

Aaron Meyers, Site Acquisition Consultant c/o New Cingular Wireless, PCS LLC (AT&T) Centerline Communications, LLC 95 Ryan Drive, Suite 1 Raynham, MA 02767 Mobile: (774) 420-4202 ameyers@clinellc.com

DATE: 10/31/2017

Melanie A. Bachman Acting Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: Notice of Exempt Modification // Site Number: CT5843 29 South Main St. West Hartford, CT 06107 (Site Name: West Hartford) N 41.760190 // W 72.743190

Dear Ms. Bachman:

New Cingular Wireless, PCS, LLC ("AT&T") currently maintains nine (9) antennas at the 89' foot level of the existing 103' foot lattice tower at 29 South Main Street. The tower is owned by Crown Castle. The property is owned by Town Center West Associates, LLC. AT&T now intends to swap six (6) antennas for its LTE upgrade. These antennas would be installed at the 89' foot level of the tower. AT&T also intends to install twelve (12) remote radio units, one (1) splitter, and two (2) DC power lines.

The current proposal involves an antenna swap only (3); three (3) antennas will be added.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Shari Cantor, Mayor of West Hartford, CT, as well as the tower owner, Crown Castle and the ground owner, Town Center West Associates, LLC.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

Attached to accommodate this filing are construction drawings dated 9/5/17 by Hudson Design Group, LLC, a structural analysis dated 8/15/2017 by GPD Engineering and Architecture Professional Corporation and an Emissions Analysis Report dated 10/4/17 by Centerline Communications, LLC.

1. The proposed modifications will not result in an increase in the height of the existing structure.

2. The proposed modifications will not require the extension of the site boundary.

3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.

4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.

5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.

6. The existing structure and its foundation can support the proposed loading as shown in the attached structural analysis by GPD Engineering and Architecture Professional Corporation, dated August 15, 2017.

For the foregoing reasons, AT&T respectfully submits that the proposed modifications to the above referenced telecommunications facility constitute an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

Aaron Meyers, Site Acquisition Consultant c/o New Cingular Wireless, PCS LLC (AT&T) Centerline Communications, LLC 95 Ryan Drive, Suite 1 Raynham, MA 02767 Mobile: (774) 420-4202 ameyers@clinellc.com

Attachments

cc: Shari Cantor - as elected official Crown Castle - as tower owner Town Center West Associates, LLC - as property owner



Radio Frequency Emissions Analysis Report

AT&T Existing Facility

Site ID: CT5843

West Hartford Parking Garage 27-31 South Main Street West Hartford, CT 6107

October 4, 2017

Centerline Communications Project Number: 950012-006

Site Compliance Summary						
Compliance Status:	COMPLIANT					
Site total MPE% of FCC general population allowable limit (Ground Level):	15.90 %					
Site total MPE% of FCC general population allowable limit (Rooftop Level):	15.90 %					



October 4, 2017

AT&T Mobility – New England Attn: John Benedetto, RF Manager 550 Cochituate Road Suite 550 – 13&14 Framingham, MA 06040

Emissions Analysis for Site: CT5843 – West Hartford Parking Garage

Centerline Communications, LLC ("Centerline") was directed to analyze the proposed AT&T facility located at **27-31 South Main Street, West Hartford, CT**, for the purpose of determining whether the emissions from the Proposed AT&T Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm2). The number of μ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

<u>General population/uncontrolled exposure</u> limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications facility that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μ W/cm²). The general population exposure limits for the 700 and 850 MHz Bands are approximately 467 μ W/cm² and 567 μ W/cm² respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 2300 MHz (WCS) bands is 1000 μ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



<u>Occupational/controlled exposure</u> limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over this or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.



CALCULATIONS

Calculations were performed for the proposed AT&T Wireless antenna facility located at **27-31 South Main Street, West Hartford, CT**, using the equipment information listed below. Calculations were performed at both the Ground Level as well as the Rooftop Walking Surface Level. All calculations were performed per the specifications under FCC OET 65. Since AT&T is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the facility. For this report the sample point is the top of a 6-foot person standing at the base of the facility as well as on the Rooftop Walking Surface.

Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. All power values expressed and analyzed are maximum power levels expected to be used on all radios.

All emissions values for additional carriers were taken from the Connecticut Siting Council (CSC) active MPE database. Values in this database are provided by the individual carriers themselves

For each sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
UMTS	850 MHz	2	30
UMTS	1900 MHz (PCS)	2	30
LTE	700 MHz	4	60
LTE	850 MHz	2	60
LTE	1900 MHz (PCS)	2	60
LTE	2300 MHz (WCS)	2	60
LTE	2100 MHz (AWS)	2	60

Table 1: Channel Data Table



The following antennas listed in *Table 2* were used in the modeling for transmission in the 700 MHz, 850 MHz, 1900 MHz (PCS) and 2300 MHz (WCS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

				Antenna	Antenna
			Duplexer /	Centerline	Centerline
			Combiner	from	from
	Antenna		Losses	Ground	Rooftop
Sector	Number	Antenna Make / Model	(dB)	Level (ft)	Level (ft)
А	1	Powerwave 7770	3	89	38
А	2	CCI OPA-65R-LCUU-H8	3	89	38
А	3	CCI TPA-65R-LCUUUU-H8	3	89	38
В	1	Powerwave 7770	3	89	38
В	2	CCI OPA-65R-LCUU-H8	3	89	38
В	3	CCI TPA-65R-LCUUUU-H8	3	89	38
С	1	Powerwave 7770	3	89	38
С	2	CCI OPA-65R-LCUU-H8	3	89	38
С	3	CCI TPA-65R-LCUUUU-H8	3	89	38

Table 2: Antenna Data

All calculations were done with respect to uncontrolled / general population threshold limits.



RESULTS FOR GROUND LEVEL CALCULATIONS

Per the calculations completed for the proposed AT&T configurations *Table 3a* shows resulting emissions power levels and percentages of the FCC's allowable general population limit at ground level.

					Total TX	Duplexer / Combiner		
Antenna			Antenna Gain	Channel	Power	Losses	ERP	
ID	Antenna Make / Model	Frequency Bands	(dBd)	Count	(W)	(dB)	(W)	MPE %
Antenna		850 MHz /						
A1	Powerwave 7770	1900 MHz (PCS)	11.4 / 13.4	4	120	3	1,072.99	0.73
		700 MHz /						
Antenna	CCI	850 MHz /	12.55 / 13.35					
A2	OPA-65R-LCUU-H8	1900 MHz (PCS)	/ 14.85	6	360	3	4,219.90	3.36
		700 MHz /						
Antenna	CCI	2300 MHz (WCS) /	12.95 / 14.45					
A3	TPA-65R-LCUUUU-H8	2100 MHz (AWS)	/ 14.25	6	360	3	4,462.13	3.04
					Sec	ctor A Compos	ite MPE%	7.13
Antenna		850 MHz /						
B1	Powerwave 7770	1900 MHz (PCS)	11.4 / 13.4	4	120	3	1,072.99	0.73
		700 MHz /						
Antenna	CCI	850 MHz /	12.55 / 13.35					
B2	OPA-65R-LCUU-H8	1900 MHz (PCS)	/ 14.85	6	360	3	4,219.90	3.36
		700 MHz /						
Antenna	CCI	2300 MHz (WCS) /	12.95 / 14.45					
B3	TPA-65R-LCUUUU-H8	2100 MHz (AWS)	/ 14.25	6	360	3	4,462.13	3.04
					Sec	ctor B Compos	ite MPE%	7.13
Antenna		850 MHz / 1900						
C1	Powerwave 7770	MHz (PCS)	11.4 / 13.4	4	120	3	1,072.99	0.73
		700 MHz /						
Antenna	CCI	850 MHz /	12.55 / 13.35					
C2	OPA-65R-LCUU-H8	1900 MHz (PCS)	/ 14.85	6	360	3	4,219.90	3.36
		700 MHz /						
Antenna	CCI	2300 MHz (WCS) /	12.95 / 14.45					
C3	TPA-65R-LCUUUU-H8	2100 MHz (AWS)	/ 14.25	6	360	3	4,462.13	3.04
					Sec	ctor C Compos	ite MPE%	7.13

Table 3a: AT&T Emissions Levels at Ground Level



The Following table (*table 4a*) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum AT&T MPE contributions per this report calculated at ground level. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, all three sectors have the same configuration yielding the same results on all three sectors. *Table 5a* below shows a summary for each AT&T Sector as well as the composite MPE value for the site at ground level.

Site Composite MPE% (Ground Level)					
Carrier	MPE%				
AT&T – Max Sector Value	7.13 %				
Sprint	8.77 %				
Site Total MPE %:	15.90 %				

Table 4a: All Carrier MPE Contributions at Ground Level

AT&T Sector A Total:	7.13 %
AT&T Sector B Total:	7.13 %
AT&T Sector C Total:	7.13 %
Site Total:	15.90 %

Table 5a: Site MPE Summary at Ground Level



FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6a* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated AT&T sector(s) at ground level. For this site, all three sectors have the same configuration yielding the same results on all three sectors at ground level.

AT&T _ Frequency Band / Technology (All Sectors at Ground Level)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm ²)	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
AT&T 850 MHz UMTS – Antenna 1	2	207.55	89	2.17	850 MHz	567	0.38%
AT&T 1900 MHz (PCS) UMTS – Antenna 1	2	328.94	89	3.43	1900 MHz (PCS)	1000	0.34%
AT&T 700 MHz LTE – Antenna 2	2	540.94	89	5.65	700 MHz	467	1.21%
AT&T 850 MHz LTE – Antenna 2	2	650.36	89	6.79	850 MHz	567	1.20%
AT&T 1900 MHz (PCS) LTE - Antenna 2	2	918.65	89	9.59	1900 MHz (PCS)	1000	0.96%
AT&T 700 MHz LTE – Antenna 3	2	593.13	89	6.19	700 MHz	467	1.33%
AT&T 2300 MHz (WCS) LTE - Antenna 3	2	837.82	89	8.74	2300 MHz (WCS)	1000	0.87%
AT&T 2100 MHz (AWS) LTE - Antenna 3	2	800.11	89	8.35	2100 MHz (AWS)	1000	0.84%
						Total:	7.13%

Table 6a: AT&T Maximum Sector MPE Power Values at Ground Level



RESULTS FOR ROOFTOP LEVEL CALCULATIONS

Per the calculations completed for the proposed AT&T configurations *Table 3b* shows resulting emissions power levels and percentages of the FCC's allowable general population limit on the parking garage rooftop walking surface.

					Total TX	Duplexer / Combiner		
Antenna			Antenna Gain	Channel	Power	Losses	ERP	
ID	Antenna Make / Model	Frequency Bands	(dBd)	Count	(W)	(dB)	(W)	MPE %
Antenna	Thitemia Make / Moder	850 MHz /	(ubu)	Count	(")	(uD)	(")	
A1	Powerwave 7770	1900 MHz (PCS)	11.4 / 13.4	4	120	3	1,072.99	6.58
		700 MHz /					,	
Antenna	CCI OPA-65R-LCUU-	850 MHz /	12.55 / 13.35					
A2	H8	1900 MHz (PCS)	/ 14.85	6	360	3	4,219.90	20.24
		700 MHz /						
Antenna	CCI TPA-65R-	2300 MHz (WCS) /	12.95 / 14.45					
A3	LCUUUU-H8	2100 MHz (AWS)	/ 14.25	6	360	3	4,462.13	20.91
					Sec	ctor A Compos	ite MPE%	47.73
Antenna		850 MHz /						
B1	Powerwave 7770	1900 MHz (PCS)	11.4 / 13.4	4	120	3	1,072.99	6.58
		700 MHz /						
Antenna	CCI OPA-65R-LCUU-	850 MHz /	12.55 / 13.35					
B2	H8	1900 MHz (PCS)	/ 14.85	6	360	3	4,219.90	20.24
		700 MHz /						
Antenna	CCI TPA-65R-	2300 MHz (WCS) /	12.95 / 14.45					
B3	LCUUUU-H8	2100 MHz (AWS)	/ 14.25	6	360	3	4,462.13	20.91
					See	ctor B Compos	ite MPE%	47.73
Antenna		850 MHz / 1900						
C1	Powerwave 7770	MHz (PCS)	11.4 / 13.4	4	120	3	1,072.99	6.58
		700 MHz /						
Antenna	CCI OPA-65R-LCUU-	850 MHz /	12.55 / 13.35					
C2	H8	1900 MHz (PCS)	/ 14.85	6	360	3	4,219.90	20.24
		700 MHz /						
Antenna	CCI TPA-65R-	2300 MHz (WCS) /	12.95 / 14.45	_	2.50		1.160.10	20.01
C3	LCUUUU-H8	2100 MHz (AWS)	/ 14.25	6	360	3	4,462.13	20.91
					See	ctor C Compos	ite MPE%	47.73

Table 3b: AT&T Emissions Levels at the Rooftop Walking Surface



The Following table (*table 4b*) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum AT&T MPE contributions on the parking garage rooftop walking surface per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, all three sectors have the same configuration yielding the same results on all three sectors. *Table 5b* below shows a summary for each AT&T Sector as well as the composite MPE value for the site on the parking garage rooftop walking surface.

Site Composite MPE%						
Carrier	MPE%					
AT&T – Max Sector Value	47.73 %					
Sprint	37.75 %					
Site Total MPE %:	85.48 %					

Table 4b: All Carrier MPE Contributions at the Rooftop Walking Surface

AT&T Sector A Total:	47.73 %
AT&T Sector B Total:	47.73 %
AT&T Sector C Total:	47.73 %
Site Total MPE %:	85.48 %

Table 5b: Site MPE Summary at the Rooftop Walking Surface



FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6b* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated AT&T sector(s). For this site, all three sectors have the same configuration yielding the same results on all three sectors on the parking garage rooftop walking surface.

AT&T _ Frequency Band / Technology (All Sectors at Rooftop Walking Surface Level)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm ²)	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
AT&T 850 MHz UMTS – Antenna 1	2	276.77	38	19.43	850 MHz	567	3.43%
AT&T 1900 MHz (PCS) UMTS – Antenna 1	2	448.87	38	31.52	1900 MHz (PCS)	1000	3.15%
AT&T 700 MHz LTE – Antenna 2	2	471.14	38	33.08	700 MHz	467	7.08%
AT&T 850 MHz LTE – Antenna 2	2	553.54	38	38.87	850 MHz	567	6.85%
AT&T 1900 MHz (PCS) LTE – Antenna 2	2	897.74	38	63.04	1900 MHz (PCS)	1000	6.30%
AT&T 700 MHz LTE – Antenna 3	2	471.14	38	33.08	700 MHz	467	7.08%
AT&T 2300 MHz (WCS) LTE - Antenna 3	2	1,007.28	38	70.73	2300 MHz (WCS)	1000	7.07%
AT&T 2100 MHz (AWS) LTE - Antenna 3	2	961.95	38	67.54	2100 MHz (AWS)	1000	6.75%
						Total:	47.73%

Table 6b: AT&T Maximum Sector MPE Power Values at the Rooftop Walking Surface



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions at both the Ground Level and Rooftop Walking Surfaces.

The anticipated maximum composite contributions from the AT&T facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here for both the Ground Level and Rooftop Walking Surfaces:

AT&T Sector	Power Density Value
(Ground Level)	(%)
Sector A:	7.13 %
Sector B:	7.13 %
Sector C:	7.13 %
AT&T Maximum Total (per sector):	7.13 %
Site Total:	15.90 %
Site Compliance Status:	COMPLIANT
AT&T Sector	Power Density Value
(Rooftop Walking Surface)	(%)
Sector A:	47.73 %
Sector B:	47.73 %
Sector C:	47.73 %
AT&T Maximum Total (per	
sector):	47.73 %
Site Total:	85.48%
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **15.90** % of the allowable FCC established general population limit sampled at the Ground Level and **85.48** % at the Rooftop Walking Surface. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan RF Engineering Director Centerline Communications, LLC

95 Ryan Drive, Suite 1 Raynham, MA 02767 Date: August 15, 2017

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Marianne Dunst		riolessional corporation
Crown Castle 3530 Toringdon Way Suite 300 Charlotte, NC 28277 (704) 405-6580		520 South Main Street, Suite 2531 Akron, Ohio 44311 (216) 927-8663 dpalkovic@gpdgroup.com
Subject:	Structural Analysis Report	
Carrier Designation:	AT&T Mobility Co-Locate Carrier Site Number: Carrier Site Name:	CT5843 West Hartford
Crown Castle Designation:	Crown Castle BU Number: Crown Castle Site Name: Crown Castle JDE Job Number: Crown Castle Work Order Number Crown Castle Application Number	:1443101
Engineering Firm Designation	: GPD Project Number:	2017777.876328.17
Site Data:	27-31 South Main St., West Hartfor Latitude 41° 45' 36.41", Longitude 40.25 Foot - Self Support and Modif	

Dear Marianne Dunst,

We are pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 1068988, in accordance with application 397668, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

Sufficient Capacity

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon a nominal 3-second gust wind speed of 100 mph per the guidelines within Appendix R. Exposure Category B with a maximum topographic factor, K_{zt} , of 1.0 and Risk Category II were used in this analysis.

We appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Benjamin Darkow

Respectfully submitted by:

Christopher J. Scheks, P.E. Connecticut #: 0030026

AOEKS 8/15/2017

TABLE OF CONTENTS

1) INTRODUCTION

2) ANALYSIS CRITERIA

- Table 1 Proposed Antenna and Cable Information
- Table 2 Existing and Reserved Antenna and Cable Information
- Table 3 Design Antenna and Cable Information

3) ANALYSIS PROCEDURE

- Table 4 Documents Provided
- 3.1) Analysis Method
- 3.2) Assumptions

4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary) Table 6 - Tower Components vs. Capacity 4.1) Recommendations

5) DISCLAIMER OF WARRANTIES

6) APPENDIX A

tnxTower Output

7) APPENDIX B

Base Level Drawing

8) APPENDIX C

Additional Calculations

1) INTRODUCTION

This tower is a 40.25 ft self support tower designed by ROHN in April of 1997. The tower was originally designed for a wind speed of 85 mph per TIA/EIA-222-E. The tower base connects to an I-Beam frame that is anchored to the parking garage deck. The base of the tower frame is 65' above grade.

The tower is supported on three legs and has two major sections. It has a triangular cross section made of bolted connections, with an "X" frame configuration. The tower is fabricated with pipe legs and angle diagonals. The tower is galvanized and has no aviation lightning.

Modifications designed by GPD (Project #: 2015777.876328.08, dated 6/3/2015) consist of installing extension plates to the tower base frame connections and extension plates to the existing stair well walls at varying elevations. These modifications have been installed and were considered in this analysis.

2) ANALYSIS CRITERIA

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon a nominal 3-second gust wind speed of 100 mph per the guidelines within Appendix R. Additionally, 50 mph with 1 inch ice thickness and 60 mph under service loads, exposure category B with topographic category 1 and crest height of 0 feet.

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
		3	CCI Antennas	OPA-65R-LCUU-H8			
		3	CCI Antennas	DTMABP7819VG12A			
		3	Ericsson	RRUS 12			
92.0	89.0	3	Ericsson	RRUS 32 B2	2	3/4	1
		3	Ericsson	RRUS E2 B29			
		3	Ericsson	RRUS 32 B66			
		1	Raycap	DC6-48-60-18-8F			

Table 1 - Proposed Antenna and Cable Information

Notes:

1) See Appendix B for the proposed feed line layout

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
		3	RFS/Celwave	APXVTM14-C-120	1	5/8	1
		3	Alcatel Lucent	TD-RRH8x20-25		5/0	
	103.0	2	RFS/Celwave	APXVSPP18-C-A20			
102.0		1	RFS/Celwave	APXV9ERR18-C-A20			
102.0		3	Alcatel Lucent	1900MHz RRH (65MHz)	3	1-1/4	
	102.0	3	Alcatel Lucent	800MHz 2X50W RRH W/FILTER		1 1/4	
		1		Sector Mount [SM 502-3]			
		3	Ericsson	RRUS-11			
	92.0	3	Sabre	C10857011 12' V-Boom	-		
		3	Powerwave Technologies	7770.00	2	3/8	
		3	CCI Antennas	TPA-65R-LCUUUU-H8	4	3/4 1-5/8	
		3	Powerwave Technologies	7020.00		1-5/0	
92.0		3	Ericsson	RRUS 32 B30	-		
	89.0	2	Raycap	DC6-48-60-18-8F			
	03.0	1	Powerwave Technologies	P65-15-XLH-RR			
		1	Andrew	SBNH-1D6565C	•		
		1	Powerwave Technologies	P65E-17-XLH-RR	-		2
		6	Powerwave Technologies	LGP2140X			
75.0	77.0	1	Lucent	KS24019-L112A	1	1/2	
75.0	75.0	1		Side Arm Mount [SO 302-1]		1/2	

Notes:

Reserved equipment; considered in this analysis Existing equipment to be removed; not considered in this analysis 1) 2)

Table 3 - Design Antenna and Cable Information

Mounting Level (ft)	Elovation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	
105.0	105.0	105.0	12	Decibel	DB980H90	12	1-5/8
105.0			3		12' Leg Mounting Frame		1-5/0
75.0	75.0 1			GPS Antenna	4	1 5/9	
75.0	/ 5.0	1		3' Side Arm		1-5/8	

3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

Document	Remarks	Reference	Source
Tower Manufacturer Drawings	Rohn Eng. File#: 345895W, Dated: 4/15/1997	1440544	CCISITES
Tower Mapping Report	GPD Project #: 2014777.876328.03, Dated: 3/04/2014	1440544	CCISITES
Base Frame Design	Greiner Project #: F101508.60, Dated: 2/20/1997	5460756	CCISITES
Parking Garage Design	Unistress Project: Towne Center Garage, Rev. 4, Dated: 10/31/1988	5460756	CCISITES
Parking Garage Modifications	GPD Project #: 2015777.876328.08, Dated: 6/3/2015	5735691	CCISITES
Modifications Passing Analysis	GPD Project #: 2015777.876328.08, Dated: 6/3/2015	5735731	CCISITES
Post Modification Inspection	GPD Project #: 2015777.876328.10, Dated 1/27/2016	6076906	CCISITES

3.1) Analysis Method

tnxTower (version 7.0.7.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.

This analysis may be affected if any assumptions are not valid or have been made in error. GPD should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Section No.	Elevation (ft)	Component Type	Size	Critical Element	Р (К)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	105.25 - 85.125	Leg	ROHN 2.5 STD	3	-15.06	63.41	23.7	Pass
T2	85.125 - 65	Leg	ROHN 2.5 STD	38	-39.11	57.07	68.5	Pass
T1	105.25 - 85.125	Diagonal	L1-1/2x1-1/2x1/8	9	-3.40	3.81	89.4	Pass
T2	85.125 - 65	Diagonal	L1-3/4x1-3/4x3/16	46	-3.12	5.09	61.3	Pass
T1	105.25 - 85.125	Top Girt	L2x2x1/8	4	-0.35	3.21	11.0	Pass
T2	85.125 - 65	Top Girt	L2x2x1/8	41	-0.15	3.21	4.8	Pass
							Summary	
						Leg (T2)	68.5	Pass
						Diagonal (T1)	89.4	Pass
						Top Girt (T1)	11.0	Pass
						Bolt Checks	87.9	Pass
						Rating =	89.4	Pass

Table 5 - Section Capacity (Summary)

Table 6 - Tower Component Stresses vs. Capacity – LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1, 2	Base Frame & Parking Garage	65	59.7	Pass

Structure Rating (max from all components) = 89	9.4%	
---	------	--

Notes:

1) See additional documentation in "Appendix C - Additional Calculations" for calculations supporting the % capacity consumed.

2) The base frame and parking garage capacity was determined based on reaction comparison from the previous modification design passing analysis (GPD Project #: 2015777.876328.08, dated 6/3/2015). See Appendix C for the reaction comparison.

4.1) Recommendations

The tower and modified parking garage have sufficient capacity to carry the proposed loading configuration. Modifications will not be required to bring them into compliance with the TIA-222-G standard for the proposed loading configuration.

5) DISCLAIMER OF WARRANTIES

GPD has not performed a site visit to the tower to verify the member sizes or antenna/coax loading. If the existing conditions are not as represented on the tower elevation contained in this report, we should be contacted immediately to evaluate the significance of the discrepancy. This is not a condition assessment of the tower or foundation. This report does not replace a full tower inspection. The tower and foundations are assumed to have been properly fabricated, erected, maintained, in good condition, twist free, and plumb.

The engineering services rendered by GPD in connection with this Structural Analysis are limited to a computer analysis of the tower structure and theoretical capacity of its main structural members. No allowance was made for any damaged, bent, missing, loose, or rusted members (above and below ground). No allowance was made for loose bolts or cracked welds.

This analysis is limited to the designated maximum wind and seismic conditions per the governing tower standards and code. Wind forces resulting in tower vibrations near the structure's resonant frequencies were not considered in this analysis and are outside the scope of this analysis. Lateral loading from any dynamic response was not evaluated under a time-domain based fatigue analysis.

GPD does not analyze the fabrication of the structure (including welding). It is not possible to have all the very detailed information needed to perform a thorough analysis of every structural sub-component and connection of an existing tower. GPD provides a limited scope of service in that we cannot verify the adequacy of every weld, plate connection detail, etc. The purpose of this report is to assess the capability of adding appurtenances usually accompanied by transmission lines to the structure.

It is the owner's responsibility to determine the amount of ice accumulation in excess of the code specified amount, if any, that should be considered in the structural analysis.

The attached sketches are a schematic representation of the analyzed tower. If any material is fabricated from these sketches, the contractor shall be responsible for field verifying the existing conditions, proper fit, and clearance in the field. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from GPD, but are beyond the scope of this report.

Miscellaneous items such as antenna mounts, etc., have not been designed or detailed as a part of our work. We recommend that material of adequate size and strength be purchased from a reputable tower manufacturer.

Towers are designed to carry gravity, wind, and ice loads. All members, legs, diagonals, struts, and redundant members provide structural stability to the tower with little redundancy. Absence or removal of a member can trigger catastrophic failure unless a substitute is provided before any removal. Legs carry axial loads and derive their strength from shorter unbraced lengths by the presence of redundant members and their connection to the diagonals with bolts or welds. If the bolts or welds are removed without providing any substitute to the frame, the leg is subjected to a higher unbraced length that immediately reduces its load carrying capacity. If a diagonal is also removed in addition to the connection, the unbraced length of the leg is greatly increased, jeopardizing its load carrying capacity. Failure of one leg can result in a tower collapse because there is no redundancy. Redundant members and diagonals are critical to the stability of the tower.

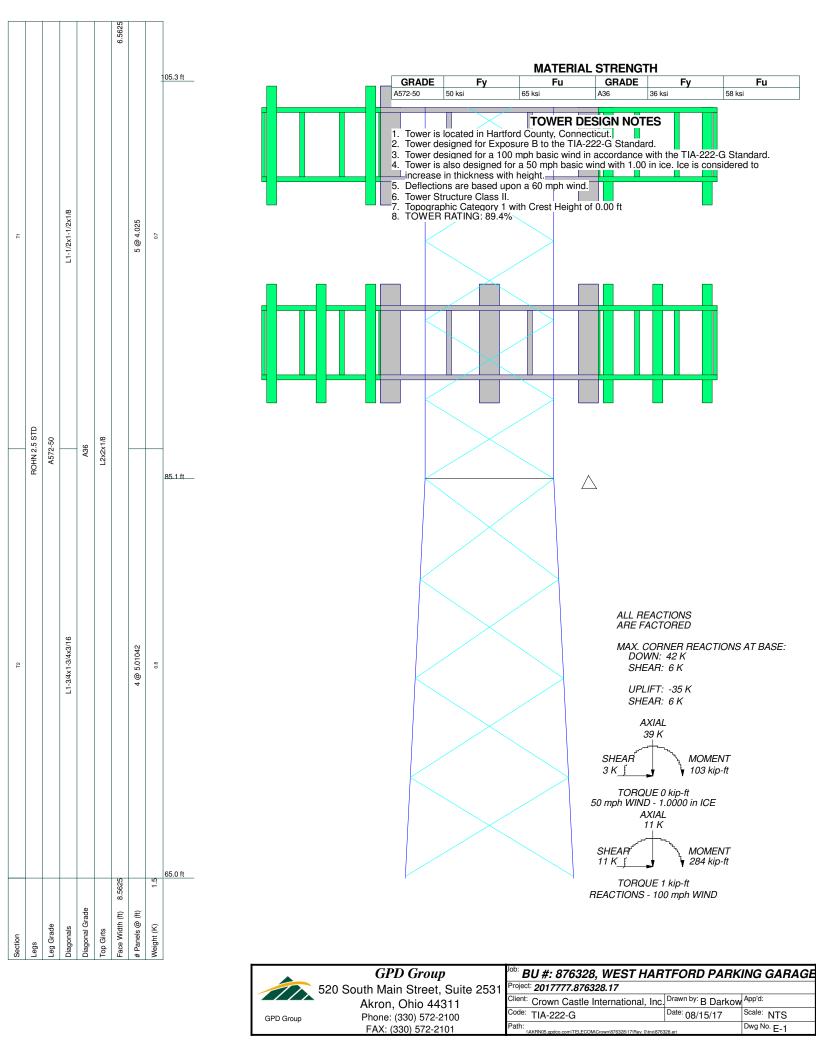
GPD makes no warranties, expressed and/or implied, in connection with this report and disclaims any liability arising from material, fabrication, and erection of this tower. GPD will not be responsible whatsoever for, or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of GPD pursuant to this report will be limited to the total fee received for preparation of this report.

APPENDIX A

TNXTOWER OUTPUT

				9					DEG		TENANCE LOADING	
						1	15.3 ft		TYPE		TYPE	ELEVATION
						Ī			APXVSPP18-C-A20 w/ Mount Pipe	102	DTMABP7819VG12A	92
									APXVSPP18-C-A20 w/ Mount Pipe APXV9ERR18-C-A20 w/ Mount Pipe	102 102	DTMABP7819VG12A	92 92
									APXV9ERR18-C-A20 w/ Mount Pipe APXVTM14-C-120 w/ Mount Pipe	102	7020.00 7020.00	92
									APXVTM14-C-120 w/ Mount Pipe	102	7020.00	92
									APXVTM14-C-120 w/ Mount Pipe	102	RRUS-11	92
									1900MHz RRH (65MHz)	102	RRUS-11	92
									1900MHz RRH (65MHz) 1900MHz RRH (65MHz)	102	RRUS-11 RRUS 12	92 92
									800MHz 2X50W RRH W/FILTER	102	RRUS 12	92
									800MHz 2X50W RRH W/FILTER	102	RRUS 12	92
	1/8								800MHz 2X50W RRH W/FILTER	102	RRUS 32 B2	92
	-1/2)				.025	~			TD-RRH8x20-25 TD-RRH8x20-25	102	RRUS 32 B2 RRUS 32 B2	92 92
	L1-1/2x1-1/2x1/8				@ 4.025	0.7			TD-RRH8x20-25	102	RRUS 32 B30	92
	5				0				8' x 2" Mount Pipe	102	RRUS 32 B30	92
	-								8' x 2" Mount Pipe 8' x 2" Mount Pipe	102 102	RRUS 32 B30 (2) RRUS E2 B29	92 92
									Sector Mount [SM 502-3]	102	(2) RRUS E2 B29 RRUS E2 B29	92
									7770.00 w/ Mount Pipe	92	RRUS 32 B66	92
									7770.00 w/ Mount Pipe	92	(2) RRUS 32 B66	92
									7770.00 w/ Mount Pipe	92	(2) DC6-48-60-18-8F Surge Suppression Unit	92
									TPA-65R-LCUUUU-H8 w/ Mount Pipe TPA-65R-LCUUUU-H8 w/ Mount Pipe		DC6-48-60-18-8F Surge Suppression	92
									TPA-65R-LCUUUU-H8 w/ Mount Pipe		Unit	
									OPA-65R-LCUU-H8 w/ Mount Pipe	92	(3) C10857011 12' V-Boom KS24019-L112A	92
									OPA-65R-LCUU-H8 w/ Mount Pipe	92	Side Arm Mount [SO 302-1]	75 75
									OPA-65R-LCUU-H8 w/ Mount Pipe DTMABP7819VG12A	92 92		
										_		
			<u>_</u>						GRADE Fy	MATERIAL Fu	STRENGTH GRADE Fy	Fu
A572-50		A36	L2x2x1/8			_			A572-50 50 ksi	65 ksi	A36 36 ksi	58 ksi
A572-50						ł	<u>15.1 ft</u>		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower is also designed for increase in thickness with Deflections are based up 	rd County, Connec sure B to the TIA-22 mph basic wind in r a 50 mph basic w height.		
						٤	<u>15.1 ft</u>		 Tower is located in Hartfo Tower designed for Expos Tower designed for a 100 Tower is also designed fo increase in thickness with 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w height. on a 60 mph wind. vith Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons	
			51			ţ	5.1 ft		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w height. on a 60 mph wind. vith Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft	
	1/16		۲٦ 			1	15.1 ft		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w height. on a 60 mph wind. vith Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft ALL REACTIONS ARE FACTORED	sidered to
	1/4×1-3/4×3/16				@ 5.01042	0.8	5.1.ft		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w height. on a 60 mph wind. vith Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft ALL REACTIONS	sidered to
	L1-3/4x1-3/4x3/16				4 @ 5.01042		15.1 ft		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w height. on a 60 mph wind. vith Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft <i>ALL REACTIONS ARE FACTORED</i> <i>MAX. CORNER REACT</i> <i>DOWN: 42 K</i>	sidered to
	L1-3/4x1-3/4x3/16				6		5.1.ft		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w height. on a 60 mph wind. vith Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft ALL REACTIONS ARE FACTORED MAX. CORNER REACT DOWN: 42 K SHEAR: 6 K UPLIFT: -35 K	sidered to
	L1-3/4x3/16				6		5.1ft		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w height. on a 60 mph wind. vith Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft ALL REACTIONS ARE FACTORED MAX. CORNER REACT DOWN: 42 K SHEAR: 6 K UPLIFT: -35 K SHEAR: 6 K AXIAL	idered to IONS AT BASE: NT
	L1-3/4x1-3/4x3/16				6		5.1.ft		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w in height. on a 60 mph wind. with Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft ALL REACTIONS ARE FACTORED MAX. CORNER REACT DOWN: 42 K SHEAR: 6 K UPLIFT: -35 K SHEAR: 6 K AXIAL 39 K SHEAR: 6 K MOMEI 3 K TORQUE 0 kip-ft 50 mph WIND - 1.0000 in ICE	idered to IONS AT BASE: NT
	L1-3/4x1/3/4x3/16				6		5.1ft		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w in height. on a 60 mph wind. with Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft ALL REACTIONS ARE FACTORED MAX. CORNER REACT DOWN: 42 K SHEAR: 6 K UPLIFT: -35 K SHEAR: 6 K AXIAL 39 K SHEAR: 6 K AXIAL 39 K SHEAR 103 kip TORQUE 0 kip-ft 50 mph WIND - 1.0000 in ICE AXIAL 11 K	sidered to IONS AT BASE: NT I-ft
	L1-3/4x1-3/4x3/16				6	80	5.1 ft		 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w in height. on a 60 mph wind. with Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft ALL REACTIONS ARE FACTORED MAX. CORNER REACT DOWN: 42 K SHEAR: 6 K UPLIFT: -35 K SHEAR: 6 K AXIAL 39 K SHEAR: 6 K MOMEI 103 kip TORQUE 0 kip-ft 50 mph WIND - 1.0000 in ICE AXIAL 11 K SHEAR SHEAR MOMEI 11 K MOMEI	sidered to IONS AT BASE: NT -ft
	L1-3/4x1/3/4x3/16	Ge		t) 8.5625	4 @	0.8			 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w on a 60 mph wind. vith Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft ALL REACTIONS ARE FACTORED MAX. CORNER REACT DOWN: 42 K SHEAR: 6 K UPLIFT: -35 K SHEAR: 6 K AXIAL 39 K SHEAR: 6 K MOMEL 3 K TORQUE 0 kip-ft 50 mph WIND - 1.0000 in ICE AXIAL 11 K SHEAR	sidered to IONS AT BASE: NT -ft
	Diagonals L1-34x1-34x3/16	Diagonal Grade	Top Girts	Face Width (ft) 8:5625	6	80			 Tower is located in Hartfo Tower designed for Exposigned for a 100 Tower designed for a 100 Tower is also designed for increase in thickness with Deflections are based up Tower Structure Class II. Topographic Category 1 w 	rd County, Connec sure B to the TIA-2: mph basic wind in r a 50 mph basic w on a 60 mph wind. vith Crest Height of	icut. 22-G Standard. accordance with the TIA-222- ind with 1.00 in ice. Ice is cons 0.00 ft ALL REACTIONS ARE FACTORED MAX. CORNER REACT DOWN: 42 K SHEAR: 6 K UPLIFT: -35 K SHEAR: 6 K UPLIFT: -35 K SHEAR: 6 K AXIAL 39 K SHEAR: 6 K MOMEN 103 kip 50 mph WIND - 1.0000 in ICE AXIAL 11 K SHEAR: 7 MOMEN 11 K SHEAR: 7 SHEAR: 7 MOMEN 11 K SHEAR: 7 MOMEN 11 K SHEAR: 7 SHEAR:	sidered to IONS AT BASE: NT -ft

	GPD Group	^{Job:} BU #: 876328, WEST HAR	TFORD PARK	ING GARA
	520 South Main Street, Suite 2531	Project: 2017777.876328.17	-	
	Akron, Ohio 44311	Client: Crown Castle International, Inc.	Drawn by: B Darkow	App'd:
GPD Group		^{Code:} TIA-222-G	Date: 08/15/17	Scale: NTS
	FAX: (330) 572-2101	Path: \AKRN05.gpdco.com\TELECOM/Crown\876328\17\Rev. 0\tnx\8763	Dwg No. E-1	



Feed Line Distribution Chart

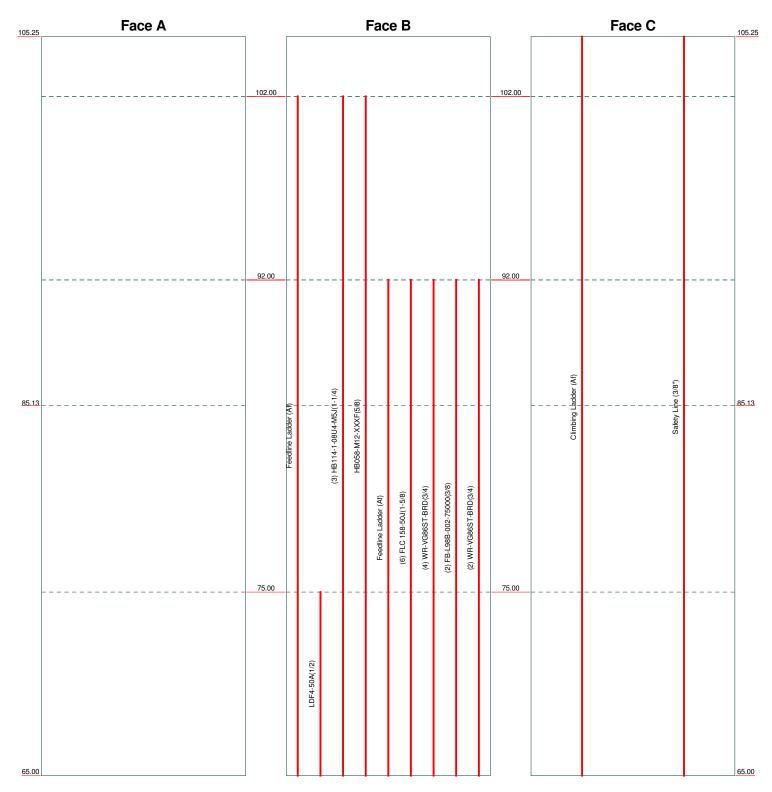
65' - 105'3'' App In Face

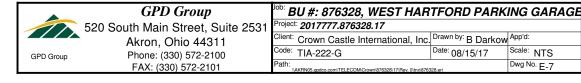
App Out Face

Flat

Round

Truss Leg





Tower Input Data

The main tower is a 3x free standing tower with an overall height of 105.25 ft above the ground line.

The base of the tower is set at an elevation of 65.00 ft above the ground line.

The face width of the tower is 6.56 ft at the top and 8.56 ft at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

- 1) Tower is located in Hartford County, Connecticut.
- 2) Basic wind speed of 100 mph.
- 3) Structure Class II.
- 4) Exposure Category B.
- Topographic Category 1. 5)
- Crest Height 0.00 ft. 6)
- 7) Nominal ice thickness of 1.0000 in.
- Ice thickness is considered to increase with height. 8)
- Ice density of 56 pcf. 9)
- 10) A wind speed of 50 mph is used in combination with ice.
- 11) Temperature drop of 50 °F.
- 12) Deflections calculated using a wind speed of 60 mph.
- 13) A non-linear (P-delta) analysis was used.
- 14) Pressures are calculated at each section.
- 15) Stress ratio used in tower member design is 1.
- 16) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

√ Include Bolts In Member Capacity

 $\sqrt{}$

Leg Bolts Are At Top Of Section $\sqrt{}$ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric

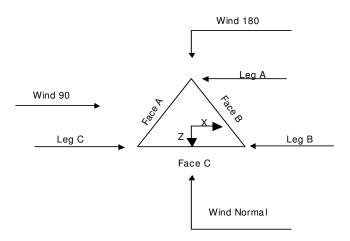
Autocalc Torque Arm Areas

Add IBC .6D+W Combination $\sqrt{}$ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder

Rules

- ng Forces s in FEA ression Allowable
- Check sist. Exemption Use TIA-222-G Tension Splice Exemption

Poles Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets



<u>Triangular Tower</u>

Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of	Section Length
					Sections	
	ft			ft		ft
T1	105.25-85.13			6.56	1	20.13
T2	85.13-65.00			6.56	1	20.13

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft		Panels		in	in
T1	105.25-85.13	4.03	X Brace	No	No	0.0000	0.0000
T2	85.13-65.00	5.01	X Brace	No	No	0.0000	1.0000

Tower Section Geometry (cont'd) Leg Diagonal Diagonal Tower Leg Leg Diagonal Elevation Type Size Grade Туре Size Grade ft T1 105.25-ROHN 2.5 STD A572-50 L1-1/2x1-1/2x1/8 Pipe Equal Angle A36 85.13 (50 ksi) (36 ksi) T2 85.13-65.00 ROHN 2.5 STD À572-50 L1-3/4x1-3/4x3/16 A36 Pipe Equal Angle (36 ksi) (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 105.25- 85.13	Equal Angle	L2x2x1/8	A36 (36 ksi)	Solid Round		A36 (36 ksi)
T2 85.13-65.00	Equal Angle	L2x2x1/8	A36 (36 ksi)	Solid Round		A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower	Gusset	Gusset	Gusset Grade	Adjust. Factor	Adjust.	Weight Mult.	Double Angle	Double Angle	Double Angle
Elevation	Area (per face)	Thickness		A_{f}	Factor A _r		Stitch Bolt Spacing	Stitch Bolt Spacing	Stitch Bolt Spacing
	0						Diagonals	Horizontals	Redundants
ft	fť	in					in	in	in
T1 105.25- 85.13	0.00	0.0000	A36 (36 ksi)	1	1	1	0.0000	0.0000	0.0000
T2 85.13- 65.00	0.00	0.0000	A36 (36 ksi)	1	1	1	0.0000	0.0000	0.0000

Tower Section Geometry (cont'd)

			K Factors'								
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
	Angles	Rounds		X	X	Х	Х	Х	Х	Х	
ft				Y	Y	Y	Y	Y	Y	Y	
T1 105.25-	Yes	Yes	1	1	1	1	1	1	1	1	
85.13				1	1	1	1	1	1	1	
T2 85.13-	Yes	Yes	1	1	1	1	1	1	1	1	
65.00				1	1	1	1	1	1	1	

¹Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-ofplane direction applied to the overall length.

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diago	onal	Top G	irt	Botton	n Girt	Mid C	Girt	Long Hor	rizontal	Short Ho	rizontal
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 105.25- 85.13	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	1	0.0000	1	0.0000	1
T2 85.13- 65.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	1	0.0000	1	0.0000	1

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal Top Girt		Bottom Girt		Mid Girt		Long Hori.	zontal	Short Horizontal			
		Bolt Size in	No.	Bolt Size	No.	Bolt Size in	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size in	No.
T1 105.25- 85.13	Flange	0.6250 A325N	4	0.5000 A325X	1	0.5000 A325X	1	0.0000 A325N	0	0.0000 A325N	0	0.0000 A325N	0	0.0000 A325N	0
T2 85.13- 65.00	Flange	0.0000 A325N	0	0.5000 A325X	1	0.5000 A325X	1	0.0000 A325N	0	0.0000 A325N	0	0.0000 A325N	0	0.0000 A325N	0

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face	Allow		Placement	Face	Lateral	#	#	Clear		Perimete	Weight
	or	Shield	Type		Offset	Offset		Per	Spacing		r	
	Leg			ft	in	(Frac FW)		Row	in	in	1	plf
											in	
Climbing	С	No	Af (CaAa)	105.25 - 65.00	-3.0000	0	1	1	3.8400	3.8400		4.81
Ladder (Af)	~	Nia	$\Lambda_{\rm H}$ (C = $\Lambda_{\rm H}$)		0,0000	0	-	4	0.0750	0.0750		0.00
Safety Line (3/8")	С	No	Ar (CaAa)	105.25 - 65.00	-3.0000	0	1	1	0.3750	0.3750		0.22
Feedline	В	No	Af (CaAa)	102.00 - 65.00	0.0000	-0.1	1	1	3.0000	3.0000		8.40
Ladder (Af)												
LDF4-	В	No	Ar (CaAa)	75.00 - 65.00	0.0000	-0.15	1	1	0.6250	0.6250		0.15
50A(1/2)	_											
HB114-1-	В	No	Ar (CaAa)	102.00 - 65.00	0.0000	-0.1	3	3	1.0000	1.5400		1.08
08U4-M5J(1-												
1/4)	-	NI-		100.00 05.00	0 0000	0.005			0.0400	0.0400		0.04
HB058-M12-	В	No	Ar (CaAa)	102.00 - 65.00	0.0000	-0.025	1	1	0.8400	0.8400		0.24
XXXF(5/8) Feedline	в	No	Af (CoAo)	92.00 - 65.00	0 0000	0.35	1	1	3.0000	3.0000		8.40
Ladder (Af)	D	INO	Af (CaAa)	92.00 - 05.00	0.0000	0.35	I	I	3.0000	3.0000		0.40
FLC 158-	В	No	Ar (CaAa)	92.00 - 65.00	0.0000	0.35	6	3	1.0000	2.0150		0.92
50J(1-5/8)	D	INO.	Ai (OaAa)	52.00 05.00	0.0000	0.00	0	0	1.0000	2.0150		0.52
WR-	В	No	Ar (CaAa)	92.00 - 65.00	0.0000	0.425	4	2	0.7950	0.7950		0.58
VG86ST-	_		/ ii (000 iu)	02.00 00.00	0.0000	0.120		-	017000	0000		0.00
BRD(3/4)												
FB-L98B-	В	No	Ar (CaAa)	92.00 - 65.00	3.5000	0.425	2	2	0.3937	0.3937		0.06
002-			()									
75000(3/8)												
WR-	В	No	Ar (CaAa)	92.00 - 65.00	0.0000	0.45	2	1	0.7950	0.7950		0.58
VG86ST-												
BRD(3/4)												

Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	A_R	A _F	$C_A A_A$	$C_A A_A$	Weight
Sectio n	Elevation ft		ft ²	ft ²	In Face ft ²	Out Face ft ²	ĸ
	105.25-85.13	Α	0.000	0.000	0.000	0.000	0.00
••		В	0.000	0.000	33.221	0.000	0.32
		С	0.000	0.000	13.635	0.000	0.10
T2	85.13-65.00	А	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	67.254	0.000	0.59
		С	0.000	0.000	13.635	0.000	0.10

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Sectio	Tower Elevation	Face or	lce Thickness	A _R	A _F	C _A A _A In Face	$C_A A_A$ Out Face	Weight
n	ft	Leg	in	ft ²	ft ²	ft ²	ft ²	K
T1	105.25-85.13	A	2.223	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	93.458	0.000	1.75
		С		0.000	0.000	31.534	0.000	0.66
T2	85.13-65.00	А	2.171	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	186.910	0.000	3.39
		С		0.000	0.000	31.114	0.000	0.64

Feed Line Center of Pressure

Section	Elevation	CP_X	CPz	CP_X	CPz
				Ice	Ice
	ft	in	in	in	in
T1	105.25-85.13	2.8747	0.3671	2.2143	0.4753
T2	85.13-65.00	5.1685	1.1013	3.9486	0.8385

			Disc	rete Tov	ver Loa	ds			
Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		$C_A A_A$ Front	C _A A _A Side	Weight
			ft ft ft	٥	ft		ft²	fť	К
APXVSPP18-C-A20 w/ Mount Pipe	A	From Leg	4.00 0.00 1.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	8.02 8.48 8.94	6.71 7.66 8.49	0.08 0.14 0.22
APXVSPP18-C-A20 w/ Mount Pipe	В	From Leg	4.00 0.00 1.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	8.02 8.48 8.94	6.71 7.66 8.49	0.08 0.14 0.22
APXV9ERR18-C-A20 w/ Mount Pipe	С	From Leg	4.00 0.00 1.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	8.50 9.16 9.79	7.18 8.46 9.60	0.08 0.15 0.23
APXVTM14-C-120 w/ Mount Pipe	A	From Leg	4.00 0.00 1.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	6.58 7.03 7.47	4.96 5.75 6.47	0.08 0.13 0.19
APXVTM14-C-120 w/ Mount Pipe	В	From Leg	4.00 0.00 1.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	6.58 7.03 7.47	4.96 5.75 6.47	0.08 0.13 0.19
APXVTM14-C-120 w/ Mount Pipe	С	From Leg	4.00 0.00 1.00	0.0000	102.00	No Ice 1/2" Ice 1" Ice	6.58 7.03 7.47	4.96 5.75 6.47	0.08 0.13 0.19
1900MHz RRH (65MHz)	A	From Leg	2.00 0.00 1.00	0.0000	102.00	No Ice 1/2" Ice	2.31 2.52 2.73	2.38 2.58 2.79	0.06 0.08 0.11
1900MHz RRH (65MHz)	В	From Leg	2.00 0.00 1.00	0.0000	102.00	1" Ice No Ice 1/2" Ice 1" Ice	2.31 2.52 2.73	2.38 2.58 2.79	0.06 0.08 0.11
1900MHz RRH (65MHz)	С	From Leg	2.00 0.00 1.00	0.0000	102.00	No Ice 1/2" Ice	2.31 2.52 2.73	2.38 2.58 2.79	0.06 0.08 0.11
800MHz 2X50W RRH	А	From Leg	2.00	0.0000	102.00	1" Ice No Ice	2.06	1.93	0.06

40.25 ft Self Support and Modified Parking Garage Structural Analysis Project Number 2017777.876328.17, Application 397668, Revision 0

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		$C_A A_A$ Front	C _A A _A Side	Weight
			ft ft ft	o	ft		ft²	fť	К
W/FILTER			0.00 0.00			1/2" Ice 1" Ice	2.24 2.43	2.11 2.29	0.09 0.11
800MHz 2X50W RRH W/FILTER	В	From Leg	2.00 0.00 0.00	0.0000	102.00	No Ice 1/2'' Ice	2.06 2.24 2.43	1.93 2.11 2.29	0.06 0.09 0.11
800MHz 2X50W RRH W/FILTER	С	From Leg	2.00 0.00 0.00	0.0000	102.00	1" Ice No Ice 1/2" Ice 1" Ice	2.06 2.24 2.43	1.93 2.11 2.29	0.06 0.09 0.11
TD-RRH8x20-25	A	From Leg	4.00 0.00 1.00	0.0000	102.00	No Ice 1/2'' Ice	4.05 4.30 4.56	1.53 1.71 1.90	0.07 0.10 0.13
TD-RRH8x20-25	В	From Leg	4.00 0.00 1.00	0.0000	102.00	1" Ice No Ice 1/2" Ice	4.05 4.30 4.56	1.53 1.71 1.90	0.07 0.10 0.13
TD-RRH8x20-25	С	From Leg	4.00 0.00 1.00	0.0000	102.00	1" Ice No Ice 1/2" Ice	4.05 4.30 4.56	1.53 1.71 1.90	0.07 0.10 0.13
8' x 2" Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	102.00	1" Ice No Ice 1/2" Ice	1.90 2.73 3.40	1.90 2.73 3.40	0.04 0.05 0.07
8' x 2" Mount Pipe	В	From Leg	4.00 0.00 0.00	0.0000	102.00	1" Ice No Ice 1/2" Ice	1.90 2.73 3.40	1.90 2.73 3.40	0.04 0.05 0.07
8' x 2" Mount Pipe	С	From Leg	4.00 0.00 0.00	0.0000	102.00	1" Ice No Ice 1/2" Ice	1.90 2.73 3.40	1.90 2.73 3.40	0.04 0.05 0.07
Sector Mount [SM 502-3]	В	None		0.0000	102.00	1" Ice No Ice 1/2" Ice	33.02 47.36 61.70	33.02 47.36 61.70	1.67 2.22 2.77
7770.00 w/ Mount Pipe	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	5.84 6.32 6.77	4.35 5.20 5.92	0.06 0.11 0.16
7770.00 w/ Mount Pipe	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	5.84 6.32 6.77	4.35 5.20 5.92	0.06 0.11 0.16
7770.00 w/ Mount Pipe	С	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	5.84 6.32 6.77	4.35 5.20 5.92	0.06 0.11 0.16
TPA-65R-LCUUUU-H8 w/ Mount Pipe	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	13.54 14.24 14.95	10.96 12.49 14.04	0.11 0.22 0.33
TPA-65R-LCUUUU-H8 w/ Mount Pipe	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	13.54 14.24 14.95	10.96 12.49 14.04	0.11 0.22 0.33
TPA-65R-LCUUUU-H8 w/ Mount Pipe	С	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	13.54 14.24 14.95	10.96 12.49 14.04	0.11 0.22 0.33
OPA-65R-LCUU-H8 w/ Mount Pipe	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	12.98 13.67 14.36	9.32 10.79 12.24	0.12 0.21 0.32

40.25 ft Self Support and Modified Parking Garage Structural Analysis Project Number 2017777.876328.17, Application 397668, Revision 0

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		$C_A A_A$ Front	C _A A _A Side	Weight
			ft ft ft	o	ft		ťť	fť	К
OPA-65R-LCUU-H8 w/ Mount Pipe	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	12.98 13.67 14.36	9.32 10.79 12.24	0.12 0.21 0.32
OPA-65R-LCUU-H8 w/ Mount Pipe	С	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	12.98 13.67 14.36	9.32 10.79 12.24	0.12 0.21 0.32
DTMABP7819VG12A	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	0.98 1.10 1.23	0.34 0.42 0.51	0.02 0.03 0.04
DTMABP7819VG12A	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	0.98 1.10 1.23	0.34 0.42 0.51	0.02 0.03 0.04
DTMABP7819VG12A	С	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	0.98 1.10 1.23	0.34 0.42 0.51	0.02 0.03 0.04
7020.00	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	0.10 0.15 0.20	0.17 0.24 0.31	0.00 0.01 0.01
7020.00	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	0.10 0.15 0.20	0.17 0.24 0.31	0.00 0.01 0.01
7020.00	С	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	0.10 0.15 0.20	0.17 0.24 0.31	0.00 0.01 0.01
RRUS-11	A	From Leg	4.00 0.00 0.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	2.78 2.99 3.21	1.19 1.33 1.49	0.05 0.07 0.09
RRUS-11	В	From Leg	4.00 0.00 0.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	2.78 2.99 3.21	1.19 1.33 1.49	0.05 0.07 0.09
RRUS-11	С	From Leg	4.00 0.00 0.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	2.78 2.99 3.21	1.19 1.33 1.49	0.05 0.07 0.09
RRUS 12	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	3.15 3.36 3.59	1.29 1.44 1.60	0.06 0.08 0.11
RRUS 12	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	3.15 3.36 3.59	1.29 1.44 1.60	0.06 0.08 0.11
RRUS 12	С	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	3.15 3.36 3.59	1.29 1.44 1.60	0.06 0.08 0.11
RRUS 32 B2	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	2.73 2.95 3.18	1.67 1.86 2.05	0.05 0.07 0.10
RRUS 32 B2	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	2.73 2.95 3.18	1.67 1.86 2.05	0.05 0.07 0.10
RRUS 32 B2	С	From Leg	4.00	0.0000	92.00	1" Ice No Ice	2.73	1.67	0.05

tnxTower Report - version 7.0.7.0

August 15, 2017 CCI BU No 876328 Page 16

40.25 ft Self Support and Modified Parking Garage Structural Analysis Project Number 2017777.876328.17, Application 397668, Revision 0

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		$C_A A_A$ Front	C _A A _A Side	Weight
			ft ft ft	o	ft		fť	fť	К
			0.00 -3.00			1/2" Ice 1" Ice	2.95 3.18	1.86 2.05	0.07 0.10
RRUS 32 B30	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	No Ice 1/2" Ice 1" Ice	2.69 2.91 3.14	1.57 1.76 1.95	0.06 0.08 0.10
RRUS 32 B30	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	No Ice 1/2'' Ice	2.69 2.91 3.14	1.57 1.76 1.95	0.06 0.08 0.10
RRUS 32 B30	С	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	2.69 2.91 3.14	1.57 1.76 1.95	0.06 0.08 0.10
(2) RRUS E2 B29	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	3.15 3.36 3.59	1.29 1.44 1.60	0.06 0.08 0.11
RRUS E2 B29	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	3.15 3.36 3.59	1.29 1.44 1.60	0.06 0.08 0.11
RRUS 32 B66	В	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	2.74 2.96 3.19	1.67 1.86 2.05	0.05 0.07 0.10
(2) RRUS 32 B66	С	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	2.74 2.96 3.19	1.67 1.86 2.05	0.05 0.07 0.10
(2) DC6-48-60-18-8F Surge Suppression Unit	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	0.92 1.46 1.64	0.92 1.46 1.64	0.02 0.04 0.06
DC6-48-60-18-8F Surge Suppression Unit	A	From Leg	4.00 0.00 -3.00	0.0000	92.00	1" Ice No Ice 1/2" Ice	0.92 1.46 1.64	0.92 1.46 1.64	0.02 0.04 0.06
(3) C10857011 12' V-Boom	В	None		0.0000	92.00	1" Ice No Ice 1/2" Ice	33.64 48.17 62.70	33.64 48.17 62.70	1.50 2.00 2.51
KS24019-L112A	A	From Leg	4.00 0.00 2.00	0.0000	75.00	1" Ice No Ice 1/2" Ice	0.14 0.20 0.26	0.14 0.20 0.26	0.01 0.01 0.01
Side Arm Mount [SO 302- 1]	A	From Leg	2.00 0.00 0.00	0.0000	75.00	1" Ice No Ice 1/2" Ice 1" Ice	1.67 2.51 3.35	3.27 4.99 6.71	0.06 0.09 0.12

Load Combinations

Comb.	Description
No.	
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 lce+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 lce+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 lce+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 lce+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 lce+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 lce+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 lce+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Tower Deflections - Service Wind

Section No.	Elevation	Horz. Deflection	Gov. Load	Tilt	Twist
	ft	in	Comb.	0	0
T1	105.25 - 85.125	0.268	43	0.0387	0.0015
T2	85.125 - 65	0.093	43	0.0316	0.0011

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
102.00	APXVSPP18-C-A20 w/ Mount Pipe	43	0.237	0.0385	0.0014	155732
92.00	7770.00 w/ Mount Pipe	43	0.145	0.0363	0.0013	58767
75.00	KS24019-L112A	43	0.038	0.0180	0.0006	77866

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
T1	105.25 - 85.125	1.175	10	0.1687	0.0065
T2	85.125 - 65	0.406	10	0.1379	0.0048

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	<i>Tilt</i> ∘	<i>Twist</i> ∘	Radius of Curvature
ft		Comb.	in	0	0	ft
102.00	APXVSPP18-C-A20 w/ Mount Pipe	10	1.037	0.1679	0.0064	35884
92.00	7770.00 w/ Mount Pipe	10	0.635	0.1584	0.0057	13541
75.00	KS24019-L112A	10	0.168	0.0785	0.0027	17942

Bolt Design Data

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of	Maximum Load per	Allowable Load	Ratio Load	Allowable Ratio	Criteria
	ft			in	Bolts	Bolt K	K	Allowable		
T1	105.25	Leg	A325N	0.6250	4	2.44	20.71	0.118	1	Bolt Tension
		Diagonal	A325X	0.5000	1	3.35	3.81	0.879	1	Member Block Shear
		Top Girt	A325X	0.5000	1	0.35	4.13	0.084	1	Member Bearing
T2	85.125	Diagonal	A325X	0.5000	1	3.09	6.20	0.498	1	Member Bearing
		Top Girt	A325X	0.5000	1	0.19	4.13	0.047	1	Member Bearing

Compression Checks

	Leg Design Data (Compression)									
Section No.	Elevation	Size	L	Lu	Kl/r	Α	Pu	φ P _n	Ratio Pu	
	ft		ft	ft		in²	K	K	ϕP_n	
T1	105.25 - 85.125	ROHN 2.5 STD	20.13	4.02	51.0 K=1.00	1.7040	-15.06	63.41	0.237 1	
T2	85.125 - 65	ROHN 2.5 STD	20.16	5.02	63.6 K=1.00	1.7040	-39.11	57.07	0.685 1	

¹ P_u / ϕP_n controls

	Diagonal Design Data (Compression)										
Section No.	Elevation	Size	L	Lu	Kl/r	A	P _u	φPn	Ratio Pu		
	ft		ft	ft		in ²	K	K	φ P _n		
T1	105.25 - 85.125	L1-1/2x1-1/2x1/8	7.70	3.60	146.0 K=1.00	0.3594	-3.40	3.81	0.894 1		
T2	85.125 - 65	L1-3/4x1-3/4x3/16	9.70	4.75	166.0 K=1.00	0.6211	-3.12	5.09	0.613 ¹		

¹ P_u / ϕP_n controls

		Top Gir	t Desig	n Dat	a (Coi	mpres	sion)		
Section No.	Elevation	Size	L	Lu	Kl/r	Α	P _u	φ P _n	Ratio P _u
	ft		ft	ft		in²	K	K	• <i>P</i> _n
T1	105.25 - 85.125	L2x2x1/8	6.56	6.11	184.6 K=1.00	0.4844	-0.35	3.21	0.110 1
T2	85.125 - 65	L2x2x1/8	6.56	6.11	184.6 K=1.00	0.4844	-0.15	3.21	0.048 ¹

¹ P_u / ϕP_n controls

Tension Checks

Leg Design Data (Tension)									
Section No.	Elevation	Size	L	L _u	Kl/r	А	P _u	φ P _n	Ratio P _u
	ft		ft	ft		in²	K	K	ϕP_n
T1	105.25 - 85.125	ROHN 2.5 STD	20.13	4.02	51.0	1.7040	9.74	76.68	0.127 1
T2	85.125 - 65	ROHN 2.5 STD	20.16	0.08	1.1	1.7040	34.99	76.68	0.456 ¹

¹ P_u / ϕP_n controls

Diagonal Design Data (Tension) Section Elevation Size L Lu Kl/r Α P_u ϕP_n Ratio No. P_u in² ft ft ft Κ Κ ϕP_n 0.365¹ L1-1/2x1-1/2x1/8 7.70 T1 105.25 -3.60 95.7 0.2109 3.35 9.18 85.125 T2 85.125 - 65 L1-3/4x1-3/4x3/16 9.70 4.75 108.5 0.3779 3.09 16.44 0.188 1

¹ P_u / ϕP_n controls

Top Girt Design Data (Tension)

Section	Elevation	Size	L	Lu	Kl/r	A	Pu	ϕP_n	Ratio
No.									P_u
	ft		ft	ft		in²	K	K	ϕP_n
T1	105.25 -	L2x2x1/8	6.56	6.11	121.2	0.3047	0.35	13.25	0.026 1
	85.125								
T2	85.125 - 65	L2x2x1/8	6.56	6.11	121.2	0.3047	0.19	13.25	0.015 ¹

¹ P_u / ϕP_n controls

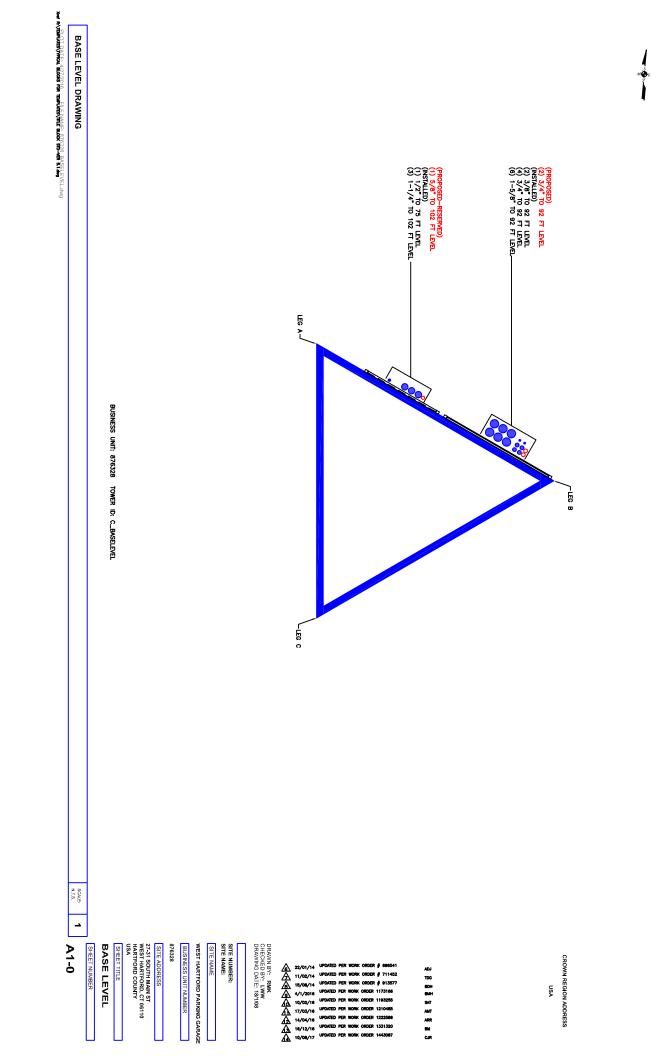
Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	øP _{allow} K	% Capacity	Pass Fail
T1	105.25 - 85.125	Leg	ROHN 2.5 STD	3	-15.06	63.41	23.7	Pass
T2	85.125 - 65	Leg	ROHN 2.5 STD	38	-39.11	57.07	68.5	Pass
T1	105.25 - 85.125	Diagonal	L1-1/2x1-1/2x1/8	9	-3.40	3.81	89.4	Pass
T2	85.125 - 65	Diagonal	L1-3/4x1-3/4x3/16	46	-3.12	5.09	61.3	Pass
T1	105.25 - 85.125	Top Girt	L2x2x1/8	4	-0.35	3.21	11.0	Pass
T2	85.125 - 65	Top Girt	L2x2x1/8	41	-0.15	3.21	4.8	Pass

Summary	ELC:	Load Case 7
Leg (T2)	68.5	Pass
Diagonal (T1)	89.4	Pass
Top Girt (T1)	11.0	Pass
Bolt Checks	87.9	Pass
Rating =	89.4	Pass

APPENDIX B

BASE LEVEL DRAWING



APPENDIX C

ADDITIONAL CALCULATIONS

		FOUNDATION ANALYSIS WORKSHEET				
	Client:	Crown Castle International, Inc.	Job No.:	2	017777.876328	.17
	Site Name:	WEST HARTFORD PARKING GARAGE	Sheet No:	1	Of	1
	Site BU #:	876328	Made By:	BD2	Date:	8/15/2017
GPD GROUP	Location:	Hartford County, Connecticut				
	Loading Type:	Wind	Code:	G		

Sources

The modified tnxTower design reactions were obtained from the design by GPD (Project #: 2015777.876328.08, dated 6/3/2015)

Uplift: 44.01 K	
Compression: 52.04 K	
Shear <mark>7.94</mark> K	

G-Code Conversion Factor: 1.35

Modified tnxTower Design Read	tions (Co	onverte	d to G-Code)
Uplift:	59.41	К	
Compression:	70.26	К	
Shear	10.72	К	

TNX Output Reactio	ons (G-Code)	
Uplift:	34.71	К
Compression:	41.96	К
Shear	6.35	К

FOUNDATION CAPACITY

Uplift Capacity =	TNX Output Modified Design Reactions	=	58.4%
<u>Compression Capacity</u> =	TNX Output Modified Design Reactions	=	59.7%
<u>Shear Capacity</u> =	TNX Output Modified Design Reactions	=	59.2%

	PROJECT INFORMATION				
SCOPE OF WORK:	ITEMS TO BE MOUNTED ON THE EXISTING LATTICE TOWER: •(1) AT&T ANTENNA (OPA-65R-LCUU-H8) @ POSITION 2 (TYP. OF 1 BETA SECTOR, TOTAL OF 2). •(1) AT&T ANTENNA (OPA-65R-LCUU-H8) @ POSITION 1 (TOTAL OF	FOR ALPHA &			
	 SECTOR) (1) AT&T RRUS-E2 (700) (TYP. OF 1 PER SECTOR, TOTAL OF 3). (1) AT&T RRUS-12 (850) (TYP. OF 1 PER SECTOR, TOTAL OF 3). (1) AT&T RRUS-32 B66 (AWS) (TYP. OF 1 PER SECTOR, TOTAL OF 3). 				at&t
	• INSTALL (1) DC6-48-60-18-8F SQUID ALONG WITH (2) DC TRUNKS (1) ALARM CABLE.	, (1) FIBER AND			
	 NEW JUMPER CABLES: COAX JUMPERS (6) PER SECTOR FROM EACH (TOTAL OF 18). NEW FIBER JUMPERS: FIBER JUMPERS (3) FROM THE SQUID TO EAC (TOTAL OF 9). 			SITE NUMBE	R: CT5843
	ITEMS TO BE MOUNTED INSIDE EXISTING EQUIPMENT: • INSTALL (1) FIBER MANAGEMENT BOX AND (1) OUTDOOR RATED DCG	ON ICE BRIDGE			TUADTEOD
	POST. • REPLACE EXISTING BBU WITH (2) 5216-IDL2 AND ADD (2) XMU'S IN PURCELL CABINET. • INSTALL (1) FIDER TRAY IN EXISTING DURGELL	N EXISTING		SITE NAME: WES	HARIFOR
	INSTALL (1) FIBER TRAY IN EXISTING PURCELL. INSTALL (6) 25AMP BREAKERS AND (3) 30AMP BREAKERS TO OUTD	DOR POWER PLANT.			
	ITEMS TO REMAIN: •(6) ANTENNAS, (9) RRU'S, (6) COAX CABLES, (4) DC POWER CABLE & (2) SURGE ARRESTOR.	S, (1) FIBER RUNS	PRUJ	ECT: LTE 4C,5C,	
	 THE 1ST SQUID INSTALLED WILL BE ALARMED TO THE LOWEST BAND INSTALLED) RRH/RRU ON THE ALPHA SECTOR. IN THE EVENT THE ALAI BE CONNECTED TO ALPHA IT WILL BE ACCEPTABLE TO ALARM TO THE SECTOR ON AN EXCEPTION BASIS. 2ND SQUID INSTALLED WILL BE ALARMED TO THE LOWEST BAND (O RRH/RRU ON THE BETA SECTOR. 	RM CABLE CANNOT CLOSEST PHYSICAL R FIRST INSTALLED)	BU# 87632	8 - WEST HARTF	ORD PARKIN
	3. 3RD SQUID INSTALLED WILL BE ALARMED TO THE LOWEST BAND (O RRH/RRU ON THE GAMMA SECTOR.	R FIRST INSTALLED)	VICINITY MAP		
		DIRECTIONS TO S			
SITE ADDRESS:	29 SOUTH MAIN STREET WEST HARTFORD, CT 06107	LEFT ONTO CAPIT ONTO I-91 N VI/	AL BLVD. THEN 0.27 MILES. TURN LEFT ON $$ THE RAMP ON THE LEFT TOWARD HARTFOF	TO WEST ST. THEN 0.16 MILES. MERGE RD. THEN 8.79 MILES. MERGE ONTO I-84	1. THIS DOCUMENT IS THE DUPLICATION OR USE WI AND USE BY GOVERNMEI
ATITUDE:	41.760190' N 41' 45' 36.7" N	EXIT, EXIT 43, TO	XIT 32A ON THE LEFT TOWARD WATERBURY. WARD W HARTFORD CENTER. THEN 0.50 MII THE 1ST LEFT ONTO TROUT BROOK DR. T	ES. TURN RIGHT ONTO PARK RD. THEN	AUTHORIZED REGULATOR
ONGITUDE:	72.743190' W 72' 44' 35.5" W		N AVE. 0.22 MILES. TURN LEFT ONTO S MA		2. THE FACILITY IS AN UNM ACCESSED BY TRAINED 1
YPE OF SITE:	LATTICE TOWER/OUTDOOR EQUIPMENT				NOT REQUIRE ANY WATER REGULATIONS REQUIRING
OWER HEIGHT:	103'−0"± 89'−0"±	A SE FE	- ANB		3. CONTRACTOR SHALL VER
CURRENT USE:	59 - U ± TELECOMMUNICATIONS FACILITY	N	Farmington		AND SHALL IMMEDIATELY PROCEEDING WITH THE V
PROPOSED USE:	TELECOMMUNICATIONS FACILITY			isham Rd - Va	
	DRAWING INDEX				
SHEET NO. DESC	RIPTION	REV.			
T—1 TITLE	SHEET	1		ENP	
GN-1 GENE	RAL NOTES	1	auto		
A-1 ROOF	& EQUIPMENT PLAN	1			
A-2 ANTEI	NNA LAYOUTS				
A-3 ELEVA	ATION	1 PA	PROJECT		L'éger
A-4 DETAI			SITE	Management of the sectory of	
					CALL T
	MATIC AND NOTES				
RF-1 PLUM	BING DIAGRAM	Bd	ATTACIO MARCA		
G—1 GROU	INDING DETAILS	1			
	ASTLE SITE NAME: WEST HARTFORD PARKING	GARAGE			
	<u>ASTLE SITE #.</u> 070320				
		SITE NUMBER: CT58			
		SITE NUMBER: CT58 SITE NAME: WEST HART CCI SITE # 876328	FORD		ED FOR CONSTRUCTION SG ED FOR REVIEW SG
	CROWN	SITE NAME: WEST HART	FORD	A 09/05/17 ISSU	

ALL CONSTRUCTION ACTIVITIES ARE TO BE COMPLETED DIRECTLY THROUGH CROWN. CONTRACTOR MUST HAVE CONSTRUCTION PO AND NTP FROM CROWN DIRECT IN ORDER TO BEGIN. PLEASE CONTACT THE CROWN CONSTRUCTION MANAGER: TAMMY NOSEK 518-860-7063

RD

GRADE

ING GARAGE

GENERAL NOTES

THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF AT&T. ANY E WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED. DUPLICATION NMENT AGENCIES FOR THE PURPOSES OF CONDUCTING THEIR LAWFULLY TORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.

UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES VATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY RING PUBLIC ACCESS PER ADA REQUIREMENTS.

VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE TELY NOTIFY THE AT&T REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE HE WORK OR BE RESPONSIBLE FOR SAME.

72 H	IOURS	
BEFORE CALL TOLL FREE 1-	ALL YOU DIG 800–922–4455 811 Service Alert	
		┥
SG AT DJC	AT&T	_
SG AT DIG	TITLE SHEET (LTE-4C,5C,6C)	
BY CHK APP'D		EV
DRAWN BY: SG	CT5843 T-1	1

GROUNDING NOTES

- THE SUBCONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE-SPECIFIC (UL, LPI, OR NFPA) LIGHTING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE SUBCONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE CONTRACTOR FOR RESOLUTION.
- 2. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- 3. THE SUBCONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR NEW GROUND ELECTRODE SYSTEMS. THE SUBCONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- EACH BTS CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, 6 AWG STRANDED COPPER OR LARGER FOR INDOOR BTS 2 AWG STRANDED COPPER FOR OUTDOOR BTS.
- EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO 8. GROUND BAR.
- ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS
- 10. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED. TO THE GROUND RING IN ACCORDANCE WITH THE NEC.
- 11. METAL CONDUIT SHALL BE MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH 6 AWS COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 12. ALL NEW STRUCTURES WITH A FOUNDATION AND/OR FOOTING HAVING 20 FT. OR MORE OF 1/2 IN. OR GREATER ELECTRICALLY CONDUCTIVE REINFORCING STEEL MUST HAVE IT BONDED TO THE GROUND RING USING AN EXOTHERMIC WELD CONNECTION USING #2 AWG SOLID BARE TINNED COPPER GROUND WIRE, PER NEC 250.50

GENERAL NOTES

1. FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR - CROWN CASTLE SUBCONTRACTOR - GENERAL CONTRACTOR (CONSTRUCTION) OWNER - AT&T MOBILITY

- 2. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.
- 3. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS
- 4. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.
- 5. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS
- 6. "KITTING LIST" SUPPLIED WITH THE BID PACKAGE IDENTIFIES ITEMS THAT WILL BE SUPPLIED BY CONTRACTOR. ITEMS NOT INCLUDED IN THE BILL OF MATERIALS AND KITTING LIST SHALL BE SUPPLIED BY THE SUBCONTRACTOR
- 7. THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 8. IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION SPACE FOR APPROVAL BY THE CONTRACTOR
- 9. SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR.
- 10. THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER
- 11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 12. SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION.
- 13. ALL CONCRETE REPAIR WORK SHALL BE DONE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301.

- FOR CONSTRUCTION OF AT&T SITES."
- AFTER MIDNIGHT
- EXPOSURE LEVELS.
- 20. APPLICABLE BUILDING CODES: CONTRACT AWARD SHALL GOVERN THE DESIGN.

SUBCONTRACTOR'S WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS

FOR ANY CONFLICTS BETWEEN SECTIONS OF LISTED CODES AND STANDARDS REGARDING MATERIAL, METHODS OF CONSTRUCTION, OR OTHER REQUIREMENTS, THE MOST RESTRICTIVE AND A SPECIFIC REQUIREMENT, THE SPECIFIC REQUIREMENT SHALL GOVERN.

				ABBREVIATIONS	
	AGL	ABOVE GRADE LEVEL	EQ	EQUAL REQ	REC
	AWG	AMERICAN WIRE GAUGE	GC	GENERAL CONTRACTOR RF	RAE
	BBU	BATTERY BACKUP UNIT	GRC	GALVANIZED RIGID CONDUIT TBD	ТО
	BTCW	BARE TINNED SOLID COPPER WIRE	MGB	MASTER GROUND BAR TBR	то
	BGR	BURIED GROUND RING	MIN	MINIMUM TBRR	to Ref
	BTS	BASE TRANSCEIVER STATION	Ρ	PROPOSED TYP	TYF
	Е	EXISTING	NTS	NOT TO SCALE UG	UNI
	EGB	EQUIPMENT GROUND BAR	RAD	RADIATION CENTER LINE (ANTENNA) VIF	VEF
	EGR	EQUIPMENT GROUND RING	REF	REFERENCE	
É		a de a de			\square



TEL: (978) 557-555 FAX: (978) 336-558

5 BEECHWOOD DRIVE

NORTH ANDOVER, MA 01845



SITE NUMBER: CT5843 SITE NAME: WEST HARTFORD CCI SITE # 876328 29 SOUTH MAIN STREET WEST HARTFORD, CT 06107 HARTFORD COUNTY



EQ EQUAL REQ REQUIRED GC GENERAL CONTRACTOR RF RADIO FREQUENCY GRC GALVANIZED RIGID CONDUIT TBD TO BE DETERMINED MGB MASTER GROUND BAR TBR TO BE REMOVED MIN MINIMUM TBRR TO BE REMOVED AND I P PROPOSED TYP TYPICAL NTS NOT TO SCALE UG UNDER GROUND RAD RADIATION CENTER LINE VIF VERIFY IN FIELD I 10/19/17 ISSUED FOR CONSTRUCTION SG AT 0 10/10/17 ISSUED FOR REVIEW SG AT A 09/05/17 ISSUED FOR REVIEW SG AT NO. DATE REVISIONS BY CHK SCALE AS SHOWN DESIGNED BY: AT DRAWN BY: SG			ABE	BREVIAT	IONS]]]]]]]]]
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	- F				REVISIONS	DRAW			P'D	111	/ONAL	FUN	Ń	1/1	, \ \	CT5843					

14. ANY NEW CONCRETE NEEDED FOR THE CONSTRUCTION SHALL BE AIR-ENTRAINED AND SHALL HAVE 4000 PSI STRENGTH AT 28 DAYS. ALL CONCRETE WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS.

15. ALL STRUCTURAL STEEL WORK SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS. ALL STRUCTURAL STEEL SHALL BE ASTM A36 (Fy = 36 ksi) UNLESS OTHERWISE NOTED. PIPES SHALL BE ASTM A53 TYPE E (Fy = 36 ks). ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED. TOUCHUP ALL SCRATCHES AND OTHER MARKS IN THE FIELD AFTER STEEL IS ERECTED USING A COMPATIBLE ZINC RICH PAINT.

16. CONSTRUCTION SHALL COMPLY WITH SPECIFICATIONS AND "GENERAL CONSTRUCTION SERVICES

17. SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.

18. THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS

19. SINCE THE CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE ADVISED TO BE WORN TO ALERT OF ANY DANGEROUS

SUBCONTRACTOR'S WORK SHALL COMPLY WITH ALL APPLICABLE NATIONAL. STATE. AND LOCAL CODES AS ADOPTED BY THE LOCAL AUTHORITY HAVING JURISDICTION (AHJ) FOR THE LOCATION. THE EDITION OF THE AHJ ADOPTED CODES AND STANDARDS IN EFFECT ON THE DATE OF BUILDING CODE: IBC 2012 WITH 2016 CT BUILDING CODE AMENDMENTS ELECTRICAL CODE: REFER TO ELECTRICAL DRAWINGS LIGHTENING CODE: REFER TO ELECTRICAL DRAWINGS

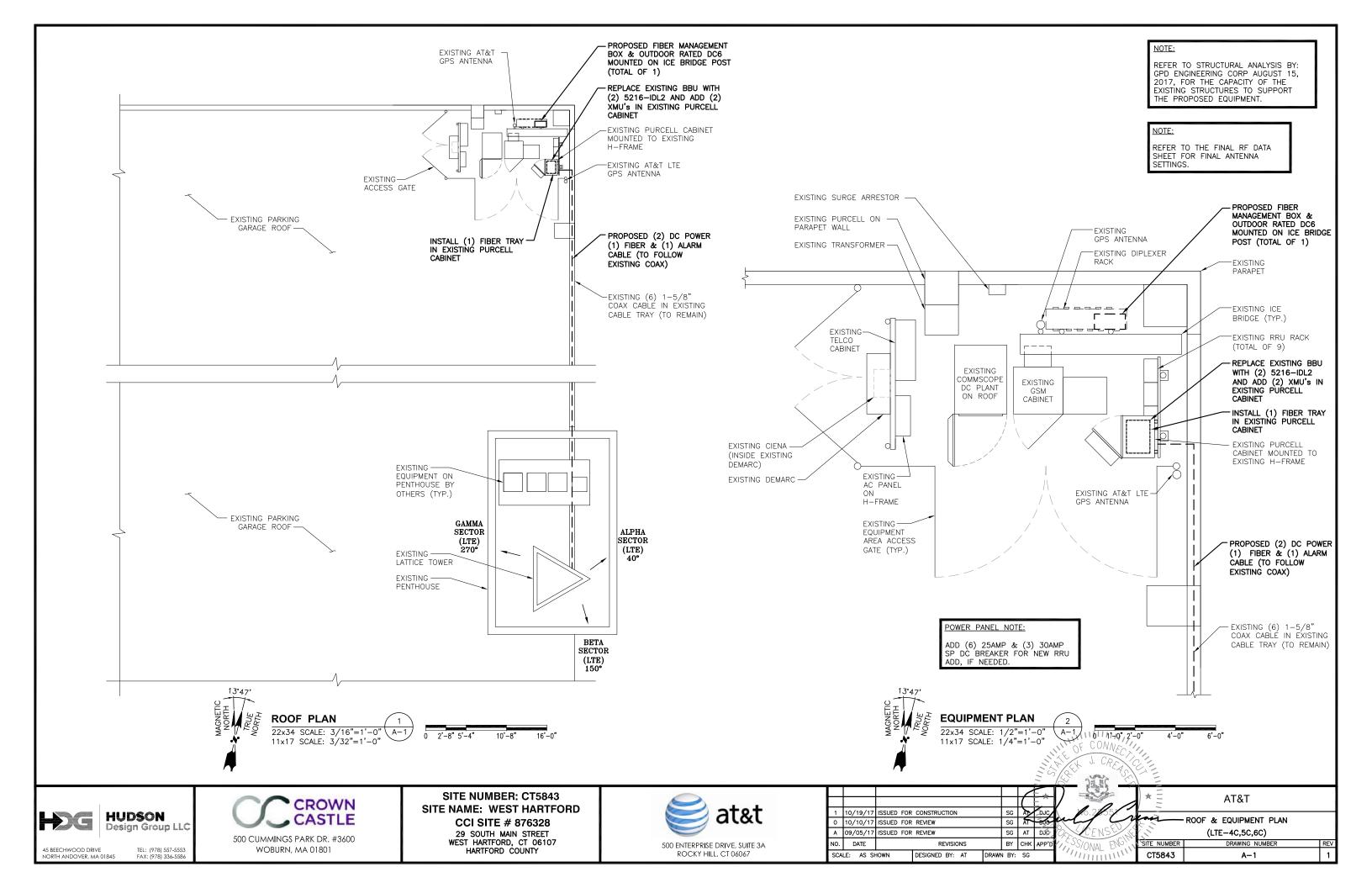
AMERICAN CONCRETE INSTITUTE (ACI) 318; BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE:

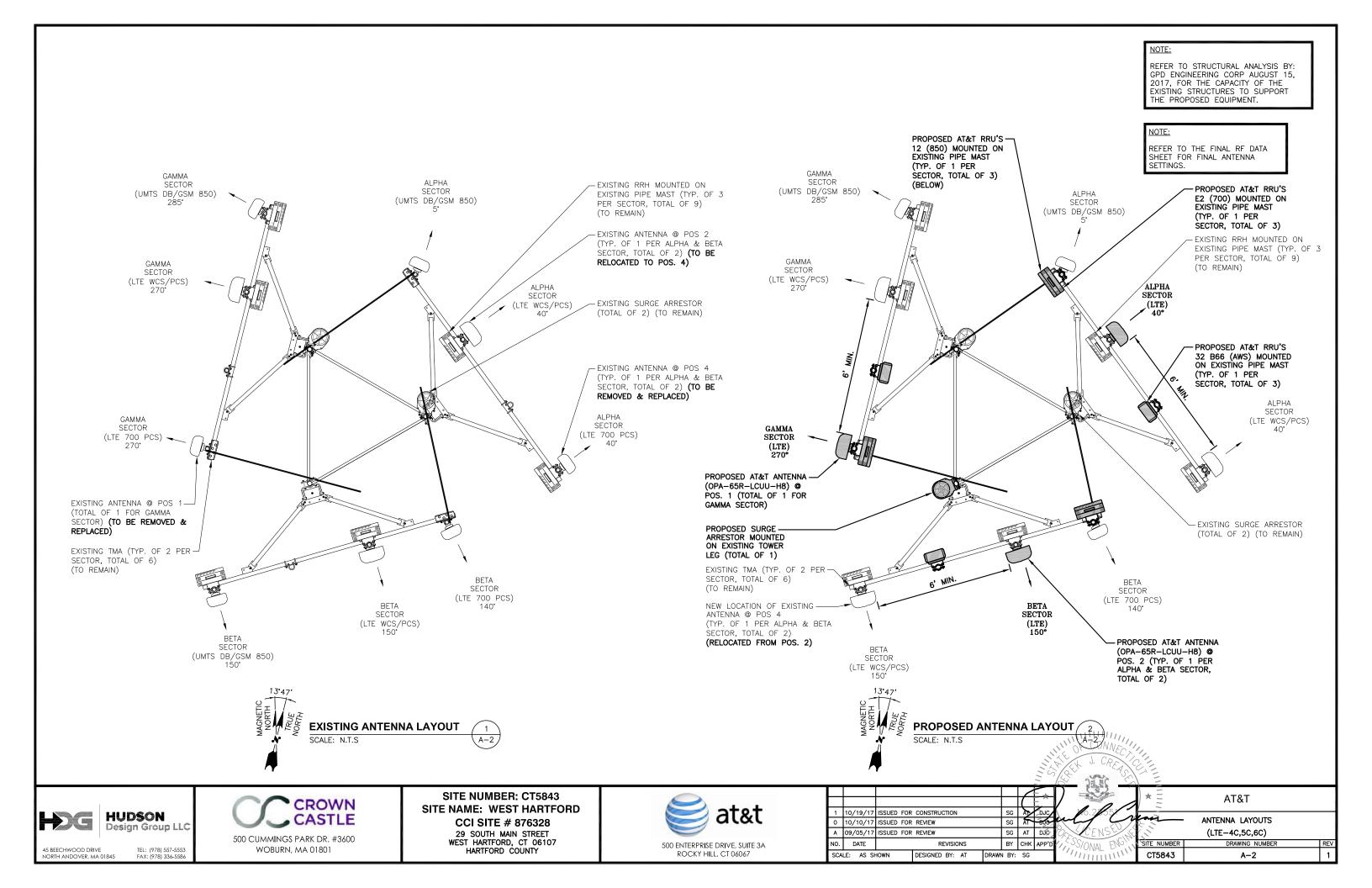
AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC)

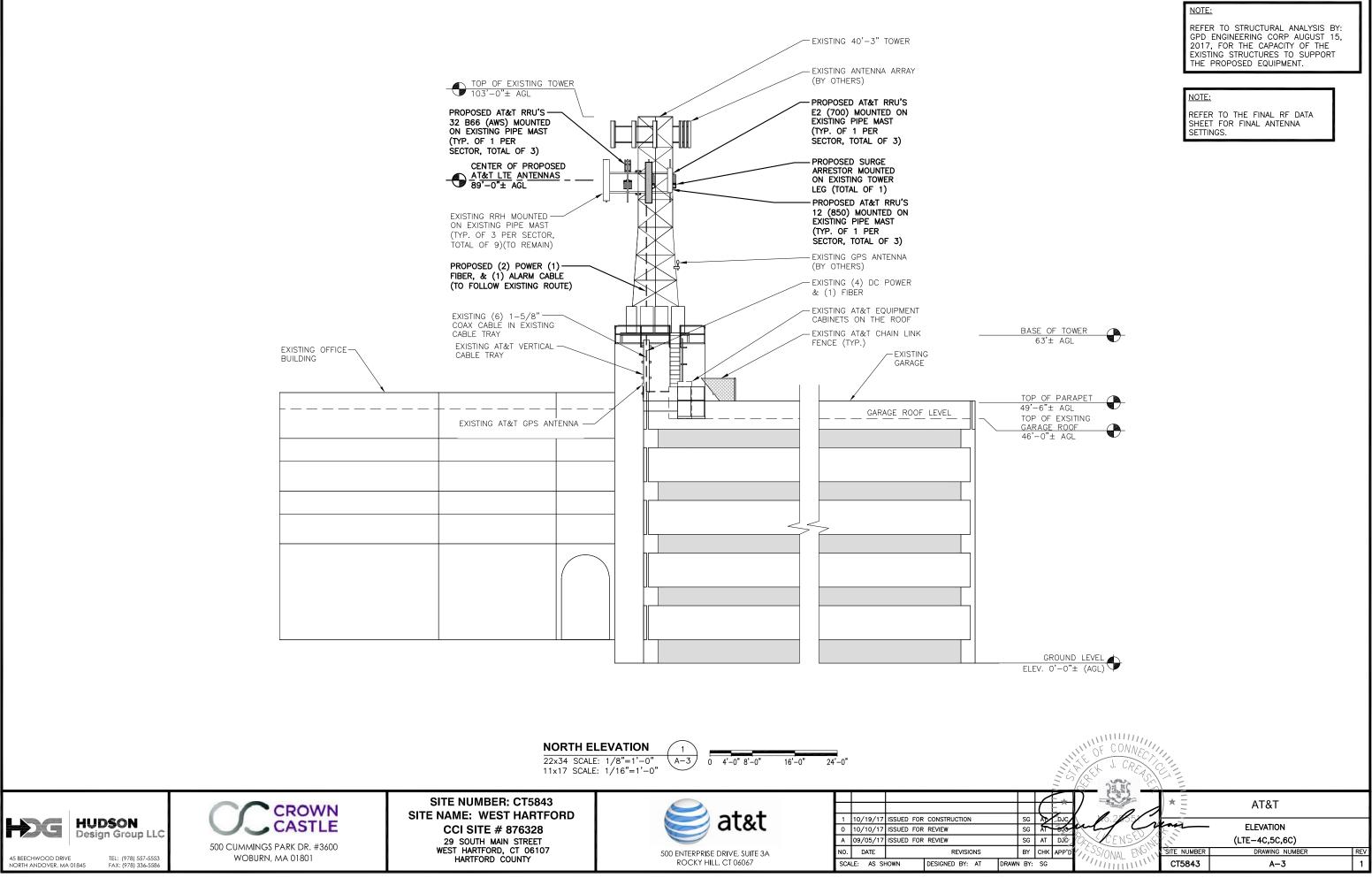
MANUAL OF STEEL CONSTRUCTION, ASD, FOURTEENTH EDITION;

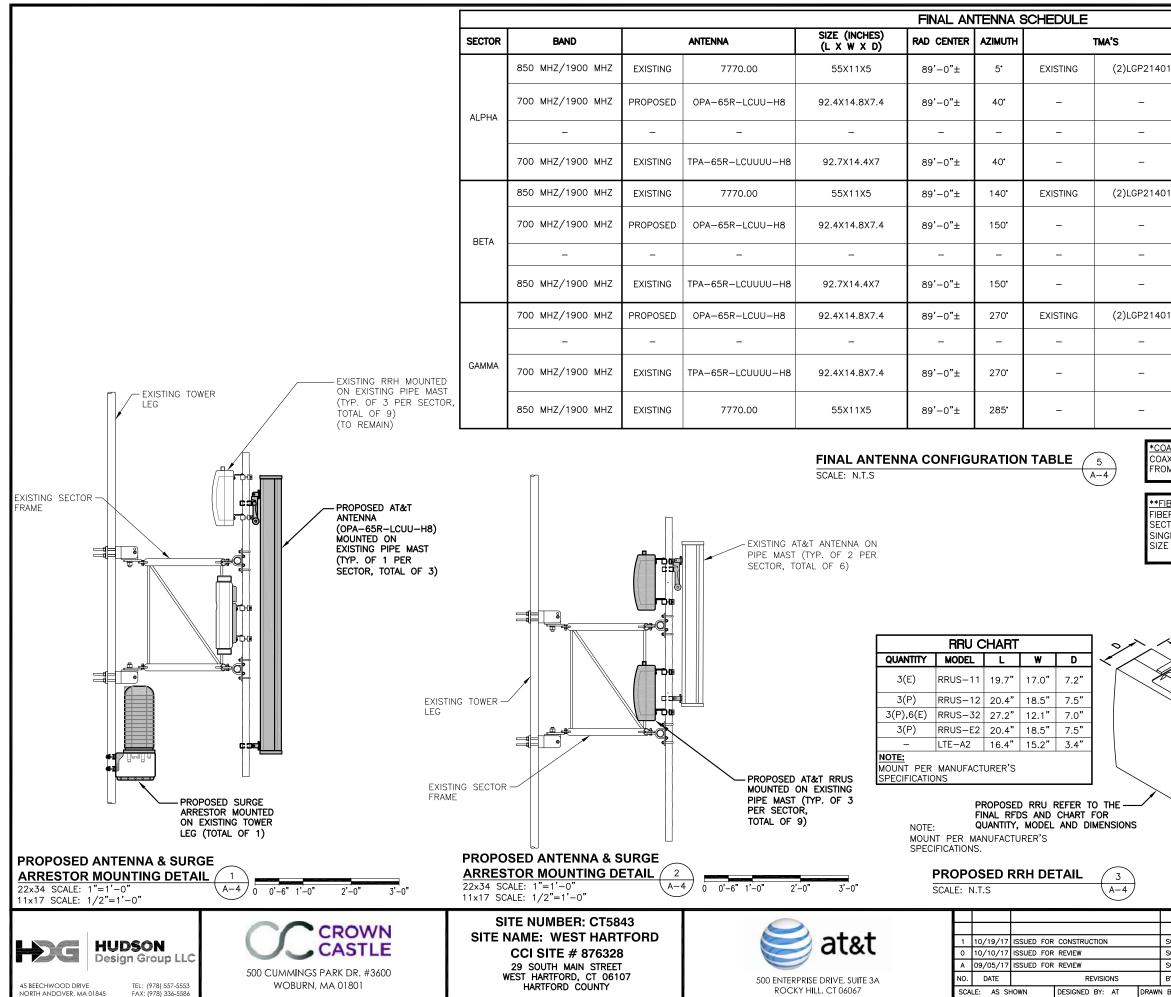
TELECOMMUNICATIONS INDUSTRY ASSOCIATION (TIA) 222-G, STRUCTURAL STANDARDS FOR STEEL

EQUIPMENT AND ANTENNA SUPPORTING STRUCTURES; REFER TO FLECTRICAL DRAWINGS FOR SPECIFIC FLECTRICAL STANDARDS









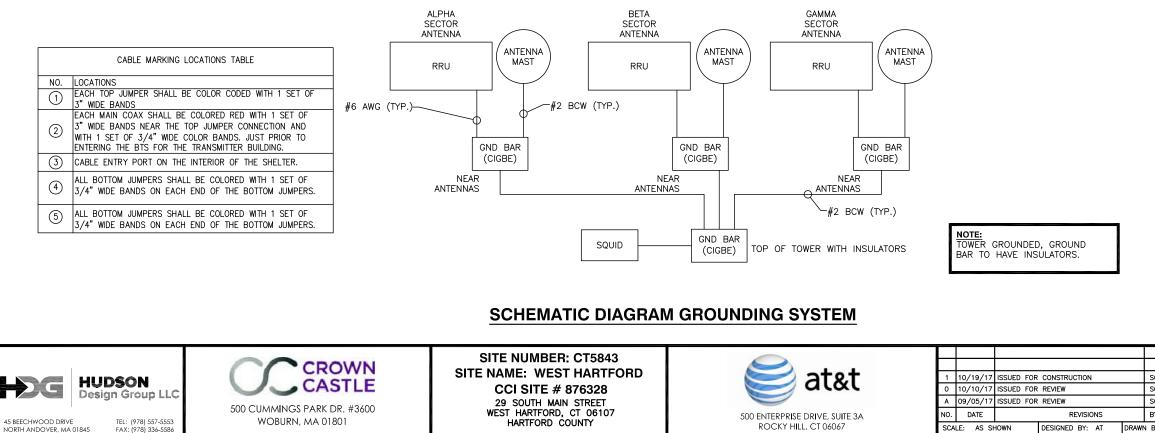
	R	RU'S	SIZE (INCHES) (L X W X D)	COAX JUMPERS	FIBER JUMPERS				
)1	-	_	_	-	-				
	PROPOSED PROPOSED EXISTING	RRUS-E2(700) RRUS-12(850) RRUS-32B2 (PCS)	20.4X18.5X7.5 20.4X18.5X7.5 –	2* 2* —	1** 1** —				
	-	-	-	-	-				
	EXISTING PROPOSED EXISTING	RRUS-11(700) RRUS-32B66(AWS) RRUS-32(WCS)	_ 27.2X12.1X7 27.2X12.1X7	_ 2* _	 1** 				
)1	-	-	-	-	-				
	PROPOSED PROPOSED EXISTING	RRUS-E2(700) RRUS-12(850) RRUS-32B2 (PCS)	20.4X18.5X7.5 20.4X18.5X7.5 —	2* 2* -	1** 1** —				
	-	-	-	_	-				
	EXISTING PROPOSED EXISTING	RRUS-11(700) RRUS-32B66(AWS) RRUS-32(WCS)		_ 2* _	_ 1** _				
)1	-	-	-	-	-				
	_	_	_	-	-				
	PROPOSED PROPOSED EXISTING	RRUS-E2(700) RRUS-12(850) RRUS-32B2 (PCS)	20.4X18.5X7.5 20.4X18.5X7.5 –	2* 2* -	1** 1** —				
	EXISTING PROPOSED EXISTING	RRUS-11(700) RRUS-32B66(AWS) RRUS-32(WCS)	_ 27.2X12.1X7 _	_ 2* _	_ 1** _				
AX JUMPERS (6) PER SECTOR M THE RRU, TOTAL OF 18 IBER JUMPER NOTE: ER JUMPERS (3) PER SECTOR CTOR, TOTAL OF 9 TO RRU. GLE PAIR POWER CABLE #12 E FROM SQUID TO EACH RRU. SUBJECT: CTOR THE CAPACITY OF THE EXISTING STRUCTURES TO SUPPORT THE PROPOSED EQUIPMENT.									
PROPOSED SURGE SUPPRESSOR MODEL NUMBER: DC6-48-60-0-8F DIMENSIONS: H24.0"x9.7"ø									
SCALE: N.T.S SCALE: N.T.S									

COAX COLOR CODING AND IDENTIFICATION NOTES

- 1. SECTOR ORIENTATION/AZIMUTH WILL VARY FROM REGION TO REGION AND IS SITE SPECIFIC. REFER TO RF REPORT FOR EACH SITE TO DETERMINE THE ANTENNA LOCATÍON AND FUNCTION OF EACH TOWER SECTOR FACE.
- 2. THE ANTENNA SYSTEM COAX SHALL BE LABELED WITH VINYL TAPE EXCEPT IN LOCATIONS WHERE ENVIRONMENTAL CONDITIONS CAUSE PHYSICAL DAMAGE, THE PHYSICAL TAGS ARE PREFERRED.
- 3. THE STANDARD IS BASED ON 8 COLORED TAPES-RED, BLUE, GREEN, YELLOW, ORANGE, BROWN, WHITE, AND VIOLET. THESE TAPES MUST BE 3/4" WIDE & UV RESISTANT SUCH AS SCOTCH 35 VINYL ELECTRICAL COLOR CODING TAPE AND SHOULD BE READILY AVAILABLE TO THE ELECTRICIAN OR SUBCONTRACTOR ON SITE.
- 4. USING COLOR BANDS ON THE CABLES, MARK ALL RF CABLE BY SECTOR AND CABLE NUMBER AS SHOWN ON "CABLE MARKING COLOR CONVENTION TABLE"
- 5. WHEN AN EXISTING COAXIAL LINE THAT IS INTENDED TO BE A SHARED LINE BETWEEN GSM/3G AND IS-136/TDMA IS ENCOUNTERED, THE SUBCONTRACTOR SHALL REMOVE THE EXISTING COLOR CODING SCHEME AND REPLACE IT WITH THE COLOR CODING AND TAGGING STANDARD THAT IS OUTLINED IN THE CURRENT VERSION OF ND-00027. IN THE ABSENCE OF AN EXISTING COLOR CODING AND TAGGING SCHEME, OR WHEN INSTALLING PROPOSED COAXIAL CABLES, THE GUIDELINE SHALL BE IMPLEMENTED AT THE SITE REGARDLESS OF TECHNOLOGY.
- 6. ALL COLOR CODE TAPE SHALL BE 3M-35 AND SHALL BE INSTALLED USING A MINIMUM OF 3 WRAPS OF TAPE AND SHALL BE NEATLY TRIMMED AND SMOOTHED OUT SO AS TO AVOID UNRAVELING.
- 7. ALL COLOR BANDS INSTALLED AT THE TOP OF THE TOWER SHALL BE A MINIMUM OF 3" WIDE, AND SHALL HAVE A MINIMUM OF 3/4" OF SPACE BETWEEN EACH COLOR.
- 8. ALL COLOR CODES SHALL BE INSTALLED SO AS TO ALIGN NEATLY WITH ONE ANOTHER FROM SIDE TO SIDE.
- 9. IF EXISTING CABLES AT THE SITE ALREADY HAVE A COLOR CODING SCHEME AND THEY ARE NOT INTENDED TO BE REUSED OR SHARED WITH THE GSM TECHNOLOGY, THE EXISTING COLOR CODING SCHEME SHALL REMAIN UNTOUCHED.

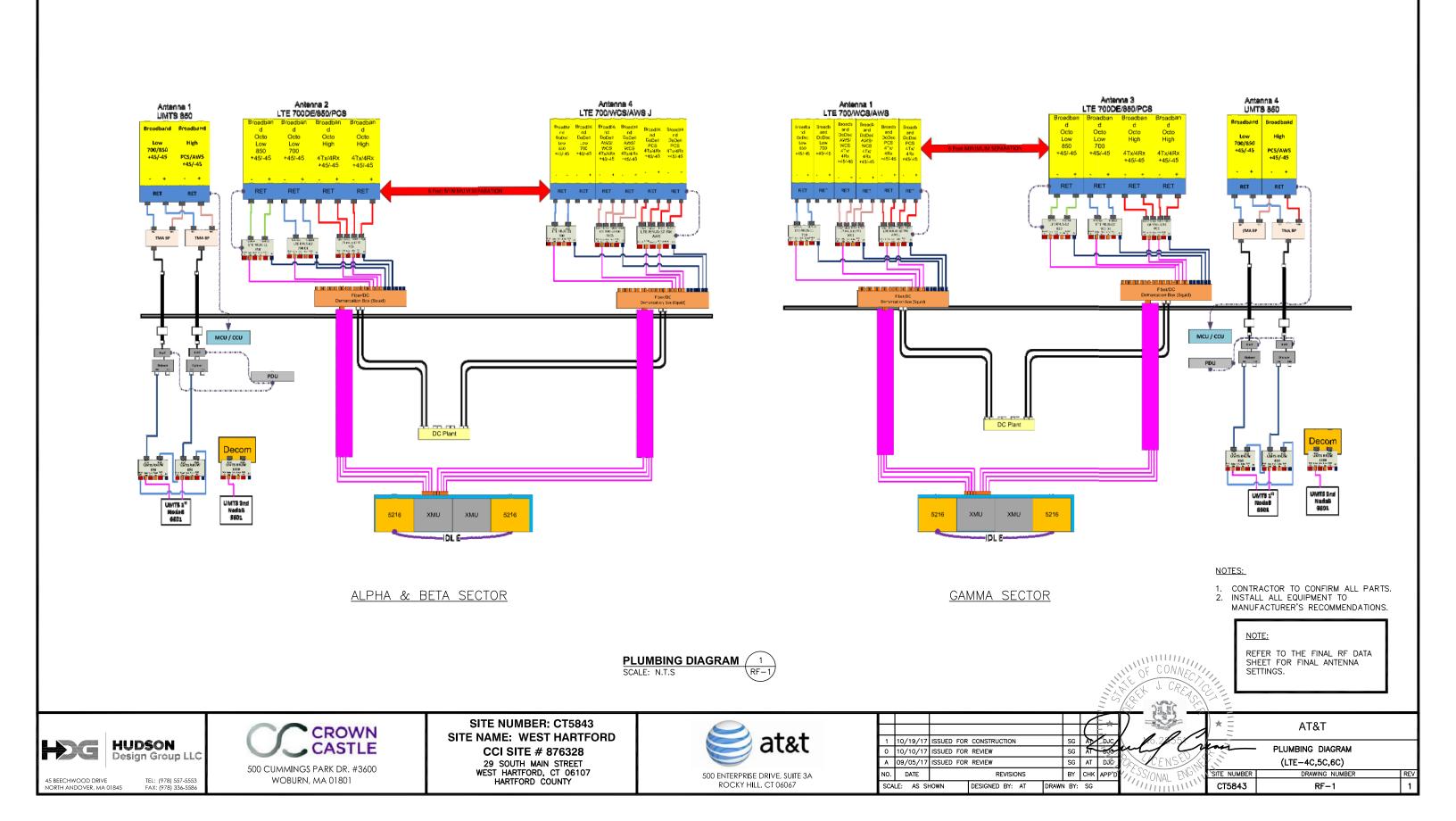
CABLE MARKING TAGS

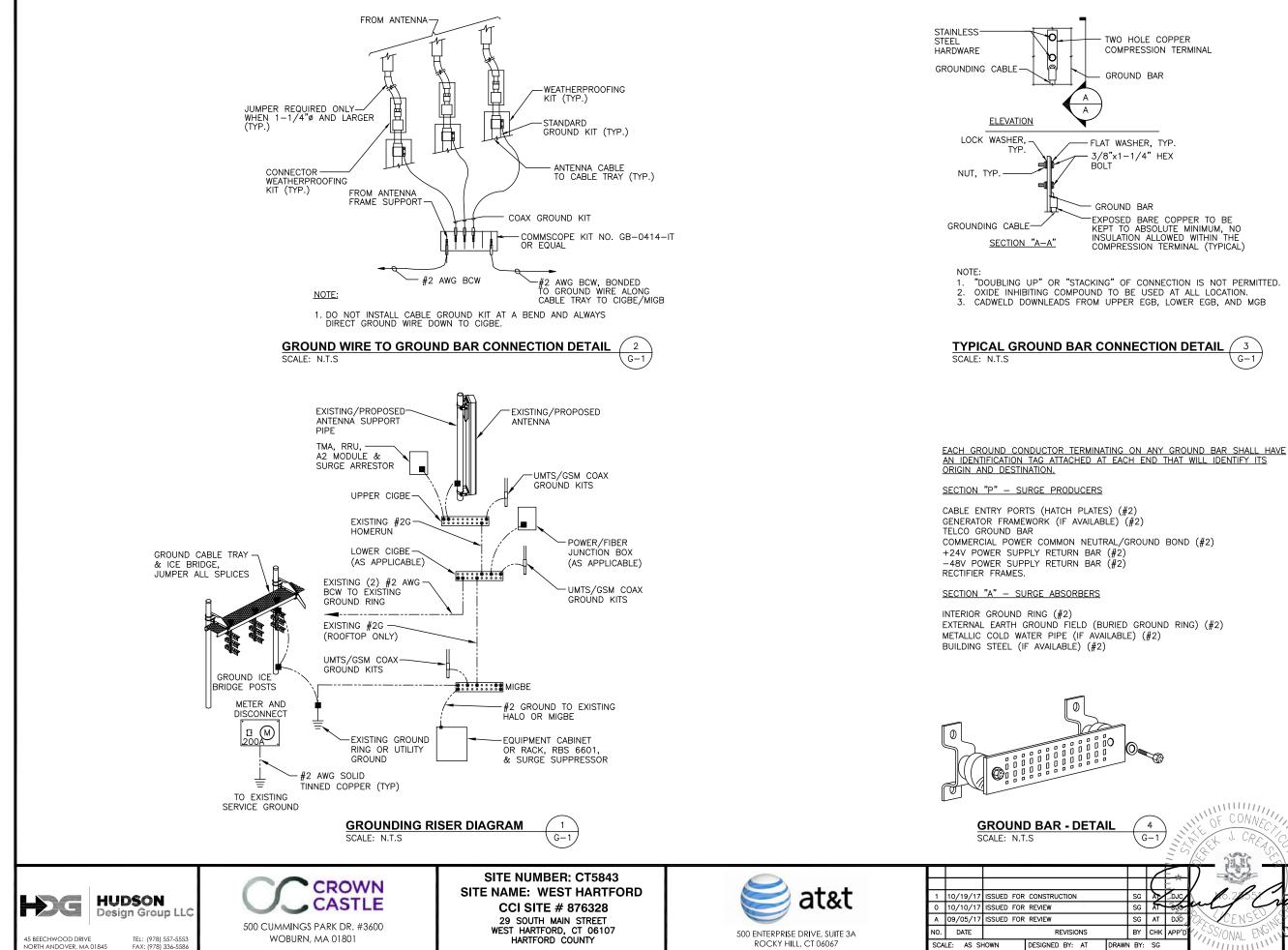
WHEN USING THE ALTERNATIVE LABELING METHOD, EACH RF CABLE SHALL BE IDENTIFIED WITH A METAL ID TAG MADE OF STAINLESS STEEL OR BRASS, THE TAG SHALL BE 1 1/2" IN DIAMETER WITH 1/4" STAMPED LETTERS AND NUMBERS INDICATING THE SECTOR, ANTENNA POSITION, AND CABLE NUMBER. THE ID MARKING LOCATIONS SHOULD BE AS PER CABLING MARKING LOCATIONS TABLE. THE TAG SHOULD BE ATTACHED WITH CORROSION PROOF WIRE AROUND THE CABLE AT THE SAME LOCATION AS DEFINED ABOVE. THE TAG SHOULD BE LABELED AS SHOWN ON THE GSM AND UMTS LINE TAG DETAIL.



NORTH ANDOVER, MA 01845

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BY	снк	APP'D	SSIONAL ENG	SITE NUMBER	DRAWING NUMBER	REV
BY:	SG			CT5843	A-5	1





TWO HOLE COPPER COMPRESSION TERMINAL

-EXPOSED BARE COPPER TO BE KEPT TO ABSOLUTE MINIMUM, NO INSULATION ALLOWED WITHIN THE COMPRESSION TERMINAL (TYPICAL)

G = 1

WINDE CONNE CONNE 4 G-1 AT&T SG AT D GROUNDING DETAILS SG AT SG AT DJC (LTE-4C,5C,6C) BY CHK APP DRAWING NUMBE TE NUMBE CT5843 G-1