Crown Castle
3530 Toringdon Way, Suite 300
Charlotte, NC 28277

December 8, 2014

Melanie A. Bachman
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: **Sprint PCS-Exempt Modification - Crown Site BU: 876315**
Sprint PCS Site ID: CT03XC020
Located at: 1116 Johnson Rd (a/k/a 1027 Racebrook Rd), Woodbridge, CT 06525

Dear Ms. Bachman:

This letter and exhibits are submitted on behalf of Sprint PCS (Sprint). Sprint is making modifications to certain existing sites in its Connecticut system in order to implement their 2.5GHz LTE technology. Please accept this letter and exhibits as notification, pursuant to § 16-50j-73 of the Regulations of Connecticut State Agencies ("R.C.S.A."), of construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Mrs. Ellen Scalettar, First Selectman for the Town of Woodbridge, and Baldwin Wonnell Racebrook LLC, Property Owner.

Sprint plans to modify the existing wireless communications facility owned by Crown Castle and located at **1116 Johnson Rd (a/k/a 1027 Racebrook Rd), Woodbridge, CT 06525**. Attached are a compound plan and elevation depicting the planned changes (Exhibit-1), and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration (Exhibit-2). Also included is a power density table report reflecting the modification to Sprint's operations at the site (Exhibit-3).

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes ("C.G.S.") § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in the R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Sprint's additional antennas will be located at the same elevation on the existing tower.

2. There will be no proposed modifications to the ground and no extension of boundaries.

3. The proposed modifications will not increase noise levels at the facility by six decibels or more.

4. A Structural Modification Report confirming that the tower and foundation can support Sprint's proposed modifications is included as Exhibit-2.

5. The operation of the additional antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table report for Sprint's modified facility is included as Exhibit-3.

For the foregoing reasons, Sprint respectfully submits the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Donna Neal.

Sincerely,

Susan Vale
Real Estate Specialist

Enclosures

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes
Tab 2: Exhibit-2: Structural Modification Report
Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Mrs. Ellen Scalettar, First Selectman
Town Hall
11 Meetinghouse Lane
Woodbridge, CT 06525

Baldwin Wonnell Racebrook LLC
1015 Racebrook Rd
Woodbridge, CT 06525

Sprint

2.5 EQUIPMENT DEPLOYMENT

SITE NUMBER:

CT03XC020

SITE NAME:

OAK LANE CC, INC. TOWER

SITE ADDRESS:

1116 JOHNSON RD
WOODBRIDGE, CT 06525

CROWN ID#: 876315

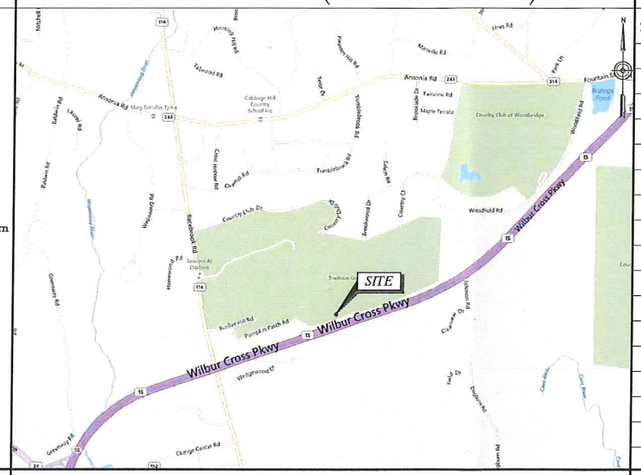
CROWN SITE NAME: OAK LANE CC, INC. TOWER (SSUSA

SHEET INFORMATION

SITE NUMBER:	CT03XC020
SITE NAME:	OAK LANE CC, INC. TOWER
SITE ADDRESS:	1116 JOHNSON RD WOODBRIDGE, CT 06525
COUNTY:	NEW HAVEN
COORDINATES: (NAD 83)	41° 19' 0.6" N 73° 0' 41.7" W
GROUND ELEV:	259'± AMSL
STRUCTURE TYPE:	MONOPOLE
STRUCTURE HEIGHT:	150'-6"± AGL
STRUCTURE RAD CENTER:	153'-0"± AGL
ZONING CLASSIFICATION:	OUTBUILDINGS
MAP-BLOCK-LOT:	3003/890/1114//

LANDLORD:	CROWN CASTLE USA 2000 CORPORATE DRIVE CANONSBURG, PA
LOCAL POWER COMPANY:	CONNECTICUT LIGHT AND POWER CONTACT CUSTOMER SERVICE (800) 286-2000
APPLICANT:	SPRINT 6580 SPRINT PARKWAY OVERLAND PARK, KANSAS 66251
ENGINEER:	JAMES QUICKSELL (845) 567-6656 EXT. 2835 JQuicksell@tectonicengineering.com
SPRINT CM:	PETER CULBERT Peter.Culbert@sprint.com
CROWN CM:	JASON D'AMICO (860) 209-0104 jason.d'amico@crowncastle.com
AAV:	AT&T

VICINITY MAP (NOT TO SCALE)



SHEET INDEX

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GENERAL NOTES

1. THIS IS AN UNMANNED TELECOMMUNICATION FACILITY AND NOT FOR HUMAN HABITATION: HANDICAP ACCESS REQUIREMENTS ARE NOT REQUIRED. FACILITY HAS NO PLUMBING OR REFRIGERANTS. THIS FACILITY SHALL MEET OR EXCEED ALL FAA AND FCC REGULATOR REQUIREMENTS.
2. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE PROJECT OWNER'S REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
3. DEVELOPMENT AND USE OF THIS SITE WILL CONFORM TO ALL APPLICABLE CODES AND ORDINANCES.
   - 2005 STATE OF CONNECTICUT BUILDING CODE.
   - ANSI/TIA/EIA-222-F-1996.
   - NATIONAL ELECTRICAL CODE, LATEST EDITION.

PROJECT DESCRIPTION

1. (1) NEW 2.5 EQUIPMENT RACK INSIDE EXIST MMBTS CABINET.
2. (3) NEW RFS APXVTM14-C-120 ANTENNAS.
3. (3) NEW TD-RRH8x20-25 RRH.
4. (1) NEW 5/8" FIBER CABLE.

AERIAL VIEW (NOT TO SCALE)

APPROVALS

THE FOLLOWING PARTIES HEREBY APPROVE AND ACCEPT THESE DOCUMENTS AND AUTHORIZE THE CONTRACTOR TO PROCEED WITH THE CONSTRUCTION DESCRIBED HEREIN. ALL DOCUMENTS ARE SUBJECT TO REVIEW BY THE LOCAL BUILDING DEPARTMENT AND MAY IMPOSE CHANGES OR MODIFICATIONS.

CONSTRUCTION: _____ DATE: _____

LEASING/
SITE ACQUISITION: _____ DATE: _____

LANDLORD/
PROPERTY OWNER: _____ DATE: _____

R.F. ENGINEER: _____ DATE: _____

CALL TOLL FREE FOR CONNECTICUT
1-800-922-4455 OR DIAL 811
CALL TWO FULL WORKING DAYS IN ADVANCE TO LOCATE BURIED UTILITY PIPES AND CABLES

Sprint
2.5 EQUIPMENT DEPLOYMENT
6580 SPRINT PARKWAY
OVERLAND PARK, KANSAS 66251

CROWN CASTLE

TECTONIC
• PLANNING
• ENGINEERING
• SURVEYING
• CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.
1279 Route 300
Newburgh, NY 12550
Phone: (845) 567-6656
Fax: (845) 567-8703
www.tectonicengineering.com

SUBMITTALS

PROJECT NO: 7225.CT03XC020

NO	DATE	DESCRIPTION	BY
0	06/14/14	FOR COMMENT	DC
1	12/05/14	FOR CONSTRUCTION	DC

DATE	REVIEWED BY
12/5/14	TMG

STATE OF CONNECTICUT
ANTONIO A. GUALTIERI
No. 25406
LICENSED PROFESSIONAL ENGINEER

SITE NUMBER:
CT03XC020

SITE NAME:
OAK LANE CC, INC. TOWER

SITE ADDRESS:
1116 JOHNSON RD
WOODBRIDGE, CT 06525

SHEET TITLE:
TITLE SHEET

SHEET NO:
T-1

DIVISION 01000–GENERAL NOTES

1. THE CONTRACTOR SHALL GIVE ALL NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY, MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS, AND LOCAL AND STATE JURISDICTIONAL CODES BEARING ON THE PERFORMANCE OF THE WORK. THE WORK PERFORMED ON THE PROJECT AND THE MATERIALS INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES.
2. THE ARCHITECT/ENGINEER HAVE MADE EVERY EFFORT TO SET FORTH IN THE CONSTRUCTION AND CONTRACT DOCUMENTS THE COMPLETE SCOPE OF WORK. THE CONTRACTOR BIDDING THE JOB IS NEVERTHELESS CAUTIONED THAT MINOR OMISSIONS OR ERRORS IN THE DRAWINGS AND OR SPECIFICATIONS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING THE PROJECT AND IMPROVEMENTS IN ACCORDANCE WITH THE INTENT OF THESE DOCUMENTS.
3. THE CONTRACTOR OR BIDDER SHALL BEAR THE RESPONSIBILITY OF NOTIFYING (IN WRITING) THE PROJECT OWNER'S REPRESENTATIVE OF ANY CONFLICTS, ERRORS, OR OMISSIONS PRIOR TO THE SUBMISSION OF CONTRACTOR'S PROPOSAL OR PERFORMANCE OF WORK.
4. THE SCOPE OF WORK SHALL INCLUDE FURNISHING ALL MATERIALS, EQUIPMENT, LABOR AND ALL OTHER MATERIALS AND LABOR DEEMED NECESSARY TO COMPLETE THE WORK/PROJECT AS DESCRIBED HEREIN.
5. THE CONTRACTOR SHALL VISIT THE JOB SITE PRIOR TO THE SUBMISSION OF BIDS OR PERFORMING WORK TO FAMILIARIZE HIMSELF WITH THE FIELD CONDITIONS AND TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
6. ONCE THE CONTRACTOR HAS RECEIVED AND ACCEPTED THE NOTICE TO PROCEED, CONTRACTOR WILL CONTACT THE CROWN CASTLE CONSTRUCTION MANAGER OF RECORD (NOTED ON THE FIRST PAGE ON THIS CONSTRUCTION DRAWING) A MINIMUM OF 48 HOURS PRIOR TO WORK START. UPON ARRIVAL TO THE JOB SITE, CONTRACTOR CREW IS REQUIRED CALL 1–800–788–7011 TO NOTIFY THE CROWN CASTLE NOC WORK HAS BEGUN.
7. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS ACCORDING TO THE MANUFACTURER'S/VENDOR'S SPECIFICATIONS UNLESS NOTED OTHERWISE OR WHERE LOCAL CODES OR ORDINANCES TAKE PRECEDENCE.
8. THE CONTRACTOR SHALL PROVIDE A FULL SET OF CONSTRUCTION DOCUMENTS AT THE SITE UPDATED WITH THE LATEST REVISIONS AND ADDENDUMS OR CLARIFICATIONS AVAILABLE FOR THE USE BY ALL PERSONNEL INVOLVED WITH THE PROJECT.
9. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER THE CONTRACT.
10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS WHICH MAY BE REQUIRED FOR THE WORK BY THE ARCHITECT/ENGINEER, THE STATE, COUNTY OR LOCAL GOVERNMENT AUTHORITY.
11. THE CONTRACTOR SHALL MAKE NECESSARY PROVISIONS TO PROTECT EXISTING IMPROVEMENTS, EASEMENTS, PAVING, CURBING, ETC. DURING CONSTRUCTION. UPON COMPLETION OF WORK, THE CONTRACTOR SHALL REPAIR ANY DAMAGE THAT MAY HAVE OCCURRED DUE TO CONSTRUCTION ON OR ABOUT THE PROPERTY.
12. THE CONTRACTOR SHALL KEEP THE GENERAL WORK AREA CLEAN AND HAZARD FREE DURING CONSTRUCTION AND DISPOSE OF ALL DIRT, DEBRIS, RUBBISH AND REMOVE EQUIPMENT NOT SPECIFIED AS REMAINING ON THE PROPERTY. PREMISES SHALL BE LEFT IN CLEAN CONDITION AND FREE FROM PAINT SPOTS, DUST, OR SMUDGES OF ANY NATURE.
13. THE CONTRACTOR SHALL COMPLY WITH ALL PERTINENT SECTIONS OF THE BASIC STATE BUILDING CODE, LATEST EDITION, AND ALL OSHA REQUIREMENTS AS THEY APPLY TO THIS PROJECT. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK SHALL BE RELOCATED AS DIRECTED BY THE ARCHITECT/ENGINEER. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR PIER DRILLING AROUND OR NEAR UTILITIES. THE CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT LIMITED TO A) FALL PROTECTION, B) CONFINED SPACE, C) ELECTRICAL SAFETY, D) TRENCHING AND EXCAVATION OF ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHICH INTERFERE WITH THE EXECUTION OF THE WORK SHALL BE REMOVED AND OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT THE POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK SUBJECT TO THE APPROVAL OF THE ARCHITECT/ENGINEER.
14. THE CONTRACTOR SHALL NOTIFY THE PROJECT OWNER'S REPRESENTATIVE IN WRITING WHERE A CONFLICT OCCURS ON ANY OF THE CONTRACT DOCUMENTS. THE CONTRACTOR IS NOT TO ORDER MATERIAL OR CONSTRUCT ANY PORTION OF THE WORK THAT IS IN CONFLICT UNTIL CONFLICT IS RESOLVED BY THE LESSEE/LICENSEE REPRESENTATIVE.
15. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, PROPERTY LINES, ETC. ON THE JOB.
16. THE CONTRACTOR SHALL NOTIFY THE THE RF ENGINEER FOR ANTENNA AZIMUTH VERIFICATION (DURING ANTENNA INSTALLATION) PRIOR TO CONDUCTING SWEEP TESTS.
17. THE CONTRACTOR SHALL SUBMIT AT THE END OF THE PROJECT A COMPLETE SET OF AS–BUILT DRAWINGS TO THE CLIENT REPRESENTATIVE.
18. REFER TO: CONSTRUCTION STANDARDS–SPRINT DOCUMENT EXHIBIT A–STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES REV. 4.0– 02.15.2011.DOCM.
19. REFER TO: WEATHER PROOFING SPECS: EXCERPT EXH A–WIHRPRF–STD CONSTR SPECS._157201110421855492.DOCM.
20. REFER TO: COLOR CODING–SPRINT NEXTEL ANT AND LINE COLOR CODING (DRAFT) V3 09–08–11.PDF
21. REFER TO LATEST DOCUMENTATION REVISION.

DIVISION 03000–CONCRETE

1.03 APPLICABLE STANDARDS (USE LATEST EDITIONS)

A. ACI–301 – SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS.
B. ACI–347 GUIDE TO FORM WORK FOR CONCRETE.
C. ASTM C33– CONCRETE AGGREGATE
D. ASTM C94 – READY MIXED CONCRETE e. ASTM C150 – PORTLAND CEMENT.
E. ASTM C260 – AIR–ENTRAINING ADMIXTURES FOR CONCRETE
F. ASTM C309– LIQUID MEMBRANE FORMING COMPOUNDS FOR CURING CONCRETE.
H. ASTM C494 – CHEMICAL ADMIXTURES FOR CONCRETE
I. ASTM A615– DEFORMED AND PLAIN BILLET–STEEL BARS FOR CONCRETE REINFORCEMENT
J. ASTM A185– STEEL WELDED WIRE FABRIC (PLAIN) FOR CONCRETE REINFORCEMENT

1.04 QUALITY ASSURANCE

CONCRETE MATERIALS AND OPERATIONS SHALL BE TESTED AND INSPECTED BY THE ARCHITECT/ENGINEER AS DIRECTED BY THE CLIENT'S REPRESENTATIVE.

3.04 SURFACE FINISHES

A. SURFACES AGAINST WHICH BACKFILL OR CONCRETE SHALL BE PLACED REQUIRE NO TREATMENT EXCEPT REPAIR OF DEFECTIVE AREAS.

B. SURFACES THAT WILL BE PERMANENTLY EXPOSED SHALL PRESENT A UNIFORM FINISH PROVIDED BY THE REMOVAL OF FINS AND THE FILLING HOLES AND OTHER IRREGULARITIES WITH DRY PACK GROUT, OR BY SACKING WITH UTILITY OR ORDINARY GROUT.

C. SURFACES THAT WOULD NORMALLY BE LEVEL AND WHICH WILL BE PERMANENTLY EXPOSED TO THE WEATHER SHALL BE SLOPED FOR DRAINAGE. UNLESS ENGINEER'S DESIGN DRAWING SPECIFIES A HORIZONTAL SURFACE OR SURFACES SUCH AS STAIR TREADS, WALLS, CURBS, AND PARAPETS SHALL BE SLOPED APPROXIMATELY 1/4" PER FOOT.

D. SURFACES THAT WILL BE COVERED BY BACKFILL OR CONCRETE SHALL BE SMOOTH SCREENED.

E. EXPOSED SLAB SURFACES SHALL BE CONSOLIDATED, SCREENED, FLOATED, AND STEEL TROWELED. HAND OR POWER–DRIVEN EQUIPMENT MAY BE USED FOR FLOATING. FLOATING SHALL BE STARTED AS SOON AS THE SCREENED SURFACE HAS ATTAINED A STIFFNESS TO PERMIT FINISHING OPERATIONS. OPERATIONS. ALL EDGES MUST HAVE A 3/4" CHAMFER.

1.04 QUALITY ASSURANCE CONCRETE MATERIALS AND OPERATIONS SHALL BE TESTED AND INSPECTED BY THE ENGINEER.

3.05 PATCHING

THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY UPON REMOVAL OF THE FORMS TO OBSERVE CONCRETE SURFACE CONDITIONS. IMPERFECTIONS SHALL BE PATCHED ACCORDING TO THE ENGINEER'S DIRECTION.

3.06 DEFECTIVE CONCRETE

THE CONTRACTOR SHALL NOTIFY OR REPLACE CONCRETE NOT CONFORMING TO REQUIRED LEVELS AND LINES, DETAILS, AND ELEVATIONS AS SPECIFIED IN ACI 301.

3.07 PROTECTION

A. IMMEDIATELY AFTER PLACEMENT. THE CONTRACTOR SHALL PROTECT THE CONCRETE FROM PREMATURE DRYING, EXCESSIVELY HOT OR COLD TEMPERATURES, AND MECHANICAL INJURY. FINISHED WORK SHALL BE PROTECTED.

B. CONCRETE SHALL BE MAINTAINED WITH MINIMAL MOISTURE LOSS AT RELATIVELY CONSTANT TEMPERATURE FOR PERIOD NECESSARY FOR HYDRATION OF CEMENT AND HARDENING OF CONCRETE.

C. ALL CONCRETE SHALL BE WATER CURED PER ACCEPTABLE PRACTICES SPECIFIED BY ACI CODE (LATEST EDITION)

DIVISION 05000 – METALS

PART 1 – GENERAL

1.01 WORK INCLUDED

A. THE WORK CONSISTS OF THE FABRICATION AND INSTALLATION OF ALL MATERIALS TO BE FURNISHED. AND WITHOUT LIMITING THE GENERALITY THEREOF, INCLUDING ALL EQUIPMENT, LABOR AND SERVICES REQUIRED FOR ALL STRUCTURAL STEEL WORK AND ALL ITEMS INCIDENTAL AS SPECIFIED AND AS SHOWN ON THE DRAWINGS:

1. STEEL FRAMING INCLUDING BEAMS, ANGLES, CHANNELS AND PLATES.
2. WELDING AND BOLTING OF ATTACHMENTS.

1.02 REFERENCE STANDARDS

A. THE WORK SHALL CONFORM TO THE CODES AND STANDARDS OF THE FOLLOWING AGENCIES AS FURTHER CITED HEREIN:

1. ASTM: AMERICAN SOCIETY FOR TESTING AND MATERIALS AS PUBLISHED IN "COMPILATION OF ASTM STANDARDS IN BUILDING CODES" OR LATEST EDITION.
2. AWS: AMERICAN WELDING SOCIETY CODE OR LATEST EDITION.
3. AISC: AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" (LATEST EDITION).

PART 2 – PRODUCTS

2.01 MATERIALS

A. STRUCTURAL STEEL: SHALL COMPLY WITH THE REQUIREMENTS OF ASTM A36 AND A992 FOR STRUCTURAL STEEL.

ALL PROPOSED STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH AISC CODE AND ASTM SPECIFICATIONS (LATEST EDITION) ALL NEW STEEL SHALL CONFORM TO THE FOLLOWING.

1. STRUCTURAL WIDE FLANGE: ASTM A992 Fy=50KSI.
2. MISCELLANEOUS STEEL (PLATES), CHANNELS, ANGLES, ETC): ASTM A36 (Fy=36KSI).
3. STRUCTURAL TUBING: ASTM A500 Gr. B (Fy=46KSI).
4. STEEL PIPE: ASTM A53 Gr B (Fy=35KSI).

2.02 WELDING

A. ALL WELDING SHALL BE DONE BY CERTIFIED WELDERS. CERTIFICATION DOCUMENTS SHALL BE MADE AVAILABLE FOR ENGINEER'S AND/OR OWNER'S REVIEW IF REQUESTED.

B. WELDING ELECTRODES FOR MANUAL SHIELDED METAL ARC WELDING SHALL CONFORM TO ASTM 1–233, E70 SERIES. BARE ELECTRODES AND GRANULAR FLUX USED IN THE SUBMERGED ARC PROCESS SHALL CONFORM TO AISC SPECIFICATIONS.

C. FIELD WELDING SHALL BE DONE AS PER AWS D1.1 REQUIREMENTS VISUAL INSPECTION IS ACCEPTABLE.

D. STUD WELDING SHALL BE ACCOMPLISHED BY CAPACITOR DISCHARGE (CD) WELDING TECHNIQUE USING CAPACITOR DISCHARGE STUD WELDER.

E. PROVIDE STUD FASTENERS OF MATERIALS AND SIZES SHOWN ON DRAWINGS OR AS RECOMMENDED BY THE MANUFACTURER FOR STRUCTURAL LOADINGS REQUIRED.

F. FOLLOW MANUFACTURERS SPECIFICATIONS AND INSTRUCTIONS TO PROPERLY SELECT AND INSTALL STUD WELDS.

2.03 BOLTING

A. BOLTS SHALL BE CONFORMING TO ASTM A35 HIGH STRENGTH HOT DIP GALVANIZED WITH ASTM A153 HEAVY HEX TYPE NUTS.

B. BOLTS SHALL BE 3/4" (MINIMUM) CONFORMING TO ASTM A325, HOT DIP GALVANIZED, ASTM A153 NUTS SHALL BE HEAVY HEX TYPE.

C. ALL CONNECTIONS SHALL BE 2 BOLTS MINIMUM.

D. EXCEPT WHERE SHOWN, ALL BEAM TO BEAM AND BEAM TO COLUMN CONNECTIONS TO BE DOUBLE ANGLED CONNECTIONS WITH HIGH STRENGTH BOLTS (THREADS EXCLUDED FROM SHEAR PLANE) AND HARDENED WASHERS.

E. STANDARD, OVERSIZED OR HORIZONTAL SHORT SLOTTED HOLES.

F. SNUG–TIGHT STRENGTH BEARING BOLTS MAY BE USED IN STANDARD HOLES CONFORMING TO ACIS, USING THE TURN OF THE NUT METHOD.

H. FULLY–TENSIONED HIGH STRENGTH (SLIP CRITICAL) SHALL BE USED IN OVERSIZED SLOT HOLES (RESPECTIVE OF SLOT ORIENTATION).

I. ALL BRACED CONNECTION, MOMENT CONNECTION AND CONNECTIONS NOTED AS "SLIP CRITICAL" SHALL BE BE SLIP CRITICAL JOINTS WITH CLASS A SURFACE CONDITIONS, UNLESS OTHERWISE NOTED.

J. EPOXY ANCHOR ASSEMBLIES SHALL BE AS MANUFACTURED BY HILTI OR ENGINEER APPROVED EQUAL, AS FOLLOWS:

BASE MATERIAL	ANCHOR SYSTEM
CONCRETE	HILTI HIT–HY 200
HOLLOW & GROUTED CMU OR BRICK	HILTI HIT–HY 70

2.04 FABRICATION

A. FABRICATION OF STEEL SHALL CONFORM TO THE AISC AND AWS

2.05 FINISH

A. STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE HOT–DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123. (LATEST EDITION) UNLESS OTHERWISE NOTED.

2.06 PROTECTION

A. UPON COMPLETION OF ERECTION, INSPECT ALL GALVANIZED STEEL AND PAINT ANY FIELD CUTS, WELDS OR GALVANIZED BREAKS WITH (2) COATS OF ZINC–RICH COLD GALVANIZING PAINT.

PART 3 – ERECTION

A. PROVIDE ALL ERECTION, EQUIPMENT, BRACING, PLANKING, FIELD BOLTS, NUTS, WASHERS, DRIFT PINS, AND SIMILAR MATERIALS WHICH DO NOT FORM A PART OF THE COMPLETED CONSTRUCTION, BUT ARE NECESSARY FOR ITS PROPER ERECTION.

B. ERECT AND ANCHOR ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC REFERENCE STANDARDS. ALL WORK SHALL BE ACCURATELY SET TO ESTABLISHED SUITABLE ATTACHMENTS TO THE CONSTRUCTION OF THE BUILDING

C. TEMPORARY BRACING, GUYING, AND SUPPORT SHALL BE PROVIDED TO KEEP THE STRUCTURE SET AND ALIGNED AT ALL TIMES DURING CONSTRUCTION, AND TO PREVENT DANGER TO PERSONS AND PROPERTY. CHECK ALL TEMPORARY LOADS AND STAY WITHIN SAFE CAPACITY OF ALL BUILDING COMPONENTS.

Sprint

2.5 EQUIPMENT DEPLOYMENT
6580 SPRINT PARKWAY
OVERLAND PARK, KANSAS 66251

CROWN CASTLE

TECTONIC • PLANNING • ENGINEERING • SURVEYING • CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.

1279 Route 300
Newburgh, NY 12550

Phone: (845) 567–6656
Fax: (845) 567–8703

www.tectonicengineering.com

SUBMITTALS

PROJECT NO: 7225.CT03XC020

NO	DATE	DESCRIPTION	BY
0	06/14/14	FOR COMMENT	DC
1	12/05/14	FOR CONSTRUCTION	DC

DATE	REVIEWED BY
12/5/14	

STATE OF CONNECTICUT
ANTONIO A. GUALTIERI
No. 25406
LICENSED PROFESSIONAL ENGINEER

SITE NUMBER:
CT03XC020

SITE NAME:
OAK LANE CC, INC. TOWER

SITE ADDRESS:
1116 JOHNSON RD
WOODBRIDGE, CT 06525

SHEET TITLE:
GENERAL NOTES

SHEET NO:
SP–1

DIVISION 13000–SPECIAL CONSTRUCTION ANTENNA INSTALLATION

PART 1 – GENERAL

1.01 WORK INCLUDED

A. ANTENNAS AND HYBRIFLEX CABLES ARE FURNISHED BY CLIENT'S REPRESENTATIVE UNDER SEPARATE CONTRACT. THE CONTRACTOR SHALL ASSIST ANTENNA INSTALLATION CONTRACTOR IN TERMS OF COORDINATION AND SITE ACCESS. ERECTION SUBCONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPERTY.

B. INSTALL ANTENNAS AS INDICATED ON DRAWINGS AND CLIENT'S REPRESENTATIVE SPECIFICATIONS.

C. INSTALL GALVANIZED STEEL ANTENNA MOUNTS AS INDICATED ON DRAWINGS.

D. INSTALL FURNISHED GALVANIZED STEEL OR ALUMINUM WAVEGUIDE AND PROVIDE PRINTOUT OF THAT RESULT

F. INSTALL HYBRIFLEX CABLES AND TERMINATIONS BETWEEN ANTENNAS AND EQUIPMENT PER MANUFACTURER'S RECOMMENDATIONS. WEATHERPROOF ALL CONNECTORS BETWEEN THE ANTENNA AND EQUIPMENT PER MANUFACTURER'S REQUIREMENTS.

G. ANTENNA AND HYBRIFLEX CABLE GROUNDING:

1. ALL EXTERIOR #6 GREEN GROUND WIRE DAISY CHAIN CONNECTIONS ARE TO BE WEATHER SEALED WITH ANDREWS CONNECTOR/SPLICE WEATHERPROOFING KIT TYPE 3221213 OR EQUIVALENT.

2. ALL HYBRIFLEX CABLE GROUNDING KITS ARE TO BE INSTALLED ON STRAIGHT RUNS OF HYBRIFLEX CABLE (NOT WITHIN BENDS). 1.02 RELATED WORK FURNISH THE FOLLOWING WORK AS SPECIFIED UNDER CONSTRUCTION DOCUMENTS, BUT COORDINATE WITH QOTHER TRADES PRIOR TO BID:

   1. FLASHING OF OPENING INTO OUTSIDE WALLS.
   2. SEALING AND CAULKING ALL OPENINGS.
   3. PAINTING.
   4. CUTTING AND PATCHING.

1.03 REQUIREMENTS OF REGULATOR AGENCIES

A. FURNISH U.L. LISTED EQUIPMENT WHERE SUCH LABEL IS AVAILABLE. INSTALL IN CONFORMANCE WITH U.L. STANDARDS WHERE APPLICABLE.

B. INSTALL ANTENNA, ANTENNA CABLES, GROUNDING SYSTEM IN ACCORDANCE WITH DRAWINGS AND SPECIFICATIONS IN EFFECT AT PROJECT LOCATION AND RECOMMENDATIONS OF STATE AND LOCAL BUILDING CODES HAVING JURISDICTION OVER SPECIFIC PORTIONS OF WORK. THIS WORK INCLUDES, BUT IS NOT LIMITED TO THE FOLLOWING:

1. EIA – ELECTRONIC INDUSTRIES ASSOCIATION RS–22. STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES.

2. FAA – FEDERAL AVIATION ADMINISTRATION ADVISORY CIRCULAR AC 70/7480–IH, CONSTRUCTION MARKING AND LIGHTING.

3. FCC – FEDERAL COMMUNICATION COMMISSION RULES AND REGULATIONS FORM 715, OBSTRUCTION MARKING AND LIGHTING SPECIFICATION FOR ANTENNA STRUCTURES

4. AISC – AMERICAN INSTITUTE OF STEEL CONSTRUCTION FOR STRUCTURAL JOINTS USING ASTM 1325 OR A490 BOLTS.

5. NEC – NATIONAL ELECTRIC CODE – ON TOWER LIGHTING KITS.

6. UL – UNDERWRITER'S LABORATORIES APPROVED ELECTRICAL PRODUCTS.

7. IN ALL CASES, PART 77 OF THE FAA RULES AND PARTS 17 AND 22 OF THE FCC RULES ARE APPLICABLE AND IN THE EVENT OF CONFLICT, SUPERSEDE ANY OTHER STANDARDS OR SPECIFICATIONS.

8. LIFE SAFETY CODE NFPA, LATEST EDITION.

DIVISION 13000–EARTHWORK

PART 1 GENERAL

1.01 WORK INCLUDED: REFER TO SURVEY AND SITE PLAN FOR WORK INCLUDED.

1.02 RELATED WORK

A. CONSTRUCTION OF EQUIPMENT FOUNDATIONS
B. INSTALLATION OF ANTENNA SYSTEM

PART 2 PRODUCTS

2.01 MATERIALS

A. ROAD AND SITE MATERIALS; FILL MATERIAL SHALL BE ACCEPTABLE, SELECT FILL SHALL BE IN ACCORDANCE WITH LOCAL DEPARTMENT OF HIGHWAY AND PUBLIC TRANSPORTATION STANDARD SPECIFICATIONS.

B. SOIL STERILIZER SHALL BE EPA REGISTERED OF LIQUID COMPOSITION AND OF PRE–EMERGENCE DESIGN.

C. SOIL STABILIZER FABRIC SHALL BE MIRAFI OR EQUAL – 600X AT ACCESS ROAD AND COMPOUND.

D. GRAVEL FILL; WELL GRADED, HARD, DURABLE, NATURAL SAND AND GRAVEL, FREE FROM ICE AND SNOW, ROOTS, SOD RUBBISH, AND OTHER DELETERIOUS OR ORGANIC MATTER.

MATERIAL SHALL CONFORM TO THE FOLLOWING GRADATION REQUIREMENTS.

GRAVEL FILL TO BE PLACED IN LIFTS OF 9" MAXIMUM THICKNESS AND 90 % DENSITY. COMPACTED TO 95

E. NO FILL OR EMBANKMENT MATERIALS SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OF EMBANKMENT

2.02 EQUIPMENT

A. COMPACTION SHALL BE ACCOMPLISHED BY MECHANICAL MEANS. LARGER AREAS SHALL BE COMPACTED BY SHEEPS FOOT, VIBRATORY OR RUBBER TIED ROLLERS WEIGHING AT LEAST FIVE TONS. SMALLER AREAS SHALL BE COMPACTED BY POWER–DRIVER, HAND HELD TAMPERS.

B. PRIOR TO OTHER EXCAVATION AND CONSTRUCTION EFFORTS GRUB ORGANIC MATERIAL TO A MINIMUM OF 6" BELOW ORIGINAL GROUND LEVEL.

C. UNLESS OTHERWISE INSTRUCTED BY CLIENT'S REPRESENTATIVE. REMOVE TREES, BRUSH AND DEBRIS FROM THE PROPERTY TO AN AUTHORIZED DISPOSAL LOCATION.

D. PRIOR TO PLACEMENT OF FILL OR BASE MATERIALS, ROLL THE SOIL.

E. WHERE UNSTABLE SOIL CONDITIONS ARE ENCOUNTERED, LINE THE GRUBBED AREAS WITH STABILIZER MAT PRIOR TO PLACEMENT OF FILL OR BASE MATERIAL.

3.03 INSTALLATION

A. THE SITE AND TURNAROUND AREAS SHALL BE AT THE SUB–BASE COURSE ELEVATION PRIOR TO FORMING FOUNDATIONS. GRADE OR FILL THE SITE AND ACCESS ROAD AS REQUIRED TO PRODUCE EVEN DISTRIBUTION OF SPOILS RESULTING FROM FOUNDATION EXCAVATIONS. THE RESULTING GRADE SHALL CORRESPOND WITH SAID SUB–BASE COURSE, ELEVATIONS ARE TO BE CALCULATED FORM FINISHED GRADES OR SLOPES INDICATED.

B. THE ACCESS ROAD SHALL BE BROUGHT TO BASE COURSE ELEVATION PRIOR TO FOUNDATION CONSTRUCTION.

C. DO NOT CREATE DEPRESSIONS WHERE WATER MAY POND.

D. THE CONTRACT INCLUDES ALL NECESSARY GRADING, BANKING, DITCHING AND COMPLETE SURFACE COURSE FOR ACCESS ROAD. ALL ROADS OR ROUTES UTILIZED FOR ACCESS TO PUBLIC THOROUGHFARE IS INCLUDED IN SCOPE OF WORK UNLESS OTHERWISE INDICATED.

E. WHEN IMPROVING AN EXISTING ACCESS ROAD, GRADE THE EXISTING ROAD TO REMOVE ANY ORGANIC MATTER AND SMOOTH THE SURFACE BEFORE PLACING FILL OR STONE.

F. PLACE FILL OR STONE IN 3" MAXIMUM LIFTS AND COMPACT BEFORE PLACING NEXT LIFT.

G. THE FINISH GRADE, INCLUDING TOP SURFACE COURSE, SHALL EXTEND A MINIMUM OF 12" BEYOND THE SITE FENCE AND SHALL COVER THE AREA AS INDICATED.

H. RIPRAP SHALL BE APPLIED TO THE SIDE SLOPES OF ALL FENCED AREAS, PARKING AREAS AND TO ALL OTHER SLOPES GREATER THAN 2:1.

I. RIPRAP SHALL BE APPLIED TO THE SIDES OF DITCHES OR DRAINAGE SWALES AS INDICATED ON PLANS.

J. RIPRAP ENTIRE DITCH FOR 6'–0" IN ALL DIRECTIONS AT CULVERT OPENINGS.

K. SEED, FERTILIZER AND STRAW COVER SHALL BE APPLIED TO ALL OTHER DISTURBED AREAS AND DITCHES, DRAINAGE, SWALES, NOT OTHERWISE RIP–RAPPED.

L. UNDER NO CIRCUMSTANCES SHALL DITCHES, SWALES OR CULVERTS BE PLACED SO THEY DIRECT WATER TOWARDS, OR PERMIT STANDING WATER IMMEDIATELY ADJACENT TO SITE. IF OWNER DESIGNS OR IF DESIGN ELEVATIONS CONFLICT WITH THIS GUIDANCE ADVISE THE OWNER IMMEDIATELY.

M. IF A DITCH LIES WITH SLOPE GREATER THAN TEN PERCENT, MOUND DIVERSIONARY HEADWALL IN THE DITCH AT CULVERT ENTRANCES. RIP–RAP THE UPSTREAM SIDE OF THE HEADWALL AS WELL AS THE DITCH FOR 6'–0" ABOVE THE CULVERT.

N. IF A DITCH LIES WITH SLOPES GREATER THAN TEN PERCENT, MOUND DIVERSIONARY HEADWALLS IN THE DITCH FOR 6'–0" ABOVE THE CULVERT ENTRANCE.

O. SEED AND FERTILIZER SHALL BE APPLIED TO SURFACE CONDITIONS WHICH WILL ENCOURAGE ROOTING. RAKE AREAS TO BE SEEDED TO EVEN THE SURFACE AND TO LOOSEN THE SOIL.

P. SOW SEED IN TWO DIRECTIONS IN TWICE THE QUANTITY RECOMMENDED BY THE SEED PRODUCER.

Q. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE GROWTH OF SEEDED AND LANDSCAPED AREAS BY WATERING UP TO THE POINT OF RELEASE FROM THE CONTRACT. CONTINUE TO REWORK BARE AREAS UNTIL COMPLETE COVERAGE IS OBTAINED.

3.04 FIELD QUALITY CONTROL

A. COMPACTION SHALL BE D–1557 FOR SITE WORK AND 95 % MAXIMUM DENSITY UNDER SLAB AREAS. AREAS OF SETTLEMENT WILL BE EXCAVATED AND REFILLED AT CONTRACTOR'S EXPENSE. REQUIRED. USE OF EROSION CONTROL MESH OR MULCH NET SHALL BE AN ACCEPTABLE ALTERNATIVE.

B. THE COMPACTION TEST RESULTS SHALL BE AVAILABLE PRIOR TO THE CONCRETE POUR.

3.05 PROTECTION

A. PROTECT SEEDED AREAS FORM EROSION BY SPREADING STRAW TO A UNIFORM LOOSE DEPTH OF 1"–2". STAKE AND TIE DOWN AS REQUIRED. USE OF EROSION CONTROL MESH OR MULCH NET SHALL BE AN ACCEPTABLE ALTERNATIVE.

B. ALL TREES PLACED IN CONJUNCTION WITH A LANDSCAPE CONTRACT SHALL BE WRAPPED, TIED WITH HOSE PROTECTED WIRE AND SECURED TO STAKES EXTENDING 2'–0" INTO THE GROUND ON FOUR SIDES OF THE TREE.

C. ALL EXPOSED AREAS SHALL BE PROTECTED AGAINST WASHOUTS AND SOIL EROSION. STRAW BALES SHALL BE PLACED AT THE INLET APPROACH TO ALL NEW OR EXISTING CULVERTS. REFER TO DETAILS ON DRAWINGS

SYMBOLS	ABBREVIATIONS
— — — G — — — G —	GROUND WIRE
— — — E — — — E —	ELECTRIC
— — — T — — — T —	TELEPHONE
—O/H— —O/H— —O/H— —O/H— —O/H—	OVERHEAD WIRE
——— — — ———	PROPERTY LINE
—X———X———X———	CHAIN LINK FENCE
A–1	ANTENNA MARK
(E)	EXISTING
(P)	PROPOSED DETAIL
DET # / SHT #	REFERENCE
	SURFACE ELEVATION

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SUBMITTALS

PROJECT NO: 7225.CT03XC020

NO	DATE	DESCRIPTION	BY
0	06/14/14	FOR COMMENT	DC
1	12/05/14	FOR CONSTRUCTION	DC

DATE	REVIEWED BY
12/5/14	

STATE OF CONNECTICUT
ANTONIO A. GUALTIERI
No. 25406
LICENSED
PROFESSIONAL ENGINEER

SITE NUMBER:
CT03XC020

SITE NAME:
OAK LANE CC, INC. TOWER

SITE ADDRESS:
1116 JOHNSON RD
WOODBRIDGE, CT 06525

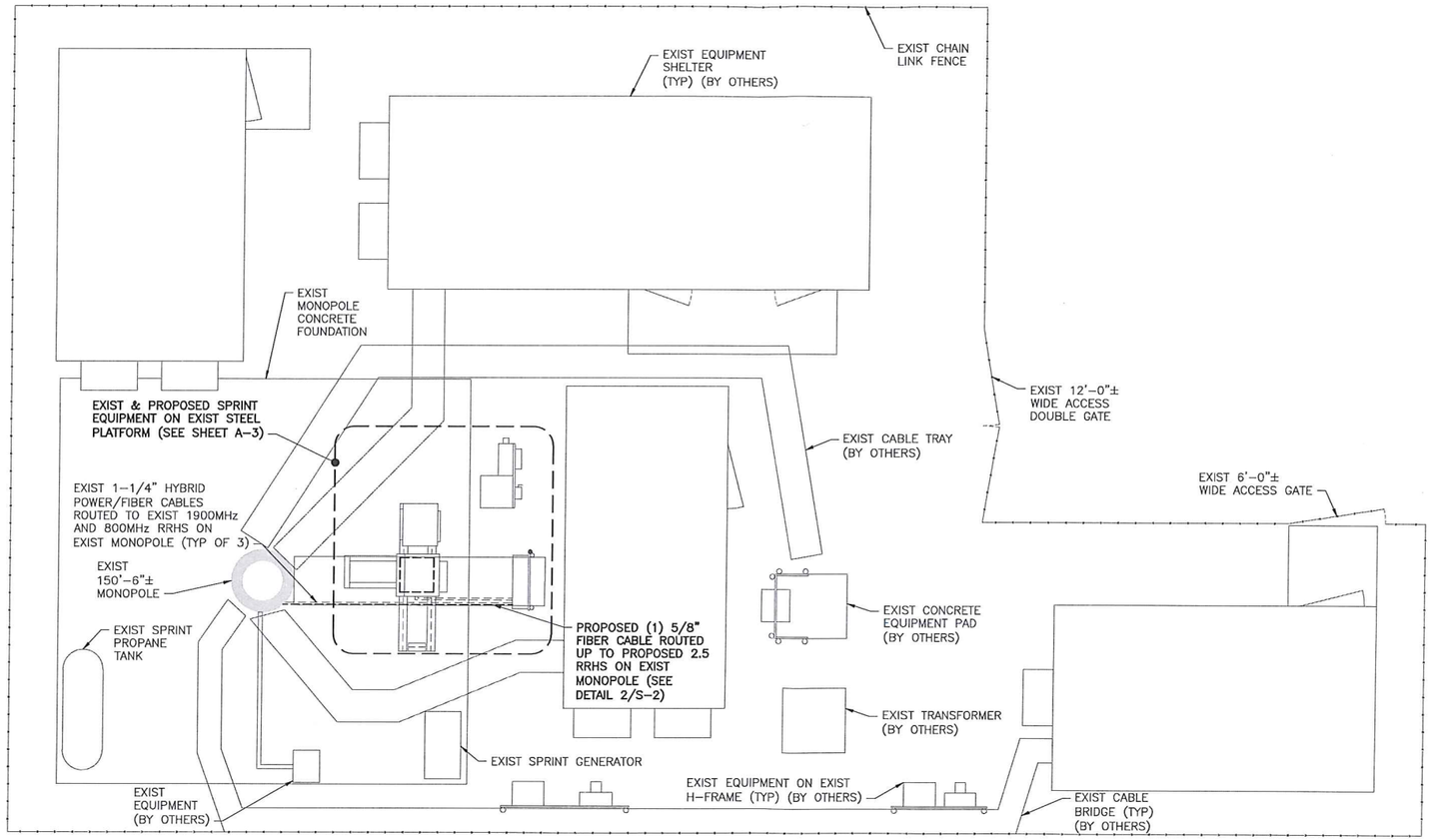
SHEET TITLE:
GENERAL NOTES

SHEET NO:
SP–2

NORTH NOTE:

NORTH SHOWN HAS BEEN ESTABLISHED USING THE USGS QUADRANGLE 7.5 MINUTE MAPS AND IS APPROXIMATE. VERIFY TRUE NORTH PRIOR TO INSTALLATION OF ANTENNAS.

APPROX. N



1 / A-2

1 / A-1 SITE PLAN

SCALE: 1/4" = 1'-0"

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12/5/14	TMQ

STATE OF CONNECTICUT SEAL ANTONIO A. GUALTIERI No. 25406 LICENSED PROFESSIONAL ENGINEER

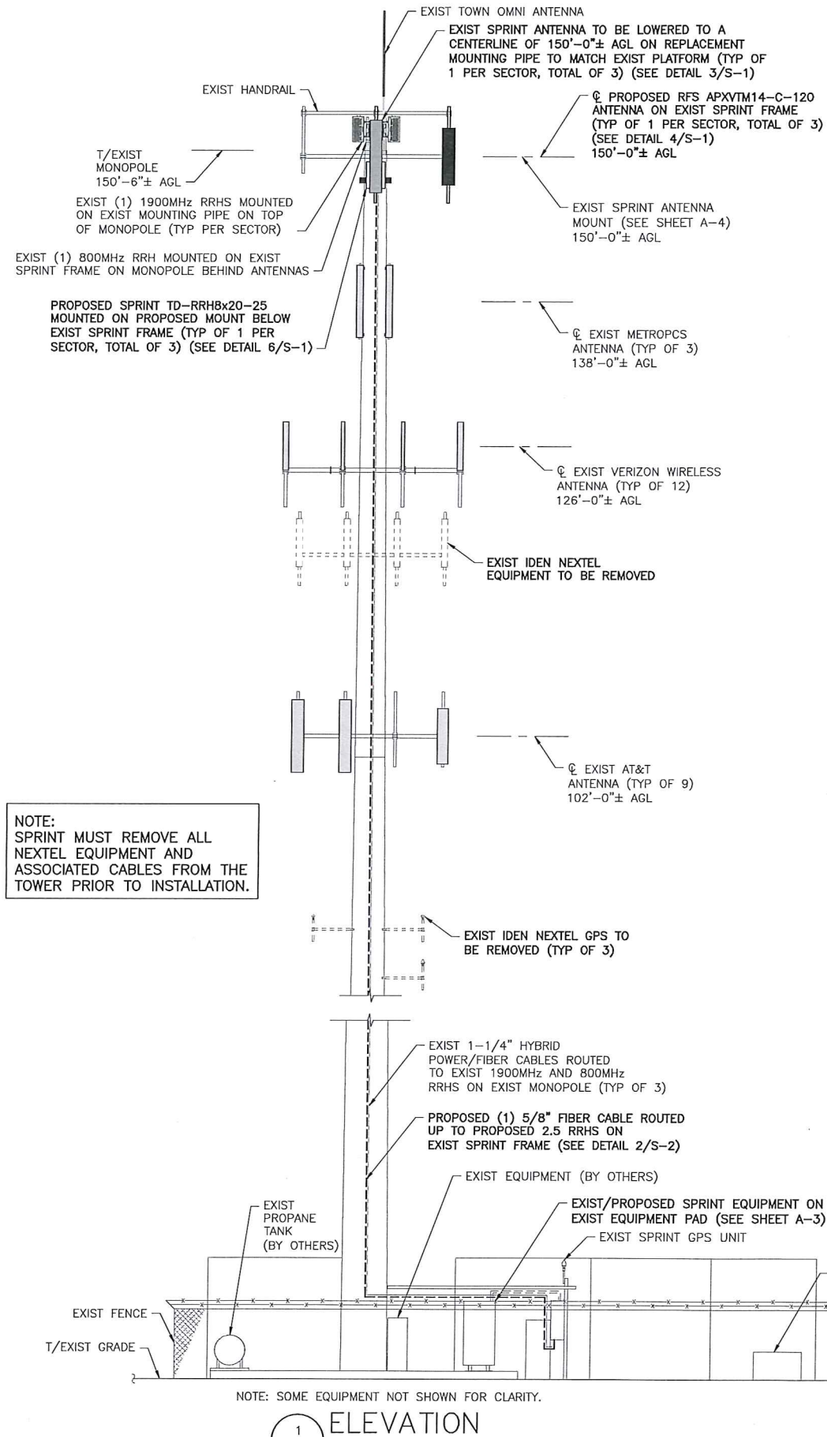
SITE NUMBER:
CT03XC020

SITE NAME:
OAK LANE CC, INC. TOWER

SITE ADDRESS:
1116 JOHNSON RD
WOODBRIDGE, CT 06525

SHEET TITLE:
SITE PLAN

SHEET NO:
A-1



NOTE: SOME EQUIPMENT NOT SHOWN FOR CLARITY.

1 / A-2 ELEVATION

SCALE: 3/16" = 1'-0"

NOTE:
SPRINT MUST REMOVE ALL NEXTEL EQUIPMENT AND ASSOCIATED CABLES FROM THE TOWER PRIOR TO INSTALLATION.

THE PROPOSED INSTALLATION, EXISTING MOUNTS AND EXISTING MONOPOLE SHALL BE ANALYZED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF CONNECTICUT (TO BE COORDINATED BY OTHERS).

THE EXISTING MOUNT HAS BEEN ANALYZED BY TECTONIC ENGINEERING AND FOUND TO BE ADEQUATE TO SUPPORT THE PROPOSED SPRINT UPGRADE ONCE THE PROPOSED MODIFICATIONS HAVE BEEN COMPLETED AS DETAILED IN THE STRUCTURAL ANALYSIS EVALUATION LETTER DATED 12/3/14.

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STATE OF CONNECTICUT
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No. 25406
LICENSED PROFESSIONAL ENGINEER

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SITE ADDRESS:
1116 JOHNSON RD
WOODBRIDGE, CT 06525

SHEET TITLE:
ELEVATION

SHEET NO:
A-2

NORTH NOTE:

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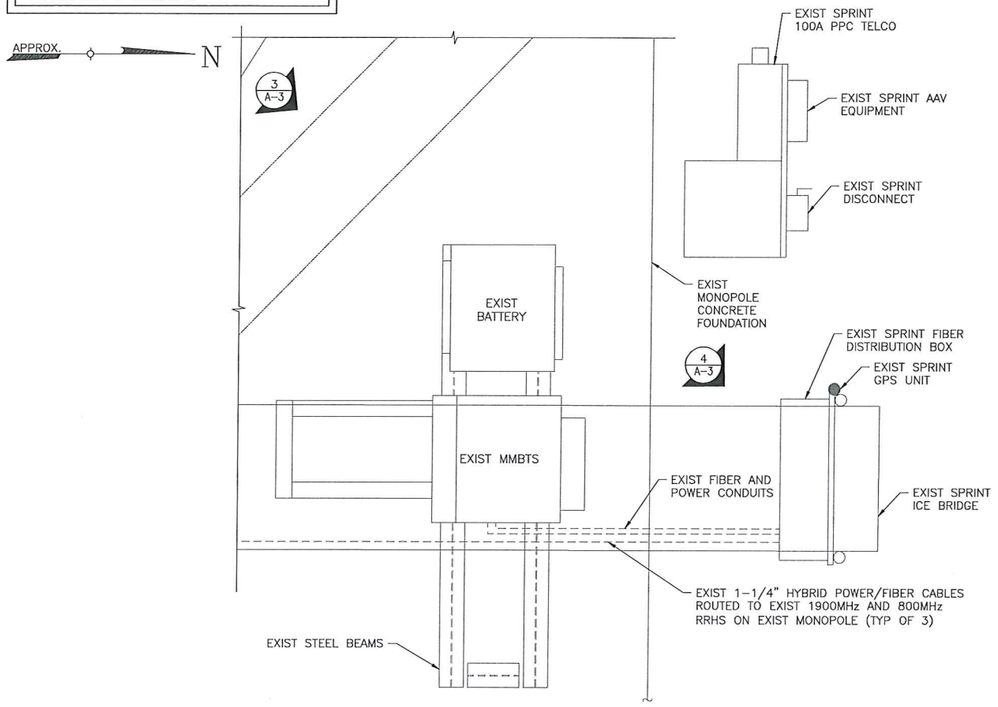
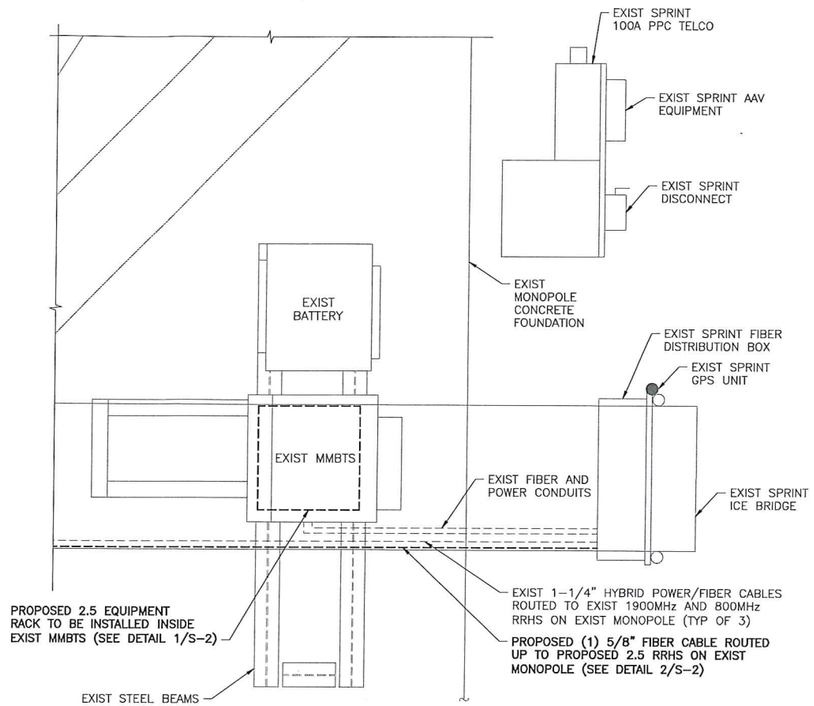


1 A-3 ENLARGED EQUIP. LAYOUT PLAN (EXIST)

SCALE: 3/4" = 1'-0"



2 A-3 ENLARGED EQUIP. LAYOUT PLAN (FINAL)

SCALE: 3/4" = 1'-0"

3 A-3 EXIST EQUIPMENT PAD

SCALE: NTS

4 A-3 EXIST FIBER DISTRIBUTION BOX

SCALE: NTS

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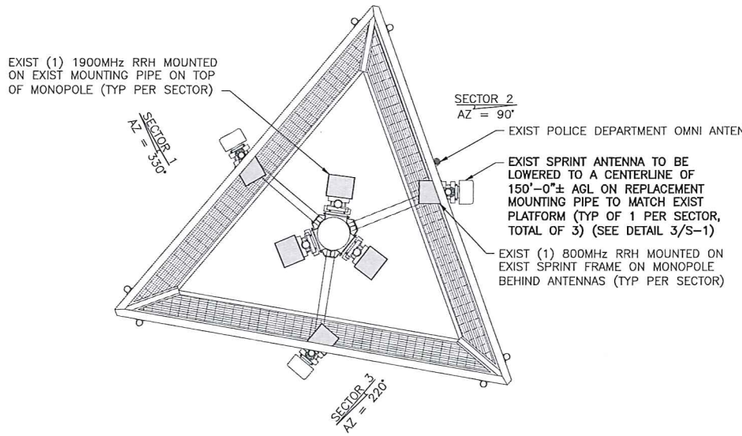
WOODBRIDGE, CT 06525

SHEET TITLE:
ENLARGED EQUIPMENT LAYOUT PLANS

SHEET NO:
A-3

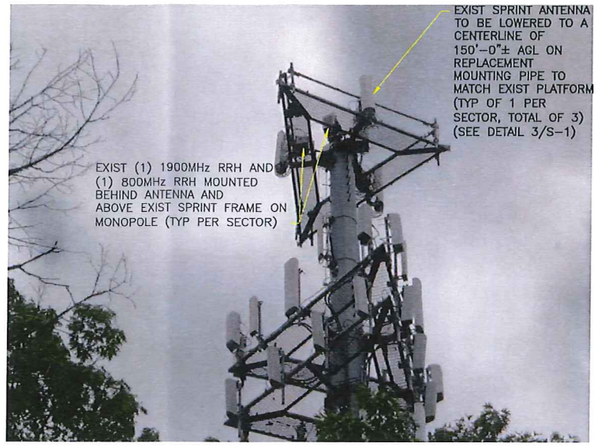
N

APPROX.

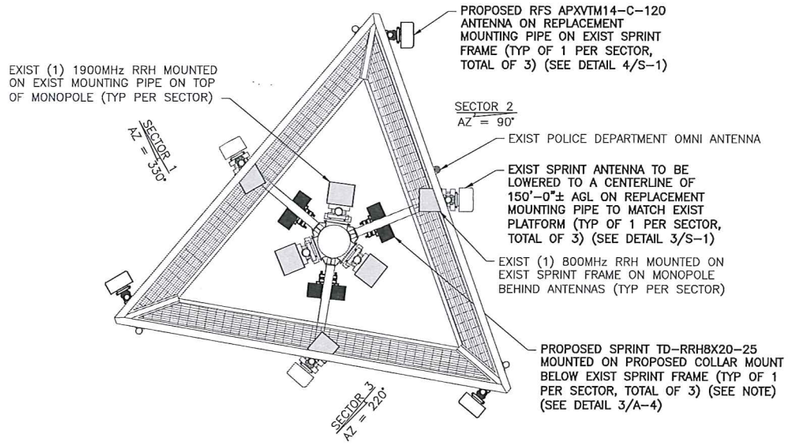


1 / A-4 ANTENNA LAYOUT PLAN (EXIST)

SCALE: 3/8" = 1'-0"



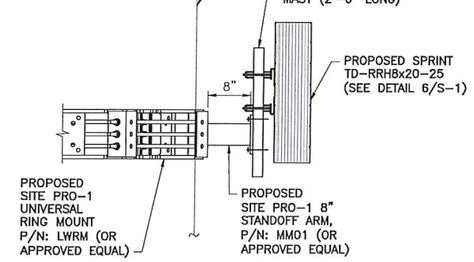
NOTE:
SPRINT MUST REMOVE ALL NEXTEL EQUIPMENT AND ASSOCIATED CABLES FROM THE TOWER PRIOR TO INSTALLATION.



NOTE: INSTALL PROPOSED COLLAR MOUNT AS NECESSARY TO AVOID CLIMBING PEGS.

2 / A-4 ANTENNA LAYOUT PLAN (FINAL)

SCALE: 3/8" = 1'-0"



3 / A-4 RRH MOUNTING DETAIL

SCALE: 1 1/2" = 1'-0"

ANTENNA DATA

Status	Exist (Proposed)	Proposed
Antenna Manufacturer	RFS-CEL WAVE	RFS-CEL WAVE
Antenna Model Number	APXVSPP18C-A20	APXVTM14-C-120
Number of Antennas	3	3
Antenna RAD Center	153' (150')	150'
Antenna Azimuth	330/90/220	330/90/220
Antenna RRH Model Number	1900MHz/800MHz RRHS	TD-RRH8x20-25
Number of RRH	3	3

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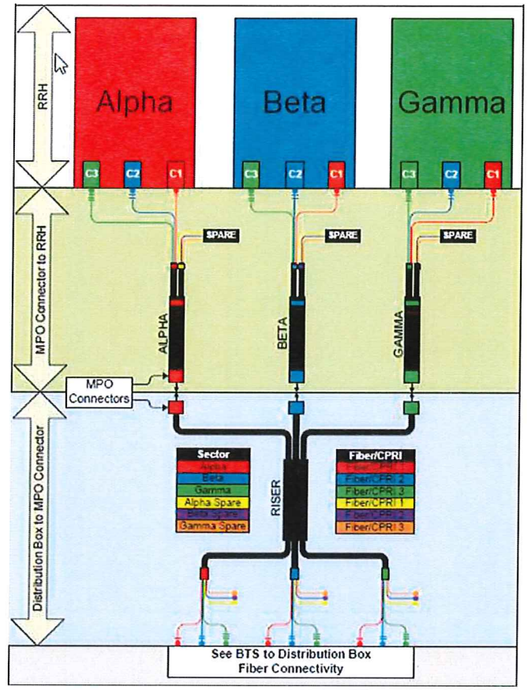
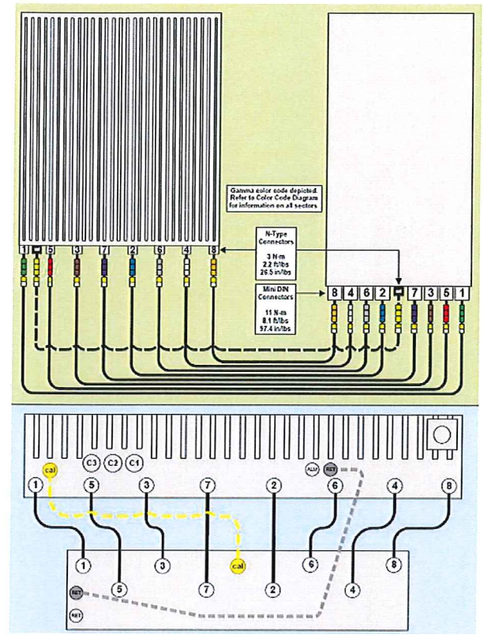
SHEET TITLE:
ANTENNA LAYOUT PLANS

SHEET NO:
A-4



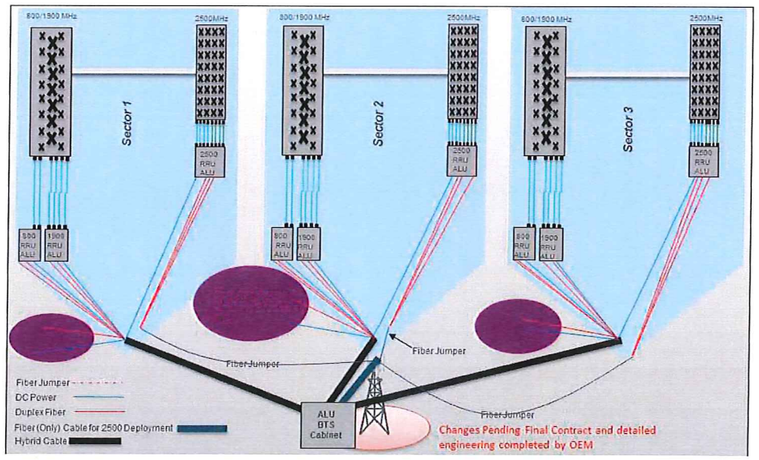
1 / A–5 2.5 CABLE COLOR CODING

SCALE: N.T.S.



2 / A–5 RRH CONNECTIVITY

SCALE: N.T.S.



3 / A–5 RAN WIRING

SCALE: N.T.S.

4 / A–5 CABLE SCENARIO

SCALE: N.T.S.

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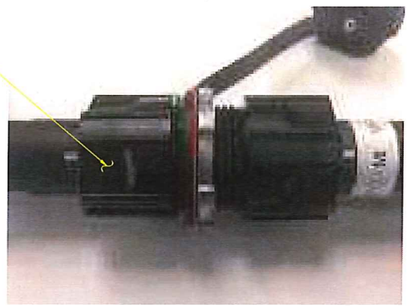
SITE ADDRESS:
1116 JOHNSON RD
WOODBRIDGE, CT 06525

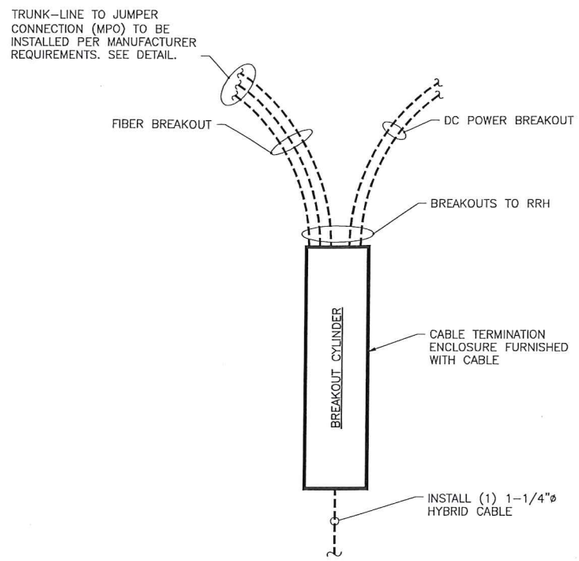
SHEET TITLE:
RAN WIRING DIAGRAM

SHEET NO:
A–5

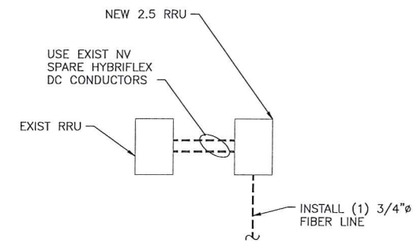
IMPORTANT!! LINE UP WHITE MARKINGS ON JUMPER AND RISER IP-MPO CONNECTOR. PUSH THE WHITE MARK ON THE JUMPER CONNECTOR FLUSH AGAINST THE RED SEAL ON THE RISER CONNECTION

IMPORTANT!! ROTATE THE BAYONET HOUSING CLOCKWISE UNTIL A CLICK SOUND IS HEARD TO ENSURE A GOOD CONNECTION





2.5 HYBRID CABLE W/FIBER & DC FEEDERS



FIBER ONLY TRUNK LINES

1 A-6 HYBRIFLEX RISER/JUMPER CONNECTION DETAILS

SCALE: N.T.S.

2 A-6 TRUNK LINE DETAILS (TYPICAL)

SCALE: N.T.S.

SPECIAL NOTES: CABLE MARKINGS AT RAD CENTER AND ALL WALL/BLDG. PENETRATIONS

- ALL COLOR CODE TAPE SHALL BE 3M-35 AND SHALL BE INSTALLED USING A MINIMUM OF (3) WRAPS OF TAPE.
- ALL COLOR BANDS INSTALLED AT THE TOWER TOP SHALL BE A MINIMUM OF 3" WIDE AND SHALL HAVE A MINIMUM OF 3/4" OF SPACING BETWEEN EACH COLOR.
- ALL COLOR BANDS INSTALLED AT OR NEAR THE GROUND MAY BE ONLY 3/4" WIDE. EACH TOP-JUMPER SHALL BE COLOR CORDED WITH (1) SET OF 3" WIDE BANDS.
- EACH MAIN COAX SHALL BE COLOR CODED WITH (1) SET OF 3" BANDS NEAR THE TOP-JUMPER CONNECTION AND WITH 3/4" COLOR BANDS JUST PRIOR TO ENTERING THE BTS OR TRANSMITTER BUILDING.
- ALL BOTTOM JUMPERS SHALL BE COLOR CODED WITH (1) SET OF 3/4" BANDS ON EACH END OF THE BOTTOM JUMPER.
- ALL COLOR CODES SHALL BE INSTALLED SO AS TO ALIGN NEATLY WITH ONE ANOTHER FROM SIDE-TO-SIDE.
- EACH COLOR BAND SHALL HAVE A MINIMUM OF (3) WRAPS AND SHALL BE NEATLY TRIMMED AND SMOOTHED OUT AS TO AVOID UNRAVELING.
- X-POLE ANTENNAS SHOULD USE "XX-1" FOR THE "+45" PORT, "XX-2" FOR THE "-45" PORT.
- COLOR BAND #4 REFERS TO THE FREQUENCY BAND: ORANGE=850, VIOLET=1900. USED ON JUMPERS ONLY.
- RF FEEDLINE SHALL BE IDENTIFIED WITH A METAL TAG (STAINLESS OR BRASS) AND STAMPED WITH THE SECTOR, ANTENNA POSITION, AND CABLE NUMBER.
- ANTENNAS MUST BE IDENTIFIED, USING THE SECTOR LETTER AND ANTENNA NUMBER, WITH A BLACK MARKER PRIOR TO INSTALLATION.

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SUBMITTALS

PROJECT NO: 7225.CT03XC020

NO	DATE	DESCRIPTION	BY
0	06/14/14	FOR COMMENT	DC
1	12/05/14	FOR CONSTRUCTION	DC

DATE	REVIEWED BY
12/5/14	TMO

STATE OF CONNECTICUT
ANTONIO A. GUALTIERI
No. 25406
LICENSED PROFESSIONAL ENGINEER

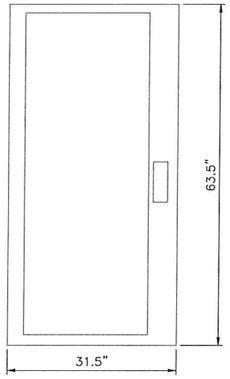
SITE NUMBER:
CT03XC020

SITE NAME:
OAK LANE CC, INC. TOWER

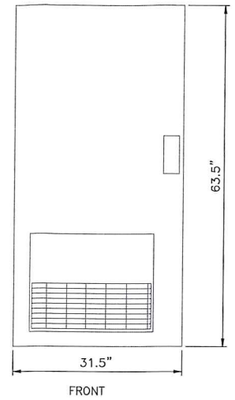
SITE ADDRESS:
1116 JOHNSON RD
WOODBRIDGE, CT 06525

SHEET TITLE:
CABLE DETAILS

SHEET NO:
A-6



9927 MMBTS MODULAR CELL
SPECIFICATIONS:
HEIGHT: 63.5"
WIDTH: 31.5"
DEPTH: 38.0"



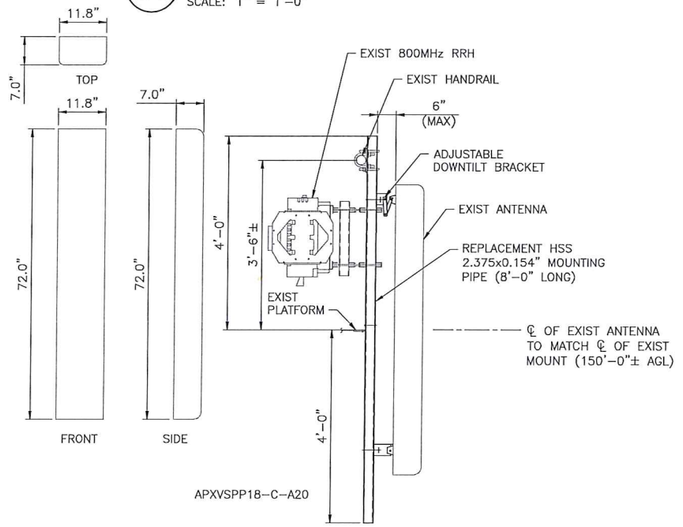
BATTERY
SPECIFICATIONS:
HEIGHT: 63.5"
WIDTH: 31.5"
DEPTH: 28.0"

1/S-1 (EXIST) MMBTS CABINET

SCALE: 1" = 1'-0"

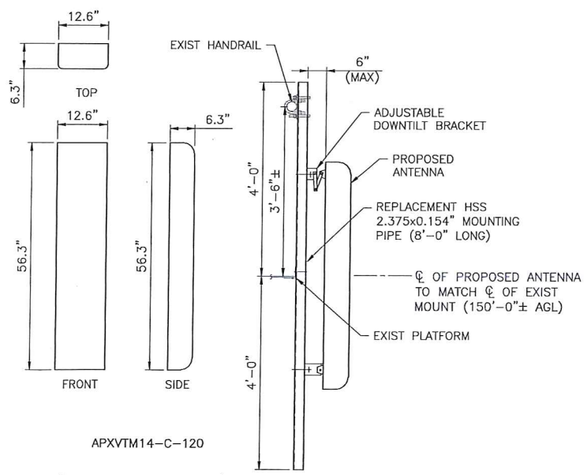
2/S-1 (EXIST) BATTERY CABINET

SCALE: 1" = 1'-0"



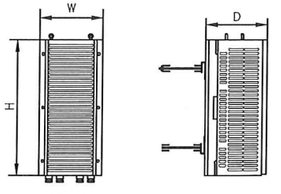
3/S-1 (EXIST) ANTENNA DETAILS

SCALE: 3/4"=1'-0"

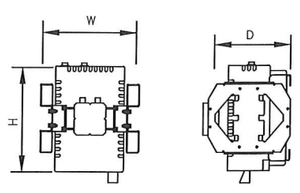


4/S-1 (PROPOSED) ANTENNA DETAIL

SCALE: 3/4"=1'-0"



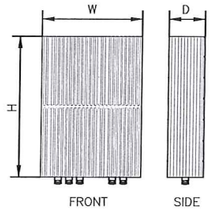
TYPE: 1900 MHz 4x45W
MODEL #: RRH 1900 4X45 65MHz
HEIGHT: 25.0"
WIDTH: 11.1"
DEPTH: 11.4"
WEIGHT: ±60 LBS.



TYPE: 800 MHz 2x50W
MODEL #: FD-RRH-2x50-800
HEIGHT: 19.7"
WIDTH: 13"
DEPTH: 10.8"
WEIGHT: ±53 LBS

5/S-1 (EXIST) RRH DETAILS

SCALE: 1 1/2"=1'-0"



TYPE: 2.5 RRH
MODEL #: TD-RRH8x20-25
HEIGHT: 26.1"
WIDTH: 18.6"
DEPTH: 6.7"
WEIGHT: ±70 LBS

6/S-1 (PROPOSED) RRH DETAIL

SCALE: N.T.S.

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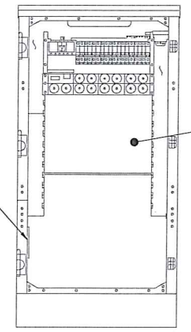
WOODBRIDGE, CT 06525

SHEET TITLE:
EQUIPMENT DETAILS

SHEET NO:
S-1

NOTE:
LOCATIONS SHOWN FOR INSTALLATION OF NEW EQUIPMENT IN EXISTING CABINET ARE APPROXIMATE. ACTUAL SPACE AVAILABLE TO BE VERIFIED IN FIELD ON A SITE BY SITE BASIS.



FRONT ELEVATION
(CABINET INTERIOR)

1 / S-2 MMBTS INTERIOR DETAIL

SCALE: N.T.S.

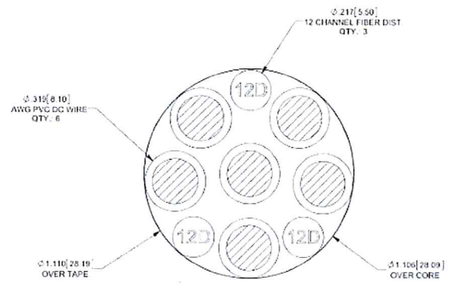
RFS HYBRIFLEX RISER CABLES SCHEDULE

Fiber Only (Existing DC Power)	Hybrid cable MN: HB058-M12-050F 12x multi-mode fiber pairs, Top: Outdoor protected connectors, Bottom:LC Connectors, 5/8 cable, 50ft	50 ft
	MN: HB058-M12-075F	75 ft
	MN: HB058-M12-100F	100 ft
	MN:HB058-M12-125F	125 ft
	MN:HB058-M12-150F	150 ft
	MN:HB058-M12-175F	175 ft
	MN:HB058-M12-200F	200 ft
8 AWG Power	Hybrid cable MN: HB114-08U3M12-050F 3x 8 AWG power pairs, 12x multi-mode fiber pairs, Outdoor rated connectors & LC Connectors, 1 1/4 cable, 50ft	50 ft
	MN: HB114-08U3M12-075F	75 ft
	MN: HB114-08U3M12-100F	100 ft
	MN: HB114-08U3M12-125F	125 ft
	MN: HB114-08U3M12-150F	150 ft
	MN: HB114-08U3M12-175F	175 ft
	MN: HB114-08U3M12-200F	200 ft
6 AWG Power	Hybrid cable MN: HB114-13U3M12-225F 3x 6 AWG power pairs, 12x multi-mode fiber pairs, Outdoor rated connectors & LC Connectors, 1 1/4 cable,225ft	225 ft
	MN: HB114-13U3M12-250F	250 ft
	MN: HB114-13U3M12-275F	275 ft
	MN: HB114-13U3M12-300F	300 ft
4 AWG Power	Hybrid cable MN: HB114-21U3M12-225F 3x 6 AWG power pairs, 12x multi-mode fiber pairs, Outdoor rated connectors & LC Connectors, 1 1/4 cable,225ft	325 ft
	MN: HB114-21U3M12-350F	350 ft
	MN: HB114-21U3M12-375F	375 ft

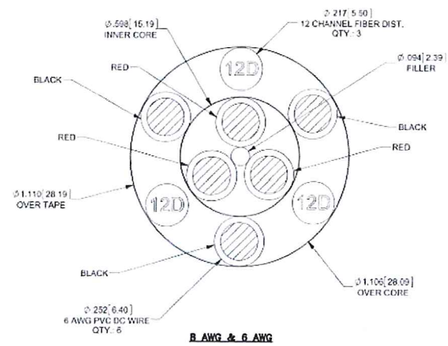
RFS HYBRIFLEX JUMPER CABLE SCHEDULE

Fiber Only	Hybrid Jumper cable MN: HBF012-M3-5F1 5 ft, 3x multi-mode fiber pairs, Outdoor & LC connectors, 1/2 cable	5 ft
	MN: HBF012-M3-10F1	10 ft
	MN: HBF012-M3-15F1	15 ft
	MN: HBF012-M3-20F1	20 ft
	MN: HBF012-M3-25F1	25 ft
	MN: HBF012-M3-30F1	30 ft
8 AWG Power	Hybrid Jumper cable MN: HBF058-08U1M3-5F1 5 ft, 1x 8 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 5/8 cable	5 ft
	MN: HBF058-08U1M3-10F1	10 ft
	MN: HBF058-08U1M3-15F1	15 ft
	MN: HBF058-08U1M3-20F1	20 ft
	MN: HBF058-08U1M3-25F1	25 ft
	MN: HBF058-08U1M3-30F1	30 ft
6 AWG Power	Hybrid Jumper cable MN: HBF058-13U1M3-5F1 5 ft, 1x 6 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 5/8 cable	5 ft
	MN: HBF058-13U1M3-10F1	10 ft
	MN: HBF058-13U1M3-15F1	15 ft
	MN: HBF058-13U1M3-20F1	20 ft
	MN: HBF058-13U1M3-25F1	25 ft
	MN: HBF058-13U1M3-30F1	30 ft
4 AWG Power	Hybrid Jumper cable MN: HBF078-21U1M3-5F1 5 ft, 1x 4 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 7/8 cable	5 ft
	MN: HBF078-21U1M3-10F1	10 ft
	MN: HBF078-21U1M3-15F1	15 ft
	MN: HBF078-21U1M3-20F1	20 ft
	MN: HBF078-21U1M3-25F1	25 ft
	MN: HBF078-21U1M3-30F1	30 ft

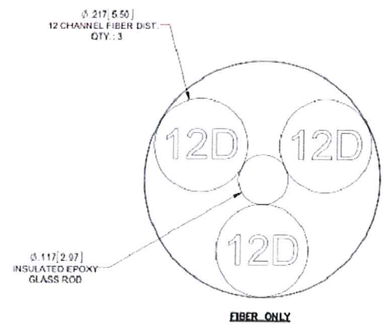
HYBRID CABLE DC CONDUCTOR SIZE GUIDELINE			
MANUF:	RFS		
CABLE	LENGTH	DC CONDUCTOR	CABLE DIAMETER
FIBER ONLY	VARIES	USE NV HYBRIFLEX	7/8"
HYBRIFLEX	<200'	8 AWG	1-1/4"
HYBRIFLEX	225-300'	6 AWG	1-1/4"
HYBRIFLEX	325-375'	4 AWG	1-1/4"



4 AWG



8 AWG & 6 AWG

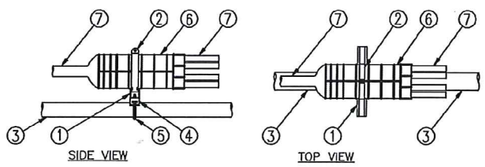


FIBER ONLY

2 / S-2 2.5 HYBRID CABLE X-SECTION AND DATA

SCALE: NTS

LEGEND:
1. P1000T-HG UNISTRUT, 12" LONG.
2. 6" PIPE HANGER.
3. EXISTING SUPPORT PIPE.
4. NEW STANDOFF BRACKET, ANDREW PART# 30848-4.
5. NEW ROUND MEMBER ADAPTER SIZED FOR EXISTING PIPE SUPPORT.
6. BREAKOUT UNIT.
7. CABLE.



3 / S-2 MEDUSA HEAD DETAIL

SCALE: NTS

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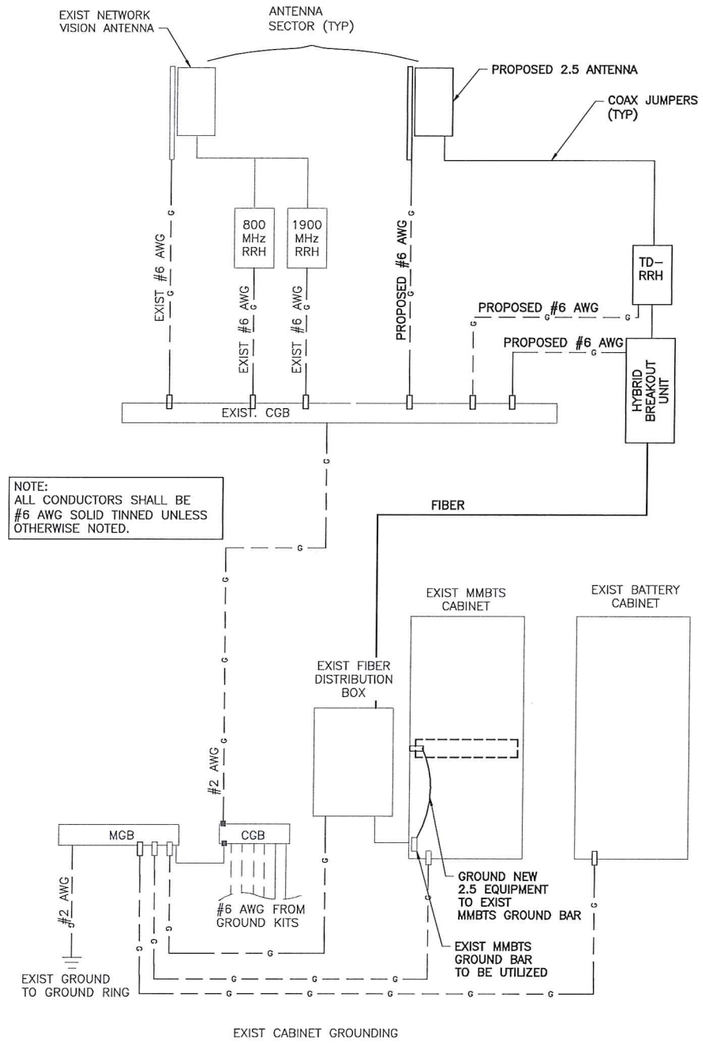
SITE ADDRESS:
1116 JOHNSON RD
WOODBRIDGE, CT 06525

SHEET TITLE:
EQUIPMENT SCHEMATIC DETAILS

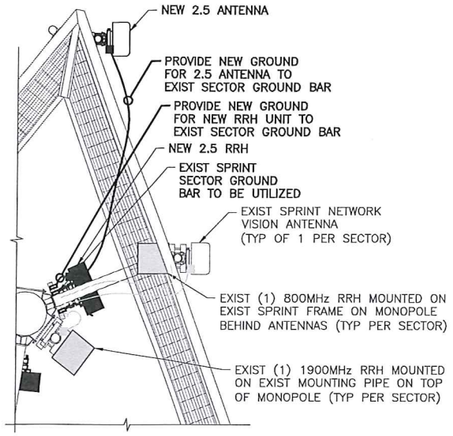
SHEET NO:
S-2



EXIST CABINET GROUNDING

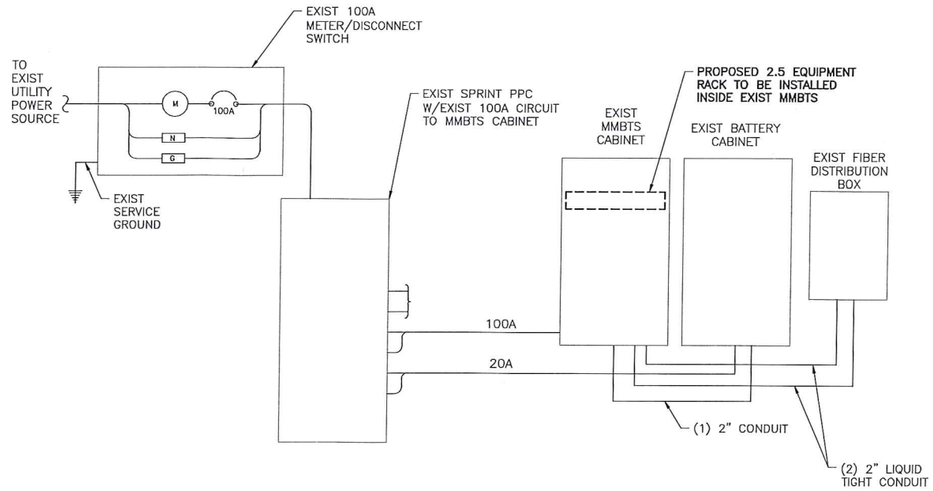
1 / E-1 TYPICAL GROUNDING ONE LINE DIAGRAM

SCALE: NTS



2 / E-1 TYPICAL ANTENNA GROUNDING PLAN

SCALE: NTS



3 / E-1 TYPICAL ELECTRICAL & TELCO PLAN

SCALE: NTS

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OAK LANE CC, INC. TOWER

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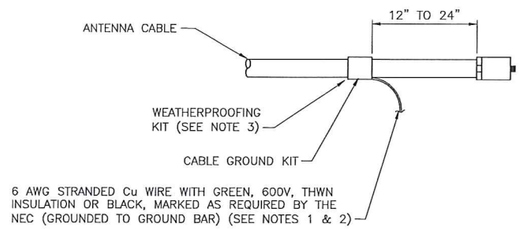
1116 JOHNSON RD
WOODBRIDGE, CT 06525

SHEET TITLE:
ELECTRICAL & GROUNDING PLANS

SHEET NO:
E-1



CONNECTION OF CABLE GROUND KIT TO ANTENNA CABLE

NOTES:

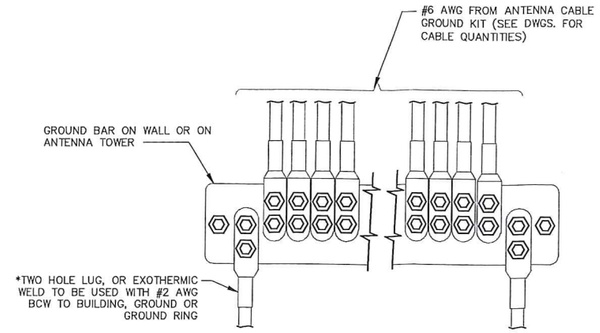
DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.

GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.

WEATHER PROOFING SHALL BE (TYPE AND PART NUMBER) AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER AND APPROVED BY CONTRACTOR.

1 / E-2 CABLE GROUNDING KIT DETAIL

SCALE: N.T.S.



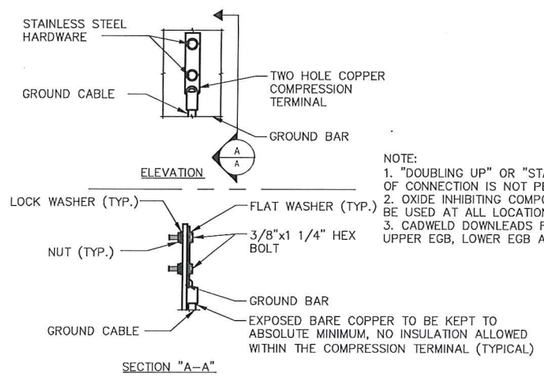
* – GROUND BARS AT THE BOTTOM OF TOWERS/MONOPOLES SHALL ONLY USE EXOTHERMIC WELDS.

– ATTACH "DO NOT DISCONNECT" LABELS TO GROUND BARS. CAN USE BRASS TAG "DO NOT DISCONNECT" AT EACH HYBRID GROUND POINT OR BACK–A–LITE PLATE LABEL ON GROUND BAR.

– CONNECT SEQUENCE– BOLT/WASHER/NO–OX/GROUND BAR/NO–OX/WASHER/LOCK–WASHER/NUT. THIS IS REPEATED FOR EACH LUG CONNECTION POINT.

4 / E-2 ANTENNA GROUND BAR DETAIL

SCALE: NTS

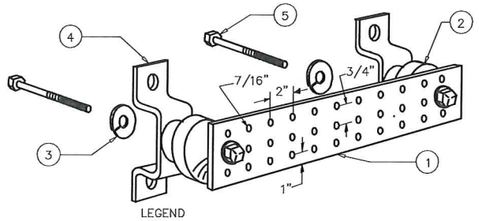


SECTION "A–A"

NOTE:
1. "DOUBLING UP" OR "STACKING" OF CONNECTION IS NOT PERMITTED.
2. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS.
3. CADWELD DOWNLEADS FROM UPPER EGB, LOWER EGB AND MGB.

2 / E-2 GROUNDING BAR CONN. DETAIL

SCALE: NTS



LEGEND

1– COPPER TINNED GROUND BAR, 1/4"X 4"X 20", OR OTHER LENGTH AS REQUIRED, HOLE CENTERS TO MATCH NEMA DOUBLE LUG CONFIGURATION
2– INSULATORS, NEWTON INSTRUMENT CAT. NO. 3061–4 OR EQUAL
3– 5/8" LOCKWASHERS OR EQUAL
4– WALL MOUNTING BRACKET, NEWTON INSTRUMENT CO. CAT NO. A–6056 OR EQUAL
5– 5/8–11 X 1" H.H.C.S.BOLTS

NOTE:
ALL BOLTS, NUTS, WASHERS AND LOCK WASHERS SHALL BE 18–8 STAINLESS STEEL.

3 / E-2 GROUNDING BAR DETAIL

SCALE: NTS

GROUNDING NOTES:

1. GROUNDING SHALL BE IN ACCORDANCE WITH NEC ARTICLE 250–GROUNDING AND BONDING.

2. ALL GROUND WIRES SHALL BE #2 AWG UNLESS NOTED OTHERWISE.

3. ALL GROUNDING WIRES SHALL PROVIDE A STRAIGHT, DOWNWARD PATH TO GROUND WITH GRADUAL BENDS AS REQUIRED. GROUND WIRES SHALL NOT BE LOOPED OR SHARPLY BENT.

4. EACH EQUIPMENT CABINET SHALL BE CONNECTED TO THE MASTER ISOLATION GROUND BAR (MGB) WITH #2 AWG INSULATED STRANDED COPPER WIRE. EQUIPMENT CABINETS WALL HAVE (2) CONNECTIONS.

5. PROVIDE DEDICATED #2 AWG COPPER GROUND WIRE FROM EACH ANTENNA MOUNTING PIPE TO ASSOCIATED CIGBE.

6. THE CONTRACTOR SHALL VERIFY THAT THE EXISTING GROUND BARS HAVE ENOUGH SPACE/HOLES FOR ADDITIONAL TWO HOLE LUGS.

7. ALL CONDUITS SHALL BE RIGID GALVANIZED STEEL AND SHALL BE PROVIDED WITH GROUNDING BUSHINGS.

8. PROVIDE GROUND CONNECTIONS FOR ALL METALLIC STRUCTURES, ENCLOSURES, RACEWAYS AND OTHER CONDUCTIVE ITEMS ASSOCIATED WITH THE INSTALLATION OF CARRIER'S EQUIPMENT.

9. WHEN CABLE LENGTH IS OVER 20' THE MANUFACTURERS GROUND KIT MUST BE INSTALLED PER THE MANUFACTURERS SPECIFICATIONS.

10. REFER TO "ANTI–THEFT UPDATE TO SPRINT GROUNDING 082412.PDF" FOR GUIDELINE TO SUSPECTED OR ACTUAL THEFT OF GROUNDING.

11. HOME RUN GROUNDS ARE NOT APPROVED BY CROWN CASTLE CONSTRUCTION STANDARDS AND THAT ANTENNA BUSS BARS SHOULD BE INSTALLED DIRECTLY TO TOWER STEEL WITHOUT INSULATORS OR DOWN CONDUCTORS.

PROTECTIVE GROUNDING SYSTEM GENERAL NOTES:

1. AT ALL TERMINATIONS AT EQUIPMENT ENCLOSURES, PANEL, AND FRAMES OF EQUIPMENT AND WHERE EXPOSED FOR GROUNDING. CONDUCTOR TERMINATION SHALL BE PERFORMED UTILIZING TWO HOLE BOLTED TONGUE COMPRESSION TYPE LUGS WITH STAINLESS STEEL SELF–TAPPING SCREWS.

2. ALL CLAMPS AND SUPPORTS USED TO SUPPORT THE GROUNDING SYSTEM CONDUCTORS AND PVC CONDUITS SHALL BE PVC TYPE (NON CONDUCTIVE). DO NOT USE METAL BRACKETS OR SUPPORTS WHICH WOULD FORM A COMPLETE RING AROUND ANY GROUNDING CONDUCTOR.

3. ALL GROUNDING CONNECTIONS SHALL BE COATED WITH A COPPER SHIELD ANTI–CORROSIVE AGENT SUCH AS T&B KOPR SHIELD. VERIFY PRODUCT WITH PROJECT MANAGER.

4. ALL BOLTS, WASHERS, AND NUTS USED ON GROUNDING CONNECTIONS SHALL BE STAINLESS STEEL.

5. INSTALL GROUND BUSHING ON ALL METALLIC CONDUITS AND BOND TO THE EQUIPMENT GROUND BUS IN THE PANEL BOARD.

6. GROUND ANTENNA BASES, FRAMES, CABLE RACKS, AND OTHER METALLIC COMPONENTS WITH #2 INSULATED TINNED STRANDED COPPER GROUNDING CONDUCTORS AND CONNECT TO INSULATED SURFACE MOUNTED GROUND BARS. CONNECTION DETAILS SHALL FOLLOW MANUFACTURER'S SPECIFICATIONS FOR GROUNDING.

7. GROUND HYBRID CABLE SHIELD AT BOTH ENDS USING MANUFACTURER'S GUIDELINES.

ELECTRICAL AND GROUNDING NOTES

1. ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE NATIONAL ELECTRICAL CODE (NEC) AS WELL AS APPLICABLE STATE AND LOCAL CODES.
2. ALL ELECTRICAL ITEMS SHALL BE U.L. APPROVED OR LISTED AND PROCURED PER SPECIFICATION REQUIREMENTS.
3. ELECTRICAL AND TELCO WIRING OUTSIDE A BUILDING AND EXPOSED TO WEATHER SHALL BE IN WATER TIGHT GALVANIZED RIGID STEEL CONDUITS OR SCHEDULE 80 PVC (AS PERMITTED BY CODE) AND WHERE REQUIRED IN LIQUID TIGHT FLEXIBLE METAL OR NONMETALLIC CONDUITS.
4. BURIED CONDUIT SHALL BE SCHEDULE 40 PVC.
5. ELECTRICAL WIRING SHALL BE COPPER WITH TYPE XHHW, THWN, OR THNN INSULATION.
6. RUN TELCO CONDUIT OR CABLE BETWEEN TELEPHONE UTILITY DEMARCATION POINT AND PROJECT OWNER CELL SITE TELCO CABINET AND BTS CABINET AS INDICATED ON THIS DRAWING PROVIDE FULL LENGTH PULL ROPE IN INSTALLED TELCO CONDUIT. PROVIDE GREENLEE CONDUIT MEASURING TAPE AT EACH END.
7. WHERE CONDUIT BETWEEN BTS AND PROJECT OWNER CELL SITE PPC AND BETWEEN BTS AND PROJECT OWNER CELL SITE TELCO SERVICE CABINET ARE UNDERGROUND USE PVC, SCHEDULE 40 CONDUIT. ABOVE THE GROUND PORTION OF THESE CONDUITS SHALL BE PVC CONDUIT.
8. ALL EQUIPMENT LOCATED OUTSIDE SHALL HAVE NEMA 3R ENCLOSURE.
9. GROUNDING SHALL COMPLY WITH NEC ART. 250.
10. GROUND HYBRID CABLE SHIELDS AT 3 LOCATIONS USING MANUFACTURER'S HYBRID CABLE GROUNDING KITS SUPPLIED BY PROJECT OWNER.
11. USE #2 COPPER STRANDED WIRE WITH GREEN COLOR INSULATION FOR ABOVE GRADE GROUNDING (UNLESS OTHERWISE SPECIFIED) AND #2 SOLID TINNED BARE COPPER WIRE FOR BELOW GRADE GROUNDING AS INDICATED ON THE DRAWING.
12. ALL GROUND CONNECTIONS TO BE BURNDY HYGROUND COMPRESSION TYPE CONNECTORS OR CADWELD EXOTHERMIC WELD. DO NOT ALLOW BARE COPPER WIRE TO BE IN CONTACT WITH GALVANIZED STEEL.
13. ROUTE GROUNDING CONDUCTORS ALONG THE SHORTEST AND STRAIGHTEST PATH POSSIBLE, EXCEPT AS OTHERWISE INDICATED. GROUNDING LEADS SHOULD NEVER BE BENT AT RIGHT ANGLE. ALWAYS MAKE AT LEAST 12" RADIUS BENDS. #2 WIRE CAN BE BENT AT 6" RADIUS WHEN NECESSARY. BOND ANY METAL OBJECTS WITHIN 6 FEET OF PROJECT OWNER EQUIPMENT OR CABINET TO MASTER GROUND BAR OR GROUNDING RING.
14. CONNECTIONS TO GROUND BARS SHALL BE MADE WITH TWO HOLE COMPRESSION TYPE COPPER LUGS. APPLY OXIDE INHIBITING COMPOUND TO ALL LOCATIONS.
15. APPLY OXIDE INHIBITING COMPOUND TO ALL COMPRESSION TYPE GROUND CONNECTIONS.
16. BOND ANTENNA MOUNTING BRACKETS, HYBRID CABLE GROUND KITS, AND RRHs TO EGB PLACED NEAR THE ANTENNA LOCATION.
17. BOND ANTENNA EGB'S AND MGB TO GROUND RING.
18. CONTRACTOR SHALL TEST COMPLETED GROUND SYSTEM AND RECORD RESULT FOR PROJECT CLOSE–OUT DOCUMENTATION. 5 OHMS MINIMUM RESISTANCE REQUIRED.
19. CONTRACTOR SHALL CONDUCT ANTENNA, HYBRID CABLES, GPS COAX AND RRH RETURN–LOSS AND DISTANCE– TO–FAULT MEASUREMENTS (SWEEP TESTS) AND RECORD RESULTS FOR PROJECT CLOSE OUT.
20. CONTRACTOR SHALL CHECK CAPACITY OF EXISTING SERVICE & PANEL ON SITE TO DETERMINE IF CAPACITY EXISTS TO ACCOMMODATE THE ADDED LOAD OF THIS PROJECT. ADVISE ENGINEER OF ANY DISCREPANCY.
21. LOCATION OF ALL OUTLET, BOXES, ETC, AND THE TYPE OF CONNECTION (PLUG OR DIRECT) SHALL BE CONFIRMED WITH THE OWNER'S REPRESENTATIVE PRIOR TO ROUGH–IN.
22. ELECTRICAL CHARACTERISTICS OF ALL EQUIPMENT (NEW AND EXISTING) SHALL BE FIELD VERIFIED WITH THE OWNERS REPRESENTATIVE AND EQUIPMENT SUPPLIER PRIOR TO ROUGH–IN OF CONDUIT AND WIRE. ALL EQUIPMENT SHALL BE PROPERLY CONNECTED ACCORDING TO THE NAMEPLATE DATA FURNISHED ON THE EQUIPMENT.

Sprint

2.5 EQUIPMENT DEPLOYMENT
6580 SPRINT PARKWAY
OVERLAND PARK, KANSAS 66251

CROWN CASTLE

TECTONIC • PLANNING • ENGINEERING • SURVEYING • CONSTRUCTION MANAGEMENT

TECTONIC Engineering & Surveying Consultants P.C.
1279 Route 300
Newburgh, NY 12550
Phone: (845) 567–6656
Fax: (845) 567–8703
www.tectonicengineering.com

SUBMITTALS

PROJECT NO: 7225.CT03XC020

NO	DATE	DESCRIPTION	BY
0	06/14/14	FOR COMMENT	DC
1	12/05/14	FOR CONSTRUCTION	DC

DATE	REVIEWED BY
12/5/14	JMP

STATE OF CONNECTICUT
ANTONIO A. GUATTIERI
No. 25406
LICENSED
PROFESSIONAL ENGINEER

SITE NUMBER:
CT03XC020

SITE NAME:
OAK LANE CC, INC. TOWER

SITE ADDRESS:
1116 JOHNSON RD
WOODBRIDGE, CT 06525

SHEET TITLE:
GROUNDING DETAILS & NOTES

SHEET NO:
E–2

PAUL J. FORD AND COMPANY
STRUCTURAL ENGINEERS
250 East Broad Street • Suite 600 • Columbus, Ohio 43215-3708

Date: **June 16, 2014**

Marianne Dunst
Crown Castle
3530 Toringdon Way, Suite 300
Charlotte, NC 28277
704.405.6580

Paul J. Ford and Company
250 E. Broad Street, Suite 600
Columbus, OH 43215
614.221.6679
jmeinerding@pjfweb.com

Subject: **Structural Analysis Report**

Carrier Designation:	***Sprint PCS* Co-Locate**	Scenario 2.5A
	Carrier Site Number:	CT03XC020
	Carrier Site Name:	N/A
Crown Castle Designation:	**Crown Castle BU Number:**	876315
	Crown Castle Site Name:	OAK LANE CC, INC. TOWER (SSUSA
	Crown Castle JDE Job Number:	288234
	Crown Castle Work Order Number:	773034
	Crown Castle Application Number:	245536 Rev. 1
Engineering Firm Designation:	**Paul J. Ford and Company Project Number:**	37513-2197.002.7805
Site Data:	**1027 Racebrook Road, Woodbridge, New Haven County, CT** **Latitude *41° 19' 0.6"*, Longitude *-73° 0' 41.7"*** **150 Foot - Monopole Tower**	

Dear Marianne Dunst,

Paul J. Ford and Company is pleased to submit this **"Structural Analysis Report"** to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 657330, in accordance with application 245536, revision 1.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC11: Existing + Reserved + Proposed Equipment **Sufficient Capacity**
Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

The structural analysis was performed for this tower in accordance with the requirements of the 2005 Connecticut Building Code and the TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 80 mph with no ice, 37.6 mph with 0.75 inch ice thickness and 50 mph under service loads.

We at *Paul J. Ford and Company* appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:

Joey Meinerding, E.I.
Structural Designer

STATE OF CONNECTICUT
JOSEPH PACHICARAH JACOBS
No. PEN 22[illegible]
LICENSED
PROFESSIONAL ENGINEER

tnxTower Report - version 6.1.4.1

JUN 16 2014

PAUL J. FORD AND COMPANY
STRUCTURAL ENGINEERS
250 East Broad Street • Suite 600 • Columbus, Ohio 43215-3708

Date: **June 16, 2014**

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jmeinerding@pjfweb.com

Subject: Structural Analysis Report

Carrier Designation:	***Sprint PCS* Co-Locate**	Scenario 2.5A
	Carrier Site Number:	CT03XC020
	Carrier Site Name:	N/A
Crown Castle Designation:	**Crown Castle BU Number:**	876315
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Respectfully submitted by:

Joey Meinerding, E.I.
Structural Designer

TABLE OF CONTENTS

1) INTRODUCTION

This tower is a 150 ft. monopole tower designed by SUMMIT in February of 1998. The tower was originally designed for a wind speed of 90 mph per TIA/EIA-222-F.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of the 2005 Connecticut Building Code and the TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 80 mph with no ice, 37.6 mph with 0.75 inch ice thickness and 50 mph under service loads.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
150.0	153.0	3	alcatel lucent	TD-RRH8x20-25	1	1-1/4	--
		3	rfs celwave	APXVTM14-C-120 w/ Mount Pipe			

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
150.0	156.0	1	rfs celwave	201-7	4	1-1/4	1
	153.0	3	rfs celwave	APXVSPP18-C-A20 w/ Mount Pipe			
	150.0	3	alcatel lucent	1900MHz RRH (65MHz)			
		3	alcatel lucent	800 EXTERNAL NOTCH FILTER			
		3	alcatel lucent	800MHZ RRH			
		9	rfs celwave	ACU-A20-N			
		1	tower mounts	Handrail Kit [NA 507-1]			
		1	tower mounts	Platform Mount [LP 403-1]			
138.0	138.0	3	rfs celwave	APXV18-206517S-C w/ Mount Pipe	6	1-5/8	1
		1	tower mounts	Pipe Mount [PM 602-3]			
126.0	126.0	1	rfs celwave	TMA-DB-T1-6Z-8AB-0Z	--	--	2
		1	tower mounts	Side Arm Mount [SO 102-1]			
124.0	126.0	6	decibel	DB844H90E-SX w/ Mount Pipe	--	--	3
		6	decibel	DB948F85E-M w/ Mount Pipe			
		3	alcatel lucent	RRH2X40-AWS	1	1-5/8	2
		1	antel	BXA-70080/4CF w/ Mount Pipe			
		2	antel	BXA-80063/4CF w/ Mount Pipe			
		3	powerwave technologies	P65.16.XL.2 w/ Mount Pipe			
		6	rfs celwave	FD9R6004/2C-3L			
		3	rymsa wireless	MG D3-800TV w/ Mount Pipe			
		3	rymsa wireless	MG D3-800Tx w/ Mount Pipe			
		1	gps	GPS_A	1	1/2	1
	124.0	1	tower mounts	Platform Mount [LP 403-1]	12	1-5/8	
117.0	118.0	12	decibel	DB844H90E-XY w/ Mount Pipe	12	1-1/4	4
	117.0	1	tower mounts	Platform Mount [LP 403-1]			
102.0	107.0	1	gps	GPS_A	1 2 1 12	3/8 5/8 1/2 1-5/8	1
	102.0	3	ericsson	RRUS-11			
		3	powerwave technologies	7770.00 w/ Mount Pipe			
		6	powerwave technologies	P65-16-XLH-RR w/ Mount Pipe			
		6	powerwave technologies	TT19-08BP111-001			
		1	raycap	DC6-48-60-18-8F			
		1	tower mounts	Platform Mount [LP 403-1]			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
86.0	87.0	2	radiall larsen	GPS0015	2	1/2	1
	86.0	2	tower mounts	Side Arm Mount [SO 701-1]			
82.0	82.0	1	tower mounts	Side Arm Mount [SO 701-1]	1	1/2	1
	81.0	1	lucent	KS24019-L112A			

Notes:
1) Existing Equipment
2) Reserved Equipment
3) Equipment To Be Removed
4) iDEN equipment to be removed. Not considered in analysis.

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	CHA, 5835.07.15, 12/17/1996	2134233	CCISITES
4-POST-MODIFICATION INSPECTION	B&T, 79751, 04/03/2009	2414121	CCISITES
4-POST-MODIFICATION INSPECTION	TEP, 25587, 11/21/2013	4137621	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Summit/PJF, 2249/29297-080, 02/23/1998	2112237	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Summit/PJF, 2249/29297-080, 02/23/1998	2134236	CCISITES

3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

1) Tower and structures were built in accordance with the manufacturer's specifications.
2) The tower and structures have been maintained in accordance with the manufacturer's specification.
3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
4) Monopole was reinforced in conformance with the referenced modification drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Paul J. Ford and Company should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	150 - 102.5	Pole	TP30.314x22x0.25	1	-7.95	1136.03	65.6	Pass
L2	102.5 - 73.5	Pole	TP34.8901x29.1576x0.3125	2	-15.62	1808.81	89.4	Pass
L3	73.5 - 62	Pole	TP36.903x34.8901x0.3963	3	-17.02	2301.21	78.0	Pass
L4	62 - 56.25	Pole	TP37.2844x35.279x0.375	4	-19.86	2316.95	90.1	Pass
L5	56.25 - 38.75	Pole	TP40.3473x37.2844x0.4498	5	-24.19	2925.48	86.0	Pass
L6	38.75 - 37.5	Pole	TP40.5661x40.3473x0.4755	6	-24.52	3088.64	82.5	Pass
L7	37.5 - 8.75	Pole	TP44.8484x40.5661x0.5282	7	-33.27	3802.81	85.7	Pass
L8	8.75 - 7.25	Pole	TP45.111x44.8484x0.5276	8	-33.75	3821.12	86.1	Pass
L9	7.25 - 0	Pole	TP46.38x45.111x0.4952	9	-36.01	3732.84	92.3	Pass
							Summary	
						Pole (L9)	92.3	Pass
						Rating =	92.3	Pass

Table 5 - Tower Component Stresses vs. Capacity

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	80.4	Pass
1	Base Plate	0	64.0	Pass
1	Base Foundation Structural Steel	0	79.3	Pass
1	Base Foundation Soil Interaction	0	18.2	Pass

Structure Rating (max from all components) =	**92.3%**

Notes:
1) See additional documentation in “Appendix C – Additional Calculations” for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the existing, reserved, and proposed loads. No modifications are required at this time.

APPENDIX A

TNXTOWER OUTPUT

Tower Input Data

There is a pole section.
This tower is designed using the TIA/EIA-222-F standard.
The following design criteria apply:

1) Tower is located in New Haven County, Connecticut.
2) Basic wind speed of 85 mph.
3) Nominal ice thickness of 0.7500 in.
4) Ice density of 56 pcf.
5) A wind speed of 38 mph is used in combination with ice.
6) Temperature drop of 50 °F.
7) Deflections calculated using a wind speed of 50 mph.
8) A non-linear (P-delta) analysis was used.
9) Pressures are calculated at each section.
10) Stress ratio used in pole design is 1.333.
11) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

	Consider Moments - Legs		Distribute Leg Loads As Uniform		Treat Feedline Bundles As Cylinder
	Consider Moments - Horizontals		Assume Legs Pinned		Use ASCE 10 X-Brace Ly Rules
	Consider Moments - Diagonals	√	Assume Rigid Index Plate		Calculate Redundant Bracing Forces
	Use Moment Magnification	√	Use Clear Spans For Wind Area		Ignore Redundant Members in FEA
√	Use Code Stress Ratios		Use Clear Spans For KL/r		SR Leg Bolts Resist Compression
√	Use Code Safety Factors - Guys		Retension Guys To Initial Tension		All Leg Panels Have Same Allowable
	Escalate Ice	√	Bypass Mast Stability Checks		Offset Girt At Foundation
	Always Use Max Kz	√	Use Azimuth Dish Coefficients	√	Consider Feedline Torque
	Use Special Wind Profile	√	Project Wind Area of Appurt.		Include Angle Block Shear Check
	Include Bolts In Member Capacity		Autocalc Torque Arm Areas		Poles
	Leg Bolts Are At Top Of Section		SR Members Have Cut Ends	√	Include Shear-Torsion Interaction
	Secondary Horizontal Braces Leg		Sort Capacity Reports By Component		Always Use Sub-Critical Flow
	Use Diamond Inner Bracing (4 Sided)		Triangulate Diamond Inner Bracing		Use Top Mounted Sockets
	Add IBC .6D+W Combination		Use TIA-222-G Tension Splice Capacity Exemption		

Tapered Pole Section Geometry

Section	*Elevation*	*Section Length*	*Splice Length*	*Number of Sides*	*Top Diameter*	*Bottom Diameter*	*Wall Thickness*	*Bend Radius*	*Pole Grade*
	ft	*ft*	*ft*		*in*	*in*	*in*	*in*	
L1	150.00-102.50	47.50	3.75	12	22.0000	30.3140	0.2500	1.0000	A607-60 (60 ksi)
L2	102.50-73.50	32.75	0.00	12	29.1576	34.8901	0.3125	1.2500	A607-65 (65 ksi)
L3	73.50-62.00	11.50	4.75	12	34.8901	36.9030	0.3963	1.5852	Reinf 63.20 ksi (63 ksi)
L4	62.00-56.25	10.50	0.00	12	35.2790	37.2844	0.3750	1.5000	A607-65 (65 ksi)
L5	56.25-38.75	17.50	0.00	12	37.2844	40.3473	0.4498	1.7992	Reinf 63.30 ksi (63 ksi)
L6	38.75-37.50	1.25	0.00	12	40.3473	40.5661	0.4755	1.9021	Reinf 62.91 ksi (63 ksi)
L7	37.50-8.75	28.75	0.00	12	40.5661	44.8484	0.5282	2.1127	Reinf 63.08 ksi (63 ksi)
L8	8.75-7.25	1.50	0.00	12	44.8484	45.1110	0.5276	2.1103	Reinf 63.08 ksi (63 ksi)
L9	7.25-0.00	7.25		12	45.1110	46.3800	0.4952	1.9808	Reinf 63.79 ksi (64 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in^2	I in^4	r in	C in	I/C in^3	J in^4	It/Q in^2	w in	w/t
L1	22.7761	17.5087	1057.2060	7.7865	11.3960	92.7699	2142.1860	8.6173	5.2260	20.904
	31.3834	24.2015	2792.0431	10.7629	15.7027	177.8071	5657.4363	11.9113	7.4542	29.817
L2	30.8657	29.0254	3082.5449	10.3266	15.1037	204.0927	6246.0718	14.2854	6.9767	22.326
	36.1209	34.7937	5309.7644	12.3788	18.0731	293.7945	10759.0222	17.1244	8.5130	27.242
L3	36.1209	44.0181	6684.9546	12.3488	18.0731	369.8851	13545.5305	21.6644	8.2884	20.914
	38.2048	46.5869	7924.9080	13.0694	19.1158	414.5747	16058.0121	22.9286	8.8279	22.275
L4	37.4627	42.1465	6553.8563	12.4956	18.2745	358.6340	13279.8896	20.7432	8.4498	22.533
	38.5996	44.5681	7749.6660	13.2136	19.3133	401.2605	15702.9244	21.9350	8.9872	23.966
L5	38.5996	53.3484	9238.8685	13.1868	19.3133	478.3681	18720.4523	26.2565	8.7868	19.535
	41.7706	57.7846	11740.5914	14.2833	20.8999	561.7530	23789.6212	28.4398	9.6076	21.36
L6	41.7706	61.0507	12388.2073	14.2741	20.8999	592.7395	25101.8665	30.0473	9.5387	20.059
	41.9971	61.3857	12593.2537	14.3524	21.0132	599.3008	25517.3462	30.2122	9.5973	20.183
L7	41.9971	68.0928	13932.5537	14.3336	21.0132	663.0368	28231.1310	33.5132	9.4562	17.904
	46.4305	75.3758	18898.2746	15.8666	23.2315	813.4769	38293.0280	37.0977	10.6039	20.077
L8	46.4305	75.2926	18877.9179	15.8669	23.2315	812.6007	38251.7798	37.0567	10.6055	20.102
	46.7023	75.7386	19215.4120	15.9609	23.3675	822.3140	38935.6344	37.2762	10.6758	20.235
L9	46.7023	71.1418	18075.4005	15.9724	23.3675	773.5278	36625.6618	35.0138	10.7626	21.734
	48.0161	73.1653	19662.0586	16.4268	24.0248	818.4054	39840.6615	36.0098	11.1027	22.421

Tower Elevation ft	Gusset Area (per face) ft^2	Gusset Thickness in	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
L1 150.00-102.50				1	1	1		
L2 102.50-73.50				1	1	1		
L3 73.50-62.00				1	1	1		
L4 62.00-56.25				1	1	1		
L5 56.25-38.75				1	1	1		
L6 38.75-37.50				1	1	1		
L7 37.50-8.75				1	1	1		
L8 8.75-7.25				1	1	1		
L9 7.25-0.00				1	1	1		

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number		C_AA_A ft^2/ft	Weight plf
LDF6-50A(1-1/4")	C	No	Inside Pole	150.00 - 0.00	1	No Ice	0.00	0.66
						1/2" Ice	0.00	0.66
						1" Ice	0.00	0.66
HB114-1-0813U4-M5J(1 1/4")	C	No	CaAa (Out Of Face)	150.00 - 0.00	2	No Ice	0.00	1.20
						1/2" Ice	0.00	2.45
						1" Ice	0.00	4.30
HB114-1-0813U4-M5J(1 1/4")	C	No	CaAa (Out Of Face)	150.00 - 0.00	1	No Ice	0.15	1.20
						1/2" Ice	0.25	2.45
						1" Ice	0.35	4.30
HB114-21U3M12-XXXF(1-1/4")	C	No	CaAa (Out Of Face)	150.00 - 0.00	1	No Ice	0.00	1.22
						1/2" Ice	0.00	2.47
						1" Ice	0.00	4.32
**********								
CR 50 1873(1-5/8")	C	No	Inside Pole	138.00 - 0.00	6	No Ice	0.00	0.83
						1/2" Ice	0.00	0.83
						1" Ice	0.00	0.83
**********								
561(1-5/8")	C	No	Inside Pole	124.00 - 0.00	12	No Ice	0.00	1.35
						1/2" Ice	0.00	1.35
						1" Ice	0.00	1.35
LDF4-50A(1/2")	C	No	Inside Pole	124.00 - 0.00	1	No Ice	0.00	0.15
						1/2" Ice	0.00	0.15
						1" Ice	0.00	0.15
HB158-1-08U8-S8J18(1-5/8)	C	No	Inside Pole	124.00 - 0.00	1	No Ice	0.00	1.30
						1/2" Ice	0.00	1.30
						1" Ice	0.00	1.30
**********								
**********								
LDF4-50A(1/2")	C	No	Inside Pole	102.00 - 0.00	1	No Ice	0.00	0.15
						1/2" Ice	0.00	0.15
						1" Ice	0.00	0.15
LDF7-50A(1-5/8")	C	No	Inside Pole	102.00 - 0.00	12	No Ice	0.00	0.82
						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
FB-L98B-002-50000(3/8)	C	No	Inside Pole	102.00 - 0.00	1	No Ice	0.00	0.06
						1/2" Ice	0.00	0.06
						1" Ice	0.00	0.06
WR-VG82ST-BRDA(5/8")	C	No	Inside Pole	102.00 - 0.00	2	No Ice	0.00	0.31
						1/2" Ice	0.00	0.31
						1" Ice	0.00	0.31
2" Conduit	C	No	Inside Pole	102.00 - 0.00	1	No Ice	0.00	1.16
						1/2" Ice	0.00	1.16
						1" Ice	0.00	1.16
**********								
LDF4-50A(1/2")	C	No	Inside Pole	86.00 - 0.00	2	No Ice	0.00	0.15
						1/2" Ice	0.00	0.15
						1" Ice	0.00	0.15
**********								
LDF4-50A(1/2")	C	No	Inside Pole	82.00 - 0.00	1	No Ice	0.00	0.15
						1/2" Ice	0.00	0.15
						1" Ice	0.00	0.15
**********								
1" Flat Reinforcement	C	No	CaAa (Out Of Face)	41.25 - 0.00	1	No Ice	0.17	0.00
						1/2" Ice	0.28	0.00
						1" Ice	0.39	0.00
3/4" Flat Reinforcement	C	No	CaAa (Out Of Face)	57.50 - 37.50	1	No Ice	0.13	0.00
						1/2" Ice	0.24	0.00
						1" Ice	0.35	0.00
3/4" Flat Reinforcement	C	No	CaAa (Out Of Face)	74.75 - 64.75	1	No Ice	0.13	0.00
						1/2" Ice	0.24	0.00
						1" Ice	0.35	0.00

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A_R ft^2	A_F ft^2	C_AA_A In Face ft^2	C_AA_A Out Face ft^2	Weight K
L1	150.00-102.50	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	7.315	0.82
L2	102.50-73.50	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	4.622	1.16
L3	73.50-62.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	2.865	0.46
L4	62.00-56.25	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	1.042	0.23
L5	56.25-38.75	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	5.299	0.71
L6	38.75-37.50	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.557	0.05
L7	37.50-8.75	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	9.219	1.16
L8	8.75-7.25	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.481	0.06
L9	7.25-0.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	2.325	0.29

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft^2	A_F ft^2	C_AA_A In Face ft^2	C_AA_A Out Face ft^2	Weight K
L1	150.00-102.50	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	14.440	1.23
L2	102.50-73.50	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	9.181	1.41
L3	73.50-62.00	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	6.048	0.56
L4	62.00-56.25	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	2.113	0.28
L5	56.25-38.75	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	11.258	0.86
L6	38.75-37.50	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	1.161	0.06
L7	37.50-8.75	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	18.323	1.41
L8	8.75-7.25	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.956	0.07
L9	7.25-0.00	A	0.750	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	4.621	0.36

Feed Line Center of Pressure

Section	Elevation	CP_X	CP_Z	CP_X Ice	CP_Z Ice
	ft	in	in	in	in
L1	150.00-102.50	-0.1868	0.1079	-0.3300	0.1905
L2	102.50-73.50	-0.1960	0.1132	-0.3544	0.2046
L3	73.50-62.00	-0.2980	0.1720	-0.5595	0.3230
L4	62.00-56.25	-0.2226	0.1285	-0.4120	0.2379
L5	56.25-38.75	-0.3606	0.2082	-0.6770	0.3908
L6	38.75-37.50	-0.5113	0.2952	-0.9194	0.5308
L7	37.50-8.75	-0.3821	0.2206	-0.6819	0.3937
L8	8.75-7.25	-0.3837	0.2215	-0.6880	0.3972
L9	7.25-0.00	-0.3842	0.2218	-0.6899	0.3983

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C_AA_A Front ft^2	C_AA_A Side ft^2	Weight K
201-7	A	From Leg	4.00 0.00 6.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	1.09 1.94 2.80	1.09 1.94 2.80	0.00 0.01 0.03
APXVSPP18-C-A20 w/ Mount Pipe	A	From Leg	4.00 0.00 3.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	8.50 9.15 9.77	6.95 8.13 9.02	0.08 0.15 0.23
APXVSPP18-C-A20 w/ Mount Pipe	B	From Leg	4.00 0.00 3.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	8.50 9.15 9.77	6.95 8.13 9.02	0.08 0.15 0.23
APXVSPP18-C-A20 w/ Mount Pipe	C	From Leg	4.00 0.00 3.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	8.50 9.15 9.77	6.95 8.13 9.02	0.08 0.15 0.23
800MHZ RRH	A	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.49 2.71 2.93	2.07 2.27 2.48	0.05 0.07 0.10
800MHZ RRH	B	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.49 2.71 2.93	2.07 2.27 2.48	0.05 0.07 0.10
800MHZ RRH	C	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.49 2.71 2.93	2.07 2.27 2.48	0.05 0.07 0.10
1900MHz RRH (65MHz)	A	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.71 2.95 3.20	2.61 2.84 3.09	0.06 0.08 0.11
1900MHz RRH (65MHz)	B	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.71 2.95 3.20	2.61 2.84 3.09	0.06 0.08 0.11
1900MHz RRH (65MHz)	C	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	2.71 2.95 3.20	2.61 2.84 3.09	0.06 0.08 0.11
(3) ACU-A20-N	A	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	0.08 0.12 0.17	0.14 0.19 0.25	0.00 0.00 0.00
(3) ACU-A20-N	B	From Leg	4.00	0.0000	150.00	No Ice	0.08	0.14	0.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C_AA_A Front ft^2	C_AA_A Side ft^2	Weight K
			0.00 0.00			1/2" Ice 1" Ice	0.12 0.17	0.19 0.25	0.00 0.00
(3) ACU-A20-N	C	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	0.08 0.12 0.17	0.14 0.19 0.25	0.00 0.00 0.00
800 EXTERNAL NOTCH FILTER	A	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	0.77 0.89 1.02	0.37 0.46 0.56	0.01 0.02 0.02
800 EXTERNAL NOTCH FILTER	B	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	0.77 0.89 1.02	0.37 0.46 0.56	0.01 0.02 0.02
800 EXTERNAL NOTCH FILTER	C	From Leg	4.00 0.00 0.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	0.77 0.89 1.02	0.37 0.46 0.56	0.01 0.02 0.02
APXVTM14-C-120 w/ Mount Pipe	A	From Leg	4.00 0.00 3.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	7.13 7.66 8.18	4.96 5.75 6.47	0.08 0.13 0.19
APXVTM14-C-120 w/ Mount Pipe	B	From Leg	4.00 0.00 3.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	7.13 7.66 8.18	4.96 5.75 6.47	0.08 0.13 0.19
APXVTM14-C-120 w/ Mount Pipe	C	From Leg	4.00 0.00 3.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	7.13 7.66 8.18	4.96 5.75 6.47	0.08 0.13 0.19
TD-RRH8x20-25	A	From Leg	4.00 0.00 3.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	4.72 5.01 5.32	1.70 1.92 2.15	0.07 0.10 0.13
TD-RRH8x20-25	B	From Leg	4.00 0.00 3.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	4.72 5.01 5.32	1.70 1.92 2.15	0.07 0.10 0.13
TD-RRH8x20-25	C	From Leg	4.00 0.00 3.00	0.0000	150.00	No Ice 1/2" Ice 1" Ice	4.72 5.01 5.32	1.70 1.92 2.15	0.07 0.10 0.13
Handrail Kit [NA 507-1]	C	None		0.0000	150.00	No Ice 1/2" Ice 1" Ice	4.80 6.70 8.60	4.80 6.70 8.60	0.25 0.29 0.34
Platform Mount [LP 403-1]	C	None		0.0000	150.00	No Ice 1/2" Ice 1" Ice	18.85 24.30 29.75	18.85 24.30 29.75	1.50 1.80 2.09
**********									
APXV18-206517S-C w/ Mount Pipe	A	From Leg	1.00 0.00 0.00	0.0000	138.00	No Ice 1/2" Ice 1" Ice	5.40 5.96 6.48	4.70 5.86 6.73	0.05 0.10 0.15
APXV18-206517S-C w/ Mount Pipe	B	From Leg	1.00 0.00 0.00	0.0000	138.00	No Ice 1/2" Ice 1" Ice	5.40 5.96 6.48	4.70 5.86 6.73	0.05 0.10 0.15
APXV18-206517S-C w/ Mount Pipe	C	From Leg	1.00 0.00 0.00	0.0000	138.00	No Ice 1/2" Ice 1" Ice	5.40 5.96 6.48	4.70 5.86 6.73	0.05 0.10 0.15
Pipe Mount [PM 602-3]	C	None		0.0000	138.00	No Ice	7.68	7.68	0.28

Description	*Face or Leg*	*Offset Type*	*Offsets: Horz Lateral Vert* ft ft ft	*Azimuth Adjustment* °	*Placement* ft		C_AA_A *Front* ft^2	C_AA_A *Side* ft^2	*Weight* K
						1/2" Ice	9.50	9.50	0.35
						1" Ice	11.32	11.32	0.43
**********									
TMA-DB-T1-6Z-8AB-0Z	A	From Leg	2.00 0.00 0.00	0.0000	126.00	No Ice 1/2" Ice 1" Ice	5.60 5.92 6.24	2.33 2.56 2.79	0.04 0.08 0.12
Side Arm Mount [SO 102-1]	A	None		0.0000	126.00	No Ice 1/2" Ice 1" Ice	1.50 1.74 1.98	1.50 1.75 2.00	0.03 0.04 0.04
**********									
GPS_A	C	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	0.30 0.37 0.46	0.30 0.37 0.46	0.00 0.00 0.01
MG D3-800Tx w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	3.57 3.98 4.39	3.42 4.12 4.78	0.03 0.07 0.11
MG D3-800Tx w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	3.57 3.98 4.39	3.42 4.12 4.78	0.03 0.07 0.11
MG D3-800Tx w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	3.57 3.98 4.39	3.42 4.12 4.78	0.03 0.07 0.11
MG D3-800TV w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	3.57 3.98 4.39	3.42 4.12 4.78	0.04 0.07 0.11
MG D3-800TV w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	3.57 3.98 4.39	3.42 4.12 4.78	0.04 0.07 0.11
MG D3-800TV w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	3.57 3.98 4.39	3.42 4.12 4.78	0.04 0.07 0.11
P65.16.XL.2 w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	8.64 9.29 9.91	5.78 6.95 7.83	0.06 0.12 0.19
P65.16.XL.2 w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	8.64 9.29 9.91	5.78 6.95 7.83	0.06 0.12 0.19
P65.16.XL.2 w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	8.64 9.29 9.91	5.78 6.95 7.83	0.06 0.12 0.19
BXA-70080/4CF w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	5.44 5.89 6.35	4.00 4.61 5.25	0.03 0.08 0.13
BXA-80063/4CF w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	5.40 5.84 6.30	3.42 4.02 4.64	0.03 0.07 0.12
BXA-80063/4CF w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	5.40 5.84 6.30	3.42 4.02 4.64	0.03 0.07 0.12

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C_AA_A Front ft^2	C_AA_A Side ft^2	Weight K
RRH2X40-AWS	A	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	2.52 2.75 2.99	1.59 1.80 2.01	0.04 0.06 0.08
RRH2X40-AWS	B	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	2.52 2.75 2.99	1.59 1.80 2.01	0.04 0.06 0.08
RRH2X40-AWS	C	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	2.52 2.75 2.99	1.59 1.80 2.01	0.04 0.06 0.08
(2) FD9R6004/2C-3L	A	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	0.37 0.45 0.54	0.08 0.14 0.20	0.00 0.01 0.01
(2) FD9R6004/2C-3L	B	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	0.37 0.45 0.54	0.08 0.14 0.20	0.00 0.01 0.01
(2) FD9R6004/2C-3L	C	From Leg	4.00 0.00 2.00	0.0000	124.00	No Ice 1/2" Ice 1" Ice	0.37 0.45 0.54	0.08 0.14 0.20	0.00 0.01 0.01
Platform Mount [LP 403-1]	C	None		0.0000	124.00	No Ice 1/2" Ice 1" Ice	18.85 24.30 29.75	18.85 24.30 29.75	1.50 1.80 2.09
**********									
**********									
7770.00 w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	6.12 6.63 7.13	4.25 5.01 5.71	0.06 0.10 0.16
7770.00 w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	6.12 6.63 7.13	4.25 5.01 5.71	0.06 0.10 0.16
7770.00 w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	6.12 6.63 7.13	4.25 5.01 5.71	0.06 0.10 0.16
(2) P65-16-XLH-RR w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	8.64 9.29 9.91	6.36 7.54 8.43	0.08 0.14 0.22
(2) P65-16-XLH-RR w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	8.64 9.29 9.91	6.36 7.54 8.43	0.08 0.14 0.22
(2) P65-16-XLH-RR w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	8.64 9.29 9.91	6.36 7.54 8.43	0.08 0.14 0.22
GPS_A	B	From Leg	4.00 0.00 5.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	0.30 0.37 0.46	0.30 0.37 0.46	0.00 0.00 0.01
(2) TT19-08BP111-001	A	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	0.64 0.76 0.88	0.52 0.62 0.74	0.02 0.02 0.03
(2) TT19-08BP111-001	B	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice	0.64 0.76 0.88	0.52 0.62 0.74	0.02 0.02 0.03

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft		C_AA_A Front ft^2	C_AA_A Side ft^2	Weight K
						1" Ice			
(2) TT19-08BP111-001	C	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	0.64 0.76 0.88	0.52 0.62 0.74	0.02 0.02 0.03
RRUS-11	A	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	3.25 3.49 3.74	1.37 1.55 1.74	0.05 0.07 0.09
RRUS-11	B	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	3.25 3.49 3.74	1.37 1.55 1.74	0.05 0.07 0.09
RRUS-11	C	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	3.25 3.49 3.74	1.37 1.55 1.74	0.05 0.07 0.09
DC6-48-60-18-8F	C	From Leg	4.00 0.00 0.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	2.57 2.80 3.04	2.57 2.80 3.04	0.02 0.04 0.07
Platform Mount [LP 403-1]	C	None		0.0000	102.00	No Ice 1/2" Ice 1" Ice	18.85 24.30 29.75	18.85 24.30 29.75	1.50 1.80 2.09
**********									
GPS0015	A	From Leg	3.00 0.00 1.00	0.0000	86.00	No Ice 1/2" Ice 1" Ice	0.21 0.27 0.34	0.21 0.27 0.34	0.00 0.00 0.01
GPS0015	C	From Leg	3.00 0.00 1.00	0.0000	86.00	No Ice 1/2" Ice 1" Ice	0.21 0.27 0.34	0.21 0.27 0.34	0.00 0.00 0.01
Side Arm Mount [SO 701-1]	A	None		0.0000	86.00	No Ice 1/2" Ice 1" Ice	0.85 1.14 1.43	1.67 2.34 3.01	0.07 0.08 0.09
Side Arm Mount [SO 701-1]	C	None		0.0000	86.00	No Ice 1/2" Ice 1" Ice	0.85 1.14 1.43	1.67 2.34 3.01	0.07 0.08 0.09
**********									
KS24019-L112A	A	From Leg	3.00 0.00 -1.00	0.0000	82.00	No Ice 1/2" Ice 1" Ice	0.16 0.22 0.30	0.16 0.22 0.30	0.01 0.01 0.01
Side Arm Mount [SO 701-1]	A	None		0.0000	82.00	No Ice 1/2" Ice 1" Ice	0.85 1.14 1.43	1.67 2.34 3.01	0.07 0.08 0.09

Tower Pressures - No Ice

G_H = 1.690

Section Elevation ft	z ft	K_Z	q_z psf	A_G ft²	Face	A_F ft²	A_R ft²	A_{leg} ft²	Leg %	C_AA_A In Face ft²	C_AA_A Out Face ft²
L1 150.00-102.50	125.31	1.464	27	103.538	A	0.000	103.538	103.538	100.00	0.000	0.000
					B	0.000	103.538		100.00	0.000	0.000
					C	0.000	103.538		100.00	0.000	7.315
L2 102.50-73.50	87.62	1.322	24	78.184	A	0.000	78.184	78.184	100.00	0.000	0.000
					B	0.000	78.184		100.00	0.000	0.000
					C	0.000	78.184		100.00	0.000	4.622
L3 73.50-62.00	67.70	1.228	23	34.401	A	0.000	34.401	34.401	100.00	0.000	0.000
					B	0.000	34.401		100.00	0.000	0.000
					C	0.000	34.401		100.00	0.000	2.865
L4 62.00-56.25	59.11	1.181	22	17.602	A	0.000	17.602	17.602	100.00	0.000	0.000
					B	0.000	17.602		100.00	0.000	0.000
					C	0.000	17.602		100.00	0.000	1.042
L5 56.25-38.75	47.38	1.109	21	56.606	A	0.000	56.606	56.606	100.00	0.000	0.000
					B	0.000	56.606		100.00	0.000	0.000
					C	0.000	56.606		100.00	0.000	5.299
L6 38.75-37.50	38.12	1.042	19	4.214	A	0.000	4.214	4.214	100.00	0.000	0.000
					B	0.000	4.214		100.00	0.000	0.000
					C	0.000	4.214		100.00	0.000	0.557
L7 37.50-8.75	22.88	1	18	102.319	A	0.000	102.319	102.319	100.00	0.000	0.000
					B	0.000	102.319		100.00	0.000	0.000
					C	0.000	102.319		100.00	0.000	9.219
L8 8.75-7.25	8.00	1	18	5.622	A	0.000	5.622	5.622	100.00	0.000	0.000
					B	0.000	5.622		100.00	0.000	0.000
					C	0.000	5.622		100.00	0.000	0.481
L9 7.25-0.00	3.61	1	18	27.638	A	0.000	27.638	27.638	100.00	0.000	0.000
					B	0.000	27.638		100.00	0.000	0.000
					C	0.000	27.638		100.00	0.000	2.325

Tower Pressure - With Ice

G_H = 1.690

Section Elevation ft	z ft	K_Z	q_z psf	t_Z in	A_G ft²	Face	A_F ft²	A_R ft²	A_{leg} ft²	Leg %	C_AA_A In Face ft²	C_AA_A Out Face ft²
L1 150.00-102.50	125.31	1.464	5	0.7500	109.476	A	0.000	109.476	109.476	100.00	0.000	0.000
						B	0.000	109.476		100.00	0.000	0.000
						C	0.000	109.476		100.00	0.000	14.440
L2 102.50-73.50	87.62	1.322	5	0.7500	81.809	A	0.000	81.809	81.809	100.00	0.000	0.000
						B	0.000	81.809		100.00	0.000	0.000
						C	0.000	81.809		100.00	0.000	9.181
L3 73.50-62.00	67.70	1.228	4	0.7500	35.838	A	0.000	35.838	35.838	100.00	0.000	0.000
						B	0.000	35.838		100.00	0.000	0.000
						C	0.000	35.838		100.00	0.000	6.048
L4 62.00-56.25	59.11	1.181	4	0.7500	18.321	A	0.000	18.321	18.321	100.00	0.000	0.000
						B	0.000	18.321		100.00	0.000	0.000
						C	0.000	18.321		100.00	0.000	2.113
L5 56.25-38.75	47.38	1.109	4	0.7500	58.794	A	0.000	58.794	58.794	100.00	0.000	0.000
						B	0.000	58.794		100.00	0.000	0.000
						C	0.000	58.794		100.00	0.000	11.258
L6 38.75-37.50	38.12	1.042	4	0.7500	4.370	A	0.000	4.370	4.370	100.00	0.000	0.000
						B	0.000	4.370		100.00	0.000	0.000
						C	0.000	4.370		100.00	0.000	1.161
L7 37.50-8.75	22.88	1	4	0.7500	105.913	A	0.000	105.913	105.913	100.00	0.000	0.000
						B	0.000	105.913		100.00	0.000	0.000
						C	0.000	105.913		100.00	0.000	18.323
L8 8.75-7.25	8.00	1	4	0.7500	5.810	A	0.000	5.810	5.810	100.00	0.000	0.000
						B	0.000	5.810		100.00	0.000	0.000
						C	0.000	5.810		100.00	0.000	0.956
L9 7.25-0.00	3.61	1	4	0.7500	28.544	A	0.000	28.544	28.544	100.00	0.000	0.000
						B	0.000	28.544		100.00	0.000	0.000
						C	0.000	28.544		100.00	0.000	4.621

Tower Pressure - Service

G_H = 1.690

Section Elevation ft	z ft	K_Z	q_z psf	A_G ft²	Face	A_F ft²	A_R ft²	A_{leg} ft²	Leg %	C_AA_A In Face ft²	C_AA_A Out Face ft²
L1 150.00-102.50	125.31	1.464	9	103.538	A	0.000	103.538	103.538	100.00	0.000	0.000
					B	0.000	103.538		100.00	0.000	0.000
					C	0.000	103.538		100.00	0.000	7.315
L2 102.50-73.50	87.62	1.322	8	78.184	A	0.000	78.184	78.184	100.00	0.000	0.000
					B	0.000	78.184		100.00	0.000	0.000
					C	0.000	78.184		100.00	0.000	4.622
L3 73.50-62.00	67.70	1.228	8	34.401	A	0.000	34.401	34.401	100.00	0.000	0.000
					B	0.000	34.401		100.00	0.000	0.000
					C	0.000	34.401		100.00	0.000	2.865
L4 62.00-56.25	59.11	1.181	8	17.602	A	0.000	17.602	17.602	100.00	0.000	0.000
					B	0.000	17.602		100.00	0.000	0.000
					C	0.000	17.602		100.00	0.000	1.042
L5 56.25-38.75	47.38	1.109	7	56.606	A	0.000	56.606	56.606	100.00	0.000	0.000
					B	0.000	56.606		100.00	0.000	0.000
					C	0.000	56.606		100.00	0.000	5.299
L6 38.75-37.50	38.12	1.042	7	4.214	A	0.000	4.214	4.214	100.00	0.000	0.000
					B	0.000	4.214		100.00	0.000	0.000
					C	0.000	4.214		100.00	0.000	0.557
L7 37.50-8.75	22.88	1	6	102.319	A	0.000	102.319	102.319	100.00	0.000	0.000
					B	0.000	102.319		100.00	0.000	0.000
					C	0.000	102.319		100.00	0.000	9.219
L8 8.75-7.25	8.00	1	6	5.622	A	0.000	5.622	5.622	100.00	0.000	0.000
					B	0.000	5.622		100.00	0.000	0.000
					C	0.000	5.622		100.00	0.000	0.481
L9 7.25-0.00	3.61	1	6	27.638	A	0.000	27.638	27.638	100.00	0.000	0.000
					B	0.000	27.638		100.00	0.000	0.000
					C	0.000	27.638		100.00	0.000	2.325

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice
14	Dead+Ice+Temp
15	Dead+Wind 0 deg+Ice+Temp
16	Dead+Wind 30 deg+Ice+Temp
17	Dead+Wind 60 deg+Ice+Temp
18	Dead+Wind 90 deg+Ice+Temp
19	Dead+Wind 120 deg+Ice+Temp
20	Dead+Wind 150 deg+Ice+Temp
21	Dead+Wind 180 deg+Ice+Temp
22	Dead+Wind 210 deg+Ice+Temp
23	Dead+Wind 240 deg+Ice+Temp
24	Dead+Wind 270 deg+Ice+Temp
25	Dead+Wind 300 deg+Ice+Temp

Comb. No.	Description
26	Dead+Wind 330 deg+Ice+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37	Dead+Wind 300 deg - Service
38	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	150 - 102.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-14.00	0.63	0.10
			Max. Mx	11	-7.96	438.61	-0.02
			Max. My	2	-7.95	0.23	441.05
			Max. Vy	11	-15.05	438.61	-0.02
			Max. Vx	8	15.18	0.23	-441.00
			Max. Torque	5			0.66
L2	102.5 - 73.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-24.54	1.37	-0.30
			Max. Mx	11	-15.63	1119.09	-0.12
			Max. My	8	-15.62	0.49	-1125.67
			Max. Vy	11	-23.46	1119.09	-0.12
			Max. Vx	8	23.59	0.49	-1125.67
			Max. Torque	9			-0.74
L3	73.5 - 62	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-26.13	1.48	-0.37
			Max. Mx	11	-17.03	1280.19	-0.14
			Max. My	8	-17.02	0.54	-1287.65
			Max. Vy	11	-24.29	1280.19	-0.14
			Max. Vx	8	24.42	0.54	-1287.65
			Max. Torque	9			-0.75
L4	62 - 56.25	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-29.31	1.66	-0.48
			Max. Mx	11	-19.87	1542.38	-0.18
			Max. My	8	-19.86	0.61	-1551.21
			Max. Vy	11	-25.60	1542.38	-0.18
			Max. Vx	8	25.73	0.61	-1551.21
			Max. Torque	9			-0.77
L5	56.25 - 38.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-34.12	1.99	-0.67
			Max. Mx	11	-24.20	2007.89	-0.25
			Max. My	8	-24.19	0.74	-2018.99
			Max. Vy	11	-27.62	2007.89	-0.25
			Max. Vx	8	27.76	0.74	-2018.99
			Max. Torque	9			-0.81
L6	38.75 - 37.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-34.49	2.01	-0.68
			Max. Mx	11	-24.53	2042.50	-0.26
			Max. My	8	-24.52	0.74	-2053.76
			Max. Vy	11	-27.77	2042.50	-0.26
			Max. Vx	8	27.90	0.74	-2053.76
			Max. Torque	9			-0.81
L7	37.5 - 8.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-44.10	2.60	-1.03
			Max. Mx	11	-33.27	2885.36	-0.38
			Max. My	8	-33.27	0.96	-2900.30
			Max. Vy	11	-30.88	2885.36	-0.38

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
			Max. Vx	8	31.01	0.96	-2900.30
			Max. Torque	2			0.90
L8	8.75 - 7.25	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-44.62	2.63	-1.04
			Max. Mx	11	-33.75	2931.79	-0.39
			Max. My	8	-33.75	0.97	-2946.91
			Max. Vy	11	-31.04	2931.79	-0.39
			Max. Vx	8	31.17	0.97	-2946.91
			Max. Torque	2			0.91
L9	7.25 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-47.07	2.79	-1.14
			Max. Mx	11	-36.01	3159.53	-0.42
			Max. My	8	-36.01	1.03	-3175.57
			Max. Vy	11	-31.80	3159.53	-0.42
			Max. Vx	8	31.93	1.03	-3175.57
			Max. Torque	2			0.94

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	14	47.07	-0.00	0.00
	Max. H_x	11	36.02	31.79	0.00
	Max. H_z	2	36.02	0.00	31.92
	Max. M_x	2	3174.74	0.00	31.92
	Max. M_z	5	3157.47	-31.79	0.00
	Max. Torsion	2	0.94	0.00	31.92
	Min. Vert	11	36.02	31.79	0.00
	Min. H_x	5	36.02	-31.79	0.00
	Min. H_z	8	36.02	0.00	-31.92
	Min. M_x	8	-3175.57	0.00	-31.92
	Min. M_z	11	-3159.53	31.79	0.00
	Min. Torsion	8	-0.94	0.00	-31.92

Tower Mast Reaction Summary

Load Combination	Vertical K	$Shear_x$ K	$Shear_z$ K	Overturning Moment, M_x kip-ft	Overturning Moment, M_z kip-ft	Torque kip-ft
Dead Only	36.02	-0.00	0.00	0.41	1.00	0.00
Dead+Wind 0 deg - No Ice	36.02	-0.00	-31.92	-3174.74	1.03	-0.94
Dead+Wind 30 deg - No Ice	36.02	15.90	-27.64	-2749.55	-1578.48	-0.89
Dead+Wind 60 deg - No Ice	36.02	27.53	-15.96	-1587.29	-2734.78	-0.61
Dead+Wind 90 deg - No Ice	36.02	31.79	-0.00	0.42	-3157.47	-0.17
Dead+Wind 120 deg - No Ice	36.02	27.53	15.96	1588.13	-2734.78	0.32
Dead+Wind 150 deg - No Ice	36.02	15.90	27.64	2750.38	-1578.48	0.73
Dead+Wind 180 deg - No Ice	36.02	-0.00	31.92	3175.57	1.03	0.94
Dead+Wind 210 deg - No Ice	36.02	-15.90	27.64	2750.38	1580.53	0.90
Dead+Wind 240 deg - No Ice	36.02	-27.53	15.96	1588.13	2736.84	0.61
Dead+Wind 270 deg - No Ice	36.02	-31.79	-0.00	0.42	3159.53	0.17
Dead+Wind 300 deg - No Ice	36.02	-27.53	-15.96	-1587.29	2736.84	-0.33
Dead+Wind 330 deg - No Ice	36.02	-15.90	-27.64	-2749.55	1580.54	-0.73
Dead+Ice+Temp	47.07	0.00	-0.00	1.14	2.79	-0.00
Dead+Wind 0 deg+Ice+Temp	47.07	0.00	-7.32	-752.41	2.96	-0.28
Dead+Wind 30 deg+Ice+Temp	47.07	3.65	-6.34	-651.47	-372.15	-0.27
Dead+Wind 60 deg+Ice+Temp	47.07	6.32	-3.66	-375.63	-646.76	-0.18
Dead+Wind 90	47.07	7.30	-0.00	1.19	-747.24	-0.05

Load Combination	Vertical	$Shear_x$	$Shear_z$	Overturning Moment, M_x	Overturning Moment, M_z	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
deg+Ice+Temp						
Dead+Wind 120 deg+Ice+Temp	47.07	6.32	3.66	378.00	-646.76	0.10
Dead+Wind 150 deg+Ice+Temp	47.07	3.65	6.34	653.84	-372.15	0.22
Dead+Wind 180 deg+Ice+Temp	47.07	0.00	7.32	754.79	2.96	0.28
Dead+Wind 210 deg+Ice+Temp	47.07	-3.65	6.34	653.84	378.07	0.27
Dead+Wind 240 deg+Ice+Temp	47.07	-6.32	3.66	378.00	652.68	0.18
Dead+Wind 270 deg+Ice+Temp	47.07	-7.30	-0.00	1.19	753.16	0.05
Dead+Wind 300 deg+Ice+Temp	47.07	-6.32	-3.66	-375.63	652.68	-0.10
Dead+Wind 330 deg+Ice+Temp	47.07	-3.65	-6.34	-651.47	378.08	-0.22
Dead+Wind 0 deg - Service	36.02	0.00	-11.04	-1099.66	1.04	-0.33
Dead+Wind 30 deg - Service	36.02	5.50	-9.56	-952.39	-546.23	-0.31
Dead+Wind 60 deg - Service	36.02	9.53	-5.52	-549.69	-946.86	-0.21
Dead+Wind 90 deg - Service	36.02	11.00	-0.00	0.42	-1093.37	-0.06
Dead+Wind 120 deg - Service	36.02	9.53	5.52	550.52	-946.86	0.11
Dead+Wind 150 deg - Service	36.02	5.50	9.56	953.22	-546.23	0.25
Dead+Wind 180 deg - Service	36.02	0.00	11.04	1100.50	1.04	0.33
Dead+Wind 210 deg - Service	36.02	-5.50	9.56	953.22	548.30	0.31
Dead+Wind 240 deg - Service	36.02	-9.53	5.52	550.52	948.93	0.21
Dead+Wind 270 deg - Service	36.02	-11.00	-0.00	0.42	1095.45	0.06
Dead+Wind 300 deg - Service	36.02	-9.53	-5.52	-549.69	948.93	-0.11
Dead+Wind 330 deg - Service	36.02	-5.50	-9.56	-952.39	548.30	-0.25

Solution Summary

	Sum of Applied Forces			Sum of Reactions			
Load Comb.	PX	PY	PZ	PX	PY	PZ	% Error
	K	K	K	K	K	K	
1	0.00	-36.02	0.00	0.00	36.02	-0.00	0.000%
2	0.00	-36.02	-31.92	0.00	36.02	31.92	0.004%
3	15.90	-36.02	-27.64	-15.90	36.02	27.64	0.000%
4	27.53	-36.02	-15.96	-27.53	36.02	15.96	0.000%
5	31.79	-36.02	0.00	-31.79	36.02	0.00	0.010%
6	27.53	-36.02	15.96	-27.53	36.02	-15.96	0.000%
7	15.90	-36.02	27.64	-15.90	36.02	-27.64	0.000%
8	0.00	-36.02	31.92	0.00	36.02	-31.92	0.004%
9	-15.90	-36.02	27.64	15.90	36.02	-27.64	0.000%
10	-27.53	-36.02	15.96	27.53	36.02	-15.96	0.000%
11	-31.79	-36.02	0.00	31.79	36.02	0.00	0.010%
12	-27.53	-36.02	-15.96	27.53	36.02	15.96	0.000%
13	-15.90	-36.02	-27.64	15.90	36.02	27.64	0.000%
14	0.00	-47.07	0.00	-0.00	47.07	0.00	0.001%
15	0.00	-47.07	-7.32	-0.00	47.07	7.32	0.000%
16	3.65	-47.07	-6.34	-3.65	47.07	6.34	0.000%
17	6.32	-47.07	-3.66	-6.32	47.07	3.66	0.000%
18	7.30	-47.07	0.00	-7.30	47.07	0.00	0.000%
19	6.32	-47.07	3.66	-6.32	47.07	-3.66	0.000%
20	3.65	-47.07	6.34	-3.65	47.07	-6.34	0.000%
21	0.00	-47.07	7.32	-0.00	47.07	-7.32	0.000%
22	-3.65	-47.07	6.34	3.65	47.07	-6.34	0.000%
23	-6.32	-47.07	3.66	6.32	47.07	-3.66	0.000%

Load Comb.	Sum of Applied Forces PX K	PY K	PZ K	Sum of Reactions PX K	PY K	PZ K	% Error
24	-7.30	-47.07	0.00	7.30	47.07	0.00	0.000%
25	-6.32	-47.07	-3.66	6.32	47.07	3.66	0.000%
26	-3.65	-47.07	-6.34	3.65	47.07	6.34	0.000%
27	0.00	-36.02	-11.04	-0.00	36.02	11.04	0.005%
28	5.50	-36.02	-9.56	-5.50	36.02	9.56	0.002%
29	9.53	-36.02	-5.52	-9.53	36.02	5.52	0.002%
30	11.00	-36.02	0.00	-11.00	36.02	0.00	0.005%
31	9.53	-36.02	5.52	-9.53	36.02	-5.52	0.002%
32	5.50	-36.02	9.56	-5.50	36.02	-9.56	0.002%
33	0.00	-36.02	11.04	-0.00	36.02	-11.04	0.005%
34	-5.50	-36.02	9.56	5.50	36.02	-9.56	0.002%
35	-9.53	-36.02	5.52	9.53	36.02	-5.52	0.002%
36	-11.00	-36.02	0.00	11.00	36.02	0.00	0.005%
37	-9.53	-36.02	-5.52	9.53	36.02	5.52	0.002%
38	-5.50	-36.02	-9.56	5.50	36.02	9.56	0.002%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	6	0.00000001	0.00000001
2	Yes	14	0.00004367	0.00007630
3	Yes	18	0.00000001	0.00006142
4	Yes	18	0.00000001	0.00006268
5	Yes	13	0.00010805	0.00014121
6	Yes	18	0.00000001	0.00006225
7	Yes	18	0.00000001	0.00006171
8	Yes	14	0.00004367	0.00007632
9	Yes	18	0.00000001	0.00006308
10	Yes	18	0.00000001	0.00006163
11	Yes	13	0.00010804	0.00014131
12	Yes	18	0.00000001	0.00006205
13	Yes	18	0.00000001	0.00006277
14	Yes	6	0.00000001	0.00001915
15	Yes	15	0.00000001	0.00013920
16	Yes	16	0.00000001	0.00006777
17	Yes	16	0.00000001	0.00006779
18	Yes	15	0.00000001	0.00013802
19	Yes	16	0.00000001	0.00006787
20	Yes	16	0.00000001	0.00006797
21	Yes	15	0.00000001	0.00013954
22	Yes	16	0.00000001	0.00006868
23	Yes	16	0.00000001	0.00006839
24	Yes	15	0.00000001	0.00013934
25	Yes	16	0.00000001	0.00006830
26	Yes	16	0.00000001	0.00006847
27	Yes	13	0.00011445	0.00006974
28	Yes	14	0.00000001	0.00012375
29	Yes	14	0.00000001	0.00013222
30	Yes	13	0.00011446	0.00006678
31	Yes	14	0.00000001	0.00012925
32	Yes	14	0.00000001	0.00012560
33	Yes	13	0.00011445	0.00006978
34	Yes	14	0.00000001	0.00013480
35	Yes	14	0.00000001	0.00012527
36	Yes	13	0.00011446	0.00006693
37	Yes	14	0.00000001	0.00012799
38	Yes	14	0.00000001	0.00013269

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	150 - 102.5	36.860	33	2.1017	0.0022
L2	106.25 - 73.5	18.835	33	1.7157	0.0013
L3	73.5 - 62	8.793	33	1.1522	0.0006
L4	66.75 - 56.25	7.244	33	1.0379	0.0005
L5	56.25 - 38.75	5.104	33	0.8811	0.0004
L6	38.75 - 37.5	2.412	33	0.5869	0.0003
L7	37.5 - 8.75	2.260	33	0.5670	0.0002
L8	8.75 - 7.25	0.128	33	0.1385	0.0001
L9	7.25 - 0	0.088	33	0.1160	0.0000

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
150.00	201-7	33	36.860	2.1017	0.0022	32806
138.00	APXV18-206517S-C w/ Mount Pipe	33	31.656	2.0247	0.0019	13668
126.00	TMA-DB-T1-6Z-8AB-0Z	33	26.579	1.9336	0.0016	6833
124.00	GPS_A	33	25.755	1.9161	0.0016	6307
102.00	7770.00 w/ Mount Pipe	33	17.312	1.6530	0.0012	3585
86.00	GPS0015	33	12.143	1.3790	0.0008	3080
82.00	KS24019-L112A	33	11.002	1.3060	0.0008	2975

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	150 - 102.5	106.140	8	6.0557	0.0063
L2	106.25 - 73.5	54.276	8	4.9446	0.0036
L3	73.5 - 62	25.353	8	3.3221	0.0018
L4	66.75 - 56.25	20.890	8	2.9930	0.0015
L5	56.25 - 38.75	14.720	8	2.5411	0.0012
L6	38.75 - 37.5	6.956	8	1.6929	0.0007
L7	37.5 - 8.75	6.521	8	1.6354	0.0007
L8	8.75 - 7.25	0.369	8	0.3997	0.0002
L9	7.25 - 0	0.254	8	0.3348	0.0001

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
150.00	201-7	8	106.140	6.0557	0.0064	11608
138.00	APXV18-206517S-C w/ Mount Pipe	8	91.171	5.8341	0.0054	4835
126.00	TMA-DB-T1-6Z-8AB-0Z	8	76.564	5.5719	0.0047	2415
124.00	GPS_A	8	74.191	5.5214	0.0046	2228
102.00	7770.00 w/ Mount Pipe	8	49.891	4.7642	0.0034	1261
86.00	GPS0015	8	35.006	3.9754	0.0024	1079
82.00	KS24019-L112A	8	31.717	3.7650	0.0022	1041

Compression Checks

Pole Design Data

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	F_a ksi	A in^2	Actual P K	Allow. P_a K	Ratio $\frac{P}{P_a}$
L1	150 - 102.5 (1)	TP30.314x22x0.25	47.50	0.00	0.0	36.000	23.6731	-7.95	852.23	0.009
L2	102.5 - 73.5 (2)	TP34.8901x29.1576x0.3125	32.75	0.00	0.0	39.000	34.7937	-15.62	1356.95	0.012
L3	73.5 - 62 (3)	TP36.903x34.8901x0.3963	11.50	0.00	0.0	37.920	45.5259	-17.02	1726.34	0.010
L4	62 - 56.25 (4)	TP37.2844x35.279x0.375	10.50	0.00	0.0	39.000	44.5681	-19.86	1738.15	0.011
L5	56.25 - 38.75 (5)	TP40.3473x37.2844x0.4498	17.50	0.00	0.0	37.980	57.7846	-24.19	2194.66	0.011
L6	38.75 - 37.5 (6)	TP40.5661x40.3473x0.4755	1.25	0.00	0.0	37.746	61.3857	-24.52	2317.06	0.011
L7	37.5 - 8.75 (7)	TP44.8484x40.5661x0.5282	28.75	0.00	0.0	37.848	75.3758	-33.27	2852.82	0.012
L8	8.75 - 7.25 (8)	TP45.111x44.8484x0.5276	1.50	0.00	0.0	37.848	75.7386	-33.75	2866.56	0.012
L9	7.25 - 0 (9)	TP46.38x45.111x0.4952	7.25	0.00	0.0	38.274	73.1653	-36.01	2800.33	0.013

Pole Bending Design Data

Section No.	Elevation ft	Size	Actual M_x kip-ft	Actual f_{bx} ksi	Allow. F_{bx} ksi	Ratio $\frac{f_{bx}}{F_{bx}}$	Actual M_y kip-ft	Actual f_{by} ksi	Allow. F_{by} ksi	Ratio $\frac{f_{by}}{F_{by}}$
L1	150 - 102.5 (1)	TP30.314x22x0.25	441.05	31.115	36.000	0.864	0.00	0.000	36.000	0.000
L2	102.5 - 73.5 (2)	TP34.8901x29.1576x0.3125	1125.68	45.978	39.000	1.179	0.00	0.000	39.000	0.000
L3	73.5 - 62 (3)	TP36.903x34.8901x0.3963	1287.65	39.039	37.920	1.030	0.00	0.000	37.920	0.000
L4	62 - 56.25 (4)	TP37.2844x35.279x0.375	1551.21	46.390	39.000	1.189	0.00	0.000	39.000	0.000
L5	56.25 - 38.75 (5)	TP40.3473x37.2844x0.4498	2018.98	43.129	37.980	1.136	0.00	0.000	37.980	0.000
L6	38.75 - 37.5 (6)	TP40.5661x40.3473x0.4755	2053.76	41.123	37.746	1.089	0.00	0.000	37.746	0.000
L7	37.5 - 8.75 (7)	TP44.8484x40.5661x0.5282	2900.30	42.784	37.848	1.130	0.00	0.000	37.848	0.000
L8	8.75 - 7.25 (8)	TP45.111x44.8484x0.5276	2946.92	43.004	37.848	1.136	0.00	0.000	37.848	0.000
L9	7.25 - 0 (9)	TP46.38x45.111x0.4952	3175.57	46.562	38.274	1.217	0.00	0.000	38.274	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V K	Actual f_v ksi	Allow. F_v ksi	Ratio $\frac{f_v}{F_v}$	Actual T kip-ft	Actual f_{vt} ksi	Allow. F_{vt} ksi	Ratio $\frac{f_{vt}}{F_{vt}}$
L1	150 - 102.5 (1)	TP30.314x22x0.25	15.18	0.641	24.000	0.054	0.13	0.004	24.000	0.000
L2	102.5 - 73.5 (2)	TP34.8901x29.1576x0.3125	23.59	0.678	26.000	0.053	0.67	0.013	26.000	0.000
L3	73.5 - 62 (3)	TP36.903x34.8901x0.3963	24.42	0.536	25.280	0.043	0.69	0.010	25.280	0.000
L4	62 - 56.25 (4)	TP37.2844x35.279x0.375	25.73	0.577	26.000	0.045	0.72	0.010	26.000	0.000
L5	56.25 - 38.75 (5)	TP40.3473x37.2844x0.4498	27.76	0.480	25.320	0.039	0.78	0.008	25.320	0.000
L6	38.75 - 37.5 (6)	TP40.5661x40.3473x0.4755	27.90	0.455	25.164	0.037	0.79	0.007	25.164	0.000

Section No.	Elevation ft	Size	Actual V K	Actual f_v ksi	Allow. F_v ksi	Ratio $\frac{f_v}{F_v}$	Actual T kip-ft	Actual f_{vt} ksi	Allow. F_{vt} ksi	Ratio $\frac{f_{vt}}{F_{vt}}$
L7	37.5 - 8.75 (7)	TP44.8484x40.5661x0.5282	31.01	0.411	25.232	0.033	0.90	0.006	25.232	0.000
L8	8.75 - 7.25 (8)	TP45.111x44.8484x0.5276	31.17	0.412	25.232	0.033	0.91	0.006	25.232	0.000
L9	7.25 - 0 (9)	TP46.38x45.111x0.4952	31.93	0.436	25.516	0.035	0.94	0.006	25.516	0.000

Pole Interaction Design Data

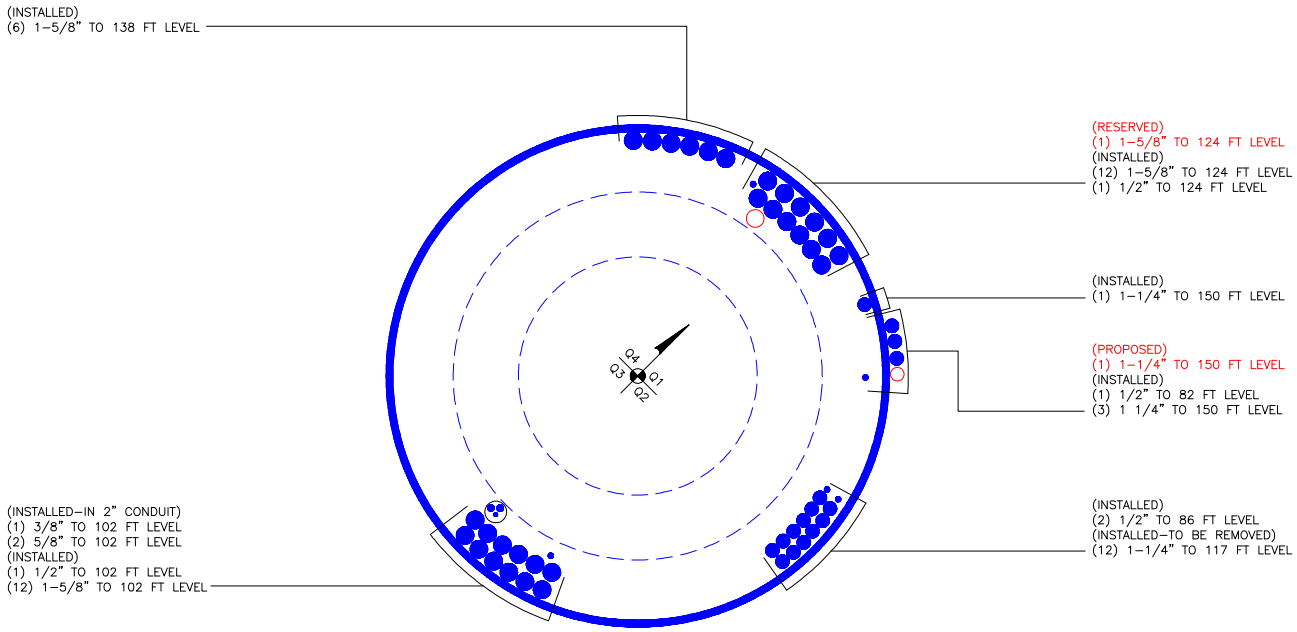
Section No.	Elevation ft	Ratio $\frac{P}{P_a}$	Ratio $\frac{f_{bx}}{F_{bx}}$	Ratio $\frac{f_{by}}{F_{by}}$	Ratio $\frac{f_v}{F_v}$	Ratio $\frac{f_{vt}}{F_{vt}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	150 - 102.5 (1)	0.009	0.864	0.000	0.054	0.000	0.874 ✔	1.333	H1-3+VT ✔
L2	102.5 - 73.5 (2)	0.012	1.179	0.000	0.053	0.000	1.191 ✔	1.333	H1-3+VT ✔
L3	73.5 - 62 (3)	0.010	1.030	0.000	0.043	0.000	1.040 ✔	1.333	H1-3+VT ✔
L4	62 - 56.25 (4)	0.011	1.189	0.000	0.045	0.000	1.201 ✔	1.333	H1-3+VT ✔
L5	56.25 - 38.75 (5)	0.011	1.136	0.000	0.039	0.000	1.147 ✔	1.333	H1-3+VT ✔
L6	38.75 - 37.5 (6)	0.011	1.089	0.000	0.037	0.000	1.100 ✔	1.333	H1-3+VT ✔
L7	37.5 - 8.75 (7)	0.012	1.130	0.000	0.033	0.000	1.142 ✔	1.333	H1-3+VT ✔
L8	8.75 - 7.25 (8)	0.012	1.136	0.000	0.033	0.000	1.148 ✔	1.333	H1-3+VT ✔
L9	7.25 - 0 (9)	0.013	1.217	0.000	0.035	0.000	1.230 ✔	1.333	H1-3+VT ✔

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$SF*P_{allow}$ K	% Capacity	Pass Fail
L1	150 - 102.5	Pole	TP30.314x22x0.25	1	-7.95	1136.03	65.6	Pass
L2	102.5 - 73.5	Pole	TP34.8901x29.1576x0.3125	2	-15.62	1808.81	89.4	Pass
L3	73.5 - 62	Pole	TP36.903x34.8901x0.3963	3	-17.02	2301.21	78.0	Pass
L4	62 - 56.25	Pole	TP37.2844x35.279x0.375	4	-19.86	2316.95	90.1	Pass
L5	56.25 - 38.75	Pole	TP40.3473x37.2844x0.4498	5	-24.19	2925.48	86.0	Pass
L6	38.75 - 37.5	Pole	TP40.5661x40.3473x0.4755	6	-24.52	3088.64	82.5	Pass
L7	37.5 - 8.75	Pole	TP44.8484x40.5661x0.5282	7	-33.27	3802.81	85.7	Pass
L8	8.75 - 7.25	Pole	TP45.111x44.8484x0.5276	8	-33.75	3821.12	86.1	Pass
L9	7.25 - 0	Pole	TP46.38x45.111x0.4952	9	-36.01	3732.84	92.3	Pass
							Summary	
						Pole (L9)	92.3	Pass
						RATING =	**92.3**	**Pass**

APPENDIX B

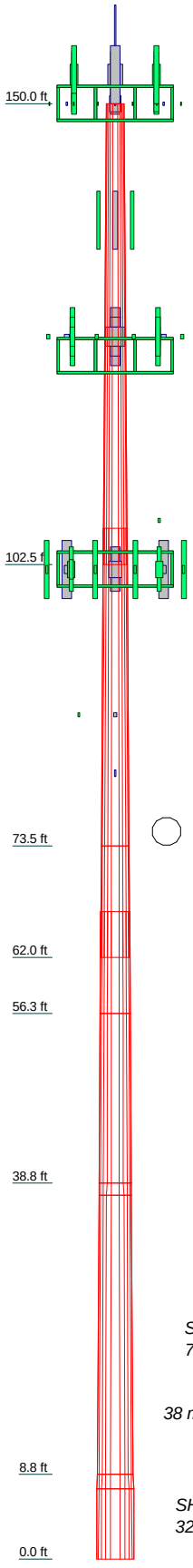
BASE LEVEL DRAWING

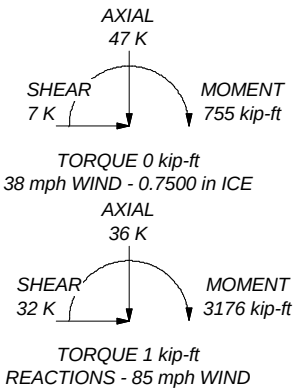

(INSTALLED)
(6) 1-5/8" TO 138 FT LEVEL
(RESERVED)
(1) 1-5/8" TO 124 FT LEVEL
(INSTALLED)
(12) 1-5/8" TO 124 FT LEVEL
(1) 1/2" TO 124 FT LEVEL
(INSTALLED)
(1) 1-1/4" TO 150 FT LEVEL
(PROPOSED)
(1) 1-1/4" TO 150 FT LEVEL
(INSTALLED)
(1) 1/2" TO 82 FT LEVEL
(3) 1 1/4" TO 150 FT LEVEL
(INSTALLED)
(2) 1/2" TO 86 FT LEVEL
(INSTALLED-TO BE REMOVED)
(12) 1-1/4" TO 117 FT LEVEL
(INSTALLED-IN 2" CONDUIT)
(1) 3/8" TO 102 FT LEVEL
(2) 5/8" TO 102 FT LEVEL
(INSTALLED)
(1) 1/2" TO 102 FT LEVEL
(12) 1-5/8" TO 102 FT LEVEL
Q4
Q1
Q3
Q2

APPENDIX C

ADDITIONAL CALCULATIONS

Section	1	2	3	4	5	6	7	8	9
Length (ft)	47.50	32.75	11.50	10.50	17.50	1.25	28.75	1.50	7.25
Number of Sides	12	12	12	12	12	12	12	12	12
Thickness (in)	0.2500	0.3125	0.3963	0.3750	0.4498	0.4755	0.5282	0.5276	0.4952
Socket Length (ft)	3.75		4.75						
Top Dia (in)	22.0000	29.1576	34.8901	35.2790	37.2844	40.3473	40.5661	44.8484	45.1110
Bot Dia (in)	30.3140	34.8901	36.9030	37.2844	40.3473	40.5661	44.8484	45.1110	46.3800
Grade	A607-60	A607-65	Reinf 63.20 ksi	A607-65	Reinf 63.30 ksi	Reinf 62.91 ksi	Reinf 63.08 ksi	Reinf 63.79 ksi	
Weight (K)	3.4	3.6	1.8	1.5	3.3	0.3	7.0	0.4	1.8





DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
201-7	150	MG D3-800TV w/ Mount Pipe	124
APXVSPP18-C-A20 w/ Mount Pipe	150	P65.16.XL.2 w/ Mount Pipe	124
APXVSPP18-C-A20 w/ Mount Pipe	150	P65.16.XL.2 w/ Mount Pipe	124
APXVSPP18-C-A20 w/ Mount Pipe	150	P65.16.XL.2 w/ Mount Pipe	124
800MHZ RRH	150	BXA-70080/4CF w/ Mount Pipe	124
800MHZ RRH	150	BXA-80063/4CF w/ Mount Pipe	124
800MHZ RRH	150	BXA-80063/4CF w/ Mount Pipe	124
1900MHz RRH (65MHz)	150	RRH2X40-AWS	124
1900MHz RRH (65MHz)	150	RRH2X40-AWS	124
1900MHz RRH (65MHz)	150	RRH2X40-AWS	124
(3) ACU-A20-N	150	(2) FD9R6004/2C-3L	124
(3) ACU-A20-N	150	(2) FD9R6004/2C-3L	124
(3) ACU-A20-N	150	(2) FD9R6004/2C-3L	124
800 EXTERNAL NOTCH FILTER	150	Platform Mount [LP 403-1]	124
800 EXTERNAL NOTCH FILTER	150	7770.00 w/ Mount Pipe	102
800 EXTERNAL NOTCH FILTER	150	7770.00 w/ Mount Pipe	102
APXVTM14-C-120 w/ Mount Pipe	150	7770.00 w/ Mount Pipe	102
APXVTM14-C-120 w/ Mount Pipe	150	(2) P65-16-XLH-RR w/ Mount Pipe	102
APXVTM14-C-120 w/ Mount Pipe	150	(2) P65-16-XLH-RR w/ Mount Pipe	102
TD-RRH8x20-25	150	(2) P65-16-XLH-RR w/ Mount Pipe	102
TD-RRH8x20-25	150	GPS_A	102
TD-RRH8x20-25	150	(2) TT19-08BP111-001	102
Handrail Kit [NA 507-1]	150	(2) TT19-08BP111-001	102
Platform Mount [LP 403-1]	150	(2) TT19-08BP111-001	102
APXV18-206517S-C w/ Mount Pipe	138	RRUS-11	102
APXV18-206517S-C w/ Mount Pipe	138	RRUS-11	102
APXV18-206517S-C w/ Mount Pipe	138	RRUS-11	102
Pipe Mount [PM 602-3]	138	DC6-48-60-18-8F	102
TMA-DB-T1-6Z-8AB-0Z	126	Platform Mount [LP 403-1]	102
Side Arm Mount [SO 102-1]	126	GPS0015	86
GPS_A	124	GPS0015	86
MG D3-800Tx w/ Mount Pipe	124	Side Arm Mount [SO 701-1]	86
MG D3-800Tx w/ Mount Pipe	124	Side Arm Mount [SO 701-1]	86
MG D3-800Tx w/ Mount Pipe	124	KS24019-L112A	82
MG D3-800TV w/ Mount Pipe	124	Side Arm Mount [SO 701-1]	82
MG D3-800TV w/ Mount Pipe	124		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A607-60	60 ksi	75 ksi	Reinf 62.91 ksi	63 ksi	79 ksi
A607-65	65 ksi	80 ksi	Reinf 63.08 ksi	63 ksi	79 ksi
Reinf 63.20 ksi	63 ksi	79 ksi	Reinf 63.79 ksi	64 ksi	80 ksi
Reinf 63.30 ksi	63 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in New Haven County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 0.75 in ice.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 92.3%

Paul J. Ford and Company
250 E. Broad Street, Suite 600
Columbus, OH 43215
Phone: 614.221.6679
FAX: 614.448.4105

Job:	***Ex 150 ft Monopole / Oak Lane CC, Inc. Tower***		
Project:	***PJF 37513-2197 / BU 876315***		
Client: CCI	Drawn by: Joey Meinerding	App'd:	
Code: TIA/EIA-222-F	Date: 06/16/14	Scale: NTS	
Path: T:\375_Crown_Castle\2013\37513-2197 BU 876315\WO 773034 BU 876315 - 002 SA\37513-2197.002.7805_Reinforced.eri		Dwg No. E-1	

PAUL J. FORD AND COMPANY
STRUCTURAL ENGINEERS
250 East Broad Street • Suite 1500 • Columbus, Ohio 43215-3708
Phone 614-221-6679 • Fax 614-448-4105 • www.PJFweb.com

v4.4 - Effective 7-12-13

Date: 6/16/2014
PJF Project: 37513-2197.002.7805
Client Ref. # 876315
Site Name: Oak Lane CC, Inc. Tower
Description: 150' Pole
Owner: CCI
Engineer: JWM

Asymmetric Anchor Rod Analysis

Moment =	3176	k-ft	TIA Ref.	F	Location =	Base Plate		
Axial =	36.0	kips	ASIF =	1.3333	η =	N/A	for BP, Rev. G Sect. 4.9.9	
Shear =	32.0	kips	Max Ratio =	105.0%	Threads =	N/A	for FP, Rev. G	
Anchor Qty =	19							

**** For Post Installed Anchors: Check anchors for embedment, epoxy/grout bond, and capacity based on proof load. ****

Item	Nominal Anchor Dia, in	Spec	Fy, ksi	Fu, ksi	Location, degrees	Anchor Circle, in	Area Override, in^2	Area, in^2	Max Net Compression, kips	Max Net Tension, kips	Load for Capacity Calc, kips	Capacity Override, kips	Capacity, kips	Capacity Ratio
1	2.250	#18J A615 Gr 75	75	100	25.53	54.00	0.00	3.98	129.42	125.63	125.63	0.00	195.00	64.4%
2	2.250	#18J A615 Gr 75	75	100	38.62	54.00	0.00	3.98	132.26	128.47	128.47	0.00	195.00	65.9%
3	2.250	#18J A615 Gr 75	75	100	51.71	54.00	0.00	3.98	136.52	132.73	132.73	0.00	195.00	68.1%
4	2.250	#18J A615 Gr 75	75	100	64.80	54.00	0.00	3.98	141.26	137.47	137.47	0.00	195.00	70.5%
5	2.250	#18J A615 Gr 75	75	100	115.53	54.00	0.00	3.98	148.59	144.80	144.80	0.00	195.00	74.3%
6	2.250	#18J A615 Gr 75	75	100	128.62	54.00	0.00	3.98	145.64	141.84	141.84	0.00	195.00	72.7%
7	2.250	#18J A615 Gr 75	75	100	141.71	54.00	0.00	3.98	140.96	137.17	137.17	0.00	195.00	70.3%
8	2.250	#18J A615 Gr 75	75	100	154.80	54.00	0.00	3.98	135.27	131.48	131.48	0.00	195.00	67.4%
9	2.250	#18J A615 Gr 75	75	100	205.53	54.00	0.00	3.98	122.04	118.25	118.25	0.00	195.00	60.6%
10	2.250	#18J A615 Gr 75	75	100	218.62	54.00	0.00	3.98	124.27	120.48	120.48	0.00	195.00	61.8%
11	2.250	#18J A615 Gr 75	75	100	231.71	54.00	0.00	3.98	128.45	124.66	124.66	0.00	195.00	63.9%
12	2.250	#18J A615 Gr 75	75	100	244.80	54.00	0.00	3.98	133.56	129.77	129.77	0.00	195.00	66.5%
13	2.250	#18J A615 Gr 75	75	100	295.53	54.00	0.00	3.98	144.92	141.13	141.13	0.00	195.00	72.4%
14	2.250	#18J A615 Gr 75	75	100	308.62	54.00	0.00	3.98	143.46	139.67	139.67	0.00	195.00	71.6%
15	2.250	#18J A615 Gr 75	75	100	321.71	54.00	0.00	3.98	140.43	136.64	136.64	0.00	195.00	70.1%
16	2.250	#18J A615 Gr 75	75	100	334.80	54.00	0.00	3.98	136.52	132.73	132.73	0.00	195.00	68.1%
17	2.250	A193 Gr B7	105	125	18.0	71.00	0.00	3.98	167.37	163.58	163.58	0.00	218.68	74.8%
18	2.250	A193 Gr B7	105	125	162.0	71.00	0.00	3.98	173.18	169.40	169.40	0.00	218.68	77.5%
19	2.250	A193 Gr B7	105	125	252.0	71.00	0.00	3.98	179.68	175.89	175.89	0.00	218.68	80.4%
														75.61

Square, Stiffened / Unstiffened Base Plate, Any Rod Material - Rev. F /G

Assumptions: 1) Rod groups at corners. Total # rods divisible by 4. Maximum total # of rods = 48 (12 per Corner).
2) Rod Spacing = Straight Center-to-Center distance between any (2) adjacent rods (same corner)
3) Clear space between bottom of leveling nut and top of concrete **not** exceeding (1)*(Rod Diameter)

Site Data

BU#: *876315*
Site Name: *Oak Lane CC, Inc. Tower*
App #:

Anchor Rod Data		
Qty:	16	
Diam:	2.25	in
Rod Material:	A615-J	
Yield, Fy:	75	ksi
Strength, Fu:	100	ksi
Bolt Circle:	54	in
Anchor Spacing:	6	in

Plate Data		
W=Side:	54	in
Thick:	3	in
Grade:	60	ksi
Clip Distance:	4	in

Stiffener Data (Welding at both sides)		
Configuration:	Unstiffened	
Weld Type:		**
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

Pole Data		
Diam:	46.38	in
Thick:	0.4375	in
Grade:	65	ksi
# of Sides:	12	"0" IF Round

Stress Increase Factor		
ASD ASIF:	1.333	

** Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

Base Reactions		
TIA Revision:	F	
Unfactored Moment, M:	2640.4	ft-kips
Unfactored Axial, P:	30.3	kips
Unfactored Shear, V:	27	kips

Reactions adjusted to account for additional anchor rods.

Anchor Rod Results

TIA F --> Maximum Rod Tension	144.8	Kips
Allowable Tension:	195.0	Kips
Anchor Rod Stress Ratio:	74.3%	**Pass**

Base Plate Results — Flexural Check

Base Plate Stress:	38.4	ksi
Allowable PL Bending Stress:	60.0	ksi
Base Plate Stress Ratio:	64.0%	**Pass**

PL Ref. Data
Yield Line (in):
29.99
Max PL Length:
29.99

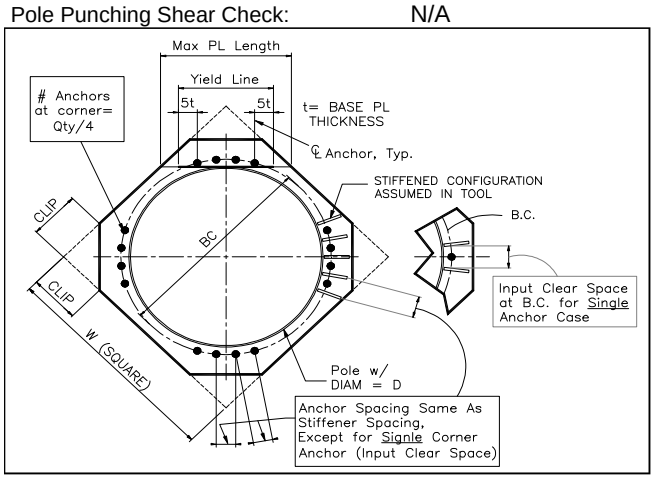
N/A - Unstiffened

Stiffener Results

Horizontal Weld :	N/A
Vertical Weld:	N/A
Plate Flex+Shear, fb/Fb+(fv/Fv)^2:	N/A
Plate Tension+Shear, ft/Ft+(fv/Fv)^2:	N/A
Plate Comp. (AISC Bracket):	N/A

Pole Results

Pole Punching Shear Check: N/A



foundation loads

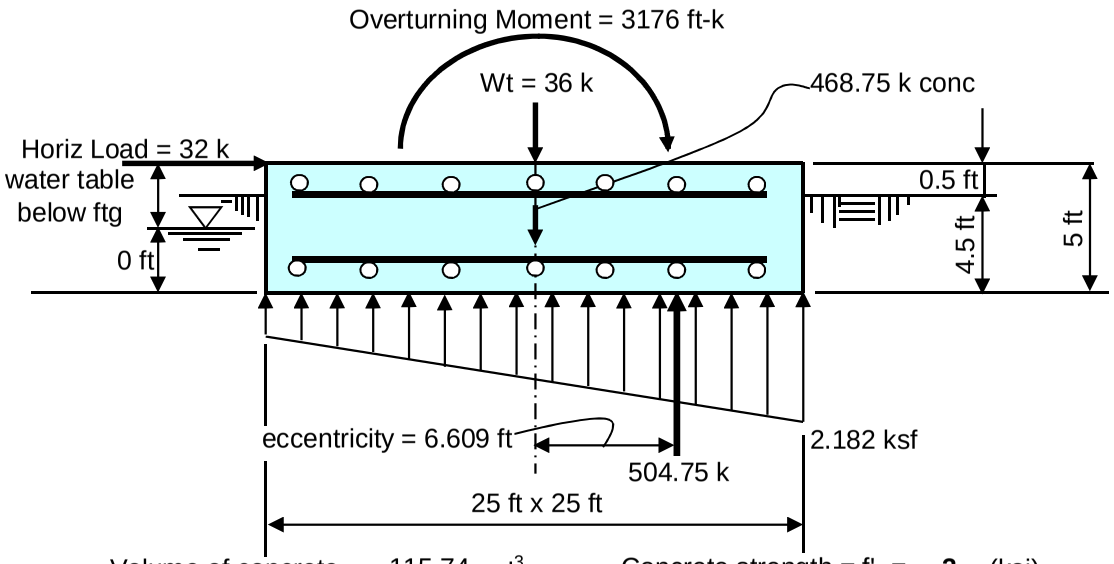
Tower or Pole Weight =	**36**	kips
Total Horizontal Force =	**32**	kips
Overturning Moment =	**3176**	ft-kips

soil properties

Safety factor against overturning =	**1.5**	
Soil density =	**100**	pcf
Allowable soil bearing =	**12**	ksf
Depth to water table =	**99**	ft

mat dimensions

depth to bottom of footing =	**4.5**	ft
Footing thickness =	**5**	ft
Footing Width =	**25**	ft
Footing Length =	**25**	ft
Tower/Pole Center Offset =	**0**	ft



Volume of concrete = 115.74 yd^3 Concrete strength = f'_c = **3** (ksi)

Rebar = (136) #9 bars by 24.5 ft long

reinforcing steel = **(34) #9 @ 8.91 in o.c. ea way top and bottom**

Summary of analysis results

Overturning Moment: (Stress Ratio = 0.793) **< CONTROLLING CRITERIA**

Calculated Overturning Moment = 3336 ft-kips

Resisting Moment = 6309.4 ft-kips

Factor of Safety against overturning = 1.891 **> 1.5 okay**

Rebar strength = F_y = **60** (ksi)

minimum cover over rebar = **3** inches

Soil Bearing (Stress Ratio = 0.182)

Net Soil Bearing Resistance = 12 ksf

Calculated Soil Bearing Pressure = 2.182 **ksf < 12 ksf okay**

Bending Moment (Stress Ratio = 0.253)

Ultimate Bending Moment Resistance = 8259 ft-kips

Calculated Ultimate Bending Moment = 2089 **ft-kips < 8259 ft-kips okay**

Bending Shear (Stress Ratio = 0.201)

Ultimate Bending Shear Resistance = 1363 kips

Calculated Ultimate Bending Shear = 274 **kips < 1363 kips okay**

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RADIO FREQUENCY FCC REGULATORY COMPLIANCE MAXIMUM PERMISSIBLE EXPOSURE (MPE) ASSESSMENT

Sprint Existing Facility

Site ID: CT03XC020

Oak Lane CC Inc. Tower

1116 Johnson Road
Woodbridge, CT 06525

September 13, 2014

EBI Project Number: 62144679

21 B Street · Burlington, MA 01803 · Tel: (781) 273.2500 · Fax: (781) 273.3311

EBI Consulting
environmental | engineering | due diligence

September 13, 2014

Sprint
Attn: RF Engineering Manager
1 International Boulevard, Suite 800
Mahwah, NJ 07495

Re: Radio Frequency Maximum Permissible Exposure (MPE) Assessment for Site:
CT03XC020 - Oak Lane CC Inc. Tower

Site Total: 55.76% - MPE% in full compliance

EBI Consulting was directed to analyze the proposed upgrades to the existing Sprint facility located at **1116 Johnson Road, Woodbridge, CT**, for the purpose of determining whether the radio frequency (RF) exposure levels from the proposed Sprint equipment upgrades on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (µW/cm2). The number of µW/cm2 calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu W/cm^2$). The general population exposure limit for the cellular band (850 MHz Band) is approximately 567 $\mu W/cm^2$, and the general population exposure limit for the 1900 MHz and 2500 MHz bands is 1000 $\mu W/cm^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed upgrades to the existing Sprint Wireless antenna facility located at **1116 Johnson Road, Woodbridge, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. All calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all emissions were calculated using the following assumptions:

1) 2 channels in the 1900 MHz Band were considered for each sector of the proposed installation.

2) 1 channel in the 800 MHz Band was considered for each sector of the proposed installation.

3) 2 channels in the 2500 MHz Band were considered for each sector of the proposed installation.

4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.

5) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

6) The antennas used in this modeling are the RFS APXVSPP18-C-A20 and the RFS APXVTM14-C-I20. This is based on feedback from the carrier with regards to anticipated antenna selection. The RFS APXVSPP18-C-A20 has a 15.9 dBd gain value at its main lobe at 1900 MHz and 13.4 dBd at its main lobe for 850 MHz. The RFS APXVTM14-C-I20 has a 15.9 dBd gain value at its main lobe at 2500 MHz. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

7) The antenna mounting height centerline for the proposed antennas is **153 feet** above ground level (AGL).

8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public threshold limits

Site ID	CT03XC020 - Oak Lane CC Inc. Tower
Site Addresss	1116 Johnson Road, Woodbridge, CT, 06525
Site Type	Monopole

Sector 1

Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain (10 db reduction)	Antenna Height (ft)	analysis height	Cable Size	Cable Loss (dB)	Additional Loss (dB)	ERP	Power Density Percentage
1a	RFS	APXVSPP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	2	40	5.9	153	147	1/2 "	0.5	0	138.69	0.23%
1a	RFS	APXVSPP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	3.4	153	147	1/2 "	0.5	0	39.00	0.11%
1B	RFS	APXVTMM14-C-120	RRH	2500 MHz	CDMA / LTE	20	2	40	5.9	153	147	1/2 "	0.5	0	138.69	0.41%
														Sector total Power Density Value:		0.75%

Sector 2

Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain (10 db reduction)	Antenna Height (ft)	analysis height	Cable Size	Cable Loss (dB)	Additional Loss (dB)	ERP	Power Density Percentage
2a	RFS	APXVSPP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	2	40	5.9	153	147	1/2 "	0.5	0	138.69	0.23%
2a	RFS	APXVSPP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	3.4	153	147	1/2 "	0.5	0	39.00	0.11%
2B	RFS	APXVTMM14-C-120	RRH	2500 MHz	CDMA / LTE	20	2	40	5.9	153	147	1/2 "	0.5	0	138.69	0.41%
														Sector total Power Density Value:		0.75%

Sector 3

Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain (10 db reduction)	Antenna Height (ft)	analysis height	Cable Size	Cable Loss (dB)	Additional Loss (dB)	ERP	Power Density Percentage
3a	RFS	APXVSPP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	2	40	5.9	153	147	1/2 "	0.5	0	138.69	0.23%
3a	RFS	APXVSPP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	3.4	153	147	1/2 "	0.5	0	39.00	0.11%
3B	RFS	APXVTMM14-C-120	RRH	2500 MHz	CDMA / LTE	20	2	40	5.9	153	147	1/2 "	0.5	0	138.69	0.41%
														Sector total Power Density Value:		0.75%

Site Composite MPE %	
Carrier	MPE %
Sprint	2.26%
Verizon Wireless	21.84%
Nextel	4.17%
AT&T	27.49%
Total Site MPE %	**55.76%**

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Summary

All calculations performed for this analysis yielded results that were well within the allowable limits for general public Maximum Permissible Exposure (MPE) to radio frequency energy.

The anticipated Maximum Composite contributions from the Sprint facility are **2.26% (0.75% from sector 1, 0.75% from sector 2 and 0.75% from sector 3)** of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level.

The anticipated composite MPE value for this site assuming all carriers present is **55.76%** of the allowable FCC established general public limit sampled at 6 feet above ground level. This total composite site value is based upon MPE values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan
RF Engineering Director

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