



Crown Castle  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065

October 19, 2021

Melanie A. Bachman  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

RE: **Notice of Exempt Modification for T-Mobile: CT11381C**  
**Crown Site ID#876338**  
**41 Manitock Hill Road, Waterford, CT 06385**  
**Latitude: 41° 21' 16.70" / Longitude: -72° 09' 1.60"**

Dear Ms. Bachman:

T-Mobile currently maintains six (6) antennas at the 117-foot mount level on the existing 136-foot self-support tower, located at 41 Manitock Hill Road, Waterford, CT. The property is owned by The City of New London. The tower is owned by Crown Castle. T-Mobile now intends to replace six (6) antennas and ancillary equipment at the 117-ft level. T-Mobile to also to replace the antenna mount at the same level. This modification/proposal includes hardware that is both 4G (LTE) and 5G capable through remote software configuration and either or both services may be turned on or off at various times.

Panned Modification:  
Tower:

Installed New:

- (3) RFS/Celwave – APXVAARR24\_43-U-NA20
- (3) Ericsson – AIR6449 B41 Antenna
- (3) Ericsson-Radio 4449 B71/B85
- (3) Ericsson Radio 4460 B25+B66
- (3) HYBRID 6x12 HCS Hybrid Cable
- (1) 6/24 4AWG Hybrid Cable
- (1) Sitepro1 Antenna Mount Per Trylon MA 8/24/2021

Remove:

- (3) Ericsson-Air 21 B4A B2P
- (3) Ericsson Air 21 B2A B4P Antennas
- (3) Generic Twin TMAs Style 1B
- (12) 1 5/8" Coaxial Cables
- (1) 1-1/4" Coax Cable
- (1) Existing antenna mount

The Foundation for a Wireless World.

CrownCastle.com

Ground:

Install New:

- (1) Emerson AAV Cabinet
- (1) RBS 6160 Cabinet
- (1) B160 battery Cabinet
- (1) BB 6648
- (1) RBS 6601
- (1) CSR IXRE V2 (GEN 2)
- (2) PSU 4813 Voltage Booster

Remove:

- (1) RBS 6131 Cabinet

The facility was approved by the Town of Waterford Planning & Zoning on September 29, 1997 via grant of Special Permit#97-112/304.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Mr. Rob Brule, First Selectman, Town of Waterford, Ms. Abby Piersall, Planning Director, Town of Waterford and the property owner, City of New London, CT. Crown Castle is the tower owner.

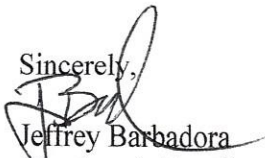
1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Jeffrey Barbadora.

Melanie A. Bachman

Page 3

Sincerely,



Jeffrey Barbadora  
Site Acquisition Specialist  
1800 W. Park Drive  
Westborough, MA 01581  
(781) 970-0053  
Jeff.Barbadora@crowncastle.com

Attachments

cc:

First Selectman, Mr. Rob Brule  
15 Rope Ferry Road  
Waterford, CT 06438  
(860)-444-5834

Town Planner, Ms Abby Piersall,  
15 Rope Ferry Road  
Waterford, CT 06438  
(860)-444-5813

Property Owner  
City of New London  
181 State Street, Treasurer's Office  
New London, CT 06320

Crown Castle, Tower Owner.

NOTICE OF GRANT OF A SPECIAL PERMIT

This is to certify that on September 29, 1997 the Waterford Planning & Zoning Commission granted Special Permit #97-112/304.

Owner of Record: City of New London

Address: 41 Manitock Hill Road

Description of Premises:

As recorded in Volume 173, Page 256 of the Waterford Land Records.

Nature of Special Permit: Special Permit granted for the construction of a 140 foot lattice design communications tower by Sprint. Co-location for additional carriers is provided for on this tower.

Applicable Zoning Regulations: Section 3.6, 5.2.1, 5.2.3 and 23.

Permit findings, stipulations and conditions are filed in the office of the Town Clerk as stated in the minutes of the Planning & Zoning Commission meeting of September 29, 1997.

PLANNING & ZONING COMMISSION

By: Pam Hagerman  
Pamela Hagerman  
Recording Secretary  
Planning & Zoning Commission

This notice is to be recorded on the land records of the Town of Waterford, indexed in the Grantor's Index under the name of the record owner.

RECEIVED FOR RECORD March 16, 1999  
4:02 P.M. ATTEST Pamela Hagerman  
TOWN CLERK

# 41 MANITOCK HILL ROAD

**Location** 41 MANITOCK HILL ROAD

**Mblu** 117 / 4375 / 1

**Acct#** 00395700

**Owner** NEW LONDON CITY OF

**Assessment** \$9,180

**Appraisal** \$13,110

**PID** 4375

**Building Count** 1

## Current Value

Appraisal			
Valuation Year	Improvements	Land	Total
2017	\$2,250	\$10,860	\$13,110

Assessment			
Valuation Year	Improvements	Land	Total
2017	\$1,580	\$7,600	\$9,180

## Parcel Addresses

Additional Addresses
No Additional Addresses available for this parcel

## Owner of Record

**Owner** NEW LONDON CITY OF  
**Co-Owner** WATER DEPT

**Sale Price** \$0  
**Certificate**  
**Book & Page** 0173/0256  
**Sale Date** 06/11/1968  
**Instrument** 00

## Ownership History

Ownership History					
Owner	Sale Price	Certificate	Book & Page	Instrument	Sale Date
NEW LONDON CITY OF	\$0		0173/0256	00	06/11/1968

## Building Information

**Building 1 : Section 1**

**Year Built:**

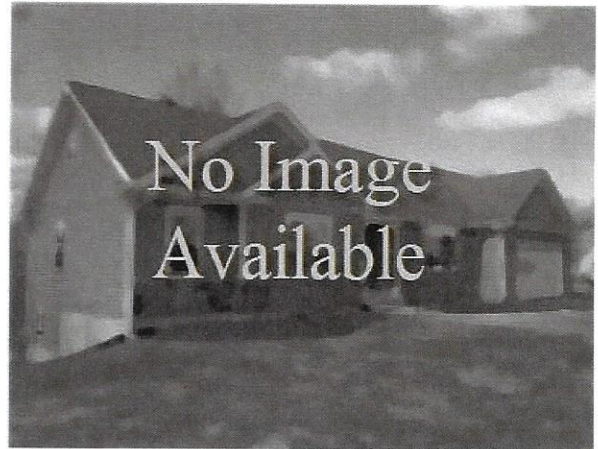
**Living Area:** 0

**Replacement Cost:** \$0

**Building Percent Good:**


Building Attributes	
Field	Description
Style	Outbuildings
Model	
Grade:	
Stories	
Occupancy	
Exterior Wall 1	
Exterior Wall 2	
Roof Structure	
Roof Cover	
Interior Wall 1	
Interior Wall 2	
Interior Flr 1	
Interior Flr 2	
Heat Fuel	
Heat Type:	
AC Percent	
Total Bedrooms:	
Full Bthrms:	
Half Baths:	
Extra Fixtures	
Total Rooms:	
Bath Style:	
Kitchen Style:	
Num Kitchens	
Fireplace(s)	
Extra Opening(s)	
Gas Fireplace(s)	
% Attic Fin	
LF Dormer	
Foundation	
Bsmt Gar(s)	
Bsmt %	
SF FBM	
SF Rec Rm	

**Building Photo**



(<http://images.vgsi.com/photos/WaterfordCTPhotos//default.jpg>)

**Building Layout**

 Building Layout

([http://images.vgsi.com/photos/WaterfordCTPhotos//Sketches/4375\\_4375.j](http://images.vgsi.com/photos/WaterfordCTPhotos//Sketches/4375_4375.j))

Building Sub-Areas (sq ft)	Legend
No Data for Building Sub-Areas	

Fin Bsmt Qual	
Bsmt Access	
Usrflid 300	
Usrflid 301	

**Extra Features**

Extra Features	<u>Legend</u>
No Data for Extra Features	

**Land**

**Land Use**

Use Code 109  
 Description Vacant W/ OB  
 Zone R-40  
 Neighborhood 600  
 Alt Land Appr No  
 Category

**Land Line Valuation**

Size (Acres) 5.5  
 Frontage 0  
 Depth 0  
 Assessed Value \$7,600  
 Appraised Value \$10,860

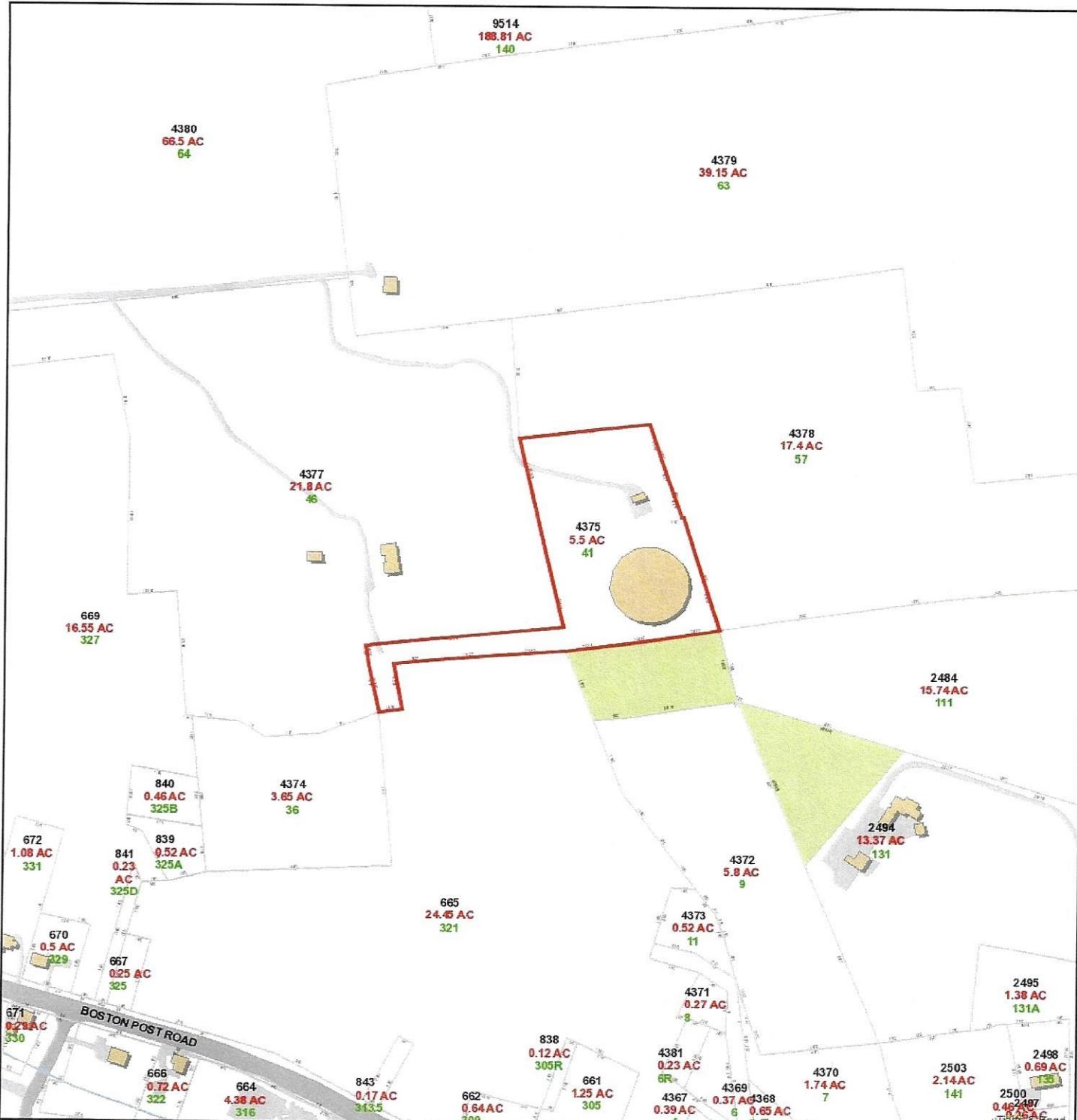
**Outbuildings**

Outbuildings						<u>Legend</u>
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
SHD1	Shed	FR	Frame	200.00 S.F.	\$2,250	1

**Valuation History**

Appraisal			
Valuation Year	Improvements	Land	Total
2020	\$2,250	\$10,860	\$13,110
4000	\$2,250	\$10,860	\$13,110

Assessment			
Valuation Year	Improvements	Land	Total
2020	\$1,580	\$7,600	\$9,180
4000	\$1,580	\$7,600	\$9,180

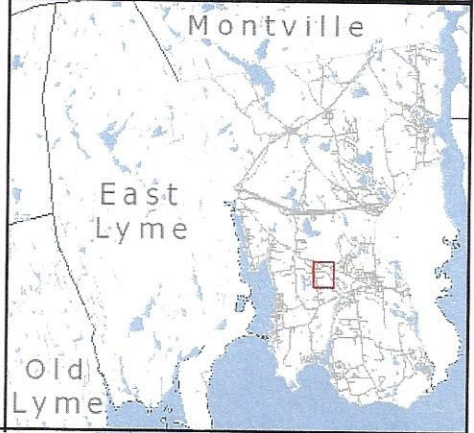
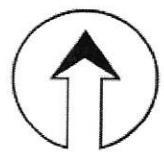


# 41 MANITOCK HILL

3/16/2021 3:40:44  
 1"=333'

## Property Information

AV PID	undefined
Street Address	undefined
Total Appraised Parcel	undefined



The information depicted on this map is for planning purposes only. It is not adequate for legal boundary definition, regulatory interpretation, or parcel-level analyses.



**Barbadora, Jeff**

---

**From:** TrackingUpdates@fedex.com  
**Sent:** Wednesday, October 20, 2021 3:18 PM  
**To:** Barbadora, Jeff  
**Subject:** FedEx Shipment 285104441839: Your package has been delivered

**CAUTION:** This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.



Hi. Your package was  
delivered Wed, 10/20/2021 at  
3:15pm.

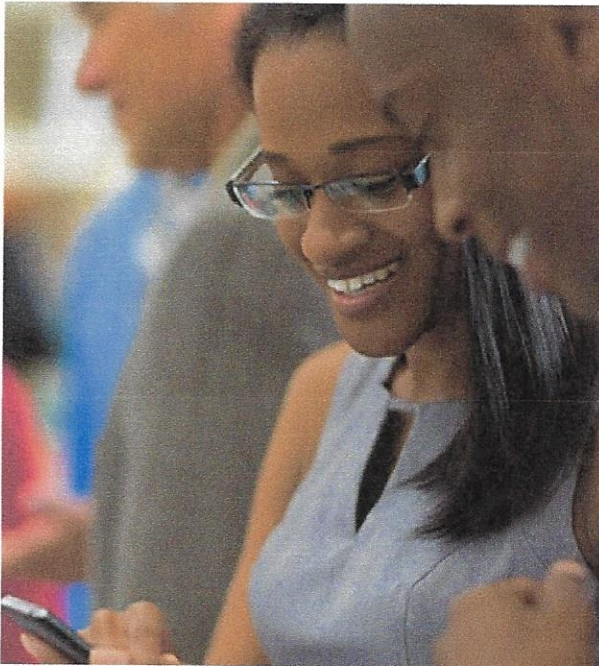


Delivered to 15 ROPE FERRY RD, WATERFORD, CT 06385

**OBTAIN PROOF OF DELIVERY**

**TRACKING NUMBER**    [285104441839](#)  
**FROM**    Jeff Barbadora  
             1800 W. Park Drive  
             WESTBOROUGH, MA, US, 01581

<b>TO</b>	Town of Waterford First Selectman Rob Brule 15 Rope Ferry Road WATERFORD, CT, US, 06385
<b>REFERENCE</b>	799001.7680
<b>SHIPPER REFERENCE</b>	799001.7680
<b>SHIP DATE</b>	Tue 10/19/2021 06:17 PM
<b>PACKAGING TYPE</b>	FedEx Envelope
<b>ORIGIN</b>	WESTBOROUGH, MA, US, 01581
<b>DESTINATION</b>	WATERFORD, CT, US, 06385
<b>SPECIAL HANDLING</b>	Deliver Weekday
<b>NUMBER OF PIECES</b>	1
<b>TOTAL SHIPMENT WEIGHT</b>	1.00 LB
<b>SERVICE TYPE</b>	FedEx Priority Overnight



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Hi. Your package was  
delivered Wed, 10/20/2021 at  
3:15pm.



Delivered to 15 ROPE FERRY RD, WATERFORD, CT 06385

**OBTAIN PROOF OF DELIVERY**

**TRACKING NUMBER** [285104532550](#)

**FROM** Jeff Barbadora  
1800 W. Park Drive  
WESTBOROUGH, MA, US, 01581

**TO** Town of Waterford  
Town Planner Abby Piersall  
15 Rope Ferry Road  
WATERFORD, CT, US, 06385

**REFERENCE** 799001.7680

**SHIPPER REFERENCE** 799001.7680

**SHIP DATE** Tue 10/19/2021 06:17 PM

**PACKAGING TYPE** FedEx Envelope

**ORIGIN** WESTBOROUGH, MA, US, 01581

**DESTINATION** WATERFORD, CT, US, 06385

**SPECIAL HANDLING** Deliver Weekday

**NUMBER OF PIECES** 1

**TOTAL SHIPMENT WEIGHT** 1.00 LB

**SERVICE TYPE** FedEx Priority Overnight



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Hi. Your package was  
delivered Wed, 10/20/2021 at  
11:00am.



Delivered to 181 STATE ST, NEW LONDON, CT 06320

**OBTAIN PROOF OF DELIVERY**

**TRACKING NUMBER** [285104234187](#)

**FROM** Jeff Barbadora  
1800 W. Park Drive  
WESTBOROUGH, MA, US, 01581

**TO** City of New London  
Treasurer's Office  
181 State Street  
NEW LONDON, CT, US, 06320

**REFERENCE** 799001.7680

**SHIPPER REFERENCE** 799001.7680

**SHIP DATE** Tue 10/19/2021 06:17 PM

**PACKAGING TYPE** FedEx Envelope

**ORIGIN** WESTBOROUGH, MA, US, 01581

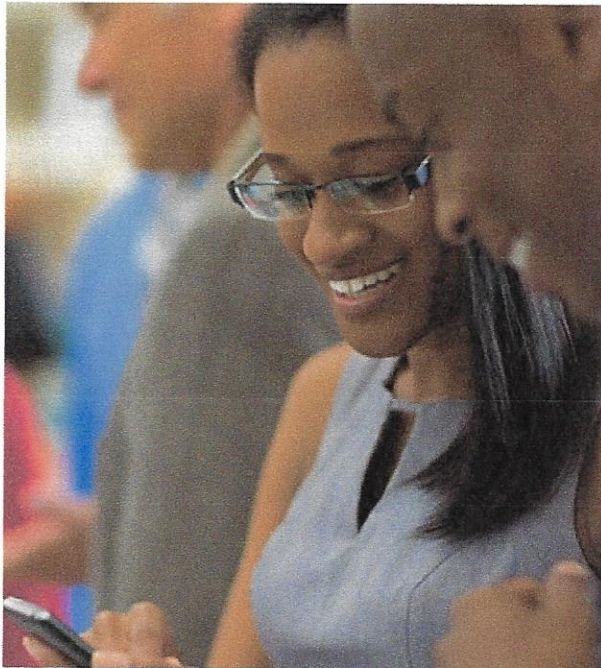
**DESTINATION** NEW LONDON, CT, US, 06320

**SPECIAL HANDLING** Deliver Weekday

**NUMBER OF PIECES** 1

**TOTAL SHIPMENT WEIGHT** 1.00 LB

**SERVICE TYPE** FedEx Priority Overnight



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MORRISON HERSHFIELD

Morrison Hershfield  
1455 Lincoln Parkway, Suite 500  
Atlanta, GA 30346  
(770) 379-8500

Date: **August 31, 2021**

**Subject:** **Structural Analysis Report**

**Carrier Designation:** **T-Mobile Co-Locate**  
**Site Number:** CT11381C  
**Site Name:** Waterford South/RT 1

**Crown Castle Designation:** **BU Number:** 876338  
**Site Name:** Waterford  
**JDE Job Number:** 675143  
**Work Order Number:** 2011627  
**Order Number:** 576542 Rev. 0

**Engineering Firm Designation:** **Morrison Hershfield Project Number:** CN8-185R1 / 2101398

**Site Data:** **41 Manitock Hill Road, Waterford, New London County,**  
**CT 06385-2000**  
**Latitude 41° 21' 16.7", Longitude -72° 9' 1.6"**  
**136 Foot – PiROD Self Support Tower**

Morrison Hershfield is pleased to submit this “**Structural Analysis Report**” to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Proposed Equipment Configuration **Sufficient Capacity – 74.6%**

This analysis utilizes an ultimate 3-second gust wind speed of 135 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 – Analysis Criteria.

Respectfully submitted by:

G. Lance Cooke, P.E. (CT License No. 28133)  
Senior Engineer



Digitally signed by  
G. Lance Cooke  
Date: 2021.08.31  
11:32:24-07'00'

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## 1) INTRODUCTION

This tower is a 136 ft Self Support tower designed by PiROD Manufactures, Inc.

The tower has been modified per reinforcement drawings prepared by Vertical Structures, Inc. in January of 2009. Reinforcement consists of secondary horizontal members between the elevations 90' and 95' and 1-1/4" tie-rod assemblies for tower legs from 80' to 90'.

## 2) ANALYSIS CRITERIA

<b>TIA-222 Revision:</b>	TIA-222-H
<b>Risk Category:</b>	II
<b>Wind Speed:</b>	135 mph
<b>Exposure Category:</b>	B
<b>Topographic Factor:</b>	1
<b>Ice Thickness:</b>	1.5 in
<b>Wind Speed with Ice:</b>	50 mph
<b>Service Wind Speed:</b>	60 mph

**Table 1 - Proposed Equipment Configuration**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
117.0	119.0	3	ericsson	AIR6449 B41_T-MOBILE	4	1-5/8
		3	rfs celwave	APXVAALL24_43-U-NA20_TMO		
		3	ericsson	RADIO 4449 B71 B85A_T-MOBILE		
		3	ericsson	RADIO 4460 B2/B25 B66_TMO		
	117.0	9	site pro 1	8' Antenna Pipe [#P2STD]		
		3	site pro 1	12' HD V-Frame [#VFA12-HD]		

**Table 2 - Other Considered Equipment**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
136.0	137.0	3	rfs celwave	APXVSP18-C-A20 w/ Mount Pipe	4	1-1/4
		3	rfs celwave	APXVTM14-C-120 w/ Mount Pipe		
		3	alcatel lucent	TD-RRH8X20-25		
	136.0	1	-	Platform Mount [LP 405-1]		
134.0	136.0	3	rfs celwave	IBC1900BB-1	-	-
		3	rfs celwave	IBC1900HG-2A		
	134.0	3	alcatel lucent	1900MHZ RRH (65MHZ)		
		3	alcatel lucent	800MHZ 2X50W RRH W/FILTER		
		1	-	Pipe Mount [PM 601-3]		
127.0	127.0	12	decibel	DB844H90E-XY w/ Mount Pipe	12	1-1/4
		1	-	Sector Mount [SM 411-3]		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
107.0	107.0	3	antel	BXA-80063/4CF w/ Mount Pipe	14	1-5/8
		6	jma wireless	MX06FRO660-03 w/ Mount Pipe		
		3	vzw	Sub6 Antenna - VZS01 w/ Mount Pipe		
		1	commscope	RC2DC-3315-PF-48		
		3	samsung	RFV01U-D1A		
		3	samsung	RFV01U-D2A		
		12	site pro 1	Crossover Plate [#VZWSMART-MSK1]		
		3	site pro 1	Tieback Assembly [#VZWSMART-SFK1]		
		3	site pro 1	V-Bracing Kit [#VZWSMART-SFK3]		
		1	-	Sector Mount [SM 402-3]		
97.0	97.0	1	cci antennas	DMP65R-BU4D w/ Mount Pipe	6 4 2	1-1/4 7/8 3/8
		1	cci antennas	DMP65R-BU6D w/ Mount Pipe		
		1	cci antennas	DMP65R-BU8D w/ Mount Pipe		
		1	cci antennas	OPA65R-BU4D w/ Mount Pipe		
		1	cci antennas	OPA65R-BU6D w/ Mount Pipe		
		1	cci antennas	OPA65R-BU8D w/ Mount Pipe		
		3	powerwave	7770.00 w/ Mount Pipe		
		3	ericsson	RRUS 4449 B5/B12		
		3	ericsson	RRUS 4478 B14_CCIV2		
		3	ericsson	RRUS 8843 B2/B66A_CCIV2		
		6	powerwave	LGP21401		
		1	raycap	DC6-48-60-18-8F		
		1	raycap	DC9-48-60-24-8C-EV		
		1	-	Sector Mount [SM 504-3]		
87.0	89.0	3	kathrein	800 10504 w/ Mount Pipe	6	7/8
	87.0	1	-	Sector Mount [SM 104-3]		
80.0	81.0	1	gps	GPS_A	1	1/2
	80.0	1	-	Side Arm Mount [SO 701-1]		
72.0	72.0	2	gps	GPS_A	2	1/2
		2	-	Side Arm Mount [SO 701-1]		

**3) ANALYSIS PROCEDURE**

**Table 3 - Documents Provided**

Document	Reference	Source
4-GEOTECHNICAL REPORTS	2035622	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	2068030	CCISITES
4-TOWER MANUFACTURER DRAWINGS	1441523	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	2125417	CCISITES
4-POST-MODIFICATION INSPECTION	2376132	CCISITES

### 3.1) Analysis Method

tnxTower (version 8.1.1.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

tnxTower was used to determine the loads on the modified structure. Additional calculations were performed to determine the stresses in the reinforced leg sections. These calculations are presented in Appendix C.

### 3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Morrison Hershfield should be notified to determine the effect on the structural integrity of the tower.

## 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	136 - 133.625	Leg	1 1/2	2	-2.83	54.73	5.2	Pass
T2	133.625 - 130	Leg	1 1/2	14	-4.40	49.84	8.8	Pass
T3	130 - 110	Leg	2	29	-38.02	117.05	32.5	Pass
T4	110 - 94.9427	Leg	2 1/4	107	-82.44	156.33	52.7	Pass
T5	94.9427 - 92.5938	Leg	2 1/4	149	-91.06	179.34	50.8	Pass
T6	92.5938 - 90	Leg	2 1/4	161	-103.76	185.75	55.9	Pass
T7	90 - 80	Leg	PiRod 105244 w/ (2) 1-1/4" Tie Rod	176	-111.72	249.51	44.8	Pass
T8	80 - 60	Leg	PiRod 105217	185	-154.00	225.60	68.3	Pass
T9	60 - 40	Leg	PiRod 105218	200	-188.77	315.72	59.8	Pass
T10	40 - 20	Leg	PiRod 105218	214	-219.27	315.72	69.5	Pass
T11	20 - 0	Leg	PiRod 105219	229	-247.63	419.86	59.0	Pass
T1	136 - 133.625	Diagonal	3/4	7	-1.00	5.77	17.4	Pass
T2	133.625 - 130	Diagonal	3/4	25	-1.48	5.88	25.2	Pass
T3	130 - 110	Diagonal	7/8	40	-4.24	9.77	43.4	Pass
T4	110 - 94.9427	Diagonal	1	115	-5.71	14.70	38.9	Pass
T5	94.9427 - 92.5938	Diagonal	1	153	-6.04	14.41	41.9	Pass
T6	92.5938 - 90	Diagonal	1	169	-7.03	14.87	47.3	Pass
T7	90 - 80	Diagonal	L3x3x3/16	180	-8.25	25.34	32.6 74.6 (b)	Pass
T8	80 - 60	Diagonal	L2 1/2x2 1/2x3/16	190	-6.50	14.57	44.6 64.3 (b)	Pass
T9	60 - 40	Diagonal	L3x3x3/16	202	-6.16	20.18	30.5 53.2 (b)	Pass
T10	40 - 20	Diagonal	L3x3x3/16	217	-6.34	16.11	39.4 52.4 (b)	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail	
T11	20 - 0	Diagonal	L3x3x5/16	232	-8.54	20.97	40.7	Pass	
T2	133.625 - 130	Horizontal	3/4	16	-0.21	3.48	6.0	Pass	
T3	130 - 110	Horizontal	3/4	43	-0.72	2.88	25.0	Pass	
T5	94.9427 - 92.5938	Secondary Horizontal	1 1/2	157	-1.58	50.44	3.1	Pass	
T6	92.5938 - 90	Secondary Horizontal	1 1/2	172	-1.80	49.85	3.6	Pass	
T1	136 - 133.625	Top Girt	6x3/8	4	-0.72	5.14	14.0	Pass	
T3	130 - 110	Top Girt	7/8	31	-0.72	6.54	11.0	Pass	
T4	110 - 94.9427	Top Girt	1	111	-1.75	8.78	19.9	Pass	
T2	133.625 - 130	Bottom Girt	7/8	19	-0.63	6.44	9.7	Pass	
T3	130 - 110	Bottom Girt	7/8	36	-1.76	5.18	34.0	Pass	
T6	92.5938 - 90	Bottom Girt	1	163	-1.80	7.17	25.1	Pass	
							Summary		
							Leg (T10)	69.5	Pass
							Diagonal (T7)	74.6	Pass
							Horizontal (T3)	25.0	Pass
							Secondary Horizontal (T6)	3.6	Pass
							Top Girt (T4)	19.9	Pass
							Bottom Girt (T3)	34.0	Pass
							Bolt Checks	71.0	Pass
							Rating =	74.6	Pass

**Table 5 - Tower Component Stresses vs. Capacity – LC7**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	39.2	Pass
1	Base Foundation (Structure)	0	20.4	Pass
1	Base Foundation (Soil Interaction)		69.5	Pass

<b>Structure Rating (max from all components) =</b>	<b>74.6%*</b>
-----------------------------------------------------	---------------

Notes:

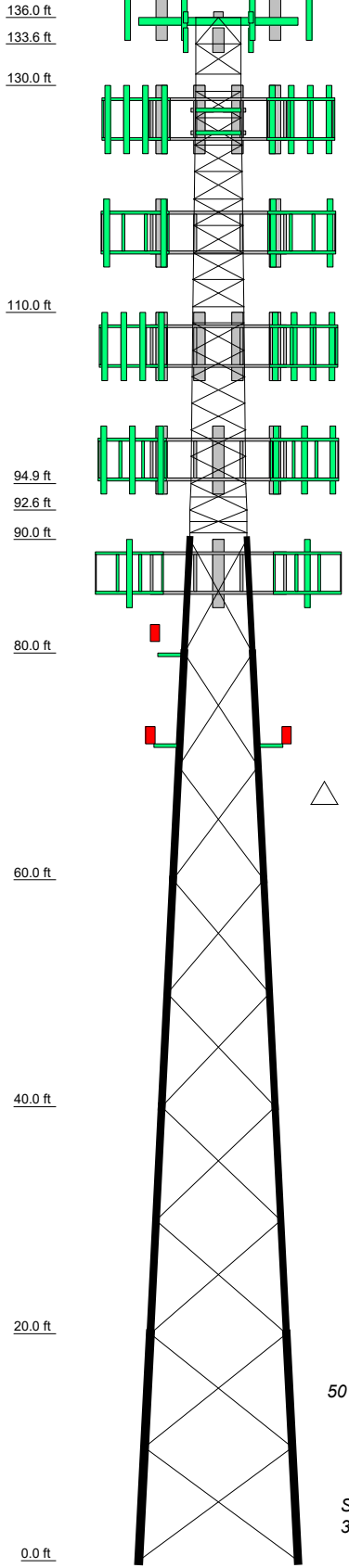
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.
- 2) \*Rating per TIA-222-H, Section 15.5.

#### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

**APPENDIX A**  
**TNXTOWER OUTPUT**

Section	T11	T10	T9	T8	T7	T6	T5	T4	T3	T2	T1
Legs	P1rod 105219	P1rod 105218	P1rod 105217	A572-50	A	A	SR 2 1/4	SR 2	SR 2	SR 1 1/2	SR 1 1/2
Leg Grade	L3x3x5/16	L3x3x3/16	L3x3x3/16	L2 1/2x2 1/2x3/16	L3x3x3/16	L3x3x3/16	SR 1	SR 7/8	SR 7/8	SR 3/4	SR 3/4
Diagonal Grade		A36	A36	A572-50	A572-50	A572-50	SR 1	SR 7/8	SR 7/8	N.A.	B
Top Girts		N.A.	N.A.	N.A.	N.A.	N.A.	C	SR 7/8	SR 7/8	N.A.	N.A.
Bottom Girts		N.A.	N.A.	N.A.	N.A.	N.A.		SR 3/4	SR 3/4	N.A.	N.A.
Horizontal											
Sec. Horizontals											
Face Width (ft)	14	12	10	8	6	4.5	4.5	4.5	4.5	4.5	4
# Panels @ (ft)			9 @ 10	8	6	6 @ 2.34983	6 @ 2.34983	6 @ 2.375	6 @ 2.375	6 @ 2.375	E D
Weight (K)	17.3	4.5	2.9	2.2	1.4	0.3	0.3	0.3	1.4	0.2	0.2



### SYMBOL LIST

MARK	SIZE	MARK	SIZE
A	PIRod 105244 w/ (2) 1-1/4" Tie Rod	E	1 @ 2.625
B	6x3/8	F	1 @ 2.34896
C	SR 1	G	1 @ 2.01042
D	1 @ 2.375		

### MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

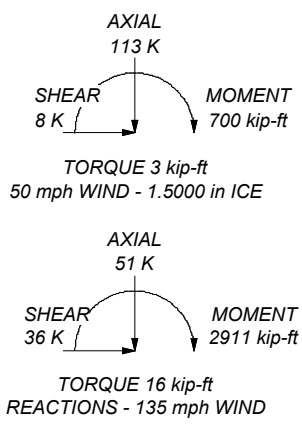
- ### TOWER DESIGN NOTES
1. Tower is located in New London County, Connecticut.
  2. Tower designed for Exposure B to the TIA-222-H Standard.
  3. Tower designed for a 135 mph basic wind in accordance with the TIA-222-H Standard.
  4. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
  5. Deflections are based upon a 60 mph wind.
  6. Tower Risk Category II.
  7. Topographic Category 1 with Crest Height of 0.00 ft
  8. TOWER RATING: 74.6%

ALL REACTIONS ARE FACTORED

MAX. CORNER REACTIONS AT BASE:

DOWN: 257 K  
SHEAR: 25 K

UPLIFT: -224 K  
SHEAR: 22 K



**Morrison Hershfield**

1455 Lincoln Parkway, Suite 500  
Atlanta, GA 30346  
Phone: (770) 379-8500  
FAX: (770) 379-8501

<b>Job: CN8-185R1 / 2101398</b>		
Project: 876338 / Waterford		
Client: Crown Castle USA	Drawn by: CKK	App'd:
Code: TIA-222-H	Date: 08/31/21	Scale: NTS
Path:		Dwg No. E-1

## Tower Input Data

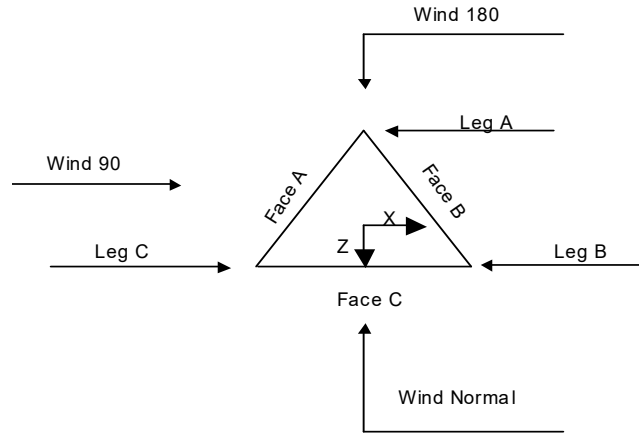
The main tower is a 3x free standing tower with an overall height of 136.00 ft above the ground line.  
 The base of the tower is set at an elevation of 0.00 ft above the ground line.  
 The face width of the tower is 4.00 ft at the top and 14.00 ft at the base.  
 This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- Tower is located in New London County, Connecticut.
- Tower base elevation above sea level: 242.00 ft.
- Basic wind speed of 135 mph.
- Risk Category II.
- Exposure Category B.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0.00 ft.
- Nominal ice thickness of 1.5000 in.
- Ice thickness is considered to increase with height.
- Ice density of 56 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 60 mph.
- Pressures are calculated at each section.
- Stress ratio used in tower member design is 1.
- Tower analysis based on target reliabilities in accordance with Annex S.
- Load Modification Factors used:  $K_{es}(F_w) = 0.95$ ,  $K_{es}(t_i) = 0.85$ .
- Maximum demand-capacity ratio is: 1.05.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification ✓ Use Code Stress Ratios ✓ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile  ✓ Include Bolts In Member Capacity  Leg Bolts Are At Top Of Section ✓ Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric	Distribute Leg Loads As Uniform Assume Legs Pinned ✓ Assume Rigid Index Plate ✓ Use Clear Spans For Wind Area ✓ Use Clear Spans For KL/r Retension Guys To Initial Tension ✓ Bypass Mast Stability Checks ✓ Use Azimuth Dish Coefficients ✓ Project Wind Area of Appurt.  Autocalc Torque Arm Areas  Add IBC .6D+W Combination ✓ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs	Use ASCE 10 X-Brace Ly Rules ✓ Calculate Redundant Bracing Forces Ignore Redundant Members in FEA ✓ SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation ✓ Consider Feed Line Torque ✓ Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption  <div style="text-align: center; background-color: #e0e0e0; padding: 2px;"><b>Poles</b></div> Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known
------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------	-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------	---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------



**Triangular Tower**

**Tower Section Geometry**

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	136.00-133.63			4.00	1	2.38
T2	133.63-130.00			4.00	1	3.63
T3	130.00-110.00			4.00	1	20.00
T4	110.00-94.94			4.50	1	15.06
T5	94.94-92.59			4.88	1	2.35
T6	92.59-90.00			4.93	1	2.59
T7	90.00-80.00			5.00	1	10.00
T8	80.00-60.00			6.00	1	20.00
T9	60.00-40.00			8.00	1	20.00
T10	40.00-20.00			10.00	1	20.00
T11	20.00-0.00			12.00	1	20.00

**Tower Section Geometry (cont'd)**

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	136.00-133.63	2.38	K Brace Down	No	Yes	0.0000	0.0000
T2	133.63-130.00	2.63	X Brace	No	Yes	0.0000	12.0000
T3	130.00-110.00	2.38	X Brace	No	Yes	6.0000	6.0000
T4	110.00-94.94	2.35	X Brace	No	No	11.5000	0.0000
T5	94.94-92.59	2.35	X Brace	No	Yes	0.0000	0.0000
T6	92.59-90.00	2.01	X Brace	No	Yes	0.0000	7.0000
T7	90.00-80.00	10.00	X Brace	No	No	0.0000	0.0000
T8	80.00-60.00	10.00	X Brace	No	No	0.0000	0.0000
T9	60.00-40.00	10.00	X Brace	No	No	0.0000	0.0000
T10	40.00-20.00	10.00	X Brace	No	No	0.0000	0.0000
T11	20.00-0.00	10.00	X Brace	No	No	0.0000	0.0000



### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 136.00-133.63	Solid Round	1 1/2	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T2 133.63-130.00	Solid Round	1 1/2	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T3 130.00-110.00	Solid Round	2	A572-50 (50 ksi)	Solid Round	7/8	A572-50 (50 ksi)
T4 110.00-94.94	Solid Round	2 1/4	A572-50 (50 ksi)	Solid Round	1	A572-50 (50 ksi)
T5 94.94-92.59	Solid Round	2 1/4	A572-50 (50 ksi)	Solid Round	1	A572-50 (50 ksi)
T6 92.59-90.00	Solid Round	2 1/4	A572-50 (50 ksi)	Solid Round	1	A572-50 (50 ksi)
T7 90.00-80.00	Truss Leg	PiRod 105244 w/ (2) 1-1/4" Tie Rod	A572-50 (50 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T8 80.00-60.00	Truss Leg	PiRod 105217	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T9 60.00-40.00	Truss Leg	PiRod 105218	A572-50 (50 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T10 40.00-20.00	Truss Leg	PiRod 105218	A572-50 (50 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T11 20.00-0.00	Truss Leg	PiRod 105219	A572-50 (50 ksi)	Equal Angle	L3x3x5/16	A36 (36 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 136.00-133.63	Flat Bar	6x3/8	A36 (36 ksi)	Solid Round		A572-50 (50 ksi)
T2 133.63-130.00	Solid Round		A572-50 (50 ksi)	Solid Round	7/8	A572-50 (50 ksi)
T3 130.00-110.00	Solid Round	7/8	A572-50 (50 ksi)	Solid Round	7/8	A572-50 (50 ksi)
T4 110.00-94.94	Solid Round	1	A572-50 (50 ksi)	Solid Round		A572-50 (50 ksi)
T6 92.59-90.00	Solid Round		A572-50 (50 ksi)	Solid Round	1	A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T1 136.00-133.63	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T2 133.63-130.00	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T3 130.00-110.00	None	Solid Round		A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
ft						
T5 94.94-92.59	Solid Round	1 1/2	A572-50 (50 ksi)	Solid Round		A572-50 (50 ksi)
T6 92.59-90.00	Solid Round	1 1/2	A572-50 (50 ksi)	Solid Round		A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>r</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in					in	in	in
T1 136.00-133.63	0.00	0.0000	A36 (36 ksi)	1	1.03	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T2 133.63-130.00	0.00	0.0000	A36 (36 ksi)	1	1.03	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T3 130.00-110.00	0.00	0.0000	A36 (36 ksi)	1	1.03	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T4 110.00-94.94	0.00	0.0000	A36 (36 ksi)	1	1.03	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T5 94.94-92.59	0.00	0.0000	A36 (36 ksi)	1	1.03	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T6 92.59-90.00	0.00	0.0000	A36 (36 ksi)	1	1.03	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T7 90.00-80.00	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T8 80.00-60.00	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T9 60.00-40.00	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T10 40.00-20.00	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T11 20.00-0.00	0.00	0.0000	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt

### Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	K Factors <sup>1</sup>									
			Legs	X Brace Diags		K Brace Diags		Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
				X	Y	X	Y					
ft												
T1 136.00-133.63	No	Yes	1	1	1	1	1	1	1	1	1	1
T2 133.63-130.00	No	Yes	1	1	1	1	1	1	1	1	1	1
T3 130.00-110.00	No	Yes	1	1	1	1	1	1	1	1	1	1
T4 110.00-94.94	No	Yes	1	1	1	1	1	1	1	1	1	1
T5 94.94-92.59	No	Yes	1	1	1	1	1	1	1	1	1	1
T6 92.59-90.00	No	Yes	1	1	1	1	1	1	1	0.5	1	1
T7 90.00-80.00	Yes	No	1	1	1	1	1	1	1	0.5	1	1
T8 80.00-60.00	Yes	No	1	1	1	1	1	1	1	1	1	1

Tower Elevation ft	Calc K Single Angles	Calc K Solid Rounds	K Factors <sup>1</sup>								
			Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
				X Y	X Y	X Y	X Y	X Y	X Y	X Y	
T9 60.00-40.00	Yes	No	1	1 1	1 1	1 1	1 1	1 1	1 1	1 1	1 1
T10 40.00-20.00	Yes	No	1	1 1	1 1	1 1	1 1	1 1	1 1	1 1	1 1
T11 20.00-0.00	Yes	No	1	1 1	1 1	1 1	1 1	1 1	1 1	1 1	1 1

<sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

### Tower Section Geometry (cont'd)

Tower Elevation ft	Truss-Leg K Factors					
	Truss-Legs Used As Leg Members			Truss-Legs Used As Inner Members		
	Leg Panels	X Brace Diagonals	Z Brace Diagonals	Leg Panels	X Brace Diagonals	Z Brace Diagonals
T7 90.00-80.00	1.29	0.5	0.85	1	1	1
T8 80.00-60.00	1	0.5	0.85	1	1	1
T9 60.00-40.00	1	0.5	0.85	1	1	1
T10 40.00-20.00	1	0.5	0.85	1	1	1
T11 20.00-0.00	1	0.5	0.85	1	0.5	0.85

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 136.00-133.63	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	1
T2 133.63-130.00	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	0.75	0.0000	1	0.0000	1
T3 130.00-110.00	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1
T4 110.00-94.94	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1
T5 94.94-92.59	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1
T6 92.59-90.00	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1	0.0000	1
T7 90.00-80.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	1	0.0000	1	0.0000	1
T8 80.00-60.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	1	0.0000	1	0.0000	1
T9 60.00-40.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	1	0.0000	1	0.0000	1
T10 40.00-20.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	1	0.0000	1	0.0000	1
T11 20.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	1	0.0000	1	0.0000	1	0.0000	1

Tower Elevation ft	Redundant Horizontal		Redundant Diagonal		Redundant Sub-Diagonal		Redundant Sub-Horizontal		Redundant Vertical		Redundant Hip		Redundant Hip Diagonal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 136.00-133.63	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 133.63-130.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3 130.00-110.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T4 110.00-94.94	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 94.94-92.59	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 92.59-90.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 90.00-80.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 80.00-60.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 60.00-40.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T10 40.00-20.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T11 20.00-0.00	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 136.00-133.63	Flange	0.6250	0	A325N	0	0.0000	0	0.0000	0	0.6250	0	0.0000	0	0.6250	0
T2 133.63-130.00	Sleeve DS	0.6250	5	A325N	0	0.0000	0	0.0000	0	0.6250	0	0.0000	0	0.6250	0
T3 130.00-110.00	Sleeve DS	0.7500	5	A325N	0	0.0000	0	0.0000	0	0.5000	0	0.0000	0	0.5000	0
T4 110.00-94.94	Flange	0.0000	0	A325N	0	0.0000	0	0.0000	0	0.5000	0	0.0000	0	0.5000	0
T5 94.94-92.59	Flange	0.0000	0	A325N	0	0.0000	0	0.0000	0	0.5000	0	0.0000	0	0.5000	0
T6 92.59-90.00	Flange	1.0000	6	A325N	0	0.0000	0	0.0000	0	0.5000	0	0.0000	0	0.5000	0
T7 90.00-80.00	Flange	1.0000	6	A325N	1	0.0000	0	0.0000	0	0.5000	0	0.0000	0	0.5000	0
T8 80.00-60.00	Flange	1.0000	6	A325N	1	0.0000	0	0.0000	0	0.5000	0	0.0000	0	0.5000	0
T9 60.00-40.00	Flange	1.0000	6	A325N	1	0.0000	0	0.0000	0	0.5000	0	0.0000	0	0.5000	0
T10 40.00-20.00	Flange	1.0000	6	A325N	1	0.0000	0	0.0000	0	0.5000	0	0.0000	0	0.5000	0
T11 20.00-0.00	Flange	1.2500	0	A-687	1	0.0000	0	0.0000	0	0.6250	0	0.0000	0	0.6250	0

### Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
LDF6-50A(1-1/4)	B	No	No	Ar (CaAa)	97.00 - 0.00	- 9.0000	0.4	6	6	0.8500 0.7500	1.5500		0.60
WR-VG86ST-BRDA(7/8)	C	No	No	Ar (CaAa)	97.00 - 0.00	- 5.0000	-0.35	6	3	0.8500 0.7500	0.8800		0.68
HB114-21U3M12-XXXF(1-1/4)	C	No	No	Ar (CaAa)	136.00 - 0.00	- 5.0000	-0.4	4	3	0.8500 0.7500	1.5400		1.22
FLC 12-50J(1/2")	C	No	No	Ar (CaAa)	80.00 - 0.00	- 5.0000	-0.47	1	1	0.8500 0.7500	0.6400		0.17
LDF7-50A(1-5/8)	B	No	No	Ar (CaAa)	107.00 - 0.00	- 4.0000	0.37	14	7	0.5000	1.9800		0.82
T-Brackets	B	No	No	Af (CaAa)	136.00 - 0.00	- 7.0000	0.4	1	1	1.0000	1.0000		8.40
***													
LDF6-50A(1-1/4)	A	No	No	Ar (CaAa)	127.00 - 0.00	- 7.0000	0.4	12	6	0.5000	1.5500		0.60
LDF4-50A(1/2)	A	No	No	Ar (CaAa)	72.00 - 0.00	- 9.0000	0.4	2	2	0.5000	0.6250		0.15
T-Brackets	A	No	No	Af (CaAa)	127.00 - 0.00	- 7.0000	0.4	1	1	1.0000	1.0000		8.40
***													
T-Brackets	C	No	No	Af (CaAa)	117.00 - 0.00	- 5.0000	0.4	1	1	1.0000	1.0000		8.40
*													
HCS 6X12 4AWG(1-5/8)	C	No	No	Ar (CaAa)	117.00 - 0.00	- 5.0000	0.4	3	2	0.5000	1.6600		2.40
HB158-21U6S24-xxM_TMO(1-5/8)	C	No	No	Ar (CaAa)	117.00 - 0.00	- 7.0000	0.45	1	1	0.5000	1.9960		2.50
***													
FXL 780 PE(7/8)	A	No	No	Ar (CaAa)	87.00 - 0.00	- 0.0000	0	6	6	0.5000	1.0900		0.25
LDF1-50A(1/4)	A	No	No	Ar (CaAa)	87.00 - 0.00	- 0.0000	0.1	1	1	0.5000	0.3450		0.06
Feedline Ladder (Af)	A	No	No	Af (CaAa)	87.00 - 0.00	- 0.0000	0	1	1	3.0000	3.0000		8.40
***													
Safety Line 3/8	C	No	No	Ar (CaAa)	90.00 - 0.00	- 0.0000	0.48	1	1	0.3750	0.3750		0.22
Safety Line 3/8	C	No	No	Ar (CaAa)	136.00 - 90.00	- 0.0000	0	1	1	0.3750	0.3750		0.22
Ladder Rung SR 3/4 (48"w 26"s)	C	No	No	Af (CaAa)	136.00 - 90.00	- 0.0000	0	1	1	1.3500	1.3500		2.71
***													

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
T1	136.00-133.63	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.237	0.000	0.02
		C	0.000	0.000	2.086	0.000	0.02
T2	133.63-130.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.362	0.000	0.03
		C	0.000	0.000	3.185	0.000	0.03
T3	130.00-110.00	A	0.000	0.000	33.320	0.000	0.27
		B	0.000	0.000	2.000	0.000	0.17
		C	0.000	0.000	23.153	0.000	0.28

Tower Section	Tower Elevation	Face	A <sub>R</sub>	A <sub>F</sub>	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face	Weight
n	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
T4	110.00-94.94	A	0.000	0.000	29.512	0.000	0.23
		B	0.000	0.000	36.842	0.000	0.27
		C	0.000	0.000	26.324	0.000	0.40
T5	94.94-92.59	A	0.000	0.000	4.604	0.000	0.04
		B	0.000	0.000	8.931	0.000	0.06
		C	0.000	0.000	5.177	0.000	0.07
T6	92.59-90.00	A	0.000	0.000	5.084	0.000	0.04
		B	0.000	0.000	9.861	0.000	0.06
		C	0.000	0.000	5.717	0.000	0.08
T7	90.00-80.00	A	0.000	0.000	27.919	0.000	0.23
		B	0.000	0.000	38.020	0.000	0.23
		C	0.000	0.000	19.791	0.000	0.27
T8	80.00-60.00	A	0.000	0.000	64.470	0.000	0.51
		B	0.000	0.000	76.040	0.000	0.47
		C	0.000	0.000	40.862	0.000	0.55
T9	60.00-40.00	A	0.000	0.000	65.470	0.000	0.52
		B	0.000	0.000	76.040	0.000	0.47
		C	0.000	0.000	40.862	0.000	0.55
T10	40.00-20.00	A	0.000	0.000	65.470	0.000	0.52
		B	0.000	0.000	76.040	0.000	0.47
		C	0.000	0.000	40.862	0.000	0.55
T11	20.00-0.00	A	0.000	0.000	65.470	0.000	0.52
		B	0.000	0.000	76.040	0.000	0.47
		C	0.000	0.000	40.862	0.000	0.55

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation	Face or Leg	Ice Thickness	A <sub>R</sub>	A <sub>F</sub>	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face	Weight
n	ft		in	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	K
T1	136.00-133.63	A	1.468	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.935	0.000	0.03
		C		0.000	0.000	5.421	0.000	0.08
T2	133.63-130.00	A	1.464	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	1.424	0.000	0.05
		C		0.000	0.000	8.266	0.000	0.12
T3	130.00-110.00	A	1.451	0.000	0.000	40.791	0.000	0.79
		B		0.000	0.000	7.803	0.000	0.25
		C		0.000	0.000	59.874	0.000	0.91
T4	110.00-94.94	A	1.428	0.000	0.000	35.949	0.000	0.69
		B		0.000	0.000	41.869	0.000	0.83
		C		0.000	0.000	67.112	0.000	1.09
T5	94.94-92.59	A	1.415	0.000	0.000	5.592	0.000	0.11
		B		0.000	0.000	12.044	0.000	0.20
		C		0.000	0.000	12.690	0.000	0.20
T6	92.59-90.00	A	1.412	0.000	0.000	6.170	0.000	0.12
		B		0.000	0.000	13.291	0.000	0.22
		C		0.000	0.000	13.995	0.000	0.22
T7	90.00-80.00	A	1.402	0.000	0.000	42.308	0.000	0.71
		B		0.000	0.000	51.158	0.000	0.86
		C		0.000	0.000	48.715	0.000	0.77
T8	80.00-60.00	A	1.375	0.000	0.000	108.302	0.000	1.69
		B		0.000	0.000	101.871	0.000	1.70
		C		0.000	0.000	103.319	0.000	1.60
T9	60.00-40.00	A	1.329	0.000	0.000	112.477	0.000	1.68
		B		0.000	0.000	101.119	0.000	1.66
		C		0.000	0.000	101.634	0.000	1.55
T10	40.00-20.00	A	1.263	0.000	0.000	110.369	0.000	1.62
		B		0.000	0.000	100.028	0.000	1.61
		C		0.000	0.000	99.182	0.000	1.50
T11	20.00-0.00	A	1.132	0.000	0.000	106.194	0.000	1.50
		B		0.000	0.000	97.868	0.000	1.52
		C		0.000	0.000	94.316	0.000	1.38

### Feed Line Center of Pressure

Section	Elevation	CP <sub>x</sub>	CP <sub>z</sub>	CP <sub>x</sub> Ice	CP <sub>z</sub> Ice
	ft	in	in	in	in
T1	136.00-133.63	2.6714	1.7636	2.0354	1.8729
T2	133.63-130.00	5.4840	3.4246	3.3407	2.9871
T3	130.00-110.00	2.7843	-3.3933	1.7174	-0.4192
T4	110.00-94.94	3.5778	0.7189	2.0426	1.7846
T5	94.94-92.59	5.9579	3.0678	3.3697	3.0596
T6	92.59-90.00	5.8788	3.0288	2.8531	2.6153
T7	90.00-80.00	3.7454	1.6723	1.6490	1.6464
T8	80.00-60.00	4.4010	1.4292	2.3914	1.4717
T9	60.00-40.00	5.2439	1.4345	3.2898	1.5330
T10	40.00-20.00	6.1833	1.6162	4.1521	1.8512
T11	20.00-0.00	6.9382	1.7606	4.8831	2.0615

### Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T1	3	HB114-21U3M12-XXXXF(1-1/4)	133.63 - 136.00	0.6000	0.3616
T1	6	T-Brackets	133.63 - 136.00	0.6000	0.3616
T1	24	Safety Line 3/8	133.63 - 136.00	0.6000	0.3616
T1	25	Ladder Rung SR 3/4 (48"w 26"s)	133.63 - 136.00	0.6000	0.3616
T2	3	HB114-21U3M12-XXXXF(1-1/4)	130.00 - 133.63	0.6000	0.4840
T2	6	T-Brackets	130.00 - 133.63	0.6000	0.4840
T2	24	Safety Line 3/8	130.00 - 133.63	0.6000	0.4840
T2	25	Ladder Rung SR 3/4 (48"w 26"s)	130.00 - 133.63	0.6000	0.4840
T3	3	HB114-21U3M12-XXXXF(1-1/4)	110.00 - 130.00	0.6000	0.4344
T3	6	T-Brackets	110.00 - 130.00	0.6000	0.4344
T3	8	LDF6-50A(1-1/4)	110.00 - 127.00	0.6000	0.4344
T3	10	T-Brackets	110.00 - 127.00	0.6000	0.4344
T3	14	T-Brackets	110.00 - 117.00	0.6000	0.4344
T3	16	HCS 6X12 4AWG(1-5/8)	110.00 - 117.00	0.6000	0.4344
T3	17	HB158-21U6S24-xxM_TMO(1-5/8)	110.00 - 117.00	0.6000	0.4344
T3	24	Safety Line 3/8	110.00 - 130.00	0.6000	0.4344
T3	25	Ladder Rung SR 3/4 (48"w 26"s)	110.00 - 130.00	0.6000	0.4344
T4	1	LDF6-50A(1-1/4)	94.94 - 97.00	0.6000	0.5497
T4	2	WR-VG86ST-BRDA(7/8)	94.94 - 97.00	0.6000	0.5497
T4	3	HB114-21U3M12-XXXXF(1-1/4)	94.94 - 110.00	0.6000	0.5497
T4	5	LDF7-50A(1-5/8)	94.94 - 107.00	0.6000	0.5497
T4	6	T-Brackets	94.94 - 110.00	0.6000	0.5497
T4	8	LDF6-50A(1-1/4)	94.94 - 110.00	0.6000	0.5497

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T4	10	T-Brackets	94.94 - 110.00	0.6000	0.5497
T4	14	T-Brackets	94.94 - 110.00	0.6000	0.5497
T4	16	HCS 6X12 4AWG(1-5/8)	94.94 - 110.00	0.6000	0.5497
T4	17	HB158-21U6S24-xxM_TMO(1-5/8)	94.94 - 110.00	0.6000	0.5497
T4	24	Safety Line 3/8	94.94 - 110.00	0.6000	0.5497
T4	25	Ladder Rung SR 3/4 (48"w 26"s)	94.94 - 110.00	0.6000	0.5497
T5	1	LDF6-50A(1-1/4)	92.59 - 94.94	0.6000	0.4214
T5	2	WR-VG86ST-BRDA(7/8)	92.59 - 94.94	0.6000	0.4214
T5	3	HB114-21U3M12-XXXF(1-1/4)	92.59 - 94.94	0.6000	0.4214
T5	5	LDF7-50A(1-5/8)	92.59 - 94.94	0.6000	0.4214
T5	6	T-Brackets	92.59 - 94.94	0.6000	0.4214
T5	8	LDF6-50A(1-1/4)	92.59 - 94.94	0.6000	0.4214
T5	10	T-Brackets	92.59 - 94.94	0.6000	0.4214
T5	14	T-Brackets	92.59 - 94.94	0.6000	0.4214
T5	16	HCS 6X12 4AWG(1-5/8)	92.59 - 94.94	0.6000	0.4214
T5	17	HB158-21U6S24-xxM_TMO(1-5/8)	92.59 - 94.94	0.6000	0.4214
T5	24	Safety Line 3/8	92.59 - 94.94	0.6000	0.4214
T5	25	Ladder Rung SR 3/4 (48"w 26"s)	92.59 - 94.94	0.6000	0.4214
T6	1	LDF6-50A(1-1/4)	90.00 - 92.59	0.6000	0.3574
T6	2	WR-VG86ST-BRDA(7/8)	90.00 - 92.59	0.6000	0.3574
T6	3	HB114-21U3M12-XXXF(1-1/4)	90.00 - 92.59	0.6000	0.3574
T6	5	LDF7-50A(1-5/8)	90.00 - 92.59	0.6000	0.3574
T6	6	T-Brackets	90.00 - 92.59	0.6000	0.3574
T6	8	LDF6-50A(1-1/4)	90.00 - 92.59	0.6000	0.3574
T6	10	T-Brackets	90.00 - 92.59	0.6000	0.3574
T6	14	T-Brackets	90.00 - 92.59	0.6000	0.3574
T6	16	HCS 6X12 4AWG(1-5/8)	90.00 - 92.59	0.6000	0.3574
T6	17	HB158-21U6S24-xxM_TMO(1-5/8)	90.00 - 92.59	0.6000	0.3574
T6	24	Safety Line 3/8	90.00 - 92.59	0.6000	0.3574
T6	25	Ladder Rung SR 3/4 (48"w 26"s)	90.00 - 92.59	0.6000	0.3574
T7	1	LDF6-50A(1-1/4)	80.00 - 90.00	0.6000	0.3989
T7	2	WR-VG86ST-BRDA(7/8)	80.00 - 90.00	0.6000	0.3989
T7	3	HB114-21U3M12-XXXF(1-1/4)	80.00 - 90.00	0.6000	0.3989
T7	5	LDF7-50A(1-5/8)	80.00 - 90.00	0.6000	0.3989
T7	6	T-Brackets	80.00 -	0.6000	0.3989



Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
			90.00		
T7	8	LDF6-50A(1-1/4)	80.00 -	0.6000	0.3989
			90.00		
T7	10	T-Brackets	80.00 -	0.6000	0.3989
			90.00		
T7	14	T-Brackets	80.00 -	0.6000	0.3989
			90.00		
T7	16	HCS 6X12 4AWG(1-5/8)	80.00 -	0.6000	0.3989
			90.00		
T7	17	HB158-21U6S24-xxM_TMO(1-5/8)	80.00 -	0.6000	0.3989
			90.00		
T7	19	FXL 780 PE(7/8)	80.00 -	0.6000	0.3989
			87.00		
T7	20	LDF1-50A(1/4)	80.00 -	0.6000	0.3989
			87.00		
T7	21	Feedline Ladder (Af)	80.00 -	0.6000	0.3989
			87.00		
T7	23	Safety Line 3/8	80.00 -	0.6000	0.3989
			90.00		
T8	1	LDF6-50A(1-1/4)	60.00 -	0.6000	0.4754
			80.00		
T8	2	WR-VG86ST-BRDA(7/8)	60.00 -	0.6000	0.4754
			80.00		
T8	3	HB114-21U3M12-XXXF(1-1/4)	60.00 -	0.6000	0.4754
			80.00		
T8	4	FLC 12-50J(1/2")	60.00 -	0.6000	0.4754
			80.00		
T8	5	LDF7-50A(1-5/8)	60.00 -	0.6000	0.4754
			80.00		
T8	6	T-Brackets	60.00 -	0.6000	0.4754
			80.00		
T8	8	LDF6-50A(1-1/4)	60.00 -	0.6000	0.4754
			80.00		
T8	9	LDF4-50A(1/2)	60.00 -	0.6000	0.4754
			72.00		
T8	10	T-Brackets	60.00 -	0.6000	0.4754
			80.00		
T8	14	T-Brackets	60.00 -	0.6000	0.4754
			80.00		
T8	16	HCS 6X12 4AWG(1-5/8)	60.00 -	0.6000	0.4754
			80.00		
T8	17	HB158-21U6S24-xxM_TMO(1-5/8)	60.00 -	0.6000	0.4754
			80.00		
T8	19	FXL 780 PE(7/8)	60.00 -	0.6000	0.4754
			80.00		
T8	20	LDF1-50A(1/4)	60.00 -	0.6000	0.4754
			80.00		
T8	21	Feedline Ladder (Af)	60.00 -	0.6000	0.4754
			80.00		
T8	23	Safety Line 3/8	60.00 -	0.6000	0.4754
			80.00		
T9	1	LDF6-50A(1-1/4)	40.00 -	0.6000	0.5541
			60.00		
T9	2	WR-VG86ST-BRDA(7/8)	40.00 -	0.6000	0.5541
			60.00		
T9	3	HB114-21U3M12-XXXF(1-1/4)	40.00 -	0.6000	0.5541
			60.00		
T9	4	FLC 12-50J(1/2")	40.00 -	0.6000	0.5541
			60.00		
T9	5	LDF7-50A(1-5/8)	40.00 -	0.6000	0.5541
			60.00		
T9	6	T-Brackets	40.00 -	0.6000	0.5541
			60.00		
T9	8	LDF6-50A(1-1/4)	40.00 -	0.6000	0.5541
			60.00		
T9	9	LDF4-50A(1/2)	40.00 -	0.6000	0.5541
			60.00		
T9	10	T-Brackets	40.00 -	0.6000	0.5541
			60.00		

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	$K_a$ No Ice	$K_a$ Ice
T9	14	T-Brackets	40.00 - 60.00	0.6000	0.5541
T9	16	HCS 6X12 4AWG(1-5/8)	40.00 - 60.00	0.6000	0.5541
T9	17	HB158-21U6S24-xxM_TMO(1-5/8)	40.00 - 60.00	0.6000	0.5541
T9	19	FXL 780 PE(7/8)	40.00 - 60.00	0.6000	0.5541
T9	20	LDF1-50A(1/4)	40.00 - 60.00	0.6000	0.5541
T9	21	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.5541
T9	23	Safety Line 3/8	40.00 - 60.00	0.6000	0.5541
T10	1	LDF6-50A(1-1/4)	20.00 - 40.00	0.6000	0.6000
T10	2	WR-VG86ST-BRDA(7/8)	20.00 - 40.00	0.6000	0.6000
T10	3	HB114-21U3M12-XXXF(1-1/4)	20.00 - 40.00	0.6000	0.6000
T10	4	FLC 12-50J(1/2")	20.00 - 40.00	0.6000	0.6000
T10	5	LDF7-50A(1-5/8)	20.00 - 40.00	0.6000	0.6000
T10	6	T-Brackets	20.00 - 40.00	0.6000	0.6000
T10	8	LDF6-50A(1-1/4)	20.00 - 40.00	0.6000	0.6000
T10	9	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
T10	10	T-Brackets	20.00 - 40.00	0.6000	0.6000
T10	14	T-Brackets	20.00 - 40.00	0.6000	0.6000
T10	16	HCS 6X12 4AWG(1-5/8)	20.00 - 40.00	0.6000	0.6000
T10	17	HB158-21U6S24-xxM_TMO(1-5/8)	20.00 - 40.00	0.6000	0.6000
T10	19	FXL 780 PE(7/8)	20.00 - 40.00	0.6000	0.6000
T10	20	LDF1-50A(1/4)	20.00 - 40.00	0.6000	0.6000
T10	21	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T10	23	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000
T11	1	LDF6-50A(1-1/4)	0.00 - 20.00	0.6000	0.6000
T11	2	WR-VG86ST-BRDA(7/8)	0.00 - 20.00	0.6000	0.6000
T11	3	HB114-21U3M12-XXXF(1-1/4)	0.00 - 20.00	0.6000	0.6000
T11	4	FLC 12-50J(1/2")	0.00 - 20.00	0.6000	0.6000
T11	5	LDF7-50A(1-5/8)	0.00 - 20.00	0.6000	0.6000
T11	6	T-Brackets	0.00 - 20.00	0.6000	0.6000
T11	8	LDF6-50A(1-1/4)	0.00 - 20.00	0.6000	0.6000
T11	9	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T11	10	T-Brackets	0.00 - 20.00	0.6000	0.6000
T11	14	T-Brackets	0.00 - 20.00	0.6000	0.6000
T11	16	HCS 6X12 4AWG(1-5/8)	0.00 - 20.00	0.6000	0.6000
T11	17	HB158-21U6S24-xxM_TMO(1-5/8)	0.00 - 20.00	0.6000	0.6000
T11	19	FXL 780 PE(7/8)	0.00 - 20.00	0.6000	0.6000
T11	20	LDF1-50A(1/4)	0.00 - 20.00	0.6000	0.6000
T11	21	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T11	23	Safety Line 3/8	0.00 - 20.00	0.6000	0.6000

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustmen t °	Placement  ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight  K	
APXVSP18-C-A20 w/ Mount Pipe	A	From Leg	4.00	0.0000	136.00	No Ice	4.60	4.01	0.10
			0.00			1/2" Ice	5.05	4.45	0.16
			1.00			1" Ice	5.50	4.89	0.23
						2" Ice	6.44	5.82	0.42
APXVSP18-C-A20 w/ Mount Pipe	B	From Leg	4.00	0.0000	136.00	No Ice	4.60	4.01	0.10
			0.00			1/2" Ice	5.05	4.45	0.16
			1.00			1" Ice	5.50	4.89	0.23
						2" Ice	6.44	5.82	0.42
APXVSP18-C-A20 w/ Mount Pipe	C	From Leg	4.00	0.0000	136.00	No Ice	4.60	4.01	0.10
			0.00			1/2" Ice	5.05	4.45	0.16
			1.00			1" Ice	5.50	4.89	0.23
						2" Ice	6.44	5.82	0.42
APXVTM14-C-120 w/ Mount Pipe	A	From Leg	4.00	0.0000	136.00	No Ice	4.09	2.86	0.08
			0.00			1/2" Ice	4.48	3.23	0.13
			1.00			1" Ice	4.88	3.61	0.19
						2" Ice	5.71	4.40	0.33
APXVTM14-C-120 w/ Mount Pipe	B	From Leg	4.00	0.0000	136.00	No Ice	4.09	2.86	0.08
			0.00			1/2" Ice	4.48	3.23	0.13
			1.00			1" Ice	4.88	3.61	0.19
						2" Ice	5.71	4.40	0.33
APXVTM14-C-120 w/ Mount Pipe	C	From Leg	4.00	0.0000	136.00	No Ice	4.09	2.86	0.08
			0.00			1/2" Ice	4.48	3.23	0.13
			1.00			1" Ice	4.88	3.61	0.19
						2" Ice	5.71	4.40	0.33
TD-RRH8X20-25	A	From Leg	4.00	0.0000	136.00	No Ice	4.05	1.53	0.07
			0.00			1/2" Ice	4.30	1.71	0.10
			1.00			1" Ice	4.56	1.90	0.13
						2" Ice	5.10	2.30	0.20
TD-RRH8X20-25	B	From Leg	4.00	0.0000	136.00	No Ice	4.05	1.53	0.07
			0.00			1/2" Ice	4.30	1.71	0.10
			1.00			1" Ice	4.56	1.90	0.13
						2" Ice	5.10	2.30	0.20
TD-RRH8X20-25	C	From Leg	4.00	0.0000	136.00	No Ice	4.05	1.53	0.07
			0.00			1/2" Ice	4.30	1.71	0.10
			1.00			1" Ice	4.56	1.90	0.13
						2" Ice	5.10	2.30	0.20
6' x 2" Mount Pipe	A	From Leg	4.00	0.0000	136.00	No Ice	1.43	1.43	0.02
			0.00			1/2" Ice	1.92	1.92	0.03
			0.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
6' x 2" Mount Pipe	B	From Leg	4.00	0.0000	136.00	No Ice	1.43	1.43	0.02
			0.00			1/2" Ice	1.92	1.92	0.03
			0.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
6' x 2" Mount Pipe	C	From Leg	4.00	0.0000	136.00	No Ice	1.43	1.43	0.02
			0.00			1/2" Ice	1.92	1.92	0.03
			0.00			1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09
Platform Mount [LP 405-1]	A	None		0.0000	136.00	No Ice	20.88	20.88	1.80
						1/2" Ice	28.89	28.89	2.28
						1" Ice	37.04	37.04	2.87
						2" Ice	53.73	53.73	4.39
***									
1900MHZ RRH (65MHZ)	A	From Leg	1.00	0.0000	134.00	No Ice	2.32	2.24	0.06
			0.00			1/2" Ice	2.53	2.44	0.08
			0.00			1" Ice	2.74	2.65	0.11
						2" Ice	3.19	3.09	0.17
1900MHZ RRH (65MHZ)	B	From Leg	1.00	0.0000	134.00	No Ice	2.32	2.24	0.06
			0.00			1/2" Ice	2.53	2.44	0.08
			0.00			1" Ice	2.74	2.65	0.11
						2" Ice	3.19	3.09	0.17
1900MHZ RRH (65MHZ)	C	From Leg	1.00	0.0000	134.00	No Ice	2.32	2.24	0.06

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K	
			0.00			1/2" Ice	2.53	2.44	0.08
			0.00			1" Ice	2.74	2.65	0.11
						2" Ice	3.19	3.09	0.17
800MHZ 2X50W RRH W/FILTER	A	From Leg	1.00	0.0000	134.00	No Ice	2.06	1.93	0.06
			0.00			1/2" Ice	2.24	2.11	0.09
			0.00			1" Ice	2.43	2.29	0.11
						2" Ice	2.83	2.68	0.17
800MHZ 2X50W RRH W/FILTER	B	From Leg	1.00	0.0000	134.00	No Ice	2.06	1.93	0.06
			0.00			1/2" Ice	2.24	2.11	0.09
			0.00			1" Ice	2.43	2.29	0.11
						2" Ice	2.83	2.68	0.17
800MHZ 2X50W RRH W/FILTER	C	From Leg	1.00	0.0000	134.00	No Ice	2.06	1.93	0.06
			0.00			1/2" Ice	2.24	2.11	0.09
			0.00			1" Ice	2.43	2.29	0.11
						2" Ice	2.83	2.68	0.17
IBC1900BB-1	A	From Leg	1.00	0.0000	134.00	No Ice	0.97	0.46	0.02
			0.00			1/2" Ice	1.09	0.56	0.03
			2.00			1" Ice	1.22	0.66	0.04
						2" Ice	1.51	0.89	0.06
IBC1900BB-1	B	From Leg	1.00	0.0000	134.00	No Ice	0.97	0.46	0.02
			0.00			1/2" Ice	1.09	0.56	0.03
			2.00			1" Ice	1.22	0.66	0.04
						2" Ice	1.51	0.89	0.06
IBC1900BB-1	C	From Leg	1.00	0.0000	134.00	No Ice	0.97	0.46	0.02
			0.00			1/2" Ice	1.09	0.56	0.03
			2.00			1" Ice	1.22	0.66	0.04
						2" Ice	1.51	0.89	0.06
IBC1900HG-2A	A	From Leg	1.00	0.0000	134.00	No Ice	0.97	0.46	0.02
			0.00			1/2" Ice	1.09	0.56	0.03
			2.00			1" Ice	1.22	0.66	0.04
						2" Ice	1.51	0.89	0.06
IBC1900HG-2A	B	From Leg	1.00	0.0000	134.00	No Ice	0.97	0.46	0.02
			0.00			1/2" Ice	1.09	0.56	0.03
			2.00			1" Ice	1.22	0.66	0.04
						2" Ice	1.51	0.89	0.06
IBC1900HG-2A	C	From Leg	1.00	0.0000	134.00	No Ice	0.97	0.46	0.02
			0.00			1/2" Ice	1.09	0.56	0.03
			2.00			1" Ice	1.22	0.66	0.04
						2" Ice	1.51	0.89	0.06
Pipe Mount [PM 601-3]	A	None		0.0000	134.00	No Ice	3.17	3.17	0.20
						1/2" Ice	3.79	3.79	0.23
						1" Ice	4.42	4.42	0.28
						2" Ice	5.76	5.76	0.40
***									
(4) DB844H90E-XY w/ Mount Pipe	A	From Leg	4.00	0.0000	127.00	No Ice	2.24	3.34	0.04
			0.00			1/2" Ice	2.61	3.73	0.08
			0.00			1" Ice	2.99	4.13	0.12
						2" Ice	3.78	4.97	0.23
(4) DB844H90E-XY w/ Mount Pipe	B	From Leg	4.00	0.0000	127.00	No Ice	2.24	3.34	0.04
			0.00			1/2" Ice	2.61	3.73	0.08
			0.00			1" Ice	2.99	4.13	0.12
						2" Ice	3.78	4.97	0.23
(4) DB844H90E-XY w/ Mount Pipe	C	From Leg	4.00	0.0000	127.00	No Ice	2.24	3.34	0.04
			0.00			1/2" Ice	2.61	3.73	0.08
			0.00			1" Ice	2.99	4.13	0.12
						2" Ice	3.78	4.97	0.23
HSS 4"x4"x4'	A	From Face	0.50	0.0000	127.00	No Ice	2.09	0.00	0.04
			0.00			1/2" Ice	2.39	0.00	0.05
			1.00			1" Ice	2.70	0.00	0.07
						2" Ice	3.33	0.00	0.12
HSS 4"x4"x4'	A	From Face	0.50	0.0000	127.00	No Ice	2.09	0.00	0.04
			0.00			1/2" Ice	2.39	0.00	0.05
			-1.00			1" Ice	2.70	0.00	0.07
						2" Ice	3.33	0.00	0.12
HSS 4"x4"x4'	B	From	0.50	0.0000	127.00	No Ice	2.09	0.00	0.04

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K
		Face	0.00			1/2" Ice 2.39	0.00	0.05
			1.00			1" Ice 2.70	0.00	0.07
						2" Ice 3.33	0.00	0.12
HSS 4"x4"x4'	B	From Face	0.50	0.0000	127.00	No Ice 2.09	0.00	0.04
			0.00			1/2" Ice 2.39	0.00	0.05
			-1.00			1" Ice 2.70	0.00	0.07
						2" Ice 3.33	0.00	0.12
HSS 4"x4"x4'	C	From Face	0.50	0.0000	127.00	No Ice 2.09	0.00	0.04
			0.00			1/2" Ice 2.39	0.00	0.05
			1.00			1" Ice 2.70	0.00	0.07
						2" Ice 3.33	0.00	0.12
HSS 4"x4"x4'	C	From Face	0.50	0.0000	127.00	No Ice 2.09	0.00	0.04
			0.00			1/2" Ice 2.39	0.00	0.05
			-1.00			1" Ice 2.70	0.00	0.07
						2" Ice 3.33	0.00	0.12
Sector Mount [SM 411-3]	A	None		0.0000	127.00	No Ice 20.53	20.53	1.07
						1/2" Ice 28.62	28.62	1.46
						1" Ice 36.63	36.63	1.97
						2" Ice 52.73	52.73	3.37
***								
AIR6449 B41_T-MOBILE	A	From Leg	4.00	0.0000	117.00	No Ice 5.27	2.03	0.11
			0.00			1/2" Ice 5.70	2.36	0.15
			2.00			1" Ice 6.14	2.70	0.20
						2" Ice 7.06	3.43	0.30
AIR6449 B41_T-MOBILE	B	From Leg	4.00	0.0000	117.00	No Ice 5.27	2.03	0.11
			0.00			1/2" Ice 5.70	2.36	0.15
			2.00			1" Ice 6.14	2.70	0.20
						2" Ice 7.06	3.43	0.30
AIR6449 B41_T-MOBILE	C	From Leg	4.00	0.0000	117.00	No Ice 5.27	2.03	0.11
			0.00			1/2" Ice 5.70	2.36	0.15
			2.00			1" Ice 6.14	2.70	0.20
						2" Ice 7.06	3.43	0.30
APXVAALL24_43-U-NA20_TMO	A	From Leg	4.00	0.0000	117.00	No Ice 14.67	5.32	0.15
			0.00			1/2" Ice 15.43	5.99	0.26
			2.00			1" Ice 16.21	6.68	0.38
						2" Ice 17.81	8.08	0.65
APXVAALL24_43-U-NA20_TMO	B	From Leg	4.00	0.0000	117.00	No Ice 14.67	5.32	0.15
			0.00			1/2" Ice 15.43	5.99	0.26
			2.00			1" Ice 16.21	6.68	0.38
						2" Ice 17.81	8.08	0.65
APXVAALL24_43-U-NA20_TMO	C	From Leg	4.00	0.0000	117.00	No Ice 14.67	5.32	0.15
			0.00			1/2" Ice 15.43	5.99	0.26
			2.00			1" Ice 16.21	6.68	0.38
						2" Ice 17.81	8.08	0.65
RADIO 4449 B71 B85A_T-MOBILE	A	From Leg	4.00	0.0000	117.00	No Ice 1.97	1.59	0.07
			0.00			1/2" Ice 2.15	1.75	0.09
			2.00			1" Ice 2.33	1.92	0.12
						2" Ice 2.72	2.28	0.17
RADIO 4449 B71 B85A_T-MOBILE	B	From Leg	4.00	0.0000	117.00	No Ice 1.97	1.59	0.07
			0.00			1/2" Ice 2.15	1.75	0.09
			2.00			1" Ice 2.33	1.92	0.12
						2" Ice 2.72	2.28	0.17
RADIO 4449 B71 B85A_T-MOBILE	C	From Leg	4.00	0.0000	117.00	No Ice 1.97	1.59	0.07
			0.00			1/2" Ice 2.15	1.75	0.09
			2.00			1" Ice 2.33	1.92	0.12
						2" Ice 2.72	2.28	0.17
RADIO 4460 B2/B25 B66_TMO	A	From Leg	4.00	0.0000	117.00	No Ice 2.14	1.69	0.11
			0.00			1/2" Ice 2.32	1.85	0.13
			2.00			1" Ice 2.51	2.02	0.16
						2" Ice 2.91	2.39	0.22
RADIO 4460 B2/B25 B66_TMO	B	From Leg	4.00	0.0000	117.00	No Ice 2.14	1.69	0.11
			0.00			1/2" Ice 2.32	1.85	0.13
			2.00			1" Ice 2.51	2.02	0.16
						2" Ice 2.91	2.39	0.22
RADIO 4460 B2/B25 B66_TMO	C	From Leg	4.00	0.0000	117.00	No Ice 2.14	1.69	0.11

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K	
			0.00			1/2" Ice	2.32	1.85	0.13
			2.00			1" Ice	2.51	2.02	0.16
						2" Ice	2.91	2.39	0.22
(3) 8' Antenna Pipe [#P2STD]	A	From Leg	4.00	0.0000	117.00	No Ice	1.90	1.90	0.03
			0.00			1/2" Ice	2.73	2.73	0.04
			0.00			1" Ice	3.40	3.40	0.06
						2" Ice	4.40	4.40	0.12
(3) 8' Antenna Pipe [#P2STD]	B	From Leg	4.00	0.0000	117.00	No Ice	1.90	1.90	0.03
			0.00			1/2" Ice	2.73	2.73	0.04
			0.00			1" Ice	3.40	3.40	0.06
						2" Ice	4.40	4.40	0.12
(3) 8' Antenna Pipe [#P2STD]	C	From Leg	4.00	0.0000	117.00	No Ice	1.90	1.90	0.03
			0.00			1/2" Ice	2.73	2.73	0.04
			0.00			1" Ice	3.40	3.40	0.06
						2" Ice	4.40	4.40	0.12
12' HD V-Frame [#VFA12-HD]	A	From Leg	2.00	0.0000	117.00	No Ice	13.20	9.20	0.66
			0.00			1/2" Ice	19.50	14.60	0.80
			0.00			1" Ice	25.80	19.50	1.01
						2" Ice	38.40	29.30	1.44
12' HD V-Frame [#VFA12-HD]	B	From Leg	2.00	0.0000	117.00	No Ice	13.20	9.20	0.66
			0.00			1/2" Ice	19.50	14.60	0.80
			0.00			1" Ice	25.80	19.50	1.01
						2" Ice	38.40	29.30	1.44
12' HD V-Frame [#VFA12-HD]	C	From Leg	2.00	0.0000	117.00	No Ice	13.20	9.20	0.66
			0.00			1/2" Ice	19.50	14.60	0.80
			0.00			1" Ice	25.80	19.50	1.01
						2" Ice	38.40	29.30	1.44
***									
BXA-80063/4CF w/ Mount Pipe	A	From Leg	4.00	0.0000	107.00	No Ice	4.83	3.65	0.03
			0.00			1/2" Ice	5.35	4.14	0.06
			0.00			1" Ice	5.88	4.64	0.11
						2" Ice	6.98	5.70	0.22
BXA-80063/4CF w/ Mount Pipe	B	From Leg	4.00	0.0000	107.00	No Ice	4.83	3.65	0.03
			0.00			1/2" Ice	5.35	4.14	0.06
			0.00			1" Ice	5.88	4.64	0.11
						2" Ice	6.98	5.70	0.22
BXA-80063/4CF w/ Mount Pipe	C	From Leg	4.00	0.0000	107.00	No Ice	4.83	3.65	0.03
			0.00			1/2" Ice	5.35	4.14	0.06
			0.00			1" Ice	5.88	4.64	0.11
						2" Ice	6.98	5.70	0.22
RC2DC-3315-PF-48	A	From Leg	4.00	0.0000	107.00	No Ice	3.79	2.51	0.03
			0.00			1/2" Ice	4.04	2.72	0.06
			0.00			1" Ice	4.30	2.94	0.10
						2" Ice	4.84	3.41	0.18
Sector Mount [SM 402-3]	A	None		0.0000	107.00	No Ice	18.87	18.87	0.85
						1/2" Ice	26.47	26.47	1.21
						1" Ice	33.99	33.99	1.70
						2" Ice	48.84	48.84	3.04
*									
(2) MX06FRO660-03 w/ Mount Pipe	A	From Leg	4.00	0.0000	107.00	No Ice	6.54	5.55	0.10
			0.00			1/2" Ice	7.06	6.05	0.18
			0.00			1" Ice	7.60	6.57	0.28
						2" Ice	8.70	7.65	0.50
(2) MX06FRO660-03 w/ Mount Pipe	B	From Leg	4.00	0.0000	107.00	No Ice	6.54	5.55	0.10
			0.00			1/2" Ice	7.06	6.05	0.18
			0.00			1" Ice	7.60	6.57	0.28
						2" Ice	8.70	7.65	0.50
(2) MX06FRO660-03 w/ Mount Pipe	C	From Leg	4.00	0.0000	107.00	No Ice	6.54	5.55	0.10
			0.00			1/2" Ice	7.06	6.05	0.18
			0.00			1" Ice	7.60	6.57	0.28
						2" Ice	8.70	7.65	0.50
Sub6 Antenna - VZS01 w/ Mount Pipe	A	From Leg	4.00	0.0000	107.00	No Ice	4.92	2.69	0.10
			0.00			1/2" Ice	5.26	3.15	0.14
			0.00			1" Ice	5.62	3.63	0.19
						2" Ice	6.37	4.64	0.29

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K	
Sub6 Antenna - VZS01 w/ Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	4.92	2.69	0.10
						1/2" Ice	5.26	3.15	0.14
						1" Ice	5.62	3.63	0.19
						2" Ice	6.37	4.64	0.29
Sub6 Antenna - VZS01 w/ Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	4.92	2.69	0.10
						1/2" Ice	5.26	3.15	0.14
						1" Ice	5.62	3.63	0.19
						2" Ice	6.37	4.64	0.29
RFV01U-D1A	A	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	1.88	1.25	0.08
						1/2" Ice	2.05	1.39	0.10
						1" Ice	2.22	1.54	0.12
						2" Ice	2.60	1.86	0.18
RFV01U-D1A	B	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	1.88	1.25	0.08
						1/2" Ice	2.05	1.39	0.10
						1" Ice	2.22	1.54	0.12
						2" Ice	2.60	1.86	0.18
RFV01U-D1A	C	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	1.88	1.25	0.08
						1/2" Ice	2.05	1.39	0.10
						1" Ice	2.22	1.54	0.12
						2" Ice	2.60	1.86	0.18
RFV01U-D2A	A	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	1.88	1.01	0.07
						1/2" Ice	2.05	1.14	0.09
						1" Ice	2.22	1.28	0.11
						2" Ice	2.60	1.59	0.15
RFV01U-D2A	B	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	1.88	1.01	0.07
						1/2" Ice	2.05	1.14	0.09
						1" Ice	2.22	1.28	0.11
						2" Ice	2.60	1.59	0.15
RFV01U-D2A	C	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	1.88	1.01	0.07
						1/2" Ice	2.05	1.14	0.09
						1" Ice	2.22	1.28	0.11
						2" Ice	2.60	1.59	0.15
(4) Crossover Plate [#VZWSMART-MSK1]	A	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	0.05	0.05	0.01
						1/2" Ice	0.07	0.07	0.02
						1" Ice	0.09	0.09	0.02
						2" Ice	0.12	0.12	0.03
(4) Crossover Plate [#VZWSMART-MSK1]	B	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	0.05	0.05	0.01
						1/2" Ice	0.07	0.07	0.02
						1" Ice	0.09	0.09	0.02
						2" Ice	0.12	0.12	0.03
(4) Crossover Plate [#VZWSMART-MSK1]	C	From Leg	4.00 0.00 0.00	0.0000	107.00	No Ice	0.05	0.05	0.01
						1/2" Ice	0.07	0.07	0.02
						1" Ice	0.09	0.09	0.02
						2" Ice	0.12	0.12	0.03
Tieback Assembly [#VZWSMART-SFK1]	A	From Leg	2.00 0.00 0.00	0.0000	107.00	No Ice	2.38	1.19	0.08
						1/2" Ice	3.41	1.71	0.11
						1" Ice	4.45	2.22	0.13
						2" Ice	5.91	3.25	0.19
Tieback Assembly [#VZWSMART-SFK1]	B	From Leg	2.00 0.00 0.00	0.0000	107.00	No Ice	2.38	1.19	0.08
						1/2" Ice	3.41	1.71	0.11
						1" Ice	4.45	2.22	0.13
						2" Ice	5.91	3.25	0.19
Tieback Assembly [#VZWSMART-SFK1]	C	From Leg	2.00 0.00 0.00	0.0000	107.00	No Ice	2.38	1.19	0.08
						1/2" Ice	3.41	1.71	0.11
						1" Ice	4.45	2.22	0.13
						2" Ice	5.91	3.25	0.19
V-Bracing Kit [#VZWSMART-SFK3]	A	From Leg	2.00 0.00 0.00	0.0000	107.00	No Ice	4.24	3.96	0.13
						1/2" Ice	4.78	4.46	0.18
						1" Ice	5.44	5.06	0.22
						2" Ice	6.76	6.26	0.31
V-Bracing Kit [#VZWSMART-SFK3]	B	From Leg	2.00 0.00 0.00	0.0000	107.00	No Ice	4.24	3.96	0.13
						1/2" Ice	4.78	4.46	0.18
						1" Ice	5.44	5.06	0.22
						2" Ice	6.76	6.26	0.31

Description	Face or Leg	Offset Type	Offsets: Horiz Lateral Vert ft ft ft	Azimuth Adjustment t °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K	
V-Bracing Kit [#VZWSMART-SFK3]	C	From Leg	2.00	0.0000	107.00	No Ice	4.24	3.96	0.13
			0.00			1/2" Ice	4.78	4.46	0.18
			0.00			1" Ice	5.44	5.06	0.22
						2" Ice	6.76	6.26	0.31
***									
DMP65R-BU8D w/ Mount Pipe	A	From Leg	4.00	0.0000	97.00	No Ice	15.89	7.89	0.14
			0.00			1/2" Ice	16.81	8.74	0.25
			0.00			1" Ice	17.76	9.60	0.38
						2" Ice	19.70	11.37	0.68
DMP65R-BU4D w/ Mount Pipe	B	From Leg	4.00	0.0000	97.00	No Ice	7.53	3.79	0.09
			0.00			1/2" Ice	8.04	4.23	0.16
			0.00			1" Ice	8.57	4.68	0.22
						2" Ice	9.68	5.63	0.39
DMP65R-BU6D w/ Mount Pipe	C	From Leg	4.00	0.0000	97.00	No Ice	11.96	5.97	0.11
			0.00			1/2" Ice	12.70	6.63	0.20
			0.00			1" Ice	13.46	7.30	0.30
						2" Ice	15.02	8.69	0.53
OPA65R-BU8D w/ Mount Pipe	A	From Leg	4.00	0.0000	97.00	No Ice	17.46	8.58	0.11
			0.00			1/2" Ice	18.46	9.49	0.22
			0.00			1" Ice	19.48	10.42	0.35
						2" Ice	21.58	12.33	0.66
OPA65R-BU4D w/ Mount Pipe	B	From Leg	4.00	0.0000	97.00	No Ice	8.10	4.03	0.08
			0.00			1/2" Ice	8.65	4.50	0.14
			0.00			1" Ice	9.21	4.98	0.21
						2" Ice	10.39	5.98	0.38
OPA65R-BU6D w/ Mount Pipe	C	From Leg	4.00	0.0000	97.00	No Ice	12.25	6.05	0.09
			0.00			1/2" Ice	13.00	6.71	0.18
			0.00			1" Ice	13.76	7.39	0.27
						2" Ice	15.34	8.79	0.51
7770.00 w/ Mount Pipe	A	From Leg	4.00	0.0000	97.00	No Ice	5.75	4.25	0.06
			0.00			1/2" Ice	6.18	5.01	0.10
			0.00			1" Ice	6.61	5.71	0.16
						2" Ice	7.49	7.16	0.29
7770.00 w/ Mount Pipe	B	From Leg	4.00	0.0000	97.00	No Ice	5.75	4.25	0.06
			0.00			1/2" Ice	6.18	5.01	0.10
			0.00			1" Ice	6.61	5.71	0.16
						2" Ice	7.49	7.16	0.29
7770.00 w/ Mount Pipe	C	From Leg	4.00	0.0000	97.00	No Ice	5.75	4.25	0.06
			0.00			1/2" Ice	6.18	5.01	0.10
			0.00			1" Ice	6.61	5.71	0.16
						2" Ice	7.49	7.16	0.29
RRUS 4449 B5/B12	A	From Leg	4.00	0.0000	97.00	No Ice	1.97	1.41	0.07
			0.00			1/2" Ice	2.14	1.56	0.09
			0.00			1" Ice	2.33	1.73	0.11
						2" Ice	2.72	2.07	0.16
RRUS 4449 B5/B12	B	From Leg	4.00	0.0000	97.00	No Ice	1.97	1.41	0.07
			0.00			1/2" Ice	2.14	1.56	0.09
			0.00			1" Ice	2.33	1.73	0.11
						2" Ice	2.72	2.07	0.16
RRUS 4449 B5/B12	C	From Leg	4.00	0.0000	97.00	No Ice	1.97	1.41	0.07
			0.00			1/2" Ice	2.14	1.56	0.09
			0.00			1" Ice	2.33	1.73	0.11
						2" Ice	2.72	2.07	0.16
RRUS 4478 B14_CCIV2	A	From Leg	4.00	0.0000	97.00	No Ice	2.02	1.25	0.06
			0.00			1/2" Ice	2.20	1.40	0.08
			0.00			1" Ice	2.39	1.55	0.10
						2" Ice	2.78	1.89	0.15
RRUS 4478 B14_CCIV2	B	From Leg	4.00	0.0000	97.00	No Ice	2.02	1.25	0.06
			0.00			1/2" Ice	2.20	1.40	0.08
			0.00			1" Ice	2.39	1.55	0.10
						2" Ice	2.78	1.89	0.15
RRUS 4478 B14_CCIV2	C	From Leg	4.00	0.0000	97.00	No Ice	2.02	1.25	0.06
			0.00			1/2" Ice	2.20	1.40	0.08
			0.00			1" Ice	2.39	1.55	0.10
						2" Ice	2.78	1.89	0.15



Description	Face or Leg	Offset Type	Offsets: Horiz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K	
RRUS 8843 B2/B66A_CCIV2	A	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	1.98	1.70	0.07
						1/2" Ice	2.16	1.86	0.10
						1" Ice	2.34	2.04	0.12
						2" Ice	2.73	2.41	0.18
RRUS 8843 B2/B66A_CCIV2	B	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	1.98	1.70	0.07
						1/2" Ice	2.16	1.86	0.10
						1" Ice	2.34	2.04	0.12
						2" Ice	2.73	2.41	0.18
RRUS 8843 B2/B66A_CCIV2	C	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	1.98	1.70	0.07
						1/2" Ice	2.16	1.86	0.10
						1" Ice	2.34	2.04	0.12
						2" Ice	2.73	2.41	0.18
(2) LGP21401	A	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	1.10	0.21	0.01
						1/2" Ice	1.24	0.27	0.02
						1" Ice	1.38	0.35	0.03
						2" Ice	1.69	0.52	0.05
(2) LGP21401	B	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	1.10	0.21	0.01
						1/2" Ice	1.24	0.27	0.02
						1" Ice	1.38	0.35	0.03
						2" Ice	1.69	0.52	0.05
(2) LGP21401	C	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	1.10	0.21	0.01
						1/2" Ice	1.24	0.27	0.02
						1" Ice	1.38	0.35	0.03
						2" Ice	1.69	0.52	0.05
DC9-48-60-24-8C-EV	A	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	2.74	4.78	0.03
						1/2" Ice	2.96	5.06	0.06
						1" Ice	3.20	5.35	0.10
						2" Ice	3.68	5.95	0.20
DC6-48-60-18-8F	B	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	0.92	0.92	0.02
						1/2" Ice	1.46	1.46	0.04
						1" Ice	1.64	1.64	0.06
						2" Ice	2.04	2.04	0.11
8' x 2" Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	1.90	1.90	0.03
						1/2" Ice	2.73	2.73	0.04
						1" Ice	3.40	3.40	0.06
						2" Ice	4.40	4.40	0.12
8' x 2" Mount Pipe	B	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	1.90	1.90	0.03
						1/2" Ice	2.73	2.73	0.04
						1" Ice	3.40	3.40	0.06
						2" Ice	4.40	4.40	0.12
8' x 2" Mount Pipe	C	From Leg	4.00 0.00 0.00	0.0000	97.00	No Ice	1.90	1.90	0.03
						1/2" Ice	2.73	2.73	0.04
						1" Ice	3.40	3.40	0.06
						2" Ice	4.40	4.40	0.12
Sector Mount [SM 504-3]	A	None		0.0000	97.00	No Ice	31.05	31.05	1.71
						1/2" Ice	43.83	43.83	2.33
						1" Ice	56.44	56.44	3.14
						2" Ice	81.28	81.28	5.36
*** 800 10504 w/ Mount Pipe	A	From Leg	4.00 0.00 2.00	0.0000	87.00	No Ice	2.69	2.26	0.04
						1/2" Ice	3.12	2.68	0.07
						1" Ice	3.56	3.12	0.11
						2" Ice	4.49	4.03	0.21
800 10504 w/ Mount Pipe	B	From Leg	4.00 0.00 2.00	0.0000	87.00	No Ice	2.69	2.26	0.04
						1/2" Ice	3.12	2.68	0.07
						1" Ice	3.56	3.12	0.11
						2" Ice	4.49	4.03	0.21
800 10504 w/ Mount Pipe	C	From Leg	4.00 0.00 2.00	0.0000	87.00	No Ice	2.69	2.26	0.04
						1/2" Ice	3.12	2.68	0.07
						1" Ice	3.56	3.12	0.11
						2" Ice	4.49	4.03	0.21
6' x 2" Mount Pipe	A	From Leg	4.00 0.00 0.00	0.0000	87.00	No Ice	1.43	1.43	0.02
						1/2" Ice	1.92	1.92	0.03
						1" Ice	2.29	2.29	0.05
						2" Ice	3.06	3.06	0.09

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>Front</sub>	C <sub>A</sub> A <sub>Side</sub>	Weight
			Horz Lateral	Vert					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
6' x 2" Mount Pipe	B	From Leg	4.00	0.0000	87.00	No Ice	1.43	1.43	0.02
			0.00			1/2" Ice	1.92	1.92	0.03
			0.00			1" Ice	2.29	2.29	0.05
6' x 2" Mount Pipe	C	From Leg	4.00	0.0000	87.00	No Ice	1.43	1.43	0.02
			0.00			1/2" Ice	1.92	1.92	0.03
			0.00			1" Ice	2.29	2.29	0.05
Sector Mount [SM 104-3]	A	None		0.0000	87.00	No Ice	30.21	30.21	0.95
						1/2" Ice	38.12	38.12	1.43
						1" Ice	46.01	46.01	2.03
***						2" Ice	62.03	62.03	3.58
GPS_A	C	From Leg	3.00	0.0000	80.00	No Ice	0.26	0.26	0.00
			0.00			1/2" Ice	0.32	0.32	0.00
			1.00			1" Ice	0.39	0.39	0.01
Side Arm Mount [SO 701-1]	C	From Leg	1.50	0.0000	80.00	No Ice	0.85	1.67	0.07
			0.00			1/2" Ice	1.14	2.34	0.08
			0.00			1" Ice	1.43	3.01	0.09
***						2" Ice	2.01	4.35	0.12
GPS_A	B	From Leg	3.00	0.0000	72.00	No Ice	0.26	0.26	0.00
			0.00			1/2" Ice	0.32	0.32	0.00
			0.00			1" Ice	0.39	0.39	0.01
GPS_A	C	From Leg	3.00	0.0000	72.00	No Ice	0.26	0.26	0.00
			0.00			1/2" Ice	0.32	0.32	0.00
			0.00			1" Ice	0.39	0.39	0.01
Side Arm Mount [SO 701-1]	B	From Leg	1.50	0.0000	72.00	No Ice	0.85	1.67	0.07
			0.00			1/2" Ice	1.14	2.34	0.08
			0.00			1" Ice	1.43	3.01	0.09
Side Arm Mount [SO 701-1]	C	From Leg	1.50	0.0000	72.00	No Ice	0.85	1.67	0.07
			0.00			1/2" Ice	1.14	2.34	0.08
			0.00			1" Ice	1.43	3.01	0.09
***						2" Ice	2.01	4.35	0.12

### Truss-Leg Properties

Section Designation	Area	Area Ice	Self Weight	Ice Weight	Equiv. Diamete r	Equiv. Diamete r Ice	Leg Area
	in <sup>2</sup>	in <sup>2</sup>	K	K	in	in	in <sup>2</sup>
PiRod 105244 w/ (2) 1-1/4" Tie Rod	1076.4663	2615.0684	0.73	0.50	7.4755	18.1602	6.1379
PiRod 105217	2296.2363	5959.0264	0.59	0.67	7.9730	20.6911	5.3014
PiRod 105218	2425.3141	5998.6741	0.72	0.65	8.4212	20.8287	7.2158
PiRod 105218	2425.3141	5951.5920	0.72	0.60	8.4212	20.6652	7.2158
PiRod 105219	2597.9095	5930.1266	1.09	0.56	9.0205	20.5907	9.4248

## Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

## Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	136 - 133.625	Leg	Max Tension	1	0.00	0.000	0.000
			Max. Compression	31	-2.83	-0.031	-0.017
			Max. Mx	8	-1.66	-0.153	-0.002
			Max. My	2	-1.67	0.004	0.157
			Max. Vy	8	0.23	-0.153	-0.002
			Max. Vx	3	-0.24	0.002	0.152
		Diagonal	Max Tension	9	0.93	0.000	0.000

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T2	133.625 - 130	Top Girt	Max. Compression	8	-1.00	0.000	0.000	
			Max. Mx	26	-0.08	0.005	0.000	
			Max. Vy	26	0.01	0.000	0.000	
			Max Tension	7	0.72	0.000	0.000	
			Max. Compression	18	-0.72	-0.051	0.000	
			Max. Mx	33	-0.08	-0.077	0.000	
			Max. My	3	0.36	0.011	-0.000	
			Max. Vy	33	0.06	-0.077	0.000	
		Leg	Max. Vx	10	-0.00	0.000	0.000	
			Max Tension	15	3.22	0.008	0.477	
			Diagonal	Max. Compression	10	-5.87	-0.458	-0.264
				Max. Mx	8	-5.26	-0.514	-0.014
				Max. My	2	-5.80	0.010	0.531
				Max. Vy	8	0.99	-0.514	-0.014
				Max. Vx	2	-1.02	0.010	0.531
				Max Tension	25	1.37	0.000	0.000
		Max. Compression		24	-1.48	0.000	0.000	
		Max. Mx		33	0.16	-0.004	-0.000	
		Horizontal	Max. My	20	1.34	-0.001	-0.000	
			Max. Vy	33	0.01	-0.004	-0.000	
			Max. Vx	20	0.00	-0.001	-0.000	
			Max Tension	2	0.34	0.000	0.000	
			Max. Compression	15	-0.21	0.000	0.000	
			Max. Mx	26	0.15	0.012	0.000	
Max. Vy	26		0.01	0.000	0.000			
Max Tension	14		0.71	0.000	0.000			
Bottom Girt	Max. Compression	3	-0.63	0.000	0.000			
	Max. Mx	26	0.12	0.014	0.000			
	Max. Vy	26	0.01	0.000	0.000			
	Leg	Max Tension	23	33.32	1.701	0.050		
		Max. Compression	10	-41.54	-0.240	-0.007		
		Max. Mx	10	-41.53	-1.707	-0.050		
		Max. My	20	-4.33	-0.001	1.510		
		Max. Vy	10	-2.94	-0.240	-0.007		
Max. Vx		20	2.56	0.001	0.230			
Diagonal		Max Tension	25	4.08	0.000	0.000		
		Max. Compression	24	-4.24	0.000	0.000		
	Max. Mx	32	0.84	-0.005	0.000			
	Max. My	24	-4.22	-0.001	0.000			
	Max. Vy	32	0.01	-0.005	0.000			
	Max. Vx	24	-0.00	0.000	0.000			
	Horizontal	Max Tension	22	0.59	0.000	0.000		
		Max. Compression	11	-0.41	0.000	0.000		
Max. Mx		26	0.28	0.014	0.000			
Max. Vy		26	-0.01	0.000	0.000			
Top Girt		Max Tension	2	0.64	0.000	0.000		
		Max. Compression	15	-0.61	0.000	0.000		
		Max. Mx	26	0.03	0.013	0.000		
		Max. Vy	26	-0.01	0.000	0.000		
	Bottom Girt	Max Tension	22	1.89	0.000	0.000		
		Max. Compression	11	-1.76	0.000	0.000		
		Max. Mx	26	0.17	0.017	0.000		
		Max. Vy	26	-0.02	0.000	0.000		
T4		110 - 94.9427	Leg	Max Tension	23	69.29	-0.200	-0.013
				Max. Compression	10	-82.44	0.012	0.012
				Max. Mx	10	-45.07	2.578	0.077
				Max. My	20	-4.48	0.006	-2.229
	Max. Vy			10	-2.94	2.578	0.077	
	Max. Vx			20	2.57	0.006	-2.229	
	Diagonal			Max Tension	13	5.68	0.000	0.000
				Max. Compression	24	-5.71	0.000	0.000
			Max. Mx	27	1.08	-0.008	-0.000	
			Max. My	12	-4.73	-0.001	-0.001	
			Max. Vy	27	0.01	-0.008	-0.000	
			Max. Vx	12	0.00	-0.001	-0.001	
			Top Girt	Max Tension	10	1.79	0.000	0.000
				Max. Compression	23	-1.75	0.000	0.000

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft			
T5	94.9427 - 92.5938	Leg	Max. Mx	26	0.05	0.019	0.000			
			Max. Vy	26	-0.02	0.000	0.000			
			Max Tension	23	76.69	-0.033	-0.013			
		Diagonal	Max. Compression	10	-91.06	-0.243	-0.001			
			Max. Mx	10	-91.02	0.309	-0.001			
			Max. My	8	-8.11	-0.012	0.238			
			Max. Vy	10	0.47	0.309	-0.002			
			Max. Vx	8	0.25	-0.012	0.238			
			Max Tension	25	5.94	-0.004	0.000			
			Max. Compression	12	-6.04	0.000	0.000			
			Max. Mx	27	1.11	-0.008	-0.000			
			Max. My	24	2.43	-0.007	0.001			
			Max. Vy	27	0.01	-0.008	-0.000			
			Max. Vx	24	-0.00	0.000	0.000			
			Secondary Horizontal	Max Tension	6	0.60	0.000	0.000		
				Max. Compression	19	-0.49	-0.001	-0.001		
				Max. Mx	36	0.23	-0.010	-0.000		
			T6	92.5938 - 90	Leg	Max. My	4	-0.47	-0.004	-0.001
Max. Vy	36	-0.02				-0.010	-0.000			
Max. Vx	16	0.00				-0.004	-0.001			
Diagonal	Max Tension	23			88.77	1.190	0.057			
	Max. Compression	10			-103.76	3.168	0.144			
	Max. Mx	10			-103.76	3.168	0.144			
	Max. My	8			-8.46	0.050	1.417			
	Max. Vy	18			-7.46	3.165	-0.022			
	Max. Vx	8			-3.30	0.050	1.417			
	Max Tension	13			6.87	-0.004	-0.000			
	Max. Compression	24			-7.03	0.000	0.000			
	Max. Mx	27			0.95	-0.008	-0.000			
	Max. My	10			-0.07	-0.004	-0.001			
	Max. Vy	27			0.01	-0.008	-0.000			
	Max. Vx	10			-0.00	0.000	0.000			
	Secondary Horizontal	Max Tension			10	1.46	-0.006	0.000		
		Max. Compression			23	-1.37	0.000	0.000		
		Max. Mx			31	-0.13	-0.012	-0.001		
Bottom Girt	Max. My	24	-1.34	-0.008	-0.001					
	Max. Vy	31	0.02	-0.012	-0.001					
	Max. Vx	24	-0.00	0.000	0.000					
	Max Tension	22	0.83	0.000	0.000					
	Max. Compression	11	-0.77	0.000	0.000					
	Max. Mx	26	0.07	0.023	0.000					
	Max. Vy	26	-0.02	0.000	0.000					
	T7	90 - 80	Leg	Max Tension	23	96.36	-3.090	-0.145		
				Max. Compression	10	-111.72	3.411	0.000		
				Max. Mx	22	93.53	-4.089	-0.013		
			Diagonal	Max. My	12	-8.85	-0.347	-6.825		
				Max. Vy	22	0.34	-4.089	-0.013		
Max. Vx				4	0.83	-0.355	-6.786			
Max Tension				23	7.58	0.107	-0.031			
Max. Compression				10	-8.25	0.000	0.000			
Max. Mx				10	-6.81	-0.109	-0.019			
Max. My				12	-7.14	-0.079	0.036			
Max. Vy				28	-0.03	0.048	-0.000			
Max. Vx				12	-0.01	0.000	0.000			
T8				80 - 60	Leg	Max Tension	23	134.31	-5.926	-0.042
						Max. Compression	10	-154.00	5.837	0.077
						Max. Mx	10	-153.58	6.104	0.049
					Diagonal	Max. My	12	-10.21	-0.347	-6.825
						Max. Vy	2	-0.39	6.010	-0.005
						Max. Vx	4	-0.27	-0.355	-6.786
	Max Tension	24	6.86			0.064	0.002			
	Max. Compression	25	-6.67			0.000	0.000			
	Max. Mx	10	5.52			0.088	0.007			
	Max. My	4	5.69			0.064	-0.009			
	Max. Vy	31	-0.03			0.061	-0.007			
	Max. Vx	28	0.00			0.000	0.000			

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T9	60 - 40	Leg	Max Tension	7	165.56	-5.558	-0.011
			Max. Compression	10	-188.77	5.504	0.030
			Max. Mx	22	147.93	-5.858	-0.072
			Max. My	8	-11.53	-0.029	5.641
			Max. Vy	22	-0.14	-5.858	-0.072
			Max. Vx	16	0.22	-0.020	5.637
		Diagonal	Max Tension	20	6.22	0.000	0.000
			Max. Compression	20	-6.22	0.000	0.000
			Max. Mx	10	4.92	0.104	0.008
			Max. My	16	5.45	0.088	0.010
			Max. Vy	29	0.04	0.069	-0.010
			Max. Vx	34	-0.00	0.000	0.000
T10	40 - 20	Leg	Max Tension	7	192.83	-4.904	0.010
			Max. Compression	18	-219.27	6.850	0.163
			Max. Mx	18	-219.27	6.850	0.163
			Max. My	8	-13.66	-0.208	5.963
			Max. Vy	37	0.41	-3.873	-0.015
			Max. Vx	16	0.29	-0.195	5.835
		Diagonal	Max Tension	20	6.12	0.000	0.000
			Max. Compression	18	-6.34	0.000	0.000
			Max. Mx	10	4.79	0.096	0.006
			Max. My	28	1.65	0.088	-0.012
			Max. Vy	29	0.05	0.080	0.011
			Max. Vx	28	0.00	0.000	0.000
T11	20 - 0	Leg	Max Tension	7	216.76	-5.142	0.007
			Max. Compression	18	-247.63	0.000	0.000
			Max. Mx	18	-234.59	6.850	0.163
			Max. My	8	-15.94	-0.486	9.052
			Max. Vy	37	-0.72	-3.873	-0.015
			Max. Vx	16	1.03	-0.474	8.825
		Diagonal	Max Tension	7	7.83	0.000	0.000
			Max. Compression	18	-8.54	0.000	0.000
			Max. Mx	18	4.03	0.136	0.021
			Max. My	16	5.69	0.115	0.023
			Max. Vy	29	0.06	0.126	-0.014
			Max. Vx	28	0.00	0.000	0.000

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	18	256.84	22.13	-12.16
	Max. H <sub>x</sub>	18	256.84	22.13	-12.16
	Max. H <sub>z</sub>	7	-224.27	-19.55	10.69
	Min. Vert	7	-224.27	-19.55	10.69
	Min. H <sub>x</sub>	7	-224.27	-19.55	10.69
	Min. H <sub>z</sub>	18	256.84	22.13	-12.16
Leg B	Max. Vert	10	255.64	-21.44	-12.69
	Max. H <sub>x</sub>	23	-222.25	18.86	11.20
	Max. H <sub>z</sub>	23	-222.25	18.86	11.20
	Min. Vert	23	-222.25	18.86	11.20
	Min. H <sub>x</sub>	10	255.64	-21.44	-12.69
	Min. H <sub>z</sub>	10	255.64	-21.44	-12.69
Leg A	Max. Vert	2	248.17	0.32	24.06
	Max. H <sub>x</sub>	4	218.70	0.55	21.20
	Max. H <sub>z</sub>	2	248.17	0.32	24.06
	Min. Vert	15	-215.91	-0.31	-21.10
	Min. H <sub>x</sub>	17	-189.26	-0.54	-18.69
	Min. H <sub>z</sub>	15	-215.91	-0.31	-21.10

## Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	42.73	0.00	0.00	2.764	-2.730	0.000
1.2 Dead+1.0 Wind 0 deg - No Ice	51.27	0.04	-34.29	-2801.719	-7.017	7.574
0.9 Dead+1.0 Wind 0 deg - No Ice	38.46	0.04	-34.29	-2802.548	-6.198	7.574
1.2 Dead+1.0 Wind 30 deg - No Ice	51.27	17.24	-29.98	-2444.319	-1409.670	16.425
0.9 Dead+1.0 Wind 30 deg - No Ice	38.46	17.24	-29.98	-2445.148	-1408.851	16.425
1.2 Dead+1.0 Wind 60 deg - No Ice	51.27	30.70	-17.84	-1441.832	-2487.177	12.724
0.9 Dead+1.0 Wind 60 deg - No Ice	38.46	30.70	-17.84	-1442.662	-2486.358	12.724
1.2 Dead+1.0 Wind 90 deg - No Ice	51.27	36.12	-0.04	-0.424	-2911.235	0.357
0.9 Dead+1.0 Wind 90 deg - No Ice	38.46	36.12	-0.04	-1.253	-2910.416	0.357
1.2 Dead+1.0 Wind 120 deg - No Ice	51.27	30.85	17.89	1452.226	-2501.170	-6.597
0.9 Dead+1.0 Wind 120 deg - No Ice	38.46	30.85	17.89	1451.397	-2500.351	-6.597
1.2 Dead+1.0 Wind 150 deg - No Ice	51.27	16.81	29.31	2424.296	-1389.960	-4.317
0.9 Dead+1.0 Wind 150 deg - No Ice	38.46	16.81	29.31	2423.467	-1389.141	-4.317
1.2 Dead+1.0 Wind 180 deg - No Ice	51.27	-0.04	33.54	2774.034	0.465	-7.574
0.9 Dead+1.0 Wind 180 deg - No Ice	38.46	-0.04	33.54	2773.204	1.284	-7.574
1.2 Dead+1.0 Wind 210 deg - No Ice	51.27	-17.24	29.98	2450.953	1403.118	-16.425
0.9 Dead+1.0 Wind 210 deg - No Ice	38.46	-17.24	29.98	2450.124	1403.937	-16.425
1.2 Dead+1.0 Wind 240 deg - No Ice	51.27	-31.35	18.22	1465.626	2510.346	-12.724
0.9 Dead+1.0 Wind 240 deg - No Ice	38.46	-31.35	18.22	1464.797	2511.165	-12.724
1.2 Dead+1.0 Wind 270 deg - No Ice	51.27	-36.12	0.04	7.058	2904.682	-0.357
0.9 Dead+1.0 Wind 270 deg - No Ice	38.46	-36.12	0.04	6.229	2905.501	-0.357
1.2 Dead+1.0 Wind 300 deg - No Ice	51.27	-30.20	-17.51	-1428.432	2464.896	6.597
0.9 Dead+1.0 Wind 300 deg - No Ice	38.46	-30.20	-17.51	-1429.261	2465.715	6.597
1.2 Dead+1.0 Wind 330 deg - No Ice	51.27	-16.81	-29.31	-2417.662	1383.407	4.317
0.9 Dead+1.0 Wind 330 deg - No Ice	38.46	-16.81	-29.31	-2418.491	1384.226	4.317
1.2 Dead+1.0 Ice+1.0 Temp	112.82	0.00	0.00	6.446	-20.553	0.000
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	112.82	0.00	-8.13	-659.898	-20.923	1.121
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	112.82	4.10	-7.13	-574.691	-354.832	2.608
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	112.82	7.25	-4.20	-333.486	-606.444	2.376
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	112.82	8.42	-0.00	6.076	-700.058	0.433
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	112.82	7.21	4.18	345.176	-605.103	-0.605
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	112.82	4.04	7.02	583.131	-351.834	-0.408
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	112.82	-0.00	8.05	669.198	-20.183	-1.121
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	112.82	-4.10	7.13	587.583	313.727	-2.608

Load Combination	Vertical	Shear <sub>x</sub>	Shear <sub>z</sub>	Overturning Moment, M <sub>x</sub>	Overturning Moment, M <sub>z</sub>	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	112.82	-7.31	4.24	348.174	568.450	-2.376
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	112.82	-8.42	0.00	6.816	658.952	-0.433
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	112.82	-7.14	-4.14	-330.489	560.887	0.605
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	112.82	-4.04	-7.02	-570.239	310.729	0.408
Dead+Wind 0 deg - Service	42.73	0.01	-7.13	-580.568	-3.508	1.580
Dead+Wind 30 deg - Service	42.73	3.58	-6.23	-506.243	-295.203	3.422
Dead+Wind 60 deg - Service	42.73	6.38	-3.71	-297.766	-519.278	2.651
Dead+Wind 90 deg - Service	42.73	7.51	-0.01	1.986	-607.463	0.077
Dead+Wind 120 deg - Service	42.73	6.42	3.72	304.077	-522.188	-1.372
Dead+Wind 150 deg - Service	42.73	3.50	6.10	506.229	-291.104	-0.901
Dead+Wind 180 deg - Service	42.73	-0.01	6.98	578.961	-1.952	-1.580
Dead+Wind 210 deg - Service	42.73	-3.58	6.23	511.772	289.742	-3.422
Dead+Wind 240 deg - Service	42.73	-6.52	3.79	306.863	519.997	-2.651
Dead+Wind 270 deg - Service	42.73	-7.51	0.01	3.542	602.003	-0.077
Dead+Wind 300 deg - Service	42.73	-6.28	-3.64	-294.980	510.547	1.372
Dead+Wind 330 deg - Service	42.73	-3.50	-6.10	-500.700	285.644	0.901

## Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-42.73	0.00	0.00	42.73	0.00	0.000%
2	0.04	-51.27	-34.29	-0.04	51.27	34.29	0.000%
3	0.04	-38.46	-34.29	-0.04	38.46	34.29	0.000%
4	17.24	-51.27	-29.98	-17.24	51.27	29.98	0.000%
5	17.24	-38.46	-29.98	-17.24	38.46	29.98	0.000%
6	30.70	-51.27	-17.84	-30.70	51.27	17.84	0.000%
7	30.70	-38.46	-17.84	-30.70	38.46	17.84	0.000%
8	36.12	-51.27	-0.04	-36.12	51.27	0.04	0.000%
9	36.12	-38.46	-0.04	-36.12	38.46	0.04	0.000%
10	30.85	-51.27	17.89	-30.85	51.27	-17.89	0.000%
11	30.85	-38.46	17.89	-30.85	38.46	-17.89	0.000%
12	16.81	-51.27	29.31	-16.81	51.27	-29.31	0.000%
13	16.81	-38.46	29.31	-16.81	38.46	-29.31	0.000%
14	-0.04	-51.27	33.54	0.04	51.27	-33.54	0.000%
15	-0.04	-38.46	33.54	0.04	38.46	-33.54	0.000%
16	-17.24	-51.27	29.98	17.24	51.27	-29.98	0.000%
17	-17.24	-38.46	29.98	17.24	38.46	-29.98	0.000%
18	-31.35	-51.27	18.22	31.35	51.27	-18.22	0.000%
19	-31.35	-38.46	18.22	31.35	38.46	-18.22	0.000%
20	-36.12	-51.27	0.04	36.12	51.27	-0.04	0.000%
21	-36.12	-38.46	0.04	36.12	38.46	-0.04	0.000%
22	-30.20	-51.27	-17.51	30.20	51.27	17.51	0.000%
23	-30.20	-38.46	-17.51	30.20	38.46	17.51	0.000%
24	-16.81	-51.27	-29.31	16.81	51.27	29.31	0.000%
25	-16.81	-38.46	-29.31	16.81	38.46	29.31	0.000%
26	0.00	-112.82	0.00	-0.00	112.82	0.00	0.000%
27	0.00	-112.82	-8.13	-0.00	112.82	8.13	0.000%
28	4.10	-112.82	-7.13	-4.10	112.82	7.13	0.000%
29	7.25	-112.82	-4.20	-7.25	112.82	4.20	0.000%
30	8.42	-112.82	-0.00	-8.42	112.82	0.00	0.000%
31	7.21	-112.82	4.18	-7.21	112.82	-4.18	0.000%
32	4.04	-112.82	7.02	-4.04	112.82	-7.02	0.000%



Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
33	-0.00	-112.82	8.05	0.00	112.82	-8.05	0.000%
34	-4.10	-112.82	7.13	4.10	112.82	-7.13	0.000%
35	-7.31	-112.82	4.24	7.31	112.82	-4.24	0.000%
36	-8.42	-112.82	0.00	8.42	112.82	-0.00	0.000%
37	-7.14	-112.82	-4.14	7.14	112.82	4.14	0.000%
38	-4.04	-112.82	-7.02	4.04	112.82	7.02	0.000%
39	0.01	-42.73	-7.13	-0.01	42.73	7.13	0.000%
40	3.58	-42.73	-6.23	-3.58	42.73	6.23	0.000%
41	6.38	-42.73	-3.71	-6.38	42.73	3.71	0.000%
42	7.51	-42.73	-0.01	-7.51	42.73	0.01	0.000%
43	6.42	-42.73	3.72	-6.42	42.73	-3.72	0.000%
44	3.50	-42.73	6.10	-3.50	42.73	-6.10	0.000%
45	-0.01	-42.73	6.98	0.01	42.73	-6.98	0.000%
46	-3.58	-42.73	6.23	3.58	42.73	-6.23	0.000%
47	-6.52	-42.73	3.79	6.52	42.73	-3.79	0.000%
48	-7.51	-42.73	0.01	7.51	42.73	-0.01	0.000%
49	-6.28	-42.73	-3.64	6.28	42.73	3.64	0.000%
50	-3.50	-42.73	-6.10	3.50	42.73	6.10	0.000%

**Maximum Tower Deflections - Service Wind**

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	136 - 133.625	3.661	43	0.2349	0.0210
T2	133.625 - 130	3.543	43	0.2348	0.0210
T3	130 - 110	3.342	43	0.2335	0.0212
T4	110 - 94.9427	2.357	42	0.2151	0.0192
T5	94.9427 - 92.5938	1.683	42	0.1839	0.0152
T6	92.5938 - 90	1.591	42	0.1769	0.0147
T7	90 - 80	1.491	42	0.1684	0.0146
T8	80 - 60	1.145	42	0.1470	0.0132
T9	60 - 40	0.611	42	0.0971	0.0090
T10	40 - 20	0.262	42	0.0609	0.0054
T11	20 - 0	0.064	42	0.0259	0.0021

**Critical Deflections and Radius of Curvature - Service Wind**

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
136.00	APXVSP18-C-A20 w/ Mount Pipe	43	3.661	0.2349	0.0210	28211
134.00	1900MHZ RRH (65MHZ)	43	3.563	0.2349	0.0210	28211
127.00	(4) DB844H90E-XY w/ Mount Pipe	43	3.178	0.2319	0.0213	14008
117.00	AIR6449 B41_T-MOBILE	43	2.683	0.2239	0.0205	42793
107.00	BXA-80063/4CF w/ Mount Pipe	42	2.217	0.2105	0.0185	152517
97.00	DMP65R-BU8D w/ Mount Pipe	42	1.767	0.1894	0.0158	15256
87.00	800 10504 w/ Mount Pipe	42	1.380	0.1605	0.0143	18208
80.00	GPS_A	42	1.145	0.1470	0.0132	22937
72.00	GPS_A	42	0.908	0.1282	0.0116	24124

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	136 - 133.625	17.534	10	1.1236	0.1009
T2	133.625 - 130	16.971	10	1.1232	0.1010
T3	130 - 110	16.008	10	1.1173	0.1019
T4	110 - 94.9427	11.300	8	1.0301	0.0924
T5	94.9427 - 92.5938	8.069	8	0.8806	0.0729
T6	92.5938 - 90	7.627	8	0.8469	0.0701
T7	90 - 80	7.148	8	0.8064	0.0698
T8	80 - 60	5.491	8	0.7044	0.0632
T9	60 - 40	2.930	8	0.4653	0.0433
T10	40 - 20	1.258	8	0.2919	0.0259
T11	20 - 0	0.305	19	0.1240	0.0099

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
136.00	APXVSP18-C-A20 w/ Mount Pipe	10	17.534	1.1236	0.1009	5886
134.00	1900MHZ RRH (65MHZ)	10	17.063	1.1235	0.1009	5886
127.00	(4) DB844H90E-XY w/ Mount Pipe	10	15.224	1.1098	0.1023	2910
117.00	AIR6449 B41_T-MOBILE	8	12.857	1.0717	0.0985	8954
107.00	BXA-80063/4CF w/ Mount Pipe	8	10.630	1.0077	0.0891	32968
97.00	DMP65R-BU8D w/ Mount Pipe	8	8.474	0.9072	0.0760	3180
87.00	800 10504 w/ Mount Pipe	8	6.619	0.7686	0.0685	3799
80.00	GPS_A	8	5.491	0.7044	0.0632	4788
72.00	GPS_A	8	4.353	0.6143	0.0554	5036

### Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load per Bolt K	Ratio Load Allowable	Allowable Ratio	Criteria
T2	133.625	Leg	A325N	0.6250	5	1.17	27.61	0.043	1.05	Bolt DS
T3	130	Leg	A325N	0.7500	5	8.31	39.76	0.209	1.05	Bolt DS
T6	92.5938	Leg	A325N	1.0000	6	14.79	54.52	0.271	1.05	Bolt Tension
T7	90	Leg	A325N	1.0000	6	16.06	54.52	0.295	1.05	Bolt Tension
		Diagonal	A325N	1.0000	1	7.58	10.16	0.746	1.05	Member Block Shear
T8	80	Leg	A325N	1.0000	6	22.39	54.52	0.411	1.05	Bolt Tension
		Diagonal	A325N	1.0000	1	6.86	10.66	0.643	1.05	Member Block Shear
T9	60	Leg	A325N	1.0000	6	27.59	54.52	0.506	1.05	Bolt Tension
		Diagonal	A325N	1.0000	1	6.22	11.68	0.532	1.05	Member Block Shear
T10	40	Leg	A325N	1.0000	6	32.14	54.52	0.590	1.05	Bolt Tension
		Diagonal	A325N	1.0000	1	6.12	11.68	0.524	1.05	Member Block Shear
T11	20	Diagonal	A325N	1.2500	1	7.83	23.70	0.330	1.05	Member Block Shear

## Compression Checks

### Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	136 - 133.625	1 1/2	2.38	2.38	76.0 K=1.00	1.7672	-2.83	52.13	0.054 <sup>1</sup>
T2	133.625 - 130	1 1/2	3.63	2.63	84.0 K=1.00	1.7672	-4.40	47.47	0.093 <sup>1</sup>
T3	130 - 110	2	20.00	2.38	57.0 K=1.00	3.1416	-38.02	111.47	0.341 <sup>1</sup>
T4	110 - 94.9427	2 1/4	15.06	2.35	50.1 K=1.00	3.9761	-82.44	148.89	0.554 <sup>1</sup>
T5	94.9427 - 92.5938	2 1/4	2.35	1.18	25.2 K=1.00	3.9761	-91.06	170.80	0.533 <sup>1</sup>
T6	92.5938 - 90	2 1/4	2.59	0.58	12.4 K=1.00	3.9761	-103.76	176.91	0.587 <sup>1</sup>
T7	90 - 80	PiRod 105244 w/ (2) 1-1/4" Tie Rod	10.02	10.02	45.4 K=1.00	6.1379	-111.72	237.63	0.470 <sup>1</sup>
T8	80 - 60	PiRod 105217	20.03	10.02	37.8 K=1.00	5.3014	-154.00	214.86	0.717 <sup>1</sup>
T9	60 - 40	PiRod 105218	20.03	10.02	32.4 K=1.00	7.2158	-188.77	300.68	0.628 <sup>1</sup>
T10	40 - 20	PiRod 105218	20.03	10.02	32.4 K=1.00	7.2158	-219.27	300.68	0.729 <sup>1</sup>
T11	20 - 0	PiRod 105219	20.03	10.02	28.4 K=1.00	9.4248	-247.63	399.87	0.619 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Truss-Leg Diagonal Data

Section No.	Elevation ft	Diagonal Size	L <sub>d</sub> ft	Kl/r	φP <sub>n</sub> K	A in <sup>2</sup>	V <sub>u</sub> K	φV <sub>n</sub> K	Stress Ratio
T7	90 - 80	0.5	1.35	109.8	276.20	0.1963	0.83	3.48	0.239
T8	80 - 60	0.5	1.47	120.0	238.57	0.1963	0.39	3.34	0.117
T9	60 - 40	0.5	1.46	119.0	324.71	0.1963	0.22	3.38	0.066
T10	40 - 20	0.5	1.46	119.0	324.71	0.1963	0.41	3.38	0.123
T11	20 - 0	0.625	1.45	94.4	424.12	0.3068	1.03	6.96	0.149

### Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	136 - 133.625	3/4	3.10	3.01	134.8 K=0.70	0.4418	-1.00	5.50	0.182 <sup>1</sup>
T2	133.625 - 130	3/4	4.78	2.32	133.5 K=0.90	0.4418	-1.48	5.60	0.265 <sup>1</sup>
T3	130 - 110	7/8	5.05	2.45	120.9 K=0.90	0.6013	-4.24	9.30	0.455 <sup>1</sup>
T4	110 - 94.9427	1	5.39	2.60	112.5 K=0.90	0.7854	-5.71	14.00	0.408 <sup>1</sup>
T5	94.9427 - 92.5938	1	5.44	2.63	113.7 K=0.90	0.7854	-6.04	13.73	0.440 <sup>1</sup>
T6	92.5938 - 90	1	5.35	2.59	111.8 K=0.90	0.7854	-7.03	14.17	0.496 <sup>1</sup>
T7	90 - 80	L3x3x3/16	11.42	5.26	109.4	1.0900	-8.25	24.13	0.342 <sup>1</sup>

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T8	80 - 60	L2 1/2x2 1/2x3/16	12.50	5.63	K=1.03 136.4	0.9020	-6.50	13.87	0.468 <sup>1</sup>
T9	60 - 40	L3x3x3/16	13.80	6.33	K=1.00 127.4	1.0900	-6.16	19.22	0.320 <sup>1</sup>
T10	40 - 20	L3x3x3/16	15.24	7.08	K=1.00 142.6	1.0900	-6.34	15.35	0.413 <sup>1</sup>
T11	20 - 0	L3x3x5/16	16.80	7.84	K=1.00 159.7 K=1.00	1.7800	-8.54	19.97	0.428 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T2	133.625 - 130	3/4	4.00	3.88	173.6	0.4418	-0.21	3.31	0.063 <sup>1</sup>
T3	130 - 110	3/4	4.43	4.26	K=0.70 190.9 K=0.70	0.4418	-0.72	2.74	0.263 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Secondary Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T5	94.9427 - 92.5938	1 1/2	4.90	2.36	83.0	1.7672	-1.58	48.04	0.033 <sup>1</sup>
T6	92.5938 - 90	1 1/2	4.96	2.39	K=1.10 84.0 K=1.10	1.7672	-1.80	47.47	0.038 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	136 - 133.625	6x3/8	4.00	2.91	322.2	2.2500	-0.72	4.90	0.147 <sup>1</sup>
T3	130 - 110	KL/R > 200 (C) - 4 7/8	4.01	3.85	K=1.00 147.7	0.6013	-0.72	6.23	0.116 <sup>1</sup>
T4	110 - 94.9427	1	4.52	4.34	K=0.70 145.7 K=0.70	0.7854	-1.75	8.36	0.209 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Bottom Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T2	133.625 - 130	7/8	4.00	3.88	148.8 K=0.70	0.6013	-0.63	6.14	0.102 <sup>1</sup>
T3	130 - 110	7/8	4.49	4.32	165.9 K=0.70	0.6013	-1.76	4.93	0.357 <sup>1</sup>
T6	92.5938 - 90	1	4.99	4.80	161.2 K=0.70	0.7854	-1.80	6.83	0.263 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Tension Checks

### Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T2	133.625 - 130	1 1/2	3.63	1.00	32.0	1.7672	3.22	79.52	0.041 <sup>1</sup>
T3	130 - 110	2	20.00	0.50	12.0	2.1885	33.32	106.69	0.312 <sup>1</sup> #
T4	110 - 94.9427	2 1/4	15.06	2.35	50.1	3.9761	69.29	178.92	0.387 <sup>1</sup>
T5	94.9427 - 92.5938	2 1/4	2.35	1.17	24.9	3.9761	76.69	178.92	0.429 <sup>1</sup>
T6	92.5938 - 90	2 1/4	2.59	0.58	12.4	3.9761	88.77	178.92	0.496 <sup>1</sup>
T7	90 - 80	PiRod 105244 w/ (2) 1-1/4" Tie Rod	10.02	10.02	45.4	6.1379	96.36	276.20	0.349 <sup>1</sup>
T8	80 - 60	PiRod 105217	20.03	10.02	37.8	5.3014	134.31	238.57	0.563 <sup>1</sup>
T9	60 - 40	PiRod 105218	20.03	10.02	32.4	7.2158	165.56	324.71	0.510 <sup>1</sup>
T10	40 - 20	PiRod 105218	20.03	10.02	32.4	7.2158	192.83	324.71	0.594 <sup>1</sup>
T11	20 - 0	PiRod 105219	20.03	10.02	28.4	9.4248	216.76	424.12	0.511 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

# Based on net area of leg in section below

### Truss-Leg Diagonal Data

Section No.	Elevation ft	Diagonal Size	L <sub>d</sub> ft	Kl/r	φP <sub>n</sub> K	A in <sup>2</sup>	V <sub>u</sub> K	φV <sub>n</sub> K	Stress Ratio
T7	90 - 80	0.5	1.35	109.8	276.20	0.1963	0.83	3.48	0.239
T8	80 - 60	0.5	1.47	120.0	238.57	0.1963	0.39	3.34	0.117
T9	60 - 40	0.5	1.46	119.0	324.71	0.1963	0.22	3.38	0.066
T10	40 - 20	0.5	1.46	119.0	324.71	0.1963	0.41	3.38	0.123
T11	20 - 0	0.625	1.45	94.4	424.12	0.3068	1.03	6.96	0.149

### Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	136 - 133.625	3/4	3.10	3.01	192.5	0.4418	0.93	19.88	0.047 <sup>1</sup>
T2	133.625 - 130	3/4	4.78	2.32	148.3	0.4418	1.37	19.88	0.069 <sup>1</sup>
T3	130 - 110	7/8	5.05	2.45	134.3	0.6013	4.08	27.06	0.151 <sup>1</sup>
T4	110 - 94.9427	1	5.39	2.60	125.0	0.7854	5.68	35.34	0.161 <sup>1</sup>
T5	94.9427 - 92.5938	1	5.44	2.63	126.3	0.7854	5.94	35.34	0.168 <sup>1</sup>
T6	92.5938 - 90	1	5.35	2.59	124.2	0.7854	6.87	35.34	0.194 <sup>1</sup>
T7	90 - 80	L3x3x3/16	11.42	5.26	69.3	0.6593	7.58	28.68	0.264 <sup>1</sup>
T8	80 - 60	L2 1/2x2 1/2x3/16	11.93	5.38	86.2	0.5183	6.86	22.55	0.304 <sup>1</sup>
T9	60 - 40	L3x3x3/16	13.13	6.02	79.5	0.6593	6.22	28.68	0.217 <sup>1</sup>
T10	40 - 20	L3x3x3/16	14.50	6.73	88.6	0.6593	6.12	28.68	0.213 <sup>1</sup>
T11	20 - 0	L3x3x5/16	16.80	7.84	105.3	1.0127	7.83	44.05	0.178 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T2	133.625 - 130	3/4	4.00	3.88	248.0	0.4418	0.34	19.88	0.017 <sup>1</sup>
T3	130 - 110	3/4	4.43	4.26	272.7	0.4418	0.72	19.88	0.036 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Secondary Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T5	94.9427 - 92.5938	1 1/2	4.90	2.36	151.0	1.7672	1.58	79.52	0.020 <sup>1</sup>
T6	92.5938 - 90	1 1/2	4.96	2.39	152.7	1.7672	1.80	79.52	0.023 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	136 - 133.625	6x3/8	4.00	2.91	322.2	2.2500	0.72	72.90	0.010 <sup>1</sup>
T3	130 - 110	7/8	4.01	3.85	211.0	0.6013	0.72	27.06	0.027 <sup>1</sup>
T4	110 - 94.9427	1	4.52	4.34	208.1	0.7854	1.79	35.34	0.051 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Bottom Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T2	133.625 - 130	7/8	4.00	3.88	212.6	0.6013	0.71	27.06	0.026 <sup>1</sup>
T3	130 - 110	7/8	4.49	4.32	237.0	0.6013	1.89	27.06	0.070 <sup>1</sup>

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub> <sup>1</sup>
T6	92.5938 - 90	1	4.99	4.80	230.3	0.7854	1.80	35.34	0.051 <sup>1</sup>

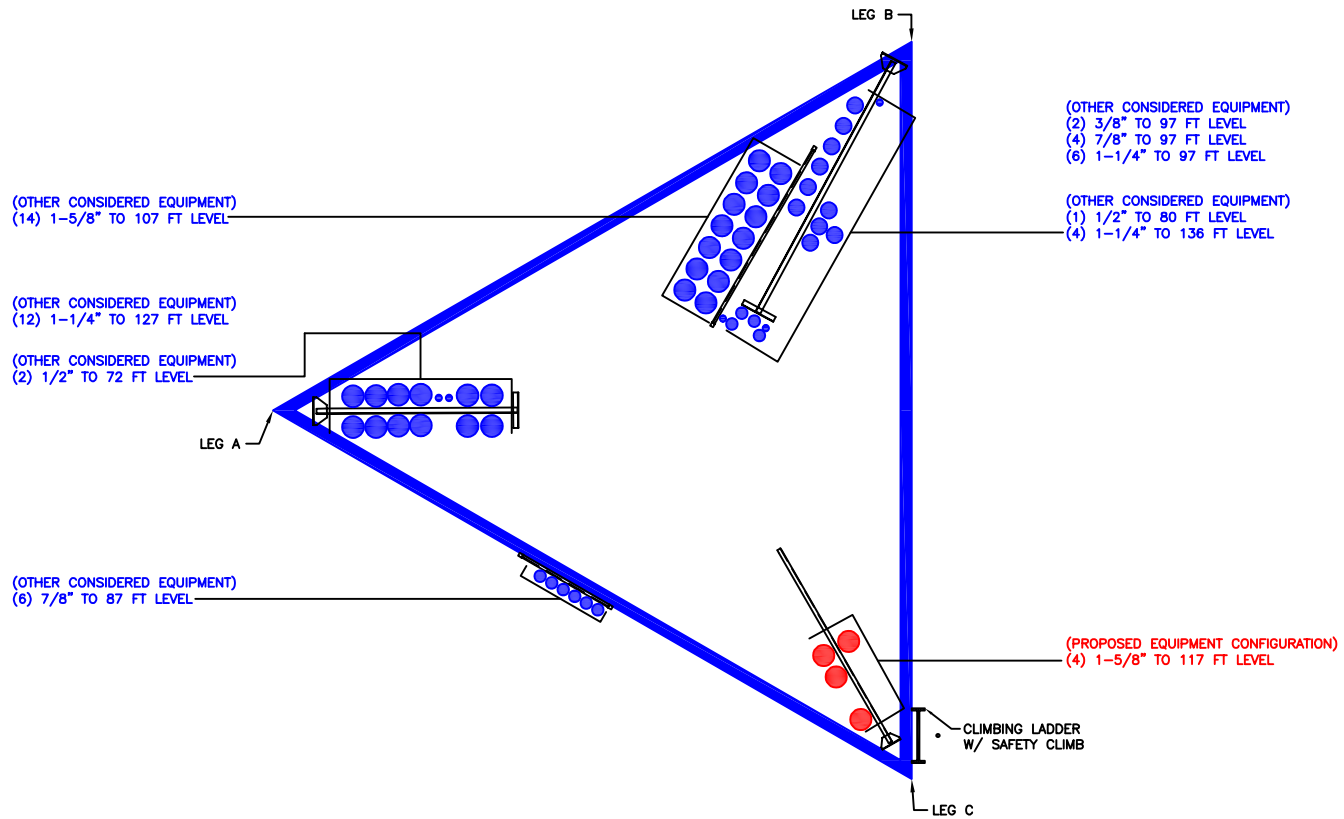
<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	φP <sub>allow</sub> K	% Capacity	Pass Fail	
T1	136 - 133.625	Leg	1 1/2	2	-2.83	54.73	5.2	Pass	
T2	133.625 - 130	Leg	1 1/2	14	-4.40	49.84	8.8	Pass	
T3	130 - 110	Leg	2	29	-38.02	117.05	32.5	Pass	
T4	110 - 94.9427	Leg	2 1/4	107	-82.44	156.33	52.7	Pass	
T5	94.9427 - 92.5938	Leg	2 1/4	149	-91.06	179.34	50.8	Pass	
T6	92.5938 - 90	Leg	2 1/4	161	-103.76	185.75	55.9	Pass	
T7	90 - 80	Leg	PiRod 105244 w/ (2) 1-1/4" Tie Rod	176	-111.72	249.51	44.8	Pass	
T8	80 - 60	Leg	PiRod 105217	185	-154.00	225.60	68.3	Pass	
T9	60 - 40	Leg	PiRod 105218	200	-188.77	315.72	59.8	Pass	
T10	40 - 20	Leg	PiRod 105218	214	-219.27	315.72	69.5	Pass	
T11	20 - 0	Leg	PiRod 105219	229	-247.63	419.86	59.0	Pass	
T1	136 - 133.625	Diagonal	3/4	7	-1.00	5.77	17.4	Pass	
T2	133.625 - 130	Diagonal	3/4	25	-1.48	5.88	25.2	Pass	
T3	130 - 110	Diagonal	7/8	40	-4.24	9.77	43.4	Pass	
T4	110 - 94.9427	Diagonal	1	115	-5.71	14.70	38.9	Pass	
T5	94.9427 - 92.5938	Diagonal	1	153	-6.04	14.41	41.9	Pass	
T6	92.5938 - 90	Diagonal	1	169	-7.03	14.87	47.3	Pass	
T7	90 - 80	Diagonal	L3x3x3/16	180	-8.25	25.34	32.6	Pass	
T8	80 - 60	Diagonal	L2 1/2x2 1/2x3/16	190	-6.50	14.57	44.6	Pass	
T9	60 - 40	Diagonal	L3x3x3/16	202	-6.16	20.18	30.5	Pass	
T10	40 - 20	Diagonal	L3x3x3/16	217	-6.34	16.11	39.4	Pass	
T11	20 - 0	Diagonal	L3x3x5/16	232	-8.54	20.97	40.7	Pass	
T2	133.625 - 130	Horizontal	3/4	16	-0.21	3.48	6.0	Pass	
T3	130 - 110	Horizontal	3/4	43	-0.72	2.88	25.0	Pass	
T5	94.9427 - 92.5938	Secondary Horizontal	1 1/2	157	-1.58	50.44	3.1	Pass	
T6	92.5938 - 90	Secondary Horizontal	1 1/2	172	-1.80	49.85	3.6	Pass	
T1	136 - 133.625	Top Girt	6x3/8	4	-0.72	5.14	14.0	Pass	
T3	130 - 110	Top Girt	7/8	31	-0.72	6.54	11.0	Pass	
T4	110 - 94.9427	Top Girt	1	111	-1.75	8.78	19.9	Pass	
T2	133.625 - 130	Bottom Girt	7/8	19	-0.63	6.44	9.7	Pass	
T3	130 - 110	Bottom Girt	7/8	36	-1.76	5.18	34.0	Pass	
T6	92.5938 - 90	Bottom Girt	1	163	-1.80	7.17	25.1	Pass	
							Summary		
							Leg (T10)	69.5	Pass
							Diagonal (T7)	74.6	Pass
							Horizontal (T3)	25.0	Pass
							Secondary Horizontal (T6)	3.6	Pass
							Top Girt (T4)	19.9	Pass
							Bottom Girt (T3)	34.0	Pass
							Bolt Checks	71.0	Pass
							<b>RATING =</b>	<b>74.6</b>	<b>Pass</b>

**APPENDIX B**  
**BASE LEVEL DRAWING**





**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

## Truss Leg Reinforcement

BU # :	876338
Site Name:	Waterford
Order:	576542 Rev. 0
Elevation:	90-80

TIA-222 Revision: H



Existing Tie Rods	
Diameter, de:	1.25 in
Unbraced Length, Le:	14.18 in
Yield Strength, Fye:	50 ksi

New Tie Rods	
Diameter, dn:	1.25 in
Unbraced Length, Ln:	14.18 in
Offset, X:	0.625 in
Yield Strength, Fyn:	50 ksi

Truss Leg	
Width, w:	12 in
Unbraced Length, Lleg:	10 ft

Reactions from tnx	
Compression, C:	111.72 kip
Tension, T:	96.36 kip

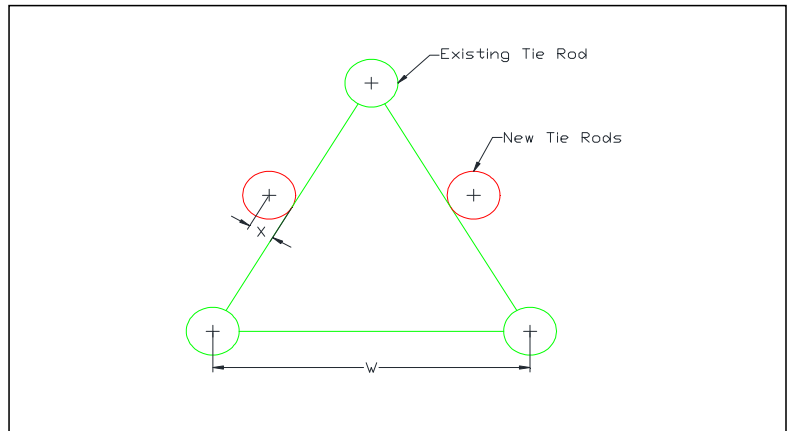
Output from tnx	
KL/r Modified Leg, KLtnx:	35.2

Length Factors	
Length Factor of Existing Tie Rods, Ke:	1
Length Factor of New Tie Rods, Kn:	1
Length Factor of the Leg, Kleg:	1

Results				
	Demand	Capacity	Rating*	Check
Compression (Existing Tie Rods), kip:	22.34	47.51	44.8%	Pass
Compression (New Tie Rods), kip:	22.34	47.51	44.8%	Pass
Compression (Modified Tie Rods), kip:	111.72	257.98	41.2%	Pass
Tension (Existing Tie Rods), kip:	19.27	55.22	33.2%	Pass
Tension (New Tie Rods), kip:	19.27	55.22	33.2%	Pass
Tension (Modified Tie Rods), kip:	96.36	276.12	33.2%	Pass

\*Section 15.5 Applied

Adjustments for tnx		
Diameter of modified truss leg, Deqv:	1.614	in
Leg K Factor Adjustment, K:	1.289	



# Self Support Anchor Rod Capacity



Site Info	
BU #	876338
Site Name	Waterford
Order #	576542 Rev. 0

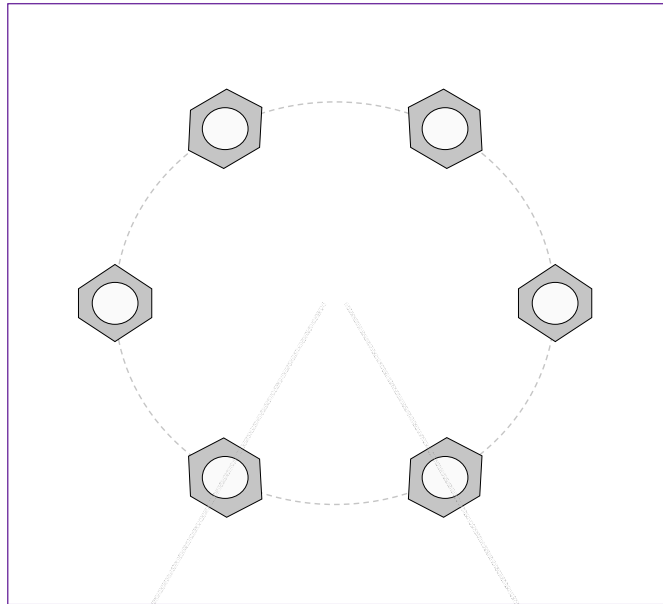
Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	Yes
$l_{ar}$ (in)	0

Applied Loads		
	Comp.	Uplift
Axial Force (kips)	256.84	224.27
Shear Force (kips)	25.25	22.28

\*TIA-222-H Section 15.5 Applied

Considered Eccentricity	
Leg Mod Eccentricity (in)	0.000
Anchor Rod N.A Shift (in)	0.000
Total Eccentricity (in)	0.000

\*Anchor Rod Eccentricity Applied



Connection Properties	Analysis Results
-----------------------	------------------

Anchor Rod Data	
(6) 1-1/4" $\phi$ bolts (A687 N; Fy=105 ksi, Fu=125 ksi)	
$l_{ar}$ (in):	0

Anchor Rod Summary		(units of kips, kip-in)
$Pu_t = 37.38$	$\phi Pn_t = 90.84$	<b>Stress Rating</b>
$Vu = 3.71$	$\phi Vn = 57.52$	<b>39.2%</b>
$Mu = n/a$	$\phi Mn = n/a$	<b>Pass</b>

# SST Unit Base Foundation



BU #: 876338  
 Site Name: Waterford  
 App. Number: 576542 Rev. 0

TIA-222 Revision: H

Top & Bot. Pad Rein. Different?:	<input type="checkbox"/>
Tower Centroid Offset?:	<input type="checkbox"/>
Block Foundation?:	<input type="checkbox"/>
Rectangular Pad?:	<input type="checkbox"/>

Superstructure Analysis Reactions		
Global Moment, <b>M</b> :	2911.24	ft-kips
Global Axial, <b>P</b> :	51.27	kips
Global Shear, <b>V</b> :	36.12	kips
Leg Compression, <b>P<sub>comp</sub></b> :	256.84	kips
Leg Comp. Shear, <b>V<sub>u,comp</sub></b> :	25.25	kips
Leg Uplift, <b>P<sub>uplift</sub></b> :	224.27	kips
Leg Uplift. Shear, <b>V<sub>u,uplift</sub></b> :	22.28	kips
Tower Height, <b>H</b> :	136	ft
Base Face Width, <b>BW</b> :	14	ft
BP Dist. Above Fdn, <b>bp<sub>dist</sub></b> :	2.5	in

Foundation Analysis Checks				
	Capacity	Demand	Rating*	Check
<i>Lateral (Sliding) (kips)</i>	243.91	36.12	14.1%	Pass
<i>Bearing Pressure (ksf)</i>	6.00	2.49	41.5%	Pass
<i>Overturning (kip*ft)</i>	4538.31	3153.55	69.5%	Pass
<i>Pier Flexure (Comp.) (kip*ft)</i>	852.07	82.06	9.2%	Pass
<i>Pier Flexure (Tension) (kip*ft)</i>	497.51	72.41	13.9%	Pass
<i>Pier Compression (kip)</i>	3374.26	260.98	7.4%	Pass
<i>Pad Flexure (kip*ft)</i>	6695.87	862.93	12.3%	Pass
<i>Pad Shear - 1-way (kips)</i>	777.96	134.87	16.5%	Pass
<i>Pad Shear - Comp 2-way (ksi)</i>	0.164	0.035	20.4%	Pass
<i>Flexural 2-way (Comp) (kip*ft)</i>	5260.90	49.24	0.9%	Pass
<i>Pad Shear - Tension 2-way (ksi)</i>	0.164	0.034	20.0%	Pass
<i>Flexural 2-way (Tension) (kip*ft)</i>	5260.90	43.45	0.8%	Pass

\*Rating per TIA-222-H Section 15.5

Structural Rating*:	20.4%
Soil Rating*:	69.5%

Pier Properties		
Pier Shape:	Circular	
Pier Diameter, <b>dpier</b> :	3.0	ft
Ext. Above Grade, <b>E</b> :	0.50	ft
Pier Rebar Size, <b>Sc</b> :	8	
Pier Rebar Quantity, <b>mc</b> :	15	
Pier Tie/Spiral Size, <b>St</b> :	4	
Pier Tie/Spiral Quantity, <b>mt</b> :	7	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, <b>cc<sub>pier</sub></b> :	3	in

Pad Properties		
Depth, <b>D</b> :	6.00	ft
Pad Width, <b>W<sub>1</sub></b> :	23.00	ft
Pad Thickness, <b>T</b> :	3.25	ft
Pad Rebar Size (Bottom dir. 2), <b>Sp<sub>2</sub></b> :	9	
Pad Rebar Quantity (Bottom dir. 2), <b>mp<sub>2</sub></b> :	46	
Pad Clear Cover, <b>cc<sub>pad</sub></b> :	3	in

Material Properties		
Rebar Grade, <b>Fy</b> :	60	ksi
Concrete Compressive Strength, <b>F'c</b> :	3	ksi
Dry Concrete Density, <b>δc</b> :	150	pcf

Soil Properties		
Total Soil Unit Weight, <b>γ</b> :	120	pcf
Ultimate Gross Bearing, <b>Qult</b> :	8.000	ksf
Cohesion, <b>Cu</b> :	0.000	ksf
Friction Angle, <b>φ</b> :	36	degrees
SPT Blow Count, <b>N<sub>blows</sub></b> :	25	
Base Friction, <b>μ</b> :		
Neglected Depth, <b>N</b> :	3.3	ft
Foundation Bearing on Rock?	No	
Groundwater Depth, <b>gw</b> :	N/A	ft

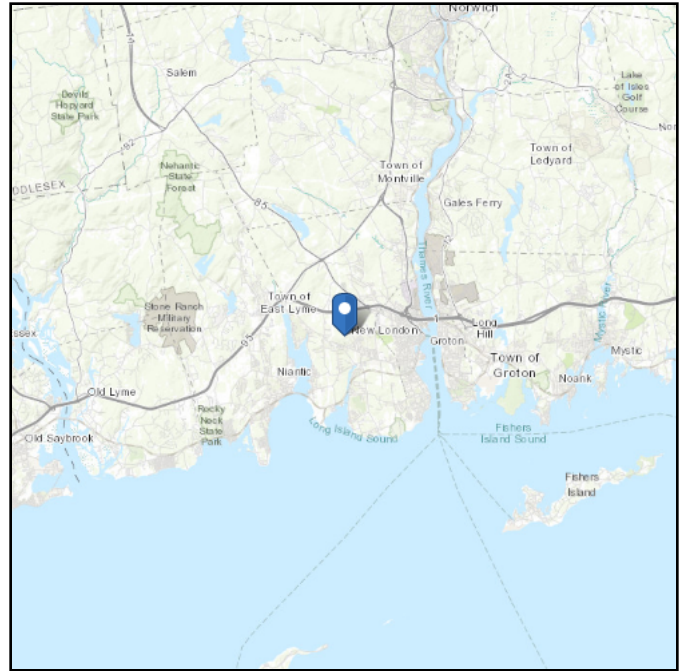
<-- Toggle between Gross and Net

# ASCE 7 Hazards Report

**Address:**  
No Address at This Location

**Standard:** ASCE/SEI 7-10  
**Risk Category:** II  
**Soil Class:** D - Stiff Soil

**Elevation:** 242 ft (NAVD 88)  
**Latitude:** 41.354639  
**Longitude:** -72.150444



## Wind

### Results:

Wind Speed:	<b>134 Vmph</b>
10-year MRI	79 Vmph
25-year MRI	89 Vmph
50-year MRI	99 Vmph
100-year MRI	109 Vmph

\*135 Vmph as required CT Design Criteria for New London County

**Data Source:** ASCE/SEI 7-10, Fig. 26.5-1A and Figs. CC-1–CC-4, and Section 26.5.2, incorporating errata of March 12, 2014

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

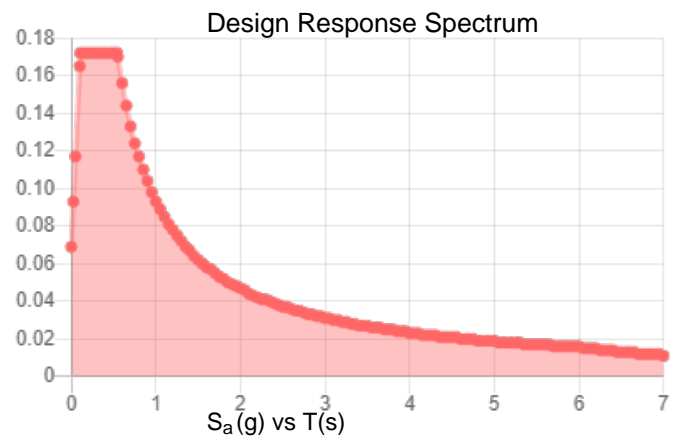
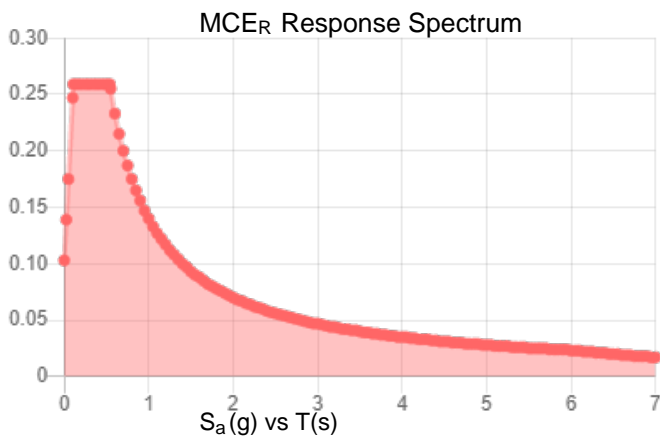
Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.

**Site Soil Class:** D - Stiff Soil

**Results:**

$S_s$ :	0.162	$S_{DS}$ :	0.172
$S_1$ :	0.058	$S_{D1}$ :	0.093
$F_a$ :	1.6	$T_L$ :	6
$F_v$ :	2.4	PGA :	0.081
$S_{MS}$ :	0.259	PGA <sub>M</sub> :	0.129
$S_{M1}$ :	0.14	$F_{PGA}$ :	1.6
		$I_e$ :	1

**Seismic Design Category** B



**Data Accessed:**

Tue Aug 31 2021

**Date Source:**

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

## Ice

---

### Results:

Ice Thickness: 0.75 in. Design Ice: 2\*0.75 in. = 1.50 in.  
Concurrent Temperature: 15 F  
Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

**Date Accessed:** Tue Aug 31 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

---

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided “as is” and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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Date: **August 24, 2021**

Darcy Tarr  
Crown Castle  
3530 Toringdon Way, Suite 300  
Charlotte, NC 28277  
(704) 405-6589



Trylon  
1825 W. Walnut Hill Lane,  
Suite 302  
Irving, TX 75038  
214-930-1730

**Subject:** **Mount Replacement Analysis Report**

**Carrier Designation:** **T-Mobile Anchor**  
**Carrier Site Number:** CT11381C  
**Carrier Site Name:** WATERFORD SOUTH/RT 1

**Crown Castle Designation:** **Crown Castle BU Number:** 876338  
**Crown Castle Site Name:** WATERFORD  
**Crown Castle JDE Job Number:** 675143  
**Crown Castle Order Number:** 576542 Rev. 0

**Engineering Firm Designation:** **Trylon Report Designation:** 190496

**Site Data:** **41 Manitock Hill Road, Waterford, New London County, CT, 06385-2000**  
**Latitude 41°21'16.70" Longitude -72°9'1.60"**

**Structure Information:** **Tower Height & Type:** **136.0 ft Self Support**  
**Mount Elevation:** **117.0 ft**  
**Mount Type:** **12.5 ft Sector Frame**

Dear Darcy Tarr,

Trylon is pleased to submit this “**Mount Replacement Analysis Report**” to determine the structural integrity of T-Mobile’s antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

**Sector Frame** **Sufficient\***  
**\*Sufficient upon completion of the changes listed in the ‘Recommendations’ section of this report.**

This analysis utilizes an ultimate 3-second gust wind speed of 135 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Andrei Florea

Respectfully Submitted by:  
Jinshan Wang, P.E



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### 9) APPENDIX E

Supplemental Drawings

## 1) INTRODUCTION

This is a proposed 3 sector 12.5 ft Sector Frame, designed by Site Pro 1.

## 2) ANALYSIS CRITERIA

<b>Building Code:</b>	2015 IBC
<b>TIA-222 Revision:</b>	TIA-222-H
<b>Risk Category:</b>	II
<b>Ultimate Wind Speed:</b>	135 mph
<b>Exposure Category:</b>	B
<b>Topographic Factor at Base:</b>	1.000
<b>Topographic Factor at Mount:</b>	1.000
<b>Ice Thickness:</b>	1.50 in
<b>Wind Speed with Ice:</b>	50 mph
<b>Seismic S<sub>s</sub>:</b>	0.162
<b>Seismic S<sub>1</sub>:</b>	0.058
<b>Live Loading Wind Speed:</b>	30 mph
<b>Man Live Load at Mid/End-Points:</b>	250 lb
<b>Man Live Load at Mount Pipes:</b>	500 lb

**Table 1 - Proposed Equipment Configuration**

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details
117.0	119.0	3	ERICSSON	AIR6449 B41_T-MOBILE	12.5 ft Sector Frame [ Site Pro 1, VFA12-HD ]
		3	RFS/CELWAVE	APXVAALL24_43-U-NA20_TMO	
		3	ERICSSON	RADIO 4449 B71 B85A_T-MOBILE	
		3	ERICSSON	RADIO 4460 B2/B25 B66_TMO	

## 3) ANALYSIS PROCEDURE

**Table 2 - Documents Provided**

Document	Remarks	Reference	Source
Crown Application	T-Mobile Application	576542 Rev.0	CCI Sites
Structural Analysis Report	Morrison Hershfield	9759292	CCI Sites
Mount Manufacturer Drawings	Site Pro 1	VFA12-HD	Trylon

### 3.1) Analysis Method

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed, using Microsoft Excel, by Trylon was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 Tower Mount Analysis (Revision B).

**3.2) Assumptions**

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 5) Prior structural modifications to the tower mounting system are assumed to be installed as shown per available data.
- 6) Steel grades have been assumed as follows, unless noted otherwise:
 

Channel, Solid Round, Angle, Plate	ASTM A36 (GR 36)
HSS (Rectangular)	ASTM A500 (GR B-46)
Pipe	ASTM A53 (GR 35)
Connection Bolts	ASTM A325

This analysis may be affected if any assumptions are not valid or have been made in error. Tylon should be notified to determine the effect on the structural integrity of the antenna mounting system.

**4) ANALYSIS RESULTS**

**Table 3 - Mount Component Stresses vs. Capacity (Sector Frame, Worst Case Sector)**

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
1,2,3	Mount Pipe(s)	MP3	117.0	69.2	Pass
	Horizontal(s)	M19		50.4	Pass
	Standoff(s)	M24		20.2	Pass
	Bracing(s)	M45		32.0	Pass
	Plate(s)	M13		57.5	Pass
	Tieback(s)	M56		2.4	Pass
	Mount Connection(s)	-		28.7	Pass

<b>Structure Rating (max from all components) =</b>	<b>69.2%</b>
-----------------------------------------------------	--------------

Notes:

- 1) See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed.
- 2) All sectors are typical
- 3) Rating per TIA-222-H, Section 15.5

**Table 4 - Tieback Connection Data Table**

Tower Connection Node No.	Existing / Proposed	Resultant End Reaction (lb)	Connected Member Type	Connected Member Size	Member Compressive Capacity (lb) <sup>3</sup>	Notes
N79A	Proposed	972.4	Leg	SR 2"	5,573.6	1
N80	Proposed	496.8	Leg	SR 2"	5,573.6	1

Notes:

- 1) Tieback connection point is within 25% of either end of the connected tower member
- 2) Tieback connection point is NOT within 25% of either end of the connected tower member
- 3) Reduced member compressive capacity according to CED-STD-10294 *Standard for Installation of Mounts and Appurtenances*

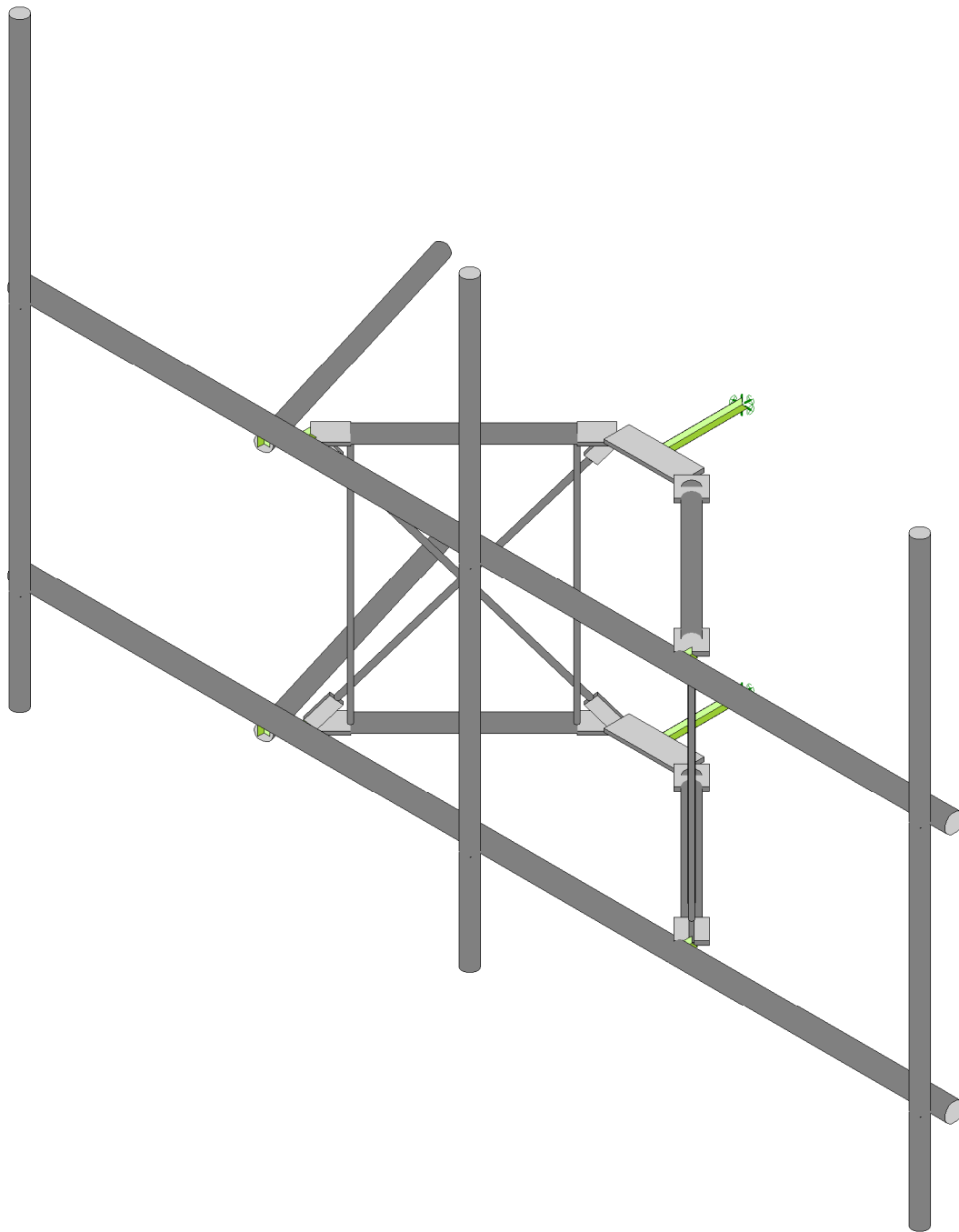
**4.1) Recommendations**

The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the proposed mount listed below must be installed.

1. Site Pro 1, VFA12-HD.
2. Install 2.375" O.D. Schd.40, 8-ft. long antenna pipes.
3. Install the tiebacks as recommended in manufacturer's drawings (Tie-Back Position 3). Tieback connection point needs to be within 25% ends of the tower leg.

No structural modifications are required at this time, provided that the above-listed changes are implemented.

**APPENDIX A**  
**WIRE FRAME AND RENDERED MODELS**

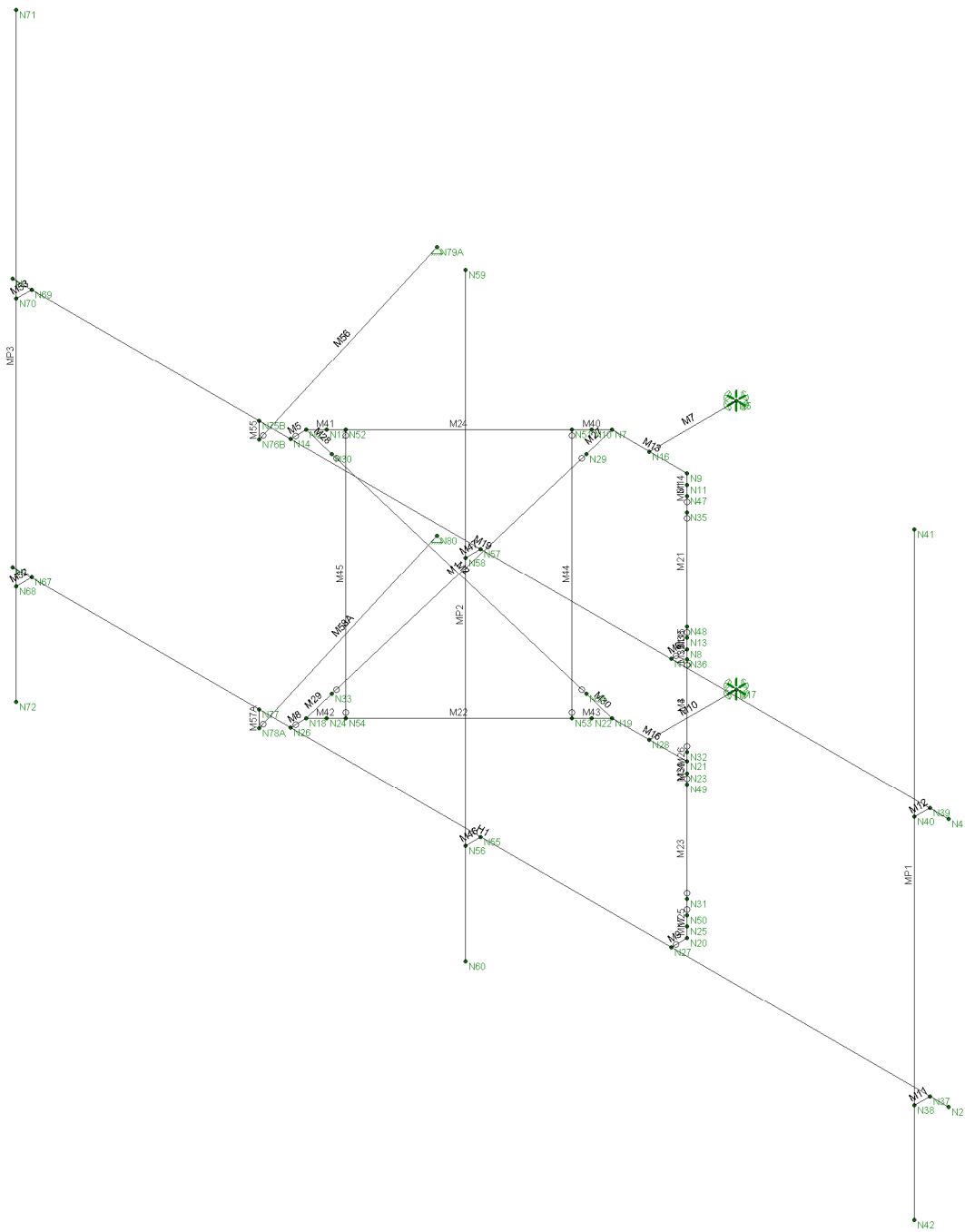


Envelope Only Solution

Trylon
AF
190496

876338
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SK - 1
Aug 24, 2021 at 12:09 PM
876338.r3d



Envelope Only Solution

Trylon
AF
190496

876338

SK - 2
Aug 24, 2021 at 12:10 PM
876338.r3d



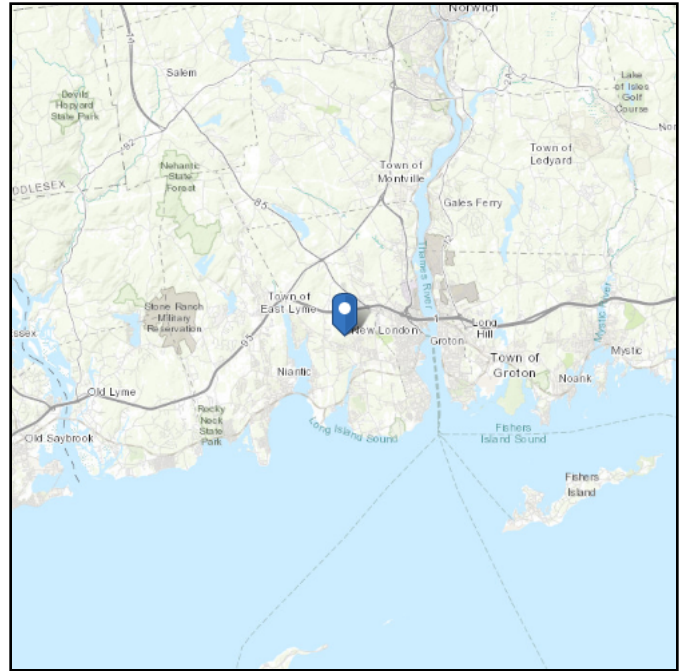
**APPENDIX B**  
**SOFTWARE INPUT CALCULATIONS**

# ASCE 7 Hazards Report

**Address:**  
No Address at This  
Location

**Standard:** ASCE/SEI 7-10  
**Risk Category:** II  
**Soil Class:** D - Stiff Soil

**Elevation:** 242 ft (NAVD 88)  
**Latitude:** 41.354639  
**Longitude:** -72.150444

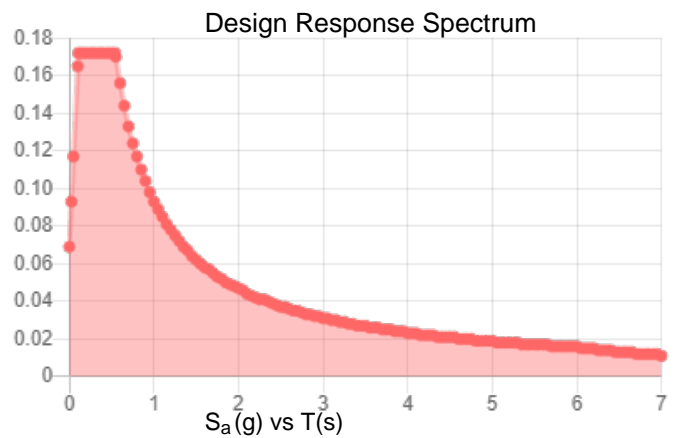
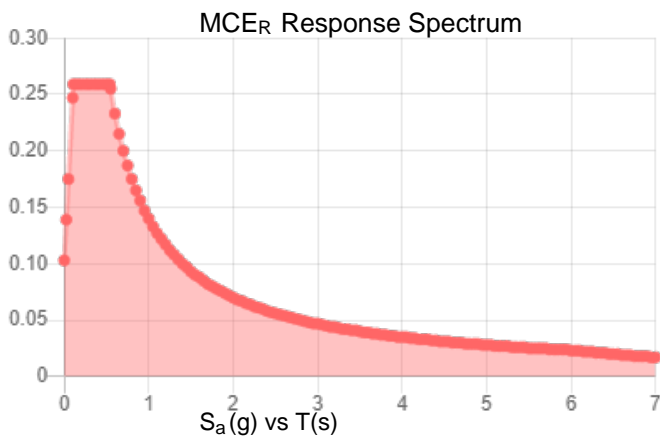


**Site Soil Class:** D - Stiff Soil

**Results:**

$S_S$ :	0.162	$S_{DS}$ :	0.172
$S_1$ :	0.058	$S_{D1}$ :	0.093
$F_a$ :	1.6	$T_L$ :	6
$F_v$ :	2.4	PGA :	0.081
$S_{MS}$ :	0.259	PGA <sub>M</sub> :	0.129
$S_{M1}$ :	0.14	F <sub>PGA</sub> :	1.6
		$I_e$ :	1

**Seismic Design Category** B



**Data Accessed:**

Tue Aug 24 2021

**Date Source:**

USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-10 Ch. 21 are available from USGS.

## Ice

---

### Results:

Ice Thickness: 0.75 in.  
Concurrent Temperature: 15 F  
Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

**Date Accessed:** Tue Aug 24 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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# Trylon

1825 W. Walnut Hill Lane Suite 120  
Irving, TX 75038

## TIA LOAD CALCULATOR 2.1

PROJECT DATA	
Job Code:	190496
Carrier Site ID:	CT11381C
Carrier Site Name:	WATERFORD SOUTH/RT 1

CODES AND STANDARDS	
Building Code:	2015 IBC
Local Building Code:	2018 CSBC
Design Standard:	TIA-222-H

STRUCTURE DETAILS		
Mount Type:	Sector Frame	--
Mount Elevation:	117.0	ft.
Number of Sectors:	3	--
Structure Type:	Self Support Tower	--
Structure Height:	136.0	ft.

ANALYSIS CRITERIA		
Structure Risk Category:	II	--
Exposure Category:	B	--
Site Class:	D - Default	--
Ground Elevation:	242	ft.

TOPOGRAPHIC DATA		
Topographic Category:	1.00	--
Topographic Feature:	N/A	--
Crest Point Elevation:	0.00	ft.
Base Point Elevation:	0.00	ft.
Crest to Mid-Height (L/2):	0.00	ft.
Distance from Crest (x):	0.00	ft.
Base Topo Factor ( $K_{zt}$ ):	1.00	--
Mount Topo Factor ( $K_{zt}$ ):	1.00	--

WIND PARAMETERS		
Design Wind Speed:	135	mph
Wind Escalation Factor ( $K_s$ ):	1.00	--
Velocity Coefficient ( $K_z$ ):	1.03	--
Directionality Factor ( $K_d$ ):	0.95	--
Gust Effect Factor ( $G_h$ ):	1.00	--
Shielding Factor ( $K_a$ ):	0.90	--
Velocity Pressure ( $q_z$ ):	45.41	psf
Ground Elevation Factor ( $K_e$ ):	0.99	--

ICE PARAMETERS		
Design Ice Wind Speed:	50	mph
Design Ice Thickness ( $t_i$ ):	1.50	in
Importance Factor ( $I_i$ ):	1.00	--
Ice Velocity Pressure ( $q_{zi}$ ):	45.41	psf
Mount Ice Thickness ( $t_{iz}$ ):	1.70	in

WIND STRUCTURE CALCULATIONS		
Flat Member Pressure:	81.74	psf
Round Member Pressure:	49.04	psf
Ice Wind Pressure:	7.39	psf

SEISMIC PARAMETERS		
Importance Factor ( $I_e$ ):	1.00	--
Short Period Accel. ( $S_s$ ):	0.16	g
1 Second Accel. ( $S_1$ ):	0.06	g
Short Period Des. ( $S_{DS}$ ):	0.17	g
1 Second Des. ( $S_{D1}$ ):	0.09	g
Short Period Coeff. ( $F_a$ ):	1.60	--
1 Second Coeff. ( $F_v$ ):	2.40	--
Response Coefficient ( $C_s$ ):	0.09	--
Amplification Factor ( $A_S$ ):	1.20	--

## LOAD COMBINATIONS [LRFD]

#	Description
1	1.4DL
2	1.2DL + 1WL 0 AZI
3	1.2DL + 1WL 30 AZI
4	1.2DL + 1WL 45 AZI
5	1.2DL + 1WL 60 AZI
6	1.2DL + 1WL 90 AZI
7	1.2DL + 1WL 120 AZI
8	1.2DL + 1WL 135 AZI
9	1.2DL + 1WL 150 AZI
10	1.2DL + 1WL 180 AZI
11	1.2DL + 1WL 210 AZI
12	1.2DL + 1WL 225 AZI
13	1.2DL + 1WL 240 AZI
14	1.2DL + 1WL 270 AZI
15	1.2DL + 1WL 300 AZI
16	1.2DL + 1WL 315 AZI
17	1.2DL + 1WL 330 AZI
18	0.9DL + 1WL 0 AZI
19	0.9DL + 1WL 30 AZI
20	0.9DL + 1WL 45 AZI
21	0.9DL + 1WL 60 AZI
22	0.9DL + 1WL 90 AZI
23	0.9DL + 1WL 120 AZI
24	0.9DL + 1WL 135 AZI
25	0.9DL + 1WL 150 AZI
26	0.9DL + 1WL 180 AZI
27	0.9DL + 1WL 210 AZI
28	0.9DL + 1WL 225 AZI
29	0.9DL + 1WL 240 AZI
30	0.9DL + 1WL 270 AZI
31	0.9DL + 1WL 300 AZI
32	0.9DL + 1WL 315 AZI
33	0.9DL + 1WL 330 AZI
34	1.2DL + 1DLi + 1WLi 0 AZI
35	1.2DL + 1DLi + 1WLi 30 AZI
36	1.2DL + 1DLi + 1WLi 45 AZI
37	1.2DL + 1DLi + 1WLi 60 AZI
38	1.2DL + 1DLi + 1WLi 90 AZI
39	1.2DL + 1DLi + 1WLi 120 AZI
40	1.2DL + 1DLi + 1WLi 135 AZI
41	1.2DL + 1DLi + 1WLi 150 AZI

#	Description
42	1.2DL + 1DLi + 1WLi 180 AZI
43	1.2DL + 1DLi + 1WLi 210 AZI
44	1.2DL + 1DLi + 1WLi 225 AZI
45	1.2DL + 1DLi + 1WLi 240 AZI
46	1.2DL + 1DLi + 1WLi 270 AZI
47	1.2DL + 1DLi + 1WLi 300 AZI
48	1.2DL + 1DLi + 1WLi 315 AZI
49	1.2DL + 1DLi + 1WLi 330 AZI
50	(1.2+0.2Sds) + 1.0E 0 AZI
51	(1.2+0.2Sds) + 1.0E 30 AZI
52	(1.2+0.2Sds) + 1.0E 45 AZI
53	(1.2+0.2Sds) + 1.0E 60 AZI
54	(1.2+0.2Sds) + 1.0E 90 AZI
55	(1.2+0.2Sds) + 1.0E 120 AZI
56	(1.2+0.2Sds) + 1.0E 135 AZI
57	(1.2+0.2Sds) + 1.0E 150 AZI
58	(1.2+0.2Sds) + 1.0E 180 AZI
59	(1.2+0.2Sds) + 1.0E 210 AZI
60	(1.2+0.2Sds) + 1.0E 225 AZI
61	(1.2+0.2Sds) + 1.0E 240 AZI
62	(1.2+0.2Sds) + 1.0E 270 AZI
63	(1.2+0.2Sds) + 1.0E 300 AZI
64	(1.2+0.2Sds) + 1.0E 315 AZI
65	(1.2+0.2Sds) + 1.0E 330 AZI
66	(0.9-0.2Sds) + 1.0E 0 AZI
67	(0.9-0.2Sds) + 1.0E 30 AZI
68	(0.9-0.2Sds) + 1.0E 45 AZI
69	(0.9-0.2Sds) + 1.0E 60 AZI
70	(0.9-0.2Sds) + 1.0E 90 AZI
71	(0.9-0.2Sds) + 1.0E 120 AZI
72	(0.9-0.2Sds) + 1.0E 135 AZI
73	(0.9-0.2Sds) + 1.0E 150 AZI
74	(0.9-0.2Sds) + 1.0E 180 AZI
75	(0.9-0.2Sds) + 1.0E 210 AZI
76	(0.9-0.2Sds) + 1.0E 225 AZI
77	(0.9-0.2Sds) + 1.0E 240 AZI
78	(0.9-0.2Sds) + 1.0E 270 AZI
79	(0.9-0.2Sds) + 1.0E 300 AZI
80	(0.9-0.2Sds) + 1.0E 315 AZI
81	(0.9-0.2Sds) + 1.0E 330 AZI
82-88	1.2D + 1.5 Lv1

#	Description
89	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP1
90	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP1
91	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP1
92	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP1
93	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP1
94	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP1
95	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP1
96	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP1
97	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP1
98	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP1
99	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP1
100	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP1
101	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP1
102	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP1
103	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP1
104	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP1
105	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP2
106	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP2
107	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP2
108	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP2
109	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP2
110	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP2
111	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP2
112	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP2
113	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP2
114	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP2
115	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP2
116	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP2
117	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP2
118	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP2
119	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP2
120	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP2

#	Description
121	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP3
122	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP3
123	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP3
124	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP3
125	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP3
126	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP3
127	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP3
128	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP3
129	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP3
130	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP3
131	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP3
132	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP3
133	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP3
134	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP3
135	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP3
136	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP3
137	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP4
138	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP4
139	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP4
140	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP4
141	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP4
142	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP4
143	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP4
144	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP4
145	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP4
146	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP4
147	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP4
148	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP4
149	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP4
150	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP4
151	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP4
152	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP4

\*This page shows an example of maintenance loads for (4) pipes, the number of mount pipe LCs may vary per site





## EQUIPMENT WIND CALCULATIONS

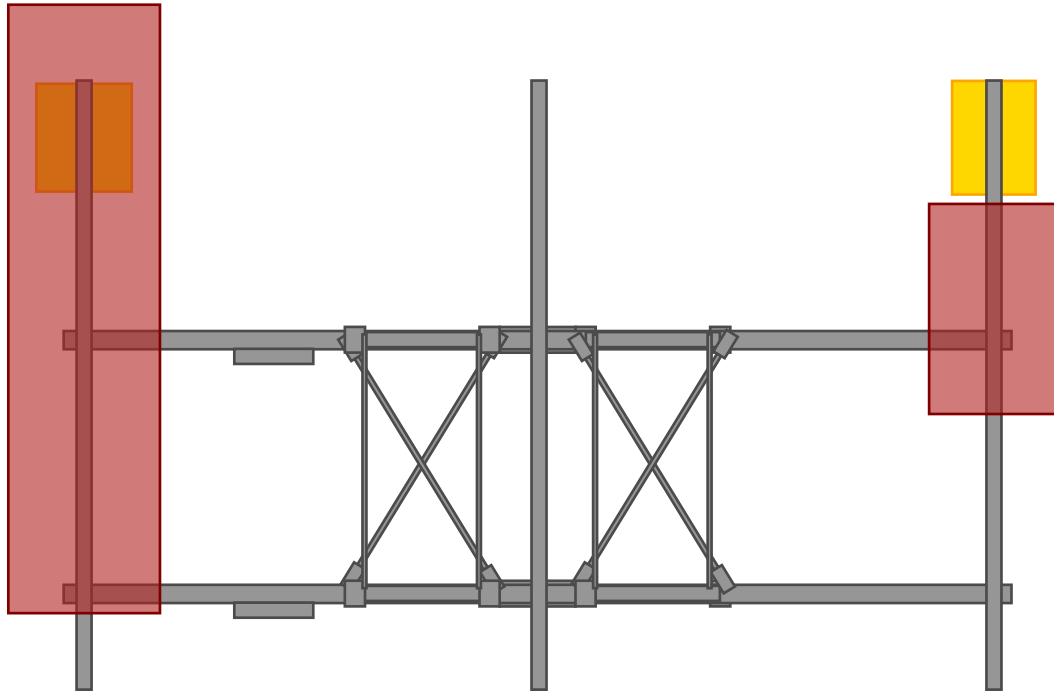
<i>Appurtenance Name</i>	<i>Qty.</i>	<i>Elevation [ft]</i>	<i>K<sub>zt</sub></i>	<i>K<sub>z</sub></i>	<i>K<sub>d</sub></i>	<i>t<sub>d</sub></i>	<i>q<sub>z</sub> [psf]</i>	<i>q<sub>zi</sub> [psf]</i>
AIR6449 B41_T-MOBILE	1	119	1.00	1.04	0.95	1.71	45.63	6.26
XVAALL24_43-U-NA20_T	1	119	1.00	1.04	0.95	1.71	45.63	6.26
OIO 4449 B71 B85A_T-MO	1	119	1.00	1.04	0.95	1.71	45.63	6.26
ADIO 4460 B2/B25 B66_TN	1	119	1.00	1.04	0.95	1.71	45.63	6.26



## **EQUIPMENT SEISMIC FORCE CALCULATIONS**

<i>Appurtenance Name</i>	<i>Qty.</i>	<i>Elevation [ft]</i>	<i>Weight [lbs]</i>	<i>F<sub>p</sub> [lbs]</i>
AIR6449 B41_T-MOBILE	1	119	114.63	11.88
APXVAALL24_43-U-NA20_TMO	1	119	149.9	15.54
RADIO 4449 B71 B85A_T-MOBILE	1	119	73.21	7.59
RADIO 4460 B2/B25 B66_TMO	1	119	109	11.30

ELEVATION VIEW

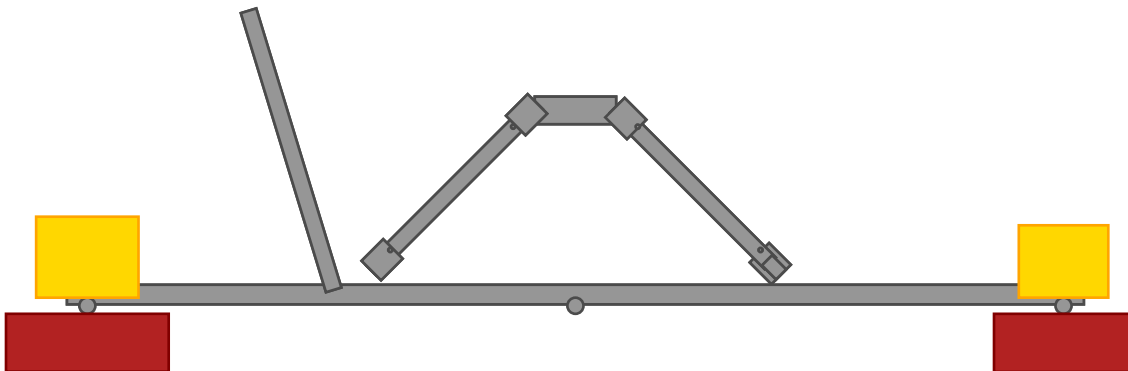


MP3

MP2

MP1

PLAN VIEW





1825 W. Walnut Hill Lane, Suite 120  
Irving, Texas 75038

Equipment Name	Total Quantity	Antenna Centerline	Mount Pipe Positions	Equipment Azimuths
AIR6449 B41_T-MOBILE	1	119	MP1	0
APXVAALL24_43-U-NA20_TMO	1	119	MP3	0
RADIO 4449 B71 B85A_T-MOBILE	1	119	MP1	0
RADIO 4460 B2/B25 B66_TMO	1	119	MP3	0

**APPENDIX C**  
**SOFTWARE ANALYSIS OUTPUT**



Company : Trylon  
 Designer : AF  
 Job Number : 190496  
 Model Name : 876338

Aug 24, 2021  
 12:10 PM  
 Checked By: JW

**(Global) Model Settings**

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	24
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Z
Global Member Orientation Plane	XY
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-16: LRFD
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	None

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR SET ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8





**(Global) Model Settings, Continued**

Seismic Code	ASCE 7-10
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

**Hot Rolled Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E...Density[k/ft...	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A992	29000	11154	.3	.65 .49	50	1.1	65	1.1
2	A36 Gr.36	29000	11154	.3	.65 .49	36	1.5	58	1.2
3	A572 Gr.50	29000	11154	.3	.65 .49	50	1.1	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65 .527	42	1.4	58	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65 .527	46	1.4	58	1.3
6	A53 Gr.B	29000	11154	.3	.65 .49	35	1.6	60	1.2
7	A1085	29000	11154	.3	.65 .49	50	1.4	65	1.3

**Cold Formed Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E5 F)	Density[k/ft^3]	Yield[ksi]	Fu[ksi]
1	A653 SS Gr33	29500	11346	.3	.65	.49	33	45
2	A653 SS Gr50/1	29500	11346	.3	.65	.49	50	65

**Hot Rolled Steel Section Sets**

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	Pipe 2.375" O.D S...	PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical	1.02	.627	.627	1.25
2	Pipe 2.875" O.D Sc...	PIPE 2.5	Beam	Pipe	A53 Gr.B	Typical	1.61	1.45	1.45	2.89
3	SR 3/4"	SR 3/4"	Beam	BAR	A36 Gr.36	Typical	.442	.016	.016	.031
4	Plate	PL 4"x0.625"	Beam	RECT	A36 Gr.36	Typical	2.5	.081	3.333	.293
5	SR 5/8"	SR 5/8 HRB	Beam	BAR	A36 Gr.36	Typical	.307	.007	.007	.015
6	D Plate	PL 2"x0.625"	Beam	RECT	A36 Gr.36	Typical	1.25	.041	.417	.131



Company : Trylon  
 Designer : AF  
 Job Number : 190496  
 Model Name : 876338

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### Cold Formed Steel Section Sets

	Label	Shape	Type	Design List	Material	Design Rules	A [in <sup>2</sup> ]	I <sub>yy</sub> [in <sup>4</sup> ]	I <sub>zz</sub> [in <sup>4</sup> ]	J [in <sup>4</sup> ]
1	CF1	162T125-18	Beam	None	A653 SS Gr33	Typical	.078	.013	.042	9e-6

### Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	N5	Reaction	Reaction	Reaction	Reaction	Reaction	
2	N17	Reaction	Reaction	Reaction	Reaction	Reaction	
3	N79A	Reaction	Reaction	Reaction			
4	N80	Reaction	Reaction	Reaction			

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area(Me...)	Surface(P...
1	Self Weight	DL			-1		6		
2	Structure Wind X	WLX						51	
3	Structure Wind Y	WLY						51	
4	Wind Load 0 AZI	WLX					12		
5	Wind Load 30 AZI	None					12		
6	Wind Load 45 AZI	None					12		
7	Wind Load 60 AZI	None					12		
8	Wind Load 90 AZI	WLY					12		
9	Wind Load 120 AZI	None					12		
10	Wind Load 135 AZI	None					12		
11	Wind Load 150 AZI	None					12		
12	Ice Weight	OL1					6	51	
13	Ice Structure Wind X	OL2						51	
14	Ice Structure Wind Y	OL3						51	
15	Ice Wind Load 0 AZI	OL2					12		
16	Ice Wind Load 30 AZI	None					12		
17	Ice Wind Load 45 AZI	None					12		
18	Ice Wind Load 60 AZI	None					12		
19	Ice Wind Load 90 AZI	OL3					12		
20	Ice Wind Load 120 AZI	None					12		
21	Ice Wind Load 135 AZI	None					12		
22	Ice Wind Load 150 AZI	None					12		
23	Seismic Load X	ELX	-.104				6		
24	Seismic Load Y	ELY		-.104			6		
25	Live Load 1 (Lv)	None					1		
26	Live Load 2 (Lv)	None					1		
27	Live Load 3 (Lv)	None					1		
28	Maintenance Load 1 (...)	None					1		
29	Maintenance Load 2 (...)	None					1		
30	Maintenance Load 3 (...)	None					1		

### Load Combinations

	Description	Sol...	PD...	SR...	BLC Fact...	BLC Fact...	BLC Fact...	BLC Fact...	BLC Fact...	BLC Fact...	BLC Fact...	BLC Fact...	BLC Fact...	BLC Fact...
1	1.4DL	Yes	Y		DL	1.4								
2	1.2DL + 1...	Yes	Y		DL	1.2	2	1	3	4	1			
3	1.2DL + 1...	Yes	Y		DL	1.2	2	.866	3	.5	5	1		
4	1.2DL + 1...	Yes	Y		DL	1.2	2	.707	3	.707	6	1		
5	1.2DL + 1...	Yes	Y		DL	1.2	2	.5	3	.866	7	1		
6	1.2DL + 1...	Yes	Y		DL	1.2	2		3	1	8	1		
7	1.2DL + 1...	Yes	Y		DL	1.2	2	-.5	3	.866	9	1		
8	1.2DL + 1...	Yes	Y		DL	1.2	2	-.707	3	.707	10	1		



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**Load Combinations (Continued)**

	Description	Sol.	PD	SR	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact
9	1.2DL + 1...	Yes	Y		DL 1.2	2	-.866	3	.5	11	1			
10	1.2DL + 1...	Yes	Y		DL 1.2	2	-1	3		4	-1			
11	1.2DL + 1...	Yes	Y		DL 1.2	2	-.866	3	-.5	5	-1			
12	1.2DL + 1...	Yes	Y		DL 1.2	2	-.707	3	-.707	6	-1			
13	1.2DL + 1...	Yes	Y		DL 1.2	2	-.5	3	-.866	7	-1			
14	1.2DL + 1...	Yes	Y		DL 1.2	2		3	-1	8	-1			
15	1.2DL + 1...	Yes	Y		DL 1.2	2	.5	3	-.866	9	-1			
16	1.2DL + 1...	Yes	Y		DL 1.2	2	.707	3	-.707	10	-1			
17	1.2DL + 1...	Yes	Y		DL 1.2	2	.866	3	-.5	11	-1			
18	0.9DL + 1...	Yes	Y		DL .9	2	1	3		4	1			
19	0.9DL + 1...	Yes	Y		DL .9	2	.866	3	.5	5	1			
20	0.9DL + 1...	Yes	Y		DL .9	2	.707	3	.707	6	1			
21	0.9DL + 1...	Yes	Y		DL .9	2	.5	3	.866	7	1			
22	0.9DL + 1...	Yes	Y		DL .9	2		3	1	8	1			
23	0.9DL + 1...	Yes	Y		DL .9	2	-.5	3	.866	9	1			
24	0.9DL + 1...	Yes	Y		DL .9	2	-.707	3	.707	10	1			
25	0.9DL + 1...	Yes	Y		DL .9	2	-.866	3	.5	11	1			
26	0.9DL + 1...	Yes	Y		DL .9	2	-1	3		4	-1			
27	0.9DL + 1...	Yes	Y		DL .9	2	-.866	3	-.5	5	-1			
28	0.9DL + 1...	Yes	Y		DL .9	2	-.707	3	-.707	6	-1			
29	0.9DL + 1...	Yes	Y		DL .9	2	-.5	3	-.866	7	-1			
30	0.9DL + 1...	Yes	Y		DL .9	2		3	-1	8	-1			
31	0.9DL + 1...	Yes	Y		DL .9	2	.5	3	-.866	9	-1			
32	0.9DL + 1...	Yes	Y		DL .9	2	.707	3	-.707	10	-1			
33	0.9DL + 1...	Yes	Y		DL .9	2	.866	3	-.5	11	-1			
34	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	1	14		15	1	
35	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	.866	14	.5	16	1	
36	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	.707	14	.707	17	1	
37	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	.5	14	.866	18	1	
38	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13		14	1	19	1	
39	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	-.5	14	.866	20	1	
40	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	-.707	14	.707	21	1	
41	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	-.866	14	.5	22	1	
42	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	-1	14		15	-1	
43	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	-.866	14	-.5	16	-1	
44	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	-.707	14	-.707	17	-1	
45	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	-.5	14	-.866	18	-1	
46	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13		14	-1	19	-1	
47	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	.5	14	-.866	20	-1	
48	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	.707	14	-.707	21	-1	
49	1.2DL + 1...	Yes	Y		DL 1.2	OL1	1	13	.866	14	-.5	22	-1	
50	(1.2+0.2S...	Yes	Y		DL 1.235	23	1	24						
51	(1.2+0.2S...	Yes	Y		DL 1.235	23	.866	24	.5					
52	(1.2+0.2S...	Yes	Y		DL 1.235	23	.707	24	.707					
53	(1.2+0.2S...	Yes	Y		DL 1.235	23	.5	24	.866					
54	(1.2+0.2S...	Yes	Y		DL 1.235	23		24	1					
55	(1.2+0.2S...	Yes	Y		DL 1.235	23	-.5	24	.866					
56	(1.2+0.2S...	Yes	Y		DL 1.235	23	-.707	24	.707					
57	(1.2+0.2S...	Yes	Y		DL 1.235	23	-.866	24	.5					
58	(1.2+0.2S...	Yes	Y		DL 1.235	23	-1	24						
59	(1.2+0.2S...	Yes	Y		DL 1.235	23	-.866	24	-.5					
60	(1.2+0.2S...	Yes	Y		DL 1.235	23	-.707	24	-.707					
61	(1.2+0.2S...	Yes	Y		DL 1.235	23	-.5	24	-.866					
62	(1.2+0.2S...	Yes	Y		DL 1.235	23		24	-1					
63	(1.2+0.2S...	Yes	Y		DL 1.235	23	.5	24	-.866					
64	(1.2+0.2S...	Yes	Y		DL 1.235	23	.707	24	-.707					
65	(1.2+0.2S...	Yes	Y		DL 1.235	23	.866	24	-.5					



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**Load Combinations (Continued)**

	Description	Sol.	PD	SR	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact	BLC Fact
66	(0.9-0.2Sd...	Yes	Y		DL	.865	23	1	24				
67	(0.9-0.2Sd...	Yes	Y		DL	.865	23	.866	24	.5			
68	(0.9-0.2Sd...	Yes	Y		DL	.865	23	.707	24	.707			
69	(0.9-0.2Sd...	Yes	Y		DL	.865	23	.5	24	.866			
70	(0.9-0.2Sd...	Yes	Y		DL	.865	23		24	1			
71	(0.9-0.2Sd...	Yes	Y		DL	.865	23	-.5	24	.866			
72	(0.9-0.2Sd...	Yes	Y		DL	.865	23	-.707	24	.707			
73	(0.9-0.2Sd...	Yes	Y		DL	.865	23	-.866	24	.5			
74	(0.9-0.2Sd...	Yes	Y		DL	.865	23	-1	24				
75	(0.9-0.2Sd...	Yes	Y		DL	.865	23	-.866	24	-.5			
76	(0.9-0.2Sd...	Yes	Y		DL	.865	23	-.707	24	-.707			
77	(0.9-0.2Sd...	Yes	Y		DL	.865	23	-.5	24	-.866			
78	(0.9-0.2Sd...	Yes	Y		DL	.865	23		24	-1			
79	(0.9-0.2Sd...	Yes	Y		DL	.865	23	.5	24	-.866			
80	(0.9-0.2Sd...	Yes	Y		DL	.865	23	.707	24	-.707			
81	(0.9-0.2Sd...	Yes	Y		DL	.865	23	.866	24	-.5			
82	1.2DL + 1...	Yes	Y		DL	1.2	25	1.5					
83	1.2DL + 1...	Yes	Y		DL	1.2	26	1.5					
84	1.2DL + 1...	Yes	Y		DL	1.2	27	1.5					
85	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	.049	3		4 .049
86	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	.043	3	.025	5 .049
87	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	.035	3	.035	6 .049
88	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	.025	3	.043	7 .049
89	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2		3	.049	8 .049
90	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	-.025	3	.043	9 .049
91	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	-.035	3	.035	10 .049
92	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	-.043	3	.025	11 .049
93	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	-.049	3		4 -.049
94	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	-.043	3	-.025	5 -.049
95	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	-.035	3	-.035	6 -.049
96	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	-.025	3	-.043	7 -.049
97	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2		3	-.049	8 -.049
98	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	.025	3	-.043	9 -.049
99	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	.035	3	-.035	10 -.049
100	1.2DL + 1...	Yes	Y		DL	1.2	28	1.5	2	.043	3	-.025	11 -.049
101	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	.049	3		4 .049
102	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	.043	3	.025	5 .049
103	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	.035	3	.035	6 .049
104	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	.025	3	.043	7 .049
105	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2		3	.049	8 .049
106	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	-.025	3	.043	9 .049
107	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	-.035	3	.035	10 .049
108	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	-.043	3	.025	11 .049
109	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	-.049	3		4 -.049
110	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	-.043	3	-.025	5 -.049
111	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	-.035	3	-.035	6 -.049
112	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	-.025	3	-.043	7 -.049
113	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2		3	-.049	8 -.049
114	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	.025	3	-.043	9 -.049
115	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	.035	3	-.035	10 -.049
116	1.2DL + 1...	Yes	Y		DL	1.2	29	1.5	2	.043	3	-.025	11 -.049
117	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	.049	3		4 .049
118	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	.043	3	.025	5 .049
119	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	.035	3	.035	6 .049
120	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	.025	3	.043	7 .049
121	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2		3	.049	8 .049
122	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	-.025	3	.043	9 .049



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**Load Combinations (Continued)**

	Description	Sol.	PD	SR	BLC Fact.	BLC Fact.	BLC Fact.	BLC Fact.	BLC Fact.	BLC Fact.	BLC Fact.	BLC Fact.	BLC Fact.	BLC Fact.
123	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	-.035	3	.035	10	.049
124	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	-.043	3	.025	11	.049
125	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	-.049	3		4	-.049
126	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	-.043	3	-.025	5	-.049
127	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	-.035	3	-.035	6	-.049
128	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	-.025	3	-.043	7	-.049
129	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2		3	-.049	8	-.049
130	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	.025	3	-.043	9	-.049
131	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	.035	3	-.035	10	-.049
132	1.2DL + 1...	Yes	Y		DL	1.2	30	1.5	2	.043	3	-.025	11	-.049

**Envelope Joint Reactions**

Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-ft]	LC	MY [lb-ft]	LC	MZ [lb-ft]	LC	
1	N5	max	904.41	33	1239.418	122	1899.267	34	295.978	98	-532.686	26	0	132
2		min	-2204.117	41	-967.517	98	509.562	26	-391.764	122	-2047.394	34	0	1
3	N17	max	1939.706	48	936.925	94	599.155	42	132.782	98	-110.646	18	0	132
4		min	402.442	23	-1208.011	118	127.52	18	-175.695	122	-640.629	42	0	1
5	N79A	max	927.117	4	293.534	4	22.538	44	0	132	0	132	0	132
6		min	-856.786	28	-271.739	28	4.663	20	0	1	0	1	0	1
7	N80	max	405.262	85	123.777	86	23.048	45	0	132	0	132	0	132
8		min	-475.155	125	-145.715	126	5.381	70	0	1	0	1	0	1
9	Totals:	max	1722.131	2	881.853	7	2526.172	40						
10		min	-1722.131	26	-881.852	31	707.68	79						

**Envelope AISC 15th(360-16): LRFD Steel Code Checks**

Member	Shape	Code Check	Loc[in]	LC Shear	...	Loc[in]	Dir	LC phi*Pnc	...	phi*Pnt	...	phi*Mn y	...	phi*Mn z	Cb	Eqn
1	MP3	PIPE 2.0	.727	40	2	.234	40	2	14916.0...	32130	1871.625	1871.625	1	H1-1b		
2	M13	PL 4"x0.625"	.604	6	1...	.185	6	y	1...76418.78	81000	1054.688	6750	1	H1-1b		
3	MP1	PIPE 2.0	.542	80	96	.135	40	2	14916.0...	32130	1871.625	1871.625	1	H1-1b		
4	M19	PIPE 2.5	.530	43.75	10	.217	45.313	10	41048.6...	50715	3596.25	3596.25	1	H1-1b		
5	MP2	PIPE 2.0	.442	40	10	.054	40	10	14916.0...	32130	1871.625	1871.625	1...	H1-1b		
6	M16	PL 4"x0.625"	.373	6	1...	.098	6	y	1...76418.78	81000	1054.688	6750	1...	H1-1b		
7	M45	SR 5/8 HRB	.337	18.75	34	.011	0	1...	2503.081	9940.19	103.542	103.542	1	H1-1a		
8	M42	PL 4"x0.625"	.310	4.512	1...	.074	2.209	y	1...79311.0...	81000	1054.688	6750	1...	H1-1b		
9	H1	PIPE 2.5	.301	43.75	1...	.098	43.75	10	41048.6...	50715	3596.25	3596.25	1	H1-1b		
10	M39	SR 5/8 HRB	.280	14.583	85	.015	0	1...	2503.081	9940.19	103.542	103.542	1	H1-1a		
11	M17	PL 4"x0.625"	.270	4.512	96	.066	2.209	y	8679311.0...	81000	1054.688	6750	1...	H1-1b		
12	M41	PL 4"x0.625"	.260	2.209	1...	.095	4.512	y	1...79311.0...	81000	1054.688	6750	1	H1-1b		
13	M40	PL 4"x0.625"	.230	0	1...	.095	2.303	y	1...79311.0...	81000	1054.688	6750	1...	H1-1b		
14	M15	PL 4"x0.625"	.220	2.209	85	.084	2.209	y	1...79311.0...	81000	1054.688	6750	1	H1-1b		
15	M24	PIPE 2.0	.213	2.188	1...	.065	2.188	1...	29810.2...	32130	1871.625	1871.625	1...	H1-1b		
16	M22	PIPE 2.0	.197	2.188	1...	.083	27.813	1...	29810.2...	32130	1871.625	1871.625	1	H1-1b		
17	M14	PL 4"x0.625"	.194	0	96	.084	0	y	1...79311.0...	81000	1054.688	6750	1	H1-1b		
18	M21	PIPE 2.0	.176	27.813	85	.055	27.813	1...	29810.2...	32130	1871.625	1871.625	1...	H1-1b		
19	M1	SR 3/4"	.159	0	1...	.007	0	1...	4289.781	14313.8...	178.929	178.929	1	H1-1b*		
20	M23	PIPE 2.0	.150	2.188	1...	.074	27.813	86	29810.2...	32130	1871.625	1871.625	1	H1-1b		
21	M44	SR 5/8 HRB	.146	40	41	.013	0	1...	2503.081	9940.19	103.542	103.542	1	H1-1b*		
22	M3	SR 3/4"	.142	44	98	.007	0	5	4289.781	14313.8...	178.929	178.929	1	H1-1b*		
23	M38	SR 5/8 HRB	.131	40	94	.017	0	1...	2503.081	9940.19	103.542	103.542	1	H1-1b*		
24	M43	PL 4"x0.625"	.112	2.209	1...	.074	4.512	y	1...79311.0...	81000	1054.688	6750	1...	H1-1b		
25	M34	PL 4"x0.625"	.092	2.209	94	.066	4.512	y	8679311.0...	81000	1054.688	6750	1	H1-1b		
26	M29	PL 2"x0.625"	.038	4.46	45	.003	4.46	y	1...39953.2...	40500	527.345	1687.5	1...	H1-1b		
27	M27	PL 2"x0.625"	.036	0	35	.003	0	y	1739953.2...	40500	527.345	1687.5	1...	H1-1b		
28	M56	PIPE 2.0	.036	42.863	4	.002	0	46	27148.6	32130	1871.625	1871.625	1	H1-1b*		



Company : Trylon  
 Designer : AF  
 Job Number : 190496  
 Model Name : 876338

Aug 24, 2021  
 12:10 PM  
 Checked By: JW

**Envelope AISC 15th(360-16): LRFD Steel Code Checks (Continued)**

Member	Shape	Code Check	Loc[in]	LC Shear	Dir	LC phi*Pnc	phi*Pnt	phi*Mn y	phi*Mn z	Cb	Eqn		
29	M25	PL 2"x0.625"	.032	0 90	.003	0	y 5	39953.2...	40500	527.345	1687.5	1...	H1-1b
30	M31	PL 2"x0.625"	.029	4.46 98	.003	4.46	y 5	39953.2...	40500	527.345	1687.5	1	H1-1b
31	M58A	PIPE 2.0	.016	42.863 85	.002	0	46	27148.6	32130	1871.625	1871.625	1	H1-1b*
32	M28	PL 2"x0.625"	.005	0 37	.001	0	y 40	39953.2...	40500	527.345	1687.5	1...	H1-1b
33	M30	PL 2"x0.625"	.005	4.46 37	.001	4.46	y 40	39953.2...	40500	527.345	1687.5	1...	H1-1b
34	M32	PL 2"x0.625"	.005	4.46 47	.001	4.46	y 44	39953.2...	40500	527.345	1687.5	1...	H1-1b
35	M26	PL 2"x0.625"	.005	0 47	.001	0	y 44	39953.2...	40500	527.345	1687.5	1...	H1-1b
36	M2	SR 3/4"	.000	0 1...	.000	0	1...	4289.781	14313.8...	178.929	178.929	1	H1-1a
37	M4	SR 3/4"	.000	0 1...	.000	0	1...	4289.781	14313.8...	178.929	178.929	1	H1-1a

**Envelope AISI S100-16: LRFD Cold Formed Steel Code Checks**

Member	Shape	Code	Loc[in]	LC Shear	Dir	LC phi*Pn	phi*Tn	phi*Mny	phi*Mnz	phi*V	phi*V	Cb	Eqn
No Data to Print ...													

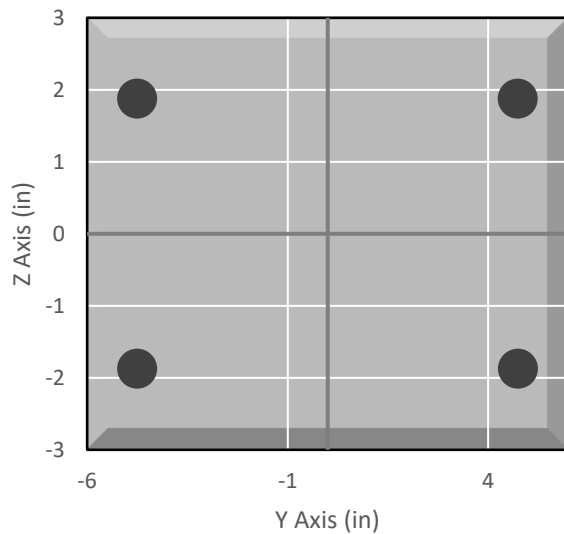
**APPENDIX D**  
**ADDITIONAL CALCUATIONS**

**BOLT TOOL 1.5.2**

Project Data	
Job Code:	190496
Carrier Site ID:	CT11381C
Carrier Site Name:	WATERFORD SOUTH/RT

Code	
Design Standard:	TIA-222-H
Slip Check:	Yes
Pretension Standard:	TIA-222-H

Bolt Properties		
Connection Type:	Threaded Rod	
Diameter:	0.625	in
Grade:	AE J429 Gr.	--
Yield Strength (Fy):	57	ksi
Ultimate Strength (Fu):	74	ksi
Number of Bolts:	4	--
Threads Included:	Yes	--
Double Shear:	No	--
Connection Pipe Size:	9.5	in

**Bolt Layout**


Connection Description
Mounting Kit to Monopole Collar

Bolt Check*		
Tensile Capacity ( $\phi T_n$ ):	12543.1	lbs
Shear Capacity ( $\phi V_n$ ):	8513.6	lbs
Tension Force ( $T_u$ ):	3786.3	lbs
Shear Force ( $V_u$ ):	610.8	lbs
Tension Usage:	28.7%	--
Shear Usage:	6.8%	--
Interaction:	28.7%	Pass
Controlling Member:	M7	--
Controlling LC:	41	--

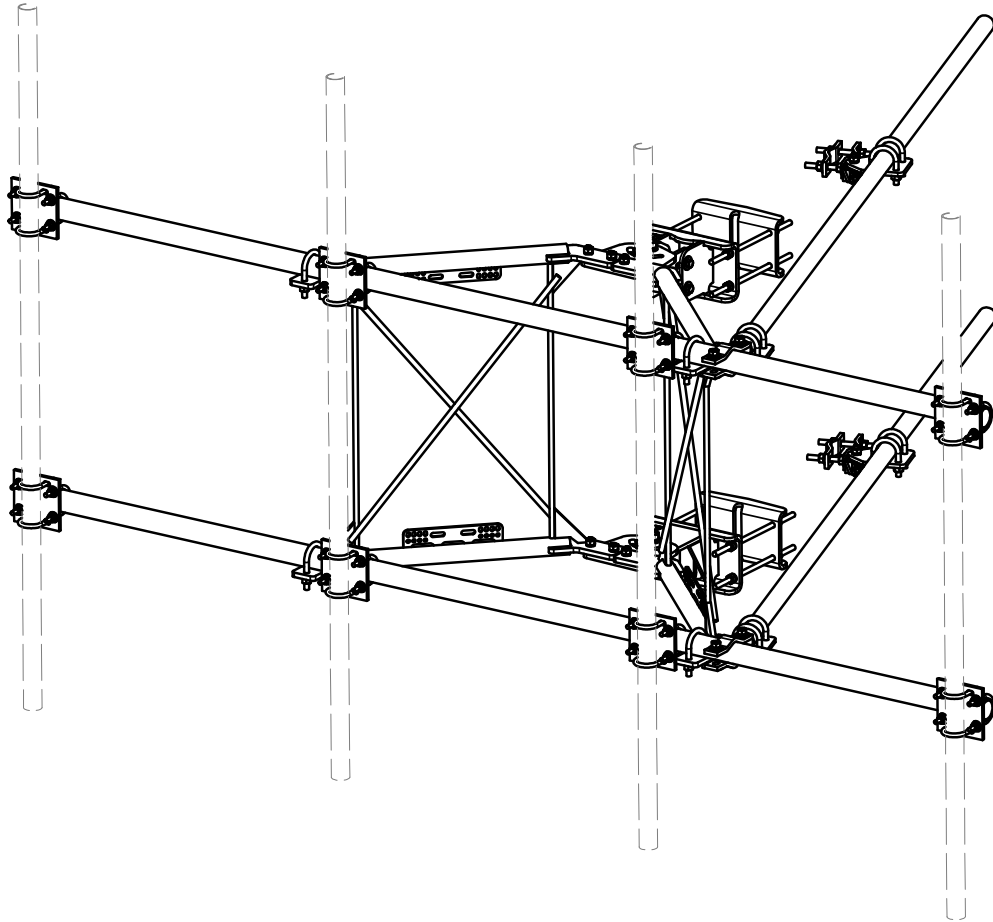
\*Rating per TIA-222-H Section 15.5

Slip Check*		
Sliding Capacity ( $\phi R_{ns}$ ):	13403.7	lbs
Torsion Capacity ( $\phi R_{nr}$ ):	5305.6	lb-ft
Sliding Force ( $V_{us}$ ):	1883.3	lbs
Torsional Force ( $T_{ur}$ ):	0.0	lb-ft
Sliding Usage:	13.4%	--
Torsion Usage:	0.0%	--
Interaction:	13.4%	Pass
Controlling Member:	M7	--
Controlling LC:	41	--

\*Rating per TIA-222-H Section 15.5



**APPENDIX E**  
**SUPPLEMENTAL DRAWINGS**



PARTS LIST						
ITEM	QTY	PART NO.	PART DESCRIPTION	LENGTH	UNIT WT.	NET WT.
1	2	X-VFAW	SUPPORT ARM		71.41	142.81
2	1	X-HDCAMTBW	CLAMP WELDMENT FOR BCAM-HD		33.86	33.86
3	1	X-MHTPHD	MULTI-HOLE TAPER PLATE WELDMENT		36.24	36.24
4	2	X-VFAPL4	VFA-HD PIVOT PLATE	12 in	15.88	31.77
5	2	X-LCBP4	BENT BACKING PLATE	13 in	19.00	38.01
6	1	X-HDCAMSS	ANGLE ADJUSTMENT WELDMENT FOR BCAM-HD		16.39	16.39
7	4	X-SPTB	SLIDING PIPE TIE BACK PLATE	5 1/2 in	5.87	23.49
8	1	X-HDCAMSP	POSITIONING PLATE WELDMENT FOR BCAM-HD		2.58	2.58
9	4	X-TBCA	TIE BACK CLIP ANGLE		2.01	8.02
10	8	SCX2	CROSSOVER PLATE	7 in	4.80	38.37
11	4	MCP	CLAMP HALF 1/2" THICK, 11-5/8" LONG	12 1/16 in	3.59	14.37
12	8	DCP	1/2" THICK, 5-3/4" CTR TO CENTER CLAMP HALF	8 1/8 in	2.36	18.90
13	2	P2126	2-3/8" X 126" (2" SCH. 40) GALVANIZED PIPE	126 in	40.75	81.50
14	2	P30150	2-7/8" X 150" (2-1/2" SCH. 40) GALVANIZED PIPE	150 in	76.94	153.87
15	4	A34212	3/4" x 2-1/2" UNC HEX BOLT (A325)	2 1/2 in	0.48	1.92
16	4	G34FW	3/4" HDG USS FLATWASHER		0.06	0.24
17	4	G34LW	3/4" HDG LOCKWASHER		0.04	0.17
18	4	G34NUT	3/4" HDG HEAVY 2H HEX NUT		0.21	0.85
19	8	G58R-18	5/8" x 18" THREADED ROD (HDG.)	18 in	0.40	3.19
20	4	G58R-12	5/8" x 12" THREADED ROD (HDG.)		1.05	4.18
21	4	G58R-8	5/8" x 8" THREADED ROD (HDG.)		0.70	2.79
22	4	X-UB5300	5/8" X 3" X 5-1/4" X 2-1/2" U-BOLT (HDG.)		1.15	4.60
23	8	X-UB5258	5/8" X 2-5/8" X 4-1/2" X 2" U-BOLT (HDG.)		1.00	8.00
24	2	G5807	5/8" x 7" HDG HEX BOLT GR5 FULL THREAD	7 in	0.70	1.41
25	1	G5806	5/8" x 6" HDG HEX BOLT GR5 FULL THREAD	6 in	0.62	0.62
26	8	G5804	5/8" x 4" HDG HEX BOLT GR5		0.44	3.55
27	4	G5802	5/8" x 2" HDG HEX BOLT GR5		0.27	1.08
28	8	A582114	5/8" x 2-1/4" HDG A325 HEX BOLT	2 1/4 in	0.31	2.50
29	25	G58FW	5/8" HDG USS FLATWASHER	1/8 in	0.07	1.76
30	66	G58LW	5/8" HDG LOCKWASHER		0.03	1.72
31	71	G58NUT	5/8" HDG HEAVY 2H HEX NUT		0.13	9.22
32	32	X-UB1300	1/2" X 3" X 5" X 2" GALV U-BOLT		0.74	23.64
33	16	X-UB1212	1/2" X 2" X 3" X 1-1/4" U-BOLT (HDG.)		0.60	9.56
34	64	G12FW	1/2" HDG USS FLATWASHER	3/32 in	0.03	2.18
35	64	G12LW	1/2" HDG LOCKWASHER	1/8 in	0.01	0.89
36	64	G12NUT	1/2" HDG HEAVY 2H HEX NUT		0.07	4.58
					TOTAL WT. #	738.06

REV	DESCRIPTION OF REVISIONS	CPD	BY	DATE
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B	CHANGED TIE-BACK BACK CONNECTION		CEK	7/31/2017
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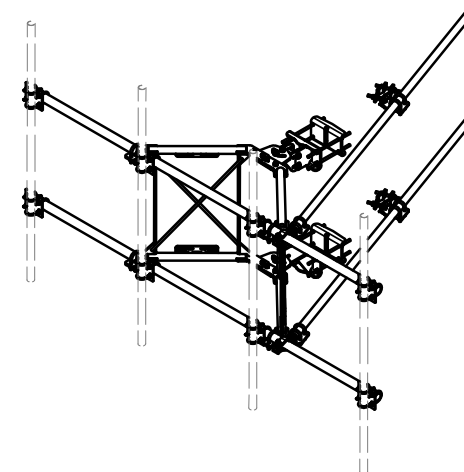
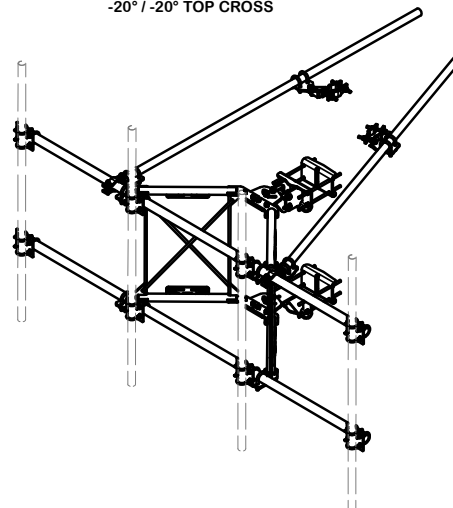
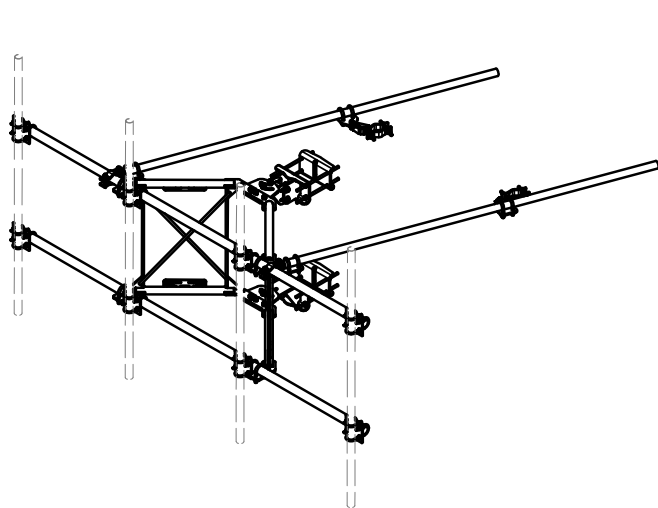
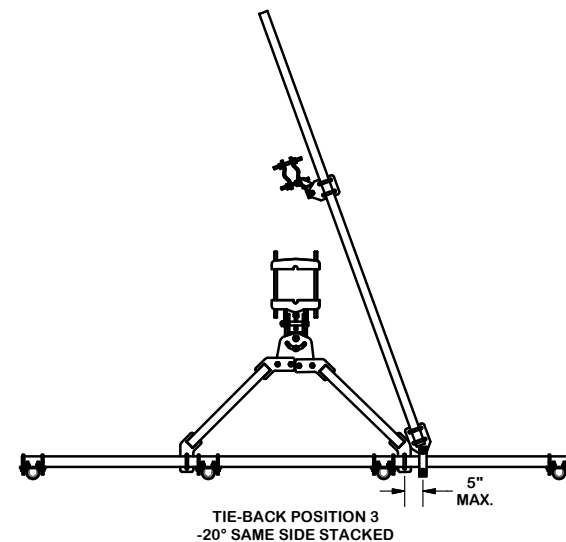
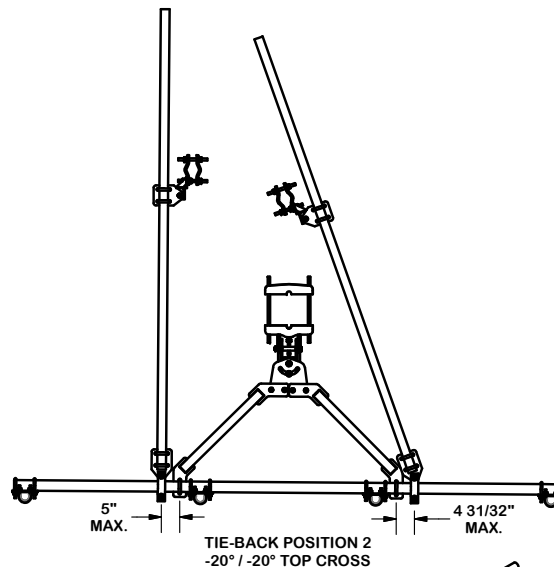
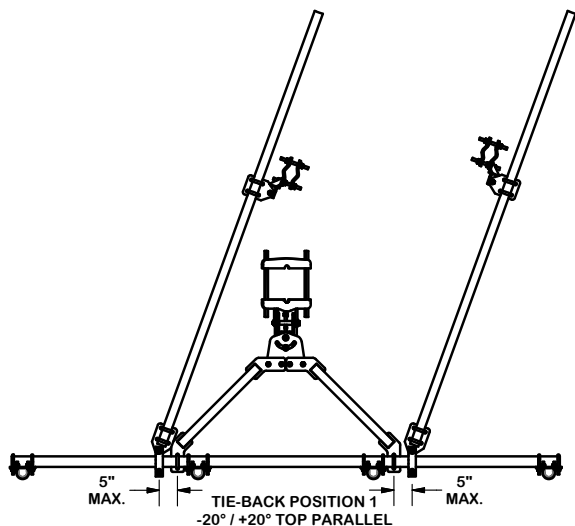
**TOLERANCE NOTES**  
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 ALL OTHER MACHINING ( $\pm 0.030"$ )  
 ALL OTHER ASSEMBLY ( $\pm 0.060"$ )**

PROPRIETARY NOTE:  
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DESCRIPTION		12' 6" HEAVY DUTY V-FRAME ASSEMBLY WITH TWO STIFF ARMS	
CPD NO.	DRAWN BY	ENG. APPROVAL	
	CEK 1/25/2017		
CLASS	SUB	DRAWING USAGE	CHECKED BY
81	02	CUSTOMER	BMC 12/13/2017

 <b>A valmont COMPANY</b>	Locations: New York, NY Atlanta, GA Los Angeles, CA Plymouth, IN Salem, OR Dallas, TX
	Engineering Support Team: 1-888-753-7446
PART NO.	VFA12-HD
DWG. NO.	VFA12-HD

# TIE-BACK POSITIONS



REV	DESCRIPTION OF REVISIONS	CPD	BY	DATE
D	UPDATED BCAM VERSION 1 TO BCAM VERSION 2		CEK	6/29/2018
C	UPDATED PIN LEG CONNECTION TO B-CAM CONNECTION		CEK	12/7/2017
B	CHANGED TIE-BACK BACK CONNECTION		CEK	7/31/2017
A	CHANGED TIE-BACK FRONT CONNECTION		CEK	2/2/2017

REVISION HISTORY

**TOLERANCE NOTES**

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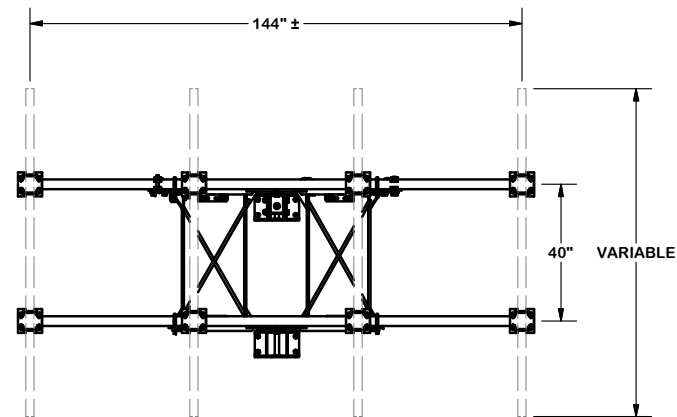
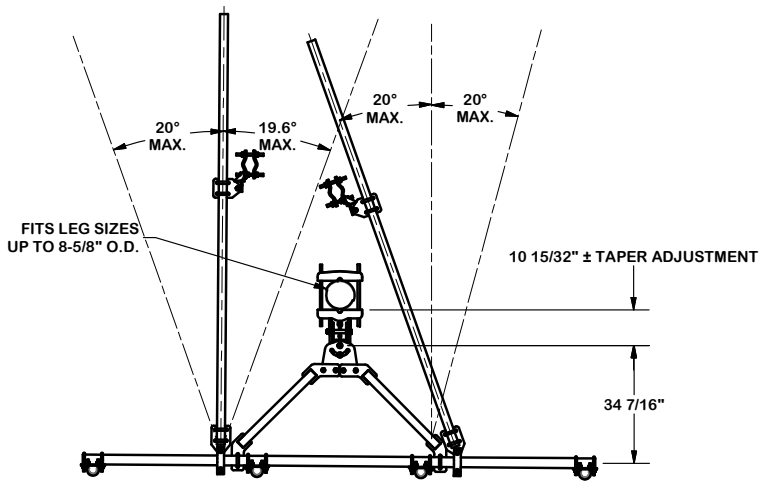
DESCRIPTION  
 12' 6" HEAVY DUTY  
 V-FRAME ASSEMBLY  
 WITH TWO STIFF ARMS

CPD NO.	DRAWN BY	ENG. APPROVAL
	CEK 1/25/2017	
CLASS	DRAWING USAGE	CHECKED BY
81	CUSTOMER	BMC 12/13/2017

**SITE PRO 1**  
 Engineering Support Team:  
 1-888-753-7446

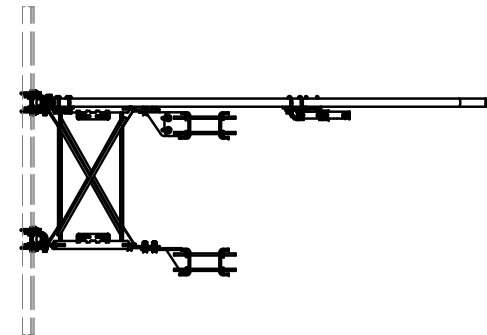
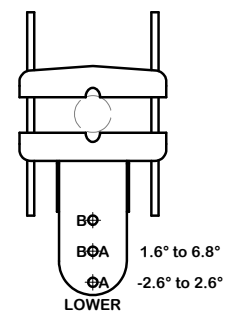
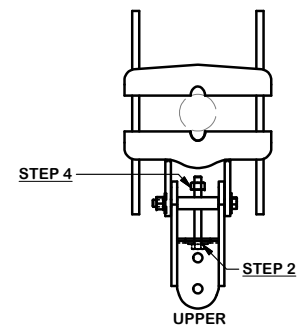
Locations:  
 New York, NY  
 Atlanta, GA  
 Los Angeles, CA  
 Plymouth, IN  
 Salem, OR  
 Dallas, TX

PART NO.	VFA12-HD
DWG. NO.	VFA12-HD



**ANGLE CALIBRATING PROCEDURE:**

1. MEASURE TOWER TAPER AND PICK LOWER BRACKET HOLE:
  - HOLE A = -2.6° TO 2.6°
  - HOLE B = 1.6° TO 6.8°
2. USE CALIBRATING BOLT TO ADJUST FRAME TO DESIRED TAPER
3. TORQUE LOCKING BOLTS TO 100 ft.-lbs.
4. ADVANCE LOCKING NUT TO POSITIONING PLATE, THEN TIGHTEN.



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**REVISION HISTORY**

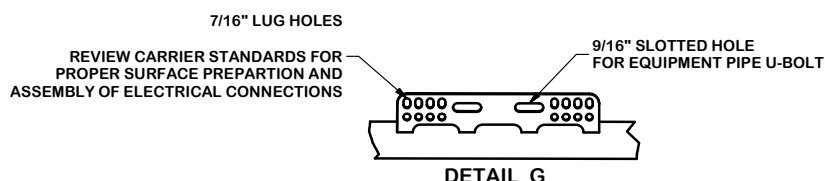
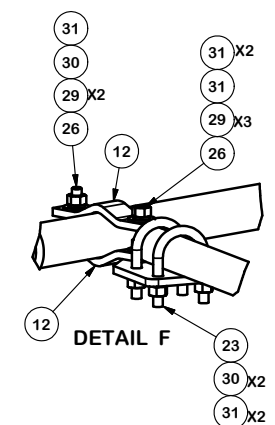
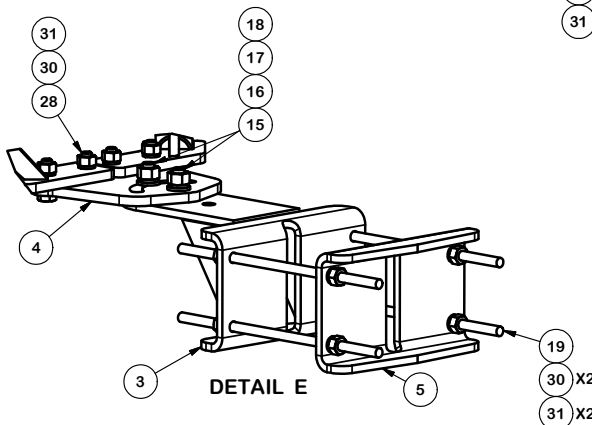
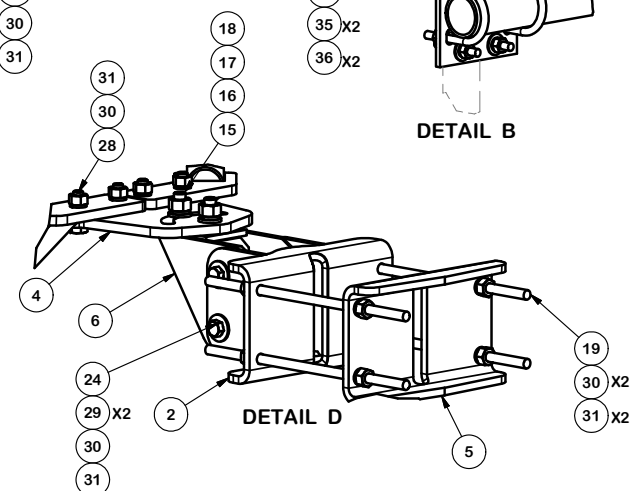
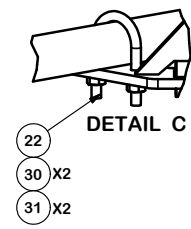
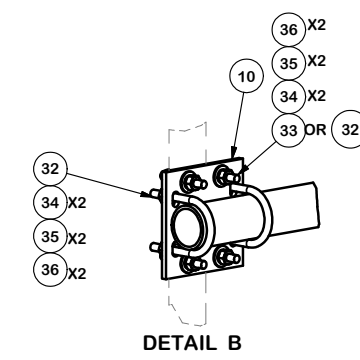
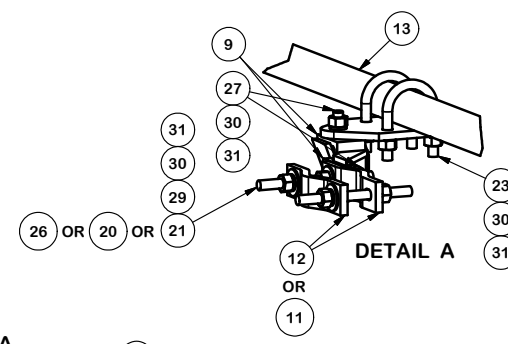
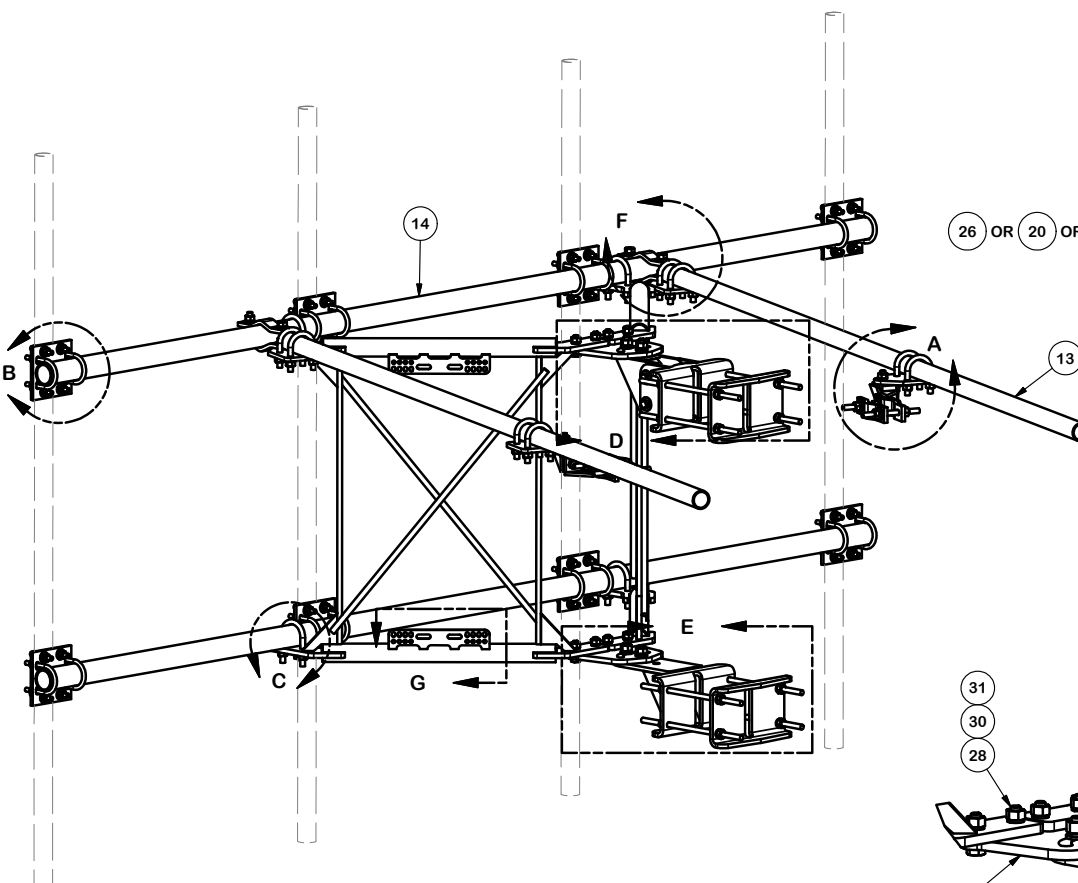
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CPD NO.	DRAWN BY	ENG. APPROVAL	
	CEK 1/25/2017		
CLASS	SUB	DRAWING USAGE	CHECKED BY
81	02	CUSTOMER	BMC 12/13/2017

 A valmont COMPANY	Locations: New York, NY Atlanta, GA Los Angeles, CA Plymouth, IN Salem, OR Dallas, TX
	Engineering Support Team: 1-888-753-7446
PART NO.	VFA12-HD
DWG. NO.	VFA12-HD



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REVISION HISTORY				

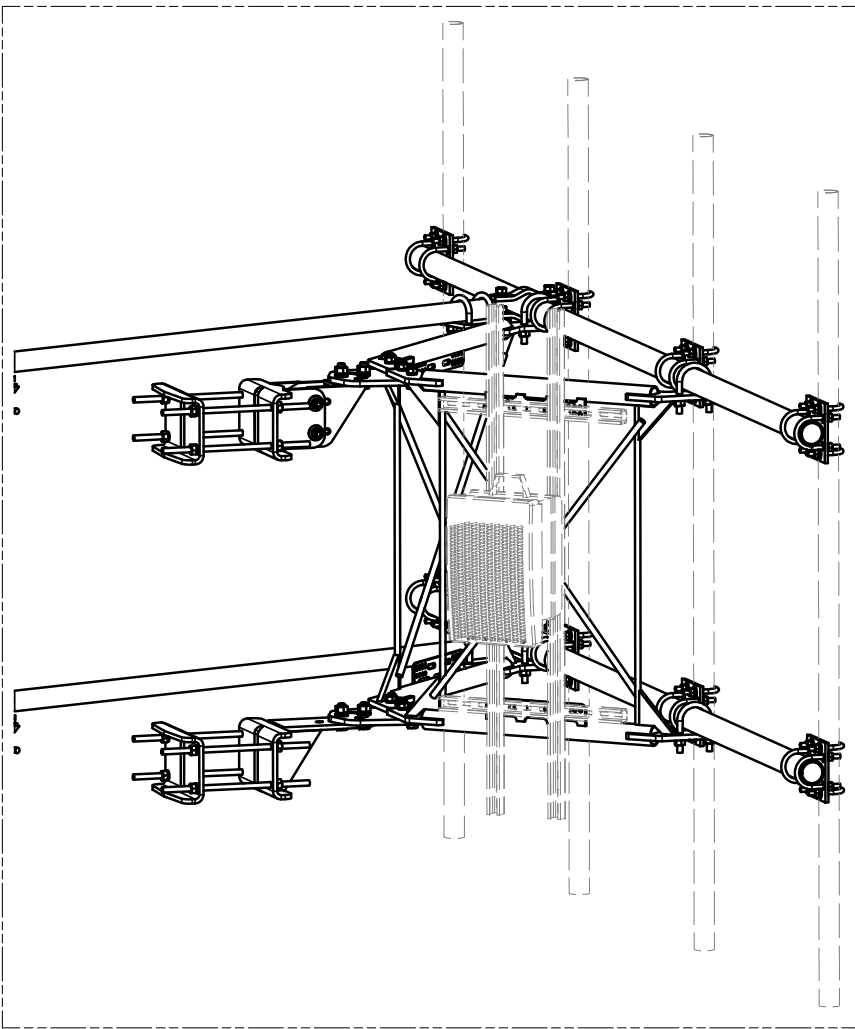
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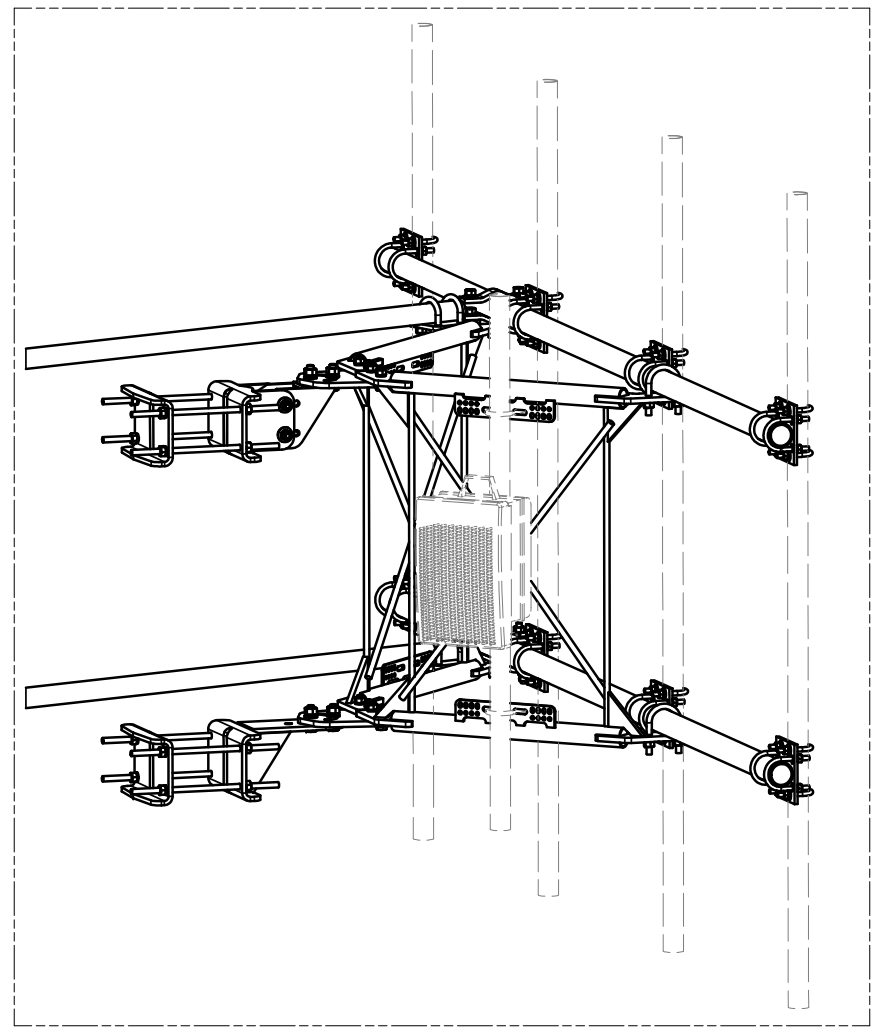
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 A valmont COMPANY	Locations: New York, NY Atlanta, GA Los Angeles, CA Plymouth, IN Salem, OR Dallas, TX	
	Engineering Support Team: 1-888-753-7446	
PART NO.	VFA12-HD	4 OF 5
DWG. NO.	VFA12-HD	



UNISTRUT AND HARDWARE  
SOLD SEPARATELY.

REQUIRES 3/8" HARDWARE



EQUIPMENT PIPE AND HARDWARE  
SOLD SEPARATELY.

REQUIRES 1/2" HARDWARE  
AND 2-3/8" TO 4-1/2" O.D. PIPE

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 WITH TWO STIFF ARMS

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	CEK 1/25/2017	
CLASS	DRAWING USAGE	CHECKED BY
81	CUSTOMER	BMC 12/13/2017

 A valmont COMPANY	Engineering Support Team: 1-888-753-7446	Locations: New York, NY Atlanta, GA Los Angeles, CA Plymouth, IN Salem, OR Dallas, TX
	PART NO. <b>VFA12-HD</b>	DWG. NO. <b>VFA12-HD</b>

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT  
EVALUATION OF HUMAN EXPOSURE POTENTIAL  
TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11381C

876338

41 Manitock Hill Road  
Waterford, Connecticut 06385

**October 13, 2021**

**EBI Project Number: 6221005725**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general population allowable limit:	<b>36.97%</b>

October 13, 2021

T-Mobile

Attn: Jason Overbey, RF Manager  
35 Griffin Road South  
Bloomfield, Connecticut 06002

Emissions Analysis for Site: CT11381C - 876338

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **41 Manitock Hill Road** in **Waterford, Connecticut** for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately  $400 \mu\text{W}/\text{cm}^2$  and  $467 \mu\text{W}/\text{cm}^2$ , respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## **CALCULATIONS**

Calculations were done for the proposed T-Mobile Wireless antenna facility located at 41 Manitock Hill Road in Waterford, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower. For power density calculations, the broadcast footprint of the AIR6449 antenna has been considered. Due to the beamforming nature of this antenna, the actual beam locations vary depending on demand and are narrow in nature. Using the broadcast footprint accounts for the potential location of beams at any given time.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 LTE channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 1 NR channel (600 MHz Band) was considered for each sector of the proposed installation. This Channel has a transmit power of 80 Watts.
- 3) 2 LTE channels (700 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 4) 4 GSM channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 5) 2 UMTS channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.

- 6) 2 LTE channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 7) 2 LTE channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 8) 1 LTE Traffic channel (LTE IC and 2C BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 60 Watts.
- 9) 1 LTE Broadcast channel (LTE IC and 2C BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 20 Watts.
- 10) 1 NR Traffic channel (BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 120 Watts.
- 11) 1 NR Broadcast channel (BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 40 Watts.
- 12) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 13) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 14) The antennas used in this modeling are the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s), the RFS APXVAALL24\_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 1900 MHz / 2100 MHz channel(s) in Sector A, the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s), the RFS APXVAALL24\_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 1900 MHz / 2100 MHz channel(s) in Sector B, the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s), the RFS APXVAALL24\_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 1900 MHz / 2100 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna

selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

- 15) The antenna mounting height centerline of the proposed antennas is 119 feet above ground level (AGL).
- 16) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 17) All calculations were done with respect to uncontrolled / general population threshold limits.

## T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Ericsson AIR 6449	Make / Model:	Ericsson AIR 6449	Make / Model:	Ericsson AIR 6449
Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz
Gain:	22.65 dBd / 17.3 dBd / 22.65 dBd / 17.3 dBd	Gain:	22.65 dBd / 17.3 dBd / 22.65 dBd / 17.3 dBd	Gain:	22.65 dBd / 17.3 dBd / 22.65 dBd / 17.3 dBd
Height (AGL):	119 feet	Height (AGL):	119 feet	Height (AGL):	119 feet
Channel Count:	4	Channel Count:	4	Channel Count:	4
Total TX Power (W):	240 Watts	Total TX Power (W):	240 Watts	Total TX Power (W):	240 Watts
ERP (W):	36,356.09	ERP (W):	36,356.09	ERP (W):	36,356.09
Antenna AI MPE %:	10.24%	Antenna BI MPE %:	10.24%	Antenna CI MPE %:	10.24%
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	RFS APXVAALL24_43-U-NA20	Make / Model:	RFS APXVAALL24_43-U-NA20	Make / Model:	RFS APXVAALL24_43-U-NA20
Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 2100 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 2100 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz / 1900 MHz / 2100 MHz
Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd / 15.45 dBd / 15.45 dBd / 15.45 dBd / 16.45 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd / 15.45 dBd / 15.45 dBd / 15.45 dBd / 16.45 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd / 15.45 dBd / 15.45 dBd / 15.45 dBd / 16.45 dBd
Height (AGL):	119 feet	Height (AGL):	119 feet	Height (AGL):	119 feet
Channel Count:	15	Channel Count:	15	Channel Count:	15
Total TX Power (W):	620 Watts	Total TX Power (W):	620 Watts	Total TX Power (W):	620 Watts
ERP (W):	19,973.23	ERP (W):	19,973.23	ERP (W):	19,973.23
Antenna A2 MPE %:	7.24%	Antenna B2 MPE %:	7.24%	Antenna C2 MPE %:	7.24%

Site Composite MPE %	
Carrier	MPE %
T-Mobile (Max at Sector A):	17.47%
Nextel	0.39%
Sprint	3.09%
Metro PCS	0.69%
AT&T	8.75%
Verizon	6.58%
<b>Site Total MPE % :</b>	<b>36.97%</b>

T-Mobile MPE % Per Sector	
T-Mobile Sector A Total:	17.47%
T-Mobile Sector B Total:	17.47%
T-Mobile Sector C Total:	17.47%
Site Total MPE % :	36.97%

### T-Mobile Maximum MPE Power Values (Sector A)

T-Mobile Frequency Band / Technology (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
T-Mobile 2500 MHz LTE IC & 2C Traffic	1	11044.63	119.0	31.10	2500 MHz LTE IC & 2C Traffic	1000	3.11%
T-Mobile 2500 MHz LTE IC & 2C Broadcast	1	1074.06	119.0	3.02	2500 MHz LTE IC & 2C Broadcast	1000	0.30%
T-Mobile 2500 MHz NR Traffic	1	22089.26	119.0	62.19	2500 MHz NR Traffic	1000	6.22%
T-Mobile 2500 MHz NR Broadcast	1	2148.13	119.0	6.05	2500 MHz NR Broadcast	1000	0.60%
T-Mobile 600 MHz LTE	2	591.73	119.0	3.33	600 MHz LTE	400	0.83%
T-Mobile 600 MHz NR	1	1577.94	119.0	4.44	600 MHz NR	400	1.11%
T-Mobile 700 MHz LTE	2	695.22	119.0	3.91	700 MHz LTE	467	0.84%
T-Mobile 1900 MHz GSM	4	1052.26	119.0	11.85	1900 MHz GSM	1000	1.19%
T-Mobile 1900 MHz UMTS	2	1052.26	119.0	5.93	1900 MHz UMTS	1000	0.59%
T-Mobile 1900 MHz LTE	2	2104.51	119.0	11.85	1900 MHz LTE	1000	1.19%
T-Mobile 2100 MHz LTE	2	2649.42	119.0	14.92	2100 MHz LTE	1000	1.49%
						<b>Total:</b>	<b>17.47%</b>

• NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.

## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector A:	17.47%
Sector B:	17.47%
Sector C:	17.47%
T-Mobile Maximum MPE % (Sector A):	17.47%
Site Total:	36.97%
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **36.97%** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

# T-Mobile

**T-MOBILE SITE NUMBER: CT11381C**

**T-MOBILE SITE NAME: WATERFORD SOUTH/RT 1**

**SITE TYPE: SELF-SUPPORT TOWER**

**TOWER HEIGHT: 136'-0"**

**T-MOBILE ANCHOR SITE CONFIGURATION: 67D5A998E OUTDOOR**

**BUSINESS UNIT #: 876338**

**SITE ADDRESS: 41 MANITOCK HILL ROAD  
WATERFORD, CT 06385**

**COUNTY: NEW LONDON**

**JURISDICTION: CONNECTICUT**

**SITING COUNCIL**

T-Mobile

35 GRIFFIN ROAD  
BLOOMFIELD, CT 06002

CROWN CASTLE

3 CORPORATE PARK DRIVE, SUITE 101  
CLIFTON PARK, NY 12065

B+T GRP

1717 S. BOULDER  
SUITE 300  
TULSA, OK 74119  
PH: (918) 587-4630  
www.btgrp.com

T-MOBILE SITE NUMBER:  
**CT11381C**

BU #: **876338**  
**WATERFORD**

41 MANITOCK HILL ROAD  
WATERFORD, CT 06385

EXISTING  
136'-0" SELF-SUPPORT TOWER

**ISSUED FOR:**

REV	DATE	DRWN	DESCRIPTION	DES./QA
0	9/01/21	KT	CONSTRUCTION	KT
1	9/02/21	KT	CONSTRUCTION	KT
1	9/24/21	YXI	CONSTRUCTION	YXI

**SITE INFORMATION**

CROWN CASTLE USA INC. WATERFORD  
SITE NAME:  
SITE ADDRESS: 41 MANITOCK HILL ROAD  
WATERFORD, CT 06385  
COUNTY: NEW LONDON  
MAP/PARCEL #: 038 0154A  
AREA OF CONSTRUCTION: EXISTING  
LATITUDE: 41.354639  
LONGITUDE: -72.150444  
LAT/LONG TYPE: NAD83  
GROUND ELEVATION: 248 FT  
CURRENT ZONING: R-40  
JURISDICTION: CONNECTICUT SITING COUNCIL  
OCCUPANCY CLASSIFICATION: U  
TYPE OF CONSTRUCTION: IIB  
A.D.A. COMPLIANCE: FACILITY IS UNMANNED AND NOT FOR  
HUMAN HABITATION  
PROPERTY OWNER: NEW LONDON CITY OF  
15 MASONIC ST  
NEW LONDON, CT 06320  
TOWER OWNER: CROWN CASTLE  
2000 CORPORATE DRIVE  
CANONSBURG, PA 15317  
CARRIER/APPLICANT: T-MOBILE  
35 GRIFFIN ROAD  
BLOOMFIELD, CT 06002  
ELECTRIC PROVIDER: BOZRAH LIGHT AND POWER CO  
TELCO PROVIDER: AT&T

**DRAWING INDEX**

SHEET #	SHEET DESCRIPTION
T-1	TITLE SHEET
T-2	GENERAL NOTES
C-1.1	OVERALL SITE PLAN
C-1.2	SITE PLAN & ENLARGED SITE PLAN
C-2	FINAL ELEVATION & ANTENNA PLANS
C-3	ANTENNA & CABLE SCHEDULE
C-4	PLUMBING DIAGRAM
C-5	EQUIPMENT SPECS
E-1	AC PANEL SCHEDULES & ONE LINE DIAGRAM
G-1	ANTENNA GROUNDING DIAGRAM
G-2	GROUNDING DETAILS
G-3	GROUNDING DETAILS
ATTACHED	MOUNT ANALYSIS
ATTACHED	MOUNT SPECS

ALL DRAWINGS CONTAINED HEREIN ARE FORMATTED FOR 24X36. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

**PROJECT DESCRIPTION**

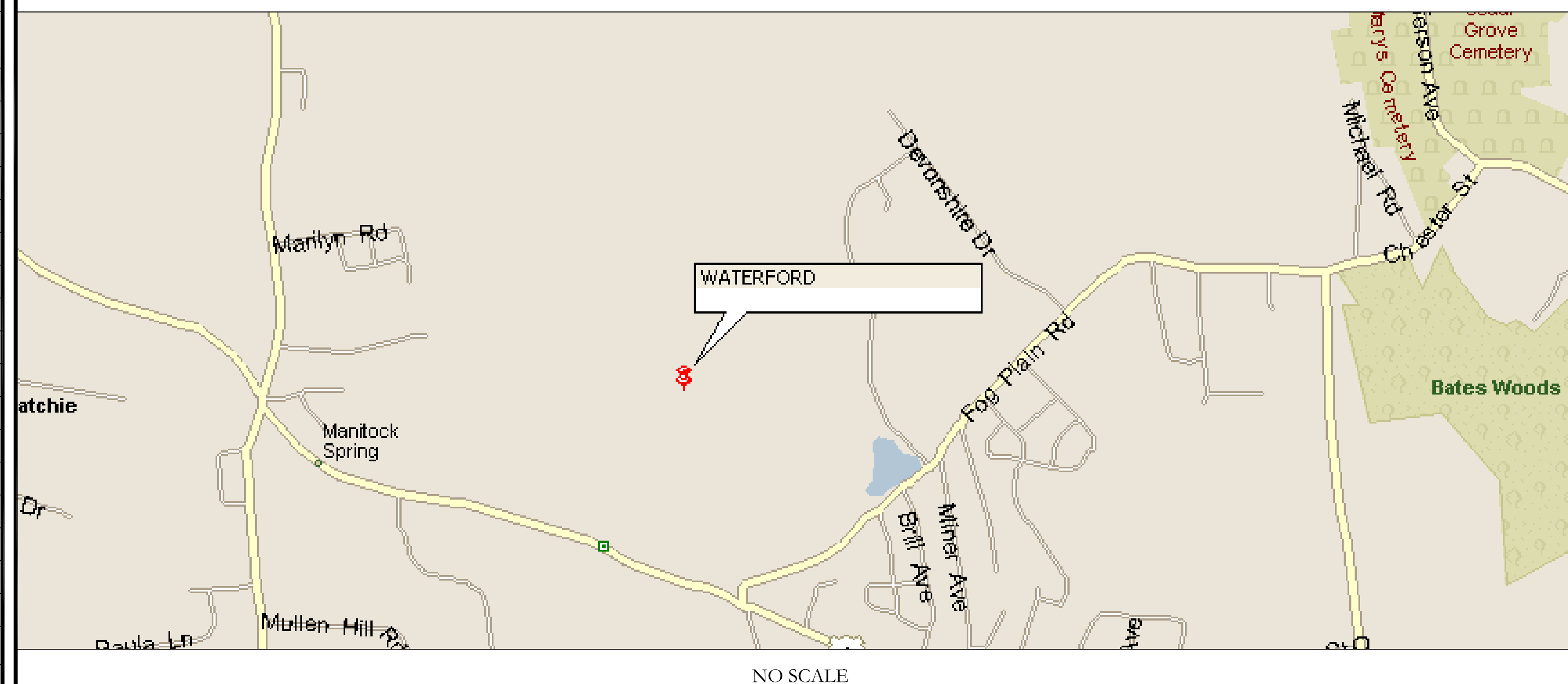
THE PURPOSE OF THIS PROJECT IS TO ENHANCE BROADBAND CONNECTIVITY AND CAPACITY TO THE EXISTING ELIGIBLE WIRELESS FACILITY.

- TOWER SCOPE OF WORK:
- REMOVE (3) MOUNTS
  - REMOVE (6) ANTENNAS
  - REMOVE (3) TMAS
  - REMOVE (12) 1-5/8" COAX CABLES
  - REMOVE (1) 1-1/4" COAX CABLE
  - INSTALL MOUNT MODIFICATION PER MA BY TRYLON DATED 8/24/21
  - INSTALL (6) ANTENNAS
  - INSTALL (6) RRHs
  - INSTALL (3) 6/24 4AWG HYBRID CABLE

- GROUND SCOPE OF WORK:
- REMOVE (1) RBS 6131 CABINET
  - RELOCATE (1) DUW30, (1) DUG20, (1) BB 6630, (1) BB 6648 EQUIPMENT FROM (E) RBS 6131 CABINET
  - INSTALL (1) EMERSON AAV CABINET
  - INSTALL (1) RBS 6160 CABINET
  - INSTALL (1) B160 BATTERY CABINET
  - INSTALL (1) BB 6648
  - INSTALL (1) RBS 6601
  - INSTALL (1) CSR IXRE V2 (GEN2)
  - INSTALL (2) PSU 4813 VOLTAGE BOOSTER

NOTE:  
PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN NOC AT (800) 788-7011 & CROWN CONSTRUCTION MANAGER.

**LOCATION MAP**



**APPLICABLE CODES/REFERENCE DOCUMENTS**

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

CODE TYPE	CODE
BUILDING	2015 IBC
MECHANICAL	2015 IMC
ELECTRICAL	2017 NEC

**REFERENCE DOCUMENTS:**

STRUCTURAL ANALYSIS:	MORRION HERSHFELD
DATED:	AUGUST 31, 2021
MOUNT ANALYSIS:	TRYLON
DATED:	8/24/21
RFDS REVISION:	9
DATED:	7/8/21
ORDER ID:	576542
REVISION:	0

**APPROVALS**

APPROVAL	SIGNATURE	DATE
PROPERTY OWNER OR REP.	_____	_____
LAND USE PLANNER	_____	_____
T-MOBILE	_____	_____
OPERATIONS	_____	_____
RF	_____	_____
NETWORK	_____	_____
BACKHAUL	_____	_____
CONSTRUCTION MANAGER	_____	_____

THE PARTIES ABOVE HEREBY APPROVE AND ACCEPT THESE DOCUMENTS AND AUTHORIZE THE CONTRACTOR TO PROCEED WITH THE CONSTRUCTION DESCRIBED HEREIN. ALL CONSTRUCTION DOCUMENTS ARE SUBJECT TO REVIEW BY THE LOCAL BUILDING DEPARTMENT AND ANY CHANGES AND MODIFICATIONS THEY MAY IMPOSE.



B&T ENGINEERING, INC.  
PEC.0001564  
Expires 2/10/22

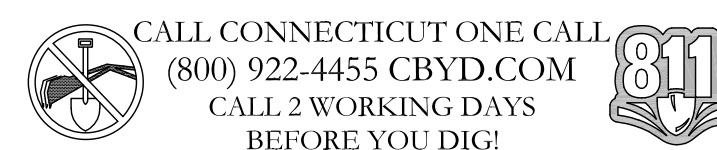
IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER:

**T-1**

REVISION:

**1**







**SITE PLAN DISCLAIMER:**  
 PROPERTY LINES AND STRUCTURES HAVE BEEN DIGITIZED FROM PREVIOUS PLAN SETS. CROWN CASTLE USA INC. HAS NOT COMPLETED A SITE SURVEY AND THEREFORE MAKES NO CLAIMS AS TO THE ACCURACY OF INFORMATION DEPICTED ON THIS SHEET.



**T-Mobile**  
 35 GRIFFIN ROAD  
 BLOOMFIELD, CT 06002

**CROWN CASTLE**  
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 PEC.0001564  
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**SHEET NUMBER:** C-1.1 **REVISION:** 1

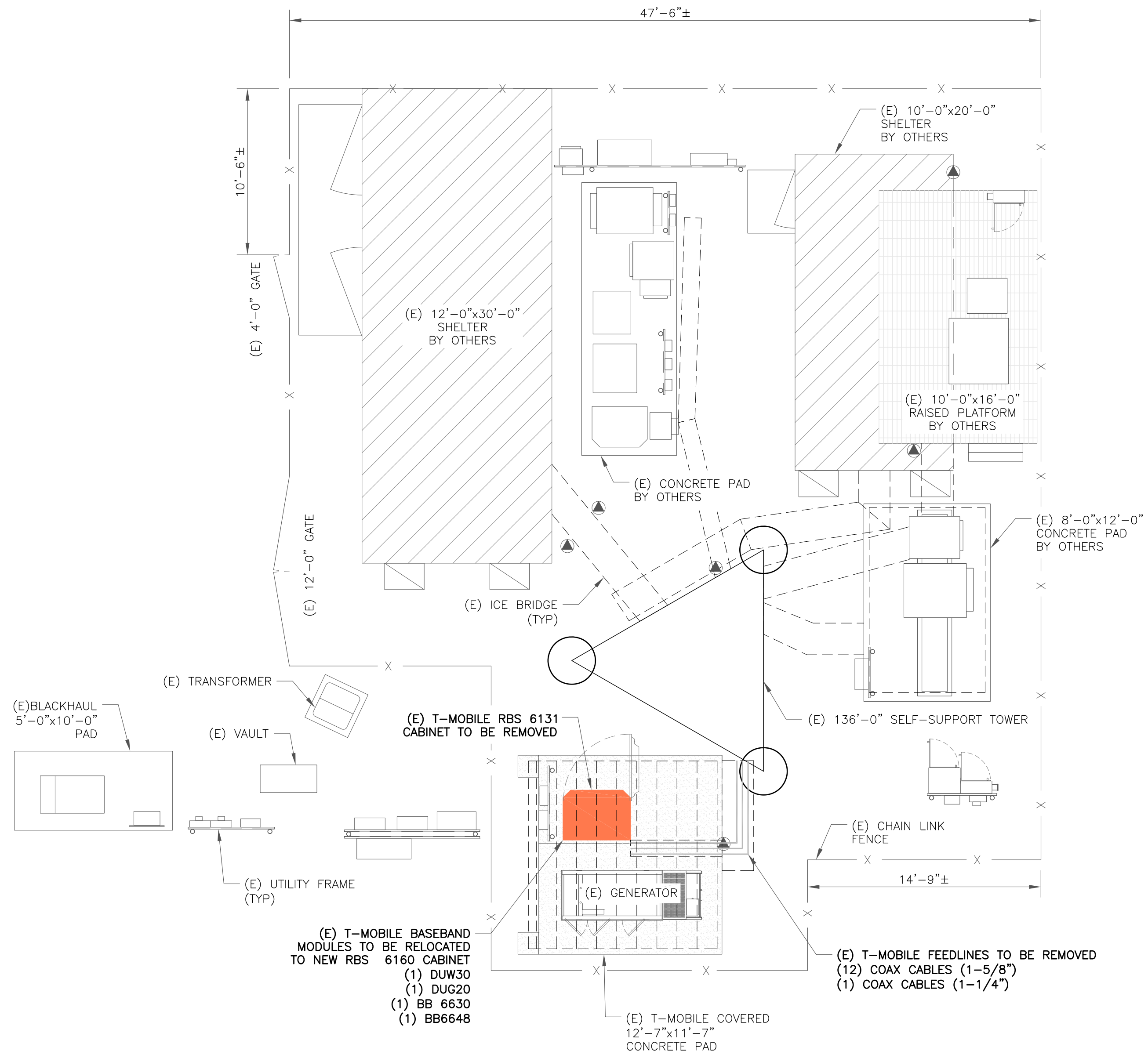
99072.004.01\_WATERFORD.dwg - SheetC-1.1 - User: yxiong - Sep 24, 2021 - 11:13am

**1 OVERALL SITE PLAN**

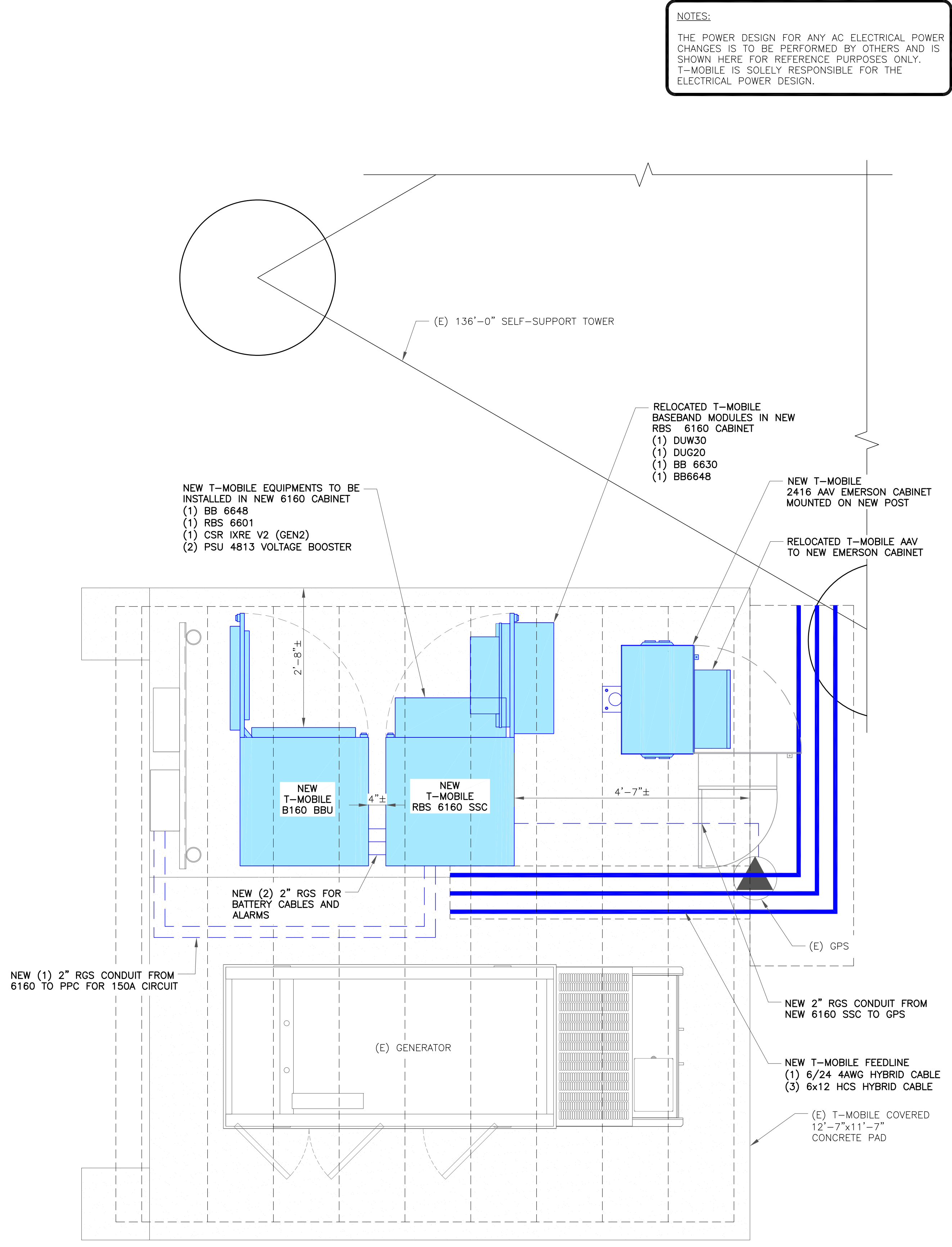
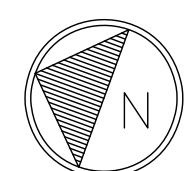
SCALE:

1" = 80'-0" (FULL SIZE)  
 1" = 160'-0" (11x17)

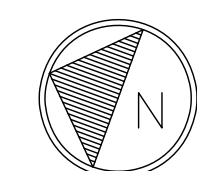
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1 SITE PLAN  
SCALE: 3/16"=1'-0" (FULL SIZE)  
3/32"=1'-0" (11x17)



2 ENLARGED SITE PLAN  
SCALE: 3/4"=1'-0" (FULL SIZE)  
3/8"=1'-0" (11x17)



NOTES:  
THE POWER DESIGN FOR ANY AC ELECTRICAL POWER CHANGES IS TO BE PERFORMED BY OTHERS AND IS SHOWN HERE FOR REFERENCE PURPOSES ONLY. T-MOBILE IS SOLELY RESPONSIBLE FOR THE ELECTRICAL POWER DESIGN.

**T-Mobile**

35 GRIFFIN ROAD  
BLOOMFIELD, CT 06002

**CROWN CASTLE**

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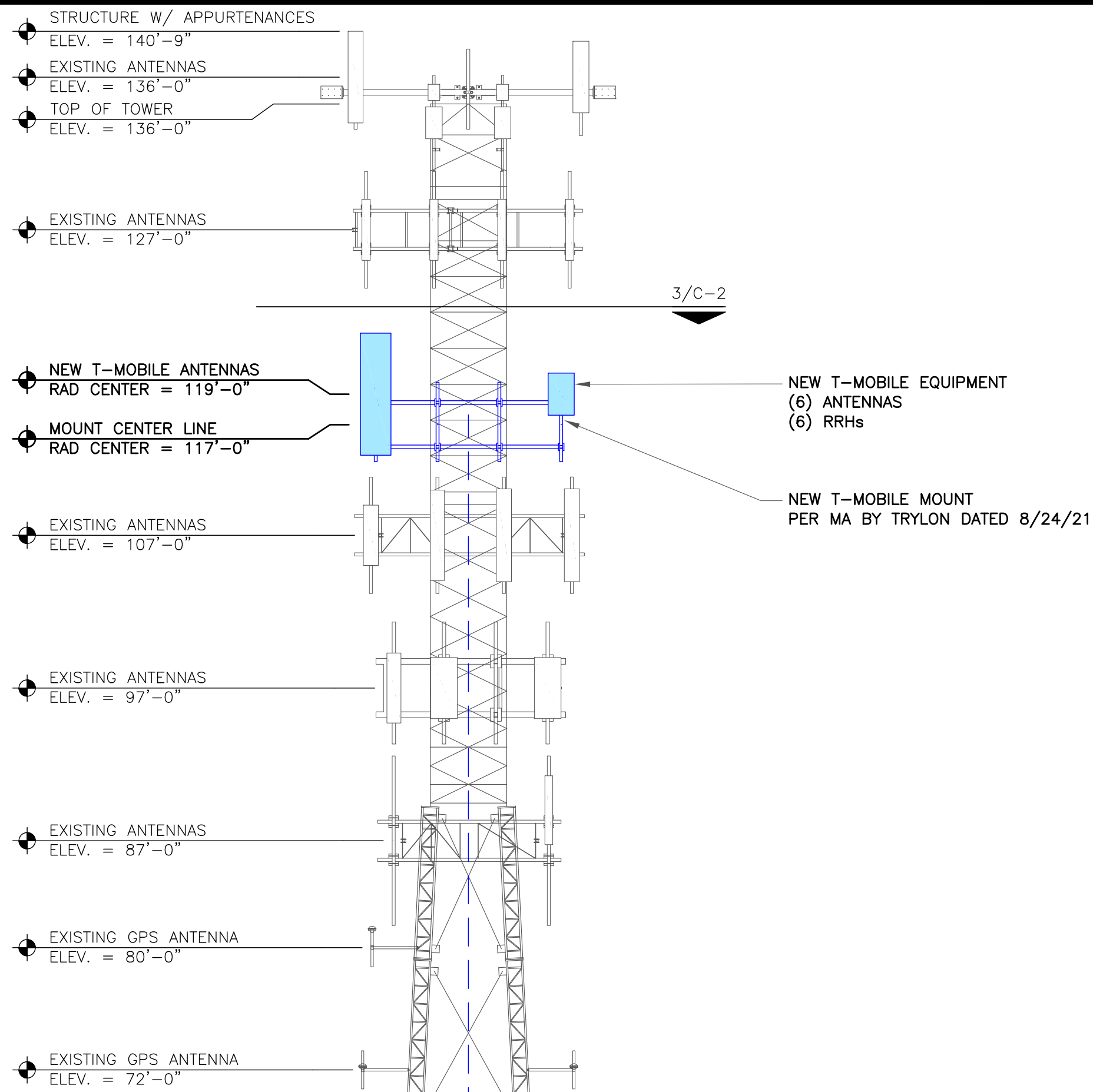
ISSUED FOR:

REV	DATE	DRWN	DESCRIPTION	DES./QA
0	9/01/21	KT	CONSTRUCTION	KT
1	9/02/21	KT	CONSTRUCTION	KT
1	9/24/21	YXI	CONSTRUCTION	YXI

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PEC.0001564  
Expires 2/10/22

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SHEET NUMBER: **C-1.2** REVISION: **1**

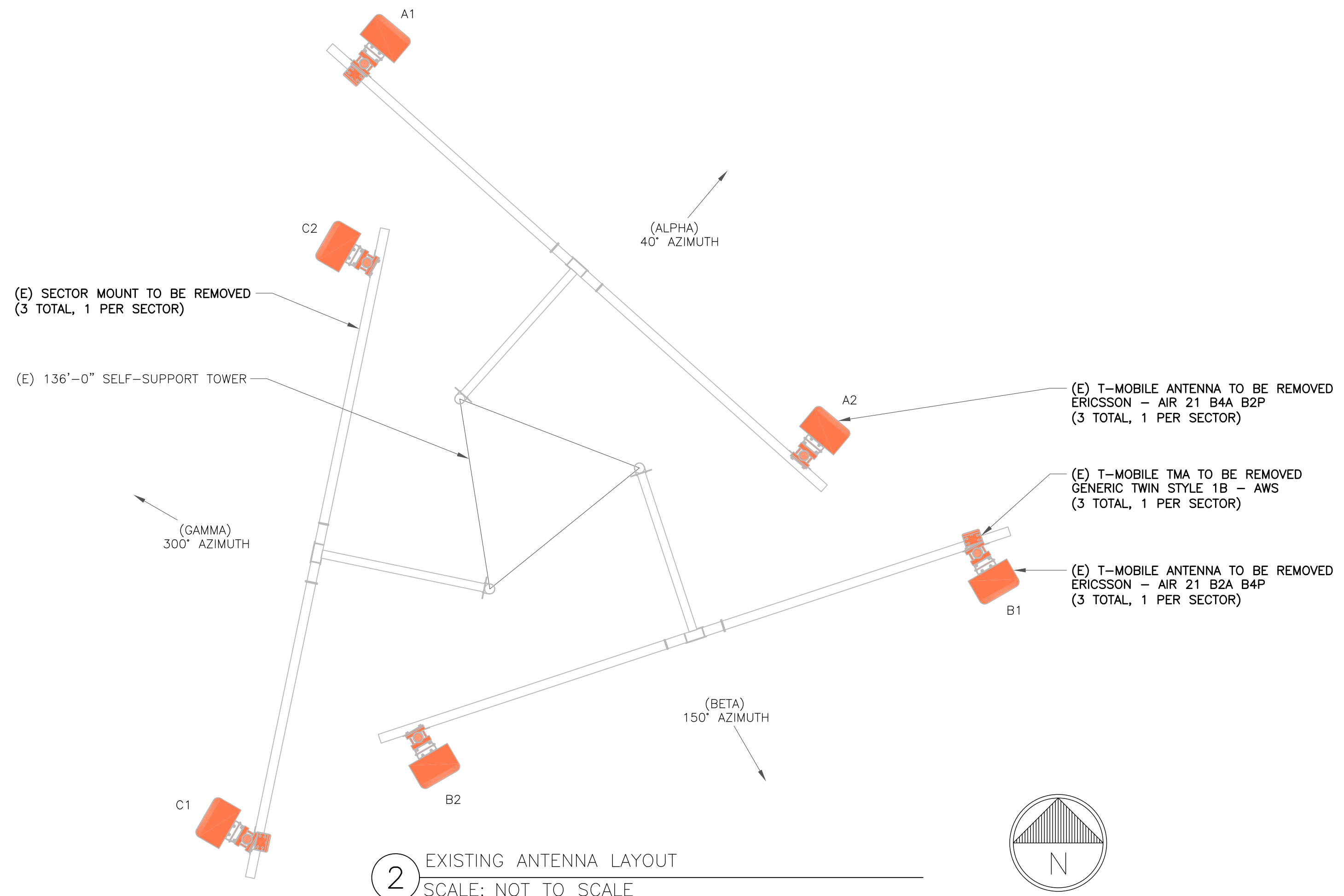


**T-MOBILE EQUIPMENT**

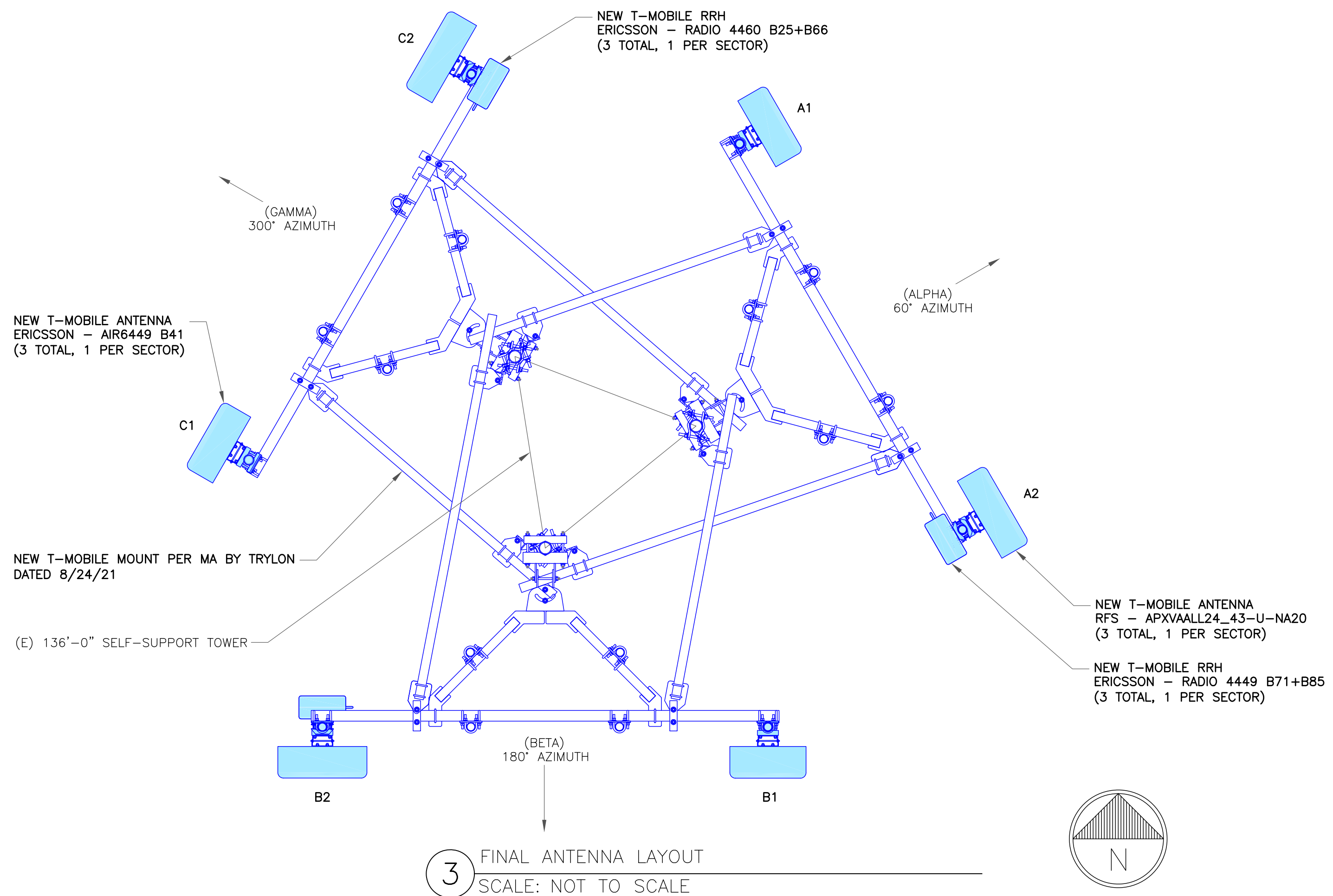
ANTENNA CL: 119'-0"  
MOUNT CL: 117'-0"

ANY AND ALL TOWER MOUNTED EQUIPMENT MUST NOT TRAP OR INTERFERE W/ EXISTING SAFETY CLIMB

1 FINAL ELEVATION  
SCALE: NOT TO SCALE



2 EXISTING ANTENNA LAYOUT  
SCALE: NOT TO SCALE



3 FINAL ANTENNA LAYOUT  
SCALE: NOT TO SCALE

**T-Mobile**

35 GRIFFIN ROAD  
BLOOMFIELD, CT 06002

**CROWN CASTLE**

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T-MOBILE SITE NUMBER:  
**CT11381C**

BU #: **876338**  
**WATERFORD**

41 MANITOCK HILL ROAD  
WATERFORD, CT 06385

EXISTING  
136'-0" SELF-SUPPORT TOWER

**ISSUED FOR:**

REV	DATE	DRWN	DESCRIPTION	DES./QA
0	9/01/21	KT	CONSTRUCTION	KT
1	9/02/21	KT	CONSTRUCTION	KT
1	9/24/21	YXI	CONSTRUCTION	YXI



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PEC.0001564  
Expires 2/10/22

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SHEET NUMBER:

**C-2**

REVISION:

**1**

**T-Mobile**

35 GRIFFIN ROAD  
BLOOMFIELD, CT 06002

**CROWN CASTLE**

3 CORPORATE PARK DRIVE, SUITE 101  
CLIFTON PARK, NY 12065

**B+T GRP**

1717 S. BOULDER  
SUITE 300  
TULSA, OK 74119  
PH: (918) 587-4630  
www.btgrp.com

T-MOBILE SITE NUMBER:  
**CT11381C**

BU #: **876338**  
**WATERFORD**

41 MANITOCK HILL ROAD  
WATERFORD, CT 06385

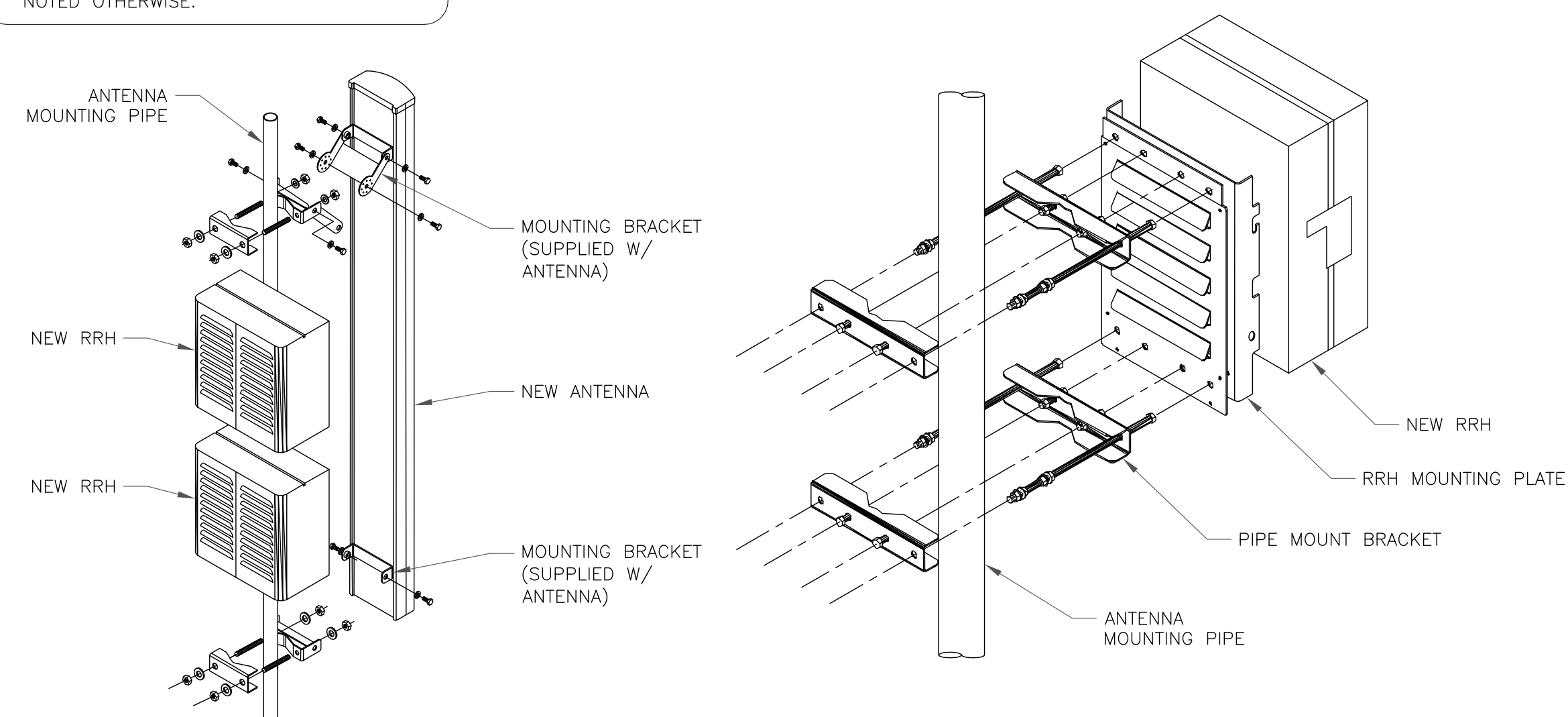
EXISTING  
136'-0" SELF-SUPPORT TOWER

RF SYSTEM SCHEDULE										
SECTOR	ANTENNA	TECH	MANUFACTURER	ANTENNA MODEL	AZIMUTH	M-TILT	E-TILT	RAD CENTER	TMA/RRU	FEEDLINE TYPE
ALPHA	A1	L2500 / N2500	ERICSSON	AIR6449 B41	60°	2'	5'/5'	119'-0"	-	-
	A2	L700 / L600 / N600 / U1900 / L2100 / L1900 / G1900	RFS	APXVAALL24_43-U-NA20	60°	2'	5'/5'/5'/5'	119'-0"	RADIO 4449 B71+B85 RADIO 4460 B25+B66	(1) 6X24 HYBRID
BETA	B1	L2500 / N2500	ERICSSON	AIR6449 B41	180°	2'	5'/5'	119'-0"	-	-
	B2	L700 / L600 / N600 / U1900 / L2100 / L1900 / G1900	RFS	APXVAALL24_43-U-NA20	180°	2'	5'/5'/5'/5'	119'-0"	RADIO 4449 B71+B85 RADIO 4460 B25+B66	(1) 6X24 HYBRID
GAMMA	C1	L2500 / N2500	ERICSSON	AIR6449 B41	300°	2'	5'/5'	119'-0"	-	-
	C2	L700 / L600 / N600 / U1900 / L2100 / L1900 / G1900	RFS	APXVAALL24_43-U-NA20	300°	2'	5'/5'/5'/5'	119'-0"	RADIO 4449 B71+B85 RADIO 4460 B25+B66	(1) 6X24 HYBRID

**1** ANTENNA AND CABLE SCHEDULE  
SCALE: NOT TO SCALE

**INSTALLER NOTES:**

1. COMPLY WITH MANUFACTURERS INSTRUCTIONS TO ENSURE THAT ALL RRHs RECEIVE ELECTRICAL POWER WITHIN 24 HOURS OF BEING REMOVED FROM THE MANUFACTURER'S PACKAGING.
2. DO NOT OPEN RRH PACKAGES IN THE RAIN.
3. ALL PIPES, BRACKETS, AND MISCELLANEOUS HARDWARE TO BE GALVANIZED UNLESS NOTED OTHERWISE.



**2** ANTENNA WITH RRHs MOUNTING DETAIL  
SCALE: NOT TO SCALE

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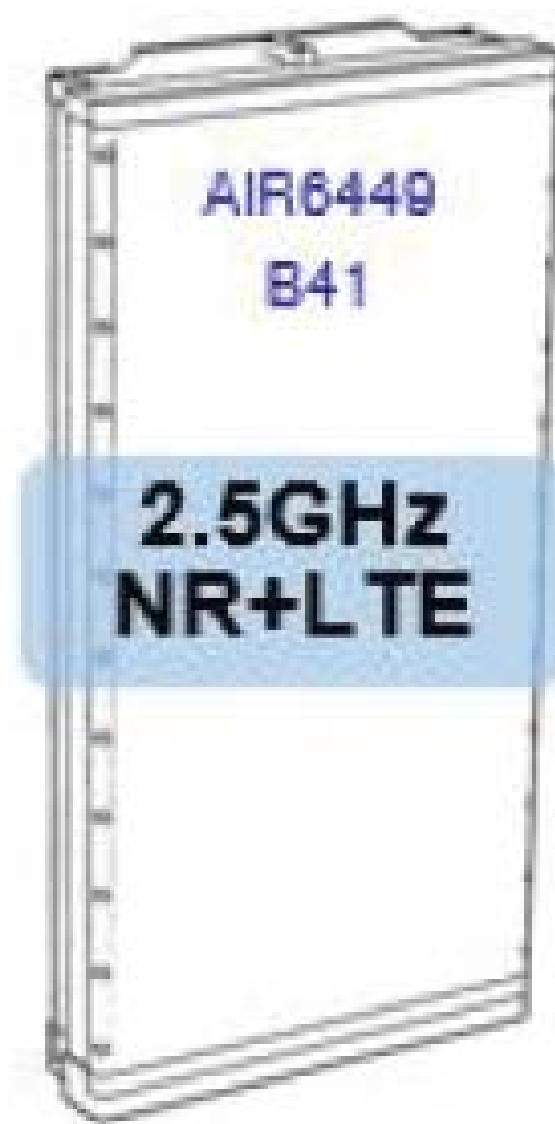
SHEET NUMBER:

**C-3**

REVISION:

**1**

DRAFT



**T-Mobile**  
35 GRIFFIN ROAD  
BLOOMFIELD, CT 06002

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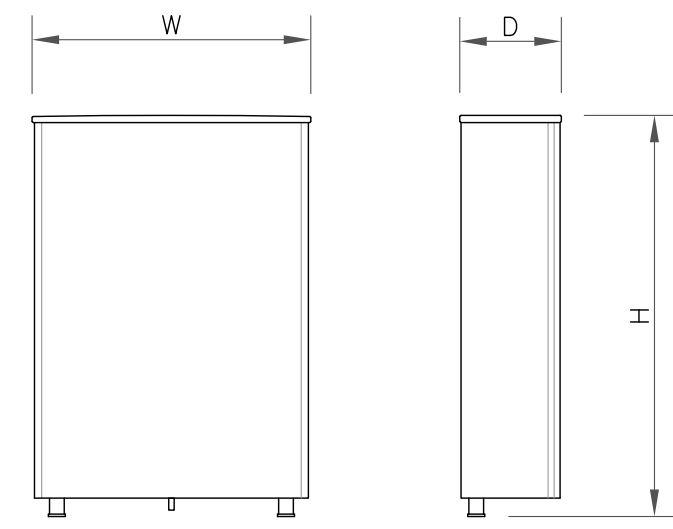


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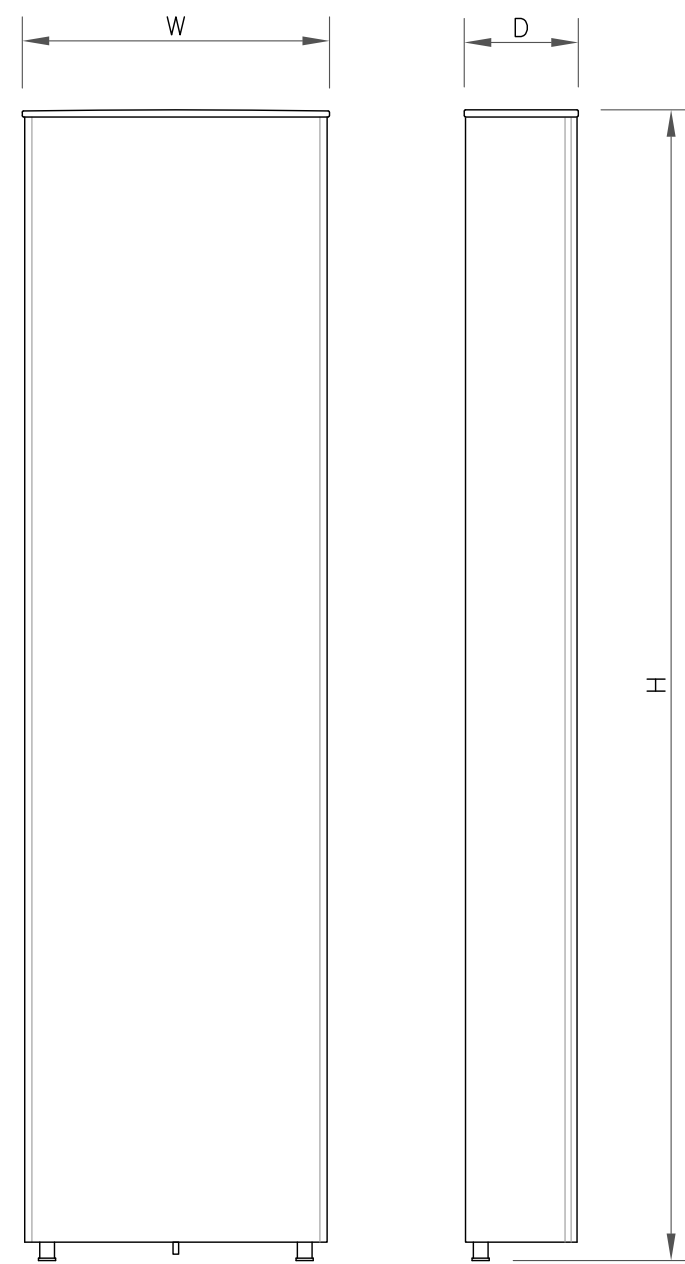
SHEET NUMBER: **C-4** REVISION: **1**

1 PLUMBING DIAGRAM  
SCALE: NOT TO SCALE



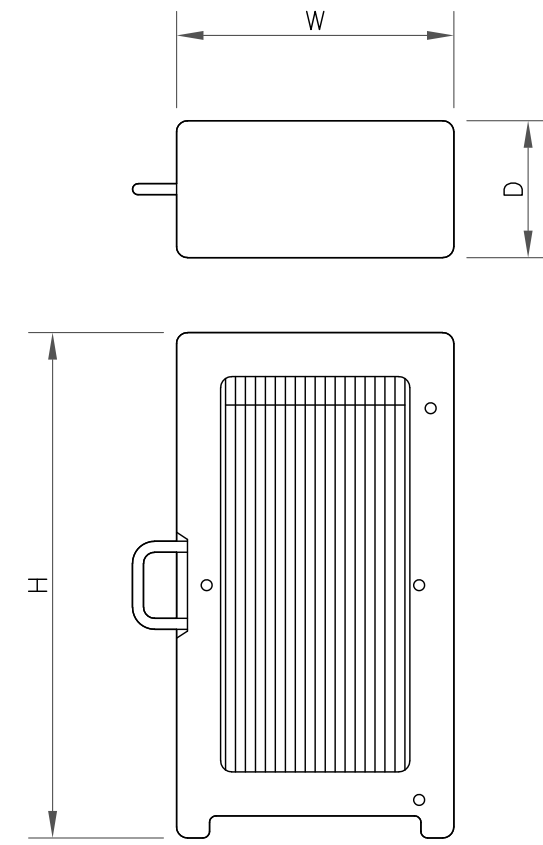
ANTENNA SPECS	
MANUFACTURER	ERICSSON
MODEL #	AIR6449 B41
WIDTH	20.51"
DEPTH	8.54"
HEIGHT	33.11"
WEIGHT	114.63 LBS

1 ANTENNA SPECS  
SCALE: NOT TO SCALE



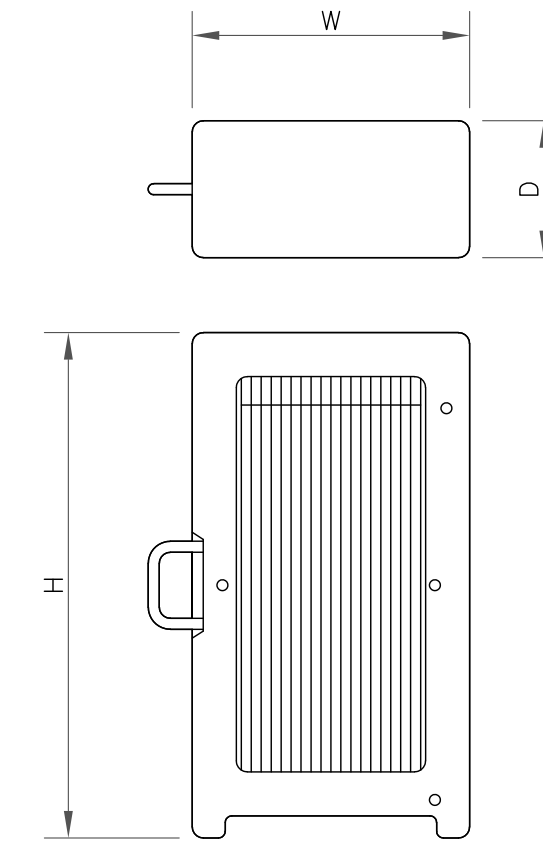
ANTENNA SPECS	
MANUFACTURER	RFS/CELWAVE
MODEL #	APXVAALL24_43-U-NA20
WIDTH	24.00"
DEPTH	8.50"
HEIGHT	95.90"
WEIGHT	149.90 LBS

2 ANTENNA SPECS  
SCALE: NOT TO SCALE



RRU SPECIFICATIONS	
MANUFACTURER	ERICSSON
MODEL #	RADIO 4480 B71+B85
WIDTH	15.70"
DEPTH	7.50"
HEIGHT	21.80"
WEIGHT	92.60 LBS

3 RRU SPECS  
SCALE: NOT TO SCALE



RRU SPECIFICATIONS	
MANUFACTURER	ERICSSON
MODEL #	RADIO 4460 B25+B66
WIDTH	15.10"
DEPTH	11.90"
HEIGHT	17"
WEIGHT	109 LBS

4 RRU SPECS  
SCALE: NOT TO SCALE

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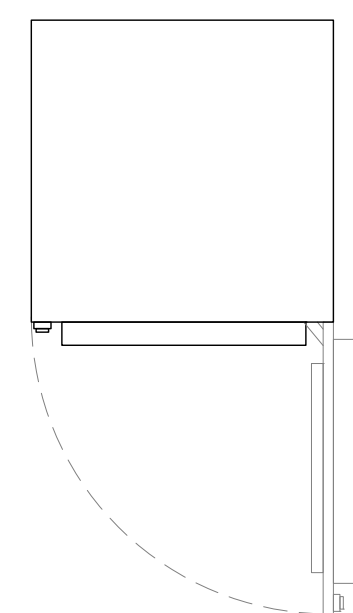
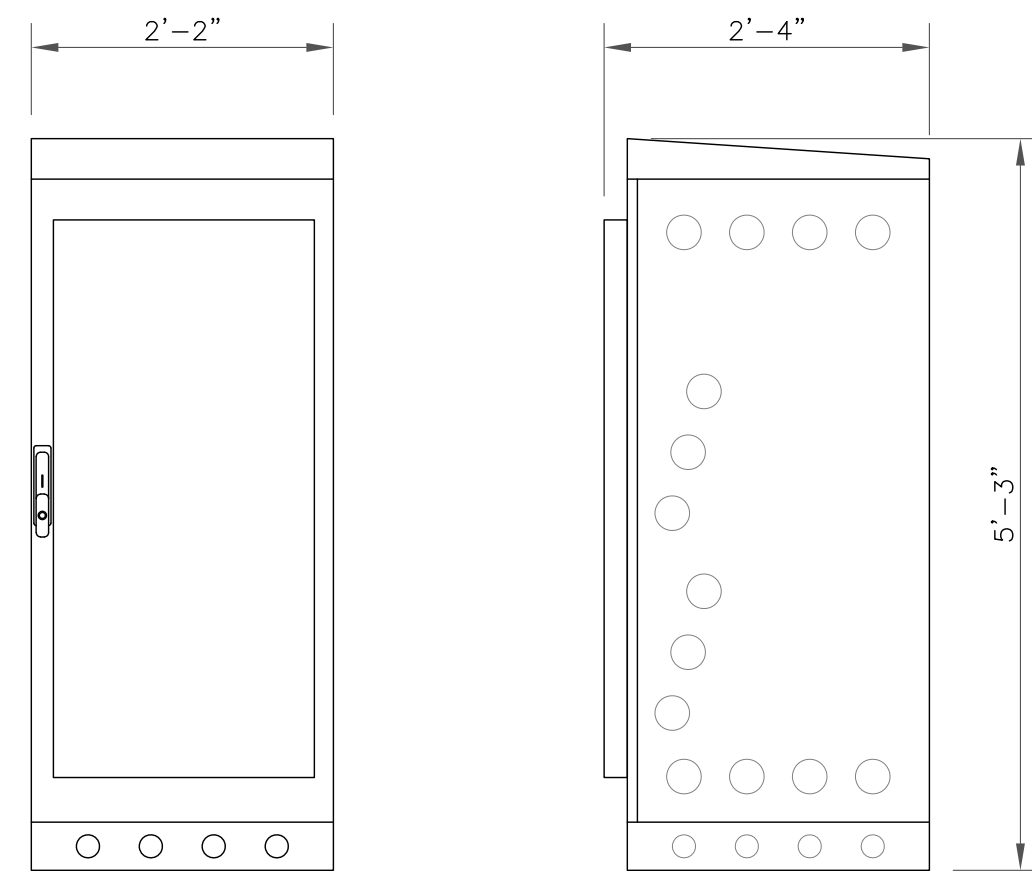
BU #: **876338**  
**WATERFORD**

41 MANITOCK HILL ROAD  
WATERFORD, CT 06385

EXISTING  
136'-0" SELF-SUPPORT TOWER

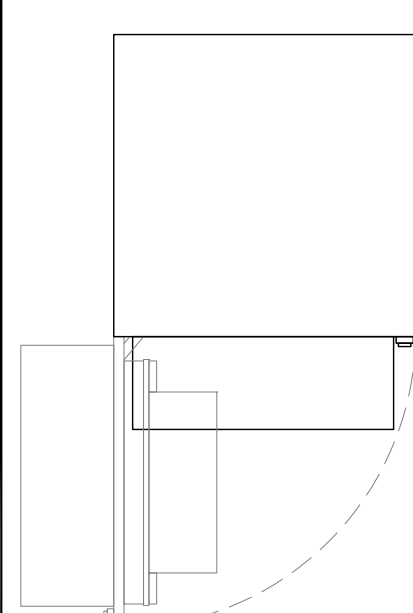
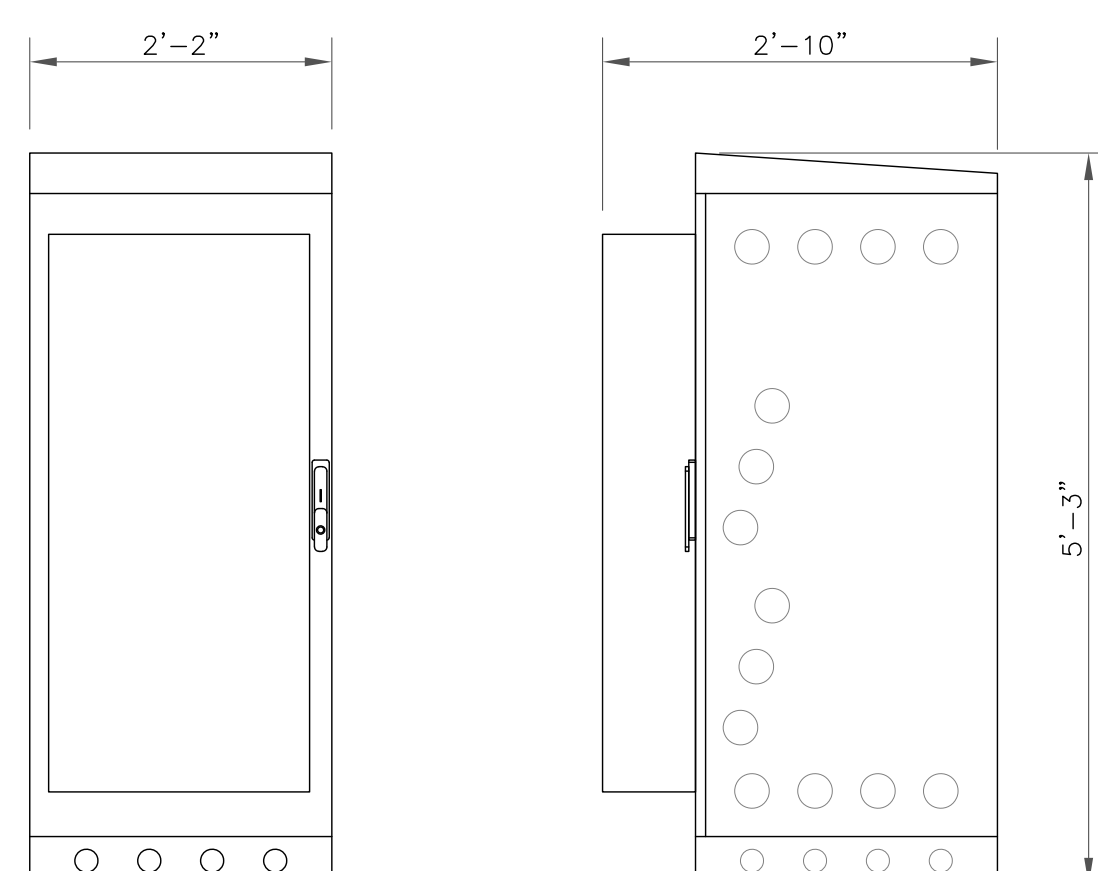
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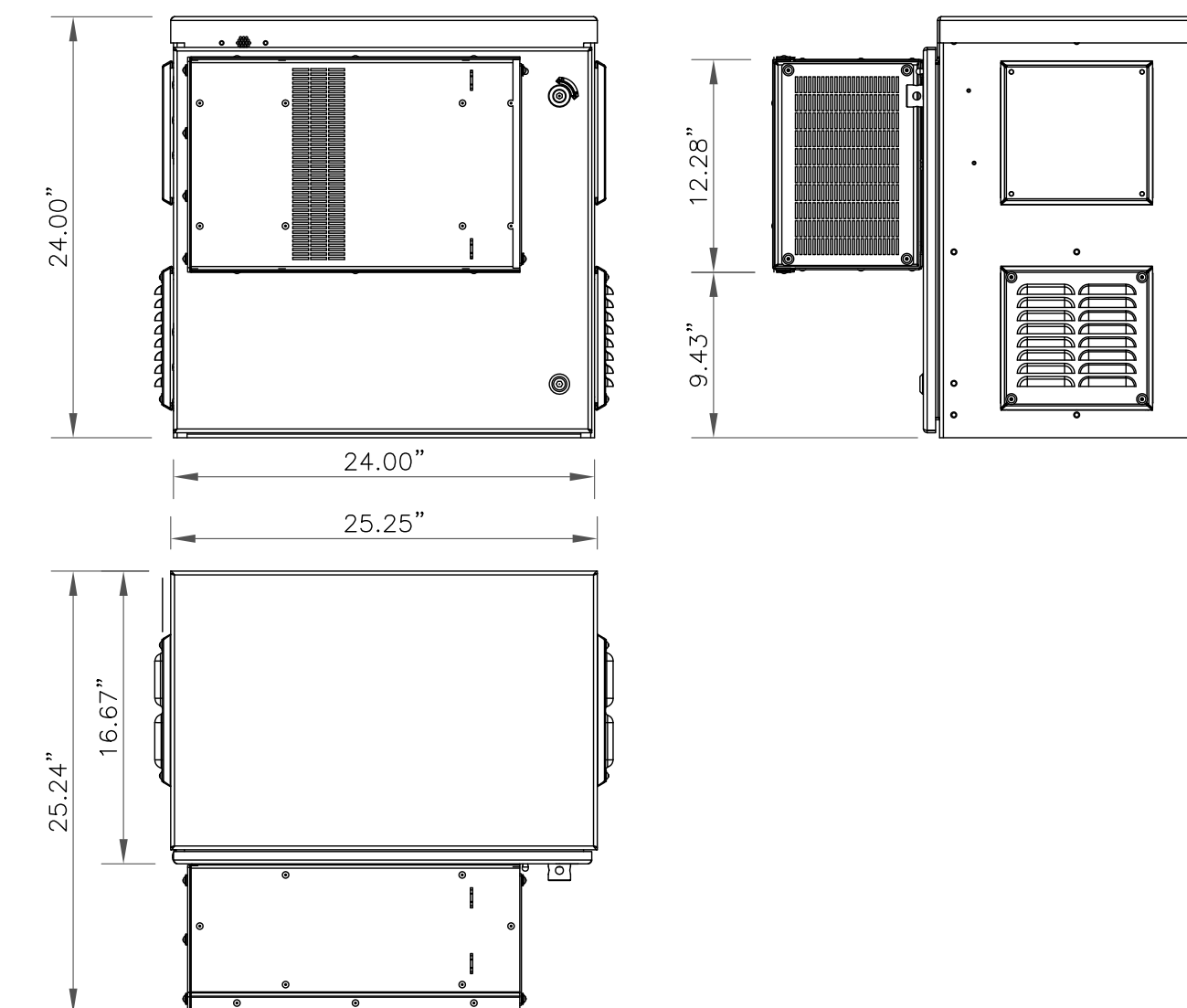
**EQUIPMENT NOTES:**  
HEIGHTxWIDTHxDEPTH: 63.0" x 26.0" x 28.0"  
(1600.0mm x 660.0mm x 711.0mm)  
WEIGHT (EMPTY): 295 LBS (134 kg)  
WEIGHT (FULLY LOADED): 2000 LBS (908 kg)

5 ERICSSON - B160  
SCALE: NOT TO SCALE



**EQUIPMENT NOTES:**  
HEIGHTxWIDTHxDEPTH: 63.0" x 26.0" x 34.0"  
(1600.0mm x 660.0mm x 864.0mm)  
WEIGHT (EMPTY): 320 LBS (145 kg)  
WEIGHT (FULLY LOADED): 1000 LBS (454 kg)

6 ERICSSON - 6160  
SCALE: NOT TO SCALE



EMERSON 2416 AAV CABINET (OUTDOOR)  
WALL OR H-FRAME MOUNTED, POLE MOUNT  
(WALL-MOUNT KIT INCLUDED)  
WEIGHT: 64.0 LBS WITHOUT BATTERIES  
ENCLOSURE: 24"x24"x16"  
BATTERY TRAY: 22"Wx13"D

8 EMERSON 2416 AAV CABINET  
SCALE: NOT TO SCALE



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**C-5**

REVISION:

**1**

**T-Mobile**

35 GRIFFIN ROAD  
BLOOMFIELD, CT 06002

**CROWN CASTLE**

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T-MOBILE SITE NUMBER:  
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BU #: **876338**  
**WATERFORD**

41 MANITOCK HILL ROAD  
WATERFORD, CT 06385

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**E-1**

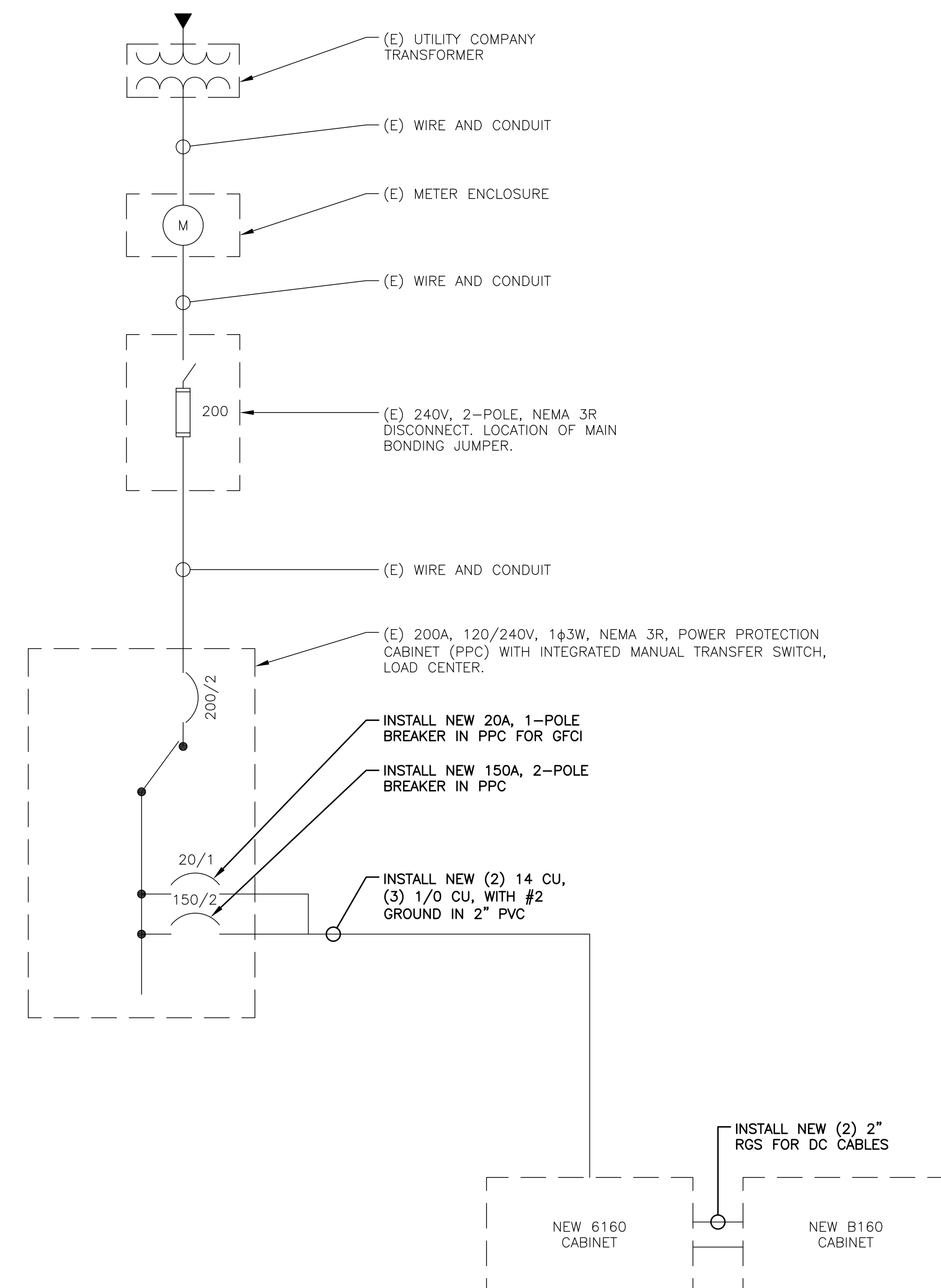
REVISION:

**1**

FINAL PANEL SCHEDULE							
LOAD	POLES	AMPS	BUS		AMPS	POLES	LOAD
			L1	L2			
-	1	-	1	2	60A	2	AC SURGE PROTECTOR
GFCI	1	20A	3	4			
6160 CABINET	2	150A	5	6		2	
PLG LT			7	8			
FAN			9	10		1	TELCO GFI
			11	12			

RATED VOLTAGE:  120/240  1 PHASE, 3 WIRE  
 RATED AMPS:  100  200  400  
 MAIN LUGS ONLY  MAIN 200 AMPS  BREAKER  FUSED SWITCH  HINGED DOOR  
 FUSED  CIRCUIT BREAKER  BRANCH DEVICES  TO BE GFCI BREAKERS  FULL NEUTRAL BUS  GROUND BAR  
 ALL BREAKERS MUST BE RATED TO INTERRUPT A SHORT CIRCUIT ISC OF 10,000 AMPS SYMMETRICAL

REPLACE EXISTING BREAKER IN POSITION 5 AND 7 WITH A NEW 2P 150A BREAKER  
 REPLACE EXISTING BREAKER IN POSITION 3 WITH A NEW 1P 20A BREAKER  
 REPLACE EXISTING WIRES FOR EXISTING RBS 6131 CABINET WITH (3) 1/0 AWG THWN (COPPER) AND (1) #2G AWG. MINIMUM CONDUIT SIZE TO BE 2".  
 IF 150A BREAKER WILL NOT PROPERLY FIT IN EXISTING PANEL, REPLACE (E) PANEL WITH SQUARE D PANEL Q012040M200RB (OR APPROVED EQUAL).  
 UPGRADE FEEDER WIRES TO MEET AMPACITY IF NEW PANEL IS REQUIRED.  
 FINAL PANEL DESIGN AND CALCULATIONS FOR WIRE SIZE WERE BASED OFF OF EXISTING DOCUMENTS AND PHOTOS



**NOTES:**

- ALL NEW CONDUCTORS TO BE INSTALLED SHALL BE COPPER. ALL CONDUCTORS SHALL BE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 UNLESS NOTED OTHERWISE.
- CONTRACTOR IS TO FIELD VERIFY ALL EXISTING ITEMS SHOWN ON THE ELECTRICAL ONE-LINE DIAGRAM AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES.
- ALL GROUNDING AND BONDING PER THE NEC.

1 AC PANEL SCHEDULE  
SCALE: NOT TO SCALE

2 ONE LINE DIAGRAM  
SCALE: NOT TO SCALE

**T-Mobile**

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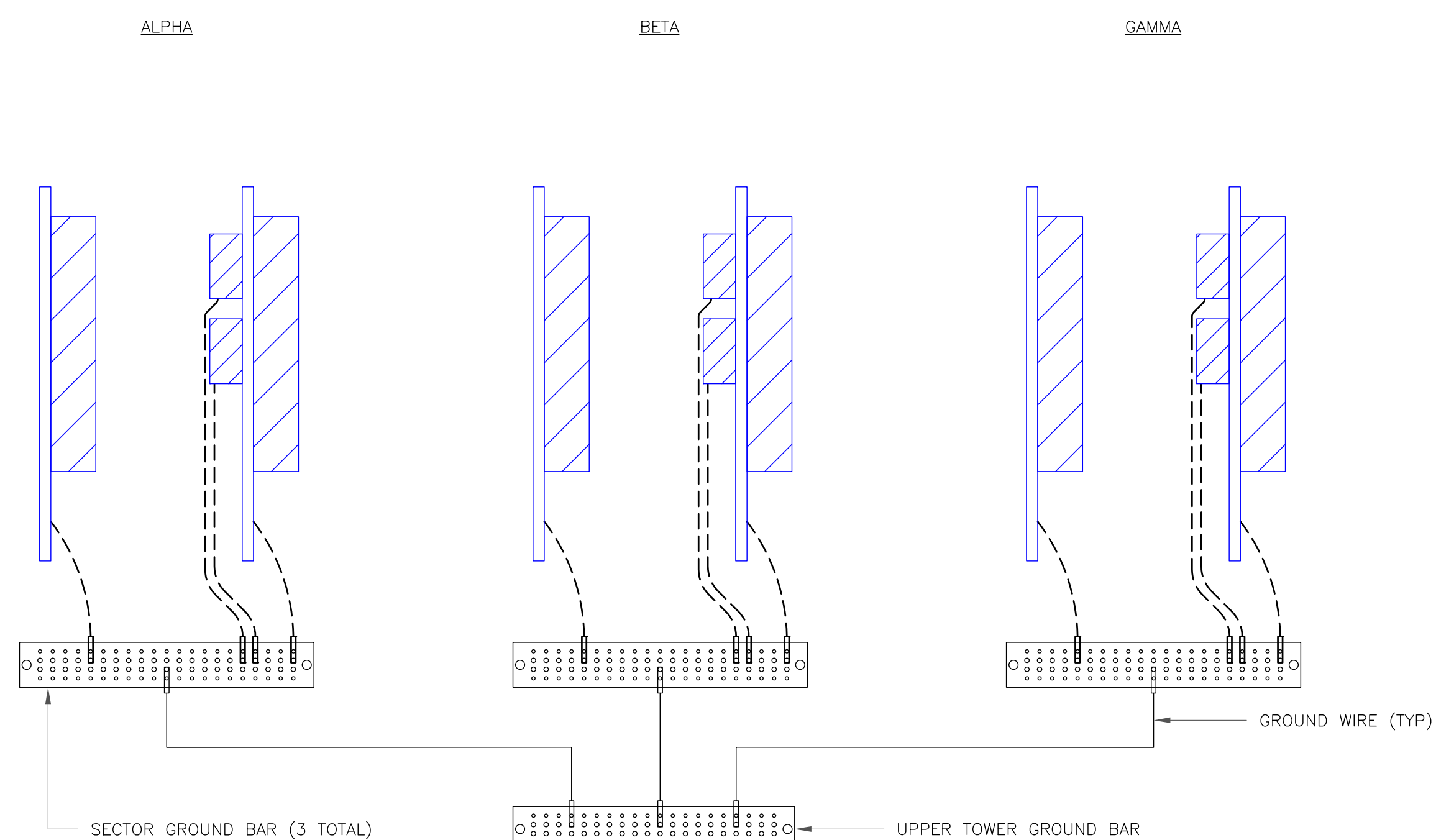
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**G-1**

REVISION:

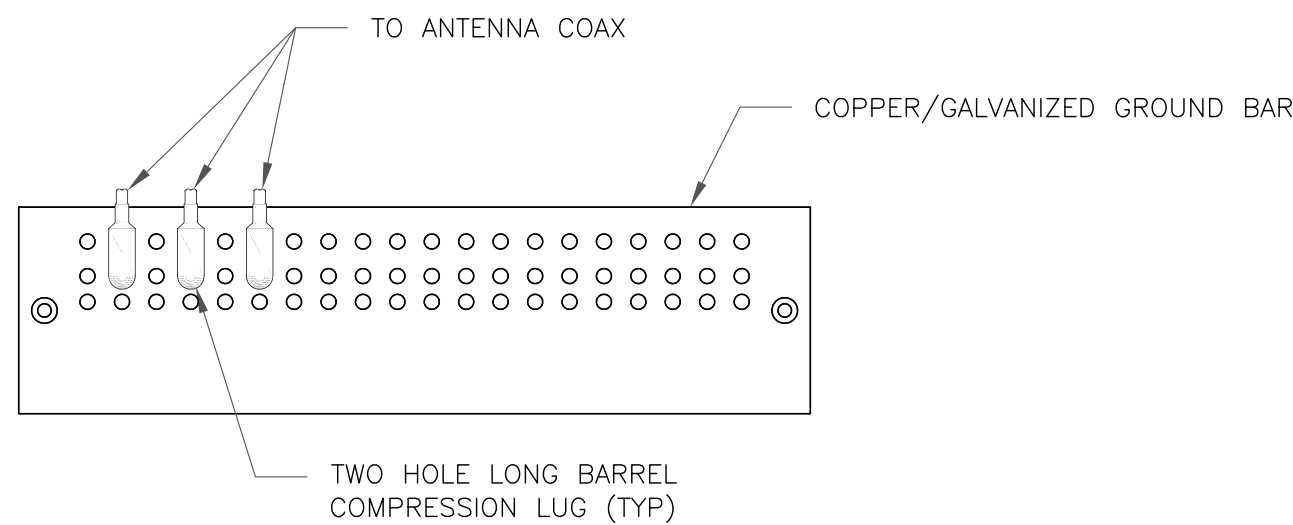
**1**



**NOTE:**  
ALL NEW GROUNDS TO BE #6 STRANDED  
COPPER WITH GREEN INSULATION UNLESS  
NOTED OTHERWISE.

**1** ANTENNA GROUNDING DIAGRAM  
SCALE: NOT TO SCALE

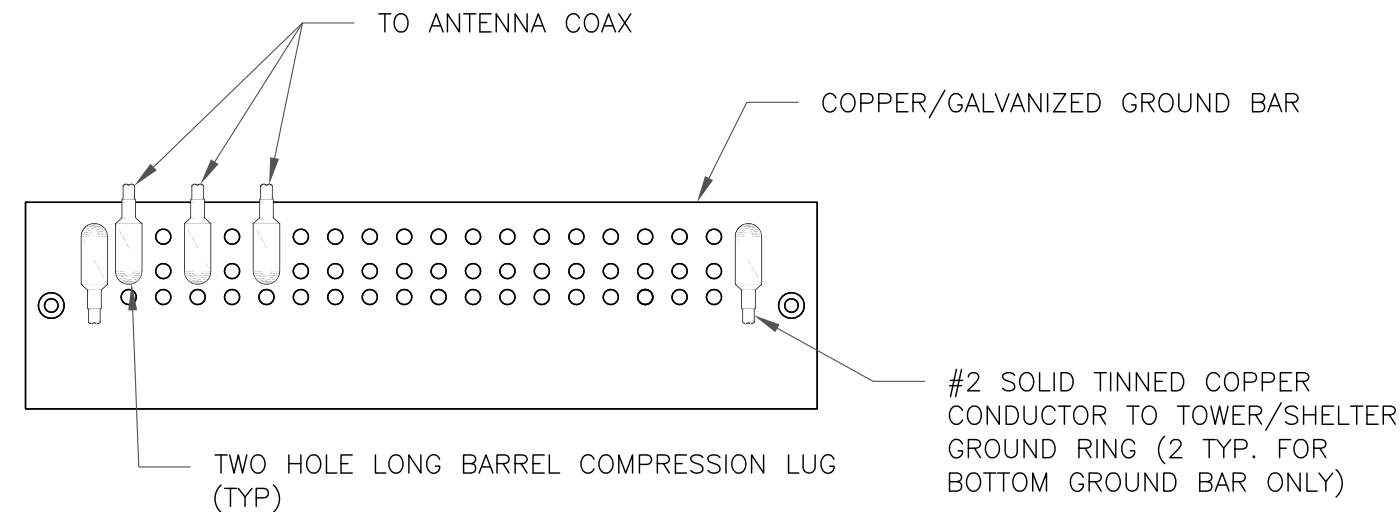




NOTES:

1. DOUBLING UP "OR STACKING" OF CONNECTIONS IS NOT PERMITTED.
2. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
3. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO ANTENNA MOUNT STEEL.

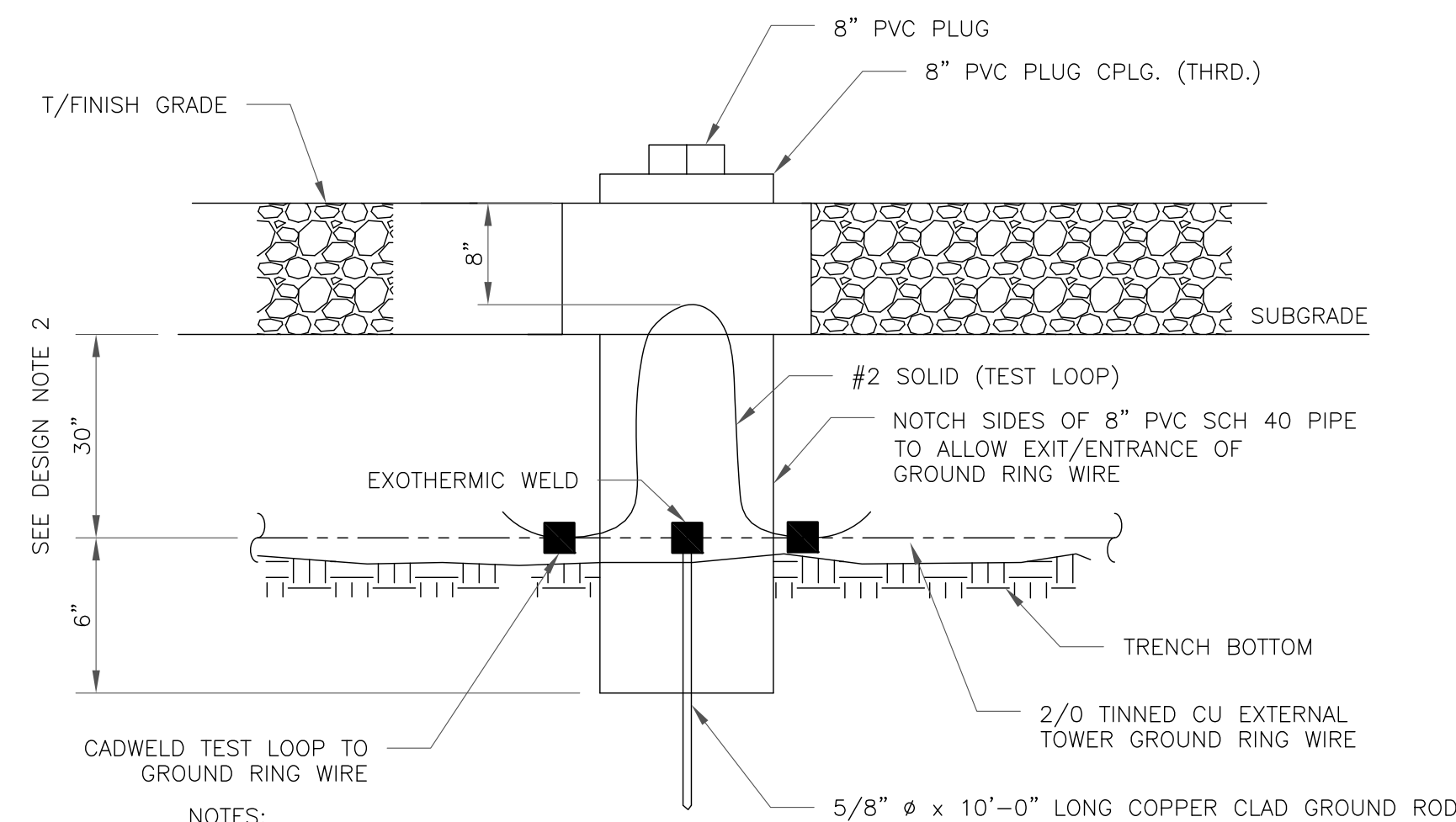
1 ANTENNA SECTOR GROUND BAR DETAIL  
SCALE: NOT TO SCALE



NOTES:

1. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
2. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO TOWER STEEL (TOWER ONLY).
3. GROUND BAR SHALL BE ISOLATED FROM BUILDING OR SHELTER.

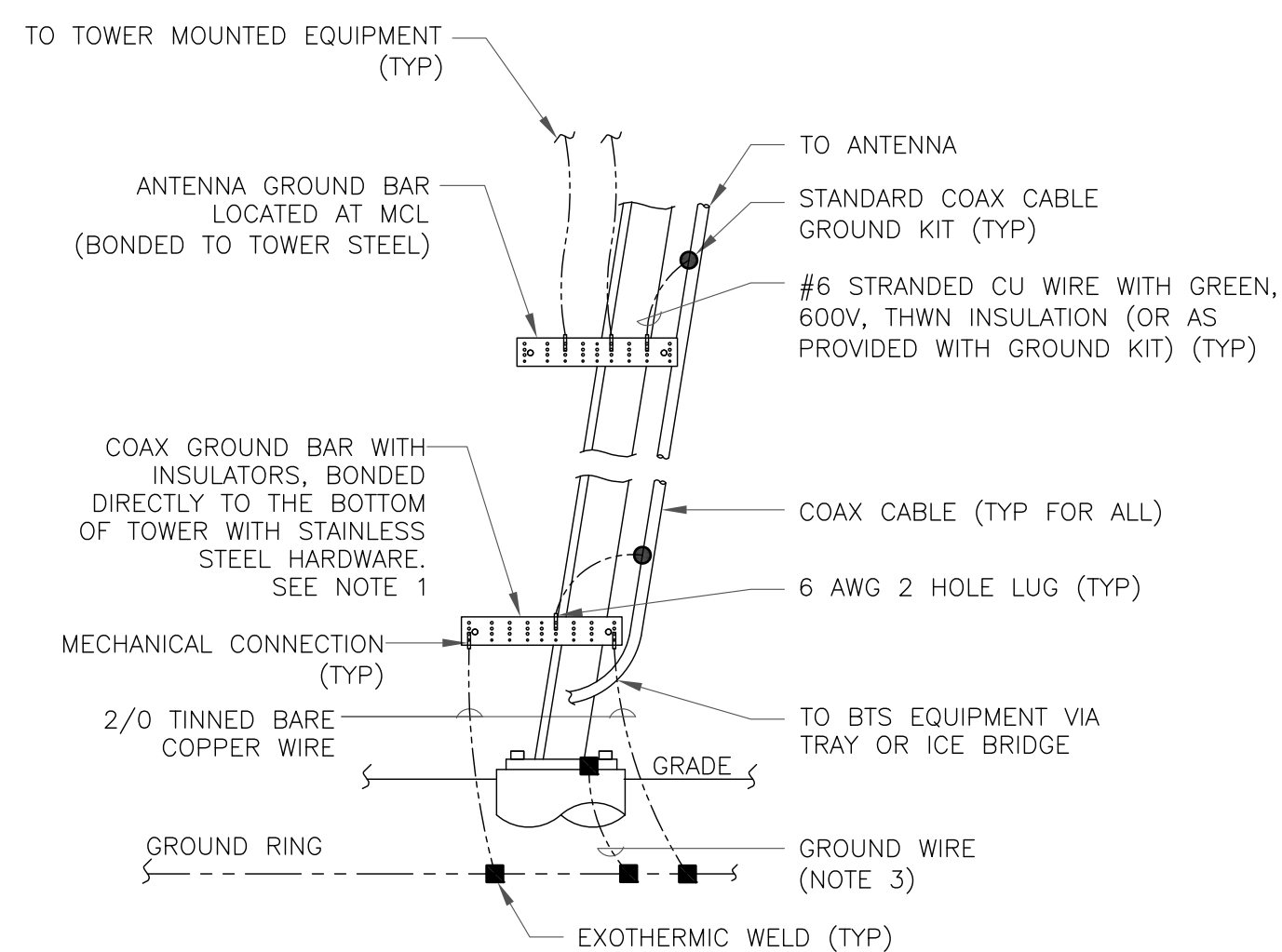
2 TOWER/SHELTER GROUND BAR DETAIL  
SCALE: NOT TO SCALE



NOTES:

1. GROUND ROD SHALL BE DRIVEN VERTICALLY, NOT TO EXCEED 45 DEGREES FROM THE VERTICAL.
2. GROUND WIRE SHALL BE MIN. 30" BELOW GRADE OR 6" BELOW FROST LINE. (WHICH EVER IS GREATER) AS PER N.E.C. ARTICLE 250-50(D).

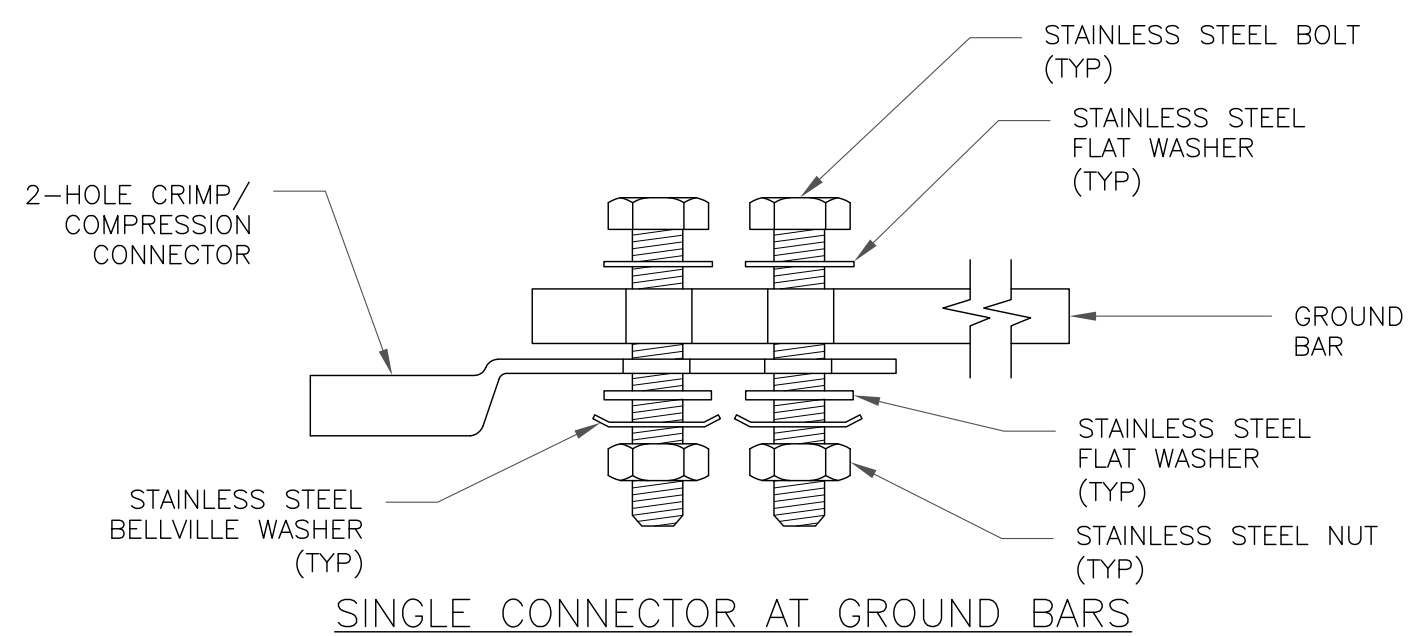
3 INSPECTION WELL DETAIL  
SCALE: NOT TO SCALE



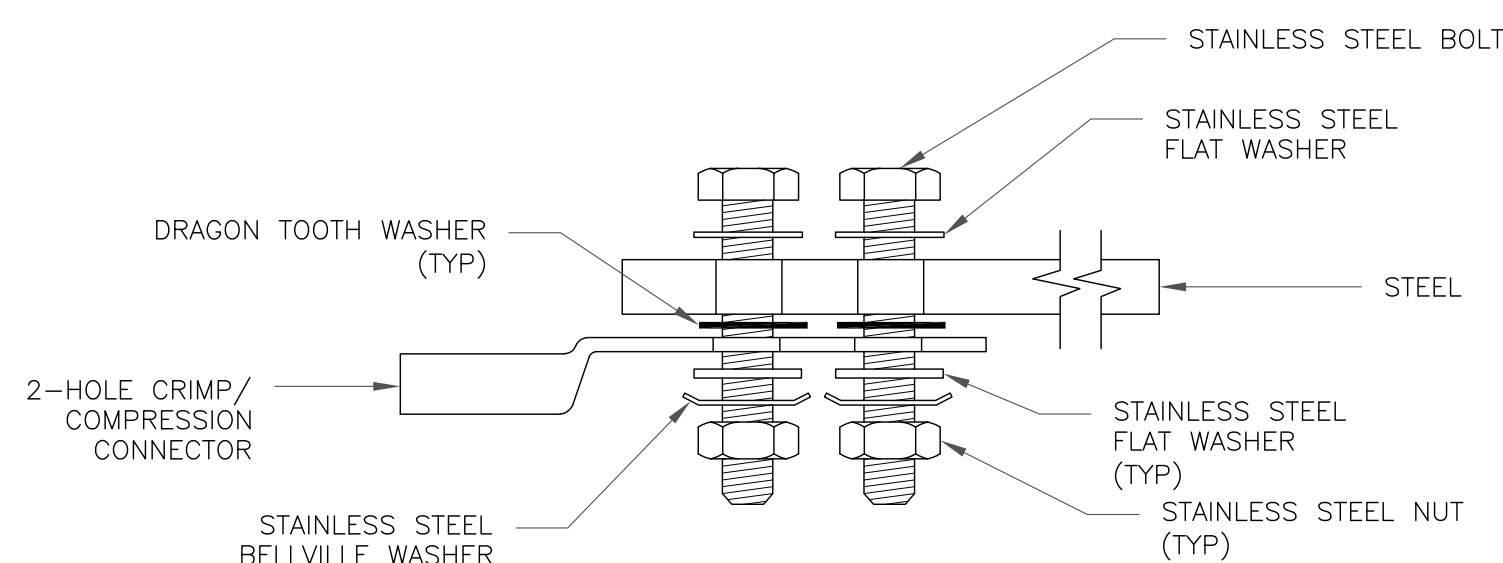
NOTES:

1. NUMBER OF GROUNDING BARS MAY VARY DEPENDING ON THE TYPE OF TOWER, ANTENNA LOCATIONS AND CONNECTION ORIENTATION. COAXIAL CABLES EXCEEDING 200 FEET ON THE TOWER SHALL HAVE GROUND KITS AT THE MIDPOINT. PROVIDE AS REQUIRED.
2. ONLY MECHANICAL CONNECTIONS ARE ALLOWED TO BE MADE TO CROWN CASTLE USA INC. TOWERS. ALL MECHANICAL CONNECTIONS SHALL BE TREATED WITH AN ANTI-OXIDANT COATING.
3. ALL TOWER GROUNDING SYSTEMS SHALL COMPLY WITH THE REQUIREMENTS OF THE RECOGNIZED EDITION OF ANSI/TIA 222 AND NFPA 780.

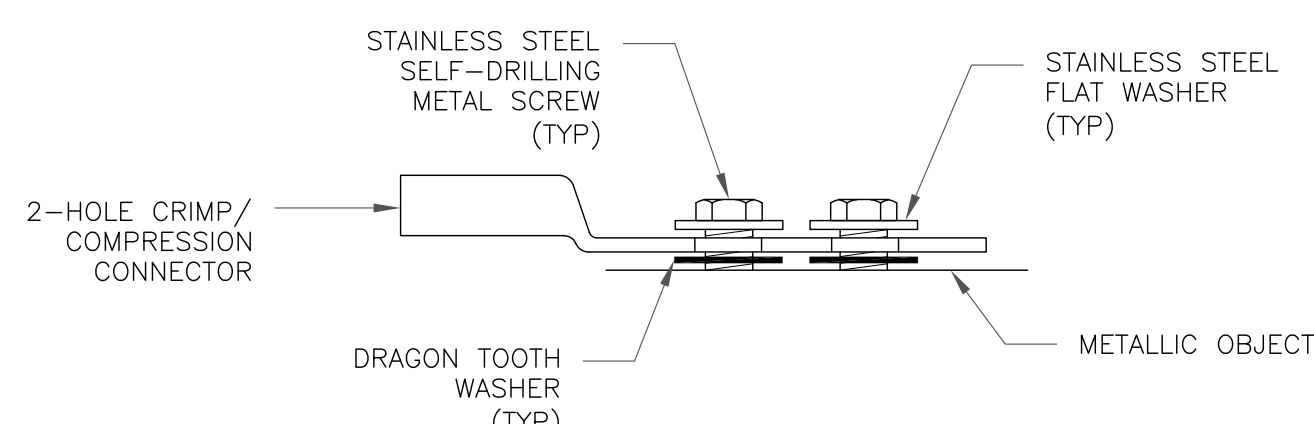
4 TYPICAL ANTENNA CABLE GROUNDING  
SCALE: NOT TO SCALE



SINGLE CONNECTOR AT GROUND BARS

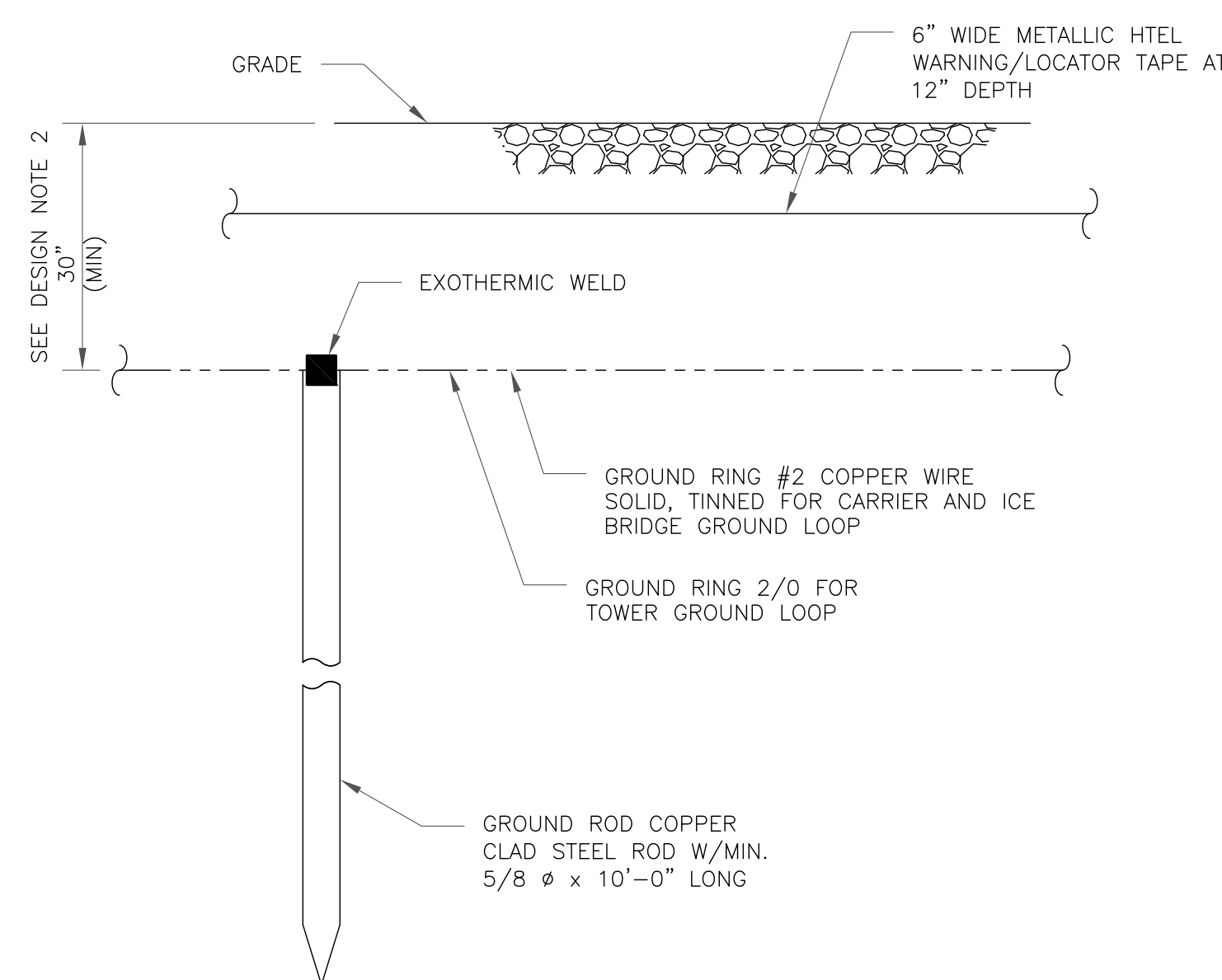


SINGLE CONNECTOR AT STEEL OBJECTS



SINGLE CONNECTOR AT METALLIC/STEEL OBJECTS

5 HARDWARE DETAIL FOR EXTERIOR CONNECTIONS  
SCALE: NOT TO SCALE



NOTES:

1. GROUND ROD SHALL BE DRIVEN VERTICALLY, NOT TO EXCEED 45 DEGREES FROM THE VERTICAL.
2. GROUND WIRE SHALL BE MIN. 30" BELOW GRADE OR 6" BELOW FROST LINE. (WHICH EVER IS GREATER) AS PER N.E.C. ARTICLE 250-50(D).

6 GROUND ROD DETAIL  
SCALE: NOT TO SCALE

T-Mobile

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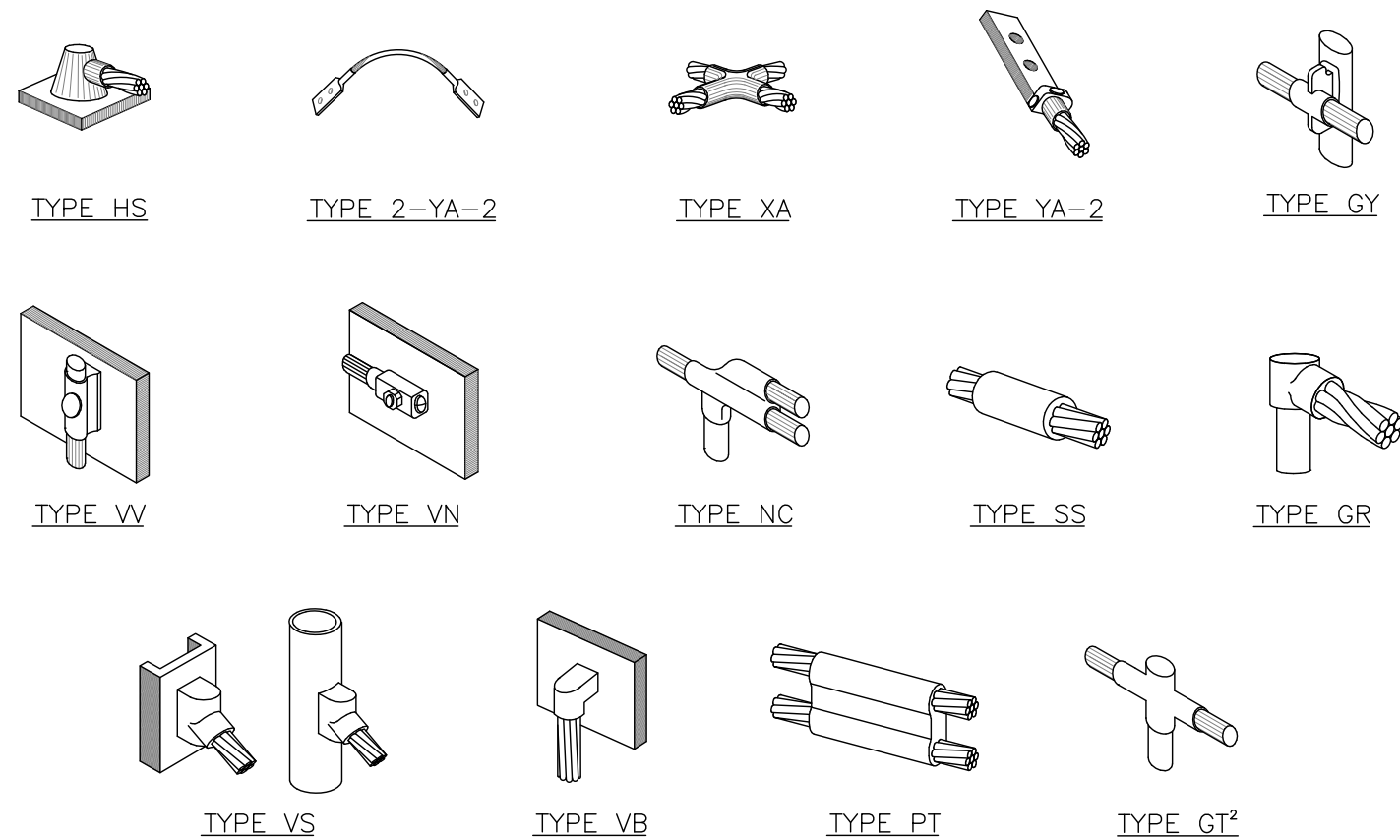
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SHEET NUMBER:

G-2

REVISION:

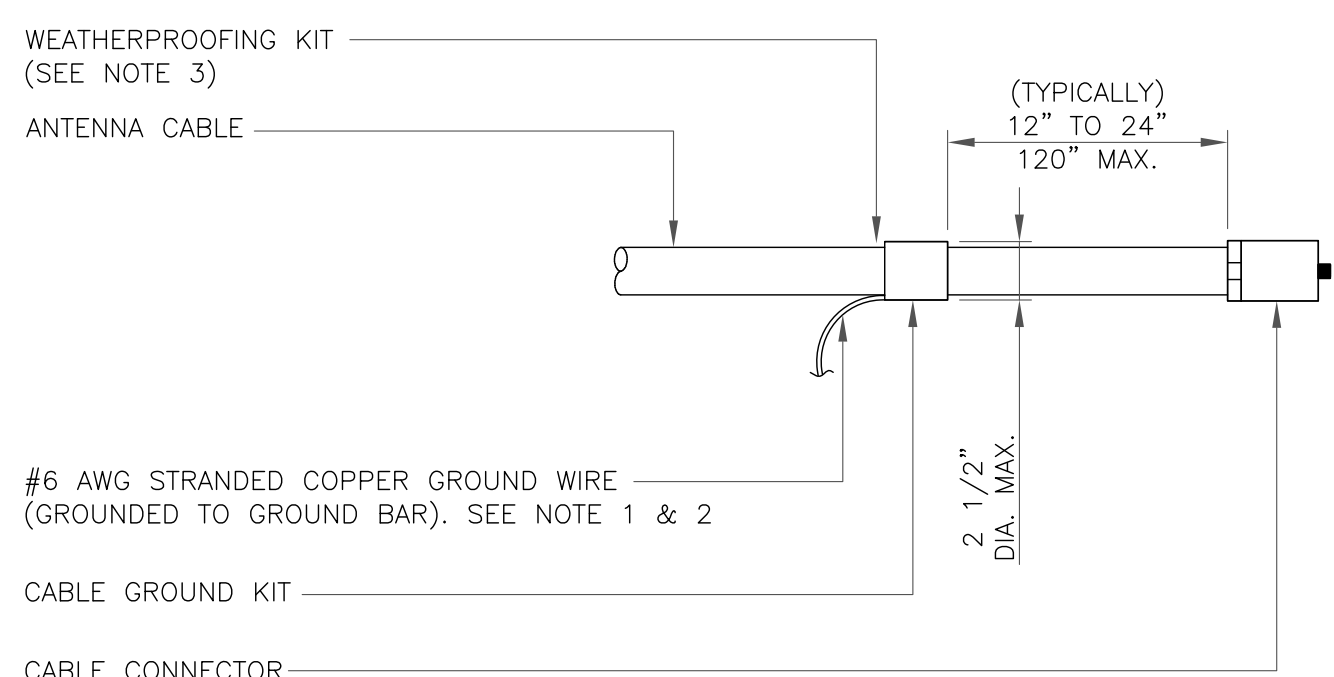
1



**NOTE:**

1. ERICO EXOTHERMIC "MOLD TYPES" SHOWN HERE ARE EXAMPLES. CONSULT WITH CONSTRUCTION MANAGER FOR SPECIFIC MOLDS TO BE USED FOR THIS PROJECT.
2. MOLD TYPE ONLY TO BE USED BELOW GRADE WHEN CONNECTING GROUND RING TO GROUND ROD.

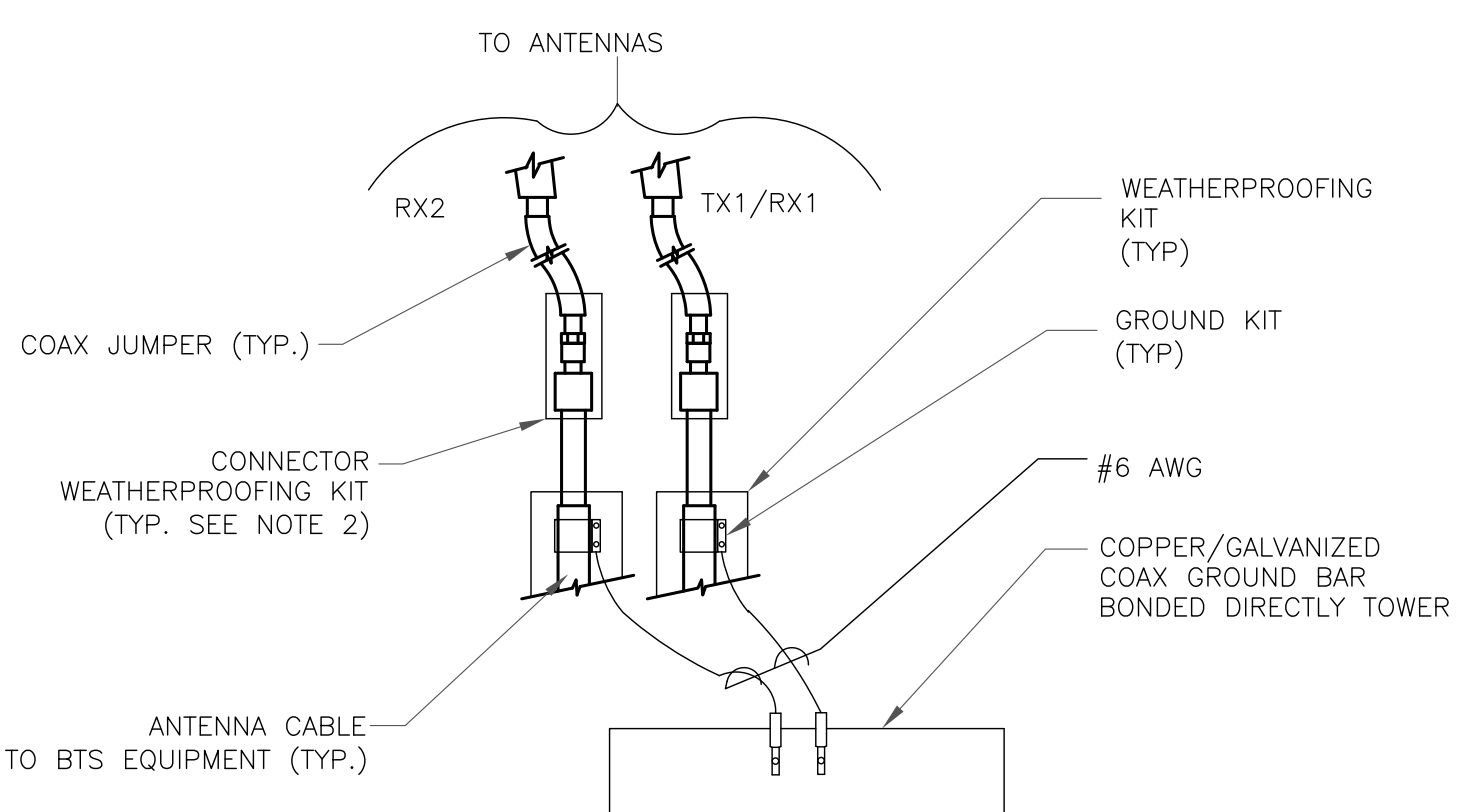
**1 CADWELD GROUNDING CONNECTIONS**  
SCALE: NOT TO SCALE



**NOTES:**

1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
2. GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
3. WEATHER PROOFING SHALL BE TWO-PART TAPE KIT. COLD SHRINK SHALL NOT BE USED.

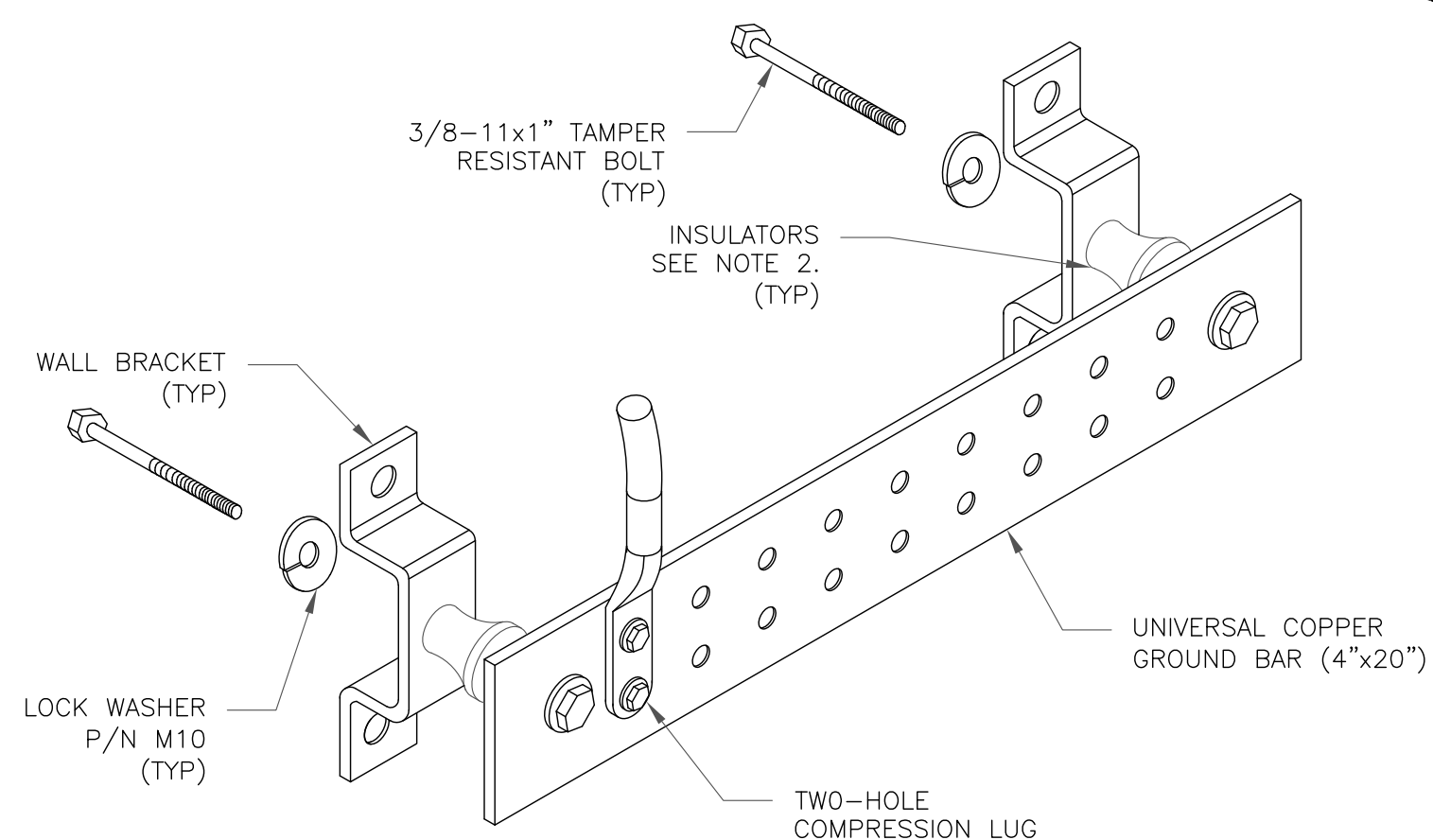
**3 CABLE GROUND KIT CONNECTION**  
SCALE: NOT TO SCALE



**NOTES:**

1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO ANTENNA GROUND BAR.
2. WEATHER PROOFING SHALL BE TWO-PART TAPE KIT. COLD SHRINK SHALL NOT BE USED.

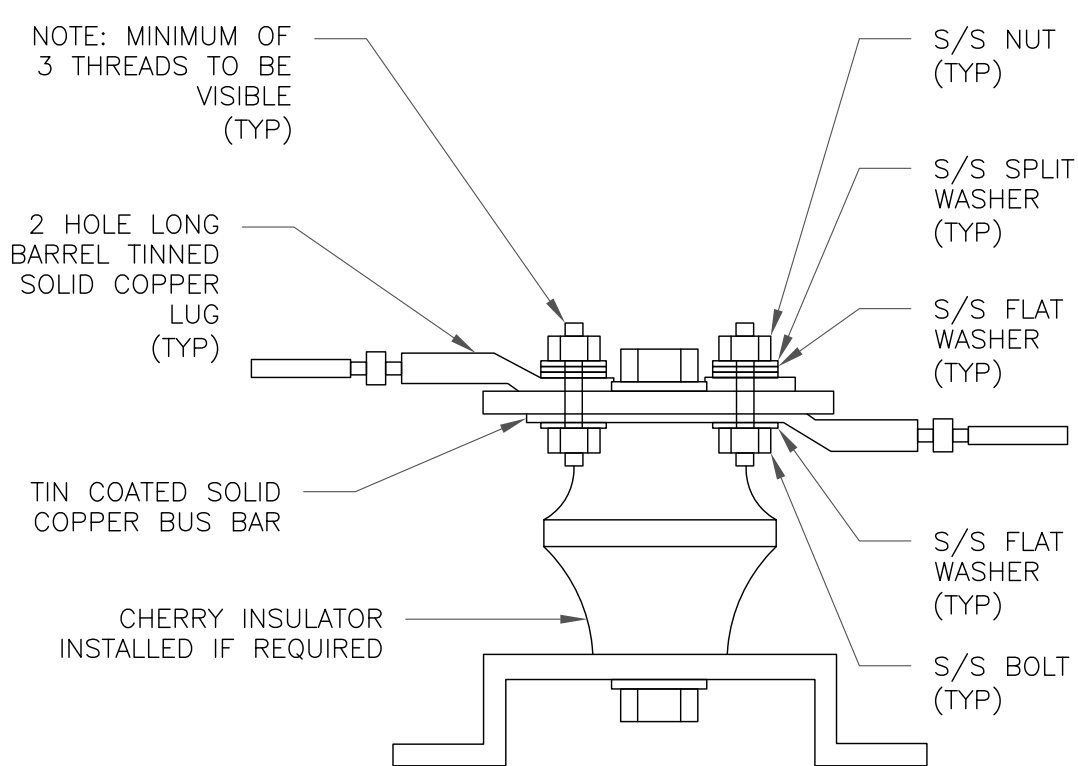
**4 GROUND CABLE CONNECTION**  
SCALE: NOT TO SCALE



**NOTES:**

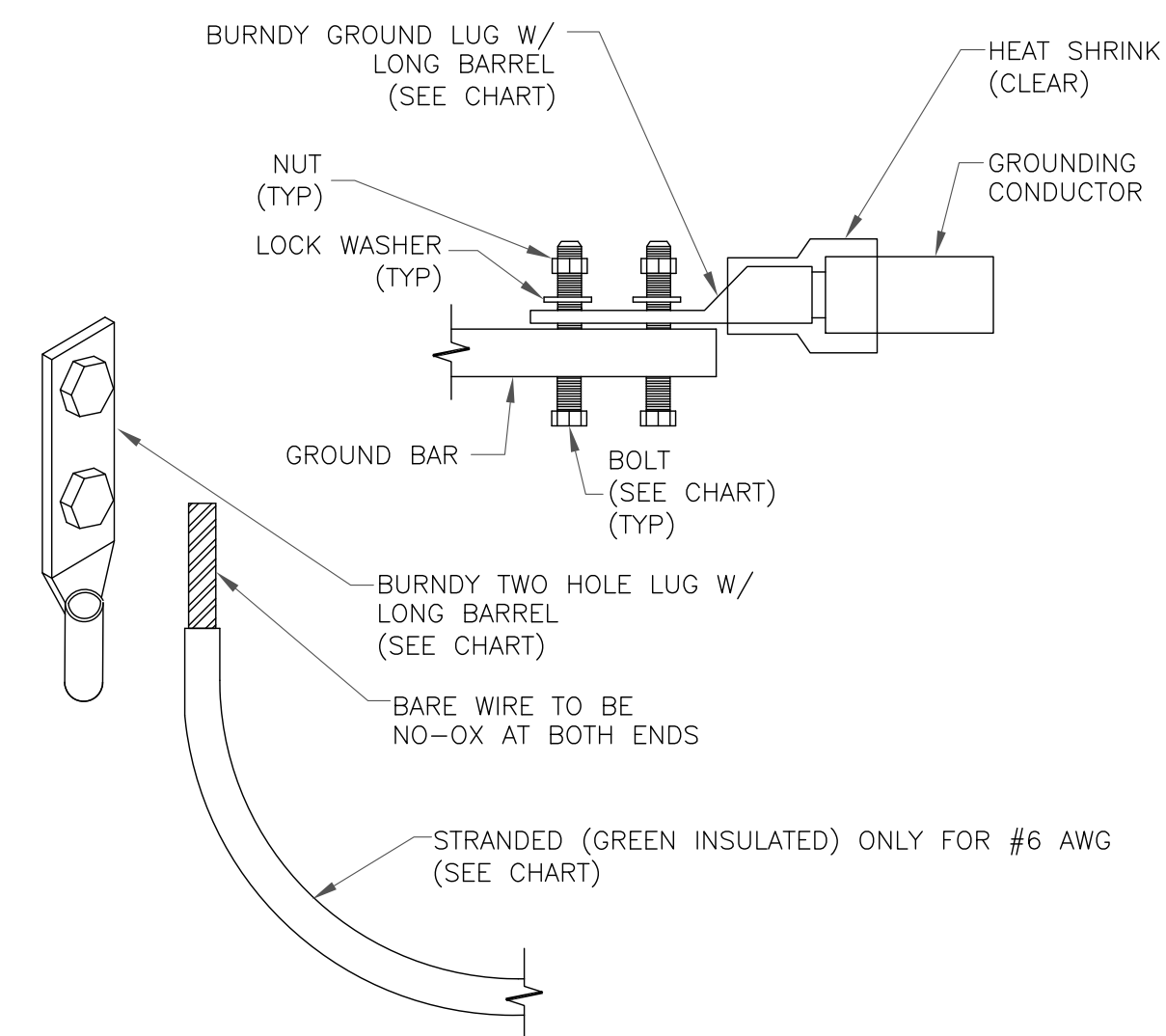
1. DOWN LEAD (HOME RUN) CONDUCTORS ARE NOT TO BE INSTALLED ON CROWN CASTLE USA INC. TOWER, PER THE GROUNDING DOWN CONDUCTOR POLICY QAS-STD-10091. NO MODIFICATION OR DRILLING TO TOWER STEEL IS ALLOWED IN ANY FORM OR FASHION, CAD-WELDING ON THE TOWER AND/OR IN THE AIR ARE NOT PERMITTED.
2. OMIT INSULATOR WHEN MOUNTING TO TOWER STEEL OR PLATFORM STEEL. USE INSULATORS WHEN ATTACHING TO BUILDING OR SHELTERS.

**6 GROUND BAR DETAIL**  
SCALE: NOT TO SCALE



**7 LUG DETAIL**  
SCALE: NOT TO SCALE

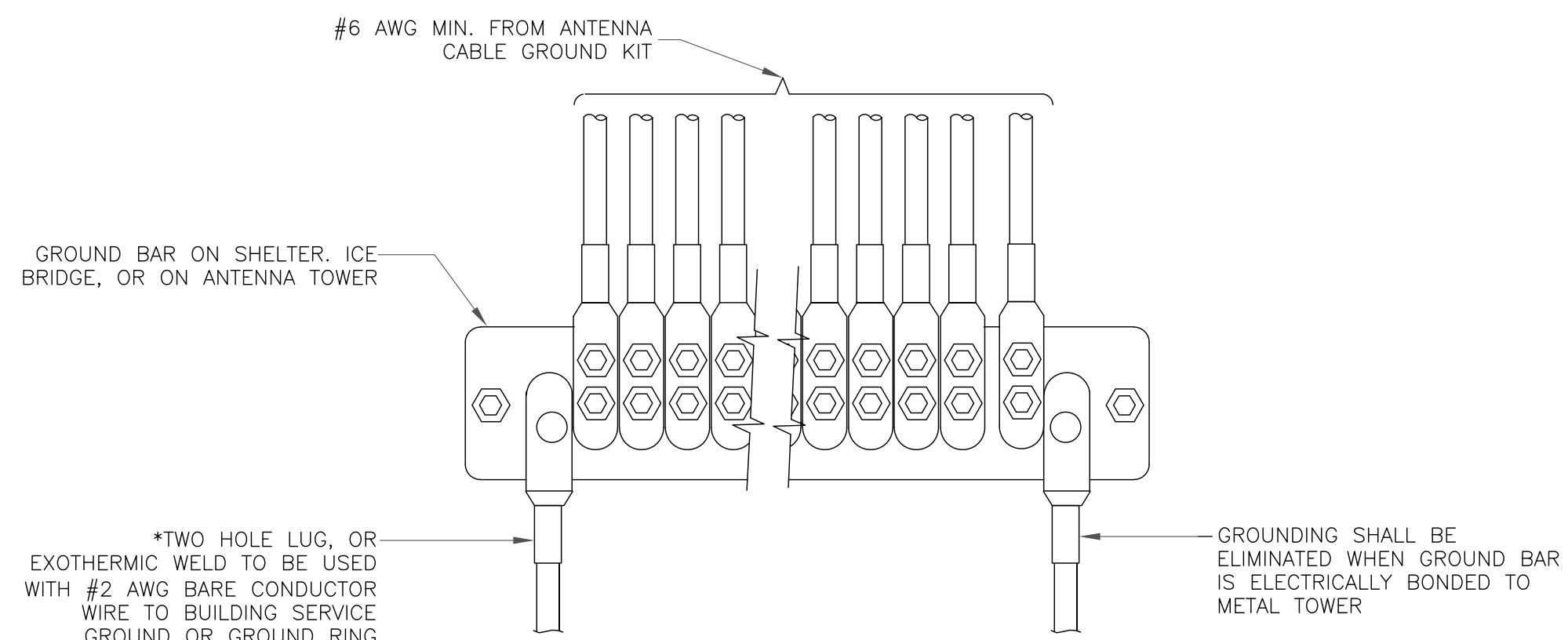
WIRE SIZE	BURNDY LUG	BOLT SIZE
#6 AWG GREEN INSULATED	YA6C-2TC38	3/8" - 16 NC S 2 BOLT
#2 AWG SOLID TINNED	YA3C-2TC38	3/8" - 16 NC S 2 BOLT
#2 AWG STRANDED	YA2C-2TC38	3/8" - 16 NC S 2 BOLT
#2/0 AWG STRANDED	YA26-2TC38	3/8" - 16 NC S 2 BOLT
#4/0 AWG STRANDED	YA28-2N	1/2" - 16 NC S 2 BOLT



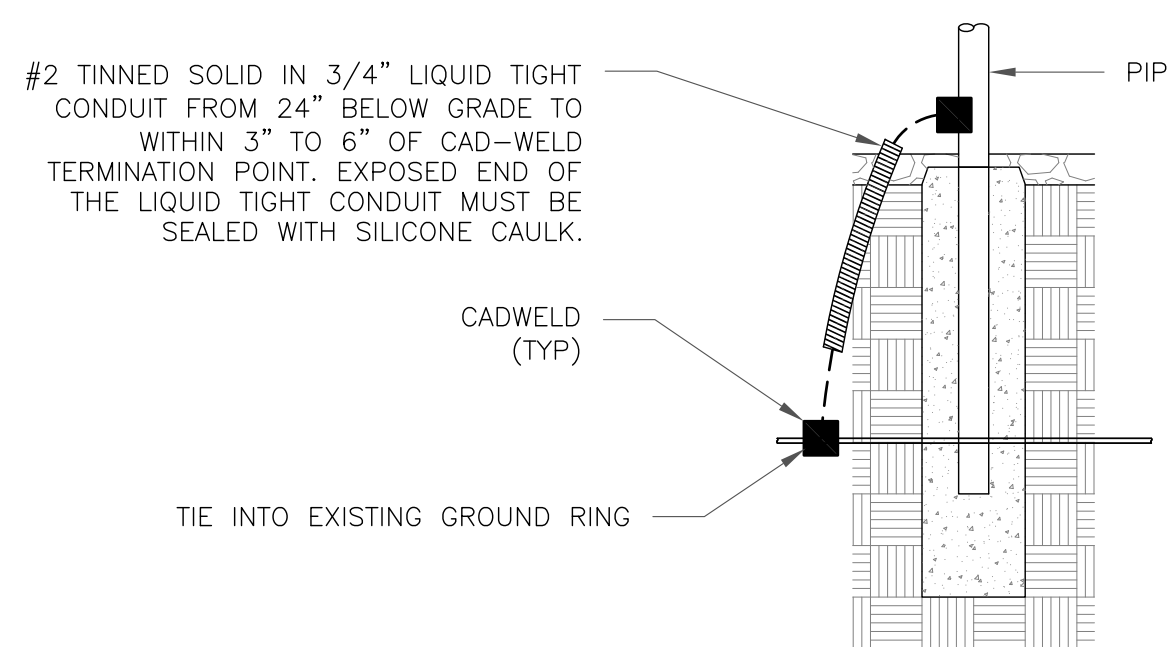
**NOTES:**

1. ALL GROUNDING LUGS ARE TO BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS. ALL HARDWARE BOLTS, NUTS, LOCK WASHERS SHALL BE STAINLESS STEEL. ALL HARDWARE ARE TO BE AS FOLLOWS: BOLT, FLAT WASHER, GROUND BAR, GROUND LUG, FLAT WASHER AND NUT.

**2 MECHANICAL LUG CONNECTION**  
SCALE: NOT TO SCALE



**5 GROUNDWIRE INSTALLATION**  
SCALE: NOT TO SCALE



**8 TRANSITIONING GROUND DETAIL**  
SCALE: NOT TO SCALE

**T-Mobile**  
35 GRIFFIN ROAD  
BLOOMFIELD, CT 06002

**CROWN CASTLE**  
3 CORPORATE PARK DRIVE, SUITE 101  
CLIFTON PARK, NY 12065

**B+T GRP**  
1717 S. BOULDER  
SUITE 300  
TULSA, OK 74119  
PH: (918) 587-4630  
www.btgrp.com

T-MOBILE SITE NUMBER:  
**CT11381C**

BU #: **876338**  
**WATERFORD**

41 MANITOCK HILL ROAD  
WATERFORD, CT 06385

EXISTING  
136'-0" SELF-SUPPORT TOWER

**ISSUED FOR:**

REV	DATE	DRWN	DESCRIPTION	DES./QA
0	9/01/21	KT	CONSTRUCTION	KT
1	9/02/21	KT	CONSTRUCTION	KT
1	9/24/21	YXI	CONSTRUCTION	YXI



B&T ENGINEERING, INC.  
PEC.0001564  
Expires 2/10/22

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER:

**G-3**

REVISION:

**1**

Date: **August 24, 2021**

Darcy Tarr  
Crown Castle  
3530 Toringdon Way, Suite 300  
Charlotte, NC 28277  
(704) 405-6589



Trylon  
1825 W. Walnut Hill Lane,  
Suite 302  
Irving, TX 75038  
214-930-1730

**Subject:** **Mount Replacement Analysis Report**

**Carrier Designation:** **T-Mobile Anchor**  
**Carrier Site Number:** CT11381C  
**Carrier Site Name:** WATERFORD SOUTH/RT 1

**Crown Castle Designation:** **Crown Castle BU Number:** 876338  
**Crown Castle Site Name:** WATERFORD  
**Crown Castle JDE Job Number:** 675143  
**Crown Castle Order Number:** 576542 Rev. 0

**Engineering Firm Designation:** **Trylon Report Designation:** 190496

**Site Data:** **41 Manitock Hill Road, Waterford, New London County, CT, 06385-2000**  
**Latitude 41°21'16.70" Longitude -72°9'1.60"**

**Structure Information:** **Tower Height & Type:** **136.0 ft Self Support**  
**Mount Elevation:** **117.0 ft**  
**Mount Type:** **12.5 ft Sector Frame**

Dear Darcy Tarr,

Trylon is pleased to submit this “**Mount Replacement Analysis Report**” to determine the structural integrity of T-Mobile’s antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

**Sector Frame**

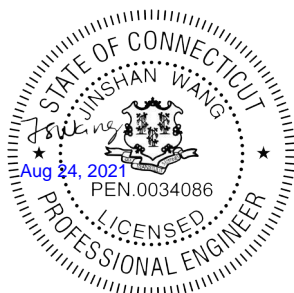
**Sufficient\***

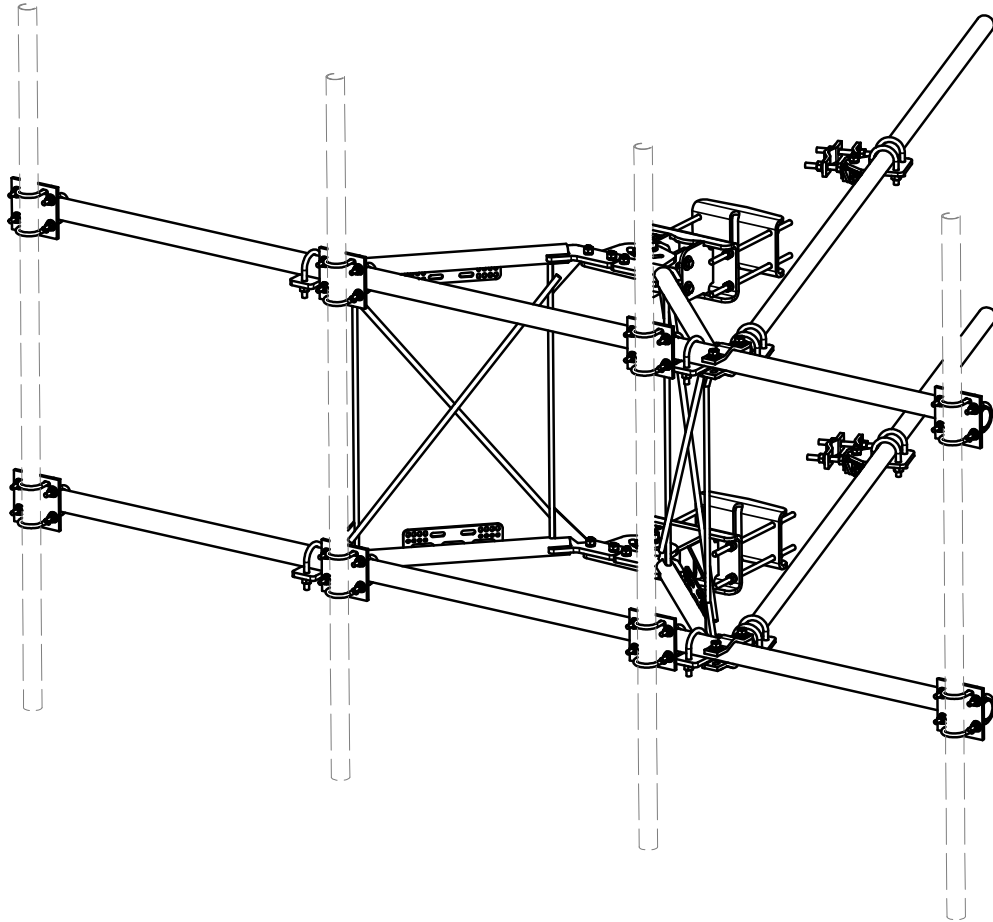
**\*Sufficient upon completion of the changes listed in the ‘Recommendations’ section of this report.**

This analysis utilizes an ultimate 3-second gust wind speed of 135 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Andrei Florea

Respectfully Submitted by:  
Jinshan Wang, P.E





PARTS LIST						
ITEM	QTY	PART NO.	PART DESCRIPTION	LENGTH	UNIT WT.	NET WT.
1	2	X-VFAW	SUPPORT ARM		71.41	142.81
2	1	X-HDCAMTBW	CLAMP WELDMENT FOR BCAM-HD		33.86	33.86
3	1	X-MHTPHD	MULTI-HOLE TAPER PLATE WELDMENT		36.24	36.24
4	2	X-VFAPL4	VFA-HD PIVOT PLATE	12 in	15.88	31.77
5	2	X-LCBP4	BENT BACKING PLATE	13 in	19.00	38.01
6	1	X-HDCAMSS	ANGLE ADJUSTMENT WELDMENT FOR BCAM-HD		16.39	16.39
7	4	X-SPTB	SLIDING PIPE TIE BACK PLATE	5 1/2 in	5.87	23.49
8	1	X-HDCAMSP	POSITIONING PLATE WELDMENT FOR BCAM-HD		2.58	2.58
9	4	X-TBCA	TIE BACK CLIP ANGLE		2.01	8.02
10	8	SCX2	CROSSOVER PLATE	7 in	4.80	38.37
11	4	MCP	CLAMP HALF 1/2" THICK, 11-5/8" LONG	12 1/16 in	3.59	14.37
12	8	DCP	1/2" THICK, 5-3/4" CTR TO CENTER CLAMP HALF	8 1/8 in	2.36	18.90
13	2	P2126	2-3/8" X 126" (2" SCH. 40) GALVANIZED PIPE	126 in	40.75	81.50
14	2	P30150	2-7/8" X 150" (2-1/2" SCH. 40) GALVANIZED PIPE	150 in	76.94	153.87
15	4	A34212	3/4" x 2-1/2" UNC HEX BOLT (A325)	2 1/2 in	0.48	1.92
16	4	G34FW	3/4" HDG USS FLATWASHER		0.06	0.24
17	4	G34LW	3/4" HDG LOCKWASHER		0.04	0.17
18	4	G34NUT	3/4" HDG HEAVY 2H HEX NUT		0.21	0.85
19	8	G58R-18	5/8" x 18" THREADED ROD (HDG.)	18 in	0.40	3.19
20	4	G58R-12	5/8" x 12" THREADED ROD (HDG.)		1.05	4.18
21	4	G58R-8	5/8" x 8" THREADED ROD (HDG.)		0.70	2.79
22	4	X-UB5300	5/8" X 3" X 5-1/4" X 2-1/2" U-BOLT (HDG.)		1.15	4.60
23	8	X-UB5258	5/8" X 2-5/8" X 4-1/2" X 2" U-BOLT (HDG.)		1.00	8.00
24	2	G5807	5/8" x 7" HDG HEX BOLT GR5 FULL THREAD	7 in	0.70	1.41
25	1	G5806	5/8" x 6" HDG HEX BOLT GR5 FULL THREAD	6 in	0.62	0.62
26	8	G5804	5/8" x 4" HDG HEX BOLT GR5		0.44	3.55
27	4	G5802	5/8" x 2" HDG HEX BOLT GR5		0.27	1.08
28	8	A582114	5/8" x 2-1/4" HDG A325 HEX BOLT	2 1/4 in	0.31	2.50
29	25	G58FW	5/8" HDG USS FLATWASHER	1/8 in	0.07	1.76
30	66	G58LW	5/8" HDG LOCKWASHER		0.03	1.72
31	71	G58NUT	5/8" HDG HEAVY 2H HEX NUT		0.13	9.22
32	32	X-UB1300	1/2" X 3" X 5" X 2" GALV U-BOLT		0.74	23.64
33	16	X-UB1212	1/2" X 2" X 3" X 1-1/4" U-BOLT (HDG.)		0.60	9.56
34	64	G12FW	1/2" HDG USS FLATWASHER	3/32 in	0.03	2.18
35	64	G12LW	1/2" HDG LOCKWASHER	1/8 in	0.01	0.89
36	64	G12NUT	1/2" HDG HEAVY 2H HEX NUT		0.07	4.58
					TOTAL WT. #	738.06

REV	DESCRIPTION OF REVISIONS	CPD	BY	DATE
D	UPDATED BCAM VERSION 1 TO BCAM VERSION 2		CEK	6/29/2018
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B	CHANGED TIE-BACK BACK CONNECTION		CEK	7/31/2017
A	CHANGED TIE-BACK FRONT CONNECTION		CEK	2/2/2017

**TOLERANCE NOTES**

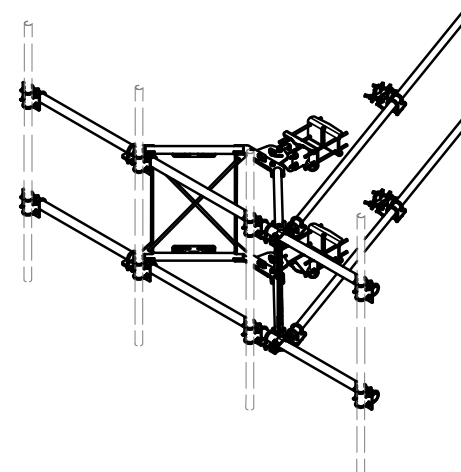
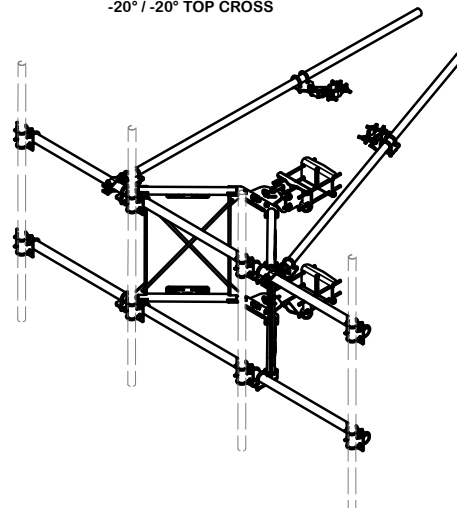
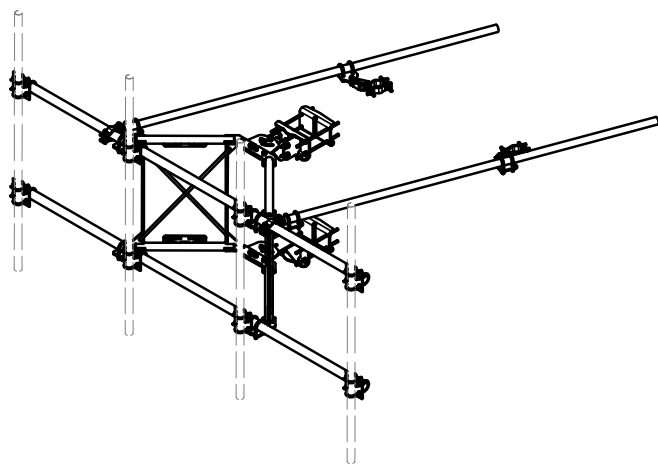
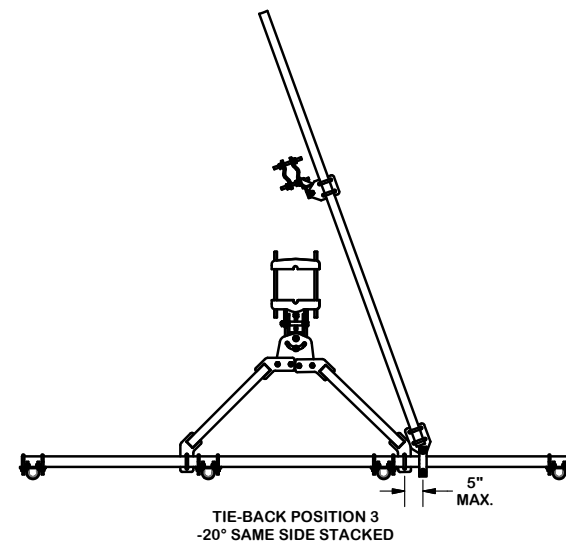
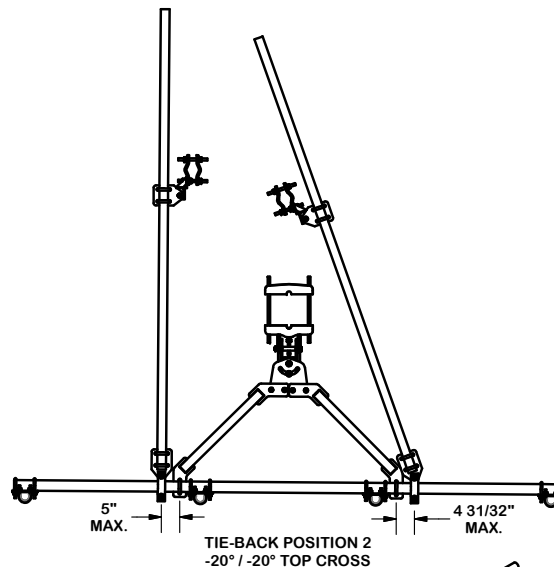
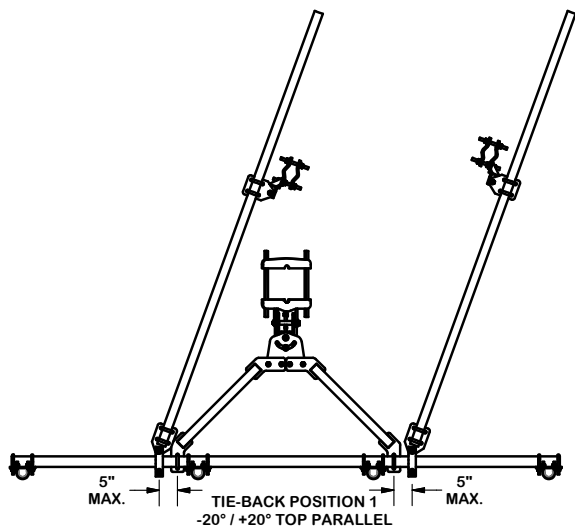
TOLERANCES ON DIMENSIONS, UNLESS OTHERWISE NOTED ARE:  
 SAWED, SHEARED AND GAS CUT EDGES ( $\pm 0.030"$ )  
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 ALL OTHER ASSEMBLY ( $\pm 0.060"$ )

PROPRIETARY NOTE:  
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DESCRIPTION		12' 6" HEAVY DUTY V-FRAME ASSEMBLY WITH TWO STIFF ARMS	
CPD NO.	DRAWN BY	ENG. APPROVAL	
	CEK 1/25/2017		
CLASS	SUB	DRAWING USAGE	CHECKED BY
81	02	CUSTOMER	BMC 12/13/2017

 A valmont COMPANY	Locations: New York, NY Atlanta, GA Los Angeles, CA Plymouth, IN Salem, OR Dallas, TX
	Engineering Support Team: 1-888-753-7446
PART NO.	VFA12-HD
DWG. NO.	VFA12-HD

# TIE-BACK POSITIONS



REV	DESCRIPTION OF REVISIONS	CPD	BY	DATE
D	UPDATED BCAM VERSION 1 TO BCAM VERSION 2		CEK	6/29/2018
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REVISION HISTORY

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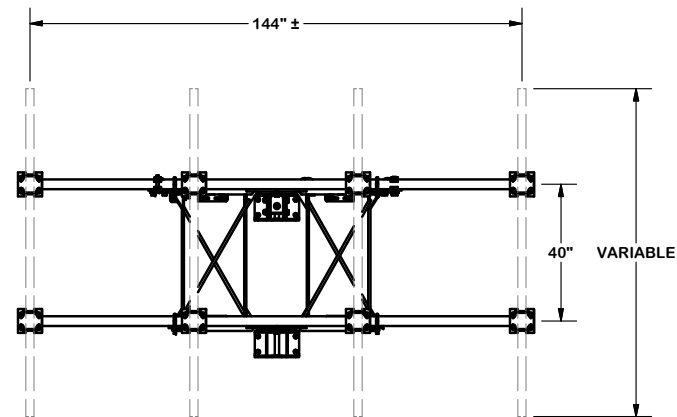
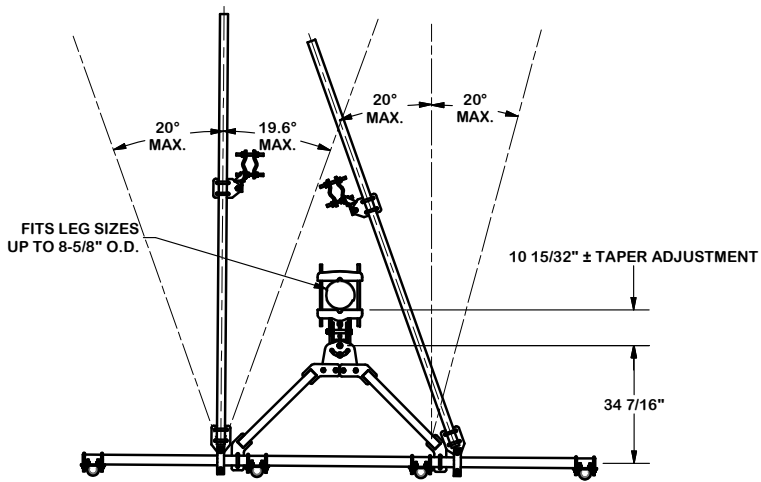
DESCRIPTION  
 12' 6" HEAVY DUTY  
 V-FRAME ASSEMBLY  
 WITH TWO STIFF ARMS

CPD NO.	DRAWN BY	ENG. APPROVAL
	CEK 1/25/2017	
CLASS	DRAWING USAGE	CHECKED BY
81	CUSTOMER	BMC 12/13/2017

**SITE PRO 1**  
 Engineering Support Team:  
 1-888-753-7446

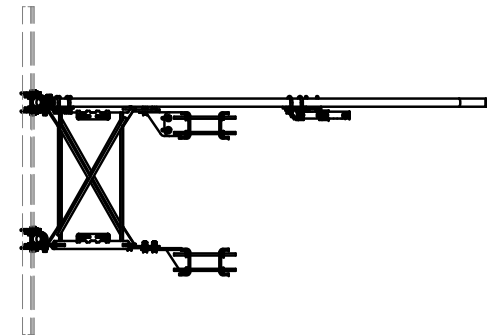
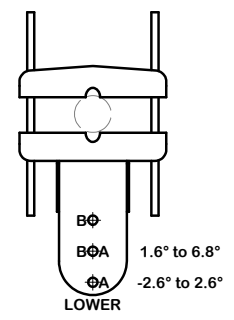
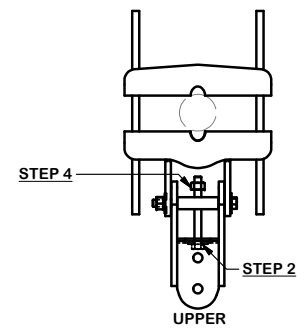
Locations:  
 New York, NY  
 Atlanta, GA  
 Los Angeles, CA  
 Plymouth, IN  
 Salem, OR  
 Dallas, TX

PART NO.	VFA12-HD
DWG. NO.	VFA12-HD



**ANGLE CALIBRATING PROCEDURE:**

1. MEASURE TOWER TAPER AND PICK LOWER BRACKET HOLE:
  - HOLE A = -2.6° TO 2.6°
  - HOLE B = 1.6° TO 6.8°
2. USE CALIBRATING BOLT TO ADJUST FRAME TO DESIRED TAPER
3. TORQUE LOCKING BOLTS TO 100 ft.-lbs.
4. ADVANCE LOCKING NUT TO POSITIONING PLATE, THEN TIGHTEN.



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**REVISION HISTORY**

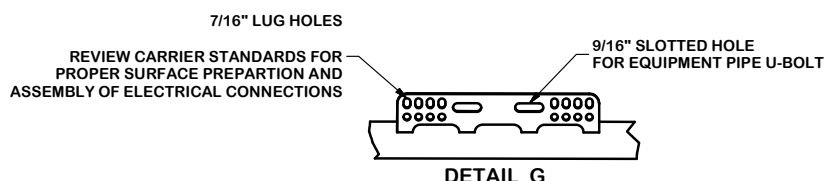
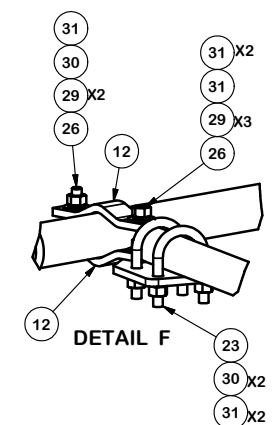
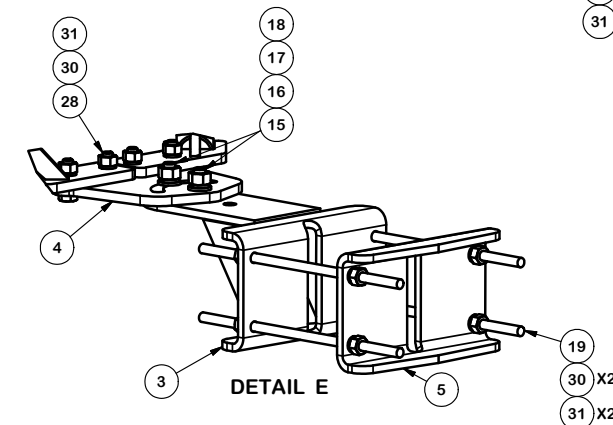
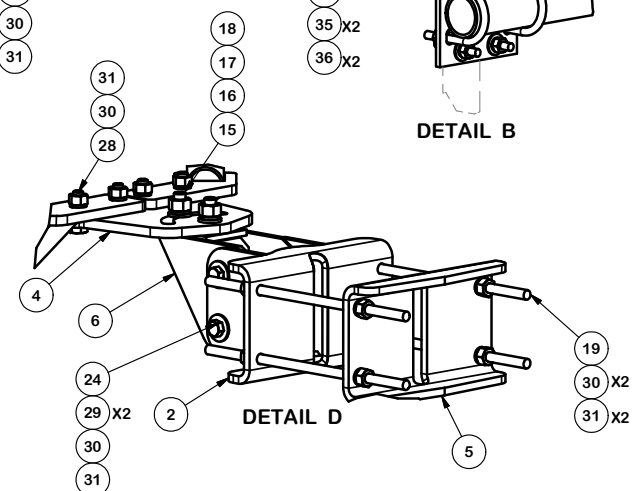
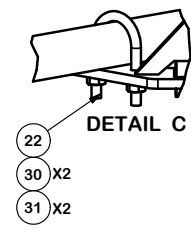
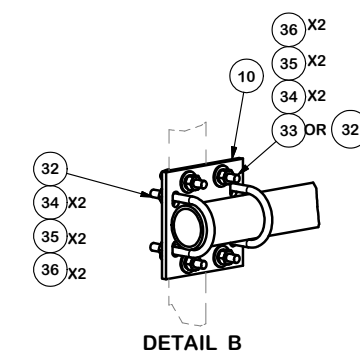
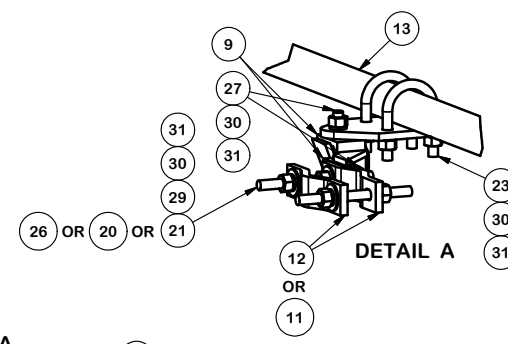
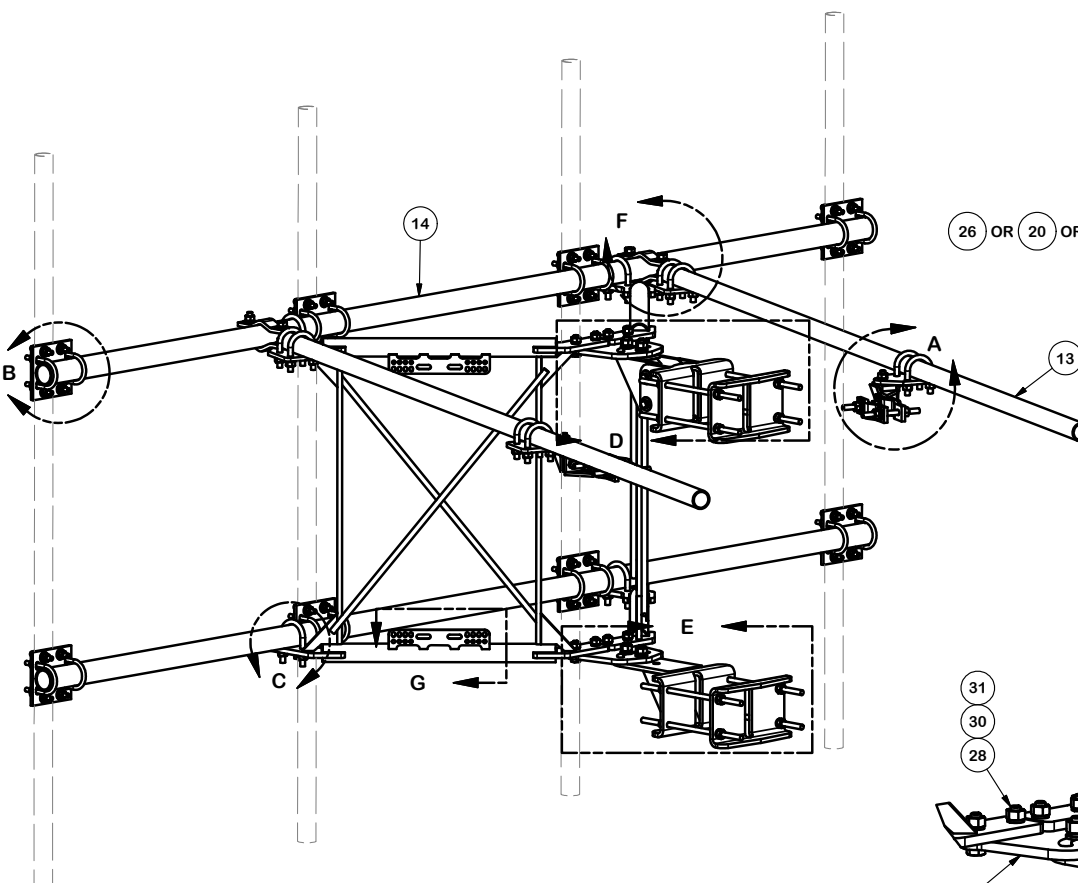
**TOLERANCE NOTES**

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DESCRIPTION		12' 6" HEAVY DUTY V-FRAME ASSEMBLY WITH TWO STIFF ARMS	
CPD NO.	DRAWN BY	ENG. APPROVAL	
	CEK 1/25/2017		
CLASS	SUB	DRAWING USAGE	CHECKED BY
81	02	CUSTOMER	BMC 12/13/2017

 A valmont COMPANY	Locations: New York, NY Atlanta, GA Los Angeles, CA Plymouth, IN Salem, OR Dallas, TX	
	Engineering Support Team: 1-888-753-7446	
PART NO.	VFA12-HD	3 OF 5 PAGE
DWG. NO.	VFA12-HD	



REV	DESCRIPTION OF REVISIONS	CPD	BY	DATE
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REVISION HISTORY				

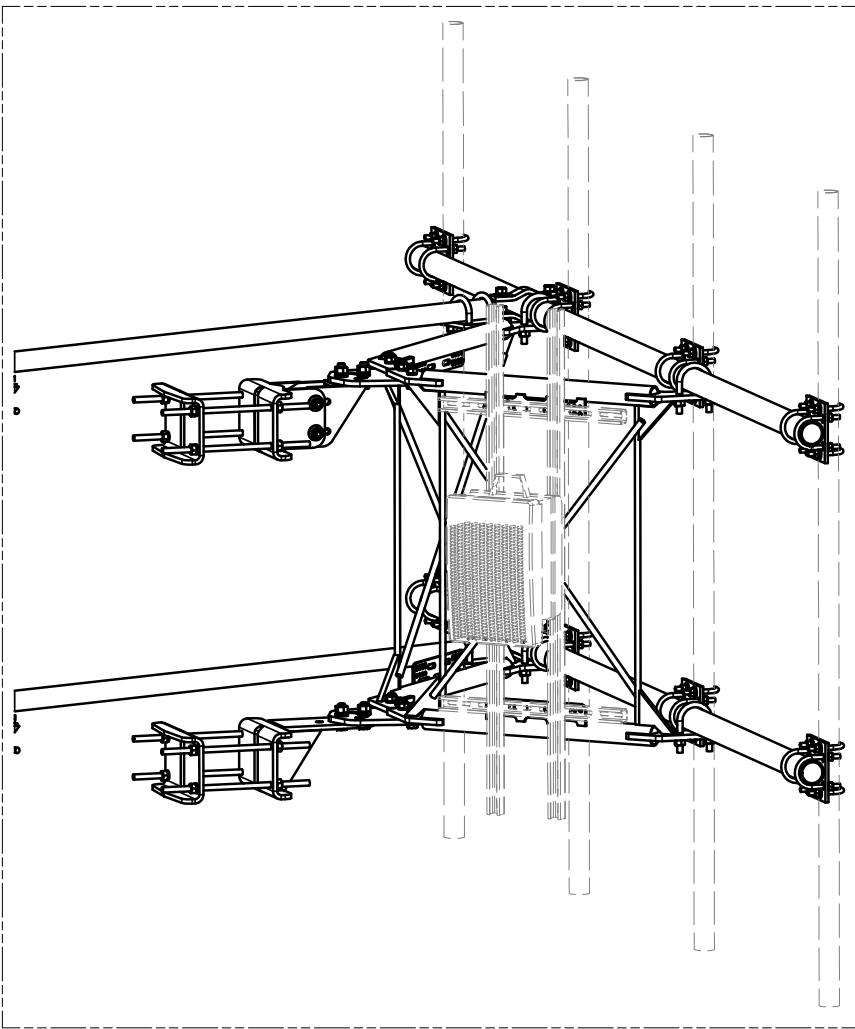
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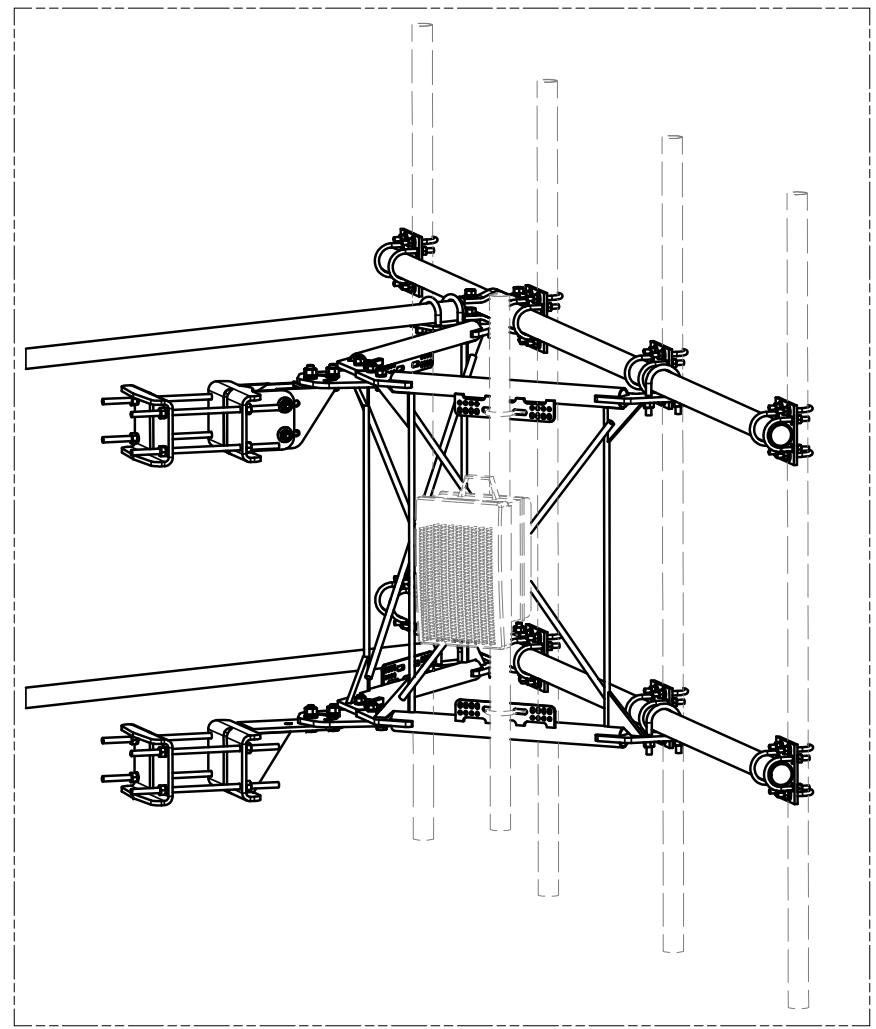
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CLASS	ENG. APPROVAL
81	BMC 12/13/2017
SUB	CHECKED BY
02	CUSTOMER

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PART NO.	VFA12-HD
DWG. NO.	VFA12-HD



UNISTRUT AND HARDWARE  
SOLD SEPARATELY.

REQUIRES 3/8" HARDWARE




EQUIPMENT PIPE AND HARDWARE  
SOLD SEPARATELY.

REQUIRES 1/2" HARDWARE  
AND 2-3/8" TO 4-1/2" O.D. PIPE

REV	DESCRIPTION OF REVISIONS	CPD	BY	DATE
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REVISION HISTORY				

TOLERANCE NOTES	
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<p><small>PROPRIETARY NOTE:            THE DATA AND TECHNIQUES CONTAINED IN THIS DRAWING ARE PROPRIETARY INFORMATION OF VALMONT INDUSTRIES AND CONSIDERED A TRADE SECRET. ANY USE OR DISCLOSURE WITHOUT THE CONSENT OF VALMONT INDUSTRIES IS STRICTLY PROHIBITED.</small></p>	

DESCRIPTION	
<p>12' 6" HEAVY DUTY V-FRAME ASSEMBLY WITH TWO STIFF ARMS</p>	
CPD NO.	DRAWN BY
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CLASS	DRAWING USAGE
81	CUSTOMER
SUB	CHECKED BY
02	BMC 12/13/2017

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PART NO. <b>VFA12-HD</b>	DWG. NO. <b>VFA12-HD</b>