



Crown Castle
3 Corporate Park Drive, Suite 101
Clifton Park, NY 12065

August 7, 2018

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: Notice of Exempt Modification for Sprint DO Macro: 806383
Sprint Site ID: CT03XC205
56 Cosgrove Road, Willington, CT 06279
Latitude: 41° 53' 32.92"/ Longitude: -71° 15' 38.15"

Dear Ms. Bachman:

Sprint currently maintains six (6) antennas at the 125-foot level of the existing 140-foot self-support tower at 56 Cosgrove Road, Willington, CT. The tower is owned by Crown Castle. The property is owned by Ms. Isabella Drobney. Sprint now intends to replace six (6) antennas with six (6) new antennas. These antennas would be installed at the 125-foot level of the tower. Sprint also intends to install twelve (12) RRH's, four (4) Hybrid cables and remove six (6) coax cables.

The facility was constructed in or about 1986. An email was sent to Ms. Margaret Dupilka of the Land Use Department in the Town of Willington inquiring on original zoning documents. On March 13, 1997 the Connecticut Siting Council in Docket No. 58 approved modifications to the tower.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.S.C.A. § 16-50j-73, a copy of this letter is being sent to First Selectman- Ms. Erika Wiczenski & Ms. Susan Yorgensen of the Planning & Zoning Commission. A copy of this letter is also being sent to the property owner, Ms. Isabella Drobney. Crown Castle is the tower owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.

Melanie A. Bachman

August 7, 2018

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5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Sprint respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Jeffrey Barbadora.

Sincerely,



Jeffrey Barbadora

Real Estate Specialist

12 Gill Street, Suite 5800, Woburn, MA 01801

781-729-0053

Jeff.Barbadora@crowncastle.com

Attachments:

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Ms. Erika Wicewski, First-Selectman
Town of Willington
40 Old Farms Road
Willington, CT 06279
(860) 487-3100

Ms. Susan Yorgensen
Town of Willington Planning and Zoning Commission
40 Old Farms Road
Willington, CT 06279
(860) 487-3123

Property owner:
Ms. Isabella Drobney
56 Cosgrove Road
Willington, CT 06279

**TOWN OF WILLINGTON
APPLICATION FOR ZONING AND BUILDING PERMIT**

NOTICE: This application must be typed or printed in ink and filed

The undersigned hereby applies for a permit to: ERECT (X), ALTER (), ENLARGE (), REPAIR (), REMOVE (), DEMOLISH (), a building or structure herein described and in accordance with plans and specifications submitted in duplicate, herewith, as shown on accompanying survey map.

LOCATION: Whiffard Hill, Willington, CT E () W () S () N () side
(Street & Number or parcel and Number)
(If no number is assigned indicate above on which side of street)

LOT # _____ ZONE _____ INTENDED USE OF BUILDING mobile telephone equipment for
To house electrical equipment for mobile telephone system

SIZE OF LOT: #FT. FRONT _____ #FT. DEEP _____ AREA OF LOT _____

SIZE OF BUILDING: 15k _____ FT. X _____ FT. NO. OF STORIES _____ TOTAL FLOOR AREA 325.5 _____ SQ. FT.
SEE ATTACHED PLAN

DISTANCE OF BUILDING FROM LOT LINES: FRONT _____ SIDE _____ SIDE _____ REAR _____

WORK WILL START ON OR ABOUT November 1, 1986

OWNER OF LAND Martin W. & Isabelle N. Droney ADDRESS Corryville Rd., W. Willington
670 METRO MOBILE CTS OF NEW

OWNER OF BUILDING METRO MOBILE CTS of Hartford ADDRESS 5 Eversley Ave., Norwalk, CT

ARCHITECT CON-SERV, INC. 667-6662-1424 ADDRESS 7 Karen Cir., 8-3 Billerica,
740 A Main Street

BUILDER NOR-EAST ENGINEERING & CONSTRUCTION CORP. ADDRESS Woburn, MA 01801

HOME IMPROVEMENT CONT. RISE: N/A

This space to be used for Buildings to be Altered, Enlarged, Repaired, Removed or Demolished, Change in use and Special Permit Application.

STATE PROPOSED WORK TO BE DONE OR CHANGE IN USE IN DETAIL:
 (Attach separate sheet if necessary)

Terry Dill
Agar 1-800-218-2833

HOW IS BUILDING PRESENTLY OCCUPIED _____
 HOW WILL BUILDING BE OCCUPIED _____
 If building is to be demolished or removed, have you notified the proper authorities or utilities, do have their respective service connections, removed or sealed and plugged, Water (), Sewer (), Electric (), Gas ().

I hereby agree to comply with the requirements of the Zoning Regulations of the Town of Willington and the provisions of the State Building Code and all other State and Health Codes. The plans and specifications submitted herewith have been prepared in accordance with and are intended to meet these requirements. I hereby certify that I am familiar with the regulations and that I have employed competent persons to assist me in the preparation of the plans and specifications. I further agree that as owner or agent for the owner that the work will be done in accordance with these regulations and that I will employ whatever competent assistance or workmen as may be needed to carry on the work in accordance with the regulations and to remove, replace, or repair any work not found to be in accordance with the regulations by any authority.

Id. of Person responsible 617-331-4871

Date of Submission 10/30/86

IMPORTANT: I hereby certify that I have read and understand the foregoing information.

SIGNED [Signature]
 Owner or duly Authorized Agent (Affidavit)

ADDRESS _____

DO NOT WRITE BELOW THIS LINE

PERMIT NO. / DATE _____	Map/Parcel _____	Building <u>455</u>
COMMENTS/CONDITIONS:	Est. Cost. Const. <u>90,000</u>	Zoning <u>1B</u>
* CERTIFICATE OF SITING & COUNCIL OF CT. NEEDS (FUND)	Taxlot <u>28,000</u>	Driveway _____
	Footprint <u>12,000</u>	Heating _____
	Build <u>35,000</u>	Electrical _____
	Site <u>15,000</u>	Plumbing _____
	<u>90,000</u>	Septic _____
		Other _____
		(chk #/cash)

ZONING AGENT/DATE [Signature] 12/21/86 BUILDING OFFICIAL/DATE [Signature] 12-11-86

Barbadora, Jeff

From: Margaret DuPilka <mdupilka@willingtonct.org>
Sent: Tuesday, August 7, 2018 2:40 PM
To: Barbadora, Jeff
Subject: 56 Cosgrove Rd
Attachments: 20180807121251923.pdf

Note sure if this is what you are looking for. Peggy

-----Original Message-----

From: TOBCopyRoom@willingtonct.org [mailto:TOBCopyRoom@willingtonct.org]
Sent: Tuesday, August 07, 2018 12:13 PM
To: Margaret DuPilka <mdupilka@willingtonct.org>
Subject: Message from "TOB-COPYROOM-COPIER"

This E-mail was sent from "TOB-COPYROOM-COPIER" (MP C4503).

Scan Date: 08.07.2018 12:12:51 (-0400)
Queries to: TOBCopyRoom@willingtonct.org

Bell Atlantic NYNEX Mobile
20 Alexander Drive
P.O. Box 5029
Wallingford, CT 06492
Telephone: 203-269-8858

Jennifer Young Gaudet
Manager - Regulator

February 25, 1997

HAND DELIVERED

RECEIVED

FEB 25 1997

CONNECTICUT
SITING COUNCIL

Mr. Joel M. Rinebold, Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, Connecticut 06051

Re: Bell Atlantic NYNEX Mobile - GPS Antennas - Windsor, Vernon, ~~Wallingford~~ Rocky Hill,
Branford, Milford, Darien, Fairfield, Guilford and North Haven Cell Sites

Dear Mr. Rinebold:

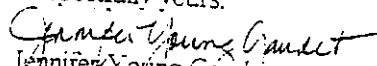
Bell Atlantic NYNEX Mobile ("BANM" or the "Company") plans to mount a small Global Positioning Satellite System ("GPS") receive-only antenna on its towers at the existing BANM facilities referenced above. Please accept this letter as notification, pursuant to R.C.S.A. § 16-50j-73, of construction which constitutes an exempt modification to each facility pursuant to R.C.S.A. § 16-50j-72(b). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to the chief elected official of each town in which a referenced facility is located, as shown on the attached Appendix A.

The addition of BANM's GPS antenna to each tower site does not constitute a modification as defined in C.G.S. § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed or altered. The addition of the GPS antenna will have no effect on any of the sites.

The planned change to each facility falls squarely within those activities provided for in R.C.S.A. § 16-50j-72(b). The height of the tower will be unaffected. In the case of monopoles, the GPS antenna will be placed on the top platform, where BANM's other antennas are located, and will not extend above the existing antennas. In the case of lattice towers, the GPS antenna will be placed on an approximately 2' sidearm at the 60' level of the tower. Attached as Appendix B is relevant tower and site data for each of the sites. The addition of the GPS antenna will have no effect on the site boundary or noise levels at any of the sites. Nor will there be any effect on the total radio frequency electromagnetic radiation power density at any of the sites, because the GPS antennas are receive-only antennas.

BANM therefore respectfully requests the Council's acknowledgment under R.C.S.A. § 16-50j-72(b) of the addition of the GPS antennas at the referenced facilities.

Respectfully yours,


Jennifer Young Gaudet
Manager - Regulator

Enclosures

Site	Address	Type of Tower	Tower Height	Docket
Windsor	482 Pigeon Hill Road	lattice		
Vernon	South Street	lattice	160'	58
Willington	Cosgrove Road	lattice	130'	58
Rocky Hill	France Street	monopole	140'	58
Branford	1801 North Main Street	monopole	140'	58
Milford	423 Oronoque Road	monopole	110'	122
Darien	Ledge Road	monopole	100'	56
Fairfield	281 Woodhouse Road	monopole	100'	155
Guilford	131 Manor Road	monopole	160'	86
North Haven	117 Washington Avenue	monopole	150'	56
		monopole	120'	117



STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL

10 Franklin Square
New Britain, Connecticut 06051
Phone: (860) 827-2935
Fax: (860) 827-2950

Exemp Mod.

March 13, 1997

Jennifer Young Gaudet
Regulatory Manager
Bell Atlantic NYNEX Mobile
20 Alexander Drive, P.O. Box 5029
Wallingford, CT 06492

Re: ~~DOCKET NO. 54~~ - Bell Atlantic NYNEX Mobile Certificate of Environmental Compatibility and Public Need for telecommunications facilities in New Haven County. Notice of Intent to Modify Guilford & Milford Facilities

DOCKET NO. 58 - Bell Atlantic NYNEX Mobile Certificate of Environmental Compatibility and Public Need for telecommunications facilities in the Towns of Glastonbury, Haddam, Hartford, Portland, Rocky Hill, Somers, Willington, Vernon, and Windsor Connecticut. Notice of Intent to Modify Rocky Hill, Vernon, [REDACTED] and Windsor Facilities.

DOCKET NO. 86 - Bell Atlantic NYNEX Mobile, Certificate of Environmental Compatibility and Public Need for telecommunications facilities in the Towns of Greenwich and Fairfield. Notice of Intent to Modify Fairfield Facility.

DOCKET NO. 117 - Bell Atlantic NYNEX Mobile Certificate of Environmental Compatibility and Public Need for a telecommunications facility in the Town of North Haven, Connecticut. Notice of Intent to Modify Facility.

DOCKET NO. 122 - Bell Atlantic NYNEX Mobile Certificate of Environmental Compatibility and Public Need for a telecommunications facility in the Town of Branford, Connecticut. Notice of Intent to Modify Facility.

DOCKET NO. 155 - Bell Atlantic NYNEX Mobile Certificate of Environmental Compatibility and Public Need for a telecommunications facility located in the Town of Darien, Connecticut. Notice of Intent to Modify Facility.

Dear Ms. Gaudet:

At a public meeting held on March 12, 1997, the Connecticut Siting Council (Council) acknowledged your notice to modify existing telecommunications facilities in Guilford, Milford, Rocky Hill, Vernon, Willington, Windsor, Fairfield, North Haven, Branford, and Darien, Connecticut, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies.

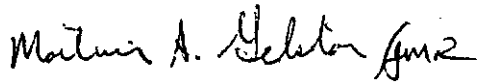
The proposed modifications are to be implemented as specified in your notice dated February 25, 1997. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequency electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. These facilities have been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequency now used on this tower. Any additional change to these facilities will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Science and Technology, Bulletin No. 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such

Jennifer Young Gaudet
Page 2

failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Thank you for your attention and cooperation.

Very truly yours,

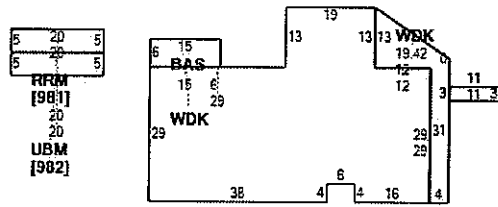


Mortimer A. Gelston
Chairman

MAG/RKE/ss

- c: Honorable Francis J. Brady, Mayor of Windsor
- Honorable Tony Muro, Mayor of Vernon
- Honorable John Patton, First Selectman of Willington
- Honorable Donald W. Unwin, Mayor of Rocky Hill
- Honorable Dominic A. Buonocore, First Selectman of Branford
- Honorable Frederick L. Lisman, Mayor of Milford
- Honorable Henry M. Sanders, First Selectman of Darien
- Honorable Paul A. Audley, First Selectman of Fairfield
- Honorable Edward J. Lynch, First Selectman of Guilford
- Honorable Anthony P. Rescigno, First Selectman of North Haven

Exterior Wall 1	Stucco/Masonry
Exterior Wall 2	Brick Veneer
Roof Structure:	Gable or Hip
Roof Cover	Asbestos Shing
Interior Wall 1	Drywall/Sheet
Interior Wall 2	Wood Panel
Interior Flr 1	Hardwood
Interior Flr 2	Linoleum
Heat Fuel	Oil
Heat Type:	Hot Water
AC Type:	None
Total Bedrooms:	4 Bedrooms
Total Bthrms:	2
Total Half Baths:	0
Total Xtra Fixtrs:	1
Total Rooms:	6
Bath Style:	Average
Kitchen Style:	Average
Fireplaces	2
Basement Garage	2



Building Sub-Areas (sq ft)			Legend
Code	Description	Gross Area	Living Area
BAS	First Floor	1,963	1,963
RRM	Rec Room	0	0
UBM	Unfinished Basement	0	0
WDK	Wood Deck	326	0
		2,289	1,963

Extra Features

Extra Features		Legend
No Data for Extra Features		

Land

Land Use

Use Code 1010
 Description Single Fam MDL-01
 Zone R80
 Neighborhood 110
 Alt Land Appr No
 Category

Land Line Valuation

Size (Acres) 4.2
 Frontage
 Depth
 Assessed Value \$45,950
 Appraised Value \$65,650

Outbuildings

Outbuildings						Legend
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
SHD1	SHED FRAME			312.00 S.F.	\$1,870	1
FGR3	GARAGE-POOR			1424.00 S.F.	\$10,250	1

Valuation History

Appraisal			
Valuation Year	Improvements	Land	Total
2016	\$180,500	\$65,650	\$246,150

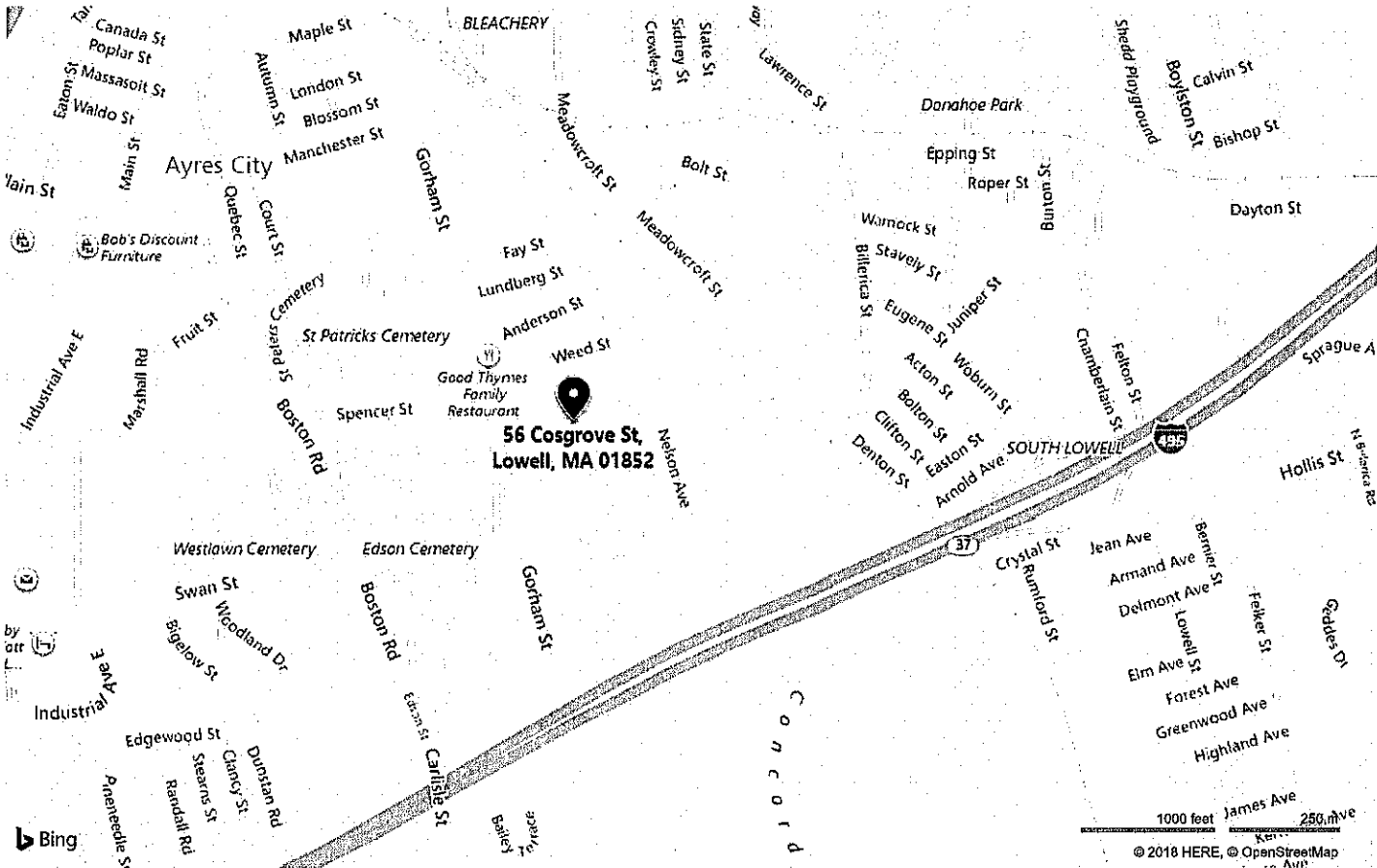
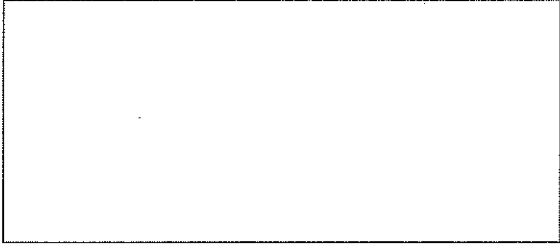
Assessment			
Valuation Year	Improvements	Land	Total
2016	\$126,360	\$45,950	\$172,310

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56 Cosgrove St, Lowell, MA 01852

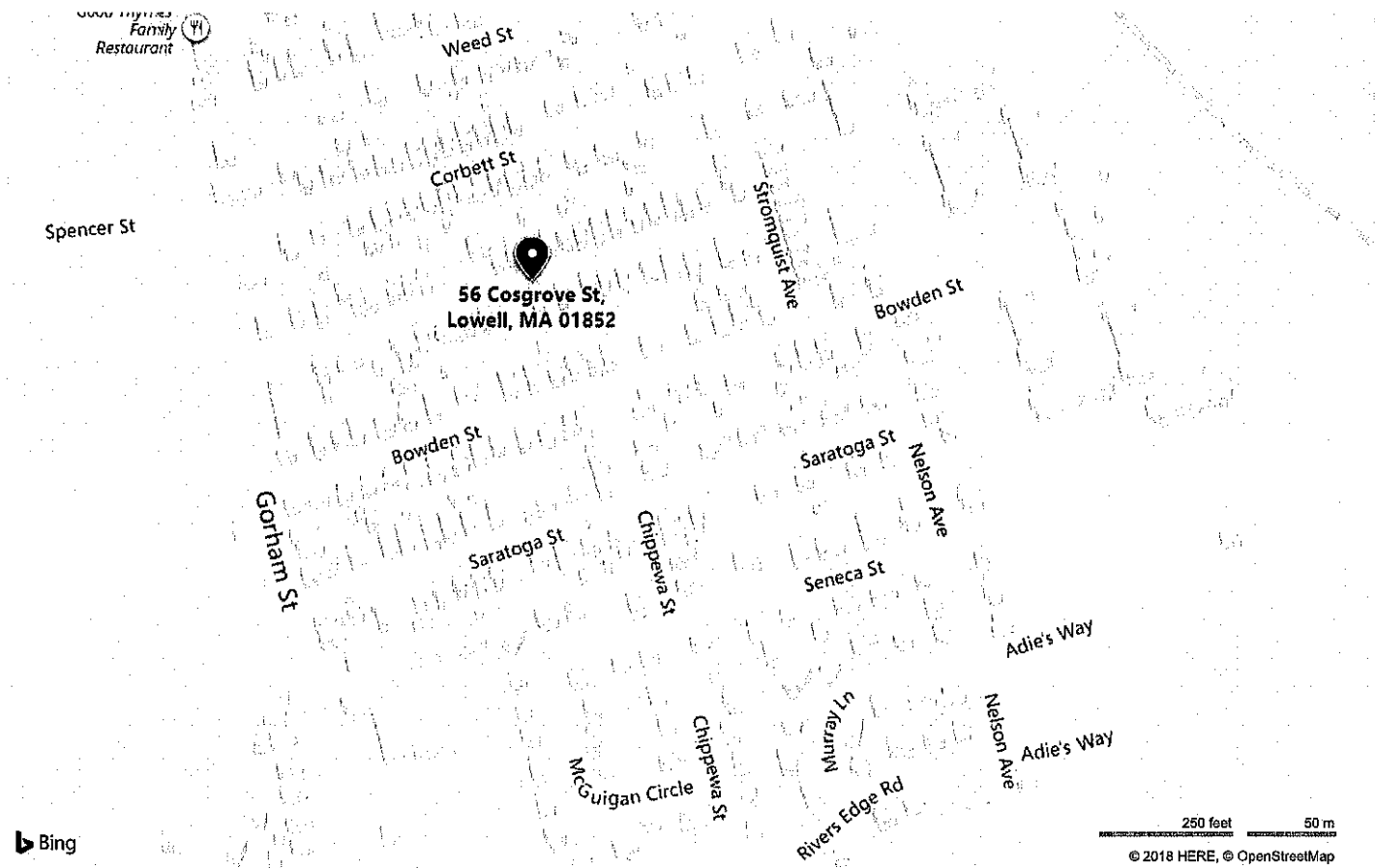
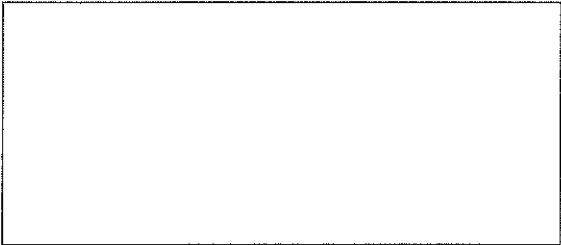
Location: 42.619881485361084, -71.30272803319164

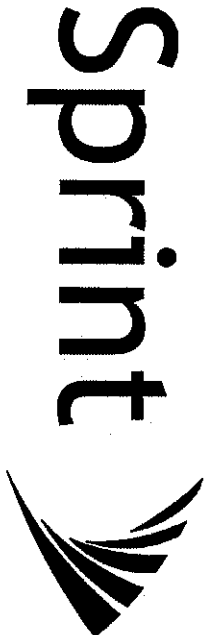




56 Cosgrove St, Lowell, MA 01852

Location: 42.619881485361084, -71.30272803319164





PROJECT: DO MACRO UPGRADE
SITE NAME: WILLINGTON (CROWN)
SITE CASCADE: CT03XC205
SITE NUMBER: 806383
SITE ADDRESS: COSGROVE ROAD
 WEST WILLINGTON, CT 06279
SITE TYPE: SELF SUPPORT TOWER
MARKET: NORTHERN CONNECTICUT

SITE INFORMATION

TOWER OWNER:
 CROWN ATLANTIC COMPANY, LLC
 2000 CORPORATE DRIVE
 CHESTERBORO, PA 15317
 (717) 485-6555

LATITUDE (NAD83):
 41° 57' 32.02" N
 41.952576

LONGITUDE (NAD83):
 72° 15' 35.15" W
 -72.25975

COUNTY:
 TOLLAND

ZONING JURISDICTION:
 CONVENTIST STRIKE COUNCIL

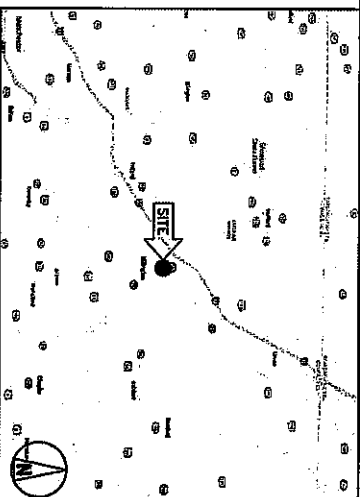
ZONING DISTRICT:
 RESIDENTIAL

POWER COMPONENT:
 CONVENTIST LINE & POWER
 (800) 286-2000

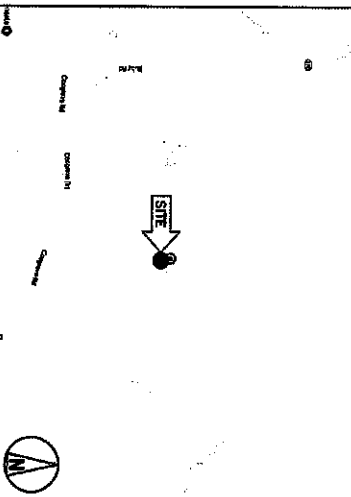
SPRINT CONSTRUCTION:
 TBD

GROWN PHL:
 SCOTT WARRICK
 (201) 239-9223

AREA MAP



LOCATION MAP



PROJECT DESCRIPTION

SPRINT PROPOSES TO MODIFY AN EXISTING UNMANAGED TELECOMMUNICATIONS FACILITY:

- INSTALL 23 EQUIPMENT RISES EXISTING N.W. MAINS CABINET
- REMOVE (0) PANEL ANTENNAS
- INSTALL (0) PANEL ANTENNAS (2 800/1900, 3 2500)
- INSTALL (12) 800s ON TOWER (6 800, 3 1900, 3 2500)
- REMOVE (0) COAX CABLES
- INSTALL (4) HYBRID CABLES

THESE PLANS HAVE BEEN DEVELOPED FOR THE MODIFICATION OF AN EXISTING UNMANAGED TELECOMMUNICATIONS FACILITY OWNED OR LEASED BY SPRINT. THIS PROJECT IS SUBJECT TO THE REVIEW AND APPROVAL OF THE LOCAL AUTHORITIES INCORPORATED. THE SCOPE OF WORK IS SUBJECT TO THE REVIEW AND APPROVAL FOR CONSTRUCTION UNLESS ACCOMPANIED BY A PASSING STRUCTURAL, ELECTRICAL, AND MECHANICAL ANALYSIS PREPARED BY A LICENSED STRUCTURAL ENGINEER, STRUCTURAL ANALYSIS MUST INCLUDE BOTH STRUCTURE AND WIND.

APPLICABLE CODES

- ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE APPLICABLE CODES AND REGULATIONS. THE APPLICABLE CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES NOTING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:
- INTERNATIONAL BUILDING CODE (2015 IBC)
 - IRC-222-6 ON LATEST EDITION
 - 2011 NATIONAL ELECTRIC CODE ON LATEST EDITION
 - ANY OTHER NATIONAL OR LOCAL APPLICABLE CODES.
 - LOCAL DESIGN ORDINANCES
 - LOCAL DESIGN ORDINANCES
 - CITY/COUNTY ORDINANCES



DRAWING INDEX

SHEET NO.	SHEET TITLE	REV.
T-1	TITLE SHEET & PROJECT DATA	0
SP-1	SPRINT SPECIFICATIONS	0
SP-2	SPRINT SPECIFICATIONS	0
SP-3	SPRINT SPECIFICATIONS	0
A-1	SITE PLAN	0
A-2	TOWER ELEVATION & CABLE PLAN	0
A-3	EQUIPMENT LAYOUT & MOUNTING DETAILS	0
A-4	CABLE DETAILS	0
A-5	FLUORESCENT DETAILS	0
A-6	ELECTRICAL & GROUNDING DETAILS	0
E-1	ELECTRICAL & GROUNDING DETAILS	0
E-2	ELECTRICAL & GROUNDING DETAILS	0

TAKE HEREBY FOR:

Sprint
 5500 Sprint Parkway
 Overland Park, Kansas 66251

PLANS PREPARED BY:

INFINIGY

FROM ZERO TO INFINIGY

The solutions are endless

1033 Franklin Street
 02125
 Phone: 617-452-2772 | Fax: 617-452-2773
 www.infinigy.com

MAIN PARTNER:

CROWN CASTLE

REGISTERED LICENSE:

STATE OF CONNECTICUT

PROFESSIONAL ENGINEER

SCOTT WARRICK

02019 REG. 8
 2/1/19 REG. 1

THESE DOCUMENTS ARE CONFIDENTIAL AND ARE REPRODUCED, ASSASSINATED OR DISTRIBUTED WITHOUT THE EXPRESS WRITTEN CONSENT OF SPRINT.

REVISIONS:

DESCRIPTION	DATE	BY	REV.
ISSUED FOR CONSTRUCTION	2/20/19	REG. 8	0
ISSUED FOR REVIEW	2/1/19	REG. 1	0

WILLINGTON (CROWN)

SITE CASCADE:
 CT03XC205

SITE ADDRESS:
 COSGROVE ROAD
 WEST WILLINGTON, CT 06279

TITLE SHEET & PROJECT DATA

SHEET NUMBER:
 T-1

1. PERFORM ANY REQUIRED SITE ENVIRONMENTAL MITIGATION.
2. PREPARE GROUND SITE: PROVIDE DE-DRUMBING, AND ROUGH AND FINI GROUNDING, AND COMPOUND SURFACE TREATMENTS.
3. MANAGE AND CONTROL ALL ACTIVITIES FOR INSTALLATION OF UTILITIES INCLUDING ELECTRICAL AND TOWER BASINS.
4. INSTALL UNDERGROUND FACILITIES INCLUDING UNDERGROUND POWER AND UNDERGROUND CONDUITS AND UNDERGROUND GROUNDING SYSTEM.
5. INSTALL ABOVE GROUND GROUNDING SYSTEMS.
6. PROVIDE NEW HIVE INSTALLATIONS AND MODIFICATIONS.
7. INSTALL "H-FRAME" CHIMNEYS AND SHELTERS AS REQUIRED.
8. INSTALL RAILS, ACCESS WAYS, CURBS AND DRAINS AS INDICATED.
9. ACCOMPLISH REQUIRED MODIFICATION OF EXISTING FACILITIES.
10. PROVIDE ANTENNA SUPPORT STRUCTURE FOUNDATIONS.
11. PROVIDE STAIRS AND EQUIPMENT PLATFORMS.
12. INSTALL COMPOUND FENCING, SOFT SHEDDING, LANDSCAPING AND ACCESS BARBERS.
13. PERFORM INSPECTION AND MATERIAL TESTING AS REQUIRED HEREINAFTER.
14. CONDUCT SITE RESISTANCE TO EARTH TESTING AS REQUIRED HEREINAFTER.
15. INSTALL FIBER GENERATOR SETS AND OTHER STANDBY POWER SOLUTIONS.
16. INSTALL TOWER ANTENNA SUPPORT STRUCTURES AND PLATFORMS ON EXISTING TOWERS AS REQUIRED.
17. INSTALL GUY SITE GROUND, IMPROVING GUY, CRAWL, MANHOLE, ANTENNAS, CROSS BOND COUNTERS, TOWER TOP WALKWAYS, LOW NOISE WALKWAYS AND RELATED EQUIPMENT.
18. PERFORM TOWERING AND GUY OUT AT ANY CONSTRUCTION CONTROL DOCUMENTS THAT MAY BE REQUIRED BY GOVERNMENT AGENCIES AND LANDLORDS.
19. PERFORM ANTIWIND AND CANK SWEEP TESTING AND MAKE ANY AND ALL NECESSARY CORRECTIONS.
20. REMAIN ON SITE LOGGED THROUGHOUT HAND-OFF AND RESPONSE TO ASSET AS NEEDED UNTIL SITE IS DEEMED SUBSTANTIALLY COMPLETE AND PLACED ON AIR.

2.3 GENERAL REQUIREMENTS FOR CIVIL CONSTRUCTION:

- A. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIALS, EXCEPT FOR THE REMAINING RUBBER, WIPEDOWN, TENDONARY FACILITIES, AND SURPLUS MATERIALS.
- B. EMPLOYER TRUCKS SHALL AT ALL TIMES BE MAINTAINED BROAD CLEAN AND CLEAR OF DEBRIS.
- C. CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO DISCOVER AND LOCATE ANY UNDERGROUND UTILITIES.
1. IN THE EVENT CONTRACTOR ENCOUNTERS ANY HAZARDOUS CONDITION WHICH HAS NOT BEEN ADVISED OR OTHERWISE LISTED, CONTRACTOR AND ALL OTHER PERSONS SHALL IMMEDIATELY STOP WORK IN THE AFFECTED AREA AND BE REMOVED EXCEPT BY WRITTEN AUTHORITY OF COMPANY.
2. CONTRACTOR AGREES TO USE ONE WHILE ON THE SITE AND SHALL NOT TAKE ANY OTHER ACTION THAT COULD BE CONSIDERED AS A HAZARDOUS CONDITION TO BE FURTHER RELEASED IN THE ENVIRONMENT, OR TO FURTHER EXPOSE INDIVIDUALS TO THE HAZARD.
- D. CONTRACTOR'S ACTIVITIES SHALL BE RESTRICTED TO THE PROJECT LIMITS. SHOULD CONTRACTOR REQUIRE THE FREEDOM TO EXCEED CONTRACTOR'S ACTIVITIES, CONTRACTOR SHALL IMMEDIATELY RETURN TO ORIGINAL CONDITION.
- E. CONDUIT TRENCH AS REQUIRED HEREIN.

2.3.3 DELIVERABLES:

- A. CONTRACTOR SHALL REVIEW, APPROVE, AND SUBMIT TO SPRINT SHOP DRAWINGS, PROJECT DATA, SURVEYS, AND SURFACE SCHEMATICS AS REQUIRED HEREINAFTER.
- B. PROJECT DOCUMENTATION INCLUDING, BUT NOT LIMITED TO, THE FOLLOWING: DOCUMENTATION SHALL BE FORWARDED IN ORIGINAL, FORWARD AND/OR UPLOADED INTO SRS.
1. ALL CORRESPONDENCE AND PRELIMINARY CONSTRUCTION REPORTS.
2. PROJECT PROGRESS REPORTS.
3. CIVIL CONSTRUCTION START DATE (POPULATE FIELD IN SRS AND/OR FORWARD NOTIFICATION).
4. ELECTRICAL SERVICE COMPLETION DATE (POPULATE FIELD IN SRS AND/OR FORWARD NOTIFICATION).

SECTION 01 400 - SUBSTITUTALS & TESTS

PART 1 - GENERAL

- 1.1 THE WORK, THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONJUNCTION WITH THE OTHER CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.
- 1.2 RELATED DOCUMENTS:
 - A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SPECIFICATION.
 - B. SPRINT STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES ARE INCLUDED IN AND MAKE A PART OF THESE SPECIFICATIONS HEREIN.
- 1.3 SUBSTITUTALS:
 - A. THE WORK IN ALL SECTIONS SHALL COMPLY WITH THE CONSTRUCTION DRAWINGS AND THESE SPECIFICATIONS.
 - B. SUBMIT THE FOLLOWING TO COMPANY REPRESENTATIVE FOR APPROVAL:
 1. CONCRETE MIX DESIGNS FOR TOWER FOUNDATIONS, ANCHORS BESS, AND CONCRETE STAIRS.
 2. CONCRETE BEAM TESTS AS SPECIFIED HEREIN.
 3. SPECIAL FINISHES FOR ANCHOR SPACES, IF ANY.
 4. ALL EQUIPMENT AND MATERIALS SO IDENTIFIED ON THE CONSTRUCTION DRAWINGS.
 5. CHEMICAL GROUNDING DESIGN.
 - C. ALTERNATES: AT THE COMPANY'S REQUEST, ANY ALTERNATES TO THE MATERIALS OR METHODS SPECIFIED SHALL BE SUBMITTED TO SPRINT'S CONSTRUCTION MANAGER FOR APPROVAL, PRIOR TO BEING SHIPPED TO SITE. SPRINT WILL REVIEW AND APPROVE OR DENY THE ALTERNATE. THE REVIEW FOR APPROVAL SHALL INCLUDE A STATEMENT OF COST REDUCTION PROPOSED FOR USE OF ALTERNATE PRODUCT.
- 1.4 TESTS AND INSPECTIONS:
 - A. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROJECT DOCUMENTATION.
 - B. CONTRACTOR SHALL ACCOMPLISH TESTING INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
 1. CANK SWEEP AND PEEK TESTS PER GROUND VERSION OF SPRINT'S 3-to-20 ANTENNA LINE ACCEPTANCE STANDARDS.
 2. AFE, STAIRS AND PORTNIT USING ELECTRONIC COMMERCIAL WALK-FOR-THE-FURGESE ANTENNA ALIGNMENT TOOL.
 3. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL CORRECTIONS TO ANY DEFECTIVE WORK IDENTIFIABLE IN SITE INSPECTION ACTIVITIES AND/OR AS A RESULT OF TESTING.
 - C. REQUIRED CLASSIFIED DOCUMENTATION INCLUDES, BUT IS NOT LIMITED TO THE FOLLOWING:
 1. AZIMUTH, DOWN TILT, AND UPLINK REPORT FROM ANTENNA ALIGNMENT TOOL.
 2. SPRINT RISK ASSESSMENT REPORT, DOWN TILT, AND UPLINK REPORT FROM ANTENNA ALIGNMENT TOOL.
 3. SWANEE BARCODE PHOTOGRAPHS OF TOWER TOP AND UNACCESSIBLE EQUIPMENT.
 4. PER SCOM OF RESULTS PRODUCED IN FIELD.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION

- 1.6 INTERVENTION: PERFORM ALL INTERVENTION ACTIVITIES AS REQUIRED BY APPLICABLE WORK.
- 1.5 COMMISSIONING: PERFORM ALL COMMISSIONING AS REQUIRED BY APPLICABLE WORK.
- 1.4 INTERVENTION: PERFORM ALL INTERVENTION ACTIVITIES AS REQUIRED BY APPLICABLE WORK.
- 3.1 REQUIREMENTS FOR TESTING:
 - A. THIRD PARTY TESTING AGENCY:
 1. WHEN THE USE OF A THIRD PARTY INDEPENDENT TESTING AGENCY IS REQUIRED, THE TESTING AGENCY SHALL BE SELECTED BY SPRINT OR A THIRD PARTY UNDERSTANDING OF LOCAL AVAILABLE MATERIALS, INCLUDING THE SOIL, ROCK, AND GROUNDWATER CONDITIONS.
 2. THE THIRD PARTY TESTING AGENCY IS TO BE FAILURE WITH THE APPLICABLE REQUIREMENTS FOR THE TESTS TO BE DONE. EQUIPMENT TO BE USED, AND ASSIGNED FIELD AND SAFETY TESTS.
 3. EXPERIENCE IN SOILS, CONCRETE, WELDING, AGREEMENT, AND AIRQUALITY TESTING USING SPT, ASD, AND OTHER METHODS IS NEEDED.
 4. EXPERIENCE IN SOILS, CONCRETE, WELDING, AGREEMENT, AND AIRQUALITY TESTING USING SPT, ASD, AND OTHER METHODS IS NEEDED.
 - B. CONTRACTOR SHALL ACCOMPLISH TESTING INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
 1. CONCRETE CANKER BREAK TESTS FOR THE TOWER AND ANCHOR FOUNDATIONS AS SPECIFIED IN SECTION: PORTLAND CEMENT CONCRETE PAVEMENT.
 2. ASBESTOS REMOVAL CONCEPTS, BUSINESS, SURFACE SANDPAPER, AND PORTLAND CEMENT TESTING AS SPECIFIED IN SECTION: HOT MIX ASPHALT PAVEMENT.
 3. FIELD QUALITY CONTROL TESTING AS SPECIFIED IN SECTION: PORTLAND CEMENT CONCRETE PAVEMENT.
 4. TESTING REQUIRED UNDER SECTION: ASBESTOS BASE FOR ACCESS ROADS.
 5. STRUCTURAL, BAROTEL COMPACTION TESTS FOR THE TOWER FOUNDATION.
 6. SITE RESISTANCE TO EARTH TESTING PER EXHIBIT: CELL SITE CONSTRUCTION SYSTEM DESIGN.
 7. ANTENNA AND CANK SWEEP TESTS PER EXHIBIT: ANTENNA TRANSMISSION LINE ACCEPTANCE STANDARDS.
 8. GROUNDING AT ANTENNA WASTES FOR GRS AND ANTENNAS.
 9. ALL OTHER TESTS REQUIRED BY COMPANY OR UNDERGROUND.
- 3.2 REQUIRED INSPECTIONS:
 - A. SCHEDULE INSPECTIONS WITH COMPANY REPRESENTATIVE.
 - B. CONDUIT INSPECTIONS INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
 1. GROUNDING SYSTEM INSTALLATION FROM TO EARTH CONCEPT OR SHOWN REPRESENTATIVE.
 2. TESTING FOR CONCRETE AND REBAR PLACEMENT PRIOR TO POUR DOCUMENTED WITH DIGITAL PHOTOGRAPHS BY CONTRACTOR APPROVED BY AFE OR SPRINT REPRESENTATIVE.
 3. COMPARISON OF SPECTRAL MATERIALS, AGGREGATE BASE FOR ROADS, PAVES AND ANCHORS, SANDWICH PANELS, AND SWIFT DIAPHRAGM FOR CONCRETE AND WOOD PILES BY INDEPENDENT THIRD PARTY AGENCY.
 4. PRE- AND POST-CONSTRUCTION ROOFTOP AND STRUCTURAL INSPECTIONS ON EXISTING FACILITIES.
 5. TOWER ERECTION SECTION STAGING AND PLATFORM ATTACHMENT DOCUMENTED BY DIGITAL PHOTOGRAPHS BY THIRD PARTY AGENCY.
 6. ANTENNA ZEMATEL, DOWN TILT AND PER SWANEE TOOL SWANEE PHOTOGRAPHS - ANTENNA ALIGNMENT TOOL (AW)

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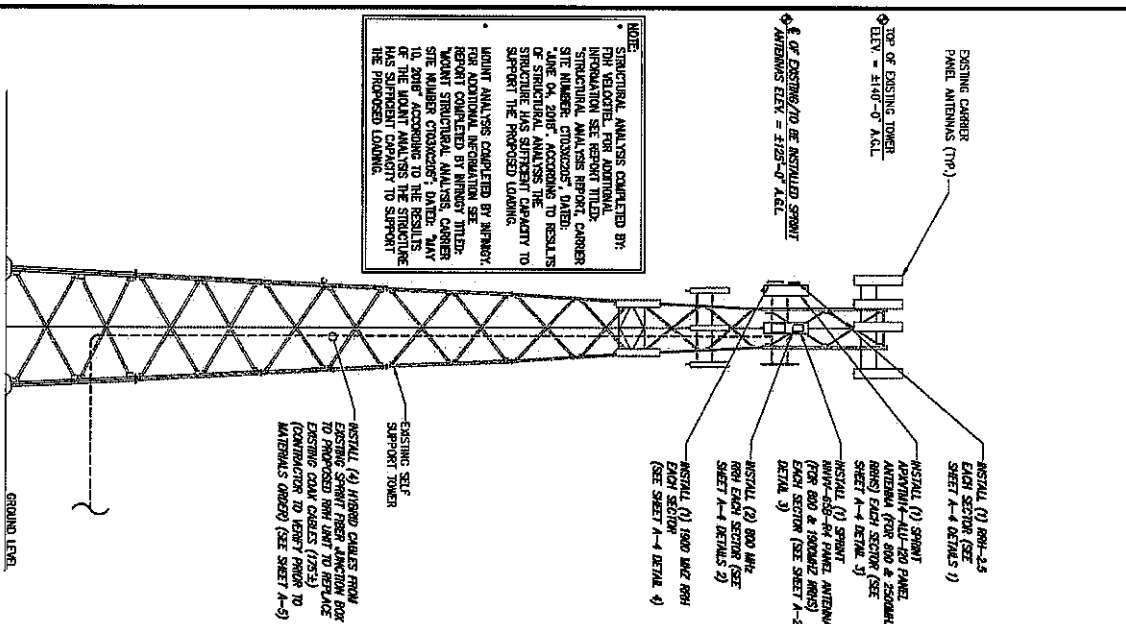
PROFESSIONAL ENGINEER
 STATE OF MASSACHUSETTS
 No. 27808
 JOHN S. STEINBERG
 CIVIL ENGINEER

REVISIONS:

NO.	DATE	BY	REASON
1	07/07/10	BO	ISSUED FOR CONSTRUCTION
2	07/07/10	BO	ISSUED FOR REVIEW

WILLINGTON (CROWN)
 UTM COORDINATE: CT03XC205
 CONCORD ROYAL ROAD
 WEST WILLINGTON, CT 06279
 SHEET SPECIFICATIONS
 SHEET NUMBER: SP-2

NOTE: SHEET 2 OF A-1 FOR ANTENNA LAYOUT



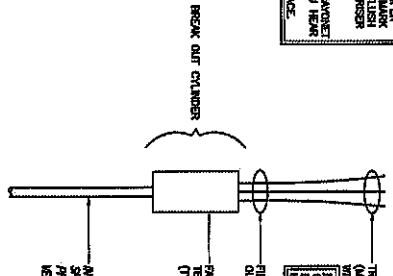
NOTE: STRUCTURAL ANALYSIS COMPLETED BY: FOR VERTICAL, FOR ADDITIONAL INFORMATION SEE REPORT TITLED: "STRUCTURAL ANALYSIS REPORT CARRIER SITE NUMBER: C0300205", DATED: JUNE 04, 2008. ACCORDS TO RESULTS OF STRUCTURAL ANALYSIS THE STRUCTURE HAS SUFFICIENT CAPACITY TO SUPPORT THE PROPOSED LOADING.

LOADING ANALYSIS COMPLETED BY: REINFORCING FOR ADDITIONAL INFORMATION SEE REPORT TITLED: "LOADING ANALYSIS REPORT CARRIER SITE NUMBER: C0300205", DATED: MAY 10, 2008. ACCORDS TO THE RESULTS OF THE LOADING ANALYSIS THE STRUCTURE HAS SUFFICIENT CAPACITY TO SUPPORT THE PROPOSED LOADING.

TOWER ELEVATION

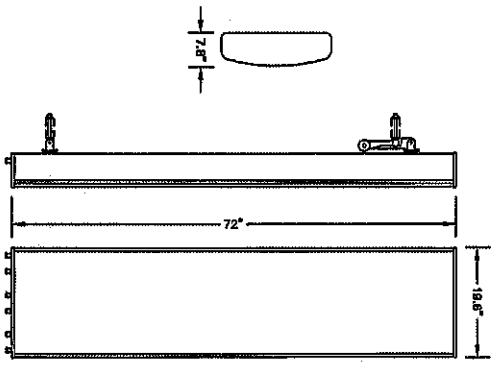
NO SCALE 1

NOTE: CONTRACTOR TO LINE UP WHITE MARKINGS ON RAMPERS AND RESET THE CONNECTION TO THE UPPER CONNECTOR. PUSH THE WHITE MARK ON THE UPPER CONNECTOR FLUSH WITH THE LOWER CONNECTOR FLUSH WITH THE LOWER CONNECTOR. A CLICK SOUND TO ENSURE A PROPER CONNECTION IS IN PLACE.



HYBRID BREAKOUT DETAIL

NO SCALE 2



ANTENNA

NO SCALE 3

ANTENNA COMPOSCOPE: HWV--65B-R4

ROD/CABLE MATERIAL: FIBERGLASS

ROD/CABLE COLOR: LIGHT GREY

DIMENSIONS: 72x18.6x1.8"

WEIGHT: 77.4 lb

CONNECTORS: (8) 4.3-10 DIN FEMALE

PLANS REVISIONS FOR:

Sprint

6880 Sprint Parkway
Oxford Park, Georgia 30251

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PLANNING:

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ENGINEERING LICENSE:

STATE OF CONNECTICUT

PROFESSIONAL ENGINEER

DAVE BY REV

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REVISION:	DESCRIPTION:	DATE:	BY:	REV:
1	ISSUE FOR CONSTRUCTION	6/24/08	DB	0
2	ISSUE FOR REVIEW	6/24/08	DB	1

PROJECT NAME:

WILLINGTON (CROWN)

SITE ADDRESS:

CT03XC205

PROJECT ADDRESS:

**COSGROVE ROAD
WEST WILLINGTON, CT 06279**

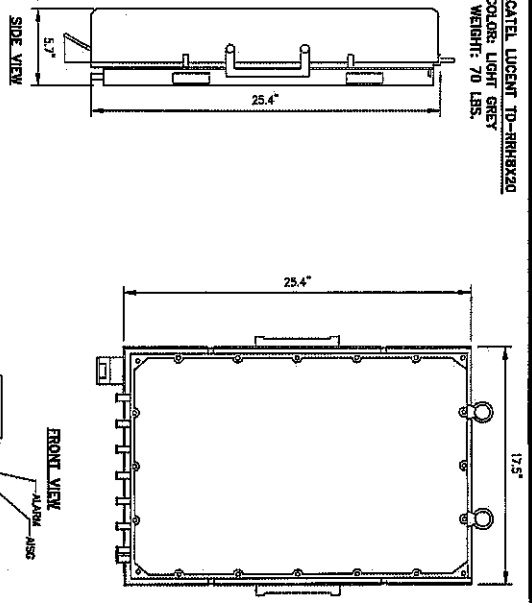
SHEET DESCRIPTION:

**TOWER ELEVATION
& CABLE PLAN**

SHEET NUMBER:

A-2

RRH: ALCATEL LUCENT TD-RRH820
 COLOR: LIGHT GREY
 WEIGHT: 70 LBS.



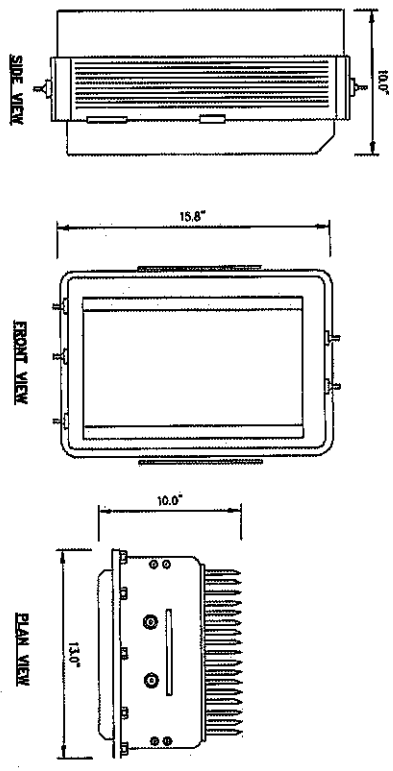
NOTES
 CONSULT WITH MANUFACTURER'S INSTRUCTIONS TO ENSURE THAT ALL RRH'S RECEIVE ELECTRICAL POWER WITHIN 24 HOURS OF BEING REMOVED FROM THE MANUFACTURER'S PACKAGING. DO NOT OPEN RRH PACKAGES IN THE RAIL.

2.5. RRH'S

NO SCALE

1

RRH: ALCATEL LUCENT RRH 800 MHz 2-50W
 COLOR: LIGHT GREY
 WEIGHT: 53 LBS.



NOTES
 CONSULT WITH MANUFACTURER'S INSTRUCTIONS TO ENSURE THAT ALL RRH'S RECEIVE ELECTRICAL POWER WITHIN 24 HOURS OF BEING REMOVED FROM THE MANUFACTURER'S PACKAGING. DO NOT OPEN RRH PACKAGES IN THE RAIL.

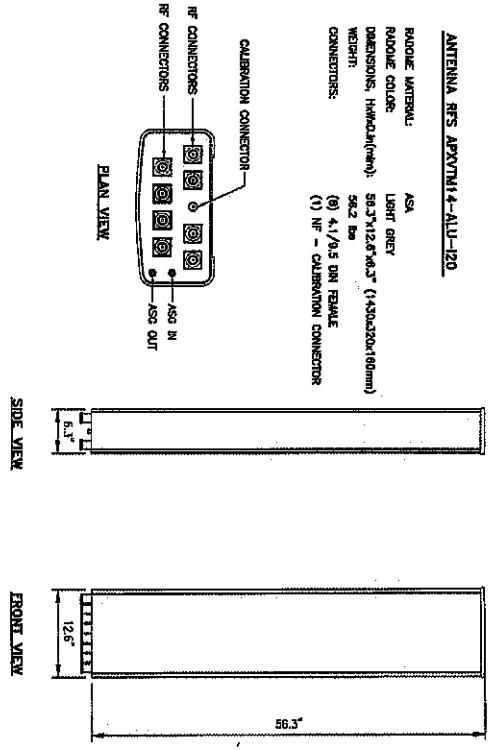
800 MHz RRH

NO SCALE

2

ANTENNA RFS APPROXIM4-ALL-120

RADIOME MATERIAL: ASA
 RADIOE COLOR: LIGHT GREY
 DIMENSIONS: HxWxD(LxHxW) 56.3"x12.6"x6.3" (1430x320x160mm)
 WEIGHT: 56.2 lbs
 CONNECTORS: (0) 41/16.5 DIN FEMALE CONNECTORS
 (1) NF - CALIBRATION CONNECTOR

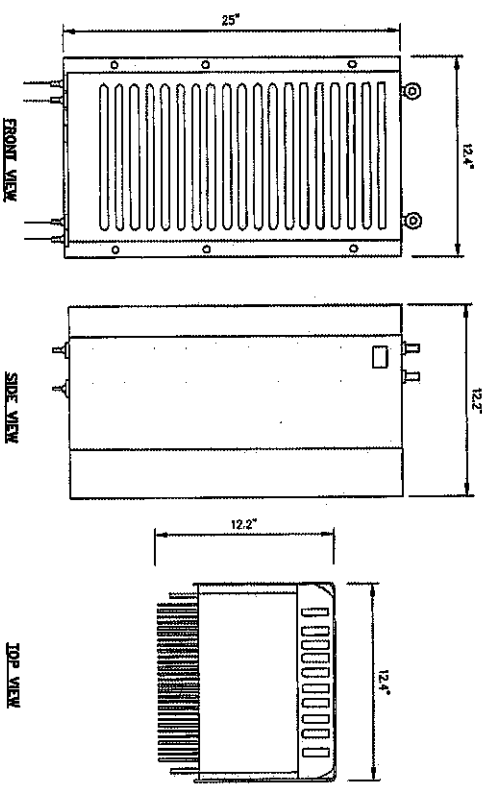


2.5. ANTENNA

NO SCALE

3

RRH: ALCATEL LUCENT 1800 MHz
 COLOR: LIGHT GREY
 WEIGHT: 70 LBS.
 (INCLUDING OPTIONAL SOLAR SHIELD)



1800 MHz's

NO SCALE

4

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 800 Sprint Center
 Overland Park, Kansas 66201

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CROWN CASTLE

STATE OF CONNECTICUT
 JOHN S. STEVENS
 No. 24705
 405-2088
 ENGINEERING

REVISIONS:
 DATE BY REV
 DESCRIPTION

DATE	BY	REV
07/24/05	MS	0
08/07/05	MS	1

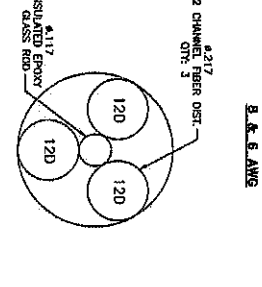
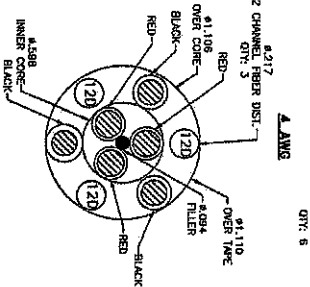
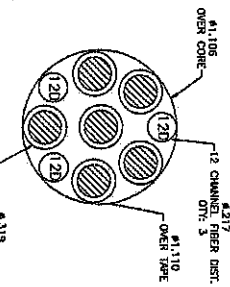
WILLINGTON (CROWN)
 DATE QUANTITY
 CT03XC205

COSGROVE ROAD
 WEST WILLINGTON, CT 06279

ROUPEMENT & MOUNTING DETAILS
 SHEET NUMBER: A-4

RFS HYBRIFLEX RISER CABLE SCHEDULE

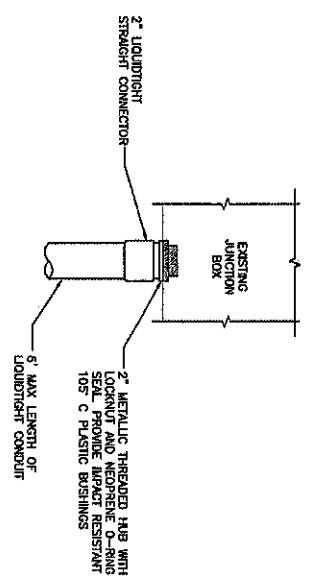
Product Code	Description	Notes
4 AWG Power	4 AWG Power Cable, 25' length, outdoor rated, 1/2" dia.	25' ft
6 AWG Power	6 AWG Power Cable, 25' length, outdoor rated, 3/4" dia.	25' ft
8 AWG Power	8 AWG Power Cable, 25' length, outdoor rated, 1" dia.	25' ft
Fiber Only (Excluding DC Power)	12 Channel Fiber Optic Cable, 25' length, outdoor rated, 1/2" dia.	25' ft



NOTE: CA TO CONFORM TO RFS RISER CABLE AND HYBRID OR FIBER JUMPER CABLE MODELS NUMBERS IF HYBRID CABLES ARE REQUIRED BEFORE PREPARING BOM.

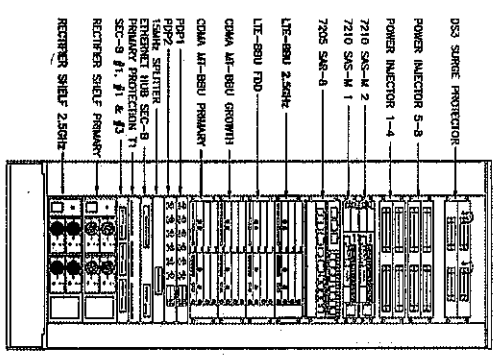
2.5. CABLE CROSS SECTION DATA

NO SCALE 1



FIBER JUNCTION BOX PENETRATION

NO SCALE 2

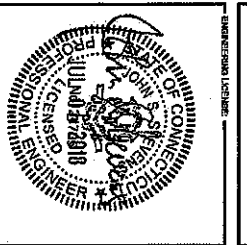


CABINET LAYOUT

NO SCALE 3



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WILLINGTON (CROWN)

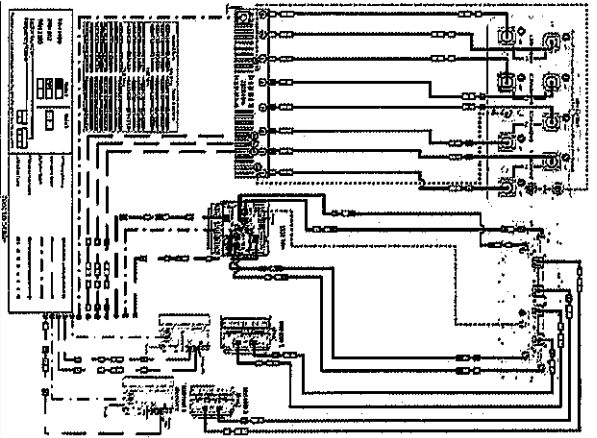
CT03XC205

COSGROVE ROAD
WEST WILLINGTON, CT 06279

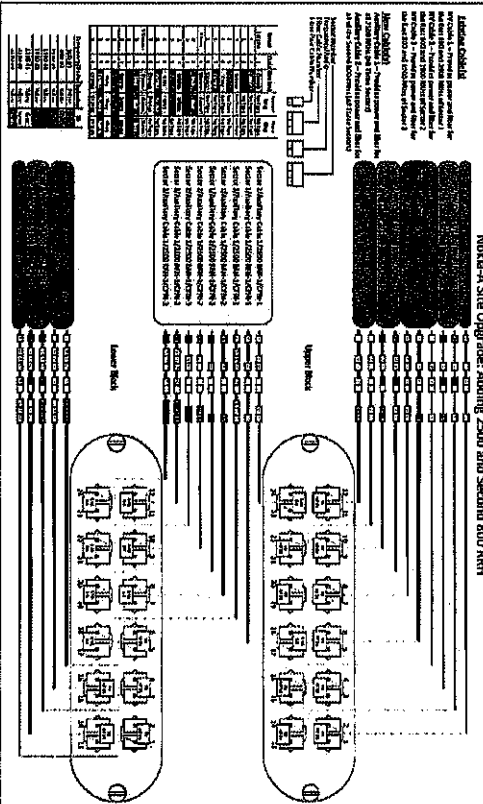
CIVIL DETAILS

A-5

ALL 211 APPROVALS AND 8 INNY-658-R4 and Fibers



PROJECT INFORMATION
 Project Name: **Nokia A Site Upgrade: Adding 2500 and Second 800 RRM**
 Client: **Sprint**
 Location: **Willington (Crown)**
 Date: **February 23, 2017**



PLUMBING DIAGRAM

NO SCALE 1

PLANS PROVIDED FOR:

 Sprint
 6580 Sprint Parkway
 Overland Park, Kansas 66251

PLANNED BY:

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PLA PARTNER:

CROWN CASTLE

REGISTERED LICENSE:

 JOSEPH S. STEPIEN
 PROFESSIONAL ENGINEER
 STATE OF CONNECTICUT
 No. 10818

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REVISIONS:

DESCRIPTION	DATE	BY	REV
ISSUED FOR CONSTRUCTION	02/23/17	MSJ	1
ISSUED FOR PERMITS	02/23/17	MSJ	2
ISSUED FOR REVIEW	02/23/17	MSJ	3

SITE NAME:
WILLINGTON (CROWN)

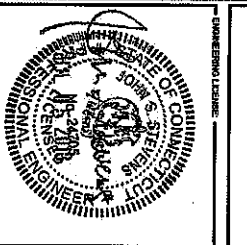
SITE ADDRESS:
CT03XC205

SHEET NUMBER:
A-6



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DESIGNED BY: JOSEPH S. STEVENS
REGISTERED PROFESSIONAL ENGINEER
STATE OF CONNECTICUT
LICENSE NO. 12708

REVISIONS:

DESCRIPTION	DATE	BY	REV
ISSUED FOR CONSTRUCTION	07/27/09	RSJ	0
ISSUED FOR REVIEW	07/14/09	RSJ	1
DATE NAME	08/11/09	RSJ	1

PROJECT: WILLINGTON (CROWN)

DATE CHECKED: CT03XC205

OFFICE ADDRESS: COSGROVE ROAD
WEST WILLINGTON, CT 06279

SHEET DESCRIPTION: ELECTRICAL & GROUNDING DETAILS

SHEET NUMBER: E-1

FINAL EQUIPMENT CONFIGURATION

SECTOR	ANTENNA MANUFACTURER	ANTENNA MODEL	OLD CENTER	NEW CENTER	HEIGHT	ORIENTATION	ANTENNA MAKE AND MODEL
1	COMSCORPE	AWV17M4-ALU-20	123	30'	30'	0°	(1) ALU 2500HZ 250-600 (1) ALU 1000HZ 400-500 (1) ALU 2500HZ 800-900-25
			123	20'	20'	0°	(1) ALU 2500HZ 800-900-25
2	COMSCORPE	AWV17M4-ALU-20	123	180'	180'	0°	(1) ALU 2500HZ 250-600 (1) ALU 1000HZ 400-500 (1) ALU 2500HZ 800-900-25
			123	190'	190'	0°	(1) ALU 2500HZ 800-900-25
3	COMSCORPE	AWV17M4-ALU-20	123	270'	270'	0°	(1) ALU 2500HZ 250-600 (1) ALU 1000HZ 400-500 (1) ALU 2500HZ 800-900-25
			123	270'	270'	0°	(1) ALU 2500HZ 800-900-25

FEEDER CABLES

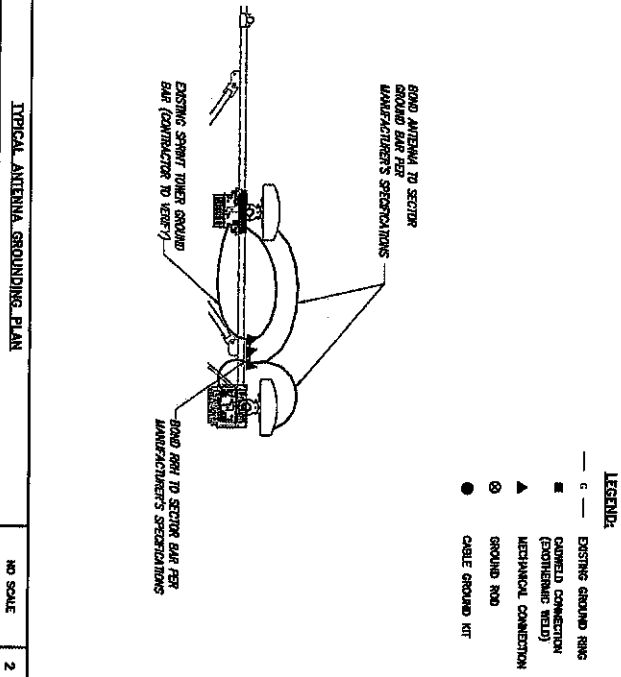
MANUFACTURER	MODEL	LENGTH	QTY
FRS	H814-SUNSHINE-1/2"	175'	(1)
FRS	H814-SUNSHINE-1/2"	175'	(1)
FRS	H814-SUNSHINE-1/2"	175'	(1)

LEGEND
EXISTING (PROPOSED)

NOTES:
1. CONTRACTOR TO VERIFY PROPOSED ANTENNA ORIENTATION & HGT. 2. CONTRACTOR TO CHECK CABLE TENSIONS PRIOR TO CONSTRUCTION.

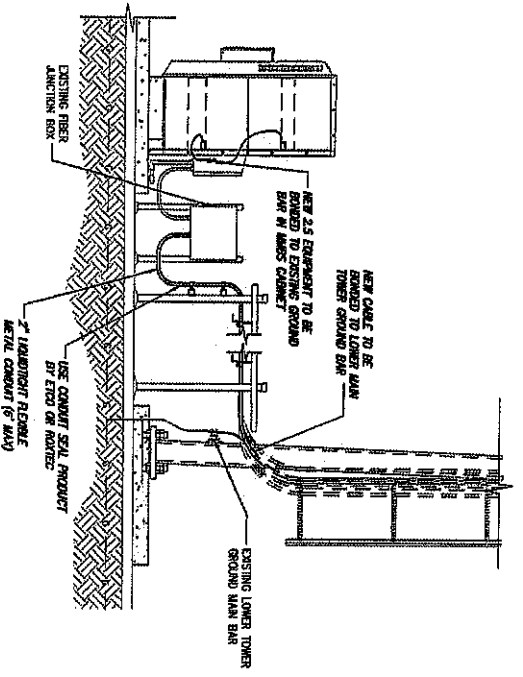
ANTENNA/CABLE SCHEDULE

NO SCALE 1



TYPICAL ANTENNA GROUNDING PLAN

NO SCALE 2



TYPICAL EQUIPMENT GROUNDING PLAN (ELEVATION)

NO SCALE 3



ENGINEERING INNOVATION

FDH Infrastructure Services, LLC
6521 Meridien Drive, Suite 107
Raleigh, North Carolina 27616
9197551012

Date: June 04, 2018

Denice Nicholson
Crown Castle
3 Corporate Park Drive, Suite 101
Clifton Park, NY 12065

Subject: Structural Analysis Report

Carrier Designation:	Sprint PCS Co-Locate	
	Carrier Site Number:	CT03XC205
	Carrier Site Name:	CT03XC205
Crown Castle Designation:	Crown Castle BU Number:	806383
	Crown Castle Site Name:	HRT 087 943325
	Crown Castle JDE Job Number:	505812
	Crown Castle Work Order Number:	1580451
	Crown Castle Order Number:	441319 Rev. 0
Engineering Firm Designation:	FDH-IS Project Number:	18SRNE1400

Site Data: COSGROVE ROAD WHIFFORD HILL, WEST WILLINGTON, Tolland County, CT
Latitude 41° 53' 32.92", Longitude -72° 15' 38.15"
140 Foot - Self Support Tower

Dear Denice Nicholson,

FDH Infrastructure Services, LLC is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 1195978, in accordance with order 441319, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Existing + Proposed Equipment **Sufficient Capacity**
Note: See Table I and Table II for the proposed and existing loading, respectively.

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 125 mph converted to a nominal 3-second gust wind speed of 97 mph per Section 1609.3 and Appendix N as required for use in the TIA-222-G Standard per Exception #5 of Section 1609.1.1. Exposure Category B with a maximum topographic factor, Kzt, of 1.000 and Risk Category II was used in this analysis.

We at FDH Infrastructure Services, LLC appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:

Morgan Gebert
Project Engineer I

Reviewed by:

Dennis D. Abel, PE
Director, New Product Development
CT PE License No. 23247



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3.2) Assumptions

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tnxTower Output

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7) APPENDIX C

Additional Calculations

1) INTRODUCTION

This tower is a 140 ft Self Support tower designed by ROHN in December of 1986. The tower was originally designed for EIA zone 'A' with 0.5" radial ice.

2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA-222-G Structural Standard for Antenna Supporting Structures and Antennas using a 3-second gust wind speed of 97 mph with no ice, 50 mph with 1 inch ice thickness and 60 mph under service loads, exposure category B with topographic category 1.

Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
124.0	125.0	3	alcatel lucent	PCS 1900MHz 4x45W-65MHz	1 3	7/8 1-1/4	-
		6	alcatel lucent	RRH2x50-800			
		3	alcatel lucent	TD-RRH8x20-25			
		3	commscope	NNV-65B-R4			
		3	rfs celwave	APXVTM14-ALU-I20			

Table 2 - Existing Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
137.0	139.0	3	alcatel lucent	RRH2X60-AWS	1 14	3/8 1-5/8	1
		3	alcatel lucent	RRH2X60-PCS			
		6	andrew	HBXX-6517DS-VTM			
		3	andrew	LNX-6514DS-VTM			
		3	andrew	LNX-8513DS-A1M			
		2	rfs celwave	DB-T1-6Z-8AB-0Z			
	6	rfs celwave	FD9R6004/2C-3L				
	137.0	1	-	Sector Mount [SM 510-3]			
124.0	125.0	6	decibel	DB980H90E-M	6	1-5/8	2
	124.0	1	-	Sector Mount [SM 502-3]	-	-	1
101.0	101.0	3	commscope	ATBT-BOTTOM-24V	1 12	1/4 1-1/4	1
		3	commscope	LNX-6515DS-VTM			
		1	-	Side Arm Mount [SO 304-3]			
		3	ems wireless	RR90-17-00DP			
		3	ericsson	KRY 112 13/1			
60.0	60.0	1	-	Side Arm Mount [SO 201-1]	1	1/2	1
		1	gps	GPS_A			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
50.0	50.0	1	gps	GPS_A	1	1/2	1

Notes:

- 1) Existing Equipment
- 2) Existing Equipment To Be Removed; Not Considered in this Analysis

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
140.0	140.0	4	--	PD10017	--	--
		4	--	3' Side Arm Mount		
131.0	131.0	6	--	PD1132	--	--
		3	--	6' Side Arm Mount		
121.0	121.0	2	--	6' STD dishes	--	--
100.0	100.0	1	--	PD1109	--	--
		1	--	6' Side Arm Mount		

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	JGI Eastern	1069386	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Rohn	1069383	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Rohn	1069394	CCISITES
4-POST-MODIFICATION INSPECTION	Sinnott Gering and Schmitt Towers, Inc.	5786395	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	FDH Velocitel	5670805	CCISITES

3.1) Analysis Method

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. FDH Infrastructure Services, LLC should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail	
T1	140 - 120	Leg	ROHN 2 STD	2	-19.66	36.84	53.4	Pass	
T2	120 - 100	Leg	ROHN 2.5 EH	39	-45.17	74.43	60.7	Pass	
T3	100 - 80	Leg	ROHN 3 EH	69	-68.51	94.34	72.6	Pass	
T4	80 - 60	Leg	ROHN 3.5 EH	90	-91.32	125.73	72.6	Pass	
T5	60 - 40	Leg	ROHN 4 X-STR	111	-113.11	159.90	70.7	Pass	
T6	40 - 20	Leg	ROHN 5 EH	132	-132.77	201.25	66.0	Pass	
T7	20 - 0	Leg	ROHN 5 X-STR	147	-153.32	201.77	76.0	Pass	
T1	140 - 120	Diagonal	L1 3/4x1 3/4x3/16	7	-3.53	8.66	40.8 59.8 (b)	Pass	
T2	120 - 100	Diagonal	L1 3/4x1 3/4x3/16	44	-3.07	5.05	60.7	Pass	
T3	100 - 80	Diagonal	L2x2x3/16	74	-3.89	4.74	82.0	Pass	
T4	80 - 60	Diagonal	L2 1/2x2 1/2x3/16	96	-4.19	7.26	57.8 67.1 (b)	Pass	
T5	60 - 40	Diagonal	L3x3x3/16	117	-4.57	9.92	46.1 73.3 (b)	Pass	
T6	40 - 20	Diagonal	L3x3x3/16	138	-5.51	6.80	81.1	Pass	
T7	20 - 0	Diagonal	L3x3x1/4	153	-5.80	7.46	77.8	Pass	
T1	140 - 120	Top Girt	L2x2x1/8	6	-0.42	3.21	13.0	Pass	
T2	120 - 100	Top Girt	L2x2x1/8	41	-0.19	3.17	5.9	Pass	
							Summary		
							Leg (T7)	76.0	Pass
							Diagonal (T3)	82.0	Pass
							Top Girt (T1)	13.0	Pass
							Bolt Checks	73.3	Pass
							RATING =	82.0	Pass

Table 6 - Tower Component Stresses vs. Capacity – LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	69.7	Pass
1	Base Foundation Structural	0	26.7	Pass
1	Base Foundation Soil Interaction	0	42.5	Pass

Structure Rating (max from all components) =	82.0%
---	--------------

Notes:

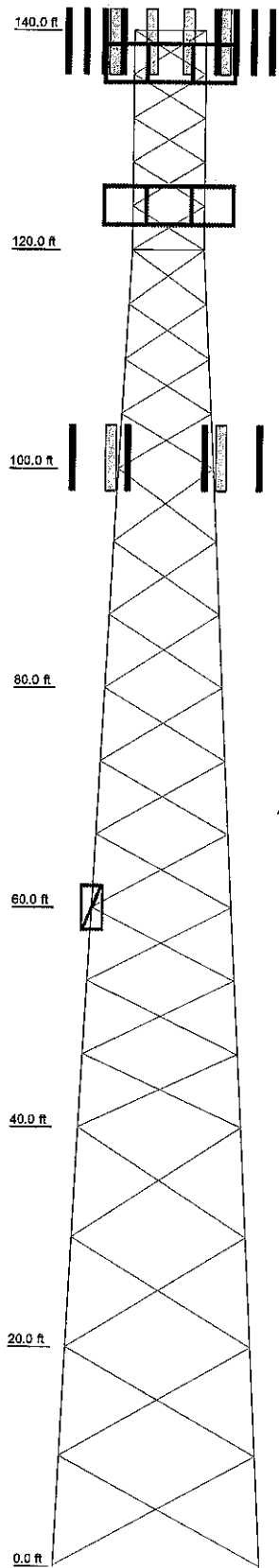
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

Section	T7	T6	T5	T4	T3	T2	T1
Legs	ROHN 5 X-STR	ROHN 5 EH	ROHN 4 X-STR	ROHN 3.5 EH	ROHN 3 EH	ROHN 2.5 EH	ROHN 2 STD
Leg Grade	A572-50	A572-50	A618-50	A618-50	A618-50	A618-50	A618-50
Diagonals	L3x3x1/4	L3x3x3/16	L3x3x3/16	L2x2x3/16	L2x2x3/16	L1 3/4x1 3/4x3/16	L2x2x1/8
Diagonal Grade	A572-50	A572-50	A572-50	A572-50	A572-50	A572-50	A572-50
Top Girts	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.	N.A.
Face Width (ft)	18.7703	16.7708	14.7708	12.8771	10.6354	8.60417	6.5625
# Panels @ (ft)	2 @ 9.95633	2 @ 10	2 @ 10	9 @ 6.66667	9 @ 6.66667	4 @ 5	5 @ 4
Weight (K)	11.3	2.6	2.6	2.2	2.0	1.6	1.0



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
LNX-8513DS-A1M w/ Mount Pipe	137	PCS 1900MHz 4x45W-65MHz	124
LNX-8513DS-A1M w/ Mount Pipe	137	PCS 1900MHz 4x45W-65MHz	124
LNX-8513DS-A1M w/ Mount Pipe	137	PCS 1900MHz 4x45W-65MHz	124
(2) HBXX-6517DS-VTM w/ Mount Pipe	137	(2) RRH2x50-800	124
(2) HBXX-6517DS-VTM w/ Mount Pipe	137	(2) RRH2x50-800	124
(2) HBXX-6517DS-VTM w/ Mount Pipe	137	(2) RRH2x50-800	124
LNX-6514DS-VTM w/ Mount Pipe	137	Empty Mount Pipe	124
LNX-6514DS-VTM w/ Mount Pipe	137	Empty Mount Pipe	124
LNX-6514DS-VTM w/ Mount Pipe	137	Empty Mount Pipe	124
RRH2X60-PCS	137	Sector Mount [SM 502-3]	124
RRH2X60-PCS	137	RR90-17-00DP w/ Mount Pipe	101
RRH2X60-PCS	137	RR90-17-00DP w/ Mount Pipe	101
(2) FD9R6004/2C-3L	137	RR90-17-00DP w/ Mount Pipe	101
(2) FD9R6004/2C-3L	137	LNX-6515DS-VTM w/ Mount Pipe	101
(2) FD9R6004/2C-3L	137	LNX-6515DS-VTM w/ Mount Pipe	101
RRH2X60-AWS	137	LNX-6515DS-VTM w/ Mount Pipe	101
RRH2X60-AWS	137	KRY 112 13/1	101
RRH2X60-AWS	137	KRY 112 13/1	101
DB-T1-6Z-8AB-0Z	137	KRY 112 13/1	101
DB-T1-6Z-8AB-0Z	137	ATBT-BOTTOM-24V	101
Sector Mount [SM 510-3]	137	ATBT-BOTTOM-24V	101
NNVV-65B-R4 w/ Mount Pipe	124	ATBT-BOTTOM-24V	101
NNVV-65B-R4 w/ Mount Pipe	124	Side Arm Mount [SO 304-3]	101
NNVV-65B-R4 w/ Mount Pipe	124	1.9" x10" Horizontal Pipe	101
APXVTM14-ALU-I20 w/ Mount Pipe	124	1.9" x10" Horizontal Pipe	101
APXVTM14-ALU-I20 w/ Mount Pipe	124	1.9" x10" Horizontal Pipe	101
APXVTM14-ALU-I20 w/ Mount Pipe	124	GPS_A	60
TD-RRH8x20-25	124	Side Arm Mount [SO 201-1]	60
TD-RRH8x20-25	124	GPS_A	50
TD-RRH8x20-25	124	GPS_A	50

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A618-50	50 ksi	70 ksi	A572-50	50 ksi	65 ksi
A36	36 ksi	58 ksi			

TOWER DESIGN NOTES

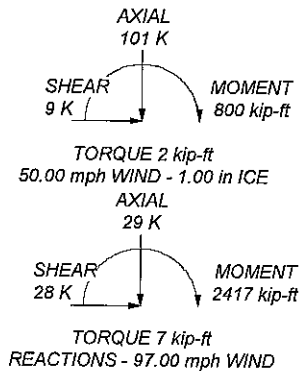
1. Tower is located in Tolland County, Connecticut.
2. Tower designed for Exposure B to the TIA-222-G Standard.
3. Tower designed for a 97.00 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50.00 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60.00 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 82%

ALL REACTIONS
ARE FACTORED

MAX. CORNER REACTIONS AT BASE:

DOWN: 158 K
SHEAR: 17 K

UPLIFT: -135 K
SHEAR: 15 K



FDH Velocitel		Job: 806383-HRT 087 943325	
6521 Meriden Drive, Suite 107		Project: 18SRNE1400	
Raleigh, North Carolina 27616		Client: Crown Castle	
Tower Analysis		Code: TIA-222-G	Drawn by: MGebert
Phone: 9197551012		Date: 06/04/18	App'd:
FAX: 9197551031		Path:	Scale: NTS
		Dwg No. E-1	

tnxTower FDH Velocitel 6521 Meridien Drive, Suite 107 Raleigh, North Carolina 27616 Phone: 9197551012 FAX: 9197551031	Job 806383-HRT 087 943325	Page 1 of 22
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	Client Crown Castle	Designed by MGebert

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 140.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 6.52 ft at the top and 18.77 ft at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Tolland County, Connecticut.

ASCE 7-10 Wind Data is used (wind speeds converted to nominal values).

Basic wind speed of 97.00 mph.

Structure Class II.

Exposure Category B.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 1.00 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50.00 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60.00 mph.

Pressures are calculated at each section.

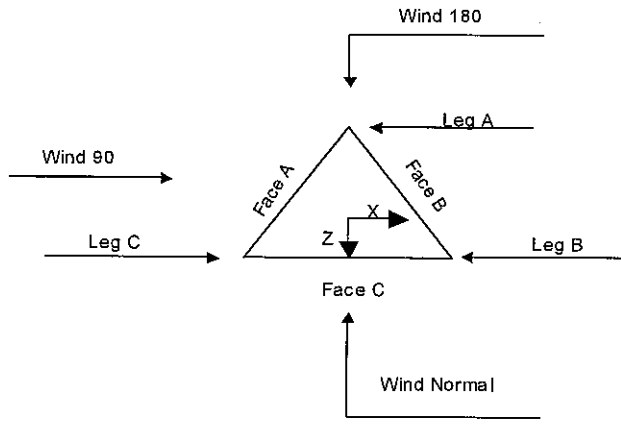
Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

- | | | |
|--|--|---|
| <ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification √ Use Code Stress Ratios √ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile √ Include Bolts In Member Capacity Leg Bolts Are At Top Of Section √ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) √ SR Members Have Cut Ends SR Members Are Concentric | <ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate √ Use Clear Spans For Wind Area √ Use Clear Spans For KL/r Retension Guys To Initial Tension √ Bypass Mast Stability Checks √ Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder | <ul style="list-style-type: none"> Use ASCE 10 X-Brace Ly Rules √ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA √ SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable √ Offset Girt At Foundation √ Consider Feed Line Torque √ Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption Use TIA-222-G Tension Splice Exemption Poles Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets |
|--|--|---|

tnxTower FDH Velocitel 6521 Meridien Drive, Suite 107 Raleigh, North Carolina 27616 Phone: 9197551012 FAX: 9197551031	Job 806383-HRT 087 943325	Page 2 of 22
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	Client Crown Castle	Designed by MGebert



Triangular Tower

Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	140.00-120.00			6.52	1	20.00
T2	120.00-100.00		08N087	6.56	1	20.00
T3	100.00-80.00		09N006	8.60	1	20.00
T4	80.00-60.00		10N007	10.64	1	20.00
T5	60.00-40.00		11N007	12.68	1	20.00
T6	40.00-20.00		12N004	14.77	1	20.00
T7	20.00-0.00		13N003	16.77	1	20.00

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	140.00-120.00	4.00	X Brace	No	No	0.00	0.00
T2	120.00-100.00	5.00	X Brace	No	No	0.00	0.00
T3	100.00-80.00	6.67	X Brace	No	No	0.00	0.00
T4	80.00-60.00	6.67	X Brace	No	No	0.00	0.00
T5	60.00-40.00	6.67	X Brace	No	No	0.00	0.00
T6	40.00-20.00	10.00	X Brace	No	No	0.00	0.00
T7	20.00-0.00	9.96	X Brace	No	No	0.00	1.00

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	Client	Crown Castle	Designed by	MGeibert

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 140.00-120.00	Pipe	ROHN 2 STD	A618-50 (50 ksi)	Single Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T2 120.00-100.00	Pipe	ROHN 2.5 EH	A618-50 (50 ksi)	Single Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T3 100.00-80.00	Pipe	ROHN 3 EH	A618-50 (50 ksi)	Single Angle	L2x2x3/16	A36 (36 ksi)
T4 80.00-60.00	Pipe	ROHN 3.5 EH	A618-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T5 60.00-40.00	Pipe	ROHN 4 X-STR	A618-50 (50 ksi)	Single Angle	L3x3x3/16	A36 (36 ksi)
T6 40.00-20.00	Pipe	ROHN 5 EH	A618-50 (50 ksi)	Single Angle	L3x3x3/16	A36 (36 ksi)
T7 20.00-0.00	Pipe	ROHN 5 X-STR	A618-50 (50 ksi)	Single Angle	L3x3x1/4	A572-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 140.00-120.00	Equal Angle	L2x2x1/8	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T2 120.00-100.00	Single Angle	L2x2x1/8	A36 (36 ksi)	Single Angle		A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
T1 140.00-120.00	0.00	0.00	A36 (36 ksi)	1.03	1	1.05	36.00	36.00	36.00
T2 120.00-100.00	0.00	0.00	A36 (36 ksi)	1.03	1	1.05	36.00	36.00	36.00
T3 100.00-80.00	0.00	0.00	A36 (36 ksi)	1.03	1	1.05	36.00	36.00	36.00
T4 80.00-60.00	0.00	0.00	A36 (36 ksi)	1.03	1	1.05	36.00	36.00	36.00
T5 60.00-40.00	0.00	0.00	A36 (36 ksi)	1.03	1	1.05	36.00	36.00	36.00
T6 40.00-20.00	0.00	0.00	A36 (36 ksi)	1.03	1	1.05	36.00	36.00	36.00
T7 20.00-0.00	0.00	0.00	A36 (36 ksi)	1.03	1	1.05	36.00	36.00	36.00

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	Client	Crown Castle	Designed by	MGeibert

Tower Section Geometry (cont'd)

Tower Elevation ft	Calc K Single Angles	Calc K Solid Rounds	Legs	K Factors ¹								
				X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace		
				X Y	X Y	X Y	X Y	X Y	X Y	X Y		
T1 140.00-120.00	Yes	No	1	1	1	1	1	1	1	1	1	1
T2 120.00-100.00	Yes	No	1	1	1	1	1	1	1	1	1	1
T3 100.00-80.00	Yes	No	1	1	1	1	1	1	1	1	1	1
T4 80.00-60.00	Yes	No	1	1	1	1	1	1	1	1	1	1
T5 60.00-40.00	Yes	No	1	1	1	1	1	1	1	1	1	1
T6 40.00-20.00	Yes	No	1	1	1	1	1	1	1	1	1	1
T7 20.00-0.00	Yes	No	1	1	1	1	1	1	1	1	1	1

¹Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 140.00-120.00	0.00	1	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75
T2 120.00-100.00	0.00	1	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75
T3 100.00-80.00	0.00	1	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75
T4 80.00-60.00	0.00	1	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75
T5 60.00-40.00	0.00	1	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75
T6 40.00-20.00	0.00	1	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75
T7 20.00-0.00	0.00	1	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75

Tower Section Geometry (cont'd)

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Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 140.00-120.00	Flange	0.63 A325N	4	0.50 A325N	1	0.50 A325N	1	0.50 A325N	0	0.50 A325N	0	0.50 A325N	0	0.50 A325N	0
T2 120.00-100.00	Flange	0.75 A325N	4	0.50 A325N	1	0.50 A325N	1	0.63 A325N	0	0.63 A325N	0	0.63 A325N	0	0.63 A325N	0
T3 100.00-80.00	Flange	0.88 A325N	4	0.50 A325N	1	0.50 A325N	0	0.00 A325N	0	0.50 A325N	0	0.50 A325N	0	0.50 A325N	0
T4 80.00-60.00	Flange	0.88 A325N	4	0.50 A325N	1	0.50 A325N	0	0.50 A325N	0	0.50 A325N	0	0.50 A325N	0	0.50 A325N	0
T5 60.00-40.00	Flange	1.00 A325N	4	0.50 A325N	1	0.63 A325N	0	0.63 A325N	0	0.63 A325N	0	0.63 A325N	0	0.63 A325N	0
T6 40.00-20.00	Flange	1.00 A325N	4	0.63 A325N	1	0.63 A325N	0	0.00 A325N	0	0.63 A325N	0	0.63 A325N	0	0.63 A325N	0
T7 20.00-0.00	Flange	1.00 A449	0	0.63 A325N	1	0.63 A325N	0	0.00 A325N	0	0.63 A325N	0	0.63 A325N	0	0.63 A325N	0

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	# Per Row	#	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
**** LDF7-50A(1-5/8)	C	No	Ar (CaAa)	137.00 - 8.00	0.00	0.3	14	8	0.50	1.98		0.82
LDF2-50(3/8) ****	C	No	Ar (CaAa)	137.00 - 8.00	0.00	0.25	1	1	0.50	0.44		0.08
HB114-08U3 M12-XXXXF(7/8)	B	No	Ar (CaAa)	124.00 - 8.00	0.00	-0.33	1	1	0.50	1.11		0.68
HB114-1-08U 4-MSF(1-1/4) *** ***	B	No	Ar (CaAa)	124.00 - 8.00	0.00	-0.35	3	3	0.50	1.54		1.30
LDF1-50A(1/4) FLC 114-50J(1-1/4) ***	A	No	Ar (CaAa)	101.00 - 8.00	0.00	0.365	1	1	0.50	0.34		0.06
LDF4-50A(1/2) ***	A	No	Ar (CaAa)	101.00 - 8.00	0.00	0.4	12	6	0.50	1.58		0.70
LDF4-50A(1/2) ***	C	No	Ar (CaAa)	60.00 - 8.00	4.00	0.26	1	1	0.50	0.63		0.15
LDF4-50A(1/2) ***	B	No	Ar (CaAa)	50.00 - 8.00	0.00	0.05	2	2	0.50	0.63		0.15
Feedline Ladder (Af)	A	No	Af (CaAa)	101.00 - 8.00	0.00	0.4	1	1	3.00	3.00		8.40
Feedline Ladder (Af)	B	No	Af (CaAa)	124.00 - 8.00	0.00	-0.35	1	1	3.00	3.00		8.40
Feedline Ladder (Af)	C	No	Af (CaAa)	137.00 - 8.00	0.00	0.3	1	1	3.00	3.00		8.40
*** Safety Line 3/8 ****	C	No	Ar (CaAa)	140.00 - 8.00	0.00	0.5	1	1	0.50	0.38		0.22

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Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	C _{AA}	Weight
				ft		ft ² /ft	plf

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation	Face	A _R	A _F	C _{AA} In Face	C _{AA} Out Face	Weight
	ft		ft ²	ft ²	ft ²	ft ²	K
T1	140.00-120.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	4.292	0.000	0.05
		C	0.000	0.000	57.122	0.000	0.34
T2	120.00-100.00	A	0.000	0.000	2.430	0.000	0.02
		B	0.000	0.000	21.460	0.000	0.26
		C	0.000	0.000	67.070	0.000	0.40
T3	100.00-80.00	A	0.000	0.000	48.610	0.000	0.34
		B	0.000	0.000	21.460	0.000	0.26
		C	0.000	0.000	67.070	0.000	0.40
T4	80.00-60.00	A	0.000	0.000	48.610	0.000	0.34
		B	0.000	0.000	21.460	0.000	0.26
		C	0.000	0.000	67.070	0.000	0.40
T5	60.00-40.00	A	0.000	0.000	48.610	0.000	0.34
		B	0.000	0.000	22.710	0.000	0.26
		C	0.000	0.000	68.320	0.000	0.41
T6	40.00-20.00	A	0.000	0.000	48.610	0.000	0.34
		B	0.000	0.000	23.960	0.000	0.27
		C	0.000	0.000	68.320	0.000	0.41
T7	20.00-0.00	A	0.000	0.000	29.166	0.000	0.20
		B	0.000	0.000	14.376	0.000	0.16
		C	0.000	0.000	40.992	0.000	0.24

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation	Face or Leg	Ice Thickness	A _R	A _F	C _{AA} In Face	C _{AA} Out Face	Weight
	ft		in	ft ²	ft ²	ft ²	ft ²	K
T1	140.00-120.00	A	2.294	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	12.067	0.000	0.24
		C		0.000	0.000	89.132	0.000	1.93
T2	120.00-100.00	A	2.256	0.000	0.000	3.738	0.000	0.08
		B		0.000	0.000	59.771	0.000	1.18
		C		0.000	0.000	102.409	0.000	2.20
T3	100.00-80.00	A	2.211	0.000	0.000	74.092	0.000	1.61
		B		0.000	0.000	59.105	0.000	1.15
		C		0.000	0.000	101.583	0.000	2.16
T4	80.00-60.00	A	2.156	0.000	0.000	73.286	0.000	1.58
		B		0.000	0.000	58.290	0.000	1.12
		C		0.000	0.000	100.571	0.000	2.11
T5	60.00-40.00	A	2.085	0.000	0.000	72.238	0.000	1.53
		B		0.000	0.000	66.754	0.000	1.17

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Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
T6	40.00-20.00	C		0.000	0.000	108.846	0.000	2.19
		A	1.981	0.000	0.000	70.714	0.000	1.46
		B		0.000	0.000	74.006	0.000	1.20
T7	20.00-0.00	C		0.000	0.000	106.520	0.000	2.09
		A	1.775	0.000	0.000	40.616	0.000	0.80
		B		0.000	0.000	41.703	0.000	0.64
		C		0.000	0.000	61.143	0.000	1.13

Feed Line Center of Pressure

Section	Elevation ft	CP _X in	CP _Z in	CP _X Ice in	CP _Z Ice in
T1	140.00-120.00	-3.68	3.46	-2.72	2.21
T2	120.00-100.00	-3.40	1.64	-2.43	0.31
T3	100.00-80.00	-3.61	-2.20	-2.82	-2.70
T4	80.00-60.00	-4.13	-2.56	-3.28	-3.15
T5	60.00-40.00	-4.49	-2.75	-3.77	-3.03
T6	40.00-20.00	-5.05	-3.21	-4.30	-3.54
T7	20.00-0.00	-4.63	-2.95	-3.94	-3.26

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
T1	2	LDF7-50A(1-5/8)	120.00 - 137.00	0.6000	0.5345
T1	3	LDF2-50(3/8)	120.00 - 137.00	0.6000	0.5345
T1	5	HB114-08U3M12-XXXXF(7/8)	120.00 - 124.00	0.6000	0.5345
T1	6	HB114-1-08U4-M5F(1-1/4)	120.00 - 124.00	0.6000	0.5345
T1	18	Feedline Ladder (Af)	120.00 - 124.00	0.6000	0.5345
T1	19	Feedline Ladder (Af)	120.00 - 137.00	0.6000	0.5345
T1	21	Safety Line 3/8	120.00 - 140.00	0.6000	0.5345
T2	2	LDF7-50A(1-5/8)	100.00 - 120.00	0.6000	0.6000
T2	3	LDF2-50(3/8)	100.00 - 120.00	0.6000	0.6000
T2	5	HB114-08U3M12-XXXXF(7/8)	100.00 - 120.00	0.6000	0.6000
T2	6	HB114-1-08U4-M5F(1-1/4)	100.00 - 120.00	0.6000	0.6000
T2	10	LDF1-50A(1/4)	100.00 - 101.00	0.6000	0.6000
T2	11	FLC 114-50J(1-1/4)	100.00 - 101.00	0.6000	0.6000

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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
T2	17	Feedline Ladder (Af)	100.00 - 101.00	0.6000	0.6000
T2	18	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T2	19	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T2	21	Safety Line 3/8	100.00 - 120.00	0.6000	0.6000
T3	2	LDF7-50A(1-5/8)	80.00 - 100.00	0.6000	0.6000
T3	3	LDF2-50(3/8)	80.00 - 100.00	0.6000	0.6000
T3	5	HB114-08U3M12-XXXXF(7/8)	80.00 - 100.00	0.6000	0.6000
T3	6	HB114-1-08U4-M5F(1-1/4)	80.00 - 100.00	0.6000	0.6000
T3	10	LDF1-50A(1/4)	80.00 - 100.00	0.6000	0.6000
T3	11	FLC 114-50J(1-1/4)	80.00 - 100.00	0.6000	0.6000
T3	17	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T3	18	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T3	19	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T3	21	Safety Line 3/8	80.00 - 100.00	0.6000	0.6000
T4	2	LDF7-50A(1-5/8)	60.00 - 80.00	0.6000	0.6000
T4	3	LDF2-50(3/8)	60.00 - 80.00	0.6000	0.6000
T4	5	HB114-08U3M12-XXXXF(7/8)	60.00 - 80.00	0.6000	0.6000
T4	6	HB114-1-08U4-M5F(1-1/4)	60.00 - 80.00	0.6000	0.6000
T4	10	LDF1-50A(1/4)	60.00 - 80.00	0.6000	0.6000
T4	11	FLC 114-50J(1-1/4)	60.00 - 80.00	0.6000	0.6000
T4	17	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T4	18	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T4	19	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T4	21	Safety Line 3/8	60.00 - 80.00	0.6000	0.6000
T5	2	LDF7-50A(1-5/8)	40.00 - 60.00	0.6000	0.6000
T5	3	LDF2-50(3/8)	40.00 - 60.00	0.6000	0.6000
T5	5	HB114-08U3M12-XXXXF(7/8)	40.00 - 60.00	0.6000	0.6000
T5	6	HB114-1-08U4-M5F(1-1/4)	40.00 - 60.00	0.6000	0.6000
T5	10	LDF1-50A(1/4)	40.00 - 60.00	0.6000	0.6000
T5	11	FLC 114-50J(1-1/4)	40.00 - 60.00	0.6000	0.6000
T5	13	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T5	15	LDF4-50A(1/2)	40.00 - 50.00	0.6000	0.6000
T5	17	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T5	18	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T5	19	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T5	21	Safety Line 3/8	40.00 - 60.00	0.6000	0.6000
T6	2	LDF7-50A(1-5/8)	20.00 - 40.00	0.6000	0.6000
T6	3	LDF2-50(3/8)	20.00 - 40.00	0.6000	0.6000
T6	5	HB114-08U3M12-XXXXF(7/8)	20.00 - 40.00	0.6000	0.6000
T6	6	HB114-1-08U4-M5F(1-1/4)	20.00 - 40.00	0.6000	0.6000
T6	10	LDF1-50A(1/4)	20.00 - 40.00	0.6000	0.6000
T6	11	FLC 114-50J(1-1/4)	20.00 - 40.00	0.6000	0.6000
T6	13	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
T6	15	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
T6	17	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T6	18	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T6	19	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T6	21	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000
T7	2	LDF7-50A(1-5/8)	8.00 - 20.00	0.6000	0.6000
T7	3	LDF2-50(3/8)	8.00 - 20.00	0.6000	0.6000
T7	5	HB114-08U3M12-XXXXF(7/8)	8.00 - 20.00	0.6000	0.6000
T7	6	HB114-1-08U4-M5F(1-1/4)	8.00 - 20.00	0.6000	0.6000
T7	10	LDF1-50A(1/4)	8.00 - 20.00	0.6000	0.6000

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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
T7	11	FLC 114-50J(1-1/4)	8.00 - 20.00	0.6000	0.6000
T7	13	LDF4-50A(1/2)	8.00 - 20.00	0.6000	0.6000
T7	15	LDF4-50A(1/2)	8.00 - 20.00	0.6000	0.6000
T7	17	Feedline Ladder (Af)	8.00 - 20.00	0.6000	0.6000
T7	18	Feedline Ladder (Af)	8.00 - 20.00	0.6000	0.6000
T7	19	Feedline Ladder (Af)	8.00 - 20.00	0.6000	0.6000
T7	21	Safety Line 3/8	8.00 - 20.00	0.6000	0.6000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A _A Front	C _A A _A Side	Weight
			Horz Lateral	Vert					
			ft	ft	°	ft	ft ²	ft ²	K

LNX-8513DS-A1M w/ Mount Pipe	A	From Leg	4.00	0.000	137.00	No Ice	8.41	7.08	0.06
			0.00			1/2" Ice	8.97	8.27	0.13
			2.00			1" Ice	9.50	9.18	0.21
LNX-8513DS-A1M w/ Mount Pipe	B	From Leg	4.00	0.000	137.00	No Ice	8.41	7.08	0.06
			0.00			1/2" Ice	8.97	8.27	0.13
			2.00			1" Ice	9.50	9.18	0.21
LNX-8513DS-A1M w/ Mount Pipe	C	From Leg	4.00	0.000	137.00	No Ice	8.41	7.08	0.06
			0.00			1/2" Ice	8.97	8.27	0.13
			2.00			1" Ice	9.50	9.18	0.21
(2) HBXX-6517DS-VIM w/ Mount Pipe	A	From Leg	4.00	0.000	137.00	No Ice	8.77	6.96	0.07
			0.00			1/2" Ice	9.34	8.18	0.14
			2.00			1" Ice	9.89	9.14	0.21
(2) HBXX-6517DS-VIM w/ Mount Pipe	B	From Leg	4.00	0.000	137.00	No Ice	8.77	6.96	0.07
			0.00			1/2" Ice	9.34	8.18	0.14
			2.00			1" Ice	9.89	9.14	0.21
(2) HBXX-6517DS-VIM w/ Mount Pipe	C	From Leg	4.00	0.000	137.00	No Ice	8.77	6.96	0.07
			0.00			1/2" Ice	9.34	8.18	0.14
			2.00			1" Ice	9.89	9.14	0.21
LNX-6514DS-VTM w/ Mount Pipe	A	From Leg	4.00	0.000	137.00	No Ice	8.32	7.00	0.06
			0.00			1/2" Ice	8.88	8.19	0.13
			2.00			1" Ice	9.40	9.08	0.20
LNX-6514DS-VTM w/ Mount Pipe	B	From Leg	4.00	0.000	137.00	No Ice	8.32	7.00	0.06
			0.00			1/2" Ice	8.88	8.19	0.13
			2.00			1" Ice	9.40	9.08	0.20
LNX-6514DS-VTM w/ Mount Pipe	C	From Leg	4.00	0.000	137.00	No Ice	8.32	7.00	0.06
			0.00			1/2" Ice	8.88	8.19	0.13
			2.00			1" Ice	9.40	9.08	0.20
RRH2X60-PCS	A	From Leg	4.00	0.000	137.00	No Ice	2.20	1.65	0.05
			0.00			1/2" Ice	2.39	1.83	0.07
			2.00			1" Ice	2.59	2.01	0.09
RRH2X60-PCS	B	From Leg	4.00	0.000	137.00	No Ice	2.20	1.65	0.05
			0.00			1/2" Ice	2.39	1.83	0.07
			2.00			1" Ice	2.59	2.01	0.09
RRH2X60-PCS	C	From Leg	4.00	0.000	137.00	No Ice	2.20	1.65	0.05
			0.00			1/2" Ice	2.39	1.83	0.07
			2.00			1" Ice	2.59	2.01	0.09
(2) FD9R6004/2C-3L	A	From Leg	4.00	0.000	137.00	No Ice	0.31	0.08	0.00

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz	Lateral Vert					
			ft	ft	°	ft	ft ²	ft ²	K
			0.00			1/2" Ice	0.39	0.12	0.01
			2.00			1" Ice	0.47	0.17	0.01
(2) FD9R6004/2C-3L	B	From Leg	4.00		0.000	No Ice	0.31	0.08	0.00
			0.00			1/2" Ice	0.39	0.12	0.01
			2.00			1" Ice	0.47	0.17	0.01
(2) FD9R6004/2C-3L	C	From Leg	4.00		0.000	No Ice	0.31	0.08	0.00
			0.00			1/2" Ice	0.39	0.12	0.01
			2.00			1" Ice	0.47	0.17	0.01
RRH2X60-AWS	C	From Leg	4.00		0.000	No Ice	1.88	1.24	0.04
			0.00			1/2" Ice	2.06	1.39	0.06
			2.00			1" Ice	2.24	1.54	0.08
RRH2X60-AWS	B	From Leg	4.00		0.000	No Ice	1.88	1.24	0.04
			0.00			1/2" Ice	2.06	1.39	0.06
			2.00			1" Ice	2.24	1.54	0.08
RRH2X60-AWS	A	From Leg	4.00		0.000	No Ice	1.88	1.24	0.04
			0.00			1/2" Ice	2.06	1.39	0.06
			2.00			1" Ice	2.24	1.54	0.08
DB-T1-6Z-8AB-0Z	A	From Leg	4.00		0.000	No Ice	4.80	2.00	0.04
			0.00			1/2" Ice	5.07	2.19	0.08
			2.00			1" Ice	5.35	2.39	0.12
DB-T1-6Z-8AB-0Z	B	From Leg	4.00		0.000	No Ice	4.80	2.00	0.04
			0.00			1/2" Ice	5.07	2.19	0.08
			2.00			1" Ice	5.35	2.39	0.12
Sector Mount [SM 510-3]	C	None			0.000	No Ice	40.10	40.10	2.40
						1/2" Ice	57.33	57.33	3.09
						1" Ice	74.56	74.56	3.78

NNVV-65B-R4 w/ Mount Pipe	A	From Leg	4.00		0.000	No Ice	12.56	7.76	0.12
			0.00			1/2" Ice	13.14	8.80	0.21
			1.00			1" Ice	13.70	9.69	0.32
NNVV-65B-R4 w/ Mount Pipe	B	From Leg	4.00		0.000	No Ice	12.56	7.76	0.12
			0.00			1/2" Ice	13.14	8.80	0.21
			1.00			1" Ice	13.70	9.69	0.32
NNVV-65B-R4 w/ Mount Pipe	C	From Leg	4.00		0.000	No Ice	12.56	7.76	0.12
			0.00			1/2" Ice	13.14	8.80	0.21
			1.00			1" Ice	13.70	9.69	0.32
APXVTM14-ALU-I20 w/ Mount Pipe	A	From Leg	4.00		0.000	No Ice	6.58	4.96	0.08
			0.00			1/2" Ice	7.03	5.75	0.13
			1.00			1" Ice	7.47	6.47	0.19
APXVTM14-ALU-I20 w/ Mount Pipe	B	From Leg	4.00		0.000	No Ice	6.58	4.96	0.08
			0.00			1/2" Ice	7.03	5.75	0.13
			1.00			1" Ice	7.47	6.47	0.19
APXVTM14-ALU-I20 w/ Mount Pipe	C	From Leg	4.00		0.000	No Ice	6.58	4.96	0.08
			0.00			1/2" Ice	7.03	5.75	0.13
			1.00			1" Ice	7.47	6.47	0.19
TD-RRH8x20-25	A	From Leg	4.00		0.000	No Ice	3.70	1.29	0.07
			0.00			1/2" Ice	3.95	1.46	0.09
			1.00			1" Ice	4.20	1.64	0.12
TD-RRH8x20-25	B	From Leg	4.00		0.000	No Ice	3.70	1.29	0.07
			0.00			1/2" Ice	3.95	1.46	0.09
			1.00			1" Ice	4.20	1.64	0.12
TD-RRH8x20-25	C	From Leg	4.00		0.000	No Ice	3.70	1.29	0.07
			0.00			1/2" Ice	3.95	1.46	0.09
			1.00			1" Ice	4.20	1.64	0.12
PCS 1900MHz	A	From Leg	4.00		0.000	No Ice	2.32	2.24	0.06
4x45W-65MHz			0.00			1/2" Ice	2.53	2.44	0.08
			1.00			1" Ice	2.74	2.65	0.11

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K	
PCS 1900MHz 4x45W-65MHz	B	From Leg	4.00 0.00 1.00	0.000	124.00	No Ice 1/2" Ice 1" Ice	2.32 2.53 2.74	2.24 2.44 2.65	0.06 0.08 0.11
PCS 1900MHz 4x45W-65MHz	C	From Leg	4.00 0.00 1.00	0.000	124.00	No Ice 1/2" Ice 1" Ice	2.32 2.53 2.74	2.24 2.44 2.65	0.06 0.08 0.11
(2) RRH2x50-800	A	From Leg	4.00 0.00 1.00	0.000	124.00	No Ice 1/2" Ice 1" Ice	2.13 2.32 2.51	1.79 1.96 2.14	0.05 0.07 0.10
(2) RRH2x50-800	B	From Leg	4.00 0.00 1.00	0.000	124.00	No Ice 1/2" Ice 1" Ice	2.13 2.32 2.51	1.79 1.96 2.14	0.05 0.07 0.10
(2) RRH2x50-800	C	From Leg	4.00 0.00 1.00	0.000	124.00	No Ice 1/2" Ice 1" Ice	2.13 2.32 2.51	1.79 1.96 2.14	0.05 0.07 0.10
Empty Mount Pipe	A	From Leg	4.00 0.00 0.00	0.000	124.00	No Ice 1/2" Ice 1" Ice	1.20 1.50 1.81	1.20 1.50 1.81	0.02 0.03 0.04
Empty Mount Pipe	B	From Leg	4.00 0.00 0.00	0.000	124.00	No Ice 1/2" Ice 1" Ice	1.20 1.50 1.81	1.20 1.50 1.81	0.02 0.03 0.04
Empty Mount Pipe	C	From Leg	4.00 0.00 0.00	0.000	124.00	No Ice 1/2" Ice 1" Ice	1.20 1.50 1.81	1.20 1.50 1.81	0.02 0.03 0.04
Sector Mount [SM 502-3]	C	None	0.00	0.000	124.00	No Ice 1/2" Ice 1" Ice	33.02 47.36 61.70	33.02 47.36 61.70	1.67 2.22 2.77
*** ***									
RR90-17-00DP w/ Mount Pipe	A	From Leg	2.00 0.00 0.00	0.000	101.00	No Ice 1/2" Ice 1" Ice	4.59 5.02 5.44	3.32 4.09 4.78	0.03 0.07 0.12
RR90-17-00DP w/ Mount Pipe	B	From Leg	2.00 0.00 0.00	0.000	101.00	No Ice 1/2" Ice 1" Ice	4.59 5.02 5.44	3.32 4.09 4.78	0.03 0.07 0.12
RR90-17-00DP w/ Mount Pipe	C	From Leg	2.00 0.00 0.00	0.000	101.00	No Ice 1/2" Ice 1" Ice	4.59 5.02 5.44	3.32 4.09 4.78	0.03 0.07 0.12
LNX-6515DS-VTM w/ Mount Pipe	A	From Leg	2.00 0.00 0.00	0.000	101.00	No Ice 1/2" Ice 1" Ice	11.68 12.40 13.14	9.84 11.37 12.91	0.08 0.17 0.27
LNX-6515DS-VTM w/ Mount Pipe	B	From Leg	2.00 0.00 0.00	0.000	101.00	No Ice 1/2" Ice 1" Ice	11.68 12.40 13.14	9.84 11.37 12.91	0.08 0.17 0.27
LNX-6515DS-VTM w/ Mount Pipe	C	From Leg	2.00 0.00 0.00	0.000	101.00	No Ice 1/2" Ice 1" Ice	11.68 12.40 13.14	9.84 11.37 12.91	0.08 0.17 0.27
KRY 112 13/1	A	From Leg	2.00 0.00 0.00	0.000	101.00	No Ice 1/2" Ice 1" Ice	0.29 0.36 0.44	0.21 0.27 0.34	0.00 0.01 0.01
KRY 112 13/1	B	From Leg	2.00 0.00 0.00	0.000	101.00	No Ice 1/2" Ice 1" Ice	0.29 0.36 0.44	0.21 0.27 0.34	0.00 0.01 0.01
KRY 112 13/1	C	From Leg	2.00 0.00 0.00	0.000	101.00	No Ice 1/2" Ice 1" Ice	0.29 0.36 0.44	0.21 0.27 0.34	0.00 0.01 0.01
ATBT-BOTTOM-24V	A	From Leg	2.00	0.000	101.00	No Ice	0.10	0.06	0.00

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz	Lateral					
			0.00						
			0.00			1/2" Ice	0.15	0.10	0.00
			0.00			1" Ice	0.20	0.15	0.01
ATBT-BOTTOM-24V	B	From Leg	2.00	0.000	101.00	No Ice	0.10	0.06	0.00
			0.00			1/2" Ice	0.15	0.10	0.00
			0.00			1" Ice	0.20	0.15	0.01
ATBT-BOTTOM-24V	C	From Leg	2.00	0.000	101.00	No Ice	0.10	0.06	0.00
			0.00			1/2" Ice	0.15	0.10	0.00
			0.00			1" Ice	0.20	0.15	0.01
Side Arm Mount [SO 304-3]	C	None		0.000	101.00	No Ice	1.76	1.76	0.07
						1/2" Ice	2.75	2.75	0.10
						1" Ice	3.74	3.74	0.12
1.9" x10' Horizontal Pipe	A	From Leg	0.50	0.000	101.00	No Ice	0.04	1.90	0.03
			0.00			1/2" Ice	0.06	2.92	0.04
			0.00			1" Ice	0.10	3.96	0.08
1.9" x10' Horizontal Pipe	B	From Leg	0.50	0.000	101.00	No Ice	0.04	1.90	0.03
			0.00			1/2" Ice	0.06	2.92	0.04
			0.00			1" Ice	0.10	3.96	0.08
1.9" x10' Horizontal Pipe	C	From Leg	0.50	0.000	101.00	No Ice	0.04	1.90	0.03
			0.00			1/2" Ice	0.06	2.92	0.04
			0.00			1" Ice	0.10	3.96	0.08

GPS_A	C	From Leg	2.00	0.000	60.00	No Ice	0.26	0.26	0.00
			0.00			1/2" Ice	0.32	0.32	0.00
			0.00			1" Ice	0.39	0.39	0.01
Side Arm Mount [SO 201-1]	C	From Leg	0.00	0.000	60.00	No Ice	2.96	2.11	0.10
			0.00			1/2" Ice	4.10	2.93	0.12
			0.00			1" Ice	5.24	3.75	0.14

GPS_A	C	From Leg	0.50	0.000	50.00	No Ice	0.26	0.26	0.00
			0.00			1/2" Ice	0.32	0.32	0.00
			0.00			1" Ice	0.39	0.39	0.01

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice

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Comb. No.	Description
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	140 - 120	Leg	Max Tension	15	14.83	-0.00	-0.00
			Max. Compression	10	-19.66	0.08	-0.01
			Max. Mx	22	-0.44	0.69	-0.01
			Max. My	16	-1.48	0.01	-0.74
			Max. Vy	3	-1.21	0.52	-0.01
			Max. Vx	4	1.20	-0.00	-0.46
		Diagonal	Max Tension	9	3.42	0.00	0.00
			Max. Compression	20	-3.53	0.00	0.00
			Max. Mx	35	0.74	0.04	-0.00
			Max. My	12	-2.06	0.00	0.00
			Max. Vy	35	-0.04	0.04	-0.00
			Max. Vx	12	-0.00	0.00	0.00
		Top Girt	Max Tension	14	0.45	0.00	0.00
			Max. Compression	11	-0.42	0.00	0.00
			Max. Mx	26	0.03	-0.09	0.00
			Max. My	26	0.04	0.00	0.00
			Max. Vy	26	0.05	0.00	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft		
T2	120 - 100	Leg	Max. Vx	26	-0.00	0.00	0.00		
			Max Tension	23	37.63	-0.11	0.00		
			Max. Compression	2	-45.17	0.26	0.01		
			Max. Mx	10	-45.10	0.26	-0.00		
			Max. My	24	-4.13	-0.00	0.20		
			Max. Vy	6	0.36	-0.25	0.00		
		Diagonal	Max. Vx	24	-0.34	-0.00	0.20		
			Max Tension	8	3.10	0.00	0.00		
			Max. Compression	8	-3.13	0.00	0.00		
			Max. Mx	35	0.94	0.04	-0.01		
			Max. My	32	0.95	0.04	-0.01		
			Max. Vy	37	0.04	0.04	-0.01		
		Top Girt	Max. Vx	32	0.00	0.00	0.00		
			Max Tension	6	0.19	0.00	0.00		
			Max. Compression	19	-0.19	0.00	0.00		
			Max. Mx	26	-0.01	-0.09	0.00		
			Max. My	26	-0.00	0.00	0.00		
			Max. Vy	26	0.05	0.00	0.00		
T3	100 - 80	Leg	Max. Vx	26	-0.00	0.00	0.00		
			Max Tension	23	58.59	-0.15	0.00		
			Max. Compression	2	-68.51	0.14	0.01		
			Max. Mx	10	-51.97	0.26	-0.00		
			Max. My	24	-4.92	-0.01	0.21		
			Max. Vy	10	0.07	0.26	-0.00		
		Diagonal	Max. Vx	2	-0.06	-0.09	0.19		
			Max Tension	4	3.84	0.00	0.00		
			Max. Compression	4	-3.89	0.00	0.00		
			Max. Mx	37	0.93	0.07	-0.01		
			Max. My	34	-1.38	0.06	-0.01		
			Max. Vy	37	0.06	0.07	-0.01		
		T4	80 - 60	Leg	Max. Vx	34	0.00	0.00	0.00
					Max Tension	23	78.64	-0.15	-0.00
					Max. Compression	2	-91.32	0.25	0.01
					Max. Mx	10	-90.44	0.25	0.00
					Max. My	24	-5.82	-0.01	0.23
					Max. Vy	33	-0.06	-0.18	-0.01
Diagonal	Max. Vx			2	-0.06	-0.09	0.22		
	Max Tension			16	4.16	0.00	0.00		
	Max. Compression			16	-4.19	0.00	0.00		
	Max. Mx			37	0.82	0.11	-0.01		
	Max. My			27	0.76	0.11	0.01		
	Max. Vy			37	0.08	0.11	-0.01		
T5	60 - 40			Leg	Max. Vx	27	-0.00	0.00	0.00
					Max Tension	23	97.25	-0.20	-0.00
					Max. Compression	2	-113.11	0.33	0.01
					Max. Mx	33	5.48	-0.56	-0.01
					Max. My	24	-6.70	-0.01	0.27
					Max. Vy	33	0.15	-0.56	-0.01
		Diagonal	Max. Vx	12	-0.06	0.01	-0.16		
			Max Tension	16	4.54	0.00	0.00		
			Max. Compression	16	-4.57	0.00	0.00		
			Max. Mx	35	1.13	0.16	0.02		
			Max. My	34	-1.32	0.13	-0.02		
			Max. Vy	37	0.09	0.14	0.02		
		Leg	Max. Vx	34	0.01	0.00	0.00		
			Max Tension	23	113.78	-0.34	-0.00		
			Max. Compression	2	-132.77	0.56	0.02		
			Max. Mx	33	6.44	-0.93	-0.00		
			Max. My	24	-8.15	-0.04	0.66		
			Max. Vy	33	0.17	-0.93	-0.00		
Leg	Max. Vx	20	-0.13	-0.05	-0.65				

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T7	20 - 0	Diagonal	Max Tension	16	5.40	0.00	0.00
			Max. Compression	16	-5.51	0.00	0.00
			Max. Mx	37	0.74	0.20	-0.02
			Max. My	27	1.57	0.19	0.03
			Max. Vy	37	0.10	0.19	0.03
			Max. Vx	27	-0.01	0.00	0.00
		Leg	Max Tension	23	135.76	0.60	-0.00
			Max. Compression	2	-159.10	0.00	-0.00
			Max. Mx	33	9.35	-0.93	-0.00
			Max. My	24	-9.43	-0.05	0.94
			Max. Vy	10	-8.14	0.00	0.00
			Max. Vx	12	2.12	0.00	0.00
		Diagonal	Max Tension	16	5.69	0.00	0.00
			Max. Compression	16	-5.80	0.00	0.00
			Max. Mx	37	0.39	0.25	-0.03
			Max. My	38	2.49	0.20	0.03
			Max. Vy	38	0.11	0.21	0.03
			Max. Vx	38	-0.01	0.00	0.00

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	18	157.50	14.81	-8.74
	Max. H _x	18	157.50	14.81	-8.74
	Max. H _z	7	-133.81	-12.93	7.61
	Min. Vert	7	-133.81	-12.93	7.61
	Min. H _x	7	-133.81	-12.93	7.61
	Min. H _z	18	157.50	14.81	-8.74
Leg B	Max. Vert	10	156.77	-14.91	-8.57
	Max. H _x	23	-135.12	13.05	7.49
	Max. H _z	23	-135.12	13.05	7.49
	Min. Vert	23	-135.12	13.05	7.49
	Min. H _x	10	156.77	-14.91	-8.57
	Min. H _z	10	156.77	-14.91	-8.57
Leg A	Max. Vert	2	158.37	-0.22	17.23
	Max. H _x	21	8.01	2.02	0.58
	Max. H _z	2	158.37	-0.22	17.23
	Min. Vert	15	-133.91	0.19	-15.02
	Min. H _x	8	10.10	-2.04	0.74
	Min. H _z	15	-133.91	0.19	-15.02

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overtuning Moment, M _x kip-ft	Overtuning Moment, M _z kip-ft	Torque kip-ft
Dead Only	24.18	-0.00	-0.00	-9.21	9.11	0.00
1.2 Dead+1.6 Wind 0 deg - No Ice	29.01	-0.02	-27.86	-2417.26	14.99	-6.84
0.9 Dead+1.6 Wind 0 deg - No Ice	21.76	-0.02	-27.86	-2414.49	12.26	-6.84

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Load Combination	Vertical K	Shear _x K	Shear _y K	Overturning Moment, M _x kip-ft	Overturning Moment, M _y kip-ft	Torque kip-ft
1.2 Dead+1.6 Wind 30 deg - No Ice	29.01	13.14	-22.80	-1995.37	-1130.03	-6.30
0.9 Dead+1.6 Wind 30 deg - No Ice	21.76	13.14	-22.80	-1992.61	-1132.76	-6.30
1.2 Dead+1.6 Wind 60 deg - No Ice	29.01	22.75	-13.13	-1159.09	-1977.53	-4.41
0.9 Dead+1.6 Wind 60 deg - No Ice	21.76	22.75	-13.13	-1156.33	-1980.27	-4.41
1.2 Dead+1.6 Wind 90 deg - No Ice	29.01	26.32	0.02	-6.99	-2278.02	-2.05
0.9 Dead+1.6 Wind 90 deg - No Ice	21.76	26.32	0.02	-4.23	-2280.75	-2.05
1.2 Dead+1.6 Wind 120 deg - No Ice	29.01	24.11	13.95	1195.56	-2070.87	1.17
0.9 Dead+1.6 Wind 120 deg - No Ice	21.76	24.11	13.95	1198.33	-2073.60	1.17
1.2 Dead+1.6 Wind 150 deg - No Ice	29.01	13.18	22.83	1977.33	-1137.06	4.36
0.9 Dead+1.6 Wind 150 deg - No Ice	21.76	13.18	22.83	1980.09	-1139.79	4.36
1.2 Dead+1.6 Wind 180 deg - No Ice	29.01	0.02	26.31	2292.06	6.87	6.30
0.9 Dead+1.6 Wind 180 deg - No Ice	21.76	0.02	26.31	2294.83	4.14	6.30
1.2 Dead+1.6 Wind 210 deg - No Ice	29.01	-13.14	22.80	1973.27	1151.89	6.30
0.9 Dead+1.6 Wind 210 deg - No Ice	21.76	-13.14	22.80	1976.03	1149.16	6.30
1.2 Dead+1.6 Wind 240 deg - No Ice	29.01	-24.09	13.91	1188.53	2088.68	4.84
0.9 Dead+1.6 Wind 240 deg - No Ice	21.76	-24.09	13.91	1191.30	2085.94	4.84
1.2 Dead+1.6 Wind 270 deg - No Ice	29.01	-26.32	-0.02	-15.11	2299.89	2.05
0.9 Dead+1.6 Wind 270 deg - No Ice	21.76	-26.32	-0.02	-12.35	2297.15	2.05
1.2 Dead+1.6 Wind 300 deg - No Ice	29.01	-22.77	-13.18	-1166.13	2003.46	-1.05
0.9 Dead+1.6 Wind 300 deg - No Ice	21.76	-22.77	-13.18	-1163.36	2000.73	-1.05
1.2 Dead+1.6 Wind 330 deg - No Ice	29.01	-13.18	-22.83	-1999.43	1158.93	-4.36
0.9 Dead+1.6 Wind 330 deg - No Ice	21.76	-13.18	-22.83	-1996.67	1156.19	-4.36
1.2 Dead+1.0 Ice+1.0 Temp	100.64	-0.00	-0.00	-34.27	48.01	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	100.64	-0.00	-8.56	-788.40	48.65	-1.73
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	100.64	4.17	-7.23	-674.42	-320.85	-1.87
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	100.64	7.23	-4.17	-404.19	-592.71	-1.54
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	100.64	8.35	0.00	-33.64	-690.80	-0.87
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	100.64	7.41	4.28	343.34	-604.76	0.04
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	100.64	4.18	7.24	606.51	-321.94	0.99
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	100.64	0.00	8.35	706.66	47.38	1.67
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	100.64	-4.17	7.23	605.87	416.87	1.87

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Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	100.64	-7.41	4.28	342.24	700.16	1.60
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	100.64	-8.35	-0.00	-34.91	786.83	0.87
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	100.64	-7.23	-4.18	-405.29	689.37	-0.04
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	100.64	-4.18	-7.24	-675.05	417.97	-0.99
Dead+Wind 0 deg - Service	24.18	-0.01	-6.66	-584.61	10.08	-1.64
Dead+Wind 30 deg - Service	24.18	3.14	-5.45	-483.73	-263.73	-1.51
Dead+Wind 60 deg - Service	24.18	5.44	-3.14	-283.75	-466.40	-1.05
Dead+Wind 90 deg - Service	24.18	6.29	0.01	-8.24	-538.25	-0.49
Dead+Wind 120 deg - Service	24.18	5.77	3.34	279.33	-488.72	0.28
Dead+Wind 150 deg - Service	24.18	3.15	5.46	466.28	-265.41	1.04
Dead+Wind 180 deg - Service	24.18	0.01	6.29	541.54	8.14	1.51
Dead+Wind 210 deg - Service	24.18	-3.14	5.45	465.31	281.95	1.51
Dead+Wind 240 deg - Service	24.18	-5.76	3.33	277.65	505.97	1.16
Dead+Wind 270 deg - Service	24.18	-6.29	-0.01	-10.18	556.47	0.49
Dead+Wind 300 deg - Service	24.18	-5.45	-3.15	-285.43	485.59	-0.25
Dead+Wind 330 deg - Service	24.18	-3.15	-5.46	-484.70	283.63	-1.04

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-24.18	0.00	0.00	24.18	0.00	0.000%
2	-0.02	-29.01	-27.86	0.02	29.01	27.86	0.000%
3	-0.02	-21.76	-27.86	0.02	21.76	27.86	0.000%
4	13.14	-29.01	-22.80	-13.14	29.01	22.80	0.000%
5	13.14	-21.76	-22.80	-13.14	21.76	22.80	0.000%
6	22.75	-29.01	-13.13	-22.75	29.01	13.13	0.000%
7	22.75	-21.76	-13.13	-22.75	21.76	13.13	0.000%
8	26.32	-29.01	0.02	-26.32	29.01	-0.02	0.000%
9	26.32	-21.76	0.02	-26.32	21.76	-0.02	0.000%
10	24.11	-29.01	13.95	-24.11	29.01	-13.95	0.000%
11	24.11	-21.76	13.95	-24.11	21.76	-13.95	0.000%
12	13.18	-29.01	22.83	-13.18	29.01	-22.83	0.000%
13	13.18	-21.76	22.83	-13.18	21.76	-22.83	0.000%
14	0.02	-29.01	26.31	-0.02	29.01	-26.31	0.000%
15	0.02	-21.76	26.31	-0.02	21.76	-26.31	0.000%
16	-13.14	-29.01	22.80	13.14	29.01	-22.80	0.000%
17	-13.14	-21.76	22.80	13.14	21.76	-22.80	0.000%
18	-24.09	-29.01	13.91	24.09	29.01	-13.91	0.000%
19	-24.09	-21.76	13.91	24.09	21.76	-13.91	0.000%
20	-26.32	-29.01	-0.02	26.32	29.01	0.02	0.000%
21	-26.32	-21.76	-0.02	26.32	21.76	0.02	0.000%
22	-22.77	-29.01	-13.18	22.77	29.01	13.18	0.000%
23	-22.77	-21.76	-13.18	22.77	21.76	13.18	0.000%
24	-13.18	-29.01	-22.83	13.18	29.01	22.83	0.000%
25	-13.18	-21.76	-22.83	13.18	21.76	22.83	0.000%
26	0.00	-100.64	0.00	0.00	100.64	0.00	0.000%
27	-0.00	-100.64	-8.56	0.00	100.64	8.56	0.000%
28	4.17	-100.64	-7.23	-4.17	100.64	7.23	0.000%
29	7.23	-100.64	-4.17	-7.23	100.64	4.17	0.000%
30	8.35	-100.64	0.00	-8.35	100.64	-0.00	0.000%
31	7.41	-100.64	4.28	-7.41	100.64	-4.28	0.000%
32	4.18	-100.64	7.24	-4.18	100.64	-7.24	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
33	0.00	-100.64	8.35	-0.00	100.64	-8.35	0.000%
34	-4.17	-100.64	7.23	4.17	100.64	-7.23	0.000%
35	-7.41	-100.64	4.28	7.41	100.64	-4.28	0.000%
36	-8.35	-100.64	-0.00	8.35	100.64	0.00	0.000%
37	-7.23	-100.64	-4.18	7.23	100.64	4.18	0.000%
38	-4.18	-100.64	-7.24	4.18	100.64	7.24	0.000%
39	-0.01	-24.18	-6.66	0.01	24.18	6.66	0.000%
40	3.14	-24.18	-5.45	-3.14	24.18	5.45	0.000%
41	5.44	-24.18	-3.14	-5.44	24.18	3.14	0.000%
42	6.29	-24.18	0.01	-6.29	24.18	-0.01	0.000%
43	5.77	-24.18	3.34	-5.77	24.18	-3.34	0.000%
44	3.15	-24.18	5.46	-3.15	24.18	-5.46	0.000%
45	0.01	-24.18	6.29	-0.01	24.18	-6.29	0.000%
46	-3.14	-24.18	5.45	3.14	24.18	-5.45	0.000%
47	-5.76	-24.18	3.33	5.76	24.18	-3.33	0.000%
48	-6.29	-24.18	-0.01	6.29	24.18	0.01	0.000%
49	-5.45	-24.18	-3.15	5.45	24.18	3.15	0.000%
50	-3.15	-24.18	-5.46	3.15	24.18	5.46	0.000%

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	140 - 120	2.66	39	0.174	0.006
T2	120 - 100	1.94	39	0.156	0.006
T3	100 - 80	1.32	39	0.127	0.005
T4	80 - 60	0.82	39	0.097	0.004
T5	60 - 40	0.46	39	0.067	0.003
T6	40 - 20	0.21	39	0.041	0.002
T7	20 - 0	0.06	39	0.021	0.001

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
137.00	LNX-8513DS-A1M w/ Mount Pipe	39	2.55	0.172	0.006	153423
124.00	NNVV-65B-R4 w/ Mount Pipe	39	2.08	0.161	0.006	47964
101.00	RR90-17-00DP w/ Mount Pipe	39	1.34	0.129	0.005	37638
60.00	GPS_A	39	0.46	0.067	0.003	40076
50.00	GPS_A	39	0.32	0.053	0.002	46134

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	140 - 120	11.04	2	0.726	0.025
T2	120 - 100	8.03	2	0.650	0.024
T3	100 - 80	5.45	2	0.527	0.020
T4	80 - 60	3.40	2	0.400	0.016
T5	60 - 40	1.89	2	0.278	0.011
T6	40 - 20	0.87	2	0.168	0.007
T7	20 - 0	0.25	2	0.086	0.003

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
137.00	LNx-8513DS-A1M w/ Mount Pipe	2	10.58	0.717	0.025	37061
124.00	NNVV-65B-R4 w/ Mount Pipe	2	8.61	0.669	0.025	11586
101.00	RR90-17-00DP w/ Mount Pipe	2	5.57	0.534	0.021	9087
60.00	GPS_A	2	1.89	0.278	0.011	9679
50.00	GPS_A	2	1.33	0.220	0.009	11153

Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	140	Leg	A325N	0.63	4	3.56	20.71	0.172	1	Bolt Tension
		Diagonal	A325N	0.50	1	3.42	5.71	0.598	1	Member Block Shear
T2	120	Top Girt	A325N	0.50	1	0.45	4.13	0.109	1	Member Bearing
		Leg	A325N	0.75	4	9.41	29.82	0.315	1	Bolt Tension
T3	100	Diagonal	A325N	0.50	1	3.10	5.71	0.543	1	Member Block Shear
		Top Girt	A325N	0.50	1	0.19	4.13	0.046	1	Member Bearing
T4	80	Leg	A325N	0.88	4	14.65	40.59	0.361	1	Bolt Tension
		Diagonal	A325N	0.50	1	3.84	6.20	0.620	1	Member Bearing
T5	60	Leg	A325N	0.88	4	19.66	40.59	0.484	1	Bolt Tension
		Diagonal	A325N	0.50	1	4.16	6.20	0.671	1	Member Bearing
T6	40	Leg	A325N	1.00	4	24.31	53.01	0.459	1	Bolt Tension
		Diagonal	A325N	0.50	1	4.54	6.20	0.733	1	Member Bearing
T7	20	Leg	A325N	1.00	4	28.45	53.01	0.537	1	Bolt Tension
		Diagonal	A325N	0.63	1	5.40	7.83	0.690	1	Member Bearing
		Diagonal	A325N	0.63	1	5.69	11.70	0.487	1	Member Bearing

Compression Checks

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Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	140 - 120	ROHN 2 STD	20.00	4.00	61.0	1.07	-19.66	36.84	0.534 ¹
T2	120 - 100	ROHN 2.5 EH	20.03	5.01	65.0	2.25	-45.17	74.43	0.607 ¹
T3	100 - 80	ROHN 3 EH	20.03	6.68	70.5	3.02	-68.51	94.34	0.726 ¹
T4	80 - 60	ROHN 3.5 EH	20.03	6.68	61.3	3.68	-91.32	125.73	0.726 ¹
T5	60 - 40	ROHN 4 X-STR	20.04	6.68	54.3	4.41	-113.11	159.90	0.707 ¹
T6	40 - 20	ROHN 5 EH	20.03	10.02	65.4	6.11	-132.77	201.25	0.660 ¹
T7	20 - 0	ROHN 5 X-STR	20.03	9.97	65.1	6.11	-153.32	201.77	0.760 ¹

¹ P_u / φP_n controls

Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	140 - 120	L1 3/4x1 3/4x3/16	7.68	3.62	126.6	0.62	-3.53	8.66	0.408 ¹
T2	120 - 100	L1 3/4x1 3/4x3/16	9.73	4.77	166.7	0.62	-3.07	5.05	0.607 ¹
T3	100 - 80	L2x2x3/16	12.27	6.06	184.5	0.71	-3.89	4.74	0.820 ¹
T4	80 - 60	L2 1/2x2 1/2x3/16	14.02	6.91	167.6	0.90	-4.19	7.26	0.578 ¹
T5	60 - 40	L3x3x3/16	15.89	7.83	157.6	1.09	-4.57	9.92	0.461 ¹
T6	40 - 20	L3x3x3/16	19.10	9.45	190.3	1.09	-5.51	6.80	0.811 ¹
T7	20 - 0	L3x3x1/4	20.80	10.30	208.8	1.44	-5.80	7.46	0.778 ¹

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¹ P_u / φP_n controls

Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
T1	140 - 120	L2x2x1/8	6.52	6.11	184.6	0.48	-0.42	3.21	0.130 ¹
T2	120 - 100	L2x2x1/8	6.56	6.16	185.8	0.48	-0.19	3.17	0.059 ¹

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Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	φP _n	Ratio P _u / φP _n
	ft		ft	ft		in ²	K	K	
K=1.00									

¹ P_u / φP_n controls

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	φP _n	Ratio P _u / φP _n
	ft		ft	ft		in ²	K	K	
T1	140 - 120	ROHN 2 STD	20.00	4.00	61.0	1.07	14.23	48.35	0.294 ¹
T2	120 - 100	ROHN 2.5 EH	20.03	5.01	65.0	2.25	37.63	101.41	0.371 ¹
T3	100 - 80	ROHN 3 EH	20.03	6.68	70.5	3.02	58.59	135.72	0.432 ¹
T4	80 - 60	ROHN 3.5 EH	20.03	6.68	61.3	3.68	78.64	165.53	0.475 ¹
T5	60 - 40	ROHN 4 X-STR	20.04	6.68	54.3	4.41	97.25	198.34	0.490 ¹
T6	40 - 20	ROHN 5 EH	20.03	10.02	65.4	6.11	113.78	275.04	0.414 ¹
T7	20 - 0	ROHN 5 X-STR	20.03	0.08	0.5	6.11	135.76	275.04	0.494 ¹

¹ P_u / φP_n controls

Diagonal Design Data (Tension)

Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	φP _n	Ratio P _u / φP _n
	ft		ft	ft		in ²	K	K	
T1	140 - 120	L1 3/4x1 3/4x3/16	7.68	3.62	83.3	0.38	3.42	16.44	0.208 ¹
T2	120 - 100	L1 3/4x1 3/4x3/16	9.73	4.77	109.0	0.38	3.10	16.44	0.188 ¹
T3	100 - 80	L2x2x3/16	12.27	6.06	119.8	0.45	3.84	19.50	0.197 ¹
T4	80 - 60	L2 1/2x2 1/2x3/16	14.02	6.91	108.2	0.59	4.16	25.60	0.163 ¹
T5	60 - 40	L3x3x3/16	15.89	7.83	101.3	0.73	4.54	31.74	0.143 ¹
T6	40 - 20	L3x3x3/16	19.10	9.45	122.3	0.71	5.40	30.97	0.174 ¹
T7	20 - 0	L3x3x1/4	20.80	10.30	134.5	0.94	5.69	45.79	0.124 ¹

¹ P_u / φP_n controls

Top Girt Design Data (Tension)

Section No.	Elevation	Size	L	L _u	Kl/r	A	P _u	φP _n	Ratio P _u / φP _n
	ft		ft	ft		in ²	K	K	
T1	140 - 120	L2x2x1/8	6.52	6.11	121.2	0.30	0.45	13.25	0.034 ¹
T2	120 - 100	L2x2x1/8	6.56	6.16	122.0	0.30	0.19	13.25	0.014 ¹

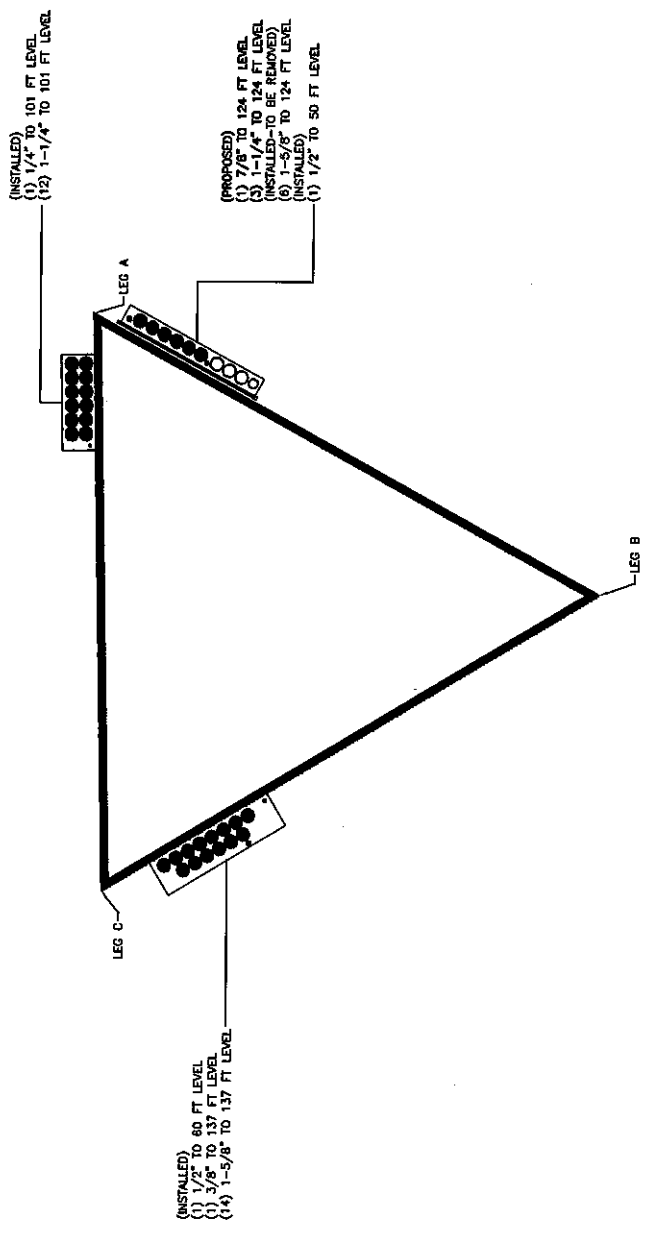
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¹ P_n / φP_n controls

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	φP _{allow} K	% Capacity	Pass Fail
T1	140 - 120	Leg	ROHN 2 STD	2	-19.66	36.84	53.4	Pass
T2	120 - 100	Leg	ROHN 2.5 EH	39	-45.17	74.43	60.7	Pass
T3	100 - 80	Leg	ROHN 3 EH	69	-68.51	94.34	72.6	Pass
T4	80 - 60	Leg	ROHN 3.5 EH	90	-91.32	125.73	72.6	Pass
T5	60 - 40	Leg	ROHN 4 X-STR	111	-113.11	159.90	70.7	Pass
T6	40 - 20	Leg	ROHN 5 EH	132	-132.77	201.25	66.0	Pass
T7	20 - 0	Leg	ROHN 5 X-STR	147	-153.32	201.77	76.0	Pass
T1	140 - 120	Diagonal	L1 3/4x1 3/4x3/16	7	-3.53	8.66	40.8	Pass
							59.8 (b)	
T2	120 - 100	Diagonal	L1 3/4x1 3/4x3/16	44	-3.07	5.05	60.7	Pass
T3	100 - 80	Diagonal	L2x2x3/16	74	-3.89	4.74	82.0	Pass
T4	80 - 60	Diagonal	L2 1/2x2 1/2x3/16	96	-4.19	7.26	57.8	Pass
							67.1 (b)	
T5	60 - 40	Diagonal	L3x3x3/16	117	-4.57	9.92	46.1	Pass
							73.3 (b)	
T6	40 - 20	Diagonal	L3x3x3/16	138	-5.51	6.80	81.1	Pass
T7	20 - 0	Diagonal	L3x3x1/4	153	-5.80	7.46	77.8	Pass
T1	140 - 120	Top Girt	L2x2x1/8	6	-0.42	3.21	13.0	Pass
T2	120 - 100	Top Girt	L2x2x1/8	41	-0.19	3.17	5.9	Pass
							Summary	
							Leg (T7)	76.0 Pass
							Diagonal (T3)	82.0 Pass
							Top Girt (T1)	13.0 Pass
							Bolt Checks	73.3 Pass
							RATING =	82.0 Pass

APPENDIX B
BASE LEVEL DRAWING



APPENDIX C
ADDITIONAL CALCULATIONS

Anchor Rod Check for Self Supporting Towers

TIA-222-G, Section 4.9.9

Site Data	
BU#:	806383
Site Name:	HRT 087 943325
App:	441319 Rev. 0

Anchor Rod Data	
Qty:	4
Diam:	1 in
Rod Material:	A449 (1/4 to 1 Incl.)
Strength (Fu):	120 ksi
Yield (Fy):	92 ksi

* Rod Circle:	in
* e:	in
* # of Rods	1 or 2

Mu = Pu x e:	ft-kips
--------------	---------

* Enter rod circle, offset (e) and number of anchor rods at the extreme fiber to consider if eccentric load due to leg reinforcement exists.

Reactions		
Eta Factor, η	0.55	Detail Type
Uplift, Pu:	135	kips
Shear, Vu:	15	kips

	in
Mu = 0.65 * I _{bar} * Vu	ft-kips

Anchor Rod Results:

Max Rod (Cu + Vu/η):	40.6	Kips
Allowable Axial, φ*Fu*Anet:	58.2	Kips
Anchor Rod Stress Ratio:	69.7%	

If Applicable;

Anchor Rod Results with Bending Considered:

When the clear distance from the top of concrete to the bottom of level nut exceeds 1.0 times the diameter of the anchor rod, the following interaction equation shall also be satisfied (see Figure 4-4 of Rev. G):

$$\left(\frac{V_u}{\phi R_{nv}}\right)^2 + \left[\left(\frac{P_u}{\phi R_{nt}}\right) + \left(\frac{M_u}{\phi R_{nm}}\right)\right]^2 <= 1$$

$\phi R_{nv} = \phi * 0.45 * F_{ub} * A_b =$		kips
$\phi R_{nt} = \phi * F_u * A_{net} =$		kips
$\phi R_{nm} = \phi * F_y * Z =$		ft-kips

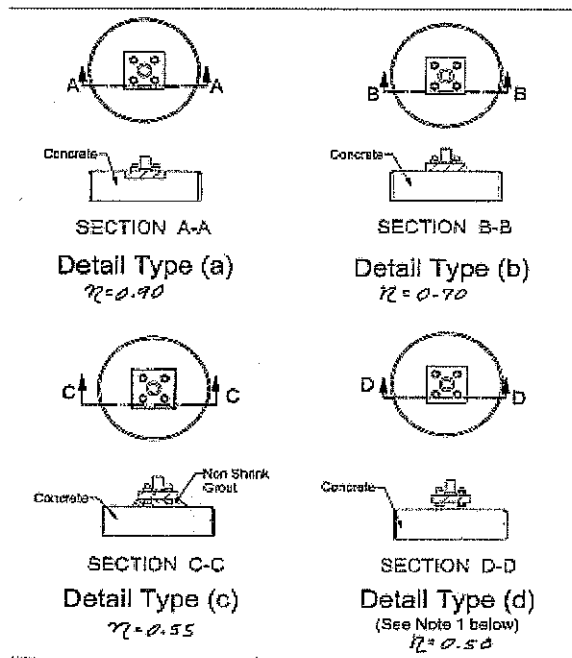


Figure 4-4 of TIA-222-G

Maximum Acceptable Ratio: %

Governing Stress Ratio: Pass

Pier and Pad Foundation



BU #: 805383
 Site Name: HRT 087 943325
 App. Number: 441319 Rev. 0

TIA-222 Revision: G
 Tower Type: Self Support

Block Foundation?:

Superstructure Analysis Reactions		
Compression, P_{comp} :	158	kips
Compression Shear, V_{u_comp} :	17	kips
Uplift, P_{uplift} :	135	kips
Uplift Shear, V_{u_uplift} :	15	kips
Tower Height, H:	140	ft
Base Face Width, BW:	18.77	ft
BP Dist. Above Fdn, bp_{dist} :	1.75	in

Foundation Analysis Checks				
	Capacity	Demand	Rating	Check
<i>Uplift (kips)</i>	317.37	135.00	42.5%	Pass
<i>Lateral (Sliding) (kips)</i>	99.58	15.00	15.1%	Pass
<i>Bearing Pressure (ksf)</i>	18.08	5.11	28.3%	Pass
<i>Pier Flexure (Comp.) (kip*ft)</i>	830.41	187.00	22.5%	Pass
<i>Pier Flexure (Tension) (kip*ft)</i>	617.21	165.00	26.7%	Pass
<i>Pier Compression (kip)</i>	1727.31	172.00	10.0%	Pass
<i>Pad Flexure (kip*ft)</i>	307.01	46.22	15.1%	Pass
<i>Pad Shear - 1-way (kips)</i>	137.16	7.94	5.8%	Pass
<i>Pad Shear - 2-way (ksi)</i>	0.16	0.03	17.1%	Pass

Soil Rating: 42.5%
 Structural Rating: 26.7%

Pier Properties		
Pier Shape:	Circular	
Pier Diameter, d_{pier} :	3.0	ft
Ext. Above Grade, E:	0.50	ft
Pier Rebar Size, S_c :	8	
Pier Rebar Quantity, m_c :	16	
Pier Tie/Spiral Size, S_t :	3	
Pier Tie/Spiral Quantity, m_t :	12	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, cc_{pier} :	3	in

Pad Properties		
Depth, D:	12.5	ft
Pad Width, W:	7.0	ft
Pad Thickness, T:	2.0	ft
Pad Rebar Size, S_p :	6	
Pad Rebar Quantity, m_p :	8	
Pad Clear Cover, cc_{pad} :	3	in

Material Properties		
Rebar Grade, F_y :	60000	psi
Concrete Compressive Strength, F'_c :	3000	psi
Dry Concrete Density, δ_c :	150	pcf

Soil Properties		
Total Soil Unit Weight, γ :	115	pcf
Ultimate Net Bearing, Q_{net} :	22.670	ksf
Cohesion, C_u :	0.000	ksf
Friction Angle, ϕ :	35	degrees
SPT Blow Count, N_{blows} :	50	
Base Friction, μ :		
Neglected Depth, N:	3.00	ft
Foundation Bearing on Rock?	No	
Groundwater Depth, gw:	None	ft

<<-Toggle between Gross and Net



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RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

SPRINT Existing Facility

Site ID: CT03XC205

Willington (Crown)
Cosgrove Road
West Willington, CT 06279

July 31, 2018

EBI Project Number: 6218005227

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general population allowable limit:	11.53 %



July 31, 2018

SPRINT

Attn: RF Engineering Manager
1 International Boulevard, Suite 800
Mahwah, NJ 07495

Emissions Analysis for Site: **CT03XC205 – Willington (Crown)**

EBI Consulting was directed to analyze the proposed SPRINT facility located at **Cosgrove Road, West Willington, CT**, for the purpose of determining whether the emissions from the Proposed SPRINT Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

General population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limits for the 850 MHz Band is approximately $567 \mu\text{W}/\text{cm}^2$. The general population exposure limit for the 1900 MHz (PCS) and 2500 MHz (BRS) bands is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



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Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed SPRINT Wireless antenna facility located at **Cosgrove Road, West Willington, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since SPRINT is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 1 CDMA channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.
- 2) 2 LTE channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 50 Watts per Channel.
- 3) 5 CDMA channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 16 Watts per Channel.
- 4) 2 LTE channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 5) 8 LTE channels (2500 MHz (BRS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.



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- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the **Commscope NNVV-65B-R4** and the **RFS APXVTM14-ALU-I20** for transmission in the 850 MHz, 1900 MHz (PCS) and 2500 MHz (BRS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerlines of the proposed panel antennas are **125 feet** above ground level (AGL) for **Sector A**, **125 feet** above ground level (AGL) for **Sector B** and **125 feet** above ground level (AGL) for Sector C.
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general population threshold limits.



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SPRINT Site Inventory and Power Data by Antenna

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Commscope NNVV-65B-R4	Make / Model:	Commscope NNVV-65B-R4	Make / Model:	Commscope NNVV-65B-R4
Gain:	12.75 / 15.05 dBd	Gain:	12.75 / 15.05 dBd	Gain:	12.75 / 15.05 dBd
Height (AGL):	125 feet	Height (AGL):	125 feet	Height (AGL):	125 feet
Frequency Bands:	850 MHz / 1900 MHz (PCS)	Frequency Bands:	850 MHz / 1900 MHz (PCS)	Frequency Bands:	850 MHz / 1900 MHz (PCS)
Channel Count:	10	Channel Count:	10	Channel Count:	10
Total TX Power(W):	280 Watts	Total TX Power(W):	280 Watts	Total TX Power(W):	280 Watts
ERP (W):	7,378.61	ERP (W):	7,378.61	ERP (W):	7,378.61
Antenna A1 MPE%	2.31 %	Antenna B1 MPE%	2.31 %	Antenna C1 MPE%	2.31 %
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	RFS APXVTM14-ALU- I20	Make / Model:	RFS APXVTM14-ALU- I20	Make / Model:	RFS APXVTM14-ALU- I20
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	125 feet	Height (AGL):	125 feet	Height (AGL):	125 feet
Frequency Bands:	2500 MHz (BRS)	Frequency Bands:	2500 MHz (BRS)	Frequency Bands:	2500 MHz (BRS)
Channel Count:	8	Channel Count:	8	Channel Count:	8
Total TX Power(W):	160 Watts	Total TX Power(W):	160 Watts	Total TX Power(W):	160 Watts
ERP (W):	6,224.72	ERP (W):	6,224.72	ERP (W):	6,224.72
Antenna A2 MPE%	1.58 %	Antenna B2 MPE%	1.58 %	Antenna C2 MPE%	1.58 %

Site Composite MPE %	
Carrier	MPE%
SPRINT – Max per sector	3.89 %
Verizon Wireless	2.97 %
T-Mobile	3.37 %
Willington FD	0.10 %
MetroPCS	0.96 %
Nextel	0.24 %
Site Total MPE %:	11.53 %

SPRINT Sector A Total:	3.89 %
SPRINT Sector B Total:	3.89 %
SPRINT Sector C Total:	3.89 %
Site Total:	11.53 %

SPRINT Frequency Band / Technology (All Sectors)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ($\mu\text{W}/\text{cm}^2$)	Frequency (MHz)	Allowable MPE ($\mu\text{W}/\text{cm}^2$)	Calculated % MPE
Sprint 850 MHz CDMA	1	376.73	125	0.96	850 MHz	567	0.17%
Sprint 850 MHz LTE	2	941.82	125	4.78	850 MHz	567	0.84%
Sprint 1900 MHz (PCS) CDMA	5	511.82	125	6.50	1900 MHz (PCS)	1000	0.65%
Sprint 1900 MHz (PCS) LTE	2	1,279.56	125	6.50	1900 MHz (PCS)	1000	0.65%
Sprint 2500 MHz (BRS) LTE	8	778.09	125	15.80	2500 MHz (BRS)	1000	1.58%
						Total:	3.89%



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the SPRINT facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

SPRINT Sector	Power Density Value (%)
Sector A:	3.89 %
Sector B:	3.89 %
Sector C:	3.89 %
SPRINT Maximum MPE % (per sector):	3.89 %
Site Total:	11.53 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **11.53 %** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Barbadora, Jeff

From: TrackingUpdates@fedex.com
Sent: Wednesday, August 8, 2018 11:30 AM
To: Barbadora, Jeff
Subject: FedEx Shipment 772913263919 Delivered

Your package has been delivered

Tracking # 772913263919

Ship date:
Tue, 8/7/2018

Jeff Barbadora

Crown Castle
WOBURN, MA 01801
US



Delivery date:
Wed, 8/8/2018 11:28
am

First Selectman-Ms. Erika
Wiecenski
Town of Willington
40 Old Farms Road
WILLINGTON, CT 06279
US



Shipment Facts

Our records indicate that the following package has been delivered.

Tracking number: 772913263919

Status: Delivered: 08/08/2018 11:28
AM Signed for By:
E.WIECHELE

Reference: 1766.6680

Signed for by: E.WIECHELE

Delivery location: WILLINGTON, CT

Delivered to: Receptionist/Front Desk

Service type: FedEx Priority Overnight®

Packaging type: FedEx® Envelope

Number of pieces: 1

Weight: 1.00 lb.

Special handling/Services: Deliver Weekday

Standard transit: 8/8/2018 by 12:00 pm

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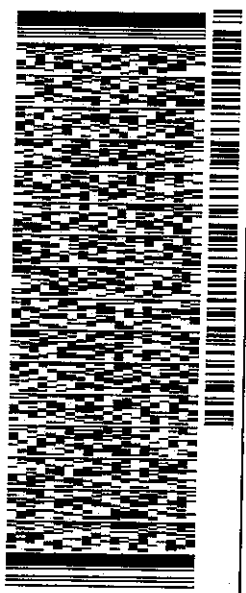
ORIGIN ID: BEDA (781) 970-0033
JEFF BARBOORA
DR OMAN CASTLE
12 GILL STREET
SUITE 3800
WOBBURN, MA 01801
UNITED STATES US

SHIP DATE: 07AUG18
ACTWGT: 0.50 LB
CAD: 104924191INLET4040
BILL SENDER

TO
MS. ISABELLA DROBNEY
DROBNEY RESIDENCE
56 COSGROVE ROAD

WILLINGTON CT 06279
(860) 429-4182 REF: 17668690
NY/ DEPT
PO:

552.J1.6309DCA5



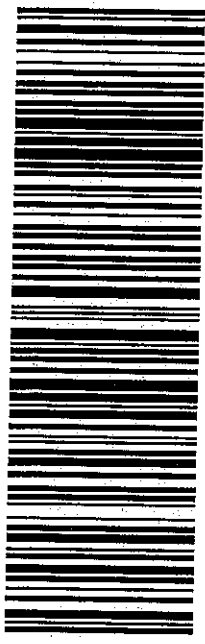
J182018972201uv

TRK# 7729 1332 6825
0201

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Tracking # 772913326825

Ship date:
Tue, 8/7/2018

Jeff Barbadora
Crown Castle
WOBURN, MA 01801
US

Delivery date:
Wed, 8/8/2018 11:37
am

Ms. Isabella Drobney
Drobney Residence
56 Cosgrove Road
WILLINGTON, CT 06279
US




Delivered



Shipment Facts

Our records indicate that the following package has been delivered.

Tracking number:	<u>772913326825</u>
Status:	Delivered: 08/08/2018 11:37 AM Signed for By: Signature not required
Reference:	1766.6680
Signed for by:	Signature not required
Delivery location:	WILLINGTON, CT
Delivered to:	Residence
Service type:	FedEx Priority Overnight®
Packaging type:	FedEx® Envelope
Number of pieces:	1
Weight:	1.00 lb.
Special handling/Services:	Deliver Weekday Residential Delivery
Standard transit:	8/8/2018 by 12:00 pm

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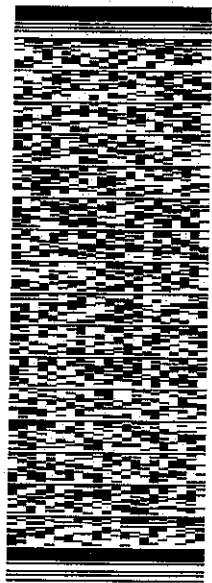
ORIGIN ID: BEDA (781) 970-0033
JEFF BARBARDOZA
OROWN CASTE
12 GILL STREET
SUITE 3800
WOBURN, MA 01801
UNITED STATES US

SHIP DATE: 07AUG18
ACT WGT: 0.50 LB
CAD: 104924197MINET4040
BILL SENDER

TO PLAN& ZONE COMM. MS. SUSAN YORGENSE
TOWN OF WILLINGTON
40 OLD FARMS ROAD

WILLINGTON CT 06279
(860) 487-3123 REF: 17666890
INV. DEPT.
PO.

552J18309/DCA5



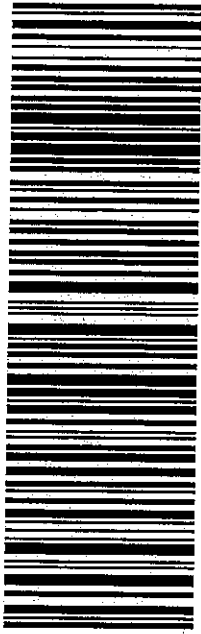
J182018072201uv

TRK# 7729 1328 2281
0201

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Barbadora, Jeff

From: TrackingUpdates@fedex.com
Sent: Wednesday, August 8, 2018 11:30 AM
To: Barbadora, Jeff
Subject: FedEx Shipment 772913282281 Delivered

Your package has been delivered

Tracking # 772913282281

Ship date:
Tue, 8/7/2018

Jeff Barbadora
Crown Castle
WOBURN, MA 01801
US

Delivery date:
**Wed, 8/8/2018 11:28
am**

Plan& Zone Comm. Ms.
Susan Yorgense
Town of Willington
40 Old Farms Road
WILLINGTON, CT 06279
US



Shipment Facts

Our records indicate that the following package has been delivered.

Tracking number: 772913282281
Status: Delivered: 08/08/2018 11:28 AM
Signed for By: E.WIECHELE
Reference: 1766.6680
Signed for by: E.WIECHELE
Delivery location: WILLINGTON, CT
Delivered to: Receptionist/Front Desk
Service type: FedEx Priority Overnight®
Packaging type: FedEx® Envelope
Number of pieces: 1
Weight: 1.00 lb.
Special handling/Services: Deliver Weekday
Standard transit: 8/8/2018 by 12:00 pm

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