



**Crown Castle**  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065

January 6, 2021

Melanie A. Bachman  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

**RE: Notice of Exempt Modification for T-Mobile:  
841301 - T-Mobile Site ID: CT11261C  
426 River Road, Willington, CT 06279  
Latitude: 41° 53' 26.72" / Longitude: -72° 17' 21.77"**

Dear Ms. Bachman:

T-Mobile currently maintains two (2) antennas at the 100-foot mount on the existing 110-foot Monopole Tower, located at 426 River Road, Willington, CT. The tower is owned by Crown Castle and the property is owned by the Willington Fire Department. T-Mobile now intends to add two (2) new antennas 1900/2100/600/700 MHz antennas. The new antennas will be installed at the 100-ft level of the tower and will be capable of providing 5G services.

**Planned Modifications:  
Tower:**

Install New:

- (2) RFS-APXVAARR24\_43-U-NA20 Antenna 1900/2100/600/700 MHz
- (2) Radio 4449 B71/B12
- (2) TMAs
- (4) 1 ¼" Coax
- (1) Hybrid

Existing to Remain:

- (2) EMS RR90-17-XXDP Antenna (Dormant)
- (2) TMAs to be relocated

**Ground:**

- Upgrade to existing ground cabinet. (Internally)
- Upgrade existing breakers.

The facility was approved by the Town of Willington Planning and Zoning Commission on August 15, 2000 by way of a Special Permit. The Special Permit was granted with conditions which this exempt modification complies with.

**The Foundation for a Wireless World.**

CrownCastle.com

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to the Honorable Erika Wiecenski, First Selectwoman for the Town of Willington, Michael D'Amato, Zoning Agent, Crown Castle as the tower owner, and the Willington Fire Department as the property owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Anne Marie Zsamba.

Sincerely,

Anne Marie Zsamba  
Site Acquisition Specialist  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065  
(201) 236-9224  
AnneMarie.Zsamba@crowncastle.com

Attachments

cc:

Erika Wiecenski, First Selectwoman (*via email only to ewiecenski@willingtonct.org*)  
Town of Willington  
40 Old Farms Road  
Willington, CT 06279  
860-487-3100

Michael D'Amato, Zoning Agent (*via email only to zoningagent@willingtonct.org*)

Melanie A. Bachman

Page 3

Town of Willington  
40 Old Farms Road  
Willington, CT 06279  
860-487-3123

Willington Fire Department, Property Owner (*via email only to [president@willingtonfire.org](mailto:president@willingtonfire.org)*)  
PO Box 161  
Willington, CT 06279

Crown Castle, Tower Owner

**From:** [Zsamba, Anne Marie](mailto:Zsamba, Anne Marie)  
**To:** [president@willingtonfire.org](mailto:president@willingtonfire.org)  
**Subject:** Notice of Exempt Modification - T-Mobile - 426 River Road, Willington - 841301  
**Date:** Wednesday, January 6, 2021 11:51:00 AM  
**Attachments:** [EM-T-MOBILE-426 RIVER ROAD WILLINGTON-841301-CT11261C-NOTICE.pdf](#)

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Dear Willington Fire Department:

Attached please find T-Mobile's exempt modification application that is being submitted to the Connecticut Siting Council today, January 6, 2021.

In light of the present circumstances with Covid-19, The Council has advised that electronic notification of this filing is acceptable. If you could kindly confirm receipt. Thank you.

Best,  
Anne Marie Zsamba

**ANNE MARIE ZSAMBA**  
Site Acquisition Specialist  
T: (201) 236-9224  
M: (518) 350-3639  
F: (724) 416-6112

**CROWN CASTLE**  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065  
[CrownCastle.com](http://CrownCastle.com)



**From:** [Zsamba, Anne Marie](#)  
**To:** ["ewieczenski@willingtonct.org"](mailto:ewieczenski@willingtonct.org)  
**Subject:** Notice of Exempt Modification - T-Mobile - 426 River Road, Willington - 841301  
**Date:** Wednesday, January 6, 2021 11:51:00 AM  
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Dear First Selectwoman Wieczenski:

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[CrownCastle.com](http://CrownCastle.com)

**From:** [Zsamba, Anne Marie](#)  
**To:** ["zoningagent@willingtonct.org"](mailto:zoningagent@willingtonct.org)  
**Subject:** Notice of Exempt Modification - T-Mobile - 426 River Road, Willington - 841301  
**Date:** Wednesday, January 6, 2021 11:51:00 AM  
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Dear Zoning Agent D'Amato:

Attached please find T-Mobile's exempt modification application that is being submitted to the Connecticut Siting Council today, January 6, 2021.

In light of the present circumstances with Covid-19, The Council has advised that electronic notification of this filing is acceptable. If you could kindly confirm receipt. Thank you.

Best,  
Anne Marie Zsamba

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Site Acquisition Specialist  
T: (201) 236-9224  
M: (518) 350-3639  
F: (724) 416-6112

**CROWN CASTLE**  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065  
[CrownCastle.com](http://CrownCastle.com)

# Exhibit A

## **Original Facility Approval**

TOWN OF WILLINGTON  
PLANNING AND ZONING COMMISSION

SPECIAL PERMIT

Date: 9/26/2000

This is to certify that the use: **Monopole Antenna Tower and Support Building for Wireless Communications Facility** located on **426 River Road** Assessors Map **34**, lot **10**, Zone **R80** has been approved **with conditions** by the Willington Planning and Zoning Commission on **8/15/2000** pursuant to **Section 13** of the Town of Willington Zoning Regulations, which findings are on file with the Commission.

Owner of Record: Willington Fire Department #1

  
Agent

Conditions:

- 1) Prior to the start of construction, any FCC and FAA approvals shall be provided to the zoning agent.
- 2) As stated at the public hearing, the applicant shall agree to comply with any technical revisions suggested by the town engineer and/or the zoning agent, and updated drawings to reflect those revisions shall be provided.
- 3) The driveway shall meet zoning regulations, as they may be waived by the zoning agent.
- 4) The elevation of the top of any antenna shall not exceed 642 feet above sea level.
- 5) The exterior lighting switch shall be arranged so any exterior lighting is not on all the time, but rather only when required by workers.
- 6) All easements shall be depicted on the final site plan
- 7) A gate shall be provided on the northeast access road at the location of the barbed wire fence.

Received for record October 25, 2000  
At Willington Stamper/Kerdie TC

Applicant should obtain a copy of the Zoning Regulations which detail specific requirements.

TOWN OF WILLINGTON  
Planning and Zoning Commission  
40 Old Farms Road, Willington, CT 06279

Application for: Special Permit  Amendment Site Plan Approval Modification

Location of property: 426 River Road, Willington, Connecticut 06279

Assessors Map #: 34 Assessors Lot #: 10 Existing Zone: R-80 Area of property: 12.6 acres

Historical District Certificate of Appropriateness is attached to this application (if applicable): N/A

Special Permit Requested: Construction of monopole and support building

Names, addresses and telephone numbers of owner/owners:

Willington Fire Department, Inc. #1 c/o Chief Tyler Millix, P.O. Box 161  
426 River Road, Willington, Connecticut 06279 (860-429-0288)

Name, address and telephone number of petitioner if other than owner:

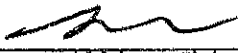
N/A

Description of existing and proposed use of land and buildings:

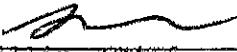
Existing use: Willington Fire Department, Inc. facility

Proposed use: Construction of a monopole and support building for a wireless communications facility and improvements to existing access driveway pursuant to Sections 7.06.04 and 7.06.04.06 of the zoning regulations.

Please submit with this application form all data and maps required in Section 13 of the Zoning Regulations. The undersigned owner(s) of the property hereby authorizes the Planning and Zoning Commission or their agent to enter and inspect premises at any reasonable hour.

Signature of owner(s):  Date: May 12, 2000  
Tyler Millix, Chief  
Willington Fire Dept., Inc. #1 Date: \_\_\_\_\_

I (we) the undersigned petitioner(s) understand that the submission of inaccurate or incomplete information shall be grounds for denial of this application by the P.Z.C.

Signature(s):  Date: May 12, 2000  
Tyler Millix, Chief  
Willington Fire Dept., Inc. #1 Date: \_\_\_\_\_

\*List of property owners within five hundred (500) feet of subject property attached hereto as Exhibit A.  
A.SPECIAL

# Exhibit B

## Property Card

# 426 RIVER RD

**Location** 426 RIVER RD

**Mblu** 34 / / 010-00 / /

**Acct#** 00242700

**Owner** WILLINGTON FIRE DEPT INC

**Assessment** \$383,350

**Appraisal** \$547,630

**PID** 4891

**Building Count** 1

## Current Value

Appraisal			
Valuation Year	Improvements	Land	Total
2018	\$453,400	\$94,230	\$547,630

Assessment			
Valuation Year	Improvements	Land	Total
2018	\$317,390	\$65,960	\$383,350

## Owner of Record

**Owner** WILLINGTON FIRE DEPT INC  
**Co-Owner**  
**Address** P O BOX 161  
WILLINGTON, CT 06279

**Sale Price** \$0  
**Certificate** 1  
**Book & Page** 80/355  
**Sale Date** 06/25/1980

## Building Information

### Building 1 : Section 1

**Year Built:** 1985  
**Living Area:** 4,266  
**Replacement Cost:** \$661,759  
**Building Percent Good:** 65  
**Replacement Cost  
Less Depreciation:** \$430,140

Building Attributes	
Field	Description
STYLE	Fire Station
MODEL	Industrial
Grade	A
Stories:	1
Occupancy	1.00

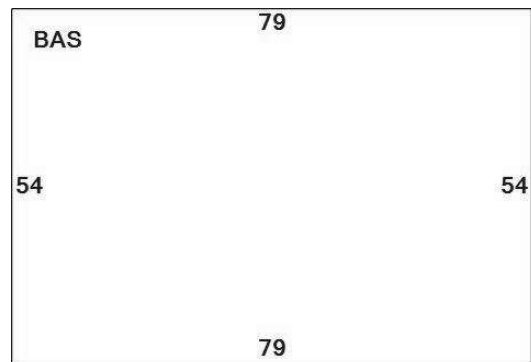
Exterior Wall 1	Typical
Exterior Wall 2	
Roof Structure	Typical
Roof Cover	Typical
Interior Wall 1	Typical
Interior Wall 2	
Interior Floor 1	Typical
Interior Floor 2	
Heating Fuel	Typical
Heating Type	Floor Furnace
AC Type	Unit/AC
Bldg Use	MUN FIRE
Total Rooms	
Total Bedrms	
Total Baths	
1st Floor Use:	
Heat/AC	None
Frame Type	Fireprf Steel
Baths/Plumbing	Average
Ceiling/Wall	-DESCRIPTION-
Rooms/Prtns	Average
Wall Height	14.00
% Comn Wall	

### Building Photo



(<http://images.vgsi.com/photos/WilmingtonCTPhotos//00\00\18\23.jpg>)

### Building Layout



(ParcelSketch.ashx?pid=4891&bid=4891)

Building Sub-Areas (sq ft)			Legend
Code	Description	Gross Area	Living Area
BAS	First Floor	4,266	4,266
		4,266	4,266

### Extra Features

Extra Features	Legend
No Data for Extra Features	

### Land

#### Land Use

Use Code	9032
Description	MUN FIRE
Zone	R80
Neighborhood	301

#### Land Line Valuation

Size (Acres)	13.16
Frontage	
Depth	
Assessed Value	\$65,960



**Outbuildings**

Outbuildings						Legend
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
LT1	LIGHTS-IN W/PL			1.00 UNITS	\$480	1
LT5	MERC VAP/FLU			1.00 UNITS	\$770	1
PAV1	PAVING-ASPHALT			15000.00 S.F.	\$21,000	1
SHD1	SHED FRAME			168.00 S.F.	\$1,010	1

**Valuation History**

Appraisal			
Valuation Year	Improvements	Land	Total
2019	\$453,400	\$94,230	\$547,630

Assessment			
Valuation Year	Improvements	Land	Total
2019	\$317,390	\$65,960	\$383,350

# Exhibit C

## **Construction Drawings**

## SCOPE OF WORK

### ITEMS TO BE INSTALLED ON & REMOVED FROM EXISTING TOWER:

- INSTALL T-MOBILE ANTENNA (APXVAARR24\_43-U-NA20) (TYP. OF 1 PER SECTOR, TOTAL OF 2).
- INSTALL T-MOBILE RADIO (4449 B71+B12) (TYP. OF 1 PER SECTOR, TOTAL OF 2).
- INSTALL T-MOBILE TMAS (AWS) (TYP. OF 1 PER SECTOR, TOTAL OF 2).
- INSTALL T-MOBILE COAX CABLES (TYP. OF 2 PER SECTOR, TOTAL OF 4).
- INSTALL T-MOBILE COAX JUMPER CABLES (TYP. OF 4 PER SECTOR, TOTAL OF 8).
- INSTALL T-MOBILE FIBER JUMPER CABLES (TYP. OF 1 PER SECTOR, TOTAL OF 2).
- INSTALL T-MOBILE 6x12 HCS HYBRID CABLE (TOTAL OF 1).

### ITEMS TO BE INSTALLED ON EXISTING EQUIPMENT PAD:

- REMOVE (1) DUS41
- INSTALL (2) ERICSSON BASEBAND 6630 UNITS
- INSTALL (6) RUS01 B4
- RELOCATE T-MOBILE TMAS (PCS) (TYP. OF 1 PER SECTOR, TOTAL OF 2).

### ITEMS TO REMAIN:

- (2) ANTENNAS, (2) TMAS, (4) COAX CABLES, (6) RUS01 B2

SITE ADDRESS: 426 RIVER ROAD  
WILLINGTON, CT 06279

LATITUDE (NAD 83): N 41° 53' 26.72"

LONGITUDE (NAD 83): W 72° 17' 21.77"

COUNTY: TOLLAND

JURISDICTION: -

LANDLORD: CROWN CASTLE INTERNATIONAL  
500 W. CUMMINGS PARK, STE 3600  
WOBURN, MA 01801

STRUCTURE TYPE: MONOPOLE

STRUCTURE HEIGHT: 110'

RAD CENTER: 100'

CURRENT USE: TELECOMMUNICATIONS FACILITY

PROPOSED USE: TELECOMMUNICATIONS FACILITY



## L600 PROJECT

# SITE NUMBER: CT11261C

SITE NAME: TOLLAND/I-84/FILL-IN

CROWN SITE NAME: WILLINGTON-RIVER RD

BU#: 841301

## T-MOBILE RAN TEMPLATE: CUSTOM

### NOTE:

ALL CONSTRUCTION ACTIVITIES ARE TO BE COMPLETED DIRECTLY THROUGH CROWN. CONTRACTOR MUST HAVE CONSTRUCTION PO AND NTP FROM CROWN DIRECT IN ORDER TO BEGIN. PRE-APPROVAL TO ENTER THE PROPERTY MUST BE OBTAINED. FOR ACCESS AUTHORIZATION, PLEASE CONTACT CROWN.



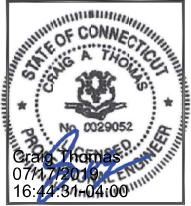
T-MOBILE NORTHEAST LLC  
103 MONARCH DRIVE  
LIVERPOOL, NY 13088



3 CORPORATE PARK DRIVE  
SUITE 101  
CLIFTON PARK, NY 12085



120 ST. JAMES AVENUE, 5TH FLOOR  
BOSTON, MA 02116



PROJECT NO: ERCC0004

DRAWN BY: AJM

CHECKED BY: CAT

SUBMITTALS	
NO.	ISSUED FOR PERMITTING
0	07/17/19

THIS DOCUMENT IS THE CREATION DESIGN, PROPERTY AND COPYRIGHTED WORK OF T-MOBILE. ANY REPLICATION OR USE WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED. REPLICATION AND USE BY GOVERNMENT AGENCIES FOR THE PURPOSES OF CONDUCTING THEIR LAWFULLY AUTHORIZED REGULATORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.

TOLLAND-I-84/FILL-IN  
CT11261C  
WILLINGTON-RIVER RD  
841301  
426 RIVER ROAD  
WILLINGTON, CT 06279

TITLE SHEET

# T-1

## DRAWING INDEX

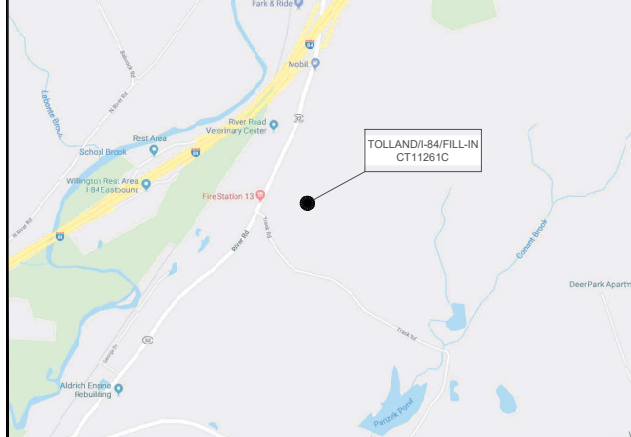
SHEET NO.	SHEET TITLE
T-1	TITLE SHEET
GN-1	GENERAL NOTES
C-1	SITE PLAN
S-1	PROPOSED TOWER ELEVATION & ANTENNA LAYOUT PLAN
S-2	EQUIPMENT DETAILS
RF-1	ANTENNA INFORMATION CHART
E-1	ONE LINE DIAGRAM
G-1	GROUNDING RISER DIAGRAM

**CROWN CASTLE SITE ID #: 841301**  
**CROWN CASTLE SITE NAME: WILLINGTON-RIVER RD**

## ENGINEERING

2018 CONNECTICUT STATE BUILDING CODE  
2018 AMENDMENT WITH 2015 INTERNATIONAL BUILDING CODE  
2009 ICC/ANSI A117.1 ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES  
2015 INTERNATIONAL MECHANICAL CODE  
2015 INTERNATIONAL ENERGY CONSERVATION CODE  
2017 NATIONAL ELECTRICAL CODE (NFPA 70 2017)  
ANSI/TIA-222-G

## VICINITY MAP



WILLINGTON RT 84 EAST TAKE EXIT 70 TURN RIGHT GO 5 MILE TO FIRE STATION ON LEFT TURN INTO THE RIGHT SIDE OF THE PARKING LOT TRASK RD AND FOLLOW, ACCESS RD ON LEFT 41.53°19.2480", -072.17°14.0820"

## GENERAL NOTES

1. THE FACILITY IS AN UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ACCESSED BY TRAINED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES NOT REQUIRE ANY WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.
2. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE T-MOBILE REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
3. HANDICAP REQUIREMENTS ARE NOT REQUIRED.
4. THIS FACILITY SHALL MEET OR EXCEED ALL FAA AND FCC REGULATORY REQUIREMENTS.
5. ALL NEW MATERIAL SHALL BE FURNISHED AND INSTALLED BY CONTRACTOR UNLESS NOTED OTHERWISE. EQUIPMENT, ANTENNAS/RADIOS AND CABLES FURNISHED BY OWNER AND INSTALLED BY CONTRACTOR.
6. NO COMMERCIAL SIGNAGE IS PROPOSED.



CALL CONNECTICUT ONE CALL  
(800) 922-4455  
CALL 3 WORKING DAYS  
BEFORE YOU DIG!



### CROWN CASTLE USA INC. SITE ACTIVITY REQUIREMENTS:

- NOTICE TO PROCEED-NO WORK SHALL COMMENCE PRIOR TO CROWN CASTLE USA INC. WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN CASTLE USA INC. NOC AT 800-788-7011 & THE CROWN CASTLE USA INC. CONSTRUCTION MANAGER.
- LOOK UP - CROWN CASTLE USA INC. SAFETY PLAN:** THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTION/HEAT, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/LOCK/TIE ANY INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR CROWN CASTLE USA INC. POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.
- BEFORE THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED.** THIS INCLUDES, BUT IS NOT LIMITED TO: BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ON-SITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.
- ALL CONSTRUCTION MEANS AND METHODS, INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSASSE A10.48 (LATEST EDITION), FEDERAL, STATE, AND LOCAL REGULATIONS, AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSIASSE A10.48 (LATEST EDITION) AND CROWN CASTLE USA INC. STANDARD CED-STD-10233, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSIF174-322 (LATEST EDITION).**
- ALL SITE WORK TO COMPLY WITH GAS-STD-1068 "INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON CROWN CASTLE USA INC. TOWER SITES" AND THE LATEST VERSION OF ANSIF174-1019-2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS."**
- IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY CROWN CASTLE USA INC., PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.**
- ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. THE CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.**
- THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.**
- THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE START OF CONSTRUCTION.**
- ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES; CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW; THIS WILL INCLUDE, BUT NOT BE LIMITED TO: A FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFETY PROCEDURES.**
- ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND PROJECT SPECIFICATIONS. LATEST APPROVED REVISION.**
- CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.**
- ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF CONTRACTOR, TOWER OWNER, CROWN CASTLE USA INC., AND/OR LOCAL UTILITIES.**
- THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.**
- THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER'S EQUIPMENT AND TOWER AREA.**
- THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.**
- THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.**
- CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.**
- THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTORS EXPENSE TO THE SATISFACTION OF OWNER.**
- CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNERS DESIGNATED LOCATION.**
- CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.**
- NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.**

### GROUNDING NOTES:

- ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GE'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- THE CONTRACTOR SHALL PERFORM IEEE 816 FALL-OFF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.
- METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO ITS EQUIPMENT.
- EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR SITS, #2 BARE SOLID TINNED COPPER FOR OUTDOOR SITS.
- CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
- ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER OR LESS UNLESS OTHERWISE INDICATED.
- ALL ALUMINUM CONDUIT AND/OR CONDUIT SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED.
- EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- ALL GROUND CONNECTIONS ABOVE GROUND (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.
- COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.
- ICE BRIDGE BONDING CONNECTIONS ON BOLTED BRIDGES AND THE TOWER GROUND BAR.
- APPROVED ANTI-OXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
- MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- BOND ALL METALLIC OBJECTS WITHIN # 8 OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.
- GROUND CONDUIT OR CONDUIT WITH FULLY APPLICABLE ANCHORAL PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS, WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS. NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED, WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e. NON-METALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUIT SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.
- GROUNDING CONNECTIONS AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE INSTALLED TO THE GROUND RING TO THE GROUND RING. A NON-METALLIC TALL "FLARED" 24" BELOW GRADE TO WITHIN 3" TO 6" OF CA-WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK (ADD TRANSFORMING GROUND STANDARD DETAIL AS WELL).
- BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO BELOW GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 20 COPPER, ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLLARS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY).

### GENERAL NOTES:

- FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:  
CARRIER: GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION  
CARRIER: TOWER OWNER: CROWN CASTLE USA INC.
- THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
- THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO: BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS, WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.
- SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
- PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CROWN CASTLE.
- ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURERS' RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND CROWN CASTLE PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- THE CONTRACTOR IS TO PERFORM A SITE INVESTIGATION AND IS TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.
- THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTORS EXPENSE TO THE SATISFACTION OF CROWN CASTLE USA INC.
- CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNERS DESIGNATED LOCATION.
- CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.

### ELECTRICAL INSTALLATION NOTES:

- ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
- CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT OBSCURED. FIRE AND HAZARDS ARE ELIMINATED.
- WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.
- ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.  
4.1. ALL ECPN CIRCUITS SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.  
4.2. ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE RATED SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 20,000 AC MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE OF THE GOVERNING JURISDICTION.
- EACH END OF EVERY POWER FEED CONDUIT, GROUNDING CONDUIT, AND TELCO CONDUIT OR CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.
- ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATINGS AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT IDS).
- PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
- ALL THE WIRAPS SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.
- ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER) WITH TYPE THW, THW, THW-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THW, THW, THW-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS OTHERWISE SPECIFIED.
- POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE THW, THW, THW-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS & BETTS (OR EQUAL), LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75° C (160° C IF AVAILABLE).
- ALL POWER AND GROUND TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSIEE AND NEC.
- ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (MC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.
- ELECTRICAL METALLIC TUBING (EMT) OR METAL-CLAD CABLE (MCC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE GRADE PVC CONDUIT.
- LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
- CONDUIT AND TUBING FITTINGS SHALL BE THREE-DR OR COMPRESSION-TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
- CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSIEE AND THE NEC.
- WIREWAYS SHALL BE METAL WITH AN EMAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS (WIREMOLD SPECIMATE WIREWAY).
- SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PAN/DTU TYPE E OR EQUAL).
- CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER, PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND COLOR UNLESS. ALL CONDUIT SHALL BE FISHER TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE TEMPORARILY CAPPED FLUSH TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE OR GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE.
- EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3R (OR BETTER) FOR EXTERIOR LOCATIONS.
- METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED EPOXY-COATED OR NON-CORRODING, SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR CROWN CASTLE USA INC. BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
- THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON ALL BREAKER CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.
- ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE FULL CORD INSTALLED.

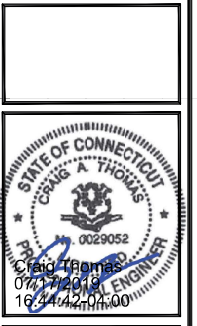
CONDUCTOR COLOR CODE		
SYSTEM	CONDUCTOR	COLOR
120/240V, 1Ø	A PHASE	BLACK
	B PHASE	RED
	NEUTRAL	WHITE
	GROUND	GREEN
	A PHASE	BLACK
120/208V, 3Ø	B PHASE	RED
	C PHASE	BLUE
	NEUTRAL	WHITE
	GROUND	GREEN
	A PHASE	BROWN
277/480V, 3Ø	B PHASE	ORANGE OR PURPLE
	C PHASE	YELLOW
	NEUTRAL	GREY
	GROUND	GREEN
	POS (+)	RED*
NEG (-)	BLACK**	

\* SEE NEC 210.5(C) (1) AND (2)  
\*\* POLARITY MARKED AT TERMINATION

**T-Mobile**  
T-MOBILE NORTHEAST LLC  
103 MONARCH DRIVE  
LIVERPOOL, NY 13088

**CROWN CASTLE**  
3 CORPORATE PARK DRIVE  
SUITE 101  
CLIFTON PARK, NY 12065

**JACOBS**  
JACOBS ENGINEERING GROUP, INC.  
120 ST. JAMES AVENUE, 5TH FLOOR  
BOSTON, MA 02116



PROJECT NO:	ERC0004
DRAWN BY:	AJM
CHECKED BY:	CAT

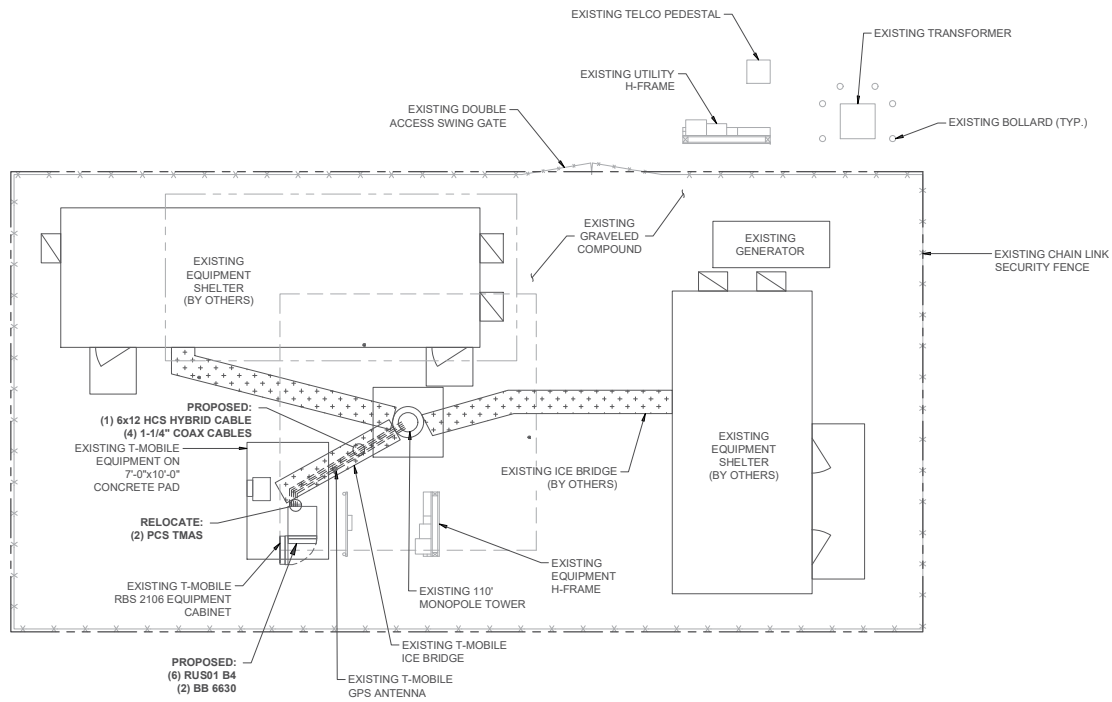
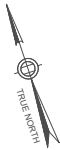
SUBMITTALS	
0	07/17/19 ISSUED FOR PERMITTING

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**TOLLANDI & HILL INC.**  
CT11261C  
WILLINGTON RIVER RD  
841301  
428 RIVER ROAD  
WILLINGTON, CT 06279

**GENERAL NOTES**

# GN-1



**NOTES:**

1. PLAN BASED ON AUTOCAD DRAWINGS ISSUED BY CROWN CASTLE ON 05/03/2017. CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS AND LOCATION/ORIENTATION OF EXISTING EQUIPMENT.



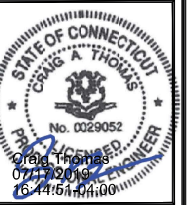
T-MOBILE NORTHEAST LLC  
103 MONARCH DRIVE  
LIVERPOOL, NY 13088



3 CORPORATE PARK DRIVE  
SUITE 101  
CLIFTON PARK, NY 12085



120 ST. JAMES AVENUE, 5TH FLOOR  
BOSTON, MA 02116



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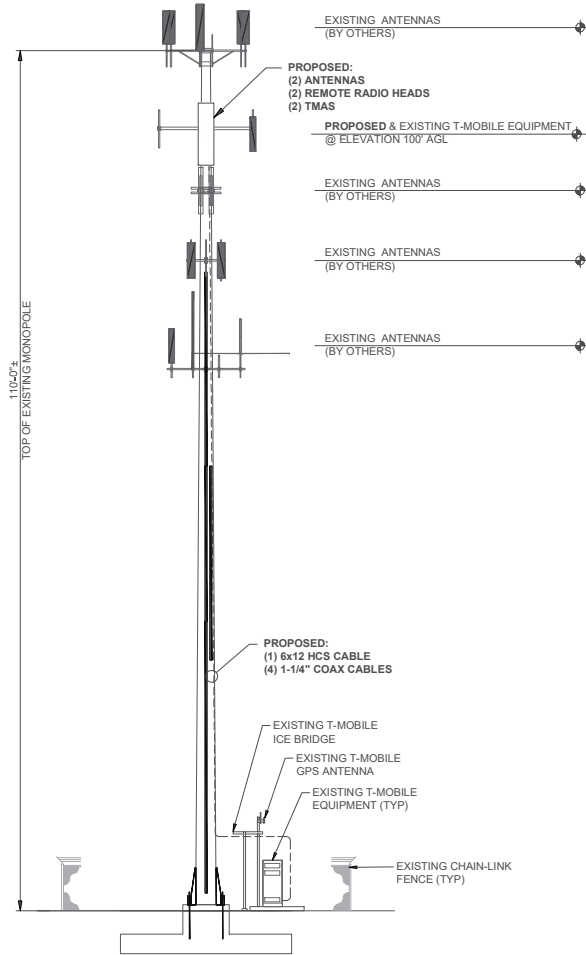
TOLLAND/IN-84/FILL-IN  
CT11261C  
WILLINGTON-RIVER RD  
841301  
428 RIVER ROAD  
WILLINGTON, CT 06279

SITE PLAN

C-1

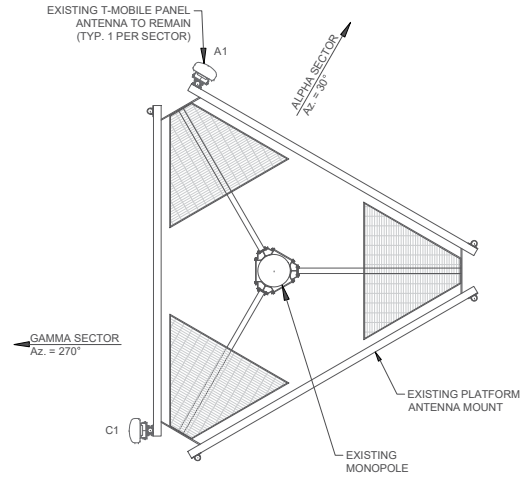
**NOTES:**

- CONTRACTOR SHALL REFER TO THE STRUCTURAL ANALYSIS REPORT; SITE NUMBER: CT11261C; SITE NAME: TOLLANDI-84/FILL-IN; CROWN BU NUMBER: 841301; CROWN SITE NAME: WILLINGTON-RIVER RD; CROWN ORDER NUMBER: 479824; ISSUED BY BLACK & VEATCH CORP., DATED ON 06/19/19. PER THIS ANALYSIS NO MODIFICATIONS ARE REQUIRED. THE CONTRACTOR SHALL VERIFY ALL EXISTING MEMBERS AND HARDWARE ARE INSTALLED PROPERLY AS DESCRIBED IN THIS REPORT.



**1** TOWER ELEVATION

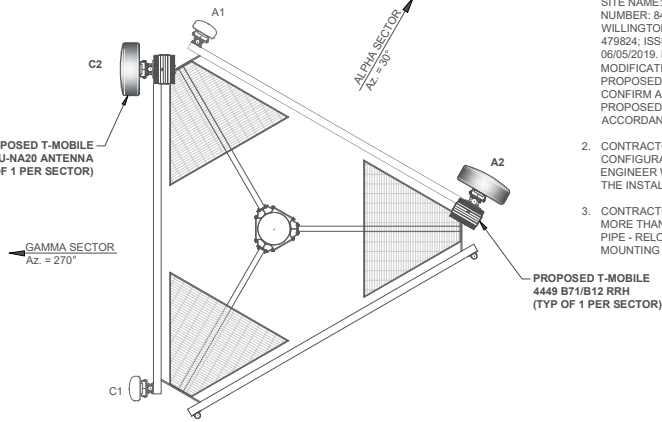
SCALE: 1/8" = 1'-0"



ELEVATION 100'

**2** EXISTING ANTENNA LAYOUT

SCALE: N.T.S.



ELEVATION 100'

**3** PROPOSED ANTENNA LAYOUT

SCALE: N.T.S.

**NOTES:**

- CONTRACTOR SHALL REFER TO THE MOUNT ANALYSIS REPORT; SITE NUMBER: CT11261C; SITE NAME: TOLLANDI-84/FILL-IN; CROWN BU NUMBER: 841301; CROWN SITE NAME: WILLINGTON-RIVER RD; CROWN ORDER NUMBER: 479824; ISSUED BY B + T GROUP, DATED ON 06/05/2019. PER THIS ANALYSIS NO MODIFICATIONS ARE REQUIRED FOR THE PROPOSED EQUIPMENT. CONTRACTOR SHALL CONFIRM ALL T-MOBILE EXISTING AND PROPOSED EQUIPMENT ARE INSTALLED IN ACCORDANCE WITH THIS REPORT.
- CONTRACTOR TO VERIFY FINAL RF CONFIGURATION AND NOTIFY CARRIER AND ENGINEER W/ ANY DISCREPANCIES PRIOR TO THE INSTALLATION.
- CONTRACTOR SHALL NOT EXCEED MOUNTING MORE THAN (2) RRHS PER ANTENNA MOUNTING PIPE - RELOCATE TO AN ADJACENT ANTENNA MOUNTING PIPE AS NEEDED.



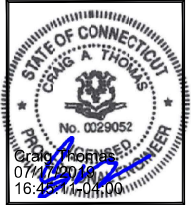
T-MOBILE NORTHEAST LLC  
103 MONARCH DRIVE  
LIVERPOOL, NY 13088



3 CORPORATE PARK DRIVE  
SUITE 101  
CLIFTON PARK, NY 12085



120 ST. JAMES AVENUE, 5TH FLOOR  
BOSTON, MA 02116



PROJECT NO:	ERC0004
DRAWN BY:	AJM
CHECKED BY:	CAT

SUBMITTALS	

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TOLLANDI-84/FILL-IN  
CT11261C  
WILLINGTON-RIVER RD  
841301  
428 RIVER ROAD  
WILLINGTON, CT 06279

PROPOSED TOWER  
ELEVATION &  
ANTENNA LAYOUT  
PLAN

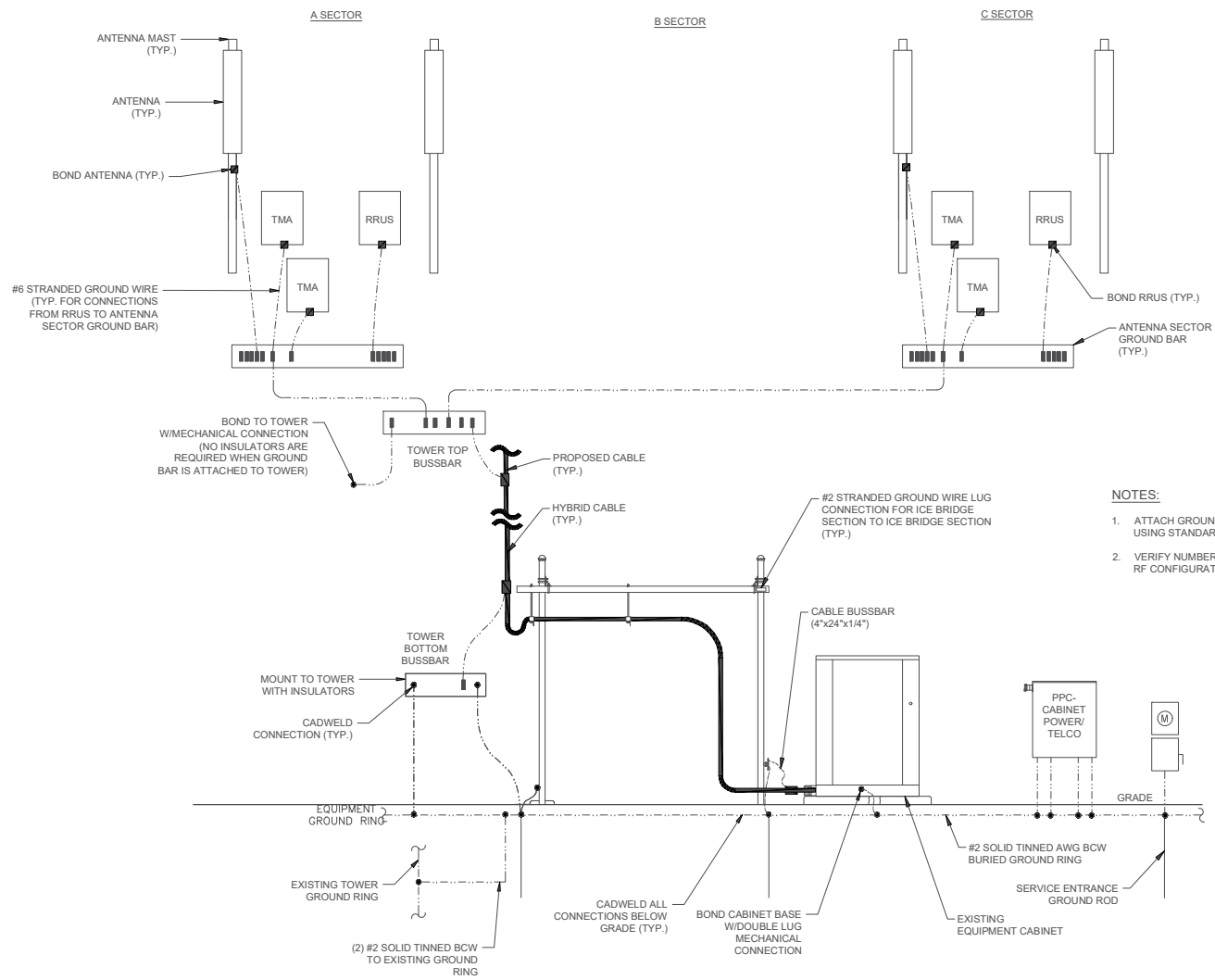
**S-1**











**NOTES:**

1. ATTACH GROUND BAR DIRECTLY TO THE TOWER USING STANDARD ADAPTER.
2. VERIFY NUMBER OF CABLES/TMAS PER T-MOBILE RF CONFIGURATION.

**GROUNDING NOTES:**

1. BELOW GROUND ALL GROUNDING CONDUCTORS TO BE #2 AWG SOLID TINNED BARE COPPER WIRE (BCW) U.O.N.
2. ABOVE GROUND ALL GROUNDING CONDUCTORS TO BE #2 AWG STRANDED INSULATED COPPER WIRE U.O.N.
3. PROVIDE BONDING AND GROUNDING CONDUCTORS WITH GREEN TYPE THWN INSULATION, U.O.N.
4. LEAVE 4' EXCESS GROUND WIRE COILED UP ABOVE GRADE. SEAL/WEATHERPROOF CONDUIT.



T-MOBILE NORTHEAST LLC  
103 MONARCH DRIVE  
LIVERPOOL, NY 13088



3 CORPORATE PARK DRIVE  
SUITE 101  
CLIFTON PARK, NY 12085



120 ST. JAMES AVENUE, 5TH FLOOR  
BOSTON, MA 02116



PROJECT NO: ERCC004

DRAWN BY: AJM

CHECKED BY: CAT

SUBMITTALS	

0 07/17/19 ISSUED FOR PERMITTING

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TOLLAND/84/FILL-IN  
CT1261C  
WILLINGTON-RIVER RD  
841301  
428 RIVER ROAD  
WILLINGTON, CT 06279

GROUNDING RISER  
DIAGRAM

G-1

# Exhibit D

## **Structural Analysis Report**

Date: **June 19, 2019**

Denice Nicholson  
Crown Castle  
3 Corporate Dr  
Clifton Park, NY 12065



Black & Veatch Corp.  
6800 W. 115th St., Suite 2292  
Overland Park, KS 66211  
(913) 458-6909

**Subject:** **Structural Analysis Report**

**Carrier Designation:** **T-Mobile Co-Locate**  
**Carrier Site Number:** CT11261C  
**Carrier Site Name:** Tolland/I-84/Fill-In

**Crown Castle Designation:** **Crown Castle BU Number:** 841301  
**Crown Castle Site Name:** WILLINGTON-RIVER RD  
**Crown Castle JDE Job Number:** 559267  
**Crown Castle Work Order Number:** 1749500  
**Crown Castle Order Number:** 479824 Rev. 0

**Engineering Firm Designation:** **Black & Veatch Corp. Project Number:** 400087

**Site Data:** **426 River Road, Willington, Tolland County, CT**  
**Latitude 41° 53' 26.72", Longitude -72° 17' 21.77"**  
**110 Foot - Monopole Tower**

Dear Denice Nicholson,

Black & Veatch Corp. is pleased to submit this “**Structural Analysis Report**” to determine the structural integrity of the above mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Proposed Equipment Configuration

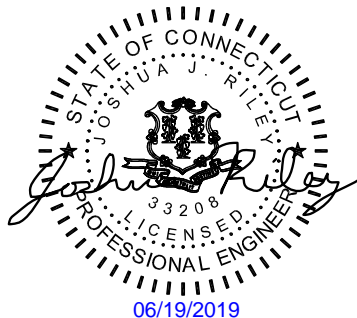
**Sufficient Capacity - 80%**

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Purich Sangpairoj / Akarapan Tongjunta

Respectfully submitted by:

Joshua J. Riley, P.E.  
Professional Engineer



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## 1) INTRODUCTION

This tower is a 110 ft Monopole tower mapped by ADP Structural & Welding.

The tower has been modified multiple times in the past to accommodate additional loading.

The tower has been modified per reinforcement drawings prepared by GPD Group, Inc., in June of 2012. Reinforcement consists of Reinforcement consists of addition of reinforcement plates at elevation 45.5' to 65.5' and addition of base plate stiffeners at elevation 0'. Refer to Legacy Modification Inspection Report by FDH Velocitel, Inc., in August of 2015. This modification has been considered effective in this analysis.

The tower has been modified per reinforcement drawings prepared by Aero Solutions, LLC. in January of 2015. Reinforcement consists of addition of reinforcement plates at elevation 1.5' to 86.5', addition of transition stiffeners at elevation 0' and (4) additional anchor rods with brackets. Refer to Modification Inspection Report by FDH Velocitel, Inc., in August of 2015. This modification has been considered effective in this analysis.

## 2) ANALYSIS CRITERIA

<b>TIA-222 Revision:</b>	TIA-222-H
<b>Risk Category:</b>	II
<b>Wind Speed:</b>	125 mph
<b>Exposure Category:</b>	B
<b>Topographic Factor:</b>	1
<b>Ice Thickness:</b>	2 in
<b>Wind Speed with Ice:</b>	50 mph
<b>Service Wind Speed:</b>	60 mph

**Table 1 - Proposed Equipment Configuration**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
100.0	100.0	1	cci tower mounts	Platform Mount [10.75' LP 712-1]	9	1-1/4
		2	ems wireless	RR90-17-00DP w/ Mount Pipe		
		2	ericsson	KRY 112 144/1		
		2	ericsson	KRY 112 489/2		
		2	ericsson	RADIO 4449 B12/B71		
		2	rfs celwave	APXVAARR24_43-U-NA20_TIA w/ Mount Pipe		

**Table 2 - Other Considered Equipment**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
110.0	113.0	3	ericsson	RRUS 11	2 4 12 1	3/8 3/4 7/8 conduit
		3	ericsson	RRUS 32		
		3	ericsson	RRUS 32 B2		
		3	kmw communications	AM-X-CD-16-65-00T-RET_TIA w/ Mount Pipe		
		6	powerwave technologies	7020.00		
		3	powerwave technologies	P65-15-XLH-RR w/ Mount Pipe		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
110.0	113.0	6	powerwave technologies	TT19-08BP111-001	2	1-5/8
		3	quintel technology	QS66512-2 w/ Mount Pipe		
		2	raycap	DC6-48-60-18-8F		
	110.0	1	cci tower mounts	Platform Mount [10.75' LP 712-1]		
		2	cci tower mounts	T-Arm Mount [TA 702-3]		
83.0	83.0	2	alcatel lucent	B13 RRH4X30-4R	2	1-5/8
		2	alcatel lucent	B66A RRH4X45		
		2	alcatel lucent	RRH2X60-PCS		
		4	andrew	SBNHH-1D65A_TIA w/ Mount Pipe		
		2	cci tower mounts	Side Arm Mount [SO 101-1]		
		2	cci tower mounts	Side Arm Mount [SO 104-1]		
69.0	74.0	1	decibel	DB810M-XC	3	1/2
	72.0	1	dapa	48212S w/ Mount Pipe		
	71.0	1	decibel	DB201-F		
	69.0	1	-	12' HSS 3x3x1/4		
		1	cci tower mounts	Side Arm Mount [SO 201-1]		

### 3) ANALYSIS PROCEDURE

**Table 3 - Documents Provided**

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Wilkinson Engineering	4710168	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Wilkinson Engineering (Mapped)	4710170	CCISITES
4-TOWER MANUFACTURER DRAWINGS	ADP Structural & Welding (Mapped)	5113552	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	GPD Group, Inc.	4945191	CCISITES
4-POST-MODIFICATION INSPECTION	FDH Velocitel, Inc.	5864402	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	Aero Solutions, LLC.	5537030	CCISITES
4-POST-MODIFICATION INSPECTION	FDH Velocitel, Inc.	5822398	CCISITES

#### 3.1) Analysis Method

tnxTower (version 8.0.5.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

tnxTower was used to determine the loads on the modified structure. Additional calculations were performed to determine the stresses in the pole and in the reinforcing elements. These calculations are presented in Appendix C.

### 3.2) Assumptions

- 1) Tower and structures were built and maintained in accordance with the manufacturer's specifications.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 3) The wind loading Exposure Category and Topographic Category for this site have been analyzed and determined by the tower owner. Black & Veatch does not assume any responsibility for its accuracy.
- 4) This analysis was performed under the assumption that all information provided to Black & Veatch is current and correct. This is to include site data, appurtenance loading, tower/foundation details, and geotechnical data. The loading on the structure is based on CAD level drawings and carrier orders provided by the owner. If any of this information is not current and correct, this report should be considered obsolete and further analysis will be required.

This analysis may be affected if any assumptions are not valid or have been made in error. Black & Veatch Corp. should be notified to determine the effect on the structural integrity of the tower.

## 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary) (Monopole Tower)**

Elevation (ft)	Component Type	Size	Critical Element	% Capacity	Pass / Fail
110 - 105	Pole	TP13.901x13.051x0.1875	Pole	22.6%	Pass
105 - 100	Pole	TP14.751x13.901x0.1875	Pole	35.0%	Pass
100 - 95	Pole	TP15.941x14.751x0.1875	Pole	52.0%	Pass
95 - 90	Pole	TP16.114x15.226x0.25	Pole	51.7%	Pass
90 - 85	Pole	TP17.002x16.114x0.25	Pole	59.9%	Pass
85 - 80	Pole	TP17.89x17.002x0.25	Pole	67.9%	Pass
80 - 79.75	Pole + Reinf.	TP17.934x17.89x0.5625	Reinf. 6 Tension Rupture	57.5%	Pass
79.75 - 74.75	Pole + Reinf.	TP18.822x17.934x0.5375	Reinf. 6 Tension Rupture	65.3%	Pass
74.75 - 69.75	Pole + Reinf.	TP19.71x18.822x0.525	Reinf. 6 Tension Rupture	72.3%	Pass
69.75 - 64.75	Pole + Reinf.	TP20.598x19.71x0.5	Reinf. 6 Tension Rupture	79.3%	Pass
64.75 - 64.25	Pole + Reinf.	TP20.686x20.598x0.5	Reinf. 6 Tension Rupture	80.0%	Pass
64.25 - 64	Pole + Reinf.	TP20.731x20.686x0.775	Reinf. 1 Tension Rupture	61.6%	Pass
64 - 59	Pole + Reinf.	TP21.619x20.731x0.75	Reinf. 1 Tension Rupture	66.8%	Pass
59 - 56.5	Pole + Reinf.	TP22.063x21.619x0.725	Reinf. 1 Tension Rupture	69.2%	Pass
56.5 - 56.25	Pole + Reinf.	TP22.107x22.063x0.975	Reinf. 1 Tension Rupture	54.3%	Pass
56.25 - 51.25	Pole + Reinf.	TP22.995x22.107x0.925	Reinf. 1 Tension Rupture	58.2%	Pass
51.25 - 49.5	Pole + Reinf.	TP24.016x22.995x0.9125	Reinf. 1 Tension Rupture	59.4%	Pass
49.5 - 44.5	Pole + Reinf.	TP23.709x22.806x0.725	Reinf. 4 Tension Rupture	61.9%	Pass
44.5 - 39.5	Pole + Reinf.	TP24.613x23.709x0.7125	Reinf. 4 Tension Rupture	64.6%	Pass
39.5 - 37.25	Pole + Reinf.	TP25.019x24.613x0.7	Reinf. 4 Tension Rupture	65.8%	Pass



37.25 - 37	Pole + Reinf.	TP25.065x25.019x0.7	Reinf. 5 Tension Rupture	64.4%	Pass
37 - 36.5	Pole + Reinf.	TP25.155x25.065x0.7	Reinf. 5 Tension Rupture	64.7%	Pass
36.5 - 36.25	Pole + Reinf.	TP25.2x25.155x0.9125	Reinf. 2 Tension Rupture	56.6%	Pass
36.25 - 34.25	Pole + Reinf.	TP25.562x25.2x0.8875	Reinf. 2 Tension Rupture	57.5%	Pass
34.25 - 34	Pole + Reinf.	TP25.607x25.562x0.725	Reinf. 3 Tension Rupture	61.0%	Pass
34 - 29	Pole + Reinf.	TP26.51x25.607x0.7	Reinf. 3 Tension Rupture	63.2%	Pass
29 - 24	Pole + Reinf.	TP27.414x26.51x0.6875	Reinf. 3 Tension Rupture	65.3%	Pass
24 - 19	Pole + Reinf.	TP28.317x27.414x0.675	Reinf. 3 Tension Rupture	67.3%	Pass
19 - 14	Pole + Reinf.	TP29.221x28.317x0.6625	Reinf. 3 Tension Rupture	69.1%	Pass
14 - 9	Pole + Reinf.	TP30.125x29.221x0.6375	Reinf. 3 Tension Rupture	70.7%	Pass
9 - 4	Pole + Reinf.	TP31.028x30.125x0.6375	Reinf. 3 Tension Rupture	72.3%	Pass
4 - 1.17	Pole + Reinf.	TP31.54x31.028x0.625	Reinf. 3 Tension Rupture	73.1%	Pass
1.17 - 0.92	Pole + Reinf.	TP31.585x31.54x0.9375	Reinf. 7 Tension Yield	54.8%	Pass
0.92 - 0	Pole + Reinf.	TP31.751x31.585x0.9375	Reinf. 7 Tension Yield	55.1%	Pass
				Summary	
			Pole	67.9%	Pass
			Reinforcement	80.0%	Pass
			Overall	80.0%	Pass

**Table 5 - Tower Component Stresses vs. Capacity (Monopole Tower) - LC7**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods (Original)	0	49.5	Pass
	Anchor Rods (Existing MODs )		45.3	Pass
1	Base Plate	0	54.9	Pass
	Stiffeners		43.4	Pass
	Pole Punching Shear		6.0	Pass
1	Base Foundation	0	44.3	Pass
	Base Foundation Soil Interaction		44.5	Pass

<b>Structure Rating (max from all components) =</b>	<b>80%</b>
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Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity. Rating per TIA-222-H Section 15.5.

#### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

**APPENDIX A**  
**TNXTOWER OUTPUT**



## Tower Input Data

The tower is a monopole.  
 This tower is designed using the TIA-222-H standard.  
 The following design criteria apply:

- 1) Tower is located in Tolland County, Connecticut.
- 2) Tower base elevation above sea level: 42.00 ft.
- 3) Basic wind speed of 125 mph.
- 4) Risk Category II.
- 5) Exposure Category B.
- 6) Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- 7) Topographic Category: 1.
- 8) Crest Height: 0.00 ft.
- 9) Nominal ice thickness of 2.0000 in.
- 10) Ice thickness is considered to increase with height.
- 11) Ice density of 56 pcf.
- 12) A wind speed of 50 mph is used in combination with ice.
- 13) Temperature drop of 50 °F.
- 14) Deflections calculated using a wind speed of 60 mph.
- 15) TIA-222-H Annex S.
- 16) A non-linear (P-delta) analysis was used.
- 17) Pressures are calculated at each section.
- 18) Stress ratio used in pole design is 1.05.
- 19) Tower analysis based on target reliabilities in accordance with Annex S.
- 20) Load Modification Factors used:  $K_{es}(F_w) = 0.95$ ,  $K_{es}(t_i) = 0.85$ .
- 21) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification Use Code Stress Ratios ✓ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile  Include Bolts In Member Capacity  Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric	Distribute Leg Loads As Uniform Assume Legs Pinned ✓ Assume Rigid Index Plate ✓ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension ✓ Bypass Mast Stability Checks ✓ Use Azimuth Dish Coefficients ✓ Project Wind Area of Appurt.  Autocalc Torque Arm Areas  Add IBC .6D+W Combination Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs	Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation ✓ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption  <div style="text-align: center; background-color: #e0e0e0; padding: 2px;"><b>Poles</b></div> Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known
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## Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft		in	in	in	in	

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	110.00-105.00	5.00	0.00	18	13.0510	13.9010	0.1875	0.7500	A572-65 (65 ksi)
L2	105.00-100.00	5.00	0.00	18	13.9010	14.7510	0.1875	0.7500	A572-65 (65 ksi)
L3	100.00-93.00	7.00	2.00	18	14.7510	15.9410	0.1875	0.7500	A572-65 (65 ksi)
L4	93.00-90.00	5.00	0.00	18	15.2260	16.1139	0.2500	1.0000	A572-65 (65 ksi)
L5	90.00-85.00	5.00	0.00	18	16.1139	17.0018	0.2500	1.0000	A572-65 (65 ksi)
L6	85.00-80.00	5.00	0.00	18	17.0018	17.8896	0.2500	1.0000	A572-65 (65 ksi)
L7	80.00-79.75	0.25	0.00	18	17.8896	17.9340	0.5625	2.2500	A572-65 (65 ksi)
L8	79.75-74.75	5.00	0.00	18	17.9340	18.8219	0.5375	2.1500	A572-65 (65 ksi)
L9	74.75-69.75	5.00	0.00	18	18.8219	19.7098	0.5250	2.1000	A572-65 (65 ksi)
L10	69.75-64.75	5.00	0.00	18	19.7098	20.5977	0.5000	2.0000	A572-65 (65 ksi)
L11	64.75-64.25	0.50	0.00	18	20.5977	20.6865	0.5000	2.0000	A572-65 (65 ksi)
L12	64.25-64.00	0.25	0.00	18	20.6865	20.7308	0.7750	3.1000	A572-65 (65 ksi)
L13	64.00-59.00	5.00	0.00	18	20.7308	21.6187	0.7500	3.0000	A572-65 (65 ksi)
L14	59.00-56.50	2.50	0.00	18	21.6187	22.0627	0.7250	2.9000	A572-65 (65 ksi)
L15	56.50-56.25	0.25	0.00	18	22.0627	22.1071	0.9750	3.9000	A572-65 (65 ksi)
L16	56.25-51.25	5.00	0.00	18	22.1071	22.9949	0.9250	3.7000	A572-65 (65 ksi)
L17	51.25-45.50	5.75	4.00	18	22.9949	24.0160	0.9125	3.6500	A572-65 (65 ksi)
L18	45.50-44.50	5.00	0.00	18	22.8057	23.7093	0.7250	2.9000	A572-65 (65 ksi)
L19	44.50-39.50	5.00	0.00	18	23.7093	24.6128	0.7125	2.8500	A572-65 (65 ksi)
L20	39.50-37.25	2.25	0.00	18	24.6128	25.0194	0.7000	2.8000	A572-65 (65 ksi)
L21	37.25-37.00	0.25	0.00	18	25.0194	25.0646	0.7000	2.8000	A572-65 (65 ksi)
L22	37.00-36.50	0.50	0.00	18	25.0646	25.1550	0.7000	2.8000	A572-65 (65 ksi)
L23	36.50-36.25	0.25	0.00	18	25.1550	25.2001	0.9125	3.6500	A572-65 (65 ksi)
L24	36.25-34.25	2.00	0.00	18	25.2001	25.5616	0.8875	3.5500	A572-65 (65 ksi)
L25	34.25-34.00	0.25	0.00	18	25.5616	25.6068	0.7250	2.9000	A572-65 (65 ksi)
L26	34.00-29.00	5.00	0.00	18	25.6068	26.5103	0.7000	2.8000	A572-65 (65 ksi)
L27	29.00-24.00	5.00	0.00	18	26.5103	27.4139	0.6875	2.7500	A572-65 (65 ksi)
L28	24.00-19.00	5.00	0.00	18	27.4139	28.3174	0.6750	2.7000	A572-65 (65 ksi)
L29	19.00-14.00	5.00	0.00	18	28.3174	29.2210	0.6625	2.6500	A572-65 (65 ksi)
L30	14.00-9.00	5.00	0.00	18	29.2210	30.1246	0.6375	2.5500	A572-65 (65 ksi)
L31	9.00-4.00	5.00	0.00	18	30.1246	31.0281	0.6375	2.5500	A572-65 (65 ksi)
L32	4.00-1.17	2.83	0.00	18	31.0281	31.5396	0.6250	2.5000	A572-65 (65 ksi)
L33	1.17-0.92	0.25	0.00	18	31.5396	31.5847	0.9375	3.7500	A572-65 (65 ksi)
L34	0.92-0.00	0.92		18	31.5847	31.7510	0.9375	3.7500	A572-65 (65 ksi)

### Tapered Pole Properties

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	It/Q in <sup>2</sup>	w in	w/t
L1	13.2234	7.6554	160.0380	4.5665	6.6299	24.1388	320.2866	3.8284	1.9670	10.491
	14.0865	8.1612	193.9057	4.8683	7.0617	27.4588	388.0665	4.0814	2.1166	11.288
L2	14.0865	8.1612	193.9057	4.8683	7.0617	27.4588	388.0665	4.0814	2.1166	11.288
	14.9496	8.6671	232.2432	5.1700	7.4935	30.9926	464.7919	4.3344	2.2662	12.086
L3	14.9496	8.6671	232.2432	5.1700	7.4935	30.9926	464.7919	4.3344	2.2662	12.086
	16.1580	9.3753	293.9523	5.5925	8.0980	36.2992	588.2913	4.6885	2.4756	13.203
L4	15.7829	11.8835	336.7223	5.3165	7.7348	43.5334	673.8877	5.9429	2.2398	8.959
	16.3239	12.5880	400.2327	5.6317	8.1859	48.8932	800.9920	6.2952	2.3960	9.584
L5	16.3239	12.5880	400.2327	5.6317	8.1859	48.8932	800.9920	6.2952	2.3960	9.584
	17.2255	13.2925	471.2654	5.9469	8.6369	54.5642	943.1509	6.6475	2.5523	10.209
L6	17.2255	13.2925	471.2654	5.9469	8.6369	54.5642	943.1509	6.6475	2.5523	10.209
	18.1270	13.9971	550.2414	6.2621	9.0879	60.5464	1101.2068	6.9999	2.7086	10.834
L7	18.0788	30.9354	1173.4032	6.1511	9.0879	129.1166	2348.3503	15.4707	2.1586	3.837
	18.1239	31.0147	1182.4454	6.1669	9.1105	129.7895	2366.4468	15.5103	2.1664	3.851
L8	18.1278	29.6789	1134.7775	6.1758	9.1105	124.5573	2271.0483	14.8423	2.2104	4.112
	19.0293	31.1937	1317.5455	6.4910	9.5615	137.7965	2636.8247	15.5998	2.3667	4.403
L9	19.0313	30.4891	1289.5461	6.4954	9.5615	134.8682	2580.7889	15.2474	2.3887	4.55
	19.9329	31.9686	1486.5335	6.8106	10.0126	148.4667	2975.0229	15.9873	2.5449	4.847
L10	19.9367	30.4859	1421.2880	6.8195	10.0126	141.9503	2844.4462	15.2459	2.5889	5.178
	20.8383	31.8950	1627.6136	7.1347	10.4636	155.5498	3257.3689	15.9505	2.7452	5.49
L11	20.8383	31.8950	1627.6136	7.1347	10.4636	155.5498	3257.3689	15.9505	2.7452	5.49
	20.9284	32.0359	1649.2806	7.1662	10.5087	156.9440	3300.7314	16.0210	2.7608	5.522
L12	20.8860	48.9792	2453.3249	7.0686	10.5087	233.4561	4909.8779	24.4943	2.2768	2.938
	20.9311	49.0884	2469.7707	7.0843	10.5313	234.5178	4942.7912	24.5489	2.2846	2.948
L13	20.9350	47.5644	2399.0947	7.0932	10.5313	227.8068	4801.3461	23.7867	2.3286	3.105
	21.8365	49.6780	2733.3391	7.4084	10.9823	248.8855	5470.2747	24.8437	2.4849	3.313
L14	21.8404	48.0796	2651.7350	7.4173	10.9823	241.4551	5306.9593	24.0444	2.5289	3.488
	22.2912	49.1012	2824.3803	7.5749	11.2078	252.0005	5652.4771	24.5553	2.6070	3.596
L15	22.2526	65.2589	3666.3557	7.4861	11.2078	327.1243	7337.5356	32.6357	2.1670	2.223
	22.2977	65.3963	3689.5598	7.5019	11.2304	328.5336	7383.9744	32.7044	2.1748	2.231
L16	22.3054	62.1895	3525.2567	7.5196	11.2304	313.9034	7055.1521	31.1006	2.2628	2.446
	23.2070	64.7962	3987.3972	7.8348	11.6814	341.3450	7980.0412	32.4043	2.4191	2.615
L17	23.2089	63.9568	3940.2009	7.8393	11.6814	337.3047	7885.5864	31.9845	2.4411	2.675
	24.2457	66.9141	4512.4310	8.2017	12.2001	369.8675	9030.7996	33.4634	2.6208	2.872
L18	23.7797	50.8110	3129.8297	7.8386	11.5853	270.1554	6263.7778	25.4103	2.7378	3.776
	23.9632	52.8902	3529.9953	8.1594	12.0443	293.0842	7064.6355	26.4501	2.8968	3.996
L19	23.9651	52.0066	3474.7965	8.1639	12.0443	288.5012	6954.1651	26.0082	2.9188	4.097
	24.8826	54.0500	3900.6850	8.4846	12.5033	311.9720	7806.5026	27.0301	3.0779	4.32
L20	24.8845	53.1295	3838.2679	8.4891	12.5033	306.9800	7681.5864	26.5698	3.0999	4.428
	25.2974	54.0329	4037.4095	8.6334	12.7099	317.6593	8080.1316	27.0216	3.1714	4.531
L21	25.2974	54.0329	4037.4095	8.6334	12.7099	317.6593	8080.1316	27.0216	3.1714	4.531
	25.3433	54.1333	4059.9521	8.6494	12.7328	318.8572	8125.2464	27.0718	3.1794	4.542
L22	25.3433	54.1333	4059.9521	8.6494	12.7328	318.8572	8125.2464	27.0718	3.1794	4.542
	25.4350	54.3340	4105.2888	8.6815	12.7787	321.2597	8215.9793	27.1722	3.1953	4.565
L23	25.4023	70.2129	5213.2402	8.6061	12.7787	407.9625	10433.340	35.1131	2.8213	3.092
	25.4481	70.3437	5242.4413	8.6221	12.8017	409.5121	10491.780	35.1785	2.8292	3.101
L24	25.4520	68.4869	5114.5740	8.6310	12.8017	399.5238	10235.877	34.2500	2.8732	3.237
	25.8190	69.5050	5346.0777	8.7593	12.9853	411.7030	10699.190	34.7591	2.9368	3.309
L25	25.8441	57.1527	4454.0736	8.8170	12.9853	343.0095	8914.0080	28.5818	3.2228	4.445
	25.8899	57.2566	4478.4239	8.8330	13.0082	344.2762	8962.7406	28.6338	3.2308	4.456
L26	25.8938	55.3378	4337.0422	8.8419	13.0082	333.4076	8679.7912	27.6742	3.2748	4.678
	26.8113	57.3454	4826.3902	9.1627	13.4672	358.3800	9659.1310	28.6781	3.4338	4.905
L27	26.8132	56.3486	4747.0951	9.1671	13.4672	352.4920	9500.4365	28.1797	3.4558	5.027
	27.7307	58.3203	5263.0517	9.4879	13.9263	377.9230	10533.028	29.1657	3.6148	5.258
L28	27.7327	57.2867	5174.6136	9.4923	13.9263	371.5726	10356.036	28.6488	3.6368	5.388
	28.6502	59.2226	5717.1251	9.8131	14.3853	397.4293	11441.773	29.6169	3.7959	5.624

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	It/Q in <sup>2</sup>	w in	w/t
L29	28.6521	58.1521	5618.8681	9.8175	14.3853	390.5989	11245.129	29.0816	3.8179	5.763
	29.5696	60.0521	6187.8121	10.1383	14.8443	416.8484	12383.766	30.0318	3.9769	6.003
L30	29.5735	57.8366	5969.9606	10.1471	14.8443	402.1726	11947.776	28.9238	4.0209	6.307
	30.4910	59.6649	6554.2033	10.4679	15.3033	428.2873	13117.030	29.8381	4.1799	6.557
L31	30.4910	59.6649	6554.2033	10.4679	15.3033	428.2873	13117.030	29.8381	4.1799	6.557
	31.4085	61.4932	7175.3716	10.7887	15.7623	455.2237	14360.184	30.7524	4.3390	6.806
L32	31.4104	60.3122	7043.3620	10.7931	15.7623	446.8487	14095.991	30.1619	4.3610	6.978
	31.9297	61.3268	7404.8088	10.9747	16.0221	462.1622	14819.361	30.6692	4.4510	7.122
L33	31.8815	91.0603	10773.774	10.8637	16.0221	672.4321	21561.725	45.5388	3.9010	4.161
	31.9274	91.1947	10821.561	10.8798	16.0450	674.4486	21657.362	45.6060	3.9089	4.17
L34	31.9274	91.1947	10821.561	10.8798	16.0450	674.4486	21657.362	45.6060	3.9089	4.17
	32.0962	91.6894	10998.633	10.9388	16.1295	681.8952	22011.739	45.8534	3.9382	4.201

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>r</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in					in	in	in
L1 110.00-105.00				1	1	1			
L2 105.00-100.00				1	1	1			
L3 100.00-93.00				1	1	1			
L4 93.00-90.00				1	1	1			
L5 90.00-85.00				1	1	1			
L6 85.00-80.00				1	1	1			
L7 80.00-79.75				1	1	0.887733			
L8 79.75-74.75				1	1	0.905226			
L9 74.75-69.75				1	1	0.905321			
L10 69.75-64.75				1	1	0.929499			
L11 64.75-64.25				1	1	0.92761			
L12 64.25-64.00				1	1	0.873476			
L13 64.00-59.00				1	1	0.877291			
L14 59.00-56.50				1	1	0.894771			
L15 56.50-56.25				1	1	0.838655			
L16 56.25-51.25				1	1	0.857295			
L17 51.25-45.50				1	1	0.860295			
L18 45.50-44.50				1	1	0.899647			
L19 44.50-39.50				1	1	0.896925			
L20 39.50-				1	1	0.904672			

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_r$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
ft	ft <sup>2</sup>	in							
L21 37.25-37.00				1	1	1.05392			
L22 37.00-36.50				1	1	1.05168			
L23 36.50-36.25				1	1	0.928468			
L24 36.25-34.25				1	1	0.94483			
L25 34.25-34.00				1	1	1.00582			
L26 34.00-29.00				1	1	1.01989			
L27 29.00-24.00				1	1	1.01821			
L28 24.00-19.00				1	1	1.01783			
L29 19.00-14.00				1	1	1.01869			
L30 14.00-9.00				1	1	1.04033			
L31 9.00-4.00				1	1	1.02397			
L32 4.00-1.17				1	1	1.03502			
L33 1.17-0.92				1	1	0.737647			
L34 0.92-0.00				1	1	0.735465			

**Feed Line/Linear Appurtenances - Entered As Round Or Flat**

Description	Sector	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter in	Perimeter in	Weight plf
Safety Line 3/8	A	No	Surface Ar (CaAa)	110.00 - 0.00	1	1	0.400 0.410	0.3750		0.22
***										
PL1.25x3.5-12	A	No	Surface Af (CaAa)	65.50 - 45.50	1	1	0.000 0.000	3.5000	9.5000	0.00
PL1.25x3.5-12	B	No	Surface Af (CaAa)	65.50 - 45.50	1	1	0.000 0.000	3.5000	9.5000	0.00
PL1.25x3.5-12	C	No	Surface Af (CaAa)	65.50 - 45.50	1	1	0.000 0.000	3.5000	9.5000	0.00
***										
CCI-SFP-06512535	A	No	Surface Af (CaAa)	36.50 - 1.50	1	1	0.000 0.000	6.5000	15.5000	0.00
CCI-SFP-06512535	B	No	Surface Af (CaAa)	36.50 - 1.50	1	1	0.000 0.000	6.5000	15.5000	0.00
CCI-SFP-06512535	B	No	Surface Af (CaAa)	36.50 - 1.50	1	1	0.000 0.000	6.5000	15.5000	0.00
CCI-SFP-06512535	C	No	Surface Af (CaAa)	36.50 - 1.50	1	1	0.000 0.000	6.5000	15.5000	0.00
CCI-SFP-06512525	A	No	Surface Af (CaAa)	56.50 - 31.50	1	1	0.000 0.000	6.5000	15.5000	0.00
CCI-SFP-06512520	B	No	Surface Af (CaAa)	56.50 - 36.50	1	1	0.000 0.000	6.5000	15.5000	0.00
CCI-SFP-06512520	C	No	Surface Af (CaAa)	56.50 - 36.50	1	1	0.000 0.000	6.5000	15.5000	0.00
CCI-SFP-04510025	A	No	Surface Af (CaAa)	81.50 - 56.50	1	1	0.000 0.000	4.5000	11.0000	0.00
CCI-SFP-04510025	B	No	Surface Af (CaAa)	81.50 - 56.50	1	1	0.000 0.000	4.5000	11.0000	0.00
CCI-SFP-04510025	C	No	Surface Af (CaAa)	81.50 - 56.50	1	1	0.000 0.000	4.5000	11.0000	0.00
***										



### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		$C_A A_A$ ft <sup>2</sup> /ft	Weight plf
***									
FB-L98-002-XXX(3/8)	C	No	No	Inside Pole	110.00 - 0.00	1	No Ice	0.00	0.06
							1/2" Ice	0.00	0.06
							1" Ice	0.00	0.06
							2" Ice	0.00	0.06
WR-VG86ST-BRD(3/4)	C	No	No	Inside Pole	110.00 - 0.00	2	No Ice	0.00	0.58
							1/2" Ice	0.00	0.58
							1" Ice	0.00	0.58
							2" Ice	0.00	0.58
LDF5-50A(7/8)	C	No	No	Inside Pole	110.00 - 0.00	12	No Ice	0.00	0.33
							1/2" Ice	0.00	0.33
							1" Ice	0.00	0.33
							2" Ice	0.00	0.33
FB-L98-002-XXX(3/8)	C	No	No	Inside Pole	110.00 - 0.00	1	No Ice	0.00	0.06
							1/2" Ice	0.00	0.06
							1" Ice	0.00	0.06
							2" Ice	0.00	0.06
WR-VG86ST-BRD(3/4)	C	No	No	Inside Pole	110.00 - 0.00	2	No Ice	0.00	0.58
							1/2" Ice	0.00	0.58
							1" Ice	0.00	0.58
							2" Ice	0.00	0.58
2" innerduct conduit	C	No	No	Inside Pole	110.00 - 0.00	1	No Ice	0.00	0.20
							1/2" Ice	0.00	0.20
							1" Ice	0.00	0.20
							2" Ice	0.00	0.20
***									
HB114-U6S12-XXX-LI(1-1/4)	C	No	No	Inside Pole	100.00 - 0.00	1	No Ice	0.00	1.70
							1/2" Ice	0.00	1.70
							1" Ice	0.00	1.70
							2" Ice	0.00	1.70
LDF6-50A(1-1/4)	C	No	No	Inside Pole	100.00 - 0.00	8	No Ice	0.00	0.60
							1/2" Ice	0.00	0.60
							1" Ice	0.00	0.60
							2" Ice	0.00	0.60
***									
HB158-1-08U8-S8J18(1-5/8)	B	No	No	Inside Pole	83.00 - 0.00	2	No Ice	0.00	1.30
							1/2" Ice	0.00	1.30
							1" Ice	0.00	1.30
							2" Ice	0.00	1.30
***									
LDF4-50A(1/2)	A	No	No	Inside Pole	69.00 - 0.00	3	No Ice	0.00	0.15
							1/2" Ice	0.00	0.15
							1" Ice	0.00	0.15
							2" Ice	0.00	0.15
***									

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_A A_A$ In Face ft <sup>2</sup>	$C_A A_A$ Out Face ft <sup>2</sup>	Weight K
L1	110.00-105.00	A	0.000	0.000	0.188	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.03
L2	105.00-100.00	A	0.000	0.000	0.188	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.03

Tower Sectio n	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
L3	100.00-93.00	A	0.000	0.000	0.263	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.09
L4	93.00-90.00	A	0.000	0.000	0.112	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.04
L5	90.00-85.00	A	0.000	0.000	0.188	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.07
L6	85.00-80.00	A	0.000	0.000	1.313	0.000	0.00
		B	0.000	0.000	1.125	0.000	0.01
		C	0.000	0.000	1.125	0.000	0.07
L7	80.00-79.75	A	0.000	0.000	0.197	0.000	0.00
		B	0.000	0.000	0.188	0.000	0.00
		C	0.000	0.000	0.188	0.000	0.00
L8	79.75-74.75	A	0.000	0.000	3.938	0.000	0.00
		B	0.000	0.000	3.750	0.000	0.01
		C	0.000	0.000	3.750	0.000	0.07
L9	74.75-69.75	A	0.000	0.000	3.938	0.000	0.00
		B	0.000	0.000	3.750	0.000	0.01
		C	0.000	0.000	3.750	0.000	0.07
L10	69.75-64.75	A	0.000	0.000	4.375	0.000	0.00
		B	0.000	0.000	4.188	0.000	0.01
		C	0.000	0.000	4.188	0.000	0.07
L11	64.75-64.25	A	0.000	0.000	0.685	0.000	0.00
		B	0.000	0.000	0.667	0.000	0.00
		C	0.000	0.000	0.667	0.000	0.01
L12	64.25-64.00	A	0.000	0.000	0.343	0.000	0.00
		B	0.000	0.000	0.333	0.000	0.00
		C	0.000	0.000	0.333	0.000	0.00
L13	64.00-59.00	A	0.000	0.000	6.854	0.000	0.00
		B	0.000	0.000	6.667	0.000	0.01
		C	0.000	0.000	6.667	0.000	0.07
L14	59.00-56.50	A	0.000	0.000	3.427	0.000	0.00
		B	0.000	0.000	3.333	0.000	0.01
		C	0.000	0.000	3.333	0.000	0.03
L15	56.50-56.25	A	0.000	0.000	0.426	0.000	0.00
		B	0.000	0.000	0.417	0.000	0.00
		C	0.000	0.000	0.417	0.000	0.00
L16	56.25-51.25	A	0.000	0.000	8.521	0.000	0.00
		B	0.000	0.000	8.333	0.000	0.01
		C	0.000	0.000	8.333	0.000	0.07
L17	51.25-45.50	A	0.000	0.000	9.799	0.000	0.00
		B	0.000	0.000	9.583	0.000	0.01
		C	0.000	0.000	9.583	0.000	0.08
L18	45.50-44.50	A	0.000	0.000	1.121	0.000	0.00
		B	0.000	0.000	1.083	0.000	0.00
		C	0.000	0.000	1.083	0.000	0.01
L19	44.50-39.50	A	0.000	0.000	5.604	0.000	0.00
		B	0.000	0.000	5.417	0.000	0.01
		C	0.000	0.000	5.417	0.000	0.07
L20	39.50-37.25	A	0.000	0.000	2.522	0.000	0.00
		B	0.000	0.000	2.438	0.000	0.01
		C	0.000	0.000	2.438	0.000	0.03
L21	37.25-37.00	A	0.000	0.000	0.280	0.000	0.00
		B	0.000	0.000	0.271	0.000	0.00
		C	0.000	0.000	0.271	0.000	0.00
L22	37.00-36.50	A	0.000	0.000	0.560	0.000	0.00
		B	0.000	0.000	0.542	0.000	0.00
		C	0.000	0.000	0.542	0.000	0.01
L23	36.50-36.25	A	0.000	0.000	0.551	0.000	0.00
		B	0.000	0.000	0.542	0.000	0.00
		C	0.000	0.000	0.271	0.000	0.00
L24	36.25-34.25	A	0.000	0.000	4.408	0.000	0.00
		B	0.000	0.000	4.333	0.000	0.01
		C	0.000	0.000	2.167	0.000	0.03
L25	34.25-34.00	A	0.000	0.000	0.551	0.000	0.00
		B	0.000	0.000	0.542	0.000	0.00
		C	0.000	0.000	0.271	0.000	0.00

Tower Section n	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
L26	34.00-29.00	A	0.000	0.000	8.313	0.000	0.00
		B	0.000	0.000	10.833	0.000	0.01
		C	0.000	0.000	5.417	0.000	0.07
L27	29.00-24.00	A	0.000	0.000	5.604	0.000	0.00
		B	0.000	0.000	10.833	0.000	0.01
		C	0.000	0.000	5.417	0.000	0.07
L28	24.00-19.00	A	0.000	0.000	5.604	0.000	0.00
		B	0.000	0.000	10.833	0.000	0.01
		C	0.000	0.000	5.417	0.000	0.07
L29	19.00-14.00	A	0.000	0.000	5.604	0.000	0.00
		B	0.000	0.000	10.833	0.000	0.01
		C	0.000	0.000	5.417	0.000	0.07
L30	14.00-9.00	A	0.000	0.000	5.604	0.000	0.00
		B	0.000	0.000	10.833	0.000	0.01
		C	0.000	0.000	5.417	0.000	0.07
L31	9.00-4.00	A	0.000	0.000	5.604	0.000	0.00
		B	0.000	0.000	10.833	0.000	0.01
		C	0.000	0.000	5.417	0.000	0.07
L32	4.00-1.17	A	0.000	0.000	2.814	0.000	0.00
		B	0.000	0.000	5.417	0.000	0.01
		C	0.000	0.000	2.708	0.000	0.04
L33	1.17-0.92	A	0.000	0.000	0.009	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00
L34	0.92-0.00	A	0.000	0.000	0.035	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.01

**Feed Line/Linear Appurtenances Section Areas - With Ice**

Tower Section n	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
L1	110.00-105.00	A	1.913	0.000	0.000	2.101	0.000	0.03
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.03
L2	105.00-100.00	A	1.904	0.000	0.000	2.091	0.000	0.03
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.03
L3	100.00-93.00	A	1.892	0.000	0.000	2.912	0.000	0.04
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.09
L4	93.00-90.00	A	1.883	0.000	0.000	1.248	0.000	0.02
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.04
L5	90.00-85.00	A	1.874	0.000	0.000	2.062	0.000	0.03
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.07
L6	85.00-80.00	A	1.863	0.000	0.000	3.735	0.000	0.05
		B		0.000	0.000	1.684	0.000	0.03
		C		0.000	0.000	1.684	0.000	0.09
L7	80.00-79.75	A	1.857	0.000	0.000	0.383	0.000	0.00
		B		0.000	0.000	0.280	0.000	0.00
		C		0.000	0.000	0.280	0.000	0.01
L8	79.75-74.75	A	1.851	0.000	0.000	7.639	0.000	0.09
		B		0.000	0.000	5.601	0.000	0.08
		C		0.000	0.000	5.601	0.000	0.13
L9	74.75-69.75	A	1.839	0.000	0.000	7.615	0.000	0.09
		B		0.000	0.000	5.589	0.000	0.08
		C		0.000	0.000	5.589	0.000	0.13
L10	69.75-64.75	A	1.825	0.000	0.000	8.300	0.000	0.10
		B		0.000	0.000	6.287	0.000	0.09
		C		0.000	0.000	6.287	0.000	0.14
L11	64.75-64.25	A	1.818	0.000	0.000	1.231	0.000	0.01
		B		0.000	0.000	1.030	0.000	0.01
		C		0.000	0.000	1.030	0.000	0.02

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_{AA}$ In Face ft <sup>2</sup>	$C_{AA}$ Out Face ft <sup>2</sup>	Weight K
L12	64.25-64.00	A	1.817	0.000	0.000	0.615	0.000	0.01
		B		0.000	0.000	0.515	0.000	0.01
		C		0.000	0.000	0.515	0.000	0.01
L13	64.00-59.00	A	1.809	0.000	0.000	12.282	0.000	0.15
		B		0.000	0.000	10.285	0.000	0.13
		C		0.000	0.000	10.285	0.000	0.19
L14	59.00-56.50	A	1.798	0.000	0.000	6.124	0.000	0.07
		B		0.000	0.000	5.131	0.000	0.07
		C		0.000	0.000	5.131	0.000	0.09
L15	56.50-56.25	A	1.794	0.000	0.000	0.695	0.000	0.01
		B		0.000	0.000	0.594	0.000	0.01
		C		0.000	0.000	0.594	0.000	0.01
L16	56.25-51.25	A	1.785	0.000	0.000	13.876	0.000	0.16
		B		0.000	0.000	11.867	0.000	0.15
		C		0.000	0.000	11.867	0.000	0.20
L17	51.25-45.50	A	1.766	0.000	0.000	15.892	0.000	0.18
		B		0.000	0.000	13.608	0.000	0.17
		C		0.000	0.000	13.608	0.000	0.23
L18	45.50-44.50	A	1.754	0.000	0.000	1.827	0.000	0.02
		B		0.000	0.000	1.430	0.000	0.02
		C		0.000	0.000	1.430	0.000	0.03
L19	44.50-39.50	A	1.741	0.000	0.000	9.087	0.000	0.10
		B		0.000	0.000	7.129	0.000	0.09
		C		0.000	0.000	7.129	0.000	0.14
L20	39.50-37.25	A	1.726	0.000	0.000	4.075	0.000	0.04
		B		0.000	0.000	3.202	0.000	0.04
		C		0.000	0.000	3.202	0.000	0.06
L21	37.25-37.00	A	1.720	0.000	0.000	0.452	0.000	0.00
		B		0.000	0.000	0.356	0.000	0.00
		C		0.000	0.000	0.356	0.000	0.01
L22	37.00-36.50	A	1.718	0.000	0.000	0.904	0.000	0.01
		B		0.000	0.000	0.711	0.000	0.01
		C		0.000	0.000	0.711	0.000	0.01
L23	36.50-36.25	A	1.717	0.000	0.000	0.809	0.000	0.01
		B		0.000	0.000	0.713	0.000	0.01
		C		0.000	0.000	0.357	0.000	0.01
L24	36.25-34.25	A	1.711	0.000	0.000	6.462	0.000	0.07
		B		0.000	0.000	5.702	0.000	0.06
		C		0.000	0.000	2.851	0.000	0.06
L25	34.25-34.00	A	1.706	0.000	0.000	0.807	0.000	0.01
		B		0.000	0.000	0.712	0.000	0.01
		C		0.000	0.000	0.356	0.000	0.01
L26	34.00-29.00	A	1.692	0.000	0.000	12.543	0.000	0.13
		B		0.000	0.000	14.217	0.000	0.16
		C		0.000	0.000	7.109	0.000	0.14
L27	29.00-24.00	A	1.663	0.000	0.000	8.930	0.000	0.09
		B		0.000	0.000	14.159	0.000	0.15
		C		0.000	0.000	7.080	0.000	0.14
L28	24.00-19.00	A	1.629	0.000	0.000	8.861	0.000	0.09
		B		0.000	0.000	14.091	0.000	0.15
		C		0.000	0.000	7.045	0.000	0.13
L29	19.00-14.00	A	1.586	0.000	0.000	8.776	0.000	0.09
		B		0.000	0.000	14.005	0.000	0.15
		C		0.000	0.000	7.003	0.000	0.13
L30	14.00-9.00	A	1.530	0.000	0.000	8.664	0.000	0.08
		B		0.000	0.000	13.893	0.000	0.14
		C		0.000	0.000	6.946	0.000	0.13
L31	9.00-4.00	A	1.445	0.000	0.000	8.494	0.000	0.08
		B		0.000	0.000	13.723	0.000	0.13
		C		0.000	0.000	6.861	0.000	0.12
L32	4.00-1.17	A	1.318	0.000	0.000	4.219	0.000	0.04
		B		0.000	0.000	6.734	0.000	0.06
		C		0.000	0.000	3.367	0.000	0.06
L33	1.17-0.92	A	1.204	0.000	0.000	0.070	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.00
L34	0.92-0.00	A	1.109	0.000	0.000	0.239	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.01

### Feed Line Center of Pressure

Section	Elevation	$CP_x$	$CP_z$	$CP_x$ Ice	$CP_z$ Ice
	ft	in	in	in	in
L1	110.00-105.00	-0.0591	-0.2932	-0.2652	-1.3154
L2	105.00-100.00	-0.0592	-0.2935	-0.2700	-1.3390
L3	100.00-93.00	-0.0592	-0.2937	-0.2750	-1.3641
L4	93.00-90.00	-0.0593	-0.2940	-0.2779	-1.3783
L5	90.00-85.00	-0.0593	-0.2942	-0.2800	-1.3884
L6	85.00-80.00	-0.0369	-0.1832	-0.2029	-1.0060
L7	80.00-79.75	-0.0164	-0.0815	-0.1234	-0.6121
L8	79.75-74.75	-0.0167	-0.0827	-0.1254	-0.6219
L9	74.75-69.75	-0.0171	-0.0849	-0.1290	-0.6398
L10	69.75-64.75	-0.0165	-0.0817	-0.1235	-0.6125
L11	64.75-64.25	-0.0125	-0.0619	-0.0908	-0.4503
L12	64.25-64.00	-0.0125	-0.0620	-0.0910	-0.4515
L13	64.00-59.00	-0.0127	-0.0629	-0.0924	-0.4581
L14	59.00-56.50	-0.0130	-0.0642	-0.0942	-0.4674
L15	56.50-56.25	-0.0112	-0.0555	-0.0926	-0.4269
L16	56.25-51.25	-0.0114	-0.0563	-0.0936	-0.4324
L17	51.25-45.50	-0.0117	-0.0579	-0.0954	-0.4432
L18	45.50-44.50	-0.0155	-0.0770	-0.1327	-0.6187
L19	44.50-39.50	-0.0157	-0.0780	-0.1329	-0.6221
L20	39.50-37.25	-0.0160	-0.0792	-0.1338	-0.6291
L21	37.25-37.00	-0.0161	-0.0797	-0.1341	-0.6314
L22	37.00-36.50	-0.0161	-0.0798	-0.1342	-0.6321
L23	36.50-36.25	-0.0118	-1.8387	-0.0920	-2.2604
L24	36.25-34.25	-0.0118	-1.8483	-0.0924	-2.2726
L25	34.25-34.00	-0.0119	-1.8574	-0.0927	-2.2843
L26	34.00-29.00	0.8384	-1.5224	0.7654	-1.9804
L27	29.00-24.00	1.8417	-1.1420	1.7850	-1.6369
L28	24.00-19.00	1.8780	-1.1646	1.8258	-1.6659
L29	19.00-14.00	1.9134	-1.1866	1.8660	-1.6916
L30	14.00-9.00	1.9480	-1.2081	1.9056	-1.7122
L31	9.00-4.00	1.9818	-1.2292	1.9449	-1.7232
L32	4.00-1.17	2.3052	-1.4431	1.8613	-1.6888
L33	1.17-0.92	-0.0598	-0.2967	-0.2367	-1.1741
L34	0.92-0.00	-0.0598	-0.2967	-0.2230	-1.1058

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

### Shielding Factor $K_a$

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	$K_a$ No Ice	$K_a$ Ice
L1	1	Safety Line 3/8	105.00 - 110.00	1.0000	1.0000
L2	1	Safety Line 3/8	100.00 - 105.00	1.0000	1.0000
L3	1	Safety Line 3/8	93.00 - 100.00	1.0000	1.0000
L5	1	Safety Line 3/8	85.00 - 90.00	1.0000	1.0000
L6	1	Safety Line 3/8	80.00 - 85.00	1.0000	1.0000
L6	29	CCI-SFP-04510025	80.00 - 81.50	1.0000	1.0000
L6	30	CCI-SFP-04510025	80.00 -	1.0000	1.0000

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
L6	31	CCI-SFP-04510025	81.50 80.00 -	1.0000	1.0000
L7	1	Safety Line 3/8	81.50 79.75 -	1.0000	1.0000
L7	29	CCI-SFP-04510025	80.00 79.75 -	1.0000	1.0000
L7	30	CCI-SFP-04510025	80.00 79.75 -	1.0000	1.0000
L7	31	CCI-SFP-04510025	80.00 79.75 -	1.0000	1.0000
L8	1	Safety Line 3/8	80.00 74.75 -	1.0000	1.0000
L8	29	CCI-SFP-04510025	79.75 74.75 -	1.0000	1.0000
L8	30	CCI-SFP-04510025	74.75 -	1.0000	1.0000
L8	31	CCI-SFP-04510025	79.75	1.0000	1.0000
L9	1	Safety Line 3/8	74.75 69.75 -	1.0000	1.0000
L9	29	CCI-SFP-04510025	74.75 69.75 -	1.0000	1.0000
L9	30	CCI-SFP-04510025	69.75 -	1.0000	1.0000
L9	31	CCI-SFP-04510025	74.75	1.0000	1.0000
L10	1	Safety Line 3/8	69.75 64.75 -	1.0000	1.0000
L10	18	PL1.25x3.5-12	64.75 -	1.0000	1.0000
L10	19	PL1.25x3.5-12	65.50	1.0000	1.0000
L10	20	PL1.25x3.5-12	64.75 -	1.0000	1.0000
L10	29	CCI-SFP-04510025	65.50 64.75 -	1.0000	1.0000
L10	30	CCI-SFP-04510025	69.75 64.75 -	1.0000	1.0000
L10	31	CCI-SFP-04510025	69.75 64.75 -	1.0000	1.0000
L11	1	Safety Line 3/8	64.75 64.25 -	1.0000	1.0000
L11	18	PL1.25x3.5-12	64.25 -	1.0000	1.0000
L11	19	PL1.25x3.5-12	64.75	1.0000	1.0000
L11	20	PL1.25x3.5-12	64.25 -	1.0000	1.0000
L11	29	CCI-SFP-04510025	64.75	1.0000	1.0000
L11	30	CCI-SFP-04510025	64.25 -	1.0000	1.0000
L11	31	CCI-SFP-04510025	64.75	1.0000	1.0000
L12	1	Safety Line 3/8	64.25 64.00 -	1.0000	1.0000
L12	18	PL1.25x3.5-12	64.25	1.0000	1.0000
L12	19	PL1.25x3.5-12	64.00 -	1.0000	1.0000
L12	20	PL1.25x3.5-12	64.25	1.0000	1.0000
L12	29	CCI-SFP-04510025	64.00 -	1.0000	1.0000
L12	30	CCI-SFP-04510025	64.25	1.0000	1.0000
L12	31	CCI-SFP-04510025	64.00 -	1.0000	1.0000

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
L13	1	Safety Line 3/8	59.00 - 64.00	1.0000	1.0000
L13	18	PL1.25x3.5-12	59.00 - 64.00	1.0000	1.0000
L13	19	PL1.25x3.5-12	59.00 - 64.00	1.0000	1.0000
L13	20	PL1.25x3.5-12	59.00 - 64.00	1.0000	1.0000
L13	29	CCI-SFP-04510025	59.00 - 64.00	1.0000	1.0000
L13	30	CCI-SFP-04510025	59.00 - 64.00	1.0000	1.0000
L13	31	CCI-SFP-04510025	59.00 - 64.00	1.0000	1.0000
L14	1	Safety Line 3/8	56.50 - 59.00	1.0000	1.0000
L14	18	PL1.25x3.5-12	56.50 - 59.00	1.0000	1.0000
L14	19	PL1.25x3.5-12	56.50 - 59.00	1.0000	1.0000
L14	20	PL1.25x3.5-12	56.50 - 59.00	1.0000	1.0000
L14	29	CCI-SFP-04510025	56.50 - 59.00	1.0000	1.0000
L14	30	CCI-SFP-04510025	56.50 - 59.00	1.0000	1.0000
L14	31	CCI-SFP-04510025	56.50 - 59.00	1.0000	1.0000
L15	1	Safety Line 3/8	56.25 - 56.50	1.0000	1.0000
L15	18	PL1.25x3.5-12	56.25 - 56.50	1.0000	1.0000
L15	19	PL1.25x3.5-12	56.25 - 56.50	1.0000	1.0000
L15	20	PL1.25x3.5-12	56.25 - 56.50	1.0000	1.0000
L15	26	CCI-SFP-06512525	56.25 - 56.50	1.0000	1.0000
L15	27	CCI-SFP-06512520	56.25 - 56.50	1.0000	1.0000
L15	28	CCI-SFP-06512520	56.25 - 56.50	1.0000	1.0000
L16	1	Safety Line 3/8	51.25 - 56.25	1.0000	1.0000
L16	18	PL1.25x3.5-12	51.25 - 56.25	1.0000	1.0000
L16	19	PL1.25x3.5-12	51.25 - 56.25	1.0000	1.0000
L16	20	PL1.25x3.5-12	51.25 - 56.25	1.0000	1.0000
L16	26	CCI-SFP-06512525	51.25 - 56.25	1.0000	1.0000
L16	27	CCI-SFP-06512520	51.25 - 56.25	1.0000	1.0000
L16	28	CCI-SFP-06512520	51.25 - 56.25	1.0000	1.0000
L17	1	Safety Line 3/8	45.50 - 51.25	1.0000	1.0000
L17	18	PL1.25x3.5-12	45.50 - 51.25	1.0000	1.0000
L17	19	PL1.25x3.5-12	45.50 - 51.25	1.0000	1.0000
L17	20	PL1.25x3.5-12	45.50 - 51.25	1.0000	1.0000
L17	26	CCI-SFP-06512525	45.50 - 51.25	1.0000	1.0000
L17	27	CCI-SFP-06512520	45.50 - 51.25	1.0000	1.0000
L17	28	CCI-SFP-06512520	45.50 - 51.25	1.0000	1.0000

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
L19	1	Safety Line 3/8	51.25 39.50 - 44.50	1.0000	1.0000
L19	26	CCI-SFP-06512525	39.50 - 44.50	1.0000	1.0000
L19	27	CCI-SFP-06512520	39.50 - 44.50	1.0000	1.0000
L19	28	CCI-SFP-06512520	39.50 - 44.50	1.0000	1.0000
L20	1	Safety Line 3/8	37.25 - 39.50	1.0000	1.0000
L20	26	CCI-SFP-06512525	37.25 - 39.50	1.0000	1.0000
L20	27	CCI-SFP-06512520	37.25 - 39.50	1.0000	1.0000
L20	28	CCI-SFP-06512520	37.25 - 39.50	1.0000	1.0000
L21	1	Safety Line 3/8	37.00 - 37.25	1.0000	1.0000
L21	26	CCI-SFP-06512525	37.00 - 37.25	1.0000	1.0000
L21	27	CCI-SFP-06512520	37.00 - 37.25	1.0000	1.0000
L21	28	CCI-SFP-06512520	37.00 - 37.25	1.0000	1.0000
L22	1	Safety Line 3/8	36.50 - 37.00	1.0000	1.0000
L22	26	CCI-SFP-06512525	36.50 - 37.00	1.0000	1.0000
L22	27	CCI-SFP-06512520	36.50 - 37.00	1.0000	1.0000
L22	28	CCI-SFP-06512520	36.50 - 37.00	1.0000	1.0000
L23	1	Safety Line 3/8	36.25 - 36.50	1.0000	1.0000
L23	22	CCI-SFP-06512535	36.25 - 36.50	1.0000	1.0000
L23	23	CCI-SFP-06512535	36.25 - 36.50	1.0000	1.0000
L23	24	CCI-SFP-06512535	36.25 - 36.50	1.0000	1.0000
L23	25	CCI-SFP-06512535	36.25 - 36.50	1.0000	1.0000
L23	26	CCI-SFP-06512525	36.25 - 36.50	1.0000	1.0000
L24	1	Safety Line 3/8	34.25 - 36.25	1.0000	1.0000
L24	22	CCI-SFP-06512535	34.25 - 36.25	1.0000	1.0000
L24	23	CCI-SFP-06512535	34.25 - 36.25	1.0000	1.0000
L24	24	CCI-SFP-06512535	34.25 - 36.25	1.0000	1.0000
L24	25	CCI-SFP-06512535	34.25 - 36.25	1.0000	1.0000
L24	26	CCI-SFP-06512525	34.25 - 36.25	1.0000	1.0000
L25	1	Safety Line 3/8	34.00 - 34.25	1.0000	1.0000
L25	22	CCI-SFP-06512535	34.00 - 34.25	1.0000	1.0000
L25	23	CCI-SFP-06512535	34.00 - 34.25	1.0000	1.0000
L25	24	CCI-SFP-06512535	34.00 - 34.25	1.0000	1.0000
L25	25	CCI-SFP-06512535	34.00 - 34.25	1.0000	1.0000
L25	26	CCI-SFP-06512525	34.00 - 34.25	1.0000	1.0000



Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
L26	1	Safety Line 3/8	29.00 - 34.00	1.0000	1.0000
L26	22	CCI-SFP-06512535	29.00 - 34.00	1.0000	1.0000
L26	23	CCI-SFP-06512535	29.00 - 34.00	1.0000	1.0000
L26	24	CCI-SFP-06512535	29.00 - 34.00	1.0000	1.0000
L26	25	CCI-SFP-06512535	29.00 - 34.00	1.0000	1.0000
L26	26	CCI-SFP-06512525	31.50 - 34.00	1.0000	1.0000
L27	1	Safety Line 3/8	24.00 - 29.00	1.0000	1.0000
L27	22	CCI-SFP-06512535	24.00 - 29.00	1.0000	1.0000
L27	23	CCI-SFP-06512535	24.00 - 29.00	1.0000	1.0000
L27	24	CCI-SFP-06512535	24.00 - 29.00	1.0000	1.0000
L27	25	CCI-SFP-06512535	24.00 - 29.00	1.0000	1.0000
L28	1	Safety Line 3/8	19.00 - 24.00	1.0000	1.0000
L28	22	CCI-SFP-06512535	19.00 - 24.00	1.0000	1.0000
L28	23	CCI-SFP-06512535	19.00 - 24.00	1.0000	1.0000
L28	24	CCI-SFP-06512535	19.00 - 24.00	1.0000	1.0000
L28	25	CCI-SFP-06512535	19.00 - 24.00	1.0000	1.0000
L29	1	Safety Line 3/8	14.00 - 19.00	1.0000	1.0000
L29	22	CCI-SFP-06512535	14.00 - 19.00	1.0000	1.0000
L29	23	CCI-SFP-06512535	14.00 - 19.00	1.0000	1.0000
L29	24	CCI-SFP-06512535	14.00 - 19.00	1.0000	1.0000
L29	25	CCI-SFP-06512535	14.00 - 19.00	1.0000	1.0000
L30	1	Safety Line 3/8	9.00 - 14.00	1.0000	1.0000
L30	22	CCI-SFP-06512535	9.00 - 14.00	1.0000	1.0000
L30	23	CCI-SFP-06512535	9.00 - 14.00	1.0000	1.0000
L30	24	CCI-SFP-06512535	9.00 - 14.00	1.0000	1.0000
L30	25	CCI-SFP-06512535	9.00 - 14.00	1.0000	1.0000
L31	1	Safety Line 3/8	4.00 - 9.00	1.0000	1.0000
L31	22	CCI-SFP-06512535	4.00 - 9.00	1.0000	1.0000
L31	23	CCI-SFP-06512535	4.00 - 9.00	1.0000	1.0000
L31	24	CCI-SFP-06512535	4.00 - 9.00	1.0000	1.0000
L31	25	CCI-SFP-06512535	4.00 - 9.00	1.0000	1.0000
L32	1	Safety Line 3/8	1.17 - 4.00	1.0000	1.0000
L32	22	CCI-SFP-06512535	1.50 - 4.00	1.0000	1.0000
L32	23	CCI-SFP-06512535	1.50 - 4.00	1.0000	1.0000
L32	24	CCI-SFP-06512535	1.50 - 4.00	1.0000	1.0000
L32	25	CCI-SFP-06512535	1.50 - 4.00	1.0000	1.0000
L33	1	Safety Line 3/8	0.92 - 1.17	1.0000	1.0000
L34	1	Safety Line 3/8	0.00 - 0.92	1.0000	1.0000

**Discrete Tower Loads**

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Lateral						Vert
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
Lightning Rod 5/8"x4'	C	From Leg	0.00		0.0000	110.00	No Ice	0.25	0.25	0.00
			0.00				1/2"	0.66	0.66	0.01
			2.00				Ice	0.97	0.97	0.01
							1" Ice	1.49	1.49	0.03
							2" Ice			
***										
Platform Mount [10.75' LP 712-1]	C	None			0.0000	110.00	No Ice	21.97	21.97	1.20
							1/2"	26.82	26.82	1.47
							Ice	31.67	31.67	1.75
							1" Ice	41.36	41.36	2.31
							2" Ice			
(2) T-Arm Mount [TA 702-3]	C	None			0.0000	110.00	No Ice	5.64	5.64	0.34
							1/2"	6.55	6.55	0.43
							Ice	7.46	7.46	0.52
							1" Ice	9.28	9.28	0.70
							2" Ice			
Transition Ladder	C	From Leg	2.00		0.0000	110.00	No Ice	6.00	6.00	0.16
			0.00				1/2"	8.00	8.00	0.24
			-3.00				Ice	10.00	10.00	0.32
							1" Ice	14.00	14.00	0.48
							2" Ice			
8'x2" Mount Pipe	A	From Leg	3.00		0.0000	110.00	No Ice	1.90	1.90	0.03
			-2.00				1/2"	2.73	2.73	0.04
			0.00				Ice	3.40	3.40	0.06
							1" Ice	4.40	4.40	0.12
							2" Ice			
8'x2" Mount Pipe	B	From Leg	3.00		0.0000	110.00	No Ice	1.90	1.90	0.03
			-2.00				1/2"	2.73	2.73	0.04
			0.00				Ice	3.40	3.40	0.06
							1" Ice	4.40	4.40	0.12
							2" Ice			
8'x2" Mount Pipe	C	From Leg	3.00		0.0000	110.00	No Ice	1.90	1.90	0.03
			-2.00				1/2"	2.73	2.73	0.04
			0.00				Ice	3.40	3.40	0.06
							1" Ice	4.40	4.40	0.12
							2" Ice			
QS66512-2 w/ Mount Pipe	A	From Leg	4.00		0.0000	110.00	No Ice	8.64	6.66	0.14
			7.00				1/2"	9.29	9.66	0.21
			3.00				Ice	9.91	10.62	0.30
							1" Ice	11.18	12.61	0.49
							2" Ice			
QS66512-2 w/ Mount Pipe	B	From Leg	4.00		0.0000	110.00	No Ice	8.64	6.66	0.14
			7.00				1/2"	9.29	9.66	0.21
			3.00				Ice	9.91	10.62	0.30
							1" Ice	11.18	12.61	0.49
							2" Ice			
QS66512-2 w/ Mount Pipe	C	From Leg	4.00		0.0000	110.00	No Ice	8.64	6.66	0.14
			7.00				1/2"	9.29	9.66	0.21
			3.00				Ice	9.91	10.62	0.30
							1" Ice	11.18	12.61	0.49
							2" Ice			
AM-X-CD-16-65-00T-RET_TIA w/ Mount Pipe	A	From Leg	3.00		0.0000	110.00	No Ice	8.26	6.36	0.07
			2.00				1/2"	8.82	7.54	0.14
			3.00				Ice	9.35	8.43	0.21
							1" Ice	10.42	10.24	0.39
							2" Ice			
AM-X-CD-16-65-00T-RET_TIA w/ Mount Pipe	B	From Leg	3.00		0.0000	110.00	No Ice	8.26	6.36	0.07
			2.00				1/2"	8.82	7.54	0.14
			3.00				Ice	9.35	8.43	0.21
							1" Ice	10.42	10.24	0.39
							2" Ice			
AM-X-CD-16-65-00T-RET_TIA w/ Mount Pipe	C	From Leg	3.00		0.0000	110.00	No Ice	8.26	6.36	0.07
			2.00				1/2"	8.82	7.54	0.14
			3.00				Ice	9.35	8.43	0.21
							1" Ice	10.42	10.24	0.39
							2" Ice			

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight K	
P65-15-XLH-RR w/ Mount Pipe	A	From Leg	4.00 -7.00 3.00	0.0000	110.00	2" Ice			
						No Ice	5.67	4.33	0.06
						1/2"	6.08	5.01	0.11
						Ice	6.49	5.67	0.16
						1" Ice	7.34	7.01	0.29
P65-15-XLH-RR w/ Mount Pipe	B	From Leg	4.00 -7.00 3.00	0.0000	110.00	2" Ice			
						No Ice	5.67	4.33	0.06
						1/2"	6.08	5.01	0.11
						Ice	6.49	5.67	0.16
						1" Ice	7.34	7.01	0.29
P65-15-XLH-RR w/ Mount Pipe	C	From Leg	4.00 -7.00 3.00	0.0000	110.00	2" Ice			
						No Ice	5.67	4.33	0.06
						1/2"	6.08	5.01	0.11
						Ice	6.49	5.67	0.16
						1" Ice	7.34	7.01	0.29
(2) 7020.00	A	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	0.10	0.17	0.00
						1/2"	0.15	0.24	0.01
						Ice	0.20	0.31	0.01
						1" Ice	0.33	0.48	0.02
(2) 7020.00	B	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	0.10	0.17	0.00
						1/2"	0.15	0.24	0.01
						Ice	0.20	0.31	0.01
						1" Ice	0.33	0.48	0.02
(2) 7020.00	C	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	0.10	0.17	0.00
						1/2"	0.15	0.24	0.01
						Ice	0.20	0.31	0.01
						1" Ice	0.33	0.48	0.02
RRUS 32	A	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	2.86	1.78	0.06
						1/2"	3.08	1.97	0.08
						Ice	3.32	2.17	0.10
						1" Ice	3.81	2.58	0.16
RRUS 32	B	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	2.86	1.78	0.06
						1/2"	3.08	1.97	0.08
						Ice	3.32	2.17	0.10
						1" Ice	3.81	2.58	0.16
RRUS 32	C	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	2.86	1.78	0.06
						1/2"	3.08	1.97	0.08
						Ice	3.32	2.17	0.10
						1" Ice	3.81	2.58	0.16
RRUS 32 B2	A	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	2.73	1.67	0.05
						1/2"	2.95	1.86	0.07
						Ice	3.18	2.05	0.10
						1" Ice	3.66	2.46	0.16
RRUS 32 B2	B	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	2.73	1.67	0.05
						1/2"	2.95	1.86	0.07
						Ice	3.18	2.05	0.10
						1" Ice	3.66	2.46	0.16
RRUS 32 B2	C	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	2.73	1.67	0.05
						1/2"	2.95	1.86	0.07
						Ice	3.18	2.05	0.10
						1" Ice	3.66	2.46	0.16
DC6-48-60-18-8F	A	From Leg	1.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	0.92	0.92	0.02
						1/2"	1.46	1.46	0.04
						Ice	1.64	1.64	0.06
						1" Ice	2.04	2.04	0.11

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight K	
DC6-48-60-18-8F	A	From Leg	1.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	0.92	0.92	0.02
						1/2"	1.46	1.46	0.04
						Ice	1.64	1.64	0.06
(2) TT19-08BP111-001	A	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	0.55	0.45	0.02
						1/2"	0.65	0.53	0.02
						Ice	0.75	0.63	0.03
(2) TT19-08BP111-001	B	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	0.55	0.45	0.02
						1/2"	0.65	0.53	0.02
						Ice	0.75	0.63	0.03
(2) TT19-08BP111-001	C	From Leg	4.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	0.55	0.45	0.02
						1/2"	0.65	0.53	0.02
						Ice	0.75	0.63	0.03
RRUS 11	A	From Leg	3.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	2.78	1.19	0.05
						1/2"	2.99	1.33	0.07
						Ice	3.21	1.49	0.10
RRUS 11	B	From Leg	3.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	2.78	1.19	0.05
						1/2"	2.99	1.33	0.07
						Ice	3.21	1.49	0.10
RRUS 11	C	From Leg	3.00 0.00 3.00	0.0000	110.00	2" Ice			
						No Ice	2.78	1.19	0.05
						1/2"	2.99	1.33	0.07
						Ice	3.21	1.49	0.10
*** Platform Mount [10.75' LP 712-1]	C	None		0.0000	100.00	2" Ice			
						No Ice	21.97	21.97	1.20
						1/2"	26.82	26.82	1.47
						Ice	31.67	31.67	1.75
Transition Ladder	C	From Leg	2.00 0.00 -3.00	0.0000	100.00	2" Ice			
						No Ice	6.00	6.00	0.16
						1/2"	8.00	8.00	0.24
						Ice	10.00	10.00	0.32
(2) 8'x2" Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	100.00	2" Ice			
						No Ice	1.90	1.90	0.03
						1/2"	2.73	2.73	0.04
						Ice	3.40	3.40	0.06
APXVAARR24_43-U- NA20_TIA w/ Mount Pipe	A	From Leg	4.00 5.00 0.00	0.0000	100.00	2" Ice			
						No Ice	20.48	11.02	0.19
						1/2"	21.23	12.55	0.32
						Ice	21.99	14.10	0.47
APXVAARR24_43-U- NA20_TIA w/ Mount Pipe	C	From Leg	4.00 5.00 0.00	0.0000	100.00	2" Ice			
						No Ice	20.48	11.02	0.19
						1/2"	21.23	12.55	0.32
						Ice	21.99	14.10	0.47
RR90-17-00DP w/ Mount Pipe	A	From Leg	4.00 -5.00 0.00	0.0000	100.00	2" Ice			
						No Ice	4.59	3.32	0.03
						1/2"	5.02	4.09	0.07
						Ice	5.44	4.78	0.12



Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight K
			0.00			Ice 2.29	2.29	0.05
						1" Ice 3.06	3.06	0.09
						2" Ice		
SBNHH-1D65A_TIA w/ Mount Pipe	A	From Leg	1.00	0.0000	83.00	No Ice 6.19	5.25	0.05
			1.00			1/2" 6.64	6.04	0.11
			0.00			Ice 7.07	6.74	0.17
						1" Ice 7.97	8.18	0.31
						2" Ice		
SBNHH-1D65A_TIA w/ Mount Pipe	A	From Leg	1.00	0.0000	83.00	No Ice 6.19	5.25	0.05
			-1.00			1/2" 6.64	6.04	0.11
			0.00			Ice 7.07	6.74	0.17
						1" Ice 7.97	8.18	0.31
						2" Ice		
SBNHH-1D65A_TIA w/ Mount Pipe	C	From Face	1.00	0.0000	83.00	No Ice 6.19	5.25	0.05
			1.00			1/2" 6.64	6.04	0.11
			0.00			Ice 7.07	6.74	0.17
						1" Ice 7.97	8.18	0.31
						2" Ice		
SBNHH-1D65A_TIA w/ Mount Pipe	C	From Face	1.00	0.0000	83.00	No Ice 6.19	5.25	0.05
			-1.00			1/2" 6.64	6.04	0.11
			0.00			Ice 7.07	6.74	0.17
						1" Ice 7.97	8.18	0.31
						2" Ice		
B13 RRH4X30-4R	C	From Leg	1.00	0.0000	83.00	No Ice 2.16	1.62	0.06
			0.00			1/2" 2.35	1.79	0.08
			0.00			Ice 2.55	1.97	0.10
						1" Ice 2.97	2.36	0.15
						2" Ice		
B13 RRH4X30-4R	A	From Face	1.00	0.0000	83.00	No Ice 2.16	1.62	0.06
			0.00			1/2" 2.35	1.79	0.08
			0.00			Ice 2.55	1.97	0.10
						1" Ice 2.97	2.36	0.15
						2" Ice		
DB-T1-6Z-8AB-0Z	B	From Leg	1.00	0.0000	83.00	No Ice 4.80	2.00	0.04
			0.00			1/2" 5.07	2.19	0.08
			0.00			Ice 5.35	2.39	0.12
						1" Ice 5.93	2.81	0.21
						2" Ice		
DB-T1-6Z-8AB-0Z	A	From Leg	1.00	0.0000	83.00	No Ice 4.80	2.00	0.04
			0.00			1/2" 5.07	2.19	0.08
			0.00			Ice 5.35	2.39	0.12
						1" Ice 5.93	2.81	0.21
						2" Ice		
B66A RRH4X45	A	From Face	1.00	0.0000	83.00	No Ice 2.58	1.63	0.06
			0.00			1/2" 2.79	1.81	0.08
			0.00			Ice 3.01	2.00	0.10
						1" Ice 3.48	2.40	0.16
						2" Ice		
B66A RRH4X45	C	From Leg	1.00	0.0000	83.00	No Ice 2.58	1.63	0.06
			0.00			1/2" 2.79	1.81	0.08
			0.00			Ice 3.01	2.00	0.10
						1" Ice 3.48	2.40	0.16
						2" Ice		
RRH2X60-PCS	C	From Leg	1.00	0.0000	83.00	No Ice 2.20	1.72	0.06
			0.00			1/2" 2.39	1.90	0.08
			0.00			Ice 2.59	2.09	0.10
						1" Ice 3.01	2.48	0.16
						2" Ice		
RRH2X60-PCS	A	From Face	1.00	0.0000	83.00	No Ice 2.20	1.72	0.06
			0.00			1/2" 2.39	1.90	0.08
			0.00			Ice 2.59	2.09	0.10
						1" Ice 3.01	2.48	0.16
						2" Ice		
***								
Side Arm Mount [SO 201-	A	From Leg	1.00	0.0000	69.00	No Ice 2.96	2.11	0.10

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>Front</sub>	C <sub>A</sub> A <sub>Side</sub>	Weight	
			Horz	Lateral						Vert
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
1]			0.00			1/2"	4.10	2.93	0.12	
			0.00			Ice	5.24	3.75	0.14	
						1" Ice	7.52	5.39	0.18	
						2" Ice				
12' HSS 3x3x1/4	A	From Leg	2.00		0.0000	69.00	No Ice	3.96	0.15	0.12
			0.00				1/2"	4.88	0.22	0.15
			0.00				Ice	5.80	0.30	0.18
							1" Ice	7.64	0.45	0.24
							2" Ice			
7'x2" Mount Pipe	A	From Leg	2.00		0.0000	69.00	No Ice	1.66	1.66	0.03
			-6.00				1/2"	2.39	2.39	0.04
			-2.00				Ice	2.83	2.83	0.06
							1" Ice	3.71	3.71	0.10
							2" Ice			
7'x2" Mount Pipe	A	From Leg	2.00		0.0000	69.00	No Ice	1.66	1.66	0.03
			-2.00				1/2"	2.39	2.39	0.04
			-2.00				Ice	2.83	2.83	0.06
							1" Ice	3.71	3.71	0.10
							2" Ice			
7'x2" Mount Pipe	A	From Leg	2.00		0.0000	69.00	No Ice	1.66	1.66	0.03
			2.00				1/2"	2.39	2.39	0.04
			-2.00				Ice	2.83	2.83	0.06
							1" Ice	3.71	3.71	0.10
							2" Ice			
DB201-F	A	From Leg	2.00		0.0000	69.00	No Ice	0.40	0.40	0.01
			-6.00				1/2"	0.72	0.72	0.01
			2.00				Ice	1.04	1.04	0.02
							1" Ice	1.68	1.68	0.02
							2" Ice			
DB810M-XC	A	From Leg	2.00		0.0000	69.00	No Ice	2.12	2.12	0.03
			2.00				1/2"	3.14	3.14	0.05
			5.00				Ice	4.18	4.18	0.07
							1" Ice	5.77	5.77	0.13
							2" Ice			
48212S w/ Mount Pipe	A	From Leg	2.00		0.0000	69.00	No Ice	4.62	3.12	0.04
			6.00				1/2"	5.03	3.83	0.08
			3.00				Ice	5.44	4.49	0.12
							1" Ice	6.28	5.88	0.23
							2" Ice			

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## Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice

Comb. No.	Description
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	110 - 105	Pole	Max Tension	26	0.00	-0.00	-0.00
			Max. Compression	26	-10.99	1.26	-0.19
			Max. Mx	20	-3.80	39.25	-0.11
			Max. My	14	-3.79	0.43	-39.03
			Max. Vy	20	-5.92	39.25	-0.11
			Max. Vx	2	-5.93	0.37	38.70
			Max. Torque	24			0.62
L2	105 - 100	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-11.42	1.41	-0.05
			Max. Mx	20	-4.03	69.30	-0.09
			Max. My	14	-4.02	0.49	-69.12
			Max. Vy	20	-6.10	69.30	-0.09
			Max. Vx	2	-6.11	0.38	68.80
			Max. Torque	24			0.62
L3	100 - 93	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-17.69	5.28	4.65
			Max. Mx	20	-6.53	114.65	0.25
			Max. My	2	-6.48	0.73	115.37
			Max. Vy	20	-9.05	114.65	0.25
			Max. Vx	2	-9.25	0.73	115.37
			Max. Torque	18			-3.01
L4	93 - 90	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-18.41	5.44	4.82
			Max. Mx	20	-6.98	160.35	-0.50
			Max. My	2	-6.94	-0.05	162.08



Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L5	90 - 85	Pole	Max. Vy	20	-9.23	160.35	-0.50
			Max. Vx	2	-9.43	-0.05	162.08
			Max. Torque	18			-3.01
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-18.99	5.58	4.96
			Max. Mx	20	-7.40	206.82	-1.27
			Max. My	2	-7.36	-0.82	209.58
			Max. Vy	20	-9.38	206.82	-1.27
			Max. Vx	2	-9.58	-0.82	209.58
L6	85 - 80	Pole	Max. Torque	18			-3.00
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-23.37	6.96	5.70
			Max. Mx	20	-8.97	260.04	-1.85
			Max. My	2	-8.92	-1.01	263.75
			Max. Vy	20	-11.31	260.04	-1.85
			Max. Vx	2	-11.64	-1.01	263.75
			Max. Torque	18			-3.40
			Max Tension	1	0.00	0.00	0.00
L7	80 - 79.75	Pole	Max. Compression	26	-23.43	6.97	5.70
			Max. Mx	20	-9.01	262.86	-1.89
			Max. My	2	-8.96	-1.05	266.66
			Max. Vy	20	-11.32	262.86	-1.89
			Max. Vx	14	11.65	5.62	-263.70
			Max. Torque	18			-3.40
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-24.54	7.08	5.83
			Max. Mx	20	-9.71	320.28	-2.66
L8	79.75 - 74.75	Pole	Max. My	2	-9.66	-1.83	325.71
			Max. Vy	20	-11.66	320.28	-2.66
			Max. Vx	14	12.01	6.45	-322.83
			Max. Torque	18			-3.40
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-25.67	7.18	5.95
			Max. Mx	20	-10.43	379.36	-3.43
			Max. My	2	-10.38	-2.62	386.44
			Max. Vy	20	-11.99	379.36	-3.43
L9	74.75 - 69.75	Pole	Max. Vx	14	12.37	7.27	-383.75
			Max. Torque	18			-3.40
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-27.97	6.47	9.05
			Max. Mx	20	-11.61	442.28	-3.08
			Max. My	2	-11.55	-3.44	453.32
			Max. Vy	20	-12.77	442.28	-3.08
			Max. Vx	14	13.39	8.06	-448.59
			Max. Torque	18			-4.03
L10	69.75 - 64.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-28.11	6.48	9.06
			Max. Mx	20	-11.69	448.67	-3.16
			Max. My	2	-11.64	-3.52	459.98
			Max. Vy	20	-12.80	448.67	-3.16
			Max. Vx	14	13.43	8.15	-455.29
			Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-28.19	6.48	9.07
L11	64.75 - 64.25	Pole	Max. Mx	20	-11.74	451.87	-3.20
			Max. My	2	-11.69	-3.55	463.32
			Max. Vy	20	-12.82	451.87	-3.20
			Max. Vx	14	13.45	8.19	-458.65
			Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-29.80	6.56	9.17
			Max. Mx	20	-12.75	516.89	-3.97
			Max. My	2	-12.70	-4.34	531.04
L12	64.25 - 64	Pole	Max. Vy	20	-13.20	516.89	-3.97
			Max. Vx	14	13.87	9.01	-526.92
			Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-29.80	6.56	9.17
L13	64 - 59	Pole	Max. Mx	20	-12.75	516.89	-3.97
			Max. My	2	-12.70	-4.34	531.04
			Max. Vy	20	-13.20	516.89	-3.97
			Max. Vx	14	13.87	9.01	-526.92
			Max. Torque	18			-4.02

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L14	59 - 56.5	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-30.61	6.60	9.22
			Max. Mx	20	-13.26	550.11	-4.36
			Max. My	2	-13.21	-4.74	565.62
			Max. Vy	20	-13.39	550.11	-4.36
			Max. Vx	14	14.09	9.41	-561.85
L15	56.5 - 56.25	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-30.71	6.61	9.22
			Max. Mx	20	-13.33	553.46	-4.40
			Max. My	2	-13.28	-4.78	569.10
			Max. Vy	20	-13.40	553.46	-4.40
			Max. Vx	14	14.10	9.46	-565.37
L16	56.25 - 51.25	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-32.61	6.68	9.31
			Max. Mx	20	-14.57	621.49	-5.18
			Max. My	2	-14.52	-5.57	639.84
			Max. Vy	20	-13.81	621.49	-5.18
			Max. Vx	14	14.56	10.27	-637.01
L17	51.25 - 45.5	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-33.28	6.71	9.35
			Max. Mx	20	-15.00	645.77	-5.46
			Max. My	2	-14.96	-5.84	665.08
			Max. Vy	20	-13.96	645.77	-5.46
			Max. Vx	14	14.72	10.56	-662.63
L18	45.5 - 44.5	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-36.14	6.78	9.43
			Max. Mx	20	-17.00	716.67	-6.24
			Max. My	2	-16.96	-6.63	738.69
			Max. Vy	20	-14.40	716.67	-6.24
			Max. Vx	14	15.22	11.38	-737.49
L19	44.5 - 39.5	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-37.73	6.85	9.52
			Max. Mx	20	-18.13	789.46	-7.02
			Max. My	14	-18.07	12.19	-814.43
			Max. Vy	20	-14.73	789.46	-7.02
			Max. Vx	14	15.58	12.19	-814.43
L20	39.5 - 37.25	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-38.46	6.88	9.55
			Max. Mx	20	-18.65	822.76	-7.38
			Max. My	14	-18.59	12.56	-849.65
			Max. Vy	20	-14.88	822.76	-7.38
			Max. Vx	14	15.74	12.56	-849.65
L21	37.25 - 37	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-38.55	6.89	9.55
			Max. Mx	20	-18.72	826.48	-7.42
			Max. My	14	-18.67	12.60	-853.58
			Max. Vy	20	-14.89	826.48	-7.42
			Max. Vx	14	15.75	12.60	-853.58
L22	37 - 36.5	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-38.73	6.89	9.56
			Max. Mx	20	-18.85	833.94	-7.50
			Max. My	14	-18.80	12.68	-861.47
			Max. Vy	20	-14.93	833.94	-7.50
			Max. Vx	14	15.79	12.68	-861.47
L23	36.5 - 36.25	Pole	Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-38.83	6.90	9.57
			Max. Mx	20	-18.92	837.67	-7.53
			Max. My	14	-18.87	12.72	-865.42

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L24	36.25 - 34.25	Pole	Max. Vy	20	-14.94	837.67	-7.53
			Max. Vx	14	15.81	12.72	-865.42
			Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-39.68	6.92	9.63
L25	34.25 - 34	Pole	Max. Mx	20	-19.51	867.70	-7.85
			Max. My	14	-19.46	13.04	-897.21
			Max. Vy	20	-15.10	867.70	-7.85
			Max. Vx	14	15.99	13.04	-897.21
			Max. Torque	18			-4.02
L26	34 - 29	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-39.77	6.92	9.64
			Max. Mx	20	-19.58	871.48	-7.89
			Max. My	14	-19.53	13.08	-901.20
			Max. Vy	20	-15.11	871.48	-7.89
L27	29 - 24	Pole	Max. Vx	14	16.00	13.08	-901.20
			Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-41.68	6.95	9.76
			Max. Mx	20	-20.91	947.89	-8.67
L28	24 - 19	Pole	Max. My	14	-20.86	13.89	-982.17
			Max. Vy	20	-15.46	947.89	-8.67
			Max. Vx	14	16.40	13.89	-982.17
			Max. Torque	18			-4.02
			Max Tension	1	0.00	0.00	0.00
L29	19 - 14	Pole	Max. Compression	26	-43.57	6.93	9.86
			Max. Mx	20	-22.27	1026.00	-9.46
			Max. My	14	-22.23	14.69	-1065.02
			Max. Vy	20	-15.80	1026.00	-9.46
			Max. Vx	14	16.76	14.69	-1065.02
L30	14 - 9	Pole	Max. Torque	18			-4.01
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-45.47	6.90	9.96
			Max. Mx	20	-23.64	1105.81	-10.24
			Max. My	14	-23.61	15.49	-1149.67
L31	9 - 4	Pole	Max. Vy	20	-16.14	1105.81	-10.24
			Max. Vx	14	17.13	15.49	-1149.67
			Max. Torque	18			-4.01
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-47.38	6.87	10.04
L32	4 - 1.17	Pole	Max. Mx	20	-25.04	1187.30	-11.02
			Max. My	14	-25.02	16.27	-1236.14
			Max. Vy	20	-16.48	1187.30	-11.02
			Max. Vx	14	17.49	16.27	-1236.14
			Max. Torque	18			-4.01
L33	1.17 - 0.92	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-51.19	6.78	10.15
			Max. Mx	20	-27.91	1355.34	-12.56
			Max. My	14	-27.90	17.83	-1414.50
			Max. Vy	20	-17.15	1355.34	-12.56
			Max. Vx	14	18.21	17.83	-1414.50
			Max. Torque	18			-4.01
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-52.24	6.76	10.17
			Max. Mx	20	-28.73	1404.12	-13.00
			Max. My	14	-28.73	18.26	-1466.27
			Max. Vy	20	-17.35	1404.12	-13.00
			Max. Vx	14	18.41	18.26	-1466.27
			Max. Torque	18			-4.01
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-52.32	6.76	10.17

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L34	0.92 - 0	Pole	Max. Mx	20	-28.82	1408.45	-13.04
			Max. My	14	-28.82	18.30	-1470.87
			Max. Vy	20	-17.34	1408.45	-13.04
			Max. Vx	14	18.41	18.30	-1470.87
			Max. Torque	18			-4.01
			Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-52.64	6.76	10.17
			Max. Mx	20	-29.10	1424.42	-13.18
			Max. My	14	-29.10	18.44	-1487.82
			Max. Vy	20	-17.38	1424.42	-13.18
			Max. Vx	14	18.44	18.44	-1487.82
			Max. Torque	18			-4.01

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	26	52.64	0.00	0.00
	Max. H <sub>x</sub>	21	21.83	17.37	-0.15
	Max. H <sub>z</sub>	2	29.10	-0.15	17.60
	Max. M <sub>x</sub>	2	1466.23	-0.15	17.60
	Max. M <sub>z</sub>	8	1416.47	-17.10	0.15
	Max. Torsion	6	3.98	-15.42	9.25
	Min. Vert	11	21.83	-14.74	-8.68
	Min. H <sub>x</sub>	8	29.10	-17.10	0.15
	Min. H <sub>z</sub>	15	21.83	0.15	-18.43
	Min. M <sub>x</sub>	14	-1487.82	0.15	-18.43
	Min. M <sub>z</sub>	20	-1424.42	17.37	-0.15
	Min. Torsion	18	-4.01	15.12	-9.08

### Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overtuning Moment, M <sub>x</sub> kip-ft	Overtuning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	24.25	-0.00	-0.00	-2.35	1.99	-0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	29.10	0.15	-17.60	-1466.23	-13.58	-1.25
0.9 Dead+1.0 Wind 0 deg - No Ice	21.83	0.15	-17.60	-1447.52	-14.02	-1.19
1.2 Dead+1.0 Wind 30 deg - No Ice	29.10	8.67	-15.32	-1278.16	-720.33	-3.01
0.9 Dead+1.0 Wind 30 deg - No Ice	21.83	8.67	-15.32	-1261.75	-712.08	-2.94
1.2 Dead+1.0 Wind 60 deg - No Ice	29.10	15.42	-9.25	-760.17	-1253.83	-3.98
0.9 Dead+1.0 Wind 60 deg - No Ice	21.83	15.42	-9.25	-750.18	-1239.13	-3.91
1.2 Dead+1.0 Wind 90 deg - No Ice	29.10	17.10	-0.15	-18.84	-1416.47	-3.89
0.9 Dead+1.0 Wind 90 deg - No Ice	21.83	17.10	-0.15	-17.88	-1399.67	-3.85
1.2 Dead+1.0 Wind 120 deg - No Ice	29.10	14.74	8.68	715.55	-1218.37	-2.76
0.9 Dead+1.0 Wind 120 deg - No Ice	21.83	14.74	8.68	707.50	-1204.02	-2.76
1.2 Dead+1.0 Wind 150 deg - No Ice	29.10	8.42	15.19	1257.38	-693.12	-0.89
0.9 Dead+1.0 Wind 150 deg - No Ice	21.83	8.42	15.19	1242.70	-685.23	-0.93

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
1.2 Dead+1.0 Wind 180 deg - No Ice	29.10	-0.15	18.43	1487.82	18.44	1.23
0.9 Dead+1.0 Wind 180 deg - No Ice	21.83	-0.15	18.43	1470.46	17.57	1.16
1.2 Dead+1.0 Wind 210 deg - No Ice	29.10	-8.82	15.57	1275.98	727.20	3.02
0.9 Dead+1.0 Wind 210 deg - No Ice	21.83	-8.82	15.57	1261.08	717.64	2.95
1.2 Dead+1.0 Wind 240 deg - No Ice	29.10	-15.12	9.08	744.77	1241.83	4.01
0.9 Dead+1.0 Wind 240 deg - No Ice	21.83	-15.12	9.08	736.36	1225.96	3.95
1.2 Dead+1.0 Wind 270 deg - No Ice	29.10	-17.37	0.15	13.18	1424.42	3.91
0.9 Dead+1.0 Wind 270 deg - No Ice	21.83	-17.37	0.15	13.72	1406.31	3.87
1.2 Dead+1.0 Wind 300 deg - No Ice	29.10	-15.41	-9.07	-733.92	1245.23	2.75
0.9 Dead+1.0 Wind 300 deg - No Ice	21.83	-15.41	-9.07	-724.28	1229.42	2.75
1.2 Dead+1.0 Wind 330 deg - No Ice	29.10	-8.41	-15.16	-1262.24	697.51	0.86
0.9 Dead+1.0 Wind 330 deg - No Ice	21.83	-8.41	-15.16	-1246.04	688.31	0.90
1.2 Dead+1.0 Ice+1.0 Temp	52.64	-0.00	-0.00	-10.17	6.76	-0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	52.64	0.02	-4.17	-401.36	4.40	-0.66
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	52.64	2.07	-3.62	-350.14	-187.59	-1.05
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	52.64	3.64	-2.15	-210.06	-331.37	-1.17
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	52.64	4.10	-0.02	-12.56	-378.02	-0.97
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	52.64	3.54	2.07	183.43	-325.28	-0.51
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	52.64	2.03	3.61	327.55	-183.57	0.09
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	52.64	-0.02	4.31	386.14	9.14	0.66
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	52.64	-2.08	3.65	330.36	201.47	1.05
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	52.64	-3.59	2.12	187.80	341.64	1.16
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	52.64	-4.13	0.02	-7.82	392.08	0.96
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	52.64	-3.65	-2.13	-206.22	342.99	0.50
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	52.64	-2.03	-3.60	-347.77	197.02	-0.09
Dead+Wind 0 deg - Service	24.25	0.03	-3.82	-318.50	-1.44	-0.27
Dead+Wind 30 deg - Service	24.25	1.88	-3.33	-277.87	-154.13	-0.65
Dead+Wind 60 deg - Service	24.25	3.35	-2.01	-165.97	-269.39	-0.87
Dead+Wind 90 deg - Service	24.25	3.72	-0.03	-5.82	-304.51	-0.85
Dead+Wind 120 deg - Service	24.25	3.20	1.89	152.82	-261.72	-0.61
Dead+Wind 150 deg - Service	24.25	1.83	3.30	269.87	-148.26	-0.20
Dead+Wind 180 deg - Service	24.25	-0.03	4.01	319.67	5.46	0.26
Dead+Wind 210 deg - Service	24.25	-1.92	3.38	273.90	158.59	0.65
Dead+Wind 240 deg - Service	24.25	-3.29	1.97	159.13	269.76	0.87
Dead+Wind 270 deg - Service	24.25	-3.78	0.03	1.08	309.19	0.85
Dead+Wind 300 deg - Service	24.25	-3.35	-1.97	-160.30	270.49	0.60
Dead+Wind 330 deg - Service	24.25	-1.83	-3.30	-274.43	152.17	0.20

### Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-24.25	0.00	0.00	24.25	0.00	0.000%
2	0.15	-29.10	-17.60	-0.15	29.10	17.60	0.000%
3	0.15	-21.83	-17.60	-0.15	21.83	17.60	0.000%
4	8.67	-29.10	-15.32	-8.67	29.10	15.32	0.000%
5	8.67	-21.83	-15.32	-8.67	21.83	15.32	0.000%
6	15.42	-29.10	-9.25	-15.42	29.10	9.25	0.000%
7	15.42	-21.83	-9.25	-15.42	21.83	9.25	0.000%
8	17.10	-29.10	-0.15	-17.10	29.10	0.15	0.000%
9	17.10	-21.83	-0.15	-17.10	21.83	0.15	0.000%
10	14.74	-29.10	8.68	-14.74	29.10	-8.68	0.000%
11	14.74	-21.83	8.68	-14.74	21.83	-8.68	0.000%
12	8.42	-29.10	15.19	-8.42	29.10	-15.19	0.000%
13	8.42	-21.83	15.19	-8.42	21.83	-15.19	0.000%
14	-0.15	-29.10	18.43	0.15	29.10	-18.43	0.000%
15	-0.15	-21.83	18.43	0.15	21.83	-18.43	0.000%
16	-8.82	-29.10	15.57	8.82	29.10	-15.57	0.000%
17	-8.82	-21.83	15.57	8.82	21.83	-15.57	0.000%
18	-15.12	-29.10	9.08	15.12	29.10	-9.08	0.000%
19	-15.12	-21.83	9.08	15.12	21.83	-9.08	0.000%
20	-17.37	-29.10	0.15	17.37	29.10	-0.15	0.000%
21	-17.37	-21.83	0.15	17.37	21.83	-0.15	0.000%
22	-15.41	-29.10	-9.07	15.41	29.10	9.07	0.000%
23	-15.41	-21.83	-9.07	15.41	21.83	9.07	0.000%
24	-8.41	-29.10	-15.16	8.41	29.10	15.16	0.000%
25	-8.41	-21.83	-15.16	8.41	21.83	15.16	0.000%
26	0.00	-52.64	0.00	0.00	52.64	0.00	0.000%
27	0.02	-52.64	-4.17	-0.02	52.64	4.17	0.000%
28	2.07	-52.64	-3.62	-2.07	52.64	3.62	0.000%
29	3.64	-52.64	-2.15	-3.64	52.64	2.15	0.000%
30	4.10	-52.64	-0.02	-4.10	52.64	0.02	0.000%
31	3.54	-52.64	2.07	-3.54	52.64	-2.07	0.000%
32	2.03	-52.64	3.61	-2.03	52.64	-3.61	0.000%
33	-0.02	-52.64	4.31	0.02	52.64	-4.31	0.000%
34	-2.08	-52.64	3.65	2.08	52.64	-3.65	0.000%
35	-3.59	-52.64	2.12	3.59	52.64	-2.12	0.000%
36	-4.13	-52.64	0.02	4.13	52.64	-0.02	0.000%
37	-3.65	-52.64	-2.13	3.65	52.64	2.13	0.000%
38	-2.03	-52.64	-3.60	2.03	52.64	3.60	0.000%
39	0.03	-24.25	-3.82	-0.03	24.25	3.82	0.000%
40	1.88	-24.25	-3.33	-1.88	24.25	3.33	0.000%
41	3.35	-24.25	-2.01	-3.35	24.25	2.01	0.000%
42	3.72	-24.25	-0.03	-3.72	24.25	0.03	0.000%
43	3.20	-24.25	1.89	-3.20	24.25	-1.89	0.000%
44	1.83	-24.25	3.30	-1.83	24.25	-3.30	0.000%
45	-0.03	-24.25	4.01	0.03	24.25	-4.01	0.000%
46	-1.92	-24.25	3.38	1.92	24.25	-3.38	0.000%
47	-3.29	-24.25	1.97	3.29	24.25	-1.97	0.000%
48	-3.78	-24.25	0.03	3.78	24.25	-0.03	0.000%
49	-3.35	-24.25	-1.97	3.35	24.25	1.97	0.000%
50	-1.83	-24.25	-3.30	1.83	24.25	3.30	0.000%

### Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00001013
2	Yes	5	0.00000001	0.00088295
3	Yes	5	0.00000001	0.00038381
4	Yes	6	0.00000001	0.00068034

5	Yes	6	0.00000001	0.00021377
6	Yes	6	0.00000001	0.00089156
7	Yes	6	0.00000001	0.00028823
8	Yes	6	0.00000001	0.00021979
9	Yes	6	0.00000001	0.00007463
10	Yes	6	0.00000001	0.00064786
11	Yes	6	0.00000001	0.00020693
12	Yes	6	0.00000001	0.00073410
13	Yes	6	0.00000001	0.00023760
14	Yes	6	0.00000001	0.00009504
15	Yes	5	0.00000001	0.00071564
16	Yes	6	0.00000001	0.00085626
17	Yes	6	0.00000001	0.00027555
18	Yes	6	0.00000001	0.00067320
19	Yes	6	0.00000001	0.00021196
20	Yes	6	0.00000001	0.00017851
21	Yes	6	0.00000001	0.00006069
22	Yes	6	0.00000001	0.00082098
23	Yes	6	0.00000001	0.00026568
24	Yes	6	0.00000001	0.00070611
25	Yes	6	0.00000001	0.00022427
26	Yes	5	0.00000001	0.00057919
27	Yes	7	0.00000001	0.00020502
28	Yes	7	0.00000001	0.00023518
29	Yes	7	0.00000001	0.00024568
30	Yes	7	0.00000001	0.00018670
31	Yes	7	0.00000001	0.00020892
32	Yes	7	0.00000001	0.00021086
33	Yes	7	0.00000001	0.00018694
34	Yes	7	0.00000001	0.00024109
35	Yes	7	0.00000001	0.00023198
36	Yes	7	0.00000001	0.00020418
37	Yes	7	0.00000001	0.00025658
38	Yes	7	0.00000001	0.00025300
39	Yes	5	0.00000001	0.00006107
40	Yes	5	0.00000001	0.00015343
41	Yes	5	0.00000001	0.00028293
42	Yes	5	0.00000001	0.00016584
43	Yes	5	0.00000001	0.00013781
44	Yes	5	0.00000001	0.00016567
45	Yes	5	0.00000001	0.00006705
46	Yes	5	0.00000001	0.00025329
47	Yes	5	0.00000001	0.00017480
48	Yes	5	0.00000001	0.00016398
49	Yes	5	0.00000001	0.00024078
50	Yes	5	0.00000001	0.00015294

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	110 - 105	16.650	39	1.6107	0.0204
L2	105 - 100	14.981	39	1.5703	0.0201
L3	100 - 93	13.374	39	1.4939	0.0199
L4	95 - 90	11.868	39	1.3781	0.0154
L5	90 - 85	10.468	39	1.2799	0.0128
L6	85 - 80	9.197	39	1.1465	0.0102
L7	80 - 79.75	8.070	39	1.0026	0.0077
L8	79.75 - 74.75	8.018	39	0.9991	0.0076
L9	74.75 - 69.75	7.011	39	0.9239	0.0065
L10	69.75 - 64.75	6.085	39	0.8442	0.0056
L11	64.75 - 64.25	5.246	39	0.7581	0.0046
L12	64.25 - 64	5.167	39	0.7495	0.0045
L13	64 - 59	5.128	39	0.7466	0.0045
L14	59 - 56.5	4.377	39	0.6862	0.0038
L15	56.5 - 56.25	4.026	39	0.6550	0.0036
L16	56.25 - 51.25	3.992	39	0.6526	0.0035

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L17	51.25 - 45.5	3.335	39	0.6026	0.0031
L18	49.5 - 44.5	3.117	39	0.5850	0.0030
L19	44.5 - 39.5	2.520	39	0.5493	0.0027
L20	39.5 - 37.25	1.979	39	0.4840	0.0023
L21	37.25 - 37	1.758	39	0.4545	0.0021
L22	37 - 36.5	1.734	39	0.4513	0.0021
L23	36.5 - 36.25	1.687	39	0.4448	0.0020
L24	36.25 - 34.25	1.664	39	0.4423	0.0020
L25	34.25 - 34	1.483	39	0.4214	0.0019
L26	34 - 29	1.461	39	0.4183	0.0019
L27	29 - 24	1.057	39	0.3549	0.0015
L28	24 - 19	0.718	39	0.2919	0.0012
L29	19 - 14	0.445	45	0.2294	0.0009
L30	14 - 9	0.238	45	0.1675	0.0006
L31	9 - 4	0.095	45	0.1051	0.0004
L32	4 - 1.17	0.017	45	0.0441	0.0002
L33	1.17 - 0.92	0.001	45	0.0096	0.0000
L34	0.92 - 0	0.001	45	0.0076	0.0000

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
110.00	Lightning Rod 5/8"x4'	39	16.650	1.6107	0.0205	4538
100.00	Platform Mount [10.75' LP 712-1]	39	13.374	1.4939	0.0200	2918
83.00	Side Arm Mount [SO 101-1]	39	8.728	1.0794	0.0090	2097
69.00	Side Arm Mount [SO 201-1]	39	5.953	0.8320	0.0054	3428

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	110 - 105	76.283	14	7.3680	0.0920
L2	105 - 100	68.706	14	7.1778	0.0910
L3	100 - 93	61.420	14	6.8221	0.0904
L4	95 - 90	54.580	14	6.3101	0.0700
L5	90 - 85	48.207	14	5.8674	0.0585
L6	85 - 80	42.403	14	5.2605	0.0463
L7	80 - 79.75	37.250	14	4.6036	0.0352
L8	79.75 - 74.75	37.010	14	4.5877	0.0349
L9	74.75 - 69.75	32.393	14	4.2435	0.0300
L10	69.75 - 64.75	28.142	14	3.8799	0.0256
L11	64.75 - 64.25	24.284	14	3.4918	0.0211
L12	64.25 - 64	23.921	14	3.4528	0.0206
L13	64 - 59	23.741	14	3.4396	0.0205
L14	59 - 56.5	20.283	14	3.1663	0.0177
L15	56.5 - 56.25	18.663	14	3.0249	0.0163
L16	56.25 - 51.25	18.505	14	3.0140	0.0162
L17	51.25 - 45.5	15.470	14	2.7862	0.0143
L18	49.5 - 44.5	14.463	14	2.7058	0.0137
L19	44.5 - 39.5	11.702	14	2.5425	0.0125
L20	39.5 - 37.25	9.197	14	2.2430	0.0105
L21	37.25 - 37	8.172	14	2.1077	0.0097
L22	37 - 36.5	8.062	14	2.0928	0.0096
L23	36.5 - 36.25	7.845	14	2.0631	0.0094
L24	36.25 - 34.25	7.737	14	2.0513	0.0093
L25	34.25 - 34	6.898	14	1.9555	0.0088
L26	34 - 29	6.796	14	1.9412	0.0087
L27	29 - 24	4.917	14	1.6486	0.0071



Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L28	24 - 19	3.344	14	1.3575	0.0056
L29	19 - 14	2.074	14	1.0680	0.0042
L30	14 - 9	1.107	14	0.7804	0.0030
L31	9 - 4	0.443	14	0.4894	0.0018
L32	4 - 1.17	0.080	14	0.2054	0.0007
L33	1.17 - 0.92	0.005	14	0.0449	0.0002
L34	0.92 - 0	0.003	14	0.0352	0.0001

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
110.00	Lightning Rod 5/8"x4'	14	76.283	7.3680	0.0958	1043
100.00	Platform Mount [10.75' LP 712-1]	14	61.420	6.8221	0.0930	676
83.00	Side Arm Mount [SO 101-1]	14	40.262	4.9546	0.0414	467
69.00	Side Arm Mount [SO 201-1]	14	27.538	3.8249	0.0250	756

### Compression Checks

### Pole Design Data

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub>
L1	110 - 105 (1)	TP13.901x13.051x0.1875	5.00	0.00	0.0	8.1613	-3.79	477.43	0.008
L2	105 - 100 (2)	TP14.751x13.901x0.1875	5.00	0.00	0.0	8.6671	-4.02	507.03	0.008
L3	100 - 93 (3)	TP15.941x14.751x0.1875	7.00	0.00	0.0	9.1730	-6.48	536.62	0.012
L4	93 - 90 (4)	TP16.1139x15.226x0.25	5.00	0.00	0.0	12.588	-6.93	736.40	0.009
L5	90 - 85 (5)	TP17.0018x16.1139x0.25	5.00	0.00	0.0	13.292 5	-7.35	777.61	0.009
L6	85 - 80 (6)	TP17.8896x17.0018x0.25	5.00	0.00	0.0	13.997 1	-8.91	818.83	0.011
L7	80 - 79.75 (7)	TP17.934x17.8896x0.562 5	0.25	0.00	0.0	31.014 7	-8.96	1814.36	0.005
L8	79.75 - 74.75 (8)	TP18.8219x17.934x0.537 5	5.00	0.00	0.0	31.193 7	-9.65	1824.83	0.005
L9	74.75 - 69.75 (9)	TP19.7098x18.8219x0.52 5	5.00	0.00	0.0	31.968 6	-10.38	1870.16	0.006
L10	69.75 - 64.75 (10)	TP20.5977x19.7098x0.5 0	5.00	0.00	0.0	31.895 0	-11.55	1865.86	0.006
L11	64.75 - 64.25 (11)	TP20.6865x20.5977x0.5 9	0.50	0.00	0.0	32.035 9	-11.64	1874.10	0.006
L12	64.25 - 64 (12)	TP20.7308x20.6865x0.77 5	0.25	0.00	0.0	49.088 4	-11.69	2871.67	0.004
L13	64 - 59 (13)	TP21.6187x20.7308x0.75 0	5.00	0.00	0.0	49.678 0	-12.69	2906.16	0.004
L14	59 - 56.5 (14)	TP22.0627x21.6187x0.72 5	2.50	0.00	0.0	49.101 2	-13.21	2872.42	0.005
L15	56.5 - 56.25 (15)	TP22.1071x22.0627x0.97 5	0.25	0.00	0.0	65.396 3	-13.28	3825.69	0.003
L16	56.25 - 51.25 (16)	TP22.9949x22.1071x0.92 5	5.00	0.00	0.0	64.796 2	-14.52	3790.58	0.004
L17	51.25 - 45.5 (17)	TP24.016x22.9949x0.912 5	5.75	0.00	0.0	64.856 9	-14.96	3794.13	0.004
L18	45.5 - 44.5 (18)	TP23.7093x22.8057x0.72 5	5.00	0.00	0.0	52.890 2	-16.96	3094.08	0.005

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub>
L19	44.5 - 39.5 (19)	TP24.6128x23.7093x0.71 25	5.00	0.00	0.0	54.050 0	-18.09	3161.92	0.006
L20	39.5 - 37.25 (20)	TP25.0194x24.6128x0.7	2.25	0.00	0.0	54.032 9	-18.59	3160.93	0.006
L21	37.25 - 37 (21)	TP25.0646x25.0194x0.7	0.25	0.00	0.0	54.133 3	-18.67	3166.80	0.006
L22	37 - 36.5 (22)	TP25.155x25.0646x0.7	0.50	0.00	0.0	54.334 0	-18.80	3178.54	0.006
L23	36.5 - 36.25 (23)	TP25.2001x25.155x0.912 5	0.25	0.00	0.0	70.343 7	-18.87	4115.11	0.005
L24	36.25 - 34.25 (24)	TP25.5616x25.2001x0.88 75	2.00	0.00	0.0	69.505 0	-19.46	4066.04	0.005
L25	34.25 - 34 (25)	TP25.6068x25.5616x0.72 5	0.25	0.00	0.0	57.256 6	-19.53	3349.51	0.006
L26	34 - 29 (26)	TP26.5103x25.6068x0.7	5.00	0.00	0.0	57.345 4	-20.86	3354.70	0.006
L27	29 - 24 (27)	TP27.4139x26.5103x0.68 75	5.00	0.00	0.0	58.320 3	-22.23	3411.74	0.007
L28	24 - 19 (28)	TP28.3174x27.4139x0.67 5	5.00	0.00	0.0	59.222 6	-23.61	3464.52	0.007
L29	19 - 14 (29)	TP29.221x28.3174x0.662 5	5.00	0.00	0.0	60.052 1	-25.02	3513.05	0.007
L30	14 - 9 (30)	TP30.1246x29.221x0.637 5	5.00	0.00	0.0	59.664 9	-26.45	3490.40	0.008
L31	9 - 4 (31)	TP31.0281x30.1246x0.63 75	5.00	0.00	0.0	61.493 2	-27.90	3597.35	0.008
L32	4 - 1.17 (32)	TP31.5396x31.0281x0.62 5	2.83	0.00	0.0	61.326 8	-28.73	3587.62	0.008
L33	1.17 - 0.92 (33)	TP31.5847x31.5396x0.93 75	0.25	0.00	0.0	91.194 7	-28.82	5334.89	0.005
L34	0.92 - 0 (34)	TP31.751x31.5847x0.937 5	0.92	0.00	0.0	91.689 4	-29.10	5363.83	0.005

### Pole Bending Design Data

Section No.	Elevation ft	Size	M <sub>ux</sub> kip-ft	φM <sub>nx</sub> kip-ft	Ratio M <sub>ux</sub> / φM <sub>nx</sub>	M <sub>uy</sub> kip-ft	φM <sub>ny</sub> kip-ft	Ratio M <sub>uy</sub> / φM <sub>ny</sub>
L1	110 - 105 (1)	TP13.901x13.051x0.1875	39.31	170.00	0.231	0.00	170.00	0.000
L2	105 - 100 (2)	TP14.751x13.901x0.1875	69.38	191.88	0.362	0.00	191.88	0.000
L3	100 - 93 (3)	TP15.941x14.751x0.1875	115.37	215.09	0.536	0.00	215.09	0.000
L4	93 - 90 (4)	TP16.1139x15.226x0.25	161.97	302.71	0.535	0.00	302.71	0.000
L5	90 - 85 (5)	TP17.0018x16.1139x0.25	209.90	337.82	0.621	0.00	337.82	0.000
L6	85 - 80 (6)	TP17.8896x17.0018x0.25	264.06	374.86	0.704	0.00	374.86	0.000
L7	80 - 79.75 (7)	TP17.934x17.8896x0.562 5	266.99	803.56	0.332	0.00	803.56	0.000
L8	79.75 - 74.75 (8)	TP18.8219x17.934x0.537 5	326.31	853.13	0.382	0.00	853.13	0.000
L9	74.75 - 69.75 (9)	TP19.7098x18.8219x0.52 5	387.32	919.19	0.421	0.00	919.19	0.000
L10	69.75 - 64.75 (10)	TP20.5977x19.7098x0.5	454.10	963.05	0.472	0.00	963.05	0.000
L11	64.75 - 64.25 (11)	TP20.6865x20.5977x0.5	460.76	971.68	0.474	0.00	971.68	0.000
L12	64.25 - 64 (12)	TP20.7308x20.6865x0.77 5	464.10	1451.96	0.320	0.00	1451.96	0.000
L13	64 - 59 (13)	TP21.6187x20.7308x0.75	531.84	1540.92	0.345	0.00	1540.92	0.000
L14	59 - 56.5 (14)	TP22.0627x21.6187x0.72 5	566.42	1560.20	0.363	0.00	1560.20	0.000
L15	56.5 - 56.25 (15)	TP22.1071x22.0627x0.97 5	569.90	2034.03	0.280	0.00	2034.03	0.000
L16	56.25 - 51.25 (16)	TP22.9949x22.1071x0.92 5	640.66	2113.35	0.303	0.00	2113.35	0.000
L17	51.25 - 45.5 (17)	TP24.016x22.9949x0.912 5	665.90	2148.71	0.310	0.00	2148.71	0.000

Section No.	Elevation ft	Size	$M_{ux}$	$\phi M_{nx}$	Ratio	$M_{uy}$	$\phi M_{ny}$	Ratio
			kip-ft	kip-ft	$\frac{M_{ux}}{\phi M_{nx}}$	kip-ft	kip-ft	$\frac{M_{uy}}{\phi M_{ny}}$
L18	45.5 - 44.5 (18)	TP23.7093x22.8057x0.725	739.53	1814.56	0.408	0.00	1814.56	0.000
L19	44.5 - 39.5 (19)	TP24.6128x23.7093x0.7125	815.05	1931.50	0.422	0.00	1931.50	0.000
L20	39.5 - 37.25 (20)	TP25.0194x24.6128x0.7	849.74	1966.71	0.432	0.00	1966.71	0.000
L21	37.25 - 37 (21)	TP25.0646x25.0194x0.7	853.67	1974.13	0.432	0.00	1974.13	0.000
L22	37 - 36.5 (22)	TP25.155x25.0646x0.7	861.56	1989.00	0.433	0.00	1989.00	0.000
L23	36.5 - 36.25 (23)	TP25.2001x25.155x0.9125	865.51	2535.39	0.341	0.00	2535.39	0.000
L24	36.25 - 34.25 (24)	TP25.5616x25.2001x0.8875	897.30	2548.96	0.352	0.00	2548.96	0.000
L25	34.25 - 34 (25)	TP25.6068x25.5616x0.725	901.30	2131.50	0.423	0.00	2131.50	0.000
L26	34 - 29 (26)	TP26.5103x25.6068x0.7	982.27	2218.82	0.443	0.00	2218.82	0.000
L27	29 - 24 (27)	TP27.4139x26.5103x0.6875	1065.12	2339.82	0.455	0.00	2339.82	0.000
L28	24 - 19 (28)	TP28.3174x27.4139x0.675	1149.78	2460.58	0.467	0.00	2460.58	0.000
L29	19 - 14 (29)	TP29.221x28.3174x0.6625	1236.25	2580.82	0.479	0.00	2580.82	0.000
L30	14 - 9 (30)	TP30.1246x29.221x0.6375	1324.53	2651.63	0.500	0.00	2651.63	0.000
L31	9 - 4 (31)	TP31.0281x30.1246x0.6375	1414.61	2818.40	0.502	0.00	2818.40	0.000
L32	4 - 1.17 (32)	TP31.5396x31.0281x0.625	1466.39	2861.36	0.512	0.00	2861.36	0.000
L33	1.17 - 0.92 (33)	TP31.5847x31.5396x0.9375	1470.99	4175.68	0.352	0.00	4175.68	0.000
L34	0.92 - 0 (34)	TP31.751x31.5847x0.9375	1487.93	4221.78	0.352	0.00	4221.78	0.000

### Pole Shear Design Data

Section No.	Elevation ft	Size	Actual	$\phi V_n$	Ratio	Actual	$\phi T_n$	Ratio
			$V_u$ K	K	$\frac{V_u}{\phi V_n}$	$T_u$ kip-ft	kip-ft	$\frac{T_u}{\phi T_n}$
L1	110 - 105 (1)	TP13.901x13.051x0.1875	5.93	143.23	0.041	0.12	172.01	0.001
L2	105 - 100 (2)	TP14.751x13.901x0.1875	6.11	152.11	0.040	0.12	194.00	0.001
L3	100 - 93 (3)	TP15.941x14.751x0.1875	9.26	160.99	0.057	1.42	217.30	0.007
L4	93 - 90 (4)	TP16.1139x15.226x0.25	9.52	220.92	0.043	2.53	306.92	0.008
L5	90 - 85 (5)	TP17.0018x16.1139x0.25	9.67	233.28	0.041	2.53	342.24	0.007
L6	85 - 80 (6)	TP17.8896x17.0018x0.25	11.69	245.65	0.048	3.04	379.48	0.008
L7	80 - 79.75 (7)	TP17.934x17.8896x0.5625	11.70	544.31	0.022	3.04	828.06	0.004
L8	79.75 - 74.75 (8)	TP18.8219x17.934x0.5375	12.04	547.45	0.022	3.03	876.61	0.003
L9	74.75 - 69.75 (9)	TP19.7098x18.8219x0.525	12.38	561.05	0.022	3.03	942.63	0.003
L10	69.75 - 64.75 (10)	TP20.5977x19.7098x0.5	13.31	559.76	0.024	3.03	985.20	0.003
L11	64.75 - 64.25 (11)	TP20.6865x20.5977x0.5	13.34	562.23	0.024	3.03	993.92	0.003
L12	64.25 - 64 (12)	TP20.7308x20.6865x0.775	13.36	861.50	0.016	3.03	1505.59	0.002
L13	64 - 59 (13)	TP21.6187x20.7308x0.75	13.75	871.85	0.016	3.02	1593.38	0.002
L14	59 - 56.5 (14)	TP22.0627x21.6187x0.725	13.94	861.73	0.016	3.02	1610.26	0.002
L15	56.5 - 56.25 (15)	TP22.1071x22.0627x0.975	13.95	1147.71	0.012	3.02	2123.99	0.001
L16	56.25 - 51.25 (16)	TP22.9949x22.1071x0.925	14.36	1137.17	0.013	3.02	2197.90	0.001
L17	51.25 - 45.5 (17)	TP24.016x22.9949x0.9125	14.50	1138.24	0.013	3.02	2232.18	0.001

Section No.	Elevation ft	Size	Actual $V_u$ K	$\phi V_n$ K	Ratio $V_u$ $\phi V_n$	Actual $T_u$ kip-ft	$\phi T_n$ kip-ft	Ratio $T_u$ $\phi T_n$
L18	45.5 - 44.5 (18)	TP23.7093x22.8057x0.725	14.95	928.22	0.016	3.02	1868.38	0.002
L19	44.5 - 39.5 (19)	TP24.6128x23.7093x0.7125	15.28	948.58	0.016	3.02	1985.44	0.002
L20	39.5 - 37.25 (20)	TP25.0194x24.6128x0.7	15.74	948.28	0.017	1.23	2019.62	0.001
L21	37.25 - 37 (21)	TP25.0646x25.0194x0.7	15.75	950.04	0.017	1.23	2027.13	0.001
L22	37 - 36.5 (22)	TP25.155x25.0646x0.7	15.79	953.56	0.017	1.23	2042.19	0.001
L23	36.5 - 36.25 (23)	TP25.2001x25.155x0.9125	15.81	1234.53	0.013	1.23	2625.83	0.000
L24	36.25 - 34.25 (24)	TP25.5616x25.2001x0.8875	15.99	1219.81	0.013	1.23	2635.81	0.000
L25	34.25 - 34 (25)	TP25.6068x25.5616x0.725	16.01	1004.85	0.016	1.23	2189.60	0.001
L26	34 - 29 (26)	TP26.5103x25.6068x0.7	16.40	1006.41	0.016	1.23	2274.83	0.001
L27	29 - 24 (27)	TP27.4139x26.5103x0.6875	16.76	1023.52	0.016	1.23	2395.62	0.001
L28	24 - 19 (28)	TP28.3174x27.4139x0.675	17.13	1039.36	0.016	1.23	2516.06	0.000
L29	19 - 14 (29)	TP29.221x28.3174x0.6625	17.49	1053.91	0.017	1.23	2635.85	0.000
L30	14 - 9 (30)	TP30.1246x29.221x0.6375	17.85	1047.12	0.017	1.23	2704.01	0.000
L31	9 - 4 (31)	TP31.0281x30.1246x0.6375	18.21	1079.21	0.017	1.23	2872.27	0.000
L32	4 - 1.17 (32)	TP31.5396x31.0281x0.625	18.41	1076.28	0.017	1.23	2913.88	0.000
L33	1.17 - 0.92 (33)	TP31.5847x31.5396x0.9375	18.41	1600.47	0.012	1.23	4295.54	0.000
L34	0.92 - 0 (34)	TP31.751x31.5847x0.9375	18.44	1609.15	0.011	1.23	4342.27	0.000

### Pole Interaction Design Data

Section No.	Elevation ft	Ratio $P_u$ $\phi P_n$	Ratio $M_{ux}$ $\phi M_{nx}$	Ratio $M_{uy}$ $\phi M_{ny}$	Ratio $V_u$ $\phi V_n$	Ratio $T_u$ $\phi T_n$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	110 - 105 (1)	0.008	0.231	0.000	0.041	0.001	0.241	1.050	4.8.2
L2	105 - 100 (2)	0.008	0.362	0.000	0.040	0.001	0.371	1.050	4.8.2
L3	100 - 93 (3)	0.012	0.536	0.000	0.057	0.007	0.553	1.050	4.8.2
L4	93 - 90 (4)	0.009	0.535	0.000	0.043	0.008	0.547	1.050	4.8.2
L5	90 - 85 (5)	0.009	0.621	0.000	0.041	0.007	0.633	1.050	4.8.2
L6	85 - 80 (6)	0.011	0.704	0.000	0.048	0.008	0.718	1.050	4.8.2
L7	80 - 79.75 (7)	0.005	0.332	0.000	0.022	0.004	0.338	1.050	4.8.2
L8	79.75 - 74.75 (8)	0.005	0.382	0.000	0.022	0.003	0.388	1.050	4.8.2
L9	74.75 - 69.75 (9)	0.006	0.421	0.000	0.022	0.003	0.428	1.050	4.8.2
L10	69.75 - 64.75 (10)	0.006	0.472	0.000	0.024	0.003	0.478	1.050	4.8.2
L11	64.75 - 64.25 (11)	0.006	0.474	0.000	0.024	0.003	0.481	1.050	4.8.2
L12	64.25 - 64 (12)	0.004	0.320	0.000	0.016	0.002	0.324	1.050	4.8.2
L13	64 - 59 (13)	0.004	0.345	0.000	0.016	0.002	0.350	1.050	4.8.2
L14	59 - 56.5 (14)	0.005	0.363	0.000	0.016	0.002	0.368	1.050	4.8.2
L15	56.5 - 56.25 (15)	0.003	0.280	0.000	0.012	0.001	0.284	1.050	4.8.2
L16	56.25 - 51.25 (16)	0.004	0.303	0.000	0.013	0.001	0.307	1.050	4.8.2
L17	51.25 - 45.5 (17)	0.004	0.310	0.000	0.013	0.001	0.314	1.050	4.8.2

Section No.	Elevation ft	Ratio	Ratio	Ratio	Ratio	Ratio	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
		$P_u$	$M_{ux}$	$M_{uy}$	$V_u$	$T_u$			
L18	45.5 - 44.5 (18)	0.005	0.408	0.000	0.016	0.002	0.413	1.050	4.8.2
L19	44.5 - 39.5 (19)	0.006	0.422	0.000	0.016	0.002	0.428	1.050	4.8.2
L20	39.5 - 37.25 (20)	0.006	0.432	0.000	0.017	0.001	0.438	1.050	4.8.2
L21	37.25 - 37 (21)	0.006	0.432	0.000	0.017	0.001	0.439	1.050	4.8.2
L22	37 - 36.5 (22)	0.006	0.433	0.000	0.017	0.001	0.439	1.050	4.8.2
L23	36.5 - 36.25 (23)	0.005	0.341	0.000	0.013	0.000	0.346	1.050	4.8.2
L24	36.25 - 34.25 (24)	0.005	0.352	0.000	0.013	0.000	0.357	1.050	4.8.2
L25	34.25 - 34 (25)	0.006	0.423	0.000	0.016	0.001	0.429	1.050	4.8.2
L26	34 - 29 (26)	0.006	0.443	0.000	0.016	0.001	0.449	1.050	4.8.2
L27	29 - 24 (27)	0.007	0.455	0.000	0.016	0.001	0.462	1.050	4.8.2
L28	24 - 19 (28)	0.007	0.467	0.000	0.016	0.000	0.474	1.050	4.8.2
L29	19 - 14 (29)	0.007	0.479	0.000	0.017	0.000	0.486	1.050	4.8.2
L30	14 - 9 (30)	0.008	0.500	0.000	0.017	0.000	0.507	1.050	4.8.2
L31	9 - 4 (31)	0.008	0.502	0.000	0.017	0.000	0.510	1.050	4.8.2
L32	4 - 1.17 (32)	0.008	0.512	0.000	0.017	0.000	0.521	1.050	4.8.2
L33	1.17 - 0.92 (33)	0.005	0.352	0.000	0.012	0.000	0.358	1.050	4.8.2
L34	0.92 - 0 (34)	0.005	0.352	0.000	0.011	0.000	0.358	1.050	4.8.2

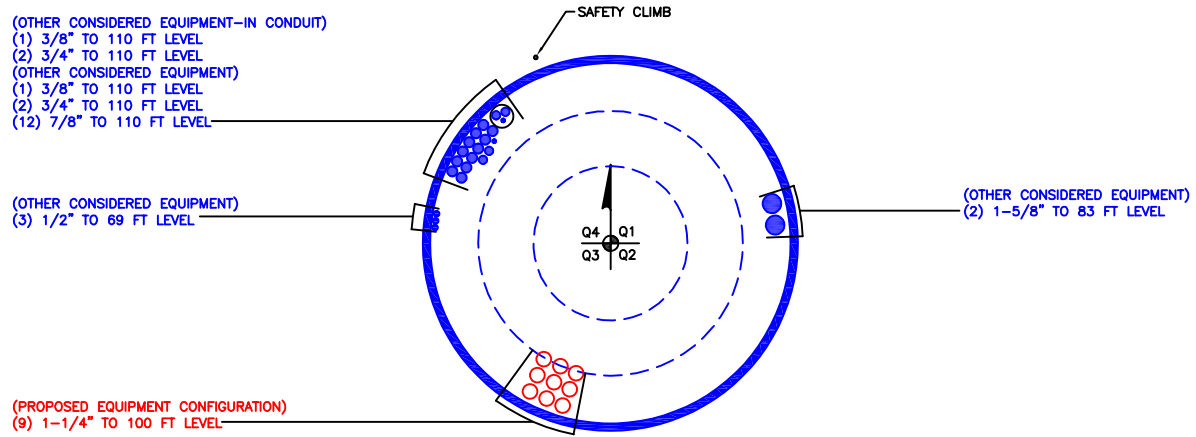
### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
L1	110 - 105	Pole	TP13.901x13.051x0.1875	1	-3.79	501.30	22.9	Pass
L2	105 - 100	Pole	TP14.751x13.901x0.1875	2	-4.02	532.38	35.4	Pass
L3	100 - 93	Pole	TP15.941x14.751x0.1875	3	-6.48	563.45	52.6	Pass
L4	93 - 90	Pole	TP16.1139x15.226x0.25	4	-6.93	773.22	52.1	Pass
L5	90 - 85	Pole	TP17.0018x16.1139x0.25	5	-7.35	816.49	60.3	Pass
L6	85 - 80	Pole	TP17.8896x17.0018x0.25	6	-8.91	859.77	68.4	Pass
L7	80 - 79.75	Pole	TP17.934x17.8896x0.5625	7	-8.96	1905.08	32.2	Pass
L8	79.75 - 74.75	Pole	TP18.8219x17.934x0.5375	8	-9.65	1916.07	37.0	Pass
L9	74.75 - 69.75	Pole	TP19.7098x18.8219x0.525	9	-10.38	1963.67	40.7	Pass
L10	69.75 - 64.75	Pole	TP20.5977x19.7098x0.5	10	-11.55	1959.15	45.6	Pass
L11	64.75 - 64.25	Pole	TP20.6865x20.5977x0.5	11	-11.64	1967.80	45.8	Pass
L12	64.25 - 64	Pole	TP20.7308x20.6865x0.775	12	-11.69	3015.25	30.9	Pass
L13	64 - 59	Pole	TP21.6187x20.7308x0.75	13	-12.69	3051.47	33.3	Pass
L14	59 - 56.5	Pole	TP22.0627x21.6187x0.725	14	-13.21	3016.04	35.0	Pass
L15	56.5 - 56.25	Pole	TP22.1071x22.0627x0.975	15	-13.28	4016.97	27.0	Pass
L16	56.25 - 51.25	Pole	TP22.9949x22.1071x0.925	16	-14.52	3980.11	29.3	Pass
L17	51.25 - 45.5	Pole	TP24.016x22.9949x0.9125	17	-14.96	3983.84	29.9	Pass
L18	45.5 - 44.5	Pole	TP23.7093x22.8057x0.725	18	-16.96	3248.78	39.4	Pass
L19	44.5 - 39.5	Pole	TP24.6128x23.7093x0.7125	19	-18.09	3320.02	40.8	Pass
L20	39.5 - 37.25	Pole	TP25.0194x24.6128x0.7	20	-18.59	3318.98	41.7	Pass
L21	37.25 - 37	Pole	TP25.0646x25.0194x0.7	21	-18.67	3325.14	41.8	Pass
L22	37 - 36.5	Pole	TP25.155x25.0646x0.7	22	-18.80	3337.47	41.8	Pass
L23	36.5 - 36.25	Pole	TP25.2001x25.155x0.9125	23	-18.87	4320.87	33.0	Pass
L24	36.25 - 34.25	Pole	TP25.5616x25.2001x0.8875	24	-19.46	4269.34	34.0	Pass
L25	34.25 - 34	Pole	TP25.6068x25.5616x0.725	25	-19.53	3516.99	40.9	Pass
L26	34 - 29	Pole	TP26.5103x25.6068x0.7	26	-20.86	3522.43	42.8	Pass
L27	29 - 24	Pole	TP27.4139x26.5103x0.6875	27	-22.23	3582.33	44.0	Pass
L28	24 - 19	Pole	TP28.3174x27.4139x0.675	28	-23.61	3637.75	45.2	Pass
L29	19 - 14	Pole	TP29.221x28.3174x0.6625	29	-25.02	3688.70	46.3	Pass
L30	14 - 9	Pole	TP30.1246x29.221x0.6375	30	-26.45	3664.92	48.3	Pass
L31	9 - 4	Pole	TP31.0281x30.1246x0.6375	31	-27.90	3777.22	48.6	Pass
L32	4 - 1.17	Pole	TP31.5396x31.0281x0.625	32	-28.73	3767.00	49.6	Pass

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail	
L33	1.17 - 0.92	Pole	TP31.5847x31.5396x0.9375	33	-28.82	5601.63	34.1	Pass	
L34	0.92 - 0	Pole	TP31.751x31.5847x0.9375	34	-29.10	5632.02	34.1	Pass	
							Summary		
							Pole (L6)	68.4	Pass
							<b>RATING =</b>	<b>68.4</b>	<b>Pass</b>

**\*NOTE: Above stress ratios for reinforced sections are approximate. More exact calculations are presented in Appendix C**

**APPENDIX B**  
**BASE LEVEL DRAWING**





**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

Site BU: 841301  
Work Order: 1749500

**Pole Geometry**

	Pole Height Above Base (ft)	Section Length (ft)	Lap Splice Length (ft)	Number of Sides	Top Diameter (in)	Bottom Diameter (in)	Wall Thickness (in)	Bend Radius (in)	Pole Material
1	110	17	2	18	13.051	15.941	0.1875	Auto	A572-65
2	95	49.5	4	18	15.23	24.016	0.25	Auto	A572-65
3	49.5	49.5	0	18	22.81	31.751	0.3125	Auto	A572-65

**Reinforcement Configuration**

	Bottom Effective Elevation (ft)	Top Effective Elevation (ft)	Type	Model	Number	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
1	46.75	64.25	plate	PL 3.5" x 1.25"	3		E1						E1						E1				
2	1.17	36.5	plate	CCI-SFP-065125	3						E2									E2			E2
3	1.17	37.25	plate	CCI-SFP-065125	1										E2								
4	34.25	56.5	plate	CCI-SFP-065125	1												E2						
5	36.5	56.5	plate	CCI-SFP-065125	2						E2												E2
6	56.5	80	plate	CCI-SFP-045100	3						E2						E2						E2
7	0	1.17	plate	ARB2 (7.25" x 1.25")	3			E2				E2										E2	
8	0	1.17	plate	ARB1 (7.25" x 1.25")	1											E2							
9																							
10																							

**Reinforcement Details**

	B (in)	H (in)	Gross Area (in <sup>2</sup> )	Pole Face to Centroid (in)	Bottom Termination Length (in)	Top Termination Length (in)	L <sub>u</sub> (in)	Net Area (in <sup>2</sup> )	Bolt Hole Size (in)	Reinforcement Material
1	3.5	1.25	4.375	0.625	15.000	15.000	12.000	2.813	1.1875	A572-65
2	6.5	1.25	8.125	0.625	33.000	33.000	19.000	6.563	1.1875	A572-65
3	6.5	1.25	8.125	0.625	33.000	33.000	19.000	6.563	1.1875	A572-65
4	6.5	1.25	8.125	0.625	33.000	33.000	19.000	6.563	1.1875	A572-65
5	6.5	1.25	8.125	0.625	33.000	33.000	19.000	6.563	1.1875	A572-65
6	4.5	1	4.5	0.5	18.000	18.000	20.000	3.250	1.1875	A572-65
7	1.25	7.25	9.0625	3.625	n/a	n/a	0.000	9.063	0.0000	A572-65
8	1.25	7.25	9.0625	3.625	n/a	n/a	0.000	9.063	0.0000	A572-65

# TNX Geometry Input

Increment (ft): 5

	Section Height (ft)	Section Length (ft)	Lap Splice Length (ft)	Number of Sides	Top Diameter (in)	Bottom Diameter (in)	Wall Thickness (in)	Tapered Pole Grade	Weight Multiplier
1	110 - 105	5		18	13.051	13.901	0.1875	A572-65	1.000
2	105 - 100	5		18	13.901	14.751	0.1875	A572-65	1.000
3	100 - 95	7	2	18	14.751	15.941	0.1875	A572-65	1.000
4	95 - 90	5		18	15.226	16.114	0.25	A572-65	1.000
5	90 - 85	5		18	16.114	17.002	0.25	A572-65	1.000
6	85 - 80	5		18	17.002	17.890	0.25	A572-65	1.000
7	80 - 79.75	0.25		18	17.890	17.934	0.5625	A572-65	0.888
8	79.75 - 74.75	5		18	17.934	18.822	0.5375	A572-65	0.905
9	74.75 - 69.75	5		18	18.822	19.710	0.525	A572-65	0.905
10	69.75 - 64.75	5		18	19.710	20.598	0.5	A572-65	0.929
11	64.75 - 64.25	0.5		18	20.598	20.686	0.5	A572-65	0.928
12	64.25 - 64	0.25		18	20.686	20.731	0.775	A572-65	0.873
13	64 - 59	5		18	20.731	21.619	0.75	A572-65	0.877
14	59 - 56.5	2.5		18	21.619	22.063	0.725	A572-65	0.895
15	56.5 - 56.25	0.25		18	22.063	22.107	0.975	A572-65	0.839
16	56.25 - 51.25	5		18	22.107	22.995	0.925	A572-65	0.857
17	51.25 - 49.5	5.75	4	18	22.995	24.016	0.9125	A572-65	0.860
18	49.5 - 44.5	5		18	22.806	23.709	0.725	A572-65	0.900
19	44.5 - 39.5	5		18	23.709	24.613	0.7125	A572-65	0.897
20	39.5 - 37.25	2.25		18	24.613	25.019	0.7	A572-65	0.905
21	37.25 - 37	0.25		18	25.019	25.065	0.7	A572-65	1.054
22	37 - 36.5	0.5		18	25.065	25.155	0.7	A572-65	1.052
23	36.5 - 36.25	0.25		18	25.155	25.200	0.9125	A572-65	0.928
24	36.25 - 34.25	2		18	25.200	25.562	0.8875	A572-65	0.945
25	34.25 - 34	0.25		18	25.562	25.607	0.725	A572-65	1.006
26	34 - 29	5		18	25.607	26.510	0.7	A572-65	1.020
27	29 - 24	5		18	26.510	27.414	0.6875	A572-65	1.018
28	24 - 19	5		18	27.414	28.317	0.675	A572-65	1.018
29	19 - 14	5		18	28.317	29.221	0.6625	A572-65	1.019
30	14 - 9	5		18	29.221	30.125	0.6375	A572-65	1.040
31	9 - 4	5		18	30.125	31.028	0.6375	A572-65	1.024
32	4 - 1.17	2.83		18	31.028	31.540	0.625	A572-65	1.035
33	1.17 - 0.92	0.25		18	31.540	31.585	0.9375	A572-65	0.738
34	0.92 - 0	0.92		18	31.585	31.751	0.9375	A572-65	0.735

## TNX Section Forces

Increment (ft):		TNX Output			
	5	Section Height (ft)	P <sub>u</sub> (K)	M <sub>ux</sub> (kip-ft)	V <sub>u</sub> (K)
1		110 - 105	3.79	39.31	5.93
2		105 - 100	4.02	69.38	6.11
3		100 - 95	6.48	115.37	9.26
4		95 - 90	6.94	162.08	9.43
5		90 - 85	7.35	209.89	9.67
6		85 - 80	8.91	264.06	11.69
7		80 - 79.75	8.96	266.99	11.70
8		79.75 - 74.75	9.65	326.31	12.04
9		74.75 - 69.75	10.38	387.32	12.38
10		69.75 - 64.75	11.55	454.10	13.31
11		64.75 - 64.25	11.64	460.76	13.34
12		64.25 - 64	11.69	464.10	13.36
13		64 - 59	12.69	531.84	13.75
14		59 - 56.5	13.21	566.42	13.94
15		56.5 - 56.25	13.28	569.91	13.95
16		56.25 - 51.25	14.52	640.66	14.36
17		51.25 - 49.5	14.96	665.90	14.50
18		49.5 - 44.5	16.96	739.53	14.95
19		44.5 - 39.5	18.09	815.05	15.28
20		39.5 - 37.25	18.59	849.74	15.74
21		37.25 - 37	18.67	853.68	15.75
22		37 - 36.5	18.80	861.56	15.79
23		36.5 - 36.25	18.87	865.51	15.81
24		36.25 - 34.25	19.46	897.30	15.99
25		34.25 - 34	19.53	901.30	16.01
26		34 - 29	20.86	982.27	16.40
27		29 - 24	22.23	1065.12	16.76
28		24 - 19	23.61	1149.78	17.13
29		19 - 14	25.02	1236.25	17.49
30		14 - 9	26.45	1324.53	17.85
31		9 - 4	27.90	1414.61	18.21
32		4 - 1.17	28.73	1466.39	18.41
33		1.17 - 0.92	28.82	1470.99	18.41
34		0.92 - 0	29.10	1487.93	18.44

# Analysis Results

Elevation (ft)	Component Type	Size	Critical Element	% Capacity	Pass / Fail
110 - 105	Pole	TP13.901x13.051x0.1875	Pole	22.6%	Pass
105 - 100	Pole	TP14.751x13.901x0.1875	Pole	35.0%	Pass
100 - 95	Pole	TP15.941x14.751x0.1875	Pole	52.0%	Pass
95 - 90	Pole	TP16.114x15.226x0.25	Pole	51.7%	Pass
90 - 85	Pole	TP17.002x16.114x0.25	Pole	59.9%	Pass
85 - 80	Pole	TP17.89x17.002x0.25	Pole	67.9%	Pass
80 - 79.75	Pole + Reinf.	TP17.934x17.89x0.5625	Reinf. 6 Tension Rupture	57.5%	Pass
79.75 - 74.75	Pole + Reinf.	TP18.822x17.934x0.5375	Reinf. 6 Tension Rupture	65.3%	Pass
74.75 - 69.75	Pole + Reinf.	TP19.71x18.822x0.525	Reinf. 6 Tension Rupture	72.3%	Pass
69.75 - 64.75	Pole + Reinf.	TP20.598x19.71x0.5	Reinf. 6 Tension Rupture	79.3%	Pass
64.75 - 64.25	Pole + Reinf.	TP20.686x20.598x0.5	Reinf. 6 Tension Rupture	80.0%	Pass
64.25 - 64	Pole + Reinf.	TP20.731x20.686x0.775	Reinf. 1 Tension Rupture	61.6%	Pass
64 - 59	Pole + Reinf.	TP21.619x20.731x0.75	Reinf. 1 Tension Rupture	66.8%	Pass
59 - 56.5	Pole + Reinf.	TP22.063x21.619x0.725	Reinf. 1 Tension Rupture	69.2%	Pass
56.5 - 56.25	Pole + Reinf.	TP22.107x22.063x0.975	Reinf. 1 Tension Rupture	54.3%	Pass
56.25 - 51.25	Pole + Reinf.	TP22.995x22.107x0.925	Reinf. 1 Tension Rupture	58.2%	Pass
51.25 - 49.5	Pole + Reinf.	TP24.016x22.995x0.9125	Reinf. 1 Tension Rupture	59.4%	Pass
49.5 - 44.5	Pole + Reinf.	TP23.709x22.806x0.725	Reinf. 4 Tension Rupture	61.9%	Pass
44.5 - 39.5	Pole + Reinf.	TP24.613x23.709x0.7125	Reinf. 4 Tension Rupture	64.6%	Pass
39.5 - 37.25	Pole + Reinf.	TP25.019x24.613x0.7	Reinf. 4 Tension Rupture	65.8%	Pass
37.25 - 37	Pole + Reinf.	TP25.065x25.019x0.7	Reinf. 5 Tension Rupture	64.4%	Pass
37 - 36.5	Pole + Reinf.	TP25.155x25.065x0.7	Reinf. 5 Tension Rupture	64.7%	Pass
36.5 - 36.25	Pole + Reinf.	TP25.2x25.155x0.9125	Reinf. 2 Tension Rupture	56.6%	Pass
36.25 - 34.25	Pole + Reinf.	TP25.562x25.2x0.8875	Reinf. 2 Tension Rupture	57.5%	Pass
34.25 - 34	Pole + Reinf.	TP25.607x25.562x0.725	Reinf. 3 Tension Rupture	61.0%	Pass
34 - 29	Pole + Reinf.	TP26.51x25.607x0.7	Reinf. 3 Tension Rupture	63.2%	Pass
29 - 24	Pole + Reinf.	TP27.414x26.51x0.6875	Reinf. 3 Tension Rupture	65.3%	Pass
24 - 19	Pole + Reinf.	TP28.317x27.414x0.675	Reinf. 3 Tension Rupture	67.3%	Pass
19 - 14	Pole + Reinf.	TP29.221x28.317x0.6625	Reinf. 3 Tension Rupture	69.1%	Pass
14 - 9	Pole + Reinf.	TP30.125x29.221x0.6375	Reinf. 3 Tension Rupture	70.7%	Pass
9 - 4	Pole + Reinf.	TP31.028x30.125x0.6375	Reinf. 3 Tension Rupture	72.3%	Pass
4 - 1.17	Pole + Reinf.	TP31.54x31.028x0.625	Reinf. 3 Tension Rupture	73.1%	Pass
1.17 - 0.92	Pole + Reinf.	TP31.585x31.54x0.9375	Reinf. 7 Tension Yield	54.8%	Pass
0.92 - 0	Pole + Reinf.	TP31.751x31.585x0.9375	Reinf. 7 Tension Yield	55.1%	Pass
				Summary	
			Pole	67.9%	Pass
			Reinforcement	80.0%	Pass
			Overall	80.0%	Pass

# Additional Calculations

Section Elevation (ft)	Moment of Inertia (in <sup>4</sup> )			Area (in <sup>2</sup> )			% Capacity*								
	Pole	Reinf.	Total	Pole	Reinf.	Total	Pole	R1	R2	R3	R4	R5	R6	R7	R8
110 - 105	194	n/a	194	8.16	n/a	8.16	22.6%								
105 - 100	232	n/a	232	8.67	n/a	8.67	35.0%								
100 - 95	275	n/a	275	9.17	n/a	9.17	52.0%								
95 - 90	400	n/a	400	12.59	n/a	12.59	51.7%								
90 - 85	471	n/a	471	13.29	n/a	13.29	59.9%								
85 - 80	550	n/a	550	14.00	n/a	14.00	67.9%								
80 - 79.75	554	617	1171	14.03	13.50	27.53	32.0%							57.5%	
79.75 - 74.75	642	675	1317	14.74	13.50	28.24	36.4%							65.3%	
74.75 - 69.75	739	736	1474	15.44	13.50	28.94	40.4%							72.3%	
69.75 - 64.75	844	799	1643	16.15	13.50	29.65	44.4%							79.3%	
64.75 - 64.25	855	806	1661	16.22	13.50	29.72	44.8%							80.0%	
64.25 - 64	861	1609	2470	16.25	26.63	42.88	30.4%	61.6%						54.3%	
64 - 59	978	1741	2719	16.96	26.63	43.58	33.0%	66.8%						58.8%	
59 - 56.5	1040	1809	2849	17.31	26.63	43.93	34.2%	69.2%						61.0%	
56.5 - 56.25	1046	2609	3656	17.34	37.50	54.84	26.9%	54.3%			43.3%	43.3%			
56.25 - 51.25	1179	2807	3987	18.05	37.50	55.55	28.9%	58.2%			46.4%	46.4%			
51.25 - 49.5	1228	2879	4107	18.29	37.50	55.79	29.5%	59.4%			47.4%	47.4%			
49.5 - 44.5	1604	1943	3547	23.21	24.38	47.58	38.6%				61.9%	61.9%			
44.5 - 39.5	1798	2083	3880	24.10	24.38	48.48	40.3%				64.6%	64.6%			
39.5 - 37.25	1889	2147	4036	24.51	24.38	48.88	41.1%				65.8%	65.8%			
37.25 - 37	1900	2183	4083	24.55	32.50	57.05	41.4%			43.7%	53.1%	64.4%			
37 - 36.5	1921	2197	4118	24.64	32.50	57.14	41.6%			43.9%	53.3%	64.7%			
36.5 - 36.25	2005	3274	5279	24.68	40.63	65.31	37.0%		56.6%	44.7%	45.5%				
36.25 - 34.25	2093	3363	5456	25.04	40.63	65.67	37.6%		57.5%	45.5%	46.3%				
34.25 - 34	2029	2436	4465	25.09	32.50	57.59	41.6%		60.0%	61.0%					
34 - 29	2254	2598	4852	25.98	32.50	58.48	43.2%		62.3%	63.2%					
29 - 24	2496	2765	5261	26.88	32.50	59.38	44.7%		64.3%	65.3%					
24 - 19	2754	2938	5691	27.78	32.50	60.28	46.0%		66.2%	67.3%					
19 - 14	3029	3116	6145	28.67	32.50	61.17	47.3%		68.0%	69.1%					
14 - 9	3322	3299	6621	29.57	32.50	62.07	48.5%		69.6%	70.7%					
9 - 4	3633	3488	7121	30.46	32.50	62.96	50.0%		71.1%	72.3%					
4 - 1.17	3817	3597	7415	30.97	32.50	63.47	50.8%		71.9%	73.1%					
1.17 - 0.92	3857	7043	10899	31.02	36.25	67.27	36.3%							54.8%	53.7%
0.92 - 0	3918	7103	11021	31.18	36.25	67.43	36.6%							55.1%	54.0%

Note: Section capacity checked in 5 degree increments.  
Rating per TIA-222-H Section 15.5.

# Monopole Base Plate Connection

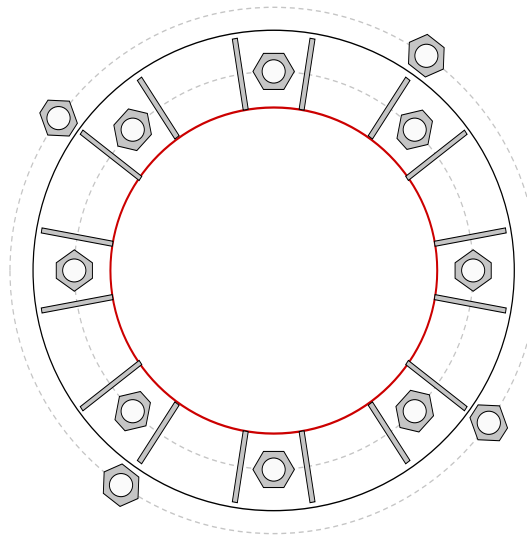


Site Info	
BU #	841301
Site Name	WILLINGTON-RIVER RD
Order #	479824 Rev.0

Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	No
$l_{ar}$ (in)	0.9375

Applied Loads	
Moment (kip-ft)	1487.93
Axial Force (kips)	29.10
Shear Force (kips)	18.44

\*TIA-222-H Section 15.5 Applied



Connection Properties	Analysis Results
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**Anchor Rod Data**  
 GROUP 1: (8) 2-1/4"  $\phi$  bolts (A615-75 N;  $F_y=75$  ksi,  $F_u=100$  ksi) on 38.75" BC  
 GROUP 2: (4) 2-1/4"  $\phi$  bolts (A193 Gr. B7 N;  $F_y=105$  ksi,  $F_u=125$  ksi) on 51.25" BC

**Base Plate Data**  
 46.75" OD x 1.5" Plate (A572-60;  $F_y=60$  ksi,  $F_u=75$  ksi)

**Stiffener Data**  
 (16) 18"H x 7"W x 0.5"T, Notch: 0.5"  
 plate:  $F_y=70$  ksi ; weld:  $F_y=70$  ksi  
 horiz. weld: 0.5" groove, 45° dbl bevel, 0.375" fillet  
 vert. weld: 0.375" fillet

**Pole Data**  
 31.751" x 0.3125" 18-sided pole (A572-65;  $F_y=65$  ksi,  $F_u=80$  ksi)

**Anchor Rod Summary** (units of kips, kip-in)

GROUP 1:		
$P_u_c = 126.4$	$\phi P_{n_c} = 243.75$	<b>Stress Rating</b>
$V_u = 2.31$	$\phi V_n = 73.13$	<b>49.5%</b>
$M_u = n/a$	$\phi M_n = n/a$	<b>Pass</b>

GROUP 2:		
$P_u_c = 162.36$	$\phi P_{n_c} = 341.25$	<b>Stress Rating</b>
$V_u = 0$	$\phi V_n = 102.38$	<b>45.3%</b>
$M_u = n/a$	$\phi M_n = n/a$	<b>Pass</b>

**Base Plate Summary**

Max Stress (ksi):	31.11	(Roark's Flexural)
Allowable Stress (ksi):	54	
Stress Rating:	<b>54.9%</b>	<b>Pass</b>

**Stiffener Summary**

Horizontal Weld:	<b>43.4%</b>	<b>Pass</b>
Vertical Weld:	<b>17.0%</b>	<b>Pass</b>
Plate Flexure+Shear:	<b>6.1%</b>	<b>Pass</b>
Plate Tension+Shear:	<b>21.7%</b>	<b>Pass</b>
Plate Compression:	<b>27.0%</b>	<b>Pass</b>

**Pole Summary**

Punching Shear:	<b>6.0%</b>	<b>Pass</b>
-----------------	-------------	-------------

## Additional Anchor Rod Calculations:

Base Reactions from tnxTower:

$$\text{Moment} := 1494.71 \cdot \text{kip} \cdot \text{ft}$$

$$\text{Axial} := 18.58 \cdot \text{kip}$$

$$\text{Shear} := 18.44 \cdot \text{kip}$$

Apply TIA-222-H Section 15.5?

No  
Yes

Original Anchor Rod Group:

$$N_{\text{existing}} := 8$$

$$BC_{\text{existing}} := 38.75 \cdot \text{in}$$

$$D_{\text{existing}} := 2.25 \cdot \text{in}$$

$$A_{\text{existing}} := 3.25 \cdot \text{in}^2$$

$$F_{u_{\text{existing}}} := 100 \text{ksi}$$

$$F_{y_{\text{existing}}} := 75 \text{ksi}$$

Existing Anchor Rod Group:

$$N_{\text{new}} := 4$$

$$BC_{\text{new}} := 51.25 \cdot \text{in}$$

$$D_{\text{new}} := 2.25 \cdot \text{in}$$

$$A_{\text{new}} := 3.25 \cdot \text{in}^2$$

$$F_{u_{\text{rod}}} := 125 \text{ksi}$$

$$F_{y_{\text{rod}}} := 105 \text{ksi}$$

--See attached CCplate output for additional anchor rod group capacity and structural rating values--





## Anchor Rod Bracket Calculations

Analyze the anchor rod bracket and all components to resist the full demand loading of the additional anchors.

**Bracket Demand Load:  
From CCI Plate**

$$P_u := 163.1 \cdot \text{kip}$$

### Tube Design (Square HSS)

**Member Size:**

$$\text{HSS } 5" \times 5" \times 1/2"$$

**Member Properties  
(AISC 15th Ed., Table 1-12):**

Outside Diameter:  $OD_{\text{HSS}} := 5 \cdot \text{in}$

Area:  $A_{\text{HSS}} := 7.88 \cdot \text{in}^2$

$$A_{e_{\text{HSS}}} := 0.75 \cdot A_{\text{HSS}} = 5.91 \cdot \text{in}^2$$

Thickness:  $t_{\text{HSS}} := 0.465 \cdot \text{in}$

Yield Strength:  $F_{y_{\text{HSS}}} := 46 \cdot \text{ksi}$

$$F_{u_{\text{HSS}}} := 62 \cdot \text{ksi}$$

Length:  $L_{\text{HSS}} := 14 \cdot \text{in}$

Moment of Inertia:  $I_{\text{HSS}} := 26.0 \cdot \text{in}^4$

Radius of Gyration:  $r_{\text{HSS}} := 1.82 \cdot \text{in}$

Inside Dimension:  $ID_{\text{HSS}} := OD_{\text{HSS}} - 2 \cdot t_{\text{HSS}} = 4.07 \cdot \text{in}$

**Bearing Check  
(AISC 15th Ed., Equation J7-1):**

$$\phi_b := 0.75$$

$$P_{u_c} = \phi_b \cdot R_n = \phi_b \cdot 1.8 \cdot F_{y_{\text{HSS}}} \cdot A_{pb}$$

$$A_{pb} := \frac{P_u}{\phi_b \cdot 1.8 \cdot F_{y_{\text{HSS}}}} = 2.63 \cdot \text{in}^2$$

$$\text{Check}_{\text{bear}} := \begin{cases} \text{"OK"} & \text{if } A_{\text{HSS}} \geq A_{pb} \\ \text{"N/G"} & \text{otherwise} \end{cases}$$

$$\text{Check}_{\text{bear}} = \text{"OK"}$$

**Compression Check  
(AISC 15th Ed., Eqs. E3-1 to E3-4):**

$$\phi_c := 0.9$$

$$K_{\text{eff}} := 1$$

$$\phi P_{u\_comp} = \phi_c \cdot F_{cr} \cdot A_g$$

$$L_c := K \cdot L_{\text{HSS}} = 14 \cdot \text{in}$$

$$F_e := \frac{\pi^2 \cdot 29000 \text{ksi}}{\left(\frac{L_c}{r_{\text{HSS}}}\right)^2} = 4837.09 \cdot \text{ksi}$$

$$\frac{L_c}{r_{\text{HSS}}} = 7.69 < 4.71 \cdot \sqrt{\frac{29000 \cdot \text{ksi}}{F_{y\_HSS}}} = 118.26$$

$$\therefore F_{cr} := 0.658 \cdot \frac{F_{y\_HSS}}{F_e} \cdot F_{y\_HSS} = 45.82 \cdot \text{ksi}$$

**(AISC 15th Ed., Equation J4-6):**

$$\phi P_{u\_comp} := \begin{cases} \phi_c \cdot F_{y\_HSS} \cdot A_{\text{HSS}} & \text{if } \frac{L_c}{r_{\text{HSS}}} \leq 25 \\ \phi_c \cdot F_{cr} \cdot A_{\text{HSS}} & \text{otherwise} \end{cases}$$

$$\phi P_{u\_comp} = 326.23 \cdot \text{kip}$$

$$\text{Check}_{\text{comp}} := \begin{cases} \text{"OK"} & \text{if } \text{Rating}_{\text{comp}} < 100\% \\ \text{"N/G"} & \text{otherwise} \end{cases}$$

$$\text{Check}_{\text{comp}} = \text{"OK"}$$

**Gusset Plate Design**

Gusset Plate width:  $w_{\text{plate}} := 7.25 \cdot \text{in}$

Gusset Plate thickness:  $t_{\text{plate}} := 1.25 \cdot \text{in}$

$$L_{\text{plate1}} := 57 \cdot \text{in}$$

$$L_{\text{plate2}} := 14 \cdot \text{in}$$

Gusset Plate Strength:  $F_{y\text{plate}} := 65 \cdot \text{ksi}$

$$F_{u\text{plate}} := 80 \cdot \text{ksi}$$

Pole thickness:  $t_{\text{pole}} := 0.3125 \cdot \text{in}$

**Shear Check  
 (AISC 15th Ed., Eqs. J4-3 and J4-4):**

$$A_g := t_{plate} \cdot L_{plate2} = 17.5 \cdot \text{in}^2$$

$$A_{nv} := A_g = 17.5 \cdot \text{in}^2$$

Shear Yielding

$$\phi_v := 1$$

$$\phi V_{plate} := \phi_v \cdot 0.6 \cdot A_g \cdot F_{yplate} = 682.5 \cdot \text{kip}$$

$$\text{Check}_{shear} := \begin{cases} \text{"OK"} & \text{if Rating}_{shear} < 100\% \\ \text{"N/G"} & \text{otherwise} \end{cases}$$

Check<sub>shear</sub> = "OK"

Shear Rupture

$$\phi_w := 0.75$$

$$\phi V_{plate} := \phi_v \cdot 0.6 \cdot A_{nv} \cdot F_{uplate} = 630 \cdot \text{kip}$$

$$\text{Check}_{shear} := \begin{cases} \text{"OK"} & \text{if Rating}_{shear} < 100\% \\ \text{"N/G"} & \text{otherwise} \end{cases}$$

Check<sub>shear</sub> = "OK"

**Gusset Plate to Pole and Base Plate  
 Weld Design (Horizontal and Vertical  
 Weld):  
 (AISC 15th Ed., Part 8)**

Gusset plate thickness:

$$t_{plate} = 1.25 \cdot \text{in}$$

Pole Grade:

$$F_{ypole} := 65 \text{ksi} \quad F_{upole} := 80 \text{ksi}$$

Base Plate Grade:

$$F_{ybase} := 60 \text{ksi} \quad F_{ubase} := 75 \text{ksi}$$

Gusset Plate Grade:

$$F_{yplate} = 65 \cdot \text{ksi} \quad F_{uplate} = 80 \cdot \text{ksi}$$

Height of vertical weld from base plate:

$$H_w := L_{plate1} = 57 \cdot \text{in}$$

$$\text{Notch}_{horiz} := 0.75 \cdot \text{in}$$

$$\text{Notch}_{vert} := 0.75 \cdot \text{in}$$

Gap between Base Plate and HSS:

$$\text{Gap} := 0 \cdot \text{in}$$

Vertical fillet weld size to pole:  
 (in sixteenths of an inch)

$$D_{vpole} := 6$$

$$\text{weldsize}_{pole} := \frac{D_{vpole}}{16} = \frac{3}{8}$$

Weld Material Grade:

$$F_{EXX} := 70 \text{ksi}$$



Check := | "OK" if Rating < 100%  
          | "INSUFFICIENT" otherwise

Check = "OK"

**Gusset Plate to HSS Weld Design**  
(AISC 15th Ed., Table 8-4)

Electrode Strength:

$F_{EXX} := 70 \text{ksi}$

Weld Size (in sixteenths of an inch):

$$D_1 := 6$$

$$\text{weldsize}_1 := \frac{D_1}{16} = \frac{3}{8}$$

Assume the worst-case installation scenario where the rod is positioned directly against the far side of the HSS.

$$\text{ecc}_2 := \text{OD}_{\text{HSS}} - t_{\text{HSS}} - \frac{D_{\text{new}}}{2} = 3.41 \cdot \text{in}$$

Load not in plane with weld group:

$$k := 0$$

$$a := \frac{\text{ecc}_2}{L_{\text{plate2}}} = 0.24$$

$$C_1 = 1$$

$$\text{Coeff}_1 := 3.35$$

$$\phi_w := 0.75$$

$$D_{\text{min1}} := \text{ceil} \left( \frac{P_u \cdot \text{in}}{\phi_w \cdot \text{Coeff}_1 \cdot C_1 \cdot L_{\text{plate2}} \cdot \text{kip}} \right) = 5$$

$$\text{minweldsize} := \frac{D_{\text{min1}}}{16} = \frac{5}{16}$$

$$\text{Check}_{\text{weld}} := \begin{cases} \text{"OK"} & \text{if } D_1 \geq D_{\text{min1}} \wedge D_1 \geq \text{Min}_{\text{weldsize}} \\ \text{"N/G"} & \text{otherwise} \end{cases}$$

$$\text{Check}_{\text{weld}} = \text{"OK"}$$

$$\phi R_{n_{\text{weld1}}} := \phi_w \cdot \text{Coeff}_1 \cdot \text{ksi} \cdot \text{in} \cdot C_1 \cdot D_1 \cdot L_{\text{plate2}} = 211.05 \cdot \text{kip}$$

$$\text{Check}_{\text{weld1}} := \begin{cases} \text{"OK"} & \text{if } \text{Rating}_{\text{weld1}} < 100\% \\ \text{"N/G"} & \text{otherwise} \end{cases}$$

$$\text{Check}_{\text{weld1}} = \text{"OK"}$$

**Gusset Plate to Pole Punching Shear Check**  
 (max per unit length):  
 (AISC 15th Ed., Section J4.2)

Assume the worst-case installation scenario where the rod is positioned directly against the far side of the HSS.

$$\phi_{sy} := 1.0$$

$$\phi_{sr} := 0.75$$

$$ecc_1 := w_{plate} + OD_{HSS} - t_{HSS} - \frac{D_{new}}{2} = 10.66 \cdot \text{in}$$

$$M_1 := Pu \cdot ecc_1 = 1738.65 \cdot \text{kip} \cdot \text{in}$$

$$S_1 := \frac{t_{plate} \cdot L_{plate1}^2}{6} = 676.87 \cdot \text{in}^3$$

$$f_{ww} := \frac{M_1}{S_1} \cdot t_{plate} \cdot 1 \text{in} = 3.21 \cdot \text{kip}$$

AISC 15th Ed., Equation J4-3:

$$\phi F_{sy} := \phi_{sy} \cdot 0.6 \cdot F_{y_{pole}} \cdot 2 \cdot t_{pole} \cdot 1 \text{in} = 24.37 \cdot \text{kip}$$

AISC 15th Ed., Equation J4-4:

$$\phi F_{sr} := \phi_{sr} \cdot 0.6 \cdot F_{u_{pole}} \cdot 2 \cdot t_{pole} \cdot 1 \text{in} = 22.5 \cdot \text{kip}$$

$$\phi F_{ww} := \min(\phi F_{sy}, \phi F_{sr}) = 22.5 \cdot \text{kip}$$

$$\text{Check}_{PS1} := \begin{cases} \text{"OK"} & \text{if Rating}_{PS1} < 100\% \\ \text{"N/G"} & \text{otherwise} \end{cases}$$

Check<sub>PS1</sub> = "OK"

**Gusset Plate to HSS Punching Shear Check**  
 (max per unit length):  
 (AISC 15th Ed., Section J4.2)

Assume the worst-case installation scenario where the rod is positioned directly against the far side of the HSS.

$$ecc_2 := OD_{HSS} - t_{HSS} - \frac{D_{new}}{2} = 3.41 \cdot \text{in}$$

$$M_2 := Pu \cdot ecc_2 = 556.17 \cdot \text{kip} \cdot \text{in}$$

$$S_2 := \frac{t_{plate} \cdot L_{plate2}^2}{6} = 40.83 \cdot \text{in}^3$$

$$f_{ww} := \frac{M_2}{S_2} \cdot t_{plate} \cdot 1 \text{in} = 17.03 \cdot \text{kip}$$

AISC 15th Ed., Equation J4-3:

$$\phi F_{sy} := \phi_{sy} \cdot 0.6 \cdot F_{y_{HSS}} \cdot 2 \cdot t_{HSS} \cdot 1 \text{in} = 25.67 \cdot \text{kip}$$

AISC 15th Ed., Equation J4-4:

$$\phi F_{sr} := \phi_{sr} \cdot 0.6 \cdot F_{u_{HSS}} \cdot 2 \cdot t_{HSS} \cdot 1 \text{in} = 25.95 \cdot \text{kip}$$

$$\phi F_{ww} := \min(\phi F_{sy}, \phi F_{sr}) = 25.67 \cdot \text{kip}$$

$$\text{Check}_{PS2} := \begin{cases} \text{"OK"} & \text{if Rating}_{PS2} < 100\% \\ \text{"N/G"} & \text{otherwise} \end{cases}$$

Check<sub>PS2</sub> = "OK"

## Embedment Depth Calculations

Projected Embedment Depth:	$L_{em} := 5.75 \cdot ft$	
Yield Strength of Rebar:	$f_y := 60ksi$	
Concrete Strength:	$f_c := 3000psi$	
Transverse Reinforcement Index:	$k_{tr} := 0$	Can be taken as 0 for design per ACI 318-14
Epoxy Factor:	$\psi_e := 1$	
Rebar Size Factor:	$\psi_s := 1$	
Casting Position Factor:	$\psi_t := 1$	
Concrete Weight Factor:	$\lambda := 1 \cdot \sqrt{psi}$	
Pier Diameter:	$D_{pier} := 6ft$	
Cover	$c_c := 3in$	
Rebar Size:	$d_s := 10$	$d_b := vlookup(d_s, Rebar, 2) \cdot in = 1.27 \cdot in$
Tie Size:	$Tie := 3$	
Number of Vertical Rebar:	$n := 32$	

The embedment depth shall be analyzed based on the design tension capacity of the anchor rods.

**Design Load:**  $\phi P_{nt} := 0.75 \cdot F_{u,rod} \cdot A_{new} = 304.69 \cdot kip$

**Development Length  
 (ACI 318-14 Chapter 25):**

$$BC_{rebar} := D_{pier} - 2 \cdot c_c - \frac{Tie \cdot in}{4} - d_b = 63.98 \cdot in$$

$$S_{rebar} := \frac{\pi \cdot BC_{rebar}}{n} = 6.281 \cdot in$$

$$c_b := \min \left( c_c + \frac{Tie}{8} \cdot in + \frac{d_b}{2}, S_{rebar} \cdot 0.5 \right) = 3.14 \cdot in$$

**ACI 318-14, Equation 25.4.2.3a:**

$$l_d := \left[ \frac{3}{40} \cdot \frac{f_y}{\lambda \cdot \sqrt{f_c}} \cdot \frac{\psi_t \cdot \psi_e \cdot \psi_s}{\min \left[ \left( \frac{c_b + k_{tr}}{d_b} \right), 2.5 \right]} \right] \cdot d_b = 42.19 \cdot in$$

**Calculate Max Distance Between Rebar and New Anchor Rods:**

$$A := \frac{1}{2} \cdot S_{\text{rebar}} = 3.141 \cdot \text{in}$$

$$B := \frac{BC_{\text{rebar}}}{2} - \frac{BC_{\text{new}}}{2} = 6.365 \cdot \text{in}$$

$$G := \sqrt{A^2 + B^2} = 7.098 \cdot \text{in}$$

$$l'_d := l_d + \frac{G}{1.5} + 3 \text{in} = 4.16 \text{ft}$$

### Epoxy Development Length:

Bond Strength:

Epoxy :=

$$\phi_{\text{bond}} := 0.65$$

$$S_b := \begin{cases} S_{bh} & \text{if Epoxy} = 0 \\ S_{bA} & \text{otherwise} \end{cases}$$

$$S_b = 1073 \text{psi}$$

$$L_{be} := \frac{\phi P_{nt}}{\pi \cdot D_{\text{new}} \cdot S_b \cdot \phi_{\text{bond}}} = 61.8 \cdot \text{in}$$

### Required Embedment Length:

Length of Breaker Tape:

$$L_{\text{min}} := \max(L_{be} + L_{BT}, l'_d + 0.25 \cdot L_{be}) = 5.65 \text{ft}$$

$$\text{Check} := \begin{cases} \text{"OK"} & \text{if } L_{\text{min}} \leq L_{\text{em}} \\ \text{"N/G"} & \text{otherwise} \end{cases}$$

### Anchor Rod Pullout Test:

$$\phi_p := 0.75$$

Is this a CA DSA site?

Yes  
 No



$$\text{Pullout} := \begin{cases} \frac{\phi_p \cdot F_{u_{\text{rod}}} \cdot A_{\text{new}}}{1.6} & \text{if } CA = 0 \\ (0.8 \cdot F_{y_{\text{rod}}} \cdot A_{\text{new}}) & \text{otherwise} \end{cases} = 190 \cdot \text{kip}$$

# Pier and Pad Foundation



**BU # :** 841301  
**Site Name:** WILLINGTON-RIV  
**App. Number:** 479824 Rev.0

**TIA-222 Revision:** H  
**Tower Type:** Monopole

**Top & Bot. Pad Rein. Different?:**   
**Block Foundation?:**

Superstructure Analysis Reactions		
Compression, <b>P<sub>comp</sub></b> :	29.1	kips
Base Shear, <b>V<sub>u,comp</sub></b> :	18.44	kips
Moment, <b>M<sub>u</sub></b> :	1487.93	ft-kips
Tower Height, <b>H</b> :	110	ft
BP Dist. Above Fdn, <b>b<sub>pdist</sub></b> :	4.625	in

Foundation Analysis Checks				
	Capacity	Demand	Rating*	Check
<i>Lateral (Sliding) (kips)</i>	160.63	18.44	10.9%	Pass
<i>Bearing Pressure (ksf)</i>	15.00	2.16	13.7%	Pass
<i>Overtuning (kip*ft)</i>	3608.91	1605.68	44.5%	Pass
<i>Pier Flexure (Comp.) (kip*ft)</i>	5182.08	1552.47	28.5%	Pass
<i>Pier Compression (kip)</i>	17184.96	51.78	0.3%	Pass
<i>Pad Flexure (kip*ft)</i>	1677.22	543.92	30.9%	Pass
<i>Pad Shear - 1-way (kips)</i>	553.09	96.75	16.7%	Pass
<i>Pad Shear - 2-way (Comp) (ksi)</i>	0.164	0.027	15.7%	Pass
<i>Flexural 2-way (Comp) (kip*ft)</i>	2004.63	931.48	44.3%	Pass

Pier Properties		
Pier Shape:	Square	
Pier Diameter, <b>dpier</b> :	6	ft
Ext. Above Grade, <b>E</b> :	1	ft
Pier Rebar Size, <b>Sc</b> :	10	
Pier Rebar Quantity, <b>mc</b> :	32	
Pier Tie/Spiral Size, <b>St</b> :	3	
Pier Tie/Spiral Quantity, <b>mt</b> :	3	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, <b>cc<sub>pier</sub></b> :	3	in

\*Rating per TIA-222-H Section 15.5

Soil Rating*:	44.5%
Structural Rating*:	44.3%

Pad Properties		
Depth, <b>D</b> :	5	ft
Pad Width, <b>W</b> :	22	ft
Pad Thickness, <b>T</b> :	2.5	ft
Pad Rebar Size (Bottom), <b>Sp</b> :	8	
Pad Rebar Quantity (Bottom), <b>mp</b> :	19	
Pad Clear Cover, <b>cc<sub>pad</sub></b> :	3	in

Material Properties		
Rebar Grade, <b>Fy</b> :	60	ksi
Concrete Compressive Strength, <b>F'c</b> :	3	ksi
Dry Concrete Density, <b>δc</b> :	150	pcf

Soil Properties		
Total Soil Unit Weight, <b>γ</b> :	125	pcf
Ultimate Gross Bearing, <b>Qult</b> :	20.000	ksf
Cohesion, <b>Cu</b> :	0.000	ksf
Friction Angle, <b>φ</b> :	36	degrees
SPT Blow Count, <b>N<sub>blows</sub></b> :		
Base Friction, <b>μ</b> :	0.35	
Neglected Depth, <b>N</b> :	2.50	ft
Foundation Bearing on Rock?	Yes	
Groundwater Depth, <b>gw</b> :	n/a	ft

<--Toggle between Gross and Net

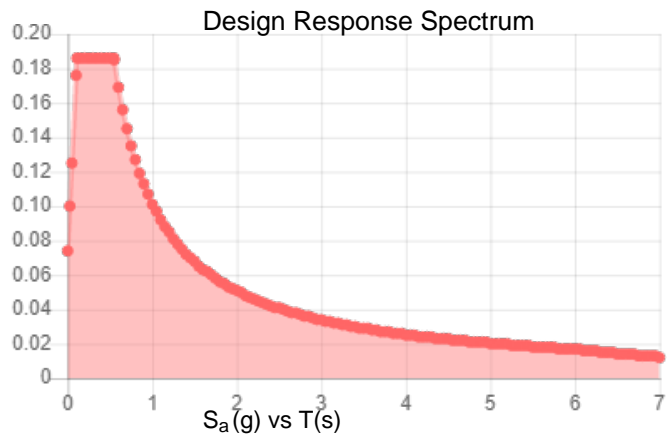
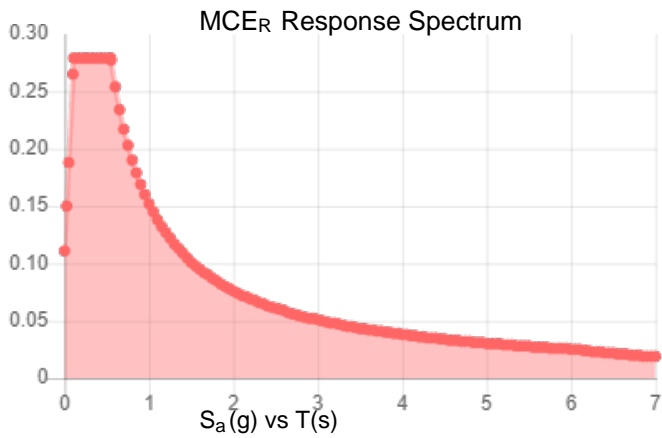


**Site Soil Class:** D - Stiff Soil

**Results:**

$S_S$ :	0.174	$S_{DS}$ :	0.186
$S_1$ :	0.063	$S_{D1}$ :	0.101
$F_a$ :	1.6	$T_L$ :	6
$F_v$ :	2.4	PGA :	0.086
$S_{MS}$ :	0.279	PGA <sub>M</sub> :	0.138
$S_{M1}$ :	0.152	F <sub>PGA</sub> :	1.6
		$I_e$ :	1

**Seismic Design Category** B



**Data Accessed:**

Mon Jun 17 2019

**Date Source:**

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

## Ice

---

**Results:**

Ice Thickness: 1.00 in.  
Concurrent Temperature: 5 F  
Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

**Date Accessed:** Mon Jun 17 2019

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

---

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# Exhibit E

## **Mount Analysis**



Date: June 5, 2019

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B+T Group  
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Tulsa, OK 74119  
(918) 587-4630  
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**Subject:** Mount Analysis Report

**Carrier Designation:** T-Mobile Equipment Change-Out  
**Carrier Site Number:** CT11261C  
**Carrier Site Name:** Tolland/I-84/Fill-In

**Crown Castle Designation:** **Crown Castle BU Number:** 841301  
**Crown Castle Site Name:** Willington-River Rd  
**Crown Castle JDE Job Number:** 559267  
**Crown Castle Order Number:** 479824, Rev.0

**Engineering Firm Designation:** **B+T Group Report Designation:** 135909.001.01.R1

**Site Data:** 426 River Road, Willington, CT, Tolland County, 06279  
Latitude 41° 53' 26.72" Longitude -72° 17' 21.77"

**Structure Information:** **Tower Height & Type:** 110 ft. Monopole  
**Mount Elevation:** 100 ft.  
**Mount Type:** 10.75 ft. Platform Mount

Dear Mr. McGuirt,

B+T Group is pleased to submit this "Mount Analysis Report" to determine the structural integrity of T-Mobile's antenna mounting system with the proposed appurtenance and equipment addition on the above mentioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount's stress level. Based on our analysis we have determined the stress level to be:

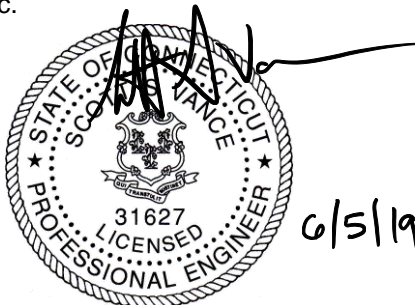
**Platform Mount**

**Sufficient**

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount structural analysis prepared by: Phanindra Kosaraju, E.I.T.

Respectfully submitted by: B&T Engineering, Inc.  
COA: PEC.0001564 Expires: 02/10/2020



Scott S. Vance, P.E.  
Engineer of Record

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Table 3 - Mount Component Stresses vs. Capacity

4.1) Recommendations

### 5) APPENDIX A

Wire Frame and Rendered Models

### 6) APPENDIX B

Software Input Calculations

### 7) APPENDIX C

Software Analysis Output



## 1) INTRODUCTION

This is a 10.75' Platform Mount, mapped by Paul J. Ford & Company & RKS.

## 2) ANALYSIS CRITERIA

<b>Building Code:</b>	2015 IBC
<b>TIA-222 Revision:</b>	TIA-222-H
<b>Risk Category:</b>	II
<b>Ultimate Wind Speed:</b>	125 mph
<b>Exposure Category:</b>	B
<b>Topographic Factor at Base:</b>	1
<b>Topographic Factor at Mount:</b>	1
<b>Ice Thickness:</b>	2 in
<b>Wind Speed with Ice:</b>	50 mph
<b>Seismic <math>S_s</math>:</b>	0.174
<b>Seismic <math>S_1</math>:</b>	0.063
<b>Live Loading Wind Speed:</b>	30 mph
<b>Man Live Load at Mid/End-Points:</b>	250 lb.
<b>Man Live Load at Mount Pipes:</b>	500 lb.

**Table 1 - Proposed Equipment Configuration**

Mount Centerline	Antenna Centerline	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount Details
100 ft	100 ft	2	EMS Wireless	RR90-17-00DP	10.75' Platform Mount
		2	RFS	APXVAARR24_43-U-NA20	
		2	Ericsson	KRY 112 144/1	
		2	Ericsson	KRY 112 489/2	
		2	Ericsson	RADIO 4449 B12/B71	

## 3) ANALYSIS PROCEDURE

**Table 2 - Documents Provided**

Document	Remarks	Reference	Source
CCI Order	Existing Loading Proposed Loading	Date: 05/28/2019	Crown Castle
Mount Mapping	Paul J. Ford & Company & RKS	Date: 04/06/2019	On File

### 3.1) Analysis Method

RISA-3D (Version 17.0.2), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed by B+T Group, was used to calculate wind loading on all appurtenances, dishes and mount members for various loading cases. Selected output from the analysis is included in Appendix B "Software Input Calculations".

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision C).

### 3.2) Assumptions

1. The mount was properly fabricated and installed in accordance with its original design and manufacturer's specifications.
2. The mount has been maintained in accordance with the manufacturer's specifications and is free of damage.
3. The configuration of antennas, mounts, and other appurtenances are as specified in Table-1.
4. All mount components have been assumed to be in sufficient condition to carry their full design capacity for the analysis.
5. Mount areas and weights are determined from field measurements, standard material properties, and/or manufacturer product data.
6. Serviceability with respect to antenna twist, tilt, roll or lateral translation is not checked and is left to the carrier or tower owner to ensure conformance.
7. All prior structural modifications, if any are assumed to be correctly installed and fully effective.
8. All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
9. The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
10. The following material grades were assumed (Unless Noted Otherwise):
  - (a) Connection Bolts : ASTM A325
  - (b) Steel Pipe : ASTM A53 (GR. 35)
  - (c) HSS (Round) : ASTM 500 (GR. B-42)
  - (d) HSS (Rectangular) : ASTM 500 (GR. B-46)
  - (e) Channel : ASTM A36 (GR. 36)
  - (f) Steel Solid Rod : ASTM A36 (GR. 36)
  - (g) Steel Plate : ASTM A36 (GR. 36)
  - (h) Steel Angle : ASTM A36 (GR. 36)
  - (i) UNISTRUT : ASTM A570 (GR. 33)

This analysis may be affected if any assumptions are not valid or have been made in error. B+T Group should be notified to determine the effect on the structural integrity of the antenna mounting system.

### 4) ANALYSIS RESULTS

**Table 3 - Mount Component Stresses vs. Capacity (Platform Mount)**

Notes	Component	Critical Member	Centerline (ft.)	% Capacity	Pass / Fail
1,2	Main Horizontals	M1	100	48.9	Pass
	Support Channels	M8	100	37.2	Pass
	Support Angle	M109	100	16.5	Pass
	Mount Pipes	M90	100	68.3	Pass

<b>Structure Rating (max from all components) =</b>	<b>68.3%</b>
---	--------------

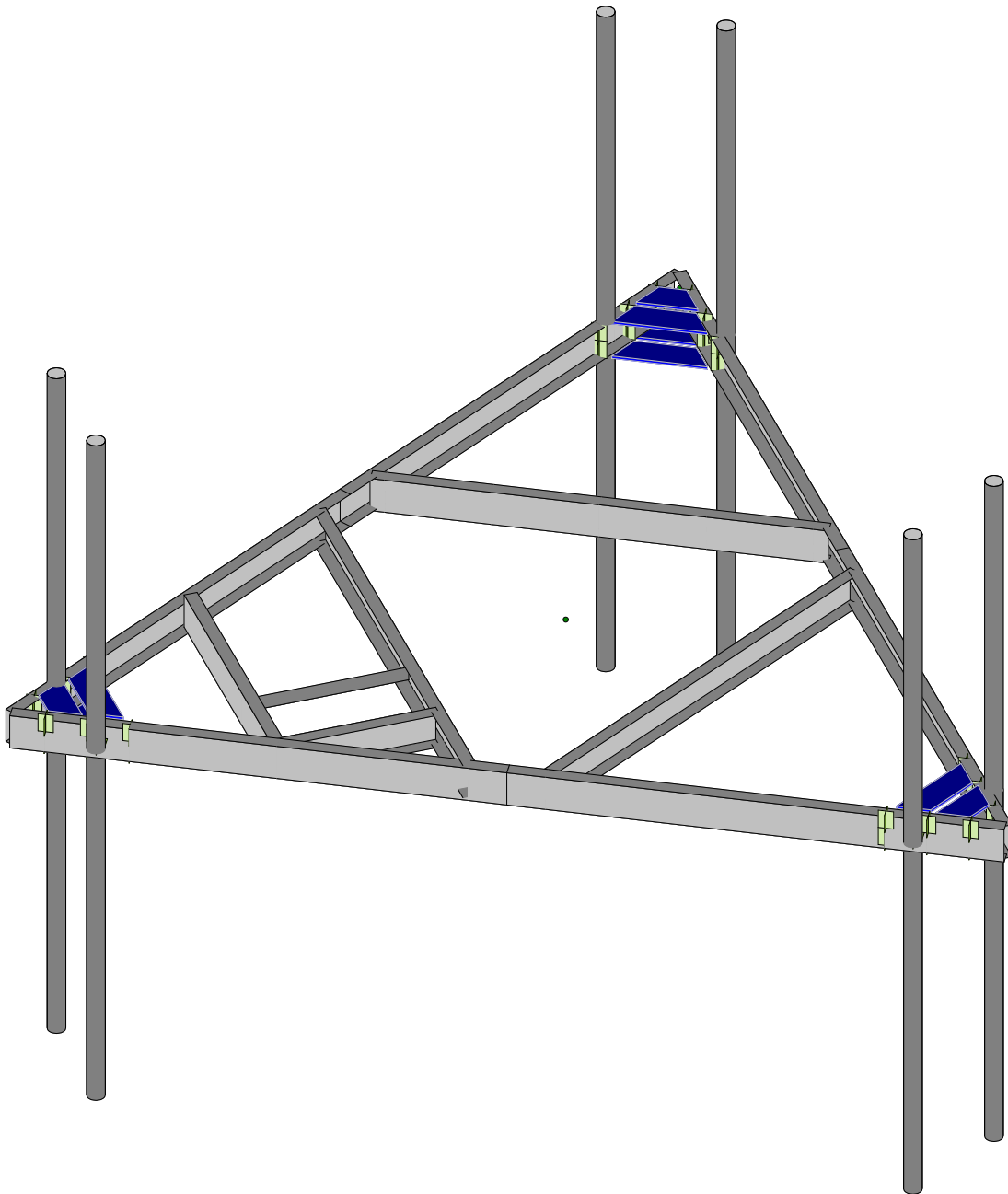
Notes:

- 1) See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed.
- 2) All sectors are typical

#### 4.1) Recommendations

The mount has sufficient capacity to carry the proposed loading configuration. No modifications are required at this time.

**APPENDIX A**  
**WIRE FRAME AND RENDERED MODELS**



Envelope Only Solution

B+T Group

PKK

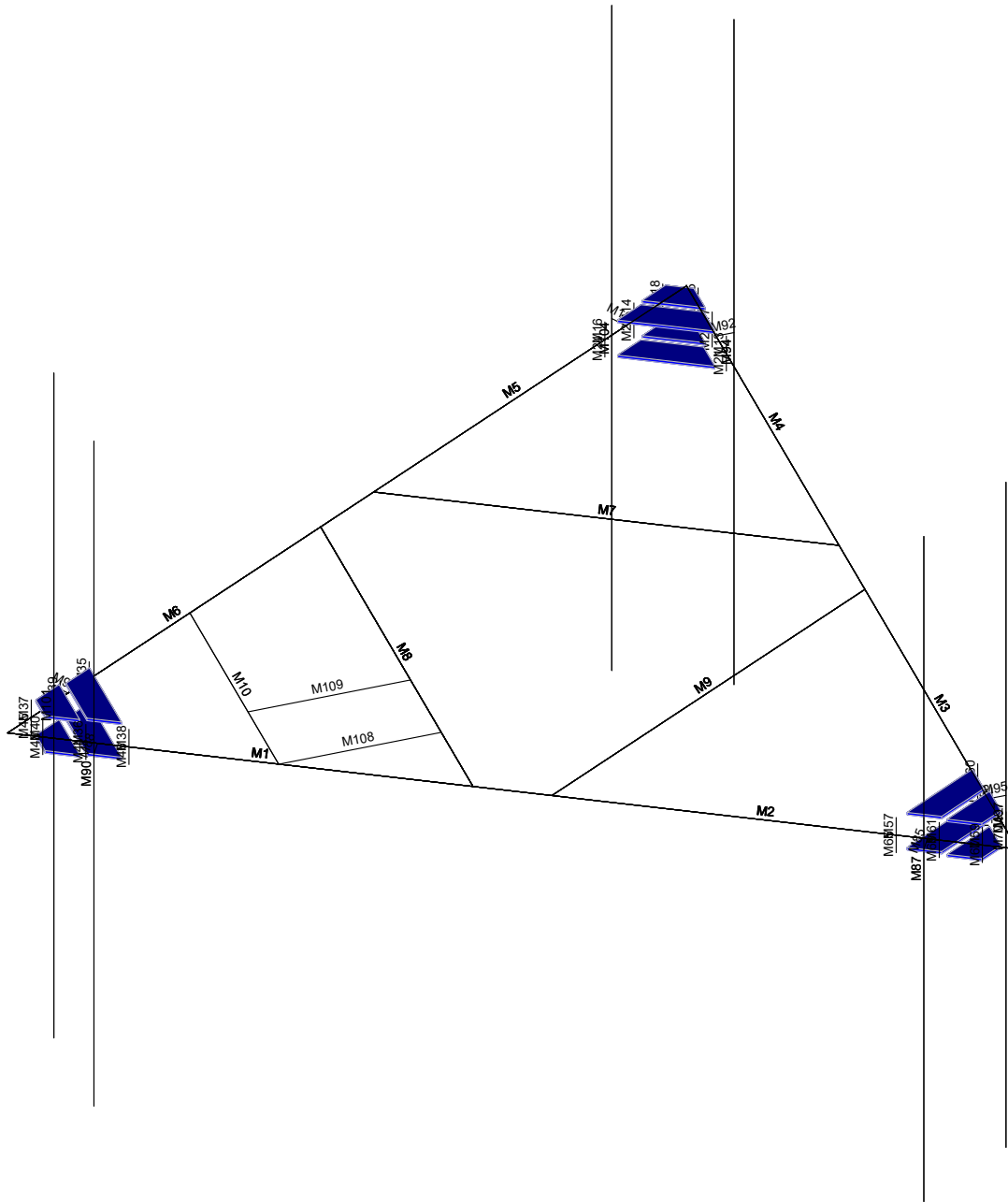
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841301 - Willington-River Rd

SK - 1

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Envelope Only Solution

B+T Group

PKK

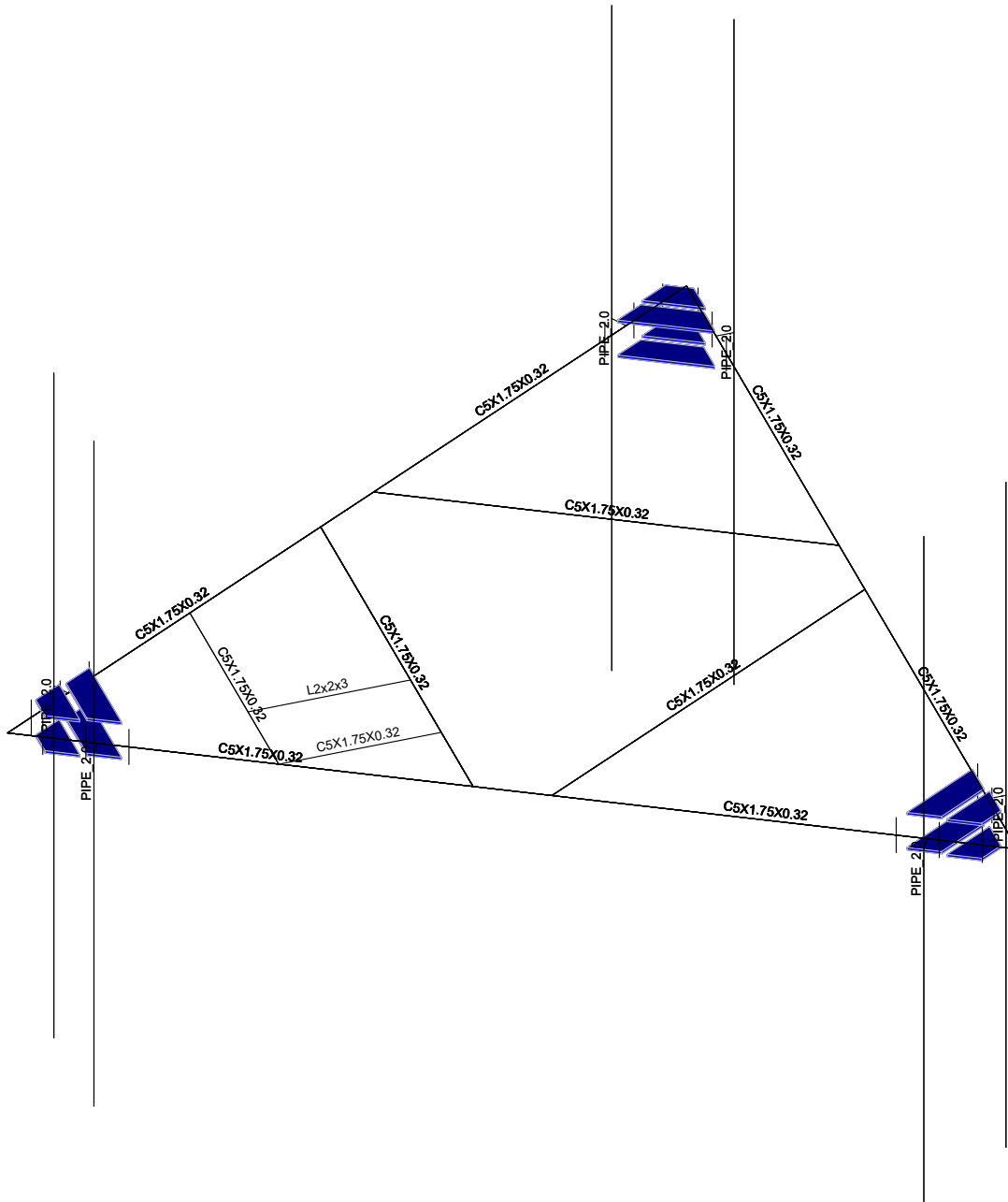
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Envelope Only Solution

B+T Group

PKK

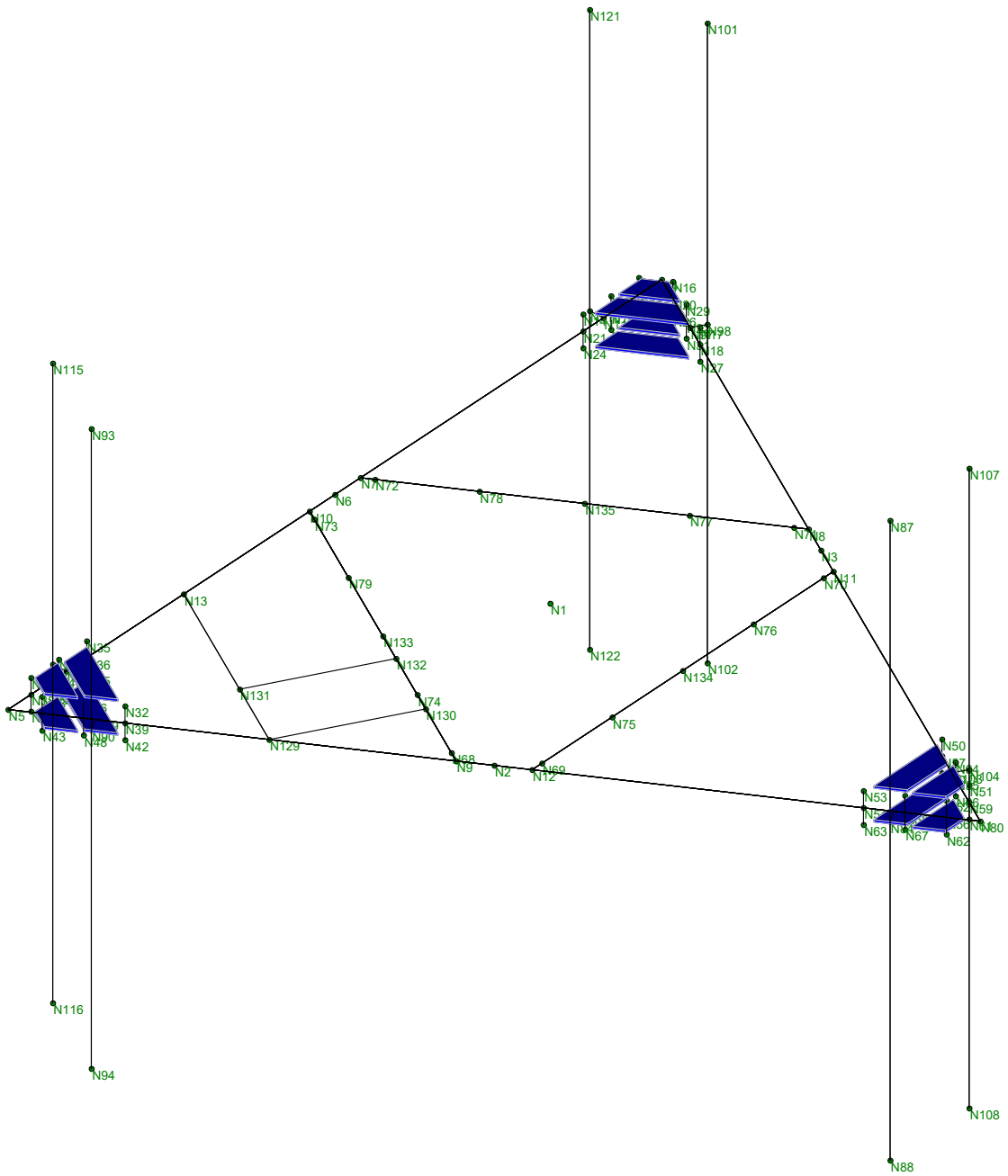
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**APPENDIX B**  
**SOFTWARE INPUT CALCULATIONS**







PROJECT	<b>135909.001.01 - Willington-R</b>		<b>PKK</b>
SUBJECT	<b>Platform Mount Mount Analysis</b>		
DATE	<b>06/04/19</b>	PAGE	OF



Manufacturer	Model	Qty	Aspect Ratio	C <sub>a</sub> flat/round	EPA <sub>N</sub> *K <sub>a</sub> (ft <sup>2</sup> )	EPA <sub>T</sub> *K <sub>a</sub> (ft <sup>2</sup> )	EPA <sub>N-ice</sub> *K <sub>a</sub> (ft <sup>2</sup> )	EPA <sub>T-ice</sub> *K <sub>a</sub> (ft <sup>2</sup> )	F <sub>A</sub> No Ice (N)	F <sub>A</sub> No Ice (T)	F <sub>A</sub> Ice (N)	F <sub>A</sub> Ice (T)
EMS Wireless	RR90-17-00DP	0.5	7.00	1.40	1.40	0.48	2.36	1.36	0.07	0.03	0.01	0.01
EMS Wireless	RR90-17-00DP	0.5	7.00	1.40	1.40	0.48	2.36	1.36	0.07	0.03	0.01	0.01
Ericsson	KRY 112 489/2	2	1.80	1.20	0.84	0.54	2.04	1.63	0.04	0.02	0.01	0.00
RFS	APXVAARR24_43-U-NA20	0.5	4.00	1.27	7.19	2.61	8.93	4.13	0.34	0.15	0.05	0.02
RFS	APXVAARR24_43-U-NA20	0.5	4.00	1.27	7.19	2.61	8.93	4.13	0.34	0.15	0.05	0.02
Ericsson	RADIO 4449 B12/B71	1	1.13	1.20	1.23	0.86	2.14	1.67	0.05	0.04	0.01	0.01
Ericsson	KRY 112 144/1	2	1.17	1.20	0.53	0.26	1.50	1.07	0.02	0.01	0.00	0.00
Ericsson	RADIO 4449 B12/B71	1	1.13	1.20	1.23	0.86	2.14	1.67	0.05	0.04	0.01	0.01
EMS Wireless	RR90-17-00DP	0.5	7.00	1.40	1.40	0.48	2.36	1.36	0.07	0.03	0.01	0.01
EMS Wireless	RR90-17-00DP	0.5	7.00	1.40	1.40	0.48	2.36	1.36	0.07	0.03	0.01	0.01
RFS	APXVAARR24_43-U-NA20	0.5	4.00	1.27	7.19	2.61	8.93	4.13	0.34	0.15	0.05	0.02
RFS	APXVAARR24_43-U-NA20	0.5	4.00	1.27	7.19	2.61	8.93	4.13	0.34	0.15	0.05	0.02











Company : B+T Group  
 Designer : PKK  
 Job Number : 135909.001.01.R1  
 Model Name : 841301 - Willington-River Rd

June 5, 2019  
 11:20 AM  
 Checked By: \_\_\_\_\_

**Load Combinations**

Description	So...	P...	S...	BLCFa...	BLCFa...	BLCFa...	BLCFa...	BLCFa...	BLCFa...	BLCFa...	BLCFa...	BLCFa...	BLCFa...
1	1.4 Dead	Yes	Y	1	1.4								
2	1.2 D + 1.0 - 0 W	Yes	Y	1	1.2	2	1						
3	1.2 D + 1.0 - 30 W	Yes	Y	1	1.2	2	.866	3	.5				
4	1.2 D + 1.0 - 60 W	Yes	Y	1	1.2	3	.866	2	.5				
5	1.2 D + 1.0 - 90 W	Yes	Y	1	1.2	3	1						
6	1.2 D + 1.0 - 120 W	Yes	Y	1	1.2	3	.866	2	-.5				
7	1.2 D + 1.0 - 150 W	Yes	Y	1	1.2	2	-.866	3	.5				
8	1.2 D + 1.0 - 180 W	Yes	Y	1	1.2	2	-1						
9	1.2 D + 1.0 - 210 W	Yes	Y	1	1.2	2	-.866	3	-.5				
10	1.2 D + 1.0 - 240 W	Yes	Y	1	1.2	3	-.866	2	-.5				
11	1.2 D + 1.0 - 270 W	Yes	Y	1	1.2	3	-1						
12	1.2 D + 1.0 - 300 W	Yes	Y	1	1.2	3	-.866	2	.5				
13	1.2 D + 1.0 - 330 W	Yes	Y	1	1.2	2	.866	3	-.5				
14	1.2 D + 1.0 - 0 W/Ice	Yes	Y	1	1.2	4	1			8	1		
15	1.2 D + 1.0 - 30 W/Ice	Yes	Y	1	1.2	4	.866	5	.5	8	1		
16	1.2 D + 1.0 - 60 W/Ice	Yes	Y	1	1.2	5	.866	4	.5	8	1		
17	1.2 D + 1.0 - 90 W/Ice	Yes	Y	1	1.2	5	1			8	1		
18	1.2 D + 1.0 - 120 W/Ice	Yes	Y	1	1.2	5	.866	4	-.5	8	1		
19	1.2 D + 1.0 - 150 W/Ice	Yes	Y	1	1.2	4	-.866	5	.5	8	1		
20	1.2 D + 1.0 - 180 W/Ice	Yes	Y	1	1.2	4	-1			8	1		
21	1.2 D + 1.0 - 210 W/Ice	Yes	Y	1	1.2	4	-.866	5	-.5	8	1		
22	1.2 D + 1.0 - 240 W/Ice	Yes	Y	1	1.2	5	-.866	4	-.5	8	1		
23	1.2 D + 1.0 - 270 W/Ice	Yes	Y	1	1.2	5	-1			8	1		
24	1.2 D + 1.0 - 300 W/Ice	Yes	Y	1	1.2	5	-.866	4	.5	8	1		
25	1.2 D + 1.0 - 330 W/Ice	Yes	Y	1	1.2	4	.866	5	-.5	8	1		
26	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	6	1			9	1.5		
27	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	6	.866	7	.5	9	1.5		
28	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	7	.866	6	.5	9	1.5		
29	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	7	1			9	1.5		
30	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	7	.866	6	-.5	9	1.5		
31	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	6	-.866	7	.5	9	1.5		
32	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	6	-1			9	1.5		
33	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	6	-.866	7	-.5	9	1.5		
34	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	7	-.866	6	-.5	9	1.5		
35	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	7	-1			9	1.5		
36	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	7	-.866	6	.5	9	1.5		
37	1.2 D + 1.5 LL a + Service...	Yes	Y	1	1.2	6	.866	7	-.5	9	1.5		
38	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	6	1			10	1.5		
39	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	6	.866	7	.5	10	1.5		
40	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	7	.866	6	.5	10	1.5		
41	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	7	1			10	1.5		
42	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	7	.866	6	-.5	10	1.5		
43	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	6	-.866	7	.5	10	1.5		
44	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	6	-1			10	1.5		
45	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	6	-.866	7	-.5	10	1.5		
46	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	7	-.866	6	-.5	10	1.5		
47	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	7	-1			10	1.5		
48	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	7	-.866	6	.5	10	1.5		
49	1.2 D + 1.5 LL b + Service...	Yes	Y	1	1.2	6	.866	7	-.5	10	1.5		
50	1.2 D + 1.5 LL c + Service...	Yes	Y	1	1.2	6	1			11	1.5		
51	1.2 D + 1.5 LL c + Service...	Yes	Y	1	1.2	6	.866	7	.5	11	1.5		
52	1.2 D + 1.5 LL c + Service...	Yes	Y	1	1.2	7	.866	6	.5	11	1.5		
53	1.2 D + 1.5 LL c + Service...	Yes	Y	1	1.2	7	1			11	1.5		
54	1.2 D + 1.5 LL c + Service...	Yes	Y	1	1.2	7	.866	6	-.5	11	1.5		
55	1.2 D + 1.5 LL c + Service...	Yes	Y	1	1.2	6	-.866	7	.5	11	1.5		
56	1.2 D + 1.5 LL c + Service...	Yes	Y	1	1.2	6	-1			11	1.5		
57	1.2 D + 1.5 LL c + Service...	Yes	Y	1	1.2	6	-.866	7	-.5	11	1.5		







**Member Point Loads (BLC 1 : Dead) (Continued)**

	Member Label	Direction	Magnitude[k.k-ft]	Location[ft,%]
8	M90	Y	-0.075	%30
9	M90	Y	0	0
10	M90	Y	0	0
11	M101	Y	-0.022	%30
12	M101	Y	0	0
13	M101	Y	0	0
14	M101	Y	0	0
15	M101	Y	0	0
16	M104	Y	-0.075	%30
17	M104	Y	0	0
18	M104	Y	0	0
19	M104	Y	0	0
20	M104	Y	0	0
21	M94	Y	-0.007	%5
22	M94	Y	-0.007	%45
23	M94	Y	0	0
24	M94	Y	0	0
25	M94	Y	0	0
26	M97	Y	-0.064	%10
27	M97	Y	-0.064	%90
28	M97	Y	0	0
29	M97	Y	0	0
30	M97	Y	0	0

**Member Point Loads (BLC 2 : 0 Wind - No Ice)**

	Member Label	Direction	Magnitude[k.k-ft]	Location[ft,%]
1	M87	Z	-0.072	%5
2	M87	Z	-0.072	%45
3	M87	Z	-0.037	%30
4	M87	Z	0	0
5	M87	Z	0	0
6	M90	Z	-0.336	%10
7	M90	Z	-0.336	%90
8	M90	Z	-0.054	%30
9	M90	Z	0	0
10	M90	Z	0	0
11	M101	Z	-0.023	%30
12	M101	Z	0	0
13	M101	Z	0	0
14	M101	Z	0	0
15	M101	Z	0	0
16	M104	Z	-0.054	%30
17	M104	Z	0	0
18	M104	Z	0	0
19	M104	Z	0	0
20	M104	Z	0	0
21	M94	Z	-0.072	%5
22	M94	Z	-0.072	%45
23	M94	Z	0	0
24	M94	Z	0	0
25	M94	Z	0	0
26	M97	Z	-0.336	%10
27	M97	Z	-0.336	%90
28	M97	Z	0	0
29	M97	Z	0	0
30	M97	Z	0	0





**Member Point Loads (BLC 4 : 0 Wind - Ice) (Continued)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft, %]
24	M94	Z	0	0
25	M94	Z	0	0
26	M97	Z	-0.054	%10
27	M97	Z	-0.054	%90
28	M97	Z	0	0
29	M97	Z	0	0
30	M97	Z	0	0

**Member Point Loads (BLC 5 : 90 Wind - Ice)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft, %]
1	M87	X	-0.005	%5
2	M87	X	-0.005	%45
3	M87	X	-0.004	%30
4	M87	X	0	0
5	M87	X	0	0
6	M90	X	-0.024	%10
7	M90	X	-0.024	%90
8	M90	X	-0.006	%30
9	M90	X	0	0
10	M90	X	0	0
11	M101	X	-0.002	%30
12	M101	X	0	0
13	M101	X	0	0
14	M101	X	0	0
15	M101	X	0	0
16	M104	X	-0.006	%30
17	M104	X	0	0
18	M104	X	0	0
19	M104	X	0	0
20	M104	X	0	0
21	M94	X	-0.005	%5
22	M94	X	-0.005	%45
23	M94	X	0	0
24	M94	X	0	0
25	M94	X	0	0
26	M97	X	-0.024	%10
27	M97	X	-0.024	%90
28	M97	X	0	0
29	M97	X	0	0
30	M97	X	0	0

**Member Point Loads (BLC 6 : 0 Wind - Service)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft, %]
1	M87	Z	-0.004	%5
2	M87	Z	-0.004	%45
3	M87	Z	-0.002	%30
4	M87	Z	0	0
5	M87	Z	0	0
6	M90	Z	-0.019	%10
7	M90	Z	-0.019	%90
8	M90	Z	-0.003	%30
9	M90	Z	0	0
10	M90	Z	0	0
11	M101	Z	-0.001	%30
12	M101	Z	0	0
13	M101	Z	0	0



**Member Point Loads (BLC 6 : 0 Wind - Service) (Continued)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
14	M101	Z	0	0
15	M101	Z	0	0
16	M104	Z	-.003	%30
17	M104	Z	0	0
18	M104	Z	0	0
19	M104	Z	0	0
20	M104	Z	0	0
21	M94	Z	-.004	%5
22	M94	Z	-.004	%45
23	M94	Z	0	0
24	M94	Z	0	0
25	M94	Z	0	0
26	M97	Z	-.019	%10
27	M97	Z	-.019	%90
28	M97	Z	0	0
29	M97	Z	0	0
30	M97	Z	0	0

**Member Point Loads (BLC 7 : 90 Wind - Service)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	M87	X	-.002	%5
2	M87	X	-.002	%45
3	M87	X	-.001	%30
4	M87	X	0	0
5	M87	X	0	0
6	M90	X	-.009	%10
7	M90	X	-.009	%90
8	M90	X	-.002	%30
9	M90	X	0	0
10	M90	X	0	0
11	M101	X	-.0007	%30
12	M101	X	0	0
13	M101	X	0	0
14	M101	X	0	0
15	M101	X	0	0
16	M104	X	-.002	%30
17	M104	X	0	0
18	M104	X	0	0
19	M104	X	0	0
20	M104	X	0	0
21	M94	X	-.002	%5
22	M94	X	-.002	%45
23	M94	X	0	0
24	M94	X	0	0
25	M94	X	0	0
26	M97	X	-.009	%10
27	M97	X	-.009	%90
28	M97	X	0	0
29	M97	X	0	0
30	M97	X	0	0

**Member Point Loads (BLC 8 : Ice)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	M87	Y	-.068	%5
2	M87	Y	-.068	%45
3	M87	Y	-.048	%30



**Member Point Loads (BLC 8 : Ice) (Continued)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
4	M87	Y	0	0
5	M87	Y	0	0
6	M90	Y	-.303	%10
7	M90	Y	-.303	%90
8	M90	Y	-.062	%30
9	M90	Y	0	0
10	M90	Y	0	0
11	M101	Y	-.029	%30
12	M101	Y	0	0
13	M101	Y	0	0
14	M101	Y	0	0
15	M101	Y	0	0
16	M104	Y	-.062	%30
17	M104	Y	0	0
18	M104	Y	0	0
19	M104	Y	0	0
20	M104	Y	0	0
21	M94	Y	-.068	%5
22	M94	Y	-.068	%45
23	M94	Y	0	0
24	M94	Y	0	0
25	M94	Y	0	0
26	M97	Y	-.303	%10
27	M97	Y	-.303	%90
28	M97	Y	0	0
29	M97	Y	0	0
30	M97	Y	0	0

**Member Point Loads (BLC 14 : Maint LL 2)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	M1	Y	-.25	%5

**Member Point Loads (BLC 16 : Maint LL 4)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	M3	Y	-.25	%5

**Member Point Loads (BLC 18 : Maint LL 6)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	M5	Y	-.25	%5

**Member Point Loads (BLC 20 : Maint LL 8)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	M2	Y	-.25	%5

**Member Point Loads (BLC 22 : Maint LL 10)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	M4	Y	-.25	%5

**Member Point Loads (BLC 24 : Maint LL 12)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
1	M6	Y	-.25	%5

**Member Point Loads (BLC 25 : Maint LL 13)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft,%]
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Company : B+T Group  
 Designer : PKK  
 Job Number : 135909.001.01.R1  
 Model Name : 841301 - Willington-River Rd

June 5, 2019  
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**Member Point Loads (BLC 25 : Maint LL 13) (Continued)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft, %]
1	M8	Y	-.25	%50

**Member Point Loads (BLC 26 : Maint LL 14)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft, %]
1	M9	Y	-.25	%50

**Member Point Loads (BLC 27 : Maint LL 15)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[ft, %]
1	M7	Y	-.25	%50

**Member Distributed Loads (BLC 2 : 0 Wind - No Ice)**

	Member Label	Direction	Start Magnitude[k/ft,...	End Magnitude[k/ft,F...	Start Location[ft, %]	End Location[ft, %]
1	M1	Z	-.022	-.022	0	0
2	M2	Z	-.022	-.022	0	0
3	M3	Z	-.022	-.022	0	0
4	M4	Z	-.022	-.022	0	0
5	M5	Z	-.022	-.022	0	0
6	M6	Z	-.022	-.022	0	0
7	M7	Z	-.021	-.021	0	0
8	M8	Z	-.021	-.021	0	0
9	M9	Z	-.021	-.021	0	0
10	M10	Z	-.019	-.019	0	0
11	M87	Z	-.008	-.008	0	0
12	M90	Z	-.008	-.008	0	0
13	M94	Z	-.008	-.008	0	0
14	M97	Z	-.008	-.008	0	0
15	M101	Z	-.008	-.008	0	0
16	M104	Z	-.008	-.008	0	0
17	M108	Z	-.018	-.018	0	0
18	M109	Z	-.008	-.008	0	0

**Member Distributed Loads (BLC 3 : 90 Wind - No Ice)**

	Member Label	Direction	Start Magnitude[k/ft,...	End Magnitude[k/ft,F...	Start Location[ft, %]	End Location[ft, %]
1	M1	X	-.022	-.022	0	0
2	M2	X	-.022	-.022	0	0
3	M3	X	-.022	-.022	0	0
4	M4	X	-.022	-.022	0	0
5	M5	X	-.022	-.022	0	0
6	M6	X	-.022	-.022	0	0
7	M7	X	-.021	-.021	0	0
8	M8	X	-.021	-.021	0	0
9	M9	X	-.021	-.021	0	0
10	M10	X	-.019	-.019	0	0
11	M87	X	-.008	-.008	0	0
12	M90	X	-.008	-.008	0	0
13	M94	X	-.008	-.008	0	0
14	M97	X	-.008	-.008	0	0
15	M101	X	-.008	-.008	0	0
16	M104	X	-.008	-.008	0	0
17	M108	X	-.018	-.018	0	0
18	M109	X	-.008	-.008	0	0

**Member Distributed Loads (BLC 4 : 0 Wind - Ice)**

	Member Label	Direction	Start Magnitude[k/ft	End Magnitude[k/ft F	Start Location[ft %]	End Location[ft %]
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**Member Distributed Loads (BLC 4 : 0 Wind - Ice) (Continued)**

	Member Label	Direction	Start Magnitude[k/ft....	End Magnitude[k/ft.F...	Start Location[ft.%,]	End Location[ft.%,]
1	M1	Z	-0.07	-0.07	0	0
2	M2	Z	-0.07	-0.07	0	0
3	M3	Z	-0.07	-0.07	0	0
4	M4	Z	-0.07	-0.07	0	0
5	M5	Z	-0.07	-0.07	0	0
6	M6	Z	-0.07	-0.07	0	0
7	M7	Z	-0.07	-0.07	0	0
8	M8	Z	-0.07	-0.07	0	0
9	M9	Z	-0.07	-0.07	0	0
10	M10	Z	-0.06	-0.06	0	0
11	M87	Z	-0.02	-0.02	0	0
12	M90	Z	-0.02	-0.02	0	0
13	M94	Z	-0.02	-0.02	0	0
14	M97	Z	-0.02	-0.02	0	0
15	M101	Z	-0.02	-0.02	0	0
16	M104	Z	-0.02	-0.02	0	0
17	M108	Z	-0.06	-0.06	0	0
18	M109	Z	-0.05	-0.05	0	0

**Member Distributed Loads (BLC 5 : 90 Wind - Ice)**

	Member Label	Direction	Start Magnitude[k/ft....	End Magnitude[k/ft.F...	Start Location[ft.%,]	End Location[ft.%,]
1	M1	X	-0.07	-0.07	0	0
2	M2	X	-0.07	-0.07	0	0
3	M3	X	-0.07	-0.07	0	0
4	M4	X	-0.07	-0.07	0	0
5	M5	X	-0.07	-0.07	0	0
6	M6	X	-0.07	-0.07	0	0
7	M7	X	-0.07	-0.07	0	0
8	M8	X	-0.07	-0.07	0	0
9	M9	X	-0.07	-0.07	0	0
10	M10	X	-0.06	-0.06	0	0
11	M87	X	-0.02	-0.02	0	0
12	M90	X	-0.02	-0.02	0	0
13	M94	X	-0.02	-0.02	0	0
14	M97	X	-0.02	-0.02	0	0
15	M101	X	-0.02	-0.02	0	0
16	M104	X	-0.02	-0.02	0	0
17	M108	X	-0.06	-0.06	0	0
18	M109	X	-0.05	-0.05	0	0

**Member Distributed Loads (BLC 6 : 0 Wind - Service)**

	Member Label	Direction	Start Magnitude[k/ft....	End Magnitude[k/ft.F...	Start Location[ft.%,]	End Location[ft.%,]
1	M1	Z	-0.01	-0.01	0	0
2	M2	Z	-0.01	-0.01	0	0
3	M3	Z	-0.01	-0.01	0	0
4	M4	Z	-0.01	-0.01	0	0
5	M5	Z	-0.01	-0.01	0	0
6	M6	Z	-0.01	-0.01	0	0
7	M7	Z	-0.01	-0.01	0	0
8	M8	Z	-0.01	-0.01	0	0
9	M9	Z	-0.01	-0.01	0	0
10	M10	Z	-0.01	-0.01	0	0
11	M87	Z	-0.002	-0.002	0	0
12	M90	Z	-0.002	-0.002	0	0
13	M94	Z	-0.002	-0.002	0	0
14	M97	Z	-0.002	-0.002	0	0







**Member Distributed Loads (BLC 28 : BLC 1 Transient Area Loads) (Continued)**

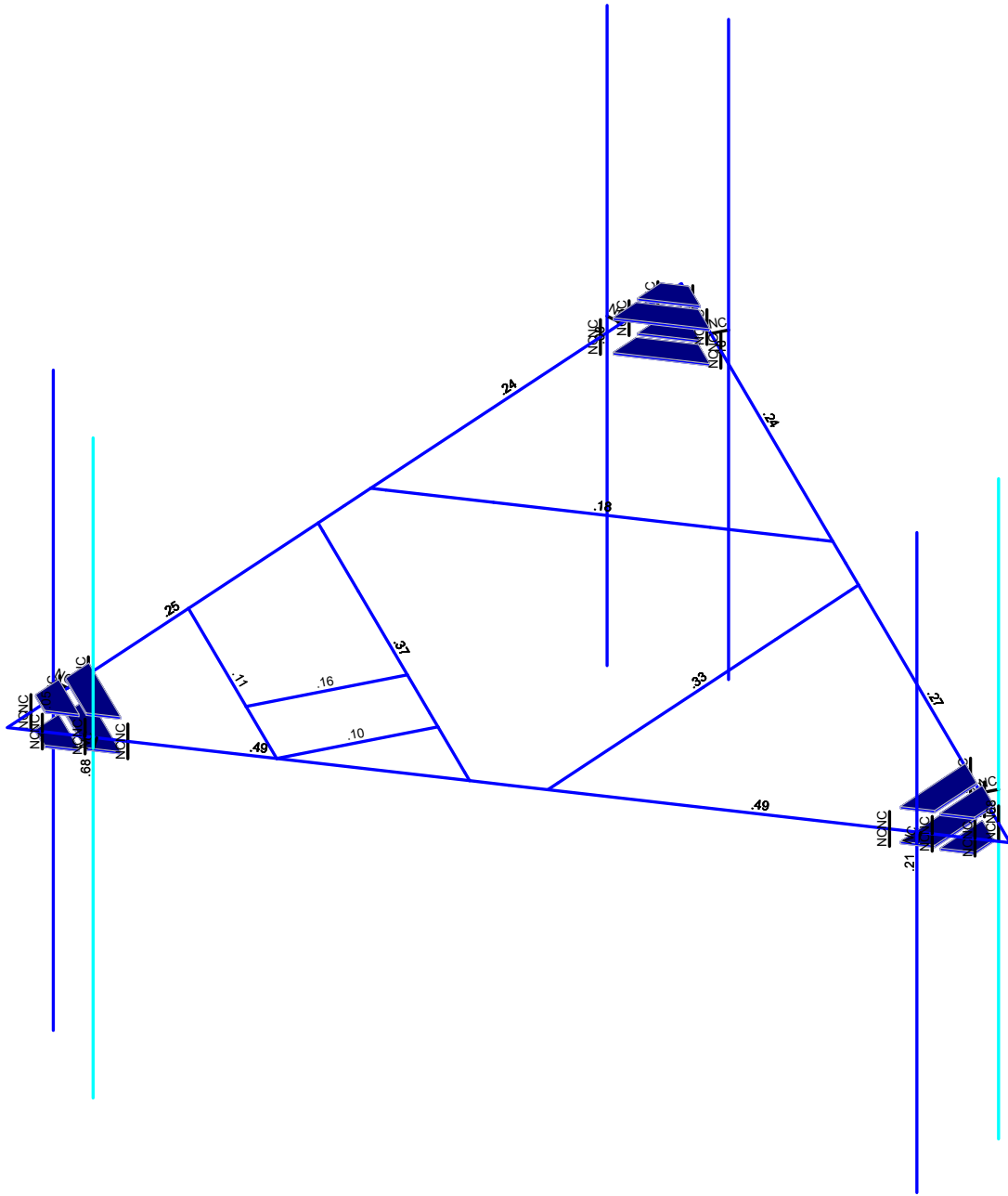
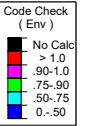
	Member Label	Direction	Start Magnitude[k/ft....	End Magnitude[k/ft.F...	Start Location[ft.%,]	End Location[ft.%,]
7	M109	Y	-.008	-.008	.005	1.792
8	M1	Y	-.0007496	-.008	.538	3.225
9	M6	Y	-.006	-.006	1.128	2.697
10	M10	Y	-.006	-.006	.735	1.95
11	M5	Y	-.005	-.005	4.768	5.375
12	M7	Y	-.003	-.005	0	1.487
13	M8	Y	-.006	-.005	3.469	4.956
14	M2	Y	-.004	-.004	4.742	5.375
15	M8	Y	-.007	-.004	0	1.487
16	M9	Y	-.003	-.004	3.469	4.956
17	M2	Y	-.003	-.008	.537	1.612
18	M2	Y	-.008	-.012	1.612	2.688
19	M2	Y	-.012	-.014	2.688	3.763
20	M3	Y	-.005	-.008	.537	2.15
21	M3	Y	-.008	-.01	2.15	3.762
22	M9	Y	-.012	-.009	0	2.478
23	M9	Y	-.009	-.006	2.478	4.956
24	M4	Y	-.005	-.005	4.768	5.375
25	M7	Y	-.003	-.005	3.469	4.956
26	M9	Y	-.006	-.005	0	1.487
27	M4	Y	-.005	-.008	.537	2.15
28	M4	Y	-.008	-.01	2.15	3.762
29	M5	Y	-.003	-.008	.537	1.612
30	M5	Y	-.008	-.012	1.612	2.687
31	M5	Y	-.012	-.014	2.687	3.762
32	M7	Y	-.006	-.009	0	2.478
33	M7	Y	-.009	-.012	2.478	4.956

**Member Distributed Loads (BLC 29 : BLC 8 Transient Area Loads)**

	Member Label	Direction	Start Magnitude[k/ft....	End Magnitude[k/ft.F...	Start Location[ft.%,]	End Location[ft.%,]
1	M1	Y	-.0004781	-.002	2.688	4.031
2	M1	Y	-.002	-.004	4.031	5.375
3	M8	Y	-.002	-.002	.311	1.311
4	M108	Y	-.003	-.003	.509	1.33
5	M6	Y	-.007	-.007	3.245	4.749
6	M8	Y	-.008	-.008	2.829	4.169
7	M10	Y	-.009	-.009	1.8	2.8
8	M109	Y	-.009	-.009	.005	1.792
9	M1	Y	-.0008245	-.009	.538	3.225
10	M6	Y	-.007	-.007	1.128	2.697
11	M10	Y	-.007	-.007	.735	1.95
12	M5	Y	-.005	-.005	4.768	5.375
13	M7	Y	-.003	-.005	0	1.487
14	M8	Y	-.006	-.005	3.469	4.956
15	M2	Y	-.005	-.005	4.742	5.375
16	M8	Y	-.008	-.005	0	1.487
17	M9	Y	-.003	-.005	3.469	4.956
18	M2	Y	-.003	-.009	.537	1.612
19	M2	Y	-.009	-.013	1.612	2.688
20	M2	Y	-.013	-.015	2.688	3.763
21	M3	Y	-.006	-.008	.537	2.15
22	M3	Y	-.008	-.011	2.15	3.762
23	M9	Y	-.013	-.01	0	2.478
24	M9	Y	-.01	-.007	2.478	4.956
25	M4	Y	-.005	-.005	4.768	5.375
26	M7	Y	-.003	-.005	3.469	4.956



**APPENDIX C**  
**SOFTWARE ANALYSIS OUTPUT**



Member Code Checks Displayed (Enveloped)  
Envelope Only Solution

B+T Group

PKK

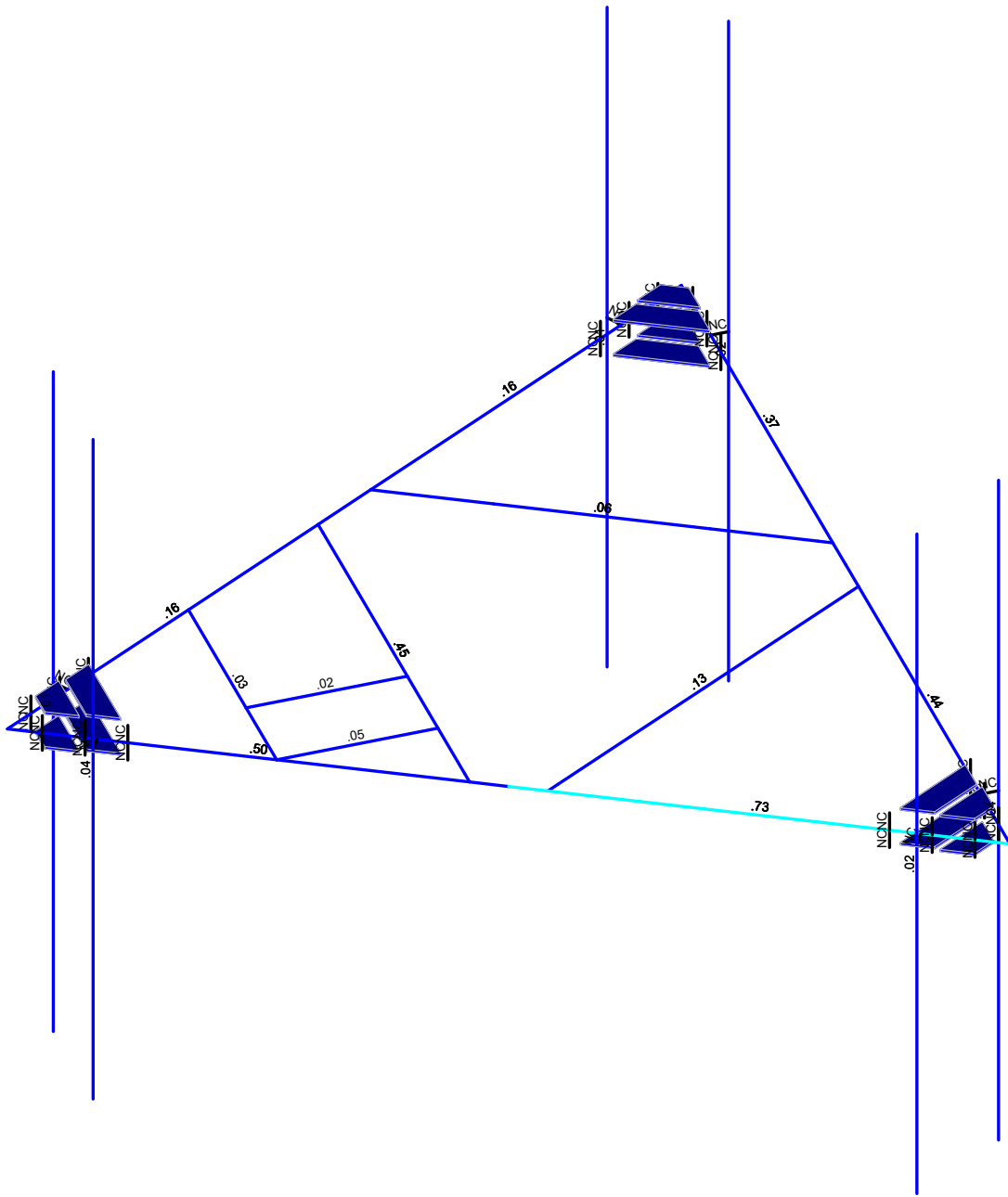
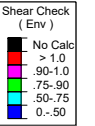
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841301 - Willington-River Rd

SK - 5

June 5, 2019 at 11:20 AM

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Member Shear Checks Displayed (Enveloped)  
Envelope Only Solution

B+T Group
PKK
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# Exhibit F

## **Power Density/RF Emissions Report**



# Transcom Engineering, Inc.

Wireless Network Design and Deployment

## Radio Frequency Emissions Analysis Report

**T-MOBILE** Existing Facility

**Site ID: CT11261C**

Tolland/I-84/ Fill-In  
426 River Road  
Willington, CT 06279

**July 22, 2019**

**Transcom Engineering Project Number: 737001-0109**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general population allowable limit:	<b>9.62 %</b>

# Transcom Engineering, Inc.

Wireless Network Design and Deployment

July 22, 2019

T-MOBILE

Attn: Jason Overbey, RF Manager  
35 Griffin Road South  
Bloomfield, CT 6009

## Emissions Analysis for Site: **CT11261C – Tolland/I-84/ Fill-In**

Transcom Engineering, Inc (“Transcom”) was directed to analyze the proposed upgrades to the T-MOBILE facility located at **426 River Road, Willington, CT**, for the purpose of determining whether the emissions from the Proposed T-MOBILE Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limits for the 600 & 700 MHz bands are approximately  $400 \mu\text{W}/\text{cm}^2$  and  $467 \mu\text{W}/\text{cm}^2$  respectively. The general population exposure limit for the 1900 MHz (PCS) and 2100 MHz (AWS) bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

# Transcom Engineering, Inc.

Wireless Network Design and Deployment

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Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

# Transcom Engineering, Inc.

Wireless Network Design and Deployment

## CALCULATIONS

Calculations were performed for the proposed upgrades to the T-MOBILE antenna facility located at **426 River Road, Willington, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-MOBILE is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. All power values expressed and analyzed are maximum power levels expected to be used on all radios.

All emissions values for additional carriers were taken from the Connecticut Siting Council (CSC) active MPE database. Values in this database are provided by the individual carriers themselves

For each sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
LTE	1900 MHz (PCS)	4	40
LTE	2100 MHz (AWS)	2	60
GSM	1900 MHz (PCS)	1	15
LTE / 5G NR	600 MHz	2	40
LTE	700 MHz	2	20

*Table 1: Channel Data Table*

# Transcom Engineering, Inc.

Wireless Network Design and Deployment

The following antennas listed in *Table 2* were used in the modeling for transmission in the 600, 700 MHz, 1900 MHz (PCS) and 2100 MHz (AWS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

Sector	Antenna Number	Antenna Make / Model	Antenna Centerline (ft)
A	1	RFS APXVAARR24_43-U-NA20	100
A	2	EMS RR90-17-XXDP (Dormant)	100
C	1	RFS APXVAARR24_43-U-NA20	100
C	2	EMS RR90-17-XXDP (Dormant)	100

*Table 2: Antenna Data*

All calculations were done with respect to uncontrolled / general population threshold limits.

Cable losses were factored in the calculations for this site. Since all **1900 MHz (PCS) & 2100 MHz (AWS)** radios are ground mounted the following cable loss values were used. For each ground mounted **1900 MHz (PCS)** radio there was **1.95 dB** of cable loss calculated into the system gains / losses for this site. For each ground mounted **2100 MHz (AWS)** radio there was **2.06 dB** of cable loss calculated into the system gains / losses for this site. These values were calculated based upon the manufacturers specifications for **160 feet** of **1-1/4"** coax.

# Transcom Engineering, Inc.

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## RESULTS

Per the calculations completed for the proposed T-MOBILE configurations *Table 3* shows resulting emissions power levels and percentages of the FCC's allowable general population limit.

Antenna ID	Antenna Make / Model	Frequency Bands	Antenna Gain (dBd)	Channel Count	Total TX Power (W)	ERP (W)	MPE %
Antenna A1	RFS APXVAARR24_43-U-NA20	1900 MHz (PCS) / 2100 MHz (AWS) / 600 MHz / 700 MHz	15.65 / 16.35 / 12.95 / 13.35	11	415	9,767.84	5.34
Antenna A2	EMS RR90-17-XXDP	Dormant	N/A	0	0	0.00	0.00
Sector A Composite MPE%							<b>5.34</b>
Antenna C1	RFS APXVAARR24_43-U-NA20	1900 MHz (PCS) / 2100 MHz (AWS) / 600 MHz / 700 MHz	15.65 / 16.35 / 12.95 / 13.35	11	415	9,767.84	5.34
Antenna C2	EMS RR90-17-XXDP	Dormant	N/A	0	0	0.00	0.00
Sector C Composite MPE%							<b>5.34</b>

*Table 3: T-MOBILE Emissions Levels*

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The Following table (*table 4*) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum T-MOBILE MPE contributions per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, both sectors have the same configuration yielding the same results on both sectors. *Table 5* below shows a summary for each T-MOBILE Sector as well as the composite MPE value for the site.

Site Composite MPE%	
Carrier	MPE%
T-MOBILE – Max Per Sector Value	<b>5.34 %</b>
Willington FD	0.49 %
AT&T	3.79 %
<b>Site Total MPE %:</b>	<b>9.62 %</b>

*Table 4: All Carrier MPE Contributions*

T-MOBILE Sector A Total:	5.34 %
T-MOBILE Sector C Total:	5.34 %
Site Total:	9.62 %

*Table 5: Site MPE Summary*

# Transcom Engineering, Inc.

Wireless Network Design and Deployment

FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated T-MOBILE sector(s). For this site, both sectors have the same configuration yielding the same results on both sectors.

T-MOBILE _ Frequency Band / Technology Max Power Values (Per Sector)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
T-Mobile 1900 MHz (PCS) LTE	4	937.69	100	15.26	1900 MHz (PCS)	1000	1.53%
T-Mobile 2100 MHz (AWS) LTE	2	1,611.21	100	13.11	2100 MHz (AWS)	1000	1.31%
T-Mobile 1900 MHz (PCS) GSM	1	351.63	100	1.43	1900 MHz (PCS)	1000	0.14%
T-Mobile 600 MHz LTE / 5G NR	2	788.97	100	6.42	600 MHz	400	1.61%
T-Mobile 700 MHz LTE	2	432.54	100	3.52	700 MHz	467	0.75%
						<b>Total:</b>	<b>5.34%</b>

*Table 6: T-MOBILE Maximum Sector MPE Power Values*



# Transcom Engineering, Inc.

Wireless Network Design and Deployment

## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-MOBILE facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-MOBILE Sector	Power Density Value (%)
Sector A:	5.34 %
Sector C:	5.34 %
T-MOBILE Maximum Total (per sector):	5.34 %
Site Total:	9.62 %
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **9.62 %** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



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