STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL
Ten Franklin Square, New Britain, CT 06051
Phone: (860) 827-2935 Fax: (860) 827-2950
E-Mail: siting.council@ct.gov
Web Site: portal.ct.gov/csc

## VIA ELECTRONIC MAIL

March 3, 2023
Mark Roberts
SAI Group
12 Industrial Way
Salem, NH 03079
Mark.Roberts@ QCDevelopment.net
RE: EM-AT\&T-153-230214 - AT\&T notice of intent to modify an existing telecommunications facility located at 27 Siemon Company Drive, Watertown, Connecticut.

Dear Mark Roberts:
The Connecticut Siting Council (Council) is in receipt of your correspondence of March 2, 2023 submitted in response to the Council's February 28, 2023 notification of an incomplete request for exempt modification with regard to the above-referenced matter.

The submission renders the request for exempt modification complete and the Council will process the request in accordance with the Federal Communications Commission 60-day timeframe.

Thank you for your attention and cooperation.

Sincerely,


Melanie Bachman
Executive Director
MAB/ANM/laf

From: Mark Roberts <mark.roberts@ qcdevelopment.net>
Sent: Thursday, March 2, 2023 9:09 AM
To: Fontaine, Lisa [Lisa.Fontaine@ct.gov](mailto:Lisa.Fontaine@ct.gov)
Cc: CSC-DL Siting Council [Siting.Council@ct.gov](mailto:Siting.Council@ct.gov)
Subject: RE: Council Incomplete - EM-AT\&T-153-230214 (Siemon Company Drive) - Watertown
Importance: High
Hello - In response to your attached incompleteness letter, please find attached the corrected Structural Analysis. A hard copy will follow by mail.

Thanks
Mark Roberts
QC Development
860-670-9068

# Chimney Design Calculations by ICC Commonwealth <br> 795 Wurlitzer Drive, North Tonawanda, NY 14120 

Customer: TEP Northeast<br>ICC Project Number: 2248<br>Site: 76 Westbury Park Road | Watertown, CT 06795<br>Chimney Description: 140' Radial Brick Chimney

## Summary:

The following is a structural analysis of a $140^{\prime}$ radial brick chimney at site mentioned in title above. With the proposed AT\&T cellular equipment modifications at the $135^{\prime}$ elevation, it was found that the chimney shell is not overstressed. This analysis assumes the brick chimney is in good condition with sound brick and mortar throughout its height. Therefore, the chimney must have the repairs noted below to make the chimney usable for AT\&T to install their proposed equipment. This analysis assumes all repairs required from list below have been completed and all antenna mounts have been designed by others. If repairs are ignored, this chimney will likely be overstressed structurally which may lead to further damage and possible chimney failure. The existing foundation was not analyzed and therefore is not a design responsibility of ICC Commonwealth.

Repairs required:

1) Rake out and point all loose and open mortar joints on the exterior of radial section of the chimney column. Approximately $20 \%$ to $25 \%$ is required.
2) Replace spalled brick faces in the radial section of the chimney. Approximately (12) to (15) are required.
3) Rake out and point all loose and open mortar joints throughout the common brick pedestal. Approximately $10 \%$ to $15 \%$ is required.
4) Remove and replace loose brickwork in the corbel section of the pedestal on the East side.
5) Partial removal and replace the deteriorated sections of the cement water table at the top of the pedestal.
6) Cut out and point the $8^{\prime}$ to $9^{\prime}$ vertical crack found on the South side of the pedestal.
7) Cut out and point the vertical crack on the West side of the brick pedestal.
8) Remove loose and deteriorated spalls withing exterior of the foundation and repair with Sika Top 123 Plus. Repair remaining cracks with Sika Flex 11FC.
9) Repair and replace (2) broken downlead anchors and reattached lightning protection downlead on the East side.
10) Install new self-taping screws to hold down the roof access panel on the side lifting.

## Analysis Results

Approved - Structure can accommodate the proposed changes. No repairs required.

Conditional Approval - Structure can accommodate the proposed changes. Repairs required.

Not Approved - Structure cannot accommodate the proposed changes without reinforcement.

All repairs should be supervised under a qualified and experienced professional. If repairs are required and not performed and supervised by a licensed professional engineer, additional inspection is required.












## Search Information

| Address: | 76 Westbury Park Rd, Watertown, CT 06795, <br> USA |
| :--- | :--- |
| Coordinates: | $41.6037641,-73.1117191$ |
| Elevation: | 482 ft |
| Timestamp: | $2022-06-03 T 19: 20: 02.757 \mathrm{Z}$ |
| Hazard Type: | Wind |



| ASCE 7-16 |  | ASCE 7-10 |  | ASCE 7-05 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| MRI 10-Year | 75 mph | MRI 10-Year | 76 mph | ASCE 7-05 Wind Speed | 98 mph |
| MRI 25-Year | 83 mph | MRI 25-Year | 85 mph |  |  |
| MRI 50-Year | 89 mph | MRI 50-Year | 91 mph |  |  |
| MRI 100-Year | 96 mph | MRI 100-Year | 97 mph |  |  |
| Risk Category I | 106 mph | Risk Category I | 108 mph |  |  |
| Risk Category II | 116 mph | Risk Category II | 119 mph |  |  |
| Risk Category III | 125 mph | Risk Category III-IV | 127 mph |  |  |
| Risk Category IV | 129 mph |  |  |  |  |

You are in a wind-borne debris region if you are also within 1 mile of the coastal
mean high water line.

The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

## Disclaimer

Hazard loads are interpolated from data provided in ASCE 7 and rounded up to the nearest whole integer. Per ASCE 7, islands and coastal areas outside the last contour should use the last wind speed contour of the coastal area - in some cases, this website will extrapolate past the last wind speed contour and therefore, provide a wind speed that is slightly higher. NOTE: For queries near wind-borne debris region boundaries, the resulting determination is sensitive to rounding which may affect whether or not it is considered to be within a wind-borne debris region.

Mountainous terrain, gorges, ocean promontories, and special wind regions shall be examined for unusual wind conditions.
While the information presented on this website is believed to be correct, ATC and its sponsors and contributors assume no responsibility or liability for its accuracy. The material presented in the report should not be used or relied upon for any specific application without competent examination and verification of its accuracy, suitability and applicability by engineers or other licensed professionals. ATC does

Height of chimney, h Define risk category Define exposure factor Basic wind speed, V

| 140 |
| :---: |
| II |
| Bt |
| 116 |
| mph |


| 0.85 |
| :---: |
| 1.0 |
| 1.0 |
| 1.0 |

Wind pressure, q

Table 1.5-1
Section 26.7.3
Attached Sheet, ASCE 7-16 wind properties

Section 26.11.1
Section 26.8.2
Round chimney, Table 26.6-1
Section 26.9
$q=0.00256 K_{z t} K_{d} K_{e} G V^{2}$ (Eq. 26.10-1)

| SECTION | $\Delta H$ <br> (ft) | $\mathrm{K}_{\mathrm{z}}$ | $\mathrm{C}_{\mathrm{f}}$ | $\begin{gathered} \hline \text { (factored) } \\ C_{f} \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{F}_{\mathrm{des}} \\ (\mathrm{psf}) \end{gathered}$ | $0.6 * F_{\text {des }}$ (psf) | Shape |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 130-140 | 1.08 | 1.11 | 1.44 | 45.63 | 27.38 | Round |
| 2 | 120-130 | 1.05 | 1.10 | 1.43 | 43.96 | 26.38 | Round |
| 3 | 110-120 | 1.03 | 0.85 | 0.86 | 25.89 | 15.53 | Round |
| 4 | 90-110 | 0.99 | 0.84 | 0.85 | 24.59 | 14.76 | Round |
| 5 | 70-90 | 0.93 | 0.84 | 0.85 | 23.10 | 13.86 | Round |
| 6 | 50-70 | 0.85 | 0.83 | 0.84 | 20.86 | 12.52 | Round |
| 7 | 30-50 | 0.76 | 0.83 | 0.83 | 18.47 | 11.08 | Round |
| 8 | 10-30 | 0.62 | 1.23 | 1.23 | 22.33 | 13.40 | Octagon |
| 9 | 0-10 | 0.57 | 1.23 | 1.23 | 20.53 | 12.32 | Octagon |

$\mathrm{F}_{\text {des }}=\mathrm{q}^{*} \mathrm{~K}_{\mathrm{z}}{ }^{*} \mathrm{C}_{\mathrm{f}}$
$\mathrm{F}_{\text {des }}<16 \mathrm{psf}$, then use 16 psf for minimum wind pressure 27.1.5 and 28.3.4
$0.6^{*} \mathrm{~F}_{\text {des }}$ based on ASD Load Combination 2

Calculate $\mathrm{K}_{\mathrm{z}}$ as mid-height elevation of section for exposure category using Table 26.10-1
Calculate C $_{f}$ from Figure 29.4-1
Rough for standard brick, very rough at locations of equipment \& antenna
$30 \%$ increase in $C_{f}$ at regions with antennas

## Input Stack Profile Data:

Starting from top of stack and working downward, enter data for each stack section to be analyzed:

$\left(\begin{array}{c}8 \\ 10 \\ 10 \\ 12 \\ 14 \\ 16 \\ 18 \\ 24 \\ 26 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0\end{array}\right)$ in
$\underbrace{0}_{0}$

## Input Wind Load and Unit Weight Data:

Starting from top of stack and working downward, enter data for each stack section to be analyzed:


## Calculate Total Number of Stack Sections:

NoSections $=$ Total number of stack sections
being analyzed

$$
\text { NoSections }:=\left\{\begin{array}{l}
\mathrm{Mp} \leftarrow 0 \\
\text { for } \mathrm{r} \in 1 . .35 \\
\begin{array}{l}
\mathrm{Mp}_{\mathrm{r}} \leftarrow 1 \text { if } \operatorname{SectHgt}_{\mathrm{r}}>0 \\
\mathrm{Mp}_{\mathrm{r}} \leftarrow 0 \text { if } \text { SectHgt }_{\mathrm{r}} \leq 0
\end{array}
\end{array}\right.
$$

$$
\sum \text { NoSections }=9
$$

$$
\underset{\sim}{N}:=\sum \text { NoSections }
$$

$$
\mathrm{N}=9 \quad(\mathbf{N} \text { is used in calculations below })
$$



## Calculate Stress:

Fa = Axial load at bottom of each stack section. This includes all dead load above the bottom of the stack section, including the stack section itself plus all other stack sections above it.

$$
\mathrm{Fa}:=\left\lvert\, \begin{aligned}
& \text { for } \mathrm{r} \in 1 . . \mathrm{N} \\
& \mathrm{Fa}_{\mathrm{r}} \leftarrow \frac{\text { DeadLoad }_{\mathrm{r}}}{\text { Area }_{\mathrm{r}}} \\
& \mathrm{Fa}
\end{aligned}\right.
$$

$\mathrm{Fb}=$ Bending stress due to wind at bottom of each stack section. This includes all wind load on the stack section itself plus the wind load on all stack sections above it.

$\mathrm{Fa}=\left(\begin{array}{c}8.264 \\ 14.681 \\ 22.148 \\ 33.165 \\ 42.621 \\ 51.064 \\ 58.909 \\ 58.496 \\ 63.432\end{array}\right) \cdot \frac{\mathrm{lb}}{\mathrm{in}^{2}} \quad \mathrm{Fb}=\left(\begin{array}{c}2.562 \\ 7.844 \\ 15.34 \\ 28.021 \\ 38.852 \\ 47.659 \\ 54.728 \\ 49.621 \\ 54.797\end{array}\right) \cdot \frac{\mathrm{lb}}{\mathrm{in}^{2}}$

The weight of the antennas is negligible to the self weight of the chimney, therefore it is essentially no change to the seismic response of the structure due to this equipment.

Allowable stresses on the chimney using Code ACI 530-13/ASCE 5-13/TMS 402-13

| Height of Chimney (h in feet) | 140 | $\mathrm{f}_{\mathrm{m}}{ }^{\prime}(\mathrm{psi}) 1,000$ |
| :--- | :--- | :--- |


| Sect. | Wall Thk. <br> (in) | $\begin{aligned} & \mathrm{OD} \\ & (\mathrm{ft}) \\ & \hline \end{aligned}$ | $\begin{gathered} \text { ID } \\ \text { (ft) } \end{gathered}$ | (ft) | h/r | $\begin{gathered} \mathrm{F}_{\mathrm{a}} \\ (\mathrm{psi}) \end{gathered}$ | $\begin{gathered} \hline \mathrm{F}_{\mathrm{bc}} \\ (\mathrm{psi}) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathrm{f}_{\mathrm{a}} \\ (\mathrm{psi}) \end{gathered}$ | $\begin{gathered} \hline \mathrm{f}_{\mathrm{bc}} \\ (\mathrm{psi}) \end{gathered}$ | $\begin{aligned} & \left(\mathrm{f}_{\mathrm{a}} / \mathrm{F}_{\mathrm{a}}\right)+ \\ & \left(\mathrm{f}_{\mathrm{bc}} / \mathrm{F}_{\mathrm{bc}}\right) \end{aligned}$ | $\begin{gathered} \hline \mathrm{f}_{\mathrm{bt}} \\ (\mathrm{psi}) \end{gathered}$ | $\begin{gathered} \mathrm{F}_{\mathrm{bt}} \\ (\mathrm{psi}) \end{gathered}$ | $\mathrm{f}_{\mathrm{bt}} / \mathrm{F}_{\mathrm{bt}}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 8 | 8.73 | 7.40 | 2.86 | 48.94 | 219 | 333 | 8.26 | 2.56 | 0.045 | -2.40 | 25 | -0.10 |
| 2 | 10 | 9.14 | 7.47 | 2.95 | 47.43 | 221 | 333 | 14.68 | 7.84 | 0.090 | -0.96 | 25 | -0.04 |
| 3 | 10 | 9.54 | 7.87 | 3.09 | 45.27 | 224 | 333 | 22.15 | 15.34 | 0.145 | 2.05 | 25 | 0.08 |
| 4 | 12 | 10.34 | 8.34 | 3.32 | 42.16 | 227 | 333 | 33.17 | 28.02 | 0.230 | 8.12 | 25 | 0.32 |
| 5 | 14 | 11.14 | 8.81 | 3.55 | 39.43 | 230 | 333 | 42.62 | 38.85 | 0.302 | 13.28 | 25 | 0.53 |
| 6 | 16 | 11.95 | 9.28 | 3.78 | 37.01 | 233 | 333 | 51.06 | 47.66 | 0.363 | 17.02 | 25 | 0.68 |
| 7 | 18 | 12.75 | 9.75 | 4.01 | 34.89 | 234 | 333 | 58.91 | 54.73 | 0.416 | 19.38 | 25 | 0.78 |

For $h / r<99: F_{a}=(1 / 4) f_{m}{ }^{\prime}[1-(h / 140 r$.
For $\left.h / r>99: F_{a}=(1 / 4) f_{m}{ }^{\prime}(70 r / h)^{2}\right]$
$F_{b c}=(1 / 3) f_{m}$
$f_{b t}=(+)$ compressive, $(-)$ tensile $=f_{b c}-0.6 * f_{a}$

## Octagonal Stresses

Chimney section


B
b
Unit Weight
Height
Dead Load Above
Moment

| 14.08 | ft |
| ---: | :--- |
| 10.08 | ft |
| 125 | pcf |
| 20 | ft |
| 449,716 | lb |
| $1,444,583$ | $\mathrm{ft}-\mathrm{lb}$ |

$\mathrm{A}_{\text {out }}=0.83 * \mathrm{~B}^{2}$
$\mathrm{A}_{\mathrm{in}}=0.83 * \mathrm{~b}^{2}$
$A_{\text {total }}=A_{\text {out }}-A_{\text {in }}\left(\mathrm{ft}^{2}\right)$

| 164.54 |
| ---: |
| 84.33 |
| $\mathrm{ft}^{2}$ |
| 80.21 |
| $\mathrm{ft}^{2}$ |



Moment of Inertia, $\mathrm{I}=(\mathrm{A} / 12) *\left[\mathrm{~B}^{2}\left(1+2 \cos ^{2}(22.5) / 4 \cos ^{2}(22.5)\right]\right.$

| $I_{\text {out }}$ <br> $I_{\text {in }}$ <br> $\mathrm{ft}^{4}$ <br> $\mathrm{I}_{\mathrm{t}}=I_{\text {out }}-I_{\text {in }}\left(\mathrm{ft}^{4}\right)$$\quad 2,156$ |
| :--- | ---: |
| $\mathrm{ft}^{4}$ |

Section Modulus, $S=I / c$ where $c=B / 2$
S


Weight of Pedestal Total Dead Load

| 200,528 | lbs |
| ---: | :--- |
|  | 650,244 |
| Ibs |  |

(Unit Weight * Height * Area)
(Weight of Pedestal + Dead Load Above)

Axial, $f_{a}=D L / A$
Bending, $f_{b}=M / S$
$f_{t}=f_{a}-f_{b}$

| 56 | psi |
| ---: | ---: |
| 44 | psi |
| 11.860 | psi |

(+) tensile, (-) compression

Allowable tensile, $\mathrm{F}_{\mathrm{a}}$ 40.0 psi

Ratio: $f_{t} / F_{a}$
0.297

PASS Must be < 1.0

## Octagonal Stresses

Chimney section


B
b
Unit Weight
Height
Dead Load Above
Moment

| 14.08 | ft |
| ---: | :--- |
| 9.75 | ft |
| 125 | pcf |
| 10 | ft |
| 639,520 | lb |
| $1,666,801$ | $\mathrm{ft}-\mathrm{lb}$ |

$\mathrm{A}_{\text {out }}=0.83 * \mathrm{~B}^{2}$
$\mathrm{A}_{\mathrm{in}}=0.83 * \mathrm{~b}^{2}$
$A_{\text {total }}=A_{\text {out }}-A_{\text {in }}\left(\mathrm{ft}^{2}\right)$

| 164.54 |
| ---: |
| 78.90 |
| 85.64 |
| $\mathrm{ft}^{2}$ |
| $\mathrm{ft}^{2}$ |



Moment of Inertia, $\mathrm{I}=(\mathrm{A} / 12) *\left[\mathrm{~B}^{2}\left(1+2 \cos ^{2}(22.5) / 4 \cos ^{2}(22.5)\right]\right.$

| $I_{\text {out }}$ <br> $I_{\text {in }}$ <br> $\mathrm{ft}^{4}$ <br> $I_{\mathrm{t}}=I_{\text {out }}-I_{\text {in }}\left(\mathrm{ft}^{4}\right)$ | 2,156 <br> $\mathrm{ft}^{4}$ |
| :--- | ---: |

Section Modulus, $\mathrm{S}=\mathrm{I} / \mathrm{c}$ where $\mathrm{c}=\mathrm{B} / 2$
S


Weight of Pedestal Total Dead Load

| 107,053 | lbs |
| ---: | :--- |
| 746,573 | lbs |

(Unit Weight * Height * Area)
(Weight of Pedestal + Dead Load Above)

Axial, $f_{a}=D L / A$
Bending, $f_{b}=M / S$
$f_{t}=f_{a}-f_{b}$

| 61 | psi |
| ---: | ---: |
| 49 | psi |
| 11.445 | psi |

(+) tensile, (-) compression

Allowable tensile, $\mathrm{F}_{\mathrm{a}}$ 40.0 psi

Ratio: $f_{t} / F_{a}$
0.286

PASS Must be < 1.0

