

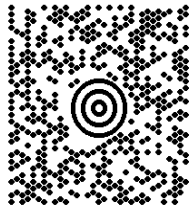
1 OF 1

1 LBS

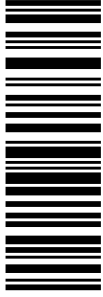
MARY CAULFIELD  
978-994-0252  
CENTERLINE COMMUNICATIONS, LLC  
95 RYAN DRIVE  
RAYNHAM MA 02767

**SHIP TO:**

DONALD TRINKS - MAYOR  
TOWN OF WINDSOR  
275 BROAD STREET  
**WINDSOR CT 06095-2940**

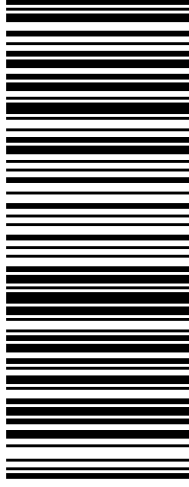


**CT 060 9-02**



**UPS GROUND**

TRACKING #: 1Z 9Y4 503 03 0816 9416



BILLING: P/P

Reference#1: CTS137: CSC filing to Mayor

UPS 20.0.32. WNTNVS0 97.04.01./2018



1 OF 1

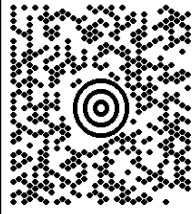
1 LBS

MARY CAULFIELD  
978.994.0252  
CENTERLINE COMMUNICATIONS, LLC  
95 RYAN DRIVE  
RAYNHAM MA 02767

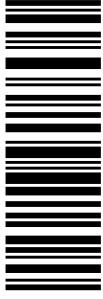
**SHIP TO:**

ERIC BARZ, AICP - TOWN PLANNER  
860.285.1981  
TOWN OF WINDSOR  
275 BROAD STREET

**WINDSOR CT 06095-2940**

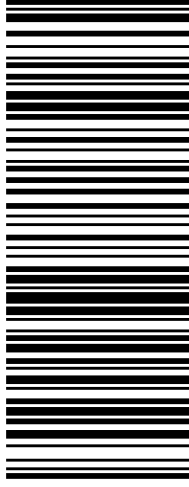


**CT 060 9-02**



**UPS GROUND**

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BILLING: P/P

Reference#1: CTS137: CSC filing to Town Planner

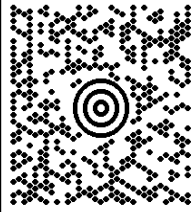


UIS 20.0.32. WNTNVS0 97.04.01./2018

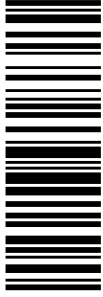
MARY CAULFIELD  
 978-994-0252  
 CENTERLINE COMMUNICATIONS, LLC  
 95 RYAN DRIVE  
 RAYNHAM MA 02767

**1 LBS** **1 OF 1**

**SHIP TO:**  
 GENESIS HEALTH VENTURES  
 1 EMERSON DRIVE  
 WINDSOR CT 06095-3204

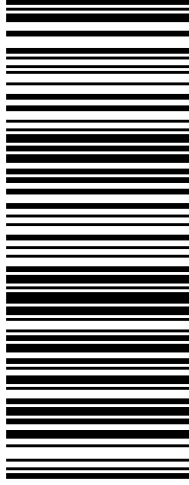


**CT 060 9-02**



**UPS GROUND**

TRACKING #: 1Z 9Y4 503 03 0123 0441



BILLING: P/P

Reference#1: CTS137: CSC filing Property Owner

US 20.0.32. WNTNVS0 97.0A.01./2018



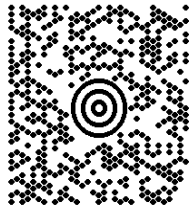
1 OF 1

1 LBS

MARY CAULFIELD  
978-994-0252  
CENTERLINE COMMUNICATIONS, LLC  
95 RYAN DRIVE  
RAYNHAM MA 02767

**SHIP TO:**

PAUL PEDICONE  
(518) 373-3530  
CROWN CASTLE  
SUITE 101  
3 CORPORATE PARK DRIVE  
**CLIFTON PARK NY 12065**

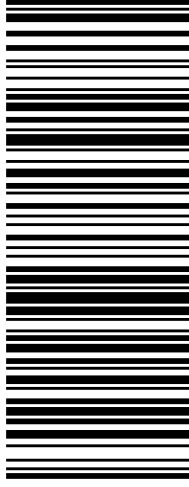


**NY 122 9-02**



**UPS GROUND**

TRACKING #: 1Z 9Y4 503 03 1294 1439



BILLING: P/P

Reference#1: CTS137: CSC filing to Crown

UIS 20.0.32. WNTNVS0 97.04.01./2018



Mary Caulfield, Site Acquisition Consultant  
c/o New Cingular Wireless, PCS LLC (AT&T)  
Centerline Communications, LLC  
95 Ryan Drive, Suite 1  
Raynham, MA 02767  
Mobile: (978) 994-0252  
[MCaulfield@centerlinecommunications.com](mailto:MCaulfield@centerlinecommunications.com)

March 19, 2018

Melanie A. Bachman  
Acting Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

**RE: Notice of Exempt Modification**  
**Site Number: CT5137 (Name: Windsor Matianuck Ave)**  
**1170 Matianuck Avenue, Windsor, CT 06095**  
**N 41.840555 // W -72.666500**

Dear Ms. Bachman:

New Cingular Wireless, PCS, LLC (“AT&T”) currently maintains 6 total antennas at the 97-foot mount level on the existing 100-foot monopine tower, located at 1170 Matianuck Avenue, Windsor, CT. The tower is owned by Crown Castle. The property is owned by Genesis Health Ventures of Bloomfield c/o AT&T Mobility. AT&T now intends to add three (3) new LTE (700/1900/2300 band) antennas for its LTE upgrade. AT&T also intends to install six (6) new remote radios and; and certain in-cabinet upgrades at the base.

Note that this facility was originally approved by the Planning and Zoning Commission of the Town of Windsor Special Use Application #54B, dated October 26, 2001.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Donald S. Trinks, Mayor for the Town of Windsor, Eric Barz, AICP, Town Planner, as well as the tower owner, Crown Castle, and the ground owner, Genesis Health Ventures of Bloomfield c/o AT&T Mobility.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

Attached to accommodate this filing are construction drawings dated March 16, 2018 by Hudson Design Group LLC, a structural analysis dated February 8, 2018 by Crown Castle and an Emissions Analysis Report dated February 28, 2018 by Centerline Communications, LLC.

1. The proposed modifications will not result in an increase in the height of the existing structure.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.
5. The proposed modifications will not cause an ineligible change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading, pursuant to the structural analysis by Crown Castle, dated February 8, 2018.

For the foregoing reasons, AT&T respectfully submits that the proposed modifications to the above referenced telecommunications facility constitute an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

---

Mary Caulfield, Site Acquisition Consultant  
c/o New Cingular Wireless, PCS LLC (AT&T)  
Centerline Communications, LLC  
95 Ryan Drive, Suite 1  
Raynham, MA 02767  
Mobile: (978) 994-0252  
[MCaulfield@centerlinecommunications.com](mailto:MCaulfield@centerlinecommunications.com)

cc: Donald S. Trinks, Mayor, Town of Windsor  
Eric Barz, AICP, Town Planner, Town of Windsor  
Crown Castle, Tower Owner  
Genesis Health Ventures of Bloomfield c/o AT&T Mobility



Date: **February 08, 2018**

Cheryl Schultz  
Crown Castle  
3530 Toringdon Way, Suite 300  
Charlotte, NC 28277

Crown Castle  
2000 Corporate Drive  
Canonsburg, PA 15317  
(724) 416-2000

**Subject: Structural Analysis Report**

**Carrier Designation:** **AT&T Mobility Co-Locate**  
**Carrier Site Number:** CT5137  
**Carrier Site Name:** Windsor Matianuck Ave

**Crown Castle Designation:** **Crown Castle BU Number:** 842878  
**Crown Castle Site Name:** WINDSOR SOUTH  
**Crown Castle JDE Job Number:** 478489  
**Crown Castle Work Order Number:** 1522339  
**Crown Castle Application Number:** 421272 Rev. 1

**Engineering Firm Designation:** **Crown Castle Project Number:** 1522339

**Site Data:** **1170 MATIANUCK AVENUE, WINDSOR, Hartford County, CT**  
**Latitude 41° 50' 22.7", Longitude -72° 40' 0"**  
**100 Foot - Monopole Tower**

Dear Cheryl Schultz,

Crown Castle is pleased to submit this **“Structural Analysis Report”** to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural ‘Statement of Work’ and the terms of Crown Castle Purchase Order Number 1522339, in accordance with application 421272, revision 1.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Existing + Proposed Equipment **Sufficient Capacity**  
Note: See Table I and Table II for the proposed and existing loading, respectively.

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 125 mph converted to a nominal 3-second gust wind speed of 97 mph per Section 1609.3 and Appendix N as required for use in the TIA-222-G Standard per Exception #5 of Section 1609.1.1. Exposure Category C and Risk Category II were used in this analysis.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

We at *Crown Castle* appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects, please give us a call.

Structural analysis prepared by: Kenneth Sukitch, E.I.T./ KB

Respectfully submitted by:

Bradley E. Byrom, P.E., S.E.  
Senior Project Engineer



## TABLE OF CONTENTS

### 1) INTRODUCTION

### 2) ANALYSIS CRITERIA

Table 1 - Proposed Antenna and Cable Information

Table 2 - Existing Antenna and Cable Information

Table 3 - Design Antenna and Cable Information

### 3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

3.1) Analysis Method

3.2) Assumptions

### 4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Table 6 – Tower Component Stresses vs. Capacity – LC5

4.1) Recommendations

### 5) APPENDIX A

tnxTower Output

### 6) APPENDIX B

Base Level Drawing

### 7) APPENDIX C

Additional Calculations



## 1) INTRODUCTION

This tower is a 100 ft Monopole tower designed by ENGINEERED ENDEAVORS, INC. in April of 2002. The tower was originally designed for a wind speed of 80 mph per TIA/EIA-222-F.

## 2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA-222-G Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a 3-second gust wind speed of 97 mph with no ice, 50 mph with 1 inch ice thickness and 60 mph under service loads, exposure category C.

**Table 1 - Proposed Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
97.0	97.0	1	andrew	SBNHH-1D65A w/ Mount Pipe	1 2	3/8 3/4	-
		1	cci antennas	HPA-65R-BUU-H6 w/ Mount Pipe			
		1	cci antennas	HPA-65R-BUU-H8 w/ Mount Pipe			
		1	cci antennas	TPA-65R-LCUUUU-H8 w/ Mount Pipe			
		3	ericsson	RRUS 32 B2			
		3	ericsson	RRUS 32 B30			
		3	powerwave technologies	LGP21401			
		1	quintel technology	QS46512-2 w/ Mount Pipe			
		1	quintel technology	QS66512-2 w/ Mount Pipe			
		1	raycap	DC6-48-60-18-8F			
		1	tower mounts	T-Arm Mount [TA 602-3]			

**Table 2 - Existing Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
97.0	97.0	2	kmw communications	AM-X-CD-16-65-00T-RET w/ Mount Pipe	-	-	2
		1	powerwave technologies	P65-17-XLH-RR w/ Mount Pipe			
		1	tower mounts	T-Arm Mount [TA 702-3]			
		3	ericsson	RRUS 11 B12	1 2 6	3/8 3/4 7/8	1
		3	kathrein	800 10121 w/ Mount Pipe			
		3	powerwave technologies	LGP21401			
		1	raycap	DC6-48-60-18-8F			

Notes:

- 1) Existing Equipment
- 2) Equipment To Be Removed; Not Considered in This Analysis

**Table 3 - Design Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
97	97	6	swedcom	7250	-	-
87	87	12	swedcom	ALP 9212	-	-
77	77	12	swedcom	ALP 9212	-	-

### 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided**

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Tectonic	4713265	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Engineered Endeavors Incorporated	4858947	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Engineered Endeavors Incorporated	6100542	CCISITES

#### 3.1) Analysis Method

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

#### 3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	100 - 80.487	Pole	TP21.1099x15x0.1875	1	-6.14	867.74	55.8	Pass
L2	80.487 - 42.779	Pole	TP32.4173x19.7523x0.3125	2	-15.57	2261.84	79.6	Pass
L3	42.779 - 0	Pole	TP45x30.3809x0.375	3	-26.72	3758.77	79.4	Pass
							Summary	
						Pole (L2)	79.6	Pass
						Rating =	79.6	Pass

**Table 6 - Tower Component Stresses vs. Capacity – LC5**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	80.4	Pass
1	Base Plate Stiffeners	0	99.9	Pass
1	Base Plate	0	73.9	Pass
1	Base Foundation	0	70.1	Pass
1	Base Foundation Soil Interaction	0	61.3	Pass

<b>Structure Rating (max from all components) =</b>	<b>99.9%</b>
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Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

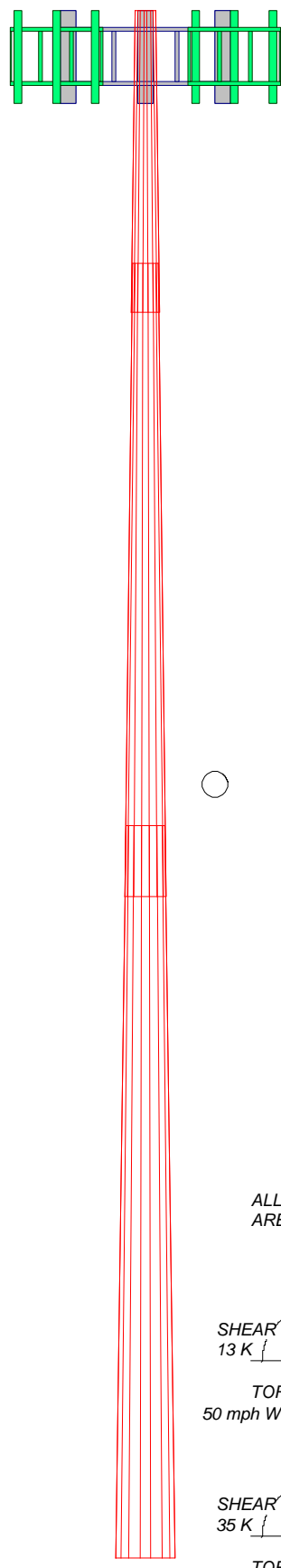
#### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time

**APPENDIX A**  
**TNXTOWER OUTPUT**

Section	1	2	3	11.4
Length (ft)	19.51	40.85	47.33	
Number of Sides	18	18	18	
Thickness (in)	0.1875	0.3125	0.3750	
Socket Length (ft)	3.14	4.55	30.3809	
Top Dia (in)	15.0000	19.7523	45.0000	
Bot Dia (in)	21.1099	32.4173		
Grade		A572-65		
Weight (K)	0.7	3.6	7.2	

100.0 ft  
80.5 ft  
42.8 ft  
0.0 ft



### DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Pine (105)	105	LGP21401	97
800 10121 w/ Mount Pipe	97	LGP21401	97
800 10121 w/ Mount Pipe	97	LGP21401	97
800 10121 w/ Mount Pipe	97	DC6-48-60-18-8F	97
SBNHH-1D65A w/ Mount Pipe	97	RRUS 32 B2	97
QS46512-2 w/ Mount Pipe	97	RRUS 32 B2	97
HPA-65R-BUU-H8 w/ Mount Pipe	97	RRUS 32 B2	97
TPA-65R-LCUUUU-H8 w/ Mount Pipe	97	RRUS 32 B30	97
HPA-65R-BUU-H6 w/ Mount Pipe	97	RRUS 32 B30	97
QS66512-2 w/ Mount Pipe	97	RRUS 32 B30	97
LGP21401	97	T-Arm Mount [TA 602-3]	97
LGP21401	97	Pine (96)	96
LGP21401	97	Pine (88)	88
RRUS 11 B12	97	Pine (78)	78
RRUS 11 B12	97	Pine (68)	68
RRUS 11 B12	97	Pine (58)	58
DC6-48-60-18-8F	97		

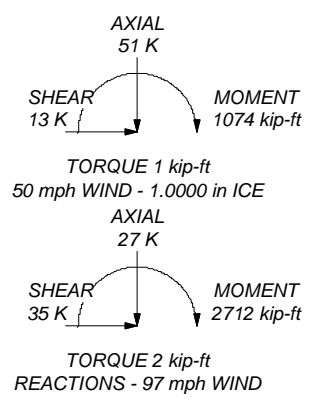
### MATERIAL STRENGTH


GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

### TOWER DESIGN NOTES

1. Tower is located in Hartford County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-G Standard.
3. Tower designed for a 97 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 79.6%

ALL REACTIONS ARE FACTORED



 <p><b>CROWN CASTLE</b> The Foundation For A Wireless World</p>	<p><b>Crown Castle</b> 2000 Corporate Drive Canonsburg, PA 15317 Phone: (724) 416-2000 FAX:</p>		<p>Job: <b>BU# 842878</b></p>
	<p>Project: <b>1522339</b></p>		<p>Client: Crown Castle</p>
	<p>Code: TIA-222-G</p>		<p>Drawn by: KGebremariam</p>
	<p>Path: C:\Users\KGebremariam\Desktop\AA\842878.eri</p>		<p>Date: 02/08/18</p>
	<p>Scale: NTS</p>		<p>App'd: [Signature]</p>

## Tower Input Data

There is a pole section.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

- 1) Tower is located in Hartford County, Connecticut.
- 2) Basic wind speed of 97 mph.
- 3) Structure Class II.
- 4) Exposure Category C.
- 5) Topographic Category 1.
- 6) Crest Height 0.00 ft.
- 7) Nominal ice thickness of 1.0000 in.
- 8) Ice thickness is considered to increase with height.
- 9) Ice density of 56 pcf.
- 10) A wind speed of 50 mph is used in combination with ice.
- 11) Temperature drop of 50 °F.
- 12) Deflections calculated using a wind speed of 60 mph.
- 13) A non-linear (P-delta) analysis was used.
- 14) Pressures are calculated at each section.
- 15) Stress ratio used in pole design is 1.
- 16) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification ✓ Use Code Stress Ratios ✓ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile  Include Bolts In Member Capacity  Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric	Distribute Leg Loads As Uniform Assume Legs Pinned ✓ Assume Rigid Index Plate ✓ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension ✓ Bypass Mast Stability Checks ✓ Use Azimuth Dish Coefficients ✓ Project Wind Area of Appurt.  Autocalc Torque Arm Areas  Add IBC .6D+W Combination ✓ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder	Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation ✓ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption Use TIA-222-G Tension Splice Exemption  <div style="text-align: center; background-color: #e0e0e0; padding: 2px;"><b>Poles</b></div> ✓ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
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## Tapered Pole Section Geometry

Section	Elevation <small>ft</small>	Section Length <small>ft</small>	Splice Length <small>ft</small>	Number of Sides	Top Diameter <small>in</small>	Bottom Diameter <small>in</small>	Wall Thickness <small>in</small>	Bend Radius <small>in</small>	Pole Grade
L1	100.00-80.49	19.51	3.14	18	15.0000	21.1099	0.1875	0.7500	A572-65 (65 ksi)
L2	80.49-42.78	40.85	4.55	18	19.7523	32.4173	0.3125	1.2500	A572-65 (65 ksi)
L3	42.78-0.00	47.33		18	30.3809	45.0000	0.3750	1.5000	A572-65 (65 ksi)

### Tapered Pole Properties

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	It/Q in <sup>2</sup>	w in	w/t
L1	15.2314	8.8153	244.3603	5.2584	7.6200	32.0683	489.0422	4.4085	2.3100	12.32
	21.4356	12.4514	688.6208	7.4275	10.7238	64.2141	1378.1477	6.2269	3.3853	18.055
L2	21.0450	19.2819	920.6019	6.9011	10.0342	91.7466	1842.4151	9.6428	2.9264	9.365
	32.9174	31.8439	4146.7161	11.3972	16.4680	251.8047	8298.8885	15.9250	5.1554	16.497
L3	32.2772	35.7145	4062.5121	10.6521	15.4335	263.2271	8130.3697	17.8606	4.6870	12.499
	45.6942	53.1149	13363.195	15.8419	22.8600	584.5667	26743.975	26.5625	7.2600	19.36
			7				0			

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>r</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in					in	in	in
L1 100.00-80.49				1	1	1			
L2 80.49-42.78				1	1	1			
L3 42.78-0.00				1	1	1			

### Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	Number Per Row	Clear Spacing	Width or Diameter	Perimeter	Weight
				ft			in	r	r	plf
							in	in	in	
***										

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	C <sub>A</sub> A <sub>A</sub>	Weight
				ft		ft <sup>2</sup> /ft	plf
*** 97 ft ***							
LDF5-50A(7/8)	B	No	Inside Pole	97.00 - 0.00	6	No Ice 1/2" Ice 1" Ice	0.33 0.33 0.33
FB-L98B-034-XXX(3/8)	B	No	Inside Pole	97.00 - 0.00	1	No Ice 1/2" Ice 1" Ice	0.06 0.06 0.06
WR-VG86ST-BRD(3/4)	B	No	Inside Pole	97.00 - 0.00	2	No Ice 1/2" Ice 1" Ice	0.58 0.58 0.58
2" Flex Conduit	B	No	Inside Pole	97.00 - 0.00	1	No Ice 1/2" Ice 1" Ice	0.36 0.36 0.36
FB-L98B-034-XXX(3/8)	B	No	Inside Pole	97.00 - 0.00	1	No Ice 1/2" Ice 1" Ice	0.06 0.06 0.06
WR-VG86ST-BRD(3/4)	B	No	Inside Pole	97.00 - 0.00	2	No Ice 1/2" Ice 1" Ice	0.58 0.58 0.58
***							

### Feed Line/Linear Appurtenances Section Areas

Tower Section n	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
L1	100.00-80.49	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.08
		C	0.000	0.000	0.000	0.000	0.00
L2	80.49-42.78	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.18
		C	0.000	0.000	0.000	0.000	0.00
L3	42.78-0.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.20
		C	0.000	0.000	0.000	0.000	0.00

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section n	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
L1	100.00-80.49	A	2.210	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.08
		C		0.000	0.000	0.000	0.000	0.00
L2	80.49-42.78	A	2.125	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.18
		C		0.000	0.000	0.000	0.000	0.00
L3	42.78-0.00	A	1.912	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.20
		C		0.000	0.000	0.000	0.000	0.00

### Feed Line Center of Pressure

Section	Elevation ft	CP <sub>x</sub> in	CP <sub>z</sub> in	CP <sub>x</sub> Ice in	CP <sub>z</sub> Ice in
L1	100.00-80.49	0.0000	0.0000	0.0000	0.0000
L2	80.49-42.78	0.0000	0.0000	0.0000	0.0000
L3	42.78-0.00	0.0000	0.0000	0.0000	0.0000

### Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight K
*** BRANCHES ***								
Pine (105)	C	None		0.0000	105.00	No Ice 45.00 1/2" 60.00 Ice 75.00 1" 75.00	45.00 60.00 75.00	0.75 0.85 0.95
Pine (96)	C	None		0.0000	96.00	No Ice 90.00	90.00	1.50



Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight	
			Horz	Lateral			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
						1/2" Ice	120.00	120.00	1.90	
						1" Ice	150.00	150.00	2.30	
Pine (88)	C	None			0.0000	88.00	No Ice	90.00	1.50	
						1/2" Ice	120.00	120.00	1.90	
						1" Ice	150.00	150.00	2.30	
Pine (78)	C	None			0.0000	78.00	No Ice	90.00	1.50	
						1/2" Ice	120.00	120.00	1.90	
						1" Ice	150.00	150.00	2.30	
Pine (68)	C	None			0.0000	68.00	No Ice	90.00	1.50	
						1/2" Ice	120.00	120.00	1.90	
						1" Ice	150.00	150.00	2.30	
Pine (58)	C	None			0.0000	58.00	No Ice	90.00	1.50	
						1/2" Ice	120.00	120.00	1.90	
						1" Ice	150.00	150.00	2.30	
*** 97 ft ***										
800 10121 w/ Mount Pipe	A	From Leg	4.00		0.0000	97.00	No Ice	5.39	4.60	0.07
			0.00				1/2" Ice	5.81	5.35	0.11
			0.00				1" Ice	6.23	6.05	0.17
800 10121 w/ Mount Pipe	B	From Leg	4.00		0.0000	97.00	No Ice	5.39	4.60	0.07
			0.00				1/2" Ice	5.81	5.35	0.11
			0.00				1" Ice	6.23	6.05	0.17
800 10121 w/ Mount Pipe	C	From Leg	4.00		0.0000	97.00	No Ice	5.39	4.60	0.07
			0.00				1/2" Ice	5.81	5.35	0.11
			0.00				1" Ice	6.23	6.05	0.17
SBNHH-1D65A w/ Mount Pipe	A	From Leg	4.00		0.0000	97.00	No Ice	5.95	5.19	0.06
			0.00				1/2" Ice	6.39	5.96	0.11
			0.00				1" Ice	6.82	6.66	0.17
QS46512-2 w/ Mount Pipe	A	From Leg	4.00		0.0000	97.00	No Ice	5.79	5.88	0.12
			0.00				1/2" Ice	6.21	6.58	0.18
			0.00				1" Ice	6.62	7.25	0.24
HPA-65R-BUU-H8 w/ Mount Pipe	B	From Leg	4.00		0.0000	97.00	No Ice	13.21	9.58	0.10
			0.00				1/2" Ice	13.90	11.05	0.20
			0.00				1" Ice	14.59	12.50	0.30
TPA-65R-LCUUUU-H8 w/ Mount Pipe	B	From Leg	4.00		0.0000	97.00	No Ice	13.54	10.96	0.11
			0.00				1/2" Ice	14.24	12.49	0.22
			0.00				1" Ice	14.95	14.04	0.33
HPA-65R-BUU-H6 w/ Mount Pipe	C	From Leg	4.00		0.0000	97.00	No Ice	9.90	8.11	0.08
			0.00				1/2" Ice	10.47	9.30	0.16
			0.00				1" Ice	11.01	10.21	0.25
QS66512-2 w/ Mount Pipe	C	From Leg	4.00		0.0000	97.00	No Ice	8.37	8.46	0.14
			0.00				1/2" Ice	8.93	9.66	0.21
			0.00				1" Ice	9.46	10.55	0.30
LGP21401	A	From Leg	4.00		0.0000	97.00	No Ice	0.00	0.21	0.01
			0.00				1/2" Ice	0.00	0.27	0.02
			0.00				1" Ice	0.00	0.35	0.03
LGP21401	B	From Leg	4.00		0.0000	97.00	No Ice	0.00	0.21	0.01
			0.00				1/2" Ice	0.00	0.27	0.02
			0.00				1" Ice	0.00	0.35	0.03
LGP21401	C	From Leg	4.00		0.0000	97.00	No Ice	0.00	0.21	0.01

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	CAAA	CAAA	Weight
			Horz	Lateral			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
			ft	ft					
			0.00			1/2"	0.00	0.27	0.02
			0.00			Ice	0.00	0.35	0.03
RRUS 11 B12	A	From Leg	4.00	0.0000	97.00	1" Ice	0.00	1.18	0.05
			0.00			No Ice	0.00	1.33	0.07
			0.00			1/2"	0.00	1.48	0.10
						Ice			
						1" Ice			
RRUS 11 B12	B	From Leg	4.00	0.0000	97.00	No Ice	0.00	1.18	0.05
			0.00			1/2"	0.00	1.33	0.07
			0.00			Ice	0.00	1.48	0.10
						1" Ice			
RRUS 11 B12	C	From Leg	4.00	0.0000	97.00	No Ice	0.00	1.18	0.05
			0.00			1/2"	0.00	1.33	0.07
			0.00			Ice	0.00	1.48	0.10
						1" Ice			
DC6-48-60-18-8F	A	From Leg	4.00	0.0000	97.00	No Ice	0.79	0.79	0.02
			0.00			1/2"	1.27	1.27	0.04
			0.00			Ice	1.45	1.45	0.05
						1" Ice			
LGP21401	A	From Leg	4.00	0.0000	97.00	No Ice	0.00	0.21	0.01
			0.00			1/2"	0.00	0.27	0.02
			0.00			Ice	0.00	0.35	0.03
						1" Ice			
LGP21401	B	From Leg	4.00	0.0000	97.00	No Ice	0.00	0.21	0.01
			0.00			1/2"	0.00	0.27	0.02
			0.00			Ice	0.00	0.35	0.03
						1" Ice			
LGP21401	C	From Leg	4.00	0.0000	97.00	No Ice	0.00	0.21	0.01
			0.00			1/2"	0.00	0.27	0.02
			0.00			Ice	0.00	0.35	0.03
						1" Ice			
DC6-48-60-18-8F	B	From Leg	4.00	0.0000	97.00	No Ice	0.79	0.79	0.02
			0.00			1/2"	1.27	1.27	0.04
			0.00			Ice	1.45	1.45	0.05
						1" Ice			
RRUS 32 B2	A	From Leg	4.00	0.0000	97.00	No Ice	2.73	1.67	0.05
			0.00			1/2"	2.95	1.86	0.07
			0.00			Ice	3.18	2.05	0.10
						1" Ice			
RRUS 32 B2	B	From Leg	4.00	0.0000	97.00	No Ice	2.73	1.67	0.05
			0.00			1/2"	2.95	1.86	0.07
			0.00			Ice	3.18	2.05	0.10
						1" Ice			
RRUS 32 B2	C	From Leg	4.00	0.0000	97.00	No Ice	2.73	1.67	0.05
			0.00			1/2"	2.95	1.86	0.07
			0.00			Ice	3.18	2.05	0.10
						1" Ice			
RRUS 32 B30	A	From Leg	4.00	0.0000	97.00	No Ice	2.69	1.57	0.06
			0.00			1/2"	2.91	1.76	0.08
			0.00			Ice	3.14	1.95	0.10
						1" Ice			
RRUS 32 B30	B	From Leg	4.00	0.0000	97.00	No Ice	2.69	1.57	0.06
			0.00			1/2"	2.91	1.76	0.08
			0.00			Ice	3.14	1.95	0.10
						1" Ice			
RRUS 32 B30	C	From Leg	4.00	0.0000	97.00	No Ice	2.69	1.57	0.06
			0.00			1/2"	2.91	1.76	0.08
			0.00			Ice	3.14	1.95	0.10
						1" Ice			
T-Arm Mount [TA 602-3]	C	None		0.0000	97.00	No Ice	11.59	11.59	0.77
						1/2"	15.44	15.44	0.99
						Ice	19.29	19.29	1.21
						1" Ice			
*****									
*****									

### Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	100 - 80.487	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-18.72	-1.41	-1.62
			Max. Mx	8	-6.15	-194.48	-1.13

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L2	80.487 - 42.779	Pole	Max. My	14	-6.18	-1.08	-192.77
			Max. Vy	8	16.92	-194.48	-1.13
			Max. Vx	14	16.79	-1.08	-192.77
			Max. Torque	18			1.56
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-36.23	-1.45	-1.67
			Max. Mx	8	-15.57	-1120.43	-4.21
			Max. My	14	-15.59	-4.12	-1113.74
			Max. Vy	8	32.12	-1120.43	-4.21
			Max. Vx	14	31.99	-4.12	-1113.74
L3	42.779 - 0	Pole	Max. Torque	18			1.55
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-50.51	-1.40	-1.61
			Max. Mx	8	-26.72	-2708.67	-8.05
			Max. My	14	-26.72	-7.96	-2695.59
			Max. Vy	8	34.93	-2708.67	-8.05
			Max. Vx	14	34.79	-7.96	-2695.59
			Max. Torque	18			1.55

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	26	50.51	0.00	0.00
	Max. H <sub>x</sub>	21	20.07	34.89	0.08
	Max. H <sub>z</sub>	3	20.07	0.08	34.76
	Max. M <sub>x</sub>	2	2695.30	0.08	34.76
	Max. M <sub>z</sub>	8	2708.67	-34.89	-0.08
	Max. Torsion	18	1.54	30.18	-17.31
	Min. Vert	9	20.07	-34.89	-0.08
	Min. H <sub>x</sub>	9	20.07	-34.89	-0.08
	Min. H <sub>z</sub>	15	20.07	-0.08	-34.76
	Min. M <sub>x</sub>	14	-2695.59	-0.08	-34.76
	Min. M <sub>z</sub>	20	-2708.57	34.89	0.08
	Min. Torsion	6	-1.53	-30.18	17.31

### Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	22.30	0.00	0.00	0.12	-0.04	0.00
1.2 Dead+1.6 Wind 0 deg - No Ice	26.76	-0.08	-34.76	-2695.30	7.87	0.77
0.9 Dead+1.6 Wind 0 deg - No Ice	20.07	-0.08	-34.76	-2677.15	7.81	0.77
1.2 Dead+1.6 Wind 30 deg - No Ice	26.76	17.38	-30.07	-2330.31	-1347.58	1.33
0.9 Dead+1.6 Wind 30 deg - No Ice	20.07	17.38	-30.07	-2314.60	-1338.46	1.33
1.2 Dead+1.6 Wind 60 deg - No Ice	26.76	30.18	-17.31	-1340.77	-2341.93	1.53
0.9 Dead+1.6 Wind 60 deg - No Ice	20.07	30.18	-17.31	-1331.75	-2326.08	1.53
1.2 Dead+1.6 Wind 90 deg - No Ice	26.76	34.89	0.08	8.05	-2708.67	1.32
0.9 Dead+1.6 Wind 90 deg - No Ice	20.07	34.89	0.08	7.95	-2690.37	1.32
1.2 Dead+1.6 Wind 120 deg	26.76	30.26	17.45	1354.72	-2349.80	0.77

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
- No Ice						
0.9 Dead+1.6 Wind 120 deg - No Ice	20.07	30.26	17.45	1345.52	-2333.89	0.77
1.2 Dead+1.6 Wind 150 deg - No Ice	26.76	17.52	30.15	2338.46	-1361.26	0.00
0.9 Dead+1.6 Wind 150 deg - No Ice	20.07	17.52	30.15	2322.61	-1352.03	0.00
1.2 Dead+1.6 Wind 180 deg - No Ice	26.76	0.08	34.76	2695.59	-7.96	-0.76
0.9 Dead+1.6 Wind 180 deg - No Ice	20.07	0.08	34.76	2677.37	-7.88	-0.76
1.2 Dead+1.6 Wind 210 deg - No Ice	26.76	-17.38	30.07	2330.60	1347.49	-1.33
0.9 Dead+1.6 Wind 210 deg - No Ice	20.07	-17.38	30.07	2314.81	1338.40	-1.33
1.2 Dead+1.6 Wind 240 deg - No Ice	26.76	-30.18	17.31	1341.04	2341.84	-1.54
0.9 Dead+1.6 Wind 240 deg - No Ice	20.07	-30.18	17.31	1331.95	2326.02	-1.54
1.2 Dead+1.6 Wind 270 deg - No Ice	26.76	-34.89	-0.08	-7.77	2708.57	-1.33
0.9 Dead+1.6 Wind 270 deg - No Ice	20.07	-34.89	-0.08	-7.75	2690.29	-1.33
1.2 Dead+1.6 Wind 300 deg - No Ice	26.76	-30.26	-17.45	-1354.44	2349.69	-0.76
0.9 Dead+1.6 Wind 300 deg - No Ice	20.07	-30.26	-17.45	-1345.31	2333.81	-0.76
1.2 Dead+1.6 Wind 330 deg - No Ice	26.76	-17.52	-30.15	-2338.17	1361.16	0.01
0.9 Dead+1.6 Wind 330 deg - No Ice	20.07	-17.52	-30.15	-2322.40	1351.96	0.01
1.2 Dead+1.0 Ice+1.0 Temp	50.51	-0.00	-0.00	1.61	-1.40	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	50.51	-0.01	-13.28	-1069.07	-0.71	0.39
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	50.51	6.64	-11.50	-925.21	-536.66	0.55
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	50.51	11.51	-6.63	-532.93	-929.22	0.57
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	50.51	13.29	0.01	2.62	-1073.19	0.43
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	50.51	11.51	6.65	537.95	-930.06	0.18
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	50.51	6.65	11.51	929.61	-538.11	-0.12
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	50.51	0.01	13.28	1072.63	-2.39	-0.39
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	50.51	-6.64	11.50	928.77	533.56	-0.55
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	50.51	-11.51	6.63	536.50	926.12	-0.57
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	50.51	-13.29	-0.01	0.95	1070.09	-0.43
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	50.51	-11.51	-6.65	-534.38	926.96	-0.18
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	50.51	-6.65	-11.51	-926.04	535.01	0.12
Dead+Wind 0 deg - Service	22.30	-0.02	-7.44	-574.91	1.64	0.17
Dead+Wind 30 deg - Service	22.30	3.72	-6.43	-497.03	-287.51	0.29
Dead+Wind 60 deg - Service	22.30	6.46	-3.70	-285.94	-499.64	0.33
Dead+Wind 90 deg - Service	22.30	7.47	0.02	1.81	-577.90	0.29
Dead+Wind 120 deg - Service	22.30	6.47	3.73	289.10	-501.33	0.17
Dead+Wind 150 deg - Service	22.30	3.75	6.45	498.96	-290.43	-0.00
Dead+Wind 180 deg - Service	22.30	0.02	7.44	575.16	-1.73	-0.17
Dead+Wind 210 deg - Service	22.30	-3.72	6.43	497.28	287.43	-0.29
Dead+Wind 240 deg - Service	22.30	-6.46	3.70	286.18	499.55	-0.33

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Service						
Dead+Wind 270 deg - Service	22.30	-7.47	-0.02	-1.57	577.82	-0.29
Dead+Wind 300 deg - Service	22.30	-6.47	-3.73	-288.86	501.24	-0.17
Dead+Wind 330 deg - Service	22.30	-3.75	-6.45	-498.72	290.35	0.00

## Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-22.30	0.00	0.00	22.30	-0.00	0.000%
2	-0.08	-26.76	-34.76	0.08	26.76	34.76	0.002%
3	-0.08	-20.07	-34.76	0.08	20.07	34.76	0.002%
4	17.38	-26.76	-30.07	-17.38	26.76	30.07	0.000%
5	17.38	-20.07	-30.07	-17.38	20.07	30.07	0.000%
6	30.18	-26.76	-17.31	-30.18	26.76	17.31	0.000%
7	30.18	-20.07	-17.31	-30.18	20.07	17.31	0.000%
8	34.90	-26.76	0.08	-34.89	26.76	-0.08	0.002%
9	34.90	-20.07	0.08	-34.89	20.07	-0.08	0.002%
10	30.26	-26.76	17.45	-30.26	26.76	-17.45	0.000%
11	30.26	-20.07	17.45	-30.26	20.07	-17.45	0.000%
12	17.52	-26.76	30.15	-17.52	26.76	-30.15	0.000%
13	17.52	-20.07	30.15	-17.52	20.07	-30.15	0.000%
14	0.08	-26.76	34.76	-0.08	26.76	-34.76	0.002%
15	0.08	-20.07	34.76	-0.08	20.07	-34.76	0.002%
16	-17.38	-26.76	30.07	17.38	26.76	-30.07	0.000%
17	-17.38	-20.07	30.07	17.38	20.07	-30.07	0.000%
18	-30.18	-26.76	17.31	30.18	26.76	-17.31	0.000%
19	-30.18	-20.07	17.31	30.18	20.07	-17.31	0.000%
20	-34.90	-26.76	-0.08	34.89	26.76	0.08	0.002%
21	-34.90	-20.07	-0.08	34.89	20.07	0.08	0.002%
22	-30.26	-26.76	-17.45	30.26	26.76	17.45	0.000%
23	-30.26	-20.07	-17.45	30.26	20.07	17.45	0.000%
24	-17.52	-26.76	-30.15	17.52	26.76	30.15	0.000%
25	-17.52	-20.07	-30.15	17.52	20.07	30.15	0.000%
26	0.00	-50.51	0.00	0.00	50.51	0.00	0.003%
27	-0.01	-50.51	-13.28	0.01	50.51	13.28	0.001%
28	6.64	-50.51	-11.50	-6.64	50.51	11.50	0.000%
29	11.51	-50.51	-6.63	-11.51	50.51	6.63	0.000%
30	13.29	-50.51	0.01	-13.29	50.51	-0.01	0.001%
31	11.51	-50.51	6.65	-11.51	50.51	-6.65	0.000%
32	6.65	-50.51	11.51	-6.65	50.51	-11.51	0.000%
33	0.01	-50.51	13.28	-0.01	50.51	-13.28	0.001%
34	-6.64	-50.51	11.50	6.64	50.51	-11.50	0.000%
35	-11.51	-50.51	6.63	11.51	50.51	-6.63	0.000%
36	-13.29	-50.51	-0.01	13.29	50.51	0.01	0.001%
37	-11.51	-50.51	-6.65	11.51	50.51	6.65	0.000%
38	-6.65	-50.51	-11.51	6.65	50.51	11.51	0.000%
39	-0.02	-22.30	-7.44	0.02	22.30	7.44	0.003%
40	3.72	-22.30	-6.43	-3.72	22.30	6.43	0.003%
41	6.46	-22.30	-3.70	-6.46	22.30	3.70	0.003%
42	7.47	-22.30	0.02	-7.47	22.30	-0.02	0.003%
43	6.47	-22.30	3.73	-6.47	22.30	-3.73	0.003%
44	3.75	-22.30	6.45	-3.75	22.30	-6.45	0.003%
45	0.02	-22.30	7.44	-0.02	22.30	-7.44	0.003%
46	-3.72	-22.30	6.43	3.72	22.30	-6.43	0.003%
47	-6.46	-22.30	3.70	6.46	22.30	-3.70	0.003%
48	-7.47	-22.30	-0.02	7.47	22.30	0.02	0.003%
49	-6.47	-22.30	-3.73	6.47	22.30	3.73	0.003%
50	-3.75	-22.30	-6.45	3.75	22.30	6.45	0.003%

### Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	6	0.00000001	0.00000001
2	Yes	12	0.00000001	0.00007721
3	Yes	12	0.00000001	0.00005824
4	Yes	15	0.00000001	0.00005427
5	Yes	14	0.00000001	0.00013193
6	Yes	15	0.00000001	0.00005066
7	Yes	14	0.00000001	0.00012290
8	Yes	12	0.00000001	0.00014102
9	Yes	12	0.00000001	0.00010502
10	Yes	15	0.00000001	0.00005450
11	Yes	14	0.00000001	0.00013232
12	Yes	15	0.00000001	0.00005338
13	Yes	14	0.00000001	0.00012956
14	Yes	12	0.00000001	0.00009890
15	Yes	12	0.00000001	0.00007397
16	Yes	15	0.00000001	0.00005086
17	Yes	14	0.00000001	0.00012343
18	Yes	15	0.00000001	0.00005473
19	Yes	14	0.00000001	0.00013301
20	Yes	12	0.00000001	0.00011497
21	Yes	12	0.00000001	0.00008606
22	Yes	15	0.00000001	0.00005245
23	Yes	14	0.00000001	0.00012726
24	Yes	15	0.00000001	0.00005332
25	Yes	14	0.00000001	0.00012945
26	Yes	6	0.00000001	0.00002962
27	Yes	13	0.00000001	0.00005817
28	Yes	14	0.00000001	0.00006181
29	Yes	14	0.00000001	0.00005721
30	Yes	13	0.00000001	0.00005920
31	Yes	14	0.00000001	0.00006151
32	Yes	14	0.00000001	0.00006124
33	Yes	13	0.00000001	0.00005879
34	Yes	14	0.00000001	0.00005739
35	Yes	14	0.00000001	0.00006206
36	Yes	13	0.00000001	0.00005863
37	Yes	14	0.00000001	0.00005793
38	Yes	14	0.00000001	0.00005814
39	Yes	11	0.00000001	0.00007163
40	Yes	11	0.00000001	0.00010171
41	Yes	11	0.00000001	0.00007553
42	Yes	11	0.00000001	0.00007546
43	Yes	11	0.00000001	0.00009690
44	Yes	11	0.00000001	0.00008787
45	Yes	11	0.00000001	0.00007199
46	Yes	11	0.00000001	0.00007639
47	Yes	11	0.00000001	0.00010540
48	Yes	11	0.00000001	0.00007495
49	Yes	11	0.00000001	0.00008131
50	Yes	11	0.00000001	0.00008746

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	100 - 80.487	15.505	43	1.4482	0.0070
L2	83.625 - 42.779	10.761	43	1.2669	0.0031
L3	47.331 - 0	3.251	43	0.6601	0.0007

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
105.00	Pine (105)	43	15.505	1.4482	0.0070	12110
97.00	800 10121 w/ Mount Pipe	43	14.610	1.4180	0.0062	12110
96.00	Pine (96)	43	14.312	1.4079	0.0060	12110
88.00	Pine (88)	43	11.979	1.3212	0.0040	5046
78.00	Pine (78)	43	9.284	1.1872	0.0022	3527
68.00	Pine (68)	43	6.926	1.0241	0.0013	3261
58.00	Pine (58)	43	4.937	0.8470	0.0009	3033

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	100 - 80.487	72.528	10	6.7856	0.0319
L2	83.625 - 42.779	50.376	10	5.9390	0.0139
L3	47.331 - 0	15.236	10	3.0953	0.0034

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
105.00	Pine (105)	10	72.528	6.7856	0.0331	2661
97.00	800 10121 w/ Mount Pipe	10	68.351	6.6449	0.0293	2661
96.00	Pine (96)	10	66.962	6.5974	0.0281	2661
88.00	Pine (88)	10	56.067	6.1927	0.0188	1107
78.00	Pine (78)	10	43.471	5.5659	0.0104	770
68.00	Pine (68)	10	32.444	4.8018	0.0059	707
58.00	Pine (58)	10	23.131	3.9714	0.0041	654

### Compression Checks

### Pole Design Data

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub>
L1	100 - 80.487 (1)	TP21.1099x15x0.1875	19.51	0.00	0.0	11.866 7	-6.14	867.74	0.007
L2	80.487 - 42.779 (2)	TP32.4173x19.7523x0.31 25	40.85	0.00	0.0	30.444 0	-15.57	2261.84	0.007
L3	42.779 - 0 (3)	TP45x30.3809x0.375	47.33	0.00	0.0	53.114 9	-26.72	3758.77	0.007

### Pole Bending Design Data



Section No.	Elevation ft	Size	$M_{ux}$ kip-ft	$\phi M_{nx}$ kip-ft	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	$M_{uy}$ kip-ft	$\phi M_{ny}$ kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
L1	100 - 80.487 (1)	TP21.1099x15x0.1875	195.04	355.26	0.549	0.00	355.26	0.000
L2	80.487 - 42.779 (2)	TP32.4173x19.7523x0.31 25	1122.34	1424.29	0.788	0.00	1424.29	0.000
L3	42.779 - 0 (3)	TP45x30.3809x0.375	2712.35	3447.32	0.787	0.00	3447.32	0.000

### Pole Shear Design Data

Section No.	Elevation ft	Size	Actual $V_u$ K	$\phi V_n$ K	Ratio $\frac{V_u}{\phi V_n}$	Actual $T_u$ kip-ft	$\phi T_n$ kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L1	100 - 80.487 (1)	TP21.1099x15x0.1875	16.96	433.87	0.039	0.77	711.38	0.001
L2	80.487 - 42.779 (2)	TP32.4173x19.7523x0.31 25	32.16	1130.92	0.028	0.77	2852.07	0.000
L3	42.779 - 0 (3)	TP45x30.3809x0.375	34.96	1879.39	0.019	0.77	6903.08	0.000

### Pole Interaction Design Data

Section No.	Elevation ft	Ratio $P_u$	Ratio $M_{ux}$	Ratio $M_{uy}$	Ratio $V_u$	Ratio $T_u$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
		$\phi P_n$	$\phi M_{nx}$	$\phi M_{ny}$	$\phi V_n$	$\phi T_n$			
L1	100 - 80.487 (1)	0.007	0.549	0.000	0.039	0.001	0.558	1.000	4.8.2
L2	80.487 - 42.779 (2)	0.007	0.788	0.000	0.028	0.000	0.796	1.000	4.8.2
L3	42.779 - 0 (3)	0.007	0.787	0.000	0.019	0.000	0.794	1.000	4.8.2

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
L1	100 - 80.487	Pole	TP21.1099x15x0.1875	1	-6.14	867.74	55.8	Pass
L2	80.487 - 42.779	Pole	TP32.4173x19.7523x0.3125	2	-15.57	2261.84	79.6	Pass
L3	42.779 - 0	Pole	TP45x30.3809x0.375	3	-26.72	3758.77	79.4	Pass
Summary								
Pole (L2)							79.6	Pass
<b>RATING =</b>							<b>79.6</b>	<b>Pass</b>

**APPENDIX B**  
**BASE LEVEL DRAWING**

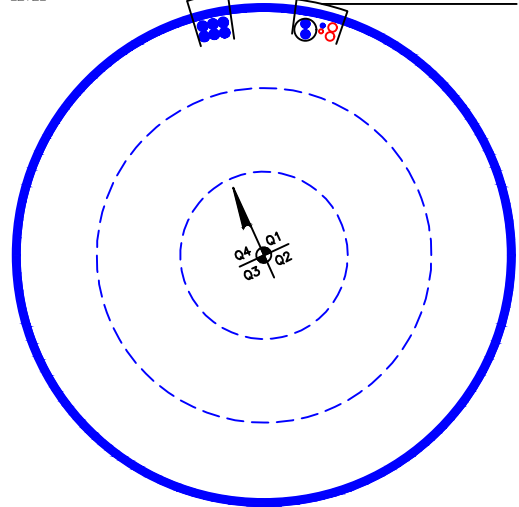


(INSTALLED)  
(6) 7/8" TO 97 FT LEVEL



(PROPOSED)  
(1) 3/8" TO 97 FT LEVEL  
(2) 3/4" TO 97 FT LEVEL  
(INSTALLED-IN CONDUIT)  
(2) 3/4" TO 97 FT LEVEL

(INSTALLED)  
(1) 3/8" TO 97 FT LEVEL



**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

## Stiffened or Unstiffened, UngROUTED, Circular Base Plate - Any Rod Material

**TIA Rev G** Assumption: Clear space between bottom of leveling nut and top of concrete **not** exceeding (1)\*(Rod Diameter)

### Site Data

BU#: 842878
Site Name: WINDSOR SOUTH
App #: 421272 - Rev. 1
Pole Manufacturer: <b>Other</b>

### Anchor Rod Data

Qty:	12	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	54	in

### Plate Data

Diam:	60	in
Thick:	1.75	in
Grade:	60	ksi
Single-Rod B-eff:	11.90	in

### Stiffener Data (Welding at both sides)

Config:	1	*
Weld Type:	Fillet	
Groove Depth:	0	<-- Disregard
Groove Angle:	45	<-- Disregard
Fillet H. Weld:	0.5	in
Fillet V. Weld:	0.5	in
Width:	7.5	in
Height:	24	in
Thick:	1.25	in
Notch:	0.5	in
Grade:	50	ksi
Weld str.:	70	ksi

### Pole Data

Diam:	45	in
Thick:	0.375	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

### Reactions

Mu:	2712	ft-kips
Axial, Pu:	27	kips
Shear, Vu:	35	kips
Eta Factor, η	0.5	TIA G (Fig. 4-4)

If No stiffeners, Criteria: **AISC LRFD** <-Only Applicable to Unstiffened Cases

### Anchor Rod Results

Max Rod (Cu+ Vu/η): 209.0 Kips  
 Allowable Axial, Φ\*Fu\*Anet: 260.0 Kips  
 Anchor Rod Stress Ratio: 80.4% **Pass**

Stiffened
AISC LRFD
φ*Tn

### Base Plate Results

Base Plate Stress: 39.9 ksi  
 Allowable Plate Stress: 54.0 ksi  
 Base Plate Stress Ratio: 73.9% **Pass**

### Flexural Check

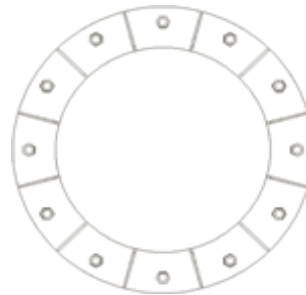
Stiffened
AISC LRFD
φ*Fy
Y.L. Length:
N/A, Roark

### Stiffener Results

Horizontal Weld : 99.9% **Pass**  
 Vertical Weld: 32.1% **Pass**  
 Plate Flex+Shear, fb/Fb+(fv/Fv)^2: 9.2% **Pass**  
 Plate Tension+Shear, ft/Ft+(fv/Fv)^2: 40.9% **Pass**  
 Plate Comp. (AISC Bracket): 43.7% **Pass**

### Pole Results

Pole Punching Shear Check: 12.0% **Pass**



\* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

\*\* Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

## Drilled Pier Foundation



BU # :	842878
Site Name:	Windsor South
App. Number:	421272 - Rev. 1

TIA-222 Revison:	G
Tower Type:	Monopole

Applied Loads		
	Comp.	Uplift
Moment (kip-ft)	2712	
Axial Force (kips)	27	
Shear Force (kips)	35	

Material Properties		
Concrete Strength, f'c:	4	ksi
Rebar Strength, Fy:	60	ksi

Pier Design Data		
Depth	39	ft
Ext. Above Grade	0.5	ft
Pier Section 1		
<i>From 0.5' above grade to 39' below grade</i>		
Pier Diameter	6.5	ft
Rebar Quantity	18	
Rebar Size	11	
Clear Cover to Ties	4	in
Tie Size	5	

Analysis Results		
Soil Lateral Capacity		
	Compression	Uplift
D <sub>v=0</sub> (ft from TOC)	7.51	-
Soil Safety Factor	2.56	-
Max Moment (kip-ft)	2953.84	-
Rating	52.0%	-
Soil Vertical Capacity		
	Compression	Uplift
Skin Friction (kips)	179.37	-
End Bearing (kips)	111.62	-
Weight of Concrete (kips)	151.45	-
Total Capacity (kips)	290.99	-
Axial (kips)	178.45	-
Rating	61.3%	-
Reinforced Concrete Capacity		
	Compression	Uplift
Critical Depth (ft from TOC)	7.62	-
Critical Moment (kip-ft)	2953.69	-
Critical Moment Capacity	4213.16	-
Rating	70.1%	-
<b>Soil Interaction Rating</b>		<b>61.3%</b>
<b>Structural Foundation Rating</b>		<b>70.1%</b>

Soil Profile			
Groundwater Depth	5	ft	# of Layers
			8

Layer	Top (ft)	Bottom (ft)	Thickness (ft)	γ <sub>soil</sub> (pcf)	γ <sub>concrete</sub> (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Comp Override (ksf)	Ultimate Skin Friction Uplift Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	2	2	110	150	0	0	0.000	0.000	0.00	0.00			Cohesionless
2	2	5	3	115	150	0	0	0.000	0.000	0.00	0.00			Cohesionless
3	5	12	7	55	87.6	0	35	0.000	0.000	0.45	0.45			Cohesionless
4	12	23	11	45	87.6	0	29	0.000	0.000	0.70	0.70			Cohesionless
5	23	28	5	40	87.6	0.4	0	0.220	0.220	0.14	0.14			Cohesive
6	28	32.5	4.5	40	87.6	0.1	0	0.055	0.055	0.04	0.04			Cohesive
7	32.5	34	1.5	40	87.6	0.1	0	0.06	0.06	0.00	0.00			Cohesive
8	34	39	5	40	87.6	0.4	0	0.22	0.22	0.00	0.00	4.485		Cohesive

# CCISeismic - Design Category

Per 2012/2015 IBC

Site BU: 842878  
 Work Order: 1522339  
 Application: 421272 Rev. 1



	Degrees	Minutes	Seconds	
Site Latitude =	41	50	22.70	41.8396 degrees
Site Longitude =	-72	40	0.00	-72.6667 degrees
Ground Supported Structure =	Yes			
Structure Class =	II			(Table 2-1)
Site Class =	D - Stiff Soil			(Table 2-11)
Spectral response acceleration short periods, $S_s$ =	0.179			<a href="#">USGS Seismic Tool</a>
Spectral response acceleration 1 s period, $S_1$ =	0.064			
Importance Factor, $I$ =	1.0			(Table 2-3)
Acceleration-based site coefficient, $F_a$ =	1.6			(Table 2-12)
Velocity-based site coefficient, $F_v$ =	2.4			(Table 2-13)
Design spectral response acceleration short period, $S_{DS}$ =	0.191			(2.7.6)
Design spectral response acceleration 1 s period, $S_{D1}$ =	0.102			(2.7.6)
Seismic Design Category - Short Period Response =	B			ASCE 7-05 Table 11.6-1
Seismic Design Category - 1s Period Response =	B			ASCE 7-05 Table 11.6-2
Worst Case Seismic Design Category =	B			ASCE 7-05 Tables 11.6-1 and 6-2



# Radio Frequency Emissions Analysis Report

AT&T Existing Facility

Site ID: CT5137

FA#: 10071337

Windsor South  
1170 Matianuck Avenue  
Windsor, CT 06095

**February 28, 2018**

**Centerline Communications Project Number: 950012-035**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general population allowable limit:	<b>6.06 %</b>





February 28, 2018

AT&T Mobility – New England  
Attn: John Benedetto, RF Manager  
550 Cochituate Road  
Suite 550 – 13&14  
Framingham, MA 06040

### Emissions Analysis for Site: **CT5137 – Windsor South**

Centerline Communications, LLC (“Centerline”) was directed to analyze the proposed AT&T facility located at **1170 Matianuck Avenue, Windsor, CT**, for the purpose of determining whether the emissions from the Proposed AT&T Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limits for the 700 and 850 MHz Bands are approximately  $467 \mu\text{W}/\text{cm}^2$  and  $567 \mu\text{W}/\text{cm}^2$  respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 2300 MHz (WCS) bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.



## CALCULATIONS

Calculations were performed for the proposed AT&T Wireless antenna facility located at **1170 Matianuck Avenue, Windsor, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since AT&T is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. All power values expressed and analyzed are maximum power levels expected to be used on all radios.

All emissions values for additional carriers were taken from the Connecticut Siting Council (CSC) active MPE database. Values in this database are provided by the individual carriers themselves

For each sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
UMTS	850 MHz	1	30
UMTS	1900 MHz (PCS)	1	30
LTE	700 MHz	2	40
LTE	2300 MHz (WCS)	4	30
LTE	1900 MHz (PCS)	4	40

*Table 1: Channel Data Table*



The following antennas listed in *Table 2* were used in the modeling for transmission in the 700 MHz, 850 MHz, 1900 MHz (PCS) and 2300 MHz (WCS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

Sector	Antenna Number	Antenna Make / Model	Antenna Centerline (ft)
A	1	Kathrein 800-10121	97
A	2	Powerwave P65-17-XLH-RR	97
A	3	CCI TPA-65R-LCUUUU-H8	97
B	1	Kathrein 800-10121	97
B	2	KMW AM-X-CD-16-65-00T-RET	97
B	3	Quintel QS66512-2	97
C	1	Kathrein 800-10121	97
C	2	KMW AM-X-CD-14-65-00T-RET	97
C	3	Quintel QS46512-2	97

*Table 2: Antenna Data*

All calculations were done with respect to uncontrolled / general population threshold limits.

## RESULTS

Per the calculations completed for the proposed AT&T configurations *Table 3* shows resulting emissions power levels and percentages of the FCC’s allowable general population limit.

Antenna ID	Antenna Make / Model	Frequency Bands	Antenna Gain (dBd)	Channel Count	Total TX Power (W)	ERP (W)	MPE %
Antenna A1	Kathrein 800-10121	850 MHz / 1900 MHz (PCS)	11.45 / 14.35	2	60	1,235.72	0.68
Antenna A2	Powerwave P65-17-XLH-RR	700 MHz	14.3	2	80	2,153.23	2.00
Antenna A3	CCI TPA-65R-LCUUUU-H8	2300 MHz (WCS) / 1900 MHz (PCS)	14.45 / 13.75	8	280	7,137.54	3.10
Sector A Composite MPE%							<b>5.78</b>
Antenna B1	Kathrein 800-10121	850 MHz / 1900 MHz (PCS)	11.45 / 14.35	2	60	1,235.72	0.68
Antenna B2	KMW AM-X-CD-16-65-00T-RET	700 MHz	13.35	2	80	1,730.17	1.61
Antenna B3	Quintel QS66512-2	2300 MHz (WCS) / 1900 MHz (PCS)	14.85 / 13.85	8	280	7,548.48	3.28
Sector B Composite MPE%							<b>5.56</b>
Antenna C1	Kathrein 800-10121	850 MHz / 1900 MHz (PCS)	11.45 / 14.35	2	60	1,235.72	0.68
Antenna C2	KMW AM-X-CD-14-65-00T-RET	700 MHz	11.85	2	80	1,224.87	1.14
Antenna C3	Quintel QS46512-2	2300 MHz (WCS) / 1900 MHz (PCS)	4.05 / 13.15	8	280	6,353.78	2.76
Sector C Composite MPE%							<b>4.57</b>

*Table 3: AT&T Emissions Levels*



The Following table (*table 4*) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum AT&T MPE contributions per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, the sector with the largest calculated MPE% is Sector A. *Table 5* below shows a summary for each AT&T Sector as well as the composite MPE value for the site.

<b>Site Composite MPE%</b>	
<b>Carrier</b>	<b>MPE%</b>
AT&T – Max Sector Value	<b>5.78 %</b>
Clearwire	0.28 %
<b>Site Total MPE %:</b>	<b>6.06 %</b>

*Table 4: All Carrier MPE Contributions*

AT&T Sector A Total:	5.78 %
AT&T Sector B Total:	5.56 %
AT&T Sector C Total:	4.57 %
<b>Site Total:</b>	<b>6.06 %</b>

*Table 5: Site MPE Summary*



FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated AT&T sector(s). For this site, the sector with the largest calculated MPE% is Sector A.

AT&T _ Frequency Band / Technology Max Power Values (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
AT&T 850 MHz UMTS	1	418.91	97	1.82	850 MHz	567	0.32%
AT&T 1900 MHz (PCS) UMTS	1	816.81	97	3.55	1900 MHz (PCS)	1000	0.35%
AT&T 700 MHz LTE	2	1,076.61	97	9.35	700 MHz	467	2.00%
AT&T 2300 MHz (WCS) LTE	4	835.84	97	14.51	2300 MHz (WCS)	1000	1.45%
AT&T 1900 MHz (PCS) LTE	4	948.55	97	16.47	1900 MHz (PCS)	1000	1.65%
						<b>Total:</b>	<b>5.78%</b>

*Table 6: AT&T Maximum Sector MPE Power Values*



## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the AT&T facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

AT&T Sector	Power Density Value (%)
Sector A:	5.78 %
Sector B:	5.56 %
Sector C:	4.57 %
AT&T Maximum Total (per sector):	5.78 %
Site Total:	6.06 %
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **6.06 %** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

A handwritten signature in black ink, appearing to read 'Scott Heffernan', is positioned above the printed name.

Scott Heffernan  
RF Engineering Director  
**Centerline Communications, LLC**  
95 Ryan Drive, Suite 1  
Raynham, MA 02767



# Property Cards

Address Search :   [Clear Search](#)

## 1170 Matianuck Ave

**Property Owner:**  
Genesis Health Ventures Of Bloomfield

**Property Co-Owner**  
C/O At&T Mobility

**Mailing Address:**  
575 Morosgo Dr Suite 13-F  
Atlanta, GA  
30324

**File Code**  
3188

**Map:**  
55

**Block:**  
479

**Lot:**  
70

**Census Tract:**  
4731.00

**Property Type:**  
Cell Tower

**Land Area (Acres):**  
0.05

**Zone:**  
AG



[Click to Enlarge](#)

## Construction Details

**Year Built:**

**Building Style:**

**Stories:**

**Living Area:**  
0 Sq/Ft

**Building ID**  
102174

**Grade**

**Exterior Wall**

**Total Rooms:**

**Bedrooms:**

**Bathrooms:**

**Half Baths:**

**Heating Type**

**Heating Fuel**

**AC Type**

## Valuation

**Assessed Land Value:**  
\$103,320

**Assessed Building Value:**  
\$94,150

**Total Assessed Value:**  
\$197,470

## Last Sale

**Last Sale Date:**  
Wednesday, July 3rd, 1985

**Last Sale Price:**  
\$0

**Qualified Sale:**  
U

**Book/Page:**

**Appraised Land Value:**  
\$147,600

**Appraised Building Value:**  
\$134,500

**Total Appraised Value:**  
\$282,100

517/ 199

**Prior Owners**

Sale Date	Owner Name	Sale Price	Book / Page

**Parcel Sketch**



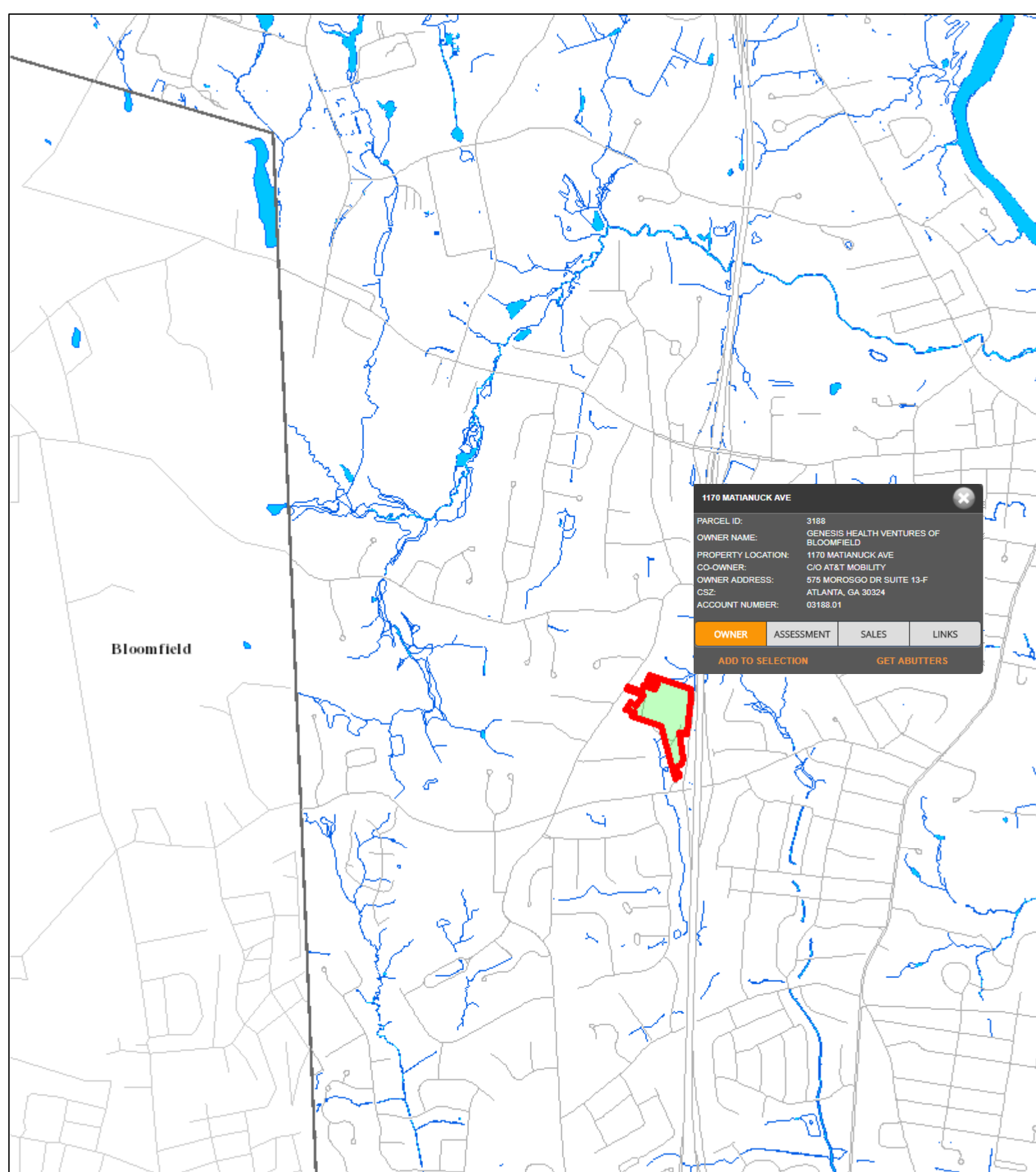
**Sub Area Detail**

Code	Gross Area (Sq Ft)	Living Area (Sq Ft)
------	--------------------	---------------------

**Outbuildings & Extra Features**

Code	Description	Appraised Value	Assessed Value
------	-------------	-----------------	----------------

- **AOF** Office Area
- **CAN** Canopy
- **EAF** Attic (Expan)(Finished)
- **FBM** Basement (Finished)
- **FDC** Carport (Det)(Framed)
- **FEP** Porch (Encl)(Finished)
- **FLL** Lower Level (Finished)
- **FST** Utility (Finished)
- **SDA** Store Display Area
- **TQS** Three-Qtr Story
- **UCB** Cabana (Encl)(Unfinished)
- **UEP** Porch (Encl)(Unfinished)
- **UOP** Porch (Open)(Unfinished)
- **UUS** Upper-Story (Unfinished)
- **APT** Apartment
- **CDN** Canopy (Det)
- **EAU** Attic (Expan)(Unfinished)
- **FCB** Cabana (Encl)(Finished)
- **FDS** Porch (Scrn)(Det)(Finished)
- **FGR** Garage (Framed)
- **FOP** Porch (Open)(Finished)
- **FUS** Upper-Story (Finished)
- **SFB** Base (Semi-Finished)
- **UAT** Attic (Unfinished)
- **UDS** Porch (Scrn)(Ded)(Unfinished)
- **UHS** Half-Story (Unfinished)
- **USP** Porch (Scrn)(Unfinished)
- **WDK** Wood Deck
- **BAS** First Floor
- **CLP** Loading Platform (Finished)
- **FAT** Attic (Finished)
- **FCP** Carport (Framed)
- **FDU** Utility (Det)(Finished)
- **FHS** Half-Story (Finished)
- **FSP** Porch (Screen)(Finished)
- **PTO** Patio
- **SPA** Service Prod Area
- **UBM** Basement (Unfinished)
- **UDU** Utility (Det)(Unfinished)
- **ULP** Loading Platform (Unfinished)
- **UST** Utility (Strg)(Unfinished)



**1170 MATIANUCK AVE**

PARCEL ID: 3188  
 OWNER NAME: GENESIS HEALTH VENTURES OF BLOOMFIELD  
 PROPERTY LOCATION: 1170 MATIANUCK AVE  
 CO-OWNER: C/O AT&T MOBILITY  
 OWNER ADDRESS: 575 MOROSGO DR SUITE 13-F  
 CSZ: ATLANTA, GA 30324  
 ACCOUNT NUMBER: 03188.01

**OWNER** ASSESSMENT SALES LINKS

[ADD TO SELECTION](#) [GET ABUTTERS](#)

Bloomfield

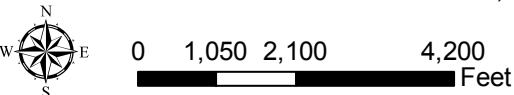
Hartford County, Connecticut

**1170 Matianuck Ave**

*Property Boundaries not legally binding for title or zoning purposes.*

Horizontal Datum is Connecticut State Plane Feet, NAD83

1 inch = 2,560 feet



*The Town of Windsor makes no warranty as to the accuracy, reliability, or completeness of the information and is not responsible for any error or omissions for results obtained from the use of the information.*

**PROJECT INFORMATION**

SCOPE OF WORK:

**ITEMS TO BE MOUNTED ON THE EXISTING TOWER:**

- NEW AT&T ANTENNAS: (TPA-65R-LCUUUU-H8) (ALPHA SECTOR, TOTAL OF 1).
- NEW AT&T ANTENNAS: (QS66512-2) (BETA SECTOR, TOTAL OF 1).
- NEW AT&T ANTENNAS: (QS46512-2) (GAMMA SECTOR, TOTAL OF 1).
- NEW AT&T RRUS: RRUS-32 (WCS) (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T RRUS: RRUS-32 B2 (PCS) (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T SURGE ARRESTOR: DC6-48-60-18-8F (TOTAL OF 1)
- COAX JUMPERS (4) PER SECTOR, FROM EACH RRU (TOTAL OF 12).
- FIBER JUMPERS (6) PER SECTOR, FROM THE SQUID TO EACH RRU (TOTAL OF 18).

**ITEMS TO BE MOUNTED AT EQUIPMENT LOCATION:**

- SWAP BB WITH 5216 & ADD XMU IN EXISTING PURCELL CABINET

**ITEMS TO REMAIN:**

- (6) ANTENNAS, (3) RRU'S, (6) 7/8" COAX CABLES, (2) DC POWER & (1) FIBER.

**SQUID ALARMING (NOT TO BE DAISY CHAINED).**

- THE 1ST SQUID INSTALLED WILL BE ALARMED TO THE LOWEST BAND (OR FIRST INSTALLED RRH/RRU ON THE ALPHA SECTOR, IN THE EVENT THE ALARM CABLE CANNOT BE CONNECTED TO ALPHA IT WILL BE ACCEPTABLE TO ALARM TO THE CLOSEST PHYSICAL SECTOR ON AN EXCEPTION BASIS.
- 2ND SQUID INSTALLED WILL BE ALARMED TO THE LOWEST BAND (OR FIRST INSTALLED) RRH/RRU ON THE BETA SECTOR.
- 3RD SQUID INSTALLED WILL BE ALARMED TO THE LOWEST BAND (OR FIRST INSTALLED) RRH/RRU ON THE GAMMA SECTOR.

FA: 10071337

SITE OWNER: CROWN CASTLE

SITE ADDRESS: 1170 MATIANUCK AVENUE  
WINDSOR, CT 06095

LATITUDE: 41.8404919 N, 41° 50' 25.77" N

LONGITUDE: 72.6664989 W, 72° 39' 59.40" W

TYPE OF SITE: MONOPINE/ OUTDOOR EQUIPMENT

STRUCTURE HEIGHT: 100'±

RAD CENTER: 97'±

CURRENT USE: TELECOMMUNICATIONS FACILITY

PROPOSED USE: TELECOMMUNICATIONS FACILITY



**SITE NUMBER: CT5137**

**SITE NAME: WINDSOR SOUTH**

**PROJECT: LTE 2C, 3C 2018 UPGRADE**

**VICINITY MAP**

**DIRECTIONS TO SITE:**  
GET ON I-91 N FROM ENTERPRISE DR. HEAD SOUTHEAST ON ENTERPRISE DR. TURN LEFT ONTO CAPITAL BLVD. USE THE LEFT LANE TO TURN LEFT ONTO STATE HWY 411. TURN LEFT TO MERGE ONTO I-91 N. FOLLOW I-91 N TO CT-178 W/PARK AVE IN WINDSOR. TAKE EXIT 36 FROM I-91 N MERGE ONTO I-91 N. TAKE EXIT 36 FOR CT-178/PARK AVE TOWARD BLOOMFIELD FOLLOW CT-178 W/PARK AVE TO EMERSON DR. TURN LEFT ONTO CT-178 W/PARK AVE TURN RIGHT ONTO EMERSON DR. TURN RIGHT TO STAY ON EMERSON DR. DESTINATION WILL BE ON THE LEFT. 1170 MATIANUCK AVE, WINDSOR, CT 06095



**GENERAL NOTES**

1. THIS DOCUMENT IS THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF AT&T. ANY DUPLICATION OR USE WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED. DUPLICATION AND USE BY GOVERNMENT AGENCIES FOR THE PURPOSES OF CONDUCTING THEIR LAWFULLY AUTHORIZED REGULATORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.
2. THE FACILITY IS AN UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ACCESSED BY TRAINED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES NOT REQUIRE ANY WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.
3. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE AT&T MOBILITY REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
4. CONSTRUCTION DRAWINGS ARE VALID FOR SIX MONTHS AFTER ENGINEER OF RECORD'S STAMPED AND SIGNED SUBMITTAL DATE LISTED HEREIN.

**DRAWING INDEX**

SHEET NO.	DESCRIPTION	REV.
T-1	TITLE SHEET	B
GN-1	GENERAL NOTES	B
A-1	COMPOUND & EQUIPMENT PLANS	B
A-2	ANTENNA LAYOUTS & ELEVATION	B
A-3	DETAILS	B
G-1	GROUNDING DETAILS	B
RF-1	RF PLUMBING DIAGRAM	B

**CCI SITE NAME: WINDSOR SOUTH**  
**CCI SITE #: 842878**

**72 HOURS**



**CALL BEFORE YOU DIG**  
CALL TOLL FREE 1-800-922-4455  
OR CALL 811

**UNDERGROUND SERVICE ALERT**

 45 BEECHWOOD DRIVE NORTH ANDOVER, MA 01845 TEL: (978) 557-5553 FAX: (978) 336-5586	 95 RYAN DRIVE RAYNHAM, MA 02767	SITE NUMBER: CT5137 SITE NAME: WINDSOR SOUTH CCI SITE #: 842878 1170 MATIANUCK AVENUE WINDSOR, CT 06095 HARTFORD COUNTY	 500 ENTERPRISE DRIVE, SUITE 3A ROCKY HILL, CT 06067	B 03/16/18 ISSUED FOR PERMITTING AM AT DJC		AT&T TITLE SHEET (LTE 2C-3C)
				A 02/08/18 ISSUED FOR REVIEW GA AT DJC		

**GROUNDING NOTES**

1. THE SUBCONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE-SPECIFIC (UL, LPI, OR NFPA) LIGHTING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE SUBCONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE CONTRACTOR FOR RESOLUTION.
2. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
3. THE SUBCONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR NEW GROUND ELECTRODE SYSTEMS. THE SUBCONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
4. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
5. EACH BTS CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, 6 AWG STRANDED COPPER OR LARGER FOR INDOOR BTS 2 AWG STRANDED COPPER FOR OUTDOOR BTS.
6. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
7. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
8. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO GROUND BAR.
9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
10. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
11. METAL CONDUIT SHALL BE MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH 6 AWS COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
12. ALL NEW STRUCTURES WITH A FOUNDATION AND/OR FOOTING HAVING 20 FT. OR MORE OF 1/2 IN. OR GREATER ELECTRICALLY CONDUCTIVE REINFORCING STEEL MUST HAVE IT BONDED TO THE GROUND RING USING AN EXOTHERMIC WELD CONNECTION USING #2 AWG SOLID BARE TINNED COPPER GROUND WIRE, PER NEC 250.50

**GENERAL NOTES**

1. FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:  
 CONTRACTOR – CENTERLINE COMMUNICATIONS  
 SUBCONTRACTOR – GENERAL CONTRACTOR (CONSTRUCTION)  
 OWNER – AT&T MOBILITY
2. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.
3. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
4. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.
5. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
6. "KITTING LIST" SUPPLIED WITH THE BID PACKAGE IDENTIFIES ITEMS THAT WILL BE SUPPLIED BY CONTRACTOR. ITEMS NOT INCLUDED IN THE BILL OF MATERIALS AND KITTING LIST SHALL BE SUPPLIED BY THE SUBCONTRACTOR.
7. THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
8. IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION SPACE FOR APPROVAL BY THE CONTRACTOR.
9. SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR.
10. THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
12. SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION.
13. ALL CONCRETE REPAIR WORK SHALL BE DONE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301.

14. ANY NEW CONCRETE NEEDED FOR THE CONSTRUCTION SHALL BE AIR-ENTRAINED AND SHALL HAVE 4000 PSI STRENGTH AT 28 DAYS. ALL CONCRETE WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS.
15. ALL STRUCTURAL STEEL WORK SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS. ALL STRUCTURAL STEEL SHALL BE ASTM A36 (Fy = 36 ksi) UNLESS OTHERWISE NOTED. PIPES SHALL BE ASTM A53 TYPE E (Fy = 36 ksi). ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED. TOUCHUP ALL SCRATCHES AND OTHER MARKS IN THE FIELD AFTER STEEL IS ERECTED USING A COMPATIBLE ZINC RICH PAINT.
16. CONSTRUCTION SHALL COMPLY WITH SPECIFICATIONS AND "GENERAL CONSTRUCTION SERVICES FOR CONSTRUCTION OF AT&T SITES."
17. SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.
18. THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.
19. SINCE THE CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE ADVISED TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.
20. APPLICABLE BUILDING CODES:  
 SUBCONTRACTOR'S WORK SHALL COMPLY WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AS ADOPTED BY THE LOCAL AUTHORITY HAVING JURISDICTION (AHJ) FOR THE LOCATION. THE EDITION OF THE AHJ ADOPTED CODES AND STANDARDS IN EFFECT ON THE DATE OF CONTRACT AWARD SHALL GOVERN THE DESIGN.  
 BUILDING CODE: IBC 2012 WITH 2016 CT STATE BUILDING CODE AMENDMENTS  
 ELECTRICAL CODE: REFER TO ELECTRICAL DRAWINGS  
 LIGHTENING CODE: REFER TO ELECTRICAL DRAWINGS  
  
 SUBCONTRACTOR'S WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS:  
  
 AMERICAN CONCRETE INSTITUTE (ACI) 318; BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE;  
  
 AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)  
 MANUAL OF STEEL CONSTRUCTION, ASD, FOURTEENTH EDITION;  
  
 TELECOMMUNICATIONS INDUSTRY ASSOCIATION (TIA) 222-G,  
 STRUCTURAL STANDARDS FOR STEEL  
  
 EQUIPMENT AND ANTENNA SUPPORTING STRUCTURES; REFER TO ELECTRICAL DRAWINGS FOR SPECIFIC ELECTRICAL STANDARDS.  
  
 FOR ANY CONFLICTS BETWEEN SECTIONS OF LISTED CODES AND STANDARDS REGARDING MATERIAL, METHODS OF CONSTRUCTION, OR OTHER REQUIREMENTS, THE MOST RESTRICTIVE REQUIREMENT SHALL GOVERN. WHERE THERE IS CONFLICT BETWEEN A GENERAL REQUIREMENT AND A SPECIFIC REQUIREMENT, THE SPECIFIC REQUIREMENT SHALL GOVERN.

ABBREVIATIONS					
AGL	ABOVE GRADE LEVEL	EQ	EQUAL	REQ	REQUIRED
AWG	AMERICAN WIRE GAUGE	GC	GENERAL CONTRACTOR	RF	RADIO FREQUENCY
BBU	BATTERY BACKUP UNIT	GRC	GALVANIZED RIGID CONDUIT	TBD	TO BE DETERMINED
BTCW	BARE TINNED SOLID COPPER WIRE	MGB	MASTER GROUND BAR	TBR	TO BE REMOVED
BGR	BURIED GROUND RING	MIN	MINIMUM	TBRR	TO BE REMOVED AND REPLACED
BTS	BASE TRANSCEIVER STATION	P	PROPOSED	TYP	TYPICAL
E	EXISTING	NTS	NOT TO SCALE	UG	UNDER GROUND
EGB	EQUIPMENT GROUND BAR	RAD	RADIATION CENTER LINE (ANTENNA)	VIF	VERIFY IN FIELD
EGR	EQUIPMENT GROUND RING	REF	REFERENCE		




45 BEECHWOOD DRIVE  
NORTH ANDOVER, MA 01845  
TEL: (978) 557-5553  
FAX: (978) 336-5586



95 RYAN DRIVE  
RAYNHAM, MA 02767

**SITE NUMBER: CT5137**  
**SITE NAME: WINDSOR SOUTH**  
**CCI SITE #: 842878**  
  
 1170 MATIANUCK AVENUE  
 WINDSOR, CT 06095  
 HARTFORD COUNTY



500 ENTERPRISE DRIVE, SUITE 3A  
ROCKY HILL, CT 06067

NO.	DATE	REVISIONS	BY	CHK	APP'D
B	03/16/18	ISSUED FOR PERMITTING	AM	AT	DJC
A	02/08/18	ISSUED FOR REVIEW	GA	AT	DJC

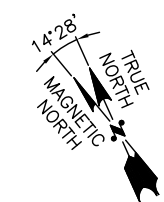
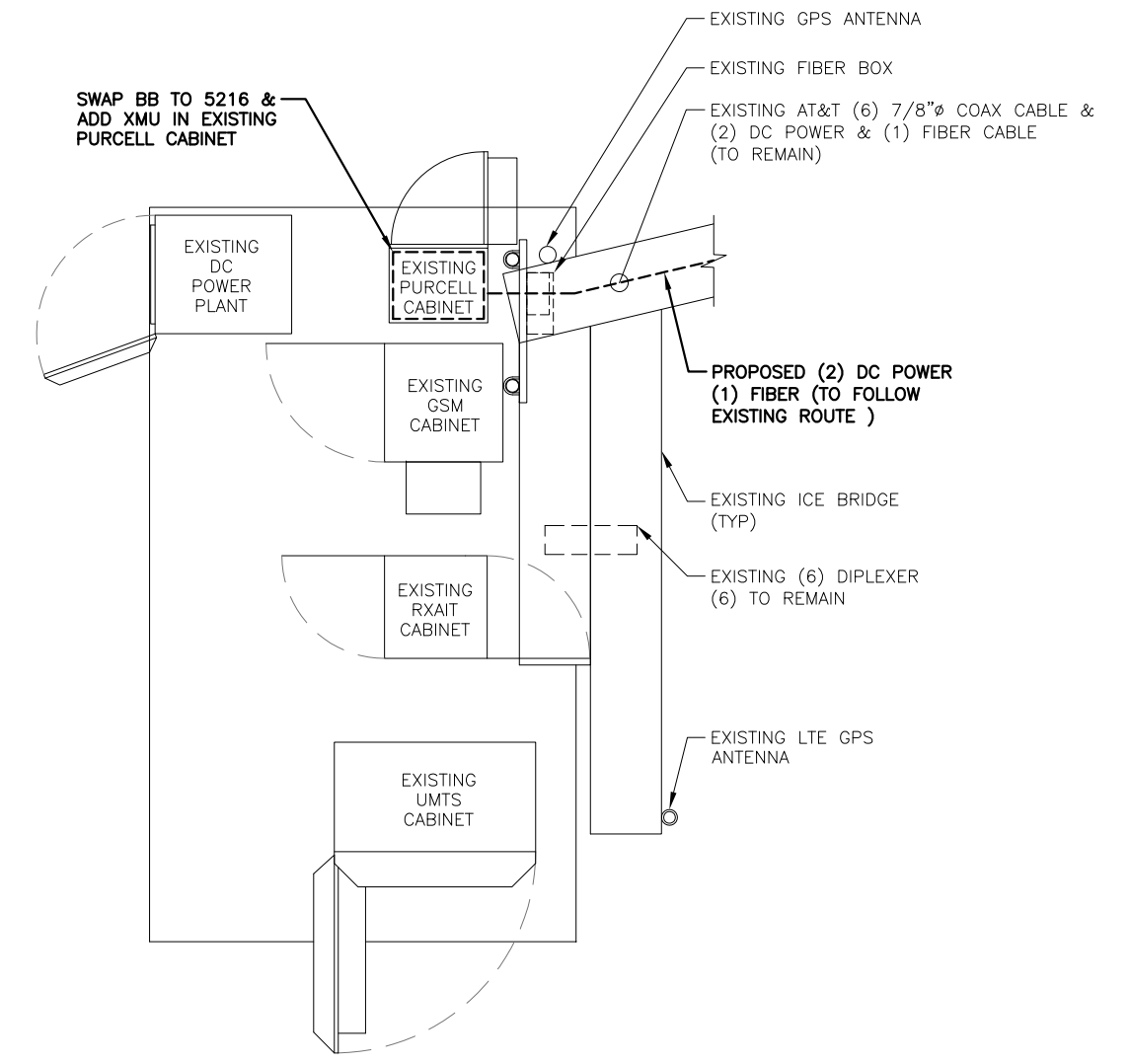
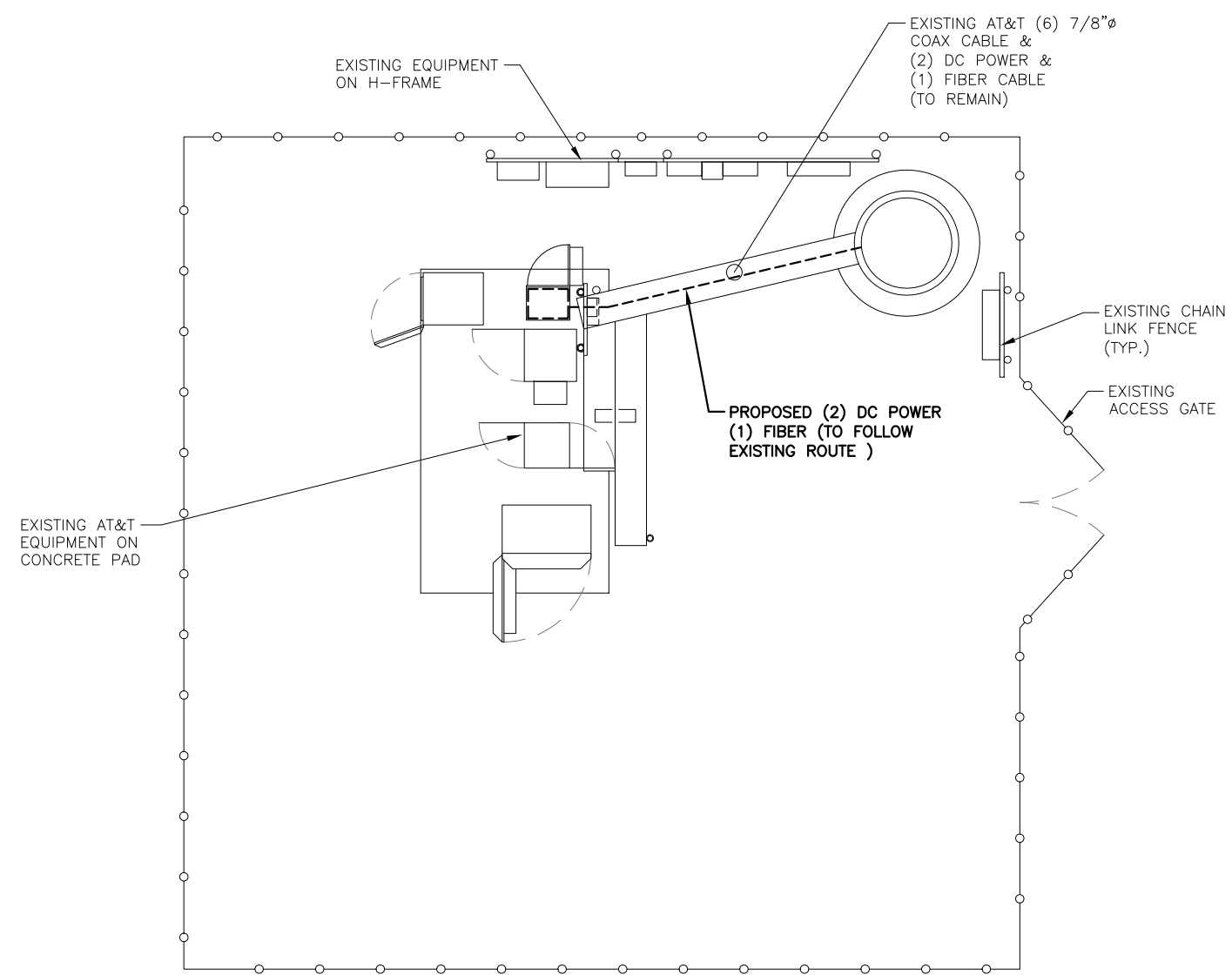
SCALE: AS SHOWN    DESIGNED BY: AT    DRAWN BY: GA

AT&T  
 GENERAL NOTES  
 (LTE 2C-3C)

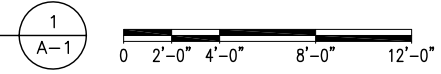
SITE NUMBER	DRAWING NUMBER	REV
CT5137	GN-1	B

NOTE:  
REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

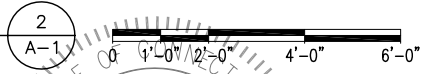
NOTE:  
ALL ANTENNAS AND RRHS TO BE INSTALLED IN ACCORDANCE WITH STRUCTURAL ANALYSIS PROVIDED BY CROWN CASTLE AND FINAL RF DATA SHEET



**COMPOUND PLAN**  
22x34 SCALE: 1/4"=1'-0"  
11x17 SCALE: 1/8"=1'-0"



**EQUIPMENT PLAN**  
22x34 SCALE: 1/2"=1'-0"  
11x17 SCALE: 1/4"=1'-0"



**HGD HUDSON Design Group LLC**  
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NORTH ANDOVER, MA 01845  
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FAX: (978) 336-5586

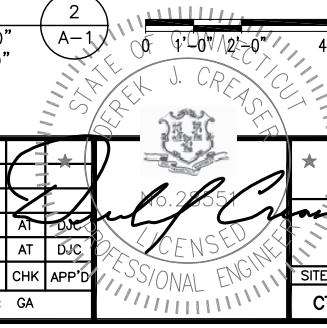
**CENTERLINE COMMUNICATIONS**  
95 RYAN DRIVE  
RAYNHAM, MA 02767

**SITE NUMBER: CT5137**  
**SITE NAME: WINDSOR SOUTH**  
**CCI SITE #: 842878**  
1170 MATIANUCK AVENUE  
WINDSOR, CT 06095  
HARTFORD COUNTY

**at&t**  
500 ENTERPRISE DRIVE, SUITE 3A  
ROCKY HILL, CT 06067

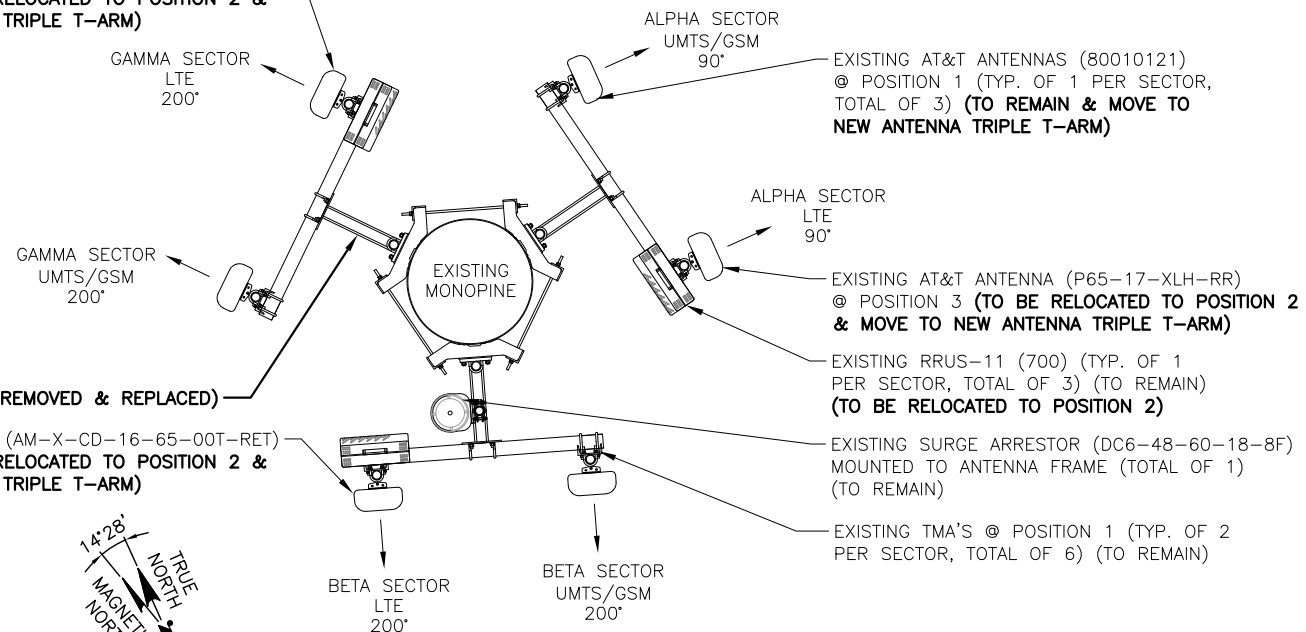
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**AT&T**  
**COMPOUND & EQUIPMENT PLAN**  
**(LTE 2C-3C)**  
SITE NUMBER: **CT5137**    DRAWING NUMBER: **A-1**    REV: **B**

EXISTING AT&T ANTENNA (AM-X-CD-14-65-00T-RET)  
 @ POSITION 3 (TO BE RELOCATED TO POSITION 2 &  
 MOVE TO NEW ANTENNA TRIPLE T-ARM)

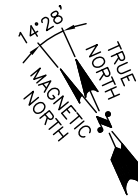


**EXISTING ANTENNA LAYOUT**

SCALE: N.T.S

1  
A-2

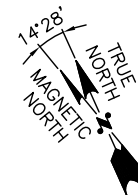
EXISTING AT&T ANTENNA (AM-X-CD-16-65-00T-RET)  
 @ POSITION 3 (TO BE RELOCATED TO POSITION 2 &  
 MOVE TO NEW ANTENNA TRIPLE T-ARM)



PROPOSED LTE AT&T ANTENNA  
 (QS46512-2) @ POSITION 4

NEW LOCATION OF EXISTING AT&T  
 ANTENNA  
 (AM-X-CD-14-65-00T-RET)  
 @ POSITION 2 (TOTAL OF 1)

PROPOSED LTE AT&T ANTENNA  
 (QS66512-2) @ POSITION 4



**PROPOSED ANTENNA LAYOUT**

SCALE: N.T.S

2  
A-2

EXISTING AT&T ANTENNAS (80010121)  
 @ POSITION 1 (TYP. OF 1 PER SECTOR,  
 TOTAL OF 3) (TO REMAIN & MOVE TO  
 NEW ANTENNA TRIPLE T-ARM)

EXISTING AT&T ANTENNA (P65-17-XLH-RR)  
 @ POSITION 3 (TO BE RELOCATED TO POSITION 2  
 & MOVE TO NEW ANTENNA TRIPLE T-ARM)

EXISTING RRUS-11 (700) (TYP. OF 1  
 PER SECTOR, TOTAL OF 3) (TO REMAIN)  
 (TO BE RELOCATED TO POSITION 2)

EXISTING SURGE ARRESTOR (DC6-48-60-18-8F)  
 MOUNTED TO ANTENNA FRAME (TOTAL OF 1)  
 (TO REMAIN)

EXISTING TMA'S @ POSITION 1 (TYP. OF 2  
 PER SECTOR, TOTAL OF 6) (TO REMAIN)

NEW LOCATION OF EXISTING RRUS-11 (700)  
 (TYP. OF 1 PER SECTOR, TOTAL OF 3)

NEW LOCATION OF EXISTING AT&T ANTENNA  
 (P65-17-XLH-RR) @ POSITION 2 (TOTAL OF 1)

PROPOSED AT&T TRIPLE T-ARM  
 (MC-K10S-12-96) (TYP OF 2)

PROPOSED AT&T SURGE ARRESTOR  
 (DC6-48-60-18-8F) (TOTAL OF 1)

PROPOSED AT&T RRUS-32 B2 (PCS)  
 (TYP. OF 1 PER SECTOR, TOTAL OF 3)

ALPHA SECTOR  
 LTE WCS/PCS  
 90°

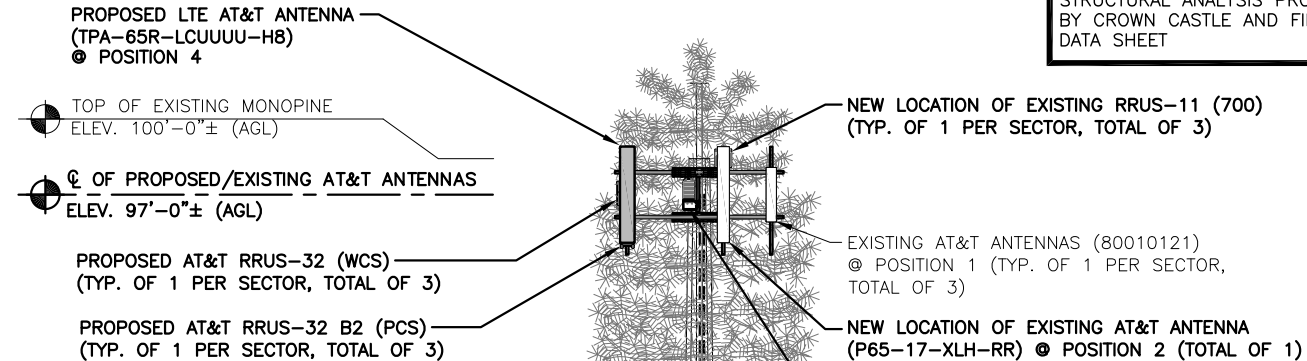
PROPOSED LTE AT&T ANTENNA  
 (TPA-65R-LCUUUU-H8)  
 @ POSITION 4

PROPOSED AT&T RRUS-32 (WCS)  
 (TYP. OF 1 PER SECTOR, TOTAL OF 3)

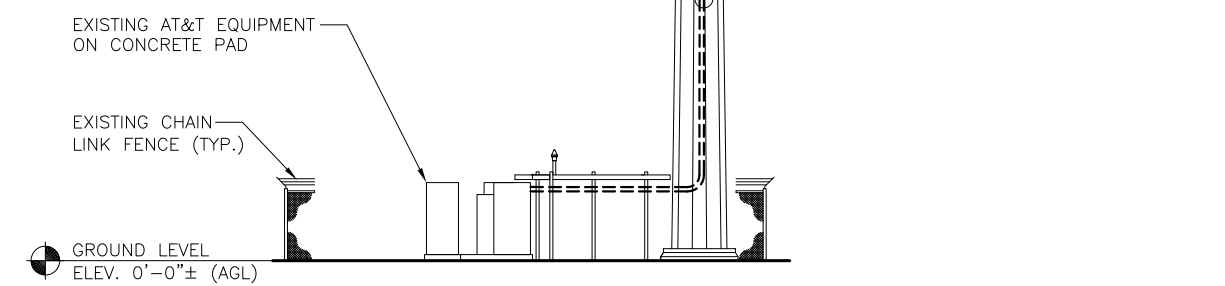
NEW LOCATION OF EXISTING AT&T ANTENNA  
 (AM-X-CD-16-65-00T-RET)  
 @ POSITION 2 (TOTAL OF 1)

NOTE:  
 REFER TO THE FINAL RF DATA SHEET  
 FOR FINAL ANTENNA SETTINGS.

NOTE:  
 ALL ANTENNAS AND RRHS TO BE  
 INSTALLED IN ACCORDANCE WITH  
 STRUCTURAL ANALYSIS PROVIDED  
 BY CROWN CASTLE AND FINAL RF  
 DATA SHEET



NOTE:  
 REPLACE EXISTING FRP BRANCHES  
 AS REPLIED TO COVERED  
 PROPOSED MOUNT



**ELEVATION**

22x34 SCALE: 1/8"=1'-0"  
 11x17 SCALE: 1/16"=1'-0"

3  
A-2

**HUDSON**  
 Design Group LLC  
 45 BEECHWOOD DRIVE  
 NORTH ANDOVER, MA 01845  
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 COMMUNICATIONS  
 95 RYAN DRIVE  
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SITE NUMBER: CT5137  
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 CCI SITE #: 842878  
 1170 MATIANUCK AVENUE  
 WINDSOR, CT 06095  
 HARTFORD COUNTY

**at&t**  
 500 ENTERPRISE DRIVE, SUITE 3A  
 ROCKY HILL, CT 06067

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**AT&T**  
 ANTENNA LAYOUT & ELEVATION  
 (LTE 2C-3C)  
 SITE NUMBER: CT5137  
 DRAWING NUMBER: A-2  
 REV: B

NOTE:  
REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

NOTE:  
ALL ANTENNAS AND RRHS TO BE INSTALLED IN ACCORDANCE WITH STRUCTURAL ANALYSIS PROVIDED BY CROWN CASTLE AND FINAL RF DATA SHEET

**FINAL ANTENNA SCHEDULE**

SECTOR	BAND	EXISTING	ANTENNA	SIZE (INCHES) (L X W X D)	RAD CENTER	AZIMUTH	TMA'S	DIPLEXER	RRU'S	SIZE (INCHES) (L X W X D)	COAX JUMPERS	FIBER JUMPERS	COAX		
ALPHA	UMTS 850/1900	EXISTING	80010121	54.5X10.3X6.6	97'-0"±	90°	EXISTING (2)LGP21401	EXISTING (2)LGP21901	-	-	-	-	(2) 7/8		
	LTE 700 BC	EXISTING	P65-17-XLH-RR	96X12X6	97'-0"±	90°	-	-	EXISTING	RRUS-11 (700)	-	-	-		
	-	-	-	-	-	-	-	-	-	-	-	-	-		
BETA	UMTS 850/1900	EXISTING	80010121	54.5X10.3X6.6	97'-0"±	200°	EXISTING (2)LGP21401	EXISTING (2)LGP21901	-	-	-	-	(2) 7/8		
	LTE 700 BC	EXISTING	AM-X-CD-16-65-00T-RET	72X11.8X5.9	97'-0"±	200°	-	-	EXISTING	RRUS-11 (700)	-	-	-		
	-	-	-	-	-	-	-	-	-	-	-	-	-		
GAMMA	UMTS 850/1900	EXISTING	80010121	54.5X10.3X6.6	97'-0"±	320°	EXISTING (2)LGP21401	EXISTING (2)LGP21901	-	-	-	-	(2) 7/8		
	LTE 700 BC	EXISTING	AM-X-CD-14-65-00T-RET	48X11.8X5.9	97'-0"±	320°	-	-	EXISTING	RRUS-11 (700)	-	-	-		
	-	-	-	-	-	-	-	-	-	-	-	-	-		
ALPHA	LTE WCS/PCS	PROPOSED	TPA-65R-LCUUUU-H8	96X14.4X8.6	97'-0"±	90°	-	-	PROPOSED	RRUS-32 B2 (PCS) RRUS-32 (WCS)	27.2X12.1X7.0 27.2X12.1X7.0	*1 *1	**2 **1	-	
	BETA	LTE WCS/PCS	PROPOSED	QS66512-2	72X12X9.6	97'-0"±	200°	-	-	PROPOSED	RRUS-32 B2 (PCS) RRUS-32 (WCS)	27.2X12.1X7.0 27.2X12.1X7.0	*1 *1	**2 **1	-
		GAMMA	LTE WCS/PCS	PROPOSED	QS46512-2	52X12X10.8	97'-0"±	320°	-	-	PROPOSED	RRUS-32 B2 (PCS) RRUS-32 (WCS)	27.2X12.1X7.0 27.2X12.1X7.0	*1 *1	**2 **1

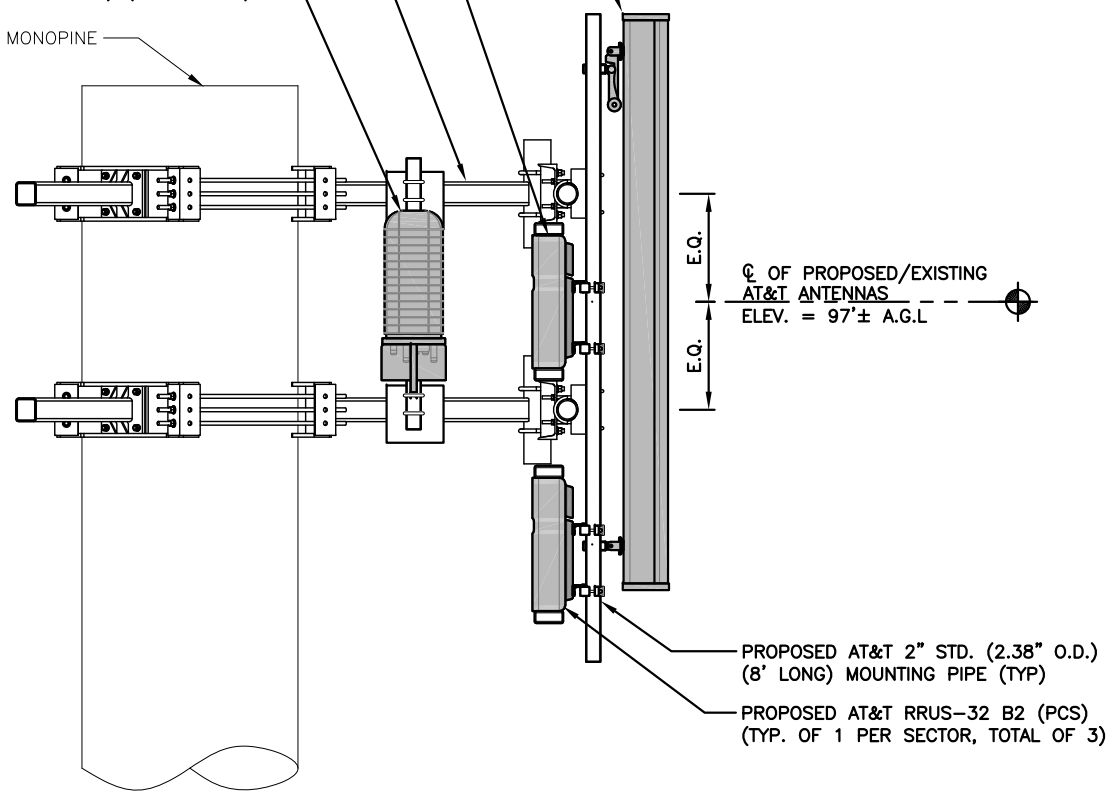
PROPOSED LTE AT&T ANTENNA (TYP OF 1 PER SECTOR, TOTAL OF 3)

PROPOSED AT&T RRUS-32 (WCS) (TYP. OF 1 PER SECTOR, TOTAL OF 3)

PROPOSED AT&T TRIPLE T-ARM (MC-K10S-12-96) (TYP OF 2)

PROPOSED AT&T SURGE ARRESTOR (DC6-48-60-18-8F) (TOTAL OF 1)

EXISTING MONOPINE



**PROPOSED LTE ANTENNA & RRH MOUNTING DETAIL**

22x34 SCALE: 3/4"=1'-0"  
11x17 SCALE: 3/8"=1'-0"

**FINAL ANTENNA CONFIGURATION TABLE**

3  
A-3

PROPOSED SURGE ARRESTOR (TOTAL OF 1)

PROPOSED PIPE MAST

**\*COAX JUMPER NOTE:**  
COAX JUMPERS (2) PER SECTOR, FROM EACH RRU (TOTAL OF 6).

**\*\*FIBER JUMPER NOTE:**  
FIBER JUMPERS (3) PER SECTOR, FROM THE SQUID TO EACH RRU (TOTAL OF 9).

**RRU CHART**

QUANTITY	MODEL	L	W	D
3(E)	RRUS-11	19.7"	17.0"	7.2"
6(P)	RRUS-32	27.2"	12.1"	7.0"

NOTE:  
MOUNT PER MANUFACTURER'S SPECIFICATIONS

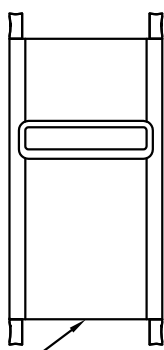
NOTE:  
SEE RFDS FOR RRH FREQUENCY AND MODEL NUMBER

PROPOSED RRU REFER TO THE FINAL RFDS AND CHART FOR QUANTITY, MODEL AND DIMENSIONS

NOTE:  
MOUNT PER MANUFACTURER'S SPECIFICATIONS.

**PROPOSED RRUS DETAIL**

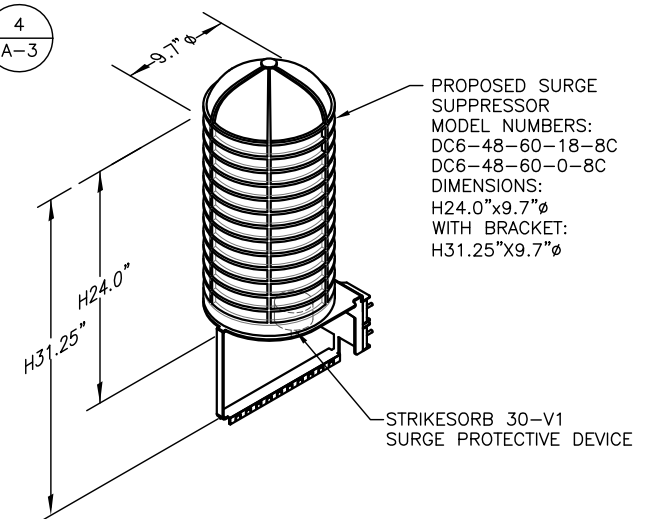
SCALE: N.T.S



**PROPOSED SURGE ARRESTOR MOUNTING DETAIL**

SCALE: N.T.S

4  
A-3



NOTE:  
MOUNT PER MANUFACTURER'S SPECIFICATIONS.

**DC SURGE SUPPRESSOR DETAIL**

SCALE: N.T.S

5  
A-3

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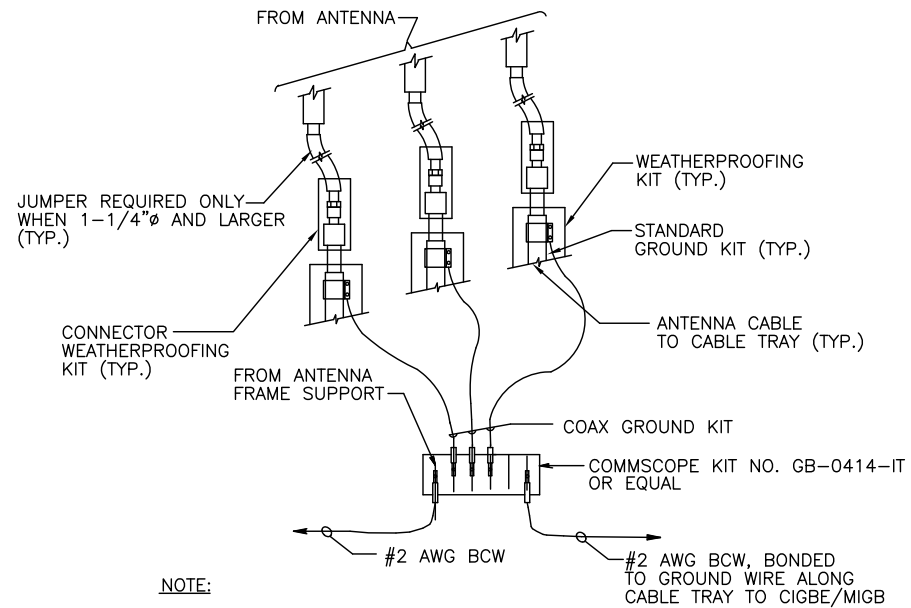
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SCALE: AS SHOWN    DESIGNED BY: AT    DRAWN BY: GA

STATE OF CONNECTICUT  
Derek J. Cramer  
REGISTERED PROFESSIONAL ENGINEER  
No. 29557

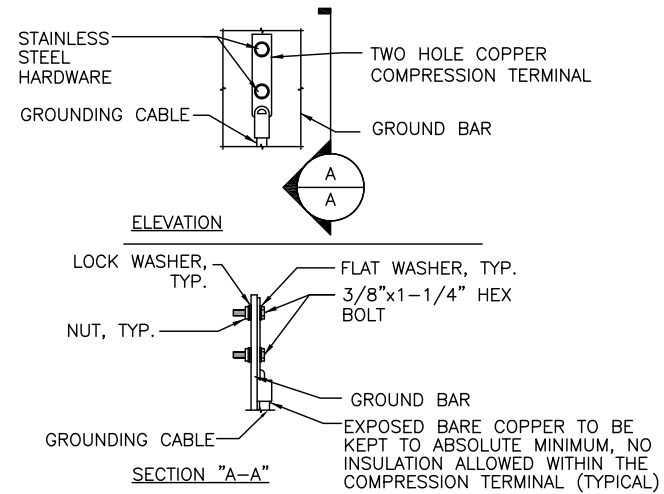
**AT&T**  
DETAILS  
(LTE 2C-3C)  
SITE NUMBER: CT5137  
DRAWING NUMBER: A-3  
REV: B





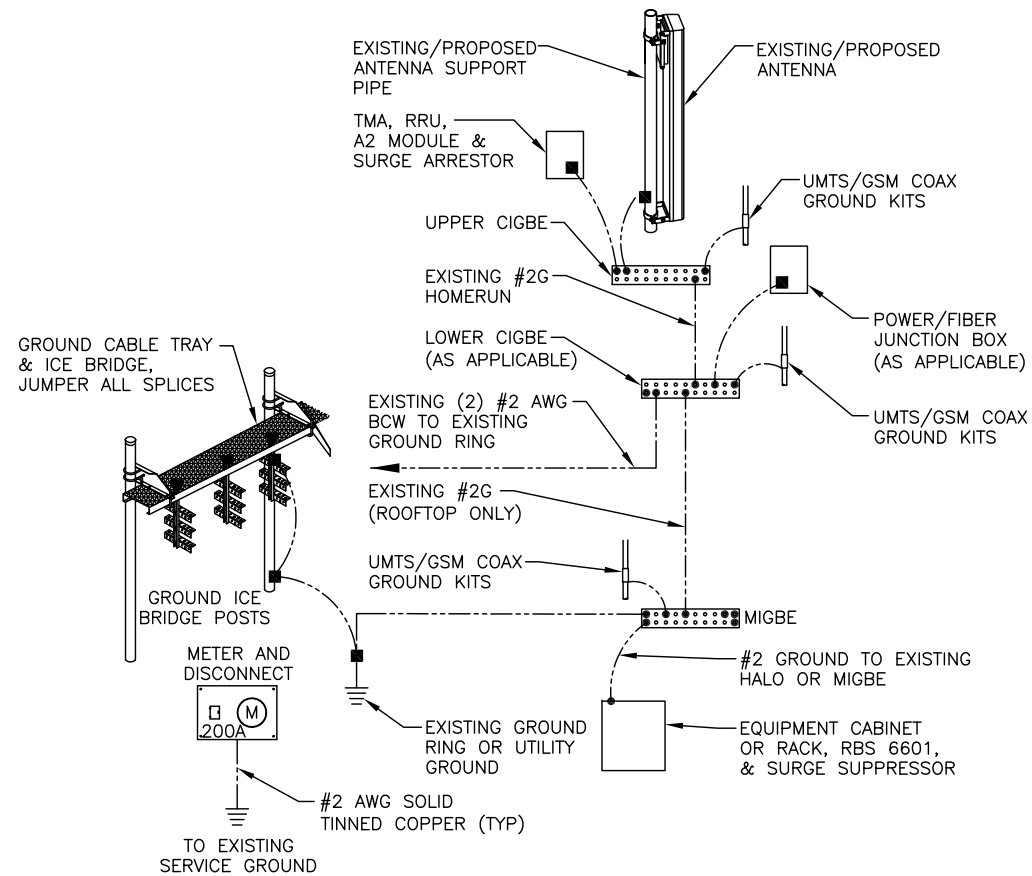
NOTE:  
 1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO CIGBE.

**GROUND WIRE TO GROUND BAR CONNECTION DETAIL** (1)  
 SCALE: N.T.S. G-1



NOTE:  
 1. "DOUBLING UP" OR "STACKING" OF CONNECTION IS NOT PERMITTED.  
 2. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATION.  
 3. CADWELD DOWNLEADS FROM UPPER EGB, LOWER EGB, AND MGB

**TYPICAL GROUND BAR CONNECTION DETAIL** (3)  
 SCALE: N.T.S. G-1



**GROUNDING RISER DIAGRAM** (2)  
 SCALE: N.T.S. G-1

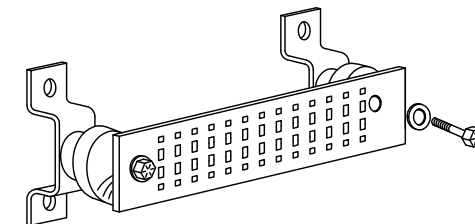
EACH GROUND CONDUCTOR TERMINATING ON ANY GROUND BAR SHALL HAVE AN IDENTIFICATION TAG ATTACHED AT EACH END THAT WILL IDENTIFY ITS ORIGIN AND DESTINATION.

**SECTION "P" - SURGE PRODUCERS**

- CABLE ENTRY PORTS (HATCH PLATES) (#2)
- GENERATOR FRAMEWORK (IF AVAILABLE) (#2)
- TELCO GROUND BAR
- COMMERCIAL POWER COMMON NEUTRAL/GROUND BOND (#2)
- +24V POWER SUPPLY RETURN BAR (#2)
- 48V POWER SUPPLY RETURN BAR (#2)
- RECTIFIER FRAMES.

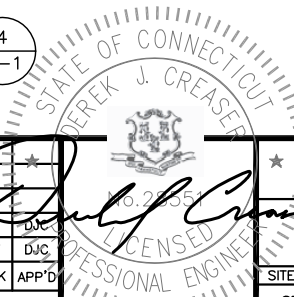
**SECTION "A" - SURGE ABSORBERS**

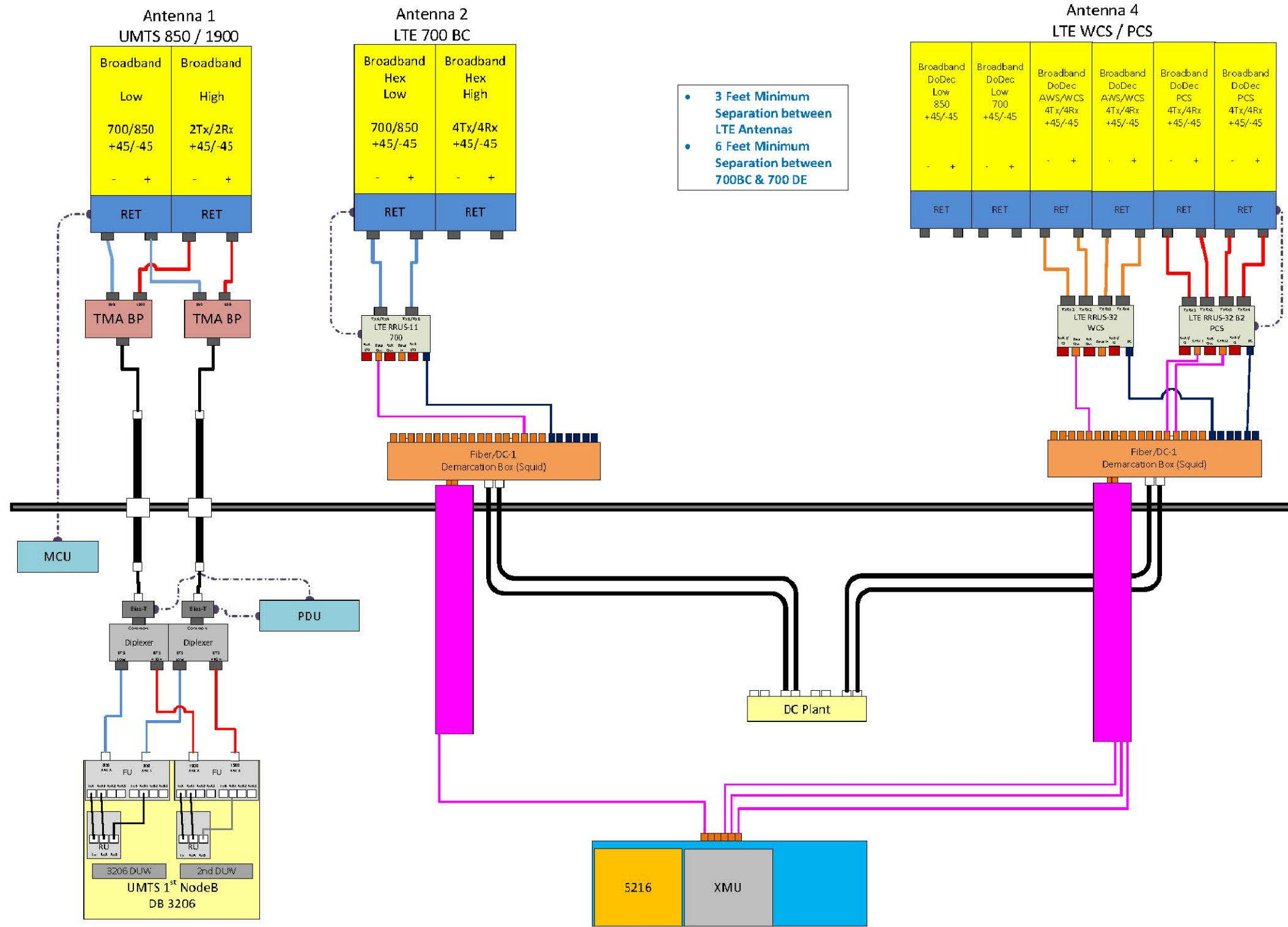
- INTERIOR GROUND RING (#2)
- EXTERNAL EARTH GROUND FIELD (BURIED GROUND RING) (#2)
- METALLIC COLD WATER PIPE (IF AVAILABLE) (#2)
- BUILDING STEEL (IF AVAILABLE) (#2)



**GROUND BAR - DETAIL** (4)  
 SCALE: N.T.S. G-1

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SCALE: AS SHOWN DESIGNED BY: AT DRAWN BY: GA					



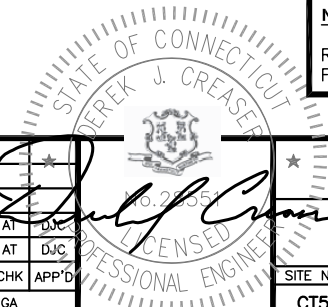


- 3 Feet Minimum Separation between LTE Antennas
- 6 Feet Minimum Separation between 700BC & 700 DE

**NOTE:**  
 1. CONTRACTOR TO CONFIRM ALL PARTS.  
 2. INSTALL ALL EQUIPMENT TO MANUFACTURER'S RECOMMENDATIONS

**NOTE:**  
 REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

**RF PLUMBING DIAGRAM** 1  
 SCALE: N.T.S. RF-1



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A	02/08/18	ISSUED FOR REVIEW	GA	AT	DJC
SCALE: AS SHOWN		DESIGNED BY: AT	DRAWN BY: GA		

<b>AT&amp;T</b>		
RF PLUMBING DIAGRAM (LTE 2C-3C)		
SITE NUMBER	DRAWING NUMBER	REV
CT5137	RF-1	B