

STATE OF CONNECTICUT  
CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

Internet: [ct.gov/csc](http://ct.gov/csc)

Daniel F. Caruso  
Chairman

February 25, 2008

Kenneth C. Baldwin, Esq.  
Robinson & Cole LLP  
280 Trumbull Street  
Hartford, CT 06103-3597

RE: **EM-VER-144-080131** – Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at Video Lane, Trumbull, Connecticut.

Dear Attorney Baldwin:

At a public meeting held on February 14, 2008, the Connecticut Siting Council (Council) acknowledged your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies.

The proposed modifications are to be implemented as specified here and in your notice dated January 31, 2008, including the placement of all necessary equipment and shelters within the tower compound. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Thank you for your attention and cooperation.

Very truly yours,

Daniel F. Caruso  
Chairman

DFC/MP/cm

c: The Honorable Raymond G. Baldwin, Jr., First Selectman, Town of Trumbull  
Harry Eberhart, Zoning Enforcement Officer, Town of Trumbull  
Crown Castle

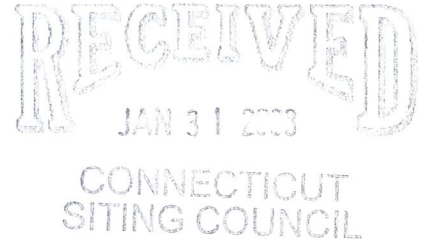
EM-VER-144-080131

280 Trumbull Street  
Hartford, CT 06103-3597  
Main (860) 275-8200  
Fax (860) 275-8299  
kbaldwin@rc.com  
Direct (860) 275-8345

ORIGINAL

January 31, 2008

*Via Hand Delivery*



S. Derek Phelps  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

Re: **Notice of Exempt Modification – Antenna Swap  
Video Lane (a/k/a 800 Booth Hill Road), Trumbull, Connecticut**

Dear Mr. Phelps:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains a wireless telecommunications facility at the above-referenced address. The Council approved Cellco’s shared use of the existing 460-foot guyed lattice tower in Docket No. 77 on August 4, 1987. On August 26, 2004, the Council granted Cellco’s request to replace six cellular antennas with six dual band antennas. Cellco now intends to modify its installation by replacing its six (6) DB774G90ESXM dual band antennas with two (2) BXA-185085/12CF antennas and one (1) BXA-185063/8CF antenna at the 238-foot level, and two (2) BXA-80080/6CF antennas and one (1) BXA-80063/6CF antenna at the 232-foot level on the existing tower. Attached behind Tab 1 are the specifications for the proposed replacement antennas.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Raymond G. Baldwin, First Selectman of the Town of Trumbull. Pursuant to a Council directive, a copy of this letter is also being sent to Francis F. Daddario, Jr., the owner of the property on which the facility is located.

The planned modifications to the facility falls squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in any increase in the overall height of the existing structures. Cellco’s replacement antennas will be located at the 232 and 238-foot levels on the existing 460-foot tower.



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# ROBINSON & COLE<sub>LLP</sub>

S. Derek Phelps  
January 31, 2008  
Page 2

2. The proposed modifications will not involve any modifications to ground-mounted equipment and, therefore, will not require the extension of the site boundaries.

3. The proposed modifications will not increase noise levels at the facility by six decibels or more.

4. The operation of the replacement antennas will not increase radio frequency (RF) power density levels at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative power density table for the modified facility is included behind Tab 2.

Also attached is a Detailed Structural Analysis confirming that the tower can support the proposed modifications. (See Tab 3).

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

Copy to:

Raymond G. Baldwin, Trumbull First Selectman  
Francis F. Daddario  
Sandy M. Carter



Slant +/- 45° Dual Polarized, Panel 85° / 18 dBi

## BXA-185085/12CF

When ordering replace "\_\_\_" with connector type.

### Mechanical specifications

Length	1840 mm	72.4 in
Width	154 mm	6.1 in
Depth	105 mm	4.1 in
Depth with t-bracket	133 mm	5.2 in
4) Weight	5.9 kg	13.0 lbs
Wind Area		
Fore/Aft	0.28 m <sup>2</sup>	3.1 ft <sup>2</sup>
Side	0.19 m <sup>2</sup>	2.1 ft <sup>2</sup>
Rated Wind Velocity (Safety factor 2.0)		
	>201 km/hr	>125 mph
Wind Load @ 100 mph (161 km/hr)		
Fore/Aft	460 N	103.4 lbs
Side	304 N	68.3 lbs

Antenna consisting of aluminum alloy with brass feedlines covered by a UV safe fiberglass radome.

### Mounting and Downtilting

Mounting brackets attach to a pipe diameter of Ø50-102 mm (2.0-4.0 in).

Mounting bracket kit #26799997

Downtilt bracket kit #26799999

The downtilt bracket kit includes the mounting bracket kit.

### Electrical specifications

Frequency Range	1850-1990 MHz
Impedance	50Ω
3) Connector(s)	NE or E-DIN 2 ports / center
1) VSWR	≤ 1.4:1
Polarization	Slant ± 45°
1) Isolation Between Ports	< -30 dB
1) Gain	18 dBi
2) Power Rating	250 W
1) Half Power Angle	
H-Plane	85°
E-Plane	5°
1) Electrical Downtilt	0°
1) Null Fill	5%
Lightning Protection	Direct Ground

Patented Dipole Design: U.S. Patent No. 6,597,324 B2

1) Typical values.

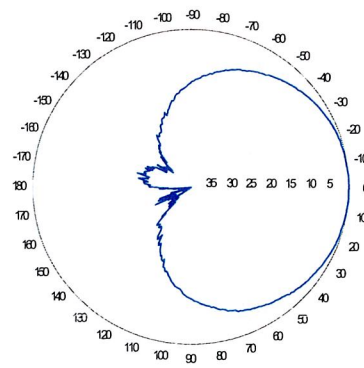
2) Power rating limited by connector only.

3) NE indicates an elongated N connector.  
E-DIN indicates an elongated DIN connector.

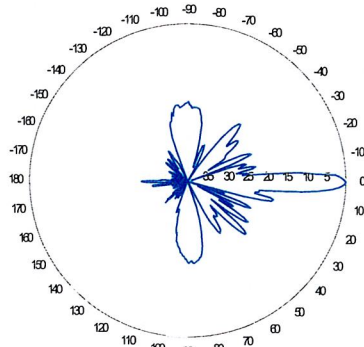
4) The antenna weight listed above does not include the bracket weight.

Improvements to mechanical and/or electrical performance of the antenna may be made without notice.

### Radiation pattern<sup>1)</sup>



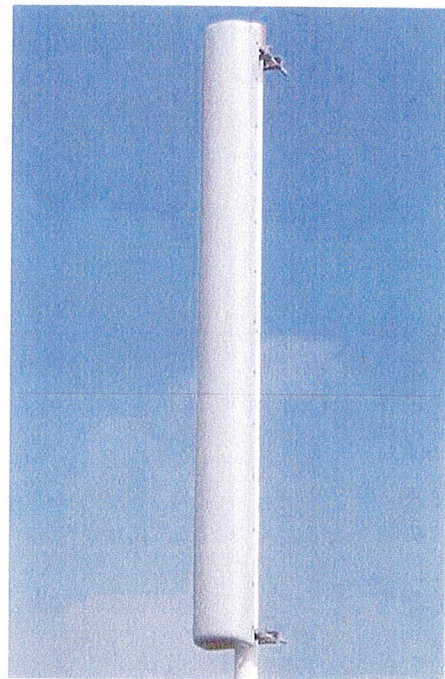
Horizontal



Vertical

Radiation patterns for all antennas are measured with the antenna mounted on a fiberglass pole.

Mounting on a metal pole will typically improve the Front-to-Back ratio.



**Amphenol Antel's Exclusive 3T (True Transmission Line Technology) Antenna Design:**

- Watercut brass feedline assembly for consistent performance.
- Unique feedline design eliminates the need for conventional solder joints in the signal path.
- A non-collinear system with access to every radiating element for broad bandwidth and superior performance.
- Air as insulation for virtually no internal signal loss.

*This Amphenol Antel antenna is under a five-year limited warranty for repair or replacement.*

**Antenna available with center-fed connectors only.**

**CF Denotes a Center-Fed Connector.**

**1850-1990 MHz**

**Amphenol Antel, Inc.**  
The Antenna Technology Company

Revision Date: 7/11/07

Amphenol Antel, Inc. 1300 Capital Drive Rockford, Illinois 61109 USA Tel. (815) 399-0001  
Toll-Free (888) 417-9562 Fax. (815) 399-0156 antel@antelinc.com www.antelinc.com

Slant +/- 45° Dual Polarized, Panel 63° / 18.5 dBi

## BXA-185063/8CF

When ordering replace "\_\_\_" with connector type.

### Mechanical specifications

Length	1238 mm	48.8 in
Width	154 mm	6.1 in
Depth	80 mm	3.2 in
Depth with t-bracket	108 mm	4.3 in
4) Weight	4.5 kg	10.0 lbs
Wind Area		
Fore/Aft	0.19 m <sup>2</sup>	2.1 ft <sup>2</sup>
Side	0.10 m <sup>2</sup>	1.1 ft <sup>2</sup>
Rated Wind Velocity (Safety factor 2.0)		
	>322 km/hr	>200 mph
Wind Load @ 100 mph (161 km/hr)		
Fore/Aft	288 N	65 lbs
Side	170 N	38 lbs

Antenna consisting of aluminum alloy with brass feedlines covered by a UV safe fiberglass radome.

### Mounting and Downtilting

Mounting brackets attach to a pipe diameter of Ø50-102 mm (2.0-4.0 in).

Mounting bracket kit #26799997

Downtilt bracket kit #26799999

The downtilt bracket kit includes the mounting bracket kit.

### Electrical specifications

Frequency Range	1850-1990 MHz
Impedance	50Ω
3) Connector(s)	NE or E-DIN 2 ports / center or bottom
1) VSWR	≤ 1.4:1
Polarization	Slant ± 45°
1) Isolation Between Ports	< -30 dB
1) Gain	18.5 dBi
2) Power Rating	250 W
1) Half Power Angle	
H-Plane	63°
E-Plane	7°
1) Electrical Downtilt	0°
1) Null Fill	5%
Lightning Protection	Direct Ground

Patented Dipole Design: U.S. Patent No. 6,597,324 B2

1) Typical values.

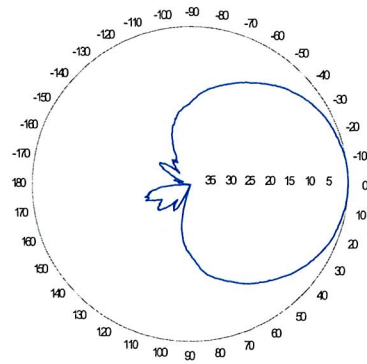
2) Power rating limited by connector only.

3) NE indicates an elongated N connector.  
E-DIN indicates an elongated DIN connector.

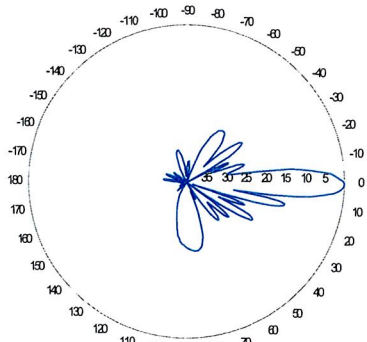
4) The antenna weight listed above does not include the bracket weight.

Improvements to mechanical and/or electrical performance of the antenna may be made without notice.

### Radiation pattern<sup>1)</sup>



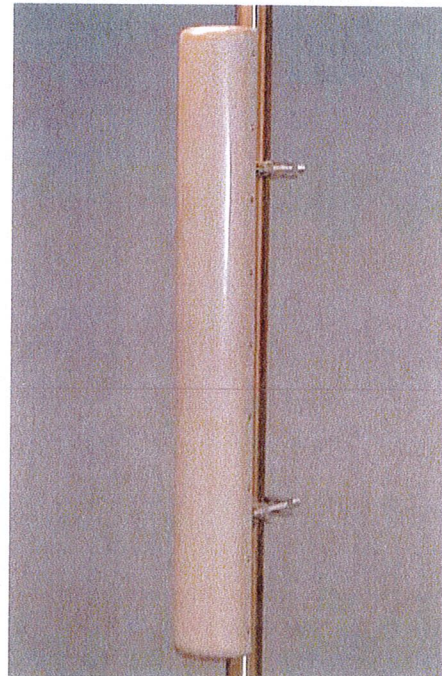
Horizontal



Vertical

Radiation patterns for all antennas are measured with the antenna mounted on a fiberglass pole.

Mounting on a metal pole will typically improve the Front-to-Back ratio.



**Amphenol Antel's  
Exclusive 3T (True  
Transmission Line  
Technology)  
Antenna Design:**

- Watercut brass feedline assembly for consistent performance.
- Unique feedline design eliminates the need for conventional solder joints in the signal path.
- A non-collinear system with access to every radiating element for broad bandwidth and superior performance.
- Air as insulation for virtually no internal signal loss.

*This Amphenol Antel antenna is under a five-year limited warranty for repair or replacement.*

Antenna can be ordered with center-fed or bottom-fed connectors.

Center-fed: BXA-185063/8CF + (NE or E-DIN)  
Bottom-fed: BXA-185063/8BF + (NE or E-DIN)

CF Denotes a Center-Fed Connector.

1850-1990 MHz



Revision Date: 7/11/07

# Slant +/- 45° Dual Polarized, Panel 80° / 13.5 dBd

## BXA-80080/6CF

When ordering replace "\_\_\_" with connector type.

### Mechanical specifications

Length	1844 mm	72.6 in
Width	285 mm	11.2 in
Depth	116 mm	4.6 in
Depth with z-bracket	156 mm	6.1 in
4) Weight	10 kg	22.0 lbs
Wind Area		
Fore/Aft	0.53 m <sup>2</sup>	5.7 ft <sup>2</sup>
Side	0.21 m <sup>2</sup>	2.3 ft <sup>2</sup>
Rated Wind Velocity (Safety factor 2.0)		
	>277 km/hr	>172 mph
Wind Load @ 100 mph (161 km/hr)		
Fore/Aft	765 N	172 lbs
Side	366 N	82 lbs

Antenna consisting of aluminum alloy with brass feedlines covered by a UV safe fiberglass radome.

### Mounting and Downtilting

Mounting brackets attach to a pipe diameter of Ø50-160 mm (2.0-6.3 in).

Mounting bracket kit #36210002

Downtilt bracket kit #36114003

### Electrical specifications

Frequency Range	806-900 MHz*
Impedance	50Ω
3) Connector(s)	NE or E-DIN 2 ports / center
1) VSWR	≤ 1.4:1
Polarization	Slant ± 45°
1) Isolation Between Ports	< -30 dB
1) Gain	13.5 dBd
2) Power Rating	500 W
1) Half Power Angle	
H-Plane	80°
E-Plane	11°
1) Electrical Downtilt	0°
1) Null Fill	5%
Lightning Protection	Direct Ground

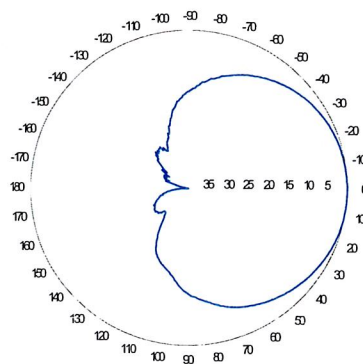
\*Also available for 870-960 MHz. Consult your sales director for more information.

Patented Dipole Design: U.S. Patent No. 6,608,600 B2

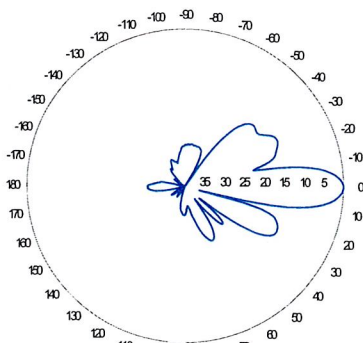
- 1) Typical values.
- 2) Power rating limited by connector only.
- 3) NE indicates an elongated N connector.  
E-DIN indicates an elongated DIN connector.
- 4) The antenna weight listed above does not include the bracket weight.

Improvements to mechanical and/or electrical performance of the antenna may be made without notice.

### Radiation pattern<sup>1)</sup>



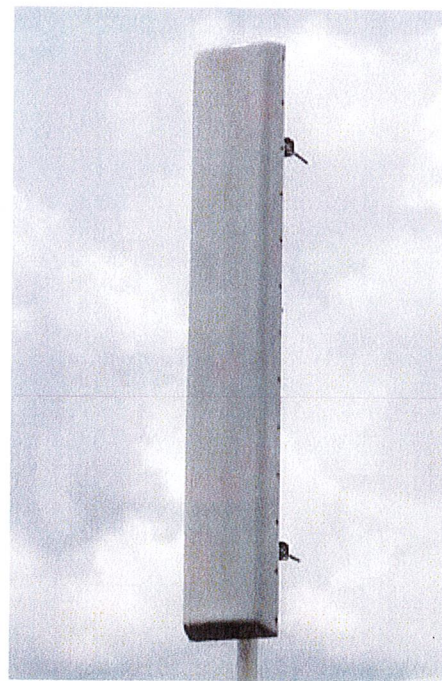
Horizontal



Vertical

Radiation patterns for all antennas are measured with the antenna mounted on a fiberglass pole.

Mounting on a metal pole will typically improve the Front-to-Back ratio.



**Amphenol Antel's Exclusive 3T (True Transmission Line Technology) Antenna Design:**

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- Air as insulation for virtually no internal signal loss.

*This Amphenol Antel antenna is under a five-year limited warranty for repair or replacement.*

**Antenna available with center-fed connectors only.**

CF Denotes a Center-Fed Connector.

**806-900 MHz**



Revision Date: 7/3/07

# Slant +/- 45° Dual Polarized, Panel 63° / 14.5 dBd

## BXA-80063/6CF

When ordering replace "\_\_\_" with connector type.

### Mechanical specifications

Length	1804 mm	71.1 in
Width	285 mm	11.2 in
Depth	126 mm	5.0 in
Depth with z-bracket	166 mm	6.5 in
4) Weight	6.8 kg	14.9 lbs
Wind Area		
Fore/Aft	0.51 m <sup>2</sup>	5.53 ft <sup>2</sup>
Side	0.23 m <sup>2</sup>	2.45 ft <sup>2</sup>
Rated Wind Velocity (Safety factor 2.0)	>318 km/hr >198 mph	
Wind Load @ 100 mph (161 km/hr)		
Fore/Aft	753 N	169.2 lbs
Side	378 N	85 lbs

Antenna consisting of aluminum alloy with brass feedlines covered by a UV safe fiberglass radome.

### Mounting and Downtilting

Mounting brackets attach to a pipe diameter of Ø50-160 mm (2.0-6.3 in).

Mounting bracket kit #36210002

Downtilt bracket kit #36114003

### Electrical specifications

Frequency Range	806-900 MHz*
Impedance	50Ω
3) Connector(s)	NE or E-DIN 2 ports / center
1) VSWR	≤ 1.4:1
Polarization	Slant ± 45°
1) Isolation Between Ports	< -30 dB
1) Gain	14.5 dBd
2) Power Rating	500 W
1) Half Power Angle	
H-Plane	63°
E-Plane	11°
1) Electrical Downtilt	0°
1) Null Fill	5%
Lightning Protection	Direct Ground

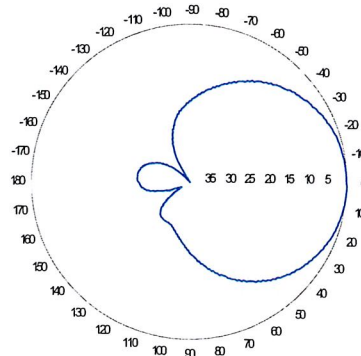
\*Also available for 870-960 MHz. Refer to model BXA-87063/6CF\_.

Patented Dipole Design: U.S. Patent No. 6,608,600 B2

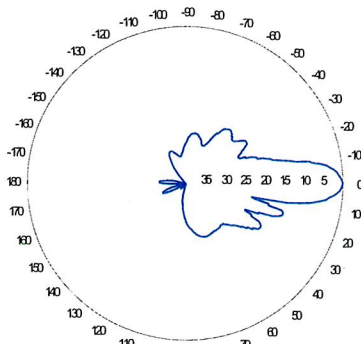
- 1) Typical values.
- 2) Power rating limited by connector only.
- 3) NE indicates an elongated N connector. E-DIN indicates an elongated DIN connector.
- 4) The antenna weight listed above does not include the bracket weight.

Improvements to mechanical and/or electrical performance of the antenna may be made without notice.

### Radiation pattern<sup>1)</sup>



Horizontal

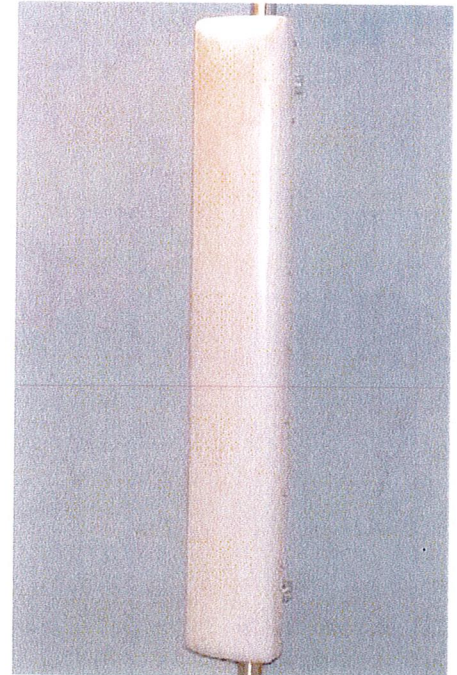


Vertical

### Featuring upper side lobe suppression.

Radiation patterns for all antennas are measured with the antenna mounted on a fiberglass pole.

Mounting on a metal pole will typically improve the Front-to-Back ratio.



**Amphenol Antel's Exclusive 3T (True Transmission Line Technology) Antenna Design:**

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- Air as insulation for virtually no internal signal loss.

*This Amphenol Antel antenna is under a five-year limited warranty for repair or replacement.*

**Antenna available with center-fed connectors only.**

**CF Denotes a Center-Fed Connector.**

**806-900 MHz**



Revision Date: 11/21/07

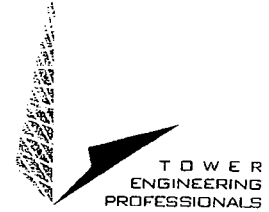




Date: **January 10, 2008**

Mr. Ben Goodhart  
Crown Castle International  
9105 Monroe Road, Suite 150  
Charlotte, NC 28270  
(o) (704) 321-3845

Tower Engineering Professionals, Inc. (TEP)  
3703 Junction Boulevard  
Raleigh, NC 27603  
(o) (919) 661-6351  
[mlackey@tepgroup.net](mailto:mlackey@tepgroup.net)



**Subject: Structural Analysis Report**

**Carrier Designation:** Verizon Wireless Co-Locate  
**Carrier Site Number:** Trumbull, CT  
**Carrier Site Name:** BRG120

**Crown Castle Designation:** Crown Castle BU Number: 873128  
Crown Castle Site Name: Trumbull  
Crown Castle JDE Job Number: 92620

**Engineering Firm Designation:** TEP Project Number: 072725

**Site Data:** 800 Booth Hill Road, Shelton, Fairfield Co., CT, 06611  
Latitude 41°-16'-44.796", Longitude -73°-11'-6.9"  
460 Foot - Guyed Tower

Dear Mr. Goodhart,

TEP is pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the aforementioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 270330, in accordance with application 57919, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

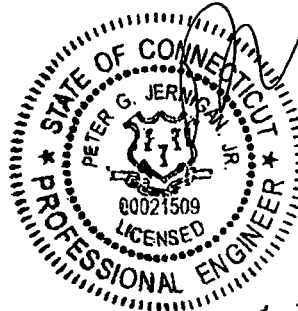
LC1: Existing + Reserved + Proposed Equipment Sufficient Capacity  
Note: See Table I and Table II for the proposed and existing/reserved loading.

This analysis has been performed in accordance with the TIA/EIA-222-F standard based on a fastest-mile wind speed of 85-mph (105-mph 3-second gust).

We at TEP appreciate the opportunity of providing our continuing professional services to you and Crown Castle International. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted,

Pete Jernigan, P.E.



1-10-2008

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## 1) INTRODUCTION

The subject tower is a 460-foot guyed tower by an unknown manufacturer.

## 2) ANALYSIS CRITERIA

The existing, reserved, and proposed antennas, transmission lines, and mountings are shown in the following tables. The site is in Fairfield County, CT. The structural analysis was performed in accordance with the ANSI/TIA/EIA-222-F-1996 (TIA), Structural Standards for Steel Antenna Towers and Antenna Supporting Structures dated June 1996. The governing winds forces are derived from the TIA Standard using a fastest-mile wind speed of 85 mph without ice (74 mph with 1/2" radial ice) for an Exposure C and Importance Factor of 1.00.

**Table 1 – Proposed Antenna and Cable Information**

Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount	Number of Feed Lines	Feed Line Size (in)
238 (Proposed)	2 1	Antel Antel	BXA-185085/12CF BXA-185063/8CF	-	- 1	- 1
232 (Proposed)	2 1	Antel Antel	BXA-80080/6CF BXA-80063/6CF	-	- 1	- 1

- Existing feed lines to be re-used

**Table 2 – Existing and Reserved Antenna and Cable Information**

Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount	Number of Feed Lines	Feed Line Size (in)
474.5 (Existing)	1	RCA	TFU20JDAS	(1) Mast	1	4" Ø
455 (Existing)	1 1 1	Unknown Unknown Decibel	20-ft Omni 20-ft Omni DB420	(3) Sidearm Mounts	1 1 1	7/8 1-5/8 7/8
449 (Existing)	1	Unknown	20-ft Omni	(1) Sidearm Mount	1	7/8
443 (SLA)	1	Decibel	DB810M-XC	(1) Sidearm Mount	1	1-1/4
440 (SLA)	1	Decibel	DB810M-XC	(1) Sidearm Mt.	1	1-5/8
420 (Existing)	1	ERI	ERI 1083-3CP	(1) Pipe Face Mount	1	3" Ø
392 (Existing)	1	Shively	SHI/SIRA 05/06-2/DA	(1) Flush Mount	3	1-5/8
380 (Existing)	1	Dielectric	TLP8J	(1) Flush Mount	1	3.15" Ø
376 (Existing)	1	Unknown	8-ft Omni	(1) Sidearm Mt.	1	1-5/8
359 (Existing)	1	Andrew	63021	(1) Flush Mount	1	1-5/8
356 (Existing)	1	Unknown	20-ft Omni	(1) Sidearm Mt	1	1-1/4

Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount	Number of Feed Lines	Feed Line Size (in)
353 (Existing)	1	Unknown	20-ft Omni	(1) Sidearm Mt.	1	7/8
352 (Existing)	1	Antel	BCD87077	(1) Sidearm Mount	1	1-5/8
351 (Existing)	1	Antenna Specialists	ASP 601	(1) Sidearm Mount	2	5/8
340 (Existing)	1	Anixter	P2072	(1) Flush Mount	1	1-5/8
336 (Existing)	1 1	Unknown Decibel	10-ft Omni DB408	(2) Sidearm Mount	1 1	1-1/4 1-1/4
333 (Existing)	1	Sinclair	SRL 101-A	(1) Sidearm Mount	-	-
328 (Existing)	1	Decibel	DB230	(1) Flush Mount	-	-
322 (Existing)	1	Radiowaves	SPD3-5.8	(1) Flush Mount	1	5/8
312 (Existing)	1	Shively	6015-2/3R-DA	(1) Pipe Face Mount	3	1-5/8
299 (Reserved)	1	Dielectric	TLP-8M	(1) Pipe Face Mount	1	3" Ø
288 (Existing)	1	Unknown	10-ft Omni Antenna	(1) Sidearm Mount	1	1-1/4
285 (Existing)	1 1	Decibel Unknown	DB212 15-ft Omni	(2) Flush Mounts	1 <sup>2</sup> 1	1-5/8 1-5/8
274 (Existing)	1	Unknown	10-ft Omni	(1) Sidearm Mount	1	1-1/4
271 (Existing)	1	Unknown	6-ft Yagi	(1) Pipe Mount	-	-
270 (Existing)	1	Unknown	6-ft Omni	(1) Sidearm Mount	1	1-5/8
268 (Existing)	1	Decibel	DB809KT3E-Y	(1) Sidearm Mount	1	1-1/4
261 (Existing)	1	Unknown	10-ft Omni	(1) Sidearm Mount	1 <sup>2</sup>	7/8
256 (Existing)	-	-	-	(1) Sidearm Mount	-	-
247 (Existing)	3	EMS Wireless	RR901702DP	(3) T-Arm Sectors	6	7/8
247 (SLA)	6	EMS Wireless	DR65-18-00DPL2Q	-	24	1-5/8
238 (Existing)	3 <sup>3</sup>	Decibel	DB774G90ESXM	-	3	7/8
232 (Existing)	3 <sup>3</sup>	Decibel	DB774G90ESXM	(3) Sidearm Mounts	3	7/8
221 (Existing)	1	Motorola	15-ft Omni Antenna	(1) Sidearm Mount	1	7/8

Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount	Number of Feed Lines	Feed Line Size (in)
208 (Existing)	1	Unknown	8-ft Omni Antenna	(1) Sidearm Mount	1	7/8
194 (Existing)	1	Scala	SRL210C4*2	(1) Sidearm Mount	1	7/8
188 (Existing)	1	Unknown	2-ft Yagi w/ Radome	(1) Flush Mount	-	-
186 (Existing)	1	Unknown	14-1/2" Corner Reflector	(1) Flush Mount	-	-
179.5 (Existing)	1	Radiowaves	SPD4-5.8NS	(1) Flush Mount	1	5/8
154 (Existing)	1	Mark Radiation Systems	P2072	(1) Pipe Mount	1	7/8
146 (Existing)	1	Andrew	PL6-65PXA	(1) Pipe Mount	1	EW52
145 (Existing)	1	Unknown	20-ft Omni Antenna	(1) Sidearm Mount	1	7/8
143 (Existing)	1	Decibel	DB264	(1) Sidearm Mount	1	7/8
117 (Existing)	1	Unknown	P9-9A48GN-U	(1) Pipe Mount	1	7/8
115 (Existing)	1	Unknown	8-ft Omni Antenna	(1) Sidearm Mount	1	7/8
110 (Existing)	1	Unknown	10-ft Grid Antenna	(1) Sidearm Mount	-	-
107 (Existing)	1	Scala	6-ft x 3-ft Grid Antenna	(1) Pipe Mount	1	7/8
100 (Existing)	1	Radiowaves	SPD2-5.8ms	(1) Flush Mount	1	5/16" Ø
76 (Existing)	1	Anixter	P2072	(1) Pipe Mount	1	1-5/8
67 (Existing)	1	Anixter	P948	-	1	7/8
	1	Scala	PR-450V	(1) Flush Mount	1	1/4
63 (Existing)	1	Scala	PR-450V	(1) Flush Mount	1	1/4
62 (Existing)	1	Scala	PR-450V	(1) Sidearm Mount	1	7/8
54 (Existing)	1	Decibel	DB496	-	1	7/8
49 (Existing)	1	Channel Master	D.75M-1	(1) Sidearm Mount	1	1/4
44 (Existing)	1	Channel Master	D1.2M-1	(1) Sidearm Mount	1	1/4
40 (Reserved)	1	Trimble	Accutime 2000	(1) Sidearm Mount	1	1/2

<sup>2</sup> - SLA feed line

<sup>3</sup> - Antennas to be removed

**Table 3 – Design Antenna and Cable Information**

Unknown

**3) ANALYSIS PROCEDURE**

**Table 4 – Documents Provided**

Document	Remarks	Crown Document ID
Antenna Mapping	Unknown, Job # Unknown, date Unknown, provided by Crown	2045248
Steel and Antenna Mapping	Pinnacle Towers Inc., Acquisition # 0588-001, dated July 19, 2000, provided by Crown	1327906
Foundation Mapping	FDH Inc., PTI no. 0588-001, dated December 17, 2002, provided by Crown	1520339
Previous Structural Analysis Report	Global Signal Services, LLC., Job no. 3007940, dated January 5, 2005, provided by Crown	2020732
Geotechnical Report	FDH Inc., FDH Project no. 04-1229E, dated January 25, 2005, provided by Crown	1418454
Base Level Drawing	Crown Castle International, BU # 873128, dated June 21, 2007	1993977
Antenna Mapping	Tower Engineering Professionals, Inc., Job no. 071092, dated September 4, 2007	-
Foundation Mapping	Tower Engineering Professionals, Inc., Job no. 071092, dated December 3, 2007	-
TIA Condition Assessment	Tower Engineering Professionals, Inc., Job no. 071794, dated December 6, 2007	-
Previous Structural Analysis Report	Tower Engineering Professionals, Inc., Job no. 072725, dated December 12, 2007	-

**3.1) Analysis Method**

RISA Tower (version 5.0.2.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various dead, live, and wind load cases. Selected output from the analysis is included in Appendix A.

**3.2) Assumptions**

1. All feed lines are installed in the locations noted on the cable routing drawing in *Appendix B*.
2. When applicable, feed lines were considered to be structural components for calculating wind loads, as allowed by the industry standard.
3. Information in the original design drawings and specifications that could not be verified by TEP is assumed to be correct. For this analysis, TEP will assume conformance with the original design drawings and specifications.
4. TEP shall assume that all tower components are in sufficient condition to carry their full design capacity.
5. Serviceability with respect to antenna twist, tilt, roll, or lateral translation, is not checked and is left to the carrier or tower owner to ensure conformance.

**4) ANALYSIS RESULTS**

**Table 5 – Tower Component Stresses vs. Capacity**

LC1: Existing + Reserved + Proposed Equipment

Notes	Component	Elevation (ft)	% Capacity	Pass/Fail
<b>RISA Tower Analysis Summary:(Monopole)</b>				
			<b>Summary</b>	
<b>Notes:</b>	<b>Component</b>	<b>Elevation</b>	<b>% Capacity</b>	<b>Pass/Fail</b>
	Legs (T20)	81-61	71.4	Pass
	Leg Bolts (T4)	401-381	39.5	Pass
	Diagonals (T10)	281-261	74.4	Pass
	Diagonal Bolts (T11)	261-241	79.0	Pass
	Top Girt / Bolts (T5)	381-361	97.4	Pass
	Middle Girt / Bolts(T11)	261-241	95.5	Pass
	Guy 1	131	67.9	Pass
	Guy 2	251	59.2	Pass
	Guy 3	381	62.6	Pass
	<b>TOWER RATING =</b>		<b>97.4</b>	<b>Pass</b>
<b>INDIVIDUAL COMPONENTS:</b>				
<b>Notes:</b>	<b>Component</b>	<b>Elevation</b>	<b>% Capacity</b>	<b>Pass/Fail</b>
1	Base Foundation	-	52.0	Pass
1	Anchor	-	101.3	Pass
<b>Structure Rating (max from all components) =</b>				<b>101.3%</b>

\* Notes:

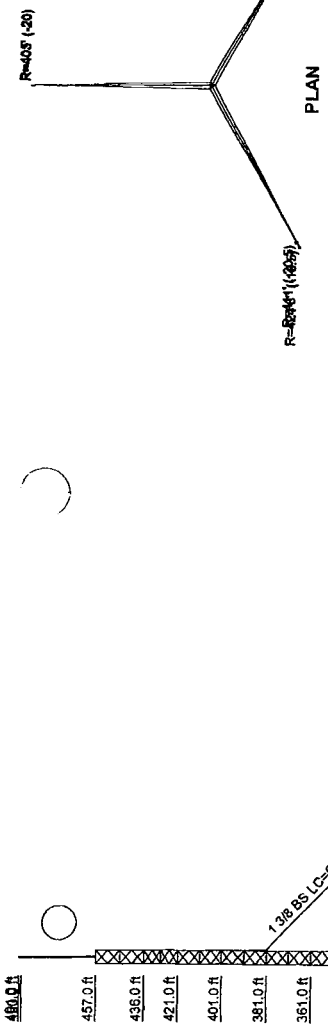
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity listed.

**4.1) Recommendations**

It should be noted that in order for the tower to pass in the current load scenario, the proposed and reserved coax must be configured as shown in Appendix B.

**APPENDIX A**  
**RISA TOWER OUTPUT**





Section	Legs	Leg Grade	Diagonals	Diagonal Grade	Top Girts	Mid Girts	Face Width (Hz) 5.833	# Pans @ (ft)	Weight (lb) 58037
L1	A	A57-50	L2 1/2x3/16	A36	L2 1/2x3/16	L2 1/2x1/4	6 0.635633	M 4 @ 5.25	2403.3
L2	D	SR 3 1/2	L2 1/2x3/16	L2 1/2x3/16	L2 1/2x3/16	L2 1/2x1/4			
L3	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L4	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L5	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L6	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L7	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L8	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L9	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L10	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L11	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L12	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L13	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L14	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L15	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L16	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L17	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L18	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L19	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L20	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L21	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L22	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L23	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			
L24	SR 3 1/4	SR 3	L3x3/16	L3x3/16	L3x3/16	L3x3/16			

**DESIGNED APPURTENANCE LOADING**

ELEVATION	TYPE	ELEVATION	TYPE	ELEVATION
445	071092 EI, 256-R-B, 346-ft, 445-ft-C (Existing Antenna 58 Mount)	445	10-Element 10-ft x 6-ft Yagi (Existing Antenna 44)	328
445	071092 EI, 256-ft (Empty Mount), 445-ft-B (Existing Antenna 59 Mount)	445	071092 EI, 265-ft, 285-ft, 326-ft, 343-ft, 439-ft, 445-ft-A (Existing Antenna 43 Mounts)	326
445	071092 EI, 256-ft-B, 346-ft, 445-ft-C (Existing Antenna 60 Mount)	445	20-FT 8 Element Dipole (Existing Antenna 43)	326
445	071092 EI, 256-ft-B, 346-ft, 445-ft-C (Existing Antenna 60 Mount)	445	SPD3-5.8 (Existing Antenna 42)	322
445	071092 EI, 256-ft-B, 346-ft, 445-ft-C (Existing Antenna 60 Mount)	445	2-Bay Shively FM w/ Radomes (Existing Antenna 41)	316 - 308
443 - 440	071092 EI, 256-ft-B, 346-ft, 445-ft-C (USA Mobility (SLA))	305 - 293	TLP-8M x 11.7-ft (Reserved)	
440	DB810M-XC (USA Mobility (SLA))	285	071092 EI, 265-ft, 285-ft, 326-ft, 343-ft, 439-ft, 445-ft-A (Existing Antenna 39 Mount)	
439	071092 EI, 265-ft, 285-ft, 326-ft, 343-ft, 439-ft, 445-ft-A (Existing Antenna 57 Mount)	278	10-ft x 2.5' dia. omni (Existing Antenna 39)	
439	071092 EI, 265-ft, 285-ft, 326-ft, 343-ft, 439-ft, 445-ft-A (Existing Antenna 57 Mount)	271	8-ft Single Element Dipole (Existing Antenna 40)	
430	20-ft x 2.5-in Omni Antenna (Existing Antenna 57)	267	15' x 8' Omni (Existing Antenna 38)	
430	20-ft x 2.5-in Omni Antenna (Existing Antenna 57)	267	10-Element 6-ft x 2-ft Yagi (Existing Antenna 37)	
400 - 388	1083-3CP (Existing Antenna 56)	267	10-ft x 2.5' dia. omni (Existing Antenna 36)	
372	2-Bay Shively FM w/ Radomes (Existing Antenna 54 Mount)	267	5-ft x 1.5' Dia Omni (Existing Antenna 35)	
372	8 x 2.5' Dia. omni (Existing Antenna 54)	260	071092 EI, 260-ft Mount (Existing Antenna 34)	
365	TLP-8M x 11.7-ft (Existing Antenna 53)	256	DB809KT35-Y (Existing Antenna 34)	
354	15' x 8' Omni (Existing Antenna 52)	256	071092 EI, 256-R-B, 346-ft, 445-ft-C (Existing Antenna 33 Mount)	
347.5	071092 347.5-ft Mount (Existing Antenna 51 Mount)	247	10-ft x 3-in Omni Antenna (Existing Antenna 33)	
347.5	10-ft x 2.5' dia. omni (Existing Antenna 51)	247	Antennas 30-32 Mounts)	
346	071092 EI, 256-ft-B, 346-ft, 445-ft-C (Existing Antenna 49 Mount)	247	071092 EI, 247-ft (Single Sector) (Existing Antennas 30-32 Mounts)	
346	071092 EI, 256-ft-B, 346-ft, 445-ft-C (Existing Antenna 50 Mount)	247	071092 EI, 247-ft (Single Sector) (Existing Antennas 30-32 Mounts)	
343	071092 EI, 265-ft, 285-ft, 326-ft, 343-ft, 439-ft, 445-ft-A (Existing Antenna 48 Mount)	247	(2) DR65-18-00DPL2Q (T-Mobile (SLA))	
343	071092 EI, 265-ft, 285-ft, 326-ft, 343-ft, 439-ft, 445-ft-A (Existing Antenna 48 Mount)	247	(2) DR65-18-00DPL2Q (T-Mobile (SLA))	
343	20-ft x 2.5-in Omni Antenna (Existing Antenna 48)	235	Antennas 24-29 Mount)	
340	8-FT Grid (Existing Antenna 47)	235	BXA-18508572CF (Verizon Proposed)	
331	071092 EI, 267-ft, 331-ft Mounts (Existing Antenna 46 Mount)	235	BXA-800806CF (Verizon Proposed)	
331	10-ft x 3-in Omni Antenna (Existing Antenna 46)	235	BXA-1850638CF (Verizon Proposed)	
330	071092 330-ft Mount (Existing Antenna 45)	235	BXA-800838CF (Verizon Proposed)	
330	Generic Ground Plane (Existing Antenna 45)	235	BXA-185098512CF (Verizon Proposed)	
		235	BXA-800806CF (Verizon Proposed)	

**Tower Engineering Professionals**  
**Job: BU# 873128 "Trumbull"**  
 Project: TEP# 072725  
 Client: Crown Castle  
 Drawn by: Matt Lackey, EIT/App'd  
 Code: TIA/EIA-222-F  
 Date: 01/10/08  
 Scale: NTS  
 Path: H:\2007\2725 - Trumbull\Structure\B\SA\LC\1873128.dwg  
 Dwg No. E-1

3703 Junction Boulevard  
 Raleigh, NC 27603  
 Phone: (919) 661-6351  
 FAX: (919) 661-6350

<b>RISA Tower</b>	Job	BU# 873128 "Trumbull"	Page	1 of 65
<b>Tower Engineering Professionals</b> 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 Fax: (919) 661-6350	Project	TEP# 072725	Date	11:08:39 01/10/08
	Client	Crown Castle	Designed by	Matt Lackey, E.I.

<b>RISA Tower</b>	Job	BU# 873128 "Trumbull"	Page	2 of 65
<b>Tower Engineering Professionals</b> 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 Fax: (919) 661-6350	Project	TEP# 072725	Date	11:08:39 01/10/08
	Client	Crown Castle	Designed by	Matt Lackey, E.I.

### Tower Input Data

The main tower is a 3x guyed tower with an overall height of 490' above the ground line. The base of the tower is set at an elevation of 0' above the ground line. The face width of the tower is 6' at the top and tapered at the base. An index plate is provided at the 3x guyed tower connection. There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Fairfield County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 74 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

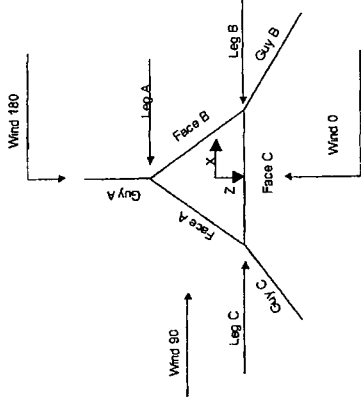
Pressures are calculated at each section.

Stress ratio used in pole design is 1.0664.

Safety factor used in guy design is 2.

Stress ratio used in tower member design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appearance mounts are not considered.

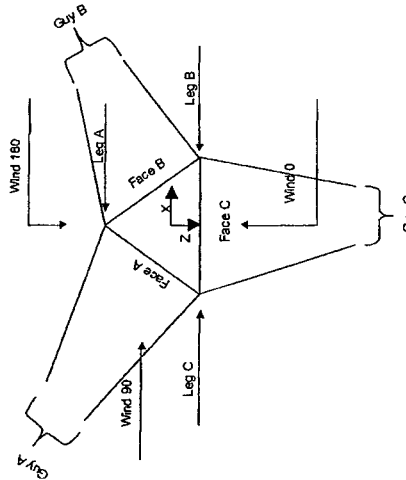


Corner & Stairmount Guyed Tower

### Options

- |  |  |   |
|--|--|---|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>Use Code Stress Ratios</li> <li>Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>Include Bolts in Member Capacity</li> <li>Leg Bolts are At Top Of Section</li> <li>Use Horizontal Braces For Leg</li> <li>Use Horizontal Bracing (W shield)</li> <li>Add IRC 184-W Combination</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>Assume Rigid Index Plate</li> <li>Use Clear Spans For Wind Area</li> <li>Use Code Safety Factors For KL/r</li> <li>Retention Guys To Initial Tension</li> <li>Bypass Mast Stability Checks</li> <li>Use Azimuth Dist. Conditions</li> <li>Project Wind Area at Appur.</li> <li>Automatic Torque Arm. Areas</li> <li>SR Members Have Chords</li> <li>SR Members Have Chords</li> <li>SR Members Have Chords</li> <li>Use Triangular Diagonal Lattice Bracing</li> </ul> | <ul style="list-style-type: none"> <li>Treat Feedline Bundles As Cylinder</li> <li>Use ASCE 10 X Brace Ly Rules</li> <li>Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>Consider Feedline Torque</li> <li>Include Angle Block Shear Check</li> <li>Include Shear Plates for Girts</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul> |
|--|--|---|

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Job		BU# 873128 "Trumbull"	
Project		TEP# 072725	
Client		Crown Castle	
Tower Engineering Professionals 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350		Date	11:08:39 01/10/08
Designed by		Matt Lackey, E.I.	



Face Guyed

Section	Elevation	Section Length	Pole Size	Pole Grade	Sackel Length
L1	490-457	33'	diameter TPU-201DAS	A53-B-35 (32 ksi)	

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor	Weight Multi.	Double Angle Spacing	Double Angle Spacing Horizontal
L1 490-457		th				in	in

Tower Section Geometry

<b>RISA Tower</b>		Page	4 of 65
Job		BU# 873128 "Trumbull"	
Project		TEP# 072725	
Client		Crown Castle	
Tower Engineering Professionals 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350		Date	11:08:39 01/10/08
Designed by		Matt Lackey, E.I.	

Tower Section	Tower Elevation	Description	Section Width	Number of Sections	Section Length
T1	457-436'		6'		21'
T2	436-421'		6'		15'
T3	421-401'		6'		20'
T4	401-381'		6'		20'
T5	381-361'		6'		20'
T6	361-341'		6'		20'
T7	341-321'		6'		20'
T8	321-301'		6'		20'
T9	301-281'		6'		20'
T10	281-261'		6'		20'
T11	261-241'		6'		20'
T12	241-221'		6'		20'
T13	221-201'		6'		20'
T14	201-181'		6'		20'
T15	181-161'		6'		20'
T16	161-141'		6'		20'
T17	141-121'		6'		20'
T18	121-101'		6'		20'
T19	101-81'		6'		20'
T20	81-61'		6'		20'
T21	61-41'		6'		20'
T22	41-20'		6'		21'
T23	20-68-1732'-0"		2'-6-3103"		133-1533"
T24	68-1732'-0"				68-1732"

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace	Has End Panels	Has Horizontal	Top Girt Offset	Bottom Girt Offset
T1	457-436'	53"	X Brace	No	No	Yes	0.0000	0.0000
T2	436-421'	39"	X Brace	No	No	Yes	0.0000	0.0000
T3	421-401'	5'	X Brace	No	No	Yes	0.0000	0.0000
T4	401-381'	5'	X Brace	No	No	Yes	0.0000	0.0000
T5	381-361'	5'	X Brace	No	No	Yes	0.0000	0.0000
T6	361-341'	5'	X Brace	No	No	Yes	0.0000	0.0000
T7	341-321'	5'	X Brace	No	No	Yes	0.0000	0.0000
T8	321-301'	5'	X Brace	No	No	Yes	0.0000	0.0000
T9	301-281'	5'	X Brace	No	No	Yes	0.0000	0.0000
T10	281-261'	5'	X Brace	No	No	Yes	0.0000	0.0000
T11	261-241'	5'	X Brace	No	No	Yes	0.0000	0.0000
T12	241-221'	5'	X Brace	No	No	Yes	0.0000	0.0000
T13	221-201'	5'	X Brace	No	No	Yes	0.0000	0.0000
T14	201-181'	5'	X Brace	No	No	Yes	0.0000	0.0000
T15	181-161'	5'	X Brace	No	No	Yes	0.0000	0.0000
T16	161-141'	5'	X Brace	No	No	Yes	0.0000	0.0000
T17	141-121'	5'	X Brace	No	No	Yes	0.0000	0.0000
T18	121-101'	5'	X Brace	No	No	Yes	0.0000	0.0000
T19	101-81'	5'	X Brace	No	No	Yes	0.0000	0.0000
T20	81-61'	5'	X Brace	No	No	Yes	0.0000	0.0000
T21	61-41'	5'	X Brace	No	No	Yes	0.0000	0.0000
T22	41-20'	53"	X Brace	No	No	Yes	0.0000	0.0000
T23	20-68-1732'-0"	4'-5-533"	X Brace	No	No	Yes	0.0000	0.0000

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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

Tower Section	Tower Elevation	Diagonal Shear	Bracing Type	Has K Brace	Has Horizontals	Top Chl Offset	Bottom Chl Offset
T24	58-1732'-9"	2'-2 7/8"	X-Brace	No	Yes	0.0000	0.0000

**Tower Section Geometry (cont'd)**

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 457'-436"	Solid Round	3	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T2 456'-421"	Solid Round	2 3/4	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T3 421'-401"	Solid Round	2 3/4	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T4 401'-381"	Arbitrary Shape	3'-5" S.R. w/ 3/16 SCH40 Half Pipe	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T5 381'-361"	Arbitrary Shape	3'-5" S.R. w/ 3/16 SCH40 Half Pipe and 1.75" x 5/16 Half Pipe	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T6 361'-341"	Solid Round	3	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T7 341'-321"	Solid Round	3	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T8 321'-301"	Solid Round	3	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T9 301'-281"	Solid Round	3	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T10 281'-261"	Solid Round	3	A572-50 (50 ksi)	Single Angle	L1 1/2x1 1/2x1/4	A36 (36 ksi)
T11 261'-241"	Solid Round	3	A572-50 (50 ksi)	Single Angle	L3x3x1/4	A36 (36 ksi)
T12 241'-221"	Solid Round	3	A572-50 (50 ksi)	Single Angle	L3x3x1/4	A36 (36 ksi)
T13 221'-201"	Solid Round	3	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T14 201'-181"	Solid Round	3 1/4	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T15 181'-161"	Solid Round	3 1/4	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T16 161'-141"	Solid Round	3 1/2	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T17 141'-121"	Solid Round	3 1/2	A572-50 (50 ksi)	Single Angle	L3x3x1/4	A36 (36 ksi)
T18 121'-101"	Solid Round	3 1/2	A572-50 (50 ksi)	Single Angle	L3x3x1/4	A36 (36 ksi)
T19 101'-81"	Solid Round	3 1/2	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T20 81'-61"	Solid Round	3 1/2	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T21 61'-41"	Solid Round	3 1/2	A572-50 (50 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T22 41'-20"	Solid Round	3 1/4	A572-50 (50 ksi)	Single Angle	L2x2x3/16	A36 (36 ksi)
T23 20'-68-1732'-9"	Solid Round	3 1/4	A572-50 (50 ksi)	Single Angle	L2x2x3/16	A36 (36 ksi)

**Tower Section Geometry (cont'd)**

Tower Elevation	Top Chl Type	Top Chl Size	Top Chl Grade	Bottom Chl Type	Bottom Chl Size	Bottom Chl Grade
T1 457'-436"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T2 456'-421"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T3 421'-401"	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)
T4 401'-381"	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)
T5 381'-361"	Double Angle	L3x2x1/4	A36 (36 ksi)	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)
T6 361'-341"	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)
T7 341'-321"	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)
T8 321'-301"	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)
T9 301'-281"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T10 281'-261"	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)	Single Angle	L2 1/2x2x1/4	A36 (36 ksi)
T11 261'-241"	Double Angle	L3 1/2x3 1/2x3/8	A36 (36 ksi)	Double Angle	L3 1/2x3 1/2x3/8	A36 (36 ksi)
T12 241'-221"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T13 221'-201"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T14 201'-181"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T15 181'-161"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T16 161'-141"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T17 141'-121"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T18 121'-101"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T19 101'-81"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T20 81'-61"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T21 61'-41"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T22 41'-20"	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)	Single Angle	L2 1/2x2x3/16	A36 (36 ksi)
T23 20'-68-1732'-9"	Single Angle	L3x2x3/16	A36 (36 ksi)	Single Angle	L3x2x3/16	A36 (36 ksi)

**Tower Section Geometry (cont'd)**



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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

Tower Elevation f	Culc K Single Angles	Culc K Solid Rounds	Legs		K Factors		Girts		Horiz.		Vert.	
			Brace Diags X Y	Brace Diags X Y	Single Diags X Y	Diags X Y	Horiz. X Y	Horiz. X Y	Vert. X Y	Vert. X Y		
T2 436-421'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T3 421-401'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T4 401-381'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T5 381-361'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T6 361-341'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T7 341-321'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T8 321-301'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T9 301-281'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T10 281-261'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T11 261-241'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T12 241-221'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T13 221-201'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T14 201-181'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T15 181-161'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T16 161-141'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T17 141-121'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T18 121-101'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T19 101-81'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T20 81-61'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T21 61-41'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T22 41-20'	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T23 20'-68-1732'-g	Yes	Yes	X	X	X	X	X	X	X	X	X	X
T24 68-1732'-g	Yes	Yes	X	X	X	X	X	X	X	X	X	X

Note: K factors are applied to member segments lengths. K-braces without inner supporting members still have the K factor in the out-of-plane direction applied to the overall length.

Tower Elevation f	Lag	Diagonal	Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
			Net Width Diagonal in	U	Net Width Diagonal in	U	Net Width Diagonal in	U	Net Width Diagonal in	U	Net Width Diagonal in	U
T1 457-436'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 436-421'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3 421-401'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T4 401-381'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T5 381-361'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T6 361-341'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T7 341-321'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T8 321-301'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T9 301-281'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T10 281-261'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T11 261-241'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T12 241-221'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T13 221-201'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T14 201-181'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T15 181-161'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T16 161-141'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T17 141-121'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T18 121-101'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T19 101-81'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T20 81-61'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T21 61-41'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T22 41-20'	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T23 20'-68-1732'-g	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T24 68-1732'-g	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

**Tower Section Geometry (cont'd)**

Tower Elevation f	Lag Component Type	Lag Bolt Size No. in	Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
			Bolt Size No. in	U	Bolt Size No. in	U	Bolt Size No. in	U	Bolt Size No. in	U	Bolt Size No. in	U	Bolt Size No. in	U
T1 457-436'	Flange	A325N 0.8750 8	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2
T2 436-421'	Flange	A325N 0.8750 8	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2
T3 421-401'	Flange	A325N 0.8750 8	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2
T4 401-381'	Flange	A325N 0.8750 8	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2
T5 381-361'	Flange	A325N 0.8750 8	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2
T6 361-341'	Flange	A325N 0.8750 8	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2
T7 341-321'	Flange	A325N 0.8750 8	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2
T8 321-301'	Flange	A325N 0.8750 8	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2
T9 301-281'	Flange	A325N 0.8750 8	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2	0.5000	2	A325N 0.5000 2

**Tower Section Geometry (cont'd)**

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Designed by		Matt Lackey, E.I.			

Tower Elevation	Leg Connection Type	Leg Size	Leg	Diagonal	Top Gtr	Bottom Gtr	Mir Gtr	Long Horizontal	Short Horizontal
$\beta$		in	in	in	in	in	in	in	in
T10 281'-261'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T11 261'-241'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T12 241'-221'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T13 221'-201'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T14 201'-181'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T15 181'-161'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T16 161'-141'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T17 141'-121'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T18 121'-101'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T19 101'-81'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T20 81'-61'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T21 61'-41'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T22 41'-20'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T23 20'-08'	Flange	0.8750	A325N	2	0.5000	2	0.5000	2	0.0000
T24 68'-1732'-0'	Flange	0.0000	A325N	2	0.5000	2	0.5000	2	0.0000
T25 68'-1732'-0'	Flange	0.0000	A325N	2	0.5000	2	0.5000	2	0.0000

Guy Elevation	Guy Grade	Guy Size	Initial Tension	% Tension	Guy Modulus	Guy Height	$L_e$	Anchor Radius	Anchor Height	Anchor Elevation	End Flying Efficiency %
131	A	3/4	6800.00	10%	24000	1.180	4261.3732	403	0.0000	-20'	100%
	B	3/4	6800.00	10%	24000	1.180	4261.3732	407.6	0.0000	-9'	100%
	C	3/4	6800.00	10%	24000	1.180	4451.3732	424.6	0.0000	-16.6	100%
251	BS	1 1/4	19200.00	10%	24000	3.280	4841.3716	405	0.0000	-20'	100%
	B	1 1/4	19200.00	10%	24000	3.280	4712.732	394	0.0000	-13'	100%
	C	1 1/4	19200.00	10%	24000	3.280	4894.5716	411	0.0000	-20.6	100%
381	BS	1 3/8	23200.00	10%	24000	3.970	5673.132	405	0.0000	-20'	100%
	B	1 3/8	23200.00	10%	24000	3.970	5544.5716	394	0.0000	-13'	100%
	C	1 3/8	23200.00	10%	24000	3.970	5718.932	411	0.0000	-20.6	100%

### Guy Data

Guy Elevation	Guy Grade	Guy Size	Initial Tension	% Tension	Guy Modulus	Guy Height	$L_e$	Anchor Radius	Anchor Height	Anchor Elevation	End Flying Efficiency %
131	A	3/4	6800.00	10%	24000	1.180	4261.3732	403	0.0000	-20'	100%
	B	3/4	6800.00	10%	24000	1.180	4261.3732	407.6	0.0000	-9'	100%
	C	3/4	6800.00	10%	24000	1.180	4451.3732	424.6	0.0000	-16.6	100%
251	BS	1 1/4	19200.00	10%	24000	3.280	4841.3716	405	0.0000	-20'	100%
	B	1 1/4	19200.00	10%	24000	3.280	4712.732	394	0.0000	-13'	100%
	C	1 1/4	19200.00	10%	24000	3.280	4894.5716	411	0.0000	-20.6	100%
381	BS	1 3/8	23200.00	10%	24000	3.970	5673.132	405	0.0000	-20'	100%
	B	1 3/8	23200.00	10%	24000	3.970	5544.5716	394	0.0000	-13'	100%
	C	1 3/8	23200.00	10%	24000	3.970	5718.932	411	0.0000	-20.6	100%

### Guy Data (cont'd)

Guy Elevation	Manul Type	Torque-Arm Spread	Torque-Arm Log Angle	Torque-Arm Style	Torque-Arm Grade	Torque-Arm Type	Torque-Arm Size
131	Corner	15'	37.1000	But Ear	A35	Double Angle	2L3X3X3/16
251	Corner				(36 ksi)		
381	Corner						

### Guy Data (cont'd)

Guy Elevation	Diagonal Grade	Diagonal Type	Upper Diagonal Size	Lower Diagonal Size	Strop	Pull-Off Grade	Pull-Off Type	Pull-Off Size
131	A372-50 (50 ksi)	Solid Round				A372-50 (50 ksi)	Solid Round	
251	A372-50 (50 ksi)	Solid Round				A372-50 (50 ksi)	Solid Round	
381	A372-50 (50 ksi)	Solid Round				A372-50 (50 ksi)	Solid Round	

### Guy Data (cont'd)

Guy Elevation	Cable Height	Cable Weight	Cable Height	Cable Weight	Intercept	Tower Intercept	Tower Intercept	Tower Intercept
$\beta$	ft	lb	ft	lb	A	B	C	D
131	502.79	403.36	525.70	403.36	156.27732	157.7116	157.7116	157.7116
251	1587.84	1545.10	1605.10	1545.10	197.3432	197.3432	197.3432	197.3432
381	2251.31	2200.82	2269.62	2200.82	287.1116	287.1116	287.1116	287.1116

### Guy Data (cont'd)

Guy Elevation	Cable Height	Cable Weight	Cable Height	Cable Weight	Intercept	Tower Intercept	Tower Intercept	Tower Intercept
$\beta$	ft	lb	ft	lb	A	B	C	D
131	502.79	403.36	525.70	403.36	156.27732	157.7116	157.7116	157.7116
251	1587.84	1545.10	1605.10	1545.10	197.3432	197.3432	197.3432	197.3432
381	2251.31	2200.82	2269.62	2200.82	287.1116	287.1116	287.1116	287.1116

### Guy Data (cont'd)

Guy Elevation	Cable Height	Cable Weight	Cable Height	Cable Weight	Intercept	Tower Intercept	Tower Intercept	Tower Intercept
$\beta$	ft	lb	ft	lb	A	B	C	D
131	502.79	403.36	525.70	403.36	156.27732	157.7116	157.7116	157.7116
251	1587.84	1545.10	1605.10	1545.10	197.3432	197.3432	197.3432	197.3432
381	2251.31	2200.82	2269.62	2200.82	287.1116	287.1116	287.1116	287.1116

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Designed by		Matt Lackey, E.I.			

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Cup Elevation	Boil Size	Thimble Amt	Boil Size	Pull Off	Diagonal
ft	in	Number	in	Number	in
131	0.7500	8	0.6250	0	0.0000
	A325N		A325N		0
251	0.0000	0	0.6250	0	0.0000
	A325N		A325N		0
381	0.0000	0	0.6250	0	0.0000
	A325N		A325N		0

### Guy Pressures

Cup Elevation	Guy Location	z	r	q	Ice Thickness
ft		ft	ft	psf	in
131	A	558"	21	16	0.5000
	B	91"	22	16	0.5000
	C	1156"	22	20	0.5000
251	A	1156"	27	20	0.5000
	B	1157"	26	20	0.5000
	C	1806"	30	23	0.5000
381	A	184"	30	23	0.5000
	B	1803"	30	23	0.5000

### Feed Line/Linear Apparutances - Entered As Round Or Flat

Description	Face	Clear	Per	Weight						
	Leg	Spacing	Row	in						
LDF5-50A (7/8 FOAM)	B	Yes	Ar (C/Ac)	432" - 14"	0.0000	-0.45	1	1.0900	1.0900	0.33
LDF7-50A (1-5/8 FOAM)	B	Yes	Ar (C/Ac)	354" - 14"	0.0000	-0.05	1	1.9800	1.9800	0.82
EW52	B	Yes	Ar (C/Ac)	146" - 14"	0.0000	-0.15	1	1.7426	1.7426	5.5505
LDF6-50A (1-1/4 FOAM)	B	Yes	Ar (C/Ac)	346" - 14"	0.0000	-0.05	1	1.5500	1.5500	0.66
LDF6-50A (1-1/4 FOAM)	B	Yes	Ar (C/Ac)	330" - 14"	0.0000	0.1	1	1.0900	1.0900	0.33
LDF5-50A (7/8 FOAM)	B	Yes	Ar (C/Ac)	184" - 14"	0.0000	0.15	1	1.0900	1.0900	0.33
LDF7-50A (1-5/8 FOAM)	B	Yes	Ar (C/Ac)	372" - 14"	0.0000	0.2	1	1.9800	1.9800	0.82
HBR-50B (3 AIR)	B	Yes	Ar (C/Ac)	410" - 14"	0.0000	0.25	1	3.0100	3.0100	1.78
LDF7-50A (1-5/8 FOAM)	B	Yes	Ar (C/Ac)	267" - 14"	0.0000	0.3	1	1.9800	1.9800	0.82

Description	Face	Clear	Per	Weight						
	Leg	Spacing	Row	in						
LDF7-50A (1-5/8 FOAM)	B	Yes	Ar (C/Ac)	76" - 14"	0.0000	0.4	1	1.9800	1.9800	0.82
LDF7-50A (1-5/8 FOAM)	B	Yes	Ar (C/Ac)	278" - 14"	0.0000	0.4	1	1.9800	1.9800	0.82
LDF7-50A (1-5/8 FOAM)	B	Yes	Ar (C/Ac)	400" - 14"	0.0000	0.475	1	1.9800	1.9800	0.82
LDF5-50A (7/8 FOAM)	B	Yes	Ar (C/Ac)	400" - 14"	0.0000	0.475	1	0.2500	0.0000	0.33
LDF5-50A (7/8 FOAM)	B	Yes	Ar (C/Ac)	445" - 400"	0.0000	0.475	1	1.0900	1.0900	0.33
LDF5-50A (7/8 FOAM)	B	No	Ar (L/eg)	204" - 14"	0.0000	0	1	1.0900	1.0900	0.33
CR-50.07070E	B	No	Ar (L/eg)	154" - 14"	0.0000	0	1	1.1700	1.1700	0.38
LDF1-50A (1/4 FOAM)	B	No	Ar (L/eg)	68" - 14"	0.0000	0	1	0.3500	0.3500	0.06
LDF1-50A (1/4 FOAM)	B	No	Ar (L/eg)	67" - 14"	0.0000	0	1	0.3500	0.3500	0.06
LDF5-50A (7/8 FOAM)	C	Yes	Ar (C/Ac)	111" - 14"	0.0000	-0.45	1	1.0900	1.0900	0.33
LDF5-50A (7/8 FOAM)	C	Yes	Ar (C/Ac)	343" - 14"	0.0000	-0.425	1	1.0900	1.0900	0.33
LDF7-50A (1-5/8 FOAM)	C	Yes	Ar (C/Ac)	400" - 14"	0.0000	-0.3	1	1.9800	1.9800	0.82
LDF7-50A (1-5/8 FOAM)	C	Yes	Ar (C/Ac)	347" - 14"	0.0000	0.25	1	1.9800	1.9800	0.82
LDF6-50A (1-1/4 FOAM)	C	Yes	Ar (C/Ac)	260" - 14"	0.0000	0.375	1	1.5500	1.5500	0.66
LDF6-50A (1-1/4 FOAM)	C	Yes	Ar (C/Ac)	285" - 14"	0.0000	0.35	1	1.5500	1.5500	0.66
LDF6-50A (1-1/4 FOAM)	C	Yes	Ar (C/Ac)	269" - 14"	0.0000	0.325	1	1.5500	1.5500	0.66
LDF5-50A (7/8 FOAM)	C	Yes	Ar (C/Ac)	107" - 14"	0.0000	0.45	1	1.0900	1.0900	0.33
LDF5-50A (7/8 FOAM)	C	Yes	Ar (C/Ac)	117" - 14"	0.0000	0.45	1	1.0900	1.0900	0.33
LDF5-50A (7/8 FOAM)	C	Yes	Ar (C/Ac)	133" - 14"	0.0000	0.45	1	1.0900	1.0900	0.33



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Client		Crown Castle		11:08:39 01/10/08
Designed by		Matt Lackey, E.I.		

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Designed by		Matt Lackey, E.I.		

Description	Face Allow or Shield Log	Clearance	Component Type	Placement	Face Offset	Lateral Offset	# Per Spacing	Width or Diameter	Parameter	Height	Weight
(SLA)											
LDF5-50A (1-5/8 FOAM) (TX # 38)	A	Yes	Ar (C/Pac)	247' - 14"	0.0000	0.2	24	12	0.2500	1.9800	0.82
LDF6-50A (1-1/4 FOAM) (TX # 46)	A	No	Ar (Leg)	328' - 14"	0.0000	0.15	1	1.5500	1.5500	0.66	
LDF7-50A (1-1/4 FOAM) (TX # 50)	A	No	Ar (Leg)	112' - 14"	0.0000	0.1	1	0.2700	0.2700	0.04	
LDF8-50A (1-1/4 FOAM) (TX # 54)	A	No	Ar (Leg)	211' - 14"	0.0000	0.1	1	0.2700	0.2700	0.04	
LDF9-50A (1-5/8 FOAM) (TX # 59)	B	Yes	Ar (C/Pac)	409' - 14"	-3.0000	0.1	1	1.9800	1.9800	0.82	
LDF10-50A (1-1/4 FOAM) (TX # 60)	B	Yes	Ar (C/Pac)	214' - 14"	-1.0000	0.425	1	1.0900	1.0900	0.33	
LDF11-50A (1-1/4 FOAM) (TX # 61)	C	Yes	Ar (C/Pac)	365' - 14"	-6.0000	-0.15	1	3.1300	3.1300	3.00	
LDF12-50A (1-1/4 FOAM) (TX # 62)	C	Yes	Ar (C/Pac)	457' - 14"	-5.0000	0	1	4.0000	4.0000	2.50	
LDF13-50A (1-1/4 FOAM) (TX # 63)	C	Yes	Ar (C/Pac)	457' - 14"	-10.0000	0.2	1	1.9000	1.9000	1.00	
LDF14-50A (1-1/4 FOAM) (TX # 64)	C	No	Ar (C/Pac)	333' - 14"	-10.0000	0.15	1	1.0000	0.0000	1.13	
LDF15-50A (1-1/4 FOAM) (TX # 65)	C	Yes	Ar (C/Pac)	49' - 14"	-1.5000	0.25	1	1.5500	1.5500	0.66	
LDF16-50A (1-1/4 FOAM) (TX # 66)	C	Yes	Ar (C/Pac)	44' - 14"	-1.5000	0.225	1	1.5500	1.5500	0.66	
LDF17-50A (1-1/4 FOAM) (TX # 67)	A	Yes	Ar (C/Pac)	256' - 14"	0.0000	-0.3	1	1.0900	1.0900	0.33	
LDF18-50A (1-1/4 FOAM) (TX # 68)	A	Yes	Ar (C/Pac)	285' - 14"	0.0000	-0.34	1	1.9800	1.9800	0.82	
LDF19-50A (1-1/4 FOAM) (TX # 69)	A	Yes	Ar (C/Pac)	40' - 14"	0.0000	-0.28	1	0.6300	0.6300	0.15	
LDF20-50A (1-1/4 FOAM) (TX # 70)	A	Yes	Ar (C/Pac)	440' - 14"	0.0000	-0.38	1	1.9800	1.9800	0.82	
LDF21-50A (1-1/4 FOAM) (TX # 71)	A	Yes	Ar (C/Pac)	443' - 14"	0.0000	-0.42	1	1.5500	1.5500	0.66	

### Feed Line/Linear Appearances - Entered As Area

Description	Face Allow or Shield	Component Type	Placement	Face Offset	Lateral Offset	# Per Spacing	Width or Diameter	Parameter	Height	Weight
(SLA)										
LDF22-50A (1-1/2 FOAM) (TX # 72)	B	No	CuAa (In Face)	175'6" - 14"	0.0600	-0.1	1	No Lec	0.00	0.15
LDF23-50A (1-1/2 FOAM) (TX # 73)	B	No	CuAa (In Face)	322' - 14"	0.0000	-0.1	1	No Lec	0.00	0.84
LDF24-50A (1-1/2 FOAM) (TX # 74)	B	No	CuAa (In Face)	346' - 14"	0.0000	-0.1	2	No Lec	0.00	0.84
LDF25-50A (1-1/2 FOAM) (TX # 75)	B	No	CuAa (In Face)	100' - 14"	0.0000	-0.15	1	No Lec	0.00	0.58
LDF26-50A (1-1/4 FOAM) (TX # 76)	B	No	CuAa (In Face)	330' - 14"	0.0000	-0.05	1	No Lec	0.00	1.91
LDF27-50A (1-1/4 FOAM) (TX # 77)	B	No	CuAa (In Face)	133'6" - 14"	0.0000	-0.05	1	No Lec	0.00	1.30
LDF28-50A (1-1/4 FOAM) (TX # 78)	C	No	CuAa (In Face)	340' - 14"	0.0000	0	1	No Lec	0.00	3.33
LDF29-50A (1-1/4 FOAM) (TX # 79)	C	No	CuAa (In Face)	54' - 14"	0.0000	0.45	1	No Lec	0.00	0.33
LDF30-50A (1-1/4 FOAM) (TX # 80)	C	No	CuAa (In Face)	62' - 14"	0.0000	0.45	1	No Lec	0.00	1.30
LDF31-50A (1-1/4 FOAM) (TX # 81)	C	No	CuAa (In Face)	68' - 14"	0.0000	0.45	1	No Lec	0.00	0.33

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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

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Description	Face Allow. or Shield	Component Type	Placement	Face Offset (ft)	Lateral Offset (ft)	#	C <sub>A</sub> A <sub>1</sub>	Weight
	Leg		β	In	(ft)		ft/lb	lb
(78 FOAM) - Ice Weight Only						1/2" Ice	0.00	1.30
(TX # 35) LDF5-50A (78 FOAM) - Ice Weight Only	B No	CrAs (In Face)	445° - 14'	-2.0000	0.425	1	No Ice 1/2" Ice	0.00 1.30
(TX # 31) LDF5-50A (78 FOAM) - Ice Weight Only	B No	CrAs (In Face)	232° - 14'	0.0000	0.05	2	No Ice 1/2" Ice	0.00 1.30
(TX # 52-53) LDF5-50A (78 FOAM) - Ice Weight Only	A No	CrAs (In Face)	232° - 14'	-1.0000	-0.425	1	No Ice 1/2" Ice	0.00 1.30
(TX # 36) LDF5-50A (78 FOAM) - Ice Weight Only	A No	CrAs (In Face)	238° - 14'	-2.0000	-0.4	3	No Ice 1/2" Ice	0.00 1.30

**Feed Line/Linear Appurtenances Section Areas**

Tower Section	Tower Elevation	Face	A <sub>s</sub>	A <sub>r</sub>	C <sub>A</sub> A <sub>1</sub>	In Face	Out Face	Weight
			ft <sup>2</sup>	ft <sup>2</sup>	ft/lb	ft	ft	lb
L1	490'-457'	A	0.000	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.000	0.00
T1	457'-436'	A	1.564	0.000	0.000	0.000	0.000	7.90
		B	1.050	0.000	0.000	0.000	0.000	6.93
T2	436'-421'	A	4.412	0.000	0.000	0.000	0.000	22.20
		B	2.725	0.000	0.000	0.000	0.000	14.83
T3	421'-401'	A	5.883	0.000	0.000	0.000	0.000	29.60
		B	5.891	0.000	0.000	0.000	0.000	35.82
T4	401'-381'	A	5.833	0.000	0.000	0.000	0.000	29.60
		B	11.184	0.000	0.000	0.000	0.000	85.58
T5	381'-361'	A	5.883	0.000	0.000	0.000	0.000	29.60
		B	15.248	0.000	0.000	0.000	0.000	97.22
T6	361'-341'	A	5.883	0.000	0.000	0.000	0.000	29.60
		B	19.524	0.000	0.000	0.000	0.000	120.06
T7	341'-321'	A	6.529	0.000	0.000	0.000	0.000	32.90
		B	24.089	0.000	0.000	0.000	0.000	149.26
T8	321'-301'	A	23.467	0.000	0.000	0.000	0.000	198.34
		B	84.657	0.000	0.000	0.000	0.000	442.80
		C	23.467	0.000	0.000	0.000	0.000	103.00
		C	23.467	0.000	0.000	0.000	0.000	206.40

Tower Section	Tower Elevation	Face	A <sub>s</sub>	A <sub>r</sub>	C <sub>A</sub> A <sub>1</sub>	In Face	Out Face	Weight
			ft <sup>2</sup>	ft <sup>2</sup>	ft/lb	ft	ft	lb
T9	301'-281'	A	9.127	0.000	0.000	0.000	0.000	46.08
		B	27.017	0.000	0.000	0.000	0.000	163.00
T10	281'-261'	A	13.428	0.000	0.000	0.000	0.000	243.08
		B	30.812	0.000	0.000	0.000	0.000	59.20
T11	261'-241'	A	25.009	0.000	0.000	0.000	0.000	181.86
		B	33.617	0.000	0.000	0.000	0.000	182.73
T12	241'-221'	A	36.104	0.000	0.000	0.000	0.000	195.80
		B	53.183	0.000	0.000	0.000	0.000	283.94
T13	221'-201'	A	36.233	0.000	0.000	0.000	0.000	203.06
		B	53.408	0.000	0.000	0.000	0.000	283.60
T14	201'-181'	A	38.018	0.000	0.000	0.000	0.000	214.44
		B	53.633	0.000	0.000	0.000	0.000	283.60
T15	181'-161'	A	38.050	0.000	0.000	0.000	0.000	223.35
		B	53.633	0.000	0.000	0.000	0.000	283.60
T16	161'-141'	A	39.517	0.000	0.000	0.000	0.000	231.57
		B	53.633	0.000	0.000	0.000	0.000	283.60
T17	141'-121'	A	40.784	0.726	0.000	0.000	0.000	283.39
		B	53.633	0.000	0.000	0.000	0.000	283.60
T18	121'-101'	A	41.967	2.904	0.000	0.000	0.000	253.22
		B	53.633	0.000	0.000	0.000	0.000	283.60
T19	101'-81'	A	44.769	2.904	0.000	0.000	0.000	255.80
		B	53.633	0.000	0.000	0.000	0.000	283.60
T20	81'-61'	A	47.267	2.904	0.000	0.000	0.000	266.90
		B	54.083	0.000	0.000	0.000	0.000	310.00
T21	61'-41'	A	44.625	2.904	0.000	0.000	0.000	280.26
		B	47.500	0.000	0.000	0.000	0.000	312.64
T22	41'-20'	A	46.353	2.904	0.000	0.000	0.000	286.38
		B	57.834	0.000	0.000	0.000	0.000	324.75
T23	20'-68'-1732"	A	48.703	3.639	0.000	0.000	0.000	300.59
		B	56.280	0.000	0.000	0.000	0.000	374.01
T24	68'-1732'-0"	A	13.915	0.871	0.000	0.000	0.000	85.88
		B	16.080	0.000	0.000	0.000	0.000	106.86
		C	0.000	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.000	0.00

**Feed Line/Linear Appurtenances Section Areas - With Ice**

Tower Section	Tower Elevation	Face	Ice Thickness	A <sub>s</sub>	A <sub>r</sub>	C <sub>A</sub> A <sub>1</sub>	In Face	Out Face	Weight
		Leg	in	ft <sup>2</sup>	ft <sup>2</sup>	ft/lb	ft	ft	lb
L1	490'-457'	A	0.500	0.000	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.000	0.000	0.00
T1	457'-436'	A	0.500	2.481	0.000	0.000	0.000	0.000	21.73



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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

Section	Elevation	Face	A <sub>h</sub>	A <sub>v</sub>	A <sub>p</sub>	A <sub>r</sub>	A <sub>s</sub>	A <sub>t</sub>	A <sub>z</sub>
	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>
T14	201'-181'	C	0.000	2.716	4.685	8.213	8.213	8.213	8.213
T15	181'-161'	A	0.000	3.285	6.843	8.213	8.213	8.213	8.213
T16	161'-141'	B	0.000	2.765	4.289	6.843	6.843	6.843	6.843
T17	141'-121'	C	0.000	2.756	4.685	6.543	6.543	6.543	6.543
T18	121'-101'	A	0.000	2.914	4.483	7.284	7.284	7.284	7.284
T19	101'-81'	B	0.000	2.756	4.685	6.843	6.843	6.843	6.843
T20	81'-61'	C	0.000	3.285	7.641	9.591	9.591	9.591	9.591
T21	61'-41'	A	0.000	2.973	5.344	7.988	7.988	7.988	7.988
T22	41'-20'	B	0.000	2.736	5.471	7.641	7.641	7.641	7.641
T23	20'-68'-1732"	C	0.000	3.185	7.641	8.213	8.213	8.213	8.213
T24	68'-1732"-0'	A	0.000	2.844	5.692	8.104	8.104	8.104	8.104
T25		B	0.000	3.285	6.843	8.213	8.213	8.213	8.213
T26		C	0.000	3.150	4.858	8.213	8.213	8.213	8.213
T27		A	0.000	3.209	5.302	8.023	8.023	8.023	8.023
T28		B	0.000	3.285	6.543	8.213	8.213	8.213	8.213
T29		C	0.000	3.150	4.858	8.213	8.213	8.213	8.213
T30		A	0.000	3.457	5.625	8.642	8.642	8.642	8.642
T31		B	0.000	3.343	5.178	8.357	8.357	8.357	8.357
T32		C	0.000	3.457	5.625	8.642	8.642	8.642	8.642
T33		A	0.000	3.285	6.543	8.213	8.213	8.213	8.213
T34		B	0.000	3.343	5.178	8.357	8.357	8.357	8.357
T35		C	0.000	3.285	6.543	8.213	8.213	8.213	8.213
T36		A	0.000	3.407	5.285	8.213	8.213	8.213	8.213
T37		B	0.000	3.578	5.809	8.944	8.944	8.944	8.944
T38		C	0.000	3.446	5.377	8.666	8.666	8.666	8.666
T39		A	0.000	3.964	6.403	9.911	9.911	9.911	9.911
T40		B	0.000	1.232	2.018	2.588	2.588	2.588	2.588
T41		C	0.000	1.225	1.597	2.574	2.574	2.574	2.574
T42		A	0.000	1.401	1.902	2.943	2.943	2.943	2.943
T43		B	0.000	0.000	0.000	0.000	0.000	0.000	0.000
T44		C	0.000	0.000	0.000	0.000	0.000	0.000	0.000
T45		A	0.000	0.000	0.000	0.000	0.000	0.000	0.000
T46		B	0.000	0.000	0.000	0.000	0.000	0.000	0.000
T47		C	0.000	0.000	0.000	0.000	0.000	0.000	0.000

Section	Elevation	CP <sub>1</sub>	CP <sub>2</sub>	CP <sub>3</sub>	CP <sub>4</sub>	CP <sub>5</sub>	CP <sub>6</sub>	CP <sub>7</sub>	CP <sub>8</sub>
	ft	in	in	in	in	in	in	in	in
L1	490'-457'	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
T1	457'-436'	-0.6959	1.5515	-0.6743	1.4730	-0.6743	1.4730	-0.6743	1.4730
T2	436'-421'	-1.4614	1.4416	-1.3768	1.2614	-1.3768	1.2614	-1.3768	1.2614
T3	421'-401'	-1.0288	1.7132	-1.0206	1.5257	-1.0206	1.5257	-1.0206	1.5257
T4	401'-381'	1.1380	2.1685	1.3044	2.1683	1.3044	2.1683	1.3044	2.1683
T5	381'-361'	1.6471	2.3085	1.8252	2.2965	1.8252	2.2965	1.8252	2.2965
T6	361'-341'	2.8400	2.9792	2.6804	2.5511	2.6804	2.5511	2.6804	2.5511
T7	341'-321'	3.1171	3.1171	3.1171	3.1171	3.1171	3.1171	3.1171	3.1171
T8	321'-301'	2.8857	1.9732	1.1371	1.8581	1.1371	1.8581	1.1371	1.8581
T9	301'-281'	2.8664	2.6837	2.9692	2.5098	2.9692	2.5098	2.9692	2.5098
T10	281'-261'	2.8560	4.0043	2.8422	3.7257	2.8422	3.7257	2.8422	3.7257
T11	261'-241'	0.9839	1.9963	1.2219	2.4938	1.2219	2.4938	1.2219	2.4938
T12	241'-221'	-0.7844	-1.1907	-0.0911	0.2173	-0.0911	0.2173	-0.0911	0.2173
T13	221'-201'	-1.2664	-0.2204	0.2204	0.2491	0.2204	0.2491	0.2204	0.2491
T14	201'-181'	-0.1472	-0.3181	0.7679	0.3895	0.7679	0.3895	0.7679	0.3895
T15	181'-161'	0.0157	-0.9344	0.9712	0.3745	0.9712	0.3745	0.9712	0.3745
T16	161'-141'	0.2873	-0.8027	1.2621	0.4099	1.2621	0.4099	1.2621	0.4099

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offset Horiz	Offset Vert	Offset Latent	Offset Moment	CP <sub>1</sub> From	CP <sub>2</sub> From	CP <sub>3</sub> From	CP <sub>4</sub> From	CP <sub>5</sub> From	CP <sub>6</sub> From	CP <sub>7</sub> From	CP <sub>8</sub> From	CP <sub>9</sub> From	CP <sub>10</sub> From	CP <sub>11</sub> From	CP <sub>12</sub> From	CP <sub>13</sub> From	CP <sub>14</sub> From	CP <sub>15</sub> From	CP <sub>16</sub> From	CP <sub>17</sub> From	CP <sub>18</sub> From	CP <sub>19</sub> From	CP <sub>20</sub> From	CP <sub>21</sub> From	CP <sub>22</sub> From	CP <sub>23</sub> From	CP <sub>24</sub> From	CP <sub>25</sub> From	CP <sub>26</sub> From	CP <sub>27</sub> From	CP <sub>28</sub> From	CP <sub>29</sub> From	CP <sub>30</sub> From	CP <sub>31</sub> From	CP <sub>32</sub> From	CP <sub>33</sub> From	CP <sub>34</sub> From	CP <sub>35</sub> From	CP <sub>36</sub> From	CP <sub>37</sub> From	CP <sub>38</sub> From	CP <sub>39</sub> From	CP <sub>40</sub> From	CP <sub>41</sub> From	CP <sub>42</sub> From	CP <sub>43</sub> From	CP <sub>44</sub> From	CP <sub>45</sub> From	CP <sub>46</sub> From	CP <sub>47</sub> From	CP <sub>48</sub> From	CP <sub>49</sub> From	CP <sub>50</sub> From	CP <sub>51</sub> From	CP <sub>52</sub> From	CP <sub>53</sub> From	CP <sub>54</sub> From	CP <sub>55</sub> From	CP <sub>56</sub> From	CP <sub>57</sub> From	CP <sub>58</sub> From	CP <sub>59</sub> From	CP <sub>60</sub> From	CP <sub>61</sub> From	CP <sub>62</sub> From	CP <sub>63</sub> From	CP <sub>64</sub> From	CP <sub>65</sub> From	CP <sub>66</sub> From	CP <sub>67</sub> From	CP <sub>68</sub> From	CP <sub>69</sub> From	CP <sub>70</sub> From	CP <sub>71</sub> From	CP <sub>72</sub> From	CP <sub>73</sub> From	CP <sub>74</sub> From	CP <sub>75</sub> From	CP <sub>76</sub> From	CP <sub>77</sub> From	CP <sub>78</sub> From	CP <sub>79</sub> From	CP <sub>80</sub> From	CP <sub>81</sub> From	CP <sub>82</sub> From	CP <sub>83</sub> From	CP <sub>84</sub> From	CP <sub>85</sub> From	CP <sub>86</sub> From	CP <sub>87</sub> From	CP <sub>88</sub> From	CP <sub>89</sub> From	CP <sub>90</sub> From	CP <sub>91</sub> From	CP <sub>92</sub> From	CP <sub>93</sub> From	CP <sub>94</sub> From	CP <sub>95</sub> From	CP <sub>96</sub> From	CP <sub>97</sub> From	CP <sub>98</sub> From	CP <sub>99</sub> From	CP <sub>100</sub> From	CP <sub>101</sub> From	CP <sub>102</sub> From	CP <sub>103</sub> From	CP <sub>104</sub> From	CP <sub>105</sub> From	CP <sub>106</sub> From	CP <sub>107</sub> From	CP <sub>108</sub> From	CP <sub>109</sub> From	CP <sub>110</sub> From	CP <sub>111</sub> From	CP <sub>112</sub> From	CP <sub>113</sub> From	CP <sub>114</sub> From	CP <sub>115</sub> From	CP <sub>116</sub> From	CP <sub>117</sub> From	CP <sub>118</sub> From	CP <sub>119</sub> From	CP <sub>120</sub> From	CP <sub>121</sub> From	CP <sub>122</sub> From	CP <sub>123</sub> From	CP <sub>124</sub> From	CP <sub>125</sub> From	CP <sub>126</sub> From	CP <sub>127</sub> From	CP <sub>128</sub> From	CP <sub>129</sub> From	CP <sub>130</sub> From	CP <sub>131</sub> From	CP <sub>132</sub> From	CP <sub>133</sub> From	CP <sub>134</sub> From	CP <sub>135</sub> From	CP <sub>136</sub> From	CP <sub>137</sub> From	CP <sub>138</sub> From	CP <sub>139</sub> From	CP <sub>140</sub> From	CP <sub>141</sub> From	CP <sub>142</sub> From	CP <sub>143</sub> From	CP <sub>144</sub> From	CP <sub>145</sub> From	CP <sub>146</sub> From	CP <sub>147</sub> From	CP <sub>148</sub> From	CP <sub>149</sub> From	CP <sub>150</sub> From	CP <sub>151</sub> From	CP <sub>152</sub> From	CP <sub>153</sub> From	CP <sub>154</sub> From	CP <sub>155</sub> From	CP <sub>156</sub> From	CP <sub>157</sub> From	CP <sub>158</sub> From	CP <sub>159</sub> From	CP <sub>160</sub> From	CP <sub>161</sub> From	CP <sub>162</sub> From	CP <sub>163</sub> From	CP <sub>164</sub> From	CP <sub>165</sub> From	CP <sub>166</sub> From	CP <sub>167</sub> From	CP <sub>168</sub> From	CP <sub>169</sub> From	CP <sub>170</sub> From	CP <sub>171</sub> From	CP <sub>172</sub> From	CP <sub>173</sub> From	CP <sub>174</sub> From	CP <sub>175</sub> From	CP <sub>176</sub> From	CP <sub>177</sub> From	CP <sub>178</sub> From	CP <sub>179</sub> From	CP <sub>180</sub> From	CP <sub>181</sub> From	CP <sub>182</sub> From	CP <sub>183</sub> From	CP <sub>184</sub> From	CP <sub>185</sub> From	CP <sub>186</sub> From	CP <sub>187</sub> From	CP <sub>188</sub> From	CP <sub>189</sub> From	CP <sub>190</sub> From	CP <sub>191</sub> From	CP <sub>192</sub> From	CP <sub>193</sub> From	CP <sub>194</sub> From	CP <sub>195</sub> From	CP <sub>196</sub> From	CP <sub>197</sub> From	CP <sub>198</sub> From	CP <sub>199</sub> From	CP <sub>200</sub> From	CP <sub>201</sub> From	CP <sub>202</sub> From	CP <sub>203</sub> From	CP <sub>204</sub> From	CP <sub>205</sub> From	CP <sub>206</sub> From	CP <sub>207</sub> From	CP <sub>208</sub> From	CP <sub>209</sub> From	CP <sub>210</sub> From	CP <sub>211</sub> From	CP <sub>212</sub> From	CP <sub>213</sub> From	CP <sub>214</sub> From	CP <sub>215</sub> From	CP <sub>216</sub> From	CP <sub>217</sub> From	CP <sub>218</sub> From	CP <sub>219</sub> From	CP <sub>220</sub> From	CP <sub>221</sub> From	CP <sub>222</sub> From	CP <sub>223</sub> From	CP <sub>224</sub> From	CP <sub>225</sub> From	CP <sub>226</sub> From	CP <sub>227</sub> From	CP <sub>228</sub> From	CP <sub>229</sub> From	CP <sub>230</sub> From	CP <sub>231</sub> From	CP <sub>232</sub> From	CP <sub>233</sub> From	CP <sub>234</sub> From	CP <sub>235</sub> From	CP <sub>236</sub> From	CP <sub>237</sub> From	CP <sub>238</sub> From	CP <sub>239</sub> From	CP <sub>240</sub> From	CP <sub>241</sub> From	CP <sub>242</sub> From	CP <sub>243</sub> From	CP <sub>244</sub> From	CP <sub>245</sub> From	CP <sub>246</sub> From	CP <sub>247</sub> From	CP <sub>248</sub> From	CP <sub>249</sub> From	CP <sub>250</sub> From	CP <sub>251</sub> From	CP <sub>252</sub> From	CP <sub>253</sub> From	CP <sub>254</sub> From	CP <sub>255</sub> From	CP <sub>256</sub> From	CP <sub>257</sub> From	CP <sub>258</sub> From	CP <sub>259</sub> From	CP <sub>260</sub> From	CP <sub>261</sub> From	CP <sub>262</sub> From	CP <sub>263</sub> From	CP <sub>264</sub> From	CP <sub>265</sub> From	CP <sub>266</sub> From	CP <sub>267</sub> From	CP <sub>268</sub> From	CP <sub>269</sub> From	CP <sub>270</sub> From	CP <sub>271</sub> From	CP <sub>272</sub> From	CP <sub>273</sub> From	CP <sub>274</sub> From	CP <sub>275</sub> From	CP <sub>276</sub> From	CP <sub>277</sub> From	CP <sub>278</sub> From	CP <sub>279</sub> From	CP <sub>280</sub> From	CP <sub>281</sub> From	CP <sub>282</sub> From	CP <sub>283</sub> From	CP <sub>284</sub> From	CP <sub>285</sub> From	CP <sub>286</sub> From	CP <sub>287</sub> From	CP <sub>288</sub> From	CP <sub>289</sub> From	CP <sub>290</sub> From	CP <sub>291</sub> From	CP <sub>292</sub> From	CP <sub>293</sub> From	CP <sub>294</sub> From	CP <sub>295</sub> From	CP <sub>296</sub> From	CP <sub>297</sub> From	CP <sub>298</sub> From	CP <sub>299</sub> From	CP <sub>300</sub> From	CP <sub>301</sub> From	CP <sub>302</sub> From	CP <sub>303</sub> From	CP <sub>304</sub> From	CP <sub>305</sub> From	CP <sub>306</sub> From	CP <sub>307</sub> From	CP <sub>308</sub> From	CP <sub>309</sub> From	CP <sub>310</sub> From	CP <sub>311</sub> From	CP <sub>312</sub> From	CP <sub>313</sub> From	CP <sub>314</sub> From	CP <sub>315</sub> From	CP <sub>316</sub> From	CP <sub>317</sub> From	CP <sub>318</sub> From	CP <sub>319</sub> From	CP <sub>320</sub> From	CP <sub>321</sub> From	CP <sub>322</sub> From	CP <sub>323</sub> From	CP <sub>324</sub> From	CP <sub>325</sub> From	CP <sub>326</sub> From	CP <sub>327</sub> From	CP <sub>328</sub> From	CP <sub>329</sub> From	CP <sub>330</sub> From	CP <sub>331</sub> From	CP <sub>332</sub> From	CP <sub>333</sub> From	CP <sub>334</sub> From	CP <sub>335</sub> From	CP <sub>336</sub> From	CP <sub>337</sub> From	CP <sub>338</sub> From	CP <sub>339</sub> From	CP <sub>340</sub> From	CP <sub>341</sub> From	CP <sub>342</sub> From	CP <sub>343</sub> From	CP <sub>344</sub> From	CP <sub>345</sub> From	CP <sub>346</sub> From	CP <sub>347</sub> From	CP <sub>348</sub> From	CP <sub>349</sub> From	CP <sub>350</sub> From	CP <sub>351</sub> From	CP <sub>352</sub> From	CP <sub>353</sub> From	CP <sub>354</sub> From	CP <sub>355</sub> From	CP <sub>356</sub> From	CP <sub>357</sub> From	CP <sub>358</sub> From	CP <sub>359</sub> From	CP <sub>360</sub> From	CP <sub>361</sub> From	CP <sub>362</sub> From	CP <sub>363</sub> From	
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Tower Engineering Professionals 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 Fax: (919) 661-6350		Date 11-08-39 01/10/08
Job BU# 873128 "Trumbull"	Project TEP# 072725	Designed by Matt Lackey, E.I.
Client Crown Castle		

Description	Price or Leg	Offset Type	Offsets: Horiz Lateral Vert	Asimuth Adjustment	Placement	C.A. Front	C.A. Side	Height
			ft' ft' ft'	°	ft'	ft'	ft'	ft'
14' x 12' Corner Reflector (Existing Antenna 19)	A	From Leg	0.50 0' 0'	-80.0000	186°	1.69 1/2" Lec	1.40 1.56	25.00 42.22
** Covered Yagi (Existing Antenna 20)	A	From Leg	1.00 0' 0'	-80.0000	188°	No Lec 1/2" Lec	1.26 1.58	25.00 40.89
** 071092 El. 184.5-ft Mount (Existing Antenna 21 Mount)	A	From Leg	1.50 0' 0'	40.0000	204° - 184°	No Lec 1/2" Lec	6.83 8.38	94.98 141.50
** 20-ft. 4 Element Dipole (Existing Antenna 21)	A	From Leg	3.00 0' 0'	40.0000	204° - 184°	No Lec 1/2" Lec	5.00 7.03	60.00 96.96
** 071092 El. 135.5-ft, 204.5-ft Mounts (Existing Antenna 22 Mount)	B	From Leg	2.50 0' 0'	50.0000	204°	No Lec 1/2" Lec	0.75 7.26	86.83 86.69
** 8' x 2.5" Dia Omni (Existing Antenna 22)	B	From Leg	3.00 4' 0'	50.0000	204°	No Lec 1/2" Lec	2.00 2.83	10.00 24.97
** 071092 El. 214-ft Mount (Existing Antenna 23 Mount)	B	From Leg	2.83 0' 0'	90.0000	214°	No Lec 1/2" Lec	1.20 8.26	72.14 110.40
** 14-ft. x 3" Omni (Existing Antenna 23)	B	From Leg	5.67 76° 0'	90.0000	214°	No Lec 1/2" Lec	4.50 6.03	50.00 82.48
** 071092 El. 232-ft, 238-ft Mount (Existing Antenna 24-29 Mount)	A	From Leg	3.50 0' 0'	0.0000	235°	No Lec 1/2" Lec	3.90 19.20	206.18 293.43
** 071092 El. 232-ft, 238-ft Mount (Existing Antenna 24-29 Mount)	B	From Leg	3.50 0' 0'	0.0000	235°	No Lec 1/2" Lec	3.90 19.20	206.18 293.43
** 071092 El. 247-ft (Single Sector) (Existing Antenna 30-32 Mounts)	A	From Leg	1.33 0' 0'	40.0000	247°	No Lec 1/2" Lec	26.02 26.02	295.54 426.93
** 071092 El. 247-ft (Single Sector) (Existing Antenna 30-32 Mounts)	B	From Leg	1.33 0' 0'	40.0000	247°	No Lec 1/2" Lec	26.02 26.02	295.54 426.93
** 071092 El. 247-ft (Single Sector) (Existing Antenna 30-32 Mounts)	C	From Leg	1.33 0' 0'	40.0000	247°	No Lec 1/2" Lec	26.02 26.02	295.54 426.93
** (2) DR65-18-00DPL2Q (T-Mobile (SLA))	A	From Leg	2.67 5' 0'	80.0000	247°	No Lec 1/2" Lec	6.30 6.73	24.00 55.86

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Tower Engineering Professionals 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 Fax: (919) 661-6350		Date 11-08-39 01/10/08
Job BU# 873128 "Trumbull"	Project TEP# 072725	Designed by Matt Lackey, E.I.
Client Crown Castle		

Description	Price or Leg	Offset Type	Offsets: Horiz Lateral Vert	Asimuth Adjustment	Placement	C.A. Front	C.A. Side	Height
			ft' ft' ft'	°	ft'	ft'	ft'	ft'
(2) DR65-18-00DPL2Q (T-Mobile (SLA))	B	From Leg	2.67 5' 0'	80.0000	247°	No Lec 1/2" Lec	2.42 2.76	24.00 55.86
(2) DR65-18-00DPL2Q (T-Mobile (SLA))	C	From Leg	2.67 5' 0'	80.0000	247°	No Lec 1/2" Lec	2.42 2.76	24.00 55.86
** 071092 El. 256-ft-B, 346-ft-A (Existing Antenna 33 Mount)	A	From Leg	1.50 0' 0'	35.0000	256°	No Lec 1/2" Lec	0.68 3.11	23.50 35.30
** 10-ft. x 3-ft. Omni Antenna (Existing Antenna 33)	A	From Leg	3.00 5' 0'	0.0000	256°	No Lec 1/2" Lec	3.00 4.03	13.30 34.29
** 071092 El. 260-ft Mount (Existing Antenna 34 Mount)	C	From Leg	1.50 0' 0'	90.0000	260°	No Lec 1/2" Lec	1.12 2.06	36.53 54.69
** DB80913E-Y (Existing Antenna 34)	C	From Leg	3.00 56° 0'	90.0000	260°	No Lec 1/2" Lec	3.39 4.55	30.00 34.57
** 071092 El. 267-ft, 331-ft Mounts (Existing Antenna 35 Mount)	B	From Leg	3.00 0' 0'	-20.0000	267°	No Lec 1/2" Lec	0.94 7.73	68.09 104.19
** 6-ft. x 1.5" Dia Omni (Existing Antenna 35)	B	From Leg	6.00 0' 0'	-20.0000	267°	No Lec 1/2" Lec	0.90 1.53	15.00 21.49
** 071092 El. 269-ft, 285-ft, 326-ft, 343-ft, 439-ft, 445-ft-A (Existing Antenna 36 Mount)	C	From Leg	3.00 0' 0'	0.0000	269°	No Lec 1/2" Lec	0.34 3.48	42.16 76.96
** 070892 271-ft Mount (Existing Antenna 37 Mount)	B	From Leg	3.00 0' 0'	40.0000	271°	No Lec 1/2" Lec	0.72 2.16	34.71 51.83
** 10-Element 6-ft. x 2-ft. Yagi (Existing Antenna 37)	B	From Leg	6.00 0' 0'	90.0000	271°	No Lec 1/2" Lec	1.00 2.08	15.00 25.00
** 15' x 8" Omni (Existing Antenna 38)	B	From Leg	0.50 76° 0'	0.0000	278°	No Lec 1/2" Lec	11.44 12.35	341.00 420.95
** 071092 El. 268-ft, 385-ft, 326-ft, 343-ft, 439-ft, 465-ft-A (Existing Antenna 39 Mount)	C	From Leg	3.00 0' 0'	0.0000	285°	No Lec 1/2" Lec	0.34 3.48	43.16 76.96
** 10-ft. x 2.5" dia. omni (Existing Antenna 39)	C	From Leg	6.00 5' 0'	0.0000	285°	No Lec 1/2" Lec	2.50 3.53	10.00 38.64
** 8-ft Single Element Dipole (T-Mobile (SLA))	A	From Leg	0.50 0.50	0.0000	285°	No Lec	2.40	25.00

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Client		Crown Castle		Designed by Matt Lackey, E.I.	

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Client		Crown Castle		Designed by Matt Lackey, E.I.	

Description	Price per Leg	Offset Type	Offset: Horizontal Vertical	Adjustment	Placement	C.A. Front	C.A. Side	Weight
			f l h	e	ft	f <sup>2</sup>	f <sup>2</sup>	lb
(Existing Antenna 40)					1/2" Ice	4.02	4.02	44.71
** 2-Bay Shively FM w/ Radomes (Existing Antenna 41)	C	None	0	0.0000	316' - 308'	No Ice	65.00	1086.00
071092 EI 256-n 346-n 10-ft x 2.5" dia. omni (Existing Antenna 51)					1/2" Ice	135.00	135.00	2388.00
** 15' x 8' Omni (Existing Antenna 52)	B	From Leg	3.00	0.0000	326'	No Ice	0.34	42.16
(Existing Antenna 53)					1/2" Ice	3.48	7.56	76.96
** TLP-8M x 11.7-ft (Existing Antenna 54)	A	From Leg	0	0.0000	326'	No Ice	5.00	45.50
(Existing Antenna 55)					1/2" Ice	7.03	7.03	82.46
** 071092 EI 372-n Mount (Existing Antenna 56 Mount)	B	From Leg	6.00	0.0000	328'	No Ice	4.50	20.00
(Existing Antenna 57 Mount)					1/2" Ice	10.65	4.20	50.00
** 8' x 2.5" Dia. Omni (Existing Antenna 58)	C	From Leg	3.00	-20.0000	330'	No Ice	0.76	1.05
(Existing Antenna 59)					1/2" Ice	3.53	4.99	63.57
** 2-Bay Shively FM w/ Radomes (Existing Antenna 60)	A	From Leg	2.50	80.0000	330'	No Ice	0.72	1.05
(Existing Antenna 61)					1/2" Ice	5.34	7.73	104.19
** 071092 EI 267-n 331-n Mounts (Existing Antenna 62 Mount)	B	From Leg	3.00	-60.0000	331'	No Ice	0.47	0.94
(Existing Antenna 63 Mount)					1/2" Ice	3.00	3.00	32.50
** 10-ft x 3-in Omni Antenna (Existing Antenna 64)	B	From Leg	6.00	-60.0000	331'	No Ice	4.03	34.29
(Existing Antenna 65)					1/2" Ice	7.56	76.96	
** 071092 EI 269-n 385-n 326-n 345-n 439-n 445-n (Existing Antenna 66)	A	From Leg	3.00	40.0000	343'	No Ice	0.34	0.50
(Existing Antenna 67)					1/2" Ice	3.48	7.56	
** 071092 EI 256-n-B 346-n (Existing Antenna 68 Mount)	A	From Leg	6.00	40.0000	343'	No Ice	5.00	30.00
(Existing Antenna 69)					1/2" Ice	7.03	7.03	66.96
** 071092 EI 256-n 346-n 445-n-C (Existing Antenna 70 Mount)	A	From Leg	1.50	-20.0000	346'	No Ice	0.47	0.68
(Existing Antenna 71 Mount)					1/2" Ice	2.24	3.11	33.20
** 20-ft x 2.5-in Omni (Existing Antenna 72)	A	From Leg	3.00	-20.0000	346'	No Ice	6.00	16.70
(Existing Antenna 73)					1/2" Ice	8.03	8.03	59.37
** 071092 EI 256-n-B 346-n 445-n-C (Existing Antenna 74 Mount)	B	From Leg	1.50	0.0000	346'	No Ice	0.47	0.68
(Existing Antenna 75 Mount)					1/2" Ice	2.24	3.11	33.20
** 10-ft x 2.5" dia. omni (Existing Antenna 76)	B	From Leg	3.00	0.0000	346'	No Ice	2.50	10.00
(Existing Antenna 77)					1/2" Ice	3.53	3.53	28.64
** 071092 347.5-n Mount	C	From Leg	1.75	-60.0000	3476"	No Ice	0.59	0.77

Description	Price per Leg	Offset Type	Offset: Horizontal Vertical	Adjustment	Placement	C.A. Front	C.A. Side	Weight
			f l h	e	ft	f <sup>2</sup>	f <sup>2</sup>	lb
(Existing Antenna 51 Mount)					1/2" Ice	3.03	2.68	53.79
** 10-ft x 2.5" dia. omni (Existing Antenna 51)	C	From Leg	3.50	-60.0000	3476"	No Ice	2.50	10.00
(Existing Antenna 52)					1/2" Ice	3.53	3.53	28.64
** 15' x 8' Omni (Existing Antenna 52)	A	From Leg	0.50	0.0000	354'	No Ice	11.44	11.44
(Existing Antenna 53)					1/2" Ice	12.35	12.35	120.00
** TLP-8M x 11.7-ft (Existing Antenna 53)	A	From Leg	0.50	0.0000	365'	No Ice	18.70	18.70
(Existing Antenna 54)					1/2" Ice	21.00	21.00	220.00
** 071092 EI 372-n Mount (Existing Antenna 54 Mount)	A	From Leg	3.00	60.0000	372'	No Ice	1.92	3.75
(Existing Antenna 55)					1/2" Ice	11.75	15.09	180.02
** 8' x 2.5" Dia. Omni (Existing Antenna 54)	A	From Leg	6.00	60.0000	372'	No Ice	2.00	10.00
(Existing Antenna 55)					1/2" Ice	2.83	3.83	24.97
** 2-Bay Shively FM w/ Radomes (Existing Antenna 55)	C	None	0	0.0000	400' - 318"	No Ice	65.00	1060.00
(Existing Antenna 56)					1/2" Ice	135.00	135.00	2388.00
** 1083-3CP (Existing Antenna 56)	A	None	0	0.0000	430' - 410"	No Ice	74.25	740.00
(Existing Antenna 57)					1/2" Ice	133.70	133.70	975.00
** 071092 EI 269-n 285-n 326-n 345-n 439-n 445-n (Existing Antenna 57 Mount)	B	From Leg	3.00	-90.0000	439'	No Ice	0.34	0.50
(Existing Antenna 58 Mount)					1/2" Ice	3.48	7.56	76.96
** 20-ft x 2.5-in Omni Antenna (Existing Antenna 57)	B	From Leg	6.00	-90.0000	439'	No Ice	5.00	30.00
(Existing Antenna 58)					1/2" Ice	7.03	7.03	66.96
** 071092 EI 256-n-B 346-n 445-n-C (Existing Antenna 59 Mount)	A	From Leg	3.00	80.0000	445'	No Ice	0.47	0.68
(Existing Antenna 60 Mount)					1/2" Ice	2.24	3.11	33.20
** 20-ft x 2.5-in Omni (Existing Antenna 59)	A	From Leg	6.00	80.0000	445'	No Ice	5.00	45.50
(Existing Antenna 60)					1/2" Ice	7.03	7.03	82.46
** 071092 EI 256-n (Empty Mount) & 445-n-B (Existing Antenna 59 Mount)	B	From Leg	1.75	35.0000	445'	No Ice	0.19	0.29
(Existing Antenna 60)					1/2" Ice	1.51	3.06	21.51
** 20-ft x 2.5-in Omni (Existing Antenna 59)	B	From Leg	3.50	35.0000	445'	No Ice	4.00	25.00
(Existing Antenna 60)					1/2" Ice	6.03	6.03	55.77
** 071092 EI 256-n-B 346-n 445-n-C (Existing Antenna 60 Mount)	C	From Leg	3.00	0.0000	445'	No Ice	0.47	0.68
(Existing Antenna 60)					1/2" Ice	2.24	3.11	33.20
** 20-ft x 2.5-in Omni (Existing Antenna 60)	C	From Leg	6.00	0.0000	445'	No Ice	4.00	25.00
(Existing Antenna 60)					1/2" Ice	6.03	6.03	55.77

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Client		Crown Castle		Designed by Matt Lackey, E.I.	

Description	Face or Leg	Offset Type	Offset Horiz Lateral Vert	Adjustment	Placement	C/A Front	C/A Side	Weight
			ft	ft	ft	ft	ft	lb
1.2M (Reserved)	A	From Leg	1.00	0.0000	305°-235'	18.70	21.00	230.00
18-in. Staircase (USA Mobility (SLA))	A	From Leg	0.00	0.0000	40°	0.29	0.64	12.20
Acoustic 2000 (USA Mobility (SLA))	A	From Leg	0.00	0.0000	40°	0.17	0.17	19.20
071092 EI 256-04B, 346-01 (USA Mobility (SLA))	A	From Leg	3.00	0.0000	440°	0.47	0.68	0.87
071092 EI 256-04B, 346-01 (USA Mobility (SLA))	A	From Leg	6.00	0.0000	440°	2.24	3.11	3.85
071092 EI 256-04B, 346-01 (USA Mobility (SLA))	B	From Leg	3.00	-30.0000	443°-440°	2.12	2.12	23.50
071092 EI 256-04B, 346-01 (USA Mobility (SLA))	B	From Leg	6.00	-30.0000	443°-440°	3.14	3.14	35.30
BXA-1850S12CF (Verizon Proposed)	A	From Leg	7.00	0.0000	235°	4.77	3.64	46.22
BXA-8060S6CF (Verizon Proposed)	A	From Leg	7.00	0.0000	235°	5.22	4.08	13.00
BXA-1850G2/8CF (Verizon Proposed)	B	From Leg	7.00	0.0000	235°	7.91	3.93	40.44
BXA-8060S6CF (Verizon Proposed)	B	From Leg	7.00	0.0000	235°	8.45	4.37	22.00
BXA-1850S12CF (Verizon Proposed)	C	From Leg	7.00	0.0000	235°	2.94	1.79	63.74
BXA-8060S6CF (Verizon Proposed)	C	From Leg	7.00	0.0000	235°	3.26	2.09	27.08
BXA-1850S12CF (Verizon Proposed)	C	From Leg	7.00	0.0000	235°	7.74	3.76	14.90
BXA-8060S6CF (Verizon Proposed)	C	From Leg	7.00	0.0000	235°	8.28	4.20	55.55
BXA-1850S12CF (Verizon Proposed)	C	From Leg	7.00	0.0000	235°	4.77	3.64	13.00
BXA-8060S6CF (Verizon Proposed)	C	From Leg	7.00	0.0000	235°	5.22	4.08	40.44
BXA-1850S12CF (Verizon Proposed)	C	From Leg	7.00	0.0000	235°	7.81	3.93	22.00
BXA-8060S6CF (Verizon Proposed)	C	From Leg	7.00	0.0000	235°	8.45	4.37	63.74

Description	Face or Leg	Offset Type	Offset Horiz Lateral Vert	Adjustment	3 dB Elevation Beam Width	Outside Diameter	Aperture Area	Weight
			ft	ft	ft	ft	ft <sup>2</sup>	lb
1.2M (Existing Antenna 1)	C	Paraboloid w/ Radome	1.75	0.0000	44°	4.00	12.17	165.00
			0'				13.09	232.19

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Client		Crown Castle		Designed by Matt Lackey, E.I.	

Description	Face or Leg	Offset Type	Offset Horiz Lateral Vert	Adjustment	3 dB Elevation Beam Width	Outside Diameter	Aperture Area	Weight
			ft	ft	ft	ft	ft <sup>2</sup>	lb
0.25M-1 (Existing Antenna 2)	C	Paraboloid w/ Radome	0.38	20.0000	49°	2.50	No Lec	4.91
			0'				1/2" Lec	5.24
6-ft x 3-ft Grid (Existing Antenna 4)	C	Grid	2.50	-20.0000	67°	5.00	No Lec	19.63
			0'				1/2" Lec	20.29
6-ft x 3-ft Grid (Existing Antenna 5)	B	Grid	0.50	0.0000	63°	5.00	No Lec	19.63
			0'				1/2" Lec	20.29
6-ft x 3-ft Grid (Existing Antenna 6)	B	Grid	0.50	-60.0000	67°	5.00	No Lec	19.63
			0'				1/2" Lec	20.29
6-ft x 3-ft Grid (Existing Antenna 7)	C	Grid	2.50	-90.0000	68°	5.00	No Lec	19.63
			0'				1/2" Lec	20.29
8-FT Grid (Existing Antenna 8)	B	Grid	0.50	0.0000	76°	8.00	No Lec	50.27
			0'				1/2" Lec	51.32
SPD2-S.8 (Existing Antenna 9)	B	Paraboloid w/ Radome	0.50	-10.0000	100°	2.00	No Lec	3.14
			0'				1/2" Lec	3.41
10-FT Grid (Existing Antenna 10)	B	Grid	4.75	20.0000	109°6'	10.00	No Lec	78.54
			0'				1/2" Lec	79.85
6-ft x 3-ft Grid (Existing Antenna 12)	C	Grid	1.00	20.0000	107°	5.00	No Lec	19.63
			0'				1/2" Lec	20.29
P-9A48GN-U (Existing Antenna 13)	C	Paraboloid w/ Radome	1.00	-60.0000	117°6'	4.00	No Lec	10.10
			0'				1/2" Lec	13.09
Andrew 6 w/ Radome (Existing Antenna 16)	B	Paraboloid w/ Radome	0.50	-45.0000	146°	6.00	No Lec	28.37
			0'				1/2" Lec	29.07
6-FT Grid (Existing Antenna 17)	B	Grid	1.00	-55.0000	154°	6.00	No Lec	28.37
			0'				1/2" Lec	29.07
SP4-S.8 (Existing Antenna 18)	C	Paraboloid w/ Radome	0.50	50.0000	179°6'	4.00	No Lec	12.57
			0'				1/2" Lec	13.09
SPD3-S.8 (Existing Antenna 42)	A	Paraboloid w/ Radome	0.50	0.0000	322°	3.00	No Lec	5.70
			0'				1/2" Lec	7.46
8-FT Grid	C	Grid	0.50	-20.0000	340°	8.00	No Lec	50.27
			0'				1/2" Lec	51.32





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Client	Crown Castle		
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Section No.	Elevation	Component Type	Condition	Gov. Load	Force	Major Axis Moment	Minor Axis Moment	
	$\beta$			Count	lb	lb-ft	lb-ft	
T4	401 - 381	Diagonal	Max. Mx	18	-3472.87	-236.30	-13.12	
			Max. My	15	13133.06	36.61	270.85	
			Max. Vx	18	-241.58	-286.30	-13.12	
			Max. Vy	15	237.92	36.62	270.85	
			Max. Tension	26	5394.28	0.00	0.00	
			Max. Compression	20	-5415.67	0.00	0.00	
			Max. Mx	19	4483.14	9.13	-4.42	
			Max. My	25	-1149.44	-9.13	4.42	
			Max. Vx	18	-174.44	-39.13	-1.41	
			Max. Vy	15	174.44	39.13	1.41	
		Top Girt		Max. Vx	4	-1.75	-5.78	6.70
				Max. Vy	4	73.14	0.00	0.00
				Max. Tension	6	-62.90	0.00	0.00
				Max. Compression	14	14.58	-25.82	0.00
				Max. Mx	22	13.43	0.00	0.00
				Max. My	14	17.22	0.00	0.00
				Max. Vx	22	-0.00	0.00	0.00
				Max. Vy	4	130.31	0.00	0.00
				Max. Tension	6	-136.51	0.00	0.00
				Max. Compression	14	16.23	-25.82	0.00
Leg		Max. Mx	22	17.22	0.00	0.00		
		Max. My	14	17.22	0.00	0.00		
		Max. Vx	22	-0.00	0.00	0.00		
		Max. Vy	25	11308.54	332.14	-0.49		
		Max. Tension	19	-130097.64	912.64	1.42		
		Max. Compression	17	110743.13	-1318.58	-7.01		
		Max. Mx	20	-5763.10	223.17	-716.64		
		Max. My	17	-463.35	-646.26	-3.88		
		Max. Vx	16	-445.63	10.99	-648.14		
		Max. Vy	26	8671.96	0.00	0.00		
Diagonal		Max. Tension	20	-3743.31	0.00	0.00		
		Max. Compression	26	2789.46	40.90	0.38		
		Max. Mx	16	17.82	-23.14	-11.81		
		Max. My	26	-6633.36	-23.14	11.81		
		Max. Vx	20	36.61	-23.14	0.38		
		Max. Vy	26	-36.61	23.14	-0.38		
		Max. Tension	25	3872.85	0.00	0.00		
		Max. Compression	19	-3872.85	0.00	0.00		
		Max. Mx	14	24.47	-25.82	0.00		
		Max. My	22	23.74	0.00	0.00		
Top Girt		Max. Vx	14	17.22	0.00	0.00		
		Max. Vy	22	-0.00	0.00	0.00		
		Max. Tension	25	529.98	0.00	0.00		
		Max. Compression	1	0.00	0.00	0.00		
		Max. Mx	14	289.14	-25.82	0.00		
		Max. My	22	371.68	0.00	0.00		
		Max. Vx	14	17.22	0.00	0.00		
		Max. Vy	22	-0.00	0.00	0.00		
		Max. Tension	25	6090.72	-1295.59	-1.40		
		Max. Compression	19	-135103.27	-106.20	46.36		
Leg		Max. Mx	17	59768.68	-1318.56	-6.99		
		Max. My	24	-35521.97	65.21	-570.19		
		Max. Vx	17	-339.44	-1318.56	-6.99		
		Max. Vy	5	539.13	-126.60	444.03		
		Max. Tension	16	7251.55	0.00	0.00		
		Max. Compression	21	-7086.49	0.00	0.00		
		Max. Mx	18	-5241.74	45.23	7.35		
		Max. My	21	15.69	-42.21	-17.33		
		Max. Vx	21	-15.69	45.23	7.35		
		Max. Vy	18	15.69	-45.23	-7.35		
Diagonal		Max. Tension	15	21407.73	0.00	0.00		
		Max. Compression	1	0.00	0.00	0.00		
		Max. Mx	1	10847.83	50.76	0.00		
		Max. My	1	0.00	0.00	0.00		
		Max. Vx	1	0.00	0.00	0.00		
		Max. Vy	1	0.00	0.00	0.00		
		Max. Tension	1	0.00	0.00	0.00		
		Max. Compression	1	0.00	0.00	0.00		
		Max. Mx	1	0.00	0.00	0.00		
		Max. My	1	0.00	0.00	0.00		

Section No.	Elevation	Component Type	Condition	Gov. Load	Force	Major Axis Moment	Minor Axis Moment	
	$\beta$			Count	lb	lb-ft	lb-ft	
T6	361 - 341	Mid Girt	Max. My	22	4090.18	0.00	0.00	
			Max. Vx	14	-33.84	0.00	0.00	
			Max. Vy	22	0.00	0.00	0.00	
			Max. Tension	25	646.23	0.00	0.00	
			Max. Compression	1	0.00	0.00	0.00	
			Max. Mx	14	394.19	-25.82	0.00	
			Max. My	22	509.38	0.00	0.00	
			Max. Vx	14	17.22	0.00	0.00	
			Max. Vy	22	-0.00	0.00	0.00	
			Max. Tension	21	70.00	0.00	0.00	
		Guy A		Top Cable Vert	31	72488.95		
				Top Cable Norm	31	52413.41		
				Top Cable Tan	31	50074.76		
				Bot Cable Vert	21	-83773.56		
				Bot Cable Norm	21	1.34		
				Bot Cable Tan	21	51212.50		
				Bottom Tension	25	70596.30		
				Top Tension	25	72603.11		
				Top Cable Vert	25	25233.45		
				Top Cable Norm	25	4991.42		
Guy B		Top Cable Tan	25	0.19				
		Bot Cable Vert	25	-46762.44				
		Bot Cable Norm	25	51049.88				
		Bot Cable Tan	25	76536.11				
		Bottom Tension	17	72601.06				
		Top Tension	17	52158.32				
		Top Cable Vert	17	50501.20				
		Top Cable Norm	17	8.03				
		Top Cable Tan	17	-46094.52				
		Bot Cable Vert	17	51624.43				
Guy C		Bot Cable Norm	17	15865.94				
		Bot Cable Tan	17	86599.23				
		Max. Compression	18	-28904.36				
		Max. Mx	26	-51683.91				
		Max. Vy	24	477.20				
		Max. Vx	26	384.92				
		Max. Tension	18	5233.30				
		Max. Compression	18	-5317.24				
		Max. Mx	19	-4548.42				
		Max. My	24	-4392.12				
Log		Max. Vy	19	-19.17				
		Max. Vx	24	33.8				
		Max. Tension	35	648.73				
		Max. Compression	6	-159.79				
		Max. Mx	14	140.20				
		Max. My	22	166.36				
		Max. Vx	14	17.22				
		Max. Vy	22	-0.00				
		Max. Tension	25	297.08				
		Max. Compression	14	-12.86				
Top Girt		Max. Mx	14	126.86				
		Max. My	22	147.91				
		Max. Vx	4	-17.22				
		Max. Vy	22	-0.00				
		Max. Tension	1	0.00				
		Max. Compression	1	0.00				
		Max. Mx	1	0.00				
		Max. My	1	0.00				
		Max. Vx	1	0.00				
		Max. Vy	1	0.00				
Mid Girt		Max. Tension	19	-53969.93				
		Max. Compression	26	45585.52				
		Max. Mx	26	-4807.77				
		Max. My	26	-48122.74				
		Max. Vx	26	-113.59				
		Max. Vy	26	-662.45				
		Max. Tension	26	7.88				
		Max. Compression	26	-105.80				
		Max. Mx	26	-117.67				
		Max. My	26	-446.14				
Diagonal		Max. Vx	26	190.42				
		Max. Vy	26	384.92				
		Max. Tension	18	5233.30				
		Max. Compression	18	-5317.24				
		Max. Mx	19	-4548.42				
		Max. My	24	-4392.12				
		Max. Vx	24	33.8				
		Max. Vy	19	-19.17				
		Max. Tension	35	648.73				
		Max. Compression	6	-159.79				
Log		Max. Mx	14	140.20				
		Max. My	22	166.36				
		Max. Vx	14	17.22				
		Max. Vy	22	-0.00				
		Max. Tension	25	297.08				
		Max. Compression	14	-12.86				
		Max. Mx	14	126.86				
		Max. My	22	147.91				
		Max. Vx	4	-17.22				
		Max. Vy	22	-0.00				
Top Girt		Max. Tension	1	0.00				
		Max. Compression	1	0.00				
		Max. Mx	1	0.00				
		Max. My	1	0.00				
		Max. Vx	1	0.00				
		Max. Vy	1	0.00				
		Max. Tension	1	0.00				
		Max. Compression	1	0.00				
		Max. Mx	1	0.00				
		Max. My	1	0.00				

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Client		Crown Castle		Designed by Matt Lackey, E.I.	

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Tower Engineering Professionals 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6531 Fax: (919) 661-6530		TEP# 072725		Date	11-08-39 01/10/08
Client		Crown Castle		Designed by Matt Lackey, E.I.	

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft	
T8	301 - 301	Diagonal	Max. Vy	18	-745.12	-117.67	208.55	
			Max. Vx	26	874.14	127.99	0.00	
			Max. Compression	18	3966.64	0.00	0.00	
		Top Gir	Max. Mx	15	-542.69	36.17	-2.46	0.00
			Max. My	18	-3566.92	35.76	7.92	-2.46
			Max. Vy	15	-163.99	36.17	-2.46	0.00
			Max. Vx	18	-2.06	35.76	7.92	-2.46
			Max. Compression	15	-162.60	0.00	0.00	
			Max. Mx	14	128.65	-25.82	0.00	
		Mid Gir	Max. My	14	172.22	0.00	0.00	
			Max. Vy	16	-0.00	0.00	0.00	
			Max. Tension	15	187.09	0.00	0.00	
			Max. Compression	14	127.18	0.00	0.00	
			Max. Mx	16	146.76	-25.82	0.00	
			Max. My	14	172.22	0.00	0.00	
		Leg	Max. Vy	16	-0.00	0.00	0.00	
			Max. Vx	15	-480.80	0.00	0.00	
			Max. Compression	15	-480.80	0.00	0.00	
Max. Mx	18		-3920.99	-106.24	260.96			
Max. My	15		-3326.64	-2.53	504.18			
Max. Vy	18		515.34	-2.53	504.18			
Diagonal	Max. Vx	15	-514.75	0.00	0.00			
	Max. Tension	18	2274.77	0.00	0.00			
	Max. Compression	20	-2298.32	0.00	0.00			
	Max. Mx	15	-471.54	37.67	-2.68			
	Max. My	18	-2141.57	7.93	6.53			
	Max. Vy	15	-163.97	37.67	-2.68			
Top Gir	Max. Vx	18	-1.70	7.93	6.53			
	Max. Tension	25	261.26	0.00	0.00			
	Max. Compression	14	150.19	-25.82	0.00			
	Max. Mx	16	152.87	0.00	0.00			
	Max. My	14	172.22	0.00	0.00			
	Max. Vy	16	-0.00	0.00	0.00			
Mid Gir	Max. Vx	15	500.88	0.00	0.00			
	Max. Tension	17	-171.02	0.00	0.00			
	Max. Compression	14	138.66	-25.82	0.00			
	Max. Mx	16	171.28	0.00	0.00			
	Max. My	14	172.22	0.00	0.00			
	Max. Vy	16	-0.00	0.00	0.00			
Leg	Max. Tension	1	0.00	0.00	0.00			
	Max. Compression	23	-6221.38	261.85	-108.18			
	Max. Mx	18	-61299.34	-265.09	-86.59			
	Max. My	15	-61376.29	4.38	284.15			
	Max. Vy	18	-549.11	-249.04	216.11			
	Max. Vx	26	364.33	-277.91	216.11			
Diagonal	Max. Tension	17	36743.35	0.00	0.00			
	Max. Compression	20	-3785.25	0.00	0.00			
	Max. Mx	16	1271.22	40.71	0.31			
	Max. My	18	-2954.93	5.77	7.75			
	Max. Vy	15	-1.99	5.77	7.75			
	Max. Vx	18	-17.77	40.71	-1.58			
Top Gir	Max. Tension	1	0.00	0.00	0.00			
	Max. Compression	15	182.53	0.00	0.00			
	Max. Mx	14	149.65	-21.96	0.00			
	Max. My	16	152.87	0.00	0.00			
	Max. Vy	14	146.65	0.00	0.00			
	Max. Vx	16	146.65	0.00	0.00			

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
T10	281 - 261	Mid Gir	Max. Vy	16	-0.00	0.00	0.00
			Max. Tension	25	333.21	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
		Leg	Max. Mx	14	136.76	-21.98	0.00
			Max. My	16	14.65	0.00	0.00
			Max. Vy	14	14.65	0.00	0.00
			Max. Vx	16	-0.00	0.00	0.00
			Max. Tension	25	14485.17	216.72	68.28
			Max. Compression	23	-98210.02	357.03	-208.01
		Diagonal	Max. Mx	23	-98210.02	357.03	-208.01
			Max. My	15	-98033.55	-4.59	410.32
			Max. Vy	18	-251.21	-274.45	-121.40
			Max. Vx	16	-278.10	30.94	322.77
			Max. Tension	20	5168.93	0.00	0.00
			Max. Compression	20	-5500.18	0.00	0.00
		Top Gir	Max. Mx	16	2484.71	8.99	-0.32
			Max. My	26	-10.88	15.99	-3.77
			Max. Vy	16	-10.77	15.99	-3.77
Max. Vx	19		-1.37	31.14	5.72		
Max. Tension	25		252.34	0.00	0.00		
Max. Compression	1		0.00	0.00	0.00		
Mid Gir	Max. Mx	14	140.03	-23.82	0.00		
	Max. My	16	172.96	0.00	0.00		
	Max. Vy	14	172.22	0.00	0.00		
	Max. Vx	16	-0.00	0.00	0.00		
	Max. Tension	17	253.28	0.00	0.00		
	Max. Compression	1	0.00	0.00	0.00		
Leg	Max. Mx	16	153.54	-21.92	0.00		
	Max. My	14	172.22	0.00	0.00		
	Max. Vy	16	-0.00	0.00	0.00		
	Max. Tension	25	34376.47	82.26	7.67		
	Max. Compression	15	-124226.21	-1.63	57.84		
	Max. Mx	11	-92332.92	-591.15	-160.98		
Diagonal	Max. My	15	-122215.08	12.95	644.38		
	Max. Vy	11	-943.04	-496.89	-158.40		
	Max. Vx	2	-957.83	73.30	538.53		
	Max. Tension	18	8683.18	0.00	0.00		
	Max. Compression	18	-7922.84	0.00	0.00		
	Max. Mx	16	146.65	-21.96	0.00		
Top Gir	Max. My	16	152.87	0.00	0.00		
	Max. Vy	16	-48.56	148.88	3.15		
	Max. Vx	18	-8.25	8.74	32.15		
	Max. Tension	23	613.78	0.00	0.00		
	Max. Compression	1	0.00	0.00	0.00		
	Max. Mx	14	448.27	96.54	0.00		
Mid Gir	Max. My	16	551.22	0.00	0.00		
	Max. Vy	14	-64.36	0.00	0.00		
	Max. Vx	16	0.00	0.00	0.00		
	Max. Tension	23	20998.22	0.00	0.00		
	Max. Compression	1	0.00	0.00	0.00		
	Max. Mx	14	1451.83	66.54	0.00		
Guy A	Max. My	14	1404.97	0.00	0.00		
	Max. Vy	14	-64.36	0.00	0.00		
	Max. Vx	16	0.00	0.00	0.00		
	Bottom Tension	21	55338.07	0.00	0.00		
	Top Tension	21	56512.49	0.00	0.00		
	Top Cable Vert	21	32571.98	0.00	0.00		
Bot Cable Vert	Top Cable Vert	21	46181.46	0.00	0.00		
	Top Cable Tan	31	2.23	0.00	0.00		
	Bot Cable Vert	21	-29871.30	0.00	0.00		

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Client		Crown Castle		Designed by Matt Lackey, E.I.	

Section No.	Elevation $\beta$	Component Type	Condition	Gov Load Comb	Force lb	Major Axis Moment $M_{x1}$	Minor Axis Moment $M_{y1}$
T12	241 - 221	Guy B	Bot Cable Tan	21	46583.34	0.00	0.00
			Bot Cable Tan	25	56873.10	157.26	-29.01
			Top Tension	25	56817.12	502.42	109.03
			Top Cable Vert	25	32751.09	16.06	-520.47
			Top Cable Norm	25	46487.91	902.42	109.04
			Top Cable Tan	25	2.51	-198.18	454.25
			Bot Cable Vert	25	-30117.14	0.00	0.00
			Bot Cable Norm	25	46823.63	0.00	0.00
			Bot Cable Tan	25	2.51	0.00	0.00
			Top Tension	17	5337.74	0.00	0.00
			Top Cable Vert	17	5614.35	0.00	0.00
			Top Cable Norm	17	3946.12	0.00	0.00
			Top Cable Tan	17	4637.49	0.00	0.00
			Bot Cable Vert	17	1.85	0.00	0.00
			Bot Cable Norm	17	-29532.76	0.00	0.00
Bot Cable Tan	17	46766.72	0.00	0.00			
Max Tension	1	1.85	0.00	0.00			
Max Compression	26	-96094.38	157.26	-29.01			
Max. Mx	24	-35036.31	502.42	109.03			
Max. My	21	-2432.77	16.06	-520.47			
Max. Vx	24	681.34	902.42	109.04			
Max. Vy	15	700.92	-198.18	454.25			
Max. Vz	18	6462.78	0.00	0.00			
Max. Tension	18	-24669.13	126.91	0.00			
Max. Mx	16	-5768.10	119.16	13.41			
Max. My	16	-44.70	129.91	13.41			
Max. Vz	18	-3.45	0.00	0.00			
Max. Tension	18	828.92	0.00	0.00			
Max. Mx	14	440.77	-21.98	0.00			
Max. My	16	527.65	0.00	0.00			
Max. Vz	14	14.65	0.00	0.00			
Max. Tension	25	421.87	0.00	0.00			
Max. Mx	14	277.19	-21.98	0.00			
Max. My	22	407.15	0.00	0.00			
Max. Vz	14	14.65	0.00	0.00			
Max. Tension	1	0.00	0.00	0.00			
Max. Mx	25	-91725.46	-306.39	-167.82			
Max. My	15	-85715.99	-379.69	-56.17			
Max. Vz	23	-87796.27	93.02	363.66			
Max. Tension	17	-251.24	-337.95	117.55			
Max. Mx	16	238.37	735.06	218.89			
Max. My	18	4307.39	0.00	0.00			
Max. Vz	18	-4403.35	0.00	0.00			
Max. Tension	18	210.96	21.54	1.39			
Max. Mx	18	-4403.35	0.00	0.00			
Max. My	23	-20.78	52.54	1.39			
Max. Vz	18	-1.50	7.22	5.81			
Max. Tension	23	420.98	0.00	0.00			
Max. Mx	14	259.79	-21.98	0.00			
Max. My	22	232.60	0.00	0.00			
Max. Vz	14	14.65	0.00	0.00			
Max. Tension	22	0.00	0.00	0.00			
Max. Mx	22	0.00	0.00	0.00			
Max. My	19	428.74	0.00	0.00			
Max. Vz	19	0.00	0.00	0.00			

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Client		Crown Castle		Designed by Matt Lackey, E.I.	

Section No.	Elevation $\beta$	Component Type	Condition	Gov Load Comb	Force lb	Major Axis Moment $M_{x1}$	Minor Axis Moment $M_{y1}$
T14	201 - 181	Leg	Max. Compression	14	247.09	0.00	0.00
			Max. Mx	22	199.60	-21.98	0.00
			Max. My	14	14.65	0.00	0.00
			Max. Vz	22	0.00	0.00	0.00
			Max. Tension	1	0.00	0.00	0.00
			Max. Mx	25	-104924.13	-324.60	-219.07
			Max. My	15	-92611.40	388.80	-93.95
			Max. Vz	22	-100993.08	55.32	394.77
			Max. Tension	15	-156.15	-386.23	28.78
			Max. Mx	21	-188.54	-19.17	-315.10
			Max. My	18	298.70	0.00	0.00
			Max. Vz	18	-239.22	0.00	0.00
			Max. Tension	23	-214.80	56.69	5.61
			Max. Mx	19	-15794.92	-0.88	5.61
			Max. My	23	-21.85	56.69	0.81
Max. Vz	19	-1.46	-0.85	5.67			
Max. Tension	23	490.25	0.00	0.00			
Max. Mx	1	0.00	0.00	0.00			
Max. My	14	252.09	-21.98	0.00			
Max. Vz	22	156.72	0.00	-0.00			
Max. Tension	14	14.65	0.00	0.00			
Max. Mx	22	0.00	0.00	0.00			
Max. My	1	-0.27	0.00	0.00			
Max. Vz	1	0.00	0.00	0.00			
Max. Tension	1	0.00	0.00	0.00			
Max. Mx	14	255.94	-21.98	0.00			
Max. My	22	298.84	0.00	0.00			
Max. Vz	14	14.65	0.00	0.00			
Max. Tension	22	-0.00	0.00	0.00			
Max. Mx	25	-105870.87	316.49	176.82			
Max. My	25	-102240.20	-110.49	-162.38			
Max. Vz	21	-101881.65	1.31	428.01			
Max. Tension	18	-220.50	-289.34	-75.98			
Max. Mx	1	189.44	143.90	0.00			
Max. My	1	189.44	0.00	0.00			
Max. Vz	1	-2151.17	57.09	0.00			
Max. Tension	12	189.44	0.00	0.00			
Max. Mx	19	-238.02	57.09	0.00			
Max. My	19	-2138.92	14.09	5.45			
Max. Vz	23	-21.95	57.09	0.93			
Max. Tension	15	1.41	47.40	-5.59			
Max. Mx	19	543.32	0.00	0.00			
Max. My	1	0.00	0.00	0.00			
Max. Vz	14	261.05	-21.98	0.00			
Max. Tension	22	307.08	0.00	0.00			
Max. Mx	14	14.65	0.00	0.00			
Max. My	22	0.00	0.00	0.00			
Max. Vz	21	61.49	0.00	0.00			
Max. Tension	1	0.00	0.00	0.00			
Max. Mx	14	267.31	-21.98	0.00			
Max. My	22	314.64	0.00	0.00			
Max. Vz	14	14.65	0.00	0.00			
Max. Tension	22	-0.00	0.00	0.00			
Max. Mx	25	-100765.69	537.22	46.01			
Max. My	25	-57185.40	-58.38	-28.65			
Max. Vz	21	-65355.27	-114.57	586.86			
Max. Tension	15	-487.27	537.22	46.01			
Max. Mx	21	-63.54	-114.57	-49.18			
Max. My	23	400.41	0.00	0.00			
Max. Vz	23	-4272.38	0.00	0.00			

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Project: TEP# 072725		Designed by: Matt Lackey, E.I.	
Client: Crown Castle			
<p><b>Tower Engineering Professionals</b>          3703 Junction Boulevard          Raleigh, NC 27603          Phone: (919) 661-6351          FAX: (919) 661-6350</p>			

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Project: TEP# 072725		Designed by: Matt Lackey, E.I.	
Client: Crown Castle			
<p><b>Tower Engineering Professionals</b>          3703 Junction Boulevard          Raleigh, NC 27603          Phone: (919) 661-6351          FAX: (919) 661-6350</p>			

Section No.	Elevation	Component Type	Condition	Guy Lead Comb.	Force lb.	Major Axis Moment lbf-ft	Minor Axis Moment lbf-ft	
T17	141 - 121	Top Girt	Max. Mx	23	-408.47	111.58	5.38	
			Max. My	21	-3918.23	101.95	19.33	
			Max. Vx	23	-40.00	111.58	19.33	0.00
			Max. Vy	21	-4.98	101.95	0.00	0.00
			Max. Tension	19	536.82	0.00	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00	0.00
			Max. Mx	14	292.02	-21.98	0.00	0.00
			Max. My	22	378.78	0.00	0.00	0.00
			Max. Vx	14	14.65	0.00	0.00	0.00
			Max. Vy	22	-0.00	0.00	0.00	0.00
			Max. Tension	23	536.02	0.00	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00	0.00
			Max. Mx	14	344.17	-21.98	0.00	0.00
			Max. My	22	391.42	0.00	0.00	0.00
			Max. Vx	14	14.65	0.00	0.00	0.00
			Max. Vy	22	-0.00	0.00	0.00	0.00
			Max. Tension	22	-0.00	0.00	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00	0.00
			Max. Mx	25	-105714.10	-200.36	-105.18	0.00
			Max. My	18	-74465.85	-947.06	-279.07	0.00
			Max. Vx	22	-72311.84	223.81	-925.60	0.00
			Max. Vy	5	-467.87	-863.86	227.13	0.00
Max. Tension	21	-472.12	76.19	-909.12	0.00			
Max. Compression	16	6924.40	0.00	0.00	0.00			
Max. Mx	18	-8422.30	15.51	6.00	0.00			
Max. My	15	-2434.38	59.60	-48.36	0.00			
Max. Vx	21	-46.09	133.35	0.00	0.00			
Max. Vy	25	1277.90	59.90	-48.36	0.00			
Max. Tension	10	-115.08	0.00	0.00	0.00			
Max. Compression	14	485.05	-21.98	0.00	0.00			
Max. Mx	22	594.85	0.00	0.00	0.00			
Max. My	14	14.65	0.00	0.00	0.00			
Max. Vx	22	-0.00	0.00	0.00	0.00			
Max. Vy	25	9707.24	0.00	0.00	0.00			
Max. Tension	15	-5319.75	0.00	0.00	0.00			
Max. Compression	14	1355.21	-21.98	0.00	0.00			
Max. Mx	22	1465.31	0.00	0.00	0.00			
Max. My	14	14.65	0.00	0.00	0.00			
Max. Vx	22	-0.00	0.00	0.00	0.00			
Max. Vy	21	22795.13	0.00	0.00	0.00			
Max. Tension	21	23987.77	0.00	0.00	0.00			
Max. Compression	21	8594.03	0.00	0.00	0.00			
Max. Mx	21	21423.67	0.00	0.00	0.00			
Max. My	21	1.26	0.00	0.00	0.00			
Max. Vx	21	-7616.23	0.00	0.00	0.00			
Max. Vy	21	21485.35	0.00	0.00	0.00			
Max. Tension	21	7.29	0.00	0.00	0.00			
Max. Compression	25	22827.37	0.00	0.00	0.00			
Max. Mx	25	22827.37	0.00	0.00	0.00			
Max. My	25	22827.37	0.00	0.00	0.00			
Max. Vx	25	21671.14	0.00	0.00	0.00			
Max. Vy	25	4.22	0.00	0.00	0.00			
Max. Tension	25	-7031.43	0.00	0.00	0.00			
Max. Compression	25	21717.45	0.00	0.00	0.00			
Max. Mx	25	3.79	0.00	0.00	0.00			
Max. My	17	22682.14	0.00	0.00	0.00			
Max. Vx	17	22367.85	0.00	0.00	0.00			
Max. Vy	17	7845.35	0.00	0.00	0.00			
Max. Tension	17	20946.87	0.00	0.00	0.00			
Max. Compression	17	4.96	0.00	0.00	0.00			

Section No.	Elevation	Component Type	Condition	Guy Lead Comb.	Force lb.	Major Axis Moment lbf-ft	Minor Axis Moment lbf-ft	
T18	121 - 101	Torque Arm Top	Max. Mx	17	-6841.12	20995.73	0.00	
			Max. My	17	2.99	0.00	0.00	
			Max. Vx	21	33131.44	0.00	0.00	0.00
			Max. Vy	15	18003.73	81.06	0.00	0.00
			Max. Tension	16	5392.19	0.00	0.00	0.00
			Max. Compression	15	-42.94	0.00	0.00	0.00
			Max. Mx	5	3408.83	0.00	0.00	0.00
			Max. My	23	-18314.84	97.41	0.00	0.00
			Max. Vx	16	-16612.37	0.00	-0.36	0.00
			Max. Vy	23	-63.03	0.00	0.00	0.00
			Max. Tension	16	0.16	0.00	0.00	0.00
			Max. Compression	25	-158676.72	-615.68	-408.63	0.00
			Max. Mx	16	-39447.89	-1086.37	-259.00	0.00
			Max. My	16	-91662.15	-596.75	-150.15	0.00
			Max. Vx	25	1031.15	530.50	618.06	0.00
			Max. Vy	16	1345.79	-586.75	303.29	0.00
			Max. Tension	18	6681.96	0.00	0.00	0.00
			Max. Compression	25	-4102.58	66.09	-11.32	0.00
			Max. Mx	16	-3543.73	-4.45	11.32	0.00
			Max. My	25	-24.24	66.09	-1.22	0.00
			Max. Vx	15	2348.32	0.00	0.00	0.00
			Max. Vy	25	-118.20	0.00	0.00	0.00
Max. Tension	14	909.76	-21.98	0.00	0.00			
Max. Compression	14	1036.32	0.00	0.00	0.00			
Max. Mx	14	14.65	0.00	0.00	0.00			
Max. My	14	985.03	0.00	0.00	0.00			
Max. Vx	14	-141.84	0.00	0.00	0.00			
Max. Vy	14	395.19	-31.98	0.00	0.00			
Max. Tension	16	240.02	0.00	0.00	0.00			
Max. Compression	16	-0.00	0.00	0.00	0.00			
Max. Mx	16	14.65	0.00	0.00	0.00			
Max. My	19	1949.22	-184976.91	335.60	0.00			
Max. Vx	25	-184976.91	-652.25	-384.86	0.00			
Max. Vy	26	-174063.46	-673.12	-293.13	0.00			
Max. Tension	21	-259.86	0.00	-1.85	0.00			
Max. Compression	21	423.97	-21.98	0.00	0.00			
Max. Mx	24	-4383.81	0.00	0.00	0.00			
Max. My	24	-63.25	73.40	-6.38	0.00			
Max. Vx	24	-4371.66	-50.03	-9.95	0.00			
Max. Vy	24	-26.11	73.40	-6.58	0.00			
Max. Tension	24	-2.56	0.00	0.00	0.00			
Max. Compression	23	720.27	0.00	0.00	0.00			
Max. Mx	14	369.95	0.00	0.00	0.00			
Max. My	16	412.16	0.00	0.00	0.00			
Max. Vx	14	14.65	0.00	0.00	0.00			
Max. Vy	16	622.94	0.00	0.00	0.00			
Max. Tension	25	-22.29	0.00	0.00	0.00			
Max. Compression	14	373.85	-21.98	0.00	0.00			
Max. Mx	16	-405.35	0.00	0.00	0.00			

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Job		Project	
Elevation $\beta$		TEP# 072725	
Component Type		Crown Castle	
Client		Designed by Matt Lackey, E.I.	
Date		11:08:39 01/10/08	

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Job		Project	
Elevation $\beta$		TEP# 072725	
Component Type		Crown Castle	
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Section No.	Elevation $\beta$	Component Type	Condition	Gov. Comp. Count	Force lb	Major Axis Moment	Minor Axis Moment	
T20	81 - 61	Leg	Max. Vy	14	14.65	0.00	0.00	
			Max. Vx	16	-0.00	0.00	0.00	
			Max. Tension	15	6795.95	3.13	256.00	0.00
			Max. Compression	25	-194035.79	670.99	1711.91	-65.11
			Max. Mx	15	-156310.14	774.39	-65.11	721.52
			Max. My	21	-175962.68	193.46	621.36	-492.65
			Max. Vy	23	691.20	301.06	0.00	0.00
			Max. Vx	20	497.71	0.00	0.00	0.00
			Max. Tension	24	2462.45	0.00	0.00	0.00
			Max. Compression	24	-2620.10	0.00	0.00	0.00
			Max. Mx	24	-919.90	77.26	-3.74	5.90
			Max. My	24	-567.98	66.13	-9.90	5.74
T21	61 - 41	Leg	Max. Vy	24	-27.09	0.00	0.00	
			Max. Vx	24	-2.55	0.00	0.00	
			Max. Tension	15	813.36	0.00	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00	0.00
			Max. Mx	14	374.93	-21.98	0.00	0.00
			Max. My	16	406.42	0.00	0.00	0.00
			Max. Vx	14	14.65	0.00	0.00	0.00
			Max. Vy	16	-0.00	0.00	0.00	0.00
			Max. Tension	15	817.36	0.00	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00	0.00
			Max. Mx	14	394.68	-21.98	0.00	0.00
			Max. My	22	448.35	0.00	0.00	0.00
Max. Vx	4	-0.65	0.00	0.00	0.00			
Max. Vy	22	448.35	0.00	0.00	0.00			
T22	41 - 20	Leg	Max. Vy	25	2541.19	35.98	310.01	
			Max. Vx	26	-191377.55	409.23	233.74	
			Max. Tension	15	173763.90	7.06	727.95	80.13
			Max. Compression	25	-86074.37	-690.23	-180.14	-517.94
			Max. Mx	21	-312.11	-598.56	80.13	0.00
			Max. My	16	-289.23	9.18	0.00	0.00
			Max. Vx	21	2511.84	0.00	0.00	0.00
			Max. Vy	24	-2896.10	0.00	0.00	0.00
			Max. Tension	15	1537.42	74.82	-3.15	-3.15
			Max. Compression	24	-1568.63	-17.06	4.20	4.20
			Max. Mx	24	-66.46	74.82	-3.15	-3.15
			Max. My	24	-66.46	74.82	0.00	0.00
T23	20 - 6.70833	Leg	Max. Vy	15	114.87	0.00	0.00	
			Max. Vx	15	962.11	0.00	0.00	
			Max. Tension	16	386.03	-21.98	0.00	0.00
			Max. Compression	14	464.99	0.00	0.00	0.00
			Max. Mx	14	14.65	0.00	0.00	0.00
			Max. My	14	14.65	0.00	0.00	0.00
			Max. Vx	15	868.31	0.00	0.00	0.00
			Max. Vy	22	-0.00	0.00	0.00	0.00
			Max. Tension	15	868.31	0.00	0.00	0.00
			Max. Compression	14	396.54	-21.98	0.00	0.00
			Max. Mx	14	431.04	0.00	0.00	0.00
			T24	6.70833 - 0	Leg	Max. Vy	16	431.04
Max. Vx	14	14.65				0.00	0.00	
Max. Tension	16	431.04				0.00	0.00	0.00
Max. Compression	14	396.54				-21.98	0.00	0.00
Max. Mx	14	431.04				0.00	0.00	0.00
Max. My	14	14.65				0.00	0.00	0.00
Max. Vx	16	431.04				0.00	0.00	0.00
Max. Vy	14	14.65				0.00	0.00	0.00
Max. Tension	16	431.04				0.00	0.00	0.00
Max. Compression	14	396.54				-21.98	0.00	0.00
Max. Mx	14	431.04				0.00	0.00	0.00

### Maximum Reactions

Location	Condition	Gov. Comp. Count	Vertical lb	Horizontal, X lb	Horizontal, Y lb
Main	Max. Vert	23	307381.78	7371.56	-3312.53
	Max. Hx	23	8346.75	1021.37	822.64
	Max. Hy	15	301346.44	-23.51	-4.88
	Max. Mx	1	0.00	-23.51	-4.88
	Max. My	1	0.00	3653.18	-6625.20
	Max. Tension	22	1246.88	187821.01	-21.51
	Max. Compression	1	0.00	-4.88	-115.80
	Max. Mx	1	0.00	362.97	0.00
	Max. My	1	0.00	0.00	0.00
	Max. Vy	1	0.00	0.00	0.00
	Max. Vx	1	0.00	0.00	0.00

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Client		Crown Castle		11:08:39 01/10/08
Designed by		Matt Lackey, E.I.		

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Client		Crown Castle		11:08:39 01/10/08
Designed by		Matt Lackey, E. I.		

Location	Condition	Gov. Load Comb.	Vertical lb	Horizontal, X lb	Horizontal, Z lb
Guy C @ 424.5 ft Elev -16.5 ft Azimuth 240 deg	Min. H <sub>x</sub>	21	295320.97	205.29	-7847.22
	Min. M <sub>x</sub>	1	0.00	-23.51	-4.88
	Min. Tension	16	-1328.44	6933.03	6933.03
	Max. Vert	10	-969.35	-3445.46	1989.98
Guy B @ 407.5 ft Elev -9 ft Azimuth 120 deg	Min. H <sub>x</sub>	10	969.35	3445.46	1989.98
	Min. H <sub>y</sub>	17	-1347.81	-3786.32	20643.37
	Min. Vert	17	-1347.81	-3786.32	20643.37
	Min. H <sub>z</sub>	10	-969.35	3445.46	1989.98
	Min. H <sub>y</sub>	10	-969.35	3445.46	1989.98
	Max. Vert	6	-875.53	3205.16	1850.44
Guy A @ 400 ft Elev -6.0 deg	Max. H <sub>x</sub>	23	-14045.44	37566.76	21685.44
	Max. H <sub>y</sub>	25	-14045.44	37566.76	21685.44
	Min. Vert	25	-14045.44	37566.76	21685.44
	Min. H <sub>x</sub>	6	-875.53	3205.16	1850.44
	Min. H <sub>y</sub>	6	-875.53	3205.16	1850.44
	Max. Vert	2	-977.37	0.77	-3626.63
Guy C @ 411 ft Elev -20.5 ft Azimuth 240 deg	Max. H <sub>x</sub>	24	-7824.71	1320.36	-32849.13
	Max. H <sub>y</sub>	2	-977.37	0.77	-3626.63
	Min. Vert	21	-14783.67	-18.31	-41771.51
	Min. H <sub>x</sub>	18	-8012.55	-1319.33	-23347.51
	Min. H <sub>y</sub>	21	-14783.67	-18.31	-41771.51
	Max. Vert	10	-2679.44	-3926.26	2266.74
Guy B @ 384 ft Elev -13 ft Azimuth 120 deg	Max. H <sub>x</sub>	10	-2679.44	-3926.26	2266.74
	Max. H <sub>y</sub>	17	-7677.28	-85204.30	49204.13
	Min. Vert	17	-7677.28	-85204.30	49204.13
	Min. H <sub>x</sub>	10	-2679.44	-3926.26	2266.74
	Min. H <sub>y</sub>	10	-2679.44	-3926.26	2266.74
	Max. Vert	6	-2396.36	3498.46	2019.65
Guy A @ 405 ft Elev -30 ft Azimuth 0 deg	Max. H <sub>x</sub>	25	-78879.59	84762.29	48934.42
	Max. H <sub>y</sub>	25	-78879.59	84762.29	48934.42
	Min. Vert	25	-78879.59	84762.29	48934.42
	Min. H <sub>x</sub>	6	-2396.36	3498.46	2019.65
	Min. H <sub>y</sub>	6	-2396.36	3498.46	2019.65
	Max. Vert	2	-2586.90	0.08	-4337.22

**Tower Mast Reaction Summary**

Load Combination	Vertical lb	Shear, lb	Shears, lb	Overtwisting Moment, ft-lb	Overturning Moment, ft-lb	Twisting Moment, ft-lb
Dead Only	18721.01	25.51	4.86	0.01	0.02	0.06
Dead+Wind 0 deg - No Ice+Guy	22868.80	232.71	-6222.36	0.01	0.02	421.51
Dead+Wind 30 deg - No Ice+Guy	221339.65	3281.25	-5092.75	0.01	0.02	668.12
Dead+Wind 60 deg - No Ice+Guy	215960.02	5486.21	-3014.39	0.01	0.02	567.39
Dead+Wind 90 deg - No Ice+Guy	221011.12	6361.45	-78.96	0.01	0.02	348.09
Dead+Wind 120 deg - No Ice+Guy	228244.38	5785.34	3005.61	0.01	0.02	175.61
Dead+Wind 150 deg - No Ice+Guy	221735.75	3171.60	5253.79	0.01	0.02	-77.18
Dead+Wind 180 deg - No Ice+Guy	217769.07	60.95	6023.58	0.01	0.02	-438.61
Dead+Wind 210 deg - No Ice+Guy	223772.19	-3836.97	5174.73	0.01	0.02	-593.19
Dead+Wind 240 deg - No Ice+Guy	231343.42	-5293.76	3026.25	0.01	0.02	-542.06
Dead+Wind 270 deg - No Ice+Guy	223638.07	-6163.09	-57.95	0.01	0.02	-430.01
Dead+Wind 300 deg - No Ice+Guy	218112.53	-5933.76	-3086.13	0.01	0.02	-380.11
Dead+Wind 330 deg - No Ice+Guy	221997.59	-3062.27	-5943.76	0.01	0.02	127.70
Dead+Wind 360 deg - No Ice+Guy	263169.40	31.22	5.76	0.01	0.02	0.06
deg+Ice+Temp+Guy	302156.44	-702.29	-825.164	0.01	0.03	492.74
deg+Ice+Temp+Guy	296703.77	31887.97	-6933.03	0.01	0.02	1238.44
deg+Ice+Temp+Guy	293461.16	7120.03	-3829.20	0.01	0.02	852.26
deg+Ice+Temp+Guy	293080.85	8115.80	-362.97	0.01	0.02	560.87
deg+Ice+Temp+Guy	300203.56	7265.31	3705.92	0.01	0.03	314.10
deg+Ice+Temp+Guy	296277.94	3953.22	6794.71	0.01	0.02	-70.88
deg+Ice+Temp+Guy	295320.97	-205.29	7847.22	0.01	0.02	-1160.51
deg+Ice+Temp+Guy	298473.57	-3653.18	6625.20	0.01	0.02	-1246.87
deg+Ice+Temp+Guy	303781.78	-7371.56	3212.53	0.01	0.03	-926.14
deg+Ice+Temp+Guy	298292.26	-8346.73	-1021.37	0.01	0.02	-396.07
deg+Ice+Temp+Guy	295879.47	-7446.62	-4379.58	0.01	0.02	-497.29
deg+Ice+Temp+Guy	298222.99	-4650.80	-6896.00	0.01	0.02	-373.47
deg+Ice+Temp+Guy	191925.61	97.24	-2248.87	0.01	0.02	148.54
Service+Guy	1180.94	1180.94	-1841.00	0.01	0.02	194.43
Service+Guy	189731.45	1961.07	-1072.15	0.01	0.02	180.36
Service+Guy	190373.67	2297.37	-15.92	0.01	0.02	118.21
Service+Guy	191569.87	2104.20	1090.35	0.01	0.02	54.65
Service+Guy	190775.13	1166.18	1883.30	0.01	0.02	-28.02

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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

Load Combination	Partial	Shear, lb	Shear, lb	Overtuming Moment, M <sub>t</sub> lb-ft	Overtuming Moment, M <sub>t</sub> lb-ft	Torque lb-ft
Service-Wind	190443.65	31.47	2142.61	0.01	0.02	-133.72
Dead-Wind 180 deg	191435.27	-1034.85	1865.70	0.01	0.02	-189.73
Service-Wind	192796.02	-1922.46	1113.72	0.01	0.02	-186.45
Dead-Wind 240 deg	191575.68	-2209.48	-2.64	0.01	0.02	-147.04
Service-Wind	190693.96	-1890.61	-1093.51	0.01	0.02	-92.56
Dead-Wind 300 deg	191086.77	-1071.95	-1886.28	0.01	0.02	46.30
Service-Wind						

### Solution Summary

Load Comb.	PX Sum of Applied Forces lb	PY Sum of Applied Forces lb	PZ Sum of Applied Forces lb	PX Sum of Reactions lb	PY Sum of Reactions lb	PZ Sum of Reactions lb	% Error
1	-0.00	-57484.20	0.00	20.32	97484.21	0.00	0.021%
2	273.87	-98279.59	-98017.42	-272.23	98277.95	97933.71	0.062%
3	47832.61	-97482.04	-82407.73	-4784.74	97480.90	82332.99	0.069%
4	82353.04	-96679.59	-48234.84	-8236.84	96680.51	47314.46	0.078%
5	95916.55	-97116.24	-531.84	-95935.79	97313.18	-4882.12	0.050%
6	47832.61	-97482.04	-82407.73	-4784.74	97480.90	82332.99	0.062%
7	47832.61	-97482.04	-82407.73	-4784.74	97480.90	82332.99	0.062%
8	47832.61	-97482.04	-82407.73	-4784.74	97480.90	82332.99	0.062%
9	-47570.99	-97486.35	82802.18	47498.14	97485.17	-82774.39	0.057%
10	-84904.06	-98238.81	49096.35	84828.06	98287.14	-49051.59	0.064%
11	-95855.56	-97452.15	-45.50	95797.91	97451.06	94.46	0.055%
12	-82674.71	-96627.52	-47606.27	82313.02	96628.51	47651.41	0.049%
13	-47590.43	-97449.99	-82524.82	47603.95	97448.92	82551.86	0.054%
14	0.00	-150113.40	0.00	-18.21	150113.38	7.03	0.013%
15	-1281.21	-151269.89	-115978.24	1281.22	151268.24	113887.00	0.048%
16	55002.92	-150108.09	-979123.26	-55016.37	150106.87	97831.99	0.043%
17	96014.26	-148938.74	-58840.88	-96008.18	148938.05	57986.04	0.039%
18	11923.65	-151192.15	-2123.15	-118603.51	150138.06	2133.86	0.041%
19	9928.84	-151192.15	-2123.15	-9928.84	151192.15	2123.15	0.041%
20	1083.09	-148954.81	96003.32	1083.09	148954.81	-96003.32	0.042%
21	-55592.54	-150118.71	95873.98	55516.64	150117.48	-95850.26	0.037%
22	-100740.33	-151288.05	54875.53	100661.05	151286.41	-54829.95	0.044%
23	-112816.55	-150067.63	-2850.81	112755.05	150066.45	2902.97	0.043%
24	-97490.77	-148864.39	-56124.81	97419.29	148863.42	56130.03	0.038%
25	-56406.72	-150062.32	-98186.19	56419.56	150061.06	98106.83	0.043%
26	94.42	-97759.42	-34005.60	-94.07	97759.29	33984.94	0.029%
27	16595.85	-97483.45	-28572.33	-16599.60	97483.36	28573.16	0.019%
28	28563.72	-97205.78	-16477.97	-28570.72	97205.81	16476.23	0.007%
29	33278.66	-97495.29	-181.63	-33284.20	97495.22	181.63	0.016%
30	33959.49	-97708.62	16988.06	-33968.06	97708.57	-16988.06	0.019%
31	16603.52	-97208.97	-33283.12	-16607.28	97208.93	33283.12	0.019%
32	16603.52	-97208.97	-33283.12	-16607.28	97208.93	33283.12	0.019%
33	16603.52	-97208.97	-33283.12	-16607.28	97208.93	33283.12	0.019%
34	16603.52	-97208.97	-33283.12	-16607.28	97208.93	33283.12	0.019%
35	33257.55	-97473.11	-153.74	-33242.82	97473.02	153.74	0.019%
36	-28542.84	-97187.77	-16517.53	28541.40	97187.79	16517.21	0.010%
37	-16517.53	-97472.36	-28667.44	16516.41	97472.28	28668.16	0.019%
38	-16517.53	-97472.36	-28667.44	16516.41	97472.28	28668.16	0.019%

### Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Iteration Tolerance
1	Yes	16	0.00000001	0.00000001
2	Yes	135	0.00019458	0.00013306
3	Yes	122	0.00019649	0.00012233
4	Yes	45	0.00019427	0.00016890
5	Yes	122	0.00019778	0.00012244
6	Yes	135	0.00019778	0.00013197
7	Yes	123	0.00019556	0.00012049
8	Yes	45	0.00019630	0.00016568
9	Yes	122	0.00019988	0.00013775
10	Yes	135	0.00019517	0.00013481
11	Yes	122	0.00019683	0.00012248
12	Yes	45	0.00019652	0.00016945
13	Yes	122	0.00019652	0.00012248
14	Yes	10	0.00000001	0.00000001
15	Yes	133	0.00019695	0.00011157
16	Yes	122	0.00019583	0.00010803
17	Yes	47	0.00019482	0.00013240
18	Yes	121	0.00019741	0.00010464
19	Yes	133	0.00019810	0.00010847
20	Yes	122	0.00019573	0.00010381
21	Yes	40	0.00019088	0.00015115
22	Yes	122	0.00019647	0.00010381
23	Yes	132	0.00019627	0.00011133
24	Yes	121	0.00019764	0.00010538
25	Yes	40	0.00019370	0.00014484
26	Yes	124	0.00019463	0.00010564
27	Yes	71	0.00019613	0.00006695
28	Yes	61	0.00019452	0.00006437
29	Yes	19	0.00019142	0.00009602
30	Yes	38	0.00019578	0.00006401
31	Yes	69	0.00019592	0.00006508
32	Yes	58	0.00019642	0.00006410
33	Yes	58	0.00019799	0.00009504
34	Yes	20	0.00019216	0.00006417
35	Yes	62	0.00019216	0.00006417
36	Yes	71	0.00019487	0.00006410
37	Yes	20	0.00019247	0.00006410
38	Yes	60	0.00019695	0.00006497

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Deflection in	Govt Limit in	Tilt in/ft	Tilt in/ft
L1	490-457	9.233	35	0.3068	0.1285
L2	457-436	7.246	35	0.1863	0.1394
L3	431-401	5.984	35	0.1719	0.1370
L4	401-381	5.307	35	0.1392	0.1372
T5	381-361	4.760	35	0.1044	0.1374

**RISATOWER**

**Tower Engineering Professionals**  
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 Raleigh, NC 27603  
 Phone: (919) 661-6351  
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**Job** BU# 873128 "Trumbull"

**Project** TEP# 072725

**Client** Crown Castle

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Section No.	Elevation	Hrs.	Deflection	Tilt	Twist	Radius of Curvature
T6	361-341	4.459	0.0720	0.1234	0.1205	22676
T7	341-321	4.233	0.0533	0.1005	0.1005	22676
T8	321-301	4.049	0.0468	0.1005	0.1005	22676
T9	301-281	3.855	0.0474	0.1046	0.1046	22676
T10	281-261	3.651	0.0435	0.0988	0.0988	22676
T11	261-241	3.447	0.0464	0.0988	0.0988	22676
T12	241-221	3.243	0.0409	0.0842	0.0842	22676
T13	221-201	3.039	0.0378	0.0842	0.0842	22676
T14	201-181	2.835	0.0378	0.0941	0.0941	22676
T15	181-161	2.631	0.0245	0.0670	0.0670	22676
T16	161-141	2.427	0.0410	0.0632	0.0632	22676
T17	141-121	2.223	0.0497	0.0587	0.0587	22676
T18	121-101	2.019	0.0503	0.0567	0.0567	22676
T19	101-81	1.815	0.0646	0.0641	0.0641	22676
T20	81-61	1.611	0.0680	0.0633	0.0633	22676
T21	61-41	1.407	0.1152	0.0600	0.0600	22676
T22	41-20	1.203	0.1406	0.0589	0.0589	22676
T23	20-6.70833	0.722	0.1598	0.0573	0.0573	22676
T24	6.70833-0	0.257	0.1724	0.0544	0.0544	22676

**Critical Deflections and Radius of Curvature - Service Wind**

Elevation	Appearance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
443'	071092 El. 256-ft-B, 346-ft, 445-ft-C	35	6.869	0.1815	0.1294	35687
443'	071092 El. 256-ft-B, 346-ft, 445-ft-C	35	6.796	0.1819	0.1294	71732
441'6"	071092 El. 256-ft-B, 346-ft, 445-ft-C	35	6.742	0.1822	0.1293	181863
440'	071092 El. 256-ft-B, 346-ft, 445-ft-C	35	6.689	0.1824	0.1293	321104
439'	071092 El. 269-ft, 285-ft, 326-ft, 343-ft, 439-ft, 445-ft-A	35	6.653	0.1826	0.1292	120210
430'	1083-3CP	35	6.321	0.1804	0.1286	181820
425'	1083-3CP	35	6.132	0.1764	0.1282	81050
420'	1083-3CP	35	5.948	0.1706	0.1278	41644
415'	1083-3CP	35	5.770	0.1634	0.1275	42525
410'	1083-3CP	35	5.599	0.1552	0.1274	46792
384'	2-Bay Shively FM w/ Radomes	35	5.276	0.1374	0.1271	47095
384'	2-Bay Shively FM w/ Radomes	35	4.924	0.1288	0.1270	28689
384'	2-Bay Shively FM w/ Radomes	35	4.924	0.1291	0.1270	28689
381'	15' x 8' Omni	35	4.760	0.1044	0.1274	15930
372'	071092 El. 372-ft Mount	35	4.596	0.0894	0.1284	43429
365'	15' x 8' Omni	35	4.498	0.0785	0.1287	85593
347'6"	071092 347.5-ft Mount	35	4.368	0.0646	0.1265	87628
346'	071092 El. 256-ft-B, 346-ft, 445-ft-C	35	4.298	0.0582	0.1237	87628
343'	071092 El. 256-ft-B, 346-ft, 445-ft-C	35	4.282	0.0569	0.1230	88111
343'	071092 El. 269-ft, 285-ft, 326-ft, 343-ft, 439-ft, 445-ft-A	35	4.252	0.0546	0.1215	90071
340'	10-FT Grid	35	4.223	0.0527	0.1200	86574
331'	10-FT Grid	35	4.154	0.0510	0.1185	75744
330'	071092 330-ft Mount	35	4.113	0.0484	0.1149	30817
328'	10-Element 10.0 x 6.0 Yagi	35	4.113	0.0479	0.1139	52307
326'	071092 El. 269-ft, 285-ft, 326-ft	35	4.095	0.0475	0.1129	45393

**Maximum Tower Deflections - Design Wind**

Elevation	Appearance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
322'	343-ft, 439-ft, 445-ft-A	35	4.085	0.0460	0.1110	22676
316'	2-Bay Shively FM w/ Radomes	35	4.002	0.0467	0.1081	22676
312'	2-Bay Shively FM w/ Radomes	35	3.964	0.0468	0.1076	22676
308'	2-Bay Shively FM w/ Radomes	35	3.925	0.0471	0.1063	22676
305'	TLP-8M x 11.7-ft	35	3.895	0.0473	0.1057	22676
299'	TLP-8M x 11.7-ft	35	3.835	0.0473	0.1041	22676
293'	TLP-8M x 11.7-ft	35	3.774	0.0469	0.1036	22676
285'	343-ft, 439-ft, 445-ft-A	35	3.692	0.0451	0.1004	96284
278'	15' x 8' Omni	35	3.600	0.0419	0.0974	154375
274'	071092 274-ft Mount	35	3.551	0.0370	0.0953	52020
269'	071092 El. 269-ft, 326-ft, 326-ft	35	3.533	0.0352	0.0934	42453
267'	071092 El. 262-ft, 331-ft Mount	35	3.517	0.0333	0.0913	37109
260'	071092 El. 260-ft Mount	35	3.469	0.0331	0.0886	27353
256'	071092 El. 256-ft-B, 346-ft, 445-ft-C	35	3.451	0.0195	0.0867	30556
251'	Guy	35	3.433	0.0124	0.0855	36908
247'	071092 El. 247-ft (Single Sector)	35	3.433	0.0096	0.0849	44771
235'	071092 El. 232-ft, 238-ft Mount	35	3.441	0.0101	0.0825	105200
214'	071092 El. 214-ft Mount	35	3.482	0.0077	0.0792	100427
204'6"	071092 El. 184.5-ft Mount	35	3.488	0.0063	0.0757	66653
199'6"	071092 El. 184.5-ft Mount	35	3.483	0.0086	0.0757	58232
197'6"	071092 El. 184.5-ft Mount	35	3.478	0.0120	0.0712	55402
189'6"	26' Covered Yagi	35	3.465	0.0164	0.0695	53159
188'	26' Covered Yagi	35	3.465	0.0164	0.0695	53159
184'6"	14' x 12' Corner Reflector	35	3.453	0.0187	0.0684	51485
179'6"	SP4.5-8	35	3.422	0.0258	0.0679	51116
154'	6-FT Grid	35	3.220	0.0453	0.0667	51089
153'	071092 El. 133-155-ft Mount	35	3.210	0.0458	0.0616	120687
148'	071092 El. 133-155-ft Mount	35	3.158	0.0480	0.0604	131103
146'	Andrew 6' w/ Radome	35	3.137	0.0487	0.0599	356192
143'	071092 El. 133-155-ft Mount	35	3.105	0.0494	0.0592	290501
137'6"	071092 El. 133-155-ft Mount	35	3.082	0.0499	0.0579	290501
133'	071092 El. 133-155-ft Mount	35	3.080	0.0494	0.0569	290501
131'	Guy	35	2.978	0.0456	0.0545	223184
117'6"	P-9A480N-U	35	2.840	0.0516	0.0526	569535
109'6"	10-FT Grid	35	2.759	0.0568	0.0510	76003
107'	6-ft x 3-ft Grid	31	2.732	0.0589	0.0621	59769
76'	SPD2-5.8	31	2.655	0.0657	0.0644	40418
68'	6-ft x 3-ft Grid	31	2.280	0.0946	0.0625	38142
67'	6-ft x 3-ft Grid	31	2.115	0.1065	0.0612	37440
63'	6-ft x 3-ft Grid	31	2.093	0.1089	0.0610	37354
62'	6-ft x 3-ft Grid	31	2.001	0.1134	0.0604	37055
49'	3'-6" x 10' Element Yagi	31	1.977	0.1138	0.0602	37025
48'	10-Element Yagi	31	1.776	0.1346	0.0594	37827
44'	1.3M	31	1.696	0.1312	0.0592	38556
40'	18-in Stairlift	31	1.376	0.1317	0.0589	43276



<b>RISATower</b>		Job	Page
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Designed by		Matt Lackey, E.I.	

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Tower Engineering Professionals 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6331 FAX: (919) 661-6350		BU# 873128 "Trumbull"	48 of 65
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Elevation	Apparatus	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
$\beta$			in	$^{\circ}$	$^{\circ}$	$\beta$
445	071092 El. 256-n, 346-n, 445-n	15	25.923	0.6068	0.3873	19426
443	071092 El. 256-n, 346-n, 445-n	15	25.692	0.6072	0.3871	35343
441.6	071092 El. 256-n, 346-n, 445-n	15	25.519	0.6075	0.3867	91679
440	071092 El. 256-n, 346-n, 445-n	15	25.346	0.6078	0.3862	152916
439	071092 El. 269-n, 285-n, 326-n	15	25.231	0.6078	0.3858	57936
430	343-n, 439-n, 445-n	15	24.180	0.6016	0.3786	141228
425	1083-3CP	15	23.592	0.5918	0.3729	29138
420	1083-3CP	15	23.015	0.5774	0.3672	16890
417	1083-3CP	15	22.454	0.5585	0.3621	17007
410	1083-3CP	15	21.911	0.5363	0.3575	18232
400	2-Buy Shively FM w/ Radomes	15	20.971	0.4870	0.3477	68211
394	2-Buy Shively FM w/ Radomes	15	20.570	0.4702	0.3429	9793
388	2-Buy Shively FM w/ Radomes	15	19.733	0.4272	0.3302	5290
381	Guy	15	19.177	0.3979	0.3290	7897
372	071092 El. 372-n Mount	15	18.617	0.3501	0.3194	14753
365	19 x 8" Omni	15	18.270	0.3194	0.3256	31066
354	071092 347.5-n Mount	15	17.792	0.2797	0.3166	32662
347.6	071092 El. 256-n, 346-n, 445-n	15	17.528	0.2622	0.3182	30664
343	071092 El. 269-n, 285-n, 326-n	15	17.468	0.2589	0.3176	32279
343	071092 El. 269-n, 285-n, 326-n	15	17.351	0.2531	0.3166	33106

### Critical Deflections and Radius of Curvature - Design Wind

Elevation	Apparatus	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
$\beta$			in	$^{\circ}$	$^{\circ}$	$\beta$
340	343-n, 439-n, 445-n	15	17.236	0.2485	0.3162	36624
331	071092 El. 267-n, 331-n Mounts	15	16.896	0.2405	0.3175	10944
328	10-Element 10-n x 6-n Yagi	15	16.782	0.2400	0.3178	87548
326	071092 El. 269-n, 285-n, 326-n	15	16.706	0.2397	0.3178	66533
321	SPD2-5.8	15	16.551	0.2399	0.3171	74111
316	2-Buy Shively FM w/ Radomes	15	16.312	0.2412	0.3131	54446
312	2-Buy Shively FM w/ Radomes	15	16.147	0.2423	0.3089	60819
308	2-Buy Shively FM w/ Radomes	15	15.980	0.2432	0.3039	76562
305	TLP-8M x 11.7-n	15	15.832	0.2436	0.3000	94987
299	TLP-8M x 11.7-n	15	15.692	0.2439	0.2974	102709
293	TLP-8M x 11.7-n	15	15.569	0.2438	0.2949	106584
285	071092 El. 269-n, 285-n, 326-n	15	15.334	0.2398	0.2882	97534
278	19 x 8" Omni	15	14.680	0.2163	0.2866	31889
271	071092 271-n Mount	15	14.390	0.1963	0.2826	13255
269	071092 El. 269-n, 285-n, 326-n	15	14.313	0.1893	0.2812	11347
267	071092 El. 267-n, 331-n Mounts	23	14.246	0.1822	0.2797	9909
260	071092 El. 267-n, 331-n Mounts	23	14.055	0.1533	0.2749	7450
256	071092 El. 256-n, 346-n, 445-n	23	13.979	0.1348	0.2727	8136
251	Guy	23	13.917	0.1116	0.2705	9466
247	071092 El. 247-n (Single Sector)	23	13.888	0.0942	0.2691	10883
235	071092 El. 232-n, 238-n Mount	23	13.887	0.0642	0.2669	20293
214	071092 El. 214-n Mount	23	13.997	0.0598	0.2581	12235
204.6	071092 El. 184.5-n Mount	23	14.014	0.0646	0.2479	19929
199.6	071092 El. 184.5-n Mount	23	14.003	0.0756	0.2411	17420
194.6	071092 El. 184.5-n Mount	23	13.974	0.0850	0.2335	16349
188	14' x 17' Covered Yagi	23	13.928	0.1013	0.2254	15867
186	14' x 17' Covered Yagi	23	13.910	0.1055	0.2230	15674
179.6	071092 El. 184.5-n Mount	23	13.885	0.1111	0.2197	15423
154	6-FT Grid	23	13.776	0.1154	0.2172	15246
153	6-FT Grid	23	13.066	0.1094	0.2171	15154
148	071092 El. 133-155-n Mounts	23	13.000	0.1051	0.2174	30011
146	Andrew 6' w/ Radome	23	12.845	0.2026	0.2114	50444
143	071092 El. 133-155-n Mounts	23	12.769	0.2049	0.2028	68793
138	20-n x 2.5-n Omni Antenna	23	12.653	0.2077	0.2052	120039
137.6	20-n x 2.5-n Omni Antenna	23	12.460	0.2102	0.2035	80704
131	6-FT Grid	23	12.285	0.2109	0.2094	77437
117.6	6-FT Grid	23	12.266	0.2109	0.2091	77170
109.6	10-FT Grid	23	11.819	0.2111	0.2080	76118
107	6-n x 3-n Grid	23	11.672	0.2203	0.2099	66165
100	SPD2-5.8	23	11.357	0.2270	0.2188	17547
76	6-n x 3-n Grid	23	10.924	0.2436	0.2386	12663
68	8-FT Grid	23	9.972	0.3139	0.2982	9941
67	6-n x 3-n Grid	23	8.692	0.4277	0.3833	8868
63	6-n x 3-n Grid	23	8.601	0.4377	0.3831	8844
54	6-n x 3-n Grid	23	8.225	0.4592	0.3810	8745
49	3'-6" x 10' Element Yagi	23	8.127	0.4600	0.3823	8739
48	1.2M	23	7.299	0.5240	0.3866	9011
40	18-n Shallow	23	6.148	0.5357	0.3871	9245
40	18-n Shallow	23	6.550	0.5786	0.3741	10288

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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

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		Client	Crown Castle	Designed by	Matt Lackey, E.I.

### Bolt Design Data

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of Bolts	Maximum Load per Bolt	Allowable Load	Ratio Load/Allowable	Allowable Bolts	Criteria
T1	457	Leg	A325N	0.8750	8	1028.90	26458.10	0.039	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	972.05	4123.34	0.236	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	137.82	4123.34	0.033	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	35.25	4123.34	0.009	1,333	Bolt Shear
		Leg	A325N	0.8750	8	2812.50	26458.10	0.106	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	1551.60	4123.34	0.376	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	35.75	4123.34	0.009	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	17.32	4123.34	0.004	1,333	Bolt Shear
		Leg	A325N	0.8750	8	7171.64	26458.10	0.271	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	2707.84	4123.34	0.657	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	36.57	4123.34	0.009	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	68.26	4123.34	0.017	1,333	Bolt Shear
		Leg	A325N	0.8750	8	1913.60	26457.90	0.226	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	4335.98	4123.34	1.052	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	193.98	4123.34	0.047	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	264.98	4123.34	0.064	1,333	Bolt Shear
		Leg	A325N	0.8750	8	3228.17	26458.00	0.122	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	3625.77	4123.34	0.879	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	10703.90	8246.68	1.298	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	323.11	4123.34	0.078	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26457.90	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	2658.62	4123.34	0.645	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	224.36	4123.34	0.054	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	148.54	4123.34	0.036	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.10	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	1993.32	4123.34	0.483	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	216.33	4123.34	0.052	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	91.55	4123.34	0.023	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.10	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	1149.66	4123.34	0.279	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	130.63	4123.34	0.032	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	250.44	4123.34	0.061	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.00	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	1892.62	4123.34	0.459	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	91.77	4123.34	0.022	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	116.61	4123.34	0.028	1,333	Bolt Shear
		Leg	A325N	0.8750	8	1810.65	26458.10	0.068	1,333	Bolt Tension

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of Bolts	Maximum Load per Bolt	Allowable Load	Ratio Load/Allowable	Allowable Bolts	Criteria
T11	261	Diagonal	A325N	0.5000	2	2650.09	4123.34	0.643	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	126.12	4123.34	0.031	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	126.64	4123.34	0.031	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.10	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	4341.59	4123.34	1.053	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	306.89	8346.68	0.037	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	10499.10	8246.68	1.275	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.10	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	3431.89	4123.34	0.832	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	414.46	4123.34	0.101	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	210.94	4123.34	0.051	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.00	0.000	1,333	Bolt Tension
		Top Girt	A325N	0.5000	2	2231.92	4123.34	0.541	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	210.49	4123.34	0.051	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.10	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	1196.61	4123.34	0.290	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	245.13	4123.34	0.059	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	258.63	4123.34	0.063	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.10	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	1075.58	4123.34	0.261	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	271.76	4123.34	0.066	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	259.75	4123.34	0.063	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.00	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	2106.14	4123.34	0.511	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	283.41	4123.34	0.065	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	268.01	4123.34	0.065	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.10	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	4211.15	4123.34	1.021	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	638.95	4123.34	0.155	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	4833.62	4123.34	1.177	1,333	Bolt Shear
		Torque Arm Top@131	A325N	0.7500	8	2891.43	18555.00	0.156	1,333	Bolt Shear
		Torque Arm Bottom@131	A325N	0.7500	8	2584.02	18555.00	0.138	1,333	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.00	0.000	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	3340.98	4123.34	0.810	1,333	Bolt Shear
		Top Girt	A325N	0.5000	2	1174.16	4123.34	0.285	1,333	Bolt Shear
		Mid Girt	A325N	0.5000	2	492.96	4123.34	0.120	1,333	Bolt Shear
		Leg	A325N	0.8750	8	243.65	26458.10	0.009	1,333	Bolt Tension
		Diagonal	A325N	0.5000	2	2191.90	4123.34	0.532	1,333	Bolt Shear

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Client		Crown Castle		Designed by Matt Lackey, E.I.	

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of Bolts	Maximum Load per Bolt	Allowable Load	Ratio Allowable Load	Criteria
T20	81	Top Gir	A325N	0.5000	2	360.14	4123.34	0.087	Bolt Shear
		Mid Gir	A325N	0.5000	2	411.17	4123.34	0.100	Bolt Shear
		Leg	A325N	0.8750	8	703.47	26457.90	0.027	Bolt Tension
		Diagonal	A325N	0.5000	2	1510.05	4123.34	0.318	Bolt Shear
		Top Gir	A325N	0.5000	2	406.68	4123.34	0.099	Bolt Shear
		Mid Gir	A325N	0.5000	2	408.68	4123.34	0.099	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.00	0.000	Bolt Tension
		Diagonal	A325N	0.5000	2	1448.05	4123.34	0.351	Bolt Shear
		Top Gir	A325N	0.5000	2	481.06	4123.34	0.117	Bolt Shear
		Mid Gir	A325N	0.5000	2	434.16	4123.34	0.105	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26458.10	0.000	Bolt Tension
		Diagonal	A325N	0.5000	2	2623.71	4123.34	0.636	Bolt Shear
		Top Gir	A325N	0.5000	2	415.45	4123.34	0.101	Bolt Shear
		Mid Gir	A325N	0.5000	2	624.82	4123.34	0.152	Bolt Shear
		Leg	A325N	0.8750	8	0.00	26457.80	0.000	Bolt Tension
		Diagonal	A325N	0.5000	2	1889.87	4123.34	0.458	Bolt Shear
		Top Gir	A325N	0.5000	2	4797.96	4123.34	1.164	Bolt Shear
		Diagonal	A325N	0.5000	2	2468.91	4123.34	0.599	Bolt Shear

Section No.	Elevation	Size	Initial Tension	Breaking Load	Actual Load	Allowable T <sub>c</sub>	Required S.F.	Actual S.F.
T5	381'(A)(790)	1 3/8 BS	23200.00	232000.30	72483.90	116000.00	2.000	3.200
	381'(B)(789)	1 3/8 BS	23200.00	232000.30	72603.10	116000.00	2.000	3.195
	381'(C)(788)	1 3/8 BS	23200.00	232000.30	72601.10	116000.00	2.000	3.196
T11	251'(A)(787)	1 1/4 BS	19200.00	192000.16	56512.50	96000.00	2.000	3.397
	251'(B)(786)	1 1/4 BS	19200.00	192000.16	56871.10	96000.00	2.000	3.379
	251'(C)(785)	1 1/4 BS	19200.00	192000.16	56514.20	96000.00	2.000	3.397
T17	131'(A)(779)	3/4 BS	6800.00	67999.83	22087.80	34000.00	2.000	2.945
	131'(B)(778)	3/4 BS	6800.00	67999.83	21814.40	34000.00	2.000	3.117
	131'(B)(777)	3/4 BS	6800.00	67999.83	22098.50	34000.00	2.000	2.944
	131'(B)(774)	3/4 BS	6800.00	67999.83	23044.50	34000.00	2.000	2.951
	131'(C)(767)	3/4 BS	6800.00	67999.83	21666.80	34000.00	2.000	3.140
	131'(C)(768)	3/4 BS	6800.00	67999.83	22367.90	34000.00	2.000	3.040

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**Compression Checks**

**Pole Design Data**

Section No.	Elevation	Size	L <sub>c</sub>	K <sub>1</sub>	F <sub>c</sub>	A	Actual	Allow	Ratio
L1	490 - 457 (1)	dieletric TFU-20DAS	33'	167.8	5.303	31.4614	3404.30	113320.00	0.031

**Pole Bending Design Data**

Section No.	Elevation	Size	Actual	Allow	Ratio	Actual	Allow	Ratio	
L1	490 - 457 (1)	dieletric TFU-20DAS	9408.08	-3.603	23.100	0.156	0.000	23.100	0.000

**Pole Interaction Design Data**

Section No.	Elevation	Size	Ratio	Ratio	Ratio	Ratio	Ratio	Ratio
L1	490 - 457 (1)	dieletric TFU-20DAS	0.021	0.156	0.060	0.177	1.066	H1-3

**Leg Design Data (Compression)**

Section No.	Elevation	Size	L	β	L <sub>c</sub>	K <sub>1</sub>	M <sub>1</sub>	Stability Index	F <sub>c</sub>	A	Actual	Allow	Ratio
T1	457 - 436	3	21'	53°	53°	84.0	18.301	7.0686	-11051.30	120654.00	0.091		
						K=1.00							
T2	436 - 421	2 3/4	15'	39°	39°	65.5	21.765	5.9396	-28765.70	139273.00	0.221		
						K=1.00							
T3	421 - 401	2 3/4	20'	5'	5'	87.3	17.519	5.9396	-68209.10	104054.00	0.654		
						K=1.00							
T4	401 - 381	3 5/8 S.R. w/ 3.5 SCH 40 Half Pipe and 3.75 x 3/16 Half Pipe	20'	5'	5'	64.5	21.932	11.0000	-130098.00	341249.00	0.539		
						K=1.00							
T5	381 - 361	3	20'	5'	5'	68.2	21.272	9.7900	-135107.00	208255.00	0.649		
						K=1.00							
T6	361 - 341	3	20'	5'	5'	80.0	19.012	7.0686	-66599.20	153391.00	0.644		
						K=1.00							
T7	341 - 321	3	20'	5'	5'	80.0	19.012	7.0686	-53969.90	153391.00	0.402		
						K=1.00							
T8	321 - 301	3	20'	5'	5'	80.0	19.012	7.0686	-49039.30	153391.00	0.365		
						K=1.00							
T9	301 - 281	3	20'	5'	5'	80.0	19.012	7.0686	-62211.40	153391.00	0.403		
						K=1.00							



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### Diagonal Design Data (Compression)

Section No.	Elevation	Size	L	$\beta$	$K/\mu$	$F_c$	A	Actual $P$	Allow. $P_u$	Ratio
T1	457-436	L2 1/2x2x3/16	7'11.58"	37°-35'2"	105.7	12,243	0.8090	-1944.09	9965.98	0.196
T2	456-421	L2 1/2x2x3/16	7'3.172"	37°-17'2"	96.9	13,368	0.8090	-3103.20	10814.70	0.287
T3	421-401	L2 1/2x2x3/16	7'9.3	36°-3'8"	104.3	12,422	0.8090	-5415.67	10049.00	0.539
T4	401-381	L2 1/2x2x3/16	7'9.1732"	35°-17'32"	102.9	12,607	0.8090	-8374.31	10198.70	0.821
T5	381-361	L2 1/2x2x3/16	7'9.3558"	35°-5'8"	103.2	12,570	0.8090	-7086.49	10168.90	0.697
T6	361-341	L2 1/2x2x3/16	7'9.31618"	34°-1'8"	104.1	12,459	0.8090	-5317.24	10079.10	0.528
T7	341-321	L2 1/2x2x3/16	7'9.31618"	33°-1'8"	104.1	12,459	0.8090	-3936.96	10079.10	0.391
T8	321-301	L2 1/2x2x3/16	7'9.31618"	32°-1'8"	104.1	12,459	0.8090	-2399.32	10079.10	0.238
T9	301-281	L2 1/2x2x3/16	7'9.31618"	30°-1'8"	104.1	12,459	0.8090	-3785.25	10079.10	0.376
T10	281-261	L1 1/2x1 1/2x1/4	7'9.31618"	31°-1'8"	138.6	7,772	0.6875	-5300.18	5343.35	0.992
T11	261-241	L3x3x1/4	7'9.31618"	31°-1'8"	83.4	14,971	1.4400	-7922.84	21588.70	0.368
T12	241-221	L3x3x1/4	7'9.31618"	31°-1'8"	83.4	14,971	1.4400	-6863.78	21588.70	0.318
T13	221-201	L2 1/2x2x3/16	7'9.31618"	31°-1'8"	103.8	12,496	0.8090	-4463.85	10109.00	0.442
T14	201-181	L2 1/2x2x3/16	7'9.31618"	31°-1'8"	103.8	12,496	0.8090	-2393.22	10109.00	0.237
T15	181-161	L2 1/2x2x3/16	7'9.31618"	31°-1'8"	103.8	12,496	0.8090	-2151.17	10109.00	0.213
T16	161-141	L3x3x1/4	7'9.31618"	31°-1'8"	83.0	15,018	1.4400	-4212.28	21636.20	0.195
T17	141-121	L3x3x1/4	7'9.31618"	31°-1'8"	83.0	15,018	1.4400	-8422.30	21636.20	0.389
T18	121-101	L2 1/2x2x3/16	7'9.31618"	31°-1'8"	103.5	12,533	0.8090	-6681.96	10139.00	0.659
T19	101-81	L2 1/2x2x3/16	7'9.31618"	31°-1'8"	103.5	12,533	0.8090	-4383.81	10139.00	0.432
T20	81-61	L2 1/2x2x3/16	7'9.31618"	31°-1'8"	103.5	12,533	0.8090	-2620.10	10139.00	0.258
T21	61-41	L2 1/2x2x3/16	7'9.31618"	31°-1'8"	103.5	12,533	0.8090	-2896.10	10139.00	0.286
T22	41-20	L2 1/2x2x3/16	7'11.58"	31°-1'8"	105.1	12,321	0.8090	-5040.77	9967.77	0.506
T23	20-6.70833	L2x2x3/16	6'7.5032"	31°-5'02"	99.4	13,059	0.7150	-1662.55	9136.87	0.178
T24	6.70833-0	L2x2x3/16	2'7.3032"	12°-3'04"	58.2	17,604	0.7150	-4937.82	12586.90	0.392

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### Top Girt Design Data (Compression)

Section No.	Elevation	Size	L	$\beta$	$K/\mu$	$F_c$	A	Actual $P$	Allow. $P_u$	Ratio
T1	457-436	L2 1/2x2x3/16	6'	53°	137.7	7,880	0.8090	-275.64	6375.26	0.043
T2	436-421	L2 1/2x2x3/16	6'	53°	137.7	7,880	0.8090	-53.94	6375.26	0.008
T3	421-401	L2 1/2x2x1/4	6'	53°	138.7	7,766	1.0600	-62.90	8232.04	0.008
T4	401-381	L2 1/2x2x1/4	6'	53°	138.7	7,766	1.0600	-378.69	8232.04	0.046
T6	361-341	L2 1/2x2x1/4	6'	53°	137.3	7,931	1.0600	-159.79	8406.99	0.019
T7	341-321	L2 1/2x2x1/4	6'	53°	138.3	7,807	1.0600	-167.40	8275.26	0.020
T17	141-121	L2 1/2x2x3/16	6'	53°	136.9	7,964	0.8090	-115.08	6442.49	0.018
T18	121-101	L2 1/2x2x3/16	6'	53°	136.9	7,964	0.8090	-118.20	6442.49	0.018
T21	61-41	L2 1/2x2x3/16	6'	53°	136.9	7,964	0.8090	-114.87	6442.49	0.018

### Mid Girt Design Data (Compression)

Section No.	Elevation	Size	L	$\beta$	$K/\mu$	$F_c$	A	Actual $P$	Allow. $P_u$	Ratio
T1	457-436	L2 1/2x2x3/16	6'	53°	137.7	7,880	0.8090	-36.86	6375.26	0.006
T2	436-421	L2 1/2x2x3/16	6'	53°	138.0	7,839	0.8090	-20.78	6342.04	0.003
T3	421-401	L2 1/2x2x1/4	6'	53°	138.7	7,766	1.0600	-136.51	8332.04	0.017
T6	361-341	L2 1/2x2x1/4	6'	53°	138.3	7,807	1.0600	-12.86	8275.26	0.002
T8	321-301	L2 1/2x2x1/4	6'	53°	138.3	7,807	1.0600	-171.02	8275.26	0.021
T17	141-121	L2 1/2x2x3/16	6'	53°	136.9	7,964	0.8090	-5819.75	6442.49	0.903
T18	121-101	L2 1/2x2x3/16	6'	53°	136.9	7,964	0.8090	-141.84	6442.49	0.022
T19	101-81	L2 1/2x2x3/16	6'	53°	136.9	7,964	0.8090	-22.79	6442.49	0.003
T21	61-41	L2 1/2x2x3/16	6'	53°	136.9	7,964	0.8090	-42.80	6442.49	0.007

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**Tower Engineering Professionals** Project TEP# 072725 Date 11:08:39 01/10/08  
3703 Junction Boulevard Raleigh, NC 27603  
Phone: (919) 661-6351 Client Crown Castle Designed by Matt Lackey, E.I.  
FAX: (919) 661-6310

### Torque-Arm Bottom Design Data

Section No.	Elevation	Size	L	Lc	K/r	F <sub>a</sub>	A	ir <sup>2</sup>	Actual P	Allow P	Ratio
T17	141 - 121 (771)	2L3x3x3/16	9'23/32"	7'2-3/4"	106.2	11,991	2.1800	-18758.80	26140.70	0.718	
					K=1.15						
T17	141 - 121 (772)	2L3x3x3/16	9'23/32"	7'2-3/4"	106.2	11,991	2.1800	-19622.50	26140.70	0.751	
					K=1.15						
T17	141 - 121 (777)	2L3x3x3/16	9'23/32"	7'2-3/4"	106.2	11,991	2.1800	-20491.20	26140.70	0.784	
					K=1.15						
T17	141 - 121 (778)	2L3x3x3/16	9'23/32"	7'2-3/4"	106.2	11,991	2.1800	-20011.30	26140.70	0.766	
					K=1.15						
T17	141 - 121 (783)	2L3x3x3/16	9'23/32"	7'2-3/4"	106.2	11,991	2.1800	-20173.40	26140.70	0.772	
					K=1.15						
T17	141 - 121 (784)	2L3x3x3/16	9'23/32"	7'2-3/4"	106.2	11,991	2.1800	-20512.20	26140.70	0.785	
					K=1.15						

### Tension Checks

### Leg Design Data (Tension)

Section No.	Elevation	Size	L	Lc	K/r	F <sub>a</sub>	A	ir <sup>2</sup>	Actual P	Allow P	Ratio
T1	457 - 436	3	21'	5'3"	84.0	30,000	7.0686	8311.18	212058.00	0.039	
T2	436 - 421	2 3/4	15'	3'9"	65.5	30,000	5.9996	22500.00	178187.00	0.126	
T3	421 - 401	2 3/4	20'	5'	87.3	30,000	5.9996	57373.10	178187.00	0.322	
T4	401 - 381	3.5" S.R. w/ 3.5 SCH40 Half Pipe	20'	5'	64.5	30,000	11.0000	111309.00	330000.00	0.337	
T5	381 - 361	3" S.R. w/ 3 SCH40 Half Pipe and 3/2 x 5/16 Half Pipe	20'	5'	68.2	30,000	9.7900	60090.70	297700.00	0.205	
T6	361 - 341	3	20'	5'	80.0	30,000	7.0686	15965.90	212058.00	0.075	
T10	281 - 261	3	20'	5'	80.0	30,000	7.0686	14485.20	212058.00	0.068	
T11	261 - 241	3	20'	5'	80.0	30,000	7.0686	34494.80	212058.00	0.163	
T19	101 - 81	3 1/2	20'	5'	68.6	30,000	9.6211	1949.22	288634.00	0.007	
T20	81 - 61	3 1/2	20'	5'	68.6	30,000	9.6211	6795.35	288634.00	0.024	
T21	61 - 41	3 1/2	20'	5'	68.6	30,000	9.6211	2541.19	288634.00	0.009	

### Leg Bending Design Data (Tension)

Section No.	Elevation	Size	Actual M <sub>x</sub> /I <sub>x</sub>	Allow F <sub>b</sub> /K <sub>t</sub>	Actual M <sub>y</sub> /I <sub>y</sub>	Allow F <sub>b</sub> /K <sub>t</sub>	Actual P	Allow P	Ratio
T1	457 - 436	3	0.00	37,500	0.00	0.00	0.00	37,500	0.000
T2	436 - 421	2 3/4	0.00	37,500	0.00	0.00	0.00	37,500	0.000
T3	421 - 401	2 3/4	0.00	37,500	0.00	0.00	0.00	37,500	0.000
T4	401 - 381	3.5" S.R. w/ 3.5 SCH40 Half Pipe	0.00	30,000	0.00	0.00	0.00	30,000	0.000

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**Tower Engineering Professionals** Project TEP# 072725 Date 11:08:39 01/10/08  
3703 Junction Boulevard Raleigh, NC 27603  
Phone: (919) 661-6351 Client Crown Castle Designed by Matt Lackey, E.I.  
FAX: (919) 661-6310

### Leg Interaction Design Data (Tension)

Section No.	Elevation	Size	Actual M <sub>x</sub> /I <sub>x</sub>	Actual M <sub>y</sub> /I <sub>y</sub>	Actual P	Allow F <sub>b</sub> /K <sub>t</sub>	Actual M <sub>x</sub> /I <sub>x</sub>	Actual M <sub>y</sub> /I <sub>y</sub>	Actual P	Allow P	Ratio
T5	381 - 361	3" S.R. w/ 3 SCH40 Half Pipe and 3/2 x 5/16 Half Pipe	0.00	0.00	0.00	30,000	0.000	0.000	0.00	30,000	0.000
T6	361 - 341	3	0.00	0.00	0.00	37,500	0.000	0.000	0.00	37,500	0.000
T10	281 - 261	3	0.00	0.00	0.00	37,500	0.000	0.000	0.00	37,500	0.000
T11	261 - 241	3	0.00	0.00	0.00	37,500	0.000	0.000	0.00	37,500	0.000
T19	101 - 81	3 1/2	0.00	0.00	0.00	37,500	0.000	0.000	0.00	37,500	0.000
T20	81 - 61	3 1/2	0.00	0.00	0.00	37,500	0.000	0.000	0.00	37,500	0.000
T21	61 - 41	3 1/2	0.00	0.00	0.00	37,500	0.000	0.000	0.00	37,500	0.000

### Diagonal Design Data (Tension)

Section No.	Elevation	Size	L	Lc	K/r	F <sub>a</sub>	A	ir <sup>2</sup>	Actual P	Allow P	Ratio
T1	457 - 436	L2 1/2x2x3/16	7'11-5/8"	3'7-3/32"	76.4	29,000	0.5189	1931.48	15046.90	0.133	
T2	436 - 421	L2 1/2x2x3/16	7'3-1/2"	3'2-1/32"	68.1	29,000	0.5189	1081.78	15046.90	0.204	
T3	421 - 401	L2 1/2x2x3/16	7'9"	3'6-3/8"	75.2	29,000	0.5189	5354.38	15046.90	0.356	
T4	401 - 381	L2 1/2x2x3/16	7'9"	3'5"	73.8	29,000	0.5189	8671.96	15046.90	0.576	
T5	381 - 361	L2 1/2x2x3/16	7'9"	3'5-5/8"	74.1	29,000	0.5189	7511.55	15046.90	0.483	
T6	361 - 341	L2 1/2x2x3/16	7'9"	3'6-1/8"	74.9	29,000	0.5189	5333.30	15046.90	0.348	

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Client		Crown Castle			
Designed by		Matt Lackey, E.I.			

Section No.	Elevation	Size	L	L <sub>w</sub>	Kl/r	F <sub>c</sub>	A	Actual P	Allow. P	Ratio
			β	β		ksi	in <sup>2</sup>	lb	lb	P/P
T7	341 - 321	L2 1/2x2x3/16	79°	3'6-1/8"	74.9	29,000	0.5189	3986.64	15046.90	0.265
T8	331 - 301	L2 1/2x2x3/16	79°	3'6-1/8"	74.9	29,000	0.5189	2274.77	15046.90	0.151
T9	301 - 281	L2 1/2x2x3/16	79°	3'6-1/8"	74.9	29,000	0.5189	3674.34	15046.90	0.244
T10	281 - 261	L1 1/2x1 1/2x1/4	79°	3'6-1/8"	100.0	29,000	0.3984	5168.93	11554.70	0.447
T11	261 - 241	L3x3x1/4	79°	3'6-1/8"	48.3	29,000	0.9628	8683.18	27921.60	0.311
T12	241 - 221	L3x3x1/4	79°	3'6-1/8"	48.3	29,000	0.9628	6848.23	27921.60	0.245
T13	221 - 201	L2 1/2x2x3/16	79°	3'6"	74.6	29,000	0.5189	4307.39	15046.90	0.286
T14	201 - 181	L2 1/2x2x3/16	79°	3'6"	74.6	29,000	0.5189	2206.70	15046.90	0.147
T15	181 - 161	L2 1/2x2x3/16	79°	3'6"	74.6	29,000	0.5189	1895.42	15046.90	0.126
T16	161 - 141	L3x3x1/4	79°	3'5-7/8"	47.9	29,000	0.9628	4006.11	27921.60	0.143
T17	141 - 121	L3x3x1/4	79°	3'5-7/8"	47.9	29,000	0.9628	6924.40	27921.60	0.248
T18	121 - 101	L2 1/2x2x3/16	79°	3'5-7/8"	74.3	29,000	0.5189	6634.77	15046.90	0.441
T19	101 - 81	L2 1/2x2x3/16	79°	3'5-7/8"	74.3	29,000	0.5189	4030.22	15046.90	0.268
T20	81 - 61	L2 1/2x2x3/16	79°	3'5-7/8"	74.3	29,000	0.5189	3402.45	15046.90	0.160
T21	61 - 41	L2 1/2x2x3/16	79°	3'5-7/8"	74.3	29,000	0.5189	2511.84	15046.90	0.167
T22	41 - 20	L2 1/2x2x3/16	79°	3'6"	75.9	29,000	0.5189	5247.43	15046.90	0.349
T23	20 - 6.70833	L2x2x3/16	13/32"	3'9"	58.0	29,000	0.4484	3779.75	13002.40	0.291
T24	6.70833 - 0	L2x2x3/16	3/16"	1'4"-7/16"	31.1	29,000	0.4484	4500.21	13002.40	0.346

### Top Girt Design Data (Tension)

Section No.	Elevation	Size	L	L <sub>w</sub>	Kl/r	F <sub>c</sub>	A	Actual P	Allow. P	Ratio
			β	β		ksi	in <sup>2</sup>	lb	lb	P/P
T1	457-436	L2 1/2x2x3/16	6	5'3"	115.0	29,000	0.5189	256.44	15046.90	0.017
T2	436-421	L2 1/2x2x3/16	6	5'3"	115.0	29,000	0.5189	71.49	15046.90	0.005
T3	421-401	L2 1/2x2x1/4	6	5'3"	116.9	29,000	0.6778	73.14	19656.60	0.004
T4	401-381	L2 1/2x2x1/4	6	5'3"	116.9	29,000	0.6778	387.95	19656.60	0.020

### Mid Girt Design Data (Tension)

Section No.	Elevation	Size	L	L <sub>w</sub>	Kl/r	F <sub>c</sub>	A	Actual P	Allow. P	Ratio
			β	β		ksi	in <sup>2</sup>	lb	lb	P/P
T1	457-436	L2 1/2x2x3/16	6	5'3"	115.0	29,000	0.5189	70.51	15046.90	0.005
T2	436-421	L2 1/2x2x3/16	6	5'3"	115.5	29,000	0.5189	34.63	15046.90	0.002
T3	421-401	L2 1/2x2x1/4	6	5'3"	116.9	29,000	0.6778	130.31	19656.60	0.007

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Client		Crown Castle			
Designed by		Matt Lackey, E.I.			

Section No.	Elevation	Size	L	L <sub>w</sub>	Kl/r	F <sub>c</sub>	A	Actual P	Allow. P	Ratio
			β	β		ksi	in <sup>2</sup>	lb	lb	P/P
T5	381 - 361	L3x2x1/4	6	5'2"	88.8	29,000	1.5506	21407.70	44988.10	0.476
T6	361 - 341	L2 1/2x2x1/4	6	5'2-3/4"	115.2	29,000	0.6778	448.72	19656.60	0.023
T7	341 - 321	L2 1/2x2x1/4	6	5'3"	116.5	29,000	0.6778	432.66	19656.60	0.022
T8	321 - 301	L3 1/2x2x1/4	6	5'3"	116.5	29,000	0.6778	261.26	19656.60	0.013
T9	301 - 281	L2 1/2x2x3/16	6	5'3"	115.0	29,000	0.5189	183.53	15046.90	0.012
T10	281 - 261	L2 1/2x2x1/4	6	5'3"	116.5	29,000	0.6778	252.24	19656.60	0.013
T11	261 - 241	L3 1/2x3 1/2x3/8	6	5'3"	64.5	29,000	3.3759	613.78	97902.20	0.006
T12	241 - 221	L2 1/2x2x3/16	6	5'3"	115.0	29,000	0.5189	828.92	15046.90	0.055
T13	221 - 201	L2 1/2x2x3/16	6	5'3"	115.0	29,000	0.5189	420.98	15046.90	0.038
T14	201 - 181	L2 1/2x2x3/16	6	5'3-1/4"	114.6	29,000	0.5189	490.35	15046.90	0.033
T15	181 - 161	L2 1/2x2x3/16	6	5'3-1/4"	114.6	29,000	0.5189	543.32	15046.90	0.036
T16	161 - 141	L2 1/2x2x3/16	6	5'3-1/4"	114.6	29,000	0.5189	536.82	15046.90	0.036
T17	141 - 121	L2 1/2x2x3/16	6	5'3"	114.2	29,000	0.5189	1277.90	15046.90	0.085
T18	121 - 101	L2 1/2x2x3/16	6	5'3"	114.2	29,000	0.5189	2348.32	15046.90	0.156
T19	101 - 81	L2 1/2x2x3/16	6	5'3"	114.2	29,000	0.5189	720.28	15046.90	0.048
T20	81 - 61	L2 1/2x2x3/16	6	5'3"	114.2	29,000	0.5189	813.26	15046.90	0.054
T21	61 - 41	L2 1/2x2x3/16	6	5'3"	114.2	29,000	0.5189	962.11	15046.90	0.064
T22	41 - 20	L2 1/2x2x3/16	6	5'3"	114.2	29,000	0.5189	820.89	15046.90	0.055
T23	20 - 6.70833	L3x2x3/16	6	5'3"	117.4	29,000	0.8866	9595.92	17069.70	0.562

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Client		Crown Castle			
Designed by		Matt Lackey, E.I.			

Section No.	Elevation	Size	L	L <sub>c</sub>	Kl/r	F <sub>c</sub>	A	Actual P	Allow. P	Ratio
	β		β	β		ksi	in <sup>2</sup>	lb	lb	P/P
T4	401 - 381	L2 1/2x2x1/4	6'	5'2"	114.8	29,000	0.6778	529.98	1965.60	0.027
T5	381 - 361	L2 1/2x2x1/4	6'	5'2-3/4"	115.2	29,000	0.6778	646.23	1965.60	0.033
T6	361 - 341	L2 1/2x2x1/4	6'	5'3"	116.5	29,000	0.6778	297.08	1965.60	0.015
T7	341 - 321	L2 1/2x2x1/4	6'	5'3-1/2"	116.5	29,000	0.6778	187.09	1965.60	0.010
T8	321 - 301	L2 1/2x2x1/4	6'	5'3"	116.5	29,000	0.6778	500.88	1965.60	0.025
T9	301 - 281	L2 1/2x2x3/16	6'	5'3-1/2"	115.0	29,000	0.5189	233.21	15046.90	0.015
T10	281 - 261	L2 1/2x2x1/4	6'	5'3"	116.5	29,000	0.6778	253.28	1965.60	0.013
T11	261 - 241	2L3x3x3/16	6'	5'3"	64.5	29,000	3.3759	20998.20	9790.20	0.214
T12	241 - 221	L2 1/2x2x3/16	6'	5'3"	115.0	29,000	0.5189	421.87	15046.90	0.028
T13	221 - 201	L2 1/2x2x3/16	6'	5'3-1/4"	114.6	29,000	0.5189	428.74	15046.90	0.028
T14	201 - 181	L2 1/2x2x3/16	6'	5'3-1/4"	114.6	29,000	0.5189	517.27	15046.90	0.034
T15	181 - 161	L2 1/2x2x3/16	6'	5'3-1/4"	114.6	29,000	0.5189	519.49	15046.90	0.035
T16	161 - 141	L2 1/2x2x3/16	6'	5'3"	114.2	29,000	0.5189	536.02	15046.90	0.036
T17	141 - 121	L2 1/2x2x3/16	6'	5'3"	114.2	29,000	0.5189	9707.24	15046.90	0.645
T18	121 - 101	L2 1/2x2x3/16	6'	5'3"	114.2	29,000	0.5189	985.93	15046.90	0.066
T19	101 - 81	L2 1/2x2x3/16	6'	5'3"	114.2	29,000	0.5189	822.34	15046.90	0.055
T20	81 - 61	L2 1/2x2x3/16	6'	5'3"	114.2	29,000	0.5189	817.36	15046.90	0.054
T21	61 - 41	L2 1/2x2x3/16	6'	5'3"	114.2	29,000	0.5189	868.31	15046.90	0.058
T22	41 - 20	L2 1/2x2x3/16	6'	5'3"	114.2	29,000	0.5189	1249.63	15046.90	0.083

### Torque-Arm Top Design Data

Section No.	Elevation	Size	L	L <sub>c</sub>	Kl/r	F <sub>c</sub>	A	Actual P	Allow. P	Ratio
	β		β	β		ksi	in <sup>2</sup>	lb	lb	P/P
T17	141 - 121 (769)	2L3x3x3/16	7'-6"	5'9-1/8"	94.6	21,600	2.1800	21897.10	47088.00	0.457
T17	141 - 121 (770)	2L3x3x3/16	7'-6"	5'9-1/8"	94.6	21,600	2.1800	21806.40	47088.00	0.463
T17	141 - 121 (775)	2L3x3x3/16	7'-6"	5'9-1/8"	94.6	21,600	2.1800	22316.00	47088.00	0.485

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Client		Crown Castle			
Designed by		Matt Lackey, E.I.			

Section No.	Elevation	Size	L	L <sub>c</sub>	Kl/r	F <sub>c</sub>	A	Actual P	Allow. P	Ratio
	β		β	β		ksi	in <sup>2</sup>	lb	lb	P/P
T17	141 - 121 (776)	2L3x3x3/16	19'32"	5'9-1/8"	94.6	21,600	2.1800	22123.80	47088.00	0.470
T17	141 - 121 (781)	2L3x3x3/16	19'32"	5'9-1/8"	94.6	21,600	2.1800	23131.40	47088.00	0.491
T17	141 - 121 (782)	2L3x3x3/16	19'32"	5'9-1/8"	94.6	21,600	2.1800	23101.20	47088.00	0.491

### Torque-Arm Bottom Design Data

Section No.	Elevation	Size	L	L <sub>c</sub>	Kl/r	F <sub>c</sub>	A	Actual P	Allow. P	Ratio
	β		β	β		ksi	in <sup>2</sup>	lb	lb	P/P
T17	141 - 121 (771)	2L3x3x3/16	9'23'32"	7'2-3/4"	113.5	21,600	2.1800	2501.69	47088.00	0.053
T17	141 - 121 (772)	2L3x3x3/16	9'23'32"	7'2-3/4"	113.5	21,600	2.1800	2714.14	47088.00	0.058
T17	141 - 121 (777)	2L3x3x3/16	9'23'32"	7'2-3/4"	113.5	21,600	2.1800	3408.83	47088.00	0.072
T17	141 - 121 (778)	2L3x3x3/16	9'23'32"	7'2-3/4"	113.5	21,600	2.1800	3386.29	47088.00	0.072
T17	141 - 121 (783)	2L3x3x3/16	9'23'32"	7'2-3/4"	113.5	21,600	2.1800	2882.10	47088.00	0.061
T17	141 - 121 (784)	2L3x3x3/16	9'23'32"	7'2-3/4"	113.5	21,600	2.1800	3198.79	47088.00	0.068

### Section Capacity Table

Section No.	Elevation	Component	Size	Critical Element	P	SP <sub>1</sub> P <sub>2</sub> P <sub>3</sub>	Capacity	Ratio
	β	Type			lb	lb	lb	P/P
L1	490 - 457	Polc	discipline TFU20UDAS	1	-2404.80	121377.65	16.6	Pass
T1	457 - 436	Leg		3	-11651.30	171495.77	6.8	Pass
T2	436 - 421	Leg		39	-38765.70	173220.90	16.7	Pass
T3	421 - 401	Leg		72	-68009.10	138703.98	49.0	Pass
T4	401 - 381	Leg	3.5" S.R. w/ 3.3 SCH40 Half Pipe	105	-130098.00	331584.90	40.3	Pass
T5	381 - 361	Leg	3" S.R. w/ 3 SCH40 Half Pipe w/ 3.75 x 3/16 Half Pipe	138	-133103.00	277603.90	45.7	Pass
T6	361 - 341	Leg		171	86598.20	179143.20	43.3	Pass
T7	341 - 321	Leg		204	-53908.90	179143.20	30.1	Pass
T8	321 - 301	Leg		236	-80038.80	179143.20	27.4	Pass
T9	301 - 281	Leg		269	-62211.40	179143.20	34.7	Pass
T10	281 - 261	Leg		302	-98230.00	179143.20	54.8	Pass
T11	261 - 241	Leg		337	-124236.00	179143.20	69.3	Pass
T12	241 - 221	Leg		370	-96094.60	179143.20	53.6	Pass
T13	221 - 201	Leg		402	-91725.50	233537.43	41.0	Pass
T14	201 - 181	Leg		435	-104924.00	233537.43	46.9	Pass
T15	181 - 161	Leg		468	-106871.00	233537.43	47.4	Pass
T16	161 - 141	Leg		501	-100766.00	271901.33	37.1	Pass



**RISATower**  
 Tower Engineering Professionals  
 3703 Junction Boulevard  
 Raleigh, NC 27603  
 Phone: (919) 661-4831  
 Fax: (919) 661-4830

Job: BU# 873128 "Trumbull"  
 Project: TEP# 072725  
 Client: Crown Castle

Page: 63 of 65  
 Date: 11.08.09 01/10/08  
 Designed by: Matt Lackey, E.I.

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Page: 64 of 65  
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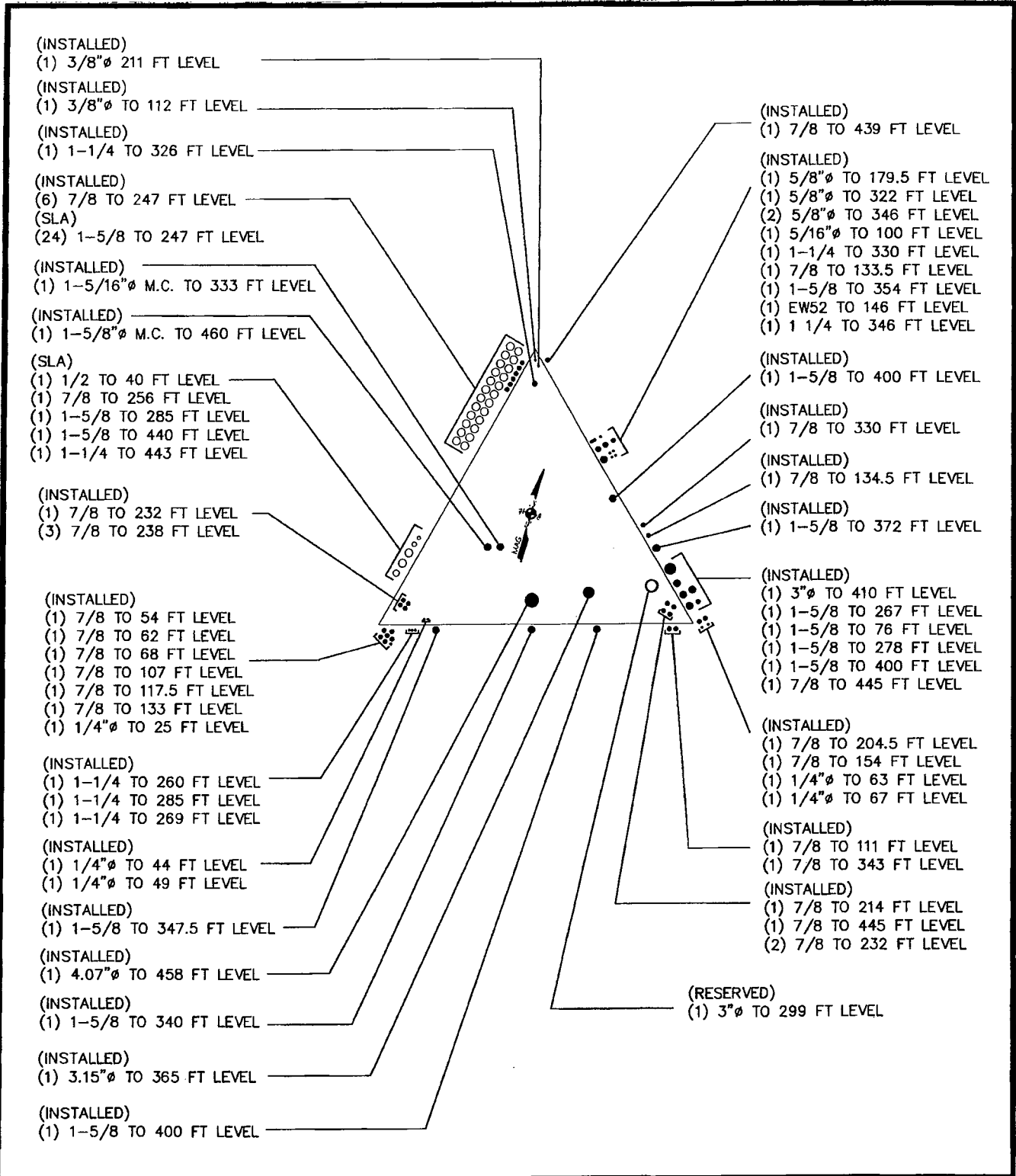
Section No.	Elevation	Component Type	Size	Critical Element	p/b	SP-y <sub>max</sub>	% Capacity	Pass/Fail
T17	141 - 121	Leg	3 1/2	534	-10874.00	271901.33	39.2	Pass
T18	121 - 101	Leg	3 1/2	567	-158677.00	271901.33	58.4	Pass
T19	101 - 81	Leg	3 1/2	650	-184977.00	271901.33	68.0	Pass
T20	81 - 61	Leg	3 1/2	666	-191318.00	271901.33	71.4	Pass
T21	61 - 41	Leg	3 1/2	699	-171687.00	263738.04	68.1	Pass
T22	41 - 20	Leg	3 1/2	732	-143312.00	239297.48	59.9	Pass
T23	20 - 6.70833	Leg	3 1/4	753	-142848.00	271657.39	52.6	Pass
T24	6.70833 - 0	Leg	L2 1/2x2x3/16	16	-1944.09	13204.67	14.7	Pass
T1	457 - 436	Diagonal	L2 1/2x2x3/16	49	-3105.20	14415.99	21.5	Pass
T2	436 - 421	Diagonal	L2 1/2x2x3/16	82	-5415.67	13395.32	28.2	Pass
T3	421 - 401	Diagonal	L2 1/2x2x3/16	121	-8734.31	13594.87	49.2	Pass
T4	401 - 381	Diagonal	L2 1/2x2x3/16	163	-7086.49	13555.14	61.6	Pass
T5	381 - 361	Diagonal	L2 1/2x2x3/16	197	-5317.24	13435.44	48.4	Pass
T6	361 - 341	Diagonal	L2 1/2x2x3/16	230	-3936.96	13435.44	29.3	Pass
T7	341 - 321	Diagonal	L2 1/2x2x3/16	247	-2299.32	13435.44	17.1	Pass
T8	321 - 301	Diagonal	L2 1/2x2x3/16	280	-3785.25	13435.44	28.2	Pass
T9	301 - 281	Diagonal	L2 1/2x2x3/16	313	-5306.18	7122.69	74.4	Pass
T10	281 - 261	Diagonal	L1 1/2x1 1/2x1/4	350	-7322.84	28737.74	27.6	Pass
T11	261 - 241	Diagonal	L3x3x1/4	395	-6863.78	28737.74	70.0	Pass
T12	241 - 221	Diagonal	L3x3x1/4	428	-4463.85	13475.30	33.1	Pass
T13	221 - 201	Diagonal	L2 1/2x2x3/16	461	-2393.22	13475.30	17.8	Pass
T14	201 - 181	Diagonal	L2 1/2x2x3/16	478	-2151.17	13475.30	16.0	Pass
T15	181 - 161	Diagonal	L2 1/2x2x3/16	509	-4512.28	28827.72	14.6	Pass
T16	161 - 141	Diagonal	L3x3x1/4	542	-8423.30	28827.72	38.2	Pass
T17	141 - 121	Diagonal	L3x3x1/4	587	-6881.96	13515.29	49.4	Pass
T18	121 - 101	Diagonal	L2 1/2x2x3/16	627	-4383.81	13515.29	32	Pass
T19	101 - 81	Diagonal	L2 1/2x2x3/16	660	-2620.10	13515.29	19.4	Pass
T20	81 - 61	Diagonal	L2 1/2x2x3/16	677	-2896.10	13515.29	21.4	Pass
T21	61 - 41	Diagonal	L2 1/2x2x3/16	713	-5040.77	13287.04	37.9	Pass
T22	41 - 20	Diagonal	L2 1/2x2x3/16	735	3779.75	17332.20	21.8	Pass
T23	20 - 6.70833	Diagonal	L2x2x3/16	756	-4937.82	16778.34	34.4	Pass
T24	6.70833 - 0	Diagonal	L3x2x3/16	8	-275.64	8498.22	3.2	Pass
T1	457 - 436	Top Girt	L2 1/2x2x3/16	8	-53.94	8498.22	0.6	Pass
T2	436 - 421	Top Girt	L2 1/2x2x3/16	43	-62.90	10973.31	0.7	Pass
T3	421 - 401	Top Girt	L2 1/2x2x3/16	76	-378.69	10973.31	3.5	Pass
T4	401 - 381	Top Girt	L2 1/2x2x3/16					Pass

Section No.	Elevation	Component Type	Size	Critical Element	p/b	SP-y <sub>max</sub>	% Capacity	Pass/Fail
T5	381 - 361	Top Girt	2L3x2x1/4	107	21407.70	59942.48	35.7	Pass
T6	361 - 341	Top Girt	L2 1/2x2x1/4	142	448.72	26202.25	4.1	Pass
T7	341 - 321	Top Girt	L2 1/2x2x1/4	173	432.66	26202.25	1.7	Pass
T8	321 - 301	Top Girt	L3 1/2x2x1/4	208	261.26	26202.25	3.9	Pass
T9	301 - 281	Top Girt	L2 1/2x2x3/16	239	183.55	20057.52	2.4	Pass
T10	281 - 261	Top Girt	L3 1/2x2x1/4	274	252.24	26202.25	1.0	Pass
T11	261 - 241	Top Girt	3L3 1/2x3 1/2x3/8	306	613.78	130803.63	2.3	Pass
T12	241 - 221	Top Girt	L2 1/2x2x3/16	340	828.92	20057.52	4.1	Pass
T13	221 - 201	Top Girt	L2 1/2x2x3/16	372	420.98	20057.52	2.1	Pass
T14	201 - 181	Top Girt	L2 1/2x2x3/16	405	490.25	20057.52	2.4	Pass
T15	181 - 161	Top Girt	L2 1/2x2x3/16	439	544.52	20057.52	2.7	Pass
T16	161 - 141	Top Girt	L2 1/2x2x3/16	472	536.82	20057.52	4.9	Pass
T17	141 - 121	Top Girt	L3 1/2x2x3/16	505	1277.90	20057.52	6.6	Pass
T18	121 - 101	Top Girt	L2 1/2x2x3/16	536	2348.32	20057.52	11.7	Pass
T19	101 - 81	Top Girt	L2 1/2x2x3/16	570	730.28	20057.52	3.6	Pass
T20	81 - 61	Top Girt	L2 1/2x2x3/16	602	813.36	20057.52	4.1	Pass
T21	61 - 41	Top Girt	L2 1/2x2x3/16	635	962.11	20057.52	4.8	Pass
T22	41 - 20	Top Girt	L2 1/2x2x3/16	668	830.89	20057.52	4.1	Pass
T23	20 - 6.70833	Top Girt	L3x2x3/16	702	9395.97	17353.91	53.2	Pass
T1	457 - 436	Mid Girt	L2 1/2x2x3/16	12	-36.86	8498.22	0.6	Pass
T2	436 - 421	Mid Girt	L2 1/2x2x3/16	46	-20.78	8433.94	0.3	Pass
T3	421 - 401	Mid Girt	L2 1/2x2x1/4	79	-136.51	10973.31	1.2	Pass
T4	401 - 381	Mid Girt	L2 1/2x2x1/4	112	539.98	26202.25	2.0	Pass
T5	381 - 361	Mid Girt	L1 1/2x2x1/4	145	646.23	26202.25	4.8	Pass
T6	361 - 341	Mid Girt	L2 1/2x2x1/4	178	397.08	26202.25	5.9	Pass
T7	341 - 321	Mid Girt	L2 1/2x2x1/4	209	187.09	26202.25	2.7	Pass
T8	321 - 301	Mid Girt	L2 1/2x2x1/4	242	300.85	26202.25	1.9	Pass
T9	301 - 281	Mid Girt	L2 1/2x2x3/16	277	231.21	20057.52	1.2	Pass
T10	281 - 261	Mid Girt	L2 1/2x2x1/4	309	353.28	26202.25	2.1	Pass
T11	261 - 241	Mid Girt	2L3 1/2x3 1/2x3/8	342	20998.20	130605.63	16.1	Pass
T12	241 - 221	Mid Girt	L2 1/2x2x3/16	376	421.87	20057.52	2.1	Pass

Job	BU# 873128 "Trumbull"	Page	65 of 65
Project	TEP# 072725	Date	11:08:39 01/10/08
Client	Crown Castle	Designed by	Matt Lackey, E.I.

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SFYP <sub>allow</sub> lb	% Capacity	Pass/Fail
T13	221 - 201	Mid Girt	L2 1/2x2x3/16	409	428.74	20057.52	2.1	Pass
T14	201 - 181	Mid Girt	L2 1/2x2x3/16	442	517.27	20057.52	2.6	Pass
T15	181 - 161	Mid Girt	L2 1/2x2x3/16	474	519.49	20057.52	4.7 (b)	Pass
T16	161 - 141	Mid Girt	L2 1/2x2x3/16	507	536.02	20057.52	4.7 (b)	Pass
T17	141 - 121	Mid Girt	L2 1/2x2x3/16	539	-5819.75	8387.84	4.9 (b)	Pass
T18	121 - 101	Mid Girt	L2 1/2x2x3/16	572	983.93	20057.52	4.9	Pass
T19	101 - 81	Mid Girt	L2 1/2x2x3/16	605	822.34	20057.52	4.1	Pass
T20	81 - 61	Mid Girt	L2 1/2x2x3/16	638	817.36	20057.52	4.1	Pass
T21	61 - 41	Mid Girt	L2 1/2x2x3/16	671	868.31	20057.52	4.3	Pass
T22	41 - 20	Mid Girt	L2 1/2x2x3/16	704	1249.63	20057.52	11.2 (b)	Pass
T5	381 - 361	Guy A@381	1/38	790	72488.90	116000.00	62.5	Pass
T11	261 - 241	Guy A@251	1/14	787	56512.50	96000.00	58.9	Pass
T17	141 - 121	Guy A@131	3/4	779	23087.80	34000.00	67.9	Pass
T5	381 - 361	Guy B@381	1/38	789	72603.10	116000.00	62.6	Pass
T11	261 - 241	Guy B@251	1/14	786	56817.10	96000.00	59.2	Pass
T17	141 - 121	Guy B@131	3/4	773	23098.50	34000.00	67.9	Pass
T5	381 - 361	Guy C@381	1/38	788	72601.10	116000.00	62.6	Pass
T11	261 - 241	Guy C@251	1/14	785	56514.50	96000.00	58.9	Pass
T17	141 - 121	Guy C@131	3/4	768	22367.90	34000.00	65.8	Pass
T17	141 - 121	Torque Arm Bottom@131	2L3x2x3/16	781	23131.40	62768.30	36.9	Pass
T17	141 - 121	Torque Arm Bottom@131	2L3x2x3/16	784	-20512.20	34845.55	58.9	Pass
Summary								
								Pole (L1)
								Leg (T20)
								Diagonal (T11)
								Top Girt (T5)
								Mid Girt (T11)
								Guy A (T17)
								Guy B (T17)
								Guy C (T17)
								Torque Arm Top (T17)
								Torque Arm Bottom (T17)
								Boil Checks
								<b>PAYING = 97.4</b>
								Pass

**APPENDIX B**  
**BASE LEVEL DRAWING**



**COAX PLAN**

N.T.S.

PREPARED BY:  
**TOWER ENGINEERING PROFESSIONALS**  
 3703 JUNCTION BOULEVARD  
 RALEIGH, NC 27603-5263  
 (919) 661-6351

PREPARED FOR:  
**CROWN CASTLE INTERNATIONAL**  
 Crown Castle USA Inc.  
 25 WILKINSON BLVD.  
 SUITE 150  
 CHARLOTTE, NC 28270

PROJECT INFORMATION:  
**TRUMBULL**  
**SITE # 873128**  
 800 BOOTH HILL ROAD  
 SHELTON, CT 06611  
 (FAIRFIELD COUNTY)

REVISION: 0  
 TEP JOB #: 072725  
 SHEET NUMBER:  
**M-1**

**APPENDIX C**  
**ADDITIONAL CALCULATIONS**



Project Name: Irum Dull  
Project #: 071092  
Date: December 12, 2007  
Design: NKL  
Check: PEH  
Page 1 of 1

### Moist Analysis

$$A_0 = (10.5ft) \times (10.5ft) = 110.25ft^2$$

$$V_{concrete} = (10.5ft)(10.5ft)(2ft) + \frac{3ft((10ft)^2 + (4.5ft)(10ft) + (4.5ft)^2)}{3}$$

$$V_{concrete} = 385.8ft^3$$

$$W_{concrete} = (385.8ft^3)(0.150 k/ft^3) = 57.87 kips$$

$$V_{soil} = (10ft)(10ft)(3ft) - 165.3ft^3 + (10.5^2 - 10^2) \cdot 3$$

$$V_{soil} = 134.8ft^3 + 30.75ft^3 = 165.5ft^3$$

$$W_{soil} = (165.5ft^3)(0.115 k/ft^3) = 19.0k$$

$$\text{Total Weight} = 19.0k + 57.87kips = 76.90 kips$$

$$\frac{76.90kips}{110.25ft^2} = 0.70 \text{ ksf}$$

$$\text{Net Bearing} = 6 \text{ ksf} - 0.70 \text{ ksf} = 5.30 \text{ ksf}$$

$$\text{Downforce} = 303.8 \text{ kips}$$

$$\text{Allowable} = 5.30 \text{ ksf} (110.25ft^2) = 584.3 \text{ kips}$$

$$\frac{303.8k}{584.3k} * 100 = \boxed{52.0\%}$$



ANCHOR CAPACITY ANALYSIS

Project Name: Trumbull  
Project #: 071092  
Date: December 12, 2007  
Design: MKL  
Check: PEH  
Page 1 of 3

Anchor A - Geotech B4 No Cap  
Anchor B - Geotech B2 Cap  
Anchor C - Geotech B3 Cap

Anchor A

$W_c = (23ft)(18.5ft)(3.3ft)(0.15kcf) = 210.6K$   
 $V_b = (23ft)(18.5ft)(2.0ft)(0.115kcf) = 97.87K$

Uplift

Eq. 1 =  $\frac{W_R}{2.0} + \frac{W_c}{1.25} = \frac{97.87K}{2} + \frac{210.6K}{1.25} = 217.4K$

Eq. 2 =  $\frac{(W_R + W_c)}{1.5} = \frac{(97.87K + 210.6K)}{1.5} = 205.6K$  controls

$\frac{93.0K}{205.6K} (100) = \boxed{45.2\%}$

Sliding

Downforce =  $(210.6K + 97.87K - 91.4K) = 217.1K$   
Friction =  $217.1K(0.4) = \underline{86.83K}$

Passive Resistance  
 $(23ft)(3.3ft)(1.7kcf) = 129Kips$

Resistance =  $\frac{(129K + 86.83K)}{1.5} = 143.9K$

$\frac{139.6K}{143.9K} = \boxed{97.0\%}$



Anchor B

$$W_{cTB} = (15ft)(15ft)(5.3ft)(0.15kcf) = 178.9k$$

$$W_{com} = (4.2ft)(7ft)(6ft)(0.15kcf) + (1.3ft)(7ft)(6ft)(0.0876kcf) = 31.24kips$$

$$W_{soil} = (2ft)(7ft)(6ft)(0.115kcf) = 9.66k$$

Uplift

$$Eq. 1 = \frac{W_R}{2.0} + \frac{W_c}{1.25} = \frac{(9.66k)}{2.0} + \frac{(178.9k + 31.24k)}{1.25} = 172.9k$$

$$Eq. 2 = \frac{(W_R + W_c)}{1.5} = \frac{(9.66k + 178.9k + 31.24k)}{1.5} = 146.5k \text{ controls}$$

$$\frac{92.9k}{146.5k} = \boxed{63.4\%}$$

Sliding

$$\text{Downforce} = 178.9k + 31.24k + 9.66k - 91.5k = 128.3k$$

$$\text{Friction} = 128.3k(0.4) = 51.32k$$

Passive Resistance

Top Block

$$(2.3ft)(15ft)(0.5ksf) = 17.25k$$

Dead Man

$$(6ft)(5.5ft)(10.75ksf) = 354.8k$$

$$\frac{(354.8k + 17.25k + 51.32k)}{1.5} = 282.2k$$

$$\frac{141.3k}{282.2k} = \boxed{50.1\%}$$





Project Name: Trumbull  
 Project #: 071092  
 Date: December 12, 2007  
 Design: MXL  
 Check: PEH  
 Page 3 of 3

Anchor C

$W_{CTB} = 178.9K$   
 $W_{COM} = 31.24K$   
 $W_{SOIL} = 9.66K$

Uplift

$E_{g1} = 172.9K$   
 $E_{g2} = 146.5K$

$\frac{91.1K}{146.5K} = \boxed{62.2\%}$

Sliding

Downforce =  $178.9K + 31.24K + 9.66K - 91.9K = 127.9K$   
 Friction =  $127.9K (0.4) = 51.16K$

Passive Resistance

Top Block  
 $(2.3ft)(15ft)(0.5Ksf) = 17.25K$

Dead Man  
 $W_{concrete\ top\ above\ grade} = 0.15Ksf (3ft) = 0.45Ksf = 450\ psf$   
 Add weight concrete to passive resistance charts

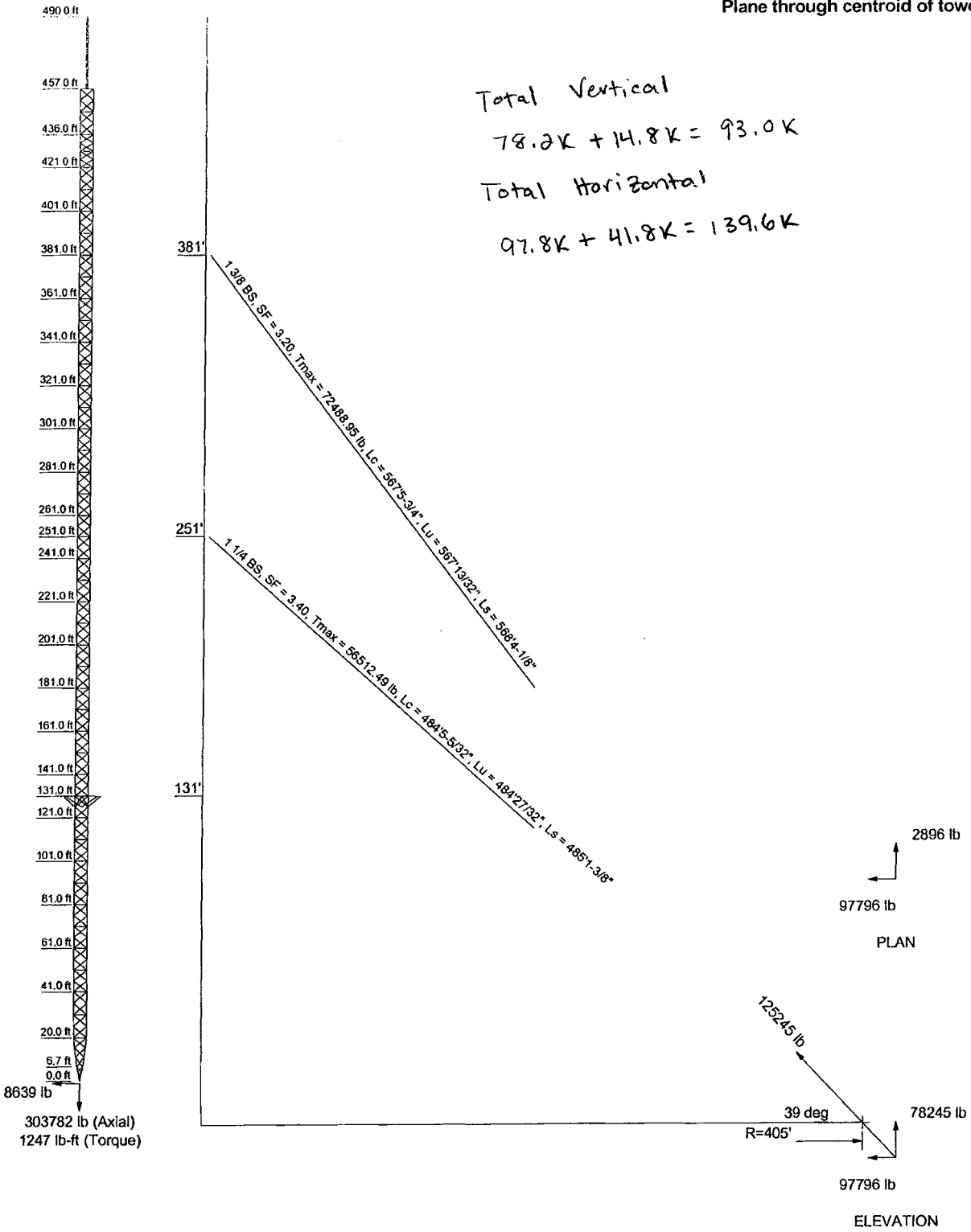
$(4.2ft)(6ft)(3.75Ksf) + (0.6ft)(6ft)(4.95Ksf) + (0.8ft)(6ft)(6.075Ksf) = 138.5\ kips$

$\frac{138.5K + 17.25K + 51.16K}{1.5} = 137.9K$

$\frac{139.7K}{137.9K} = \boxed{101.3\%}$

**Guy Tensions and Tower Reactions**  
 TIA/EIA-222-F - 85 mph/74 mph 0.5000 in Ice

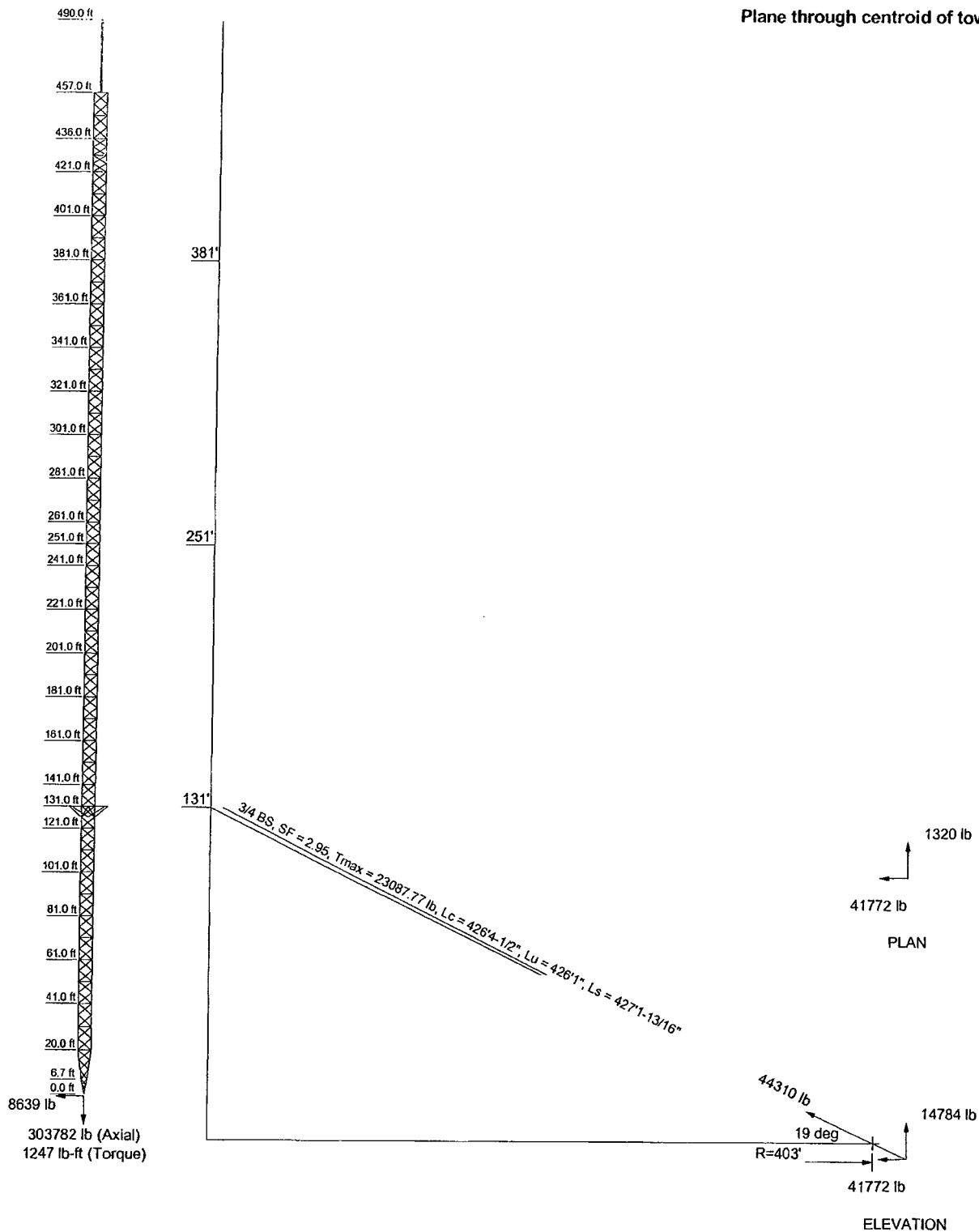
Maximum Values  
 Anchor 'A'@405 ft Azimuth 0 deg Elev -20 ft  
 Plane through centroid of tower



<b>Tower Engineering Professionals</b>		Job: <b>BU# 873128 "Trumbull"</b>	
3703 Junction Boulevard		Project: <b>TEP# 072725</b>	
Raleigh, NC 27603		Client: Crown Castle	Drawn by: Matt Lackey, E.I.
Phone: (919) 661-6351		Code: TIA/EIA-222-F	Date: 01/10/08
FAX: (919) 661-6350		Scale: NTS	
		Path: H:\2007\2725 Trumbull\Structures\RISALC\1873128.dwg	Dwg No. E-6

**Guy Tensions and Tower Reactions**  
 TIA/EIA-222-F - 85 mph/74 mph 0.5000 in Ice

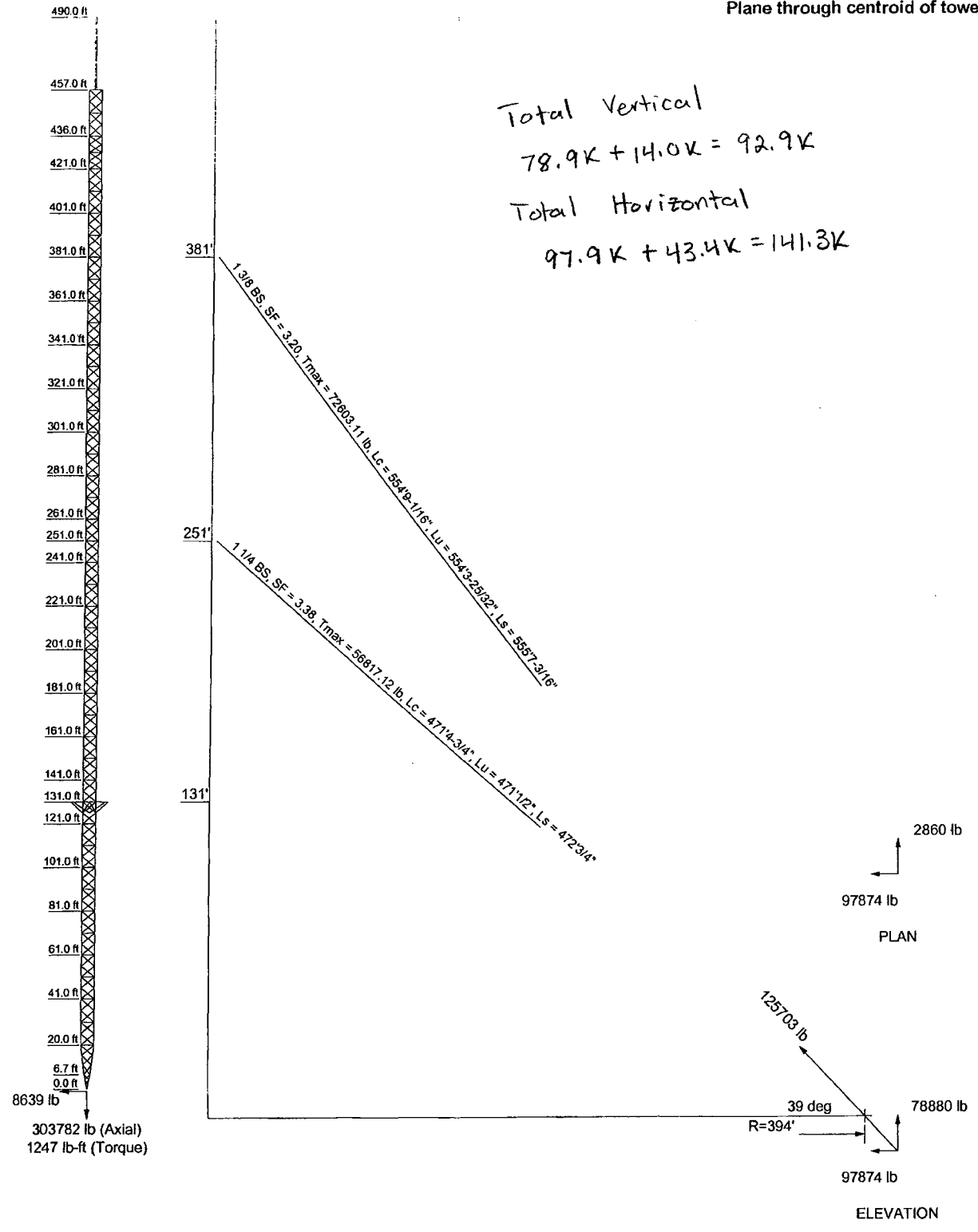
Maximum Values  
 Anchor 'A'@403 ft Azimuth 0 deg Elev -20 ft  
 Plane through centroid of tower



<b>Tower Engineering Professionals</b> 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	Job: <b>BU# 873128 "Trumbull"</b>		
	Project: <b>TEP# 072725</b>		
	Client: Crown Castle	Drawn by: Matt Lackey, E.I.	App'd:
	Code: TIA/EIA-222-F	Date: 01/10/08	Scale: NTS
	Path: H:\2007\2725 Trumbull III\Structures\RISALC1\873128.dwg		Dwg No. E-6

**Guy Tensions and Tower Reactions**  
 TIA/EIA-222-F - 85 mph/74 mph 0.5000 in Ice

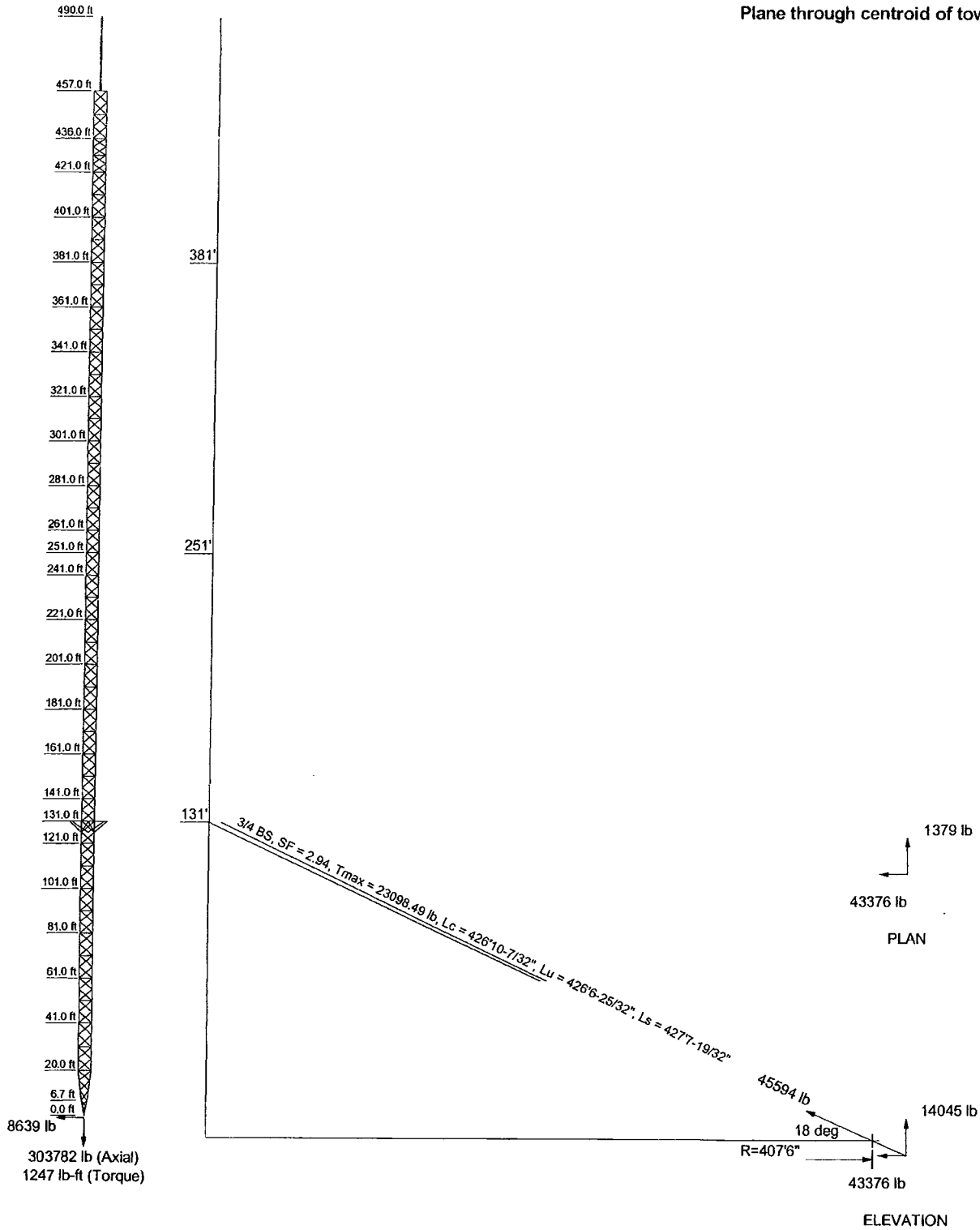
Maximum Values  
 Anchor 'B' @ 394 ft Azimuth 120 deg Elev -13 ft  
 Plane through centroid of tower



<b>Tower Engineering Professionals</b> 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	Job: <b>BU# 873128 "Trumbull"</b>		
	Project: <b>TEP# 072725</b>		
	Client: <b>Crown Castle</b>	Drawn by: <b>Matt Lackey, E.I.</b>	App'd:
	Code: <b>TIA/EIA-222-F</b>	Date: <b>01/10/08</b>	Scale: <b>NTS</b>
	Path: <b>H:\20072725 Trumbull\Structures\FISAILC\1873128.ed</b>		Dwg No. <b>E-6</b>

**Guy Tensions and Tower Reactions**  
 TIA/EIA-222-F - 85 mph/74 mph 0.5000 in Ice

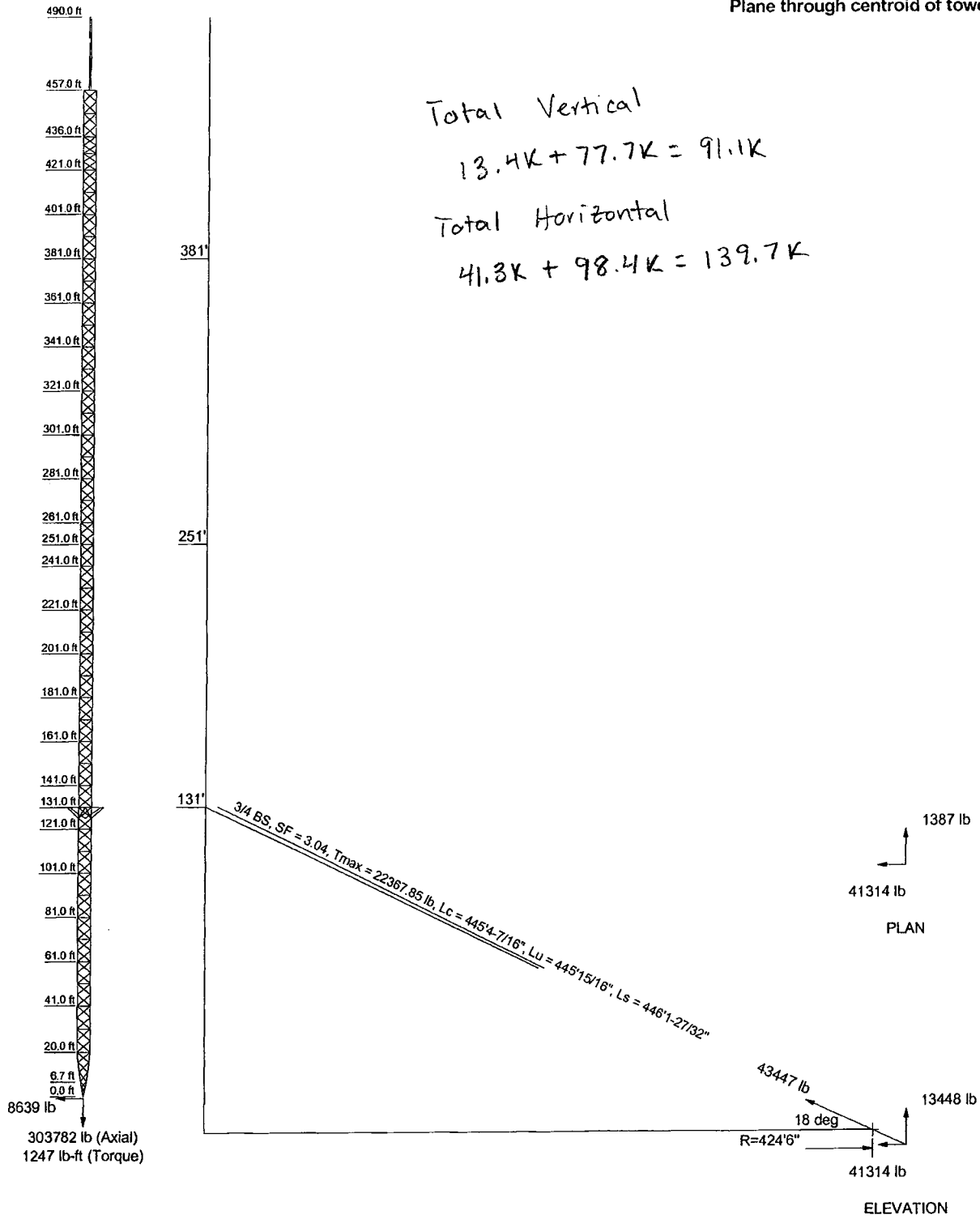
Maximum Values  
 Anchor 'B' @ 407.5 ft Azimuth 120 deg Elev -9 ft  
 Plane through centroid of tower



<b>Tower Engineering Professionals</b>		Job: <b>BU# 873128 "Trumbull"</b>	
3703 Junction Boulevard		Project: <b>TEP# 072725</b>	
Raleigh, NC 27603		Client: Crown Castle	Drawn by: Matt Lackey, E.I.
Phone: (919) 661-6351		Code: TIA/EIA-222-F	Date: 01/10/08
FAX: (919) 661-6350		Path: H:\2007\2725 Trumbull\Structures\RISALC\1873128.dwg	Scale: NTS
			Dwg No: E-6

**Guy Tensions and Tower Reactions**  
 TIA/EIA-222-F - 85 mph/74 mph 0.5000 in Ice

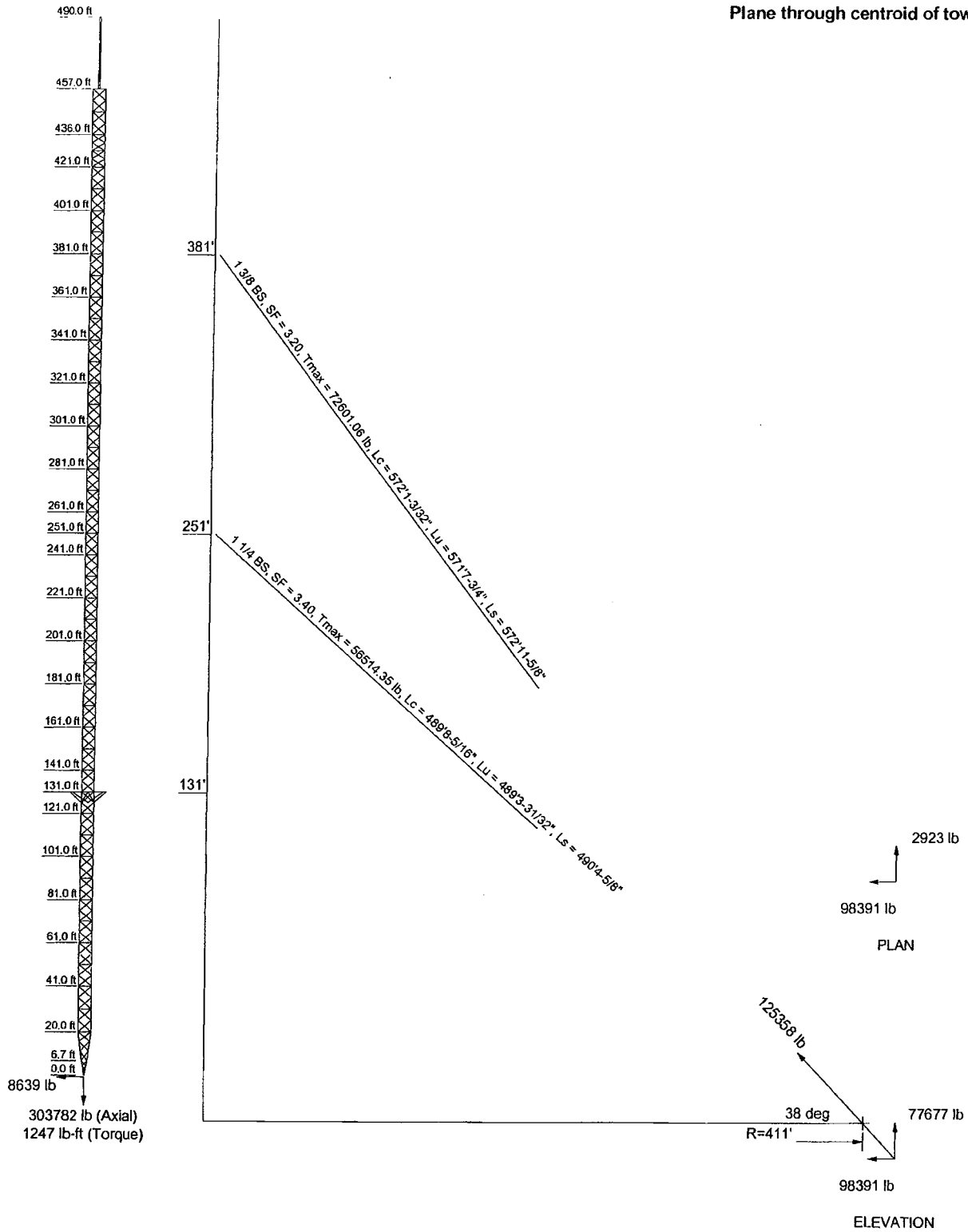
Maximum Values  
 Anchor 'C' @ 424.5 ft Azimuth 240 deg Elev -16.5 ft  
 Plane through centroid of tower



<b>Tower Engineering Professionals</b> 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	Job: <b>BU# 873128 "Trumbull"</b>		
	Project: <b>TEP# 072725</b>		
	Client: <b>Crown Castle</b>	Drawn by: <b>Matt Lackey, E.I.</b>	App'd:
	Code: <b>TIA/EIA-222-F</b>	Date: <b>01/10/08</b>	Scale: <b>NTS</b>
	Path: H:\2007\2725 Trumbull\Structures\RISALC\1873128.en		Dwg No: <b>E-6</b>

**Guy Tensions and Tower Reactions**  
 TIA/EIA-222-F - 85 mph/74 mph 0.5000 in Ice

**Maximum Values**  
 Anchor 'C'@411 ft Azimuth 240 deg Elev -20.5 ft  
 Plane through centroid of tower



<b>Tower Engineering Professionals</b> 3703 Junction Boulevard Raleigh, NC 27603 Phone: (919) 661-6351 FAX: (919) 661-6350	Job: <b>BU# 873128 "Trumbull"</b>		
	Project: <b>TEP# 072725</b>		
	Client: Crown Castle	Drawn by: Matt Lackey, E.I.	App'd:
	Code: TIA/EIA-222-F	Date: 01/10/08	Scale: NTS
	Path: H:\20072725 Trumbull III\Structure\RISALC1\873128.eri		Dwg No. E-6