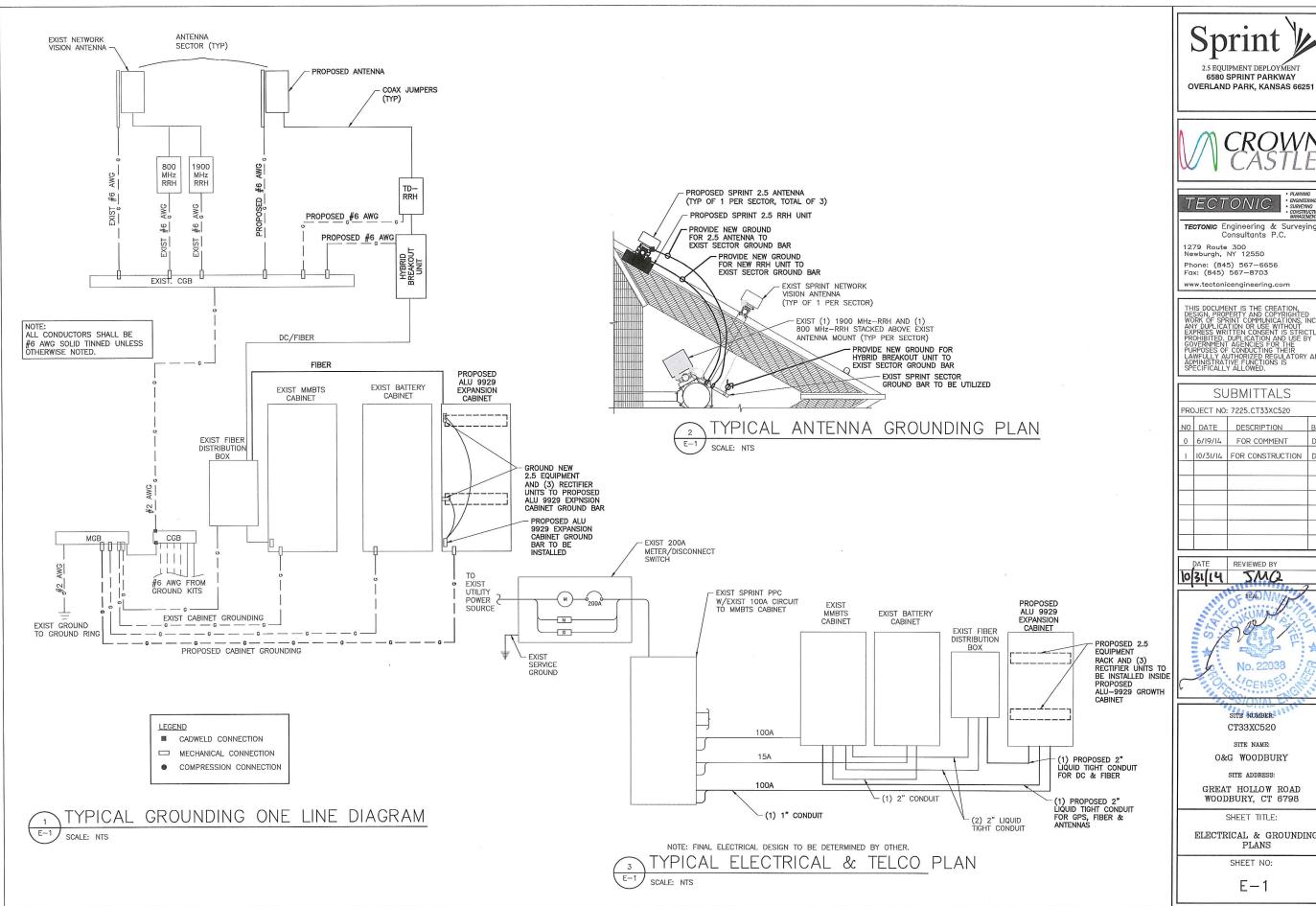
SITE ADDRESS:

GREAT HOLLOW ROAD WOODBURY, CT 6798

SHEET TITLE: EQUIPMENT SCHEMATIC DETAILS

SHEET NO:

S-2







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O&G WOODBURY

SITE ADDRESS:

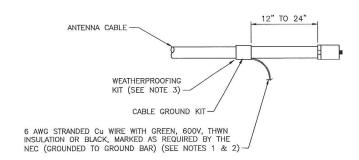
GREAT HOLLOW ROAD WOODBURY, CT 6798

SHEET TITLE:

ELECTRICAL & GROUNDING PLANS

SHEET NO:

E-1



CONNECTION OF CABLE GROUND KIT TO ANTENNA CABLE

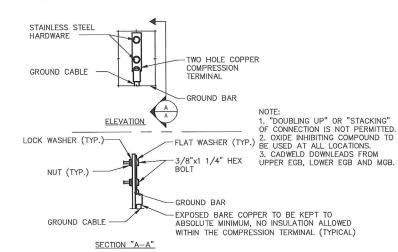
NOTES:

DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR

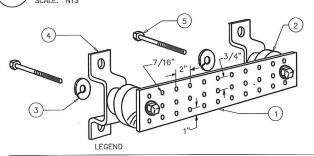
GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE

WEATHER PROOFING SHALL BE (TYPE AND PART NUMBER) AS SUPPLIED OR RECOMMENDED BY CABLE

CABLE GROUNDING KIT DETAIL SCALE: N.T.S.



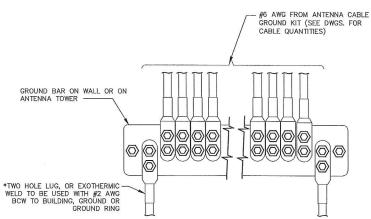
GROUNDING BAR CONN. DETAIL



- COPPER TINNED GROUND BAR, 1/4"X 4"X 20", OR OTHER LENGTH AS REQUIRED, HOLE CENTERS TO MATCH NEMA DOUBLE LUG CONFIGURATION
- INSULATORS, NEWTON INSTRUMENT CAT. NO. 3061-4 OR EQUAL
- 5/8" LOCKWASHERS OR EQUAL
- 4- WALL MOUNTING BRACKET, NEWTON INSTRUMENT CO. CAT NO. A-6056 OR EQUAL 5- 5/8-11 X 1" H.H.C.S.BOLTS

ALL BOLTS, NUTS, WASHERS AND LOCK WASHERS SHALL BE 18-8 STAINLESS STEEL.

GROUNDING BAR DETAIL SCALE: NTS



- * GROUND BARS AT THE BOTTOM OF TOWERS/MONOPOLES SHALL ONLY USE EXOTHERMIC WELDS
- ATTACH "DO NOT DISCONNECT" LABELS TO GROUND BARS. CAN USE BRASS TAG "DO NOT DISCONNECT" AT EACH HYBRID GROUND POINT OR BACK-A-LITE PLATE LABEL ON GROUND BAR.
- CONNECT SEQUENCE- BOLT/WASHER/NO-OX/GROUND BAR/NO-OX/WASHER/LOCK-WASHER/NUT. THIS IS REPEATED FOR EACH

ANTENNA GROUND BAR DETAIL

GROUNDING NOTES:

- 1. GROUNDING SHALL BE IN ACCORDANCE WITH NEC ARTICLE 250-GROUNDING AND BONDING,
- 2. ALL GROUND WIRES SHALL BE #2 AWG UNLESS NOTED OTHERWISE.
- 3. ALL GROUNDING WIRES SHALL PROVIDE A STRAIGHT, DOWNWARD PATH TO GROUND WITH
- 4. EACH EQUIPMENT CABINET SHALL BE CONNECTED TO THE MASTER ISOLATION GROUND BAR (MGB) WITH #2 AWG INSULATED STRANDED COPPER WIRE. EQUIPMENT CABINETS WALL HAVE (2)
- 5. PROVIDE DEDICATED #2 AWG COPPER GROUND WIRE FROM FACH ANTENNA MOUNTING PIPE
- 6. THE CONTRACTOR SHALL VERIFY THAT THE EXISTING GROUND BARS HAVE ENOUGH SPACE/HOLES FOR ADDITIONAL TWO HOLE LUGS.
- 7. ALL CONDUITS SHALL BE RIGID GALVANIZED STEEL AND SHALL BE PROVIDED WITH GROUNDING BUSHINGS.
- 8. PROVIDE GROUND CONNECTIONS FOR ALL METALLIC STRUCTURES, ENCLOSURES, RACEWAYS AND OTHER CONDUCTIVE ITEMS ASSOCIATED WITH THE INSTALLATION OF CARRIER'S EQUIPMENT.
- 9. WHEN CABLE LENGTH IS OVER 20' THE MANUFACTURERS GROUND KIT MUST BE INSTALLED PER THE MANUFACTURERS SPECIFICATIONS.
- 10. REFER TO "ANTI-THEFT UPDATE TO SPRINT GROUNDING 082412.PDF" FOR GUIDELINE TO SUSPECTED OR ACTUAL THEFT OF GROUNDING.
- 11. HOME RUN GROUNDS ARE NOT APPROVED BY CROWN CASTLE CONSTRUCTION STANDARDS AND THAT ANTENNA BUSS BARS SHOULD BE INSTALLED DIRECTLY TO TOWER STEEL WITHOUT INSULATORS OR DOWN CONDUCTORS.

PROTECTIVE GROUNDING SYSTEM GENERAL NOTES:

- 1. AT ALL TERMINATIONS AT EQUIPMENT ENCLOSURES, PANEL, AND FRAMES OF EQUIPMENT AND WHERE EXPOSED FOR GROUNDING. CONDUCTOR TERMINATION SHALL BE PERFORMED UTILIZING TWO HOLE BOLTED TONGUE COMPRESSION TYPE LUGS WITH STAINLESS STEEL SELF-TAPPING SCREWS
- 2. ALL CLAMPS AND SUPPORTS USED TO SUPPORT THE GROUNDING SYSTEM CONDUCTORS AND PVC CONDUITS SHALL BE PVC TYPE (NON CONDUCTIVE). DO NOT USE METAL BRACKETS OR SUPPORTS WHICH WOULD FORM A COMPLETE RING AROUND ANY GROUNDING CONDUCTOR.
- 3. ALL GROUNDING CONNECTIONS SHALL BE COATED WITH A COPPER SHIELD ANTI-CORROSIVE AGENT SUCH AS T&B KOPR SHIELD. VERIFY PRODUCT WITH PROJECT MANAGER.
- 4. ALL BOLTS, WASHERS, AND NUTS USED ON GROUNDING CONNECTIONS SHALL BE STAINLESS STEEL,
- 5. INSTALL GROUND BUSHING ON ALL METALLIC CONDUITS AND BOND TO THE EQUIPMENT GROUND BUS IN THE PANEL BOARD.
- 6. GROUND ANTENNA BASES, FRAMES, CABLE RACKS, AND OTHER METALLIC COMPONENTS WITH #2 INSULATED TINNED STRANDED COPPER GROUNDING CONDUCTORS AND CONNECT TO INSULATED SURFACE MOUNTED GROUND BARS. CONNECTION DETAILS SHALL FOLLOW MANUFACTURER'S SPECIFICATIONS FOR GROUNDING.
- 7. GROUND HYBRID CABLE SHIELD AT BOTH ENDS USING MANUFACTURER'S GUIDELINES.

ELECTRICAL AND GROUNDING NOTES

- 1. ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE NATIONAL ELECTRICAL CODE (NEC) AS WELL AS APPLICABLE STATE AND LOCAL CODES.
- ALL ELECTRICAL ITEMS SHALL BE U.L. APPROVED OR LISTED AND PROCURED PER SPECIFICATION REQUIREMENTS.
- 3. ELECTRICAL AND TELCO WRING OUTSIDE A BUILDING AND EXPOSED TO WEATHER SHALL BE IN WATER TIGHT GALVANIZED RIGID STEEL CONDUITS OR SCHEDULE 80 PVC (AS PERMITTED BY CODE) AND WHERE REQUIRED IN LIQUID TIGHT FLEXIBLE METAL OR NONMETALLIC CONDUITS
- 4. BURIED CONDUIT SHALL BE SCHEDULE 40 PVC.
- ELECTRICAL WIRING SHALL BE COPPER WITH TYPE XHHW, THWN, OR THNN INSULATION.
- 6. RUN TELCO CONDUIT OR CABLE BETWEEN TELEPHONE UTILITY DEMARCATION POINT AND PROJECT OWNER CELL SITE TELCO CABINET AND BTS CABINET AS INDICATED ON THIS DRAWING PROVIDE FULL LENGTH PULL ROPE IN INSTALLED TELCO PROVIDE GREENLEE CONDUIT MEASURING TAPE AT EACH END.
- 7. WHERE CONDUIT BETWEEN BTS AND PROJECT OWNER CELL SITE PPC AND BETWEEN BTS AND PROJECT OWNER CELL SITE TELCO SERVICE CABINET ARE UNDERGROUND USE PVC, SCHEDULE 40 CONDUIT, ABOVE THE GROUND PORTION OF THESE
- 8. ALL EQUIPMENT LOCATED OUTSIDE SHALL HAVE NEMA 3R ENCLOSURE.
- 9. GROUNDING SHALL COMPLY WITH NEC ART, 250.
- 10. GROUND HYBRID CABLE SHIELDS AT 3 LOCATIONS USING MANUFACTURER'S HYBRID CABLE GROUNDING KITS SUPPLIED BY PROJECT OWNER.
- 11. USE #2 COPPER STRANDED WIRE WITH GREEN COLOR INSULATION FOR ABOVE GRADE GROUNDING (UNLESS OTHERWISE SPECIFIED) AND #2 SOLID TINNED BARE COPPER WIRE FOR BELOW GRADE GROUNDING AS INDICATED ON THE DRAWING
- 12. ALL GROUND CONNECTIONS TO BE BURNDY HYGROUND COMPRESSION TYPE CONNECTORS OR CADWELD EXOTHERMIC WELD. DO NOT ALLOW BARE COPPER WIRE TO BE IN CONTACT WITH GALVANIZED STEEL.
- 13. ROUTE GROUNDING CONDUCTORS ALONG THE SHORTEST AND STRAIGHTEST PATH POSSIBLE, EXCEPT AS OTHERWISE INDICATED. GROUNDING LEADS SHOULD NEVER BE BENT AT RIGHT ANGLE. ALWAYS MAKE AT LEAST 12" RADIUS BENDS. #2 WIRE CAN BE BENT AT 6" RADIUS WHEN NECESSARY. BOND ANY METAL OBJECTS WITHIN 6 FEET OF PROJECT OWNER EQUIPMENT OR CABINET TO MASTER GROUND BAR OR
- 14. CONNECTIONS TO GROUND BARS SHALL BE MADE WITH TWO HOLE COMPRESSION TYPE COPPER LUGS. APPLY OXIDE INHIBITING COMPOUND TO ALL LOCATIONS.
- 15. APPLY OXIDE INHIBITING COMPOUND TO ALL COMPRESSION TYPE GROUND
- 16. BOND ANTENNA MOUNTING BRACKETS, HYBRID CABLE GROUND KITS, AND RRHs TO EGB PLACED NEAR THE ANTENNA LOCATION
- 17. BOND ANTENNA EGB'S AND MGB TO GROUND RING.
- 18. CONTRACTOR SHALL TEST COMPLETED GROUND SYSTEM AND RECORD RESULT FOR PROJECT CLOSE-OUT DOCUMENTATION, 5 OHMS MINIMUM RESISTANCE REQUIRED.
- 19. CONTRACTOR SHALL CONDUCT ANTENNA, HYBRID CABLES, GPS COAX AND RRH RETURN-LOSS AND DISTANCE- TO-FAULT MEASUREMENTS (SWEEP TESTS) AND RECORD RESULTS FOR PROJECT CLOSE OUT.
- 20. CONTRACTOR SHALL CHECK CAPACITY OF EXISTING SERVICE & PANEL ON SITE TO DETERMINE IF CAPACITY EXISTS TO ACCOMMODATE THE ADDED LOAD OF THIS PROJECT. ADVISE ENGINEER OF ANY DISCREPANCY.
- 21. LOCATION OF ALL OUTLET, BOXES, ETC, AND THE TYPE OF CONNECTION (PLUG OR DIRECT) SHALL BE CONFIRMED WITH THE OWNER'S REPRESENTATIVE PRIOR TO
- 22. ELECTRICAL CHARACTERISTICS OF ALL EQUIPMENT (NEW AND EXISTING) SHALL BE FIELD VERIFIED WITH THE OWNERS REPRESENTATIVE AND EQUIPMENT SUPPLIER PRIOR TO ROUGH—IN OF CONDUIT AND WIRE. ALL EQUIPMENT SHALL BE PROPERLY CONNECTED ACCORDING TO THE NAMEPLATE DATA FURNISHED ON THE EQUIPMENT.



6580 SPRINT PARKWAY **OVERLAND PARK, KANSAS 66251**



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PRO	JECT NO	: 7225.CT33XC520	
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1	10/31/14	FOR CONSTRUCTION	DO
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REVIEWED BY 10/31/4 JMG



SITE NUMBER CT33XC520

SITE NAME: O&G WOODBURY

GREAT HOLLOW ROAD WOODBURY, CT 6798

SHEET TITLE:

GROUNDING DETAILS & NOTES

SHEET NO:

E-2

Date: June 12, 2014

Veronica Harris Crown Castle 1200 McArthur Blvd Mahwah, NJ 07430



FDH Engineering, Inc. 6521 Meridien Drive Raleigh, NC 27616 (919) 755-1012

Scenario 2.5A

Subject: Structural Analysis Report

Carrier Designation: Sprint PCS Co-Locate

Carrier Site Number: CT33XC520

Crown Castle Designation: Crown Castle BU Number: 876380

> Crown Castle Site Name: **O&G WOODBURY**

Crown Castle JDE Job Number: 288224 Crown Castle Work Order Number: 773382

Crown Castle Application Number: 246007 Rev. 1

Engineering Firm Designation: FDH Engineering, Inc. Project Number: 1467431400

Site Data: Great Hollow Road, WOODBURY, Litchfield County, CT

Latitude 41° 31' 19.2", Longitude -73° 13' 15.6"

138.5 Foot - Monopole Tower

Dear Veronica Harris,

FDH Engineering, Inc. is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 655905, in accordance with application 246007, revision 1.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment

Mark S. Dages By Nr

Sufficient Capacity

Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

The analysis has been performed in accordance with the TIA/EIA-222-F standard and 2005 CT State Building Code based upon a wind speed of 80 mph fastest mile.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at FDH Engineering, Inc. appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:

Reviewed by:

Mark S. Girgis, El Project Engineer

Bradley R. Newman, PE Senior Project Engineer CT PE License No. 29630

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1) INTRODUCTION

2) ANALYSIS CRITERIA

- Table 1 Proposed Antenna and Cable Information
- Table 2 Existing and Reserved Antenna and Cable Information
- Table 3 Design Antenna and Cable Information

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- Table 4 Documents Provided
- 3.1) Analysis Method
- 3.2) Assumptions

4) ANALYSIS RESULTS

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- Table 6 Tower Component Stresses vs. Capacity
- 4.1) Recommendations

5) APPENDIX A

tnxTower Output

6) APPENDIX B

Base Level Drawing

7) APPENDIX C

Additional Calculations

1) INTRODUCTION

This tower is a 138.5 ft Monopole tower designed by ENGINEERED ENDEAVORS, INC. in April of 2003. The tower was originally designed for a wind speed of 85 mph per TIA/EIA-222-F. The tower was originally designed to be 150-ft but was only constructed to a height of 139 ft. The tower has been modified multiple times in the past to accommodate additional loading. The tower was modified per reinforcement drawings prepared by Semaan Engineering Solutions, Inc. in November of 2005. Reinforcement consists of base plate stiffeners. The tower was modified again per reinforcement drawings prepared by GPD Group in December of 2011. Reinforcement consists of additional anchor rods.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 80 mph with no ice, 28.1 mph with 1 inch ice thickness and 50 mph under service loads.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Flevation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
		3	alcatel lucent	TD-RRH8x20-25			
104.0	108.0	3	rfs celwave	APXVTM14-C-120 w/ Mount Pipe	1	1-1/4	-

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note		
139.0	140.0	6	ericsson	RRUS-11	_	_	1		
155.0	139.0	1	crown mounts	Side Arm Mount [SO 102-3]	_	_	'		
	148.0	1	dbspectra	DS9A09F36D-N	-	-	2		
		6	powerwave technologies	LGP2140X					
		6	css	XDUO1416-80 w/ Mount Pipe					
	139.0	1	kathrein	800 10764 w/ Mount Pipe					
	139.0	1	kmw communications	AM-X-CD-14-65-00T-RET w/ Mount Pipe	12	1-1/4			
138.0			1	kmw communications	AM-X-CD-16-65-00T-RET w/ Mount Pipe	2 1	7/16 3/8	1	
		1	raycap	DC6-48-60-18-8F					
				6	powerwave technologies	LGP21401			
	120.0	6	powerwave technologies	LGP21901					
	138.0	1	crown mounts	Platform Mount [LP 303-1]					
		1	bird technologies group	TTA-429-83H-08179	1 2	1/2 1-1/4	2		
		1	crown mounts	Side Arm Mount [SO 309-1]		1-1/-			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
137.0	147.0	1	telewave	ANT150F6	1	1/2	1
137.0	137.0	1	crown mounts	5' x 2' Pipe Mount	Į į	1/2	'
		3	antel	BXA-171085-8BF-EDIN-2 w/ Mount Pipe			
129.0	129.0	3	antel	BXA-70063/6CF-2 W/Mount Pipe	18	1-5/8	1
		6	antel	LPA-80080/6CF w/ Mount Pipe			
		1	crown mounts	Platform Mount [LP 304-1]			
		1	crown mounts	Platform Mount [LP 304-1]			
119.0	119.0 119.0		decibel	DB846G90A-XY w/ Mount Pipe	12	1-5/8	1
105.0	405.0	3	alcatel lucent	1900MHz RRH (65MHz)			_
105.0	105.0 105.0		crown mounts	Side Arm Mount [SO 102-3]	-	-	1
	108.0	3	rfs celwave	APXVSPP18-C-A20 w/ Mount Pipe			
		9	rfs celwave	ACU-A20-N		1-1/4	
104.0		3	alcatel lucent	800 EXTERNAL NOTCH FILTER	3		1
	104.0	3	alcatel lucent	800MHZ RRH			
		1	crown mounts	Miscellaneous [NA 507-1]			
		1	crown mounts	Platform Mount [LP 712-1]			
		12	airtech	KN-1870-15-4803			
87.0	87.0	1	crown mounts	Platform Mount [LP 305-1]	12	1-5/8	1
07.0	07.0	6	rfs celwave	APXV18-209014-C w/ Mount Pipe	12	1-3/0	1
70.0	71.0	1	lucent	KS24019-L112A	1	1/2	1
70.0		1	crown mounts	Side Arm Mount [SO 701-1]	'	1/2	'

Notes:

Existing Equipment
 Reserved Equipment

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
150	150	12	decibel	DB980F90	-	-
140	140	12	decibel	DB980F90	-	-
130	130	12	decibel	DB980F90	-	-
120	120	12	decibel	DB980F90	-	-
109	109	12	decibel	DB980F90	-	-
100	100	12	decibel	DB980F90		

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Clarence Welti Assoc, Inc.	1531967	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Engineered Endeavors, Inc.	2122534	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Engineered Endeavors, Inc.	1533002	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	Semaan Engineering Solutions	2055776	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	GPD Group	3030835	CCISITES
4-POST-INSTALLATION INSPECTION	GPD Group	3420974	CCISITES

3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.
- 5) Tower dimensions for elevation 108.5' to 138.5' was taken from Crown Castle Structural Analysis Report dated March 19, 2013 (Project No. 587330).

This analysis may be affected if any assumptions are not valid or have been made in error. FDH Engineering, Inc. should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	⊟evation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	138.5 - 108.5	Pole	TP24.5x17.375x0.1875	1	-6.79	752.20	77.8	Pass
L2	108.5 - 83.75	Pole	TP31.88x24.5x0.25	2	-11.75	1249.44	86.0	Pass
L3	83.75 - 43	Pole	TP43.42x30.0382x0.3125	3	-20.86	2131.32	87.2	Pass
L4	43 - 0	Pole	TP55.5x41.0206x0.3125	4	-32.94	2629.01	96.4	Pass
							Summary	
						Pole (L4)	96.4	Pass
						Rating =	96.4	Pass

Table 6 - Tower Component Stresses vs. Capacity - LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	67.5	Pass
1	Base Plate	0	99.3	Pass
1	Base Foundation	0	76.3	Pass
1	Flange Bolts	108.5	44.2	Pass
1	Flange Plate	108.5	30.8	Pass

Structure Rating (max from all components) =	99.3%
--	-------

Notes:

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the existing, reserved, and proposed loading. No modifications are required at this time.

¹⁾ See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

APPENDIX A TNXTOWER OUTPUT

0.1875 30.00 <u>ε</u>. 48 31.8800 0.2500 24.75 4.50 7 8 1.9 A572-65 45.25 0.3125 43.4200 က 8 43.0 ft AXIAL 54 K 49.00 SHEAR 55.5000 41.0206 8 TORQUE 0 kip-ft 28 mph WIND - 1.0000 in ICE AXIAL 33 K SHEAR' 0.0 ft 16.6 TORQUE 2 kip-ft Socket Length (ft) Number of Sides REACTIONS - 80 mph WIND Thickness (in) Top Dia (in) Bot Dia (in) Weight (K) Length (ft) Grade

DESIGNED APPURTENANCE LOADING

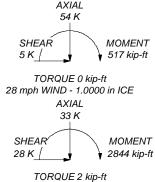
TYPE	ELEVATION	TYPE	ELEVATION
(2) RRUS-11	139	(4) DB846G90A-XY w/ Mount Pipe	119
(2) RRUS-11	139	(4) DB846G90A-XY w/ Mount Pipe	119
(2) RRUS-11	139	(4) DB846G90A-XY w/ Mount Pipe	119
Side Arm Mount [SO 102-3]	139	Platform Mount [LP 304-1]	119
TTA-429-83H-08179	138	1900MHz RRH (65MHz)	105
DS9A09F36D-N	138	1900MHz RRH (65MHz)	105
Side Arm Mount [SO 309-1]	138	1900MHz RRH (65MHz)	105
(2) LGP21401	138	Side Arm Mount [SO 102-3]	105
(2) LGP21401	138	APXVTM14-C-120 w/ Mount Pipe	104
(2) LGP21401	138	APXVTM14-C-120 w/ Mount Pipe	104
(2) LGP2140X	138	APXVTM14-C-120 w/ Mount Pipe	104
(2) LGP2140X	138	(3) ACU-A20-N	104
(2) LGP2140X	138	APXVSPP18-C-A20 w/ Mount Pipe	104
(2) LGP21901	138	(3) ACU-A20-N	104
(2) LGP21901	138	APXVSPP18-C-A20 w/ Mount Pipe	104
(2) LGP21901	138	(3) ACU-A20-N	104
(2) XDUO1416-80 w/ Mount Pipe	138	APXVSPP18-C-A20 w/ Mount Pipe	104
(2) XDUO1416-80 w/ Mount Pipe	138	TD-RRH8x20-25	104
(2) XDUO1416-80 w/ Mount Pipe	138	TD-RRH8x20-25	104
AM-X-CD-16-65-00T-RET w/ Mount	138	TD-RRH8x20-25	104
Pipe	100	800 EXTERNAL NOTCH FILTER	104
800 10764 w/ Mount Pipe	138	800MHZ RRH	104
AM-X-CD-14-65-00T-RET w/ Mount	138	800 EXTERNAL NOTCH FILTER	104
Pipe		800MHZ RRH	104
DC6-48-60-18-8F	138	800 EXTERNAL NOTCH FILTER	104
Platform Mount [LP 303-1]	138	800MHZ RRH	104
ANT150F6	137	Platform Mount [LP 712-1]	104
5' x 2' Pipe Mount	137	Miscellaneous [NA 507-1]	104
(2) LPA-80080/6CF w/ Mount Pipe	129	(4) KN-1870-15-4803	87
(2) LPA-80080/6CF w/ Mount Pipe	129	(4) KN-1870-15-4803	87
(2) LPA-80080/6CF w/ Mount Pipe	129	(4) KN-1870-15-4803	87
BXA-171085-8BF-EDIN-2 w/ Mount	129	(2) APXV18-209014-C w/ Mount Pipe	87
Pipe		(2) APXV18-209014-C w/ Mount Pipe	87
BXA-171085-8BF-EDIN-2 w/ Mount	129		87
Pipe		(2) APXV18-209014-C w/ Mount Pipe Empty Pipe Mount	87
BXA-171085-8BF-EDIN-2 w/ Mount Pipe	129	Empty Pipe Mount	87
1 .	120	Empty Pipe Mount	87
BXA-70063/6CF-2 W/Mount Pipe	129	Platform Mount [LP 305-1]	87
BXA-70063/6CF-2 W/Mount Pipe	129	KS24019-L112A	-
BXA-70063/6CF-2 W/Mount Pipe	-		70
Platform Mount [LP 304-1]	129	Side Arm Mount [SO 701-1]	70

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
AE72 GE	65 kgi	90 kei			

TOWER DESIGN NOTES

- 1. Tower is located in Litchfield County, Connecticut.
- Tower is located in Electrical County, Connecticut.
 Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
 Tower is also designed for a 28 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
 Deflections are based upon a 50 mph wind.
 TOWER RATING: 96.4%



FDH

FDH Engineering, Inc. 6521 Meridien Drive Raleigh, NC 27616 Phone: (919) 755-1012 FAX: (919) 755-1031

Job: O&G Woodbury	, BU# 876380	
Project: 1467431400		
Client: Crown Castle	Drawn by: Mark S. Girgis	App'd:
	Date: 06/12/14	Scale: NTS
Path:	IncCTR/SSS ONG Woodbury 146741400AnskristONG Woodbury anskristONG Woodbury. 87680.ex	Dwg No. E-

Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

- 1) Tower is located in Litchfield County, Connecticut.
- 2) Basic wind speed of 80 mph.
- 3) Nominal ice thickness of 1.0000 in.
- 4) Ice thickness is considered to increase with height.
- 5) Ice density of 56 pcf.
- 6) A wind speed of 28 mph is used in combination with ice.
- 7) Temperature drop of 50 °F.
- 8) Deflections calculated using a wind speed of 50 mph.
- 9) A non-linear (P-delta) analysis was used.
- 10) Pressures are calculated at each section.
- 11) Stress ratio used in pole design is 1.333.
- 12) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- √ Use Code Stress Ratios
- √ Use Code Safety Factors Guys
 - Escalate Ice
 Alw ays Use Max Kz
 Use Special Wind Profile
 Include Bolts In Member Capacity
 Leg Bolts Are At Top Of Section
 Secondary Horizontal Braces Leg
 Use Diamond Inner Bracing (4 Sided)
 Add IBC .6D+W Combination

Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt. Autocalc Torque ArmAreas SR Members Have Cut Ends
- √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Use TIA-222-G Tension Splice Capacity Exemption

Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allow able Offset Girt At Foundation

- √ Consider Feedline Torque Include Angle Block Shear Check Poles
- √ Include Shear-Torsion Interaction Alw ays Use Sub-Critical Flow Use Top Mounted Sockets

Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	fť	ft	Sides	in	in	in	in	
L1	138.50-108.50	30.00	0.00	18	17.3750	24.5000	0.1875	0.7500	A572-65 (65 ksi)
L2	108.50-83.75	24.75	4.50	18	24.5000	31.8800	0.2500	1.0000	À572-65 (65 ksi)
L3	83.75-43.00	45.25	6.00	18	30.0382	43.4200	0.3125	1.2500	A572-65 (65 ksi)
L4	43.00-0.00	49.00		18	41.0206	55.5000	0.3125	1.2500	À572-65 (65 ksi)

Tapered Pole Properties

Section	Tin Dia	1500	- 1	-		1/0	- 1	14/0	147	14/4
Jeculon	Lip Dia.	Area	1	- 1		1/C	J	IVQ	vv	VV/L
		. 2	. 1			. 3	. 1	. 2		
	in	in-	in⁴	in	in	in	in⁴	in-	in	
	111	111	111	111	111	111	111	111	111	

Section	Tip Dia.	Area	1	r	С	I/C	J	It/Q	W	w/t
	in	in²	in⁴	in	in	in³	in⁴	in²	in	
L1	17.6430	10.2287	381.7542	6.1016	8.8265	43.2509	764.0106	5.1153	2.7280	14.549
	24.8780	14.4690	1080.5242	8.6309	12.4460	86.8170	2162.4702	7.2359	3.9820	21.237
L2	24.8780	19.2424	1429.6167	8.6088	12.4460	114.8656	2861.1145	9.6230	3.8720	15.488
	32.3718	25.0984	3172.3563	11.2287	16.1950	195.8844	6348.8868	12.5516	5.1709	20.684
L3	31.8529	29.4842	3291.4698	10.5526	15.2594	215.7012	6587.2706	14.7449	4.7367	15.158
	44.0898	42.7573	10038.132	15.3032	22.0574	455.0922	20089.472	21.3827	7.0919	22.694
			1				6			
L4	43.4538	40.3774	8453.5159	14.4514	20.8385	405.6687	16918.155	20.1925	6.6696	21.343
							1			
	56.3562	54.7391	21062.822	19.5916	28.1940	747.0675	42153.359	27.3748	9.2180	29.498
			0				0			

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset GradeAdjust. Factor Ar	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft ²	in				in	in
L1 138.50-			1	1	1		
108.50							
L2 108.50-			1	1	1		
83.75							
L3 83.75-			1	1	1		
43.00							
L4 43.00-0.00			1	1	1		

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face Allow	Component	Placement	Total	Number	Clear	Width or	Perimete	Weight
	or Shield	Type		Number	Per Row	Spacing	Diamete	r	
	Leg		ft			in	r		plf
							in	in	

Feed Line/Linear Appurtenances - Entered As Area

Description	Face	Allow	Component	Placement	Total		C_AA_A	Weight
	or	Shield	Type		Number			
	Leg		• •	ft			f t² /ft	plf
Safety Line 3/8	С	No	CaAa (Out Of	138.50 - 0.00	1	No Ice	0.04	0.22
			Face)			1/2" lce	0.14	0.75
						1" lce	0.24	1.28
						2" lce	0.44	2.34
***						4" Ice	0.84	4.46
	^	NI-	la el de Dele	400.00 0.00	0	NI- I	0.00	0.00
LDF6-50A(1-1/4")	Α	No	Inside Pole	138.00 - 0.00	6	No Ice	0.00	0.66
						1/2" lce	0.00	0.66
						1" lce	0.00	0.66
						2" lce	0.00	0.66
	_				_	4" Ice	0.00	0.66
LCF114-50J(1-1/4")	Α	No	Inside Pole	138.00 - 0.00	6	No Ice	0.00	0.70
						1/2" lce	0.00	0.70
						1" Ice	0.00	0.70
						2" lce	0.00	0.70
	_					4" Ice	0.00	0.70
FB-L98B-002-75000(Α	No	Inside Pole	138.00 - 0.00	1	No Ice	0.00	0.06
3/8")						1/2" lce	0.00	0.06
						1" Ice	0.00	0.06
						2" lce	0.00	0.06
						4" lce	0.00	0.06
WR-VG122ST-	Α	No	Inside Pole	138.00 - 0.00	2	No Ice	0.00	0.14
BRDA (7/16)						1/2" lce	0.00	0.14
						1" lce	0.00	0.14
						2" lce	0.00	0.14

Description	Face or	Allow Shield	Component Type	Placement	Total Number		C_AA_A	Weight
	Leg		. 7/	ft			ft²/ft	plf
						4" lce	0.00	0.14
LDF4-50A(1/2")	Α	No	CaAa (Out Of	138.00 - 0.00	1	No Ice	0.00	0.15
			Face)			1/2" lce	0.00	0.84
						1" lce	0.00	2.14
						2" lce	0.00	6.58
	_				_	4" lce	0.00	22.78
LDF6-50A(1-1/4")	Α	No	CaAa (Out Of	138.00 - 0.00	2	No Ice	0.00	0.66
			Face)			1/2" lce	0.00	1.91
						1" lce	0.00	3.78
						2" lce 4" lce	0.00	9.33 27.78
***						4 ice	0.00	21.10
LDF4-50A(1/2")	Α	No	Inside Pole	137.00 - 0.00	1	No Ice	0.00	0.15
LDI 4-30A(1/2)	^	NO	IIISIUE FUIE	137.00 - 0.00	'	1/2" lce	0.00	0.15
						1" lce	0.00	0.15
						2" lce	0.00	0.15
						4" lce	0.00	0.15
***								• • • • • • • • • • • • • • • • • • • •
LDF7-50A(1-5/8")	Α	No	Inside Pole	129.00 - 0.00	17	No Ice	0.00	0.82
						1/2" lce	0.00	0.82
						1" lce	0.00	0.82
						2" lce	0.00	0.82
						4" lce	0.00	0.82
LDF7-50A(1-5/8")	Α	No	CaAa (Out Of	129.00 - 0.00	1	No Ice	0.20	0.82
			Face)			1/2" lce	0.30	2.33
						1" lce	0.40	4.46
						2" lce	0.60	10.54
***						4" lce	1.00	30.04
LDF7-50A(1-5/8")	С	No	Inside Pole	119.00 - 0.00	12	No Ice	0.00	0.82
LDI 7-30A(1-3/8)	C	NO	IIISIUE FUIE	119.00 - 0.00	12	1/2" lce	0.00	0.82
						1" lce	0.00	0.82
						2" lce	0.00	0.82
						4" lce	0.00	0.82

HB114-1-0813U4-M5J(В	No	Inside Pole	104.00 - 0.00	3	No Ice	0.00	1.20
1 1/4")						1/2" lce	0.00	1.20
ŕ						1" lce	0.00	1.20
						2" lce	0.00	1.20
						4" lce	0.00	1.20
HB114-1-0813U4-M5J(В	No	CaAa (Out Of	104.00 - 0.00	1	No Ice	0.15	1.20
1 1/4")			Face)			1/2" lce	0.25	2.45
						1" lce	0.35	4.30
						2" lce	0.55	9.85
***						4" lce	0.95	28.27
LDF7-50A(1-5/8")	В	No	Incido Polo	87.00 - 0.00	12	No Ice	0.00	0.92
LDF7-50A(1-5/6)	Ь	No	Inside Pole	67.00 - 0.00	12	1/2" lce	0.00 0.00	0.82 0.82
						1" lce	0.00	0.82
						2" lce	0.00	0.82
						4" lce	0.00	0.82
***						50	0.00	3.02
LDF4-50A(1/2")	В	No	Inside Pole	70.00 - 0.00	1	No Ice	0.00	0.15
` '						1/2" lce	0.00	0.15
						1" lce	0.00	0.15
						2" lce	0.00	0.15
						4" lce	0.00	0.15

Feed Line/Linear Appurtenances Section Areas

Tower Sectio	Tower Elevation	Face	A_R	A_F	C _A A _A In Face	C _A A _A Out Face	Weight
n	ft		ft ²	ft ²	ft ²	f t²	K
L1	138.50-108.50	Α	0.000	0.000	0.000	4.059	0.60
		В	0.000	0.000	0.000	0.000	0.00

Tower Sectio	Tower Elevation	Face	A_R	A_F	C _A A _A In Face	C _A A _A Out Face	Weight
n	ft		ft²	ft ²	ft ²	ft ²	K
		С	0.000	0.000	0.000	1.125	0.11
L2	108.50-83.75	Α	0.000	0.000	0.000	4.901	0.62
		В	0.000	0.000	0.000	3.118	0.13
		С	0.000	0.000	0.000	0.928	0.25
L3	83.75-43.00	Α	0.000	0.000	0.000	8.069	1.01
		В	0.000	0.000	0.000	6.275	0.60
		С	0.000	0.000	0.000	1.528	0.41
L4	43.00-0.00	Α	0.000	0.000	0.000	8.514	1.07
		В	0.000	0.000	0.000	6.622	0.64
		С	0.000	0.000	0.000	1.613	0.43

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Sectio	Elevation	or	Thickness			In Face	Out Face	
n	ft	Leg	in	ft ²	ft ²	ft ²	f t²	K
L1	138.50-108.50	Α	1.171	0.000	0.000	0.000	8.859	1.02
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	8.149	0.15
L2	108.50-83.75	Α	1.136	0.000	0.000	0.000	10.524	0.98
		В		0.000	0.000	0.000	7.720	0.21
		С		0.000	0.000	0.000	6.552	0.28
L3	83.75-43.00	Α	1.080	0.000	0.000	0.000	17.328	1.62
		В		0.000	0.000	0.000	15.535	0.76
		С		0.000	0.000	0.000	10.788	0.46
L4	43.00-0.00	Α	1.000	0.000	0.000	0.000	17.804	1.65
		В		0.000	0.000	0.000	15.911	0.79
		С		0.000	0.000	0.000	10.902	0.48

Feed Line Center of Pressure

Section	Elevation	CP _X	CPz	CP _X	CPz
				lce	lce
	ft	in	in	in	in
L1	138.50-108.50	-0.0443	-0.1690	-0.2456	-0.1831
L2	108.50-83.75	0.1030	-0.1492	0.0471	-0.1326
L3	83.75-43.00	0.1345	-0.1363	0.1077	-0.1092
L4	43.00-0.00	0.1382	-0.1401	0.1167	-0.1182

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
	J		Vert ft ft ft	٥	ft		ft ²	ft ²	Κ
(2) RRUS-11	A	From Leg	1.00 0.00 1.00	0.0000	139.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	2.94 3.17 3.41 3.91 5.02	1.25 1.41 1.59 1.96 2.82	0.06 0.07 0.10 0.15 0.30
(2) RRUS-11	В	From Leg	1.00 0.00 1.00	0.0000	139.00	No Ice 1/2" Ice 1" Ice	2.94 3.17 3.41 3.91	1.25 1.41 1.59 1.96	0.06 0.07 0.10 0.15

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft ft ft	٥	ft		ft ²	ft ²	K
						2" lce	5.02	2.82	0.30
(2) RRUS-11	С	From Leg	1.00	0.0000	139.00	4" Ice No Ice	2.94	1.25	0.06
(2) 14 (33 11	Ü	110111 Log	0.00	0.0000	100.00	1/2"	3.17	1.41	0.07
			1.00			lce	3.41	1.59	0.10
						1" lce 2" lce 4" lce	3.91 5.02	1.96 2.82	0.15 0.30
Side Arm Mount [SO 102-	С	None		0.0000	139.00	No Ice	3.00	3.00	0.08
3]	O	None		0.0000	100.00	1/2"	3.48	3.48	0.11
•						lce	3.96	3.96	0.14
						1" lce	4.92	4.92	0.20
***						2" lce 4" lce	6.84	6.84	0.32
TTA-429-83H-08179	Α	From Leg	1.00	0.0000	138.00	No Ice	1.05	1.05	0.02
			0.00			1/2"	1.21	1.21	0.03
			0.00			lce	1.38	1.38	0.04
						1" lce	1.74	1.74	0.07
						2" lce 4" lce	2.57	2.57	0.16
DS9A09F36D-N	Α	From Leg	1.00	0.0000	138.00	No Ice	5.76	5.76	0.05
200,100,002,11		09	0.00	0.000	.00.00	1/2"	7.71	7.71	0.09
			10.00			lce	9.68	9.68	0.14
						1" Ice	13.67	13.67	0.29
						2" lce 4" lce	20.51	20.51	0.73
Side Arm Mount [SO 309-	Α	From Leg	0.50	0.0000	138.00	No Ice	2.82	2.20	0.04
1]		09	0.00	0.000	.00.00	1/2"	4.07	3.16	0.06
-			0.00			lce	5.32	4.12	0.08
						1" lce	7.82	6.04	0.13
***						2" lce 4" lce	12.82	9.88	0.22
(2) LGP21401	Α	From Leg	4.00	0.0000	138.00	No Ice	0.00	0.23	0.01
,		· ·	0.00			1/2"	0.00	0.31	0.02
			0.00			lce	0.00	0.40	0.03
						1" lce 2" lce	0.00 0.00	0.61 1.12	0.05
						4" lce	0.00	1.12	0.14
(2) LGP21401	В	From Leg	4.00	0.0000	138.00	No Ice	0.00	0.23	0.01
			0.00			1/2"	0.00	0.31	0.02
			0.00			lce	0.00	0.40	0.03
						1" lce 2" lce	0.00	0.61 1.12	0.05 0.14
						4" lce	0.00		0.11
(2) LGP21401	С	From Leg	4.00	0.0000	138.00	No Ice	0.00	0.23	0.01
			0.00			1/2"	0.00	0.31	0.02
			0.00			lce	0.00	0.40	0.03
						1" lce 2" lce	0.00 0.00	0.61 1.12	0.05 0.14
						4" lce	0.00	1.12	0.14
(2) LGP2140X	Α	From Leg	4.00	0.0000	138.00	No Ice	0.00	0.38	0.01
			0.00			1/2"	0.00	0.49	0.02
			1.00			lce 1" lce	0.00 0.00	0.62 0.89	0.03 0.05
						2" lce 4" lce	0.00	1.54	0.03
(2) LGP2140X	В	From Leg	4.00	0.0000	138.00	No Ice	0.00	0.38	0.01
• •		J	0.00			1/2"	0.00	0.49	0.02
			1.00			lce	0.00	0.62	0.03
						1" lce 2" lce	0.00	0.89	0.05
						4" Ice	0.00	1.54	0.13
(2) LGP2140X	С	From Leg	4.00	0.0000	138.00	No Ice	0.00	0.38	0.01

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	o	ft		ft ²	ft ²	K
			0.00			1/2"	0.00	0.49	0.02
			1.00			lce	0.00	0.62	0.03
						1" lce	0.00	0.89	0.05
						2" lce 4" lce	0.00	1.54	0.13
(2) LGP21901	Α	From Leg	4.00	0.0000	138.00	No Ice	0.00	0.18	0.01
(=) ======			6.00			1/2"	0.00	0.25	0.01
			0.00			lce	0.00	0.32	0.01
						1" lce	0.00	0.49	0.02
						2" lce	0.00	0.94	0.07
(2) CD24004	ь	<u>Гиото I от</u>	4.00	0.0000	420.00	4" Ice	0.00	0.40	0.04
(2) LGP21901	В	From Leg	4.00 6.00	0.0000	138.00	No Ice 1/2"	0.00 0.00	0.18 0.25	0.01
			0.00			lce	0.00	0.25	0.01 0.01
			0.00			1" Ice	0.00	0.49	0.02
						2" lce	0.00	0.94	0.07
						4" Ice			
(2) LGP21901	С	From Leg	4.00	0.0000	138.00	No Ice	0.00	0.18	0.01
			6.00			1/2"	0.00	0.25	0.01
			0.00			lce 1" lce	0.00 0.00	0.32 0.49	0.01 0.02
						2" lce	0.00	0.49	0.02
						4" lce	0.00	0.04	0.07
(2) XDUO1416-80 w/	Α	From Leg	4.00	0.0000	138.00	No Ice	6.73	4.09	0.04
Mount Pipe		_	0.00			1/2"	7.21	4.74	0.09
			1.00			lce	7.69	5.38	0.15
						1" lce	8.69	6.80	0.28
						2" lce 4" lce	10.82	9.91	0.66
(2) XDUO1416-80 w/	В	From Leg	4.00	0.0000	138.00	No Ice	6.73	4.09	0.04
Mount Pipe			0.00			1/2"	7.21	4.74	0.09
			1.00			lce	7.69	5.38	0.15
						1" lce	8.69	6.80	0.28
						2" lce	10.82	9.91	0.66
(2) XDUO1416-80 w/	С	From Leg	4.00	0.0000	138.00	4" Ice No Ice	6.73	4.09	0.04
Mount Pipe	O	110III Log	0.00	0.0000	130.00	1/2"	7.21	4.74	0.09
			1.00			lce	7.69	5.38	0.15
						1" lce	8.69	6.80	0.28
						2" lce	10.82	9.91	0.66
AM-X-CD-16-65-00T-RET	۸	From Leg	4.00	0.0000	138.00	4" Ice No Ice	8.50	6.30	0.07
w / Mount Pipe	Α	110III Leg	0.00	0.0000	130.00	1/2"	9.15	7.48	0.07
w/ wount ripe			1.00			lce	9.77	8.37	0.14
						1" lce	11.03	10.18	0.38
						2" Ice	13.68	14.02	0.87
000 40704 ··· / Maxima Din a	ь.	<u>Гиото I от</u>	4.00	0.0000	120.00	4" Ice	0.00	4.00	0.00
800 10764 w / Mount Pipe	В	From Leg	4.00 0.00	0.0000	138.00	No Ice 1/2"	6.20 6.69	4.29 4.99	0.06 0.11
			1.00			lce	7.18	5.66	0.17
			1.00			1" lce	8.19	7.10	0.30
						2" lce	10.33	10.30	0.67
						4" Ice			
AM-X-CD-14-65-00T-RET	С	From Leg	4.00	0.0000	138.00	No Ice	5.74	4.02	0.03
w / Mount Pipe			0.00 1.00			1/2" Ice	6.20 6.66	4.63 5.28	0.08 0.13
			1.00			1" lce	7.62	6.68	0.13
						2" lce	9.67	9.74	0.23
						4" Ice			
DC6-48-60-18-8F	В	From Leg	4.00	0.0000	138.00	No Ice	2.57	4.32	0.03
			0.00			1/2"	2.80	4.60	0.06
			1.00			lce 1" lce	3.04 3.54	4.88 5.49	0.10 0.18
						2" lce	3.54 4.66	6.80	0.18
						4" lce		0.00	5.10

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
	J		Vert		_		- 2	- 2	
			ft ft ft	o	ft		ft ²	ft ²	K
Platform Mount [LP 303-1]	С	None	-	0.0000	138.00	No Ice	14.66	14.66	1.25
						1/2" lce	18.87 23.08	18.87 23.08	1.48 1.71
						1" lce	31.50	31.50	2.18
						2" lce	48.34	48.34	3.10
***						4" lce			
ANT150F6	Α	From Leg	1.00	0.0000	137.00	No Ice	4.80	4.80	0.03
			0.00			1/2"	6.83	6.83	0.07
			10.00			lce 1" lce	8.87 13.01	8.87 13.01	0.11 0.25
						2" lce	21.03	21.03	0.68
		_				4" Ice			
5' x 2' Pipe Mount	Α	From Leg	0.50 0.00	0.0000	137.00	No Ice 1/2"	1.00 1.39	1.00 1.39	0.03 0.04
			0.00			lce	1.70	1.70	0.05
						1" lce	2.35	2.35	0.08
						2" lce 4" lce	3.78	3.78	0.20
***						4 100			
(2) LPA-80080/6CF w/	Α	From Leg	4.00	0.0000	129.00	No Ice	4.56	10.73	0.05
Mount Pipe			0.00 0.00			1/2" Ice	5.11 5.61	11.99 12.97	0.11 0.19
			0.00			1" lce	6.65	14.98	0.36
						2" lce	8.83	19.22	0.86
(2) LPA-80080/6CF w/	В	From Leg	4.00	0.0000	129.00	4" lce No lce	4.56	10.73	0.05
Mount Pipe		110111 209	0.00	0.0000	120.00	1/2"	5.11	11.99	0.11
·			0.00			lce	5.61	12.97	0.19
						1" lce 2" lce	6.65 8.83	14.98 19.22	0.36 0.86
						4" lce	0.00	10.22	0.00
(2) LPA-80080/6CF w /	С	From Leg	4.00	0.0000	129.00	No Ice	4.56	10.73	0.05
Mount Pipe			0.00 0.00			1/2" Ice	5.11 5.61	11.99 12.97	0.11 0.19
			0.00			1" lce	6.65	14.98	0.36
						2" lce 4" lce	8.83	19.22	0.86
BXA-171085-8BF-EDIN-2	Α	From Leg	4.00	0.0000	129.00	No Ice	3.18	3.35	0.03
w / Mount Pipe		=09	0.00	0.000	0.00	1/2"	3.56	3.97	0.06
			0.00			lce 1" lce	3.96 4.85	4.60 5.89	0.10 0.19
						2" lce	6.77	8.89	0.19
	_	_				4" lce			
BXA-171085-8BF-EDIN-2 w/Mount Pipe	В	From Leg	4.00 0.00	0.0000	129.00	No Ice 1/2"	3.18 3.56	3.35 3.97	0.03 0.06
w/ wount ripe			0.00			lce	3.96	4.60	0.00
						1" lce	4.85	5.89	0.19
						2" lce 4" lce	6.77	8.89	0.49
BXA-171085-8BF-EDIN-2	С	From Leg	4.00	0.0000	129.00	No Ice	3.18	3.35	0.03
w / Mount Pipe			0.00			1/2"	3.56	3.97	0.06
			0.00			lce 1" lce	3.96	4.60	0.10 0.19
						2" lce	4.85 6.77	5.89 8.89	0.19
DVA 70000/005 0		E	4.00	0.0000	400.00	4" lce			
BXA-70063/6CF-2 W/Mount Pipe	Α	From Leg	4.00 0.00	0.0000	129.00	No Ice 1/2"	7.75 8.29	5.18 6.11	0.04 0.10
vv/iviburit ripe			0.00			lce	8.85	6.92	0.10
			-			1" lce	9.97	8.59	0.31
						2" lce 4" lce	12.34	12.13	0.75
BXA-70063/6CF-2	В	From Leg	4.00	0.0000	129.00	No Ice	7.75	5.18	0.04
W/Mount Pipe		3	0.00			1/2"	8.29	6.11	0.10
			0.00			lce	8.85	6.92	0.16

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	0	ft		ft ²	ft ²	K
						1" lce 2" lce 4" lce	9.97 12.34	8.59 12.13	0.31 0.75
BXA-70063/6CF-2 W/Mount Pipe	С	From Leg	4.00 0.00 0.00	0.0000	129.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.75 8.29 8.85 9.97 12.34	5.18 6.11 6.92 8.59 12.13	0.04 0.10 0.16 0.31 0.75
Platform Mount [LP 304-1]	С	None		0.0000	129.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	17.46 22.44 27.42 37.38 57.30	17.46 22.44 27.42 37.38 57.30	1.35 1.62 1.90 2.45 3.55
(4) DB846G90A-XY w/ Mount Pipe	Α	From Leg	4.00 0.00 0.00	0.0000	119.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	5.23 5.78 6.30 7.37 9.69	7.53 8.72 9.62 11.45 15.60	0.04 0.10 0.16 0.32 0.77
(4) DB846G90A-XY w/ Mount Pipe	В	From Leg	4.00 0.00 0.00	0.0000	119.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	5.23 5.78 6.30 7.37 9.69	7.53 8.72 9.62 11.45 15.60	0.04 0.10 0.16 0.32 0.77
(4) DB846G90A-XY w/ Mount Pipe	С	From Leg	4.00 0.00 0.00	0.0000	119.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	5.23 5.78 6.30 7.37 9.69	7.53 8.72 9.62 11.45 15.60	0.04 0.10 0.16 0.32 0.77
Platform Mount [LP 304-1]	С	None		0.0000	119.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	17.46 22.44 27.42 37.38 57.30	17.46 22.44 27.42 37.38 57.30	1.35 1.62 1.90 2.45 3.55
*** 1900MHz RRH (65MHz)	Α	From Leg	1.00 0.00 0.00	0.0000	105.00	No Ice 1/2" Ice 1" Ice 2" Ice	2.70 2.94 3.18 3.70 4.85	2.77 3.01 3.26 3.78 4.93	0.06 0.08 0.11 0.18 0.35
1900MHz RRH (65MHz)	В	From Leg	1.00 0.00 0.00	0.0000	105.00	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	2.70 2.94 3.18 3.70 4.85	2.77 3.01 3.26 3.78 4.93	0.06 0.08 0.11 0.18 0.35
1900MHz RRH (65MHz)	С	From Leg	1.00 0.00 0.00	0.0000	105.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	2.70 2.94 3.18 3.70 4.85	2.77 3.01 3.26 3.78 4.93	0.06 0.08 0.11 0.18 0.35
Side Arm Mount [SO 102- 3]	С	None		0.0000	105.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.00 3.48 3.96 4.92 6.84	3.00 3.48 3.96 4.92 6.84	0.08 0.11 0.14 0.20 0.32

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	0	ft		ft ²	ft ²	K
***			4.00		40400		- 40	4.00	
APXVTM14-C-120 w / Mount Pipe	Α	From Leg	4.00 0.00	0.0000	104.00	No Ice 1/2"	7.13 7.66	4.96 5.75	0.08 0.13
Would Tipe			4.00			lce	8.18	6.47	0.19
						1" Ice	9.26	8.01	0.34
						2" lce	11.53	11.41	0.75
APXVTM14-C-120 w/	В	From Log	4.00	0.0000	104.00	4" lce No lce	7.13	4.96	0.08
Mount Pipe	Ь	From Leg	0.00	0.0000	104.00	1/2"	7.13 7.66	4.90 5.75	0.08
Would Tipe			4.00			lce	8.18	6.47	0.19
						1" Ice	9.26	8.01	0.34
						2" lce	11.53	11.41	0.75
APXVTM14-C-120 w/	С	From Leg	4.00	0.0000	104.00	4" Ice No Ice	7.13	4.96	0.08
Mount Pipe	O	110III Log	0.00	0.0000	104.00	1/2"	7.16	5.75	0.13
, , , , , , , , , , , , , , , , , , ,			4.00			lce	8.18	6.47	0.19
						1" Ice	9.26	8.01	0.34
						2" lce 4" lce	11.53	11.41	0.75
(3) ACU-A20-N	Α	From Leg	4.00	0.0000	104.00	No Ice	0.08	0.14	0.00
(5) 1155 1155 11			0.00			1/2"	0.12	0.19	0.00
			0.00			lce	0.17	0.25	0.00
						1" lce 2" lce	0.30 0.67	0.40	0.01 0.04
						4" lce	0.67	0.80	0.04
APXVSPP18-C-A20 w/	Α	From Leg	4.00	0.0000	104.00	No Ice	8.50	6.95	0.08
Mount Pipe			0.00			1/2"	9.15	8.13	0.15
			4.00			lce	9.77	9.02	0.23
						1" lce 2" lce	11.03 13.68	10.84 14.85	0.41 0.91
						4" lce	10.00	1 1.00	0.01
(3) ACU-A20-N	В	From Leg	4.00	0.0000	104.00	No Ice	0.08	0.14	0.00
			0.00 0.00			1/2"	0.12 0.17	0.19 0.25	0.00 0.00
			0.00			lce 1" lce	0.17	0.25	0.00
						2" lce	0.67	0.80	0.04
	_					4" Ice			
APXVSPP18-C-A20 w/	В	From Leg	4.00 0.00	0.0000	104.00	No Ice 1/2"	8.50 9.15	6.95 8.13	0.08 0.15
Mount Pipe			4.00			lce	9.15	9.02	0.15
			1.00			1" lce	11.03	10.84	0.41
						2" Ice	13.68	14.85	0.91
(3) ACU-A20-N	С	From Log	4.00	0.0000	104.00	4" Ice No Ice	0.08	0.14	0.00
(3) ACO-AZO-N	C	From Leg	0.00	0.0000	104.00	1/2"	0.08	0.14	0.00
			0.00			lce	0.17	0.25	0.00
						1" lce	0.30	0.40	0.01
						2" lce 4" lce	0.67	0.80	0.04
APXVSPP18-C-A20 w/	С	From Leg	4.00	0.0000	104.00	No Ice	8.50	6.95	0.08
Mount Pipe		_	0.00			1/2"	9.15	8.13	0.15
			4.00			lce	9.77	9.02	0.23
						1" lce 2" lce	11.03 13.68	10.84 14.85	0.41 0.91
						4" lce	10.00	14.00	0.01
TD-RRH8x20-25	Α	From Leg	4.00	0.0000	104.00	No Ice	4.72	1.70	0.07
			0.00			1/2"	5.01	1.92	0.10
			4.00			lce 1" lce	5.32 5.95	2.14 2.62	0.13 0.20
						2" lce	7.31	3.68	0.40
		_				4" Ice			
TD-RRH8x20-25	В	From Leg	4.00	0.0000	104.00	No Ice	4.72	1.70	0.07
			0.00 4.00			1/2" Ice	5.01 5.32	1.92 2.14	0.10 0.13
						1" lce	5.95	2.62	0.20

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	o	ft		ft ²	ft ²	K
						2" lce 4" lce	7.31	3.68	0.40
TD-RRH8x20-25	С	From Leg	4.00	0.0000	104.00	No Ice	4.72	1.70	0.07
12 14 4 16/120 20		09	0.00	0.000		1/2"	5.01	1.92	0.10
			4.00			lce	5.32	2.14	0.13
						1" lce	5.95	2.62	0.20
						2" lce 4" lce	7.31	3.68	0.40
800 EXTERNAL NOTCH	Α	From Leg	4.00	0.0000	104.00	No Ice	0.77	0.37	0.01
FILTER	,,	110m 20g	0.00	0.0000	101.00	1/2"	0.89	0.46	0.02
			0.00			lce	1.02	0.56	0.02
						1" Ice	1.30	0.79	0.04
						2" lce	1.97	1.34	0.11
800MHZ RRH	Α	From Leg	1.00	0.0000	104.00	4" Ice No Ice	2.49	2.07	0.05
SOOIVII LE IXIXII	^	110III Leg	0.00	0.0000	104.00	1/2"	2.49	2.27	0.03
			0.00			lce	2.93	2.48	0.10
						1" Ice	3.41	2.93	0.16
						2" lce	4.46	3.93	0.32
OOO EVEEDNAL NOTOLL	Б.		4.00	0.0000	404.00	4" lce	0.77	0.07	0.04
800 EXTERNAL NOTCH FILTER	В	From Leg	4.00 0.00	0.0000	104.00	No Ice 1/2"	0.77 0.89	0.37 0.46	0.01 0.02
FILTER			0.00			lce	1.02	0.46	0.02
			0.00			1" lce	1.30	0.79	0.04
						2" lce	1.97	1.34	0.11
						4" Ice			
800MHZ RRH	В	From Leg	1.00	0.0000	104.00	No Ice	2.49	2.07	0.05
			0.00			1/2"	2.71	2.27	0.07 0.10
			0.00			lce 1" lce	2.93 3.41	2.48 2.93	0.10
						2" lce	4.46	3.93	0.32
						4" lce			
800 EXTERNAL NOTCH	С	From Leg	4.00	0.0000	104.00	No Ice	0.77	0.37	0.01
FILTER			0.00			1/2"	0.89	0.46	0.02
			0.00			lce 1" lce	1.02 1.30	0.56 0.79	0.02 0.04
						2" lce	1.97	1.34	0.04
						4" lce			0
800MHZ RRH	С	From Leg	1.00	0.0000	104.00	No Ice	2.49	2.07	0.05
			0.00			1/2"	2.71	2.27	0.07
			0.00			lce 1" lce	2.93 3.41	2.48 2.93	0.10 0.16
						2" lce	4.46	3.93	0.10
						4" lce	7.70	0.00	0.02
Platform Mount [LP 712-1]	С	None		0.0000	104.00	No Ice	24.53	24.53	1.34
						1/2"	29.94	29.94	1.65
						lce	35.35	35.35	1.96
						1" lce 2" lce	46.17 67.81	46.17 67.81	2.58 3.82
						4" lce	07.01	07.01	3.02
Miscellaneous [NA 507-1]	С	None		0.0000	104.00	No Ice	4.80	4.80	0.25
-						1/2"	6.70	6.70	0.29
						lce	8.60	8.60	0.34
						1" lce 2" lce	12.40 20.00	12.40 20.00	0.44 0.64
						4" lce	20.00	20.00	0.04

(4) KN-1870-15-4803	Α	From Leg	4.00	0.0000	87.00	No Ice	0.83	0.39	0.01
			0.00 0.00			1/2" Ice	0.97 1.11	0.50	0.02 0.02
			0.00			1" lce	1.11	0.62 0.89	0.02
						2" lce	2.14	1.52	0.11
						4" Ice			
(4) KN-1870-15-4803	В	From Leg	4.00	0.0000	87.00	No Ice	0.83	0.39	0.01
			0.00			1/2"	0.97	0.50	0.02

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	0	ft		ft ²	ft ²	K
			0.00			lce	1.11	0.62	0.02
						1" lce 2" lce 4" lce	1.42 2.14	0.89 1.52	0.04 0.11
(4) KN-1870-15-4803	С	From Leg	4.00	0.0000	87.00	No Ice	0.83	0.39	0.01
			0.00			1/2"	0.97	0.50	0.02
			0.00			lce 1" lce	1.11 1.42	0.62 0.89	0.02 0.04
						2" lce 4" lce	2.14	1.52	0.11
(2) APXV18-209014-C w/	Α	From Leg	4.00	0.0000	87.00	No Ice	3.72	3.31	0.04
Mount Pipe			0.00			1/2"	4.13	4.02	0.07
			0.00			lce 1" lce	4.56 5.51	4.68 6.07	0.11 0.21
						2" lce 4" lce	7.55	9.05	0.52
(2) APXV18-209014-C w/	В	From Leg	4.00	0.0000	87.00	No Ice	3.72	3.31	0.04
Mount Pipe			0.00 0.00			1/2" Ice	4.13 4.56	4.02 4.68	0.07 0.11
			0.00			1" lce	5.51	6.07	0.11
						2" lce 4" lce	7.55	9.05	0.52
(2) APXV18-209014-C w/	С	From Leg	4.00	0.0000	87.00	No Ice	3.72	3.31	0.04
Mount Pipe			0.00			1/2"	4.13	4.02	0.07
			0.00			lce 1" lce	4.56 5.51	4.68 6.07	0.11 0.21
						2" lce 4" lce	7.55	9.05	0.52
Empty Pipe Mount	Α	From Leg	4.00	0.0000	87.00	No Ice	1.43	1.43	0.02
			0.00 0.00			1/2" Ice	1.50 1.57	1.50 1.57	0.03 0.04
			0.00			1" lce	1.71	1.71	0.04
						2" lce 4" lce	2.00	2.00	0.07
Empty Pipe Mount	В	From Leg	4.00	0.0000	87.00	No Ice	1.43	1.43	0.02
			0.00			1/2"	1.50	1.50	0.03
			0.00			lce 1" lce	1.57 1.71	1.57 1.71	0.04 0.05
						2" lce 4" lce	2.00	2.00	0.07
Empty Pipe Mount	С	From Leg	4.00	0.0000	87.00	No Ice	1.43	1.43	0.02
			0.00			1/2"	1.50	1.50	0.03
			0.00			lce 1" lce	1.57 1.71	1.57 1.71	0.04 0.05
						2" lce 4" lce	2.00	2.00	0.07
Platform Mount [LP 305-1]	С	None		0.0000	87.00	No Ice	18.01	18.01	1.12
						1/2"	23.33	23.33	1.35
						lce	28.65	28.65	1.58
						1" lce 2" lce 4" lce	39.29 60.57	39.29 60.57	2.05 2.97

KS24019-L112A	Α	From Leg	3.00	0.0000	70.00	No Ice	0.16	0.16	0.01
			0.00			1/2"	0.22	0.22	0.01
			1.00			lce 1" lce	0.30 0.48	0.30 0.48	0.01 0.02
						2" lce	0.95	0.95	0.06
						4" lce			
Side Arm Mount [SO 701-	Α	From Leg	1.50	0.0000	70.00	No Ice	0.85	1.67	0.07
1]			0.00			1/2"	1.14	2.34	0.08
			0.00			lce 1" lce	1.43 2.01	3.01 4.35	0.09 0.12
						2" lce	3.17	7.03	0.12
						4" lce			

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement	C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	o	ft	ft ²	f ť	K

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice
14	Dead+lce+Temp
15	Dead+Wind 0 deg+lce+Temp
16	Dead+Wind 30 deg+lce+Temp
17	Dead+Wind 60 deg+lce+Temp
18	Dead+Wind 90 deg+lce+Temp
19	Dead+Wind 120 deg+lce+Temp
20	Dead+Wind 150 deg+lce+Temp
21	Dead+Wind 180 deg+lce+Temp
22	Dead+Wind 210 deg+lce+Temp
23	Dead+Wind 240 deg+lce+Temp
24	Dead+Wind 270 deg+lce+Temp
25	Dead+Wind 300 deg+lce+Temp
26	Dead+Wind 330 deg+lce+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35 36	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37 38	Dead+Wind 300 deg - Service
<u> </u>	Dead+Wind 330 deg - Service

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	۰
L1	138.5 - 108.5	37.345	28	2.6086	0.0121
L2	108.5 - 83.75	22.064	28	2.1129	0.0040
L3	88.25 - 43	14.157	28	1.6030	0.0021
L4	49 - 0	4.174	28	0.8042	0.0008

Critical Deflections and Radius of Curvature - Service Wind

Elevation	evation Appurtenance		Gov. Deflection Load		Twist	Radius of Curvature	
ft		Comb.	in	٥	0	ft	
139.00	(2) RRUS-11	28	37.345	2.6086	0.0122	11562	
138.00	TTA-429-83H-08179	28	37.075	2.6018	0.0120	11562	
137.00	ANT150F6	28	36.534	2.5882	0.0117	11562	
129.00	(2) LPA-80080/6CF w / Mount Pipe	28	32.239	2.4765	0.0091	6085	
119.00	(4) DB846G90A-XY w / Mount Pipe	28	27.061	2.3191	0.0063	2963	
105.00	1900MHz RRH (65MHz)	28	20.538	2.0315	0.0035	2042	
104.00	APXVTM14-C-120 w / Mount Pipe	28	20.115	2.0071	0.0034	2075	
87.00	(4) KN-1870-15-4803	28	13.734	1.5720	0.0021	2810	
70.00	` KS24019-L112A	28	8.651	1.1942	0.0013	2673	

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	138.5 - 108.5	95.123	2	6.6426	0.0305
L2	108.5 - 83.75	56.262	2	5.3884	0.0100
L3	88.25 - 43	36.121	3	4.0907	0.0053
L4	49 - 0	10.659	3	2.0538	0.0019

Critical Deflections and Radius of Curvature - Design Wind

Elevation	on Appurtenance		Gov. Deflection Load		Twist	Radius of Curvature	
ft		Comb.	in	0	0	ft	
139.00	(2) RRUS-11	2	95.123	6.6426	0.0311	4652	
138.00	TTA-429-83H-08179	2	94.436	6.6254	0.0307	4652	
137.00	ANT150F6	2	93.062	6.5910	0.0299	4652	
129.00	(2) LPA-80080/6CF w / Mount Pipe	2	82.143	6.3093	0.0234	2447	
119.00	(4) DB846G90A-XY w/Mount Pipe	2	68.975	5.9115	0.0160	1190	
105.00	1900MHz RRH (65MHz)	2	52.376	5.1814	0.0089	816	
104.00	APXVTM14-C-120 w / Mount Pipe	2	51.299	5.1196	0.0085	829	
87.00	(4) KN-1870-15-4803	3	35.043	4.0118	0.0052	1112	
70.00	KS24019-L112A	3	22.083	3.0487	0.0034	1053	

Compression Checks

Pole Design Data										
Section No.	Elevation	Size	L	Lu	KI/r	Fa	Α	Actual P	Allow. Pa	Ratio P
	ft		ft	ft		ksi	in²	K	K	Pa
L1	138.5 - 108.5 (1)	TP24.5x17.375x0.1875	30.00	0.00	0.0	39.000	14.4690	-6.80	564.29	0.012

Section	Elevation	Size	L	Lu	KI/r	Fa	Α	Actual	Allow.	Ratio
No.								P	P_a	P
	ft		ft	ft		ksi	in ²	K	K	Pa
L2	108.5 - 83.75 (2)	TP31.88x24.5x0.25	24.75	0.00	0.0	39.000	24.0337	-11.75	937.31	0.013
L3	83.75 - 43 (3)	TP43.42x30.0382x0.3125	45.25	0.00	0.0	39.000	40.9973	-20.85	1598.89	0.013
L4	43 - 0 (4)	TP55.5x41.0206x0.3125	49.00	0.00	0.0	36.030	54.7391	-32.94	1972.25	0.017

	Pole Bending Design Data									
Section No.	Elevation	Size	Actual M _x	Actual f _{bx}	Allow. F _{bx}	Ratio f _{bx}	Actual M _y	Actual f _{by}	Allow. F _{by}	Ratio f _{by}
	ft		kip-ft	ksi	ksi	F_{bx}	kip-ft	ksi	ksi	F _{by}
L1	138.5 - 108.5 (1)	TP24.5x17.375x0.1875	288.76	39.913	39.000	1.023	0.00	0.000	39.000	0.000
L2	108.5 - 83.75 (2)	TP31.88x24.5x0.25	661.17	44.187	39.000	1.133	0.00	0.000	39.000	0.000
L3	83.75 - 43 (3)	TP43.42x30.0382x0.3125	1562.4 7	44.827	39.000	1.149	0.00	0.000	39.000	0.000
L4	43 - 0 (4)	TP55.5x41.0206x0.3125	2844.3 5	45.688	36.030	1.268	0.00	0.000	36.030	0.000

		Pol	e She	ar De	esign	Data	1			
Section No.	Elevation ft	Size	Actual V K	Actual f _v ksi	Allow. F _v ksi	Ratio f _v	Actual T kip-ft	Actual f _{vt} ksi	Allow. F _{vt} ksi	Ratio f _{vt}
L1	138.5 - 108.5	TP24.5x17.375x0.1875	14.10	0.975	26.000	-F _ν 0.075	0.15	0.010	26.000	$\overline{F_{vt}}$ 0.000
12	(1) 108.5 - 83.75	TP31.88x24.5x0.25	19.59	0.815	26.000	0.063	0.15	0.005	26.000	0.000
 L3	(2) 83.75 - 43 (3)	TP43.42x30.0382x0.3125	24.47	0.597	26.000	0.046	0.06	0.001	26.000	0.000
L3 L4	43 - 0 (4)	TP55.5x41.0206x0.3125	27.86	0.509	26.000	0.046	0.08	0.001	26.000	0.000

			Pol	e Inter	action	Desig	n Data		
Section No.	Elevation	Ratio P	Ratio f _{bx}	Ratio f _{by}	Ratio f _v	Ratio f _{vt}	Comb. Stress	Allow. Stress	Criteria
	ft	Pa	F _{bx}	F _{by}	F _v	F_{vt}	Ratio	Ratio	
L1	138.5 - 108.5 (1)	0.012	1.023	0.000	0.075	0.000	1.037	1.333	H1-3+VT
L2	108.5 - 83.75 (2)	0.013	1.133	0.000	0.063	0.000	1.147	1.333	H1-3+VT
L3	83.75 - 43 (3)	0.013	1.149	0.000	0.046	0.000	1.163	1.333	H1-3+VT
L4	43 - 0 (4)	0.017	1.268	0.000	0.039	0.000	1.285	1.333	H1-3+VT

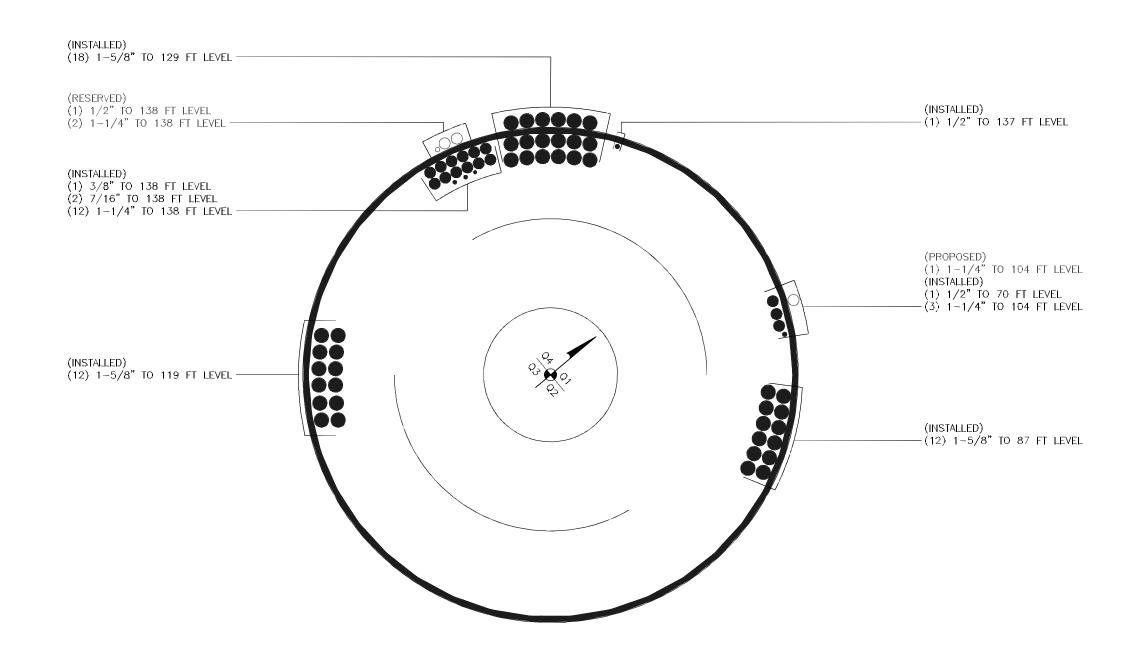
			Section Capac	ity Tak	ole			
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
L1	138.5 - 108.5	Pole	TP24.5x17.375x0.1875	1	-6.80	752.20	77.8	Pass
L2	108.5 - 83.75	Pole	TP31.88x24.5x0.25	2	-11.75	1249.44	86.0	Pass
L3	83.75 - 43	Pole	TP43.42x30.0382x0.3125	3	-20.85	2131.32	87.2	Pass

tnxTower Report - version 6.1.4.1

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail
L4	43 - 0	Pole	TP55.5x41.0206x0.3125	4	-32.94	2629.01	96.4 Summary	Pass
						Pole (L4) RATING =	96.4 96.4	Pass Pass

APPENDIX B BASE LEVEL DRAWING





APPENDIX C ADDITIONAL CALCULATIONS

Stiffened or Unstiffened, Exterior Flange Plate - Any Bolt Material TIA Rev F

Site Data

BU#: 876380

Site Name: O&G Woodbury App #: 246007, Rev. 1

Pole Manufacturer:	Other
--------------------	-------

В	olt Data		
Qty:	24		
Diameter (in.):	1	Bolt Fu:	120
Bolt Material:	A325	Bolt Fy:	92
N/A:	75	< Disregard	Bolt Fty:
N/A:	55	< Disregard	44.00
Circle (in.):	28		

Plate Data					
Diam:	31	in			
Thick, t:	1.5	in			
Grade (Fy):	60	ksi			
Strength, Fu:	75	ksi			
Single-Rod B-eff:	3.24	in			

Stiffener Data	Stiffener Data (Welding at Both Sides)							
Config:	0	*						
Weld Type:	Fillet							
Groove Depth:	0.25	< Disregard						
Groove Angle:	45	< Disregard						
Fillet H. Weld:	0.25	in						
Fillet V. Weld:	0.25	in						
Width:	3	in						
Height:	8	in						
Thick:	0.5	in						
Notch:	0.375	in						
Grade:	36	ksi						
Weld str.:	70	ksi						

Pole Data						
Diam:	24.5	in				
Thick:	0.1875	in				
Grade:	65	ksi				
# of Sides:	18	"0" IF Round				
Fu	80	ksi				
Reinf. Fillet Weld	0	"0" if None				

Stress Increase Factor						
ASIF:	1.333					

Reactions		
Moment:	288.76	ft-kips
Axial:	6.8	kips
Shear:	14.1	kips
Elevation:	108.5	feet

If No stiffeners, Criteria:	AISC ASD	<-Only Applcable to Unstif	fened Cases
Flange Bolt Results		-	Rigid

Flange Bolt Results

Bolt Tension Capacity, B:	46.07 kips
Max Bolt directly applied T:	20.34 Kips
Min. PL "tc" for B cap. w/o Pry:	1.340 in
In PL "treg" for actual T w/ Pry:	0.689 in

Min PL "t1" for actual T w/o Pry: 0.891 in T allowable w/o Prying: 46.07 kips α'<0 case

Prying Force, Q: 0.00 kips Total Bolt Tension=T+Q: 20.34 kips Non-Prying Bolt Stress Ratio, T/B: 44.2% Pass

Exterior Flange Plate Results Flexural Check Compression Side Plate Stress: 18.5 ksi

Allowable Plate Stress: 60.0 ksi Compression Plate Stress Ratio: 30.8% Pass

No Prying

Tension Side Stress Ratio, (treq/t)^2: 21.1% Pass

Rigid
Service ASD
0.75*Fy*ASIF
Comp. Y.L. Length:
13.56

Service, ASD Fty*ASIF

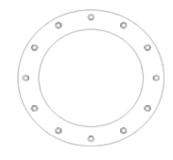
n/a

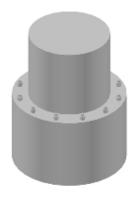
Stiffener Results

Horizontal Weld: n/a Vertical Weld: n/a Plate Flex+Shear, fb/Fb+(fv/Fv)^2: n/a Plate Tension+Shear, ft/Ft+(fv/Fv)^2: n/a Plate Comp. (AISC Bracket): n/a

Pole Results

Pole Punching Shear Check: n/a





^{* 0 =} none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

^{**} Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes



Input Cells in Yellow

ENGINEERING INNOVATION
FDH Engineering, Inc., 6521 Meridien Dr. Raleigh, NC 27616, Ph. 919.755.1012, Fax 919.755.1031

Anchor Rod Design

Legend Input Output/ Notes

8,151

A193 B7

Non-Upset

2.25

3.98

3.25

125

131.250

1.750

Pull Down

Pull Down

Pull Dowr

kips

kips

Bolt Grade

Thread Form

d new (in)

Ag new Ae new

Fy new

Tnew

Vnew

*Note: Use Anchor Rod Transfer Plate Design Tab in Conjunction

O&G Woodbury Site Name: 876380 Job No. : Elevation: 0

Code (F or G):	F	Pull Down
Anchor Bolts (Yes or No)	Yes	Pull Down
P (from RISA)	33	kips
V (from RISA)	28	kips
M (from DICA)	2011	ft king

Existing	g Rods	
у	32	in
No. Bolts	12	
BC	64	in
I	24453.12	in ⁴
Bolt Grade	A615-75	Pull Down
Thread Form	Non-Upset	-
d (in)	2.25	Pull Down
Ag	3.98	in ²
Ae	3.25	in ²
Fy	75	ksi
Fu	100	ksi

Itot	32604.16	in ⁴
Т	131.250	kips
V	1.750	kips

Itot	32004.10	
T	131.250	kips
V	1.750	kips
V	1.750	kips

Capacity (%)				Pullout Test Value	
Tn/Ω	194.5	kips	OK	67.48	218.90 kips
Tn/Ω, new	218.9	kips	OK	59.96	210.50 KIPS
øTn	260	kips			
øTn, new	325	kips			

= 0.8*Fu*Ae (anchor bolts only)
I = (No. Bolts/8)*BC ² *Ag

= (M*y*Ag)/Itot-P*(Ag/Atotal) $Tn/\Omega = 0.33*Fu*Ag*(4/3)$

Equations:

Notes:

*Ag and Ae are taken from AISC 13th Ed. Manual (pg. 7-83)

se use CAD

*I calc. will only work for symmetric bolt group, otherwis						
Interaction Equation Check	s (Rev. G: Sect	tion 4.9.9)	(1			
Detail Type (hover for detail)	С	Pull Down	ı			
ŋ	0.55		l			
I _{ar} , for Detail Type d only	2.25	in	(1			
øRnt	194.5	kips	ľ			
øRnv	119.4	kips	ı			
øRnm	94.922	kip-in	ı			
Mu	2.559375	kip-in	ı			
(Pu+Vu/ŋ)/øRnt < 1?	0.691	OK	ı			
(Vu/øRnv) ² +	NA		(
((Pu/øRnt)+(Mu/øRnm))2	NA		ľ			

works for Rev F also)

øTn = 0.75*Fu*Ae (non anchor bolts)

top of concrete to bottom of leveling nut)

only applicable for Detail Type d)

N	ow Dodo		Danid Embadment Laureth	for New De	, do
New Rods		Req'd Embedment Length	for New Ro	as	
y new	32	in	f'c, caisson's concrete strength	4000	psi
No. Bolts new	4		fy, rebar yield strength	60000	psi
BC new	64	in	d _b , diameter of vertical rebar	1	in

Note For New Anchor Rods:	
Williams Bars (Upset)	
A722 (Fy=127.7 ksi, Fu=150 ksi)	
A615-75 (Fv=75 ksi, Fu=100 ksi)	

I _d (vertical rebar dev. Length)	28.460	in
I _{dH} (Hilti dev. length)	76.630	in
G/1.5	3.000	in

Total Embed. Length of New Bolts	76.63	
	6.39	ft

Req'd Embedment Length for New Rods		
f'c, caisson's concrete strength	4000	psi
fy, rebar yield strength	60000	psi
d _b , diameter of vertical rebar	1	in
vertical rebar cage BC ø	73	in
vertical rebar top cover distance	3	in
т. Ultimate Hilti Bond Resistance	1.8	ksi

I _d (vertical rebar dev. Length)	28.460	in
I _{dH} (Hilti dev. length)	76.630	in
G/1.5	3.000	in

Stiffened or Unstiffened, Ungrouted, Circular Base Plate - Any Rod Material

TIA Rev F

Site Data

BU#: 876380 Site Name: O&G Woodbury

App#: 246007, Rev. 1

Po	e Manufacturer:	Other

Anchor Rod Data		
*Qty:	24	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	64	in

Plate Data		
Diam:	70	in
Thick:	1.5	in
Grade:	60	ksi
Single-Rod B-eff:	7.34	in

Stiffener Data (Welding at both sides)		
Config:	1	*
Weld Type:	Fillet	
Groove Depth:	0.25	< Disregard
Groove Angle:	45	< Disregard
Fillet H. Weld:	0.375	in
Fillet V. Weld:	0.25	in
Width:	7	in
Height:	16	in
Thick:	0.5	in
Notch:	0.75	in
Grade:	50	ksi
Weld str.:	70	ksi

Pole Data		
Diam:	55.5	in
Thick:	0.3125	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

Stress	Increase F	actor
ASIF:	1.333	

Reactions		
*Moment:	4245	ft-kips
Axial:	33	kips
Shear:	28	kips

*Anchor Rod quantiy and Moment are modified due to consideration of modifications.

AISC ASD <-Only Applicable to Unstiffened Cases If No stiffeners, Criteria:

Anchor Rod Results

Maximum Rod Tension: 131.3 Kips Allowable Tension: 195.0 Kips Anchor Rod Stress Ratio: 67.3% Pass

Base Plate Results	Flexural Check
Base Plate Stress:	58.0 ksi
Allowable Plate Stress:	60.0 ksi
Base Plate Stress Ratio:	96.6% Pass

Stiffened Service, ASD 0.75*Fy*ASIF Y.L. Length: N/A, Roark

Stiffened

Service, ASD

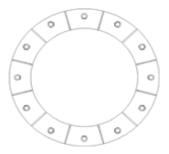
Fty*ASIF

Stiffener Results

Horizontal Weld: 99.3% Pass Vertical Weld: 69.8% Pass 37.8% Pass Plate Flex+Shear, fb/Fb+(fv/Fv)^2: Plate Tension+Shear, ft/Ft+(fv/Fv)^2: 83.5% Pass Plate Comp. (AISC Bracket): 95.2% Pass

Pole Results

Pole Punching Shear Check: 18.2% Pass





Analysis Date: 6/12/2014

^{* 0 =} none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

^{**} Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

(Bearing and Stability Checks) Tool for TIA Rev F or G - Application (MP, SST with unitbase)

Site Data

BU#: 876380 Site Name: O&G Woodbury App #: 246007, Rev. 1

Enter Load Factors Below:			
For P (DL) 1.2 < Enter Factor			
For P,V, and M (WL) 1.35 < Enter Factor			

Pad & Pier Data			
Base PL Dist. Above Pier:	0	in	
Pier Dist. Above Grade:	12	in	
Pad Bearing Depth, D:	6.5	ft	
Pad Thickness, T:	3	ft	
Pad Width=Length, L:	23	ft	
Pier Cross Section Shape:	Square	<pull down<="" td=""></pull>	
Enter Pier Side Width:	7	ft	
Concrete Density:	150.0	pcf	
Pier Cross Section Area:	49.00	ft^2	
Pier Height:	4.50	ft	
Soil (above pad) Height:	3.50	ft	

Soil Parameters			
Unit Weight, γ:	125.0	pcf	
Ultimate Bearing Capacity, qn:	12.00	ksf	
Strength Reduct. factor, φ:	0.75		
Angle of Friction, Φ:	34.0	degrees	
Undrained Shear Strength, Cu:	0.00	ksf	
Allowable Bearing: φ*qn:	9.00	ksf	
Passive Pres. Coeff., Kp	3.54		

Forces/Moments due to Wind and Lateral Soil			
Minimum of (φ*Ultimate Pad			
Passive Force, Vu):	37.8	kips	
Pad Force Location Above D:	1.35	ft	
φ(Passive Pressure Moment):		ft-kips	
Factored O.T. M(WL), "1.6W":	4122.9	ft-kips	
Factored OT (MW-Msoil), M1	4071.87	ft-kips	

Resistance due to Foundation Gravity			
Soil Wedge Projection grade, a:	2.36	ft	
Sum of Soil Wedges Wt:	29.83	kips	
Soil Wedges ecc, K1:	7.54	ft	
Ftg+Soil above Pad wt:	481.1	kips	
Unfactored (Total ftg-soil Wt):	510.96	kips	
1.2D. No Soil Wedges.	616.95	kips	
0.9D. With Soil Wedges	489.56	kips	

Resistance due to Cohesion (Vertical)		
φ*(1/2*Cu)(Total Vert. Planes)	0.00	kips
Cohesion Force Eccentricity, K2	0.00	ft

Monopole Base Reaction Forces			
TIA Revision:	F	<pull down<="" td=""></pull>	
Unfactored DL Axial, PD:	33	kips	
Unfactored WL Axial, PW:	0	kips	
Unfactored WL Shear, V:	28	kips	
Unfactored WL Moment, M:	2844	ft-kips	

Load Factor	Shaft Factored Loads		
1.20	1.2D+1.6W, Pu:	39.6	kips
0.90	0.9D+1.6W, Pu:	29.7	kips
1.05	Vu:	37.8	kips
1.35	Mu:	3839.4	ft-kips

1.2D+1.6W Load Combination, Bearing Results:

(<u>No Soil Wedges</u>) [Reaction+Conc+Soil]	616.95	P1="1.2D+1.6W" (Kips)
Factored "1.6W" Overturning Moment (MW-Msoil), M1	4071.87	ft-kips

Orthogonal Direction:

ecc1 = M1/P1 = 6.60 ft Orthogonal qu= 2.74 ksf qu/ ϕ *qn Ratio= **30.41%** Pass

Diagonal Direction:

ecc2 = (0.707M1)/P1 = 4.67 ft Diagonal qu= 3.30 ksf qu/ ϕ *qn Ratio= **36.70%** Pass

Run <-- Press Upon Completing All Input

Overturning Stability Check

0.9D+1.6W Load Combination, Bearing Results:

(w/ Soil Wedges) [Reaction+Conc+Soil]	489.56	P2="0.9D+1.6W" (Kips)
Factored "1.6W" Overturning Moment (MW-Msoil) - 0.9(M of Wedge + M of Cohesion), M2	3869.56	ft-kips
Outless and an O. MO/DO		

Orthogonal ecc3 = M2/P2 = 7.90 ft
Ortho Non Bearing Length,NBL= 15.81 ft
Orthogonal qu= 2.96 ksf
Diagonal qu= 3.50 ksf

Max Reaction Moment (ft-kips) so that qu=φ*qn = 100%			
Capacity Rating			
Actual M:	2844.00		
M Orthogonal:	3727.14	76.31%	Pass
M Diagonal:	3727.14	76.31%	Pass



RADIO FREQUENCY FCC REGULATORY COMPLIANCE MAXIMUM PERMISSIBLE EXPOSURE (MPE) ASSESSMENT

Sprint Existing Facility

Site ID: CT33XC520

O&G Woodbury

Great Hollow Road Woodbury, CT 06798

July 14, 2014

EBI Project Number: 62143788

21 B Street Burlington, MA 01803 Tel: (781) 273.2500 Fax: (781) 273.3311



July 14, 2014

Sprint Attn: RF Engineering Manager 1 International Boulevard, Suite 800 Mahwah, NJ 07495

Re: Radio Frequency Maximum Permissible Exposure (MPE) Assessment for Site: CT33XC520 - O&G Woodbury

Site Total: 50.30% - MPE% in full compliance

EBI Consulting was directed to analyze the proposed upgrades to the existing Sprint facility located at Great Hollow Road, Woodbury, CT, for the purpose of determining whether the radio frequency (RF) exposure levels from the proposed Sprint equipment upgrades on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm2). The number of μ W/cm2 calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μ W/cm²). The general population exposure limit for the cellular band (850 MHz Band) is approximately 567 μ W/cm², and the general population exposure limit for the 1900 MHz and 2500 MHz bands is 1000 μ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed upgrades to the existing Sprint Wireless antenna facility located at Great Hollow Road, Woodbury, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. All calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all emissions were calculated using the following assumptions:

- 1) 2 channels in the 1900 MHz Band were considered for each sector of the proposed installation.
- 2) 1 channel in the 800 MHz Band was considered for each sector of the proposed installation
- 3) 2 channels in the 2500 MHz Band were considered for each sector of the proposed installation.
- 4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 5) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.



- 6) The antennas used in this modeling are the RFS APXVSPP18-C-A20 and the RFS APXVTM14-C-I20. This is based on feedback from the carrier with regards to anticipated antenna selection. The RFS APXVSPP18-C-A20 has a 15.9 dBd gain value at its main lobe at 1900 MHz and 13.4 dBd at its main lobe for 850 MHz. The RFS APXVTM14-C-I20 has a 15.9 dBd gain value at its main lobe at 2500 MHz. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antenna mounting height centerline for the proposed antennas is **108 feet** above ground level (AGL).
- 8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public threshold limits

Site ID CT33XC520 - 0&G Woodbury Site Addresss Great Hollow Road, Woodbury, CT, 06798 Site Type Monopole Sector 1			
Site Type Monopole			
Sector 1			
Sector 1			
Power			
Fower Out Per Antenna Gain	Power		
Antenna Channel Number of Composite (10 db Antenna analysis Cable Loss Additional	Density		
Number Antenna Make Antenna Model Radio Type Frequency Band Technology (Watts) Channels Power reduction) Height (ft) height (fb) Loss (dB) Loss (dB) ERP	Percentage		
1a RFS APXVSPP18-C-A20 RRH 1900 MHz CDMA/LTE 20 2 40 5.9 108 102 1/2" 0.5 0 138.6			
1a RFS APXVSPP18-C-A20 RRH 850 MHz CDMA/LTE 20 1 20 3.4 108 102 1/2" 0.5 0 39.0			
1B RFS APXVTMM14-C-120 RRH 2500 MHz CDMA / LTE 20 2 40 5.9 108 102 1/2 " 0.5 0 138.6	0.85%		
Sector total Power Density Value: 1.56			
Sector 2			
Section 2			
Power			
Power Out Per Antenna Gain	Power		
Antenna Channel Number of Composite (10 db Antenna analysis Cable Loss Additional	Density		
Number Antenna Make Antenna Model Radio Type Frequency Band Technology (Watts) Channels Power reduction Height (ft) height Cable Size (dB) Loss (dB) ERP	Percentage		
2a RFS APX/SP18-C-A20 RRH 1900 MHz CDMA/LTE 20 2 40 5.9 108 102 1/2" 0.5 0 138.6			
2a RFS APXVSPP18-C-A20 RRH 850 MHz CDMA/LTE 20 1 20 3.4 108 102 1/2" 0.5 0 39.0			
2B RFS APXVTMM14-C-120 RRH 2500 MHz CDMA / LTE 20 2 40 5.9 108 102 1/2 " 0.5 0 138.6	0.85%		
Sector total Power Density Value: 1.56'			
Sector 3			
Section 3			
Power			
Fower Out Per Antenna Gain	Power		
Antenna Channel Number of Composite (10 db Antenna analysis Cable Loss Additional	Density		
Number Antenna Make Antenna Model Radio Type Frequency Band Technology (Watts) Channels Power reduction) Height (ft) hei	Percentage		
3a RFS APXVSPP18-C-A20 RRH 1900 MHz CDMA/LTE 20 2 40 5.9 108 102 1/2" 0.5 0 138.6			
3a RFS APXVSPP18-C-A20 RRH 850 MHz CDMA/LTE 20 1 20 3.4 108 102 1/2" 0.5 0 39.0			
3B RFS APXVTMM14-C-120 RRH 2500 MHz CDMA / LTE 20 2 40 5.9 108 102 1/2 " 0.5 0 138.6	0.85%		
Sector total Power Density Value: 1.56			

Site Composite MPE %		
Carrier	MPE %	
Sprint	4.69%	
AT&T	16.64%	
Verizon Wireless	17.93%	
Nextel	5.28%	
T-Mobile	4.47%	
CL&P	1.29%	
Total Site MPE %	50.30%	



Summary

All calculations performed for this analysis yielded results that were well within the allowable limits for general public Maximum Permissible Exposure (MPE) to radio frequency energy.

The anticipated Maximum Composite contributions from the Sprint facility are 4.69% (1.56% from sector 1, 1.56% from sector 2 and 1.56% from sector 3) of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level.

The anticipated composite MPE value for this site assuming all carriers present is **50.30**% of the allowable FCC established general public limit sampled at 6 feet above ground level. This total composite site value is based upon MPE values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

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RF Engineering Director

EBI Consulting

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