

STATE OF CONNECTICUT CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051 Phone: (860) 827-2935 Fax: (860) 827-2950 E-Mail: siting.council@ct.gov Web Site: portal.ct.gov/csc

VIA ELECTRONIC MAIL

March 3, 2023

Mark Roberts
SAI Group
12 Industrial Way
Salem, NH 03079
Mark.Roberts@QCDevelopment.net

RE: **EM-AT&T-153-230214** – AT&T notice of intent to modify an existing telecommunications facility located at 27 Siemon Company Drive, Watertown, Connecticut.

Dear Mark Roberts:

The Connecticut Siting Council (Council) is in receipt of your correspondence of March 2, 2023 submitted in response to the Council's February 28, 2023 notification of an incomplete request for exempt modification with regard to the above-referenced matter.

The submission renders the request for exempt modification complete and the Council will process the request in accordance with the Federal Communications Commission 60-day timeframe.

Thank you for your attention and cooperation.

Sincerely,

Melanie Bachman Executive Director

MAB/ANM/laf

From: Mark Roberts <mark.roberts@qcdevelopment.net>

Sent: Thursday, March 2, 2023 9:09 AM **To:** Fontaine, Lisa <Lisa.Fontaine@ct.gov>

Cc: CSC-DL Siting Council <Siting.Council@ct.gov>

Subject: RE: Council Incomplete - EM-AT&T-153-230214 (Siemon Company Drive) - Watertown

Importance: High

Hello – In response to your attached incompleteness letter, please find attached the corrected Structural Analysis. A hard copy will follow by mail.

Thanks

Mark Roberts QC Development 860-670-9068

Chimney Design Calculations by ICC Commonwealth 795 Wurlitzer Drive, North Tonawanda, NY 14120

<u>Customer:</u> TEP Northeast <u>ICC Project Number:</u> 2248

<u>Site:</u> 76 Westbury Park Road | Watertown, CT 06795 <u>Chimney Description:</u> 140' Radial Brick Chimney

Summary:

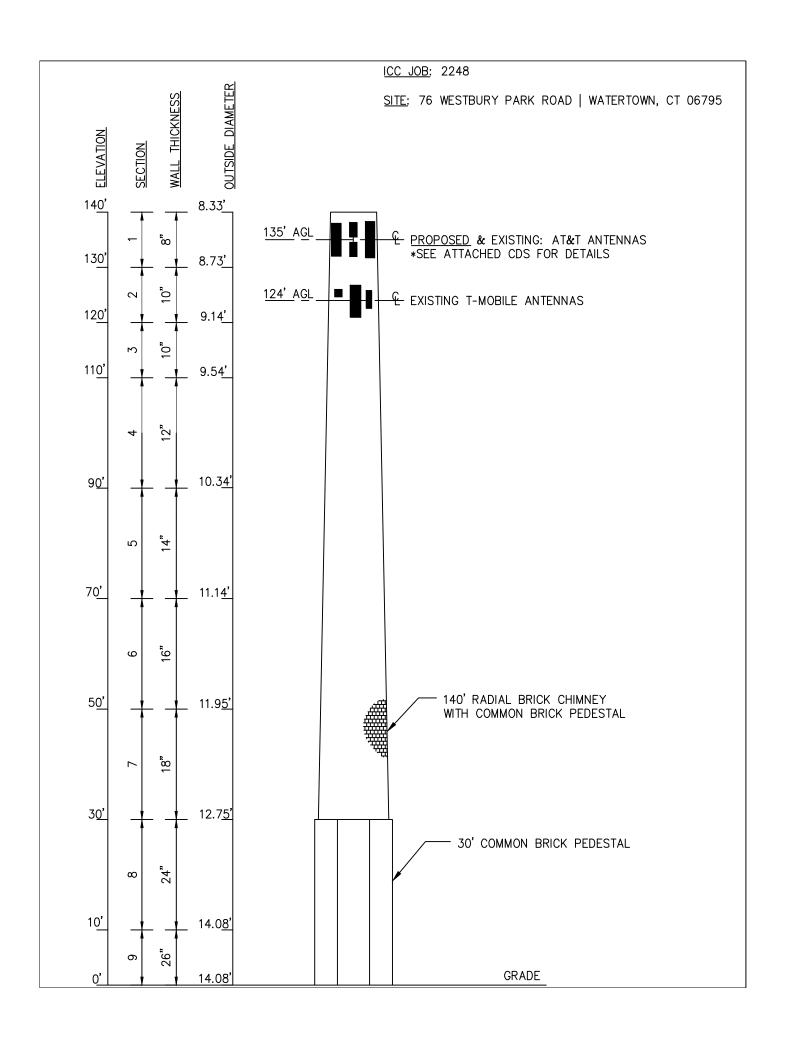
The following is a structural analysis of a 140' radial brick chimney at site mentioned in title above. With the proposed AT&T cellular equipment modifications at the 135' elevation, it was found that the chimney shell is not overstressed. This analysis assumes the brick chimney is in good condition with sound brick and mortar throughout its height. Therefore, the chimney must have the repairs noted below to make the chimney usable for AT&T to install their proposed equipment. This analysis assumes all repairs required from list below have been completed and all antenna mounts have been designed by others. If repairs are ignored, this chimney will likely be overstressed structurally which may lead to further damage and possible chimney failure. The existing foundation was not analyzed and therefore is not a design responsibility of ICC Commonwealth.

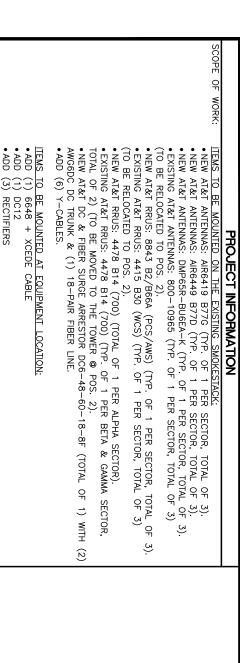
Repairs required:

- 1) Rake out and point all loose and open mortar joints on the exterior of radial section of the chimney column. Approximately 20% to 25% is required.
- 2) Replace spalled brick faces in the radial section of the chimney. Approximately (12) to (15) are required.
- 3) Rake out and point all loose and open mortar joints throughout the common brick pedestal. Approximately 10% to 15% is required.
- 4) Remove and replace loose brickwork in the corbel section of the pedestal on the East side.
- 5) Partial removal and replace the deteriorated sections of the cement water table at the top of the pedestal.
- 6) Cut out and point the 8' to 9' vertical crack found on the South side of the pedestal.
- 7) Cut out and point the vertical crack on the West side of the brick pedestal.
- 8) Remove loose and deteriorated spalls withing exterior of the foundation and repair with Sika Top 123 Plus. Repair remaining cracks with Sika Flex 11FC.
- 9) Repair and replace (2) broken downlead anchors and reattached lightning protection downlead on the East side.
- 10) Install new self-taping screws to hold down the roof access panel on the side lifting.

	Analysis Results
	Approved – Structure can accommodate the proposed changes. No repairs required.
Х	Conditional Approval - Structure can accommodate the proposed changes. Repairs required.
	Not Approved - Structure cannot accommodate the proposed changes without reinforcement.
	All repairs should be supervised under a qualified and experienced professional. If repairs are required and not performed and supervised by a licensed professional engineer, additional inspection is required.









SITE NUMBER: CT1 130

SITE NAME: WATERTOWN

FA CODE: 10035384

PACE ID: MRCTB055444, MRCTB053352, MRCTB054358, MRCTB061738, MRCTB056234, MRCTB054363

PROJECT: 5G NR 1SR CBAND, 4T4R RETRO OFIT, 5G NR SR UPGRADE

VICINITY MAP

DIRECTIONS TO SITE:

STRUCTURE HEIGHT:

140'-0"±

135'-0"± LTE, 136'-8"± 3.45GHZ, 133'-0"± C-BAND

TYPE OF SITE:

SMOKESTACK / INDOOR EQUIPMENT

SITE ADDRESS:

41.6033250 N,

73.1116661 W,

73° 06' 42.00" W 41° 36' 11.97" N 76 WESTBURY PARK ROAD WATERTOWN, CT 06795

ITEMS TO REMAIN:
•(3) ANTENNAS, (8) RRU'S, (2) DC6 SURGE ARRESTORS, (4) DC POWER & (2) FIBER.

EXISTING AT&T (12) COAX CABLES.

EXISTING AT&T ANTENNAS: P65-15-XLH-RR (TYIEXISTING AT&T ANTENNAS: HPA-65R-BU6AA(TYP. EXISTING AT&T RRUS: RRUS-12 B2 (TYP. OF 1 PAISTING AT&T TMA'S: TT19-08BP111 (TYP. OF

R (TYP. OF 1 PER SEA(TYP. OF 1 PER SECTOR, OF 1 PER SECTOR

EXISTING AT&T TMA'S: TT19-08BP111 (TYP. OF 1 PER SECTOR, TOTAL OF EXISTING AT&T DIPLEXER: CM1007-DBPXBC-003 (TYP. OF 2 PER SECTOR

TEMS TO BE REMOVED:

CURRENT USE:

ROPOSED USE:

TELECOMMUNICATIONS FACILITY TELECOMMUNICATIONS FACILITY

DRAWING INDEX

REV

HEAD NORTHEAST ON ENTERPRISE DR TOWARD CAPITAL BLVD. 0.3 MILES. TURN LEFT 0.3 MILES. TURN LEFT TO MERGE ONTO 1-91 S HAVEN. 9.1 MILES. TAKE EXIT 18 TO MERGE ONTO 1-691 W TOWARD MERIDEN/WATER TAKE EXIT 1 TO MERGE ONTO 1-84 W TOWARD WATERBURY/DANBURY. 8.7 MILES. TAKE EXIT 1 TO MERGE ONTO 1-84 W TOWARD WATERBURY/DANBURY. 35 ON THE LEFT TOWARD OAKVILLE/WATERTOWN. 0.5 MILES. SLIGHT LEFT AT E AURORA ST. 62 FT. COLONTO RUDY AVE. 0.2 MILES. CONTINUE 0.3 MILES. TURN LEFT AT CAPITAL BLVD

MERGE ONTO 1-91 S TOWARD NEW CONTINUE STRAIGHT NUE ONTO MAIN ST. Y. 8.0 MILES. XIT 20 TO CT-73

THIS DOCUMENT IS THE DUPLICATION OR USE AND USE BY GOVERN AUTHORIZED REGULATOR THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF AT&T. ANY E WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED. DUPLICATION INMENT AGENCIES FOR THE PURPOSES OF CONDUCTING THEIR LAWFULLY ATTORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.

GENERAL NOTES

- THE FACILITY IS AN UNI ACCESSED BY TRAINED ONE NOT REQUIRE ANY WATER REGULATIONS REQUIRING N UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY INED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY JIRING PUBLIC ACCESS PER ADA REQUIREMENTS.
- CONTRACTOR SHALL VAND SHALL IMMEDIATE BEFORE PROCEEDING VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE IELY NOTIFY THE AT&T MOBILITY REPRESENTATIVE IN WRITING OF DISCREPANCIES WITH THE WORK OR BE RESPONSIBLE FOR SAME.
- CONSTRUCTION DRAWINGS ARE VALID FOR SIX MONTHS AFTER ENGINEER OF RECORD'S STAMPED AND SIGNED SUBMITTAL DATE LISTED HEREIN.

SN-1

STRUCTURAL NOTES

A-4

DETAILS

A-3

DETAILS

A-2

ANTENNA LAYOUTS & ELEVATION

G-1

GROUNDING DETAILS

GN-1

GENERAL NOTES

TITLE SHEET

SHEET NO.

DESCRIPTION

A-1

COMPOUND & EQUIPMENT PLANS

RF-1

RF PLUMBING DIAGRAM

HUDSON
Design Group LLC

72 HOURS



CALL TO

UNDERGROUND SERVICE ALERT

OR CALL 811



SITE NUMBER: CT1130 SITE NAME: WATERTOWN





CONSTRUCTION

TITLE SHEET

NR 1SR CBAND, 4T4R RETROFIT, 5G NR SR CT1130

at&t

GROUNDING NOTES

- IHE SUBCONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE—SPECIFIC (UL, LPI, OR NFPA) LIGHTING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE SUBCONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE CONTRACTOR FOR RESOLUTION.
- 5 ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC. В
- THE SUBCONTRACTOR SHALL PERFORM IEEE FALL—OF—POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81 STANDARDS) FOR NEW GROUND ELECTRODE SYSTEMS. SUBCONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS. ŦE
- METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- Ģ EACH BTS CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 AWG STRANDED COPPER OR LARGER FOR INDOOR BTS AND #2 AWG STRANDED COPPER FOR OUTDOOR BTS.
- ნ. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- 7. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- œ ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED GROUND BAR. OR BOLTED TO
- 9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- <u>.</u> MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- <u>-</u> METAL CONDUIT SHALL BE MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 AWG COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 12. ALL NEW STRUCTURES WITH A FOUNDATION AND/OR FOOTING HAVING 20 FT. OR MORE OF 1/2 IN. OR GREATER ELECTRICALLY CONDUCTIVE REINFORCING STEEL MUST HAVE IT BONDED TO THE GROUND RING USING AN EXOTHERMIC WELD CONNECTION USING #2 AWG SOLID BARE TINNED COPPER GROUND WIRE, PER NEC 250.50

GENERAL NOTES

- FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:
- CONTRACTOR SAI SUBCONTRACTOR GENERAL CONTRACTOR (CONSTRUCTION) OWNER AT&T MOBILITY
- PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.
- ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AN LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE DECIMATIONS. AND

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4. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE

REGULATIONS.

UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.

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- 6. "KITTING LIST" SUPPLIED WITH THE BID PACKAGE IDENTIFIES ITEMS THAT WILL BE SUPPLIED CONTRACTOR. ITEMS NOT INCLUDED IN THE BILL OF MATERIALS AND KITTING LIST SHALL BE SUPPLIED BY THE SUBCONTRACTOR. Вή
- .7 THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- œ THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, IDECONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION SPACE FOR APPROVAL . 목품
- 9 SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR.
- <u>.</u> THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
- SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION. S
- SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION.
- 13. ALL CONCRETE REPAIR WORK SHALL BE DONE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301.

ANY NEW CONCRETE NEEDED FOR THE CONSTRUCTION SHALL BE AIR—ENTRAINED HAVE 4000 PSI STRENGTH AT 28 DAYS. ALL CONCRETE WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS. AND , SHALL

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- 5. ALL STRUCTURAL STEEL WORK SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS. ALL STRUCTURAL STEEL SHALL BE ASTM A36 (Fy = 36 ksi) UNLESS OTHERWISE NOTED. PIPES SHALL BE ASTM A53 TYPE E (Fy = 36 ksi). ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED. TOUCH UP ALL SCRATCHES AND OTHER MARKS IN THE FIELD AFTER STEEL IS ERECTED USING A COMPATIBLE ZINC RICH PAINT.
- 16. CONSTRUCTION SHALL COMPLY WITH SPECIFICATIONS AND "GENERAL CONSTRUCTION SERVICES FOR CONSTRUCTION OF AT&T SITES."
- 17. SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.
- <u>.</u> THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION, ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.
- 19. SINCE THE CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE ADVISED TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.

20. APPLICABLE BUILDING CODES:

SUBCONTRACTOR'S WORK SHALL COMPLY WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AS ADOPTED BY THE LOCAL AUTHORITY HAVING JURISDICTION (AHJ) FOR THE LOCATION. THE EDITION OF THE AHJ ADOPTED CODES AND STANDARDS IN EFFECT ON THE DATE OF CONTRACT AWARD SHALL GOVERN THE DESIGN.

BUILDING CODE: IBC 2021 WITH 2022 CT STATE BUILDING CODE AMENDMENTS ELECTRICAL CODE: 2020 NATIONAL ELECTRICAL CODE (NFPA 70-2020)

SUBCONTRACTOR'S STANDARDS: WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING

AMERICAN INSTITUTE OF STEEL CONSTRUCTION CONSTRUCTION, ASD, FOURTEENTH EDITION; AMERICAN CONCRETE INSTITUTE (ACI) 316; BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE; (AISC) MANUAL 윾 STEEL

TELECOMMUNICATIONS INDUSTRY ASSOCIATION (TIA) 222-H, STRUCTURAL STANDARDS FOR STEEL

FOR ANY CONFLICTS BETWEEN SECTIONS OF LISTED CODES AND STANDARDS REGARDING MATERIAL, METHODS OF CONSTRUCTION, OR OTHER REQUIREMENTS, THE MOST RESTRICTIVE REQUIREMENT SHALL GOVERN. WHERE THERE IS CONFLICT BETWEEN A GENERAL REQUIREMENT AND A SPECIFIC REQUIREMENT, THE SPECIFIC REQUIREMENT SHALL GOVERN.

HUDSON
Design Group LLC

SITE NUMBER: CT1130 SITE NAME: WATERTOW WATERTOWN

76 WESTBURY PARK ROAD WATERTOWN, CT 06795 LITCHFIELD COUNTY

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	AS SHOWN		11/22 ISSUED FOR REVIEW	ISSUED FOR	ISSUED FOR			EGR	
	DESIGNED BY: HC	REVISIONS	REVIEW	21/22 ISSUED FOR CONSTRUCTION	30/23 ISSUED FOR CONSTRUCTION			EQUIPMENT GROUND RING	
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QUIPMENT GROUP

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UNDER GROUND

VERIFY IN FIELD

BTCW BBU

BARE TINNED SOLID COPPER WIRE BATTERY BACKUP

> MASTER GROUND BAR GALVANIZED RIGID CONDUIT

TBR **GBT**

TO BE REMOVED

TO BE DETERMINED RADIO FREQUENCY

AWG AGL

AMERICAN WIRE GAUGE

LINO

GRC S ĘQ

BOVE GRADE LEVEL

EQUAL

ABBREVIATIONS

GENERAL CONTRACTOR

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REQ

REQUIRED

BGR

BURIED GROUND

RING

Ĭ MGB

TBRR

TO BE RE TYPICAL

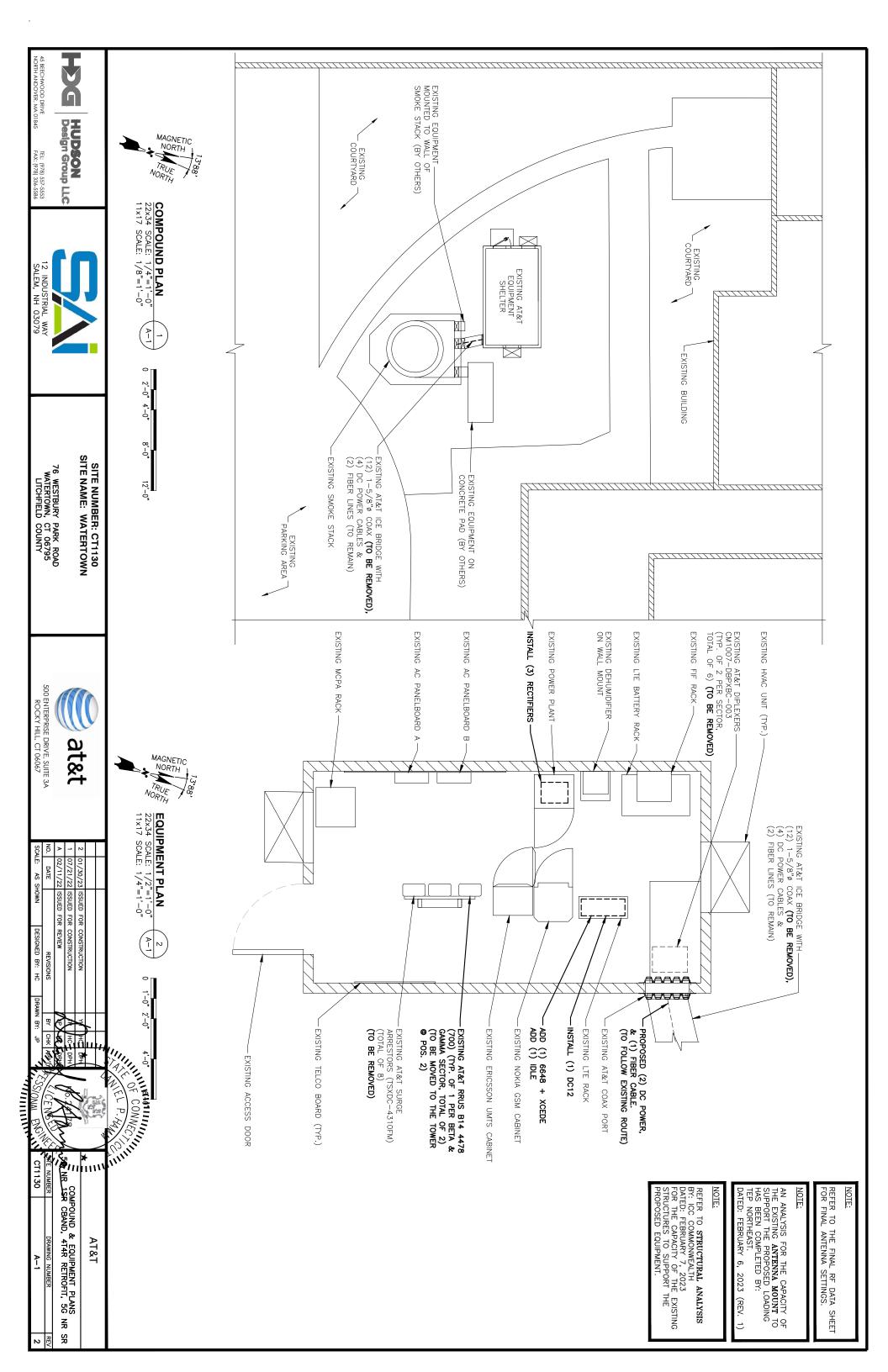
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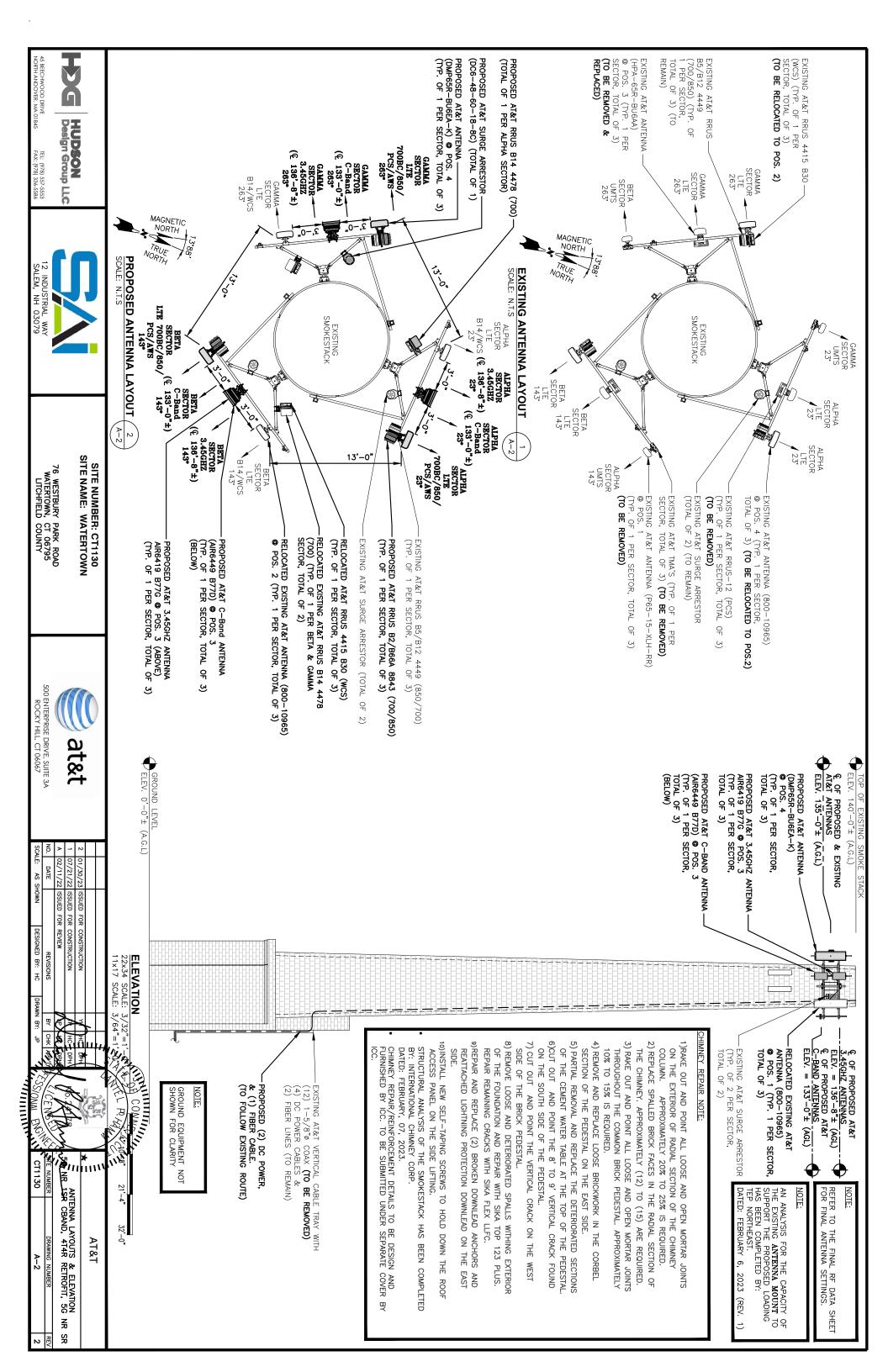
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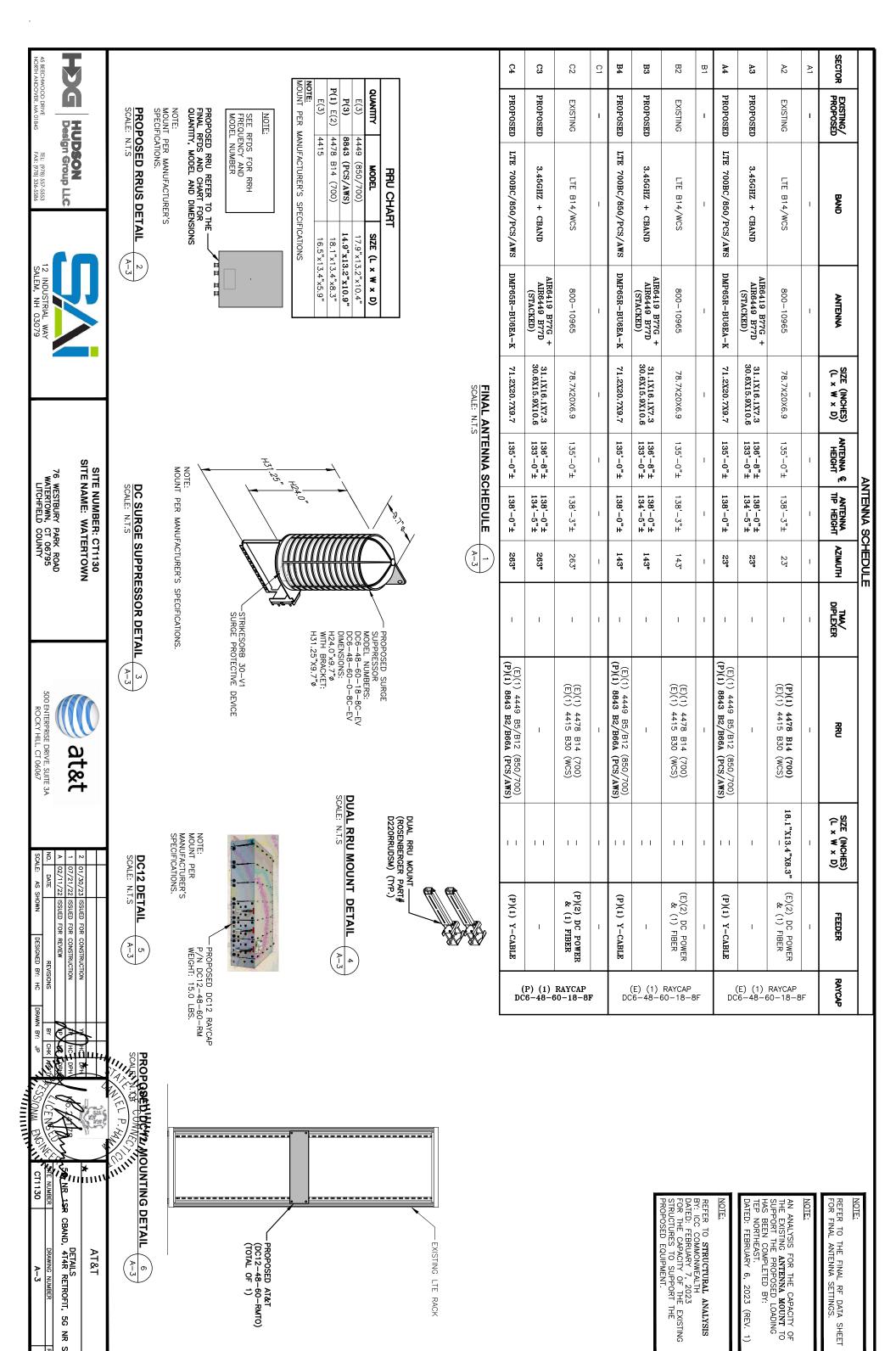
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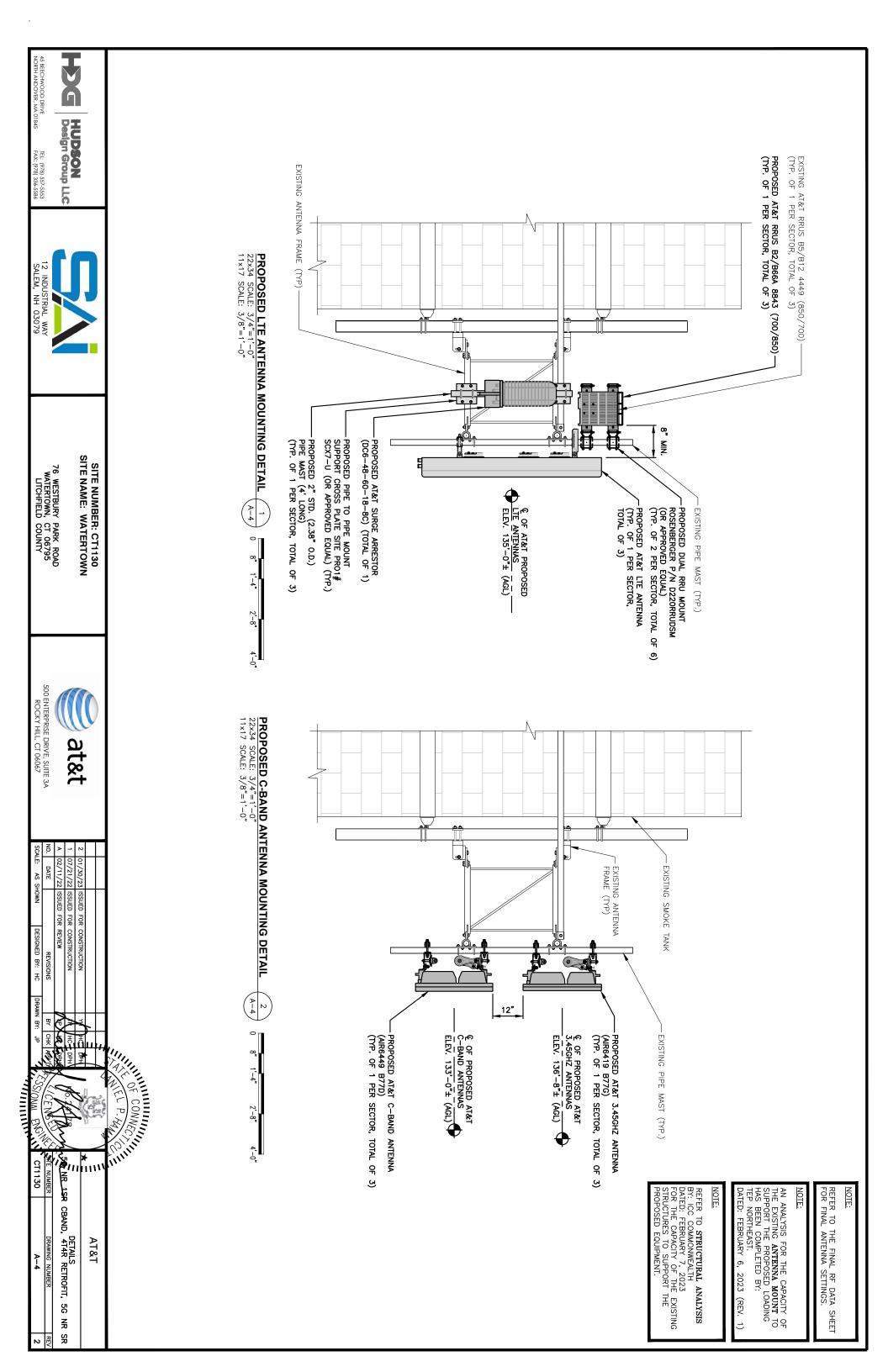
> PROPOSED MINIMUM







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STRUCTURAL NOTES:

- DESIGN REQUIREMENTS ARE PER STATE BUILDING CODE AND APPLICABLE SUPPLEMENTS, INTERNATIONAL BUILDING CODE, EIA/TIA-222-H STRUCTURAL STANDARDS FOR STEEL ANTENNA, TOWERS AND ANTENNA SUPPORTING
- STRUCTURES.

 STRUCTURES.

 CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. ANY UNUSUAL CONDITIONS SHALL BE REPORTED TO THE ATTENTION OF THE CONSTRUCTION MANAGER AND ENGINEER OF RECORD.

 DESIGN AND CONSTRUCTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".

 STRUCTURAL STEEL SHALL CONFORM TO ASTM A392 (Fy=50 ksi), MISCELLANDEOUS STEEL SHALL CONFORM TO ASTM A396 UNLESS OTHERWISE
- 5. STEEL PIPE SHALL CONFORM TO ASTM A500 "COLD-FORMED WELDED & STEEL PIPE STALL CONFORM TO ASTM A500 "COLD-FORMED WELDED & SEAMLESS CARBON STEEL STRUCTURAL TUBING", GRADE B, OR ASTM A53 PIPE STEEL BLACK AND HOT-DIPPED ZINC-COATED WELDED AND SEAMLESS TYPE E OR S, GRADE B. PIPE SIZES INDICATED ARE NOMINAL. ACTUAL OUTSIDE DIAMETER IS LARGER.

 6. STRUCTURAL CONNECTION BOLTS SHALL BE HIGH STRENGTH BOLTS (BEARING TYPE) AND CONFORM TO ASTM A325 TYPE-X "HIGH STRENGTH BOLTS FOR STRUCTURAL JOINTS, INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS". ALL BOLTS SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS", UNLESS OTHERWISE NOTED.

 8. ALL BOLTS, ANCHORS AND MISCELLANDOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC—COATING (HOT-DIP) ON IRON AND STEEL LARDWARE" INNIFESS OTHERWISE NOTED.
- HARDWARE", WILL HOLES, SAW CUTS AND ALL DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED WITH AN ARRONG ZINC REPAIR PAINT COMPLYING WITH REQUIREMENTS OF ASTM A780. GALVANIZING REPAIR PAINT SHALL HAVE 65 PERCENT ZINC BY WEIGHT, ZIRP BY DUNCAN GALVANIZING, GALVANIZING, REPAIR PAINT SHALL HAVE 65 PERCENT ZINC BY WEIGHT, ZIRP BY DUNCAN GALVANIZING, GALVANIZING, REPAIR PAINT SHALL BE NOT NOT LESS THAN 4 COATS (ALLOW TIME TO DRY BETWEEN COATS) WITH A RESULTING COATING THICKNESS OF APPLIED GALVANIZING, REPAIR PAINT SHALL BE NOT NOT LESS THAN 4 COATS (ALLOW TIME TO DRY BETWEEN COATS) WITH A RESULTING COATING THICKNESS REQUIRED BY ASTM A123 OR A153 AS APPLICABLE.

 10. CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES, APPEARANCE AND QUALITY OF WELDS, AND FOR METHODS USED IN CORRECTING WELDING. ALL WELDING SHALL CONFORM TO AISC AND DIJ. WHERE FILLET WELD SIZES AND WELDING SHALL CONFORM TO AISC AND DIJ. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "STEEL CONSTRUCTION MANUAL". 14TH EDITION. INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISTITING OR ROUNT-CONFORMING MATERALS OR CONDITIONS SHALL BE REPORTED TO THE CONSTRUCTION MANUACES APPROVAL.

 12. UNISTRUCTION MANAGER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL BE FORMED STEEL CHANNEL STRUT FRAMING AS MANUFACTURED BY UNISTRUT CORP., WAYNE, MI OR EQUAL. STRUT MEMBERS SHALL BE 15/8'x126A, UNILESS OTHERWISE MOSTED, AND SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION.

 13. EPOXY ANCHOR ASSEMBLY SHALL CONSIST OF STAINLESS STEEL ANCHOR ROD WITH HUTS & WASHERS. AN INTERNALLY THREADED INSERT, A SCREEN TUBE AND A EPOXY ADHESIVE. THE ANCHORING SYSTEM SHALL BE THE HILTH-HIT HY-270 AND OR HY-200 SYSTEMS (AS SPECIFIED IN DWG.) OR ENGINEERS APPROVED EQUAL.

 14. EXPANSION BOLTS SHALL CONFORM TO FEDERAL SPECIFICATION FF-S-325, GROWMENDATIONS.

 15. LUMBER SHALL COMPLY WITH THE REQUIREMENTS OF THE AMERICAN INSTITUTE OF THE AMERICAN INSTITUTE.
- 15. LUMBER SHALL COMPLY WITH THE REQUIREMENTS OF THE AMERICAN INSTITUCT.

 OF TIMBER CONSTRUCTION AND THE NATIONAL FOREST PRODUCTS ASSOCIATION'S NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION. ALL LUMBER SHALL BE PRESSURE TREATED AND SHALL BE STRUCTURAL GRADE NO. 2 OR BETTER.

 16. WHERE ROOF PENETRATIONS ARE REQUIRED, THE CONTRACTOR SHALL CONTACT AND COORDINATE RELATED WORK WITH THE BUILDING OWNER AND THE EXISTING ROOF INSTALLER. WORK SHALL BE PERFORMED IN SUCH A MANNER AS TO NOT VOID THE EXISTING ROOF WARRANTY. ROOF SHALL BE WATERTIGHT.

 17. ALL FIBERGLASS MEMBERS USED ARE AS MANUFACTURED BY STRONGWELL COMPANY OF BRISTOL, VA 24203. ALL DESIGN CRITERIA FOR THESE MEMBERS IS BASED ON INFORMATION PROVIDED IN THE DESIGN MANUAL. ALL REQUIREMENTS PUBLISHED IN SAID MANUAL MUST BE STRICTLY ADHERED TO.

 18. NO MATERIALS TO BE ORDERED AND NO WORK TO BE COMPLETED UNTIL SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED IN WRITING.

 DRAWINGS HAVE BEEN REVIEWED AND APPROVED IN WRITING. 17. 16.
- 19.

SPECIAL INSPECTIONS (REFERENCE IBC CHAPTER 17):

GENERAL: WHERE APPLICATION IS MADE FOR CONSTRUCTION, THE OWNER OR THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE ACTING AS THE OWNER'S AGENT SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PERFORM INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE INSPECTION CHECKLIST ABOVE.

THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND ENGINEERS OF RECORD INVOLVED IN THE DESIGN OF THE PROJECT ARE PERMITTED TO ACT AS THE APPROVED AGENCY AND THEIR PERSONNEL ARE PERMITTED TO ACT AS THE SPECIAL INSPECTOR FOR THE WORK DESIGNED BY THEM, PROVIDED THOSE PERSONNEL MEET THE QUALIFICATION REQUIREMENTS. STATEMENT OF SPECIAL INSPECTIONS: THE APPLICANT SHALL SUBMIT A STATEMENT OF SPECIAL INSPECTIONS PREPARED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE IN ACCORDANCE WITH SECTION 107.1 AS A CONDITION FOR ISSUANCE. THIS STATEMENT SHALL BE IN ACCORDANCE WITH SECTION 1705.

REPORT REQUIREMENT: SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FUNNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS SHALL BE SUBMITTED.

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SSUED FOR CONSTRUCTION P R R

R CONSTRUCTION R REVIEW

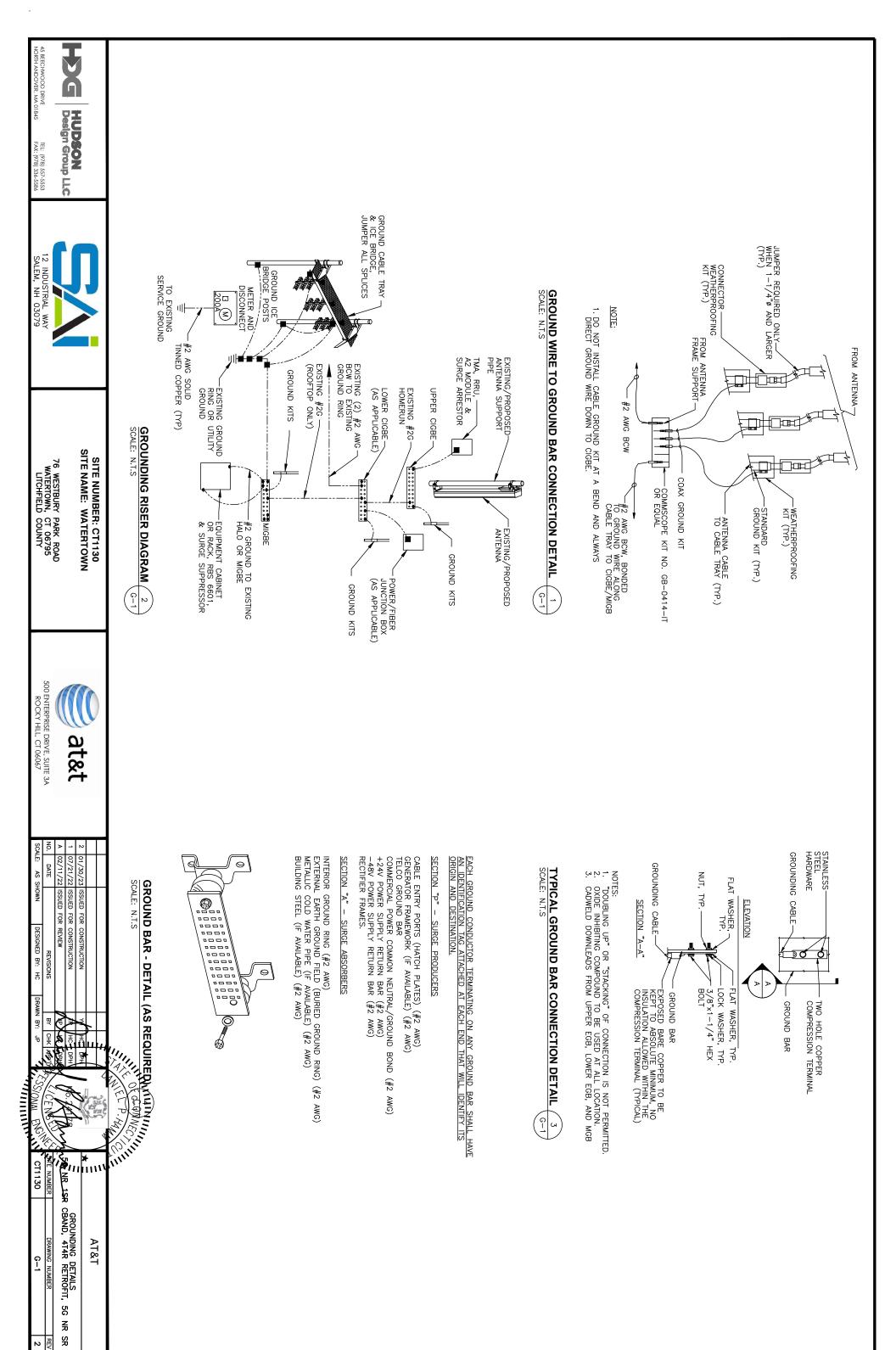
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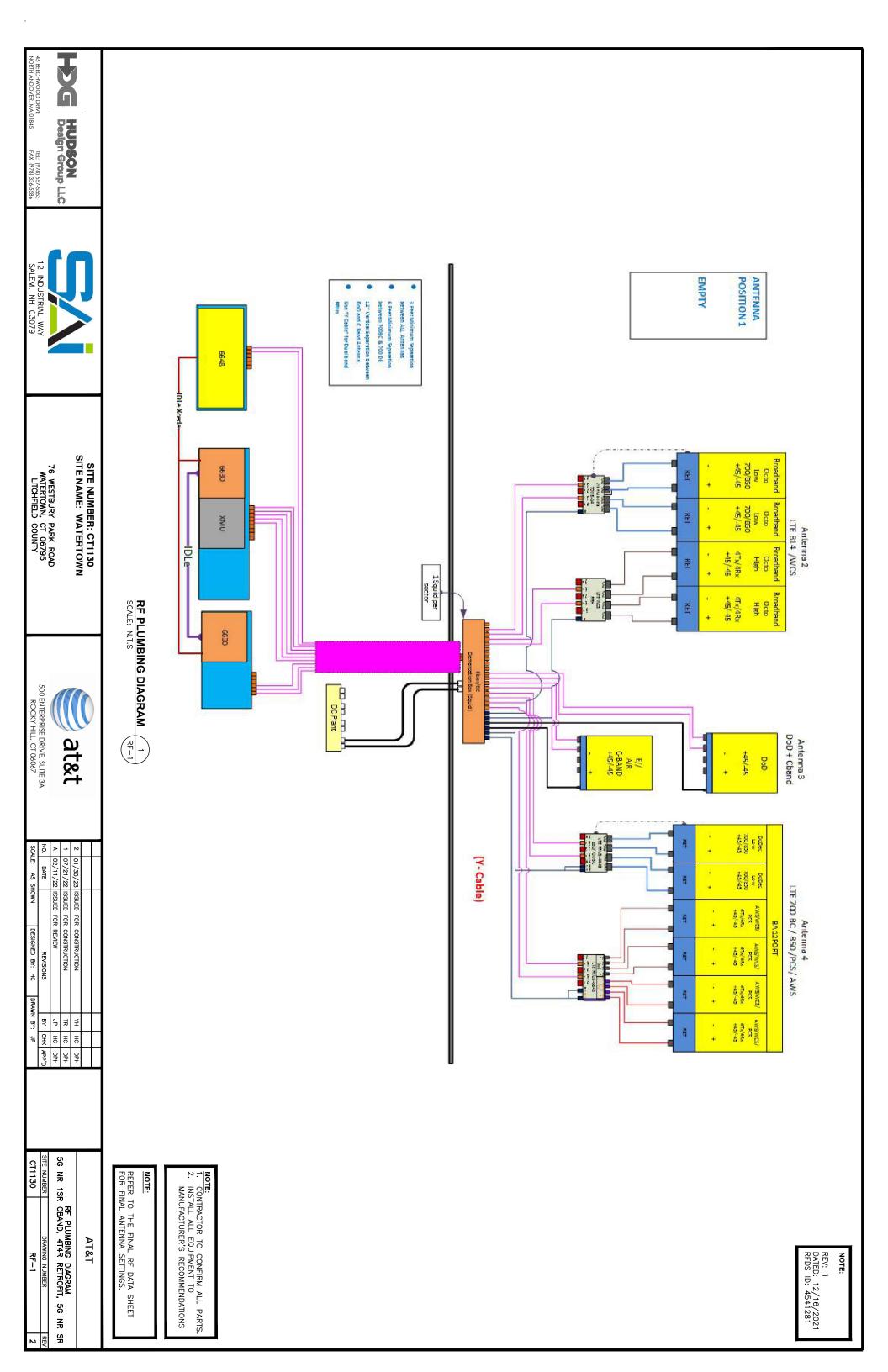
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76 WESTBURY PARK ROAD WATERTOWN, CT 06795
LITCHFIELD COUNTY

SITE NUMBER: CT1130 SITE NAME: WATERTOWN

HUDSON
Design Group LLC







Search Information

Address: 76 Westbury Park Rd, Watertown, CT 06795,

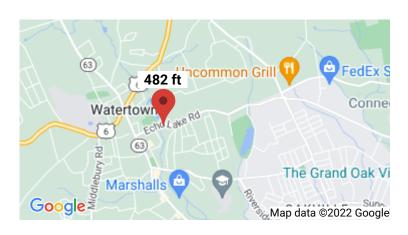
USA

Coordinates: 41.6037641, -73.1117191

Elevation: 482 ft

Timestamp: 2022-06-03T19:20:02.757Z

Hazard Type: Wind



ASCE 7-16	ASCE 7-10	ASCE 7-05
MRI 10-Year 75 mph	MRI 10-Year 76 mph	ASCE 7-05 Wind Speed 98 mph
MRI 25-Year 83 mph	MRI 25-Year 85 mph	
MRI 50-Year 89 mph	MRI 50-Year 91 mph	
MRI 100-Year 96 mph	MRI 100-Year 97 mph	
Risk Category I 106 mph	Risk Category I 108 mph	
Risk Category II 116 mph	Risk Category II 119 mph	
Risk Category III 125 mph	Risk Category III-IV 127 mph	
Risk Category IV A 129 mph		

You are in a wind-borne debris region if you are also within 1 mile of the coastal mean high water line.

The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

Disclaimer

Hazard loads are interpolated from data provided in ASCE 7 and rounded up to the nearest whole integer. Per ASCE 7, islands and coastal areas outside the last contour should use the last wind speed contour of the coastal area – in some cases, this website will extrapolate past the last wind speed contour and therefore, provide a wind speed that is slightly higher. NOTE: For queries near wind-borne debris region boundaries, the resulting determination is sensitive to rounding which may affect whether or not it is considered to be within a wind-borne debris region.

Mountainous terrain, gorges, ocean promontories, and special wind regions shall be examined for unusual wind conditions.

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Spreadsheet calculates wind pressures on sections of the chimny using Code ASCE 7-16 Wind Criteria

Height of chimney, h	140	ft	
Define risk category	II		Table 1.5-1
Define exposure factor	В		Section 26.7.3
Basic wind speed, V	116	mph	Attached Sheet, ASCE 7-16 wind properties
		<u></u>	
Gust factor, G	0.85		Section 26.11.1
Topographic factor, K _{zt}	1.0		Section 26.8.2
Directionality factor, K _d	1.0		Round chimney, Table 26.6-1
Ground elevation factor, K _e	1.0		Section 26.9
Wind pressure, q	29.28	psf	$q = 0.00256K_{zt}K_dK_eGV^2$ (Eq. 26.10-1)

	ΔΗ			(factored)	F _{des}	0.6*F _{des}		1
SECTION	(ft)	K _z	C_f	C _f	(psf)	(psf)	Shape	
1	130-140	1.08	1.11	1.44	45.63	27.38	Round	Antenna
2	120-130	1.05	1.10	1.43	43.96	26.38	Round	Antenna
3	110-120	1.03	0.85	0.86	25.89	15.53	Round	1
4	90-110	0.99	0.84	0.85	24.59	14.76	Round]
5	70-90	0.93	0.84	0.85	23.10	13.86	Round]
6	50-70	0.85	0.83	0.84	20.86	12.52	Round	
7	30-50	0.76	0.83	0.83	18.47	11.08	Round	

1.23

1.23

22.33

20.53

13.40

12.32

Octagon

Octagon

1.23

1.23

 $F_{des} = q*K_z*C_f$

8

9

 F_{des} < 16 psf, then use 16 psf for minimum wind pressure 27.1.5 and 28.3.4

0.62

0.57

 $0.6*F_{des}$ based on ASD Load Combination 2

10-30

0-10

Calculate $\rm K_z$ as mid-height elevation of section for exposure category using Table 26.10-1 Calculate $\rm C_f$ from Figure 29.4-1

Rough for standard brick, very rough at locations of equipment & antenna

30% increase in C_f at regions with antennas

Input Stack Profile Data:

Starting from top of stack and working downward, enter data for each stack section to be analyzed:

	100	1	(104.76			(8)			120	١
	104.76			109.68			10			120	
	109.68			114.48			10			120	
	114.58			124.08			12			240	
	124.08			133.68			14			240	
	133.68			143.4			16			240	
	143.4			153			18			240	
	169			169			24			240	
	169			169			26			120	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
TopOD :=	0	in	BtmOD :=	0	in	WallThk :=	0	in	SectHgt :=	0	in
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0			0			0			0	
	0		(0			0	J		0	1

Input Wind Load and Unit Weight Data:

Starting from top of stack and working downward, enter data for each stack section to be analyzed:

	(27.38)		(1)		(122)	
	26.38		1		122	
	15.38		1 1		120	
	14.61				120	
	13.72		1		120	
	12.39		1		120	
	11.08		1		120	
	13.40		1		125	
	12.32		1		125	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0	11.	0		0	11.
DesignWindLoad :=	0	$\frac{\text{lb}}{2}$ WindCoefficient :=	0	UnitWeight :=	0	1b
	0	ft ²	0		0	ft ³
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		0		0	
	0		$\left(0\right)$		0	

Calculate Total Number of Stack Sections:

NoSections = Total number of stack sections being analyzed

$$\sum_{N} \text{NoSections} = 9$$

$$\sum_{N} = \sum_{N} \text{NoSections}$$

N = 9 (**N** is used in calculations below)

Calculate Dead Loads at Bottom of Each Stack Section:

DeadLoad = Total dead load at bottom of each *individual* stack section *all by itself*

$$\begin{array}{c|c} \text{DeadLoad} \coloneqq & \text{DL}_1 \leftarrow \text{SectWgt}_1 \\ & \text{for } r \in 2 ... \, \text{N} \\ & & \text{Mp} \leftarrow \text{DL}_{r-1} + \text{SectWgt}_r \\ & & \text{DL}_r \leftarrow \text{Mp} \\ & & \text{DL} \end{array}$$

$$\begin{array}{c} \left(\begin{array}{c} 20.096 \\ 45.973 \\ 72.697 \\ 140.135 \\ 224.346 \\ 327.005 \\ 449.716 \\ 639.52 \\ 740.913 \end{array}\right) \text{ lb· } 1000 \\ \end{array}$$

Calculate Stress:

Fa = Axial load at bottom of each stack section. This includes all dead load above the bottom of the stack section, including the stack section itself plus all other stack sections above it.

$$Fa := \begin{cases} \text{for } r \in 1..N \\ Fa_r \leftarrow \frac{\text{DeadLoad}_r}{\text{Area}_r} \end{cases}$$

Fb = Bending stress due to wind at bottom of each stack section. This includes all wind load on the stack section itself plus the wind load on all stack sections above it.

$$Fb := \begin{cases} \text{for } r \in 1..N \\ Fb_r \leftarrow \frac{TotalSectionMoment_r}{SectionMod_r} \end{cases}$$

$$Fa = \begin{pmatrix} 8.264 \\ 14.681 \\ 22.148 \\ 33.165 \\ 42.621 \\ 51.064 \\ 58.909 \\ 58.496 \\ 63.432 \end{pmatrix} \cdot \frac{lb}{in^2}$$

$$Fb = \begin{pmatrix} 2.562 \\ 7.844 \\ 15.34 \\ 28.021 \\ 38.852 \\ 47.659 \\ 54.728 \\ 49.621 \\ 54.797 \end{pmatrix} \cdot \frac{lb}{in^2}$$

The weight of the antennas is negligible to the self weight of the chimney, therefore it is essentially no change to the seismic response of the structure due to this equipment.

Allowable stresses on the chimney using Code ACI 530-13/ASCE 5-13/TMS 402-13

Height of Chimney (h in feet)

140

f_m (psi) 1,000

	Wall Thk.	OD	ID	r		F _a	F _{bc}	f _a	f _{bc}	$(f_a/F_a) +$	f _{bt}	F _{bt}	
Sect.	(in)	(ft)	(ft)	(ft)	h/r	(psi)	(psi)	(psi)	(psi)	(f_{bc}/F_{bc})	(psi)	(psi)	f _{bt} / F _{bt}
1	8	8.73	7.40	2.86	48.94	219	333	8.26	2.56	0.045	-2.40	25	-0.10
2	10	9.14	7.47	2.95	47.43	221	333	14.68	7.84	0.090	-0.96	25	-0.04
3	10	9.54	7.87	3.09	45.27	224	333	22.15	15.34	0.145	2.05	25	0.08
4	12	10.34	8.34	3.32	42.16	227	333	33.17	28.02	0.230	8.12	25	0.32
5	14	11.14	8.81	3.55	39.43	230	333	42.62	38.85	0.302	13.28	25	0.53
6	16	11.95	9.28	3.78	37.01	233	333	51.06	47.66	0.363	17.02	25	0.68
7	18	12.75	9.75	4.01	34.89	234	333	58.91	54.73	0.416	19.38	25	0.78

For h/r < 99: $F_a = (1/4)f_m [1 - (h/140r)]$

For h/r > 99: $F_a = (1/4)f_m'(70r/h)^2$]

 $F_{bc} = (1/3)f_{m}$

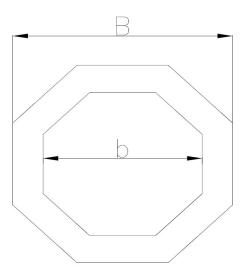
 f_{bt} = (+) compressive, (-) tensile = f_{bc} - 0.6* f_a

Octagonal Stresses

Chimney section	8

В	14.08	ft
b	10.08	ft
Unit Weight	125	pcf
Height	20	ft
Dead Load Above	449,716	lb
Moment	1,444,583	ft-lb

$$A_{out} = 0.83*B^{2}$$
 164.54 ft²
 $A_{in} = 0.83*b^{2}$ 84.33 ft²
 $A_{total} = A_{out} - A_{in}$ (ft²) 80.21 ft²



Moment of Inertia, $I = (A/12)*[B^2(1+2cos^2(22.5)/4cos^2(22.5)]$

l _{out}	2,156	ft ⁴
I _{in}	566	ft ⁴
$I_t = I_{out} - I_{in} (ft^4)$	1,589	ft ⁴

Section Modulus, S = I/c where c = B/2

S 226 ft³

Weight of Pedestal	200,528 lbs	(Unit Weight * Height * Area)
Total Dead Load	650,244 lbs	(Weight of Pedestal + Dead Load Above)

Axial,
$$f_a = DL / A$$
 56 psi
Bending, $f_b = M / S$ 44 psi
 $f_t = f_a - f_b$ 11.860 psi (+) tensile, (-) compression

Allowable tensile, F_a 40.0 psi

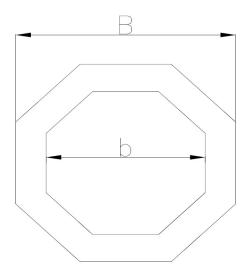
Ratio: f_t / F_a 0.297 PASS Must be < 1.0

Octagonal Stresses

Chimney section	
-----------------	--

В	14.08	ft
b	9.75	ft
Unit Weight	125	pcf
Height	10	ft
Dead Load Above	639,520	lb
Moment	1,666,801	ft-lk

$$A_{out} = 0.83*B^{2}$$
 164.54 ft²
 $A_{in} = 0.83*b^{2}$ 78.90 ft²
 $A_{total} = A_{out} - A_{in}$ (ft²) 85.64 ft²



Moment of Inertia, $I = (A/12)*[B^2(1+2cos^2(22.5)/4cos^2(22.5)]$

9

l _{out}	2,156	ft ⁴
I _{in}	496	ft ⁴
$I_{t} = I_{out} - I_{in} (ft^{4})$	1,660	ft ⁴

Section Modulus, S = I/c where c = B/2

S 236 ft³

Weight of Pedestal	107,053 lbs	(Unit Weight * Height * Area)
Total Dead Load	746,573 lbs	(Weight of Pedestal + Dead Load Above)

Axial,
$$f_a = DL / A$$

Bending, $f_b = M / S$
 $f_t = f_a - f_b$

psi

psi

psi

(+) tensile, (-) compression

Allowable tensile, F_a 40.0 psi

Ratio: f_t / F_a 0.286 PASS Must be < 1.0