



Crown Castle
3 Corporate Park Drive, Suite 101
Clifton Park, NY 12065

March 17, 2016

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: Notice of Exempt Modification for T-Mobile / L700 Crown Site BU: 801487
T-Mobile Site ID: CTHA160A
848 East Street, Suffield, CT 06078
Latitude: 41° 57' 25.2" / Longitude: -72° 37' 32.6"

Dear Ms. Bachman:

T-Mobile currently maintains three (3) antennas at the 164 foot level of the existing 165 foot monopole at 848 East Street in Suffield, CT. The Tower Owner is Crown Castle and the Property Owner is the Town of Suffield. T-Mobile now intends to replace all antennas with six (6) new antennas, keep all coax, remove RET cable, add one (1) Hybrid cable, replace all TMA with three (3) new TMAs, add six (6) RRUs and remove one (1) cabinet.

This facility was approved by the Town of Suffield in File #740 on May 1, 2000. This approval included the condition(s) that:

1. The heights of the respective mono-pole towers, including antennae, shall not exceed 199-feet (Site A); 120-feet (Site B); and 174-feet (Site C);
2. Each tower shall be certified as "self-collapsing" by a Connecticut registered professional engineer;
3. Details drawings are to be submitted with each request for building permits for both towers and related facilities;
4. FCC licenses shall be produced prior to the issuance of the permits for the company leasing space on the towers;
5. The Zoning Enforcement Officer shall review each proposal for zoning conformance prior to the issuance of the building permits;
6. All utilities are to be underground;
7. Site plans are to be revised.

This modification complies with the aforementioned condition(s).

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In

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accordance with R.S.C.A. § 16-50j-73, a copy of this letter is being sent to The Honorable Melissa M. Mack, First Selectman for the Town of Suffield for both the town Mayor notification and the Property Owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Kimberly Myl.

Sincerely,

Kimberly Myl
Real Estate Specialist
Crown Castle
1200 MacArthur Boulevard, Suite 200
Mahwah, New Jersey 07430
201-236-9069
kimberly.myl@crowncastle.com

Attachments:

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

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cc: The Honorable Melissa M. Mack, First Selectman for the Town of Suffield
Town Hall
83 Mountain Road
Suffield, CT 06078

GENERAL NOTES:

- FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:
PROJECT MANAGEMENT – CROWN CASTLE
CONTRACTOR – GENERAL CONTRACTOR (CONSTRUCTION)
OWNER – T-MOBILE
OEM – ORIGINAL EQUIPMENT MANUFACTURER
- PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF PROJECT MANAGEMENT.
- ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK.
- ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- DRAWINGS PROVIDED HERE ARE NOT TO SCALE UNLESS OTHERWISE NOTED AND ARE INTENDED TO SHOW OUTLINE ONLY.
- UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY PROJECT MANAGEMENT.
- CONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. CONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. CONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH PROJECT MANAGEMENT.
- THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF THE OWNER.
- CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION.
- THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER THE CONTRACT.
- CONTRACTOR SHALL NOTIFY DEWBERRY 48 HOURS IN ADVANCE OF POURING CONCRETE, OR BACKFILLING TRENCHES, SEALING ROOF AND WALL PENETRATIONS & POST DOWNS, FINISHING NEW WALLS OR FINAL ELECTRICAL CONNECTIONS FOR ENGINEER REVIEW.
- CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. CONTRACTOR SHALL NOTIFY PROJECT MANAGEMENT OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.
- THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY CONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.
- SINCE THE CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE ADVISED TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.

SITE WORK GENERAL NOTES:

- THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE START OF CONSTRUCTION.
- ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO:
A) FALL PROTECTION
B) CONFINED SPACE
C) ELECTRICAL SAFETY
D) TRENCHING & EXCAVATION.
- ALL SITE WORK SHALL BE AS INDICATED ON THE DRAWINGS AND PROJECT SPECIFICATIONS.
- IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES, TOP SOIL AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF CONTRACTOR, OWNER AND/OR LOCAL UTILITIES.
- CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION.
- THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE T-MOBILE SPECIFICATION FOR SITE SIGNAGE.
- THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE TRANSMISSION EQUIPMENT AND TOWER AREAS.
- NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.
- THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION, SEE SOIL COMPACTION NOTES.
- THE AREAS OF THE OWNER'S PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION.
- EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL JURISDICTION'S GUIDELINES FOR EROSION AND SEDIMENT CONTROL.

ELECTRICAL INSTALLATION NOTES:

- ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE LOCAL CODES.
- CONTRACTOR SHALL MODIFY EXISTING CABLE TRAY SYSTEM AS REQUIRED TO SUPPORT RF AND TRANSPORT CABLE TO THE NEW BTS EQUIPMENT. CONTRACTOR SHALL SUBMIT MODIFICATIONS TO PROJECT MANAGEMENT FOR APPROVAL.
- CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED.
- WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC AND TELCORDIA.
- ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC AND TELCORDIA.
- CABLES SHALL NOT BE ROUTED THROUGH LADDER-STYLE CABLE TRAY RUNGS.
- EACH END OF EVERY POWER, POWER PHASE CONDUCTOR (I.E., HOTS), GROUNDING, AND T1 CONDUCTOR AND CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2 INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC & OSHA, AND MATCH EXISTING INSTALLATION REQUIREMENTS.
- ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS. ALL EQUIPMENT SHALL BE LABELED WITH THEIR VOLTAGE RATING, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING, AND BRANCH CIRCUIT ID NUMBERS (I.E., PANELBOARD AND CIRCUIT ID'S).
- PANELBOARDS (ID NUMBERS) AND INTERNAL CIRCUIT BREAKERS (CIRCUIT ID NUMBERS) SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS.
- ALL TIE WRAPS SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.
- POWER, CONTROL, AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE CONDUCTOR (SIZE 14 AWG OR LARGER), 600V, OIL RESISTANT THHN OR THWN-2, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.
- POWER PHASE CONDUCTORS (I.E., HOTS) SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2 INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL) PHASE CONDUCTOR COLOR CODES SHALL CONFORM WITH THE NEC & OSHA AND MATCH EXISTING INSTALLATION REQUIREMENTS.
- SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE CONDUCTOR (SIZE 6 AWG OR LARGER), 600V, OIL RESISTANT THHN OR THWN-2 GREEN INSULATION, CLASS B STRANDED COPPER CABLE RATED FOR 90°C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.
- SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED OUTDOORS, OR BELOW GRADE, SHALL BE SINGLE CONDUCTOR #2 AWG SOLID TINNED COPPER CABLE, UNLESS OTHERWISE SPECIFIED.
- POWER AND CONTROL WIRING, NOT IN TUBING OR CONDUIT, SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (SIZE 14 AWG OR LARGER), 600V, OIL RESISTANT THHN OR THWN-2, CLASS B STRANDED COPPER CABLE RATED FOR 90°C (WET AND DRY) OPERATION; WITH OUTER JACKET; LISTED OR LABELED FOR THE LOCATION USED, UNLESS OTHERWISE SPECIFIED.
- ALL POWER AND POWER GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRENUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRENUTS SHALL BE RATED FOR OPERATION AT NO LESS THAN 75°C (90°C IF AVAILABLE).
- RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE, AND NEC.
- NEW RACEWAY OR CABLE TRAY WILL MATCH THE EXISTING INSTALLATION WHERE POSSIBLE.
- ELECTRICAL METALLIC TUBING (EMT) OR RIGID NONMETALLIC CONDUIT (I.E., RIGID PVC SCHEDULE 40, OR RIGID PVC SCHEDULE 80 FOR LOCATIONS SUBJECT TO PHYSICAL DAMAGE) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.
- ELECTRICAL METALLIC TUBING (EMT), ELECTRICAL NONMETALLIC TUBING (ENT), OR RIGID NONMETALLIC CONDUIT (RIGID PVC, SCHEDULE 40) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- GALVANIZED STEEL INTERMEDIATE METALLIC CONDUIT (IMC) SHALL BE USED FOR OUTDOOR LOCATIONS ABOVE GRADE.
- RIGID NONMETALLIC CONDUIT (I.E., RIGID PVC SCHEDULE 40 OR RIGID PVC SCHEDULE 80) SHALL BE USED UNDERGROUND; DIRECT BURIED, IN AREAS OF OCCASIONAL LIGHT VEHICLE TRAFFIC OR ENCASED IN REINFORCED CONCRETE IN AREAS OF HEAVY VEHICLE TRAFFIC.
- LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
- CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION-TYPE AND APPROVED FOR THE LOCATION USED. SETSCREW FITTINGS ARE NOT ACCEPTABLE.
- CABINETS, BOXES, AND WIREWAYS SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE, AND NEC.
- CABINETS, BOXES, AND WIREWAYS TO MATCH THE EXISTING INSTALLATION WHERE POSSIBLE.
- WIREWAYS SHALL BE EPOXY-COATED (GRAY) AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARD; SHALL BE PANDUIT TYPE E (OR EQUAL); AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS.
- EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES, AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET STEEL, SHALL MEET OR EXCEED UL 50, AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS.
- METAL RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED, OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.
- NONMETALLIC RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.
- THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM PROJECT MANAGEMENT BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
- THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD AGAINST LIFE AND PROPERTY.

CONCRETE AND REINFORCING STEEL NOTES:

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST-IN-PLACE CONCRETE.
- ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS, UNLESS NOTED OTHERWISE. A HIGHER STRENGTH (4000 PSI) MAY BE USED. ALL CONCRETING WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS.
- REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 WELDED STEEL WIRE FABRIC UNLESS NOTED OTHERWISE (UNO). SPLICES SHALL BE CLASS "B" AND ALL HOOKS SHALL BE STANDARD, UNO.
- THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
CONCRETE CAST AGAINST EARTH.....3 IN.
CONCRETE EXPOSED TO EARTH OR WEATHER:
#6 AND LARGER2 IN.
#5 AND SMALLER & WWF.....1 1/2 IN.
CONCRETE NOT EXPOSED TO EARTH OR WEATHER OR NOT CAST AGAINST THE GROUND:
SLAB AND WALL3/4 IN.
BEAMS AND COLUMNS.....1 1/2 IN.
- A CHAMFER 3/4" SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNO, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.
- INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHOR, SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE. THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO MANUFACTURER'S RECOMMENDATION FOR EMBEDMENT DEPTH OR AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR CONTRACTOR APPROVAL WHEN DRILLING HOLES IN CONCRETE. SPECIAL INSPECTIONS, REQUIRED BY GOVERNING CODES, SHALL BE PERFORMED IN ORDER TO MAINTAIN MANUFACTURER'S MAXIMUM ALLOWABLE LOADS. ALL EXPANSION/WEDGE ANCHORS SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED. EXPANSION BOLTS SHALL BE PROVIDED BY RAMSET/REDHEAD OR APPROVED EQUAL.
- CONCRETE CYLINDER TEST IS NOT REQUIRED FOR SLAB ON GRADE WHEN CONCRETE IS LESS THAN 50 CUBIC YARDS (IBC 1905.6.2.3) IN THAT EVENT THE FOLLOWING RECORDS SHALL BE PROVIDED BY THE CONCRETE SUPPLIER;
(A) RESULTS OF CONCRETE CYLINDER TESTS PERFORMED AT THE SUPPLIER'S PLANT,
(B) CERTIFICATION OF MINIMUM COMPRESSIVE STRENGTH FOR THE CONCRETE GRADE SUPPLIED.
FOR GREATER THAN 50 CUBIC YARDS THE GC SHALL PERFORM THE CONCRETE CYLINDER TEST.
- AS AN ALTERNATIVE TO ITEM 7, TEST CYLINDERS SHALL BE TAKEN INITIALLY AND THEREAFTER FOR EVERY 50 YARDS OF CONCRETE FROM EACH DIFFERENT BATCH PLANT.
- EQUIPMENT SHALL NOT BE PLACED ON NEW PADS FOR SEVEN DAYS AFTER PAD IS POURED, UNLESS IT IS VERIFIED BY CYLINDER TESTS THAT COMPRESSIVE STRENGTH HAS BEEN ATTAINED.

STRUCTURAL STEEL NOTES:

- ALL STEEL WORK SHALL BE PAINTED OR GALVANIZED IN ACCORDANCE WITH THE DRAWINGS UNLESS NOTED OTHERWISE. STRUCTURAL STEEL SHALL BE ASTM-A-36 UNLESS OTHERWISE NOTED ON THE SITE SPECIFIC DRAWINGS. STEEL DESIGN, INSTALLATION AND BOLTING SHALL BE PERFORMED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "MANUAL OF STEEL CONSTRUCTION".
- ALL WELDING SHALL BE PERFORMED USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION". PAINTED SURFACES SHALL BE TOUCHED UP.
- BOLTED CONNECTIONS SHALL BE ASTM A325 BEARING TYPE (3/4"Ø) CONNECTIONS AND SHALL HAVE MINIMUM OF TWO BOLTS UNLESS NOTED OTHERWISE.
- NON-STRUCTURAL CONNECTIONS FOR STEEL GRATING MAY USE 5/8" DIA. ASTM A 307 BOLTS UNLESS NOTED OTHERWISE.
- INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHOR, SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE. THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO MANUFACTURER'S RECOMMENDATION FOR EMBEDMENT DEPTH OR AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR CONTRACTOR APPROVAL WHEN DRILLING HOLES IN CONCRETE. SPECIAL INSPECTIONS, REQUIRED BY GOVERNING CODES, SHALL BE PERFORMED IN ORDER TO MAINTAIN MANUFACTURER'S MAXIMUM ALLOWABLE LOADS. ALL EXPANSION/WEDGE ANCHORS SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED. EXPANSION BOLTS SHALL BE PROVIDED BY RAMSET/REDHEAD OR APPROVED EQUAL.
- CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ENGINEER REVIEW & APPROVAL ON PROJECTS REQUIRING STRUCTURAL STEEL.
- ALL STRUCTURAL STEEL WORK SHALL BE DONE IN ACCORDANCE WITH AISC SPECIFICATIONS.

CONSTRUCTION NOTES:

- FIELD VERIFICATION:
CONTRACTOR SHALL FIELD VERIFY SCOPE OF WORK, T-MOBILE ANTENNA PLATFORM LOCATION AND ANTENNAS TO BE REPLACED.
- COORDINATION OF WORK:
CONTRACTOR SHALL COORDINATE RF WORK AND PROCEDURES WITH PROJECT MANAGEMENT.
- CABLE LADDER RACK:
CONTRACTOR SHALL FURNISH AND INSTALL CABLE LADDER RACK, CABLE TRAY, AND CONDUIT AS REQUIRED TO SUPPORT CABLES TO THE NEW BTS LOCATION.
- GROUNDING OF ALL EQUIPMENT AND ANTENNAS IS NOT CONSIDERED PART OF THE SCOPE OF THIS PROJECT AND IS THE RESPONSIBILITY OF THE OWNER AND CONTRACTOR AT THE TIME OF CONSTRUCTION. ALL EQUIPMENT AND ANTENNAS TO BE INSTALLED AND GROUNDED IN ACCORDANCE WITH GOVERNING BUILDING CODE, MANUFACTURER RECOMMENDATIONS AND OWNER SPECIFICATIONS.



T-MOBILE NORTHEAST LLC
35 GRIFFIN ROAD SOUTH
BLOOMFIELD, CT 06002



CROWN CASTLE
3 CORPORATE PARK DRIVE, SUITE 101
CLIFTON PARK, NY 12065

**CTHA160A
CT SUFFIELD 3 CAC**

CONSTRUCTION DRAWINGS

NO.	DATE	DESCRIPTION
0	03/16/16	ISSUED AS FINAL
A	03/15/16	ISSUED FOR REVIEW



Dewberry Engineers Inc.
600 PARSIPPANY ROAD
SUITE 301
PARSIPPANY, NJ 07054
PHONE: 973.739.9400
FAX: 973.739.9710



DRAWN BY: RA

REVIEWED BY: BSH

CHECKED BY: GHN

PROJECT NUMBER: 50066258

JOB NUMBER: 50078120

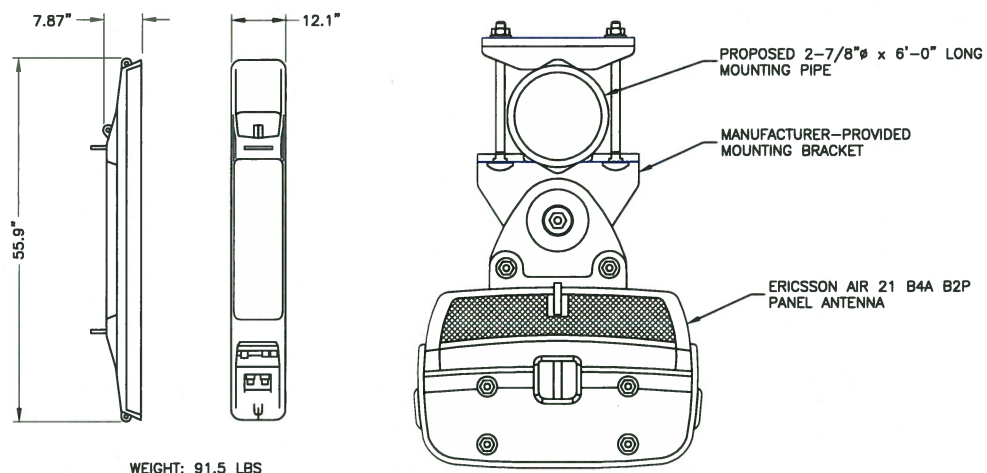
SITE ADDRESS:

848 EAST STREET
SUFFIELD, CT 06078
HARTFORD COUNTY

SHEET TITLE

GENERAL NOTES

SHEET NUMBER



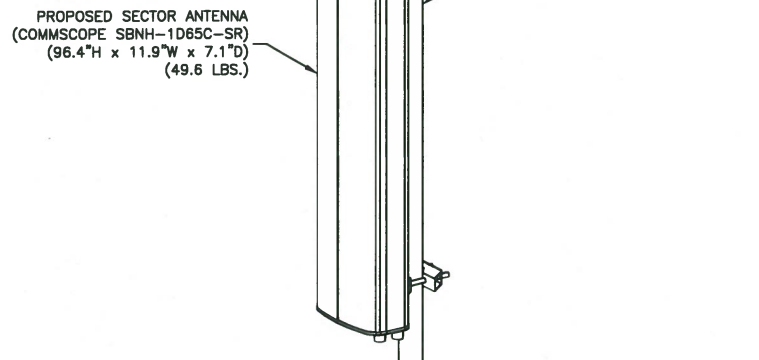
NOTES:

1. MOUNT ANTENNAS PER MANUFACTURER'S RECOMMENDATIONS.
2. GROUND ANTENNAS AND MOUNTS PER MANUFACTURER'S RECOMMENDATIONS AND T-MOBILE STANDARDS.
3. CONFIRM REQUIRED ANTENNAS WITH THE LATEST RFDS.

ANTENNA DETAIL

SCALE: N.T.S.

1



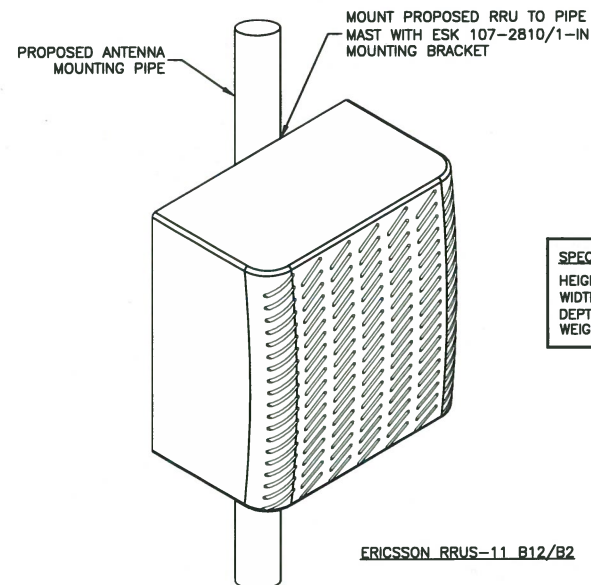
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3. CONFIRM REQUIRED ANTENNAS WITH THE LATEST RFDS.

ISOMETRIC ANTENNA DETAIL

SCALE: N.T.S.

2



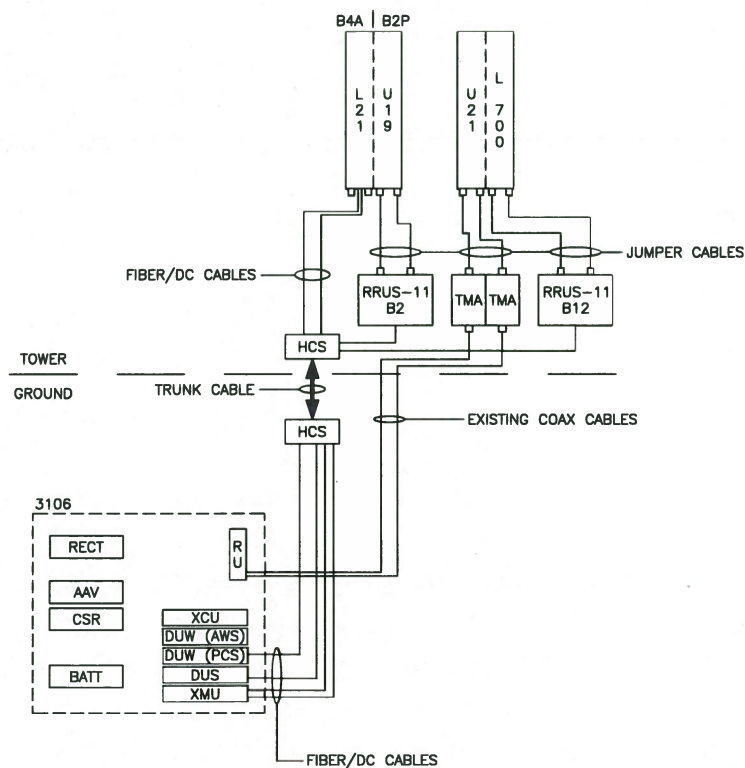
RRU NOTES:

1. MOUNT EQUIPMENT PER MANUFACTURER'S RECOMMENDATIONS.
2. GROUND EQUIPMENT AND MOUNTS PER MANUFACTURER'S RECOMMENDATIONS AND T-MOBILE STANDARDS.
3. CONFIRM REQUIRED EQUIPMENT WITH THE LATEST RFDS.

RRUS-11 - REMOTE RADIO UNIT

SCALE: N.T.S.

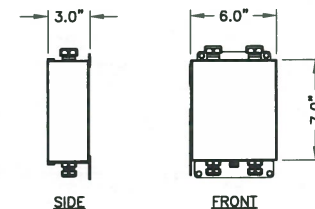
3



SITE CONFIGURATION 701D_WU21

SCALE: N.T.S.

5



ERICSSON KRY 112 144/1

NOTES:

1. MOUNT EQUIPMENT PER MANUFACTURER'S RECOMMENDATIONS.
2. GROUND EQUIPMENT AND MOUNTS PER MANUFACTURER'S RECOMMENDATIONS AND T-MOBILE STANDARDS.
3. CONFIRM REQUIRED EQUIPMENT WITH THE LATEST RFDS.

DUAL-PORT TMA DETAIL

SCALE: N.T.S.

4

DESIGN CONFIGURATION								
ANTENNAS	EXISTING	PROPOSED	COAX		HYBRID PROPOSED	COAX/HYBRID LENGTH	TMA PROPOSED	RRU PROPOSED
			EXISTING	PROPOSED				
ALPHA	-	ERICSSON AIR 21 B4A B2P	(4) 1-5/8"	-		213'-0"	-	(1) RRUS-11 B2
	RFS APX16PV-16PVL	COMMSCOPE SBNH-1D65C-SR					(1) KRY 112 144/1	(1) RRUS-11 B12
BETA	-	ERICSSON AIR 21 B4A B2P	(4) 1-5/8"	-	(1) 1-5/8"	213'-0"	-	(1) RRUS-11 B2
	RFS APX16PV-16PVL	COMMSCOPE SBNH-1D65C-SR					(1) KRY 112 144/1	(1) RRUS-11 B12
GAMMA	-	ERICSSON AIR 21 B4A B2P	(4) 1-5/8"	-		213'-0"	-	(1) RRUS-11 B2
	RFS APX16PV-16PVL	COMMSCOPE SBNH-1D65C-SR					(1) KRY 112 144/1	(1) RRUS-11 B12

T-Mobile

T-MOBILE NORTHEAST LLC
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CROWN CASTLE

CROWN CASTLE
3 CORPORATE PARK DRIVE, SUITE 101
CLIFTON PARK, NY 12065

**CTHA160A
CT SUFFIELD 3 CAC**

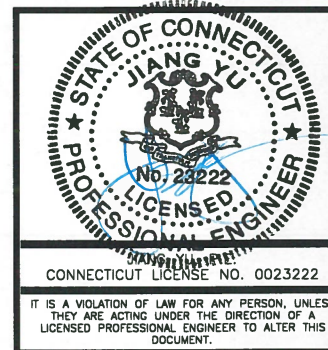
CONSTRUCTION DRAWINGS

0 03/16/16 ISSUED AS FINAL
A 03/15/16 ISSUED FOR REVIEW

Dewberry

Dewberry Engineers Inc.

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DRAWN BY: RA

REVIEWED BY: BSH

CHECKED BY: GHN

PROJECT NUMBER: 50066258

JOB NUMBER: 50078120

SITE ADDRESS:

848 EAST STREET
SUFFIELD, CT 06078
HARTFORD COUNTY

SHEET TITLE

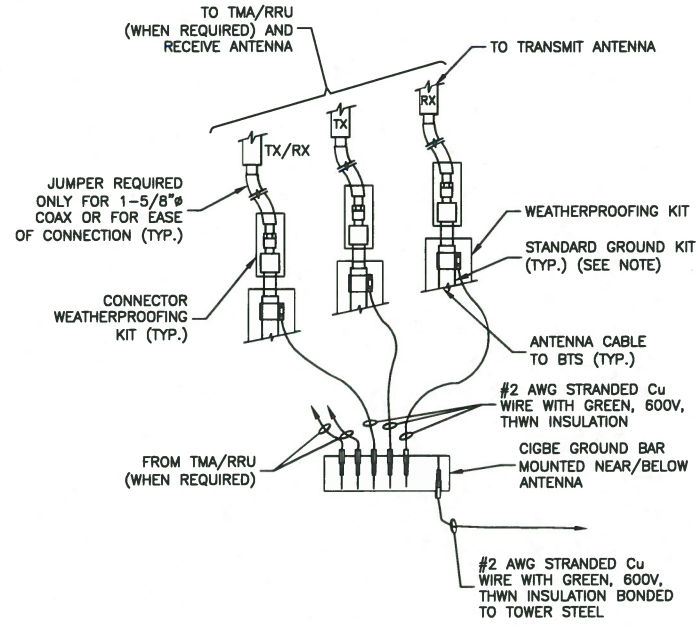
CONSTRUCTION
DETAILS

SHEET NUMBER

C-3

GROUNDING NOTES:

1. THE CONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE-SPECIFIC (UL, LPI, OR NFPA) LIGHTNING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE CONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE ENGINEER FOR RESOLUTION.
2. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS. ALL AVAILABLE GROUNDING ELECTRODES SHALL BE CONNECTED TOGETHER IN ACCORDANCE WITH THE NEC.
3. THE CONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS. USE OF OTHER METHODS MUST BE PRE-APPROVED BY THE ENGINEER IN WRITING.
4. THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS ON TOWER SITES AND 10 OHMS OR LESS ON ROOFTOP SITES. WHEN ADDING ELECTRODES, CONTRACTOR SHALL MAINTAIN A MINIMUM DISTANCE BETWEEN THE ADDED ELECTRODE AND ANY OTHER EXISTING ELECTRODE EQUAL TO THE BURIED LENGTH OF THE ROD. IDEALLY, CONTRACTOR SHALL STRIVE TO KEEP THE SEPARATION DISTANCE EQUAL TO TWICE THE BURIED LENGTH OF THE RODS.
5. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT.
6. METAL CONDUIT AND TRAY SHALL BE GROUNDING AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH 6 AWG COPPER WIRE AND UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
7. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO TRANSMISSION EQUIPMENT.
8. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED. BACK-TO-BACK CONNECTIONS ON OPPOSITE SIDES OF THE GROUND BUS ARE PERMITTED.
9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
10. USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED. IN ALL CASES, BENDS SHALL BE MADE WITH A MINIMUM BEND RADIUS OF 8 INCHES.
11. EACH INTERIOR TRANSMISSION CABINET FRAME/PLINTH SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH 6 AWG STRANDED, GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRE UNLESS NOTED OTHERWISE IN THE DETAILS. EACH OUTDOOR CABINET FRAME/PLINTH SHALL BE DIRECTLY CONNECTED TO THE BURIED GROUND RING WITH 2 AWG SOLID TIN-PLATED COPPER WIRE UNLESS NOTED OTHERWISE IN THE DETAILS.
12. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING, SHALL BE 2 AWG SOLID TIN-PLATED COPPER UNLESS OTHERWISE INDICATED.
13. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE. CONNECTIONS TO ABOVE GRADE UNITS SHALL BE MADE WITH EXOTHERMIC WELDS WHERE PRACTICAL OR WITH 2-HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAINLESS STEEL HARDWARE, INCLUDING SET SCREWS. HIGH PRESSURE CRIMP CONNECTORS MAY ONLY BE USED WITH WRITTEN PERMISSION FROM T-MOBILE MARKET REPRESENTATIVE.
14. EXOTHERMIC WELDS SHALL BE PERMITTED ON TOWERS ONLY WITH THE EXPRESS APPROVAL OF THE TOWER MANUFACTURER OR THE CONTRACTOR'S STRUCTURAL ENGINEER.
15. ALL WIRE TO WIRE GROUND CONNECTIONS TO THE INTERIOR GROUND RING SHALL BE FORMED USING HIGH PRESS CRIMPS OR SPLIT BOLT CONNECTORS WHERE INDICATED IN THE DETAILS.
16. ON ROOFTOP SITES WHERE EXOTHERMIC WELDS ARE A FIRE HAZARD COPPER COMPRESSION CAP CONNECTORS MAY BE USED FOR WIRE TO WIRE CONNECTORS. 2-HOLE MECHANICAL TYPE BRASS CONNECTORS WITH STAINLESS STEEL HARDWARE, INCLUDING SET SCREWS SHALL BE USED FOR CONNECTION TO ALL ROOFTOP TRANSMISSION EQUIPMENT AND STRUCTURAL STEEL.
17. COAX BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR USING TWO-HOLE MECHANICAL TYPE BRASS CONNECTORS AND STAINLESS STEEL HARDWARE.
18. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
19. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
20. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
21. BOND ALL METALLIC OBJECTS WITHIN 6 FT OF THE BURIED GROUND RING WITH 2 AWG SOLID TIN-PLATED COPPER GROUND CONDUCTOR. DURING EXCAVATION FOR NEW GROUND CONDUCTORS, IF EXISTING GROUND CONDUCTORS ARE ENCOUNTERED, BOND EXISTING GROUND CONDUCTORS TO NEW CONDUCTORS.
22. GROUND CONDUCTORS USED IN THE FACILITY GROUND AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC PLASTIC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (E.G., NON-METALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT WITH LISTED BONDING FITTINGS.

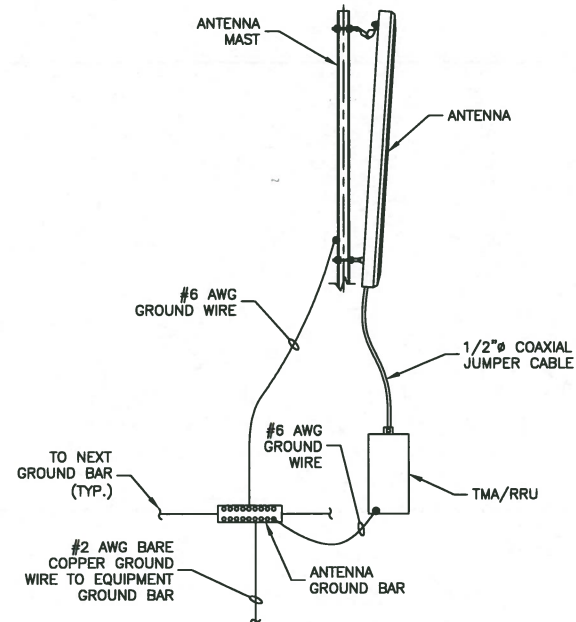


- NOTE:**
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO CIGBE.

CONNECTION OF GROUND WIRES TO GROUNDING BAR (CIGBE)

SCALE: N.T.S.

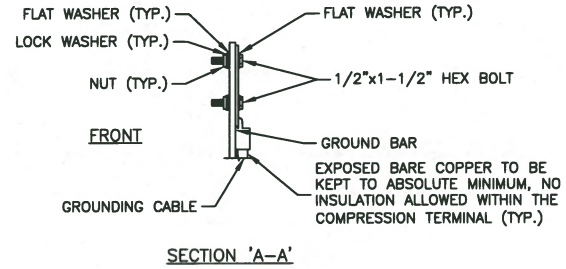
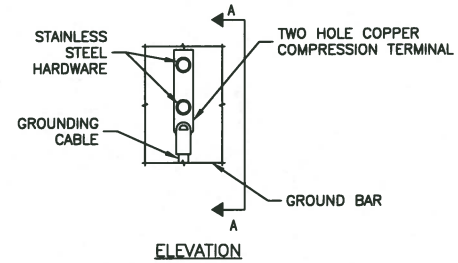
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TYPICAL ANTENNA GROUNDING DETAIL

SCALE: N.T.S.

3

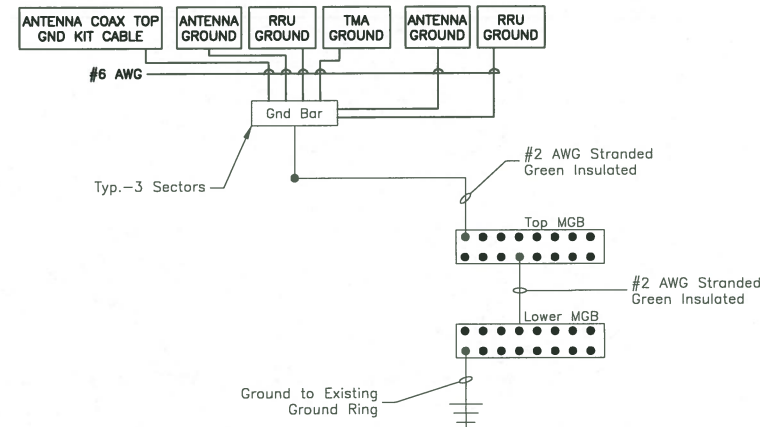


- NOTES:**
1. DOUBLING UP OR STACKING OF CONNECTIONS IS NOT PERMITTED.
 2. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS.

TYPICAL GROUND BAR MECHANICAL CONNECTION DETAIL

SCALE: N.T.S.

2



- NOTES:**
1. BOND ANTENNA GROUNDING KIT CABLE TO TOP CIGBE.
 2. BOND ANTENNA GROUNDING KIT CABLE TO BOTTOM CIGBE.
 3. SCHEMATIC GROUNDING DIAGRAM IS TYPICAL FOR EACH SECTOR.
 4. VERIFY EXISTING GROUND SYSTEM IS INSTALLED PER T-MOBILE STANDARDS.

SCHEMATIC GROUNDING DIAGRAM

SCALE: N.T.S.

4

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CT SUFFIELD 3 CAC**

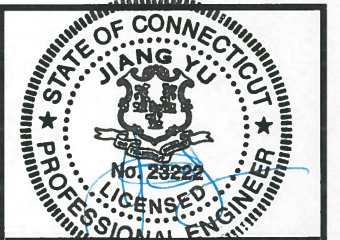
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DRAWN BY: RA

REVIEWED BY: BSH

CHECKED BY: GHN

PROJECT NUMBER: 50066258

JOB NUMBER: 50078120

SITE ADDRESS:

848 EAST STREET
SUFFIELD, CT 06078
HARTFORD COUNTY

SHEET TITLE

GROUNDING NOTES & DETAILS

SHEET NUMBER

E-1



Date: March 03, 2016

Charles Trask
Crown Castle
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Charlotte, NC 28277
980.209.8228

Paul J. Ford and Company
250 E. Broad Street, Suite 600
Columbus, OH 43215
614.221.6679
jsommer@pjfweb.com

Subject: Structural Analysis Report

Carrier Designation: *T-Mobile Co-Locate*
Carrier Site Number: CTHA160A
Carrier Site Name: Suffield/Stony Brook

Crown Castle Designation:
Crown Castle BU Number: 801487
Crown Castle Site Name: CT SUFFIELD 3 CAC 801487
Crown Castle JDE Job Number: 365816
Crown Castle Work Order Number: 1202045
Crown Castle Application Number: 333732 Rev. 3

Engineering Firm Designation: Paul J. Ford and Company Project Number: 37516-0632.002.7805

Site Data: 848 East Street, Suffield, Hartford County, CT
Latitude 41° 57' 25.2", Longitude -72° 37' 32.6"
165.5 Foot - Monopole Tower

Dear Charles Trask,

Paul J. Ford and Company is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 879354, in accordance with application 333732, revision 3.



The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

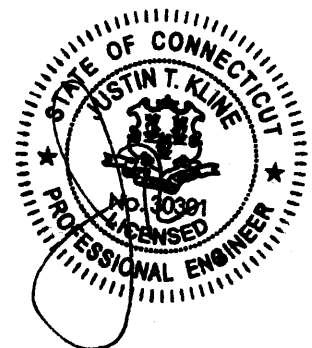
LC7: Existing + Reserved + Proposed Equipment **Sufficient Capacity**
Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

The structural analysis was performed for this tower in accordance with the requirements of the 2005 Connecticut Building Code and the TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 85 mph with no ice, 37.6 mph with 0.75 inch ice thickness and 50 mph under service loads.

We at Paul J. Ford and Company appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:


Jonathan Sommer, EI
Structural Designer 



Date: **March 03, 2016**

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1) INTRODUCTION

The structural analysis was performed for this tower in accordance with the requirements of the 2005 Connecticut Building Code and the TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 85 mph with no ice, 37.6 mph with 0.75 inch ice thickness and 50 mph under service loads.

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Table 1 - Proposed Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
164.0	163.0	3	ericsson	ERICSSON AIR 21 B4A B2P w/ Mount Pipe	1	1-5/8	-
		3	ericsson	RRUS 11 B2			
	161.0	3	commscope	SBNH-1D65C-SR w/ Mount Pipe			
		3	ericsson	KRY 112 144/1			
		3	ericsson	RRUS 11 B12			

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note			
164.0	164.0	6	remec	S20057A-1	1	1/8	3			
		3	rfs celwave	APX16PV-16PVL w/ Mount Pipe						
		1	tower mounts	T-Arm Mount [TA 602-3]				12	1-5/8	1
153.0	153.0	3	antel	BXA-171063-12BF w/ Mount Pipe	2	1-5/8	2			
		1	antel	BXA-70080-6CF-EDIN-4 w/ Mount Pipe				-	-	3
		2	rfs celwave	APX75-866514-CT2 w/ Mount Pipe						
		3	alcatel lucent	RRH2X60-AWS						
		3	alcatel lucent	RRH2X60-PCS						
		3	alcatel lucent	RRH2x60-700						
		6	commscope	SBNHH-1D65B w/ Mount Pipe						
		2	rfs celwave	DB-T1-6Z-8AB-0Z						
		6	antel	LPA-80080/6CF w/ Mount Pipe				12	1-5/8	1
1	tower mounts	Platform Mount [LP 304-1]								
145.0	145.0	3	kathrein	742 213 w/ Mount Pipe	6	1-5/8				
		1	tower mounts	Pipe Mount [PM 601-3]						

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
136.0	138.0	1	dragonwave	A-ANT-18G-2-C	3	1/2	1
		1	dragonwave	A-ANT-23G-1-C			
	136.0	2	dragonwave	HORIZON COMPACT			
		1	samsung telecommunications	WIMAX DAP HEAD			
		1	tower mounts	Pipe Mount [PM 601-3]			
		1	tower mounts	Side Arm Mount [SO 104-3]			
	134.0	1	kathrein	840 10054 w/ Mount Pipe			

Notes:

- 1) Existing Equipment
- 2) Reserved Equipment
- 3) Equipment To Be Removed

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Clough, Harbour & Associates LLP, 8961.07.07, 5/19/200	2373668	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	FWT, 21283000, 5/25/20000	1118795	CCISITES
4-TOWER MANUFACTURER DRAWINGS	FWT, 21283000, 5/25/20000	961597	CCISITES

3.1) Analysis Method

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Paul J. Ford and Company should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	165.5 - 136.83	Pole	TP24.279x17x0.1875	1	-4.75	720.46	60.0	Pass
L2	136.83 - 95.5	Pole	TP34.4x23.0992x0.3125	2	-10.46	1698.78	73.2	Pass
L3	95.5 - 47	Pole	TP46.06x32.6322x0.375	3	-19.84	2732.81	70.9	Pass
L4	47 - 0	Pole	TP57.275x43.7899x0.375	4	-25.00	2975.59	73.7	Pass
							Summary	
						Pole (L4)	73.7	Pass
						Rating =	73.7	Pass

Table 5 - Tower Component Stresses vs. Capacity – LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	70.5	Pass
1	Base Plate	0	35.4	Pass
1	Base Foundation Steel	0	45.0	Pass
1	Base Foundation Soil Interaction	0	38.0	Pass

Structure Rating (max from all components) =	73.7%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The monopole and its foundation have sufficient capacity to carry the existing, reserved, and proposed loads. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

Tower Input Data

There is a pole section.
 This tower is designed using the TIA/EIA-222-F standard.
 The following design criteria apply:

- 1) Tower is located in Hartford County, Connecticut.
- 2) Basic wind speed of 85 mph.
- 3) Nominal ice thickness of 0.7500 in.
- 4) Ice density of 56 pcf.
- 5) A wind speed of 38 mph is used in combination with ice.
- 6) Temperature drop of 50 °F.
- 7) Deflections calculated using a wind speed of 50 mph.
- 8) A non-linear (P-delta) analysis was used.
- 9) Pressures are calculated at each section.
- 10) Stress ratio used in pole design is 1.333.
- 11) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification ✓ Use Code Stress Ratios ✓ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric	Distribute Leg Loads As Uniform Assume Legs Pinned ✓ Assume Rigid Index Plate ✓ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension ✓ Bypass Mast Stability Checks ✓ Use Azimuth Dish Coefficients ✓ Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder	Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation ✓ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption Use TIA-222-G Tension Splice Exemption <div style="text-align: center; background-color: #e0e0e0; padding: 2px;">Poles</div> ✓ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
--	--	---

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	165.50-136.83	28.67	3.17	18	17.0000	24.2790	0.1875	0.7500	A572-65 (65 ksi)
L2	136.83-95.50	44.50	4.50	18	23.0992	34.4000	0.3125	1.2500	A572-65 (65 ksi)
L3	95.50-47.00	53.00	6.00	18	32.6322	46.0600	0.3750	1.5000	A572-65 (65 ksi)
L4	47.00-0.00	53.00		18	43.7899	57.2750	0.3750	1.5000	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	It/Q in ²	w in	w/t
L1	17.2623 24.6535	10.0055 14.3375	357.3078 1051.3254	5.9684 8.5525	8.6360 12.3337	41.3742 85.2398	715.0858 2104.0342	5.0037 7.1701	2.6620 3.9431	14.197 21.03

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	I/Q in ²	w in	w/t
L2	24.2730	22.6015	1482.6447	8.0893	11.7344	126.3505	2967.2404	11.3029	3.5155	11.249
	34.9307	33.8105	4963.4065	12.1011	17.4752	284.0257	9933.3440	16.9085	5.5044	17.614
L3	34.2933	38.3942	5047.2690	11.4513	16.5772	304.4711	10101.179	19.2007	5.0833	13.555
	46.7705	54.3766	14338.262	16.2182	23.3985	612.7861	28695.391	27.1935	7.4466	19.857
L4	46.0156	51.6746	12305.273	15.4123	22.2453	553.1639	24626.738	25.8422	7.0470	18.792
	58.1586	67.7252	27702.083	20.1995	29.0957	952.1023	55440.618	33.8690	9.4204	25.121

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _r	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
L1 165.50-136.83				1	1	1			
L2 136.83-95.50				1	1	1			
L3 95.50-47.00				1	1	1			
L4 47.00-0.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C _A A _A ft ² /ft	Weight plf
AL7-50(1-5/8")	C	No	Inside Pole	164.00 - 0.00	12	No Ice	0.52
						1/2" Ice	0.52
						1" Ice	0.52
MLE Hybrid 9Power/18Fiber RL 2(1-5/8") ***	C	No	Inside Pole	164.00 - 0.00	1	No Ice	1.07
						1/2" Ice	1.07
						1" Ice	1.07
AVA7-50(1-5/8")	C	No	Inside Pole	153.00 - 0.00	12	No Ice	0.72
						1/2" Ice	0.72
						1" Ice	0.72
HB158-1-08U8-S8J18(1-5/8") ***	C	No	Inside Pole	153.00 - 0.00	2	No Ice	1.30
						1/2" Ice	1.30
						1" Ice	1.30
LCF158-50J(1-5/8") ***	C	No	Inside Pole	145.00 - 0.00	6	No Ice	0.92
						1/2" Ice	0.92
						1" Ice	0.92
FSJ4-50B(1/2")	C	No	CaAa (Out Of Face)	136.00 - 0.00	1	No Ice	0.14
						1/2" Ice	0.76
						1" Ice	2.00
FSJ4-50B(1/2") ***	C	No	CaAa (Out Of Face)	136.00 - 0.00	2	No Ice	0.14
						1/2" Ice	0.76
						1" Ice	2.00

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustmen t °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K	
164									
T-Arm Mount [TA 602-3]	C	None		0.0000	164.00	No Ice	11.59	11.59	0.77
						1/2"	15.44	15.44	0.99
						Ice	19.29	19.29	1.21
						1" Ice			
SBNH-1D65C-SR w/ Mount Pipe	A	From Face	4.00	0.0000	164.00	No Ice	11.68	9.84	0.08
			0.00			1/2"	12.40	11.37	0.17
			-3.00			Ice	13.14	12.91	0.27
						1" Ice			
SBNH-1D65C-SR w/ Mount Pipe	B	From Face	4.00	0.0000	164.00	No Ice	11.68	9.84	0.08
			0.00			1/2"	12.40	11.37	0.17
			-3.00			Ice	13.14	12.91	0.27
						1" Ice			
SBNH-1D65C-SR w/ Mount Pipe	C	From Face	4.00	0.0000	164.00	No Ice	11.68	9.84	0.08
			0.00			1/2"	12.40	11.37	0.17
			-3.00			Ice	13.14	12.91	0.27
						1" Ice			
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	A	From Face	4.00	0.0000	164.00	No Ice	6.82	5.63	0.11
			0.00			1/2"	7.34	6.47	0.17
			-1.00			Ice	7.85	7.25	0.23
						1" Ice			
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	B	From Face	4.00	0.0000	164.00	No Ice	6.82	5.63	0.11
			0.00			1/2"	7.34	6.47	0.17
			-1.00			Ice	7.85	7.25	0.23
						1" Ice			
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	C	From Face	4.00	0.0000	164.00	No Ice	6.82	5.63	0.11
			0.00			1/2"	7.34	6.47	0.17
			-1.00			Ice	7.85	7.25	0.23
						1" Ice			
KRY 112 144/1	A	From Face	4.00	0.0000	164.00	No Ice	0.41	0.20	0.01
			0.00			1/2"	0.50	0.27	0.01
			-3.00			Ice	0.59	0.35	0.02
						1" Ice			
KRY 112 144/1	B	From Face	4.00	0.0000	164.00	No Ice	0.41	0.20	0.01
			0.00			1/2"	0.50	0.27	0.01
			-3.00			Ice	0.59	0.35	0.02
						1" Ice			
KRY 112 144/1	C	From Face	4.00	0.0000	164.00	No Ice	0.41	0.20	0.01
			0.00			1/2"	0.50	0.27	0.01
			-3.00			Ice	0.59	0.35	0.02
						1" Ice			
RRUS 11 B12	A	From Face	4.00	0.0000	164.00	No Ice	3.31	1.36	0.05
			0.00			1/2"	3.55	1.54	0.07
			-3.00			Ice	3.80	1.73	0.10
						1" Ice			
RRUS 11 B12	B	From Face	4.00	0.0000	164.00	No Ice	3.31	1.36	0.05
			0.00			1/2"	3.55	1.54	0.07
			-3.00			Ice	3.80	1.73	0.10
						1" Ice			
RRUS 11 B12	C	From Face	4.00	0.0000	164.00	No Ice	3.31	1.36	0.05
			0.00			1/2"	3.55	1.54	0.07
			-3.00			Ice	3.80	1.73	0.10
						1" Ice			
RRUS 11 B2	A	From Face	4.00	0.0000	164.00	No Ice	3.31	1.36	0.05
			0.00			1/2"	3.55	1.54	0.07
			-1.00			Ice	3.80	1.73	0.10
						1" Ice			
RRUS 11 B2	B	From Face	4.00	0.0000	164.00	No Ice	3.31	1.36	0.05
			0.00			1/2"	3.55	1.54	0.07
			-1.00			Ice	3.80	1.73	0.10
						1" Ice			
RRUS 11 B2	C	From Face	4.00	0.0000	164.00	No Ice	3.31	1.36	0.05

Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C _{AA} _{Front}	C _{AA} _{Side}	Weight
			Horz	Lateral	Vert					
			0.00				1/2"	3.55	1.54	0.07
			-1.00				Ice	3.80	1.73	0.10
							1" Ice			
153										
Platform Mount [LP 304-1]	C	None		0.0000	153.00	No Ice	17.46	17.46	1.35	
						1/2"	22.44	22.44	1.62	
						Ice	27.42	27.42	1.90	
						1" Ice				
(2) LPA-80080/6CF w/ Mount Pipe	A	From Face	4.00	0.0000	153.00	No Ice	4.56	10.73	0.05	
			0.00			1/2"	5.11	11.99	0.11	
			0.00			Ice	5.61	12.97	0.19	
						1" Ice				
(2) LPA-80080/6CF w/ Mount Pipe	B	From Face	4.00	0.0000	153.00	No Ice	4.56	10.73	0.05	
			0.00			1/2"	5.11	11.99	0.11	
			0.00			Ice	5.61	12.97	0.19	
						1" Ice				
(2) LPA-80080/6CF w/ Mount Pipe	C	From Face	4.00	0.0000	153.00	No Ice	4.56	10.73	0.05	
			0.00			1/2"	5.11	11.99	0.11	
			0.00			Ice	5.61	12.97	0.19	
						1" Ice				
(2) SBNHH-1D65B w/ Mount Pipe	A	From Face	4.00	0.0000	153.00	No Ice	8.64	7.07	0.07	
			0.00			1/2"	9.30	8.26	0.14	
			0.00			Ice	9.92	9.18	0.21	
						1" Ice				
(2) SBNHH-1D65B w/ Mount Pipe	B	From Face	4.00	0.0000	153.00	No Ice	8.64	7.07	0.07	
			0.00			1/2"	9.30	8.26	0.14	
			0.00			Ice	9.92	9.18	0.21	
						1" Ice				
(2) SBNHH-1D65B w/ Mount Pipe	C	From Face	4.00	0.0000	153.00	No Ice	8.64	7.07	0.07	
			0.00			1/2"	9.30	8.26	0.14	
			0.00			Ice	9.92	9.18	0.21	
						1" Ice				
RRH2X60-PCS	A	From Face	4.00	0.0000	153.00	No Ice	2.57	2.01	0.06	
			0.00			1/2"	2.79	2.22	0.08	
			0.00			Ice	3.02	2.43	0.10	
						1" Ice				
RRH2X60-PCS	B	From Face	4.00	0.0000	153.00	No Ice	2.57	2.01	0.06	
			0.00			1/2"	2.79	2.22	0.08	
			0.00			Ice	3.02	2.43	0.10	
						1" Ice				
RRH2X60-PCS	C	From Face	4.00	0.0000	153.00	No Ice	2.57	2.01	0.06	
			0.00			1/2"	2.79	2.22	0.08	
			0.00			Ice	3.02	2.43	0.10	
						1" Ice				
RRH2X60-AWS	A	From Face	4.00	0.0000	153.00	No Ice	2.19	1.43	0.04	
			0.00			1/2"	2.40	1.61	0.06	
			0.00			Ice	2.61	1.80	0.08	
						1" Ice				
RRH2X60-AWS	B	From Face	4.00	0.0000	153.00	No Ice	2.19	1.43	0.04	
			0.00			1/2"	2.40	1.61	0.06	
			0.00			Ice	2.61	1.80	0.08	
						1" Ice				
RRH2X60-AWS	C	From Face	4.00	0.0000	153.00	No Ice	2.19	1.43	0.04	
			0.00			1/2"	2.40	1.61	0.06	
			0.00			Ice	2.61	1.80	0.08	
						1" Ice				
RRH2x60-700	A	From Face	4.00	0.0000	153.00	No Ice	3.96	1.82	0.06	
			0.00			1/2"	4.27	2.08	0.08	
			0.00			Ice	4.60	2.36	0.11	
						1" Ice				
RRH2x60-700	B	From Face	4.00	0.0000	153.00	No Ice	3.96	1.82	0.06	
			0.00			1/2"	4.27	2.08	0.08	
			0.00			Ice	4.60	2.36	0.11	
						1" Ice				
RRH2x60-700	C	From Face	4.00	0.0000	153.00	No Ice	3.96	1.82	0.06	

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	CA _{AA} Front ft ²	CA _{AA} Side ft ²	Weight K	
			0.00		1/2"	4.27	2.08	0.08	
			0.00		Ice	4.60	2.36	0.11	
					1" Ice				
(2) DB-T1-6Z-8AB-0Z	C	From Face	4.00	0.0000	153.00	No Ice	5.60	2.33	0.04
			0.00			1/2"	5.92	2.56	0.08
			0.00			Ice	6.24	2.79	0.12
						1" Ice			
145 Pipe Mount [PM 601-3]	C	None		0.0000	145.00	No Ice	4.39	4.39	0.20
						1/2"	5.48	5.48	0.24
						Ice	6.57	6.57	0.28
						1" Ice			
742 213 w/ Mount Pipe	A	From Face	1.00	0.0000	145.00	No Ice	5.37	4.62	0.05
			0.00			1/2"	5.95	6.00	0.09
			0.00			Ice	6.50	6.98	0.15
						1" Ice			
742 213 w/ Mount Pipe	B	From Face	1.00	0.0000	145.00	No Ice	5.37	4.62	0.05
			0.00			1/2"	5.95	6.00	0.09
			0.00			Ice	6.50	6.98	0.15
						1" Ice			
742 213 w/ Mount Pipe	C	From Face	1.00	0.0000	145.00	No Ice	5.37	4.62	0.05
			0.00			1/2"	5.95	6.00	0.09
			0.00			Ice	6.50	6.98	0.15
						1" Ice			
136 Side Arm Mount [SO 104-3]	C	None		0.0000	136.00	No Ice	3.30	3.30	0.29
						1/2"	4.13	4.13	0.32
						Ice	4.96	4.96	0.35
						1" Ice			
Pipe Mount [PM 601-3]	C	None		0.0000	136.00	No Ice	4.39	4.39	0.20
						1/2"	5.48	5.48	0.24
						Ice	6.57	6.57	0.28
						1" Ice			
840 10054 w/ Mount Pipe	A	From Face	1.00	0.0000	136.00	No Ice	5.41	2.39	0.05
			0.00			1/2"	5.83	2.92	0.09
			-2.00			Ice	6.26	3.47	0.13
						1" Ice			
WIMAX DAP HEAD	A	From Face	1.00	0.0000	136.00	No Ice	1.80	0.78	0.03
			0.00			1/2"	1.99	0.92	0.04
			0.00			Ice	2.18	1.07	0.06
						1" Ice			
HORIZON COMPACT	A	From Face	1.00	0.0000	136.00	No Ice	0.84	0.43	0.01
			0.00			1/2"	0.97	0.52	0.02
			0.00			Ice	1.10	0.63	0.03
						1" Ice			
HORIZON COMPACT	C	From Face	1.00	0.0000	136.00	No Ice	0.84	0.43	0.01
			0.00			1/2"	0.97	0.52	0.02
			0.00			Ice	1.10	0.63	0.03
						1" Ice			

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft ²	Weight K	
A-ANT-23G-1-C	B	Paraboloid w/Shroud (HP)	From Leg	1.00	0.0000		136.00	1.27	No Ice	1.28	0.02
				0.00					1/2" Ice	1.45	0.03
				2.00					1" Ice	1.62	0.04

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft ²	Weight K	
A-ANT-18G-2-C	C	Paraboloid w/Shroud (HP)	From Leg	1.00 0.00 2.00	0.0000		136.00	2.17	No Ice 1/2" Ice 1" Ice	3.72 4.01 4.30	0.03 0.04 0.05

Tower Pressures - No Ice

$G_H = 1.690$

Section Elevation ft	z ft	K _Z	q _z psf	A _G ft ²	F a c e	A _F ft ²	A _R ft ²	A _{leg} ft ²	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
L1 165.50-136.83	150.32	1.542	29	49.311	A	0.000	49.311	49.311	100.00	0.000	0.000
					B	0.000	49.311	100.00	0.000	0.000	
					C	0.000	49.311	100.00	0.000	0.000	
L2 136.83-95.50	115.19	1.429	26	100.405	A	0.000	100.405	100.405	100.00	0.000	0.000
					B	0.000	100.405	100.00	0.000	0.000	
					C	0.000	100.405	100.00	0.000	2.106	
L3 95.50-47.00	70.60	1.243	23	161.328	A	0.000	161.328	161.328	100.00	0.000	0.000
					B	0.000	161.328	100.00	0.000	0.000	
					C	0.000	161.328	100.00	0.000	2.522	
L4 47.00-0.00	22.69	1	19	200.908	A	0.000	200.908	200.908	100.00	0.000	0.000
					B	0.000	200.908	100.00	0.000	0.000	
					C	0.000	200.908	100.00	0.000	2.444	

Tower Pressure - With Ice

$G_H = 1.690$

Section Elevation ft	z ft	K _Z	q _z psf	t _z in	A _G ft ²	F a c e	A _F ft ²	A _R ft ²	A _{leg} ft ²	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
L1 165.50-136.83	150.32	1.542	6	0.7500	52.895	A	0.000	52.895	52.895	100.00	0.000	0.000
						B	0.000	52.895	100.00	0.000	0.000	
						C	0.000	52.895	100.00	0.000	0.000	
L2 136.83-95.50	115.19	1.429	5	0.7500	105.571	A	0.000	105.571	105.571	100.00	0.000	0.000
						B	0.000	105.571	100.00	0.000	0.000	
						C	0.000	105.571	100.00	0.000	8.181	
L3 95.50-47.00	70.60	1.243	4	0.7500	167.390	A	0.000	167.390	167.390	100.00	0.000	0.000
						B	0.000	167.390	100.00	0.000	0.000	
						C	0.000	167.390	100.00	0.000	9.797	
L4 47.00-0.00	22.69	1	4	0.7500	206.783	A	0.000	206.783	206.783	100.00	0.000	0.000
						B	0.000	206.783	100.00	0.000	0.000	
						C	0.000	206.783	100.00	0.000	9.494	

Tower Pressure - Service

$G_H = 1.690$

Section Elevation	<i>z</i>	<i>K_z</i>	<i>q_z</i>	<i>A_G</i>	<i>F</i> <i>a</i> <i>c</i> <i>e</i>	<i>A_F</i>	<i>A_R</i>	<i>A_{leg}</i>	<i>Leg</i> <i>%</i>	<i>C_AA_A</i> <i>In</i> <i>Face</i> <i>ft²</i>	<i>C_AA_A</i> <i>Out</i> <i>Face</i> <i>ft²</i>
<i>ft</i>	<i>ft</i>		<i>psf</i>	<i>ft²</i>		<i>ft²</i>	<i>ft²</i>	<i>ft²</i>		<i>ft²</i>	<i>ft²</i>
L1 165.50- 136.83	150.32	1.542	10	49.311	A	0.000	49.311	49.311	100.00	0.000	0.000
					B	0.000	49.311		100.00	0.000	0.000
					C	0.000	49.311		100.00	0.000	0.000
L2 136.83- 95.50	115.19	1.429	9	100.40	A	0.000	100.405	100.405	100.00	0.000	0.000
				5	B	0.000	100.405		100.00	0.000	0.000
					C	0.000	100.405		100.00	0.000	2.106
L3 95.50- 47.00	70.60	1.243	8	161.32	A	0.000	161.328	161.328	100.00	0.000	0.000
				8	B	0.000	161.328		100.00	0.000	0.000
					C	0.000	161.328		100.00	0.000	2.522
L4 47.00-0.00	22.69	1	6	200.90	A	0.000	200.908	200.908	100.00	0.000	0.000
				8	B	0.000	200.908		100.00	0.000	0.000
					C	0.000	200.908		100.00	0.000	2.444

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 60 deg - No Ice
5	Dead+Wind 90 deg - No Ice
6	Dead+Wind 120 deg - No Ice
7	Dead+Wind 150 deg - No Ice
8	Dead+Wind 180 deg - No Ice
9	Dead+Wind 210 deg - No Ice
10	Dead+Wind 240 deg - No Ice
11	Dead+Wind 270 deg - No Ice
12	Dead+Wind 300 deg - No Ice
13	Dead+Wind 330 deg - No Ice
14	Dead+Ice+Temp
15	Dead+Wind 0 deg+Ice+Temp
16	Dead+Wind 30 deg+Ice+Temp
17	Dead+Wind 60 deg+Ice+Temp
18	Dead+Wind 90 deg+Ice+Temp
19	Dead+Wind 120 deg+Ice+Temp
20	Dead+Wind 150 deg+Ice+Temp
21	Dead+Wind 180 deg+Ice+Temp
22	Dead+Wind 210 deg+Ice+Temp
23	Dead+Wind 240 deg+Ice+Temp
24	Dead+Wind 270 deg+Ice+Temp
25	Dead+Wind 300 deg+Ice+Temp
26	Dead+Wind 330 deg+Ice+Temp
27	Dead+Wind 0 deg - Service
28	Dead+Wind 30 deg - Service
29	Dead+Wind 60 deg - Service
30	Dead+Wind 90 deg - Service
31	Dead+Wind 120 deg - Service
32	Dead+Wind 150 deg - Service
33	Dead+Wind 180 deg - Service
34	Dead+Wind 210 deg - Service
35	Dead+Wind 240 deg - Service
36	Dead+Wind 270 deg - Service
37	Dead+Wind 300 deg - Service
38	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	165.5 - 136.83	Pole	Max Tension	2	0.00	-0.00	-0.00
			Max. Compression	14	-10.11	0.01	-0.99
			Max. Mx	5	-4.80	-199.59	-0.33
			Max. My	8	-4.75	-0.00	-204.15
			Max. Vy	5	13.21	-199.59	-0.33
			Max. Vx	8	13.53	-0.00	-204.15
			Max. Torque	5			-1.08
L2	136.83 - 95.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-17.05	0.50	-1.03
			Max. Mx	5	-10.48	-822.08	-1.62
			Max. My	8	-10.46	-1.28	-833.48
			Max. Vy	5	17.18	-822.08	-1.62
			Max. Vx	2	-17.35	0.57	833.23
			Max. Torque	11			1.18
L3	95.5 - 47	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-27.87	0.77	-1.17
			Max. Mx	5	-19.85	-1720.84	-3.58
			Max. My	2	-19.84	1.45	1739.68
			Max. Vy	5	21.07	-1720.84	-3.58
			Max. Vx	2	-21.23	1.45	1739.68
			Max. Torque	12			1.18
L4	47 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-43.75	1.15	-1.37
			Max. Mx	5	-33.63	-2947.15	-5.74
			Max. My	2	-33.63	2.43	2974.48
			Max. Vy	5	25.21	-2947.15	-5.74
			Max. Vx	2	-25.37	2.43	2974.48
			Max. Torque	12			1.21

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	165.5 - 136.83	37.593	33	2.1816	0.0054
L2	140 - 95.5	26.280	33	1.9523	0.0032
L3	100 - 47	12.618	33	1.2663	0.0012
L4	53 - 0	3.388	33	0.6025	0.0004

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
164.00	T-Arm Mount [TA 602-3]	33	36.905	2.1708	0.0053	16717
153.00	Platform Mount [LP 304-1]	33	31.900	2.0866	0.0044	6686
145.00	Pipe Mount [PM 601-3]	33	28.385	2.0106	0.0038	4076
138.00	A-ANT-23G-1-C	33	25.463	1.9261	0.0033	3318
136.00	Side Arm Mount [SO 104-3]	33	24.659	1.8983	0.0032	3310

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	165.5 - 136.83	108.183	8	6.2762	0.0158
L2	140 - 95.5	75.690	2	5.6206	0.0090
L3	100 - 47	36.379	2	3.6500	0.0033
L4	53 - 0	9.776	13	1.7382	0.0011

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
164.00	T-Arm Mount [TA 602-3]	8	106.205	6.2456	0.0155	5954
153.00	Platform Mount [LP 304-1]	8	91.830	6.0053	0.0130	2380
145.00	Pipe Mount [PM 601-3]	2	81.736	5.7877	0.0112	1449
138.00	A-ANT-23G-1-C	2	73.341	5.5455	0.0097	1177
136.00	Side Arm Mount [SO 104-3]	2	71.033	5.4658	0.0093	1174

Compression Checks

Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	F _a ksi	A in ²	Actual P K	Allow. P _a K	Ratio $\frac{P}{P_a}$
L1	165.5 - 136.83 (1)	TP24.279x17x0.1875	28.67	0.00	0.0	39.000	13.8585	-4.75	540.48	0.009
L2	136.83 - 95.5 (2)	TP34.4x23.0992x0.3125	44.50	0.00	0.0	39.000	32.6770	-10.46	1274.40	0.008
L3	95.5 - 47 (3)	TP46.06x32.6322x0.375	53.00	0.00	0.0	39.000	52.5672	-19.84	2050.12	0.010
L4	47 - 0 (4)	TP57.275x43.7899x0.375	53.00	0.00	0.0	39.000	57.2373	-25.00	2232.25	0.011

Pole Bending Design Data

Section No.	Elevation ft	Size	Actual M _x kip-ft	Actual f _{bx} ksi	Allow. F _{bx} ksi	Ratio $\frac{f_{bx}}{F_{bx}}$	Actual M _y kip-ft	Actual f _{by} ksi	Allow. F _{by} ksi	Ratio $\frac{f_{by}}{F_{by}}$
L1	165.5 - 136.83 (1)	TP24.279x17x0.1875	204.15	30.769	39.000	0.789	0.00	0.000	39.000	0.000
L2	136.83 - 95.5 (2)	TP34.4x23.0992x0.3125	833.48	37.712	39.000	0.967	0.00	0.000	39.000	0.000
L3	95.5 - 47 (3)	TP46.06x32.6322x0.375	1739.80	36.466	39.000	0.935	0.00	0.000	39.000	0.000
L4	47 - 0 (4)	TP57.275x43.7899x0.375	2144.12	37.880	39.000	0.971	0.00	0.000	39.000	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V K	Actual f _v ksi	Allow. F _v ksi	Ratio $\frac{f_v}{F_v}$	Actual T kip-ft	Actual f _{vt} ksi	Allow. F _{vt} ksi	Ratio $\frac{f_{vt}}{F_{vt}}$
L1	165.5 -	TP24.279x17x0.1875	13.53	0.976	26.000	0.076	0.00	0.000	26.000	0.000

Section No.	Elevation ft	Size	Actual V K	Actual f_v ksi	Allow. F_v ksi	Ratio $\frac{f_v}{F_v}$	Actual T kip-ft	Actual f_{vt} ksi	Allow. F_{vt} ksi	Ratio $\frac{f_{vt}}{F_{vt}}$
L2	136.83 (1) 136.83 - 95.5 (2)	TP34.4x23.0992x0.3125	17.33	0.530	26.000	0.041	0.42	0.009	26.000	0.000
L3	95.5 - 47 (3)	TP46.06x32.6322x0.375	21.25	0.404	26.000	0.031	0.99	0.010	26.000	0.000
L4	47 - 0 (4)	TP57.275x43.7899x0.375	22.90	0.400	26.000	0.030	1.00	0.009	26.000	0.000

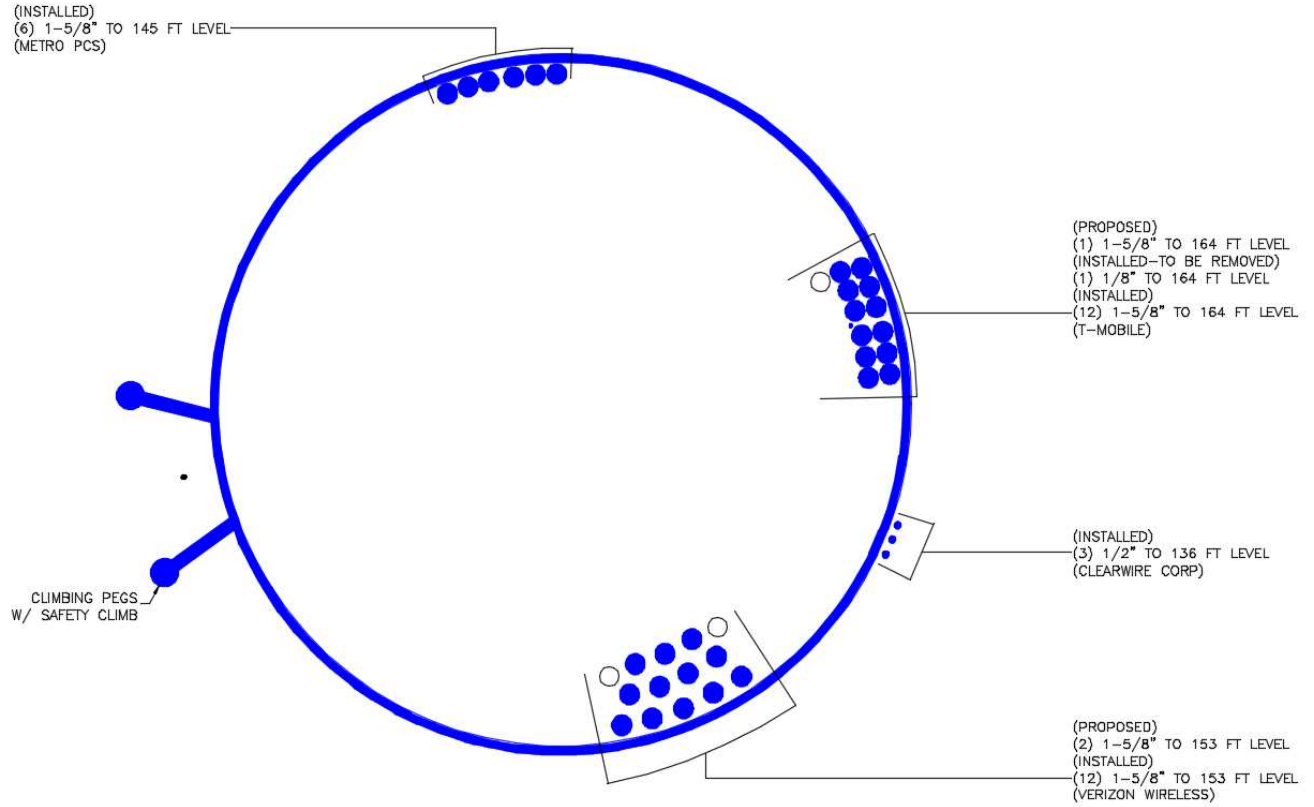
Pole Interaction Design Data

Section No.	Elevation ft	Ratio P P_a	Ratio f_{bx} F_{bx}	Ratio f_{by} F_{by}	Ratio f_v F_v	Ratio f_{vt} F_{vt}	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	165.5 - 136.83 (1)	0.009	0.789	0.000	0.076	0.000	0.799	1.333	H1-3+VT ✓
L2	136.83 - 95.5 (2)	0.008	0.967	0.000	0.041	0.000	0.976	1.333	H1-3+VT ✓
L3	95.5 - 47 (3)	0.010	0.935	0.000	0.031	0.000	0.945	1.333	H1-3+VT ✓
L4	47 - 0 (4)	0.011	0.971	0.000	0.030	0.000	0.983	1.333	H1-3+VT ✓

Section Capacity Table

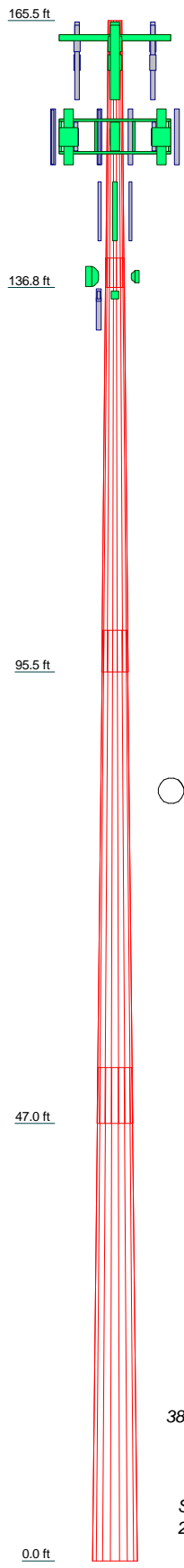
Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF* P_{allow} K	% Capacity	Pass Fail	
L1	165.5 - 136.83	Pole	TP24.279x17x0.1875	1	-4.75	720.46	60.0	Pass	
L2	136.83 - 95.5	Pole	TP34.4x23.0992x0.3125	2	-10.46	1698.78	73.2	Pass	
L3	95.5 - 47	Pole	TP46.06x32.6322x0.375	3	-19.84	2732.81	70.9	Pass	
L4	47 - 0	Pole	TP57.275x43.7899x0.375	4	-25.00	2975.59	73.7	Pass	
							Summary		
							Pole (L4)	73.7	Pass
							RATING =	73.7	Pass

APPENDIX B BASE LEVEL DRAWING



APPENDIX C
ADDITIONAL CALCULATIONS

Section	1	2	3	4
Length (ft)	28.67	44.50	53.00	53.00
Number of Sides	18	18	18	18
Thickness (in)	0.1875	0.3125	0.3750	0.3750
Socket Length (ft)	3.17	4.50	6.00	43.7899
Top Dia (in)	17.0000	23.0992	32.6322	43.7899
Bot Dia (in)	24.2790	34.4000	46.0600	57.2750
Grade		A572-65		
Weight (K)	1.2	4.3	8.4	10.8



DESIGNED APPURTENANCE LOADING

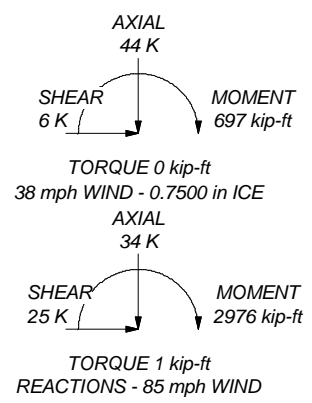
TYPE	ELEVATION	TYPE	ELEVATION
T-Arm Mount [TA 602-3]	164	(2) SBNHH-1D65B w/ Mount Pipe	153
SBNH-1D65C-SR w/ Mount Pipe	164	(2) SBNHH-1D65B w/ Mount Pipe	153
SBNH-1D65C-SR w/ Mount Pipe	164	RRH2X60-PCS	153
SBNH-1D65C-SR w/ Mount Pipe	164	RRH2X60-PCS	153
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	164	RRH2X60-PCS	153
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	164	RRH2X60-AWS	153
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	164	RRH2X60-AWS	153
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	164	RRH2X60-AWS	153
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	164	RRH2X60-700	153
KRY 112 144/1	164	RRH2x60-700	153
KRY 112 144/1	164	RRH2x60-700	153
KRY 112 144/1	164	(2) DB-T1-6Z-8AB-0Z	153
RRUS 11 B12	164	Pipe Mount [PM 601-3]	145
RRUS 11 B12	164	742 213 w/ Mount Pipe	145
RRUS 11 B12	164	742 213 w/ Mount Pipe	145
RRUS 11 B2	164	742 213 w/ Mount Pipe	145
RRUS 11 B2	164	Side Arm Mount [SO 104-3]	136
RRUS 11 B2	164	Pipe Mount [PM 601-3]	136
Platform Mount [LP 304-1]	153	840 10054 w/ Mount Pipe	136
(2) LPA-80080/6CF w/ Mount Pipe	153	WIMAX DAP HEAD	136
(2) LPA-80080/6CF w/ Mount Pipe	153	HORIZON COMPACT	136
(2) LPA-80080/6CF w/ Mount Pipe	153	HORIZON COMPACT	136
(2) SBNHH-1D65B w/ Mount Pipe	153	A-ANT-23G-1-C	136
		A-ANT-18G-2-C	136

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in Hartford County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 38 mph basic wind with 0.75 in ice.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 73.7%



Paul J. Ford and Company
 250 E. Broad Street Suite 600
 Columbus, OH 43215
 Phone: 614.221.6679
 FAX: 614.448.4105

Job: **166-ft Monopole / CT SUFFIELD 3 CAC 801487**
 Project: **PJF 37516-0632 / BU 801487**
 Client: Crown Castle | Drawn by: Jonathan Sommer | App'd:
 Code: TIA/EIA-222-F | Date: 03/03/16 | Scale: NTS
 Path: | Dwg No. E-1

Stiffened or Unstiffened, UngROUTed, Circular Base Plate - Any Rod Material

TIA Rev F

Site Data	
BU#:	801487
Site Name:	CT SUFFIELD 3 CAC 801-
App #:	
Pole Manufacturer:	Other

Reactions		
Moment:	2976	ft-kips
Axial:	34	kips
Shear:	25	kips

Anchor Rod Data		
Qty:	16	
Diam:	2.25	in
Rod Material:	A615-J	
Strength (Fu):	100	ksi
Yield (Fy):	75	ksi
Bolt Circle:	64	in

If No stiffeners, Criteria: **AISC ASD** <-Only Applicable to Unstiffened Cases

Anchor Rod Results						
Maximum Rod Tension:	137.4 Kips		<table border="1"> <tr><td>Rigid</td></tr> <tr><td>Service, ASD</td></tr> <tr><td>Fty*ASIF</td></tr> </table>	Rigid	Service, ASD	Fty*ASIF
Rigid						
Service, ASD						
Fty*ASIF						
Allowable Tension:	195.0 Kips					
Anchor Rod Stress Ratio:	70.5% Pass					

Plate Data		
Diam:	70	in
Thick:	2.75	in
Grade:	60	ksi
Single-Rod B-eff:	11.36	in

Base Plate Results		Flexural Check					
Base Plate Stress:	21.2 ksi		<table border="1"> <tr><td>Rigid</td></tr> <tr><td>Service ASD</td></tr> <tr><td>0.75*Fy*ASIF</td></tr> <tr><td>Y.L. Length: 28.56</td></tr> </table>	Rigid	Service ASD	0.75*Fy*ASIF	Y.L. Length: 28.56
Rigid							
Service ASD							
0.75*Fy*ASIF							
Y.L. Length: 28.56							
Allowable Plate Stress:	60.0 ksi						
Base Plate Stress Ratio:	35.4% Pass						

Stiffener Data (Welding at both sides)		
Config:		*
Weld Type:		
Groove Depth:		in **
Groove Angle:		degrees
Fillet H. Weld:		<-- Disregard
Fillet V. Weld:		in
Width:		in
Height:		in
Thick:		in
Notch:		in
Grade:		ksi
Weld str.:		ksi

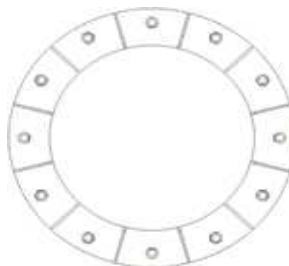
n/a

Stiffener Results	
Horizontal Weld :	n/a
Vertical Weld:	n/a
Plate Flex+Shear, fb/Fb+(fv/Fv)^2:	n/a
Plate Tension+Shear, ft/Ft+(fv/Fv)^2:	n/a
Plate Comp. (AISC Bracket):	n/a

Pole Results	
Pole Punching Shear Check:	n/a

Pole Data		
Diam:	57.275	in
Thick:	0.375	in
Grade:	65	ksi
# of Sides:	18	"0" IF Round
Fu	80	ksi
Reinf. Fillet Weld	0	"0" if None

Stress Increase Factor	
ASIF:	1.333



* 0 = none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

** Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

Foundation Loads:

Pole weight or tower leg compression = 34 (kips)
 Horizontal load at top of pier = 25 (kips)
 Overturning moment at top of pier = 2976 (ft-kips)

Design criteria:

Safety factor against overturning = 1.5

Soil Properties:

Soil density = 120 (pcf)
 Allowable soil bearing = 5 (ksf)
 Depth to water table = 19 (ft)

Dimensions:

Pier shape (round or square) R ("R" or "S")
 Pier width = 7.5 (ft)
 Pier height above grade = 0.5 (ft)
 depth to bottom of footing = 6.5 (ft)
 Footing thickness = 2.5 (ft)
 Footing width = 30 (ft)
 Footing length = 30 (ft)

Concrete:

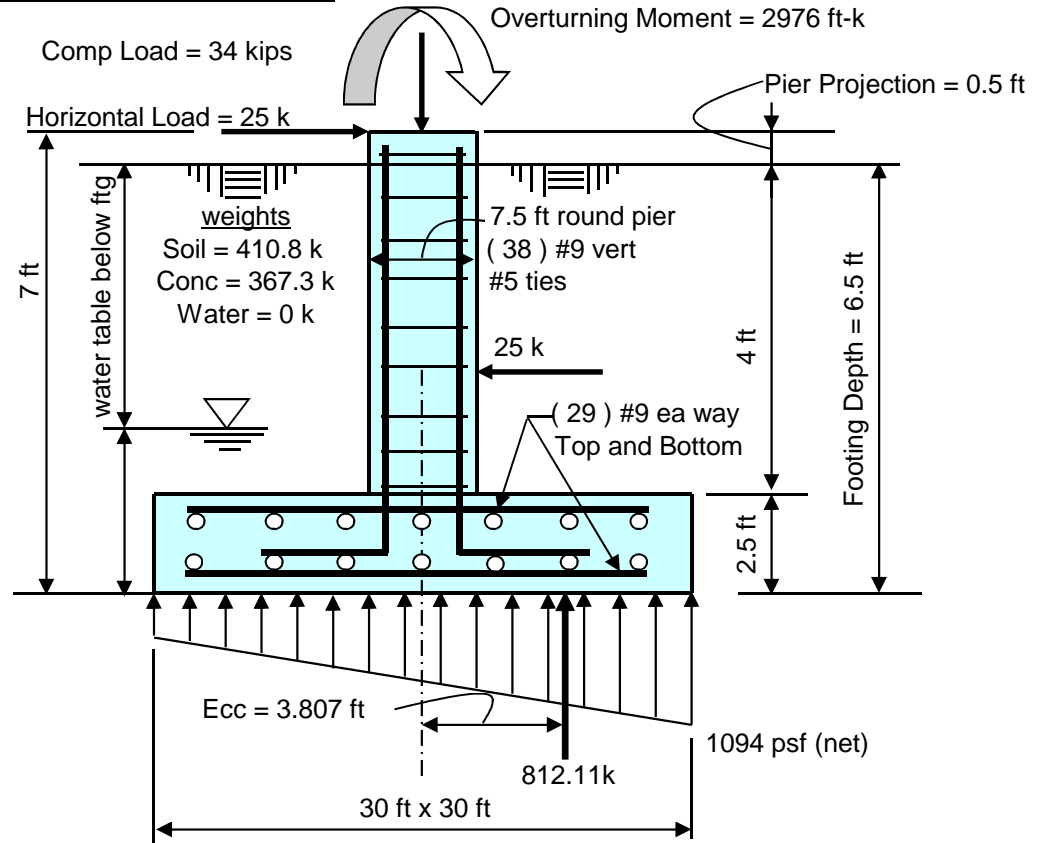
Concrete strength = 3 (ksi)
 Rebar strength = 60 (ksi)
 ultimate load factor = 1.3

Reinforcing Steel:

Pad
 minimum cover over rebar = 3.5 inches
 size of pad rebar = #9 bar
 quantity of pad rebar = 29 (ea direction)

Reinforcing Steel:

Pier
 size of vert rebar in pier = #9 bar
 vertical rebar quantity = 38
 size of pier ties = #5 bar
 minimum cover over rebar = 3 inches
 Total volume of concrete = 90.7 cu yd



Summary of analysis results	
Maximum Net Soil Bearing = 1.094 ksf Allowable Net Soil Bearing = 5 ksf Soil Bearing Stress Ratio = 0.22 Okay	Ult Bending Shear Capacity = 110 psi Ult Bending Shear Stress = 26 psi Bending Shear Stress Ratio = 0.24 Okay
Ftg Overturning Resistance = 12182 ft-kips Overturning Moment = 3092 ft-kips Required Overturning Safety Factor = 1.5 Overturning Safety Factor = 3.94 Ratio = 0.38 Okay	Pad Bending Moment Capacity = 3114 ft-k Pad Bending Moment = 1389 ft-k Bending Moment Stress Ratio = 0.45 OK

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT
EVALUATION OF HUMAN EXPOSURE POTENTIAL
TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CTHA160A

Suffield/Stony Brook
848 East Street South
Suffield, CT 06078

March 11, 2016

EBI Project Number: 6216001504

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general public allowable limit:	5.27 %

March 11, 2016

T-Mobile USA
Attn: Jason Overbey, RF Manager
35 Griffin Road South
Bloomfield, CT 06002

Emissions Analysis for Site: **CTHA160A – Suffield/Stony Brook**

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **848 East Street South, Suffield, CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limit for the 700 MHz Band is $467 \mu\text{W}/\text{cm}^2$, and the general population exposure limit for the PCS and AWS bands is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at **848 East Street South, Suffield, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 GSM channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel
- 2) 2 UMTS channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 3) 2 LTE channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 4) 1 LTE channel (700 MHz Band) was considered for each sector of the proposed installation. This channel has a transmit power of 30 Watts.
- 5) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.

- 6) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antennas used in this modeling are the **Ericsson AIR21 B4A/B2P** for 1900 MHz (PCS) and 2100 MHz (AWS) channels and the **Commscope SBNHH-1D65C-SR** for 2100 MHz (AWS) and 700 MHz channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The **Ericsson AIR21 B4A/B2P** has a maximum gain of **15.9 dBd** at its main lobe at both 1900 MHz and 2100 MHz. The **Commscope SBNHH-1D65C-SR** has a maximum gain of **15.3 dBd** at its main lobe at 2100 MHz and has a maximum gain of **13.6 dBd** at its main lobe at 700 MHz. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antenna mounting height centerlines of the proposed antennas are **161 & 163 feet** above ground level (AGL).
- 9) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general public threshold limits.

T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Ericsson AIR21 B4A/B2P	Make / Model:	Ericsson AIR21 B4A/B2P	Make / Model:	Ericsson AIR21 B4A/B2P
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	163	Height (AGL):	163	Height (AGL):	163
Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz(PCS) / 2100 MHz (AWS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power(W):	120	Total TX Power(W):	120	Total TX Power(W):	120
ERP (W):	4,668.54	ERP (W):	4,668.54	ERP (W):	4,668.54
Antenna A1 MPE%	0.68	Antenna B1 MPE%	0.68	Antenna C1 MPE%	0.68
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Commscope SBNHH-1D65C-SR	Make / Model:	Commscope SBNHH-1D65C-SR	Make / Model:	Commscope SBNHH-1D65C-SR
Gain:	15.3 / 13.6 dBd	Gain:	15.3 / 13.6 dBd	Gain:	15.3 / 13.6 dBd
Height (AGL):	161	Height (AGL):	161	Height (AGL):	161
Frequency Bands	2100 MHz (AWS) / 700 MHz	Frequency Bands	2100 MHz (AWS) / 700 MHz	Frequency Bands	2100 MHz (AWS) / 700 MHz
Channel Count	3	Channel Count	3	Channel Count	3
Total TX Power(W):	150	Total TX Power(W):	150	Total TX Power(W):	150
ERP (W):	4,753.39	ERP (W):	4,753.39	ERP (W):	4,753.39
Antenna A2 MPE%	0.83	Antenna B2 MPE%	0.83	Antenna C2 MPE%	0.83

Site Composite MPE%	
Carrier	MPE%
T-Mobile	1.51
Clearwire	0.11 %
MetroPCS	0.63 %
Verizon Wireless	3.02 %
Site Total MPE %:	5.27 %

T-Mobile Sector 1 Total:	1.51 %
T-Mobile Sector 2 Total:	1.51 %
T-Mobile Sector 3 Total:	1.51 %
Site Total:	5.27 %

T-Mobile _per sector	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ($\mu\text{W}/\text{cm}^2$)	Frequency (MHz)	Allowable MPE ($\mu\text{W}/\text{cm}^2$)	Calculated % MPE
T-Mobile 1900 MHz (PCS) GSM/UMTS	2	1167.14	163	3.40	1900	1000	0.34 %
T-Mobile 2100 MHz (AWS) UMTS	2	1167.14	163	3.40	2100	1000	0.34 %
T-Mobile 2100 MHz (AWS) LTE	2	2033.07	161	6.08	2100	1000	0.61 %
T-Mobile 700 MHz LTE	1	687.26	161	1.03	700	467	0.22 %
						Total:	1.51%

Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general public exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general public exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector 1:	1.51 %
Sector 2:	1.51 %
Sector 3 :	1.51 %
T-Mobile Total:	1.51 %
Site Total:	5.27 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **5.27%** of the allowable FCC established general public limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



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