

RACHEL A. SCHWARTZMAN

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May 14, 2015

Attorney Melanie Bachman  
Acting Executive Director  
Connecticut Siting Council  
Ten Franklin Square  
New Britain, CT 06501

**Re: Notice of Exempt Modification  
Verizon Wireless/T-Mobile Equipment Upgrade  
Site ID CT11041D  
53 Dayton Road, Waterford, CT**

Dear Attorney Bachman:

This office represents T-Mobile Northeast LLC ("T-Mobile") and has been retained to file exempt modification filings with the Connecticut Siting Council on its behalf.

In this case, Verizon Wireless owns the existing lattice telecommunications tower and related facility at 53 Dayton Road, Waterford, CT (-72.13940/41.377739). T-Mobile intends to replace six (6) existing antennas with six (6) new antennas and related equipment at this existing telecommunications facility in Waterford ("Waterford Facility"). Please accept this letter as notification, pursuant to R.C.S.A. §16-50j-73, of construction which constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to the first selectman, Daniel M. Steward, and the property owner, Verizon Wireless.

The existing Waterford Facility consists of a 180-foot lattice tower.<sup>1</sup> T-Mobile plans to replace six (6) existing antennas on T-arms with six (6) new antennas on T-arms at a centerline of 164 feet. T-Mobile will also add three (3) remote radio units (RRUs) mounted behind some of the new antennas at the 164-foot centerline and reuse existing spare fiber cables. (See the plans dated February 26, 2015 attached hereto as **Exhibit A**). The existing Waterford Facility is structurally capable of supporting T-Mobile's proposed modifications, as indicated in the structural analysis dated April 22, 2015, and attached hereto as **Exhibit B**.

<sup>1</sup> While the online docket for the Connecticut Siting Council does not provide a docket or petition number for approval of this structure, it does reference this structure in connection with a notices of intent captioned EM-OCI-152-990728, TS-AT&T-152-010426, EM-T-MOBILE-152-030617, EM-VER-152-041007, EM-CING-132-134-152-165-166-070726, EM-WTFD-152-080627, TS-METROPCS-152-090107, EM-T-MOBILE-152-090204, EM-VER-152-120622, EM-T-MOBILE-152-120702, EM-CING-152-121114, EM-VER-152-130214, EM-VER-152-150305.

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The planned modifications to the Waterford Facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modification will not increase the height of the tower. T-Mobile's existing antennas are at a centerline of 164 feet; the replacement antennas will be installed at the same 164-foot level. The enclosed tower drawing confirms that the proposed modification will not increase the height of the tower.

2. The proposed modifications will not require an extension on the site boundaries or lease area, as depicted on Sheet LE-1 of Exhibit A. T-Mobile's equipment will be located entirely within the existing compound area.

3. The proposed modification to the Facility will not increase the noise levels at the existing facility by six decibels or more.

4. The operation of the replacement antennas and equipment will not increase the total radio frequency (RF) power density, measured at the base of the tower, to a level at or above the applicable standard. According to a Radio Frequency Emissions Analysis Report prepared by EBI dated April 27, 2015, T-Mobile's operations would add 4.83% of the FCC Standard. Therefore, the calculated "worst case" power density for the planned combined operation at the site including all of the proposed antennas would be 46.48% of the FCC Standard as calculated for a mixed frequency site as evidenced by the engineering exhibit attached hereto as **Exhibit C**.

For the foregoing reasons, T-Mobile respectfully submits that the proposed replacement antennas and equipment at the Waterford Facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Upon acknowledgement of this exempt modification, T-Mobile shall commence construction approximately sixty days from the receipt of the Council's decision.

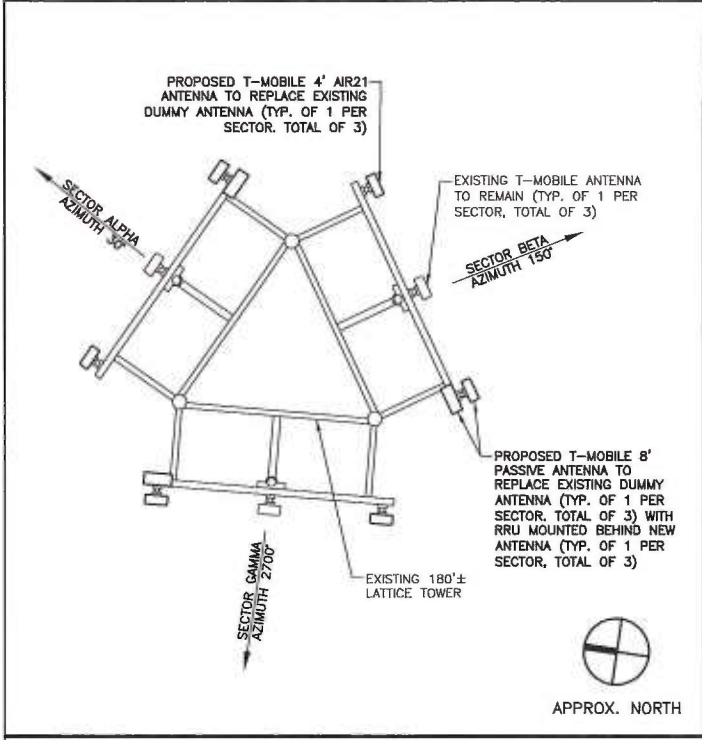
Sincerely,



Rachel A. Schwartzman, Esq.

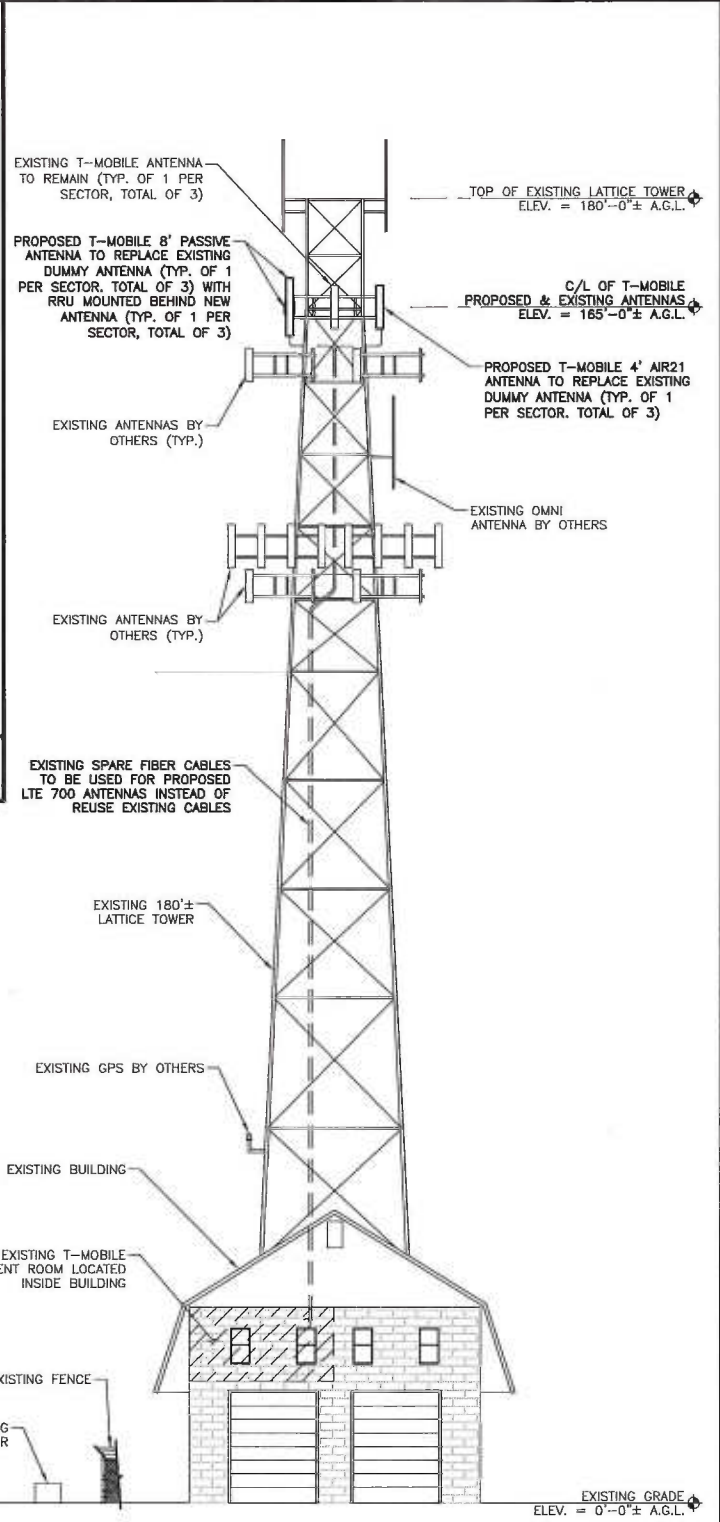
cc: Town of Waterford, First Selectman Daniel M. Steward  
Verizon Wireless  
Jamie Ford, EBI Consulting

# **EXHIBIT A**



ANTENNA CONFIGURATION

NTS



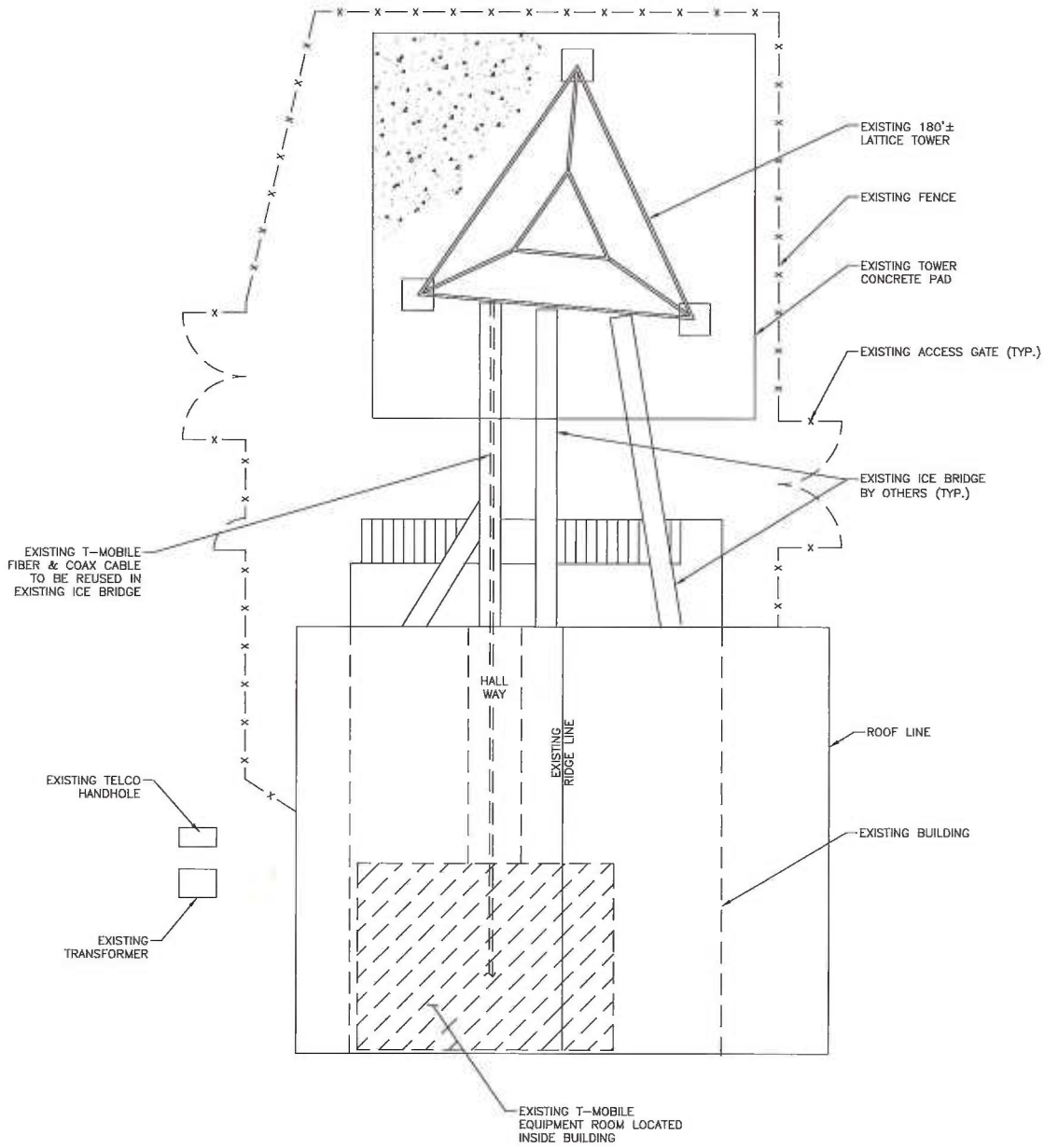
TOWER ELEVATION

SCALE: 1" = 25'-0"

CONFIGURATION  
**702CU**

NOTE:  
ALL EQUIPMENT LOCATIONS ARE APPROXIMATE AND ARE SUBJECT TO APPROVAL BY LESSEE/LICENSEE STRUCTURAL AND RF ENGINEERS.

PREPARED BY: 21 B Street   Burlington, MA 01803 Tel: (781) 273-2500   Fax: (781) 273-3311 www.ebiconsulting.com EBI JOB NO.: 811500084	CLIENT: T-Mobile Northeast, LLC 35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 860.692.7100	SITE INFO: CT11041D WATERFORD/ I-95/X82 53 DAYTON ROAD WATERFORD, CT 06385	SUBMITTALS			DRAWN BY:	SHEET NO:
			NO.	DATE	DESCRIPTION	BY	LF
A	02/26/15	FOR REVIEW	LF		DATE: 02/26/15		




CONFIGURATION  
**702CU**



NOTE:  
 ALL EQUIPMENT LOCATIONS ARE APPROXIMATE AND ARE SUBJECT TO APPROVAL BY LESSEE/LICENSEE STRUCTURAL AND RF ENGINEERS.

**SITE PLAN**

SCALE: 1/16" = 1'-0"

PREPARED BY:  21 B Street   Burlington, MA 01803 Tel: (781) 273-2500   Fax: (781) 273-3311 www.ebiconsulting.com	CLIENT: <b>T-Mobile Northeast, LLC</b> 35 GRIFFIN ROAD SOUTH BLOOMFIELD, CT 06002 860.692.7100	SITE INFO: <b>CT11041D          WATERFORD/          I-95/X82</b> 53 DAYTON ROAD WATERFORD, CT 06385	SUBMITTALS			DRAWN BY:	SHEET NO:
			NO.	DATE	DESCRIPTION	BY	LF
A	02/26/15	FOR REVIEW	LF	CHECKED BY:			
				BB			
				DATE:			
				02/26/15			

# **EXHIBIT B**

**Structural Analysis Report**

*180-ft Existing ROHN Lattice Tower*

*Proposed T-Mobile  
Antenna Upgrade*

*T-Mobile Site Ref: CT11041D*

*Verizon Site Ref: Waterford*

*53 Dayton Road  
Waterford, CT*

*Centek Project No. 15049.002*

*Date: April 22, 2015*



**Prepared for:**  
T-Mobile USA  
35 Griffin Road  
Bloomfield, CT 06002

*CENTEK Engineering, Inc.*  
*Structural Analysis – 180' ROHN Lattice Tower*  
*T-Mobile Antenna Upgrade – CT11041D*  
*Waterford, CT*  
*April 22, 2015*

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## Introduction

The purpose of this report is to summarize the results of the non-linear, P- $\Delta$  structural analysis of the antenna upgrade proposed by T-Mobile on the existing lattice tower located in Waterford, Connecticut.

The host tower is a 180-ft, nine-section, three legged, self-supporting tapered lattice tower originally designed and manufactured by ROHN Industries Inc., Eng file no. 38103AE, dated August 24, 1998. The tower geometry, structure member sizes and anchor bolt properties were obtained from a previous structural analysis report prepared by Centek Engineering job no. 15001.009 dated February 6, 2015. Original foundation loading was obtained from a structural analysis report prepared by URS Corporation, signed and sealed on November 8, 2002.

Antenna and appurtenance inventory were taken from the aforementioned Centek structural report, a tower mapping report prepared by Eastern Communications dated March 30, 2015 and a T-Mobile RF data sheet.

The tower consists of nine (9) tapered vertical sections with steel pipe legs conforming to ASTM A572-50. Horizontal and diagonal lateral support bracing consists of steel pipe construction conforming to ASTM A36M-42. The vertical tower sections are connected by bolted flange plates with the diagonal and horizontal bracing to pipe legs consisting of bolted connections. The width of the tower face is 8.54-ft at the top and 25.48-ft at the base.

T-Mobile proposes the removal of six (6) panel antennas and the installation of six (6) panel antennas and three (3) remote radio heads mounted on the existing T-Arms. Refer to the Antenna and Appurtenance Summary below for a detailed description of the proposed antenna and appurtenance configuration.

## Antenna and Appurtenance Summary

The existing and proposed loads considered in the analysis consist of the following:

- TOWN (Existing):  
Antennas: Five (5) 15-ft Omni-directional whip antennas, two (2) 9-ft Omni-directional whip antennas and one (1) 8-ft Omni-directional whip antenna mounted on two (2) 6-ft side-arms and one (1) lightweight T-Arm to the top of the the tower.  
Coax Cables: One (1) 1-5/8"  $\varnothing$  and seven (7) 7/8"  $\varnothing$  coax cables (face mounted).
- AT&T (Existing):  
Antennas: Six (6) Powerwave 7770 panel antennas, two (2) Andrew SBNH-1D6565C panel antennas, one (1) KMW AM-X-CD-17-65-00T panel antennas, six (6) Powerwave LGP21401 TMA's, six (6) Powerwave LGP13519 Diplexers, six (6) Ericsson RRUS-11 remote radio heads and one (1) Raycap DC6-48-60-18-8F surge arrester mounted on three (3) 14' boom gates at a RAD center elevation of 157-ft above grade level.  
Coax Cables: Twelve (12) 1-5/8"  $\varnothing$  coax cables and one (1) 3" dia. flex conduit (face mounted).
- VERIZON (Existing/Reserved):

Antennas: Six (6) Antel LPA-80063-6CF panel antennas, six (6) Antel BXA-70063-6CF panel antennas, six (6) LPA-171063-12CF panel antennas, three (3) Alcatel-Lucent RRH-2x40-07U remote radio heads, three (3) Alcatel-Lucent RRH-2x60-PCS remote radio heads, three (3) Alcatel-Lucent RRH-2x60-AWS remote radio heads and two (2) RFS DB-T1-6Z-8AB-0Z main distribution boxes mounted to three (3) existing 14-ft boom gates with a RAD center elevation of 135-ft above existing grade.  
Coax Cables: Eighteen (18) 1-5/8" Ø coax cables and two (2) 1-5/8" Ø fiber cables running on a leg/face of the existing tower as specified in Section 3 of this report.

- **TOWN (Existing):**  
Antennas: One (1) 12-ft Omni-directional whip antenna mounted on a 6-ft side-arm with an elevation of 146-ft above grade level.  
Coax Cables: One (1) 7/8" Ø coax cable (face mounted).
- **VERIZON (Existing):**  
GPS: One (1) GPS antenna mounted on a 1-ft standoff arm with a RAD center elevation of 51-ft above grade level.  
Coax Cables: One (1) 1/2" Ø coax cable (face mounted).
- **T-Mobile (Existing to Remain):**  
Antennas: Three (3) Ericsson AIR21 B2A/B4P panel antennas and three (3) RFS ATMAA1412D-1A20 TMA's mounted to three (3) 10-ft T-Arms with a RAD center elevation of 164-ft above grade level.  
Coax Cables: Eighteen (18) 1-5/8" Ø coax cables and one (1) 1-5/8" Ø fiber cable (face mounted).
- **T-Mobile (Existing to Remove):**  
Antennas: Six (6) EMS RR90-17-02DP panel antennas mounted to three (3) 10-ft T-Arms with a RAD center elevation of 164-ft above grade level.
- **T-Mobile (Proposed):**  
Antennas: Three (3) Ericsson AIR21 B4A/B12P panel antennas, three (3) Andrew LNX-6515DS panel antennas and three (3) Ericsson RRUS-11 remote radio heads mounted to three (3) 10-ft T-Arms with a RAD center elevation of 164-ft above grade level.

*CENTEK Engineering, Inc.*  
*Structural Analysis – 180' ROHN Lattice Tower*  
*T-Mobile Antenna Upgrade – CT11041D*  
*Waterford, CT*  
*April 22, 2015*

### *Primary Assumptions Used in the Analysis*

- The tower structure's theoretical capacity not including any assessment of the condition of the tower.
- The tower carries the horizontal and vertical loads due to the weight of antennas, ice load and wind.
- Tower is properly installed and maintained.
- Tower is in plumb condition.
- Tower loading for antennas and mounts as listed in this report.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds are fabricated with ER-70S-6 electrodes.
- All members are assumed to be as specified in the original tower design documents.
- All members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
- All member protective coatings are in good condition.
- All tower members were properly designed, detailed, fabricated, installed and have been properly maintained since erection.
- Any deviation from the analyzed antenna loading will require a new analysis for verification of structural adequacy.
- All coax cables should be routed as specified in section 3 of this report.

## Analysis

The existing tower was analyzed using a comprehensive computer program entitled tnxTower. The program analyzes the tower, considering the worst case loading condition. The tower is considered as loaded by concentric forces along the tower shaft, and the model assumes that the shaft members are subjected to bending, axial, and shear forces.

The existing tower was analyzed for the controlling basic wind speed (fastest mile) with no ice and a 75% reduction of wind force with ½ inch accumulative ice to determine stresses in members as per guidelines of TIA/EIA-222-F-96 entitled "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures", the American Institute of Steel Construction (AISC) and the Manual of Steel Construction; Allowable Stress Design (ASD).

The controlling wind speed is determined by evaluating the local available wind speed data as provided in Appendix K of the CSBC<sup>1</sup> and the wind speed data available in the TIA/EIA-222-F-96 Standard. The higher of the two wind speeds is utilized in preparation of the tower analysis.

## Tower Loading

Tower loading was determined by the basic wind speed as applied to projected surface areas with modification factors per TIA/EIA-222-F, gravity loads of the tower structure and its components, and the application of ½" radial ice on the tower structure and its components.

Basic Wind Speed:	New London; v = 85 mph (fastest mile)	[Section 16 of TIA/EIA-222-F-96]
	Waterford; v = 115 mph (3 second gust) equivalent to v = 95 mph (fastest mile) <i>Appendix-K wind speed controls.</i>	[Appendix K of the 2005 CT Building Code Supplement]
Load Cases:	<u>Load Case 1</u> ; 95 mph wind speed w/ no ice plus gravity load – used in calculation of tower stresses.	[Section 2.3.16 of TIA/EIA-222-F-96]
	<u>Load Case 2</u> ; 82 mph wind speed w/ ½" radial ice plus gravity load – used in calculation of tower stresses. The 82 mph wind speed velocity represents 75% of the wind pressure generated by the 95 mph wind speed.	[Section 2.3.16 of TIA/EIA-222-F-96]
	<u>Load Case 3</u> ; Seismic – not checked	[Section 1614.5 of State Bldg. Code 2005] does not control in the design of this structure type

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<sup>1</sup> The 2005 Connecticut State Building Code as amended by the 2009 CT State Supplement. (CSBC)

## Tower Capacity

Tower stresses were calculated utilizing the structural analysis software tnxTower. Allowable stresses were determined based on Table 5 of the TIA/EIA code with a 1/3 increase per Section 3.1.1.1 of the same code.

- Calculated stresses were found to be within allowable limits. In Load Case 2, per tnxTower "Section Capacity Table", this tower was found to be at **78.3%** of its total capacity.

Tower Section	Elevation	Stress Ratio (percentage of capacity)	Result
Leg (T9)	0'-0"-20'-0"	57.1%	<b>PASS</b>
Diagonal (T7)	40'-0"-60'-0"	76.1%	<b>PASS</b>
Horizontal (T4)	100'-0"-120'-0"	78.3%	<b>PASS</b>

## Foundation and Anchors

The existing foundation information was unavailable for use in this structural analysis. Review of the foundation design consisted of a comparison of the proposed applied loads obtained from the tower design calculations; governing Load Case 2 with the original tower loading obtained from the aforementioned URS structural design report signed and sealed on November 8, 2002.

- The tower reactions developed from the governing Load Case 2 were used in the verification of the foundation:

Reactions	Vector	Proposed Base Reactions
Base	Shear	<b>83 kips</b>
	Compression	<b>82 kips</b>
	Moment	<b>8686 kip-ft</b>
Leg	Shear	<b>50 kips</b>
	Uplift	<b>365 kips</b>
	Compression	<b>420 kips</b>

- The anchor bolts were found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (percentage of capacity)	Result
Anchor Bolts	Tension	33.6%	<b>PASS</b>

CENTEK Engineering, Inc.  
Structural Analysis – 180' ROHN Lattice Tower  
T-Mobile Antenna Upgrade – CT11041D  
Waterford, CT  
April 22, 2015

- The foundation was found to be within allowable limits.

Tower Reactions	Original Tower Reactions	Proposed Reactions	Result
Leg Compression	732.9 kips	420 kips	<b>PASS</b>
Leg Uplift	621.3 kips	365 kips	<b>PASS</b>
Base Shear	141.8 kips	83 kips	<b>PASS</b>
Overturning Moment	14472.6 kip-ft	8686 kip-ft	<b>PASS</b>

### Conclusion

This analysis shows that the subject tower **is adequate** to support the proposed modified antenna configuration.

The analysis is based, in part, on the information provided to this office by T-Mobile. If the existing conditions are different than the information in this report, Centek Engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:



Timothy J. Lynn, PE  
Structural Engineer



*CENTEK Engineering, Inc.*  
*Structural Analysis – 180' ROHN Lattice Tower*  
*T-Mobile Antenna Upgrade – CT11041D*  
*Waterford, CT*  
*April 22, 2015*

*Standard Conditions for Furnishing of  
Professional Engineering Services on  
Existing Structures*

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of Centek Engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to Centek Engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the “as new” condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222
- All services performed, results obtained, and recommendations made are in accordance with generally accepted engineering principles and practices. Centek Engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

*CENTEK Engineering, Inc.*  
*Structural Analysis – 180' ROHN Lattice Tower*  
*T-Mobile Antenna Upgrade – CT11041D*  
*Waterford, CT*  
*April 22, 2015*

## GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM

tnxTower, is an integrated structural analysis and design software package for Designed specifically for the telecommunications industry, tnxTower, formerly RISATower, automates much of the tower analysis and design required by the TIA/EIA 222 Standard.

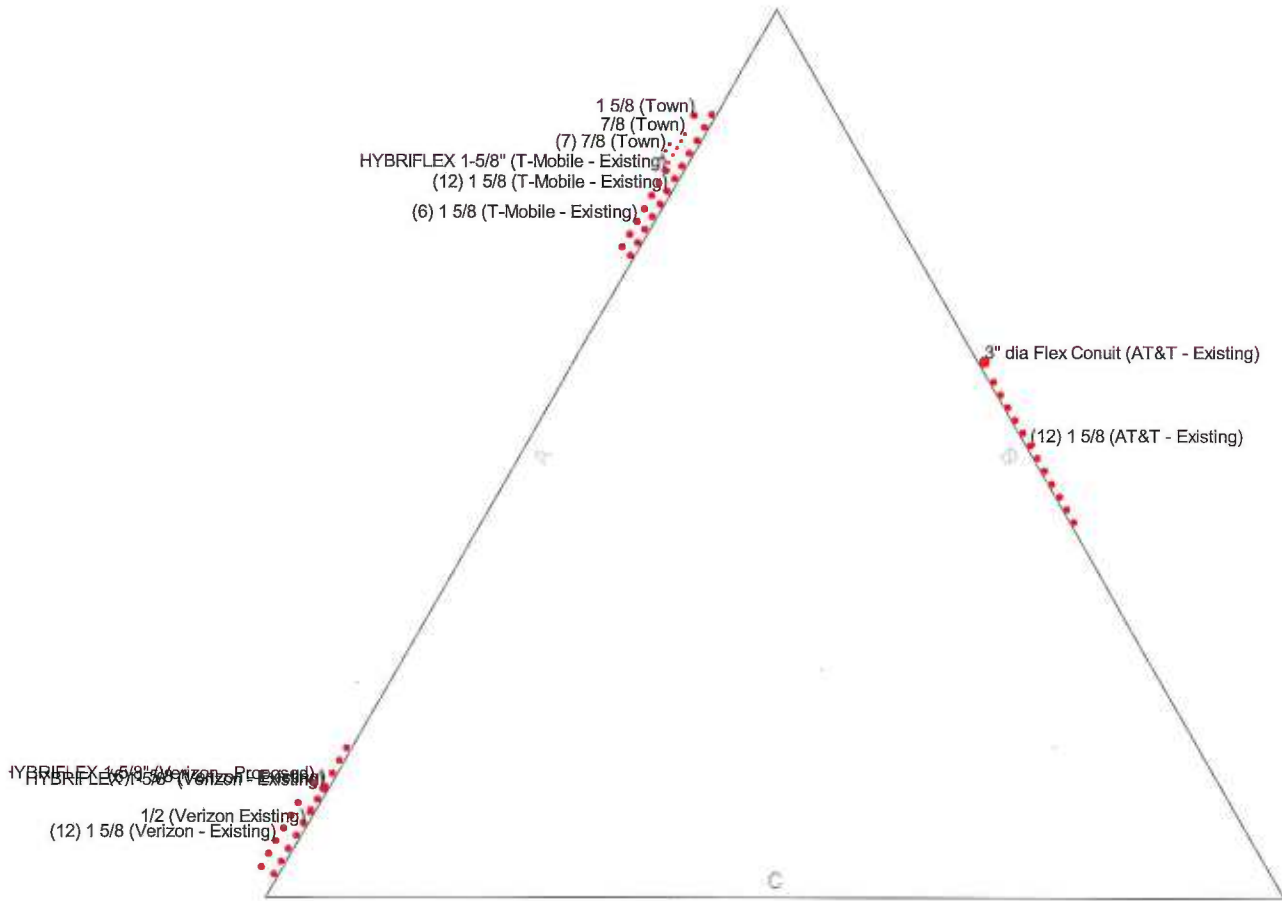
### tnxTower Features:

- tnxTower can analyze and design 3- and 4-sided guyed towers, 3- and 4-sided self-supporting towers and either round or tapered ground mounted poles with or without guys.
- The program analyzes towers using the TIA-222-G (2005) standard or any of the previous TIA/EIA standards back to RS-222 (1959). Steel design is checked using the AISC ASD 9th Edition or the AISC LRFD specifications.
- Linear and non-linear (P-delta) analyses can be used in determining displacements and forces in the structure. Wind pressures and forces are automatically calculated.
- Extensive graphics plots include material take-off, shear-moment, leg compression, displacement, twist, feed line, guy anchor and stress plots.
- tnxTower contains unique features such as True Cable behavior, hog rod take-up, foundation stiffness and much more.



# Feedline Plan

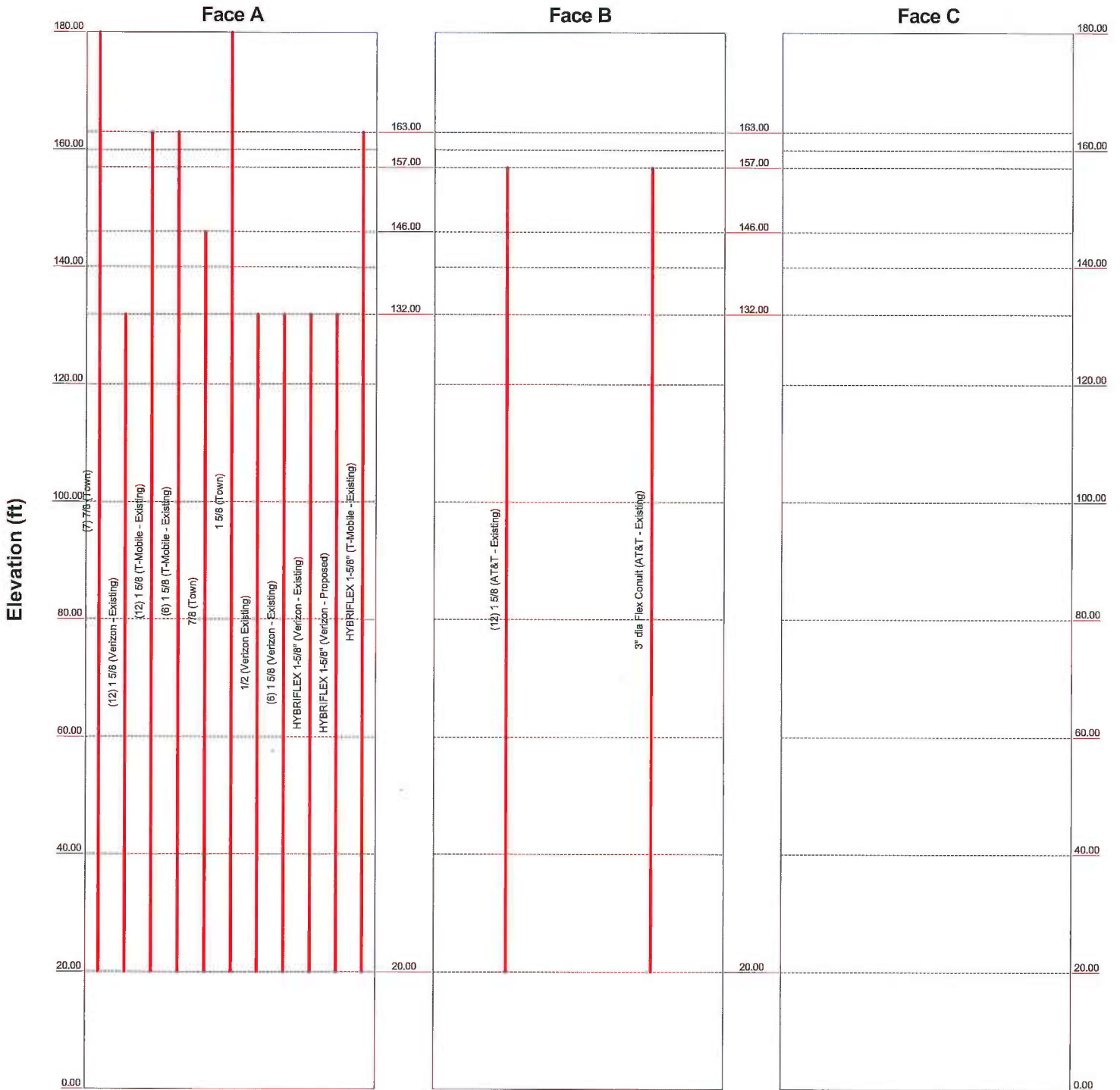
Round    
  Flat    
  App In Face    
  App Out Face



<b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587		Job: <b>15049.002 - CT11041D</b>	
		Project: <b>180' Lattice Tower - 53 Dayton Road Waterford, CT</b>	
Client: T-Mobile	Drawn by: T.JL	App'd:	
Code: TIA/EIA-222-F	Date: 04/22/15	Scale: NTS	
Path:	Dwg No. E-7		

# Feedline Distribution Chart 0' - 180'

— Round   
 — Flat   
 — App In Face   
 — App Out Face   
 — Truss Leg



<b>Centek Engineering Inc.</b>		<b>Job: 15049.002 - CT11041D</b>	
63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587		Project: <b>180' Lattice Tower - 53 Dayton Road Waterford, CT</b>	
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Path:		Dwg No. E-7	

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 1 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

## Tower Input Data

The main tower is a 3x free standing tower with an overall height of 180.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 8.54 ft at the top and 25.48 ft at the base.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Basic wind speed of 95 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 82 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

Weld together tower sections have flange connections..

Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications..

Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards..

Welds are fabricated with ER-70S-6 electrodes..

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

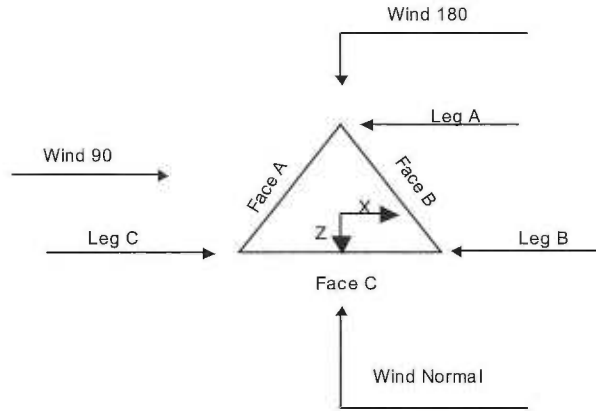
Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

## Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Treat Feedline Bundles As Cylinder
Consider Moments - Horizontals	Assume Legs Pinned	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Diagonals	√ Assume Rigid Index Plate	√ Calculate Redundant Bracing Forces
Use Moment Magnification	√ Use Clear Spans For Wind Area	Ignore Redundant Members in FEA
√ Use Code Stress Ratios	√ Use Clear Spans For KL/r	SR Leg Bolts Resist Compression
√ Use Code Safety Factors - Guys	√ Retension Guys To Initial Tension	√ All Leg Panels Have Same Allowable
Escalate Ice	Bypass Mast Stability Checks	Offset Girt At Foundation
Always Use Max Kz	√ Use Azimuth Dish Coefficients	√ Consider Feedline Torque
Use Special Wind Profile	√ Project Wind Area of Appurt.	Include Angle Block Shear Check
√ Include Bolts In Member Capacity	√ Autocalc Torque Arm Areas	Poles
Leg Bolts Are At Top Of Section	SR Members Have Cut Ends	Include Shear-Torsion Interaction
√ Secondary Horizontal Braces Leg	√ Sort Capacity Reports By Component	Always Use Sub-Critical Flow
Use Diamond Inner Bracing (4 Sided)	√ Triangulate Diamond Inner Bracing	Use Top Mounted Sockets
Add IBC .6D+W Combination		

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 2 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> T.J.L.



Triangular Tower

**Tower Section Geometry**

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	180.00-160.00			8.54	1	20.00
T2	160.00-140.00			8.63	1	20.00
T3	140.00-120.00			8.71	1	20.00
T4	120.00-100.00			10.79	1	20.00
T5	100.00-80.00			12.93	1	20.00
T6	80.00-60.00			15.33	1	20.00
T7	60.00-40.00			17.83	1	20.00
T8	40.00-20.00			20.48	1	20.00
T9	20.00-0.00			22.98	1	20.00

**Tower Section Geometry (cont'd)**

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	180.00-160.00	6.67	K Brace Down	No	Yes	0.0000	0.0000
T2	160.00-140.00	6.67	K Brace Down	No	Yes	0.0000	0.0000
T3	140.00-120.00	6.67	K Brace Down	No	Yes	0.0000	0.0000
T4	120.00-100.00	6.67	K Brace Down	No	Yes	0.0000	0.0000
T5	100.00-80.00	10.00	K Brace Down	No	Yes	0.0000	0.0000
T6	80.00-60.00	10.00	K Brace Down	No	Yes	0.0000	0.0000

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	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Tower Section	Tower Elevation ft	Diagonal Spacing ft	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset in	Bottom Girt Offset in
T7	60.00-40.00	10.00	K Brace Down	No	Yes	0.0000	0.0000
T8	40.00-20.00	10.00	K Brace Down	No	Yes	0.0000	0.0000
T9	20.00-0.00	10.00	K Brace Down	No	Yes	0.0000	0.0000

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 180.00-160.00	Pipe	ROHN 3 STD	A572-50 (50 ksi)	Pipe	ROHN 2 STD	A36M-42 (42 ksi)
T2 160.00-140.00	Pipe	ROHN 4 STD	A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A36M-42 (42 ksi)
T3 140.00-120.00	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A36M-42 (42 ksi)
T4 120.00-100.00	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Pipe	ROHN 2.5 STD	A36M-42 (42 ksi)
T5 100.00-80.00	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Pipe	ROHN 3 EH	A36M-42 (42 ksi)
T6 80.00-60.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Pipe	ROHN 3 EH	A36M-42 (42 ksi)
T7 60.00-40.00	Pipe	ROHN 10 EH	A572-50 (50 ksi)	Pipe	ROHN 3 EH	A36M-42 (42 ksi)
T8 40.00-20.00	Pipe	ROHN 12 EH	A572-50 (50 ksi)	Pipe	ROHN 3.5 EH	A36M-42 (42 ksi)
T9 20.00-0.00	Pipe	ROHN 12 EH	A572-50 (50 ksi)	Pipe	ROHN 3.5 EH	A36M-42 (42 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 180.00-160.00	Pipe	ROHN 1.5 STD	A36M-42 (42 ksi)	Solid Round		A36 (36 ksi)
T2 160.00-140.00	Pipe	ROHN 2 STD	A36M-42 (42 ksi)	Solid Round		A36 (36 ksi)
T3 140.00-120.00	Pipe	ROHN 2 STD	A36M-42 (42 ksi)	Solid Round		A36 (36 ksi)
T4 120.00-100.00	Pipe	ROHN 2 STD	A36M-42 (42 ksi)	Solid Round		A36 (36 ksi)
T5 100.00-80.00	Pipe	ROHN 2 X-STR	A36M-42 (42 ksi)	Solid Round		A36 (36 ksi)
T6 80.00-60.00	Pipe	ROHN 2.5 STD	A36M-42 (42 ksi)	Solid Round		A36 (36 ksi)
T7 60.00-40.00	Pipe	ROHN 2.5 STD	A36M-42 (42 ksi)	Solid Round		A36 (36 ksi)
T8 40.00-20.00	Pipe	ROHN 3 STD	A36M-42 (42 ksi)	Solid Round		A36 (36 ksi)
T9 20.00-0.00	Pipe	ROHN 3 STD	A36M-42 (42 ksi)	Solid Round		A36 (36 ksi)

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### Tower Section Geometry (cont'd)

Tower Elevation <i>ft</i>	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T1 180.00-160.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 1.5 STD	A572-50 (50 ksi)
T2 160.00-140.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T3 140.00-120.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T4 120.00-100.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 2 STD	A572-50 (50 ksi)
T5 100.00-80.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 2 X-STR	A572-50 (50 ksi)
T6 80.00-60.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 2.5 STD	A36 (36 ksi)
T7 60.00-40.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 2.5 STD	A36 (36 ksi)
T8 40.00-20.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3 STD	A36 (36 ksi)
T9 20.00-0.00	None	Flat Bar		A36 (36 ksi)	Pipe	ROHN 3 STD	A36 (36 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation <i>ft</i>	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T1 180.00-160.00	Solid Round		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A572-50 (50 ksi)
T2 160.00-140.00	Solid Round		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A572-50 (50 ksi)
T3 140.00-120.00	Solid Round		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A572-50 (50 ksi)
T4 120.00-100.00	Solid Round		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A572-50 (50 ksi)
T5 100.00-80.00	Solid Round		A572-50 (50 ksi)	Equal Angle	L2x2x1/8	A572-50 (50 ksi)
T6 80.00-60.00	Solid Round		A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A572-50 (50 ksi)
T7 60.00-40.00	Solid Round		A572-50 (50 ksi)	Equal Angle	L3x3x3/16	A572-50 (50 ksi)
T8 40.00-20.00	Solid Round		A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T9 20.00-0.00	Solid Round		A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)

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### Tower Section Geometry (cont'd)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft <sup>2</sup>	in					in	in
T1 180.00-160.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T2 160.00-140.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T3 140.00-120.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T4 120.00-100.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T5 100.00-80.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T6 80.00-60.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T7 60.00-40.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T8 40.00-20.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T9 20.00-0.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000

### Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	K Factors <sup>1</sup>								
			Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
				X Y	X Y	X Y	X Y	X Y	X Y	X Y	X Y
T1 180.00-160.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T2 160.00-140.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T3 140.00-120.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T4 120.00-100.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T5 100.00-80.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T6 80.00-60.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T7 60.00-40.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T8 40.00-20.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T9 20.00-0.00	Yes	Yes	1	1	1	1	1	1	1	1	1

<sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

### Tower Section Geometry (cont'd)

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Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 180.00-160.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 160.00-140.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3 140.00-120.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T4 120.00-100.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 100.00-80.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 80.00-60.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 60.00-40.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 40.00-20.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 20.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg Bolt Size in	Leg No.	Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
				Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 180.00-160.00	Flange	0.8750	4	0.6250	3	0.6250	2	0.3750	0	0.6250	0	0.6250	2	0.6250	0
T2 160.00-140.00	Flange	1.0000	4	0.6250	3	0.6250	2	0.3750	0	0.6250	0	0.6250	2	0.6250	0
T3 140.00-120.00	Flange	1.0000	6	0.6250	3	0.6250	2	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T4 120.00-100.00	Flange	1.0000	8	0.6250	3	0.6250	2	0.6250	0	0.6250	0	0.6250	2	0.6250	0
T5 100.00-80.00	Flange	1.0000	12	0.7500	3	0.7500	2	0.6250	0	0.6250	0	0.7500	2	0.6250	0
T6 80.00-60.00	Flange	1.0000	12	0.7500	3	0.7500	2	0.6250	0	0.6250	0	0.7500	2	0.6250	0
T7 60.00-40.00	Flange	1.0000	16	0.7500	3	0.7500	2	0.6250	0	0.6250	0	0.7500	2	0.6250	0
T8 40.00-20.00	Flange	1.0000	16	0.7500	3	0.7500	2	0.6250	0	0.6250	0	0.7500	2	0.6250	0
T9 20.00-0.00	Flange	1.0000	24	0.7500	3	0.7500	2	0.6250	0	0.6250	0	0.7500	2	0.6250	0
		A354-BC		A325N		A325N		A325N		A325N		A325N		A325N	

### Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
1 5/8	B	Yes	Ar (CfAe)	157.00 - 20.00	0.0000	0	12	12	1.9800	1.9800		1.04



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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
(AT&T - Existing) 7/8 (Town)	A	Yes	Ar (CfAe)	180.00 - 20.00	4.0000	0.33	7	4	1.1100	1.1100		0.54
1 5/8 (Verizon - Existing)	A	Yes	Ar (CfAe)	132.00 - 20.00	0.0000	-0.44	12	6	1.9800	1.9800		1.04
1 5/8 (T-Mobile - Existing)	A	Yes	Ar (CfAe)	163.00 - 20.00	0.0000	0.3	12	12	1.9800	1.9800		1.04
1 5/8 (T-Mobile - Existing)	A	Yes	Ar (CfAe)	163.00 - 20.00	3.0000	0.26	6	6	1.9800	1.9800		1.04
7/8 (Town)	A	Yes	Ar (CfAe)	146.00 - 20.00	4.0000	0.35	1	1	1.1100	1.1100		0.54
1 5/8 (Town)	A	Yes	Ar (CfAe)	180.00 - 20.00	4.0000	0.37	1	1	1.9800	1.9800		1.04
1/2 (Verizon Existing)	A	Yes	Ar (CfAe)	132.00 - 20.00	0.0000	-0.42	1	1	0.5800	0.5800		0.25
1 5/8 (Verizon - Existing)	A	Yes	Ar (CfAe)	132.00 - 20.00	0.0000	-0.37	6	6	1.9800	1.9800		1.04
HYBRIFLEX 1-5/8" (Verizon - Existing)	A	Yes	Ar (CfAe)	132.00 - 20.00	0.0000	-0.38	1	1	0.0000	1.9800		1.90
HYBRIFLEX 1-5/8" (Verizon - Proposed)	A	Yes	Ar (CfAe)	132.00 - 20.00	3.0000	-0.38	1	1	0.0000	1.9800		1.90
3" dia Flex Conuit (AT&T - Existing)	B	Yes	Ar (CfAe)	157.00 - 20.00	0.0000	-0.1	1	1	3.0000	3.0000		5.00
HYBRIFLEX 1-5/8" (T-Mobile - Existing)	A	Yes	Ar (CfAe)	163.00 - 20.00	3.0000	0.31	1	1	1.9800	1.9800		1.90

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
T1	180.00-160.00	A	20.105	0.000	0.000	0.000	0.16
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00
T2	160.00-140.00	A	73.955	0.000	0.000	0.000	0.51
		B	37.910	0.000	0.000	0.000	0.30
		C	0.000	0.000	0.000	0.000	0.00
T3	140.00-120.00	A	103.550	0.000	0.000	0.000	0.79
		B	44.600	0.000	0.000	0.000	0.35
		C	0.000	0.000	0.000	0.000	0.00
T4	120.00-100.00	A	122.417	0.000	0.000	0.000	0.97
		B	44.600	0.000	0.000	0.000	0.35
		C	0.000	0.000	0.000	0.000	0.00

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Tower Section	Tower Elevation ft	Face	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_{AA}$ In Face ft <sup>2</sup>	$C_{AA}$ Out Face ft <sup>2</sup>	Weight K
T5	100.00-80.00	A	122.417	0.000	0.000	0.000	0.97
		B	44.600	0.000	0.000	0.000	0.35
		C	0.000	0.000	0.000	0.000	0.00
T6	80.00-60.00	A	122.417	0.000	0.000	0.000	0.97
		B	44.600	0.000	0.000	0.000	0.35
		C	0.000	0.000	0.000	0.000	0.00
T7	60.00-40.00	A	122.417	0.000	0.000	0.000	0.97
		B	44.600	0.000	0.000	0.000	0.35
		C	0.000	0.000	0.000	0.000	0.00
T8	40.00-20.00	A	122.417	0.000	0.000	0.000	0.97
		B	44.600	0.000	0.000	0.000	0.35
		C	0.000	0.000	0.000	0.000	0.00
T9	20.00-0.00	A	0.000	66.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	$A_R$ ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$C_{AA}$ In Face ft <sup>2</sup>	$C_{AA}$ Out Face ft <sup>2</sup>	Weight K
T1	180.00-160.00	A	0.500	33.188	0.000	0.000	0.000	0.41
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.00
T2	160.00-140.00	A	0.500	114.455	0.000	0.000	0.000	1.26
		B		56.327	0.000	0.000	0.000	0.64
		C		0.000	0.000	0.000	0.000	0.00
T3	140.00-120.00	A	0.500	160.217	0.000	0.000	0.000	1.93
		B		66.267	0.000	0.000	0.000	0.76
		C		0.000	0.000	0.000	0.000	0.00
T4	120.00-100.00	A	0.500	189.083	0.000	0.000	0.000	2.36
		B		66.267	0.000	0.000	0.000	0.76
		C		0.000	0.000	0.000	0.000	0.00
T5	100.00-80.00	A	0.500	189.083	0.000	0.000	0.000	2.36
		B		66.267	0.000	0.000	0.000	0.76
		C		0.000	0.000	0.000	0.000	0.00
T6	80.00-60.00	A	0.500	189.083	0.000	0.000	0.000	2.36
		B		66.267	0.000	0.000	0.000	0.76
		C		0.000	0.000	0.000	0.000	0.00
T7	60.00-40.00	A	0.500	189.083	0.000	0.000	0.000	2.36
		B		66.267	0.000	0.000	0.000	0.76
		C		0.000	0.000	0.000	0.000	0.00
T8	40.00-20.00	A	0.500	189.083	0.000	0.000	0.000	2.36
		B		66.267	0.000	0.000	0.000	0.76
		C		0.000	0.000	0.000	0.000	0.00
T9	20.00-0.00	A	0.500	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.00

### Feed Line Shielding

Section	Elevation ft	Face	$A_R$ ft <sup>2</sup>	$A_R$ Ice ft <sup>2</sup>	$A_F$ ft <sup>2</sup>	$A_F$ Ice ft <sup>2</sup>
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<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 9 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section	Elevation	Face	$A_R$	$A_R$ Ice	$A_F$	$A_F$ Ice
	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>
T1	180.00-160.00	A	1.579	3.786	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T2	160.00-140.00	A	7.066	14.987	0.000	0.000
		B	3.622	7.376	0.000	0.000
		C	0.000	0.000	0.000	0.000
T3	140.00-120.00	A	9.250	19.638	0.000	0.000
		B	3.984	8.122	0.000	0.000
		C	0.000	0.000	0.000	0.000
T4	120.00-100.00	A	10.154	21.551	0.000	0.000
		B	3.700	7.553	0.000	0.000
		C	0.000	0.000	0.000	0.000
T5	100.00-80.00	A	8.447	17.281	0.000	0.000
		B	3.077	6.056	0.000	0.000
		C	0.000	0.000	0.000	0.000
T6	80.00-60.00	A	8.410	16.983	0.000	0.000
		B	3.064	5.952	0.000	0.000
		C	0.000	0.000	0.000	0.000
T7	60.00-40.00	A	8.007	16.183	0.000	0.000
		B	2.917	5.672	0.000	0.000
		C	0.000	0.000	0.000	0.000
T8	40.00-20.00	A	9.047	17.664	0.000	0.000
		B	3.296	6.191	0.000	0.000
		C	0.000	0.000	0.000	0.000
T9	20.00-0.00	A	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000

### Feed Line Center of Pressure

Section	Elevation	$CP_x$	$CP_z$	$CP_x$ Ice	$CP_z$ Ice
	ft	in	in	in	in
T1	180.00-160.00	-2.5641	-9.0339	-2.8894	-10.1605
T2	160.00-140.00	0.2265	-17.7741	0.0454	-18.9667
T3	140.00-120.00	-5.1188	-14.3121	-5.6634	-15.4000
T4	120.00-100.00	-9.2095	-13.5484	-10.1032	-14.6998
T5	100.00-80.00	-10.1805	-15.0628	-11.4994	-16.8303
T6	80.00-60.00	-10.7455	-15.9705	-12.3881	-18.2151
T7	60.00-40.00	-12.0924	-18.0360	-13.9452	-20.5794
T8	40.00-20.00	-12.1221	-18.1296	-14.3039	-21.1680
T9	20.00-0.00	0.0000	0.0000	0.0000	0.0000

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	$C_{AA}$ Front	$C_{AA}$ Side	Weight
			ft ft ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 10 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	CAA		Weight	
			Horz	Lateral Vert			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
15' 2.5" Dia omni (Town)	C	From Leg	3.00	0.00	0.0000	177.00	No Ice	3.75	3.75	0.05
			0.00	0.00			1/2" Ice	5.28	5.28	0.08
			7.50							
15' 2.5" Dia omni (Town)	C	From Leg	3.00	0.00	0.0000	177.00	No Ice	3.75	3.75	0.05
			0.00	0.00			1/2" Ice	5.28	5.28	0.08
			7.50							
Filter (Town)	C	From Leg	3.00	0.00	0.0000	177.00	No Ice	0.44	0.44	0.01
			0.00	0.00			1/2" Ice	0.55	0.55	0.02
			0.00							
15' 2.5" Dia omni (Town)	C	From Leg	0.00	0.00	0.0000	180.00	No Ice	3.75	3.75	0.05
			0.00	0.00			1/2" Ice	5.28	5.28	0.08
			7.50							
15' 2.5" Dia omni (Town)	A	From Leg	0.00	0.00	0.0000	180.00	No Ice	3.75	3.75	0.05
			0.00	0.00			1/2" Ice	5.28	5.28	0.08
			7.50							
9-ft Omni (Town)	A	From Leg	6.00	0.00	0.0000	176.00	No Ice	2.25	2.25	0.03
			0.00	0.00			1/2" Ice	3.18	3.18	0.05
			4.50							
8' x 3" Dia Omni (Town)	A	From Leg	6.00	0.00	0.0000	176.00	No Ice	2.40	2.40	0.03
			0.00	-4.00			1/2" Ice	3.19	3.19	0.04
15' 2.5" Dia omni (Town)	B	From Leg	0.00	0.00	0.0000	180.00	No Ice	3.75	3.75	0.05
			0.00	0.00			1/2" Ice	5.28	5.28	0.08
			7.50							
9-ft Omni (Town)	B	From Leg	6.00	0.00	0.0000	176.00	No Ice	2.25	2.25	0.03
			0.00	0.00			1/2" Ice	3.18	3.18	0.05
			4.50							
Rohn 6' Side-Arm(1) (Town)	A	From Leg	3.00	0.00	0.0000	178.00	No Ice	10.60	10.60	0.14
			0.00	0.00			1/2" Ice	15.40	15.40	0.21
			0.00							
Rohn 6' Side-Arm(1) (Town)	B	From Leg	3.00	0.00	0.0000	178.00	No Ice	10.60	10.60	0.14
			0.00	0.00			1/2" Ice	15.40	15.40	0.21
			0.00							
Pirod 15' T-Frame Sector Mount (1) (Town)	C	From Leg	6.00	0.00	0.0000	177.00	No Ice	15.00	15.00	0.50
			0.00	0.00			1/2" Ice	20.60	20.60	0.65
			0.00							
AIR21 B4A B12P B5P (T-Mobile - Proposed)	A	From Leg	3.00	-6.00	0.0000	164.00	No Ice	11.50	8.90	0.15
			0.00	0.00			1/2" Ice	12.12	9.49	0.22
AIR21 B2A/B4P (T-Mobile - Existing)	A	From Leg	3.00	-2.00	0.0000	164.00	No Ice	6.53	4.36	0.08
			0.00	0.00			1/2" Ice	6.98	4.77	0.12
LNX-6515DS (T-Mobile - Proposed)	A	From Leg	3.00	2.00	0.0000	164.00	No Ice	11.45	7.70	0.06
			0.00	0.00			1/2" Ice	12.06	8.29	0.12
AIR21 B4A B12P B5P (T-Mobile - Proposed)	B	From Leg	3.00	-6.00	0.0000	164.00	No Ice	11.50	8.90	0.15
			0.00	0.00			1/2" Ice	12.12	9.49	0.22
AIR21 B2A/B4P (T-Mobile - Existing)	B	From Leg	3.00	-2.00	0.0000	164.00	No Ice	6.53	4.36	0.08
			0.00	0.00			1/2" Ice	6.98	4.77	0.12
LNX-6515DS (T-Mobile - Proposed)	B	From Leg	3.00	2.00	0.0000	164.00	No Ice	11.45	7.70	0.06
			0.00	0.00			1/2" Ice	12.06	8.29	0.12
AIR21 B4A B12P B5P (T-Mobile - Proposed)	C	From Leg	3.00	-6.00	0.0000	164.00	No Ice	11.50	8.90	0.15
			0.00	0.00			1/2" Ice	12.12	9.49	0.22

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>		15049.002 - CT11041D		<b>Page</b>		11 of 43	
	<b>Project</b>		180' Lattice Tower - 53 Dayton Road Waterford, CT		<b>Date</b>		16:53:44 04/22/15	
	<b>Client</b>		T-Mobile		<b>Designed by</b>		TJL	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub>		Weight	
			Horz	Lateral Vert			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
AIR21 B2A/B4P (T-Mobile - Existing)	C	From Leg	3.00 -2.00 0.00		0.0000	164.00	No Ice 1/2" Ice	6.53 6.98	4.36 4.77	0.08 0.12
LNx-6515DS (T-Mobile - Proposed)	C	From Leg	3.00 2.00 0.00		0.0000	164.00	No Ice 1/2" Ice	11.45 12.06	7.70 8.29	0.06 0.12
ATMAA1412D-1A20 TMA (T-Mobile - Existing)	A	From Leg	3.00 -2.00 0.00		0.0000	164.00	No Ice 1/2" Ice	1.17 1.31	0.47 0.57	0.01 0.02
ATMAA1412D-1A20 TMA (T-Mobile - Existing)	B	From Leg	3.00 -2.00 0.00		0.0000	164.00	No Ice 1/2" Ice	1.17 1.31	0.47 0.57	0.01 0.02
ATMAA1412D-1A20 TMA (T-Mobile - Existing)	C	From Leg	3.00 -2.00 0.00		0.0000	164.00	No Ice 1/2" Ice	1.17 1.31	0.47 0.57	0.01 0.02
RRUS-11 (T-Mobile - Proposed)	A	From Leg	3.00 2.00 0.00		0.0000	164.00	No Ice 1/2" Ice	2.99 3.23	1.25 1.41	0.05 0.07
RRUS-11 (T-Mobile - Proposed)	B	From Leg	3.00 2.00 0.00		0.0000	164.00	No Ice 1/2" Ice	2.99 3.23	1.25 1.41	0.05 0.07
RRUS-11 (T-Mobile - Proposed)	C	From Leg	3.00 2.00 0.00		0.0000	164.00	No Ice 1/2" Ice	2.99 3.23	1.25 1.41	0.05 0.07
Valmont 10' Wireless Frame (3) (T-Mobile - Existing)	A	From Leg	3.00 0.00		0.0000	164.00	No Ice 1/2" Ice	30.70 42.00	30.70 42.00	0.71 0.86
(2) 7770.00 (AT&T - Existing)	A	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
(2) 7770.00 (AT&T - Existing)	B	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
(2) 7770.00 (AT&T - Existing)	C	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	5.88 6.31	2.93 3.27	0.04 0.07
(2) LGP21401 TMA (AT&T - Existing)	A	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	0.95 1.09	0.37 0.48	0.02 0.02
(2) LGP21401 TMA (AT&T - Existing)	B	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	0.95 1.09	0.37 0.48	0.02 0.02
(2) LGP21401 TMA (AT&T - Existing)	C	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	0.95 1.09	0.37 0.48	0.02 0.02
(2) LGP13519 Diplexer (AT&T - Existing)	A	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	0.27 0.34	0.18 0.25	0.01 0.01
(2) LGP13519 Diplexer (AT&T - Existing)	B	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	0.27 0.34	0.18 0.25	0.01 0.01
(2) LGP13519 Diplexer (AT&T - Existing)	C	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	0.27 0.34	0.18 0.25	0.01 0.01
SBNH-1D6565C (AT&T - Existing)	A	From Leg	6.00 0.00		0.0000	157.00	No Ice 1/2" Ice	11.41 12.03	7.70 8.29	0.06 0.13

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	<b>Project</b>		180' Lattice Tower - 53 Dayton Road Waterford, CT		<b>Date</b>		16:53:44 04/22/15	
	<b>Client</b>		T-Mobile		<b>Designed by</b>		TJL	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight	
			Horz	Vert			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
SBNH-1D6565C (AT&T - Existing)	B	From Leg	6.00	0.00	0.0000	157.00	No Ice 1/2" Ice	11.41 12.03	7.70 8.29	0.06 0.13
AM-X-CD-17-65-00T-RET (AT&T - Existing)	C	From Leg	6.00	0.00	0.0000	157.00	No Ice 1/2" Ice	11.31 11.93	6.80 7.38	0.06 0.12
(2) RRUS-11 (AT&T - Existing)	A	From Leg	6.00	0.00	0.0000	157.00	No Ice 1/2" Ice	2.99 3.23	1.25 1.41	0.05 0.07
(2) RRUS-11 (AT&T - Existing)	B	From Leg	6.00	0.00	0.0000	157.00	No Ice 1/2" Ice	2.99 3.23	1.25 1.41	0.05 0.07
(2) RRUS-11 (AT&T - Existing)	C	From Leg	6.00	0.00	0.0000	157.00	No Ice 1/2" Ice	2.99 3.23	1.25 1.41	0.05 0.07
DC6-48-60-18-8F Surge Arrestor (AT&T - Existing)	A	From Leg	6.00	0.00	0.0000	157.00	No Ice 1/2" Ice	2.23 2.45	2.23 2.45	0.02 0.04
Rohn 6'x14' Boom Gate (3) (AT&T - Existing)	A	From Leg	3.00	0.00	0.0000	157.00	No Ice 1/2" Ice	52.00 61.90	52.00 61.90	1.75 2.19
12' x 2" Dia Omni (Town)	C	From Leg	6.00	0.00	0.0000	146.00	No Ice 1/2" Ice	2.40 3.63	2.40 3.63	0.03 0.05
Rohn 6' Side-Arm(1) (Town)	C	From Leg	3.00	0.00	0.0000	142.00	No Ice 1/2" Ice	10.60 15.40	10.60 15.40	0.14 0.21
Rohn 6'x14' Boom Gate (3) (Verizon - Existing)	A	From Leg	3.00	0.00	0.0000	135.00	No Ice 1/2" Ice	52.00 61.90	52.00 61.90	1.75 2.19
1' Standoff Pipe (Verizon - Existing)	C	From Leg	0.50	0.00	0.0000	50.00	No Ice 1/2" Ice	0.16 0.23	0.16 0.23	0.01 0.01
GPS (Verizon - Existing)	C	From Leg	1.00	0.00	0.0000	51.00	No Ice 1/2" Ice	1.00 1.50	1.00 1.50	0.01 0.01
LPA-80063-6CF (Verizon - Reserved)	A	From Leg	5.00	-6.00	0.0000	135.00	No Ice 1/2" Ice	10.31 10.87	9.01 9.55	0.03 0.10
LPA-171063-12CF (Verizon - Reserved)	A	From Leg	5.00	-4.00	0.0000	135.00	No Ice 1/2" Ice	5.99 6.46	6.05 6.52	0.01 0.06
BXA-70063/6CF (Verizon - Reserved)	A	From Leg	5.00	-2.00	0.0000	135.00	No Ice 1/2" Ice	7.73 8.27	4.16 4.60	0.01 0.05
BXA-70063/6CF (Verizon - Reserved)	A	From Leg	5.00	2.00	0.0000	135.00	No Ice 1/2" Ice	7.73 8.27	4.16 4.60	0.01 0.05
LPA-171063-12CF (Verizon - Reserved)	A	From Leg	5.00	4.00	0.0000	135.00	No Ice 1/2" Ice	5.99 6.46	6.05 6.52	0.01 0.06
LPA-80063-6CF (Verizon - Reserved)	A	From Leg	5.00	6.00	0.0000	135.00	No Ice 1/2" Ice	10.31 10.87	9.01 9.55	0.03 0.10
LPA-80063-6CF (Verizon - Reserved)	B	From Leg	5.00	-6.00	0.0000	135.00	No Ice 1/2" Ice	10.31 10.87	9.01 9.55	0.03 0.10

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	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight	
			Horz	Lateral			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
LPA-171063-12CF (Verizon - Reserved)	B	From Leg	5.00 -4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	5.99 6.46	6.05 6.52	0.01 0.06
BXA-70063/6CF (Verizon - Reserved)	B	From Leg	5.00 -2.00 0.00		0.0000	135.00	No Ice 1/2" Ice	7.73 8.27	4.16 4.60	0.01 0.05
BXA-70063/6CF (Verizon - Reserved)	B	From Leg	5.00 2.00 0.00		0.0000	135.00	No Ice 1/2" Ice	7.73 8.27	4.16 4.60	0.01 0.05
LPA-171063-12CF (Verizon - Reserved)	B	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	5.99 6.46	6.05 6.52	0.01 0.06
LPA-80063-6CF (Verizon - Reserved)	B	From Leg	5.00 6.00 0.00		0.0000	135.00	No Ice 1/2" Ice	10.31 10.87	9.01 9.55	0.03 0.10
LPA-80063-6CF (Verizon - Reserved)	C	From Leg	5.00 -6.00 0.00		0.0000	135.00	No Ice 1/2" Ice	10.31 10.87	9.01 9.55	0.03 0.10
LPA-171063-12CF (Verizon - Reserved)	C	From Leg	5.00 -4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	5.99 6.46	6.05 6.52	0.01 0.06
BXA-70063/6CF (Verizon - Reserved)	C	From Leg	5.00 -2.00 0.00		0.0000	135.00	No Ice 1/2" Ice	7.73 8.27	4.16 4.60	0.01 0.05
BXA-70063/6CF (Verizon - Reserved)	C	From Leg	5.00 2.00 0.00		0.0000	135.00	No Ice 1/2" Ice	7.73 8.27	4.16 4.60	0.01 0.05
LPA-171063-12CF (Verizon - Reserved)	C	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	5.99 6.46	6.05 6.52	0.01 0.06
LPA-80063-6CF (Verizon - Reserved)	C	From Leg	5.00 6.00 0.00		0.0000	135.00	No Ice 1/2" Ice	10.31 10.87	9.01 9.55	0.03 0.10
RRH2x40-07-U (Verizon - Reserved)	A	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	2.25 2.45	1.23 1.39	0.05 0.07
RRH2x40-07-U (Verizon - Reserved)	B	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	2.25 2.45	1.23 1.39	0.05 0.07
RRH2x40-07-U (Verizon - Reserved)	C	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	2.25 2.45	1.23 1.39	0.05 0.07
RRH2x60-AWS (Verizon - Reserved)	A	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	3.78 4.09	2.07 2.35	0.06 0.08
RRH2x60-AWS (Verizon - Reserved)	B	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	3.78 4.09	2.07 2.35	0.06 0.08
RRH2x60-AWS (Verizon - Reserved)	C	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	3.78 4.09	2.07 2.35	0.06 0.08
RRH2x60-PCS (Verizon - Reserved)	A	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	2.51 2.73	1.55 1.74	0.06 0.07
RRH2x60-PCS (Verizon - Reserved)	B	From Leg	5.00 4.00 0.00		0.0000	135.00	No Ice 1/2" Ice	2.51 2.73	1.55 1.74	0.06 0.07

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 14 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			ft ft ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
RRH2x60-PCS (Verizon - Reserved)	C	From Leg	5.00 4.00 0.00	0.0000	135.00	No Ice 1/2" Ice	2.51 2.73	1.55 1.74	0.06 0.07
DB-T1-6Z-8AB-0Z (Verizon - Reserved)	A	From Leg	5.00 4.00 0.00	0.0000	135.00	No Ice 1/2" Ice	5.60 5.92	2.33 2.56	0.04 0.08
DB-T1-6Z-8AB-0Z (Verizon - Reserved)	B	From Leg	5.00 4.00 0.00	0.0000	135.00	No Ice 1/2" Ice	5.60 5.92	2.33 2.56	0.04 0.08

### Tower Pressures - No Ice

$$G_H = 1.121$$

Section Elevation	z	K <sub>z</sub>	q <sub>z</sub>	A <sub>G</sub>	F <sub>a</sub>	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>AA</sub> In Face	C <sub>AA</sub> Out Face
ft	ft		psf	ft <sup>2</sup>	c	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 180.00-160.00	170.00	1.597	37	177.533	A	0.000	43.226	11.667	26.99	0.000	0.000
					B	0.000	24.700		47.23	0.000	0.000
					C	0.000	24.700		47.23	0.000	0.000
T2 160.00-140.00	150.00	1.541	36	180.900	A	0.000	97.766	15.000	15.34	0.000	0.000
					B	0.000	65.164		23.02	0.000	0.000
					C	0.000	30.876		48.58	0.000	0.000
T3 140.00-120.00	130.00	1.48	34	204.284	A	0.000	129.678	18.577	14.33	0.000	0.000
					B	0.000	75.994		24.44	0.000	0.000
					C	0.000	35.378		52.51	0.000	0.000
T4 120.00-100.00	110.00	1.411	33	248.257	A	0.000	153.324	22.125	14.43	0.000	0.000
					B	0.000	81.962		26.99	0.000	0.000
					C	0.000	41.062		53.88	0.000	0.000
T5 100.00-80.00	90.00	1.332	31	297.001	A	0.000	161.683	28.819	17.82	0.000	0.000
					B	0.000	89.236		32.30	0.000	0.000
					C	0.000	47.713		60.40	0.000	0.000
T6 80.00-60.00	70.00	1.24	29	349.552	A	0.000	171.787	35.927	20.91	0.000	0.000
					B	0.000	99.317		36.17	0.000	0.000
					C	0.000	57.781		62.18	0.000	0.000
T7 60.00-40.00	50.00	1.126	26	401.056	A	0.000	174.469	35.938	20.60	0.000	0.000
					B	0.000	101.742		35.32	0.000	0.000
					C	0.000	60.059		59.84	0.000	0.000
T8 40.00-20.00	30.00	1	23	455.891	A	0.000	186.775	42.611	22.81	0.000	0.000
					B	0.000	114.709		37.15	0.000	0.000
					C	0.000	73.405		58.05	0.000	0.000
T9 20.00-0.00	10.00	1	23	505.891	A	0.000	76.126	42.611	55.97	0.000	0.000
					B	0.000	76.126		55.97	0.000	0.000
					C	0.000	76.126		55.97	0.000	0.000

### Tower Pressure - With Ice



<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 15 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJJ

$$G_H = 1.121$$

Section Elevation	z	K <sub>z</sub>	q <sub>z</sub>	t <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
ft	ft		psf	in	ft <sup>2</sup>	e	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>3</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 180.00-160.00	170.00	1.597	28	0.5000	179.200	A	0.000	63.337	15.000	23.68	0.000	0.000
						B	0.000	33.935		44.20	0.000	0.000
						C	0.000	33.935		44.20	0.000	0.000
T2 160.00-140.00	150.00	1.541	27	0.5000	182.567	A	0.000	139.561	18.333	13.14	0.000	0.000
						B	0.000	89.044		20.59	0.000	0.000
						C	0.000	40.093		45.73	0.000	0.000
T3 140.00-120.00	130.00	1.48	26	0.5000	205.953	A	0.000	185.530	21.916	11.81	0.000	0.000
						B	0.000	103.096		21.26	0.000	0.000
						C	0.000	44.951		48.76	0.000	0.000
T4 120.00-100.00	110.00	1.411	24	0.5000	249.927	A	0.000	218.998	25.465	11.63	0.000	0.000
						B	0.000	110.180		23.11	0.000	0.000
						C	0.000	51.466		49.48	0.000	0.000
T5 100.00-80.00	90.00	1.332	23	0.5000	298.671	A	0.000	228.946	32.160	14.05	0.000	0.000
						B	0.000	117.354		27.40	0.000	0.000
						C	0.000	57.144		56.28	0.000	0.000
T6 80.00-60.00	70.00	1.24	21	0.5000	351.222	A	0.000	239.918	39.269	16.37	0.000	0.000
						B	0.000	128.133		30.65	0.000	0.000
						C	0.000	67.818		57.90	0.000	0.000
T7 60.00-40.00	50.00	1.126	20	0.5000	402.726	A	0.000	243.718	39.281	16.12	0.000	0.000
						B	0.000	131.413		29.89	0.000	0.000
						C	0.000	70.818		55.47	0.000	0.000
T8 40.00-20.00	30.00	1	17	0.5000	457.561	A	0.000	256.284	45.953	17.93	0.000	0.000
						B	0.000	144.941		31.70	0.000	0.000
						C	0.000	84.865		54.15	0.000	0.000
T9 20.00-0.00	10.00	1	17	0.5000	507.561	A	0.000	88.317	45.953	52.03	0.000	0.000
						B	0.000	88.317		52.03	0.000	0.000
						C	0.000	88.317		52.03	0.000	0.000

### Tower Pressure - Service

$$G_H = 1.121$$

Section Elevation	z	K <sub>z</sub>	q <sub>z</sub>	A <sub>G</sub>	F a c e	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>
ft	ft		psf	ft <sup>2</sup>	e	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 180.00-160.00	170.00	1.597	10	177.533	A	0.000	43.226	11.667	26.99	0.000	0.000
					B	0.000	24.700		47.23	0.000	0.000
					C	0.000	24.700		47.23	0.000	0.000
T2 160.00-140.00	150.00	1.541	10	180.900	A	0.000	97.766	15.000	15.34	0.000	0.000
					B	0.000	65.164		23.02	0.000	0.000
					C	0.000	30.876		48.58	0.000	0.000
T3 140.00-120.00	130.00	1.48	9	204.284	A	0.000	129.678	18.577	14.33	0.000	0.000
					B	0.000	75.994		24.44	0.000	0.000
					C	0.000	35.378		52.51	0.000	0.000
T4 120.00-100.00	110.00	1.411	9	248.257	A	0.000	153.324	22.125	14.43	0.000	0.000
					B	0.000	81.962		26.99	0.000	0.000
					C	0.000	41.062		53.88	0.000	0.000
T5 100.00-80.00	90.00	1.332	9	297.001	A	0.000	161.683	28.819	17.82	0.000	0.000
					B	0.000	89.236		32.30	0.000	0.000
					C	0.000	47.713		60.40	0.000	0.000
T6 80.00-60.00	70.00	1.24	8	349.552	A	0.000	171.787	35.927	20.91	0.000	0.000
					B	0.000	99.317		36.17	0.000	0.000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 16 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F <sub>a c e</sub>	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>MA</sub> In Face	C <sub>MA</sub> Out Face
ft	ft		psf	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T7 60.00-40.00	50.00	1.126	7	401.056	C	0.000	57.781	35.938	62.18	0.000	0.000
					A	0.000	174.469		20.60	0.000	0.000
					B	0.000	101.742		35.32	0.000	0.000
T8 40.00-20.00	30.00	1	6	455.891	C	0.000	60.059	42.611	59.84	0.000	0.000
					A	0.000	186.775		22.81	0.000	0.000
					B	0.000	114.709		37.15	0.000	0.000
T9 20.00-0.00	10.00	1	6	505.891	C	0.000	73.405	42.611	58.05	0.000	0.000
					A	0.000	76.126		55.97	0.000	0.000
					B	0.000	76.126		55.97	0.000	0.000
					C	0.000	76.126		55.97	0.000	0.000

### Tower Forces - No Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F <sub>a c e</sub>	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.16	1.25	A	0.243	2.457	0.6	1	1	25.946	2.64	131.87	A
			B	0.139	2.812	0.58	1	1	14.323			
			C	0.139	2.812	0.58	1	1	14.323			
T2 160.00-140.00	0.81	1.83	A	0.54	1.853	0.719	1	1	70.289	5.20	259.93	A
			B	0.36	2.148	0.636	1	1	41.456			
			C	0.171	2.697	0.585	1	1	18.058			
T3 140.00-120.00	1.14	2.50	A	0.635	1.787	0.776	1	1	100.567	6.88	344.19	A
			B	0.372	2.122	0.641	1	1	48.680			
			C	0.173	2.688	0.585	1	1	20.707			
T4 120.00-100.00	1.32	3.13	A	0.618	1.794	0.765	1	1	117.221	7.68	384.12	A
			B	0.33	2.219	0.626	1	1	51.275			
			C	0.165	2.716	0.584	1	1	23.978			
T5 100.00-80.00	1.32	4.62	A	0.544	1.849	0.721	1	1	116.596	7.44	371.82	A
			B	0.3	2.295	0.616	1	1	54.973			
			C	0.161	2.733	0.583	1	1	27.825			
T6 80.00-60.00	1.32	5.62	A	0.491	1.911	0.693	1	1	119.079	7.31	365.34	A
			B	0.284	2.339	0.611	1	1	60.700			
			C	0.165	2.716	0.584	1	1	33.740			
T7 60.00-40.00	1.32	5.88	A	0.435	1.999	0.667	1	1	116.286	6.78	338.90	A
			B	0.254	2.426	0.603	1	1	61.332			
			C	0.15	2.772	0.581	1	1	34.921			
T8 40.00-20.00	1.32	7.52	A	0.41	2.045	0.656	1	1	122.450	6.49	324.26	A
			B	0.252	2.433	0.602	1	1	69.088			
			C	0.161	2.731	0.583	1	1	42.811			
T9 20.00-0.00	0.00	7.82	A	0.15	2.77	0.582	1	1	44.271	3.18	158.77	C
			B	0.15	2.77	0.582	1	1	44.271			
			C	0.15	2.77	0.582	1	1	44.271			
Sum Weight:	8.73	40.16						OTM	4714.06 kip-ft	53.58		

### Tower Forces - No Ice - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F <sub>a c e</sub>	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 17 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> T.J.L.

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.16	1.25	A	0.243	2.457	0.6	0.825	1	25.946	2.64	131.87	A
			B	0.139	2.812	0.58	0.825	1	14.323			
			C	0.139	2.812	0.58	0.825	1	14.323			
T2 160.00-140.00	0.81	1.83	A	0.54	1.853	0.719	0.825	1	70.289	5.20	259.93	A
			B	0.36	2.148	0.636	0.825	1	41.456			
			C	0.171	2.697	0.585	0.825	1	18.058			
T3 140.00-120.00	1.14	2.50	A	0.635	1.787	0.776	0.825	1	100.567	6.88	344.19	A
			B	0.372	2.122	0.641	0.825	1	48.680			
			C	0.173	2.688	0.585	0.825	1	20.707			
T4 120.00-100.00	1.32	3.13	A	0.618	1.794	0.765	0.825	1	117.221	7.68	384.12	A
			B	0.33	2.219	0.626	0.825	1	51.275			
			C	0.165	2.716	0.584	0.825	1	23.978			
T5 100.00-80.00	1.32	4.62	A	0.544	1.849	0.721	0.825	1	116.596	7.44	371.82	A
			B	0.3	2.295	0.616	0.825	1	54.973			
			C	0.161	2.733	0.583	0.825	1	27.825			
T6 80.00-60.00	1.32	5.62	A	0.491	1.911	0.693	0.825	1	119.079	7.31	365.34	A
			B	0.284	2.339	0.611	0.825	1	60.700			
			C	0.165	2.716	0.584	0.825	1	33.740			
T7 60.00-40.00	1.32	5.88	A	0.435	1.999	0.667	0.825	1	116.286	6.78	338.90	A
			B	0.254	2.426	0.603	0.825	1	61.332			
			C	0.15	2.772	0.581	0.825	1	34.921			
T8 40.00-20.00	1.32	7.52	A	0.41	2.045	0.656	0.825	1	122.450	6.49	324.26	A
			B	0.252	2.433	0.602	0.825	1	69.088			
			C	0.161	2.731	0.583	0.825	1	42.811			
T9 20.00-0.00	0.00	7.82	A	0.15	2.77	0.582	0.825	1	44.271	3.18	158.77	C
			B	0.15	2.77	0.582	0.825	1	44.271			
			C	0.15	2.77	0.582	0.825	1	44.271			
Sum Weight:	8.73	40.16						OTM	4714.06 kip-ft	53.58		

### Tower Forces - No Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.16	1.25	A	0.243	2.457	0.6	0.8	1	25.946	2.64	131.87	A
			B	0.139	2.812	0.58	0.8	1	14.323			
			C	0.139	2.812	0.58	0.8	1	14.323			
T2 160.00-140.00	0.81	1.83	A	0.54	1.853	0.719	0.8	1	70.289	5.20	259.93	A
			B	0.36	2.148	0.636	0.8	1	41.456			
			C	0.171	2.697	0.585	0.8	1	18.058			
T3 140.00-120.00	1.14	2.50	A	0.635	1.787	0.776	0.8	1	100.567	6.88	344.19	A
			B	0.372	2.122	0.641	0.8	1	48.680			
			C	0.173	2.688	0.585	0.8	1	20.707			
T4 120.00-100.00	1.32	3.13	A	0.618	1.794	0.765	0.8	1	117.221	7.68	384.12	A
			B	0.33	2.219	0.626	0.8	1	51.275			
			C	0.165	2.716	0.584	0.8	1	23.978			
T5 100.00-80.00	1.32	4.62	A	0.544	1.849	0.721	0.8	1	116.596	7.44	371.82	A
			B	0.3	2.295	0.616	0.8	1	54.973			
			C	0.161	2.733	0.583	0.8	1	27.825			
T6 80.00-60.00	1.32	5.62	A	0.491	1.911	0.693	0.8	1	119.079	7.31	365.34	A
			B	0.284	2.339	0.611	0.8	1	60.700			
			C	0.165	2.716	0.584	0.8	1	33.740			
T7 60.00-40.00	1.32	5.88	A	0.435	1.999	0.667	0.8	1	116.286	6.78	338.90	A
			B	0.254	2.426	0.603	0.8	1	61.332			

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	Job	15049.002 - CT11041D	Page	18 of 43
	Project	180' Lattice Tower - 53 Dayton Road Waterford, CT	Date	16:53:44 04/22/15
	Client	T-Mobile	Designed by	TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T8 40.00-20.00	1.32	7.52	C	0.15	2.772	0.581	0.8	1	34.921	6.49	324.26	A
			A	0.41	2.045	0.656	0.8	1	122.450			
			B	0.252	2.433	0.602	0.8	1	69.088			
T9 20.00-0.00	0.00	7.82	C	0.161	2.731	0.583	0.8	1	42.811	3.18	158.77	C
			A	0.15	2.77	0.582	0.8	1	44.271			
			B	0.15	2.77	0.582	0.8	1	44.271			
Sum Weight:	8.73	40.16	C	0.15	2.77	0.582	0.8	1	44.271	53.58		
								OTM	4714.06 kip-ft			

### Tower Forces - No Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.16	1.25	A	0.243	2.457	0.6	0.85	1	25.946	2.64	131.87	A
			B	0.139	2.812	0.58	0.85	1	14.323			
			C	0.139	2.812	0.58	0.85	1	14.323			
T2 160.00-140.00	0.81	1.83	A	0.54	1.853	0.719	0.85	1	70.289	5.20	259.93	A
			B	0.36	2.148	0.636	0.85	1	41.456			
			C	0.171	2.697	0.585	0.85	1	18.058			
T3 140.00-120.00	1.14	2.50	A	0.635	1.787	0.776	0.85	1	100.567	6.88	344.19	A
			B	0.372	2.122	0.641	0.85	1	48.680			
			C	0.173	2.688	0.585	0.85	1	20.707			
T4 120.00-100.00	1.32	3.13	A	0.618	1.794	0.765	0.85	1	117.221	7.68	384.12	A
			B	0.33	2.219	0.626	0.85	1	51.275			
			C	0.165	2.716	0.584	0.85	1	23.978			
T5 100.00-80.00	1.32	4.62	A	0.544	1.849	0.721	0.85	1	116.596	7.44	371.82	A
			B	0.3	2.295	0.616	0.85	1	54.973			
			C	0.161	2.733	0.583	0.85	1	27.825			
T6 80.00-60.00	1.32	5.62	A	0.491	1.911	0.693	0.85	1	119.079	7.31	365.34	A
			B	0.284	2.339	0.611	0.85	1	60.700			
			C	0.165	2.716	0.584	0.85	1	33.740			
T7 60.00-40.00	1.32	5.88	A	0.435	1.999	0.667	0.85	1	116.286	6.78	338.90	A
			B	0.254	2.426	0.603	0.85	1	61.332			
			C	0.15	2.772	0.581	0.85	1	34.921			
T8 40.00-20.00	1.32	7.52	A	0.41	2.045	0.656	0.85	1	122.450	6.49	324.26	A
			B	0.252	2.433	0.602	0.85	1	69.088			
			C	0.161	2.731	0.583	0.85	1	42.811			
T9 20.00-0.00	0.00	7.82	A	0.15	2.77	0.582	0.85	1	44.271	3.18	158.77	C
			B	0.15	2.77	0.582	0.85	1	44.271			
			C	0.15	2.77	0.582	0.85	1	44.271			
Sum Weight:	8.73	40.16	C	0.15	2.77	0.582	0.85	1	44.271	53.58		
								OTM	4714.06 kip-ft			

### Tower Forces - With Ice - Wind Normal To Face

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 19 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.41	1.84	A	0.353	2.164	0.634	1	1	40.137	2.69	134.71	A
			B	0.189	2.632	0.588	1	1	19.964			
			C	0.189	2.632	0.588	1	1	19.964			
T2 160.00-140.00	1.90	2.52	A	0.764	1.794	0.868	1	1	121.142	6.51	325.29	A
			B	0.488	1.916	0.691	1	1	61.558			
			C	0.22	2.532	0.595	1	1	23.839			
T3 140.00-120.00	2.68	3.27	A	0.901	1.925	0.984	1	1	182.537	10.10	504.92	A
			B	0.501	1.899	0.698	1	1	71.940			
			C	0.218	2.536	0.594	1	1	26.714			
T4 120.00-100.00	3.11	4.01	A	0.876	1.892	0.962	1	1	210.586	10.92	545.84	A
			B	0.441	1.989	0.669	1	1	73.723			
			C	0.206	2.576	0.592	1	1	30.448			
T5 100.00-80.00	3.11	5.54	A	0.767	1.795	0.87	1	1	199.109	9.25	462.31	A
			B	0.393	2.078	0.649	1	1	76.132			
			C	0.191	2.625	0.589	1	1	33.639			
T6 80.00-60.00	3.11	6.73	A	0.683	1.776	0.808	1	1	193.848	8.29	414.45	A
			B	0.365	2.138	0.638	1	1	81.733			
			C	0.193	2.619	0.589	1	1	39.946			
T7 60.00-40.00	3.11	7.09	A	0.605	1.801	0.757	1	1	184.441	7.26	363.22	A
			B	0.326	2.228	0.624	1	1	82.042			
			C	0.176	2.679	0.586	1	1	41.483			
T8 40.00-20.00	3.11	9.00	A	0.56	1.834	0.73	1	1	187.087	6.66	333.23	A
			B	0.317	2.252	0.621	1	1	90.033			
			C	0.185	2.645	0.588	1	1	49.862			
T9 20.00-0.00	0.00	9.40	A	0.174	2.685	0.585	1	1	51.704	2.70	134.82	C
			B	0.174	2.685	0.585	1	1	51.704			
			C	0.174	2.685	0.585	1	1	51.704			
Sum Weight:	20.57	49.38						OTM	5950.02 kip-ft	64.38		

### Tower Forces - With Ice - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.41	1.84	A	0.353	2.164	0.634	0.825	1	40.137	2.69	134.71	A
			B	0.189	2.632	0.588	0.825	1	19.964			
			C	0.189	2.632	0.588	0.825	1	19.964			
T2 160.00-140.00	1.90	2.52	A	0.764	1.794	0.868	0.825	1	121.142	6.51	325.29	A
			B	0.488	1.916	0.691	0.825	1	61.558			
			C	0.22	2.532	0.595	0.825	1	23.839			
T3 140.00-120.00	2.68	3.27	A	0.901	1.925	0.984	0.825	1	182.537	10.10	504.92	A
			B	0.501	1.899	0.698	0.825	1	71.940			
			C	0.218	2.536	0.594	0.825	1	26.714			
T4 120.00-100.00	3.11	4.01	A	0.876	1.892	0.962	0.825	1	210.586	10.92	545.84	A
			B	0.441	1.989	0.669	0.825	1	73.723			
			C	0.206	2.576	0.592	0.825	1	30.448			
T5 100.00-80.00	3.11	5.54	A	0.767	1.795	0.87	0.825	1	199.109	9.25	462.31	A
			B	0.393	2.078	0.649	0.825	1	76.132			
			C	0.191	2.625	0.589	0.825	1	33.639			
T6 80.00-60.00	3.11	6.73	A	0.683	1.776	0.808	0.825	1	193.848	8.29	414.45	A
			B	0.365	2.138	0.638	0.825	1	81.733			
			C	0.193	2.619	0.589	0.825	1	39.946			
T7 60.00-40.00	3.11	7.09	A	0.605	1.801	0.757	0.825	1	184.441	7.26	363.22	A
			B	0.326	2.228	0.624	0.825	1	82.042			

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 20 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T8 40.00-20.00	3.11	9.00	C	0.176	2.679	0.586	0.825	1	41.483	6.66	333.23	A
			A	0.56	1.834	0.73	0.825	1	187.087			
			B	0.317	2.252	0.621	0.825	1	90.033			
T9 20.00-0.00	0.00	9.40	C	0.185	2.645	0.588	0.825	1	49.862	2.70	134.82	C
			A	0.174	2.685	0.585	0.825	1	51.704			
			B	0.174	2.685	0.585	0.825	1	51.704			
Sum Weight:	20.57	49.38	C	0.174	2.685	0.585	0.825	1	51.704	64.38		
								OTM	5950.02			
									kip-ft			

### Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.41	1.84	A	0.353	2.164	0.634	0.8	1	40.137	2.69	134.71	A
			B	0.189	2.632	0.588	0.8	1	19.964			
			C	0.189	2.632	0.588	0.8	1	19.964			
T2 160.00-140.00	1.90	2.52	A	0.764	1.794	0.868	0.8	1	121.142	6.51	325.29	A
			B	0.488	1.916	0.691	0.8	1	61.558			
			C	0.22	2.532	0.595	0.8	1	23.839			
T3 140.00-120.00	2.68	3.27	A	0.901	1.925	0.984	0.8	1	182.537	10.10	504.92	A
			B	0.501	1.899	0.698	0.8	1	71.940			
			C	0.218	2.536	0.594	0.8	1	26.714			
T4 120.00-100.00	3.11	4.01	A	0.876	1.892	0.962	0.8	1	210.586	10.92	545.84	A
			B	0.441	1.989	0.669	0.8	1	73.723			
			C	0.206	2.576	0.592	0.8	1	30.448			
T5 100.00-80.00	3.11	5.54	A	0.767	1.795	0.87	0.8	1	199.109	9.25	462.31	A
			B	0.393	2.078	0.649	0.8	1	76.132			
			C	0.191	2.625	0.589	0.8	1	33.639			
T6 80.00-60.00	3.11	6.73	A	0.683	1.776	0.808	0.8	1	193.848	8.29	414.45	A
			B	0.365	2.138	0.638	0.8	1	81.733			
			C	0.193	2.619	0.589	0.8	1	39.946			
T7 60.00-40.00	3.11	7.09	A	0.605	1.801	0.757	0.8	1	184.441	7.26	363.22	A
			B	0.326	2.228	0.624	0.8	1	82.042			
			C	0.176	2.679	0.586	0.8	1	41.483			
T8 40.00-20.00	3.11	9.00	A	0.56	1.834	0.73	0.8	1	187.087	6.66	333.23	A
			B	0.317	2.252	0.621	0.8	1	90.033			
			C	0.185	2.645	0.588	0.8	1	49.862			
T9 20.00-0.00	0.00	9.40	A	0.174	2.685	0.585	0.8	1	51.704	2.70	134.82	C
			B	0.174	2.685	0.585	0.8	1	51.704			
			C	0.174	2.685	0.585	0.8	1	51.704			
Sum Weight:	20.57	49.38						OTM	5950.02	64.38		
									kip-ft			

### Tower Forces - With Ice - Wind 90 To Face

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 21 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.41	1.84	A	0.353	2.164	0.634	0.85	1	40.137	2.69	134.71	A
			B	0.189	2.632	0.588	0.85	1	19.964			
			C	0.189	2.632	0.588	0.85	1	19.964			
T2 160.00-140.00	1.90	2.52	A	0.764	1.794	0.868	0.85	1	121.142	6.51	325.29	A
			B	0.488	1.916	0.691	0.85	1	61.558			
			C	0.22	2.532	0.595	0.85	1	23.839			
T3 140.00-120.00	2.68	3.27	A	0.901	1.925	0.984	0.85	1	182.537	10.10	504.92	A
			B	0.501	1.899	0.698	0.85	1	71.940			
			C	0.218	2.536	0.594	0.85	1	26.714			
T4 120.00-100.00	3.11	4.01	A	0.876	1.892	0.962	0.85	1	210.586	10.92	545.84	A
			B	0.441	1.989	0.669	0.85	1	73.723			
			C	0.206	2.576	0.592	0.85	1	30.448			
T5 100.00-80.00	3.11	5.54	A	0.767	1.795	0.87	0.85	1	199.109	9.25	462.31	A
			B	0.393	2.078	0.649	0.85	1	76.132			
			C	0.191	2.625	0.589	0.85	1	33.639			
T6 80.00-60.00	3.11	6.73	A	0.683	1.776	0.808	0.85	1	193.848	8.29	414.45	A
			B	0.365	2.138	0.638	0.85	1	81.733			
			C	0.193	2.619	0.589	0.85	1	39.946			
T7 60.00-40.00	3.11	7.09	A	0.605	1.801	0.757	0.85	1	184.441	7.26	363.22	A
			B	0.326	2.228	0.624	0.85	1	82.042			
			C	0.176	2.679	0.586	0.85	1	41.483			
T8 40.00-20.00	3.11	9.00	A	0.56	1.834	0.73	0.85	1	187.087	6.66	333.23	A
			B	0.317	2.252	0.621	0.85	1	90.033			
			C	0.185	2.645	0.588	0.85	1	49.862			
T9 20.00-0.00	0.00	9.40	A	0.174	2.685	0.585	0.85	1	51.704	2.70	134.82	C
			B	0.174	2.685	0.585	0.85	1	51.704			
			C	0.174	2.685	0.585	0.85	1	51.704			
Sum Weight:	20.57	49.38						OTM	5950.02 kip-ft	64.38		

### Tower Forces - Service - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.16	1.25	A	0.243	2.457	0.6	1	1	25.946	0.73	36.53	A
			B	0.139	2.812	0.58	1	1	14.323			
			C	0.139	2.812	0.58	1	1	14.323			
T2 160.00-140.00	0.81	1.83	A	0.54	1.853	0.719	1	1	70.289	1.44	72.00	A
			B	0.36	2.148	0.636	1	1	41.456			
			C	0.171	2.697	0.585	1	1	18.058			
T3 140.00-120.00	1.14	2.50	A	0.635	1.787	0.776	1	1	100.567	1.91	95.34	A
			B	0.372	2.122	0.641	1	1	48.680			
			C	0.173	2.688	0.585	1	1	20.707			
T4 120.00-100.00	1.32	3.13	A	0.618	1.794	0.765	1	1	117.221	2.13	106.40	A
			B	0.33	2.219	0.626	1	1	51.275			
			C	0.165	2.716	0.584	1	1	23.978			
T5 100.00-80.00	1.32	4.62	A	0.544	1.849	0.721	1	1	116.596	2.06	103.00	A
			B	0.3	2.295	0.616	1	1	54.973			
			C	0.161	2.733	0.583	1	1	27.825			
T6 80.00-60.00	1.32	5.62	A	0.491	1.911	0.693	1	1	119.079	2.02	101.20	A
			B	0.284	2.339	0.611	1	1	60.700			
			C	0.165	2.716	0.584	1	1	33.740			
T7 60.00-40.00	1.32	5.88	A	0.435	1.999	0.667	1	1	116.286	1.88	93.88	A
			B	0.254	2.426	0.603	1	1	61.332			

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 22 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T8 40.00-20.00	1.32	7.52	C	0.15	2.772	0.581	1	1	34.921	1.80	89.82	A
			A	0.41	2.045	0.656	1	1	122.450			
			B	0.252	2.433	0.602	1	1	69.088			
T9 20.00-0.00	0.00	7.82	C	0.161	2.731	0.583	1	1	42.811	0.88	43.98	C
			A	0.15	2.77	0.582	1	1	44.271			
			B	0.15	2.77	0.582	1	1	44.271			
Sum Weight:	8.73	40.16	C	0.15	2.77	0.582	1	1	44.271	14.84		
								OTM	1305.83 kip-ft			

### Tower Forces - Service - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.16	1.25	A	0.243	2.457	0.6	0.825	1	25.946	0.73	36.53	A
			B	0.139	2.812	0.58	0.825	1	14.323			
			C	0.139	2.812	0.58	0.825	1	14.323			
T2 160.00-140.00	0.81	1.83	A	0.54	1.853	0.719	0.825	1	70.289	1.44	72.00	A
			B	0.36	2.148	0.636	0.825	1	41.456			
			C	0.171	2.697	0.585	0.825	1	18.058			
T3 140.00-120.00	1.14	2.50	A	0.635	1.787	0.776	0.825	1	100.567	1.91	95.34	A
			B	0.372	2.122	0.641	0.825	1	48.680			
			C	0.173	2.688	0.585	0.825	1	20.707			
T4 120.00-100.00	1.32	3.13	A	0.618	1.794	0.765	0.825	1	117.221	2.13	106.40	A
			B	0.33	2.219	0.626	0.825	1	51.275			
			C	0.165	2.716	0.584	0.825	1	23.978			
T5 100.00-80.00	1.32	4.62	A	0.544	1.849	0.721	0.825	1	116.596	2.06	103.00	A
			B	0.3	2.295	0.616	0.825	1	54.973			
			C	0.161	2.733	0.583	0.825	1	27.825			
T6 80.00-60.00	1.32	5.62	A	0.491	1.911	0.693	0.825	1	119.079	2.02	101.20	A
			B	0.284	2.339	0.611	0.825	1	60.700			
			C	0.165	2.716	0.584	0.825	1	33.740			
T7 60.00-40.00	1.32	5.88	A	0.435	1.999	0.667	0.825	1	116.286	1.88	93.88	A
			B	0.254	2.426	0.603	0.825	1	61.332			
			C	0.15	2.772	0.581	0.825	1	34.921			
T8 40.00-20.00	1.32	7.52	A	0.41	2.045	0.656	0.825	1	122.450	1.80	89.82	A
			B	0.252	2.433	0.602	0.825	1	69.088			
			C	0.161	2.731	0.583	0.825	1	42.811			
T9 20.00-0.00	0.00	7.82	A	0.15	2.77	0.582	0.825	1	44.271	0.88	43.98	C
			B	0.15	2.77	0.582	0.825	1	44.271			
			C	0.15	2.77	0.582	0.825	1	44.271			
Sum Weight:	8.73	40.16						OTM	1305.83 kip-ft	14.84		

### Tower Forces - Service - Wind 60 To Face



<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 23 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.16	1.25	A	0.243	2.457	0.6	0.8	1	25.946	0.73	36.53	A
			B	0.139	2.812	0.58	0.8	1	14.323			
			C	0.139	2.812	0.58	0.8	1	14.323			
T2 160.00-140.00	0.81	1.83	A	0.54	1.853	0.719	0.8	1	70.289	1.44	72.00	A
			B	0.36	2.148	0.636	0.8	1	41.456			
			C	0.171	2.697	0.585	0.8	1	18.058			
T3 140.00-120.00	1.14	2.50	A	0.635	1.787	0.776	0.8	1	100.567	1.91	95.34	A
			B	0.372	2.122	0.641	0.8	1	48.680			
			C	0.173	2.688	0.585	0.8	1	20.707			
T4 120.00-100.00	1.32	3.13	A	0.618	1.794	0.765	0.8	1	117.221	2.13	106.40	A
			B	0.33	2.219	0.626	0.8	1	51.275			
			C	0.165	2.716	0.584	0.8	1	23.978			
T5 100.00-80.00	1.32	4.62	A	0.544	1.849	0.721	0.8	1	116.596	2.06	103.00	A
			B	0.3	2.295	0.616	0.8	1	54.973			
			C	0.161	2.733	0.583	0.8	1	27.825			
T6 80.00-60.00	1.32	5.62	A	0.491	1.911	0.693	0.8	1	119.079	2.02	101.20	A
			B	0.284	2.339	0.611	0.8	1	60.700			
			C	0.165	2.716	0.584	0.8	1	33.740			
T7 60.00-40.00	1.32	5.88	A	0.435	1.999	0.667	0.8	1	116.286	1.88	93.88	A
			B	0.254	2.426	0.603	0.8	1	61.332			
			C	0.15	2.772	0.581	0.8	1	34.921			
T8 40.00-20.00	1.32	7.52	A	0.41	2.045	0.656	0.8	1	122.450	1.80	89.82	A
			B	0.252	2.433	0.602	0.8	1	69.088			
			C	0.161	2.731	0.583	0.8	1	42.811			
T9 20.00-0.00	0.00	7.82	A	0.15	2.77	0.582	0.8	1	44.271	0.88	43.98	C
			B	0.15	2.77	0.582	0.8	1	44.271			
			C	0.15	2.77	0.582	0.8	1	44.271			
Sum Weight:	8.73	40.16						OTM	1305.83 kip-ft	14.84		

### Tower Forces - Service - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K	e						ft <sup>2</sup>	K	plf	
T1 180.00-160.00	0.16	1.25	A	0.243	2.457	0.6	0.85	1	25.946	0.73	36.53	A
			B	0.139	2.812	0.58	0.85	1	14.323			
			C	0.139	2.812	0.58	0.85	1	14.323			
T2 160.00-140.00	0.81	1.83	A	0.54	1.853	0.719	0.85	1	70.289	1.44	72.00	A
			B	0.36	2.148	0.636	0.85	1	41.456			
			C	0.171	2.697	0.585	0.85	1	18.058			
T3 140.00-120.00	1.14	2.50	A	0.635	1.787	0.776	0.85	1	100.567	1.91	95.34	A
			B	0.372	2.122	0.641	0.85	1	48.680			
			C	0.173	2.688	0.585	0.85	1	20.707			
T4 120.00-100.00	1.32	3.13	A	0.618	1.794	0.765	0.85	1	117.221	2.13	106.40	A
			B	0.33	2.219	0.626	0.85	1	51.275			
			C	0.165	2.716	0.584	0.85	1	23.978			
T5 100.00-80.00	1.32	4.62	A	0.544	1.849	0.721	0.85	1	116.596	2.06	103.00	A
			B	0.3	2.295	0.616	0.85	1	54.973			
			C	0.161	2.733	0.583	0.85	1	27.825			
T6 80.00-60.00	1.32	5.62	A	0.491	1.911	0.693	0.85	1	119.079	2.02	101.20	A
			B	0.284	2.339	0.611	0.85	1	60.700			
			C	0.165	2.716	0.584	0.85	1	33.740			
T7 60.00-40.00	1.32	5.88	A	0.435	1.999	0.667	0.85	1	116.286	1.88	93.88	A
			B	0.254	2.426	0.603	0.85	1	61.332			

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	Job	15049.002 - CT11041D	Page	24 of 43
	Project	180' Lattice Tower - 53 Dayton Road Waterford, CT	Date	16:53:44 04/22/15
	Client	T-Mobile	Designed by	TJL

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	R <sub>R</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K							ft <sup>2</sup>	K	plf	
T8 40.00-20.00	1.32	7.52	C	0.15	2.772	0.581	0.85	1	34.921	1.80	89.82	A
			A	0.41	2.045	0.656	0.85	1	122.450			
			B	0.252	2.433	0.602	0.85	1	69.088			
T9 20.00-0.00	0.00	7.82	C	0.161	2.731	0.583	0.85	1	42.811	0.88	43.98	C
			A	0.15	2.77	0.582	0.85	1	44.271			
			B	0.15	2.77	0.582	0.85	1	44.271			
Sum Weight:	8.73	40.16	C	0.15	2.77	0.582	0.85	1	44.271	14.84		
								OTM	1305.83			
									kip-ft			

### Force Totals

Load Case	Vertical Forces	Sum of Forces X	Sum of Forces Z	Sum of Overturning Moments, M <sub>x</sub>	Sum of Overturning Moments, M <sub>z</sub>	Sum of Torques
	K	K	K	kip-ft	kip-ft	kip-ft
Leg Weight	21.15					
Bracing Weight	19.01					
Total Member Self-Weight	40.16					
Total Weight	57.26					
Wind 0 deg - No Ice		-0.04	-74.74	-7991.75	27.15	-42.41
Wind 30 deg - No Ice		37.31	-64.71	-6925.38	-3940.60	-89.89
Wind 45 deg - No Ice		52.79	-52.82	-5662.48	-5584.63	-105.16
Wind 60 deg - No Ice		64.67	-37.34	-4017.20	-6846.58	-113.28
Wind 90 deg - No Ice		74.70	0.04	-46.46	-7912.15	-106.31
Wind 120 deg - No Ice		64.71	37.41	3922.90	-6851.78	-70.86
Wind 135 deg - No Ice		52.85	52.88	5566.52	-5591.98	-45.18
Wind 150 deg - No Ice		37.38	64.75	6827.27	-3949.60	-16.43
Wind 180 deg - No Ice		0.04	74.74	7888.45	16.76	42.41
Wind 210 deg - No Ice		-37.31	64.71	6822.08	3984.50	89.89
Wind 225 deg - No Ice		-52.79	52.82	5559.17	5628.54	105.16
Wind 240 deg - No Ice		-64.67	37.34	3913.90	6890.49	113.28
Wind 270 deg - No Ice		-74.70	-0.04	-56.85	7956.05	106.31
Wind 300 deg - No Ice		-64.71	-37.41	-4026.20	6895.68	70.86
Wind 315 deg - No Ice		-52.85	-52.88	-5669.82	5635.88	45.18
Wind 330 deg - No Ice		-37.38	-64.75	-6930.58	3993.50	16.43
Member Ice	9.22					
Total Weight Ice	82.26					
Wind 0 deg - Ice		-0.03	-82.90	-8879.12	52.25	-55.97
Wind 30 deg - Ice		41.41	-71.78	-7699.21	-4341.91	-111.30
Wind 45 deg - Ice		58.57	-58.60	-6301.17	-6162.44	-128.42
Wind 60 deg - Ice		71.75	-41.42	-4479.67	-7559.74	-136.80
Wind 90 deg - Ice		82.87	0.03	-83.18	-8739.02	-125.65
Wind 120 deg - Ice		71.78	41.48	4312.23	-7563.77	-80.83
Wind 135 deg - Ice		58.62	58.64	6132.45	-6168.16	-49.27
Wind 150 deg - Ice		41.46	71.81	7528.81	-4348.90	-14.35
Wind 180 deg - Ice		0.03	82.90	8704.68	44.17	55.97
Wind 210 deg - Ice		-41.41	71.78	7524.77	4438.33	111.30
Wind 225 deg - Ice		-58.57	58.60	6126.73	6258.87	128.42
Wind 240 deg - Ice		-71.75	41.42	4305.23	7656.16	136.80
Wind 270 deg - Ice		-82.87	-0.03	-91.26	8835.44	125.65
Wind 300 deg - Ice		-71.78	-41.48	-4486.66	7660.20	80.83
Wind 315 deg - Ice		-58.62	-58.64	-6306.88	6264.58	49.27
Wind 330 deg - Ice		-41.46	-71.81	-7703.24	4445.33	14.35
Total Weight	57.26					
Wind 0 deg - Service		-0.01	-20.70	-2231.07	6.66	-11.75

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	Job	15049.002 - CT11041D	Page	25 of 43
	Project	180' Lattice Tower - 53 Dayton Road Waterford, CT	Date	16:53:44 04/22/15
	Client	T-Mobile	Designed by	TJL

Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, $M_x$ kip-ft	Sum of Overturning Moments, $M_z$ kip-ft	Sum of Torques kip-ft
Wind 30 deg - Service		10.34	-17.92	-1935.68	-1092.44	-24.90
Wind 45 deg - Service		14.62	-14.63	-1585.84	-1547.85	-29.13
Wind 60 deg - Service		17.91	-10.34	-1130.09	-1897.42	-31.38
Wind 90 deg - Service		20.69	0.01	-30.16	-2192.59	-29.45
Wind 120 deg - Service		17.92	10.36	1069.38	-1898.86	-19.63
Wind 135 deg - Service		14.64	14.65	1524.68	-1549.88	-12.52
Wind 150 deg - Service		10.36	17.94	1873.92	-1094.93	-4.55
Wind 180 deg - Service		0.01	20.70	2167.88	3.78	11.75
Wind 210 deg - Service		-10.34	17.92	1872.48	1102.88	24.90
Wind 225 deg - Service		-14.62	14.63	1522.65	1558.29	29.13
Wind 240 deg - Service		-17.91	10.34	1066.89	1907.86	31.38
Wind 270 deg - Service		-20.69	-0.01	-33.04	2203.03	29.45
Wind 300 deg - Service		-17.92	-10.36	-1132.58	1909.30	19.63
Wind 315 deg - Service		-14.64	-14.65	-1587.88	1560.33	12.52
Wind 330 deg - Service		-10.36	-17.94	-1937.12	1105.37	4.55

### Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 45 deg - No Ice
5	Dead+Wind 60 deg - No Ice
6	Dead+Wind 90 deg - No Ice
7	Dead+Wind 120 deg - No Ice
8	Dead+Wind 135 deg - No Ice
9	Dead+Wind 150 deg - No Ice
10	Dead+Wind 180 deg - No Ice
11	Dead+Wind 210 deg - No Ice
12	Dead+Wind 225 deg - No Ice
13	Dead+Wind 240 deg - No Ice
14	Dead+Wind 270 deg - No Ice
15	Dead+Wind 300 deg - No Ice
16	Dead+Wind 315 deg - No Ice
17	Dead+Wind 330 deg - No Ice
18	Dead+Ice+Temp
19	Dead+Wind 0 deg+Ice+Temp
20	Dead+Wind 30 deg+Ice+Temp
21	Dead+Wind 45 deg+Ice+Temp
22	Dead+Wind 60 deg+Ice+Temp
23	Dead+Wind 90 deg+Ice+Temp
24	Dead+Wind 120 deg+Ice+Temp
25	Dead+Wind 135 deg+Ice+Temp
26	Dead+Wind 150 deg+Ice+Temp
27	Dead+Wind 180 deg+Ice+Temp
28	Dead+Wind 210 deg+Ice+Temp
29	Dead+Wind 225 deg+Ice+Temp
30	Dead+Wind 240 deg+Ice+Temp
31	Dead+Wind 270 deg+Ice+Temp
32	Dead+Wind 300 deg+Ice+Temp
33	Dead+Wind 315 deg+Ice+Temp
34	Dead+Wind 330 deg+Ice+Temp
35	Dead+Wind 0 deg - Service
36	Dead+Wind 30 deg - Service

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	Project	180' Lattice Tower - 53 Dayton Road Waterford, CT	Date	16:53:44 04/22/15
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Comb. No.	Description
37	Dead+Wind 45 deg - Service
38	Dead+Wind 60 deg - Service
39	Dead+Wind 90 deg - Service
40	Dead+Wind 120 deg - Service
41	Dead+Wind 135 deg - Service
42	Dead+Wind 150 deg - Service
43	Dead+Wind 180 deg - Service
44	Dead+Wind 210 deg - Service
45	Dead+Wind 225 deg - Service
46	Dead+Wind 240 deg - Service
47	Dead+Wind 270 deg - Service
48	Dead+Wind 300 deg - Service
49	Dead+Wind 315 deg - Service
50	Dead+Wind 330 deg - Service

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	180 - 160	Leg	Max Tension	32	5.40	-0.48	-0.27
			Max. Compression	19	-8.59	1.43	0.03
			Max. Mx	5	4.02	-1.66	0.73
			Max. My	14	-1.90	-0.00	-2.47
			Max. Vy	10	0.84	-1.65	-0.03
			Max. Vx	14	1.21	-0.00	-2.47
		Diagonal	Max Tension	3	4.99	0.00	0.00
			Max. Compression	3	-5.05	0.00	0.00
			Max. Mx	34	3.92	0.02	0.00
			Max. My	22	-0.21	0.00	0.00
			Max. Vy	34	0.01	0.00	0.00
			Max. Vx	22	-0.00	0.00	0.00
		Horizontal	Max Tension	4	2.85	-0.01	0.00
			Max. Compression	12	-2.81	0.00	0.00
			Max. Mx	32	0.10	-0.02	-0.01
			Max. My	15	-0.51	-0.01	-0.01
			Max. Vy	32	-0.01	-0.02	-0.01
			Max. Vx	15	0.00	-0.01	-0.01
		Top Girt	Max Tension	27	0.81	-0.01	0.00
			Max. Compression	19	-0.81	-0.01	-0.00
			Max. Mx	32	-0.28	-0.01	-0.00
			Max. My	27	-0.24	-0.01	-0.00
			Max. Vy	32	-0.01	-0.01	-0.00
			Max. Vx	19	-0.00	-0.01	0.00
Inner Bracing	Max Tension	19	0.01	0.00	0.00		
	Max. Compression	19	-0.01	0.00	0.00		
	Max. Mx	18	-0.00	-0.01	0.00		
	Max. My	19	0.00	0.00	-0.00		
	Max. Vy	18	0.01	0.00	0.00		
	Max. Vx	19	0.00	0.00	0.00		
T2	160 - 140	Leg	Max Tension	15	37.93	-0.82	-0.25
			Max. Compression	19	-45.85	0.73	0.21
			Max. Mx	10	9.68	1.69	-0.03
			Max. My	6	-2.06	0.00	-3.07
			Max. Vy	10	-1.13	-1.65	-0.03
		Diagonal	Max. Vx	6	1.91	0.00	2.47
			Max Tension	4	14.21	0.00	0.00
			Max. Compression	4	-14.29	0.00	0.00
			Max. Mx	34	12.46	0.03	0.00

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 15049.002 - CT11041D	<b>Page</b> 27 of 43
	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> T.J.L.

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft		
T3	140 - 120	Horizontal	Max. My	22	-2.66	0.00	0.00		
			Max. Vy	34	-0.02	0.00	0.00		
			Max. Vx	22	-0.00	0.00	0.00		
			Max Tension	21	7.87	-0.01	0.00		
			Max. Compression	12	-7.81	-0.02	-0.01		
			Max. Mx	32	0.47	-0.05	-0.02		
			Top Girt	Max. My	5	-2.84	-0.03	-0.02	
				Max. Vy	32	-0.02	-0.05	-0.02	
				Max. Vx	5	0.00	-0.03	-0.02	
				Max Tension	5	5.96	-0.01	0.01	
				Max. Compression	13	-5.90	-0.02	-0.01	
				Max. Mx	32	-2.18	-0.03	-0.02	
				Max. My	5	-2.65	-0.02	-0.02	
				Max. Vy	32	-0.02	-0.03	-0.02	
				Max. Vx	15	0.00	-0.02	-0.02	
		Inner Bracing		Max Tension	13	0.10	0.00	0.00	
				Max. Compression	13	-0.10	0.00	0.00	
				Max. Mx	18	-0.00	-0.01	0.00	
				Max. My	32	-0.01	0.00	0.00	
				Max. Vy	18	-0.01	0.00	0.00	
				Max. Vx	32	-0.00	0.00	0.00	
			Leg	Max Tension	15	87.44	-0.10	0.01	
				Max. Compression	19	-101.83	0.43	0.04	
				Max. Mx	10	46.70	2.27	0.04	
				Max. My	6	-9.15	-0.08	-3.16	
				Max. Vy	10	2.35	-1.64	0.04	
				Max. Vx	14	3.06	-0.06	-1.90	
				Diagonal	Max Tension	29	18.73	0.00	0.00
					Max. Compression	29	-18.87	0.00	0.00
					Max. Mx	34	15.69	0.05	0.00
		Max. My			22	-4.85	0.00	0.00	
		Max. Vy			34	-0.02	0.00	0.00	
		Max. Vx			22	-0.00	0.00	0.00	
		Horizontal			Max Tension	21	11.54	-0.02	0.01
					Max. Compression	29	-11.83	-0.03	-0.01
					Max. Mx	32	1.01	-0.05	-0.02
			Max. My		10	-1.22	-0.04	-0.03	
			Max. Vy		32	-0.02	-0.05	-0.02	
			Max. Vx		27	0.01	-0.04	-0.03	
			Top Girt		Max Tension	22	8.39	-0.01	0.01
					Max. Compression	30	-8.95	-0.03	-0.01
					Max. Mx	32	-3.98	-0.04	-0.02
				Max. My	24	3.47	0.01	0.02	
				Max. Vy	32	-0.02	-0.04	-0.02	
				Max. Vx	24	-0.01	0.01	0.02	
Inner Bracing	Max Tension			30	0.16	0.00	0.00		
	Max. Compression			30	-0.16	0.00	0.00		
	Max. Mx			18	-0.00	-0.01	0.00		
	Max. My	32		-0.02	0.00	0.00			
	Max. Vy	18		0.01	0.00	0.00			
	Max. Vx	32		-0.00	0.00	0.00			
	Leg	Max Tension		32	139.16	-1.80	-0.44		
		Max. Compression		19	-160.70	1.80	0.22		
		Max. Mx		22	136.57	-1.80	0.66		
		Max. My	31	-16.04	0.00	-2.32			
		Max. Vy	27	0.30	-1.78	-0.23			
		Max. Vx	23	-0.46	-0.00	2.31			
		Diagonal	Max Tension	29	19.10	0.00	0.00		
			Max. Compression	29	-19.28	0.00	0.00		
			Max. Mx	34	16.05	0.06	0.00		
Max. My			22	-5.52	0.00	0.00			
T4		120 - 100							

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	Client	T-Mobile	Designed by	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T5	100 - 80	Horizontal	Max. Vy	34	-0.03	0.00	0.00
			Max. Vx	22	-0.00	0.00	0.00
			Max. Tension	21	13.18	-0.02	0.01
			Max. Compression	29	-13.45	-0.03	-0.01
			Max. Mx	32	1.53	-0.05	-0.02
			Max. My	27	-1.36	-0.05	-0.03
		Top Girt	Max. Vy	32	-0.02	-0.05	-0.02
			Max. Vx	27	0.00	-0.05	-0.03
			Max. Tension	21	12.08	-0.02	0.01
			Max. Compression	29	-12.36	-0.03	-0.01
			Max. Mx	32	-3.44	-0.04	-0.02
			Max. My	27	0.13	-0.04	-0.02
		Inner Bracing	Max. Vy	32	-0.02	-0.04	-0.02
			Max. Vx	27	0.00	-0.04	-0.02
			Max. Tension	29	0.21	0.00	0.00
			Max. Compression	29	-0.21	0.00	0.00
			Max. Mx	18	-0.00	-0.02	0.00
			Max. My	32	-0.01	0.00	0.00
		Leg	Max. Vy	18	-0.01	0.00	0.00
			Max. Vx	32	0.00	0.00	0.00
			Max. Tension	32	182.21	-1.29	-0.06
			Max. Compression	19	-208.28	1.60	0.12
			Max. Mx	22	154.65	-1.80	0.66
			Max. My	31	-16.35	0.00	-2.32
		Diagonal	Max. Vy	27	-0.19	-1.78	-0.23
			Max. Vx	22	0.34	0.89	2.12
			Max. Tension	29	23.92	0.00	0.00
			Max. Compression	29	-24.22	0.00	0.00
			Max. Mx	20	22.83	0.15	0.00
			Max. My	22	-6.78	0.00	0.00
		Horizontal	Max. Vy	20	-0.05	0.00	0.00
			Max. Vx	22	0.00	0.00	0.00
			Max. Tension	21	14.25	-0.04	0.01
			Max. Compression	29	-14.61	-0.05	-0.01
			Max. Mx	32	1.97	-0.07	-0.02
			Max. My	27	-1.80	-0.07	-0.02
		Top Girt	Max. Vy	32	-0.03	-0.07	-0.02
			Max. Vx	27	0.00	-0.07	-0.02
			Max. Tension	21	13.75	-0.03	-0.01
			Max. Compression	29	-14.27	-0.05	-0.01
			Max. Mx	32	-3.88	-0.07	-0.02
			Max. My	27	-0.01	-0.07	-0.02
Inner Bracing	Max. Vy	32	-0.03	-0.07	-0.02		
	Max. Vx	27	0.00	-0.07	-0.02		
	Max. Tension	29	0.25	0.00	0.00		
	Max. Compression	29	-0.25	0.00	0.00		
	Max. Mx	18	-0.00	-0.02	0.00		
	Max. My	32	-0.01	0.00	0.00		
T6	80 - 60	Leg	Max. Vy	18	0.01	0.00	0.00
			Max. Vx	32	0.00	0.00	0.00
			Max. Tension	32	229.23	-2.33	-0.08
			Max. Compression	19	-261.30	2.55	0.05
			Max. Mx	32	228.98	-2.57	-0.11
			Max. My	31	-19.78	-0.01	-2.50
		Diagonal	Max. Vy	10	0.21	-2.17	-0.05
			Max. Vx	31	0.29	-0.01	-2.50
			Max. Tension	29	22.92	0.00	0.00
			Max. Compression	29	-23.30	0.00	0.00
			Max. Mx	20	22.28	0.19	0.00
			Max. My	22	-6.19	0.00	0.00
			Max. Vy	20	-0.06	0.00	0.00

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	Client	T-Mobile	Designed by	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft		
T7	60 - 40	Horizontal	Max. Vx	22	-0.00	0.00	0.00		
			Max Tension	21	15.18	-0.06	0.01		
			Max. Compression	29	-15.49	-0.08	-0.01		
			Max. Mx	32	2.45	-0.11	-0.03		
			Max. My	27	-2.29	-0.11	-0.03		
			Max. Vy	32	-0.05	-0.11	-0.03		
		Top Girt	Max. Vx	27	0.00	-0.11	-0.03		
			Max Tension	21	14.65	-0.05	0.01		
			Max. Compression	29	-15.02	-0.07	-0.01		
			Max. Mx	32	-3.52	-0.10	-0.03		
			Max. My	27	0.35	-0.10	-0.03		
			Max. Vy	32	-0.04	-0.10	-0.03		
		Inner Bracing	Max. Vx	27	0.00	-0.10	-0.03		
			Max Tension	29	0.26	0.00	0.00		
			Max. Compression	29	-0.26	0.00	0.00		
			Max. Mx	18	-0.01	-0.05	0.00		
			Max. My	32	-0.02	0.00	0.00		
			Max. Vy	18	-0.02	0.00	0.00		
		Leg		Horizontal	Max. Vx	32	-0.00	0.00	0.00
					Max Tension	32	271.93	-2.13	-0.01
					Max. Compression	19	-310.44	2.92	0.23
				Diagonal	Max. Mx	22	267.42	-3.04	0.61
					Max. My	31	-24.40	-0.06	-3.80
					Max. Vy	27	0.23	-3.03	-0.25
					Max. Vx	23	-0.36	-0.06	3.77
					Max Tension	29	21.89	0.00	0.00
					Max. Compression	29	-22.35	0.00	0.00
				Horizontal	Max. Mx	20	20.97	0.23	0.00
					Max. My	22	-5.76	0.00	0.00
					Max. Vy	20	-0.07	0.00	0.00
					Max. Vx	22	-0.00	0.00	0.00
					Max Tension	28	15.31	0.00	0.00
					Max. Compression	29	-15.57	-0.10	-0.01
Top Girt	Max. Mx	32	2.91	-0.12	-0.03				
	Max. My	27	-2.74	-0.12	-0.03				
	Max. Vy	32	-0.05	-0.12	-0.03				
	Max. Vx	27	0.00	-0.12	-0.03				
	Max Tension	21	15.21	-0.07	0.01				
	Max. Compression	29	-15.60	-0.09	-0.01				
Inner Bracing	Max. Mx	32	-3.46	-0.12	-0.03				
	Max. My	24	-1.01	-0.05	0.03				
	Max. Vy	32	-0.05	-0.12	-0.03				
	Max. Vx	27	0.00	-0.11	-0.03				
	Max Tension	29	0.27	0.00	0.00				
	Max. Compression	29	-0.27	0.00	0.00				
Leg		Horizontal	Max. Mx	18	-0.01	-0.07	0.00		
			Max. My	32	0.21	0.00	0.00		
			Max. Vy	18	0.03	0.00	0.00		
		Diagonal	Max. Vx	32	-0.00	0.00	0.00		
			Max Tension	32	310.99	-3.31	0.00		
			Max. Compression	19	-356.88	2.08	-0.03		
			Max. Mx	32	310.62	-4.03	-0.03		
			Max. My	31	-25.18	-0.06	-3.80		
			Max. Vy	24	0.22	3.38	-0.07		
		Horizontal	Max. Vx	22	0.26	1.43	3.39		
			Max Tension	29	22.33	0.00	0.00		
			Max. Compression	29	-23.00	0.00	0.00		
			Max. Mx	20	21.87	0.33	0.00		
			Max. My	22	-5.52	0.00	0.00		
			Max. Vy	20	-0.09	0.00	0.00		
Diagonal	Max. Vx	22	-0.00	0.00	0.00				

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	Project	180' Lattice Tower - 53 Dayton Road Waterford, CT	Date	16:53:44 04/22/15
	Client	T-Mobile	Designed by	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T9	20 - 0	Horizontal	Max Tension	28	17.02	0.00	0.00	
			Max. Compression	29	-16.90	-0.17	-0.01	
			Max. Mx	32	3.34	-0.20	-0.04	
			Max. My	27	-3.18	-0.20	-0.04	
			Max. Vy	32	-0.07	-0.20	-0.04	
			Max. Vx	27	0.00	-0.20	-0.04	
		Top Girt	Max Tension	28	16.61	-0.14	0.00	
			Max. Compression	29	-16.69	-0.15	-0.01	
			Max. Mx	32	-2.53	-0.19	-0.04	
			Max. My	27	1.33	-0.18	-0.04	
			Max. Vy	32	-0.07	-0.19	-0.04	
			Max. Vx	27	0.00	-0.18	-0.04	
		Inner Bracing	Max Tension	29	0.29	0.00	0.00	
			Max. Compression	29	-0.29	0.00	0.00	
			Max. Mx	18	-0.01	-0.13	0.00	
			Max. My	32	0.23	0.00	0.00	
			Max. Vy	18	-0.05	0.00	0.00	
			Max. Vx	32	-0.00	0.00	0.00	
		Leg	Max Tension	32	348.61	0.62	-0.01	
			Max. Compression	19	-401.36	0.00	-0.00	
			Max. Mx	19	-379.81	9.31	-0.05	
			Max. My	34	-25.09	4.94	4.04	
			Max. Vy	19	1.02	9.31	-0.05	
			Max. Vx	34	0.52	4.94	4.04	
			Diagonal	Max Tension	28	21.82	0.00	0.00
				Max. Compression	28	-22.60	0.00	0.00
				Max. Mx	21	21.02	0.39	0.00
				Max. My	30	3.89	0.00	-0.00
				Max. Vy	21	-0.10	0.00	0.00
				Max. Vx	30	-0.00	0.00	0.00
			Horizontal	Max Tension	28	17.79	0.00	0.00
				Max. Compression	29	-16.38	-0.14	-0.01
				Max. Mx	15	3.28	-0.18	-0.03
				Max. My	27	-3.61	-0.17	-0.04
				Max. Vy	32	-0.07	-0.17	-0.04
				Max. Vx	27	0.00	-0.17	-0.04
Top Girt	Max Tension	28	16.73	-0.17	0.00			
	Max. Compression	29	-17.50	-0.18	-0.01			
	Max. Mx	32	-2.94	-0.21	-0.04			
	Max. My	24	-1.49	-0.12	0.04			
	Max. Vy	32	-0.08	-0.21	-0.04			
	Max. Vx	24	-0.00	-0.12	0.04			
Inner Bracing	Max Tension	29	0.30	0.00	0.00			
	Max. Compression	29	-0.30	0.00	0.00			
	Max. Mx	18	-0.01	-0.16	0.00			
	Max. My	24	0.26	0.00	-0.00			
	Max. Vy	18	-0.05	0.00	0.00			
	Max. Vx	24	-0.00	0.00	0.00			

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	30	416.02	41.98	-27.83
	Max. H <sub>x</sub>	30	416.02	41.98	-27.83
	Max. H <sub>z</sub>	21	-348.10	-36.72	26.63



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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg B	Min. Vert	22	-361.35	-38.95	26.07
	Min. H <sub>x</sub>	22	-361.35	-38.95	26.07
	Min. H <sub>z</sub>	29	402.77	39.79	-28.30
	Max. Vert	24	412.53	-42.58	-26.71
	Max. H <sub>x</sub>	32	-365.45	39.67	25.03
	Max. H <sub>z</sub>	33	-352.24	37.70	25.12
Leg A	Min. Vert	32	-365.45	39.67	25.03
	Min. H <sub>x</sub>	24	412.53	-42.58	-26.71
	Min. H <sub>z</sub>	25	399.32	-40.67	-26.74
	Max. Vert	19	420.37	-1.28	50.38
	Max. H <sub>x</sub>	31	31.57	9.53	2.00
	Max. H <sub>z</sub>	19	420.37	-1.28	50.38
	Min. Vert	27	-357.59	1.26	-46.72
	Min. H <sub>x</sub>	23	31.22	-9.61	1.97
	Min. H <sub>z</sub>	27	-357.59	1.26	-46.72

### Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	57.26	0.00	0.00	-51.72	21.97	0.00
Dead+Wind 0 deg - No Ice	57.26	-0.04	-74.74	-7823.35	27.20	-42.51
Dead+Wind 30 deg - No Ice	57.26	37.31	-64.70	-6779.66	-3856.31	-90.13
Dead+Wind 45 deg - No Ice	57.26	52.79	-52.82	-5543.61	-5465.47	-105.44
Dead+Wind 60 deg - No Ice	57.26	64.66	-37.33	-3933.30	-6700.69	-113.58
Dead+Wind 90 deg - No Ice	57.26	74.69	0.04	-46.86	-7743.76	-106.63
Dead+Wind 120 deg - No Ice	57.26	64.70	37.40	3838.31	-6706.01	-71.10
Dead+Wind 135 deg - No Ice	57.26	52.84	52.88	5447.10	-5472.95	-45.35
Dead+Wind 150 deg - No Ice	57.26	37.38	64.75	6681.12	-3865.39	-16.50
Dead+Wind 180 deg - No Ice	57.26	0.04	74.74	7719.77	16.91	42.51
Dead+Wind 210 deg - No Ice	57.26	-37.31	64.70	6675.93	3900.57	90.12
Dead+Wind 225 deg - No Ice	57.26	-52.79	52.82	5439.77	5509.73	105.45
Dead+Wind 240 deg - No Ice	57.26	-64.66	37.33	3829.37	6744.89	113.59
Dead+Wind 270 deg - No Ice	57.26	-74.69	-0.04	-57.12	7787.78	106.63
Dead+Wind 300 deg - No Ice	57.26	-64.70	-37.40	-3942.14	6749.85	71.10
Dead+Wind 315 deg - No Ice	57.26	-52.84	-52.88	-5550.82	5516.80	45.34
Dead+Wind 330 deg - No Ice	57.26	-37.38	-64.75	-6784.75	3909.30	16.50
Dead+Ice+Temp	82.26	0.00	0.00	-87.40	48.26	0.00
Dead+Wind 0 deg+Ice+Temp	82.26	-0.03	-82.89	-8670.90	52.33	-56.16
Dead+Wind 30 deg+Ice+Temp	82.26	41.40	-71.77	-7519.08	-4237.65	-111.68
Dead+Wind 45 deg+Ice+Temp	82.26	58.57	-58.59	-6154.27	-6015.06	-128.86
Dead+Wind 60 deg+Ice+Temp	82.26	71.74	-41.42	-4376.03	-7379.29	-137.28
Dead+Wind 90 deg+Ice+Temp	82.26	82.86	0.03	-83.75	-8530.57	-126.10
Dead+Wind 120 deg+Ice+Temp	82.26	71.77	41.48	4207.42	-7383.41	-81.15
Dead+Wind 135 deg+Ice+Temp	82.26	58.61	58.64	5984.54	-6020.85	-49.48
Dead+Wind 150 deg+Ice+Temp	82.26	41.46	71.81	7347.85	-4244.66	-14.43
Dead+Wind 180 deg+Ice+Temp	82.26	0.03	82.89	8495.86	44.48	56.16
Dead+Wind 210 deg+Ice+Temp	82.26	-41.40	71.77	7343.85	4334.63	111.68
Dead+Wind 225 deg+Ice+Temp	82.26	-58.57	58.59	5978.93	6112.03	128.87
Dead+Wind 240 deg+Ice+Temp	82.26	-71.74	41.42	4200.58	7476.18	137.28
Dead+Wind 270 deg+Ice+Temp	82.26	-82.86	-0.03	-91.55	8627.24	126.10
Dead+Wind 300 deg+Ice+Temp	82.26	-71.77	-41.47	-4382.73	7479.90	81.15
Dead+Wind 315 deg+Ice+Temp	82.26	-58.61	-58.64	-6159.73	6117.34	49.48
Dead+Wind 330 deg+Ice+Temp	82.26	-41.46	-71.80	-7522.93	4341.22	14.43
Dead+Wind 0 deg - Service	57.26	-0.01	-20.70	-2204.59	23.43	-11.78
Dead+Wind 30 deg - Service	57.26	10.34	-17.92	-1915.47	-1052.36	-24.97

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	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>y</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>y</sub> kip-ft	Torque kip-ft
Dead+Wind 45 deg - Service	57.26	14.62	-14.63	-1573.03	-1498.08	-29.21
Dead+Wind 60 deg - Service	57.26	17.91	-10.34	-1126.97	-1840.28	-31.47
Dead+Wind 90 deg - Service	57.26	20.69	0.01	-50.39	-2129.20	-29.54
Dead+Wind 120 deg - Service	57.26	17.92	10.36	1025.83	-1841.71	-19.69
Dead+Wind 135 deg - Service	57.26	14.64	14.65	1471.47	-1500.14	-12.56
Dead+Wind 150 deg - Service	57.26	10.35	17.94	1813.30	-1054.84	-4.57
Dead+Wind 180 deg - Service	57.26	0.01	20.70	2101.01	20.58	11.77
Dead+Wind 210 deg - Service	57.26	-10.34	17.92	1811.87	1096.37	24.96
Dead+Wind 225 deg - Service	57.26	-14.62	14.63	1469.45	1542.13	29.21
Dead+Wind 240 deg - Service	57.26	-17.91	10.34	1023.37	1884.31	31.47
Dead+Wind 270 deg - Service	57.26	-20.69	-0.01	-53.23	2173.20	29.53
Dead+Wind 300 deg - Service	57.26	-17.92	-10.36	-1129.44	1885.70	19.69
Dead+Wind 315 deg - Service	57.26	-14.64	-14.65	-1575.07	1544.13	12.56
Dead+Wind 330 deg - Service	57.26	-10.35	-17.93	-1916.88	1098.83	4.57

### Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-57.26	0.00	-0.00	57.26	-0.00	0.000%
2	-0.04	-57.26	-74.74	0.04	57.26	74.74	0.005%
3	37.31	-57.26	-64.71	-37.31	57.26	64.70	0.006%
4	52.79	-57.26	-52.82	-52.79	57.26	52.82	0.006%
5	64.67	-57.26	-37.34	-64.66	57.26	37.33	0.006%
6	74.70	-57.26	0.04	-74.69	57.26	-0.04	0.006%
7	64.71	-57.26	37.41	-64.70	57.26	-37.40	0.005%
8	52.85	-57.26	52.88	-52.84	57.26	-52.88	0.005%
9	37.38	-57.26	64.75	-37.38	57.26	-64.75	0.005%
10	0.04	-57.26	74.74	-0.04	57.26	-74.74	0.005%
11	-37.31	-57.26	64.71	37.31	57.26	-64.70	0.005%
12	-52.79	-57.26	52.82	52.79	57.26	-52.82	0.005%
13	-64.67	-57.26	37.34	64.66	57.26	-37.33	0.006%
14	-74.70	-57.26	-0.04	74.69	57.26	0.04	0.006%
15	-64.71	-57.26	-37.41	64.70	57.26	37.40	0.006%
16	-52.85	-57.26	-52.88	52.84	57.26	52.88	0.006%
17	-37.38	-57.26	-64.75	37.38	57.26	64.75	0.005%
18	0.00	-82.26	0.00	-0.00	82.26	-0.00	0.000%
19	-0.03	-82.26	-82.90	0.03	82.26	82.89	0.007%
20	41.41	-82.26	-71.78	-41.40	82.26	71.77	0.007%
21	58.57	-82.26	-58.60	-58.57	82.26	58.59	0.007%
22	71.75	-82.26	-41.42	-71.74	82.26	41.42	0.007%
23	82.87	-82.26	0.03	-82.86	82.26	-0.03	0.007%
24	71.78	-82.26	41.48	-71.77	82.26	-41.48	0.007%
25	58.62	-82.26	58.64	-58.61	82.26	-58.64	0.007%
26	41.46	-82.26	71.81	-41.46	82.26	-71.81	0.007%
27	0.03	-82.26	82.90	-0.03	82.26	-82.89	0.007%
28	-41.41	-82.26	71.78	41.40	82.26	-71.77	0.007%
29	-58.57	-82.26	58.60	58.57	82.26	-58.59	0.007%
30	-71.75	-82.26	41.42	71.74	82.26	-41.42	0.007%
31	-82.87	-82.26	-0.03	82.86	82.26	0.03	0.007%
32	-71.78	-82.26	-41.48	71.77	82.26	41.47	0.007%
33	-58.62	-82.26	-58.64	58.61	82.26	58.64	0.007%
34	-41.46	-82.26	-71.81	41.46	82.26	71.80	0.007%
35	-0.01	-57.26	-20.70	0.01	57.26	20.70	0.002%
36	10.34	-57.26	-17.92	-10.34	57.26	17.92	0.002%
37	14.62	-57.26	-14.63	-14.62	57.26	14.63	0.002%
38	17.91	-57.26	-10.34	-17.91	57.26	10.34	0.002%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
39	20.69	-57.26	0.01	-20.69	57.26	-0.01	0.002%
40	17.92	-57.26	10.36	-17.92	57.26	-10.36	0.002%
41	14.64	-57.26	14.65	-14.64	57.26	-14.65	0.002%
42	10.36	-57.26	17.94	-10.35	57.26	-17.94	0.002%
43	0.01	-57.26	20.70	-0.01	57.26	-20.70	0.002%
44	-10.34	-57.26	17.92	10.34	57.26	-17.92	0.002%
45	-14.62	-57.26	14.63	14.62	57.26	-14.63	0.002%
46	-17.91	-57.26	10.34	17.91	57.26	-10.34	0.002%
47	-20.69	-57.26	-0.01	20.69	57.26	0.01	0.002%
48	-17.92	-57.26	-10.36	17.92	57.26	10.36	0.002%
49	-14.64	-57.26	-14.65	14.64	57.26	14.65	0.002%
50	-10.36	-57.26	-17.94	10.35	57.26	17.93	0.002%

### Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	6	0.00000001	0.00005735
2	Yes	6	0.00000001	0.00032183
3	Yes	6	0.00000001	0.00032929
4	Yes	6	0.00000001	0.00033414
5	Yes	6	0.00000001	0.00033754
6	Yes	6	0.00000001	0.00033626
7	Yes	6	0.00000001	0.00032745
8	Yes	6	0.00000001	0.00032347
9	Yes	6	0.00000001	0.00032115
10	Yes	6	0.00000001	0.00032213
11	Yes	6	0.00000001	0.00032804
12	Yes	6	0.00000001	0.00033143
13	Yes	6	0.00000001	0.00033426
14	Yes	6	0.00000001	0.00033597
15	Yes	6	0.00000001	0.00033119
16	Yes	6	0.00000001	0.00032730
17	Yes	6	0.00000001	0.00032369
18	Yes	6	0.00000001	0.00010234
19	Yes	6	0.00000001	0.00047367
20	Yes	6	0.00000001	0.00048453
21	Yes	6	0.00000001	0.00049110
22	Yes	6	0.00000001	0.00049548
23	Yes	6	0.00000001	0.00049262
24	Yes	6	0.00000001	0.00047913
25	Yes	6	0.00000001	0.00047291
26	Yes	6	0.00000001	0.00046930
27	Yes	6	0.00000001	0.00047127
28	Yes	6	0.00000001	0.00048128
29	Yes	6	0.00000001	0.00048661
30	Yes	6	0.00000001	0.00049072
31	Yes	6	0.00000001	0.00049228
32	Yes	6	0.00000001	0.00048501
33	Yes	6	0.00000001	0.00047967
34	Yes	6	0.00000001	0.00047505
35	Yes	6	0.00000001	0.00031830
36	Yes	6	0.00000001	0.00032328
37	Yes	6	0.00000001	0.00032605
38	Yes	6	0.00000001	0.00032807
39	Yes	6	0.00000001	0.00032585

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40	Yes	6	0.00000001	0.00031653
41	Yes	6	0.00000001	0.00031139
42	Yes	6	0.00000001	0.00030761
43	Yes	6	0.00000001	0.00030735
44	Yes	6	0.00000001	0.00031528
45	Yes	6	0.00000001	0.00032009
46	Yes	6	0.00000001	0.00032415
47	Yes	6	0.00000001	0.00032708
48	Yes	6	0.00000001	0.00032377
49	Yes	6	0.00000001	0.00032108
50	Yes	6	0.00000001	0.00031882

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	180 - 160	3.009	50	0.1533	0.1326
T2	160 - 140	2.360	50	0.1500	0.1310
T3	140 - 120	1.749	50	0.1273	0.1052
T4	120 - 100	1.240	50	0.1014	0.0719
T5	100 - 80	0.833	50	0.0764	0.0461
T6	80 - 60	0.538	50	0.0577	0.0319
T7	60 - 40	0.312	50	0.0422	0.0211
T8	40 - 20	0.150	50	0.0264	0.0120
T9	20 - 0	0.050	42	0.0132	0.0056

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
180.00	15' 2.5" Dia omni	50	3.009	0.1533	0.1326	694671
178.00	Rohn 6' Side-Arm(1)	50	2.943	0.1534	0.1330	694671
177.00	15' 2.5" Dia omni	50	2.911	0.1535	0.1332	694671
176.00	9-ft Omni	50	2.878	0.1536	0.1334	694671
164.00	AIR21 B4A B12P B5P	50	2.488	0.1520	0.1330	216866
157.00	(2) 7770.00	50	2.264	0.1477	0.1286	114378
146.00	12' x 2" Dia Omni	50	1.924	0.1353	0.1148	51040
142.00	Rohn 6' Side-Arm(1)	50	1.806	0.1299	0.1084	42642
135.00	Rohn 6'x14' Boom Gate (3)	50	1.612	0.1207	0.0967	41531
51.00	GPS	50	0.232	0.0350	0.0167	75380
50.00	1' Standoff Pipe	50	0.223	0.0342	0.0163	76039

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	180 - 160	11.034	34	0.5302	0.5305
T2	160 - 140	8.787	34	0.5183	0.5150

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T3	140 - 120	6.647	34	0.4507	0.4258
T4	120 - 100	4.802	34	0.3701	0.3041
T5	100 - 80	3.273	34	0.2861	0.1995
T6	80 - 60	2.134	34	0.2196	0.1391
T7	60 - 40	1.248	34	0.1625	0.0922
T8	40 - 20	0.605	34	0.1027	0.0526
T9	20 - 0	0.202	26	0.0517	0.0244

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
180.00	15' 2.5" Dia omni	34	11.034	0.5302	0.5305	268271
178.00	Rohn 6' Side-Arm(1)	34	10.808	0.5304	0.5307	268271
177.00	15' 2.5" Dia omni	34	10.695	0.5305	0.5307	268271
176.00	9-ft Omni	34	10.582	0.5305	0.5307	268271
164.00	AIR21 B4A B12P B5P	34	9.233	0.5247	0.5231	83730
157.00	(2) 7770.00	34	8.454	0.5114	0.5064	41748
146.00	12' x 2" Dia Omni	34	7.265	0.4745	0.4589	17615
142.00	Rohn 6' Side-Arm(1)	34	6.850	0.4587	0.4372	14426
135.00	Rohn 6'x14' Boom Gate (3)	34	6.156	0.4307	0.3962	13716
51.00	GPS	34	0.929	0.1352	0.0731	19564
50.00	1' Standoff Pipe	34	0.896	0.1322	0.0710	19688

### Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load/Allowable	Allowable Ratio	Criteria
T1	180	Leg	A325N	0.8750	4	1.35	26.45	0.051	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	1.68	6.44	0.261	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	1.42	6.44	0.221	1.333	Bolt Shear
		Top Girt	A325N	0.6250	2	0.41	6.44	0.063	1.333	Bolt Shear
T2	160	Leg	A325N	1.0000	4	9.48	34.56	0.274	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	4.76	6.44	0.740	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	3.94	6.44	0.611	1.333	Bolt Shear
		Top Girt	A325N	0.6250	2	2.98	6.44	0.463	1.333	Bolt Shear
T3	140	Leg	A325N	1.0000	6	14.57	34.56	0.422	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	6.29	6.44	0.977	1.333	Bolt Shear
		Horizontal	A325N	0.6250	2	5.92	6.44	0.918	1.333	Bolt Shear
		Top Girt	A325N	0.6250	2	4.48	6.44	0.695	1.333	Bolt Shear
T4	120	Leg	A325N	1.0000	8	17.40	34.56	0.503	1.333	Bolt Tension
		Diagonal	A325N	0.6250	3	6.43	6.44	0.998	1.333	Bolt Shear

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T5	100	Horizontal	A325N	0.6250	2	6.72	6.44	1.043	✓	1.333 Bolt Shear
		Top Girt	A325N	0.6250	2	6.18	6.44	0.959	✓	1.333 Bolt Shear
		Leg	A325N	1.0000	12	15.18	34.56	0.439	✓	1.333 Bolt Tension
		Diagonal	A325N	0.7500	3	8.07	9.28	0.870	✓	1.333 Bolt Shear
T6	80	Horizontal	A325N	0.7500	2	7.31	9.28	0.787	✓	1.333 Bolt Shear
		Top Girt	A325N	0.7500	2	7.14	9.28	0.769	✓	1.333 Bolt Shear
		Leg	A325N	1.0000	12	19.10	34.56	0.553	✓	1.333 Bolt Tension
		Diagonal	A325N	0.7500	3	7.77	9.28	0.837	✓	1.333 Bolt Shear
T7	60	Horizontal	A325N	0.7500	2	7.74	9.28	0.835	✓	1.333 Bolt Shear
		Top Girt	A325N	0.7500	2	7.51	9.28	0.810	✓	1.333 Bolt Shear
		Leg	A325N	1.0000	16	17.00	34.56	0.492	✓	1.333 Bolt Tension
		Diagonal	A325N	0.7500	3	7.45	9.28	0.803	✓	1.333 Bolt Shear
T8	40	Horizontal	A325N	0.7500	2	7.79	9.28	0.839	✓	1.333 Bolt Shear
		Top Girt	A325N	0.7500	2	7.80	9.28	0.841	✓	1.333 Bolt Shear
		Leg	A325N	1.0000	16	19.44	34.56	0.562	✓	1.333 Bolt Tension
		Diagonal	A325N	0.7500	3	7.67	9.28	0.826	✓	1.333 Bolt Shear
T9	20	Horizontal	A325N	0.7500	2	8.51	9.28	0.917	✓	1.333 Bolt Shear
		Top Girt	A325N	0.7500	2	8.34	9.28	0.899	✓	1.333 Bolt Shear
		Leg	A354-BC	1.0000	24	14.53	32.40	0.448	✓	1.333 Bolt Tension
		Diagonal	A325N	0.7500	3	7.53	9.28	0.812	✓	1.333 Bolt Shear
		Horizontal	A325N	0.7500	2	8.90	9.28	0.959	✓	1.333 Bolt Shear
		Top Girt	A325N	0.7500	2	8.75	9.28	0.943	✓	1.333 Bolt Shear

### Compression Checks

### Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	180 - 160	ROHN 3 STD	20.00	6.67	68.8 K=1.00	21.168	2.2285	-8.59	47.17	0.182
T2	160 - 140	ROHN 4 STD	20.00	6.67	53.0 K=1.00	23.877	3.1741	-45.85	75.79	0.605 ✓
T3	140 - 120	ROHN 5 EH	20.04	6.68	43.6 K=1.00	25.320	6.1120	-101.83	154.75	0.658 ✓
T4	120 - 100	ROHN 6 EH	20.04	6.68	36.5 K=1.00	26.311	8.4049	-160.70	221.14	0.727 ✓
T5	100 - 80	ROHN 8 EH	20.05	10.02	41.8 K=1.00	25.578	12.7627	-208.29	326.44	0.638 ✓

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P/P <sub>a</sub>
T6	80 - 60	ROHN 10 EH	20.05	10.03	33.2 K=1.00	26.753	16.1007	-261.30	430.75	0.607
T7	60 - 40	ROHN 10 EH	20.06	10.03	33.2 K=1.00	26.752	16.1007	-310.44	430.73	0.721
T8	40 - 20	ROHN 12 EH	20.05	10.03	27.8 K=1.00	27.426	19.2423	-356.88	527.73	0.676
T9	20 - 0	ROHN 12 EH	20.05	10.03	27.8 K=1.00	27.426	19.2423	-401.36	527.73	0.761

### Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P/P <sub>a</sub>
T1	180 - 160	ROHN 2 STD	7.94	7.67	117.0 K=1.00	10.914	1.0745	-5.05	11.73	0.431
T2	160 - 140	ROHN 2.5 STD	7.96	7.62	96.5 K=1.00	14.505	1.7040	-14.29	24.72	0.578
T3	140 - 120	ROHN 2.5 STD	8.58	8.21	104.0 K=1.00	13.250	1.7040	-18.63	22.58	0.825
T4	120 - 100	ROHN 2.5 STD	9.29	8.89	112.6 K=1.00	11.718	1.7040	-19.22	19.97	0.963
T5	100 - 80	ROHN 3 EH	12.60	12.01	126.9 K=1.00	9.277	3.0159	-23.81	27.98	0.851
T6	80 - 60	ROHN 3 EH	13.40	12.73	134.4 K=1.00	8.264	3.0159	-23.17	24.92	0.930
T7	60 - 40	ROHN 3 EH	14.32	13.69	144.6 K=1.00	7.143	3.0159	-21.84	21.54	1.014
T8	40 - 20	ROHN 3.5 EH	15.24	14.53	133.5 K=1.00	8.383	3.6784	-22.85	30.84	0.741
T9	20 - 0	ROHN 3.5 EH	16.20	15.52	142.6 K=1.00	7.346	3.6784	-22.17	27.02	0.820

### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P/P <sub>a</sub>
T1	180 - 160	ROHN 1.5 STD	8.60	4.15	80.1 K=1.00	18.999	0.7995	-2.81	15.19	0.185
T2	160 - 140	ROHN 2 STD	8.68	4.15	63.3 K=1.00	22.141	1.0745	-7.81	23.79	0.328
T3	140 - 120	ROHN 2 STD	10.10	4.82	73.4 K=1.00	20.294	1.0745	-11.78	21.81	0.540
T4	120 - 100	ROHN 2 STD	12.22	5.83	88.9 K=1.00	17.170	1.0745	-13.45	18.45	0.729
T5	100 - 80	ROHN 2 X-STR	14.13	6.71	105.0	13.532	1.4773	-14.61	19.99	0.731

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	<b>Project</b> 180' Lattice Tower - 53 Dayton Road Waterford, CT	<b>Date</b> 16:53:44 04/22/15
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T6	80 - 60	ROHN 2.5 STD	16.58	7.84	K=1.00 99.3	13.063	1.7040	-15.49	22.26	0.696
T7	60 - 40	ROHN 2.5 STD	19.16	9.13	K=1.00 115.6	10.900	1.7040	-15.57	18.57	0.838
T8	40 - 20	ROHN 3 STD	21.73	10.33	K=1.00 106.6	12.129	2.2285	-16.90	27.03	0.625
T9	20 - 0	ROHN 3 STD	24.23	11.58	K=1.00 119.5	10.359	2.2285	-16.38	23.09	0.709

### Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	180 - 160	ROHN 1.5 STD	8.54	4.12	79.5 K=1.00	17.139	0.7995	-0.81	13.70	0.059
T2	160 - 140	ROHN 2 STD	8.63	4.17	63.6 K=1.00	19.331	1.0745	-5.90	20.77	0.284
T3	140 - 120	ROHN 2 STD	8.71	4.17	63.5 K=1.00	19.334	1.0745	-8.95	20.78	0.431
T4	120 - 100	ROHN 2 STD	10.79	5.16	78.7 K=1.00	17.251	1.0745	-12.36	18.54	0.667
T5	100 - 80	ROHN 2 X-STR	12.93	6.19	96.9 K=1.00	14.443	1.4773	-14.27	21.34	0.669
T6	80 - 60	ROHN 2.5 STD	15.33	7.31	92.5 K=1.00	15.149	1.7040	-15.02	25.81	0.582
T7	60 - 40	ROHN 2.5 STD	17.83	8.47	107.2 K=1.00	12.683	1.7040	-15.60	21.61	0.722
T8	40 - 20	ROHN 3 STD	20.48	9.79	101.0 K=1.00	13.762	2.2285	-16.69	30.67	0.544
T9	20 - 0	ROHN 3 STD	22.98	10.96	113.0 K=1.00	11.647	2.2285	-17.50	25.96	0.674

### Inner Bracing Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P P <sub>a</sub>
T1	180 - 160	L2x2x1/8	4.27	4.27	128.9 K=1.00	8.989	0.4844	-0.01	4.35	0.003
T2	160 - 140	L2x2x1/8	4.31	4.31	130.3 K=1.00	8.802	0.4844	-0.10	4.26	0.024
T3	140 - 120	L2x2x1/8	4.35	4.35	131.5 K=1.00	8.641	0.4844	-0.16	4.19	0.037
T4	120 - 100	L2x2x1/8	5.39	5.39	162.9 K=1.00	5.631	0.4844	-0.21	2.73	0.078



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	<b>Client</b>	T-Mobile	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T5	100 - 80	L2x2x1/8	6.47	6.47	195.1 K=1.00	3.921	0.4844	-0.25	1.90	0.130
T6	80 - 60	L2 1/2x2 1/2x3/16	7.67	7.67	185.8 K=1.00	4.325	0.9020	-0.26	3.90	0.067
T7	60 - 40	L3x3x3/16	8.92	8.92	179.5 K=1.00	4.635	1.0900	-0.27	5.05	0.053
T8	40 - 20	L3 1/2x3 1/2x1/4	10.24	10.24	177.1 K=1.00	4.763	1.6900	-0.29	8.05	0.036
T9	20 - 0	L3 1/2x3 1/2x1/4	11.49	11.49	198.7 K=1.00	3.783	1.6900	-0.30	6.39	0.047

### Tension Checks

### Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T1	180 - 160	ROHN 3 STD	20.00	6.67	68.8	30.000	2.2285	5.40	66.85	0.081
T2	160 - 140	ROHN 4 STD	20.00	6.67	53.0	30.000	3.1741	37.93	95.22	0.398
T3	140 - 120	ROHN 5 EH	20.04	6.68	43.6	30.000	6.1120	87.44	183.36	0.477
T4	120 - 100	ROHN 6 EH	20.04	6.68	36.5	30.000	8.4049	139.16	252.15	0.552
T5	100 - 80	ROHN 8 EH	20.05	10.02	41.8	30.000	12.7627	182.21	382.88	0.476
T6	80 - 60	ROHN 10 EH	20.05	10.03	33.2	30.000	16.1007	229.23	483.02	0.475
T7	60 - 40	ROHN 10 EH	20.06	10.03	33.2	30.000	16.1007	271.93	483.02	0.563
T8	40 - 20	ROHN 12 EH	20.05	10.03	27.8	30.000	19.2423	310.99	577.27	0.539
T9	20 - 0	ROHN 12 EH	20.05	10.03	27.8	30.000	19.2423	348.61	577.27	0.604

### Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T1	180 - 160	ROHN 2 STD	7.94	7.67	117.0	25.200	1.0745	4.99	27.08	0.184
T2	160 - 140	ROHN 2.5 STD	7.96	7.62	96.5	25.200	1.7040	14.21	42.94	0.331

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	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T3	140 - 120	ROHN 2.5 STD	8.36	8.00	101.3	25.200	1.7040	18.73	42.94	0.436
T4	120 - 100	ROHN 2.5 STD	9.04	8.65	109.5	25.200	1.7040	19.10	42.94	0.445
T5	100 - 80	ROHN 3 EH	12.25	11.66	123.1	25.200	3.0159	23.92	76.00	0.315
T6	80 - 60	ROHN 3 EH	12.99	12.32	130.1	25.200	3.0159	22.92	76.00	0.302
T7	60 - 40	ROHN 3 EH	13.85	13.23	139.7	25.200	3.0159	21.89	76.00	0.288
T8	40 - 20	ROHN 3.5 EH	14.77	14.07	129.2	25.200	3.6784	22.33	92.70	0.241
T9	20 - 0	ROHN 3.5 EH	15.71	15.04	138.1	25.200	3.6784	21.82	92.70	0.235

### Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T1	180 - 160	ROHN 1.5 STD	8.60	4.15	80.1	30.000	0.7995	2.85	23.98	0.119
T2	160 - 140	ROHN 2 STD	8.68	4.15	63.3	30.000	1.0745	7.87	32.24	0.244
T3	140 - 120	ROHN 2 STD	10.10	4.82	73.4	30.000	1.0745	11.54	32.24	0.358
T4	120 - 100	ROHN 2 STD	12.22	5.83	88.9	30.000	1.0745	13.18	32.24	0.409
T5	100 - 80	ROHN 2 X-STR	14.13	6.71	105.0	30.000	1.4773	14.25	44.32	0.322
T6	80 - 60	ROHN 2.5 STD	16.58	7.84	99.3	21.600	1.7040	15.18	36.81	0.412
T7	60 - 40	ROHN 2.5 STD	19.16	9.13	115.6	21.600	1.7040	15.31	36.81	0.416
T8	40 - 20	ROHN 3 STD	21.73	10.33	106.6	21.600	2.2285	17.02	48.13	0.354
T9	20 - 0	ROHN 3 STD	24.23	11.58	119.5	21.600	2.2285	17.79	48.13	0.370

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio $\frac{P}{P_a}$
T1	180 - 160	ROHN 1.5 STD	8.54	4.12	79.5	25.200	0.7995	0.81	20.15	0.040

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	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P/P <sub>a</sub>
T2	160 - 140	ROHN 2 STD	8.63	4.17	63.6	25.200	1.0745	5.96	27.08	0.220
T3	140 - 120	ROHN 2 STD	8.71	4.17	63.5	25.200	1.0745	8.39	27.08	0.310
T4	120 - 100	ROHN 2 STD	10.79	5.16	78.7	25.200	1.0745	12.08	27.08	0.446
T5	100 - 80	ROHN 2 X-STR	12.93	6.19	96.9	25.200	1.4773	13.75	37.23	0.369
T6	80 - 60	ROHN 2.5 STD	15.33	7.31	92.5	25.200	1.7040	14.65	42.94	0.341
T7	60 - 40	ROHN 2.5 STD	17.83	8.47	107.2	25.200	1.7040	15.21	42.94	0.354
T8	40 - 20	ROHN 3 STD	20.48	9.79	101.0	25.200	2.2285	16.61	56.16	0.296
T9	20 - 0	ROHN 3 STD	22.98	10.96	113.0	25.200	2.2285	16.73	56.16	0.298

### Inner Bracing Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>n</sub> ft	Kl/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P K	Allow. P <sub>a</sub> K	Ratio P/P <sub>a</sub>
T1	180 - 160	L2x2x1/8	4.27	4.27	81.8	30.000	0.4844	0.01	14.53	0.001
T2	160 - 140	L2x2x1/8	4.31	4.31	82.7	30.000	0.4844	0.10	14.53	0.007
T3	140 - 120	L2x2x1/8	4.35	4.35	83.5	30.000	0.4844	0.16	14.53	0.011
T4	120 - 100	L2x2x1/8	5.39	5.39	103.4	30.000	0.4844	0.21	14.53	0.015
T5	100 - 80	L2x2x1/8	6.47	6.47	123.9	30.000	0.4844	0.25	14.53	0.017
T6	80 - 60	L2 1/2x2 1/2x3/16	7.67	7.67	118.2	30.000	0.9020	0.26	27.06	0.010
T7	60 - 40	L3x3x3/16	8.92	8.92	113.9	30.000	1.0900	0.27	32.70	0.008
T8	40 - 20	L3 1/2x3 1/2x1/4	10.24	10.24	112.7	30.000	1.6900	0.29	50.70	0.006
T9	20 - 0	L3 1/2x3 1/2x1/4	11.49	11.49	126.5	30.000	1.6900	0.30	50.70	0.006

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P <sub>allow</sub> K	% Capacity	Pass Fail
T1	180 - 160	Leg	ROHN 3 STD	3	-8.59	62.88	13.7	Pass
T2	160 - 140	Leg	ROHN 4 STD	42	-45.85	101.02	45.4	Pass

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	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P <sub>allow</sub> K	% Capacity	Pass Fail
T3	140 - 120	Leg	ROHN 5 EH	81	-101.83	206.29	49.4	Pass
T4	120 - 100	Leg	ROHN 6 EH	120	-160.70	294.78	54.5	Pass
T5	100 - 80	Leg	ROHN 8 EH	159	-208.29	435.15	47.9	Pass
T6	80 - 60	Leg	ROHN 10 EH	186	-261.30	574.19	45.5	Pass
T7	60 - 40	Leg	ROHN 10 EH	213	-310.44	574.16	54.1	Pass
T8	40 - 20	Leg	ROHN 12 EH	240	-356.88	703.47	50.7	Pass
T9	20 - 0	Leg	ROHN 12 EH	267	-401.36	703.47	57.1	Pass
T1	180 - 160	Diagonal	ROHN 2 STD	14	-5.05	15.63	32.3	Pass
T2	160 - 140	Diagonal	ROHN 2.5 STD	53	-14.29	32.95	43.4	Pass
T3	140 - 120	Diagonal	ROHN 2.5 STD	93	-18.63	30.10	55.5 (b) 61.9	Pass
T4	120 - 100	Diagonal	ROHN 2.5 STD	132	-19.22	26.62	73.3 (b) 72.2	Pass
T5	100 - 80	Diagonal	ROHN 3 EH	171	-23.81	37.30	74.8 (b) 63.8	Pass
T6	80 - 60	Diagonal	ROHN 3 EH	198	-23.17	33.22	65.3 (b) 69.7	Pass
T7	60 - 40	Diagonal	ROHN 3 EH	225	-21.84	28.72	76.1	Pass
T8	40 - 20	Diagonal	ROHN 3.5 EH	252	-22.85	41.11	55.6	Pass
T9	20 - 0	Diagonal	ROHN 3.5 EH	279	-22.17	36.02	62.0 (b) 61.5	Pass
T1	180 - 160	Horizontal	ROHN 1.5 STD	13	-2.81	20.25	13.9	Pass
T2	160 - 140	Horizontal	ROHN 2 STD	52	-7.81	31.71	16.6 (b) 24.6	Pass
T3	140 - 120	Horizontal	ROHN 2 STD	91	-11.78	29.07	45.8 (b) 40.5	Pass
T4	120 - 100	Horizontal	ROHN 2 STD	130	-13.45	24.59	68.9 (b) 54.7	Pass
T5	100 - 80	Horizontal	ROHN 2 X-STR	169	-14.61	26.65	78.3 (b) 54.8	Pass
T6	80 - 60	Horizontal	ROHN 2.5 STD	196	-15.49	29.67	59.1 (b) 52.2	Pass
T7	60 - 40	Horizontal	ROHN 2.5 STD	223	-15.57	24.76	62.6 (b) 62.9	Pass
T8	40 - 20	Horizontal	ROHN 3 STD	250	-16.90	36.03	63.0 (b) 46.9	Pass
T9	20 - 0	Horizontal	ROHN 3 STD	277	-16.38	30.77	68.8 (b) 53.2	Pass
T1	180 - 160	Top Girt	ROHN 1.5 STD	6	-0.81	18.26	71.9 (b) 4.4	Pass
T2	160 - 140	Top Girt	ROHN 2 STD	45	-5.90	27.69	4.7 (b) 21.3	Pass
T3	140 - 120	Top Girt	ROHN 2 STD	84	-8.95	27.69	34.7 (b) 32.3	Pass
T4	120 - 100	Top Girt	ROHN 2 STD	123	-12.36	24.71	52.1 (b) 50.0	Pass
T5	100 - 80	Top Girt	ROHN 2 X-STR	162	-14.27	28.44	72.0 (b) 50.2	Pass
T6	80 - 60	Top Girt	ROHN 2.5 STD	189	-15.02	34.41	57.7 (b) 43.7	Pass
T7	60 - 40	Top Girt	ROHN 2.5 STD	216	-15.60	28.81	60.7 (b) 54.1	Pass
T8	40 - 20	Top Girt	ROHN 3 STD	243	-16.69	40.88	63.1 (b) 40.8	Pass
T9	20 - 0	Top Girt	ROHN 3 STD	270	-17.50	34.60	67.5 (b) 50.6	Pass
T1	180 - 160	Inner Bracing	L2x2x1/8	38	-0.01	5.80	70.7 (b) 0.2	Pass
T2	160 - 140	Inner Bracing	L2x2x1/8	77	-0.10	5.68	1.8	Pass
T3	140 - 120	Inner Bracing	L2x2x1/8	116	-0.16	5.58	2.8	Pass
T4	120 - 100	Inner Bracing	L2x2x1/8	155	-0.21	3.64	5.9	Pass

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	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P <sub>allow</sub> K	% Capacity	Pass Fail
T5	100 - 80	Inner Bracing	L2x2x1/8	183	-0.25	2.53	9.8	Pass
T6	80 - 60	Inner Bracing	L2 1/2x2 1/2x3/16	209	-0.26	5.20	5.0	Pass
T7	60 - 40	Inner Bracing	L3x3x3/16	236	-0.27	6.73	4.0	Pass
T8	40 - 20	Inner Bracing	L3 1/2x3 1/2x1/4	263	-0.29	10.73	2.7	Pass
T9	20 - 0	Inner Bracing	L3 1/2x3 1/2x1/4	291	-0.30	8.52	3.6	Pass
Summary								
Leg (T9)							57.1	Pass
Diagonal (T7)							76.1	Pass
Horizontal (T4)							78.3	Pass
Top Girt (T4)							72.0	Pass
Inner Bracing (T5)							9.8	Pass
Bolt Checks							78.3	Pass
<b>RATING =</b>							<b>78.3</b>	<b>Pass</b>

## Network Modernization RFDS v3.0



Site ID <b>CT11041D</b>	Latitude 41.37774
Site Name Waterford/ I-95/ X82	Longitude -72.13940
Address 53 Dayton Road, Waterford, CT 06385	Site Type Structure (Non-Building)
Market Connecticut	Site Class Self Support Tower
	Landlord Tower Co.

Configuration

702CU

Approvals	
Market RF	
Market Development	
RFDS Revision	
RFDS Final	<input checked="" type="checkbox"/>
Work Order #	
Date	10/10/2013
NOC#	(888) 218-6664

### Site Information

Existing Configuration				Cabinet #	Proposed Configuration			
1	2	3	4		1	2	3	4
UMTS	GSM/UMTS/LTE	GSM		Technology	UMTS	GSM/UMTS/LTE	GSM	
3106	6102	S8000		Cabinet type	3106	6102	S8000	
	2			CBU				
	1			DUW30		2		
	1			DUL20				
				DUG20		1		
				DUS41			1	
				RBS6601				
				aTRU/TRX				
				RU22 B4				
				RUS01 B2				
	6			RUS01 B4		6		

- Relocate cabinet
- Add cabinet
- Swap cabinet
- Remove cabinet
- Make cabinet dark

#### Scope of Work

Swap DUL with DUS41

### ALPHA - Scope of Work

- Add new mount
- Relocate antenna
- Add antenna
- Swap antenna
- Remove antenna
- Add TMA
- Swap TMA
- Remove TMA
- Add RRU
- Swap existing RRU
- Remove RRU
- Consolidate coax cables
- Add coax cables
- Add fiber cables
- Add hybrid combiner
- Add filter combiner

Swap a dummy antenna with L700 passive antenna. Add RRUS at antenna. Use spare fiber for LTE 700. Swap a dummy with B4 LTE antenna.

### BETA - Scope of Work

- Add new mount
- Relocate antenna
- Add antenna
- Swap antenna
- Remove antenna
- Add TMA
- Swap TMA
- Remove TMA
- Add RRU
- Swap existing RRU
- Remove RRU
- Consolidate coax cables
- Add coax cables
- Add fiber cables
- Add hybrid combiner
- Add filter combiner

Swap a dummy antenna with L700 passive antenna. Add RRUS at antenna. Use spare fiber for LTE 700. Swap a dummy with B4 LTE antenna.

### GAMMA - Scope of Work

- Add new mount
- Relocate antenna
- Add antenna
- Swap antenna
- Remove antenna
- Add TMA
- Swap TMA
- Remove TMA
- Add RRU
- Swap existing RRU
- Remove RRU
- Consolidate coax cables
- Add coax cables
- Add fiber cables
- Add hybrid combiner
- Add filter combiner

Swap a dummy antenna with L700 passive antenna. Add RRUS at antenna. Use spare fiber for LTE 700. Swap a dummy with B4 LTE antenna.

### DELTA - Scope of Work

- Add new mount
- Relocate antenna
- Add antenna
- Swap antenna
- Remove antenna
- Add TMA
- Swap TMA
- Remove TMA
- Add RRU
- Swap existing RRU
- Remove RRU
- Consolidate coax cables
- Add coax cables
- Add fiber cables
- Add hybrid combiner
- Add filter combiner

# Network Modernization RFDS v3.0



<b>Site ID</b> CT11041D	Latitude 41.37774
<b>Site Name</b> Waterford/ I-95/ X82	Longitude -72.13940
<b>Address</b> 53 Dayton Road, Waterford, CT 06385	Site Type Structure (Non-Building)
<b>Market</b> Connecticut	Site Class Self Support Tower
	Landlord Tower Co.

702CU

Approvals	
Market RF	
Market Development	
RFDS Revision	
RFDS Final	<input checked="" type="checkbox"/>
Date	10/10/2013

## ALPHA (view from behind)

Existing Configuration				Mount	Proposed Configuration			
<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>		<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
Dual pole RR90_17_02DP EMS	GSM/UMTS UMTS/LTE B2 B4 A P Quad pole AIR21 B2A/B4P Ericsson 165 30 Yes Yes 2 2 0 2	Dual pole RR90_17_02DP EMS		Technology Band Active/Passive Ant. Type Ant. Model Ant. Vendor Ant. Height Azimuth RET deployed E-Tilt M-Tilt	LTE B4 A Quad pole AIR21 B4A/B12P-8 Ericsson 165 30 Yes 3 0	GSM/UMTS UMTS B2 B4 A P Quad pole AIR21 B2A/B4P Ericsson 165 30 Yes Yes 2 2 0 2	LTE B12 P Dual pole LNX-6515DS-VTM Commscope 167 30 Yes 2 0	
	1 dd B4			TMA # TMA Type RRU # RRU Type Used Coax # Coax Type Coax Length (ft) Fiber (CPRI) # Splitter # Combiner # Combiner Type		1 dd B4	1 RUS11_B	
	2 (LMU) 2 1-5/8" 1-5/8" 223 2					2 (LMU) 2 1-5/8" 1-5/8" 223 2		

- |  |  |
|--|--|
| <input type="checkbox"/> Add new mount           | <input checked="" type="checkbox"/> Add RRU      |
| <input type="checkbox"/> Relocate antenna        | <input type="checkbox"/> Swap existing RRU       |
| <input type="checkbox"/> Add antenna             | <input type="checkbox"/> Remove RRU              |
| <input checked="" type="checkbox"/> Swap antenna | <input type="checkbox"/> Consolidate coax cables |
| <input type="checkbox"/> Remove antenna          | <input type="checkbox"/> Add coax cables         |
| <input type="checkbox"/> Add TMA                 | <input type="checkbox"/> Add fiber cables        |
| <input type="checkbox"/> Swap TMA                | <input type="checkbox"/> Add hybrid combiner     |
| <input type="checkbox"/> Remove TMA              | <input type="checkbox"/> Add filter combiner     |

### Scope of work

Swap a dummy antenna with L700 passive antenna. Add RRUS at antenna. Use spare fiber for LTE 700. Swap a dummy with B4 LTE antenna.

## BETA (view from behind)

Existing Configuration				Mount	Proposed Configuration			
<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>		<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
Dual pole RR90_17_02DP EMS	GSM/UMTS UMTS/LTE B2 B4 A P Quad pole AIR21 B2A/B4P Ericsson 165 150 Yes Yes 2 2 0 4	Dual pole RR90_17_02DP EMS		Technology Band Active/Passive Ant. Type Ant. Model Ant. Vendor Ant. Height Azimuth RET deployed E-Tilt M-Tilt	LTE B4 A Quad pole AIR21 B4A/B12P-8 Ericsson 165 150 Yes 3 0	GSM/UMTS UMTS B2 B4 A P Quad pole AIR21 B2A/B4P Ericsson 165 150 Yes Yes 2 2 0 4	LTE B12 P Dual pole LNX-6515DS-VTM Commscope 167 30 Yes 2 0	
	1 dd B4			TMA # TMA Type RRU # RRU Type Used Coax # Coax Type Coax Length (ft) Fiber (CPRI) # Splitter # Combiner # Combiner Type		1 dd B4	1 RUS11_B	
	2 (LMU) 2 1-5/8" 1-5/8" 218 2					2 (LMU) 2 1-5/8" 1-5/8" 218 2		

- |  |  |
|--|--|
| <input type="checkbox"/> Add new mount           | <input checked="" type="checkbox"/> Add RRU      |
| <input type="checkbox"/> Relocate antenna        | <input type="checkbox"/> Swap existing RRU       |
| <input type="checkbox"/> Add antenna             | <input type="checkbox"/> Remove RRU              |
| <input checked="" type="checkbox"/> Swap antenna | <input type="checkbox"/> Consolidate coax cables |
| <input type="checkbox"/> Remove antenna          | <input type="checkbox"/> Add coax cables         |
| <input type="checkbox"/> Add TMA                 | <input type="checkbox"/> Add fiber cables        |
| <input type="checkbox"/> Swap TMA                | <input type="checkbox"/> Add hybrid combiner     |
| <input type="checkbox"/> Remove TMA              | <input type="checkbox"/> Add filter combiner     |

### Scope of work

Swap a dummy antenna with L700 passive antenna. Add RRUS at antenna. Use spare fiber for LTE 700. Swap a dummy with B4 LTE antenna.

Network Modernization RFDS v3.0



Site ID	CT11041D	Latitude	41.37774
Site Name	Waterford/ I-95/ X82	Longitude	-72.13940
Address	53 Dayton Road, Waterford, CT 06385	Site Type	Structure (Non-Building)
Market	Connecticut	Site Class	Self Support Tower
		Landlord	Tower Co.

Configuration  
**702CU**

Approvals	
Market RF	
Market Development	
RFDS Revision	
RFDS Final	<input checked="" type="checkbox"/>
Date	10/10/2013

GAMMA (view from behind)

Existing Configuration					Proposed Configuration			
<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Technology	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Band	LTE	B2	UMTS	LTE
Dual pole	B2 B4	Dual pole		Active/Passive	B4	A	P	B12
RR90_17_02DP	A P	RR90_17_02DP		Ant. Type	A	Quad pole	Quad pole	P
EMS	AIR21 B2A/B4P	EMS		Ant. Model	AIR21 B4A/B12P-S	AIR21 B2A/B4P	LNK-6515DS-VTM	
	Ericsson			Ant. Vendor	Ericsson	Ericsson	Commscope	
	165			Ant. Height	165	165	167	
	270			Azimuth	270	270	270	
	Yes Yes			RET deployed	Yes	Yes	Yes	
	2 5			E-Tilt	3	2	2	
	0 0			M-Tilt	0	0	0	
				TMA #		1		
	1			TMA Type		dd B4		
	dd B4			RRU #			1	
				RRU Type			RUS11 B	
	2 (LMU)	2		Used Coax #		2 (LMU)	2	
	1-5/8"	1-5/8"		Coax Type		1-5/8"	1-5/8"	
	226			Coax Length (ft)		226		
	2			Fiber (CPRI) #	2	2	2	
				Splitter #				
				Combiner #				
				Combiner Type				

- Add new mount
- Relocate antenna
- Add antenna
- Swap antenna
- Remove antenna
- Add TMA
- Swap TMA
- Remove TMA
- Add RRU
- Swap existing RRU
- Remove RRU
- Consolidate coax cables
- Add coax cables
- Add fiber cables
- Add hybrid combiner
- Add filter combiner

Scope of work

Swap a dummy antenna with L700 passive antenna. Add RRUS at antenna. Use spare fiber for LTE 700. Swap a dummy with B4 LTE antenna.

DELTA (view from behind)

Existing Configuration					Proposed Configuration			
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Technology	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
				Band				
				Active/Passive				
				Ant. Type				
				Ant. Model				
				Ant. Vendor				
				Ant. Height				
				Azimuth				
				RET deployed				
				E-Tilt				
				M-Tilt				
				TMA #				
				TMA Type				
				RRU #				
				RRU Type				
				Used Coax #				
				Coax Type				
				Coax Length (ft)				
				Fiber (CPRI) #				
				Splitter #				
				Combiner #				
				Combiner Type				

- Add new mount
- Relocate antenna
- Add antenna
- Swap antenna
- Remove antenna
- Add TMA
- Swap TMA
- Remove TMA
- Add RRU
- Swap existing RRU
- Remove RRU
- Consolidate coax cables
- Add coax cables
- Add fiber cables
- Add hybrid combiner
- Add filter combiner

Scope of work



# Product Specifications

COMMScope®

POWERED BY



## LNX-6515DS-VTM

**Andrew® Antenna, 698–896 MHz, 65° horizontal beamwidth, RET compatible**

- Excellent choice to maximize both coverage and capacity in suburban and rural applications
- Fully compatible with Andrew remote electrical tilt system for greater OpEx savings
- Exceptional horizontal pattern roll-off and strong front-to-back ratio
- Extended bandwidth allows one antenna to serve multiple frequency allocations
- Great solution to maximize network coverage and capacity
- The RF connectors are designed for IP67 rating and the radome for IP56 rating
- The values presented on this datasheet have been calculated based on N-P-BASTA White Paper version 9.6 by the NGMN Alliance

### Electrical Specifications

#### Frequency Band, MHz

Gain by all Beam Tilts, average, dBi  
Gain by all Beam Tilts Tolerance, dB

Gain by Beam Tilt, average, dBi

Beamwidth, Horizontal, degrees  
Beamwidth, Horizontal Tolerance, degrees  
Beamwidth, Vertical, degrees  
Beamwidth, Vertical Tolerance, degrees  
Beam Tilt, degrees

USLS, dB

Front-to-Back Total Power at 180° ± 30°, dB

CPR at Boresight, dB

CPR at Sector, dB

Isolation, dB

VSWR | Return Loss, dB

PIM, 3rd Order, 2 x 20 W, dBc

Input Power per Port, maximum, watts

Polarization

Impedance

#### 698–806

16.6

±0.4

0° | 16.6

4° | 16.6

8° | 16.4

65

±1

9.7

±0.6

0–8

18

25

24

15

30

1.4 | 15.6

-153

400

±45°

50 ohm

#### 806–896

16.9

±0.3

0° | 17.0

4° | 17.0

8° | 16.8

64

±0.9

8.6

±0.4

0–8

18

23

27

13

30

1.4 | 15.6

-153

400

±45°

50 ohm

### General Specifications

Antenna Brand

Andrew®

Antenna Type

DualPol®

Band

Single band

Brand

DualPol® | Teletilt®

Operating Frequency Band

698 – 896 MHz

Number of Ports, all types

2

### Mechanical Specifications

Color

Light gray

Lightning Protection

dc Ground

Radiator Material

Aluminum

# Product Specifications

COMMSCOPE®

LNx-6515DS-VTM



Radome Material	Fiberglass, UV resistant
RF Connector Interface	7-16 DIN Female
RF Connector Location	Bottom
RF Connector Quantity, total	2
Wind Loading, maximum	878.0 N @ 150 km/h 197.4 lbf @ 150 km/h
Wind Speed, maximum	241.0 km/h   149.8 mph

## Dimensions

Depth	181.0 mm   7.1 in
Length	2449.0 mm   96.4 in
Width	301.0 mm   11.9 in
Net Weight	22.8 kg   50.3 lb

## Remote Electrical Tilt (RET) Information

Model with Factory Installed AISG 1.1 Actuator	LNx-6515DS-R2M
Model with Factory Installed AISG 2.0 Actuator	LNx-6515DS-A1M
RET System	Teletilt®

## Regulatory Compliance/Certifications

Agency	Classification
RoHS 2011/65/EU	Compliant by Exemption
China RoHS SJ/T 11364-2006	Above Maximum Concentration Value (MCV)
ISO 9001:2008	Designed, manufactured and/or distributed under this quality management system



## Included Products

DB380-3 — Pipe Mounting Kit for 2.4 - 4.5 in (60 - 115 mm) OD round members. Used for wide panel antennas. Includes three clamp sets.

DB5083D — Downtilt Mounting Kit for 2.4"-4.5" (60-115 mm) OD round members. Consists of two DB5083 heavy-duty, galvanized steel downtilt mounting brackets. This kit is compatible with the DB380-3 pipe mount for panel antennas with three mounting points.

Prepared (S. 3601 read) / XKRLUND		No. 101/1551-LZA 701 6001 Uen	
Approved (Doc. 1641 read) EAB/FJB/KK (Cecilia Ertas)	Created	Date 2012-10-11	Rev G
		Reference	

Table 7 AIR Unit Dimensions

AIR Unit Type	Height (A)	Width (B)	Depth (C)	Distance between mounting brackets (D)	Depth including mounting brackets (E)	Distance between stud holes on mounting brackets (F)	Distance between lower mounting bracket and the lower end of the AIR unit (G)
AIR 11 B8A B20P	1982 mm	307 mm	200 mm	1017 mm	239 mm	110 mm	484 mm
AIR 11 B20A B8P	1982 mm	307 mm	200 mm	1017 mm	239 mm	110 mm	484 mm
AIR 21 B1A B3P	1427 mm	307 mm	200 mm	1017 mm	241 mm	110 mm	243 mm
AIR 21 B2A B4P	1427 mm	307 mm	200 mm	1017 mm	241 mm	110 mm	243 mm
AIR 21 B4A B2P	1427 mm	307 mm	200 mm	1017 mm	241 mm	110 mm	243 mm
AIR 21 B4A B12P BSP	2432 mm	307 mm	222 mm	1017 mm	264 mm	110 mm	708 mm

Table 8 lists the weight of the AIR unit with and without installation kit.

Table 8 AIR Unit Weight with and without Installation Kit

AIR Unit Type	Weight without Installation Kit	Weight with Single Unit Pole Installation Kit, 55-115 mm	Weight with Single Unit Pole Installation Kit, 110-220 mm	Weight with Single Unit Wall Installation Kit	Weight with Triple Unit Pole Installation Kit, 114.3 mm <sup>(1)</sup>	Weight with Triple Unit Pole Installation Kit, 139.7 mm <sup>(1)</sup>	Weight with Down Tilt Installation Kit and Single Unit Pole Clamp, 110-220 mm
AIR 11 B8A B20P	39.5 kg	43.9 kg	47.2 kg	42.8 kg	49.7 kg	49.7 kg	N/A <sup>(2)</sup>
AIR 11 B20A B8P	39.5 kg	43.9 kg	47.2 kg	42.8 kg	49.7 kg	49.7 kg	N/A <sup>(2)</sup>
AIR 21 B1A B3P	41.0 kg	45.4 kg	48.7 kg	44.3 kg	51.2 kg	51.2 kg	55.6 kg
AIR 21 B2A B4P	41.5 kg	45.9 kg	49.2 kg	44.8 kg	51.7 kg	51.7 kg	56.1 kg
AIR 21 B4A B2P	41.0 kg	45.4 kg	48.7 kg	44.3 kg	51.2 kg	51.2 kg	55.6 kg
AIR 21 B4A B12P BSP	57.0 kg	61.4 kg	64.7 kg	60.3 kg	67.2 kg	67.2 kg	N/A <sup>(2)</sup>

(1) Weight includes one AIR unit and the triple unit pole installation kit. After hoisting and installing, two more AIR units can be installed on the triple unit pole installation kit.

(2) Not Applicable.

The color of the radome is gray, and the color of the rear and side solar shields are light gray.

### 3.4 Space Requirements

This section describes the space requirements for installing the AIR unit. A complete installation description is found in *Installing Antenna Integrated Radio Units*.

# RRUS 11

## Frequency (AT&T)

- ✓ Band 12 (Lower 700 MHz)
- ✓ Band 4 (AWS, 17/2100 MHz) — 2Q2011

## RF Characteristics

- ✓ Output power: 2x30 Watts
- ✓ 2x2 MIMO Capable
- ✓ IBW of 20 MHz
- ✓ Rx Sens.: Better than -105 dBm (5 MHz)

## RET/TMA Support

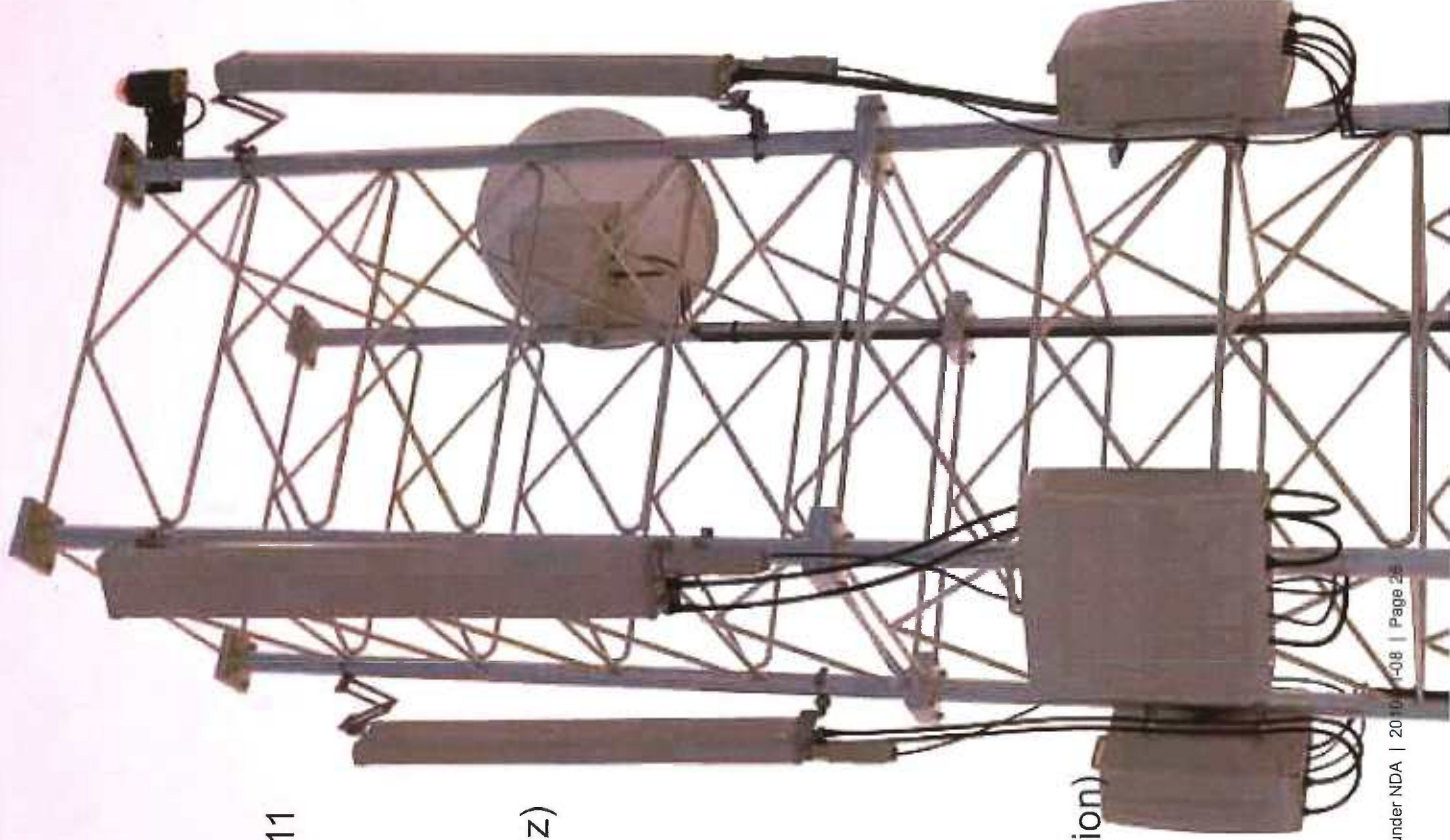
- ✓ AISG 2.0 Compatible
- ✓ Via RET Port and Centre Conductor
- ✓ Cascading
- ✓ 30 VDC Bias

## Environmental

- ✓ Self Convection
- ✓ Temperature -40 to 131 F

## Power

- ✓ Input voltage: -48 VDC or AC (exemption)
- ✓ Fuse size: 13 – 32 A
  - Recommended: 25 A
- ✓ Power Consumption:
  - Typical 200 Watts
  - Max 310 Watts
  - Excl. RET and TMA load



# RRUS 11 Mechanics

- Wall and pole mounting brackets
- Reused from RRUW and RRU22
  - Vertical Mount Only

## Clearing distances:

- Above  $\geq$  16 in.
- Below  $\geq$  12 in.
- Side  $\geq$  0 mm

## DC connector

- Bayonet
- Screw terminals in connector plug
- Supported outer cable diameter: 6-18 mm

## CPRI connector

- LCD with proprietary cover
- Separate cover available from 1Q2011

## Size & Weight

- Band 4: 44 lbs
- Band 12: 50 lbs
- 17.8" x 17.3" x 7.2" incl. sun shield



# **EXHIBIT C**

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT  
EVALUATION OF HUMAN EXPOSURE POTENTIAL  
TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11041D

Waterford / I-95 / X82  
53 Dayton Road  
Waterford, CT 06385

**April 27, 2015**

**EBI Project Number: 6215001623**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general public allowable limit:	<b>46.48 %</b>

April 27, 2015

T-Mobile USA  
Attn: Jason Overbey, RF Manager  
35 Griffin Road South  
Bloomfield, CT 06002

Emissions Analysis for Site: **CT11041D – Waterford / I-95 / X82**

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **53 Dayton Road, Waterford, CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limit for the 700 MHz Band is  $467 \mu\text{W}/\text{cm}^2$ , and the general population exposure limit for the PCS and AWS bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at **53 Dayton Road, Waterford, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 GSM channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel
- 2) 2 UMTS channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 2 LTE channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 4) 1 LTE channel (700 MHz Band) was considered for each sector of the proposed installation. This channel has a transmit power of 30 Watts.
- 5) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.

- 6) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antennas used in this modeling are the **Ericsson AIR21 B2A/B4P** and the **Ericsson AIR21 B4A/B12P** for 1900 MHz (PCS) and 2100 MHz (AWS) channels and the **Commscope LNX-6515DS-VTM** for 700 MHz channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The **Ericsson AIR21 B2A/B4P** and the **Ericsson AIR21 B4A/B12P** have a maximum gain of **15.9 dBd** at its main lobe for 1900 MHz and 2100 MHz. The **Commscope LNX-6515DS-VTM** has a maximum gain of **14.6 dBd** at its main lobe for 700 MHz. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antenna mounting height centerlines of the proposed antennas is **164 feet** above ground level (AGL).
- 9) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.

### T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Ericsson AIR21 B4A/B12P□	Make / Model:	Ericsson AIR21 B4A/B12P□	Make / Model:	Ericsson AIR21 B4A/B12P□
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	164	Height (AGL):	164	Height (AGL):	164
Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)
Channel Count	2	Channel Count	2	# PCS Channels:	2
Total TX Power:	120	Total TX Power:	120	# AWS Channels:	120
ERP (W):	4,668.54	ERP (W):	4,668.54	ERP (W):	4,668.54
Antenna A1 MPE%	0.67	Antenna B1 MPE%	0.67	Antenna C1 MPE%	0.67
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Ericsson AIR21 B2A/B4P□	Make / Model:	Ericsson AIR21 B2A/B4P□	Make / Model:	Ericsson AIR21 B2A/B4P□
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	164	Height (AGL):	164	Height (AGL):	164
Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power:	120	Total TX Power:	120	Total TX Power:	120
ERP (W):	4,668.54	ERP (W):	4,668.54	ERP (W):	4,668.54
Antenna A2 MPE%	0.67	Antenna B2 MPE%	0.67	Antenna C2 MPE%	0.67
Antenna #:	3	Antenna #:	3	Antenna #:	3
Make / Model:	Commscope LNX-6515DS-VTM	Make / Model:	Commscope LNX-6515DS-VTM	Make / Model:	Commscope LNX-6515DS-VTM
Gain:	14.6 dBd	Gain:	14.6 dBd	Gain:	14.6 dBd
Height (AGL):	164	Height (AGL):	164	Height (AGL):	164
Frequency Bands	700 MHz	Frequency Bands	700 MHz	Frequency Bands	700 MHz
Channel Count	1	Channel Count	1	Channel Count	1
Total TX Power:	30	Total TX Power:	30	Total TX Power:	30
ERP (W):	865.21	ERP (W):	865.21	ERP (W):	865.21
Antenna A3 MPE%	0.27	Antenna B3 MPE%	0.27	Antenna C3 MPE%	0.27

Site Composite MPE%	
Carrier	MPE%
T-Mobile	4.83
AT&T	13.91 %
Verizon Wireless	20.62 %
MetroPCS	5.02 %
Public Safety	2.10 %
<b>Site Total MPE %:</b>	<b>46.48 %</b>

T-Mobile Sector 1 Total:	1.61 %
T-Mobile Sector 2 Total:	1.61 %
T-Mobile Sector 3 Total:	1.61 %
<b>Site Total:</b>	<b>46.48 %</b>

## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector 1:	1.61 %
Sector 2:	1.61 %
Sector 3 :	1.61 %
T-Mobile Total:	4.83 %
Site Total:	46.48 %
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **46.48%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



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