

March 21, 2014

Melanie A. Bachman
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

## RE: Sprint PCS-Exempt Modification - Crown Site BU: 806370 Sprint PCS Site ID: CT03XC091 <br> Located at: 570 New Park Avenue, West Hartford, CT 06110

Dear Ms. Bachman:
This letter and exhibits are submitted on behalf of Sprint PCS (Sprint). Sprint is making modifications to certain existing sites in its Connecticut system in order to implement their 2.5 GHz LTE technology. Please accept this letter and exhibits as notification, pursuant to § 16$50 j-73$ of the Regulations of Connecticut State Agencies ("R.C.S.A."), of construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to The Honorable Scott Slifka, Mayor for the Town of West Hartford.

Sprint plans to modify the existing wireless communications facility owned by Crown Castle and located at 570 New Park Avenue, West Hartford, CT 06110. Attached are a compound plan and elevation depicting the planned changes (Exhibit-1), and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration (Exhibit-2). Also included is a power density table report reflecting the modification to Sprint's operations at the site (Exhibit-3).

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes ("C.G.S.") § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in the R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing tower. Sprint's additional antennas will be located at the same elevation on the existing tower.
2. There will be no proposed modifications to the ground and no extension of boundaries.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more.
4. A Structural Modification Report confirming that the tower and foundation can support Sprint's proposed modifications is included as Exhibit-2.
5. The operation of the additional antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table report for Sprint's modified facility is included as Exhibit-3.

For the foregoing reasons, Sprint respectfully submits the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Donna Neal.

Sincerely,


Jeff Barbadora
Real Estate Specialist

## Enclosures

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes
Tab 2: Exhibit-2: Structural Modification Report
Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: The Honorable Scott Slifka, Mayor<br>Town of West Hartford<br>50 South Main Street<br>West Hartford, CT 06107



THESE OUTLINE SPECIFICATIONS IN CONJUNCTION WITH THE SPRINT STANDARD CONSTRUCTION SPECIFICATIONS, INCLUDING CONTRACT DOCUMENTS SECTION 01 - 100 - SCOPE OF WORK
PART 1 - GENERAL


1.2 RELATED DOCUMENTS:
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2. GR-7-COCORE GENERIC REOURIMEMTS FOR THE PHYSICAL DESIIN AND
3. GR-1089 CORE, ELECTROMAGNETC COMPADBUTY AND EECCRICAL SAEET
4. NATONAL FRE PROTECTON ASSOCLATON CODES AND STANDARD (NFPA) INCLUDNG NFPA 70
(LFE SAEEY CODE).
5. AMERICAN SOCIETY FOR TESTNG OF MATERLLLS (ASTM)
6. INSTTUTE OF ELECTRONIC AND ELECTRICAL ENGINEERS (IEEE)
7. AMERICAN CONCRETE INSTIUTE (ACI)
8. AMERICAN WIRE PRODUCERS ASSOCIATON (AWPA)
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13. ERICK INDUSTRY ASSOCITION (BA)
14. AMERICAN WELDING SOCIETY (AWS)
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16. SHET METAL AND AR CONDTIONING CONTRACTORS' NATONAL ASSOCIATION
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Sprint
C580 Sprint Parkway
Oveland Park, Kansas 66251

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site adoress: $\quad$ NEW Park ave WEST HARTFORD, CT 06110

SPRINT SPECIFICATIONS
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6. Install "H-frames", cabinets and shelitrs as indicated.
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8. ACCOMPUSH REQUIRED MODIICATON OF EXISTNG FACIITESS.
9. PROVIDE ANIENNA SUPPORT STRUCTURE FOUNDATONS.
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15. ALL CORRESPONDENCE AND PRELMMINARY CONSTRUCTION REPORTS.
16. PROUECT PROGRESS REPORTS.
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4. AL ECOUPMENT AND MATERALS SO IDEMIFED ON THE CONSTRUCTON
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7. ANTENA AND COAX SWESP TESTS PER EXHIBT: ANIENNA TRANSMISSION UNE
8. GROUNDING AT ANIENNA MASTS FOR GPS AND ANTENNAS
g. All OTHER TESTS REQUIRED gY COMPANY OR JURISICICTON.
3.3 REQUIRED INSPECTONS
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SPRINT SPECIFICATIONS
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6. CONCREIE TESTNG / SAMPLES
7. PLACING of ANCHOR BoLts IN TOWER FOUNDATON
8. BUILDING/WATER TANK FROM ROAD FOR TENANT IMPROVEMENTS OR COMMENTS.
9. SHELIER FOUNDATON--FORMS AND STEEL BEFORE POURING.
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11. Coax Cable eniky into shetre.
12. PLATORM MECHANICAL CONNECTONS TO TOWER/MONOPOLE.
13. ROOFTOP PRE AND POST CONSTRUCTION PHOTOS TO INCLUDE PENETRATONS
14. PHotos of tower top coax line color coding and color cooing at
15. photos of al appropriate company or regulatory signage.
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Sprint
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27. antenna ground bar and equipment ground bar.
28. Adomonal grounding points on towers above 200'.
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1. cable tray and/or waveguide arioge
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4. master bus aar.
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10. ANIENNA AND MAST GROUNDING.
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SPRINT SPECIFICATIONS







RFS Hybrillex riser cable schedule


RFS HYBRIFLEX JUMPER CABLE SCHEDULE


## 

EIGER ONLY



ALU 2.5 ALU SCENARIO 1


RAN WIRING DIAGRAM

| Sprint 6580 Sprint ParkwayOverland Park, Kansas 662 |  |
| :---: | :---: |
| INFINIGY8 W33 Walervliet Shaker RAlbany, NY 12205Office \# (518) $690-0790$ J08 RUMEER $351-000$ |  |
| $W$ Crown |  |
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| 570 NEW PARK AVE. WEST HARTFORD, CT 06110 |  |
| plumbing diagram |  |
| A- |  |




Date: January 13, 2014
Patrick Byrum
Crown Castle
3530 Toringdon Way Suite 300
Charlotte, NC 28277
(704) 405-6532

GPD Group
520 S. Main St., Suite 2531
Akron, OH 44311
(614) 859-1607
dpalkovic@gpdgroup.com

## Subject:

## Carrier Designation:

## Crown Castle Designation:

Engineering Firm Designation:

## Site Data:

## Structural Analysis Report

## Sprint PCS Co-Locate <br> Carrier Site Number: <br> Carrier Site Name:

Crown Castle BU Number:
Crown Castle Site Name:
Crown Castle JDE Job Number: 252989
Crown Castle Work Order Number: 696085
Crown Castle Application Number: 208261 Rev. 3
GPD Group Project Number: $\quad 2014777.806370 .01$
570 NEW PARK AVENUE, WEST HARTFORD, Hartford County, CT Latitude $41^{\circ} 44$ ' $10.5^{\prime \prime}$, Longitude -72으' $14.2^{\prime \prime}$
150 Foot - Valmont Monopole Tower

Dear Patrick Byrum,
GPD Group is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 607755, in accordance with application 208261, revision 3.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

> LC7: Existing + Reserved + Proposed Equipment
> Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

Sufficient Capacity

The analysis has been performed in accordance with the TIA/EIA-222-F standard and 2005 CT State Building Code based upon a wind speed of 80 mph fastest mile.

We at GPD Group appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:


John N. Kabak, P.E.
Connecticut \#: PEN. 0028336

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tnxTower Output

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Additional Calculations

## 1) INTRODUCTION

The existing monopole has three major sections connected by slip joints. It has 12 sides and is evenly tapered from 61.5" (flat-flat) at the base to 26.19 " (flat-flat) at the top. The structure is galvanized and has no tower lighting.

This tower is a 150 ft Monopole tower designed by VALMONT in May of 1990 . The tower was originally designed for a wind speed of 125 mph per EIA-222-D.

## 2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 80 mph with no ice, 28 mph with 1 inch ice thickness (in accordance with ASCE 7 ice conditions) and 50 mph under service loads.

Table 1 - Proposed Antenna and Cable Information

| Mounting <br> Level (ft) | Center <br> Line <br> Elevation <br> (ft) | Number <br> of <br> Antennas | Antenna <br> Manufacturer | Antenna Model | Number <br> of Feed <br> Lines | Feed <br> Size (in) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 134.0 | 135.0 | 3 | Alcatel Lucent | TD-RRH8x20-25 | 1 | $5 / 8$ |
|  |  | 3 | RFS Celwave | APXVTM14-C-120 | 1 |  |

Notes:

1) See Appendix B for the proposed coax layout.

Table 2 - Existing and Reserved Antenna and Cable Information
$\left.\begin{array}{|c|c||c|c|c|c|c|c|}\hline \begin{array}{c}\text { Mounting } \\ \text { Level (ft) }\end{array} & \begin{array}{c}\text { Center } \\ \text { Line } \\ \text { Elevation } \\ \text { (ft) }\end{array} & \begin{array}{c}\text { Number } \\ \text { of } \\ \text { Antennas }\end{array} & \begin{array}{c}\text { Antenna } \\ \text { Manufacturer }\end{array} & \text { Antenna Model } & \begin{array}{c}\text { Number } \\ \text { of Feed } \\ \text { Lines }\end{array} & \begin{array}{c}\text { Feed } \\ \text { Size (in) }\end{array} & \text { Sote }\end{array}\right\}$

Notes:

1) Abandoned Equipment
2) Reserved Equipment

Table 3 - Design Antenna and Cable Information

| Mounting <br> Level (ft) | Center <br> Line <br> Elevation <br> (ft) | Number <br> of <br> Antennas | Antenna <br> Manufacturer | Antenna Model | Number <br> of Feed <br> Lines | Feed <br> Line <br> Size (in) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 147 | 147 | 1 |  | Platform |  |  |
| 140 | 3 |  | PD10017 |  |  |  |
| 134 | 140 | 6 |  | PD10017 |  |  |

Table 4 - Documents Provided

| Document | Remarks | Reference | Source |
| :---: | :---: | :---: | :---: |
| 4-GEOTECHNICAL REPORTS | T.E.P., Project \#: 082233.01 <br> dated 9/3/08 | 2308053 | CCISITES |
| 4-TOWER FOUNDATION <br> DRAWINGS/DESIGN/SPECS | TEP Project \#: 082233 , dated: <br> $8 / 26 / 08$ | 2308022 | CCISITES |
| 4-TOWER MANUFACTURER <br> DRAWINGS | Valmont Order\#: $10704-90$, dated: <br> $5 / 22 / 90$ | 260794 | CCISITES |
| 4-TOWER STRUCTURAL <br> ANALYSIS REPORTS | P-sec Project \#: 9184, dated: <br> $7 / 2 / 13$ | 3903130 | CCISITES |

## 3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

## 3.2) Assumptions

1) Tower and structures were built in accordance with the manufacturer's specifications.
2) The tower and structures have been maintained in accordance with the manufacturer's specification.
3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
4) When applicable, transmission cables are considered as structural components for calculating wind loads as allowed by TIA/EIA-222-F.
5) Mount sizes, weights, and manufacturers are best estimates based on site photos provided and were determined without the benefit of a site visit by GPD.
6) All member connections and foundation steel reinforcing are assumed designed to meet or exceed the load carrying capacity of the connected member and surrounding soils respectively unless otherwise specified in this report.
7) All equipment model numbers, quantities, and centerline elevations are as provided in the CCl CAD package dated 1/2/2014 with any adjustments as noted below.
a) Per application \# 208261 revision 3, three of the existing antennas model: PCS 1900 MHz $4 \times 45 \mathrm{~W}-65 \mathrm{MHz}$ should have mount center line at 136' rather than 137'.

This analysis may be affected if any assumptions are not valid or have been made in error. GPD Group should be notified to determine the effect on the structural integrity of the tower.

## 4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

| Section <br> No. | Elevation (ft) | Component <br> Type | Size | Critical <br> Element | P(K) | $\mathbf{S F}^{\star} \mathbf{P}$ allow <br> $(\mathbf{K})$ | \% <br> Capacity | Pass / Fail |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L1 | $150-96.8333$ | Pole | TP39.21×26.19x0.3125 | 1 | -10.47 | 1962.20 | 35.0 | Pass |
| L2 | $96.8333-48$ | Pole | TP50.55x37.1973x0.406 | 2 | -21.29 | 3291.40 | 44.5 | Pass |
| L3 | $48-0$ | Pole | TP61.5x48.023x0.5 | 3 | -40.47 | 5071.45 | 45.9 | Pass |
|  |  |  |  |  |  |  | Summary |  |
|  |  |  |  |  |  | Pole (L3) | 45.9 | Pass |
|  |  |  |  |  |  | Rating $=$ | 45.9 | Pass |

Table 6 - Tower Component Stresses vs. Capacity - LC7

| Notes | Component | Elevation (ft) | \% Capacity | Pass / Fail |
| :---: | :---: | :---: | :---: | :---: |
| 1 | Anchor Rods | 0 | 40.7 | Pass |
| 1 | Base Plate | 0 | 30.1 | Pass |
| 1 | Base Foundation | 0 | 26.3 | Pass |
| 1 | Base Foundation <br> Soil Interaction | 0 | 70.5 | Pass |


| Structure Rating (max from all components) $=$ | $70.5 \%$ |
| :--- | :--- |

Notes:

1) See additional documentation in "Appendix C - Additional Calculations" for calculations supporting the \% capacity consumed.

## 4.1) Recommendations

The existing tower and its foundation are sufficient for the proposed loading and do not require modifications.

## 5) DISCLAIMER OF WARRANTIES

GPD GROUP has not performed a site visit to the tower to verify the member sizes or antenna/coax loading. If the existing conditions are not as represented on the tower elevation contained in this report, we should be contacted immediately to evaluate the significance of the discrepancy. This is not a condition assessment of the tower or foundation. This report does not replace a full tower inspection. The tower and foundations are assumed to have been properly fabricated, erected, maintained, in good condition, twist free, and plumb.

The engineering services rendered by GPD GROUP in connection with this Structural Analysis are limited to a computer analysis of the tower structure and theoretical capacity of its main structural members. All tower components have been assumed to only resist dead loads when no other loads are applied. No allowance was made for any damaged, bent, missing, loose, or rusted members (above and below ground). No allowance was made for loose bolts or cracked welds.

GPD GROUP does not analyze the fabrication of the structure (including welding). It is not possible to have all the very detailed information needed to perform a thorough analysis of every structural sub-component and connection of an existing tower. GPD GROUP provides a limited scope of service in that we cannot verify the adequacy of every weld, plate connection detail, etc. The purpose of this report is to assess the feasibility of adding appurtenances usually accompanied by transmission lines to the structure.

It is the owner's responsibility to determine the amount of ice accumulation in excess of the code specified amount, if any, that should be considered in the structural analysis.

The attached sketches are a schematic representation of the analyzed tower. If any material is fabricated from these sketches, the contractor shall be responsible for field verifying the existing conditions, proper fit, and clearance in the field. Any mentions of structural modifications are reasonable estimates and should not be used as a precise construction document. Precise modification drawings are obtainable from GPD GROUP, but are beyond the scope of this report.

Miscellaneous items such as antenna mounts, etc., have not been designed or detailed as a part of our work. We recommend that material of adequate size and strength be purchased from a reputable tower manufacturer.

GPD GROUP makes no warranties, expressed and/or implied, in connection with this report and disclaims any liability arising from material, fabrication, and erection of this tower. GPD GROUP will not be responsible whatsoever for, or on account of, consequential or incidental damages sustained by any person, firm, or organization as a result of any data or conclusions contained in this report. The maximum liability of GPD GROUP pursuant to this report will be limited to the total fee received for preparation of this report.

## APPENDIX A

TNXTOWER OUTPUT


DESIGNED APPURTENANCE LOADING

| TYPE | ELEVATION | TYPE | ELEVATION |
| :---: | :---: | :---: | :---: |
| Platform Mount [LP 602-1] | 146 | 800MHz 2X50W RRH W/FILTER | 137 |
| BXA-70063-6CF-EDIN-4 w/ Mount Pipe | 146 | PCS 1900MHz 4x45W-65MHz | 137 |
|  |  | PCS 1900MHz 4x45W-65MHz | 137 |
| BXA-70063-6CF-EDIN-4 w/ Mount Pipe | 146 | PCS 1900MHz 4x45W-65MHz | 137 |
|  |  | PCS 1900MHz 4x45W-65MHz | 137 |
| BXA-70063-6CF-EDIN-4 w/ Mount Pipe | 146 | PCS 1900MHz 4x45W-65MHz | 137 |
|  | 146 | PCS 1900MHz 4x45W-65MHz | 137 |
| BXA-171063-12CF-EDIN-2 w/ Mount Pipe |  | Pipe Mount 6'x2.375' | 137 |
| BXA-171063-12CF-EDIN-2 w/ Mount Pipe | 146 | Pipe Mount 6'x2.375' | 137 |
|  |  | Pipe Mount 6'x2.375" | 137 |
| BXA-171063-12CF-EDIN-2 w/ Mount Pipe | 146 | Platform Mount [LP 602-1] | 134 |
|  |  | APXVTM14-C-120 w/ Mount Pipe | 134 |
| RRH2x40-AWS | 146 | APXVTM14-C-120 w/ Mount Pipe | 134 |
| RRH $2 \times 40$-AWS | 146 | APXVTM14-C-120 w/ Mount Pipe | 134 |
| RRH2x40-AWS | 146 | TD-RRH8x20-25 | 134 |
| DB-T1-6Z-8AB-0Z | 146 | TD-RRH8×20-25 | 134 |
| BXA-171063-8BF-2 w/ Mount Pipe | 146 | TD-RRH8x20-25 | 134 |
| BXA-171063-12CF-EDIN-2 w/ Mount Pipe | 146 | APXVSPP18-C-A20 w/ Mount Pipe | 134 |
|  |  | APXVSPP18-C-A20 w/ Mount Pipe | 134 |
| BXA-171063-12CF-EDIN-2 w/ Mount Pipe | 146 | APXVSPP18-C-A20 w/ Mount Pipe | 134 |
| BXA-70063-6CF-EDIN-5 w/ Mount Pipe | 146 | IBC1900BB-1 | 134 |
|  |  | IBC1900BB-1 | 134 |
| BXA-70063-6CF-EDIN-5 w/ Mount Pipe | 146 | IBC1900BB-1 | 134 |
|  |  | IBC1900HG-2A | 134 |
| BXA-70063-6CF-EDIN-5 w/ Mount Pipe | 146 | IBC1900HG-2A | 134 |
|  |  | IBC1900HG-2A | 134 |
| Side Arm Mount [SO 102-3] | 137 | Pipe Mount 6'x2.375' | 134 |
| $800 \mathrm{MHz} 2 \mathrm{2X50W}$ RRH W/FILTER | 137 | Pipe Mount 6'x2.375' | 134 |
| 800MHz 2X50W RRH W/FILTER | 137 | Pipe Mount 6'x2.375' | 134 |
|  |  | Side Arm Mount [SO 702-1] | 117 |
|  |  | BCD-87010 | 117 |

MATERIAL STRENGTH

| GRADE | Fy | Fu | GRADE | Fy | Fu |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathrm{s}-22$ | 65 ksi | 80 ksi |  |  |  |  |

TOWER DESIGN NOTES
Tower is located in Hartford County, Connecticut
2. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 28 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 50 mph wind
5. TOWER RATING: 45.9\%


TORQUE 0 kip-ft 28 mph WIND - 1.0000 in ICE

AXIAL


TORQUE 1 kip-ft
REACTIONS - 80 mph WIND


Feed Line Distribution Chart
0' - 150'
$\qquad$ Found $\quad$ Fla $\qquad$ App In Face $\qquad$ App Out Face $\qquad$ Truss Leg


## Tower Input Data

There is a pole section.
This tower is designed using the TIA/EIA-222-F standard.
The following design criteria apply:

1) Tower is located in Hartford County, Connecticut.
2) Basic wind speed of 80 mph .
3) Nominal ice thickness of 1.0000 in.
4) Ice thickness is considered to increase with height.
5) Ice density of 56 pcf.
6) A wind speed of 28 mph is used in combination with ice.
7) Temperature drop of $50{ }^{\circ} \mathrm{F}$.
8) Deflections calculated using a wind speed of 50 mph .
9) A non-linear (P-delta) analysis was used.
10) Pressures are calculated at each section.
11) Stress ratio used in pole design is 1.333 .
12) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

|  |  |  |
| :--- | :--- | :--- |
| Consider Moments - Legs | Distribute Leg Loads As Uniform | Treat Feedline Bundles As Cylinder |
| Consider Moments - Horizontals | Assume Legs Pinned | Use ASCE 10 X-Brace Ly Rules |
| Consider Moments - Diagonals | $\sqrt{ }$ Assume Rigid Index Plate | Calculate Redundant Bracing Forces |
| Use Moment Magnification | $\sqrt{ }$ Use Clear Spans For Wind Area | Ignore Redundant Members in FEA |
| $\sqrt{ }$ Use Code Stress Ratios | $\sqrt{ }$ Use Clear Spans For KL/r | SR Leg Bolts Resist Compression |
| $\sqrt{ }$ Use Code Safety Factors - Guys | Retension Guys To Initial Tension | All Leg Panels Have Same Allowable |
| $\sqrt{ }$ Escalate Ice | $\sqrt{ }$ Bypass Mast Stability Checks | Offset Girt At Foundation |
| Always Use Max Kz | $\sqrt{ }$ Use Azimuth Dish Coefficients | $\sqrt{ }$ Consider Feedline Torque |
| Use Special Wind Profile | $\sqrt{ }$ Project Wind Area of Appurt. | Include Angle Block Shear Check |
| Include Bolts In Member Capacity | Autocalc Torque Arm Areas | Poles |
| Leg Bolts Are At Top Of Section | SR Members Have Cut Ends | $\sqrt{ }$ Include Shear-Torsion Interaction |
| Secondary Horizontal Braces Leg | Sort Capacity Reports By Component | Always Use Sub-Critical Flow |
| Use Diamond Inner Bracing (4 Sided) | Triangulate Diamond Inner Bracing | Use Top Mounted Sockets |
| Add IBC $.6 \mathrm{D}+$ W Combination | Use TIA-222-G Tension Splice |  |
|  | Capacity Exemption |  |

## Tapered Pole Section Geometry

| Section | Elevation ft | Section <br> Length <br> ft | Splice Length ft | Number of Sides | Top Diameter in | Bottom Diameter in | Wall <br> Thickness <br> in | Bend Radius in | Pole Grade |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L1 | 150.00-96.83 | 53.17 | 5.67 | 12 | 26.1900 | 39.2100 | 0.3125 | 1.2500 | $\begin{gathered} \mathrm{S}-22 \\ (65 \mathrm{ksi}) \end{gathered}$ |
| L2 | 96.83-48.00 | 54.50 | 7.00 | 12 | 37.1973 | 50.5500 | 0.4060 | 1.6240 | $\begin{gathered} \mathrm{S}-22 \\ (65 \mathrm{ksi}) \end{gathered}$ |
| L3 | 48.00-0.00 | 55.00 |  | 12 | 48.0230 | 61.5000 | 0.5000 | 2.0000 | $\begin{gathered} \mathrm{S}-22 \\ (65 \mathrm{ksi}) \end{gathered}$ |

## Tapered Pole Properties

| Section | Tip Dia. <br> in | Area <br> $i n^{2}$ | $l n^{4}$ | $r$ <br> $i n$ | $C$ <br> $i n$ | $l / C$ <br> $i n^{3}$ | $J$ <br> $i n^{4}$ | $I t / Q$ <br> $i n^{2}$ | $w$ <br> $i n$ | $w / t$ |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 27.1139 | 26.0392 | 2225.6599 | 9.2641 | 13.5664 | 164.0565 | 4509.7903 | 12.8157 | 6.1814 | 19.781 |
| L1 | 40.5932 | 39.1406 | 7558.8706 | 13.9253 | 20.3108 | 372.1605 | 15316.321 | 19.2638 | 9.6708 | 30.946 |

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## Feed Line/Linear Appurtenances - Entered As Area

\begin{tabular}{|c|c|c|c|c|c|c|c|c|}
\hline Description \& $$
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
$$ \& Allow Shield \& Component Type \& ft \& Total Number \& \& $C_{A} A_{A}$

$\mathrm{ft}^{2} / \mathrm{ft}$ \& | Weight |
| :--- |
| plf | <br>

\hline \multirow[t]{5}{*}{LDF6-50A(1-1/4")} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{No} \& \multirow[t]{5}{*}{Inside Pole} \& \multirow[t]{5}{*}{146.00-8.00} \& \multirow[t]{5}{*}{12} \& No Ice \& 0.00 \& 0.66 <br>
\hline \& \& \& \& \& \& 1/2" Ice \& 0.00 \& 0.66 <br>
\hline \& \& \& \& \& \& 1 ' Ice \& 0.00 \& 0.66 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 0.00 \& 0.66 <br>
\hline \& \& \& \& \& \& 4" Ice \& 0.00 \& 0.66 <br>
\hline LDF6-50A(1-1/4") \& C \& No \& Inside Pole \& 146.00-8.00 \& 6 \& No Ice \& 0.00 \& 0.66 <br>
\hline \& \& \& \& \& \& 1/2" Ice \& 0.00 \& 0.66 <br>
\hline \& \& \& \& \& \& 1 ' Ice \& 0.00 \& 0.66 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 0.00 \& 0.66 <br>
\hline \& \& \& \& \& \& 4" Ice \& 0.00 \& 0.66 <br>
\hline HB158-1-08U8-S8J18( \& C \& No \& Inside Pole \& 146.00-8.00 \& 1 \& No Ice \& 0.00 \& 1.30 <br>
\hline 1-5/8) \& \& \& \& \& \& 1/2" Ice \& 0.00 \& 1.30 <br>
\hline \& \& \& \& \& \& 1 " Ice \& 0.00 \& 1.30 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 0.00 \& 1.30 <br>
\hline \& \& \& \& \& \& 4" Ice \& 0.00 \& 1.30 <br>
\hline HB114-1-08U4-M5J(1 \& A \& No \& Inside Pole \& 134.00-8.00 \& 3 \& No Ice \& 0.00 \& 1.08 <br>
\hline 1/4") \& \& \& \& \& \& 1/2" Ice \& 0.00 \& 1.08 <br>
\hline \& \& \& \& \& \& 1 " Ice \& 0.00 \& 1.08 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 0.00 \& 1.08 <br>
\hline \& \& \& \& \& \& 4" Ice \& 0.00 \& 1.08 <br>
\hline HB058-M12- \& C \& No \& Inside Pole \& 134.00-8.00 \& 1 \& No Ice \& 0.00 \& 0.24 <br>
\hline XXXF(5/8") \& \& \& \& \& \& 1/2" Ice \& 0.00 \& 0.24 <br>
\hline \& \& \& \& \& \& 1 " Ice \& 0.00 \& 0.24 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 0.00 \& 0.24 <br>
\hline \& \& \& \& \& \& 4" Ice \& 0.00 \& 0.24 <br>
\hline LDF5-50A(7/8") \& C \& No \& Inside Pole \& 117.00-8.00 \& 1 \& No Ice \& 0.00 \& 0.33 <br>
\hline \& \& \& \& \& \& 1/2" Ice \& 0.00 \& 0.33 <br>
\hline \& \& \& \& \& \& 1 " Ice \& 0.00 \& 0.33 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 0.00 \& 0.33 <br>
\hline \& \& \& \& \& \& 4" Ice \& 0.00 \& 0.33 <br>
\hline Safety Line (3/8") \& B \& No \& CaAa (Out Of \& 150.00-8.00 \& 1 \& No Ice \& 0.04 \& 0.22 <br>
\hline \& \& \& Face) \& \& \& 1/2" Ice \& 0.14 \& 0.75 <br>
\hline \& \& \& \& \& \& 1 " Ice \& 0.24 \& 1.28 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 0.44 \& 2.34 <br>
\hline \& \& \& \& \& \& 4" Ice \& 0.84 \& 4.46 <br>
\hline Step Pegs \& B \& No \& CaAa (Out Of \& 150.00-8.00 \& 1 \& No Ice \& 0.08 \& 2.72 <br>
\hline \& \& \& Face) \& \& \& 1/2" Ice \& 0.18 \& 3.51 <br>
\hline \& \& \& \& \& \& 1 " Ice \& 0.28 \& 4.92 <br>
\hline \& \& \& \& \& \& 2" Ice \& 0.48 \& 9.56 <br>
\hline
\end{tabular}

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Feed Line/Linear Appurtenances Section Areas

| Tower <br> Sectio | Tower <br> Elevation <br> $n$ | ft | Face | $A_{R}$ | $A_{F}$ | $C_{A} A_{A}$ <br> $I n ~ F a c e ~_{n}$ <br> $f t^{2}$ | $C_{A} A_{A}$ <br> Out Face |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L1 | $150.00-96.83$ | A | 0.000 | 0.000 | 0.000 | 0.000 | Weight |
|  |  | B | 0.000 | 0.000 | 0.000 | 6.247 | 0.12 |
|  |  | C | 0.000 | 0.000 | 0.000 | 0.000 | 0.16 |
| L2 | $96.83-48.00$ | A | 0.000 | 0.000 | 0.000 | 0.000 | 0.16 |
|  |  | B | 0.000 | 0.000 | 0.000 | 5.738 | 0.14 |
|  |  | C | 0.000 | 0.000 | 0.000 | 0.000 | 0.67 |
| L3 | $48.00-0.00$ | A | 0.000 | 0.000 | 0.000 | 0.000 | 0.13 |
|  |  | B | 0.000 | 0.000 | 0.000 | 4.700 | 0.12 |
|  |  | C | 0.000 | 0.000 | 0.000 | 0.000 | 0.55 |

Feed Line/Linear Appurtenances Section Areas - With Ice

\begin{tabular}{|c|c|c|c|c|c|c|c|c|}
\hline Tower Sectio $n$ \& Tower Elevation ft \& $$
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
$$ \& Ice Thickness in \& $A_{R}$

$f t^{2}$ \& $A_{F}$

$f t^{2}$ \& $C_{A} A_{A}$ In Face $f t^{2}$ \& $$
\begin{gathered}
C_{A} A_{A} \\
\text { Out Face } \\
\text { ft }^{2}
\end{gathered}
$$ \& Weight

K <br>
\hline \multirow[t]{3}{*}{L1} \& \multirow[t]{3}{*}{150.00-96.83} \& A \& \multirow[t]{3}{*}{1.170} \& 0.000 \& 0.000 \& 0.000 \& 0.000 \& 0.12 <br>
\hline \& \& B \& \& 0.000 \& 0.000 \& 0.000 \& 31.128 \& 0.38 <br>
\hline \& \& C \& \& 0.000 \& 0.000 \& 0.000 \& 0.000 \& 0.66 <br>
\hline \multirow[t]{3}{*}{L2} \& \multirow[t]{3}{*}{96.83-48.00} \& A \& \multirow[t]{3}{*}{1.098} \& 0.000 \& 0.000 \& 0.000 \& 0.000 \& 0.16 <br>
\hline \& \& B \& \& 0.000 \& 0.000 \& 0.000 \& 28.591 \& 0.35 <br>
\hline \& \& C \& \& 0.000 \& 0.000 \& 0.000 \& 0.000 \& 0.67 <br>
\hline \multirow[t]{3}{*}{L3} \& \multirow[t]{3}{*}{48.00-0.00} \& A \& \multirow[t]{3}{*}{1.000} \& 0.000 \& 0.000 \& 0.000 \& 0.000 \& 0.13 <br>
\hline \& \& B \& \& 0.000 \& 0.000 \& 0.000 \& 22.268 \& 0.27 <br>
\hline \& \& C \& \& 0.000 \& 0.000 \& 0.000 \& 0.000 \& 0.55 <br>
\hline
\end{tabular}

Feed Line Center of Pressure

| Section | Elevation | $C P_{X}$ | $C P_{Z}$ | $C P_{X}$ <br> Ice | $C P_{Z}$ <br> Ice <br> in |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | ft | in | in | in | in |
|  | $150.00-96.83$ | 0.1463 | 0.0845 | 0.5912 | 0.3413 |
| L1 | $96.83-48.00$ | 0.1480 | 0.0854 | 0.6285 | 0.3629 |
| L2 | $48.00-0.00$ | 0.1224 | 0.0707 | 0.5195 | 0.2999 |

## Discrete Tower Loads

\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& \[
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
\] \& \begin{tabular}{l}
Offset \\
Type
\end{tabular} \& Offsets: Horz Lateral Vert ft ft ft \& \begin{tabular}{l}
Azimuth Adjustmen \(t\) \\
○
\end{tabular} \& Placement

ft \& \& $C_{A} A_{A}$ Front

$$
f t^{2}
$$ \& $C_{A} A_{A}$ Side $f t^{2}$ \& Weight

K <br>
\hline \multirow[t]{5}{*}{Platform Mount [LP 602-1]} \& \multirow[t]{5}{*}{C} \& \multirow[t]{5}{*}{None} \& \& 0.0000 \& 146.00 \& No Ice \& 32.03 \& 32.03 \& 1.34 <br>
\hline \& \& \& \& \& \& 1/2" \& 38.71 \& 38.71 \& 1.80 <br>
\hline \& \& \& \& \& \& Ice \& 45.39 \& 45.39 \& 2.26 <br>
\hline \& \& \& \& \& \& 1" Ice \& 58.75 \& 58.75 \& 3.17 <br>
\hline \& \& \& \& \& \& 2 " Ice \& 85.47 \& 85.47 \& 5.00 <br>
\hline
\end{tabular}


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| Description | $\begin{gathered} \text { Face } \\ \text { or } \\ \text { Leg } \end{gathered}$ | Offset Type | Offsets: Horz Lateral Vert ft ft ft | Azimuth Adjustmen $t$ <br> 0 | Placement <br> ft |  | $C_{A} A_{A}$ Front $f t^{2}$ | $C_{A} A_{A}$ Side <br> $f t^{2}$ | Weight |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | $\begin{aligned} & \text { 1" Ice } \\ & \text { 2" Ice } \\ & \text { 4" Ice } \end{aligned}$ | $\begin{aligned} & 3.72 \\ & 4.86 \end{aligned}$ | $\begin{aligned} & 3.61 \\ & 4.74 \end{aligned}$ | $\begin{aligned} & 0.17 \\ & 0.35 \end{aligned}$ |
| $\begin{gathered} \text { PCS } 1900 \mathrm{MHz} 4 \times 45 \mathrm{~W}- \\ 65 \mathrm{MHz} \end{gathered}$ | C | From Leg | $\begin{aligned} & 1.00 \\ & 0.00 \\ & -1.00 \end{aligned}$ | 0.0000 | 137.00 | No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice | $\begin{aligned} & 2.71 \\ & 2.95 \\ & 3.20 \\ & 3.72 \\ & 4.86 \end{aligned}$ | $\begin{aligned} & 2.61 \\ & 2.85 \\ & 3.09 \\ & 3.61 \\ & 4.74 \end{aligned}$ | $\begin{aligned} & 0.06 \\ & 0.08 \\ & 0.11 \\ & 0.17 \\ & 0.35 \end{aligned}$ |
| $\begin{gathered} \text { PCS } 1900 \mathrm{MHz} 4 \times 45 \mathrm{~W}- \\ 65 \mathrm{MHz} \end{gathered}$ | A | From Leg | $\begin{aligned} & 1.00 \\ & 0.00 \\ & 0.00 \end{aligned}$ | 0.0000 | 137.00 | No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice | $\begin{aligned} & 2.71 \\ & 2.95 \\ & 3.20 \\ & 3.72 \\ & 4.86 \end{aligned}$ | $\begin{aligned} & 2.61 \\ & 2.85 \\ & 3.09 \\ & 3.61 \\ & 4.74 \end{aligned}$ | $\begin{aligned} & 0.06 \\ & 0.08 \\ & 0.11 \\ & 0.17 \\ & 0.35 \end{aligned}$ |
| $\begin{gathered} \text { PCS } 1900 \mathrm{MHz} 4 \times 45 \mathrm{~W}- \\ 65 \mathrm{MHz} \end{gathered}$ | B | From Leg | $\begin{aligned} & 1.00 \\ & 0.00 \\ & 0.00 \end{aligned}$ | 0.0000 | 137.00 | No Ice 1/2" Ice 1" Ice 2" Ice <br> 4" Ice | $\begin{aligned} & 2.71 \\ & 2.95 \\ & 3.20 \\ & 3.72 \\ & 4.86 \end{aligned}$ | $\begin{aligned} & 2.61 \\ & 2.85 \\ & 3.09 \\ & 3.61 \\ & 4.74 \end{aligned}$ | $\begin{aligned} & 0.06 \\ & 0.08 \\ & 0.11 \\ & 0.17 \\ & 0.35 \end{aligned}$ |
| $\begin{gathered} \text { PCS } 1900 \mathrm{MHz} 4 \times 45 \mathrm{~W}- \\ 65 \mathrm{MHz} \end{gathered}$ | C | From Leg | $\begin{aligned} & 1.00 \\ & 0.00 \\ & 0.00 \end{aligned}$ | 0.0000 | 137.00 | No Ice <br> 1/2" <br> Ice <br> 1" Ice <br> 2" Ice <br> 4" Ice | $\begin{aligned} & 2.71 \\ & 2.95 \\ & 3.20 \\ & 3.72 \\ & 4.86 \end{aligned}$ | $\begin{aligned} & 2.61 \\ & 2.85 \\ & 3.09 \\ & 3.61 \\ & 4.74 \end{aligned}$ | $\begin{aligned} & 0.06 \\ & 0.08 \\ & 0.11 \\ & 0.17 \\ & 0.35 \end{aligned}$ |
| Pipe Mount 6'x2.375" | A | From Leg | $\begin{aligned} & 1.00 \\ & 0.00 \\ & 0.00 \end{aligned}$ | 0.0000 | 137.00 | No Ice <br> 1/2" <br> Ice <br> 1" Ice <br> 2" Ice <br> 4" Ice | $\begin{aligned} & 1.43 \\ & 1.92 \\ & 2.29 \\ & 3.06 \\ & 4.70 \end{aligned}$ | $\begin{aligned} & 1.43 \\ & 1.92 \\ & 2.29 \\ & 3.06 \\ & 4.70 \end{aligned}$ | $\begin{aligned} & 0.03 \\ & 0.04 \\ & 0.05 \\ & 0.09 \\ & 0.23 \end{aligned}$ |
| Pipe Mount 6'x2.375" | B | From Leg | $\begin{aligned} & 1.00 \\ & 0.00 \\ & 0.00 \end{aligned}$ | 0.0000 | 137.00 | No Ice <br> 1/2" <br> Ice <br> 1" Ice <br> 2" Ice <br> 4" Ice | $\begin{aligned} & 1.43 \\ & 1.92 \\ & 2.29 \\ & 3.06 \\ & 4.70 \end{aligned}$ | $\begin{aligned} & 1.43 \\ & 1.92 \\ & 2.29 \\ & 3.06 \\ & 4.70 \end{aligned}$ | $\begin{aligned} & 0.03 \\ & 0.04 \\ & 0.05 \\ & 0.09 \\ & 0.23 \end{aligned}$ |
| Pipe Mount 6'x2.375" | C | From Leg | $\begin{aligned} & 1.00 \\ & 0.00 \\ & 0.00 \end{aligned}$ | 0.0000 | 137.00 | No Ice <br> 1/2" <br> Ice <br> 1" Ice <br> 2" Ice <br> 4" Ice | $\begin{aligned} & 1.43 \\ & 1.92 \\ & 2.29 \\ & 3.06 \\ & 4.70 \end{aligned}$ | $\begin{aligned} & 1.43 \\ & 1.92 \\ & 2.29 \\ & 3.06 \\ & 4.70 \end{aligned}$ | $\begin{aligned} & 0.03 \\ & 0.04 \\ & 0.05 \\ & 0.09 \\ & 0.23 \end{aligned}$ |
| Platform Mount [LP 602-1] | C | None |  | 0.0000 | 134.00 | No Ice <br> 1/2" <br> Ice <br> 1" Ice <br> 2" Ice <br> 4" Ice | $\begin{aligned} & 32.03 \\ & 38.71 \\ & 45.39 \\ & 58.75 \\ & 85.47 \end{aligned}$ | $\begin{aligned} & 32.03 \\ & 38.71 \\ & 45.39 \\ & 58.75 \\ & 85.47 \end{aligned}$ | $\begin{aligned} & 1.34 \\ & 1.80 \\ & 2.26 \\ & 3.17 \\ & 5.00 \end{aligned}$ |
| APXVTM14-C-120 w/ Mount Pipe | A | From Leg | $\begin{aligned} & 4.00 \\ & 0.00 \\ & 1.00 \end{aligned}$ | 0.0000 | 134.00 | No Ice <br> 1/2" <br> Ice <br> 1" Ice <br> 2" Ice <br> 4" Ice | $\begin{gathered} 7.13 \\ 7.66 \\ 8.18 \\ 9.26 \\ 11.53 \end{gathered}$ | $\begin{gathered} 4.96 \\ 5.75 \\ 6.47 \\ 8.01 \\ 11.41 \end{gathered}$ | $\begin{aligned} & 0.08 \\ & 0.13 \\ & 0.19 \\ & 0.34 \\ & 0.75 \end{aligned}$ |
| APXVTM14-C-120 w/ Mount Pipe | B | From Leg | $\begin{aligned} & 4.00 \\ & 0.00 \\ & 1.00 \end{aligned}$ | 0.0000 | 134.00 | No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice | $\begin{gathered} 7.13 \\ 7.66 \\ 8.18 \\ 9.26 \\ 11.53 \end{gathered}$ | $\begin{gathered} 4.96 \\ 5.75 \\ 6.47 \\ 8.01 \\ 11.41 \end{gathered}$ | $\begin{aligned} & 0.08 \\ & 0.13 \\ & 0.19 \\ & 0.34 \\ & 0.75 \end{aligned}$ |
| APXVTM14-C-120 w/ Mount Pipe | C | From Leg | $\begin{aligned} & 4.00 \\ & 0.00 \end{aligned}$ | 0.0000 | 134.00 | No Ice 1/2" | $\begin{aligned} & 7.13 \\ & 7.66 \end{aligned}$ | $\begin{aligned} & 4.96 \\ & 5.75 \end{aligned}$ | $\begin{aligned} & 0.08 \\ & 0.13 \end{aligned}$ |



\begin{tabular}{|c|c|c|c|c|c|c|c|c|c|}
\hline Description \& \[
\begin{gathered}
\text { Face } \\
\text { or } \\
\text { Leg }
\end{gathered}
\] \& \begin{tabular}{l}
Offset \\
Type
\end{tabular} \& Offsets: Horz Lateral Vert ft ft ft \& \begin{tabular}{l}
Azimuth Adjustmen \(t\) \\
0
\end{tabular} \& Placement \& \& \(C_{A} A_{A}\) Front
\[
f t^{2}
\] \& \(C_{A} A_{A}\)
Side

$t t^{2}$ \& Weight

K <br>
\hline \multirow{10}{*}{IBC1900HG-2A} \& \multirow{7}{*}{C} \& \multirow{7}{*}{From Leg} \& 0.00 \& \multirow{7}{*}{0.0000} \& \multirow{7}{*}{134.00} \& 1/2" \& 1.27 \& 0.65 \& 0.03 <br>
\hline \& \& \& \multirow[t]{3}{*}{0.00} \& \& \& Ice \& 1.43 \& 0.77 \& 0.04 <br>
\hline \& \& \& \& \& \& 1 " Ice \& 1.76 \& 1.04 \& 0.06 <br>

\hline \& \& \& \& \& \& $$
\begin{aligned}
& \text { 2" Ice } \\
& \text { 4" Ice }
\end{aligned}
$$ \& 2.53 \& 1.69 \& 0.15 <br>

\hline \& \& \& 4.00 \& \& \& No Ice \& 1.13 \& 0.53 \& 0.02 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" \& 1.27 \& 0.65 \& 0.03 <br>
\hline \& \& \& \multirow[t]{4}{*}{0.00} \& \& \& Ice \& 1.43 \& 0.77 \& 0.04 <br>
\hline \& \multirow{8}{*}{A} \& \multirow{8}{*}{From Leg} \& \& \multirow{8}{*}{0.0000} \& \multirow{8}{*}{134.00} \& $1{ }^{\prime \prime}$ Ice \& 1.76 \& 1.04 \& 0.06 <br>
\hline \& \& \& \& \& \& 2" Ice \& 2.53 \& 1.69 \& 0.15 <br>
\hline \& \& \& \& \& \& 4" Ice \& \& \& <br>
\hline \multirow[t]{6}{*}{Pipe Mount 6'x2.375"} \& \& \& 4.00 \& \& \& No Ice \& 1.43 \& 1.43 \& 0.03 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" \& 1.92 \& 1.92 \& 0.04 <br>
\hline \& \& \& \multirow[t]{4}{*}{0.00} \& \& \& Ice \& 2.29 \& 2.29 \& 0.05 <br>
\hline \& \& \& \& \& \& 1" Ice \& 3.06 \& 3.06 \& 0.09 <br>
\hline \& \& \& \& \& \& 2" Ice \& 4.70 \& 4.70 \& 0.23 <br>
\hline \& \multirow{7}{*}{B} \& \multirow{7}{*}{From Leg} \& \& \multirow{7}{*}{0.0000} \& \& 4" Ice \& \& \& <br>
\hline \multirow[t]{6}{*}{Pipe Mount 6'x2.375"} \& \& \& 4.00 \& \& \multirow[t]{6}{*}{134.00} \& No Ice \& 1.43 \& 1.43 \& 0.03 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" \& 1.92 \& 1.92 \& 0.04 <br>
\hline \& \& \& \multirow[t]{4}{*}{0.00} \& \& \& Ice \& 2.29 \& 2.29 \& 0.05 <br>
\hline \& \& \& \& \& \& $1{ }^{\prime \prime}$ Ice \& 3.06 \& 3.06 \& 0.09 <br>
\hline \& \& \& \& \& \& 2" Ice \& 4.70 \& 4.70 \& 0.23 <br>
\hline \& \& \& \& \& \& 4" Ice \& \& \& <br>
\hline \multirow[t]{6}{*}{Pipe Mount 6'x2.375"} \& \multirow[t]{6}{*}{C} \& \multirow[t]{6}{*}{From Leg} \& 4.00 \& \multirow[t]{6}{*}{0.0000} \& \multirow[t]{6}{*}{134.00} \& No Ice \& 1.43 \& 1.43 \& 0.03 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" \& 1.92 \& 1.92 \& 0.04 <br>
\hline \& \& \& \multirow[t]{4}{*}{0.00} \& \& \& Ice \& 2.29 \& 2.29 \& 0.05 <br>
\hline \& \& \& \& \& \& 1" Ice \& 3.06 \& 3.06 \& 0.09 <br>
\hline \& \& \& \& \& \& 2" Ice \& 4.70 \& 4.70 \& 0.23 <br>
\hline \& \& \& \& \& \& 4" Ice \& \& \& <br>
\hline \multirow[t]{6}{*}{Side Arm Mount [SO 7021]} \& \multirow[t]{6}{*}{A} \& \multirow[t]{6}{*}{From Leg} \& \& \multirow[t]{6}{*}{0.0000} \& \multirow[t]{6}{*}{117.00} \& No Ice \& 1.00 \& 1.43 \& 0.03 <br>

\hline \& \& \& $$
0.00
$$ \& \& \& 1/2" \& 1.00 \& 2.05 \& \[

0.04
\] <br>

\hline \& \& \& \multirow[t]{4}{*}{0.00} \& \& \& Ice \& 1.00 \& 2.67 \& 0.05 <br>
\hline \& \& \& \& \& \& 1" Ice \& 1.00 \& 3.91 \& 0.07 <br>
\hline \& \& \& \& \& \& 2" Ice \& 1.00 \& 6.39 \& 0.12 <br>
\hline \& \& \& \& \& \& 4" Ice \& \& \& <br>
\hline \multirow[t]{6}{*}{BCD-87010} \& \multirow[t]{6}{*}{A} \& \multirow[t]{6}{*}{From Leg} \& 4.00 \& \multirow[t]{6}{*}{0.0000} \& \multirow[t]{6}{*}{117.00} \& No Ice \& 2.90 \& 2.90 \& 0.03 <br>
\hline \& \& \& 0.00 \& \& \& 1/2" \& 4.05 \& 4.05 \& 0.05 <br>
\hline \& \& \& \multirow[t]{4}{*}{5.00} \& \& \& Ice \& 5.21 \& 5.21 \& 0.08 <br>
\hline \& \& \& \& \& \& 1" Ice \& 7.01 \& 7.01 \& 0.16 <br>
\hline \& \& \& \& \& \& 2" Ice \& 9.85 \& 9.85 \& 0.41 <br>
\hline \& \& \& \& \& \& 4" Ice \& \& \& <br>
\hline
\end{tabular}

## Load Combinations

| Comb. No. | Description |
| :---: | :---: |
| 1 | Dead Only |
| 2 | Dead+Wind 0 deg - No Ice |
| 3 | Dead+Wind 30 deg - No Ice |
| 4 | Dead+Wind 60 deg - No Ice |
| 5 | Dead+Wind 90 deg - No Ice |
| 6 | Dead+Wind 120 deg - No Ice |
| 7 | Dead+Wind 150 deg - No Ice |
| 8 | Dead+Wind 180 deg - No Ice |
| 9 | Dead+Wind 210 deg - No lce |
| 10 | Dead+Wind 240 deg - No Ice |
| 11 | Dead+Wind 270 deg - No Ice |
| 12 | Dead+Wind 300 deg - No Ice |
| 13 | Dead+Wind 330 deg - No Ice |

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| Comb. | Description |
| :---: | :--- |
| No. |  |
| 14 | Dead+Ice+Temp |
| 15 | Dead+Wind 0 deg+Ice+Temp |
| 16 | Dead+Wind 30 deg+Ice+Temp |
| 17 | Dead+Wind 60 deg+Ice+Temp |
| 18 | Dead+Wind 90 deg+Ice+Temp |
| 19 | Dead+Wind 120 deg+Ice+Temp |
| 20 | Dead+Wind 150 deg+Ice+Temp |
| 21 | Dead+Wind 180 deg+Ice+Temp |
| 22 | Dead+Wind 210 deg+Ice+Temp |
| 23 | Dead+Wind 240 deg+Ice+Temp |
| 24 | Dead+Wind 270 deg+Ice+Temp |
| 25 | Dead+Wind 300 deg+Ice+Temp |
| 26 | Dead+Wind 330 deg+Ice+Temp |
| 27 | Dead+Wind 0 deg - Service |
| 28 | Dead+Wind 30 deg - Service |
| 29 | Dead+Wind 60 deg - Service |
| 30 | Dead+Wind 90 deg - Service |
| 31 | Dead+Wind 120 deg - Service |
| 32 | Dead+Wind 150 deg - Service |
| 33 | Dead+Wind 180 deg - Service |
| 34 | Dead+Wind 210 deg - Service |
| 35 | Dead+Wind 240 deg - Service |
| 36 | Dead+Wind 270 deg - Service |
| 37 | Dead+Wind 300 deg - Service |
| 38 | Dead+Wind 330 deg - Service |

Maximum Tower Deflections - Service Wind

| Section <br> No. | Elevation | Horz. <br> Deflection <br> in | Gov. <br> Load | Tilt | Twist |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | ft | $150-96.8333$ | 16.313 | 31 | $\circ$ |

## Critical Deflections and Radius of Curvature - Service Wind

| Elevation | Appurtenance | Gov. <br> Load | Deflection | Tilt | Twist | Radius of <br> Curvature |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ft |  | Comb. | in | o |  | o |


|  | Maximum Tower Defiections $=$ Design Wind |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Section | Elevation | Horz. | Gov. | Tilt | Twist |
| No. | Deflection | Load | $\circ$ | $\circ$ |  |
|  | ft | in | Comb. | $\circ$ | 0.0034 |
| L1 | $150-96.8333$ | 41.706 | 6 | 2.3827 | 0.0011 |
| L2 | $102.5-48$ | 19.729 | 6 | 1.8428 | 0.0004 |
| L3 | $55-0$ | 5.600 | 6 | 0.9299 |  |


| Elevation | Appurtenance | Gov. <br> Load | Deflection | Tilt | Twist | Radius of <br> Curvature |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $f t$ |  | Comb. | in | $\circ$ | $\circ$ | o |
| 146.00 | Platform Mount [LP 602-1] | 6 | 39.729 | 2.3460 | 0.0031 | 28435 |
| 137.00 | Side Arm Mount [SO 102-3] | 6 | 35.308 | 2.2617 | 0.0026 | 10936 |
| 134.00 | Platform Mount [LP 602-1] | 6 | 33.852 | 2.2324 | 0.0024 | 8885 |
| 117.00 | Side Arm Mount [SO 702-1] | 6 | 25.894 | 2.0456 | 0.0015 | 4307 |

## Compression Checks

## Pole Design Data

| Section No. | Elevation | Size | $L$ | $L_{u}$ | Kl/r | $F_{a}$ | $A$ | Actual $P$ | Allow. $P_{a}$ | Ratio $P$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ft |  | $f t$ | ft |  | ksi | $i n^{2}$ | K | K | $P_{a}$ |
| L1 | $150-96.8333$ <br> (1) | TP39.21x26.19x0.3125 | 53.17 | 0.00 | 0.0 | 39.000 | 37.7442 | -10.47 | 1472.02 | 0.007 |
| L2 | 96.8333-48 <br> (2) | TP50.55x37.1973x0.406 | 54.50 | 0.00 | 0.0 | 39.000 | 63.3122 | -21.29 | 2469.17 | 0.009 |
| L3 | 48-0 (3) | TP61.5×48.023×0.5 | 55.00 | 0.00 | 0.0 | 38.739 | 98.2100 | -40.47 | 3804.54 | 0.011 |


| Pole Bending Design Data |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Section No. | Elevation <br> $f t$ | Size | $\begin{gathered} \text { Actual } \\ M_{x} \\ \text { kip-ft } \end{gathered}$ | $\begin{gathered} \text { Actual } \\ f_{b x} \\ k s i \end{gathered}$ | Allow. $F_{b x}$ ksi | $\begin{gathered} \text { Ratio } \\ \frac{f_{b x}}{F_{b x}} \end{gathered}$ | Actual $M_{y}$ kip-ft | Actual $f_{b y}$ ksi | Allow. $F_{b y}$ ksi | Ratio $\frac{f_{b y}}{F_{b y}}$ |
| L1 | $150-96.8333$ <br> (1) | TP39.21x26.19x0.3125 | 516.47 | 17.913 | 39.000 | 0.459 | 0.00 | 0.000 | 39.000 | 0.000 |
| L2 | $\begin{gathered} 96.8333-48 \\ (2) \end{gathered}$ | TP50.55x37.1973x0.406 | $\begin{gathered} 1421.4 \\ 8 \end{gathered}$ | 22.767 | 39.000 | 0.584 | 0.00 | 0.000 | 39.000 | 0.000 |
| L3 | 48-0 (3) | TP61.5×48.023x0.5 | $\begin{gathered} 2843.3 \\ 2 \end{gathered}$ | 23.303 | 38.739 | 0.602 | 0.00 | 0.000 | 38.739 | 0.000 |

## Pole Shear Design Data

| Section No. | Elevation <br> $f t$ | Size | Actual <br> V <br> K | Actual $f_{v}$ ksi | Allow. $F_{v}$ ksi | $\begin{gathered} \text { Ratio } \\ f_{v} \\ \hline F_{v} \end{gathered}$ | $\begin{gathered} \text { Actual } \\ T \\ \text { kip-ft } \end{gathered}$ | Actual $f_{v t}$ ksi | Allow. $F_{v t}$ ksi | Ratio $\frac{f_{v t}}{F_{v t}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L1 | $150-96.8333$ <br> (1) | TP39.21x26.19x0.3125 | 15.87 | 0.420 | 26.000 | 0.033 | 0.33 | 0.005 | 26.000 | 0.000 |
| L2 | $\begin{gathered} 96.8333-48 \\ (2) \end{gathered}$ | TP50.55x37.1973×0.406 | 22.26 | 0.352 | 26.000 | 0.027 | 0.33 | 0.002 | 26.000 | 0.000 |
| L3 | 48-0 (3) | TP61.5x48.023x0.5 | 29.46 | 0.300 | 26.000 | 0.023 | 0.33 | 0.001 | 26.000 | 0.000 |


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| Section No. | Elevation <br> ft | Ratio $P$ $P_{a}$ | Ratio $\begin{gathered} f_{b x} \\ \hline F_{b x} \\ \hline \end{gathered}$ | Ratio $\begin{gathered} f_{b y} \\ \hline F_{b y} \\ \hline \end{gathered}$ | $\begin{gathered} \text { Ratio } \\ f_{v} \\ \hline F_{v} \\ \hline \end{gathered}$ | Ratio $\begin{gathered} f_{v t} \\ \hline F_{v t} \\ \hline \end{gathered}$ | Comb. Stress Ratio | Allow. Stress Ratio | Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L2 | $\begin{gathered} 96.8333-48 \\ (2) \end{gathered}$ | 0.009 | 0.584 | 0.000 | 0.027 | 0.000 | $0.593$ | 1.333 | H1-3+VT |
| L3 | 48-0 (3) | 0.011 | 0.602 | 0.000 | 0.023 | 0.000 | $0.612$ | 1.333 | H1-3+VT |

## Section Capacity Table

| Section No. | $\begin{aligned} & \text { Elevation } \\ & \mathrm{ft} \end{aligned}$ | Component Type | Size | Critical Element | $\begin{aligned} & \hline P \\ & K \end{aligned}$ | $\begin{gathered} S F^{*} P_{\text {allow }} \\ K \end{gathered}$ | $\begin{gathered} \% \\ \text { Capacity } \end{gathered}$ | Pass Fail |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L1 | 150-96.8333 | Pole | TP39.21×26.19×0.3125 | 1 | -10.47 | 1962.20 | 35.0 | Pass |
| L2 | 96.8333-48 | Pole | TP50.55x37.1973×0.406 | 2 | -21.29 | 3291.40 | 44.5 | Pass |
| L3 | 48-0 | Pole | TP61.5×48.023×0.5 | 3 | -40.47 | 5071.45 | 45.9 | Pass |
|  |  |  |  |  |  | Summary | ELC: | Load Case 7 |
|  |  |  |  |  |  | Pole (L3) | 45.9 | Pass |
|  |  |  |  |  |  | Rating = | 45.9 | Pass |

## APPENDIX B

## BASE LEVEL DRAWING



## APPENDIX C

## ADDITIONAL CALCULATIONS

## Stiffened or Unstiffened, Ungrouted, Circular Base Plate - Any Rod Material

TIA Rev F
Site Data
BU\#: 806370
Site Name: HRT 099943226
App \#: 208261 Rev. 3
Pole Manufacturer: Other

| Anchor Rod Data |  |  |  |  |
| ---: | :---: | :--- | :---: | :---: |
| Qty: | 24 |  |  |  |
| Diam: | 2.25 | in |  |  |
| Rod Material: | A615-J |  |  |  |
| Strength (Fu) | 100 |  |  | ksi |
| Yield (Fy): | 75 | ksi |  |  |
| Bolt Circle: | 70.17 | in |  |  |


| Plate Data |  |  |
| ---: | :---: | :--- |
| Diam: | 76.17 | in |
| Thick: | 3 | in |
| Grade: | 60 | ksi |
| Single-Rod B-eff: | 8.24 | in |


| Stiffener Data (Welding at both sides) |  |  |
| :---: | :---: | :---: |
| Config: | 0 | in ** |
| Weld Type: |  |  |
| Groove Depth: |  |  |
| Groove Angle: |  | degrees |
| Fillet H. Weld: |  | <-- Disregard |
| Fillet V. Weld: |  |  |
| Width: |  | in |
| Height: |  | in |
| Thick: |  | in |
| Notch: |  | in |
| Grade: |  | ksi |
| Weld str.: |  | ksi |


| Pole Data |  |  |
| ---: | :---: | :--- |
| Diam: | 61.5 | in |
| Thick: | 0.5 | in |
| Grade | 65 | ksi |
| \# of Sides: | 12 | "0" IF Round |
| Fu | 80 | ksi |
|  | 0 | " 0 " if None |


| Stress Increase Factor |  |
| :---: | :---: |
| ASIF: 1.333 |  |


| Reactions |  |  |
| ---: | :---: | :--- |
| Moment: | 2843 | $\mathrm{ft}-\mathrm{kips}$ |
| Axial: | 40 | kips |
| Shear: | 29 | kips |


| If No stiffeners, Criteria: | AISC ASD $<-O n l y ~ A p p l c a b l e ~ t o ~ U n s t i f f e n e d ~ C a s e s ~$ |
| :--- | :--- |

Anchor Rod Results
Maximum Rod Tension:
Allowable Tension:
Anchor Rod Stress Ratio:

## Base Plate Results

Base Plate Stress:
Allowable Plate Stress:
Base Plate Stress Ratio:

## n/a

## Stiffener Results

Horizontal Weld : n/a

Vertical Weld: /a

Plate Flex+Shear, fb/Fb+(fv/Fv)^2: n/a
Plate Tension+Shear, $\mathrm{ft} / \mathrm{Ft}+(\mathrm{fv} / \mathrm{Fv})^{\wedge} 2$ : $\mathrm{n} / \mathrm{a}$
Plate Comp. (AISC Bracket): n/a

## Pole Results

Pole Punching Shear Check: n/a

40.7\% Pass

| Flexural Check | Rigid |
| :---: | :---: |
| 18.1 ksi | Service ASD |
| 60.0 ksi | 0.75*Fy*ASIF |
| 30.1\% Pass | Y.L. Length: $33.79$ |


| Site Number | 806370 |
| :--- | ---: |
| Site Name | HRT 099 943226 |

## Caisson Analysis

| Pier Properties |  |  |  |
| :--- | ---: | ---: | ---: |
| Moment | $2843 \mathrm{kip}-\mathrm{ft}$ | Analysis Properties |  |
|  | 29 kip | TIA Code | F |
| Shear |  | Soil Safety Factor | 2.00 |
| Pier Diameter | 9.0 ft | Water Table Depth | 14.0 ft |
| Height Above Grade | 0.50 ft | 5.0 ft |  |
| Depth Below Grade | 24.50 ft | Ignored Soil Depth | Cohesion Based on |
| Donut Diameter | ft | Max Soil Capacity | PLS Caisson |
| Donut Depth | ft |  | $110 \%$ |


| Soil Properties |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Top of Soil <br> Layer <br> $(\mathrm{ft})$ | Layer <br> Thickness <br> $(\mathrm{ft})$ | Bottom of <br> Soil Layer <br> $(\mathrm{ft})$ | Soil Unit <br> Weight (pcf) | Cohesion <br> (psf) | Friction <br> Angle <br> (degrees) |
| Soil.Layer | Soil.Top | Soil.Thick | Soil.Bottom | Soil.Weight | Soil.Cohesion | Soil.Phi |
| 1 | 0.00 | 14 | 14.00 | 100 | 300 | 30 |
| 2 | 14.00 | 11 | 25.00 | 98 | 100 | 23 |
| 3 |  |  |  |  |  |  |
| 4 |  |  |  |  |  |  |
| 5 |  |  |  |  |  |  |
| 6 |  |  |  |  |  |  |
| 7 |  |  |  |  |  |  |
| 8 |  |  |  |  |  |  |
| 10 |  |  |  |  |  |  |


| Critical Depths Below Grade |  | Results |  |
| :--- | :--- | :--- | :---: |
| Rotation Axis | 15.56 ft | Soil Capacity |  |
| Zero Shear | 6.23 ft | Max Pier Moment |  |


| Moment At User Defined Depths Below Grade |  |  |
| :---: | :---: | :---: |
| kip-ft | kip- ft |  |
| kip-ft | kip- ft |  |

## Moment Capacity of Drilled Concrete Shaft (Caisson) for TIA Rev F or G

Note: Shaft assumed to have ties, not spiral, transverse reinforcing

Site Data
BU\#: 806370
Site Name: HRT 099943226
App \#: 208261 Rev. 3

| Enter Load Factors Below: |  |  |
| :---: | :---: | :---: |
| For M (WL) | 1.3 | <---- Enter Factor |
| For P (DL) | 1.3 | <---- Enter Factor |


| Pier Properties |  |
| :---: | :---: |
| Concrete: |  |
| Pier Diameter | 9.0 |
| Concrete Area = | $9160.9 \mathrm{in}^{2}$ |
| Reinforcement: |  |
| Clear Cover to Tie= | 3.00 in |
| Horiz. Tie Bar Size= | 3 |
| Vert. Cage Diameter = | 8.33 |
| Vert. Cage Diameter = | 99.98 in |
| Vertical Bar Size $=$ | 10 |
| Bar Diameter = | 1.27 |
| Bar Area = | $1.27 \mathrm{in}^{2}$ |
| Number of Bars = | 60 |
| As Total= | $76.2 \mathrm{in}^{2}$ |
| A s/ Aconc, Rho: | 0.0083 0.83\% |

ACI 10.5, ACI 21.10.4, and IBC 1810.
Min As for Flexural, Tension Controlled, Shafts:
$(3)^{*}($ Sqrt (f'c)/Fy: 0.0027
$200 /$ Fy: 0.0033

Minimum Rho Check:
Actual Req'd Min. Rho: $0.33 \%$
Provided Rho:
$=0.83 \%$

| Ref. Shaft Max Axial Capacities, $\phi$ Max(Pn or Tn): |  |  |
| :---: | :---: | :---: |
| $\begin{aligned} & \text { Max Pu }=(\varphi=0.65) \mathrm{Pn} . \\ & \text { Pn per ACl } 318(10-2) \end{aligned}$ |  |  |
|  | 14423.73 | kips |
| at $\mathrm{Mu}=(\phi=0.65) \mathrm{Mn}=$ | 10145.61 | ft-kips |
|  |  |  |
| Max Tu, ( $\varphi=0.9$ ) Tn= | 4114.8 | kips |
| at $\mathrm{Mu}=\phi=(0.90) \mathrm{Mn}=$ | 0.00 | ft-kips |


| Maximum Shaft Superimposed Forces |  |  |
| :---: | :---: | :---: |
| TIA Revision: | F | ft-kips (* Note) kips |
| Max. Service Shaft M: | 3020.738 |  |
| Max. Service Shaft P: | 40 |  |
| Max Axial Force Type: | Comp. |  |

(*) Note: Max Shaft Superimposed Moment does not necessarily equal to the shaft top reaction moment

| Load Factor | Shaft Factored Loads |  |
| :---: | :---: | :---: |
| 1.30 | Mu: | 3926.959 |
| ftt-kips |  |  |
|  | Pu: | 52 |


| Material Properties |  |  |
| :---: | :---: | :---: |
| Concrete Comp. strength, f'c = | 3000 | psi |
| Reinforcement yield strength, Fy = | 60 | ksi |
| Reinforcing Modulus of Elasticity, $\mathrm{E}=$ | 29000 | ksi |
| Reinforcement yield strain = | 0.00207 |  |
| Limiting compressive strain $=$ | 0.003 |  |
| ACl 318 Code |  |  |
| Select Analysis ACI Code= | 2005 |  |
| Seismic Properties |  |  |
| Seismic Design Category = | B |  |
| Seismic Risk = | Low |  |


| Solve <br> (Run) |
| :--- |
| <-- Press Upon Completing All Input |

## Results:

Governing Orientation Case: 2


Case 1
Dist. From Edge to Neutral Axis:


Case 2
Extreme Steel Strain, $\epsilon$ t:
21.17 in
0.0117
et $>0.0050$, Tension Controlled
Reduction Factor, $\varphi$ : 0.900

| Output Note: Negative Pu=Tension |  |  |
| ---: | :---: | :--- | :--- |
| For Axial Compression, $\varphi$ Pn $=$ Pu: | 52.00 | kips |
| Drilled Shaft Moment Capacity, $\varphi$ Mn: | $\mathbf{1 4 9 0 9 . 8 4}$ | ft-kips |
| Drilled Shaft Superimposed Mu: | 3926.96 | ft-kips |
|  |  |  |
| (Mu/ $\varphi$ Mn, Drilled Shaft Flexure CSR: | $\mathbf{2 6 . 3 \%}$ |  |

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# RADIO FREQUENCY FCC REGULATORY COMPLIANCE MAXIMUM PERMISSIBLE EXPOSURE (MPE) ASSESSMENT 

Sprint Existing Facility<br>Site ID: CT03XC091<br>HRT 099943226 (West Hartford Crown)<br>570 New Park Avenue<br>West Hartford, CT 06110<br>March 20, 2014

EBI Project Number: 62141241

March 20, 2014

Sprint
Attn: RF Engineering Manager
1 International Boulevard, Suite 800
Mahwah, NJ 07495
Re: Radio Frequency Maximum Permissible Exposure (MPE) Assessment for Site:
CT03XC091-HRT 099943226 (West Hartford Crown)
Site Total: $\mathbf{4 0 . 8 7 0 \%}$ - MPE \% in full compliance
EBI Consulting was directed to analyze the proposed upgrades to the existing Sprint facility located at 570 New Park Avenue, West Hartford, CT, for the purpose of determining whether the radio frequency (RF) exposure levels from the proposed Sprint equipment upgrades on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (\% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu \mathrm{W} / \mathrm{cm} 2$ ). The number of $\mu \mathrm{W} / \mathrm{cm} 2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR $1.1307(b)(1)-(b)(3)$, to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu \mathrm{W} / \mathrm{cm}^{2}$ ). The general population exposure limit for the cellular band ( 850 MHz Band) is approximately $567 \mu \mathrm{~W} / \mathrm{cm}^{2}$, and the general population exposure limit for the 1900 MHz and 2500 MHz bands band is $1000 \mu \mathrm{~W} / \mathrm{cm}^{2}$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## CALCULATIONS

Calculations were done for the proposed upgrades to the existing Sprint Wireless antenna facility located at 570 New Park Avenue, West Hartford, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65 . All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario. Actual values seen from this site will be dramatically less than those shown in this report. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all emissions were calculated using the following assumptions:

1) 7 channels in the 1900 MHz Band were considered for each sector of the proposed installation.
2) 1 channel in the 800 MHz Band was considered for each sector of the proposed installation
3) 2 channels in the 2500 MHz Band were considered for each sector of the proposed installation.
4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
5) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications was used in this direction.
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6) The antennas used in this modeling are the RFS APXVSPP18-C-A20 and the RFS APXVTMM-C-120. This is based on feedback from the carrier with regards to anticipated antenna selection. The RFS APXVSPP18-C-A20 has a 15.9 dBd gain value at its main lobe at 1900 MHz and 13.4 dBd at its main lobe for 850 MHz . The RFS APXVTMM-C-120 has a 15.9 dBd gain value at its main lobe at 2500 MHz . All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario.
7) The antenna mounting height centerline for the proposed antennas is $\mathbf{1 3 5}$ feet above ground level (AGL)
8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public threshold limits

|  | Site ID | CT03XC091- HRT 099943226 (West Hartford Crown) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Site Addresss | 570 New Park Avenue, West Hartford, CT 06110 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | Site Type | Monopole |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Sector 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Antenna Number | Antenna Make | Antenna Model | Radio Type | Frequency Band | Technology | Power <br> Out Per Channel (Watts) | Number of Channels | Composite Power | Antenna Gain in direction of sample point (dBd) | Antenna Height (ft) | analysis height | Cable Size | Cable Loss <br> (dB) | Additional Loss (dB) | ERP | Power <br> Density Percentage |
| 1a | RFS | APXVSPP18-C-A20 | RRH | 1900 MHz | CDMA / LTE | 20 | 7 | 140 | 15.9 | 135 | 129 | 1/2 " | 0.5 | 3 | 2432.9212 | 5.25599\% |
| 1a | RFS | APXVSPP18-C-A20 | RRH | 850 MHz | CDMA / LTE | 20 | 1 | 20 | 13.4 | 135 | 129 | 1/2" | 0.5 | 3 | 195.44744 | 0.74469\% |
| 1B | RFS | APXVTMM14-C-120 | RRH | 2500 MHz | CDMA / LTE | 20 | 2 | 40 | 13.4 | 135 | 129 | 1/2" | 0.5 | 3 | 390.89489 | 1.48937\% |
| Sector total Power Density Value: $7.490 \%$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Sector 2 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Antenna Number | Antenna Make | Antenna Model | Radio Type | Frequency Band | Technology | Power Out Per Channel (Watts) | Number of Channels | Composite Power | Antenna Gain in direction of sample point (dBd) | Antenna Height (ft) | analysis height | Cable Size | Cable Loss <br> (dB) | Additional Loss (dB) | ERP | Power <br> Density Percentage |
| 2a | RFS | APXVSPP18-C-A20 | RRH | 1900 MHz | CDMA / LTE | 20 | 7 | 140 | 15.9 | 135 | 129 | 1/2" | 0.5 | 3 | 2432.9212 | 5.25599\% |
| 2 a | RFS | APXVSPP18-C-A20 | RRH | 850 MHz | CDMA / LTE | 20 | 1 | 20 | 13.4 | 135 | 129 | 1/2" | 0.5 | 3 | 195.44744 | 0.74469\% |
| 2B | RFS | APXVTMM14-C-120 | RRH | 2500 MHz | CDMA / LTE | 20 | 2 | 40 | 13.4 | 135 | 129 | 1/2" | 0.5 | 3 | 390.89489 | 1.48937\% |
| Sector total Power Density Value: $7.490 \%$ |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Sector 3 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Antenna Number | Antenna Make | Antenna Model | Radio Type | Frequency Band | Technology | Power Out Per Channel (Watts) | Number of Channels | Composite Power | Antenna Gain in direction of sample point (dBd) | Antenna Height (ft) | analysis height | Cable Size | Cable Loss <br> (dB) | Additional Loss (dB) | ERP |  |
| 3a | RFS | APXVSPP18-C-A20 | RRH | 1900 MHz | CDMA / LTE | 20 | 7 | 140 | 15.9 | 135 | 129 | 1/2" | 0.5 | 3 | 2432.9212 | 5.25599\% |
| 3 a | RFS | APXVSPP18-C-A20 | RRH | 850 MHz | CDMA / LTE | 20 | 1 | 20 | 13.4 | 135 | 129 | 1/2" | 0.5 | 3 | 195.44744 | 0.74469\% |
| 3B | RFS | APXVTMM14-C-120 | RRH | 2500 MHz | CDMA / LTE | 20 | 2 | 40 | 13.4 | 135 | 129 | 1/2" | 0.5 | 3 | 390.89489 | 1.48937\% |
|  |  |  |  |  |  |  |  |  |  |  |  | Sector to | tal Power D | ensity Value: | 7.490\% |  |


| Site Composite MPE \% |  |
| :---: | :---: |
| Carrier | MPE \% |
| Sprint | $22.470 \%$ |
| Sensus (CL\&P) | $0.840 \%$ |
| Verizon Wireless | $16.710 \%$ |
| XM Satellite Radio | $0.850 \%$ |
| Total Site MPE \% | $40.870 \%$ |

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## Summary

All calculations performed for this analysis yielded results that were well within the allowable limits for general public Maximum Permissible Exposure (MPE) to radio frequency energy.

The anticipated Maximum Composite contributions from the Sprint facility are $\mathbf{2 2 . 4 7 0 \%}$ ( $\mathbf{7 . 4 9 0} \%$ from each sector) of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level.

The anticipated composite MPE value for this site assuming all carriers present is $\mathbf{4 0 . 8 7 0 \%}$ of the allowable FCC established general public limit sampled at 6 feet above ground level. This total composite site value is based upon MPE values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a $5 \%$ contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable $100 \%$ threshold standard per the federal government.


## Scott Heffernan

RF Engineering Director

EBI Consulting
21 B Street
Burlington, MA 01803

