March 14, 2016

VIA EMAIL AND OVERNIGHT DELIVERY

Ms. Melanie A. Bachman Acting Executive Director Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

RE: T-Mobile Northeast LLC - CT11414A Notice of Exempt Modification Pimpewaug Road, Wilton, CT

LAT: 41.21247 N LNG: -73.42794 W

Dear Ms. Bachman:

T-Mobile Northeast LLC ("T-Mobile") currently maintains two (2) antennas at the 132' level on the existing 125' lattice transmission tower located at Pimpewaug Road in Wilton, CT. The tower is owned by Eversource Energy. T-Mobile now intends to replace the two (2) existing antennas with two (2) new 1900/2100 MHz antennas on a replacement pipe mast. These antennas would be installed at the 132' level of the tower. T-Mobile will also remove an existing accelerator antenna and mounting pipe, install coax cables, reuse existing coax cables, remove coax cable, replace two (2) existing GMA with four (4) GMA and replace an equipment cabinet on an existing concrete pad. See the plans attached as **Exhibit A**. With modifications, the existing facility is structurally capable of supporting T-Mobile's proposed installation, as indicated in the structural analysis dated June 5, 2014, and attached as **Exhibit B**.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. 16-50j-72(b)(2). In accordance with R.C.S.A. 16-50j-73, a copy of this letter is being sent to First Selectman Lynne Vanderslice, as well as the property owner, Eversource. Please see the engineering letter of approval from Eversource attached as **Exhibit C.**

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. 16-50j-72(b)(s).

The proposed modifications will not result in an increase in the height of the
existing structure. T-Mobile's existing antennas are at a centerline of 132' AGL;
the replacement antennas will be installed at the same level. The attached tower
drawing confirms that the proposed modification will not increase the height of the
tower.

- 2. The proposed modifications will not require the extension of the site boundary or lease area, as depicted on the attached site plan. T-Mobile's equipment will be located entirely within the existing compound area.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria. The incremental effect of the proposed changes will be negligible.
- 4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard. As indicated in the attached power density calculations, T-Mobile's operations at the site will result in a power density of 4.64%; the combined site operations will result in a total power density of 4.64% as evidenced by the power density calculations attached as Exhibit D..
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitute an exempt modification under R.C.S.A. 16-50j-72(b)(2).

Respectfully submitted,

Bv:

Eric Dahl, Agent for T-Mobile

edahl@comcast.net

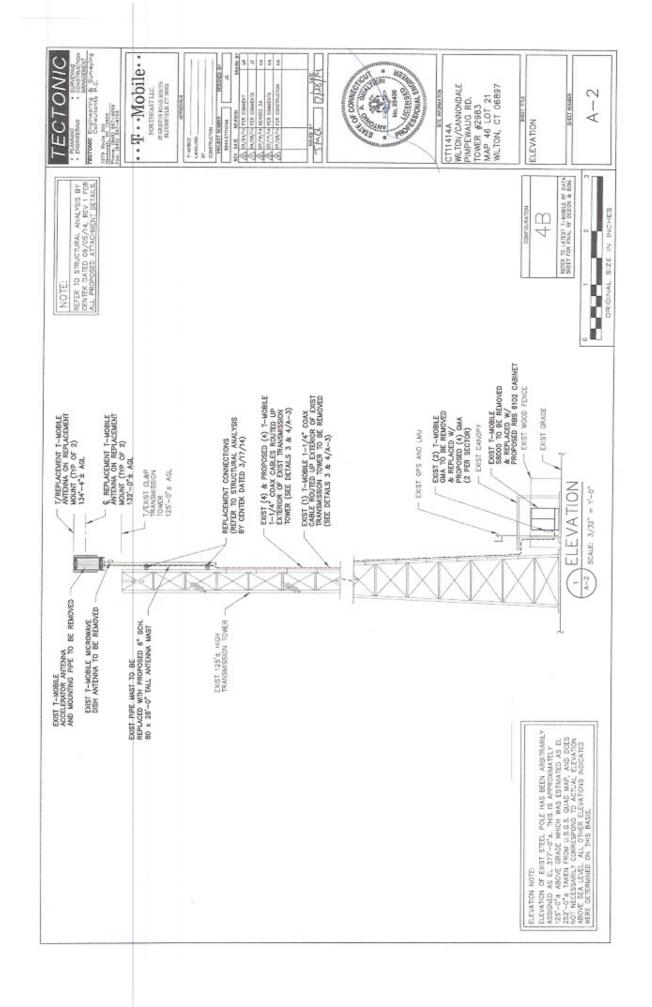
860-227-1975

Attachments

cc: First Selectman Lynne Vanderslice, Town of Wilton

Eversource - as tower and property owner

EXHIBIT A



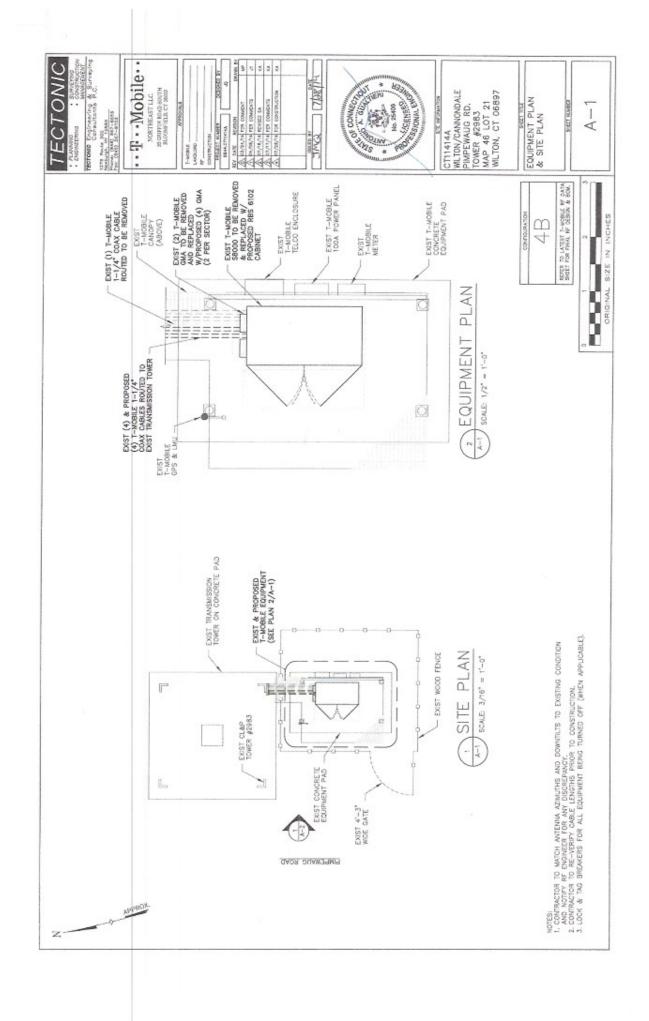


EXHIBIT B



Centered on Solutions™

Structural Analysis of Antenna Mast and CL&P Tower

T-Mobile Site Ref: CT11414A

CL&P Structure No. 2983 125' Electric Transmission Lattice Tower

> Pimpewaug Road Wilton, CT

CENTEK Project No. 14025.008

Date: March 17, 2014 Rev 1: June 5, 2014



Prepared for: T-Mobile Towers 4 Sylvan Way Parsippany, NJ 07054 CENTEK Engineering, Inc. Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5, 2014

Table of Contents

SECTION 1 - REPORT

- INTRODUCTION
- PRIMARY ASSUMPTIONS USED IN THE ANALYSIS
- ANALYSIS
- DESIGN BASIS
- RESULTS
- CONCLUSION

SECTION 2 - CONDITIONS & SOFTWARE

- STANDARD ENGINEERING CONDITIONS
- GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAMS
 - RISA 3-D
 - PLS TOWER

SECTION 3 - DESIGN CRITERIA

- CRITERIA FOR DESIGN OF PCS FACILITIES ON OR EXTENDING ABOVE METAL ELECTRIC TRANSMISSON TOWERS
- NU DESIGN CRITERIA TABLE
- PCS SHAPE FACTOR CRITERIA
- WIRE LOADS SHEET

SECTION 4 - DRAWINGS

- T-1 TITLE SHEET
- N-1 DESIGN BASIS AND GENERAL NOTES
- N-2 STRUCTURAL STEEL NOTES
- MI-1 MODIFICATION INSPECTION REQUIREMENTS
- S-1 TOWER ELEVATION AND FEEDLINE PLAN
- S-2 ANTENNA MAST DETAILS

SECTION 5 - EIA/TIA-222-F LOAD CALCULATIONS FOR ANTENNA MAST ANALYSIS

ANTENNA MAST WIND & ICE LOAD

TABLE OF CONTENTS

CENTEK Engineering. Inc. Structural Analysis — 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade — CT11414A Wilton, CT Rev 1 ~ June 5, 2014

SECTION 6 - ANTENNA MAST ANALYSIS PER EIA/TIA-222F

- LOAD CASES AND COMBINATIONS (TIA/EIA LOADING)
- RISA 3-D ANALYSIS REPORT
- MAST CONNECTION ANALYSIS

SECTION 7 - NECS/NU LOAD CALCULATIONS FOR OBTAINING REACTIONS APPLIED TO UTILITY STRUCTURE

- ANTENNA MAST WIND LOAD
- SECTION 8 ANTENNA MAST ANALYSIS PER NESC/NU FOR OBTAINING REACTIONS APPLIED TO UTILITY STRUCTURE
- LOAD CASES AND COMBINATIONS (NESC/NU LOADING)
- RISA 3-D ANALYSIS REPORT
- SECTION 9 PLS TOWER RESULTS FROM ANTENNA MAST
 REACTIONS CALCULATED IN RISA WITH NESC/NU
 CRITERIA
- COAX CABLE LOAD ON CL&P TOWER CALCULATION
- PLS REPORT
- FOUNDATION ANALYSIS

SECTION 10 - REFERENCE MATERIAL

- RFDS SHEET
- EQUIPMENT CUT SHEETS

Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5, 2014

Introduction

The purpose of this report is to analyze the existing antenna mast and 125' CL&P tower located on Pimpewaug Road in Wilton, CT for the proposed T-Mobile antenna upgrade.

The existing and proposed loads consist of the following:

T-MOBILE (Existing to be Removed):

Antennas: One (1) EMS MTRR75-17-XXXDPL accellerator and one (1) microwave dish mounted to the existing antenna mast with a RAD center elevation of 132-ft above grade. Coax Cables: One (1) 1-1/4" Ø coax cables running on a leg of the existing tower as indicated in section 4 of this report Mast: Pipe 4" Sch. 80 (O.D. = 4.5").

T-MOBILE (Existing to Remain):

Coax Cables: Four (4) 1-1/4" Ø coax cables running on a leg of the existing tower as indicated in section 4 of this report.

T-MOBILE (Proposed):

Antennas: Two (2) RFS APX16DWV-16DWVS-E-A20 panel antennas flush mounted to the proposed antenna mast with a RAD center elevation of 132-ft above grade.

Coax Cables: Four (4) 1-1/4" Ø coax cables running on a leg of the existing tower as indicated in section 4 of this report.

Mast: Pipe 6" Sch. 80 (O.D. = 6.625") x 28-ft Long.

Primary assumptions used in the analysis

- Allowable steel stresses are defined by AISC-ASD 9th edition for design of the antenna mast and antenna supporting elements.
- ASCE Manual No. 10-97, "Design of Latticed Steel Transmission Structures", defines allowable steel stresses for evaluation of the CL&P utility tower.
- All utility tower members are adequately protected to prevent corrosion of steel members.
- All proposed antenna mounts are modeled as listed above.
- All coaxial cable will be installed as indicated in Section 4 of this report.
- Antenna mast will be properly installed and maintained.
- No residual stresses exist due to incorrect tower erection.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds conform to the requirements of AWS D1.1.
- Antenna mast and utility tower will be in plumb condition.
- Utility tower was properly installed and maintained and all members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
- Any deviation from the analyzed loading will require a new analysis for verification of structural adequacy.

CENTEK Engineering, Inc. Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5, 2014

Analysis

Structural analysis of the existing Antenna Mast was independently completed using the current version of RISA-3D computer program licensed to CENTEK Engineering, Inc.

The existing antenna mast consisting of a 4" Sch. 40 pipe conforming to ASTM A53 Grade B (Fy = 35ksi) connected at four points to the existing tower was analyzed for its ability to resist loads prescribed by the TIA/EIA standard. Section 5 of this report details these gravity and lateral wind loads. NESC prescribed loads were also applied to the PCS Mast in order to obtain reactions needed for analyzing the CL&P tower structure. These loads are developed in Section 7 of this report. Load cases and combinations used in RISA-3D for TIA/EIA loading and for NESC/NU loading are listed in report Sections 6 and 8, respectively.

An envelope solution was first made to determine maximum and minimum forces, stresses, and deflections to confirm the selected section as adequate. Additional analyses were then made to determine the NESC forces to be applied to the CL&P tower structure.

The RISA-3D program contains a library of all AISC shapes and corresponding section properties are computed and applied directly within the program. The program's Steel Code Check option was also utilized. The forces calculated in RISA-3D using NESC guidelines were then applied to the CL&P tower using PLS-Tower. Maximum usage for the tower was calculated considering the additional forces from the mast and associated appurtenances.

Design Basis

Our analysis was performed in accordance with EIA-222-F-1996, ASCE Manual No. 10-97, "Design of Latticed Steel Transmission Structures", NESC C2-2007 and Northeast Utilities Design Criteria.

The CL&P tower structure, considering existing and future conductor and shield wire loading, with the proposed antenna mast was analyzed under two conditions:

UTILITY TOWER ANALYSIS

The purpose of this analysis is to determine the adequacy of the existing utility structure to support the proposed antenna loads. The loading and design requirements were analyzed in accordance with the NU Design Criteria Table, NESC C2-2007 ~ Construction Grade B, and ASCE Manual No. 10-97, "Design of Latticed Steel Transmission Structures".

Load cases considered:

Lood Coop 1: NESC Home

Load C	ase 1: NESC Heavy	
Wind F	ressure	4.0 psf
Radial	Ice Thickness	0.5"
Vertica	Overload Capacity Factor	1.50
Wind C	Overload Capacity Factor	2.50
Wire T	ension Overload Capacity Factor	1.65
	ase 2: NESC Extreme	
Wind S	peed 1	10 mph (1)
Radial	Ice Thickness	0"
Note 1:	NESC C2-2007, Section25, Rule 250C: Extre Loading, 1.25 x Gust Response Factor (wind second gust)	

Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A

Wilton, CT Rev 1 ~ June 5, 2014

ANTENNA MAST ANALYSIS

Antenna Mast, appurtenances and connections to the utility tower were analyzed and designed in accordance with the NU Design Criteria Table, TIA/EIA-222-F, and AISC-ASD standards.

Load cases considered:

Load Case 1:

Wind Speed...... 85 mph (2)

Radial Ice Thickness...... 0"

Load Case 2:

Wind Pressure...... 75% of 85 mph wind pressure

Radial Ice Thickness...... 0.5"

Note 2: Per NU Mast Design Criteria Exception 1.

Results

MAST ASSEMBLY

The existing pipe mast was determined to be structurally inadequate. Replacement of the existing antenna mast with a 6 SCH. 80 Pipe x 28-ft long (O.D. = 6.625"), conforming to ASTM A53, Grade B, $F_v = 35$ ksi specifications will be required.

Member	Stress Ratio (% of capacity)	Result
6" Sch. 80 Mast	64.3%	PASS
HSS6x6x1/4 Brace	31.9%	PASS
Mast Connection to CL&P Tower	24.0% (1)	PASS

Note 1 - 1/3 increase in allowable stress not used for connection to tower per OTRM 059.

UTILITY TOWER

This analysis finds that the subject utility structure is adequate to support the antenna mast and related appurtenances. The tower stresses meet the requirements set forth by the ASCE Manual No. 10-97, "Design of Latticed Steel Transmission Structures", for the applied NESC Heavy and Hi-Wind load cases. The detailed analysis results are provided in Section 9 of this report. The analysis results are summarized as follows:

A maximum usage of 96.41% occurs in the utility tower under the NESC Extreme loading condition.

TOWER SECTION:

The utility structure was found to be within allowable limits.

Tower Member	Stress Ratio (% of capacity)	Result
Angle g25X	96.41%	PASS

FOUNDATION AND ANCHORS

The existing foundation consists of a 13-ft square x 13.5-ft long reinforced concrete pier on a 18.0-ft square x 2.0-ft thick reinforced concrete pad. The base of the tower is connected to the foundation by one (1) anchor stub angle per leg embedded into the concrete foundation. Foundation information was obtained from NUSCO drawing # 01055-60000.

Structural Analysis - 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade - CT11414A Wilton, CT

Rev 1 ~ June 5, 2014

BASE REACTIONS:

From PLS-Tower analysis of CL&P tower based on NESC/NU prescribed loads.

Load Case	Shear	Uplift	Compression
NESC Heavy Wind	8.44 kips	81.83 kips	105.40 kips
NESC Extreme Wind	17.46 kips	146.07 kips	158.71 kips

Note 1 - 10% increase to be applied to the above tower base reactions for foundation verification per OTRM 051

FOUNDATION:

The foundation was found to be within allowable limits.

Foundation	Design Limit	Allowable Limit	Proposed Loading (2)	Result
Reinforced Conc. Pad and Pier	Overturning	1.0 FS ⁽¹⁾	1.46 FS ⁽¹⁾	PASS

Note 1: FS denotes Factor of Safety

Note 2: 10% increase to PLS base reactions used in foundation analysis per OTRM 051.

Conclusions and Recommendations

This analysis shows that the subject utility tower with the replacement of the existing antenna mast as detailed in section 4 of this report is adequate to support the proposed T-Mobile equipment upgrade.

The analysis is based, in part on the information provided to this office by Northeast Utilities and T-Mobile. If the existing conditions are different than the information in this report, CENTEK engineering, Inc. must be contacted for resolution of any potential issues.

NO. 18884 O ET

Please feel free to call with any questions or comments.

Respectfully Submitted by:

Carlo F. Centore, PE

Principal ~ Structural Engineer

Prepared by:

Timothy J. Lynn, PE Structural Engineer

CENTEK Engineering, Inc. Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5, 2014

STANDARD CONDITIONS FOR FURNISHING OF PROFESSIONAL ENGINEERING SERVICES ON EXISTING STRUCTURES

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the
 antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CENTEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to CENTEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the "as new" condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222.
- All services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. CENTEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5. 2014

<u>GENERAL DESCRIPTION OF STRUCTURAL</u> ANALYSIS PROGRAM~RISA-3D

RISA-3D Structural Analysis Program is an integrated structural analysis and design software package for buildings, bridges, tower structures, etc.

Modeling Features:

- Comprehensive CAD-like graphic drawing/editing capabilities that let you draw, modify and load elements as well as snap, move, rotate, copy, mirror, scale, split, merge, mesh, delete, apply, etc.
- Versatile drawing grids (orthogonal, radial, skewed)
- Universal snaps and object snaps allow drawing without grids
- Versatile general truss generator
- Powerful graphic select/unselect tools including box, line, polygon, invert, criteria, spreadsheet selection, with locking
- Saved selections to quickly recall desired selections
- · Modification tools that modify single items or entire selections
- · Real spreadsheets with cut, paste, fill, math, sort, find, etc.
- Dynamic synchronization between spreadsheets and views so you can edit or view any data in the plotted views or in the spreadsheets
- Simultaneous view of multiple spreadsheets
- Constant in-stream error checking and data validation
- Unlimited undo/redo capability
- Generation templates for grids, disks, cylinders, cones, arcs, trusses, tanks, hydrostatic loads, etc.
- Support for all units systems & conversions at any time
- Automatic interaction with RISASection libraries
- Import DXF, RISA-2D, STAAD and ProSteel 3D files
- Export DXF, SDNF and ProSteel 3D files

Analysis Features:

- Static analysis and P-Delta effects
- Multiple simultaneous dynamic and response spectra analysis using Gupta, CQC or SRSS mode combinations
- Automatic inclusion of mass offset (5% or user defined) for dynamic analysis
- Physical member modeling that does not require members to be broken up at intermediate joints
- State of the art 3 or 4 node plate/shell elements
- High-end automatic mesh generation draw a polygon with any number of sides to create a
 mesh of well-formed quadrilateral (NOT triangular) elements.
- Accurate analysis of tapered wide flanges web, top and bottom flanges may all taper independently
- Automatic rigid diaphragm modeling
- Area loads with one-way or two-way distributions
- Multiple simultaneous moving loads with standard AASHTO loads and custom moving loads for bridges, cranes, etc.
- Torsional warping calculations for stiffness, stress and design
- Automatic Top of Member offset modeling
- Member end releases & rigid end offsets
- Joint master-slave assignments
- Joints detachable from diaphragms
- Enforced joint displacements
- 1-Way members, for tension only bracing, slipping, etc.

Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5, 2014

- 1-Way springs, for modeling soils and other effects
- Euler members that take compression up to their buckling load, then turn off.
- Stress calculations on any arbitrary shape
- Inactive members, plates, and diaphragms allows you to quickly remove parts of structures from consideration
- Story drift calculations provide relative drift and ratio to height
- Automatic self-weight calculations for members and plates
- Automatic subgrade soil spring generator

Graphics Features:

- Unlimited simultaneous model view windows
- · Extraordinary "true to scale" rendering, even when drawing
- · High-speed redraw algorithm for instant refreshing
- Dynamic scrolling stops right where you want
- Plot & print virtually everything with color coding & labeling
- · Rotate, zoom, pan, scroll and snap views
- Saved views to quickly restore frequent or desired views
- Full render or wire-frame animations of deflected model and dynamic mode shapes with frame and speed control
- Animation of moving loads with speed control
- High quality customizable graphics printing

Design Features:

- Designs concrete, hot rolled steel, cold formed steel and wood
- ACI 1999/2002, BS 8110-97, CSA A23.3-94, IS456:2000,EC 2-1992 with consistent bar sizes through adjacent spans
- Exact integration of concrete stress distributions using parabolic or rectangular stress blocks
- Concrete beam detailing (Rectangular, T and L)
- Concrete column interaction diagrams
- Steel Design Codes: AISC ASD 9th, LRFD 2nd & 3rd, HSS Specification, CAN/CSA-S16.1-1994 & 2004, BS 5950-1-2000, IS 800-1984, Euro 3-1993 including local shape databases
- AISI 1999 cold formed steel design
- NDS 1991/1997/2001 wood design, including Structural Composite Lumber, multi-ply, full sawn
- Automatic spectra generation for UBC 1997, IBC 2000/2003
- Generation of load combinations: ASCE, UBC, IBC, BOCA, SBC, ACI
- Unbraced lengths for physical members that recognize connecting elements and full lengths of members
- Automatic approximation of K factors
- Tapered wide flange design with either ASD or LRFD codes
- Optimization of member sizes for all materials and all design codes, controlled by standard or user-defined lists of available sizes and criteria such as maximum depths
- Automatic calculation of custom shape properties
- Steel Shapes: AISC, HSS, CAN, ARBED, British, Euro, Indian, Chilean
- Light Gage Shapes: AISI, SSMA, Dale / Incor, Dietrich, Marino\WARE
- Wood Shapes: Complete NDS species/grade database
- Full seamless integration with RISAFoot (Ver 2 or better) for advanced footing design and detailing
- Plate force summation tool

Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5, 2014

Results Features:

Graphic presentation of color-coded results and plotted designs

- Color contours of plate stresses and forces with quadratic smoothing, the contours may also be animated
- Spreadsheet results with sorting and filtering of: reactions, member & joint deflections, beam & plate forces/stresses, optimized sizes, code designs, concrete reinforcing, material takeoffs, frequencies and mode shapes

Standard and user-defined reports

- Graphic member detail reports with force/stress/deflection diagrams and detailed design calculations and expanded diagrams that display magnitudes at any dialed location
- Saved solutions quickly restore analysis and design results.

CENTEK Engineering, Inc. Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5, 2014

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM~PLS-TOWER

PLS-TOWER is a Microsoft Windows program for the analysis and design of steel latticed towers used in electric power lines or communication facilities. Both self-supporting and guyed towers can be modeled. The program performs design checks of structures under user specified loads. For electric power structures it can also calculate maximum allowable wind and weight spans and interaction diagrams between different ratios of allowable wind and weight spans.

Modeling Features:

- Powerful graphics module (stress usages shown in different colors)
- Graphical selection of joints and members allows graphical editing and checking
- Towers can be shown as lines, wire frames or can be rendered as 3-d polygon surfaces
- Can extract geometry and connectivity information from a DXF CAD drawing
- CAD design drawings, title blocks, drawing borders or photos can be tied to structure model
- XML based post processor interface
- Steel Detailing Neutral File (SDNF) export to link with detailing packages
- Can link directly to line design program PLS-CADD
- Automatic generation of structure files for PLS-CADD
- Databases of steel angles, rounds, bolts, guys, etc.
- Automatic generation of joints and members by symmetries and interpolations
- Automated mast generation (quickly builds model for towers that have regular repeating sections) via graphical copy/paste
- Steel angles and rounds modeled either as truss, beam or tension-only elements
- Guys are easily handled (can be modeled as exact cable elements)

Analysis Features:

- Automatic handling of tension-only members
- Automatic distribution of loads in 2-part suspension insulators (v-strings, horizontal vees, etc.)
- Automatic calculation of tower dead, ice, and wind loads as well as drag coefficients according to:
 - ASCE 74-1991
 - NESC 2002
 - NESC 2007
 - IEC 60826:2003
 - EN50341-1:2001 (CENELEC)
 - EN50341-3-9:2001 (UK NNA)
 - EN50341-3-17:2001 (Portugal NNA)
 - ESAA C(b)1-2003 (Australia)
 - TPNZ (New Zealand)
 - REE (Spain)
 - EIA/TIA 222-F
 - ANSI/TIA 222-G
 - CSA S37-01
- Automated microwave antenna loading as per EIA/TIA 222-F and ANSI/TIA 222-G
- Minimization of problems caused by unstable joints and mechanisms
- Automatic bandwidth minimization and ability to solve large problems
- Design checks according to (other standards can be added easily):
 - ASCE Standard 10-90

Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT

Rev 1 ~ June 5, 2014

- AS 3995 (Australian Standard 3995)
- BS 8100 (British Standard 8100)
- EN50341-1 (CENELEC, both empirical and analytical methods are available)
- ECCS 1985
- NGT-ECCS
- PN-90/B-03200
- EIA/TIA 222-F
- ANSI/TIA 222-G
- CSA S37-01
- EDF/RTE Resal
- IS 802 (India Standard 802)

Results Features:

- Design summaries printed for each group of members
- Easy to interpret text, spreadsheet and graphics design summaries
- Automatic determination of allowable wind and weight spans
- Automatic determination of interaction diagrams between allowable wind and weight spans
- Capability to batch run multiple tower configurations and consolidate the results
- Automated optimum angle member size selection and bolt quantity determination

Tool for interactive angle member sizing and bolt quantity determination.

CENTEK Engineering, Inc. Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5, 2014

Criteria for Design of PCS Facilities On or Extending Above Metal Electric Transmission Towers & Analysis of Transmission Towers
Supporting PCS Masts (1)

Introduction

This criteria is the result from an evaluation of the methods and loadings specified by the separate standards, which are used in designing telecommunications towers and electric transmission towers. That evaluation is detailed elsewhere, but in summary; the methods and loadings are significantly different. This criteria specifies the manner in which the appropriate standard is used to design PCS facilities including masts and brackets (hereafter referred to as "masts"), and to evaluate the electric transmission towers to support PCS masts. The intent is to achieve an equivalent level of safety and security under the extreme design conditions expected in Connecticut and Massachusetts.

ANSI Standard TIA/EIA-222 (Rev. F) covering the design of telecommunications structures specifies a working strength/allowable stress design approach. This approach applies the loads from extreme weather loading conditions, and designs the structure so that it does not exceed some defined percentage of failure strength (allowable stress).

ANSI Standard C2-2007 (National Electrical Safety Code) covering the design of electric transmission metal structures is based upon an ultimate strength/yield stress design approach. This approach applies a multiplier (overload capacity factor) to the loads possible from extreme weather loading conditions, and designs the structure so that it does not exceed its ultimate strength (yield stress).

Each standard defines the details of how loads are to be calculated differently. Most of the NU effort in "unifying" both codes was to establish what level of strength each approach would provide, and then increasing the appropriate elements of each to achieve a similar level of security under extreme weather loadings.

Two extreme weather conditions are considered. The first is an extreme wind condition (hurricane) based upon a 50-year recurrence (2% annual probability). The second is a winter condition combining wind and ice loadings.

The following sections describe the design criteria for any PCS mast extending above the top of an electric transmission tower, and the analysis criteria for evaluating the loads on the transmission tower from such a mast from the lower portions of such a mast, and loads on the pre-existing electric lower portions of such a mast, and loads on the pre-existing electric transmission tower and the conductors it supports.

Note 1: Prepared from documentation provide from Northeast Utilities.

CENTEK Engineering, Inc. Structural Analysis – 125-ft CL&P Tower # 2983 T-Mobile Antenna Upgrade – CT11414A Wilton, CT Rev 1 ~ June 5, 2014

PCS Mast

The PCS facility (mast, external cable/trays, including the initial and any planned future support platforms, antennas, etc. extending the full height above the top level of the electric transmission structure) shall be designed in accordance with the provisions of TIA/EIA-222 (Rev. F) with two exceptions:

- An 85 mph extreme wind speed shall be used for locations in all counties throughout the NU system.
- The allowable stress increase of TIA Section 3.1.1.1 is allowed for the mast section, but is disallowed for the mast to structure connection design.

The combined wind and ice condition shall consider ½" radial ice in combination with the wind load (0.75 Wi) as specified in TIA section 2.3.16.

ELECTRIC TRANSMISSION TOWER

The electric transmission tower shall be analyzed using yield stress theory in accordance with the attached table titled "NU Design Criteria". This specifies uniform loadings (different from the TIA loadings) on the each of the following components of the installed facility:

- PCS mast for its total height above ground level, including the initial and planned future support platforms, antennas, etc. above the top of an electric transmission structure.
- Conductors are related devices and hardware.
- Electric transmission structure. The loads from the PCS facility and from the electric conductors shall be applied to the structure at conductor and PCS mast attachment points, where those load transfer to the tower.

The uniform loadings and factors specified for the above components in the table are based upon the National Electrical Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to TIA and its loads and factors with the exceptions noted above. (Note that the NESC does not require the projected wind surfaces of structures and equipment to be increased by the ice covering.)

In the event that the electric transmission tower is not sufficient to support the additional loadings of the PCS mast, reinforcement will be necessary to upgrade the strength of the overstressed members.



Northeast Utilities Overhead Transmission Standards



Attachment A

NU Design Criteria

			(HdW) Basic Wind Speed	Q (PSE)	X Height Factor	Gust Factor	Load or Stress Factor	Force Coef - Shape Factor
Ē	TIA/EIA	Antenna Mount	TIA	TIA (.75WI)	TIA	TIA	TIA, Section 3.1.1.1 disallowed for connection design	TIA
Ice Condition	leavy	Tower/Pole Analysis with antennas extending above top of Tower/Pole (Yield Stress)		4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
lce	NESC Heavy	Tower/Pole Analysis with Antennas below top of Tower/Pole (on two faces)		4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
100		Conductors:			Conductor	loads provided b	yNU	
ndtion	TIA/EIA	Antenna Mount	85	TIA	TIA	TIA	TIA, Section 3.1.1.1 disallowed for connection design	TIA
High Wind Condtion	ctreme d	Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use	1.	Section 25, Rule 25 25 x Gust Respons round level based o	e Factor		1.6 Flat Surfaces 1.3 Round Surfaces
High M	NESC Extreme Wind	Tower/Pole Analysis with Antennas below top of Tower/Pole	Use		Section 25, Rule 25 ground level based			1.6 Flat Surfaces 1.3 Round Surfaces
	~	Conductors:			Conductor	loads provided b	yNU	
treme	Wind ton*	Tower/Pole Analysis with antennas extending above top of Tower/Pole		4PSF Wind Load	ion 25, Rule 250D: 1.25 x round level based o	Gust Response	Factor	1.6 Flat Surfaces 1.3 Round Surfaces
NESC Extreme	Ice with Wind Conditon*	Tower/Pole Analysis with Antennas below top of Tower/Pole		PSF Wind Load	ion 25, Rule 250D: ground level based			1.6 Flat Surfaces 1.3 Round Surfaces
~	- T	Conductors:			Conductor	loads provided by	yNU	
		* Only for Structures Installed aft						

Communication Anteni	nas on Transmission Str	uctures (CL&P	& WMECo Only)
Northeast Utilities	Design	OTRM 059	Rev.1
Approved by: KMS (NU)	NU Confidential Information	Page 7 of 9	03/17/2011



Northeast Utilities Overhead Transmission Standards



Shape Factor Criteria shall be per TIA Shape Factors.

2) STEP 2 - The electric transmission structure analysis and evaluation shall be performed in accordance with NESC requirements and shall include the mast and antenna loads determined from NESC applied loading conditions (not TIA/EIA Loads) on the structure and mount as specified below, and shall include the wireless communication mast and antenna loads per NESC criteria)

The structure shall be analyzed using yield stress theory in accordance with Attachment A, "NU Design Criteria." This specifies uniform loadings (different from the TIA loadings) on each of the following components of the installed facility:

- a) Wireless communication mast for its total height above ground level, including the initial and any planned future equipment (Support Platforms, Antennas, TMA's etc.) above the top of an electric transmission structure.
- b) Conductors and related devices and hardware (wire loads will be provided by NU).
- c) Electric Transmission Structure
 - i) The loads from the wireless communication equipment components based on NESC and NU Criteria in Attachment A, and from the electric conductors shall be applied to the structure at conductor and wireless communication mast attachment points, where those loads transfer to the tower.
 - ii) Shape Factor Multiplier:

NESC Structure Shape	Cd
Polyround (for polygonal steel poles)	1.3
Flat	1.6
Open Lattice	3.2

iii) When Coaxial Cables are mounted along side the pole structure, the shape multiplier shall be:

Mount Type	Cable Cd	Pole Cd
Coaxial Cables on outside periphery (One layer)	1.45	1.45
Coaxial Cables mounted on stand offs	1.6	1.3

d) The uniform loadings and factors specified for the above components in Attachment A, "NU Design Criteria" are based upon the National Electric Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to the TIA and its loads and factors with the exceptions noted above.

Note: The NESC does not require ice load be included in the supporting structure. (Ice on conductors and shield wire only, and NU will provide these loads).

 e) Mast reaction loads shall be evaluated for local effects on the transmission structure members at the attachment points.

Communication Antenn	as on Transmission Str	uctures (CL&P	& WMECo Only)
Northeast Utilities	Design	OTRM 059	Rev.1
Approved by: KMS (NU)	NU Confidential Information	Page 3 of 9	03/17/2011



Connecticut Light & Power

ANTENNA MAST DESIGN CL&P STRUCT. NO. 2983 T-MOBILE CT11414A PIMPEWAUG ROAD WILTON, CT 06897



SITE ADDRESS: PROJECT COORDINATES: CL&P STRUCT NO: CL&P CONTACT: T-MOBILE SITE REF.: T-MOBILE CONTACT: ANTENNA CL HEIGHT: ENGINEER OF RECORD: CENTEK CONTACT: SHEET INDEX SHI. NO. DESCRIPTION	PIMPEWAUG ROAD WILTON, CT 06897		
AP STRUC AP STR			10000 100201159
AP STRUC AP CONTA MOBILE SI MOBILE CI TENNA CL GINEER OF GINEER OF	LAT: 41"-12"-45.09N LON: 73"-25"-40.16W ELEV:±252" AMSL		HOLARDON (
MOBILE SI MOBILE CO MOBILE CO MOBILE CO CENTRA CL GINEER OF GINEER OF GINEER OF CONTRACT CONT	2983		20 20
MOBILE COMOBILE COMOB	ROBERT GRAY 860.728.6125		17. PL/
MOBILE CO TENNA CL GINEER OF MTEK CON NTEK CON	CT11414A		AVS
GINEER OF GINEER OF NTEK CON	MARK RICHARD 850.692.7143	(0	
OINEER OF	132"-0"	111111	
NTEK CON	CENTEK ENGINEERING, INC. 63-2 NORTH BRANFORD ROAD BRANFORD, CT 06405	114	
NO.	CARLO F. CENTORE, PE 203.488.0580 ext. 122		The state of the s
NO.			
	NO	REV.	
T-1 TITLE SHEET		-	2000 2000 2000 2000 2000 2000 2000 200
N-1 DESIGN BASIS	S & GENERAL NOTES	-	
N-2 STRUCTURAL	STEEL NOTES	-	4A 8883 389
MI-1 MODIFICATION	NSPECTION REQUIREMENTS	-	T-MOBIL
S-1 TOWER ELEV	ELEVATION & FEEDLINE PLAN	-	
S-2 ANTENNA MA	MAST DETAILS	-	5040 A5 S-CW1 5040 A5 S-CW1 (CB ND 16035,008
			TITLE SHEET
			Bettr 10.

ă	DESIGN BASIS	GENERAL NOTES	10. TOWER REINFORCEMENTS SHALL BF CONDUCTED BY FIFLD	Design Dr. T.A. Design Bri. T.A. CHI'D Dr. GRO
÷	GOVERNING CODE: 2003 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2005 CT STATE BUILDING CODE AND 2009 AMENOMENTS.	1. REFER TO STRUCTURAL ANALYSIS AND REINFORCEMENT DESIGN PREPARED BY CENTEK ENGINEERING, INC., FOR T-MOBILE DATED 6/5/14.	CREWS EXPERIENCED IN THE ASSEMBLY AND ERECTION OF TRANSMISSION STRUCTURES, ALL SAFETY PROCEDURES, RIGGING SAIOL METHODS SHALL BE STANDARD TO THE INDUSTRY AND IN COMPLIANCE WITH OSHAL.	
5.	TA/EIA-222-F-1996, ASCE MANUAL NO. 72 — "DESIGN OF STEEL TRANSMISSION POLE STRUCTURES SECOND EDITION", NESC C2-2007 AND NORTHEAST UTILITIES DESIGN CRITERIA.	2. TOWER GEOMETRY AND STRUCTURE MEMBER SIZES WERE OBTAINED FROM A TOWER MAPPING REPORT PREPARED BY KM CONSULTING ENGINEERS DATED JULY 13, 1998.	11. EXISTING COAXIAL CABLES AND ALL ACCESSORIES SHALL BE RELOCATED AS NECESSARY AND REINSTALLED BY THE CONTRACTOR WITHOUT INTERRUPTION IN SERVICE WHERE THEY ARE IN CONTRICT WITH THE TOWICE DENICOLOGICAL	KANALIYEEN
ri	DESIGN CRITERIA		WORK,	HOLSENS HOL CORE
	WIND LOAD. (PCS MAST) BASIC WIND SPEED (Y) =85 MPH (FASTEST MILE); BASED	SHALL BE CONDUCTED ON DAYS WITH LESS THAN 15 MPH WIND PRESENT. NO MEMBER SHALL BE LEFT DISCONNECTED FOR THE NEXT WORKING DAY.	12. IF ANY FIELD CONDITIONS EXIST WHICH PRECLUDE COMPLIANCE WITH THE DRAWINGS, THE CONTRACTOR SHALL MANEDATELY NOTIFY THE ENGINEER AND SHALL PROCEED	60 AG 0,440
	ON TA/EA-222F AND NU MAST DESIGN CRITERIA EXCEPTION 1.	4. ALL STEEL REINFORCEMENT SHOWN HEREIN APPLIES TO ALL SIDES OF THE TOWER.	WITH ATTECTED WORK ATTER CONFLICT IS SATISFACTORILY RESOLVED.	700000 SE VI +1/7 VI +1/7
	WIND LOAD: (UTILITY POLE & FOUNDATION) BASIC WIND SPEED (V) =110 MPH (3-SECOND GUST) BASED ON NESC C2-2007, SECTION 25 RULE 250C.	 ALL REPLACEMENT STEEL MEMBERS SHALL BE INSTALLED WITH A325-N BOLTS (SIZE TO MATCH EXISTING). UNLESS OTHERWISE NOTED BELOW. 	13. ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.	
		のこののでは「	14. NO DRILLING WELDING OR TAPING IS PERMITTED ON CLAP OWNED EQUIPMENT.	
		UNDERPINNING, TEMPORARY ANCHORS, GUYING, BARRICADES, ETC. AS MAY BE REQUIRED FOR THE PROTICECTION OF EXISTING PROPERTY, CONSTRUCTION WORKERS, AND FOR PUBLIC SAFETY, MAINTAIN EXISTING SITE OPERATIONS AND COORDINATE WORK WITH TOWER OWNER.		-
		7. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE GOVERNING BUILDING CODE.		CHO CHO
		B. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULLES, OR RECULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS SCOPE OF WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN		174A manage common part of the c
		9. BEFORE BEGINNING THE WORK, THE CONTRACTOR IS RESPONSIBLE FOR MAKING SUCH INVESTIGATIONS CONCERNING PHYSICAL CONDITIONS (SHIRPLET AND		CL117
		SUBSURFACE) AT OR CONTIGUOUS TO THE SITE WHICH MAY AFFECT PERFORMANCE AND COST OF THE WORK. THIS INCLUDES YERPHING ALL DIMENSIONS, ELEVATIONS, ANGLES, AND EXISTING CONDITIONS AT THE SITE, PRIOR TO FEBRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA. CONTRACTORS SHALL TAKE PIELD.		900 3/17/14 8040 48 8040 78 94 16080.00
		NECESSARY TO ASSUR		DESIGN BASIS AND GENTRAL NOTES
				N-1

THEORY STREET	8013	NAMENTAL LEG ARAM IN BUILDING SU 13161840 BUILDING SU	M WWW 1/2 / M	TON SHAPE	one on particular and a second and a second and a second	CLEP STRUCTURE 2863	A/17/14 AN SHOWN AN HARMADON ANNERS A	7-2
	A NAME OF THE OWNER OWNER OF THE OWNER OW		To.	THE SECTION AND THE SECTION AN			Definition of the state of the	Z

2. CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES APPEARANCE AND QUALITY OF WELDS, AND MELDING PROCESSES—SHALL—BE-QUALIFIED—IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES". ALL WELDING SHALL BE DONE USING THE SOFFEDUED ELECTRODES AND WELDING SHALL CONFORM TO ASIC AND D1.1 WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLET UZ 4 IN THE ASIC "MANUAL OF STEEL CONFINENCIAL" STHE EDITION. AT THE COMPLETION OF WELDING, ALL DAMAGE TO GALVANIZED COATING SHALL BE REPAIRED.

(FY = 36 KSI). STRUCTURAL HSS (RECTANGULAR SHAPES)---ASTM A500 GRADE B, (FY = 46 KSI) STRUCTURAL HSS (ROUND SHAPES)---ASTM A500

ö o.

œ

PIPE---ASTM A53 GRADE B (FY = 35 KSI)

ωĬ

FASTENER SPECIFICATIONS

r

ď

m 0 0

÷

GRADE B, (FY = 42 KSI)

STRUCTURAL STEEL (OTHER SHAPES) --- ASTM A36

STRUCTURAL STEEL (W SHAPES) --- ASTM A992

(FY = 50 KSI)

ď

THE ENGINEER SHALL BE NOTFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITING OR NO CONFORMING MATERIALS OR CONDITIONS TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW.

ij

U-BOLTS——ASTM A3077
ANCHOR RODS——ASTM F1554
WELDING ELECTRODES——ASTM ETOXX FOR A36 & STZ_GRED STEELS, ASTM EBOXX FOR A572_GRE5
STEEL

CONNECTION BOLTS---ASTM A325-N, UNLESS OTHERWISE SCHEDULED.
U-BOLTS---ASTM A307

CONNECTION ANGLES SHALL HAVE A MINIMUM THICKNESS

OF 1/4 INCHES.

4

5

CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL, DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE THE FOLLOWING: SECTION PROFILES, SIZE, CONNECTION ATTACHMENTS, REINFORCING, ANCHORAGE, SIZE AND TYPE OF FASTEMERS AND ACCESSORIES, INCLUDE ERECTION DRAWINGS, ELEVATIONS AND DETAILS.

STRUCTURAL CONNECTION BOLTS SHALL CONFORM TO ASTM A225. ALL BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL HAVE A MINIMUM OF TWO BOLTS, UNLESS OTHERWISE ON THE DRAWINGS.

LOCK WASHER ARE NOT PERMITTED FOR A325 BOLTED STEEL ASSEMBLIES.

16. 17

SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED.

COMMENCEMENT OF STRUCTURAL STEEL WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.

5

AFTER ERECTION OF STRUCTURES, TOUCHUP ALL WELDS, ABRASIONS AND NON-GALVANIZED SURFACES WITH A 95% ORGANIC ZINC RICH PAINT IN ACCORDANCE WITH ASIN 780.

ALL STEEL MATERIAL (EXPOSED TO WEATHER) SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT DIPPED GALVANIZED) COATINGS" ON IRONS AND STEEL PRODUCTS.

10.

LEVEL AND PLUMB INDIVIDUAL MEMBERS OF THE STRUCTURE TO AN ACCURACY OF 1:500, BUT NOT TO

FABRICATE BEAMS WITH MILL CAMBER UP.

6

20,

FIT AND SHOP ASSEMBLE FABRICATIONS IN THE LARGEST PRACTICAL SECTIONS FOR DELIVERY TO SITE.

PROVIDE ALL PLATES, CLIP ANGLES, CLOSURE PIECES, STRAP ANCHORS, MISCELLANEOUS PIECES AND HOLES REQUIRED TO COMPLETE THE STRUCTURE.

ó

INSTALL FABRICATIONS PLUMB AND LEVEL, ACCURATELY FITTED, AND FREE FROM DISTORTIONS OR DEFECTS.

œ 6

1

EXCEED 1/4" IN THE FULL HEIGHT OF THE COLUMN.

MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION.

60

STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST PROWISIONS OF AISC MANUAL OF STEEL CONSTRUCTION.

u)

ALL BOLTS, ANCHORS AND MISCELLANGOUS HARDWARE SHALL BE GALYANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING (HOT-DIP) ON IRON AND STEEL

HARDWARE".

ALL STRUCTURAL STEEL IS DESIGNED BY ALLOWABLE STRESS DESIGN (ASD).

STRUCTURAL STEE!

MATERIAL SPECIFICATIONS

ò

12

	PHE-CONSTROTON		DURNA CONSTRUCTION		POST-CONSTRUCTION
SCHEDULED	REPORT ITEM	SCHEDULED	REPORT ITEM	SOMEDALED	METORY ITEM
×	EOR MODIFICATION INSPECTION DRAWING	1	FOUNDATIONS	×	MODIFICATION INSPECTOR RECORD REDLINE DRAWING
×	EOR APPROVED SHOP DRAWINGS	1	EARTHWORK: BACKFILL MATERIAL & COMPACTION	1	POST-INSTALLED ANCHOR ROD PULL-OUT TEST
	EOR APPROVED POST-INSTALLED ANCHOR MPI	-	REBAR & FORMWORK GEOMETIPY VERIFICATION	×	PHOTOGRAPHS
1	FABRICATION INSPECTION	1	CONCRETE TESTING		
	FABRICATOR CERTIFIED WELDER INSPECTION	×	STEEL INSPECTION		
×	MATERIAL CERTIFICATIONS	1	POST INSTALLED ANCHOR ROD VERFICATION		
		1	BASE PLATE GROUT VERFICATION		
		1	CONTRACTOR'S CERTIFIED WELD INSPECTION		
		×	ON-SITE COLD GALVANIZING VERPICATION		
		×	CONTRACTOR AS-BULT REDLINE DRAWINGS		
NOTES:	1. RETER TO MODIFICATION INSPECTION NOTES FOR ADDITIONAL REQUIREMENTS. 2. Y'S PROVIETE DOCUMENT REQUIRED FOR INCLUSION IN MODIFICATION INSPECTION FINAL REPORT. 3. "-" DENOTES DOCUMENT REQUIRED FOR INCLUSION IN MODIFICATION INSPECTION FINAL REPORT. 4. EDR - ENGINEER FOR RECORD. 4. MRII - "MANUFACTIBETS OF RECORD.	ADDITIONAL REQUIREMENTS ON IN MODIFICATION INSPECTIONS IN MODIFICATION IN	TON FINAL REPORT. SPECTION FINAL REPORT.		

WINDS ON STREET TO WINDS THE WARRY OF STREET THE WINDS T

W1 P1/21/A 6 W1 P1/21/A 6

GENERAL

- THE MODIFICATION INSPECTION IS A VISUAL INSPECTION OF STRUCTURAL MODIFICATIONS, TO INCLUDE A REVIEW AND COMPILATION OF SPECIFIED SUBMITALS AND CONSTRUCTION INSPECTIONS, AS AN ASSURANCE OF COMPLANCE WITH THE CONSTRUCTION DOCUMENTS PREPARED UNDER THE DIRECTION OF THE ENGINEER OF RECORD (EOR)
- THE MODIFICATION INSPECTION IS TO CONFIRM INSTALLATION CONFIGURATION AND GENERAL WORKMANSHIP AND IS NOT A REVIEW OF THE MODIFICATION DESIGN, OWNERSHIP OF THE MODIFICATION DESIGN FFECTIVENESS AND INTENT RESIDES WITH THE ENGINEER OF RECORD. 'n
- TO ENSURE COMPLIANCE WITH THE MODIFICATION INSPECTION REQUIREMENTS THE GENERAL CONTRACTOR (GC) AND THE MODIFICATION INSPECTOR (M) COMMENCE COMMUNICATION UPON AUTHORIZATION TO PROCEED BY THE CLIENT, EACH PARTY SHALL BE PROMOTIVE IN CONTACTION THE OTHER. THE EOR SHALL BE CONTACTED SPECIFIC GC/MI CONTACT INFORMATION IS NOT MADE m
- THE GC SHALL PROVIDE THE MI WITH A MINIMUM OF BUSINESS DAYS NOTICE OF IMPENDING INSPECTIONS.
- WHEN POSSIBLE, THE OC AND MI SHALL BE ON SITE DURING THE MODIFICATION INSPECTION TO HAVE ANY NOTED DEFICIENCES ADDRESSED DURING THE INITIAL MODIFICATION INSPECTION. ió

MODIFICATION INSPECTOR (MI)

- 6 THE MI SHALL CONTACT THE GC UPON AUTHORIZATION THE CLIENT TO:
- REVIEW THE MODIFICATION INSPECTION REPORT
- REQUIREMENTS.

 WORK WITH THE GC IN DEVELOPMENT OF A SCHEDULE FOR ON—SITE INSPECTIONS.

 DISCUSS CRITICAL INSPECTIONS AND PROJECT CONCERNS.
- THE MI IS RESPONSIBLE FOR COLLECTION OF ALL INSPECTION AND TEST REPORTS, REVIEWING REPORTS FOR ADHERENCE TO THE CONTRACT DOCUMENTS, CONDUCTING ON-STEE INSPECTIONS AND COMPILATION & SUBMISSION OF THE MODIFICATION INSPECTION REPORT TO THE CLIENT AND THE EOR.

GENERAL CONTRACTOR (GC)

- BY THE THE GC IS REQUIRED TO CONTACT THE GC UPON AUTHORIZATION TO PROCEED WITH CONSTRUCTION CLIENT TO:
- REQUIREMENTS.

 WORK WITH THE IN DEVELOPMENT OF A SCHEDULE FOR ON-SITE INSPECTIONS.

 DISCUSS CRITICAL INSPECTIONS.
- THE GC IS RESPONSIBLE FOR COORDINATING AND SCHEDULING IN ADVANCE ALL REQUIRED INSPECTIONS AND TESTS WITH THE MI. oi.

CORRECTION OF FAILING MODIFICATION INSPECTION

- SHOULD THE STRUCTURAL MODIFICATION NOT COMPLY WITH THE REQUIREMENTS OF THE CONSTRUCTION DOCUMENTS, THE GC SHALL WORK WITH THE MODIFICATION INSPECTOR IN A VIABLE REMEDIATION PLAN AS FOLLOWS:
- GC MAY WORK WITH CORRECT ALL DEFICIENCIES TO COMPLY WITH THE CONTRACT DOCUMENTS AND COORDINATE WITH THE MI FOR A FOLLOW UP INSPECTION. WITH CLIENT AUTHORIZATION, THE GC MAY WORK WITHE EOR TO REAMALYZE THE MODIFICATION USING THE AS-BUILT CONDITION.

JEINE :

REQUIRED PHOTOGRAPHS

THE GC AND MI SHALL AT MINIMUM PHOTO DOCUMENT THE FOLLOWING FOR INCLUSION IN THE MODIFICATION INSPECTION REPORT;

CL&P STRUCTURE 2983 CT11414A

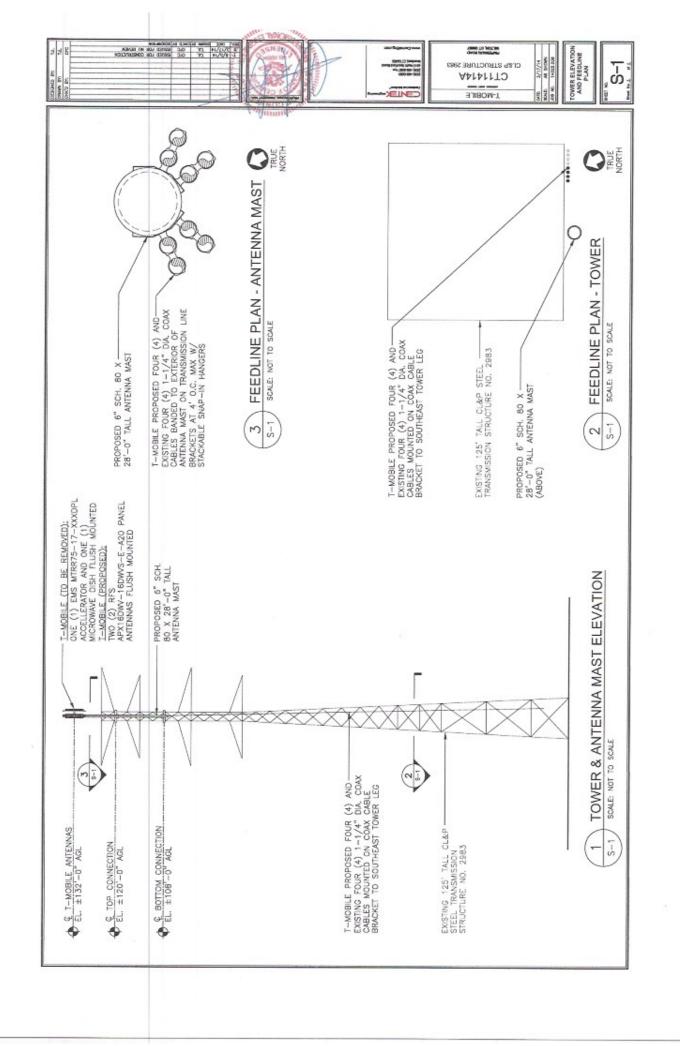
SJIBOM-T

- PRE-CONSTRUCTION: GENERAL CONDITION OF THE SITE.
 DURING CONSTRUCTION: RAW MATERIALS, CRITICAL
 DETAILS, WELD PREPARATION, BOLT INSTALLATION &
 TORQUE, FINAL INSTALED CONDITION & SURFACE
- COATING REPAIRS.

 POST-CONSTRUCTION: FINAL CONDITION OF THE SITE

<u>-</u>|

MODIFICATION INSPECTION REQUIREMENTS



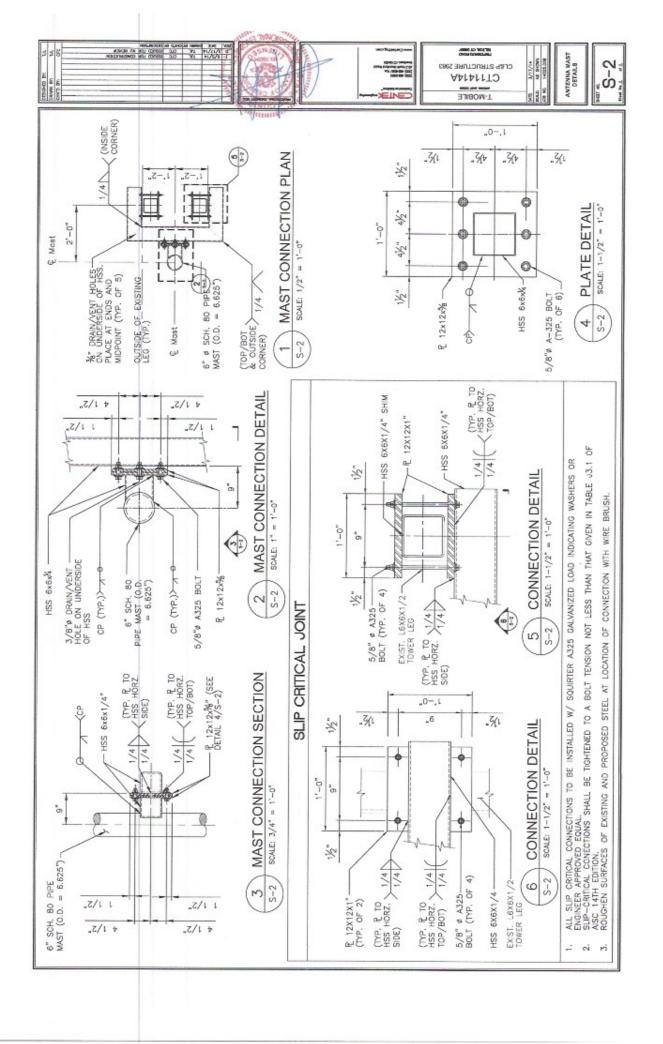
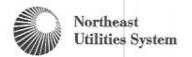


EXHIBIT C



56 Prospect Street, Hartford, CT 06103

Northeast Utilities Service Company P.O. Box 270 Hartford, CT 06141-0270 (203) 665-5000

July 29, 2014

Mr. Mark Richard T-Mobile 35 Griffin Rd. Bloomfield, CT 06002

RE: T-Mobile Antenna Site, CT-11 414A, Pimpewaug Rd., Wilton CT, structure 2983.

Dear Mr. Richard:

Based on our reviews of the site drawings, the structural analysis and foundation review provided by Centek Engineering, along with a third party review performed by Paul J Ford Systems we have reviewed for acceptance this modification.

Since there are no outstanding structural or site related issues to resolve at this time, construction at these locations may begin as soon as scheduling allows. You may contact Mr. O'Brien (860-665-6987); once the lease issues are secured you may then contact Mr. John Landry directly (860-665-5425) to begin the construction arrangements

/)/

Transmission Line Engineering

Ref: CT11414A-MDRN-CD.PDF

14025.008 - CT11414A Structural Analysis Rev1 14-06-05.pdf

EXHIBIT D



RADIO FREQUENCY EMISSIONS ANALYSIS REPORT **EVALUATION OF HUMAN EXPOSURE POTENTIAL** TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11414A

Wilton / Cannondale Pimewaug Road Wilton, CT 06897

January 8, 2015

Site Compliance	Summary
Compliance Status:	COMPLIANT
Site total MPE% of FCC general public	4.64 %
allowable limit:	4.04 %



January 8, 2015

T-Mobile USA Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, CT 06002

Emissions Analysis for Site: CT11414A - Wilton / Cannondale

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **Pimewaug Road**, **Wilton**, **CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm²). The number of μ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μW/cm²). The general population exposure limit for both the PCS and AWS bands is 1000 μW/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at Pimewaug Road, Wilton, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 2 GSM channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel
- 2) 2 UMTS channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 2 LTE channels (AWS Band 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.



- 5) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 6) The antennas used in this modeling are the RFS APX16DWV-16DWVS-E-A20 for 1900 MHz (PCS) and 2100 MHz (AWS) channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The RFS APX16DWV-16DWVS-E-A20 has a maximum gain of 16.3 dBd at its main lobe. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- The antenna mounting height centerline of the proposed antennas is 132 feet above ground level (AGL).
- 8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves. Per the Connecticut Siting Council active database there are no additional carriers listed at this facility.

All calculations were done with respect to uncontrolled / general public threshold limits.

Tel: (781) 273.2500 Fax: (781) 273.3311



T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	В
Antenna #:	1	Antenna #:	1
Make / Model:	RFS APX16DWV- 16DWVS-E-A20	Make / Model:	RFS APX16DWV- 16DWVS-E-A20
Gain:	16.3 dBd	Gain:	16.3 dBd
Height (AGL):	132	Height (AGL):	132
Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)
Channel Count	6	Channel Count	6
Total TX Power:	240	Total TX Power:	240
ERP (W):	10,237.91	ERP (W):	10,237.91
Antenna Al MPE%	2.32	Antenna B1 MPE%	2.32

Site Composite	MPE%
Carrier	MPE%
T-Mobile	4.64 %
Site Total MPE %:	4.64 %

T-Mobile Sector 1 Total:	2.32 %
T-Mobile Sector 2 Total:	2.32 %
	CONTRACTOR OF STREET
Site Total:	4.64 %



Summary

All calculations performed for this analysis yielded results that were within the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector 1:	2.32 %
Sector 2:	2.32 %
T-Mobile Total:	4.64 %
Site Total:	4.64 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is 4.64% of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

Tel: (781) 273.2500 Fax: (781) 273.3311

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan

RF Engineering Director

EBI Consulting

21 B Street

Burlington, MA 01803