

56 Prospect Street, P.O. Box 270 Hartford, CT 06103

Kathleen M. Shanley Manager – Transmission Siting

Tel: (860) 728-4527

March 8, 2022

Melanie A. Bachman Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: Notice of Exempt Modification

Eversource Site # ES-055 Willimantic 349R Mountain Street, Windham, CT 06280

Latitude: 41-42-10.84 N / Longitude: 72-13-17.01 W

Dear Ms. Bachman:

The Connecticut Light and Power Company doing business as Eversource Energy ("Eversource") currently maintains multiple antennas at various mounting heights on an existing 196-foot self-support tower located at 349R Mountain Street in Windham. See Attachment A, Parcel Map and Property Card. The tower and property are owned by SBA Properties Inc., doing business as SBA Communications Corporation ("SBA"). Eversource and SBA have entered into an agreement allowing the modification of Eversource's equipment on the existing tower. See Attachment B, Letter of Authorization. Eversource is seeking the Connecticut Siting Council's authorization for the installation of one 15-foot 7-inch omnidirectional antenna to be mounted at 153 feet above ground level ("AGL") on a four-foot stand-off mount, and the removal of two 4-foot 3-inch omni-directional antennas and associated mounts. The antenna will be mounted to the tower on a new 4-foot stand-off mount. See Attachment C, Mount Analysis. There will be no changes to the area of the fenced compound, the tower or existing antennas and equipment mounted on the tower. The tower and existing and proposed equipment are depicted on Attachment D, Construction Drawings, dated February 28, 2022 and Attachment E, Structural Analysis, dated February 9, 2022. The Connecticut Siting Council approved Eversource's use of the tower at this location in Petition No. 910 in August 2009.

The modification is required to eliminate transmitter induced noise issues from two antennas previously installed as part of Eversource's program to update its obsolete analog voice radio communications system to a modern digital voice communications system (refer to EM-EVER-163-201002, dated October 19, 2020). The transmitter issue manifests as passive intermodulation, or PIM, noise located on the receive frequencies, which limits the system level coverage capability of the site.

Please accept this letter as notification, pursuant to Regulations of Connecticut State Agencies ("R.C.S.A.") §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this notice is being delivered to Tom DeVivo, Mayor of the Town of Windham, Jim Rivers, Town Manager of the Town of Windham, and

Matthew Vertefeuille, Director of Code Enforcement of the Town of Windham, via private carrier. Proof of delivery is attached. See <u>Attachment F</u>, Proof of Delivery of Notice.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2):

- 1. There will be no change to the height of the existing tower.
- 2. The proposed modifications will not require extension of the site boundary.
- 3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The operation of the new antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard as shown in the attached Radio Frequency Emissions Report, dated February 25, 2022. (Attachment G Power Density Report¹)
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Eversource respectfully submits that the proposed modifications to the above referenced telecommunications facility constitute an exempt modification under R.C.S.A. § 16-50j-72(b)(2). One original and two copies of this notice are enclosed.

Communications regarding this Notice of Exempt Modification should be directed to Kathleen Shanley at (860) 728-4527.

By:

Kathleen M. Shanley

Manager – Transmission Siting

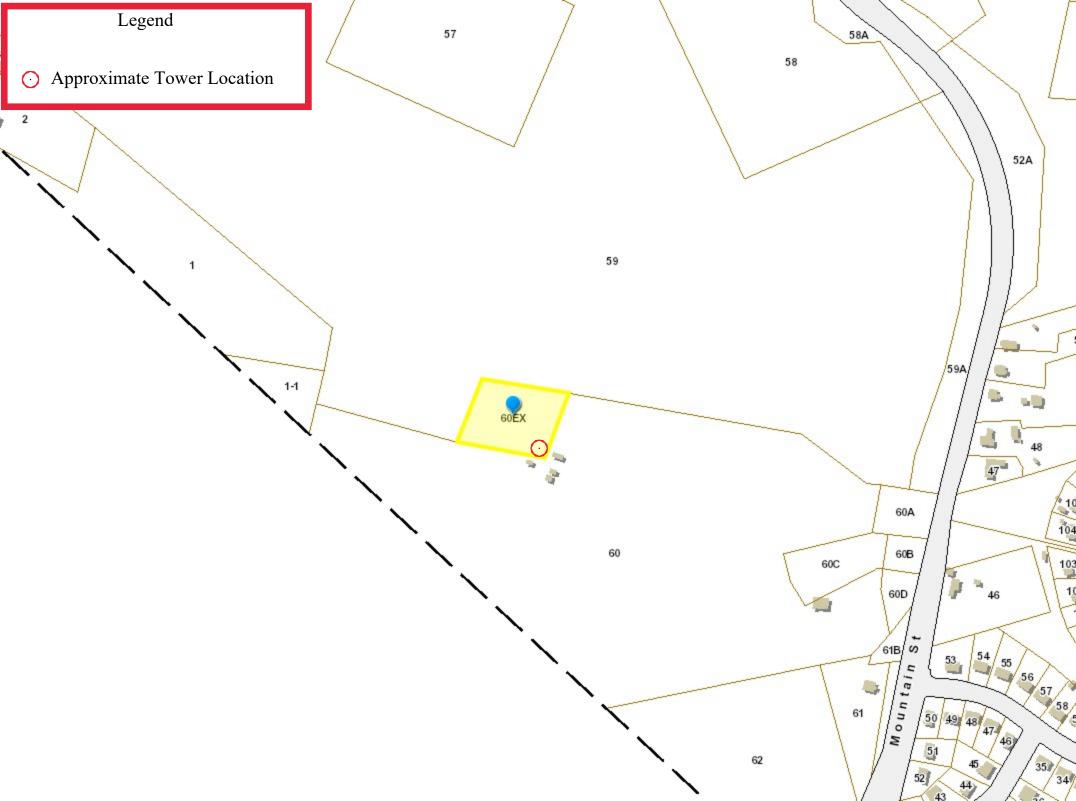
cc: Honorable Tom DeVivo, Mayor, Town of Windham Jim Rivers, Town Manager, Town of Windham Matthew Vertefeuille, Director of Code Enforcement SBA

Attachments

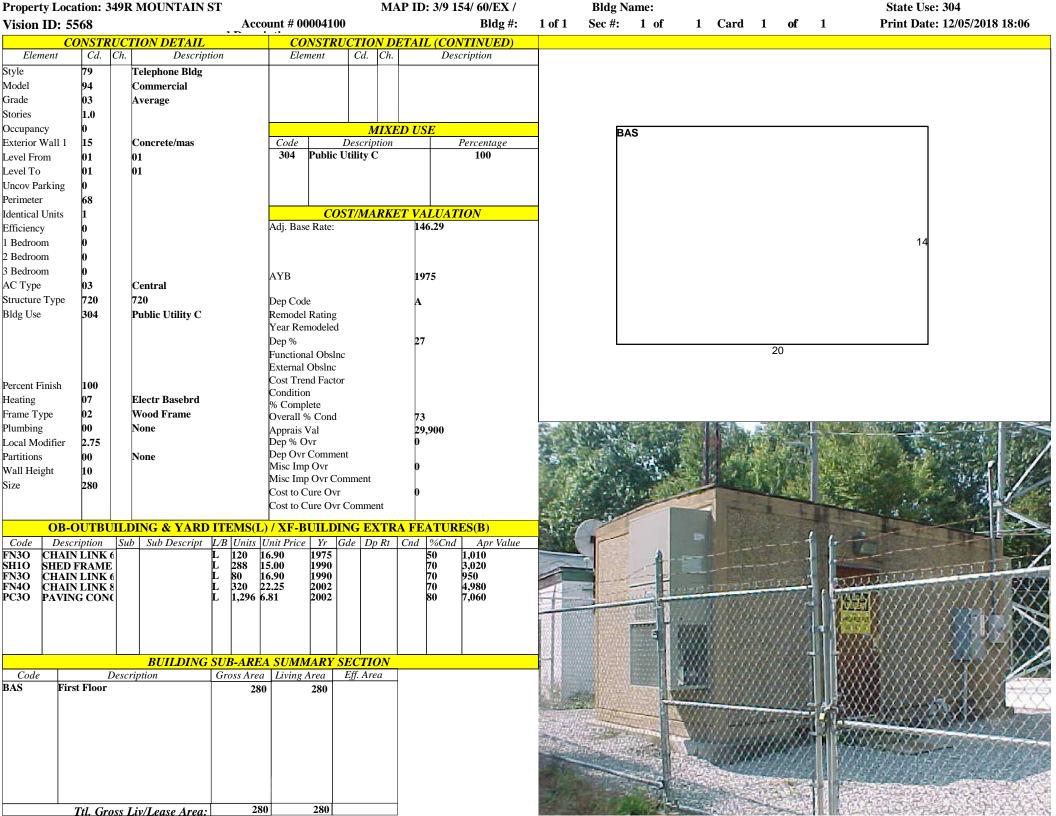
- A. Parcel Map and Property Card
- B. Letter of Authorization
- C. Mount Analysis
- D. Construction Drawings
- E. Structural Analysis
- F. Proof of Delivery of Notice
- G. Power Density Report

¹ Receive-only antennas are not included in the Power Density Report, as they are irrelevant to the % MPE calculations.





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SBA Communications Corporation 8051 Congress Avenue Boca Raton, FL 33487-1307

> T + 561.995.7670 F + 561.995.7626

> > sbasite.com

LETTER OF AUTHORIZATION

SBA Site ID: CTo6462-A, Mountain Street

Property Located at: 349 Mountain Street, Windham, CT, 06226

THE CITY/COUNTY OF: Windham / Windham/Windham

APPLICATION FOR ZONING/USE/BUILDING PERMIT

This letter authorizes Eversource and its authorized agents to file for all necessary zoning, planning and building permits (local, state and federal) for the purposes of installing, operating and maintaining a telecommunications facility on the existing tower on the property referenced above on behalf of SBA Properties, LLC.

All approval conditions that may be granted to Eversource in connection with above referenced facility relating to this specific application are the sole responsibility of Eversource.

SBA Properties, LLC

Jason Silberstein

Executive VP, Site Leasing

Date: 3/02/2022





November 29, 2021

MOUNT EVALUATION LETTER

Site Number: 5568

Site Name: WILLIMANTIC
Site Data: 349 Mountain St.

Windham, CT 06280

Latitude: 41° 42′ 10.84″ Longitude: -72° 13′ 17.01″

Black & Veatch Corporation is pleased to submit this "Mount Evaluation Letter" to determine the structural integrity of antenna mounting system on the above-mentioned site. The purpose of this evaluation is to determine the capacity of the system in supporting the final loading in the attached "Loading Summary".

Based on our evaluation we have determined the existing antenna mounting system to be:

SUFFICIENT

Proposed Mounting System

SitePro 1 (USF-4U) 48" Ultimate Universal Stand-off Frame

This analysis analyzes the worst-case scenario for the proposed custom omni stand-off frame. All levels are deemed sufficient. The proposed mounting system will be capable of supporting the proposed equipment, under the following conditions:

- Contractor shall be responsible for the means and methods of construction.
- Contractor shall inspect the condition of all existing and proposed structural members, all relevant members and connections and report any deficiencies to the engineer prior to installation of any new antennas and other equipment.

The scope of this evaluation pertains only to the proposed antenna mounting system and does not include examination of the loads imparted by the antenna mounting system to the existing tower and its structural components. This document was prepared based on information provided to Black & Veatch. If existing conditions do not reflect those represented, this analysis is no longer valid.

Please contact Josh Riley in our Overland Park Office at 913-458-2522 if you have any questions or comments.

Sincerely, Black & Veatch Corporation

Prepared By: Joohwan Jung Submitted By: Josh Riley, P.E.

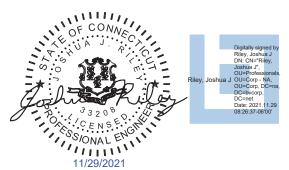




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- 3. REFERENCES
- 4. ASSUMPTIONS
- 5. RESULTS SUMMARY

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APPENDIX 2: RISA PRINTOUTS

APPENDIX 3: ATTACHMENTS



1. LOADING SUMMARY

	Appurtenance											
Carrier	Position	Sector	Antenna RAD Center (ft)	Mount Centerline (ft)	Qty	Туре	Manufacturer	Model				
Eversource	Eversource 1 - 161.2		161.2	153	1	Omni	dbSpectra	SP2D00P36D-D				



2. ANALYSIS CRITERIA SUMMARY

ANAI	ANALYSIS CRITERIA							
STANDARD	TIA-222-H							
WIND SPEED	Ultimate of 140 mph							
WIND SPEED WITH ICE	50 mph with 1" radial ice thickness							
EXPOSURE CATEGORY	В							
RISK CATEGORY	III							
TOPO CATEGORY	Flat							
CREST HEIGHT	N/A							

3. REFERENCES

- American Institute of Steel Construction, AISC 15th Edition
- Telecommunications Industry Association Standard, TIA-222-H & 2018 Connecticut State Building Code
- Antenna Mount Assembly Drawing (Model: USF-4U) by SitePro 1, dated 02/16/2011

4. ASSUMPTIONS

This analysis may be affected if any assumptions are not valid or have been made in error. Black & Veatch should be notified to determine the effect on the structural integrity of the antenna mounting system.

- The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- The configuration of antennas, mounts, and other appurtenances are as specified in the Loading Summary and the referenced drawings.
- All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- Sector frame center line: located equidistant between top & bottom boom; Platform center line: located at the base perimeter of platform, unless otherwise specified.
- Steel grades have been assumed as follows, unless noted otherwise:

Channel, Solid Round, Angle, Plate

ASTM A36 (GR 36)

HSS (Rectangular)

Pipe

ASTM A53 (GR B-35)

ASTM A335

Connection Bolts ASTM A325



5. RESULTS SUMMARY

Name	Bending Str	ess Ratio	Shear Stress Ratio		
Arm: HSS3X3X3	50.1%	Pass	20.4%	Pass	
Bracing: Pipe 2.0 Std	34.2%	Pass	6.3%	Pass	
Mount Pipe: Pipe 3.0 Std	29.3%	Pass	19.9%	Pass	

^{*}Von Mises SR = (Max Von Mises Value From RISA-3D)/(0.9*Fy)

^{**}Capacity rating per TIA-222-H Section 15.5.



APPENDIX 1: MOUNT ANALYSIS REPORT



Client: Eversource Computed By: Joohwan Jung
Site Name: WILLIMANTIC (5568) Date: 11/29/2021

Verified By: Josh Riley
Title: MOUNT ANALYSIS REPORT Date: 11/29/2021

Dead and Live Loads

Maintenance Live Load: $L_V = 250$ lb Installation Live Load: $L_M = 0$ lb

Appurtenance Dead Loads								
Name	Weight (lb)							
SP2D00P36D-D	45							



Site Name: WILLIMANTIC (5568) Date: 11/29/2021

Verified By: Josh Riley

Appurtenance Wind Loading			<u>Equations</u>	TIA-222-H
Exposure Category =	В		$K_z = 2.01 (z / z_g)^{2/\alpha}$	2.6.5.2
Risk Category =	Ш			
Topographic Category =	1		$K_h = e^{(f \cdot z/H)}$	2.6.6.2.1
Basic Wind Speed, V =	140	mph		
Height Above Ground, z =	161.2	ft	$K_{zt} = \left[1 + K_c K_t / K_h\right]^2$	2.6.6.2.1
Crest Height, H =	N/A	ft		
Velocity Pressure Coefficient, K _z =	1.13		$K_e = e^{-0.000032^{+25}}$	2.6.8
Topographic Factor, K_{zt} =	1.00			
Wind Directionality Factor, K_d =	0.95		$q_z = 0.00256 K_z K_{zt} K_e K_d V^2$	2.6.11.6
Shielding Factor, K _a =	0.90			
Ground Elevation Factor, K_e =	1.000		$F_A = q_z G_h(EPA)$	2.6.11.2
Wind Velocity Pressure, q_z =	53.99	psf		
Gust Effect Factor, G_h =	1.00		$F_{M} = q_{z}G_{h}C_{f}D_{p}$	2.6.11.2

Appurtenance Wind Loads										
Name	Height	Width	Depth		Norma	l	Tangential			
ivanie	(ft)	(ft)	(ft)	Ca	EPA FT2	F _A (lb)	Ca	EPA FT2	F _A (lb)	
SP2D00P36D-D	15.60	0.25	0.25	2.00	7.02	379.02	2.00	7.02	379.02	
	·	·				·		·		



Site Name: WILLIMANTIC (5568) Date: 11/29/2021

Verified By: Josh Riley

Member Wind Loading			<u>Equations</u>	TIA-222-H
Exposure Category =	В		$K_z = 2.01 (z / z_g)^{2/\alpha}$	2.6.5.2
Risk Category =	Ш			
Topographic Category =	1		$K_h = e^{(f \cdot z/H)}$	2.6.6.2.1
Basic Wind Speed, V =	140	mph		
Height Above Ground, z =	161.2	ft	$K_{zt} = \left[1 + K_c K_t / K_h\right]^2$	2.6.6.2.1
Crest Height, H =	N/A	ft		
Velocity Pressure Coefficient, $K_z =$	1.13		$K_e = e^{-0.000032^{-2S}}$	2.6.8
Topographic Factor, K_{zt} =	1.00			
Wind Directionality Factor, K_d =	0.95		$q_z = 0.00256 K_z K_{zt} K_e K_d V^2$	2.6.11.6
Shielding Factor, K _a =	0.90			
Ground Elevation Factor, K_e =	1.000		$F_A = q_z G_h(EPA)$	2.6.11.2
Wind Velocity Pressure, q_z =	53.99	psf		
Gust Effect Factor, $G_h =$	1.00		$F_{M} = q_{z}G_{h}C_{f}D_{p}$	2.6.11.2
D.C. and L.	\A/: al	Landa		1

Member Wind Loads											
Name	Depth (ft)	Width (ft)	C _f	D _p (ft)	F _M (lb)						
Arm: HSS3X3X3	0.25	0.25	2	0.25	27.00						
Bracing: Pipe 2.0 Std	0.20		1.2	0.20	12.82						
Mount Pipe: Pipe 3.0 Std	0.29		1.2	0.29	18.90						



Site Name: WILLIMANTIC (5568) Date: 11/29/2021

Verified By: Josh Riley

Appurtenance Ice Dead Loading			<u>Equations</u>	TIA-222-H
Exposure Category =	В			
Risk Category =	Ш		$K_h = e^{(f \cdot z/H)}$	2.6.6.2.1
Topographic Category =	1			
Height Above Ground, z =	161.2	ft	$K_{zt} = \left[1 + K_c K_t / K_h\right]^2$	2.6.6.2.1
Crest Height, H =	N/A	ft		
Design Ice Thickness, T _i =	1.00	in	$K_{iz} = (z/33)^{0.10}$	2.6.10
Importance Factor, I =	1.15			
Topographic Factor, K_{zt} =	1.00		$T_{iz} = T_i I K_{iz} (K_{zt})^{0.35}$	2.6.10
Height Escalation Factor, K_{iz} =	1.17			
Factored Ice Thickness, T_{iz} =	1.35	in	$DL_{ice}=[(H_{ice}*D_{ice}*W_{ice}) - (H*W*D)]*56pcf$	
Grating Ice Dead Load, $D_{Gice} =$	6.29	psf		

Appurtenance Ice Dead Loads										
Name	Height w/ ice	Width w/ice	Depth w/ ice	V_{ice}	DL _{ice}					
ivaille	(ft)	(ft)	(ft)	(ft ³)	(lb)					
SP2D00P36D-D	15.82	0.47	0.47	2.59	145.02					



Site Name: WILLIMANTIC (5568) Date: 11/29/2021

Verified By: Josh Riley

Member Ice Dead Loading			<u>Equations</u>	TIA-222-H
Exposure Category =	В			
Risk Category =	Ш		$K_h = e^{(f \cdot z/H)}$	2.6.6.2.1
Topographic Category =	1			
Height Above Ground, z =	161.2	ft	$K_{zt} = [1 + K_c K_t / K_h]^2$	2.6.6.2.1
Crest Height, H =	N/A	ft		
Design Ice Thickness, T _i =	1.00	in	$K_{iz} = (z/33)^{0.10}$	2.6.10
Importance Factor, I =	1.15			
Topographic Factor, K_{zt} =	1.00		$T_{iz} = T_i I K_{iz} (K_{zt})^{0.35}$	2.6.10
Height Escalation Factor, K_{iz} =	1.17			
Factored Ice Thickness, T_{iz} =	1.35	in	Aiz = pi*Tiz*(Dc+Tiz)	2.6.10
Grating Ice Dead Load, D _{Gice} =	6.29	psf		
			DL _{ice} =Aiz*56pcf	

Member Ice Dead Loads						
Name	Depth w/ ice (ft)	Width w/ ice (ft)	Dc (ft)	Aiz (ft²)	DL _{ice} (lb/ft)	
Arm: HSS3X3X3	0.47	0.47	0.35	0.16	9.20	
Bracing: Pipe 2.0 Std	0.42		0.20	0.11	6.13	
Mount Pipe: Pipe 3.0 Std	0.52		0.29	0.14	7.98	



Site Name: WILLIMANTIC (5568) Date: 11/29/2021

Verified By: Josh Riley

Appurtenance Ice Wind Loading			<u>Equations</u>	TIA-222-H
Exposure Category =	В		$K_z = 2.01 (z / z_g)^{2/\alpha}$	2.6.5.2
Risk Category =	Ш			
Topographic Category =	1		$K_h = e^{(f \cdot z/H)}$	2.6.6.2.1
Ice Wind Speed, V_{ice} =	50	mph		
Height Above Ground, z =	161.2	ft	$K_{zt} = \left[1 + K_c K_t / K_h\right]^2$	2.6.6.2.1
Crest Height, H =	N/A	ft		
Velocity Pressure Coefficient, K _z =	1.13	psf	$K_e = e^{-0.000032^{-25}}$	2.6.8
Topographic Factor, K_{zt} =	1.00			
Wind Directionality Factor, K_d =	0.95		$q_z = 0.00256 K_z K_{zt} K_e K_d V^2$	2.6.11.6
Shielding Factor, $K_a =$	0.90			
Ground Elevation Factory, K_e =	1.000		$F_{A(ice)} = q_{z(ice)}G_h(EPA)_{A(ice)}$	2.6.11.2
Ice Wind Velocity Pressure, $q_{z(ice)}$ =	6.887			
Factored Ice Thickness, T_{iz} =	1.35	in	$F_{M(ice)} = q_{z(ice)}G_hC_fD_{p(ice)}$	2.6.11.2
Gust Effect Factor, G _h =	1			1

Appurtenance Ice Wind Loads									
Name	Height Width Depth		Normal			Tangential			
ivallie	w/ Ice (ft) w/ Ice (ft) w/ Ice (ft)	Ca	EPA FT2	F _A (lb)	Ca	EPA FT2	F _A (lb)		
SP2D00P36D-D	15.82	0.47	0.47	-	10.92	75.20	-	10.92	75.20
			·						



Site Name: WILLIMANTIC (5568) Date: 11/29/2021

Verified By: Josh Riley

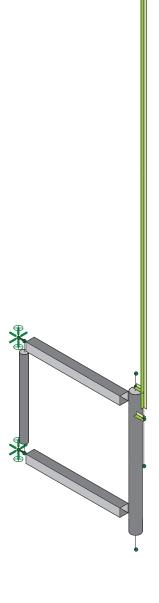
Member Ice Wind Loading			<u>Equations</u>	TIA-222-H
Exposure Category =	В		$K_z = 2.01 (z / z_g)^{2/\alpha}$	2.6.5.2
Risk Category =	Ш			
Topographic Category =	1		$K_h = e^{(f \cdot z/H)}$	2.6.6.2.1
Ice Wind Speed, V_{ice} =	50	mph		
Height Above Ground, z =	161.2	ft	$K_{zt} = \left[1 + K_c K_t / K_h\right]^2$	2.6.6.2.1
Crest Height, H =	N/A	ft		
Velocity Pressure Coefficient, K_z =	1.13	psf	$K_{e} = e^{-0.000032^{-2}S}$	2.6.8
Topographic Factor, K_{zt} =	1.00			
Wind Directionality Factor, K_d =	0.95		$q_z = 0.00256 K_z K_{zt} K_e K_d V^2$	2.6.11.6
Shielding Factor, K _a =	0.90			
Ground Elevation Factory, K_e =	1.000		$F_{A(ice)} = q_{z(ice)}G_h(EPA)_{A(ice)}$	2.6.11.2
Ice Wind Velocity Pressure, $q_{z(ice)}$ =	6.887			
Factored Ice Thickness, T_{iz} =	1.35	in	$F_{M(ice)} = q_{z(ice)}G_hC_fD_{p(ice)}$	2.6.11.2
Gust Effect Factor, G _h =	1			1

Member Ice Wind Loads							
Name	Depth w/ Ice (ft)	Width w/ Ice (ft)	C _f	D _{p(ice)} (ft)	F _{M(ice)} (lb/ft)		
Arm: HSS3X3X3	0.47	0.47	2	0.47	6.54		
Bracing: Pipe 2.0 Std	0.42		1.2	0.42	3.49		
Mount Pipe: Pipe 3.0 Std	0.52		1.2	0.52	4.27		



APPENDIX 2: RISA PRINTOUTS

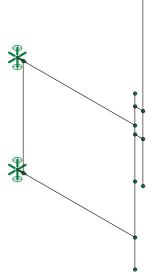




Envelope Only Solution

Black & Veatch		SK - 1
Joohwan Jung	Willimantic USF-4U Model	Nov 28, 2021 at 9:29 AM
405025.2021.2200		Willimantic USF-4U Model.r3d





Envelope Only Solution

Black & Veatch		SK - 2
Joohwan Jung	Willimantic USF-4U Model	Nov 28, 2021 at 9:29 AM
405025.2021.2200		Willimantic USF-4U Model.r3d





Member Code Checks Displayed (Enveloped) Envelope Only Solution

Black & Veatch		SK - 3
Joohwan Jung	Willimantic USF-4U Model	Nov 28, 2021 at 9:29 AM
405025.2021.2200		Willimantic USF-4U Model.r3d

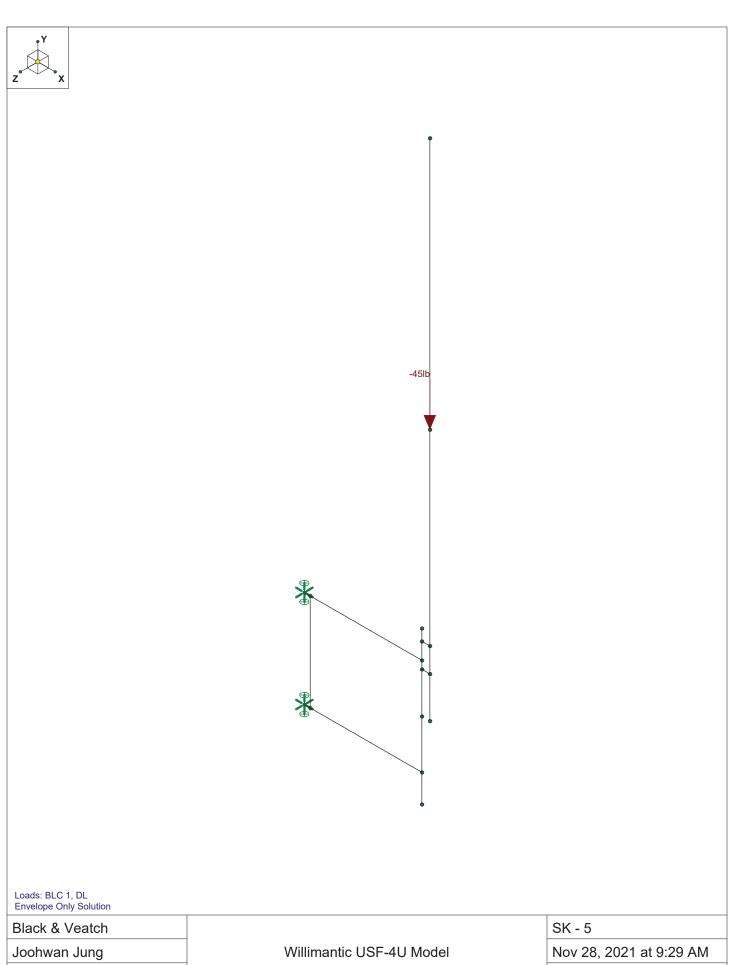




Member Shear Checks Displayed (Enveloped) Envelope Only Solution

Black & Veatch	
Joohwan Jung	Willimantic USF-4U Model
405025.2021.2200	

SK	-	4
2V	_	4



Willimantic USF-4U Model.r3d 405025.2021.2200



Company Designer Job Number Model Name

: Black & Veatch : Joohwan Jung : 405025.2021.2200 : Willimantic USF-4U Model

Nov 28, 2021 9:29 AM Checked By:___

(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	24
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Υ
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	None
Cold Formed Steel Code	None
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	None

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parme Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR_SET_ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8



Company : Black & Veatch
Designer : Joohwan Jung
Job Number : 405025.2021.2200
Model Name : Willimantic USF-4U Model

Nov 28, 2021 9:29 AM Checked By:_

(Global) Model Settings, Continued

Seismic Code	ASCE 7-16
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
RX	3
RZ	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	l or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	4
Cd X	4
Rho Z	1
Rho X	1

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (/1	Density[k/f	. Yield[ksi]	Ry	Fu[ksi]	Rt
1	A992	29000	11154	.3	.65	.49	50	1.1	65	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
3	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42	1.4	58	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46	1.4	58	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2
7	A1085	29000	11154	.3	.65	.49	50	1.4	65	1.3

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design Rul	. A [in2]	lyy [in4]	Izz [in4]	J [in4]
1	Arm	HSS3X3X3	Beam	None	A53 Gr.B	Typical	1.89	2.46	2.46	4.03
2	Bracing	PIPE 2.0	Column	None	A53 Gr.B	Typical	1.02	.627	.627	1.25
3	Mount Pipe	PIPE 3.0	Column	None	A53 Gr.B	Typical	2.07	2.85	2.85	5.69

General Material Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E5 F)	Density[k/ft^3]
1	gen Conc3NW	3155	1372	.15	.6	.145
2	gen Conc4NW	3644	1584	.15	.6	.145
3	gen Conc3LW	2085	906	.15	.6	.11
4	gen Conc4LW	2408	1047	.15	.6	.11
5	gen Alum	10100	4077	.3	1.29	.173
6	gen Steel	29000	11154	.3	.65	.49
7	gen Plywood	1800	38	0	.3	.035
8	RIGID	1e+6		.3	0	0



Company : Black & Veatch
Designer : Joohwan Jung
Job Number : 405025.2021.2200
Model Name : Willimantic USF-4U Model

Nov 28, 2021 9:29 AM Checked By:_

Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	N1	Reaction	Reaction	Reaction		Reaction	
2	N3	Reaction	Reaction	Reaction		Reaction	

Member Primary Data

	Label	I Joint	J Joint	K Joint	Rotate(deg)	Section/Shape	Type	Design List	Material	Design Rules
1	M1	N1	N2		, ,,	Arm	Beam	None	A53 Gr.B	Typical
2	M2	N3	N4			Arm	Beam	None	A53 Gr.B	Typical
3	M3	N5	N6			Bracing	Column	None	A53 Gr.B	Typical
4	M4	N7	N8			Mount Pipe	Column	None	A53 Gr.B	Typical
5	M5	N9	N10			RIGID	None	None	RIGID	Typical
6	M6	N12	N13			RIGID	None	None	RIGID	Typical
7	M7	N15	N14			RIGID	None	None	RIGID	Typical

Member Advanced Data

	Label	I Release	J Release	I Offset[in]	J Offset[in]	T/C Only	Physical	Defl Rat	.Analysis	Inactive	Seismic
1	M1					,	Yes				None
2	M2						Yes				None
3	M3						Yes	** NA **			None
4	M4						Yes	** NA **			None
5	M5						Yes	** NA **			None
6	M6						Yes	** NA **			None
7	M7						Yes	** NA **			None

Hot Rolled Steel Design Parameters

	Label	Shape	Length[in]	Lbyy[in]	Lbzz[in]	Lcomp top[in]	Lcomp bot[in]	L-torqu	Kyy	Kzz	Cb	Function
1	M1	Arm	43.5			Lbyy]					Lateral
2	M2	Arm	43.5			Lbyy						Lateral
3	M3	Bracing	36									Lateral
4	M4	Mount Pipe	56.5									Lateral

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(Me	Surface(P
1	DL	DĽ		-1		1				, i
2	Maintenance LL - LV	LL				1				
3	Installation LL - LM	LL				1				
4	Wind - 0 Deg (X)	WL				1		4		
5	Wind - 30 Deg (X)	WL				1		4		
6	Wind - 60 Deg (X)	WL				1		4		
7	Wind - 90 Deg (X)	WL				1		4		
8	Wind - 120 Deg (X)	WL				1		4		
9	Wind - 150 Deg (X)	WL				1		4		
10	Wind - 180 Deg (X)	WL				1		4		
11	Wind - 210 Deg (X)	WL				1		4		
12	Wind - 240 Deg (X)	WL				1		4		
13	Wind - 270 Deg (X)	WL				1		4		
14	Wind - 300 Deg (X)	WL				1		4		
15	Wind - 330 Deg (X)	WL				1		4		
16	Wind - 0 Deg (Z)	WL				1		4		
17	Wind - 30 Deg (Z)	WL				1		4		
18	Wind - 60 Deg (Z)	WL				1		4		



Company : Black & Veatch
Designer : Joohwan Jung
Job Number : 405025.2021.2200
Model Name : Willimantic USF-4U : Willimantic USF-4U Model

Nov 28, 2021 9:29 AM Checked By:_

Basic Load Cases (Continued)

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(Me	Surface(P
19	Wind - 90 Deg (Z)	WL			, and the second	1		4	,	,
20	Wind - 120 Deg (Z)	WL				1		4		
21	Wind - 150 Deg (Z)	WL				1		4		
22	Wind - 180 Deg (Z)	WL				1		4		
23	Wind - 210 Deg (Z)	WL				1		4		
24	Wind - 240 Deg (Z)	WL				1		4		
25	Wind - 270 Deg (Z)	WL				1		4		
26	Wind - 300 Deg (Z)	WL				1		4		
27	Wind - 330 Deg (Z)	WL				1		4		
28	Ice DL	DL				1		4		
29	Ice Wind - 0 Deg (X)	WL				1		4		
30	Ice Wind - 30 Deg (X)	WL				1		4		
31	Ice Wind - 60 Deg (X)	WL				1		4		
32	Ice Wind - 90 Deg (X)	WL				1		4		
33	Ice Wind - 120 Deg (X)	WL				1		4		
34	Ice Wind - 150 Deg (X)	WL				1		4		
35	Ice Wind - 180 Deg (X)	WL				1		4		
36	Ice Wind - 210 Deg (X)	WL				1		4		
37	Ice Wind - 240 Deg (X)	WL				1		4		
38	Ice Wind - 270 Deg (X)	WL				1		4		
39	Ice Wind - 300 Deg (X)	WL				1		4		
40	Ice Wind - 330 Deg (X)	WL				1		4		
41	Ice Wind - 0 Deg (Z)	WL				1		4		
42	Ice Wind - 30 Deg (Z)	WL				1		4		
43	Ice Wind - 60 Deg (Z)	WL				1		4		
44	Ice Wind - 90 Deg (Z)	WL				1		4		
45	Ice Wind - 120 Deg (Z)	WL				1		4		
46	Ice Wind - 150 Deg (Z)	WL				1		4		
47	Ice Wind - 180 Deg (Z)	WL				1		4		
48	Ice Wind - 210 Deg (Z)	WL				1		4		
49	Ice Wind - 240 Deg (Z)	WL				1		4		
50	Ice Wind - 270 Deg (Z)	WL				1		4		
51	Ice Wind - 300 Deg (Z)	WL				1		4		
	Ice Wind - 330 Deg (Z)	WL				1		4		

Load Combinations

	Description	S P	SR	.B	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	<u>—</u> В	Fa
1	WIND LOAD COMBOS (140 MPH)																						
2	1.2DL + WL (0 DEG)	Yes Y		1	1.2	4	1	16	1														
3	1.2DL + WL (30 DEG)	Yes Y		1	1.2	5	1	17	1														
4	1.2DL + WL (60 DEG)	Yes Y		1	1.2	6	1	18	1														
5	1.2DL + WL (90 DEG)	Yes Y		1	1.2	7	1	19	1														
6	1.2DL + WL (120 DEG)	Yes Y		1	1.2	8	1	20	1														
7	1.2DL + WL (150 DEG)	Yes Y		1	1.2	9	1	21	1														
8	1.2DL + WL (180 DEG)	Yes Y		1	1.2	10	1	22	1														
9	1.2DL + WL (210 DEG)	Yes Y		1	1.2	11	1	23	1														
10	1.2DL + WL (240 DEG)	Yes Y		1	1.2	12	1	24	1														
11	1.2DL + WL (270 DEG)	Yes Y		1	1.2	13	1	25	1														
12	1.2DL + WL (300 DEG)	Yes Y		1	1.2	14	1	26	1														
13	1.2DL + WL (330 DEG)	Yes Y		1	1.2	15	1	27	1														
14																							
15	MOUNT LOAD COMBOS (30 MP																						
16	1.4DL	Yes Y		1	1.4																		
17	1.2DL + 1.5LV	Yes Y		1	1.2	2	1.5																
18	1.2DL + 1.5LM + WL (0 DEG)	Yes Y		1	1.2	3	1.5	4	.046	16	.046												



Company Designer Job Number Model Name

: Black & Veatch: Joohwan Jung: 405025.2021.2200: Willimantic USF-4U Model

Nov 28, 2021 9:29 AM Checked By:___

Load Combinations (Continued)

	Description	S P	SR	В	Fa	.B	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa	В	Fa
19	1.2DL + 1.5LM + WL (30 DEG)	Yes Y		1	1.2	3	1.5	5	.046	17	.046												
20	1.2DL + 1.5LM + WL (60 DEG)	Yes Y		1	1.2						.046												
21	1.2DL + 1.5LM + WL (90 DEG)	Yes Y		1	1.2	3	1.5	7	.046	19	.046												
22	1.2DL + 1.5LM + WL (120 DEG)	Yes Y		1	1.2	3	1.5				.046												
23	1.2DL + 1.5LM + WL (150 DEG)	Yes Y		1	1.2	3	1.5	9	.046	21	.046												
24	1.2DL + 1.5LM + WL (180 DEG)	Yes Y		1	1.2	3	1.5	10	.046	22	.046												
25	1.2DL + 1.5LM + WL (210 DEG)	Yes Y		1	1.2	3					.046												
26	1.2DL + 1.5LM + WL (240 DEG)	Yes Y		1	1.2	3	1.5	12	.046	24	.046												
27	1.2DL + 1.5LM + WL (270 DEG)	Yes Y		1	1.2	3	1.5				.046												
28	1.2DL + 1.5LM + WL (300 DEG)	Yes Y		1	1.2	3	1.5	14	.046	26	.046												
29	1.2DL + 1.5LM + WL (330 DEG)	Yes Y		1	1.2	3	1.5	15	.046	27	.046												
30																							
31	ICE LOAD COMBOS (1", 50 MPH)																						
32	1.2DL + Ice DL + Ice WL (0 DEG)	Yes Y		1	1.2	28	1	29	1	41	1												
33	1.2DL + Ice DL + Ice WL (30 DEG)	Yes Y		1	1.2	28	1	30	1	42	1												
34	1.2DL + Ice DL + Ice WL (60 DEG)	Yes Y		1	1.2	28	1	31	1	43	1												
35	1.2DL + Ice DL + Ice WL (90 DEG)	Yes Y		1	1.2	28	1	32	1	44	1												
36	1.2DL + Ice DL + Ice WL (120 DE	Yes Y		1	1.2	28	1	33	1	45	1												
37	1.2DL + Ice DL + Ice WL (150 DE	Yes Y		1	1.2	28	1	34	1	46													
38	1.2DL + Ice DL + Ice WL (180 DE	Yes Y		1	1.2	28	1	35	1	47	1												
39	1.2DL + Ice DL + Ice WL (210 DE	Yes Y		1	1.2	28	1	36	1	48	1												
40	1.2DL + Ice DL + Ice WL (240 DE	Yes Y		1	1.2	28	1	37	1	49													
41	1.2DL + Ice DL + Ice WL (270 DE	Yes Y		1	1.2	28	1	38	1	50													
42	1.2DL + Ice DL + Ice WL (300 DE	Yes Y		1	1.2			39		51	1												
43	1.2DL + Ice DL + Ice WL (330 DE	Yes Y		1		28		40		52	1												
44																							

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-ft]	LC	MY [lb-ft]	LC	MZ [lb-ft]	LC
1	N1	max	1184.609	2	750.686	8	1338.067	5	0	43	2585.525	11	0	43
2		min	-1488.448	8	-587.305	2	-1338.067	11	0	2	-2585.525	5	0	2
3	N3	max	786.229	8	749.55	2	635.852	11	0	43	434.876	5	0	43
4		min	-482.389	2	-588.446	8	-635.852	5	0	2	-434.876	11	0	2
5	Totals:	max	702.219	2	537.238	17	702.215	5						
6		min	-702.22	8	162.24	8	-702.215	11						

Envelope AISC 15th(360-16): LRFD Steel Code Checks

	Member	Shape	Code Check	Loc[LC	Shear	Loc[Dir	LC	phi*Pnc	phi*Pnt	phi*Mn	.phi*Mn	Cb Eqn
1	M1	HSS3X3X3	.501	0	11	.204	2.266	Z						2 H1-1b
2	M2	HSS3X3X3	.228	43.5	10	.115	43.5	Z	11	55265	59535	5171.25	5171.25	2H1-1b
3	M3	PIPE 2.0	.342	0	11	.063	0		11	28843	32130	1871.625	1871.625	2H1-1b
4	M4	PIPE 3.0	.293	13.5	. 9	.199	13.5		11	57908	65205	5748.75	5748.75	1H1-1b



APPENDIX 3: ATTACHMENTS



220 MHz Antenna – Omnidirectional, Low-PIM/Hi-PIP, Unity Gain Models - SP2D00P36D-D

Specification	ons
Design Type	True Corporate Feed
Frequency Range	217-220 MHz
Passive Intermodulation – PIM (2 x 20W sources)	-150 dBc, 3 rd Order
Bandwidth	3 MHz
Gain - dBd (average over BW)	0 dBd
Isolation, min.	40 dB
Configuration	Dual antenna
Beam Tilt (electrical down-tilt)	None (0°)
Vertical Beamwidth (E-Plane)	60°
Impedance Ohms	50
VSWR / Return Loss dB	1.5 : 1 / 14 dB (min.)
Average Power Rating	500 W (each antenna)
Peak Instantaneous Power	25 kW (each antenna)
Polarization	Vertical
Lightning Protection	Direct Ground
Connector	7/16 DIN female
Equivalent Flat-Plate Area	2.59 sq. ft.
Lateral Wind-load Thrust @100mph	109 lbf.
Wind Speed rating	160 mph (without ice) 136 mph (½" radial ice)
Total Length	15.6 feet
Mounting Mast Length	35 inches
Mounting Hardware (Included)	DSH3V4N
Top Sway Brace	DSH2H3S
(Recommended if side mounting antennas)	(order separately)
Mast O.D.	3.5 inches
Radome color	Horizon Blue
Radome O.D.	3.0 inches
Weight, antenna, and hardware	45 lbs. (approx.)
Shipping Weight	80 lbs. (approx.)
Invertibility	Antennas are physically invertible, but the patterns are optimized for upright mount.



Features and Benefits

Antennas from dbSpectra provide long term, trouble-free service in severe environments!

Design is tested to stringent Peak Instantaneous
Power (PIP) levels of 25 KW using dbSpectra's
12-channel P25 PIP test bed. High PIP level is
demanded by today's digital systems.

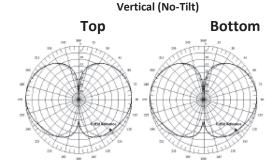
True Corporate Feed Array – provides for excellent gain and pattern consistency across a wider frequency range.

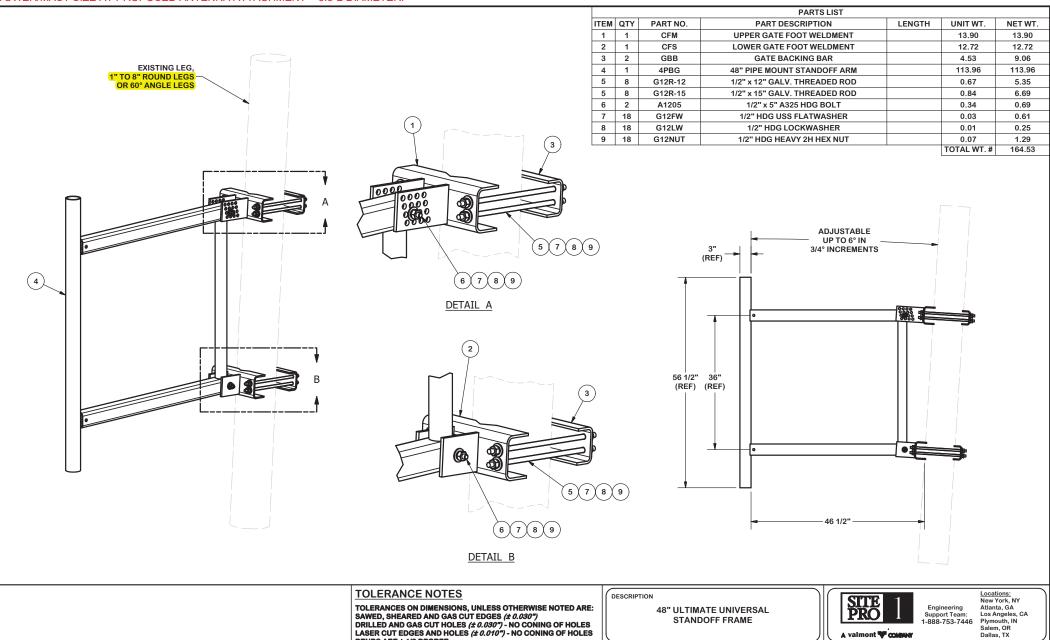
PIM Rated Design - better than -150 dBc.

Sturdy Construction – Heavy-wall fiberglass radome minimizes tip deflection.

Excellent Lightning Protection – heavy internal conductor DC ground.

Radiation Pattern





CPD NO.

81 01

CLASS SUB

DRAWN BY

DRAWING USAGE

CUSTOMER

2/4/2011

RCH

ENG. APPROVAL

CHECKED BY

BMC 2/16/2011

BENDS ARE ± 1/2 DEGREE ALL OTHER MACHINING (± 0.030")

ALL OTHER ASSEMBLY (± 0.060")

PROPRIETARY NOTE:
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MOUSTRIES AND CONSIDERED A TRADE SECRET. ANY USE OR DISCLOSURE WITHOUT THE CONSENT OF
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USF-4U

USF-4U

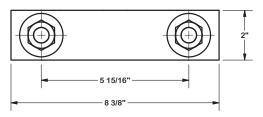
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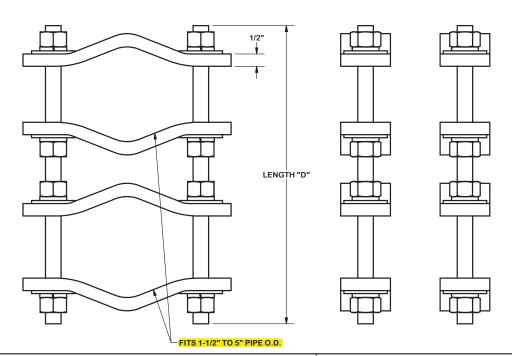
DWG. NO.

Dallas, TX

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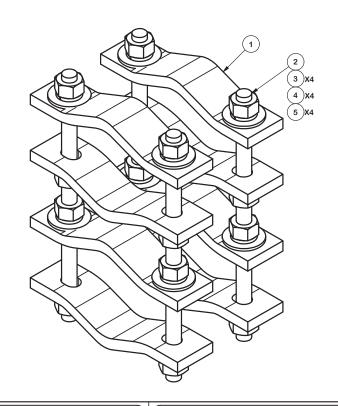






	PARTS LIST					
ITEM	ITEM QTY PART NO. PART DESCRIPTION LEN		LENGTH	UNIT WT.	NET WT.	
1	8	DCP	CLAMP HALF, 1/2" THICK, 8-3/8"		2.40	19.20
2	В	С	5/8" THREADED ROD D E		F	
3	16	G58NUT	5/8" HDG HEAVY 2H HEX NUT 0.13 2.08		2.08	
4	4 16 G58LW 5/8" HDG LOCKWASHER 0.03 0.		0.42			
5	5 16 G58FW 5/8" HDG USS FLATWASHER 0.07 1.13		1.13			

	VARIABLE PARTS TABLE					
ASSEMBLY "A"	QTY "B"	PART "C"	LENGTH "D"	UNIT WT. "E"	NET WT. "F"	TOTAL WEIGHT
DCP12K	4	G58R-12	12"	1.05	4.18	27.01
DCP18K	4	G58R-18	18"	1.57	6.27	29.10



TOLERANCE NOTES

TOLERANCES ON DIMENSIONS, UNLESS OTHERWISE NOTED ARE: SAWED, SHEARED AND GAS CUT EDGES (\$ 0.030") DRILLED AND GAS CUT HOLES (\$ 0.030") - NO CONING OF HOLES LASER CUT EDGES AND HOLES (\$ 0.010") - NO CONING OF HOLES

BENDS ARE ± 1/2 DEGREE

ALL OTHER MACHINING (± 0.030") ALL OTHER ASSEMBLY (± 0.060")

PROPRIETARY NOTE:
THE DATA AND TECHNIQUES CONTAINED IN THIS DRAWING ARE PROPRIETARY INFORMATION OF VALMONT
INDUSTRIES AND CONSIDERED A TRADE SECRET. ANY USE OR DISCLOSURE WITHOUT THE CONSENT OF
VALMONT INDUSTRIES IS STRUCTLY PROHIBITED.

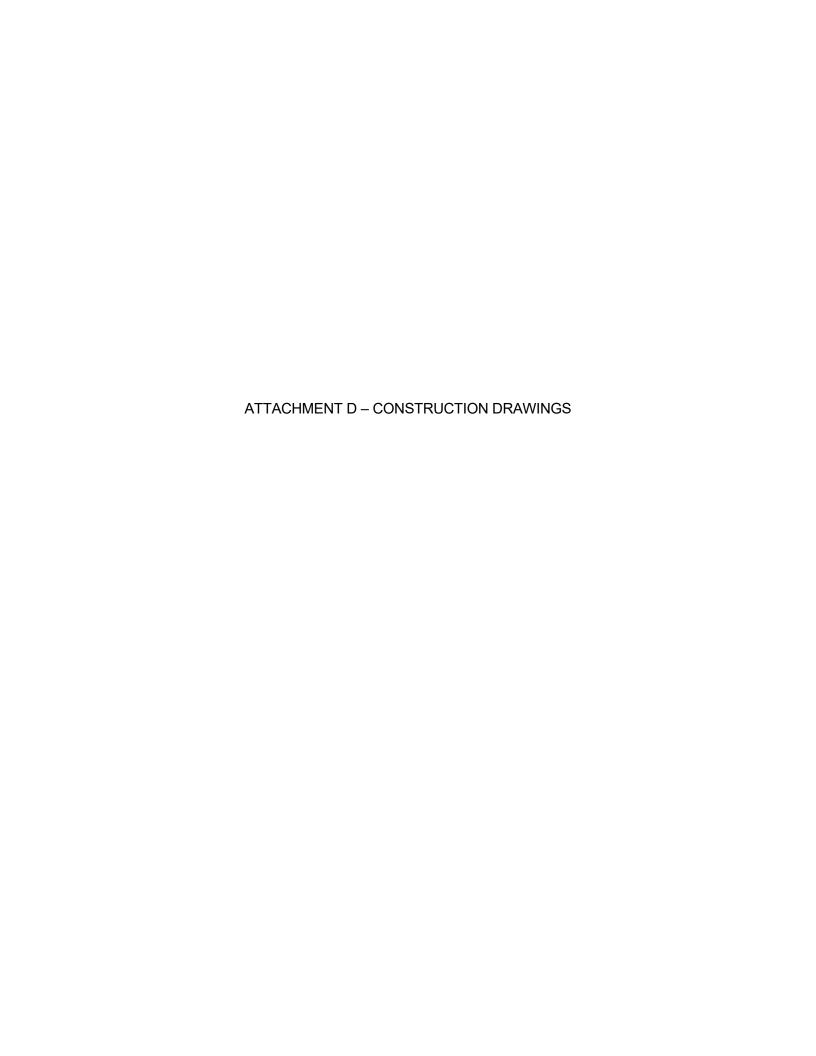
DESCRIPTION

PIPE TO PIPE CLAMP SET 1-1/2" TO 5" PIPE 1/2" THICK CLAMP



Locations: New York, NY Atlanta, GA Los Angeles, CA Plymouth, IN Salem, OR Dallas, TX Engineering Support Team: 1-888-753-7446

CLASS SUB DRAWING USAGE CUSTOMER CEK 1/22/2013 CLASS SUB DRAWING USAGE CUSTOMER CEK 1/22/2013 DRAWING USAGE CUSTOMER CEK 1/22/2013 DCPxxK							
CLASS SUB DRAWING USAGE CHECKED BY DWG. NO.		CPD NO	D.	DRAWN BY	ENG. APPROVAL	PART NO.	_
				KC8 8/21/2012		SEE ASSEMBLY "A"	_ o }
81 01 CUSTOMER CEK 1/22/2013 DCPxxK	1	CLASS	SUB	DRAWING USAGE	CHECKED BY	DWG. NO.	TI S
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WILLIMANTIC (CT06462-A) **349 MOUNTAIN ST WINDHAM, CT 06280**

PROJECT SUMMARY

THE GENERAL SCOPE OF WORK CONSISTS OF THE FOLLOWING:

- 1. INSTALL (1) NEW OMNI/WHIP ANTENNA AT ELEVATION 169'-0"± AGL INSTEAD OF (2) OMNI/WHIP ANTENNAS, (1) AT ELEVATION 165'-1 1/2"± AGL AND (1) AT ELEVATION 157'-1 1/2"± AGL
- 2. INSTALL (1) NEW RACK WITH DMR EQUIPMENT IN EXISTING SHELTER

GOVERNING CODES

2018 CONNECTICUT STATE BUILDING CODE (2015 IBC BASIS) 2017 NATIONAL ELECTRIC CODE

GENERAL NOTES

THE FACILITY IS LINMANNED AND NOT FOR HUMAN HABITATION A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON DRAINAGE; NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.

SITE INFORMATION

SITE NAME: SITE ID NUMBER:

WILLIMANTIC (CT06462-A)

WINDHAM, CT 06280

BLOCK: LOT: ZONE:

LATITUDE: 41° 42′ 10.84″ N 72° 13′ 17.01″ W ELEVATION:

FEMA/FIRM DESIGNATION

2.05± AC (BOOK: 0631, PAGE: 0299) ACREAGE:

CONTACT INFORMATION

APPLICANTS: EVERSOURCE ENERGY 107 SELDEN STREET BERLIN, CT 06037

PROPERTY OWNER: SBA PROPERTIES INC 8051 CONGRESS AVE BOCA RATON, FL 33487

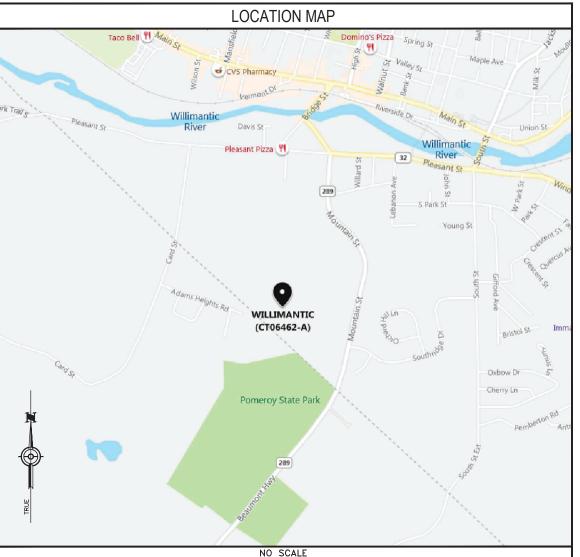
EVERSOURCE ENERGY PROJECT MANAGER: NIKOLL PRECI

(860) 655-3079

TELCO PROVIDER: FRONTIER (800) 921-8102

(800) 286-2000

CALL BEFORE YOU DIG:



DESIGN TYPE

SITE UPGRADE SELF-SUPPORT TOWER

	DRAWING INDEX		
SHEET NO:	SHEET TITLE		
T-1	TITLE SHEET		
C-1	SITE PLAN		
C-2	TOWER ELEVATION		
G-1	GROUNDING DETAILS		
N-1	NOTES & SPECIFICATIONS		
N-2	NOTES & SPECIFICATIONS		
N-3	NOTES & SPECIFICATIONS		

DO NOT SCALE DRAWINGS

SUBCONTRACTOR SHALL VERIFY ALL PLANS & EXISTING DIMENSIONS & CONDITIONS ON THE JOB SITE & SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME



SERVICE ALERT UTILITIES PROTECTION CENTER, INC.

48 HOURS BEFORE YOU DIG

EVERSURCE

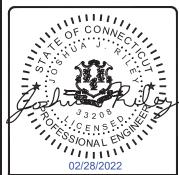
BERLIN, CT 06037 PHONE: (800) 286-2000



6800 W 115TH ST, SUITE 2292 OVERLAND PARK, KS 66211 PHONE: (913) 458-2522

	PROJECT NO:	405025
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П	CHECKED BY:	TH

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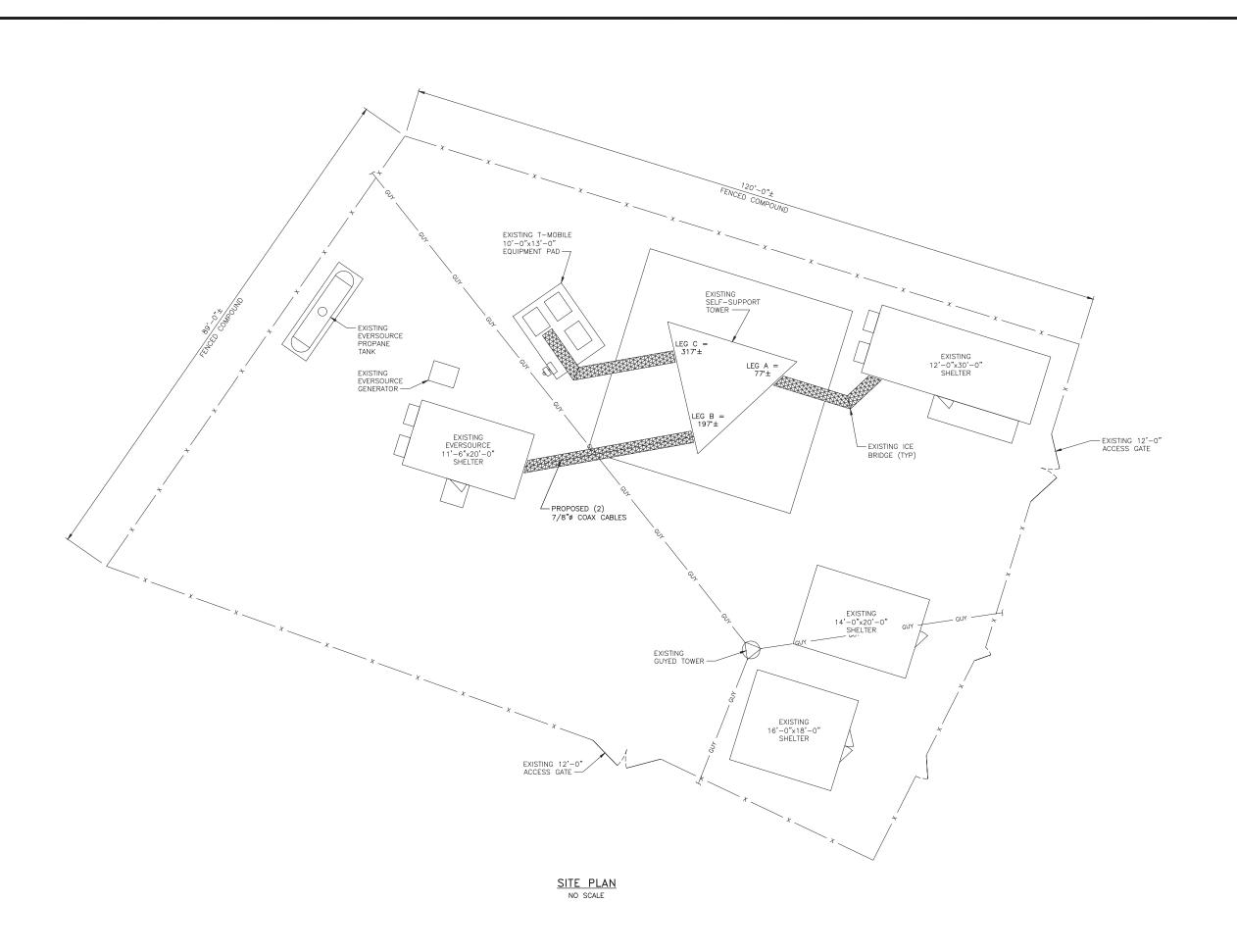
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WILLIMANTIC (CT06462-A) 349 MOUNTAIN ST WINDHAM, CT 06280

SHEET TITLE

TITLE SHEET

T-1





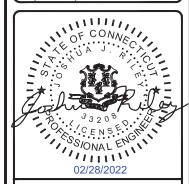
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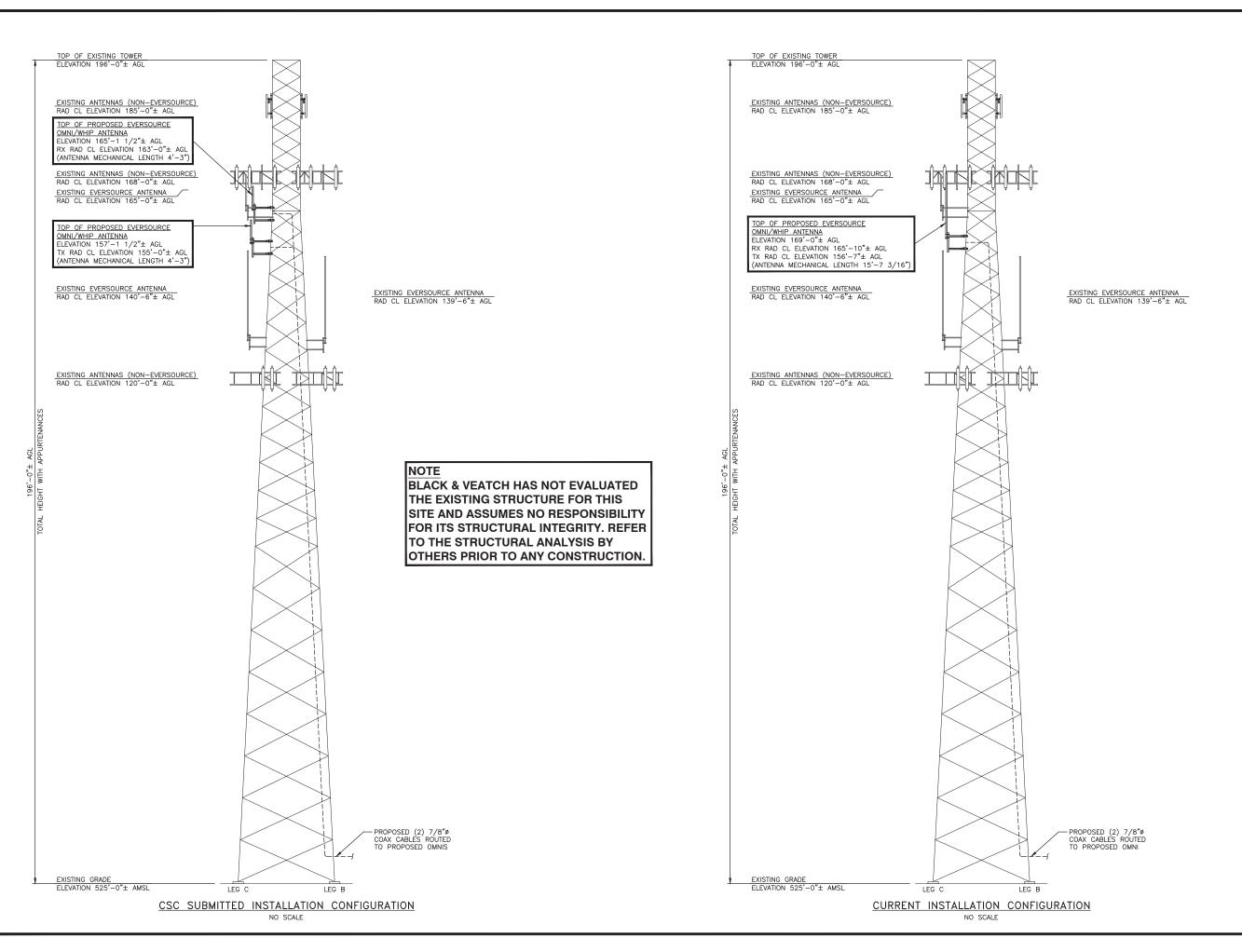
SHEET TITLE

SITE PLAN

SHEET NUMBER

C-1







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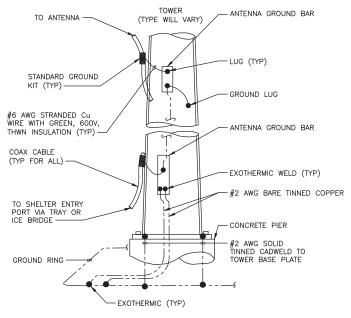
WILLIMANTIC (CT06462-A) 349 MOUNTAIN ST WINDHAM, CT 06280

SHEET TITLE

TOWER ELEVATION & ANTENNA EQUIPMENT

SHEET NUMBER

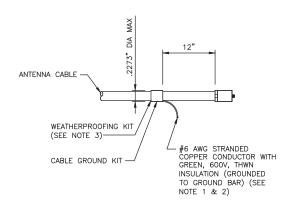
C-2



<u>NOTE</u>

NUMBER OF GROUND BARS MAY VARY DEPENDING ON THE TYPE OF TOWER, ANTENNA LOCATION AND CONNECTION ORIENTATION. PROVIDE AS REQUIRED.

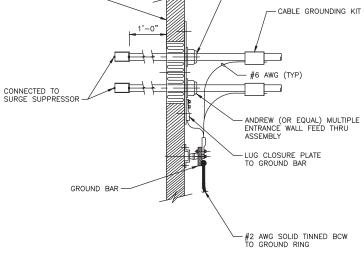
ANTENNA CABLE GROUNDING NO SCALE



NOTES

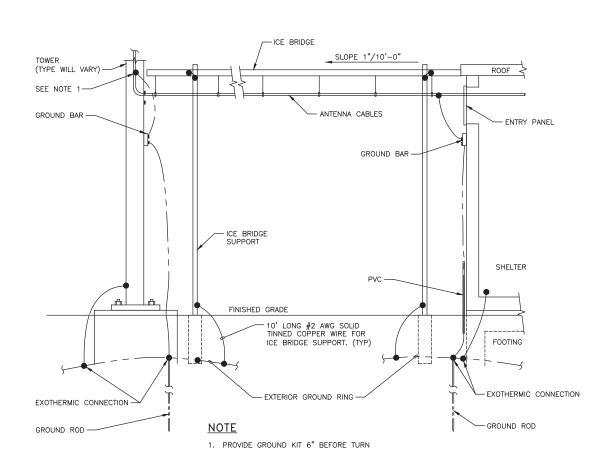
- 1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
- 2. GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
- 3. WEATHER PROOFING SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.

CONNECTION OF CABLE GROUND KIT TO ANTENNA CABLE NO SCALE

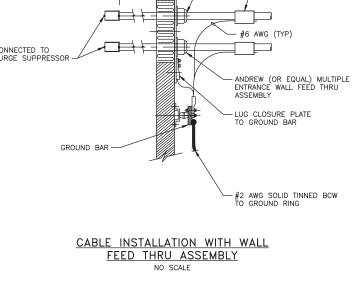


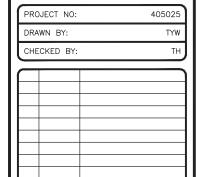
INDOOR | OUTDOOR

- ANTENNA CABLES (TYP)



ICE BRIDGE AND ANTENNA CABLE DETAIL NO SCALE





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DATE DESCRIPTION

EVERSURCE

107 SELDEN STREET BERLIN, CT 06037

PHONE: (800) 286-2000

BLACK & VEATCH

6800 W 115TH ST, SUITE 2292 OVERLAND PARK, KS 66211

PHONE: (913) 458-2522



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WILLIMANTIC (CT06462-A) 349 MOUNTAIN ST WINDHAM, CT 06280

SHEET TITLE

GROUNDING DETAILS

SHEET NUMBER

G-1

DESIGN BASIS

1. GOVERNING CODE: 2018 CONNECTICUT STATE BUILDING CODE (2015 IBC BASIS).

GENERAL CONDITIONS

- IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO COMPLY WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL BUILDING CODES, PERMIT CONDITIONS AND SAFETY CODES DURING CONSTRUCTION.
- THE ENGINEER IS NOT: A GUARANTOR OF THE INSTALLING CONTRACTOR'S WORK; RESPONSIBLE FOR SAFETY IN, ON OR ABOUT THE WORK SITE; IN CONTROL OF THE SAFETY OR ADEQUACY OF ANY BUILDING COMPONENT, SCAFFOLDING OR SUPERINTENDING THE WORK.
- THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL PERMITS, INSPECTIONS, TESTING AND CERTIFICATES NEEDED FOR LEGAL OCCUPANCY OF THE FINISHED PROJECT.
- THE CONTRACTOR IS RESPONSIBLE TO REVIEW THIS COMPLETE PLAN SET AND VERIFY THE EXISTING CONDITIONS SHOWN IN THESE PLANS AS THEY RELATE TO THE WORK PRIOR TO SUBMITTING PRICE. SIGNIFICANT DEVIATIONS FROM WHAT IS SHOWN AFFECTING THE WORK SHALL BE REPORTED IMMEDIATELY TO THE CONSTRUCTION MANAGER.
- 5. DETAILS INCLUDED IN THIS PLAN SET ARE TYPICAL AND APPLY TO SIMILAR CONDITIONS.
- EXISTING ELECTRICAL AND MECHANICAL FIXTURES, PIPING, WIRING, AND EQUIPMENT OBSTRUCTING THE WORK SHALL BE REMOVED AND/OR RELOCATED AS DIRECTED BY THE CONSTRUCTION MANAGER. TEMPORARY SERVICE INTERRUPTIONS MUST BE COORDINATED WITH OWNER.
- 7. THE CONTRACTOR SHALL DILIGENTLY PROTECT THE EXISTING BUILDING/SITE CONDITIONS AND THOSE OF ANY ADJOINING BUILDING/SITES AND RESTORE ANY DAMAGE CAUSED BY HIS ACTIVITIES TO THE
- THE CONTRACTOR SHALL SAFEGUARD AGAINST: CREATING A FIRE HAZARD, AFFECTING TENANT EGRESS OR COMPROMISING BUILDING SITE SECURITY MEASURES.
- THE CONTRACTOR SHALL REMOVE ALL DEBRIS AND CONSTRUCTION WASTE FROM THE SITE EACH DAY. WORK AREAS SHALL BE SWEPT AND MADE CLEAN AT THE END OF EACH WORK DAY.
- THE CONTRACTOR'S HOURS OF WORK SHALL BE IN ACCORDANCE WITH LOCAL CODES AND ORDINANCES AND BE APPROVED BY OWNER.
- 11. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE CONSTRUCTION MANAGER IF ASBESTOS IS ENCOUNTERED DURING THE EXECUTION OF HIS WORK. THE CONTRACTOR SHALL CEASE ALL ACTIVITIES WHERE THE ASBESTOS MATERIAL IS FOUND UNTIL NOTIFIED BY THE CONSTRUCTION

THERMAL & MOISTURE PROTECTION

- FIRE—STOP ALL PENETRATIONS FOR ELECTRICAL CONDUITS OR WAVEGUIDE CABLING THROUGH BUILDING WALLS, FLOORS, AND CEILINGS SHALL BE FIRESTOPPED WITH ACCEPTED MATERIALS TO MAINTAIN THE FIRE RATING OF THE EXISTING ASSEMBLY. ALL FILL MATERIAL SHALL BE SHAPED, FITTED, AND PERMANENTLY SECURED IN PLACE. FIRESTOPPING SHALL BE INSTALLED IN ACCORDANCE
- 2. HILTI CP620 FIRE FOAM OR 3M FIRE BARRIER FILL, VOID OR CAVITY MATERIAL OR ACCEPTED EQUAL SHALL BE APPLIED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND ASSOCIATED UNDERWRITERS LABORATORIES (UL) SYSTEM NUMBER.
- FIRESTOPPING SHALL BE APPLIED AS SOON AS PRACTICABLE AFTER PENETRATIONS ARE MADE AND EQUIPMENT INSTALLED.
- FIRESTOPPED PENETRATIONS SHALL BE LEFT EXPOSED AND MADE AVAILABLE FOR INSPECTION BEFORE CONCEALING SUCH PENETRATIONS. FIRESTOPPING MATERIAL CERTIFICATES SHALL BE MADE AVAILABLE AT THE TIME OF INSPECTION.
- 5. ANY BUILDING ROOF PENETRATION AND/OR RESTORATION SHALL BE PERFORMED SO THAT THE ROOF WARRANTY IN PLACE IS NOT COMPROMISED. CONTRACTOR SHALL ARRANGE FOR OWNER'S ROOFING CONTRACTOR TO PERFORM ANY AND ALL ROOFING WORK IF SO REQUIRED BY EXISTING ROOF WARRANTY. OTHERWISE, ROOF SHALL BE MADE WATERTIGHT WITH LIKE CONSTRUCTION AS SOON AS PRACTICABLE AND AT COMPLETION OF CONSTRUCTION.
- ALL PENETRATIONS INTO AND/OR THROUGH BUILDING EXTERIOR WALLS SHALL BE SEALED WITH
- WHERE CONDUIT AND CABLES PENETRATES FIRE RATED WALLS AND FLOORS, FIRE GROUT ALL PENETRATIONS IN ORDER TO MAINTAIN THE FIRE RATING USING A LISTED FIRE SEALING DEVICE OR
- 8. CONTRACTOR TO REMOVE AND RE-INSTALL ALL FIRE PROOFING AS REQUIRED DURING

SUBMITTALS

- 1. CONTRACTOR TO SUBMIT SHOP DRAWINGS TO ENGINEER FOR REVIEW PRIOR TO FABRICATION.
- 2. CONTRACTOR TO NOTIFY ENGINEER FOR INSPECTION PRIOR TO CLOSING PENETRATIONS.
- CONTRACTORS SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. THE ENGINEER SHALL BE NOTIFIED OF ANY CONDITIONS WHICH PRECLUDE COMPLETION OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- ALL STEEL MATERIAL EXPOSED TO WEATHER SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 " ZINC (HOT-DIPPED GALVANIZED) COATINGS" ON IRON AND STEEL
- 5. THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS FOR REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW.

STEEL

MATERIAL

WIDE FLANGE: ASTM A572, GR 50

TUBING: ASTM A500, GR C

ASTM A53, GR B AND ASTM A572, GR 50 ANGLE: ASTM A570, GR 50 AND ASTM A36

ASTM A325 BOLTS:

GRATING: TYPE GW-2 (1"x3/16" BARS)

MISC. MATERIAL: ASTM A36

ALL STEEL SHAPES SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 WITH A COATING WEIGHT OF 2 07/SE.

- DAMAGED GALVANIZED SURFACES SHALL BE CLEANED WITH A WIRE BRUSH AND PAINTED WITH TWO
 COATS OF COLD ZINC, "GALVANOX", "DRY GALV", "ZINC IT", OR APPROVED EQUIVALENT, IN
 ACCORDANCE WITH MANUFACTURER'S GUIDELINES. TOUCH UP DAMAGED NON GALVANIZED STEEL WITH
 ACCORDANCE WITH MANUFACTURER'S GUIDELINES.
- 3. DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AISC "MANUAL OF STEEL CONSTRUCTION" 13TH EDITION.
- THE STEEL STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER COMPLETION. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION.
- 5. ALL STEEL ELEMENTS SHALL BE INSTALLED PLUMB AND LEVEL.
- 6. TOWER MANUFACTURER'S DESIGNS SHALL PREVAIL FOR TOWER

SITE GENERAL

- CONTRACTOR SHALL FOLLOW CONDITIONS OF ALL APPLICABLE PERMITS AND WORK IN ACCORDANCE WITH OSHA REGULATIONS.
- 2. THESE PLANS DEPICT KNOWN UNDERGROUND STRUCTURES, CONDUITS, AND/OR PIPELINES. THE LOCATIONS FOR THESE ELEMENTS ARE BASED UPON THE VARIOUS RECORD DRAWINGS AVAILABLE. THE CONTRACTOR IS HEREBY ADVISED THAT THESE DRAWINGS MAY NOT ACCURATELY DEPICT AS—BUILT LOCATIONS AND OTHER UNKNOWN STRUCTURES. THE CONTRACTOR SHALL THEREFORE DETERMINE THE EXACT LOCATION OF EXISTING UNDERGROUND ELEMENTS AND EXCAVATE WITH CARE AFTER CALLING MARKOUT SERVICE AT 1-800-272-4480 48 HOURS BEFORE DIGGING, DRILLING OR
- ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, FIBER OPTIC, AND OTHER UTILITIES WHERE ENCOUNTERED, SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION, SHALL BE RELOCATED AS DIRECTED BY ENGINEER. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR PIER DRILLING AROUND OR NEAR UTILITIES. CONTRACTOR SHALL HAND DIG UTILITIES AS NEEDED, CONTRACTOR SHALL PROVIDE BUT IS NOT LIMITED TO, APPROPRIATE A) FALL PROTECTION, B) CONFINED SPACE ENTRY, C) ELECTRICAL SAFETY, AND D) TRENCHING AND EXCAVATION
- 4. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES, AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- 5. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC, FIBER OPTIC, OR OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED, AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT THE POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF THE CONSTRUCTION MANAGER.
- 6. CONTRACTOR IS RESPONSIBLE FOR REPAIRING OR REPLACING STRUCTURES OR UTILITIES DAMAGED
- CONTRACTOR SHALL PROTECT EXISTING PAVED AND GRAVEL SURFACES, CURBS, LANDSCAPE AND STRUCTURES AND RESTORE SITE OR PRE-CONSTRUCTION CONDITION WITH AS GOOD, OR BETTER, MATERIALS. NEW MATERIALS SHALL MATCH EXISTING THICKNESS AND TYPE.
- THE CONTRACTOR SHALL SHORE ALL TRENCH EXCAVATIONS GREATER THAN 5 FEET IN DEPTH OR LESS WHERE SOIL CONDITIONS ARE DEEMED UNSTABLE. ALL SHEETING AND/OR SHORING METHODS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER.
- THE CONTRACTOR IS RESPONSIBLE FOR MANAGING GROUNDWATER LEVELS IN THE VICINITY OF EXCAVATIONS TO PROTECT ADJACENT PROPERTIES AND NEW WORK. GROUNDWATER SHALL BE DRAINED IN ACCORDANCE WITH LOCAL SEDIMENTATION AND EROSION CONTROL GUIDELINES.



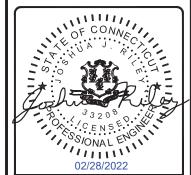
107 SELDEN STREET BERLIN, CT 06037 PHONE: (800) 286-2000



6800 W 115TH ST, SUITE 2292 OVERLAND PARK, KS 66211 PHONE: (913) 458-2522

PROJECT NO:	405025
DRAWN BY:	TYW
CHECKED BY:	TH

1	12/10/21	ISSUED FOR FILING
0	06/18/20	ISSUED FOR FILING
REV	DATE	DESCRIPTION



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WILLIMANTIC (CT06462-A) 349 MOUNTAIN ST WINDHAM, CT 06280

SHEET TITLE

NOTES & SPECIFICATIONS

SHEET NUMBER

N-1

ELECTRICAL

- CONTRACTOR SHALL VERIFY EXISTING ELECTRIC SERVICE TYPE AND CAPACITY AND ORDER NEW ELECTRIC SERVICE FROM LOCAL ELECTRIC UTILITY, WHERE APPLICABLE.
- ALL ELECTRICAL WORK SHALL BE IN ACCORDANCE WITH ALL APPLICABLE CODES, AND SHALL BE
 ACCEPTABLE TO ALL AUTHORITIES HAVING JURISDICTION. WHERE A CONFLICT EXISTS BETWEEN CODES,
 PLAN AND SPECIFICATIONS, OR AUTHORITIES HAVING JURISDICTION, THE MORE STRINGENT
 AUTHORITIES SHALL APPLY.
- CONTRACTOR SHALL PROVIDE ALL LABOR, MATERIALS, INSURANCE, EQUIPMENT, INSTALLATION, CONSTRUCTION TOOLS, TRANSPORTATION, ETC, FOR A COMPLETE AND PROPERLY OPERATIVE SYSTEM ENERGIZED THROUGHOUT AND AS INDICATED ON THE DRAWINGS AND AS SPECIFIED HEREIN AND/OR OTHERWISE REQUIRED.
- 4. ALL ELECTRICAL CONDUCTORS SHALL BE 100% COPPER AND SHALL HAVE TYPE THHN INSULATION UNLESS INDICATED OTHERWISE.
- CONDUIT SHALL BE THREADED RIGID GALVANIZED STEEL OR EMT WITH ONLY COMPRESSION TYPE COUPLINGS AND CONNECTORS. ALL MADE UP WRENCH TIGHT.
- ALL BURIED CONDUIT SHALL BE MINIMUM SCH 40 PVC UNLESS NOTED OTHERWISE, OR AS PER LOCAL CODE REQUIREMENTS.
- . PROVIDE FLEXIBLE STEEL CONDUIT OR LIQUID TIGHT FLEXIBLE STEEL CONDUIT TO ALL VIBRATING EQUIPMENT, INCLUDING HVAC UNITS, TRANSFORMERS, MOTORS, ETC, OR WHERE EQUIPMENT IS PLACED UPON A SLAB ON GRADE.
- 8. ALL BRANCH CIRCUITS AND FEEDERS SHALL HAVE A SEPARATE GREEN INSULATED EQUIPMENT GROUNDING CONDUCTOR BONDED TO ALL ENCLOSURES, PULLBOXES, ETC.
- 9. CONDUIT AND CABLE WITHIN CORRIDORS SHALL BE CONCEALED AND EXPOSED ELSEWHERE, UNLESS
- 10. ELECTRICAL MATERIALS INSTALLED ON ROOFTOP SHALL BE LISTED FOR NEMA 3R USE. —AND ALL WIRING WITHIN A VENTILATION DUCT SHALL BE LISTED FOR SUCH USE. IN GENERAL WIRING METHODS WITHIN A DUCT SHALL BE AN MC CABLE WITH SMOOTH OR CORRUGATED METAL JACKET AND HAVE NO OUTER COVERING OVER THE METAL JACKET. INTERLOCKED ARMOR TYPE OF MC CABLE IS NOT ACCEPTABLE FOR THIS APPLICATION. CONTRACTOR CAN ALSO USE TYPE MI CABLE IN THE VENTILATION DUCT PROVIDED IT DOES NOT HAVE ANY OUTER COVERINGS OVER THE METAL EXTERIOR.
- 11. WIRING DEVICES SHALL BE SPECIFICATION GRADE, AND WIRING DEVICE COVER PLATES SHALL BE PLASTIC WITH ENGRAVING AS SPECIFIED.

GROUNDING

- 1. #6 THWN SHALL BE STRANDED #6 COPPER WITH GREEN THWN INSULATION SUITABLE FOR WET
- 2. #2 THWN SHALL BE STRANDED #2 COPPER WITH THWN INSULATION SUITABLE FOR WET
- 3. #2 BARE TINNED SHALL BE SOLID COPPER TINNED. ALL BURIED WIRE SHALL MEET THIS CRITERIA.
- ALL LUGS SHALL BE 2-HOLE, LONG BARREL, TINNED SOLID COPPER UNLESS OTHERWISE SPECIFIED, LUGS SHALL BE THOMAS AND BETTS SERIES 548##BE OR EQUIVALENT (IE #2 THWN - 54856BE, #2 SOLID - 54856BE, AND #6 THWN - 54852BE).
- 5. ALL HARDWARE, BOLTS, NUTS, AND WASHERS SHALL BE 18-8 STAINLESS STEEL. EVERY CONNECTION SHALL BE BOLT-FLAT WASHER-BUSS-LUG-FLAT WASHER-BELLEVILLE WASHER-NUT IN THAT EXACT ORDER. BACK-TO-BACK LUGGING, BOLT-FLAT WASHER-LUG-BUSS-LUG-FLAT WASHER-BELLEVILLE WASHER-NUT, IN THAT EXACT ORDER, IS ACCEPTED WHERE NECESSARY TO CONNECT MANY LUGS TO A BUSS BAR. STACKING OF LUGS, BUSS-LUG-LUG, IS NOT ACCEPTABLE.
- WHERE CONNECTIONS ARE MADE TO STEEL OR DISSIMILAR METALS, A THOMAS AND BETTS DRAGON TOOTH WASHER MODEL DTWXXX SHALL BE USED BETWEEN THE LOG AND THE STEEL, BOLT-FLAT WASHER-STEEL-DRAGON TOOTH WASHER-LUG-FLAT WASHER-BELEVILE WASHER-NUT.
- ALL CONNECTIONS, INTERIOR AND EXTERIOR, SHALL BE MADE WITH THOMAS AND BETTS
 KPOR-SHIELD. COAT ALL WIRES BEFORE LUGGING AND COAT ALL SURFACES BEFORE CONNECTING.
- THE MINIMUM BEND RADIUS SHALL BE 8 INCHES FOR #6 WIRE AND SMALLER AND 12 INCHES FOR WIRE LARGER THAN #6.
- 9. ALL CONNECTIONS TO THE GROUND RING SHALL BE EXOTHERMIC WELD.
- 10. BOND THE FENCE TO THE GROUND RING AT EACH CORNER, AND AT EACH GATE POST WITH #2 SOLID TINNED WIRE. EXOTHERMIC WELD BOTH ENDS.
- 11. GROUND KITS SHALL BE SOLID COPPER STRAP WITH #6 WIRE 2-HOLE COMPRESSION CRIMPED LUGS AND SHALL BE SEALED ACCORDING TO MANUFACTURER INSTRUCTIONS.
- 12. FERROUS METAL CLIPS WHICH COMPLETELY SURROUND THE GROUNDING CONDUCTOR SHALL BE USED.
- 13. GROUND BARS SHALL BE FURNISHED AND INSTALLED WITH PRE-DRILLED HOLE DIAMETERS AND SPACINGS. GROUND BARS SHALL NEITHER BE FIELD FABRICATED NOR NEW HOLES DRILLED. GROUND LUGS SHALL MATCH THE SPACING ON THE BAR. HARDWARE DIAMETER SHALL BE MINIMUM 3.8 INCH.
- 14. MGB GROUND CONNECTION SHALL BE EXOTHERMIC WELDED TO THE GROUND SYSTEM.
- ALL CABLE TRAY AND/OR PLATFORM STEEL SHALL BE BONDED TOGETHER WITH JUMPERS (#6 IN EQUIPMENT ROOM, #2 ELSEWHERE AND HOMERUN).

ANTENNA & CABLE NOTES

- 1. THE CONTRACTOR SHALL FURNISH AND INSTALL ALL TRANSMISSION CABLES, JUMPERS, CONNECTORS, GROUNDING STRAPS, ANTENNAS, MOUNTS AND HARDWARE. ALL MATERIALS SHALL BE INSPECTED BY THE CONTRACTOR FOR DAMAGE UPON DELIVERY. JUMPERS SHALL BE SUPPLIED AT ANTENNAS AND EQUIPMENT INSIDE SHELTER COORDINATE LENGTH OF JUMP CABLES WITH EVERSOURCE. COORDINATE AND VERIEY ALL OF THE MATERIALS TO BE PROVIDED WITH EVERSOURCE PRIOR TO SUBMITTING BID AND OPDERING MATERIALS.
- AFTER INSTALLATION, THE TRANSMISSION LINE SYSTEM SHALL BE PIM/SWEEP TESTED FOR PROPER INSTALLATION AND DAMAGE WITH ANTENNAS CONNECTED. CONTRACTOR TO OBTAIN LATEST TESTING PROCEDURES FROM EVERSOURCE PRIOR TO BIDDING.
- 3. ANTENNA CABLES SHALL BE COLOR CODED AT THE FOLLOWING LOCATIONS:
 - AT THE ANTENNAS.
 - AT THE WAVEGUIDE ENTRY PLATE ON BOTH SIDES OF THE EQUIPMENT SHELTER WALL.
 - JUMPER CABLES AT THE EQUIPMENT ENTER.
- 4. SYSTEM INSTALLATION:
- THE CONTRACTOR SHALL INSTALL ALL CABLES AND ANTENNAS TO THE MANUFACTURER'S SPECIFICATIONS. THE CONTRACTOR IS RESPONSIBLE FOR THE PROCUREMENT AND INSTALLATION OF THE FOIL OWING:
- ALL CONNECTORS, ASSOCIATED CABLE MOUNTING, AND GROUNDING HARDWARE.
- WALL MOUNTS, STANDOFFS, AND ASSOCIATED HARDWARE.
- 1/2 INCH HELIAX ANTENNA JUMPERS OF APPROPRIATE LENGTHS.
- 5. MINIMUM BENDING RADIUS FOR COAXIAL CABLES:
 - 7/8 INCH, RMIN = 15 INCHES
 - 1 5/8 INCH, RMIN = 25 INCHES
- 6. CABLE SHALL BE INSTALLED WITH A MINIMUM NUMBER OF BENDS WHERE POSSIBLE. CABLE SHALL NOT BE LEFT UNTERMINATED AND SHALL BE SEALED IMMEDIATELY AFTER BEING INSTALLED.
- 7. ALL CABLE CONNECTIONS OUTSIDE SHALL BE COVERED WITH WATERPROOF SPLICING KIT.
- 8. CONTRACTOR SHALL VERIFY EXACT LENGTH AND DIRECTION OF TRAVEL IN FIELD PRIOR TO CONSTRUCTION.
- 9. CABLE SHALL BE FURNISHED WITHOUT SPLICES AND WITH CONNECTORS AT EACH END.



107 SELDEN STREET BERLIN, CT 06037 PHONE: (800) 286-2000



6800 W 115TH ST, SUITE 2292 OVERLAND PARK, KS 66211 PHONE: (913) 458-2522

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WILLIMANTIC (CT06462-A) 349 MOUNTAIN ST WINDHAM, CT 06280

SHEET TITLE

NOTES & SPECIFICATIONS

SHEET NUMBER

N-2

<u>SYMBOLS</u>

EXOTHERMIC CONNECTION COMPRESSION CONNECTION 5/8"øx10-'0" COPPER CLAD STEEL GROUND ROD. TEST GROUND ROD WITH INSPECTION SLEEVE GROUNDING CONDUCTOR \bigcirc A KEY NOTES CHAINLINK FENCE WOOD FENCE LEASE AREA ICE BRIDGE CABLE TRAY GAS LINE UNDERGROUND ELECTRICAL/TELCO UNDERGROUND ELECTRICAL/CONTROL UNDERGROUND ELECTRICAL UNDERGROUND TELCO PROPERTY LINE (PL)

ABBREVIATIONS

<u>ABB</u>	REVIATIONS		
AC	ALTERNATING CURRENT	MGB	MASTER GROUNDING BAR
AIC	AMPERAGE INTERRUPTION CAPACITY	MIN	MINIMUM
ANI	AUXILIARY NETWORK INTERFACE	MW	MICROWAVE
ATM	ASYNCHRONOUS TRANSFER MODE	MTS	MANUAL TRANSFER SWITCH
ATS	AUTOMATIC TRANSFER SWITCH	NEC	NATIONAL ELECTRICAL CODE
AWG	AMERICAN WIRE GAUGE	ОС	ON CENTER
AWS	ADVANCED WIRELESS SERVICES	PP	POLARIZING PRESERVING
BATT	BATTERY	PCU	PRIMARY CONTROL UNIT
BBU	BASEBAND UNIT	PDU	PROTOCOL DATA UNIT
BTC	BARE TINNED COPPER CONDUCTOR	PWR	POWER
BTS	BASE TRANSCEIVER STATION	RECT	RECTIFIER
CCU	CLIMATE CONTROL UNIT	RET	REMOTE ELECTRICAL TILT
CDMA	CODE DIVISION MULTIPLE ACCESS	RMC	RIGID METALLIC CONDUIT
CHG	CHARGING	RF	RADIO FREQUENCY
CLU	CLIMATE UNIT	RUC	RACK USER COMMISSIONING
СОММ	COMMON	RRH	REMOTE RADIO HEAD
DC	DIRECT CURRENT	RRU	REMOTE RADIO UNIT
DIA	DIAMETER	RWY	RACEWAY
DWG	DRAWING	SFP	SMALL FORM-FACTOR PLUGGABLE
EC	ELECTRICAL CONDUCTOR	SIAD	SMART INTEGRATED ACCESS DEVICE
EMT	ELECTRICAL METALLIC TUBING	SSC	SITE SOLUTIONS CABINET
FIF	FACILITY INTERFACE FRAME	T1	1544KBPS DIGITAL LINE
GEN	GENERATOR	TDMA	TIME-DIVISION MULTIPLE ACCESS
GPS	GLOBAL POSITIONING SYSTEM	TMA	TOWER MOUNT AMPLIFIER
GSM	GLOBAL SYSTEM FOR MOBILE	TVSS	TRANSIENT VOLTAGE SUPPRESSION SYSTEM
HVAC	HEAT/VENTILATION/AIR CONDITIONING	TYP	TYPICAL
ICF	INTERCONNECTION FRAME	UMTS	UNIVERSAL MOBILE TELECOMMUNICATION SYSTEM
IGR	INTERIOR GROUNDING RING (HALO)	UPS	UNINTERRUPTIBLE POWER SUPPLY (DC POWER PLANT)
LTE	LONG TERM EVOLUTION		(: -::-:: 5)



107 SELDEN STREET BERLIN, CT 06037 PHONE: (800) 286–2000



6800 W 115TH ST, SUITE 2292 OVERLAND PARK, KS 66211 PHONE: (913) 458–2522

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WILLIMANTIC (CT06462-A) 349 MOUNTAIN ST WINDHAM, CT 06280

SHEET TITLE

NOTES & SPECIFICATIONS

SHEET NUMBER

N-3

REFERENCE CUTSHEETS





220 MHz Antenna – Omnidirectional, Low-PIM/Hi-PIP, Unity Gain Models - SP2D00P36D-D

Specifications					
Design Type	True Corporate Feed				
Frequency Range	217-220 MHz				
Passive Intermodulation – PIM (2 x 20W sources)	-150 dBc, 3 rd Order				
Bandwidth	3 MHz				
Gain - dBd (average over BW)	0 dBd				
Isolation, min.	40 dB				
Configuration	Dual antenna				
Beam Tilt (electrical down-tilt)	None (0°)				
Vertical Beamwidth (E-Plane)	60°				
Impedance Ohms	50				
VSWR / Return Loss dB	1.5 : 1 / 14 dB (min.)				
Average Power Rating	500 W (each antenna)				
Peak Instantaneous Power	25 kW (each antenna)				
Polarization	Vertical				
Lightning Protection	Direct Ground				
Connector	7/16 DIN female				
Equivalent Flat-Plate Area	2.59 sq. ft.				
Lateral Wind-load Thrust @100mph	109 lbf.				
Wind Speed rating	160 mph (without ice) 136 mph (½" radial ice)				
Total Length	15.6 feet				
Mounting Mast Length	35 inches				
Mounting Hardware (Included)	DSH3V4N				
Top Sway Brace	DSH2H3S				
(Recommended if side mounting antennas)	(order separately)				
Mast O.D.	3.5 inches				
Radome color	Horizon Blue				
Radome O.D.	3.0 inches				
Weight, antenna, and hardware	45 lbs. (approx.)				
Shipping Weight	80 lbs. (approx.)				
Invertibility	Antennas are physically invertible, but the patterns are optimized for upright mount.				



Features and Benefits

Antennas from dbSpectra provide long term, trouble-free service in severe environments!

Design is tested to stringent Peak Instantaneous
Power (PIP) levels of 25 KW using dbSpectra's
12-channel P25 PIP test bed. High PIP level is
demanded by today's digital systems.

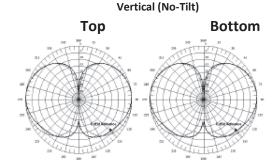
True Corporate Feed Array – provides for excellent gain and pattern consistency across a wider frequency range.

PIM Rated Design – better than -150 dBc.

Sturdy Construction – Heavy-wall fiberglass radome minimizes tip deflection.

Excellent Lightning Protection – heavy internal conductor DC ground.

Radiation Pattern





ORIGINALLY PROPOSED ANTENNAS, REMOVED OR REPLACED

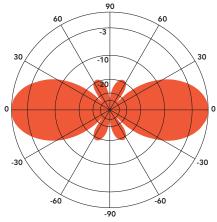
ANT220F2DIN

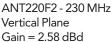
FIBERGLASS COLLINEAR ANTENNA 2.5 dBd

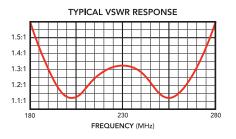
The Telewave ANT220F2 is an extremely rugged collinear antenna, with moderate gain and wide vertical beamwidth. This compact antenna produces 2.5 dBd gain, and is designed for operation in all environmental conditions. The antenna is constructed with brass and copper elements, with a path to ground potential for lightning impulse protection. The ANT220F2 is an excellent choice for wireless PTC systems in urban or rural areas.

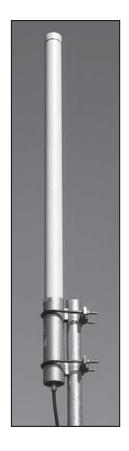
All junctions are fully soldered to prevent RF intermodulation, and each antenna is completely protected within a rugged, high-tech radome to ensure survivability in the worst environments. The "Cool Blue" radome provides maximum protection from corrosive gases, ultraviolet radiation, icing, salt spray, acid rain, and wind blown abrasives.

The ANT220F2 includes the ANTC485 dual clamp set for mounting to a 1.5" to 3" O.D. support pipe, and a 24" removable RG-213 DIN-Male jumper.



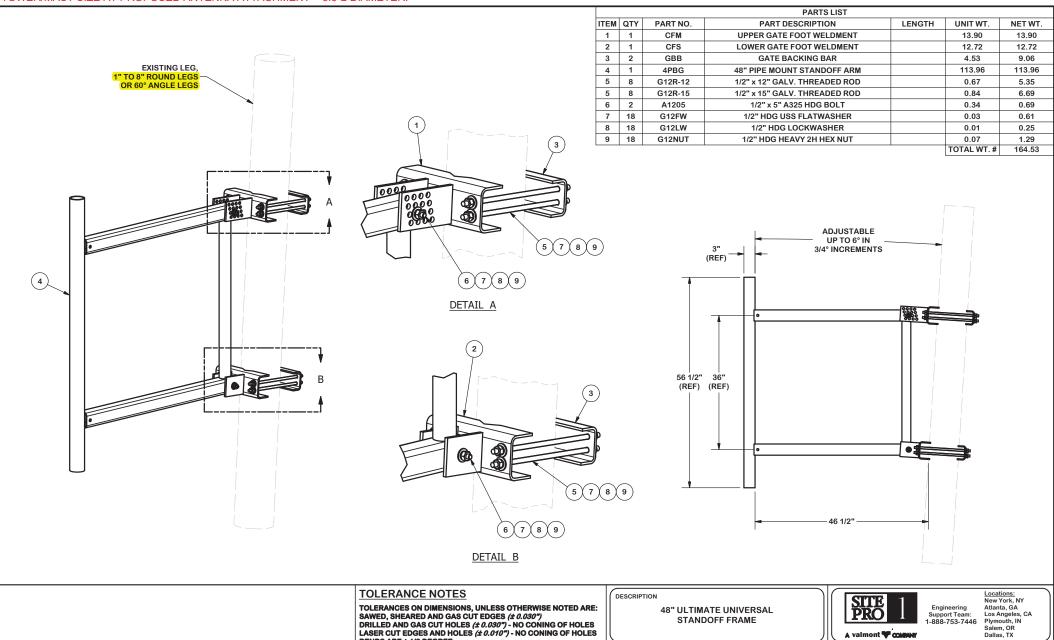






ONE SITE PRO 1 P/N DCP12K CLAMP SET REQUIRED.

SPECIFICATIONS			
Frequency (continuous)	195-260 MHz	Dimensions (L x base diam.) in.	51 x 2.75
Gain	2.5 dBd	Tower weight (antenna + clamps)	11 lb.
Power rating (typ.)	500 watts	Shipping weight	14 lb.
Impedance	50 ohms	Wind rating / with 0.5" ice	200 / 150 MPH
VSWR	1.5:1 or less	Maximum exposed area	1.1 ft. ²
Pattern	Omnidirectional	Lateral thrust at 100 MPH	44 lb.
Vertical beamwidth	38°	Bending moment at top clamp	47 ft. lb.
Termination	7-16 DIN-F	(100 MPH, 40 PSF flat plate equiv.)	



CPD NO.

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CHECKED BY

BMC 2/16/2011

BENDS ARE ± 1/2 DEGREE ALL OTHER MACHINING (± 0.030")

ALL OTHER ASSEMBLY (± 0.060")

PROPRIETARY NOTE:
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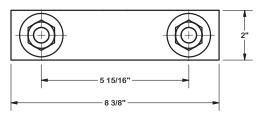
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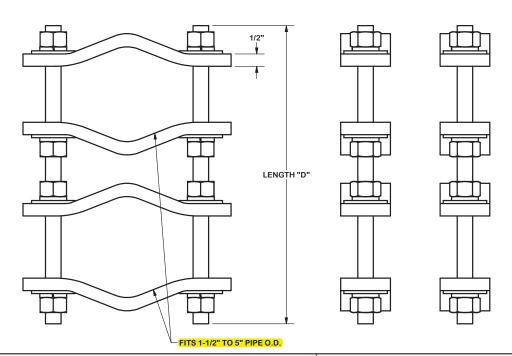
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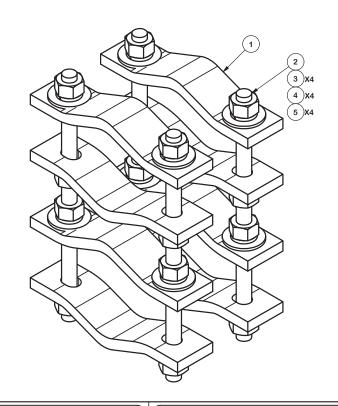






ITEM	ITEM QTY PART NO. PART DESCRIPTION				UNIT WT.	NET WT.
1	1 8 DCP CLAMP HALF, 1/2" THICK, 8-3/8"				2.40	19.20
2	В	С	5/8" THREADED ROD	D	E	F
3	3 16 G58NUT 5/8" HDG HEAVY 2H HEX NUT			0.13	2.08	
4	4 16 G58LW 5/8" HDG LOCKWASHER			0.03	0.42	
5	16	G58FW	5/8" HDG USS FLATWASHER		0.07	1.13

VARIABLE PARTS TABLE								
ASSEMBLY "A"	QTY "B"	PART "C"	LENGTH "D"	UNIT WT. "E"	NET WT. "F"	TOTAL WEIGHT		
DCP12K	4	G58R-12	12"	1.05	4.18	27.01		
DCP18K	4	G58R-18	18"	1.57	6.27	29.10		



TOLERANCE NOTES

TOLERANCES ON DIMENSIONS, UNLESS OTHERWISE NOTED ARE: SAWED, SHEARED AND GAS CUT EDGES (\$ 0.030") DRILLED AND GAS CUT HOLES (\$ 0.030") - NO CONING OF HOLES LASER CUT EDGES AND HOLES (\$ 0.010") - NO CONING OF HOLES

BENDS ARE ± 1/2 DEGREE

ALL OTHER MACHINING (± 0.030") ALL OTHER ASSEMBLY (± 0.060")

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DESCRIPTION

PIPE TO PIPE CLAMP SET 1-1/2" TO 5" PIPE 1/2" THICK CLAMP



Locations: New York, NY Atlanta, GA Los Angeles, CA Plymouth, IN Salem, OR Dallas, TX Engineering Support Team: 1-888-753-7446

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Tower Engineering Solutions

Phone (972) 483-0607, Fax (972) 975-9615 1320 Greenway Drive, Suite 600, Irving, Texas 75038

Structural Analysis Report

Existing 196 ft Rohn Self Supporting Tower

Customer Name: SBA Communications Corp

Customer Site Number: CT06462-A-2

Customer Site Name: Mountain Street

Carrier Name: Connecticut Light & Power (App#: 186494, V1)

Carrier Site ID / Name: ES-055 / Willimantic

Site Location: 349 Mountain Street

Windham, Connecticut

Windham County

Latitude: 41.703011

Longitude: -72.221391

Analysis Result:

Max Structural Usage: 53.6% [Pass]

Max Foundation Usage: 44.0% [Pass]

Additional Usage Caused by New Mount/Mount Modification: N/A

Report Prepared By: Sital Shrestha



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Report Prepared By: Sital Shrestha

Introduction

The purpose of this report is to summarize the analysis results on the 196 ft Rohn Self Supporting Tower to support the proposed antennas and transmission lines in addition to those currently installed. Any modification listed under Sources of Information was assumed completed and was included in this analysis.

Sources of Information

Tower Drawings	Original fabrications drawings prepared by ROHN Industries, Inc. Dated 09-27-2001. Drawing No C011214. Eng. File No 49204TT. Previous structural report prepared by FDH Velocitel. Dated 05-10-2017. Project No 17QEIQ1400.
Foundation Drawing	Original foundation drawings prepared by ROHN Industries, Inc. Dated 08-31-2001.
	Drawing No A012046-1. Eng. File No 49204TT.
Geotechnical Report	Geotechnical report prepared by BL Companies. Dated 12-01-2000. Project No
	00C672-C.
Modification Drawings	N/A

Analysis Criteria

The comprehensive analysis was performed in accordance with the requirements and stipulations of the TIA-222-H. In accordance with this standard, the structure was analyzed using **TESTowers**, a proprietary analysis software. The program considers the structure as an elastic 3-D model with second-order effects and temperature effects incorporated in the analysis. The analysis was performed using multiple wind directions.

Wind Speed Used in the Analysis: 121.0 mph (3-Sec. Gust) (Ultimate wind speed)
Wind Speed with Ice: 50 mph (3-Sec. Gust) with 1" radial ice concurrent

Service Load Wind Speed: 60 mph + 0" Radial ice

Standard/Codes: TIA-222-H / 2018 IBC / 2018 Connecticut State Building Code

Exposure Category: B
Risk Category: II
Topographic Category: 1
Crest Height: 0 ft

Seismic Parameters: $S_S = 0.192, S_1 = 0.055$

This structural analysis is based upon the tower being classified as a Risk Category II; however, if a different classification is required subsequent to the date hereof, the tower classification will be changed to meet such requirement and a new structural analysis will be run.

Existing Antennas, Mounts and Transmission Lines

The table below summarizes the antennas, mounts and transmission lines that were considered in the analysis as existing on the tower.

Items	Elevation (ft.)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner
1	1		Antel BXA-80080/4CF - Panel	Direct	/2\ 1 F /0"	Vorizon
2 185.0		6	Rfs Celwave FD9R6004/2C-3L Diplexers	Direct	(3) 1 5/8"	Verizon
3		3	Ericsson AIR21-6449 B41 - Panel			
4		3	RFS APXVAARR24_43-U-NA20 - Panel			
5		3	Ericsson AIR32 KRD901146-1 - Panel		(9) 1 5/8" Coax	Verizon/T-
6	169.0	3	72" x 12" x 6" Panel	(3) 10' T Frames	(2) 1 5/8"	Mobile
7	169.0 	3	Ericsson KRY11271 TMA's	(3) 10 111011163	Hybrid	(A-11)1
8		3	Commscope SDX192 6Q-43 Diplexers		Trybria	(/ 11)1
9		3	Ericsson 4449 B71 + B85 RRU's			
10		3	Ericsson 4415 B25 RRU's			
-	167.0	1	Commscope DB586-Y Omni	(1) Sidearm (Commscope S-200)		
-	166.5	1	RFS 458-2 Omni	(1) Sidearm (Commscope S-400)		
-	165.0	1	RFS BA1312-0 Omni	(1) Sidearm		
-	164.0	1	Powerwave LGP104 TMA	(Commscope S-400)		
-	163.0	1	Telewave ANT220F2 Omni	(1) Sidearm (Site Pro USF-4U)	- (8) 7/8" Coax	Connecticut
-	155.0	1	Telewave ANT220F2 Omni	(1) Sidearm (Site Pro USF-4U)	(1) 1/2" Coax	Light & Power
-	140.4	1	RFS 220-3AN Omni	(1) 6' Sidearm (Commscope S-600)		
-	139.5	1	RFS 220-7N Omni	(1) 4' Sidearm (Wireless Solutions WS-S400)		
-	137.0	1	Kreco CO-36A Omni	(1) 6' Sidearm (Commscope S-600)		
19		2	Commscope sbnhh-1d45b - Panel			
20		4	Commscope SBNHH-1D65B - Panel			
21		3	Samsung MT6407-77A - Panel			
22	120.0	3	Samsung B2/B66A RRH-BR049 (RFV01U-D1A) RRU's	(3) 10' T-Frames	(8) 1 5/8" (2) 1 5/8" Fiber	Verizon
23		3	Samsung B5/B13 RRH-BR04C (RFV01U-D2A) RRU's			
24		2	Rfs Celwave DB-T1-6Z-8AB-0Z Junction Box			

¹ Verizon has a separate lease under (A-09).

Proposed Carrier's Final Configuration of Antennas, Mounts and Transmission Lines

Information pertaining to the proposed carrier's final configuration of antennas and transmission lines was provided by SBA Communications Corp. The proposed antennas and lines are listed below.

Items	Elevation (ft)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner
11	167.0	1	Commscope DB586-Y -Omni	(1) Sidearm (Commscope S-200)		
12	166.5	1	RFS 458-2-Omni	(1) Sidearm (Commscope S-400)		
13	165.0	1	RFS BA1312-0- Omni	(1) Sidoorm (Commono S 400)		
14	164.0	1	Powerwave LGP104-TMA	(1) Sidearm (Commscope S-400)	(8) 7/8" Coax	Connecticut
15	161.2	1	dbSpectra SP2D00P36D-D-Omni	(1) Sidearm (Site Pro USF-4U)	(1) 1/2" Coax	Light &
16	140.4	1	RFS 220-3AN- Omni	(1) Sidearm (Commscope S-600)	(1) 1/2 COAX	Power
17	139.5	1	RFS 220-7N- Omni	(1) Sidearm (Wireless Solutions		
1/	133.3	1	111 3 220-71 1- OHIIII	WS-S400)		
18	137.0	1	Kreco CO-36A- Omni	(1) Sidearm (Commscope S-600)		

See the attached coax layout for the line placement considered in the analysis.

Analysis Results

The results of the structural analysis, performed for the wind and ice loading and antenna equipment as defined above, are summarized as the following:

Tower Component	Legs	Diagonals	Horizontals	Anchor Bolts
Max. Usage:	53.6%	53.3%	3.5%	35.0%
Pass/Fail	Pass	Pass	Pass	Pass

Foundations

	Compression (Kips)	Uplift (Kips)	Shear (Kips)
Analysis Reactions	238.1	198.2	25.7

The foundation has been investigated using the supplied documents and soils report and was found adequate. Therefore, no modification to the foundation will be required.

Service Load Condition (Rigidity):

Operational characteristics of the tower are found to be within the limits prescribed by TIA-222 for the installed antennas. The maximum twist/sway at the elevation of the proposed equipment is 0.1574 degrees under the operational wind speed as specified in the Analysis Criteria.

Conclusions

Based on the analysis results, the existing structure and its foundation were found to be adequate to safely support the existing and proposed equipment and meet the minimum requirements per the TIA-222 Standard under the design basic wind speed as specified in the Analysis Criteria.

Standard Conditions

- 1. This analysis was performed based on the information supplied to (TES) Tower Engineering Solutions, LLC. Verification of the information provided was not included in the Scope of Work for TES. The accuracy of the analysis is dependent on the accuracy of the information provided.
- 2. The structural analysis was performance based upon the evidence available at the time of this report. All information provided by the client is considered to be accurate.
- 3. The analyses will be performed based on the codes as specified by the client or based on the best knowledge of the engineering staff of TES. In the absence of information to the contrary, all work will be performed in accordance with the latest relevant revision of ANSI/TIA-222. If wind speed and/or ice loads are different from the minimum values recommended by the ANSI/TIA-222 standard or other codes, TES should be notified in writing and the applicable minimum values provided by the client.
- 4. The configuration of the existing mounts, antennas, coax and other appurtenances were supplied by the customer for the current structural analysis. TES has not visited the tower site to verify the adequacy of the information provided. If there is any discrepancy found in the report regarding the existing conditions, TES should be notified immediately to evaluate the effect of the discrepancy on the analysis results.
- 5. The client will assume responsibility for rework associated with the differences in initially provided information, including tower and foundation information, existing and/or proposed equipment and transmission lines.
- 6. If a feasibility analysis was performed, final acceptance of changed conditions shall be based upon a rigorous structural analysis.

Structure: CT06462-A-2-SBA

Site Name: Mountain Street Code: TIA-222-H 2/9/2022

Type:Self SupportBase Shape:TriangleBasic WS:121.00Height:196.00 (ft)Base Width:23.00Basic Ice WS:50.00

Base Elev: 0.00 (ft) Top Width: 6.60 Operational WS: 60.00 Page: 1



		S	Section Properties			Y
Sect	Leg Mem	bers	Diagonal Members	Horizontal Members		
1	PX 8" DIA PIPE		SAE 4X4X0.25		188.	
2	PSP ROHN 8 EH	S	SAE 4X4X0.25		100.	···
3	PSP ROHN 8 EH	S	SAE 3.5X3.5X0.25			454
4	PX 6" DIA PIPE		SAE 3.5X3.5X0.25		\$10	
5	PSP ROHN 6 EH	S	SAE 3X3X0.25			- M-
6	PX 5" DIA PIPE		SAE 2.5X2.5X0.25			
7	PX 4" DIA PIPE		SAE 2.5X2.5X0.25			ЧЖИ
8	PX 3" DIA PIPE		SAE 2X2X0.1875	SAE 1.75X1.75X0.1875	160.	.00
9-10	PST 3" DIA PIPE		SAE 2X2X0.25			
11	PST 3" DIA PIPE		SAE 1.75X1.75X0.1875	SAE 1.75X1.75X0.1875	S8	
		Disc	crete Appurtenance	 S	38	l 🔀 i
Attacl					140.	.00
Elev (f		Qty	Description			
185.0			Antel BXA-80080/4CF		67	KX
185.0			Rfs Celwave FD9R6004/2C	-3L Diplexers	\$7	1281
169.0			Ericsson AIR21-6449 B41			
169.0			RFS APXVAARR24_43-U-N		120.	
169.0			Ericsson AIR32 KRD901146	5-1		n niphon n
169.0			72" x 12" x 6" Panel		\$6	
169.0			Ericsson KRY11271 TMA's	D'ala an		
169.0			Commscope SDX192 6Q-43	•		
169.0		_	Ericsson 4449 B71 + B85 R	KUS	100.	.00
169.0 169.0		3	Ericsson 4415 B25 RRU's 10' T Frames			
164.8			Commscope DB586-Y Omn	i	\$5	
164.8			Powerwave LGP104 TMA	I		$\langle \cdot \rangle$
164.8			Sidearm (Commscope/Andr	ew S-200)	80.0	00
160.6			RFS BA1312-0 Omni	CW 0 200)		
160.6			Sidearm (Commscope S-40	0)		
159.8			Sidearm (Commscope S-40	,	\$4	KIX
159.8			RFS 458-2 Omni	,		1 🔀 1
153.3			SP2D00P36D-D		60.0	00
152.8	38 152.88	1	Sidearm (Site Pro USF-4U)			
131.0	00 137.00		Kreco CO-36A Omni		\$3	
131.0	00 131.00	1	6' Sidearm (Commscope S-	600)		
130.0	7 140.40	1	RFS 220-3AN Omni			
130.0	7 130.07	1	6' Sidearm (Commscope/Ar	drew S-600)	40.0	00
130.0	00 139.50	1	RFS 220-7N Omni			
130.0			4' Sidearm (Wireless Solution	ons WS-S400)	\$2	
120.0			Commscope sbnhh-1d45b			
120.0			Commscope SBNHH-1D65I	3	20.0	00
120.0			Samsung MT6407-77A		20.0	
120.0		3	Samsung B2/B66A RRH-BF	` ,		1 × 1
120.0		3	Samsung B5/B13 RRH-BR0		\$1	$K \mid \mathcal{A}$
120.0			Rfs Celwave DB-T1-6Z-8AE	3-U∠ Junction Box		
120.0	00 120.00		10' T-Frames			
Elev	Elav	Lin	ear Appurtenances			X
From (Qty	Description		and the state of t	
-	00 196.00	<u> </u>	·		Z	
	00 196.00	0			-11 W	
	00 196.00	0				
J.		U	op 200 (.aaaoi)			

Structure: CT06462-A-2-SBA

Site Name: Mountain Street Code: TIA-222-H 2/9/2022

121.00 Self Support Base Shape: **Basic WS:** Type: Triangle 50.00 23.00 Basic Ice WS: **Base Width:** Height: 196.00 (ft) **Operational WS:** 60.00 Page: 2 Top Width: 6.60 0.00 (ft) Base Elev:



3.00	120.00	2	1 5/8" Fiber
3.00	120.00	8	1 5/8" Coax
3.00	160.00	8	7/8" Coax
3.00	160.00	1	1/2" Coax
0.00	160.00	1	W/G Ladder (CLP)
3.00	169.00	2	1 5/8" Hybrid
3.00	169.00	9	1 5/8" Coax
0.00	169.00	1	W/G Ladder (TMO)
3.00	185.00	3	1 5/8" Coax
0.00	185.00	1	W/G Ladder (VZW)
3.00	196.00	0	Step bolts (ladder)

Base Reactions

Overturning

Max Uplift:	-198.24 (kips	Moment:	4432.37	(ft-kips)
Max Down:	238.06 (kips	Total Down:	46.62	(kips)
Max Shear:	25.68 (kips	Total Shear:	42.07	(kips)

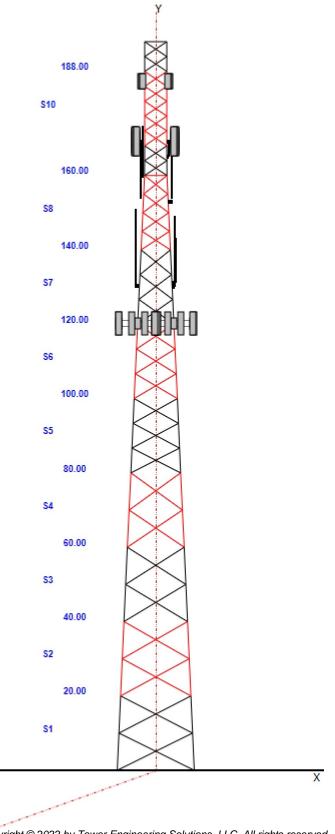
Leg

Structure: CT06462-A-2-SBA

Site Name: Mountain Street Code: TIA-222-H 2/9/2022

121.00 Base Shape: Self Support Triangle Basic WS: Type: 50.00 Base Width: 23.00 **Basic Ice WS:** Height: 196.00 (ft) Top Width: 6.60 **Operational WS:** 60.00 Page: 3 **Base Elev:** 0.00 (ft)





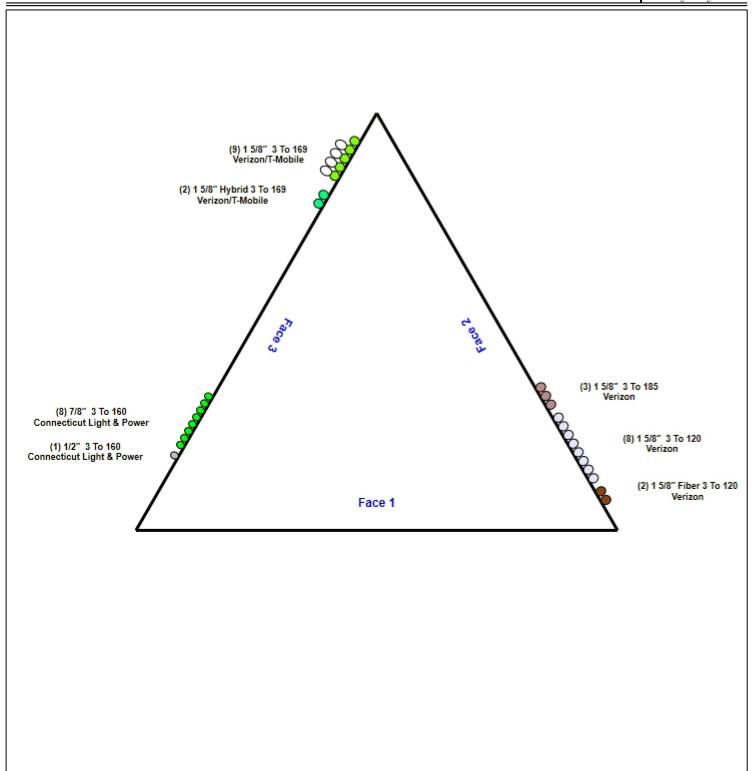
Structure: CT06462-A-2-SBA - Coax Line Placement

Type: Self Support 2/9/2022

Site Name: Mountain Street Height: 196.00 (ft) LES

Tower Engineering Solutions

Page: 4



Loading Summary

Structure: CT06462-A-2-SBA **Code:** TIA-222-H 2/9/2022

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II Page: 5



Discrete Appurtenances Properties

		-	N	lo Ice	lc	Э						
Attach Elev (ft)	Description	Qty	Weight (lb)	CaAa (sf)	Weight (lb)	CaAa (sf)	Len (in)	Width (in)	Depth (in)	Ka	Orientation Factor	Vert Ecc (ft)
185.00	Antel BXA-80080/4CF	3	14.30	4.800	87.68	6.070	48.200	11.200	5.900	1.00	1.00	0.000
185.00	Rfs Celwave FD9R6004/2C-3L	6	3.10	0.310	8.54	0.569	6.500	5.800	1.500	1.00	0.60	0.000
169.00	Ericsson AIR21-6449 B41	3	103.00	22.840	195.23	25.425	88.000	33.000	20.000	0.80	0.82	0.000
169.00	RFS APXVAARR24_43-U-NA20	3	128.00	20.240	405.88	21.503	95.900	24.000	8.700	0.80	0.72	0.000
169.00	Ericsson AIR32 KRD901146-1	3	132.20	6.050	242.46	6.768	56.000	12.000	8.700	0.80	0.67	0.000
169.00	72" x 12" x 6" Panel	3	45.00	8.130	156.02	10.008	72.000	12.000	6.000	0.80	0.79	0.000
169.00	Ericsson KRY11271 TMA's	3	11.00	1.140	20.59	2.000	13.230	10.340	6.300	0.80	0.60	0.000
169.00	Commscope SDX192 6Q-43	3	6.50	0.240	17.55	0.256	6.930	4.170	2.910	0.80	0.60	0.000
169.00	Ericsson 4449 B71 + B85 RRU's	3	75.00	1.950	125.69	2.335	17.900	13.100	10.600	0.80	0.67	0.000
169.00	Ericsson 4415 B25 RRU's	3	46.00	1.840	79.29	2.221	16.500	13.400	5.900	0.80	0.67	0.000
169.00	10' T Frames	3	500.00	15.000	734.78	23.804	0.000	0.000	0.000	0.75	0.75	0.000
164.81	Commscope DB586-Y Omni	1	8.25	1.010	39.42	1.657	52.560	2.500	2.500	1.00	1.00	2.190
164.81	Powerwave LGP104 TMA	1	7.00	0.230	12.73	0.386	7.000	4.000	1.200	1.00	1.00	-0.810
164.81	Sidearm (Commscope/Andrew	1	40.00	2.630	94.00	6.645	10.000	0.000	0.000	1.00	1.00	0.000
160.67	RFS BA1312-0 Omni	1	4.40	1.730	55.43	3.804	104.000	2.000	2.000	1.00	1.00	4.330
160.67	Sidearm (Commscope S-400)	1	53.32	3.500	124.71	8.829	10.000	0.000	0.000	1.00	1.00	0.000
159.85	Sidearm (Commscope S-400)	1	41.00	3.500	95.86	8.795	10.000	0.000	0.000	1.00	1.00	0.000
159.85	RFS 458-2 Omni	1	22.00	3.720	58.09	6.679	159.600	2.800	2.800	1.00	1.00	6.650
153.30	SP2D00P36D-D	1	45.00	5.530	103.51	9.293	189.600	3.500	3.500	1.00	1.00	7.900
152.88	Sidearm (Site Pro USF-4U)	1	165.00	5.150	329.55	9.688	20.000	0.000	0.000	1.00	1.00	0.000
131.00	Kreco CO-36A Omni	1	12.00	0.750	29.94	1.339	144.000	0.620	0.620	1.00	1.00	6.000
131.00	6' Sidearm (Commscope S-600)	1	70.00	5.150	162.33	12.831	15.000	0.000	0.000	1.00	1.00	0.000
130.07	RFS 220-3AN Omni	1	24.00	5.680	119.09	10.490	248.400	2.750	2.750	1.00	1.00	10.33
130.07	6' Sidearm (Commscope/Andrew	1	70.00	5.150	138.82	9.623	15.000	0.000	0.000	1.00	1.00	0.000
130.00	RFS 220-7N Omni	1	22.00	5.320	157.72	9.776	228.000	2.800	2.800	1.00	1.00	9.500
130.00	4' Sidearm (Wireless Solutions	1	53.32	3.500	123.70	8.753	10.000	0.000	0.000	1.00	1.00	0.000
120.00	Commscope sbnhh-1d45b	2	96.00	14.770	328.32	15.789	76.800	22.300	12.200	0.80	0.80	0.000
120.00	Commscope SBNHH-1D65B	4	50.71	8.050	171.98	8.865	72.000	11.850	7.100	0.80	0.83	0.000
120.00	Samsung MT6407-77A	3	87.10	4.700	169.00	5.385	35.120	16.060	5.510	0.80	0.70	0.000
120.00	Samsung B2/B66A RRH-BR049	3	84.40	1.880	129.75	2.229	15.000	15.000	10.000	0.80	0.67	0.000
120.00	Samsung B5/B13 RRH-BR04C	3	70.30	1.880	111.17	2.229	15.000	15.000	8.100	0.80	0.67	0.000
120.00	Rfs Celwave DB-T1-6Z-8AB-0Z	2	44.00	4.800	131.14	5.351	24.000	24.000	10.000	0.80	0.67	0.000
120.00	10' T-Frames	3	500.00	15.000	838.38	27.633	0.000	0.000	0.000	0.75	0.75	0.000

Totals: 71 6,547.13 13,243.42 Number of Appurtenances : 33

Loading Summary

Structure: CT06462-A-2-SBA **Code:** TIA-222-H 2/9/2022

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II Page: 6



Linear Appurtenances Properties

Elev. From	Elev. To			Width	Weight	Pct In	Spread On	Bundling	Cluster Dia	Out of	Spacing	Orientation	Ka
(ft)	(ft)	Description	Qty	(in)	(lb/ft)	Block		Arrangement		Zone	· ·	Factor	Override
1.00	196.00	Safety Climb	1	0.38	0.27	100.00	1	Individual NR		N	1.00	1.00	
3.00	196.00	Step bolts (ladder)		0.63	1.04	100.00	3	Individual NR		Ν	1.00	1.00	
3.00	196.00	Step bolts (ladder)		0.63	1.04	100.00	1	Individual NR		Ν	1.00	1.00	
3.00	196.00	Step bolts (ladder)		0.63	1.04	100.00	2	Individual NR		Ν	1.00	1.00	
0.00	185.00	W/G Ladder (VZW)	1	2.00	6.00	100.00	2	Individual NR		Ν	1.00	1.00	
3.00	185.00	1 5/8" Coax	3	1.98	1.04	100.00	2	Individual IR		Ν	0.50	0.64	
0.00	169.00	W/G Ladder (TMO)	1	2.50	6.00	100.00	3	Individual NR		N	1.00	1.00	
3.00	169.00	1 5/8" Coax	9	1.98	1.04	50.00	3	Block		Ν	0.50	1.00	
3.00	169.00	1 5/8" Hybrid	2	2.00	1.10	100.00	3	Individual IR		N	0.50	1.00	
0.00	160.00	W/G Ladder (CLP)	1	3.00	6.00	100.00	3	Individual NR		Ν	1.00	1.00	
3.00	160.00	1/2" Coax	1	0.65	0.16	100.00	3	Individual IR		N	1.00	1.00	
3.00	160.00	7/8" Coax	8	1.11	0.52	100.00	3	Individual IR		Ν	0.50	1.00	
3.00	120.00	1 5/8" Coax	8	1.98	1.04	100.00	2	Individual IR		N	0.50	0.42	
3.00	120.00	1 5/8" Fiber	2	1.98	1.04	100.00	2	Individual IR		Ν	0.50	0.76	

Structure: CT06462-A-2-SBA **Code**: TIA-222-H 2/9/2022

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: ||



Load Case: 1.2D + 1.0W Normal Wind 1.2D + 1.0W 121 mph Wind at Normal To Face

Wind Load Factor: 1.00 Wind Importance Factor: 1.00

Dead Load Factor: 1.20 Ice Dead Load Factor: 0.00

Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	lce Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	22.30	31.267	28.80	0.00	0.13	2.84	1.00	1.00	0.00	43.67	85.58	0.00	6,467.1	0.0	2349.60	1225.61	3,575.21
2	30.0	22.32	28.860	28.80	0.00	0.14	2.81	1.00	1.00	0.00	41.35	98.41	0.00	5,655.3	0.0	2205.88	1411.97	3,617.85
3	50.0	25.83	23.184	28.80	0.00	0.14	2.81	1.00	1.00	0.00	35.08	98.41	0.00	5,249.6	0.0	2166.27	1633.85	3,800.12
4	70.0	28.43	21.246	22.13	0.00	0.13	2.84	1.00	1.00	0.00	31.63	98.41	0.00	4,775.7	0.0	2171.13	1798.72	3,969.85
5	90.0	30.55	22.280	22.12	0.00	0.15	2.76	1.00	1.00	0.00	32.63	98.41	0.00	4,421.7	0.0	2336.98	1932.62	4,269.60
6	110.0	32.35	16.430	18.58	0.00	0.14	2.80	1.00	1.00	0.00	25.69	98.41	0.00	3,849.5	0.0	1979.23	2046.66	4,025.90
7	130.0	33.93	14.331	15.03	0.00	0.14	2.79	1.00	1.00	0.00	22.39	65.41	0.00	3,024.9	0.0	1803.83	1461.38	3,265.21
8	150.0	35.35	12.808	11.69	0.00	0.15	2.76	1.00	1.00	0.00	19.44	65.41	0.00	2,332.0	0.0	1613.38	1522.36	3,135.74
9	164.0	36.26	4.976	4.67	0.00	0.17	2.69	1.00	1.00	0.00	7.64	17.81	0.00	833.3	0.0	632.37	439.28	1,071.66
10	178.0	37.12	12.376	11.67	0.00	0.17	2.68	1.00	1.00	0.00	19.03	13.41	0.00	1,645.7	0.0	1611.60	310.22	1,921.82
11	192.0	37.93	5.252	4.67	0.00	0.18	2.66	1.00	1.00	0.00	7.92	0.25	0.00	506.9	0.0	680.23	5.80	686.04
														38,761.8	0.0	<u> </u>		33,338.99

Load Case: 1.2D + 1.0W 60° Wind 1.2D + 1.0W 121 mph Wind at 60° From Face

Wind Load Factor: 1.00

Dead Load Factor: 1.20

Pool Load Factor: 0.00

Wind Importance Factor: 1.00

Ice Dead Load Factor:0.00Ice Importance Factor:1.00

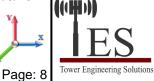
		Total	Total	Ice								Ice					
	Wind	Flat	Round	Round					Ice	Eff	Linear	Linear	Total		Struct	Linear	Total
Sect	Height	qz Area	Area	Area	Sol				Thick	Area	Area	Area	Weight	Weight	Force	Force	Force
Seq	(ft)	(psf) (sqft)	(sqft)	(sqft)	Ratio	Cf	Df	Dr	(in)	(sqft)	(sqft)	(sqft)	(lb)	Ice (Ib)	(lb)	(lb)	(lb)
1	10.0	22.30 31.267	28.80	0.00	0.13	2.84	0.80	1.00	0.00	37.42	85.58	0.00	6,467.1	0.0	2013.18	1225.61	3,238.78
2	30.0	22.32 28.860	28.80	0.00	0.14	2.81	0.80	1.00	0.00	35.58	98.41	0.00	5,655.3	0.0	1897.97	1411.97	3,309.94
3	50.0	25.83 23.184	28.80	0.00	0.14	2.81	0.80	1.00	0.00	30.44	98.41	0.00	5,249.6	0.0	1879.96	1633.85	3,513.80
4	70.0	28.43 21.246	22.13	0.00	0.13	2.84	0.80	1.00	0.00	27.38	98.41	0.00	4,775.7	0.0	1879.45	1798.72	3,678.17
5	90.0	30.55 22.280	22.12	0.00	0.15	2.76	0.80	1.00	0.00	28.18	98.41	0.00	4,421.7	0.0	2017.89	1932.62	3,950.51
6	110.0	32.35 16.430	18.58	0.00	0.14	2.80	0.80	1.00	0.00	22.40	98.41	0.00	3,849.5	0.0	1726.05	2046.66	3,772.72
7	130.0	33.93 14.331	15.03	0.00	0.14	2.79	0.80	1.00	0.00	19.53	65.41	0.00	3,024.9	0.0	1572.97	1461.38	3,034.35
8	150.0	35.35 12.808	11.69	0.00	0.15	2.76	0.80	1.00	0.00	16.88	65.41	0.00	2,332.0	0.0	1400.83	1522.36	2,923.19
9	164.0	36.26 4.976	4.67	0.00	0.17	2.69	0.80	1.00	0.00	6.64	17.81	0.00	833.3	0.0	549.98	439.28	989.26
10	178.0	37.12 12.376	11.67	0.00	0.17	2.68	0.80	1.00	0.00	16.56	13.41	0.00	1,645.7	0.0	1402.00	310.22	1,712.21
11	192.0	37.93 5.252	4.67	0.00	0.18	2.66	0.80	1.00	0.00	6.87	0.25	0.00	506.9	0.0	589.99	5.80	595.80
													38,761.8	0.	D		30,718.73

Structure: CT06462-A-2-SBA **Code**: TIA-222-H 2/9/2022

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II



Load Case: 1.2D + 1.0W 90° Wind 1.2D + 1.0W 121 mph Wind at 90° From Face

Wind Load Factor: 1.00 Wind Importance Factor: 1.00

Dead Load Factor: 1.20 Ice Dead Load Factor: 0.00

Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	22.30	31.267	28.80	0.00	0.13	2.84	0.85	1.00	0.00	38.98	85.58	0.00	6,467.1	0.0	2097.28	1225.61	3,322.89
2	30.0	22.32	28.860	28.80	0.00	0.14	2.81	0.85	1.00	0.00	37.02	98.41	0.00	5,655.3	0.0	1974.94	1411.97	3,386.92
3	50.0	25.83	23.184	28.80	0.00	0.14	2.81	0.85	1.00	0.00	31.60	98.41	0.00	5,249.6	0.0	1951.54	1633.85	3,585.38
4	70.0	28.43	21.246	22.13	0.00	0.13	2.84	0.85	1.00	0.00	28.44	98.41	0.00	4,775.7	0.0	1952.37	1798.72	3,751.09
5	90.0	30.55	22.280	22.12	0.00	0.15	2.76	0.85	1.00	0.00	29.29	98.41	0.00	4,421.7	0.0	2097.66	1932.62	4,030.28
6	110.0	32.35	16.430	18.58	0.00	0.14	2.80	0.85	1.00	0.00	23.22	98.41	0.00	3,849.5	0.0	1789.35	2046.66	3,836.01
7	130.0	33.93	14.331	15.03	0.00	0.14	2.79	0.85	1.00	0.00	20.25	65.41	0.00	3,024.9	0.0	1630.69	1461.38	3,092.06
8	150.0	35.35	12.808	11.69	0.00	0.15	2.76	0.85	1.00	0.00	17.52	65.41	0.00	2,332.0	0.0	1453.97	1522.36	2,976.33
9	164.0	36.26	4.976	4.67	0.00	0.17	2.69	0.85	1.00	0.00	6.89	17.81	0.00	833.3	0.0	570.57	439.28	1,009.86
10	178.0	37.12	12.376	11.67	0.00	0.17	2.68	0.85	1.00	0.00	17.18	13.41	0.00	1,645.7	0.0	1454.40	310.22	1,764.62
11	192.0	37.93	5.252	4.67	0.00	0.18	2.66	0.85	1.00	0.00	7.13	0.25	0.00	506.9	0.0	612.55	5.80	618.36
														38.761.8	0.0	<u> </u>		31.373.79

Load Case: 0.9D + 1.0W Normal Wind 0.9D + 1.0W 121 mph Wind at Normal To Face

Wind Load Factor: 1.00

Dead Load Factor: 0.90

1.00

Wind Importance Factor:

Ice Dead Load Factor: 0.00 Ice Importance Factor: 1.00

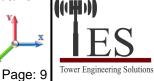
Sect Seq	Wind Height (ft)	Total Flat qz Area (psf) (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	22.30 31.267	28.80	0.00	0.13	2.84	1.00	1.00	0.00	43.67	85.58	0.00	4,850.4	0.0	2349.60	1225.61	3,575.21
2	30.0	22.32 28.860	28.80	0.00	0.14	2.81	1.00	1.00	0.00	41.35	98.41	0.00	4,241.5	0.0	2205.88	1411.97	3,617.85
3	50.0	25.83 23.184	28.80	0.00	0.14	2.81	1.00	1.00	0.00	35.08	98.41	0.00	3,937.2	0.0	2166.27	1633.85	3,800.12
4	70.0	28.43 21.246	22.13	0.00	0.13	2.84	1.00	1.00	0.00	31.63	98.41	0.00	3,581.8	0.0	2171.13	1798.72	3,969.85
5	90.0	30.55 22.280	22.12	0.00	0.15	2.76	1.00	1.00	0.00	32.63	98.41	0.00	3,316.3	0.0	2336.98	1932.62	4,269.60
6	110.0	32.35 16.430	18.58	0.00	0.14	2.80	1.00	1.00	0.00	25.69	98.41	0.00	2,887.1	0.0	1979.23	2046.66	4,025.90
7	130.0	33.93 14.331	15.03	0.00	0.14	2.79	1.00	1.00	0.00	22.39	65.41	0.00	2,268.6	0.0	1803.83	1461.38	3,265.21
8	150.0	35.35 12.808	11.69	0.00	0.15	2.76	1.00	1.00	0.00	19.44	65.41	0.00	1,749.0	0.0	1613.38	1522.36	3,135.74
9	164.0	36.26 4.976	4.67	0.00	0.17	2.69	1.00	1.00	0.00	7.64	17.81	0.00	625.0	0.0	632.37	439.28	1,071.66
10	178.0	37.12 12.376	11.67	0.00	0.17	2.68	1.00	1.00	0.00	19.03	13.41	0.00	1,234.3	0.0	1611.60	310.22	1,921.82
11	192.0	37.93 5.252	4.67	0.00	0.18	2.66	1.00	1.00	0.00	7.92	0.25	0.00	380.2	0.0	680.23	5.80	686.04
													29.071.3	0.0	0		33.338.99

Structure: CT06462-A-2-SBA **Code**: TIA-222-H 2/9/2022

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II



Load Case: 0.9D + 1.0W 60° Wind 0.9D + 1.0W 121 mph Wind at 60° From Face

Wind Load Factor: 1.00 Wind Importance Factor: 1.00

Dead Load Factor: 0.90 Ice Dead Load Factor: 0.00

Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	lce Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	22.30	31.267	28.80	0.00	0.13	2.84	0.80	1.00	0.00	37.42	85.58	0.00	4,850.4	0.0	2013.18	1225.61	3,238.78
2	30.0	22.32	28.860	28.80	0.00	0.14	2.81	0.80	1.00	0.00	35.58	98.41	0.00	4,241.5	0.0	1897.97	1411.97	3,309.94
3	50.0	25.83	23.184	28.80	0.00	0.14	2.81	0.80	1.00	0.00	30.44	98.41	0.00	3,937.2	0.0	1879.96	1633.85	3,513.80
4	70.0	28.43	21.246	22.13	0.00	0.13	2.84	0.80	1.00	0.00	27.38	98.41	0.00	3,581.8	0.0	1879.45	1798.72	3,678.17
5	90.0	30.55	22.280	22.12	0.00	0.15	2.76	0.80	1.00	0.00	28.18	98.41	0.00	3,316.3	0.0	2017.89	1932.62	3,950.51
6	110.0	32.35	16.430	18.58	0.00	0.14	2.80	0.80	1.00	0.00	22.40	98.41	0.00	2,887.1	0.0	1726.05	2046.66	3,772.72
7	130.0	33.93	14.331	15.03	0.00	0.14	2.79	0.80	1.00	0.00	19.53	65.41	0.00	2,268.6	0.0	1572.97	1461.38	3,034.35
8	150.0	35.35	12.808	11.69	0.00	0.15	2.76	0.80	1.00	0.00	16.88	65.41	0.00	1,749.0	0.0	1400.83	1522.36	2,923.19
9	164.0	36.26	4.976	4.67	0.00	0.17	2.69	0.80	1.00	0.00	6.64	17.81	0.00	625.0	0.0	549.98	439.28	989.26
10	178.0	37.12	12.376	11.67	0.00	0.17	2.68	0.80	1.00	0.00	16.56	13.41	0.00	1,234.3	0.0	1402.00	310.22	1,712.21
11	192.0	37.93	5.252	4.67	0.00	0.18	2.66	0.80	1.00	0.00	6.87	0.25	0.00	380.2	0.0	589.99	5.80	595.80
														29,071.3	0.0	<u> </u>		30,718.73

Load Case: 0.9D + 1.0W 90° Wind 0.9D + 1.0W 121 mph Wind at 90° From Face

Wind Load Factor: 1.00

Dead Load Factor: 0.90

Wind Importance Factor: 1.00

Ice Dead Load Factor:0.00Ice Importance Factor:1.00

Sect Seq	Wind Height (ft)	Total Flat qz Area (psf) (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	lce Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (Ib)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	22.30 31.267	28.80	0.00	0.13	2.84	0.85	1.00	0.00	38.98	85.58	0.00	4,850.4	0.0	2097.28	1225.61	3,322.89
2	30.0	22.32 28.860	28.80	0.00	0.14	2.81	0.85	1.00	0.00	37.02	98.41	0.00	4,241.5	0.0	1974.94	1411.97	3,386.92
3	50.0	25.83 23.184	28.80	0.00	0.14	2.81	0.85	1.00	0.00	31.60	98.41	0.00	3,937.2	0.0	1951.54	1633.85	3,585.38
4	70.0	28.43 21.246	22.13	0.00	0.13	2.84	0.85	1.00	0.00	28.44	98.41	0.00	3,581.8	0.0	1952.37	1798.72	3,751.09
5	90.0	30.55 22.280	22.12	0.00	0.15	2.76	0.85	1.00	0.00	29.29	98.41	0.00	3,316.3	0.0	2097.66	1932.62	4,030.28
6	110.0	32.35 16.430	18.58	0.00	0.14	2.80	0.85	1.00	0.00	23.22	98.41	0.00	2,887.1	0.0	1789.35	2046.66	3,836.01
7	130.0	33.93 14.331	15.03	0.00	0.14	2.79	0.85	1.00	0.00	20.25	65.41	0.00	2,268.6	0.0	1630.69	1461.38	3,092.06
8	150.0	35.35 12.808	11.69	0.00	0.15	2.76	0.85	1.00	0.00	17.52	65.41	0.00	1,749.0	0.0	1453.97	1522.36	2,976.33
9	164.0	36.26 4.976	4.67	0.00	0.17	2.69	0.85	1.00	0.00	6.89	17.81	0.00	625.0	0.0	570.57	439.28	1,009.86
10	178.0	37.12 12.376	11.67	0.00	0.17	2.68	0.85	1.00	0.00	17.18	13.41	0.00	1,234.3	0.0	1454.40	310.22	1,764.62
11	192.0	37.93 5.252	4.67	0.00	0.18	2.66	0.85	1.00	0.00	7.13	0.25	0.00	380.2	0.0	612.55	5.80	618.36
													29,071.3	0.	0		31,373.79

Structure: CT06462-A-2-SBA Code: TIA-222-H 2/9/2022

Site Name: Mountain Street **Exposure:** В Height: 196.00 (ft) Crest Height: 0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: Struct Class: II 0.85 Topography: 1



Tower Engineering Solutions

Load Case: 1.2D + 1.0Di + 1.0Wi Normal Wind 1.2D + 1.0Di + 1.0Wi 50 mph Wind at Normal From Face

Wind Load Factor: 1.00 Wind Importance Factor: 1.00

Dead Load Factor: 1.20 Ice Dead Load Factor: 1.00

Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat qz Area (psf) (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	3.81 31.267	49.02	20.23	0.18	2.68	1.00	1.00	0.89	59.24	115.94	11.68	10,548.	4081.6	513.83	309.27	823.10
2	30.0	3.81 28.860	50.18	21.38	0.19	2.63	1.00	1.00	0.99	57.60	136.52	13.21	10,319.	4663.6	491.16	359.55	850.71
3	50.0	4.41 23.184	50.07	21.27	0.19	2.62	1.00	1.00	1.04	51.90	137.73	13.90	9,782.0	4532.4	508.99	421.41	930.39
4	70.0	4.86 21.246	42.84	20.72	0.19	2.62	1.00	1.00	1.08	45.80	138.56	14.37	9,154.7	4379.0	495.52	468.60	964.11
5	90.0	5.22 22.280	46.53	24.41	0.24	2.48	1.00	1.00	1.11	49.34	139.20	14.74	9,084.5	4662.8	543.07	500.82	1,043.89
6	110.0	5.52 16.430	41.48	22.90	0.23	2.50	1.00	1.00	1.13	40.51	139.73	15.04	8,056.9	4207.5	474.69	534.16	1,008.86
7	130.0	5.79 14.331	36.28	21.26	0.24	2.45	1.00	1.00	1.15	35.51	92.86	15.29	6,312.3	3287.5	429.30	455.81	885.11
8	150.0	6.04 12.808	35.04	23.35	0.29	2.32	1.00	1.00	1.16	33.70	89.25	19.39	5,524.3	3192.3	401.03	467.46	868.50
9	164.0	6.19 4.976	13.87	9.20	0.33	2.22	1.00	1.00	1.17	13.42	23.51	4.70	1,899.8	1066.5	156.71	119.00	275.72
10	178.0	6.34 12.376	34.78	23.11	0.33	2.21	1.00	1.00	1.18	33.58	18.61	7.50	3,752.8	2107.1	400.41	97.63	498.04
11	192.0	6.48 5.252	2 15.29	10.63	0.36	2.14	1.00	1.00	1.19	14.75	0.25	1.59	1,289.9	783.0	174.03	7.75	181.78
													75,725.0	36963.2	2	•	8,330.22

Load Case: 1.2D + 1.0Di + 1.0Wi 60° Wind 1.2D + 1.0Di + 1.0Wi 50 mph Wind at 60° From Face 1.00

1.00 Wind Load Factor: Wind Importance Factor: 1.20 **Dead Load Factor:**

Ice Importance Factor: 1.00 Ice Dead Load Factor: 1.00

Sect Seq	Wind Height (ft)	Total Flat qz Area (psf) (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	lce Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	3.81 31.26	7 49.02	20.23	0.18	2.68	0.80	1.00	0.89	52.99	115.94	11.68	10,548.	4081.6	459.59	309.27	768.86
2	30.0	3.81 28.86	0 50.18	21.38	0.19	2.63	0.80	1.00	0.99	51.82	136.52	13.21	10,319.	4663.6	441.94	359.55	801.49
3	50.0	4.41 23.18	4 50.07	21.27	0.19	2.62	0.80	1.00	1.04	47.26	137.73	13.90	9,782.0	4532.4	463.51	421.41	884.91
4	70.0	4.86 21.24	6 42.84	20.72	0.19	2.62	0.80	1.00	1.08	41.55	138.56	14.37	9,154.7	4379.0	449.55	468.60	918.14
5	90.0	5.22 22.28	0 46.53	24.41	0.24	2.48	0.80	1.00	1.11	44.88	139.20	14.74	9,084.5	4662.8	494.02	500.82	994.85
6	110.0	5.52 16.43	0 41.48	22.90	0.23	2.50	0.80	1.00	1.13	37.23	139.73	15.04	8,056.9	4207.5	436.19	534.16	970.35
7	130.0	5.79 14.33	1 36.28	21.26	0.24	2.45	0.80	1.00	1.15	32.64	92.86	15.29	6,312.3	3287.5	394.65	455.81	850.46
8	150.0	6.04 12.80	8 35.04	23.35	0.29	2.32	0.80	1.00	1.16	31.14	89.25	19.39	5,524.3	3192.3	370.55	467.46	838.02
9	164.0	6.19 4.97	6 13.87	9.20	0.33	2.22	0.80	1.00	1.17	12.42	23.51	4.70	1,899.8	1066.5	145.09	119.00	264.09
10	178.0	6.34 12.37	6 34.78	23.11	0.33	2.21	0.80	1.00	1.18	31.11	18.61	7.50	3,752.8	2107.1	370.90	97.63	468.53
11	192.0	6.48 5.25	2 15.29	10.63	0.36	2.14	0.80	1.00	1.19	13.69	0.25	1.59	1,289.9	783.0	161.63	7.75	169.39
													75.725.0	36963.2	2	-	7.929.10

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II



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Load Case: 1.2D + 1.0Di + 1.0Wi 90° Wind 1.2D + 1.0Di + 1.0Wi 50 mph Wind at 90° From Face

Wind Load Factor: 1.00 Wind Importance Factor: 1.00

Dead Load Factor: 1.20 Ice Dead Load Factor: 1.00

Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	3.81	31.267	49.02	20.23	0.18	2.68	0.85	1.00	0.89	54.55	115.94	11.68	10,548.	4081.6	473.15	309.27	782.42
2	30.0	3.81	28.860	50.18	21.38	0.19	2.63	0.85	1.00	0.99	53.27	136.52	13.21	10,319.	4663.6	454.24	359.55	813.80
3	50.0	4.41	23.184	50.07	21.27	0.19	2.62	0.85	1.00	1.04	48.42	137.73	13.90	9,782.0	4532.4	474.88	421.41	896.28
4	70.0	4.86	21.246	42.84	20.72	0.19	2.62	0.85	1.00	1.08	42.62	138.56	14.37	9,154.7	4379.0	461.04	468.60	929.64
5	90.0	5.22	22.280	46.53	24.41	0.24	2.48	0.85	1.00	1.11	46.00	139.20	14.74	9,084.5	4662.8	506.28	500.82	1,007.11
6	110.0	5.52	16.430	41.48	22.90	0.23	2.50	0.85	1.00	1.13	38.05	139.73	15.04	8,056.9	4207.5	445.82	534.16	979.98
7	130.0	5.79	14.331	36.28	21.26	0.24	2.45	0.85	1.00	1.15	33.36	92.86	15.29	6,312.3	3287.5	403.31	455.81	859.12
8	150.0	6.04	12.808	35.04	23.35	0.29	2.32	0.85	1.00	1.16	31.78	89.25	19.39	5,524.3	3192.3	378.17	467.46	845.64
9	164.0	6.19	4.976	13.87	9.20	0.33	2.22	0.85	1.00	1.17	12.67	23.51	4.70	1,899.8	1066.5	148.00	119.00	267.00
10	178.0	6.34	12.376	34.78	23.11	0.33	2.21	0.85	1.00	1.18	31.73	18.61	7.50	3,752.8	2107.1	378.28	97.63	475.91
11	192.0	6.48	5.252	15.29	10.63	0.36	2.14	0.85	1.00	1.19	13.96	0.25	1.59	1,289.9	783.0	164.73	7.75	172.49
														75.725.0	36963.2	2	•	8,029.38

Load Case: 1.0D + 1.0W Normal Wind 1.0D + 1.0W 60 mph Wind at Normal To Face

Wind Load Factor: 1.00
Dead Load Factor: 1.00
Ice Dead Load Factor: 0.00

Ice Importance Factor: 1.00

1.00

Wind Importance Factor:

Total Total lce Ice Wind Flat Round Round Ice Eff Linear Linear **Total** Struct Linear **Total** Sect Height αz Area Area Area Sol Thick Area Area Area Weight Weight Force Force Force Cf Seq (ft) (psf) (sqft) (sqft) (sqft) Ratio Df Dr (in) (sqft) (sqft) (sqft) (lb) Ice (lb) (lb) (lb) (lb) 1 10.0 5.48 31.267 28.80 0.00 0.13 2.84 1.00 1.00 0.00 47.57 85.58 0.00 5,389.3 0.0 629.21 301.36 930.57 0.00 592.56 347.18 2 30.0 5.49 28.860 28.80 0.14 2.81 1.00 1.00 0.00 45.17 98.41 0.00 4,712.8 0.0 939.74 3 599.72 50.0 6.35 23.184 28.80 0.00 0.14 2.81 1.00 1.00 0.00 39.50 98.41 0.00 4,374.7 0.0 401.74 1,001.45 4 70.0 6.99 21.246 22.13 0.00 0.13 2.84 1.00 1.00 0.00 33.77 98.41 0.00 3,979.7 0.0 569.94 442.28 1,012.21 5 90.0 7.51 22.280 22.12 0.00 0.15 2.76 1.00 1.00 0.00 34.84 98.41 0.00 3.684.7 0.0 613.49 475.20 1,088.69 110.0 18.58 0.00 0.00 26.96 98.41 0.0 510.73 503.24 6 7.96 16 430 0.14 2.80 1.00 1.00 0.00 3,207.9 1,013.97 7 130.0 0.00 0.14 2.79 0.00 22.85 65.41 0.0 452.57 359.33 8.34 14.331 15.03 1.00 1.00 0.00 2,520.7 811.90 0.00 396.71 8 150.0 8.69 12.808 11.69 0.00 0.15 2.76 1.00 1.00 19.44 65.41 0.00 1,943.4 0.0 374.33 771.03 9 694.4 108.01 164.0 4.67 0.00 0.17 2.69 1.00 1.00 0.00 7.64 17.81 0.00 0.0 155.49 263.50 8.92 4.976 10 178.0 9.13 12.376 11.67 0.00 0.17 2.68 1.00 1.00 0.00 19.03 13.41 0.00 1,371.4 0.0 396.27 76.28 472.55 11 192.0 9.33 5.252 4.67 0.00 0.18 2.66 1.00 1.00 0.00 7.92 0.25 0.00 422.4 0.0 167.26 1.43 168.69 32,301.5 0.0 8,474.31

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II

0.00



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Load Case: 1.0D + 1.0W 60° Wind 1.0D + 1.0W 60 mph Wind at 60° From Face

Wind Load Factor: 1.00 Wind Importance Factor: 1.00

Dead Load Factor: 1.00 lce Dead Load Factor: 0.00

Ice Dead Load Factor:

Ice Importance Factor: 1.00

Ice Importance Factor:

1.00

Sect Seq	Wind Height (ft)	qz	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	lce Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	10.0	5.48	31.267	28.80	0.00	0.13	2.84	0.80	1.00	0.00	41.31	85.58	0.00	5,389.3	0.0	546.49	301.36	847.85
2	30.0	5.49	28.860	28.80	0.00	0.14	2.81	0.80	1.00	0.00	39.40	98.41	0.00	4,712.8	0.0	516.85	347.18	864.03
3	50.0	6.35	23.184	28.80	0.00	0.14	2.81	0.80	1.00	0.00	34.86	98.41	0.00	4,374.7	0.0	529.31	401.74	931.05
4	70.0	6.99	21.246	22.13	0.00	0.13	2.84	0.80	1.00	0.00	29.52	98.41	0.00	3,979.7	0.0	498.22	442.28	940.49
5	90.0	7.51	22.280	22.12	0.00	0.15	2.76	0.80	1.00	0.00	30.39	98.41	0.00	3,684.7	0.0	535.03	475.20	1,010.23
6	110.0	7.96	16.430	18.58	0.00	0.14	2.80	0.80	1.00	0.00	23.67	98.41	0.00	3,207.9	0.0	448.48	503.24	951.72
7	130.0	8.34	14.331	15.03	0.00	0.14	2.79	0.80	1.00	0.00	19.98	65.41	0.00	2,520.7	0.0	395.80	359.33	755.13
8	150.0	8.69	12.808	11.69	0.00	0.15	2.76	0.80	1.00	0.00	16.88	65.41	0.00	1,943.4	0.0	344.44	374.33	718.77
9	164.0	8.92	4.976	4.67	0.00	0.17	2.69	0.80	1.00	0.00	6.64	17.81	0.00	694.4	0.0	135.23	108.01	243.24
10	178.0	9.13	12.376	11.67	0.00	0.17	2.68	0.80	1.00	0.00	16.56	13.41	0.00	1,371.4	0.0	344.73	76.28	421.01
11	192.0	9.33	5.252	4.67	0.00	0.18	2.66	0.80	1.00	0.00	6.87	0.25	0.00	422.4	0.0	145.07	1.43	146.50
														32,301.5	0.0)	-	7,830.02

Load Case: 1.0D + 1.0W 90° Wind 1.0D + 1.0W 60 mph Wind at 90° From Face

Wind Load Factor: 1.00

Dead Load Factor: 1.00

Wind Importance Factor: 1.00

Total Total lce Ice Wind Flat Round Round Ice Eff Linear Linear **Total** Struct Linear **Total** Sect Height αz Area Area Area Sol Thick Area Area Area Weight Weight Force Force Force Cf Seq (ft) (psf) (sqft) (sqft) (sqft) Ratio Df Dr (in) (sqft) (sqft) (sqft) (lb) Ice (lb) (lb) (lb) (lb) 1 10.0 5.48 31.267 28.80 0.00 0.13 2.84 0.85 1.00 0.00 42.88 85.58 0.00 5,389.3 0.0 567.17 301.36 868.53 0.00 0.14 2.81 40.85 535.77 347.18 2 30.0 5.49 28.860 28.80 0.85 1.00 0.00 98.41 0.00 4,712.8 0.0 882.96 3 50.0 6.35 23.184 28.80 0.00 0.14 2.81 0.85 1.00 0.00 36.02 98.41 0.00 4,374.7 0.0 546.91 401.74 948.65 70.0 6.99 21.246 22.13 0.00 0.13 2.84 0.85 1.00 0.00 30.58 98.41 0.00 3,979.7 0.0 516.15 442.28 958.42 5 90.0 7.51 22.280 22.12 0.00 0.15 2.76 0.85 1.00 0.00 31.50 98.41 0.00 3.684.7 0.0 554.64 475.20 1,029.84 110.0 18.58 0.00 0.14 2.80 0.85 1.00 0.00 24.49 98.41 0.0 464.04 503.24 6 7.96 16 430 0.00 3,207.9 967.28 7 130.0 8.34 14.331 15.03 0.00 0.14 2.79 0.85 1.00 0.00 20.70 65.41 0.00 0.0 409.99 359.33 769.32 2,520.7 0.00 65.41 357.51 8 150.0 8.69 12.808 11.69 0.00 0.15 2.76 0.85 1.00 17.52 0.00 1,943.4 0.0 374.33 731.83 9 694.4 108.01 164.0 4.67 0.00 0.17 2.69 0.85 1.00 0.00 6.89 17.81 0.00 0.0 140.30 8.92 4.976 248.31 10 178.0 9.13 12.376 11.67 0.00 0.17 2.68 0.85 1.00 0.00 17.18 13.41 0.00 1,371.4 0.0 357.61 76.28 433.89 11 192.0 9.33 5.252 4.67 0.00 0.18 2.66 0.85 1.00 0.00 7.13 0.25 0.00 422.4 0.0 150.62 1.43 152.04 32,301.5 0.0 7,991.09

Force/Stress Compression Summary

Structure: CT06462-A-2-SBA **Code**: EIA/TIA-222-H 2/9/2022

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II



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4.00 100 100 100 41.38 50.00 88.54 0.7 Member X



			LEG MEMBER	S								
Sect	Top Elev Member	Force (kips)	Load Case	Len (ft)	Bı X	acing	у % Z	KL/R	Fy (ksi)	Mem Cap	Leg Use %	Controls
	FIGA MEHIDEI	(кіра)	Loau Case	(11)				IXL/IX	(KSI)	(Kips)	036 /0	Controls
1	20 PX - 8" DIA PIPE	-231.81	1.2D + 1.0W Normal Wind	10.02	100	100	100	41.77	50.00	505.44	45.9	Member X
2	40 PSP - ROHN 8 EHS	-207.01	1.2D + 1.0W Normal Wind	10.02	100	100	100	41.17	50.00	386.42	53.6	Member X
3	60 PSP - ROHN 8 EHS	-180.20	1.2D + 1.0W Normal Wind	10.02	100	100	100	41.17	50.00	386.42	46.6	Member X
4	80 PX - 6" DIA PIPE	-153.19	1.2D + 1.0W Normal Wind	10.02	100	100	100	54.90	50.00	303.24	50.5	Member X
5	100 PSP - ROHN 6 EHS	-127.75	1.2D + 1.0W Normal Wind	6.68	100	100	100	36.01	50.00	274.76	46.5	Member X
6	120 PX - 5" DIA PIPE	-98.79	1.2D + 1.0W Normal Wind	6.68	100	100	100	43.56	50.00	239.34	41.3	Member X
7	140 PX - 4" DIA PIPE	-69.26	1.2D + 1.0W Normal Wind	6.68	100	100	100	54.15	50.00	160.15	43.2	Member X
8	160 PX - 3" DIA PIPE	-45.42	1.2D + 1.0W Normal Wind	5.01	100	100	100	52.73	50.00	110.90	41.0	Member X
9	168 PST - 3" DIA PIPE	-19.34	1.2D + 1.0W Normal Wind	4.00	100	100	100	41.38	50.00	88.54	21.8	Member X
10	188 PST - 3" DIA PIPE	-7.95	1.2D + 1.0W Normal Wind	4.00	100	100	100	41.38	50.00	88.54	9.0	Member X

-0.65 1.2D + 1.0W Normal Wind

Splices

11 196 PST - 3" DIA PIPE

			Top Splic	е					Bottom Sp	lice			
Sect	Top Elev	Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts	Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts
1	20	1.2D + 1.0W Normal Wind	213.52	0.00	0.0			1.2D + 1.0W Normal Wind	238.41	0.00			
2	40	1.2D + 1.0W Normal Wind	187.09	0.00	0.0			1.2D + 1.0W Normal Wind	213.52	0.00		1 A325	8
3	60	1.2D + 1.0W Normal Wind	159.81	0.00	0.0			1.2D + 1.0W Normal Wind	187.09	0.00		1 A325	8
4	80	1.2D + 1.0W Normal Wind	132.74	0.00	0.0			1.2D + 1.0W Normal Wind	159.81	0.00		1 A325	8
5	100	1.2D + 1.0W Normal Wind	103.37	0.00	0.0			1.2D + 1.0W Normal Wind	132.74	0.00		1 A325	6
6	120	1.2D + 1.0W Normal Wind	74.67	0.00	0.0			1.2D + 1.0W Normal Wind	103.37	0.00		1 A325	6
7	140	1.2D + 1.0W Normal Wind	48.46	0.00	0.0			1.2D + 1.0W Normal Wind	74.67	0.00		1 A325	4
8	160	1.2D + 1.0W Normal Wind	22.43	0.00	0.0			1.2D + 1.0W Normal Wind	48.46	0.00		7/8 A325	4
9	168	1.2D + 1.0W Normal Wind	10.70	0.00	0.0			1.2D + 1.0W Normal Wind	22.43	0.00		7/8 A325	4
10	188	1.2D + 1.0W Normal Wind	0.99	0.00	0.0			1.2D + 1.0W Normal Wind	10.70	0.00		7/8 A325	4
11	196	1.2D + 1.0Di + 1.0Wi 60° Wind	0.21	0.00	0.0			1.2D + 1.0W Normal Wind	0.99	0.00		3/4 A325	4

				Н	ORIZO	NTA	L MEI	MBE	RS								
Sect	Top Elev	Member	Force (kips)		Len (ft)		acing Y		KL/R	Fy (ksi)		Num Bolts		Shear Cap (kips)	Сар	Use %	Controls
1	20										0.00	0	0				
2	40										0.00	0	0				
3	60										0.00	0	0				
4	80										0.00	0	0				
5	100										0.00	0	0				
6	120										0.00	0	0				
7	140										0.00	0	0				
8	160	SAE - 1.75X1.75X0.1875	-0.28	1.2D + 1.0W 90° Wind	6.69	50	50	50	118.51	36.00	12.46	1	1	13.81	13.05	2.3	Member Z
9	168										0.00	0	0				
10	188										0.00	0	0				
11	196	SAE - 1.75X1.75X0.1875	-0.01	0.9D + 1.0W 60° Wind	6.60	100	100	100	230.90	36.00	3.33	1	1	13.81	13.05	0.3	Member Z

					DIAGO	NAL	MEM	BER:	S								
	Тор		Force		Len	Br	racing	j %		Fy	Mem Cap	Num	Num	Shear Cap	Bear Cap	Use	
Sect	Elev	Member	(kips)	Load Case	(ft)	X	Υ	Z	KL/R	(ksi)	(kips)	Bolts	Holes	(kips)	(kips)	%	Controls
1	20	SAE - 4X4X0.25	-7.54	1.2D + 1.0W 90° Wind	24.62	50	50	50	185.84	36.00	16.08	1	1	19.87	20.8	46.9	Member Z
2	40	SAE - 4X4X0.25	-7.77	1.2D + 1.0W 90° Wind	22.81	50	50	50	172.16	36.00	18.73	1	1	19.87	20.8	41.5	Member Z
3	60	SAE - 3.5X3.5X0.25	-7.38	1.2D + 1.0W 90° Wind	21.03	50	50	50	181.81	36.00	14.63	1	1	19.87	20.8	50.4	Member Z

Force/Stress Compression Summary

Structure: CT06462-A-2-SBA **Code**: EIA/TIA-222-H 2/9/2022

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II





					DIAGO	NAL	MEMI	BER	S								
Sect	Top Elev		Force (kips)	Load Case	Len (ft)	Br X	acing Y	ј % Z	KL/R	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Сар		Controls
4	80	SAE - 3.5X3.5X0.25	-6.68	1.2D + 1.0W 90° Wind	19.26	50	50	50	166.49	36.00	17.45	1	1	19.87	20.8	38.3	Member Z
5	100	SAE - 3X3X0.25	-6.04	1.2D + 1.0W 90° Wind	15.99	50	50	50	162.02	36.00	15.70	1	1	19.87	20.8	38.5	Member Z
6	120	SAE - 2.5X2.5X0.25	-5.29	1.2D + 1.0W 90° Wind	14.23	50	50	50	173.91	36.00	11.26	1	1	19.87	20.8	47.0	Member Z
7	140	SAE - 2.5X2.5X0.25	-4.06	1.2D + 1.0W Normal Wind	12.43	50	50	50	151.85	36.00	14.77	1	1	13.81	17.4	29.4	Bolt Shear
8	160	SAE - 2X2X0.1875	-3.13	1.2D + 1.0W 90° Wind	9.86	50	50	50	150.21	36.00	9.01	1	1	13.81	13.0	34.7	Member Z
9	168	SAE - 2X2X0.25	-3.23	1.2D + 1.0W 90° Wind	7.78	50	50	50	119.49	36.00	18.65	1	1	13.81	17.4	23.4	Bolt Shear
10	188	SAE - 2X2X0.25	-1.35	1.2D + 1.0W Normal Wind	7.72	50	50	50	118.82	36.00	18.82	1	1	13.81	17.4	9.8	Bolt Shear
11	196	SAE - 1.75X1.75X0.187	ŧ -0.23	1.2D + 1.0W 90° Wind	7.72	50	50	50	135.00	36.00	9.74	1	1	13.81	13.0	2.4	Member Z

Force/Stress Tension Summary

Structure: CT06462-A-2-SBA Code: EIA/TIA-222-H

Site Name: Mountain Street **Exposure:** В Height: 196.00 (ft) Crest Height: 0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: ||



2/9/2022



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LEG MEMBERS

						Mem		
	Тор		Force		Fy	Cap	Leg	
Sect	Elev	Member	(kips)	Load Case	(ksi)	(kips)	Use %	Controls
1	20	PX - 8" DIA PIPE	193.76	0.9D + 1.0W 60° Wind	50	574.20	33.7	Member
2	40	PSP - ROHN 8 EHS	172.94	0.9D + 1.0W 60° Wind	50	437.40	39.5	Member
3	60	PSP - ROHN 8 EHS	150.91	0.9D + 1.0W 60° Wind	50	437.40	34.5	Member
4	80	PX - 6" DIA PIPE	128.33	0.9D + 1.0W 60° Wind	50	378.00	34.0	Member
5	100	PSP - ROHN 6 EHS	106.93	0.9D + 1.0W 60° Wind	50	302.09	35.4	Member
6	120	PX - 5" DIA PIPE	81.78	0.9D + 1.0W 60° Wind	50	274.95	29.7	Member
7	140	PX - 4" DIA PIPE	57.50	0.9D + 1.0W 60° Wind	50	198.45	29.0	Member
8	160	PX - 3" DIA PIPE	37.13	0.9D + 1.0W 60° Wind	50	135.90	27.3	Member
9	168	PST - 3" DIA PIPE	14.13	0.9D + 1.0W 60° Wind	50	100.35	14.1	Member
10	188	PST - 3" DIA PIPE	5.95	0.9D + 1.0W 60° Wind	50	100.35	5.9	Member
11	196	PST - 3" DIA PIPE	0.30	0.9D + 1.0W 60° Wind	50	100.35	0.3	Member

Splices

			Top Splic	е				Bottom Splice								
Sect	Top Elev	Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts	Load Case	Force (kips)		Use %	Bolt Type	Num Bolts			
1	20	0.9D + 1.0W 60° Wind	177.69	0.00	0.0			0.9D + 1.0W 60° Wind	199.7	0.00						
2	40	0.9D + 1.0W 60° Wind	155.69	0.00	0.0			0.9D + 1.0W 60° Wind	177.6	424.08	41.9	1 A325	5 8			
3	60	0.9D + 1.0W 60° Wind	133.06	0.00	0.0			0.9D + 1.0W 60° Wind	155.6	424.08	36.7	1 A325	5 8			
4	80	0.9D + 1.0W 60° Wind	110.25	0.00	0.0			0.9D + 1.0W 60° Wind	133.0	424.08	31.4	1 A325	5 8			
5	100	0.9D + 1.0W 60° Wind	85.31	0.00	0.0			0.9D + 1.0W 60° Wind	110.2	318.06	34.7	1 A325	5 6			
6	120	0.9D + 1.0W 60° Wind	60.01	0.00	0.0			0.9D + 1.0W 60° Wind	85.31	318.06	26.8	1 A325	5 6			
7	140	0.9D + 1.0W 60° Wind	39.46	0.00	0.0			0.9D + 1.0W 60° Wind	60.01	212.04	28.3	1 A325	5 4			
8	160	0.9D + 1.0W 60° Wind	16.60	0.00	0.0			0.9D + 1.0W 60° Wind	39.46	166.24	23.7	7/8 A325	5 4			
9	168	0.9D + 1.0W 60° Wind	6.18	0.00	0.0			0.9D + 1.0W 60° Wind	16.60	166.24	10.0	7/8 A325	5 4			
10	188	0.9D + 1.0W 60° Wind	0.42	0.00	0.0			0.9D + 1.0W 60° Wind	6.18	166.24	3.7	7/8 A325	5 4			
11	196		0.00	0.00	0.0			0.9D + 1.0W 60° Wind	0.42	120.40	0.3	3/4 A325	5 4			

	HORIZONTAL MEMBERS												
Sect	Top Elev	Member	Force (kips)		Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls
1	20	-			36	0.00	0	0					
2	40	-			36	0.00	0	0					
3	60	-			36	0.00	0	0					
4	80	-			36	0.00	0	0					
5	100	-			36	0.00	0	0					
6	120	-			36	0.00	0	0					
7	140	-			36	0.00	0	0					
8	160	SAE - 1.75X1.75X0.1875	0.27	0.9D + 1.0W 90° Wind	36	20.09	1	1	13.81	9.79	7.50	3.5	Blck Shear
9	168	-			36	0.00	0	0					
10	188	-			36	0.00	0	0					
11	196	SAE - 1.75X1.75X0.1875	0.02	1.2D + 1.0W Normal Wi	36	20.09	1	1	13.81	9.79	7.50	0.3	Blck Shear

	DIAGONAL MEMBERS												
Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls
1	20	SAE - 4X4X0.25	7.66 1.2	2D + 1.0W 90° Wind	36	62.86	1	1	19.87	14.35	16.62	53.3	Bolt Bear
2	40	SAE - 4X4X0.25	7.61 0.9	9D + 1.0W 90° Wind	36	62.86	1	1	19.87	14.35	16.62	53.0	Bolt Bear
3	60	SAE - 3.5X3.5X0.25	7.25 0.9	9D + 1.0W 90° Wind	36	54.76	1	1	19.87	14.35	16.62	50.5	Bolt Bear
4	80	SAE - 3.5X3.5X0.25	6.63 1.2	2D + 1.0W 90° Wind	36	54.76	1	1	19.87	14.35	16.62	46.2	Bolt Bear

Force/Stress Tension Summary

Structure: CT06462-A-2-SBA Code: EIA/TIA-222-H

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II



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2/9/2022



				DIAGONAL	. MEME	BERS							DIAGONAL MEMBERS												
Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls												
5	100	SAE - 3X3X0.25	5.98 1.20) + 1.0W 90° Wind	36	46.66	1	1	19.87	14.35	13.90	43.0	Blck Shear												
6	120	SAE - 2.5X2.5X0.25	5.24 1.20	0 + 1.0W 90° Wind	36	38.56	1	1	19.87	14.35	12.54	41.8	Blck Shear												
7	140	SAE - 2.5X2.5X0.25	4.10 0.90	0 + 1.0W 90° Wind	36	38.56	1	1	13.81	13.05	12.71	32.3	Blck Shear												
8	160	SAE - 2X2X0.1875	3.22 1.20	0 + 1.0W 90° Wind	36	23.00	1	1	13.81	9.79	7.50	43.0	Blck Shear												
9	168	SAE - 2X2X0.25	3.29 1.20	0 + 1.0W 90° Wind	36	30.46	1	1	13.81	13.05	9.99	32.9	Blck Shear												
10	188	SAE - 2X2X0.25	1.25 0.90	0 + 1.0W 60° Wind	36	30.46	1	1	13.81	13.05	9.99	12.5	Blck Shear												
11	196	SAE - 1.75X1.75X0.1875	0.24 1.20) + 1.0W 90° Wind	36	20.09	1	1	13.81	9.79	7.50	3.2	Blck Shear												

Seismic Section Forces

Structure: CT06462-A-2-SBA Code: TIA-222-H

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II



2/9/2022



Load Case: 1.2D + 1.0Ev + 1.0Eh

Dead Load Factor Ke 1.0698 **TL** 6.0000 1.20 **Sds** 0.204 **Ss** 0.1920 **Fa** 1.6000 **Seismic Load Factor Cs** 0.0459 1.00 **Sd1** 0.088 **S1** 0.0550 **Fv** 2.4000 **Kg** 0.0000 Seismic Importance Factor **T** 0.6396 1.00 **W1** 18.96 **R** 3.0000 **Vs** 1.7825 **f1** 1.5634

Elev (ft)	Wz (lb)	Lateral Fsz (Ibs)	Vertical Ev (lbs)
10.00	5389.2	24.59	220.86
30.00	4712.7	69.00	193.13
50.00	4374.6	110.05	179.28
70.00	3979.7	142.55	163.09
90.00	3684.7	171.77	151.00
110.00	5916.1	353.31	242.45
130.00	2772.0	187.74	113.60
150.00	2216.3	172.22	90.83
164.00	807.42	64.33	33.09
178.00	4573.0	448.89	187.40
192.00	422.41	38.07	17.31
	(ft) 10.00 30.00 50.00 70.00 90.00 110.00 130.00 150.00 164.00 178.00		Elev (ft) Wz (lb) Fsz (lbs) 10.00 5389.2 24.59 30.00 4712.7 69.00 50.00 4374.6 110.05 70.00 3979.7 142.55 90.00 3684.7 171.77 110.00 5916.1 353.31 130.00 2772.0 187.74 150.00 2216.3 172.22 164.00 807.42 64.33 178.00 4573.0 448.89

Load Case: 0.9D + 1.0Ev + 1.0Eh

Fa 1.6000 **Ke** 1.0698 **TL** 6.0000 **Dead Load Factor** 0.90 **Sds** 0.204 **Ss** 0.1920 **Seismic Load Factor** 1.00 **Sd1** 0.088 **S1** 0.0550 **Fv** 2.4000 **Kg** 0.0000 **Cs** 0.0459 Seismic Importance Factor Vs 1.7825 T 0.6396 **f1** 1.5634 1.00 **W1** 18.96 **R** 3.0000

Sec		Elev (ft)	Wz (lb)	Lateral Fsz (Ibs)	Vertical Ev (lbs)	
	1		5389.2	24.59	220.86	
	2	30.00	4712.7	69.00	193.13	
	3	50.00	4374.6	110.05	179.28	
	4	70.00	3979.7	142.55	163.09	
	5	90.00	3684.7	171.77	151.00	
	6	110.00	5916.1	353.31	242.45	
	7	130.00	2772.0	187.74	113.60	
	8	150.00	2216.3	172.22	90.83	
	9	164.00	807.42	64.33	33.09	
	10	178.00	4573.0	448.89	187.40	
	11	192.00	422.41	38.07	17.31	

Support Forces Summary

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: ||





Gh:	0.85	Topography: 1	S	truct Clas	s: II	Page: 18
Load (Case	Node	FX (kips)	FY (kips)	FZ (kips)	(-) = Uplift (+) = Down
1.2D + 1.0W	Normal Wind	1	0.01	238.06	-25.68	
		1a	8.40	-95.74	-8.20	
		1b	-8.41	-95.70	-8.19	
1.2D + 1.0W	60° Wind	1	-2.67	120.81	-12.62	
		1a	-12.23	120.34	4.05	
		1b	-19.29	-194.53	-11.16	
1.2D + 1.0W	/ 90° Wind	1	-3.18	15.55	-1.01	
	00 111110	1a	-19.30	199.98	9.40	
		1b	-17.65	-168.91	-8.39	
0.00 . 1.00	/ Normal Wind	1	0.01	222.00		
0.9D + 1.0W	Normal Wind	1 1a	0.01 8.62	233.98 -99.53	-25.42 -8.33	
		1b	-8.63	-99.49	-8.32	
0.9D + 1.0W	60° Wind	1	-2.68	116.83	-12.36	
		1a	-12.00	116.37	3.92	
		1b	-19.51 	-198.24	-11.29 	
0.9D + 1.0W	90° Wind	1	-3.18	11.66	-0.75	
		1a	-19.07	195.94	9.27	
		1b	-17.87	-172.63	-8.52	
1.2D + 1.0Di	i + 1.0Wi Normal Wind	1	0.00	85.09	-4.65	
		1a	3.60	1.35	-2.87	
		1b	-3.60	1.39	-2.87	
1 2D ± 1 0Di	i + 1.0Wi 60° Wind	1	-0.66	56.27	-1.47	
1.20 + 1.00	1 + 1.0001 00 Willia	1a	-1.59	56.14	0.18	
		1b	-6.40	-24.58	-3.70	
1.2D + 1.0Di	i + 1.0Wi 90° Wind	1	-0.78	29.27	1.48	
		1a	-3.37	76.28	1.52	
		1b	-5.95 	-17.71	-3.00	
1.2D + 1.0E	v + 1.0Eh	1	0.00	27.21	5.24	
		1a	5.89	10.50	-3.49	
		1b	-5.89	10.50	-3.49	
0.9D + 1.0E	v + 1.0Eh	1	0.00	23.31	5.50	
		1a	6.12	6.62	-3.62	
		1b	-6.12	6.62	-3.62	
 10D ± 10W	/ Normal Wind	1	0.00	68.26	-7.07	
1.0D 1 1.0W	Normal Willa	1a	1.58	-14.72	-1.77	
		1b	-1.58	-14.69	-1.76	
400 400						
1.0D + 1.0W	ν ους vvind	1	-0.69	39.12	-3.78	
		1a 1b	-3.61 -4.32	38.99 -39.25	1.30 -2.50	
				-39.20	-2.30	
$1.0D \pm 1.0W$	90° Wind	1	-0.82	12.95	-0.86	
1.00 + 1.000				E0.77	0.00	
1.00 + 1.000		1a 1b	-5.39 -3.91	58.77 -32.87	2.66 -1.80	

	Leg			Ove	Overturning				
N	/lax Uplift:	-198.24	(kips)	Moment:	4432.37	(ft-kips)			
M	ax Down:	238.06	(kips)	Total Down:	46.62	(kips)			
Ma	ax Shear:	25.68	(kips)	Total Shear:	42.07	(kips)			

Analysis Summary

Structure: CT06462-A-2-SBA **Code:** TIA-222-H 2/9/2022

Site Name:Mountain StreetExposure:BHeight:196.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 0.85 Topography: 1 Struct Class: II Page: 20



Max Reactions

	Leg		Ove	Overturning				
Max Uplift:	-198.24	(kips)	Moment:	4432.37	(ft-kips)			
Max Down:	238.06	(kips)	Total Down:	46.62	(kips)			
Max Shear:	25.68	(kips)	Total Shear:	42.07	(kips)			

Anchor Bolts

Bolt Size (in.): 1.00 Number Bolts: 10 Type: UnGrouted

Yield Strength (Ksi): 109.00 Tensile Strength (Ksi): 125.00

Interaction Ratios: Length: 0.75

Tensile: 0.35 Compression: 0.31

Max Usages

Max Leg: 53.6% (1.2D + 1.0W Normal Wind - Sect 2) Max Diag: 53.3% (1.2D + 1.0W 90° Wind - Sect 1) Max Horiz: 3.5% (0.9D + 1.0W 90° Wind - Sect 8)

Max Deflection, Twist and Sway

Load Case	Elevation (ft)	Deflection (ft)	Twist (deg)	Sway (deg)
0.9D + 1.0Ev + 1.0Eh - Normal To Face	120.00	0.0239	-0.0011	0.0227
	126.67	0.0265	0.0012	0.0245
	133.33	0.0294	-0.0012	0.0266
	155.00	0.0405	0.0015	0.0343
	160.00	0.0413	0.0000	0.0355
	164.00	0.0460	0.0015	0.0367
	168.00	0.0487	0.0014	0.0384
	184.00	0.0596	0.0012	0.0395
0.9D + 1.0W 121 mph Wind at 60° From Face	120.00	0.4045	-0.1400	0.3913
	126.67	0.4501	0.0425	0.3929
	133.33	0.5016	-0.2014	0.4391
	155.00	0.6884	-0.2779	0.5296
	160.00	0.7365	-0.2875	0.5216
	164.00	0.7742	0.0672	0.5709
	168.00	0.8160	-0.2855	0.5742
	184.00	0.9770	-0.2812	0.5831
0.9D + 1.0W 121 mph Wind at 90° From Face	120.00	0.4095	-0.1907	0.3987
	126.67	0.4565	0.0875	0.3748
	133.33	0.5076	-0.2757	0.4416
	155.00	0.6964	-0.3888	0.5212
	160.00	0.7455	-0.4044	0.4936
	164.00	0.7837	0.1254	0.5702
	168.00	0.8260	-0.4041	0.5803
	184.00	0.9887	-0.4010	0.5908

0.9D + 1.0W 121 mph Wind at Normal To Face	120.00	0.4275	-0.0708	0.4135
	126.67	0.4766	-0.0900	0.4872
	133.33	0.5303	0.0142	0.5186
	155.00	0.7281	-0.1119	0.6104
	160.00	0.7789	-0.0001	0.6402
	164.00	0.8205	-0.1075	0.6139
	168.00	0.8638	-0.1053	0.6115
	184.00	1.0351	-0.1004	0.6206
4.00 - 4.01/4.00				
1.0D + 1.0W 60 mph Wind at 60° From Face	120.00	0.1000	-0.0342	0.0963
	126.67	0.1113	0.0066	0.0968
	133.33	0.1238	-0.0493	0.1081
	155.00	0.1697	-0.0655	0.1299
	160.00	0.1815	-0.0670 0.0098	0.1280 0.1399
	164.00	0.1908	-0.0636	0.1399
	168.00 184.00	0.2010 0.2404	-0.0636 -0.0585	0.1410
			-0.0303	0.1427
1.0D + 1.0W 60 mph Wind at 90° From Face	120.00	0.1012	-0.0408	0.0979
	126.67	0.1129	0.0191	0.0922
	133.33	0.1253	-0.0589	0.1087
	155.00	0.1716	-0.0784	0.1284
	160.00	0.1837	-0.0802	0.1215
	164.00	0.1931	0.0240	0.1397
	168.00	0.2034	-0.0761	0.1426
	184.00	0.2432	-0.0699	0.1445
1.0D + 1.0W 60 mph Wind at Normal To Face	120.00	0.1059	-0.0166	0.1017
	126.67	0.1179	-0.0211	0.1199
	133.33	0.1312	0.0031	0.1274
	155.00	0.1799	-0.0252	0.1501
	160.00	0.1923	0.0000	0.1574
	164.00	0.2025	-0.0236	0.1508
	168.00	0.2131	-0.0228	0.1502
	184.00	0.2552	-0.0209	0.1524
1.2D + 1.0Di + 1.0Wi 50 mph Wind at 60° From Face	120.00	0.1051	-0.0421	0.1010
1.20 1 1.001 1 1.0001 00 mph wind at 00 110m1 acc	126.67	0.1154	0.0070	0.1006
	133.33	0.1299	-0.0610	0.1134
	155.00	0.1774	-0.0804	0.1354
	160.00	0.1888	-0.0822	0.1312
	164.00	0.1984	0.0099	0.1449
	168.00	0.2097	-0.0779	0.1470
	184.00	0.2509	-0.0717	0.1485
1.2D + 1.0Di + 1.0Wi 50 mph Wind at 90° From Face	120.00	0.1056	-0.0503	0.1022
	126.67	0.1161	0.0231	0.0936
	133.33	0.1305	-0.0729	0.1131
	155.00	0.1782	-0.0964	0.1321
	160.00	0.1903	-0.0986	0.1212
	164.00	0.1994	0.0282	0.1439
	168.00 184.00	0.2107 0.2520	-0.0934 -0.0858	0.1477 0.1495
		0.2520	-0.0056	0.1493
1.2D + 1.0Di + 1.0Wi 50 mph Wind at Normal From Face	120.00	0.1082	-0.0222	0.1043
	126.67	0.1207	-0.0280	0.1260
	133.33	0.1340	0.0049	0.1335
	155.00	0.1837	-0.0332	0.1546
	160.00	0.1955	0.0000	0.1640
	164.00	0.2069	-0.0313	0.1532
	168.00	0.2177	-0.0303	0.1531
	184.00	0.2606	-0.0278	0.1554
1.2D + 1.0Ev + 1.0Eh - Normal To Face	120.00	0.0239	0.0011	0.0227
	126.67	0.0265	0.0012	0.0246
	133.33	0.0294	-0.0012	0.0267
	155.00	0.0405	-0.0015	0.0344
	160.00	0.0413	0.0000	0.0355
	164.00	0.0460	0.0015	0.0367
	168.00	0.0487	-0.0014	0.0385
	184.00	0.0596	-0.0012	0.0396

1.2D + 1.0W 121 mph Wind at 60° From Face	120.00	0.4050	-0.1400	0.3918	
	126.67	0.4507	0.0425	0.3935	
	133.33	0.5022	-0.2014	0.4398	
	155.00	0.6892	-0.2780	0.5303	
	160.00	0.7374	-0.2876	0.5224	
	164.00	0.7751	0.0673	0.5717	
	168.00	0.8170	-0.2856	0.5749	
	184.00	0.9782	-0.2812	0.5840	
1.2D + 1.0W 121 mph Wind at 90° From Face	120.00	0.4100	-0.1908	0.3991	
·	126.67	0.4570	0.0875	0.3753	
	133.33	0.5081	-0.2757	0.4423	
	155.00	0.6973	-0.3889	0.5221	
	160.00	0.7464	-0.4045	0.4944	
	164.00	0.7847	0.1255	0.5711	
	168.00	0.8270	-0.4042	0.5813	
	184.00	0.9899	-0.4012	0.5916	
1.2D + 1.0W 121 mph Wind at Normal To Face	120.00	0.4280	-0.0707	0.4140	
·	126.67	0.4772	-0.0899	0.4878	
	133.33	0.5309	0.0141	0.5191	
	155.00	0.7291	-0.1118	0.6113	
	160.00	0.7799	-0.0001	0.6412	
	164.00	0.8216	-0.1074	0.6149	
	168.00	0.8649	-0.1052	0.6122	
	184.00	1.0365	-0.1003	0.6216	

Last revised on 1-3-2022



Mot Foun	dation Design for Salf Sun	norting Tower	Date			
Wat Four	Mat Foundation Design for Self Supporting Tower					
Customer Name:	sustomer Name: SBA Communications Corp TIA Standard:					
Site Name:		Structure Height (Ft.):	196			
Site Nmber:	CT06462-A-2-SBA	Engineer Name:	J. Tibbetts			
Engr. Number:	123543	Engineer Login ID:				

Foundation Info Obtained	from:
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Analysis or Design?

Number of Tower Legs:

Base Reactions (Factored):

(1). Individual Leg:

Axial Load (Kips):

Shear Force (Kips):

(2). Tower Base:

Total Vertical Load (Kips):

4432.4 Moment (Kips-ft):

238.1

25.7

46.6

2.2

0

36

4.00

Foundation Geometries: Leg distance (Center-to-Center ft.):

23.0 Diameter of Pier (ft.): Round

Tower center to mat center (ft): Length of Pad (ft.):

Thickness of Pad (ft):

Mods required -Yes/No ?: Pier Height A. G. (ft.):

Depth of Base BG (ft.):

Total Shear Force (Kips):

Width of Pad (ft.):

Uplift Force (Kips):

Drawings/Calculations

Analysis

3 Legs

198.2

42.1

No

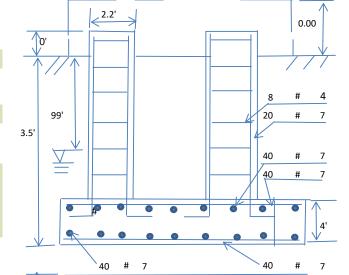
0.00

3.5

36

40

40



6.640

0.00

13.279

18.0

Mat Center

4.72

11.360

23.0

Material Properties and Reabr Info:

Concrete Strength (psi):	3000	Steel Elastic Modulus:	29000	ksi
Vertical bar yield (ksi)	60	Tie steel yield (ksi):	60	
Vertical Rebar Size #:	7	Tie / Stirrup Size #:	4	
Qty. of Vertical Rebars:	20	Tie Spacing (in):	6.0	
Pad Rebar Yield (Ksi):	60	Pad Steel Rebar Size (#):	7	
Concrete Cover (in.):	3	Unit Weight of Concrete:	150.0	pcf
5 1 111 1 11 (11				

Rebar at the bottom of the concrete pad:

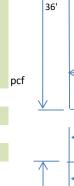
Qty. of Rebar in Pad (L): 40

Rebar at the top of the concrete pad:

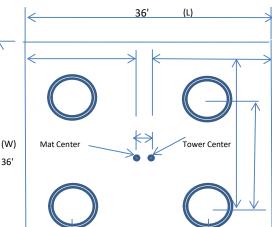
Qty. of Rebar in Pad (L):

Qty. of Rebar in Pad (W):

40 Qty. of Rebar in Pad (W):



(W)



19.919

Soil Design Parameters:

Soil Unit Weight (pcf):	120.0	Soil Buoyant Weight:	50.0	Pcf
Water Table B.G.S. (ft):	99.0	Unit Weight of Water:	62.4	pcf
Ultimate Bearing Pressure (psf):	4000	Consider ties in concrete shear st	rength:	Yes
Consider Soil Lateral Resistance ?	Yes	Enter soil C (psf) or Phi (deg.):	30.0	Deg.
		Depth to ignor lateral resistance	1.0	Ft.

2/9/2022 TES Engr. Number: 123543 Page 2/2 Date:

Apply 1.35 for e/v	w per G/H: 1	
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Foundation Analysis and Design: Uplift Strength Reduction Factor:	0.75	Compre	ession Strength Reduction Factor:	0.75		
Total Dry Soil Volume (cu. Ft.):	2.57	Total Di	ry Soil Weight (Kips):	0.31		
Total Buoyant Soil Volume (cu. Ft.):	0.00	Total Bu	uoyant Soil Weight (Kips):	0.00		
Total Effective Soil Weight (Kips):	0.31	Weight	from the Concrete Block at Top (K):	0.00		
Total Dry Concrete Volume (cu. Ft.):	5184.08	Total Di	ry Concrete Weight (Kips):	777.61		
Total Buoyant Concrete Volume (cu. Ft.):	0.00	Total Bu	uoyant Concrete Weight (Kips):	0.00		
Total Effective Concrete Weight (Kips):	777.61	Total Ve	ertical Load on Base (Kips):	824.54		
					Load/	
Check Soil Capacities:					Capacity Ratio	
Calculated Maxium Net Soil Pressure under the base (psf):	1205.34	<	Allowable Factored Soil Bearing (psf):	3000	0.40	OK!
Allowable Foundation Overturning Resistance (kips-ft.):	13441.4	>	Design Factored Momont (kips-ft):	4601	0.34	OK!
Factor of Safety Against Overturning (O. R. Moment/Design Moment):	2.92	OK!				
Check the capacities of Reinforceing Concrete:						
Strength reduction factor (Flexure and axial tension):	0.90	Strengt	h reduction factor (Shear):	0.75		
Strength reduction factor (Axial compresion):	0.65	Wind Lo	oad Factor on Concrete Design:	1.00		
					Load/	
(1) Concrete Pier:					Capacity Ratio	
Vertical Steel Rebar Area (sq. in./each):	0.60		Tie / Stirrup Area (sq. in./each):	0.20		
Calculated Moment Capacity (Mn,Kips-Ft):	358.1		Design Factored Moment (Mu, Kips-Ft)	0.2	0.00	OK!
Calculated Shear Capacity (Kips):	70.5		Design Factored Shear (Kips):	25.7	0.36	OK!
Calculated Tension Capacity (Tn, Kips):	648.0		Design Factored Tension (Tu Kips):	198.2	0.31	OK!
Calculated Compression Capacity (Pn, Kips):	709.9		Design Factored Axial Load (Pu Kips):	238.1	0.34	OK!
Moment & Tension Strength Combination:	0.00		Check Tie Spacing (Design/Req'd):	0.57		
Pier Reinforcement Ratio:	0.022		nforcement Ratio is satisfied per ACI			
(2).Concrete Pad:						
One-Way Design Shear Capacity (L or W Direction, Kips):	1581.6		One-Way Factored Shear (L/W-Dir Kips	399.0	0.25	OK!
One-Way Design Shear Capacity (Diagonal Dir., Kips):	1332.7		One-Way Factored Shear (Dia. Dir, Kips	358.9	0.27	OK!
Lower Steel Pad Reinforcement Ratio (L or W-Direct.):	0.0012		Lower Steel Reinf. Ratio (Dia. Dir.):	0.0011		
Lower Steel Pad Moment Capacity (L or W-Dir. Kips-ft):	4742.2	>	Moment at Bottom (L-Direct. K-Ft):	2097.8	0.44	OK!
Lower Steel Pad Moment Capacity (Dia. Direction,K-ft):	4597.7		Moment at Bottom (Dia. Dir. K-Ft):	1744.6	0.38	OK!
Upper Steel Pad Reinforcement Ratio (L or W -Direction):	0.0012		Upper Steel Reinf. Ratio (Dia. Dir.):	0.0011		
Upper Steel Pad Moment Capacity (L or W-Dir., Kips-ft):	4742.2	>	Moment at the top (L-Dir Kips-Ft):	977.2	0.21	OK!
Upper Steel Pad Moment Capacity (Dia. Direction, K-ft):	4597.7	>	Moment at the top (Dia. Dir., K-Ft):	605.5	0.13	OK!
Punching Failure Capacity From Down Load (Kips):	1632.4	>	Punch. Failure Factored Shear (K):	238.1	0.15	OK!
Punching Failure Capacity From Uplift (Kips):	1474.3	>	Punch. Failure Factored Shear (K):	198.2	0.13	OK!
(3). Check Max. eccentricity of Loading:						
The maximum eccentricity of Loading:	5.58	ft.	Allowable eccentricity (0.45 W, ft.):	16.2		OK!
		-		-		J



Raf: ES-055 Dep: cdt

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SHIP DATE: 07MAR22 ACTWGT: 1.00 LB MAN CAD: 0517347/CAFE3509

NEW HAVEN, CT 065115920 UNITED STATES US

BILL THIRD PARTY

TO HONORABLE TOM DEVIVI MAYOR TOWN OF WINDHAM 979 MAIN STREET

WILLIMANTIC CT 06226

DEPT: CDT

FedEx Express

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SHIP DATE: 07MAR22 ACTWGT: 1.00 LB MAN CAD: 0517347/CAFE3509

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WILLIMANTIC CT 06226

REF: ES-055 WILLIMANTIC



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ORIGIN ID:EFBA (203) 562-9885 SHIPPING JOSEPH MERRITT CO. 60 HAMILTON STREET

SHIP DATE: 07MAR22 ACTWGT: 1.00 LB MAN CAD: 0517347/CAFE3509

NEW HAVEN, CT 065115920 UNITED STATES US

BILL THIRD PARTY

TO

CONNECTICUT SITING COUNCIL 10 FRANKLIN SQUARE

NEW BRITAIN CT 06051

REF: CT578150

DEPT: CDT



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TO CT06462-A, MOUNTAIN STREET SBA COMMUNICATIONS CORP 8051 CONMGRESS AVENUE

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Calculated Radio Frequency Emissions Report



ES-055 – Willimantic

349 Mountain Road

Windham, CT 06280

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1. Introduction

The purpose of this report is to investigate compliance with applicable FCC regulations for the Eversource installation on the self-support tower at 349 Mountain Street in Windham, CT. Eversource has recently installed one omnidirectional antenna for both transmit and receive purposes as part of its 220 MHz communications system. The original proposal consisted of two omnidirectional antennas – one for transmit and one receive-only antenna.

This report considers the updated antenna configuration as detailed by Eversource along with % MPE (Maximum Permissible Exposure) measurements around the existing tower taken prior to the modifications to determine FCC compliance of the facility. Please note that there is a guyed tower within the same compound (see below), which would contribute to the % MPE measurements recorded during the field survey.

In order to account for any configuration changes by others since the field measurements were recorded, application materials posted on the CT Siting Council website for the subject site were reviewed. That research indicates that Verizon has recently completed modifications to their equipment and power density information for that installation as detailed in its Notice of Exempt Modification filing dated August 4, 2021 (EM-VER-163-210805) is considered for completeness.



Figure 1: View of ES-055 Willimantic

Site Address	349 Mountain Street
Latitude	41° 42' 10.84" N
Longitude	73° 13' 17.01" W
Site Elevation AMSL	525'
Survey Engineer	Marc Salas
Survey Date/Time	6/29/2020; 1:30 PM – 2:15 PM

able 1: Survey Information

ES-055 Willimantic 1 February 25, 2022



2. FCC Guidelines for Evaluating RF Radiation Exposure Limits

In 1985, the FCC established rules to regulate radio frequency (RF) exposure from FCC licensed antenna facilities. In 1996, the FCC updated these rules, which were further amended in August 1997 by OET Bulletin 65 Edition 97-01. These new rules include Maximum Permissible Exposure (MPE) limits for transmitters operating between 300 kHz and 100 GHz. The FCC MPE limits are based upon those recommended by the National Council on Radiation Protection and Measurements (NCRP), developed by the Institute of Electrical and Electronics Engineers, Inc., (IEEE) and adopted by the American National Standards Institute (ANSI).

The FCC general population/uncontrolled limits set the maximum exposure to which most people may be subjected. General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure.

Public exposure to radio frequencies is regulated and enforced in units of milliwatts per square centimeter (mW/cm²). The general population exposure limits for the various frequency ranges are defined in the attached "FCC Limits for Maximum Permissible Exposure (MPE)" in Attachment B of this report.

Higher exposure limits are permitted under the occupational/controlled exposure category, but only for persons who are exposed as a consequence of their employment and who have been made fully aware of the potential for exposure, and they must be able to exercise control over their exposure. General population/uncontrolled limits are five times more stringent than the levels that are acceptable for occupational, or radio frequency trained individuals. Attachment B contains excerpts from OET Bulletin 65 and defines the Maximum Exposure Limit.

Finally, it should be noted that the MPE limits adopted by the FCC for both general population/uncontrolled exposure and for occupational/controlled exposure incorporate a substantial margin of safety and have been established to be well below levels generally accepted as having the potential to cause adverse health effects.

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3. Power Density Calculation Methods

The power density calculation results were generated using the following formula as outlined in FCC bulletin OET 65, and Connecticut Siting Council recommendations:

Power Density =
$$\left(\frac{1.6^2 \times 1.64 \times ERP}{4\pi \times R^2}\right)$$
 X Off Beam Loss

Where:

EIRP = Effective Isotropic Radiated Power = 1.64 x ERP

R = Radial Distance = $\sqrt{(H^2 + V^2)}$

H = Horizontal Distance from antenna

V = Vertical Distance from radiation center of antenna

Ground reflection factor of 1.6

Off Beam Loss is determined by the selected antenna pattern

These calculations assume that the antennas are operating at 100 percent capacity and full power, and that all antenna channels are transmitting simultaneously. Obstructions (trees, buildings, etc.) that would normally attenuate the signal are not taken into account. The calculations assume even terrain in the area of study and do not consider actual terrain elevations which could attenuate the signal. As a result, the calculated power density and corresponding % MPE levels reported below are much higher than the actual levels will be from the installation.

4. Installed Antenna Configuration

Table 2 below lists the technical details of the Eversource installation. These parameters are applied to the above calculation methods in order to calculate the % MPE values of the recently installed Eversource equipment. Any receive only antennas are not included in the % MPE calculations and are therefore not listed in the table below.

Operator	Antenna Model	TX Freq. (MHz)	Ant Gain (dBd)	Power ERP (Watts)	Number of Channels	Vertical Beamwidth	Length (ft)	Antenna Centerline Height (ft)
Eversource	dB Spectra SP2D00P36D-D	217	0.0	124	4	60°	15.6	156.5

Table 2: Eversource Antenna Configuration (Proposed)^{1 2}

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¹ Transmit power assumes 0 dB of cable loss.

² Transmit antenna height listed is based on the Tower Engineering Solutions Structural Analysis Report dated February 9, 2022.



5. Measurement Procedure

Frequencies from 300 KHz to 50 GHz were measured using the Narda Probe EA 5091, E-Field, shaped, FCC probe in conjunction with the NBM550 survey meter. The EA 5091 probe is "shaped" such that in a mixed signal environment (i.e.: more than one frequency band is used in a particular location), it accurately measures the percent of MPE.

From FCC OET Bulletin No. 65 - Edition 97-01 — "A useful characteristic of broadband probes used in multiple-frequency RF environments is a frequency-dependent response that corresponds to the variation in MPE limits with frequency. Broadband probes having such a "shaped" response permit direct assessment of compliance at sites where RF fields result from antennas transmitting over a wide range of frequencies. Such probes can express the composite RF field as a percentage of the applicable MPEs".

Probe Description - As suggested in FCC OET Bulletin No. 65 - Edition 97-01, the response of the measurement instrument should be essentially isotropic, (i.e., independent of orientation or rotation angle of the probe). For this reason, the Narda EA 5091 probe was used for these measurements.

Sampling Description - At each measurement location, a spatially averaged measurement is collected over the height of an average human body. The NBM550 survey meter performs a time average measurement while the user slowly moves the probe over a distance range of 20 cm to 200 cm (about 6 feet) above ground level. The results recorded at each measurement location include average values over the spatial distance.

Instrumentation Information - A summary of specifications for the equipment used is provided in the table below.

Manufacturer	Narda Microwave	Narda Microwave					
Probe	EA 5091, Serial# 0116						
Calibration Date	May 2020						
Calibration Interval	24 Months						
Meter	NBM550, Serial# E-1069						
Calibration Date	May 2020						
Calibration Interval	24 Months						
	Enggueray Panca	Field Measured	Standard	Measurement			
Probe Specifications Frequency Range Field Measured Standard							
1 Tobe opecinications	300 KHz-50 GHz	Electric Field	U.S. FCC 1997	0.2 – 600 % of			
	500 KHZ-50 GHZ	Electric Field	Occupational/Controlled	Standard			

Table 3: Instrumentation Information

Instrument Measurement Uncertainty - The total measurement uncertainty of the NARDA measurement probe and meter is no greater than ± 3 dB (0.5% to 6%), ± 1 dB (6% to 100%), ± 2 dB (100% to 600%). The factors which contribute to this include the probe's frequency response deviation, calibration uncertainty, ellipse ratio, and isotropic response³. Every effort is taken to reduce the overall uncertainty during measurement collection including pointing the probe directly at the likely highest source of emissions.

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³ For further details, please refer to Narda Safety Test Solutions NBM550 Probe Specifications, pg. 64 http://www.narda-sts.us/pdf_files/DataSheets/NBM-Probes_DataSheet.pdf



6. Surveyed and Calculated % MPE Results

Measured and calculated results and a description of each survey location are detailed in the table below. Measurements were recorded on June 29, 2020 between 1:30 PM and 2:15 PM. The calculated % MPE contribution from the recently installed Eversource equipment along with the net change of Verizon's % MPE from its recent petition (13.70%)⁴ compared to Verizon's % MPE (10.00%) listed in the CT Siting Council power density database dated July 7, 2021 was then added to the measured % MPE values in the "Composite % MPE" column. These calculated values for the Eversource 220 MHz equipment incorporate the antenna pattern of the antenna model specified by Eversource to determine the "Off Beam Loss" factor shown in the power density formula from Section 3. All % MPE values are in reference to the FCC Uncontrolled/General Population exposure limit.

Table 4 below lists 16 measurements recorded in the vicinity of the tower. The highest spatially averaged measurement was 5.71% (Average Uncontrolled / General Population MPE) and was recorded at Location 11 by the mailbox for 875 Mountain Street. The highest composite (measured + calculated) % MPE value is calculated to be 9.46% (Average Uncontrolled / General Population) and is also calculated to occur at Location 11.

Meas. Location	Location Description	Latitude	Longitude	Dist. From Site (feet)	Measured % MPE (Uncontrolled / General)	Calculated % MPE (Eversource Proposed)	Calculated Verizon % MPE change from EM- VER-163-210805	Composite % MPE (Uncontrolled / General)
1	Double swing gate on east side of fenced compound	41.70297	-72.22119	56	1.86%	0.77%		6.33%
2	Single swing on east side of fenced compound near wood framed shelter	41.70285	-72.22124	72	2.58%	1.06%		7.34%
3	Double swing gate on south side of fenced compound	41.70282	-72.22148	74	2.31%	1.11%		7.12%
4	Near the NE corner of the fenced compound	41.70310	-72.22131	40	< 1.00%	0.48%		< 4.18%
5	Near the NW corner of the fenced compound	41.70318	-72.22157	80	< 1.00%	1.14%		< 4.84%
6	NE of compound, along power line access way	41.70308	-72.22095	121	1.19%	1.57%		6.46%
7	Along gravel access road	41.70283	-72.22066	208	< 1.00%	1.30%		< 5.00%
8	Along gravel access road	41.70271	-72.22025	328	< 1.00%	0.67%	3.70%	< 4.37%
9	Along gravel access road at bend	41.70261	-72.21965	495	< 1.00%	0.32%	3./0%	< 4.02%
10	At tower access road gate	41.70199	-72.21879	801	< 1.00%	0.12%		< 3.82%
11	By mailbox for 875 Mountain Street	41.70191	-72.21744	1149	5.71%	0.06%		9.46%
12	By stop sign on Southridge Drive at Mountain Street intersection	41.70127	-72.21729	1284	5.12%	0.05%		8.86%
13	Spring View Lane, end of cul-de-sac	41.69999	-72.21718	1593	3.45%	0.03%		7.18%
14	SW corner of Orchard Hill Lane split	41.70165	-72.21337	2244	3.82%	0.02%		7.54%
15	By stop sign on Young Street at Lebanon Avenue intersection	41.70667	-72.21476	2246	3.01%	0.02%		6.73%
16	Adam Heights Road, at dead end	41.70277	-72.22406	735	1.52%	0.15%		5.36%

Table 4: Measured and Calculated % MPE Results 5

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⁴ Connecticut Siting Council Notice of Exempt Modification, 349R Mountain Street, Windham (Willimantic), CT dated August 4, 2021 https://portal.ct.gov/-/media/CSC/2 EMS-medialibrary/Windham Willimantic/MountainSt/Verizon/em-ver-163-210805 filing 349R-Mountain-Street-Windham-Willimantic-CT-with-Attachments.pdf

⁵ Due to measurement uncertainty at low levels (See Table 3), any readings outside the measurement range of the probe (< 1.00 % FCC General Population/Uncontrolled MPE) are noted as such.



Figures 2 and 3 below are aerial views⁶ of the tower location and the surrounding area, along with the measurement locations listed in Table 4.



Figure 2: Measurement Points – Zoom In



Figure 3: All Measurement Points

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⁶ Map showing location of telecommunications facility and the surrounding area. *Google Earth*, https://earth.google.com/web/.



7. Conclusion

A number of accessible areas around the tower at 349 Mountain Street in Windham, CT were surveyed and found to be well within the mandated General Population/Uncontrolled limits for Maximum Permissible Exposure, as delineated in the Federal Communications Commission's Radio Frequency exposure rules published in 47 CFR 1.1307(b)(1)-(b)(3).

The highest spatially averaged % MPE measurement of all surveyed points based on the 1997 FCC standard for exposure to the general population is 5.71% MPE. This measurement was recorded at Location 11 by the mailbox for 875 Mountain Street.

The highest composite (measured + calculated) power density is 9.46% of the FCC General Population MPE limit with the recently installed Eversource 220 MHz equipment and net % MPE change of Verizon's equipment modifications (as documented in their latest petition and the CT Siting Council power density database), is also calculated to occur at Location 11.

The above analysis concludes that RF exposure at ground level around the tower will be below the maximum power density limits as outlined by the FCC in the OET Bulletin 65 Ed. 97-01.

As noted previously, the calculated % MPE levels are more conservative (higher) than the actual levels will be from the finished installation.

8. Statement of Certification

I certify to the best of my knowledge that the statements in this report are true and accurate. The calculations follow guidelines set forth in FCC OET Bulletin 65 Edition 97-01, IEEE Std. C95.1, and IEEE Std. C95.3.

Kerth Willande

February 25, 2022

Report Prepared By:

Keith Vellante Director of RF Services C Squared Systems, LLC Date

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Attachment A: References

OET Bulletin 65 - Edition 97-01 - August 1997 Federal Communications Commission Office of Engineering & Technology

<u>IEEE C95.1-2005, IEEE Standard Safety Levels With Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz</u> IEEE-SA Standards Board

IEEE C95.3-2002 (R2008), IEEE Recommended Practice for Measurements and Computations of Radio Frequency Electromagnetic Fields With Respect to Human Exposure to Such Fields, 100 kHz-300 GHz IEEE-SA Standards Board

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Attachment B: FCC Limits for Maximum Permissible Exposure (MPE)

(A) Limits for Occupational/Controlled Exposure⁷

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm ²)	Averaging Time $ E ^2$, $ H ^2$ or S (minutes)
0.3-3.0	614	1.63	(100)*	6
3.0-30	1842/f	4.89/f	$(900/f^2)*$	6
30-300	61.4	0.163	1.0	6
300-1500	-	-	f/300	6
1500-100,000	-	-	5	6

(B) Limits for General Population/Uncontrolled Exposure⁸

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm ²)	Averaging Time $ E ^2$, $ H ^2$ or S (minutes)
0.3-1.34	614	1.63	(100)*	30
1.34-30	824/f	2.19/f	$(180/f^2)*$	30
30-300	27.5	0.073	0.2	30
300-1500	-	-	f/1500	30
1500-100,000	-	-	1.0	30

f = frequency in MHz * Plane-wave equivalent power density

Table 5: FCC Limits for Maximum Permissible Exposure (MPE)

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⁷ Occupational/controlled limits apply in situations in which persons are exposed as a consequence of their employment provided those persons are fully aware of the potential for exposure and can exercise control over their exposure. Limits for occupational/controlled exposure also apply in situations when an individual is transient through a location where occupational/controlled limits apply provided he or she is made aware of the potential for exposure

⁸ General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure



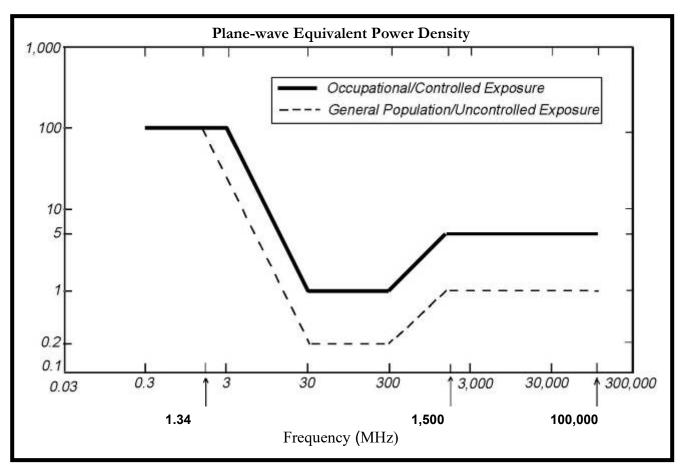


Figure 4: Graph of FCC Limits for Maximum Permissible Exposure (MPE)



Attachment C: Eversource Antenna Data Sheet and Electrical Patterns

217 MHz

Manufacturer: dB Spectra

Model #: SP2D00P36D-D

Frequency Band: 217-220 MHz

Gain: 0 dBd

Vertical Beamwidth: 60° Horizontal Beamwidth: 360°

Polarization: Vertical-Polarization

Length: 15.6'

