



10 INDUSTRIAL AVE,
SUITE 3
MAHWAH NJ 07430

PHONE: 201.684.0055
FAX: 201.684.0066

May 12, 2020

Melanie A. Bachman
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

T-Mobile Northeast LLC – CTHA524A
Tower Share Application
1542 Boston Post Road, Westbrook, CT 06498
Latitude- 41.2818000000
Longitude- -72.4375000000

Dear Ms. Bachman,

This letter and the attachments are submitted on behalf of T-Mobile Northeast LLC (“T-Mobile”). T-Mobile plans to install antennas and related equipment at the tower site located at 1542 Boston Post Road in Westbrook, Connecticut (also known as 1546 Boston Post Road). This tower was approved by the Connecticut Siting Council via Docket No. 485 on August 25, 2019. The T-Mobile equipment is currently situated on the temporary tower at the same address that was approved by the Siting Council on December 7, 2017 via Petition No. 1330. The T-Mobile equipment on the temporary tower will be removed and relocated.

T-Mobile will install six (6) 600/700/1900/2100 MHz panel antennas, and three (3) RRUs at the 104’ level of the existing 130’ monopole tower. Three (3) hybrid cables will also be installed. T-Mobile’s equipment cabinets will be placed on a new concrete slab within the existing ground facility. The proposed ground equipment will include a 25kw diesel back-up generator. Included are plans by Centek Engineering, dated May 5, 2020, depicting the proposed site and attached as **Exhibit A**. Also included is a structural analysis prepared by Centek Engineering, dated May 5, 2020, confirming that the existing tower is structurally capable of supporting the proposed equipment. This is attached and detailed in **Exhibit B**. Additionally, an analysis of the proposed mounts is attached and incorporated as **Exhibit C**.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies 16-50aa, of T-Mobile’s intent to share a telecommunications facility pursuant to R.C.S.A. 16-50j-88. In accordance with R.C.S.A., a copy of this letter is being sent to Noel Bishop, First Selectmen of the Town of Westbrook, Eric Knapp, Planning, Zoning & Development Coordinator for the Town of Westbrook, the tower owner, MCM Communications, LLC, and the property owner, Connecticut Water Company. Please see the attached letter from MCM Communications, LLC authorizing the proposed shared use of this facility attached as **Exhibit D**.

The planned modifications of the facility fall squarely within those activities explicitly provided for in R.C.S.A. 16-50j-89.

1. The proposed modification will not result in an increase in the height of the existing structure. The top of the monopole tower is 130'; T-Mobile's proposed antennas will be located at a center line height of 104'.
2. The proposed modifications will not result in the increase of the site boundary, as depicted on the attached site plan.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed local and state criteria. T-Mobile's plans include the installation of an emergency back-up generator; noise associated with this installation is exempt from State and local noise standards. The incremental effect of the proposed changes will be negligible.
4. The operation of the proposed antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard. As indicated in the attached power density calculations, the site operations will result in a total power density of 6.18%, as evidenced by **Exhibit E**.

Connecticut General Statutes 16-50aa indicates that the Council must approve the shared use of a telecommunications facility provided it finds the shared use is technically, legally, environmentally, and economically feasible and meets public safety concerns. As demonstrated in this letter, T-Mobile respectfully submits that the shared use of this facility satisfies these criteria.

- A. Technical Feasibility. The existing monopole tower has been deemed structurally capable of supporting T-Mobile's proposed loading. The structural analysis is included as **Exhibit B**.
- B. Legal Feasibility. As referenced above, C.G.S. 16-50aa has been authorized to issue orders approving the shared use of an existing tower such as this monopole tower in Westbrook. Under the authority granted to the Council, an order of the Council approving the requested shared use would permit T-Mobile to obtain a building permit for the proposed installation. Further, a Letter of Authorization is included as **Exhibit D**, authorizing T-Mobile to file this application for shared use.
- C. Environmental Feasibility. The proposed shared use of this facility would have minimal environmental impact. The installation of T-Mobile equipment at the 104' level of the existing 130' tower would have an insignificant visual impact on the area around the tower. T-Mobile's ground equipment would be installed on a concrete slab within the existing facility compound. T-Mobile's shared use would therefore not cause any significant alteration in the physical or environmental characteristics of the existing site. Additionally, as evidenced by **Exhibit E**, the proposed antennas would not increase radio frequency emissions to a level at or above the Federal Communications Commission safety standard.
- D. Economic Feasibility. T-Mobile will be entering into an agreement with the owner of this facility to mutually agreeable terms. As previously mentioned, the Letter of Authorization has been provided by the owner to assist T-Mobile with this tower sharing application.
- E. Public Safety Concerns. As discussed above, the monopole tower is structurally capable of supporting T-Mobile's proposed loading. T-Mobile is not aware of any public safety concerns relative to the proposed sharing of the existing tower. T-Mobile's intentions of providing new and improved wireless service through the shared use of this facility is expected to enhance the safety and welfare of local residents and individuals traveling through Westbrook and nearby the facility.

Sincerely,

Kyle Richers

Kyle Richers
Transcend Wireless
10 Industrial Ave., Suite 3
Mahwah, New Jersey
krichers@transcendwireless.com
908-447-4716

CC: Noel Bishop- First Selectmen
Eric Knapp- Zoning Official
MCM Communications, LLC- Tower Owner
Connecticut Water Company- Property Owner

UPS Internet Shipping: View/Print Label

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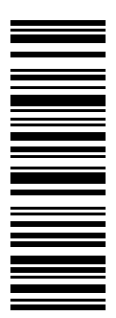
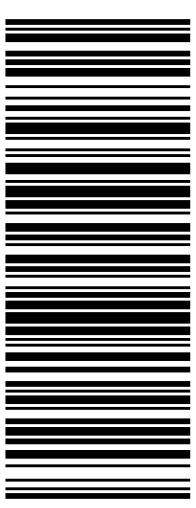

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<p>NEIL GUERRIERO 3473040176 TRANSCEND WIRELESS 10 INDUSTRIAL AVE MAHWAH NJ 07430</p> <p>SHIP TO: NOEL BISHOP TOWN OF WESTBROOK 866 BOSTON POST ROAD WESTBROOK CT 06498-1881</p>	<p style="text-align: right;">1 OF 1</p> <p style="text-align: center;">CT 063 5-02</p> 	<p style="text-align: center;">UPS GROUND</p> <p>TRACKING #: 1Z V25 742 42 9985 9406</p> 	<p style="text-align: center;"></p> <p style="text-align: center;">BILLING: P/P SIGNATURE REQUIRED</p> <p>Reference# 1: CTHA524A CSC EO</p> <p style="text-align: right; font-size: small;">UPS 22.0.11. WNTNVS0 25.0A 04/2020</p>
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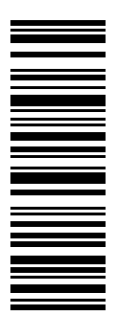
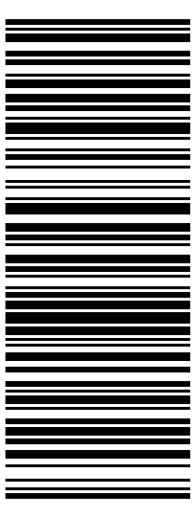

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<p>NEIL GUERRIERO 3473040176 TRANSCEND WIRELESS 10 INDUSTRIAL AVE MAHWAH NJ 07430</p> <p>SHIP TO: ERIC KNAPP TOWN OF WESTBROOK 866 BOSTON POST ROAD WESTBROOK CT 06498-1881</p>	<p style="text-align: right;">1 OF 1</p> <p style="text-align: center;">CT 063 5-02</p> 	<p style="text-align: center;">UPS GROUND</p> <p>TRACKING #: 1Z V25 742 42 9868 9413</p> 	<p style="text-align: center;">BILLING: P/P SIGNATURE REQUIRED</p> <p>Reference# 1: CTHA524A CSC ZO</p> <p style="text-align: right; font-size: small;">UPS 22.0.11. WNTNVS0 25.0A 04/2020</p> 
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<p>NEIL GUERRIERO 3473040176 TRANSCEND WIRELESS 10 INDUSTRIAL AVE MAHWAH NJ 07430</p> <p>SHIP TO: MCM COMMUNICATIONS, LLC 40 WOODLAND STREET HARTFORD CT 06105-2327</p>	<p style="text-align: right;">1 OF 1</p> <p style="text-align: center;">CT 061 9-03</p> 	<p style="text-align: center;">UPS GROUND</p> <p>TRACKING #: 1Z V25 742 42 9753 9425</p> 	<p style="text-align: center;">BILLING: P/P SIGNATURE REQUIRED</p> <p>Reference# 1: CTHA524A CSC PO</p> <p style="text-align: right; font-size: small;">UPS 22.0.11. WNTNVS0 25.0A 04/2020</p> 
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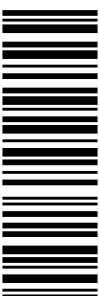
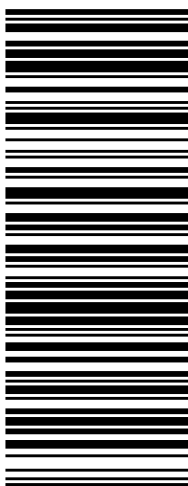

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<p>NEIL GUERRIERO 3473040176 TRANSCEND WIRELESS 10 INDUSTRIAL AVE MAHWAH NJ 07430</p> <p>SHIP TO: CONNECTICUT WATER COMPANY 93 W. MAIN STREET CLINTON CT 06413-1645</p>	<p style="text-align: right;">1 OF 1</p> <p style="text-align: center;">CT 065 2-01</p> 	<p style="text-align: center;">UPS GROUND</p> <p>TRACKING #: 1Z V25 742 42 9640 9433</p> 	<p style="text-align: center;">BILLING: P/P SIGNATURE REQUIRED</p> <p>Reference# 1: CTHA524A CSC 4</p> <p style="text-align: right; font-size: small;">UPS 22.0.11. WNTNVS0 25.0A 04/2020</p> 
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1542 BOSTON POST RD

Location 1542 BOSTON POST RD

Mblu 182 / / 007 / /

Acct# C0071600

Owner CONN WATER CO

Assessment \$325,720

Appraisal \$465,300

PID 770

Building Count 1

Current Value

Appraisal			
Valuation Year	Improvements	Land	Total
2016	\$64,020	\$401,280	\$465,300

Assessment			
Valuation Year	Improvements	Land	Total
2016	\$44,820	\$280,900	\$325,720

Owner of Record

Owner CONN WATER CO
Co-Owner
Address 93 W MAIN ST
CLINTON, CT 06413

Sale Price \$0
Certificate
Book & Page 15/287
Sale Date 01/01/1901
Instrument 25

Ownership History

Ownership History					
Owner	Sale Price	Certificate	Book & Page	Instrument	Sale Date
CONN WATER CO	\$0		15/287	25	01/01/1901

Building Information

Building 1 : Section 1

Year Built:
Living Area: 0
Replacement Cost: \$0
Building Percent Good:
Replacement Cost
Less Depreciation: \$0

Building Attributes

Field	Description
Style	Vacant Commercial
Model	
Grade:	
Stories	
Occupancy	
Exterior Wall 1	
Exterior Wall 2	
Roof Structure	
Roof Cover	
Interior Wall 1	
Interior Wall 2	
Interior Flr 1	
Interior Flr 2	
Heat Fuel	
Heat Type:	
AC Type:	
Total Bedrooms:	
Full Bthrms:	
Half Baths:	
Extra Fixtures	
Total Rooms:	
Bath Style:	
Kitchen Style:	
Extra Kitchens	
Fireplace(s)	
Usrflid 103	
Gas Fireplace(s)	
Stacks	
Bsmt Garage(s)	
Usrflid 107	
Callback	
Fireplaces	
Fin Bsmnt	
Fin Bsmnt Qual	
Bsmt Heat	
Int Vs Ext	
Usrflid 300	
Usrflid 301	

Building Photo



(<http://images.vgsi.com/photos2/WestbrookCTPhotos/\00\00\84\50.jpg>)

Building Layout

(http://images.vgsi.com/photos2/WestbrookCTPhotos//Sketches/770_770.j)

Building Sub-Areas (sq ft)	Legend
No Data for Building Sub-Areas	

Extra Features	<u>Legend</u>
No Data for Extra Features	

Land

Land Use	Land Line Valuation
Use Code 200	Size (Acres) 0.61
Description Comm Land	Depth
Zone NCD	Assessed Value \$280,900
Neighborhood BP	Appraised Value \$401,280
Alt Land Appr No	
Category	

Outbuildings

Outbuildings							<u>Legend</u>
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #	Comment
TCM	Telecomm			360.00 S.F.&HGT	\$10,710	1	
TCS	Telecomm Site			1.00 UNITS	\$540	1	
TCS	Telecomm Site			1.00 UNITS	\$540	1	
TCS	Telecomm Site			1.00 UNITS	\$540	1	
TCM	Telecomm			1.00 S.F.&HGT	\$20	1	
TCS	Telecomm Site			0.00 UNITS	\$51,670	1	

Valuation History

Appraisal			
Valuation Year	Improvements	Land	Total
2019	\$64,020	\$401,280	\$465,300
2018	\$64,020	\$401,280	\$465,300
2017	\$587,350	\$401,280	\$988,630

Assessment			
Valuation Year	Improvements	Land	Total
2019	\$44,820	\$280,900	\$325,720
2018	\$44,820	\$280,900	\$325,720
2017	\$411,150	\$280,900	\$692,050

CTHA524A



Property Information

Property ID 182/007
Location 1542 BOSTON POST RD
Owner CONN WATER CO



MAP FOR REFERENCE ONLY NOT A LEGAL DOCUMENT

Town of Westbrook, CT makes no claims and no warranties, expressed or implied, concerning the validity or accuracy of the GIS data presented on this map.

Geometry updated October 2018
 Data updated 11/19/2018

DOCKET NO. 485 – MCM Holdings, LLC and Cellco Partnership d/b/a Verizon Wireless application for a Certificate of Environmental Compatibility and Public Need for the construction, maintenance, and operation of a telecommunications facility at one of two sites: 1542 Boston Post Road or at the end of Kirtland Street, Westbrook, Connecticut. } Connecticut
} Siting
} Council

August 15, 2019

Decision and Order

Pursuant to Connecticut General Statutes §16-50p, and the foregoing Findings of Fact and Opinion, the Connecticut Siting Council (Council) finds that the effects associated with the construction, maintenance, and operation of a telecommunications facility, including effects on the natural environment, ecological balance, public health and safety, scenic, historic, and recreational values, agriculture, forests and parks, air and water purity, and fish, aquaculture and wildlife are not disproportionate, either alone or cumulatively with other effects, when compared to need, are not in conflict with the policies of the State concerning such effects, and are not sufficient reason to deny the application, and therefore directs that a Certificate of Environmental Compatibility and Public Need, as provided by General Statutes § 16-50k, be issued to MCM Holdings, LLC, hereinafter referred to as the Certificate Holder, for a telecommunications facility at Site A, located at 1542 Boston Post Road, Westbrook, Connecticut. The Council denies certification of Site B, located at the end of Kirtland Street, Westbrook, Connecticut.

Unless otherwise approved by the Council, the Site A facility shall be constructed, operated, and maintained substantially as specified in the Council's record in this matter, and subject to the following conditions:

1. The tower shall be constructed as a monopole at a height of 130 feet above ground level to provide the proposed wireless services, sufficient to accommodate the antennas of Cellco Partnership d/b/a Verizon Wireless and other entities, both public and private. The height of the tower may be extended after the date of this Decision and Order pursuant to regulations of the Federal Communications Commission.
2. The Certificate Holder shall prepare a Development and Management (D&M) Plan for this site in compliance with Sections 16-50j-75 through 16-50j-77 of the Regulations of Connecticut State Agencies. The D&M Plan shall be submitted to and approved by the Council prior to the commencement of facility construction and shall include:
 - a) final site plan(s) for development of the facility that employ the governing standard in the State of Connecticut for tower design in accordance with the currently adopted International Building Code and include specifications for the tower, tower foundation, antennas and any modifications to the equipment compound including, but not limited to, fencing, radio equipment, access road, utility installation and emergency backup generator;
 - b) the tower shall be designed with a yield point to ensure that the tower setback radius remains within the boundaries of the subject property;
 - c) construction plans for site clearing, grading, water drainage and stormwater control, and erosion and sedimentation controls consistent with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control, as amended;
 - d) provisions for landscaping;
 - e) proposed hours and days of the week for construction activities; and
 - f) a schedule for the removal of the existing temporary tower.

3. Prior to the commencement of operation, the Certificate Holder shall provide the Council worst-case modeling of the electromagnetic radio frequency power density of all proposed entities' antennas at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin No. 65, August 1997. The Certificate Holder shall ensure a recalculated report of the electromagnetic radio frequency power density be submitted to the Council if and when circumstances in operation cause a change in power density above the levels calculated and provided pursuant to this Decision and Order.
4. Upon the establishment of any new federal radio frequency standards applicable to frequencies of this facility, the facility granted herein shall be brought into compliance with such standards.
5. The Certificate Holder shall provide the Council with a copy of necessary permits from any other state or federal agency with concurrent jurisdiction prior to the commencement of construction.
6. The Certificate Holder shall permit public or private entities to share space on the proposed tower for fair consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing.
7. Unless otherwise approved by the Council, if the facility authorized herein is not fully constructed with at least one fully operational wireless telecommunications carrier providing wireless service within eighteen months from the date of the mailing of the Council's Findings of Fact, Opinion, and Decision and Order (collectively called "Final Decision"), this Decision and Order shall be void, and the Certificate Holder shall dismantle the tower and remove all associated equipment or reapply for any continued or new use to the Council before any such use is made. The time between the filing and resolution of any appeals of the Council's Final Decision shall not be counted in calculating this deadline. Authority to monitor and modify this schedule, as necessary, is delegated to the Executive Director. The Certificate Holder shall provide written notice to the Executive Director of any schedule changes as soon as is practicable.
8. Any request for extension of the time period referred to in Condition 7 shall be filed with the Council not later than 60 days prior to the expiration date of this Certificate and shall be served on all parties and intervenors, as listed in the service list, and the Town of Westbrook.
9. If the facility ceases to provide wireless services for a period of one year, this Decision and Order shall be void, and the Certificate Holder shall dismantle the tower and remove all associated equipment or reapply for any continued or new use to the Council within 90 days from the one year period of cessation of service. The Certificate Holder may submit a written request to the Council for an extension of the 90 day period not later than 60 days prior to the expiration of the 90 day period.
10. Any nonfunctioning antenna, and associated antenna mounting equipment, on this facility shall be removed within 60 days of the date the antenna ceased to function.
11. In accordance with Section 16-50j-77 of the Regulations of Connecticut State Agencies, the Certificate Holder shall provide the Council with written notice two weeks prior to the commencement of site construction activities. In addition, the Certificate Holder shall provide the Council with written notice of the completion of site construction, and the commencement of site operation.
12. The Certificate Holder shall remit timely payments associated with annual assessments and invoices submitted by the Council for expenses attributable to the facility under Conn. Gen. Stat. §16-50v.

13. This Certificate may be transferred in accordance with Conn. Gen. Stat. §16-50k(b), provided both the Certificate Holder/transferor and the transferee are current with payments to the Council for their respective annual assessments and invoices under Conn. Gen. Stat. §16-50v. In addition, both the Certificate Holder/transferor and the transferee shall provide the Council a written agreement as to the entity responsible for any quarterly assessment charges under Conn. Gen. Stat. §16-50v(b)(2) that may be associated with this facility.
14. The Certificate Holder shall maintain the facility and associated equipment, including but not limited to, the tower, tower foundation, antennas, equipment compound, radio equipment, access road, utility line and landscaping in a reasonable physical and operational condition that is consistent with this Decision and Order and a Development and Management Plan to be approved by the Council.
15. If the Certificate Holder is a wholly-owned subsidiary of a corporation or other entity and is sold/transferred to another corporation or other entity, the Council shall be notified of such sale and/or transfer and of any change in contact information for the individual or representative responsible for management and operations of the Certificate Holder within 30 days of the sale and/or transfer.
16. This Certificate may be surrendered by the Certificate Holder upon written notification and acknowledgment by the Council.

We hereby direct that a copy of the Findings of Fact, Opinion, and Decision and Order be served on each person listed in the Service List, dated April 4, 2019, and notice of issuance published in the *Harbor News*.

By this Decision and Order, the Council disposes of the legal rights, duties, and privileges of each party named or admitted to the proceeding in accordance with Section 16-50j-17 of the Regulations of Connecticut State Agencies.

Exhibit A
Construction Drawings



WIRELESS COMMUNICATIONS FACILITY

WESTBROOK SOUTH
 SITE ID: CTHA524A
 1542 BOSTON POST ROAD
 WESTBROOK, CT 06498

T-MOBILE RF CONFIGURATION
 67D97DB_1xAIR+1OP

PROJECT SUMMARY

1. THE PROPOSED SCOPE OF WORK CONSISTS OF A MODIFICATION TO THE EXISTING UNMANNED TELECOMMUNICATIONS FACILITY INCLUDING THE FOLLOWING:

- A. REMOVE (6) EXISTING ANTENNAS, TYP. (2) PER SECTOR
- B. INSTALL (3) NEW RFS ANTENNAS, TYP. (1) PER SECTOR
- C. INSTALL (3) NEW AIR32 ANTENNAS, TYP. (1) SECTOR
- D. INSTALL (3) NEW RADIO 4449 B71+B12, TYP. (1) PER SECTOR
- E. REMOVE EXISTING ANTENNA MOUNT
- F. INSTALL (1) NEW QUAD ANTENNA MOUNTING PLATFORM
- G. INSTALL (3) NEW 6x12 HYBRID CABLES
- H. REMOVE EXISTING UTILITY CABINET
- I. INSTALL NEW PPC CABINET
- J. REMOVE EXISTING ODE EQUIPMENT CABINET FOR NEW ERICSSON RBS 6102
- K. EXPAND EXISTING CONC. SLAB-ON-GRADE TO A 10'X20' CONC. SLAB-ON-GRADE
- L. INSTALL NEW COAX CABLE ICE BRIDGE
- M. INSTALL NEW AC DC DIESEL GENERATOR

PROJECT INFORMATION

SITE NAME: WESTBROOK SOUTH
 SITE ID: CTHA524A
 SITE ADDRESS: 1542 BOSTON POST ROAD WESTBROOK, CT 06498
 APPLICANT: T-MOBILE NORTHEAST, LLC 35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002
 CONTACT PERSON: DAN REID (PROJECT MANAGER) TRANSCEND WIRELESS, LLC (203) 592-8291
 ENGINEER: CENTEK ENGINEERING, INC. 63-2 NORTH BRANFORD RD. BRANFORD, CT 06405
 PROJECT COORDINATES: LATITUDE: 41°-16'-55.01" N LONGITUDE: 72°-26'-14.78" W GROUND ELEVATION: 50± AMSL
 SITE COORDINATES AND GROUND ELEVATION REFERENCED FROM GOOGLE EARTH.

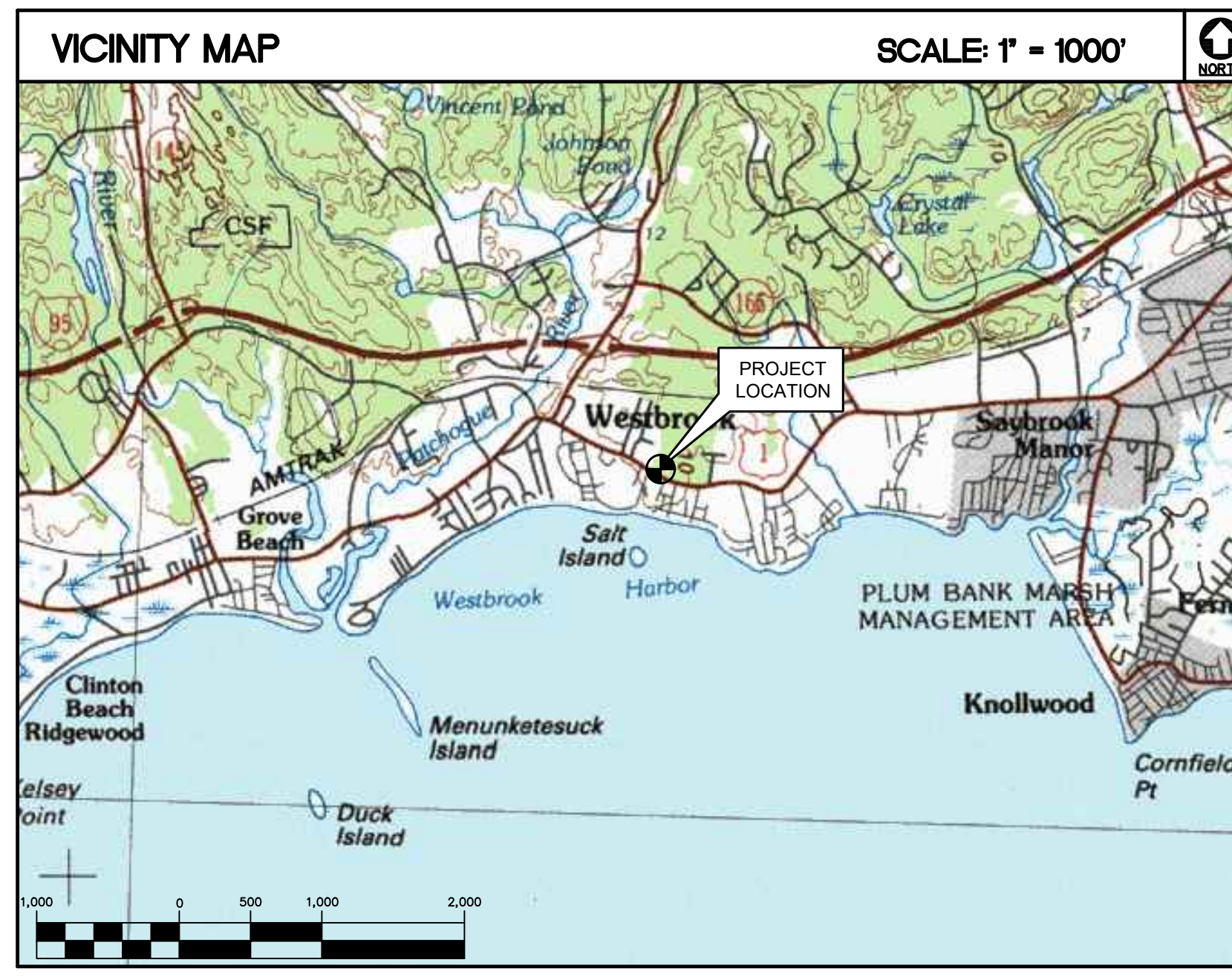
SHEET INDEX

SHT. NO.	DESCRIPTION	REV.
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SITE DIRECTIONS

FROM: 35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 **TO:** 1542 BOSTON POST ROAD WESTBROOK, CT 06498

- HEAD NORTH ON GRIFFIN ROAD S. TOWARD HARTMAN RD. 0.30 MI.
- TAKE THE 2ND RIGHT ONTO DAY HILL RD. 0.14 MI.
- TAKE THE 1ST RIGHT ONTO BLUE HILLS AVENUE EXT/CT-187. CONTINUE TO FOLLOW CT-187. 0.64 MI.
- STAY STRAIGHT TO GO ONTO BLUE HILLS AVE/CT-187. 1.24 MI.
- TURN LEFT ONTO OLD WINDSOR RD/CT-305. CONTINUE TO FOLLOW CT-305. 2.33 MI.
- MERGE ONTO I-91 S TOWARD HARTFORD. 16.58 MI.
- MERGE ONTO CT-9 S VIA EXIT 22S ON THE LEFT TOWARD MIDDLETOWN/OLD SAYBROOK. 29.24 MI.
- MERGE ONTO I-95 S TOWARD N Y CITY/NEW HAVEN. 3.48 MI.
- TAKE THE CT-166/SPENCER PLAIN ROAD EXIT, EXIT 66. 0.32 MI.
- TURN LEFT ONTO SPENCER PLAIN RD/CT-166. 0.39 MI.
- TURN RIGHT ONTO BOSTON POST RD/US-1 S. 1.32 MI.
- 1542 BOSTON POST RD, WESTBROOK, CT IS ON THE RIGHT.



GENERAL NOTES

- ALL WORK SHALL BE IN ACCORDANCE WITH THE 2015 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2018 CONNECTICUT SUPPLEMENT, INCLUDING THE TIA/EIA-222 REVISION "G" "STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND SUPPORTING STRUCTURES." 2017 CONNECTICUT FIRE SAFETY CODE, NATIONAL ELECTRICAL CODE AND LOCAL CODES.
- CONTRACTOR SHALL REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. CONTRACTOR SHALL COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS TO ALL SUBCONTRACTORS AND ALL RELATED PARTIES. THE SUBCONTRACTORS SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT AFFECTS THEIR WORK.
- CONTRACTOR SHALL PROVIDE A COMPLETE BUILD-OUT WITH ALL FINISHES, STRUCTURAL, MECHANICAL, AND ELECTRICAL COMPONENTS AND PROVIDE ALL ITEMS AS SHOWN OR INDICATED ON THE DRAWINGS OR IN THE WRITTEN SPECIFICATIONS.
- CONTRACTOR SHALL FURNISH ALL MATERIAL, LABOR AND EQUIPMENT TO COMPLETE THE WORK AND FURNISH A COMPLETED JOB ALL IN ACCORDANCE WITH LOCAL AND STATE GOVERNING AUTHORITIES AND OTHER AUTHORITIES HAVING LAWFUL JURISDICTION OVER THE WORK.
- CONTRACTOR SHALL SECURE AND PAY FOR ALL PERMITS AND ALL INSPECTIONS REQUIRED AND SHALL ALSO PAY FEES REQUIRED FOR THE GENERAL CONSTRUCTION, PLUMBING, ELECTRICAL AND HVAC. PERMITS SHALL BE PAID FOR BY THE RESPECTIVE SUBCONTRACTORS.
- CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS ON SITE AT ALL TIMES AND INSURE DISTRIBUTION OF NEW DRAWINGS TO SUBCONTRACTORS AND OTHER RELEVANT PARTIES AS SOON AS THEY ARE MADE AVAILABLE. ALL OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONTRACT AREA. THE CONTRACTOR SHALL FURNISH AN "AS-BUILT" SET OF DRAWINGS TO OWNER UPON COMPLETION OF PROJECT.
- LOCATION OF EQUIPMENT, AND WORK SUPPLIED BY OTHERS THAT IS DIAGRAMMATICALLY INDICATED ON THE DRAWINGS SHALL BE DETERMINED BY THE CONTRACTOR. THE CONTRACTOR SHALL DETERMINE LOCATIONS AND DIMENSIONS SUBJECT TO STRUCTURAL CONDITIONS AND WORK OF THE SUBCONTRACTORS.
- THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY.
- DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
- ALL UTILITY WORK SHALL BE IN ACCORDANCE WITH LOCAL UTILITY COMPANY REQUIREMENTS AND SPECIFICATIONS.
- ALL EQUIPMENT AND PRODUCTS PURCHASED ARE TO BE REVIEWED BY CONTRACTOR AND ALL APPLICABLE SUBCONTRACTORS FOR ANY CONDITION PER MFR.'S RECOMMENDATIONS. CONTRACTOR TO SUPPLY THESE ITEMS AT NO COST TO OWNER OR CONSTRUCTION MANAGER.
- ANY AND ALL ERRORS, DISCREPANCIES, AND "MISSED" ITEMS ARE TO BE BROUGHT TO THE ATTENTION OF THE T-MOBILE CONSTRUCTION MANAGER DURING THE BIDDING PROCESS BY THE CONTRACTOR. ALL THESE ITEMS ARE TO BE INCLUDED IN THE BID. NO "EXTRA" WILL BE ALLOWED FOR MISSED ITEMS.
- CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ON-SITE SAFETY FROM THE TIME THE JOB IS AWARDED UNTIL ALL WORK IS COMPLETE AND ACCEPTED BY THE OWNER.
- CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE CONSTRUCTION MANAGER FOR REVIEW.
- THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND EXISTING CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA.
- COORDINATION, LAYOUT, FURNISHING AND INSTALLATION OF CONDUIT AND ALL APPURTENANCES REQUIRED FOR PROPER INSTALLATION OF ELECTRICAL AND TELECOMMUNICATION SERVICE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
- THE CONTRACTOR SHALL CONTACT "CALL BEFORE YOU DIG" AT LEAST 48 HOURS PRIOR TO ANY EXCAVATIONS AT 1-800-922-4455. ALL UTILITIES SHALL BE IDENTIFIED AND CLEARLY MARKED. CONTRACTOR SHALL MAINTAIN AND PROTECT MARKED UTILITIES THROUGHOUT PROJECT COMPLETION.
- CONTRACTOR SHALL COMPLY WITH OWNERS ENVIRONMENTAL ENGINEER ON ALL METHODS AND PROVISIONS FOR ALL EXCAVATION ACTIVITIES INCLUDING SOIL DISPOSAL. ALL BACKFILL MATERIALS TO BE PROVIDED BY THE CONTRACTOR.

REVISIONS	DATE	BY	CHK'D BY	DESCRIPTION
C	05/05/20	KANUR		PRELIMINARY CD - REVISED PER CLIENT COMMENTS
D	04/28/20	KANUR		PRELIMINARY CD - REVISED PER CLIENT COMMENTS
A	04/15/20	KANUR		PRELIMINARY CD - REVISED PER CLIENT COMMENTS
A				ISSUED FOR CLIENT REVIEW

PROFESSIONAL ENGINEER SEAL

T-Mobile

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 (203) 488-0380
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 632 North Branford Road
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T-MOBILE NORTHEAST LLC
 WIRELESS COMMUNICATIONS FACILITY
WESTBROOK SOUTH
SITE ID: CTHA524A
 1542 BOSTON POST ROAD
 WESTBROOK, CT 06498

DATE: 04/01/20
 SCALE: AS NOTED
 JOB NO. 19027.46

TITLE SHEET

T-1

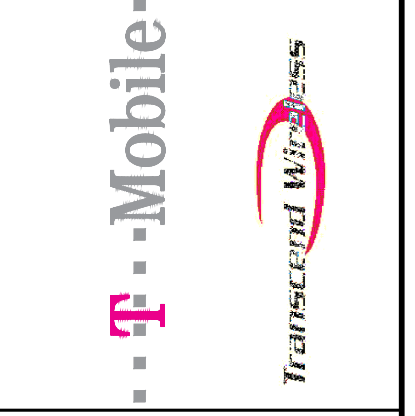
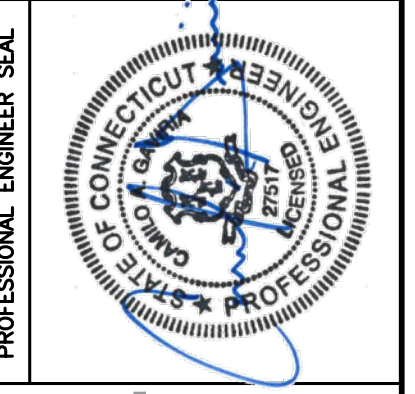
Sheet No. 1 of 10



1 SITE LOCATION PLAN
C-1 SCALE: NOT TO SCALE



REV.	DATE	DRAWN BY	CHK'D BY	DESCRIPTION
C	05/05/20	KAWIR	CAG	PRELIMINARY CD - REVISED PER CLIENT COMMENTS
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A	04/15/20	KAWIR	CAG	PRELIMINARY CD - ISSUED FOR CLIENT REVIEW



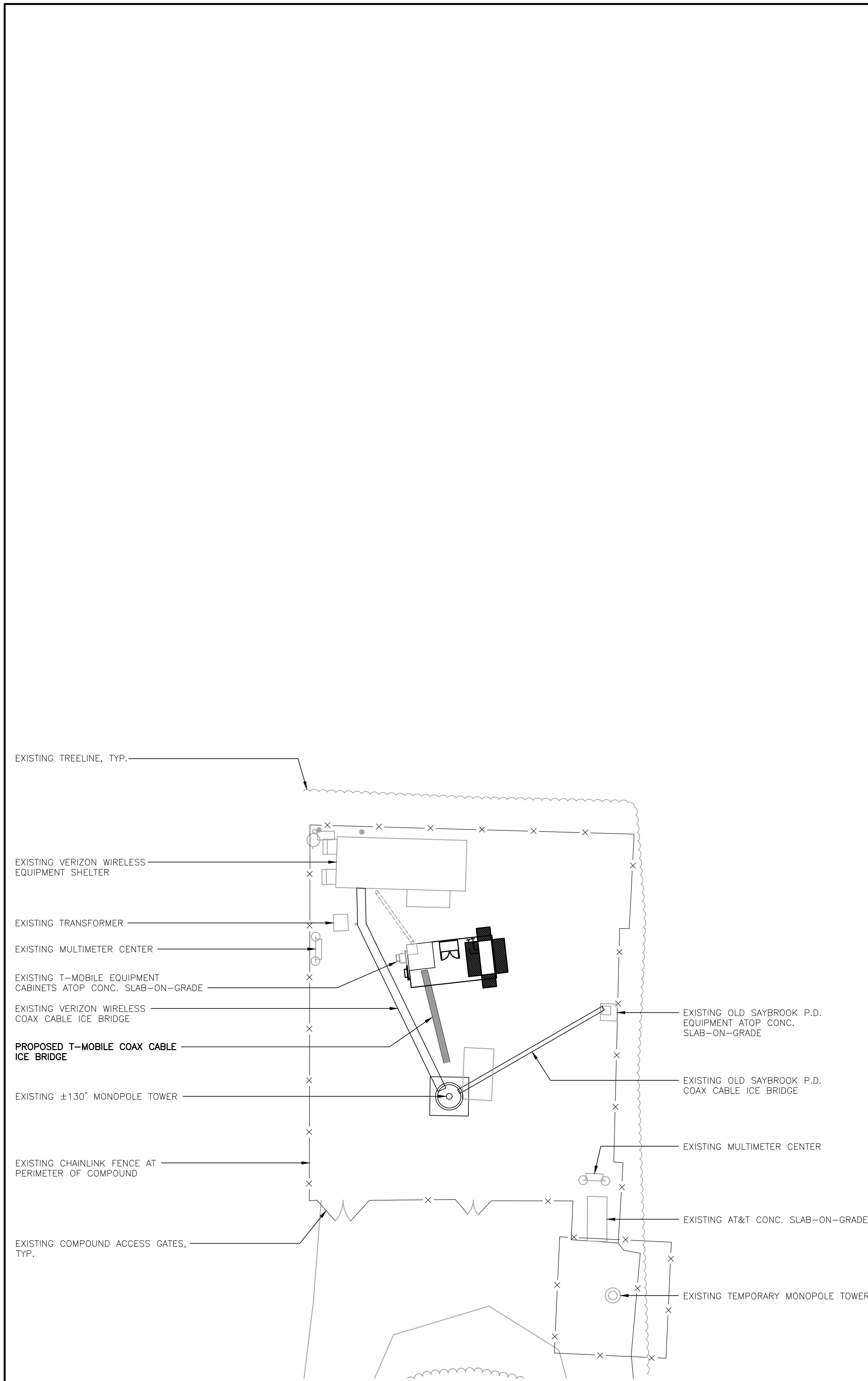
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SITE LOCATION PLAN

C-1
Sheet No. 3 of 10



1 **COMPOUND PLAN**
 C-2 SCALE: 1" = 15'
 APPROXIMATE NORTH

TOP OF EXISTING WHIP ANTENNA
 EL. ±136'-8" A.G.L.

TOP OF EXISTING MONOPOLE TOWER
 EL. ±130'-0" A.G.L.

TOP OF T-MOBILE ANTENNAS
 EL. ±108'-0" A.G.L.

Q OF T-MOBILE ANTENNAS
 EL. ±104'-0" A.G.L.

MOUNT ANALYSIS NOTE:
 REFER TO MOUNT ANALYSIS REPORT PREPARED BY CENTEK ENGINEERING INC. DATED 05/04/20

STRUCTURAL NOTE:
 REFER TO STRUCTURAL ANALYSIS REPORT OF TOWER AND FOUNDATION AS PREPARED BY CENTEK ENGINEERING INC. DATED 05/05/20

EXISTING T-MOBILE COAX CABLES, TYP.
 • INSTALL (3) NEW 6x12 HYBRID CABLES

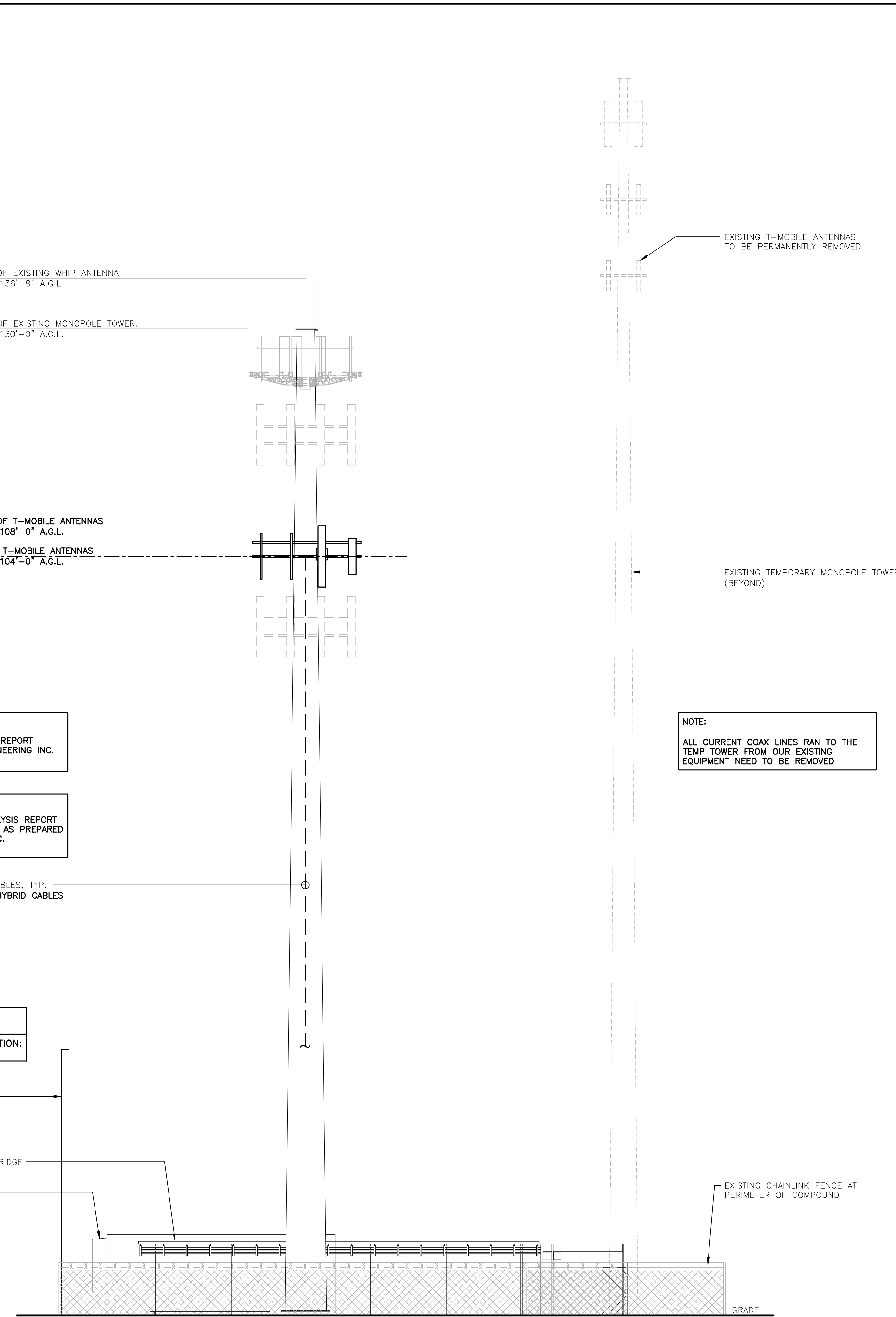
T-MOBILE RAN TEMPLATE:
 67D97DB MUAC

T-MOBILE RF CONFIGURATION:
 67D97DB_1xAIR+10P

EXISTING SNET POLE #664

EXISTING COAX CABLE ICE BRIDGE

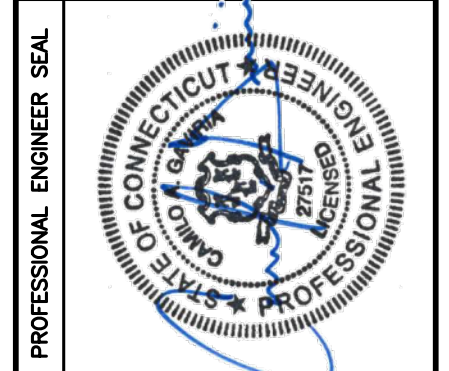
EXISTING VERIZON WIRELESS EQUIPMENT SHELTER



2 **SOUTH ELEVATION - PROPOSED**
 C-2 SCALE: 1/8" = 1'-0"

NOTE:
 ALL CURRENT COAX LINES RAN TO THE TEMP TOWER FROM OUR EXISTING EQUIPMENT NEED TO BE REMOVED

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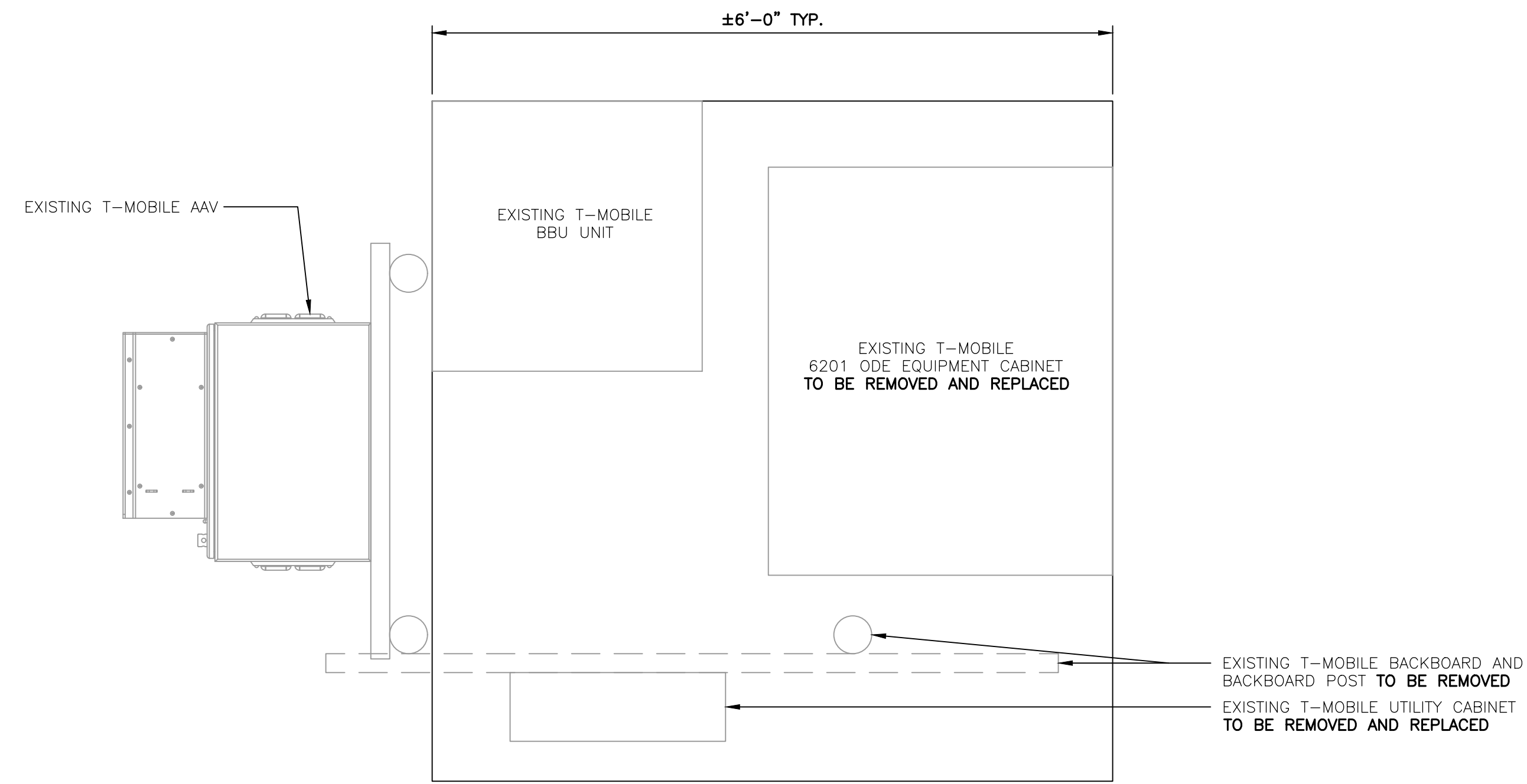
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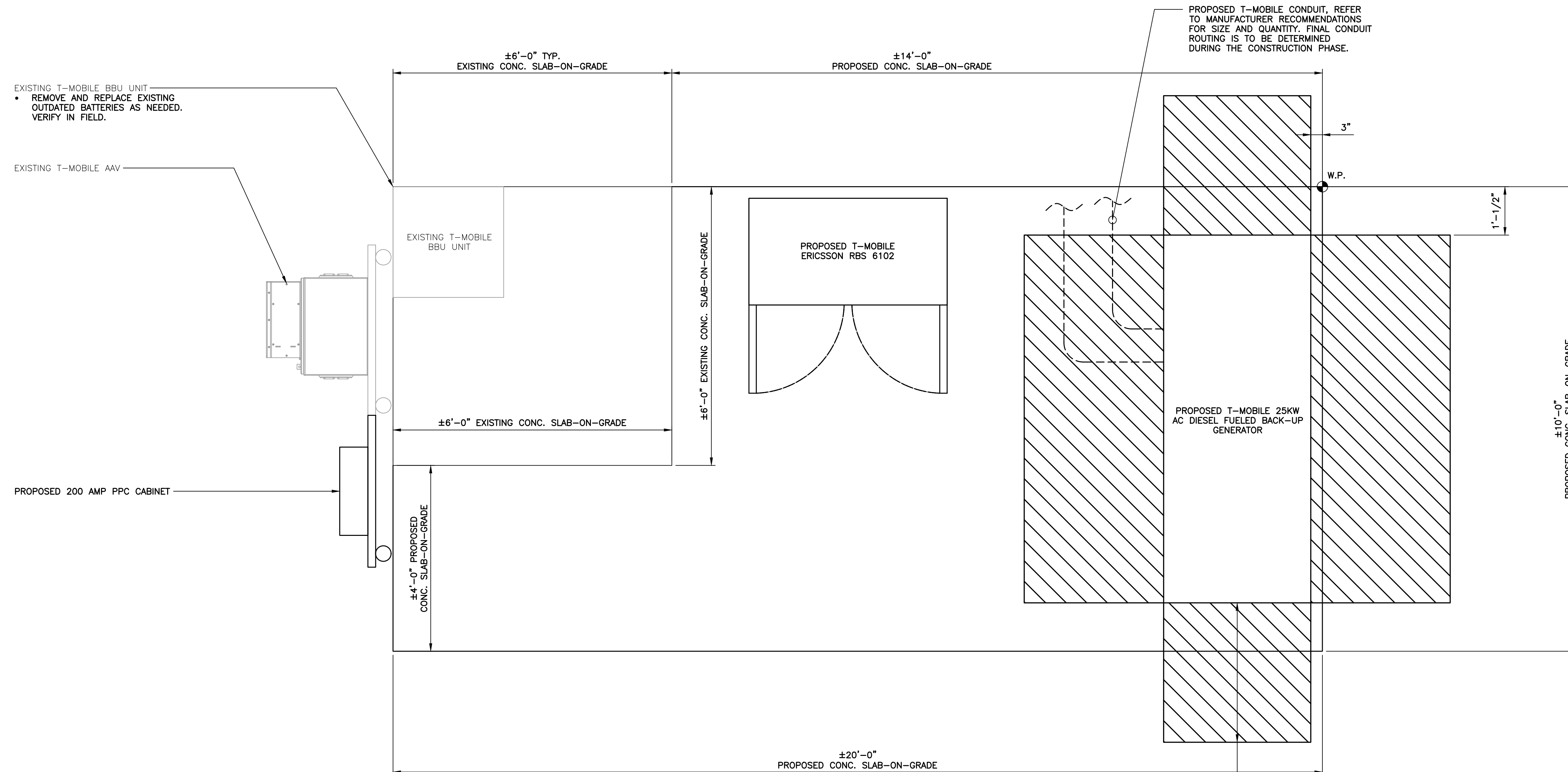
DATE: 04/01/20
 SCALE: AS NOTED
 JOB NO. 19027.46

COMPOUND PLAN
 &
 AND ELEVATION

C-2
 Sheet No. 4 of 10



1
C-2 **EQUIPMENT LAYOUT PLAN - EXISTING**
SCALE: 1" = 1'-0"

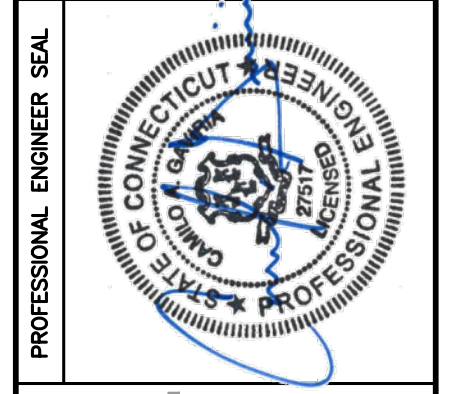


2
C-2 **EQUIPMENT LAYOUT PLAN - PROPOSED**
SCALE: 3/4" = 1'-0"



36" MIN. SIDE CLEARANCE REQUIRED (TYP.)

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A	04/15/20	KAWIR	CAG	PRELIMINARY CD - ISSUED FOR CLIENT REVIEW

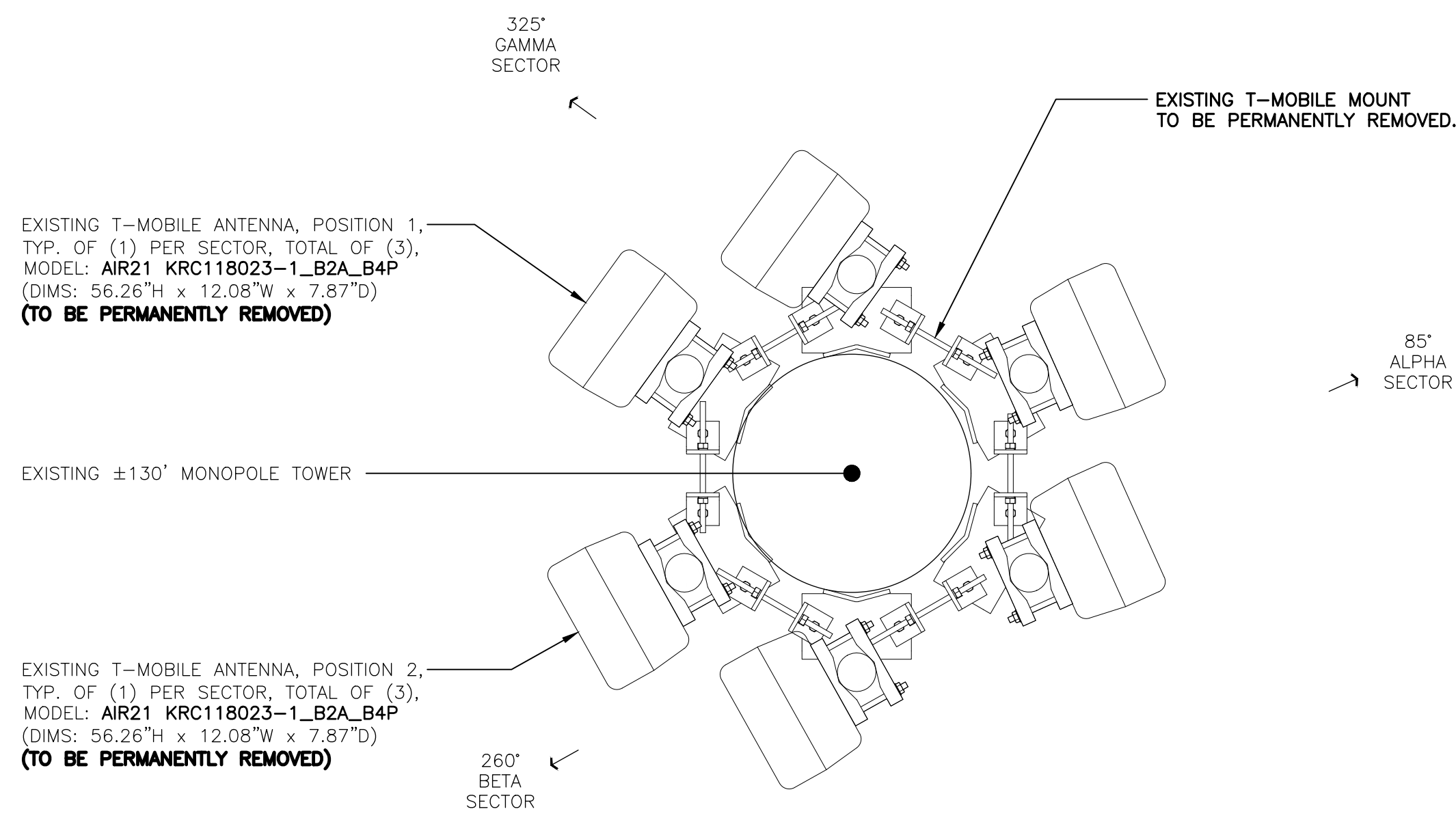


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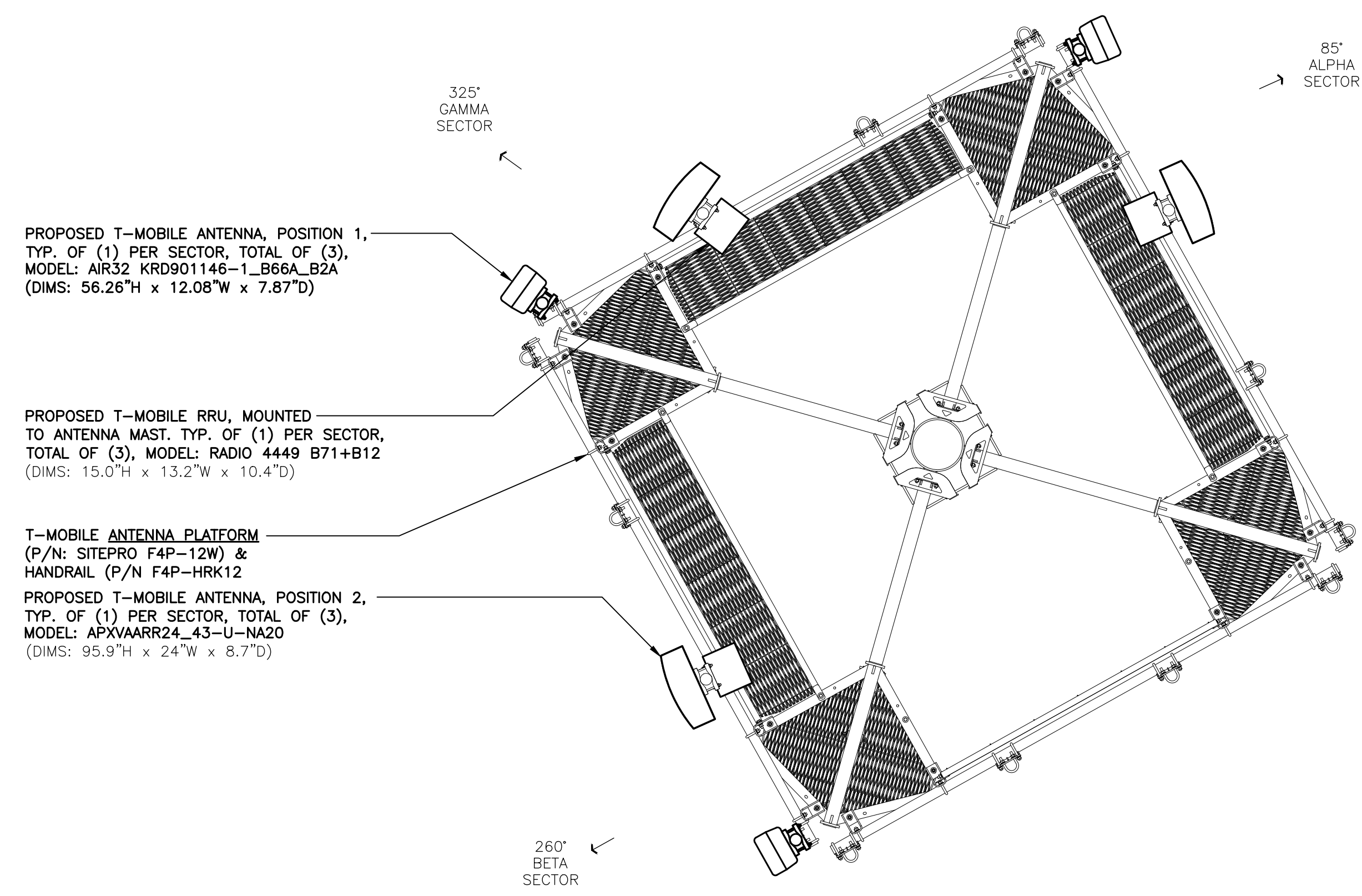
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WESTBROOK, CT 06498

DATE: 04/01/20
SCALE: AS NOTED
JOB NO. 19027.46

EQUIPMENT PLAN

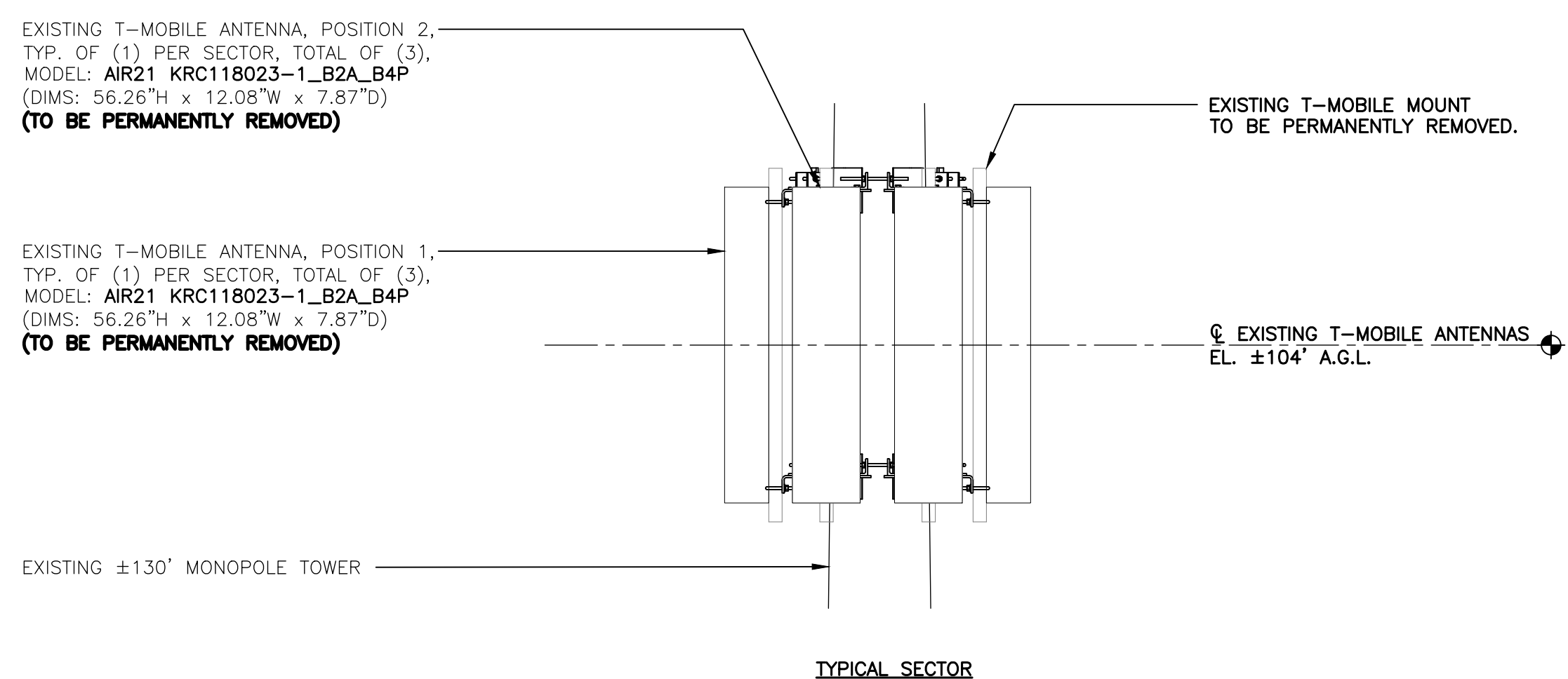


1 EXISTING ANTENNA MOUNTING CONFIGURATION TYP.
 SCALE: 1" = 1'-0"
 104'-0" ELEVATION

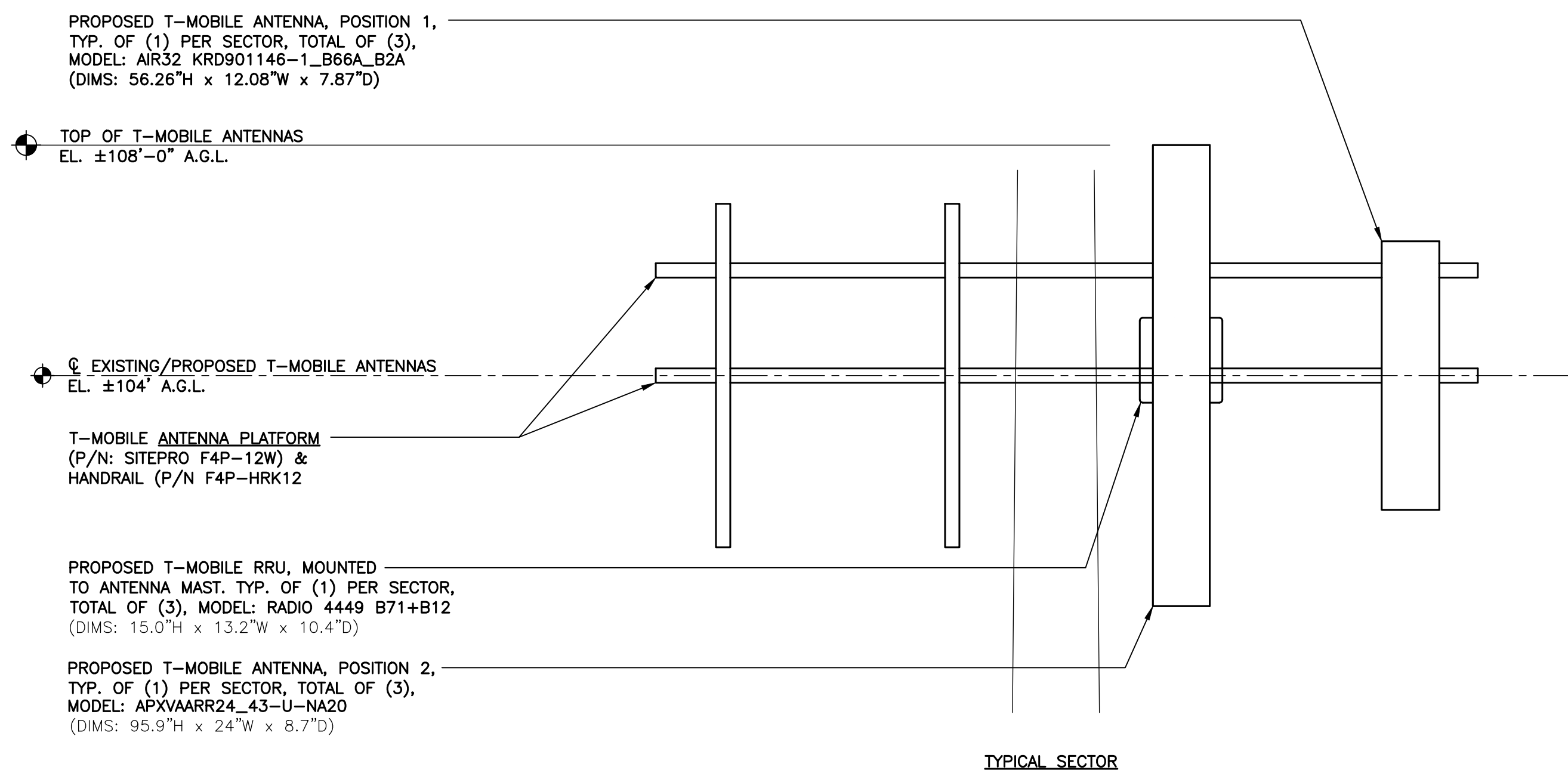


2 PROPOSED ANTENNA MOUNTING CONFIGURATION TYP.
 SCALE: 3/8" = 1'-0"
 104'-0" ELEVATION

ADDITIONAL ANTENNA MAST NOTE:
 REPLACE EXISTING PIPE MAST WITH 2" STD (O.D. = 2.5") x 8'-0" LONG PIPE
 © RFS APXVAARR24_43-U-NA20

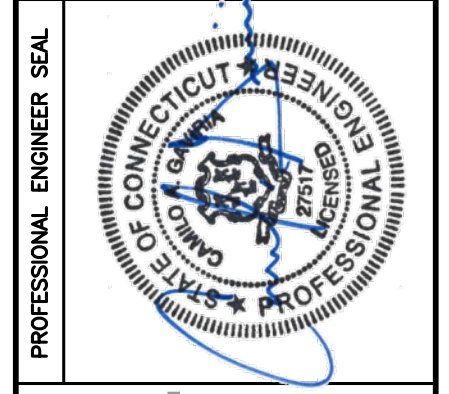


3 ANTENNA ELEVATION - EXISTING
 SCALE: 1" = 1'-0"



4 ANTENNA ELEVATION - PROPOSED
 SCALE: 3/8" = 1'-0"

REV.	DATE	BY	CHK'D BY	DESCRIPTION
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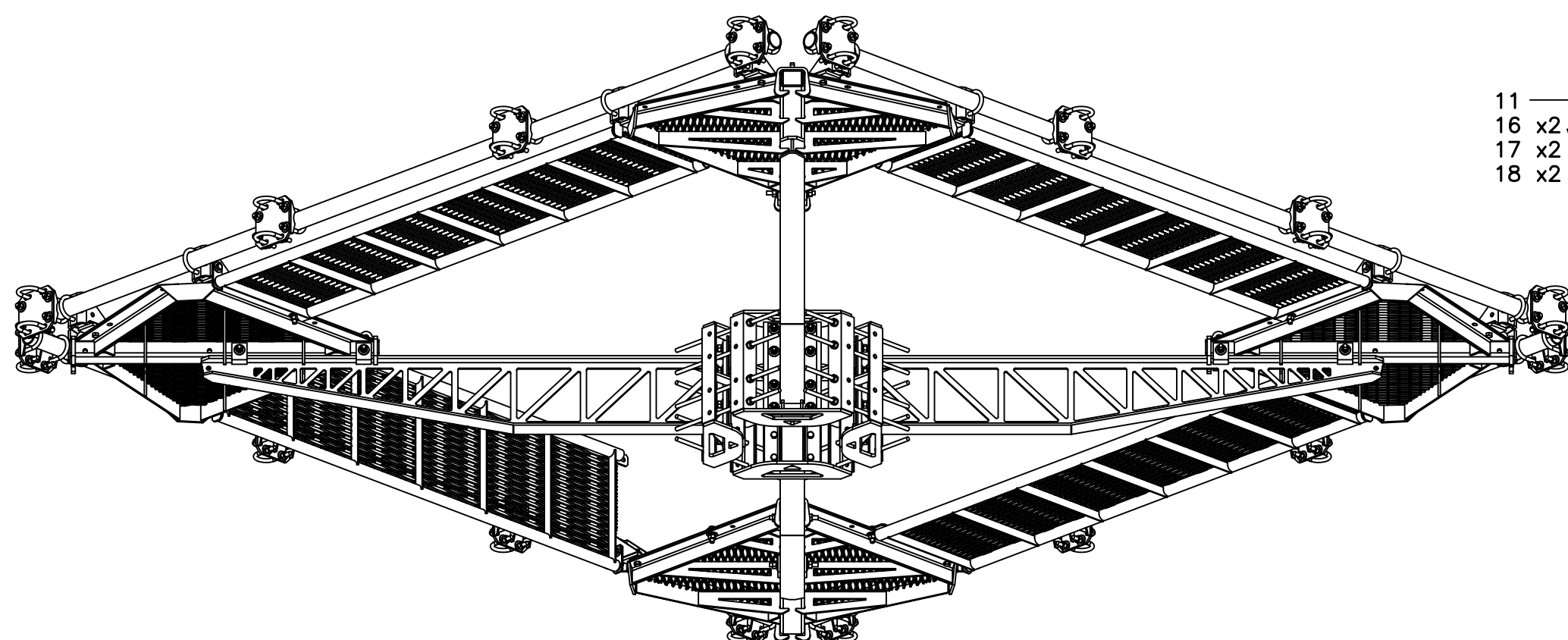
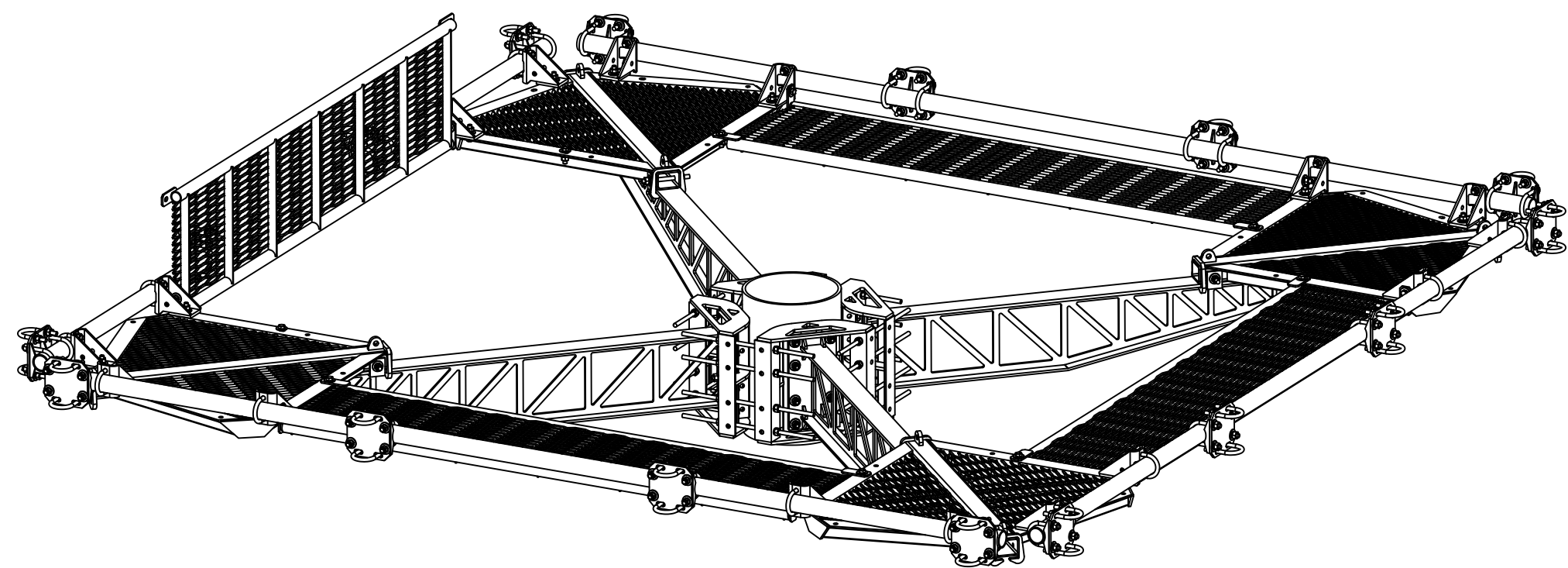
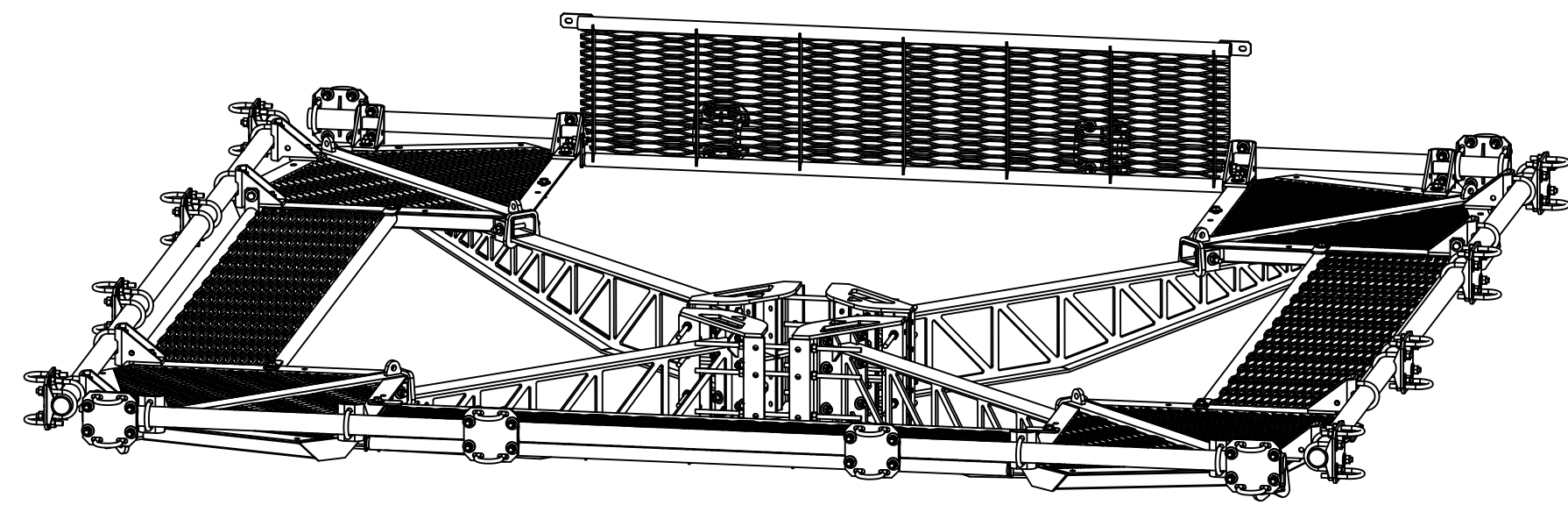
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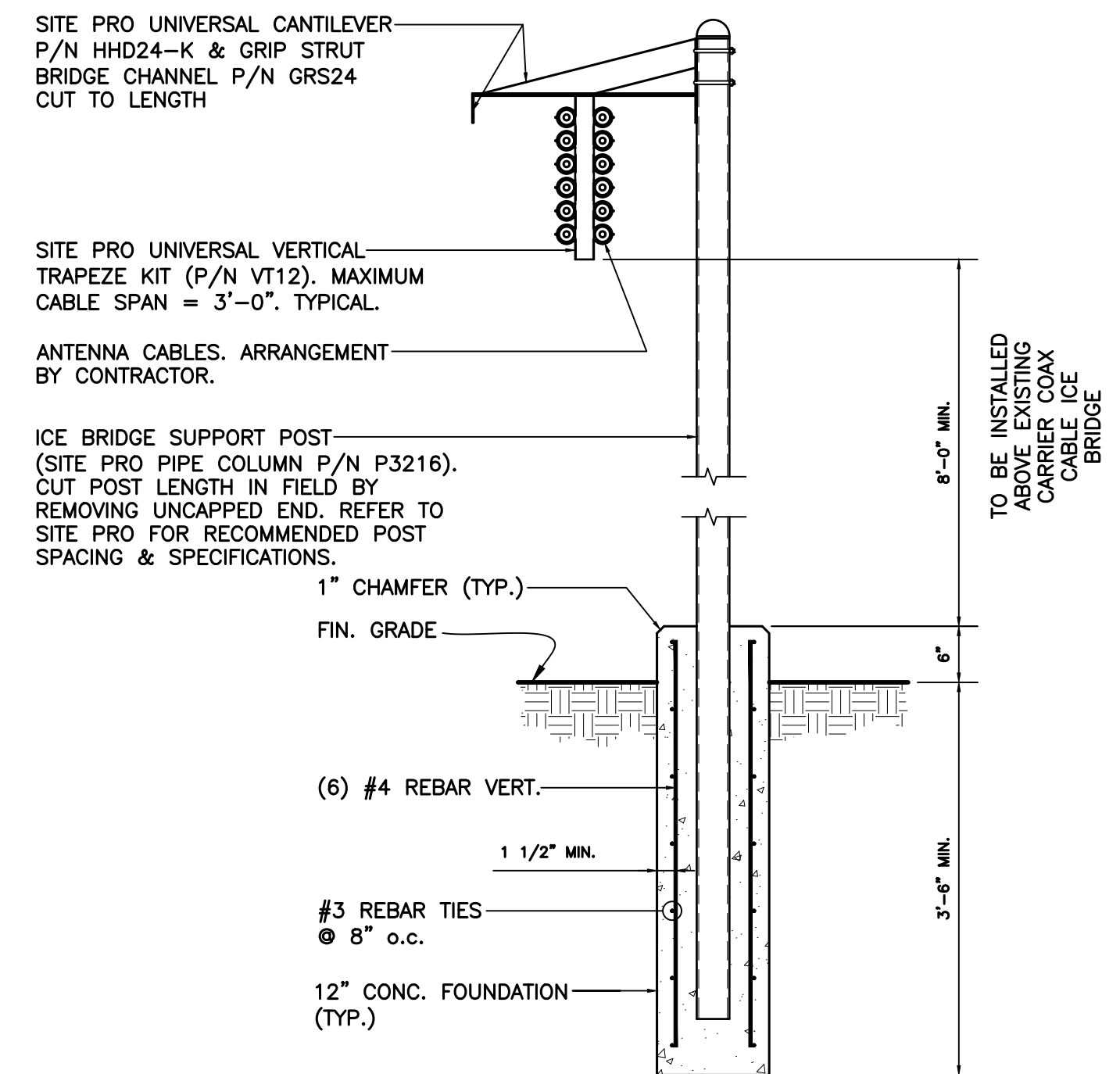
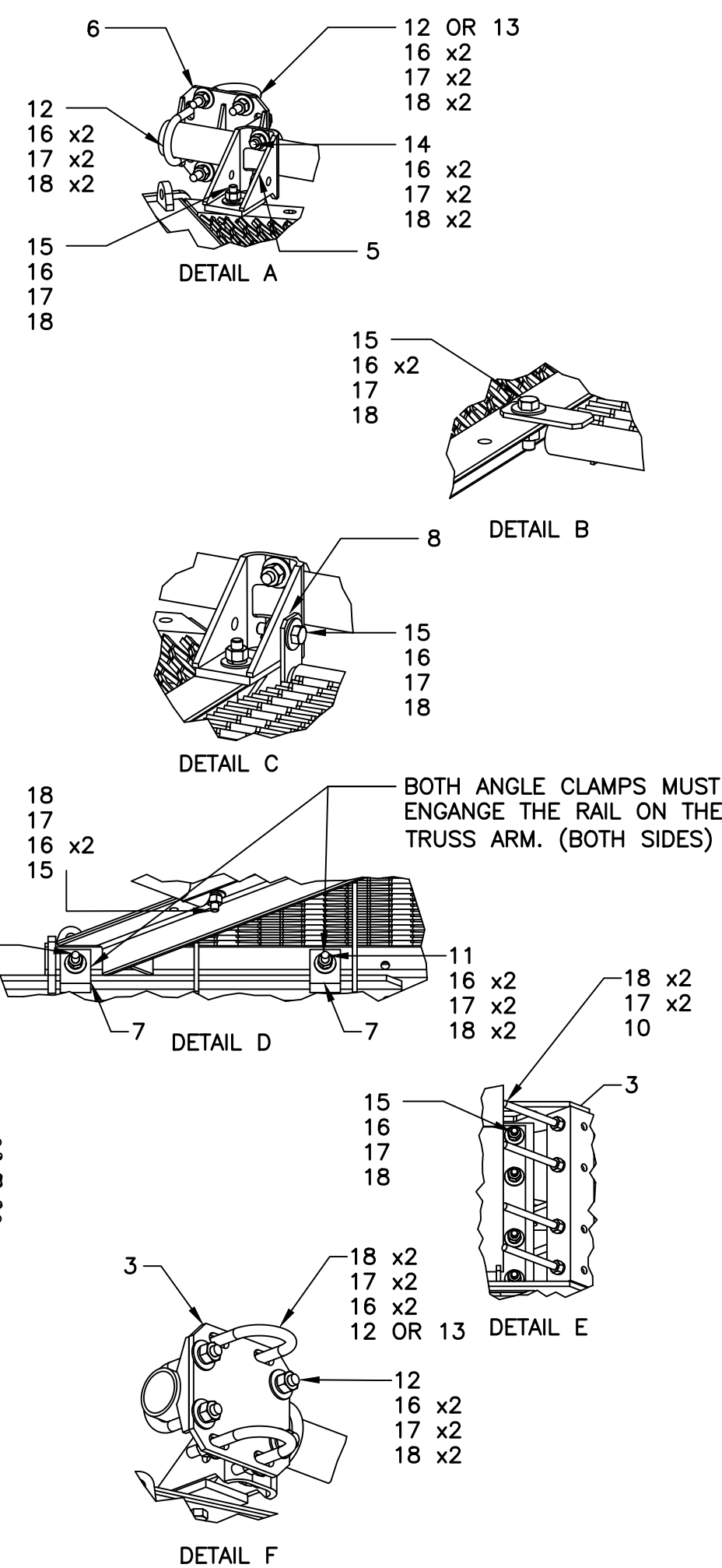
DATE: 04/01/20
 SCALE: AS NOTED
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ANTENNA PLANS

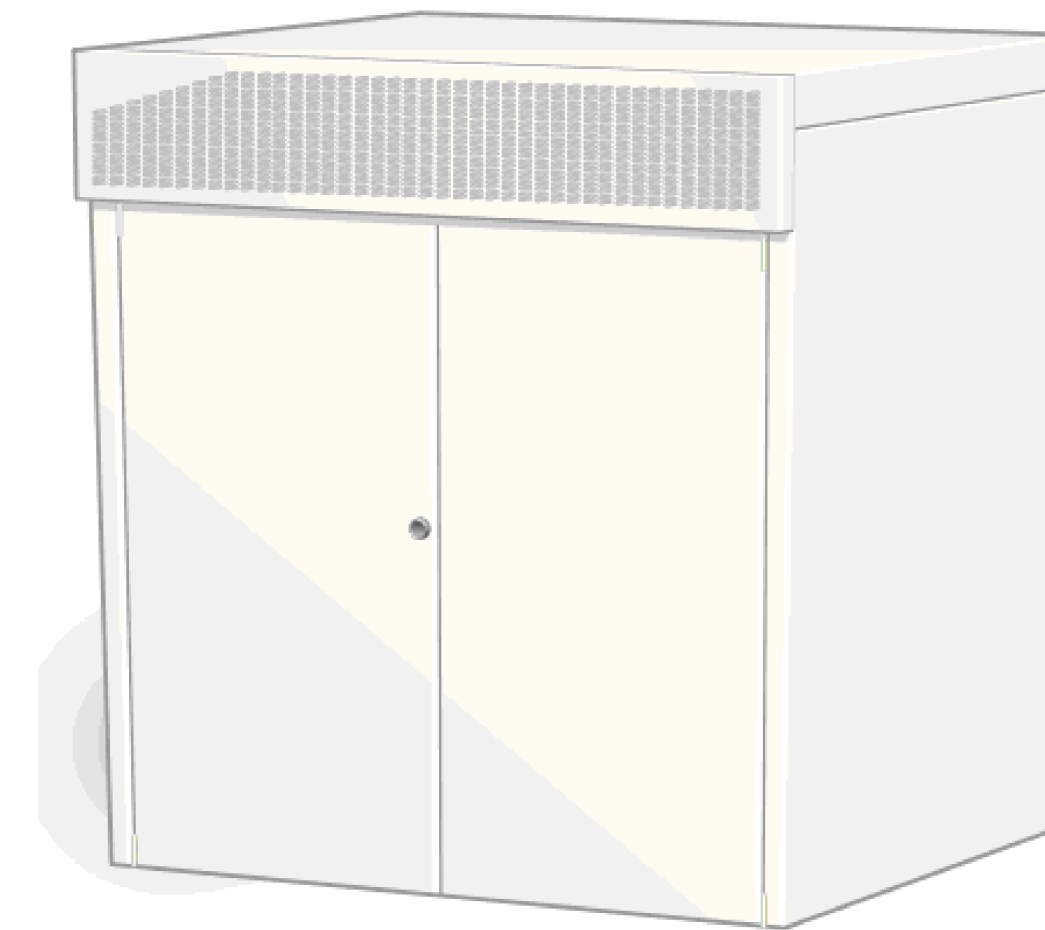
SITEPRO QUAD ANTENNA PLATFORM P/N: F4P-12W PARTS LIST						
ITEM	QTY	PART NO.	PART DESCRIPTION	LENGTH	UNIT WT.	NET WT.
1	4	X-LPP-CW	LOW PROFILE PLATFORM CORNER WELDMENT		198.75	795.01
2	4	X-LPP-SA14	SIDE ARM WELDMENT FOR 14' LOW PROFILE PLATFORM		151.68	606.71
3	4	X-RM4HD	WELDMENT FOR 4-SIDED HEAVY DUTY RING MOUNT		71.27	285.08
4	4	X-LPP-W14	WALKWAY FOR 14' LOW PROFILE PLATFORM		116.45	465.81
5	16	X-LPP-PC	FACE PIPE CONNECTION BRACKET FORTRESS PLATFORM		7.01	112.15
6	16	X-SCX3-FR	FORTRESS CROSSOVER PLATE		6.61	105.82
7	16	X-LPP-A7	CORNER WELDMENT ATTACHMENT ANGLE	2 1/2 IN	1.27	20.33
8	8	X-LPP-H	HINGE FOR LOW PROFILE PLATFORM WALKWAY		2.78	22.22
9	4	P30174	2-7/8" O.D. x 174" SCH.40 PIPE	174 IN	84.20	336.78
10	16	G58R-48	5/8" x 48" THREADED ROD (HDG.)	48 IN	0.40	6.38
10	16	G58R-24	5/8" x 24" THREADED ROD (HDG.)	24 IN	0.40	6.38
11	8	G58R-8	5/8" x 8" THREADED ROD (HDG.)		0.70	5.58
12	64	X-UB5300	5/8" x 3" x 5-1/4" x 2-1/2" U-BOLT (HDG.)		1.15	73.56
13	32	X-UB5258	5/8" x 2-5/8" x 4-1/2" x 2" U-BOLT (HDG.)		1.00	32.00
14	16	X-UB5304	5/8" x 3" x 4-1/4" x 2-1/2" U-BOLT (HDG.)		0.98	15.60
15	64	G58214	5/8" x 2-1/4" HDG HEX BOLT GR5		0.29	18.66
16	256	G58FW	5/8" HDG USS FLATWASHER	1/8 IN	0.07	18.04
17	272	G58LW	5/8" HDG LOCKWASHER		0.03	7.10
18	272	G58NUT	5/8" HDG HEAVY 2H HEX NUT		0.13	35.33
TOTAL WT.#					3056.14	



1 QUAD-PLATFORM MOUNT ASSEMBLY
C-5 NOT TO SCALE

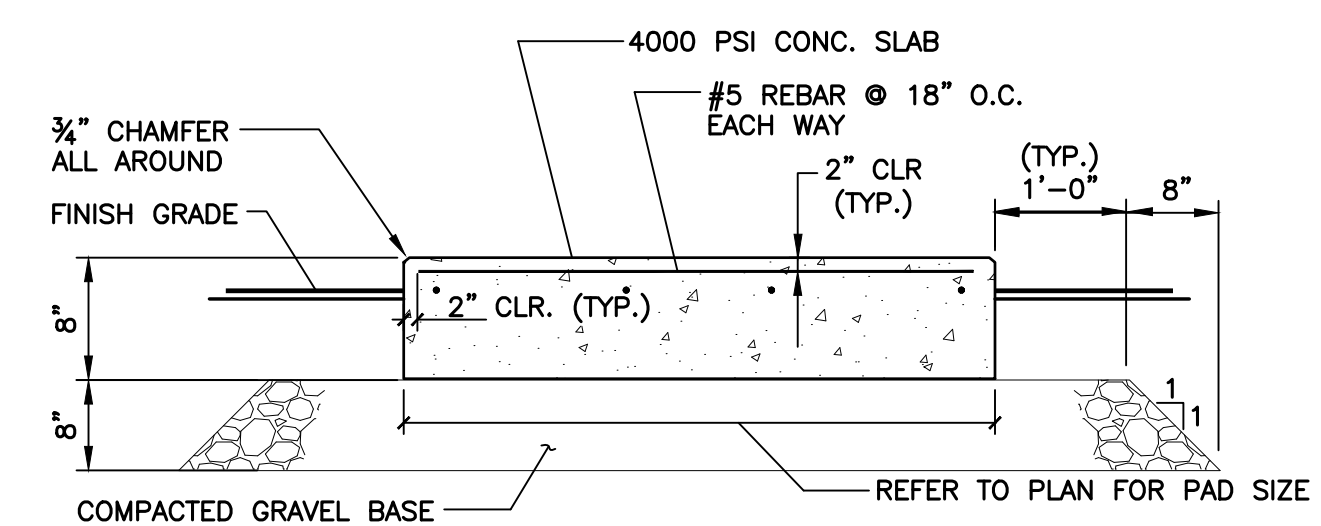


2 SINGLE-POST ICE BRIDGE DETAIL
C-5 NOT TO SCALE

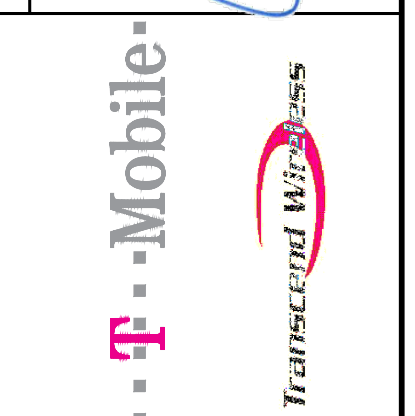
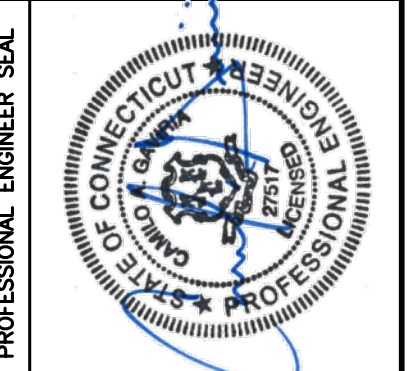


EQUIPMENT CABINET	DIMENSIONS	WEIGHT
EQUIPMENT: ERICSSON MODEL: 6102	57.09"H x 51.18"W x 27.56"D	727.53-LBS

3 ERICSSON RADIO CABINET DETAIL
C-5 SCALE: NTS



4 TYPICAL CONCRETE PAD DETAIL
C-5 NOT TO SCALE



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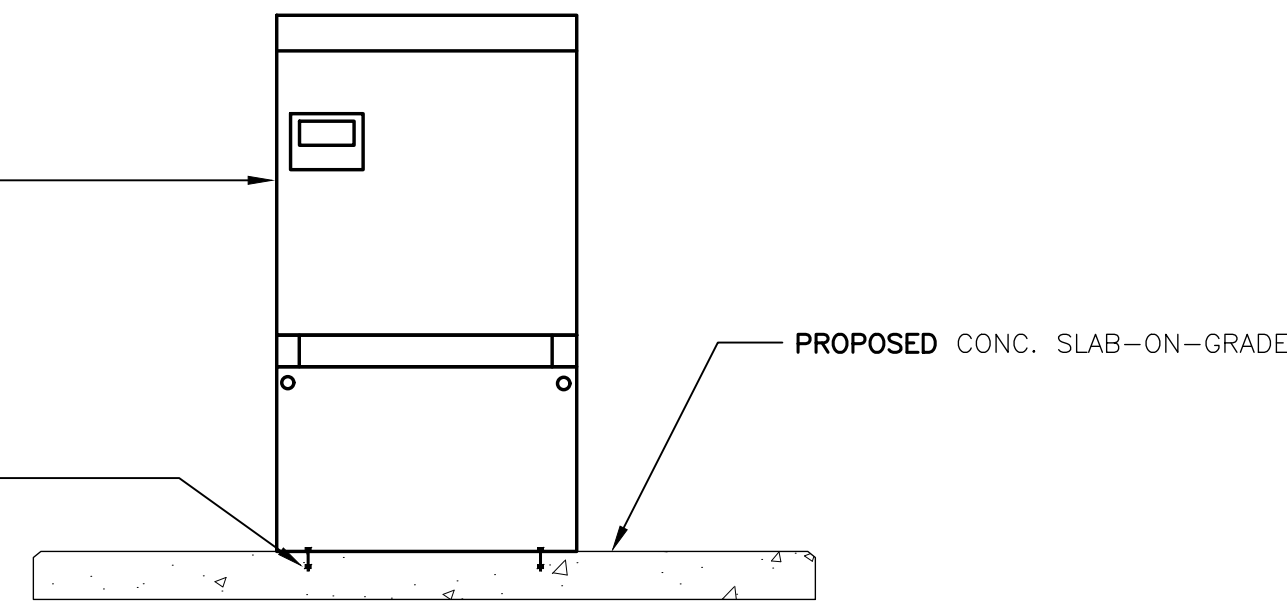
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DETAILS
C-5
Sheet No. Z of 10

PROPOSED T-MOBILE 25KW AC DIESEL FUELED BACK-UP GENERATOR

(4) - 5/8" HILTI KWIK BOLT TZ (MIN.) W/ 4" EMBED. COORDINATE QUANTITY WITH MANUFACTURER SPECIFICATIONS.

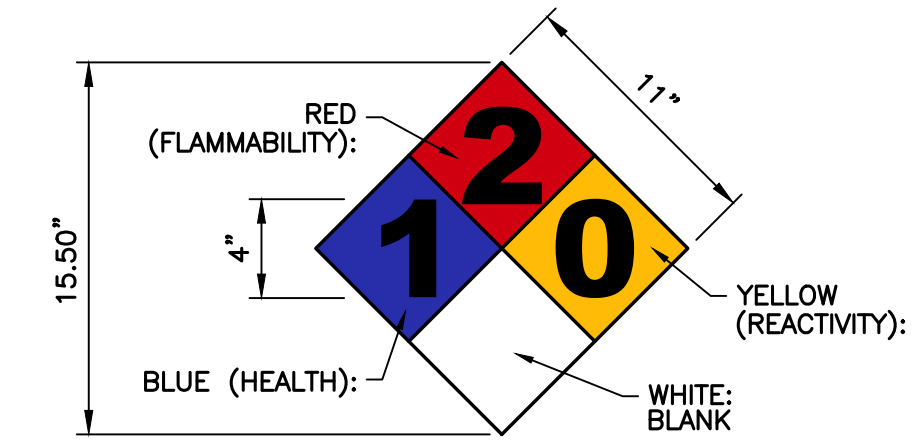


1 EQUIPMENT FASTENING DETAIL
C-6 SCALE: 1/2" = 1'-0"



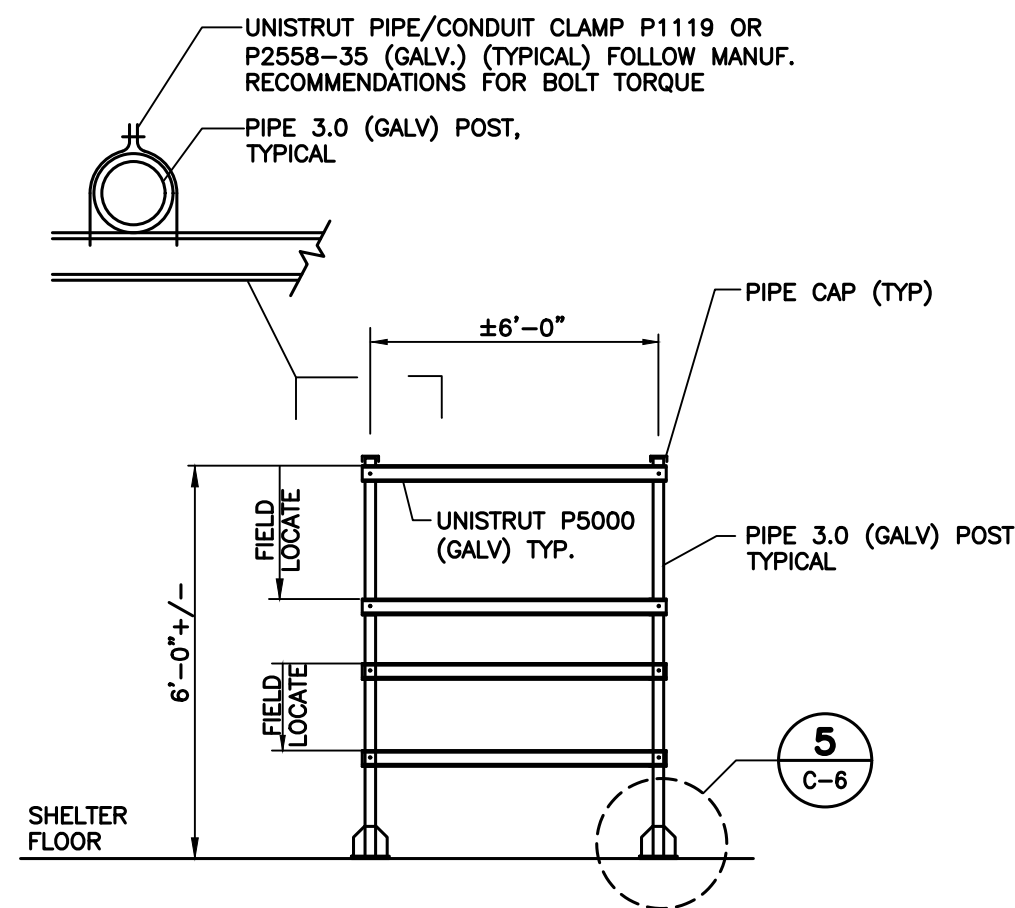
BACKUP POWER GENERATOR	
EQUIPMENT	DIMENSIONS
MAKE: GENERAC	
MODEL: 25 KW AC DIESEL (211 GALLON TANK)	95.0"L x 38.0"W x 99.0"H
NOTES: 1. CONTRACTOR TO COORDINATE FINAL EQUIPMENT MODEL SELECTION WITH T-MOBILE CONSTRUCTION MANAGER PRIOR TO ORDERING.	

2 BACK-UP GENERATOR DETAIL
C-6 NOT TO SCALE



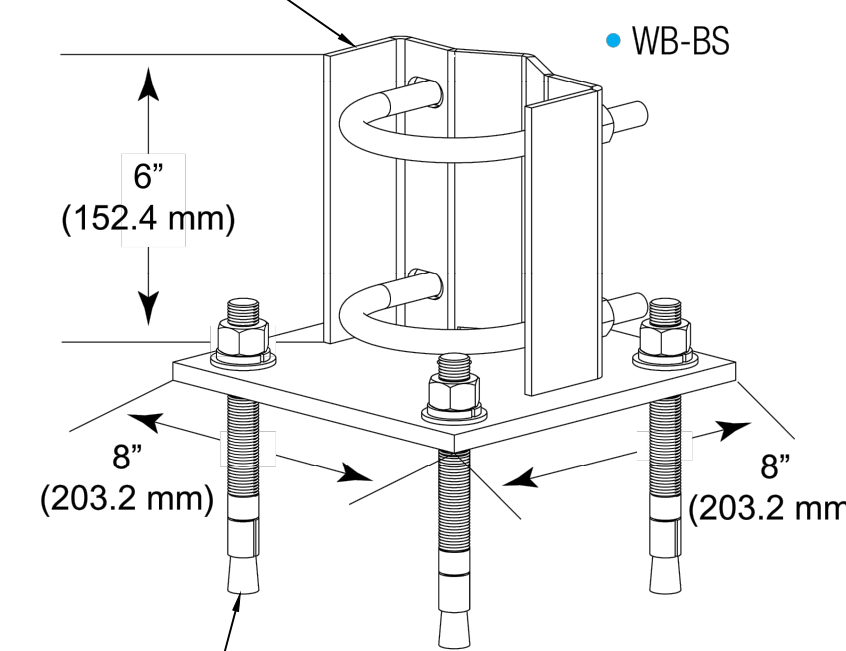
SIGN NAME: REGULATORY, NFPA 704 HAZARD ID
 DESCRIPTION: MOUNT ON GENERATOR ACCESS DOOR. CONSULT WITH GENERATOR MANUFACTURER MSDS SHEET FOR BLUE AND RES POSITIONS
 NOTES:
 1) SIGNS EXPOSED TO WEATHER SHOULD BE CHECKED ANNUALLY FOR READABILITY.
 2) SIGNS MUST BE UPDATED IF CHEMICAL STORAGE OR HAZARD INFORMATION FOR THE LOCATION CHANGES.
 3) THE GC MUST REVIEW WITH LOCAL JURISDICTION WHEN FILLING FOR PERMITS, AS EACH JURISDICTION MAY HAVE DIFFERENT REQUIREMENTS AND COMPLY WITH POSTING REQUIREMENTS OR DIRECTIVES FROM THE LOCAL JURISDICTION.

3 NFPA 704 DIAMOND SIGNAGE DETAIL
C-6 1 1/2" = 1'



4 PROPOSED EQUIPMENT MOUNTING FRAME DETAIL
C-6 SCALE: NOT TO SCALE

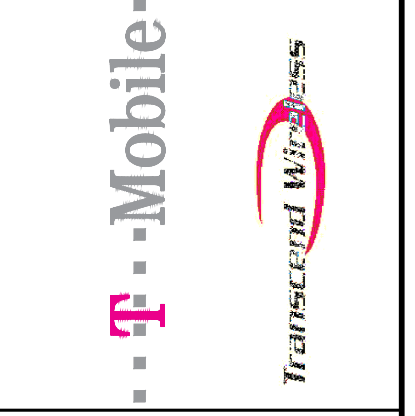
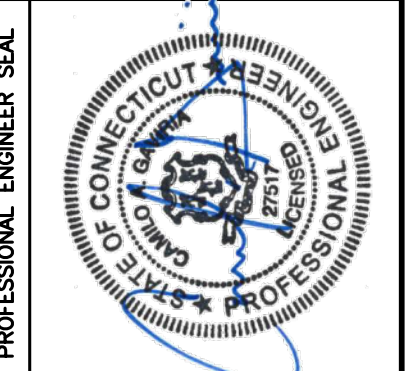
PROPOSED COMMSCOPE WAVEGUIDE BRIDGE BASE SHOE (P/N: WB-BS), TYPICAL OF (2)



(4) PROPOSED HILTI M10 HDA-P UNDERCUT ANCHOR (3.9" MIN. EMBED)

5 EQUIPMENT FRAME POST ATTACHMENT DETAIL
C-6 SCALE: NOT TO SCALE

REV.	DATE	BY	DESCRIPTION
C	05/05/20	KANUR	PRELIMINARY CD - REVISED PER CLIENT COMMENTS
B	04/28/20	KANUR	PRELIMINARY CD - REVISED PER CLIENT COMMENTS
A	04/15/20	KANUR	PRELIMINARY CD - ISSUED FOR CLIENT REVIEW

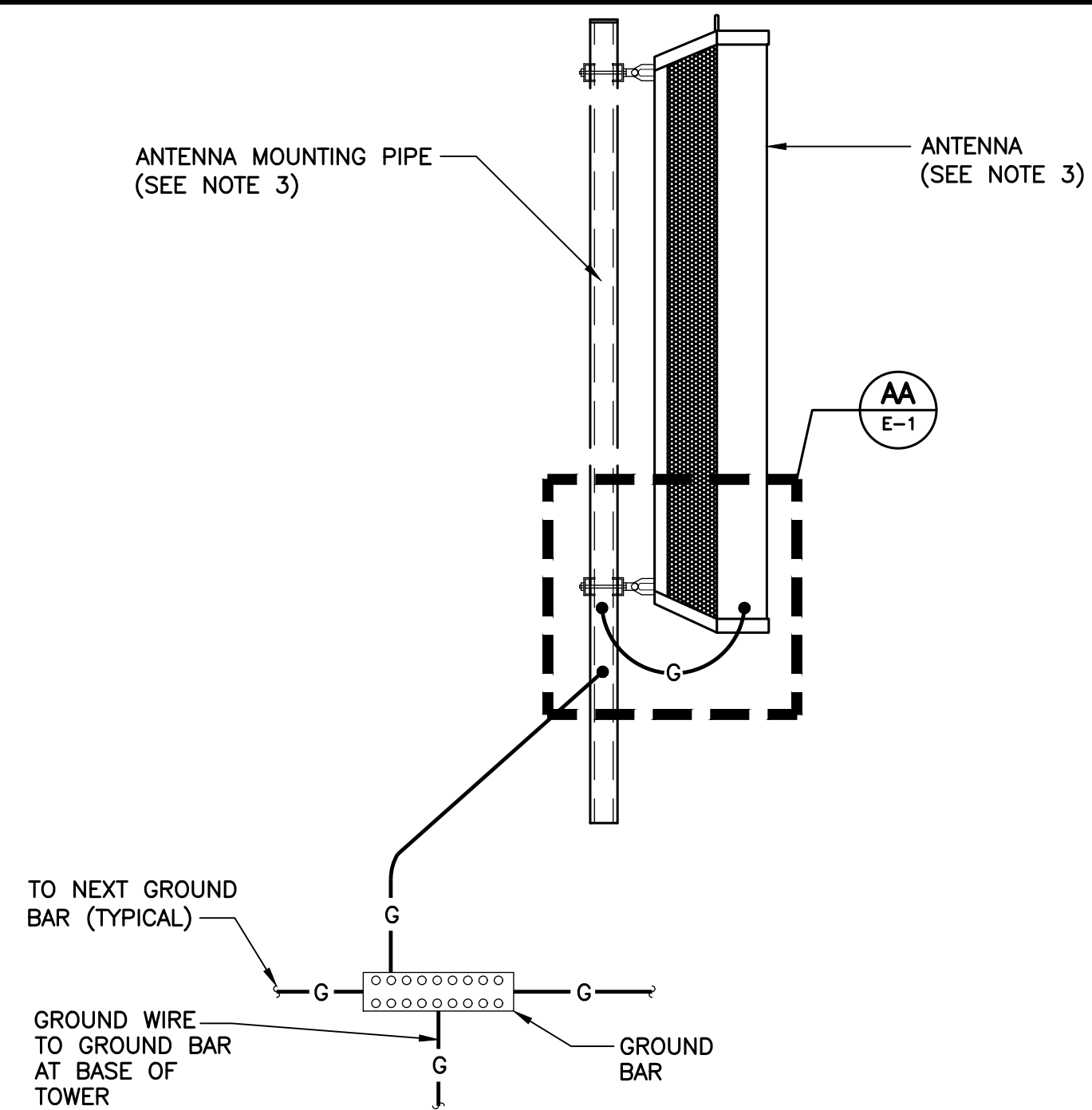


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T-MOBILE NORTHEAST LLC
 WIRELESS COMMUNICATIONS FACILITY
WESTBROOK SOUTH
 SITE ID: CTHA524A
 1542 BOSTON POST ROAD
 WESTBROOK, CT 06498

DATE: 04/01/20
 SCALE: AS NOTED
 JOB NO. 19027.46

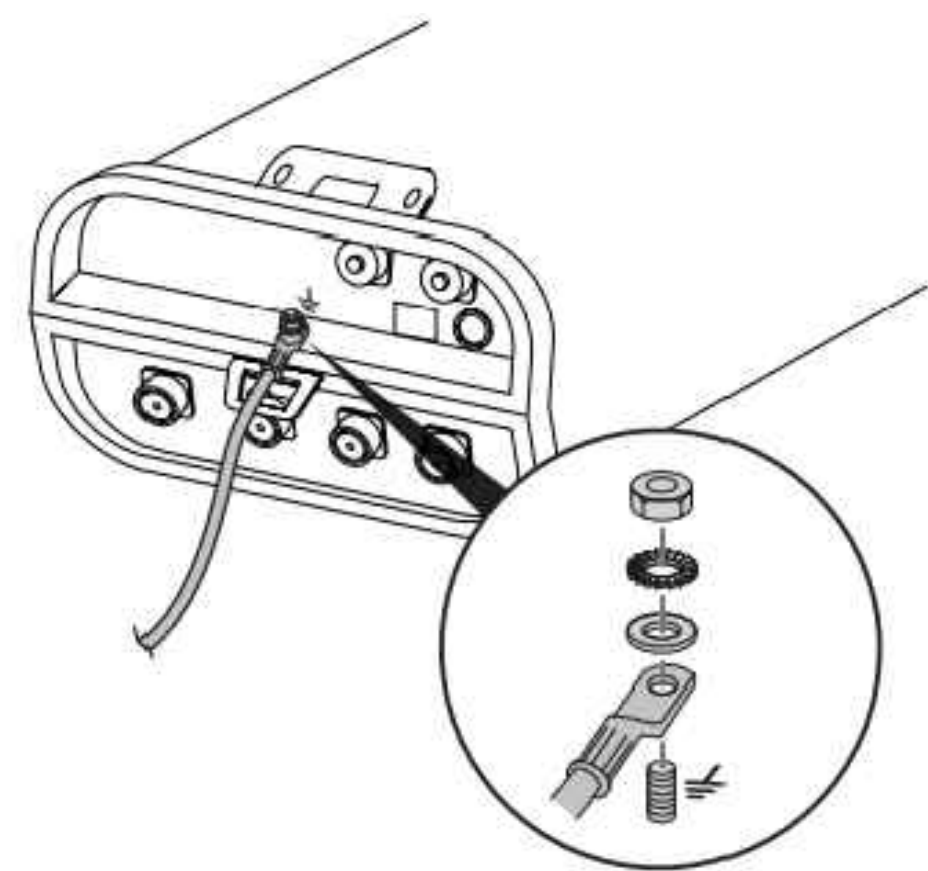
DETAILS



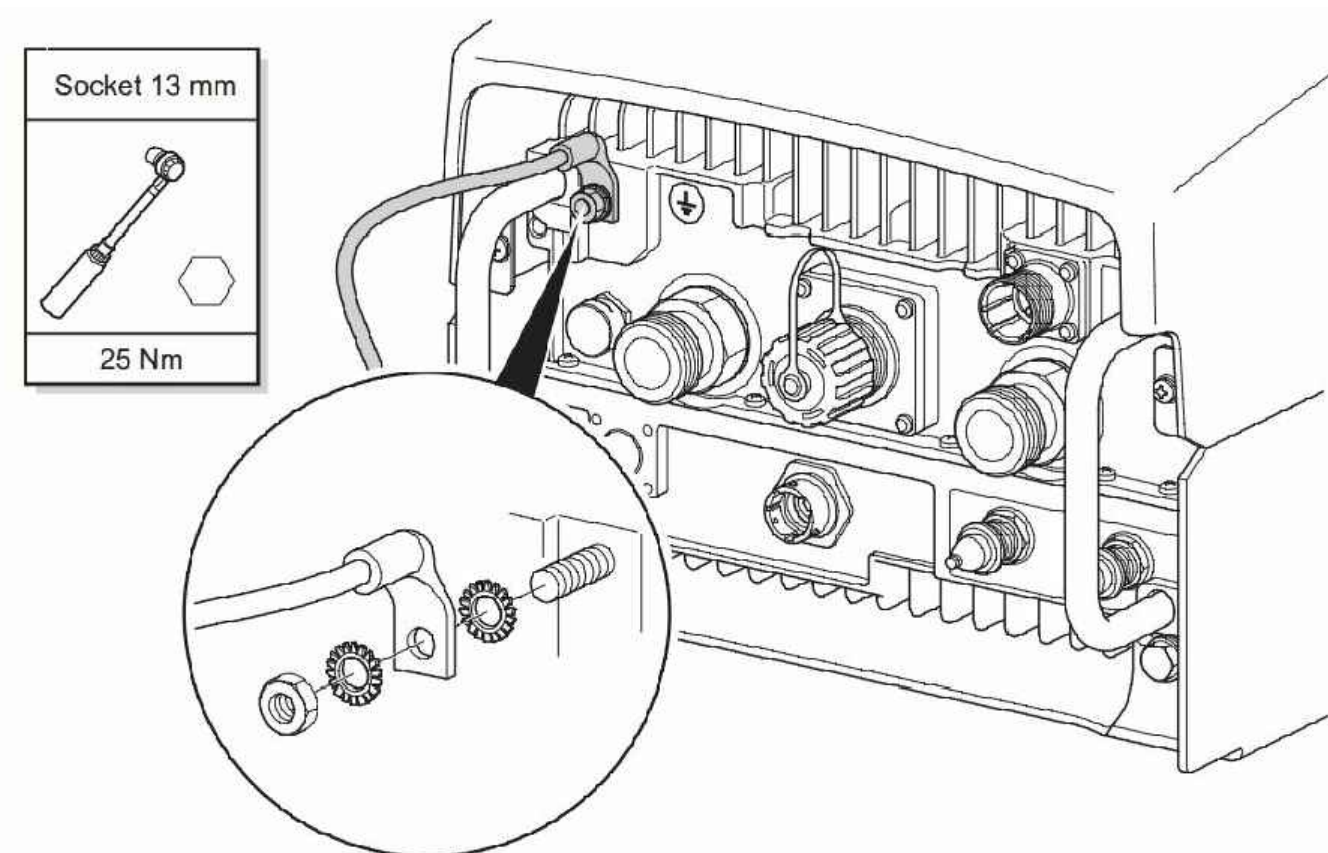
NOTES:

- BOND COAXIAL CABLE GROUND KITS TO EACH OWNER'S GROUND BAR ALONG ENTIRE COAX RUN FROM ANTENNA TO SHELTER.
- BOND ALL EQUIPMENT TO GROUND PER NEC AND MANUFACTURERS SPECIFICATIONS.
- DETAIL IS TYPICAL FOR ALL ANTENNA SECTORS, INCLUDING GPS ANTENNA.

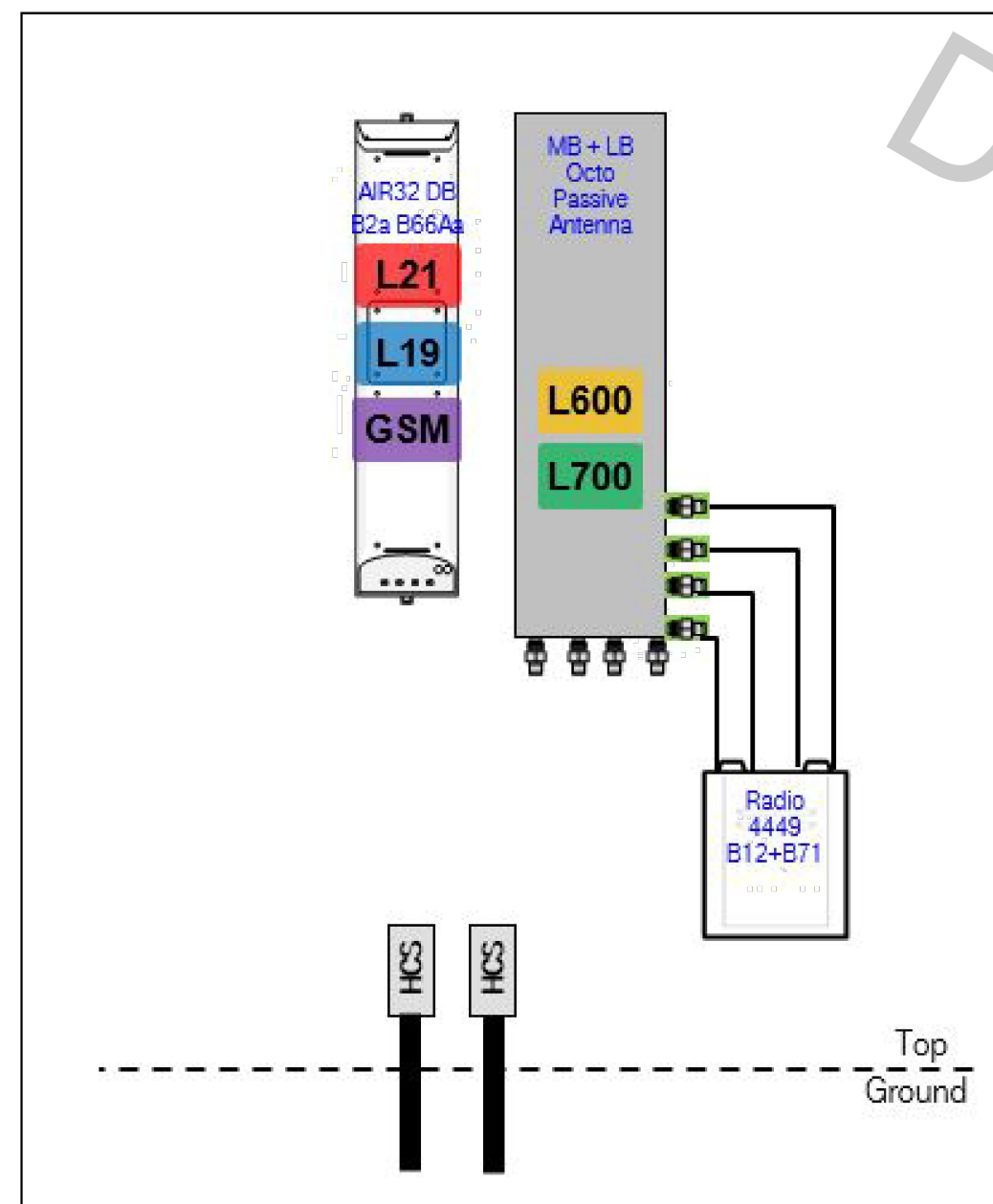
1 TYPICAL ANTENNA GROUNDING DETAIL
E-1 SCALE: NONE



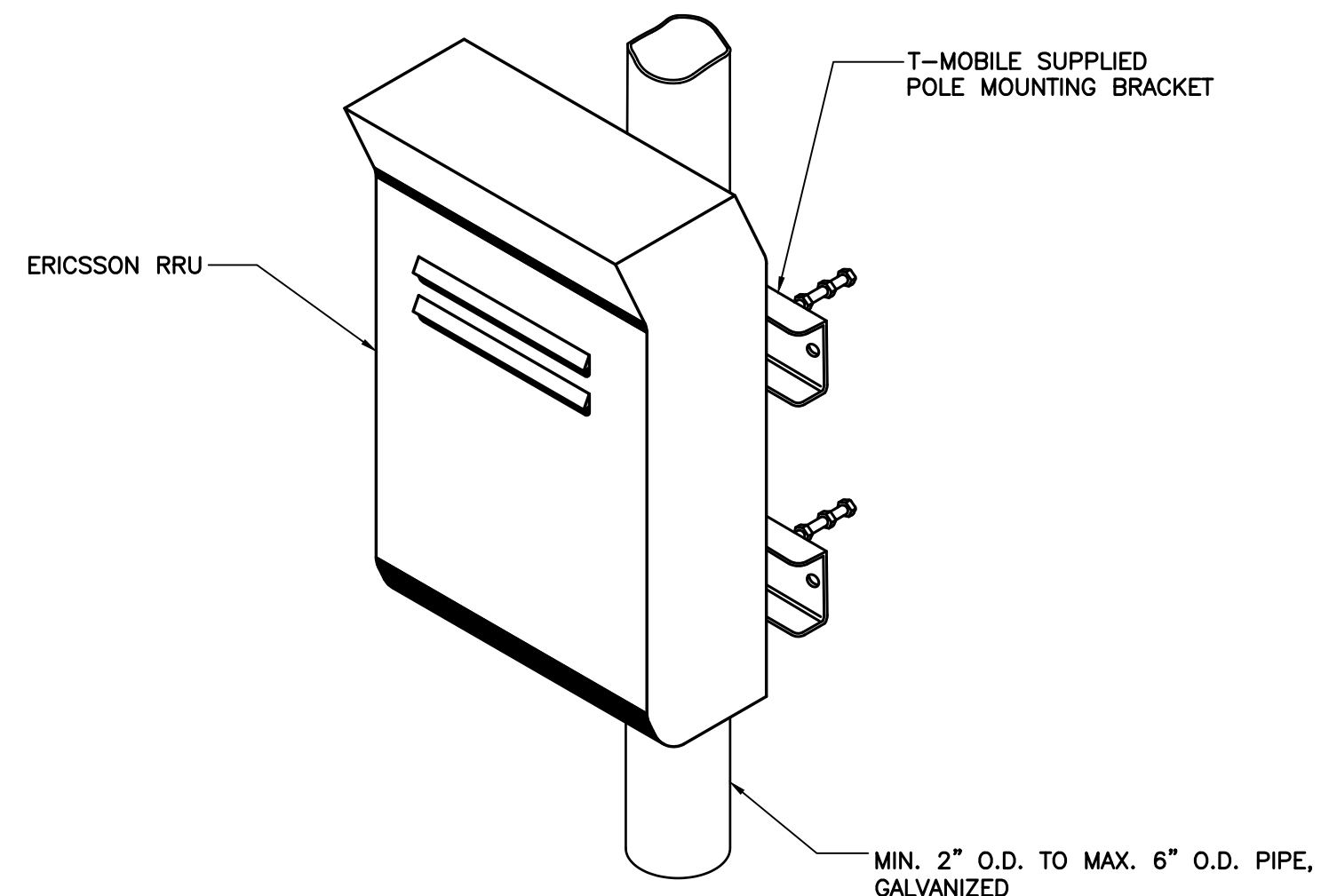
2 TYPICAL ANTENNA GROUNDING DETAIL
E-1 SCALE: NOT TO SCALE



3 TYPICAL RRU GROUNDING DETAIL
E-1 NOT TO SCALE



4 PROPOSED PLUMBING DIAGRAM
E-1 SCALE: NONE



NOTES:

- T-MOBILE SHALL SUPPLY RRU, AND RRU POLE-MOUNTING BRACKET. CONTRACTOR SHALL SUPPLY POLE/PIPE AND INSTALL ALL MOUNTING HARDWARE INCLUDING ERICSSON RRU POLE-MOUNTING BRACKET. CONTRACTOR SHALL INSTALLS RRU AND MAKES CABLE TERMINATIONS.
- NO PAINTING OF THE RRU OR SOLAR SHIELD IS ALLOWED.

5 TYPICAL RRUS MOUNTING DETAILS
E-1 SCALE: NOT TO SCALE

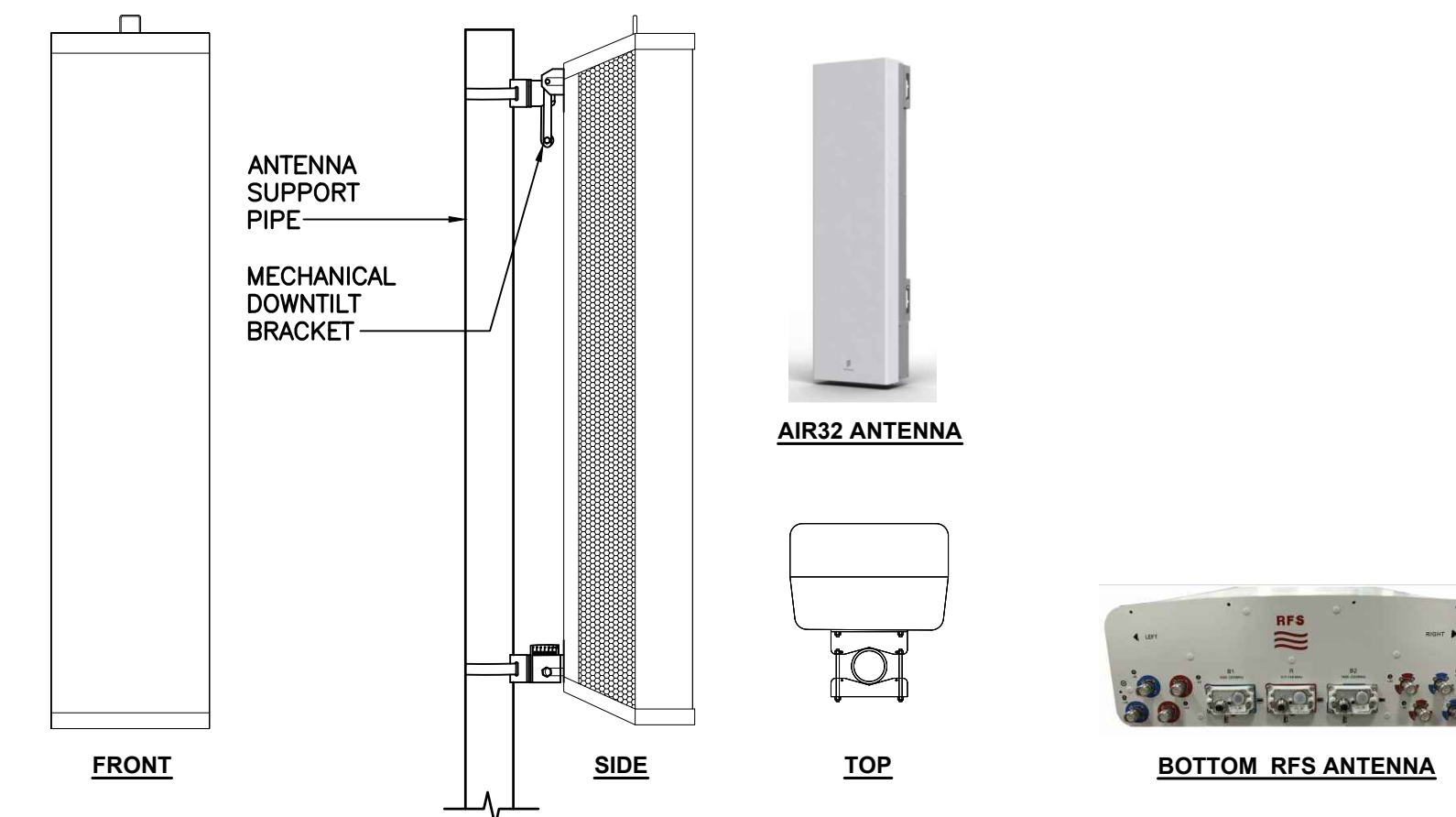


RRU (REMOTE RADIO UNIT)			
EQUIPMENT	DIMENSIONS	WEIGHT	CLEARANCES
MAKE: ERICSSON MODEL: RADIO 4449 B71B12	14.9"L x 13.2"W x 10.4"D	74 LBS.	ABOVE: 16" MIN. BELOW: 12" MIN. FRONT: 36" MIN.

NOTES:

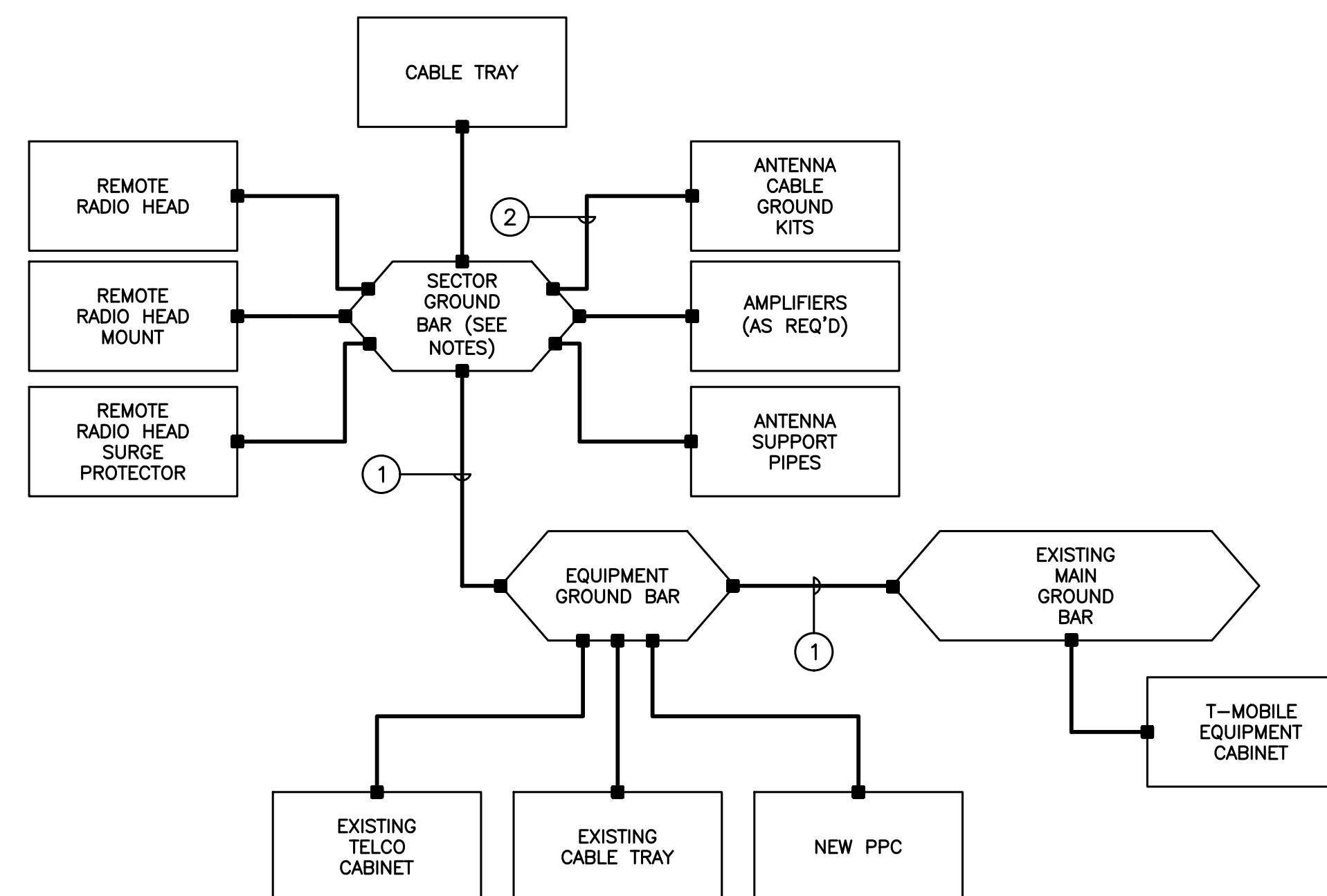
- CONTRACTOR TO COORDINATE FINAL EQUIPMENT MODEL SELECTION WITH T-MOBILE CONSTRUCTION MANAGER PRIOR TO ORDERING.

6 PROPOSED RRU DETAIL
E-1 SCALE: NOT TO SCALE



ALPHA/BETA/GAMMA ANTENNA		
EQUIPMENT	DIMENSIONS	WEIGHT
MAKE: ERICSSON MODEL: AIR32 KRD901146-1	56.6"L x 12.9"W x 8.7"D	132.2 LBS.
MAKE: RFS MODEL: APXVAARR24_43-U-NA20	95.9"L x 24"W x 8.7"D	153 LBS.

7 PROPOSED ANTENNA DETAIL
E-1 SCALE: NONE

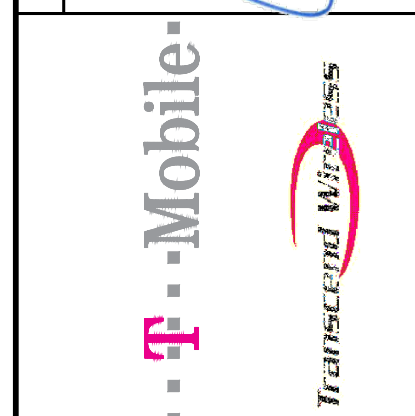
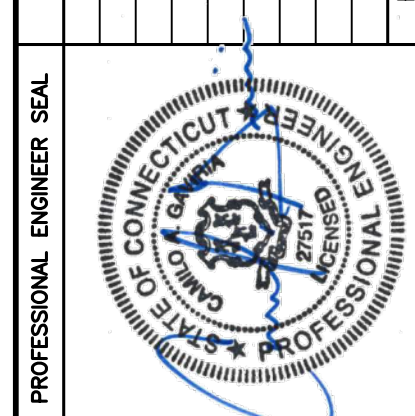


GROUNDING SCHEMATIC NOTES

- #2 AWG
 - #6 AWG
- GENERAL NOTES:**
- ALL SURGE SUPPRESSION EQUIPMENT SHALL BE BONDED TO GROUND PER MANUFACTURER'S SPECIFICATIONS
 - UNLESS OTHERWISE NOTED OR REQUIRED BY CODE, GROUND CONDUCTORS SHOWN SHALL BE #2 AWG (SOLID TINNED BCW - EXTERIOR; STRANDED GREEN INSULATED - INTERIOR).
 - ALL SECTOR GROUND BARS SHALL BE BONDED TOGETHER WITH #2 AWG SOLID TINNED BCW.
 - BOND ALL EQUIPMENT CABINETS AND BATTERY CABINETS TO GROUND PER MANUFACTURER'S SPECIFICATIONS.
 - COORDINATE ALL ROOF MOUNTED EQUIPMENT WITH OWNER.
 - ALL ROOF MOUNTED AMPLIFIERS AND ASSOCIATED EQUIPMENT SHALL BE BONDED TO THE SECTOR GROUND BAR PER MANUFACTURER'S SPECIFICATIONS.
 - ALL GROUNDING SHALL BE IN ACCORDANCE WITH NEC AND OWNER'S REQUIREMENTS.

8 TYPICAL GROUNDING SCHEMATIC DETAIL
E-1 SCALE: NOT TO SCALE

REV.	DATE	BY	CHK'D BY	DESCRIPTION
A	04/13/20	KANUR	CAG	PRELIMINARY CD - ISSUED FOR CLIENT REVIEW
B	04/28/20	KANUR	CAG	PRELIMINARY CD - REVISED PER CLIENT COMMENTS
C	05/05/20	KANUR	CAG	PRELIMINARY CD - REVISED PER CLIENT COMMENTS



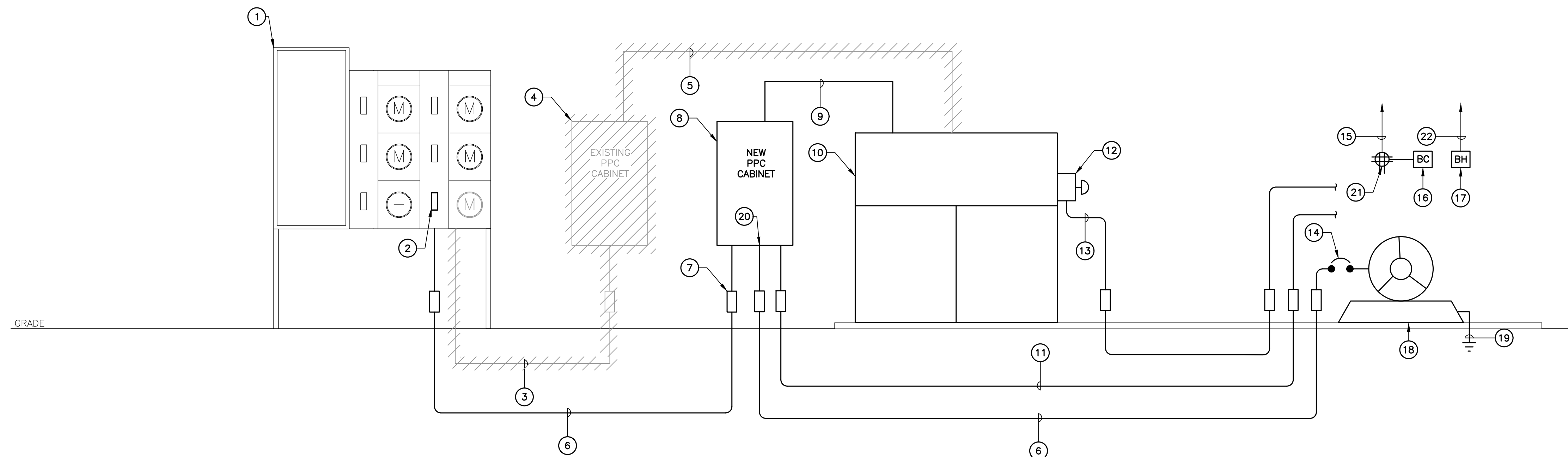
CENTEK engineering
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(203) 488-0380
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652 North Branford Road
Branford, CT 06405
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T-MOBILE NORTHEAST LLC
WIRELESS COMMUNICATIONS FACILITY
WESTBROOK SOUTH
SITE ID: CTHA524A
1542 BOSTON POST ROAD
WESTBROOK, CT 06498

DATE: 04/01/20
SCALE: AS NOTED
JOB NO. 19027.46

TYPICAL ELECTRICAL DETAILS

E-1

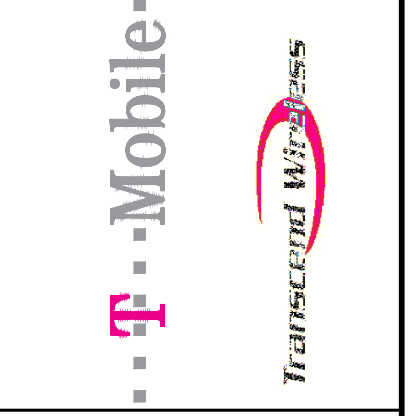
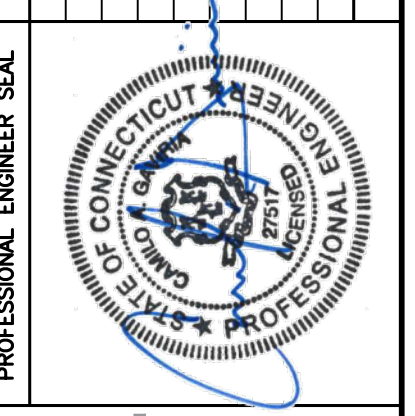


1 ELECTRICAL RISER DIAGRAM
E-2 NOT TO SCALE

RISER DIAGRAM NOTES

- ① EXISTING METER CENTER
- ② REPLACE EXISTING T-MOBILE 100A/2P CIRCUIT BREAKER WITH NEW 200A/2P CIRCUIT BREAKER.
- ③ CONDUITS AND CONDUCTORS FEEDING EXISTING 100A PPC TO BE REMOVED.
- ④ EXISTING 100A PPC CABINET TO BE REMOVED AND REPLACED.
- ⑤ CONDUITS AND CONDUCTORS SERVING EXISTING RADIO EQUIPMENT CABINETS TO BE REMOVED
- ⑥ (3) #3/0 AWG, (1) #6 AWG GROUND, 2" CONDUIT.
- ⑦ EXPANSION COUPLING (TYPICAL).
- ⑧ NEW 200A PPC CABINET WITH INTEGRAL AUTOMATIC TRANSFER SWITCH.
- ⑨ (3) #1 AWG, (1) #6 AWG GROUND, 1-1/4" CONDUIT CONNECTED TO NEW 125A/2P CIRCUIT BREAKER IN PPC.
- ⑩ NEW T-MOBILE EQUIPMENT CABINET.
- ⑪ 1" CONDUIT WITH CONTROL AND ALARM CONDUCTORS FROM GENERATOR TO TRANSFER SWITCH IN PPC CABINET. INSTALL CONDUCTORS AS REQUIRED BY MANUFACTURER.
- ⑫ REMOTE GENERATOR SHUT OFF SWITCH IN BREAK GLASS ENCLOSURE MOUNTED IN LOCATION APPROVED BY LOCAL FIRE MARSHAL. INSTALL ALL REQUIRED SIGNAGE.
- ⑬ 3/4" CONDUIT AND CONDUCTORS REQUIRED FOR PROPER OPERATION OF EMERGENCY GENERATOR SHUT OFF SWITCH.
- ⑭ 200A, 240V, MAIN CIRCUIT BREAKER AT GENERATOR OUTPUT.
- ⑮ DEDICATED 20A, 120V, CIRCUIT IN 3/4" CONDUIT FROM PPC TO GENERATOR BATTERY CHARGER AND DUPLEX RECEPTACLE.
- ⑯ GENERATOR BATTERY CHARGER.
- ⑰ GENERATOR BLOCK HEATER.
- ⑱ 25 KW AC DIESEL FUELED BACK UP GENERATOR
- ⑲ GROUND GENERATOR PER NEC AND MANUFACTURER'S SPECIFICATIONS.
- ⑳ EXTEND GENERATOR POWER OUTPUT CONDUCTORS TO EMERGENCY LUGS IN TRANSFER SWITCH IN PPC CABINET.
- ㉑ DUPLEX GFCI RECEPTACLE IN WEATHERPROOF ENCLOSURE MOUNT IN CONVENIENT LOCATION AT GENERATOR.
- ㉒ DEDICATED 20A, 120V, CIRCUIT IN 3/4" CONDUIT FROM PPC TO GENERATOR BLOCK HEATER.

REV.	DATE	BY	CHK'D BY	DESCRIPTION
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DATE: 04/01/20
 SCALE: AS NOTED
 JOB NO. 19027.46

ELECTRICAL
 RISER
 DETAILS

E-2
 Sheet No. 10 of 10

Exhibit B
Structural Analysis

Structural Analysis Report

130-ft Rohn Monopole

*Proposed T-Mobile
Antenna Upgrade (L600)*

T-Mobile Site Ref: CTHA524A

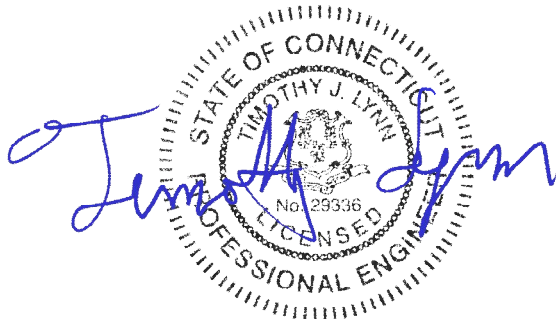
*1546 Boston Post Road
Westbrook, CT*

Centek Project No. 19027.46

~~*Date: April 14, 2020*~~

Rev 2: May 5, 2020

Max Stress Ratio = 60.6%



Prepared for:
T-Mobile USA
35 Griffin Road
Bloomfield, CT 06002

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- INTRODUCTION
- ANTENNA AND APPURTENANCE SUMMARY
- PRIMARY ASSUMPTIONS USED IN THE ANALYSIS
- ANALYSIS
- TOWER LOADING
- TOWER CAPACITY
- FOUNDATION AND ANCHORS
- CONCLUSION

SECTION 2 – CONDITIONS & SOFTWARE

- STANDARD ENGINEERING CONDITIONS
- GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM

SECTION 3 – CALCULATIONS

- tnxTower INPUT/OUTPUT SUMMARY
- tnxTower DETAILED OUTPUT
- ANCHOR BOLT AND BASE PLATE ANALYSIS
- FOUNDATION ANALYSIS

SECTION 4 – REFERENCE MATERIAL

- RF DATA SHEET
- ANTENNA CUT SHEETS

Introduction

The purpose of this report is to summarize the results of the non-linear, P- Δ structural analysis of the antenna installation proposed by T-Mobile on the monopole (tower) located in Westbrook, CT.

The host tower is a 130-ft tall, three-section, eighteen sided, tapered monopole, originally designed and manufactured by Rohn job no; 231452, dated November 18, 2019. The tower geometry, structure member sizes and foundation system information were obtained from the original design documents.

Antenna and appurtenance information were obtained from a previous structural analysis report prepared by All-Points Technology Corp.; job no; CT1931591 dated January 17, 2020.

The tower is made up of three (3) tapered vertical sections consisting of A572-65 pole sections. The vertical tower sections are slip joint connected. The diameter of the pole (flat-flat) is 28.0-in at the top and 65.0-in at the base.

Antenna and Appurtenance Summary

- **VERIZON (Reserved):**
Antennas: Nine (9) NHH-65B-R2B panel antennas, nine (9) Remote Radio Heads and two (2) raycap main distribution boxes mounted on a 13-ft platform with rails with a RAD center elevation of 126-ft above the existing tower base plate.
Coax Cables: Nine (9) 1-5/8" \varnothing coax cables running on the inside of the existing tower.
- **AT&T (Existing to Remain):**
Antennas: Three (3) KMW EPBQ-654L8H8-L2 panel antennas, three (3) Ericsson 4415 B25 remote radio heads, three (3) Ericsson 4449 B5 remote radio heads and two (2) surge arrestors mounted on (3) 12-ft V-frames with a RAD center elevation of 115-ft above grade.
Coax Cables: One (1) fiber trunk and four (4) DC trunks running on the inside of the existing tower.
- **T-MOBILE (Proposed):**
Antennas: Three (3) RFS APXVAARR24_43 panel antennas, three (3) Ericsson AIR32 panel antennas and three (3) Ericsson 4449 B71 B12 remote radio heads mounted on a 14-ft platform with rails (SitePro F4P-12W / HRK12) with a RAD center elevation of 104-ft above the existing tower base plate.
Coax Cables: Three (3) 6x12 fiber cables running on the inside of the existing tower.

Primary Assumptions Used in the Analysis

- The tower structure's theoretical capacity not including any assessment of the condition of the tower.
- The tower carries the horizontal and vertical loads due to the weight of antennas, ice load and wind.
- Tower is properly installed and maintained.
- Tower is in plumb condition.
- Tower loading for antennas and mounts as listed in this report.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds are fabricated with ER-70S-6 electrodes.
- All members are assumed to be as specified in the original tower design documents or reinforcement drawings.
- All members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
- All member protective coatings are in good condition.
- All tower members were properly designed, detailed, fabricated, installed and have been properly maintained since erection.
- Any deviation from the analyzed antenna loading will require a new analysis for verification of structural adequacy.
- All existing coax cables to be installed as indicated in this report.

Analysis

The existing tower was analyzed using a comprehensive computer program entitled tnxTower. The program analyzes the tower, considering the worst case loading condition. The tower is considered as loaded by concentric forces along the tower, and the model assumes that the tower members are subjected to bending, axial, and shear forces.

The existing tower was analyzed for the controlling basic wind speed (3-second gust) with no ice and the applicable wind and ice combination to determine stresses in members as per guidelines of TIA-222-G-2005 entitled “Structural Standard for Antenna Support Structures and Antennas”, the American Institute of Steel Construction (AISC) and the Manual of Steel Construction; Load and Resistance Factor Design (LRFD).

The controlling wind speed is determined by evaluating the local available wind speed data as provided in Appendix N of the CSBC¹ and the wind speed data available in the TIA-222-G-2005 Standard.

Tower Loading

Tower loading was determined by the basic wind speed as applied to projected surface areas with modification factors per TIA-222-G-2005, gravity loads of the tower structure and its components, and the application of 0.75” radial ice on the tower structure and its components.

Basic Wind Speed:	Westbrook; $v = 105$ mph (V_{asd})	[Appendix N of the 2018 CT Building Code]
Load Cases:	<u>Load Case 1</u> ; 105 mph wind speed w/ no ice plus gravity load – used in calculation of tower stresses and rotation.	[Appendix N of the 2018 CT Building Code]
	<u>Load Case 2</u> ; 50 mph wind speed w/ 0.75” radial ice plus gravity load – used in calculation of tower stresses.	[Annex B of TIA-222-G-2005]

¹ The 2015 International Building Code as amended by the 2018 Connecticut State Building Code (CSBC).

Tower Capacity

- Calculated stresses were found to be within allowable limits. This tower was found to be at **34.2%** of its total capacity.

Tower Section	Elevation	Stress Ratio (percentage of capacity)	Result
Pole Shaft (L1)	83.71'-130.00'	34.2%	PASS

Foundation and Anchors

The foundation consists of an 8.25-ft diameter x 4.5-ft long reinforced concrete pier on a 32.0-ft x 2.0-ft thick reinforce concrete pad. The sub-grade conditions used in the analysis of the foundation were obtained from the aforementioned original design documents. The base of the tower is connected to the foundation by means of (22) 2.25"Ø, ASTM A615-75 anchor bolts embedded approximately 6-ft into the concrete foundation structure.

- The tower base reactions developed from the governing Load Case 1 were used in the verification of the foundation and its anchors:

Location	Vector	Proposed Reactions
Base	Shear	43 kips
	Compression	63 kips
	Moment	3941 kip-ft

- The foundation was found to be within allowable limits.

Foundation	Design Limit	TIA-222-G Section 9.4 FS ⁽¹⁾	Proposed Loading (FS) ⁽¹⁾	Result
Reinforced Concrete Pad and Pier	OTM ⁽²⁾	1.0	2.83	PASS

Note 1: FS denotes Factor of Safety.

Note 2: OTM denotes Overturning Moment.

- The anchor bolts and base plate were found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (percentage of capacity)	Result
Anchor Bolts	Combined Axial and Shear	39.0%	PASS
Base Plate	Bending	60.6%	PASS

Conclusion

This analysis shows that the subject tower **is adequate** to support the proposed modified antenna configuration.

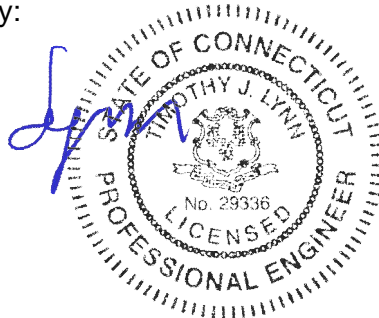
The analysis is based, in part, on the information provided to this office by T-Mobile. If the existing conditions are different than the information in this report, Centek Engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:



Timothy J. Lynn, PE
Structural Engineer



*Standard Conditions for Furnishing of
Professional Engineering Services on
Existing Structures*

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of Centek Engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to Centek Engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the “as new” condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222
- All services performed, results obtained, and recommendations made are in accordance with generally accepted engineering principles and practices. Centek Engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

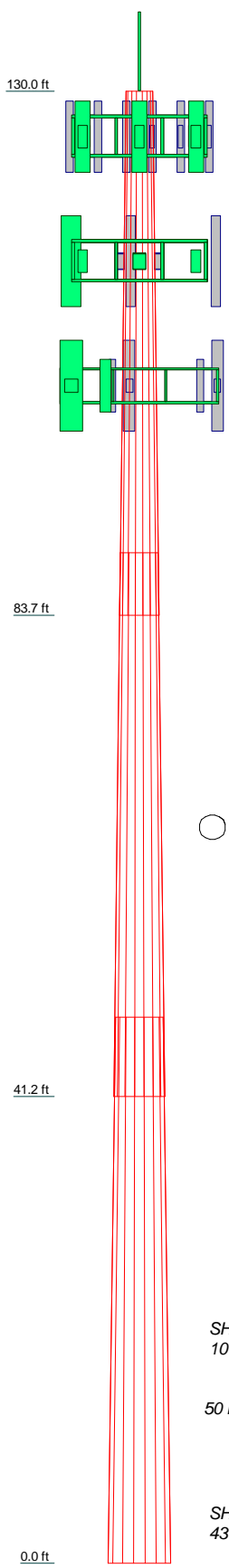
GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM

TnxTower, is an integrated structural analysis and design software package for Designed specifically for the telecommunications industry, TnxTower, formerly ERITower, automates much of the tower analysis and design required by the TIA/EIA 222 Standard.

TnxTower Features:

- TnxTower can analyze and design 3- and 4-sided guyed towers, 3- and 4-sided self-supporting towers and either round or tapered ground mounted poles with or without guys.
- The program analyzes towers using the TIA-222-G (2005) standard or any of the previous TIA/EIA standards back to RS-222 (1959). Steel design is checked using the AISC ASD 9th Edition or the AISC LRFD specifications.
- Linear and non-linear (P-delta) analyses can be used in determining displacements and forces in the structure. Wind pressures and forces are automatically calculated.
- Extensive graphics plots include material take-off, shear-moment, leg compression, displacement, twist, feed line, guy anchor and stress plots.
- TnxTower contains unique features such as True Cable behavior, hog rod take-up, foundation stiffness and much more.

Section	1	2	3
Length (ft)	46.280	48.000	48.210
Number of Sides	18	18	18
Thickness (in)	0.313	0.625	0.750
Socket Length (ft)	5.500	7.000	7.000
Top Dia (in)	28.000	39.297	50.418
Bot Dia (in)	41.530	53.780	65.000
Grade	5.4	A572-65	22.2
Weight (K)		14.9	42.5



DESIGNED APPURTENANCE LOADING

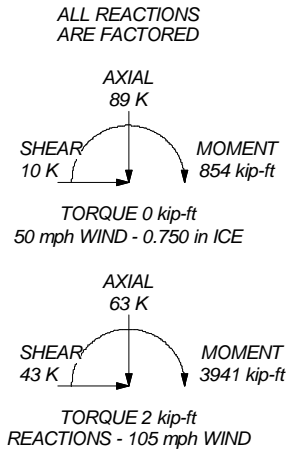
TYPE	ELEVATION	TYPE	ELEVATION
BOA4357	130	4415 B25 (ATI)	115
(3) NHH-65B-R2B (Verizon)	126	(2) DC6-48-60-18-8F Surge Arrestor (ATI)	115
(3) NHH-65B-R2B (Verizon)	126	12' V-Frame (ATI)	115
(3) RRH2x40-AWS (Verizon)	126	12' V-Frame (ATI)	115
(3) RRH2x40-AWS (Verizon)	126	12' V-Frame (ATI)	115
(3) RRH2x40-AWS (Verizon)	126	AIR32 (T-Mobile)	104
(2) RC2DC-3315-PF-48 (Verizon)	126	APXVAARR24-43 (T-Mobile)	104
13' Platform w/Rails (Verizon)	126	AIR32 (T-Mobile)	104
EPBQ-654L8H8-L2 (ATI)	115	APXVAARR24-43 (T-Mobile)	104
EPBQ-654L8H8-L2 (ATI)	115	AIR32 (T-Mobile)	104
EPBQ-654L8H8-L2 (ATI)	115	APXVAARR24-43 (T-Mobile)	104
4449 B5/B12 (ATI)	115	Radio 4449 B71 B12 (T-Mobile)	104
4449 B5/B12 (ATI)	115	Radio 4449 B71 B12 (T-Mobile)	104
4449 B5/B12 (ATI)	115	Radio 4449 B71 B12 (T-Mobile)	104
4415 B25 (ATI)	115	F4P-12W Quad Platform w/ Handrail (T-Mobile)	104
4415 B25 (ATI)	115		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower designed for Exposure D to the TIA-222-G Standard.
2. Tower designed for a 105 mph basic wind in accordance with the TIA-222-G Standard.
3. Tower is also designed for a 50 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 60 mph wind.
5. Tower Structure Class II.
6. Topographic Category 1 with Crest Height of 0.000 ft
7. TOWER RATING: 34.2%



Centek Engineering Inc.		
63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587		
Job: 19027.46 - CTHA524A	Project: 130-ft Rohn Monopole - 1546 Boston Post Rd., Westbrook, CT	
Client: T-Mobile	Drawn by: T.JL	App'd:
Code: TIA-222-G	Date: 05/05/20	Scale: NTS
Path:	Dwg No. E-1	

tnxTower Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	Job 19027.46 - CTHA524A	Page 1 of 21
	Project 130-ft Rohn Monopole - 1546 Boston Post Rd., Westbrook, CT	Date 06:46:57 05/05/20
	Client T-Mobile	Designed by TJL

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Basic wind speed of 105 mph.

Structure Class II.

Exposure Category D.

Topographic Category 1.

Crest Height 0.000 ft.

Nominal ice thickness of 0.750 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

<ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification √ Use Code Stress Ratios √ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric 	<ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension √ Bypass Mast Stability Checks Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs 	<ul style="list-style-type: none"> Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption Use TIA-222-G Tension Splice Exemption <li style="text-align: center;">Poles √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known
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Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of Sides	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft		in	in	in	in	
L1	130.000-83.710	46.290	5.500	18	28.000	41.530	0.313	1.250	A572-65 (65 ksi)

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Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L2	83.710-41.210	48.000	7.000	18	39.297	53.780	0.625	2.500	A572-65 (65 ksi)
L3	41.210-0.000	48.210		18	50.418	65.000	0.750	3.000	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	It/Q in ²	w in	w/t
L1	28.384	27.463	2659.779	9.829	14.224	186.992	5323.059	13.734	4.378	14.01
	42.122	40.883	8774.843	14.632	21.097	415.924	17561.231	20.445	6.759	21.63
L2	41.492	76.716	14495.338	13.729	19.963	726.107	29009.749	38.365	5.816	9.306
	54.513	105.446	37640.619	18.870	27.320	1377.756	75330.767	52.733	8.365	13.384
L3	53.230	118.235	36849.748	17.632	25.612	1438.751	73747.984	59.129	7.554	10.071
	65.887	152.947	79767.326	22.809	33.020	2415.728	159639.611	76.488	10.120	13.493

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
L1 130.000-83.710				1	1	1			
L2 83.710-41.210				1	1	1			
L3 41.210-0.000				1	1	1			

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	C _{AAA} ft ² /ft	Weight klf
7/8	C	No	Yes	Inside Pole	130.000 - 0.000	1	No Ice 1/2" Ice 1" Ice	0.000 0.000 0.001
1 5/8 (Verizon)	B	No	Yes	Inside Pole	126.000 - 3.000	9	No Ice 1/2" Ice 1" Ice	0.000 0.000 0.001
RG6-Fiber (AT&T)	C	No	Yes	Inside Pole	116.000 - 3.000	1	No Ice 1/2" Ice 1" Ice	0.000 0.000 0.001
#8 AWG Copper Wire (AT&T)	C	No	Yes	Inside Pole	116.000 - 3.000	4	No Ice 1/2" Ice 1" Ice	0.000 0.000 0.000
HYBRIFLEX 1-1/4" (T-Mobile)	A	No	Yes	Inside Pole	104.000 - 3.000	3	No Ice 1/2" Ice 1" Ice	0.000 0.000 0.001

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Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L1	130.000-83.710	A	0.000	0.000	0.000	0.000	0.079
		B	0.000	0.000	0.000	0.000	0.396
		C	0.000	0.000	0.000	0.000	0.064
L2	83.710-41.210	A	0.000	0.000	0.000	0.000	0.166
		B	0.000	0.000	0.000	0.000	0.398
		C	0.000	0.000	0.000	0.000	0.074
L3	41.210-0.000	A	0.000	0.000	0.000	0.000	0.149
		B	0.000	0.000	0.000	0.000	0.358
		C	0.000	0.000	0.000	0.000	0.068

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L1	130.000-83.710	A	1.685	0.000	0.000	0.000	0.000	0.079
		B		0.000	0.000	0.000	0.000	0.396
		C		0.000	0.000	0.000	0.000	0.064
L2	83.710-41.210	A	1.597	0.000	0.000	0.000	0.000	0.166
		B		0.000	0.000	0.000	0.000	0.398
		C		0.000	0.000	0.000	0.000	0.074
L3	41.210-0.000	A	1.430	0.000	0.000	0.000	0.000	0.149
		B		0.000	0.000	0.000	0.000	0.358
		C		0.000	0.000	0.000	0.000	0.068

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K	
BOA4357	C	From Face	1.000	0.000	130.000	No Ice	1.383	1.383	0.007
			0.000			1/2" Ice	2.100	2.100	0.018
			3.500			1" Ice	2.637	2.637	0.033
(3) NHH-65B-R2B (Verizon)	A	From Face	3.000	0.000	126.000	No Ice	11.187	8.687	0.071
			0.000			1/2" Ice	11.691	9.169	0.150

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz	Vert						
			ft	ft	°	ft	ft ²	ft ²	K	
(3) NHH-65B-R2B (Verizon)	B	From Face	0.000		0.000	126.000	1" Ice	12.202	9.658	0.236
			3.000				No Ice	11.187	8.687	0.071
			0.000				1/2" Ice	11.691	9.169	0.150
(3) NHH-65B-R2B (Verizon)	C	From Face	0.000		0.000	126.000	1" Ice	12.202	9.658	0.236
			3.000				No Ice	11.187	8.687	0.071
			0.000				1/2" Ice	11.691	9.169	0.150
(3) RRH2x40-AWS (Verizon)	A	From Face	0.000		0.000	126.000	1" Ice	12.202	9.658	0.236
			3.000				No Ice	2.161	1.420	0.044
			0.000				1/2" Ice	2.360	1.590	0.061
(3) RRH2x40-AWS (Verizon)	B	From Face	0.000		0.000	126.000	1" Ice	2.565	1.768	0.082
			3.000				No Ice	2.161	1.420	0.044
			0.000				1/2" Ice	2.360	1.590	0.061
(3) RRH2x40-AWS (Verizon)	C	From Face	0.000		0.000	126.000	1" Ice	2.565	1.768	0.082
			3.000				No Ice	2.161	1.420	0.044
			0.000				1/2" Ice	2.360	1.590	0.061
(2) RC2DC-3315-PF-48 (Verizon)	C	From Face	0.000		0.000	126.000	1" Ice	2.565	1.768	0.082
			3.000				No Ice	3.015	1.965	0.025
			0.000				1/2" Ice	3.234	2.153	0.051
13' Platform w/Rails (Verizon)	C	None			0.000	126.000	1" Ice	3.460	2.349	0.081
							No Ice	31.300	31.300	1.822
							1/2" Ice	40.200	40.200	2.452
EPBQ-654L8H8-L2 (AT&T)	A	From Face	0.000		0.000	115.000	1" Ice	49.100	49.100	3.082
			3.000				No Ice	18.089	7.033	0.095
			6.000				1/2" Ice	18.722	7.619	0.188
EPBQ-654L8H8-L2 (AT&T)	B	From Face	0.000		0.000	115.000	1" Ice	19.362	8.213	0.290
			3.000				No Ice	18.089	7.033	0.095
			6.000				1/2" Ice	18.722	7.619	0.188
EPBQ-654L8H8-L2 (AT&T)	C	From Face	0.000		0.000	115.000	1" Ice	19.362	8.213	0.290
			3.000				No Ice	18.089	7.033	0.095
			6.000				1/2" Ice	18.722	7.619	0.188
4449 B5/B12 (AT&T)	A	From Face	0.000		0.000	115.000	1" Ice	19.362	8.213	0.290
			0.500				No Ice	1.968	1.408	0.071
			0.000				1/2" Ice	2.144	1.564	0.090
4449 B5/B12 (AT&T)	B	From Face	0.000		0.000	115.000	1" Ice	2.328	1.727	0.111
			0.500				No Ice	1.968	1.408	0.071
			0.000				1/2" Ice	2.144	1.564	0.090
4449 B5/B12 (AT&T)	C	From Face	0.000		0.000	115.000	1" Ice	2.328	1.727	0.111
			0.500				No Ice	1.968	1.408	0.071
			0.000				1/2" Ice	2.144	1.564	0.090
4415 B25 (AT&T)	A	From Face	0.000		0.000	115.000	1" Ice	2.328	1.727	0.111
			0.500				No Ice	1.843	0.820	0.046
			0.000				1/2" Ice	2.012	0.943	0.060
4415 B25 (AT&T)	B	From Face	0.000		0.000	115.000	1" Ice	2.190	1.075	0.077
			0.500				No Ice	1.843	0.820	0.046
			0.000				1/2" Ice	2.012	0.943	0.060
4415 B25 (AT&T)	C	From Face	0.000		0.000	115.000	1" Ice	2.190	1.075	0.077
			0.500				No Ice	1.843	0.820	0.046
			0.000				1/2" Ice	2.012	0.943	0.060
(2) DC6-48-60-18-8F Surge Arrestor (AT&T)	C	From Face	0.000		0.000	115.000	1" Ice	2.190	1.075	0.077
			0.500				No Ice	1.909	1.909	0.020
			0.000				1/2" Ice	2.098	2.098	0.039
12' V-Frame (AT&T)	A	None			0.000	115.000	1" Ice	2.294	2.294	0.062
							No Ice	9.220	12.970	0.300
							1/2" Ice	9.220	12.970	0.400
12' V-Frame (AT&T)	B	None			0.000	115.000	1" Ice	9.220	12.970	0.500
							No Ice	9.220	12.970	0.300
							1/2" Ice	9.220	12.970	0.400

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz	Vert						
			ft	ft	°	ft	ft ²	ft ²	K	
12' V-Frame (AT&T)	C	None			0.000	115.000	1" Ice	9.220	12.970	0.500
							No Ice	9.220	12.970	0.300
							1/2" Ice	9.220	12.970	0.400
AIR32 (T-Mobile)	A	From Face	3.000	0.000	0.000	104.000	1" Ice	9.220	12.970	0.500
							No Ice	6.510	4.712	0.133
							1/2" Ice	6.887	5.068	0.179
APXVAARR24-43 (T-Mobile)	A	From Face	3.000	0.000	0.000	104.000	1" Ice	7.271	5.431	0.230
							No Ice	20.243	8.889	0.153
							1/2" Ice	20.890	9.487	0.266
AIR32 (T-Mobile)	B	From Face	3.000	0.000	0.000	104.000	1" Ice	21.544	10.092	0.387
							No Ice	6.510	4.712	0.133
							1/2" Ice	6.887	5.068	0.179
APXVAARR24-43 (T-Mobile)	B	From Face	3.000	0.000	0.000	104.000	1" Ice	7.271	5.431	0.230
							No Ice	20.243	8.889	0.153
							1/2" Ice	20.890	9.487	0.266
AIR32 (T-Mobile)	C	From Face	3.000	0.000	0.000	104.000	1" Ice	21.544	10.092	0.387
							No Ice	6.510	4.712	0.133
							1/2" Ice	6.887	5.068	0.179
APXVAARR24-43 (T-Mobile)	C	From Face	3.000	0.000	0.000	104.000	1" Ice	7.271	5.431	0.230
							No Ice	20.243	8.889	0.153
							1/2" Ice	20.890	9.487	0.266
Radio 4449 B71 B12 (T-Mobile)	A	From Face	3.000	0.000	0.000	104.000	1" Ice	21.544	10.092	0.387
							No Ice	1.639	1.291	0.074
							1/2" Ice	1.799	1.436	0.091
Radio 4449 B71 B12 (T-Mobile)	B	From Face	3.000	0.000	0.000	104.000	1" Ice	1.966	1.587	0.111
							No Ice	1.639	1.291	0.074
							1/2" Ice	1.799	1.436	0.091
Radio 4449 B71 B12 (T-Mobile)	C	From Face	3.000	0.000	0.000	104.000	1" Ice	1.966	1.587	0.111
							No Ice	1.639	1.291	0.074
							1/2" Ice	1.799	1.436	0.091
F4P-12W Quad Platform w/ Handrail (T-Mobile)	C	None			0.000	104.000	1" Ice	1.966	1.587	0.111
							No Ice	35.000	35.000	2.500
							1/2" Ice	41.000	41.000	3.100
							1" Ice	47.000	47.000	3.700

Tower Pressures - No Ice

$$G_H = 1.100$$

Section Elevation	z	K _Z	q _z	A _G	F _a	A _F	A _R	A _{leg}	Leg %	C _A A _A In Face	C _A A _A Out Face
ft	ft		ksf	ft ²	e	ft ²	ft ²	ft ²		ft ²	ft ²
L1 130.000-83.710	105.568	1.446	0.039	135.989	A	0.000	135.989	135.989	100.00	0.000	0.000
					B	0.000	135.989			0.000	0.000
					C	0.000	135.989			0.000	0.000
L2 83.710-41.210	61.816	1.318	0.035	170.010	A	0.000	170.010	170.010	100.00	0.000	0.000
					B	0.000	170.010			0.000	0.000
					C	0.000	170.010			0.000	0.000
L3	20.517	1.088	0.029	204.534	A	0.000	204.534	204.534	100.00	0.000	0.000

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Section Elevation	z	K _Z	q _z	A _G	F _{a c e}	A _F	A _R	A _{leg}	Leg %	C _{AA} _{In Face}	C _{AA} _{Out Face}
ft	ft		ksf	ft ²		ft ²	ft ²	ft ²		ft ²	ft ²
41.210-0.000					B	0.000	204.534		100.00	0.000	0.000
					C	0.000	204.534		100.00	0.000	0.000

Tower Pressure - With Ice

$G_H = 1.100$

Section Elevation	z	K _Z	q _z	t _z	A _G	F _{a c e}	A _F	A _R	A _{leg}	Leg %	C _{AA} _{In Face}	C _{AA} _{Out Face}
ft	ft		ksf	in	ft ²		ft ²	ft ²	ft ²		ft ²	ft ²
L1 130.000-83.710	105.568	1.446	0.009	1.685	148.988	A	0.000	148.988	148.988	100.00	0.000	0.000
						B	0.000	148.988		100.00	0.000	0.000
						C	0.000	148.988		100.00	0.000	0.000
L2 83.710-41.210	61.816	1.318	0.008	1.597	181.945	A	0.000	181.945	181.945	100.00	0.000	0.000
						B	0.000	181.945		100.00	0.000	0.000
						C	0.000	181.945		100.00	0.000	0.000
L3 41.210-0.000	20.517	1.088	0.007	1.430	215.504	A	0.000	215.504	215.504	100.00	0.000	0.000
						B	0.000	215.504		100.00	0.000	0.000
						C	0.000	215.504		100.00	0.000	0.000

Tower Pressure - Service

$G_H = 1.100$

Section Elevation	z	K _Z	q _z	A _G	F _{a c e}	A _F	A _R	A _{leg}	Leg %	C _{AA} _{In Face}	C _{AA} _{Out Face}
ft	ft		ksf	ft ²		ft ²	ft ²	ft ²		ft ²	ft ²
L1 130.000-83.710	105.568	1.446	0.011	135.989	A	0.000	135.989	135.989	100.00	0.000	0.000
					B	0.000	135.989		100.00	0.000	0.000
					C	0.000	135.989		100.00	0.000	0.000
L2 83.710-41.210	61.816	1.318	0.010	170.010	A	0.000	170.010	170.010	100.00	0.000	0.000
					B	0.000	170.010		100.00	0.000	0.000
					C	0.000	170.010		100.00	0.000	0.000
L3 41.210-0.000	20.517	1.088	0.009	204.534	A	0.000	204.534	204.534	100.00	0.000	0.000
					B	0.000	204.534		100.00	0.000	0.000
					C	0.000	204.534		100.00	0.000	0.000

Tower Forces - No Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F _{a c e}	e	C _F	q _z	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K				ksf			ft ²	K	klf	
L1 130.000-83.710	0.539	5.383	A	1	0.65	0.039	1	1	135.989	3.767	0.081	C
			B	1	0.65		1	1	135.989			
			C	1	0.65		1	1	135.989			
L2	0.637	14.877	A	1	0.65	0.035	1	1	170.010	4.283	0.101	C

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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
83.710-41.210			B	1	0.65		1	1	170.010			
			C	1	0.65		1	1	170.010			
L3 41.210-0.000	0.575	22.243	A	1	0.65	0.029	1	1	204.534	4.293	0.104	C
			B	1	0.65		1	1	204.534			
			C	1	0.65		1	1	204.534			
Sum Weight:	1.751	42.503						OTM	750.441 kip-ft	12.342		

Tower Forces - No Ice - Wind 45 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
L1 130.000-83.710	0.539	5.383	A	1	0.65	0.039	1	1	135.989	3.767	0.081	C
			B	1	0.65		1	1	135.989			
			C	1	0.65		1	1	135.989			
L2 83.710-41.210	0.637	14.877	A	1	0.65	0.035	1	1	170.010	4.283	0.101	C
			B	1	0.65		1	1	170.010			
			C	1	0.65		1	1	170.010			
L3 41.210-0.000	0.575	22.243	A	1	0.65	0.029	1	1	204.534	4.293	0.104	C
			B	1	0.65		1	1	204.534			
			C	1	0.65		1	1	204.534			
Sum Weight:	1.751	42.503						OTM	750.441 kip-ft	12.342		

Tower Forces - No Ice - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
L1 130.000-83.710	0.539	5.383	A	1	0.65	0.039	1	1	135.989	3.767	0.081	C
			B	1	0.65		1	1	135.989			
			C	1	0.65		1	1	135.989			
L2 83.710-41.210	0.637	14.877	A	1	0.65	0.035	1	1	170.010	4.283	0.101	C
			B	1	0.65		1	1	170.010			
			C	1	0.65		1	1	170.010			
L3 41.210-0.000	0.575	22.243	A	1	0.65	0.029	1	1	204.534	4.293	0.104	C
			B	1	0.65		1	1	204.534			
			C	1	0.65		1	1	204.534			
Sum Weight:	1.751	42.503						OTM	750.441 kip-ft	12.342		

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Tower Forces - No Ice - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
L1 130.000-83.710	0.539	5.383	A	1	0.65	0.039	1	1	135.989	3.767	0.081	C
			B	1	0.65		1	1	135.989			
			C	1	0.65		1	1	135.989			
L2 83.710-41.210	0.637	14.877	A	1	0.65	0.035	1	1	170.010	4.283	0.101	C
			B	1	0.65		1	1	170.010			
			C	1	0.65		1	1	170.010			
L3 41.210-0.000	0.575	22.243	A	1	0.65	0.029	1	1	204.534	4.293	0.104	C
			B	1	0.65		1	1	204.534			
			C	1	0.65		1	1	204.534			
Sum Weight:	1.751	42.503						OTM	750.441 kip-ft	12.342		

Tower Forces - With Ice - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
L1 130.000-83.710	0.539	8.892	A	1	1.2	0.009	1	1	148.988	1.728	0.037	C
			B	1	1.2		1	1	148.988			
			C	1	1.2		1	1	148.988			
L2 83.710-41.210	0.637	18.979	A	1	1.2	0.008	1	1	181.945	1.919	0.045	C
			B	1	1.2		1	1	181.945			
			C	1	1.2		1	1	181.945			
L3 41.210-0.000	0.575	26.623	A	1	1.2	0.007	1	1	215.504	1.894	0.046	C
			B	1	1.2		1	1	215.504			
			C	1	1.2		1	1	215.504			
Sum Weight:	1.751	54.495						OTM	339.826 kip-ft	5.540		

Tower Forces - With Ice - Wind 45 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
L1 130.000-83.710	0.539	8.892	A	1	1.2	0.009	1	1	148.988	1.728	0.037	C
			B	1	1.2		1	1	148.988			
			C	1	1.2		1	1	148.988			
L2 83.710-41.210	0.637	18.979	A	1	1.2	0.008	1	1	181.945	1.919	0.045	C
			B	1	1.2		1	1	181.945			
			C	1	1.2		1	1	181.945			
L3 41.210-0.000	0.575	26.623	A	1	1.2	0.007	1	1	215.504	1.894	0.046	C
			B	1	1.2		1	1	215.504			
			C	1	1.2		1	1	215.504			
Sum Weight:	1.751	54.495						OTM	339.826	5.540		

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	q _z	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K				ksf			ft ²	K	klf	
									kip-ft			

Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	q _z	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K				ksf			ft ²	K	klf	
L1 130.000-83.710	0.539	8.892	A	1	1.2	0.009	1	1	148.988	1.728	0.037	C
			B	1	1.2		1	1	148.988			
			C	1	1.2		1	1	148.988			
L2 83.710-41.210	0.637	18.979	A	1	1.2	0.008	1	1	181.945	1.919	0.045	C
			B	1	1.2		1	1	181.945			
			C	1	1.2		1	1	181.945			
L3 41.210-0.000	0.575	26.623	A	1	1.2	0.007	1	1	215.504	1.894	0.046	C
			B	1	1.2		1	1	215.504			
			C	1	1.2		1	1	215.504			
Sum Weight:	1.751	54.495						OTM	339.826 kip-ft	5.540		

Tower Forces - With Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	q _z	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K				ksf			ft ²	K	klf	
L1 130.000-83.710	0.539	8.892	A	1	1.2	0.009	1	1	148.988	1.728	0.037	C
			B	1	1.2		1	1	148.988			
			C	1	1.2		1	1	148.988			
L2 83.710-41.210	0.637	18.979	A	1	1.2	0.008	1	1	181.945	1.919	0.045	C
			B	1	1.2		1	1	181.945			
			C	1	1.2		1	1	181.945			
L3 41.210-0.000	0.575	26.623	A	1	1.2	0.007	1	1	215.504	1.894	0.046	C
			B	1	1.2		1	1	215.504			
			C	1	1.2		1	1	215.504			
Sum Weight:	1.751	54.495						OTM	339.826 kip-ft	5.540		

Tower Forces - Service - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	q _z	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K				ksf			ft ²	K	klf	

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Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
L1 130.000-83.710	0.539	5.383	A	1	0.65	0.011	1	1	135.989	1.100	0.024	C
			B	1	0.65		1	1	135.989			
			C	1	0.65		1	1	135.989			
L2 83.710-41.210	0.637	14.877	A	1	0.65	0.010	1	1	170.010	1.251	0.029	C
			B	1	0.65		1	1	170.010			
			C	1	0.65		1	1	170.010			
L3 41.210-0.000	0.575	22.243	A	1	0.65	0.009	1	1	204.534	1.254	0.030	C
			B	1	0.65		1	1	204.534			
			C	1	0.65		1	1	204.534			
Sum Weight:	1.751	42.503						OTM	219.248 kip-ft	3.606		

Tower Forces - Service - Wind 45 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
L1 130.000-83.710	0.539	5.383	A	1	0.65	0.011	1	1	135.989	1.100	0.024	C
			B	1	0.65		1	1	135.989			
			C	1	0.65		1	1	135.989			
L2 83.710-41.210	0.637	14.877	A	1	0.65	0.010	1	1	170.010	1.251	0.029	C
			B	1	0.65		1	1	170.010			
			C	1	0.65		1	1	170.010			
L3 41.210-0.000	0.575	22.243	A	1	0.65	0.009	1	1	204.534	1.254	0.030	C
			B	1	0.65		1	1	204.534			
			C	1	0.65		1	1	204.534			
Sum Weight:	1.751	42.503						OTM	219.248 kip-ft	3.606		

Tower Forces - Service - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
L1 130.000-83.710	0.539	5.383	A	1	0.65	0.011	1	1	135.989	1.100	0.024	C
			B	1	0.65		1	1	135.989			
			C	1	0.65		1	1	135.989			
L2 83.710-41.210	0.637	14.877	A	1	0.65	0.010	1	1	170.010	1.251	0.029	C
			B	1	0.65		1	1	170.010			
			C	1	0.65		1	1	170.010			
L3 41.210-0.000	0.575	22.243	A	1	0.65	0.009	1	1	204.534	1.254	0.030	C
			B	1	0.65		1	1	204.534			
			C	1	0.65		1	1	204.534			
Sum Weight:	1.751	42.503						OTM	219.248 kip-ft	3.606		

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Tower Forces - Service - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C _F	q _z ksf	D _F	D _R	A _E ft ²	F K	w klf	Ctrl. Face
L1 130.000-83.710	0.539	5.383	A	1	0.65	0.011	1	1	135.989	1.100	0.024	C
0			B	1	0.65		1	1	135.989			
			C	1	0.65		1	1	135.989			
L2 83.710-41.210	0.637	14.877	A	1	0.65	0.010	1	1	170.010	1.251	0.029	C
			B	1	0.65		1	1	170.010			
			C	1	0.65		1	1	170.010			
L3 41.210-0.000	0.575	22.243	A	1	0.65	0.009	1	1	204.534	1.254	0.030	C
			B	1	0.65		1	1	204.534			
			C	1	0.65		1	1	204.534			
Sum Weight:	1.751	42.503						OTM	219.248 kip-ft	3.606		

Force Totals

Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M _x kip-ft	Sum of Overturning Moments, M _z kip-ft	Sum of Torques kip-ft
Leg Weight	42.503					
Bracing Weight	0.000					
Total Member Self-Weight	42.503			0.300	0.000	
Total Weight	52.324			0.300	0.000	
Wind 0 deg - No Ice		0.000	-26.816	-2438.151	0.000	0.000
Wind 30 deg - No Ice		13.362	-23.223	-2111.461	-1213.403	0.584
Wind 45 deg - No Ice		18.896	-18.962	-1723.945	-1716.011	0.826
Wind 60 deg - No Ice		23.143	-13.408	-1218.926	-2101.676	1.011
Wind 90 deg - No Ice		26.724	0.000	0.300	-2426.806	1.168
Wind 120 deg - No Ice		23.143	13.408	1219.525	-2101.676	1.011
Wind 135 deg - No Ice		18.896	18.962	1724.545	-1716.011	0.826
Wind 150 deg - No Ice		13.362	23.223	2112.060	-1213.403	0.584
Wind 180 deg - No Ice		0.000	26.816	2438.751	0.000	0.000
Wind 210 deg - No Ice		-13.362	23.223	2112.060	1213.403	-0.584
Wind 225 deg - No Ice		-18.896	18.962	1724.545	1716.011	-0.826
Wind 240 deg - No Ice		-23.143	13.408	1219.525	2101.676	-1.011
Wind 270 deg - No Ice		-26.724	0.000	0.300	2426.806	-1.168
Wind 300 deg - No Ice		-23.143	-13.408	-1218.926	2101.676	-1.011
Wind 315 deg - No Ice		-18.896	-18.962	-1723.945	1716.011	-0.826
Wind 330 deg - No Ice		-13.362	-23.223	-2111.461	1213.403	-0.584
Member Ice	11.992					
Total Weight Ice	76.809			1.615	0.000	
Wind 0 deg - Ice		0.000	-9.793	-835.478	0.000	0.000
Wind 30 deg - Ice		4.885	-8.481	-723.329	-417.095	0.194
Wind 45 deg - Ice		6.908	-6.925	-590.299	-589.861	0.274
Wind 60 deg - Ice		8.461	-4.896	-416.931	-722.429	0.336
Wind 90 deg - Ice		9.770	0.000	1.615	-834.190	0.388
Wind 120 deg - Ice		8.461	4.896	420.161	-722.429	0.336
Wind 135 deg - Ice		6.908	6.925	593.529	-589.861	0.274
Wind 150 deg - Ice		4.885	8.481	726.559	-417.095	0.194

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Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M_x kip-ft	Sum of Overturning Moments, M_z kip-ft	Sum of Torques kip-ft
Wind 180 deg - Ice		0.000	9.793	838.708	0.000	0.000
Wind 210 deg - Ice		-4.885	8.481	726.559	417.095	-0.194
Wind 225 deg - Ice		-6.908	6.925	593.529	589.861	-0.274
Wind 240 deg - Ice		-8.461	4.896	420.161	722.429	-0.336
Wind 270 deg - Ice		-9.770	0.000	1.615	834.190	-0.388
Wind 300 deg - Ice		-8.461	-4.896	-416.931	722.429	-0.336
Wind 315 deg - Ice		-6.908	-6.925	-590.299	589.861	-0.274
Wind 330 deg - Ice		-4.885	-8.481	-723.329	417.095	-0.194
Total Weight	52.324			0.300	0.000	
Wind 0 deg - Service		0.000	-7.835	-712.115	0.000	0.000
Wind 30 deg - Service		3.904	-6.785	-616.670	-354.507	0.171
Wind 45 deg - Service		5.521	-5.540	-503.454	-501.348	0.241
Wind 60 deg - Service		6.762	-3.917	-355.908	-614.023	0.295
Wind 90 deg - Service		7.808	0.000	0.300	-709.013	0.341
Wind 120 deg - Service		6.762	3.917	356.508	-614.023	0.295
Wind 135 deg - Service		5.521	5.540	504.054	-501.348	0.241
Wind 150 deg - Service		3.904	6.785	617.270	-354.507	0.171
Wind 180 deg - Service		0.000	7.835	712.715	0.000	0.000
Wind 210 deg - Service		-3.904	6.785	617.270	354.507	-0.171
Wind 225 deg - Service		-5.521	5.540	504.054	501.348	-0.241
Wind 240 deg - Service		-6.762	3.917	356.508	614.023	-0.295
Wind 270 deg - Service		-7.808	0.000	0.300	709.013	-0.341
Wind 300 deg - Service		-6.762	-3.917	-355.908	614.023	-0.295
Wind 315 deg - Service		-5.521	-5.540	-503.454	501.348	-0.241
Wind 330 deg - Service		-3.904	-6.785	-616.670	354.507	-0.171

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 45 deg - No Ice
7	0.9 Dead+1.6 Wind 45 deg - No Ice
8	1.2 Dead+1.6 Wind 60 deg - No Ice
9	0.9 Dead+1.6 Wind 60 deg - No Ice
10	1.2 Dead+1.6 Wind 90 deg - No Ice
11	0.9 Dead+1.6 Wind 90 deg - No Ice
12	1.2 Dead+1.6 Wind 120 deg - No Ice
13	0.9 Dead+1.6 Wind 120 deg - No Ice
14	1.2 Dead+1.6 Wind 135 deg - No Ice
15	0.9 Dead+1.6 Wind 135 deg - No Ice
16	1.2 Dead+1.6 Wind 150 deg - No Ice
17	0.9 Dead+1.6 Wind 150 deg - No Ice
18	1.2 Dead+1.6 Wind 180 deg - No Ice
19	0.9 Dead+1.6 Wind 180 deg - No Ice
20	1.2 Dead+1.6 Wind 210 deg - No Ice
21	0.9 Dead+1.6 Wind 210 deg - No Ice
22	1.2 Dead+1.6 Wind 225 deg - No Ice
23	0.9 Dead+1.6 Wind 225 deg - No Ice
24	1.2 Dead+1.6 Wind 240 deg - No Ice
25	0.9 Dead+1.6 Wind 240 deg - No Ice
26	1.2 Dead+1.6 Wind 270 deg - No Ice

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Comb. No.	Description
27	0.9 Dead+1.6 Wind 270 deg - No Ice
28	1.2 Dead+1.6 Wind 300 deg - No Ice
29	0.9 Dead+1.6 Wind 300 deg - No Ice
30	1.2 Dead+1.6 Wind 315 deg - No Ice
31	0.9 Dead+1.6 Wind 315 deg - No Ice
32	1.2 Dead+1.6 Wind 330 deg - No Ice
33	0.9 Dead+1.6 Wind 330 deg - No Ice
34	1.2 Dead+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 45 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
39	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
40	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
41	1.2 Dead+1.0 Wind 135 deg+1.0 Ice+1.0 Temp
42	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
43	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
44	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
45	1.2 Dead+1.0 Wind 225 deg+1.0 Ice+1.0 Temp
46	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
47	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
48	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
49	1.2 Dead+1.0 Wind 315 deg+1.0 Ice+1.0 Temp
50	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
51	Dead+Wind 0 deg - Service
52	Dead+Wind 30 deg - Service
53	Dead+Wind 45 deg - Service
54	Dead+Wind 60 deg - Service
55	Dead+Wind 90 deg - Service
56	Dead+Wind 120 deg - Service
57	Dead+Wind 135 deg - Service
58	Dead+Wind 150 deg - Service
59	Dead+Wind 180 deg - Service
60	Dead+Wind 210 deg - Service
61	Dead+Wind 225 deg - Service
62	Dead+Wind 240 deg - Service
63	Dead+Wind 270 deg - Service
64	Dead+Wind 300 deg - Service
65	Dead+Wind 315 deg - Service
66	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	130 - 83.71	Pole	Max Tension	34	0.000	0.000	0.000
			Max. Compression	34	-31.398	0.000	-1.675
			Max. Mx	10	-15.141	-740.103	-0.342
			Max. My	18	-15.133	0.000	-745.963
			Max. Vy	10	28.582	-740.103	-0.342
			Max. Vx	18	28.732	0.000	-745.963
			Max. Torque	10			-1.873
			Max Tension	1	0.000	0.000	0.000
L2	83.71 - 41.21	Pole	Max. Compression	34	-52.287	0.000	-1.675
			Max. Mx	10	-31.908	-2042.586	-0.363
			Max. My	18	-31.904	0.000	-2054.601
			Max. Vy	10	35.060	-2042.586	-0.363
			Max. Vx	18	35.210	0.000	-2054.601

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	Client T-Mobile	Designed by TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L3	41.21 - 0	Pole	Max. Torque	10			-1.872
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	34	-88.549	0.000	-1.675
			Max. M _x	10	-62.777	-3922.220	-0.366
			Max. M _y	18	-62.777	0.000	-3941.430
			Max. V _y	10	42.775	-3922.220	-0.366
			Max. V _x	18	42.923	0.000	-3941.430
			Max. Torque	10			-1.872

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	34	88.549	0.000	0.000
	Max. H _x	26	62.789	42.758	0.000
	Max. H _z	2	62.789	0.000	42.906
	Max. M _x	2	3940.696	0.000	42.906
	Max. M _z	10	3922.220	-42.758	0.000
	Max. Torsion	26	1.871	42.758	0.000
	Min. Vert	31	47.091	30.234	30.339
	Min. H _x	10	62.789	-42.758	0.000
	Min. H _z	18	62.789	0.000	-42.906
	Min. M _x	18	-3941.430	0.000	-42.906
	Min. M _z	26	-3922.220	42.758	0.000
	Min. Torsion	10	-1.871	-42.758	0.000

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	52.324	0.000	0.000	0.300	0.000	0.000
1.2 Dead+1.6 Wind 0 deg - No Ice	62.789	0.000	-42.906	-3940.696	0.000	0.000
0.9 Dead+1.6 Wind 0 deg - No Ice	47.091	0.000	-42.906	-3930.675	0.000	0.000
1.2 Dead+1.6 Wind 30 deg - No Ice	62.789	21.379	-37.157	-3412.696	-1961.107	0.935
0.9 Dead+1.6 Wind 30 deg - No Ice	47.091	21.379	-37.157	-3404.029	-1956.078	0.935
1.2 Dead+1.6 Wind 45 deg - No Ice	62.789	30.234	-30.339	-2786.389	-2773.426	1.323
0.9 Dead+1.6 Wind 45 deg - No Ice	47.091	30.234	-30.339	-2779.329	-2766.313	1.322
1.2 Dead+1.6 Wind 60 deg - No Ice	62.789	37.029	-21.453	-1970.168	-3396.741	1.620
0.9 Dead+1.6 Wind 60 deg - No Ice	47.091	37.029	-21.453	-1965.204	-3388.029	1.619
1.2 Dead+1.6 Wind 90 deg - No Ice	62.789	42.758	0.000	0.366	-3922.220	1.871
0.9 Dead+1.6 Wind 90 deg - No Ice	47.091	42.758	0.000	0.273	-3912.161	1.870

<p style="text-align: center;">tnxTower</p> <p style="text-align: center;">Centek Engineering Inc. 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587</p>	<p style="text-align: center;">Job</p> <p style="text-align: center;">19027.46 - CTHA524A</p>	<p style="text-align: center;">Page</p> <p style="text-align: center;">15 of 21</p>
	<p style="text-align: center;">Project</p> <p style="text-align: center;">130-ft Rohn Monopole - 1546 Boston Post Rd., Westbrook, CT</p>	<p style="text-align: center;">Date</p> <p style="text-align: center;">06:46:57 05/05/20</p>
	<p style="text-align: center;">Client</p> <p style="text-align: center;">T-Mobile</p>	<p style="text-align: center;">Designed by</p> <p style="text-align: center;">TJL</p>

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
1.2 Dead+1.6 Wind 120 deg - No Ice	62.789	37.029	21.453	1970.900	-3396.742	1.621
0.9 Dead+1.6 Wind 120 deg - No Ice	47.091	37.029	21.453	1965.750	-3388.030	1.620
1.2 Dead+1.6 Wind 135 deg - No Ice	62.789	30.234	30.339	2787.121	-2773.427	1.324
0.9 Dead+1.6 Wind 135 deg - No Ice	47.091	30.234	30.339	2779.876	-2766.314	1.323
1.2 Dead+1.6 Wind 150 deg - No Ice	62.789	21.379	37.157	3413.429	-1961.108	0.936
0.9 Dead+1.6 Wind 150 deg - No Ice	47.091	21.379	37.157	3404.576	-1956.079	0.935
1.2 Dead+1.6 Wind 180 deg - No Ice	62.789	0.000	42.906	3941.430	0.000	0.000
0.9 Dead+1.6 Wind 180 deg - No Ice	47.091	0.000	42.906	3931.222	0.000	0.000
1.2 Dead+1.6 Wind 210 deg - No Ice	62.789	-21.379	37.157	3413.429	1961.108	-0.936
0.9 Dead+1.6 Wind 210 deg - No Ice	47.091	-21.379	37.157	3404.576	1956.079	-0.935
1.2 Dead+1.6 Wind 225 deg - No Ice	62.789	-30.234	30.339	2787.121	2773.427	-1.324
0.9 Dead+1.6 Wind 225 deg - No Ice	47.091	-30.234	30.339	2779.876	2766.314	-1.323
1.2 Dead+1.6 Wind 240 deg - No Ice	62.789	-37.029	21.453	1970.900	3396.742	-1.621
0.9 Dead+1.6 Wind 240 deg - No Ice	47.091	-37.029	21.453	1965.750	3388.030	-1.620
1.2 Dead+1.6 Wind 270 deg - No Ice	62.789	-42.758	0.000	0.366	3922.220	-1.871
0.9 Dead+1.6 Wind 270 deg - No Ice	47.091	-42.758	0.000	0.273	3912.161	-1.870
1.2 Dead+1.6 Wind 300 deg - No Ice	62.789	-37.029	-21.453	-1970.168	3396.741	-1.620
0.9 Dead+1.6 Wind 300 deg - No Ice	47.091	-37.029	-21.453	-1965.204	3388.029	-1.619
1.2 Dead+1.6 Wind 315 deg - No Ice	62.789	-30.234	-30.339	-2786.389	2773.426	-1.323
0.9 Dead+1.6 Wind 315 deg - No Ice	47.091	-30.234	-30.339	-2779.329	2766.313	-1.322
1.2 Dead+1.6 Wind 330 deg - No Ice	62.789	-21.379	-37.157	-3412.696	1961.107	-0.935
0.9 Dead+1.6 Wind 330 deg - No Ice	47.091	-21.379	-37.157	-3404.029	1956.078	-0.935
1.2 Dead+1.0 Ice+1.0 Temp	88.549	0.000	0.000	1.675	0.000	0.000
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	88.549	0.000	-9.793	-850.387	0.000	0.000
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	88.549	4.885	-8.481	-736.224	-424.581	0.197
1.2 Dead+1.0 Wind 45 deg+1.0 Ice+1.0 Temp	88.549	6.908	-6.925	-600.805	-600.448	0.278
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	88.549	8.461	-4.896	-424.323	-735.396	0.340
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	88.549	9.770	0.000	1.741	-849.162	0.393
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	88.549	8.461	4.896	427.804	-735.396	0.340
1.2 Dead+1.0 Wind 135 deg+1.0 Ice+1.0 Temp	88.549	6.908	6.925	604.286	-600.448	0.278
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	88.549	4.885	8.481	739.705	-424.581	0.197

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	Client T-Mobile	Designed by TJL

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	88.549	0.000	9.793	853.868	0.000	0.000
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	88.549	-4.885	8.481	739.705	424.581	-0.197
1.2 Dead+1.0 Wind 225 deg+1.0 Ice+1.0 Temp	88.549	-6.908	6.925	604.286	600.448	-0.278
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	88.549	-8.461	4.896	427.804	735.396	-0.340
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	88.549	-9.770	0.000	1.741	849.162	-0.393
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	88.549	-8.461	-4.896	-424.323	735.396	-0.340
1.2 Dead+1.0 Wind 315 deg+1.0 Ice+1.0 Temp	88.549	-6.908	-6.925	-600.805	600.448	-0.278
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	88.549	-4.885	-8.481	-736.224	424.581	-0.197
Dead+Wind 0 deg - Service	52.324	0.000	-7.835	-718.192	0.000	0.000
Dead+Wind 30 deg - Service	52.324	3.904	-6.785	-621.932	-357.531	0.171
Dead+Wind 45 deg - Service	52.324	5.521	-5.540	-507.749	-505.625	0.242
Dead+Wind 60 deg - Service	52.324	6.762	-3.917	-358.944	-619.262	0.296
Dead+Wind 90 deg - Service	52.324	7.808	0.000	0.305	-715.062	0.342
Dead+Wind 120 deg - Service	52.324	6.762	3.917	359.554	-619.262	0.296
Dead+Wind 135 deg - Service	52.324	5.521	5.540	508.359	-505.625	0.242
Dead+Wind 150 deg - Service	52.324	3.904	6.785	622.542	-357.531	0.171
Dead+Wind 180 deg - Service	52.324	0.000	7.835	718.802	0.000	0.000
Dead+Wind 210 deg - Service	52.324	-3.904	6.785	622.542	357.531	-0.171
Dead+Wind 225 deg - Service	52.324	-5.521	5.540	508.359	505.625	-0.242
Dead+Wind 240 deg - Service	52.324	-6.762	3.917	359.554	619.262	-0.296
Dead+Wind 270 deg - Service	52.324	-7.808	0.000	0.305	715.062	-0.342
Dead+Wind 300 deg - Service	52.324	-6.762	-3.917	-358.944	619.262	-0.296
Dead+Wind 315 deg - Service	52.324	-5.521	-5.540	-507.749	505.625	-0.242
Dead+Wind 330 deg - Service	52.324	-3.904	-6.785	-621.932	357.531	-0.171

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.000	-52.324	0.000	0.000	52.324	0.000	0.000%
2	0.000	-62.789	-42.906	0.000	62.789	42.906	0.000%
3	0.000	-47.091	-42.906	0.000	47.091	42.906	0.000%
4	21.379	-62.789	-37.157	-21.379	62.789	37.157	0.000%
5	21.379	-47.091	-37.157	-21.379	47.091	37.157	0.000%
6	30.234	-62.789	-30.339	-30.234	62.789	30.339	0.000%
7	30.234	-47.091	-30.339	-30.234	47.091	30.339	0.000%
8	37.029	-62.789	-21.453	-37.029	62.789	21.453	0.000%
9	37.029	-47.091	-21.453	-37.029	47.091	21.453	0.000%
10	42.758	-62.789	0.000	-42.758	62.789	0.000	0.000%
11	42.758	-47.091	0.000	-42.758	47.091	0.000	0.000%
12	37.029	-62.789	21.453	-37.029	62.789	-21.453	0.000%
13	37.029	-47.091	21.453	-37.029	47.091	-21.453	0.000%
14	30.234	-62.789	30.339	-30.234	62.789	-30.339	0.000%
15	30.234	-47.091	30.339	-30.234	47.091	-30.339	0.000%
16	21.379	-62.789	37.157	-21.379	62.789	-37.157	0.000%
17	21.379	-47.091	37.157	-21.379	47.091	-37.157	0.000%
18	0.000	-62.789	42.906	0.000	62.789	-42.906	0.000%
19	0.000	-47.091	42.906	0.000	47.091	-42.906	0.000%
20	-21.379	-62.789	37.157	21.379	62.789	-37.157	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
21	-21.379	-47.091	37.157	21.379	47.091	-37.157	0.000%
22	-30.234	-62.789	30.339	30.234	62.789	-30.339	0.000%
23	-30.234	-47.091	30.339	30.234	47.091	-30.339	0.000%
24	-37.029	-62.789	21.453	37.029	62.789	-21.453	0.000%
25	-37.029	-47.091	21.453	37.029	47.091	-21.453	0.000%
26	-42.758	-62.789	0.000	42.758	62.789	0.000	0.000%
27	-42.758	-47.091	0.000	42.758	47.091	0.000	0.000%
28	-37.029	-62.789	-21.453	37.029	62.789	21.453	0.000%
29	-37.029	-47.091	-21.453	37.029	47.091	21.453	0.000%
30	-30.234	-62.789	-30.339	30.234	62.789	30.339	0.000%
31	-30.234	-47.091	-30.339	30.234	47.091	30.339	0.000%
32	-21.379	-62.789	-37.157	21.379	62.789	37.157	0.000%
33	-21.379	-47.091	-37.157	21.379	47.091	37.157	0.000%
34	0.000	-88.549	0.000	0.000	88.549	0.000	0.000%
35	0.000	-88.549	-9.793	0.000	88.549	9.793	0.000%
36	4.885	-88.549	-8.481	-4.885	88.549	8.481	0.000%
37	6.908	-88.549	-6.925	-6.908	88.549	6.925	0.000%
38	8.461	-88.549	-4.896	-8.461	88.549	4.896	0.000%
39	9.770	-88.549	0.000	-9.770	88.549	-0.000	0.000%
40	8.461	-88.549	4.896	-8.461	88.549	-4.896	0.000%
41	6.908	-88.549	6.925	-6.908	88.549	-6.925	0.000%
42	4.885	-88.549	8.481	-4.885	88.549	-8.481	0.000%
43	0.000	-88.549	9.793	0.000	88.549	-9.793	0.000%
44	-4.885	-88.549	8.481	4.885	88.549	-8.481	0.000%
45	-6.908	-88.549	6.925	6.908	88.549	-6.925	0.000%
46	-8.461	-88.549	4.896	8.461	88.549	-4.896	0.000%
47	-9.770	-88.549	0.000	9.770	88.549	-0.000	0.000%
48	-8.461	-88.549	-4.896	8.461	88.549	4.896	0.000%
49	-6.908	-88.549	-6.925	6.908	88.549	6.925	0.000%
50	-4.885	-88.549	-8.481	4.885	88.549	8.481	0.000%
51	0.000	-52.324	-7.835	0.000	52.324	7.835	0.000%
52	3.904	-52.324	-6.785	-3.904	52.324	6.785	0.000%
53	5.521	-52.324	-5.540	-5.521	52.324	5.540	0.000%
54	6.762	-52.324	-3.917	-6.762	52.324	3.917	0.000%
55	7.808	-52.324	0.000	-7.808	52.324	0.000	0.000%
56	6.762	-52.324	3.917	-6.762	52.324	-3.917	0.000%
57	5.521	-52.324	5.540	-5.521	52.324	-5.540	0.000%
58	3.904	-52.324	6.785	-3.904	52.324	-6.785	0.000%
59	0.000	-52.324	7.835	0.000	52.324	-7.835	0.000%
60	-3.904	-52.324	6.785	3.904	52.324	-6.785	0.000%
61	-5.521	-52.324	5.540	5.521	52.324	-5.540	0.000%
62	-6.762	-52.324	3.917	6.762	52.324	-3.917	0.000%
63	-7.808	-52.324	0.000	7.808	52.324	0.000	0.000%
64	-6.762	-52.324	-3.917	6.762	52.324	3.917	0.000%
65	-5.521	-52.324	-5.540	5.521	52.324	5.540	0.000%
66	-3.904	-52.324	-6.785	3.904	52.324	6.785	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00000467
3	Yes	4	0.00000001	0.00000001
4	Yes	4	0.00000001	0.00008606
5	Yes	4	0.00000001	0.00005341

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6	Yes	4	0.00000001	0.00009521
7	Yes	4	0.00000001	0.00005887
8	Yes	4	0.00000001	0.00007785
9	Yes	4	0.00000001	0.00004815
10	Yes	4	0.00000001	0.00001378
11	Yes	4	0.00000001	0.00000852
12	Yes	4	0.00000001	0.00008858
13	Yes	4	0.00000001	0.00005504
14	Yes	4	0.00000001	0.00009518
15	Yes	4	0.00000001	0.00005884
16	Yes	4	0.00000001	0.00007983
17	Yes	4	0.00000001	0.00004938
18	Yes	4	0.00000001	0.00000467
19	Yes	4	0.00000001	0.00000001
20	Yes	4	0.00000001	0.00007983
21	Yes	4	0.00000001	0.00004938
22	Yes	4	0.00000001	0.00009518
23	Yes	4	0.00000001	0.00005884
24	Yes	4	0.00000001	0.00008858
25	Yes	4	0.00000001	0.00005504
26	Yes	4	0.00000001	0.00001378
27	Yes	4	0.00000001	0.00000852
28	Yes	4	0.00000001	0.00007785
29	Yes	4	0.00000001	0.00004815
30	Yes	4	0.00000001	0.00009521
31	Yes	4	0.00000001	0.00005887
32	Yes	4	0.00000001	0.00008606
33	Yes	4	0.00000001	0.00005341
34	Yes	4	0.00000001	0.00000001
35	Yes	4	0.00000001	0.00011907
36	Yes	4	0.00000001	0.00012074
37	Yes	4	0.00000001	0.00012126
38	Yes	4	0.00000001	0.00012068
39	Yes	4	0.00000001	0.00011920
40	Yes	4	0.00000001	0.00012141
41	Yes	4	0.00000001	0.00012221
42	Yes	4	0.00000001	0.00012184
43	Yes	4	0.00000001	0.00012034
44	Yes	4	0.00000001	0.00012184
45	Yes	4	0.00000001	0.00012221
46	Yes	4	0.00000001	0.00012141
47	Yes	4	0.00000001	0.00011920
48	Yes	4	0.00000001	0.00012068
49	Yes	4	0.00000001	0.00012126
50	Yes	4	0.00000001	0.00012074
51	Yes	4	0.00000001	0.00000001
52	Yes	4	0.00000001	0.00000001
53	Yes	4	0.00000001	0.00000001
54	Yes	4	0.00000001	0.00000001
55	Yes	4	0.00000001	0.00000001
56	Yes	4	0.00000001	0.00000001
57	Yes	4	0.00000001	0.00000001
58	Yes	4	0.00000001	0.00000001
59	Yes	4	0.00000001	0.00000001
60	Yes	4	0.00000001	0.00000001
61	Yes	4	0.00000001	0.00000001
62	Yes	4	0.00000001	0.00000001
63	Yes	4	0.00000001	0.00000001
64	Yes	4	0.00000001	0.00000001
65	Yes	4	0.00000001	0.00000001
66	Yes	4	0.00000001	0.00000001

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Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	130 - 83.71	4.918	59	0.343	0.001
L2	89.21 - 41.21	2.255	59	0.243	0.000
L3	48.21 - 0	0.648	59	0.123	0.000

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
130.000	BOA4357	59	4.918	0.343	0.001	124606
126.000	(3) NHH-65B-R2B	59	4.635	0.333	0.001	124606
115.000	EPBQ-654L8H8-L2	59	3.869	0.308	0.001	41535
104.000	AIR32	59	3.138	0.282	0.001	23963

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	130 - 83.71	26.954	18	1.876	0.007
L2	89.21 - 41.21	12.363	18	1.335	0.002
L3	48.21 - 0	3.554	18	0.674	0.001

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
130.000	BOA4357	18	26.954	1.876	0.007	22818
126.000	(3) NHH-65B-R2B	18	25.407	1.826	0.006	22818
115.000	EPBQ-654L8H8-L2	18	21.209	1.688	0.005	7605
104.000	AIR32	18	17.203	1.545	0.003	4387

Compression Checks

Pole Design Data

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio $\frac{P_u}{\phi P_n}$
L1	130 - 83.71 (1)	TP41.53x28x0.313	46.290	0.000	0.0	39.288	-15.133	2723.550	0.006
L2	83.71 - 41.21 (2)	TP53.78x39.297x0.625	48.000	0.000	0.0	101.256	-31.904	7522.850	0.004
L3	41.21 - 0 (3)	TP65x50.418x0.75	48.210	0.000	0.0	152.947	-62.777	11363.200	0.006

Pole Bending Design Data

Section No.	Elevation ft	Size	M _{ux} kip-ft	φM _{ux} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	M _{uy} kip-ft	φM _{uy} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
L1	130 - 83.71 (1)	TP41.53x28x0.313	745.962	2218.300	0.336	0.000	2218.300	0.000
L2	83.71 - 41.21 (2)	TP53.78x39.297x0.625	2054.600	7861.858	0.261	0.000	7861.858	0.000
L3	41.21 - 0 (3)	TP65x50.418x0.75	3941.433	14956.333	0.264	0.000	14956.333	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V _u K	φV _n K	Ratio $\frac{V_u}{\phi V_n}$	Actual T _u kip-ft	φT _n kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L1	130 - 83.71 (1)	TP41.53x28x0.313	28.732	1361.780	0.021	0.000	4447.308	0.000
L2	83.71 - 41.21 (2)	TP53.78x39.297x0.625	35.210	3761.420	0.009	0.000	15771.916	0.000
L3	41.21 - 0 (3)	TP65x50.418x0.75	42.923	5681.600	0.008	0.000	30001.917	0.000

Pole Interaction Design Data

Section No.	Elevation ft	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	Ratio $\frac{M_{uy}}{\phi M_{uy}}$	Ratio $\frac{V_u}{\phi V_n}$	Ratio $\frac{T_u}{\phi T_n}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	130 - 83.71 (1)	0.006	0.336	0.000	0.021	0.000	0.342	1.000	4.8.2 ✓
L2	83.71 - 41.21 (2)	0.004	0.261	0.000	0.009	0.000	0.266	1.000	4.8.2 ✓
L3	41.21 - 0 (3)	0.006	0.264	0.000	0.008	0.000	0.269	1.000	4.8.2 ✓

Section Capacity Table

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Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail	
L1	130 - 83.71	Pole	TP41.53x28x0.313	1	-15.133	2723.550	34.2	Pass	
L2	83.71 - 41.21	Pole	TP53.78x39.297x0.625	2	-31.904	7522.850	26.6	Pass	
L3	41.21 - 0	Pole	TP65x50.418x0.75	3	-62.777	11363.200	26.9	Pass	
							Summary		
							Pole (L1)	34.2	Pass
							RATING =	34.2	Pass

Program Version 8.0.5.0 - 11/28/2018 File:J:\Jobs\1902700.WI\46_CTHA524A\05_Structural\Tower\Backup Documentation\Rev (2)\Calcs\ERI Files\130' Monopole_ Westbrook_CT.eri

Anchor Bolt and Base Plate Analysis:

Input Data:

Tower Reactions:

Overturing Moment =	$M_U := 3941 \cdot \text{ft-kips}$	(Input From trnTower)
Shear Force =	Shear := 43-kips	(Input From trnTower)
Axial Force =	$R_U := 63 \cdot \text{kips}$	(Input From trnTower)

Anchor Bolt Data:

ASTMA615 Grade 75		
Number of Anchor Bolts =	$N := 22$	(User Input)
Diameter of Bolt Circle =	$D_{BC} := 71.5 \cdot \text{in}$	(User Input)
Bolt "Column" Distance =	$l := 3.0 \cdot \text{in}$	(User Input)
Bolt Ultimate Strength =	$F_U := 100 \cdot \text{ksi}$	(User Input)
Bolt Yield Strength =	$F_y := 75 \cdot \text{ksi}$	(User Input)
Bolt Modulus =	$E := 29000 \cdot \text{ksi}$	(User Input)
Diameter of Anchor Bolts =	$D := 2.25 \cdot \text{in}$	(User Input)
Threads per Inch =	$n := 4.5$	(User Input)
Top of Concrete to Bot Leveling Nut =	$l_{ar} := 2 \cdot \text{in}$	(User Input)
Anchor Rod Force Correction Factor =	$n_c = 1$	Table 2-1 Addendum 3

Base Plate Data:

UseASTMA572 Grade 50		
Plate Yield Strength =	$F_{yf} := 50 \cdot \text{ksi}$	(User Input)
Base Plate Thickness =	$t_{TP} := 2.5 \cdot \text{in}$	(User Input)
Base Plate Diameter =	$D_{OD} := 76.5 \cdot \text{in}$	(User Input)
Outer Pole Diameter =	$D_T := 65 \cdot \text{in}$	(User Input)
Pole Wall Thickness =	$t_T := 0.75 \cdot \text{in}$	(User Input)
Pole Design Yield Strength =	$F_{yp} := 65 \cdot \text{ksi}$	(User Input)
	$\eta := 0.5$	For Ungrouted Base Plate per TIA-222-G Section 4.9.9

Anchor Bolt Analysis:

GrossArea of Bolt = $A_g := \frac{\pi}{4} \cdot D^2 = 3.976 \cdot \text{in}^2$

NetArea of Bolt = $A_n := \frac{\pi}{4} \cdot \left(D - \frac{0.9743 \cdot \text{in}}{n} \right)^2 = 3.248 \cdot \text{in}^2$

Tensile Root Diameter = $d_{rt} := D - \frac{0.9743 \cdot \text{in}}{n} = 2.033 \cdot \text{in}$

Plastic Section Modulus = $Z := \frac{d_{rt}^3}{6} = 1.401 \cdot \text{in}^3$

Maximum Anchor Rod Force = $P_u := \frac{n_c \cdot \pi \cdot M_u}{N \cdot D_{BC}} + \frac{R_u}{N} = 97.3 \cdot \text{kips}$

Maximum Shear Force = $V_u := \frac{\text{Shear}}{N} = 2 \cdot \text{kips}$

Design Tensile Strength = $\Phi R_{nt} := 0.8 \cdot F_u \cdot A_n = 259.815 \cdot \text{k}$

Bolt % of Capacity = $\frac{\left(P_u + \frac{V_u}{\eta} \right)}{\Phi R_{nt}} \cdot 100 = 39$

Condition1 = $\text{Condition1} := \text{if} \left[\frac{\left(P_u + \frac{V_u}{\eta} \right)}{\Phi R_{nt}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right]$

Condition1 = "OK"

Design Shear Strength = $\Phi R_{nv} := 0.75 \cdot 0.45 \cdot F_u \cdot A_g = 134.193 \cdot \text{k}$

Design Flexural Strength = $\Phi R_{nm} := 0.9 \cdot F_y \cdot Z = 94.597 \cdot \text{in} \cdot \text{k}$

$M_u := \begin{cases} 0 & \text{if } l_{ar} < D \\ 0.65 \cdot l_{ar} \cdot V_u & \text{otherwise} \end{cases} = 0 \cdot \text{in} \cdot \text{k}$

Bolt % of Capacity = $\left[\left(\frac{V_u}{\Phi R_{nv}} \right)^2 + \left(\frac{P_u}{\Phi R_{nt}} + \frac{M_u}{\Phi R_{nm}} \right)^2 \right] \cdot 100 = 14.1$

Condition2 = $\text{Condition2} := \text{if} \left[\left(\frac{V_u}{\Phi R_{nv}} \right)^2 + \left(\frac{P_u}{\Phi R_{nt}} + \frac{M_u}{\Phi R_{nm}} \right)^2 \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right]$

Condition2 = "OK"

Base Plate Analysis:

Strength Resistance Factor for Yielding due to Bending =

$$\phi_b := 0.9$$

Strength Resistance Factor for Yielding due to Shear =

$$\phi_v := 1.0$$

Outside Fillet Horizontal Leg Dimension =

$$w_1 := 0.25 \text{ in}$$

Effective Pole Outside Diameter =

$$D_e := D_T + w_1 = 65.25 \text{ in}$$

Effective Base Plate Outside Diameter =

$$D_{oe} := \begin{cases} D_{OD} & \text{if } D_{OD} \leq (D_{BC} + 6 \cdot t_{TP}) \\ (D_{BC} + 6 \cdot t_{TP}) & \text{otherwise} \end{cases} = 76.5 \text{ in}$$

Half-Angle Between Radial Lines Extending from Pole
 Centerline Through Midpoints Between Adjacent Anchor

$$\theta_1 := \frac{\pi}{N} = 0.143$$

Rods =

Angle Defining Limiting Effective Base Plate Width
 Based on Plate Thickness =

$$\theta_2 := \text{asin}\left(\frac{12 \cdot t_{TP}}{D_{BC}}\right) = 0.433$$

Angle Defining Limiting Effective Base Plate Width
 Based on Distance Between Anchor Rod Bolt Circle and
 Effective Pole Outside Diameter =

$$\theta_3 := \text{acos}\left(\frac{D_{BC} + D_e}{2 \cdot D_{BC}}\right) = 0.297$$

Governing Angle Defining Effective Base Plate Width
 Resisting Bending =

$$\theta := \min(\theta_1, \theta_2, \theta_3) = 0.143$$

Effective Moment Arm of Anchor Rod Force =

$$x := 0.5 \cdot (D_{BC} - D_e) = 3.125 \text{ in}$$

Effective Base Plate Width Resisting Bending from
 Transverse Bend Line =

$$B_{et} := D_{BC} \cdot \sin(\theta) = 10.176 \text{ in}$$

Effective Base Plate Width Resisting Bending from
 Radial Bend Lines =

$$B_{er} := (D_{oe} - D_e) \cdot \sin(\theta) = 1.601 \text{ in}$$

Total Effective Base Plate Width Resisting Bending =

$$B_{eff} := B_{et} + B_{er} = 11.777 \text{ in}$$

Required Base Plate Thickness =

$$t_{TP,Req} := \sqrt{\frac{4 \cdot P_u \cdot x}{\phi_b \cdot F_{yf} \cdot B_{eff}}} = 1.515 \text{ in}$$

Plate Bending Stress % of Capacity =

$$\frac{t_{TP,Req}}{t_{TP}} = 60.6\%$$

Condition2 =

$$\text{Condition3} := \text{if}\left(\frac{t_{TP,Req}}{t_{TP}} < 1.00, \text{"Ok"}, \text{"Overstressed"}\right)$$

Condition3 = "Ok"

Required Base Plate Thickness =

$$t_{TP,Req} := \frac{\phi_b \cdot t_T \cdot F_{yp}}{\phi_v \cdot 0.6 \cdot F_{yf}} = 1.463 \text{ in}$$

Plate Bending Stress % of Capacity =

$$\frac{t_{TP,Req}}{t_{TP}} = 58.5\%$$

Condition2 =

$$\text{Condition4} := \text{if}\left(\frac{t_{TP,Req}}{t_{TP}} < 1.00, \text{"Ok"}, \text{"Overstressed"}\right)$$

Condition4 = "Ok"

Standard Monopole Foundation:

Input Data:

Tower Data

Overturing Moment = OM := 3941 ·ft-kips (User Input)
 Shear Force = Shear := 43-kip (User Input)
 Axial Force = Axial := 53-kip (User Input)
 Tower Height = H_t := 130-ft (User Input)

Footing Data:

Overall Depth of Footing = D_f := 6-ft (User Input)
 Length of Pier = L_p := 4.5-ft (User Input)
 Extension of Pier Above Grade = L_{pag} := 0.5-ft (User Input)
 Diameter of Pier = d_p := 8.25-ft (User Input)
 Thickness of Footing = T_f := 2-ft (User Input)
 Width of Footing = W_f := 32-ft (User Input)

Anchor Bolt Data:

Length of Anchor Bolts = L_{st} := 84-in (User Input)
 Projection of Anchor Bolts Above Pier = A_{BP} := 12-in (User Input)
 Anchor Bolt Diameter = d_{anchor} := 2.25-in (User Input)
 Base Plate Bolt Circle = MP := 71.5-in (User Input)

Material Properties:

Concrete Compressive Strength = f_c := 4500-psi (User Input)
 Steel Reinforcement Yield Strength = f_y := 60000-psi (User Input)
 Anchor Bolt Yield Strength = f_{ya} := 75000-psi (User Input)
 Internal Friction Angle of Soil = Φ_s := 30-deg (User Input)
 Ultimate Soil Bearing Capacity = q_u := 12000-psf (User Input)
 Allowable Soil Bearing Capacity = q_a := $\frac{q_u}{2}$ = 6000-psf (User Input)
 Unit Weight of Soil = γ_{soil} := 110-pcf (User Input)
 Unit Weight of Concrete = γ_{conc} := 150-pcf (User Input)
 Foundation Bouyancy = Bouyancy := 0 (User Input) (Yes=1 / No=0)
 Depth to Neglect = n := 0-ft (User Input)
 Cohesion of Clay Type Soil = c := 0-ksf (User Input) (Use 0 for Sandy Soil)
 Seismic Zone Factor = Z := 2 (User Input) (UBC-1997 Fig 23-2)
 Coefficient of Friction Between Concrete = μ := 0.45 (User Input)

Pier Reinforcement:

Bar Size =	$BS_{\text{pier}} := 11$	(User Input)	
Bar Diameter =	$d_{\text{bpier}} := 1.41 \cdot \text{in}$	(User Input)	
Number of Bars =	$NB_{\text{pier}} := 38$	(User Input)	
Clear Cover of Reinforcement =	$Cvr_{\text{pier}} := 3 \cdot \text{in}$	(User Input)	
Reinforcement Location Factor =	$\alpha_{\text{pier}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Coating Factor =	$\beta_{\text{pier}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Concrete Strength Factor =	$\lambda_{\text{pier}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Reinforcement Size Factor =	$\gamma_{\text{pier}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Diameter of Tie =	$d_{\text{Tie}} := 3 \cdot \text{in}$	(User Input)	

Pad Reinforcement:

Bar Size =	$BS_{\text{top}} := 10$	(User Input)	(Top of Pad)
Bar Diameter =	$d_{\text{btop}} := 1.27 \cdot \text{in}$	(User Input)	(Top of Pad)
Number of Bars =	$NB_{\text{top}} := 62$	(User Input)	(Top of Pad)
Bar Size =	$BS_{\text{bot}} := 10$	(User Input)	(Bottom of Pad)
Bar Diameter =	$d_{\text{bbot}} := 1.27 \cdot \text{in}$	(User Input)	(Bottom of Pad)
Number of Bars =	$NB_{\text{bot}} := 62$	(User Input)	(Bottom of Pad)
Clear Cover of Reinforcement =	$Cvr_{\text{pad}} := 3.0 \cdot \text{in}$	(User Input)	
Reinforcement Location Factor =	$\alpha_{\text{pad}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Coating Factor =	$\beta_{\text{pad}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Concrete Strength Factor =	$\lambda_{\text{pad}} := 1.0$	(User Input)	(ACI-2008 12.2.4)
Reinforcement Size Factor =	$\gamma_{\text{pad}} := 1.0$	(User Input)	(ACI-2008 12.2.4)

Calculated Factors:

Pier Reinforcement Bar Area =	$A_{\text{bpier}} := \frac{\pi \cdot d_{\text{bpier}}^2}{4} = 1.561 \cdot \text{in}^2$
Pad Top Reinforcement Bar Area =	$A_{\text{btop}} := \frac{\pi \cdot d_{\text{btop}}^2}{4} = 1.267 \cdot \text{in}^2$
Pad Bottom Reinforcement Bar Area =	$A_{\text{bbot}} := \frac{\pi \cdot d_{\text{bbot}}^2}{4} = 1.267 \cdot \text{in}^2$
Coefficient of Lateral Soil Pressure =	$K_p := \frac{1 + \sin(\Phi_s)}{1 - \sin(\Phi_s)} = 3$

Stability of Footing:

Adjusted Concrete Unit Weight =

$$\gamma_c := \text{if}(\text{Bouyancy} = 1, \gamma_{\text{conc}} - 62.4\text{pcf}, \gamma_{\text{conc}}) = 150\text{-pcf}$$

Adjusted Soil Unit Weight =

$$\gamma_s := \text{if}(\text{Bouyancy} = 1, \gamma_{\text{soil}} - 62.4\text{pcf}, \gamma_{\text{soil}}) = 110\text{-pcf}$$

Passive Pressure =

$$P_{pn} := K_p \cdot \gamma_s \cdot n + c \cdot 2 \cdot \sqrt{K_p} = 0\text{-ksf}$$

$$P_{pt} := K_p \cdot \gamma_s \cdot (D_f - T_f) + c \cdot 2 \cdot \sqrt{K_p} = 1.32\text{-ksf}$$

$$P_{top} := \text{if}[n < (D_f - T_f), P_{pt}, P_{pn}] = 1.32\text{-ksf}$$

$$P_{bot} := K_p \cdot \gamma_s \cdot D_f + c \cdot 2 \cdot \sqrt{K_p} = 1.98\text{-ksf}$$

$$P_{ave} := \frac{P_{top} + P_{bot}}{2} = 1.65\text{-ksf}$$

$$T_p := \text{if}[n < (D_f - T_f), T_f, (D_f - n)] = 2$$

$$A_p := W_f \cdot T_p = 64$$

Ultimate Shear =

$$S_u := P_{ave} \cdot A_p = 105.6\text{-kip}$$

Weight of Concrete Pad =

$$WT_c := \left[(W_f^2 \cdot T_f) + d_p^2 \cdot L_p \right] \cdot \gamma_c = 353.142\text{-kip}$$

Weight of Soil Above Footing =

$$WT_{s1} := \left[(W_f^2 - d_p^2) \cdot (L_p - L_{pag} - n) \right] \cdot \gamma_s = 420.61\text{-kip}$$

Weight of Soil Wedge at Back Face =

$$WT_{s2} := \left(\frac{D_f^2 \cdot \tan(\phi_s)}{2} \cdot W_f \right) \cdot \gamma_s = 36.581\text{-kip}$$

Weight of Soil Wedge at back face Corners =

$$WT_{s3} := 2 \cdot \left[(D_f)^3 \cdot \frac{\tan(\phi_s)}{3} \right] \cdot \gamma_s = 9.145\text{-kips}$$

Total Weight =

$$WT_{tot} := WT_c + WT_{s1} + \text{Axial} = 826.755\text{-kip}$$

Resisting Weight =

$$WT_R := 0.9 \cdot WT_c + 0.75 \cdot WT_{s1} + 0.75 \cdot \text{Axial} = 673.037\text{-kip}$$

Resisting Moment =

$$M_r := (WT_R) \cdot \frac{W_f}{2} + 0.75 \cdot S_u \cdot \frac{T_f}{3} + 0.75 \cdot \left[(WT_{s2} + WT_{s3}) \cdot \left(W_f + \frac{D_f \cdot \tan(\phi_s)}{3} \right) \right] = 11958\text{-kip-ft}$$

Overtuning Moment =

$$M_{ot} := \text{OM} + \text{Shear} \cdot (L_p + T_f) = 4221\text{-kip-ft}$$

Factor of Safety Actual =

$$FS := \frac{M_r}{M_{ot}} = 2.83$$

Factor of Safety Required =

$$FS_{req} := 1$$

$$\text{OverTurning_Moment_Check} := \text{if}(FS \geq FS_{req}, \text{"Okay"}, \text{"No Good"})$$

$$\text{OverTurning_Moment_Check} = \text{"Okay"}$$

Shear Capacity in Pier:

Shear Resistance of Pier =
$$S_p := \frac{P_{ave} \cdot A_p + \mu \cdot W_{T_{tot}}}{FS_{req}} = 477.64 \text{ kips}$$

Shear_Check := if(S_p > Shear, "Okay", "No Good")

Shear_Check = "Okay"

Bearing Pressure Caused by Footing:

Area of the Mat =
$$A_{mat} := W_f^2 = 1.024 \times 10^3$$

Section Modulus of Mat =
$$S := \frac{W_f^3}{6} = 5461.33 \text{ ft}^3$$

Maximum Pressure in Mat =
$$P_{max} := \frac{W_{T_{tot}}}{A_{mat}} + \frac{M_{ot}}{S} = 1.58 \text{ ksf}$$

Max_Pressure_Check := if(P_{max} < .75·q_u, "Okay", "No Good")

Max_Pressure_Check = "Okay"

Minimum Pressure in Mat =
$$P_{min} := \frac{W_{T_{tot}}}{A_{mat}} - \frac{M_{ot}}{S} = 0.035 \text{ ksf}$$

Min_Pressure_Check := if((P_{min} ≥ 0) · (P_{min} < .75·q_u), "Okay", "No Good")

Min_Pressure_Check = "Okay"

Distance to Resultant of Pressure Distribution =
$$X_p := \frac{P_{max}}{P_{max} - P_{min}} \cdot \frac{1}{3} = 10.905$$

Distance to Kern =
$$X_k := \frac{W_f}{6} = 5.333$$

Since Resultant Force is Not in Kern, Area to which Pressure is Applied Must be Reduced.

Eccentricity =
$$e := \frac{M_{ot}}{W_{T_{tot}}} = 3.191$$

Adjusted Soil Pressure =
$$P_a := \frac{2 \cdot W_{T_{tot}}}{3 \cdot W_f \left(\frac{W_f}{2} - e \right)} = 1.345 \text{ ksf}$$

q_{adj} := if(P_{min} < 0, P_a · P_{max}) = 1.58·ksf

Pressure_Check := if(q_{adj} < q_a, "Okay", "No Good")

Pressure_Check = "Okay"

Concrete Bearing Capacity:

Strength Reduction Factor = $\Phi_c := 0.65$ (ACI-2008 9.3.2.2)

Bearing Strength Between Pier and Pad = $P_b := \Phi_c \cdot 0.85 \cdot f_c \cdot \frac{\pi \cdot d_p^2}{4} = 1.914 \times 10^4 \text{ kips}$ (ACI-2008 10.14)

Bearing_Check := if($P_b > \text{Axial}$, "Okay", "No Good")

Bearing_Check = "Okay"

Shear Strength of Concrete:

Beam Shear: (Critical section located at a distance d from the face of Pier) (ACI 11.3.1.1)

$\Phi_c := 0.85$ (ACI 9.3.2.5)

$d := T_f - C_{vr_pad} - d_{bbot} = 1.644$

$d_1 := \frac{W_f}{2} - \frac{d_p}{2}$

$d_2 := d_1 - d$

$L := \left(\frac{W_f}{2} - e \right) \cdot 3$

Slope := if($L > W_f$, $\frac{P_{max} - P_{min}}{W_f}$, $\frac{q_{adj}}{L}$)

$V_{req} := \left[(q_{adj} - \text{Slope} \cdot d_1) + \left(\frac{\text{Slope} \cdot d_1}{2} \right) \right] \cdot W_f \cdot d_1$

$V_{Avail} := \Phi_c \cdot 2 \cdot \sqrt{f_c \cdot \psi} \cdot W_f \cdot d$ (ACI-2008 11.2.1.1)

Beam_Shear_Check := if($V_{req} < V_{Avail}$, "Okay", "No Good")

Beam_Shear_Check = "Okay"

Punching Shear: (Critical Section Located at a distance of d/2 from the face of pier) (ACI 11.11.1.2)

Critical Perimeter of Punching Shear = $b_o := (d_p + d) \cdot \pi = 31.1$

Area Included Inside Perimeter = $A_{bo} := \frac{\pi \cdot (d_p + d)^2}{4} = 76.9$

Area Outside of Perimeter = $A_{out} := A_{mat} - A_{bo} = 947.1$

Guess Value =

$$v_u := 1 \text{ksf}$$

(From "Foundation Analysis and design", By Joseph Bowles, Eq. 8-9)

Given

$$d^2 + d_p \cdot d = \frac{W_{T_{tot}}}{\pi \cdot v_u}$$

$$v_u := \text{Find}(v_u) = 16.2 \cdot \text{ksf}$$

$$V_u := v_u \cdot d \cdot W_f = 851.1 \cdot \text{kips}$$

Required Shear Strength =

$$V_{req} := V_u = 851.1 \cdot \text{kips}$$

Available Shear Strength =

$$V_{Avail} := \phi_c \cdot 4 \cdot \sqrt{f_c} \cdot \text{psi} \cdot b_o \cdot d = 1678.5 \cdot \text{kip} \quad (\text{ACI-2008 11.11.2.1})$$

$$\text{Punching_Shear_Check} := \text{if}(V_{req} < V_{Avail}, \text{"Okay"}, \text{"No Good"})$$

$$\text{Punching_Shear_Check} = \text{"Okay"}$$

Steel Reinforcement in Pad:

Required Reinforcement for Bending:

Strength Reduction Factor =

$$\phi_m := .90 \quad (\text{ACI-2008 9.3.2.1})$$

$$q_b := q_{adj} - d_1 \cdot \text{Slope} = 1.007 \cdot \text{ksf}$$

Maximum Bending at Face of Pier =

$$M_n := \frac{1}{\phi_m} \cdot \left[(q_{adj} - q_b) \cdot \frac{d_1^2}{3} + q_b \cdot \frac{d_1^2}{2} \right] \cdot W_f = 3482.1 \cdot \text{kip-ft}$$

$$\beta := \begin{cases} 0.85 & \text{if } 2500 \cdot \text{psi} \leq f_c \leq 4000 \cdot \text{psi} \\ 0.65 & \text{if } f_c > 8000 \cdot \text{psi} \end{cases} = 0.6$$

$$\left[\left[\left[\left[\frac{f_c}{\text{psi}} - 4000 \right] \right] \right] \cdot 0.5 \right] \text{ otherwise} \quad (\text{ACI-2008 10.2.7.3})$$

$$R_n := \frac{M_n}{W_f \cdot d^2} = 279.5 \cdot \text{psi}$$

$$\rho := \frac{0.85 \cdot f_c}{f_y} \left(1 - \sqrt{1 - \frac{2 \cdot R_n}{0.85 \cdot f_c}} \right) = 0.0048$$

$$\rho_{min} := \rho = 0.00484$$

Required Reinforcement for Temperature and Shrinkage:

$$\rho_{sh} := \begin{cases} .0018 & \text{if } f_y \geq 60000\text{-psi} \\ .0020 & \text{otherwise} \end{cases} \quad (\text{ACI-2008 7.12.2.1})$$

Check Bottom Bars:

$$A_s := \begin{cases} \rho_{min} \cdot W_f \cdot d & \text{if } \rho_{min} > \frac{\rho_{sh}}{2} \\ \rho_{sh} \cdot W_f \cdot \frac{d}{2} & \text{otherwise} \end{cases} = 36.691 \cdot \text{in}^2$$

$$A_{s\text{prov.bot}} := A_{\text{bbot}} \cdot NB_{\text{bot}} = 78.5 \cdot \text{in}^2$$

$$\text{Pad_Reinforcement_Bot} := \text{if}(A_{s\text{prov.bot}} > A_s, \text{"Okay"}, \text{"No Good"})$$

Pad_Reinforcement_Bot = "Okay"

Check Temp Shrinkage Reinforcement:

$$A_s := \rho_{sh} \cdot (W_f \cdot T_f) = 16.6 \cdot \text{in}^2$$

$$A_{s\text{prov.top}} := A_{\text{btop}} \cdot NB_{\text{top}} = 78.5 \cdot \text{in}^2$$

$$A_{s\text{prov.tot}} := A_{s\text{prov.bot}} + A_{s\text{prov.top}} = 157.1 \cdot \text{in}^2$$

$$\text{Pad_Reinforcement_Temp} := \text{if}(A_{s\text{prov.tot}} > A_s, \text{"Okay"}, \text{"No Good"})$$

Pad_Reinforcement_Temp = "Okay"

Development Length Pad Reinforcement:

Bar Spacing =

$$B_{s\text{Pad}} := \frac{W_f - 2 \cdot C_{vr\text{pad}} - NB_{\text{bot}} \cdot d_{\text{bbot}}}{NB_{\text{bot}} - 1} = 4.91 \cdot \text{in}$$

Spacing or Cover Dimension =

$$c := \text{if}\left(C_{vr\text{pad}} < \frac{B_{s\text{Pad}}}{2}, C_{vr\text{pad}}, \frac{B_{s\text{Pad}}}{2}\right) = 2.453 \cdot \text{in}$$

Transverse Reinforcement Index =

$$k_{tr} := 0 \quad (\text{ACI-2008 12.2.3})$$

$$L_{\text{dbt}} := \frac{3 \cdot f_y \cdot \alpha_{\text{pad}} \cdot \beta_{\text{pad}} \cdot \gamma_{\text{pad}} \cdot \lambda_{\text{pad}}}{40 \cdot \sqrt{f_c \cdot \text{psi}} \cdot \frac{c + k_{tr}}{d_{\text{bbot}}}} \cdot d_{\text{bbot}} = 44.1 \cdot \text{in}$$

Minimum Development Length =

$$L_{\text{dbmin}} := 12 \cdot \text{in} \quad (\text{ACI-2008 12.2.1})$$

$$L_{\text{dbtCheck}} := \text{if}(L_{\text{dbt}} \geq L_{\text{dbmin}}, \text{"Use L.dbt"}, \text{"Use L.dbmin"})$$

Available Length in Pad =

$$L_{\text{Pad}} := \frac{W_f}{2} - \frac{d_p}{2} - C_{vr\text{pad}} = 139.5 \cdot \text{in}$$

$$L_{\text{pad_Check}} := \text{if}(L_{\text{Pad}} > L_{\text{dbt}}, \text{"Okay"}, \text{"No Good"})$$

Lpad_Check = "Okay"

Steel Reinforcement in Pier:

Area of Pier =

$$A_p := d_p^2 = 9801 \cdot \text{in}^2$$

$$A_{smin} := 0.01 \cdot 0.5 \cdot A_p = 49.01 \cdot \text{in}^2 \quad (\text{ACI-2008 10.8.4 \& 10.9.1})$$

$$A_{sprov} := N_{B_{pier}} \cdot A_{B_{pier}} = 59.34 \cdot \text{in}^2$$

$$\text{Steel_Area_Check} := \text{if}(A_{sprov} > A_{smin}, \text{"Okay"}, \text{"No Good"})$$

Steel_Area_Check = "Okay"

NOTE: Anchor Bolts are not accounted for in reinforcement calculation and will provide additional reinforcement to satisfy minimum requirement of steel.

Bar Spacing In Pier =

$$B_{sPier} := \frac{d_p \cdot \pi}{N_{B_{pier}}} - d_{B_{pier}} = 6.775 \cdot \text{in}$$

Diameter of Reinforcement Cage =

$$\text{Diam}_{cage} := d_p - 2 \cdot C_{vr_{pier}} = 93 \cdot \text{in}$$

Maximum Moment in Pier =

$$M_p := \left[\text{OM} + \text{Shear} \cdot \left(L_p + \frac{A_{BP}}{2} \right) \right] = 49872 \cdot \text{in} \cdot \text{kips}$$

Pier Check evaluated from outside program and results are listed below;

$$(D \ N \ n \ P_u \ M_{xu}) := \left(d_p, 12 \ N_{B_{pier}} \ B_{s_{pier}} \ \frac{\text{Axial} \cdot 1.333}{\text{kips}} \ \frac{M_p}{\text{in} \cdot \text{kips}} \right)$$

$$(D \ N \ n \ P_u \ M_{xu}) = (99 \ 38 \ 11 \ 70.6 \ 49872)$$

$$(\phi P_n \ \phi M_{xn} \ f_{sp} \ \rho) := (0 \ 0 \ 0 \ 0)$$

$$(\phi P_n \ \phi M_{xn} \ f_{sp} \ \rho) := \phi P'_n (D, N, n, P_u, M_{xu})^T$$

$$(\phi P_n \ \phi M_{xn} \ f_{sp} \ \rho) = (190.5 \ 1.3 \times 10^5 \ -60 \ 0)$$

$$\text{Axial_Load_Check} := \text{if}(\phi P_n \geq P_u, \text{"Okay"}, \text{"No Good"})$$

Axial_Load_Check = "Okay"

$$\text{Bending_Check} := \text{if}(\phi M_{xn} \geq M_{xu}, \text{"Okay"}, \text{"No Good"})$$

Bending_Check = "Okay"

Development Length Pier Reinforcement:

Available Length in Foundation:

$$L_{\text{pier}} := L_p - C_{\text{vr}}_{\text{pier}} = 51 \cdot \text{in}$$

$$L_{\text{pad}} := T_f - C_{\text{vr}}_{\text{pad}} = 21 \cdot \text{in}$$

Tension:

(ACI-2008 12.2.3)

Spacing or Cover Dimension =

$$c := \text{if} \left(C_{\text{vr}}_{\text{pier}} < \frac{B_{\text{sPier}}}{2}, C_{\text{vr}}_{\text{pier}}, \frac{B_{\text{sPier}}}{2} \right) = 3 \cdot \text{in}$$

Transverse Reinforcement =

$$k_{\text{tr}} := 0$$

(ACI-2008 12.2.3)

$$L_{\text{dbt}} := \frac{3 \cdot f_y \cdot \alpha_{\text{pier}} \cdot \beta_{\text{pier}} \cdot \gamma_{\text{pier}} \cdot \lambda_{\text{pier}}}{40 \cdot \sqrt{f_c \cdot \text{psi}} \cdot \left(\frac{c + k_{\text{tr}}}{d_{\text{bpier}}} \right)} \cdot d_{\text{bpier}} = 44.46 \cdot \text{in}$$

Minimum Development Length =

$$L_{\text{dh}} := \frac{1200 \cdot d_{\text{bpier}}}{\sqrt{\frac{f_c}{\text{psi}}}} \cdot .7 = 17.656 \cdot \text{in} \quad (\text{ACI } 12.2.1)$$

Pier reinforcement bars are standard 90 degree hooks and therefore development in the pad is computed as follows:

$$L_{\text{db}} := \max(L_{\text{dbt}}, L_{\text{dbmin}})$$

$$L_{\text{tension_Check}} := \text{if}(L_{\text{pier}} + L_{\text{pad}} > L_{\text{db}}, \text{"Okay"}, \text{"No Good"})$$

$$L_{\text{tension_Check}} = \text{"Okay"}$$

Compression:

(ACI-2008 12.3.2)

$$L_{\text{dbc1}} := \frac{.02 \cdot d_{\text{bpier}} \cdot f_y}{\sqrt{f_c \cdot \text{psi}}} = 25.223 \cdot \text{in}$$

$$L_{\text{dbmin}} := 0.0003 \cdot \frac{\text{in}^2}{l_b} \cdot (d_{\text{bpier}} \cdot f_y) = 25.38 \cdot \text{in}$$

$$L_{\text{dbc}} := \text{if}(L_{\text{dbc1}} \geq L_{\text{dbmin}}, L_{\text{dbc1}}, L_{\text{dbmin}}) = 25.38 \cdot \text{in}$$

$$L_{\text{compression_Check}} := \text{if}(L_{\text{pier}} + L_{\text{pad}} > L_{\text{dbc}}, \text{"Okay"}, \text{"No Good"})$$

$$L_{\text{compression_Check}} = \text{"Okay"}$$

RAN Template: 67D97DB MUAC	A&L Template: 67D97DB_1xAIR+1OP
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Section 1 - Site Information

Site ID: CTHA524A
Status: Draft
Version: 3
Project Type: L600
Approved: Not Approved
Approved By: Not Approved
Last Modified: 6/12/2019 8:00 PM
Last Modified By: GSM1900DKallas2

Site Name: CT Water Westbrook Watertank
Site Class: Watertank
Site Type: Structure Non Building
Plan Year:
Market: CONNECTICUT CT
Vendor: Ericsson
Landlord: <undefined>

Latitude: 41.2818000000
Longitude: -72.4375000000
Address: 1546 Boston Post Rd
City, State: Westbrook, CT
Region: NORTHEAST

RAN Template: 67D97DB MUAC

AL Template: 67D97DB_1xAIR+1OP

Sector Count: 3

Antenna Count: 6

Coax Line Count: 0

TMA Count: 0

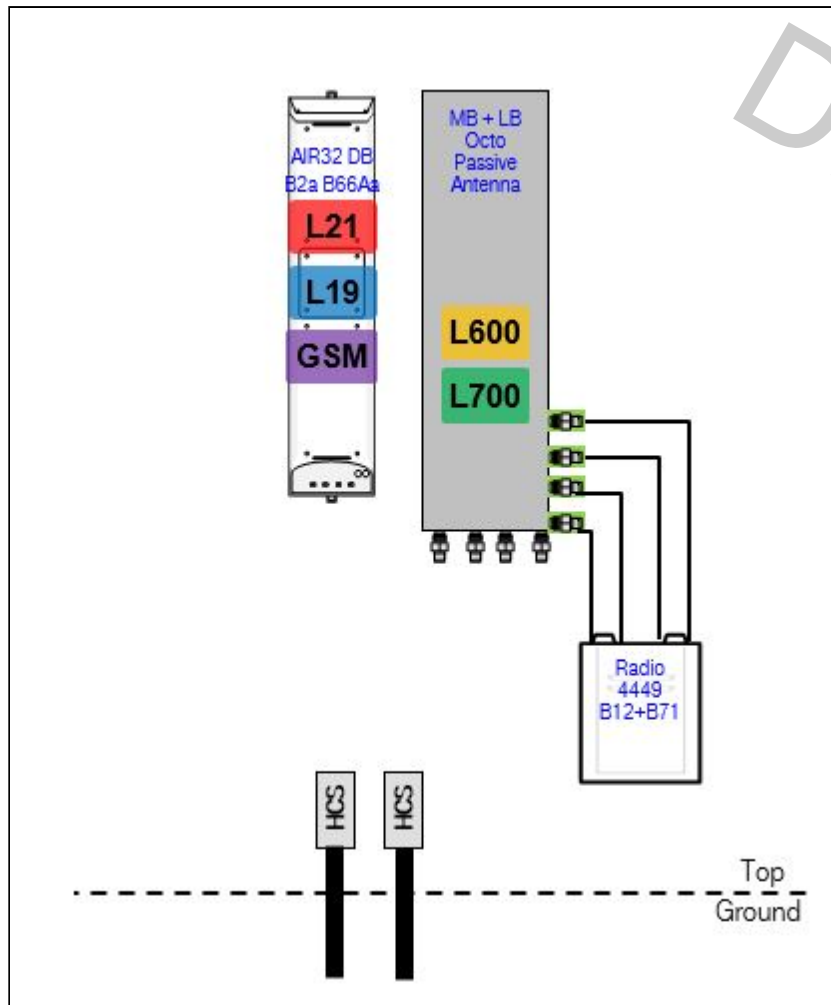
RRU Count: 3

Section 2 - Existing Template Images

----- This section is intentionally blank. -----

Section 3 - Proposed Template Images

67D97DB.JPG



Notes:

DRAFT

Section 4 - Siteplan Images

----- This section is intentionally blank. -----

DRAFT

RAN Template: 67D97DB MUAC	A&L Template: 67D97DB_1xAIR+1OP
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Section 5 - RAN Equipment

Existing RAN Equipment		
Template: 5A U19 Shutdown		
Enclosure	1	2
Enclosure Type	Legacy ODE	Ancillary Equipment (Ericsson)
Baseband	DUW30 DUS41	
Hybrid Cable System		Ericsson 9x18 HCS *Select Length* Ericsson 6x12 HCS *Select Length & AWG*

Proposed RAN Equipment	
Template: 67D97DB MUAC	
Enclosure	1
Enclosure Type	RBS 6102 MU AC
Baseband	DUG20 G1900 BB 6630 L2100 L1900 L700 L600 N600
Hybrid Cable System	Ericsson 6x12 HCS *Select Length & AWG* (x 2)

RAN Scope of Work:

This is a new tower. Must order everything new.

RAN Template: 67D97DB MUAC	A&L Template: 67D97DB_1xAIR+1OP
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Section 6 - A&L Equipment

Existing Template: 5A_2xAIR U19 Shutdown
Proposed Template: 67D97DB_1xAIR+1OP

Sector 1 (Existing) view from behind				
Coverage Type	A - Outdoor Macro			
Antenna	1		2	
Antenna Model	Ericsson - AIR21 KRC118023-1_B2P_B4A (Quad)		Ericsson - AIR21 KRC118023-1_B2P_B4A (Quad)	
Azimuth	85		85	
M. Tilt	0		0	
Height	100		100	
Ports	P1	P2	P3	P4
Active Tech.	U2100		L2100	
Dark Tech.				
Restricted Tech.				
Decomm. Tech.				
E. Tilt	2		2	
Cables	Fiber Jumper - 15 ft.		Fiber Jumper - 15 ft.	
TMA's				
Diplexers / Combiners				
Radio				
Sector Equipment				

Unconnected Equipment:

Cable: Fiber Jumper - 15 ft.

Sector Equipment: RRUS11 B12

Scope of Work:

RAN Template: 67D97DB MUAC	A&L Template: 67D97DB_1xAIR+1OP
--------------------------------------	---

Sector 1 (Proposed) view from behind

Coverage Type	A - Outdoor Macro							
Antenna	1				2			
Antenna Model	Ericsson - AIR32 KRD901146-1_B66A_B2A (Octo)				RFS - APXVAARR24_43-U-NA20 (Octo)			
Azimuth	85				85			
M. Tilt	0				0			
Height	110				110			
Ports	P1	P2	P3	P4	P5	P6	P7	P8
Active Tech.	L2100	L2100	L1900 G1900	L1900 G1900	L700 L600 N600	L700 L600 N600		
Dark Tech.								
Restricted Tech.								
Decomm. Tech.								
E. Tilt	2	2	2	2	2	2		
Cables					Coax Jumper (x2)	Coax Jumper (x2)		
TMA's								
Diplexers / Combiners								
Radio					Radio 4449 B71+B85 (At Antenna)	SHARED Radio 4449 B71+B85 (At Antenna)		
Sector Equipment								

104-ft per T-MOBILE

104-ft per T-MOBILE

Unconnected Equipment:

Scope of Work:

This is a new tower. Must order everything new.

*A dashed border indicates shared equipment. Any connected equipment is denoted with the SHARED keyword.

RAN Template: 67D97DB MUAC	A&L Template: 67D97DB_1xAIR+1OP
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Sector 2 (Existing) view from behind				
Coverage Type	A - Outdoor Macro			
Antenna	1		2	
Antenna Model	Ericsson - AIR21 KRC118023-1_B2P_B4A (Quad)		Ericsson - AIR21 KRC118023-1_B2P_B4A (Quad)	
Azimuth	260		260	
M. Tilt	0		0	
Height	100		100	
Ports	P1	P2	P3	P4
Active Tech.	U2100		L2100	
Dark Tech.				
Restricted Tech.				
Decomm. Tech.				
E. Tilt	2		2	
Cables	Fiber Jumper - 15 ft.		Fiber Jumper - 15 ft.	
TMA's				
Diplexers / Combiners				
Radio				
Sector Equipment				
Unconnected Equipment:				
<div style="display: flex; justify-content: space-between; align-items: center;"> <div style="border: 1px solid #ccc; border-radius: 5px; padding: 2px 5px;">Cable: Fiber Jumper</div> <div style="border: 1px solid #ccc; border-radius: 5px; padding: 2px 5px;">Sector Equipment: RRUS11 B12</div> </div>				
Scope of Work:				

RAN Template: 67D97DB MUAC	A&L Template: 67D97DB_1xAIR+1OP
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Sector 2 (Proposed) view from behind

Coverage Type	A - Outdoor Macro							
Antenna	1				2			
Antenna Model	Ericsson - AIR32 KRD901146-1_B66A_B2A (Octo)				RFS - APXVAARR24_43-U-NA20 (Octo)			
Azimuth	260				260			
M. Tilt	0				0			
Height	110				110			
Ports	P1	P2	P3	P4	P5	P6	P7	P8
Active Tech.	L2100	L2100	L1900 G1900	L1900 G1900	L700 L600 N600	L700 L600 N600		
Dark Tech.								
Restricted Tech.								
Decomm. Tech.								
E. Tilt	2	2	2	2	2	2		
Cables					Coax Jumper (x2)	Coax Jumper (x2)		
TMA's								
Diplexers / Combiners								
Radio					Radio 4449 B71+B85 (At Antenna)	SHARED Radio 4449 B71+B85 (At Antenna)		
Sector Equipment								

104-ft per T-MOBILE

104-ft per T-MOBILE

Unconnected Equipment:

Scope of Work:

This is a new tower. Must order everything new.

*A dashed border indicates shared equipment. Any connected equipment is denoted with the SHARED keyword.

RAN Template: 67D97DB MUAC	A&L Template: 67D97DB_1xAIR+1OP
--------------------------------------	---

Sector 3 (Existing) view from behind				
Coverage Type	A - Outdoor Macro			
Antenna	1		2	
Antenna Model	Ericsson - AIR21 KRC118023-1_B2P_B4A (Quad)		Ericsson - AIR21 KRC118023-1_B2P_B4A (Quad)	
Azimuth	325		325	
M. Tilt	0		0	
Height	100		100	
Ports	P1	P2	P3	P4
Active Tech.	U2100		L2100	
Dark Tech.				
Restricted Tech.				
Decomm. Tech.				
E. Tilt	2		2	
Cables	Fiber Jumper - 15 ft.		Fiber Jumper - 15 ft.	
TMA's				
Diplexers / Combiners				
Radio				
Sector Equipment				
Unconnected Equipment:				
<div style="display: flex; justify-content: space-between;"> Cable: Fiber Jumper - 15 ft. Sector Equipment: RRUS11 B12 </div>				
Scope of Work:				

RAN Template: 67D97DB MUAC	A&L Template: 67D97DB_1xAIR+1OP
--------------------------------------	---

Sector 3 (Proposed) view from behind

Coverage Type	A - Outdoor Macro							
Antenna	1				2			
Antenna Model	Ericsson - AIR32 KRD901146-1_B66A_B2A (Octo)				RFS - APXVAARR24_43-U-NA20 (Octo)			
Azimuth	325				325			
M. Tilt	0				0			
Height	110				110			
Ports	P1	P2	P3	P4	P5	P6	P7	P8
Active Tech.	L2100	L2100	L1900 G1900	L1900 G1900	L700 L600 N600	L700 L600 N600		
Dark Tech.								
Restricted Tech.								
Decomm. Tech.								
E. Tilt	2	2	2	2	2	2		
Cables					Coax Jumper (x2)	Coax Jumper (x2)		
TMA's								
Diplexers / Combiners								
Radio					Radio 4449 B71+B85 (At Antenna)	SHARED Radio 4449 B71+B85 (At Antenna)		
Sector Equipment								

104-ft per T-MOBILE

104-ft per T-MOBILE

Unconnected Equipment:

Scope of Work:

This is a new tower. Must order everything new.

*A dashed border indicates shared equipment. Any connected equipment is denoted with the SHARED keyword.

RAN Template: 67D97DB MUAC	A&L Template: 67D97DB_1xAIR+1OP
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Section 7 - Power Systems Equipment

Existing Power Systems Equipment

----- This section is intentionally blank. -----

Proposed Power Systems Equipment



Dual Slant Polarized Quad Band (8 Port) Antenna, 617-746/617-746/1695-2200/1695-2200MHz, 65deg, 15/15/18/18dBi, 2.4m (8ft), VET, RET, 0-12°/0-12°/2-12°/2-12°

FEATURES / BENEFITS

This antenna provides a 8 Port multi-band flexible platform for advanced use for flexible use in deployment scenarios for encompassing 600MHz, 700MHz, AWS & PCS applications.



- ➔ 24 Inch Width For Easier Zoning
- ➔ Field Replaceable (Integrated) AISG RET platform for reduced environmental exposure and long lasting quality
- ➔ Superior elevation pattern performance across the entire electrical down tilt range
- ➔ Includes three AISG RET motors - Includes 0.5m AISG jumper for optional daisy chain of two high band RET motors for one single AISG point of high band tilt control.
- ➔ Low band arrays driven by a single RET motor

Technical Features

LOW BAND LEFT ARRAY (617-746 MHZ) [R1]

Frequency Band	MHz	617-698	698-746
Gain	dBi	15.1	15.5
Horizontal Beamwidth @3dB	Deg	65	62
Vertical Beamwidth @3dB	Deg	11.4	10.4
Electrical Downtilt Range	Deg	0-12	0-12
Upper Side Lobe Suppression 0 to +20	dB	19	20
Front-to-Back, at +/-30°, Copolar	dB	25	24
Cross Polar Discrimination (XPD) @ Boresight	dB	19	19
Cross Polar Discrimination (XPD) @ +/-60	dB	5	3
3rd Order PIM 2 x 43dBm	dBc		-153
VSWR	-	1.5:1	1.5:1
Cross Polar Isolation	dB	25	25
Maximum Effective Power per Port	Watt	250	250

LOW BAND RIGHT ARRAY (617-746 MHZ) [R2]

Frequency Band	MHz	617-698	698-746
Gain	dBi	14.8	15.1
Horizontal Beamwidth @3dB	Deg	65	62
Vertical Beamwidth @3dB	Deg	11.4	10.3
Electrical Downtilt Range	Deg	0-12	0-12
Upper Side Lobe Suppression 0 to +20	dB	19	20
Front-to-Back, at +/-30°, Copolar	dB	25	23
Cross Polar Discrimination (XPD) @ Boresight	dB	19	19
Cross Polar Discrimination (XPD) @ +/-60	dB	5	3
3rd Order PIM 2 x 43dBm	dBc		-153
VSWR	-	1.5:1	1.5:1
Cross Polar Isolation	dB	25	25
Maximum Effective Power per Port	Watt	250	250



Dual Slant Polarized Quad Band (8 Port) Antenna, 617-746/617-746/1695-2200/1695-2200MHz, 65deg, 15/15/18/18dBi, 2.4m (8ft), VET, RET, 0-12°/0-12°/2-12°/2-12°

ELECTRICAL SPECIFICATIONS

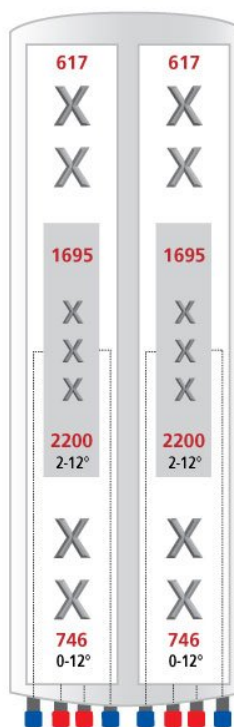
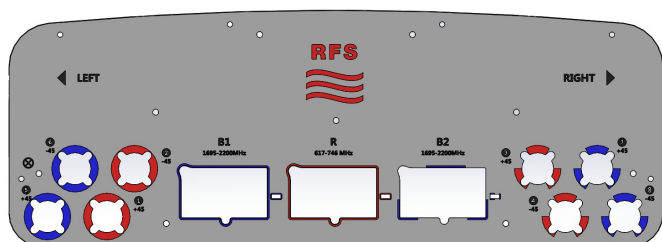
Impedance	Ohm	50.0
Polarization	Deg	±45°

MECHANICAL SPECIFICATIONS

Dimensions - H x W x D	mm (in)	2436 x 609 x 222 (95.9 x 24 x 8.7)
Weight (Antenna Only)	kg (lb)	58 (128)
Weight (Mounting Hardware only)	kg (lb)	11.5 (25.3)
Shipping Weight	kg (lb)	80 (176)
Connector type		8 x 4.3-10 female at bottom + 6 AISG connectors (3 male, 3 female)
Adjustment mechanism		Integrated RET solution AISG compliant (Field Replaceable) + Manual Override + External Tilt Indicator
Mounting Hardware Material		Galvanized steel
Radome Material / Color		Fiber Glass / Light Grey RAL7035

TESTING AND ENVIRONMENTAL

Temperature Range	°C (°F)	-40 to 60 (-40 to 140)
Lightning protection		IEC 61000-4-5
Survival/Rated Wind Velocity	km/h	241 (150)
Environmental		ETSI 300-019-2-4 Class 4.1E



ORDERING INFORMATION

Order No.	Configuration	Mounting Hardware	Mounting pipe Diameter	Shipping Weight
APXVAARR24_43-U-NA20	Field Replace RET included (3)	APM40-5E Beam tilt kit (included)	60-120mm	80 Kg

Exhibit C
Mount Analysis

Structural Analysis Report

Antenna Mount Analysis

T-Mobile Site #: CTHA524A

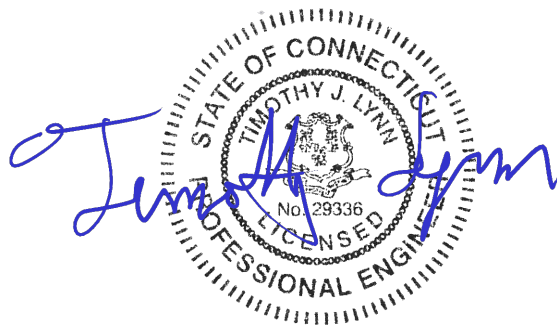
*1542 Boston Post Road
Westbrook, CT*

Centek Project No. 19027.46

~~Date: April 14, 2020~~

Rev 1: May, 2020

Max Stress Ratio = 57.6%



Prepared for:

*T-Mobile USA
35 Griffin Road
Bloomfield, CT 06002*

Table of Contents

SECTION 1 – REPORT

- ANTENNA AND APPURTENANCE SUMMARY
- STRUCTURE LOADING
- CONCLUSION

SECTION 2 – CALCULATIONS

- WIND LOAD ON APPURTENANCES
- RISA3D OUTPUT REPORT

SECTION 3 – REFERENCE MATERIALS (NOT INCLUDED WITHIN REPORT)

- RF DATA SHEET, DATED 04/06/2020.

Rev.1 May 4, 2020

Mr. Dan Reid
Transcend Wireless
10 Industrial Ave
Mahwah, NJ 07430

Re: *Structural Letter ~ Antenna Mount*
T-Mobile – Site Ref: CTHA524A
1542 Boston Post Road
Westbrook, CT 06498

Centek Project No. 19027.46

Dear Mr. Reid,

Centek Engineering, Inc. has reviewed the T-Mobile antenna installation at the above referenced site. The purpose of the review is to determine the structural adequacy of the proposed mount, consisting of one (1) 12-ft Quad Platform (SitePro P/N F4P-12W) and handrail (P/N F4P-HRK12) to support the equipment configuration, using Pipe 2.5 STD X 8'-0" long behind position 2 antennas. The review considered the effects of wind load, dead load and ice load in accordance with the 2015 International Building Code as modified by the 2018 Connecticut State Building Code (CTBC) including ASCE 7-10 and ANSI/TIA-222-G *Structural Standards for Steel Antenna Towers and Supporting Structures*.

The loads considered in this analysis consist of the following:


- **T-Mobile:**
Platform: Three (3) RFS APXVAARR24-43-NA20 panel antennas, three (3) Ericsson AIR32 KR0901146-1_B66A_B2A panel antennas, three (3) Ericsson 4449 B71_B12 remote radio units mounted on the quad platform with a RAD center elevation of 104 ft +/- AGL.

The antenna mount was analyzed per the requirements of the 2015 International Building Code as modified by the 2018 Connecticut State Building Code considering a nominal design wind speed of 105 mph for Westbrook as required in Appendix N of the 2018 Connecticut State Building Code.

A structural analysis of tower and foundation needs to be completed prior to any work.


Based on our review of the installation, it is our opinion that the **subject antenna mount has sufficient capacity** to support the aforementioned antenna configuration. If there are any questions regarding this matter, please feel free to call

Respectfully Submitted by:


Timothy J. Lynn, PE
Structural Engineer



Prepared by:


Fernando J. Palacios
Engineer

CEN TEK Engineering, Inc.
Structural Analysis – Mount Analysis
T-Mobile Site Ref. ~ CTHA524A
Westbrook, CT
Rev.1 ~ May 4, 2020

Section 2 - Calculations

**Development of Design Heights, Exposure Coefficients,
 and Velocity Pressures Per TIA-222-G**

Wind Speeds

Basic Wind Speed	V := 105	mph	(User Input - 2018 CSBC Appendix N)
Basic Wind Speed with Ice	V _i := 50	mph	(User Input per Annex B of TIA-222-G)

Input

Structure Type =	Structure_Type := Pole		(User Input)
Structure Category =	SC := 11		(User Input)
Exposure Category =	Exp := D		(User Input)
Structure Height =	h := 130	ft	(User Input)
Height to Center of Antennas =	z := 104	ft	(User Input)
Radial Ice Thickness =	t _i := 0.75	in	(User Input per Annex B of TIA-222-G)
Radial Ice Density =	I _d := 56.00	pcf	(User Input)
Topographic Factor =	K _{zt} := 1.0		(User Input)
	K _a := 1.0		(User Input)
Gust Response Factor =	G _H = 1.1		(User Input)

Output

Wind Direction Probability Factor =	$K_d := \begin{cases} \text{if Structure_Type = Pole} & 0.95 \\ \text{if Structure_Type = Lattice} & 0.85 \end{cases} = 0.95$	(Per Table 2-2 of TIA-222-G)
		(Per Table 2-3 of TIA-222-G)

Importance Factors =	$I_{Wind} := \begin{cases} \text{if SC = 1} & 1 \\ & 0.87 \\ \text{if SC = 2} & 1.00 \\ & 1.15 \\ \text{if SC = 3} & \end{cases}$
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	$I_{Wind_w_Ice} := \begin{cases} \text{if SC = 1} & 1 \\ & 0 \\ \text{if SC = 2} & 1.00 \\ \text{if SC = 3} & 1.00 \end{cases}$
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$$K_{iz} := \left(\frac{z}{33}\right)^{0.1} = 1.122$$

	$I_{ice} := \begin{cases} \text{if SC = 1} & 1 \\ & 0 \\ \text{if SC = 2} & 1.00 \\ \text{if SC = 3} & 1.25 \end{cases}$
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Velocity Pressure Coefficient Antennas =	t _{iz} := 2.0 • t _i • I _{ice} • K _{iz} • K _{zt} ^{0.35} = 1.682
	K _z := 2.01 • $\left(\frac{z}{zg}\right)^{\alpha}$ = 1.443

Velocity Pressure w/o Ice Antennas = $q_z := 0.00256 \cdot K_d \cdot K_z \cdot V^2 \cdot I_{Wind} = 39$ psf

Velocity Pressure with Ice Antennas = $q_{z_{ice}} := 0.00256 \cdot K_d \cdot K_z \cdot V_i^2 \cdot I_{Wind} = 9$ psf

Development of Wind & Ice Load on Antennas

Antenna Data:

Antenna Model =	RFS APXVAARR24_43-U-NA20
Antenna Shape =	Flat (User Input)
Antenna Height =	$L_{ant} := 95.9$ in (User Input)
Antenna Width =	$W_{ant} := 19.7$ in (User Input)
Antenna Thickness =	$T_{ant} := 8.7$ in (User Input)
Antenna Weight =	$WT_{ant} := 114.7$ lbs (User Input)
Number of Antennas =	$N_{ant} := 1$ (User Input)
Antenna Aspect Ratio =	$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 4.9$

Antenna Force Coefficient = $Ca_{ant} = 1.31$

Wind Load (without ice)

Surface Area for One Antenna = $SA_{antF} := \frac{L_{ant} \cdot W_{ant}}{144} = 13.1$ sf

Total Antenna Wind Force Front = $F_{ant} := qz \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot SA_{antF} = 729$ lbs

Surface Area for One Antenna = $SA_{antS} := \frac{L_{ant} \cdot T_{ant}}{144} = 5.8$ sf

Total Antenna Wind Force Side = $F_{ant} := qz \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot SA_{antS} = 322$ lbs

Wind Load (with ice)

Surface Area for One Antenna w/ Ice = $SA_{ICEantF} := \frac{(L_{ant} + 2 \cdot t_{iz}) \cdot (W_{ant} + 2 \cdot t_{iz})}{144} = 15.9$ sf

Total Antenna Wind Force w/ Ice Front = $Fi_{ant} := qz_{ice} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot SA_{ICEantF} = 200$ lbs

Surface Area for One Antenna w/ Ice = $SA_{ICEantS} := \frac{(L_{ant} + 2 \cdot t_{iz}) \cdot (T_{ant} + 2 \cdot t_{iz})}{144} = 8.3$ sf

Total Antenna Wind Force w/ Ice Side = $Fi_{ant} := qz_{ice} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot SA_{ICEantS} = 105$ lbs

Gravity Load (without ice)

Weight of All Antennas = $WT_{ant} \cdot N_{ant} = 115$ lbs

Gravity Loads (ice only)

Volume of Each Antenna = $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 2 \cdot 10^4$ cu in

Volume of Ice on Each Antenna = $V_{ice} := (L_{ant} + 2 \cdot t_{iz}) \cdot (W_{ant} + 2 \cdot t_{iz}) \cdot (T_{ant} + 2 \cdot t_{iz}) - V_{ant} = 1 \cdot 10^4$ cu in

Weight of Ice on Each Antenna = $W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 363$ lbs

Weight of Ice on All Antennas = $W_{ICEant} \cdot N_{ant} = 363$ lbs

Development of Wind & Ice Load on Antennas

Antenna Data:

Antenna Model =	Ericsson KRD901146-1_B66A_B2A	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 56.65$	in (User Input)
Antenna Width =	$W_{ant} := 12.77$	in (User Input)
Antenna Thickness =	$T_{ant} := 8.66$	in (User Input)
Antenna Weight =	$WT_{ant} := 132.2$	lbs (User Input)
Number of Antennas =	$N_{ant} := 1$	(User Input)
Antenna Aspect Ratio =	$AR_{ant} := \frac{L_{ant}}{W_{ant}} = 4.4$	

Antenna Force Coefficient = $Ca_{ant} = 1.29$

Wind Load (without ice)

Surface Area for One Antenna = $SA_{antF} := \frac{L_{ant} \cdot W_{ant}}{144} = 5$ sf

Total Antenna Wind Force Front = $F_{ant} := qz \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot SA_{antF} = 275$ lbs

Surface Area for One Antenna = $SA_{antS} := \frac{L_{ant} \cdot T_{ant}}{144} = 3.4$ sf

Total Antenna Wind Force Side = $F_{ant} := qz \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot SA_{antS} = 186$ lbs

Wind Load (with ice)

Surface Area for One Antenna w/ Ice = $SA_{ICEantF} := \frac{(L_{ant} + 2 \cdot t_{iz}) \cdot (W_{ant} + 2 \cdot t_{iz})}{144} = 6.7$ sf

Total Antenna Wind Force w/ Ice Front = $F_{ant} := qz_{ice} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot SA_{ICEantF} = 83$ lbs

Surface Area for One Antenna w/ Ice = $SA_{ICEantS} := \frac{(L_{ant} + 2 \cdot t_{iz}) \cdot (T_{ant} + 2 \cdot t_{iz})}{144} = 5$ sf

Total Antenna Wind Force w/ Ice Side = $F_{ant} := qz_{ice} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot SA_{ICEantS} = 62$ lbs

Gravity Load (without ice)

Weight of All Antennas = $WT_{ant} \cdot N_{ant} = 132$ lbs

Gravity Loads (ice only)

Volume of Each Antenna = $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 6265$ cu in

Volume of Ice on Each Antenna = $V_{ice} := (L_{ant} + 2 \cdot t_{iz}) \cdot (W_{ant} + 2 \cdot t_{iz}) \cdot (T_{ant} + 2 \cdot t_{iz}) - V_{ant} = 5379$ cu in

Weight of Ice on Each Antenna = $W_{ICEant} := \frac{V_{ice}}{1728} \cdot \rho = 174$ lbs

Weight of Ice on All Antennas = $W_{ICEant} \cdot N_{ant} = 174$ lbs

Development of Wind & Ice Load on RRUS's

RRUS Data:

RRUS Model =	Ericsson 4449 B71B12	
RRUS Shape =	Flat	(User Input)
RRUS Height =	$L_{RRUS} := 14.9$	in (User Input)
RRUS Width =	$W_{RRUS} := 13.2$	in (User Input)
RRUS Thickness =	$T_{RRUS} := 10.4$	in (User Input)
RRUS Weight =	$WT_{RRUS} := 74$	lbs (User Input)
Number of RRUS's =	$N_{RRUS} := 1$	
RRUS Aspect Ratio =	$Ar_{RRUS} := \frac{L_{RRUS}}{W_{RRUS}} = 1.1$	
RRUS Force Coefficient =	$Ca_{RRUS} = 1.2$	

Wind Load (without ice)

Surface Area for One RRUS = $SA_{RRUSF} := \frac{L_{RRUS} \cdot W_{RRUS}}{144} = 1.4$ sf

Total RRUS Wind Force = $F_{RRUS} := qz \cdot G_H \cdot Ca_{RRUS} \cdot K_a \cdot SA_{RRUSF} = 70$ lbs

Surface Area for One RRUS = $SA_{RRUSL} := \frac{L_{RRUS} \cdot T_{RRUS}}{144} = 1.1$ sf

Total RRUS Wind Force = $F_{RRUS} := qz \cdot G_H \cdot Ca_{RRUS} \cdot K_a \cdot SA_{RRUSL} = 55$ lbs

Wind Load (with ice)

Surface Area for One RRUS w/ Ice = $SA_{ICERRUSF} := \frac{(L_{RRUS} + 2 \cdot t_{iz}) \cdot (W_{RRUS} + 2 \cdot t_{iz})}{144} = 2.1$ sf

Total RRUS Wind Force w/ Ice = $F_{IRRUS} := qz_{ice} \cdot G_H \cdot Ca_{RRUS} \cdot K_a \cdot SA_{ICERRUSF} = 24$ lbs

Surface Area for One RRUS w/ Ice = $SA_{ICERRUSL} := \frac{(L_{RRUS} + 2 \cdot t_{iz}) \cdot (T_{RRUS} + 2 \cdot t_{iz})}{144} = 1.7$ sf

Total RRUS Wind Force w/ Ice = $F_{IRRUS} := qz_{ice} \cdot G_H \cdot Ca_{RRUS} \cdot K_a \cdot SA_{ICERRUSL} = 20$ lbs

Gravity Load (without ice)

Weight of All RRUSs = $WT_{RRUS} \cdot N_{RRUS} = 74$ lbs

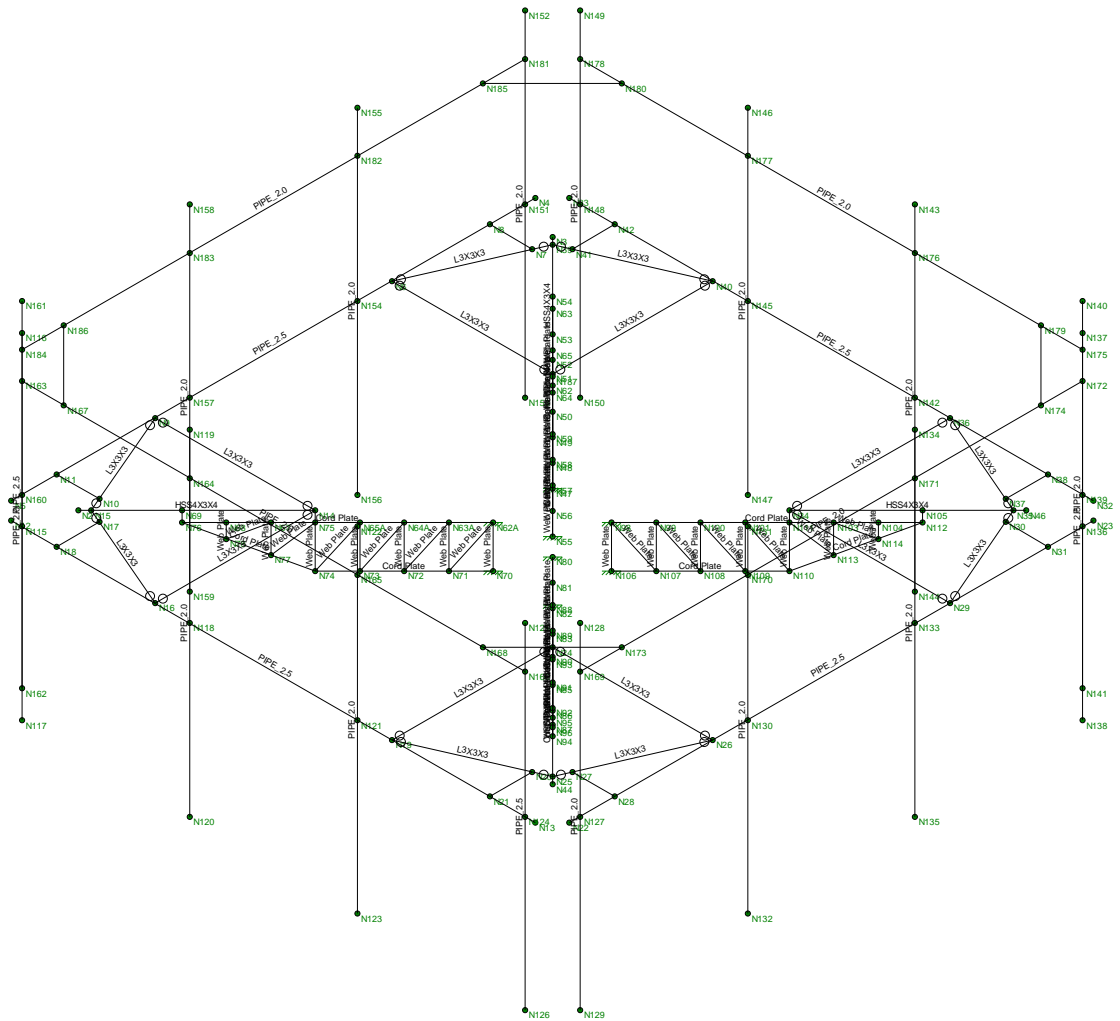
Gravity Loads (ice only)

Volume of Each RRUS = $V_{RRUS} := L_{RRUS} \cdot W_{RRUS} \cdot T_{RRUS} = 2045$ cu in

Volume of Ice on Each RRUS = $V_{ice} := (L_{RRUS} + 2 \cdot t_{iz}) \cdot (W_{RRUS} + 2 \cdot t_{iz}) \cdot (T_{RRUS} + 2 \cdot t_{iz}) - V_{RRUS} = 2119$ cu in

Weight of Ice on Each RRUS = $W_{ICERRUS} := \frac{V_{ice}}{1728} \cdot Id = 69$ lbs

Weight of Ice on All RRUSs = $W_{ICERRUS} \cdot N_{RRUS} = 69$ lbs



Envelope Only Solution

Centek

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FD	ÞFJG	ÈÈ↑↑↑↑		È	€	
FE	ÞFJH	ÈÈ↑↑↑↑	È	È	€	
FF	ÞFJI	ÈÈ↑↑↑↑	€	EG	€	
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Q̑	T Ī G	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̒	T Ī H	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̓	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̔	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̕	T Ī J	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̖	T Ī €	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̗	T Ī F	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̘	T Ī G	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̙	T Ī H	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̚	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̛	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̜	T Ī J	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̝	T Ī €	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̞	T Ī F	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̟	T Ī G	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̠	T Ī H	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̡	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̢	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̣	T Ī J	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̤	T Ī €	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̥	T Ī F	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̦	T Ī G	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̧	T Ī H	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̨	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̩	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̪	T Ī J	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̫	T Ī €	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̬	T Ī F	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̭	T Ī G	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̮	T Ī H	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̯	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̰	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̱	T Ī J	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̲	T Ī €	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̳	T Ī F	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̴	T Ī G	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̵	T Ī H	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̶	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̷	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̸	T Ī J	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̹	T Ī €	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̺	T Ī F	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̻	T Ī G	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̼	T Ī H	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̽	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̾	T Ī Ī	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€
Q̿	T Ī J	Ÿ	Ě Ě Ě Ě	Ě Ě Ě Ě	€	€

A Ya Vyf'8 jglfjVi hYX' @ UXg'f6 @ (: ' K j b X' k j h ÷ W' L' fl - dgZL' fV' cb j bi YXL

	T ^ (a ^\ Á æ ^\	Oá ^ & c\ }	Ú cæ o Á æ } a ^ Á Z D Ì È È	Ò) á Á æ } a ^ Á Z D Ì È È • - á	Ú cæ o Á æ } & cæ j } Z d Á á	Ò) á Á æ } & cæ j } Z d Á á
Ï Ï	T FFF	Ý	ÈÈÈJ	ÈÈÈJ	€	€
Ï Ï	T FFJ	Ý	ÈÈÈJ	ÈÈÈJ	€	€
Ï Ì	T FFÌ	Ý	ÈÈÈJ	ÈÈÈJ	€	€
Ï J	T FÉ	Ý	ÈÈÈJ	ÈÈÈJ	€	€
Ì €	T FÉ	Ý	ÈÈÈJ	ÈÈÈJ	€	€

A Ya Vyf'8 jglfjVi hYX' @ UXg'f6 @) : ' K j b X' L' fl ' dgZL

	T ^ (a ^\ Á æ ^\	Oá ^ & c\ }	Ú cæ o Á æ } a ^ Á Z D Ì È È	Ò) á Á æ } a ^ Á Z D Ì È È • - á	Ú cæ o Á æ } & cæ j } Z d Á á	Ò) á Á æ } & cæ j } Z d Á á
F	THF	Ý	ÈÈÈ	ÈÈÈ	€	€
G	THG	Ý	ÈÈÈ	ÈÈÈ	€	€
H	THH	Ý	ÈÈÈ	ÈÈÈ	€	€
I	TH	Ý	ÈÈÈ	ÈÈÈ	€	€
Í	THÍ	Ý	ÈÈÈ	ÈÈÈ	€	€
Î	THÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
Ï	THÏ	Ý	ÈÈÈ	ÈÈÈ	€	€
J	THJ	Ý	ÈÈÈ	ÈÈÈ	€	€
F€	TÌ€	Ý	ÈÈÈ	ÈÈÈ	€	€
FF	TÌF	Ý	ÈÈÈ	ÈÈÈ	€	€
FG	TÌG	Ý	ÈÈÈ	ÈÈÈ	€	€
FH	TÌH	Ý	ÈÈÈ	ÈÈÈ	€	€
FÌ	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
FÍ	TÌÍ	Ý	ÈÈÈ	ÈÈÈ	€	€
FÌ	TÌÉ	Ý	ÈÈÈ	ÈÈÈ	€	€
FÌ	TÌF	Ý	ÈÈÈ	ÈÈÈ	€	€
FÌ	TÌG	Ý	ÈÈÈ	ÈÈÈ	€	€
FJ	TÌH	Ý	ÈÈÈ	ÈÈÈ	€	€
G€	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
GF	TÌÍ	Ý	ÈÈÈ	ÈÈÈ	€	€
GG	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
GH	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
G	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
G	TÌJ	Ý	ÈÈÈ	ÈÈÈ	€	€
G	TÌ€	Ý	ÈÈÈ	ÈÈÈ	€	€
G	TÌF	Ý	ÈÈÈ	ÈÈÈ	€	€
G	TÌG	Ý	ÈÈÈ	ÈÈÈ	€	€
GJ	TÌH	Ý	ÈÈÈ	ÈÈÈ	€	€
H€	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
HF	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
HG	TÌJ	Ý	ÈÈÈ	ÈÈÈ	€	€
H	TÌ€	Ý	ÈÈÈ	ÈÈÈ	€	€
H	TÌF	Ý	ÈÈÈ	ÈÈÈ	€	€
H	TÌG	Ý	ÈÈÈ	ÈÈÈ	€	€
H	TÌH	Ý	ÈÈÈ	ÈÈÈ	€	€
H	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
H	TÌÍ	Ý	ÈÈÈ	ÈÈÈ	€	€
HJ	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
I€	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
IF	TÌÌ	Ý	ÈÈÈ	ÈÈÈ	€	€
IG	TÌJ	Ý	ÈÈÈ	ÈÈÈ	€	€
IH	TÌ€	Ý	ÈÈÈ	ÈÈÈ	€	€

A Ya Vyf'8]gfl]Vi hYX' @ UXg'f6 @ ' +: 'K]bX'N'fl dgZL'fT c b]h]bi YXL

	T \ (à\ Áæ ^)	Ôã^&ã }	ÙcæoÁ æ} ã à^Z ÐÈÈ Ò) áÁ æ} ã à^Z ÐÈÈ •-á	ÙcæoÁ} &æ] } ZđÁ á	Ò) áÁ} &æ] } ZđÁ á
H	T Í J	Z	ÈÈÈ	ÈÈÈ	€
H	T Í €	Z	ÈÈÈ	ÈÈÈ	€
H	T Í F	Z	ÈÈÈ	ÈÈÈ	€
H	T Í G	Z	ÈÈÈ	ÈÈÈ	€
H	T Í H	Z	ÈÈÈ	ÈÈÈ	€
HJ	T Í I	Z	ÈÈÈ	ÈÈÈ	€
I €	T Í Í	Z	ÈÈÈ	ÈÈÈ	€
I F	T Í Î	Z	ÈÈÈ	ÈÈÈ	€
I G	T Í Ï	Z	ÈÈÈ	ÈÈÈ	€
I H	T Í Ì	Z	ÈÈÈ	ÈÈÈ	€
I I	T Í J	Z	ÈÈÈ	ÈÈÈ	€
I Í	T Í €	Z	ÈÈÈ	ÈÈÈ	€
I Î	T Í F	Z	ÈÈÈ	ÈÈÈ	€
I Ï	T Í G	Z	ÈÈÈ	ÈÈÈ	€
I Ì	T Í H	Z	ÈÈÈ	ÈÈÈ	€
I J	T Í I	Z	ÈÈÈ	ÈÈÈ	€
I €	T Í Î	Z	ÈÈÈ	ÈÈÈ	€
I F	T Í J	Z	ÈÈÈ	ÈÈÈ	€
I G	T J €	Z	ÈÈÈ	ÈÈÈ	€
I H	T J F	Z	ÈÈÈ	ÈÈÈ	€
I I	T J G	Z	ÈÈÈ	ÈÈÈ	€
I Í	T J H	Z	ÈÈÈ	ÈÈÈ	€
I Î	T J I	Z	ÈÈÈ	ÈÈÈ	€
I Ï	T J Í	Z	ÈÈÈ	ÈÈÈ	€
I Ì	T J Î	Z	ÈÈÈ	ÈÈÈ	€
I J	T J Ï	Z	ÈÈÈ	ÈÈÈ	€
I €	T J Ì	Z	ÈÈÈ	ÈÈÈ	€
I F	T J J	Z	ÈÈÈ	ÈÈÈ	€
I G	T F €€	Z	ÈÈÈ	ÈÈÈ	€
I H	T F €F	Z	ÈÈÈ	ÈÈÈ	€
I I	T F €G	Z	ÈÈÈ	ÈÈÈ	€
I Í	T Í	Z	ÈÈÈG	ÈÈÈG	€
I Ï	T G G	Z	ÈÈÈG	ÈÈÈG	€
I Ì	T F G F	Z	ÈÈÈG	ÈÈÈG	€
I Ï	T F G H	Z	ÈÈÈG	ÈÈÈG	€
I J	T F €	Z	ÈÈÈG	ÈÈÈG	€
I €	T F €	Z	ÈÈÈG	ÈÈÈG	€
I F	T F € J	Z	ÈÈÈG	ÈÈÈG	€
I G	T F F €	Z	ÈÈÈG	ÈÈÈG	€
I H	T F F F	Z	ÈÈÈG	ÈÈÈG	€
I I	T F F G	Z	ÈÈÈG	ÈÈÈG	€
I Í	T F F H	Z	ÈÈÈG	ÈÈÈG	€
I Î	T F F I	Z	ÈÈÈG	ÈÈÈG	€
I Ï	T F F Í	Z	ÈÈÈG	ÈÈÈG	€
I Ì	T F F Î	Z	ÈÈÈG	ÈÈÈG	€
I J	T F F Ï	Z	ÈÈÈG	ÈÈÈG	€
I €	T F F Ì	Z	ÈÈÈG	ÈÈÈG	€
I F	T F F J	Z	ÈÈÈG	ÈÈÈG	€
I G	T F G €	Z	ÈÈÈG	ÈÈÈG	€

A Ya Vyf'8]gfl]Vi hYX' @ UXg'f6 @ ' ; : '6 @ ' & HfUbg]Ybh'5 fYU' @ UXgk

T \ (à\|Áæ ^) Ôã^&ã } ÙcæoÁ æ} ã à^Z ÐÈÈ Ò) áÁ æ} ã à^Z ÐÈÈ •-á ÙcæoÁ} &æ] } ZđÁ á Ò) áÁ} &æ] } ZđÁ á
 ÛÒÈÈÔÁ^!•ã } Á Ì ÈÈ www.centekeng.com ÆFáã ÆÁÔáVP ÔÉ G CE ÔET ÔÈÈáÁ Ûæ^ÁJ

9bj YcdY>c]bh8]gd`UWā Ybtg`fŷ cbh]bi YXL

	R ā c	Y ā ā	S Ō	Y ā ā	S Ō	Z ā ā	S Ō	Y ĀŪ [cāā] ĀĒS Ō	Y ĀŪ [cāā] ĀĒS Ō	Z ĀŪ [cāā] ĀĒS Ō	Y ĀŪ [cāā] ĀĒS Ō	Z ĀŪ [cāā] ĀĒS Ō
FH		{ ā	€	G	ĒĒĒ	H	ĒĒĒG	F ĒĒĒ Ī ^ Ē	G Ē Ē F Ī ^ Ē	G ĒĒĒĠ Ē	Ī	
FHU	p Ī G	{ ə	ĒĒĒH	Ī	ĒĒĒ	Ī	€	Ī FĒĒĒ Ġ Ē	Ī HĒĒ J F ^ Ē	Ī ĒĒĒ Ī ^ Ē	F	
FI€		{ ā	ĒĒĒĪ	G	ĒĒĒĪ	H	ĒĒĒĪ	F ĒĒĒĒ F ^ Ē	G ĒĒĒĒ Ī ^ Ē	G Ē ĒĒĒ Ī ^ Ē	Ī	
FIF	p Ī H	{ ə	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī ĒĒĒĒ Ġ Ē	Ī Ī ĒĒ Ī ^ Ē	Ī HĒĒ Ī Ġ Ē	F	
FIG		{ ā	ĒĒĒFG	G	ĒĒĒĠ	H	ĒĒĒĪ	F ĒĒĒĒ Ī ^ Ē	G ĒĒĒĒ Ī Ġ Ē	G ĒĒĒĒ Ī ^ Ē	Ī	
FIH	p Ī Ī	{ ə	ĒĒĒFG	Ī	ĒĒĒFH	Ī	ĒĒĒĪ	Ī Ī ĒĒ Ī ^ Ē	Ī FĒĒĒ Ġ Ē	Ī Ī ĒĒ Ī ^ Ē	F	
FII		{ ā	ĒĒĒGH	G	ĒĒĒ F	H	ĒĒĒĠ	F Ē Ē Ī ^ Ē	G Ē Ē F Ī ^ Ē	F ĒĒĒ Ī ^ Ē	Ī	
FII	p Ī Ī	{ ə	ĒĒĒĪ	Ī	ĒĒĒFH	Ī	ĒĒĒĪ	Ī ĒĒ Ī F Ē Ē	Ī ĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ Ī ^ Ē	F	
FII		{ ā	ĒĒĒĠ	G	ĒĒĒ	H	ĒĒĒ Ī	G ĒĒĒ F Ī ^ Ē	G ĒĒĒ Ē Ī ^ Ē	F ĒĒĒĒ Ī ^ Ē	Ī	
FII	p Ī Ī	{ ə	ĒĒĒ Ī	Ī	ĒĒĒĪ	Ī	ĒĒĒ Ī	Ī ĒĒ Ī Ī ^ Ē	Ī ĒĒ Ī Ġ Ē	Ī Ī ĒĒ Ī ^ Ē	F	
FII		{ ā	ĒĒĒ F	F	ĒĒĒ G	H	ĒĒĒĪ	G ĒĒĒ Ī Ġ Ē	G ĒĒĒ F Ī ^ Ē	F ĒĒĒ Ī ^ Ē	Ī	
FIJ	p Ī Ī	{ ə	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĠ	Ī ĒĒ Ī Ī ^ Ē	Ī ĒĒ Ī Ġ Ē	Ī Ī ĒĒ Ī ^ Ē	F	
FIE		{ ā	ĒĒĒ J	G	ĒĒĒ J	H	ĒĒĒ G	F Ē Ē Ī ^ Ē	G ĒĒĒ Ī ^ Ē	F ĒĒĒ Ī Ġ Ē	Ī	
FIF	p Ī Ī	{ ə	ĒĒĒ	Ī	ĒĒĒĪ	Ī	ĒĒĒ Ī	Ī ĒĒ Ġ Ī ^ Ē	Ī ĒĒ Ī F Ē Ē	Ī Ī ĒĒ Ī ^ Ē	F	
FIG		{ ā	ĒĒĒ J	F	ĒĒĒ Ī	H	ĒĒĒ Ī	F Ē Ē F Ē Ē	G ĒĒĒ Ī Ġ Ē	F ĒĒĒ Ī Ī ^ Ē	Ī	
FIH	p Ī €	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī F	{ ə	ĒĒĒĪ	Ī	ĒĒĒĒH	G	ĒĒĒĪ	F FĒĒ Ī ^ Ē	H FĒĒ Ġ Ē	Ī HĒĒ F Ē Ē	G	
FII		{ ā	ĒĒĒĪ	G	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī ĒĒĒ Ī Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī ^ Ē	Ī	
FII	p Ī G	{ ə	ĒĒĒ F	Ī	ĒĒĒĪ	G	ĒĒĒĠ	F FĒĒ Ī ^ Ē	F ĒĒ Ī Ġ Ē	Ī FĒĒ F Ē Ē	G	
FII		{ ā	ĒĒĒĠ	G	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ġ Ē	Ī ĒĒĒ Ī ^ Ē	F ĒĒĒ Ġ Ī ^ Ē	Ī	
FII	p Ī H	{ ə	ĒĒĒĪ	Ī	ĒĒĒĪ	G	ĒĒĒ Ī	F ĒĒ Ī Ġ Ē	F ĒĒ Ī Ġ Ē	Ī ĒĒ Ī Ī ^ Ē	G	
FIE		{ ā	ĒĒĒ Ī	G	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī ĒĒĒ Ī Ġ Ē	Ī ĒĒĒ Ġ Ī ^ Ē	F ĒĒĒ F Ī Ī ^ Ē	Ī	
FIF	p Ī Ī	{ ə	ĒĒĒ	Ī	ĒĒĒFH	G	ĒĒĒ F	F Ē Ē Ī Ġ Ē	F ĒĒ Ī Ġ Ē	Ī HĒĒ Ġ Ē	G	
FIG		{ ā	ĒĒĒ F	F	ĒĒĒ G	Ī	ĒĒĒ	Ī ĒĒĒ F Ē Ē	Ī ĒĒĒ Ī F Ē Ē	F ĒĒĒ Ī Ī ^ Ē	Ī	
FIH	p Ī Ī	{ ə	ĒĒĒ Ī	Ī	ĒĒĒĪ	G	ĒĒĒ Ī	F Ī ĒĒ Ī ^ Ē	F ĒĒ Ī ^ Ē	Ī HĒĒ Ī Ī ^ Ē	G	
FII		{ ā	ĒĒĒ F	F	ĒĒĒ	Ī	ĒĒĒ Ī	Ī ĒĒĒ Ī Ī ^ Ē	Ī ĒĒĒ Ġ Ē	F ĒĒĒ Ī Ġ Ē	Ī	
FII	p Ī Ī	{ ə	ĒĒĒFG	Ī	ĒĒĒĪ	G	ĒĒĒ Ī	F Ī ĒĒ Ī ^ Ē	F ĒĒ Ī Ġ Ē	Ī HĒĒ Ī Ī ^ Ē	G	
FII		{ ā	ĒĒĒ J	G	ĒĒĒ	Ī	ĒĒĒ F	Ī ĒĒĒ Ī Ī ^ Ē	Ī ĒĒĒ F Ē Ē	F ĒĒĒ Ī Ġ Ē	Ī	
FII	p Ī Ī	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	€	Ī	ĒĒĒĠ	G	ĒĒĒ F	G FĒĒ Ī Ġ Ē	F Ī ĒĒ Ī ^ Ē	Ī Ī ĒĒ Ġ Ē	G	
FIE		{ ā	ĒĒĒĠ	F	ĒĒĒĪ	Ī	ĒĒĒĠ	Ī ĒĒĒ Ī Ī ^ Ē	Ī ĒĒĒ Ī Ī ^ Ē	G Ē ĒĒĠ Ē	Ī	
FIF	p Ī €	{ ə	ĒĒĒĪ	Ī	ĒĒĒĪ	G	ĒĒĒĪ	G ĒĒ Ī Ī ^ Ē	F Ī ĒĒ Ī Ġ Ē	Ī FĒĒ Ī Ī ^ Ē	G	
FIG		{ ā	ĒĒĒĪ	F	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī ĒĒĒ Ī Ī ^ Ē	Ī ĒĒĒ F Ē Ē	G ĒĒĒ Ī Ī ^ Ē	Ī	
FIH	p Ī F	{ ə	ĒĒĒ F	Ī	ĒĒĒĪ	G	ĒĒĒ Ī	G HĒĒ Ī Ī ^ Ē	F FĒĒ F Ī Ī ^ Ē	Ī ĒĒĒ Ī Ī ^ Ē	G	
FII		{ ā	ĒĒĒĪ	F	ĒĒĒĠ	Ī	ĒĒĒ FH	Ī ĒĒĒ Ī Ī ^ Ē	Ī ĒĒĒ Ī Ī ^ Ē	G ĒĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī G	{ ə	ĒĒĒ Ī	Ī	ĒĒĒFG	G	ĒĒĒ Ġ	G Ī ĒĒ Ī Ī ^ Ē	F FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ Ī Ī ^ Ē	G	
FII		{ ā	ĒĒĒĠ	F	ĒĒĒĪ	Ī	ĒĒĒ F	Ī ĒĒĒ Ī Ī ^ Ē	Ī ĒĒĒ Ī Ī ^ Ē	G Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī H	{ ə	ĒĒĒ Ī	Ī	ĒĒĒFG	G	ĒĒĒ Ī	F Ī ĒĒ Ī Ī ^ Ē	F ĒĒ Ī Ī ^ Ē	Ī HĒĒ Ī Ī ^ Ē	G	
FII		{ ā	ĒĒĒ Ī	G	ĒĒĒĪ	Ī	ĒĒĒ Ī	Ī ĒĒĒ Ī F Ē Ē	Ī ĒĒĒ Ī Ī ^ Ē	F ĒĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī Ī	{ ə	ĒĒĒ F	Ī	ĒĒĒĪ	G	ĒĒĒ Ī	F Ī ĒĒ Ī Ī ^ Ē	F ĒĒ Ī Ī ^ Ē	Ī HĒĒ Ī Ī ^ Ē	G	
FIE		{ ā	ĒĒĒĪ	F	ĒĒĒ	Ī	ĒĒĒĠ	Ī ĒĒĒ Ī Ī ^ Ē	Ī ĒĒĒ F Ē Ē	F ĒĒĒ Ī Ī ^ Ē	Ī	
FIF	p Ī Ī	{ ə	ĒĒĒ Ī	Ī	ĒĒĒFH	G	ĒĒĒ Ī	G Ī ĒĒ Ī Ī ^ Ē	F ĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ Ī Ī ^ Ē	G	
FIG		{ ā	ĒĒĒ G	F	ĒĒĒ G	Ī	ĒĒĒ Ī	Ī ĒĒĒ Ī Ī ^ Ē	Ī ĒĒĒ Ī Ī ^ Ē	F ĒĒĒ Ī Ī ^ Ē	Ī	
FIH	p Ī Ī	{ ə	ĒĒĒ Ī	Ī	ĒĒĒĪ	G	ĒĒĒ Ī	F Ī ĒĒ F Ē Ē	F ĒĒ Ī Ī ^ Ē	Ī HĒĒ Ī Ī ^ Ē	G	
FII		{ ā	ĒĒĒ J	F	ĒĒĒ	Ī	ĒĒĒ Ī	Ī ĒĒĒ Ī Ī ^ Ē	Ī ĒĒĒ Ī Ī ^ Ē	F ĒĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī Ī	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	F
FII	p Ī J	{ ə	ĒĒĒĪ	F	ĒĒĒĠ	G	ĒĒĒĪ	G FĒĒ Ī Ī ^ Ē	G FĒĒ Ī Ī ^ Ē	Ī Ī ĒĒ F Ē Ē	Ī	
FII		{ ā	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī	ĒĒĒĪ	Ī Ē Ē Ġ Ē	Ī ĒĒĒ F Ē Ē	F Ē ĒĒ Ī Ī ^ Ē	Ī	
FII	p Ī J	{ ə	€	Ī	€	Ī	€	Ī	€	Ī	€	Ī
FII		{ ā	€	F	€	F	€	F	€	F	€	

9bj YcdY>c]bh8]gd`UWw Yblg f'f' cb]bi YXL

	Rãc	YÄá	SÖ	YÄá	SÖ	ZÄá	SÖ	YÄU] cæã) AËESÖ	YÄU] cæã) AËESÖ	ZÄU] cæã) AËESÖ		
FJ€		{ ä	ÈÈFÍ	Í	ÈÈFH	Î	ÈÈFÍ	I ÈÈÈÍ^ÄÈ	I ÈÈÈJ^ÄÈ	F ÈÈÈÍ^ÄÈ	F	
FJF	bFEF	{ æ	ÈÈGJ	F	ÈÈÈË	G	ÈÈH	F ÈÈÈÍ^ÄÈ	G ÈÈÈÍ^ÄÈ	I ÈÈÈÍ^ÄÈ	Í	
FJG		{ ä	ÈÈGJ	Í	ÈÈGG	Î	ÈÈHG	I ÈÈÈG^ÄÈ	I ÈÈÈH^ÄÈ	F ÈÈÈG^ÄÈ	F	
FJH	bFEH	{ æ	ÈÈG	F	ÈÈÈJ	G	ÈÈH	F ÈÈÈG^ÄÈ	G ÈÈÈG^ÄÈ	I ÈÈÈÍ^ÄÈ	Í	
FJI		{ ä	ÈÈG	Í	ÈÈHÍ	Î	ÈÈHÍ	I ÈÈÈH^ÄÈ	I ÈÈÈJ^ÄÈ	F ÈÈÈG^ÄÈ	F	
FJÍ	bFEÍ	{ æ	ÈÈJÍ	F	ÈÈF	G	ÈÈJÍ	F ÈÈÈÍ^ÄÈ	G ÈÈÈÍ^ÄÈ	I ÈÈÈÍ^ÄÈ	Í	
FJÏ		{ ä	ÈÈJ	Í	ÈÈG	Î	ÈÈH	I ÈÈÈÍ^ÄÈ	I ÈÈÈG^ÄÈ	F ÈÈÈG^ÄÈ	F	
FJÏ	bFEÏ	{ æ	ÈÈGJ	F	ÈÈF	G	ÈÈGJ	F ÈÈÈH^ÄÈ	G ÈÈÈJ^ÄÈ	I ÈÈÈÍ^ÄÈ	Í	
FJÏ		{ ä	ÈÈEF	Í	ÈÈF	Î	ÈÈEJ	I ÈÈÈÍ^ÄÈ	I ÈÈÈF^ÄÈ	F ÈÈÈG^ÄÈ	F	
FJJ	bFEÏ	{ æ	€	Í	€	Î	€	Í	€	Í	€	Í
G€€		{ ä	€	F	€	F	€	F	€	F	€	F
G€F	bFEÏ	{ æ	€	G	ÈÈEG	G	ÈÈEF	F ÈÈÈJ^ÄÈ	G ÈÈÈÍ^ÄÈ	Í ÈÈÈG^ÄÈ	Í	
G€G		{ ä	ÈÈEG	I	ÈÈÈË	Î	€	Í ÈÈÈÍ^ÄÈ	I ÈÈÈG^ÄÈ	G ÈÈÈJ^ÄÈ	F	
G€H	bFEÏ	{ æ	ÈÈEH	G	ÈÈÈË	G	ÈÈÈË	F ÈÈÈG^ÄÈ	G ÈÈÈG^ÄÈ	Í ÈÈÈG^ÄÈ	Í	
G€I		{ ä	ÈÈÈË	I	ÈÈÈF	Î	ÈÈÈË	Í ÈÈÈÍ^ÄÈ	I ÈÈÈG^ÄÈ	G ÈÈÈH^ÄÈ	F	
G€Í	bFEJ	{ æ	ÈÈEJ	G	ÈÈÈË	G	ÈÈF	F ÈÈÈG^ÄÈ	G ÈÈÈÍ^ÄÈ	Í ÈÈÈJ^ÄÈ	Í	
G€Ï		{ ä	ÈÈFÍ	I	ÈÈEG	Î	ÈÈÈF	Í ÈÈÈG^ÄÈ	I ÈÈÈÍ^ÄÈ	F ÈÈÈJ^ÄÈ	F	
G€Ï	bFFE	{ æ	ÈÈFÍ	G	ÈÈÈJ	G	ÈÈG	F ÈÈÈJ^ÄÈ	G ÈÈÈH^ÄÈ	Í ÈÈÈÍ^ÄÈ	Í	
G€Ï		{ ä	ÈÈGG	I	ÈÈH	Î	ÈÈÈJ	Í ÈÈÈG^ÄÈ	I ÈÈÈJ^ÄÈ	F ÈÈÈÍ^ÄÈ	F	
G€J	bFFF	{ æ	ÈÈJ	F	ÈÈÈJ	G	ÈÈË	F ÈÈÈH^ÄÈ	G ÈÈÈÍ^ÄÈ	Í ÈÈÈJ^ÄÈ	Í	
G€€		{ ä	ÈÈJ	Í	ÈÈGJ	Î	ÈÈJ	I ÈÈÈG^ÄÈ	I ÈÈÈJ^ÄÈ	F ÈÈÈH^ÄÈ	F	
GFF	bFFG	{ æ	ÈÈG	F	ÈÈF	G	ÈÈGG	F ÈÈÈH^ÄÈ	G ÈÈÈJ^ÄÈ	Í ÈÈÈÍ^ÄÈ	Í	
GFG		{ ä	ÈÈEJ	I	ÈÈF	Î	ÈÈEJ	I ÈÈÈÍ^ÄÈ	I ÈÈÈF^ÄÈ	F ÈÈÈG^ÄÈ	F	
GFH	bFFH	{ æ	ÈÈJ	G	ÈÈÈJ	G	ÈÈF	F ÈÈÈJ^ÄÈ	G ÈÈÈÍ^ÄÈ	Í ÈÈÈG^ÄÈ	Í	
GFI		{ ä	ÈÈJ	I	ÈÈHÍ	Î	ÈÈI	Í ÈÈÈG^ÄÈ	I ÈÈÈÍ^ÄÈ	F ÈÈÈF^ÄÈ	F	
GFI	bFFI	{ æ	ÈÈH	F	ÈÈF	G	ÈÈÍ	F ÈÈÈÍ^ÄÈ	G ÈÈÈÍ^ÄÈ	Í ÈÈÈÍ^ÄÈ	Í	
GFI		{ ä	ÈÈH	I	ÈÈG	Î	ÈÈF	I ÈÈÈÍ^ÄÈ	I ÈÈÈJ^ÄÈ	F ÈÈÈJ^ÄÈ	F	
GFI	bFFÍ	{ æ	ÈÈI	I	ÈÈI	Í	ÈÈH	I ÈÈÈG^ÄÈ	H ÈÈÈÍ^ÄÈ	Í ÈÈÈG^ÄÈ	H	
GFI		{ ä	ÈÈG	F	ÈÈI	H	ÈÈGH	G ÈÈÈG^ÄÈ	G ÈÈÈÍ^ÄÈ	F ÈÈÈJ^ÄÈ	I	
GFI	bFFÍ	{ æ	ÈÈEG	I	ÈÈI	Í	ÈÈH	I ÈÈÈÍ^ÄÈ	G ÈÈÈH^ÄÈ	Í ÈÈÈH^ÄÈ	G	
G€€		{ ä	ÈÈI	F	ÈÈI	H	ÈÈFI	G ÈÈÈG^ÄÈ	I ÈÈÈÍ^ÄÈ	G ÈÈÈG^ÄÈ	I	
G€F	bFFÍ	{ æ	ÈÈH	Í	ÈÈI	Í	ÈÈ	Í ÈÈÈG^ÄÈ	H ÈÈÈÍ^ÄÈ	I ÈÈÈG^ÄÈ	Í	
G€G		{ ä	ÈÈHI	F	ÈÈI	H	ÈÈGÍ	H ÈÈÈJ^ÄÈ	G ÈÈÈÍ^ÄÈ	F ÈÈÈG^ÄÈ	I	
G€H	bFFÍ	{ æ	ÈÈF	I	ÈÈEJ	G	ÈÈGF	I ÈÈÈÍ^ÄÈ	H ÈÈÈG^ÄÈ	I ÈÈÈJ^ÄÈ	G	
G€I		{ ä	ÈÈG	F	ÈÈH	H	ÈÈG	G ÈÈÈG^ÄÈ	G ÈÈÈJ^ÄÈ	F ÈÈÈÍ^ÄÈ	I	
G€Ï	bFFJ	{ æ	ÈÈEF	I	ÈÈEJ	G	ÈÈË	H ÈÈÈÍ^ÄÈ	H ÈÈÈG^ÄÈ	Í ÈÈÈH^ÄÈ	G	
G€Ï		{ ä	ÈÈHG	F	ÈÈH	H	ÈÈG	G ÈÈÈF^ÄÈ	Í ÈÈÈÍ^ÄÈ	F ÈÈÈG^ÄÈ	I	
G€Ï	bFG€	{ æ	ÈÈG	Í	ÈÈEJ	G	ÈÈH	Í ÈÈÈG^ÄÈ	Í ÈÈÈG^ÄÈ	I ÈÈÈF^ÄÈ	G	
G€Ï		{ ä	ÈÈG	F	ÈÈH	H	ÈÈJH	H ÈÈÈÍ^ÄÈ	G ÈÈÈJ^ÄÈ	F ÈÈÈG^ÄÈ	H	
G€J	bFGF	{ æ	ÈÈI	I	ÈÈI	G	ÈÈF	F ÈÈÈÍ^ÄÈ	H ÈÈÈH^ÄÈ	I ÈÈÈG^ÄÈ	F	
G€€		{ ä	ÈÈG	F	ÈÈJ	H	ÈÈI	Í ÈÈÈG^ÄÈ	G ÈÈÈG^ÄÈ	F ÈÈÈH^ÄÈ	Í	
G€F	bFGG	{ æ	ÈÈJÍ	Í	ÈÈI	G	ÈÈJÍ	H ÈÈÈJ^ÄÈ	H ÈÈÈÍ^ÄÈ	Í ÈÈÈG^ÄÈ	F	
G€G		{ ä	ÈÈHH	F	ÈÈJ	H	ÈÈI	Í ÈÈÈH^ÄÈ	Í ÈÈÈÍ^ÄÈ	G ÈÈÈJ^ÄÈ	Í	
G€H	bFGH	{ æ	ÈÈI	I	ÈÈI	G	ÈÈG	G ÈÈÈÍ^ÄÈ	Í ÈÈÈH^ÄÈ	I ÈÈÈJ^ÄÈ	F	
G€I		{ ä	ÈÈF	G	ÈÈJ	H	ÈÈÈË	Í ÈÈÈG^ÄÈ	G ÈÈÈG^ÄÈ	F ÈÈÈJ^ÄÈ	H	
G€Í	bFGÍ	{ æ	ÈÈÍ	I	ÈÈI	G	ÈÈEJ	F ÈÈÈH^ÄÈ	I ÈÈÈÍ^ÄÈ	Í ÈÈÈJ^ÄÈ	I	
G€Ï		{ ä	ÈÈG	F	ÈÈGG	Î	ÈÈFÍ	G ÈÈÈF^ÄÈ	G ÈÈÈÍ^ÄÈ	F ÈÈÈÍ^ÄÈ	G	
G€Ï	bFGÏ	{ æ	ÈÈJÍ	Í	ÈÈI	G	ÈÈEJ	F ÈÈÈÍ^ÄÈ	H ÈÈÈG^ÄÈ	Í ÈÈÈG^ÄÈ	F	
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9bj YcdY>c]bh8]gd`UWw Yblg f'f' cb]bi YXL

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FĪ	TGH	ŠYHYH	ĪĪ Ī	FFĪ Ī H ĪĪĪ € Ī
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GE	TĜ	ŠYHYH	ĪĪ Ī	ĪĪ Ī ĪĪ Ī
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GG	THE	Ô Ī à\^ ÅHESŌ	ĪĪ Ī	ĪĪ Ī F ĪĪ Ī
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Exhibit D

Letter of Authorization



March 9, 2020

**Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051**

Re: Letter of Authorization

Applicant: T-Mobile Northeast, LLC

Site Address: 1542 Boston Post Road, Westbrook, CT 06498

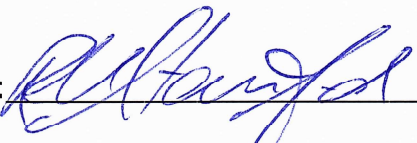
To Whom it May Concern:

T-Mobile Northeast LLC is seeking to install an antenna facility at the above referenced location. MCM Holdings, LLC is the owner of the tower at the above referenced address, which was previously approved for construction by the CT Siting Council under docket #485.

Permission is hereby granted to T-Mobile Northeast LLC and its agents for the purpose of consummating any applications necessary to gain the required land use approvals or permits on the above-referenced structure from the CT Siting Council and/or Town of Westbrook.

Any fees or charges associated with all applications or permits, and any conditions placed on the Applicant shall be the responsibility of T-Mobile Northeast LLC, its subsidiaries and agents.

Sincerely,

By: 
Name: Robert M. Stanford

Title: National Site Development Manager
Hereunto Duly Authorized

Exhibit E
Emissions Analysis Report



RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CTHA524A

CT Water Westbrook Watertank
1546 Boston Post Road
Westbrook, Connecticut 06498

May 6, 2020

EBI Project Number: 6219001696

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general population allowable limit:	6.18%

May 6, 2020

T-Mobile

Attn: Jason Overbey, RF Manager
35 Griffin Road South
Bloomfield, Connecticut 06002

Emissions Analysis for Site: CTHA524A - CT Water Westbrook Watertank

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **1546 Boston Post Road in Westbrook, Connecticut** for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately $400 \mu\text{W}/\text{cm}^2$ and $467 \mu\text{W}/\text{cm}^2$, respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at 1546 Boston Post Road in Westbrook, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 LTE channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 2 LTE channels (700 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 4 GSM channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 4) 2 LTE channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 5) 2 LTE channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.

- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the Ericsson AIR32 for the 1900 MHz / 1900 MHz / 2100 MHz channel(s), the RFS APXVAARR24_43-U-NA20 for the 600 MHz / 700 MHz channel(s) in Sector A, the Ericsson AIR32 for the 1900 MHz / 1900 MHz / 2100 MHz channel(s), the RFS APXVAARR24_43-U-NA20 for the 600 MHz / 700 MHz channel(s) in Sector B, the Ericsson AIR32 for the 1900 MHz / 1900 MHz / 2100 MHz channel(s), the RFS APXVAARR24_43-U-NA20 for the 600 MHz / 700 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerline of the proposed antennas is 104 feet above ground level (AGL).
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 11) **No emissions for additional carriers were included because the site location is not in the Connecticut Siting Council active database.**
- 12) All calculations were done with respect to uncontrolled / general population threshold limits.

T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Ericsson AIR32	Make / Model:	Ericsson AIR32	Make / Model:	Ericsson AIR32
Frequency Bands:	1900 MHz / 1900 MHz / 2100 MHz	Frequency Bands:	1900 MHz / 1900 MHz / 2100 MHz	Frequency Bands:	1900 MHz / 1900 MHz / 2100 MHz
Gain:	15.35 dBd / 15.35 dBd / 15.85 dBd	Gain:	15.35 dBd / 15.35 dBd / 15.85 dBd	Gain:	15.35 dBd / 15.35 dBd / 15.85 dBd
Height (AGL):	104 feet	Height (AGL):	104 feet	Height (AGL):	104 feet
Channel Count:	8	Channel Count:	8	Channel Count:	8
Total TX Power (W):	360 Watts	Total TX Power (W):	360 Watts	Total TX Power (W):	360 Watts
ERP (W):	12,841.53	ERP (W):	12,841.53	ERP (W):	12,841.53
Antenna A1 MPE %:	4.27%	Antenna B1 MPE %:	4.27%	Antenna C1 MPE %:	4.27%
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	RFS APXVAARR24_43-U-NA20	Make / Model:	RFS APXVAARR24_43-U-NA20	Make / Model:	RFS APXVAARR24_43-U-NA20
Frequency Bands:	600 MHz / 700 MHz	Frequency Bands:	600 MHz / 700 MHz	Frequency Bands:	600 MHz / 700 MHz
Gain:	12.95 dBd / 13.35 dBd	Gain:	12.95 dBd / 13.35 dBd	Gain:	12.95 dBd / 13.35 dBd
Height (AGL):	104 feet	Height (AGL):	104 feet	Height (AGL):	104 feet
Channel Count:	4	Channel Count:	4	Channel Count:	4
Total TX Power (W):	120 Watts	Total TX Power (W):	120 Watts	Total TX Power (W):	120 Watts
ERP (W):	2,481.08	ERP (W):	2,481.08	ERP (W):	2,481.08
Antenna A2 MPE %:	1.91%	Antenna B2 MPE %:	1.91%	Antenna C2 MPE %:	1.91%

Site Composite MPE %	
Carrier	MPE %
T-Mobile (Max at Sector A):	6.18%
Site Total MPE % :	6.18%

T-Mobile Sector A Total:	6.18%
T-Mobile Sector B Total:	6.18%
T-Mobile Sector C Total:	6.18%
Site Total:	6.18%

T-Mobile Maximum MPE Power Values (Sector A)

T-Mobile Frequency Band / Technology (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ($\mu\text{W}/\text{cm}^2$)	Frequency (MHz)	Allowable MPE ($\mu\text{W}/\text{cm}^2$)	Calculated % MPE
T-Mobile 1900 MHz GSM	4	1028.30	110.0	12.22	1900 MHz GSM	1000	1.22%
T-Mobile 1900 MHz LTE	2	2056.61	110.0	12.22	1900 MHz LTE	1000	1.22%
T-Mobile 2100 MHz LTE	2	2307.55	110.0	13.71	2100 MHz LTE	1000	1.37%
T-Mobile 600 MHz LTE	2	591.73	110.0	3.52	600 MHz LTE	400	0.88%
T-Mobile 700 MHz LTE	2	648.82	110.0	3.86	700 MHz LTE	467	0.83%
						Total:	6.18%

Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector A:	6.18%
Sector B:	6.18%
Sector C:	6.18%
T-Mobile Maximum MPE % (Sector A):	6.18%
Site Total:	6.18%
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **6.18%** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.