

Creve Coeur, MO 63141

Phone: (314) 513-0147

www.crowncastle.com

November 23, 2021

Melanie A. Bachman **Executive Director** Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: **Notice of Exempt Modification for Sprint**

Crown Site ID#842875; T-Mobile Site ID#CTHA267A

99 Day Hill Road, WINDSOR, CT 06095

Latitude: 41° 52′ 16.10″/ Longitude: -72° 40′ 16.00″

Dear Ms. Bachman:

Sprint currently maintains (6) antennas at the 131-foot mounts on the existing 168-foot Monopole Tower located at 99 Day Hill Road, WINDSOR. The property is owned by The Town of Windsor and the Tower by Crown Castle. Sprint now intends to replace six (6) antennas. This modification/proposal includes hardware that is both 4G(LTE) and 5G capable through remote software configuration and either or both services may be turned on or off at various times.

Planned Modifications:

Tower:

REMOVE AND REPLACE

- (3) RFS APXV9ERR18-C-A20 Antennas (**REMOVE**), (3) RFS –
- APXVAALL24 43 U NA20 Antennas (**REPLACE**)
- (3) RFS- APXVTM14-C-120 Antennas (REMOVE), (3) Ericsson AIR6449 B41 Antennas (REPLACE)
- (3) Lucent TD-RRH8X20-25 Remote Radio Heads (REMOVE), (3) Ericsson Radio 4460 B25 + B66 Remote Radio Heads (REPLACE)
- (3) Lucent 1900MHZ Remote Radio Heads (**REMOVE**), (3) Ericsson Radio 4480 B71+B85 Remote Radio heads (REPLACE)
- (3) Lucent 800MHZ W/ Filter Remote Radio heads (**REMOVE**)
- (3) Hybrid Cable (**REMOVE**), (4) Hybrid Cables (1-5/8") (**REPLACE**)
- (3) Ret Cable (**REMOVE**)
- (1) Antenna Platform (Modify)

Ground:

REMOVE:

(2) Legacy Sprint Equipment Cabinets

INSTALL:

- (1) 6160 battery cabinet
- (2) B160 battery cabinet

The Foundation for a Wireless World. CrownCastle.com

1 Cityplace Dr, Suite 490

Creve Coeur, MO 63141

Phone: (314) 513-0147

www.crowncastle.com

- (1) PSU4813 Booster in (P) cabinet
- (3) BB6648 in (P) cabinets
- (1) IXRE Router in (P) cabinet
- (1) 2" LFTC BET. 6160 & B160
- (1) 2" Underground PVC BET. 6160 & PPC
- (1) 1" Underground PVC BET. 6160 & AAV

The facility was approved by the Town of Windsor Planning and Zoning Commission by way of Special Use Application #292A on October 10, 2000.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. §16-50j-72(b)(2). In accordance with R.C.S.A. §16-50j-73, a copy of this letter is being sent to Donald Trinks, Mayor for the Town of Windsor, as well as Eric Barz, Town Planner, and Crown Castle who is the tower owner.

- 1. The proposed modifications will not result in an increase in the height of the existing tower.
- 2. The proposed modifications will not require the extension of the site boundary.
- 3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Sprint respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. §16-50j-72(b)(2).

Sincerely,

Ersilia Davis
NETWORK BUILDING + CONSULTING
Project Manager
1777 Sentry Parkway W | VEVA 17, Suite 400
Blue Bell, PA 19422
edavis@nbcllc.com
(551)804-0667

The Foundation for a Wireless World.

CrownCastle.com



1 Cityplace Dr, Suite 490

Creve Coeur, MO 63141

Phone: (314) 513-0147

www.crowncastle.com

cc:

Donald Trinks, Mayor Town Hall Council Chambers 275 Broad Street Windsor, CT 06095 860- 285-1800 (Via Fedex)

Eric Barz, Town Planner Planning Department 275 Broad Street Windsor, CT 06095 (860) 285-1981 (Via Fedex)

Crown Castle, Tower Owner



11/24/21, 10:50 AM

TRACK ANOTHER SHIPMENT

775292595247





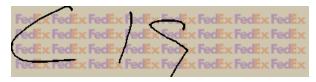


Delivered Wednesday, 11/24/2021 at 10:19 am



DELIVERED

Signed for by: A.POSNIAK



GET STATUS UPDATES

OBTAIN PROOF OF DELIVERY

FROM

Ersilia Davis

1777 Sentry Parkway VEVA 17, Suite 210 Blue Bell, PA US 19422 551-804-0667 то

Donald Tinks, Mayor Town of Windsor

275 Broad Street WINDSOR, CT US 06095 860-285-1800

MANAGE DELIVERY

Travel History

TIME ZONE

Local Scan Time

Wednesday, November 24, 2021

10:19 AM WINDSOR, CT Delivered
 8:47 AM WINDSOR LOCKS, CT At local FedEx facility

8:47 AM WINDSOR LOCKS, CT On FedEx vehicle for delivery

7:50 AM WINDSOR LOCKS, CT At local FedEx facility

5:58 AM EAST GRANBY, CT At destination sort facility



TRACK ANOTHER SHIPMENT

775292638699





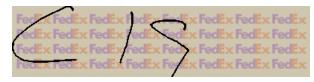


Delivered Wednesday, 11/24/2021 at 10:19 am



DELIVERED

Signed for by: A.POSNIAK



GET STATUS UPDATES OBTAIN PROOF OF DELIVERY

FROM

Ersilia Davis

1777 Sentry Parkway VEVA 17, Suite 210 Blue Bell, PA US 19422 551-804-0667 то

Eric Barz, /Town Manager Town of Windsor

> 275 Broad Street WINDSOR, CT US 06095 860-285-1981

MANAGE DELIVERY ✓

Travel History

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10:19 AM WINDSOR, CT Delivered
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 8:47 AM WINDSOR LOCKS, CT On FedEx vehicle for delivery

7:50 AM WINDSOR LOCKS, CT At local FedEx facility

5:58 AM EAST GRANBY, CT At destination sort facility

11/24/21, 10:53 AM Detailed Tracking

5:09 AM NEWARK, NJ Departed FedEx hub

Tuesday, November 23,

2021

11:35 PM NEWARK, NJ Arrived at FedEx hub

10:13 PM NEWBURGH, NY Left FedEx origin facility

6:24 PM NEWBURGH, NY Picked up

11:32 AM Shipment information sent to FedEx

Expand History 🗸

Shipment Facts

TRACKING NUMBERSERVICE
WEIGHT
775292638699
FedEx Priority Overnight
1 lbs / 0.45 kgs

DELIVERY ATTEMPTS DELIVERED TO TOTAL PIECES

1 Receptionist/Front Desk 1

TOTAL SHIPMENT WEIGHTTERMSSHIPPER REFERENCE1 lbs / 0.45 kgsShipper100788/NBC Windsor

PACKAGINGSPECIAL HANDLING SECTIONSHIP DATEFedEx EnvelopeDeliver Weekday11/23/21 ③

STANDARD TRANSIT

ACTUAL DELIVERY

11/24/21 before 11:30 am ②

11/24/21 at 10:19 am

Exhibit A

Original Facility Approval

VOL 1 249 PG 156

I, Anita M. Mips, Chairperson of the Windsor Town Planning and Zoning Commission, hereby certify that on October 10, 2000 the Planning and Zoning Commission of the Town of Windsor granted approval of Special Use Application #292A for a Wireless Telecommunications Tower with a monopole height of 170 feet plus 20-foot Town public service whip antennas for a total height of 190 feet, under Zoning Regulations Sections 12.2 & 2.2.19E(1).

This approval also includes the following waivers in accordance with Zoning Regulations Section 12.1:

- 1) a waiver of the height requirement for 15 feet; and
- 2) a waiver of the fall zone distance requirement for 151 feet in relation to the distance of the tower from Day Hill Road, 380 feet being required, 229 feet being proposed.

proposed.	,	
Said Special Use was granted for the prope	rty located at:	99 Day Hill Road
The owner of record of said parcel is:		Town of Windsor
Dated at Windsor, Connecticut, this	day of Noven	nber, 2000
Quelinip	_ Chairperson	
Public Act #75-317		
Received for Record thisday of		, 2000
	Attest: Town Clerk	
	RECEIVED FOR WINDSOR TOWN	RECORD I CLERK

00 NOV 30 PM 12: 58

VOL 1249 PG 156

BY KATELENE N. 2001

Exhibit B

Property Card

99 DAY HILL RD Property Location Map ID 42/ 108/ 1/D / Bldg Name State Use 903W Vision ID 3481 Account # 09310.00 Bldg # 1 Sec # 1 of 1 Card # 1 of 1 Print Date 4/17/2020 9:33:04 AM **CURRENT OWNER CURRENT ASSESSMENT** TOPO UTILITIES STRT / ROAD LOCATION 1 Leve 2 Public Water 1 Paved Description Code Appraised Assessed WINDSOR TOWN OF 6164 3 Public Sewer IEX COM LN 21 766.900 536.830 PUBLIC WORKS GARAGE 4 Gas EX COM BL 22 1.633.800 1.143.660 SUPPLEMENTAL DATA WINDSOR, CT EX CM OTB 25 156.300 109.410 275 BROAD STREET Alt Prol ID 9310 CTRACT 4735.01 0 INC: CBLOCK GH DIST WINDSOR CT 06095 **HEART VISION** 1376200 2007 **GL YEAR** 9310 GIS ID Assoc Pid# Total 2,557,000 1.789.900 RECORD OF OWNERSHIP SALE DATE | Q/U | V/I | SALE PRICE VC **BK-VOL/PAGE** PREVIOUS ASSESSMENTS (HISTORY) Code Year Code Assessed Year Code Assessed Year Assessed WINDSOR TOWN OF 0334 0257 11-04-1977 2019 21 536,830 2018 21 536,830 2017 21 536,830 22 972,790 1.143.660 22 1.143.660 22 25 109,410 25 109,410 25 103,460 Total 1789900 Total 1789900 Total 1613080 **EXEMPTIONS** OTHER ASSESSMENTS This signature acknowledges a visit by a Data Collector or Assessor Year Code Description Amount Code Description Number Amount Comm Int 2011 BAAX MUNICIPAL 0.00 APPRAISED VALUE SUMMARY Appraised Bldg. Value (Card) 1,595,900 Total 0.00 ASSESSING NEIGHBORHOOD Appraised Xf (B) Value (Bldg) 37,900 Nbhd Sub Nbhd Name В Tracing Batch Appraised Ob (B) Value (Bldg) 156,300 300 Α 766,900 Appraised Land Value (Bldg) **NOTES** Special Land Value 9310.00 .04 AC REMOVED & ASSESSED 2,557,000 Total Appraised Parcel Value 0042-0108-0001 D TO AT&T WIRELESS FOR CELL Valuation Method BTR89-CLERICAL TOWER 10/01/2001 **CANOPY ADDED 10-92** ADDED STORAGE SHED 10/04 REF:V1277 P506 7-18-01 ELEC EASEMENT V1277 P511 Total Appraised Parcel Value 2.557,000 BUILDING PERMIT RECORD **VISIT / CHANGE HISTORY** Permit Id Issue Date Type Description Amount Insp Date % Comp Date Comp Comments Date ld Type Is О Purpost/Result 10-01-2019 UPGRADE FIRE ALARM SYS E-190462 03-12-2019 FP 08-16-2019 100 01-18-1990 JM 43 Change - Reinspection Rer Fire Protect B041271 08-24-2004 NC New Construct STORAGE SHED 04-13-1988 GH 00 Measur+Listed LAND LINE VALUATION SECTION В Use Code Description Land Type Land Units Unit Price I. Factor Site Index Nhbd Adi Adj Unit Pric Land Value Zone Cond. Nbhd. Notes Location Adjustment 903W Municipal MDL-9 ΝZ 5.000 AC 82,000 1.00000 1.00 300 1.600 656,000 903W Municipal MDL-9 NΖ 6.760 AC 82.000 | 1.00000 0 0.20 1.000 110,900 Parcel Total Land Area: 11.7600 Total Card Land Units 11.760 AC Total Land Value 766,900
 Property Location
 99 DAY HILL RD
 Map ID
 42/ 108/ 1/D /
 Bldg Name
 State Use 903W

 Vision ID
 3481
 Account # 09310.00
 Bldg # 1
 Sec # 1 of 1
 Card # 1 of 1
 Print Date 4/17/2020 9:33:06 AM

Vision .	Í D 348	1		Α	.ccount #	09310.	.00			•				Bldg		
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BAS (36,300 sf)

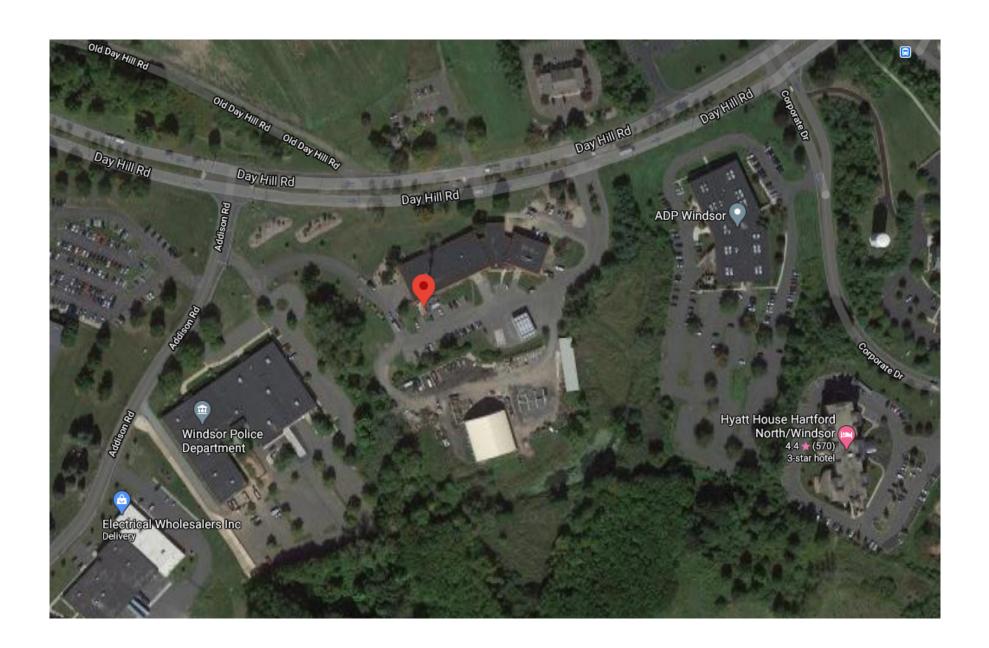


Exhibit C

Construction Drawings

T--Mobile---

T-MOBILE SITE NUMBER: CTHA267A

CTHA267A T-MOBILE SITE NAME:

SITE TYPE: **MONOPOLE**

168'-0" **TOWER HEIGHT:**

BUSINESS UNIT #:842875

99 DAY HILL ROAD **SITE ADDRESS:** WINDSOR, CT 06095

COUNTY: HARTFORD

JURISDICTION: TOWN OF WINDSOR

T-MOBILE SPRINT RETAIN SITE CONFIGURATION: 67E5998E_1xAIR+1OP

SITE INFORMATION

HARTFORD

CROWN CASTLE USA INC. SITE NAME:

SITE ADDRESS:

WINDSORDAY HILL 99 DAY HILL ROAD WINDSOR, CT 06095

COUNTY: MAP/PARCEL#:

9310.01 AREA OF CONSTRUCTION: EXISTING

LATITUDE: 41.87116388° (41° 52' 16.10") -72.67107500° (-72° 40' 16.00") LONGITUDE LAT/LONG TYPE: NAD83

GROUND ELEVATION: 166 FT CURRENT ZONING: 903W IURISDICTION: HARTFORD COUNTY

OCCUPANCY CLASSIFICATION: U

TYPE OF CONSTRUCTION:

A.D.A. COMPLIANCE: FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION

PROPERTY OWNER:

AT&T MOBILITY 575 MOROSGO DR SUITE 13-F WEST TOWER ATTN: NREA TAX DEPT

ATLANTA, GA 30324

CROWN CASTLE TOWER OWNER:

2000 CORPORATE DRIVE CANONSBURG, PA 15317

CARRIER/APPLICANT:

35 GRIFFIN ROAD BLOOMFIELD, CT 06002

ELECTRIC PROVIDER:

TELCO PROVIDER:

PROJECT TEAM

A&E FIRM

1033 WATERVLIET SHAKER RD.

ALBANY, NY 12205

CROWN CASTLE USA INC. DISTRICT

CONTACTS

1500 CORPORATE DRIVE CANONSBURG, PA 15317

TRICIA PELON - PROJECT MANAGER TRICIA PELON@CRÓWNCASTLE.COM

JASON D'AMICO - CONSTRUCTION MANAGER JASON.DAMICO@CROWNCASTLE.COM

DRAWING INDEX

SHEET#	SHEET DESCRIPTION
T-1	TITLE SHEET
T-2	GENERAL NOTES
C-1	SITE PLAN & ENLARGED SITE PLAN
C-2	FINAL ELEVATION & ANTENNA PLANS
C-3	ANTENNA & CABLE SCHEDULE
C-4	PLUMBING DIAGRAM
C-5	EQUIPMENT SPECS
C-6	EQUIPMENT SPECS
E-1	AC PANEL SCHEDULE & ONE LINE DIAGRAM
G-1	ANTENNA GROUNDING DIAGRAM
G-2	GROUNDING DETAILS
ALL DRAW	TNGS CONTAINED HEREIN ARE FORMATTED FOR

---. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OF BE RESPONSIBLE FOR SAME

PROJECT DESCRIPTION

THE PURPOSE OF THIS PROIECT IS TO ENHANCE BROADBAND CONNECTIVITY AND CAPACITY TO THE EXISTING ELIGIBLE WIRELESS FACILITY

TOWER SCOPE OF WORK:

- REMOVE (6) ANTENNAS REMOVE (9) RRUs
- REMOVE (3) HYBRID CABLES • REMOVE (3) RET CABLES
- INSTALL (6) ANTENNAS
- INSTALL (6) RRUs INSTALL (4) HYBRID CABLES
- MODIFY (1) ANTENNA PLATFORM

- REMOVE (2) EQUIPMENT CABINETS
- INSTALL (1) 6160 & (1) B160 BATTERY CABINETS • INSTALL (1) PSU4813 BOOSTER IN (P) CABINET
- INSTALL (3) BB6648 IN (P) CABINETS
- INSTALL (1) IXRE ROUTER IN (P) CABINET

PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN NOC AT (800) 788-7011 & CROWN CONSTRUCTION MANAGER.



LOCATION MAP

APPLICABLE CODES/REFERENCE **DOCUMENTS**

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

 $\frac{\text{CODE TYPE}}{\text{BUILDING}}$ 2018 CT STATE BUILDING CODE MECHANICAL

REFERENCE DOCUMENTS:

STRUCTURAL ANALYSIS: MORRISON HERSHFIELD DATED: 10/08/2021

10/05/2021

MOUNT ANALYSIS: GPD ENGINEERING AND ARCHITECTURE DATED: PROFESSIONAL CORPORATION

RFDS REVISION: 1

DATED: 08/03/2021 ORDER ID: 584557

REVISION: (

CALL CONNECTICUT ONE CALL (800) 922-4455 CBYD.COM

APPROVALS

SIGNATURE DATE PROPERTY OWNER OR REP LAND USE PLANNER T-MOBILE OPERATIONS NETWORK BACKHAUL

CONSTRUCTION MANAGER

THE PARTIES ABOVE HEREBY APPROVE AND ACCEPT THESE DOCUMENTS AND AUTHORIZE THE CONTRACTOR TO PROCEED WITH THE CONSTRUCTION DESCRIBED HEREIN. ALL CONSTRUCTION DOCUMENTS ARE SUBJECT TO REVIEW BY THE LOCAL BUILDING DEPARTMENT AND ANY CHÂNGES AND MODIFICATIONS THEY MAY IMPOSE

BLOOMFIELD, CT 06002



the solutions are endless

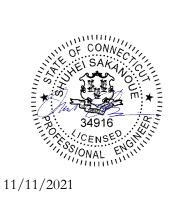
T-MOBILE SITE NUMBER: CTHA267A

BU #: **842875** WINDSORDAY HILL

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

	ISSUED FOR:								
REV	DATE	DRWN	DESCRIPTION	DES./QA					
Α	03/17/21	RCD	PRELIMINARY	SS					
0	04/01/21	BMM	FINAL	SS					
1	10/08/21	CB	FINAL	SS					
2	11/11/21	TJ	SA UPDATE	SS					



IT IS A VIOLATION OF LAW FOR ANY PERSON NLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT

CROWN CASTLE USA INC. SITE ACTIVITY REQUIREMENTS:

- NOTICE TO PROCEED- NO WORK SHALL COMMENCE PRIOR TO CROWN CASTLE USA INC. WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE STUDY OF THE CROWN CASTLE USA INC. NOC AT 800-788-7011 & THE CROWN CASTLE USA INC. CONSTRUCTION MANAGER.
- "LOOK UP" CROWN CASTLE USA INC. SAFETY CLIMB REQUIREMENT: THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGET AND REPORTED TO YOUR CROWN CASTLE USA INC. POC OR CALL THE NOC TO GENERATE A SAFET MAINTENANCE AND CONTRACTOR NOTICE TICKET.
- PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS
- ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND CROWN CASTLE USA INC. STANDARD CED-STD-10253, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA-322
- ALL SITE WORK TO COMPLY WITH DAS-STD-10068 "INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON CROWN CASTLE USA INC. TOWER SITE," CED-STD-10294 "STANDARD FOR INSTALLATION OF MOUNTS AND APPURTENANCES," AND LATEST VERSION OF ANSI/TIA-1019-A-2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS." IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR
- SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY CROWN CASTLE USA INC. PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK, ALL WORK CARRIED OUT SHALL COMPLY V ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES. ORDINANCES AND APPLICABLE REGULATIONS
- THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- RECOMMENDATIONS ONLESS SPECIFICALLY STATED OTHERWISE.

 9. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE START OF CONSTRUCTION.

 10. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONSINTED REASES. PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFETY PROCEDURES
- ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.
- SPECIFICATIONS, LATEST APPROVED REVISION.

 CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.

 ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE
- EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF CONTRACTOR, TOWER OWNER, CROWN CASTLE USA INC., AND/OR LOCAL UTILITIES.
- THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.
- THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER'S EQUIPMENT
- 16. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
- THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
- CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION, EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
- 19. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION
- 20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.
- 22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

GENERAL NOTES:

- FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:
 CONTRACTOR: GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION
 CARRIER: T-MOBILE
 - FOWER OWNER: CROWN CASTLE USA INC.
- TOWER OWNER: CROWN CASTLE USA INC.
 THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY
 EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS
 ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE
 WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY
 ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
- MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
 THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF
 CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS,
 TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR
 PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED
 TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE
 INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY.
 NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL
 DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT,
 AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS,
 GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER
 CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.
 SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO
 ASSIST IN THE FABRICATION AND OR PLACEMENT OF CONSTRUCTION FLEMENTS BUT IT IS THE SOLE RESPONSIBILITY.
- ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS.
- CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CÚTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.

 PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL WISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CROWN CASTLE.

 ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LONG AND LONG AND LONG AND LONG THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.

 UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.

 THE CONTRACTOR SHALL INSTALL ALL FOLIPMENT AND MATERIALS, AND ACCORDANCE WITH MANUFACTURER'S

- THE CONTRACTOR SHALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.

 THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S

 RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.

 IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL

 PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND CROWN CASTLE PRIOR TO PROCEEDING

 WITH ANY SUCH CHANGE OF INSTALLATION.

 CONTRACTOR IS TO PEFFORM A SITE INVESTIGATION AND IS TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR

 POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN
- THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMEN<mark>TS, PAVEMENTS, CURBS, LANDSC</mark>APING AND STRUCTURES, ANY
- DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF CROWN CASTLE USA INC. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY, ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION
- DESIGNALE DESCRIPTION.

 CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDI<mark>TION. TRASH AND</mark> DEBRIS SHOULD BE REMOVED FROM SITE ON

CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST—IN—PLACE CONCRETE.
 UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED
- TO BE 1000 psf.
- TO BE 1000 psf.
 ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) OF 3000 psi AT 28 DAYS, UNLESS NOTED
 OTHERWISE. NO MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS
 APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90°F AT TIME OF PLACEMENT. CONCRETE EXPOSED TO FREEZE—THAW CYCLES SHALL CON<mark>TAIN</mark> AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR
- ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER-TO-CEMENT RATIO (W/C) OF 0.45.
- ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE STANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS: #4 BARS AND SMALLER
- ON DRAWINGS CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH...
- CONCRETE EXPOSED TO EARTH OR WEATHER
 #6 BARS AND LARGER..... #5 BARS AND SMALLER.
- CONCRETE NOT EXPOSED TO EARTH OR WEATHER: BEAMS AND COLUMNS...
- A TOOLED EDGE OR A 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

GREENFIELD GROUNDING NOTES:

- ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC
- THE CONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.

 THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE
- METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT METAL CONDOIN AND THAT STREET BE GROUNDED WIND WADE ELECTROCKEE CONTINUOUS WITH ESTED BONDING THINGS ON BY BONDING ACCOUNT WITH THE DECK WHILE OF AN INCIDENCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS.

 CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
- CONNECTIONS OF THE GROUND BOS SHALL NOT BE DOUBLED OF OR STACKED BACK TO MEETINGS TO PPOSITE SIDE OF THE GROUND BOS ARE PERMITTED.
 ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.
 ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
 USE OF 90' BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45' BENDS CAN BE ADEQUATELY SUPPORTED.
 EXCITERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.

- ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.

 COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.

 ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.

- APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE ESED ON ALL COMPRESSION AND BOLLED GOVER GROUND CONNECTIONS.

 ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.

 MISCELLANGROUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.

 BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) \$\frac{1}{2}\$ BARE SOLID TINNED COPPER GROUND CONDUCTORS.

 GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LICHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS,

 METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.

 ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON-METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD-WELD TERMINATION
- POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SLICCONE CALLK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).

 BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/O COPPER, ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY).

ELECTRICAL INSTALLATION NOTES:

- ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE
- FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
 CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED. AND TRIP HAZARDS ARE FLIMINATED

- AND TRIP HAZARDS ARE ELIMINATED.
 WRINING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.
 ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.
 ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO
 REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.
 ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT
 CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERYIPY AVAILABLE SHORT CIRCUIT CURRENT DOES
 NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT
 ADDRIED CODE DEET THE COVERNING MUSICIPATION.
- NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE PRE THE GOVERNING JURISDICTION.

 EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.

 ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND ADDRESS OF THE PARTY OF THE PARTY OF THE PROPERTY OF THE PARTY OF T CIRCUIT ID'S).

- PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
 ALL TIE WRAPS SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.
 ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER)
 WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.

 POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS
- OTHERWISE SPECIFIED.
- POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75 C (90° C IF AVAILABLE).
- RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE
- 15. ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR
- EXPOSED INDOOR LOCATIONS.
- ELECTRICAL METALLIC TUBING (EMT) OR METAL-CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.

 SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE
- GRADE PVC CONDUIT. 18. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION
- COURS OR FLEXIBLE METALLIC CONDOIT (LIQUID-THE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.

 CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION—TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
- CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND
- WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS
- (WIREMOLD SPECMATE WIREWAY).
 SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).
- SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).

 CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE

 DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE

 LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES
 IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN

 A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERFENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUITS SHALL BE TEMPARILY CAPPED FILUSH TO FINISH GRADE TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE. ROUPENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY—COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3R (OR
- BETTER) FOR EXTERIOR LOCATIONS.
- METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY—COATED OR NON—CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS
- NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATEL

 NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS. 27. THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR CROWN CASTLE USA INC.
- BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.

 THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY. 28.
- 29. INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "T-MOBILE".
- ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.

CONDUCTOR COLOR CODE								
SYSTEM	CONDUCTOR	COLOR						
	A PHASE	BLACK						
120/240V, 1Ø	B PHASE	RED						
120/2400, 10	NEUTRAL	WHITE						
	GROUND	GREEN						
	A PHASE	BLACK						
	B PHASE	RED						
120/208V, 3Ø	C PHASE	BLUE						
	NEUTRAL	WHITE						
	GROUND	GREEN						
	A PHASE	BROWN						
	B PHASE	ORANGE OR PURPLE						
277/480V, 3Ø	C PHASE	YELLOW						
	NEUTRAL	GREY						
	GROUND	GREEN						
DC VOLTAGE	POS (+)	RED**						
DC VOLIAGE	NEG (-)	BLACK**						

* SEE NEC 210.5(C)(1) AND (2)
** POLARITY MARKED AT TERMINATION

ANTENNA EXISTING FACILITY INTERFACE FRAME GEN GPS GSM LTE MGB MW GENERATOR GLOBAL POSITIONING SYSTEM GLOBAL SYSTEM FOR MOBILE LONG TERM EVOLUTION MASTER GROUND BAR MICROWAVE (N) NEC NEW NATIONAL FLECTRIC CODE (P) PROPOSED POWER PLANT QTY RECT QUANTITY RECTIFIER RADIO BASE STATION REMOTE ELECTRIC TILT
RADIO FREQUENCY DATA SHEET
REMOTE RADIO HEAD
REMOTE RADIO UNIT TOWER MOUNTED AMPLIFIER UMTS UNIVERSAL MOBILE TELECOMMUNICATIONS SYSTEM APWA UNIFORM COLOR CODE:

WHITE PROPOSED EXCAVATION TEMPORARY SURVEY MARKINGS ELECTRIC POWER LINES, CABLES, CONDUIT, AND LIGHTING CABLES

YELLOW GAS, OIL, STEAM, PETROLEUM, OR GASEOUS MATERIALS COMMUNICATION, ALARM OR SIGNAL LINES, CABLES, OR CONDUIT AND TRAFFIC LOOPS

POTABLE WATER RECLAIMED WATER, IRRIGATION, AND SLURRY LINES

SEWERS AND DRAIN LINES.

CROWN 2000 CORPORATE DRIVE

35 GRIFFIN ROAD

BLOOMFIELD, CT 06002

NFINIGY&

the solutions are endless

CANONSBURG, PA 15317

T-MOBILE SITE NUMBER: CTHA267A

BU #: **842875** WINDSORDAY HILL

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

Œ			ISSUI	ED FOR:	
	REV	DATE	DRWN	DESCRIPTION	DES./Q
	Α	03/17/21	RCD	PRELIMINARY	SS
	0	04/01/21	BMM	FINAL	SS
	1	10/08/21	CB	FINAL	SS
	2	11/11/21	TJ	SA UPDATE	SS
	ш				



IT IS A VIOLATION OF LAW FOR ANY PERSON UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER. TO ALTER THIS DOCUMENT

SHEET NUMBER

NOTE:

1. PLANS BASED ON SITE PLAN PROVIDED BY TOWER OWNER
AND SITE VISIT PERFORMED BY INFINICY. CONTRACTOR SHALL
FIELD VERIFY ALL DIMENSIONS AND LOCATION/ORIENTATION
OF EXISTING T-MOBILE EQUIPMENT.

NOTES:

THE POWER DESIGN FOR ANY AC ELECTRICAL POWER CHANGES IS TO BE PERFORMED BY OTHERS AND IS SHOWN HERE FOR REFERENCE PURPOSES ONLY.
T—MOBILE IS SOLELY RESPONSIBLE FOR THE ELECTRICAL POWER DESIGN.

T··Mobile··

35 GRIFFIN ROAD BLOOMFIELD, CT 06002



INFINIGY FROM ZERO TO INFINIGY

the solutions are endless
3 Watervijet Shaker Rd | Albany, NY 12205

T-MOBILE SITE NUMBER: **CTHA267A**

BU #: 842875 WINDSORDAY HILL

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

ISSUED FOR:									
REV	DATE	DRWN	DESCRIPTION	DES./Q/					
Α	03/17/21	RCD	PRELIMINARY	SS					
0	04/01/21	BMM	FINAL	SS					
1	10/08/21	CB	FINAL	SS					
2	11/11/21	TJ	SA UPDATE	SS					



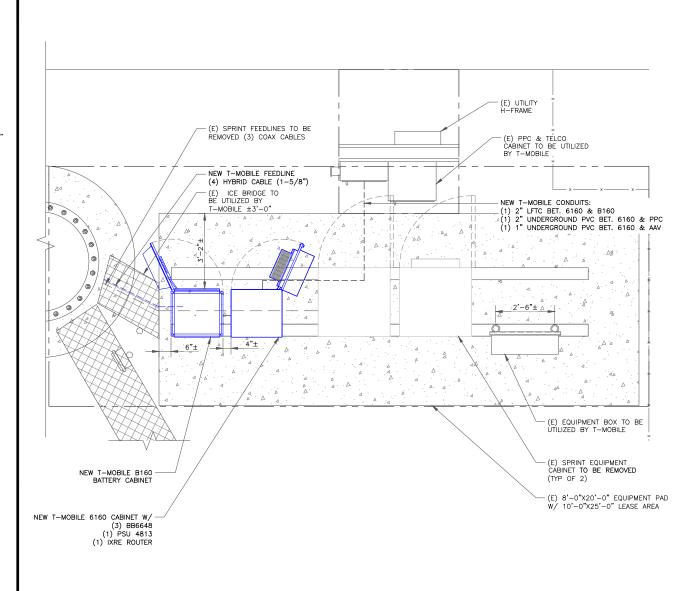
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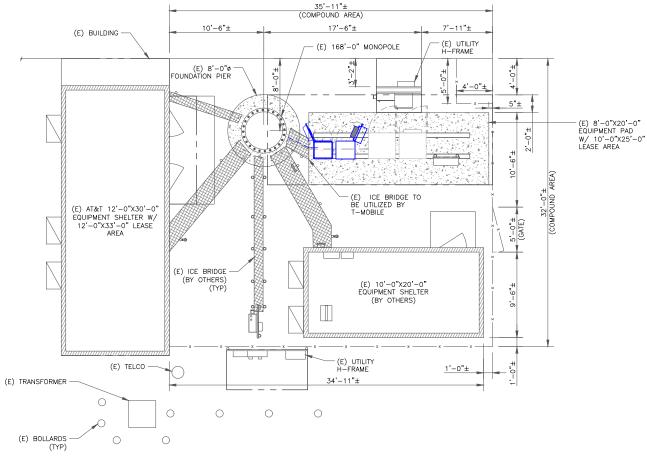
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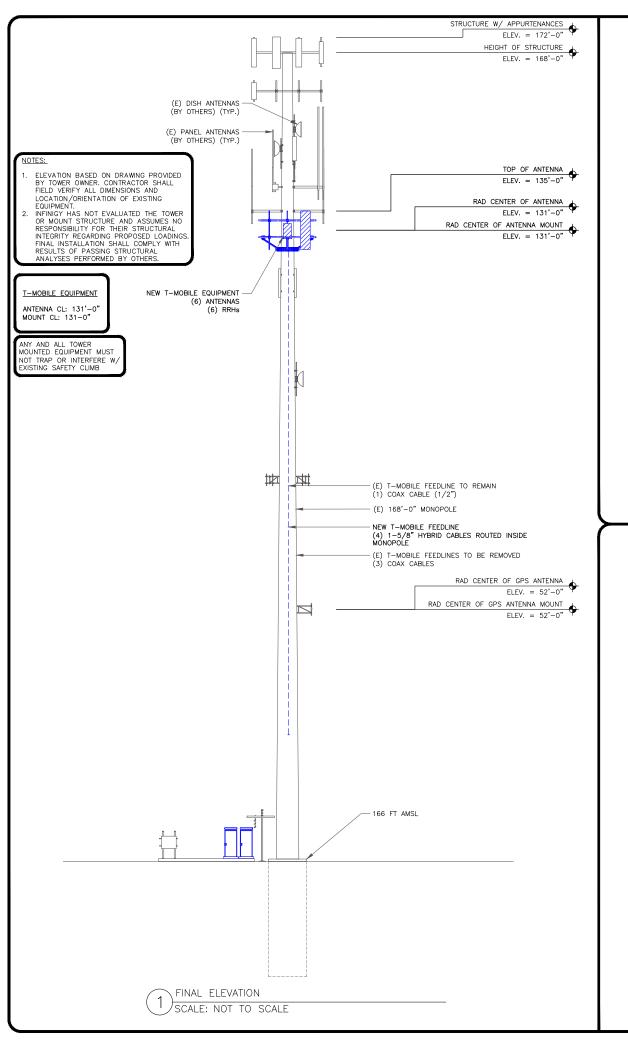
SITE PLAN

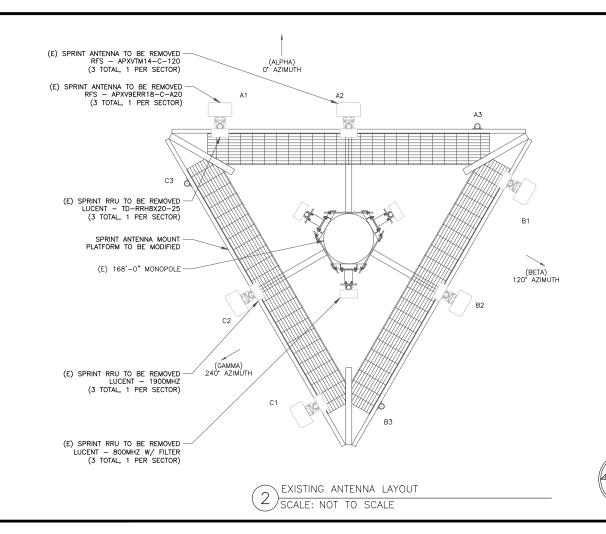
SCALE: 4' 2' 0 4' 3/32"=1'-0" (FULL SIZE) 3/32"=1'-0" (11x17)



ENLARGED SITE PLAN

1/2"=1'-0" (FULL SIZE)
2' 1' 0 2' 1/4"=1'-0" (11x17)







35 GRIFFIN ROAD BLOOMFIELD, CT 06002



CANONSBURG, PA 15317

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T-MOBILE SITE NUMBER: CTHA267A

BU #: **842875** WINDSORDAY HILL

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

	(ALCUA)	
A1	A2 (ALPHA) O' AZIMUTH A3	
NEW T-MOBILE RRU ERICSSON - RADIO 4480 B71+B85 (3 TOTAL, 1 PER SECTOR)		
C3 NEW T-MOBILE RRU ERICSSON - RADIO 4460 B25+B66 (3 1071AL, 1 PER SECTOR)		B1
(GAMMA) 240° AZIMUTH		NEW T-MOBILE ANTENNA RFS - APXVAALL24_43-U-NA20 (3 TOTAL, 1 PER SECTOR)
(E) 168'-0" MONOPOLE	B2	NEW T-MOBILE ANTENNA
	(BETA) 120° AZIMUTH	ERICSSON — AIR6449 B41 (3 TOTAL, 1 PER SECTOR)
MODIFIED T-MOBILE PLATFORM (SEE NOTE)	C B3	
	FINAL ANTENNA LAYOUT SCALE: NOT TO SCALE	N

ſ	ISSUED FOR:									
REV	DATE	DRWN	DESCRIPTION	DES./QA						
Α	03/17/21	RCD	PRELIMINARY	SS						
0	04/01/21	BMM	FINAL	SS						
1	10/08/21	CB	FINAL	SS						
2	11/11/21	TJ	SA UPDATE	SS						



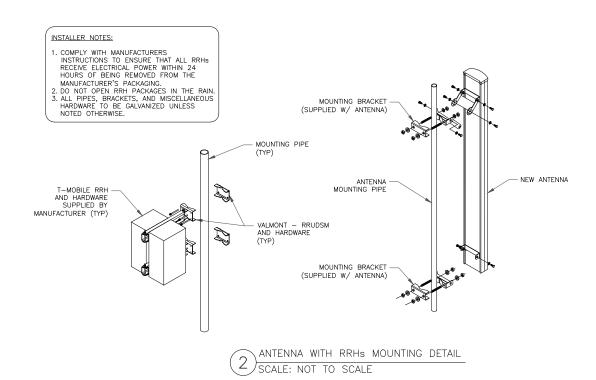
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SHEET NUMBER:

						ANTENNA SCHEDULE				
SECTOR	POS.	TECHNOLOGY	RAD CENTER	AZIMUTH	ANTENNA MANUFACTURER	ANTENNA MODEL	MECH. TILT	ELECT. TILT	TOWER MOUNTED EQUIPMENT	FEEDLINE TYPE
ALPHA	A1	L700, L600, N600, L1900, G1900, L2100	131'-0"	0,	RFS	APXVAALL24_43-U-NA20	0.		(1) ERICSSON - RRUS 4480 B71+B85 (1) ERICSSON - RRUS 4460 B25+B66	
ALPHA	A2	L2500, N2500	131'-0"	0*	ERICSSON	AIR6449 B41	0,			(4) 1-5/8" HYBRID (SHARED)
ALPHA	A3]
BETA	B1	L700, L600, N600, L1900, G1900, L2100	131'-0"	120°	RFS	APXVAALL24_43-U-NA20	0,		(1) ERICSSON - RRUS 4480 B71+B85 (1) ERICSSON - RRUS 4460 B25+B66	
BETA	B2	L2500, N2500	131'-0"	120°	ERICSSON	AIR6449 B41	0,			(4) 1-5/8" HYBRID (SHARED)
BETA	В3									
GAMMA	C1	L700, L600, N600, L1900, G1900, L2100	131'-0"	240°	RFS	APXVAALL24_43-U-NA20	0,		(1) ERICSSON - RRUS 4480 B71+B85 (1) ERICSSON - RRUS 4460 B25+B66	
GAMMA	C2	L2500, N2500	131'-0"	240°	ERICSSON	AIR6449 B41	0,			(4) 1-5/8" HYBRID (SHARED)
GAMMA	С3									

ANTENNA AND CABLE SCHEDULE
SCALE: NOT TO SCALE



NOTE:

 CONTRACTOR SHALL INSTALL 3RD DUAL RRH MOUNT TO ACCOMMODATE ALL RRH BRACKETS HOLES IF NECESSARY. T - Mobile - - -



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Phone: 518-690-0790 | Fax: 518-690-0793

T-MOBILE SITE NUMBER: **CTHA267A**

BU #: **842875 WINDSORDAY HILL**

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

	ISSUED FOR:								
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0	04/01/21	BMM	FINAL	SS					
1	10/08/21	CB	FINAL	SS					
2	11/11/21	TJ	SA UPDATE	SS					



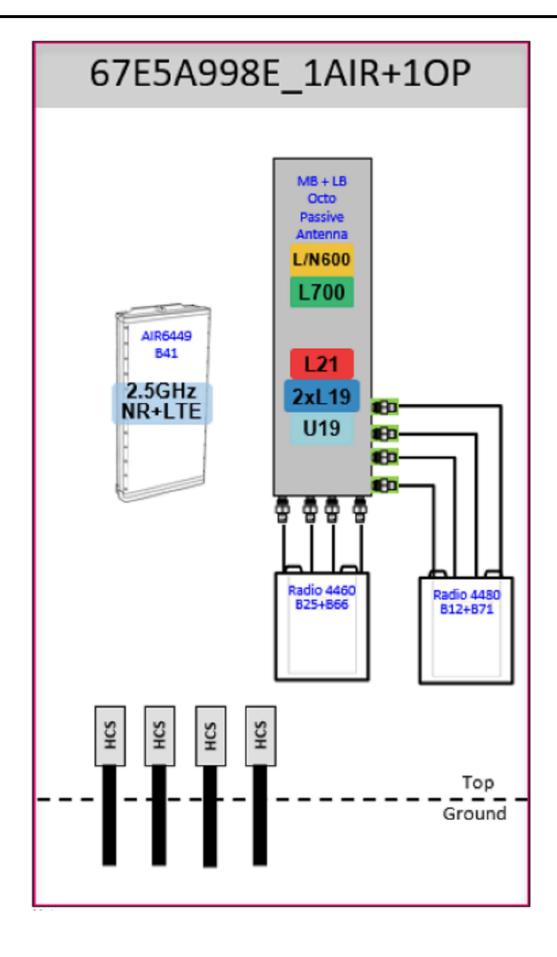
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SHEET NUMBER:

C-3

2





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T-MOBILE SITE NUMBER: **CTHA267A**

BU #: 842875 WINDSORDAY HILL

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

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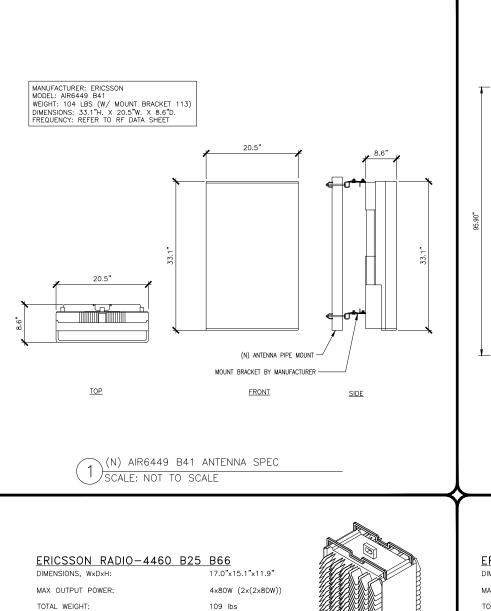


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SHEET NUMBER:

C-4

2



-40° TO 55° C

SIDE VIEW

(N) RADIO 4460 SPEC

(4) SCALE: NOT TO SCALE

(N) ANTENNA PIPE MOUNT

MOUNTING BRACKET — PROVIDED BY MANUFACTURER

(N) RADIO 4460

(N) ANTENNA PIPE MOUNT

(N) RADIO 4460

MOUNTING BRACKET — PROVIDED BY MANUFACTURER

<u>PLAN VIEW</u>

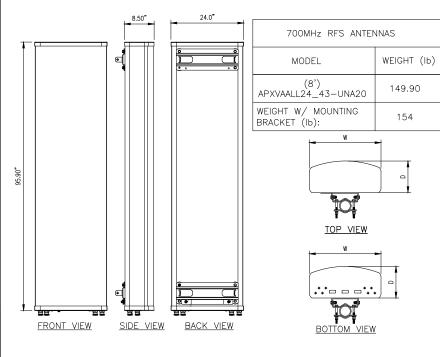
SIDE VIEW

TEMPERATURE:

מוחוחוווויים,

TOP VIEW

FRONT VIEW



(N) APXVAALL24_43-UNA20 ANTENNA SPEC $(2)^{(N)}$ SCALE: NOT TO SCALE

NOT USED SCALE: NOT TO SCALE



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T-MOBILE SITE NUMBER: CTHA267A

BU #: **842875** WINDSORDAY HILL

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

	ISSUED FOR:										
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0	04/01/21	BMM	FINAL	SS							
1	10/08/21	CB	FINAL	SS							
2	11/11/21	TJ	SA UPDATE	SS							

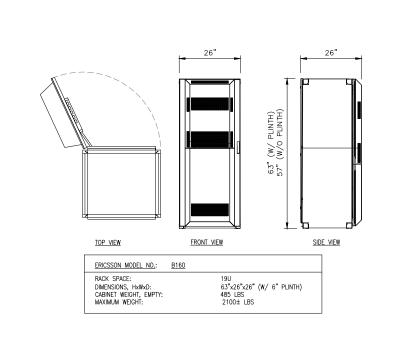


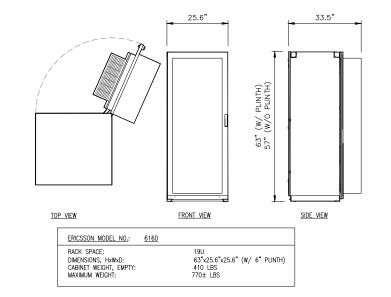
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ERICSSON RADIO-4480 DIMENSIONS, WxDxH: MAX OUTPUT POWER: TOTAL WEIGHT: TEMPERATURE:	B71 B85 21.8"x15.7"x7.5" 4x80W (2x(2x80W)) 93 lbs -40° T0 55° C	
TOP VIEW	(N) ANTENNA PIPE — MOUNTING BRACKET — PROVIDED BY MANUFACTURER (N) RADIO 4480 —	PLAN VIEW
FRONT VIEW	(N) ANTENNA — PIPE MOUNT (N) RADIO 4480 — SIDE VIEW MOUNTING BRACKET — PROVIDED BY MANUFACTURER	SIDE VIEW
h	ADIO 4480 SPEC :: NOT TO SCALE	

NOT USED (6) SCALE: NOT TO SCALE





NEW EQUIPMENT CABINET PER PLAN

(4) KWIK BOLT TZ-CS 1/2

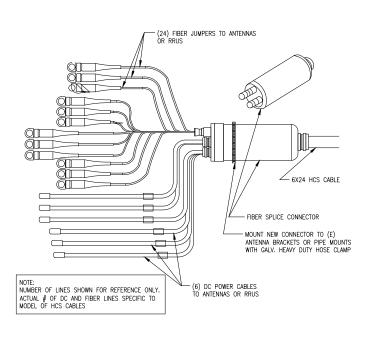
(3 1/4) ESR-1917 SPECIAL INSPECTION REQUIRED

(E) CONC. PAD

(N) EQUIPMENT CABINET MOUNTING DETAIL SCALE: NOT TO SCALE

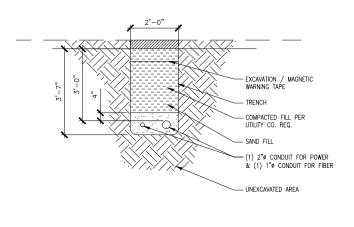
(N) B160 CABINET DETAIL
SCALE: NOT TO SCALE

(N) 6160 CABINET DETAIL SCALE: NOT TO SCALE



(N) 6X12 HCS CABLE DETAIL

(4) SCALE: NOT TO SCALE



(N) CONDUIT TRENCH DETAIL

SCALE: NOT TO SCALE

NOT USED
SCALE: NOT TO SCALE

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BLOOMFIELD, CT 06002



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T-MOBILE SITE NUMBER: **CTHA267A**

BU #: **842875 WINDSORDAY HILL**

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

	ISSUED FOR:											
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2

NOTES:

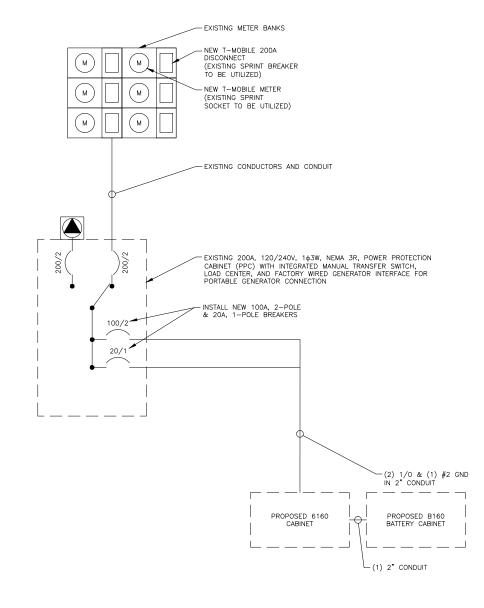
 EXISTING DISTRIBUTION PANEL WAS NOT ACCESSIBLE DURING SITE VISIT PERFORMED BY INFINIGY ON 07/21/20. CONTRACTOR SHALL INFORM ENGINEER IF THERE ARE ANY DISCREPANCIES IN PANEL SCHEDULE.

T-MOBILE PANEL SCHEDULE												
MAIN: 200A MAIN BREAKER	MAIN: 200A MAIN BREAKER VOTAGE/PHASE: 120/2					40V, 1-PHASE, 3-WIRE				SHORT CIRCUIT CURRENT RATING:		
MOUNTING: INSIDE PPC ENCL	OSURE		ENCLOSUR	E: NEMA 3R					SURGE PRO	DTECTION DE	EVICE: YES	
					PHASE LO	ADS (VA)						
DESCRIPTION	LOAD (VA)	C or NC	C/B	CIR No.	Α	В	CIR No.	C/B	CorNC	LOAD (VA)	DESCRIPTION	
6160	7000	С	100	1	7001		2	60	С	1	SURGE PROTECTION	
6160	7000	С	100	3		7001	4	00	С	1	30KGL FROTECTION	
6160 GFI	180	С	20	5	380		6	20	NC	200	TOWER LIGHTS	
				7		200	8	20	NC	200	TOWEREIGITIS	
				9	0		10					
				11		0	12					
				13	0		14					
BLANK				15		0	16				BLANK	
				17	0		18				BEAIN	
				19		0	20					
				21	0		22					
				23		0	24					
BASE LOAD (VA) =					7381	7201						
25% OF CONTINUOUS LOAD (VA) =					1750	1750	C = CONTINUOUS LOAD; NC = NON-CONTINUOUS LOAD					
TOTAL LOAD (VA) =					9131	8951						
	TOTAL LOAD (A) =						NEW BREAKER TO BE SAME TYPE AND HAVE SAME AIC RATING AS EXISTING.					

CUSTOMER HAS NOT PROVIDED LOADS FOR EQUIPMENT CABINETS THEREFORE THE CABINET LOADS SHOWN ARE ESTIMATED VALUES.

NOTES:

- ALL NEW CONDUCTORS TO BE INSTALLED SHALL BE COPPER. ALL CONDUCTORS SHALL BE THHW, THWN, THWN-2, XHHW, OR XHHW-2 UNLESS NOTED OTHERWISE.
- CONTRACTOR IS TO FIELD VERIFY ALL EXISTING ITEMS SHOWN ON THE ELECTRICAL ONE—LINE DIAGRAM AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES.
- 3. ALL GROUNDING AND BONDING PER THE NEC.





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T-MOBILE SITE NUMBER: **CTHA267A**

BU #: **842875 WINDSORDAY HILL**

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

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SHEET NUMBER:

 E_{-1}

2

1) AC PANEL SCHEDULE SCALE: NOT TO SCALE GROUND WIRE (TYP)

SECTOR GROUND BAR (3 TOTAL)

<u>BETA</u>

<u>ALPHA</u>

NOTE

<u>GAMMA</u>

ALL NEW GROUNDS TO BE #6 STRANDED COPPER WITH GREEN INSULATION UNLESS NOTED OTHERWISE.

ANTENNA GROUNDING DIAGRAM
SCALE: NOT TO SCALE



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2	11/11/21	TJ	SA UPDATE	SS							



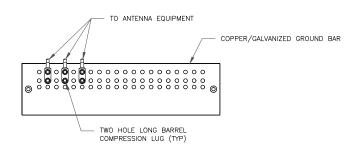
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SHEET NUMBER:

G-1

2



NOTES:

- 1. DOUBLING UP "OR STACKING" OF CONNECTIONS IS NOT PERMITTED.
- 2. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
- 3. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO ANTENNA MOUNT STEEL.

ANTENNA SECTOR GROUND BAR DETAIL SCALE: NOT TO SCALE

TO BASE STATION CABINET & HYBRID CABLES

COPPER/GALVANIZED GROUND BAR

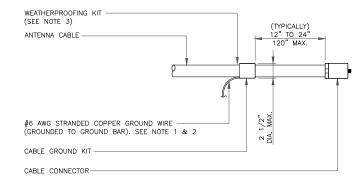
COPPER/GALVANIZED GROUND BAR

#2 SOLID TINNED COPPER
CONDUCTOR TO TOWER/SHELTER
GROUND RING (2 TYP. FOR
BOTTOM GROUND BAR ONLY)

NOTES:

- 1. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
- 2. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO TOWER STEEL (TOWER ONLY).
- 3. GROUND BAR SHALL BE ISOLATED FROM BUILDING OR SHELTER.

TOWER/SHELTER GROUND BAR DETAIL
SCALE: NOT TO SCALE



NOTES:

- DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
- GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
- 3. WEATHER PROOFING SHALL BE TWO-PART TAPE KIT, COLD SHRINK SHALL NOT

CABLE GROUND KIT CONNECTION
SCALE: NOT TO SCALE

NOT USED

SCALE: NOT TO SCALE





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T-MOBILE SITE NUMBER: **CTHA267A**

BU #: 842875 WINDSORDAY HILL

99 DAY HILL ROAD WINDSOR, CT 06095

EXISTING 168'-0" MONOPOLE

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1	10/08/21	CB	FINAL	SS							
2	11/11/21	TJ	SA UPDATE	SS							



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SHEET NUMBER:

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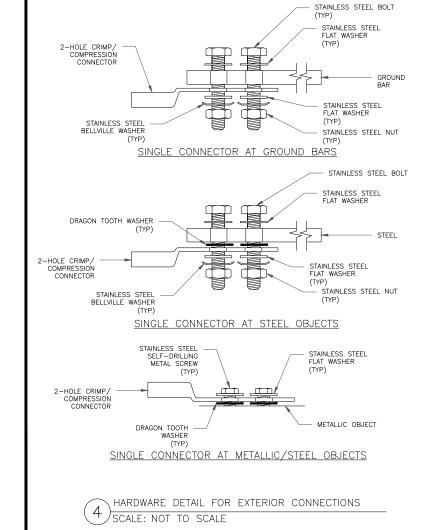




Exhibit D

Structural Analysis Report



Date: October 08, 2021

MORRISON HERSHFIELD

Morrison Hershfield 1455 Lincoln Park, Suite 500 Atlanta, GA 30346

(770)379-8500

Subject: Structural Analysis Report

Carrier Designation:Site Number:CTHA267ASite Name:CT54XC787

Crown Castle Designation: BU Number: 842875

Site Name: Windsorday Hill

 JDE Job Number:
 684573

 Work Order Number:
 2014534

 Order Number:
 584557 Rev. 0

Engineering Firm Designation: Morrison Hershfield Project Number: CN7-415R1 / 2101398

Site Data: 99 Day Hill Road, Windsor, Hartford County, CT 06095

Latitude 41° 52′ 16.1″, Longitude -72° 40′ 16″

168 Foot - Summit Monopole Tower

Morrison Hershfield is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Proposed Equipment Configuration

Sufficient Capacity-99.4%

This analysis utilizes an ultimate 3-second gust wind speed of 116 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Respectfully submitted by:

G. Lance Cooke, P.E. (CT License No. PEN.0028133) Senior Engineer No. 28133

No. 28133

No. 28133

Digitally signed by G. Lance

Cooke

Cooke

Date: 2021.10.08 09:31:30-07'00'

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tnxTower Output

6) APPENDIX B

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7) APPENDIX C

Additional Calculations

1) INTRODUCTION

This tower is a 168 ft Summit monopole tower designed by Paul J. Ford and Company.

2) ANALYSIS CRITERIA

TIA-222 Revision: TIA-222-H

Risk Category:

Wind Speed: 116 mph

Exposure Category:CTopographic Factor:1Ice Thickness:1.5 inWind Speed with Ice:50 mphService Wind Speed:60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)		Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)								
		3	ericsson	AIR6449 B41_T-MOBILE										
	131.0	131.0	131.0	131.0	3	rfs/celwave	APXVAALL24_43-U-NA20_TMO							
					131.0	131.0	131.0	131.0	131.0	131.0	131.0	3	ericsson	RADIO 4460 B2/B25 B66_TMO
130.0			3	ericsson	Radio 4480_TMOV2	3	1-5/8							
		2	Site Pro 1	Support rail kit[#HRK14-U]										
		1	Site Pro 1	V-style reinforcement kit [PRK-SFS]										
		1	-	Platform Mount [LP 1201-1]										

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
	170.0	1	rfi antennas	CC807-08		
		2	cci antennas	DMP65R-BU6D w/ Mount Pipe		
		1	cci antennas	DMP65R-BU8D w/ Mount Pipe		
		1	cci antennas	TPA-65R-LCUUUU-H8 w/Mount Pipe		
		3	kathrein	800 10121 w/ Mount Pipe		
		2	kathrein	80010965 w/ Mount Pipe		
		1	kathrein	80010966 w/ Mount Pipe		
		2		QS66512-2 w/ Mount Pipe	12	1-5/8
168.0		6	kathrein	860 10025	6	3/4 3/8 1-1/4
100.0	168.0	3	ericsson	RRUS 32 B30	2	
		3	ericsson	RRUS 32 B66	1	
		3	ericsson	RRUS 4415 B25		
		3	ericsson	RRUS 4449 B5/B12		
		3	ericsson	RRUS 4478 B14		
		3	cci antennas	DTMABP7819VG12A		
		1 raycap		DC6-48-60-0-8F		
		2	raycap	DC6-48-60-18-8F		
		1	-	Platform Mount [LP 1201-1_KCKR- HR-1]		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	
152.0	152.0	1	rfs/celwave	SC3-W100ASTX	- 1	EU 90-FR	
132.0	152.0	1	-	Pipe Mount [PM 601-1]	'	EU 90-FK	
		1	rfi antennas	BPA7496-180-11 w/ Mount Pipe			
147.0	148.0	1	rfs/celwave	SC3-W100ASTX	1	EU 90-FR	
147.0		1	kathrein	782 10876	1	1/2	
	147.0	1	-	Pipe Mount [PM 601-1]			
		1	kathrein	782 10876			
143.0	143.0	1	pctel	MPRD2449	1	1/4	
		1	-	Pipe Mount [PM 602-1]			
	148.0	1	bird technologies group	432E-83I-01-T			
	140.0		2	rfi antennas	CC807-11	2	7/8
140.0		1	telewave	ANT450F6	1	1/4	
		1	motorola	PTP400 w/Mount Pipe			
		1	ericsson	RIU			
		2	-	Side Arm Mount [SO 306-1]			
135.0	144.0	2	telewave	ANT450F6	2	1/2	
133.0	135.0	2	-	Side Arm Mount [SO 702-1]		1/2	
		3	jma wireless	MX08FRO665-21 w/ Mount Pipe			
		3	fujitsu	TA08025-B604			
120.0	120.0	3	fujitsu	TA08025-B605	1	1-1/2	
		1	raycap	RDIDC-9181-PF-48			
		1	tower mounts	Commscope MC-PK8-DSH			
100.0	100.0	1	rfs/celwave	SC3-W100ASTX	- 1	EU 90-FR	
100.0	100.0		-	Pipe Mount [PM 601-1]	<u> </u>	LO 90-FK	
79.0	79.0	2	-	Side Arm Mount [SO 901-1]	-	-	
52.0	52.0	1	pctel	GPS-TMG-HR-26NCM	- 1	1/2	
JZ.U	5∠.∪	1	-	- Side Arm Mount [SO 701-1]		1/2	

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Reference	Source
4-GEOTECHNICAL REPORTS	4529457	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	4529456	CCISITES
4-TOWER MANUFACTURER DRAWINGS	4589719	CCISITES

3.1) Analysis Method

tnxTower (version 8.1.1.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Morrison Hershfield should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	168 - 119.25	Pole	TP34.288x24x0.25	1	-15.74	1615.32	41.3	Pass
L2	119.25 - 78.5	Pole	TP42.387x32.8911x0.2813	2	-26.33	2248.05	69.1	Pass
L3	78.5 - 38.75	Pole	TP50.213x40.7166x0.375	3	-37.76	3547.28	61.2	Pass
L4	38.75 - 0	Pole	TP57.64x48.1441x0.375	4	-52.99	4186.71	72.5	Pass
							Summary	
						Pole (L4)	72.5	Pass
						Rating =	72.5	Pass

Table 5 - Tower Component Stresses vs. Capacity - LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	67.5	Pass
1	Base Plate	0	48.8	Pass
1	Base Foundation (Structure)	0	99.4	Pass
1	Base Foundation (Soil Interaction)	U	24.4	Pass

	Structure Rating (max from all components) =	99.4%*
--	--	--------

Notes:

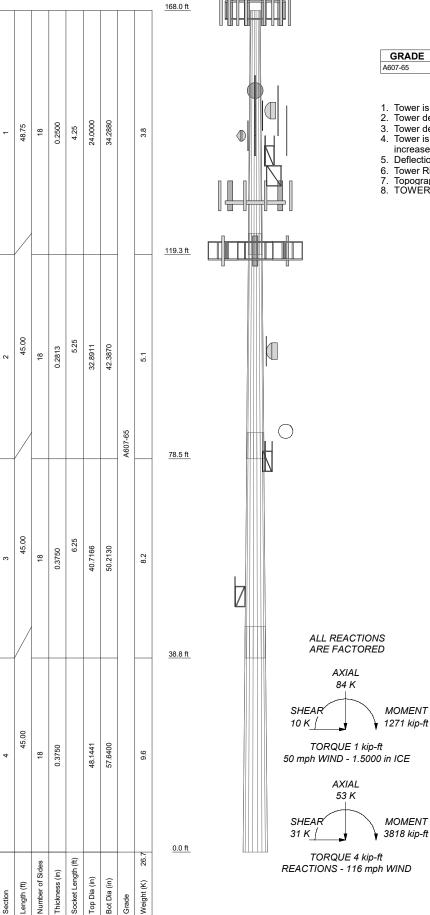
4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

¹⁾ See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

^{2) *}Rating per TIA-222-H, Section 15.5.

APPENDIX A TNXTOWER OUTPUT

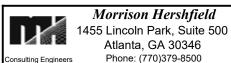


MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A607-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

- 1. Tower is located in Hartford County, Connecticut.
- 2. Tower designed for Exposure C to the TIA-222-H Standard.
- Tower designed for a 116 mph basic wind in accordance with the TIA-222-H Standard.
 Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to
 - increase in thickness with height.
- 5. Deflections are based upon a 60 mph wind.
- Tower Risk Category II.
 Topographic Category 1 with Crest Height of 0.00 ft
 TOWER RATING: 72.5%



FAX: (770)379-8501

^{Job:} CN7-415R1 / 21013		
Project: 842875 / Windsorday		
Client: Crown Castle USA	Drawn by: CSA	App'd:
Code: TIA-222-H	Date: 10/08/21	Scale: NTS
Path:		Dwg No. F-

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower is located in Hartford County, Connecticut.

Tower base elevation above sea level: 166.00 ft.

Basic wind speed of 116 mph.

Risk Category II.

Exposure Category C.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0.00 ft.
Nominal ice thickness of 1.5000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used. Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Tower analysis based on target reliabilities in accordance with Annex S.

Load Modification Factors used: $K_{es}(F_w) = 0.95$, $K_{es}(t_i) = 0.85$.

Maximum demand-capacity ratio is: 1.05.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs
Consider Moments - Horizontals
Consider Moments - Diagonals
Use Moment Magnification

√ Use Code Stress Ratios

 ✓ Use Code Safety Factors - Guys Escalate Ice

Always Use Max Kz Use Special Wind Profile

Include Bolts In Member Capacity

Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
 √ Use Clear Spans For Wind Area
 Use Clear Spans For KL/r
 Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks

 ✓ Bypass Mast Stability C
- √ Use Azimuth Dish Coefficients
 - Project Wind Area of Appurt.

Autocalc Torque Arm Areas

Add IBC .6D+W Combination Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

 ✓ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption

Poles

- √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
- √ Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	168.00-119.25	48.75	4.25	18	24.0000	34.2880	0.2500	1.0000	A607-65 (65 ksi)
L2	119.25-78.50	45.00	5.25	18	32.8911	42.3870	0.2813	1.1250	À607-65 (65 ksi)

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	fť	fť	Sides	in	in	in	in	
L3	78.50-38.75	45.00	6.25	18	40.7166	50.2130	0.3750	1.5000	A607-65 (65 ksi)
L4	38.75-0.00	45.00		18	48.1441	57.6400	0.3750	1.5000	À607-65 (65 ksi)

				Tape	red Pol	e Prop	erties				
Section	Tip Dia	. Area	I in⁴	r in	C	I/C in³	J in⁴	It/Q in²	w	w/t	_
L1	24.3317 34.778 ⁴				12.1920 17.4183	110.1540 226.9711	2687.7623 7912.1063	9.4246 13.5071	3.7840 5.5947		
L2	34.2658 42.9975	3 29.1104	3910.9583	11.5765	16.7087 21.5326	234.0675 390.9892	7827.0628 16849.101	14.5580 18.7972	5.2938 6.9651	18.823	3
L3	42.4119	48.0166	9872.7116	3 14.3213	20.6841	477.3102	9 19758.414 0	24.0129	6.5061	1 17.35	
	50.9298	59.3197	7 18614.760 7	17.6925	25.5082	729.7558	37254.015 2	29.6655	8.1775	5 21.807	7
L4	50.1681	56.8571	16391.389 9	16.9580	24.4572	670.2076	32804.348 0	28.4340	7.8134	4 20.836	5
	58.4713	8 68.1597	28238.617 8	7 20.3291	29.2811	964.3968	56514.392 7	34.0863	9.4846	5 25.292	<u></u>
Tow	rer	Gusset	Gusset G	usset Grade	Adjust. Factor	Adjust.	Weight M	lult. Double	Angle Do	ouble Angle	Double Angle
Eleva		Area T per face)	hickness		A_f	Factor A,	J	Stitch Spa	n Bolt S cing	Stitch Bolt Spacing Horizontals	Stitch Bolt Spacing Redundants
ft		ft ²	in					Ŭ.	n .	in	in
L1 168.00					1	1	1				
L2 119.2					1	1	1				
L3 78.50	-38.75				7	1	1				

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Exclude From	Componen t	Placement	Total Number	Number Per Row	Start/En d	Width or Diamete	Perimete r	Weight
		Torque	Type	ft			Position	r		plf
		Calculation	1					in	in	

Safety Line 3/8"	С	No	Surface Ar (CaAa)	168.00 - 8.00	1	1	0.000	0.3750		0.22
Climbing Pegs	С	No	Surface Ar (CaAa)	168.00 - 8.00	1	1	-0.050 0.050	0.7050		1.80
***			(53714)	2.00			3.300			

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Exclude From	Componen t	Placement	Total Number		$C_A A_A$	Weight
	Leg		Torque Calculation	Туре	ft			ft²/ft	plf

LDF6-50A(1-1/4)	Α	No	No	Inside Pole	168.00 - 3.00	1	No Ice	0.00	0.60
							1/2" Ice 1" Ice	0.00 0.00	0.60 0.60
***							2" Ice	0.00	0.60
LDF7-50A(1-5/8)	С	No	No	Inside Pole	168.00 - 3.00	12	No Ice 1/2" Ice	0.00 0.00	0.82 0.82

L4 38.75-0.00

Description		Allow Shield	Exclude	Componen	Placement	Total Number		$C_A A_A$	Weight
	or Leg	Sriieiu	From Torque Calculation	t Type	ft	Number		ft²/ft	plf
							1" Ice	0.00	0.82
							2" Ice	0.00	0.82
FB-L98B-034-	С	No	No	Inside Pole	168.00 - 3.00	2	No Ice	0.00	0.05
XXXXXX(3/8)							1/2" Ice	0.00	0.05
()							1" Ice	0.00	0.05
							2" Ice	0.00	0.05
WR-VG86ST-	С	No	No	Inside Pole	168.00 - 3.00	6	No Ice	0.00	0.58
BRD(3/4)	•					· ·	1/2" Ice	0.00	0.58
2.12(0,1)							1" Ice	0.00	0.58
							2" lce	0.00	0.58
***							2 100	0.00	0.00
EU 90-	Α	No	No	Inside Pole	152.00 - 3.00	1	No Ice	0.00	0.34
FR(ELLIPTICAL)	,,	110	110	moido i olo	102.00 0.00	•	1/2" Ice	0.00	0.34
TIN(LLLII TIOAL)							1" lce	0.00	0.34
							2" Ice	0.00	0.34
***							2 100	0.00	0.54
LDF4-50A(1/2)	Α	No	No	Incido Polo	147.00 - 3.00	1	No Ice	0.00	0.15
LDI 4-30A(1/2)	^	NO	NO	Iliside Fole	147.00 - 3.00	'	1/2" Ice	0.00	0.15
							1" Ice		0.15
								0.00	
E11.00	•				447.00 0.00		2" Ice	0.00	0.15
EU 90-	Α	No	No	Inside Pole	147.00 - 3.00	1	No Ice	0.00	0.34
FR(ELLIPTICAL)							1/2" Ice	0.00	0.34
							1" Ice	0.00	0.34
							2" Ice	0.00	0.34

LDF1-50A(1/4)	Α	No	No	Inside Pole	143.00 - 3.00	1	No Ice	0.00	0.06
							1/2" Ice	0.00	0.06
							1" Ice	0.00	0.06
							2" Ice	0.00	0.06

CAT5E(1/4)	Α	No	No	Inside Pole	140.00 - 3.00	1	No Ice	0.00	0.04
							1/2" Ice	0.00	0.04
							1" Ice	0.00	0.04
							2" Ice	0.00	0.04
LDF5-50A(7/8)	Α	No	No	Inside Pole	140.00 - 3.00	2	No Ice	0.00	0.33
, ,							1/2" Ice	0.00	0.33
							1" Ice	0.00	0.33
							2" Ice	0.00	0.33

LDF4-50A(1/2)	Α	No	No	Inside Pole	135.00 - 3.00	2	No Ice	0.00	0.15
,							1/2" Ice	0.00	0.15
							1" Ice	0.00	0.15
							2" Ice	0.00	0.15

HB158-21U6S24-	В	No	No	Inside Pole	130.00 - 3.00	3	No Ice	0.00	2.50
xxM TMO(1-5/8)							1/2" Ice	0.00	2.50
(. 5.5)							1" Ice	0.00	2.50
							2" Ice	0.00	2.50

CU12PSM9P6XXX	С	No	No	Inside Pole	120.00 - 0.00	1	No Ice	0.00	2.35
(1-1/2)	_					-	1/2" Ice	0.00	2.35
(1.172)							1" Ice	0.00	2.35
							2" Ice	0.00	2.35
***							2 100	0.00	2.00
EU 90-	Α	No	No	Inside Pole	100.00 - 3.00	1	No Ice	0.00	0.34
FR(ELLIPTICAL)		140	140	mode i oic	100.00 - 0.00	•	1/2" Ice	0.00	0.34
IN(LLLII HOAL)							1" Ice	0.00	0.34
***							2" Ice	0.00	0.34
LDF4-50A(1/2)	В	No	No	Inside Pole	52.00 - 3.00	1	No Ice	0.00	0.15
LDF4-30A(1/2)	ם	INO	INO	molue Fule	32.00 - 3.00	ı			
							1/2" Ice	0.00	0.15
							1" Ice	0.00	0.15
***							2" Ice	0.00	0.15

Feed Line/Linear Appurtenances Section Areas

Tower Sectio	Tower Elevation	Face	A_R	A_F	C _A A _A In Face	C _A A _A Out Face	Weight
n	ft		ft ²	ft ²	ft²	ft ²	Κ
L1	168.00-119.25	Α	0.000	0.000	0.000	0.000	0.07
		В	0.000	0.000	0.000	0.000	0.08
		С	0.000	0.000	5.265	0.000	0.76
L2	119.25-78.50	Α	0.000	0.000	0.000	0.000	0.11
		В	0.000	0.000	0.000	0.000	0.31
		С	0.000	0.000	4.401	0.000	0.73
L3	78.50-38.75	Α	0.000	0.000	0.000	0.000	0.11
		В	0.000	0.000	0.000	0.000	0.30
		С	0.000	0.000	4.293	0.000	0.71
L4	38.75-0.00	Α	0.000	0.000	0.000	0.000	0.10
		В	0.000	0.000	0.000	0.000	0.27
		С	0.000	0.000	3.321	0.000	0.63

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Sectio	Tower Elevation	Face or	Ice Thickness	A_R	A_F	C _A A _A In Face	C _A A _A Out Face	Weight
วะเเบ n	Elevali011 ft	Leg	in	ft²	ft ²	ft ²	ft ²	K
L1	168.00-119.25	Α	1.476	0.000	0.000	0.000	0.000	0.07
		В		0.000	0.000	0.000	0.000	0.08
		С		0.000	0.000	34.042	0.000	1.11
L2	119.25-78.50	Α	1.422	0.000	0.000	0.000	0.000	0.11
		В		0.000	0.000	0.000	0.000	0.31
		С		0.000	0.000	28.456	0.000	1.02
L3	78.50-38.75	Α	1.350	0.000	0.000	0.000	0.000	0.11
		В		0.000	0.000	0.000	0.000	0.30
		С		0.000	0.000	26.904	0.000	0.98
L4	38.75-0.00	Α	1.210	0.000	0.000	0.000	0.000	0.10
		В		0.000	0.000	0.000	0.000	0.27
		С		0.000	0.000	19.924	0.000	0.83

Feed Line Center of Pressure

Section	Elevation	CP _X	CPz	CP _X Ice	CP _z Ice
	ft	in	in	in	in
L1	168.00-119.25	0.0000	0.9675	0.0000	2.8186
L2	119.25-78.50	0.0000	0.9777	0.0000	2.9975
L3	78.50-38.75	0.0000	0.9838	0.0000	3.0291
L4	38.75-0.00	0.0000	0.7775	0.0000	2.3955

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L1	2	Safety Line 3/8"	119.25 - 168.00	1.0000	1.0000
L1	3	Climbing Pegs	119.25 - 168.00	1.0000	1.0000
L2	2	Safety Line 3/8"	78.50 - 119.25	1.0000	1.0000
L2	3	Climbing Pegs	78.50 - 119.25	1.0000	1.0000

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L3	2	Safety Line 3/8"	38.75 - 78.50	1.0000	1.0000
L3	3	Climbing Pegs	38.75 - 78.50	1.0000	1.0000
L4 L4	2 3	Safety Line 3/8" Climbing Pegs			

			Disc	rete Tov	ver Loa	ds			
Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	۰	ft		ft²	ft²	K
800 10121 w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	168.00	No Ice 1/2" Ice 1" Ice 2" Ice	3.60 4.00 4.42 5.29	2.95 3.34 3.74 4.59	0.07 0.11 0.17 0.30
800 10121 w/ Mount Pipe	В	From Leg	4.00 0.00 0.00	0.0000	168.00	No Ice 1/2" Ice 1" Ice	3.60 4.00 4.42 5.29	2.95 3.34 3.74 4.59	0.07 0.11 0.17 0.30
800 10121 w/ Mount Pipe	С	From Leg	4.00 0.00 0.00	0.0000	168.00	2" Ice No Ice 1/2" Ice 1" Ice	3.60 4.00 4.42 5.29	2.95 3.34 3.74 4.59	0.07 0.11 0.17 0.30
80010965 w/ Mount Pipe	Α	From Leg	4.00 0.00 0.00	0.0000	168.00	2" Ice No Ice 1/2" Ice 1" Ice	12.26 13.03 13.80 15.41	5.79 6.47 7.17 8.60	0.14 0.23 0.33 0.57
80010965 w/ Mount Pipe	В	From Leg	4.00 0.00 0.00	0.0000	168.00	2" Ice No Ice 1/2" Ice 1" Ice	12.26 13.03 13.80 15.41	5.79 6.47 7.17 8.60	0.14 0.23 0.33 0.57
80010966 w/ Mount Pipe	С	From Leg	4.00 0.00 0.00	0.0000	168.00	2" Ice No Ice 1/2" Ice 1" Ice	14.61 15.47 16.35 18.14	6.84 7.63 8.42 10.06	0.16 0.27 0.39 0.68
QS66512-2 w/ Mount Pipe	Α	From Leg	4.00 0.00 0.00	0.0000	168.00	2" Ice No Ice 1/2" Ice 1" Ice	4.04 4.42 4.82 5.63	4.18 4.57 4.97 5.79	0.14 0.21 0.29 0.48
QS66512-2 w/ Mount Pipe	В	From Leg	4.00 0.00 0.00	0.0000	168.00	2" Ice No Ice 1/2" Ice 1" Ice	4.04 4.42 4.82 5.63	4.18 4.57 4.97 5.79	0.14 0.21 0.29 0.48
TPA-65R-LCUUUU-H8 w/ Mount Pipe	С	From Leg	4.00 0.00 0.00	0.0000	168.00	2" Ice No Ice 1/2" Ice 1" Ice	11.85 12.77 13.71 15.64	8.99 9.88 10.79 12.66	0.11 0.21 0.32 0.58
CC807-08	С	From Leg	4.00 0.00 2.00	0.0000	168.00	2" Ice No Ice 1/2" Ice 1" Ice 2" Ice	2.85 3.83 4.67 5.85	2.85 3.83 4.67 5.85	0.03 0.05 0.07 0.15

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft		ft		ft²	ft²	κ
			ft ft	۰					
RRUS 4478 B14	Α	From Leg	4.00	0.0000	168.00	No Ice	1.84	1.06	0.06
			0.00			1/2"	2.01	1.20	0.08
			0.00			Ice 1" Ice	2.19 2.57	1.34 1.66	0.09 0.14
						2" Ice			
RRUS 4478 B14	В	From Leg	4.00	0.0000	168.00	No Ice	1.84	1.06	0.06
			0.00			1/2"	2.01	1.20	0.08
			0.00			Ice 1" Ice 2" Ice	2.19 2.57	1.34 1.66	0.09 0.14
RRUS 4478 B14	С	From Leg	4.00	0.0000	168.00	No Ice	1.84	1.06	0.06
	•		0.00	0.000		1/2"	2.01	1.20	0.08
			0.00			lce	2.19	1.34	0.09
						1" Ice 2" Ice	2.57	1.66	0.14
RRUS 32 B66	Α	From Leg	4.00	0.0000	168.00	No Ice	2.74	1.67	0.05
		3	0.00			1/2"	2.96	1.86	0.07
			0.00			Ice	3.19	2.05	0.10
						1" Ice	3.68	2.46	0.16
						2" Ice			
RRUS 32 B66	В	From Leg	4.00	0.0000	168.00	No Ice	2.74	1.67	0.05
			0.00			1/2"	2.96	1.86	0.07
			0.00			Ice	3.19	2.05	0.10
						1" Ice 2" Ice	3.68	2.46	0.16
RRUS 32 B66	С	From Leg	4.00	0.0000	168.00	No Ice	2.74	1.67	0.05
			0.00			1/2"	2.96	1.86	0.07
			0.00			Ice	3.19	2.05	0.10
						1" Ice 2" Ice	3.68	2.46	0.16
RRUS 32 B30	Α	From Leg	4.00	0.0000	168.00	No Ice	2.69	1.57	0.06
			0.00			1/2"	2.91	1.76	0.08
			0.00			Ice	3.14	1.95	0.10
						1" Ice 2" Ice	3.61	2.35	0.16
RRUS 32 B30	В	From Leg	4.00	0.0000	168.00	No Ice	2.69	1.57	0.06
		•	0.00			1/2"	2.91	1.76	0.08
			0.00			Ice	3.14	1.95	0.10
						1" Ice	3.61	2.35	0.16
	_					2" Ice			
RRUS 32 B30	С	From Leg	4.00	0.0000	168.00	No Ice	2.69	1.57	0.06
			0.00 0.00			1/2"	2.91	1.76	0.08
			0.00			Ice 1" Ice	3.14 3.61	1.95 2.35	0.10 0.16
						2" Ice			
(2) 860 10025	Α	From Leg	4.00	0.0000	168.00	No Ice	0.14	0.12	0.00
			0.00			1/2"	0.20	0.17	0.00
			0.00			Ice	0.26	0.23	0.01
						1" Ice 2" Ice	0.41	0.38	0.01
(2) 860 10025	В	From Leg	4.00	0.0000	168.00	No Ice	0.14	0.12	0.00
		-	0.00			1/2"	0.20	0.17	0.00
			0.00			Ice	0.26	0.23	0.01
						1" Ice 2" Ice	0.41	0.38	0.01
(2) 860 10025	С	From Leg	4.00	0.0000	168.00	No Ice	0.14	0.12	0.00
,_,	-		0.00	2.0000	. 55.56	1/2"	0.20	0.17	0.00
			0.00			Ice	0.26	0.23	0.01
						1" Ice	0.41	0.38	0.01
						2" Ice			••
TMABP7819VG12A	Α	From Leg	4.00	0.0000	168.00	No Ice	0.98	0.34	0.02
		3	0.00			1/2"	1.10	0.42	0.03
			0.00			Ice	1.23	0.51	0.04
						1" Ice	1.52	0.71	0.06
						2" Ice			

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft ft ft	۰	ft		ft²	ft²	Κ
DTMABP7819VG12A	В	From Leg	4.00	0.0000	168.00	No Ice	0.98	0.34	0.02
			0.00			1/2"	1.10	0.42	0.03
			0.00			Ice 1" Ice 2" Ice	1.23 1.52	0.51 0.71	0.04 0.06
DTMABP7819VG12A	С	From Leg	4.00	0.0000	168.00	No Ice	0.98	0.34	0.02
		_	0.00			1/2"	1.10	0.42	0.03
			0.00			Ice 1" Ice 2" Ice	1.23 1.52	0.51 0.71	0.04 0.06
DC6-48-60-0-8F	Α	From Leg	4.00	0.0000	168.00	No Ice	0.92	0.92	0.02
		3	0.00			1/2"	1.46	1.46	0.04
			0.00			Ice	1.64	1.64	0.06
(0) DOG 40 00 40 05	•	F	4.00	0.0000	400.00	1" Ice 2" Ice	2.04	2.04	0.11
(2) DC6-48-60-18-8F	Α	From Leg	4.00 0.00	0.0000	168.00	No Ice 1/2"	0.92 1.46	0.92 1.46	0.02 0.04
			0.00			Ice	1.64	1.64	0.04
			0.00			1" Ice 2" Ice	2.04	2.04	0.11
6' x 2" Mount Pipe	С	From Leg	1.00	0.0000	168.00	No Ice	1.43	1.43	0.02
			0.00			1/2"	1.92	1.92	0.03
			0.00			Ice 1" Ice 2" Ice	2.29 3.06	2.29 3.06	0.05 0.09
Platform Mount [LP 1201-	С	None		0.0000	168.00	No Ice	37.61	37.61	2.63
1_KCKR-HR-1]						1/2"	45.62	45.62	3.48
***						Ice 1" Ice 2" Ice	53.59 69.65	53.59 69.65	4.46 6.85
DMP65R-BU6D w/ Mount	Α	From Leg	4.00	0.0000	168.00	No Ice	11.96	5.97	0.11
Pipe		1 Tolli Log	0.00	0.0000	100.00	1/2"	12.70	6.63	0.20
•			0.00			Ice	13.46	7.30	0.30
						1" Ice	15.02	8.69	0.53
DMP65R-BU6D w/ Mount	В	From Leg	4.00	0.0000	168.00	2" Ice No Ice	11.96	5.97	0.11
Pipe		1 Tolli Log	0.00	0.0000	100.00	1/2"	12.70	6.63	0.20
ps			0.00			Ice	13.46	7.30	0.30
						1" Ice 2" Ice	15.02	8.69	0.53
DMP65R-BU8D w/ Mount	С	From Leg	4.00	0.0000	168.00	No Ice	15.89	7.89	0.14
Pipe			0.00 0.00			1/2" Ice	16.81 17.76	8.74 9.60	0.25 0.38
			0.00			1" Ice 2" Ice	19.70	11.37	0.68
RRUS 4449 B5/B12	Α	From Leg	4.00	0.0000	168.00	No Ice	1.97	1.41	0.07
		_	0.00			1/2"	2.14	1.56	0.09
			0.00			Ice	2.33	1.73	0.11
						1" Ice 2" Ice	2.72	2.07	0.16
RRUS 4449 B5/B12	В	From Leg	4.00	0.0000	168.00	No Ice	1.97	1.41	0.07
11.100 11.10 20,212		1 10m 20g	0.00	0.0000	100.00	1/2"	2.14	1.56	0.09
			0.00			Ice	2.33	1.73	0.11
						1" Ice 2" Ice	2.72	2.07	0.16
RRUS 4449 B5/B12	С	From Leg	4.00	0.0000	168.00	No Ice	1.97	1.41	0.07
			0.00			1/2"	2.14	1.56	0.09
			0.00			Ice 1" Ice	2.33	1.73	0.11
RRUS 4415 B25	Α	From Leg	4.00	0.0000	168.00	2" Ice No Ice	2.72 1.64	2.07 0.68	0.16 0.04
INNO 44 10 DZ0	A	rioni Leg	0.00	0.0000	100.00	1/2"	1.64	0.68	0.04
			0.00			Ice	1.97	0.73	0.07
						1" Ice	2.33	1.18	0.11

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	۰	ft		ft²	ft²	K
RRUS 4415 B25	В	From Leg	4.00 0.00 0.00	0.0000	168.00	2" Ice No Ice 1/2" Ice 1" Ice 2" Ice	1.64 1.80 1.97 2.33	0.68 0.79 0.91 1.18	0.04 0.06 0.07 0.11
RRUS 4415 B25	С	From Leg	4.00 0.00 0.00	0.0000	168.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.64 1.80 1.97 2.33	0.68 0.79 0.91 1.18	0.04 0.06 0.07 0.11
*** Pipe Mount [PM 601-1] ***	Α	From Leg	0.50 0.00 0.00	0.0000	152.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.32 1.58 1.84 2.40	1.32 1.58 1.84 2.40	0.07 0.08 0.09 0.13
BPA7496-180-11 w/ Mount Pipe	Α	From Leg	1.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	6.07 6.53 6.99 7.91	5.17 6.05 6.81 8.37	0.04 0.09 0.15 0.29
782 10876	Α	From Leg	1.00 0.00 1.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.59 0.69 0.80 1.04	0.23 0.31 0.39 0.57	0.01 0.01 0.02 0.04
Pipe Mount [PM 601-1]	В	From Leg	0.50 0.00 0.00	0.0000	147.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.32 1.58 1.84 2.40	1.32 1.58 1.84 2.40	0.07 0.08 0.09 0.13
782 10876	С	From Leg	1.00 0.00 0.00	0.0000	143.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.59 0.69 0.80 1.04	0.23 0.31 0.39 0.57	0.01 0.01 0.02 0.04
Pipe Mount [PM 602-1]	С	From Leg	0.50 0.00 0.00	0.0000	143.00	No Ice 1/2" Ice 1" Ice 2" Ice	2.78 3.21 3.64 4.54	2.78 3.21 3.64 4.54	0.09 0.11 0.14 0.21
(2) CC807-11	Α	From Leg	4.00 0.00 8.00	0.0000	140.00	No Ice 1/2" Ice 1" Ice 2" Ice	5.27 7.04 8.83 12.45	5.27 7.04 8.83 12.45	0.05 0.09 0.14 0.27
ANT450F6	В	From Leg	4.00 0.00 8.00	0.0000	140.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.86 2.67 3.30 4.28	1.86 2.67 3.30 4.28	0.02 0.04 0.05 0.11
PTP400 w/Mount Pipe	В	From Leg	4.00 0.00 0.00	0.0000	140.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00	0.00 0.00 0.00 0.00
432E-83I-01-T	Α	From Leg	4.00 0.00 8.00	0.0000	140.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.42 1.57 1.73 2.06	0.87 0.99 1.12 1.41	0.03 0.04 0.05 0.09

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		$C_A A_A$ Front	C_AA_A Side	Weight
			ft ft ft	0	ft		ft²	ft²	K
RIU	В	From Leg	4.00	0.0000	140.00	No Ice	0.16	0.12	0.00
			0.00 0.00			1/2" Ice	0.21 0.27	0.16 0.22	0.00 0.01
			0.00			1" Ice 2" Ice	0.42	0.35	0.01
Side Arm Mount [SO 306-	Α	From Leg	2.00	0.0000	140.00	No Ice	0.41	2.26	0.04
1]			0.00			1/2"	0.81	3.83	0.06
			0.00			Ice 1" Ice 2" Ice	1.23 2.08	5.48 9.37	0.09 0.19
Side Arm Mount [SO 306-	В	From Leg	2.00	0.0000	140.00	No Ice	0.41	2.26	0.04
1]			0.00			1/2"	0.81	3.83	0.06
			0.00			Ice 1" Ice	1.23 2.08	5.48 9.37	0.09 0.19
***						2" lce	2.00	0.01	0.15
ANT450F6	Α	From Leg	6.00	0.0000	135.00	No Ice	1.86	1.86	0.02
			0.00			1/2"	2.67	2.67	0.04
			9.00			Ice 1" Ice 2" Ice	3.30 4.28	3.30 4.28	0.05 0.11
ANT450F6	В	From Leg	6.00	0.0000	135.00	No Ice	1.86	1.86	0.02
			0.00 9.00			1/2" Ice	2.67 3.30	2.67 3.30	0.04 0.05
			9.00			1" Ice	4.28	4.28	0.03
						2" Ice	0	0	• • • • • • • • • • • • • • • • • • • •
4' x 2" Pipe Mount	Α	From Leg	6.00	0.0000	135.00	No Ice	0.79	0.79	0.03
			0.00 0.00			1/2" Ice	1.03 1.28	1.03 1.28	0.04 0.04
			0.00			1" Ice 2" Ice	1.81	1.81	0.07
4' x 2" Pipe Mount	В	From Leg	6.00	0.0000	135.00	No Ice	0.79	0.79	0.03
			0.00			1/2"	1.03	1.03	0.04
			0.00			lce 1" lce	1.28 1.81	1.28 1.81	0.04 0.07
						2" Ice	1.01	1.01	0.07
Side Arm Mount [SO 702-	Α	From Leg	3.00	0.0000	135.00	No Ice	0.62	1.49	0.03
1]			0.00 0.00			1/2" Ice	0.74 0.89	2.07 2.54	0.04 0.06
			0.00			1" Ice	1.25	3.55	0.00
						2" Ice			
Side Arm Mount [SO 702-	В	From Leg	3.00	0.0000	135.00	No Ice	0.62	1.49	0.03
1]			0.00 0.00			1/2" Ice	0.74 0.89	2.07 2.54	0.04 0.06
			0.00			1" Ice	1.25	3.55	0.12
***						2" Ice			
Platform Mount [LP 1201-	С	None		0.0000	130.00	No Ice	18.38	18.38	2.10
1]	Ü	110110		0.0000	100.00	1/2"	22.11	22.11	2.65
·						Ice	25.87	25.87	3.26
***						1" Ice 2" Ice	33.47	33.47	4.66
APXVAALL24_43-U-	Α	From Leg	4.00	0.0000	130.00	No Ice	14.67	5.32	0.15
NA20_TMO			0.00			1/2"	15.43	5.99	0.26
			1.00			Ice	16.21	6.68	0.38
						1" Ice 2" Ice	17.81	8.08	0.65
APXVAALL24_43-U-	В	From Leg	4.00	0.0000	130.00	No Ice	14.67	5.32	0.15
NA20_TMO		J	0.00			1/2"	15.43	5.99	0.26
			1.00			Ice	16.21	6.68	0.38
						1" Ice 2" Ice	17.81	8.08	0.65
APXVAALL24_43-U-	С	From Leg	4.00	0.0000	130.00	No Ice	14.67	5.32	0.15
NA20_TMO			0.00			1/2"	15.43	5.99	0.26

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft ft ft	o	ft		ft²	ft²	K
			1.00			Ice 1" Ice	16.21 17.81	6.68 8.08	0.38 0.65
AIDG440 D44 T MODILE	^	From Log	4.00	0.0000	120.00	2" Ice	E 07	2.02	0.11
AIR6449 B41_T-MOBILE	Α	From Leg	4.00 0.00	0.0000	130.00	No Ice 1/2"	5.27 5.70	2.03 2.36	0.11 0.15
			1.00			Ice	6.14	2.70	0.10
			1.00			1" Ice	7.06	3.43	0.30
AIDEAAO BAA T MODU E	В	From Log	4.00	0.0000	120.00	2" Ice	E 07	2.02	0.11
AIR6449 B41_T-MOBILE	В	From Leg	4.00 0.00	0.0000	130.00	No Ice 1/2"	5.27 5.70	2.03 2.36	0.11 0.15
			1.00			Ice	6.14	2.70	0.13
			1.00			1" Ice	7.06	3.43	0.30
						2" lce	7.00	0.10	0.00
AIR6449 B41_T-MOBILE	С	From Leg	4.00	0.0000	130.00	No Ice	5.27	2.03	0.11
_		_	0.00			1/2"	5.70	2.36	0.15
			1.00			Ice	6.14	2.70	0.20
						1" Ice 2" Ice	7.06	3.43	0.30
RADIO 4460 B2/B25	Α	From Leg	4.00	0.0000	130.00	No Ice	2.14	1.69	0.11
B66_TMO			0.00			1/2"	2.32	1.85	0.13
			1.00			Ice	2.51	2.02	0.16
						1" Ice	2.91	2.39	0.22
DADIO 4400 DO/DOC	_	Гиана I ан	4.00	0.0000	420.00	2" Ice	0.44	4.00	0.44
RADIO 4460 B2/B25	В	From Leg	4.00	0.0000	130.00	No Ice	2.14	1.69	0.11
B66_TMO			0.00 1.00			1/2" Ice	2.32 2.51	1.85 2.02	0.13 0.16
			1.00			1" Ice	2.91	2.39	0.10
						2" Ice	2.01	2.00	0.22
RADIO 4460 B2/B25	С	From Leg	4.00	0.0000	130.00	No Ice	2.14	1.69	0.11
B66_TMO		J	0.00			1/2"	2.32	1.85	0.13
			1.00			Ice	2.51	2.02	0.16
						1" Ice	2.91	2.39	0.22
D II 4400 TMOVO			4.00	0.0000	400.00	2" Ice	0.00	4.40	0.00
Radio 4480_TMOV2	Α	From Leg	4.00	0.0000	130.00	No Ice 1/2"	2.88	1.40	0.08 0.10
			0.00 1.00			lce	3.09 3.31	1.56 1.73	0.10
			1.00			1" Ice	3.78	2.09	0.13
						2" lce	0.70	2.00	0.10
Radio 4480 TMOV2	В	From Leg	4.00	0.0000	130.00	No Ice	2.88	1.40	0.08
_			0.00			1/2"	3.09	1.56	0.10
			1.00			Ice	3.31	1.73	0.13
						1" Ice	3.78	2.09	0.19
Dadia 4490 TMOV2	0	From Log	4.00	0.0000	120.00	2" Ice	2.00	1 10	0.00
Radio 4480_TMOV2	С	From Leg	4.00 0.00	0.0000	130.00	No Ice 1/2"	2.88 3.09	1.40 1.56	0.08 0.10
			1.00			Ice	3.31	1.73	0.10
			1.00			1" Ice	3.78	2.09	0.19
						2" Ice			
6' x 2" Mount Pipe	Α	From Leg	4.00	0.0000	130.00	No Ice	1.43	1.43	0.02
			0.00			1/2"	1.92	1.92	0.03
			0.00			Ice	2.29	2.29	0.05
						1" Ice 2" Ice	3.06	3.06	0.09
6' x 2" Mount Pipe	В	From Leg	4.00	0.0000	130.00	No Ice	1.43	1.43	0.02
0 X 2 Mount i ipe	ь	1 Tolli Leg	0.00	0.0000	130.00	1/2"	1.92	1.92	0.02
			0.00			Ice	2.29	2.29	0.05
						1" Ice	3.06	3.06	0.09
6' x 2" Mount Pipe	С	From Leg	4.00	0.0000	130.00	2" Ice	1.43	1.43	0.02
0 X 2 WOULD PIPE	C	rioni Leg	0.00	0.0000	130.00	No Ice 1/2"	1.43	1.43	0.02
			0.00			Ice	2.29	2.29	0.05
			0.00			1" Ice	3.06	3.06	0.09
						2" Ice			
Support rail kit[#HRK14-U]	С	None		0.0000	130.00	No Ice	6.36	6.36	0.26
						1/2"	8.52	8.52	0.30

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	۰	ft		ft ²	ft²	K
						Ice 1" Ice 2" Ice	10.62 14.64	10.62 14.64	0.46 0.77
Support rail kit[#HRK14-U]	С	None		0.0000	130.00	No Ice 1/2" Ice 1" Ice	6.36 8.52 10.62 14.64	6.36 8.52 10.62 14.64	0.26 0.30 0.46 0.77
V-style reinforcement kit[PRK-SFS]	С	None		0.0000	130.00	2" Ice No Ice 1/2" Ice 1" Ice 2" Ice	11.84 16.96 22.08 32.32	11.84 16.96 22.08 32.32	0.28 0.64 0.32 0.36
*** ***									
MX08FRO665-21 w/ Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice	8.01 8.52 9.04 10.11	4.23 4.69 5.16 6.12	0.11 0.19 0.29 0.52
MX08FRO665-21 w/ Mount Pipe	В	From Leg	4.00 0.00 0.00	0.0000	120.00	No Ice 1/2" Ice 1" Ice	8.01 8.52 9.04 10.11	4.23 4.69 5.16 6.12	0.11 0.19 0.29 0.52
MX08FRO665-21 w/ Mount Pipe	С	From Leg	4.00 0.00 0.00	0.0000	120.00	2" Ice No Ice 1/2" Ice 1" Ice	8.01 8.52 9.04 10.11	4.23 4.69 5.16 6.12	0.11 0.19 0.29 0.52
TA08025-B604	Α	From Leg	4.00 0.00 0.00	0.0000	120.00	2" Ice No Ice 1/2" Ice 1" Ice	1.96 2.14 2.32 2.71	0.98 1.11 1.25 1.55	0.06 0.08 0.10 0.15
TA08025-B604	В	From Leg	4.00 0.00 0.00	0.0000	120.00	2" Ice No Ice 1/2" Ice 1" Ice	1.96 2.14 2.32 2.71	0.98 1.11 1.25 1.55	0.06 0.08 0.10 0.15
TA08025-B604	С	From Leg	4.00 0.00 0.00	0.0000	120.00	2" Ice No Ice 1/2" Ice 1" Ice	1.96 2.14 2.32 2.71	0.98 1.11 1.25 1.55	0.06 0.08 0.10 0.15
TA08025-B605	Α	From Leg	4.00 0.00 0.00	0.0000	120.00	2" Ice No Ice 1/2" Ice 1" Ice	1.96 2.14 2.32 2.71	1.13 1.27 1.41 1.72	0.08 0.09 0.11 0.16
TA08025-B605	В	From Leg	4.00 0.00 0.00	0.0000	120.00	2" Ice No Ice 1/2" Ice 1" Ice	1.96 2.14 2.32 2.71	1.13 1.27 1.41 1.72	0.08 0.09 0.11 0.16
TA08025-B605	С	From Leg	4.00 0.00 0.00	0.0000	120.00	2" Ice No Ice 1/2" Ice 1" Ice	1.96 2.14 2.32 2.71	1.13 1.27 1.41 1.72	0.08 0.09 0.11 0.16
RDIDC-9181-PF-48	Α	From Leg	4.00 0.00 0.00	0.0000	120.00	2" Ice No Ice 1/2" Ice 1" Ice 2" Ice	2.01 2.19 2.37 2.76	1.17 1.31 1.46 1.78	0.02 0.04 0.06 0.11

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	0	ft		ft²	ft²	K
(2) 8' x 2" Mount Pipe	Α	From Leg	4.00 0.00 0.00	0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.90 2.73 3.40 4.40	1.90 2.73 3.40 4.40	0.03 0.04 0.06 0.12
(2) 8' x 2" Mount Pipe	В	From Leg	4.00 0.00 0.00	0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.90 2.73 3.40 4.40	1.90 2.73 3.40 4.40	0.03 0.04 0.06 0.12
(2) 8' x 2" Mount Pipe	С	From Leg	4.00 0.00 0.00	0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.90 2.73 3.40 4.40	1.90 2.73 3.40 4.40	0.03 0.04 0.06 0.12
Commscope MC-PK8-DSH	С	None		0.0000	120.00	No Ice 1/2" Ice 1" Ice 2" Ice	34.24 62.95 91.66 149.08	34.24 62.95 91.66 149.08	1.75 2.10 2.45 3.15
Pipe Mount [PM 601-1]	В	From Leg	1.00 0.00 0.00	0.0000	100.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.32 1.58 1.84 2.40	1.32 1.58 1.84 2.40	0.07 0.08 0.09 0.13
1' x 2-1/2"	Α	From Leg	2.00 0.00 0.00	0.0000	79.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.16 0.23 0.31 0.51	0.16 0.23 0.31 0.51	0.03 0.03 0.03 0.04
1' x 2-1/2"	В	From Leg	2.00 0.00 0.00	0.0000	79.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.16 0.23 0.31 0.51	0.16 0.23 0.31 0.51	0.03 0.03 0.03 0.04
Side Arm Mount [SO 901- 1]	Α	From Leg	1.00 0.00 0.00	0.0000	79.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.33 0.46 0.62 1.01	0.62 0.78 0.97 1.43	0.11 0.11 0.12 0.15
Side Arm Mount [SO 901- 1]	В	From Leg	1.00 0.00 0.00	0.0000	79.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.33 0.46 0.62 1.01	0.62 0.78 0.97 1.43	0.11 0.11 0.12 0.15
GPS-TMG-HR-26NCM	С	From Leg	3.00 0.00 0.00	0.0000	52.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.13 0.18 0.24 0.37	0.13 0.18 0.24 0.37	0.00 0.00 0.01 0.01
2' x 2" Pipe Mount	С	From Leg	3.00 0.00 0.00	0.0000	52.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.02 0.05 0.09 0.19	0.02 0.05 0.09 0.19	0.01 0.01 0.01 0.01
Side Arm Mount [SO 701- 1]	С	From Leg	1.50 0.00 0.00	0.0000	52.00	No Ice 1/2" Ice 1" Ice 2" Ice	0.85 1.14 1.43 2.01	1.67 2.34 3.01 4.35	0.07 0.08 0.09 0.12

					Dish	es					
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				ft	۰	0	ft	ft		ft²	K
SC3-W100ASTX	Α	Paraboloid w/Shroud (HP)	From Leg	1.00 0.00 0.00	-32.0000		152.00	3.29	No Ice 1/2" Ice 1" Ice 2" Ice	8.51 8.95 9.38 10.26	0.04 0.09 0.13 0.22
***										.0.20	0.22
SC3-W100ASTX	В	Paraboloid w/Shroud (HP)	From Leg	1.00 0.00 1.00	57.0000		147.00	3.29	No Ice 1/2" Ice 1" Ice 2" Ice	8.51 8.95 9.38 10.26	0.04 0.09 0.13 0.22

MPRD2449	С	Paraboloid w/Radome	From Leg	1.00 0.00 0.00	10.0000		143.00	2.17	No Ice 1/2" Ice 1" Ice 2" Ice	3.69 3.98 4.27 4.84	0.04 0.06 0.08 0.12
***	_		_								
SC3-W100ASTX	В	Paraboloid w/Shroud (HP)	From Leg	1.00 0.00 0.00	37.0000		100.00	3.29	No Ice 1/2" Ice 1" Ice	8.51 8.95 9.38 10.26	0.04 0.09 0.13 0.22
***									2" Ice	10.20	0.22

Load Combinations

Comb.	Description
No.	·
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26 27	1.2 Dead+1.0 Ice+1.0 Temp
2 <i>1</i> 28	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
20 29	
30	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 100 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
00	2 Sada O Tima 2.10 dag 10 100 10 Tamp
	D 1 : 0.4.4.0

Comb.	Description
No.	
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Sectio	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
n	ft	Type		Load		Moment	Moment
No.				Comb.	K	kip-ft	kip-ft
L1	168 - 119.25	Pole	Max Tension	27	0.00	-0.00	-0.00
			Max. Compression	26	-34.06	-0.33	3.82
			Max. Mx	8	-15.78	-502.47	-1.75
			Max. My	2	-15.76	-2.68	504.73
			Max. Vy	8	18.04	-502.47	-1.75
			Max. Vx	2	-18.17	-2.68	504.73
			Max. Torque	22			-3.04
L2	119.25 - 78.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-50.61	-1.05	3.48
			Max. Mx	8	-26.37	-1407.08	-9.99
			Max. My	2	-26.34	-2.91	1418.44
			Max. Vý	8	24.81	-1407.08	-9.99
			Max. Vx	2	-25.17	-2.91	1418.44
			Max. Torque	22			-3.81
L3	78.5 - 38.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-65.30	-1.13	2.85
			Max. Mx	8	-37.78	-2435.09	-20.79
			Max. My	2	-37.76	-0.48	2459.93
			Max. Vy	8	28.14	-2435.09	-20.79
			Max. Vx	2	-28.51	-0.48	2459.93
			Max. Torque	22			-3.89
L4	38.75 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-84.11	-1.13	2.14
			Max. Mx	8	-52.99	-3772.37	-33.67
			Max. My	2	-52.99	3.02	3813.20
			Max. Vy	8	31.06	-3772.37	-33.67
			Max. Vx	2	-31.41	3.02	3813.20
			Max. Torque	22			-3.66

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	27	84.11	0.01	10.21
	Max. H _x	20	53.01	30.95	0.18
	Max. H _z	3	39.76	0.08	31.37
	Max. M _x	2	3813.20	0.08	31.37
	$Max. M_z$	8	3772.37	-31.02	-0.28
	Max. Torsion	10	3.05	-26.77	-15.84
	Min. Vert	7	39.76	-26.75	15.83

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Min. H _x	8	53.01	-31.02	-0.28
	Min. H _z	15	39.76	-0.11	-31.24
	Min. M _x	14	-3796.92	-0.11	-31.24
	Min. M_z	20	-3759.41	30.95	0.18
	Min. Torsion	22	-3.66	26.68	15.91

Tower Mast Reaction Summary

Load Combination	Vertical	Shear _x	Shear _z	Overturning Moment, M_x	Overturning Moment, M_z	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	44.17	0.00	0.00	-0.64	-0.65	0.00
1.2 Dead+1.0 Wind 0 deg -	53.01	-0.08	-31.37	-3813.20	3.02	1.21
No Ice						
0.9 Dead+1.0 Wind 0 deg -	39.76	-0.08	-31.37	-3761.77	3.26	1.20
No Ice	F2 04	45.00	07.00	2222.00	4000.07	0.40
1.2 Dead+1.0 Wind 30 deg - No Ice	53.01	15.29	-27.36	-3332.80	-1862.27	-0.40
0.9 Dead+1.0 Wind 30 deg -	39.76	15.29	-27.36	-3287.78	-1836.94	-0.39
No Ice	00.10	10.20	27.00	0201.10	1000.01	0.00
1.2 Dead+1.0 Wind 60 deg -	53.01	26.75	-15.83	-1936.85	-3252.40	-1.73
No Ice						
0.9 Dead+1.0 Wind 60 deg -	39.76	26.75	-15.83	-1910.53	-3208.44	-1.71
No Ice	=0.04	0.4.00				
1.2 Dead+1.0 Wind 90 deg -	53.01	31.02	0.28	33.67	-3772.37	-2.84
No Ice 0.9 Dead+1.0 Wind 90 deg -	39.76	31.02	0.28	33.47	-3721.42	-2.81
No Ice	33.70	31.02	0.20	33.47	-5721.42	-2.01
1.2 Dead+1.0 Wind 120 deg	53.01	26.77	15.84	1924.26	-3251.47	-3.05
- No Ice						
0.9 Dead+1.0 Wind 120 deg	39.76	26.77	15.84	1898.67	-3207.55	-3.02
- No Ice	50.04	45.45	07.07	2007.40	4070 54	0.00
1.2 Dead+1.0 Wind 150 deg - No Ice	53.01	15.45	27.07	3287.16	-1873.54	-2.30
0.9 Dead+1.0 Wind 150 deg	39.76	15.45	27.07	3243.25	-1848.18	-2.28
- No Ice	00.70	10.40	27.07	0240.20	10-10.10	2.20
1.2 Dead+1.0 Wind 180 deg	53.01	0.11	31.24	3796.92	-11.66	-1.19
- No Ice						
0.9 Dead+1.0 Wind 180 deg	39.76	0.11	31.24	3746.12	-11.34	-1.18
- No Ice	53.01	-15.21	27.20	2240.04	1046 70	0.12
1.2 Dead+1.0 Wind 210 deg - No Ice	55.01	-15.21	21.20	3310.81	1846.72	0.12
0.9 Dead+1.0 Wind 210 deg	39.76	-15.21	27.20	3266.53	1822.08	0.11
- No Ice	000		0	0200.00	.022.00	• • • • • • • • • • • • • • • • • • • •
1.2 Dead+1.0 Wind 240 deg	53.01	-26.78	15.72	1914.95	3255.94	1.74
- No Ice						
0.9 Dead+1.0 Wind 240 deg	39.76	- 26.78	15.72	1889.41	3212.34	1.72
- No Ice	E2 01	20.05	0.10	-16.70	2750 44	3.07
1.2 Dead+1.0 Wind 270 deg - No Ice	53.01	-30.95	-0.18	-10.70	3759.41	3.07
0.9 Dead+1.0 Wind 270 deg	39.76	-30.95	-0.18	-16.35	3709.08	3.04
- No Ice	00.10	00.00	0.10	10.00	0,00.00	0.01
1.2 Dead+1.0 Wind 300 deg	53.01	-26.68	-15.91	-1932.70	3233.00	3.66
- No Ice						
0.9 Dead+1.0 Wind 300 deg	39.76	-26.68	-15.91	-1906.58	3189.80	3.63
- No Ice	E2 01	15 10	27.10	2204 54	1064 44	2.60
1.2 Dead+1.0 Wind 330 deg - No Ice	53.01	-15.42	-27.18	-3301.54	1864.44	2.69
0.9 Dead+1.0 Wind 330 deg	39.76	-15.42	-27.18	-3257.00	1839.66	2.67
- No Ice	00.70	10.72	27.10	3201.00	1000.00	2.07
1.2 Dead+1.0 Ice+1.0 Temp	84.11	0.00	-0.00	-2.14	-1.13	-0.00
1.2 Dead+1.0 Wind 0	84.11	-0.01	-10.21	-1267.14	-2.09	0.42
deg+1.0 Ice+1.0 Temp						_
1.2 Dead+1.0 Wind 30	84.11	5.04	-8.88	-1105.49	-626.94	-0.23
deg+1.0 Ice+1.0 Temp						

Load	Vertical	Shear _x	Shearz	Overturning	Overturning	Torque
Combination	14		14	Moment, M _x	Moment, Mz	
105 11015 100	K	K	K	kip-ft	kip-ft	kip-ft
1.2 Dead+1.0 Wind 60	84.11	8.77	-5.14	-643.02	-1089.42	-0.78
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 90	84.11	10.15	0.05	3.78	-1260.59	-1.17
deg+1.0 lce+1.0 Temp	24.44		- 10	222.24	4007.70	
1.2 Dead+1.0 Wind 120	84.11	8.77	5.13	632.94	-1087.52	-1.22
deg+1.0 Ice+1.0 Temp	24.44			1000 10	200.40	
1.2 Dead+1.0 Wind 150	84.11	5.06	8.81	1089.13	-626.49	-0.91
deg+1.0 Ice+1.0 Temp	24.44		40.40	40-0-4-		
1.2 Dead+1.0 Wind 180	84.11	0.01	10.18	1259.17	-2.00	-0.41
deg+1.0 Ice+1.0 Temp	24.44			4000 4=	224.22	o 1=
1.2 Dead+1.0 Wind 210	84.11	-5.02	8.85	1096.17	621.28	0.17
deg+1.0 Ice+1.0 Temp	24.44		- 10	200 = 1	1000 11	
1.2 Dead+1.0 Wind 240	84.11	-8.78	5.12	633.74	1088.11	0.78
deg+1.0 Ice+1.0 Temp	24.44	10.11			10== =1	
1.2 Dead+1.0 Wind 270	84.11	-10.14	-0.03	-4.18	1255.51	1.22
deg+1.0 Ice+1.0 Temp	24.44				1001.11	
1.2 Dead+1.0 Wind 300	84.11	-8.75	-5.14	-639.02	1081.14	1.35
deg+1.0 Ice+1.0 Temp	04.44	5.05	0.04	4000.00	000.07	0.00
1.2 Dead+1.0 Wind 330	84.11	-5.05	-8.84	-1096.62	622.27	0.99
deg+1.0 Ice+1.0 Temp				0=100		
Dead+Wind 0 deg - Service	44.17	-0.02	-7.91	-954.06	0.27	0.31
Dead+Wind 30 deg - Service	44.17	3.85	-6.89	-833.93	-466.18	-0.10
Dead+Wind 60 deg - Service	44.17	6.74	-3.99	-484.83	-813.80	-0.44
Dead+Wind 90 deg - Service	44.17	7.82	0.07	7.92	-943.81	-0.72
Dead+Wind 120 deg -	44.17	6.74	3.99	480.69	-813.56	-0.77
Service						
Dead+Wind 150 deg -	44.17	3.89	6.82	821.50	-469.00	-0.58
Service						
Dead+Wind 180 deg -	44.17	0.03	7.87	948.98	-3.41	-0.30
Service						
Dead+Wind 210 deg -	44.17	-3.83	6.85	827.42	461.31	0.03
Service						
Dead+Wind 240 deg -	44.17	-6.75	3.96	478.35	813.70	0.44
Service						
Dead+Wind 270 deg -	44.17	-7.80	-0.05	-4.69	939.58	0.78
Service	44.4-			400		
Dead+Wind 300 deg -	44.17	-6.72	-4.01	-483.80	807.95	0.93
Service		2.22		222 / -		
Dead+Wind 330 deg -	44.17	-3.89	-6.85	-826.10	465.74	0.68
Service						

Solution Summary

	Sun	n of Applied Force	es		Sum of Reaction	ns	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0.00	-44.17	0.00	0.00	44.17	0.00	0.000%
2	-0.08	-53.01	-31.37	0.08	53.01	31.37	0.000%
3	-0.08	-39.76	-31.37	0.08	39.76	31.37	0.000%
4	15.29	-53.01	-27.36	-15.29	53.01	27.36	0.000%
5	15.29	-39.76	-27.36	-15.29	39.76	27.36	0.000%
6	26.75	-53.01	-15.83	-26.75	53.01	15.83	0.000%
7	26.75	-39.76	-15.83	-26.75	39.76	15.83	0.000%
8	31.02	-53.01	0.28	-31.02	53.01	-0.28	0.000%
9	31.02	-39.76	0.28	-31.02	39.76	-0.28	0.000%
10	26.77	-53.01	15.84	-26.77	53.01	-15.84	0.000%
11	26.77	-39.76	15.84	-26.77	39.76	-15.84	0.000%
12	15.45	-53.01	27.07	-15.45	53.01	- 27.07	0.000%
13	15.45	-39.76	27.07	-15.45	39.76	-27.07	0.000%
14	0.11	-53.01	31.24	-0.11	53.01	-31.24	0.000%
15	0.11	-39.76	31.24	-0.11	39.76	-31.24	0.000%
16	-15.21	-53.01	27.20	15.21	53.01	- 27.20	0.000%
17	-15.21	-39.76	27.20	15.21	39.76	-27.20	0.000%
18	-26.78	-53.01	15.72	26.78	53.01	-15.72	0.000%
19	-26.78	-39.76	15.72	26.78	39.76	-15.72	0.000%
20	-30.95	-53.01	-0.18	30.95	53.01	0.18	0.000%

	Sun	n of Applied Force	es		Sum of Reaction	ns	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
21	-30.95	-39.76	-0.18	30.95	39.76	0.18	0.000%
22	-26.68	-53.01	-15.91	26.68	53.01	15.91	0.000%
23	-26.68	-39.76	-15.91	26.68	39.76	15.91	0.000%
24	-15.42	-53.01	-27.18	15.42	53.01	27.18	0.000%
25	-15.42	-39.76	-27.18	15.42	39.76	27.18	0.000%
26	0.00	-84.11	0.00	-0.00	84.11	0.00	0.000%
27	-0.01	-84.11	-10.21	0.01	84.11	10.21	0.000%
28	5.04	-84.11	-8.88	-5.04	84.11	8.88	0.000%
29	8.77	-84.11	-5.14	-8.77	84.11	5.14	0.000%
30	10.15	-84.11	0.05	-10.15	84.11	-0.05	0.000%
31	8.77	-84.11	5.13	-8.77	84.11	-5.13	0.000%
32	5.06	-84.11	8.81	-5.06	84.11	-8.81	0.000%
33	0.01	-84.11	10.18	-0.01	84.11	-10.18	0.000%
34	-5.02	-84.11	8.85	5.02	84.11	-8.85	0.000%
35	-8.78	-84.11	5.12	8.78	84.11	-5.12	0.000%
36	-10.14	-84.11	-0.03	10.14	84.11	0.03	0.000%
37	- 8.75	-84.11	-5.14	8.75	84.11	5.14	0.000%
38	-5.05	-84.11	-8.84	5.05	84.11	8.84	0.000%
39	-0.02	-44.17	-7.91	0.02	44.17	7.91	0.000%
40	3.85	-44.17	-6.89	-3.85	44.17	6.89	0.000%
41	6.74	-44.17	-3.99	-6.74	44.17	3.99	0.000%
42	7.82	-44.17	0.07	-7.82	44.17	-0.07	0.000%
43	6.74	-44.17	3.99	-6.74	44.17	-3.99	0.000%
44	3.89	-44.17	6.82	-3.89	44.17	-6.82	0.000%
45	0.03	-44.17	7.87	-0.03	44.17	-7.87	0.000%
46	-3.83	-44.17	6.85	3.83	44.17	-6.85	0.000%
47	-6.75	-44.17	3.96	6.75	44.17	-3.96	0.000%
48	-7.80	-44.17	-0.05	7.80	44.17	0.05	0.000%
49	-6.72	-44.17	-4.01	6.72	44.17	4.01	0.000%
50	-3.89	-44.17	-6.85	3.89	44.17	6.85	0.000%

Non-Linear Convergence Results

Load	Converged?	Number	Displacement	Force
Combination	-	of Cycles	Tolerance	Tolerance
1	Yes	4	0.0000001	0.0000001
2	Yes	5	0.0000001	0.00006135
3	Yes	4	0.0000001	0.00066537
4	Yes	6	0.0000001	0.00012934
5	Yes	5	0.0000001	0.00095880
6	Yes	6	0.0000001	0.00013541
7	Yes	6	0.0000001	0.00004463
8	Yes	5	0.0000001	0.00011242
9	Yes	5	0.0000001	0.00005362
10	Yes	6	0.0000001	0.00012526
11	Yes	5	0.0000001	0.00092963
12	Yes	6	0.0000001	0.00013260
13	Yes	5	0.0000001	0.00098475
14	Yes	5	0.0000001	0.00007220
15	Yes	4	0.0000001	0.00077069
16	Yes	6	0.0000001	0.00012902
17	Yes	5	0.0000001	0.00095786
18	Yes	6	0.0000001	0.00012692
19	Yes	5 5	0.0000001	0.00094263
20	Yes		0.0000001	0.00018024
21	Yes	5	0.0000001	0.00008578
22	Yes	6	0.0000001	0.00013635
23	Yes	6	0.0000001	0.00004522
24	Yes	6	0.0000001	0.00012391
25	Yes	5	0.0000001	0.00091848
26	Yes	4	0.0000001	0.00003514
27	Yes	5	0.0000001	0.00082769
28	Yes	6	0.0000001	0.00015385
29	Yes	6	0.0000001	0.00015705
30	Yes	5	0.0000001	0.00082747
31	Yes	6	0.0000001	0.00014906

32	Yes	6	0.0000001	0.00015239
33	Yes	5	0.0000001	0.00081541
34	Yes	6	0.0000001	0.00015106
35	Yes	6	0.0000001	0.00014999
36	Yes	5	0.0000001	0.00082458
37	Yes	6	0.0000001	0.00015592
38	Yes	6	0.0000001	0.00014997
39	Yes	4	0.0000001	0.00011771
40	Yes	4	0.0000001	0.00066293
41	Yes	4	0.0000001	0.00074161
42	Yes	4	0.0000001	0.00018613
43	Yes	4	0.0000001	0.00061273
44	Yes	4	0.0000001	0.00072438
45	Yes	4	0.0000001	0.00011810
46	Yes	4	0.0000001	0.00066751
47	Yes	4	0.0000001	0.00062609
48	Yes	4	0.0000001	0.00020708
49	Yes	4	0.0000001	0.00077665
50	Yes	4	0.0000001	0.00060579

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	٥	۰
L1	168 - 119.25	28.965	40	1.4983	0.0035
L2	123.5 - 78.5	15.858	40	1.2456	0.0038
L3	83.75 - 38.75	7.160	40	0.8018	0.0016
L4	45 - 0	2.101	40	0.4246	0.0007

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	٥	٥	ft
168.00	800 10121 w/ Mount Pipe	40	28.965	1.4983	0.0035	49121
152.00	SC3-W100ASTX	40	24.028	1.4304	0.0036	15350
148.00	SC3-W100ASTX	40	22.817	1.4110	0.0037	12280
147.00	BPA7496-180-11 w/ Mount Pipe	40	22.516	1.4060	0.0037	11695
143.00	MPRD2449	40	21.325	1.3845	0.0038	9823
140.00	(2) CC807-11	40	20.445	1.3672	0.0038	8771
135.00	ANT450F6	40	19.005	1.3354	0.0039	7441
130.00	Platform Mount [LP 1201-1]	40	17.606	1.2995	0.0039	6462
120.00	MX08FRO665-21 w/ Mount Pipe	40	14.953	1.2128	0.0037	5513
100.00	SC3-W100ASTX	40	10.297	0.9915	0.0026	5470
79.00	1' x 2-1/2"	40	6.353	0.7503	0.0014	5353
52.00	GPS-TMG-HR-26NCM	40	2.751	0.4888	0.0007	4916

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	•
L1	168 - 119.25	115.632	4	5.9903	0.0145
L2	123.5 - 78.5	63.366	4	4.9814	0.0153
L3	83.75 - 38.75	28.626	4	3.2077	0.0065
L4	45 - 0	8.402	4	1.6984	0.0026

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	۰	۰	ft
168.00	800 10121 w/ Mount Pipe	4	115.632	5.9903	0.0145	12582
152.00	SC3-W100ASTX	4	95.949	5.7195	0.0150	3930
148.00	SC3-W100ASTX	4	91.121	5.6422	0.0152	3143
147.00	BPA7496-180-11 w/ Mount Pipe	4	89.923	5.6219	0.0153	2993
143.00	MPRD2449	4	85.175	5.5364	0.0156	2513
140.00	(2) CC807-11	4	81.664	5.4671	0.0158	2243
135.00	ANT450F6	4	75.923	5.3401	0.0159	1902
130.00	Platform Mount [LP 1201-1]	4	70.342	5.1967	0.0158	1650
120.00	MX08FRO665-21 w/ Mount Pipe	4	59.756	4.8507	0.0148	1405
100.00	SC3-W100ASTX	4	41.162	3.9662	0.0104	1384
79.00	1' x 2-1/2"	4	25.400	3.0015	0.0056	1346
52.00	GPS-TMG-HR-26NCM	4	10.998	1.9552	0.0030	1232

Compression Checks

Pole Design Data

Section No.	Elevation	Size	L	Lu	KI/r	Α	P_u	ϕP_n	Ratio P _u
	ft		ft	ft		in²	K	K	ΦP_n
L1	168 - 119.25 (1)	TP34.288x24x0.25	48.75	0.00	0.0	26.297 5	-15.74	1538.40	0.010
L2	119.25 - 78.5 (2)	TP42.387x32.8911x0.281	45.00	0.00	0.0	36.598 3	-26.33	2141.00	0.012
L3	78.5 - 38.75 (3)	TP50.213x40.7166x0.375	45.00	0.00	0.0	57.749 8	-37.76	3378.36	0.011
L4	38.75 - 0 (4)	TP57.64x48.1441x0.375	45.00	0.00	0.0	68.159 7	-52.99	3987.34	0.013

Pole Bending Design Data

Section	Elevation	Size	M _{ux}	ϕM_{nx}	Ratio	M_{uy}	ϕM_{ny}	Ratio
No.					M_{ux}		, ,	M_{uy}
	ft		kip-ft	kip-ft	ϕM_{nx}	kip-ft	kip-ft	$\frac{M_{uy}}{\phi M_{ny}}$
L1	168 - 119.25 (1)	TP34.288x24x0.25	509.05	1206.64	0.422	0.00	1206.64	0.000
L2	119.25 - 78.5 (2)	TP42.387x32.8911x0.2813	1425.03	2002.83	0.712	0.00	2002.83	0.000
L3	78.5 - 38.75 (3)	TP50.213x40.7166x0.375	2465.93	3912.78	0.630	0.00	3912.78	0.000
L4	38.75 - 0 (4)	TP57.64x48.1441x0.375	3817.80	5107.74	0.747	0.00	5107.74	0.000

Pole Shear Design Data

Section No.	Elevation	Size	Actual V _u	ϕV_n	Ratio V _u	Actual T _u	φ <i>T</i> _n	Ratio T _u
	ft		K	K	ϕV_n	kip-ft	kip-ft	$\frac{T_u}{\phi T_n}$
L1	168 - 119.25 (1)	TP34.288x24x0.25	18.28	461.52	0.040	1.13	1339.48	0.001
L2	119.25 - 78.5 (2)	TP42.387x32.8911x0.2 813	25.15	642.30	0.039	0.33	2306.11	0.000
L3	78.5 - 38.75 (3)	TP50.213x40.7166x0.3 75	28.48	1013.51	0.028	0.40	4306.46	0.000
L4	38.75 - 0 (4)	TP57.64x48.1441x0.37 5	31.38	1196.20	0.026	0.40	5998.93	0.000

Pole Interaction Design Data

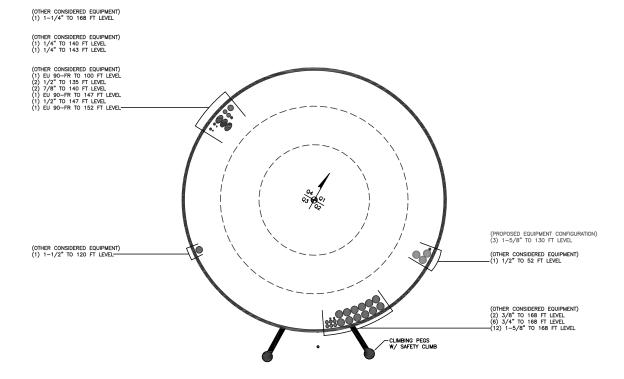
Section No.	Elevation	Ratio P _u	Ratio M _{ux}	Ratio M _{uy}	Ratio Vu	Ratio T _u	Comb. Stress	Allow. Stress	Criteria
	ft	ϕP_n	φ M _{nx}	φ <i>M</i> _{ny}	ϕV_n	φ <i>T</i> _n	Ratio	Ratio	
L1	168 - 119.25 (1)	0.010	0.422	0.000	0.040	0.001	0.434	1.050	4.8.2
L2	119.25 - 78.5 (2)	0.012	0.712	0.000	0.039	0.000	0.725	1.050	4.8.2
L3	78.5 - 38.75 (3)	0.011	0.630	0.000	0.028	0.000	0.642	1.050	4.8.2
L4	38.75 - 0 (4)	0.013	0.747	0.000	0.026	0.000	0.761	1.050	4.8.2

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	øP _{allow} K	% Capacity	Pass Fail
L1	168 - 119.25	Pole	TP34.288x24x0.25	1	-15.74	1615.32	41.3	Pass
L2	119.25 - 78.5	Pole	TP42.387x32.8911x0.2813	2	-26.33	2248.05	69.1	Pass
L3	78.5 - 38.75	Pole	TP50.213x40.7166x0.375	3	-37.76	3547.28	61.2	Pass
L4	38.75 - 0	Pole	TP57.64x48.1441x0.375	4	-52.99	4186.71	72.5	Pass
							Summary	
						Pole (L4)	72.5	Pass
						RATING =	72.5	Pass

APPENDIX B BASE LEVEL DRAWING

BUSINESS UNIT: 842875 TOWER ID: C_BASELEVEL





APPENDIX C ADDITIONAL CALCULATIONS

Monopole Base Plate Connection

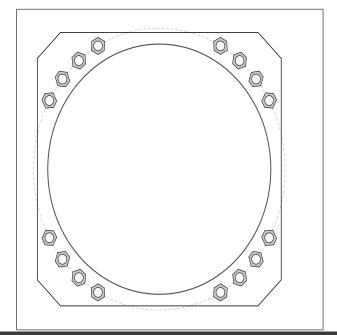


Site Info						
BU #	842875					
Site Name	Windsorday Hill					
Order #	584557					

Analysis Considerations	
TIA-222 Revision	Н
Grout Considered:	No
I _{ar} (in)	1.75

Applied Loads						
Moment (kip-ft)	3817.80					
Axial Force (kips)	52.99					
Shear Force (kips)	31.38					

^{*}TIA-222-H Section 15.5 Applied



Connection Properties

Anchor Rod Data

(16) 2-1/4" ø bolts (A615-75 N; Fy=75 ksi, Fu=100 ksi) on 65" BC Anchor Spacing: 6 in

Base Plate Data

63" W x 3.25" Plate (A572-55; Fy=55 ksi, Fu=70 ksi); Clip: 6 in

Stiffener Data

N/A

Pole Data

57.64" x 0.375" 18-sided pole (A607-65; Fy=65 ksi, Fu=80 ksi)

Analysis Results

Anchor Rod Summary	(u	nits of kips, kip-in)
Pu_t = 172.81	φPn_t = 243.75	Stress Rating
Vu = 1.96	φVn = 149.1	67.5%
Mu = n/a	φMn = n/a	Pass
Base Plate Summary		
Max Stress (ksi):	25 37	(Flexural)

Base Plate Summary		
Max Stress (ksi):	25.37	(Flexural)
Allowable Stress (ksi):	49.5	
Stress Rating:	48.8%	Pass

CCIplate - Version 4.1.2 Analysis Date: 10/8/2021

Drilled Pier Foundation

BU #: 842875 Site Name: Windsorday Hill Order Number: 584557 Rev.0 TIA-222 Revison: H Tower Type: Monopole

Applied Loads							
	Comp.	Uplift					
Moment (kip-ft)	3817.8						
Axial Force (kips)	53.01						
Shear Force (kips)	31.34						
Material Properties							
Concrete Strength, f'c:	3	ksi					
Rebar Strength, Fy:	60	ksi					
Ti- Vi-II Character F. A.	40	1:					

Tie Yield Strength, Fyt:	40	KSI]
Pier D	esign Data		Rebar & Pier Options
Depth	24	ft	
Ext. Above Grade	0.5	ft	Embedded Pole Inputs
Pier	Belled Pier Inputs		
From 0.5' above g	1		
Pier Diameter	8	ft	
Rebar Quantity	24		
Rebar Size	11		
Clear Cover to Ties	4	in	

Tie Size

	Max Moment (kip-ft)	4055.62	-
	Rating*	24.4%	-
	Soil Vertical Check	Compression	Uplift
Override	Skin Friction (kips)	1187.52	-
(ksi)	End Bearing (kips)	3015.93	-
	Weight of Concrete (kips)	150.11	-
	Total Capacity (kips)	4203.45	-
	Axial (kips)	203.12	-
er Options	Rating*	4.6%	
	Reinforced Concrete Flexure	Compression	Uplift
Pole Inputs	Critical Depth (ft from TOC)	8.22	-
er Inputs	Critical Moment (kip-ft)	4054.08	-
	Critical Moment Capacity	6983.70	-
	Rating*	55.3%	-
	Reinforced Concrete Shear	Compression	Uplift
	Critical Double (ft forms TOC)	20.50	

Compression 8.73 5.20

20.58

859.02

822.97

Soil Lateral Check

D_{v=0} (ft from TOC)

Soil Safety Factor

Critical Depth (ft from TOC)

Critical Shear (kip)

Critical Shear Capacity

Structural Foundation Rating*	99.4%
Soil Interaction Rating*	24.4%
*Rating per TIA-222-H Section	n 15.5



Check Limitation		
Apply TIA-222-H Section 15.5:	V	
N/A		
Additional Longitudinal Reb	ar	
Input Effective Depths (else Actual):		
Shear Design Options		
Check Shear along Depth of Pier:	V	Ī
Utilize Shear-Friction Methodology:		Ī
Override Critical Depth:		

Go to Soil Calculations

	Soil Profile													
Groundwa	Groundwater Depth 5 # of Layers 5													
Layer	Top (ft)	Bottom (ft)	Thickness (ft)	Y _{soil} (pcf)	Y _{concrete} (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)		Ultimate Skin Friction Comp Override (ksf)	I Ultimate Skin	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	5	5	100	150	0	0	0.000	0.000	0.00	0.00			Cohesionless
2	5	10	5	60	87.6	0	35	0.000	0.000	0.00	0.00			Cohesionless
3	10	15	5	55	87.6	0	33	0.000	0.000	0.00	0.00			Cohesionless
4	15	17	2	80	87.6	0	40	0.000	0.000	0.00	0.00			Cohesionless
5	17	24	7	95	87.6	20	0	9.000	9.000			80		Cohesive



Address:

No Address at This Location

ASCE 7 Hazards Report

Standard: ASCE/SEI 7-16

Risk Category: ^Ⅱ

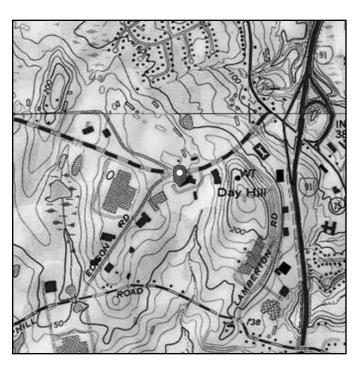
Soil Class: D - Default (see

Section 11.4.3)

Elevation: 166.35 ft (NAVD 88)

Latitude: 41.871139

Default (see Longitude: -72.671111





Wind

Results:

Wind Speed: 116 Vmph
10-year MRI 75 Vmph
25-year MRI 83 Vmph
50-year MRI 90 Vmph
100-year MRI 97 Vmph

Data Source: ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2

Date Accessed: Thu Oct 07 2021

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings need not be protected against wind-borne debris.



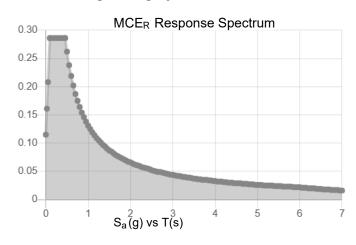
Seismic

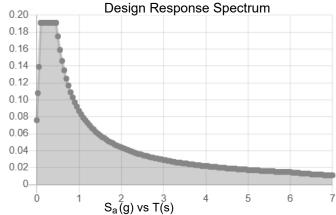
Site Soil Class: D - Default (see Section 11.4.3)

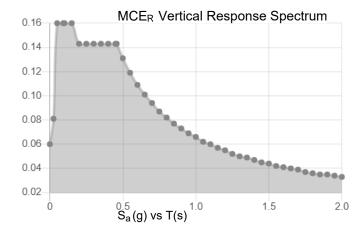
Results:

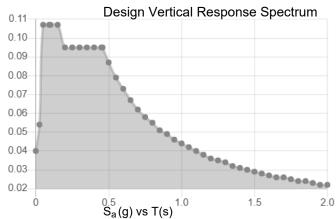
S _s :	0.179	S _{D1} :	0.087
S ₁ :	0.055	T _L :	6
F _a :	1.6	PGA:	0.095
F_{ν} :	2.4	PGA _M :	0.152
S _{MS} :	0.286	F _{PGA} :	1.6
S _{M1} :	0.131	l _e :	1
S _{DS} :	0.191	C _v :	0.7

Seismic Design Category B









Data Accessed: Thu Oct 07 2021

Date Source: USGS Seismic Design Maps based on ASCE/SEI 7-16 and ASCE/SEI 7-16 Table 1.5-2. Additional data for site-specific ground motion procedures in

accordance with ASCE/SEI 7-16 Ch. 21 are available from USGS.



Ice

Results:

Ice Thickness: 1.50 in.

Concurrent Temperature: 5 F

Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-16, Figs. 10-2 through 10-8

Date Accessed: Thu Oct 07 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 500-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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Exhibit E

Mount Analysis

Date: October 5, 2021



520 South Main Street, Suite 2531 Akron, Ohio 44311 (216) 927-8663 CrownMA@gpdgroup.com

Subject: Mount Modification Report

Carrier Designation: T-Mobile Equipment Change-Out

Carrier Site Number: CTHA267A
Carrier Site Name: CT54XC787

Crown Castle Designation: BU Number: 842875

Site Name: WINDSORDAY HILL

JDE Job Number: 684573 **Order Number:** 584557 Rev. 0

Engineering Firm Designation: GPD Report Designation: 2021777.842875.04

Site Data: 99 Day Hill Road, Windsor, Hartford County, CT 06095

Latitude 41°52' 16.10" Longitude -72°40' 16.00"

Structure Information: Tower Height & Type: 168.0 ft Monopole Tower

Mount Elevation: 130.0 ft

Mount Type: 14.0 ft Platform Mount

GPD is pleased to submit this "**Mount Modification Report**" to determine the structural integrity of T-Mobile's antenna mounting system with the proposed appurtenance and equipment addition on the above mentioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

Platform Mount Sufficient*

*See Section 4.1 of this report for the loading and structural modifications required in order for the mount to support the loading listed in Table 1.

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Brandon Brookbank

Respectfully Submitted by:

Christopher J. Scheks, P.E. Connecticut #: 0030026

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Mount Modification Design Drawings (MDD)

1) INTRODUCTION

This is an existing 14.0' Platform Mount.

2) ANALYSIS CRITERIA

TIA-222 Revision: TIA-222-H

Risk Category:

Ultimate Wind Speed: 125 mph

Exposure Category: C
Topographic Factor at Base: 1
Topographic Factor at Mount: 1
Ice Thickness: 2.0 in
Wind Speed with Ice: 50 mph
Live Loading Wind Speed: 30 mph
Man Live Load at Mid/End-Points: 250 lb
Man Live Load at Mount Pipes: 500 lb

Table 1 - Proposed Equipment Configuration

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	
		3	Ericsson	AIR6449 B41_T-MOBILE	14.0 ft.
130.0 131.0		3	RFS/Celwave	APXVAALL24_43-U-NA20_TMO	Platform
		3	Ericsson	RADIO 4460 B2/B25 B66_TMO	Mount
		3	Ericsson	Radio 4480_TMOV2	Widuit

3) ANALYSIS PROCEDURE

Table 2 - Documents Provided

Document	Remarks	Reference	Source
CCI Application	Crown Order Number 584557 Rev. 0	-	CCI
RF Data Sheet	Sprint Retain Site ID: CTHA267A, dated 7/9/2021	-	CCI
Mount Modification Design Drawings	GPD Project #: 2021777.842875.04, dated 10/05/2021	-	GPD

3.1) Analysis Method

RISA-3D Edition (Version 17.0.2), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed by GPD, using Microsoft Excel, was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 Mount Analysis (Revision D).

3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) This analysis assumes all information reference in Table 2 is current and correct.
- 5) The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 6) The mount was modeled from site photos. Member information and dimensions not provided have been assumed based on previous experience with similar mounts. No guarantee can be made as to the accuracy of these assumptions without a complete mount mapping.

7) Steel grades have been assumed as follows, unless noted otherwise:

Angle, Plate HSS (Rectangular) ASTM A36 (GR 36) ASTM A500 (GR B-46) ASTM A53 (GR 35)

Pipe

This analysis may be affected if any assumptions are not valid or have been made in error. GPD should be notified to determine the effect on the structural integrity of the antenna mounting system.

4) ANALYSIS RESULTS

Table 3 - Mount Component Stresses vs. Capacity (Platform Mount)

able 5 - Mount Component Stresses vs. Capacity (Flatform Mount)								
Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail			
	Face Horizontal	M23		54.7	Pass			
	Inner Horizontal	M46		41.3	Pass			
	Corner Angle	M25		6.5	Pass			
1,3	Standoff Arm (Outer)	M4		26.5	Pass			
	Standoff Arm (Inner)	M27		44.9	Pass			
	Support Rail	M69	130.0	47.1	Pass			
	Support Rail Corner Connection	M22		32.4	Pass			
	Pipe Mount	A 1		53.9	Pass			
	Grating Brace	M52A		10.0	Pass			
	Reinforcement Angle	M55		13.6	Pass			
2,3	Reinforcement to Tower Connection	-		2.1	Pass			

Structure Rating (max from all components) =	54.7%³
--	--------

Notes:

¹⁾ See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed.

²⁾ See additional documentation in "Appendix D - Additional Calculations" for calculations supporting the % capacity consumed.

³⁾ Ratings per TIA-222-H section 15.5.

4.1) Recommendations

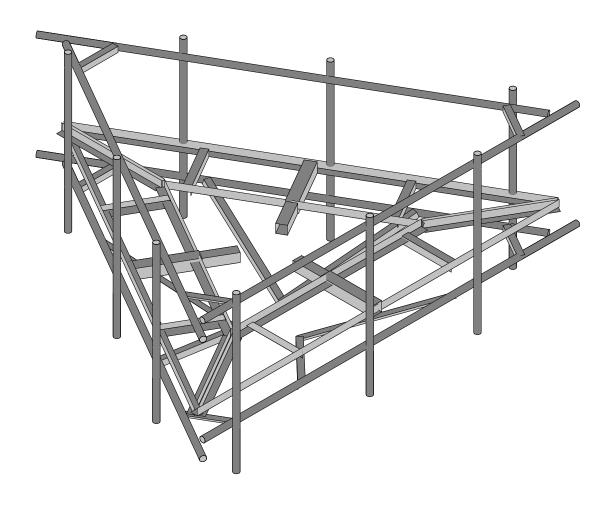
The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the structural modifications listed below must be completed.

- 1. Install (2) support rail kits, Site Pro 1 HRK14-U
- 2. Install v-style reinforcement kit, Site Pro 1 PRK-SFS

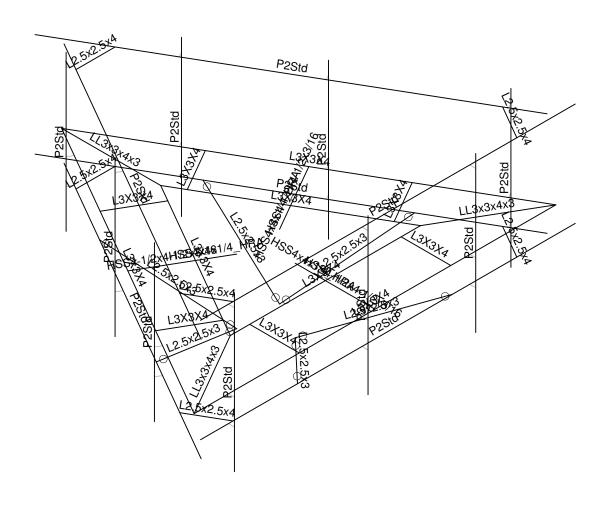
Engineering detail drawings have been provided in Appendix E – Mount Modification Design Drawings (MDD). Connection from the mount to the tower and local stresses on the tower are sufficient.

APPENDIX A WIRE FRAME AND RENDERED MODELS

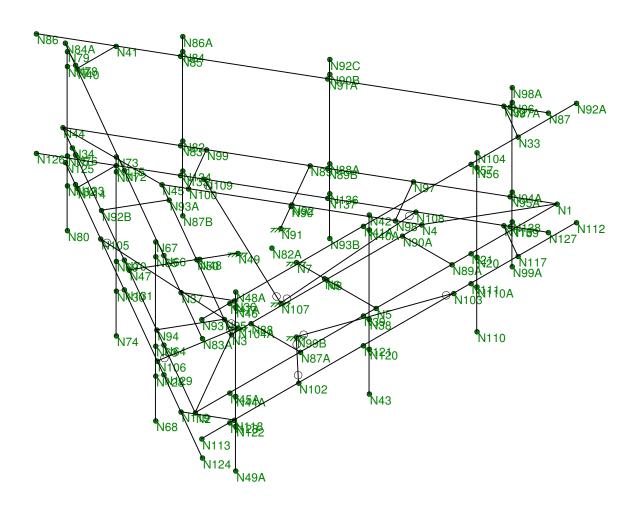




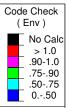


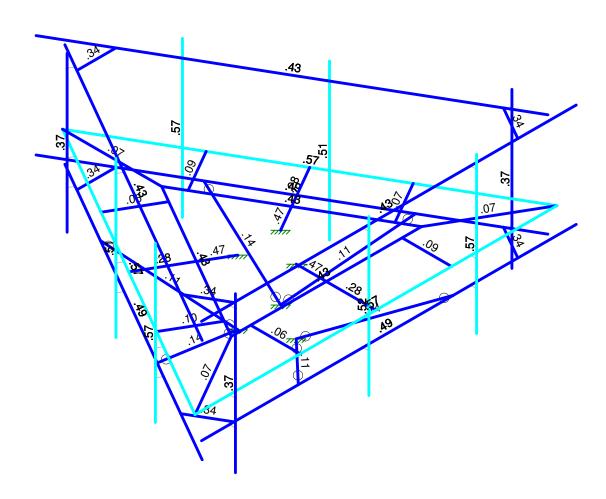




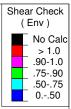


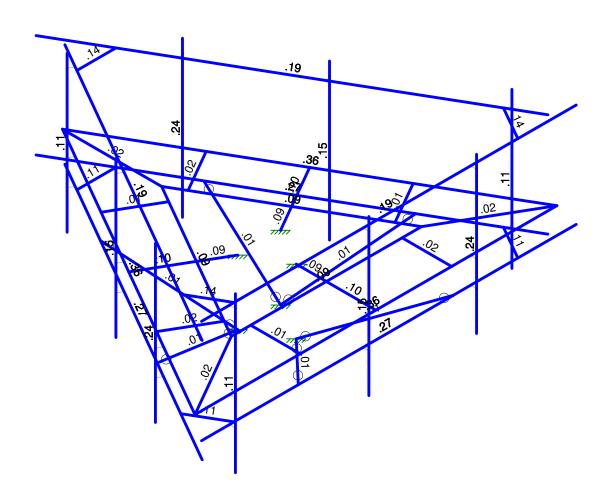




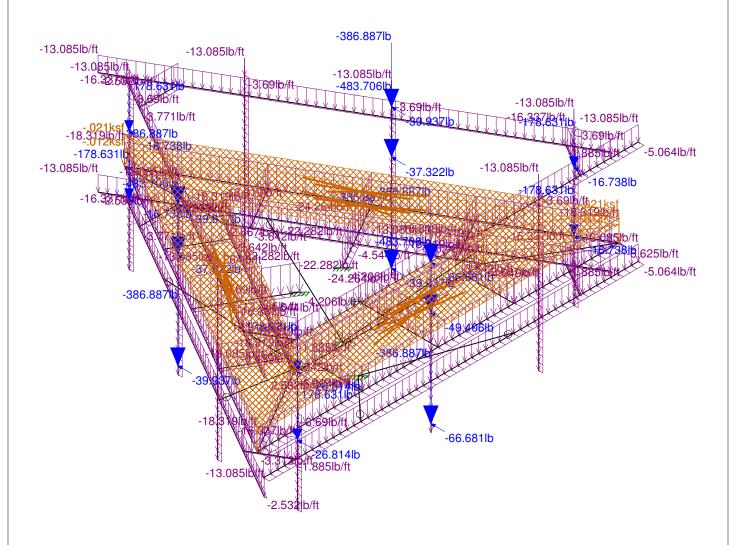












APPENDIX B SOFTWARE INPUT CALCULATIONS



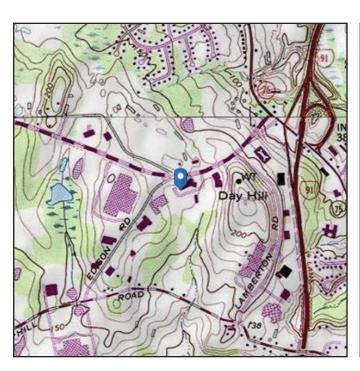
Address:

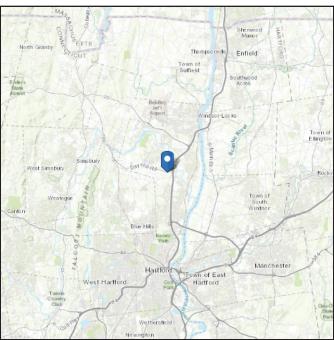
No Address at This Location

ASCE 7 Hazards Report

Standard: ASCE/SEI 7-10 Elevation: 166.35 ft (NAVD 88)

Risk Category: || Latitude: 41.871139 Soil Class: D - Stiff Soil Longitude: -72.671111





Thu Aug 26 2021

Wind

Results:

Wind Speed: 125 Vmph per 2018 Connecticut Building Code Appendix N

10-year MRI 76 Vmph 25-year MRI 86 Vmph 50-year MRI 92 Vmph 100-year MRI 99 Vmph

Date Sociessed: ASCE(SE2672002,1Fig. 26.5-1A and Figs. CC-1–CC-4, and Section 26.5.2, incorporating errata of March 12, 2014

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

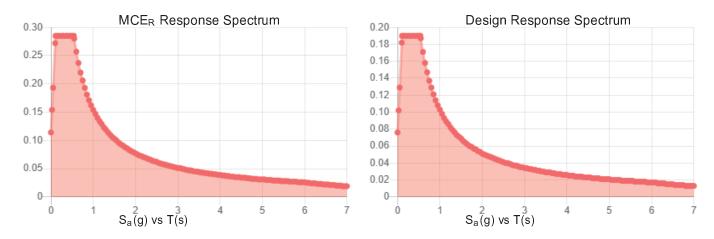
Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.



Seismic

Site Soil Class: Results:	D - Stiff Soil			
S _s :	0.178	S _{DS} :	0.19	
S_1 :	0.064	S _{D1} :	0.103	
F _a :	1.6	T _L :	6	
F _v :	2.4	PGA:	0.089	
S _{MS} :	0.285	PGA _M :	0.142	
S _{M1} :	0.154	F _{PGA} :	1.6	
		l. :	1	

Seismic Design Category B



Data Accessed: Thu Aug 26 2021

Date Source: USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating

Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with

ASCE/SEI 7-10 Ch. 21 are available from USGS.



lce

Results:

Ice Thickness: 1.00 in.

Concurrent Temperature: 5 F

Gust Speed: 50 mph

Data Source: Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Thu Aug 26 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE 7 standard.

In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE 7 Hazard Tool.



Structure Ir	formation	
Structure Type:	Monopole	
Structure Height:	168	ft
z (Mount Centerline) =	130	ft
Gh (Mount Gust Effect Factor) =	1.00	
Risk Category:	II	

Cod	le Specifications	
TIA/EIA Code:	Н	
Ultimate Wind Speed (No Ice) =	125	mph (3-s gust)
Ultimate Wind Speed (With Ice) =	50	mph (3-s gust)
Ice Thickness	2	in
Exposure Category	С	
Tower Base Elevation (AMSL)	166	ft

Торо	graphic Inputs	
Topographic Feature:	N/A	

			;				No Ice	Ice Output				
Mount Components	Member Type	Length (in)	Side (Longest seeing wind) (in)	Other Side (in)	Calculated Dc, for ice weight (in)	Dc, for ice weight (in)	Area Type (Round or Flat)	K _a	User's Wind Multiplier	Normal Wind Force (lb/ft)*	Normal Ice Wind Force (lb/ft)*	Ice Weight (lb/ft)*
Face Horizontal	Angle	168.000	3	3		4.24	Flat	0.90	1.00	25.26	7.36	18.32
Inner Horizontal	Angle	88.000	3	3		4.24	Flat	0.90	1.00	25.26	5.93	18.32
Corner Angle	Other	48.000	3	6.375	0	0.00	Flat	0.90	1.00	21.47	5.19	6.43
Standoff Arm (Outer)	Square/Rect.	24.000	4.5	4.5		6.36	Flat	0.90	1.00	25.12	5.83	24.26
Standoff Arm (Inner)	Square/Rect.	13.000	4	4		5.66	Flat	0.90	1.00	20.77	5.40	22.28
Support Rail	Pipe	174.000	2.375	2.375		2.38	Round	0.90	1.00	12.00	5.63	13.08
Support Rail Corner Connection	Angle	18.000	2.5	2.5		3.54	Flat	0.90	1.00	14.81	4.19	16.34
Pipe Mount	Pipe	72.000	2.375	2.375		2.38	Round	0.90	1.00	12.00	4.10	13.08
Grating Brace	Angle	23.000	3	3		4.24	Flat	0.90	1.00	17.96	4.67	18.32
Reinforcement Angle	Angle	53.000	2.5	2.5		3.54	Flat	0.90	1.00	19.72	4.89	16.34

*All forces are unfactored.

	Appurtenances Appurtenance Model Loading Elevation (ft) Height (in) Front Width (in) Side Depth (in) (3) AIR6449 B41_T-MOBILE 131 33.11 20.51 8.54 (3) APXVAALL24_43-U-NA20_TMO 131 95.9 24 8.5 (3) BADIO 4460 B2/B2 B66_TMO 131 17 15.1 11.9							Shielding		No	Ice	Ice Out	put	
	Appurtenance Model	Loading Elevation (ft)	Height (in)	Front Width (in)	Side Depth (in)	Wt (lbs)	Type for Area	Front Shielding (%)	Side Shielding (%)	K _a and/or block shielding	Normal Wind Force (lbs)*	Wt (lbs) (no ice)*	Normal Wind Force (lbs) (w/ ice)*	Wt (lbs) (only ice)*
(3)	AIR6449 B41_T-MOBILE	131	33.11	20.51	8.54	114.63	CFD	0%	0%	0.90	240.00	114.63	53.63	219.71
(3)	APXVAALL24_43-U-NA20_TMO	131	95.9	24	8.5	149.9	CFD	0%	0%	0.90	668.09	149.90	133.36	593.89
(3)	RADIO 4460 B2/B25 B66_TMO	131	17	15.1	11.9	109	Flat	0%	0%	0.90	97.42	109.00	21.55	128.23
(3)	Radio 4480_TMOV2	131	22	15.7	7.5	81	Flat	0%	0%	0.90	131.08	81.00	27.85	127.48

*All forces are unfactored.

APPENDIX C SOFTWARE ANALYSIS OUTPUT

Oct 5, 2021 3:34 PM Checked By:_

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (\1E	.Density[k/ft	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A992	29000	11154	.3	.65	.49	50	1.1	65	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
3	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42	1.4	58	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46	1.4	58	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2
7	A1085	29000	11154	.3	.65	.49	50	1.4	65	1.3

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design	. Material	Design	. A [in2]	lyy [in4]	Izz [in4]	J [in4]
1	Face Horizontal	L3X3X4	None	None	A36 Gr.36	Typical	1.44	1.23	1.23	.031
2	Inner Horizontal	L3X3X4	None	None	A36 Gr.36	Typical	1.44	1.23	1.23	.031
3	Corner Angle	LL3x3x4x3	None	None	A36 Gr.36	Typical	2.88	5.48	2.46	.063
4	Standoff Arm (Outer)	HSS4-1/2x4-1/2x3/16	None	None	A500 Gr.B Re	Typical	3.234	10.044	10.044	15.038
5	Standoff Arm (Inner)	HSS4x4x1/4 HRA	None	None	A500 Gr.B Re	Typical	3.75	8.828	8.828	13.184
6	Support Rail	P2Std	None	None	A53 Gr.B	Typical	1.075	.666	.666	1.331
7	Support Rail Corner Connection	L2.5x2.5x4	None	None	A36 Gr.36	Typical	1.19	.692	.692	.026
8	Pipe Mount	P2Std	None	None	A53 Gr.B	Typical	1.075	.666	.666	1.331
9	Grating Brace	L3X3X4	None	None	A36 Gr.36	Typical	1.44	1.23	1.23	.031
10	Reinforcement Angle	L2.5x2.5x3	None	None	A36 Gr.36	Typical	.901	.535	.535	.011

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(Me	Surface(P
1	Dead	DĽ		-1	_		18		3	,
2	No Ice Wind 0 deg	None					18	37		
3	No Ice Wind 30 deg	None					36	72		
4	No Ice Wind 60 deg	None					36	74		
5	No Ice Wind 90 deg	None					18	36		
6	No Ice Wind 120 deg	None					36	74		
7	No Ice Wind 150 deg	None					36	72		
8	No Ice Wind 180 deg	None					18	37		
9	No Ice Wind 210 deg	None					36	72		
10	No Ice Wind 240 deg	None					36	74		
11	No Ice Wind 270 deg	None					18	36		
12	No Ice Wind 300 deg	None					36	74		
13	No Ice Wind 330 deg	None					36	72		
14	Ice Weight	None					18	42	3	
15	Ice Wind 0 deg	None					18	37		
16	Ice Wind 30 deg	None					36	72		
17	Ice Wind 60 deg	None					36	74		
18	Ice Wind 90 deg	None					18	36		
19	Ice Wind 120 deg	None					36	74		
20	Ice Wind 150 deg	None					36	72		
21	Ice Wind 180 deg	None					18	37		
22	Ice Wind 210 deg	None					36	72		
23	Ice Wind 240 deg	None					36	74		
24	Ice Wind 270 deg	None					18	36		
25	Ice Wind 300 deg	None					36	74		
26	Ice Wind 330 deg	None					36	72		
27	Live Load - A1	None					1			
28	Live Load - A2	None					1			
29	Live Load - A3	None					1			
30	Live Load - B1	None					1			



Oct 5, 2021 3:34 PM Checked By:_

Basic Load Cases (Continued)

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(Me	Surface(P
31	Live Load - B2	None					11			
32	Live Load - B3	None					1			
33	Live Load - C1	None					1			
34	Live Load - C2	None					1			
35	Live Load - C3	None					1			
36	Live Load - M1 (Start)	None					1			
37	Live Load - M1 (Midd	None					1			
38	Live Load - M1 (End)	None					1			
39	Live Load - M4 (Start)	None					1			
40	Live Load - M4 (Midd	None					1			
41	Live Load - M4 (End)	None					1			
42	Live Load - M5 (Start)	None					1			
43	Live Load - M5 (Midd	None					1			
44	Live Load - M5 (End)	None					1			
45	Live Load - M23 (Start)	None					1			
46	Live Load - M23 (Mid	None					1			
47	Live Load - M23 (End)	None					1			
48	Live Load - M26 (Start)	None					1			
49	Live Load - M26 (Mid	None					1			
50	Live Load - M26 (End)	None					1			
51	Live Load - M27 (Start)	None					1			
52	Live Load - M27 (Mid	None					1			
53	Live Load - M27 (End)	None					1			
54	Live Load - M45 (Start)	None					1			
55	Live Load - M45 (Mid	None					1			
56	Live Load - M45 (End)	None					1			
57	Live Load - M47B (St	None					2			
58	Live Load - M47B (Mi	None					2			
59	Live Load - M47B (E	None					2			
60	Live Load - M48 (Start)	None					1			
61	Live Load - M48 (Mid	None					1			
62	Live Load - M48 (End)	None					1			
63	Live Load - M48B (St	None					2			
64	Live Load - M48B (Mi	None					2			
65	Live Load - M48B (E	None					2			
66	Live Load - M49 (Start)	None					1			
67	Live Load - M49 (Mid	None					1			
68	Live Load - M49 (End)	None					1			
69	Live Load - M52 (Start)	None					2			
70	Live Load - M52 (Mid	None					2			
71	Live Load - M52 (End)						2			
	BLC 1 Transient Area	None						60		
72	BLC 14 Transient Are	None						69		
73	DLU 14 Transient Are	None		1			1	69		

Load Combinations

	Description	S	PDelta	S	В	Fa	В	Fa	В	Fa	B Fa	a B	.Fa	B F	a I	B F	a I	3 Fa	. B	. Fa	. B	Fa
1	1.4 Dead	Yes	Υ		1	1.4	0		0		0	0		0		0		0				
2	1.2 Dead + 1.0 Wind	Yes	Υ		1	1.2	2	1	0		0	0		0		0		0				
3	0.9 Dead + 1.0 Wind	Yes	Υ		1	.9	2	1	0		0	0		0		0		0				
4	1.2 Dead + 1.0 Wind	Yes	Υ		1	1.2	3	1	0		0	0		0		0		0				
5	0.9 Dead + 1.0 Wind	Yes	Υ		1	.9	3	1	0		0	0		0		0		0				
6	1.2 Dead + 1.0 Wind	Yes	Υ		1	1.2	4	1	0		0	0		0		0		0				
7	0.9 Dead + 1.0 Wind	Yes	Υ		1	.9	4	1	0		0	0		0		0		0				
8	1.2 Dead + 1.0 Wind	Yes	Υ		1	1.2	5	1	0		0	0		0		0		0				
9	0.9 Dead + 1.0 Wind	Yes	Υ		1	.9	5	1	0		0	0		0		0		0				

Oct 5, 2021 3:34 PM Checked By:_

Load Combinations (Continued)

	Description	S	PDelta	S	В.,	Fa.	В	Fa	В	Fa	В	Fa	B Fa	. B	Fa	В	Fa	В	Fa	В	Fa	В	Fa
10	1.2 Dead + 1.0 Wind	-	Y		1	1.2		1	0		0		0	0		0		0					
11	0.9 Dead + 1.0 Wind	. Yes	Υ		1	.9		1	0		0		0	0		0		0					
12	1.2 Dead + 1.0 Wind	. Yes	Υ		1	1.2	7	1	0		0		0	0		0		0					
13	0.9 Dead + 1.0 Wind	. Yes	Υ		1	.9	7	1	0		0		0	0		0		0					
14	1.2 Dead + 1.0 Wind	. Yes	Υ		1	1.2	8	1	0		0		0	0		0		0					
15	0.9 Dead + 1.0 Wind	. Yes	Υ		1	.9	8	1	0		0		0	0		0		0					
16	1.2 Dead + 1.0 Wind	. Yes	Υ		1	1.2	9	1	0		0		0	0		0		0					
17	0.9 Dead + 1.0 Wind	. Yes	Υ		1	.9	9	1	0		0		0	0		0		0					
18	1.2 Dead + 1.0 Wind	. Yes	Υ		1	1.2		1	0		0		0	0		0		0					
19	0.9 Dead + 1.0 Wind	. Yes	Y		1		10	1	0		0		0	0		0		0					
20	1.2 Dead + 1.0 Wind	. Yes	Υ		1	1.2	11	1	0		0		0	0		0		0					
21	0.9 Dead + 1.0 Wind	. Yes	Y		1	.9		1	0		0		0	0		0		0					
22	1.2 Dead + 1.0 Wind		Υ		1	1.2		1	0		0		0	0		0		0					
23	0.9 Dead + 1.0 Wind		Υ		1	.9	12	1	0		0		0	0		0		0					
24	1.2 Dead + 1.0 Wind		Υ		1	1.2			0		0		0	0		0		0					
25	0.9 Dead + 1.0 Wind	_	Y		1	.9			0		0		0	0		0		0					
26	1.2 Dead + 1.0 Ice Wi.		Υ		1	1.2			14			1_	0	0		0		0					
27	1.2 Dead + 1.0 Ice Wi.		Υ		1	1.2		1	14			_1_	0	0		0		0					
28	1.2 Dead + 1.0 Ice Wi.		Υ		1	1.2		1	14			1_	0	0		0		0					
29	1.2 Dead + 1.0 Ice Wi.	_	Y		1	1.2			14			_1_	0	0		0		0					
30		_	Υ		1	1.2	_		14			1_	0	0		0		0					
31	1.2 Dead + 1.0 Ice Wi.		Y		1		20	1	14			_1_	0	0		0		0					
32	1.2 Dead + 1.0 Ice Wi.	_	Y		1	1.2		1	14			1_	0	0		0		0					
33	1.2 Dead + 1.0 Ice Wi.		Y		1	1.2		1	14			1_	0	0		0		0					
34	1.2 Dead + 1.0 Ice Wi.		Υ		1	1.2			14			1_	0	0		0		0					
35	1.2 Dead + 1.0 Ice Wi.		<u>Y</u>		1	1.2	_		14			1_	0	0	-	0		0					
36	1.2 Dead + 1.0 Ice Wi.	_	Y		1		25		14	_		1_	0	0		0		0					
37	1.2 Dead + 1.0 Ice Wi.	_	<u>Y</u>		1	1.2			14			1_	0	0		0		0					
38	1.2 Dead + 1.5 Live		<u>Y</u>		1	1.2		1.5		.058			0	0		0		0					
39	1.2 Dead + 1.5 Live		<u>Y</u>		1	1.2		1.5					0	0		0		0					
40			<u>Y</u>		1	1.2		1.5		_	-		0	0		0		0					
41	1.2 Dead + 1.5 Live	_	<u>Y</u>		1			1.5		.058	_		0	0		0		0					
42	1.2 Dead + 1.5 Live		<u>Y</u>		1	1.2		1.5		.058			0	0		0		0					
43	1.2 Dead + 1.5 Live		Y 		1	1.2				.058			0	0		0		0					
44	1.2 Dead + 1.5 Live	_			1	1.2		1.5		.058			0	0		0		0					
45	1.2 Dead + 1.5 Live	_	<u>Y</u>		1	1.2		1.5		.058	_		0	0		0		0					
46			Y 		1					.058			0	0		0		0					
47	1.2 Dead + 1.5 Live 1.2 Dead + 1.5 Live		<u> Ү</u> Ү		1	1.2				.058			0	0		0		0					
48	1.2 Dead + 1.5 Live		<u>т</u> Ү		1	1.2				.058			0	0		0		0					
49	1.2 Dead + 1.5 Live	-	<u> </u>		1									0		0							
	1.2 Dead + 1.5 Live		<u> Ү</u> Ү		1					.058			0	0		0		0					
51	1.2 Dead + 1.5 Live		<u> Ү</u> Ү		<u> </u>					.058			0	0		0		0					
			<u> Ү</u> Ү		1					.058			0	0		0		0					
54	_		<u>ү</u> Ү		1					.058			0	0		0		0					
55		_	<u>т</u> Ү		1			1.5					0	0		0		0					
56			Y		1					.058			0	0		0		0					
57	1.2 Dead + 1.5 Live		<u> Ү</u> Ү		1					.058			0	0		0		0					
58		_	Y		1					.058			0	0		0		0					
59			Y		1					.058			0	0		0		0					
60		-	Y		1					.058			0	0		0		0					
61	1.2 Dead + 1.5 Live		Y		1					.058			0	0		0		0					
	1.2 Dead + 1.5 Live		Y		1					.058			0	0		0		0					
	1.2 Dead + 1.5 Live		Y		1					.058			0	0		0		0					
	1.2 Dead + 1.5 Live		Y		1					.058			0	0		0		0					
65			Y		1					.058			0	0		0		0					
	1.2 Dead + 1.5 Live		Ϋ́		1					.058			0	0		0		0					
						1.4	120	1.0			J												



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Load Combinations (Continued)

Description S	PDelta	S B Fa	. B Fa E	3 Fa B Fa	. B Fa	. B Fa	. B Fa.	B Fa	В	Fa	<u>—</u> В	Fa
67 1.2 Dead + 1.5 Live Yes	Υ	1 1.2	29 1.5	7 .058 0	0	0	0	0				
68 1.2 Dead + 1.5 Live Yes	Υ	1 1.2	29 1.5	8 .058 0	0	0	0	0				
69 1.2 Dead + 1.5 Live Yes	Υ	1 1.2	29 1.5	9 .058 0	0	0	0	0				
70 1.2 Dead + 1.5 Live Yes	Υ	1 1.2	29 1.5	10 .058 0	0	0	0	0				
71 1.2 Dead + 1.5 Live Yes	Υ	1 1.2	29 1.5	11 .058 0	0	0	0	0				
72 1.2 Dead + 1.5 Live Yes	Υ	1 1.2	29 1.5	12 .058 0	0	0	0	0				
73 1.2 Dead + 1.5 Live Yes	Υ	1 1.2	29 1.5	13 .058 0	0	0	0	0				
74 1.2 Dead + 1.5 Live Yes	Υ			2 .058 0	0	0	0	0				
75 1.2 Dead + 1.5 Live Yes	Υ			3 .058 0	0	0	0	0				
76 1.2 Dead + 1.5 Live Yes	Υ			4 .058 0	0	0	0	0				
77 1.2 Dead + 1.5 Live Yes	Υ	1 1.2	30 1.5	5 .058 0	0	0	0	0				
78 1.2 Dead + 1.5 Live Yes	Υ	1 1.2	30 1.5	6 .058 0	0	0	0	0				
79 1.2 Dead + 1.5 Live Yes	Υ		30 1.5		0	0	0	0				
80 1.2 Dead + 1.5 Live Yes	Υ			8 .058 0	0	0	0	0				
81 1.2 Dead + 1.5 Live Yes	Υ			9 .058 0	0	0	0	0				
82 1.2 Dead + 1.5 Live Yes	Υ			10 .058 0	0	0	0	0				
83 1.2 Dead + 1.5 Live Yes	Y			11 .058 0	0	0	0	0				
84 1.2 Dead + 1.5 Live Yes	Υ			12 .058 0	0	0	0	0				
85 1.2 Dead + 1.5 Live Yes	<u> </u>			13 .058 0	0	0	0	0			$ \bot $	
86 1.2 Dead + 1.5 Live Yes	Υ	1 1.2		2 .058 0	0	0	0	0			\Box	
87 1.2 Dead + 1.5 Live Yes	Y			3 .058 0	0	0	0	0			_	
88 1.2 Dead + 1.5 Live Yes	Υ			4 .058 0	0	0	0	0				
89 1.2 Dead + 1.5 Live Yes	<u>Y</u>		31 1.5		0	0	0	0			_	
90 1.2 Dead + 1.5 Live Yes	Y			6 .058 0	0	0	0	0				
91 1.2 Dead + 1.5 Live Yes	Y	1 1.2		7 .058 0	0	0	0	0			_	
92 1.2 Dead + 1.5 Live Yes	Y			8 .058 0	0	0	0	0				
93 1.2 Dead + 1.5 Live Yes	<u>Y</u>			9 .058 0	0	0	0	0			_	
94 1.2 Dead + 1.5 Live Yes	<u>Y</u>			10 .058 0	0	0	0	0				
95 1.2 Dead + 1.5 Live Yes	<u>Y</u>			11 .058 0	0	0	0	0			_	
96 1.2 Dead + 1.5 Live Yes	Y			12 .058 0	0	0	0	0				
97 1.2 Dead + 1.5 Live Yes	<u>Y</u>			13 .058 0	0	0	0	0			\rightarrow	
98 1.2 Dead + 1.5 Live Yes	<u>Y</u>			2 .058 0	0	0	0	0			-	
99 1.2 Dead + 1.5 Live Yes	Y		32 1.5		0	0	0	0				
100 1.2 Dead + 1.5 Live Yes	<u> Ү</u> Ү			4 .058 0	0	0	0	0				
101 1.2 Dead + 1.5 Live Yes	<u> Ү</u> Ү	1 1.2		5 .058 0 6 .058 0	0	0	0	0			\rightarrow	
103 1.2 Dead + 1.5 Live Yes					0	0	0	0			-	
104 1.2 Dead + 1.5 Live Yes					0	0	0					
105 1.2 Dead + 1.5 Live Yes	<u> Ү</u> Ү			9 .058 0	0	0	0	0			\rightarrow	
106 1.2 Dead + 1.5 Live Yes	<u>т</u> Ү			10 .058 0	0	0	0	0				
107 1.2 Dead + 1.5 Live Yes	Y			11 .058 0				0			-	
108 1.2 Dead + 1.5 Live Yes	<u>T</u>			12.058 0	0	0	0	0				
109 1.2 Dead + 1.5 Live Yes	<u> </u>			13.058 0	0	0	0	0			\dashv	
110 1.2 Dead + 1.5 Live Yes	<u>T</u>			2 .058 0	0	0	0	0				
111 1.2 Dead + 1.5 Live Yes				3 .058 0	0	0	0	0			\dashv	
112 1.2 Dead + 1.5 Live Yes				4 .058 0	0	0	0	0				
113 1.2 Dead + 1.5 Live Yes	Ϋ́			5 .058 0	0	0	0	0				
114 1.2 Dead + 1.5 Live Yes	Y			6 .058 0	0	0	0	0				
115 1.2 Dead + 1.5 Live Yes	Ÿ			7 .058 0	0	0	0	0			\neg	
116 1.2 Dead + 1.5 Live Yes	Ÿ			8 .058 0	0	0	0	0				
117 1.2 Dead + 1.5 Live Yes	Ϋ́			9 .058 0	0	0	0	0			\neg	
118 1.2 Dead + 1.5 Live Yes	Y			10 .058 0	0	0	0	0				
119 1.2 Dead + 1.5 Live Yes	Ϋ́			11 .058 0	0	0	0	0			\neg	
120 1.2 Dead + 1.5 Live Yes	Y			12.058 0	0	0	0	0				
121 1.2 Dead + 1.5 Live Yes				13 .058 0	0	0	0	0			\neg	
122 1.2 Dead + 1.5 Live Yes				2 .058 0	0	0	0	0				
123 1.2 Dead + 1.5 Live Yes	Y			3 .058 0	0	0	0	0				



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Load Combinations (Continued)

Description S PDelta S B Fa B Fa B	Fa B Fa	. B Fa B.	FaB Fa R	Fa B F	_
			u D i u D.	ı a D ı	-a
124 1.2 Dead + 1.5 Live Yes Y 1 1 1.2 34 1.5 4 .058 0	0 0	0)		
125 1.2 Dead + 1.5 Live Yes Y 1 1.2 34 1.5 5 .058 0		0 0	0		
126 1.2 Dead + 1.5 Live Yes Y 1 1.2 34 1.5 6 .058 0		0 0			
1 1 1 1 1 1 1 1		0 0			
128 1.2 Dead + 1.5 Live Yes Y 1 1.2 34 1.5 8 .058 0		0 0			
129 1.2 Dead + 1.5 Live Yes Y 1 1.2 34 1.5 9 .058 0	0 0	0 0	0		
130 1.2 Dead + 1.5 Live Yes Y 1 1.2 34 1.5 10 .058 0) 0	0 0)		
131 1.2 Dead + 1.5 Live Yes Y 1 1.2 34 1.5 11 .058 0	0 0	0 0	0		
132 1.2 Dead + 1.5 Live Yes Y 1 1.2 34 1.5 12 .058 0		0 0	0		
133 1.2 Dead + 1.5 Live Yes Y 1 1.2 34 1.5 13 .058 0		0 0			\neg
134 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 2 .058 0		0 0			
		0 0			
136 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 4 .058 0		0 0			
137 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 5 .058 0		0 0			
138 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 6 .058 0	0	0 0	0		
139 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 7 .058 0	0 0	0 0)		
140 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 8 .058 0		0 0	0		
141 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 9 .058 0		0 0			
142 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 10 .058 0		0 0			
143 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 11 .058 0		0 0			
		0 0			
145 1.2 Dead + 1.5 Live Yes Y 1 1.2 35 1.5 13 .058 0		0 0			
146 1.2 Dead + 1.5 Live_VYes Y 1 1.2 36 1.5 0 0		0 0			
147 1.2 Dead + 1.5 Live_VYes Y 1 1.2 37 1.5 0 0	0 0	0 0	0		
148 1.2 Dead + 1.5 Live_VYes	0 0	0 0)		
149 1.2 Dead + 1.5 Live_VYes Y 1 1.2 39 1.5 0 0	0	0 0	0		
150 1.2 Dead + 1.5 Live_VYes Y 1 1.2 40 1.5 0 0		0 0			
151 1.2 Dead + 1.5 Live_VYes Y 1 1.2 41 1.5 0 0		0 0		1 1 1	
152 1.2 Dead + 1.5 Live_VYes Y 1 1.2 42 1.5 0 0		0 0			
		0 0			
154 1.2 Dead + 1.5 Live_VYes Y 1 1.2 44 1.5 0 0		0 0			
155 1.2 Dead + 1.5 Live_VYes Y 1 1.2 45 1.5 0 0	0	0 0	0		
156 1.2 Dead + 1.5 Live_VYes Y 1 1.2 46 1.5 0 0	0	0 0	0 0		
157 1.2 Dead + 1.5 Live_VYes Y	0	0 0	0		
158 1.2 Dead + 1.5 Live_VYes Y 1 1.2 48 1.5 0 0	0	0 0	0		
159 1.2 Dead + 1.5 Live_VYes Y 1 1.2 49 1.5 0 0		0 0			
160 1.2 Dead + 1.5 Live_VYes Y 1 1.2 50 1.5 0 0		0 0			
		0 0		+ + +	
		0 0			
163 1.2 Dead + 1.5 Live_VYes Y 1 1.2 53 1.5 0 0		0 0			
164 1.2 Dead + 1.5 Live_VYes Y 1 1.2 54 1.5 0 0		0 0			
165 1.2 Dead + 1.5 Live_VYes Y 1 1.2 55 1.5 0 0	0	0 0	0		
166 1.2 Dead + 1.5 Live_VYes Y 1 1.2 56 1.5 0 0	0	0 0	0		
167 1.2 Dead + 1.5 Live_VYes Y 1 1.2 57 1.5 0 0	0	0 0			
168 1.2 Dead + 1.5 Live_VYes Y 1 1.2 58 1.5 0 0		0 0			
169 1.2 Dead + 1.5 Live_VYes Y 1 1.2 59 1.5 0 0		0 0			
		0 0			
171 1.2 Dead + 1.5 Live_V. Yes Y 1 1.2 61 1.5 0 0		0 0			
172 1.2 Dead + 1.5 Live_VYes Y 1 1.2 62 1.5 0 0		0 0			
173 1.2 Dead + 1.5 Live_VYes Y 1 1.2 63 1.5 0 0		0 0			
174 1.2 Dead + 1.5 Live_VYes Y 1 1.2 64 1.5 0 0	0	0 0			
175 1.2 Dead + 1.5 Live_VYes Y 1 1.2 65 1.5 0 0	0	0 0	0		
176 1.2 Dead + 1.5 Live_VYes Y 1 1.2 66 1.5 0 0		0 0			
177 1.2 Dead + 1.5 Live_VYes Y 1 1.2 67 1.5 0 0		0 0			
178 1.2 Dead + 1.5 Live_VYes Y 1 1.2 68 1.5 0 0		0 0			
		0 0			
180 1.2 Dead + 1.5 Live_VYes Y 1 1.2 70 1.5 0 0	0 0	0 0	0 0		



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Load Combinations (Continued)

	Description	S	PDelta	S I	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa B	Fa	В	Fa	В	Fa	В	Fa	В	Fa
181	1.2 Dead + 1.5 Live_V	Yes	Υ		1	1.2	71	1.5	0		0		0	0		0		0					

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N7	max	1068.397	15	2961.665	37	2169.149	18	.328	175	1.759	7	7.705	37
2		min	-1495.412	2	650.211	13	-2165.446	7	498	167	-1.903	18	1.68	13
3	N49	max	2381.286	14	2961.81	29	1783.1	21	-1.373	15	1.906	15	75	3
4		min	-2170.576	3	644.728	21	-2155.456	8	-6.601	28	-2.052	2	-4.043	32
5	N91	max	2350.101	14	2960.706	32	2032.179	20	6.763	34	1.796	5	645	25
6		min	-2134.754	3	654.112	5	-1664.01	9	1.471	7	-1.937	16	-3.745	32
7	N99B	max	2328.606	26	1129.724	26	431.787	181	0	143	0	143	0	7
8		min	60.673	15	42.026	15	-871.408	179	0	167	0	167	0	34
9	N104A	max	35.329	167	1125.974	30	2171.73	30	0	173	0	63	0	26
10		min	-924.91	169	54.35	23	65.417	23	0	23	0	173	0	15
11	N107	max	-3.022	9	1126.024	35	-83.527	7	0	23	0	103	0	179
12		min	-1474.45	173	54.675	7	-1848.159	34	0	30	0	179	0	13
13	Totals:	max	4933.199	15	12047.433	28	4856.996	21						
14		min	-4933.204	2	2891.378	19	-4857.003	8						

Envelope AISC 15th(360-16): LRFD Steel Code Checks

	Member	Shape	Code	.Loc[LC	Shear	Loc[in]	Dir	LC	phi*Pncphi*Pntphi*Mnphi*MnCb Eqn
1	M23	L3X3X4	.574	84	32	.360	84	y	36	3944.532 46656 1.688 2.744 1 H2-1
2	M45	L3X3X4	.572	84	36	.361	84	y	29	3944.532 46656 1.688 2.731 1 H2-1
3	M1	L3X3X4	.571	84	28	.362	84	У	32	3944.532 46656 1.688 2.74 1 H2-1
4	B1	P2Std	.566	54	167	.236	42		30	22066 33862.5 1.998 1.998 1 H1-1b
5	C1	P2Std	.566	54	173	.237	42		32	22066 33862.5 1.998 1.998 1 H1-1b
6	A1	P2Std	.566	54	179	.237	42		26	22066 33862.5 1.998 1 H1-1b
7	A2	P2Std	.520	42	2	.146	42		167	22066 33862.5 1.998 1 H1-1b
8	B2	P2Std	.508	42	10	.146	42		173	22066 33862.5 1.998 1.998 1 H1-1b
9	C2	P2Std	.507	42	18	.145	42		179	22066 33862.5 1.998 1.998 1 H1-1b
10	M71	P2Std	.495	48.9	173	.269	48.938		173	4969.533 33862.5 1.998 1.998 2 H3-6
11	M69	P2Std	.495	48.9	167	.269	48.937		167	4969.533 33862.5 1.998 1.998 2 H3-6
12	M73	P2Std	.495	48.9	179	.269	48.938		179	4969.533 33862.5 1.998 1.998 2 H3-6
13	M49	HSS4x4x1	.471	0	32	.089	0	У	36	154502 155250 18.22 18.22 1 H1-1b
14	M27	HSS4x4x1	.471	0	29	.090	0	У	32	154502 155250 18.22 18.22 1 H1-1b
15	M5	HSS4x4x1	.467	0	36	.090	0	y	28	154502 155250 18.22 18.22 1 H1-1b
16	M46	L3X3X4	.434	44.1	32	.094	88.326	Z	35	14270 46656 1.688 3.273 1 H2-1
17	M47B	P2Std	.432	48.9	167	.191	48.937		167	4969.533 33862.5 1.998 1.998 2 H1-1b
18	M48B	P2Std	.432	48.9	173	.191	48.938		173	4969.533 33862.5 1.998 1.998 2 H1-1b
19	M52	P2Std	.432	48.9	179	.191	48.938		179	4969.533 33862.5 1.998 1.998 2 H1-1b
20	M24	L3X3X4	.431	44.1	29	.087	88.326	Z	31	14270 46656 1.688 3.275 1 H2-1
21	M2	L3X3X4	.429	44.1	36	.093	88.326	Z	27	14270 46656 1.688 3.273 1 H2-1
22	C3	P2Std	.366	54	175	.111	42		175	22066 33862.5 1.998 1.998 1 H1-1b
23	B3	P2Std	.366	54	169	.111	42		169	22066 33862.5 1.998 1 H1-1b
24	A3	P2Std	.366	54	181	.111	42		181	22066 33862.5 1.998 1 H1-1b
25	M57	L2.5x2.5x4	.340	18.1	173	.142	0	У	173	35790 38556 1.114 2.537 1 H2-1
26	M22	L2.5x2.5x4	.340	18.1	179	.142	0	У	179	35790 38556 1.114 2.537 1 H2-1
27	M62	L2.5x2.5x4	.340	18.1	167	.110	0	У	167	35790 38556 1.114 2.537 1 H2-1
28	M61	L2.5x2.5x4	.340	18.1	179	.111	0	У	179	35790 38556 1.114 2.537 1 H2-1
29	M24A	L2.5x2.5x4	.340	18.1	167	.142	0	ý	167	35790 38556 1.114 2.537 1 H2-1
30	M75	L2.5x2.5x4	.340	18.1	173	.111	0	y	173	35790 38556 1.114 2.537 1 H2-1
31	M4	HSS4-1/2x	.278	1	29	.101	1	ý	29	13224213390318.057 18.057 1 H1-1b
32	M48	HSS4-1/2x	.277	1	37	.101	11	У	37	13224213390318.057 18.057 1 H1-1b
33	M26	HSS4-1/2x	.276	0	33	.101	1	y	33	132242 133903 18.057 18.057 1 H1-1b

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Envelope AISC 15th(360-16): LRFD Steel Code Checks (Continued)

	Member	Shape	Code	.Loc[LC	Shear	. Loc[in]	Dir	LC	phi*Pncphi*Pnt	phi*Mn	.phi*Mn	<u>.Cb Eqn</u>
34	M55	L2.5x2.5x3	.143	27.3	26	.008	54.672	y	35	14829 29192.4	.873	1.659	1 H2-1
35	M59	L2.5x2.5x3	.143	27.3	35	.008	0	y	31	14829 29192.4	.873	1.659	1 H2-1
36	M57A	L2.5x2.5x3	.143	27.3	30	.008	0	у	26	14829 29192.4	.873	1.659	1 H2-1
37	M56	L2.5x2.5x3	.110	27.3	26	.008	0	y	167	14829 29192.4	.873	1.659	1 H2-1
38	M58	L2.5x2.5x3	.110	27.3	29	.008	54.672	y	173	14829 29192.4	.873	1.659	1 H2-1
39	M60	L2.5x2.5x3	.110	27.3	34	.008	54.672	у	179	14829 29192.4	.873	1.659	1 H2-1
40	M52A	L3X3X4	.105	23	8	.022	0	у	29	43009 46656	1.688	3.756	2 H2-1
41	M50	L3X3X4	.095	23	2	.021	0	у	26	43009 46656	1.688	3.756	1 H2-1
42	M54	L3X3X4	.093	23	16	.021	0	у	34	43009 46656	1.688	3.756	2 H2-1
43	M53	L3X3X4	.073	23	20	.011	0	Z	179	43009 46656	1.688	3.756	2 H2-1
44	M25	LL3x3x4x3	.068	45.0	26	.018	0	у	167	75234 93312	7.427	4.379	1H1-1b
45	M47	LL3x3x4x3	.068	46	29	.018	0	y	173	75234 93312	7.427	4.379	2H1-1b
46	M3	LL3x3x4x3	.066	42.1	34	.018	0	у	179	75234 93312	7.427	4.379	1H1-1b
47	M49A	L3X3X4	.064	23	4	.011	0	у	167	43009 46656	1.688	3.756	2 H2-1
48	M51	L3X3X4	.063	23	12	.011	0	v	173	43009 46656	1.688	3.756	2 H2-1

Envelope AISC 15th(360-16): LRFD Steel Code Checks

	Member	Shape	Code Check Actual	Code Check Allowable	Ratio (Act./Allow.)	Loc[in]	LC	Shear Check	Shear Check Allowable	Ratio (Act./Allow.)	Loc[in]	phi*Pnc [lb]	phi*Pnt [lb]	phi*Mn y-y [k-ft]	phi*Mn z-z [k-ft]	Cb	Eqn
1	M23	L3X3X4	0.574	1.05	0.547*	84	32	0.36	1.05	0.343*	84	3944.532	46656	1.688	2.744	1.646	H2-1
2	M45	L3X3X4	0.572	1.05	0.545*	84	36	0.361	1.05	0.344*	84	3944.532	46656	1.688	2.731	1.625	H2-1
3	M1	L3X3X4	0.571	1.05	0.544*	84	28	0.362	1.05	0.345*	84	3944.532	46656	1.688	2.74	1.639	H2-1
4	B1	P2Std	0.566	1.05	0.539*	54	167	0.236	1.05	0.225*	42	22066.014	33862.5	1.998	1.998	1.429	H1-1b
5	C1	P2Std	0.566	1.05	0.539*	54	173	0.237	1.05	0.226*	42	22066.014	33862.5	1.998	1.998	1.429	H1-1b
6	A1	P2Std	0.566	1.05	0.539*	54	179	0.237	1.05	0.226*	42	22066.014	33862.5	1.998	1.998	1.429	H1-1b
7	A2	P2Std	0.52	1.05	0.495*	42	2	0.146	1.05	0.139*	42	22066.014	33862.5	1.998	1.998	1.911	H1-1b
8	B2	P2Std	0.508	1.05	0.484*	42	10	0.146	1.05	0.139*	42	22066.014	33862.5	1.998	1.998	1.909	H1-1b
9	C2	P2Std	0.507	1.05	0.483*	42	18	0.145	1.05	0.138*	42	22066.014	33862.5	1.998	1.998	1.909	H1-1b
10	M71	P2Std	0.495	1.05	0.471*	48.94	173	0.269	1.05	0.256*	48.938	4969.533	33862.5	1.998	1.998	2.178	H3-6
11	M69	P2Std	0.495	1.05	0.471*	48.94	167	0.269	1.05	0.256*	48.937	4969.533	33862.5	1.998	1.998	2.178	H3-6
12	M73	P2Std	0.495	1.05	0.471*	48.94	179	0.269	1.05	0.256*	48.938	4969.533	33862.5	1.998	1.998	2.178	H3-6
13	M49	HSS4x4x1/4_HRA	0.471	1.05	0.449*	0	32	0.089	1.05	0.085*	0	154502.105	155250	18.22	18.22	1.199	H1-1b
14	M27	HSS4x4x1/4_HRA	0.471	1.05	0.449*	0	29	0.09	1.05	0.086*	0	154502.105	155250	18.22	18.22	1.199	H1-1b
15	M5	HSS4x4x1/4_HRA	0.467	1.05	0.445*	0	36	0.09	1.05	0.086*	0	154502.105	155250	18.22	18.22	1.199	H1-1b
16	M46	L3X3X4	0.434	1.05	0.413*	44.16	32	0.094	1.05	0.09*	88.326	14270.535	46656	1.688	3.273	1.564	H2-1
17	M47B	P2Std	0.432	1.05	0.411*	48.94	167	0.191	1.05	0.182*	48.937	4969.533	33862.5	1.998	1.998	2.252	H1-1b
18	M48B	P2Std	0.432	1.05	0.411*	48.94	173	0.191	1.05	0.182*	48.938	4969.533	33862.5	1.998	1.998	2.252	H1-1b
19	M52	P2Std	0.432	1.05	0.411*	48.94	179	0.191	1.05	0.182*	48.938	4969.533	33862.5	1.998	1.998	2.252	H1-1b
20	M24	L3X3X4	0.431	1.05	0.41*	44.16	29	0.087	1.05	0.083*	88.326	14270.535	46656	1.688	3.275	1.568	H2-1
21	M2	L3X3X4	0.429	1.05	0.409*	44.16	36	0.093	1.05	0.089*	88.326	14270.535	46656	1.688	3.273	1.564	H2-1
22	C3	P2Std	0.366	1.05	0.349*	54	175	0.111	1.05	0.106*	42	22066.014	33862.5	1.998	1.998	1.815	H1-1b
23	B3	P2Std	0.366	1.05	0.349*	54	169	0.111	1.05	0.106*	42	22066.014	33862.5	1.998	1.998	1.814	H1-1b
24	A3	P2Std	0.366	1.05	0.349*	54	181	0.111	1.05	0.106*	42	22066.014	33862.5	1.998	1.998	1.815	H1-1b
25	M57	L2.5x2.5x4	0.34	1.05	0.324*	18.12	173	0.142	1.05	0.135*	0	35790.473	38556	1.114	2.537	1.687	H2-1
26	M22	L2.5x2.5x4	0.34	1.05	0.324*	18.12	179	0.142	1.05	0.135*	0	35790.473	38556	1.114	2.537	1.688	H2-1
27	M62	L2.5x2.5x4	0.34	1.05	0.324*	18.12	167	0.11	1.05	0.105*	0	35790.473	38556	1.114	2.537	1.301	H2-1
28	M61	L2.5x2.5x4	0.34	1.05	0.324*	18.12	179	0.111	1.05	0.106*	0	35790.473	38556	1.114	2.537	1.301	H2-1
29	M24A	L2.5x2.5x4	0.34	1.05	0.324*	18.12	167	0.142	1.05	0.135*	0	35790.473	38556	1.114	2.537	1.688	H2-1
30	M75	L2.5x2.5x4	0.34	1.05	0.324*	18.12	173	0.111	1.05	0.106*	0	35790.473	38556	1.114	2.537	1.301	H2-1
31	M4	HSS4-1/2x4-1/2x3/16	0.278	1.05	0.265*	1	29	0.101	1.05	0.096*	1	132242.822	133903.1	18.057	18.057	1.711	H1-1b
32	M48	HSS4-1/2x4-1/2x3/16	0.277	1.05	0.264*	1	37	0.101	1.05	0.096*	1	132242.822	133903.1	18.057	18.057	1.711	H1-1b
33	M26	HSS4-1/2x4-1/2x3/16	0.276	1.05	0.263*	07.04	33	0.101	1.05	0.096*		132242.822	133903.1	18.057	18.057	1.711	H1-1b
34	M55 M59	L2.5x2.5x3 L2.5x2.5x3	0.143	1.05 1.05	0.136* 0.136*	27.34	26 35	0.008	1.05	0.008*	54.672	14829.276	29192.4 29192.4	0.873	1.659	1.136	H2-1
36		L2.5x2.5x3 L2.5x2.5x3	0.143 0.143	1.05	0.136*	27.34	30	0.008	1.05	0.008*	0	14829.276		0.873	1.659 1.659	1.136	H2-1
36	M57A M56					27.34	26		1.05		0	14829.276	29192.4	0.873 0.873	1.659	1.136	H2-1 H2-1
38	M58	L2.5x2.5x3 L2.5x2.5x3	0.11	1.05 1.05	0.105* 0.105*	27.34	29	0.008	1.05 1.05	0.008*	54.672	14829.276 14829.276	29192.4 29192.4	0.873	1.659	1.136	H2-1
						_											
39 40	M60 M52A	L2.5x2.5x3 L3X3X4	0.11 0.105	1.05 1.05	0.105* 0.1*	27.34	34	0.008	1.05	0.008* 0.021*	54.672	14829.276 43009.694	29192.4 46656	0.873 1.688	1.659 3.756	1.136	H2-1
40	M52A M50	L3X3X4 L3X3X4	0.105	1.05	0.15	23	2	0.022	1.05	0.021"	0	43009.694	46656		3.756	1.764	H2-1 H2-1
41	M54	L3X3X4 L3X3X4		1.05	0.09*	23		0.021	1.05	0.02*	0		46656	1.688	3.756	2.263	H2-1
43	M53	L3X3X4 L3X3X4	0.093	1.05	0.089*	23	16 20	0.021	1.05	0.02"	0	43009.694 43009.694	46656	1.688	3.756	2.263	H2-1 H2-1
43	M25	L3X3X4 LL3x3x4x3	0.073	1.05	0.07	45.04	26	0.011	1.05	0.017*	0	75234.506	93312	7.427	4.379	1.851	H1-1b
44	M47	LL3x3x4x3 LL3x3x4x3	0.068	1.05	0.065*	45.04	29	0.018	1.05	0.017*	0	75234.506	93312	7.427	4.379	2.064	H1-1b
46	M3	LL3X3X4X3 LL3X3X4X3	0.068	1.05	0.065*	42.17	34	0.018	1.05	0.017*	0	75234.506	93312	7.427	4.379	1.772	H1-1b
46	M49A	L3X3X4X3 L3X3X4	0.066	1.05	0.063*	23	4	0.018	1.05	0.017"	0		46656	1.688	3.756	2.3	H1-1b
48						23	12			0.01*	0	43009.694					H2-1
48	M51	L3X3X4	0.063	1.05	0.06*	23	12	0.011	1.05	0.01	U	43009.694	46656	1.688	3.756	2.303	ΠZ-1

APPENDIX D ADDITIONAL CALCULATIONS



TIA-222-H CONNECTION CHECK Mount to Tower Connection - Typ. All Sectors 2021777.842875.04

Bolt Information								
Bolt Diameter (d)	0.625	in						
Net Tensile Area (An)	0.226	in ²						
# of Bolts Total (n)	4							
Bolt Grade	A325N							
Bolt Tensile Strength (Fub)	120	ksi						

RISA 3D Reactions								
Moment (M)	0.00	k-ft						
Axial (T)	-2.33	kips						
Shear (V)	1.18	kips						

Bolt Capacity									
Nominal Tensile Strength (R _{nt})	27.120	kips							
Nominal Shear Strength (R _{nv})	18.41	kips							
Bolt Tensile Force (T _{ub})	-0.58	kips							
Bolt Shear Force (V _{ub})	0.294	kips							
$T_{ub}/\varphi R_{nt}$	-0.02862								
$V_{ub}/\varphi R_{nv}$	0.02128								
$(V_{ub}/\varphi R_{nv})^2 + (T_{ub}/\varphi R_{nt})^2$	0.00127								
Bolt Capacity =	2.1%	OK							

APPENDIX E

MOUNT MODIFICATION DESIGN DRAWINGS (MDD)

MOUNT DESIGN DRAWINGS PREPARED FOR CROWN CASTLE

SITE NAME: WINDSORDAY HILL

BU NUMBER: 842875

SITE ADDRESS:
99 DAY HILL ROAD
WINDSOR, CT 06095
HARTFORD COUNTY, USA

ATTENTION ALL CONTRACTORS, ANYTIME YOU ACCESS A CROWN SITE FOR ANY REASON YOU ARE TO CALL THE CROWN NOC UPON ARRIVAL AND DEPARTURE, DAILY AT 800-788-7011.

PERFORMED WORK SHALL NOT DAMAGE ANY EXISTING STRUCTURE, MOUNTS, SAFETY CLIMB, OR EQUIPMENT WHILE ON SITE. SHOULD DAMAGE OCCUR, CONTACT CROWN EOR AT EORAPPROVAL@CROWNCASTLE.COM



SAFETY CLIMB: 'LOOK UP'

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DRAWINGS INCLUDED

Di di tittii too ii	1020525
SHEET NUMBER	DESCRIPTION
S-1 S-2 S-3 S-4	TITLE PAGE GENERAL NOTES MOUNT MODIFICATION SCHEDULE DETAILS/PARTS

PROJECT CONTACTS:

1. CROWN PROJECT MANAGER:

TRICIA PELON (518) 373-3507 TRICIA.PELON@CROWNCASTLE.COM 3 CORPORATE PARK DRIVE, SUITE 101 CLIFTON PARK, NY 12065

2. ENGINEER OF RECORD:

GPD ENGINEERING AND ARCHITECTURE PROFESSIONAL CORPORATION 520 SOUTH MAIN STREET, SUITE 2531 AKRON, OH 44311 (330) 572-2100 FOR QUESTIONS PLEASE EMAIL: CROWNMODS@GPDGROUP.COM

TOWER INFORMATION

TOWER HEIGHT / TYPE: 168.0 FT MONOPOLE

TOWER LOCATION: LAT: 41° 52' 16.10" DATUM: (NAD 1983) LONG: -72° 40' 16.00"

WORK ORDER #: CCI/WO #: NA
ORDER #: 584557 REV #: 0

SITE ADDRESS: 99 DAY HILL ROAD

WINDSOR, CT 06095 HARTFORD COUNTY, USA

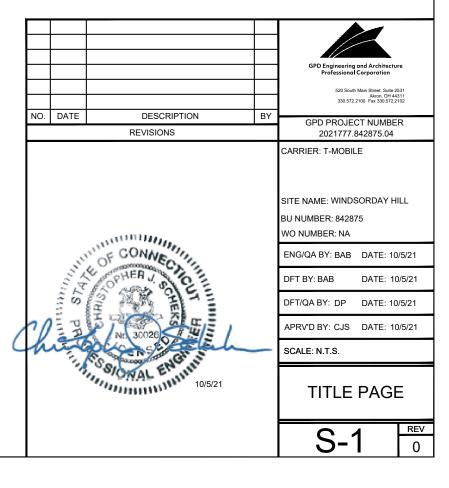
CODE COMPLIANCE

GOVERNING CODES: TIA-222-H

WIND SPEEDS: 125 MPH 3 SECOND GUST

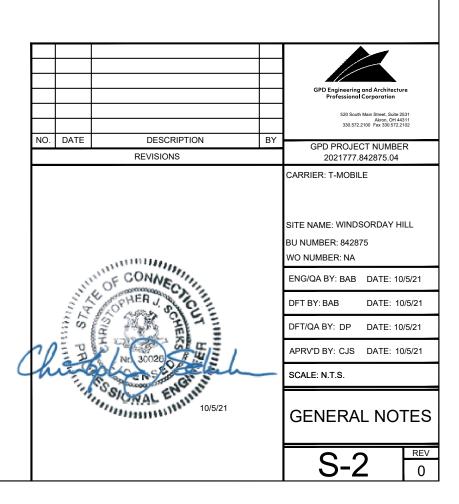
50 MPH 3 SECOND GUST (W/ ICE)

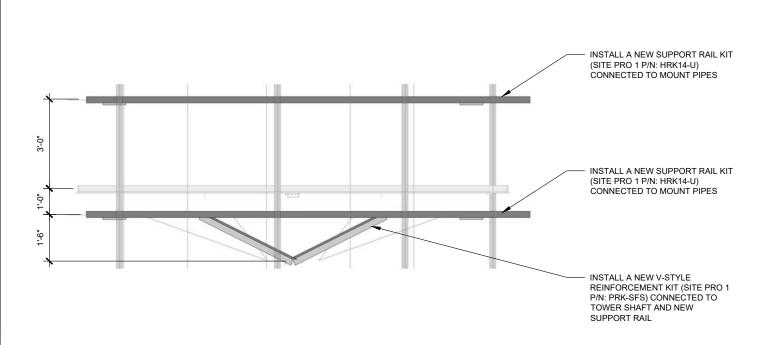
ICE THICKNESS: 2 IN
RISK CATEGORY: II
EXPOSURE CATEGORY: C
TOPO CATEGORY: 1



GENERAL NOTES

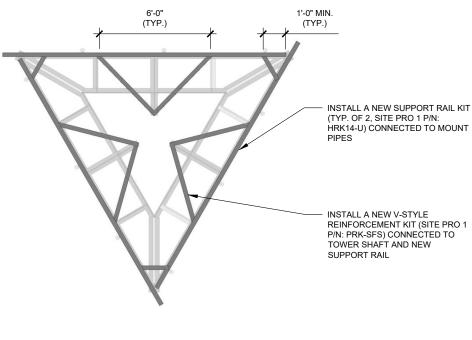
- DETAILED DRAWINGS AND NOTES SHALL GOVERN GENERAL NOTES AND TYPICAL DETAILS. CONTACT VENDOR POINT OF CONTACT (POC) AND ENGINEER OF RECORD (EOR) FOR CLARIFICATION AS NEEDED.
- DO NOT SCALE DRAWINGS.
- 3. FOR THIS MODIFICATION, THE TOWER AND MOUNT HAS BEEN ASSUMED TO BE IN GOOD CONDITION WITHOUT ANY STRUCTURAL DEFECTS, UNO. IF THE GC DISCOVERS ANY INDICATION OF AN EXISTING STRUCTURAL DEFECT, CONTACT THE CROWN POC AND EOR IMMEDIATELY.
- 4. ALL MANUFACTURER'S HARDWARE ASSEMBLY INSTRUCTIONS SHALL BE FOLLOWED, UNO. CONFLICTING NOTES SHALL BE BROUGHT TO THE ATTENTION OF THE EOR AND THE POC.
- 5. CONTRACTOR PERSONNEL SHALL NOT DRILL HOLES IN ANY NEW OR EXISTING STRUCTURAL MEMBERS, OTHER THAN THOSE DRILLED HOLES SHOWN ON STRUCTURAL DRAWINGS, WITHOUT THE APPROVAL OF THE EOR.
- 6. ANY HARDWARE REMOVED FROM THE EXISTING MOUNT SHALL BE REPLACED WITH NEW HARDWARE OF EQUAL SIZE AND QUALITY, UNO. NO EXISTING FASTENERS SHALL BE REUSED.
- 7. ALL JOINTS USING ASTM A325 OR A490 BOLTS, U-BOLTS, V-BOLTS, AND THREADED RODS SHALL BE SNUG TIGHTENED, UNO.
- 8. A NUT LOCKING DEVICE SHALL BE INSTALLED ON ALL PROPOSED AND/OR REPLACED SNUG TIGHTENED ASTM A325 OR A490 BOLTS, U-BOLTS, V-BOLTS, AND THREADED RODS.
- 9. ALL JOINTS ARE BEARING TYPE CONNECTIONS UNO. IF NO BOLT LENGTH IS GIVEN IN THE BILL OF MATERIALS, THE CONNECTION MAY INCLUDE THREADS IN THE SHEAR PLANES, AND THE GC IS RESPONSIBLE FOR SIZING THE LENGTH OF THE BOLT.
- 10. IF ASTM A325 OR A490 BOLTS, AND/OR THREADED RODS ARE SPECIFIED TO BE PRE-TENSIONED, THESE SHALL BE INSTALLED AND TIGHTENED TO THE PRE-TENSIONED CONDITION ACCORDING TO THE REQUIREMENTS OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM HIGH STRENGTH BOLTS.
- 11. ALL PROPOSED AND/OR REPLACED BOLTS SHALL BE OF SUFFICIENT LENGTH SUCH THAT THE END OF THE BOLT BE AT LEAST FLUSH WITH THE FACE OF THE NUT. IT IS NOT PERMITTED FOR THE BOLT END TO BE BELOW THE FACE OF THE NUT AFTER TIGHTENING IS COMPLETED.







 $\frac{\text{NOTE:}}{\text{1. DETAIL IS TYPICAL OF ALL (3) SECTORS, ONLY}}$ ONE SECTOR SHOWN FOR CLARITY.



2	PLAN VIEW
S-3	

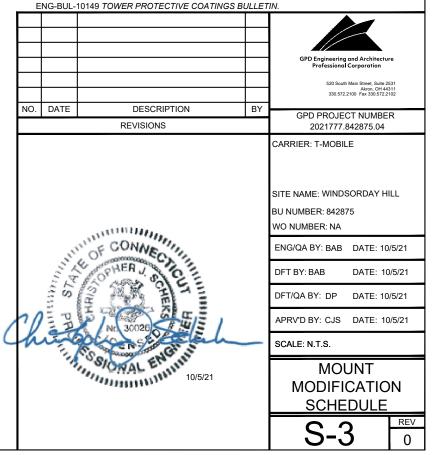
	MOUNT MODIFICATION SCHEDULE					
	ELEVATION (FT) MODIFICATION REFERENCE SHE					
Α	130.0	INSTALL A NEW SUPPORT RAIL KIT ABOVE PLATFORM CONNECTED TO MOUNT PIPES.	S-3 & S-4			
В	130.0	INSTALL A NEW SUPPORT RAIL KIT BELOW PLATFORM CONNECTED TO MOUNT PIPES.	S-3 & S-4			
C	130.0	INSTALL A NEW V-STYLE REINFORCEMENT KIT CONNECTED TO TOWER SHAFT AND NEW LOWER SUPPORT RAIL.	S-3 & S-4			

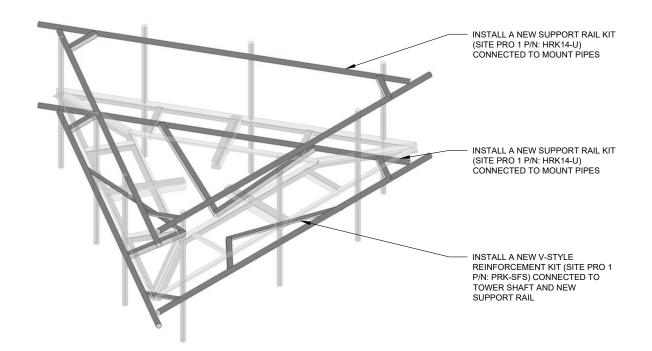
PRIOR TO FABRICATION AND INSTALLATION, CONTRACTOR SHALL FIELD VERIFY ALL LENGTHS AND QUANTITIES GIVEN. LENGTH AND QUANTITIES PROVIDED ARE FOR QUOTING PURPOSES ONLY AND SHALL NOT BE USED FOR FABRICATION.

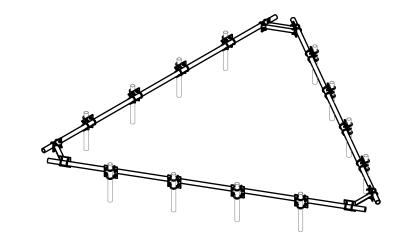
- ANY SUBSTITUTION OF PARTS SPECIFIED IN THIS DESIGN PACKAGE SHALL REQUIRE ENGINEER APPROVAL PRIOR TO FABRICATION.
 ALL MATERIAL REMOVED FROM MOUNT SHALL BE DISPOSED OF BY CONTRACTOR OFF SITE.

- NOTES:

 1. ALL PIPE TO BE ASTM A53 GRADE B (Fy = 35 KSI) MATERIAL. MATERIAL TEST REPORTS ARE REQUIRED.
- 2. ALL EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED PER ASTM A153 / A153M OR A123, AS APPLICABLE. FIELD DRILLED OR CUT MATERIAL TO BE COATED WITH TWO BRUSH COATS OF CROWN APPROVED ZINC RICH PAINT IN ACCORDANCE WITH

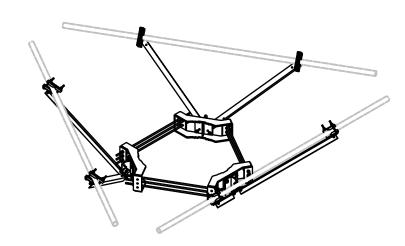






3 ISOMETRIC VIEW

4 HRK14-U SUPPORT RAIL KIT





5 PRK-SFS V-STYLE REINFORCEMENT KIT

MOUNT DESIGN DRAWINGS PREPARED FOR CROWN CASTLE

SITE NAME: WINDSORDAY HILL

BU NUMBER: 842875

SITE ADDRESS:
99 DAY HILL ROAD
WINDSOR, CT 06095
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Di di tittii too ii	1020525
SHEET NUMBER	DESCRIPTION
S-1 S-2 S-3 S-4	TITLE PAGE GENERAL NOTES MOUNT MODIFICATION SCHEDULE DETAILS/PARTS

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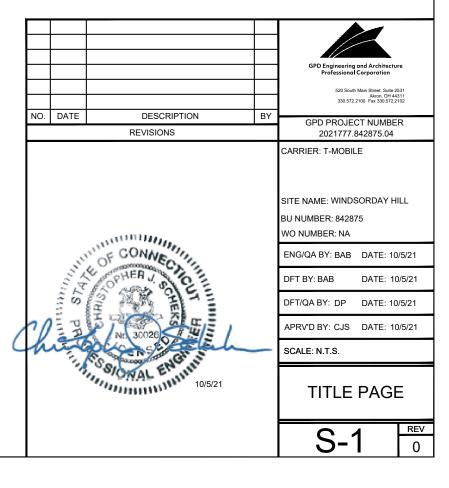
CODE COMPLIANCE

GOVERNING CODES: TIA-222-H

WIND SPEEDS: 125 MPH 3 SECOND GUST

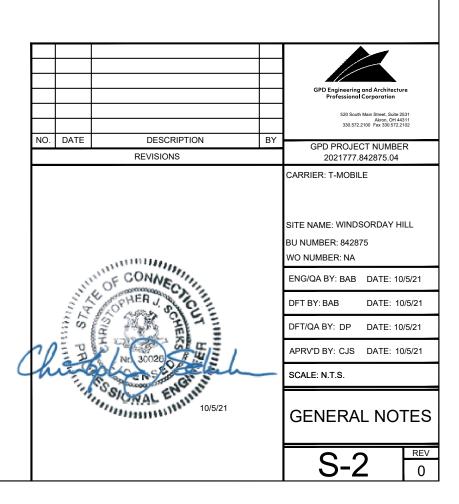
50 MPH 3 SECOND GUST (W/ ICE)

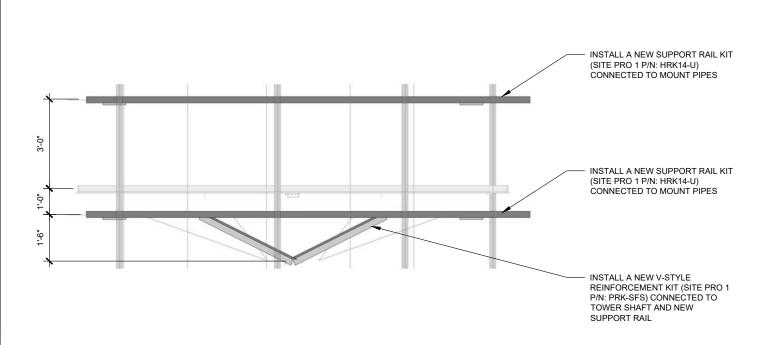
ICE THICKNESS: 2 IN
RISK CATEGORY: II
EXPOSURE CATEGORY: C
TOPO CATEGORY: 1



GENERAL NOTES

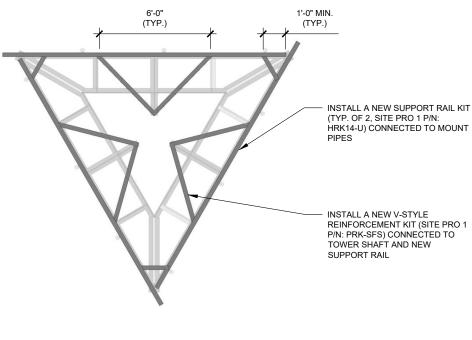
- DETAILED DRAWINGS AND NOTES SHALL GOVERN GENERAL NOTES AND TYPICAL DETAILS. CONTACT VENDOR POINT OF CONTACT (POC) AND ENGINEER OF RECORD (EOR) FOR CLARIFICATION AS NEEDED.
- DO NOT SCALE DRAWINGS.
- 3. FOR THIS MODIFICATION, THE TOWER AND MOUNT HAS BEEN ASSUMED TO BE IN GOOD CONDITION WITHOUT ANY STRUCTURAL DEFECTS, UNO. IF THE GC DISCOVERS ANY INDICATION OF AN EXISTING STRUCTURAL DEFECT, CONTACT THE CROWN POC AND EOR IMMEDIATELY.
- 4. ALL MANUFACTURER'S HARDWARE ASSEMBLY INSTRUCTIONS SHALL BE FOLLOWED, UNO. CONFLICTING NOTES SHALL BE BROUGHT TO THE ATTENTION OF THE EOR AND THE POC.
- 5. CONTRACTOR PERSONNEL SHALL NOT DRILL HOLES IN ANY NEW OR EXISTING STRUCTURAL MEMBERS, OTHER THAN THOSE DRILLED HOLES SHOWN ON STRUCTURAL DRAWINGS, WITHOUT THE APPROVAL OF THE EOR.
- 6. ANY HARDWARE REMOVED FROM THE EXISTING MOUNT SHALL BE REPLACED WITH NEW HARDWARE OF EQUAL SIZE AND QUALITY, UNO. NO EXISTING FASTENERS SHALL BE REUSED.
- 7. ALL JOINTS USING ASTM A325 OR A490 BOLTS, U-BOLTS, V-BOLTS, AND THREADED RODS SHALL BE SNUG TIGHTENED, UNO.
- 8. A NUT LOCKING DEVICE SHALL BE INSTALLED ON ALL PROPOSED AND/OR REPLACED SNUG TIGHTENED ASTM A325 OR A490 BOLTS, U-BOLTS, V-BOLTS, AND THREADED RODS.
- 9. ALL JOINTS ARE BEARING TYPE CONNECTIONS UNO. IF NO BOLT LENGTH IS GIVEN IN THE BILL OF MATERIALS, THE CONNECTION MAY INCLUDE THREADS IN THE SHEAR PLANES, AND THE GC IS RESPONSIBLE FOR SIZING THE LENGTH OF THE BOLT.
- 10. IF ASTM A325 OR A490 BOLTS, AND/OR THREADED RODS ARE SPECIFIED TO BE PRE-TENSIONED, THESE SHALL BE INSTALLED AND TIGHTENED TO THE PRE-TENSIONED CONDITION ACCORDING TO THE REQUIREMENTS OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM HIGH STRENGTH BOLTS.
- 11. ALL PROPOSED AND/OR REPLACED BOLTS SHALL BE OF SUFFICIENT LENGTH SUCH THAT THE END OF THE BOLT BE AT LEAST FLUSH WITH THE FACE OF THE NUT. IT IS NOT PERMITTED FOR THE BOLT END TO BE BELOW THE FACE OF THE NUT AFTER TIGHTENING IS COMPLETED.







 $\frac{\text{NOTE:}}{\text{1. DETAIL IS TYPICAL OF ALL (3) SECTORS, ONLY}}$ ONE SECTOR SHOWN FOR CLARITY.



2	PLAN VIEW
S-3	

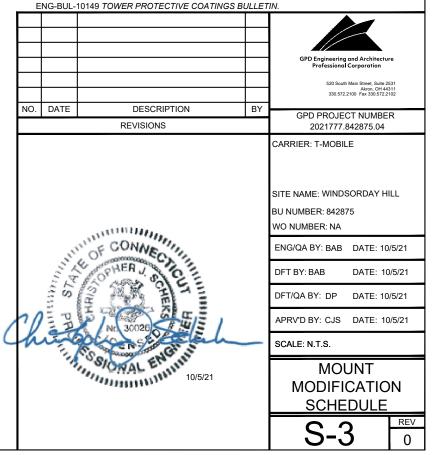
	MOUNT MODIFICATION SCHEDULE					
	ELEVATION (FT) MODIFICATION REFERENCE SHE					
Α	130.0	INSTALL A NEW SUPPORT RAIL KIT ABOVE PLATFORM CONNECTED TO MOUNT PIPES.	S-3 & S-4			
В	130.0	INSTALL A NEW SUPPORT RAIL KIT BELOW PLATFORM CONNECTED TO MOUNT PIPES.	S-3 & S-4			
C	130.0	INSTALL A NEW V-STYLE REINFORCEMENT KIT CONNECTED TO TOWER SHAFT AND NEW LOWER SUPPORT RAIL.	S-3 & S-4			

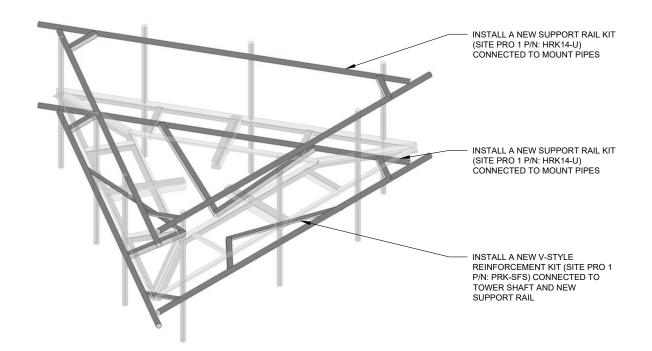
PRIOR TO FABRICATION AND INSTALLATION, CONTRACTOR SHALL FIELD VERIFY ALL LENGTHS AND QUANTITIES GIVEN. LENGTH AND QUANTITIES PROVIDED ARE FOR QUOTING PURPOSES ONLY AND SHALL NOT BE USED FOR FABRICATION.

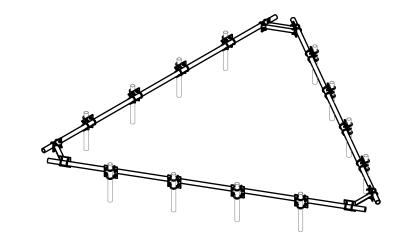
- ANY SUBSTITUTION OF PARTS SPECIFIED IN THIS DESIGN PACKAGE SHALL REQUIRE ENGINEER APPROVAL PRIOR TO FABRICATION.
 ALL MATERIAL REMOVED FROM MOUNT SHALL BE DISPOSED OF BY CONTRACTOR OFF SITE.

- NOTES:

 1. ALL PIPE TO BE ASTM A53 GRADE B (Fy = 35 KSI) MATERIAL. MATERIAL TEST REPORTS ARE REQUIRED.
- 2. ALL EXPOSED STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED PER ASTM A153 / A153M OR A123, AS APPLICABLE. FIELD DRILLED OR CUT MATERIAL TO BE COATED WITH TWO BRUSH COATS OF CROWN APPROVED ZINC RICH PAINT IN ACCORDANCE WITH

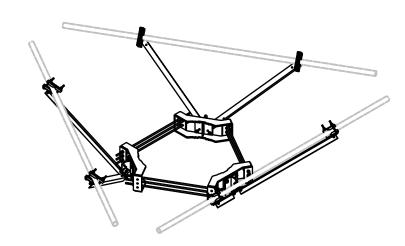






3 ISOMETRIC VIEW

4 HRK14-U SUPPORT RAIL KIT





5 PRK-SFS V-STYLE REINFORCEMENT KIT

Exhibit F

Power Density/RF Emissions Report



RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CTHA267A

CTHA267A 99 Day Hill Road Windsor, Connecticut 06095

November 11, 2021

EBI Project Number: 6221007048

Site Compliance Summary				
Compliance Status:	COMPLIANT			
Site total MPE% of FCC general population allowable limit:	20.28%			



November 11, 2021

T-Mobile Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, Connecticut 06002

Emissions Analysis for Site: CTHA267A - CTHA267A

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **99 Day Hill Road** in **Windsor, Connecticut** for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm²). The number of μ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μ W/cm²). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately 400 μ W/cm² and 467 μ W/cm², respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is 1000 μ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at 99 Day Hill Road in Windsor, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower. For power density calculations, the broadcast footprint of the AlR6449 antenna has been considered. Due to the beamforming nature of this antenna, the actual beam locations vary depending on demand and are narrow in nature. Using the broadcast footprint accounts for the potential location of beams at any given time.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 LTE channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) I NR channel (600 MHz Band) was considered for each sector of the proposed installation. This Channel has a transmit power of 80 Watts.
- 3) 2 LTE channels (700 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 4) 4 GSM channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 5) 2 LTE channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.



- 6) 2 LTE channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 7) I LTE Traffic channel (LTE IC and 2C BRS Band 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 60 Watts.
- 8) I LTE Broadcast channel (LTE IC and 2C BRS Band 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 20 Watts.
- 9) I NR Traffic channel (BRS Band 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of I20 Watts.
- 10) I NR Broadcast channel (BRS Band 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 40 Watts.
- 11) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 12) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 13) The antennas used in this modeling are the RFS APXVAALL24_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 2100 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s) in Sector A, the RFS APXVAALL24_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 2100 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 2100 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 1900 MHz / 1900 MHz / 2100 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied



specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

- 14) The antenna mounting height centerline of the proposed antennas is 131 feet above ground level (AGL).
- 15) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 16) All calculations were done with respect to uncontrolled / general population threshold limits.



T-Mobile Site Inventory and Power Data

Sector:	Α	Sector:	В	Sector:	С
Antenna #:	I	Antenna #:	ı	Antenna #:	I
Make / Model:	RFS APXVAALL24_43- U-NA20	Make / Model:	RFS APXVAALL24_43- U-NA20	Make / Model:	RFS APXVAALL24_43- U-NA20
Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 2100 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 2100 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 2100 MHz
Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd / 15.45 dBd / 15.45 dBd / 16.45 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd / 15.45 dBd / 15.45 dBd / 16.45 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd / 15.45 dBd / 15.45 dBd / 16.45 dBd
Height (AGL):	131 feet	Height (AGL):	131 feet	Height (AGL):	131 feet
Channel Count:	13	Channel Count:	13	Channel Count:	13
Total TX Power (W):	560 Watts	Total TX Power (W):	560 Watts	Total TX Power (W):	560 Watts
ERP (W):	17,868.72	ERP (W):	17,868.72	ERP (W):	17,868.72
Antenna A1 MPE %:	5.43%	Antenna B1 MPE %:	5.43%	Antenna CI MPE %:	5.43%
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Ericsson AIR 6449	Make / Model:	Ericsson AIR 6449	Make / Model:	Ericsson AIR 6449
Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz
Gain:	22.65 dBd / 17.3 dBd / 22.65 dBd / 17.3 dBd	Gain:	22.65 dBd / 17.3 dBd / 22.65 dBd / 17.3 dBd	Gain:	22.65 dBd / 17.3 dBd / 22.65 dBd / 17.3 dBd
Height (AGL):	I3I feet	Height (AGL):	I3I feet	Height (AGL):	I3I feet
Channel Count:	4	Channel Count:	4	Channel Count:	4
Total TX Power (W):	240 Watts	Total TX Power (W):	240 Watts	Total TX Power (W):	240 Watts
ERP (W):	36,356.09	ERP (W):	36,356.09	ERP (W):	36,356.09
Antenna A2 MPE %:	8.37%	Antenna B2 MPE %:	8.37%	Antenna C2 MPE %:	8.37%

Site Composite MPE %				
Carrier	MPE %			
T-Mobile (Max at Sector A):	13.79%			
Clearwire	0.08%			
AT&T	5.12%			
Metro PCS	0.93%			
Bloomfield PD	0.01%			
Muni Ant I	0.35%			
Site Total MPE % :	20.28%			

T-Mobile MPE % Per Sector					
T-Mobile Sector A Total: 13.79%					
T-Mobile Sector B Total:	13.79%				
T-Mobile Sector C Total:	13.79%				
Site Total MPE % : 20.28%					

T-Mobile Maximum MPE Power Values (Sector A)							
T-Mobile Frequency Band / Technology (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (μW/cm²)	Calculated % MPE
T-Mobile 600 MHz LTE	2	591.73	131.0	2.72	600 MHz LTE	400	0.68%
T-Mobile 600 MHz NR	I	1577.94	131.0	3.63	600 MHz NR	400	0.91%
T-Mobile 700 MHz LTE	2	695.22	131.0	3.20	700 MHz LTE	467	0.69%
T-Mobile 1900 MHz GSM	4	1052.26	131.0	9.68	1900 MHz GSM	1000	0.97%
T-Mobile 1900 MHz LTE	2	2104.51	131.0	9.68	1900 MHz LTE	1000	0.97%
T-Mobile 2100 MHz LTE	2	2649.42	131.0	12.19	2100 MHz LTE	1000	1.22%
T-Mobile 2500 MHz LTE IC & 2C Traffic	I	11044.63	131.0	25.41	2500 MHz LTE IC & 2C Traffic	1000	2.54%
T-Mobile 2500 MHz LTE IC & 2C Broadcast	I	1074.06	131.0	2.47	2500 MHz LTE I C & 2 C Broadcast	1000	0.25%
T-Mobile 2500 MHz NR Traffic	I	22089.26	131.0	50.83	2500 MHz NR Traffic	1000	5.08%
T-Mobile 2500 MHz NR Broadcast	I	2148.13	131.0	4.94	2500 MHz NR Broadcast	1000	0.49%
						Total:	13.79%

[•] NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.



Summary

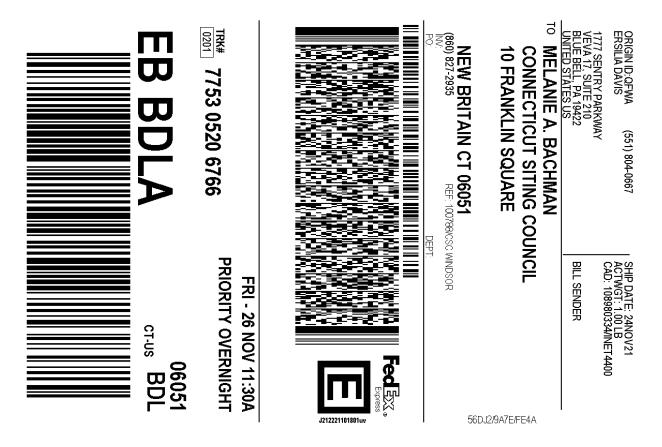
All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)		
Sector A:	13.79%		
Sector B:	13.79%		
Sector C:	13.79%		
T-Mobile Maximum	13.79%		
MPE % (Sector A):	13./7/0		
Site Total:	20.28%		
Site Compliance Status:	COMPLIANT		

The anticipated composite MPE value for this site assuming all carriers present is **20.28**% of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



After printing this label:

- 1. Use the 'Print' button on this page to print your label to your laser or inkjet printer.
- 2. Fold the printed page along the horizontal line.
- 3. Place label in shipping pouch and affix it to your shipment so that the barcode portion of the label can be read and scanned.

Warning: Use only the printed original label for shipping. Using a photocopy of this label for shipping purposes is fraudulent and could result in additional billing charges, along with the cancellation of your FedEx account number.

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