

EM-SPRINT-142-130109

497 Old Post Road

Tolland



RECEIVED
JUL 10 2014

1 Robbins Road
Westford, MA 01886

CONNECTICUT
SITING COUNCIL

July 9, 2014

State of Connecticut
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: Notification of Construction Completion on telecommunication facilities

To whom it may concern:

Alcatel Lucent hereby acknowledges that the list of attached sites have completed construction per the approval granted on the specified date. Please advise if further information is needed..

Very truly yours,

Martha Powers

Martha Powers
Lead Development Manager
Alcatel-Lucent
Sprint Vision Project
1 Robbins Road
Westford, MA 01886

Cc: FST, Siterra

EM/TS #	Address	Town	Sprint ID	Decision Date
EM-SPRINT-062-130912	1065 Wintergreen Avenue	Hamden	CT03XC003	10/15/2013
EM-SPRINT-NEXTEL-060-130118	10 Tanner Marsh Road	Guilford	CT03XC022	2/14/2013
EM-SPRINT-004-130822	181 Montevideo Road	Avon	CT03XC053	9/6/2013
EM-SPRINT-NEXTEL-155-130214A	1358 New Britain Ave.	West Hartford	CT03XC057	3/1/2013
EM-SPRINT-NEXTEL-164-130201	440 Hayden Station Road	Windsor	CT03XC065	3/8/2013
EM-SPRINT-NEXTEL-132-130201	59 McGuire Road	South Windsor	CT03XC066	3/1/2013
EM-SPRINT-NEXTEL-054-130201	299 Paxton Way	Glastonbury	CT03XC081	3/1/2013
EM-SPRINT-NEXTEL-094-130214E	36 Prospect Street	Newington	CT03XC084	3/1/2013
EM-SPRINT-110-130725	10 Sparks Street	Plainville	CT03XC086	8/8/2013
EM-SPRINT-007-130314	260 Beckley Road	Kensington	CT03XC088	4/5/2013
EM-SPRINT-NEXTEL-155-130201	570 New Park Avenue	West Hartford	CT03XC091	3/1/2013
EM-SPRINT-NEXTEL-106-130201	430 Middlesex Turnpike	Old Saybrook	CT03XC102	3/1/2013
EM-SPRINT-NEXTEL-105-130201	30 Short Hills Road	Old Lyme	CT03XC104	3/1/2013
EM-SPRINT-NEXTEL-152-130201	41 Manitock Hill Road	Waterford	CT03XC105	3/1/2013
EM-SPRINT-NEXTEL-045-130201	93 Roxbury Road	East Lyme	CT03XC110	3/1/2013
EM-SPRINT-152-130114	45R Fargo Road	Waterford	CT03XC112	2/14/2013
EM-SPRINT-NEXTEL-027-130201	48 Cow Hill Road	Clinton	CT03XC156	3/1/2013
EM-SPRINT-NEXTEL-082-130201	238 Meridan Road	Middlefield	CT03XC160	3/8/2013
EM-SPRINT-047-130109	160 Plantation Road	East Windsor	CT03XC202	2/7/2013
EM-SPRINT-NEXTEL-077-130214	53 Slater Street	Manchester	CT03XC211	3/1/2013
EM-SPRINT-142-130109	497 Old Post Road	Tolland	CT03XC212	2/7/2013
EM-SPRINT-NEXTEL-042-130222	94 East High Street	East Hampton	CT03XC335	3/8/2013
EM-SPRINT-057-121226	Butternut Hollow Road	Greenwich	CT03XC343	1/11/2013
EM-SPRINT-158-130213	515 Boston Post Road	Westport	CT03XC355	3/1/2013
EM-SPRINT-046-130402	206 Everett Road	Easton	CT03XC362	4/19/2013
EM-SPRINT-085-130322	474 MAIN STREET	MONROE	CT03XC365	4/5/2013
EM-SPRINT-086-131011	57 Cook Drive	Montville	CT03XC365	10/25/2013
EM-SPRINT-118-130322	76 EAST RIDGE	RIDGEFIELD	CT03XC370	4/5/2013
EM-SPRINT-097-131230	20 Barnabas Road	Newtown	CT03XC383	1/21/2014
EM-SPRINT-051-130207	3965 Congress Street	Fairfield	CT03XC385	3/1/2013
EM-SPRINT-NEXTEL-094-130214A	123 Costello Road	Newington	CT23XC555	3/1/2013
EM-SPRINT-119-131008	699 Old Main Street	Rocky Hill	CT23XC556	10/25/2013
EM-SPRINT-077-131008	60 Adams Street	Manchester	CT23XC557	10/25/2013
EM-SPRINT-NEXTEL-080-130123	462 West Main Street	Meriden	CT25XC840	2/14/2013
EM-SPRINT-096-130920	18 Hilltop View Lane	New Milford	CT33XC095	10/4/2013
EM-SPRINT-157-130213	237 Godfrey Road	Weston	CT33XC522	3/1/2013
EM-SPRINT-018-131008	20 Vale Road	Brookfield	CT33XC525	10/25/2013
EM-SPRINT-077-130528	595 Keeney Street	Manchester	CT33XC538	6/14/2013
EM-SPRINT-NEXTEL-129-130214	400 Main Street	Somers	CT33XC554	3/1/2013
EM-SPRINT-047-130322	15 CHAMBERLAIN	BROADBROOK	CT33XC565	4/5/2013
EM-SPRINT-004-130502	277 Huckleberry Road	Avon	CT33XC589	5/17/2013

EM-SPRINT-143-130604	218 Wheeler Road	Torrington	CT33XC592	6/28/2013
EM-SPRINT-140-130724	583 Chapel Street	Thomaston	CT33XC603	8/8/2013
EM-SPRINT-103-130920	Charles Marshall Drive	Norwalk	CT33XC802	10/4/2013
EM-SPRINT-NEXTEL-064-130214	439-455 Homestead Ave.	Hartford	CT43XC805	3/1/2013
EM-SPRINT-064-130311	99 Meadow Street	Hartford	CT43XC806	4/5/2013
EM-SPRINT-083-131127	290 Preston Ave.	Middletown	CT43XC816	12/16/2013
EM-SPRINT-128-130920	530 Bushy Hill Road	Simsbury	CT43XC825	10/4/2013
EM-SPRINT-164-130405A	340 Bloomfield Avenue	Windsor	CT43XC826	4/19/2013
EM-SPRINT-077-130109	239 Middle Turnpike	Manchester	CT43XC827	2/13/2013
EM-SPRINT-165-130118	2-4 Volunteer Drive	Windsor Locks	CT43XC828	2/14/2013
EM-SPRINT-NEXTEL-139-130214	44 Fyler Place	Suffield	CT43XC829	3/8/2013
EM-SPRINT-111-130712	171 Town Hill Road	Plymouth	CT54XC712	7/26/2013
EM-SPRINT-009-130322	38 Spring Hill Road	Bethel	CT54XC749	4/5/2013
EM-SPRINT-154-131011	315 Spencer Plains Road	Westbrook	CT54XC758	10/25/2013
EM-SPRINT-023-130405	14 Canton Springs Road	Canton	CT54XC760	4/19/2013
EM-SPRINT-104-130606	153 Old Salem Road	Norwich	CT54XC775	6/28/2013
EM-SPRINT-164-130405B	99 Day Hill Road	Windsor	CT54XC787	4/19/2013
EM-SPRINT-132-130920	300 Governor's Highway	South Windsor	CT60XC014	10/4/2013
EM-SPRINT-094-130108	605 Willard Avenue	Newington	CT60XC018	1/25/2013
EM-SPRINT-146-130506	197 South Street	Vernon	CT60XC935	5/24/2013
EM-SPRINT-146-130311	777 Talcottville Road	Vernon	CT70XC147	4/5/2013
EM-SPRINT-126-130531	62 Birdseye Road	Shelton	CT73XC004	6/21/2013



STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@ct.gov

www.ct.gov/csc

February 7, 2013

Patricia Masterson
Site Acquisition Manager
Goodman Networks
Two Willow Street, Suite 101
Southborough, MA 01745

RE: **EM-SPRINT-142-130109** – Sprint Spectrum L.P. notice of intent to modify an existing telecommunications facility located at 497 Old Post Road, Tolland, Connecticut.

Dear Ms. Masterson:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

- Any deviation from the proposed modification as specified in this notice and supporting materials with Council shall render this acknowledgement invalid;
- Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
- Within 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- The validity of this action shall expire one year from the date of this letter; and
- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration;

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated January 9, 2013. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

Very truly yours,

Linda Roberts
Executive Director

LR/CDM/jb

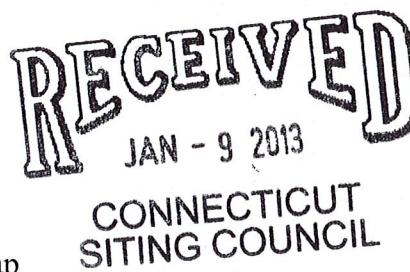
c: The Honorable Jack Scavone, Chairman Town Council, Town of Tolland
Steven R. Werbner, Town Manager, Town of Tolland
Linda Farmer, Town Planner, Town of Tolland
Old Post Road Holdings





January 9, 2013

Linda Roberts
Executive Director
Connecticut Siting Counsel
Ten Franklin Square
New Britain, CT 06051
Linda Roberts, Executive Director



Re: Notice of Exempt Modification – Antenna Swap
497 Old Post Road, Tolland, Connecticut

Dear Ms. Roberts:

Sprint is planning to consolidate multiple network technologies into one seamless network with the goal of increasing efficiency and enhancing network coverage, call quality and data speeds for customers across Connecticut. Pursuant §16-50j-73 to of the Regulations of Connecticut State Agencies (RCSA), please accept this letter and attachments as notification of Sprint's intent to make exempt modifications, under RCSA §16-50j-72(b)(2), to its existing telecommunications facility at 497 Old Post Road, Tolland, Connecticut. In accordance with RCSA §16-50j-73, a copy of this letter was sent to Jack Scavone, Chair, Tolland Town Council.

Sprint currently maintains six (6) antennas at 147 feet on the existing 150 foot tower at the address referenced above. Sprint intends to replace its existing six (6) CDMA antennas with three (3) Multimodal antennas at their same current height of 147 feet. Sprint will a be replacing its existing six (6) lines of coaxial cable with three (3) smaller lines of Hybriflex cable and installing six (6) RRH's. Sprint will also be swapping three (3) existing ground cabinets with three (3) new cabinets and adding one (1) fiber junction box. This work will result in a net reduction of antennas, from six (6) to three (3), and will not increase the height of the tower or the size compound. Please find included with this letter compound, elevation and overhead drawings which depict Sprint's proposed modifications.

Sprint's planned modifications fall squarely within the activities permitted in RCSA §16-50j-72(b)(2) in that:

1. The proposed modifications will not increase the existing tower height;
2. The proposed modifications will not extend the boundaries of the site by any dimension;

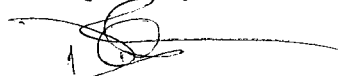
3. The proposed modifications will not increase the noise levels at the existing facility by six (6) decibels or more;
4. The proposed modifications will not increase the total radio frequency electromagnetic radiation power density to or above the standards adopted by the Federal Communications Commission. Please find included with this letter a Radio Frequency Emissions Analysis Report.

Also included with this letter is a Structural Assessment confirming that the foundation and tower are sufficient to support Sprint's proposed modifications.

For the foregoing reasons, Sprint respectfully submits that its proposed modifications to the existing tower located at the address referenced above constitute an exempt modification under RSCA §16-50j-72(b)(2).

Please do not hesitate to contact me at (214) 478-3516 or dtorres@goodmannetworks.com if you have any questions. Thank you for your consideration.

Respectfully,



David Torres
Goodman Networks

Attachments

Copy to:

Jack Scavone, Chair, Tolland Town Council

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT
EVALUATION OF HUMAN EXPOSURE POTENTIAL
TO NON-IONIZING EMISSIONS

Sprint Existing Facility

Site ID: CT03XC212

BANM Tower
497 Old Post Road
Tolland, CT 06084

October 19, 2012

October 19, 2012

Sprint
Attn: RF Engineering Manager
1 International Boulevard, Suite 800
Mahwah, NJ 07495

Re: Emissions Values for Site: CT03XC212 – BANM Tower

EBI Consulting was directed to analyze the proposed upgrades to the existing Sprint facility located at 497 Old Post Road, Tolland, CT, for the purpose of determining whether the emissions from the proposed Sprint equipment upgrades on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limit for the cellular band is approximately $567 \mu\text{W}/\text{cm}^2$, and the general population exposure limit for the PCS band is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed upgrades to the existing Sprint Wireless antenna facility located at 497 Old Post Road, Tolland, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario. Actual values seen from this site will be dramatically less than those shown in this report. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all emissions were calculated using the following assumptions:

- 1) 3 CDMA Carriers (1900 MHz) were considered for each sector of the proposed installation.
- 2) 1 CDMA Carrier (850 MHz) was considered for each sector of the proposed installation
- 3) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 4) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The actual gain in this direction was used per the manufactures supplied specifications.
- 5) The antenna used in this modeling is the APXVSP18-C-A20. This is based on feedback from the carrier with regards to anticipated antenna selection. This antenna has a 15.9 dBd gain value at its main lobe at 1900 MHz and 13.4 dBd at its main lobe for 850 MHz. All calculations were performed assuming the main lobe of the antenna was focused at the base of the tower to present a worst case scenario.

- 6) The antenna mounting height centerline of the proposed antennas is **147 feet** above ground level (AGL)
- 7) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public threshold limits

Site ID	CT03K212 - BANIM Tower
Site Address	497 Old Post Road, Tolland, CT, 06084
Site Type	Guyed Tower

Antenna Number	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	Power Out Per Channel (Watts)	Number of Channels	Composite Power	Antenna Gain in direction of sample point (dBd)	Antenna Height (ft)	Antenna analysis height (ft)	Antenna Height (Meters)	Cable Loss (dB)	Additional Loss	ERP	Power Density Value	Power Density Percentage
1a	RFS	APXVSP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	3	60	15.9	147	141	43	0.5	0	2080.4211	37.62002	3.76200%
1a	RFS	APXVSP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	147	141	43	0.5	0	385.96892	7.051764	1.24370%
Sector total Power Density Value: 5.006%																	
2a	RFS	APXVSP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	3	60	15.9	147	141	43	0.5	0	2080.4211	37.62002	3.76200%
2a	RFS	APXVSP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	147	141	43	0.5	0	385.96892	7.051764	1.24370%
Sector total Power Density Value: 5.006%																	
3a	RFS	APXVSP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	3	60	15.9	147	141	43	0.5	0	2080.4211	37.62002	3.76200%
3a	RFS	APXVSP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	13.4	147	141	43	0.5	0	385.96892	7.051764	1.24370%
Sector total Power Density Value: 5.006%																	

Site Composite MPE %	
Carrier	MPE %
Sprint	15.017%
NE Paging	0.540%
Hamden Commons	3.310%
Conn Radio Rocky Hill	4.810%
Air Touch	0.630%
Verizon Wireless	37.940%
Pocket	5.150%
AT&T	32.270%
Total Site MPE %	
99.667%	

Summary

All calculations performed for this analysis yielded results that were well within the allowable limits for general public exposure to RF Emissions.

The anticipated Maximum Composite contributions from the Sprint facility are **15.017% (5.006% from each sector)** of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level.

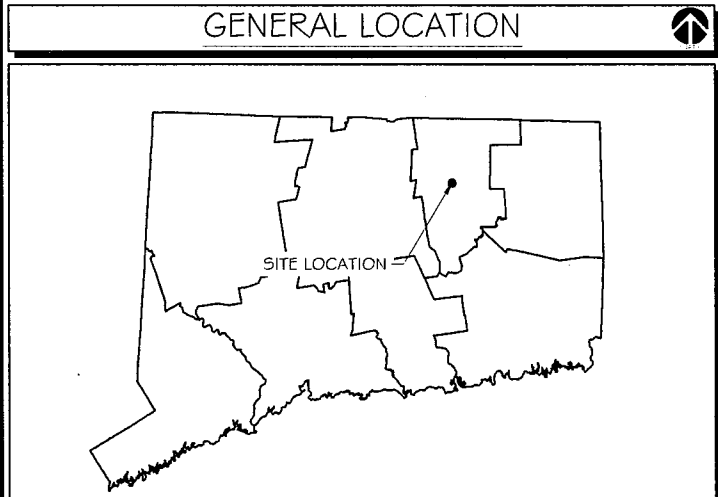
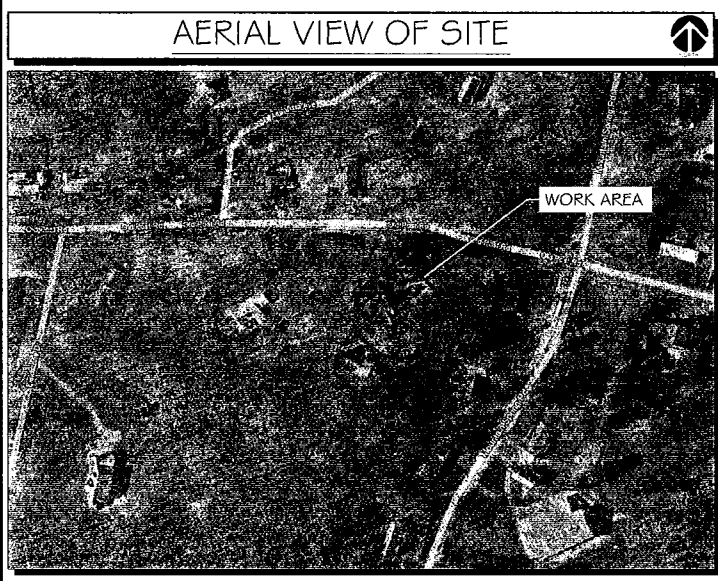
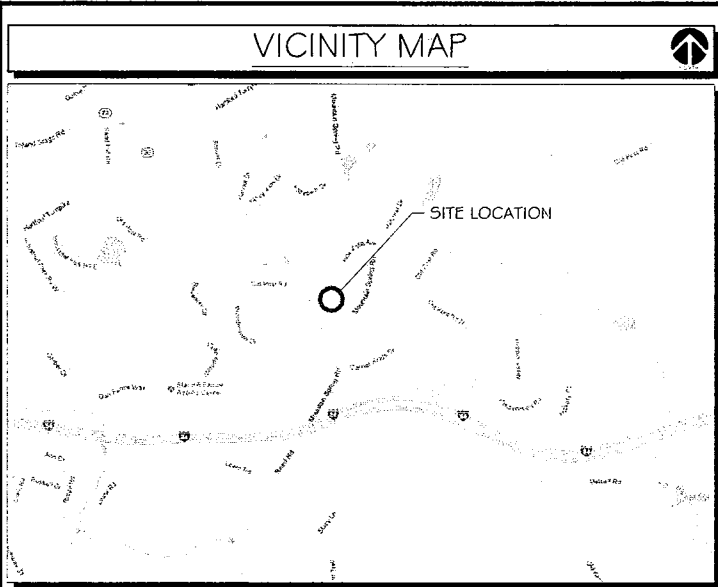
The anticipated composite MPE value for this site assuming all carriers present is **99.667%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government



Scott Heffernan
RF Engineering Director

EBI Consulting
21 B Street
Burlington, MA 01803



DRIVING DIRECTIONS:
 I-91 NORTH TO I-84 EAST. TAKE EXIT 67 (ROUTE 31). TAKE A RIGHT OF THE EXIT AND GO 1/2 MILE AND TAKE A LEFT ONTO REED ROAD. GO APPROX 1 MILE AND GO OVER THE I-84 OVERPASS. TAKE A LEFT ONTO OLD POST ROAD. TAKE THE FIRST DRIVEWAY ON THE LEFT, NUMBER 497.

CODE COMPLIANCE

ALL WORK AND MATERIALS SHALL BE PERFORMED AND INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL COVERING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES.

- INTERNATIONAL BUILDING CODE 2009
- ACCESSIBILITY CODE IBC 2009, CHAPTER 11 & ICC/ANSI A117.1-2003
- 2008 NATIONAL ELECTRIC CODE
- FIRE/LIFE SAFETY CODE- IFC 2009
- ENERGY CODE IECC 2009

PROJECT NOTES

- THIS IS AN UNMANNED TELECOMMUNICATIONS FACILITY CONSISTING OF BTS EQUIPMENT AND ANTENNAS.
- SIGNALS FROM THE ANTENNA SHALL NOT INTERFERE WITH ANY EXISTING COMMUNICATION SITES. ALL ITEMS SHOWN HEREON ARE EXISTING UNLESS OTHERWISE NOTED.
- THE PROPOSED ANTENNAS ARE ATTACHED TO EITHER BUILDING OR ANTENNA FRAME OR TO BOTH.
- THE PROPOSED WORK WILL HAVE NO EFFECT ON STRUCTURAL STABILITY. ALL WORK SHALL BE PERFORMED IN STRICT ADHERENCE WITH OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION REGULATIONS.
- REFERENCE SPRINT STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES FOR GENERAL REQUIREMENTS.
- THIS IS AN UNMANNED FACILITY. NO SOLID WASTE. THE SITE WILL CREATE NO TRASH, THUS REQUIRES NO DUMPSTER.
- EQUIPMENT IS UNMANNED AND NOT FOR HUMAN HABITATION. HANDICAP ACCESS IS THEREFORE NOT REQUIRED.
- OWNER & TENANT MAY, FROM TIME TO TIME AT TENANT'S OPTION, REPLACE THIS EXHIBIT WITH AN EXHIBIT SETTING FORTH THE LEGAL DESCRIPTION OF THE SITE, OR WITH ENGINEERED OR AS-BUILT DRAWING DEPICTING THE SITE OR ILLUSTRATING STRUCTURAL MODIFICATIONS OR CONSTRUCTION PLANS OF THE SITE. ANY VISUAL OR TEXTUAL REPRESENTATION OF THE EQUIPMENT LOCATED WITHIN THE SITE CONTAINED IN THESE OTHER DOCUMENTS IS ILLUSTRATIVE ONLY, AND DOES NOT LIMIT THE RIGHTS OF SPRINT AS PROVIDED FOR IN THE AGREEMENT. THE LOCATIONS OF ANY ACCESS AND UTILITY EASEMENTS ARE ILLUSTRATIVE ONLY. ACTUAL LOCATIONS MAY BE DETERMINED BY TENANT AND/OR THE SERVICING UTILITY COMPANY IN COMPLIANCE WITH LOCAL LAWS AND REGULATIONS.

PROJECT DESCRIPTION

APPLICANT PROPOSED TO INSTALL ANTENNAS AND WEATHERPROOF EQUIPMENT CABINETS FOR AN UNMANNED PERSONAL COMMUNICATIONS SYSTEM WIRELESS CALL SITE AT AN EXISTING TELECOMMUNICATIONS FACILITY. PROPOSED FACILITY IS NOT STAFFED AND IS VISITED ONCE A MONTH FOR MAINTENANCE PURPOSES ONLY; THEREFORE, SANITARY, SEWER, GAS, POTABLE WATER AND PLUMBING ARE NOT REQUIRED.

MEMBER OF THE NATIONAL UNDERGROUND UTILITY COUNCIL

TO OBTAIN LOCATION OF PARTICIPANTS' UNDERGROUND FACILITIES BEFORE YOU DIG IN CONNECTICUT
CALL BEFORE YOU DIG 811 OR 1-800-922-4455

CONNECTICUT PUBLIC ACT 87-71 REQUIRES MIN. 2 WORKING DAYS NOTICE BEFORE YOU EXCAVATE.

DO NOT SCALE DRAWINGS:

CONTRACTOR SHALL VERIFY ALL PLANS & EXISTING DIMENSIONS & CONDITIONS ON THE JOB SITE & SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

APPROVALS

CONSTRUCTION PROJECT MANAGER: _____

SITE ACQUISITION: _____

SPRINT REPRESENTATIVE: _____

RF ENGINEER: _____

LANDLORD/ OWNER: _____

CONSTRUCTION DRAWINGS

Sprint

BANM TOWER CT03XC212

497 OLD POST ROAD
TOLLAND, CT 06084
TOLLAND COUNTY
GUYED TOWER

SHEET INDEX

GENERAL:		STRUCTURAL:	
T-1	TITLE SHEET	S-1	STRUCTURAL DETAILS
SP-1	SPECIFICATIONS	UTILITY & GROUNDING:	
SP-2	SPECIFICATIONS	E-1	UTILITY & GROUNDING SITE PLAN & NOTES
SP-3	SPECIFICATIONS	E-2	UTILITY DETAILS
SITE:		E-3	GROUNDING DETAILS & NOTES
C-1	OVERALL SITE PLAN	E-4	GROUNDING DETAILS
A-1	EQUIPMENT PLAN	E-5	GROUNDING DETAILS
A-2	SITE ELEVATION & NOTES		
A-3	ANTENNA DETAILS & COAX SCHEDULE		
A-4	ANTENNA PLUMBING DIAGRAM & SPECIFICATIONS		
A-5	RF INFORMATION & COAX COLOR CODING		
A-6	EQUIPMENT DETAILS & SPECIFICATIONS		
A-7	EQUIPMENT DETAILS & SPECIFICATIONS		

PROJECT INFORMATION

APPLICANT ID:	HOSPITAL:
SITE NAME: BANM TOWER	ROCKVILLE GENERAL HOSPITAL
SITE #: CT03XC212	31 UNION STREET
PROPERTY LANDLORD:	VERNON ROCKVILLE, CT 06066
OLD POST ROAD HOLDINGS, LLC	PH: (860) 872-0501
4564 BERSAGLIO STREET	FIRE HOUSE:
LAS VEGAS, NV 89135	VERNON FIRE DEPARTMENT STATION 4
SITE ADDRESS:	30 HYE STREET
497 OLD POST ROAD	VERNON ROCKVILLE, CT 06066
TOLLAND, CT 06084	PH.: (860) 875-2543
TOLLAND COUNTY	APPLICANT:
ZONING CLASSIFICATION: R-2A RESIDENCE	SPRINT
SITE DATA:	6391 SPRINT PARKWAY
LATITUDE: 41°51'38.52" N (41.8607°)	OVERLAND PARK, KS 66251
LONGITUDE: 72°24'12.29" W (-72.4034°)	PLANS PREPARED BY:
GROUND ELEVATION: 915 FT AMSL	RAMAKER & ASSOCIATES, INC.
POWER COMPANY:	1120 DALLAS STREET
CONNECTICUT LIGHT & POWER	SAUK CITY, WI 53583
PH.: (800) 286-2000	CONTACT: KEITH BOHNSACK, P.E., PROJECT MANAGER
TELEPHONE COMPANY:	PH.: (608) 643-4100
AT&T	FAX: (608) 643-7999
PH.: (800) 288-2020	

Sprint

6391 Sprint Parkway
Overland Park, KS 66251

Alcatel-Lucent

RAMAKER & ASSOCIATES, INC.

1120 Dallas Street, Sauk City, WI 53583
 Phone: 608-643-4100 Fax: 608-643-7999
 www.Ramaker.com

**NETWORK VISION
MMBTS LAUNCH
NORTHERN CT MARKET**

Certification & Seal:
I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Licensed Professional Engineer under the laws of the State of Connecticut.

James R. Skowronski
Signature: _____ Date: 1/08/2013

C	1/08/13	FINAL CDS ISSUED
B	10/26/12	FINAL PRELIM CDS
A	10/03/12	90% CD REVIEW
MARK	DATE	DESCRIPTION
ISSUE	FINAL	DATE ISSUED 01/08/2013
PROJECT TITLE:		
BANM TOWER SITE#: CT03XC212		
PROJECT INFORMATION: 497 OLD POST ROAD TOLLAND, CT 06084 TOLLAND COUNTY		
SHEET TITLE: TITLE SHEET		
SCALE: NONE		
PROJECT NUMBER	23002	
SHEET NUMBER	T-1	

DIVISION 1 - GENERAL REQUIREMENTS
SECTION 01100 - SCOPE OF WORK
PART 1 - GENERAL
1.1 THE WORK: These Standard Construction Specifications in conjunction with the other Contract Documents and the Construction Drawings describe the Work to be performed by the Contractor.
1.3 PRECEDENCE: Should conflicts occur between the Standard Construction Specifications for Wireless Sites including the Standard Construction Details for Wireless Sites and the Construction Drawings, information on the Construction Drawings shall take precedence. Notify Company designated representative of conflicts prior to construction.
1.4 NATIONALLY RECOGNIZED CODES AND STANDARDS:
A. The Work shall comply with applicable national codes and standards, latest edition, and portions thereof, included but not limited to the following:
1. GR-63-CORE NEBS Requirements: Physical Protection
2. GR-78-CORE Generic Requirements for the Physical Design and Manufacture of Telecommunications Equipment.
3. National Fire Protection Association Codes and Standards (NFPA) including NFPA 70 (National Electrical Code - "NEC") and NFPA 101 (Life Safety Code).
4. American Society for Testing of Materials (ASTM)
5. Institute of Electronic and Electrical Engineers (IEEE)
6. American Concrete Institute (ACI)
7. American Wire Producers Association (AWPA)
8. Concrete Reinforcing Steel Institute (CRSI)
9. American Association of State Highway and Transportation Officials (AASHTO)
10. Portland Cement Association (PCA)
11. National Concrete Masonry Association (NCMA)
12. Brick Industry Association (BIA)
13. American Welding Society (AWS)
14. National Roofing Contractors Association (NRCA)
15. Sheet Metal and Air Conditioning Contractors' National Association (SMACNA)
16. Door and Hardware Institute (DHI)
17. Occupational Safety and Health Act (OSHA)
18. Applicable building codes including Uniform Building Code, Southern Building Code, BOCA, and the International Building Code.

SECTION 01300 - CELL SITE CONSTRUCTION
3.1 GENERAL REQUIREMENTS FOR CIVIL CONSTRUCTION:
A. Contractor shall keep the site free from accumulating waste material, debris, and trash. At the completion of the work, Contractor shall remove from the site all remaining rubbish, implements, temporary facilities, and surplus materials.
B. Equipment rooms shall at all times be maintained "broom clean" and clear of debris.
C. Contractor shall take all reasonable precautions to discover and locate any Hazardous Condition.
1. In the event Contractor encounters any hazardous condition which has not been abated or otherwise mitigated, Contractor and all other persons shall immediately stop Work in the affected area and notify Company in writing. The Work in the affected area shall not be resumed except by written notification by Company.
2. Contractor agrees to use care while on the Site and shall not take any action that will or may result in or cause the hazardous condition to be further released in the environment, or to further expose individuals to the hazard.
D. Contractor's activities shall be restricted to the project limits. Should areas outside the project limits be affected by Contractor's activities, Contractor shall immediately return them to original condition
E. Conduct testing as required herein.

DIVISION 2 - SITE CONSTRUCTION
SECTION 02300 - EARTHWORK
PART 3 - EXECUTION
3.4 TRENCHING AND BACKFILLING: The Contractor shall perform all excavation of every description and of whatever substances encountered, to the depths indicated on the Construction Drawings or as otherwise specified.
A. Protection of Existing Utilities: The Contractor shall check with the local utilities and the respective utility locator companies prior to starting excavation operations in each respective area to ascertain the locations of known utility lines. The locations, number and types of existing utility lines detailed on the Construction Drawings are approximate and do not represent exact information. The Contractor shall be responsible for repairing all lines damaged during excavation and all associated operations. All utility lines uncovered during the excavation operations, shall be protected from damage during excavation and associated operations. All repairs shall be approved by the utility company.
B. Hand Digging: Unless approved in writing otherwise, all digging within an existing cell site compound is to be done by hand.
C. During excavation, material suitable for backfilling shall be stockpiled in an orderly manner a sufficient distance from the banks of the trench to avoid

overloading and to prevent slides or cave-ins. All excavated materials not required or suitable for backfill shall be removed and disposed of at the Contractor's expense.
D. Grading shall be done as may be necessary to prevent surface water from flowing into trenches or other excavations, and any water accumulating therein shall be removed by pumping or by other approved method.
E. Sheeting and shoring shall be done as necessary for the protection of the work and for the safety of personnel. Unless otherwise indicated, excavation shall be by open cut, except that short sections of a trench may be tunneled if, the conduit can be safely and properly installed and backfill can be properly tamped in such tunnel sections. Earth excavation shall comprise all materials and shall include clay, silt, sand, muck, gravel, hardpan, loose shale, and loose stone.
F. Trenches shall be of necessary width for the proper laying of the conduit or cable, and the banks shall be as nearly vertical as practicable. The bottom of the trenches shall be accurately graded to provide uniform bearing and support for each section of the conduit or cable on undisturbed soil at every point along its entire length. Except where rock is encountered, care shall be taken not to excavate below the depths indicated. Where rock excavations are necessary, the rock shall be excavated to a minimum over depth of 6 inches below the trench depths indicated on the Construction Drawings or specified. Over depths in the rock excavation and unauthorized over depths shall be thoroughly back filled and tamped to the appropriate grade. Whenever wet or otherwise unstable soil that is incapable of properly supporting the conduit or cable is encountered in the bottom of the trench, such soil shall be removed to a minimum over depth of 6 inches and the trench backfilled to the proper grade with earth of other suitable material, as hereinafter specified.
G. Backfilling of Trenches. Trenches shall not be backfilled until all specified tests have been performed and accepted. Where compacted backfill is not indicated the trenches shall be carefully backfilled with select material such as excavated soils that are free of roots, sod, rubbish or stones, deposited in 6 inch layers and thoroughly and carefully rammed until the conduit or cable has a cover of not less than 1 foot. The remainder of the backfill material shall be granular in nature and shall not contain roots, sod, rubbish, or stones of 2-1/2 inch maximum dimension. Backfill shall be carefully placed in the trench and in 1 foot layers and each layer tamped. Settling the backfill with water will be permitted. The surface shall be graded to a reasonable uniformity and the mounding over the trenches left in a uniform and neat condition.
H. Except as otherwise required, compacted backfill shall be used under concrete pads, walkways, concrete paving, and asphalt concrete paving. The first 1 foot cover shall be of select materials such as excavated soils that are free of roots, sod, rubbish, or stones. The Company may reject any onsite or borrow materials which are considered unsuitable for the intended use of the fill.
I. All fills shall be compacted to a dry density equal to at least 90 percent of the maximum dry density determined in accordance with ASTM D1557. The maximum density and optimum moisture content shall be determined by the Contractor on basis of laboratory tests conducted on the materials used in the fill.
J. Adequacy of compaction shall be determined on the basis of in-place density determinations that shall be conducted by the Contractor while the fills are being placed. The results of these tests shall be the basis on which satisfactory completion of the work is judged. If the fills fail to meet the specified densities, the Contractor shall remove and recompact the soils until the specified densities are achieved.

3.6 REMOVAL OF WATER: The Contractor shall provide and maintain adequate dewatering equipment to remove and dispose of all surface and ground water entering excavations and other parts of the work. Each excavation shall be kept dry during sub-grade preparation and continually thereafter until the construction to be provided therein is completed to the extent that no damage from hydrostatic pressure, flotation, or other cause will result. Ground water level shall be maintained at least 12 inches below the bottom of each excavation. Removal of water shall be in accordance with all state, federal, and local regulations. Contractor shall submit water removal plan to the Company.
3.10 UNAUTHORIZED EXCAVATION: Except where otherwise authorized, indicated, or specified, all material excavated below the bottom of concrete structures which will be supported by the sub-grade shall be replaced with concrete placed monolithic with the concrete above. Material excavated below structures supported on piers shall be replaced with approved material. The material shall be compacted to a density equal to or greater than the density of the adjacent undisturbed soil.
3.11 STRUCTURE EXCAVATION: Excavation for structures shall be done to lines and elevations indicated on the Construction Drawings and to the limits required to perform the construction work.
A. Excavated materials free of trash, rocks, roots, and other foreign materials, and which meet the specified requirements, may be used as required for the fills, embankments, and backfills constructed under these specifications.
3.12 STABILIZATION: Sub-grades for structures and the bottom of trenches shall be firm, dense, and thoroughly compacted.
A. Trench sub-grades which run beneath roads, or pass through structural backfill, shall be compacted to 95 percent of maximum density as determined by ASTM D1557.
B. Sub-grades for structures and trench bottoms which are otherwise solid, but which become soft on top due to construction operations, shall be reinforced with one or more layers of crushed rock or gravel.

3.13 STRUCTURE BACKFILL: Backfill around and outside of structures shall be deposited in layers not to exceed 6 inches in uncompacted thickness and mechanically compacted, using acceptable compaction techniques, to at least 95 percent of maximum density as determined by ASTM D1557, with a moisture content of plus or minus 3 percent of optimum, as determined by ASTM D698 when that test is appropriate, or to 70 percent relative density as determined by ASTM D4253 and D4254 when those tests are appropriate. Compaction of structure backfill by rolling will be permitted provided the desired compaction is obtained and damage to the structure is prevented. Compaction of structure backfill by inundation with water will not be permitted.
A. Material for structure backfill shall be composed of earth only and shall contain no wood, grass, roots, broken concrete, stones, trash, or debris of any kind.
B. No backfill shall be deposited or compacted in water.

C. All backfill material shall consist of loose earth having a moisture content such that the required density of the compacted soil will be obtained with the compaction method used. Moisture content shall be distributed uniformly, and water for correction of moisture content shall be added sufficiently in advance so proper moisture distribution and compaction will be obtained. Granular material shall be wet, not just damp, when compacted.
D. Particular care shall be taken to compact structure backfill which will be beneath pipes, drives, roads, or other surface construction or structures. In addition, wherever a trench will pass through structure backfill, the structure backfill shall be placed and compacted to an elevation at least 12 inches above the top of the pipe before the trench is excavated.
3.18 DISPOSITION OF MATERIALS: Excess excavated earth and construction material shall be removed from the job site and legally disposed of by the Contractor.

DIVISION 3 - CONCRETE
SECTION 03300 - CAST-IN-PLACE CONCRETE
PART 1 - GENERAL
Contact engineer or construction manager for complete concrete specifications if such work is required.
SECTION 03600 - GROUT
PART 1 - GENERAL
Contact engineer or construction manager for complete grout specifications if such work is required.
DIVISION 5 - METALS

SECTION 05120 - ICE BRIDGE AND OTHER STRUCTURAL STEEL
PART 2 - PRODUCTS
2.1 ICE BRIDGE MATERIALS:
A. Ice Bridge posts shall be fabricated of 3-inch schedule 40 galvanized steel, ASTM A-53, Grade B (seamless). Posts shall be installed a minimum of 3 feet 6 inches below finish grade and backfilled with 3000 p.s.i concrete. Post tops shall be capped with steel pipe caps. Maximum horizontal separation between posts shall be 8 feet on center.
B. Ice Bridge material shall be McNichols "Grip Stru" 10 diamond plank, 24 inches wide and 3 inches deep; part number 103014 or approved equal.
C. Ice Bridge components shall be hot dip galvanized and connected in an electrically continuous fashion per the manufacturer's recommendations. Any site penetrations or saw cuts to galvanized metal shall be treated with two coats of a zinc rich cold galvanizing paint as per ASTM A 780 standards.

2.2 STRUCTURAL STEEL MATERIALS: Conform to the latest edition of applicable standards and to all applicable codes and requirements of local authorities having jurisdiction, whichever is more stringent. All structural steel shall be in accordance with the latest applicable requirements of AISC, ASTM, ACI, CRSI, AWS and all other applicable standards.
2.3 All steel shall be galvanized in accordance with ASTM A36 unless noted on the construction drawings.
2.4 Rolled steel shapes, plates and bars shall be no less than 3/16 inches in thickness and shall comply with ASTM A-36 as a minimum.
2.5 Steel pipe shall comply with ASTM A-501 or ASTM A-53, Type E or S, Grade B, A-500 Grade B steel may be substituted.
2.6 Steel tube shall comply with ASTM A-500, Grade B.
2.7 Galvanized steel grating shall be a minimum 3/4 inch x 1/8 inch at 3/16 inches on center.
2.8 Galvanized checkered plate shall be a minimum 3/16 inch.

PART 3 - EXECUTION
3.1 ICE BRIDGE:
A. The Contractor is responsible for installing an Ice Bridge and support posts between the BTS radio equipment and the tower. At no point shall the Ice Bridge structure be mechanically connected to the tower. Cabling supports shall be designed to accept snap-in type hangers and accommodate a minimum coax or waveguide bending radius of 20 inches.
B. Each tier shall be vertically and horizontally aligned with the cable entry ports on the shelter consisting of three tiers capable of holding 5 runs of 1-5/8 inch coaxial cable each for a total of 15 coaxial cables. The cover shall be aligned to allow for easy access to the cabling and be of sufficient width and durability to prevent damage to the cable that might otherwise be caused by falling ice, bolts, nuts or hand tools. The entire structure, including cover, shall be sufficiently rigid to prevent cable damage caused by movement of the structure. Cover shall continue to within 2 inches of shelter wall and waveguide ladder.
C. Provision shall be made to ground the Ice Bridge structure as specified in Division 16.

3.2 STRUCTURAL STEEL FABRICATION: All shop fabrication and assembly of structural steel shall be in accordance with AISC specifications and as indicated on the approved shop drawings. All materials shall be properly marked for field assembly and for identification as to the location for which it is intended. Materials shall be fabricated and delivered in an order to expedite erection and minimize field handling of materials.
3.3 WELDING:
A. Welding shall be performed by a certified welder and shall conform to requirements for shielded metal arc welding of the Standard Code for Arc and Gas Welding of the American Welding Society (AWS D1.1)
B. Electrodes shall comply with AWS Code and shall be classified E-70 electrodes as a minimum. Where finishing is required, complete the assembly, including welding of units, before the start of finishing operations. Provide finish surfaces of exposed members that are free from markings, burrs and other defects.
C. Welded construction shall comply with AWS Code for procedures, appearance and quality of welds and methods used in correcting welded work. Assemble and weld

built-up sections by methods that will produce correct dimensions without warp.
3.4 CONNECTIONS:
A. Contractor shall provide all hardware required to complete field erection of structure as indicated by Contract Documents or these specifications.
B. High strength threaded fasteners shall be installed in accordance with AISC Specifications for Structural Joints Using ASTM A-325 or A-490 Bolts. Use A-325N bearing-type connection bolts unless noted otherwise.
C. Grating and plates shall be fastened with saddle clips. The necessary holes to complete all phases of construction shall be provided and called out on the approved shop drawings. All holes shall be drilled or punched perpendicular to metal surfaces, flame cut or burned holes will not be permitted.
D. All unfinished threaded fasteners shall comply with ASTM A-307, Grade A, regular low-carbon steel bolts and nuts with hexagonal heads.
E. All high strength threaded fasteners shall be heavy hexagonal bolts and nuts with hardened washers, all from quenched and tempered medium carbon steel complying with ASTM A-325.
3.5 REPAIR: Repair all damaged galvanized steel with "Galvanox," "Dry Galv," or "Zinc-It," or approved equal, per the manufacturer's instructions.

SECTION 07500 - ROOF CUTTING, PATCHING AND REPAIR
PART 1 - GENERAL
1.4 SUBMITTALS:
A. Pre-Construction Roof Condition Analysis Reports: Complete a roof inspection and report prior to the installation of Sprint equipment on any rooftop build requiring roof penetration. At a minimum inspect all areas impacted by the addition of the Sprint equipment.
Roof inspection reports should be uploaded into SMS using task # 234.
B. New Roofing Material Product Data: Submit manufacturer's product data and installation instructions for each material and product used.
C. Shop Drawings: Provide large scale shop drawings for installation of all parts of the work. Provide plans, and details of seams, connections and accessory items. Show layouts of tapered insulation and locations of drains. Show interfaces and relationships to work of other trades.
D. Certification and Warranty:
1. Upon completion of work of this Section, submit certification by existing roof manufacturer acknowledging that all work performed is acceptable and that the entire roof remains under warranty.
2. Maintain existing warranty if applicable. Take no action which would void existing warranty.

PART 2 - PRODUCTS
2.1 MATERIALS:
A. Existing Roof Materials and Compatibility: Furnish specific product acceptable to manufacturer of roofing membrane which will not compromise the roofing manufacturer's warranty.
B. Substrate Board: Glass-mat, water-resistant gypsum ASTM C 1177 or match existing.
C. Vapor Retarder: Match existing.
D. Insulation: Extruded polystyrene board insulation, or match existing.
E. Tapered Insulation: Fabricated to provide proper drainage.
F. Recovery Board over Insulation: Match existing, mechanically fastened.
G. Membrane and Flashing: Match existing.
H. Sheet Metal Accessories: Follow SMACNA and NRCA recommendations. Materials and finishes to match existing.
I. Ballast: Match existing.
J. Walkway Protection Board: Compatible with membrane.

PART 3 - EXECUTION
3.1 INSTALLATION:
A. Inspect substrate and report unsatisfactory conditions in writing. Beginning work on site means Contractor's acceptance of existing roof conditions.
B. Comply with roof system manufacturer's instructions and recommendations on any penetrations, repairs, etc.
C. Install insulation with tightly butted joints and neatly fitted around penetrations.
D. Where applicable, distribute ballast uniformly to 10 pounds per square foot or more as required by Factory Mutual. Obtain approval of ballast weight from the building owner before loading roof.
E. Install walkway protection over an additional layer of membrane at locations indicated and where required to provide access to roof mounted equipment.
F. Restore or replace damaged components. Protect work from damage.

SECTION 07840 - FIRESTOPPING
PART 1 - GENERAL
1.5 QUALITY ASSURANCE:
A. Comply with governing codes and regulations. Provide products of acceptable manufacturers which have been in satisfactory use in similar service for three years. Use experienced installers. Deliver, handle, and store materials in accordance with manufacturer's instructions.
B. Fire Performance: ASTM E 119, ASTM E 814, and local regulations.

Sprint

6391 Sprint Parkway
Overland Park, KS 66251

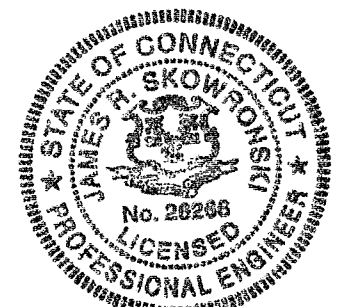
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**NETWORK VISION
MMBTS LAUNCH
NORTHERN CT MARKET**

Certification & Seal:
I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Licensed Professional Engineer under the laws of the State of Connecticut.



Signature: James H. Skowronski Date: 1/08/2013

MARK	DATE	DESCRIPTION
C	1/08/13	FINAL CD'S ISSUED
B	1/02/12	FINAL PRELIM CD'S
A	1/03/12	90% CD REVIEW

ISSUE PHASE: FINAL DATE ISSUED: 01/08/2013

PROJECT TITLE:
**BANM TOWER
SITE#: CT03XC212**

PROJECT INFORMATION:
497 OLD POST ROAD
TOLLAND, CT 06084
TOLLAND COUNTY

SHEET TITLE:
SPECIFICATIONS

SCALE: NONE

PROJECT NUMBER	SHEET NUMBER
23002	SP-1

- G. Support miscellaneous electrical components as required to produce the same structural safety factors as specified for raceway supports. Install metal channel racks for mounting cabinets, panelboards, disconnects, control enclosures, pull boxes, junction boxes, transformers and other devices.
- H. In open overhead spaces, cast boxes threaded to raceways need not be supported separately except where used for fixture support. Support sheet metal boxes directly from the building structure or by bar hangers. Where bar hangers are used, attach the bar to raceways on opposite sides of the box and support the raceway with a listed type of fastener not more than 24" (600 mm) from the box.
- I. Install conduit sealing fittings for conduit penetrations of concrete wall exterior or below grade as specified or required by code.
- J. Unless otherwise indicated on the drawings, fasten electrical items and their supporting hardware securely to the structure in accordance with the following:
 1. Fasten by means of wood screws on wood,
 2. Toggle bolts on hollow masonry units,
 3. Concrete inserts or expansion bolts on concrete or solid masonry,
 4. Machine screws, welded threaded studs, or spring-tension clamps on steel,
 5. Explosive devices for attaching hangers to structure shall not be permitted.
 6. Do not weld conduit, pipe straps, or items other than threaded studs to steel structures.
 7. In partitions of light steel construction, use sheet metal screws.
- K. Ensure that the load applied by any fastener does not exceed 25 percent of the proof test load.
- L. Use vibration and shock-resistant fasteners for attachments to concrete slabs.

SECTION 16001 - ELECTRICAL MATERIALS AND EQUIPMENT

PART 2 - PRODUCTS

2.1 DISCONNECT SWITCHES:

- A. Furnish and install externally operated, quick-make, quick-break, safety, fused and non-fused heavy duty disconnect switches where shown on the drawings and where required by NEC. Switches shall be safety type as manufactured by Square "D", I-T-E, Cutler-Hammer/Westinghouse, GE, or approved equal.
- B. Switches shall be rated for horsepower of motors controlled. Indoor switches shall be mounted in NEMA 1 enclosures, except as indicated. Switches located exterior to building shall be mounted in NEMA 3R enclosures except as indicated. Switches utilized as service entrance equipment shall be so labeled.
- C. Disconnect switches shall be provided at all equipment.
- D. Furnish Class R fuse kits for all fused switches utilizing RK-1 or RK-5 fuses.

2.2 CIRCUIT BREAKERS FOR INSTALLATION INTO PANELBOARDS:

- A. For application in panelboards, provide circuit breakers of the same manufacturer as the Original Equipment Manufacturer (OEM) panel, integral to the cabinet.
- B. Circuit breaker configuration (bolt-on or clip-on) shall match that of breakers installed and shipped with the cabinet.
- C. Amps Interrupting Capacity (AIC) of field supplied and installed circuit breakers shall not be less than the printed withstand and interrupting rating of the load center.

2.3 SEPARATELY ENCLOSED CIRCUIT BREAKERS:

- A. Furnish and install where indicated molded case circuit breakers, trip indicating, trip free, thermal magnetic type with electrical characteristics and ratings as indicated. Short circuit withstand and interrupting rating shall be as required by the fault current indicated.
- B. Provide NEMA 1 enclosures indoor, NEMA 3R outdoor enclosure except as otherwise indicated. Circuit breaker handles shall be lockable in the OFF position.
- C. Provide service entrance label where indicated.
- D. Provide equipment by Square "D", General Electric, Siemens, or Cutler-Hammer/Westinghouse.

2.7 CABLE TRAY:

- A. Furnish and install a complete cable tray system as indicated on the drawings and as manufactured by B-Line Systems, Inc., Square "D" Company or approved equal.
- B. Cable tray, fittings and accessories shall be steel, hot-dipped galvanized after fabrication or aluminum as indicated.
- C. Cable tray shall be ladder-type, trough-type, channel-type, or as indicated.
- D. Cable tray system shall be furnished with all dimensions, covers, necessary tees, crosses, risers, elbows, connectors, hangers, etc. of same material as cable tray and as shown on drawings and as required by cable tray manufacturer.
- E. Barriers shall be installed in cable tray to separate cables of different systems such as low and high voltage, telephone, data, etc. Barriers shall be of same material as cable tray.
- F. Cable tray shall be installed level and, plumb in accordance with manufacturer's instructions.

2.9 COMMUNICATION CABLING FOR CELL SITE T1 CIRCUITS:

- A. This specification applies to the T1 circuit to be installed by this Contractor between the Network Interface Unit (NIU) and the Company radio equipment.
- B. In indoor locations and in underground conduits in dry climates cabling shall be PVC-insulated tinned solid copper 24 - 24 AWG twisted pairs, UL Type CMR, with overall braided shield and PVC jacket, except as otherwise recommended by the manufacturer.
- C. In underground conduits in wet climates, provide Outdoor plant cable, gel filled,

24 - 24AWG twisted pairs.

D. Exception: In all cases for installations in Lucent BTS markets, utilize the T1 cable shipped with the BTS, whenever the cable length is sufficient for the installation.

E. Adhere to Bellcore standards for cable color coding.

2.12 GROUNDING ELECTRODES AND CONDUCTORS:

- A. Comply with Exhibit C - Cell Site Grounding Design.
- B. Equipment Grounding Conductor:
 1. Bare copper conductor or insulated green wire ground as specified herein.

2.13 BOXES AND COVERS:

- A. Pull and junction boxes shall be sized in accordance with NEC requirements and shall be installed so that the conductors in them are accessible without removing any part of the structure.
- B. Interior switch and outlet boxes flush mounted in finished areas shall be code gauge pressed plated steel, Midland Ross or approved equal, suitable for the device to be installed. Covers shall be as hereinafter specified in paragraph "Device Plates in Finished Areas."
- C. Device and pull boxes surface-mounted above accessible ceilings and within unfinished enclosed Mechanical rooms shall be as specified above sized for the conductors within and shall have pressed plated steel screw attached covers.
- D. Interior switch, pull, junction and outlet boxes surface mounted in unfinished industrial areas shall be (cast aluminum or) plated cast alloy, threaded, suitable for the device to be installed, Crouse-Hinds FS/FD series or approved equal. Covers shall be screw attached plated iron alloy suitable for the box and device. Switch plate covers shall be "guarded" style.
- E. Pull boxes exterior to the building and in interior industrial areas shall be plated cast alloy, heavy duty, weatherproof, dust proof, with gasket, plated iron alloy cover and stainless steel cover screws, Crouse-Hinds WAB series or equal.
- F. Conduit outlet bodies shall be plated cast alloy with similar gasketed covers. Outlet bodies shall be of the configuration and size suitable for the application. Provide Crouse-Hinds Form 8 or equal.
- G. Exterior switch and outlet boxes shall be recessed mounted except as noted, cast aluminum or plated cast alloy with wet location, Crouse-Hinds series WLRD covers, or equal. Masonry boxes mounted recessed in exterior wall shall be furnished with weatherproof covers.
- H. Manufacturer for boxes and covers shall be Hoffman, Square "D", Crouse-Hinds, Cooper, Adale, Appleton, O-Z Gedney, Raco, or approved equal.

2.21 LIGHTNING PROTECTION:

- A. Comply with the latest revisions of Exhibit D - Cell Site Lightning - Surge Protection and Exhibit C - Cell Site Grounding Design.

2.26 SURGE SUPPRESSION

- A. Except as otherwise required, surge suppression devices are Company furnished materials

PART 3 - EXECUTION

3.1 GROUNDING:

- A. Electrical services, circuits and systems, enclosures and equipment shall be grounded in accordance with Article 250 of the National Electrical Code.
- B. Grounding shall be provided as indicated for feeder, branch circuit, control, and instrument circuits.
- C. Equipment Grounding Conductor: Furnish and install a separate insulated green wire grounding conductor with circuit conductors for all feeders and branch circuits.
- D. Furnish and install an insulated green wire grounding conductor in non-metallic raceways unless designated otherwise for telephone or data cables.
- E. Telephone and communication system services, circuits, enclosures and equipment shall be grounded in accordance with paragraph 800-33 and paragraph 800-40 of the National Electrical Code.
- F. Separately derived AC systems that are required to be grounded by the NEC shall be grounded in accordance with paragraph 250-26 of the NEC.

G. Furnish and install insulated copper ground conductors in conduit from main electrical service equipment or electrical room ground bus and connect to main metallic water service entrance (if available) with ground clamps. Connect ground conductor to the street side of water main where a dielectric main water fitting is installed.

H. Furnish and install ground fault protection where required by code and as required by the specifications and drawings. Installation of ground fault protection shall be in accordance with NEC.

3.3 CONDUIT AND CONDUCTOR INSTALLATION:

- A. Conduit and conductors shall be sized as required by NEC and shall be installed continuous and complete from outlet to outlet, panels and junction boxes.
 1. In order to closely follow the lines of the structure, maintain close proximity to the structure and keep conduits in tight envelopes. Changes in direction to route around obstacles shall be made with conduit outlet bodies in exposed locations except as otherwise indicated, and in accordance with good construction practice.
 2. Other changes in direction shall be made with trade elbows, keeping conduits grouped in tight envelopes following the lines of the structure and maintaining close proximity to the structure except as otherwise indicated, and in accordance with good construction practice.
 3. Route conduits according to the envelopes, areas, details and sections, if any, identified on the drawings.
- B. Conduits shall be fastened securely in place with approved non-perforated straps

and hangers. Explosive devices for attaching hangers to structure will not be permitted. Conduits shall be concealed in finished areas. Conduit shall be exposed in unfinished areas.

C. Conduit shall be installed in a neat and workmanlike manner, parallel and perpendicular to structure wall and ceiling lines. Conduit shall be installed as required by the design of the structure and placed in concrete forms so as not to interfere with reinforcing or strength of slabs, joists or beams. Conduit shall clear all pipes and ducts and depressions in floors. Permission of Engineer shall be obtained as to location of conduit in reinforced concrete slabs, joists and beams.

D. All conduit shall be fished to clear obstructions. Ends of conduits shall be temporarily capped to prevent concrete, plaster or dirt from entering.

E. Conduits shall be rigidly clamped to boxes by galvanized malleable iron bushing on inside and galvanized malleable iron locknut on outside and inside.

F. EMT conduits (if allowed) shall have approved EMT threaded type box connectors and couplings. Set screw connectors and couplings shall not be acceptable.

G. Conductors shall be pulled in accordance with accepted good practice. Where more than one conductor is installed in the same conduit all conductors within the conduit shall be pulled simultaneously. Pull shall not deform conductors. Approved type lubricant may be used in pulling conductors where required.

H. Splices and taps shall be kept to a minimum and made in accordance with the NEC.

I. Where conduit crosses an expansion joint, an expansion and deflection fitting shall be installed in the conduit.

J. Conduit Entrance Seals:

1. All conduits penetrating new concrete walls exterior or below grade shall be sealed at penetrations with conduit entrance seal, Type FSK by O-Z/Gedney or approved equal.
2. All conduit penetrating existing concrete walls exterior or below grade shall be sealed on both sides with O-Z/Gedney Type CSML seals.

K. Conduits and cables passing through all floors, fire rated walls, and smoke partitions shall be sealed in accordance with NEC-300-21. Furnish and install O-Z/Gedney fire seal fittings or approved equal at those locations. At the Contractor's option, Specified Technologies Inc. Model PEN200, UL-Listed fire resistant silicone foam sealant installed in accordance with the manufacturer's recommendations may be utilized. All unused openings and sleeves shall be sealed as herein specified.

L. A #16 gauge (1.3 mm²) steel pull wire shall be left in all empty conduits.

M. PVC conduits shall be installed using fittings, solvents, glues, and methodology as recommended by the manufacturer.

N. Provide adequate length of conductors within electrical enclosures and train the conductors to terminal points with no excess. Do not bend conductors sharper than eight times the cable outside diameter. Make terminations so there is no bare conductor at the terminal. Bundle multiple conductors, with conductors larger than No. 10 AWG in individual circuit bundles.

O. Tighten electrical connectors and terminals, including screws and bolts, in accordance with the manufacturer's published torque tightening values. Where manufacturer's torquing requirements are not indicated, tighten connectors and terminals to comply with tightening torques specified in UL 486A and 486B.

P. Utilize flexible liquid tight conduit for final connection in exterior, damp wet, or corrosive locations, and elsewhere as indicated on the drawings.

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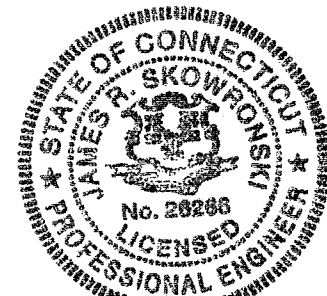
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Signature: *James R. Skowronski* Date: 1/08/2013

MARK	DATE	DESCRIPTION
C	1/08/13	FINAL CDS ISSUED
B	1/02/12	FINAL PRELIM CDS
A	10/03/12	90% CD REVIEW

ISSUE PHASE: FINAL DATE ISSUED: 01/08/2013

PROJECT TITLE:
**BANM TOWER
SITE#: CT03XC212**

PROJECT INFORMATION:
497 OLD POST ROAD
TOLLAND, CT 06084
TOLLAND COUNTY

SHEET TITLE:
SPECIFICATIONS

SCALE: NONE

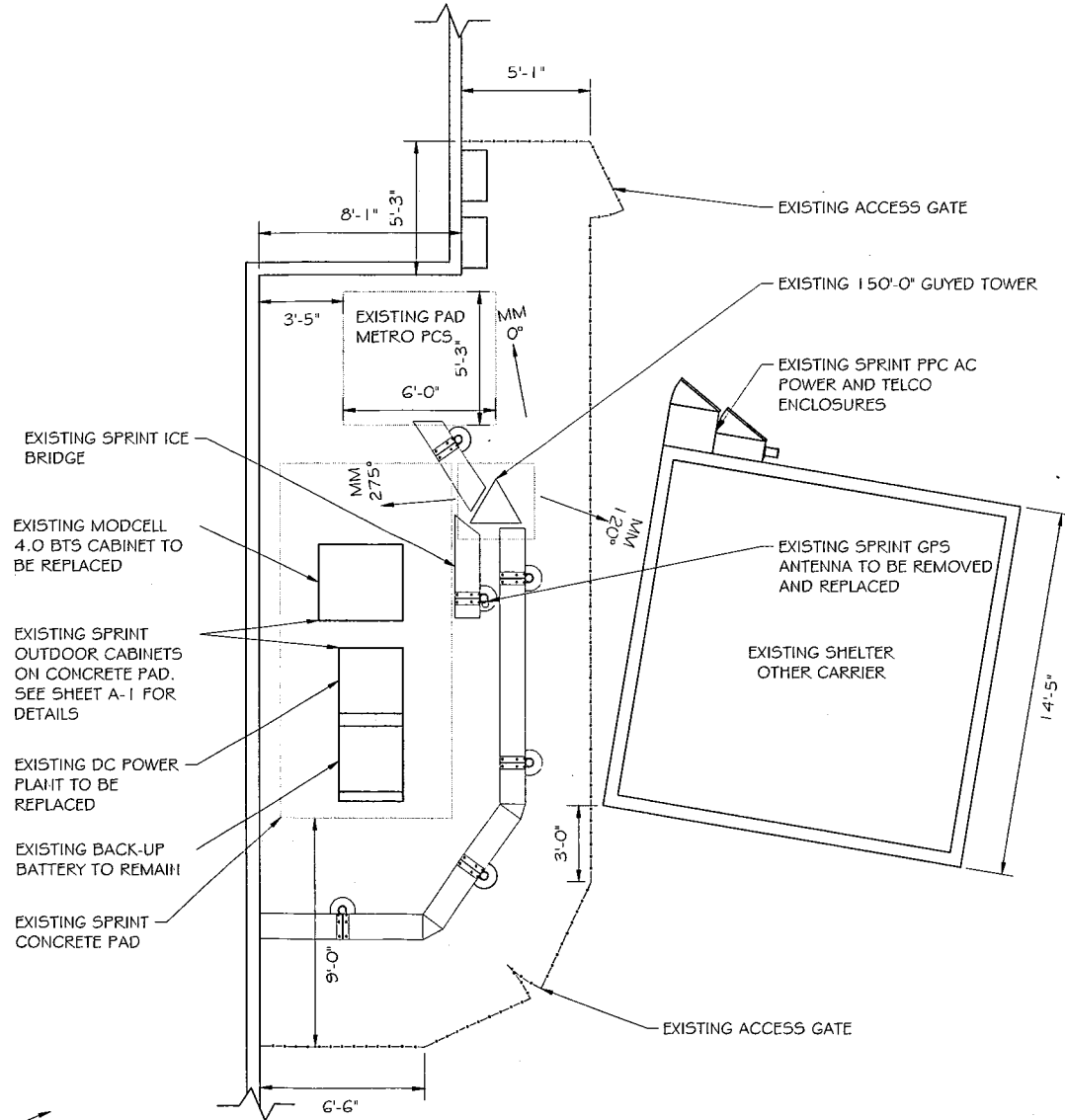
PROJECT NUMBER	23002
SHEET NUMBER	SP-3

VICINITY MAP



GENERAL NOTES:

1. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE CODES, ORDINANCES, LAWS, AND REGULATIONS OF ALL MUNICIPALITIES, UTILITIES COMPANY, OR OTHER PUBLIC AUTHORITIES.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS THAT MAY BE REQUIRED BY ANY FEDERAL, STATE, COUNTY, OR MUNICIPAL AUTHORITIES.
3. THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER, IN WRITING, OF ANY CONFLICTS, ERRORS OR OMISSIONS PRIOR TO THE SUBMISSION OF BIDS OR PERFORMANCE OF WORK. MINOR OMISSIONS OR ERRORS IN THE BID DOCUMENTS SHALL NOT RELIEVE THE CONTRACTOR FROM RESPONSIBILITY FOR THE OVERALL INTENT OF THESE DRAWINGS.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL EXISTING SITE IMPROVEMENTS PRIOR TO COMMENCING CONSTRUCTION. THE CONTRACTOR SHALL REPAIR ANY DAMAGE CAUSED AS A RESULT OF CONSTRUCTION OF THE FACILITY.
5. THE SCOPE OF WORK FOR THIS PROJECT SHALL INCLUDE PROVIDING ALL MATERIALS, EQUIPMENT, AND LABOR REQUIRED TO COMPLETE THIS PROJECT. ALL EQUIPMENT SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
6. THE CONTRACTOR SHALL VISIT THE PROJECT SITE PRIOR TO SUBMITTING A BID TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
7. CONTRACTOR SHALL VERIFY ANTENNA ELEVATION AND AZIMUTH WITH RF ENGINEERING PRIOR TO INSTALLATION.
8. TRANSMITTER EQUIPMENT AND ANTENNAS ARE DESIGNED TO MEET ANSI/EIA/TIA 222-G REQUIREMENTS.
9. ALL STRUCTURAL ELEMENTS SHALL BE HOT DIPPED GALVANIZED STEEL.
10. CONTRACTOR SHALL MAKE A UTILITY "ONE-CALL" TO LOCATE ALL UTILITIES PRIOR TO EXCAVATING.
11. IF ANY UNDERGROUND UTILITIES OR STRUCTURES EXIST BENEATH THE PROJECT AREA, CONTRACTOR MUST LOCATE IT AND CONTACT THE APPLICANT & THE OWNER'S REPRESENTATIVE.
12. OCCUPANCY IS LIMITED TO PERIODIC MAINTENANCE AND INSPECTION BY TECHNICIANS APPROXIMATELY 2 TIMES PER MONTH.
13. RAMAKER & ASSOCIATES HAS NOT PERFORMED A STRUCTURAL ANALYSIS FOR THIS PROJECT. PRIOR TO THE INSTALLATION OF THE PROPOSED EQUIPMENT OR MODIFICATION OF THE EXISTING STRUCTURE, A STRUCTURAL ANALYSIS SHALL BE PERFORMED BY SPRINT'S AGENT TO CERTIFY THAT THE EXISTING/PROPOSED COMMUNICATION STRUCTURE AND COMPONENTS ARE STRUCTURALLY ADEQUATE TO SUPPORT ALL EXISTING AND PROPOSED ANTENNAS, COAXIAL CABLES, AND OTHER APPURTENANCES.
14. PROPERTY LINE INFORMATION WAS PREPARED USING DEEDS, TAX MAPS, AND PLANS OF RECORD AND SHOULD NOT BE CONSTRUED AS AN ACCURATE BOUNDARY SURVEY.
15. THIS PLAN IS SUBJECT TO ALL EASEMENTS AND RESTRICTIONS OF RECORD.
16. THE PROPOSED FACILITY WILL CAUSE ONLY A "DE MINIMIS" INCREASE IN STORMWATER RUNOFF; THEREFORE, NO DRAINAGE STRUCTURES ARE PROPOSED.
17. NO SIGNIFICANT NOISE, SMOKE, DUST, OR ODOR WILL RESULT FROM THIS FACILITY.
18. THE FACILITY IS UNMANNED AND NOT INTENDED FOR HUMAN HABITATION (NO HANDICAP ACCESS REQUIRED).
19. POWER TO THE FACILITY WILL BE MONITORED BY A SEPARATE METER.



EXISTING HOUSE

SITE PLAN

SCALE: 1" = 7.5'



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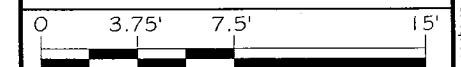
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B	10/26/12	FINAL PRELIM CDS
A	10/03/12	90% CD REVIEW

ISSUE PHASE: FINAL DATE ISSUED: 01/08/2013

PROJECT TITLE:
BANM TOWER
SITE#: CT03XC212

PROJECT INFORMATION:
 497 OLD POST ROAD
 TOLLAND, CT 06084
 TOLLAND COUNTY

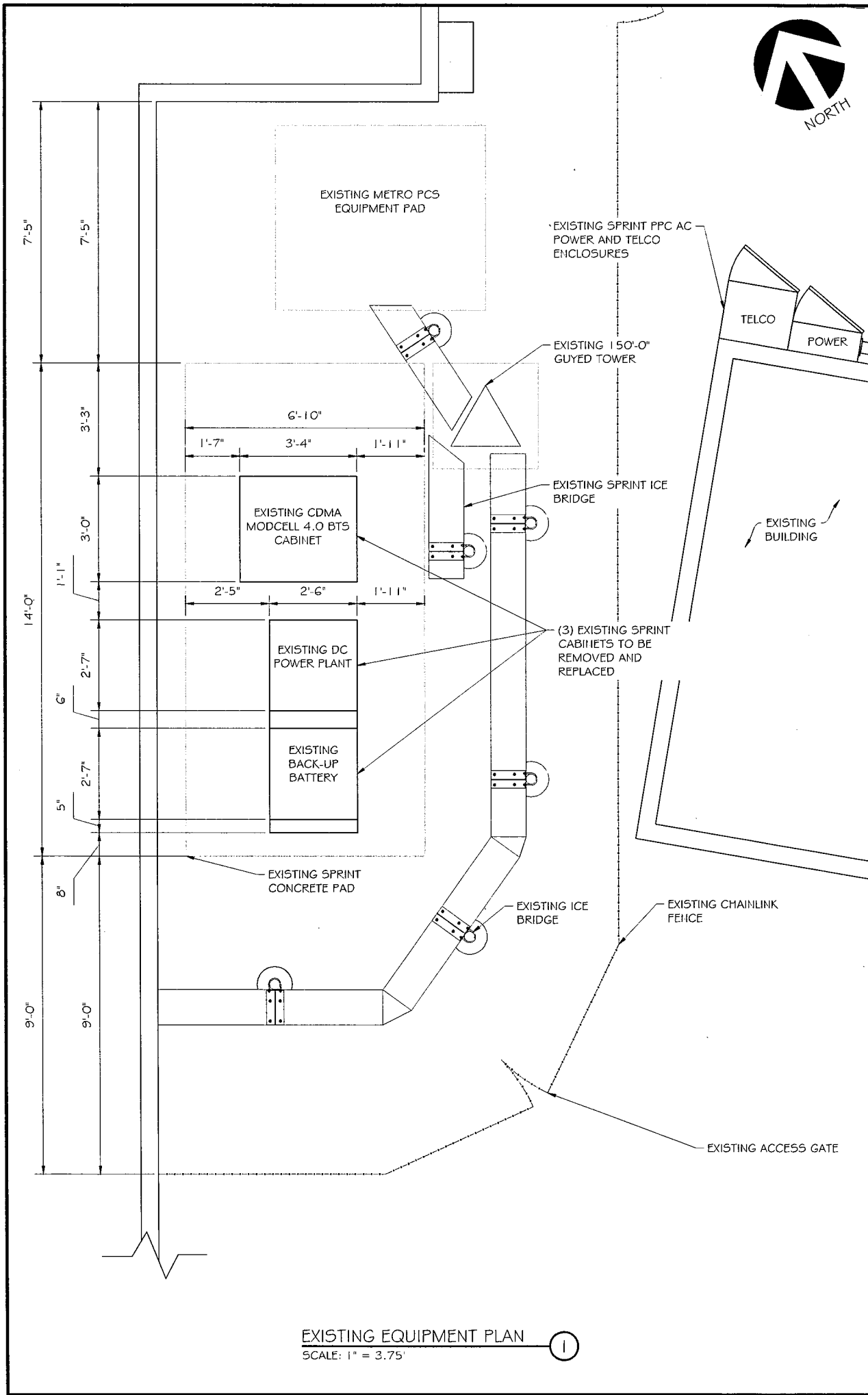
SHEET TITLE:
OVERALL SITE PLAN



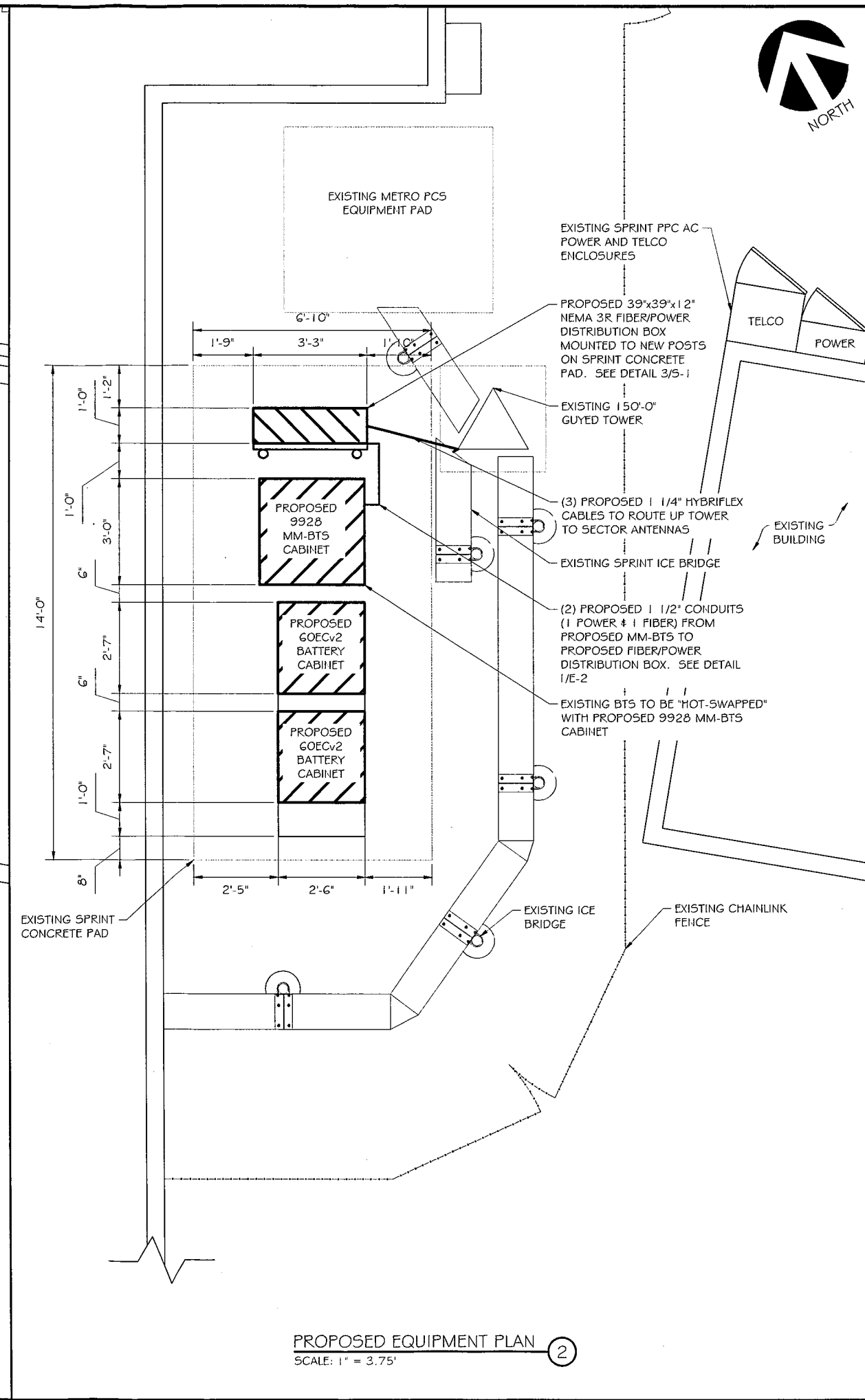
11" x 17" - 1" = 7.5'
 22" x 34" - 1" = 3.75'

PROJECT NUMBER: 23002

SHEET NUMBER: C-1



EXISTING EQUIPMENT PLAN ①
 SCALE: 1" = 3.75'



PROPOSED EQUIPMENT PLAN ②
 SCALE: 1" = 3.75'

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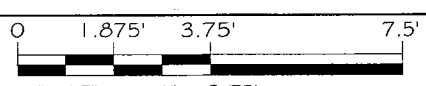
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A	10/03/12	90% CD REVIEW
MARK	DATE	DESCRIPTION
ISSUE PHASE	FINAL	DATE ISSUED 01/08/2013

PROJECT TITLE:
**BANM TOWER
 SITE#: CT03XC212**

PROJECT INFORMATION:
 497 OLD POST ROAD
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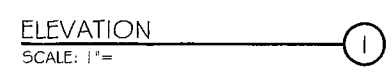
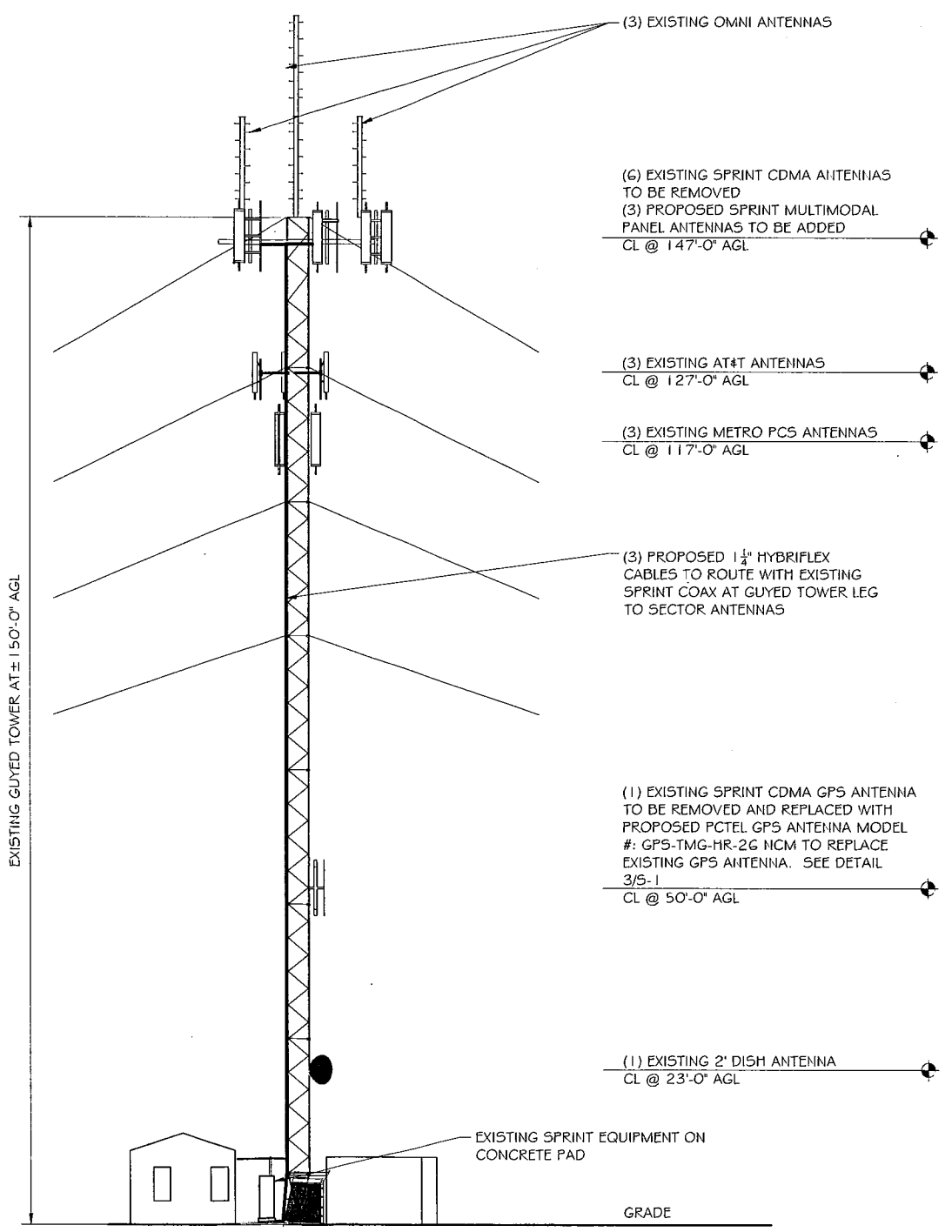
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EQUIPMENT PLAN



11" x 17"	- 1" = 3.75'
22" x 34"	- 1" = 1.875'
PROJECT NUMBER	23002
SHEET NUMBER	A-1

NOTES:

- I. SCOPE**
 A. THIS SECTION COVERS THE SPECIFICATIONS FOR ANTENNA AND COAXIAL CABLE INSTALLATION OF: ANTENNAS, COAXIAL, CONNECTIONS, AND ICE BRIDGE.
 B. REFERENCE SPRINT STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES FOR GENERAL REQUIREMENTS.
- II. ANTENNAS:**
 A. ANTENNAS SHALL BE PLUMB AND INSTALLED SO THAT THE ENTIRE WHIP EXTENDS ABOVE VERTICAL PIPE MOUNT. DIRECTIONAL ANTENNAS SHALL BE ORIENTED TO PROPER AZIMUTH, PROVIDED ON THE RF SPECIFICATION SHEET. NOTE: THE ANTENNA MAY BE ORIENTED USING THE REFLECTOR AS THE REFERENCE, ADJUSTING ITS AZIMUTH 180 DEGREES FROM MAXIMUM ANTENNA RADIATION.
 B. MICROWAVE ANTENNAS (DISHS) SHALL BE ASSEMBLED PER MANUFACTURER'S DRAWINGS. STIFF ARMS AND RADOMES SHALL BE INSTALLED WITH POLARIZATION PROVIDED BY RF SPECIFICATION SHEET. IF PATH IS NOT READY TO ALIGN, DISH SHOULD BE POINTED TOWARD CALCULATED AZIMUTH, OR DIRECTION OF FIELD STAKE DENOTING OPPOSITE END. 2 STIFF ARMS SHALL BE PROVIDED FOR MICROWAVE DISHS 6'-0" IN DIAMETER OR GREATER.
 C. A TRANSIT SHALL BE USED TO PROPERLY ALIGN CELLULAR AND MICROWAVE ANTENNAS.
- III. COAXIAL CABLE:**
 A. COAXIAL CABLE SHALL BE SUPPORTED WITH SNAP-IN HANGERS. SNAP-IN HANGERS SHOULD BE USED EVERY 3 FEET THE ENTIRE HEIGHT OF THE TOWER. ANGLE ADAPTERS OR ROUND MEMBER ADAPTERS WITH BUTTERFLY CLAMPS SHALL BE USED ELSEWHERE, I.E. SIDEARMS, PLATFORMS, AND MICROWAVE MOUNTS.
 B. COAXIAL CABLE SHALL ALSO BE SUPPORTED WITH HOISTING GRIPS, INSTALLED AT MAXIMUM INTERVALS OF 200 FEET. HOISTING GRIPS SHALL BE ATTACHED WITH SHACKLES, BOLTED IN THE 7/8" HOLE OF WAVEGUIDE LADDER.
 C. ALL JUMPERS USED BETWEEN COAXIAL CABLE AND ANTENNA SHALL BE SUPPORTED WITHIN 18 INCHES OF ANTENNA, USING BUTTERFLY CLAMPS WITH ANGLE ADAPTERS OR ROUND MEMBER ADAPTERS AROUND PIPES. CELLULAR ANTENNAS TYPICALLY USE 6' JUMPERS; MICROWAVE DISHS USE 3' JUMPERS.
 D. COAXIAL CABLE SHALL BE NEATLY BENT WHEN REQUIRED, USING A MINIMUM BENDING RADIUS OF 10 TIMES THE DIAMETER OF THE COAXIAL CABLE. DRIP LOOPS SHOULD BEGIN AT THE ICE BRIDGE. THE END IN THE COAXIAL CABLE SHOULD BE AT A LOWER HEIGHT THAN THE ENTRY PORT.
 E. COAXIAL CABLE SHALL BE SUPPORTED WITH SNAP-IN HANGERS ON THE WAVEGUIDE LADDER UNDER ICE BRIDGE. COAXIAL CABLE SHOULD BE NEATLY CUT 1/8" INSIDE BUILDING AND TERMINATED AT THE QUARTER WAVE SHORTS.
 F. CONNECTORS WILL NORMALLY BE PROVIDED FIRST OFF REEL FROM FACTORY. CONNECTORS TERMINATED IN BUILDING SHALL BE NEATLY INSTALLED PER MANUFACTURER'S SPECIFICATIONS.
 G. COAXIAL CABLES SHOULD BE LABELED WITH TAGS INSIDE THE BUILDING.
 H. USE 2" WIDE COLORED TAPE TO INDICATE SECTORS. CONTRACTOR TO USE SECTOR COLOR CODING AS INDICATED IN THESE DRAWINGS OR AS PROVIDED BY SPRINT.
 I. ALL EXCEPTIONS NEED TO BE VERIFIED WITH THE PROJECT MANAGER.
- IV. CONNECTORS:**
 A. ALL CONNECTIONS AND GROUNDING KITS SHALL BE WEATHERPROOFED USING COLD SHRINK OR ANDREW APPROVED WEATHER STRIPPING. NOTE: NO PORTION OF CONNECTOR SHALL BE EXPOSED TO THE ELEMENTS.
 B. COAXIAL CABLE SHALL BE GROUNDED USING GROUNDING KITS AT THE TOP (BELOW THE BEND), BOTTOM (ABOVE THE BEND ON TOWER GROUND BAR), AND ON BUILDING GROUND BAR BEFORE ENTRY INTO WAVEGUIDE PORTS. 4" CABLE BOOTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
 C. GROUNDING KITS SHALL BE NEATLY INSTALLED SO THAT THE JUMPER RUNS IN THE SAME DIRECTION AS THE COAXIAL AND GROUND BAR. JUMPER WIRE SHOULD RUN IN A DIRECT PATH TO THE GROUND BAR/TOWER LADDER, BUT HAVE ADEQUATE SLACK FOR EXPANSION, CONTRACTION, AND REPAIR. NON-OXIDE GREASE SHOULD BE APPLIED BETWEEN LUG AND BARTOWER.
 D. TOWER GROUND BAR SHALL BE INSTALLED ON THE ANGLE BEHIND THE FIRST DIAGONAL WAVEGUIDE LADDER RUNG, ABOVE 8'-6". GROUND BAR SHALL BE ISOLATED FROM ANGLE USING NEWTON BUSHINGS PROVIDED.



Sprint

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James R. Skowronski
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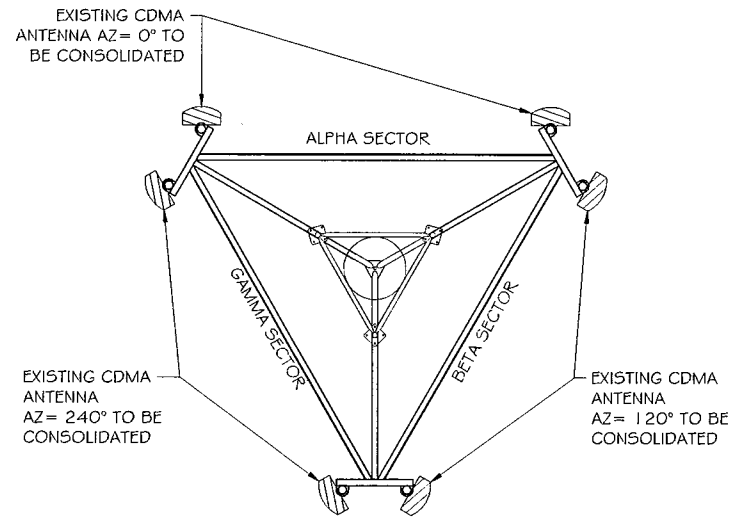
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B	10/26/12	FINAL PRELIM CD'S
A	10/03/12	90% CD REVIEW
ISSUE PHASE	FINAL	DATE ISSUED 01/08/2013

PROJECT TITLE:
**BANM TOWER
 SITE#: CT03XC212**

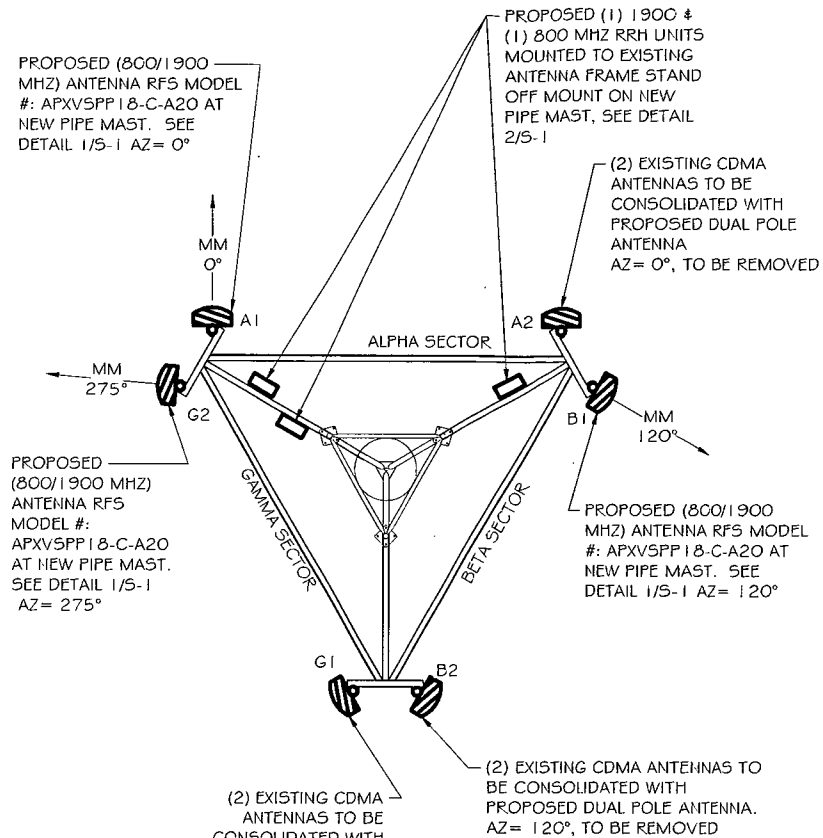
PROJECT INFORMATION:
 497 OLD POST ROAD
 TOLLAND, CT 06084
 TOLLAND COUNTY

SHEET TITLE:
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 & NOTES**

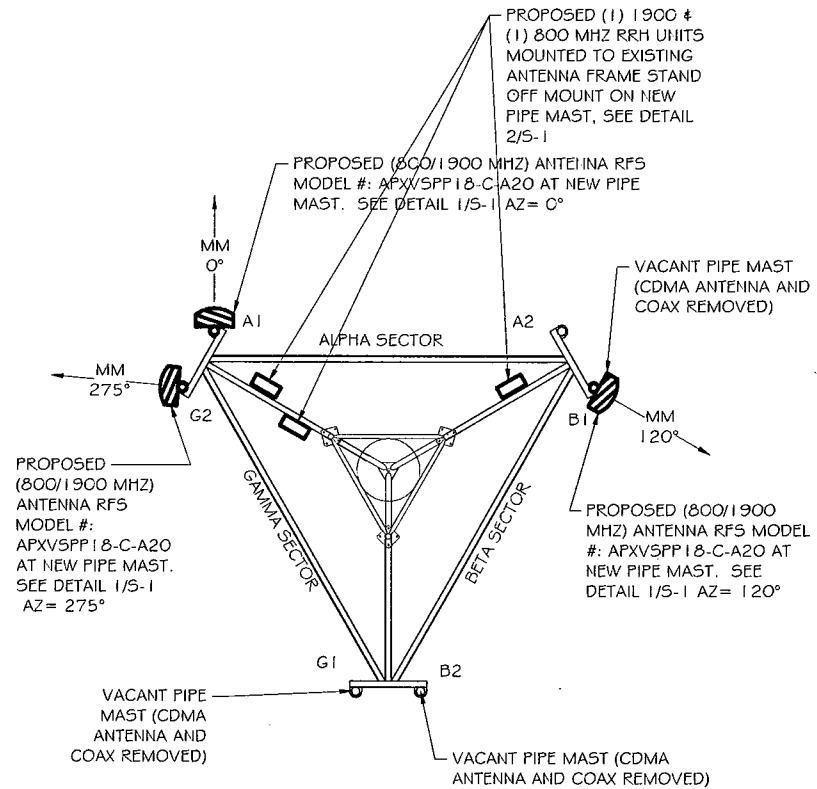
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1 1/2" x 17"	- 1" = 25'
22" x 34"	- 1" = 12.5'
PROJECT NUMBER	23002
SHEET NUMBER	A-2



EXISTING ANTENNA LAYOUT
 SCALE: NTS



INTERIM ANTENNA LAYOUT
 SCALE: NTS



FINAL ANTENNA LAYOUT
 SCALE: NTS



Sprint

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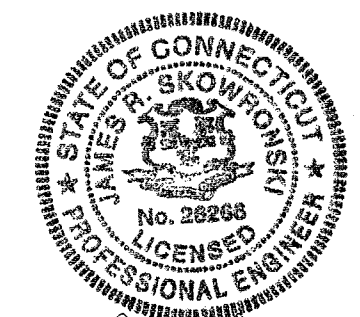
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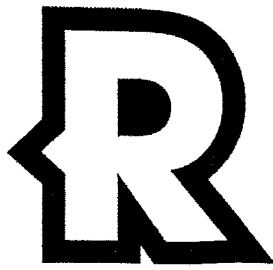
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**ANTENNA DETAILS
 & COAX SCHEDULE**

SCALE: NONE

PROJECT NUMBER: 23002
 SHEET NUMBER: A-3

ANTENNA AND COAXIAL CABLE SCHEDULE

SECTOR	POS.	AZIMUTH	ANTENNA CENTERLINE	ANTENNA STATUS	TECH.	ANTENNA MAKE/ MODEL	MECH. DOWNTILT (°)	ELEC. DOWNTILT (°)	RRHs	CABLE SIZE	CABLE LENGTH
ALPHA	A-1	0°	147'-0"	PROPOSED	MULTIMODAL	RFS/APXV5PP18-C-A20	1900(0), 800(0)	1900(-1), 800(0)	(1) 1900, (1) 800	(1) 1 1/2" HYBRIFLEX HYBRID CABLE RFS #HB114-1-08U4-M5J	±180'-0"
	A-2	0°	147'-0"	PROPOSED CONSOLIDATED CDMA TO BE REMOVED	CDMA	T.B.D. (DUAL POLE CDMA)	-	-	-	EX. TO BE REMOVED	-
BETA	B-1	120°	147'-0"	PROPOSED	MULTIMODAL	RFS/APXV5PP18-C-A20	1900(0), 800(0)	1900(-2), 800(-2)	(1) 1900, (1) 800	(1) 1 1/2" HYBRIFLEX HYBRID CABLE RFS #HB114-1-08U4-M5J	±180'-0"
	B-2	120°	147'-0"	PROPOSED CONSOLIDATED CDMA TO BE REMOVED	CDMA	T.B.D. (DUAL POLE CDMA)	-	-	-	EX. TO BE REMOVED	-
GAMMA	G-1	240°	147'-0"	PROPOSED CONSOLIDATED CDMA TO BE REMOVED	CDMA	T.B.D. (DUAL POLE CDMA)	-	-	-	EX. TO BE REMOVED	-
	G-2	275°	147'-0"	PROPOSED	MULTIMODAL	RFS/APXV5PP18-C-A20	1900(0), 800(0)	1900(-3), 800(-8)	(1900, (1) 800	(1) 1 1/2" HYBRIFLEX HYBRID CABLE RFS #HB114-1-08U4-M5J	±180'-0"



RAMAKER
& ASSOCIATES, INC.

BANM TOWER (CT03XC212)

**PREPARED FOR:
SPRINT**

**PREPARED BY:
RAMAKER & ASSOCIATES, INC.
JOB NUMBER: 23002**

**STRUCTURAL ASSESSMENT
150-FOOT GUYED TOWER**

1120 Dallas Street, Sauk City, WI 53583
Phone: 608-643-4100 ▲ Fax: 608-643-7999
www.ramaker.com

STRUCTURAL ASSESSMENT

SITE: BANM Tower (CT03XC212)
497 Old Post Road
Tolland, Tolland County, Connecticut 06084

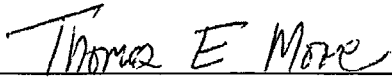
PREPARED FOR: Alcatel-Lucent
600 Mountain Avenue
Murray Hill, New Jersey 07974

CONTACT PERSON: Alcatel-Lucent
John Szilezy
Site Acquisition Manager
john.szilezy@alcatel-lucent.com

PREPARED BY: Ramaker & Associates, Inc.
1120 Dallas Street
Sauk City, Wisconsin 53583
Telephone: (608) 643-4100
Facsimile: (608) 643-7999

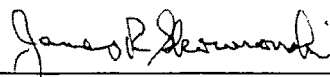
RAMAKER JOB NUMBER: 23002

DATE OF REPORT ISSUANCE: November 9, 2012



Thomas E. Moore
Structural Engineer

11/9/12
Date



James R. Skowronski, P.E.
Supervising Engineer

11/9/12
Date

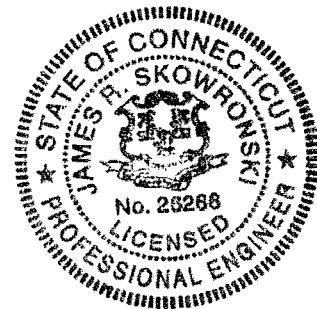


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SECTION 1
EXECUTIVE SUMMARY

This report summarizes the structural analysis conducted by Ramaker & Associates, Inc. (Ramaker & Associates) for Alcatel-Lucent (ALU) on behalf of Sprint, who intends to install additional equipment on an existing 150-foot guyed tower. The tower site is located in Tolland, Tolland County, Connecticut.

ALU is proposing to install three (3) RFS APXVSP18-C-A20 panel antennas on the existing mounts at a centerline elevation of 147 feet AGL. ALU is also proposing to install three (3) ALU 1900 MHz RRH units and three (3) ALU 800 MHz RRH units on the existing Frame. The proposed equipment shall be fed with three (3) 1-1/4 inch fiber/power hybrid cables. Three of the six existing CDMA panel antennas shall be removed during the interim phase, and then the other three CDMA panel antennas shall be removed for the final antenna layout.

Results of our analysis show that the tower will be stressed to a maximum of 92.8 percent of capacity under proposed loading conditions. All foundation reactions are less than the modified original design reactions. Therefore, it is anticipated that the existing foundations will provide adequate strength under proposed loading conditions.

In summary, the tower will pass the TIA-222-G code requirements under proposed loading conditions.

SECTION 2

INTRODUCTION

2.1 PROJECT INFORMATION

This report summarizes the structural analysis conducted by Ramaker & Associates, Inc. (Ramaker & Associates) for ALU, who intends to install additional equipment on an existing tower.

2.2 PURPOSE OF REPORT

The analysis activities of this report were conducted for the purposes of creating and analyzing a model of the subject structure under the required loading conditions. Base reactions from the resulting model were also determined for tower foundation and support development. Recommendations regarding the analysis results, loading configuration, and structural modifications are also provided.

2.3 SCOPE OF SERVICES

Ramaker & Associates developed a finite element model (FEM) of the tower, using tnxTower, for member force, joint deflection, and structure reaction determinations. Subsequently, this report was drafted to provide our engineering recommendations. All information contained herein is valid only for the described structure configuration and loading conditions. Ramaker & Associates reserves the right to modify our recommendations should alterations to the tower loading occur.

SECTION 3

MODEL DEVELOPMENT

3.1 INTRODUCTION

Ramaker & Associates, Inc. developed a FEM of the tower superstructure using the tower drawings and site photos. Required static loads consisting of the antenna configuration, wind forces, ice loads, and linear appurtenances (including cable loads) were then applied to the FEM. As a result, all member forces, allowable capacities, and base reactions were computed. Additionally, potentially overstressed members were identified.

3.2 EXISTING STRUCTURE INFORMATION

Tower information was gathered from the tower drawings by Pirod, file number A-115329, drawing number 204110-B, dated 1/13/99. The structural analysis by Tectonic, work order 2650.CT330, dated 5/15/02, was also used. The foundation plan by Tectonic, work order 2204.02, dated 1/7/99, was also used.

3.3 EXISTING TOWER LOADS

Ramaker & Associates understands that the existing antenna, cable, and appurtenance configurations are as shown in the following chart:

Elevation	Appurtenance	Mount	Coax
170	Decibel DB420	Top Mount	7/8
150	20' Dipole	Face Mount	7/8
	4' Lightning Rod	Leg Mount	-
147	(2) Decibel DB420	Delta Mount	(2) 7/8
	Decibel DB222		7/8
	** (6) Decibel DB980H90E-M **		(6) 1-5/8
125	(2) EMS MB96RR900200DPBL	Delta Mount	(9) 7/8
	EMS FS90-11-00NAL2		
	(6) ClearGain DD1900W800		
120	Decibel DB225	Leg Mount	7/8
115	(3) 6' Panel Antennas	Leg Mounted	(6) 1-1/4
96	10' Omni	Torque Arm	7/8
80	15' Omni	4' Standoff	7/8
55	Decibel DB225	Leg Mount	7/8
50	GPS Antenna	3' Standoff	1/2
35	4' Dish	Leg Mount	7/8

BANM TOWER (CT03XC212)

Three (3) of the six (6) CDMA antennas at 147 feet AGL were assumed to be removed under the interim loading conditions and the remaining three (3) antennas shall be removed for the final antenna layout.

3.4 PROPOSED TOWER LOADS

Ramaker & Associates understands that the total antenna loading for the tower will consist of the aforementioned existing antennas and the following proposed antennas:

Elevation	Appurtenance	Mount	Coax
147	(3) RFS APXVSPP18-C-A20	Existing Delta Mount	(3) 1-1/4 Fiber/Power
	(3) ALU 1900MHz RRH		
	(3) ALU 800MHz RRH		

The proposed fiber/power cables were assumed to be installed next to the existing Sprint coax.

3.5 WIND AND ICE LOAD

Wind forces used in model development are in compliance with the TIA-222-G Standard. These guidelines call for an analysis to be performed, which assumes a basic wind speed (3-second gust) of 105 miles-per-hour (mph) without ice in Tolland County. The tower is also designed for a 50 mph basic wind speed with 1.00-inch of radial ice. The tower was analyzed using the following parameters: Structure Class II, Topographic Category 1, and Exposure Category C.

SECTION 4

ANALYSIS RESULTS

4.1 ANALYSIS RESULTS

The tower superstructure was analyzed with the combined existing and proposed antenna loading with and without radial ice. The computed maximum tower member stress capacities are as follows:

Component Type	Percent Capacity
Leg	77.3
Diagonal	70.8
Horizontal	30.8
Guy Line	92.8
Bolts	30.8
RATING	92.8

4.2 BASE REACTIONS

The computed maximum reactions under the corresponding maximum moment are as follows:

Load Type	Original Design	Original Design * 1.35	Proposed Model
Axial (k)	118.0	159.30	135.80
Shear (k)	2.6	3.51	1.59
Anchor A Uplift (k)	46.5	62.78	37.80
Anchor A Lateral (k)	35.5	47.93	30.93
Anchor B Uplift (k)	71.8	96.93	41.75
Anchor B Lateral (k)	35.5	47.93	30.92
Anchor C Uplift (k)	70.4	95.04	66.23
Anchor C Lateral (k)	37.6	50.76	31.22

All foundation reactions are less than the modified original design reactions. Therefore, it is anticipated that the existing foundations will provide adequate strength under proposed loading conditions.

**SECTION 5
LIMITATIONS**

The recommendations contained within this report were developed using general project information provided by the owner, tower manufacturer, general field observations, reference information and laboratory testing data, as applicable. All recommendations pertain only to the proposed tower construction, location, and loading as described in this report. Ramaker & Associates assumes no responsibility for failures caused by factors beyond our control. These include but are not limited to the following:

1. Missing, corroding, and/or deteriorating members
2. Improper manufacturing and/or construction
3. Improper maintenance

Ramaker & Associates assumes no responsibility for modifications completed prior to or hereafter in which Ramaker & Associates was not directly involved. These modifications include but are not limited to the following:

1. Replacing or strengthening bracing members
2. Reinforcing or extending vertical members
3. Installing or removing antenna mounting gates or side arms
4. Changing loading configurations

Furthermore, Ramaker & Associates hereby states that this document represents the entire report and that it assumes no liability for any factual changes that may occur after the date of this report. All representations, recommendations and conclusions are based on the information contained and set forth herein. If you are aware of any information contrary to that contained herein, or if you are aware of any defects arising from the original design, material, fabrication and erection deficiencies, you should disregard this report and immediately contact Ramaker & Associates. Ramaker & Associates isn't liable for any representation, recommendation, or conclusion not expressly stated herein.

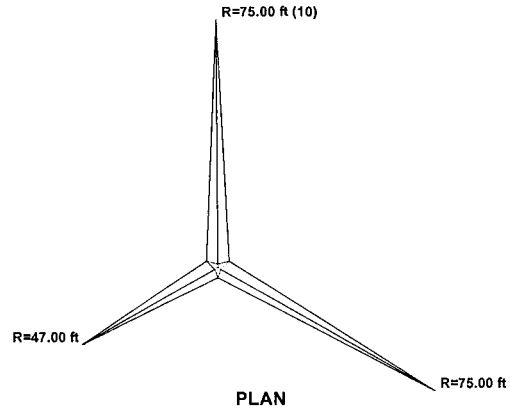
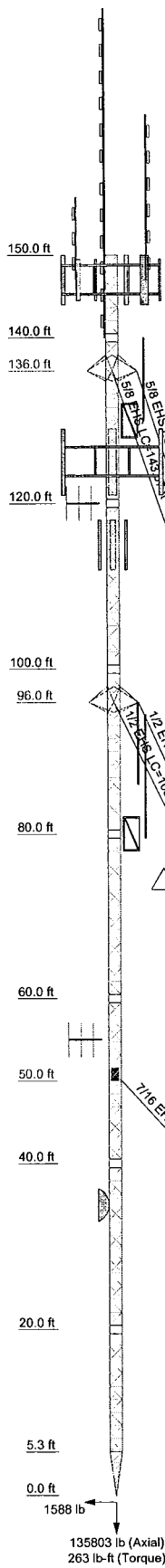
The tower owner is responsible for verifying that the existing loading on the tower is consistent with the loading applied to the tower within this report.

SECTION 6
REFERENCES

1. 2009 International Building Code.
2. Telecommunications Industries Association, Structural Standard for Antenna Supporting Structures and Antennas, TIA Standard ANSI/TIA-222-G 2005, Washington, D.C.

APPENDIX A
TOWER FIGURES

Section	T1	T2	T3	T4	T5	T6	T7	T8	T9	Weight (lb)
Legs					SR 1 3/4					5712.2
Leg Grade					A572-50					171.5
Diagonals					SR 5/8					516.7
Diagonal Grade					A36					171.5
Top Girts					SR 3/4					516.7
Bottom Girts					SR 3/4					516.7
Horizontals					SR 3/4					516.7
Top Guy Pull-Offs					N.A.					516.7
Bot Guy Pull-Offs					N.A.					516.7
Face Width (ft)										171.5
# Panels @ (ft)										171.5
Weight (lb)										5712.2



DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
DB420	170	4' Standoff	130
5" Dia 20' Omni	150	MB96RR900200DPBL w/Mount Pipe	125
DB420	147	MB96RR900200DPBL w/Mount Pipe	125
DB222	147	FS90-11-00NAL2 w/Mount Pipe	125
DB420	147	CG DD1900W800	125
DB980H90E-M w/Mount Pipe	147	CG DD1900W800	125
DB980H90E-M w/Mount Pipe	147	CG DD1900W800	125
DB980H90E-M w/Mount Pipe	147	Piord Delta Mount (3)	125
APXVSP18-C-A20 w/Mount Pipe	147	DB225-A	120
APXVSP18-C-A20 w/Mount Pipe	147	6' x 8" Panel Antenna w/Mount Pipe	115
APXVSP18-C-A20 w/Mount Pipe	147	6' x 8" Panel Antenna w/Mount Pipe	115
1900MHz 4x40W RRH	147	6' x 8" Panel Antenna w/Mount Pipe	115
1900MHz 4x40W RRH	147	10' Omni	96
1900MHz 4x40W RRH	147	15' Omni	80
800MHz 2x50W RRH	147	4' Standoff	80
800MHz 2x50W RRH	147	DB225-A	55
800MHz 2x50W RRH	147	GPS	50
Piord Delta Mount (3)	147	3' Standoff	50
10' Omni	130	4 FT DISH	35

SYMBOL LIST

MARK	SIZE	MARK	SIZE
A	4 @ 1.3125		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

TOWER DESIGN NOTES

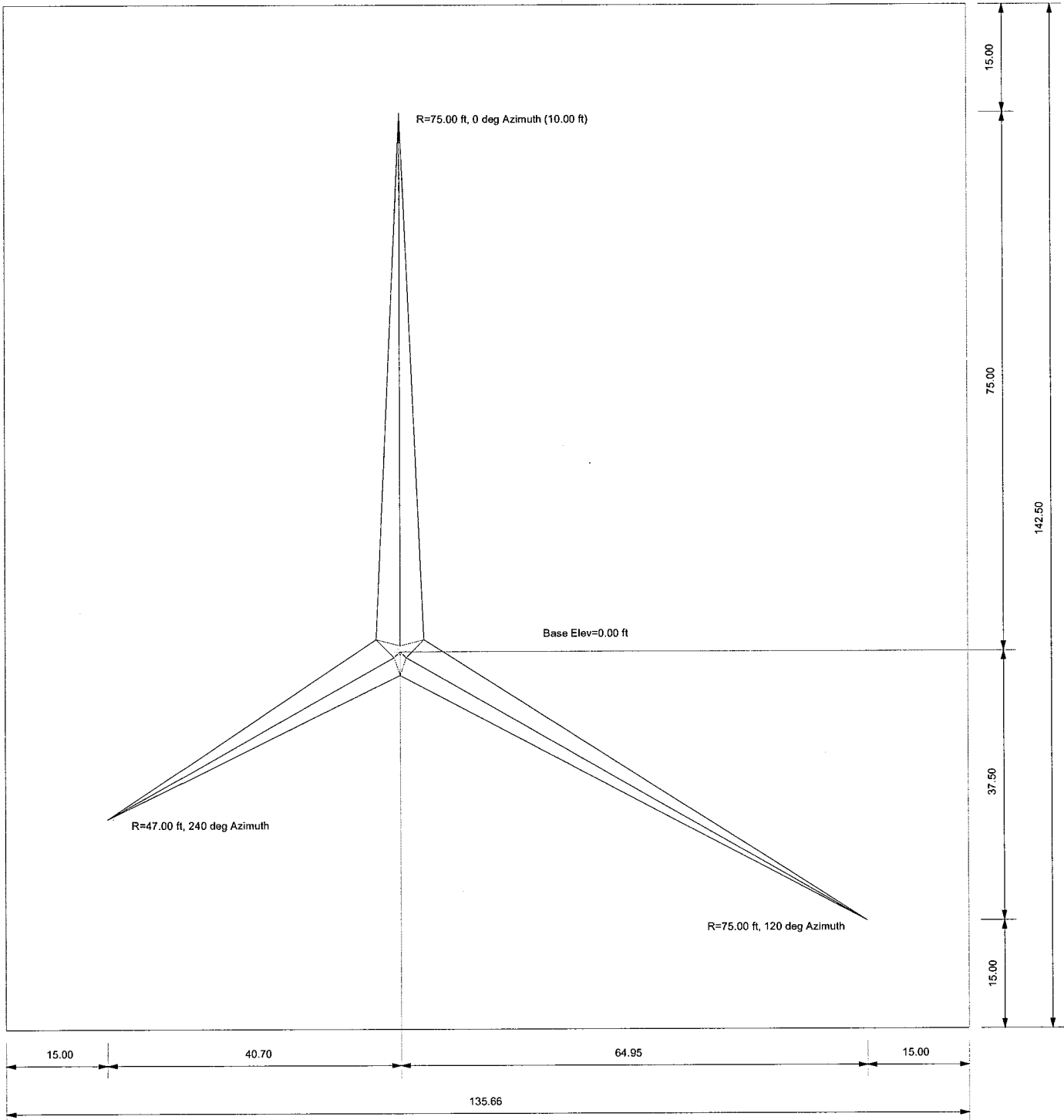
1. Tower is located in Tolland County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-G Standard.
3. Tower designed for a 105 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 92.8%




ALL REACTIONS ARE FACTORED

<p>RAMAKER & ASSOCIATES, INC. Consulting Engineers</p>	Ramaker & Associates, Inc. 1120 Dallas Street Sauk City, WI 53583 Phone: (608) 643-4100 FAX: (608) 643-7999		Job: BANM Tower (CT03XC212) Project: 23002	
	Client: Alcatel-Lucent / Sprint	Drawn by: tmoore	App'd:	
	Code: TIA-222-G	Date: 11/09/12	Scale: NTS	
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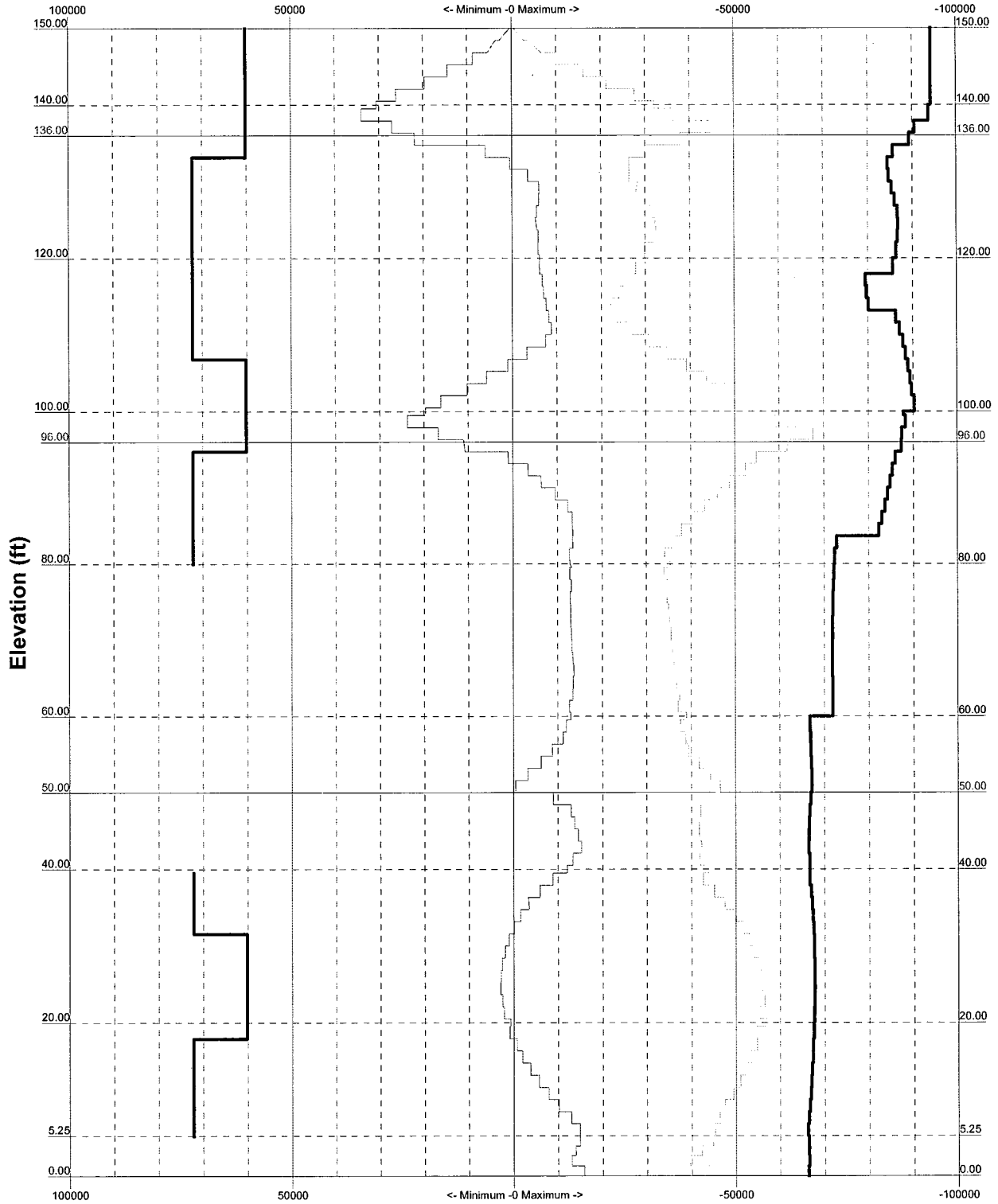
Plot Plan
Total Area - 0.44 Acres




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	1120 Dallas Street Sauk City, WI 53583		Project: 23002	Drawn by: tmoore
	Phone: (608) 643-4100 FAX: (608) 643-7999		Client: Alcatel-Lucent / Sprint	Date: 11/09/12
			Code: TIA-222-G	Scale: NTS
			Path: I:\23000\23002\Structural\Risa\23002.dwg	Dwg No. E-2

TIA-222-G - 105 mph/50 mph 1.000 in Ice Exposure C

Leg Capacity ——— Leg Compression (lb)



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	Client: Alcatel-Lucent / Sprint	Drawn by: tmoore	App'd:	
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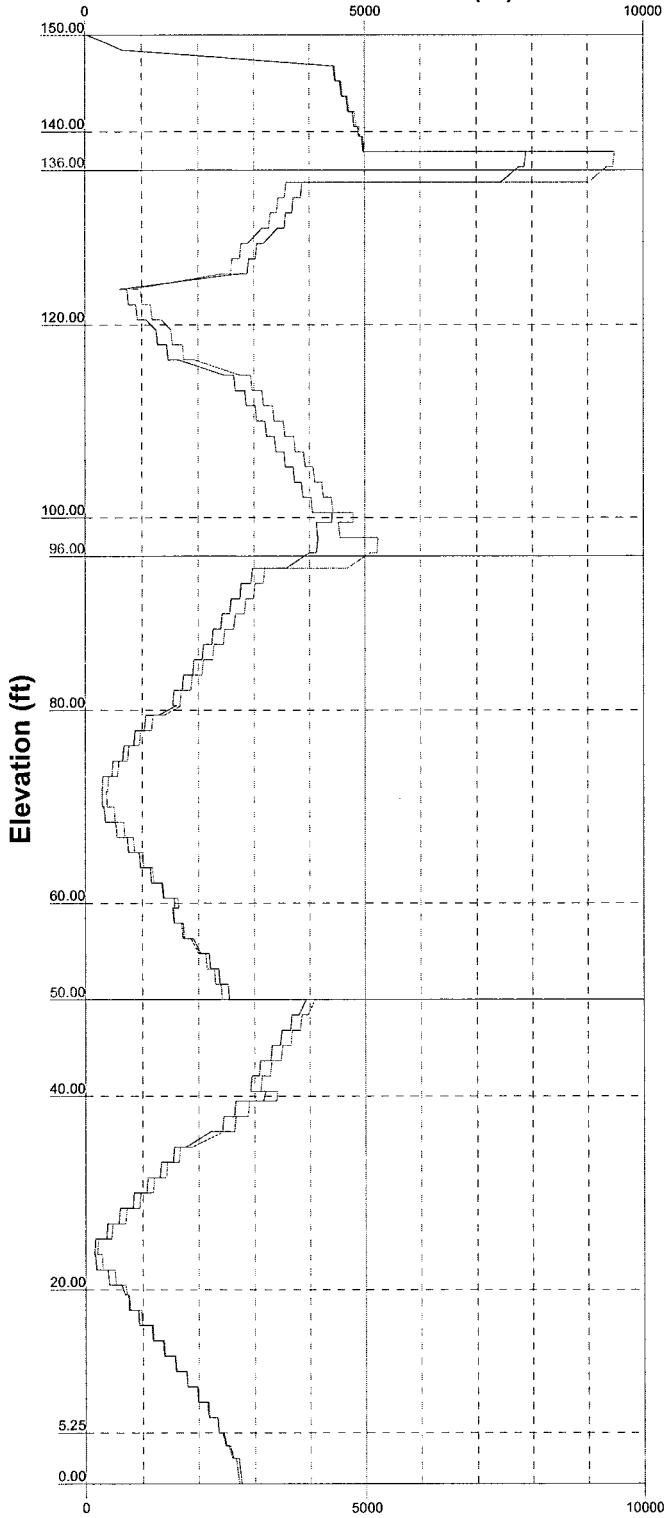
Vx

Vz

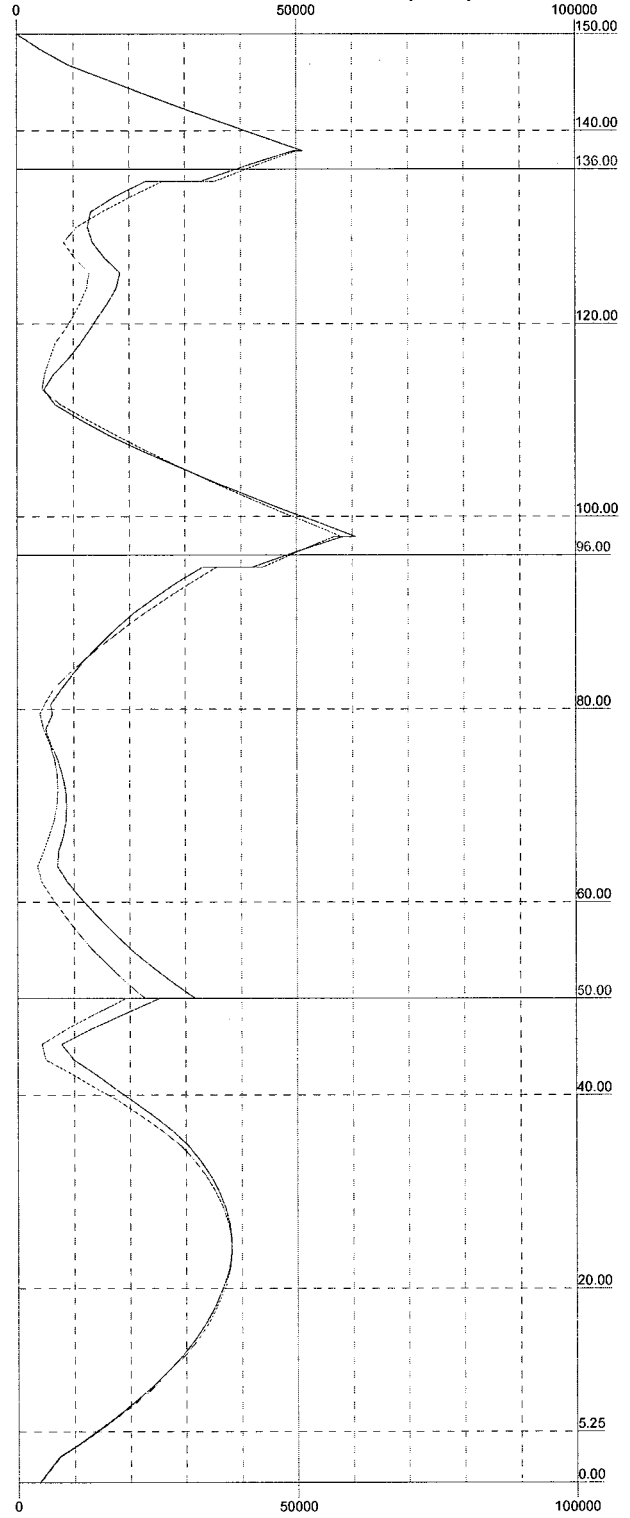
Mx


Mz

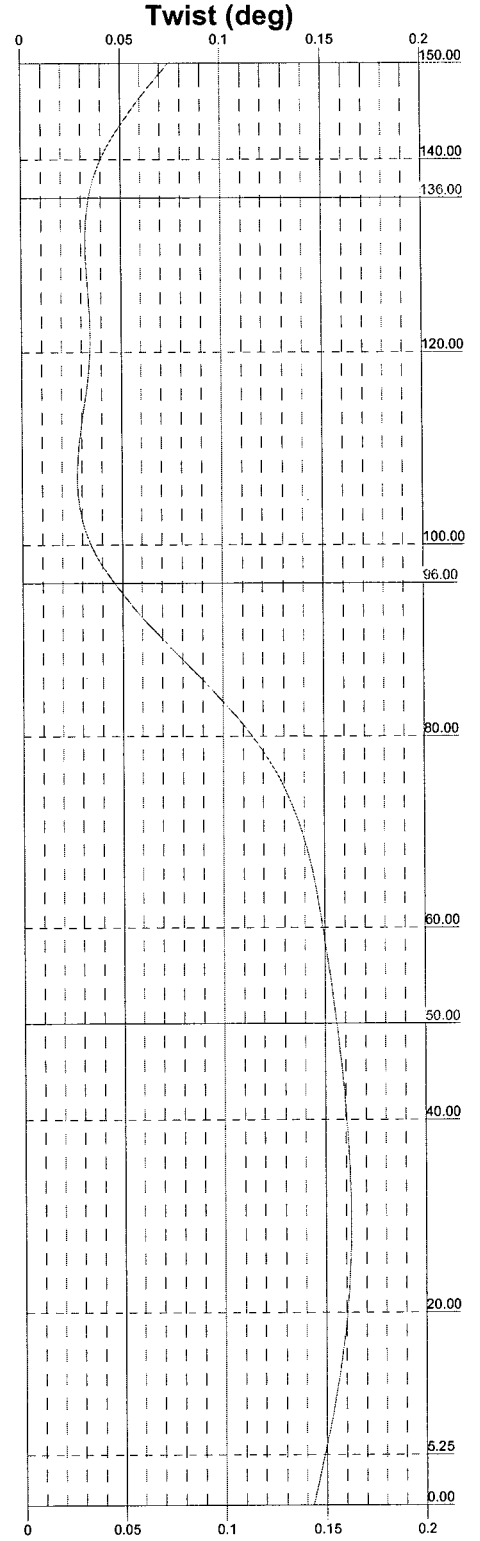
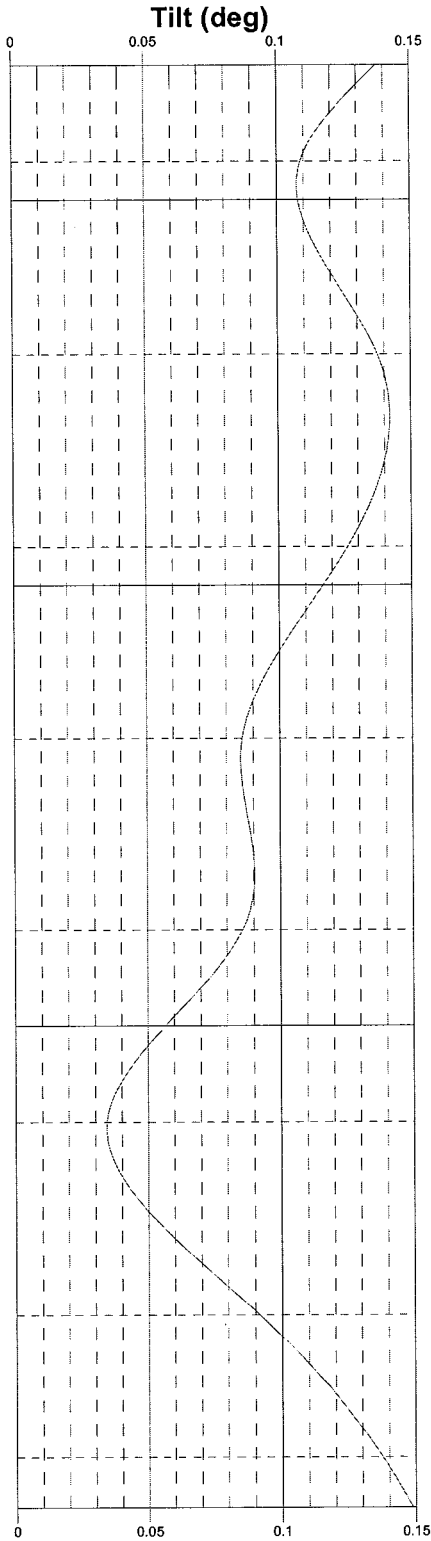
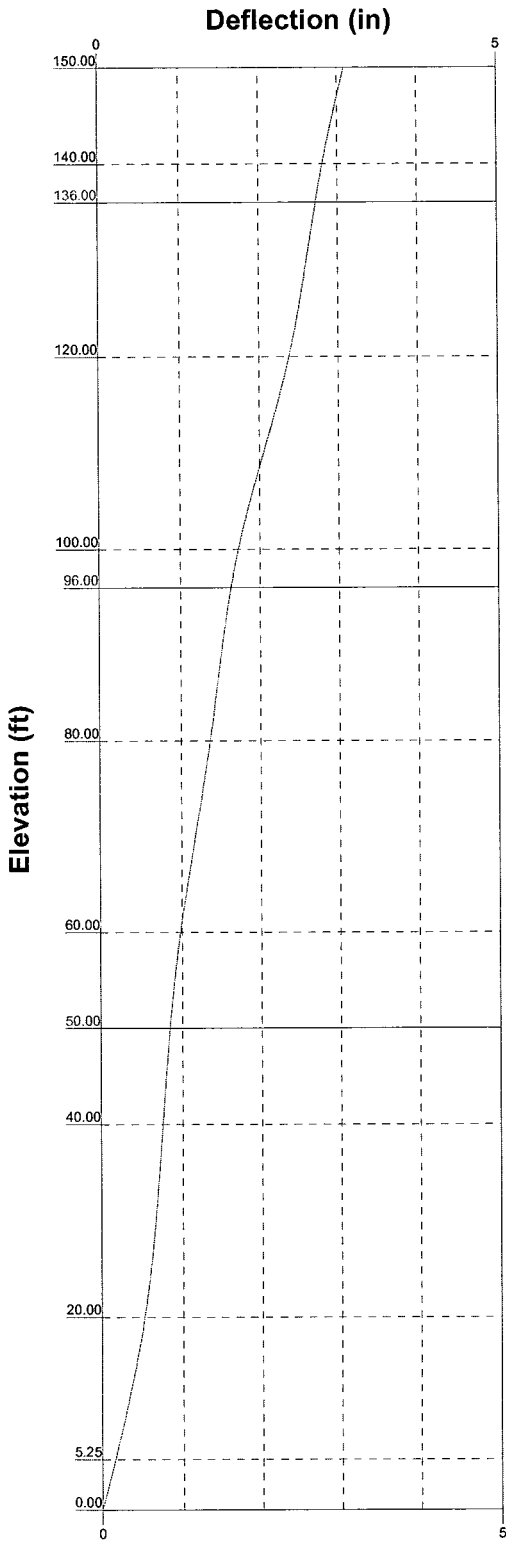
Global Mast Shear (lb)



Global Mast Moment (lb-ft)



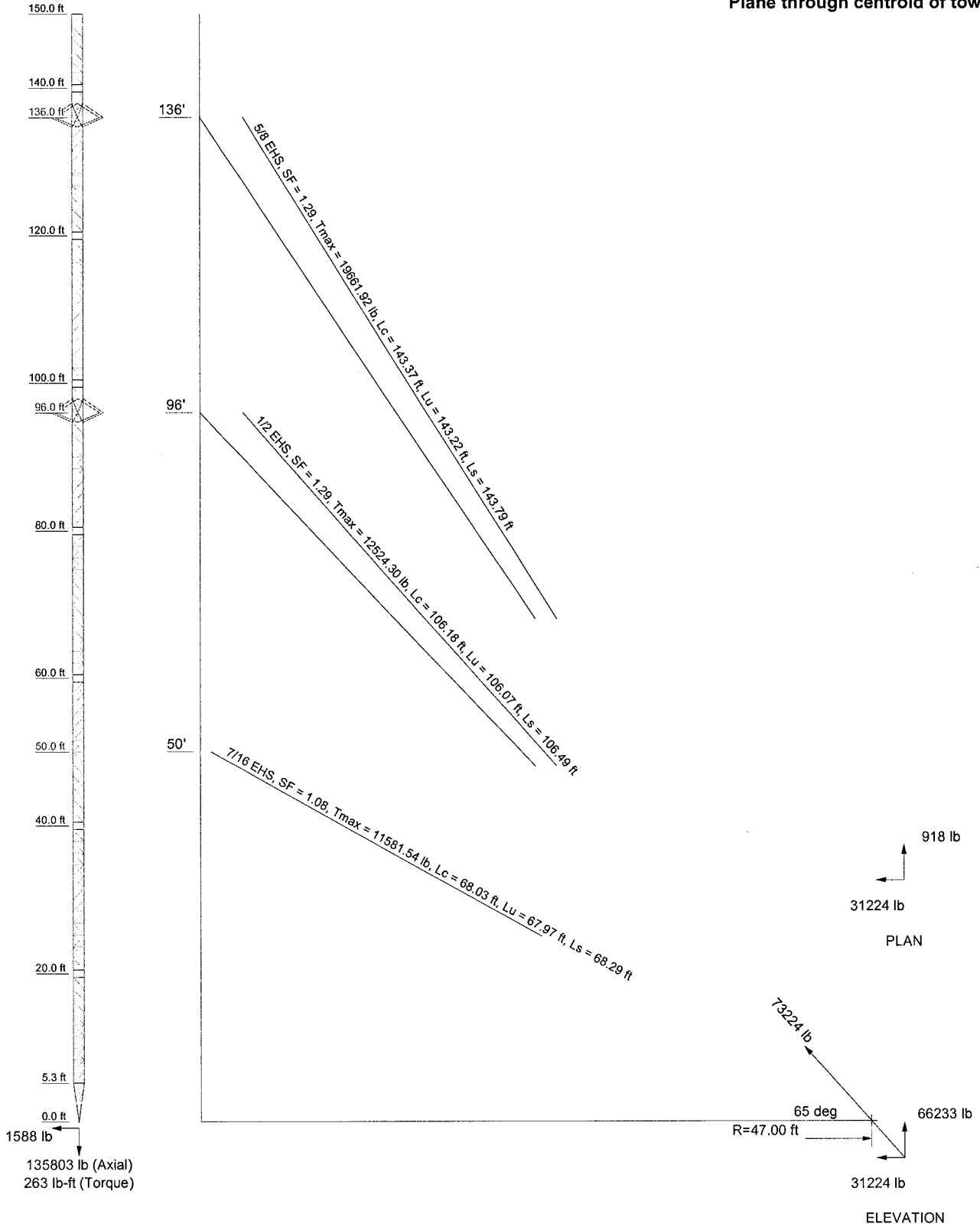
 <p>RAMAKER & ASSOCIATES, INC. Consulting Engineers</p>	<p>Ramaker & Associates, Inc.</p>		<p>Job: BANM Tower (CT03XC212)</p>		
	<p>1120 Dallas Street Sauk City, WI 53583</p>		<p>Project: 23002</p>		
	<p>Phone: (608) 643-4100</p>		<p>Client: Alcatel-Lucent / Sprint</p>		
	<p>FAX: (608) 643-7999</p>		<p>Drawn by: tmoore</p>		
			<p>Date: 11/09/12</p>		
		<p>Code: TIA-222-G</p>		<p>Scale: NTS</p>	
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 <p>Ramaker & Associates, Inc. Consulting Engineers</p>	<p>1120 Dallas Street Sauk City, WI 53583 Phone: (608) 643-4100 FAX: (608) 643-7999</p>		<p>Job: BANM Tower (CT03XC212)</p>	
	<p>Project: 23002</p>		<p>Client: Alcatel-Lucent / Sprint</p>	
	<p>Code: TIA-222-G</p>		<p>Drawn by: tmoore</p>	
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Guy Tensions and Tower Reactions
TIA-222-G - 105 mph/50 mph 1.0000 in Ice Exposure C

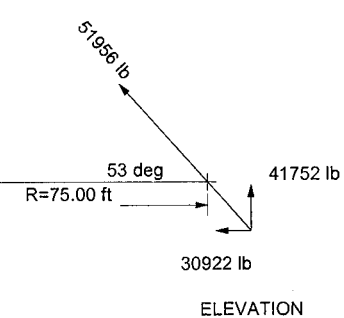
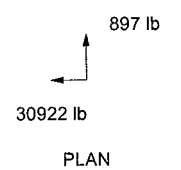
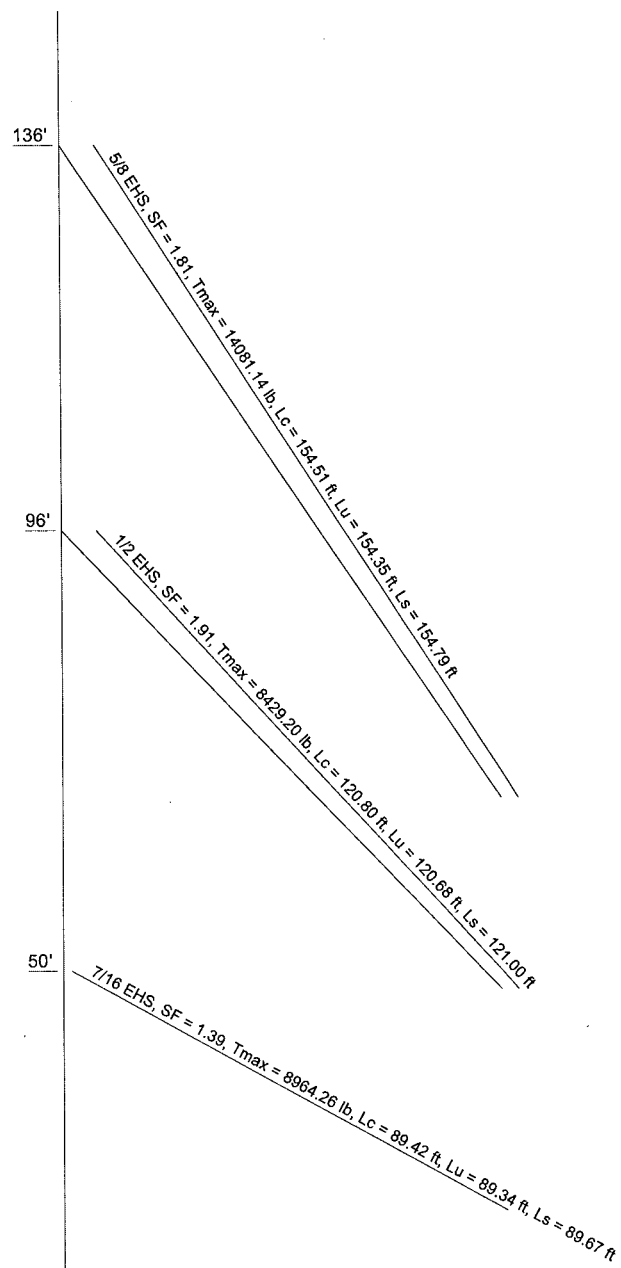
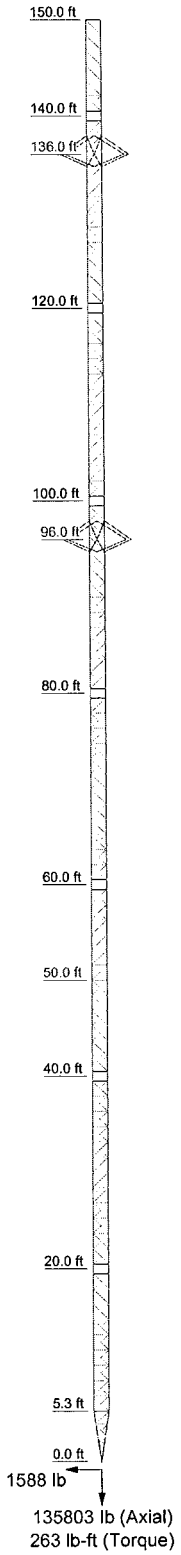
Maximum Values
Anchor 'C' @ 47 ft Azimuth 240 deg Elev 0 ft
Plane through centroid of tower



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	Project: 23002		Client: Alcatel-Lucent / Sprint
	Code: TIA-222-G		Drawn by: tmoore
	Date: 11/09/12		App'd:
	Scale: NTS		Path: I:\23000\23002\Structural\Risn\23002.eri
	Dwg No. E-6		

Guy Tensions and Tower Reactions
TIA-222-G - 105 mph/50 mph 1.0000 in Ice Exposure C

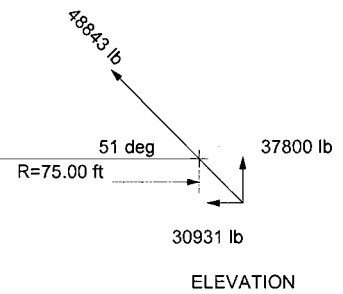
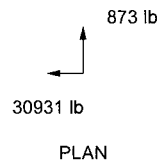
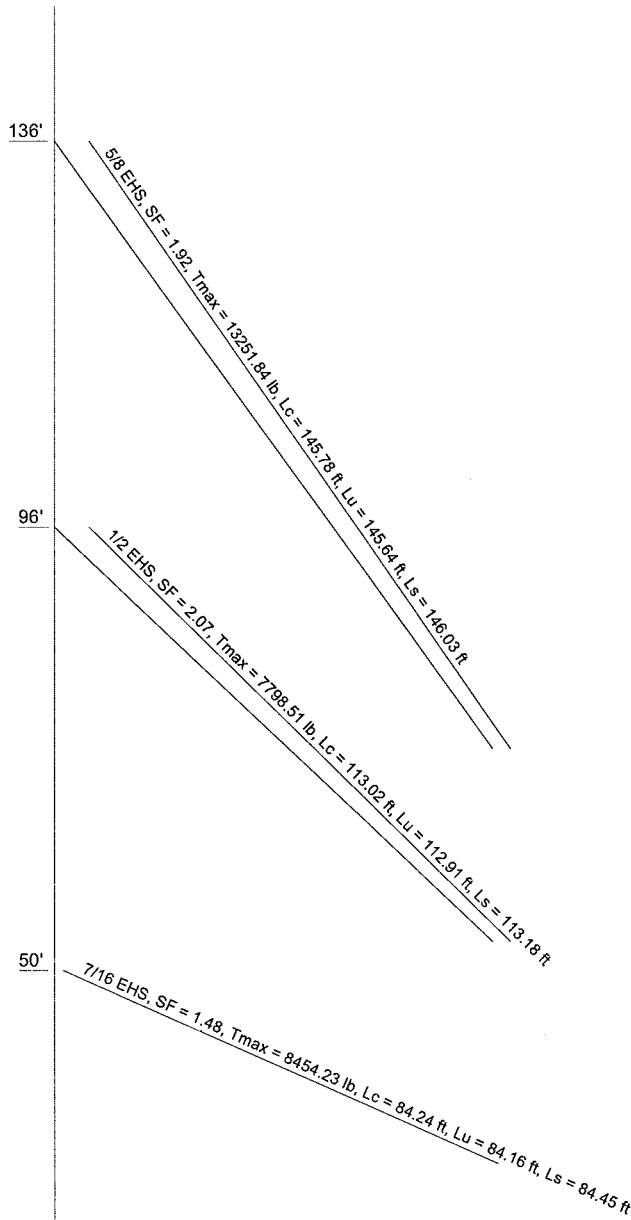
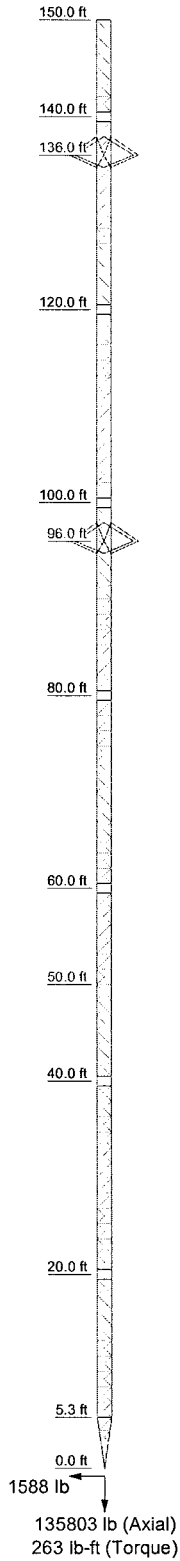
Maximum Values
Anchor 'B'@75 ft Azimuth 120 deg Elev 0 ft
Plane through centroid of tower




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	Project: 23002		
	Client: Alcatel-Lucent / Sprint	Drawn by: tmoore	App'd:
	Code: TIA-222-G	Date: 11/09/12	Scale: NTS
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Guy Tensions and Tower Reactions
TIA-222-G - 105 mph/50 mph 1.0000 in Ice Exposure C

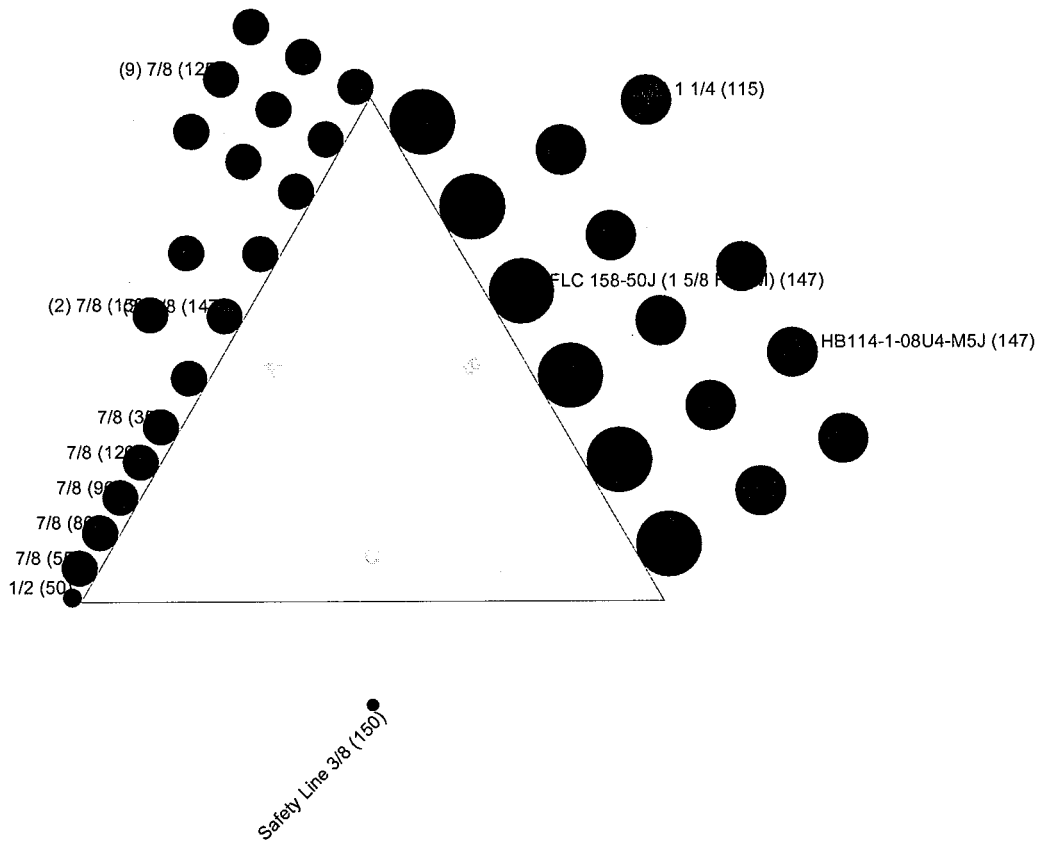
Maximum Values
Anchor 'A'@75 ft Azimuth 0 deg Elev 10 ft
Plane through centroid of tower




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	Code: TIA-222-G	Date: 11/09/12	Scale: NTS			
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Feedline Plan

_____ Round _____ Flat _____ App In Face _____ App Out Face

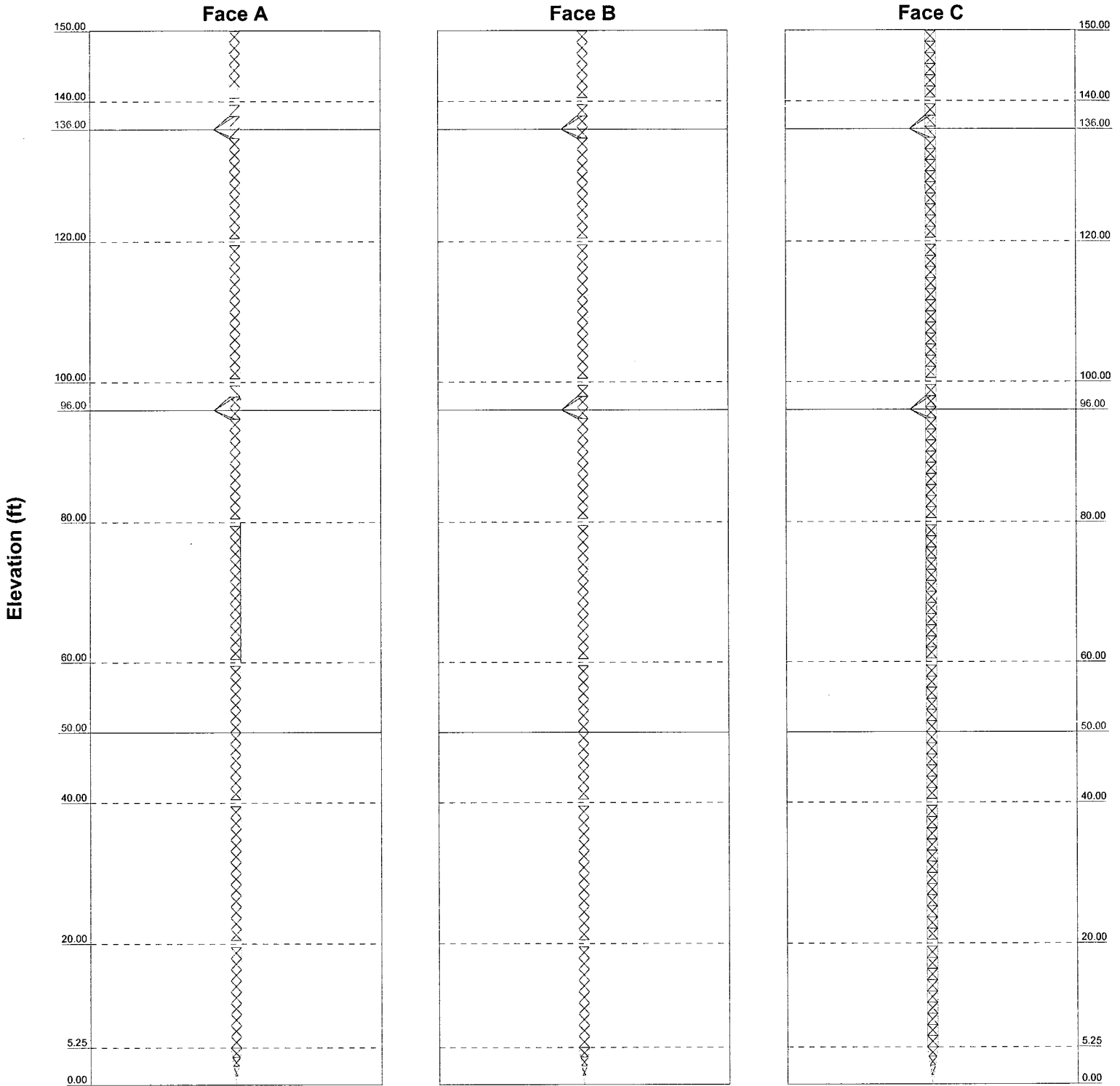


 RAMAKER & ASSOCIATES, INC. Consulting Engineers	Ramaker & Associates, Inc.		Job: BANM Tower (CT03XC212)		
	1120 Dallas Street		Project: 23002		
	Sauk City, WI 53583		Client: Alcatel-Lucent / Sprint	Drawn by: tmoore	App'd:
	Phone: (608) 643-4100		Code: TIA-222-G	Date: 11/09/12	Scale: NTS
	FAX: (608) 643-7999		Path: I:\230002\23002\Structural\Risat\23002.dwg		Dwg No: E-7

Stress Distribution Chart

0' - 150'

> 100%
 90%-100%
 75%-90%
 50%-75%
 < 50% Overstress



<p>RAMAKER & ASSOCIATES, INC. Consulting Engineers</p>	Ramaker & Associates, Inc.		Job: BANM Tower (CT03XC212)	
	1120 Dallas Street		Project: 23002	
	Sauk City, WI 53583		Client: Alcatel-Lucent / Sprint	
	Phone: (608) 643-4100		Drawn by: tmoore	App'd:
	FAX: (608) 643-7999		Code: TIA-222-G	Date: 11/09/12
		Scale: NTS		Dwg No. E-8
		Path: I:\23000\23002\Structural\Ris\23002.dwg		

APPENDIX B
TOWER CALCULATIONS

<i>tnxTower</i> Ramaker & Associates, Inc. 1120 Dallas Street Sauk City, WI 53583 Phone: (608) 643-4100 FAX: (608) 643-7999	Job BANM Tower (CT03XC212)	Page 1 of 38
	Project 23002	Date 13:55:14 11/09/12
	Client Alcatel-Lucent / Sprint	Designed by tmoore

Tower Input Data

The main tower is a 3x guyed tower with an overall height of 150.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 1.50 ft at the top and tapered at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Tolland County, Connecticut.

Basic wind speed of 105 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

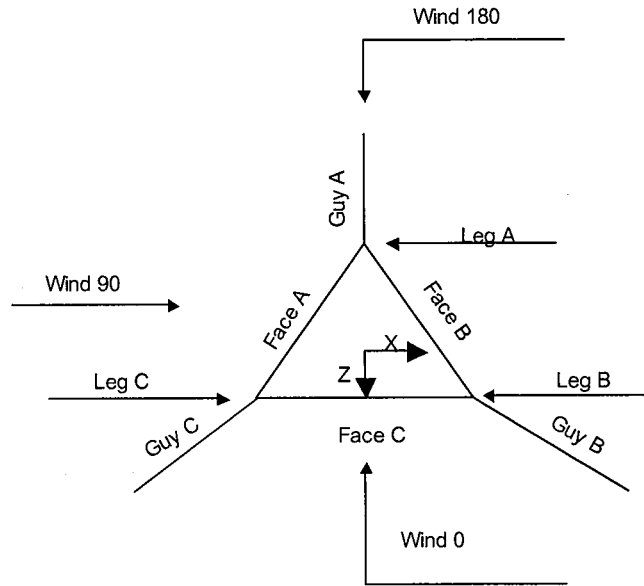
Pressures are calculated at each section.

Safety factor used in guy design is 1.

Stress ratio used in tower member design is 1.

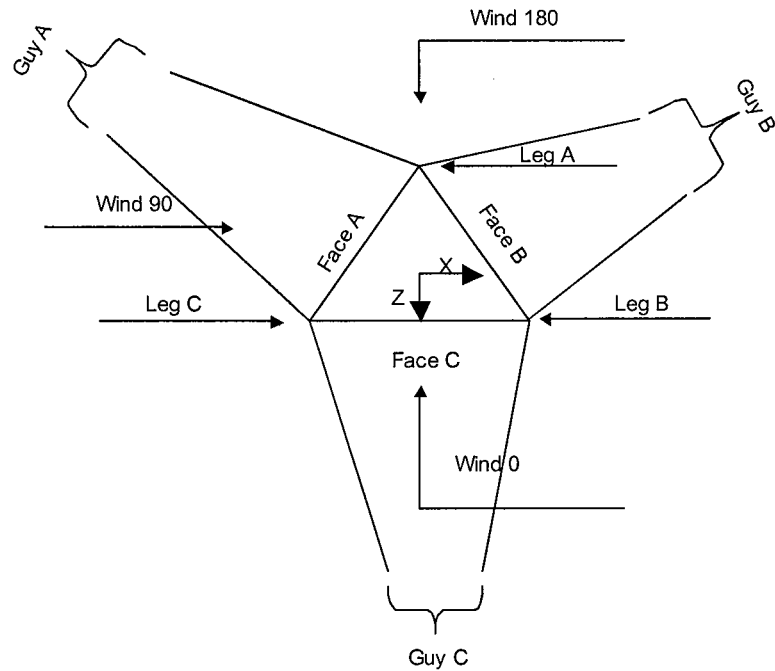
Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

tnxTower Ramaker & Associates, Inc. 1120 Dallas Street Sauk City, WI 53583 Phone: (608) 643-4100 FAX: (608) 643-7999	Job BANM Tower (CT03XC212)	Page 2 of 38
	Project 23002	Date 13:55:14 11/09/12
	Client Alcatel-Lucent / Sprint	Designed by tmoore



Corner & Starmount Guyed Tower

tnxTower Ramaker & Associates, Inc. 1120 Dallas Street Sauk City, WI 53583 Phone: (608) 643-4100 FAX: (608) 643-7999	Job BANM Tower (CT03XC212)	Page 3 of 38
	Project 23002	Date 13:55:14 11/09/12
	Client Alcatel-Lucent / Sprint	Designed by tmoore



Face Guyed

Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	<i>ft</i>			<i>ft</i>		<i>ft</i>
T1	150.00-140.00			1.50	1	10.00
T2	140.00-120.00			1.50	1	20.00
T3	120.00-100.00			1.50	1	20.00
T4	100.00-80.00			1.50	1	20.00
T5	80.00-60.00			1.50	1	20.00
T6	60.00-40.00			1.50	1	20.00
T7	40.00-20.00			1.50	1	20.00
T8	20.00-5.25			1.50	1	14.75
T9	5.25-0.00			1.50	1	5.25

Tower Section Geometry (cont'd)

tnxTower Ramaker & Associates, Inc. 1120 Dallas Street Sauk City, WI 53583 Phone: (608) 643-4100 FAX: (608) 643-7999	Job BANM Tower (CT03XC212)	Page 4 of 38
	Project 23002	Date 13:55:14 11/09/12
	Client Alcatel-Lucent / Sprint	Designed by tmoore

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	150.00-140.00	1.58	X Brace	No	Steps	0.0000	6.0000
T2	140.00-120.00	1.58	X Brace	No	Steps	6.0000	6.0000
T3	120.00-100.00	1.58	X Brace	No	Steps	6.0000	6.0000
T4	100.00-80.00	1.58	X Brace	No	Steps	6.0000	6.0000
T5	80.00-60.00	1.58	X Brace	No	Steps	6.0000	6.0000
T6	60.00-40.00	1.58	X Brace	No	Steps	6.0000	6.0000
T7	40.00-20.00	1.58	X Brace	No	Steps	6.0000	6.0000
T8	20.00-5.25	1.58	X Brace	No	Steps	6.0000	0.0000
T9	5.25-0.00	1.31	X Brace	No	Yes	0.0000	0.0000

Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
T1 150.00-140.00	Solid Round	1 3/4	A572-50 (50 ksi)	Solid Round	5/8	A36 (36 ksi)
T2 140.00-120.00	Solid Round	1 3/4	A572-50 (50 ksi)	Solid Round	5/8	A36 (36 ksi)
T3 120.00-100.00	Solid Round	1 3/4	A572-50 (50 ksi)	Solid Round	5/8	A36 (36 ksi)
T4 100.00-80.00	Solid Round	1 3/4	A572-50 (50 ksi)	Solid Round	5/8	A36 (36 ksi)
T5 80.00-60.00	Solid Round	1 3/4	A572-50 (50 ksi)	Solid Round	5/8	A36 (36 ksi)
T6 60.00-40.00	Solid Round	1 3/4	A572-50 (50 ksi)	Solid Round	5/8	A36 (36 ksi)
T7 40.00-20.00	Solid Round	1 3/4	A572-50 (50 ksi)	Solid Round	5/8	A36 (36 ksi)
T8 20.00-5.25	Solid Round	1 3/4	A572-50 (50 ksi)	Solid Round	5/8	A36 (36 ksi)
T9 5.25-0.00	Solid Round	1 3/4	A572-50 (50 ksi)	Solid Round	5/8	A36 (36 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
ft						
T1 150.00-140.00	Solid Round	3/4	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T2 140.00-120.00	Solid Round	3/4	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T3 120.00-100.00	Solid Round	3/4	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T4 100.00-80.00	Solid Round	3/4	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T5 80.00-60.00	Solid Round	3/4	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T6 60.00-40.00	Solid Round	3/4	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T7 40.00-20.00	Solid Round	3/4	A572-50 (50 ksi)	Solid Round	3/4	A572-50 (50 ksi)

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Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T8 20.00-5.25	Solid Round	3/4	(50 ksi) A572-50	Solid Round	3/4	(50 ksi) A572-50
T9 5.25-0.00	Solid Round		(50 ksi) A572-50 (50 ksi)	Solid Round	3/4	(50 ksi) A572-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T1 150.00-140.00	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T2 140.00-120.00	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T3 120.00-100.00	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T4 100.00-80.00	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T5 80.00-60.00	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T6 60.00-40.00	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T7 40.00-20.00	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T8 20.00-5.25	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)
T9 5.25-0.00	None	Flat Bar		A36 (36 ksi)	Solid Round	3/4	A572-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
T1 150.00-140.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T2 140.00-120.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T3 120.00-100.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T4 100.00-80.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T5 80.00-60.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T6 60.00-40.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000
T7 40.00-20.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000

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Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T9 5.25-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 150.00-140.00	Sleeve DS	0.6250	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T2 140.00-120.00	Sleeve DS	0.6250	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T3 120.00-100.00	Sleeve DS	0.6250	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T4 100.00-80.00	Sleeve DS	0.6250	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T5 80.00-60.00	Sleeve DS	0.6250	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T6 60.00-40.00	Sleeve DS	0.6250	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T7 40.00-20.00	Sleeve DS	0.6250	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T8 20.00-5.25	Sleeve DS	0.6250	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T9 5.25-0.00	Sleeve DS	0.6250	4	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	

Guy Data

Guy Elevation	Guy Grade	Guy Size	Initial Tension	%	Guy Modulus	Guy Weight	L_u	Anchor Radius	Anchor Azimuth Adj.	Anchor Elevation	End Fitting Efficiency
ft			lb		ksi	plf	ft	ft	°	ft	%
136	EHS	A	5/8	4240.00	10%	21000	0.813	145.66	75.00	0.0000	100%
		B	5/8	4240.00	10%	21000	0.813	154.38	75.00	0.0000	100%
		C	5/8	4240.00	10%	21000	0.813	143.24	47.00	0.0000	100%
96	EHS	A	1/2	2690.00	10%	21000	0.517	112.92	75.00	0.0000	100%
		B	1/2	2690.00	10%	21000	0.517	120.70	75.00	0.0000	100%
		C	1/2	2690.00	10%	21000	0.517	106.09	47.00	0.0000	100%
50	EHS	A	7/16	2080.00	10%	21000	0.399	84.17	75.00	0.0000	100%
		B	7/16	2080.00	10%	21000	0.399	89.34	75.00	0.0000	100%
		C	7/16	2080.00	10%	21000	0.399	67.97	47.00	0.0000	100%

Guy Data(cont'd)

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Guy Elevation ft	Mount Type	Torque-Arm Spread ft	Torque-Arm Leg Angle °	Torque-Arm Style	Torque-Arm Grade	Torque-Arm Type	Torque-Arm Size
136	Torque Arm	6.00	20.0000	Wing	A36 (36 ksi)	Pipe	P2.5x.203
96	Torque Arm	6.00	20.0000	Wing	A36 (36 ksi)	Pipe	P2.5x.203
50	Corner						

Guy Data (cont'd)

Guy Elevation ft	Diagonal Grade	Diagonal Type	Upper Diagonal Size	Lower Diagonal Size	Is Strap	Pull-Off Grade	Pull-Off Type	Pull-Off Size
136.00	A572-50 (50 ksi)	Solid Round			No	A572-50 (50 ksi)	Solid Round	3/4
96.00	A572-50 (50 ksi)	Solid Round			No	A572-50 (50 ksi)	Solid Round	3/4
50.00	A572-50 (50 ksi)	Solid Round			No	A572-50 (50 ksi)	Solid Round	3/4

Guy Data (cont'd)

Guy Elevation ft	Cable Weight		Cable Weight			Tower Intercept		Tower Intercept	
	A lb	B lb	C lb	D lb	A ft	B ft	C ft	D ft	
136	118.42	125.51	116.46		2.01 2.4 sec/pulse	2.26 2.6 sec/pulse	1.94 2.4 sec/pulse		
96	58.38	62.40	54.85		1.22 1.9 sec/pulse	1.39 2.0 sec/pulse	1.07 1.8 sec/pulse		
50	33.58	35.65	27.12		0.68 1.4 sec/pulse	0.76 1.5 sec/pulse	0.44 1.1 sec/pulse		

Guy Data (cont'd)

Guy Elevation ft	Calc K Single Angles	Calc K Solid Rounds	Torque Arm		Pull Off		Diagonal	
			K _x	K _y	K _x	K _y	K _x	K _y
136	No	Yes	1	1	1	1	1	1
96	No	Yes	1	1	1	1	1	1
50	No	No			1	1	1	1

Guy Data (cont'd)

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Guy Elevation ft	Torque-Arm				Pull Off				Diagonal			
	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U
136	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75
96	0.0000 A325N	0	0.0000	1	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75
50	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75

Guy Pressures

Guy Elevation ft	Guy Location	z ft	q- psf	q- Ice psf	Ice Thickness in
136	A	73.00	28	6	2.1653
	B	68.00	28	6	2.1500
	C	68.00	28	6	2.1500
96	A	53.00	27	6	2.0970
	B	48.00	26	6	2.0764
	C	48.00	26	6	2.0764
50	A	30.00	24	5	1.9810
	B	25.00	23	5	1.9452
	C	25.00	23	5	1.9452

Guy-Tensioning Information

Temperature At Time Of Tensioning																	
Guy Elevation ft	H ft	V ft	0 F		20 F		40 F		60 F		80 F		100 F		120 F		
			Initial Tension lb	Intercept ft	Initial Tension lb	Intercept ft	Initial Tension lb	Intercept ft	Initial Tension lb	Intercept ft	Initial Tension lb	Intercept ft	Initial Tension lb	Intercept ft	Initial Tension lb	Intercept ft	
136	A	73.33	126.00	4721	1.81	4561	1.87	4400	1.94	4240	2.01	4080	2.09	3921	2.17	3762	2.26
	B	73.33	136.00	4668	2.05	4525	2.12	4383	2.18	4240	2.26	4098	2.34	3956	2.42	3814	2.51
	C	45.37	136.00	4432	1.86	4368	1.89	4304	1.91	4240	1.94	4176	1.97	4112	2.00	4048	2.04
96	A	73.33	86.00	3199	1.02	3029	1.08	2859	1.14	2690	1.22	2521	1.30	2353	1.39	2187	1.49
	B	73.33	96.00	3136	1.19	2987	1.25	2838	1.32	2690	1.39	2542	1.47	2395	1.56	2249	1.66
	C	45.37	96.00	2913	0.99	2838	1.02	2764	1.04	2690	1.07	2616	1.10	2542	1.13	2468	1.17
50	A	74.13	40.00	2805	0.50	2563	0.55	2321	0.61	2080	0.68	1841	0.77	1604	0.88	1373	1.02
	B	74.13	50.00	2723	0.58	2508	0.63	2294	0.69	2080	0.76	1868	0.85	1657	0.96	1450	1.09
	C	46.13	50.00	2514	0.37	2369	0.39	2224	0.41	2080	0.44	1936	0.47	1792	0.51	1649	0.56

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
FLC 158-50J (1 5/8 FOAM) (147)	B	No	Ar (CaAa)	147.00 - 0.00	0.0000	0	6	6	1.0000	2.0200		0.92
HB114-1-08U4-M5J (147)	B	No	Ar (CaAa)	147.00 - 0.00	6.5000	0.24	3	3	1.5400	1.5400		1.08
7/8 (125)	A	No	Ar (CaAa)	125.00 - 0.00	0.0000	0.4	9	3	0.7500	1.1100		0.54

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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
1 1/4 (115)	B	No	Ar (CaAa)	115.00 - 0.00	3.5000	0.08	6	5	1.5000	1.5500		0.66

7/8 (150)	A	No	Ar (CaAa)	150.00 - 0.00	2.0000	0.05	2	2	1.1100	1.1100		0.54
7/8 (147)	A	No	Ar (CaAa)	147.00 - 0.00	0.0000	0.05	3	3	1.1100	1.1100		0.54
7/8 (120)	A	No	Ar (CaAa)	120.00 - 0.00	0.0000	-0.24	1	1	1.1100	1.1100		0.54
7/8 (96)	A	No	Ar (CaAa)	96.00 - 0.00	0.0000	-0.31	1	1	1.1100	1.1100		0.54
7/8 (80)	A	No	Ar (CaAa)	80.00 - 0.00	0.0000	-0.38	1	1	1.1100	1.1100		0.54
7/8 (55)	A	No	Ar (CaAa)	55.00 - 0.00	0.0000	-0.45	1	1	1.1100	1.1100		0.54
1/2 (50)	A	No	Ar (CaAa)	50.00 - 0.00	0.0000	-0.5	1	1	0.5800	0.5800		0.25
7/8 (35)	A	No	Ar (CaAa)	35.00 - 0.00	0.0000	-0.17	1	1	1.1100	1.1100		0.54

Safety Line 3/8 (150)	C	No	Ar (CaAa)	150.00 - 0.00	3.0000	0	1	1	0.3750	0.3750		0.22

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight lb
T1	150.00-140.00	A	0.000	0.000	4.551	0.000	22.14
		B	0.000	0.000	11.718	0.000	61.32
		C	0.000	0.000	0.375	0.000	2.20
T2	140.00-120.00	A	0.000	0.000	16.095	0.000	78.30
		B	0.000	0.000	33.480	0.000	175.20
		C	0.000	0.000	0.750	0.000	4.40
T3	120.00-100.00	A	0.000	0.000	33.300	0.000	162.00
		B	0.000	0.000	47.430	0.000	234.60
		C	0.000	0.000	0.750	0.000	4.40
T4	100.00-80.00	A	0.000	0.000	35.076	0.000	170.64
		B	0.000	0.000	52.080	0.000	254.40
		C	0.000	0.000	0.750	0.000	4.40
T5	80.00-60.00	A	0.000	0.000	37.740	0.000	183.60
		B	0.000	0.000	52.080	0.000	254.40
		C	0.000	0.000	0.750	0.000	4.40
T6	60.00-40.00	A	0.000	0.000	39.985	0.000	194.20
		B	0.000	0.000	52.080	0.000	254.40
		C	0.000	0.000	0.750	0.000	4.40
T7	40.00-20.00	A	0.000	0.000	42.785	0.000	207.50
		B	0.000	0.000	52.080	0.000	254.40
		C	0.000	0.000	0.750	0.000	4.40
T8	20.00-5.25	A	0.000	0.000	31.963	0.000	155.02
		B	0.000	0.000	38.409	0.000	187.62
		C	0.000	0.000	0.553	0.000	3.25
T9	5.25-0.00	A	0.000	0.000	11.377	0.000	55.18
		B	0.000	0.000	13.671	0.000	66.78
		C	0.000	0.000	0.197	0.000	1.16

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Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight lb
T1	150.00-140.00	A	2.319	0.000	0.000	22.517	0.000	297.37
		B		0.000	0.000	31.984	0.000	547.44
		C		0.000	0.000	5.013	0.000	78.53
T2	140.00-120.00	A	2.294	0.000	0.000	61.784	0.000	874.42
		B		0.000	0.000	91.055	0.000	1547.92
		C		0.000	0.000	9.926	0.000	153.99
T3	120.00-100.00	A	2.256	0.000	0.000	97.208	0.000	1545.50
		B		0.000	0.000	128.830	0.000	2213.05
		C		0.000	0.000	9.774	0.000	149.42
T4	100.00-80.00	A	2.211	0.000	0.000	104.946	0.000	1663.42
		B		0.000	0.000	140.706	0.000	2397.59
		C		0.000	0.000	9.594	0.000	144.12
T5	80.00-60.00	A	2.156	0.000	0.000	116.421	0.000	1835.59
		B		0.000	0.000	139.630	0.000	2342.55
		C		0.000	0.000	9.375	0.000	137.76
T6	60.00-40.00	A	2.085	0.000	0.000	126.748	0.000	1964.52
		B		0.000	0.000	138.233	0.000	2271.63
		C		0.000	0.000	9.089	0.000	129.71
T7	40.00-20.00	A	1.981	0.000	0.000	137.509	0.000	2072.13
		B		0.000	0.000	136.205	0.000	2169.73
		C		0.000	0.000	8.674	0.000	118.45
T8	20.00-5.25	A	1.817	0.000	0.000	97.859	0.000	1402.03
		B		0.000	0.000	98.093	0.000	1483.61
		C		0.000	0.000	5.913	0.000	75.00
T9	5.25-0.00	A	1.553	0.000	0.000	31.732	0.000	416.44
		B		0.000	0.000	33.574	0.000	463.49
		C		0.000	0.000	1.827	0.000	20.35

Feed Line Center of Pressure

Section	Elevation ft	CP _x in	CP _z in	CP _x Ice in	CP _z Ice in
T1	150.00-140.00	0.7093	-0.5637	0.0000	0.0000
T2	140.00-120.00	0.7452	-0.7321	0.0000	0.0000
T3	120.00-100.00	0.7676	-1.0020	0.0000	0.0000
T4	100.00-80.00	0.8036	-0.9756	0.0000	0.0000
T5	80.00-60.00	0.7261	-0.9307	0.0000	0.0000
T6	60.00-40.00	0.6582	-0.8845	0.0000	0.0000
T7	40.00-20.00	0.5889	-0.8492	0.0304	-0.0590
T8	20.00-5.25	0.5763	-0.8439	0.0566	-0.1052
T9	5.25-0.00	0.5059	-0.8619	0.0000	0.0000

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice

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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K_a No Ice	K_a Ice
T1	1	FLC 158-50J (1 5/8 FOAM)	140.00 - 147.00	0.6000	0.0000
T1	2	HB114-1-08U4-M5J	140.00 - 147.00	0.6000	0.0000
T1	6	7/8	140.00 - 150.00	0.6000	0.0000
T1	7	7/8	140.00 - 147.00	0.6000	0.0000
T1	15	Safety Line 3/8	140.00 - 150.00	0.6000	0.0000
T2	1	FLC 158-50J (1 5/8 FOAM)	120.00 - 140.00	0.6000	0.0000
T2	2	HB114-1-08U4-M5J	120.00 - 140.00	0.6000	0.0000
T2	3	7/8	120.00 - 125.00	0.6000	0.0000
T2	6	7/8	120.00 - 140.00	0.6000	0.0000
T2	7	7/8	120.00 - 140.00	0.6000	0.0000
T2	15	Safety Line 3/8	120.00 - 140.00	0.6000	0.0000
T3	1	FLC 158-50J (1 5/8 FOAM)	100.00 - 120.00	0.6000	0.0000
T3	2	HB114-1-08U4-M5J	100.00 - 120.00	0.6000	0.0000
T3	3	7/8	100.00 - 120.00	0.6000	0.0000
T3	4	1 1/4	100.00 - 115.00	0.6000	0.0000
T3	6	7/8	100.00 - 120.00	0.6000	0.0000
T3	7	7/8	100.00 - 120.00	0.6000	0.0000
T3	8	7/8	100.00 - 120.00	0.6000	0.0000
T3	15	Safety Line 3/8	100.00 - 120.00	0.6000	0.0000
T4	1	FLC 158-50J (1 5/8 FOAM)	80.00 - 100.00	0.6000	0.0000
T4	2	HB114-1-08U4-M5J	80.00 - 100.00	0.6000	0.0000
T4	3	7/8	80.00 - 100.00	0.6000	0.0000
T4	4	1 1/4	80.00 - 100.00	0.6000	0.0000
T4	6	7/8	80.00 - 100.00	0.6000	0.0000
T4	7	7/8	80.00 - 100.00	0.6000	0.0000
T4	8	7/8	80.00 - 100.00	0.6000	0.0000
T4	9	7/8	80.00 - 96.00	0.6000	0.0000
T4	15	Safety Line 3/8	80.00 - 100.00	0.6000	0.0000
T5	1	FLC 158-50J (1 5/8 FOAM)	60.00 - 80.00	0.6000	0.0000
T5	2	HB114-1-08U4-M5J	60.00 - 80.00	0.6000	0.0000
T5	3	7/8	60.00 - 80.00	0.6000	0.0000
T5	4	1 1/4	60.00 - 80.00	0.6000	0.0000
T5	6	7/8	60.00 - 80.00	0.6000	0.0000
T5	7	7/8	60.00 - 80.00	0.6000	0.0000
T5	8	7/8	60.00 - 80.00	0.6000	0.0000
T5	9	7/8	60.00 - 80.00	0.6000	0.0000
T5	10	7/8	60.00 - 80.00	0.6000	0.0000
T5	15	Safety Line 3/8	60.00 - 80.00	0.6000	0.0000
T6	1	FLC 158-50J (1 5/8 FOAM)	40.00 - 60.00	0.6000	0.0000
T6	2	HB114-1-08U4-M5J	40.00 - 60.00	0.6000	0.0000
T6	3	7/8	40.00 - 60.00	0.6000	0.0000
T6	4	1 1/4	40.00 - 60.00	0.6000	0.0000
T6	6	7/8	40.00 - 60.00	0.6000	0.0000
T6	7	7/8	40.00 - 60.00	0.6000	0.0000
T6	8	7/8	40.00 - 60.00	0.6000	0.0000
T6	9	7/8	40.00 - 60.00	0.6000	0.0000
T6	10	7/8	40.00 - 60.00	0.6000	0.0000
T6	11	7/8	40.00 - 55.00	0.6000	0.0000
T6	12	1/2	40.00 - 50.00	0.6000	0.0000
T6	15	Safety Line 3/8	40.00 - 60.00	0.6000	0.0000
T7	1	FLC 158-50J (1 5/8 FOAM)	20.00 - 40.00	0.6000	0.0342
T7	2	HB114-1-08U4-M5J	20.00 - 40.00	0.6000	0.0342
T7	3	7/8	20.00 - 40.00	0.6000	0.0342
T7	4	1 1/4	20.00 - 40.00	0.6000	0.0342
T7	6	7/8	20.00 - 40.00	0.6000	0.0342
T7	7	7/8	20.00 - 40.00	0.6000	0.0342
T7	8	7/8	20.00 - 40.00	0.6000	0.0342
T7	9	7/8	20.00 - 40.00	0.6000	0.0342
T7	10	7/8	20.00 - 40.00	0.6000	0.0342
T7	11	7/8	20.00 - 40.00	0.6000	0.0342
T7	12	1/2	20.00 - 40.00	0.6000	0.0342
T7	13	7/8	20.00 - 35.00	0.6000	0.0342

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Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
T7	15	Safety Line 3/8	20.00 - 40.00	0.6000	0.0342
T8	1	FLC 158-50J (1 5/8 FOAM)	5.25 - 20.00	0.6000	0.0675
T8	2	HB114-1-08U4-M5J	5.25 - 20.00	0.6000	0.0675
T8	3	7/8	5.25 - 20.00	0.6000	0.0675
T8	4	1 1/4	5.25 - 20.00	0.6000	0.0675
T8	6	7/8	5.25 - 20.00	0.6000	0.0675
T8	7	7/8	5.25 - 20.00	0.6000	0.0675
T8	8	7/8	5.25 - 20.00	0.6000	0.0675
T8	9	7/8	5.25 - 20.00	0.6000	0.0675
T8	10	7/8	5.25 - 20.00	0.6000	0.0675
T8	11	7/8	5.25 - 20.00	0.6000	0.0675
T8	12	1/2	5.25 - 20.00	0.6000	0.0675
T8	13	7/8	5.25 - 20.00	0.6000	0.0675
T8	15	Safety Line 3/8	5.25 - 20.00	0.6000	0.0675
T9	1	FLC 158-50J (1 5/8 FOAM)	0.00 - 5.25	0.5561	0.0000
T9	2	HB114-1-08U4-M5J	0.00 - 5.25	0.5561	0.0000
T9	3	7/8	0.00 - 5.25	0.5561	0.0000
T9	4	1 1/4	0.00 - 5.25	0.5561	0.0000
T9	6	7/8	0.00 - 5.25	0.5561	0.0000
T9	7	7/8	0.00 - 5.25	0.5561	0.0000
T9	8	7/8	0.00 - 5.25	0.5561	0.0000
T9	9	7/8	0.00 - 5.25	0.5561	0.0000
T9	10	7/8	0.00 - 5.25	0.5561	0.0000
T9	11	7/8	0.00 - 5.25	0.5561	0.0000
T9	12	1/2	0.00 - 5.25	0.5561	0.0000
T9	13	7/8	0.00 - 5.25	0.5561	0.0000
T9	15	Safety Line 3/8	0.00 - 5.25	0.5561	0.0000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			ft ft ft	°	ft	ft ²	ft ²	lb
DB420	A	From Face	0.50 0.00 0.00	0.0000	170.00	No Ice 3.33 1/2" Ice 5.99 1" Ice 8.66	3.33 5.99 8.66	34.00 44.20 54.40
5" Dia 20' Omni	A	From Face	0.50 0.00 0.00	0.0000	150.00	No Ice 10.00 1/2" Ice 12.05 1" Ice 14.12	10.00 12.05 14.12	60.00 128.11 209.10

DB420	A	From Leg	4.00 0.00 10.00	0.0000	147.00	No Ice 3.33 1/2" Ice 5.99 1" Ice 8.66	3.33 5.99 8.66	34.00 44.20 54.40
DB222	B	From Leg	4.00 0.00 10.00	0.0000	147.00	No Ice 1.60 1/2" Ice 2.88 1" Ice 4.16	1.60 2.88 4.16	16.00 20.80 25.60
DB420	C	From Leg	4.00 0.00 5.00	0.0000	147.00	No Ice 3.33 1/2" Ice 5.99 1" Ice 8.66	3.33 5.99 8.66	34.00 44.20 54.40
DB980H90E-M w/Mount Pipe	A	From Face	4.00 4.00	0.0000	147.00	No Ice 4.27 1/2" Ice 4.86	3.86 4.95	34.05 69.84

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement ft	C _A A _A Front ft ²	C _A A _A Side ft ²	Weight lb	
			Horz ft	Vert ft						
DB980H90E-M w/Mount Pipe	B	From Face	0.00		0.0000	147.00	1" Ice	5.37	5.75	116.19
			4.00				No Ice	4.27	3.86	34.05
			4.00				1/2" Ice	4.86	4.95	69.84
DB980H90E-M w/Mount Pipe	C	From Face	0.00		0.0000	147.00	1" Ice	5.37	5.75	116.19
			4.00				No Ice	4.27	3.86	34.05
			4.00				1/2" Ice	4.86	4.95	69.84
APXVSPP18-C-A20 w/Mount Pipe	A	From Face	0.00		0.0000	147.00	1" Ice	5.37	5.75	116.19
			4.00				No Ice	8.56	6.95	82.55
			-4.00				1/2" Ice	9.21	8.13	147.99
APXVSPP18-C-A20 w/Mount Pipe	B	From Face	0.00		0.0000	147.00	1" Ice	9.83	9.03	225.42
			4.00				No Ice	8.56	6.95	82.55
			-4.00				1/2" Ice	9.21	8.13	147.99
APXVSPP18-C-A20 w/Mount Pipe	C	From Face	0.00		0.0000	147.00	1" Ice	9.83	9.03	225.42
			4.00				No Ice	8.56	6.95	82.55
			-4.00				1/2" Ice	9.21	8.13	147.99
1900MHz 4x40W RRH	A	From Face	0.00		0.0000	147.00	1" Ice	9.83	9.03	225.42
			4.00				No Ice	2.71	2.61	59.50
			0.00				1/2" Ice	2.95	2.84	82.62
1900MHz 4x40W RRH	B	From Face	0.00		0.0000	147.00	1" Ice	3.20	3.09	108.98
			4.00				No Ice	2.71	2.61	59.50
			0.00				1/2" Ice	2.95	2.84	82.62
1900MHz 4x40W RRH	C	From Face	0.00		0.0000	147.00	1" Ice	3.20	3.09	108.98
			4.00				No Ice	2.71	2.61	59.50
			0.00				1/2" Ice	2.95	2.84	82.62
800MHz 2x50W RRH	A	From Face	0.00		0.0000	147.00	1" Ice	3.20	3.09	108.98
			4.00				No Ice	2.40	2.25	64.00
			0.00				1/2" Ice	2.61	2.46	86.12
800MHz 2x50W RRH	B	From Face	0.00		0.0000	147.00	1" Ice	2.83	2.68	111.30
			4.00				No Ice	2.40	2.25	64.00
			0.00				1/2" Ice	2.61	2.46	86.12
800MHz 2x50W RRH	C	From Face	0.00		0.0000	147.00	1" Ice	2.83	2.68	111.30
			4.00				No Ice	2.40	2.25	64.00
			0.00				1/2" Ice	2.61	2.46	86.12
Pirod Delta Mount (3)	C	None	0.00		0.0000	147.00	1" Ice	2.83	2.68	111.30
							No Ice	32.94	32.94	740.00
							1/2" Ice	47.60	47.60	1000.00
***** 10' Omni	B	From Face	0.00		0.0000	130.00	1" Ice	62.26	62.26	1260.00
			4.00				No Ice	2.50	2.50	30.00
			0.00				1/2" Ice	3.53	3.53	48.64
4' Standoff	B	From Face	5.00		0.0000	130.00	1" Ice	4.58	4.58	73.79
			2.00				No Ice	2.72	2.72	50.00
			0.00				1/2" Ice	4.91	4.91	89.00
***** MB96RR900200DPBL w/Mount Pipe	A	From Leg	0.00		0.0000	125.00	1" Ice	7.10	7.10	128.00
			6.00				No Ice	11.47	9.48	65.20
			0.00				1/2" Ice	12.08	10.90	148.67
MB96RR900200DPBL w/Mount Pipe	B	From Leg	0.00		0.0000	125.00	1" Ice	12.71	12.17	246.45
			6.00				No Ice	11.47	9.48	65.20
			0.00				1/2" Ice	12.08	10.90	148.67
FS90-11-00NAL2 w/Mount Pipe	C	From Leg	0.00		0.0000	125.00	1" Ice	12.71	12.17	246.45
			6.00				No Ice	11.47	9.48	65.20
			0.00				1/2" Ice	12.08	10.90	148.67
CG DD1900W800	A	From Leg	0.00		0.0000	125.00	1" Ice	12.71	12.17	246.45
			4.00				No Ice	1.28	0.31	12.10
			0.00				1/2" Ice	1.44	0.41	19.30
			0.00				1" Ice	1.60	0.52	28.37

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight lb
CG DD1900W800	B	From Leg	4.00 0.00 0.00	0.0000	125.00	No Ice 1.28 1/2" Ice 1.44 1" Ice 1.60	0.31 0.41 0.52	12.10 19.30 28.37
CG DD1900W800	C	From Leg	4.00 0.00 0.00	0.0000	125.00	No Ice 1.28 1/2" Ice 1.44 1" Ice 1.60	0.31 0.41 0.52	12.10 19.30 28.37
Pirod Delta Mount (3)	C	None		0.0000	125.00	No Ice 32.94 1/2" Ice 47.60 1" Ice 62.26	32.94 47.60 62.26	740.00 1000.00 1260.00
***** DB225-A	C	From Leg	1.00 0.00 0.00	0.0000	120.00	No Ice 3.21 1/2" Ice 5.78 1" Ice 8.35	3.21 5.78 8.35	37.00 48.10 59.20
***** 6' x 8" Panel Antenna w/Mount Pipe	A	From Leg	1.00 0.00 0.00	0.0000	115.00	No Ice 6.10 1/2" Ice 6.67 1" Ice 7.19	6.36 7.54 8.43	65.55 118.26 182.63
6' x 8" Panel Antenna w/Mount Pipe	B	From Leg	1.00 0.00 0.00	0.0000	115.00	No Ice 6.10 1/2" Ice 6.67 1" Ice 7.19	6.36 7.54 8.43	65.55 118.26 182.63
6' x 8" Panel Antenna w/Mount Pipe	C	From Leg	1.00 0.00 0.00	0.0000	115.00	No Ice 6.10 1/2" Ice 6.67 1" Ice 7.19	6.36 7.54 8.43	65.55 118.26 182.63
***** 10' Omni	B	From Leg	2.50 0.00 -5.00	0.0000	96.00	No Ice 2.50 1/2" Ice 3.53 1" Ice 4.58	2.50 3.53 4.58	30.00 48.64 73.79
***** 15' Omni	B	From Face	4.00 0.00 7.00	0.0000	80.00	No Ice 4.50 1/2" Ice 6.03 1" Ice 7.58	4.50 6.03 7.58	40.00 72.48 114.58
4' Standoff	B	From Face	2.00 0.00 0.00	0.0000	80.00	No Ice 2.72 1/2" Ice 4.91 1" Ice 7.10	2.72 4.91 7.10	50.00 89.00 128.00
***** DB225-A	C	From Leg	1.00 0.00 0.00	0.0000	55.00	No Ice 3.21 1/2" Ice 5.78 1" Ice 8.35	3.21 5.78 8.35	37.00 48.10 59.20
***** GPS	A	From Leg	3.00 0.00 0.00	0.0000	50.00	No Ice 1.00 1/2" Ice 1.50 1" Ice 2.00	1.00 1.50 2.00	10.00 15.00 20.00
3' Standoff	A	From Leg	0.00 0.00 0.00	0.0000	50.00	No Ice 2.00 1/2" Ice 3.70 1" Ice 5.40	2.00 3.70 5.40	38.00 67.00 96.00

Dishes

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Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft ²	Weight lb	
4 FT DISH	C	Paraboloid w/o Radome	From Leg	0.00 0.00 0.00	0.0000		35.00	4.00	No Ice 1/2" Ice 1" Ice	12.56 13.09 13.62	170.00 237.19 304.38

Force Totals (Does not include forces on guys)

Load Case	Vertical Forces lb	Sum of Forces X lb	Sum of Forces Z lb	Sum of Torques lb-ft
Leg Weight	3684.84			
Bracing Weight	2027.34			
Total Member Self-Weight	5712.19			
Guy Weight	1168.38			
Total Weight	13184.12			
Wind 0 deg - No Ice		339.52	-14338.97	946.57
Wind 30 deg - No Ice		7714.51	-12943.83	164.11
Wind 60 deg - No Ice		13607.13	-7856.08	-772.38
Wind 90 deg - No Ice		15795.27	-209.04	-1520.87
Wind 120 deg - No Ice		12565.58	6862.69	-1674.86
Wind 150 deg - No Ice		6183.20	10715.70	-1102.38
Wind 180 deg - No Ice		-43.47	14066.72	-649.29
Wind 210 deg - No Ice		-7483.86	12937.26	87.58
Wind 240 deg - No Ice		-13351.89	7708.72	766.88
Wind 270 deg - No Ice		-15674.26	12.59	1269.19
Wind 300 deg - No Ice		-12325.36	-7065.85	1387.35
Wind 330 deg - No Ice		-6188.47	-10712.66	1102.38
Member Ice	15684.41			
Guy Ice	12339.52			
Total Weight Ice	76535.90			
Wind 0 deg - Ice		90.06	-6396.45	157.17
Wind 30 deg - Ice		3256.48	-5529.48	-125.55
Wind 60 deg - Ice		5570.34	-3216.04	-388.44
Wind 90 deg - Ice		6416.91	-55.45	-547.25
Wind 120 deg - Ice		5584.52	3120.23	-545.61
Wind 150 deg - Ice		3173.74	5498.69	-338.00
Wind 180 deg - Ice		-11.53	6368.21	-80.90
Wind 210 deg - Ice		-3195.30	5527.74	192.31
Wind 240 deg - Ice		-5540.73	3198.94	388.44
Wind 270 deg - Ice		-6384.81	3.34	480.49
Wind 300 deg - Ice		-5520.80	-3174.12	469.34
Wind 330 deg - Ice		-3175.14	-5497.88	338.00
Total Weight	13184.12			
Wind 0 deg - Service		110.87	-4682.11	309.08
Wind 30 deg - Service		2519.02	-4226.56	53.59
Wind 60 deg - Service		4443.14	-2565.25	-252.21
Wind 90 deg - Service		5157.64	-68.26	-496.61
Wind 120 deg - Service		4103.05	2240.88	-546.89
Wind 150 deg - Service		2019.00	3499.00	-359.96
Wind 180 deg - Service		-14.20	4593.21	-212.01
Wind 210 deg - Service		-2443.71	4224.41	28.60
Wind 240 deg - Service		-4359.80	2517.13	250.41
Wind 270 deg - Service		-5118.13	4.11	414.43
Wind 300 deg - Service		-4024.61	-2307.22	453.01
Wind 330 deg - Service		-2020.73	-3498.01	359.96

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Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice+1.0 Guy
3	1.2 Dead+1.6 Wind 30 deg - No Ice+1.0 Guy
4	1.2 Dead+1.6 Wind 60 deg - No Ice+1.0 Guy
5	1.2 Dead+1.6 Wind 90 deg - No Ice+1.0 Guy
6	1.2 Dead+1.6 Wind 120 deg - No Ice+1.0 Guy
7	1.2 Dead+1.6 Wind 150 deg - No Ice+1.0 Guy
8	1.2 Dead+1.6 Wind 180 deg - No Ice+1.0 Guy
9	1.2 Dead+1.6 Wind 210 deg - No Ice+1.0 Guy
10	1.2 Dead+1.6 Wind 240 deg - No Ice+1.0 Guy
11	1.2 Dead+1.6 Wind 270 deg - No Ice+1.0 Guy
12	1.2 Dead+1.6 Wind 300 deg - No Ice+1.0 Guy
13	1.2 Dead+1.6 Wind 330 deg - No Ice+1.0 Guy
14	1.2 Dead+1.0 Ice+1.0 Temp+Guy
15	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp+1.0 Guy
16	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp+1.0 Guy
17	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp+1.0 Guy
18	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp+1.0 Guy
19	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp+1.0 Guy
20	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp+1.0 Guy
21	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp+1.0 Guy
22	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp+1.0 Guy
23	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp+1.0 Guy
24	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp+1.0 Guy
25	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp+1.0 Guy
26	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp+1.0 Guy
27	Dead+Wind 0 deg - Service+Guy
28	Dead+Wind 30 deg - Service+Guy
29	Dead+Wind 60 deg - Service+Guy
30	Dead+Wind 90 deg - Service+Guy
31	Dead+Wind 120 deg - Service+Guy
32	Dead+Wind 150 deg - Service+Guy
33	Dead+Wind 180 deg - Service+Guy
34	Dead+Wind 210 deg - Service+Guy
35	Dead+Wind 240 deg - Service+Guy
36	Dead+Wind 270 deg - Service+Guy
37	Dead+Wind 300 deg - Service+Guy
38	Dead+Wind 330 deg - Service+Guy

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
T1	150 - 140	Leg	Max Tension	12	30579.04	-703.00	-417.89
			Max. Compression	2	-32150.82	6.87	50.71
			Max. Mx	5	-957.17	861.24	6.05
			Max. My	2	12529.19	-28.39	-901.96
			Max. Vy	5	1723.83	0.28	4.22
		Diagonal	Max. Vx	8	1835.44	24.35	-16.70
			Max Tension	3	4005.69	0.00	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft	
T2	140 - 120	Horizontal	Max. Compression	3	-3985.38	0.00	0.00	
			Max. Mx	15	1196.41	-1.95	-0.24	
			Max. My	9	-3975.56	1.26	1.91	
			Max. Vy	15	5.41	-1.95	-0.24	
			Max. Vx	9	-1.75	1.26	1.91	
			Max Tension	8	678.77	0.00	0.00	
			Top Girt	Max. Compression	2	-591.72	0.00	0.00
				Max. Mx	21	56.55	2.95	0.00
				Max. My	4	-19.53	0.00	-0.00
				Max. Vy	21	-7.87	0.00	0.00
				Max. Vx	4	0.00	0.00	0.00
				Max Tension	10	134.44	0.00	0.00
		Bottom Girt		Max. Compression	4	-154.33	0.00	0.00
				Max. Mx	14	0.26	2.95	0.00
				Max. My	16	11.55	0.00	0.00
				Max. Vy	14	7.87	0.00	0.00
				Max. Vx	16	0.00	0.00	0.00
				Max Tension	8	1489.11	0.00	0.00
			Leg	Max. Compression	2	-1552.71	0.00	0.00
				Max. Mx	14	40.32	2.95	0.00
				Max. My	4	-621.96	0.00	0.00
				Max. Vy	14	7.87	0.00	0.00
				Max. Vx	4	-0.00	0.00	0.00
				Max Tension	12	34027.23	774.95	461.03
		Diagonal		Max. Compression	10	-44901.01	102.87	-17.56
				Max. Mx	11	-1339.29	868.27	0.05
				Max. My	8	-16502.61	-1.82	-934.23
				Max. Vy	5	1728.01	-862.67	2.37
				Max. Vx	8	1836.19	-1.82	-934.23
				Max Tension	3	4897.34	0.00	0.00
			Horizontal	Max. Compression	5	-5208.03	-1.72	0.96
				Max. Mx	10	-2639.61	-3.46	0.65
				Max. My	9	-3695.35	2.28	3.05
				Max. Vy	17	6.45	-3.20	0.10
				Max. Vx	9	-2.80	1.95	3.05
				Max Tension	2	1772.47	0.00	0.00
		Top Girt		Max. Compression	1	0.00	0.00	0.00
				Max. Mx	21	894.02	2.91	0.00
				Max. My	5	1248.02	0.00	-0.00
				Max. Vy	21	-7.75	0.00	0.00
				Max. Vx	5	0.00	0.00	0.00
				Max Tension	10	631.25	0.00	0.00
Bottom Girt	Max. Compression		4	-885.03	0.00	0.00		
	Max. Mx		14	-65.12	2.91	0.00		
	Max. My		4	124.45	0.00	0.00		
	Max. Vy		14	7.75	0.00	0.00		
	Max. Vx		4	-0.00	0.00	0.00		
	Max Tension		21	641.70	0.00	0.00		
	Guy A	Max. Compression	1	0.00	0.00	0.00		
		Max. Mx	14	224.51	2.91	0.00		
		Max. My	5	596.32	0.00	-0.00		
		Max. Vy	14	7.75	0.00	0.00		
		Max. Vx	5	0.00	0.00	0.00		
		Bottom Tension	9	13151.02				
Top Tension		9	13251.84					
Top Cable Vert		9	11506.77					
Top Cable Norm		9	6573.00					
Top Cable Tan		9	36.09					
Bot Cable Vert		9	-11271.41					
Bot Cable Norm		9	6773.95					
Bot Cable Tan	9	134.27						

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
		Guy B	Bottom Tension	11	13972.42		
			Top Tension	11	14081.14		
			Top Cable Vert	11	12444.10		
			Top Cable Norm	11	6589.49		
			Top Cable Tan	11	40.23		
			Bot Cable Vert	11	-12199.86		
			Bot Cable Norm	11	6809.73		
			Bot Cable Tan	11	139.79		
		Guy C	Bottom Tension	5	19555.39		
			Top Tension	5	19661.92		
			Top Cable Vert	5	18644.19		
			Top Cable Norm	5	6243.61		
			Top Cable Tan	5	50.84		
			Bot Cable Vert	5	-18445.73		
			Bot Cable Norm	5	6489.50		
			Bot Cable Tan	5	233.71		
		Top Guy Pull-Off	Max Tension	4	8047.59	0.00	0.00
			Max. Compression	10	-3436.56	0.00	0.00
			Max. Mx	14	1969.56	2.91	0.00
			Max. My	5	2486.84	0.00	-0.00
			Max. Vy	14	7.75	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00
		Bottom Guy Pull-Off	Max Tension	8	4574.87	0.00	0.00
			Max. Compression	10	-6004.70	0.00	0.00
			Max. Mx	17	-41.65	2.91	0.00
			Max. My	5	-698.06	0.00	-0.00
			Max. Vy	17	7.75	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00
		Torque Arm Top	Max Tension	2	18478.35	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	16	8555.37	30.76	0.00
			Max. My	17	10790.19	0.00	-0.09
			Max. Vy	16	-33.58	0.00	0.00
			Max. Vx	17	0.10	0.00	0.00
		Torque Arm Bottom	Max Tension	13	4962.70	0.00	0.00
			Max. Compression	3	-20848.17	0.00	0.00
			Max. Mx	17	-3067.31	28.22	0.00
			Max. My	17	-12014.43	0.00	0.04
			Max. Vy	17	-33.56	0.00	0.00
			Max. Vx	17	-0.04	0.00	0.00
T3	120 - 100	Leg	Max Tension	4	19634.23	630.68	-449.70
			Max. Compression	2	-57378.44	-2.66	75.21
			Max. Mx	5	-52746.98	762.81	-32.40
			Max. My	2	2379.28	122.46	-749.61
			Max. Vy	5	1643.17	-58.28	-29.29
			Max. Vx	2	-1548.09	-12.48	23.98
		Diagonal	Max Tension	3	3036.48	0.00	0.00
			Max. Compression	5	-3857.75	0.00	0.00
			Max. Mx	2	2356.42	-4.08	0.01
			Max. My	2	-1462.98	1.37	-0.84
			Max. Vy	15	6.81	-3.65	0.11
			Max. Vx	2	-0.77	1.37	-0.84
		Horizontal	Max Tension	21	1577.05	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	22	1447.82	2.84	0.00
			Max. My	5	1231.22	0.00	-0.00
			Max. Vy	22	-7.57	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00
		Top Girt	Max Tension	2	987.26	0.00	0.00
			Max. Compression	4	-233.06	0.00	0.00
			Max. Mx	14	146.85	2.84	0.00

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
T4	100 - 80	Bottom Girt	Max. My	5	731.46	0.00	-0.00
			Max. Vy	14	7.57	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00
			Max Tension	5	1584.59	0.00	0.00
			Max. Compression	6	-1103.97	0.00	0.00
			Max. Mx	23	-4.34	2.84	0.00
		Leg	Max. My	5	645.79	0.00	-0.00
			Max. Vy	23	7.57	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00
			Max Tension	4	23737.81	-709.48	486.31
			Max. Compression	2	-67576.11	-7.68	3.24
			Max. Mx	5	-52758.35	-872.23	-22.93
		Diagonal	Max. My	2	-59213.30	-160.39	811.76
			Max. Vy	5	1628.86	-872.23	-22.93
			Max. Vx	2	-1550.21	-147.88	798.69
			Max Tension	3	2633.34	0.00	0.00
			Max. Compression	3	-4015.39	0.00	0.00
			Max. Mx	2	-2245.78	-5.18	0.42
		Horizontal	Max. My	6	-2383.67	1.69	2.65
			Max. Vy	23	7.47	-4.50	0.52
			Max. Vx	6	2.43	1.69	2.65
			Max Tension	6	2260.81	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	25	1731.66	2.76	0.00
		Top Girt	Max. My	6	2068.85	0.00	-0.00
			Max. Vy	25	-7.35	0.00	0.00
			Max. Vx	6	0.00	0.00	0.00
			Max Tension	2	1045.79	0.00	0.00
			Max. Compression	4	-176.78	0.00	0.00
			Max. Mx	23	582.52	2.76	0.00
		Bottom Girt	Max. My	2	137.19	0.00	-0.00
			Max. Vy	23	7.35	0.00	0.00
			Max. Vx	2	0.00	0.00	0.00
			Max Tension	2	1247.85	0.00	0.00
			Max. Compression	12	-130.97	0.00	0.00
			Max. Mx	17	325.13	2.76	0.00
		Guy A	Max. My	6	755.59	0.00	-0.00
			Max. Vy	17	7.35	0.00	0.00
			Max. Vx	6	0.00	0.00	0.00
			Bottom Tension	9	7754.65		
			Top Tension	9	7798.51		
			Top Cable Vert	9	5971.72		
Top Cable Norm	9		5015.46				
Top Cable Tan	9		24.00				
Bot Cable Vert	9		-5842.20				
Bot Cable Norm	9		5098.90				
Bot Cable Tan	9		67.26				
Guy B	Bottom Tension		11	8380.28			
	Top Tension	11	8429.20				
	Top Cable Vert	11	6734.64				
	Top Cable Norm	11	5069.06				
	Top Cable Tan	11	26.58				
	Bot Cable Vert	11	-6597.56				
	Bot Cable Norm	11	5166.84				
	Bot Cable Tan	11	71.48				
	Guy C	Bottom Tension	5	12476.24			
		Top Tension	5	12524.30			
		Top Cable Vert	5	11323.05			
		Top Cable Norm	5	5352.22			
Top Cable Tan		5	19.95				
Bot Cable Vert		5	-11211.89				

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft	
T5	80 - 60	Top Guy Pull-Off	Bot Cable Norm	5	5471.39			
			Bot Cable Tan	5	117.51			
			Max Tension	4	7046.42	0.00	0.00	
			Max. Compression	6	-2611.96	0.00	0.00	
			Max. Mx	23	2605.95	2.76	0.00	
			Max. My	2	4217.34	0.00	-0.00	
			Max. Vy	23	7.35	0.00	0.00	
			Max. Vx	2	0.00	0.00	0.00	
			Bottom Guy Pull-Off	Max Tension	8	4792.37	0.00	0.00
				Max. Compression	6	-4865.47	0.00	0.00
				Max. Mx	23	-1433.53	2.76	0.00
				Max. My	2	1674.15	0.00	-0.00
		Max. Vy		23	7.35	0.00	0.00	
		Max. Vx		2	0.00	0.00	0.00	
		Torque Arm Top	Max Tension	2	10698.71	0.00	0.00	
			Max. Compression	1	0.00	0.00	0.00	
			Max. Mx	17	5524.81	29.67	0.00	
			Max. My	2	6086.29	0.00	0.07	
			Max. Vy	17	-32.40	0.00	0.00	
			Max. Vx	2	-0.08	0.00	0.00	
		Torque Arm Bottom	Max Tension	13	4888.82	0.00	0.00	
			Max. Compression	3	-14420.86	0.00	0.00	
			Max. Mx	17	-2053.14	27.24	0.00	
			Max. My	16	-6947.49	0.00	-0.03	
			Max. Vy	17	-32.39	0.00	0.00	
			Max. Vx	16	-0.03	0.00	0.00	
			Leg	Max Tension	1	0.00	0.00	0.00
				Max. Compression	17	-38979.35	-13.49	-60.87
				Max. Mx	11	-21315.34	-323.74	73.60
				Max. My	2	-30738.81	-22.12	-357.71
				Max. Vy	6	-881.55	93.73	62.86
				Max. Vx	2	845.36	29.97	-136.06
		Diagonal		Max Tension	8	866.76	0.00	0.00
				Max. Compression	5	-2534.05	0.00	0.00
				Max. Mx	17	-626.41	-4.52	-0.55
				Max. My	6	-1531.61	2.88	1.28
				Max. Vy	17	7.37	-4.52	-0.55
				Max. Vx	6	1.17	0.00	0.00
		Horizontal	Max Tension	16	2314.37	0.00	0.00	
			Max. Compression	1	0.00	0.00	0.00	
			Max. Mx	24	2148.93	2.66	0.00	
			Max. My	5	1688.93	0.00	-0.00	
Max. Vy	24		-7.09	0.00	0.00			
Max. Vx	5		0.00	0.00	0.00			
Top Girt	Max Tension	21	1137.88	0.00	0.00			
	Max. Compression	10	-69.55	0.00	0.00			
	Max. Mx	17	425.15	2.66	0.00			
	Max. My	6	1126.44	0.00	-0.00			
	Max. Vy	17	7.09	0.00	0.00			
	Max. Vx	6	0.00	0.00	0.00			
Bottom Girt	Max Tension	21	1137.62	0.00	0.00			
	Max. Compression	7	-10.28	0.00	0.00			
	Max. Mx	22	382.89	2.66	0.00			
	Max. My	5	847.17	0.00	-0.00			
	Max. Vy	22	7.09	0.00	0.00			
	Max. Vx	5	0.00	0.00	0.00			
T6	60 - 40	Leg	Max Tension	1	0.00	0.00	0.00	
			Max. Compression	6	-46579.48	-69.75	-130.02	
			Max. Mx	5	-27143.69	-570.47	-48.25	
			Max. My	2	-22574.71	-82.07	605.78	
			Max. Vy	5	-1171.90	-566.46	59.15	

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
		Diagonal	Max. Vx	2	1138.65	-82.07	605.78
			Max Tension	3	1920.34	0.00	0.00
			Max. Compression	5	-3640.35	0.00	0.00
			Max. Mx	16	-391.78	-5.07	-0.64
			Max. My	3	-2149.79	1.62	-1.21
		Horizontal	Max. Vy	16	7.71	-5.07	-0.64
			Max. Vx	3	-1.12	0.00	0.00
			Max Tension	16	2651.80	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	25	2576.02	2.54	0.00
		Top Girt	Max. My	5	1817.58	0.00	-0.00
			Max. Vy	25	-6.77	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00
			Max Tension	2	1182.60	0.00	0.00
			Max. Compression	12	-132.72	0.00	0.00
		Bottom Girt	Max. Mx	21	399.35	2.54	0.00
			Max. My	5	938.67	0.00	-0.00
			Max. Vy	21	-6.77	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00
			Max Tension	2	1951.17	0.00	0.00
		Guy A	Max. Compression	4	-485.07	0.00	0.00
			Max. Mx	21	537.88	2.54	0.00
			Max. My	5	1156.32	0.00	-0.00
			Max. Vy	21	-6.77	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00
		Guy B	Bottom Tension	9	8438.51	0.00	0.00
			Top Tension	9	8454.23	0.00	0.00
			Top Cable Vert	9	4029.10	0.00	0.00
			Top Cable Norm	9	7432.38	0.00	0.00
			Top Cable Tan	9	7.80	0.00	0.00
		Guy C	Bot Cable Vert	9	-3967.87	0.00	0.00
			Bot Cable Norm	9	7447.30	0.00	0.00
			Bot Cable Tan	9	46.00	0.00	0.00
			Bottom Tension	11	8944.61	0.00	0.00
			Top Tension	11	8964.26	0.00	0.00
		Top Guy Pull-Off	Top Cable Vert	11	5025.81	0.00	0.00
			Top Cable Norm	11	7422.89	0.00	0.00
			Top Cable Tan	11	5.72	0.00	0.00
			Bot Cable Vert	11	-4956.47	0.00	0.00
			Bot Cable Norm	11	7445.61	0.00	0.00
		Leg	Bot Cable Tan	11	47.67	0.00	0.00
			Bottom Tension	5	11562.07	0.00	0.00
			Top Tension	5	11581.54	0.00	0.00
			Top Cable Vert	5	8498.26	0.00	0.00
			Top Cable Norm	5	7868.32	0.00	0.00
		T7	Top Cable Tan	5	34.12	0.00	0.00
			Bot Cable Vert	5	-8439.10	0.00	0.00
			Bot Cable Norm	5	7903.04	0.00	0.00
			Bot Cable Tan	5	71.23	0.00	0.00
			Max Tension	2	4512.26	0.00	0.00
		40 - 20	Max. Compression	1	0.00	0.00	0.00
			Max. Mx	16	2318.41	2.54	0.00
			Max. My	5	3557.85	0.00	-0.00
			Max. Vy	16	-6.77	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00
			Max Tension	10	2938.90	-40.76	37.70
			Max. Compression	3	-57097.87	28.95	-93.91
			Max. Mx	5	-40733.76	601.85	-173.63
			Max. My	2	-41493.02	89.43	-580.44
			Max. Vy	5	-1166.23	19.09	-60.77
			Max. Vx	2	1140.67	-0.45	37.07

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft		
T8	20 - 5.25	Diagonal	Max Tension	3	1530.96	0.00	0.00		
			Max. Compression	5	-3564.14	0.00	0.00		
			Max. Mx	5	-865.25	-5.41	0.13		
			Max. My	9	-2185.01	3.11	-1.51		
			Max. Vy	21	7.75	-5.36	-0.69		
			Max. Vx	9	-1.38	0.00	0.00		
		Horizontal	Max Tension	2	3325.23	0.00	0.00		
			Max. Compression	1	0.00	0.00	0.00		
			Max. Mx	24	2533.33	2.37	0.00		
			Max. My	6	1853.77	0.00	-0.00		
			Max. Vy	24	-6.31	0.00	0.00		
			Max. Vx	6	0.00	0.00	0.00		
		Top Girt	Max Tension	21	1307.80	0.00	0.00		
			Max. Compression	6	-252.93	0.00	0.00		
			Max. Mx	21	399.64	2.37	0.00		
			Max. My	5	959.39	0.00	-0.00		
			Max. Vy	21	-6.31	0.00	0.00		
			Max. Vx	5	0.00	0.00	0.00		
		Bottom Girt	Max Tension	2	1530.91	0.00	0.00		
			Max. Compression	1	0.00	0.00	0.00		
			Max. Mx	23	469.76	2.37	0.00		
			Max. My	6	911.74	0.00	-0.00		
			Max. Vy	23	-6.31	0.00	0.00		
			Max. Vx	6	0.00	0.00	0.00		
		Leg		Max Tension	10	895.52	82.93	-86.45	
					3	-57098.87	-24.79	-20.93	
					6	-40024.77	154.66	-0.58	
					6	-40516.19	-126.37	-162.31	
					11	-251.79	74.25	-66.75	
					2	-266.65	-26.14	42.86	
				Diagonal	Max Tension	9	1157.88	0.00	0.00
					Max. Compression	2	-2501.07	0.00	0.00
					Max. Mx	5	-355.38	4.95	-0.46
					Max. My	6	-1408.27	-3.18	-1.16
					Max. Vy	20	6.91	-4.81	0.38
					Max. Vx	6	1.08	-3.18	-1.16
				Horizontal	Max Tension	2	3162.04	0.00	0.00
					Max. Compression	1	0.00	0.00	0.00
					Max. Mx	26	2925.54	2.11	0.00
					Max. My	5	1964.37	0.00	-0.00
Max. Vy	26				5.63	0.00	0.00		
Max. Vx	5				0.00	0.00	0.00		
Top Girt	Max Tension			2	1773.54	0.00	0.00		
	Max. Compression			12	-23.80	0.00	0.00		
	Max. Mx	26	1447.52	2.11	0.00				
	Max. My	6	850.33	0.00	-0.00				
	Max. Vy	26	5.63	0.00	0.00				
	Max. Vx	6	0.00	0.00	0.00				
Bottom Girt	Max Tension	16	6127.33	0.00	0.00				
	Max. Compression	1	0.00	0.00	0.00				
	Max. Mx	14	5167.08	2.11	0.00				
	Max. My	5	4611.50	0.00	-0.00				
	Max. Vy	14	5.63	0.00	0.00				
	Max. Vx	5	0.00	0.00	0.00				
Leg		Max Tension	1	0.00	0.00	0.00			
			17	-47223.71	-11.29	0.78			
		Max. Compression	2	-19797.77	136.66	-16.91			
			6	-41232.04	-20.10	-378.45			
		Max. Vy	2	130.15	-21.50	254.62			
			6	400.04	-15.80	-376.88			
		Max. Vx	1	0.00	0.00	0.00			
			1	0.00	0.00	0.00			
		Diagonal	Max Tension	1	0.00	0.00	0.00		
				1	0.00	0.00	0.00		

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial lb	Major Axis Moment lb-ft	Minor Axis Moment lb-ft
		Horizontal	Max. Compression	6	-4255.07	0.00	0.00
			Max. Mx	5	-3430.21	4.00	-2.90
			Max. My	6	-3269.24	3.65	-2.94
			Max. Vy	5	8.51	0.00	0.00
			Max. Vx	6	-6.16	3.65	-2.94
			Max Tension	18	3116.34	0.00	0.00
			Max. Compression	1	0.00	0.00	0.00
			Max. Mx	14	2452.26	0.98	0.00
			Max. My	5	1957.65	0.00	-0.00
			Max. Vy	14	3.47	0.00	0.00
			Max. Vx	5	0.00	0.00	0.00

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical lb	Horizontal, X lb	Horizontal, Z lb
Mast	Max. Vert	17	135802.58	107.67	-96.97
	Max. H _x	11	82172.02	1423.00	57.91
	Max. H _z	13	93283.69	826.81	866.67
	Max. M _x	1	0.00	38.63	-18.14
	Max. M _z	1	0.00	38.63	-18.14
	Max. Torsion	6	263.45	-659.62	-492.22
	Min. Vert	28	51670.09	-142.24	283.36
	Min. H _x	4	86036.61	-1056.08	656.65
	Min. H _z	8	70213.90	65.80	-1586.52
	Min. M _x	1	0.00	38.63	-18.14
	Min. M _z	1	0.00	38.63	-18.14
	Min. Torsion	2	-177.74	73.87	749.65
	Max. Vert	10	-3826.78	-882.24	510.75
	Guy C @ 47 ft Elev 0 ft Azimuth 240 deg	Max. H _x	10	-3826.78	-882.24
Max. H _z		3	-65897.55	-26562.65	16134.42
Min. Vert		5	-66232.50	-27370.81	15040.80
Min. H _x		5	-66232.50	-27370.81	15040.80
Min. H _z		9	-4749.09	-1402.42	494.12
Max. Vert		6	-1786.06	604.66	354.58
Guy B @ 75 ft Elev 0 ft Azimuth 120 deg	Max. H _x	11	-41751.90	26989.90	15096.13
	Max. H _z	13	-40907.76	25958.64	15639.97
	Min. Vert	11	-41751.90	26989.90	15096.13
	Min. H _x	6	-1786.06	604.66	354.58
	Min. H _z	6	-1786.06	604.66	354.58
	Max. Vert	2	-1396.66	-3.58	-571.26
Guy A @ 75 ft Elev 10 ft Azimuth 0 deg	Max. H _x	11	-20221.78	750.16	-16169.64
	Max. H _z	2	-1396.66	-3.58	-571.26
	Min. Vert	9	-37800.47	412.46	-30931.14
	Min. H _x	6	-31371.27	-873.16	-25485.13
	Min. H _z	9	-37800.47	412.46	-30931.14

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Tower Mast Reaction Summary

Load Combination	Vertical	Shear _x	Shear _z	Overturning Moment, M _x	Overturning Moment, M _z	Torque
	lb	lb	lb	lb-ft	lb-ft	lb-ft
Dead Only	52071.19	-38.63	18.14	0.00	0.00	0.61
1.2 Dead+1.6 Wind 0 deg - No Ice+1.0 Guy	107521.36	-73.87	-749.65	0.00	0.00	177.74
1.2 Dead+1.6 Wind 30 deg - No Ice+1.0 Guy	104480.14	569.87	-624.74	0.00	0.00	71.43
1.2 Dead+1.6 Wind 60 deg - No Ice+1.0 Guy	86036.61	1056.08	-656.65	0.00	0.00	-130.80
1.2 Dead+1.6 Wind 90 deg - No Ice+1.0 Guy	103333.09	839.95	-173.66	0.00	0.00	-253.52
1.2 Dead+1.6 Wind 120 deg - No Ice+1.0 Guy	105106.08	659.62	492.22	0.00	0.00	-263.45
1.2 Dead+1.6 Wind 150 deg - No Ice+1.0 Guy	90637.40	376.46	1214.12	0.00	0.00	-93.33
1.2 Dead+1.6 Wind 180 deg - No Ice+1.0 Guy	70213.90	-65.80	1586.52	0.00	0.00	-27.55
1.2 Dead+1.6 Wind 210 deg - No Ice+1.0 Guy	80735.86	-684.52	1312.49	0.00	0.00	54.47
1.2 Dead+1.6 Wind 240 deg - No Ice+1.0 Guy	87468.90	-1125.75	673.10	0.00	0.00	99.92
1.2 Dead+1.6 Wind 270 deg - No Ice+1.0 Guy	82172.02	-1423.00	-57.91	0.00	0.00	126.84
1.2 Dead+1.6 Wind 300 deg - No Ice+1.0 Guy	70930.11	-1375.41	-735.31	0.00	0.00	138.40
1.2 Dead+1.6 Wind 330 deg - No Ice+1.0 Guy	93283.69	-826.81	-866.67	0.00	0.00	101.99
1.2 Dead+1.0 Ice+1.0 Temp+Guy	129319.80	-227.76	168.70	0.00	0.00	0.40
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp+1.0 Guy	133897.15	-239.00	-1.25	0.00	0.00	17.93
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp+1.0 Guy	135356.46	-172.28	21.43	0.00	0.00	9.25
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp+1.0 Guy	135802.58	-107.67	96.97	0.00	0.00	-17.89
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp+1.0 Guy	134589.94	-70.08	193.43	0.00	0.00	-40.39
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp+1.0 Guy	132542.34	-79.17	268.02	0.00	0.00	-35.59
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp+1.0 Guy	131341.89	-138.44	326.38	0.00	0.00	-1.45
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp+1.0 Guy	130772.01	-230.54	338.88	0.00	0.00	8.13
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp+1.0 Guy	129911.89	-326.98	317.36	0.00	0.00	12.73
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp+1.0 Guy	129212.83	-387.98	264.38	0.00	0.00	17.59
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp+1.0 Guy	130087.85	-401.26	183.30	0.00	0.00	17.73
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp+1.0 Guy	131262.19	-371.60	88.51	0.00	0.00	9.54
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp+1.0 Guy	132341.91	-316.37	18.58	0.00	0.00	1.93
Dead+Wind 0 deg - Service+Guy	51751.26	-19.41	-329.98	0.00	0.00	33.79
Dead+Wind 30 deg - Service+Guy	51670.09	142.24	-283.36	0.00	0.00	11.96
Dead+Wind 60 deg - Service+Guy	51767.07	258.92	-155.70	0.00	0.00	-22.16
Dead+Wind 90 deg - Service+Guy	51830.48	309.38	10.50	0.00	0.00	-50.75
Dead+Wind 120 deg - Service+Guy	52036.13	268.19	175.40	0.00	0.00	-53.40
Dead+Wind 150 deg - Service+Guy	52430.77	130.80	315.94	0.00	0.00	-18.31
Dead+Wind 180 deg - Service+Guy	52838.09	-36.32	359.00	0.00	0.00	-4.85
Dead+Wind 210 deg - Service+Guy	53056.48	-203.58	316.61	0.00	0.00	13.33
Dead+Wind 240 deg - Service+Guy	53045.11	-325.11	185.57	0.00	0.00	22.75
Dead+Wind 270 deg - Service+Guy	52869.78	-376.57	13.52	0.00	0.00	27.23
Dead+Wind 300 deg - Service+Guy	52523.32	-329.21	-154.72	0.00	0.00	26.71
Dead+Wind 330 deg - Service+Guy	52075.36	-208.71	-280.14	0.00	0.00	19.19

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX lb	PY lb	PZ lb	PX lb	PY lb	PZ lb	
1	-0.00	-13183.94	0.00	1.02	13184.04	-0.49	0.009%
2	635.09	-15611.38	-27521.85	-633.96	15611.14	27515.26	0.021%
3	14507.22	-15495.53	-24286.62	-14506.06	15495.31	24280.63	0.019%
4	24460.87	-15407.92	-14107.18	-24459.27	15407.91	14109.72	0.009%
5	28317.69	-15478.18	-412.05	-28313.91	15478.03	413.11	0.012%
6	24180.08	-15580.15	13211.73	-24176.16	15580.00	-13210.55	0.013%
7	13826.23	-15569.73	23940.98	-13822.92	15569.63	-23939.95	0.011%
8	-161.41	-15562.79	27616.79	163.94	15562.78	-27616.70	0.008%
9	-14138.19	-15678.64	24276.12	14134.59	15678.55	-24274.44	0.012%
10	-24052.50	-15766.25	13871.41	24049.31	15766.16	-13869.88	0.011%
11	-28124.07	-15695.99	97.72	28121.63	15695.93	-96.52	0.008%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX lb	PY lb	PZ lb	PX lb	PY lb	PZ lb	
12	-24025.46	-15594.01	-13669.42	24026.70	15594.01	13667.20	0.008%
13	-13834.67	-15604.44	-23936.11	13835.29	15604.32	23932.18	0.013%
14	-0.00	-78936.98	0.00	0.22	78936.97	-0.45	0.001%
15	204.31	-78969.75	-9831.72	-204.31	78969.72	9830.92	0.001%
16	5161.70	-78826.40	-8614.19	-5161.79	78826.38	8613.61	0.001%
17	8811.04	-78717.69	-5065.25	-8810.84	78717.64	5064.22	0.001%
18	10085.56	-78807.77	-151.15	-10085.14	78807.75	151.41	0.001%
19	8656.70	-78935.85	4740.23	-8656.01	78935.82	-4739.89	0.001%
20	4853.64	-78918.35	8384.55	-4852.80	78918.33	-8384.23	0.001%
21	-125.78	-78904.21	9803.48	126.51	78904.20	-9803.18	0.001%
22	-5100.52	-79047.56	8612.45	5101.27	79047.55	-8611.92	0.001%
23	-8781.42	-79156.27	5048.16	8781.08	79156.26	-5048.24	0.000%
24	-10053.46	-79066.19	99.04	10053.17	79066.17	-100.64	0.002%
25	-8592.98	-78938.11	-4794.12	8593.02	78938.10	4792.85	0.002%
26	-4855.04	-78955.61	-8383.74	4855.10	78955.59	8382.93	0.001%
27	129.61	-13188.90	-5616.70	-129.33	13188.89	5615.69	0.007%
28	2960.66	-13165.25	-4956.45	-2959.54	13165.25	4955.44	0.011%
29	4992.01	-13147.37	-2879.02	-4990.13	13147.37	2878.10	0.015%
30	5779.12	-13161.71	-84.09	-5777.56	13161.71	83.73	0.011%
31	4934.71	-13182.52	2696.27	-4933.67	13182.52	-2696.11	0.007%
32	2821.68	-13180.40	4885.91	-2820.93	13180.40	-4885.58	0.006%
33	-32.94	-13178.98	5636.08	33.30	13178.98	-5635.68	0.004%
34	-2885.34	-13202.62	4954.31	2885.11	13202.62	-4953.93	0.003%
35	-4908.67	-13220.50	2830.90	4908.11	13220.50	-2830.67	0.004%
36	-5739.61	-13206.16	19.94	5739.02	13206.16	-20.02	0.004%
37	-4903.16	-13185.35	-2789.68	4902.83	13185.35	2789.17	0.004%
38	-2823.40	-13187.48	-4884.92	2823.34	13187.48	4884.12	0.006%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	13	0.00000001	0.00011155
2	Yes	26	0.00014357	0.00013209
3	Yes	26	0.00014076	0.00013794
4	Yes	26	0.00014178	0.00014561
5	Yes	26	0.00009601	0.00009991
6	Yes	26	0.00009707	0.00009290
7	Yes	25	0.00010760	0.00008018
8	Yes	21	0.00013521	0.00007045
9	Yes	21	0.00013024	0.00014049
10	Yes	22	0.00009612	0.00012400
11	Yes	22	0.00008137	0.00009953
12	Yes	24	0.00014185	0.00009700
13	Yes	26	0.00010954	0.00008293
14	Yes	18	0.00000001	0.00007740
15	Yes	22	0.00011743	0.00004640
16	Yes	22	0.00009955	0.00004947
17	Yes	19	0.00013036	0.00011623
18	Yes	21	0.00010767	0.00005930
19	Yes	21	0.00000001	0.00004619
20	Yes	20	0.00000001	0.00005190
21	Yes	19	0.00000001	0.00005183
22	Yes	17	0.00000001	0.00010260
23	Yes	17	0.00000001	0.00009982
24	Yes	17	0.00000001	0.00010869

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25	Yes	19	0.0000001	0.00005768
26	Yes	21	0.0000001	0.00004254
27	Yes	12	0.0000001	0.00010647
28	Yes	12	0.0000001	0.00008553
29	Yes	12	0.0000001	0.00008424
30	Yes	12	0.0000001	0.00008781
31	Yes	12	0.0000001	0.00009937
32	Yes	12	0.0000001	0.00008871
33	Yes	12	0.0000001	0.00010112
34	Yes	12	0.0000001	0.00011838
35	Yes	12	0.0000001	0.00013057
36	Yes	12	0.0000001	0.00013057
37	Yes	12	0.0000001	0.00011849
38	Yes	12	0.0000001	0.00010364

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	150 - 140	3.089	29	0.1384	0.0758
T2	140 - 120	2.812	29	0.1111	0.0417
T3	120 - 100	2.379	29	0.1375	0.0343
T4	100 - 80	1.728	29	0.1236	0.0354
T5	80 - 60	1.359	29	0.0844	0.1148
T6	60 - 40	0.981	29	0.0851	0.1473
T7	40 - 20	0.757	29	0.0337	0.1628
T8	20 - 5.25	0.524	30	0.0928	0.1589
T9	5.25 - 0	0.159	30	0.1363	0.1462

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
170.00	DB420	29	3.089	0.1384	0.0758	13836
150.00	5" Dia 20' Omni	29	3.089	0.1384	0.0758	13836
147.00	DB420	29	3.001	0.1308	0.0644	13836
136.00	Guy	29	2.723	0.1058	0.0373	9163
130.00	10' Omni	29	2.605	0.1157	0.0355	17729
125.00	MB96RR900200DPBL w/Mount Pipe	29	2.503	0.1276	0.0354	14280
120.00	DB225-A	29	2.379	0.1375	0.0343	9574
115.00	6' x 8" Panel Antenna w/Mount Pipe	29	2.224	0.1415	0.0310	16617
96.00	Guy	29	1.632	0.1140	0.0473	10518
80.00	15' Omni	29	1.359	0.0844	0.1148	26081
55.00	DB225-A	29	0.909	0.0723	0.1517	21881
50.00	Guy	29	0.850	0.0554	0.1561	31281
35.00	4 FT DISH	29	0.714	0.0390	0.1640	23617

Maximum Tower Deflections - Design Wind

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	150 - 140	23.881	3	1.2827	0.6012
T2	140 - 120	21.239	3	1.1509	0.4340
T3	120 - 100	16.882	3	1.1102	0.2958
T4	100 - 80	12.185	3	0.9155	0.2586
T5	80 - 60	9.447	3	0.5468	0.6287
T6	60 - 40	7.574	2	0.4881	0.7519
T7	40 - 20	6.203	2	0.3118	0.8080
T8	20 - 5.25	4.108	2	0.7628	0.7871
T9	5.25 - 0	1.215	2	1.0487	0.7231

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
170.00	DB420	3	23.881	1.2827	0.6012	3156
150.00	5" Dia 20' Omni	3	23.881	1.2827	0.6012	3156
147.00	DB420	3	23.060	1.2379	0.5460	3156
136.00	Guy	3	20.311	1.1218	0.3901	2230
130.00	10' Omni	3	19.029	1.1048	0.3471	5578
125.00	MB96RR900200DPBL w/Mount Pipe	3	17.983	1.1102	0.3218	4864
120.00	DB225-A	3	16.882	1.1102	0.2958	2753
115.00	6' x 8" Panel Antenna w/Mount Pipe	3	15.681	1.0929	0.2627	5832
96.00	Guy	3	11.479	0.8362	0.3136	1734
80.00	15' Omni	3	9.447	0.5468	0.6287	9155
55.00	DB225-A	2	7.205	0.4352	0.7664	5082
50.00	Guy	2	6.870	0.3700	0.7825	8196
35.00	4 FT DISH	2	5.821	0.3808	0.8123	3210

Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt lb	Allowable Load lb	Ratio Load Allowable	Allowable Ratio	Criteria
T1	150	Leg	A325N	0.6250	4	680.28	24850.50	0.027 ✓	1	Bolt DS
T2	140	Leg	A325N	0.6250	4	7643.25	24850.50	0.308 ✓	1	Bolt DS
T3	120	Leg	A325N	0.6250	4	0.00	24850.50	0.000 ✓	1	Bolt DS
T4	100	Leg	A325N	0.6250	4	4908.29	24850.50	0.198 ✓	1	Bolt DS
T5	80	Leg	A325N	0.6250	4	0.00	24850.50	0.000 ✓	1	Bolt DS
T6	60	Leg	A325N	0.6250	4	0.00	24850.50	0.000 ✓	1	Bolt DS
T7	40	Leg	A325N	0.6250	4	0.00	24850.50	0.000 ✓	1	Bolt DS
T8	20	Leg	A325N	0.6250	4	174.16	24850.50	0.007 ✓	1	Bolt DS
T9	5.25	Leg	A325N	0.6250	4	0.00	24850.50	0.000 ✓	1	Bolt DS

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Guy Design Data

Section No.	Elevation ft	Size	Initial Tension lb	Breaking Load lb	Actual T_u lb	Allowable ϕT_n lb	Required S.F.	Actual S.F.
T2	136.00 (A) (720)	5/8 EHS	4240.00	42399.99	13251.80	25440.00	1.000	1.920 ✓
	136.00 (A) (721)	5/8 EHS	4240.00	42399.99	13060.00	25440.00	1.000	1.948 ✓
	136.00 (B) (714)	5/8 EHS	4240.00	42399.99	13624.30	25440.00	1.000	1.867 ✓
	136.00 (B) (715)	5/8 EHS	4240.00	42399.99	14081.10	25440.00	1.000	1.807 ✓
	136.00 (C) (704)	5/8 EHS	4240.00	42399.99	19661.90	25440.00	1.000	1.294 ✓
	136.00 (C) (705)	5/8 EHS	4240.00	42399.99	19387.80	25440.00	1.000	1.312 ✓
T4	96.00 (A) (742)	1/2 EHS	2690.00	26900.04	7798.51	16140.00	1.000	2.070 ✓
	96.00 (A) (743)	1/2 EHS	2690.00	26900.04	7672.17	16140.00	1.000	2.104 ✓
	96.00 (B) (736)	1/2 EHS	2690.00	26900.04	8226.18	16140.00	1.000	1.962 ✓
	96.00 (B) (737)	1/2 EHS	2690.00	26900.04	8429.20	16140.00	1.000	1.915 ✓
	96.00 (C) (726)	1/2 EHS	2690.00	26900.04	12524.30	16140.00	1.000	1.289 ✓
	96.00 (C) (727)	1/2 EHS	2690.00	26900.04	12433.00	16140.00	1.000	1.298 ✓
T6	50.00 (A) (752)	7/16 EHS	2080.00	20800.02	8454.23	12480.00	1.000	1.476 ✓
	50.00 (B) (751)	7/16 EHS	2080.00	20800.02	8964.26	12480.00	1.000	1.392 ✓
	50.00 (C) (748)	7/16 EHS	2080.00	20800.02	11581.50	12480.00	1.000	1.078 ✓

Compression Checks

Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	A in ²	Mast Stability Index	P_u lb	ϕP_n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	150 - 140	1 3/4	10.00	1.58	43.4 K=1.00	2.4053	1.00	-32150.80	94294.90	0.341 ¹ ✓
T2	140 - 120	1 3/4	20.00	1.58	43.4 K=1.00	2.4053	0.96	-44901.00	90507.10	0.496 ¹ ✓
T3	120 - 100	1 3/4	20.00	1.58	43.4 K=1.00	2.4053	0.96	-57378.40	90266.90	0.636 ¹ ✓
T4	100 - 80	1 3/4	20.00	1.58	43.4 K=1.00	2.4053	0.93	-67576.10	87434.40	0.773 ¹ ✓
T5	80 - 60	1 3/4	20.00	1.58	43.4 K=1.00	2.4053	0.76	-38979.40	71902.10	0.542 ¹ ✓
T6	60 - 40	1 3/4	20.00	1.58	43.4 K=1.00	2.4053	0.71	-44060.30	66673.50	0.661 ¹ ✓
T7	40 - 20	1 3/4	20.00	1.58	43.4 K=1.00	2.4053	0.72	-48288.40	67658.00	0.714 ¹ ✓
T8	20 - 5.25	1 3/4	14.75	1.58	43.4 K=1.00	2.4053	0.71	-48232.90	67418.70	0.715 ¹ ✓

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	Mast Stability Index	P _u lb	φP _n lb	Ratio P _u / φP _n
T9	5.25 - 0	1 3/4	5.32	1.33	36.5 K=1.00	2.4053	0.68	-46976.60	66371.00	0.708 ¹ ✓

¹ P_u / φP_n controls

Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio P _u / φP _n
T1	150 - 140	5/8	2.18	0.98	75.6 K=1.00	0.3068	-3985.38	7356.83	0.542 ¹ ✓
T2	140 - 120	5/8	2.18	0.98	75.6 K=1.00	0.3068	-5208.03	7356.83	0.708 ¹ ✓
T3	120 - 100	5/8	2.18	0.98	75.6 K=1.00	0.3068	-3857.75	7356.83	0.524 ¹ ✓
T4	100 - 80	5/8	2.18	0.98	75.6 K=1.00	0.3068	-4015.39	7356.83	0.546 ¹ ✓
T5	80 - 60	5/8	2.18	0.98	75.6 K=1.00	0.3068	-2534.05	7356.83	0.344 ¹ ✓
T6	60 - 40	5/8	2.18	0.98	75.6 K=1.00	0.3068	-3640.35	7356.83	0.495 ¹ ✓
T7	40 - 20	5/8	2.18	0.98	75.6 K=1.00	0.3068	-3564.14	7356.83	0.484 ¹ ✓
T8	20 - 5.25	5/8	2.18	0.98	75.6 K=1.00	0.3068	-2501.07	7356.83	0.340 ¹ ✓
T9	5.25 - 0	5/8	1.43	0.77	59.3 K=1.00	0.3068	-4255.07	8259.22	0.515 ¹ ✓

¹ P_u / φP_n controls

Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio P _u / φP _n
T1	150 - 140	3/4	1.50	1.35	82.3 K=0.95	0.4418	-591.72	12110.70	0.049 ¹ ✓

¹ P_u / φP_n controls

Top Girt Design Data (Compression)

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	150 - 140	3/4	1.50	1.35	82.3 K=0.95	0.4418	-154.33	12110.70	0.013 ¹
T2	140 - 120	3/4	1.50	1.35	82.3 K=0.95	0.4418	-885.03	12110.70	0.073 ¹
T3	120 - 100	3/4	1.50	1.35	82.3 K=0.95	0.4418	-233.06	12110.70	0.019 ¹
T4	100 - 80	3/4	1.50	1.35	82.3 K=0.95	0.4418	-176.78	12110.70	0.015 ¹
T5	80 - 60	3/4	1.50	1.35	82.3 K=0.95	0.4418	-69.55	12110.70	0.006 ¹
T6	60 - 40	3/4	1.50	1.35	82.3 K=0.95	0.4418	-132.72	12110.70	0.011 ¹
T7	40 - 20	3/4	1.50	1.35	82.3 K=0.95	0.4418	-252.93	12110.70	0.021 ¹
T8	20 - 5.25	3/4	1.50	1.35	82.3 K=0.95	0.4418	-23.80	12110.70	0.002 ¹

¹ P_u / φP_n controls

Bottom Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	150 - 140	3/4	1.50	1.35	82.3 K=0.95	0.4418	-1552.71	12110.70	0.128 ¹
T3	120 - 100	3/4	1.50	1.35	82.3 K=0.95	0.4418	-1103.97	12110.70	0.091 ¹
T4	100 - 80	3/4	1.50	1.35	82.3 K=0.95	0.4418	-130.98	12110.70	0.011 ¹
T5	80 - 60	3/4	1.50	1.35	82.3 K=0.95	0.4418	-10.28	12110.70	0.001 ¹
T6	60 - 40	3/4	1.50	1.35	82.3 K=0.95	0.4418	-485.07	12110.70	0.040 ¹

¹ P_u / φP_n controls

Top Guy Pull-Off Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T2	140 - 120	3/4	1.50	1.35	82.3 K=0.95	0.4418	-3436.56	12110.70	0.284 ¹
T4	100 - 80	3/4	1.50	1.35	82.3 K=0.95	0.4418	-2611.96	12110.70	0.216 ¹

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
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¹ P_u / φP_n controls

Bottom Guy Pull-Off Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T2	140 - 120	3/4	1.50	1.35	82.3 K=0.95	0.4418	-6004.70	12110.70	0.496 ¹ ✓
T4	100 - 80	3/4	1.50	1.35	82.3 K=0.95	0.4418	-4865.47	12110.70	0.402 ¹ ✓

¹ P_u / φP_n controls

Torque-Arm Bottom Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T2	140 - 120 (710)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-20848.20	50402.20	0.414 ¹ ✓
T2	140 - 120 (711)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-15500.00	50402.20	0.308 ¹ ✓
T2	140 - 120 (718)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-16038.80	50402.20	0.318 ¹ ✓
T2	140 - 120 (719)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-20810.30	50402.20	0.413 ¹ ✓
T2	140 - 120 (724)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-16201.50	50402.20	0.321 ¹ ✓
T2	140 - 120 (725)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-15584.20	50402.20	0.309 ¹ ✓
T4	100 - 80 (732)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-14420.90	50402.20	0.286 ¹ ✓
T4	100 - 80 (733)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-9618.70	50402.20	0.191 ¹ ✓
T4	100 - 80 (740)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-10274.30	50402.20	0.204 ¹ ✓
T4	100 - 80 (741)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-14294.70	50402.20	0.284 ¹ ✓
T4	100 - 80 (746)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-10646.80	50402.20	0.211 ¹ ✓
T4	100 - 80 (747)	P2.5x.203	3.36	3.28	41.6 K=1.00	1.7040	-9871.78	50402.20	0.196 ¹ ✓

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¹ $P_u / \phi P_n$ controls

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	150 - 140	1 3/4	10.00	1.58	43.4	1.2339	30579.00	60150.90	0.508 ¹
T2	140 - 120	1 3/4	20.00	1.58	43.4	1.2339	34027.20	60150.90	0.566 ¹
T3	120 - 100	1 3/4	20.00	1.58	43.4	1.2339	19634.20	60150.90	0.326 ¹
T4	100 - 80	1 3/4	20.00	1.58	43.4	1.2339	23737.80	60150.90	0.395 ¹
T7	40 - 20	1 3/4	20.00	1.58	43.4	1.2339	2938.90	60150.90	0.049 ¹
T8	20 - 5.25	1 3/4	14.75	1.58	43.4	1.2339	895.52	60150.90	0.015 ¹

¹ $P_u / \phi P_n$ controls

Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	150 - 140	5/8	2.18	0.98	75.6	0.3068	4005.69	9940.20	0.403 ¹
T2	140 - 120	5/8	2.18	0.98	75.6	0.3068	4897.34	9940.20	0.493 ¹
T3	120 - 100	5/8	2.18	0.98	75.6	0.3068	3036.48	9940.20	0.305 ¹
T4	100 - 80	5/8	2.18	0.98	75.6	0.3068	2633.34	9940.20	0.265 ¹
T5	80 - 60	5/8	2.18	0.98	75.6	0.3068	866.76	9940.20	0.087 ¹
T6	60 - 40	5/8	2.18	0.98	75.6	0.3068	1920.34	9940.20	0.193 ¹
T7	40 - 20	5/8	2.18	0.98	75.6	0.3068	1530.96	9940.20	0.154 ¹
T8	20 - 5.25	5/8	2.18	0.98	75.6	0.3068	1157.88	9940.20	0.116 ¹

¹ $P_u / \phi P_n$ controls

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Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	150 - 140	3/4	1.50	1.35	86.7	0.4418	678.77	19880.40	0.034 ¹
T2	140 - 120	3/4	1.50	1.35	86.7	0.4418	1772.47	19880.40	0.089 ¹
T3	120 - 100	3/4	1.50	1.35	86.7	0.4418	1577.05	19880.40	0.079 ¹
T4	100 - 80	3/4	1.50	1.35	86.7	0.4418	2260.81	19880.40	0.114 ¹
T5	80 - 60	3/4	1.50	1.35	86.7	0.4418	2314.37	19880.40	0.116 ¹
T6	60 - 40	3/4	1.50	1.35	86.7	0.4418	2651.80	19880.40	0.133 ¹
T7	40 - 20	3/4	1.50	1.35	86.7	0.4418	3325.23	19880.40	0.167 ¹
T8	20 - 5.25	3/4	1.50	1.35	86.7	0.4418	3162.04	19880.40	0.159 ¹
T9	5.25 - 0	3/4	0.75	0.60	38.7	0.4418	3116.34	19880.40	0.157 ¹

¹ P_u / φP_n controls

Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	150 - 140	3/4	1.50	1.35	86.7	0.4418	134.44	19880.40	0.007 ¹
T2	140 - 120	3/4	1.50	1.35	86.7	0.4418	631.25	19880.40	0.032 ¹
T3	120 - 100	3/4	1.50	1.35	86.7	0.4418	987.26	19880.40	0.050 ¹
T4	100 - 80	3/4	1.50	1.35	86.7	0.4418	1045.79	19880.40	0.053 ¹
T5	80 - 60	3/4	1.50	1.35	86.7	0.4418	1137.88	19880.40	0.057 ¹
T6	60 - 40	3/4	1.50	1.35	86.7	0.4418	1182.60	19880.40	0.059 ¹
T7	40 - 20	3/4	1.50	1.35	86.7	0.4418	1307.80	19880.40	0.066 ¹
T8	20 - 5.25	3/4	1.50	1.35	86.7	0.4418	1773.54	19880.40	0.089 ¹

¹ P_u / φP_n controls

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Bottom Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	150 - 140	3/4	1.50	1.35	86.7	0.4418	1489.11	19880.40	0.075 ¹
T2	140 - 120	3/4	1.50	1.35	86.7	0.4418	641.70	19880.40	0.032 ¹
T3	120 - 100	3/4	1.50	1.35	86.7	0.4418	1584.59	19880.40	0.080 ¹
T4	100 - 80	3/4	1.50	1.35	86.7	0.4418	1247.85	19880.40	0.063 ¹
T5	80 - 60	3/4	1.50	1.35	86.7	0.4418	1137.62	19880.40	0.057 ¹
T6	60 - 40	3/4	1.50	1.35	86.7	0.4418	1951.17	19880.40	0.098 ¹
T7	40 - 20	3/4	1.50	1.35	86.7	0.4418	1530.91	19880.40	0.077 ¹
T8	20 - 5.25	3/4	1.50	1.35	86.7	0.4418	6127.33	19880.40	0.308 ¹

¹ P_u / φP_n controls

Top Guy Pull-Off Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T2	140 - 120	3/4	1.50	1.35	86.7	0.4418	8047.59	19880.40	0.405 ¹
T4	100 - 80	3/4	1.50	1.35	86.7	0.4418	7046.42	19880.40	0.354 ¹
T6	60 - 40	3/4	1.50	1.35	86.7	0.4418	4512.26	19880.40	0.227 ¹

¹ P_u / φP_n controls

Bottom Guy Pull-Off Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T2	140 - 120	3/4	1.50	1.35	86.7	0.4418	4574.87	19880.40	0.230 ¹
T4	100 - 80	3/4	1.50	1.35	86.7	0.4418	4792.37	19880.40	0.241 ¹

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
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¹ P_u / φP_n controls

Torque-Arm Top Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T2	140 - 120 (706)	P2.5x.203	3.66	3.58	45.3	1.7040	18211.00	55211.20	0.330 ¹ ✓
T2	140 - 120 (707)	P2.5x.203	3.66	3.58	45.3	1.7040	13719.20	55211.20	0.248 ¹ ✓
T2	140 - 120 (716)	P2.5x.203	3.66	3.58	45.3	1.7040	14084.20	55211.20	0.255 ¹ ✓
T2	140 - 120 (717)	P2.5x.203	3.66	3.58	45.3	1.7040	18478.40	55211.20	0.335 ¹ ✓
T2	140 - 120 (722)	P2.5x.203	3.66	3.58	45.3	1.7040	13118.80	55211.20	0.238 ¹ ✓
T2	140 - 120 (723)	P2.5x.203	3.66	3.58	45.3	1.7040	13058.90	55211.20	0.237 ¹ ✓
T4	100 - 80 (728)	P2.5x.203	3.66	3.58	45.3	1.7040	10599.80	55211.20	0.192 ¹ ✓
T4	100 - 80 (729)	P2.5x.203	3.66	3.58	45.3	1.7040	8676.57	55211.20	0.157 ¹ ✓
T4	100 - 80 (738)	P2.5x.203	3.66	3.58	45.3	1.7040	8963.43	55211.20	0.162 ¹ ✓
T4	100 - 80 (739)	P2.5x.203	3.66	3.58	45.3	1.7040	10698.70	55211.20	0.194 ¹ ✓
T4	100 - 80 (744)	P2.5x.203	3.66	3.58	45.3	1.7040	7662.31	55211.20	0.139 ¹ ✓
T4	100 - 80 (745)	P2.5x.203	3.66	3.58	45.3	1.7040	7434.56	55211.20	0.135 ¹ ✓

¹ P_u / φP_n controls

Torque-Arm Bottom Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T2	140 - 120 (710)	P2.5x.203	3.36	3.28	41.6	1.7040	4010.60	55211.20	0.073 ¹ ✓
T2	140 - 120 (711)	P2.5x.203	3.36	3.28	41.6	1.7040	3189.55	55211.20	0.058 ¹ ✓
T2	140 - 120 (718)	P2.5x.203	3.36	3.28	41.6	1.7040	3097.03	55211.20	0.056 ¹ ✓
T2	140 - 120 (719)	P2.5x.203	3.36	3.28	41.6	1.7040	3937.35	55211.20	0.071 ¹ ✓

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Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio P _u / φP _n
T2	140 - 120 (724)	P2.5x.203	3.36	3.28	41.6	1.7040	4953.95	55211.20	0.090 ¹ ✓
T2	140 - 120 (725)	P2.5x.203	3.36	3.28	41.6	1.7040	4962.70	55211.20	0.090 ¹ ✓
T4	100 - 80 (732)	P2.5x.203	3.36	3.28	41.6	1.7040	4181.14	55211.20	0.076 ¹ ✓
T4	100 - 80 (733)	P2.5x.203	3.36	3.28	41.6	1.7040	4148.40	55211.20	0.075 ¹ ✓
T4	100 - 80 (740)	P2.5x.203	3.36	3.28	41.6	1.7040	3959.67	55211.20	0.072 ¹ ✓
T4	100 - 80 (741)	P2.5x.203	3.36	3.28	41.6	1.7040	4145.98	55211.20	0.075 ¹ ✓
T4	100 - 80 (746)	P2.5x.203	3.36	3.28	41.6	1.7040	4762.66	55211.20	0.086 ¹ ✓
T4	100 - 80 (747)	P2.5x.203	3.36	3.28	41.6	1.7040	4888.82	55211.20	0.089 ¹ ✓

¹ P_u / φP_n controls

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	φP _{allow} lb	% Capacity	Pass Fail
T1	150 - 140	Leg	1 3/4	2	30579.00	60150.90	50.8	Pass
		Diagonal	5/8	14	-3985.38	7356.83	54.2	Pass
		Horizontal	3/4	23	-591.72	12110.70	4.9	Pass
		Top Girt	3/4	5	-154.33	12110.70	1.3	Pass
		Bottom Girt	3/4	7	-1552.71	12110.70	12.8	Pass
T2	140 - 120	Leg	1 3/4	52	34027.20	60150.90	56.6	Pass
		Diagonal	5/8	123	-5208.03	7356.83	70.8	Pass
		Horizontal	3/4	80	1772.47	19880.40	8.9	Pass
		Top Girt	3/4	55	-885.03	12110.70	7.3	Pass
		Bottom Girt	3/4	57	641.70	19880.40	3.2	Pass
		Guy A@136	5/8	720	13251.80	25440.00	52.1	Pass
		Guy B@136	5/8	715	14081.10	25440.00	55.4	Pass
		Guy C@136	5/8	704	19661.90	25440.00	77.3	Pass
		Top Guy Pull-Off@136	3/4	708	8047.59	19880.40	40.5	Pass
		Bottom Guy Pull-Off@136	3/4	712	-6004.70	12110.70	49.6	Pass
		Torque Arm Top@136	P2.5x.203	717	18478.40	55211.20	33.5	Pass
Torque Arm Bottom@136	P2.5x.203	710	-20848.20	50402.20	41.4	Pass		
T3	120 - 100	Leg	1 3/4	145	-57378.40	90266.90	63.6	Pass
		Diagonal	5/8	153	-3857.75	7356.83	52.4	Pass
		Horizontal	3/4	158	1577.05	19880.40	7.9	Pass
		Top Girt	3/4	146	987.26	19880.40	5.0	Pass
		Bottom Girt	3/4	151	-1103.97	12110.70	9.1	Pass
T4	100 - 80	Leg	1 3/4	237	-67576.10	87434.40	77.3	Pass
		Diagonal	5/8	325	-4015.39	7356.83	54.6	Pass
		Horizontal	3/4	299	2260.81	19880.40	11.4	Pass
		Top Girt	3/4	238	1045.79	19880.40	5.3	Pass
		Bottom Girt	3/4	241	1247.85	19880.40	6.3	Pass
		Guy A@96	1/2	742	7798.51	16140.00	48.3	Pass
		Guy B@96	1/2	737	8429.20	16140.00	52.2	Pass

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Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	ϕP_{allow} lb	% Capacity	Pass Fail
T5	80 - 60	Guy C@96	1/2	726	12524.30	16140.00	77.6	Pass
		Top Guy Pull-Off@96	3/4	730	7046.42	19880.40	35.4	Pass
		Bottom Guy Pull-Off@96	3/4	735	-4865.47	12110.70	40.2	Pass
		Torque Arm Top@96	P2.5x.203	739	10698.70	55211.20	19.4	Pass
		Torque Arm Bottom@96	P2.5x.203	732	-14420.90	50402.20	28.6	Pass
		Leg	1 3/4	328	-38979.40	71902.10	54.2	Pass
		Diagonal	5/8	337	-2534.05	7356.83	34.4	Pass
		Horizontal	3/4	342	2314.37	19880.40	11.6	Pass
		Top Girt	3/4	330	1137.88	19880.40	5.7	Pass
		Bottom Girt	3/4	333	1137.62	19880.40	5.7	Pass
T6	60 - 40	Leg	1 3/4	420	-44060.30	66673.50	66.1	Pass
		Diagonal	5/8	428	-3640.35	7356.83	49.5	Pass
		Horizontal	3/4	434	2651.80	19880.40	13.3	Pass
		Top Girt	3/4	422	1182.60	19880.40	5.9	Pass
		Bottom Girt	3/4	425	1951.17	19880.40	9.8	Pass
		Guy A@50	7/16	752	8454.23	12480.00	67.7	Pass
		Guy B@50	7/16	751	8964.26	12480.00	71.8	Pass
		Guy C@50	7/16	748	11581.50	12480.00	92.8	Pass
		Top Guy Pull-Off@50	3/4	469	4512.26	19880.40	22.7	Pass
		T7	40 - 20	Leg	1 3/4	512	-48288.40	67658.00
Diagonal	5/8			597	-3564.14	7356.83	48.4	Pass
Horizontal	3/4			526	3325.23	19880.40	16.7	Pass
Top Girt	3/4			514	1307.80	19880.40	6.6	Pass
Bottom Girt	3/4			517	1530.91	19880.40	7.7	Pass
T8	20 - 5.25	Leg	1 3/4	605	-48232.90	67418.70	71.5	Pass
		Diagonal	5/8	669	-2501.07	7356.83	34.0	Pass
		Horizontal	3/4	667	3162.04	19880.40	15.9	Pass
		Top Girt	3/4	606	1773.54	19880.40	8.9	Pass
		Bottom Girt	3/4	609	6127.33	19880.40	30.8	Pass
T9	5.25 - 0	Leg	1 3/4	675	-46976.60	66371.00	70.8	Pass
		Diagonal	5/8	684	-4255.07	8259.22	51.5	Pass
		Horizontal	3/4	688	3116.34	19880.40	15.7	Pass
Summary								
Leg (T4)							77.3	Pass
Diagonal (T2)							70.8	Pass
Horizontal (T7)							16.7	Pass
Top Girt (T8)							8.9	Pass
Bottom Girt (T8)							30.8	Pass
Guy A (T6)							67.7	Pass
Guy B (T6)							71.8	Pass
Guy C (T6)							92.8	Pass
Top Guy Pull-Off (T2)							40.5	Pass
Bottom Guy Pull-Off (T2)							49.6	Pass
Torque Arm Top (T2)							33.5	Pass
Torque Arm Bottom (T2)							41.4	Pass
Bolt Checks							30.8	Pass
RATING =							92.8	Pass