Melanie A. Bachman
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051
RE: Sprint PCS-Exempt Modification - Crown Site BU: 876328
Sprint PCS Site ID: CT03XC075
Located at: 27-31 South Main Street, West Hartford, CT 06110
Dear Ms. Bachman:

This letter and exhibits are submitted on behalf of Sprint PCS (Sprint). Sprint is making modifications to certain existing sites in its Connecticut system in order to implement their 2.5GHz LTE technology. Please accept this letter and exhibits as notification, pursuant to § 16$50 j-73$ of the Regulations of Connecticut State Agencies ("R.C.S.A."), of construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to The Honorable Scott Slifka, Mayor for the Town of West Hartford.

Sprint plans to modify the existing wireless communications facility owned by Crown Castle and located at 27-31 South Main Street, West Hartford, CT 06110. Attached are a compound plan and elevation depicting the planned changes (Exhibit-1), and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration (Exhibit-2). Also included is a power density table report reflecting the modification to Sprint's operations at the site (Exhibit-3).

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes ("C.G.S.") § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in the R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Sprint's additional antennas will be located at the same elevation on the existing tower.
2. There will be no proposed modifications to the ground and no extension of boundaries.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more.
4. A Structural Modification Report confirming that the tower and foundation can support Sprint's proposed modifications is included as Exhibit-2.
5. The operation of the additional antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table report for Sprint's modified facility is included as Exhibit-3.

For the foregoing reasons, Sprint respectfully submits the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Donna Neal.

Sincerely,


Jeff Barbadora
Real Estate Specialist

## Enclosures

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes
Tab 2: Exhibit-2: Structural Modification Report
Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: The Honorable Scott Slifka, Mayor<br>Town of West Hartford<br>50 South Main Street<br>West Hartford, CT 06107



THESE OUULINE SPECIFICATIONS IN CONJUNCTION WITH THE SPRINT STANDARD CONSTRUCTION SPECIFICATIONS, INCLUDING CONTRACT DOCUMENTS SECTION 01100 - SCOPE OF WORK
PART 1 - GENERAL


1.2 REATED DOCUMENS:
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3. GR-78-CORE GENERC REQUREMENTS FOR THE PHTSICAL DESIGN AND


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6. American concrete instive (ACl)
B. AMERICAN WRE PRODUCERS ASSOCUTION (AWPA)
7. CONCREETE REINFORCING StEEL instivit (CRSI)
8. AMERICAN ASSOCIATION OF STAIE HIGHWAY AND TRANSPORTATION OFHCIALS
9. PORTLAND CEMENT ASSOCIATION (PCA)
10. NATONAL CONCREIE MASONRY ASSOCIATON (NCMA)
11. BRICK INDUSTRY ASSOCIATON (BA)
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part 3 - execution




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## SECTION 01200 - COMPANY FURNISHED MATERIAL AND EQUIPMENT

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## Sprint



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WEST HARTFORD, CT 06110

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2. Concreit break tests as specifed herein.
3. SPECML FNISHES FOR INERIOR SPACES, IF ANY.
4. AL EQUUPMENT AND MATERALS SO IDENIIED ON THE CONSTRUCTION
5. Chemical grounding desion
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20. STRUCTURAL BACKFLL COMPACTION TESTS FOR THE TOWER FOUNDATON.
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23. grounding at antenna masts for grs and antennas
24. AL OTHER TESTS REQUIRED EY COMPANY OR JURISOICTON.
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B. CONDUCT INSPECTONS INCLUDING BUT NOT UMITED TO THE FOLOWING:



25. PRE- AND POST-CONSTRUCTON ROOFTOP AND STRUCTURAL INSPECTIONS ON
26. TOWER ERECTON SECTON STACKING AND PLATORRM ATTACHMENT DOCUMENIED
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27-31 SOUTH MAIN ST
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SPRINT SPECIFICATIONS
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## CONTUE FROM SP-2

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4. TOWER STEEL AS BENG INSTALED INTO HOLE (SHOW ANCHOR STEEL ON
5. PHOTOS OF TOWER SECTION STACKING
6. CONCREEE TESTNG / SAMPIES.
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10. Shelter foundaton pour wit narator in use.
11. COAX CABLE ENRY INTO SHELITR
12. PLATORM MECHANICAL CONNECTONS TO TOWER/MONOPOLE.
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14. Photos of tower top coax line color cooing ano color cooing at
15. Photos of All approprate compant or regulatory signage.
16. PHotos of Equipmen bolt down insid shelitr.
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25. All ground test wels
26. ANTENNA GROUND BAR AND EQUIPMENT GRound bar.
27. Adomonal grounding points on towers above 200'.
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29. GPS ANIENNAS.
30. Cable tray ano/or waveguid bride.
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33. telco board and nit
34. ELECTRICAL DIITRIBUTION WALL
35. CABE ENTRY wTH SURGE SUPPRESSION.
36. Entrance to equipment room.
37. COAX WEATHERPROOFNG-TOP AND BOTOM OF TOWE
38. CoAX Grounding -top and bottom of tower
39. ANIENNA AND MAST GROUNDING.
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WEST HARTFORD
PARKING GARAGE

CT03XC075
27-31 SOUTH MAIN ST HARTFORD, CT 06110

SPRINT SPECIFICATIONS
SP-3








ALU 2.5 ALU SCENARIO 1

ran wiring diagram


RF 2.5 ALU SCENARIO 1




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WEST HARTFORD PARKING GARAGE

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| CT03XC075 |

27-31 SOUTH MAIN S WEST HARTFORD, CT 06110

PLUMBING DIAGRAM



Date: March $\mathbf{7}^{\text {th }}, 2014$
Cheryl Schultz
Crown Castle
3530 Toringdon WaySuite 300
Charlotte, NC 28277
(704) 405-6632

## Subject:

Carrier Designation:

## Crown Castle Designation:

## Structural Analysis Report

Sprint PCS Co-Locate
Carrier Site Number: Carrier Site Name:

Crown Castle BU Number: 876328
Crown Castle Site Name:
Crown Castle JDE Job Number: 251934
Crown Castle Work Order Number: 696196
Crown Castle Application Number: 205590 Rev. 2
GPD Group Project Number: 2014777.876328.01
27-31 South Main St., West Hartford, Hartford County, CT 06110 Latitude $41^{\circ} 45^{\prime} 36.41$ ", Longitude -72 ${ }^{\circ} 44^{\prime} 35.25 "$
40.25 Foot - Rohn Self Support Tower

Dear Ms. Cheryl Schultz,
GPD Group is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 607348, in accordance with application 205590, revision 2.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

```
LC5: Existing + Proposed Equipment
Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.
```

Sufficient Capacity

The analysis has been performed in accordance with the TIA/EIA-222-F standard and the 2005 Connecticut State Building Code based upon a wind speed of 80 mph fastest mile.

We at GPD Group appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.
Respectfully submitted by:


John N. Kabak, P.E.
Connecticut \#: PEN. 0028336

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Table 3 - Design Antenna and Cable Information

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Table 4 - Documents Provided
3.1) Analysis Method
3.2) Assumptions

## 4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)
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4.1) Recommendations
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tnxTower Output

## 6) APPENDIX B

Base Level Drawing
7) APPENDIX C

Additional Calculations

## 1) INTRODUCTION

The tower is supported on three legs and has eight two sections. It has a triangular cross section made of bolted connections, with an " $X$ " frame configuration. The tower is fabricated with pipe round legs, angle diagonals. The tower is galvanized and has no tower lightning.

This tower is a 40 ft Self Support tower designed by ROHN in April of 1997. The tower was originally designed for a wind speed of 85 mph per TIA/EIA-222-E.

## 2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 80 mph with no ice, 38 mph with 1 inch ice thickness (in accordance with ASCE 7 Ice conditions) and 50 mph under service loads.

Table 1 - Proposed Antenna and Cable Information

| Mounting <br> Level (ft) | Center <br> Line <br> Elevation <br> (ft) | Number <br> of <br> Antennas | Antenna <br> Manufacturer | Antenna Model | Number <br> of Feed <br> Lines | Feed <br> Line (in) | Note |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 102.0 | 103.0 | 3 | Alcatel lucent | TD-RRH8x20-25 | 1 | $5 / 8$ | 1 |
|  | 3 | RFS Celwave | APXVTM14-C-120 | 1 |  | 1 |  |

Notes:

1) See Appendix B for the proposed coax layout.

Table 2 - Existing and Reserved Antenna and Cable Information

| Mounting Level (ft) | Center Line Elevation (ft) | $\begin{array}{\|l} \text { Number } \\ \text { of } \\ \text { Antennas } \end{array}$ | Antenna Manufacturer | Antenna Model | Number of Feed Lines | Feed Line Size (in) | Note |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 102.0 | 103.0 | 1 | RFS Celwave | APXV9ERR18-C-A20 | 3 | 1-1/4 |  |
|  |  | 2 | RFS Celwave | APXVSPP18-C-A20 |  |  |  |
|  |  | 1 | Alcatel Lucent | 800MHz 2X50W RRH W/FILTER |  |  |  |
|  |  | 2 | Alcatel Lucent | 1900MHz RRH (65MHz) |  |  |  |
|  | 102.0 | 1 | Alcatel Lucent | $1900 \mathrm{MHz} \mathrm{RRH}(65 \mathrm{MHz})$ |  |  |  |
|  |  | 2 | Alcatel Lucent | 800MHz 2X50W RRH W/FILTER |  |  |  |
|  |  | 1 |  | Sector Mount [SM 502-3] |  |  |  |
| 92.0 | 92.0 | 1 |  | T-Arm Mount [TA 702-3] | $\begin{aligned} & 6 \\ & 2 \end{aligned}$ | $\begin{gathered} 1-5 / 8 \\ 3 / 4 \end{gathered}$ |  |
|  |  | 3 | Ericsson | RRUS-11 |  |  |  |
|  |  | 12 | Powerwave Technologies | LGP2140X |  |  |  |
|  | 89.0 | 3 | Powerwave Technologies | 7770.00 |  |  |  |
|  |  | 1 | Andrew | SBNH-1D6565C |  |  |  |
|  |  | 1 | Powerwave Technologies | P65-15-XLH-RR |  |  |  |
|  |  | 1 | Powerwave Technologies | P65E-17-XLH-RR |  |  |  |
| 75.0 | 77.0 | 1 | Lucent | KS24019-L112A | 1 | 1/2 |  |
|  | 75.0 | 1 |  | Side Arm Mount [SO 302-1] |  |  |  |

Table 3 - Design Antenna and Cable Information

| Mounting <br> Level (ft) | Center <br> Line <br> Elevation <br> (ft) | Number <br> of <br> Antennas | Antenna <br> Manufacturer | Antenna Model | Number <br> of Feed <br> Lines | Feed <br> Line <br> Size (in) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 105 | 105 | 12 |  | DB980H90 | 12 | $1-5 / 8$ |
| 75 | 3 |  | Leg Mounting Frame | 12 | $1-5 / 8$ |  |

## 3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

| Document | Remarks | Reference | Source |
| :---: | :---: | :---: | :---: |
| Tower Manufacturer Drawings | Rohn Eng. File\#: $345895 W$, <br> dated: $4 / 15 / 1997$ | Doc ID\#: 1440544 | Crown DMZ |
| Tower Mapping Report | GPD Project \#: 2014777.876328 .03, <br> dated: $03 / 04 / 2014$ | D. Palkovic | GPD |

## 3.1) Analysis Method

tnxTower (version 6.1.3.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

## 3.2) Assumptions

1) Tower and structures were built in accordance with the manufacturer's specifications.
2) The tower and structures have been maintained in accordance with the manufacturer's specification.
3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.
5) Mount sizes, weights, and manufacturers are best estimates based on site photos provided and were determined without the benefit of a site visit by GPD.
6) All member connections and foundation steel reinforcing are assumed designed to meet or exceed the load carrying capacity of the connected member and surrounding soils respectively unless otherwise specified in this report.
7) The capacity of the rooftop was not evaluated in this analysis.
8) All equipment model numbers, quantities, and centerline elevations are as provided in the CCl CAD package dated 01/02/2014 with any adjustments as noted below.

This analysis may be affected if any assumptions are not valid or have been made in error. GPD Group should be notified to determine the effect on the structural integrity of the tower.

## 4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

| Section No. | Elevation (ft) | Component Type | Size | Critical Element | P (K) | SF* ${ }_{\text {allow }}(\mathrm{K})$ | \% Capacity | Pass / Fail |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 105.25-85.125 | Leg | ROHN 2.5 STD | 2 | -13.64 | 54.96 | 24.8 | Pass |
| T2 | 85.125-65 | Leg | ROHN 2.5 STD | 38 | -35.90 | 50.20 | 71.5 | Pass |
| T1 | 105.25-85.125 | Diagonal | L1 1/2x1 1/2x1/8 | 8 | -2.65 | 3.35 | $\begin{gathered} 79.0 \\ 94.3 \text { (b) } \end{gathered}$ | Pass |
| T2 | 85.125-65 | Diagonal | L1 3/4x1 3/4x3/16 | 46 | -2.76 | 4.49 | 61.6 | Pass |
| T1 | 105.25-85.125 | Top Girt | L2x2x1/8 | 5 | -0.30 | 2.83 | 10.7 | Pass |
| T2 | 85.125-65 | Top Girt | L2x2x1/8 | 41 | -0.12 | 3.60 | 3.4 | Pass |
|  |  |  |  |  |  |  | Summary |  |
|  |  |  |  |  |  | Leg (T2) | 71.5 | Pass |
|  |  |  |  |  |  | Diagonal (T1) | 94.3 | Pass |
|  |  |  |  |  |  | Top Girt (T1) | 10.7 | Pass |
|  |  |  |  |  |  | Bolt Checks | 94.3 | Pass |
|  |  |  |  |  |  | Rating = | 94.3 | Pass |

Table 6 - Tower Component Stresses vs. Capacity - LC5

| Notes | Component | Elevation (ft) | \% Capacity | Pass / Fail |
| :---: | :---: | :---: | :---: | :---: |
| - | Anchor Bolts | 65 | 54.3 | Pass |
| 1 | Base I-Beam Frame | 65 | 67.3 | Pass |


| Structure Rating (max from all components) $=$ | $94.3 \%$ |
| :--- | :---: |
| Notes: |  |

1) See additional documentation in "Appendix C - Additional Calculations" for calculations supporting the \% capacity consumed.

## 4.1) Recommendations

The design of the tower and its foundations are sufficient for the proposed loading and does not require modifications.

## 5) DISCLAIMER OF WARRANTIES

GPD GROUP has not performed a site visit to the tower to verify the member sizes or antenna/coax loading. If the existing conditions are not as represented on the tower elevation contained in this report, we should be contacted immediately to evaluate the significance of the discrepancy. This is not a condition assessment of the tower or foundation. This report does not replace a full tower inspection. The tower and foundations are assumed to have been properly fabricated, erected, maintained, in good condition, twist free, and plumb.

The engineering services rendered by GPD GROUP in connection with this Structural Analysis are limited to a computer analysis of the tower structure and theoretical capacity of its main structural members. All tower components have been assumed to only resist dead loads when no other loads are applied. No allowance was made for any damaged, bent, missing, loose, or rusted members (above and below ground). No allowance was made for loose bolts or cracked welds.

GPD GROUP does not analyze the fabrication of the structure (including welding). It is not possible to have all the very detailed information needed to perform a thorough analysis of every structural sub-component and connection of an existing tower. GPD GROUP provides a limited scope of service in that we cannot verify the adequacy of every weld, plate connection detail, etc. The purpose of this report is to assess the feasibility of adding appurtenances usually accompanied by transmission lines to the structure.

It is the owner's responsibility to determine the amount of ice accumulation in excess of the code specified amount, if any, that should be considered in the structural analysis.

The attached sketches are a schematic representation of the analyzed tower. If any material is fabricated from these sketches, the contractor shall be responsible for field verifying the existing conditions, proper fit, and clearance in the field. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from GPD GROUP, but are beyond the scope of this report.

Miscellaneous items such as antenna mounts, etc., have not been designed or detailed as a part of our work. We recommend that material of adequate size and strength be purchased from a reputable tower manufacturer.

Towers are designed to carry gravity, wind, and ice loads. All members, legs, diagonals, struts, and redundant members provide structural stability to the tower with little redundancy. Absence or removal of a member can trigger catastrophic failure unless a substitute is provided before any removal. Legs carry axial loads and derive their strength from shorter unbraced lengths by the presence of redundant members and their connection to the diagonals with bolts or welds. If the bolts or welds are removed without providing any substitute to the frame, the leg is subjected to a higher unbraced length that immediately reduces its load carrying capacity. If a diagonal is also removed in addition to the connection, the unbraced length of the leg is greatly increased, jeopardizing its load carrying capacity. Failure of one leg can result in a tower collapse because there is no redundancy. Redundant members and diagonals are critical to the stability of the tower.

GPD GROUP makes no warranties, expressed and/or implied, in connection with this report and disclaims any liability arising from material, fabrication, and erection of this tower. GPD GROUP will not be responsible whatsoever for, or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of GPD GROUP pursuant to this report will be limited to the total fee received for preparation of this report.

## APPENDIX A

TNXTOWER OUTPUT


Feed Line Distribution Chart
65' - 105'3'
Round $\quad$ Flat $\qquad$ App In Face _App Out Face $\qquad$ Truss Leg


## Tower Input Data

The main tower is a 3 x free standing tower with an overall height of 105.25 ft above the ground line.
The base of the tower is set at an elevation of 65.00 ft above the ground line.
The face width of the tower is 6.56 ft at the top and 8.56 ft at the base.
This tower is designed using the TIA/EIA-222-F standard.
The following design criteria apply:
Tower is located in Hartford County, Connecticut.
Basic wind speed of 80 mph .
Nominal ice thickness of 1.0000 in.
Ice thickness is considered to increase with height.
Ice density of 56 pcf.
A wind speed of 38 mph is used in combination with ice.
Temperature drop of $50^{\circ} \mathrm{F}$.
Deflections calculated using a wind speed of 50 mph .
Pressures are calculated at each section.
Stress ratio used in tower member design is 1.333 .
Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

[^0]Distribute Leg Loads As Uniform
Assume Legs Pinned
$\sqrt{ }$ Assume Rigid Index Plate
$\sqrt{ }$ Use Clear Spans For Wind Area
$\sqrt{ }$ Use Clear Spans For KL/r Retension Guys To Initial Tension
$\sqrt{ }$ Bypass Mast Stability Checks
$\sqrt{ }$ Use Azimuth Dish Coefficients
$\sqrt{ }$ Project Wind Area of Appurt. Autocalc Torque Arm Areas
$\sqrt{ }$ SR Members Have Cut Ends
$\sqrt{ }$ Sort Capacity Reports By Component
Triangulate Diamond Inner Bracing
Use TIA-222-G Tension Splice Capacity
Exemption

Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules
$\sqrt{ }$ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression
$\sqrt{ }$ All Leg Panels Have Same Allowable
$\sqrt{ }$ Offset Girt At Foundation
$\checkmark$ Consider Feedline Torque
$\sqrt{ }$ Include Angle Block Shear Check

Include Shear-Torsion Interaction
Always Use Sub-Critical Flow
Use Top Mounted Sockets


Triangular Tower

Tower Section Geometry

| Tower <br> Section | Tower | Assembly | Description | Section <br> Database | Number <br> of |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Section <br> Length |  |
|  | $f t$ |  | $f t$ | Sections |  |

## Tower Section Geometry (cont'd)

| Tower | Tower | Diagonal | Bracing | Has | Has | Top Girt | Bottom Girt |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section | Elevation | Spacing |  | Type | K Brace | Horizontals | Offset |

## Tower Section Geometry (cont'd)

| Tower Elevation $f t$ | $\begin{gathered} \text { Leg } \\ \text { Type } \end{gathered}$ | Leg Size | Leg Grade | Diagonal Type | Diagonal Size | Diagonal Grade |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 105.25-85.13 | Pipe | ROHN 2.5 STD | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Equal Angle | L1 1/2x1 1/2x1/8 | $\begin{gathered} \text { A36 } \\ (36 \mathrm{ksi}) \end{gathered}$ |
| T2 85.13-65.00 | Pipe | ROHN 2.5 STD | $\begin{gathered} \text { A572-50 } \\ (50 \mathrm{ksi}) \end{gathered}$ | Equal Angle | L1 3/4x1 3/4x3/16 | $\begin{gathered} \mathrm{A} 36 \\ (36 \mathrm{ksi}) \end{gathered}$ |

Tower Section Geometry (cont'd)

| Tower | Top Girt | Top Girt | Top Girt | Bottom Girt | Bottom Girt |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Elevation | Type | Size | Grade | Type | Size |

Tower Section Geometry (cont'd)

| Tower Elevation <br> ft | Gusset <br> Area (perface) $f t^{2}$ | Gusset Thickness in | Gusset Grade | Adjust. Factor $A_{f}$ | Adjust. <br> Factor <br> $A_{r}$ | Weight Mult. | Double Angle <br> Stitch Bolt <br> Spacing <br> Diagonals <br> in | Double Angle <br> Stitch Bolt Spacing Horizontals in |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 105.25-85.13 | 0.00 | 0.0000 | A36 | 1 | 1 | 1 | 36.0000 | 36.0000 |
| T2 85.13-65.00 | 0.00 | 0.0000 | (36 ksi) A36 <br> (36 ksi) | 1 | 1 | 1 | 36.0000 | 36.0000 |

## Tower Section Geometry (cont'd)

| Tower <br> Elevation | Calc <br> K <br> Single <br> Angles | Calc K Solid Rounds | K Factors ${ }^{\text {I }}$ |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Legs | $X$ | $\begin{gathered} K \\ \text { Brace } \end{gathered}$ | Single <br> Diags | Girts | Horiz. | Sec. <br> Horiz. | Inner <br> Brace |
|  |  |  |  | Brace |  |  |  |  |  |  |
|  |  |  |  | Diags | Diags |  |  |  |  |  |
|  |  |  |  | X | X | $X$ | $X$ | $X$ | $X$ | $X$ |
| $f t$ |  |  |  | $Y$ | $Y$ | $Y$ | $Y$ | $Y$ | $Y$ | $Y$ |
| T1 105.25-85.13 | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
|  |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
| T2 85.13-65.00 | Yes | No | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 |
|  |  |  |  | 1 | 1 | 1 | 1 | 1 | 1 | 1 |

${ }^{I}$ Note: $K$ factors are applied to member segment lengths. $K$-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

## Tower Section Geometry (cont'd)

| Tower Elevation $f t$ | Leg |  | Diagonal |  | Top Girt |  | Bottom Girt |  | Mid Girt |  | Long Horizontal |  | Short Horizontal |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Net Width Deduct in | $U$ | Net Width Deduct in |  | Net Width Deduct in | $U$ | $\begin{gathered} \text { Net Width } \\ \text { Deduct } \\ \text { in } \end{gathered}$ | $U$ | Net Width Deduct in | $U$ | Net Width Deduct in | $U$ | Net Width Deduct in | $U$ |
| T1 105.25-85.13 | 0.0000 | 1 | 0.0000 | 0.75 | 0.0000 | 0.75 | 0.0000 | 0.75 | 0.0000 | 0.75 | 0.0000 | 0.75 | 0.0000 | 0.75 |
| T2 85.13-65.00 | 0.0000 | 1 | 0.0000 | 0.75 | 0.0000 | 0.75 | 0.0000 | 0.75 | 0.0000 | 0.75 | 0.0000 | 0.75 | 0.0000 | 0.75 |

## Tower Section Geometry (cont'd)

| Tower Elevation $f t$ | Leg <br> Connection Type | Leg |  | Diagonal |  | Top Girt |  | Bottom Girt |  | Mid Girt |  | Long Horizontal |  | Short Horizontal |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Bolt Size in | No. | Bolt Size in |  | Bolt Size in | No. | Bolt Size in | No. | Bolt Size in | No. | $\begin{gathered} \text { Bolt Size } \\ \text { in } \end{gathered}$ | No. | Bolt Size in | No. |
| T1 105.25-85.13 | Flange | 0.6250 | 4 | 0.5000 | 1 | 0.5000 | 1 | 0.6250 | 0 | 0.6250 | 0 | 0.6250 | 0 | 0.6250 | 0 |
|  |  | A325N |  | A325N |  | A325N |  | A325N |  | A325N |  | A325N |  | A325N |  |
| T2 85.13-65.00 | Flange | 0.6250 | 4 | 0.5000 | 1 | 0.6250 | 0 | 0.6250 | 0 | 0.6250 | 0 | 0.6250 | 0 | 0.6250 | 0 |
|  |  | A325N |  | A325N |  | A325N |  | A325N |  | A325N |  | A325N |  | A325N |  |

## Feed Line/Linear Appurtenances - Entered As Round Or Flat

| Description | Face <br> or | Allow <br> Shield | Component <br> Type | Placement | Face <br> Offset | Lateral <br> Offset <br> (Frac FW) | \# | \# <br> Per <br> Row | Clear <br> Spacing <br> in | Width or <br> Diameter <br> in | Perimeter | Weight |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| in |  |  |  |  |  |  |  |  |  |  |  |  |

## Feed Line/Linear Appurtenances Section Areas

| Tower <br> Section | Tower <br> Elevation <br> $f t$ | Face | $A_{R}$ | $A_{F}$ | $C_{A} A_{A}$ <br> In Face | $C_{A} A_{A}$ <br> Out Face | Weight |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t^{2}$ | $f t^{2}$ | ${f t^{2}}^{f t^{2}}$ |  |  |
| T 1 | $105.25-85.13$ | A | 0.000 | 0.000 | 0.000 | 0.000 | 0.00 |
|  |  | B | 15.991 | 11.329 | 0.000 | 0.000 | 0.37 |
|  |  | C | 0.621 | 6.360 | 0.000 | 0.000 | 0.00 |
| T 2 | $85.13-65.00$ | A | 0.000 | 0.000 | 0.000 | 0.000 | 0.00 |
|  |  | B | 30.766 | 16.471 | 0.000 | 0.000 | 0.63 |
|  |  | C | 0.629 | 6.440 | 0.000 | 0.000 | 0.00 |

## Feed Line/Linear Appurtenances Section Areas - With Ice

| Tower <br> Section | Tower <br> Elevation <br> $f t$ | Face <br> or | Ice <br> Thickness | $A_{R}$ | $A_{F}$ | $C_{A} A_{A}$ <br> In Face $^{2}$ | $C_{A} A_{A}$ <br> Out Face | Weight |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ft | Leg | in | $f t^{2}$ | $f t^{2}$ | $f t^{2}$ | ${f t^{2}}^{2}$ | K |
| T1 | $105.25-85.13$ | A | 1.136 | 0.000 | 0.000 | 0.000 | 0.000 | 0.00 |
|  |  | B |  | 21.933 | 30.892 | 0.000 | 0.000 | 1.13 |
|  |  | C |  | 4.383 | 8.868 | 0.000 | 0.000 | 0.00 |
| T 2 | $85.13-65.00$ | A | 1.104 | 0.000 | 0.000 | 0.000 | 0.000 | 0.00 |
|  |  | B |  | 31.208 | 55.271 | 0.000 | 0.000 | 1.85 |
|  |  | C |  | 4.331 | 8.908 | 0.000 | 0.000 | 0.00 |

## Feed Line Shielding

| Section | Elevation | Face | $A_{R}$ | $A_{R}$ | $A_{F}$ | $A_{F}$ <br> Ice |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |
|  | $f t$ |  | $f t^{2}$ | $f t^{2}$ | $f t^{2}$ | $f t^{2}$ |
|  | $105.25-85.13$ | A | 0.000 | 0.000 | 0.000 | 0.000 |
|  |  | B | 0.000 | 4.888 | 1.650 | 3.313 |
|  |  | C | 0.000 | 0.000 | 0.000 | 0.000 |
|  |  |  | A | 0.000 | 0.000 | 0.000 |
|  |  | B | 0.000 | 7.347 | 3.133 | 5.000 |
|  |  | C | 0.000 | 0.000 | 0.000 | 0.004 |
|  |  |  |  |  |  |  |

## Feed Line Center of Pressure

| Section | Elevation | $C P_{X}$ | $C P_{Z}$ | $C P_{X}$ | $C P_{Z}$ <br> Ice |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Ine | in | in |

Discrete Tower Loads

| Description | Face <br> or Leg | $\begin{aligned} & \text { Offset } \\ & \text { Type } \end{aligned}$ | Offsets: <br> Horz <br> Lateral Vert $f t$ ft ft | Azimuth Adjustment <br> 0 | Placement |  | $C_{A} A_{A}$ <br> Front <br> $f t^{2}$ | $C_{A} A_{A}$ Side | Weight |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Sector Mount [SM 502-3] | C | None |  | 0.0000 | 102.00 | No Ice | 33.02 | 33.02 | 1.67 |
|  |  |  |  |  |  | 1/2" Ice | 47.36 | 47.36 | 2.22 |
|  |  |  |  |  |  | $1{ }^{\prime \prime}$ Ice | 61.70 | 61.70 | 2.77 |
|  |  |  |  |  |  | 2" Ice | 90.38 | 90.38 | 3.88 |
|  |  |  |  |  |  | 4 " Ice | 147.74 | 147.74 | 6.08 |
| APXVSPP18-C-A20 w/ Mount Pipe | A | From Leg | 4.00 | 0.0000 | 102.00 | No Ice | 8.26 | 6.71 | 0.08 |
|  |  |  | 0.00 |  |  | 1/2" Ice | 8.81 | 7.66 | 0.14 |
|  |  |  | 1.00 |  |  | $1{ }^{\prime \prime}$ Ice | 9.36 | 8.49 | 0.22 |
|  |  |  |  |  |  | 2" Ice | 10.50 | 10.20 | 0.39 |
|  |  |  |  |  |  | 4" Ice | 12.88 | 13.98 | 0.87 |
| APXVSPP18-C-A20 w/ Mount Pipe | B | From Leg |  | 0.0000 | 102.00 | No Ice | 8.26 | 6.71 | 0.08 |
|  |  |  | $0.00$ |  |  | 1/2" Ice | 8.81 | 7.66 | 0.14 |
|  |  |  | 1.00 |  |  | $1{ }^{\prime \prime}$ Ice | 9.36 | 8.49 | 0.22 |
|  |  |  |  |  |  | 2 " Ice | 10.50 | 10.20 | 0.39 |
|  |  |  |  |  |  | $4{ }^{\prime \prime}$ Ice | 12.88 | 13.98 | 0.87 |
| APXV9ERR18-C-A20 w/ Mount Pipe | C | From Leg |  | 0.0000 | 102.00 | No Ice | 8.73 | 7.18 | 0.08 |
|  |  |  | $0.00$ |  |  | $1 / 2^{\prime \prime} \text { Ice }$ | 9.49 | 8.46 | 0.15 |
|  |  |  | 1.00 |  |  | $1^{\prime \prime}$ Ice | 10.21 | 9.60 | 0.23 |
|  |  |  |  |  |  | 2" Ice | 11.60 | 11.53 | 0.41 |
|  |  |  |  |  |  | 4 " Ice | 14.51 | 15.77 | 0.94 |
| $1900 \mathrm{MHz} \mathrm{RRH}(65 \mathrm{MHz})$ | A | From Leg | 4.00 | 0.0000 | 102.00 | No Ice | 2.70 | 2.77 | 0.06 |
|  |  |  | 0.00 |  |  | 1/2" Ice | 2.94 | 3.01 | 0.08 |
|  |  |  | 1.00 |  |  | $1^{\prime \prime}$ Ice | 3.18 | 3.26 | 0.11 |
|  |  |  |  |  |  | 2 " Ice | 3.70 | 3.78 | 0.18 |
|  |  |  |  |  |  | 4 " Ice | 4.85 | 4.93 | 0.35 |
| $1900 \mathrm{MHz} \mathrm{RRH}(65 \mathrm{MHz})$ | B | From Leg |  | 0.0000 | 102.00 | No Ice | 2.70 | 2.77 | 0.06 |
|  |  |  | 0.00 |  |  | 1/2" Ice | 2.94 | 3.01 | 0.08 |
|  |  |  | 1.00 |  |  | $1{ }^{\prime \prime}$ Ice | 3.18 | 3.26 | 0.11 |
|  |  |  |  |  |  | 2 " Ice | 3.70 | 3.78 | 0.18 |
|  |  |  |  |  |  | $4{ }^{\prime \prime}$ Ice | 4.85 | 4.93 | 0.35 |
| 1900 MHz RRH ( 65 MHz ) | C | From Leg |  | 0.0000 | 102.00 | No Ice | 2.70 | 2.77 | 0.06 |
|  |  |  | 0.00 |  |  | 1/2" Ice | 2.94 | 3.01 | 0.08 |
|  |  |  | 0.00 |  |  | 1" Ice | 3.18 | 3.26 | 0.11 |
|  |  |  |  |  |  | 2" Ice | 3.70 | 3.78 | 0.18 |
|  |  |  |  |  |  | $4{ }^{\prime \prime}$ Ice | 4.85 | 4.93 | 0.35 |
| 800MHz 2X50W RRH W/FILTER | A | From Leg |  | 0.0000 | 102.00 | No Ice | 2.40 | 2.25 | 0.06 |
|  |  |  | 0.00 |  |  | 1/2" Ice | 2.61 | 2.46 | 0.09 |
|  |  |  | 0.00 |  |  | 1 " Ice | 2.83 | 2.68 | 0.11 |
|  |  |  |  |  |  | 2" Ice | 3.30 | 3.13 | 0.17 |
|  |  |  |  |  |  | 4 " Ice | 4.34 | 4.15 | 0.34 |
| 800MHz 2X50W RRH W/FILTER | B | From Leg | 4.00 | 0.0000 | 102.00 | No Ice | 2.40 | 2.25 | 0.06 |
|  |  |  | 0.00 |  |  | 1/2" Ice | 2.61 | 2.46 | 0.09 |
|  |  |  | 0.00 |  |  | 1 " Ice | 2.83 | 2.68 | 0.11 |
|  |  |  |  |  |  | 2 " Ice | 3.30 | 3.13 | 0.17 |
|  |  |  |  |  |  | 4 " Ice | 4.34 | 4.15 | 0.34 |
| 800MHz 2X50W RRH W/FILTER | C | From Leg |  | 0.0000 | 102.00 | No Ice | 2.40 | 2.25 | 0.06 |
|  |  |  | 0.00 |  |  | 1/2" Ice | 2.61 | 2.46 | 0.09 |
|  |  |  | 1.00 |  |  | 1 " Ice | 2.83 | 2.68 | 0.11 |
|  |  |  |  |  |  | 2 " Ice | 3.30 | 3.13 | 0.17 |
|  |  |  |  |  |  | 4" Ice | 4.34 | 4.15 | 0.34 |
| APXVTM14-C-120 w/ Mount Pipe | A | From Leg | 4.00 | 0.0000 | 102.00 | No Ice | 7.13 | 4.96 | 0.08 |
|  |  |  | 0.00 |  |  | 1/2" Ice | 7.66 | 5.75 | 0.13 |
|  |  |  | 1.00 |  |  | 1 " Ice | 8.18 | 6.47 | 0.19 |
|  |  |  |  |  |  | 2 " Ice | 9.26 | 8.01 | 0.34 |
|  |  |  |  |  |  | 4 " Ice | 11.53 | 11.41 | 0.75 |
| APXVTM14-C-120 w/ Mount Pipe | B | From Leg | 4.00 | 0.0000 | 102.00 | No Ice | 7.13 | 4.96 | 0.08 |
|  |  |  | 0.00 |  |  | 1/2" Ice | 7.66 | 5.75 | 0.13 |
|  |  |  | 1.00 |  |  | 1" Ice | 8.18 | 6.47 | 0.19 |
|  |  |  |  |  |  | 2 " Ice | 9.26 | 8.01 | 0.34 |
|  |  |  |  |  |  | 4" Ice | 11.53 | 11.41 | 0.75 |
| APXVTM14-C-120 w/ Mount Pipe | C | From Leg | 4.00 | 0.0000 | 102.00 | No Ice | 7.13 | 4.96 | 0.08 |

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& \[
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
\] \& \begin{tabular}{l}
Offset \\
Type
\end{tabular} \& \begin{tabular}{l}
Offsets: \\
Horz \\
Lateral Vert \(f t\) \(f t\) ft
\end{tabular} \& \begin{tabular}{l}
Azimuth Adjustment \\
0
\end{tabular} \& Placement

$f t$ \& \& $C_{A} A_{A}$
Front

$f t^{2}$ \& $C_{A} A_{A}$
Side

$f t^{2}$ \& Weight <br>
\hline \multirow{9}{*}{TD-RRH8x20-25} \& \multirow{7}{*}{A} \& \multirow{7}{*}{From Leg} \& 0.00 \& \multirow{7}{*}{0.0000} \& \multirow{7}{*}{102.00} \& 1/2" Ice \& 7.66 \& 5.75 \& 0.13 <br>
\hline \& \& \& 1.00 \& \& \& $1{ }^{\prime \prime}$ Ice \& 8.18 \& 6.47 \& 0.19 <br>
\hline \& \& \& \& \& \& 2" Ice \& 9.26 \& 8.01 \& 0.34 <br>
\hline \& \& \& \& \& \& 4" Ice \& 11.53 \& 11.41 \& 0.75 <br>
\hline \& \& \& 4.00 \& \& \& No Ice \& 4.72 \& 1.70 \& 0.07 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 5.01 \& 1.92 \& 0.10 <br>
\hline \& \& \& 1.00 \& \& \& $1{ }^{\prime \prime}$ Ice \& 5.32 \& 2.15 \& 0.13 <br>
\hline \& \multirow{5}{*}{B} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{102.00} \& 2" Ice \& 5.95 \& 2.62 \& 0.20 <br>
\hline \& \& \& \& \& \& 4" Ice \& 7.31 \& 3.68 \& 0.40 <br>
\hline \multirow[t]{5}{*}{TD-RRH8x20-25} \& \& \& 4.00 \& \& \& No Ice \& 4.72 \& 1.70 \& 0.07 <br>

\hline \& \& \& 0.00 \& \& \& $$
1 / 2^{\prime \prime} \text { Ice }
$$ \& 5.01 \& 1.92 \& 0.10 <br>

\hline \& \& \& 1.00 \& \& \& 1" Ice \& 5.32 \& 2.15 \& 0.13 <br>
\hline \& \multirow{5}{*}{C} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{102.00} \& 2" Ice \& 5.95 \& 2.62 \& 0.20 <br>
\hline \& \& \& \& \& \& 4" Ice \& 7.31 \& 3.68 \& 0.40 <br>
\hline \multirow[t]{5}{*}{TD-RRH8x20-25} \& \& \& 4.00 \& \& \& No Ice \& 4.72 \& 1.70 \& 0.07 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 5.01 \& 1.92 \& 0.10 <br>
\hline \& \& \& 1.00 \& \& \& 1" Ice \& 5.32 \& 2.15 \& 0.13 <br>
\hline \& \multirow{5}{*}{A} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{102.00} \& 2 " Ice \& 5.95 \& 2.62 \& 0.20 <br>
\hline \& \& \& \& \& \& 4" Ice \& 7.31 \& 3.68 \& 0.40 <br>
\hline \multirow[t]{5}{*}{7'x2 1/2" Pipe Mount} \& \& \& \& \& \& No Ice \& 2.01 \& 2.01 \& 0.04 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 2.59 \& 2.59 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& $1{ }^{\prime \prime}$ Ice \& 3.02 \& 3.02 \& 0.07 <br>
\hline \& \multirow{5}{*}{B} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{102.00} \& 2 " Ice \& 3.90 \& 3.90 \& 0.13 <br>
\hline \& \& \& \& \& \& 4" Ice \& 5.78 \& 5.78 \& 0.30 <br>
\hline \multirow[t]{5}{*}{7'x2 1/2" Pipe Mount} \& \& \& \& \& \& No Ice \& 2.01 \& 2.01 \& 0.04 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 2.59 \& 2.59 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& $1{ }^{\prime \prime}$ Ice \& 3.02 \& 3.02 \& 0.07 <br>
\hline \& \multirow{5}{*}{C} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{102.00} \& 2 " Ice \& 3.90 \& 3.90 \& 0.13 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 5.78 \& 5.78 \& 0.30 <br>
\hline \multirow[t]{5}{*}{7'x2 1/2" Pipe Mount} \& \& \& \& \& \& No Ice \& 2.01 \& 2.01 \& 0.04 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 2.59 \& 2.59 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& 1 " Ice \& 3.02 \& 3.02 \& 0.07 <br>
\hline \& \multirow{5}{*}{C} \& \multirow{5}{*}{None} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{92.00} \& 2" Ice \& 3.90 \& 3.90 \& 0.13 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 5.78 \& 5.78 \& 0.30 <br>
\hline \multirow[t]{5}{*}{T-Arm Mount [TA 702-3]} \& \& \& \& \& \& No Ice \& 5.64 \& 5.64 \& 0.34 <br>
\hline \& \& \& \& \& \& 1/2" Ice \& 6.55 \& 6.55 \& 0.43 <br>
\hline \& \& \& \& \& \& $1^{\prime \prime}$ Ice \& 7.46 \& 7.46 \& 0.52 <br>
\hline \& \multirow{6}{*}{A} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{92.00} \& 2 " Ice \& 9.28 \& 9.28 \& 0.70 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 12.92 \& 12.92 \& 1.06 <br>
\hline \multirow[t]{5}{*}{7770.00 w/ Mount Pipe} \& \& \& \& \& \& No Ice \& 6.22 \& 4.35 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 6.77 \& 5.20 \& 0.11 <br>
\hline \& \& \& -3.00 \& \& \& 1" Ice \& 7.30 \& 5.92 \& 0.16 <br>
\hline \& \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{92.00} \& 2 " Ice \& 8.38 \& 7.41 \& 0.29 <br>
\hline \& \multirow{4}{*}{B} \& \& \& \& \& 4 " Ice \& 10.69 \& 10.76 \& 0.68 <br>
\hline \multirow[t]{5}{*}{7770.00 w/ Mount Pipe} \& \& \& \& \& \& No Ice \& 6.22 \& 4.35 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 6.77 \& 5.20 \& 0.11 <br>
\hline \& \& \& -3.00 \& \& \& 1" Ice \& 7.30 \& 5.92 \& 0.16 <br>
\hline \& \multirow{6}{*}{C} \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{92.00} \& 2 " Ice \& 8.38 \& 7.41 \& 0.29 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 10.69 \& 10.76 \& 0.68 <br>
\hline \multirow[t]{5}{*}{7770.00 w/ Mount Pipe} \& \& \& \& \& \& \& 6.22 \& 4.35 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 6.77 \& 5.20 \& 0.11 <br>
\hline \& \& \& -3.00 \& \& \& 1 " Ice \& 7.30 \& 5.92 \& 0.16 <br>
\hline \& \& \multirow{5}{*}{From Leg} \& \& \multirow{5}{*}{0.0000} \& \multirow{5}{*}{92.00} \& 2 " Ice \& 8.38 \& 7.41 \& 0.29 <br>
\hline \& \multirow{4}{*}{A} \& \& \& \& \& 4" Ice \& 10.69 \& 10.76 \& 0.68 <br>
\hline \multirow[t]{5}{*}{P65-15-XLH-RR w/ Mount Pipe} \& \& \& \& \& \& No Ice \& 6.55 \& 4.38 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 7.36 \& 5.51 \& 0.11 <br>
\hline \& \& \& -3.00 \& \& \& $1{ }^{\prime \prime}$ Ice \& 8.10 \& 6.50 \& 0.17 <br>
\hline \& \multirow{7}{*}{B} \& \multirow{7}{*}{From Leg} \& \& \multirow{7}{*}{0.0000} \& \multirow{7}{*}{92.00} \& 2" Ice \& 9.42 \& 8.17 \& 0.31 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 12.24 \& 11.86 \& 0.72 <br>
\hline \multirow[t]{5}{*}{SBNH-1D6565C w/ Mount Pipe} \& \& \& \& \& \& No Ice \& 11.45 \& 9.60 \& 0.09 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 12.06 \& 11.02 \& 0.18 <br>
\hline \& \& \& -3.00 \& \& \& 1" Ice \& 12.69 \& 12.29 \& 0.27 <br>
\hline \& \& \& \& \& \& 2" Ice \& 14.03 \& 14.51 \& 0.50 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 17.05 \& 19.14 \& 1.12 <br>
\hline
\end{tabular}

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& \[
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
\] \& \[
\begin{aligned}
\& \text { Offset } \\
\& \text { Type }
\end{aligned}
\] \& \begin{tabular}{l}
Offsets: \\
Horz \\
Lateral \\
Vert \\
\(f t\) \\
\(f t\) \\
ft
\end{tabular} \& Azimuth Adjustment \& Placement

$f t$ \& \& | $C_{A} A_{A}$ |
| :--- |
| Front |
| $f t^{2}$ | \& $C_{A} A_{A}$

Side

$f t^{2}$ \& Weight <br>
\hline \multirow[t]{5}{*}{P65E-17-XLH-RR w/ Mount Pipe} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& 0.50 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{92.00} \& No Ice \& 11.47 \& 8.70 \& 0.10 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" Ice \& 12.08 \& 10.11 \& 0.18 <br>
\hline \& \& \& -3.00 \& \& \& $1{ }^{\prime \prime}$ Ice \& 12.71 \& 11.38 \& 0.27 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 14.07 \& 13.58 \& 0.49 <br>
\hline \& \& \& \& \& \& 4" Ice \& 17.08 \& 18.18 \& 1.10 <br>
\hline \multirow[t]{5}{*}{(4) LGP2140X} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 0.50 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{92.00} \& No Ice \& 1.26 \& 0.38 \& 0.01 <br>
\hline \& \& \& 0.00 \& \& \& 1/2' Ice \& 1.42 \& 0.49 \& 0.02 <br>
\hline \& \& \& 0.00 \& \& \& $1^{\prime \prime}$ Ice \& 1.58 \& 0.62 \& 0.03 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 1.94 \& 0.89 \& 0.05 <br>
\hline \& \& \& \& \& \& 4" Ice \& 2.75 \& 1.54 \& 0.13 <br>
\hline \multirow[t]{5}{*}{(4) LGP2140X} \& \multirow[t]{5}{*}{B} \& \multirow[t]{5}{*}{From Leg} \& 0.50 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{92.00} \& No Ice \& 1.26 \& 0.38 \& 0.01 <br>
\hline \& \& \& 0.00 \& \& \& 1/2' Ice \& 1.42 \& 0.49 \& 0.02 <br>
\hline \& \& \& 0.00 \& \& \& $1^{\prime \prime}$ Ice \& 1.58 \& 0.62 \& 0.03 <br>
\hline \& \& \& \& \& \& $2{ }^{\prime \prime}$ Ice \& 1.94 \& 0.89 \& 0.05 <br>
\hline \& \& \& \& \& \& 4" Ice \& 2.75 \& 1.54 \& 0.13 <br>
\hline \multirow[t]{5}{*}{(4) LGP2140X} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& 0.50 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{92.00} \& No Ice \& 1.26 \& 0.38 \& 0.01 <br>
\hline \& \& \& 0.00 \& \& \& 1/2' Ice \& 1.42 \& 0.49 \& 0.02 <br>
\hline \& \& \& 0.00 \& \& \& $1^{\prime \prime}$ Ice \& 1.58 \& 0.62 \& 0.03 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 1.94 \& 0.89 \& 0.05 <br>
\hline \& \& \& \& \& \& 4 " Ice \& 2.75 \& 1.54 \& 0.13 <br>
\hline \multirow[t]{5}{*}{RRUS-11} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 0.50 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{92.00} \& No Ice \& 2.94 \& 1.19 \& 0.06 <br>

\hline \& \& \& $$
0.00
$$ \& \& \& 1/2" Ice \& 3.17 \& 1.35 \& 0.07 <br>

\hline \& \& \& \& \& \& $1{ }^{1 \prime}$ Ice \& 3.41 \& 1.52 \& 0.10 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 3.91 \& 1.89 \& 0.15 <br>
\hline \& \& \& \& \& \& 4" Ice \& 5.02 \& 2.72 \& 0.30 <br>
\hline \multirow[t]{5}{*}{RRUS-11} \& \multirow[t]{5}{*}{B} \& \multirow[t]{5}{*}{From Leg} \& 0.50 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{92.00} \& No Ice \& 2.94 \& 1.19 \& 0.06 <br>

\hline \& \& \& $$
0.00
$$ \& \& \& 1/2' Ice \& 3.17 \& 1.35 \& 0.07 <br>

\hline \& \& \& \& \& \& 1" Ice \& 3.41 \& 1.52 \& 0.10 <br>
\hline \& \& \& \& \& \& $2{ }^{\prime \prime}$ Ice \& 3.91 \& 1.89 \& 0.15 <br>
\hline \& \& \& \& \& \& 4" Ice \& 5.02 \& 2.72 \& 0.30 <br>
\hline \multirow[t]{5}{*}{RRUS-11} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{From Leg} \& 0.50 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{92.00} \& No Ice \& 2.94 \& 1.19 \& 0.06 <br>

\hline \& \& \& $$
0.00
$$ \& \& \& 1/2' Ice \& 3.17 \& 1.35 \& 0.07 <br>

\hline \& \& \& \& \& \& 1" Ice \& 3.41 \& 1.52 \& 0.10 <br>
\hline \& \& \& \& \& \& $2{ }^{\prime \prime}$ Ice \& 3.91 \& 1.89 \& 0.15 <br>
\hline \& \& \& \& \& \& 4" Ice \& 5.02 \& 2.72 \& 0.30 <br>
\hline \multirow[t]{5}{*}{Side Arm Mount [SO 302-1]} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 2.00 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{75.00} \& No Ice \& 1.67 \& 3.27 \& 0.06 <br>
\hline \& \& \& 0.00 \& \& \& 1/2' Ice \& 2.51 \& 4.99 \& 0.09 <br>
\hline \& \& \& 0.00 \& \& \& 1" Ice \& 3.35 \& 6.71 \& 0.12 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 5.03 \& 10.15 \& 0.19 <br>
\hline \& \& \& \& \& \& 4" Ice \& 8.39 \& 17.03 \& 0.32 <br>
\hline \multirow[t]{5}{*}{KS24019-L112A} \& \multirow[t]{5}{*}{A} \& \multirow[t]{5}{*}{From Leg} \& 4.00 \& \multirow[t]{5}{*}{0.0000} \& \multirow[t]{5}{*}{75.00} \& No Ice \& 0.16 \& 0.16 \& 0.01 <br>
\hline \& \& \& 0.00 \& \& \& 1/2' Ice \& 0.22 \& 0.22 \& 0.01 <br>
\hline \& \& \& 2.00 \& \& \& $1^{\prime \prime}$ Ice \& 0.30 \& 0.30 \& 0.01 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 0.48 \& 0.48 \& 0.02 <br>
\hline \& \& \& \& \& \& 4" Ice \& 0.95 \& 0.95 \& 0.06 <br>
\hline
\end{tabular}

## Load Combinations

| Comb. | Description |
| :---: | :--- |
| No. |  |
| 1 | Dead Only |
| 2 | Dead+Wind 0 deg - No Ice |
| 3 | Dead+Wind 30 deg - No Ice |
| 4 | Dead+Wind 60 deg - No Ice |
| 5 | Dead+Wind 90 deg - No Ice |
| 6 | Dead+Wind 120 deg - No Ice |
| 7 | Dead+Wind 150 deg - No Ice |
| 8 | Dead+Wind 180 deg - No Ice |
| 9 | Dead+Wind 210 deg - No Ice |
| 10 | Dead+Wind 240 deg - No Ice |
| 11 | Dead+Wind 270 deg - No Ice |
| 12 | Dead+Wind 300 deg - No Ice |
| 13 | Dead+Wind 330 deg - No Ice |
| 14 | Dead+Ice+Temp |
| 15 | Dead+Wind 0 deg+Ice+Temp |
| 16 | Dead+Wind 30 deg+Ice+Temp |
| 17 | Dead+Wind 60 deg+Ice+Temp |
| 18 | Dead+Wind 90 deg+Ice+Temp |
| 19 | Dead+Wind 120 deg+Ice+Temp |
| 20 | Dead+Wind 150 deg+Ice+Temp |
| 21 | Dead+Wind 180 deg+Ice+Temp |
| 22 | Dead+Wind 210 deg+Ice+Temp |
| 23 | Dead+Wind 240 deg+Ice+Temp |
| 24 | Dead+Wind 270 deg+Ice+Temp |
| 25 | Dead+Wind 300 deg+Ice+Temp |
| 26 | Dead+Wind 330 deg+Ice+Temp |
| 27 | Dead+Wind 0 deg - Service |
| 28 | Dead+Wind 30 deg - Service |
| 29 | Dead+Wind 60 deg - Service |
| 30 | Dead+Wind 90 deg - Service |
| 31 | Dead+Wind 120 deg - Service |
| 32 | Dead+Wind 150 deg - Service |
| 33 | Dead+Wind 180 deg - Service |
| 34 | Dead+Wind 210 deg - Service |
| 35 | Dead+Wind 240 deg - Service |
| 36 | Dead+Wind 270 deg - Service |
| 37 | Dead+Wind 300 deg - Service |
| 38 | Dead+Wind 330 deg - Service |
|  |  |

## Maximum Tower Deflections - Service Wind

| Section <br> No. | Elevation | Horz. <br> Deflection | Gov. <br> Load | Tilt | Twist |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | ft | in | Comb. | $\circ$ | $\circ$ |
| T1 | $105.25-85.125$ | 0.416 | 31 | 0.0617 | 0.0090 |
| T2 | $85.125-65$ | 0.139 | 31 | 0.0487 | 0.0060 |
|  |  |  |  |  |  |

## Critical Deflections and Radius of Curvature - Service Wind

| Elevation | Appurtenance | Gov. | Deflection | Tilt | Twist | Radius of <br> Curvature |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $f t$ |  | Load |  | in | $\circ$ | $\circ$ |
| 102.00 |  | Comb. | in |  |  |  |
| 92.00 | Sector Mount [SM 502-3] | 31 | 0.366 | 0.0610 | 0.0086 | 94538 |
| 75.00 | T-Arm Mount [TA 702-3] | 31 | 0.221 | 0.0564 | 0.0073 | 35675 |


|  | Maximum Tower Deflections a Design Wind |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Section | Elevation | Horz. | Gov. | Tilt |
| No. | Deflection | Load | Twist |  |
|  | $f t$ | in | Comb. | $\circ$ |
| T1 | $105.25-85.125$ | 1.054 | 6 | 0.1557 |
| T2 | $85.125-65$ | 0.353 | 6 | 0.1230 |

## Critical Deflections and Radius of Curvature - Design Wind

| Elevation | Appurtenance | Gov. <br> Load | Deflection | Tilt | Twist | Radius of <br> Curvature |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $f t$ |  | Comb. | in | $\circ$ | on |  |
| 102.00 |  | 6 | 0.928 | 0.1539 | 0.0220 | 37458 |
| 92.00 | Sector Mount [SM 502-3] | 6 | 0.560 | 0.1426 | 0.0186 | 14135 |
| 75.00 | T-Arm Mount [TA 702-3] | 6 | 0.143 | 0.0696 | 0.0083 | 18729 |

Bolt Design Data

| Section No. | Elevation <br> ft | Component Type | Bolt Grade | Bolt Size in | Number Of <br> Bolts | Maximum <br> Load per Bolt K | Allowable Load K | Ratio <br> Load <br> Allowable | Allowable Ratio | Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 105.25 | Leg | A325N | 0.6250 | 4 | 2.52 | 13.50 | 0.186 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 0.5000 | 1 | 2.62 | 2.08 | , | 1.333 | Member Block Shear |
|  |  | Top Girt | A325N | 0.5000 | 1 | 0.30 | 2.72 | 0.109 | 1.333 | Member Bearing |
| T2 | 85.125 | Leg | A325N | 0.6250 | 4 | 7.57 | 13.39 | 0.565 | 1.333 | Bolt Tension |
|  |  | Diagonal | A325N | 0.5000 | 1 | 2.75 | 3.81 | 0.724 | 1.333 | Member Block Shear |

## Compression Checks

| Leg Design Data (Compression) |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section No. | Elevation | Size | $L$ | $L_{u}$ | Kl/r | $F_{a}$ | A | Actual P | Allow. $P_{a}$ | $\begin{aligned} \text { Ratio } \\ P \end{aligned}$ |
|  | $f t$ |  | $f t$ | $f t$ |  | ksi | in ${ }^{2}$ | K | ${ }_{K}$ | $P_{a}$ |
| T1 | 105.25-85.125 | ROHN 2.5 STD | 20.13 | 4.02 | $\begin{gathered} 51.0 \\ \mathrm{~K}=1.00 \end{gathered}$ | 24.197 | 1.7040 | -13.64 | 41.23 | $0.331$ |
| T2 | 85.125-65 | ROHN 2.5 STD | 20.16 | 5.02 | $\begin{gathered} 63.6 \\ \mathrm{~K}=1.00 \end{gathered}$ | 22.099 | 1.7040 | -35.90 | 37.66 | ${ }^{0.953}$ |

## Diagonal Design Data (Compression)

| Section No. | Elevation | Size | $L$ | $L_{u}$ | Kl/r | $F_{a}$ | A | $\begin{gathered} \text { Actual } \\ P \end{gathered}$ | Allow. $P_{a}$ | $\begin{aligned} & \text { Ratio } \\ & P \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | ksi | $i n^{2}$ | K | K | $P_{a}$ |
| T1 | 105.25-85.125 | L1 1/2x1 1/2x1/8 | 7.70 | 3.60 | $\begin{gathered} 146.0 \\ \mathrm{~K}=1.00 \end{gathered}$ | 7.002 | 0.3594 | -2.65 | 2.52 | 1.054 |
| T2 | 85.125-65 | L1 3/4x1 3/4x3/16 | 9.70 | 4.75 | $\begin{gathered} 166.0 \\ K=1.00 \end{gathered}$ | 5.418 | 0.6211 | -2.76 | 3.36 | 0.822 |

## Top Girt Design Data (Compression)

| Section No. | Elevation | Size | $L$ | $L_{u}$ | Kl/r | $F_{a}$ | A | Actual $P$ | Allow. $P_{a}$ | $\begin{gathered} \text { Ratio } \\ P \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | ksi | $i n^{2}$ | K | $K$ | $P_{a}$ |
| T1 | 105.25-85.125 | L2x2x1/8 | 6.56 | 6.11 | $\begin{gathered} 184.6 \\ K=1.00 \end{gathered}$ | 4.384 | 0.4844 | -0.30 | 2.12 | $\begin{gathered} 0.143 \\ y \end{gathered}$ |
| T2 | 85.125-65 | L2x $2 \times 1 / 8$ | 6.56 | 6.32 | $\begin{gathered} 163.6 \\ K=0.86 \end{gathered}$ | 5.581 | 0.4844 | -0.12 | 2.70 | $\begin{gathered} 0.046 \\ y \end{gathered}$ |

## Tension Checks

## Leg Design Data (Tension)

| Section No. | Elevation | Size | $L$ | $L_{u}$ | Kl/r | $F_{a}$ | A | Actual <br> $P$ | Allow. <br> $P_{a}$ | $\begin{gathered} \text { Ratio } \\ P \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | ksi | in ${ }^{2}$ | K | K | $P_{a}$ |
| T1 | 105.25-85.125 | ROHN 2.5 STD | 20.13 | 4.02 | 51.0 | 30.000 | 1.7040 | 10.07 | 51.12 | 0.197 |
| T2 | 85.125-65 | ROHN 2.5 STD | 20.16 | 5.02 | 63.6 | 30.000 | 1.7040 | 30.29 | 51.12 | 0.593 |

Diagonal Design Data (Tension)

| Section No. | Elevation <br> ft | Size | $L$ <br> ft | $L_{u}$ <br> ft | Kl/r | $F_{a}$ <br> ksi | A $i n^{2}$ | Actual $P$ $K$ | Allow. $P_{a}$ K | $\begin{gathered} \text { Ratio } \\ P \\ \hline P_{a} \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 105.25-85.125 | L1 1/2x1 1/2x1/8 | 7.70 | 3.60 | 95.7 | 29.000 | 0.2109 | 2.62 | 6.12 | 0.428 |
| T2 | 85.125-65 | L1 3/4x1 3/4x3/16 | 9.70 | 4.75 | 108.5 | 29.000 | 0.3779 | 2.75 | 10.96 | 0.251 |

Top Girt Design Data (Tension)

| Section No. | Elevation | Size | $L$ | $L_{u}$ | Kl/r | $F_{a}$ | A | Actual P | Allow. $P_{a}$ | Ratio $P$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $f t$ |  | $f t$ | $f t$ |  | ksi | $i n^{2}$ | K | K | $P_{a}$ |
| T1 | 105.25-85.125 | L2x2x1/8 | 6.56 | 6.11 | 121.2 | 29.000 | 0.3047 | 0.30 | 8.84 | 0.033 |
| T2 | 85.125-65 | L2x $2 \times 1 / 8$ | 6.56 | 6.32 | 121.2 | 21.600 | 0.4844 | 0.23 | 10.46 | $0.022^{*}$ |

DL controls

## Section Capacity Table

| Section No. | Elevation $f t$ | Component Type | Size | Critical Element | $\begin{aligned} & P \\ & K \end{aligned}$ | $\begin{gathered} S F^{*} P_{\text {allow }} \\ K \end{gathered}$ | \% <br> Capacity | Pass <br> Fail |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| T1 | 105.25-85.125 | Leg | ROHN 2.5 STD | 2 | -13.64 | 54.96 | 24.8 | Pass |
| T2 | 85.125-65 | Leg | ROHN 2.5 STD | 38 | -35.90 | 50.20 | 71.5 | Pass |
| T1 | 105.25-85.125 | Diagonal | L1 1/2x1 1/2x1/8 | 8 | -2.65 | 3.35 | 79.0 | Pass |
|  |  |  |  |  |  |  | 94.3 (b) |  |
| T2 | 85.125-65 | Diagonal | L1 3/4x1 3/4x3/16 | 46 | -2.76 | 4.49 | 61.6 | Pass |
| T1 | 105.25-85.125 | Top Girt | L2x $2 \times 1 / 8$ | 5 | -0.30 | 2.83 | 10.7 | Pass |
| T2 | 85.125-65 | Top Girt | L2x $2 \times 1 / 8$ | 41 | -0.12 | 3.60 | 3.4 | Pass |
|  |  |  |  |  |  | Summary | ELC: | Load Case 5 |
|  |  |  |  |  |  | Leg (T2) | 71.5 | Pass |
|  |  |  |  |  |  | Diagonal (T1) | 94.3 | Pass |
|  |  |  |  |  |  | Top Girt (T1) | 10.7 | Pass |
|  |  |  |  |  |  | Bolt Checks | 94.3 | Pass |
|  |  |  |  |  |  | Rating = | 94.3 | Pass |

## APPENDIX B

## BASE LEVEL DRAWING



## APPENDIX C

## ADDITIONAL CALCULATIONS

Beam Properties: $\quad W_{12} \times 35$

$$
\begin{array}{lll}
b_{f}=6.56^{\prime \prime} & d=12.5^{\prime \prime} & F_{y}=36 \mathrm{ksi} \\
t_{f}=0.520^{\prime \prime} & t_{w}=0.30^{\prime \prime} \quad & I_{x}=285 \mathrm{in}^{4} \\
f_{b}=\frac{M I_{c}}{I}=\frac{36 \mathrm{k} \times \frac{9^{\prime}}{4} \times \frac{12^{\prime \prime}}{1} \times \frac{12-5^{\prime \prime}}{2}}{285 \mathrm{in}^{4}}=21.3 \mathrm{ksi} \\
F_{b}=0.66 \mathrm{Fy} \times 5 . F==31.68 \mathrm{ksi}
\end{array}
$$

Beam rating: $\frac{f_{b}}{F_{b}} \times 100=\frac{21.3}{31.68 \mathrm{ksi}}=6.7 .3 \%$
environmental | engineering | due diligence

# RADIO FREQUENCY FCC REGULATORY COMPLIANCE MAXIMUM PERMISSIBLE EXPOSURE (MPE) ASSESSMENT 

## Sprint Existing Facility

Site ID: CT03XC075
West Hartford Parking Garage
27-31 South Main Street
West Hartford, CT 06110
April 7, 2014

EBI Project Number: 62141886

April 7, 2014

Sprint
Attn: RF Engineering Manager
1 International Boulevard, Suite 800
Mahwah, NJ 07495
Re: Radio Frequency Maximum Permissible Exposure (MPE) Assessment for Site:
CT03XC075 - West Hartford Parking Garage
Site Total: $\mathbf{6 7 . 7 8 9 \%}$ - MPE\% in full compliance (At Ground Level)

## Site Total: 237.188\% - MPE \% Not in full compliance (At Rooftop Level)

EBI Consulting was directed to analyze the proposed upgrades to the existing Sprint facility located at 27-31 South Main Street, West Hartford, CT, for the purpose of determining whether the radio frequency (RF) exposure levels from the proposed Sprint equipment upgrades on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (\% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu \mathrm{W} / \mathrm{cm} 2$ ). The number of $\mu \mathrm{W} / \mathrm{cm} 2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter $\left(\mu \mathrm{W} / \mathrm{cm}^{2}\right)$. The general population exposure limit for the cellular band ( 850 MHz Band) is approximately $567 \mu \mathrm{~W} / \mathrm{cm}^{2}$, and the general population exposure limit for the 1900 MHz and 2500 MHz
bands is $1000 \mu \mathrm{~W} / \mathrm{cm}^{2}$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## CALCULATIONS

Calculations were done for the proposed upgrades to the existing Sprint Wireless antenna facility located at 27-31 South Main Street, West Hartford, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario. Actual values seen from this site will be dramatically less than those shown in this report. For this report the sample point is the top of a 6 foot person standing at the base of the tower. Additionally, calculations were performed for the 6 foot person standing on the roof level of the parking garage.

For all calculations, all emissions were calculated using the following assumptions:

1) 4 channels in the 1900 MHz Band were considered for each sector of the proposed installation.
2) 1 channel in the 800 MHz Band was considered for each sector of the proposed installation
3) 2 channels in the 2500 MHz Band were considered for each sector of the proposed installation.
4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
5) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. Additionally, Calculations were performed for the 6 foot person standing on the roof level of the parking garage. The maximum gain of the antenna per the antenna manufactures supplied specifications was used in this direction in both scenarios.
environmental | engineering | due diligence
6) The antennas used in this modeling are the RFS APXVSPP18-C-A20 and the RFS APXVTMM-C-120. This is based on feedback from the carrier with regards to anticipated antenna selection. The RFS APXVSPP18-C-A20 has a 15.9 dBd gain value at its main lobe at 1900 MHz and 13.4 dBd at its main lobe for 850 MHz . The RFS APXVTMM-C-120 has a 15.9 dBd gain value at its main lobe at 2500 MHz . All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario.
7) The antenna mounting height centerline for the proposed antennas is $\mathbf{1 0 3}$ feet above ground level (AGL) and 52 feet above the rooftop level (ARL).
8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public threshold limits

|  | Site ID | CT03XC075 - West Hartford Parking Garage (ground Level) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Site Addresss | 27-31 South Main Street, West Hartford, CT 06110 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Site Type | Self Support Tower |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Sector 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Antenna Number | Antenna Make | Antenna Model | Radio Type | Frequency Band | Technology | Power Out Per Channel (Watts) | Number of Channels | Composite Power | Antenna Gain in direction of sample point (dBd) | Antenna Height (ft) | analysis height | Cable Size | Cable Loss <br> (dB) | Additional Loss (dB) | ERP | $\begin{array}{\|c\|} \hline \text { Power } \\ \text { Density } \\ \text { Percentage } \\ \hline \end{array}$ |
| 1a | RFS | APXVSPP18-C-A20 | RRH | 1900 MHz | CDMA / LTE | 20 | 4 | 80 | 15.9 | 103 | 97 | 1/2" | 0.5 | 3 | 1390.2407 | 5.31193\% |
| 1a | RFS | APXVSPP18-C-A20 | RRH | 850 MHz | CDMA / LTE | 20 | 1 | 20 | 13.4 | 103 | 97 | 1/2 " | 0.5 | 3 | 195.44744 | 1.31707\% |
| 1B | RFS | APXVTMM14-C-120 | RRH | 2500 MHz | CDMA / LTE | 20 | 2 | 40 | 13.4 | 103 | 97 | 1/2 " | 0.5 | 3 | 390.89489 | 2.63414\% |
| Sector total Power Density Value: $9.263 \%$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Sector 2 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Antenna Number | Antenna Make | Antenna Model | Radio Type | Frequency Band | Technology | Power Out Per Channel (Watts) | Number of Channels | Composite Power | Antenna Gain in direction of sample point (dBd) | Antenna Height (ft) | analysis height | Cable Size | Cable Loss <br> (dB) | Additional Loss (dB) | ERP | Power <br> Density Percentage |
| 2a | RFS | APXVSPP18-C-A20 | RRH | 1900 MHz | CDMA / LTE | 20 | 4 | 80 | 15.9 | 103 | 97 | 1/2" | 0.5 | 3 | 1390.2407 | 5.31193\% |
| 2a | RFS | APXVSPP18-C-A20 | RRH | 850 MHz | CDMA / LTE | 20 | 1 | 20 | 13.4 | 103 | 97 | 1/2 " | 0.5 | 3 | 195.44744 | 1.31707\% |
| 2 B | RFS | APXVTMM14-C-120 | RRH | 2500 MHz | CDMA / LTE | 20 | 2 | 40 | 13.4 | 103 | 97 | 1/2" | 0.5 | 3 | 390.89489 | 2.63414\% |
| Sector total Power Density Value: $9.263 \%$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Sector 3 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Antenna Number | Antenna Make | Antenna Model | Radio Type | Frequency Band | Technology | Power <br> Out Per <br> Channel <br> (Watts) | Number of Channels | Composite Power | Antenna Gain in direction of sample point (dBd) | Antenna Height (ft) | analysis height | Cable Size | Cable Loss <br> (dB) | Additional Loss (dB) | ERP | Power <br> Density <br> Percentage |
| 3a | RFS | APXVSPP18-C-A20 | RRH | 1900 MHz | CDMA / LTE | 20 | 4 | 80 | 15.9 | 103 | 97 | $1 / 2$ " | 0.5 | 3 | 1390.2407 | 5.31193\% |
| 3 a | RFS | APXVSPP18-C-A20 | RRH | 850 MHz | CDMA / LTE | 20 | 1 | 20 | 13.4 | 103 | 97 | 1/2" | 0.5 |  | 195.44744 | 1.31707\% |
| 3B | RFS | APXVTMM14-C-120 | RRH | 2500 MHz | CDMA / LTE | 20 | 2 | 40 | 13.4 | 103 | 97 | 1/2 " | 0.5 |  | 390.89489 | 2.63414\% |
|  |  |  |  |  |  |  |  |  |  |  |  | Sector to | tal Power D | ensity Value: | 9.263\% |  |


| Site Composite MPE \% |  |
| :---: | :---: |
| Carrier | MPE \% |
| Sprint | $27.789 \%$ |
| AT\&T | $40.000 \%$ |
| Total Site MPE \% | $67.789 \%$ |


|  | Site ID | CT03XC075 - West Hartford Parking Garage (Roof Level) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Site Addresss | 27 - 31 South Main Street, West Hartford, CT 06110 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Site Type | Self Support Tower |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Sector 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Antenna Number | Antenna Make | Antenna Model | Radio Type | Frequency Band | Technology | Power Out Per Channel (Watts) | Number of Channels | Composite Power | Antenna Gain in direction of sample point (dBd) | Antenna Height (ft) | analysis height | Cable Size | Cable Loss <br> (dB) | Additional Loss (dB) | ERP | Power Density Percentage |
| 1a | RFS | APXVSPP18-C-A20 | RRH | 1900 MHz | CDMA / LTE | 20 | 4 | 80 | 15.9 | 52 | 46 | 1/2 " | 0.5 | 3 | 1390.2407 | 23.62002\% |
| 1a | RFS | APXVSPP18-C-A20 | RRH | 850 MHz | CDMA / LTE | 20 | 1 | 20 | 13.4 | 52 | 46 | $1 / 2^{\prime \prime}$ | 0.5 | 3 | 195.44744 | 5.85649\% |
| 1B | RFS | APXVTMM14-C-120 | RRH | 2500 MHz | CDMA / LTE | 20 | 2 | 40 | 13.4 | 52 | 46 | 1/2 " | 0.5 | 3 | 390.89489 | 11.71298\% |
| Sector total Power Density Value: $\quad 41.189 \%$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Sector 2 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Antenna Number | Antenna Make | Antenna Model | Radio Type | Frequency Band | Technology | Power Out Per Channel (Watts) | Number of Channels | Composite Power | Antenna Gain in direction of sample point (dBd) | Antenna Height (ft) | analysis height | Cable Size | Cable Loss <br> (dB) | Additional Loss (dB) | ERP | Power Density Percentage |
| 2a | RFS | APXVSPP18-C-A20 | RRH | 1900 MHz | CDMA / LTE | 20 | 4 | 80 | 15.9 | 52 | 46 | 1/2" | 0.5 | 3 | 1390.2407 | 23.62002\% |
| 2a | RFS | APXVSPP18-C-A20 | RRH | 850 MHz | CDMA / LTE | 20 | 1 | 20 | 13.4 | 52 | 46 | 1/2" | 0.5 | 3 | 195.44744 | 5.85649\% |
| 2B | RFS | APXVTMM14-C-120 | RRH | 2500 MHz | CDMA / LTE | 20 | 2 | 40 | 13.4 | 52 | 46 | 1/2 " | 0.5 | 3 | 390.89489 | 11.71298\% |
| Sector total Power Density Value: $\quad 41.189 \%$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Sector 3 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Antenna Number | Antenna Make | Antenna Model | Radio Type | Frequency Band | Technology | Power Out Per Channel (Watts) | Number of Channels | Composite Power | Antenna Gain in direction of sample point (dBd) | Antenna Height (ft) | analysis height | Cable Size | Cable Loss <br> (dB) | Additional Loss (dB) | ERP | Power Density Percentage |
| 3a | RFS | APXVSPP18-C-A20 | RRH | 1900 MHz | CDMA / LTE | 20 | 4 | 80 | 15.9 | 52 | 46 | 1/2" | 0.5 | 3 | 1390.2407 | 23.62002\% |
| 3 a | RFS | APXVSPP18-C-A20 | RRH | 850 MHz | CDMA / LTE | 20 | 1 | 20 | 13.4 | 52 | 46 | $1 / 2{ }^{\prime \prime}$ | 0.5 | 3 | 195.44744 | 5.85649\% |
| 3B | RFS | APXVTMM14-C-120 | RRH | 2500 MHz | CDMA / LTE | 20 | 2 | 40 | 13.4 | 52 | 46 | 1/2" | 0.5 | 3 | 390.89489 | 11.71298\% |
|  |  |  |  |  |  |  |  |  |  |  |  | Sector to | tal Power De | ensity Value: | 41.189\% |  |


| Site Composite MPE \% |  |
| :---: | :---: |
| Carrier | MPE \% |
| Sprint | $123.568 \%$ |
| AT\&T | $113.620 \%$ |
| Total Site MPE \% | $237.188 \%$ |

## Summary

All calculations performed for this analysis yielded results that were well within the allowable limits for general public Maximum Permissible Exposure (MPE) to radio frequency energy at ground level. At the Rooftop level there are areas that may exceed the general public Maximum Permissible Exposure (MPE) to radio frequency energy.

The anticipated Maximum Composite contributions from the Sprint facility are 27.789\% (9.263\% from each sector) of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level.

The anticipated composite MPE value for this site assuming all carriers present is $\mathbf{5 7 . 8 6 2 \%}$ of the allowable FCC established general public limit sampled at 6 feet above ground level. This total composite site value is based upon MPE values listed in the Connecticut Siting Council database for existing carrier emissions.

Additionally, Since the Connecticut Siting Council database has values listed for the parking garage rooftop level as well these calculations were performed. The anticipated Maximum Composite contributions from the Sprint facility for a 6 foot person at the parking garage rooftop level is $\mathbf{1 2 3 . 5 6 8 \%}$ $\mathbf{( 4 1 . 1 8 9 \%}$ from each sector) of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the parking garage rooftop level.

The anticipated composite MPE value for this site assuming all carriers present is $\mathbf{2 3 7 . 1 8 8 \%}$ of the allowable FCC established general public limit for a 6 foot person at the parking garage rooftop level. This total composite site value is based upon MPE values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a $5 \%$ contribution to the composite value will require measures to bring the site into compliance.


## Scott Heffernan

RF Engineering Director

## EBI Consulting

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[^0]:    Consider Moments - Legs
    Consider Moments - Horizontals
    Consider Moments - Diagonals
    Use Moment Magnification
    $\sqrt{ }$ Use Code Stress Ratios
    $\sqrt{ }$ Use Code Safety Factors - Guys
    $\sqrt{ }$ Escalate Ice
    Always Use Max Kz
    Use Special Wind Profile
    $\sqrt{ }$ Include Bolts In Member Capacity Leg Bolts Are At Top Of Section
    $\sqrt{ }$ Secondary Horizontal Braces Leg
    Use Diamond Inner Bracing (4 Sided)
    Add IBC .6D+W Combination

