



**Crown Castle**  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065

June 7, 2022

Melanie A. Bachman  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

**RE: Notice of Exempt Modification for T-Mobile: CTHA534A**  
**Crown Site ID# 806378**  
**126 Pioneer Heights Road, Somers, CT 06071**  
**Latitude: 41° 56' 55.98" / Longitude: -72° 29' 31.55"**

Dear Ms. Bachman:

T-Mobile currently maintains six (6) antennas at the 115-foot mount on the existing 161'-5"-foot monopole tower located at 126 Pioneer Heights Road, Somers, CT. The property and tower are owned by Crown Castle. T-Mobile now intends to replace six (6) antennas and ancillary equipment at the 115 ft level. This modification/proposal includes hardware that is both 4G (LTE) and 5G capable through remote software configuration and either or both services may be turned on or off at various times.

**Panned Modification:**

**Tower:**

Installed New:

- (3) Ericsson Air 6419 B41 Antennas
- (3) Commscope- W-65A-R1 Antennas
- (3) Ericsson-Radio 4460 B25+ B66 RRU
- (1) Hybrid Cable 6x24

Remove:

- (6) Ericsson – AIR21 KRC118023-1\_B2P\_B4A Antennas
- (3) Ericsson Twin Style 1BX -KRY 112 144/2 TMAs
- (3) Ericsson Twin Style 1A-KRY 112 71/2 TMAs
- (12) 1-5/8" Coaxial Cables

**Ground:**

Install New:

- (1.) RBS 6601
- (1) RP 6651
- (1) PSU 4813 Voltage Booster
- (1) IXRE Router

The Foundation for a Wireless World.  
CrownCastle.com

- (3) Rectifiers for 6160 cabinet
- (6) 40amp Breakers
- (3) 50amp SPDS

The facility was approved by the Connecticut Siting Council on, Docket No58 on July 11, 1986.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Tim Keeney, First Selectman, Town of Somers and Jennifer Roy, Zoning Enforcement Officer, Town of Somers. Crown Castle is the property and tower owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Jeffrey Barbadora.

Sincerely,



Jeffrey Barbadora  
Site Acquisition Specialist  
1800 W. Park Drive  
Westborough, MA 01581  
(781) 970-0053  
Jeff.Barbadora@crowncastle.com

Melanie A. Bachman

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Attachments

cc:

Tim Keeney, First Selectman  
Town of Somers  
600 Main Street  
Somers, CT 06071  
860-763-8200

Jennifer Roy, Zoning Enforcement Officer  
Town of Somers  
600 Main Street  
Somers, CT 06071  
860-763-8220

Crown Castle – Property & Tower Owner

DOCKET NO. 58

AN APPLICATION OF HARTFORD CELLULAR  
COMPANY FOR A CERTIFICATE OF  
ENVIRONMENTAL COMPATIBILITY AND PUBLIC  
NEED FOR THE CONSTRUCTION, MAINTENANCE,  
AND OPERATION OF FACILITIES TO PROVIDE  
CELLULAR SERVICE IN HARTFORD, TOLLAND AND  
MIDDLESEX COUNTIES.

CONNECTICUT SITING  
COUNCIL

July 11, 1986.

D E C I S I O N A N D O R D E R

Pursuant to the foregoing opinion, the Connecticut Siting Council (Council) hereby directs that a Certificate of Environmental Compatibility and Public Need as provided by Section 16-50k of the General Statutes of Connecticut (CGS) be issued to the Hartford Cellular Company for the construction, maintenance, and operation of cellular mobile phone telecommunication towers and associated equipment in the towns of Glastonbury, Haddam, Hartford, Portland, Rocky Hill, Somers, Vernon, Windsor, and Willington subject to the conditions below.

- 1) The proposed Bloomfield and Middlefield sites are rejected without prejudice.
- 2) The antennas on the Glastonbury tower shall be mounted no higher than the 180' level of this existing tower.
- 3) The Portland and Rocky Hill towers shall be monopoles.
- 4) The towers shall be no taller than necessary to provide the proposed service, and in no event shall exceed total heights, including antennas, of
  - a) 193' at the Haddam site;
  - b) 173' at the Portland site;

- c) 153' at the Rocky Hill site;
- d) 173' at the Somers site;
- e) 173' at the Vernon site;
- f) 153' at the Willington site;
- g) 173' at the Windsor site.

5) The Hartford site receive antennas shall be mounted below the top of the high point of the building to preclude visibility.

6) Any future actions requiring the removal of the existing Glastonbury tower to be shared by the certificate holder shall also apply to the equipment mounted on that tower by the certificate holder, regardless of that equipment's status under Chapter 277a of the CGS.

7) The certificate holder shall submit a development and management (D&M) plan for the Haddam, Portland, Rocky Hill, Somers, Vernon and Windsor sites pursuant to Sections 16-50j-75 through 16-50j-77 of the Regulations of State Agencies (RSA), except that irrelevant items in Section 16-50j-76 need only be identified as such. In addition to the requirements of Section 16-50j-76, the D&M plan shall provide plans for evergreen screening around the fenced perimeter at the Haddam, Somers, Vernon, and Windsor sites. The D&M plan shall include a proposal for painting the approved monopole structures to blend with the sky. The D&M plan must be approved prior to facility construction. Any changes to specifications in the D&M plan must be approved by the Council prior to facility operation.

8) All certified facilities shall be constructed, operated, and maintained as specified in the Council's record and in the

site plan required by order number 7.

9) The certificate holder shall comply with any future radiofrequency (RF) standards promulgated by state or federal regulatory agencies. Upon the establishment of any new governmental RF standards, the facilities granted in this decision shall continue to be in compliance with such standards.

10) The certificate holder shall permit public or private entities to share space on the towers approved herein, for due consideration received, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing. In addition to complying with Section 16-50j-73 of the RSA, the certificate holder shall notify the Council of the addition of any equipment to any approved tower.

11) A fence not lower than 8' shall surround each tower and associated equipment.

12) Unless necessary to comply with order 13, no lights shall be installed on any of these towers.

13) The facilities' construction and any future tower sharing shall be in accordance with all applicable federal, state, and municipal laws and regulations. Shared uses by entities not subject to jurisdiction pursuant to Section 16-50k of the CGS shall be subject to all applicable federal, state, and municipal laws and regulations.

14) Construction activities shall take place during daylight working hours.

15) This decision and order shall be void and the towers and associate equipment shall be dismantled and removed, or reapplication for any new use shall be made to the Council before any such new use is made, if the towers do not provide or permanently cease to provide cellular service following completion of construction.

16) This decision and order shall be void if all construction authorized herein is not completed within three years of the issuance of this decision, or within three years of the completion of any appeal if appeal of this decision is taken, unless otherwise approved by the Council.

Pursuant to CGS Section 16-50p, we hereby direct that a copy of the decision and order shall be served on each person listed below. A notice of the issuance shall be published in the Hartford Courant, Middletown Press, Manchester Journal Inquirer, and the Willimantic Chronicle.

The parties to the proceeding are:

Metro Mobile (applicant)  
5 Eversley Avenue  
Norwalk, Connecticut 06855  
ATTN: Armand Mascioli  
General Manager

Howard L. Slater, Esq. (its attorneys)  
Scott A. Gursky, Esq.  
Byrne, Slater, Sandler,  
Shulman & Rouse, P.C.  
111 Pearl Street  
Hartford, Connecticut 06103

Richard Rubin, Esq.  
Fleischman and Walsh, P.C.  
1725 N Street, N.W.  
Washington, D. C. 20036

Mr. William Wamester  
1225 Randolph Road  
Middletown, Connecticut 06457

The Southern New England Telephone Company  
227 Church Street  
New Haven, Connecticut 06506  
ATTN: Peter J. Tyrrell, Esq.

Mr. James W. Tilney

represented by:  
Patricia A. Ayars  
Samuel Baily, Jr.  
Robinson & Cole  
One Commercial Plaza  
Hartford, CT. 06103-3597

Mr. Samuel DuBosar, Chairman  
Bessie Bennett, Esq.  
Town Plan & Zoning Commission  
P.O. Box 337  
Bloomfield, Connecticut 06002

Town of Somers

represented by:

Mr. Robert F. Peters  
Town Counsel  
Tatoian, Devline, Peters  
& Davis  
11 South Road  
P.O. Box 415  
Somers, CT. 06071

Town of Haddam  
represented by:

Lucy R. Petrella  
Chairperson  
Town Office Building  
Route 9A  
P.O. Box 87  
Haddam, CT. 06438

Midstate Regional Planning Agency

represented by:

Thomas M. Gilligan  
Regional Planner  
P.O. Box 139  
Middletown, CT. 06457



Dr. Donald P. LaSalle  
Director  
Talcott Mountain Science Center  
Montevideo Road  
Avon, Connecticut 06001

Barnard Tilson (service waived)  
Secretary  
Avon Planning and Zoning  
60 West Main Street  
Avon, Connecticut 06001

Alden Giddings  
33 Privelege Road  
Bloomfield, Connecticut 06002

Town of Bloomfield

represented by:

Joseph M. Suggs, Jr.  
Deputy Mayor  
Town Hall  
880 Bloomfield Avenue  
P.O. Box 337  
Bloomfield, CT. 06002  
(service waived)

Town of Middlefield

represented by:

David Silverstone, Esq.  
Silverstone & Koontz  
37 Lewis Street  
Hartford, CT. 06103

with a copy to:

Geoffrey Colegrove  
Midstate Regional Planning Agency  
100 DeKoven Drive  
Middletown, CT. 06457

Zoning Commission  
Town of Somers

represented by:

Joseph A. Paradis  
Chairman  
Town Hall  
600 Main Street  
P.O. Box 803  
Somers, CT. 06071

Barbara Sirwilo, Secretary (service waived)  
Planning & Zoning Commission  
Town of Rocky Hill  
600 Old Main Street  
P.O. Box 657  
Rocky Hill, Connecticut 06067

H. Robert Goodrich (service waived)  
Goodrich Lane  
Portland, Connecticut 06480

The Honorable Richard P. Antonetti  
State Representative (service waived)  
5 Sachem Circle  
Meriden, Connecticut 06450

John Hevrin  
R.D. #1 - Plains Road  
Haddam, Connecticut 06438

Norman and Darlene Manning (represented by)

Elizabeth Allen, Esq.  
P.O. Box 467  
Higganum, CT. 06441  
(service waived)

C E R T I F I C A T I O N

The undersigned members of the Connecticut Siting Council hereby certify that they have heard this case or read the record thereof, and that we voted as follows:


Dated at New Britain, Connecticut, this 11th day of July, 1986.

<u>Council Members</u>	<u>Vote Cast</u>
_____) Gloria Dibble Pond Chairperson	Absent
<i>Patricia J. Shea</i> _____) Commissioner John Downey Designee: Patricia Shea	Yes
<i>Stanley Pac</i> _____) Commissioner Stanley Pac Designee: Christopher Cooper	Yes
<i>Owen L. Clark</i> _____) Owen L. Clark	Yes
<i>Mortimer A. Gelston</i> _____) Mortimer A. Gelston	Yes
<i>James G. Horsfall</i> _____) James G. Horsfall	Yes
_____) Pamela B. Katz	Absent
<i>William H. Smith</i> _____) William H. Smith	Yes
<i>Colin C. Tait</i> _____) Colin C. Tait	Yes

STATE OF CONNECTICUT            )  
  :  
COUNTY OF HARTFORD            )        ss.        New Britain, July 11, 1986

I hereby certify that the foregoing is a true and correct copy of the decision and order issued by the Connecticut Siting Council, State of Connecticut.

ATTEST:

  
\_\_\_\_\_  
Christopher S. Wood, Executive Director  
Connecticut Siting Council



Property Information

Property Location	126 PIONEER HEIGHTS
Owner	FARNHAM LENA G & FAYE F GATELY
Co-Owner	C/O CROWN ATLANTIC CO LLC
Mailing Address	PMB 353 4017 WASHINGTON R MCMURRAY PA 15317
Land Use	299 Vac Comm Lnd
Land Class	C
Zoning Code	A-1
Census Tract	5382.01

Neighborhood	C
Acreage	0.5
Utilities	
Lot Setting/Desc	
Book / Page	0280/0125
Additional Info	

Primary Construction Details

Year Built	0
Building Desc.	Vac Comm Lnd
Building Style	UNKNOWN
Building Grade	
Stories	
Occupancy	
Exterior Walls	
Exterior Walls 2	NA
Roof Style	
Roof Cover	
Interior Walls	
Interior Walls 2	NA
Interior Floors 1	
Interior Floors 2	NA

Heating Fuel	
Heating Type	
AC Type	
Bedrooms	0
Full Bathrooms	0
Half Bathrooms	0
Extra Fixtures	0
Total Rooms	0
Bath Style	NA
Kitchen Style	NA
Fin Bsmt Area	0
Fin Bsmt Quality	0
Bsmt Gar	0
Fireplaces	0

(\*Industrial / Commercial Details)

Building Use	Vacant
Building Condition	
Sprinkler %	NA
Heat / AC	NA
Frame Type	NA
Baths / Plumbing	NA
Ceiling / Wall	NA
Rooms / Prtns	NA
Wall Height	NA
First Floor Use	NA
Foundation	NA

Photo



Sketch





# Town of Somers, CT

Property Listing Report

Map Block Lot 01-13-A

Building # 1

PID 1814

Account 00228200

## Valuation Summary (Assessed value = 70% of Appraised Value)

Item	Appraised	Assessed
Buildings	0	0
Extras	0	0
Improvements		
Outbuildings	111600	78200
Land	335000	234500
<b>Total</b>	<b>446600</b>	<b>312700</b>

## Sub Areas

Subarea Type	Gross Area (sq ft)	Living Area (sq ft)
<b>Total Area</b>	<b>0</b>	<b>0</b>

## Outbuilding and Extra Features

Type	Description
Fence, Chain	400 LF
PreCast Cell Shed	315 SF
PreCast Cell Shed	192 SF
Tower	160 LF
PreCast Cell Shed	200 SF

## Sales History

Owner of Record	Book/ Page	Sale Date	Sale Price
FARNHAM LENA G & FAYE F GATELY	0280/0125	2008-08-21	0
FARNHAM CLARENCE D JR ET AL	0255/0671	2005-11-28	0

# Town of Somers, Connecticut - Assessment Parcel Map

Parcel: 01-13-A

Address: 126 PIONEER HEIGHTS



Approximate Scale: 1 inch = 100 feet

Map Produced May 2022

Disclaimer: This map is for informational purposes only. All information is subject to verification by any user. The Town of Somers and its mapping contractors assume no legal responsibility for the information contained herein.

**Barbadora, Jeff**

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**From:** TrackingUpdates@fedex.com  
**Sent:** Wednesday, June 8, 2022 10:53 AM  
**To:** Barbadora, Jeff  
**Subject:** FedEx Shipment 777063289760: Your package has been delivered

**CAUTION:** This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.



Hi. Your package was  
delivered Wed, 06/08/2022 at  
10:50am.



Delivered to 600 MAIN ST, SOMERS, CT 06071  
Received by D.MURPHY

**OBTAIN PROOF OF DELIVERY**

TRACKING NUMBER [777063289760](#)



FROM	Jeff Barbadora 1800 W. Park Drive WESTBOROUGH, MA, US, 01581
TO	Town of Somers Tim Keeney, First Selectman 600 Main Street SOMERS, CT, US, 06071
REFERENCE	799001.7680
SHIPPER REFERENCE	799001.7680
SHIP DATE	Tue 6/07/2022 05:41 PM
DELIVERED TO	Shipping/Receiving
PACKAGING TYPE	FedEx Envelope
ORIGIN	WESTBOROUGH, MA, US, 01581
DESTINATION	SOMERS, CT, US, 06071
SPECIAL HANDLING	Deliver Weekday
NUMBER OF PIECES	1
TOTAL SHIPMENT WEIGHT	1.00 LB
SERVICE TYPE	FedEx Priority Overnight

**Barbadora, Jeff**

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**From:** TrackingUpdates@fedex.com  
**Sent:** Wednesday, June 8, 2022 10:54 AM  
**To:** Barbadora, Jeff  
**Subject:** FedEx Shipment 777063322990: Your package has been delivered

**CAUTION:** This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.



Hi. Your package was  
delivered Wed, 06/08/2022 at  
10:50am.



Delivered to 600 MAIN ST, SOMERS, CT 06071  
Received by D.MURPHY

**OBTAIN PROOF OF DELIVERY**

TRACKING NUMBER [777063322990](#)

<b>FROM</b>	Jeff Barbadora 1800 W. Park Drive WESTBOROUGH, MA, US, 01581
<b>TO</b>	Town of Somers Jennifer Roy, CZEO 600 Main Street SOMERS, CT, US, 06071
<b>REFERENCE</b>	799001.7680
<b>SHIPPER REFERENCE</b>	799001.7680
<b>SHIP DATE</b>	Tue 6/07/2022 05:41 PM
<b>DELIVERED TO</b>	Shipping/Receiving
<b>PACKAGING TYPE</b>	FedEx Envelope
<b>ORIGIN</b>	WESTBOROUGH, MA, US, 01581
<b>DESTINATION</b>	SOMERS, CT, US, 06071
<b>SPECIAL HANDLING</b>	Deliver Weekday
<b>NUMBER OF PIECES</b>	1
<b>TOTAL SHIPMENT WEIGHT</b>	0.50 LB
<b>SERVICE TYPE</b>	FedEx Priority Overnight

Date: **April 05, 2022**



B+T Group  
1717 S. Boulder, Suite 300  
Tulsa, OK 74119  
(918) 587-4630

**Subject:** **Structural Analysis Report**

**Carrier Designation:** **Metro PCS Co-Locate**  
**Site Number:** CTHA534A  
**Site Name:** Crown Somers Lattice Tower

**Crown Castle Designation:** **BU Number:** 806378  
**Site Name:** HRT 086 943248  
**JDE Job Number:** 709736  
**Work Order Number:** 2097308  
**Order Number:** 609062 Rev. 0

**Engineering Firm Designation:** **B+T Group Project Number:** 136290.007.01

**Site Data:** **126 Pioneer Heights Rd, Somers, Tolland County, CT**  
**Latitude 41° 56' 55.98", Longitude -72° 29' 31.55"**  
**160 Foot - Self Support Tower**

B+T Group is pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Proposed Equipment Configuration

**Sufficient Capacity - 88.5%**

This analysis utilizes an ultimate 3-second gust wind speed of 117 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Carlon Bethell II

Respectfully submitted by: B+T Engineering, Inc.  
COA: PEC.0001564; Expires: 02/01/2023



Chad E. Tuttle, P.E.

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## 1) INTRODUCTION

This tower is a 160 ft. Self Support tower designed by Rohn.

The tower has been modified multiple times to accommodate additional loading.

## 2) ANALYSIS CRITERIA

<b>TIA-222 Revision:</b>	TIA-222-H
<b>Risk Category:</b>	II
<b>Wind Speed:</b>	117 mph
<b>Exposure Category:</b>	C
<b>Topographic Factor:</b>	1
<b>Ice Thickness:</b>	1.5 in
<b>Wind Speed with Ice:</b>	50 mph
<b>Service Wind Speed:</b>	60 mph

**Table 1 - Proposed Equipment Configuration**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
113.0	114.0	3	Commscope	VV-65A-R1_TMO	4	1-5/8
		3	Ericsson	AIR 6419 B41_TMO		
		3	Ericsson	RADIO 4449 B71 B85A_T-MOBILE		
		3	Ericsson	RADIO 4460 B2/B25 B66_TMO		
	3	Rfs Celwave	APXVAALL24_43-U-NA20_TMO			
	113.0	1	--	Sector Mount [SM 503-3]		

**Table 2 - Other Considered Equipment**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
157.0	158.0	1	Raycap	RVZDC-6627-PF-48_CCIV2	14	1-5/8
		3	Samsung Telecomm.	RF4439D-25A		
		3	Samsung Telecomm.	RF4440D-13A		
	157.0	2	Antel	LPA-80063/4CF		
		2	Antel	LPA-80063/4CFX5		
		6	Commscope	NHH-65B-R2B		
		2	Rfs Celwave	APL866513-42T6		
		3	Samsung Telecomm.	MT6407-77A		
		3	--	BSAMNT-SBS-1-2		
		3	Site Pro1	SPTB-NP Tie back Kit		
1	--	Sector Mount [SM 505-3]				
147.0	147.0	3	Fujitsu	TA08025-B604	1	1-1/2
		3	Fujitsu	TA08025-B605		
		3	Jma Wireless	MX08FRO665-21		
		1	Raycap	RDIDC-9181-PF-48		
		1	--	Commscope MTC3975083 (3)		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
135.0	137.0	3	CCI Antennas	HPA-65R-BUU-H8	12 4 2	1-1/4 3/4 3/8
		3	CCI Antennas	TPA-65R-LCUUUU-H8		
		3	Ericsson	RRUS 11		
		3	Ericsson	RRUS 4415 B25		
		3	Ericsson	RRUS 4426 B66		
		3	Ericsson	WCS RRUS-32-B30		
		3	Kaelus	DBC0062F1V51-1		
		6	Powerwave Tech.	7020.00		
		3	Powerwave Tech.	7770.00		
		3	Powerwave Tech.	TT19-08BP111-001		
		2	Raycap	DC6-48-60-18-8F		
		135.0	1	--		
125.0	126.0	3	Alcatel Lucent	TD-RRH8x20-25	3 1	1-1/4 5/8
		1	Rfs Celwave	APXV9ERR18-C-A20		
		2	Rfs Celwave	APXVSPP18-C-A20		
		3	Rfs Celwave	APXVTM14-C-120		
	125.0	3	Alcatel Lucent	1900MHz RRH (65MHz)		
		3	Alcatel Lucent	800MHz 2X50W RRH W/FILTER		
		1	--	Sector Mount [SM 402-3]		
57.0	60.0	1	Gps	GPS_A	1	1/2
	57.0	1	--	Side Arm Mount [SO 202-1]		
48.0	48.0	1	--	Side Arm Mount [SO 202-1]	1	1/2

### 3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Reference	Source
Tower Manufacturer Drawing	7175605	CCI Sites
Mount Analysis Report	10272575	CCI Sites
Tower Modification Drawing	866858	CCI Sites
Tower Modification Drawing/PMI	1278690	CCI Sites
Tower Modification Drawing	2961397	CCI Sites
Post Modification Inspection	2961404	CCI Sites
Tower Modification Drawing	3265393	CCI Sites
Post Modification Inspection	3684249	CCI Sites
Tower Modification Drawing	5615504	CCI Sites
Post Modification Inspection	5852475	CCI Sites
Tower Modification Drawing	7498454	CCI Sites
Post Modification Inspection	8011021	CCI Sites
Tower Modification Drawing	5264915	CCI Sites
Foundation Drawing	262063	CCI Sites

Document	Reference	Source
Geotech Report	1275233	CCI Sites
Crown CAD Package	Date: 03/28/2022	CCI Sites

### 3.1) Analysis Method

tnxTower (version 8.1.1.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

tnxTower was used to determine the loads on the modified structure. Additional calculations were performed to determine the stresses in the reinforced leg sections. These calculations are presented in Appendix C.

### 3.2) Assumptions

- 1) The tower and structures were maintained in accordance with the - TIA-222 standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. B+T Group should be notified to determine the effect on the structural integrity of the tower.

## 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	160 - 140	Leg	ROHN 2 STD	2	-19.913	38.684	51.5	Pass
T2	140 - 135	Leg	ROHN 2.5 EH	38	-25.397	78.149	32.5	Pass
T3	135 - 130	Leg	ROHN 2.5 EH	50	-35.767	78.149	45.8	Pass
T4	130 - 125	Leg	ROHN 2.5 EH	59	-44.078	78.149	56.4	Pass
T5	125 - 120	Leg	ROHN 2.5 EH	68	-53.774	78.149	68.8	Pass
T6	120 - 113.333	Leg	ROHN 3 EH	77	-63.946	99.059	64.6	Pass
T7	113.333 - 106.667	Leg	ROHN 3 EH	86	-77.956	99.059	78.7	Pass
T8	106.667 - 100	Leg	ROHN 3 EH	95	-91.616	129.331	70.8	Pass
T9	100 - 93.3333	Leg	ROHN 3.5 EH	107	-104.423	132.012	79.1	Pass
T10	93.3333 - 86.6667	Leg	ROHN 3.5 EH	116	-116.891	132.011	88.5	Pass
T11	86.6667 - 80	Leg	ROHN 3.5 EH	125	-128.480	161.634	79.5	Pass
T12	80 - 73.3333	Leg	ROHN 4 X-STR	137	-140.636	167.898	83.8	Pass
T13	73.3333 - 66.6667	Leg	ROHN 4 X-STR	146	-151.443	196.788	77.0	Pass
T14	66.6667 - 60	Leg	ROHN 4 X-STR	158	-162.824	196.814	82.7	Pass
T15	60 - 50	Leg	ROHN 5 EH	170	-176.946	211.314	83.7	Pass
T16	50 - 40	Leg	ROHN 5 EH	179	-193.191	265.798	72.7	Pass
T17	40 - 30	Leg	ROHN 5 X-STR	191	-209.685	265.818	78.9	Pass
T18	30 - 20	Leg	ROHN 5 X-STR	203	-225.572	283.206	79.6	Pass



Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail	
T19	20 - 0	Leg	B+T_BU 806378 - 6.625"x0.34" pipe w/ 2" SR	245	-258.236	306.709	84.2	Pass	
T1	160 - 140	Diagonal	L1 3/4x1 3/4x3/16	9	-3.291	11.646	28.3	Pass	
T2	140 - 135	Diagonal	L1 3/4x1 3/4x3/16	45	-3.312	8.949	37.0	Pass	
T3	135 - 130	Diagonal	L1 3/4x1 3/4x3/16	54	-4.219	8.112	52.0	Pass	
T4	130 - 125	Diagonal	L1 3/4x1 3/4x3/16	63	-4.298	7.371	58.3	Pass	
T5	125 - 120	Diagonal	L2x2x3/16	72	-4.867	10.175	47.8	Pass	
T6	120 - 113.333	Diagonal	L2 1/2x2 1/2x1/4	81	-5.423	19.771	27.4	Pass	
T7	113.333 - 106.667	Diagonal	L2 1/2x2 1/2x1/4	90	-6.800	17.939	37.9	Pass	
T8	106.667 - 100	Diagonal	L2 1/2x2 1/2x1/4	99	-6.753	15.768	42.8	Pass	
T9	100 - 93.3333	Diagonal	L2 1/2x2 1/2x3/16	111	-6.763	12.495	54.1	Pass	
T10	93.3333 - 86.6667	Diagonal	L2 1/2x2 1/2x1/4	120	-6.774	15.132	44.8	Pass	
T11	86.6667 - 80	Diagonal	2L2 1/2x2 1/2x3/16x1/4	129	-7.420	38.394	19.3	Pass	
T12	80 - 73.3333	Diagonal	L 3x3x3/16	141	-6.908	16.573	41.7	Pass	
T13	73.3333 - 66.6667	Diagonal	L 3x3x3/16	150	-7.765	13.948	55.7	Pass	
T14	66.6667 - 60	Diagonal	L 3x3x3/16	162	-7.575	12.860	58.9	Pass	
T15	60 - 50	Diagonal	2L3x3x3/16x1/4	174	-8.757	39.409	22.2	Pass	
T16	50 - 40	Diagonal	2L3x3x3/16x1/4	183	-9.702	35.624	27.2	Pass	
T17	40 - 30	Diagonal	2L3x3x1/4x1/4	195	-9.855	45.249	21.8	Pass	
T18	30 - 20	Diagonal	2L3x3x1/4x1/4	213	-10.695	72.440	14.8	Pass	
T19	20 - 0	Diagonal	2L3 1/2x3 1/2x1/4x1/4	249	-10.871	56.951	19.1	Pass	
T18	30 - 20	Horizontal	L3x3x3/16	205	-3.912	6.453	60.6	Pass	
T8	106.667 - 100	Secondary Horizontal	L1 3/4x1 3/4x1/4	103	-1.590	7.913	20.1	Pass	
T11	86.6667 - 80	Secondary Horizontal	L2x2x3/16	133	-2.229	6.440	34.6	Pass	
T13	73.3333 - 66.6667	Secondary Horizontal	L1 3/4x1 3/4x1/4	154	-2.628	4.441	59.2	Pass	
T14	66.6667 - 60	Secondary Horizontal	L2x2x3/16	166	-2.825	4.702	60.1	Pass	
T16	50 - 40	Secondary Horizontal	L2 1/2x2 1/2x3/16	187	-3.351	7.398	45.3	Pass	
T17	40 - 30	Secondary Horizontal	L3x3x1/4	199	-3.638	14.905	24.4	Pass	
T1	160 - 140	Top Girt	L2x2x1/8	5	-0.622	4.273	14.6	Pass	
T2	140 - 135	Top Girt	L2x2x1/8	40	-0.440	4.273	10.3	Pass	
T18	30 - 20	Redund Horz 1 Bracing	L2x2x3/16	233	-3.912	13.765	28.4	Pass	
T18	30 - 20	Redund Diag 1 Bracing	L2x2x3/16	231	-2.277	9.970	22.8	Pass	
							Summary		
							Leg (T10)	88.5	Pass
							Diagonal (T14)	58.9	Pass
							Horizontal (T18)	60.6	Pass
							Secondary Horizontal	60.1	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
						(T14)		
						Top Girt (T1)	14.6	Pass
						Redund Horz 1 Bracing (T18)	28.4	Pass
						Redund Diag 1 Bracing (T18)	22.8	Pass
						Bolt Checks	84.0	Pass
						Rating =	88.5	Pass

**Table 5 - Tower Component Stresses vs. Capacity – LC7**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1,2	Anchor Rods	Base	66.4	Pass
1,2	Base Foundation (Structure)	Base	41.4	Pass
1,2	Base Foundation (Soil Interaction)	Base	72.7	Pass

<b>Structure Rating (max from all components) =</b>	<b>88.5%</b>
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Notes:

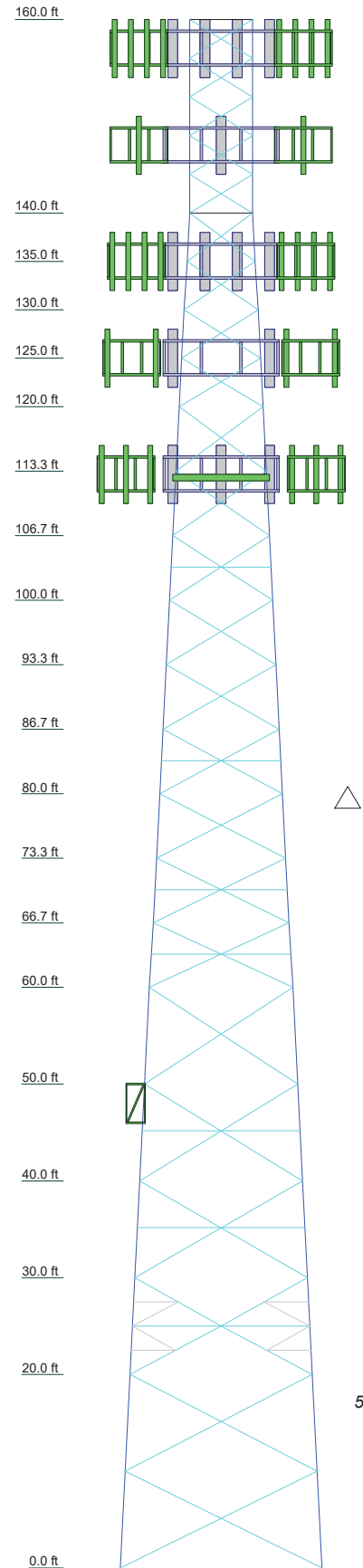
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.
- 2) Rating per TIA-222-H Section 15.5

#### 4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

**APPENDIX A**  
**TNXTOWER OUTPUT**

Section	T1	T2	T3	T4	T5	T6	T7	T8	T9	T10	T11	T12	T13	T14	T15	T16	T17	T18	T19
Legs	ROHN 2 STD																		
Leg Grade	ROHN 2.5 EH																		
Diagonals	ROHN 3.5 EH																		
Diagonal Grade	ROHN 3 EH																		
Top Girts	L1 3/4x1 3/4x3/16																		
Horizontals	L2x2x1/8																		
Sec. Horizontals	N.A.																		
Red. Horizontals	N.A.																		
Red. Diagonals	N.A.																		
Face Width (ft)	20.8646																		
# Panels @ (ft)	5 @ 4																		
Weight (K)	20.9																		



### SYMBOL LIST

MARK	SIZE	MARK	SIZE
A	B+T_BU 806378 - 6.625"x0.34" pipe w/ 2" SR	D	L2 1/2x2 1/2x1/4
B	L2x2x3/16	E	2L2 1/2x2 1/2x3/16x1/4
C	L2 1/2x2 1/2x3/16	F	L1 3/4x1 3/4x1/4

### MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

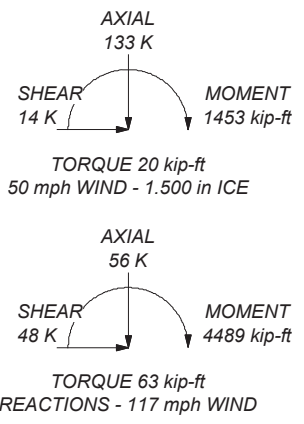
- ### TOWER DESIGN NOTES
1. Tower is located in Tolland County, Connecticut.
  2. Tower designed for Exposure C to the TIA-222-H Standard.
  3. Tower designed for a 117 mph basic wind in accordance with the TIA-222-H Standard.
  4. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
  5. Deflections are based upon a 60 mph wind.
  6. Tower Risk Category II.
  7. Topographic Category 1 with Crest Height of 0'
  8. TIA-222-H Annex S
  9. TOWER RATING: 88.5%


ALL REACTIONS  
ARE FACTORED

MAX. CORNER REACTIONS AT BASE:

DOWN: 267 K  
SHEAR: 30 K

UPLIFT: -228 K  
SHEAR: 26 K



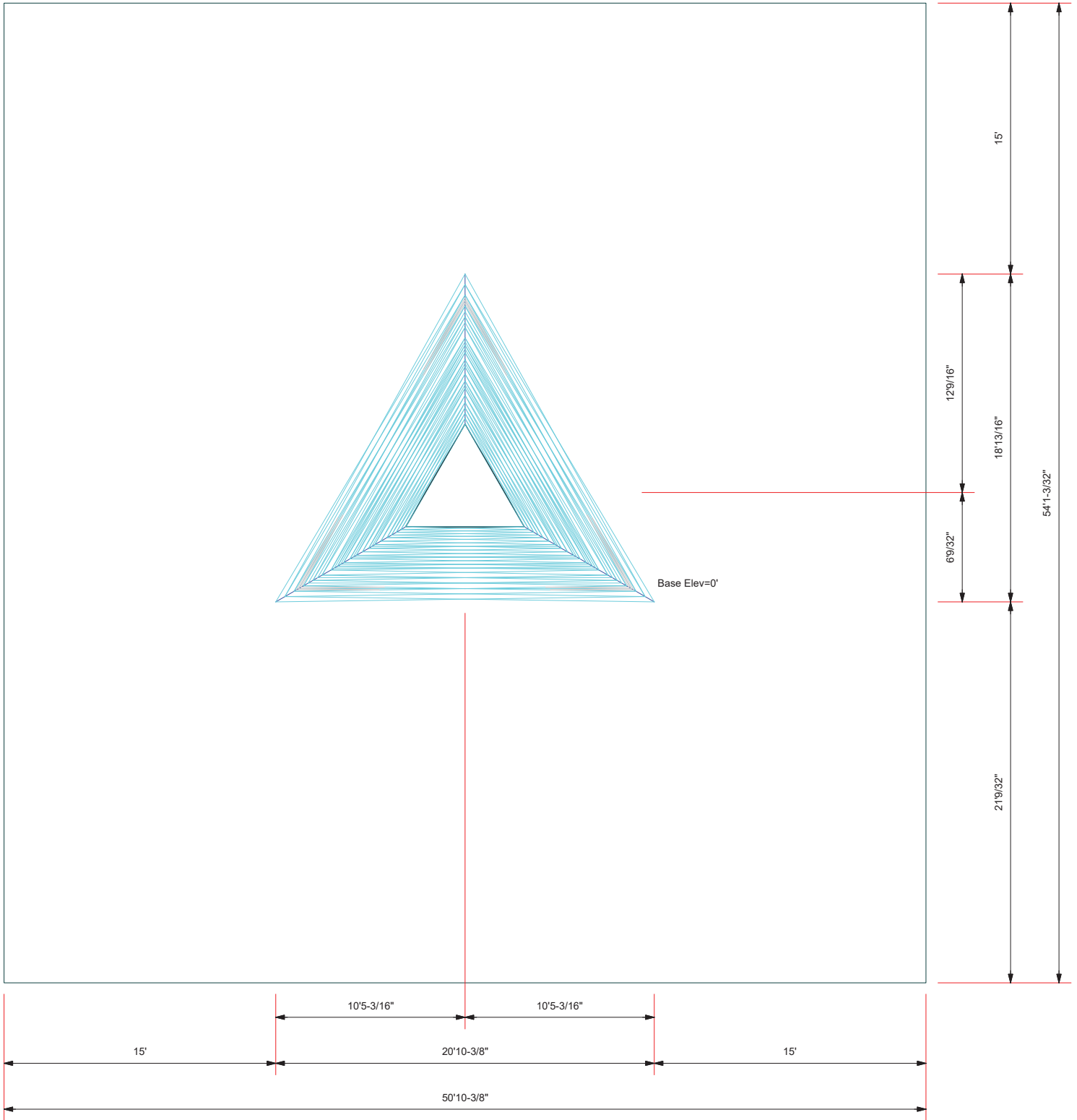


**B+T Group**  
1717 S. Boulder, Suite 300  
Tulsa, OK 74119  
Phone: (918) 587-4630  
FAX: (918) 295-0265

Job: **136290.007.01 - HRT 086 943248, CT (BU# 806378)**

Project:	Client: Crown Castle	Drawn by: Jayaraj B	App'd:
Code: TIA-222-H	Date: 04/05/22	Scale: NTS	Dwg No. E-1

**Plot Plan**  
**Total Area - 0.06 Acres**



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 FAX: (918) 295-0265

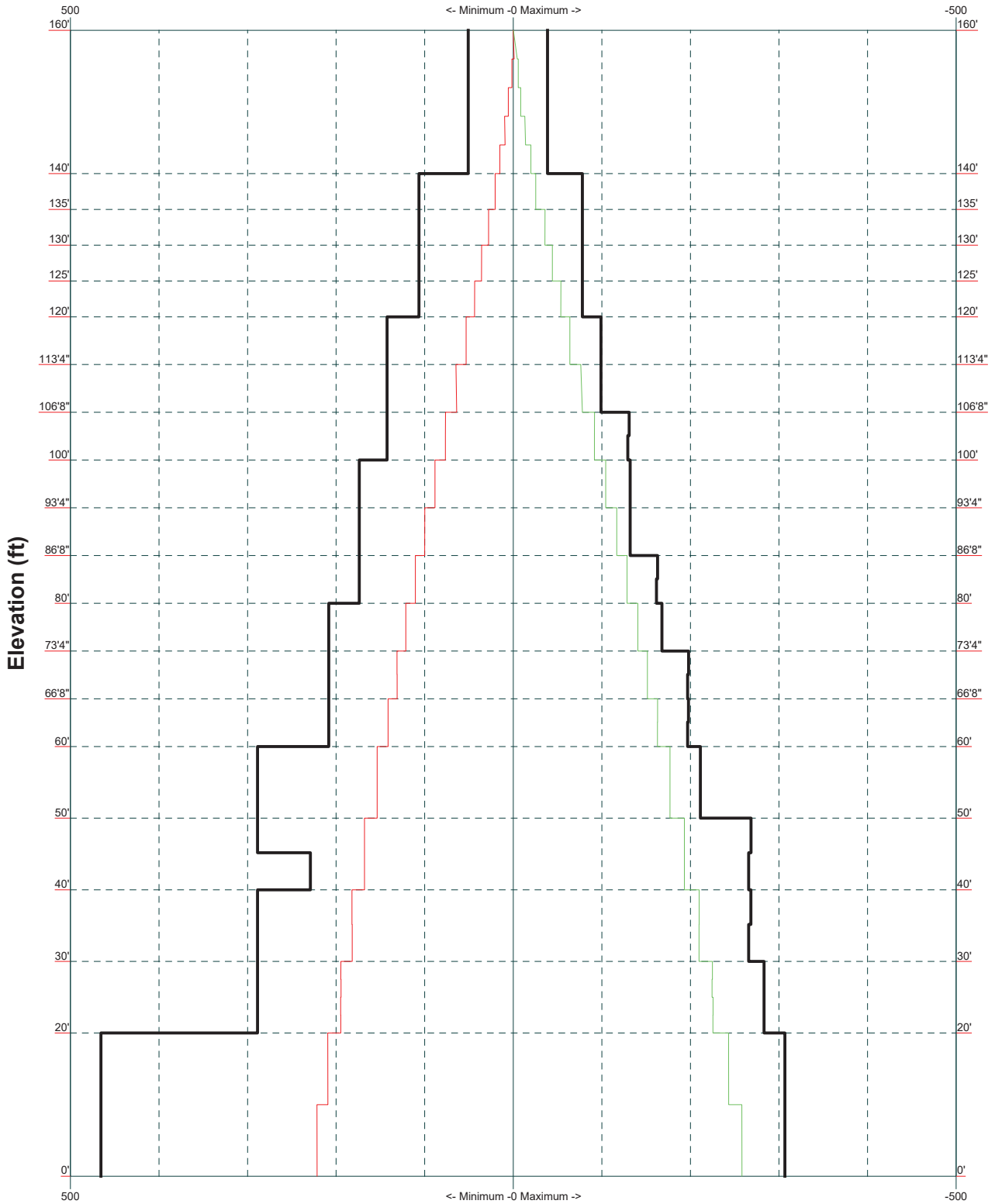
Job: <b>136290.007.01 - HRT 086 943248, CT (BU# 806376)</b>		
Project:		
Client: Crown Castle	Drawn by: Jayaraj B	App'd:
Code: TIA-222-H	Date: 04/05/22	Scale: NTS
Path:		Dwg No. E-2


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# TIA-222-H - 117 mph/50 mph 1.500 in Ice Exposure C

Leg Capacity ———

Leg Compression (K)




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Project:		
Client: Crown Castle	Drawn by: Jayaraj B	App'd:
Code: TIA-222-H	Date: 04/05/22	Scale: NTS
Path:		Dwg No. E-3

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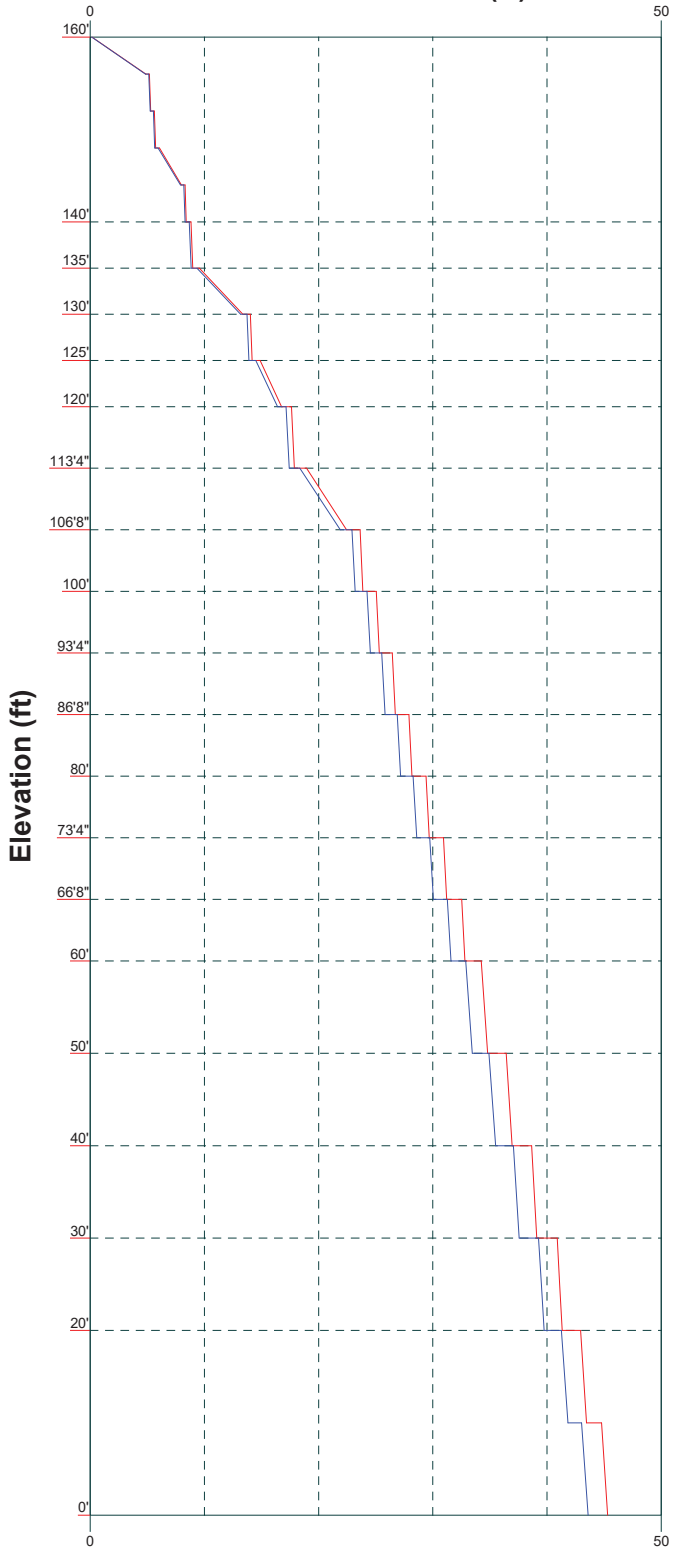
Vx

Vz

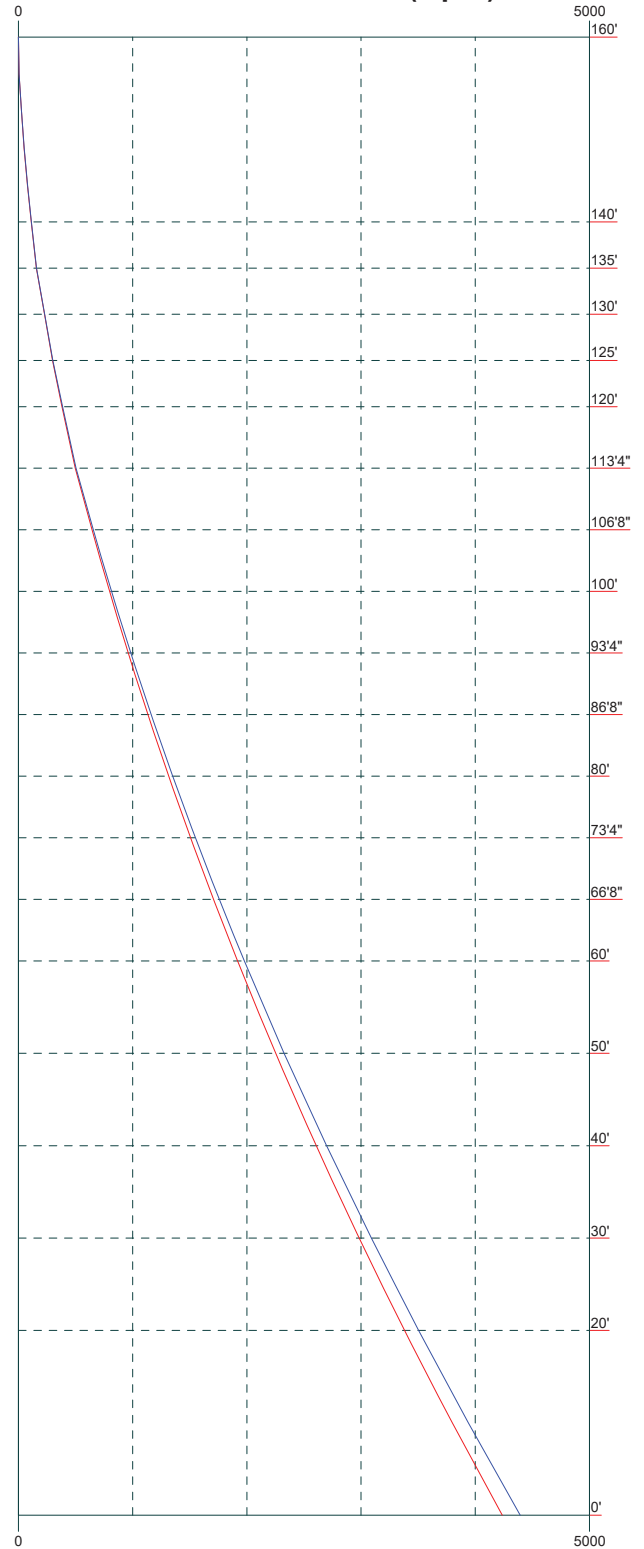
Mx

Mz

Global Mast Shear (K)



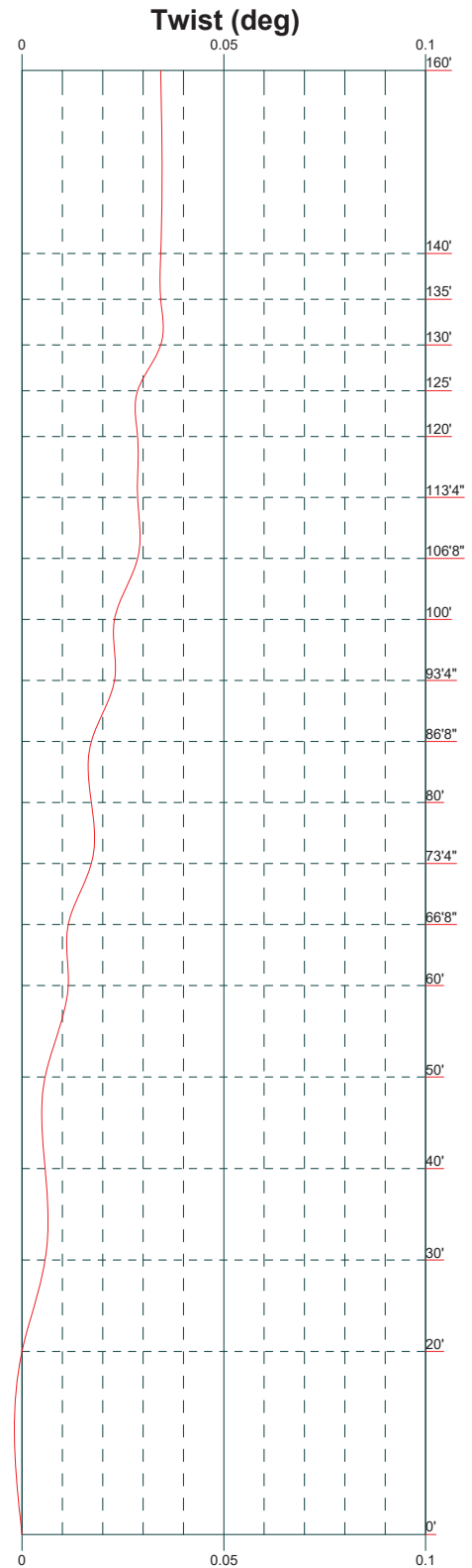
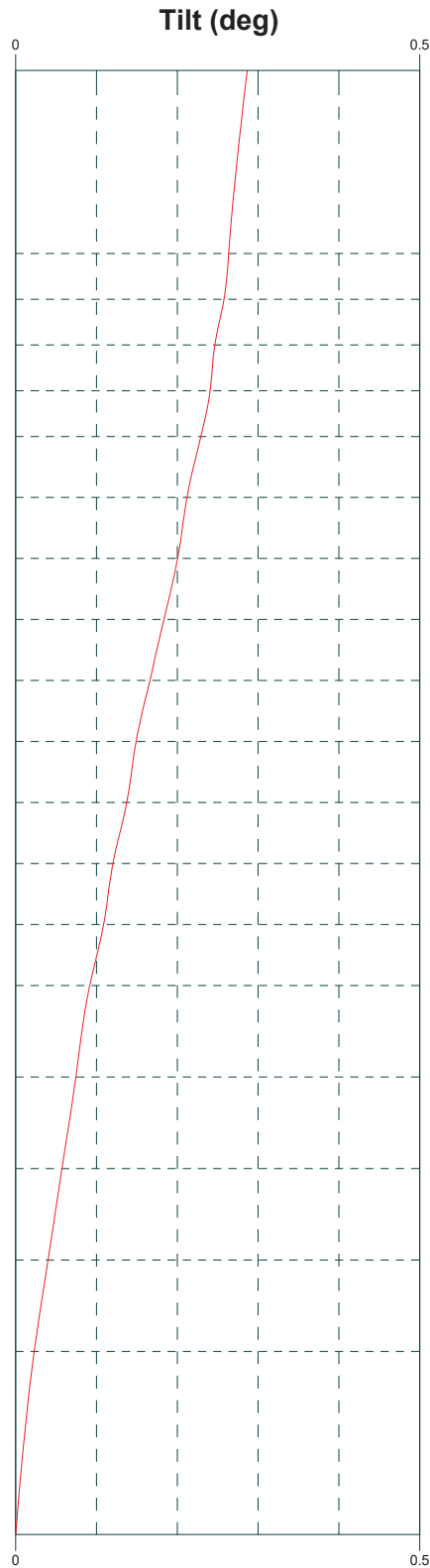
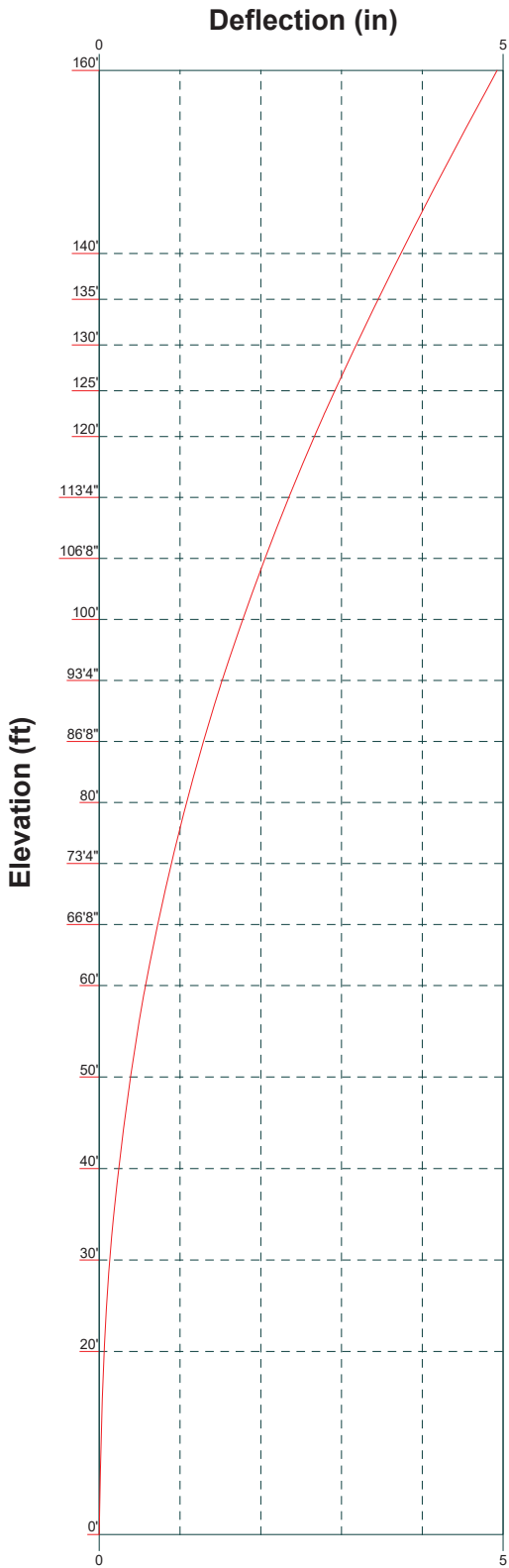
Global Mast Moment (kip-ft)



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 FAX: (918) 295-0265

Job: <b>136290.007.01 - HRT 086 943248, CT (BU# 806376)</b>		
Project:		
Client: Crown Castle	Drawn by: Jayaraj B	App'd:
Code: TIA-222-H	Date: 04/05/22	Scale: NTS
Path:		Dwg No. E-4

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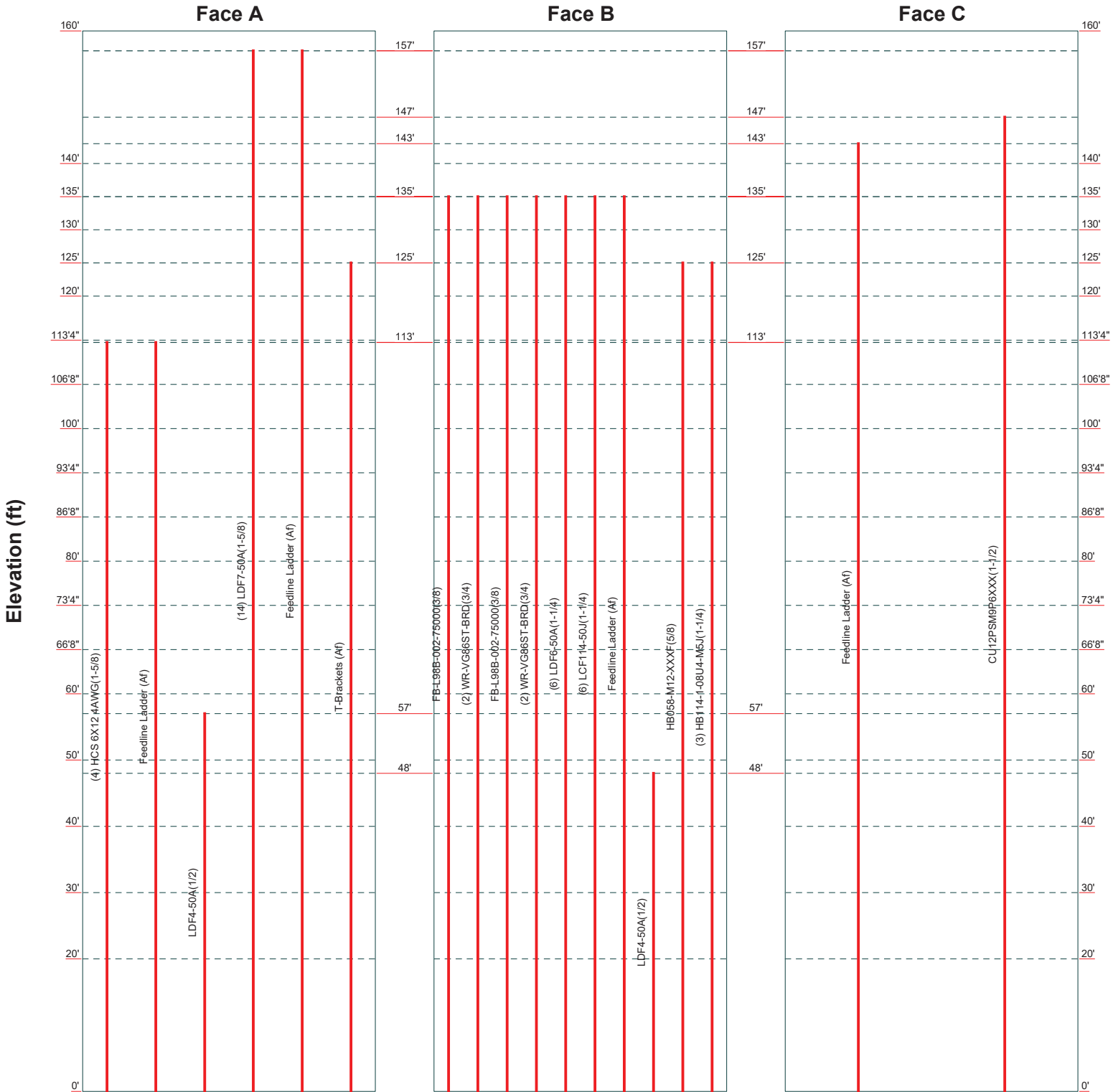
Job: <b>136290.007.01 - HRT 086 943248, CT (BU# 806376)</b>		
Project:		
Client: Crown Castle	Drawn by: Jayaraj B	App'd:
Code: TIA-222-H	Date: 04/05/22	Scale: NTS
Path:		Dwg No. E-5



# Feed Line Distribution Chart

## 0' - 160'

— Round   
 — Flat   
 — App In Face   
 — App Out Face   
 — Truss Leg




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Job: <b>136290.007.01 - HRT 086 943248, CT (BU# 806376)</b>		
Project:		
Client: Crown Castle	Drawn by: Jayaraj B	App'd:
Code: TIA-222-H	Date: 04/05/22	Scale: NTS
Path:		Dwg No. E-7

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<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 1 of 47
	<b>Project</b>	<b>Date</b> 12:01:41 04/05/22
	<b>Client</b> Crown Castle	<b>Designed by</b> Jayaraj B

## Tower Input Data

The main tower is a 3x free standing tower with an overall height of 160' above the ground line.

The base of the tower is set at an elevation of 0' above the ground line.

The face width of the tower is 6'6-1/4" at the top and 20'10-3/8" at the base.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower is located in Tolland County, Connecticut.

Tower base elevation above sea level: 396'.

Basic wind speed of 117 mph.

Risk Category II.

Exposure Category C.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0'.

Nominal ice thickness of 1.500 in.

Ice thickness is considered to increase with height.

Ice density of 56.000 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50.000 °F.

Deflections calculated using a wind speed of 60 mph.

TIA-222-H Annex S.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.

Tower analysis based on target reliabilities in accordance with Annex S.

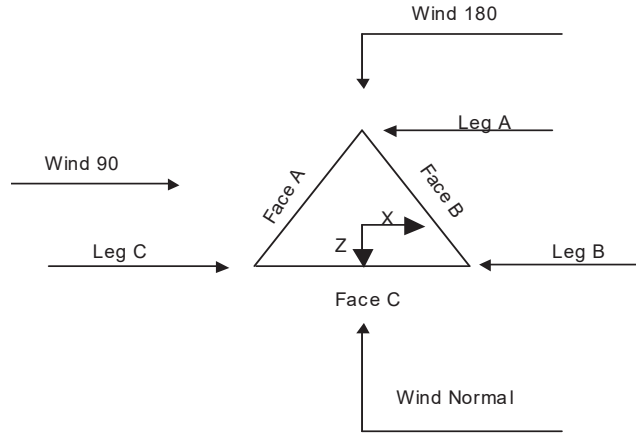
Load Modification Factors used:  $K_{es}(F_w) = 0.95$ ,  $K_{es}(t_i) = 0.85$ .

Maximum demand-capacity ratio is: 1.05.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

<ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>Include Bolts In Member Capacity</li> <li>Leg Bolts Are At Top Of Section</li> <li>√ Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>SR Members Have Cut Ends</li> <li>SR Members Are Concentric</li> </ul>	<ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>√ Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>√ Bypass Mast Stability Checks</li> <li>√ Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>Autocalc Torque Arm Areas</li> <li>Add IBC .6D+W Combination</li> <li>√ Sort Capacity Reports By Component</li> <li>Triangulate Diamond Inner Bracing</li> <li>Treat Feed Line Bundles As Cylinder</li> <li>Ignore KL/ry For 60 Deg. Angle Legs</li> </ul>	<ul style="list-style-type: none"> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>√ Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>√ SR Leg Bolts Resist Compression</li> <li>All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feed Line Torque</li> <li>√ Include Angle Block Shear Check</li> <li>Use TIA-222-H Bracing Resist. Exemption</li> <li>Use TIA-222-H Tension Splice Exemption</li> <li style="background-color: #e0e0e0;">Poles</li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> <li>Pole Without Linear Attachments</li> <li>Pole With Shroud Or No Appurtenances</li> <li>Outside and Inside Corner Radii Are Known</li> </ul>
--	---	--



**Triangular Tower**

**Tower Section Geometry**

<i>Tower Section</i>	<i>Tower Elevation</i>	<i>Assembly Database</i>	<i>Description</i>	<i>Section Width</i>	<i>Number of Sections</i>	<i>Section Length</i>
	<i>ft</i>			<i>ft</i>		<i>ft</i>
T1	160'-140'			6'6-1/4"	1	20'
T2	140'-135'			6'6-3/4"	1	5'
T3	135'-130'			7'7/8"	1	5'
T4	130'-125'			7'7"	1	5'
T5	125'-120'			8'1-1/8"	1	5'
T6	120'-113'4"			8'7-1/4"	1	6'8"
T7	113'4"-106'8"			9'3-3/8"	1	6'8"
T8	106'8"-100'			9'11-1/2"	1	6'8"
T9	100'-93'4"			10'7-5/8"	1	6'8"
T10	93'4"-86'8"			11'3-25/32"	1	6'8"
T11	86'8"-80'			11'11-31/32"	1	6'8"
T12	80'-73'4"			12'8-1/8"	1	6'8"
T13	73'4"-66'8"			13'4-1/2"	1	6'8"
T14	66'8"-60'			14'7/8"	1	6'8"
T15	60'-50'			14'9-1/4"	1	10'
T16	50'-40'			15'9-1/4"	1	10'
T17	40'-30'			16'9-1/4"	1	10'
T18	30'-20'			17'9-3/4"	1	10'
T19	20'-0'			18'10-1/4"	1	20'

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 3 of 47
	<b>Project</b>	<b>Date</b> 12:01:41 04/05/22
	<b>Client</b> Crown Castle	<b>Designed by</b> Jayaraj B

### Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	160'-140'	4'	X Brace	No	No	0.000	0.000
T2	140'-135'	5'	X Brace	No	No	0.000	0.000
T3	135'-130'	5'	X Brace	No	No	0.000	0.000
T4	130'-125'	5'	X Brace	No	No	0.000	0.000
T5	125'-120'	5'	X Brace	No	No	0.000	0.000
T6	120'-113'4"	6'8"	X Brace	No	No	0.000	0.000
T7	113'4"-106'8"	6'8"	X Brace	No	No	0.000	0.000
T8	106'8"-100'	6'8"	X Brace	No	Yes	0.000	0.000
T9	100'-93'4"	6'8"	X Brace	No	No	0.000	0.000
T10	93'4"-86'8"	6'8"	X Brace	No	No	0.000	0.000
T11	86'8"-80'	6'8"	X Brace	No	Yes	0.000	0.000
T12	80'-73'4"	6'8"	X Brace	No	No	0.000	0.000
T13	73'4"-66'8"	6'8"	X Brace	No	Yes	0.000	0.000
T14	66'8"-60'	6'8"	X Brace	No	Yes	0.000	0.000
T15	60'-50'	10'	X Brace	No	No	0.000	0.000
T16	50'-40'	10'	X Brace	No	Yes	0.000	0.000
T17	40'-30'	10'	X Brace	No	Yes	0.000	0.000
T18	30'-20'	5'	Double K1	No	Yes	0.000	0.000
T19	20'-0'	10'	X Brace	No	No	0.000	0.000

### Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
T1 160'-140'	Pipe	ROHN 2 STD	A572-50 (50 ksi)	Single Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T2 140'-135'	Pipe	ROHN 2.5 EH	A572-50 (50 ksi)	Single Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T3 135'-130'	Pipe	ROHN 2.5 EH	A572-50 (50 ksi)	Single Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T4 130'-125'	Pipe	ROHN 2.5 EH	A572-50 (50 ksi)	Single Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T5 125'-120'	Pipe	ROHN 2.5 EH	A572-50 (50 ksi)	Single Angle	L2x2x3/16	A36 (36 ksi)
T6 120'-113'4"	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T7 113'4"-106'8"	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T8 106'8"-100'	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T9 100'-93'4"	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T10 93'4"-86'8"	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T11 86'8"-80'	Pipe	ROHN 3.5 EH	A572-50 (50 ksi)	Double Angle	2L2 1/2x2 1/2x3/16x1/4	A36 (36 ksi)
T12 80'-73'4"	Pipe	ROHN 4 X-STR	A572-50 (50 ksi)	Single Angle	L 3x3x3/16	A36 (36 ksi)
T13 73'4"-66'8"	Pipe	ROHN 4 X-STR	A572-50 (50 ksi)	Single Angle	L 3x3x3/16	A36 (36 ksi)
T14 66'8"-60'	Pipe	ROHN 4 X-STR	A572-50 (50 ksi)	Single Angle	L 3x3x3/16	A36 (36 ksi)
T15 60'-50'	Pipe	ROHN .5 EH	A572-50	Double Angle	2L3x3x3/16x1/4	A36

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Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T16 50'-40'	Pipe	ROHN. 5 EH	(50 ksi) A572-50	Double Angle	2L3x3x3/16x1/4	(36 ksi) A36
T17 40'-30'	Pipe	ROHN 5 X-STR	(50 ksi) A572-50	Double Angle	2L3x3x1/4x1/4	(36 ksi) A572-50
T18 30'-20'	Pipe	ROHN 5 X-STR	(50 ksi) A572-50	Double Angle	2L3x3x1/4x1/4	(50 ksi) A572-50
T19 20'-0'	Arbitrary Shape	B+T BU 806378 - 6.625"x0.34" pipe w/ 2" SR	(50 ksi) A572-50	Double Angle	2L3 1/2x3 1/2x1/4x1/4	(50 ksi) A572-50

### Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 160'-140'	Equal Angle	L2x2x1/8	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T2 140'-135'	Equal Angle	L2x2x1/8	A36 (36 ksi)	Single Angle		A36 (36 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T18 30'-20'	None	Single Angle		A36 (36 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T8 106'8"-100'	Equal Angle	L1 3/4x1 3/4x1/4	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T11 86'8"-80'	Equal Angle	L2x2x3/16	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T13 73'4"-66'8"	Equal Angle	L1 3/4x1 3/4x1/4	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T14 66'8"-60'	Equal Angle	L2x2x3/16	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T16 50'-40'	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T17 40'-30'	Equal Angle	L3x3x1/4	A36 (36 ksi)	Single Angle		A36 (36 ksi)

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### Tower Section Geometry (cont'd)

Tower Elevation	Redundant Bracing Grade	Redundant Type	Redundant Size	K Factor	
<i>ft</i>					
T18 30'-20'	A36 (36 ksi)	Horizontal (1) Diagonal (1)	Equal Angle Equal Angle	L2x2x3/16 L2x2x3/16	1 1

### Tower Section Geometry (cont'd)

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
<i>ft</i>	<i>ft<sup>2</sup></i>	<i>in</i>					<i>in</i>	<i>in</i>	<i>in</i>
T1 160'-140'	0.000	0.188	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T2 140'-135'	0.000	0.188	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T3 135'-130'	0.000	0.188	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T4 130'-125'	0.000	0.188	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T5 125'-120'	0.000	0.188	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T6 120'-113'4"	0.000	0.188	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T7 113'4"-106'8"	0.000	0.188	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T8 106'8"-100'	0.000	0.188	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T9 100'-93'4"	0.000	0.500	A572-50 (50 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T10 93'4"-86'8"	0.000	0.500	A572-50 (50 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T11 86'8"-80'	0.000	0.500	A572-50 (50 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T12 80'-73'4"	0.000	0.500	A572-50 (50 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T13 73'4"-66'8"	0.000	0.500	A572-50 (50 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T14 66'8"-60'	0.000	0.500	A572-50 (50 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T15 60'-50'	0.000	0.250	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T16 50'-40'	0.000	0.250	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T17 40'-30'	0.000	0.250	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T18 30'-20'	0.000	0.250	A36 (36 ksi)	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T19 20'-0'	0.000	0.250	A36 (36 ksi)	1.1	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt

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### Tower Section Geometry (cont'd)

Tower Elevation  ft	Calc K Single Angles	Calc K Solid Rounds	Legs	K Factors <sup>1</sup>							
				X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
				X Y	X Y	X Y	X Y	X Y	X Y	X Y	
T1 160'-140'	Yes	No	1	1	1	1	1	1	1	1	1
T2 140'-135'	Yes	No	1	1	1	1	1	1	1	1	1
T3 135'-130'	Yes	No	1	1	1	1	1	1	1	1	1
T4 130'-125'	Yes	No	1	1	1	1	1	1	1	1	1
T5 125'-120'	Yes	No	1	1	1	1	1	1	1	1	1
T6 120'-113'4"	Yes	No	1	1	1	1	1	1	1	1	1
T7 113'4"-106'8"	Yes	No	1	1	1	1	1	1	1	1	1
T8 106'8"-100'	No	No	1	1	1	1	1	1	1	1	1
T9 100'-93'4"	Yes	No	1	1	1	1	1	1	1	0.5	1
T10 93'4"-86'8"	Yes	No	1	1	1	1	1	1	1	1	1
T11 86'8"-80'	No	No	1	1	1	1	1	1	1	1	1
T12 80'-73'4"	Yes	No	1	1	1	1	1	1	1	0.5	1
T13 73'4"-66'8"	No	No	1	1	1	1	1	1	1	1	1
T14 66'8"-60'	No	No	1	1	1	1	1	1	1	0.5	1
T15 60'-50'	Yes	No	1	1	1	1	1	1	1	1	1
T16 50'-40'	No	No	1	1	1	1	1	1	1	1	1
T17 40'-30'	No	No	1	1	1	1	1	1	1	0.5	1
T18 30'-20'	Yes	No	1	1	1	1	1	1	1	0.5	1
T19 20'-0'	Yes	No	1	1	1	1	1	1	1	1	1

<sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

### Tower Section Geometry (cont'd)







Tower Elevation ft	Redundant Horizontal		Redundant Diagonal		Redundant Sub-Diagonal		Redundant Sub-Horizontal		Redundant Vertical		Redundant Hip		Redundant Hip Diagonal	
	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T3 135'-130'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T4 130'-125'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T5 125'-120'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T6 120'-113'4"	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T7 113'4"-106'8"	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T8 106'8"-100'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T9 100'-93'4"	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T10 93'4"-86'8"	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T11 86'8"-80'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T12 80'-73'4"	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T13 73'4"-66'8"	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T14 66'8"-60'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T15 60'-50'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T16 50'-40'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T17 40'-30'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T18 30'-20'	0.625 A325N	1	0.625 A325N	1	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0
T19 20'-0'	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0	0.625 A325N	0

**Feed Line/Linear Appurtenances - Entered As Round Or Flat**

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight klf
HCS 6X12 4AWG(1-5/8) Feedline Ladder (Af)	A	No	No	Ar (CaAa)	113' - 0'	0.000	-0.35	4	4	0.850 0.750	1.660		0.002
*	A	No	No	Af (CaAa)	113' - 0'	0.000	-0.37	1	1	3.000	3.000		0.008
LDF4-50A(1/2)	A	No	No	Ar (CaAa)	57' - 0'	0.000	0.39	1	1	0.500	0.630		0.000
LDF7-50A(1-5/8) Feedline Ladder (Af)	A	No	No	Ar (CaAa)	157' - 0'	0.000	0.35	14	7	0.850 0.750	1.980		0.001
*	A	No	No	Af (CaAa)	157' - 0'	0.000	0.37	1	1	3.000	3.000		0.008
FB-L98B-002-	B	No	No	Ar (CaAa)	135' - 0'	0.000	-0.4	1	1	0.500	0.394		0.000

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Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight klf
75000(3/8)													
WR-VG86ST-BRD(3/4)	B	No	No	Ar (CaAa)	135' - 0'	0.000	-0.4	2	2	0.850 0.750	0.795		0.001
FB-L98B-002-75000(3/8)	B	No	No	Ar (CaAa)	135' - 0'	0.000	-0.4	1	1	0.500	0.394		0.000
WR-VG86ST-BRD(3/4)	B	No	No	Ar (CaAa)	135' - 0'	0.000	-0.4	2	2	0.850 0.750	0.795		0.001
LDF6-50A(1-1/4)	B	No	No	Ar (CaAa)	135' - 0'	0.000	-0.42	6	3	0.850 0.750	1.550		0.001
LCF114-50J(1-1/4)	B	No	No	Ar (CaAa)	135' - 0'	0.000	-0.37	6	3	0.850 0.750	1.580		0.001
Feedline Ladder (Af)	B	No	No	Af (CaAa)	135' - 0'	0.000	-0.385	1	1	3.000	3.000		0.008
*													
LDF4-50A(1/2)	B	No	No	Ar (CaAa)	48' - 0'	0.000	-0.35	1	1	0.500	0.630		0.000
HB058-M12-XXXF(5/8)	B	No	No	Ar (CaAa)	125' - 0'	0.000	-0.35	1	1	0.500	0.840		0.000
HB114-1-08U4-M5J(1-1/4)	B	No	No	Ar (CaAa)	125' - 0'	0.000	-0.35	3	3	0.850 0.750	1.540		0.001
T-Brackets (Af)	A	No	No	Af (CaAa)	125' - 0'	0.000	-0.35	1	1	1.000	1.000		0.008
*													
Feedline Ladder (Af)	C	No	No	Af (CaAa)	143' - 0'	0.000	0.35	1	1	3.000	3.000		0.008
*													
CU12PSM9P6XXX(1-1/2)	C	No	No	Ar (CaAa)	147' - 0'	0.000	0.37	1	1	0.850 0.750	1.600		0.002
*													
*													
*													

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	C <sub>AA</sub> ft <sup>2</sup> /ft	Weight klf
*								
*								
*								

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>	Weight K
T1	160'-140'	A	0.000	0.000	55.624	0.000	0.338
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	2.620	0.000	0.042

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			Jayaraj B

<i>Tower Section</i>	<i>Tower Elevation ft</i>	<i>Face</i>	<i>A<sub>R</sub></i> <i>ft<sup>2</sup></i>	<i>A<sub>F</sub></i> <i>ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub></i> <i>In Face</i> <i>ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub></i> <i>Out Face</i> <i>ft<sup>2</sup></i>	<i>Weight</i> <i>K</i>
T2	140'-135'	A	0.000	0.000	16.360	0.000	0.099
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	3.300	0.000	0.054
T3	135'-130'	A	0.000	0.000	16.360	0.000	0.099
		B	0.000	0.000	13.874	0.000	0.093
		C	0.000	0.000	3.300	0.000	0.054
T4	130'-125'	A	0.000	0.000	16.360	0.000	0.099
		B	0.000	0.000	13.874	0.000	0.093
		C	0.000	0.000	3.300	0.000	0.054
T5	125'-120'	A	0.000	0.000	17.193	0.000	0.141
		B	0.000	0.000	16.604	0.000	0.111
		C	0.000	0.000	3.300	0.000	0.054
T6	120'-113'4"	A	0.000	0.000	22.924	0.000	0.189
		B	0.000	0.000	22.138	0.000	0.148
		C	0.000	0.000	4.400	0.000	0.072
T7	113'4"-106'8"	A	0.000	0.000	30.296	0.000	0.303
		B	0.000	0.000	22.138	0.000	0.148
		C	0.000	0.000	4.400	0.000	0.072
T8	106'8"-100'	A	0.000	0.000	30.684	0.000	0.309
		B	0.000	0.000	22.138	0.000	0.148
		C	0.000	0.000	4.400	0.000	0.072
T9	100'-93'4"	A	0.000	0.000	30.684	0.000	0.309
		B	0.000	0.000	22.138	0.000	0.148
		C	0.000	0.000	4.400	0.000	0.072
T10	93'4"-86'8"	A	0.000	0.000	30.684	0.000	0.309
		B	0.000	0.000	22.138	0.000	0.148
		C	0.000	0.000	4.400	0.000	0.072
T11	86'8"-80'	A	0.000	0.000	30.684	0.000	0.309
		B	0.000	0.000	22.138	0.000	0.148
		C	0.000	0.000	4.400	0.000	0.072
T12	80'-73'4"	A	0.000	0.000	30.684	0.000	0.309
		B	0.000	0.000	22.138	0.000	0.148
		C	0.000	0.000	4.400	0.000	0.072
T13	73'4"-66'8"	A	0.000	0.000	30.684	0.000	0.309
		B	0.000	0.000	22.138	0.000	0.148
		C	0.000	0.000	4.400	0.000	0.072
T14	66'8"-60'	A	0.000	0.000	30.684	0.000	0.309
		B	0.000	0.000	22.138	0.000	0.148
		C	0.000	0.000	4.400	0.000	0.072
T15	60'-50'	A	0.000	0.000	46.468	0.000	0.464
		B	0.000	0.000	33.207	0.000	0.221
		C	0.000	0.000	6.600	0.000	0.108
T16	50'-40'	A	0.000	0.000	46.657	0.000	0.464
		B	0.000	0.000	33.711	0.000	0.223
		C	0.000	0.000	6.600	0.000	0.108
T17	40'-30'	A	0.000	0.000	46.657	0.000	0.464
		B	0.000	0.000	33.837	0.000	0.223
		C	0.000	0.000	6.600	0.000	0.108
T18	30'-20'	A	0.000	0.000	46.657	0.000	0.464
		B	0.000	0.000	33.837	0.000	0.223
		C	0.000	0.000	6.600	0.000	0.108
T19	20'-0'	A	0.000	0.000	93.313	0.000	0.929
		B	0.000	0.000	67.675	0.000	0.446
		C	0.000	0.000	13.200	0.000	0.215

**Feed Line/Linear Appurtenances Section Areas - With Ice**

<b>Job</b>	136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b>	12 of 47
<b>Project</b>		<b>Date</b>	12:01:41 04/05/22
<b>Client</b>	Crown Castle	<b>Designed by</b>	Jayaraj B

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
T1	160'-140'	A	1.483	0.000	0.000	62.911	0.000	1.304
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	5.587	0.000	0.111
T2	140'-135'	A	1.471	0.000	0.000	18.470	0.000	0.382
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	6.241	0.000	0.132
T3	135'-130'	A	1.465	0.000	0.000	18.456	0.000	0.381
		B		0.000	0.000	29.893	0.000	0.424
		C		0.000	0.000	6.230	0.000	0.131
T4	130'-125'	A	1.460	0.000	0.000	18.441	0.000	0.380
		B		0.000	0.000	29.837	0.000	0.423
		C		0.000	0.000	6.219	0.000	0.131
T5	125'-120'	A	1.454	0.000	0.000	20.713	0.000	0.448
		B		0.000	0.000	38.055	0.000	0.520
		C		0.000	0.000	6.207	0.000	0.130
T6	120'-113'4"	A	1.447	0.000	0.000	27.584	0.000	0.595
		B		0.000	0.000	50.620	0.000	0.690
		C		0.000	0.000	8.258	0.000	0.173
T7	113'4"-106'8"	A	1.438	0.000	0.000	42.737	0.000	0.872
		B		0.000	0.000	50.477	0.000	0.686
		C		0.000	0.000	8.235	0.000	0.172
T8	106'8"-100'	A	1.429	0.000	0.000	43.462	0.000	0.883
		B		0.000	0.000	50.326	0.000	0.682
		C		0.000	0.000	8.211	0.000	0.171
T9	100'-93'4"	A	1.420	0.000	0.000	43.384	0.000	0.880
		B		0.000	0.000	50.166	0.000	0.678
		C		0.000	0.000	8.186	0.000	0.171
T10	93'4"-86'8"	A	1.410	0.000	0.000	43.300	0.000	0.876
		B		0.000	0.000	49.996	0.000	0.674
		C		0.000	0.000	8.159	0.000	0.170
T11	86'8"-80'	A	1.399	0.000	0.000	43.211	0.000	0.872
		B		0.000	0.000	49.814	0.000	0.670
		C		0.000	0.000	8.130	0.000	0.169
T12	80'-73'4"	A	1.387	0.000	0.000	43.115	0.000	0.867
		B		0.000	0.000	49.618	0.000	0.665
		C		0.000	0.000	8.099	0.000	0.167
T13	73'4"-66'8"	A	1.375	0.000	0.000	43.011	0.000	0.863
		B		0.000	0.000	49.407	0.000	0.659
		C		0.000	0.000	8.066	0.000	0.166
T14	66'8"-60'	A	1.361	0.000	0.000	42.898	0.000	0.858
		B		0.000	0.000	49.176	0.000	0.654
		C		0.000	0.000	8.029	0.000	0.165
T15	60'-50'	A	1.342	0.000	0.000	66.431	0.000	1.300
		B		0.000	0.000	73.283	0.000	0.969
		C		0.000	0.000	11.967	0.000	0.245
T16	50'-40'	A	1.315	0.000	0.000	67.042	0.000	1.294
		B		0.000	0.000	75.218	0.000	0.978
		C		0.000	0.000	11.861	0.000	0.241
T17	40'-30'	A	1.283	0.000	0.000	66.572	0.000	1.275
		B		0.000	0.000	74.981	0.000	0.964
		C		0.000	0.000	11.730	0.000	0.237
T18	30'-20'	A	1.240	0.000	0.000	65.963	0.000	1.251
		B		0.000	0.000	73.825	0.000	0.937
		C		0.000	0.000	11.560	0.000	0.231
T19	20'-0'	A	1.132	0.000	0.000	128.811	0.000	2.379
		B		0.000	0.000	141.743	0.000	1.739
		C		0.000	0.000	22.252	0.000	0.434

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 13 of 47
	<b>Project</b>	<b>Date</b> 12:01:41 04/05/22
	<b>Client</b> Crown Castle	<b>Designed by</b> Jayaraj B

### Feed Line Center of Pressure

Section	Elevation	CP <sub>x</sub>	CP <sub>z</sub>	CP <sub>x</sub> Ice	CP <sub>z</sub> Ice
	ft	in	in	in	in
T1	160'-140'	-2.354	-12.448	-2.504	-9.867
T2	140'-135'	-4.165	-10.907	-4.942	-7.847
T3	135'-130'	-2.254	-20.883	-2.214	-20.243
T4	130'-125'	-2.364	-21.974	-2.330	-21.302
T5	125'-120'	-2.438	-23.111	-2.850	-22.858
T6	120'-113'4"	-2.484	-23.683	-3.015	-24.196
T7	113'4"-106'8"	-6.284	-21.170	-7.053	-21.307
T8	106'8"-100'	-6.229	-20.715	-7.097	-20.981
T9	100'-93'4"	-6.997	-22.935	-7.908	-23.201
T10	93'4"-86'8"	-7.281	-23.956	-8.254	-24.275
T11	86'8"-80'	-6.792	-22.941	-7.884	-23.547
T12	80'-73'4"	-7.281	-24.463	-8.574	-25.535
T13	73'4"-66'8"	-6.854	-23.532	-8.183	-24.702
T14	66'8"-60'	-6.942	-23.982	-8.363	-25.359
T15	60'-50'	-8.843	-29.543	-10.165	-30.773
T16	50'-40'	-8.148	-28.535	-9.444	-30.858
T17	40'-30'	-8.221	-29.165	-9.613	-31.984
T18	30'-20'	-7.228	-26.459	-8.678	-29.479
T19	20'-0'	-9.248	-32.712	-10.585	-35.352

### Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T1	7	LDF7-50A(1-5/8)	140.00 - 157.00	0.6000	0.6000
T1	9	Feedline Ladder (Af)	140.00 - 157.00	0.6000	0.6000
T1	24	Feedline Ladder (Af)	140.00 - 143.00	0.6000	0.6000
T1	26	CU12PSM9P6XXX(1-1/2)	140.00 - 147.00	0.6000	0.6000
T2	7	LDF7-50A(1-5/8)	135.00 - 140.00	0.6000	0.6000
T2	9	Feedline Ladder (Af)	135.00 - 140.00	0.6000	0.6000
T2	24	Feedline Ladder (Af)	135.00 - 140.00	0.6000	0.6000
T2	26	CU12PSM9P6XXX(1-1/2)	135.00 - 140.00	0.6000	0.6000
T3	7	LDF7-50A(1-5/8)	130.00 - 135.00	0.6000	0.6000
T3	9	Feedline Ladder (Af)	130.00 - 135.00	0.6000	0.6000
T3	11	FB-L98B-002-75000(3/8)	130.00 - 135.00	0.6000	0.6000
T3	12	WR-VG86ST-BRD(3/4)	130.00 - 135.00	0.6000	0.6000
T3	13	FB-L98B-002-75000(3/8)	130.00 - 135.00	0.6000	0.6000
T3	14	WR-VG86ST-BRD(3/4)	130.00 - 135.00	0.6000	0.6000

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T3	15	LDF6-50A(1-1/4)	130.00 - 135.00	0.6000	0.6000
T3	16	LCF114-50J(1-1/4)	130.00 - 135.00	0.6000	0.6000
T3	17	Feedline Ladder (Af)	130.00 - 135.00	0.6000	0.6000
T3	24	Feedline Ladder (Af)	130.00 - 135.00	0.6000	0.6000
T3	26	CU12PSM9P6XXX(1-1/2)	130.00 - 135.00	0.6000	0.6000
T4	7	LDF7-50A(1-5/8)	125.00 - 130.00	0.6000	0.6000
T4	9	Feedline Ladder (Af)	125.00 - 130.00	0.6000	0.6000
T4	11	FB-L98B-002-75000(3/8)	125.00 - 130.00	0.6000	0.6000
T4	12	WR-VG86ST-BRD(3/4)	125.00 - 130.00	0.6000	0.6000
T4	13	FB-L98B-002-75000(3/8)	125.00 - 130.00	0.6000	0.6000
T4	14	WR-VG86ST-BRD(3/4)	125.00 - 130.00	0.6000	0.6000
T4	15	LDF6-50A(1-1/4)	125.00 - 130.00	0.6000	0.6000
T4	16	LCF114-50J(1-1/4)	125.00 - 130.00	0.6000	0.6000
T4	17	Feedline Ladder (Af)	125.00 - 130.00	0.6000	0.6000
T4	24	Feedline Ladder (Af)	125.00 - 130.00	0.6000	0.6000
T4	26	CU12PSM9P6XXX(1-1/2)	125.00 - 130.00	0.6000	0.6000
T5	7	LDF7-50A(1-5/8)	120.00 - 125.00	0.6000	0.6000
T5	9	Feedline Ladder (Af)	120.00 - 125.00	0.6000	0.6000
T5	11	FB-L98B-002-75000(3/8)	120.00 - 125.00	0.6000	0.6000
T5	12	WR-VG86ST-BRD(3/4)	120.00 - 125.00	0.6000	0.6000
T5	13	FB-L98B-002-75000(3/8)	120.00 - 125.00	0.6000	0.6000
T5	14	WR-VG86ST-BRD(3/4)	120.00 - 125.00	0.6000	0.6000
T5	15	LDF6-50A(1-1/4)	120.00 - 125.00	0.6000	0.6000
T5	16	LCF114-50J(1-1/4)	120.00 - 125.00	0.6000	0.6000
T5	17	Feedline Ladder (Af)	120.00 - 125.00	0.6000	0.6000
T5	20	HB058-M12-XXXF(5/8)	120.00 - 125.00	0.6000	0.6000
T5	21	HB114-1-08U4-M5J(1-1/4)	120.00 - 125.00	0.6000	0.6000
T5	22	T-Brackets (Af)	120.00 - 125.00	0.6000	0.6000
T5	24	Feedline Ladder (Af)	120.00 - 125.00	0.6000	0.6000
T5	26	CU12PSM9P6XXX(1-1/2)	120.00 - 125.00	0.6000	0.6000
T6	7	LDF7-50A(1-5/8)	113.33 - 120.00	0.6000	0.6000

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T6	9	Feedline Ladder (Af)	113.33 - 120.00	0.6000	0.6000
T6	11	FB-L98B-002-75000(3/8)	113.33 - 120.00	0.6000	0.6000
T6	12	WR-VG86ST-BRD(3/4)	113.33 - 120.00	0.6000	0.6000
T6	13	FB-L98B-002-75000(3/8)	113.33 - 120.00	0.6000	0.6000
T6	14	WR-VG86ST-BRD(3/4)	113.33 - 120.00	0.6000	0.6000
T6	15	LDF6-50A(1-1/4)	113.33 - 120.00	0.6000	0.6000
T6	16	LCF114-50J(1-1/4)	113.33 - 120.00	0.6000	0.6000
T6	17	Feedline Ladder (Af)	113.33 - 120.00	0.6000	0.6000
T6	20	HB058-M12-XXXF(5/8)	113.33 - 120.00	0.6000	0.6000
T6	21	HB114-1-08U4-M5J(1-1/4)	113.33 - 120.00	0.6000	0.6000
T6	22	T-Brackets (Af)	113.33 - 120.00	0.6000	0.6000
T6	24	Feedline Ladder (Af)	113.33 - 120.00	0.6000	0.6000
T6	26	CU12PSM9P6XXX(1-1/2)	113.33 - 120.00	0.6000	0.6000
T7	1	HCS 6X12 4AWG(1-5/8)	106.67 - 113.00	0.6000	0.6000
T7	3	Feedline Ladder (Af)	106.67 - 113.00	0.6000	0.6000
T7	7	LDF7-50A(1-5/8)	106.67 - 113.33	0.6000	0.6000
T7	9	Feedline Ladder (Af)	106.67 - 113.33	0.6000	0.6000
T7	11	FB-L98B-002-75000(3/8)	106.67 - 113.33	0.6000	0.6000
T7	12	WR-VG86ST-BRD(3/4)	106.67 - 113.33	0.6000	0.6000
T7	13	FB-L98B-002-75000(3/8)	106.67 - 113.33	0.6000	0.6000
T7	14	WR-VG86ST-BRD(3/4)	106.67 - 113.33	0.6000	0.6000
T7	15	LDF6-50A(1-1/4)	106.67 - 113.33	0.6000	0.6000
T7	16	LCF114-50J(1-1/4)	106.67 - 113.33	0.6000	0.6000
T7	17	Feedline Ladder (Af)	106.67 - 113.33	0.6000	0.6000
T7	20	HB058-M12-XXXF(5/8)	106.67 - 113.33	0.6000	0.6000
T7	21	HB114-1-08U4-M5J(1-1/4)	106.67 - 113.33	0.6000	0.6000
T7	22	T-Brackets (Af)	106.67 - 113.33	0.6000	0.6000
T7	24	Feedline Ladder (Af)	106.67 - 113.33	0.6000	0.6000
T7	26	CU12PSM9P6XXX(1-1/2)	106.67 - 113.33	0.6000	0.6000
T8	1	HCS 6X12 4AWG(1-5/8)	100.00 - 106.67	0.6000	0.6000
T8	3	Feedline Ladder (Af)	100.00 - 106.67	0.6000	0.6000



Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T8	7	LDF7-50A(1-5/8)	100.00 - 106.67	0.6000	0.6000
T8	9	Feedline Ladder (Af)	100.00 - 106.67	0.6000	0.6000
T8	11	FB-L98B-002-75000(3/8)	100.00 - 106.67	0.6000	0.6000
T8	12	WR-VG86ST-BRD(3/4)	100.00 - 106.67	0.6000	0.6000
T8	13	FB-L98B-002-75000(3/8)	100.00 - 106.67	0.6000	0.6000
T8	14	WR-VG86ST-BRD(3/4)	100.00 - 106.67	0.6000	0.6000
T8	15	LDF6-50A(1-1/4)	100.00 - 106.67	0.6000	0.6000
T8	16	LCF114-50J(1-1/4)	100.00 - 106.67	0.6000	0.6000
T8	17	Feedline Ladder (Af)	100.00 - 106.67	0.6000	0.6000
T8	20	HB058-M12-XXXF(5/8)	100.00 - 106.67	0.6000	0.6000
T8	21	HB114-1-08U4-M5J(1-1/4)	100.00 - 106.67	0.6000	0.6000
T8	22	T-Brackets (Af)	100.00 - 106.67	0.6000	0.6000
T8	24	Feedline Ladder (Af)	100.00 - 106.67	0.6000	0.6000
T8	26	CU12PSM9P6XXX(1-1/2)	100.00 - 106.67	0.6000	0.6000
T9	1	HCS 6X12 4AWG(1-5/8)	93.33 - 100.00	0.6000	0.6000
T9	3	Feedline Ladder (Af)	93.33 - 100.00	0.6000	0.6000
T9	7	LDF7-50A(1-5/8)	93.33 - 100.00	0.6000	0.6000
T9	9	Feedline Ladder (Af)	93.33 - 100.00	0.6000	0.6000
T9	11	FB-L98B-002-75000(3/8)	93.33 - 100.00	0.6000	0.6000
T9	12	WR-VG86ST-BRD(3/4)	93.33 - 100.00	0.6000	0.6000
T9	13	FB-L98B-002-75000(3/8)	93.33 - 100.00	0.6000	0.6000
T9	14	WR-VG86ST-BRD(3/4)	93.33 - 100.00	0.6000	0.6000
T9	15	LDF6-50A(1-1/4)	93.33 - 100.00	0.6000	0.6000
T9	16	LCF114-50J(1-1/4)	93.33 - 100.00	0.6000	0.6000
T9	17	Feedline Ladder (Af)	93.33 - 100.00	0.6000	0.6000
T9	20	HB058-M12-XXXF(5/8)	93.33 - 100.00	0.6000	0.6000
T9	21	HB114-1-08U4-M5J(1-1/4)	93.33 - 100.00	0.6000	0.6000
T9	22	T-Brackets (Af)	93.33 - 100.00	0.6000	0.6000
T9	24	Feedline Ladder (Af)	93.33 - 100.00	0.6000	0.6000
T9	26	CU12PSM9P6XXX(1-1/2)	93.33 - 100.00	0.6000	0.6000
T10	1	HCS 6X12 4AWG(1-5/8)	86.67 - 93.33	0.6000	0.6000
T10	3	Feedline Ladder (Af)	86.67 - 93.33	0.6000	0.6000
T10	7	LDF7-50A(1-5/8)	86.67 - 93.33	0.6000	0.6000
T10	9	Feedline Ladder (Af)	86.67 - 93.33	0.6000	0.6000
T10	11	FB-L98B-002-75000(3/8)	86.67 - 93.33	0.6000	0.6000
T10	12	WR-VG86ST-BRD(3/4)	86.67 - 93.33	0.6000	0.6000
T10	13	FB-L98B-002-75000(3/8)	86.67 - 93.33	0.6000	0.6000
T10	14	WR-VG86ST-BRD(3/4)	86.67 - 93.33	0.6000	0.6000
T10	15	LDF6-50A(1-1/4)	86.67 - 93.33	0.6000	0.6000
T10	16	LCF114-50J(1-1/4)	86.67 - 93.33	0.6000	0.6000
T10	17	Feedline Ladder (Af)	86.67 - 93.33	0.6000	0.6000
T10	20	HB058-M12-XXXF(5/8)	86.67 - 93.33	0.6000	0.6000
T10	21	HB114-1-08U4-M5J(1-1/4)	86.67 - 93.33	0.6000	0.6000
T10	22	T-Brackets (Af)	86.67 - 93.33	0.6000	0.6000
T10	24	Feedline Ladder (Af)	86.67 - 93.33	0.6000	0.6000
T10	26	CU12PSM9P6XXX(1-1/2)	86.67 - 93.33	0.6000	0.6000
T11	1	HCS 6X12 4AWG(1-5/8)	80.00 - 86.67	0.6000	0.6000
T11	3	Feedline Ladder (Af)	80.00 - 86.67	0.6000	0.6000

# tnxTower

**B+T Group**  
1717 S. Boulder, Suite 300  
Tulsa, OK 74119  
Phone: (918) 587-4630  
FAX: (918) 295-0265

## Job

136290.007.01 - HRT 086 943248, CT (BU# 806378)

## Page

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## Project

## Date

12:01:41 04/05/22

## Client

Crown Castle

## Designed by

Jayaraj B

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	$K_a$ No Ice	$K_a$ Ice
T11	7	LDF7-50A(1-5/8)	80.00 - 86.67	0.6000	0.6000
T11	9	Feedline Ladder (Af)	80.00 - 86.67	0.6000	0.6000
T11	11	FB-L98B-002-75000(3/8)	80.00 - 86.67	0.6000	0.6000
T11	12	WR-VG86ST-BRD(3/4)	80.00 - 86.67	0.6000	0.6000
T11	13	FB-L98B-002-75000(3/8)	80.00 - 86.67	0.6000	0.6000
T11	14	WR-VG86ST-BRD(3/4)	80.00 - 86.67	0.6000	0.6000
T11	15	LDF6-50A(1-1/4)	80.00 - 86.67	0.6000	0.6000
T11	16	LCF114-50J(1-1/4)	80.00 - 86.67	0.6000	0.6000
T11	17	Feedline Ladder (Af)	80.00 - 86.67	0.6000	0.6000
T11	20	HB058-M12-XXXX(5/8)	80.00 - 86.67	0.6000	0.6000
T11	21	HB114-1-08U4-M5J(1-1/4)	80.00 - 86.67	0.6000	0.6000
T11	22	T-Brackets (Af)	80.00 - 86.67	0.6000	0.6000
T11	24	Feedline Ladder (Af)	80.00 - 86.67	0.6000	0.6000
T11	26	CU12PSM9P6XXX(1-1/2)	80.00 - 86.67	0.6000	0.6000
T12	1	HCS 6X12 4AWG(1-5/8)	73.33 - 80.00	0.6000	0.6000
T12	3	Feedline Ladder (Af)	73.33 - 80.00	0.6000	0.6000
T12	7	LDF7-50A(1-5/8)	73.33 - 80.00	0.6000	0.6000
T12	9	Feedline Ladder (Af)	73.33 - 80.00	0.6000	0.6000
T12	11	FB-L98B-002-75000(3/8)	73.33 - 80.00	0.6000	0.6000
T12	12	WR-VG86ST-BRD(3/4)	73.33 - 80.00	0.6000	0.6000
T12	13	FB-L98B-002-75000(3/8)	73.33 - 80.00	0.6000	0.6000
T12	14	WR-VG86ST-BRD(3/4)	73.33 - 80.00	0.6000	0.6000
T12	15	LDF6-50A(1-1/4)	73.33 - 80.00	0.6000	0.6000
T12	16	LCF114-50J(1-1/4)	73.33 - 80.00	0.6000	0.6000
T12	17	Feedline Ladder (Af)	73.33 - 80.00	0.6000	0.6000
T12	20	HB058-M12-XXXX(5/8)	73.33 - 80.00	0.6000	0.6000
T12	21	HB114-1-08U4-M5J(1-1/4)	73.33 - 80.00	0.6000	0.6000
T12	22	T-Brackets (Af)	73.33 - 80.00	0.6000	0.6000
T12	24	Feedline Ladder (Af)	73.33 - 80.00	0.6000	0.6000
T12	26	CU12PSM9P6XXX(1-1/2)	73.33 - 80.00	0.6000	0.6000
T13	1	HCS 6X12 4AWG(1-5/8)	66.67 - 73.33	0.6000	0.6000
T13	3	Feedline Ladder (Af)	66.67 - 73.33	0.6000	0.6000
T13	7	LDF7-50A(1-5/8)	66.67 - 73.33	0.6000	0.6000
T13	9	Feedline Ladder (Af)	66.67 - 73.33	0.6000	0.6000
T13	11	FB-L98B-002-75000(3/8)	66.67 - 73.33	0.6000	0.6000
T13	12	WR-VG86ST-BRD(3/4)	66.67 - 73.33	0.6000	0.6000
T13	13	FB-L98B-002-75000(3/8)	66.67 - 73.33	0.6000	0.6000
T13	14	WR-VG86ST-BRD(3/4)	66.67 - 73.33	0.6000	0.6000
T13	15	LDF6-50A(1-1/4)	66.67 - 73.33	0.6000	0.6000
T13	16	LCF114-50J(1-1/4)	66.67 - 73.33	0.6000	0.6000
T13	17	Feedline Ladder (Af)	66.67 - 73.33	0.6000	0.6000
T13	20	HB058-M12-XXXX(5/8)	66.67 - 73.33	0.6000	0.6000
T13	21	HB114-1-08U4-M5J(1-1/4)	66.67 - 73.33	0.6000	0.6000
T13	22	T-Brackets (Af)	66.67 - 73.33	0.6000	0.6000
T13	24	Feedline Ladder (Af)	66.67 - 73.33	0.6000	0.6000
T13	26	CU12PSM9P6XXX(1-1/2)	66.67 - 73.33	0.6000	0.6000
T14	1	HCS 6X12 4AWG(1-5/8)	60.00 - 66.67	0.6000	0.6000
T14	3	Feedline Ladder (Af)	60.00 - 66.67	0.6000	0.6000
T14	7	LDF7-50A(1-5/8)	60.00 - 66.67	0.6000	0.6000
T14	9	Feedline Ladder (Af)	60.00 - 66.67	0.6000	0.6000
T14	11	FB-L98B-002-75000(3/8)	60.00 - 66.67	0.6000	0.6000
T14	12	WR-VG86ST-BRD(3/4)	60.00 - 66.67	0.6000	0.6000
T14	13	FB-L98B-002-75000(3/8)	60.00 - 66.67	0.6000	0.6000
T14	14	WR-VG86ST-BRD(3/4)	60.00 - 66.67	0.6000	0.6000
T14	15	LDF6-50A(1-1/4)	60.00 - 66.67	0.6000	0.6000
T14	16	LCF114-50J(1-1/4)	60.00 - 66.67	0.6000	0.6000
T14	17	Feedline Ladder (Af)	60.00 - 66.67	0.6000	0.6000
T14	20	HB058-M12-XXXX(5/8)	60.00 - 66.67	0.6000	0.6000
T14	21	HB114-1-08U4-M5J(1-1/4)	60.00 - 66.67	0.6000	0.6000
T14	22	T-Brackets (Af)	60.00 - 66.67	0.6000	0.6000
T14	24	Feedline Ladder (Af)	60.00 - 66.67	0.6000	0.6000
T14	26	CU12PSM9P6XXX(1-1/2)	60.00 - 66.67	0.6000	0.6000

# tnxTower

**B+T Group**  
1717 S. Boulder, Suite 300  
Tulsa, OK 74119  
Phone: (918) 587-4630  
FAX: (918) 295-0265

## Job

136290.007.01 - HRT 086 943248, CT (BU# 806378)

## Page

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## Project

## Date

12:01:41 04/05/22

## Client

Crown Castle

## Designed by

Jayaraj B

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	$K_a$ No Ice	$K_a$ Ice
T15	1	HCS 6X12 4AWG(1-5/8)	50.00 - 60.00	0.6000	0.6000
T15	3	Feedline Ladder (Af)	50.00 - 60.00	0.6000	0.6000
T15	5	LDF4-50A(1/2)	50.00 - 57.00	0.6000	0.6000
T15	7	LDF7-50A(1-5/8)	50.00 - 60.00	0.6000	0.6000
T15	9	Feedline Ladder (Af)	50.00 - 60.00	0.6000	0.6000
T15	11	FB-L98B-002-75000(3/8)	50.00 - 60.00	0.6000	0.6000
T15	12	WR-VG86ST-BRD(3/4)	50.00 - 60.00	0.6000	0.6000
T15	13	FB-L98B-002-75000(3/8)	50.00 - 60.00	0.6000	0.6000
T15	14	WR-VG86ST-BRD(3/4)	50.00 - 60.00	0.6000	0.6000
T15	15	LDF6-50A(1-1/4)	50.00 - 60.00	0.6000	0.6000
T15	16	LCF114-50J(1-1/4)	50.00 - 60.00	0.6000	0.6000
T15	17	Feedline Ladder (Af)	50.00 - 60.00	0.6000	0.6000
T15	20	HB058-M12-XXXF(5/8)	50.00 - 60.00	0.6000	0.6000
T15	21	HB114-1-08U4-M5J(1-1/4)	50.00 - 60.00	0.6000	0.6000
T15	22	T-Brackets (Af)	50.00 - 60.00	0.6000	0.6000
T15	24	Feedline Ladder (Af)	50.00 - 60.00	0.6000	0.6000
T15	26	CU12PSM9P6XXX(1-1/2)	50.00 - 60.00	0.6000	0.6000
T16	1	HCS 6X12 4AWG(1-5/8)	40.00 - 50.00	0.6000	0.6000
T16	3	Feedline Ladder (Af)	40.00 - 50.00	0.6000	0.6000
T16	5	LDF4-50A(1/2)	40.00 - 50.00	0.6000	0.6000
T16	7	LDF7-50A(1-5/8)	40.00 - 50.00	0.6000	0.6000
T16	9	Feedline Ladder (Af)	40.00 - 50.00	0.6000	0.6000
T16	11	FB-L98B-002-75000(3/8)	40.00 - 50.00	0.6000	0.6000
T16	12	WR-VG86ST-BRD(3/4)	40.00 - 50.00	0.6000	0.6000
T16	13	FB-L98B-002-75000(3/8)	40.00 - 50.00	0.6000	0.6000
T16	14	WR-VG86ST-BRD(3/4)	40.00 - 50.00	0.6000	0.6000
T16	15	LDF6-50A(1-1/4)	40.00 - 50.00	0.6000	0.6000
T16	16	LCF114-50J(1-1/4)	40.00 - 50.00	0.6000	0.6000
T16	17	Feedline Ladder (Af)	40.00 - 50.00	0.6000	0.6000
T16	19	LDF4-50A(1/2)	40.00 - 48.00	0.6000	0.6000
T16	20	HB058-M12-XXXF(5/8)	40.00 - 50.00	0.6000	0.6000
T16	21	HB114-1-08U4-M5J(1-1/4)	40.00 - 50.00	0.6000	0.6000
T16	22	T-Brackets (Af)	40.00 - 50.00	0.6000	0.6000
T16	24	Feedline Ladder (Af)	40.00 - 50.00	0.6000	0.6000
T16	26	CU12PSM9P6XXX(1-1/2)	40.00 - 50.00	0.6000	0.6000
T17	1	HCS 6X12 4AWG(1-5/8)	30.00 - 40.00	0.6000	0.6000
T17	3	Feedline Ladder (Af)	30.00 - 40.00	0.6000	0.6000
T17	5	LDF4-50A(1/2)	30.00 - 40.00	0.6000	0.6000
T17	7	LDF7-50A(1-5/8)	30.00 - 40.00	0.6000	0.6000
T17	9	Feedline Ladder (Af)	30.00 - 40.00	0.6000	0.6000
T17	11	FB-L98B-002-75000(3/8)	30.00 - 40.00	0.6000	0.6000
T17	12	WR-VG86ST-BRD(3/4)	30.00 - 40.00	0.6000	0.6000
T17	13	FB-L98B-002-75000(3/8)	30.00 - 40.00	0.6000	0.6000
T17	14	WR-VG86ST-BRD(3/4)	30.00 - 40.00	0.6000	0.6000
T17	15	LDF6-50A(1-1/4)	30.00 - 40.00	0.6000	0.6000
T17	16	LCF114-50J(1-1/4)	30.00 - 40.00	0.6000	0.6000
T17	17	Feedline Ladder (Af)	30.00 - 40.00	0.6000	0.6000
T17	19	LDF4-50A(1/2)	30.00 - 40.00	0.6000	0.6000
T17	20	HB058-M12-XXXF(5/8)	30.00 - 40.00	0.6000	0.6000
T17	21	HB114-1-08U4-M5J(1-1/4)	30.00 - 40.00	0.6000	0.6000
T17	22	T-Brackets (Af)	30.00 - 40.00	0.6000	0.6000
T17	24	Feedline Ladder (Af)	30.00 - 40.00	0.6000	0.6000
T17	26	CU12PSM9P6XXX(1-1/2)	30.00 - 40.00	0.6000	0.6000
T18	1	HCS 6X12 4AWG(1-5/8)	20.00 - 30.00	0.6000	0.6000
T18	3	Feedline Ladder (Af)	20.00 - 30.00	0.6000	0.6000
T18	5	LDF4-50A(1/2)	20.00 - 30.00	0.6000	0.6000
T18	7	LDF7-50A(1-5/8)	20.00 - 30.00	0.6000	0.6000
T18	9	Feedline Ladder (Af)	20.00 - 30.00	0.6000	0.6000
T18	11	FB-L98B-002-75000(3/8)	20.00 - 30.00	0.6000	0.6000
T18	12	WR-VG86ST-BRD(3/4)	20.00 - 30.00	0.6000	0.6000
T18	13	FB-L98B-002-75000(3/8)	20.00 - 30.00	0.6000	0.6000
T18	14	WR-VG86ST-BRD(3/4)	20.00 - 30.00	0.6000	0.6000

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 19 of 47
	<b>Project</b>	<b>Date</b> 12:01:41 04/05/22
	<b>Client</b> Crown Castle	<b>Designed by</b> Jayaraj B

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	$K_a$ No Ice	$K_a$ Ice
T18	15	LDF6-50A(1-1/4)	20.00 - 30.00	0.6000	0.6000
T18	16	LCF114-50J(1-1/4)	20.00 - 30.00	0.6000	0.6000
T18	17	Feedline Ladder (Af)	20.00 - 30.00	0.6000	0.6000
T18	19	LDF4-50A(1/2)	20.00 - 30.00	0.6000	0.6000
T18	20	HB058-M12-XXXF(5/8)	20.00 - 30.00	0.6000	0.6000
T18	21	HB114-1-08U4-M5J(1-1/4)	20.00 - 30.00	0.6000	0.6000
T18	22	T-Brackets (Af)	20.00 - 30.00	0.6000	0.6000
T18	24	Feedline Ladder (Af)	20.00 - 30.00	0.6000	0.6000
T18	26	CU12PSM9P6XXX(1-1/2)	20.00 - 30.00	0.6000	0.6000
T19	1	HCS 6X12 4AWG(1-5/8)	0.00 - 20.00	0.6000	0.6000
T19	3	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T19	5	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T19	7	LDF7-50A(1-5/8)	0.00 - 20.00	0.6000	0.6000
T19	9	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T19	11	FB-L98B-002-75000(3/8)	0.00 - 20.00	0.6000	0.6000
T19	12	WR-VG86ST-BRD(3/4)	0.00 - 20.00	0.6000	0.6000
T19	13	FB-L98B-002-75000(3/8)	0.00 - 20.00	0.6000	0.6000
T19	14	WR-VG86ST-BRD(3/4)	0.00 - 20.00	0.6000	0.6000
T19	15	LDF6-50A(1-1/4)	0.00 - 20.00	0.6000	0.6000
T19	16	LCF114-50J(1-1/4)	0.00 - 20.00	0.6000	0.6000
T19	17	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T19	19	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T19	20	HB058-M12-XXXF(5/8)	0.00 - 20.00	0.6000	0.6000
T19	21	HB114-1-08U4-M5J(1-1/4)	0.00 - 20.00	0.6000	0.6000
T19	22	T-Brackets (Af)	0.00 - 20.00	0.6000	0.6000
T19	24	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T19	26	CU12PSM9P6XXX(1-1/2)	0.00 - 20.00	0.6000	0.6000

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	$C_{AA}$ Front	$C_{AA}$ Side	Weight
			Horz Lateral	Vert					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
(2) APL866513-42T6 w/ Mount Pipe	A	From Leg	4.000	0.000	157'	No Ice	3.960	4.250	0.034
						1/2" Ice	4.440	4.740	0.074
						1" Ice	4.930	5.250	0.122
						2" Ice	5.980	6.300	0.243
(2) LPA-80063/4CF w/ Mount Pipe	B	From Leg	4.000	0.000	157'	No Ice	6.385	6.603	0.038
						1/2" Ice	6.784	7.232	0.104
						1" Ice	7.192	7.876	0.176
						2" Ice	8.035	9.214	0.344
(2) LPA-80063/4CFX5 w/ Mount Pipe	C	From Leg	4.000	0.000	157'	No Ice	6.385	6.603	0.038
						1/2" Ice	6.784	7.232	0.104
						1" Ice	7.192	7.876	0.176
						2" Ice	8.035	9.214	0.344
(2) NHH-65B-R2B w/ Mount Pipe	A	From Leg	4.000	0.000	157'	No Ice	4.090	3.290	0.069
						1/2" Ice	4.480	3.670	0.132
						1" Ice	4.880	4.060	0.205
						2" Ice	5.700	4.860	0.385
(2) NHH-65B-R2B w/ Mount Pipe	B	From Leg	4.000	0.000	157'	No Ice	4.090	3.290	0.069
						1/2" Ice	4.480	3.670	0.132

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b>		136290.007.01 - HRT 086 943248, CT (BU# 806378)		<b>Page</b>		20 of 47	
	<b>Project</b>				<b>Date</b>		12:01:41 04/05/22	
	<b>Client</b>		Crown Castle		<b>Designed by</b>		Jayaraj B	

Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Lateral	Vert					
					0'		1" Ice	4.880	4.060	0.205
							2" Ice	5.700	4.860	0.385
(2) NHH-65B-R2B w/ Mount Pipe	C	From Leg	4.000	0.000		157'	No Ice	4.090	3.290	0.069
			0'				1/2" Ice	4.480	3.670	0.132
			0'				1" Ice	4.880	4.060	0.205
MT6407-77A w/ Mount Pipe	A	From Leg	4.000	0.000		157'	2" Ice	5.700	4.860	0.385
			0'				No Ice	4.907	2.682	0.096
			0'				1/2" Ice	5.256	3.145	0.136
			0'				1" Ice	5.615	3.624	0.180
MT6407-77A w/ Mount Pipe	B	From Leg	4.000	0.000		157'	2" Ice	6.362	4.631	0.288
			0'				No Ice	4.907	2.682	0.096
			0'				1/2" Ice	5.256	3.145	0.136
			0'				1" Ice	5.615	3.624	0.180
MT6407-77A w/ Mount Pipe	C	From Leg	4.000	0.000		157'	2" Ice	6.362	4.631	0.288
			0'				No Ice	4.907	2.682	0.096
			0'				1/2" Ice	5.256	3.145	0.136
			0'				1" Ice	5.615	3.624	0.180
RF4439D-25A	A	From Leg	4.000	0.000		157'	2" Ice	6.362	4.631	0.288
			0'				No Ice	1.865	1.252	0.075
			1'				1/2" Ice	2.035	1.394	0.093
			1'				1" Ice	2.212	1.544	0.114
RF4439D-25A	B	From Leg	4.000	0.000		157'	2" Ice	2.589	1.866	0.165
			0'				No Ice	1.865	1.252	0.075
			1'				1/2" Ice	2.035	1.394	0.093
			1'				1" Ice	2.212	1.544	0.114
RF4439D-25A	C	From Leg	4.000	0.000		157'	2" Ice	2.589	1.866	0.165
			0'				No Ice	1.865	1.252	0.075
			1'				1/2" Ice	2.035	1.394	0.093
			1'				1" Ice	2.212	1.544	0.114
RF4440D-13A	A	From Leg	4.000	0.000		157'	2" Ice	2.589	1.866	0.165
			0'				No Ice	1.865	1.129	0.073
			1'				1/2" Ice	2.035	1.267	0.090
			1'				1" Ice	2.212	1.411	0.110
RF4440D-13A	B	From Leg	4.000	0.000		157'	2" Ice	2.589	1.723	0.159
			0'				No Ice	1.865	1.129	0.073
			1'				1/2" Ice	2.035	1.267	0.090
			1'				1" Ice	2.212	1.411	0.110
RF4440D-13A	C	From Leg	4.000	0.000		157'	2" Ice	2.589	1.723	0.159
			0'				No Ice	1.865	1.129	0.073
			1'				1/2" Ice	2.035	1.267	0.090
			1'				1" Ice	2.212	1.411	0.110
RVZDC-6627-PF-48_CCIV2	C	From Leg	4.000	0.000		157'	2" Ice	2.589	1.723	0.159
			0'				No Ice	4.056	3.098	0.032
			1'				1/2" Ice	4.316	3.335	0.068
			1'				1" Ice	4.582	3.580	0.109
15' x 2" Pipe Mount	A	From Leg	4.000	0.000		157'	2" Ice	5.138	4.092	0.203
			0'				No Ice	3.563	3.563	0.055
			0'				1/2" Ice	5.091	5.091	0.081
			0'				1" Ice	6.635	6.635	0.118
15' x 2" Pipe Mount	B	From Leg	4.000	0.000		157'	2" Ice	9.775	9.775	0.219
			0'				No Ice	3.563	3.563	0.055
			0'				1/2" Ice	5.091	5.091	0.081
			0'				1" Ice	6.635	6.635	0.118
15' x 2" Pipe Mount	C	From Leg	4.000	0.000		157'	2" Ice	9.775	9.775	0.219
			0'				No Ice	3.563	3.563	0.055
			0'				1/2" Ice	5.091	5.091	0.081
			0'				1" Ice	6.635	6.635	0.118

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b>	<b>Page</b>	
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<b>Client</b>	Crown Castle	<b>Designed by</b> Jayaraj B	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Lateral						Vert
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
Mount Reinforcement Specifications	C	None			0.000	157'	2" Ice	9.775	9.775	0.219
							No Ice	28.630	28.630	0.280
							1/2" Ice	37.310	37.310	0.670
							1" Ice	45.800	45.800	0.940
Side Arm Mount [SO 102-3]	C	None			0.000	157'	2" Ice	62.380	62.380	1.630
							No Ice	3.600	3.600	0.075
							1/2" Ice	4.180	4.180	0.105
							1" Ice	4.750	4.750	0.135
Sector Mount [SM 505-3]	C	None			0.000	157'	2" Ice	5.900	5.900	0.195
							No Ice	31.660	31.660	1.725
							1/2" Ice	44.640	44.640	2.356
							1" Ice	57.440	57.440	3.189
							2" Ice	82.680	82.680	5.447
* MX08FRO665-21 w/ Mount Pipe	A	From Leg	4.000	0.000	0.000	147'	No Ice	8.010	4.230	0.108
							1/2" Ice	8.520	4.690	0.194
							1" Ice	9.040	5.160	0.292
							2" Ice	10.110	6.120	0.522
MX08FRO665-21 w/ Mount Pipe	B	From Leg	4.000	0.000	0.000	147'	No Ice	8.010	4.230	0.108
							1/2" Ice	8.520	4.690	0.194
							1" Ice	9.040	5.160	0.292
							2" Ice	10.110	6.120	0.522
MX08FRO665-21 w/ Mount Pipe	C	From Leg	4.000	0.000	0.000	147'	No Ice	8.010	4.230	0.108
							1/2" Ice	8.520	4.690	0.194
							1" Ice	9.040	5.160	0.292
							2" Ice	10.110	6.120	0.522
TA08025-B604	A	From Leg	4.000	0.000	0.000	147'	No Ice	1.964	0.981	0.064
							1/2" Ice	2.138	1.112	0.081
							1" Ice	2.320	1.250	0.100
							2" Ice	2.705	1.548	0.148
TA08025-B604	B	From Leg	4.000	0.000	0.000	147'	No Ice	1.964	0.981	0.064
							1/2" Ice	2.138	1.112	0.081
							1" Ice	2.320	1.250	0.100
							2" Ice	2.705	1.548	0.148
TA08025-B604	C	From Leg	4.000	0.000	0.000	147'	No Ice	1.964	0.981	0.064
							1/2" Ice	2.138	1.112	0.081
							1" Ice	2.320	1.250	0.100
							2" Ice	2.705	1.548	0.148
TA08025-B605	A	From Leg	4.000	0.000	0.000	147'	No Ice	1.964	1.129	0.075
							1/2" Ice	2.138	1.267	0.093
							1" Ice	2.320	1.411	0.114
							2" Ice	2.705	1.723	0.164
TA08025-B605	B	From Leg	4.000	0.000	0.000	147'	No Ice	1.964	1.129	0.075
							1/2" Ice	2.138	1.267	0.093
							1" Ice	2.320	1.411	0.114
							2" Ice	2.705	1.723	0.164
TA08025-B605	C	From Leg	4.000	0.000	0.000	147'	No Ice	1.964	1.129	0.075
							1/2" Ice	2.138	1.267	0.093
							1" Ice	2.320	1.411	0.114
							2" Ice	2.705	1.723	0.164
RDIDC-9181-PF-48	A	From Leg	4.000	0.000	0.000	147'	No Ice	2.012	1.168	0.022
							1/2" Ice	2.189	1.311	0.040
							1" Ice	2.373	1.461	0.060
							2" Ice	2.763	1.784	0.110
(2) 8' x 2" Mount Pipe	A	From Leg	4.000	0.000	0.000	147'	No Ice	1.900	1.900	0.029
							1/2" Ice	2.728	2.728	0.044
							1" Ice	3.401	3.401	0.063

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b>		136290.007.01 - HRT 086 943248, CT (BU# 806378)		<b>Page</b>		22 of 47	
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	<b>Client</b>		Crown Castle		<b>Designed by</b>		Jayaraj B	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Lateral						Vert
(2) 8' x 2" Mount Pipe	B	From Leg	4.000	0'	0.000	147'	2" Ice	4.396	4.396	0.119
							No Ice	1.900	1.900	0.029
							1/2" Ice	2.728	2.728	0.044
							1" Ice	3.401	3.401	0.063
(2) 8' x 2" Mount Pipe	C	From Leg	4.000	0'	0.000	147'	2" Ice	4.396	4.396	0.119
							No Ice	1.900	1.900	0.029
							1/2" Ice	2.728	2.728	0.044
							1" Ice	3.401	3.401	0.063
Commscope MTC3975083 (3)	C	None	4.000	0'	0.000	147'	2" Ice	4.396	4.396	0.119
							No Ice	23.850	23.850	1.260
							1/2" Ice	34.120	34.120	1.803
							1" Ice	44.390	44.390	2.345
* 7770.00 w/ Mount Pipe	A	From Leg	4.000	0'	0.000	135'	2" Ice	64.930	64.930	3.431
							No Ice	5.746	4.254	0.055
							1/2" Ice	6.179	5.014	0.103
							1" Ice	6.607	5.711	0.157
7770.00 w/ Mount Pipe	B	From Leg	4.000	0'	0.000	135'	2" Ice	7.488	7.155	0.287
							No Ice	5.746	4.254	0.055
							1/2" Ice	6.179	5.014	0.103
							1" Ice	6.607	5.711	0.157
7770.00 w/ Mount Pipe	C	From Leg	4.000	0'	0.000	135'	2" Ice	7.488	7.155	0.287
							No Ice	5.746	4.254	0.055
							1/2" Ice	6.179	5.014	0.103
							1" Ice	6.607	5.711	0.157
HPA-65R-BUU-H8 w/ Mount Pipe	A	From Leg	4.000	0'	0.000	135'	2" Ice	7.488	7.155	0.287
							No Ice	12.250	8.330	0.105
							1/2" Ice	13.190	9.230	0.194
							1" Ice	14.160	10.150	0.297
HPA-65R-BUU-H8 w/ Mount Pipe	B	From Leg	4.000	0'	0.000	135'	2" Ice	16.140	12.050	0.543
							No Ice	12.250	8.330	0.105
							1/2" Ice	13.190	9.230	0.194
							1" Ice	14.160	10.150	0.297
HPA-65R-BUU-H8 w/ Mount Pipe	C	From Leg	4.000	0'	0.000	135'	2" Ice	16.140	12.050	0.543
							No Ice	12.250	8.330	0.105
							1/2" Ice	13.190	9.230	0.194
							1" Ice	14.160	10.150	0.297
TPA-65R-LCUUUU-H8 w/ Mount Pipe	A	From Leg	4.000	0'	0.000	135'	2" Ice	16.140	12.050	0.543
							No Ice	11.850	8.990	0.115
							1/2" Ice	12.770	9.880	0.210
							1" Ice	13.710	10.790	0.319
TPA-65R-LCUUUU-H8 w/ Mount Pipe	B	From Leg	4.000	0'	0.000	135'	2" Ice	15.640	12.660	0.580
							No Ice	11.850	8.990	0.115
							1/2" Ice	12.770	9.880	0.210
							1" Ice	13.710	10.790	0.319
TPA-65R-LCUUUU-H8 w/ Mount Pipe	C	From Leg	4.000	0'	0.000	135'	2" Ice	15.640	12.660	0.580
							No Ice	11.850	8.990	0.115
							1/2" Ice	12.770	9.880	0.210
							1" Ice	13.710	10.790	0.319
TT19-08BP111-001	A	From Leg	4.000	0'	0.000	135'	2" Ice	15.640	12.660	0.580
							No Ice	0.545	0.442	0.016
							1/2" Ice	0.641	0.530	0.022
							1" Ice	0.743	0.626	0.029
TT19-08BP111-001	B	From Leg	4.000	0'	0.000	135'	2" Ice	0.971	0.840	0.049
							No Ice	0.545	0.442	0.016
							1/2" Ice	0.641	0.530	0.022
							1" Ice	0.743	0.626	0.029

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b>	<b>Page</b>	
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<b>Client</b>	Crown Castle	<b>Designed by</b> Jayaraj B	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Vert					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
TT19-08BP111-001	C	From Leg	4.000	0.000	135'	2" Ice	0.971	0.840	0.049
			0'			No Ice	0.545	0.442	0.016
			2'			1/2" Ice	0.641	0.530	0.022
						1" Ice	0.743	0.626	0.029
(2) 7020.00	A	From Leg	4.000	0.000	135'	2" Ice	0.971	0.840	0.049
			0'			No Ice	0.102	0.175	0.002
			2'			1/2" Ice	0.147	0.239	0.005
						1" Ice	0.199	0.311	0.009
(2) 7020.00	B	From Leg	4.000	0.000	135'	2" Ice	0.326	0.476	0.022
			0'			No Ice	0.102	0.175	0.002
			2'			1/2" Ice	0.147	0.239	0.005
						1" Ice	0.199	0.311	0.009
(2) 7020.00	C	From Leg	4.000	0.000	135'	2" Ice	0.326	0.476	0.022
			0'			No Ice	0.102	0.175	0.002
			2'			1/2" Ice	0.147	0.239	0.005
						1" Ice	0.199	0.311	0.009
DC6-48-60-18-8F	A	From Leg	4.000	0.000	135'	2" Ice	0.326	0.476	0.022
			0'			No Ice	0.850	0.850	0.019
			2'			1/2" Ice	1.356	1.356	0.036
						1" Ice	1.532	1.532	0.055
DC6-48-60-18-8F	C	From Leg	4.000	0.000	135'	2" Ice	1.914	1.914	0.101
			0'			No Ice	0.850	0.850	0.019
			2'			1/2" Ice	1.356	1.356	0.036
						1" Ice	1.532	1.532	0.055
RRUS 11	A	From Leg	4.000	0.000	135'	2" Ice	1.914	1.914	0.101
			0'			No Ice	2.784	1.187	0.048
			2'			1/2" Ice	2.992	1.334	0.068
						1" Ice	3.207	1.490	0.092
RRUS 11	B	From Leg	4.000	0.000	135'	2" Ice	3.658	1.833	0.150
			0'			No Ice	2.784	1.187	0.048
			2'			1/2" Ice	2.992	1.334	0.068
						1" Ice	3.207	1.490	0.092
RRUS 11	C	From Leg	4.000	0.000	135'	2" Ice	3.658	1.833	0.150
			0'			No Ice	2.784	1.187	0.048
			2'			1/2" Ice	2.992	1.334	0.068
						1" Ice	3.207	1.490	0.092
RRUS 4415 B25	A	From Leg	4.000	0.000	135'	2" Ice	3.658	1.833	0.150
			0'			No Ice	1.644	0.679	0.044
			2'			1/2" Ice	1.804	0.791	0.056
						1" Ice	1.972	0.913	0.071
RRUS 4415 B25	B	From Leg	4.000	0.000	135'	2" Ice	2.329	1.183	0.109
			0'			No Ice	1.644	0.679	0.044
			2'			1/2" Ice	1.804	0.791	0.056
						1" Ice	1.972	0.913	0.071
RRUS 4415 B25	C	From Leg	4.000	0.000	135'	2" Ice	2.329	1.183	0.109
			0'			No Ice	1.644	0.679	0.044
			2'			1/2" Ice	1.804	0.791	0.056
						1" Ice	1.972	0.913	0.071
DBC0062F1V51-1	A	From Leg	4.000	0.000	135'	2" Ice	2.329	1.183	0.109
			0'			No Ice	0.220	0.711	0.007
			2'			1/2" Ice	0.289	0.818	0.012
						1" Ice	0.366	0.932	0.018
DBC0062F1V51-1	B	From Leg	4.000	0.000	135'	2" Ice	0.543	1.182	0.037
			0'			No Ice	0.220	0.711	0.007
			2'			1/2" Ice	0.289	0.818	0.012
						1" Ice	0.366	0.932	0.018
					2" Ice	0.543	1.182	0.037	



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<b>Client</b>	Crown Castle	<b>Designed by</b> Jayaraj B	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight
			Horz	Lateral					
DBC0062F1V51-1	C	From Leg	4.000	0.000	135'	No Ice	0.220	0.711	0.007
			0'			1/2" Ice	0.289	0.818	0.012
			2'			1" Ice	0.366	0.932	0.018
						2" Ice	0.543	1.182	0.037
						No Ice	1.644	0.725	0.048
RRUS 4426 B66	A	From Leg	4.000	0.000	135'	No Ice	1.644	0.725	0.048
			0'			1/2" Ice	1.804	0.842	0.061
			2'			1" Ice	1.972	0.969	0.076
						2" Ice	2.329	1.244	0.115
						No Ice	1.644	0.725	0.048
RRUS 4426 B66	B	From Leg	4.000	0.000	135'	No Ice	1.644	0.725	0.048
			0'			1/2" Ice	1.804	0.842	0.061
			2'			1" Ice	1.972	0.969	0.076
						2" Ice	2.329	1.244	0.115
						No Ice	1.644	0.725	0.048
RRUS 4426 B66	C	From Leg	4.000	0.000	135'	No Ice	1.644	0.725	0.048
			0'			1/2" Ice	1.804	0.842	0.061
			2'			1" Ice	1.972	0.969	0.076
						2" Ice	2.329	1.244	0.115
						No Ice	1.644	0.725	0.048
WCS RRUS-32-B30	A	From Leg	4.000	0.000	135'	No Ice	3.314	2.424	0.077
			0'			1/2" Ice	3.558	2.638	0.105
			2'			1" Ice	3.809	2.860	0.136
						2" Ice	4.333	3.324	0.211
						No Ice	3.314	2.424	0.077
WCS RRUS-32-B30	B	From Leg	4.000	0.000	135'	No Ice	3.314	2.424	0.077
			0'			1/2" Ice	3.558	2.638	0.105
			2'			1" Ice	3.809	2.860	0.136
						2" Ice	4.333	3.324	0.211
						No Ice	3.314	2.424	0.077
WCS RRUS-32-B30	C	From Leg	4.000	0.000	135'	No Ice	3.314	2.424	0.077
			0'			1/2" Ice	3.558	2.638	0.105
			2'			1" Ice	3.809	2.860	0.136
						2" Ice	4.333	3.324	0.211
						No Ice	3.314	2.424	0.077
Sector Mount [SM 504-3]	C	None		0.000	135'	No Ice	31.050	31.050	1.708
						1/2" Ice	43.830	43.830	2.326
						1" Ice	56.440	56.440	3.143
						2" Ice	81.280	81.280	5.358
						No Ice	4.600	4.010	0.095
* APXVSPP18-C-A20 w/ Mount Pipe	A	From Leg	4.000	0.000	125'	No Ice	4.600	4.010	0.095
			0'			1/2" Ice	5.050	4.450	0.160
			1'			1" Ice	5.500	4.890	0.235
						2" Ice	6.440	5.820	0.419
						No Ice	4.600	4.010	0.095
APXV9ERR18-C-A20 w/ Mount Pipe	B	From Leg	4.000	0.000	125'	No Ice	4.600	4.010	0.095
			0'			1/2" Ice	5.050	4.450	0.160
			1'			1" Ice	5.500	4.890	0.235
						2" Ice	6.440	5.820	0.419
						No Ice	4.600	4.010	0.095
APXVSPP18-C-A20 w/ Mount Pipe	C	From Leg	4.000	0.000	125'	No Ice	4.600	4.010	0.095
			0'			1/2" Ice	5.050	4.450	0.160
			1'			1" Ice	5.500	4.890	0.235
						2" Ice	6.440	5.820	0.419
						No Ice	4.600	4.010	0.095
APXVTM14-C-120 w/ Mount Pipe	A	From Leg	4.000	0.000	125'	No Ice	4.090	2.860	0.077
			0'			1/2" Ice	4.480	3.230	0.127
			1'			1" Ice	4.880	3.610	0.185
						2" Ice	5.710	4.400	0.331
						No Ice	4.090	2.860	0.077
APXVTM14-C-120 w/ Mount Pipe	B	From Leg	4.000	0.000	125'	No Ice	4.090	2.860	0.077
			0'			1/2" Ice	4.480	3.230	0.127
			1'			1" Ice	4.880	3.610	0.185
						2" Ice	5.710	4.400	0.331
						No Ice	4.090	2.860	0.077
APXVTM14-C-120 w/ Mount Pipe	C	From Leg	4.000	0.000	125'	No Ice	4.090	2.860	0.077
			0'			1/2" Ice	4.480	3.230	0.127
			1'			1" Ice	4.880	3.610	0.185
						2" Ice	5.710	4.400	0.331
						No Ice	4.090	2.860	0.077

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	<b>Project</b>				<b>Date</b>		12:01:41 04/05/22	
	<b>Client</b>		Crown Castle		<b>Designed by</b>		Jayaraj B	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight
			Horz	Vert			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
TD-RRH8x20-25	A	From Leg	4.000	0.000	125'	No Ice	4.045	1.535	0.070
			0'			1/2" Ice	4.298	1.714	0.097
			1'			1" Ice	4.557	1.901	0.128
						2" Ice	5.098	2.295	0.201
						No Ice	4.045	1.535	0.070
TD-RRH8x20-25	B	From Leg	4.000	0.000	125'	No Ice	4.045	1.535	0.070
			0'			1/2" Ice	4.298	1.714	0.097
			1'			1" Ice	4.557	1.901	0.128
						2" Ice	5.098	2.295	0.201
						No Ice	4.045	1.535	0.070
TD-RRH8x20-25	C	From Leg	4.000	0.000	125'	No Ice	4.045	1.535	0.070
			0'			1/2" Ice	4.298	1.714	0.097
			1'			1" Ice	4.557	1.901	0.128
						2" Ice	5.098	2.295	0.201
						No Ice	4.045	1.535	0.070
800MHz 2X50W RRH W/FILTER	A	From Leg	4.000	0.000	125'	No Ice	2.058	1.932	0.064
			0'			1/2" Ice	2.240	2.109	0.086
			0'			1" Ice	2.429	2.293	0.111
						2" Ice	2.829	2.684	0.172
						No Ice	2.058	1.932	0.064
800MHz 2X50W RRH W/FILTER	B	From Leg	4.000	0.000	125'	No Ice	2.058	1.932	0.064
			0'			1/2" Ice	2.240	2.109	0.086
			0'			1" Ice	2.429	2.293	0.111
						2" Ice	2.829	2.684	0.172
						No Ice	2.058	1.932	0.064
800MHz 2X50W RRH W/FILTER	C	From Leg	4.000	0.000	125'	No Ice	2.058	1.932	0.064
			0'			1/2" Ice	2.240	2.109	0.086
			0'			1" Ice	2.429	2.293	0.111
						2" Ice	2.829	2.684	0.172
						No Ice	2.058	1.932	0.064
1900MHz RRH (65MHz)	A	From Leg	4.000	0.000	125'	No Ice	2.313	2.375	0.060
			0'			1/2" Ice	2.517	2.581	0.084
			0'			1" Ice	2.728	2.794	0.111
						2" Ice	3.174	3.243	0.176
						No Ice	2.313	2.375	0.060
1900MHz RRH (65MHz)	B	From Leg	4.000	0.000	125'	No Ice	2.313	2.375	0.060
			0'			1/2" Ice	2.517	2.581	0.084
			0'			1" Ice	2.728	2.794	0.111
						2" Ice	3.174	3.243	0.176
						No Ice	2.313	2.375	0.060
1900MHz RRH (65MHz)	C	From Leg	4.000	0.000	125'	No Ice	2.313	2.375	0.060
			0'			1/2" Ice	2.517	2.581	0.084
			0'			1" Ice	2.728	2.794	0.111
						2" Ice	3.174	3.243	0.176
						No Ice	2.313	2.375	0.060
Sector Mount [SM 402-3]	C	None		0.000	125'	No Ice	18.870	18.870	0.851
						1/2" Ice	26.470	26.470	1.210
						1" Ice	33.990	33.990	1.696
						2" Ice	48.840	48.840	3.044
						No Ice	18.870	18.870	0.851
* VV-65A-R1_TMO w/ Mount Pipe	A	From Leg	4.000	0.000	113'	No Ice	4.460	2.690	0.054
			0'			1/2" Ice	4.910	3.100	0.097
			1'			1" Ice	5.360	3.520	0.149
						2" Ice	6.320	4.410	0.281
						No Ice	4.460	2.690	0.054
VV-65A-R1_TMO w/ Mount Pipe	B	From Leg	4.000	0.000	113'	No Ice	4.460	2.690	0.054
			0'			1/2" Ice	4.910	3.100	0.097
			1'			1" Ice	5.360	3.520	0.149
						2" Ice	6.320	4.410	0.281
						No Ice	4.460	2.690	0.054
VV-65A-R1_TMO w/ Mount Pipe	C	From Leg	4.000	0.000	113'	No Ice	4.460	2.690	0.054
			0'			1/2" Ice	4.910	3.100	0.097
			1'			1" Ice	5.360	3.520	0.149
						2" Ice	6.320	4.410	0.281
						No Ice	4.460	2.690	0.054
AIR 6419 B41_TMO w/ Mount Pipe	A	From Leg	4.000	0.000	113'	No Ice	6.580	3.500	0.111
			0'			1/2" Ice	7.060	3.900	0.162
			1'			1" Ice	7.570	4.320	0.220
						2" Ice	8.620	5.200	0.359
						No Ice	6.580	3.500	0.111

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b>		136290.007.01 - HRT 086 943248, CT (BU# 806378)		<b>Page</b>		26 of 47	
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	<b>Client</b>		Crown Castle		<b>Designed by</b>		Jayaraj B	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight
			Horz	Vert			Front	Side	
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
AIR 6419 B41_TMO w/ Mount Pipe	B	From Leg	4.000	0.000	113'	No Ice	6.580	3.500	0.111
			0'			1/2" Ice	7.060	3.900	0.162
			1'			1" Ice	7.570	4.320	0.220
						2" Ice	8.620	5.200	0.359
AIR 6419 B41_TMO w/ Mount Pipe	C	From Leg	4.000	0.000	113'	No Ice	6.580	3.500	0.111
			0'			1/2" Ice	7.060	3.900	0.162
			1'			1" Ice	7.570	4.320	0.220
						2" Ice	8.620	5.200	0.359
APXVAALL24_43-U-NA20_TMO w/ Mount Pipe	A	From Leg	4.000	0.000	113'	No Ice	14.690	6.870	0.183
			0'			1/2" Ice	15.460	7.550	0.311
			1'			1" Ice	16.230	8.250	0.453
						2" Ice	17.820	9.670	0.782
APXVAALL24_43-U-NA20_TMO w/ Mount Pipe	B	From Leg	4.000	0.000	113'	No Ice	14.690	6.870	0.183
			0'			1/2" Ice	15.460	7.550	0.311
			1'			1" Ice	16.230	8.250	0.453
						2" Ice	17.820	9.670	0.782
APXVAALL24_43-U-NA20_TMO w/ Mount Pipe	C	From Leg	4.000	0.000	113'	No Ice	14.690	6.870	0.183
			0'			1/2" Ice	15.460	7.550	0.311
			1'			1" Ice	16.230	8.250	0.453
						2" Ice	17.820	9.670	0.782
RADIO 4449 B71 B85A_T-MOBILE	A	From Leg	4.000	0.000	113'	No Ice	1.970	1.587	0.073
			0'			1/2" Ice	2.147	1.749	0.093
			1'			1" Ice	2.331	1.918	0.116
						2" Ice	2.721	2.280	0.170
RADIO 4449 B71 B85A_T-MOBILE	B	From Leg	4.000	0.000	113'	No Ice	1.970	1.587	0.073
			0'			1/2" Ice	2.147	1.749	0.093
			1'			1" Ice	2.331	1.918	0.116
						2" Ice	2.721	2.280	0.170
RADIO 4449 B71 B85A_T-MOBILE	C	From Leg	4.000	0.000	113'	No Ice	1.970	1.587	0.073
			0'			1/2" Ice	2.147	1.749	0.093
			1'			1" Ice	2.331	1.918	0.116
						2" Ice	2.721	2.280	0.170
RADIO 4460 B2/B25 B66_TMO	A	From Leg	4.000	0.000	113'	No Ice	2.139	1.686	0.109
			0'			1/2" Ice	2.321	1.850	0.131
			1'			1" Ice	2.511	2.022	0.156
						2" Ice	2.912	2.387	0.217
RADIO 4460 B2/B25 B66_TMO	B	From Leg	4.000	0.000	113'	No Ice	2.139	1.686	0.109
			0'			1/2" Ice	2.321	1.850	0.131
			1'			1" Ice	2.511	2.022	0.156
						2" Ice	2.912	2.387	0.217
RADIO 4460 B2/B25 B66_TMO	C	From Leg	4.000	0.000	113'	No Ice	2.139	1.686	0.109
			0'			1/2" Ice	2.321	1.850	0.131
			1'			1" Ice	2.511	2.022	0.156
						2" Ice	2.912	2.387	0.217
5' x 2" Pipe Mount	A	From Leg	4.000	0.000	113'	No Ice	1.188	1.188	0.018
			0'			1/2" Ice	1.496	1.496	0.027
			0'			1" Ice	1.807	1.807	0.040
						2" Ice	2.458	2.458	0.076
5' x 2" Pipe Mount	B	From Leg	4.000	0.000	113'	No Ice	1.188	1.188	0.018
			0'			1/2" Ice	1.496	1.496	0.027
			0'			1" Ice	1.807	1.807	0.040
						2" Ice	2.458	2.458	0.076
5' x 2" Pipe Mount	C	From Leg	4.000	0.000	113'	No Ice	1.188	1.188	0.018
			0'			1/2" Ice	1.496	1.496	0.027
			0'			1" Ice	1.807	1.807	0.040
						2" Ice	2.458	2.458	0.076
15' x 2" Pipe Mount	A	From Leg	4.000	0.000	113'	No Ice	3.563	3.563	0.055

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 27 of 47
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	<b>Client</b> Crown Castle	<b>Designed by</b> Jayaraj B

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>Front</sub>	C <sub>A</sub> A <sub>Side</sub>	Weight
			Horz	Lateral					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
			0'			1/2" Ice	5.091	5.091	0.081
			0'			1" Ice	6.635	6.635	0.118
			0'			2" Ice	9.775	9.775	0.219
15' x 2" Pipe Mount	B	From Leg	4.000	0.000	113'	No Ice	3.563	3.563	0.055
			0'			1/2" Ice	5.091	5.091	0.081
			0'			1" Ice	6.635	6.635	0.118
			0'			2" Ice	9.775	9.775	0.219
15' x 2" Pipe Mount	C	From Leg	4.000	0.000	113'	No Ice	3.563	3.563	0.055
			0'			1/2" Ice	5.091	5.091	0.081
			0'			1" Ice	6.635	6.635	0.118
			0'			2" Ice	9.775	9.775	0.219
10' x 2" Mount Pipe	A	From Leg	4.000	0.000	113'	No Ice	2.375	2.375	0.037
			0'			1/2" Ice	3.403	3.403	0.054
			0'			1" Ice	4.448	4.448	0.079
			0'			2" Ice	5.911	5.911	0.148
10' x 2" Mount Pipe	B	From Leg	4.000	0.000	113'	No Ice	2.375	2.375	0.037
			0'			1/2" Ice	3.403	3.403	0.054
			0'			1" Ice	4.448	4.448	0.079
			0'			2" Ice	5.911	5.911	0.148
10' x 2" Mount Pipe	C	From Leg	4.000	0.000	113'	No Ice	2.375	2.375	0.037
			0'			1/2" Ice	3.403	3.403	0.054
			0'			1" Ice	4.448	4.448	0.079
			0'			2" Ice	5.911	5.911	0.148
Pipe Mount [PM 601-3]	C	None		0.000	113'	No Ice	3.170	3.170	0.195
						1/2" Ice	3.790	3.790	0.232
						1" Ice	4.420	4.420	0.279
						2" Ice	5.760	5.760	0.401
Sector Mount [SM 503-3]	C	None		0.000	113'	No Ice	30.430	30.430	1.690
						1/2" Ice	43.020	43.020	2.296
						1" Ice	55.430	55.430	3.097
						2" Ice	79.890	79.890	5.269
* GPS_A	A	From Leg	2.000	0.000	57'	No Ice	0.255	0.255	0.001
			0'			1/2" Ice	0.320	0.320	0.005
			3'			1" Ice	0.393	0.393	0.010
			0'			2" Ice	0.561	0.561	0.025
Side Arm Mount [SO 202-1]	A	From Leg	1.000	0.000	57'	No Ice	1.780	2.970	0.110
			0'			1/2" Ice	2.240	3.570	0.133
			0'			1" Ice	2.750	4.190	0.163
			0'			2" Ice	3.890	5.550	0.249
* Side Arm Mount [SO 202-1]	C	From Leg	1.000	0.000	48'	No Ice	1.780	2.970	0.110
			0'			1/2" Ice	2.240	3.570	0.133
			0'			1" Ice	2.750	4.190	0.163
			0'			2" Ice	3.890	5.550	0.249
*									
*									
*									

**Load Combinations**

<p><b>tnxTower</b></p> <p><b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	<p><b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)</p>	<p><b>Page</b> 28 of 47</p>
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	<p><b>Client</b> Crown Castle</p>	<p><b>Designed by</b> Jayaraj B</p>

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	160 - 140	Leg	Max Tension	23	15.146	-0.018	-0.016
			Max. Compression	10	-19.913	0.087	0.014
			Max. Mx	6	-1.668	0.733	-0.015

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	<b>Project</b>	<b>Date</b>	12:01:41 04/05/22
<b>Client</b>	Crown Castle	<b>Designed by</b> Jayaraj B	

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T2	140 - 135	Diagonal	Max. My	24	-1.561	0.002	-0.844
			Max. Vy	19	-1.314	0.581	0.015
			Max. Vx	12	1.358	0.001	-0.511
			Max Tension	25	3.228	0.000	0.000
			Max. Compression	12	-3.291	0.000	0.000
			Max. Mx	38	0.504	0.027	-0.000
			Max. My	20	-2.298	0.003	0.005
			Max. Vy	38	-0.023	0.027	-0.000
			Max. Vx	20	-0.001	0.000	0.000
			Max Tension	6	0.623	0.000	0.000
			Max. Compression	19	-0.622	0.000	0.000
			Max. Mx	26	-0.004	-0.053	0.000
		Max. My	26	-0.007	0.000	0.000	
		Max. Vy	26	0.032	0.000	0.000	
		Max. Vx	26	0.000	0.000	0.000	
		Max Tension	23	20.426	-0.070	-0.025	
		Max. Compression	10	-25.397	0.037	0.016	
		Max. Mx	14	18.380	-0.073	-0.006	
		Max. My	8	-2.986	0.009	-0.056	
		Max. Vy	14	0.039	-0.073	-0.006	
		Max. Vx	8	0.053	0.009	0.014	
		Max Tension	23	3.048	0.000	0.000	
		Max. Compression	10	-3.312	0.000	0.000	
		Max. Mx	27	0.354	0.019	0.003	
		Max. My	30	0.614	0.017	0.003	
		Max. Vy	37	0.021	0.018	-0.003	
		Max. Vx	30	-0.001	0.000	0.000	
		Max Tension	22	0.342	0.000	0.000	
		Max. Compression	3	-0.316	0.000	0.000	
		Max. Mx	26	0.025	-0.053	0.000	
Max. My	26	0.022	0.000	0.002			
Max. Vy	26	0.032	0.000	0.000			
Max. Vx	26	-0.001	0.000	0.000			
T3	135 - 130	Leg	Max Tension	23	27.886	-0.026	-0.038
			Max. Compression	10	-35.767	0.070	0.024
			Max. Mx	14	25.026	-0.073	-0.006
			Max. My	20	-4.730	-0.009	-0.259
			Max. Vy	10	0.039	0.070	0.024
			Max. Vx	8	-0.132	-0.009	0.258
		Diagonal	Max Tension	12	4.261	0.000	0.000
			Max. Compression	12	-4.219	0.000	0.000
			Max. Mx	38	1.308	0.023	0.002
			Max. My	16	-4.163	-0.002	-0.004
			Max. Vy	38	0.023	0.021	-0.002
			Max. Vx	28	-0.001	0.000	0.000
		Top Girt	Max Tension	23	35.857	-0.120	-0.032
			Max. Compression	10	-44.078	0.021	0.038
			Max. Mx	10	-44.052	0.143	0.032
			Max. My	20	-4.791	-0.009	-0.259
			Max. Vy	10	-0.054	0.143	0.032
			Max. Vx	20	-0.147	-0.009	-0.259
T4	130 - 125	Leg	Max Tension	13	4.231	0.000	0.000
			Max. Compression	12	-4.298	0.000	0.000
			Max. Mx	38	0.491	0.028	-0.003
			Max. My	20	3.849	0.013	-0.004
			Max. Vy	36	0.025	0.024	-0.004
			Max. Vx	36	0.002	0.000	0.000
		Diagonal	Max Tension	23	43.666	-0.040	-0.056
			Max. Compression	10	-53.774	0.143	0.032
			Max. Mx	10	-53.774	0.143	0.032
			Max. My	20	-6.043	-0.016	-0.371
			Max. Vy	20	-0.054	0.143	0.032
			Max. Vx	20	-0.147	-0.009	-0.259
T5	125 - 120	Leg	Max Tension	23	43.666	-0.040	-0.056
			Max. Compression	10	-53.774	0.143	0.032
			Max. Mx	10	-53.774	0.143	0.032
		Diagonal	Max. My	20	-6.043	-0.016	-0.371
			Max. Vy	20	-0.054	0.143	0.032
			Max. Vx	20	-0.147	-0.009	-0.259

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 30 of 47
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	<b>Client</b> Crown Castle	<b>Designed by</b> Jayaraj B

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft		
T6	120 - 113.333	Diagonal	Max. Vy	10	0.054	0.143	0.032		
			Max. Vx	20	0.175	-0.016	-0.371		
			Max Tension	12	4.849	0.000	0.000		
			Max. Compression	12	-4.867	0.000	0.000		
			Max. Mx	38	1.418	0.033	0.002		
			Max. My	4	-4.749	-0.003	0.007		
		Leg	Max. Vy	38	0.029	0.029	-0.003		
			Max. Vx	28	-0.002	0.000	0.000		
			Max Tension	23	53.239	-0.392	-0.067		
			Max. Compression	10	-63.946	0.028	0.055		
			Max. Mx	10	-63.906	0.421	0.066		
			Max. My	20	-6.216	-0.016	-0.371		
			Max. Vy	10	-0.106	0.421	0.066		
			Max. Vx	8	0.149	-0.016	0.370		
			Diagonal	Max Tension	12	5.327	0.000	0.000	
				Max. Compression	10	-5.423	0.000	0.000	
Max. Mx	27	1.233		0.065	0.008				
Max. My	8	4.678		0.051	0.011				
T7	113.333 - 106.667	Leg	Max. Vy	27	-0.042	0.065	0.008		
			Max. Vx	30	-0.003	0.000	0.000		
			Max Tension	23	64.711	-0.392	-0.067		
			Max. Compression	10	-77.956	-0.063	0.037		
			Max. Mx	10	-76.249	0.421	0.066		
			Max. My	20	-8.395	-0.057	-0.555		
		Diagonal	Max. Vy	14	-1.204	-0.391	-0.016		
			Max. Vx	8	1.093	0.002	0.175		
			Max Tension	12	6.541	0.000	0.000		
			Max. Compression	10	-6.800	0.000	0.000		
			Max. Mx	38	1.825	0.067	0.005		
			Max. My	22	-5.974	-0.001	-0.011		
			Max. Vy	38	-0.044	0.067	0.005		
			Max. Vx	29	-0.003	0.000	0.000		
			Leg	Max Tension	23	76.591	0.078	-0.026	
				Max. Compression	10	-91.673	-0.063	0.037	
Max. Mx	10	-91.629		0.589	-0.001				
Max. My	20	-8.729		-0.057	-0.555				
Max. Vy	10	0.229		0.589	-0.001				
Max. Vx	20	-0.266		-0.057	-0.555				
Diagonal	Max Tension	13		6.617	0.048	-0.003			
	Max. Compression	10		-6.753	0.000	0.000			
	Max. Mx	27		1.752	0.078	-0.010			
	Max. My	20		5.751	0.062	-0.012			
Secondary Horizontal	Max. Vy	27	-0.047	0.078	-0.010				
	Max. Vx	36	0.003	0.000	0.000				
	Max Tension	20	0.384	0.000	0.000				
	Max. Compression	21	-0.322	0.000	0.000				
	Max. Mx	30	0.156	0.036	0.002				
	Max. My	22	-0.213	0.013	0.006				
	Max. Vy	30	-0.033	0.036	0.002				
	Max. Vx	37	-0.002	0.000	0.000				
	T8	106.667 - 100	Leg	Max Tension	23	88.417	-0.386	-0.062	
				Max. Compression	10	-104.423	-0.115	0.024	
Max. Mx				10	-104.354	0.408	0.062		
Max. My				20	-9.465	-0.003	-0.360		
Max. Vy				10	-0.130	0.408	0.062		
Max. Vx				8	-0.139	-0.003	0.360		
Diagonal			Max Tension	16	6.563	0.000	0.000		
			Max. Compression	10	-6.763	0.000	0.000		
			Max. Mx	27	1.629	0.068	-0.007		
			Max. My	27	-0.047	0.078	-0.010		
T9			100 - 93.3333	Leg	Max. Vy	27	-0.047	0.078	-0.010
					Max. Vx	36	0.003	0.000	0.000
					Max Tension	20	0.384	0.000	0.000
					Max. Compression	21	-0.322	0.000	0.000
					Max. Mx	30	0.156	0.036	0.002
					Max. My	22	-0.213	0.013	0.006
	Diagonal	Max. Vy		30	-0.033	0.036	0.002		
		Max. Vx		37	-0.002	0.000	0.000		
		Max Tension		23	88.417	-0.386	-0.062		
		Max. Compression		10	-104.423	-0.115	0.024		
T9	100 - 93.3333	Leg	Max. Mx	10	-104.354	0.408	0.062		
			Max. My	20	-9.465	-0.003	-0.360		
			Max. Vy	10	-0.130	0.408	0.062		
			Max. Vx	8	-0.139	-0.003	0.360		
			Max Tension	16	6.563	0.000	0.000		
			Max. Compression	10	-6.763	0.000	0.000		
		Diagonal	Max. Mx	27	1.629	0.068	-0.007		
			Max. My	27	-0.047	0.078	-0.010		
			Max. Vx	36	0.003	0.000	0.000		
			Max Tension	20	0.384	0.000	0.000		

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft				
T10	93.3333 - 86.6667	Leg	Max. My	35	-2.124	0.056	-0.009				
			Max. Vy	37	0.045	0.062	0.006				
			Max. Vx	35	0.003	0.000	0.000				
			Max Tension	23	99.985	0.001	-0.050				
		Diagonal		Max. Compression	Max. Compression	10	-116.891	0.408	0.062		
					Max. Mx	10	-116.891	0.408	0.062		
				Max. My	Max. My	20	-9.944	-0.027	-0.453		
					Max. Vy	10	0.117	0.408	0.062		
				Max. Vx	Max. Vx	20	0.128	-0.027	-0.453		
					Max Tension	10	6.762	0.000	0.000		
				Max. Compression	Max. Compression	10	-6.774	0.000	0.000		
					Max. Mx	27	1.641	0.081	0.009		
				Max. My	Max. My	36	-1.093	0.058	-0.010		
					Max. Vy	37	0.051	0.072	-0.010		
T11	86.6667 - 80	Leg	Max. Vx	36	0.003	0.000	0.000				
			Max Tension	23	110.563	-0.704	0.005				
			Max. Compression	10	-128.535	-0.027	0.048				
			Max. Mx	10	-128.480	0.845	-0.004				
		Diagonal		Max. My	Max. My	20	-10.264	-0.027	-0.453		
					Max. Vy	10	0.314	0.845	-0.004		
				Max. Vx	Max. Vx	20	-0.216	-0.027	-0.453		
					Max Tension	23	6.875	-0.082	0.004		
				Max. Compression	Max. Compression	10	-7.420	0.000	0.000		
					Max. Mx	27	1.525	-0.133	0.017		
				Max. My	Max. My	36	-2.115	-0.089	0.020		
					Max. Vy	37	-0.077	-0.120	-0.014		
				Max. Vx	Max. Vx	36	0.005	0.000	0.000		
					Max Tension	20	0.432	0.000	0.000		
T12	80 - 73.3333	Leg	Max. Compression	21	-0.363	0.000	0.000				
			Max. Mx	35	0.073	0.051	0.001				
			Max. My	22	-0.290	0.015	0.005				
			Max. Vy	35	0.040	0.051	0.001				
		Diagonal		Max. Vx	Max. Vx	37	-0.001	0.000	0.000		
					Max Tension	23	121.397	-0.090	-0.048		
				Max. Compression	Max. Compression	10	-140.636	-0.142	0.011		
					Max. Mx	33	6.921	-0.270	-0.014		
				Max. My	Max. My	20	-11.165	-0.004	-0.522		
					Max. Vy	10	-0.093	0.107	0.046		
				Max. Vx	Max. Vx	20	0.151	-0.004	-0.522		
					Max Tension	10	6.977	0.000	0.000		
				Max. Compression	Max. Compression	10	-6.908	0.000	0.000		
					Max. Mx	27	1.841	0.096	0.012		
Max. My	Max. My	36	-1.110	0.073	-0.013						
	Max. Vy	37	0.059	0.088	-0.012						
Max. Vx	Max. Vx	36	0.003	0.000	0.000						
	Max Tension	23	131.275	-0.967	0.012						
T13	73.3333 - 66.6667	Leg	Max. Compression	10	-151.503	0.107	0.046				
			Max. Mx	10	-151.480	1.137	-0.012				
			Max. My	20	-11.389	-0.004	-0.522				
			Max. Vy	10	0.565	1.137	-0.012				
		Diagonal		Max. Vx	Max. Vx	20	-0.233	-0.004	-0.522		
					Max Tension	23	7.149	0.057	-0.001		
				Max. Compression	Max. Compression	10	-7.765	0.000	0.000		
					Max. Mx	27	1.368	0.119	-0.015		
				Max. My	Max. My	36	1.366	0.115	-0.017		
					Max. Vy	37	0.064	0.112	0.013		
				Max. Vx	Max. Vx	36	0.004	0.000	0.000		
					Max Tension	20	0.605	0.000	0.000		
				Secondary Horizontal		Max. Compression	Max. Compression	21	-0.363	0.000	0.000
							Max. Mx	35	0.073	0.051	0.001
Max. My	Max. My	22	-0.290	0.015	0.005						
	Max. Vy	35	0.040	0.051	0.001						
Max. Vx	Max. Vx	37	-0.001	0.000	0.000						
	Max Tension	23	121.397	-0.090	-0.048						



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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
		Horizontal					
T14	66.6667 - 60	Leg	Max. Compression	21	-0.515	0.000	0.000
			Max. Mx	35	0.084	0.062	0.003
			Max. My	38	-0.092	0.061	0.004
			Max. Vy	35	0.043	0.062	0.003
			Max. Vx	38	-0.002	0.000	0.000
			Max Tension	23	141.349	-0.911	0.021
		Diagonal	Max. Compression	10	-162.876	-0.703	0.016
			Max. Mx	10	-162.853	1.079	-0.021
			Max. My	20	-12.271	-0.008	-0.894
			Max. Vy	10	-0.573	1.079	-0.021
			Max. Vx	20	0.354	-0.008	-0.894
			Max Tension	11	7.025	0.040	0.003
			Max. Compression	10	-7.575	0.000	0.000
			Max. Mx	27	1.892	0.099	0.016
Secondary Horizontal	Max. My	35	-2.270	0.069	-0.018		
	Max. Vy	37	0.063	0.095	-0.015		
T15	60 - 50	Leg	Max. Vx	35	-0.004	0.000	0.000
			Max Tension	22	0.574	0.019	-0.001
			Max. Compression	11	-0.486	0.000	0.000
			Max. Mx	37	0.082	0.061	0.005
			Max. My	38	0.029	0.061	0.005
			Max. Vy	37	0.045	0.061	0.005
		Diagonal	Max. Vx	38	-0.002	0.000	0.000
			Max Tension	23	153.724	0.003	-0.083
			Max. Compression	10	-176.946	0.537	0.151
			Max. Mx	33	13.019	-0.737	-0.021
			Max. My	20	-13.329	-0.144	-1.198
			Max. Vy	10	0.181	0.537	0.151
			Max. Vx	20	0.222	-0.144	-1.198
			Max Tension	10	8.609	0.000	0.000
T16	50 - 40	Leg	Max. Compression	10	-8.757	0.000	0.000
			Max. Mx	37	1.072	-0.234	0.025
			Max. My	18	-8.343	-0.038	0.035
			Max. Vy	37	-0.111	-0.234	0.025
			Max. Vx	29	0.007	0.000	0.000
			Max Tension	23	168.039	-1.824	0.020
		Diagonal	Max. Compression	10	-193.241	-0.206	0.078
			Max. Mx	10	-193.238	2.141	-0.019
			Max. My	20	-13.818	-0.144	-1.198
			Max. Vy	10	0.696	2.141	-0.019
T17	40 - 30	Leg	Max. Vx	20	-0.384	-0.144	-1.198
			Max Tension	23	8.762	-0.135	-0.008
			Max. Compression	10	-9.702	0.000	0.000
			Max. Mx	27	2.599	-0.214	0.036
			Max. My	20	6.545	-0.159	0.042
			Max. Vy	37	-0.111	-0.206	-0.032
		Secondary Horizontal	Max. Vx	36	-0.007	0.000	0.000
			Max Tension	20	0.835	0.000	0.000
			Max. Compression	21	-0.730	0.000	0.000
			Max. Mx	36	0.351	0.091	0.003
T17	40 - 30	Leg	Max. My	22	-0.584	0.033	0.009
			Max. Vy	36	0.059	0.091	0.003
			Max. Vx	37	-0.002	0.000	0.000
			Max Tension	23	182.066	-2.467	0.001
			Max. Compression	10	-209.771	2.949	0.001
			Max. Mx	10	-209.685	-3.306	0.005
T17	40 - 30	Leg	Max. My	20	-15.487	-0.447	-2.044
			Max. Vy	10	1.256	2.949	0.001

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft	
T18	30 - 20	Diagonal	Max. Vx	20	0.577	-0.447	-2.044	
			Max Tension	23	8.804	-0.197	0.023	
			Max. Compression	10	-9.855	0.000	0.000	
			Max. Mx	37	0.628	-0.331	0.034	
			Max. My	18	-9.582	-0.031	0.059	
			Max. Vy	37	-0.140	-0.331	0.034	
			Max. Vx	35	0.009	0.000	0.000	
			Max Tension	20	1.484	0.000	0.000	
			Secondary Horizontal	Max. Compression	21	-1.282	0.000	0.000
				Max. Mx	36	-0.228	0.163	0.014
				Max. My	22	-1.029	0.063	0.019
				Max. Vy	36	0.083	0.163	0.014
				Max. Vx	37	-0.004	0.000	0.000
				Max Tension	23	194.903	-2.113	0.090
		Max. Compression		10	-225.572	2.499	-0.091	
		Max. Mx		10	-224.521	3.545	0.066	
		Max. My		20	-16.156	-0.447	-2.044	
		Max. Vy		10	-2.756	3.545	0.066	
		Leg	Max. Vx	24	0.585	-0.436	1.982	
			Max Tension	23	9.512	-0.141	-0.005	
			Max. Compression	10	-10.696	0.000	0.000	
			Max. Mx	10	7.061	-0.232	0.006	
			Max. My	37	-0.162	-0.029	0.014	
			Max. Vy	27	-0.083	-0.155	0.013	
			Max. Vx	29	0.004	0.000	0.000	
			Max Tension	20	0.692	0.000	0.000	
			Max. Compression	21	-0.614	0.000	0.000	
			Max. Mx	37	0.409	0.119	0.097	
			Max. My	27	0.041	0.112	0.097	
			Max. Vy	37	-0.072	0.119	0.097	
			Max. Vx	27	0.012	0.000	0.000	
			Max Tension	10	2.591	0.000	0.000	
Redund Horz 1 Bracing	Max. Compression	23	-2.189	0.000	0.000			
	Max. Mx	26	0.788	-0.024	0.000			
	Max. My	26	0.853	0.000	0.001			
	Max. Vy	26	0.021	0.000	0.000			
	Max. Vx	26	-0.001	0.000	0.000			
	Max Tension	23	1.286	0.000	0.000			
	Max. Compression	10	-1.701	0.000	0.000			
	Max. Mx	26	-0.276	-0.029	0.000			
	Max. My	26	-0.259	0.000	-0.001			
	Max. Vy	26	0.022	0.000	0.000			
	Max. Vx	26	-0.001	0.000	0.000			
	Max Tension	23	221.605	-0.916	-0.091			
	Redund Diag 1 Bracing	Max. Compression	10	-258.236	0.000	-0.000		
		Max. Mx	27	-110.600	-2.484	0.014		
Max. My		20	-19.216	-0.083	-1.443			
Max. Vy		27	-0.457	-2.484	0.014			
Max. Vx		20	-0.303	-0.083	-1.443			
Max Tension		10	10.094	0.000	0.000			
Max. Compression		10	-10.871	0.000	0.000			
Max. Mx		37	-0.124	-0.474	0.041			
Max. My		20	7.038	-0.292	0.071			
Max. Vy		37	-0.172	-0.474	0.041			
Max. Vx		36	-0.010	0.000	0.000			
T19		20 - 0	Leg	Max. Vx	26	-0.001	0.000	0.000
				Max Tension	23	221.605	-0.916	-0.091
				Max. Compression	10	-258.236	0.000	-0.000
	Max. Mx			27	-110.600	-2.484	0.014	
	Max. My			20	-19.216	-0.083	-1.443	
	Max. Vy			27	-0.457	-2.484	0.014	
	Max. Vx			20	-0.303	-0.083	-1.443	
	Max Tension			10	10.094	0.000	0.000	
	Max. Compression			10	-10.871	0.000	0.000	
	Max. Mx			37	-0.124	-0.474	0.041	
	Max. My			20	7.038	-0.292	0.071	
	Max. Vy			37	-0.172	-0.474	0.041	
	Max. Vx			36	-0.010	0.000	0.000	
	Diagonal			Max Tension	10	10.094	0.000	0.000
Max. Compression		10	-10.871	0.000	0.000			
Max. Mx		37	-0.124	-0.474	0.041			
Max. My		20	7.038	-0.292	0.071			
Max. Vy		37	-0.172	-0.474	0.041			
Max. Vx		36	-0.010	0.000	0.000			

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### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	18	257.447	24.079	-15.672
	Max. H <sub>x</sub>	18	257.447	24.079	-15.672
	Max. H <sub>z</sub>	7	-213.294	-20.467	13.493
	Min. Vert	7	-213.294	-20.467	13.493
	Min. H <sub>x</sub>	7	-213.294	-20.467	13.493
	Min. H <sub>z</sub>	18	257.447	24.079	-15.672
Leg B	Max. Vert	10	266.502	-25.504	-16.097
	Max. H <sub>x</sub>	23	-228.228	21.956	14.031
	Max. H <sub>z</sub>	23	-228.228	21.956	14.031
	Min. Vert	23	-228.228	21.956	14.031
	Min. H <sub>x</sub>	10	266.502	-25.504	-16.097
	Min. H <sub>z</sub>	10	266.502	-25.504	-16.097
Leg A	Max. Vert	2	253.094	-0.515	27.960
	Max. H <sub>x</sub>	21	15.226	4.943	1.196
	Max. H <sub>z</sub>	2	253.094	-0.515	27.960
	Min. Vert	15	-207.404	0.450	-23.714
	Min. H <sub>x</sub>	8	20.444	-5.008	1.595
	Min. H <sub>z</sub>	15	-207.404	0.450	-23.714

### Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overtuning Moment, M <sub>x</sub> kip-ft	Overtuning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	46.694	-0.000	-0.000	-25.675	29.202	0.000
1.2 Dead+1.0 Wind 0 deg - No Ice	56.033	-0.002	-44.195	-4235.726	33.944	-17.308
0.9 Dead+1.0 Wind 0 deg - No Ice	42.025	-0.002	-44.195	-4228.024	25.184	-17.308
1.2 Dead+1.0 Wind 30 deg - No Ice	56.033	20.840	-36.089	-3507.367	-1973.005	-33.389
0.9 Dead+1.0 Wind 30 deg - No Ice	42.025	20.840	-36.089	-3499.665	-1981.766	-33.389
1.2 Dead+1.0 Wind 60 deg - No Ice	56.033	37.203	-21.472	-2090.312	-3531.406	-53.775
0.9 Dead+1.0 Wind 60 deg - No Ice	42.025	37.203	-21.472	-2082.609	-3540.167	-53.775
1.2 Dead+1.0 Wind 90 deg - No Ice	56.033	45.957	0.002	-31.908	-4323.750	-62.582
0.9 Dead+1.0 Wind 90 deg - No Ice	42.025	45.957	0.002	-24.206	-4332.510	-62.582
1.2 Dead+1.0 Wind 120 deg - No Ice	56.033	41.627	24.029	2230.428	-3883.022	-42.639
0.9 Dead+1.0 Wind 120 deg - No Ice	42.025	41.627	24.029	2238.131	-3891.782	-42.639
1.2 Dead+1.0 Wind 150 deg - No Ice	56.033	22.073	38.220	3630.984	-2078.684	-5.424
0.9 Dead+1.0 Wind 150 deg - No Ice	42.025	22.073	38.220	3638.686	-2087.445	-5.424
1.2 Dead+1.0 Wind 180 deg - No Ice	56.033	0.002	41.653	3993.055	36.140	17.308
0.9 Dead+1.0 Wind 180 deg - No Ice	42.025	0.002	41.653	4000.757	27.380	17.308

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Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
1.2 Dead+1.0 Wind 210 deg - No Ice	56.033	-20.840	36.089	3445.747	2043.090	33.389
0.9 Dead+1.0 Wind 210 deg - No Ice	42.025	-20.840	36.089	3453.449	2034.329	33.389
1.2 Dead+1.0 Wind 240 deg - No Ice	56.033	-39.405	22.743	2119.216	3758.285	53.775
0.9 Dead+1.0 Wind 240 deg - No Ice	42.025	-39.405	22.743	2126.919	3749.525	53.775
1.2 Dead+1.0 Wind 270 deg - No Ice	56.033	-45.957	-0.002	-29.712	4393.834	62.582
0.9 Dead+1.0 Wind 270 deg - No Ice	42.025	-45.957	-0.002	-22.010	4385.074	62.582
1.2 Dead+1.0 Wind 300 deg - No Ice	56.033	-39.426	-22.758	-2201.524	3796.311	42.639
0.9 Dead+1.0 Wind 300 deg - No Ice	42.025	-39.426	-22.758	-2193.821	3787.551	42.639
1.2 Dead+1.0 Wind 330 deg - No Ice	56.033	-22.073	-38.220	-3692.604	2148.769	5.424
0.9 Dead+1.0 Wind 330 deg - No Ice	42.025	-22.073	-38.220	-3684.902	2140.008	5.424
1.2 Dead+1.0 Ice+1.0 Temp	132.950	-0.000	-0.000	-131.655	67.348	-0.000
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	132.950	-0.001	-13.377	-1419.372	67.157	-5.889
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	132.950	6.550	-11.349	-1228.729	-565.689	-12.657
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	132.950	11.644	-6.724	-778.580	-1052.157	-19.060
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	132.950	13.962	0.001	-131.846	-1266.196	-19.853
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	132.950	12.327	7.120	545.938	-1105.657	-12.300
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	132.950	6.746	11.687	994.967	-582.530	-1.954
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	132.950	0.001	12.979	1128.406	67.538	5.889
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	132.950	-6.550	11.349	965.418	700.385	12.657
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	132.950	-11.989	6.923	529.098	1210.803	19.060
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	132.950	-13.962	-0.001	-131.465	1400.891	19.853
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	132.950	-11.982	-6.920	-795.421	1216.402	12.300
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	132.950	-6.746	-11.687	-1258.278	717.225	1.954
Dead+Wind 0 deg - Service	46.694	-0.001	-12.363	-1193.645	28.898	-4.796
Dead+Wind 30 deg - Service	46.694	5.834	-10.102	-991.488	-528.648	-9.248
Dead+Wind 60 deg - Service	46.694	10.410	-6.009	-597.768	-961.495	-14.891
Dead+Wind 90 deg - Service	46.694	12.851	0.001	-25.979	-1181.365	-17.327
Dead+Wind 120 deg - Service	46.694	11.635	6.716	602.264	-1058.832	-11.804
Dead+Wind 150 deg - Service	46.694	6.175	10.692	991.416	-557.902	-1.499
Dead+Wind 180 deg - Service	46.694	0.001	11.659	1092.175	29.506	4.796
Dead+Wind 210 deg - Service	46.694	-5.834	10.102	940.138	587.051	9.248
Dead+Wind 240 deg - Service	46.694	-11.020	6.360	571.478	1063.304	14.891
Dead+Wind 270 deg - Service	46.694	-12.851	-0.001	-25.371	1239.768	17.327
Dead+Wind 300 deg - Service	46.694	-11.026	-6.365	-628.555	1073.831	11.804
Dead+Wind 330 deg - Service	46.694	-6.175	-10.692	-1042.767	616.306	1.499

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## Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.000	-46.694	0.000	0.000	46.694	0.000	0.000%
2	-0.002	-56.033	-44.195	0.002	56.033	44.195	0.000%
3	-0.002	-42.025	-44.195	0.002	42.025	44.195	0.000%
4	20.840	-56.033	-36.089	-20.840	56.033	36.089	0.000%
5	20.840	-42.025	-36.089	-20.840	42.025	36.089	0.000%
6	37.203	-56.033	-21.472	-37.203	56.033	21.472	0.000%
7	37.203	-42.025	-21.472	-37.203	42.025	21.472	0.000%
8	45.957	-56.033	0.002	-45.957	56.033	-0.002	0.000%
9	45.957	-42.025	0.002	-45.957	42.025	-0.002	0.000%
10	41.627	-56.033	24.029	-41.627	56.033	-24.029	0.000%
11	41.627	-42.025	24.029	-41.627	42.025	-24.029	0.000%
12	22.073	-56.033	38.220	-22.073	56.033	-38.220	0.000%
13	22.073	-42.025	38.220	-22.073	42.025	-38.220	0.000%
14	0.002	-56.033	41.653	-0.002	56.033	-41.653	0.000%
15	0.002	-42.025	41.653	-0.002	42.025	-41.653	0.000%
16	-20.840	-56.033	36.089	20.840	56.033	-36.089	0.000%
17	-20.840	-42.025	36.089	20.840	42.025	-36.089	0.000%
18	-39.405	-56.033	22.743	39.405	56.033	-22.743	0.000%
19	-39.405	-42.025	22.743	39.405	42.025	-22.743	0.000%
20	-45.957	-56.033	-0.002	45.957	56.033	0.002	0.000%
21	-45.957	-42.025	-0.002	45.957	42.025	0.002	0.000%
22	-39.426	-56.033	-22.758	39.426	56.033	22.758	0.000%
23	-39.426	-42.025	-22.758	39.426	42.025	22.758	0.000%
24	-22.073	-56.033	-38.220	22.073	56.033	38.220	0.000%
25	-22.073	-42.025	-38.220	22.073	42.025	38.220	0.000%
26	0.000	-132.950	0.000	0.000	132.950	0.000	0.000%
27	-0.001	-132.950	-13.377	0.001	132.950	13.377	0.000%
28	6.550	-132.950	-11.349	-6.550	132.950	11.349	0.000%
29	11.644	-132.950	-6.724	-11.644	132.950	6.724	0.000%
30	13.962	-132.950	0.001	-13.962	132.950	-0.001	0.000%
31	12.327	-132.950	7.120	-12.327	132.950	-7.120	0.000%
32	6.746	-132.950	11.687	-6.746	132.950	-11.687	0.000%
33	0.001	-132.950	12.979	-0.001	132.950	-12.979	0.000%
34	-6.550	-132.950	11.349	6.550	132.950	-11.349	0.000%
35	-11.989	-132.950	6.923	11.989	132.950	-6.923	0.000%
36	-13.962	-132.950	-0.001	13.962	132.950	0.001	0.000%
37	-11.982	-132.950	-6.920	11.982	132.950	6.920	0.000%
38	-6.746	-132.950	-11.687	6.746	132.950	11.687	0.000%
39	-0.001	-46.694	-12.363	0.001	46.694	12.363	0.000%
40	5.834	-46.694	-10.102	-5.834	46.694	10.102	0.000%
41	10.410	-46.694	-6.009	-10.410	46.694	6.009	0.000%
42	12.851	-46.694	0.001	-12.851	46.694	-0.001	0.000%
43	11.635	-46.694	6.716	-11.635	46.694	-6.716	0.000%
44	6.175	-46.694	10.692	-6.175	46.694	-10.692	0.000%
45	0.001	-46.694	11.659	-0.001	46.694	-11.659	0.000%
46	-5.834	-46.694	10.102	5.834	46.694	-10.102	0.000%
47	-11.020	-46.694	6.360	11.020	46.694	-6.360	0.000%
48	-12.851	-46.694	-0.001	12.851	46.694	0.001	0.000%
49	-11.026	-46.694	-6.365	11.026	46.694	6.365	0.000%
50	-6.175	-46.694	-10.692	6.175	46.694	10.692	0.000%

## Maximum Tower Deflections - Service Wind

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	160 - 140	4.925	49	0.286	0.034
T2	140 - 135	3.734	49	0.265	0.034
T3	135 - 130	3.455	49	0.258	0.033
T4	130 - 125	3.180	49	0.249	0.032
T5	125 - 120	2.918	49	0.239	0.031
T6	120 - 113.333	2.662	49	0.227	0.029
T7	113.333 - 106.667	2.349	49	0.214	0.028
T8	106.667 - 100	2.051	49	0.199	0.026
T9	100 - 93.3333	1.778	49	0.182	0.024
T10	93.3333 - 86.6667	1.521	49	0.167	0.021
T11	86.6667 - 80	1.289	49	0.151	0.019
T12	80 - 73.3333	1.083	49	0.135	0.018
T13	73.3333 - 66.6667	0.894	49	0.121	0.015
T14	66.6667 - 60	0.724	49	0.106	0.012
T15	60 - 50	0.573	49	0.091	0.010
T16	50 - 40	0.388	49	0.075	0.008
T17	40 - 30	0.242	49	0.058	0.006
T18	30 - 20	0.130	43	0.040	0.004
T19	20 - 0	0.061	43	0.022	0.003

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
157'	(2) APL866513-42T6 w/ Mount Pipe	49	4.742	0.283	0.034	103129
147'	MX08FRO665-21 w/ Mount Pipe	49	4.139	0.273	0.035	39665
135'	7770.00 w/ Mount Pipe	49	3.455	0.258	0.033	95370
125'	APXVSPP18-C-A20 w/ Mount Pipe	49	2.918	0.239	0.031	43528
113'	VV-65A-R1_TMO w/ Mount Pipe	49	2.334	0.213	0.028	49866
57'	GPS_A	49	0.512	0.086	0.009	23359
48'	Side Arm Mount [SO 202-1]	49	0.356	0.071	0.007	34087

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	160 - 140	17.685	11	1.020	0.124
T2	140 - 135	13.428	11	0.947	0.123
T3	135 - 130	12.429	11	0.923	0.121
T4	130 - 125	11.441	11	0.892	0.117
T5	125 - 120	10.501	11	0.856	0.111
T6	120 - 113.333	9.585	11	0.813	0.105
T7	113.333 - 106.667	8.464	11	0.766	0.100
T8	106.667 - 100	7.393	11	0.711	0.094
T9	100 - 93.3333	6.413	11	0.651	0.087
T10	93.3333 - 86.6667	5.491	11	0.599	0.077
T11	86.6667 - 80	4.658	11	0.543	0.069
T12	80 - 73.3333	3.917	11	0.484	0.064
T13	73.3333 - 66.6667	3.234	11	0.433	0.054

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T14	66.6667 - 60	2.621	11	0.382	0.045
T15	60 - 50	2.074	11	0.328	0.035
T16	50 - 40	1.409	11	0.268	0.028
T17	40 - 30	0.882	11	0.207	0.020
T18	30 - 20	0.477	11	0.144	0.015
T19	20 - 0	0.222	11	0.081	0.010

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
157'	(2) APL866513-42T6 w/ Mount Pipe	11	17.031	1.010	0.124	29219
147'	MX08FRO665-21 w/ Mount Pipe	11	14.877	0.975	0.125	11238
135'	7770.00 w/ Mount Pipe	11	12.429	0.923	0.121	28060
125'	APXVSPP18-C-A20 w/ Mount Pipe	11	10.501	0.856	0.111	12310
113'	VV-65A-R1_TMO w/ Mount Pipe	11	8.409	0.763	0.100	14200
57'	GPS_A	11	1.856	0.308	0.032	6396
48'	Side Arm Mount [SO 202-1]	11	1.294	0.257	0.026	9590

### Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load per Bolt K	Ratio Load Allowable	Allowable Ratio	Criteria	
T1	160	Leg	A325N	0.625	4	3.787	20.340	0.186	✓	1.05	Bolt Tension
		Diagonal	A325N	0.500	1	3.228	6.199	0.521	✓	1.05	Member Bearing
		Top Girt	A325N	0.500	1	0.623	4.133	0.151	✓	1.05	Member Bearing
T2	140	Diagonal	A325N	0.500	1	3.048	6.199	0.492	✓	1.05	Member Bearing
		Top Girt	A325N	0.500	1	0.440	4.133	0.107	✓	1.05	Member Bearing
T3	135	Diagonal	A325N	0.500	1	4.261	6.199	0.687	✓	1.05	Member Bearing
T4	130	Diagonal	A325N	0.500	1	4.231	6.199	0.682	✓	1.05	Member Bearing
T5	125	Leg	A325N	0.750	4	10.916	30.101	0.363	✓	1.05	Bolt Tension
		Diagonal	A325N	0.500	1	4.849	6.199	0.782	✓	1.05	Member Bearing
T6	120	Diagonal	A325X	0.500	1	5.327	7.504	0.710	✓	1.05	Gusset Bearing
T7	113.333	Diagonal	A325X	0.500	1	6.541	7.504	0.872	✓	1.05	Gusset Bearing
T8	106.667	Leg	A325N	0.875	4	19.148	41.556	0.461	✓	1.05	Bolt Tension
		Diagonal	A325X	0.500	1	6.617	7.504	0.882	✓	1.05	Gusset Bearing
		Secondary Horizontal	A325N	0.625	1	1.590	7.178	0.222	✓	1.05	Gusset Bearing
T11	86.6667	Leg	A325N	0.875	4	27.639	41.556	0.665	✓	1.05	Bolt Tension
		Secondary Horizontal	A325N	0.625	1	2.229	6.831	0.326	✓	1.05	Member Block Shear

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Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load per Bolt K	Ratio Load Allowable	Allowable Ratio	Criteria
T13	73.3333	Secondary Horizontal	A325N	0.625	1	2.628	8.128	0.323 ✓	1.05	Member Block Shear
T14	66.6667	Leg	A325N	1.000	4	35.331	54.517	0.648 ✓	1.05	Bolt Tension
		Secondary Horizontal	A325N	0.625	1	2.825	6.831	0.414 ✓	1.05	Member Block Shear
T15	60	Diagonal	A325N	0.625	1	8.609	13.920	0.618 ✓	1.05	Gusset Bearing
T16	50	Leg	A325N	1.000	4	41.993	54.517	0.770 ✓	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	8.762	13.920	0.629 ✓	1.05	Gusset Bearing
		Secondary Horizontal	A325N	0.625	1	3.351	8.482	0.395 ✓	1.05	Member Bearing
T17	40	Diagonal	A325N	0.625	1	8.804	13.920	0.632 ✓	1.05	Gusset Bearing
		Secondary Horizontal	A325N	0.625	1	3.638	10.440	0.348 ✓	1.05	Member Bearing
T18	30	Leg	A325N	1.000	6	32.462	54.517	0.595 ✓	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	9.512	13.920	0.683 ✓	1.05	Gusset Bearing
		Horizontal	A325N	0.625	1	3.912	7.830	0.500 ✓	1.05	Member Bearing
		Redund Horiz 1 Bracing	A325N	0.625	1	3.912	8.135	0.481 ✓	1.05	Member Block Shear
		Redund Diag 1 Bracing	A325N	0.625	1	2.277	8.135	0.280 ✓	1.05	Member Block Shear
T19	20	Diagonal	A325N	0.625	1	10.094	13.920	0.725 ✓	1.05	Gusset Bearing

### Compression Checks

### Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	160 - 140	ROHN 2 STD	20'	4'	61.0 K=1.00	1.075	-19.913	36.842	0.540 <sup>1</sup> ✓
T2	140 - 135	ROHN 2.5 EH	5'3/32"	5'3/32"	65.0 K=1.00	2.254	-25.397	74.427	0.341 <sup>1</sup> ✓
T3	135 - 130	ROHN 2.5 EH	5'3/32"	5'3/32"	65.0 K=1.00	2.254	-35.767	74.427	0.481 <sup>1</sup> ✓
T4	130 - 125	ROHN 2.5 EH	5'3/32"	5'3/32"	65.0 K=1.00	2.254	-44.078	74.427	0.592 <sup>1</sup> ✓
T5	125 - 120	ROHN 2.5 EH	5'3/32"	5'3/32"	65.0 K=1.00	2.254	-53.774	74.427	0.722 <sup>1</sup> ✓
T6	120 - 113.333	ROHN 3 EH	6'8-1/8"	6'8-1/8"	70.5 K=1.00	3.016	-63.946	94.342	0.678 <sup>1</sup> ✓
T7	113.333 - 106.667	ROHN 3 EH	6'8-1/8"	6'8-1/8"	70.5 K=1.00	3.016	-77.956	94.342	0.826 <sup>1</sup> ✓
T8	106.667 - 100	ROHN 3 EH	6'8-1/8"	3'5-3/8"	36.4 K=1.00	3.016	-91.616	123.172	0.744 <sup>1</sup> ✓



<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 40 of 47
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	<b>Client</b> Crown Castle	<b>Designed by</b> Jayaraj B

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T9	100 - 93.3333	ROHN 3.5 EH	6'8-1/8"	6'8-1/8"	61.3 K=1.00	3.678	-104.423	125.726	0.831 <sup>1</sup>
T10	93.3333 - 86.6667	ROHN 3.5 EH	6'8-1/8"	6'8-1/8"	61.3 K=1.00	3.678	-116.891	125.725	0.930 <sup>1</sup>
T11	86.6667 - 80	ROHN 3.5 EH	6'8-1/8"	3'5-3/16'	31.5 K=1.00	3.678	-128.480	153.937	0.835 <sup>1</sup>
T12	80 - 73.3333	ROHN 4 X-STR	6'8-5/32'	6'8-5/32'	54.3 K=1.00	4.407	-140.636	159.903	0.880 <sup>1</sup>
T13	73.3333 - 66.6667	ROHN 4 X-STR	6'8-5/32'	3'5-3/32'	27.8 K=1.00	4.407	-151.443	187.417	0.808 <sup>1</sup>
T14	66.6667 - 60	ROHN 4 X-STR	6'8-5/32'	3'5-1/32'	27.8 K=1.00	4.407	-162.824	187.442	0.869 <sup>1</sup>
T15	60 - 50	ROHN. 5 EH	10'7/32"	10'7/32"	65.4 K=1.00	6.112	-176.946	201.251	0.879 <sup>1</sup>
T16	50 - 40	ROHN. 5 EH	10'7/32"	5'1-15/16"	33.7 K=1.00	6.112	-193.191	253.141	0.763 <sup>1</sup>
T17	40 - 30	ROHN 5 X-STR	10'7/32"	5'1-29/32"	33.7 K=1.00	6.112	-209.685	253.160	0.828 <sup>1</sup>
T18	30 - 20	ROHN 5 X-STR	10'7/32"	2'6-1/16"	16.3 K=1.00	6.112	-225.572	269.720	0.836 <sup>1</sup>
T19	20 - 0	B+T_BU 806378 - 6.625"x0.34" pipe w/ 2" SR	20'13/32"	10'7/32"	75.6 K=1.00	9.855	-258.236	292.104	0.884 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	160 - 140	L1 3/4x1 3/4x3/16	7'8-3/16'	3'7-15/32"	126.6 K=1.00	0.621	-3.291	11.092	0.297 <sup>1</sup>
T2	140 - 135	L1 3/4x1 3/4x3/16	8'5-15/32"	4'1-19/32"	144.4 K=1.00	0.621	-3.312	8.523	0.389 <sup>1</sup>
T3	135 - 130	L1 3/4x1 3/4x3/16	8'10-15/32"	4'4-3/32'	151.7 K=1.00	0.621	-4.219	7.725	0.546 <sup>1</sup>
T4	130 - 125	L1 3/4x1 3/4x3/16	9'3-19/32"	4'6-21/32"	159.1 K=1.00	0.621	-4.298	7.020	0.612 <sup>1</sup>
T5	125 - 120	L2x2x3/16	9'8-25/32"	4'9-1/4"	145.3 K=1.00	0.715	-4.867	9.691	0.502 <sup>1</sup>
T6	120 - 113.333	L2 1/2x2 1/2x1/4	11'1-7/8"	5'6-1/32'	134.5 K=1.00	1.190	-5.423	18.829	0.288 <sup>1</sup>
T7	113.333 - 106.667	L2 1/2x2 1/2x1/4	11'8-15/32"	5'9-5/16'	141.2 K=1.00	1.190	-6.800	17.085	0.398 <sup>1</sup>
T8	106.667 - 100	L2 1/2x2 1/2x1/4	12'3-7/32"	6'1-15/16"	150.6 K=1.00	1.190	-6.753	15.017	0.450 <sup>1</sup>
T9	100 - 93.3333	L2 1/2x2 1/2x3/16	12'10-1/8"	6'5-3/32'	147.3 K=0.95	0.902	-6.763	11.900	0.568 <sup>1</sup>
T10	93.3333 - 86.6667	L2 1/2x2 1/2x1/4	13'5-5/32"	6'8-5/8"	153.7 K=0.94	1.190	-6.774	14.411	0.470 <sup>1</sup>

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 41 of 47
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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T11	86.6667 - 80	2L2 1/2x2 1/2x3/16x1/4	14'5/16"	7'3/16"	113.1 K=1.00	1.805	-7.420	36.566	0.203 <sup>1</sup> ✓
T12	80 - 73.3333	2L 'a' > 40.153 in - 129 L 3x3x3/16	14'7-19/ 32"	7'3-5/8"	140.6 K=0.96	1.090	-6.908	15.784	0.438 <sup>1</sup> ✓
T13	73.3333 - 66.6667	L 3x3x3/16	15'3-3/3 2"	7'7-3/8"	153.2 K=1.00	1.090	-7.765	13.284	0.585 <sup>1</sup> ✓
T14	66.6667 - 60	L 3x3x3/16	15'10-21 /32"	7'11-5/3 2"	159.6 K=1.00	1.090	-7.575	12.248	0.618 <sup>1</sup> ✓
T15	60 - 50	2L3x3x3/16x1/4	18'3-1/1 6"	9'5/16"	121.9 K=1.00	2.180	-8.757	37.533	0.233 <sup>1</sup> ✓
T16	50 - 40	2L 'a' > 51.562 in - 174 2L3x3x3/16x1/4	19'1-3/1 6"	9'6-7/8"	129.3 K=1.00	2.180	-9.702	33.927	0.286 <sup>1</sup> ✓
T17	40 - 30	2L 'a' > 54.686 in - 183 2L3x3x1/4x1/4	19'11-23 /32"	10'1/4"	135.4 K=1.00	2.875	-9.855	43.094	0.229 <sup>1</sup> ✓
T18	30 - 20	2L 'a' > 57.429 in - 195 2L3x3x1/4x1/4	10'8-1/1 6"	10'1-29/ 32"	105.1 K=1.00	2.875	-10.695	68.991	0.155 <sup>1</sup> ✓
T19	20 - 0	2L3 1/2x3 1/2x1/4x1/4  2L 'a' > 63.481 in - 249	22'8-1/4" '	11'1-3/1 6"	129.0 K=1.00	3.375	-10.871	54.239	0.200 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T18	30 - 20	L3x3x3/16	18'4"	8'9-25/3 2"	225.3 K=1.00	1.090	-3.912	6.146	0.637 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Secondary Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T8	106.667 - 100	L1 3/4x1 3/4x1/4	10'3-7/1 6"	4'11-31/ 32"	175.7 K=1.00	0.813	-1.590	7.536	0.211 <sup>1</sup> ✓
T11	86.6667 - 80	L2x2x3/16	12'3-15/ 16"	5'11-31/ 32"	182.7 K=1.00	0.715	-2.229	6.134	0.363 <sup>1</sup> ✓
T13	73.3333 -	L1 3/4x1 3/4x1/4	13'8-19/ 6"	6'8-1/32'	234.5	0.813	-2.628	4.230	0.621 <sup>1</sup> ✓

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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T14	66.6667 - 60	L2x2x3/16	14'4-31/32"	7'7/32"	213.8 K=1.00	0.715	-2.825	4.478	0.631 <sup>1</sup> ✓
T16	50 - 40	L2 1/2x2 1/2x3/16	16'3-1/16"	7'10-3/4"	191.4 K=1.00	0.902	-3.351	7.046	0.476 <sup>1</sup> ✓
T17	40 - 30	L3x3x1/4	17'3-5/16"	8'4-7/8"	170.4 K=1.00	1.440	-3.638	14.195	0.256 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	160 - 140	L2x2x1/8	6'6-1/4"	6'1-3/8"	184.6 K=1.00	0.484	-0.622	4.070	0.153 <sup>1</sup> ✓
T2	140 - 135	L2x2x1/8	6'6-3/4"	6'1-3/8"	184.6 K=1.00	0.484	-0.440	4.070	0.108 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Horizontal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T18	30 - 20	L2x2x3/16	4'7"	4'1-7/32"	124.9 K=1.00	0.715	-3.912	13.110	0.298 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Diagonal (1) Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T18	30 - 20	L2x2x3/16	5'4-1/32"	4'9-27/32"	146.8 K=1.00	0.715	-2.277	9.495	0.240 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

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## Tension Checks

## Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	160 - 140	ROHN 2 STD	20'	4'	61.0	1.075	15.146	48.354	0.313 <sup>1</sup>
T2	140 - 135	ROHN 2.5 EH	5'3/32"	5'3/32"	65.0	2.254	20.426	101.409	0.201 <sup>1</sup>
T3	135 - 130	ROHN 2.5 EH	5'3/32"	5'3/32"	65.0	2.254	27.886	101.409	0.275 <sup>1</sup>
T4	130 - 125	ROHN 2.5 EH	5'3/32"	5'3/32"	65.0	2.254	35.857	101.409	0.354 <sup>1</sup>
T5	125 - 120	ROHN 2.5 EH	5'3/32"	5'3/32"	65.0	2.254	43.666	101.409	0.431 <sup>1</sup>
T6	120 - 113.333	ROHN 3 EH	6'8-1/8"	6'8-1/8"	70.5	3.016	53.239	135.717	0.392 <sup>1</sup>
T7	113.333 - 106.667	ROHN 3 EH	6'8-1/8"	6'8-1/8"	70.5	3.016	64.711	135.717	0.477 <sup>1</sup>
T8	106.667 - 100	ROHN 3 EH	6'8-1/8"	3'5-3/8"	36.4	3.016	76.591	135.717	0.564 <sup>1</sup>
T9	100 - 93.3333	ROHN 3.5 EH	6'8-1/8"	6'8-1/8"	61.3	3.678	88.417	165.529	0.534 <sup>1</sup>
T10	93.3333 - 86.6667	ROHN 3.5 EH	6'8-1/8"	6'8-1/8"	61.3	3.678	99.985	165.529	0.604 <sup>1</sup>
T11	86.6667 - 80	ROHN 3.5 EH	6'8-1/8"	3'2-31/32"	29.8	3.678	110.563	165.529	0.668 <sup>1</sup>
T12	80 - 73.3333	ROHN 4 X-STR	6'8-5/32"	6'8-5/32"	54.3	4.407	121.397	198.335	0.612 <sup>1</sup>
T13	73.3333 - 66.6667	ROHN 4 X-STR	6'8-5/32"	3'3-1/16'	26.4	4.407	131.275	198.335	0.662 <sup>1</sup>
T14	66.6667 - 60	ROHN 4 X-STR	6'8-5/32"	3'3-3/32"	26.5	4.407	141.349	198.335	0.713 <sup>1</sup>
T15	60 - 50	ROHN. 5 EH	10'7/32"	10'7/32"	65.4	6.112	153.724	275.039	0.559 <sup>1</sup>
T16	50 - 40	ROHN. 5 EH	10'7/32"	4'10-1/4'	31.7	6.112	168.039	275.039	0.611 <sup>1</sup>
T17	40 - 30	ROHN 5 X-STR	10'7/32"	4'10-9/32"	31.7	6.112	182.066	275.039	0.662 <sup>1</sup>
T18	30 - 20	ROHN 5 X-STR	10'7/32"	2'6-1/16'	16.3	6.112	194.903	275.039	0.709 <sup>1</sup>
T19	20 - 0	B+T_BU 806378 - 6.625"x0.34" pipe w/ 2" SR	20'13/32"	10'7/32"	75.6	9.855	221.605	443.471	0.500 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

## Diagonal Design Data (Tension)

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 44 of 47
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	<b>Client</b> Crown Castle	<b>Designed by</b> Jayaraj B

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	160 - 140	L1 3/4x1 3/4x3/16	7'8-3/16'	3'7-15/32"	83.3	0.378	3.228	16.440	0.196 <sup>1</sup>
T2	140 - 135	L1 3/4x1 3/4x3/16	8'5-15/32"	4'1-19/32"	94.7	0.378	3.048	16.440	0.185 <sup>1</sup>
T3	135 - 130	L1 3/4x1 3/4x3/16	8'10-15/32"	4'4-3/32"	99.4	0.378	4.261	16.440	0.259 <sup>1</sup>
T4	130 - 125	L1 3/4x1 3/4x3/16	9'3-19/32"	4'6-21/32"	104.1	0.378	4.231	16.440	0.257 <sup>1</sup>
T5	125 - 120	L2x2x3/16	9'8-25/32"	4'9-1/4"	94.8	0.448	4.849	19.504	0.249 <sup>1</sup>
T6	120 - 113.333	L2 1/2x2 1/2x1/4	11'1-7/8"	5'6-1/32"	87.5	0.775	5.327	33.726	0.158 <sup>1</sup>
T7	113.333 - 106.667	L2 1/2x2 1/2x1/4	11'8-15/32"	5'9-5/16"	91.8	0.775	6.541	33.726	0.194 <sup>1</sup>
T8	106.667 - 100	L2 1/2x2 1/2x1/4	12'3-7/32"	6'1-15/16"	96.2	0.775	6.617	33.726	0.196 <sup>1</sup>
T9	100 - 93.3333	L2 1/2x2 1/2x3/16	12'10-1/8"	6'5-3/32"	99.1	0.902	6.563	29.225	0.225 <sup>1</sup>
T10	93.3333 - 86.6667	L2 1/2x2 1/2x1/4	13'5-5/32"	6'8-5/8"	104.9	1.190	6.762	38.556	0.175 <sup>1</sup>
T11	86.6667 - 80	2L2 1/2x2 1/2x3/16x1/4	14'5/16"	7'3/16"	108.2	1.805	6.875	58.472	0.118 <sup>1</sup>
T12	80 - 73.3333	2L 'a' > 40.153 in - 129 L 3x3x3/16	14'7-19/32"	7'3-5/8"	93.3	1.090	6.977	35.311	0.198 <sup>1</sup>
T13	73.3333 - 66.6667	L 3x3x3/16	15'3-3/32"	7'7-3/8"	97.3	1.090	7.149	35.311	0.202 <sup>1</sup>
T14	66.6667 - 60	L 3x3x3/16	15'10-21/32"	7'11-5/32"	101.3	1.090	7.025	35.311	0.199 <sup>1</sup>
T15	60 - 50	2L3x3x3/16x1/4	18'3-1/16"	9'5/16"	116.9	1.424	8.609	61.937	0.139 <sup>1</sup>
T16	50 - 40	2L 'a' > 51.562 in - 175 2L3x3x3/16x1/4	19'1-3/16"	9'6-7/8"	122.3	1.424	8.762	61.937	0.141 <sup>1</sup>
T17	40 - 30	2L 'a' > 54.686 in - 183 2L3x3x1/4x1/4	19'11-23/32"	10'1/4"	129.3	1.875	8.804	91.406	0.096 <sup>1</sup>
T18	30 - 20	2L 'a' > 57.429 in - 195 2L3x3x1/4x1/4	10'8-1/16"	10'1-29/32"	93.1	1.875	9.512	91.406	0.104 <sup>1</sup>
T19	20 - 0	2L3 1/2x3 1/2x1/4x1/4 2L 'a' > 60.930 in - 256	21'9-15/32"	10'7-13/16"	118.5	2.250	10.094	109.688	0.092 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Tension)

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b> 136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b> 45 of 47
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Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T18	30 - 20	L3x3x3/16	18'4"	8'9-25/32"	171.3	0.712	3.912	30.973	0.126 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Secondary Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T8	106.667 - 100	L1 3/4x1 3/4x1/4	10'3-7/16"	4'11-31/32"	226.8	0.469	1.590	20.391	0.078 <sup>1</sup> ✓
T11	86.6667 - 80	L2x2x3/16	12'3-15/16"	5'11-31/32"	233.3	0.431	2.229	18.739	0.119 <sup>1</sup> ✓
T13	73.3333 - 66.6667	L1 3/4x1 3/4x1/4	13'8-19/32"	6'8-1/32"	302.7	0.469	2.628	20.391	0.129 <sup>1</sup> ✓
T14	66.6667 - 60	L2x2x3/16	14'4-31/32"	7'7/32"	273.0	0.431	2.825	18.739	0.151 <sup>1</sup> ✓
T16	50 - 40	L2 1/2x2 1/2x3/16	16'3-1/16"	7'10-3/4"	243.6	0.571	3.351	24.840	0.135 <sup>1</sup> ✓
T17	40 - 30	L3x3x1/4	17'3-5/16"	8'4-7/8"	216.9	0.939	3.638	40.863	0.089 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	160 - 140	L2x2x1/8	6'6-1/4"	6'1-3/8"	121.2	0.305	0.623	13.254	0.047 <sup>1</sup> ✓
T2	140 - 135	L2x2x1/8	6'6-3/4"	6'1-3/8"	121.2	0.305	0.440	13.254	0.033 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Horizontal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T18	30 - 20	L2x2x3/16	4'7"	4'1-7/32"	84.6	0.431	3.912	18.739	0.209 <sup>1</sup>

<b>tnxTower</b>  <b>B+T Group</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	<b>Job</b>	136290.007.01 - HRT 086 943248, CT (BU# 806378)	<b>Page</b>	46 of 47
	<b>Project</b>		<b>Date</b>	12:01:41 04/05/22
	<b>Client</b>	Crown Castle	<b>Designed by</b>	Jayaraj B

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
									✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Redundant Diagonal (1) Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T18	30 - 20	L2x2x3/16	5'4-1/32'	4'9-27/32"	98.6	0.431	2.277	18.739	0.122 <sup>1</sup>
									✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	φP <sub>allow</sub> K	% Capacity	Pass Fail
T1	160 - 140	Leg	ROHN 2 STD	2	-19.913	38.684	51.5	Pass
T2	140 - 135	Leg	ROHN 2.5 EH	38	-25.397	78.149	32.5	Pass
T3	135 - 130	Leg	ROHN 2.5 EH	50	-35.767	78.149	45.8	Pass
T4	130 - 125	Leg	ROHN 2.5 EH	59	-44.078	78.149	56.4	Pass
T5	125 - 120	Leg	ROHN 2.5 EH	68	-53.774	78.149	68.8	Pass
T6	120 - 113.333	Leg	ROHN 3 EH	77	-63.946	99.059	64.6	Pass
T7	113.333 - 106.667	Leg	ROHN 3 EH	86	-77.956	99.059	78.7	Pass
T8	106.667 - 100	Leg	ROHN 3 EH	95	-91.616	129.331	70.8	Pass
T9	100 - 93.3333	Leg	ROHN 3.5 EH	107	-104.423	132.012	79.1	Pass
T10	93.3333 - 86.6667	Leg	ROHN 3.5 EH	116	-116.891	132.011	88.5	Pass
T11	86.6667 - 80	Leg	ROHN 3.5 EH	125	-128.480	161.634	79.5	Pass
T12	80 - 73.3333	Leg	ROHN 4 X-STR	137	-140.636	167.898	83.8	Pass
T13	73.3333 - 66.6667	Leg	ROHN 4 X-STR	146	-151.443	196.788	77.0	Pass
T14	66.6667 - 60	Leg	ROHN 4 X-STR	158	-162.824	196.814	82.7	Pass
T15	60 - 50	Leg	ROHN. 5 EH	170	-176.946	211.314	83.7	Pass
T16	50 - 40	Leg	ROHN. 5 EH	179	-193.191	265.798	72.7	Pass
T17	40 - 30	Leg	ROHN 5 X-STR	191	-209.685	265.818	78.9	Pass
T18	30 - 20	Leg	ROHN 5 X-STR	203	-225.572	283.206	79.6	Pass
T19	20 - 0	Leg	B+T_BU 806378 - 6.625"x0.34" pipe w/ 2" SR	245	-258.236	306.709	84.2	Pass
T1	160 - 140	Diagonal	L1 3/4x1 3/4x3/16	9	-3.291	11.646	28.3	Pass
T2	140 - 135	Diagonal	L1 3/4x1 3/4x3/16	45	-3.312	8.949	37.0	Pass
T3	135 - 130	Diagonal	L1 3/4x1 3/4x3/16	54	-4.219	8.112	52.0	Pass
T4	130 - 125	Diagonal	L1 3/4x1 3/4x3/16	63	-4.298	7.371	58.3	Pass
T5	125 - 120	Diagonal	L2x2x3/16	72	-4.867	10.175	47.8	Pass
T6	120 - 113.333	Diagonal	L2 1/2x2 1/2x1/4	81	-5.423	19.771	27.4	Pass
T7	113.333 - 106.667	Diagonal	L2 1/2x2 1/2x1/4	90	-6.800	17.939	37.9	Pass
T8	106.667 - 100	Diagonal	L2 1/2x2 1/2x1/4	99	-6.753	15.768	42.8	Pass
T9	100 - 93.3333	Diagonal	L2 1/2x2 1/2x3/16	111	-6.763	12.495	54.1	Pass

**tnxTower**

**B+T Group**  
 1717 S. Boulder, Suite 300  
 Tulsa, OK 74119  
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**Job**  
 136290.007.01 - HRT 086 943248, CT (BU# 806378)

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**Project**  
 Date 12:01:41 04/05/22

**Client**  
 Crown Castle  
 Designed by Jayaraj B

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
T10	93.3333 - 86.6667	Diagonal	L2 1/2x2 1/2x1/4	120	-6.774	15.132	44.8	Pass
T11	86.6667 - 80	Diagonal	2L2 1/2x2 1/2x3/16x1/4	129	-7.420	38.394	19.3	Pass
T12	80 - 73.3333	Diagonal	L 3x3x3/16	141	-6.908	16.573	41.7	Pass
T13	73.3333 - 66.6667	Diagonal	L 3x3x3/16	150	-7.765	13.948	55.7	Pass
T14	66.6667 - 60	Diagonal	L 3x3x3/16	162	-7.575	12.860	58.9	Pass
T15	60 - 50	Diagonal	2L3x3x3/16x1/4	174	-8.757	39.409	22.2	Pass
T16	50 - 40	Diagonal	2L3x3x3/16x1/4	183	-9.702	35.624	27.2	Pass
T17	40 - 30	Diagonal	2L3x3x1/4x1/4	195	-9.855	45.249	21.8	Pass
T18	30 - 20	Diagonal	2L3x3x1/4x1/4	213	-10.695	72.440	14.8	Pass
T19	20 - 0	Diagonal	2L3 1/2x3 1/2x1/4x1/4	249	-10.871	56.951	19.1	Pass
T18	30 - 20	Horizontal	L3x3x3/16	205	-3.912	6.453	60.6	Pass
T8	106.667 - 100	Secondary Horizontal	L1 3/4x1 3/4x1/4	103	-1.590	7.913	20.1	Pass
T11	86.6667 - 80	Secondary Horizontal	L2x2x3/16	133	-2.229	6.440	34.6	Pass
T13	73.3333 - 66.6667	Secondary Horizontal	L1 3/4x1 3/4x1/4	154	-2.628	4.441	59.2	Pass
T14	66.6667 - 60	Secondary Horizontal	L2x2x3/16	166	-2.825	4.702	60.1	Pass
T16	50 - 40	Secondary Horizontal	L2 1/2x2 1/2x3/16	187	-3.351	7.398	45.3	Pass
T17	40 - 30	Secondary Horizontal	L3x3x1/4	199	-3.638	14.905	24.4	Pass
T1	160 - 140	Top Girt	L2x2x1/8	5	-0.622	4.273	14.6	Pass
T2	140 - 135	Top Girt	L2x2x1/8	40	-0.440	4.273	10.3	Pass
T18	30 - 20	Redund Horz 1 Bracing	L2x2x3/16	233	-3.912	13.765	28.4	Pass
T18	30 - 20	Redund Diag 1 Bracing	L2x2x3/16	231	-2.277	9.970	22.8	Pass
						Summary		
						Leg (T10)	88.5	Pass
						Diagonal (T14)	58.9	Pass
						Horizontal (T18)	60.6	Pass
						Secondary Horizontal (T14)	60.1	Pass
						Top Girt (T1)	14.6	Pass
						Redund Horz 1 Bracing (T18)	28.4	Pass
						Redund Diag 1 Bracing (T18)	22.8	Pass
						Bolt Checks	84.0	Pass
						<b>RATING =</b>	<b>88.5</b>	<b>Pass</b>

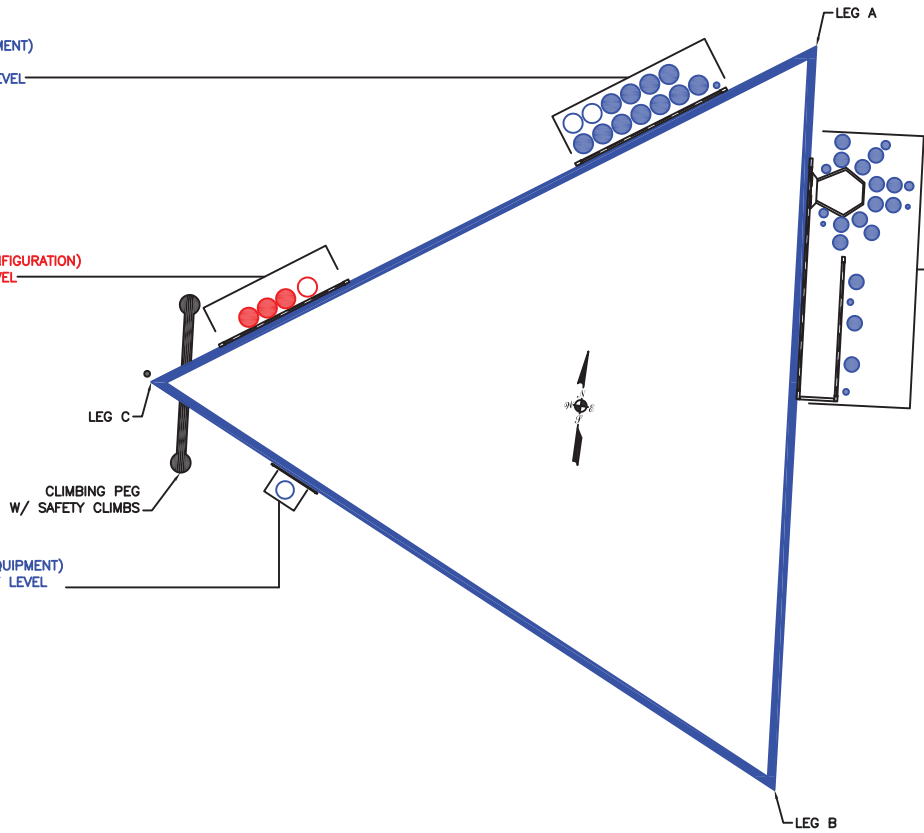


**APPENDIX B**  
**BASE LEVEL DRAWING**

(OTHER CONSIDERED EQUIPMENT)  
(1) 1/2" TO 57 FT LEVEL  
(13) 1-5/8" TO 157 FT LEVEL

(PROPOSED EQUIPMENT CONFIGURATION)  
(4) 1-5/8" TO 113 FT LEVEL

(OTHER CONSIDERED EQUIPMENT)  
(1) 1/2" TO 48 FT LEVEL  
(1) 5/8" TO 125 FT LEVEL  
(3) 1-1/4" TO 125 FT LEVEL  
(OTHER CONSIDERED EQUIPMENT)  
(2) 3/8" TO 135 FT LEVEL  
(4) 3/4" TO 135 FT LEVEL  
(12) 1-1/4" TO 135 FT LEVEL



BUSINESS UNIT: 806378

**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

PROJECT 136290.007.01 - HRT 086 943248, CT  
 SUBJECT Reinforced Tower Legs  
 DATE 04/05/22  
 v3.4.2



Tower Information	
TIA-222 Rev.	H
Apply TIA-222-H Section 15.5	Yes

Calculation Type	Original Member		Modification					Section Geometry													Leg Capacity					Results									
	Elevation (ft)	Leg Type	Type	Analysis Method	Intermediate Connection	Leg Crushing?	BP & Angle?	Custom Area Input			Custom MOIxx Input			Custom MOIyy Input			Leg Comp. Load P <sub>y</sub> (k)	Leg Fy (ksi)	Reinf. Fy (ksi)	L (in)	a (in)	K Leg	K Mod	K Comp.	Custom h (in)		Fe (ksi)	Fcr (ksi)	φ	Leg Crushing Capacity (k)	Reinf. Tension Yield Capacity (k)	Reinf. Leg Comp. Capacity (k)	Original leg Capacity (k)	Spacing Req.	Leg Load Final Rating
								Area <sub>1</sub> (in <sup>2</sup> )	Area <sub>2</sub> (in <sup>2</sup> )	Area <sub>3</sub> (in <sup>2</sup> )	I <sub>1</sub> (in <sup>4</sup> )	I <sub>2</sub> (in <sup>4</sup> )	I <sub>3</sub> (in <sup>4</sup> )	I <sub>1</sub> (in <sup>4</sup> )	I <sub>2</sub> (in <sup>4</sup> )	I <sub>3</sub> (in <sup>4</sup> )																			
Analysis	0-20	Custom	Custom	Built Up	Pinned	No	No	6.71	1.14	9.85	33.2	0.8	45.5	33.2	0.8	45.5	258.24	50	50	120.2	19.5	1.0	1.00	1.0		61.5122	35.6	0.90	413.5	315.6	244.0	O.K. @ 0.7	77.9%	Passing	

PROJECT	136290.007.01 - HRT 086 943248, CT				
SUBJECT	Bolted Angle Connection Analysis				
DATE	04/05/22	PAGE	1	OF	1

v2.5.0



TIA-222 Rev. H  
 Apply TIA-222-H Section 15.5? Yes

**Max Rating** 81.6%

Elevation (ft)	Component	Angle			Bolt				Coping Dimensions (in)						Tens. Load (k)	Comp. Load (k)	Tens. Capacity (k)	Comp. Capacity (k)	Rating	Limit State				
		Qty	Size	Grade	Qty	Size	Grade	Edge Dist. (in)	Gage (in)	Pitch (in)	Coping	A	B	C							D	E		
1 60-80	Diagonal	1	L3X3X(3/16+3/16)	A36	1	1/2	A325N	0.75	1.5		Allowable								7.15	7.57	8.84	8.84	81.6%	Compression - Bolt Shear
2 80-86.7	Diagonal	2	2 1/2X2 1/2X(3/16+3/16)	A36	1	1/2	A325N	0.75	1.25		Allowable								6.88	7.42	17.67	17.67	40.0%	Compression - Bolt Shear
3 86.7-93.4	Diagonal	1	L2 1/2X2 1/2X(1/4+3/16)	A36	1	1/2	A325N	0.75	1.25		Allowable								6.76	6.77	8.84	8.84	73.0%	Compression - Bolt Shear
4 93.4-100	Diagonal	1	2 1/2X2 1/2X(3/16+3/16)	A36	1	1/2	A325N	0.75	1.25		Allowable								6.56	6.76	8.84	8.84	72.9%	Compression - Bolt Shear

# Self Support Anchor Rod Capacity

Site Info	
BU #	806378
Site Name	HRT 086 943248, CT
Order #	609062, Rev# 0

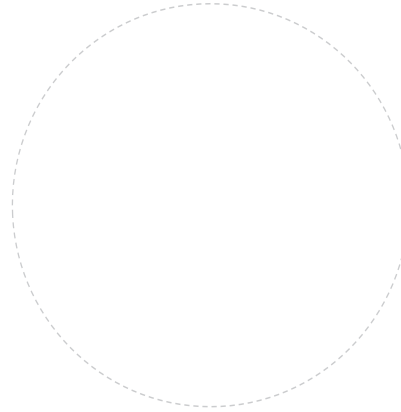
Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	Yes
$l_{ar}$ (in)	0

Applied Loads		
	Comp.	Uplift
Axial Force (kips)	267.00	228.00
Shear Force (kips)	30.00	26.00

\*TIA-222-H Section 15.5 Applied

Considered Eccentricity	
Leg Mod Eccentricity (in)	0.000
Anchor Rod N.A Shift (in)	0.000
Total Eccentricity (in)	0.000

\*Anchor Rod Eccentricity Applied



Connection Properties	Analysis Results
-----------------------	------------------

Anchor Rod Data
(6) 1" $\phi$ bolts (A449 N; Fy=92 ksi, Fu=120 ksi)
$l_{ar}$ (in): 0

Anchor Rod Summary		(units of kips, kip-in)
$Pu_t = 38$	$\phi Pn_t = 54.54$	<b>Stress Rating</b>
$Vu = 4.33$	$\phi Vn = 35.34$	<b>66.4%</b>
$Mu = n/a$	$\phi Mn = n/a$	<b>Pass</b>

# Pier and Pad Foundation



BU #: 806378  
 Site Name: HRT 086 943248, C  
 App. Number: 609062, Rev# 0

TIA-222 Revision: H  
 Tower Type: Self Support

Top & Bot. Pad Rein. Different?:   
 Block Foundation?:   
 Rectangular Pad?:

Superstructure Analysis Reactions		
Compression, $P_{comp}$ :	267	kips
Compression Shear, $V_{u,comp}$ :	30	kips
Uplift, $P_{uplift}$ :	228	kips
Uplift Shear, $V_{u,uplift}$ :	26	kips
Tower Height, $H$ :	160	ft
Base Face Width, $BW$ :	20.86	ft
BP Dist. Above Fdn, $bp_{dist}$ :	3	in

Foundation Analysis Checks				
	Capacity	Demand	Rating*	Check
<i>Uplift (kips)</i>	298.70	228.00	72.7%	Pass
<i>Lateral (Sliding) (kips)</i>	114.59	26.00	21.6%	Pass
<i>Bearing Pressure (ksf)</i>	12.38	4.37	33.6%	Pass
<i>Pier Flexure (Comp.) (kip*ft)</i>	1318.44	315.00	22.8%	Pass
<i>Pier Flexure (Tension) (kip*ft)</i>	879.83	273.00	29.6%	Pass
<i>Pier Compression (kip)</i>	2315.08	285.18	11.7%	Pass
<i>Pad Flexure (kip*ft)</i>	565.50	143.49	24.2%	Pass
<i>Pad Shear - 1-way (kips)</i>	194.10	43.73	21.5%	Pass
<i>Pad Shear - 2-way (Comp) (ksi)</i>	0.164	0.059	34.0%	Pass
<i>Flexural 2-way (Comp) (kip*ft)</i>	1037.39	189.00	17.4%	Pass
<i>Pad Shear - 2-way (Uplift) (ksi)</i>	0.164	0.071	41.4%	Pass
<i>Flexural 2-way (Tension) (kip*ft)</i>	1037.39	163.80	15.0%	Pass

\*Rating per TIA-222-H Section 15.5

Structural Rating*:	41.4%
Soil Rating*:	72.7%

Pier Properties		
Pier Shape:	Circular	
Pier Diameter, $dpier$ :	3.5	ft
Ext. Above Grade, $E$ :	0.5	ft
Pier Rebar Size, $Sc$ :	9	
Pier Rebar Quantity, $mc$ :	16	
Pier Tie/Spiral Size, $St$ :	3	
Pier Tie/Spiral Quantity, $mt$ :	12	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, $cc_{pier}$ :	3	in

Pad Properties		
Depth, $D$ :	12	ft
Pad Width, $W_1$ :	10	ft
Pad Thickness, $T$ :	2	ft
Pad Rebar Size (Bottom dir. 2), $Sp_2$ :	7	
Pad Rebar Quantity (Bottom dir. 2), $mp_2$ :	11	
Pad Clear Cover, $cc_{pad}$ :	3	in

Material Properties		
Rebar Grade, $F_y$ :	60	ksi
Concrete Compressive Strength, $F'_c$ :	3	ksi
Dry Concrete Density, $\delta_c$ :	150	pcf

Soil Properties		
Total Soil Unit Weight, $\gamma$ :	117	pcf
Ultimate Gross Bearing, $Q_{ult}$ :	16.500	ksf
Cohesion, $C_u$ :	0.000	ksf
Friction Angle, $\phi$ :	34	degrees
SPT Blow Count, $N_{blows}$ :		
Base Friction, $\mu$ :	0.3	
Neglected Depth, $N$ :	3.33	ft
Foundation Bearing on Rock?	No	
Groundwater Depth, $gw$ :	10.5	ft

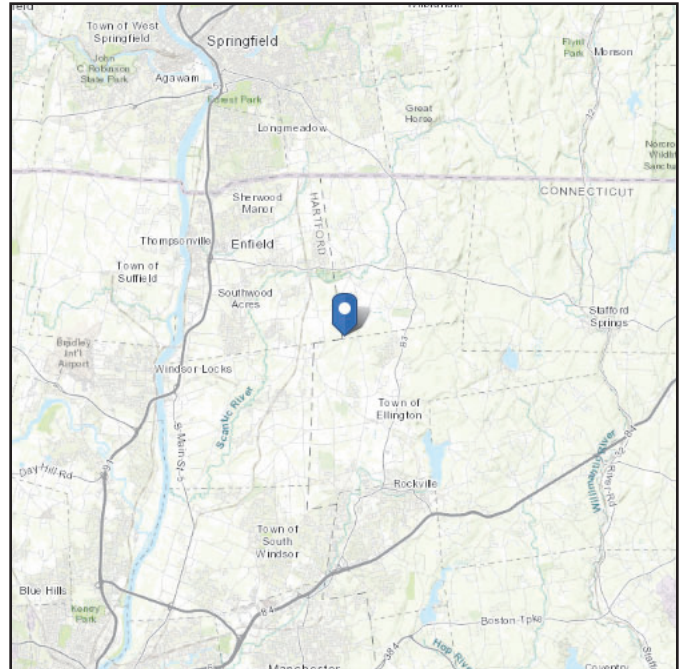
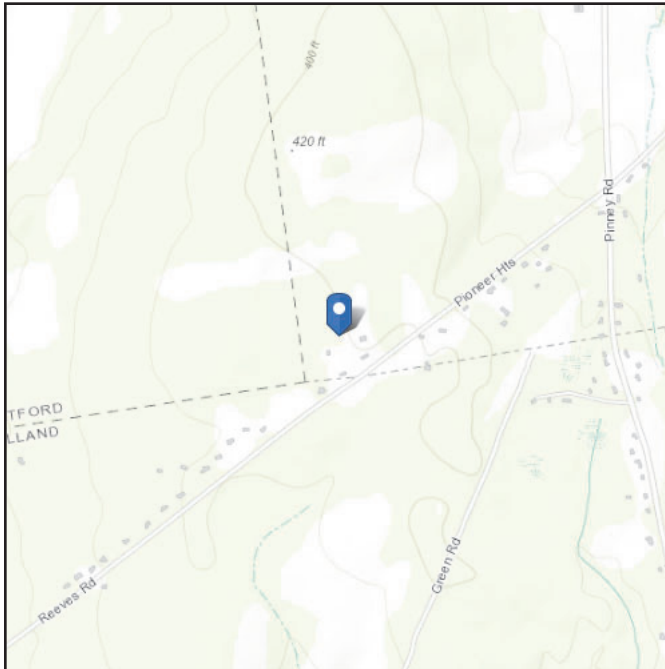
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# ASCE 7 Hazards Report

**Address:**  
No Address at This  
Location

**Standard:** ASCE/SEI 7-16  
**Risk Category:** II  
**Soil Class:** D - Default (see  
Section 11.4.3)

**Elevation:** 396.21 ft (NAVD 88)  
**Latitude:** 41.948883  
**Longitude:** -72.492097



## Wind

### Results:

Wind Speed	117 Vmph
10-year MRI	75 Vmph
25-year MRI	83 Vmph
50-year MRI	90 Vmph
100-year MRI	97 Vmph

Data Source: ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2  
Date Accessed: Tue Apr 05 2022

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings need not be protected against wind-borne debris.

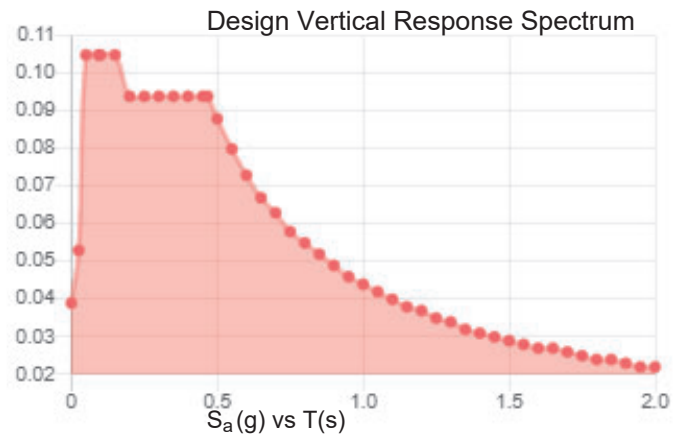
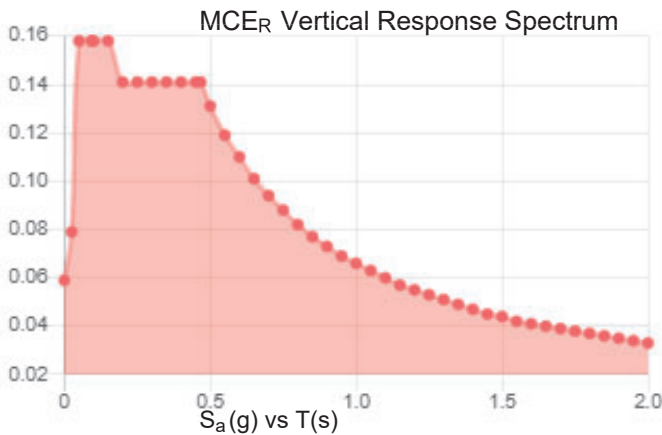
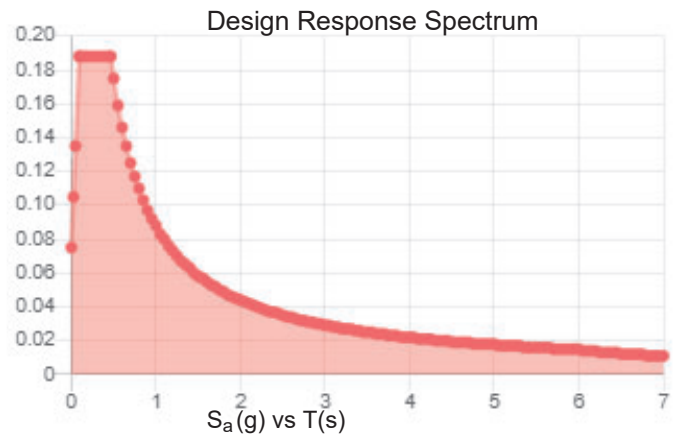
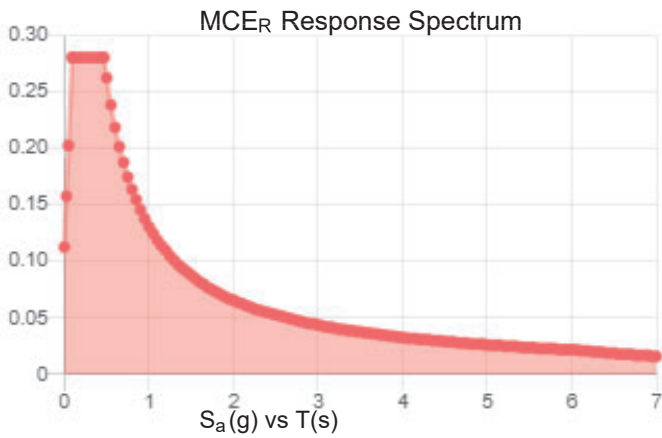


**Site Soil Class:** D - Default (see Section 11.4.3)

**Results:**

$S_s$ :	0.176	$S_{D1}$ :	0.088
$S_1$ :	0.055	$T_L$ :	6
$F_a$ :	1.6	PGA :	0.093
$F_v$ :	2.4	PGA <sub>M</sub> :	0.148
$S_{MS}$ :	0.281	$F_{PGA}$ :	1.6
$S_{M1}$ :	0.131	$I_e$ :	1
$S_{DS}$ :	0.188	$C_v$ :	0.7

**Seismic Design Category** B



**Data Accessed:** Tue Apr 05 2022

**Date Source:**

USGS Seismic Design Maps based on ASCE/SEI 7-16 and ASCE/SEI 7-16 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-16 Ch. 21 are available from USGS.

## Ice

---

**Results:**

Ice Thickness: 1.50 in.  
Concurrent Temperature: 5 F  
Gust Speed 50 mph

**Data Source:** Standard ASCE/SEI 7-16, Figs. 10-2 through 10-8

**Date Accessed:** Tue Apr 05 2022

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 500-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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Date: April 1, 2022



USA Engineering, Inc.  
2818 Cypress Ridge Blvd., Suite 110  
Wesley Chapel, FL 33544  
(813) 994-0365

**Subject:** Mount Analysis Report

**Carrier Designation:** T-Mobile Anchor\_Phase 3  
**Carrier Site Number:** CTHA534A  
**Carrier Site Name:** Crown Somers Lattice Tower

**Crown Castle Designation:** Crown Castle BU Number: 806378  
Crown Castle Site Name: HRT 086 943248  
Crown Castle JDE Job Number: 709736  
Crown Castle Order Number: 609062, Rev. 0

**Engineering Firm Designation:** USA Engineering Designation: 22-00394

**Site Data:** 126 PIONEER HEIGHTS RD, SOMERS, TOLLAND COUNTY, CT, 06071  
Latitude 41° 56' 55.98" Longitude -72° 29' 31.55"

**Structure Information:** Tower Height & Type: 161.4 ft Self Support  
Mount Elevation: 113 ft  
Mount Type: 12.5 ft V-Frame Mount

USA Engineering is pleased to submit this "Mount Analysis Report" to determine the structural integrity of T-Mobile's antenna mounting system with the proposed appurtenance and equipment addition on the above mentioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

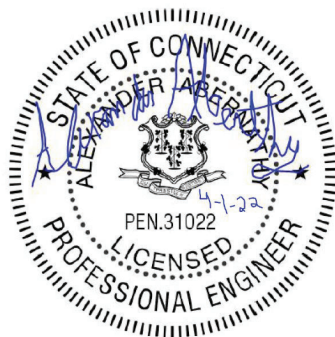
The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

**V-Frame Mount (typical)**

**Sufficient**

This analysis utilizes an ultimate 3-second gust wind speed of 117 mph as required by the 2015 International Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria."

Mount structural analysis prepared by: Nicole Hoffman, EI  
Respectfully Submitted by:



Alexander Abernathy, PE  
CT PE No: PEN.0031022

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- 2) **ANALYSIS CRITERIA**
  - Table 1 - Final Equipment Configuration
- 3) **ANALYSIS PROCEDURE**
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  - 3.1) Analysis Method
  - 3.2) Assumptions
- 4) **ANALYSIS RESULTS**
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  - Table 4 - Tieback Connection Data Table
  - 4.1) Recommendations
- 5) **APPENDIX A**
  - Wire Frame and Rendered Models
- 6) **APPENDIX B**
  - Software Input Calculations
- 7) **APPENDIX C**
  - Software Analysis Output
- 8) **APPENDIX D**
  - Additional Calculations

## 1) INTRODUCTION

This is an existing 3-sector 12.5' V-Frame Mount, designed by Site Pro.

## 2) ANALYSIS CRITERIA

<b>Building Code:</b>	2015 International Building Code
<b>TIA-222 Revision:</b>	TIA-222-H
<b>Risk Category:</b>	II
<b>Ultimate Wind Speed:</b>	117 mph
<b>Exposure Category:</b>	C
<b>Ice Thickness:</b>	1.5 in
<b>Wind Speed with Ice:</b>	30 mph
<b>Seismic S<sub>s</sub>:</b>	0.176
<b>Seismic S<sub>1</sub>:</b>	0.055
<b>Live Loading Wind Speed:</b>	30 mph
<b>Man Live Load at Mid/End-Points:</b>	250 lb
<b>Man Live Load at Mount Pipes:</b>	250 lb

**Table 1 - Final Equipment Configuration**

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details
113	114	3	COMMSCOPE	VV-65A-R1_TMO	12.5' V-Frame Mount (Site-Pro #VFA12-HD)
		3	ERICSSON	AIR 6419 B41_TMO	
		3	RFS/CELWAVE	APXVAALL24_43-U-NA20_TMO	
		3	ERICSSON	RADIO 4449 B71 B85A_T-MOBILE	
		3	ERICSSON	RADIO 4460 B2/B25 B66_TMO	

## 3) ANALYSIS PROCEDURE

**Table 2 - Documents Provided**

Document	Remarks	Reference	Source
Crown App	Order Number 609062, Rev. 0	Dated 03/28/22	CCI Site
Structural Analysis Report	B+T Group	Dated 10/05/21	CCI Site
Antenna Mount Pictures	-	Dated 05/16/21	CCI Site

### 3.1) Analysis Method

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed, using Microsoft Excel, by USA Engineering was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Mount Analysis* (Revision E).

### 3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) Steel grades have been assumed as follows, unless noted otherwise:
 

Channel, Solid Round, Angle, Plate	ASTM A36 (GR 36)
HSS (Rectangular)	ASTM 500 (GR B-46)
Pipe	ASTM A53 (GR 35)
Connection Bolts	ASTM A325
- 5) The structure and all components are assumed to be plumb, in good condition; and free from deformation, rust, corrosion, and cracks.
- 6) The supporting documents and drawings provided to *USA Engineering, Inc.* resemble the actual field conditions.

This analysis may be affected if any assumptions are not valid or have been made in error. USA Engineering should be notified to determine the effect on the structural integrity of the antenna mounting system.

### 4) ANALYSIS RESULTS

**Table 3 - Mount Component Stresses vs. Capacity (V-Frame Mount)**

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
1,3	Solid Rod	M27	113	83.1	Pass
	Antenna Pipe	POSITION_2	113	59.3	Pass
2,3	Tower Connection	-	113	16.3	Pass

<b>Structure Rating (max from all components) =</b>	<b>83.1%</b>
---	--------------

Notes:

- 1) See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed
- 2) See additional documentation in "Appendix D - Additional Calculations" supporting the % capacity consumed
- 3) All sectors are typical

**Table 4 - Tieback Connection Data Table**

Tower Connection Node No.	Existing / Proposed	Resultant End Reaction (lb)	Connected Member Type	Connected Member Size	Member Compressive Capacity (lb) <sup>3</sup>	Notes
N62	Existing	515	Tower Leg	ROHN 3 EH	1415	2
N61	Existing	634				

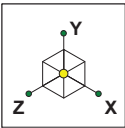
Notes:

- 1) Tieback connection point is within 25% of either end of the connected tower member
- 2) Tieback connection point is NOT within 25% of either end of the connected tower member
- 3) Reduced member compressive capacity according to CED-STD-10294 *Standard for Installation of Mounts and Appurtenances*

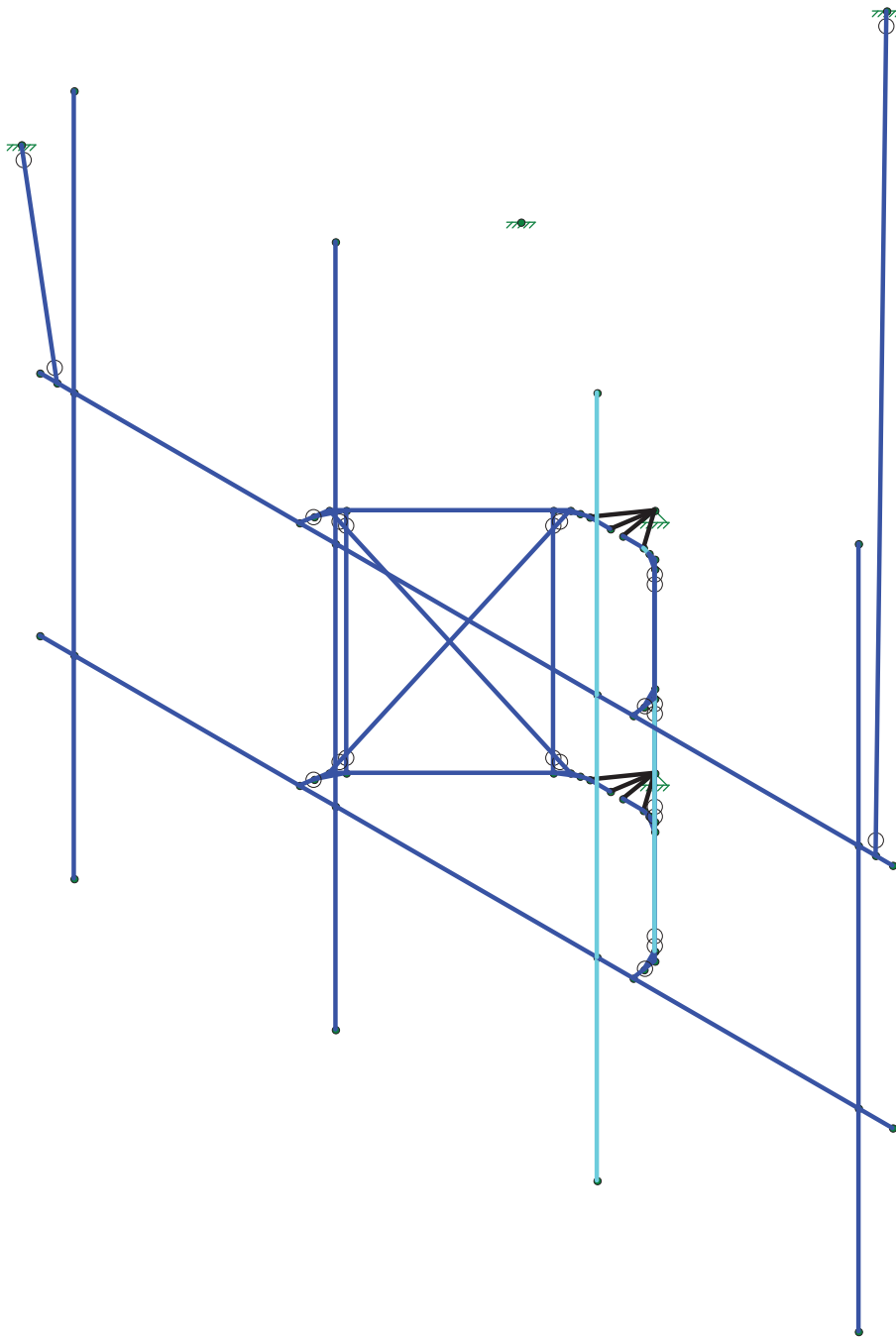
### 4.1) Recommendations

The mount has sufficient capacity to carry the proposed loading configuration. No modifications are required at this time.

**APPENDIX A**  
**WIRE FRAME AND RENDERED MODELS**



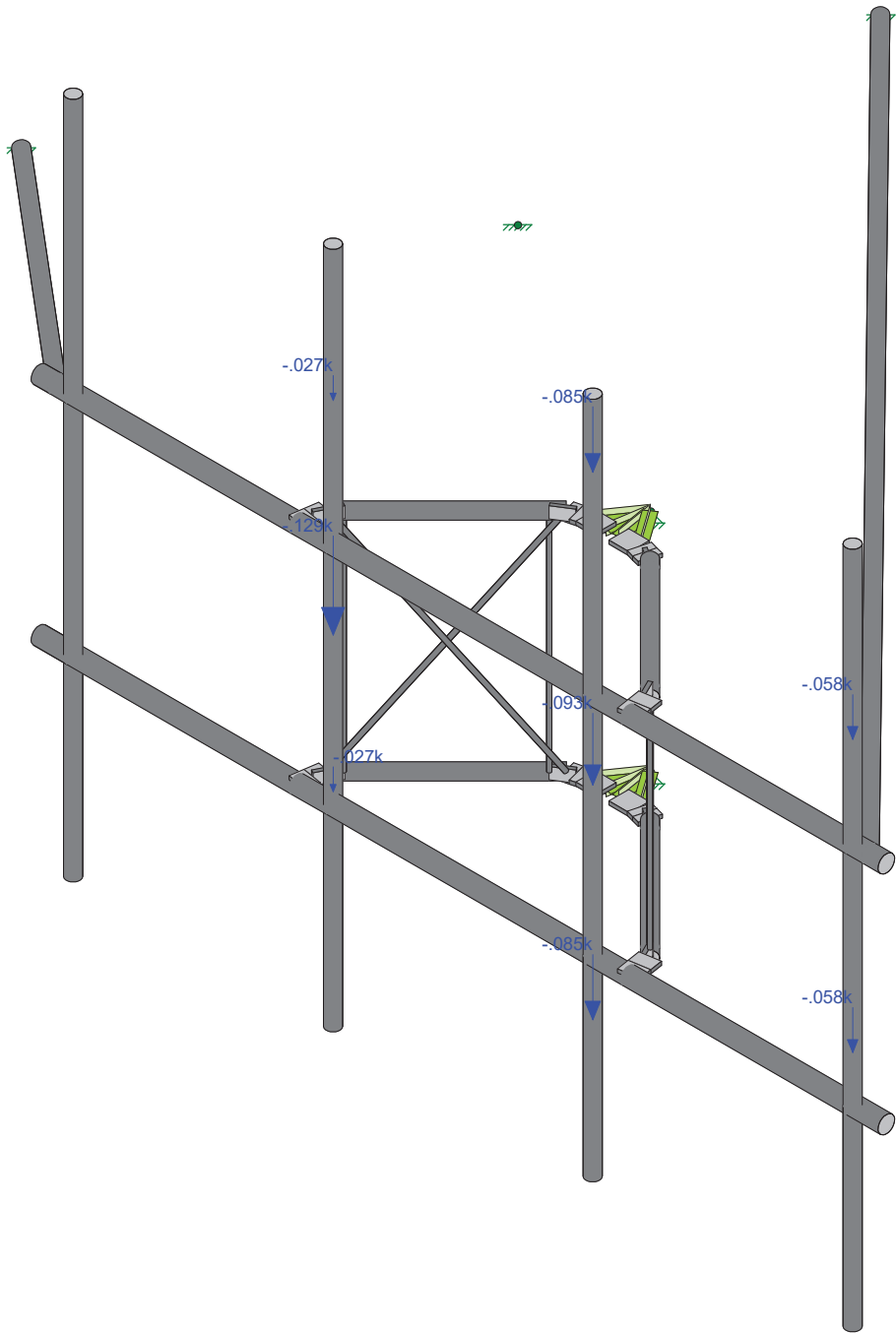
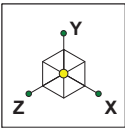
Code Check ( Env )	
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Red	> 1.0
Pink	.90-1.0
Green	.75-.90
Light Blue	.50-.75
Dark Blue	0-.50



Envelope Only Solution

USA Engineering, Inc.	Site-Pro #VFA12-HD	SK - 1
Nicole Hoffman, EI		Mar 31, 2022 at 9:11 AM
		Site-Pro #VFA12-HD.r3d





Loads: BLC 1, DEAD LOAD  
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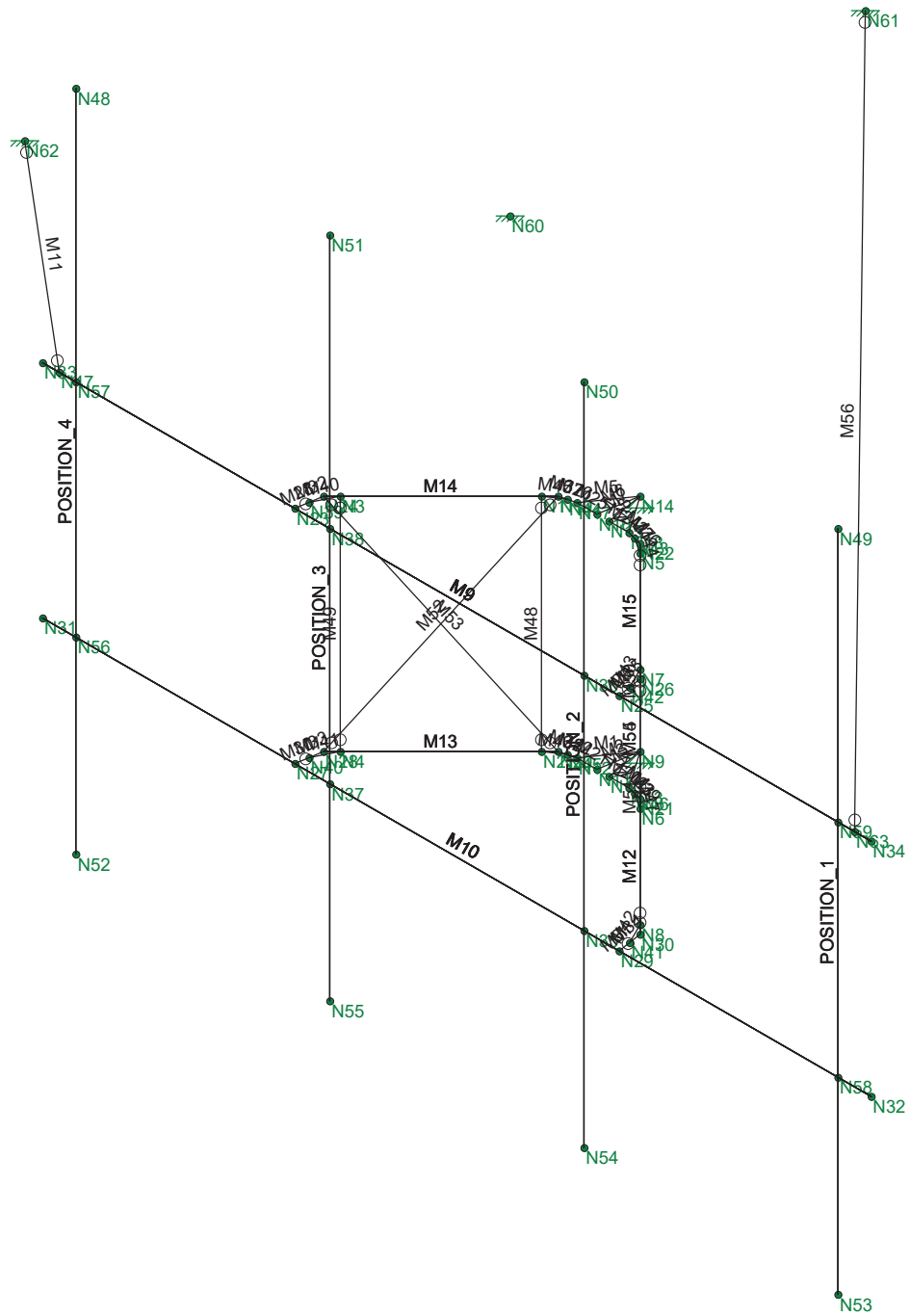
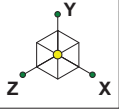
Nicole Hoffman, EI

Site-Pro #VFA12-HD

SK - 2

Mar 31, 2022 at 9:11 AM

Site-Pro #VFA12-HD.r3d



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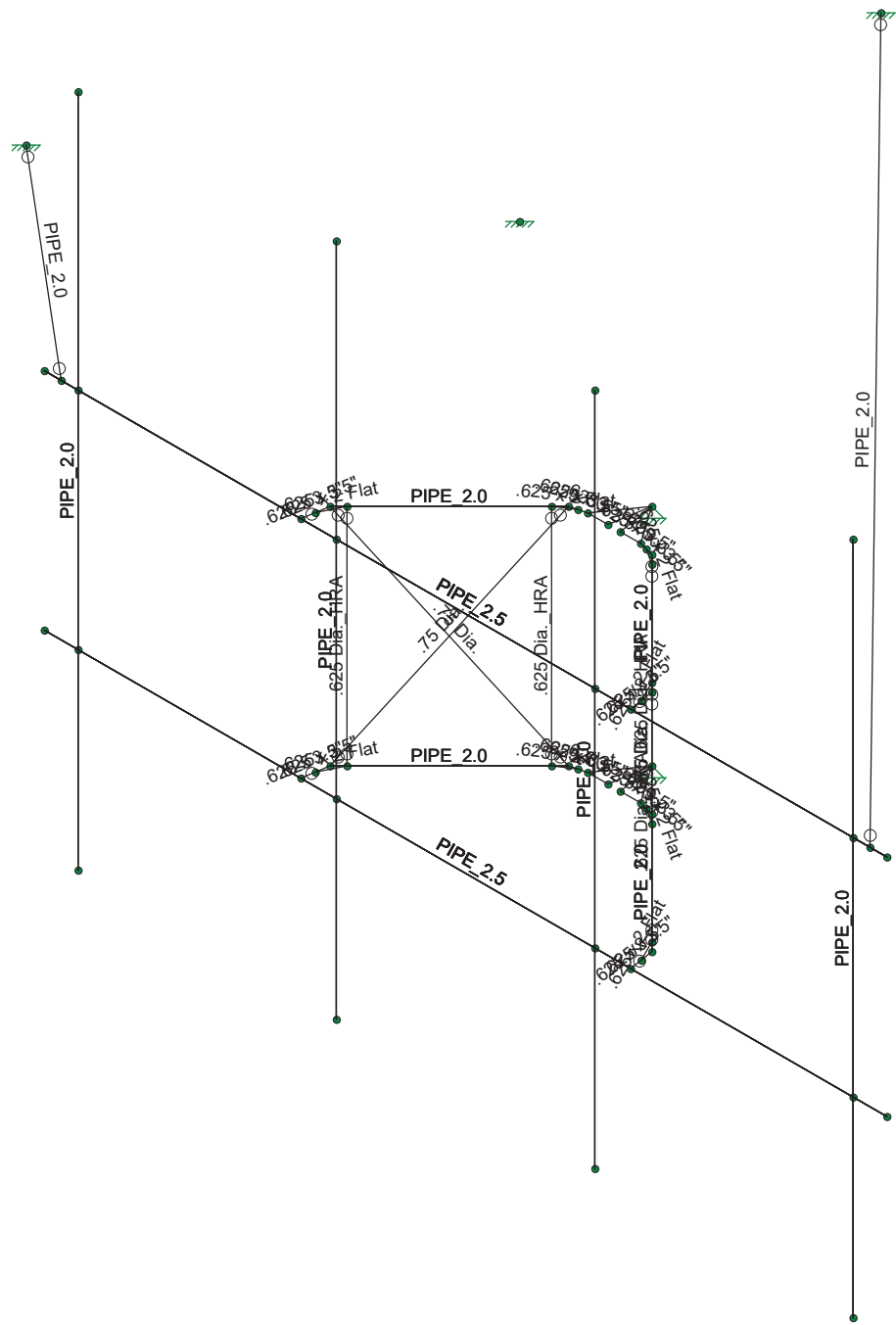
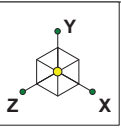
Nicole Hoffman, EI

Site-Pro #VFA12-HD

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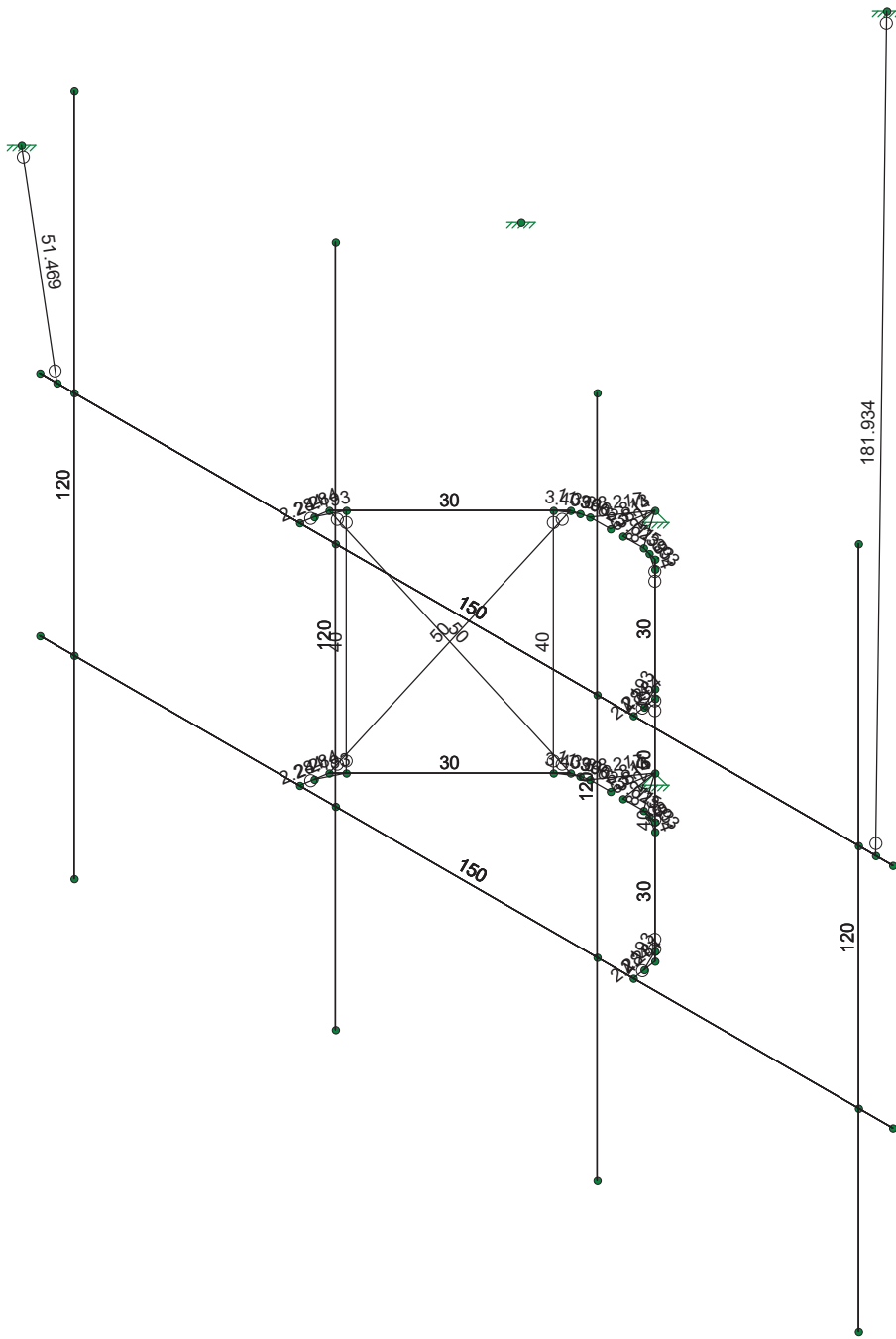
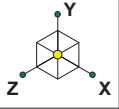


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USA Engineering, Inc.  
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Site-Pro #VFA12-HD.r3d



Member Length (in) Displayed  
Envelope Only Solution

USA Engineering, Inc.

Nicole Hoffman, EI

Site-Pro #VFA12-HD

SK - 5

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Site-Pro #VFA12-HD.r3d

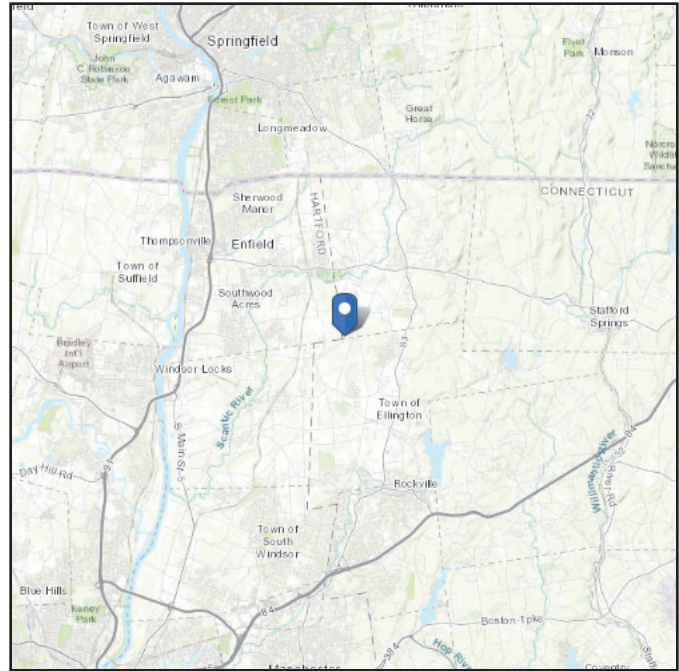
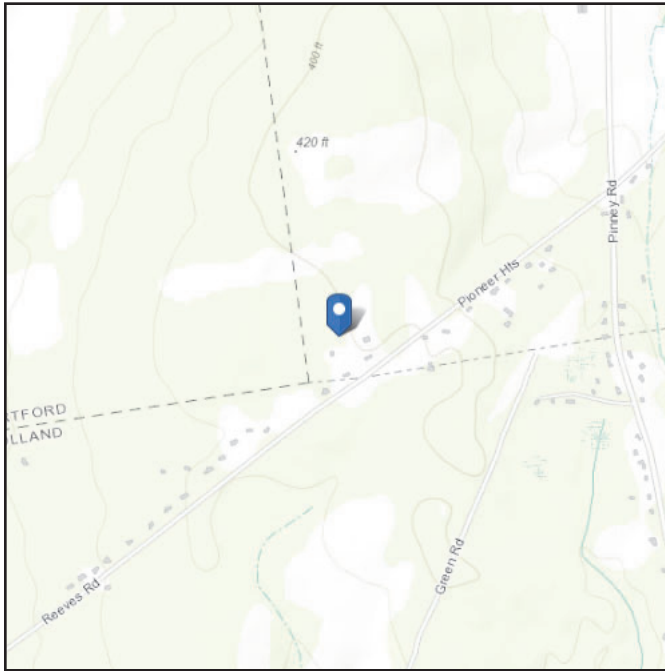
**APPENDIX B**  
**SOFTWARE INPUT CALCULATIONS**

# ASCE 7 Hazards Report

**Address:**  
No Address at This Location

**Standard:** ASCE/SEI 7-16  
**Risk Category:** II  
**Soil Class:** D - Default (see Section 11.4.3)

**Elevation:** 395.3 ft (NAVD 88)  
**Latitude:** 41.948889  
**Longitude:** -72.492222



## Wind

### Results:

Wind Speed	117 Vmph
10-year MRI	75 Vmph
25-year MRI	83 Vmph
50-year MRI	90 Vmph
100-year MRI	97 Vmph

Data Source: ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2  
Date Accessed: Thu Mar 31 2022

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

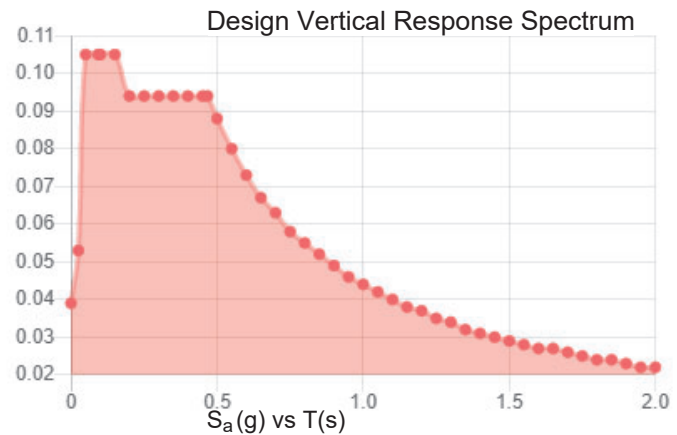
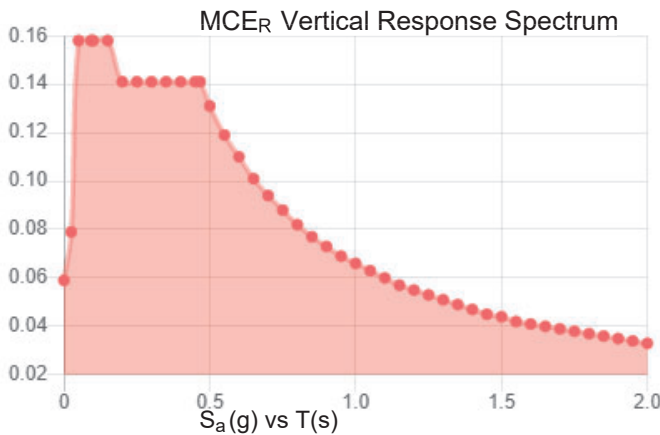
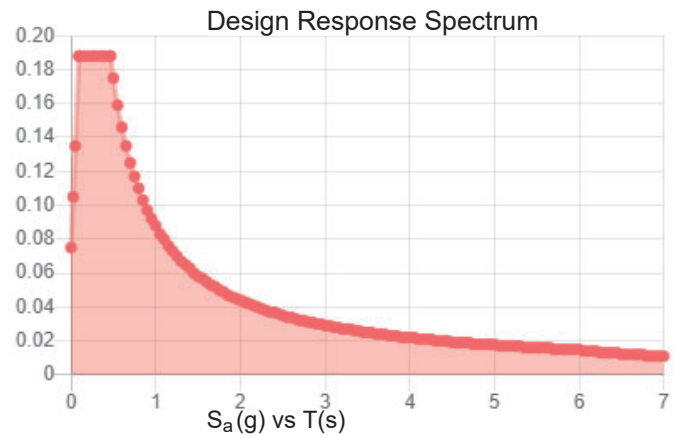
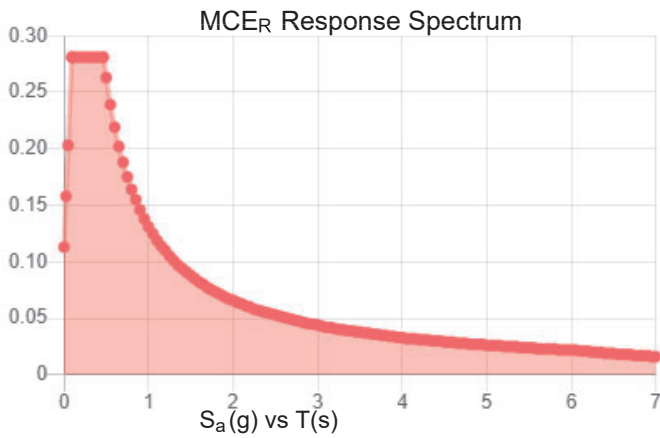
Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings need not be protected against wind-borne debris.

**Site Soil Class:** D - Default (see Section 11.4.3)

**Results:**

$S_s$ :	0.176	$S_{D1}$ :	0.088
$S_1$ :	0.055	$T_L$ :	6
$F_a$ :	1.6	PGA :	0.093
$F_v$ :	2.4	PGA <sub>M</sub> :	0.148
$S_{MS}$ :	0.281	$F_{PGA}$ :	1.6
$S_{M1}$ :	0.131	$I_e$ :	1
$S_{DS}$ :	0.188	$C_v$ :	0.7

**Seismic Design Category** B



**Data Accessed:** Thu Mar 31 2022

**Date Source:**

USGS Seismic Design Maps based on ASCE/SEI 7-16 and ASCE/SEI 7-16 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-16 Ch. 21 are available from USGS.

## Ice

---

**Results:**

Ice Thickness: 1.50 in.  
Concurrent Temperature: 5 F  
Gust Speed 50 mph

**Data Source:** Standard ASCE/SEI 7-16, Figs. 10-2 through 10-8

**Date Accessed:** Thu Mar 31 2022

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 500-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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TIA-222-H : Design Wind Force on Appurtenances

Design Parameters

Annex S: Analysis of Existing Antenna Supporting Structures	
$K_{zt} / F_w$	0.95

2.6.4 Basic Wind Speed and Design Ice Thickness	
V (mph), Basic Ultimate Wind Speed, 3-second gust:	117
V <sub>i</sub> (mph), Basic Ultimate Wind Speed with ice:	50
t (in), Design Ice Thickness:	1.50

2.6.5 Exposure Category and Velocity Pressure Coefficient			
Exposure Category, B, C or D:	C	$Z_t$ :	900
z (ft), AGL elevation of discrete appurtenance:	114	$g'$ :	9.5
		$K_z$ min:	0.85
		$K_z$ max:	2.01
		$K_d$ :	1.30

2.6.6 Topographic Effects			
Topographic Category, 1, 2, 3, 4 or Site-Specific:	1		
Topographic Feature (Site-Specific):	Flat/Rolling Terrain	$K_t$ :	-
z (ft), height AGL at the base of the structure:	-	f:	-
H (ft), height of crest above surrounding terrain:	-	$K_d$ :	-
L (ft), distance to the crest from surrounding terrain (Site-Specific):	-	$K_d$ :	-
x (ft), distance from the crest (Site-Specific):	-	$K_{dt}$ :	1.00

2.6.7 Rooftop Wind Speed-Up Factor			
Structures/Appurtenances supported on enclosed buildings:	No		
z <sub>r</sub> (ft), height of structure above rooftop:	-		
H <sub>r</sub> (ft), height of windward side of building:	-		
W <sub>r</sub> (ft), width of windward side of building:	-	H <sub>2</sub> :	-
X <sub>r</sub> (ft), horizontal distance from windward face to center of structure:	-	H <sub>2</sub> :	-
H (ft), height of parapet:	-	$K_z$ :	1.00

2.6.8 Ground Elevation Factor			
z <sub>g</sub> (ft), mean elevation of base of structure above sea level:	-	$K_g$ :	1.00

2.6.9 Gust Effect Factor			
Structure: (M) monopole, (S) self-support, (G) guyed, (A) appurtenance:	A	$G_s$ :	1.00
h (ft), height of structure from base (only affects G <sub>s</sub> for SS):	161	$K_{gt}$ :	0.95

2.6.10 Design Ice Thickness			
Consider Ice (Override):	-	$K_{it}$ :	1.13
Risk Category, I, II, III or IV:	II	$t_g$ :	1.70

2.6.11.6 Velocity Pressure		
$q_s = 0.00256 K_t K_d K_e K_z K_x V^2$ (psf):		41.1
$q_{s,i} = 0.00256 K_t K_{d,i} K_e K_z K_x V_i^2$ (psf):		7.5

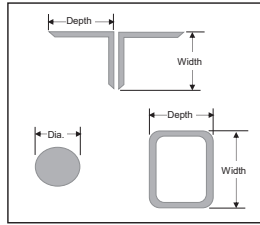
2.6.11.2 Design Wind Force on Appurtenances

Antenna(s)											$K_e$ 0.90								
Sector	Position	Description	Model	# of Items	(F)lat (HSS) (R)ound	Element Height (in)	Element Width or Dia. (in)	Element Depth or Dia. (in)	(HSS) Thickness (in)	Wind-Tunnel EPA (Override) (ft <sup>2</sup> )		Weight (lb)	No Ice		Ice				
										Front	Side		$\theta$ (°)	F <sub>x</sub> (lb)	$\theta$ (°)	F <sub>x</sub> (lb)	F <sub>x</sub> (lb)	F <sub>x</sub> (lb)	Ice Weight (lb)
All Sectors	1	AIR 6419 B41_TMO		1	F	36.25	20.91	9.02	-	-	-	96.5	0	234	90	107	54	28	166.16
All Sectors	2	APXVAALL24_43-U-NA20_TMO		1	F	95.9	24	8.5	-	-	-	149.9	0	750	90	323	160	80	414.72
All Sectors	3	VV-65A-R1_TMO		1	F	54.7	12	4.6	-	-	-	33.3	0	218	90	101	53	31	133.92

RRU(s) / TMA(s) / Diplexer(s) / Filter(s)											$K_e$ 0.90								
Sector	Position	Description	Model	# of Items	(F)lat (HSS) (R)ound	Element Height (in)	Element Width or Dia. (in)	Element Depth or Dia. (in)	(HSS) Thickness (in)	Wind-Tunnel EPA (Override) (ft <sup>2</sup> )		Weight (lb)	No Ice		Ice				
										Front	Side		$\theta$ (°)	F <sub>x</sub> (lb)	$\theta$ (°)	F <sub>x</sub> (lb)	F <sub>x</sub> (lb)	F <sub>x</sub> (lb)	Ice Weight (lb)
All Sectors	2	RADIO 4449 B71 B85A_T-MOBILE		1	F	17.91	13.2	10.63	-	-	-	73.21	90	59	0	73	17	20	79.28
All Sectors	3	RADIO 4460 B2/B25 B66_TMO		1	F	17	15.1	11.9	-	-	-	109	90	62	0	79	18	21	88.00

Uniform Distributed Load

Member	Width or Dia. (in)	Depth (in)	(HSS) Thickness (in)	No Ice UWL (lb/ft)	Ice UWL (lb/ft)	UDL of Ice (lb/ft)
Pipe	2.875	-	-	11	4	9.49
Pipe	2.375	-	-	9	4	8.45
Solid Rod	0.75	-	-	3	3	5.08
Solid Rod	0.625	-	-	2	3	4.82
Other	2	0.625	-	12	6	7.87
Other	0.625	3.5	-	22	8	10.90
-	-	-	-	-	-	-
-	-	-	-	-	-	-
-	-	-	-	-	-	-
-	-	-	-	-	-	-
-	-	-	-	-	-	-
-	-	-	-	-	-	-
-	-	-	-	-	-	-
-	-	-	-	-	-	-
-	-	-	-	-	-	-



Annex C: Design Wind Force on Typical Microwave Antennas

Sector	Position	Description	Model	$\theta$ (°)	Diameter (in)	Dish Type: P / PR / CS / G	$C_d / C_s$	No Ice		Ice	
								F <sub>AW</sub> / F <sub>SW</sub> (lb)	F <sub>AW</sub> / F <sub>SW</sub> (lb)	Ice Weight (lb)	Ice Weight (lb)
								-	-	-	-
								-	-	-	-
								-	-	-	-
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## TIA-222-H : 2.7 Seismic Load Effects



### Design Parameters

Table 2-3: Importance Factors	
Structure Risk Category	II
Importance Factor, I	1.00

2.7.4 Seismic Design Parameters	
Short Periods, $S_S$	0.176
Period of 1 Second, $S_1$	0.055
Long-Period Transition Period, $T_L$	8
Fundamental Period of Structure, T	0.146

2.7.5 Design Spectral Response Acceleration Parameters	
Short-period Site Coefficients, $F_a$	1.6
Long-period Site Coefficients, $F_v$	2.4
Design spectral response acceleration parameters at short periods, $S_{DS}$	0.188
Design spectral response acceleration parameters at a period of 1 second, $S_{D1}$	0.088

2.7.8.1 Amplification Factor	
Structure Height (ft)	161.4
Mount Rad Center (ft)	113
Amplification Factor, $A_s$	1.00

Loads	
Dead Load of Structures and Appurtenances, D (lb)	1017
Dead Load of Guy Assemblies, $D_g$ (lb)	0

### 2.7.6 Vertical Seismic Load Effect

Vertical Seismic Load Effect, $E_v$ (lb) = $0.2 S_{DS} (D + D_g)$ =	38.24
---	-------

### 2.7.7 Horizontal Seismic Load Effect

Redundancy Factor, $\rho$	1.00		
Effects of Horizontal Seismic Forces, $Q_E$	95.60	Total Weight, W (lb)	1017
		Seismic Response Coefficient, $C_s$	0.094
		Response Modification Coefficient, R	2
		Total Seismic Shear Force, $V_s$	95.60
Horizontal Seismic Load Effect, $E_h$ (lb) = $\rho Q_E$			95.60

**Note:** Seismic loads do not control.

**APPENDIX C**  
**SOFTWARE ANALYSIS OUTPUT**



**(Global) Model Settings**

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	12
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISACONNECTION CODE	None
Cold Formed Steel Code	None
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	None

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parme Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	Yes
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR SET ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8



**(Global) Model Settings, Continued**

Seismic Code	ASCE 7-10
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

**Joint Coordinates and Temperatures**

	Label	X [in]	Y [in]	Z [in]	Temp [F]	Detach From Diap...
1	N1	-8.927223	40	8.927223	0	
2	N2	-8.927223	0	8.927223	0	
3	N3	-27.135223	40	27.135223	0	
4	N4	-27.135223	0	27.135223	0	
5	N5	8.927223	40	8.927223	0	
6	N6	8.927223	0	8.927223	0	
7	N7	27.135223	40	27.135223	0	
8	N8	27.135223	0	27.135223	0	
9	N9	0	0	0	0	
10	N10	1.102459	0	6.724627	0	
11	N11	-1.096784	0	6.724627	0	
12	N12	-4.721784	0	6.724627	0	
13	N13	4.727459	0	6.724627	0	
14	N14	0	40	-1.8e-15	0	
15	N15	1.102459	40	6.724627	0	
16	N16	-1.096784	40	6.724627	0	
17	N17	-4.721784	40	6.724627	0	
18	N18	4.727459	40	6.724627	0	
19	N19	-7.424621	40	7.424621	0	
20	N20	-7.424621	0	7.424621	0	
21	N21	7.424621	0	7.424621	0	
22	N22	7.424621	40	7.424621	0	
23	N23	-29.35	40	33.150325	0	
24	N24	-28.637825	40	28.637825	0	
25	N25	29.35	40	33.150325	0	
26	N26	28.637825	40	28.637825	0	
27	N27	-29.35	-7.1e-15	33.150325	0	
28	N28	-28.637825	0	28.637825	0	
29	N29	29.35	7.1e-15	33.150325	0	
30	N30	28.637825	0	28.637825	0	



**Joint Coordinates and Temperatures (Continued)**

	Label	X [in]	Y [in]	Z [in]	Temp [F]	Detach From Diap...
31	N31	-75	1.42e-14	33.150325	0	
32	N32	75	3.55e-14	33.150325	0	
33	N33	-75	40	33.150325	0	
34	N34	75	40	33.150325	0	
35	N35	23	0	33.150325	0	
36	N36	23	39.999996	33.150325	0	
37	N37	-23	0	33.150325	0	
38	N38	-23	39.999996	33.150325	0	
39	N39	-28.993912	40	30.894075	0	
40	N40	-28.993912	-3.55e-15	30.894075	0	
41	N41	28.993912	3.55e-15	30.894075	0	
42	N42	28.993912	40	30.894075	0	
43	N43	6.07604	40	7.074624	0	
44	N44	-6.073203	40	7.074624	0	
45	N45	-6.073203	0	7.074624	0	
46	N46	6.07604	0	7.074624	0	
47	N47	-72	40	33.150325	0	
48	N48	-69	86	33.150325	0	
49	N49	69	86	33.150325	0	
50	N50	23	85.999996	33.150325	0	
51	N51	-23	85.999996	33.150325	0	
52	N52	-69	-34	33.150325	0	
53	N53	69	-34	33.150325	0	
54	N54	23	-34	33.150325	0	
55	N55	-23	-34	33.150325	0	
56	N56	-69	1.42e-14	33.150325	0	
57	N57	-69	40	33.150325	0	
58	N58	69	3.55e-14	33.150325	0	
59	N59	69	40	33.150325	0	
60	N60	-55.685433	40	-32.15	0	
61	N61	-55.685433	40	-96.45	0	
62	N62	-111.370867	40	0.	0	
63	N63	72	40	33.150325	0	

**Joint Boundary Conditions**

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	N14	Reaction	Reaction	Reaction			
2	N9	Reaction	Reaction	Reaction			
3	N60	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
4	N61	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
5	N62	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction

**Hot Rolled Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E...Density[k/ft...	Yield[ksi]	Ry	Fu[ksi]	Rt	
1	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
2	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	58	1.2
3	A992	29000	11154	.3	.65	.49	50	1.1	58	1.2
4	A500 Gr.42	29000	11154	.3	.65	.49	42	1.3	58	1.1
5	A500 Gr.46	29000	11154	.3	.65	.49	46	1.2	58	1.1
6	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2



**Member Primary Data**

	Label	I Joint	J Joint	K Joint	Rotate(d...	Section/Sh...	Type	Design List	Material	Design Rules
1	M1	N12	N9			RIGID	None	None	RIGID	Typical
2	M2	N9	N11			RIGID	None	None	RIGID	Typical
3	M3	N9	N10			RIGID	None	None	RIGID	Typical
4	M4	N9	N13			RIGID	None	None	RIGID	Typical
5	M5	N17	N14			RIGID	None	None	RIGID	Typical
6	M6	N14	N16			RIGID	None	None	RIGID	Typical
7	M7	N14	N15			RIGID	None	None	RIGID	Typical
8	M8	N14	N18			RIGID	None	None	RIGID	Typical
9	M9	N33	N34			PIPE 2.5	Beam	Pipe	A53 Gr.B	Typical
10	M10	N31	N32			PIPE 2.5	Beam	Pipe	A53 Gr.B	Typical
11	M11	N47	N62			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
12	M12	N21	N30			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
13	M13	N20	N28			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
14	M14	N19	N24			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
15	M15	N22	N26			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
16	POSITION_4	N48	N52			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
17	POSITION_3	N51	N55			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
18	POSITION_2	N50	N54			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
19	POSITION_1	N49	N53			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical
20	M20	N13	N10		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
21	M21	N11	N12		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
22	M22	N12	N45		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
23	M23	N13	N46		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
24	M24	N18	N15		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
25	M25	N16	N17		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
26	M26	N17	N44		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
27	M27	N18	N43		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
28	M28	N23	N39		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
29	M29	N25	N42		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
30	M30	N27	N40		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
31	M31	N29	N41		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
32	M32	N39	N24		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
33	M33	N40	N28		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
34	M34	N41	N30		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
35	M35	N42	N26		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
36	M36	N43	N22		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
37	M37	N44	N19		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
38	M38	N45	N20		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
39	M39	N46	N21		90	.625 x 3.5"	Beam	None	A36 Gr.36	Typical
40	M40	N39	N3			.625 x 2 Flat	Beam	None	A36 Gr.36	Typical
41	M41	N40	N4			.625 x 2 Flat	Beam	None	A36 Gr.36	Typical
42	M42	N41	N8			.625 x 2 Flat	Beam	None	A36 Gr.36	Typical
43	M43	N42	N7			.625 x 2 Flat	Beam	None	A36 Gr.36	Typical
44	M44	N5	N43			.625 x 2 Flat	Beam	None	A36 Gr.36	Typical
45	M45	N44	N1			.625 x 2 Flat	Beam	None	A36 Gr.36	Typical
46	M46	N2	N45			.625 x 2 Flat	Beam	None	A36 Gr.36	Typical
47	M47	N46	N6			.625 x 2 Flat	Beam	None	A36 Gr.36	Typical
48	M48	N1	N2			.625 Dia._H...	Beam	None	A36 Gr.36	Typical
49	M49	N3	N4			.625 Dia._H...	Beam	None	A36 Gr.36	Typical
50	M50	N5	N6			.625 Dia._H...	Beam	None	A36 Gr.36	Typical
51	M51	N7	N8			.625 Dia._H...	Beam	None	A36 Gr.36	Typical
52	M52	N19	N28			.75 Dia.	Beam	None	A36 Gr.36	Typical
53	M53	N20	N24			.75 Dia.	Beam	None	A36 Gr.36	Typical
54	M54	N21	N26			.75 Dia.	Beam	None	A36 Gr.36	Typical
55	M55	N30	N22			.75 Dia.	Beam	None	A36 Gr.36	Typical
56	M56	N63	N61			PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical



**Member Advanced Data**

	Label	I Release	J Release	I Offset[in]	J Offset[in]	T/C Only	Physical	Defl Rat..	Analysis ...	Inactive	Seismic...
1	M1						Yes	** NA **			None
2	M2						Yes	** NA **			None
3	M3						Yes	** NA **			None
4	M4						Yes	** NA **			None
5	M5						Yes	** NA **			None
6	M6						Yes	** NA **			None
7	M7						Yes	** NA **			None
8	M8						Yes	** NA **			None
9	M9						Yes				None
10	M10						Yes				None
11	M11	BenPIN	BenPIN				Yes				None
12	M12						Yes				None
13	M13						Yes				None
14	M14						Yes				None
15	M15						Yes				None
16	POSITION_4						Yes				None
17	POSITION_3						Yes				None
18	POSITION_2						Yes				None
19	POSITION_1						Yes				None
20	M20						Yes				None
21	M21						Yes				None
22	M22						Yes				None
23	M23						Yes				None
24	M24						Yes				None
25	M25						Yes				None
26	M26						Yes				None
27	M27						Yes	Default			None
28	M28	BenPIN					Yes	Default			None
29	M29	BenPIN					Yes	Default			None
30	M30	BenPIN					Yes	Default			None
31	M31	BenPIN					Yes	Default			None
32	M32						Yes				None
33	M33						Yes				None
34	M34						Yes				None
35	M35						Yes				None
36	M36						Yes				None
37	M37						Yes				None
38	M38						Yes				None
39	M39						Yes				None
40	M40						Yes				None
41	M41						Yes				None
42	M42						Yes				None
43	M43						Yes				None
44	M44						Yes				None
45	M45						Yes				None
46	M46						Yes				None
47	M47						Yes				None
48	M48	BenPIN	BenPIN				Yes	Default			None
49	M49	BenPIN	BenPIN				Yes	Default			None
50	M50	BenPIN	BenPIN				Yes				None
51	M51	BenPIN	BenPIN				Yes				None
52	M52	BenPIN	BenPIN				Yes	Default			None
53	M53	BenPIN	BenPIN				Euler Buc...	Yes			None
54	M54	BenPIN	BenPIN				Euler Buc...	Yes			None
55	M55	BenPIN	BenPIN				Euler Buc...	Yes			None
56	M56	BenPIN	BenPIN				Yes				None





### Hot Rolled Steel Design Parameters

	Label	Shape	Length[in]	Lbyy[in]	Lbzz[in]	Lcomp top[in]	Lcomp bot[in]	L-torqu...	Kyy	Kzz	Cb	Function
1	M9	PIPE 2.5	150	58.7	58.7	Lbyy						Lateral
2	M10	PIPE 2.5	150	58.7	58.7	Lbyy						Lateral
3	M11	PIPE 2.0	51.469			Lbyy						Lateral
4	M12	PIPE 2.0	30			Lbyy						Lateral
5	M13	PIPE 2.0	30			Lbyy						Lateral
6	M14	PIPE 2.0	30			Lbyy						Lateral
7	M15	PIPE 2.0	30			Lbyy						Lateral
8	POSITION_4	PIPE 2.0	120			Lbyy						Lateral
9	POSITION_3	PIPE 2.0	120			Lbyy						Lateral
10	POSITION_2	PIPE 2.0	120			Lbyy						Lateral
11	POSITION_1	PIPE 2.0	120			Lbyy						Lateral
12	M20	.625 x 3.5"	3.625			Lbyy						Lateral
13	M21	.625 x 3.5"	3.625			Lbyy						Lateral
14	M22	.625 x 3.5"	1.396			Lbyy						Lateral
15	M23	.625 x 3.5"	1.393			Lbyy						Lateral
16	M24	.625 x 3.5"	3.625			Lbyy						Lateral
17	M25	.625 x 3.5"	3.625			Lbyy						Lateral
18	M26	.625 x 3.5"	1.396			Lbyy						Lateral
19	M27	.625 x 3.5"	1.393			Lbyy						Lateral
20	M28	.625 x 3.5"	2.284			Lbyy						Lateral
21	M29	.625 x 3.5"	2.284			Lbyy						Lateral
22	M30	.625 x 3.5"	2.284			Lbyy						Lateral
23	M31	.625 x 3.5"	2.284			Lbyy						Lateral
24	M32	.625 x 3.5"	2.284			Lbyy						Lateral
25	M33	.625 x 3.5"	2.284			Lbyy						Lateral
26	M34	.625 x 3.5"	2.284			Lbyy						Lateral
27	M35	.625 x 3.5"	2.284			Lbyy						Lateral
28	M36	.625 x 3.5"	1.393			Lbyy						Lateral
29	M37	.625 x 3.5"	1.396			Lbyy						Lateral
30	M38	.625 x 3.5"	1.396			Lbyy						Lateral
31	M39	.625 x 3.5"	1.393			Lbyy						Lateral
32	M40	.625 x 2 Flat	4.193			Lbyy						Lateral
33	M41	.625 x 2 Flat	4.193			Lbyy						Lateral
34	M42	.625 x 2 Flat	4.193			Lbyy						Lateral
35	M43	.625 x 2 Flat	4.193			Lbyy						Lateral
36	M44	.625 x 2 Flat	3.4			Lbyy						Lateral
37	M45	.625 x 2 Flat	3.403			Lbyy						Lateral
38	M46	.625 x 2 Flat	3.403			Lbyy						Lateral
39	M47	.625 x 2 Flat	3.4			Lbyy						Lateral
40	M48	.625 Dia. H...	40	30	30	Lbyy						Lateral
41	M49	.625 Dia. H...	40	30	30	Lbyy						Lateral
42	M50	.625 Dia. H...	40	30	30	Lbyy						Lateral
43	M51	.625 Dia. H...	40	30	30	Lbyy						Lateral
44	M52	.75 Dia.	50			Lbyy						Lateral
45	M53	.75 Dia.	50			Lbyy						Lateral
46	M54	.75 Dia.	50			Lbyy						Lateral
47	M55	.75 Dia.	50			Lbyy						Lateral
48	M56	PIPE 2.0	181.934			Lbyy						Lateral

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area(Me... Surface(...		
1	DEAD LOAD	DL		-1			8			
2	LIVE LOAD (Lv)	LL				1				
3	WIND FRONT	WLZ					8	48		



**Basic Load Cases (Continued)**

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area(Me... Surface(...
4	WIND SIDE	WLX					8	48
5	Wind Front @ 30 mph	WLZ					8	48
6	Wind Side @ 30 mph	WLX					8	48
7	UDL	LL						
8	LIVE LOAD (Lm)	LL				1		
9	DEAD LOAD (ICE)	DL					8	48
10	WIND FRONT (ICE)	WLZ					8	48
11	WIND SIDE (ICE)	WLX					8	48

**Load Combinations**

	Description	S...P...	S...B...	Fa...B...	Fa...B...	Fa...B...	Fa...B...	Fa...B...	Fa...B...	Fa...B...	Fa...B...	Fa...B...	Fa...B...	Fa...B...	Fa...B...
1	DEAD LOAD	Y... Y	1 1												
2	1.4 DEAD LOAD	Y... Y	1 1.4												
3	DEAD LOAD + Lv	Y... Y	1 1	2 1											
4	1.2 DEAD LOAD + 1.5 Lv	Y... Y	1 1.2	2 1.5											
5	DEAD LOAD + UDL	Y... Y	1 1							7 1					
6	1.2 DEAD LOAD + 1.5 UDL	Y... Y	1 1.2							7 1.5					
7	1.2 DEAD LOAD + 1.0 WIND 0	Y... Y	1 1.2		3 1										
8	1.2 DEAD LOAD + 1.0 WIND 30	Y... Y	1 1.2		3 .869	4 .5									
9	1.2 DEAD LOAD + 1.0 WIND 60	Y... Y	1 1.2		3 .5	4 .869									
10	1.2 DEAD LOAD + 1.0 WIND 90	Y... Y	1 1.2			4 1									
11	1.2 DEAD LOAD + 1.0 WIND 120	Y... Y	1 1.2		3 -.5	4 .869									
12	1.2 DEAD LOAD + 1.0 WIND 150	Y... Y	1 1.2		3 -.8...	4 .5									
13	1.2 DEAD LOAD + 1.0 WIND 180	Y... Y	1 1.2		3 -1										
14	1.2 DEAD LOAD + 1.0 WIND 210	Y... Y	1 1.2		3 -.8...	4 -.5									
15	1.2 DEAD LOAD + 1.0 WIND 240	Y... Y	1 1.2		3 -.5	4 -.8...									
16	1.2 DEAD LOAD + 1.0 WIND 270	Y... Y	1 1.2			4 -1									
17	1.2 DEAD LOAD + 1.0 WIND 300	Y... Y	1 1.2		3 .5	4 -.8...									
18	1.2 DEAD LOAD + 1.0 WIND 330	Y... Y	1 1.2		3 .869	4 -.5									
19	0.9 DEAD LOAD + 1.0 WIND 0	Y... Y	1 .9		3 1										
20	0.9 DEAD LOAD + 1.0 WIND 30	Y... Y	1 .9		3 .869	4 .5									
21	0.9 DEAD LOAD + 1.0 WIND 60	Y... Y	1 .9		3 .5	4 .869									
22	0.9 DEAD LOAD + 1.0 WIND 90	Y... Y	1 .9			4 1									
23	0.9 DEAD LOAD + 1.0 WIND 120	Y... Y	1 .9		3 -.5	4 .869									
24	0.9 DEAD LOAD + 1.0 WIND 150	Y... Y	1 .9		3 -.8...	4 .5									
25	0.9 DEAD LOAD + 1.0 WIND 180	Y... Y	1 .9		3 -1										
26	0.9 DEAD LOAD + 1.0 WIND 210	Y... Y	1 .9		3 -.8...	4 -.5									
27	0.9 DEAD LOAD + 1.0 WIND 240	Y... Y	1 .9		3 -.5	4 -.8...									
28	0.9 DEAD LOAD + 1.0 WIND 270	Y... Y	1 .9			4 -1									
29	0.9 DEAD LOAD + 1.0 WIND 300	Y... Y	1 .9		3 .5	4 -.8...									
30	0.9 DEAD LOAD + 1.0 WIND 330	Y... Y	1 .9		3 .869	4 -.5									
31	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0 ...	Y... Y	1 1.2				5 1				8 1.5				
32	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2				5 .869	6 .5			8 1.5				
33	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2				5 .5	6 .869			8 1.5				
34	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2					6 1			8 1.5				
35	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2				5 -.5	6 .869			8 1.5				
36	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2				5 -.8...	6 .5			8 1.5				
37	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2				5 -1				8 1.5				
38	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2				5 -.8...	6 -.5			8 1.5				
39	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2				5 -.5	6 -.8...			8 1.5				
40	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2					6 -1			8 1.5				
41	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2				5 .5	6 -.8...			8 1.5				
42	1.2 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 1.2				5 .869	6 -.5			8 1.5				
43	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0 ...	Y... Y	1 .9				5 1				8 1.5				
44	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y... Y	1 .9				5 .869	6 .5			8 1.5				



**Load Combinations (Continued)**

Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	
45	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9						5	.5	6	.869			8	1.5		
46	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9								6	1			8	1.5		
47	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9						5	-.5	6	.869			8	1.5		
48	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9						5	-.8...	6	.5			8	1.5		
49	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9						5	-1					8	1.5		
50	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9						5	-.8...	6	-.5			8	1.5		
51	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9						5	-.5	6	-.8...			8	1.5		
52	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9								6	-1			8	1.5		
53	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9						5	.5	6	-.8...			8	1.5		
54	0.9 DEAD LOAD + 1.5 LIVE LOAD + 1.0...	Y...	Y	1	.9						5	.869	6	-.5			8	1.5		
55	1.0 DEAD LOAD + 1.0 DEAD LOAD (ICE)	Y...	Y	1	1								9	1						
56	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	1				
57	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	.869	11	.5		
58	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	.5	11	.869		
59	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1			11	1		
60	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	-.5	11	.869		
61	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	-.8...	11	-.5		
62	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	-1				
63	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	-.8...	11	-.5		
64	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	-.5	11	-.8...		
65	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1			11	-1		
66	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	.5	11	-.8...		
67	1.2 DEAD LOAD + 1.0 DEAD LOAD (IC...	Y...	Y	1	1.2								9	1	10	.869	11	-.5		

**Joint Loads and Enforced Displacements (BLC 2 : LIVE LOAD (Lv))**

Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,ra...]
1 N32	L	Y	-.25

**Joint Loads and Enforced Displacements (BLC 8 : LIVE LOAD (Lm))**

Joint Label	L,D,M	Direction	Magnitude[(k,k-ft), (in,ra...]
1 N49	L	Y	-.25

**Member Point Loads (BLC 1 : DEAD LOAD)**

Member Label	Direction	Magnitude[k,k-ft]	Location[in, %]
1 POSITION_1	Y	-.058	%25
2 POSITION_2	Y	-.085	%10
3 POSITION_3	Y	-.027	%20
4 POSITION_1	Y	-.058	%65
5 POSITION_2	Y	-.085	%80
6 POSITION_3	Y	-.027	%70
7 POSITION_2	Y	-.093	%50
8 POSITION_3	Y	-.129	%50

**Member Point Loads (BLC 3 : WIND FRONT)**

Member Label	Direction	Magnitude[k,k-ft]	Location[in, %]
1 POSITION_1	Z	-.117	%25
2 POSITION_2	Z	-.375	%10
3 POSITION_3	Z	-.109	%20
4 POSITION_1	Z	-.117	%65
5 POSITION_2	Z	-.375	%80
6 POSITION_3	Z	-.109	%70
7 POSITION_2	Z	-.059	%50
8 POSITION_3	Z	-.062	%50



**Member Point Loads (BLC 4 : WIND SIDE)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[in, %]
1	POSITION 1	X	-.053	%25
2	POSITION 2	X	-.162	%10
3	POSITION 3	X	-.051	%20
4	POSITION 1	X	-.053	%65
5	POSITION 2	X	-.162	%80
6	POSITION 3	X	-.051	%70
7	POSITION 2	X	-.073	%50
8	POSITION 3	X	-.079	%50

**Member Point Loads (BLC 5 : Wind Front @ 30 mph)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[in, %]
1	POSITION 1	Z	-.008	%25
2	POSITION 2	Z	-.026	%10
3	POSITION 3	Z	-.008	%20
4	POSITION 1	Z	-.008	%65
5	POSITION 2	Z	-.026	%80
6	POSITION 3	Z	-.008	%70
7	POSITION 2	Z	-.004	%50
8	POSITION 3	Z	-.004	%50

**Member Point Loads (BLC 6 : Wind Side @ 30 mph)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[in, %]
1	POSITION 1	X	-.004	%25
2	POSITION 2	X	-.011	%10
3	POSITION 3	X	-.004	%20
4	POSITION 1	X	-.004	%65
5	POSITION 2	X	-.011	%80
6	POSITION 3	X	-.004	%70
7	POSITION 2	X	-.005	%50
8	POSITION 3	X	-.005	%50

**Member Point Loads (BLC 9 : DEAD LOAD (ICE))**

	Member Label	Direction	Magnitude[k,k-ft]	Location[in, %]
1	POSITION 1	Y	-.083	%25
2	POSITION 2	Y	-.207	%10
3	POSITION 3	Y	-.067	%20
4	POSITION 1	Y	-.083	%65
5	POSITION 2	Y	-.207	%80
6	POSITION 3	Y	-.067	%70
7	POSITION 2	Y	-.079	%50
8	POSITION 3	Y	-.088	%50

**Member Point Loads (BLC 10 : WIND FRONT (ICE))**

	Member Label	Direction	Magnitude[k,k-ft]	Location[in, %]
1	POSITION 1	Z	-.027	%25
2	POSITION 2	Z	-.08	%10
3	POSITION 3	Z	-.026	%20
4	POSITION 1	Z	-.027	%65
5	POSITION 2	Z	-.08	%80
6	POSITION 3	Z	-.026	%70
7	POSITION 2	Z	-.017	%50
8	POSITION 3	Z	-.018	%50

**Member Point Loads (BLC 11 : WIND SIDE (ICE))**

	Member Label	Direction	Magnitude[k,k-ft]	Location[in, %]
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**Member Point Loads (BLC 11 : WIND SIDE (ICE)) (Continued)**

	Member Label	Direction	Magnitude[k,k-ft]	Location[in, %]
1	POSITION 1	X	-0.14	%25
2	POSITION 2	X	-0.04	%10
3	POSITION 3	X	-0.15	%20
4	POSITION 1	X	-0.14	%65
5	POSITION 2	X	-0.04	%80
6	POSITION 3	X	-0.15	%70
7	POSITION 2	X	-0.02	%50
8	POSITION 3	X	-0.021	%50

**Member Distributed Loads (BLC 3 : WIND FRONT)**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in, %]	End Location[in, %]
1	M9	PZ	-0.11	-0.11	0	0
2	M10	PZ	-0.11	-0.11	0	0
3	M11	PZ	-0.09	-0.09	0	0
4	M12	PZ	-0.09	-0.09	0	0
5	M13	PZ	-0.09	-0.09	0	0
6	M14	PZ	-0.09	-0.09	0	0
7	M15	PZ	-0.09	-0.09	0	0
8	POSITION 4	PZ	-0.09	-0.09	0	0
9	POSITION 3	PZ	-0.09	-0.09	0	0
10	POSITION 2	PZ	-0.09	-0.09	0	0
11	POSITION 1	PZ	-0.09	-0.09	0	0
12	M56	PZ	-0.09	-0.09	0	0
13	M52	PZ	-0.03	-0.03	0	0
14	M53	PZ	-0.03	-0.03	0	0
15	M54	PZ	-0.03	-0.03	0	0
16	M55	PZ	-0.03	-0.03	0	0
17	M48	PZ	-0.02	-0.02	0	0
18	M49	PZ	-0.02	-0.02	0	0
19	M50	PZ	-0.02	-0.02	0	0
20	M51	PZ	-0.02	-0.02	0	0
21	M40	PZ	-0.12	-0.12	0	0
22	M41	PZ	-0.12	-0.12	0	0
23	M42	PZ	-0.12	-0.12	0	0
24	M43	PZ	-0.12	-0.12	0	0
25	M44	PZ	-0.12	-0.12	0	0
26	M45	PZ	-0.12	-0.12	0	0
27	M46	PZ	-0.12	-0.12	0	0
28	M47	PZ	-0.12	-0.12	0	0
29	M20	PZ	-0.04	-0.04	0	0
30	M21	PZ	-0.04	-0.04	0	0
31	M22	PZ	-0.04	-0.04	0	0
32	M23	PZ	-0.04	-0.04	0	0
33	M24	PZ	-0.04	-0.04	0	0
34	M25	PZ	-0.04	-0.04	0	0
35	M26	PZ	-0.04	-0.04	0	0
36	M27	PZ	-0.04	-0.04	0	0
37	M28	PZ	-0.04	-0.04	0	0
38	M29	PZ	-0.04	-0.04	0	0
39	M30	PZ	-0.04	-0.04	0	0
40	M31	PZ	-0.04	-0.04	0	0
41	M32	PZ	-0.04	-0.04	0	0
42	M33	PZ	-0.04	-0.04	0	0
43	M34	PZ	-0.04	-0.04	0	0
44	M35	PZ	-0.04	-0.04	0	0



**Member Distributed Loads (BLC 3 : WIND FRONT) (Continued)**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft....	Start Location[in, %]	End Location[in, %]
45	M36	PZ	-0.004	-0.004	0	0
46	M37	PZ	-0.004	-0.004	0	0
47	M38	PZ	-0.004	-0.004	0	0
48	M39	PZ	-0.004	-0.004	0	0

**Member Distributed Loads (BLC 4 : WIND SIDE)**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft....	Start Location[in, %]	End Location[in, %]
1	M9	PX	-0.011	-0.011	0	0
2	M10	PX	-0.011	-0.011	0	0
3	M11	PX	-0.009	-0.009	0	0
4	M12	PX	-0.009	-0.009	0	0
5	M13	PX	-0.009	-0.009	0	0
6	M14	PX	-0.009	-0.009	0	0
7	M15	PX	-0.009	-0.009	0	0
8	POSITION_4	PX	-0.009	-0.009	0	0
9	POSITION_3	PX	-0.009	-0.009	0	0
10	POSITION_2	PX	-0.009	-0.009	0	0
11	POSITION_1	PX	-0.009	-0.009	0	0
12	M56	PX	-0.009	-0.009	0	0
13	M52	PX	-0.003	-0.003	0	0
14	M53	PX	-0.003	-0.003	0	0
15	M54	PX	-0.003	-0.003	0	0
16	M55	PX	-0.003	-0.003	0	0
17	M48	PX	-0.002	-0.002	0	0
18	M49	PX	-0.002	-0.002	0	0
19	M50	PX	-0.002	-0.002	0	0
20	M51	PX	-0.002	-0.002	0	0
21	M40	PX	-0.012	-0.012	0	0
22	M41	PX	-0.012	-0.012	0	0
23	M42	PX	-0.012	-0.012	0	0
24	M43	PX	-0.012	-0.012	0	0
25	M44	PX	-0.012	-0.012	0	0
26	M45	PX	-0.012	-0.012	0	0
27	M46	PX	-0.012	-0.012	0	0
28	M47	PX	-0.012	-0.012	0	0
29	M20	PX	-0.004	-0.004	0	0
30	M21	PX	-0.004	-0.004	0	0
31	M22	PX	-0.004	-0.004	0	0
32	M23	PX	-0.004	-0.004	0	0
33	M24	PX	-0.004	-0.004	0	0
34	M25	PX	-0.004	-0.004	0	0
35	M26	PX	-0.004	-0.004	0	0
36	M27	PX	-0.004	-0.004	0	0
37	M28	PX	-0.004	-0.004	0	0
38	M29	PX	-0.004	-0.004	0	0
39	M30	PX	-0.004	-0.004	0	0
40	M31	PX	-0.004	-0.004	0	0
41	M32	PX	-0.004	-0.004	0	0
42	M33	PX	-0.004	-0.004	0	0
43	M34	PX	-0.004	-0.004	0	0
44	M35	PX	-0.004	-0.004	0	0
45	M36	PX	-0.004	-0.004	0	0
46	M37	PX	-0.004	-0.004	0	0
47	M38	PX	-0.004	-0.004	0	0
48	M39	PX	-0.004	-0.004	0	0



**Member Distributed Loads (BLC 5 : Wind Front @ 30 mph)**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in, %]	End Location[in, %]
1	M9	PZ	-0.007612	-0.007612	0	0
2	M10	PZ	-0.007612	-0.007612	0	0
3	M11	PZ	-0.006228	-0.006228	0	0
4	M12	PZ	-0.006228	-0.006228	0	0
5	M13	PZ	-0.006228	-0.006228	0	0
6	M14	PZ	-0.006228	-0.006228	0	0
7	M15	PZ	-0.006228	-0.006228	0	0
8	POSITION 4	PZ	-0.006228	-0.006228	0	0
9	POSITION 3	PZ	-0.006228	-0.006228	0	0
10	POSITION 2	PZ	-0.006228	-0.006228	0	0
11	POSITION 1	PZ	-0.006228	-0.006228	0	0
12	M56	PZ	-0.006228	-0.006228	0	0
13	M52	PZ	-0.002076	-0.002076	0	0
14	M53	PZ	-0.002076	-0.002076	0	0
15	M54	PZ	-0.002076	-0.002076	0	0
16	M55	PZ	-0.002076	-0.002076	0	0
17	M48	PZ	-0.001384	-0.001384	0	0
18	M49	PZ	-0.001384	-0.001384	0	0
19	M50	PZ	-0.001384	-0.001384	0	0
20	M51	PZ	-0.001384	-0.001384	0	0
21	M40	PZ	-0.008304	-0.008304	0	0
22	M41	PZ	-0.008304	-0.008304	0	0
23	M42	PZ	-0.008304	-0.008304	0	0
24	M43	PZ	-0.008304	-0.008304	0	0
25	M44	PZ	-0.008304	-0.008304	0	0
26	M45	PZ	-0.008304	-0.008304	0	0
27	M46	PZ	-0.008304	-0.008304	0	0
28	M47	PZ	-0.008304	-0.008304	0	0
29	M20	PZ	-0.002768	-0.002768	0	0
30	M21	PZ	-0.002768	-0.002768	0	0
31	M22	PZ	-0.002768	-0.002768	0	0
32	M23	PZ	-0.002768	-0.002768	0	0
33	M24	PZ	-0.002768	-0.002768	0	0
34	M25	PZ	-0.002768	-0.002768	0	0
35	M26	PZ	-0.002768	-0.002768	0	0
36	M27	PZ	-0.002768	-0.002768	0	0
37	M28	PZ	-0.002768	-0.002768	0	0
38	M29	PZ	-0.002768	-0.002768	0	0
39	M30	PZ	-0.002768	-0.002768	0	0
40	M31	PZ	-0.002768	-0.002768	0	0
41	M32	PZ	-0.002768	-0.002768	0	0
42	M33	PZ	-0.002768	-0.002768	0	0
43	M34	PZ	-0.002768	-0.002768	0	0
44	M35	PZ	-0.002768	-0.002768	0	0
45	M36	PZ	-0.002768	-0.002768	0	0
46	M37	PZ	-0.002768	-0.002768	0	0
47	M38	PZ	-0.002768	-0.002768	0	0
48	M39	PZ	-0.002768	-0.002768	0	0

**Member Distributed Loads (BLC 6 : Wind Side @ 30 mph)**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in, %]	End Location[in, %]
1	M9	PX	-0.007612	-0.007612	0	0
2	M10	PX	-0.007612	-0.007612	0	0
3	M11	PX	-0.006228	-0.006228	0	0
4	M12	PX	-0.006228	-0.006228	0	0
5	M13	PX	-0.006228	-0.006228	0	0



**Member Distributed Loads (BLC 6 : Wind Side @ 30 mph) (Continued)**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in, %]	End Location[in, %]
6	M14	PX	-0.006228	-0.006228	0	0
7	M15	PX	-0.006228	-0.006228	0	0
8	POSITION 4	PX	-0.006228	-0.006228	0	0
9	POSITION 3	PX	-0.006228	-0.006228	0	0
10	POSITION 2	PX	-0.006228	-0.006228	0	0
11	POSITION 1	PX	-0.006228	-0.006228	0	0
12	M56	PX	-0.006228	-0.006228	0	0
13	M52	PX	-0.002076	-0.002076	0	0
14	M53	PX	-0.002076	-0.002076	0	0
15	M54	PX	-0.002076	-0.002076	0	0
16	M55	PX	-0.002076	-0.002076	0	0
17	M48	PX	-0.001384	-0.001384	0	0
18	M49	PX	-0.001384	-0.001384	0	0
19	M50	PX	-0.001384	-0.001384	0	0
20	M51	PX	-0.001384	-0.001384	0	0
21	M40	PX	-0.008304	-0.008304	0	0
22	M41	PX	-0.008304	-0.008304	0	0
23	M42	PX	-0.008304	-0.008304	0	0
24	M43	PX	-0.008304	-0.008304	0	0
25	M44	PX	-0.008304	-0.008304	0	0
26	M45	PX	-0.008304	-0.008304	0	0
27	M46	PX	-0.008304	-0.008304	0	0
28	M47	PX	-0.008304	-0.008304	0	0
29	M20	PX	-0.002768	-0.002768	0	0
30	M21	PX	-0.002768	-0.002768	0	0
31	M22	PX	-0.002768	-0.002768	0	0
32	M23	PX	-0.002768	-0.002768	0	0
33	M24	PX	-0.002768	-0.002768	0	0
34	M25	PX	-0.002768	-0.002768	0	0
35	M26	PX	-0.002768	-0.002768	0	0
36	M27	PX	-0.002768	-0.002768	0	0
37	M28	PX	-0.002768	-0.002768	0	0
38	M29	PX	-0.002768	-0.002768	0	0
39	M30	PX	-0.002768	-0.002768	0	0
40	M31	PX	-0.002768	-0.002768	0	0
41	M32	PX	-0.002768	-0.002768	0	0
42	M33	PX	-0.002768	-0.002768	0	0
43	M34	PX	-0.002768	-0.002768	0	0
44	M35	PX	-0.002768	-0.002768	0	0
45	M36	PX	-0.002768	-0.002768	0	0
46	M37	PX	-0.002768	-0.002768	0	0
47	M38	PX	-0.002768	-0.002768	0	0
48	M39	PX	-0.002768	-0.002768	0	0

**Member Distributed Loads (BLC 9 : DEAD LOAD (ICE))**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in, %]	End Location[in, %]
1	M9	Y	-0.009	-0.009	0	0
2	M10	Y	-0.009	-0.009	0	0
3	M11	Y	-0.008	-0.008	0	0
4	M12	Y	-0.008	-0.008	0	0
5	M13	Y	-0.008	-0.008	0	0
6	M14	Y	-0.008	-0.008	0	0
7	M15	Y	-0.008	-0.008	0	0
8	POSITION 4	Y	-0.008	-0.008	0	0
9	POSITION 3	Y	-0.008	-0.008	0	0
10	POSITION 2	Y	-0.008	-0.008	0	0





**Member Distributed Loads (BLC 9 : DEAD LOAD (ICE)) (Continued)**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in.%]	End Location[in.%]
11	POSITION 1	Y	-0.008	-0.008	0	0
12	M56	Y	-0.008	-0.008	0	0
13	M52	Y	-0.005	-0.005	0	0
14	M53	Y	-0.005	-0.005	0	0
15	M54	Y	-0.005	-0.005	0	0
16	M55	Y	-0.005	-0.005	0	0
17	M48	Y	-0.005	-0.005	0	0
18	M49	Y	-0.005	-0.005	0	0
19	M50	Y	-0.005	-0.005	0	0
20	M51	Y	-0.005	-0.005	0	0
21	M40	Y	-0.008	-0.008	0	0
22	M41	Y	-0.008	-0.008	0	0
23	M42	Y	-0.008	-0.008	0	0
24	M43	Y	-0.008	-0.008	0	0
25	M44	Y	-0.008	-0.008	0	0
26	M45	Y	-0.008	-0.008	0	0
27	M46	Y	-0.008	-0.008	0	0
28	M47	Y	-0.008	-0.008	0	0
29	M20	Y	-0.011	-0.011	0	0
30	M21	Y	-0.011	-0.011	0	0
31	M22	Y	-0.011	-0.011	0	0
32	M23	Y	-0.011	-0.011	0	0
33	M24	Y	-0.011	-0.011	0	0
34	M25	Y	-0.011	-0.011	0	0
35	M26	Y	-0.011	-0.011	0	0
36	M27	Y	-0.011	-0.011	0	0
37	M28	Y	-0.011	-0.011	0	0
38	M29	Y	-0.011	-0.011	0	0
39	M30	Y	-0.011	-0.011	0	0
40	M31	Y	-0.011	-0.011	0	0
41	M32	Y	-0.011	-0.011	0	0
42	M33	Y	-0.011	-0.011	0	0
43	M34	Y	-0.011	-0.011	0	0
44	M35	Y	-0.011	-0.011	0	0
45	M36	Y	-0.011	-0.011	0	0
46	M37	Y	-0.011	-0.011	0	0
47	M38	Y	-0.011	-0.011	0	0
48	M39	Y	-0.011	-0.011	0	0

**Member Distributed Loads (BLC 10 : WIND FRONT (ICE))**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in.%]	End Location[in.%]
1	M9	PZ	-0.004	-0.004	0	0
2	M10	PZ	-0.004	-0.004	0	0
3	M11	PZ	-0.004	-0.004	0	0
4	M12	PZ	-0.004	-0.004	0	0
5	M13	PZ	-0.004	-0.004	0	0
6	M14	PZ	-0.004	-0.004	0	0
7	M15	PZ	-0.004	-0.004	0	0
8	POSITION 4	PZ	-0.004	-0.004	0	0
9	POSITION 3	PZ	-0.004	-0.004	0	0
10	POSITION 2	PZ	-0.004	-0.004	0	0
11	POSITION 1	PZ	-0.004	-0.004	0	0
12	M56	PZ	-0.004	-0.004	0	0
13	M52	PZ	-0.003	-0.003	0	0
14	M53	PZ	-0.003	-0.003	0	0
15	M54	PZ	-0.003	-0.003	0	0



**Member Distributed Loads (BLC 10 : WIND FRONT (ICE)) (Continued)**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in, %]	End Location[in, %]
16	M55	PZ	-0.003	-0.003	0	0
17	M48	PZ	-0.003	-0.003	0	0
18	M49	PZ	-0.003	-0.003	0	0
19	M50	PZ	-0.003	-0.003	0	0
20	M51	PZ	-0.003	-0.003	0	0
21	M40	PZ	-0.006	-0.006	0	0
22	M41	PZ	-0.006	-0.006	0	0
23	M42	PZ	-0.006	-0.006	0	0
24	M43	PZ	-0.006	-0.006	0	0
25	M44	PZ	-0.006	-0.006	0	0
26	M45	PZ	-0.006	-0.006	0	0
27	M46	PZ	-0.006	-0.006	0	0
28	M47	PZ	-0.006	-0.006	0	0
29	M20	PZ	-0.005	-0.005	0	0
30	M21	PZ	-0.005	-0.005	0	0
31	M22	PZ	-0.005	-0.005	0	0
32	M23	PZ	-0.005	-0.005	0	0
33	M24	PZ	-0.005	-0.005	0	0
34	M25	PZ	-0.005	-0.005	0	0
35	M26	PZ	-0.005	-0.005	0	0
36	M27	PZ	-0.005	-0.005	0	0
37	M28	PZ	-0.005	-0.005	0	0
38	M29	PZ	-0.005	-0.005	0	0
39	M30	PZ	-0.005	-0.005	0	0
40	M31	PZ	-0.005	-0.005	0	0
41	M32	PZ	-0.005	-0.005	0	0
42	M33	PZ	-0.005	-0.005	0	0
43	M34	PZ	-0.005	-0.005	0	0
44	M35	PZ	-0.005	-0.005	0	0
45	M36	PZ	-0.005	-0.005	0	0
46	M37	PZ	-0.005	-0.005	0	0
47	M38	PZ	-0.005	-0.005	0	0
48	M39	PZ	-0.005	-0.005	0	0

**Member Distributed Loads (BLC 11 : WIND SIDE (ICE))**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in, %]	End Location[in, %]
1	M9	PX	-0.004	-0.004	0	0
2	M10	PX	-0.004	-0.004	0	0
3	M11	PX	-0.004	-0.004	0	0
4	M12	PX	-0.004	-0.004	0	0
5	M13	PX	-0.004	-0.004	0	0
6	M14	PX	-0.004	-0.004	0	0
7	M15	PX	-0.004	-0.004	0	0
8	POSITION 4	PX	-0.004	-0.004	0	0
9	POSITION 3	PX	-0.004	-0.004	0	0
10	POSITION 2	PX	-0.004	-0.004	0	0
11	POSITION 1	PX	-0.004	-0.004	0	0
12	M56	PX	-0.004	-0.004	0	0
13	M52	PX	-0.003	-0.003	0	0
14	M53	PX	-0.003	-0.003	0	0
15	M54	PX	-0.003	-0.003	0	0
16	M55	PX	-0.003	-0.003	0	0
17	M48	PX	-0.003	-0.003	0	0
18	M49	PX	-0.003	-0.003	0	0
19	M50	PX	-0.003	-0.003	0	0
20	M51	PX	-0.003	-0.003	0	0



**Member Distributed Loads (BLC 11 : WIND SIDE (ICE)) (Continued)**

	Member Label	Direction	Start Magnitude[k/ft...	End Magnitude[k/ft...	Start Location[in,%]	End Location[in,%]
21	M40	PX	-0.006	-0.006	0	0
22	M41	PX	-0.006	-0.006	0	0
23	M42	PX	-0.006	-0.006	0	0
24	M43	PX	-0.006	-0.006	0	0
25	M44	PX	-0.006	-0.006	0	0
26	M45	PX	-0.006	-0.006	0	0
27	M46	PX	-0.006	-0.006	0	0
28	M47	PX	-0.006	-0.006	0	0
29	M20	PX	-0.005	-0.005	0	0
30	M21	PX	-0.005	-0.005	0	0
31	M22	PX	-0.005	-0.005	0	0
32	M23	PX	-0.005	-0.005	0	0
33	M24	PX	-0.005	-0.005	0	0
34	M25	PX	-0.005	-0.005	0	0
35	M26	PX	-0.005	-0.005	0	0
36	M27	PX	-0.005	-0.005	0	0
37	M28	PX	-0.005	-0.005	0	0
38	M29	PX	-0.005	-0.005	0	0
39	M30	PX	-0.005	-0.005	0	0
40	M31	PX	-0.005	-0.005	0	0
41	M32	PX	-0.005	-0.005	0	0
42	M33	PX	-0.005	-0.005	0	0
43	M34	PX	-0.005	-0.005	0	0
44	M35	PX	-0.005	-0.005	0	0
45	M36	PX	-0.005	-0.005	0	0
46	M37	PX	-0.005	-0.005	0	0
47	M38	PX	-0.005	-0.005	0	0
48	M39	PX	-0.005	-0.005	0	0

**Member Area Loads**

Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[ksf]
No Data to Print ...						

**Envelope Member End Reactions**

Member	Member End		Axial[k]	LC	y Shear...	LC	z Shear...	LC	Torque[k-ft]	LC	y-y Mo...	LC	z-z Mo...	LC	
1	M1	I	max	1.183	67	-0.065	45	.203	65	-0.024	21	.069	24	-.06	48
2			min	-.182	24	-.493	61	.01	22	-.097	61	-.166	67	-.338	61
3		J	max	1.183	67	-0.065	45	.203	65	-0.024	21	.098	25	.016	14
4			min	-.182	24	-.493	61	.01	22	-.097	61	-.098	19	-.016	32
5	M2	I	max	0	56	.003	56	0	62	0	54	0	56	.002	56
6			min	0	62	.001	25	0	56	0	61	0	62	0	25
7		J	max	0	56	.003	56	0	62	0	54	0	56	0	59
8			min	0	62	.001	25	0	56	0	61	0	62	0	24
9	M3	I	max	0	56	.003	56	0	56	0	66	0	62	.002	56
10			min	0	62	.001	25	0	62	0	19	0	56	0	25
11		J	max	0	56	.003	56	0	56	0	66	0	62	0	65
12			min	0	62	.001	25	0	62	0	19	0	56	0	23
13	M4	I	max	2.389	57	.79	59	-.069	28	.034	63	.098	25	.088	63
14			min	.196	26	.281	29	-.4	59	.005	44	-.098	19	.022	22
15		J	max	2.389	57	.79	59	-.069	28	.034	63	.025	26	-.162	28
16			min	.196	26	.281	29	-.4	59	.005	44	-.3	57	-.454	59
17	M5	I	max	.829	30	-.099	49	.033	29	-.022	24	.271	12	-.057	44
18			min	-1.543	12	-.642	66	-.214	60	-.147	56	-.185	30	-.391	65
19		J	max	.829	30	-.099	49	.033	29	-.022	24	.181	25	.051	56

**Envelope Member End Reactions (Continued)**

Member	Member End		Axial[k]	LC	y Shear...	LC	z Shear...	LC	Torque[k-ft]	LC	y-y Mo...	LC	z-z Mo...	LC
20		min	-1.543	12	-.642	66	-.214	60	-.147	56	-.184	7	-.008	25
21	M6	max	0	56	.003	56	0	62	0	54	0	56	.002	56
22		min	0	62	.001	25	0	56	0	59	0	62	0	25
23		max	0	56	.003	56	0	62	0	54	0	56	0	60
24		min	0	62	.001	25	0	56	0	59	0	62	0	20
25	M7	max	0	56	.003	56	0	56	0	67	0	62	.002	56
26		min	0	62	.001	25	0	62	0	44	0	56	0	25
27		max	0	56	.003	56	0	56	0	67	0	62	0	63
28		min	0	62	.001	25	0	62	0	44	0	56	0	24
29	M8	max	.099	20	1.175	56	.403	66	.1	56	.181	25	.118	67
30		min	-2.394	64	.284	26	.046	22	0	25	-.184	7	.021	24
31		max	.099	20	1.175	56	.403	66	.1	56	.304	62	-.164	29
32		min	-2.394	64	.284	26	.046	22	0	25	-.113	19	-.687	58
33	M9	max	0	67	0	67	0	67	0	67	0	67	0	67
34		min	0	1	0	1	0	1	0	1	0	1	0	1
35		max	0	67	0	67	0	67	0	67	0	67	0	67
36		min	0	1	0	1	0	1	0	1	0	1	0	1
37	M10	max	0	67	0	67	0	67	0	67	0	67	0	67
38		min	0	1	0	1	0	1	0	1	0	1	0	1
39		max	0	67	.375	4	0	67	0	67	0	67	0	67
40		min	0	1	0	1	0	1	0	1	0	1	0	1
41	M11	max	.497	23	.026	67	.014	24	.064	30	0	67	0	67
42		min	-.508	17	.007	19	-.014	18	-.071	12	0	1	0	1
43		max	.504	23	-.007	54	.014	30	.064	30	0	67	0	67
44		min	-.515	17	-.026	56	-.014	12	-.071	12	0	1	0	1
45	M12	max	1.332	57	1.156	59	.215	57	.057	63	.041	25	-.08	29
46		min	-.151	26	.103	30	-.035	26	.012	19	-.052	7	-.206	59
47		max	1.653	58	-.023	22	.02	26	.031	62	.061	8	.281	60
48		min	.166	27	-1.018	67	-.201	57	.008	43	-.056	26	.048	30
49	M13	max	.686	67	.635	64	.06	24	-.023	21	.051	7	-.032	47
50		min	-.243	24	.044	54	-.105	67	-.09	61	-.042	25	-.165	61
51		max	.857	66	.048	53	.088	67	-.019	21	.068	12	.177	64
52		min	.049	48	-.454	66	-.043	24	-.077	61	-.055	30	.038	19
53	M14	max	.656	30	.327	57	.19	12	-.021	24	.081	19	-.036	46
54		min	-.795	12	-.007	64	-.16	30	-.115	67	-.089	13	-.208	65
55		max	.08	67	.437	67	.132	30	-.017	24	.148	24	.147	57
56		min	-.45	60	-.016	17	-.15	12	-.106	67	-.162	18	.033	24
57	M15	max	.375	20	.246	18	.083	20	.085	56	.102	13	-.081	28
58		min	-.717	14	-.106	62	-.164	14	.008	25	-.092	19	-.354	56
59		max	.022	58	.672	67	.123	15	.082	56	.096	20	.148	67
60		min	-.452	15	-.101	10	-.054	21	.004	25	-.098	14	.047	24
61	POSITION...	max	0	67	0	21	0	18	0	67	0	67	0	67
62		min	0	1	0	65	0	60	0	1	0	1	0	1
63		max	0	67	0	59	0	58	0	67	0	67	0	67
64		min	0	1	0	16	0	16	0	1	0	1	0	1
65	POSITION...	max	0	67	0	10	0	7	0	67	0	67	0	67
66		min	0	1	0	61	0	62	0	1	0	1	0	1
67		max	0	67	0	59	0	57	0	67	0	67	0	67
68		min	0	1	0	28	0	14	0	1	0	1	0	1
69	POSITION...	max	0	67	0	10	.001	7	0	67	0	67	0	67
70		min	0	1	0	65	-.001	13	0	1	0	1	0	1
71		max	0	67	0	59	0	7	0	67	0	67	0	67
72		min	0	1	0	16	0	13	0	1	0	1	0	1
73	POSITION...	max	.375	54	0	22	0	32	0	67	0	67	0	67
74		min	0	1	-.002	40	0	14	0	1	0	1	0	1
75		max	0	67	0	59	0	18	0	67	0	67	0	67
76		min	0	1	0	28	0	62	0	1	0	1	0	1



**Envelope Member End Reactions (Continued)**

Member	Member End		Axial[k]	LC	y Shear...	LC	z Shear...	LC	Torque[k-ft]	LC	y-y Mo...	LC	z-z Mo...	LC	
77	M20	I	max	0	25	0	62	-.001	54	0	67	0	67	0	62
78			min	0	7	0	56	-.003	56	0	1	0	19	0	56
79		J	max	0	25	0	56	.003	67	0	67	0	67	0	62
80			min	0	7	0	62	.001	19	0	1	0	19	0	56
81	M21	I	max	0	25	0	62	-.001	54	0	67	0	67	0	62
82			min	0	7	0	56	-.003	56	0	1	0	19	0	56
83		J	max	0	25	0	56	.003	67	0	67	0	67	0	62
84			min	0	7	0	62	.001	19	0	1	0	19	0	56
85	M22	I	max	1.022	67	.136	24	-.064	45	-.081	22	-.008	49	.069	24
86			min	-.122	24	-.624	67	-.481	61	-.293	61	-.195	57	-.166	67
87		J	max	1.022	67	.136	24	-.063	45	-.081	22	-.016	49	.055	25
88			min	-.122	24	-.623	67	-.479	61	-.293	61	-.25	61	-.099	7
89	M23	I	max	2.068	57	1.253	57	-.279	29	.317	60	-.109	27	.3	57
90			min	.205	26	.061	26	-.769	59	.115	30	-.333	56	-.025	26
91		J	max	2.067	57	1.252	57	-.278	29	.317	60	-.142	27	.155	56
92			min	.205	26	.062	26	-.767	59	.115	30	-.42	56	-.036	25
93	M24	I	max	0	13	0	62	-.001	54	0	67	0	67	0	62
94			min	0	19	0	56	-.003	56	0	1	0	19	0	56
95		J	max	0	13	0	56	.003	67	0	67	0	67	0	62
96			min	0	19	0	62	.001	19	0	1	0	19	0	56
97	M25	I	max	0	25	0	62	-.001	54	0	67	0	67	0	62
98			min	0	7	0	56	-.003	56	0	1	0	19	0	56
99		J	max	0	25	0	56	.003	67	0	67	0	67	0	62
100			min	0	7	0	62	.001	19	0	1	0	19	0	56
101	M26	I	max	.645	30	.883	12	-.098	50	-.082	24	.006	43	.271	12
102			min	-1.276	12	-.519	30	-.646	66	-.364	66	-.204	64	-.185	30
103		J	max	.645	30	.883	12	-.097	50	-.082	24	-.005	43	.171	13
104			min	-1.276	12	-.519	30	-.643	66	-.364	66	-.279	64	-.128	19
105	M27	I	max	.029	20	.118	20	-.287	26	.521	56	-.107	19	.113	19
106			min	-2.081	64	-1.259	61	-1.19	56	.111	26	-.466	59	-.304	62
107		J	max	.029	20	.118	20	-.286	26	.521	56	-.141	19	.106	19
108			min	-2.081	64	-1.259	61	-1.187	56	.111	26	-.601	59	-.191	13
109	M28	I	max	.981	18	.579	18	.4	57	-.055	24	0	67	0	67
110			min	-.934	24	-.548	24	.05	48	-.261	57	0	1	0	1
111		J	max	.982	18	.579	18	.404	57	-.055	24	.077	57	.104	24
112			min	-.934	24	-.548	24	.051	48	-.261	57	.01	48	-.11	18
113	M29	I	max	.592	20	.378	14	.567	66	.252	67	0	67	0	67
114			min	-.659	14	-.336	20	.199	23	.059	24	0	1	0	1
115		J	max	.592	20	.379	14	.571	66	.252	67	.108	66	.064	20
116			min	-.659	14	-.336	20	.2	23	.059	24	.038	23	-.072	14
117	M30	I	max	.362	30	.209	30	.584	64	-.064	20	0	67	0	67
118			min	-.403	12	-.237	12	.065	54	-.269	64	0	1	0	1
119		J	max	.362	30	.209	30	.588	64	-.064	20	.111	64	.045	12
120			min	-.403	12	-.237	12	.066	54	-.269	64	.013	54	-.04	30
121	M31	I	max	.416	8	.19	26	1.189	59	.352	60	0	67	0	67
122			min	-.332	26	-.245	8	.191	29	.064	30	0	1	0	1
123		J	max	.416	8	.191	26	1.192	59	.352	60	.227	59	.047	8
124			min	-.332	26	-.245	8	.193	29	.064	30	.037	29	-.036	26
125	M32	I	max	.145	18	.267	18	.711	57	-.034	24	-.015	24	.106	24
126			min	-.051	24	-.222	24	.166	24	-.162	57	-.071	57	-.112	18
127		J	max	.145	18	.267	18	.715	57	-.034	24	.065	57	.148	24
128			min	-.051	24	-.222	24	.167	24	-.162	57	.008	48	-.163	18
129	M33	I	max	.001	30	.077	30	1.002	64	-.039	20	-.018	19	.045	12
130			min	-.262	56	-.121	12	.173	43	-.158	64	-.087	64	-.041	30
131		J	max	.001	30	.078	30	1.006	64	-.039	20	.104	64	.069	12
132			min	-.262	56	-.122	12	.175	43	-.158	64	.01	54	-.055	30
133	M34	I	max	-.035	20	.155	63	1.777	59	.189	60	-.02	30	.048	8



Company : USA Engineering, Inc.  
 Designer : Nicole Hoffman, EI  
 Job Number :  
 Model Name : Site-Pro #VFA12-HD

Mar 31, 2022  
 9:12 AM  
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**Envelope Member End Reactions (Continued)**

Member	Member End		Axial[k]	LC	y Shear...	LC	z Shear...	LC	Torque[k-ft]	LC	y-y Mo...	LC	z-z Mo...	LC	
134		min	-.503	65	-.078	20	.279	29	.036	30	-.129	60	-.036	26	
135	J	max	-.035	20	.155	63	1.781	59	.189	60	.21	59	.062	8	
136		min	-.503	65	-.078	20	.28	29	.036	30	.032	29	-.057	26	
137	M35	I	max	.164	8	.133	26	.654	67	.153	56	-.019	24	.064	20
138		min	-.022	64	-.173	8	.283	24	.032	24	-.053	67	-.074	14	
139	J	max	.164	8	.133	26	.658	67	.153	56	.072	66	.096	20	
140		min	-.022	64	-.173	8	.284	24	.032	24	.033	23	-.098	14	
141	M36	I	max	.034	20	.119	20	-.548	30	.253	56	.053	56	.103	19
142		min	-1.705	61	-1.108	61	-2.717	56	.049	26	.002	26	-.184	13	
143	J	max	.034	20	.118	20	-.548	30	.253	56	-.056	19	.092	19	
144		min	-1.705	61	-1.107	61	-2.715	56	.049	26	-.264	59	-.101	13	
145	M37	I	max	.375	30	.701	12	-.192	54	-.043	24	.052	67	.164	13
146		min	-.816	12	-.409	30	-1.487	65	-.205	67	.007	25	-.123	19	
147	J	max	.375	30	.701	12	-.191	54	-.043	24	-.002	54	.089	13	
148		min	-.816	12	-.409	30	-1.485	65	-.205	67	-.121	64	-.081	19	
149	M38	I	max	.656	57	.117	24	-.168	47	-.044	21	.026	63	.053	25
150		min	-.078	24	-.465	57	-1.05	61	-.162	61	.006	22	-.096	7	
151	J	max	.656	57	.117	24	-.167	47	-.044	21	-.004	48	.042	25	
152		min	-.078	24	-.465	57	-1.047	61	-.162	61	-.096	61	-.05	7	
153	M39	I	max	1.043	57	.865	57	-.563	26	.153	62	.009	15	.147	56
154		min	.133	26	.035	26	-1.168	59	.052	19	-.017	56	-.035	25	
155	J	max	1.043	57	.864	57	-.563	26	.153	62	-.058	27	.052	7	
156		min	.134	26	.036	26	-1.166	59	.052	19	-.151	56	-.04	25	
157	M40	I	max	.898	30	.307	57	.049	18	-.01	24	.002	12	.171	57
158		min	-.95	12	.06	24	-.048	24	-.051	57	-.002	30	.04	24	
159	J	max	.9	30	.303	57	.05	18	-.01	24	.016	18	.065	63	
160		min	-.952	12	.059	24	-.049	24	-.051	57	-.015	24	.001	48	
161	M41	I	max	.391	18	.415	64	.018	30	-.012	20	0	24	.223	64
162		min	-.336	24	.072	19	-.019	12	-.047	64	-.001	56	.043	43	
163	J	max	.393	18	.411	64	.019	30	-.012	20	.006	30	.078	64	
164		min	-.339	24	.071	19	-.02	12	-.047	64	-.006	12	.006	54	
165	M42	I	max	.778	57	.588	62	.014	26	.05	62	.003	56	.388	59
166		min	-.236	26	.074	19	-.024	57	.01	30	0	26	.064	29	
167	J	max	.779	57	.583	62	.015	26	.05	62	.005	26	.184	59	
168		min	-.238	26	.073	19	-.024	57	.01	30	-.007	8	.034	29	
169	M43	I	max	.482	20	.162	7	.034	14	.047	56	0	20	.184	67
170		min	-.734	14	.022	50	-.026	20	.008	24	-.002	14	.065	23	
171	J	max	.485	20	.16	7	.035	14	.047	56	.01	14	.156	66	
172		min	-.737	14	.02	50	-.027	20	.008	24	-.009	20	.036	22	
173	M44	I	max	.054	21	1.535	58	.005	22	.049	56	.007	18	-.085	20
174		min	-.529	15	.261	30	-.029	65	.008	25	-.004	24	-.268	56	
175	J	max	.056	21	1.531	58	.004	21	.049	56	.003	19	-.16	30	
176		min	-.532	15	.26	30	-.029	64	.008	25	-.007	13	-.702	58	
177	M45	I	max	.291	30	-.093	43	.027	12	-.009	24	.006	30	-.033	43
178		min	-.494	12	-.843	65	-.018	30	-.047	67	-.007	12	-.364	65	
179	J	max	.289	30	-.094	43	.026	11	-.009	24	.003	9	-.007	47	
180		min	-.492	12	-.847	65	-.017	29	-.047	67	-.002	27	-.139	61	
181	M46	I	max	.515	66	.577	57	.004	23	-.01	21	.002	14	-.008	54
182		min	-.057	23	.105	48	-.018	67	-.037	61	-.001	20	-.152	64	
183	J	max	.516	66	.573	57	.004	24	-.01	21	.002	24	-.038	48	
184		min	-.059	23	.104	48	-.019	67	-.037	61	-.004	67	-.303	61	
185	M47	I	max	1.1	58	-.192	3	.043	57	.029	62	.001	26	-.161	28
186		min	.057	27	-.455	2	.002	26	.009	19	-.008	57	-.434	56	
187	J	max	1.099	58	-.193	3	.042	57	.029	62	.005	62	-.079	29	
188		min	.06	27	-.456	2	.003	27	.009	19	0	30	-.318	59	
189	M48	I	max	.592	64	.005	65	.005	62	0	29	0	67	0	67
190		min	-.067	18	-.005	59	-.005	56	0	59	0	1	0	1	

**Envelope Member End Reactions (Continued)**

Member	Member End		Axial[k]	LC	y Shear...	LC	z Shear...	LC	Torque[k-ft]	LC	y-y Mo...	LC	z-z Mo...	LC	
191		J	max	.613	64	.005	59	.005	56	0	29	0	67	0	67
192			min	-.063	30	-.005	65	-.005	62	0	59	0	1	0	1
193	M49	I	max	.559	66	.005	65	.005	62	0	26	0	67	0	67
194			min	-.029	18	-.005	59	-.005	56	0	4	0	1	0	1
195		J	max	.58	66	.005	59	.005	56	0	26	0	67	0	67
196			min	-.026	30	-.005	65	-.005	62	0	4	0	1	0	1
197	M50	I	max	1.179	59	.005	65	.005	62	0	28	0	67	0	67
198			min	-.091	18	-.005	59	-.005	56	0	4	0	1	0	1
199		J	max	1.2	59	.005	59	.005	56	0	28	0	67	0	67
200			min	-.087	18	-.005	65	-.005	62	0	4	0	1	0	1
201	M51	I	max	1.091	66	.005	65	.005	62	0	28	0	67	0	67
202			min	-.051	10	-.005	59	-.005	56	0	4	0	1	0	1
203		J	max	1.112	66	.005	59	.005	56	0	28	0	67	0	67
204			min	-.048	22	-.005	65	-.005	62	0	4	0	1	0	1
205	M52	I	max	-.165	54	.013	67	.005	64	0	29	0	67	0	67
206			min	-1.853	64	-.003	23	-.005	8	-.002	60	0	1	0	1
207		J	max	-.161	54	.003	24	.005	58	0	29	0	67	0	67
208			min	-1.834	65	-.013	66	-.005	14	-.002	60	0	1	0	1
209	M53	I	max	.922	57	.013	60	.005	63	0	25	0	67	0	67
210			min	0	56	-.003	29	-.005	8	-.002	57	0	1	0	1
211		J	max	.897	57	.003	30	.005	58	0	25	0	67	0	67
212			min	0	56	-.013	60	-.005	14	-.002	57	0	1	0	1
213	M54	I	max	.817	2	.007	15	.005	30	0	20	0	67	0	67
214			min	0	3	-.003	20	-.005	11	0	14	0	1	0	1
215		J	max	.81	2	.003	21	.005	24	0	20	0	67	0	67
216			min	0	3	-.007	14	-.005	17	0	14	0	1	0	1
217	M55	I	max	-.437	30	.013	58	.005	60	0	27	0	67	0	67
218			min	-3.487	56	-.003	26	-.005	17	-.001	4	0	1	0	1
219		J	max	-.44	30	.003	27	.005	67	0	27	0	67	0	67
220			min	-3.505	56	-.013	57	-.005	11	-.001	4	0	1	0	1
221	M56	I	max	.593	30	.092	67	.047	23	.025	4	0	67	0	67
222			min	-.609	12	.024	19	-.047	17	-.011	26	0	1	0	1
223		J	max	.618	30	-.024	54	.047	29	.025	4	0	67	0	67
224			min	-.634	12	-.092	56	-.047	11	-.011	26	0	1	0	1

**Envelope Joint Reactions**

	Joint		X [k]	LC	Y [k]	LC	Z [k]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N14	max	.485	23	1.852	56	.577	19	0	67	0	67	0	67
2		min	-1.115	66	.432	26	-2.659	62	0	1	0	1	0	1
3	N9	max	1.036	4	1.248	63	2.549	56	0	67	0	67	0	67
4		min	-.119	28	.427	20	.052	25	0	1	0	1	0	1
5	N60	max	0	67	0	67	0	67	0	67	0	67	0	67
6		min	0	1	0	1	0	1	0	1	0	1	0	1
7	N61	max	.401	30	.093	62	.473	30	.018	4	0	67	.018	4
8		min	-.411	12	.023	30	-.484	12	-.008	26	0	1	-.008	26
9	N62	max	.392	23	.026	65	.316	23	-.049	30	0	67	.041	30
10		min	-.403	17	.007	24	-.321	17	-.055	12	0	1	-.046	12
11	Totals:	max	1.33	22	3.094	67	2.247	19						
12		min	-1.33	16	.915	19	-2.247	13						



Company : USA Engineering, Inc.  
 Designer : Nicole Hoffman, EI  
 Job Number :  
 Model Name : Site-Pro #VFA12-HD

Mar 31, 2022  
 9:12 AM  
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**Envelope AISC 15th(360-16): LRFD Steel Code Checks**

Member	Shape	Code	Loc[in]	LC	Shear	Ch...	Loc[in]	Dir	LC	phi*Pnc	phi*Pnt	phi*Mn	phi*Mn	Cb	Eqn
1	M27	.625 x 3.5"	.693	1.393	62	.831	1.393	y	56	70.653	70.875	.923	5.168	1...	H1-1b
2	M26	.625 x 3.5"	.322	1.396	64	.571	0	y	64	70.652	70.875	.923	5.168	1...	H1-1b
3	M31	.625 x 3.5"	.252	2.284	58	.541	2.284	y	60	70.279	70.875	.923	5.168	1...	H1-1b
4	M23	.625 x 3.5"	.499	1.393	56	.526	1.393	y	59	70.653	70.875	.923	5.168	1...	H1-1b
5	M22	.625 x 3.5"	.292	1.396	57	.463	1.396	y	63	70.652	70.875	.923	5.168	1...	H1-1b
6	M36	.625 x 3.5"	.302	1.393	62	.418	1.393	y	56	70.653	70.875	.923	5.168	1...	H1-1b
7	M30	.625 x 3.5"	.123	2.284	64	.413	0	y	64	70.279	70.875	.923	5.168	1...	H1-1b
8	M28	.625 x 3.5"	.089	2.284	57	.403	0	y	57	70.279	70.875	.923	5.168	1...	H1-1b
9	M29	.625 x 3.5"	.124	2.284	64	.387	2.284	y	67	70.279	70.875	.923	5.168	1...	H1-1b
10	M37	.625 x 3.5"	.139	1.396	64	.327	0	y	64	70.652	70.875	.923	5.168	1...	H1-1b
11	M34	.625 x 3.5"	.232	2.284	58	.294	0	y	62	70.279	70.875	.923	5.168	1...	H1-1b
12	M39	.625 x 3.5"	.180	1.393	56	.261	1.393	y	59	70.653	70.875	.923	5.168	1...	H1-1b
13	M38	.625 x 3.5"	.113	1.396	57	.260	1.396	y	63	70.652	70.875	.923	5.168	1...	H1-1b
14	M32	.625 x 3.5"	.083	0	57	.250	0	y	57	70.279	70.875	.923	5.168	1...	H1-1b
15	M33	.625 x 3.5"	.120	2.284	64	.246	0	y	64	70.279	70.875	.923	5.168	1...	H1-1b
16	M44	.625 x 2 Flat	.427	3.4	62	.236	0	y	56	39.75	40.5	.527	1.688	1...	H1-1b
17	M35	.625 x 3.5"	.086	2.284	64	.234	2.284	y	67	70.279	70.875	.923	5.168	1...	H1-1b
18	M45	.625 x 2 Flat	.220	0	64	.190	3.403	y	66	39.749	40.5	.527	1.688	1...	H1-1b
19	M42	.625 x 2 Flat	.244	0	59	.182	0	y	62	39.365	40.5	.527	1.688	1...	H1-1b
20	M40	.625 x 2 Flat	.105	0	62	.167	0	y	57	39.365	40.5	.527	1.688	1...	H1-1b
21	M41	.625 x 2 Flat	.138	0	66	.162	0	y	64	39.365	40.5	.527	1.688	1...	H1-1b
22	M12	PIPE 2.0	.427	2.188	58	.155	0		60	29.81	32.13	1.872	1.872	2...	H1-1b
23	M43	.625 x 2 Flat	.112	0	65	.143	0	y	56	39.365	40.5	.527	1.688	1...	H1-1b
24	M46	.625 x 2 Flat	.188	3.403	57	.143	0	y	63	39.749	40.5	.527	1.688	1...	H1-1b
25	M9	PIPE 2.5	.263	45.3...	17	.120	103.1...		13	41.696	50.715	3.596	3.596	1...	H1-1b
26	M15	PIPE 2.0	.326	2.188	64	.119	28.125		67	29.81	32.13	1.872	1.872	2...	H1-1b
27	M13	PIPE 2.0	.229	2.188	66	.117	0		64	29.81	32.13	1.872	1.872	2...	H1-1b
28	M14	PIPE 2.0	.209	2.188	62	.112	27.813		67	29.81	32.13	1.872	1.872	2...	H1-1b
29	M47	.625 x 2 Flat	.285	0	56	.110	3.4	y	62	39.75	40.5	.527	1.688	1...	H1-1b
30	M10	PIPE 2.5	.209	104....	65	.078	46.875		11	41.696	50.715	3.596	3.596	2...	H1-1b
31	POSITION 4	PIPE 2.0	.110	85	12	.078	85		11	9.837	32.13	1.872	1.872	2...	H1-1b
32	POSITION 1	PIPE 2.0	.278	46.25	13	.067	46.25		18	9.837	32.13	1.872	1.872	3...	H1-1b
33	POSITION 2	PIPE 2.0	.593	45	7	.048	46.25		12	9.837	32.13	1.872	1.872	2...	H1-1b
34	M11	PIPE 2.0	.020	51.4...	23	.045	0		12	25.77	32.13	1.872	1.872	1...	H1-1b*
35	POSITION 3	PIPE 2.0	.222	46.25	7	.042	46.25		11	9.837	32.13	1.872	1.872	2...	H1-1b
36	M56	PIPE 2.0	.212	90.9...	67	.024	0		57	4.279	32.13	1.872	1.872	1...	H1-1b
37	M52	.75 Dia.	.129	0	64	.017	0		59	1.404	14.314	.179	.179	1...	H1-1b*
38	M53	.75 Dia.	.000	0	67	.014	50		57	1.404	14.314	.179	.179	1	H1-1a
39	M51	.625 Dia. H...	.622	21.6...	66	.012	0		4	1.88	9.94	.104	.104	1	H1-1a
40	M49	.625 Dia. H...	.339	21.6...	66	.012	0		4	1.88	9.94	.104	.104	1	H1-1a
41	M48	.625 Dia. H...	.357	21.6...	64	.010	0		59	1.88	9.94	.104	.104	1	H1-1a
42	M55	.75 Dia.	.245	50	56	.010	0		58	1.404	14.314	.179	.179	1...	H1-1a*
43	M50	.625 Dia. H...	.669	21.6...	59	.008	0		59	1.88	9.94	.104	.104	1...	H1-1a
44	M54	.75 Dia.	.000	0	67	.005	50		14	1.404	14.314	.179	.179	1	H1-1a
45	M25	.625 x 3.5"	.000	3.625	62	.000	3.625	z	67	69.385	70.875	.923	5.168	2...	H1-1b
46	M21	.625 x 3.5"	.000	3.625	62	.000	3.625	z	67	69.385	70.875	.923	5.168	2...	H1-1b
47	M24	.625 x 3.5"	.000	0	62	.000	0	z	67	69.385	70.875	.923	5.168	2...	H1-1b
48	M20	.625 x 3.5"	.000	0	62	.000	0	z	67	69.385	70.875	.923	5.168	2...	H1-1b



**APPENDIX D**  
**ADDITIONAL CALCULATIONS**



## Single Bolt - Pinned Connection

### DESIGN PARAMETERS:

#### RISA Reactions

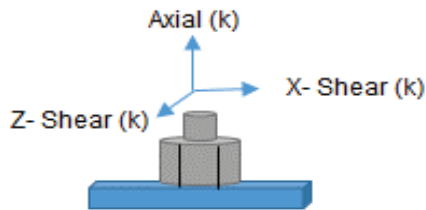
Axial (k)	1.852
X-Shear (k)	2.689
Z-Shear (k)	1.115

#### Bolt Design

Diameter (in)	3/4
Material	Group A
Thread Cond. / Loading	Group A - N / S
Clear Distance, $l_c$ (in)	1.123

#### Connected Element Design

Thickness, $t$ (in)	0.375
Specified Min. Yield Stress, $F_y$ (ksi)	36
Specified Min. Tensile Strength, $F_u$ (ksi)	58



### RESULTS:

#### Shear Bolt Capacity

Design Capacity, $V_{ult}$ (k)	17.9
Required Capacity, $V_{max}$ (k)	2.911
16.3% <b>PASS</b>	

#### Bearing Strength at Bolt Holes

Design Bearing Strength, $\Phi R_n$ (k)	21.9827
13.2% <b>PASS</b>	

#### Tensile Bolt Capacity

Design Capacity, $T_{ult}$ (k)	29.8
Required Capacity, $T_{max}$ (k)	1.852
6.2% <b>PASS</b>	

#### Combined Tensile & Shear

$(T_{max} / T_{ult})^{5/3} + (V_{max} / V_{ult})^{5/3} \leq 1.0$	0.0582
5.8% <b>PASS</b>	

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT  
EVALUATION OF HUMAN EXPOSURE POTENTIAL  
TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CTHA534A

Crown Somers Lattice Tower  
126 Pioneer Heights Road  
Somers, Connecticut 06071

**April 21, 2022**

**EBI Project Number: 6222002669**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general population allowable limit:	<b>28.73%</b>

April 21, 2022

T-Mobile

Attn: Jason Overbey, RF Manager  
35 Griffin Road South  
Bloomfield, Connecticut 06002

Emissions Analysis for Site: CTHA534A - Crown Somers Lattice Tower

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **126 Pioneer Heights Road in Somers, Connecticut** for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately  $400 \mu\text{W}/\text{cm}^2$  and  $467 \mu\text{W}/\text{cm}^2$ , respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## **CALCULATIONS**

Calculations were done for the proposed T-Mobile Wireless antenna facility located at 126 Pioneer Heights Road in Somers, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 LTE channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 1 NR channel (600 MHz Band) was considered for each sector of the proposed installation. This Channel has a transmit power of 80 Watts.
- 3) 2 LTE channels (700 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 4) 2 LTE channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.
- 5) 2 UMTS channels (AWS Band - 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 6) 2 LTE channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.

- 7) 1 LTE Traffic channel (LTE 1C and 2C BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 60 Watts.
- 8) 1 LTE Broadcast channel (LTE 1C and 2C BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 20 Watts.
- 9) 1 NR Traffic channel (BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 120 Watts.
- 10) 1 NR Broadcast channel (BRS Band - 2500 MHz) was considered for each sector of the proposed installation. This Channel has a transmit power of 40 Watts.
- 11) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 12) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 13) The antennas used in this modeling are the Ericsson AIR 6419 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s), the RFS APXVAALL24\_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz channel(s), the Commscope VV-65A-RI for the 1900 MHz / 2100 MHz / 2100 MHz channel(s) in Sector A, the Ericsson AIR 6419 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s), the RFS APXVAALL24\_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz channel(s), the Commscope VV-65A-RI for the 1900 MHz / 2100 MHz / 2100 MHz channel(s) in Sector B, the Ericsson AIR 6419 for the 2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz channel(s), the RFS APXVAALL24\_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz channel(s), the Commscope VV-65A-RI for the 1900 MHz / 2100 MHz / 2100 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all



calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

- 14) The antenna mounting height centerline of the proposed antennas is 114 feet above ground level (AGL).
- 15) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 16) All calculations were done with respect to uncontrolled / general population threshold limits.

## T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Ericsson AIR 6419	Make / Model:	Ericsson AIR 6419	Make / Model:	Ericsson AIR 6419
Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz	Frequency Bands:	2500 MHz / 2500 MHz / 2500 MHz
Gain:	22.05 dBd / 15.55 dBd / 22.05 dBd / 15.55 dBd	Gain:	22.05 dBd / 15.55 dBd / 22.05 dBd / 15.55 dBd	Gain:	22.05 dBd / 15.55 dBd / 22.05 dBd / 15.55 dBd
Height (AGL):	114 feet	Height (AGL):	114 feet	Height (AGL):	114 feet
Channel Count:	4	Channel Count:	4	Channel Count:	4
Total TX Power (W):	240.00 Watts	Total TX Power (W):	240.00 Watts	Total TX Power (W):	240.00 Watts
ERP (W):	31,011.95	ERP (W):	31,011.95	ERP (W):	31,011.95
Antenna A1 MPE %:	<b>9.56%</b>	Antenna B1 MPE %:	<b>9.56%</b>	Antenna C1 MPE %:	<b>9.56%</b>
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	RFS APXVAALL24_43-U-NA20	Make / Model:	RFS APXVAALL24_43-U-NA20	Make / Model:	RFS APXVAALL24_43-U-NA20
Frequency Bands:	600 MHz / 600 MHz / 700 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz	Frequency Bands:	600 MHz / 600 MHz / 700 MHz
Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd	Gain:	12.95 dBd / 12.95 dBd / 13.65 dBd
Height (AGL):	114 feet	Height (AGL):	114 feet	Height (AGL):	114 feet
Channel Count:	5	Channel Count:	5	Channel Count:	5
Total TX Power (W):	200.00 Watts	Total TX Power (W):	200.00 Watts	Total TX Power (W):	200.00 Watts
ERP (W):	4,151.83	ERP (W):	4,151.83	ERP (W):	4,151.83
Antenna A2 MPE %:	<b>3.05%</b>	Antenna B2 MPE %:	<b>3.05%</b>	Antenna C2 MPE %:	<b>3.05%</b>
Antenna #:	3	Antenna #:	3	Antenna #:	3
Make / Model:	Commscope VV-65A-RI	Make / Model:	Commscope VV-65A-RI	Make / Model:	Commscope VV-65A-RI
Frequency Bands:	1900 MHz / 2100 MHz / 2100 MHz	Frequency Bands:	1900 MHz / 2100 MHz / 2100 MHz	Frequency Bands:	1900 MHz / 2100 MHz / 2100 MHz
Gain:	15.55 dBd / 16.05 dBd / 16.05 dBd	Gain:	15.55 dBd / 16.05 dBd / 16.05 dBd	Gain:	15.55 dBd / 16.05 dBd / 16.05 dBd
Height (AGL):	114 feet	Height (AGL):	114 feet	Height (AGL):	114 feet
Channel Count:	6	Channel Count:	6	Channel Count:	6
Total TX Power (W):	300.00 Watts	Total TX Power (W):	300.00 Watts	Total TX Power (W):	300.00 Watts
ERP (W):	11,555.97	ERP (W):	11,555.97	ERP (W):	11,555.97
Antenna A3 MPE %:	<b>3.56%</b>	Antenna B3 MPE %:	<b>3.56%</b>	Antenna C3 MPE %:	<b>3.56%</b>





Site Composite MPE %	
Carrier	MPE %
T-Mobile (Max at Sector A):	16.17%
Dish	0.8%
AT&T	3.06%
Nextel	0.26%
Verizon	7.73%
Sprint	0.71%
Site Total MPE % :	28.73%

T-Mobile MPE % Per Sector	
T-Mobile Sector A Total:	16.17%
T-Mobile Sector B Total:	16.17%
T-Mobile Sector C Total:	16.17%
Site Total MPE % :	28.73%

## T-Mobile Maximum MPE Power Values (Sector A)

T-Mobile Frequency Band / Technology (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
T-Mobile 2500 MHz LTE IC & 2C Traffic	1	9619.47	114.0	29.65	2500 MHz LTE IC & 2C Traffic	1000	2.96%
T-Mobile 2500 MHz LTE IC & 2C Broadcast	1	717.84	114.0	2.21	2500 MHz LTE IC & 2C Broadcast	1000	0.22%
T-Mobile 2500 MHz NR Traffic	1	19238.94	114.0	59.30	2500 MHz NR Traffic	1000	5.93%
T-Mobile 2500 MHz NR Broadcast	1	1435.69	114.0	4.43	2500 MHz NR Broadcast	1000	0.44%
T-Mobile 600 MHz LTE	2	591.73	114.0	3.65	600 MHz LTE	400	0.91%
T-Mobile 600 MHz NR	1	1577.94	114.0	4.86	600 MHz NR	400	1.22%
T-Mobile 700 MHz LTE	2	695.22	114.0	4.29	700 MHz LTE	467	0.92%
T-Mobile 1900 MHz LTE	2	2153.53	114.0	13.28	1900 MHz LTE	1000	1.33%
T-Mobile 2100 MHz UMTS	2	1208.15	114.0	7.45	2100 MHz UMTS	1000	0.74%
T-Mobile 2100 MHz LTE	2	2416.30	114.0	14.90	2100 MHz LTE	1000	1.49%
						<b>Total:</b>	<b>16.17%</b>

• NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.



## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector A:	16.17%
Sector B:	16.17%
Sector C:	16.17%
T-Mobile Maximum MPE % (Sector A):	16.17%
Site Total:	28.73%
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **28.73%** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



**METRO PCS SITE NUMBER: CTHA534A**

**METRO PCS SITE NAME: CROWN SOMERS LATTICE TOWER**

**SITE TYPE: SELF-SUPPORT TOWER**

**TOWER HEIGHT: 161'-5"**

**BUSINESS UNIT #: 806378**

**SITE ADDRESS: 126 PIONEER HEIGHTS RD SOMERS, CT 06071**

**COUNTY: TOLLAND**

**JURISDICTION: CONNECTICUT SITING COUNCIL**

**T-MOBILE ANCHOR SITE CONFIGURATION: 67D5D998E 6160**



**METRO PCS SITE NUMBER: CTHA534A**

**BU #: 806378**

**HRT 086 943248**

**126 PIONEER HEIGHTS RD SOMERS, CT 06071**

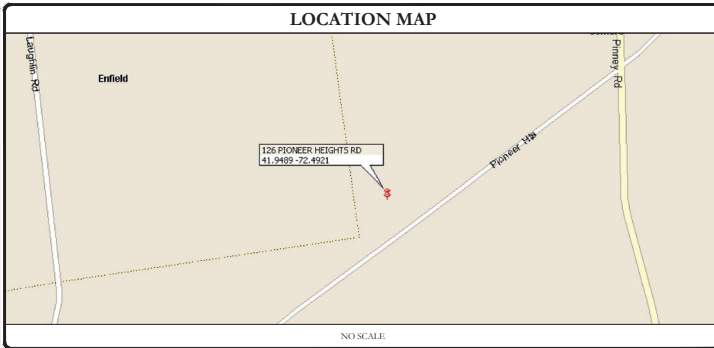
**EXISTING 161'-5" SELF-SUPPORT TOWER**

**ISSUED FOR:**

REV	DATE	BY	DESCRIPTION	DES. QTY
A	4/26/22	ANP	PRELIMINARY REVIEW	NT
0	3/27/22	ANP	CONSTRUCTION	MT

SITE INFORMATION	
CROWN CASTLE USA INC.	HRT 086 943248
SITE NAME:	126 PIONEER HEIGHTS RD SOMERS, CT 06071
SITE ADDRESS:	TOLLAND
COUNTY:	0903129-1814
MAP/PARCEL #:	EXISTING
AREA OF CONSTRUCTION:	419488839
LATITUDE:	-72.4920979
LONGITUDE:	NAD83
LAT/LONG TYPE:	400 FT
GROUND ELEVATION:	A-1
CURRENT ZONING:	CONNECTICUT SITING COUNCIL
JURISDICTION:	OCCUPANCY CLASSIFICATION: U
TYPE OF CONSTRUCTION:	IBB
A.D.A. COMPLIANCE:	FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION
PROPERTY OWNER:	FARNHAM LENA G & FAY GATELY 4017 WASHINGTON RD MCMURRAY, PA 15317
TOWER OWNER:	CROWN CASTLE 200 CORPORATE DRIVE CANONSBURG, PA 15317
CARRIER/APPLICANT:	T-MOBILE 35 GRIFFIN ROAD BLOOMFIELD, CT 06002
ELECTRIC PROVIDER:	NORTHEAST UTILITIES
TELCO PROVIDER:	NOT PROVIDED

DRAWING INDEX	
SHEET #	SHEET DESCRIPTION
T-1	TITLE SHEET
T-2	GENERAL NOTES
C-1.1	OVERALL SITE PLAN
C-1.2	SITE PLAN & ENLARGED SITE PLAN
C-2	FINAL ELEVATION & ANTENNA PLANS
C-3	ANTENNA & CABLE SCHEDULE
C-4	PLUMBING DIAGRAM
C-5	EQUIPMENT SPECS
E-1	AC PANEL SCHEDULES & ONE LINE DIAGRAM
G-1	ANTENNA GROUNDING DIAGRAM
G-2	GROUNDING DETAILS
G-3	GROUNDING DETAILS



ALL DRAWINGS CONTAINED HEREIN ARE FORMATTED FOR CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

PROJECT TEAM	
A&E FIRM:	B+T GROUP 177 S. BOULDER AVE. TULSA, OK 74119 MARVIN PHILLIPS marvin.phillips@btgrp.com
CROWN CASTLE USA INC. DISTRICT CONTACTS:	1200 MACARTHUR BLVD, SUITE 200 MAHWAH, NJ 07430
	TRICIA PELON - PROJECT MANAGER TRICIA.PELON@CROWNCASTLE.COM
	JASON D'AMICO - CONSTRUCTION MANAGER JASON.DAMICO@CROWNCASTLE.COM

PROJECT DESCRIPTION	
THE PURPOSE OF THIS PROJECT IS TO ENHANCE BROADBAND CONNECTIVITY AND CAPACITY TO THE EXISTING ELIGIBLE WIRELESS FACILITY.	
TOWER SCOPE OF WORK:	<ul style="list-style-type: none"> <li>REMOVE (0) ANTENNAS</li> <li>INSTALL (0) ANTENNAS</li> <li>INSTALL (3) RRHs</li> <li>INSTALL (1) 6/24 4WG HYBRID CABLE</li> </ul>
GROUND SCOPE OF WORK:	<ul style="list-style-type: none"> <li>INSTALL (1) RP 663</li> <li>INSTALL (1) RIS 660</li> <li>INSTALL (3) RECTIFIERS FOR 6160 CABINET</li> <li>INSTALL (6) 40 AMP BREAKERS</li> <li>INSTALL (3) ADDITIONAL 50 AMP SPDs</li> <li>INSTALL (1) DRE ROUTER</li> <li>INSTALL (1) PSU-4813 VOLTAGE BOOSTER</li> </ul>

**NOTE:**  
PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN NOC AT (800) 788-7011 & CROWN CONSTRUCTION MANAGER.

APPLICABLE CODES/REFERENCE DOCUMENTS	
ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:	
CODE TYPE	CODE
BUILDING	2015 IBC
MECHANICAL	2015 IMC
ELECTRICAL	2017 NEC
<b>REFERENCE DOCUMENTS:</b>	
STRUCTURAL ANALYSIS:	B+T GROUP
DATED:	4/5/22
MOUNT ANALYSIS:	USA ENGINEERING, INC.
DATED:	4/17/22
RFRS REVISION:	6
DATED:	3/10/22
ORDER ID:	609062
REVISION:	0

APPROVALS		
APPROVAL	SIGNATURE	DATE
PROPERTY OWNER OR REP.	_____	_____
LAND USE PLANNER	_____	_____
T-MOBILE	_____	_____
OPERATIONS	_____	_____
RF	_____	_____
NETWORK	_____	_____
BACKHAUL	_____	_____
CONSTRUCTION MANAGER	_____	_____

**B&T ENGINEERING, INC.**  
PEC.0001564  
Expires 2/10/23

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE SUPERVISION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER:	REVISION:
T-1	0

13629008001\_HRT\_086\_943248.dwg - Sheet1-1 - User: cmhrypoppe - May 27, 2022 - 8:58am

### CROWN CASTLE USA INC. SITE ACTIVITY REQUIREMENTS:

- NOTICE TO PROCEED - NO WORK SHALL COMMENCE PRIOR TO CROWN CASTLE USA INC. WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PERMIT ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN CASTLE USA INC. HOC AT 800-788-2011 & THE CROWN CASTLE USA INC. CONSTRUCTION MANAGER.
- "LOCK UP" - CROWN CASTLE USA INC. SAFETY CLIMB REQUIREMENT: THE INTERIOR OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY. THE STRUCTURE SHALL INCLUDE, BUT NOT BE LIMITED TO FINISHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO LARPER/BLAKE INTERFERE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR CROWN CASTLE USA INC. POC OR CALL THE NCC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.
- PRIOR TO THE START OF CONSTRUCTION ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THESE INCLUDE, BUT NOT ARE LIMITED TO: BUILDING, ELECTRICAL, METEOROLOGICAL, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ON-SITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.
- ALL CONSTRUCTION MEANS AND METHODS, INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONSIDERED HEREIN, AND SHALL MEET ANY APPLICABLE AIDAR (LATEST EDITION), FEDERAL, STATE, AND LOCAL REGULATIONS, AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL REFER TO ANSI/AISE A10.48 (LATEST EDITION) AND CROWN CASTLE USA INC. STANDARD CSD-370-1005, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS III CONSTRUCTION, TO VERIFY THE SUPPORTING STRUCTURES IN ACCORDANCE WITH ANSI/AISE-322 (LATEST EDITION).
- ALL SITE WORK TO COMPLY WITH OAS-STD-1008 "INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON CROWN CASTLE USA INC. TOWER SITES" CSD-1007-1004 "STANDARD FOR INSTALLATION OF MOUNTS AND APPURTENANCES" AND LATEST VERSION OF ANSI/AISE A10.48 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS".
- IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPPOSE AN ALTERNATIVE FOR APPROVAL BY CROWN CASTLE USA INC. PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLIANCE WITH ALL LOCAL ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND COUNTY ORDINANCES AND LOCAL JURISDICTIONAL CODES, REGULATIONS AND APPLICABLE REGULATIONS.
- THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE START OF CONSTRUCTION. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHEN ENCOUNTERED IN THE WORK, SHALL BE PROTECTED BY ALL THE MEANS AND METHODS REQUIRED FOR THE PROPER EXECUTION OF THE WORK. SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING THROUGH OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO (A) FALL PROTECTION BY COVERED SPACE (2) ELECTRICAL SAFETY (3) TRENCHING AND EXCAVATION (3).
- ALL SITE WORK SHALL BE COMPLETED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
- THE AREA OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
- CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE PLANTING, CONSTRUCTION EROSION CONTROL, MEASUREMENT OF REVEALED CHANGES TO BE IN CONFORMANCE WITH THE LOCAL ORDINANCES FOR EROSION AND SEDIMENT CONTROL.
- THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, FURNISHINGS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
- CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COVANA CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITIES. DEBRIS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- CONTRACTOR SHALL MAINTAIN PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.
- NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND, FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

### GREENFIELD GROUNDING NOTES:

- ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER SYSTEMS) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- THE CONTRACTOR SHALL PERFORM IEEE FALL-OR-POTENTIAL RESISTANCE TO EARTH TESTING (SEE IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS. THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUNDING AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.
- METAL CONDUIT AND TRAY SHALL BE GROUNDING AND MAINTAIN ELECTRICAL CONTACT WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CONDUCTORS TO ITS EQUIPMENT.
- EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR ETS; #2 BARE SOLID THINWED COPPER FOR OUTDOOR ETS.
- CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
- ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID THINWED COPPER UNLESS OTHERWISE INDICATED.
- ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- USE OF RPT BONDS IN THE PROTECTION GROUNDING CONNECTIONS SHALL BE AVOIDED WHEN 45 BENS CAN BE ADEQUATELY SUPPORTED.
- EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS COMPS.
- COMPRESSOR GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.
- ICE BRIDGE BONDING CONNECTIONS SHALL BE ELECTRICALLY BONDED OR BONDED TO THE BRIDGE AND THE TOWER GROUND BAR.
- APPROVED ANTI-OXIDANT COMPOUNDS (i.e., CONDUCTIVE GEL) WITH PASTE SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- ALL EXTERIOR GROUND CONNECTIONS SHALL BE MADE WITH A CORROSION RESISTANT MATERIAL.
- MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BODIES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- BOND ALL METALLIC OBJECTS WITHIN A 10' RADIUS OF GROUNDING RING WITH (#2) BARE SOLID THINWED COPPER CONDUCTOR.
- METAL SUPPORT CLIPS OR UNITS THROUGH WALLS OR FLOORS, WHICH IF IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC FLEXIBLE CONDUIT SUCH AS PVC CONDUIT SHALL BE USED.
- GROUND CONNECTIONS USED FOR THE FACILITY OPERATIONS AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE BONDED TO THE GROUND RING.
- ALL GROUNDING SYSTEMS THAT TRANSMIT BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID THINWED COPPER IN 3/4" NON-METALLIC FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3' OF 60-ohm WELDED TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK. (ADD TRANSITIONING GROUNDING STANDARDS AS WELL.)
- BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE PROTECTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS TO THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING TO THE EXISTING GROUNDING SYSTEM. THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/0 COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM AND BUILDING MAIN WATER FEEDERS OR MONITORING MAIN FEEDS (DN-1).

### GENERAL NOTES:

- FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY: CONTRACTOR - GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION. OWNER - CROWN CASTLE USA INC. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKFORCE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
- THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION, THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. THE USER HEREBY RELEASES THE CONTRACTOR FROM ANY AND ALL LIABILITY OF ANY KIND FOR INJURY OR DAMAGE TO PERSONS OR PROPERTY ARISING FROM THE USE OF THESE DRAWINGS. THE CONTRACTOR SHALL NOT BE RESPONSIBLE FOR THE IDENTIFICATION OF NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS, WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES AND SPECIFICATIONS, THE GREATER TAKE PRECEDENCE. FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.
- CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS, THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
- CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS, THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
- ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLIANCE WITH ALL LOCAL ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND COUNTY ORDINANCES AND LOCAL JURISDICTIONAL CODES, REGULATIONS AND APPLICABLE REGULATIONS.
- RECOMMENDED EQUIPMENT SHALL BE SPECIFICALLY STATED OTHERWISE.
- IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPPOSE AN ALTERNATIVE FOR APPROVAL BY CROWN CASTLE USA INC. PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS, THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
- CONTRACTOR SHALL MAINTAIN PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.

### CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 306, ASTM A1184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST-IN-PLACE CONCRETE.
- ALL CONCRETE SHALL BE DESIGNATED AND TESTED FOR DESIGN AND BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSURED TO BE 1000 PSI.
- ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (FC) OF 3000 PSI AT 28 DAYS, UNLESS NOTED OTHERWISE. NO MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90°F AT TIME OF PLACEMENT.
- CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL CONTAIN AN ENTRAINING ADMIXTURE. AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND FC CLASS SPECIFICATIONS (VERY SEVERE). CEMENT USED TO BE TYPE 1 PORTLAND CEMENT WITH A MAXIMUM WATER-TO-CEMENT RATIO (W/C) OF 0.45.
- ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED REINFORCING (WELDS) SHALL CONFORM TO ASTM A1035. ALL STEEL SHALL BE CLASS "M" TENSION GRADES, UNLESS NOTED OTHERWISE. ALL HOOPS SHALL BE STANDARD 90 DEGREE HOOPS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (FY) OF STANDARD DEFORMED BARS ARE AS FOLLOWS: #4 BARS AND SMALLER..... 40 ksi #5 BARS AND LARGER..... 50 ksi #6 THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS: CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH..... 3" #6 BARS AND LARGER..... 3" #5 BARS AND SMALLER..... 1-1/2" CONCRETE NOT EXPOSED TO EARTH OR WEATHER: #4 BARS AND LARGER..... 1-1/2" #5 BARS AND SMALLER..... 1-1/2" #6 BARS AND LARGER..... 1-1/2" #5 BARS AND SMALLER..... 1-1/2" BEAMS AND COLUMNS: #4 BARS AND LARGER..... 1-1/2" #5 BARS AND SMALLER..... 1-1/2" NO TOLERED EDGE OR 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED ENDS OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

### ELECTRICAL INSTALLATION NOTES:

- ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
- CONDUIT ROUTING AND SCHEMATIC: CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED AND TRIP HAZARDS ARE ELIMINATED.
- WIRING: CONTRACTOR SHALL SEGREGATE AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.
- ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.
- ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 ACI MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.2.4 NEC OR THE MOST CURRENT APPROVED CODE THAT GOVERNS THE JURISDICTION.
- END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELLO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OF ELECTRICAL TYPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.
- ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LABELS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT NUMBER (A, PANEL BOARD AND CIRCUIT #S).
- PANEL BORDERS (C NUMBER) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
- ALL WIRING SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.
- ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUIT (#14 OR LARGER) WITH TYPE THINW, THINW-2, SHNW, SHNW-2, THE THIN-2, RW, OR RW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THINW, THINW-2, SHNW, SHNW-2, THE THIN-2, RW, OR RW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STILED, CONDUCTOR WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE FOR OPERATION NOT LESS THAN 70° C (160° F) AVAILABLE.
- POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STILED, CONDUCTOR WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE FOR OPERATION NOT LESS THAN 70° C (160° F) AVAILABLE.
- ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.
- ELECTRICAL METALLIC TUBING (EMT) OR METAL-CLAD CABLE (MCC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- SCHEDULE 40 PVC UNDERGROUND OR STRAIGHTS AND SCHEDULE 80 PVC FOR ALL EMBOWS/BOUNDS AND ALL APPROVED ABOVE GROUND LOCATIONS.
- LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OR SHOCK OR FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) IS NEEDED.
- CONDUIT SHALL BE PROTECTED FROM COMPRESSION-TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
- CONDUIT BODIES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND THE NEC.
- WIREMOUTH SPECIFICS: WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS.
- CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-POROSITY STAMPS AND HANGERS. EMPLOYEE LEVELS 15'-6" ABOVE FINISHED FLOOR LEVELS SHALL BE PROTECTED FROM COLLAPSE BY STRUCTURE. CABLES SHALL BE PERMITTED TO FOLLOW THE LINES OF THE STRUCTURE, MAINLINE CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN SIZE AND/OR TYPE SHALL BE MADE WITH CONDUIT SPLITTING TOOLS TO PREVENT DAMAGE TO THE CONDUIT. CONDUITS SHALL BE MADE WITH CONDUIT SPLITTING TOOLS TO PREVENT CONCRETE, PLASTER OR DRY FROM EXTERIOR CONDUITS. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED STEEL L-BRACKETS OR ANCHORS. CONDUITS SHALL BE MADE WITH CONDUIT SPLITTING TOOLS TO PREVENT DAMAGE TO THE CONDUIT. CONDUITS SHALL BE MADE WITH CONDUIT SPLITTING TOOLS TO PREVENT CONCRETE, PLASTER OR DRY FROM EXTERIOR CONDUITS. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED STEEL L-BRACKETS OR ANCHORS.
- EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPoxy-COATED SHEET STEEL, SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3R (OR BETTER) FOR EXTERIOR LOCATIONS.
- NONMETALLIC RECEPTACLE SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEAREST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WV) OR BETTER FOR EXTERIOR LOCATIONS.
- THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR CROWN CASTLE USA INC. BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
- CONTRACTOR SHALL PROVIDE NECESSARY TAPING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.
- INSTALL LAMICOD LABEL ON THE METER CENTER TO SHOW "METER STOP".
- ALL EMPLOYERS/CONTRACTORS THAT ARE INSTALLED ARE TO HAVE A METERSIDE TAPE PULL CORD INSTALLED.

### APWA UNIFORM COLOR CODE:

SYSTEM	CONDUCTOR	COLOR
120/240V, 10	A PHASE	RED
	B PHASE	BLACK
	NEUTRAL	WHITE
120/208V, 3Ø	A PHASE	RED
	B PHASE	BLACK
	C PHASE	BLUE
277/480V, 3Ø	PHASE	BROWN
	B PHASE	ORANGE OR PURPLE
	C PHASE	YELLOW
DC VOLTAGE		N/A
NEG (-)		RED*
NEG (-)		BLACK**

### ABBREVIATIONS:

- ANT - ANTENNA
- EXT - EXISTING
- FAC - FACILITY INTERFACE FRAME
- GEN - GENERATOR
- GIS - GEOSPATIAL INFORMATION SYSTEM
- GS - GLOBAL SYSTEM FOR MOBILE LOCALIZATION
- HW - HOUSE WIRE
- MC - METAL CLAD
- MCP - METAL CLAD PANEL
- MIP - MOUNTING POINT
- NCC - NATIONAL ELECTRICAL CODE
- OP - OPERATOR
- PF - POWER FACTOR
- PRO - PROTECTED
- REI - RECEPTACLE
- RMS - RADIO FREQUENCY SYSTEM
- RES - REMOTE ELECTRIC TTY
- RFD - RADIO FREQUENCY DATA SHEET
- RWD - REMOTE RADIO WELDED
- RWA - REMOTE RADIO WELDED
- SH - SMART INTEGRATED DEVICE
- TMA - TOWER MOUNTED AMPLIFIER
- TYP - TYPICAL
- UNITS - UNIVERSAL MOBILE TELECOMMUNICATIONS SYSTEM
- W.P. - WORK POINT

**T-Mobile**  
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 BLOOMFIELD, CT 06002

**CROWN CASTLE**  
 1200 MAGARTHUR BLVD, SUITE 200  
 MAHWAH, NJ 07430

**BAT GRP**  
 1772 S. BOULDER  
 SUITE 200  
 TULSA, OK 74119  
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 www.batgrp.com

METRO PCS SITE NUMBER:  
**CTH4534A**

BU #: 806378  
 HRT 086 943248

126 PIONEER HEIGHTS RD  
 SOMERS, CT 06071

EXISTING  
 161'-5" SELF-SUPPORT TOWER

**ISSUED FOR:**

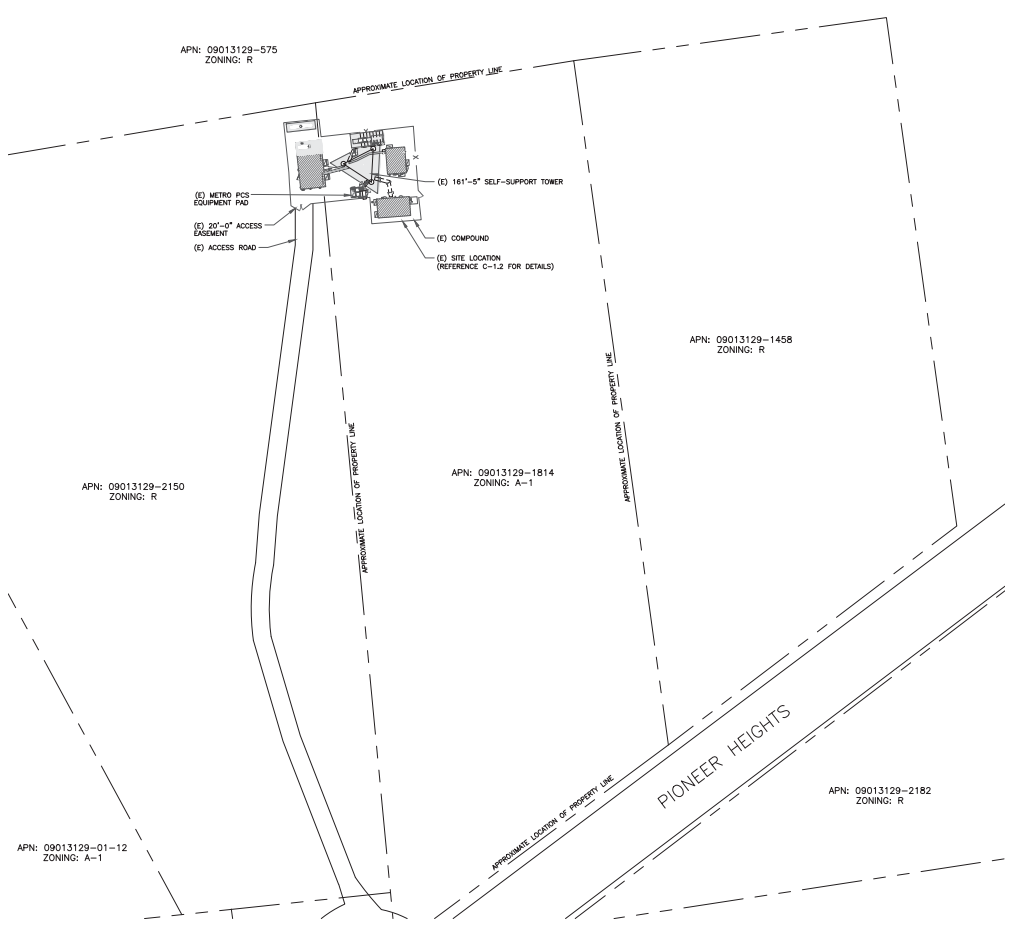
REV.	DATE	BY	DESCRIPTION	DES. OR
A	4/26/22	ANP	PRELIMINARY REVIEW	NOT
B	5/27/22	ANP	CONSTRUCTION	YES

**B&T ENGINEERING, INC.**  
 P.E.C. 00010564  
 Expires 2/10/23

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SHEET NUMBER: **T-2** REVISION: **0**

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METRO PCS SITE NUMBER:  
**CTHA534A**  
 BU #: 806378  
 HRT 086 943248  
 126 PIONEER HEIGHTS RD  
 SOMERS, CT 06071  
 EXISTING  
 161'-5" SELF-SUPPORT TOWER

**ISSUED FOR:**

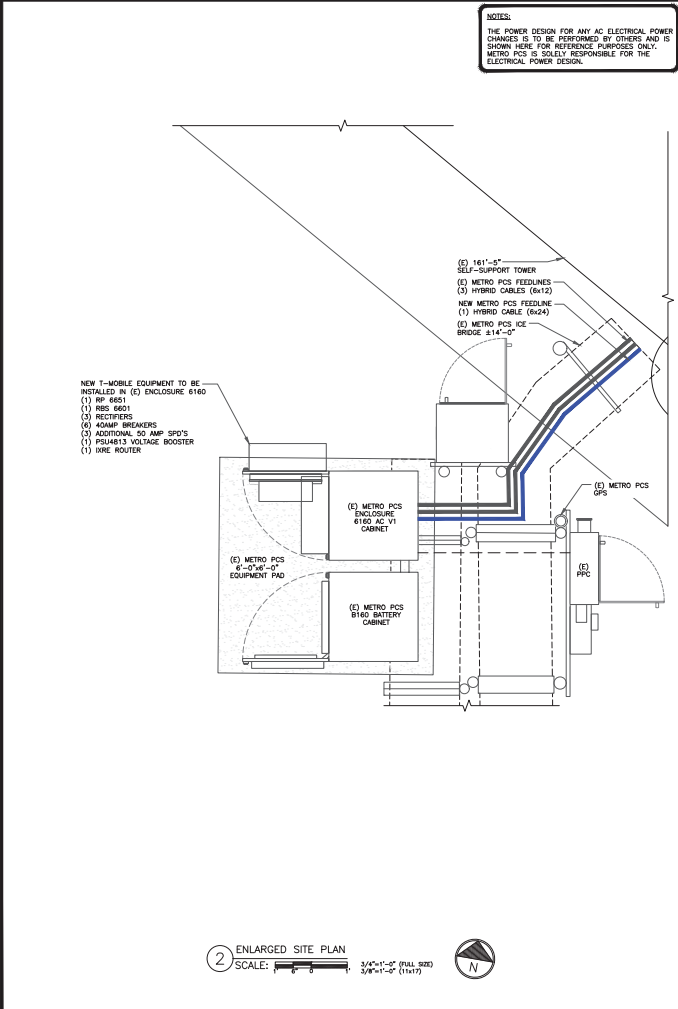
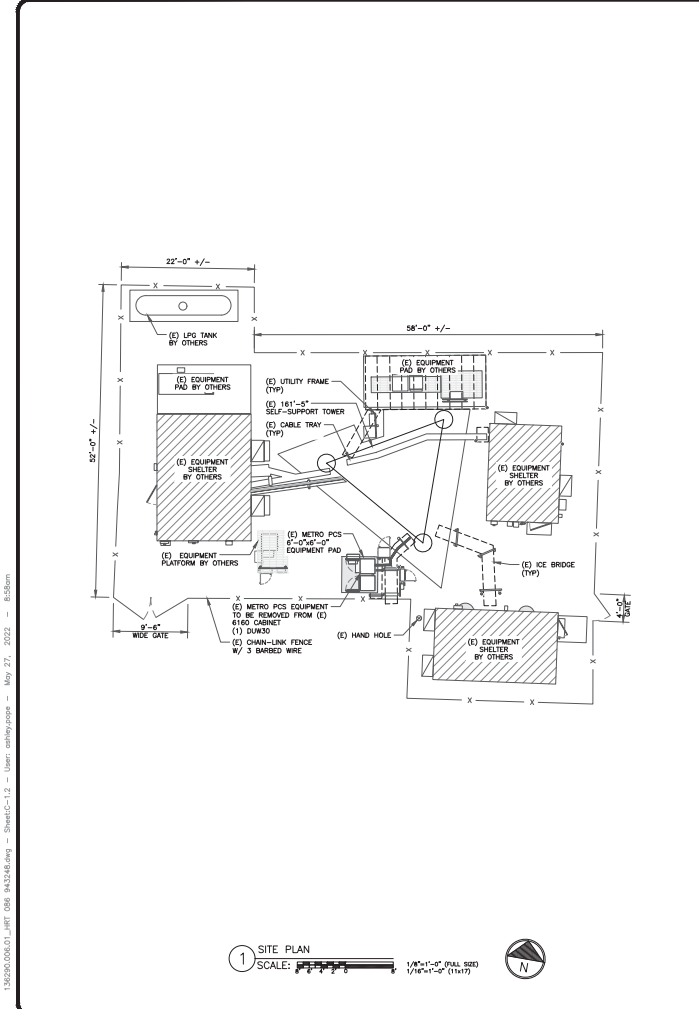
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B	5/27/22	ANP	CONSTRUCTION	BT	



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1 OVERALL SITE PLAN  
 SCALE: 1"=30'-0" (TALL BUD)  
 1"=60'-0" (T1117)  
 N

SHEET NUMBER: **C-1.1** REVISION: **0**



**T-Mobile**  
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METRO PCS SITE NUMBER:  
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**ISSUED FOR:**

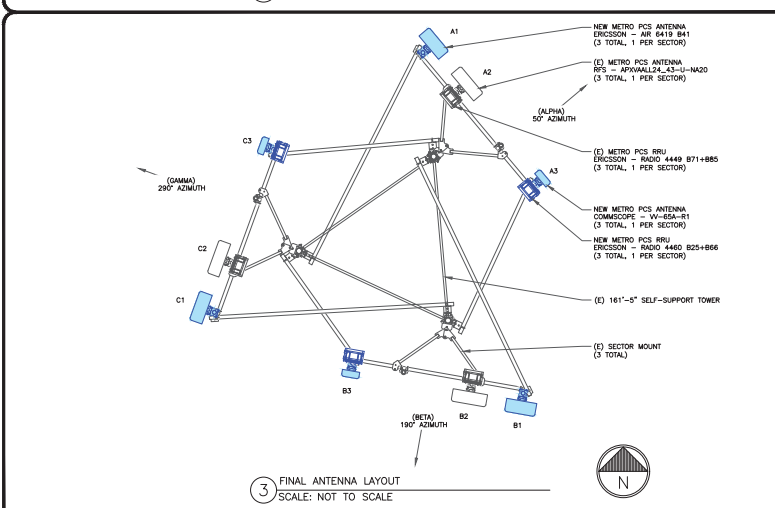
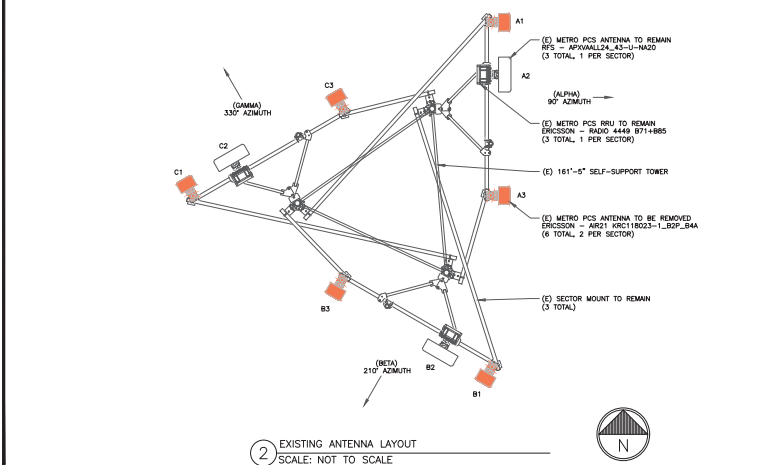
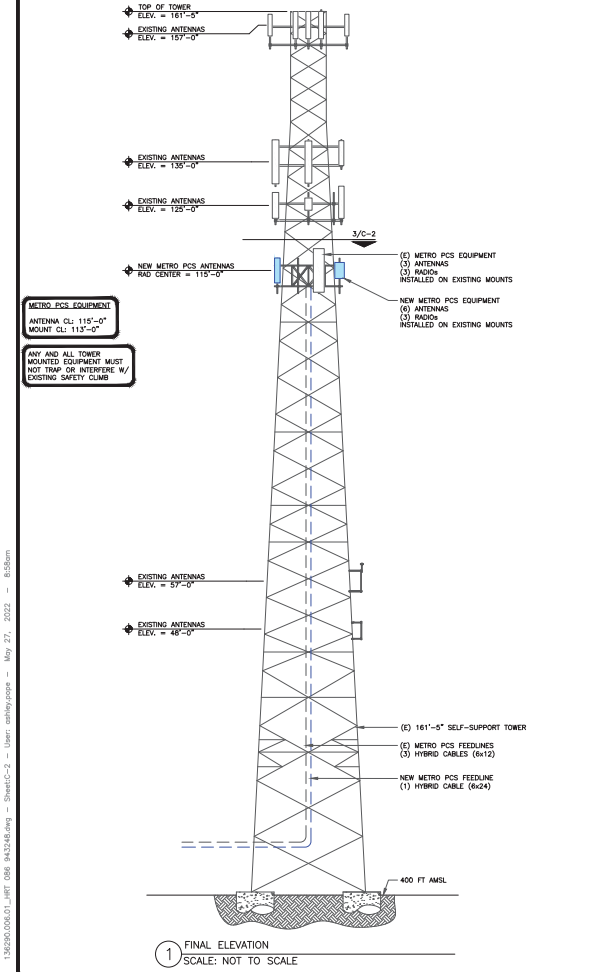
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A	4/26/22	ANP	PRELIMINARY REVIEW	MT
B	5/27/22	ANP	CONSTRUCTION	MT

**B&T ENGINEERING, INC.**  
 PEC.0001564  
 Expires 2/10/23

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SHEET NUMBER: **C-1.2**      REVISION: **0**

13629010601\_1MET\_086\_843248.dwg - Sheet: C-1.2 - User: ashley.pope - May 27, 2022 - 8:58am



**T-Mobile**  
 35 GRIFFIN ROAD  
 BLOOMFIELD, CT 06002

**CROWN CASTLE**  
 1200 MACARTHUR BLVD, SUITE 200  
 MAHWAH, NJ 07430

**B+T GRP**  
 1717 E. BOULDER  
 SUITE 200  
 TULSA, OK 74119  
 PH: (918) 581-4830  
 www.btg.com

METRO PCS SITE NUMBER:  
**CTHA534A**  
 BU #: 806378  
 HRT 086 943248  
 126 PIONEER HEIGHTS RD  
 SOMERS, CT 06071  
 EXISTING  
 161'-0" SELF-SUPPORT TOWER

ISSUED FOR:			
REV	DATE	BY	DESCRIPTION
A	4/26/22	ANP	PRELIMINARY REVIEW
B	3/27/22	ANP	CONSTRUCTION

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SHEET NUMBER: **C-2**      REVISION: **0**

1:3029010801\_1MET 086 843248.dwg - SheetC-2 - User: camley.pope - May 27, 2022 - 8:35am



METRO PCS SITE NUMBER:  
**CTHA534A**

BU #: 806378  
HRT 086 943248

126 PIONEER HEIGHTS RD  
SOMERS, CT 06071

EXISTING  
161'-5" SELF-SUPPORT TOWER

**ISSUED FOR:**

REV	DATE	DRWN	DESCRIPTION	DES. Q2
A	4/26/22	ANP	PRELIMINARY REVIEW	MT
B	5/27/22	ANP	CONSTRUCTION	MT



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SHEET NUMBER: **C-3** REVISION: **0**

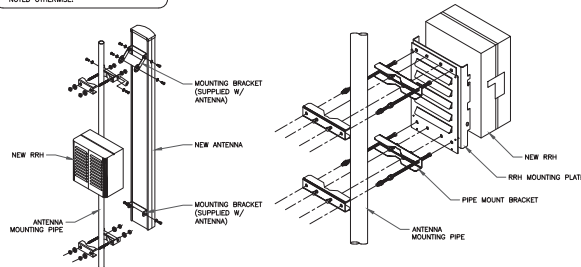
**RF SYSTEM SCHEDULE**

SECTOR	ANTENNA	TECH	MANUFACTURER	ANTENNA MODEL	AZIMUTH	M-TILT	E-TILT	RFD CENTER	TMA/RRU	FEEDLINE TYPE
ALPHA	A1	L2500/N2500	ERICSSON	AR 6419 B41	50°	0°	2/2°	115°-0"	-	-
	A2	L700/L600/N600	RFS	APXWALL24_43-U-N420	50°	0°	2/2°	115°-0"	4449 871+885	(1) 6x12 HYBRID
	A3	L2100/L1900/ U2100	COMMSCOPE	W-65A-R1	50°	0°	2/2°	115°-0"	4460 825+866	(1) 4/24 HYBRID
BETA	B1	L2500/N2500	ERICSSON	AR 6419 B41	190°	0°	2/2°	115°-0"	-	-
	B2	L700/L600/N600	RFS	APXWALL24_43-U-N420	190°	0°	2/2°	115°-0"	4449 871+885	(1) 6x12 HYBRID
	B3	L2100/L1900/ U2100	COMMSCOPE	W-65A-R1	190°	0°	2/2°	115°-0"	4460 825+866	-
GAMMA	C1	L2500/N2500	ERICSSON	AR 6419 B41	290°	0°	2/2°	115°-0"	-	-
	C2	L700/L600/N600	RFS	APXWALL24_43-U-N420	290°	0°	2/2°	115°-0"	4449 871+885	(1) 6x12 HYBRID
	C3	L2100/L1900/ U2100	COMMSCOPE	W-65A-R1	290°	0°	2/2°	115°-0"	4460 825+866	-

1 ANTENNA AND CABLE SCHEDULE  
SCALE: NOT TO SCALE

**INSTALLER NOTES:**

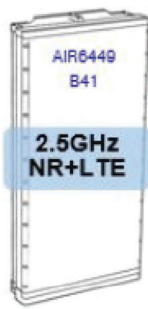
1. COMPLY WITH MANUFACTURERS INSTRUCTIONS TO ENSURE THAT ALL RRRs RECEIVE ELECTRICAL POWER WITHIN 24 HOURS OF BEING REMOVED FROM THE MANUFACTURER'S PACKAGING.
2. DO NOT OPEN RRR PACKAGES IN THE RAIN.
3. ALL PIPES, BRACKETS, AND MISCELLANEOUS HARDWARE TO BE GALVANIZED UNLESS NOTED OTHERWISE.



2 ANTENNA WITH RRR MOUNTING DETAIL  
SCALE: NOT TO SCALE

1:30:00:00:01\_JNET 036 843248.dwg - SheetC-3 - User: camley.pope - May 27, 2022 - 8:35am





1 PLUMBING DIAGRAM  
SCALE: NOT TO SCALE

**T-Mobile**  
55 GRIFFIN ROAD  
BLOOMFIELD, CT 06002

**CROWN CASTLE**  
1200 MACARTHUR BLVD, SUITE 200  
MAYFAH, NJ 07430

**B+T GRP**  
177 E. BOULDER  
SUITE 200  
TULSA, OK 74119  
PH: 918.587.4630  
www.btgrp.com

METRO PCS SITE NUMBER:  
**CTHA534A**

BU #: 806378  
HRT 086 943248

126 PIONEER HEIGHTS RD  
SOMERS, CT 06071

EXISTING  
161'-5" SELF-SUPPORT TOWER

ISSUED FOR:

REV	DATE	BY	DESCRIPTION	DES. Q2
A	4/26/22	ANP	PRELIMINARY REVIEW	NO
0	3/27/22	ANP	CONSTRUCTION	NO

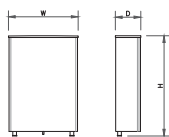


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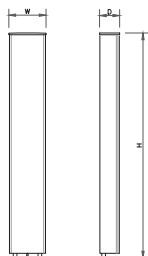
SHEET NUMBER: <b>C-4</b>	REVISION: <b>0</b>
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130280000001\_HRT\_086\_843248.dwg - SheetC-4 - User: amheyppope - May 27, 2022 - 8:35am



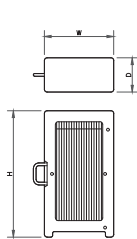
ANTENNA SPECS	
MANUFACTURER	ERICSSON
MODEL #	AIR 6419 B41
WIDTH	20.91"
DEPTH	9.02"
HEIGHT	36.25"
WEIGHT	96.50 LBS

1 ANTENNA SPECS  
SCALE: NOT TO SCALE



ANTENNA SPECS	
MANUFACTURER	COMMSCOPE
MODEL #	VV-65A-R1
WIDTH	12.00"
DEPTH	4.60"
HEIGHT	54.70"
WEIGHT	33.30 LBS

2 ANTENNA SPECS  
SCALE: NOT TO SCALE



RRU SPECIFICATIONS	
MANUFACTURER	ERICSSON
MODEL #	4460 B25+B66
WIDTH	15.10"
DEPTH	11.90"
HEIGHT	17.0"
WEIGHT	109.0 LBS

3 RRU SPECS  
SCALE: NOT TO SCALE

2 General Product Overview

PSU 4813



A. Mounting brackets	Mount for 12" rack installation. Mount can be removed.
B. DC input terminals	Connects to power source. Mount can be removed.
C. Output power ports	Provides for connecting power leads to the antenna system.
D. Power switches	For switching output to connected equipment.
E. Fuse assemblies	Protects fuse terminals for overcurrenting DC output.
F. DC power cables	Connects terminals for connecting power cables to antenna system.
G. Ground connection	Grounds chassis.

Ericsson P34 #13 Voltage Booster Design Specification

Electrical Operating Limits	
Input Voltage	33.3 - 38.5 VDC
Input Current	400 mA
Output Voltage	48 VDC
Output Current	100 mA
Output Power	4.8 W
Efficiency	85%
Temperature Range	-40 to +70 °C
Humidity	5 to 95% RH
Shock	100 g, 11ms, 1000 cycles
Vibration	1.0 g, 1000 cycles
Weight	11.2 lbs (5.1 kg)
Dimensions (H x W x D)	1.7 x 11.9 x 17.0 in (43.2 x 302.1 x 430.0 mm)

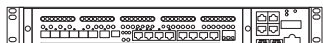
PSU 4813 VOLTAGE BOOSTER	
Size (HxWxD)	1.7x11.9x17.0 in
Weight	11.2 LBS

4 PSU 4813 VOLTAGE BOOSTER SPECS  
SCALE: NOT TO SCALE



NOKIA CSR IXR3 V1 ROUTER	
WEIGHT	11.2 LBS
SIZE (HxWxD)	1.7x11.25x10.0 IN.

5 NOKIA CSR IXR3 V1 ROUTER  
SCALE: NOT TO SCALE



ERICSSON - RBS 6601	
WEIGHT	22.0 LBS
SIZE (HxWxD)	2.6x19.0x13.78 IN.

6 ERICSSON - RBS 6601  
SCALE: NOT TO SCALE

7 NOT USED  
SCALE: NOT TO SCALE

8 NOT USED  
SCALE: NOT TO SCALE

**T-Mobile**  
5 GRIFFIN ROAD  
BLOOMFIELD, CT 06002

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177 S. BOULDER  
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METRO PCS SITE NUMBER:  
**CTHA534A**  
BU #: 806378  
HRT 086 943248  
126 PIONEER HEIGHTS RD  
SOMERS, CT 06071  
EXISTING  
161'-5" SELF-SUPPORT TOWER

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0	3/27/22	ANP	CONSTRUCTION	MTJ

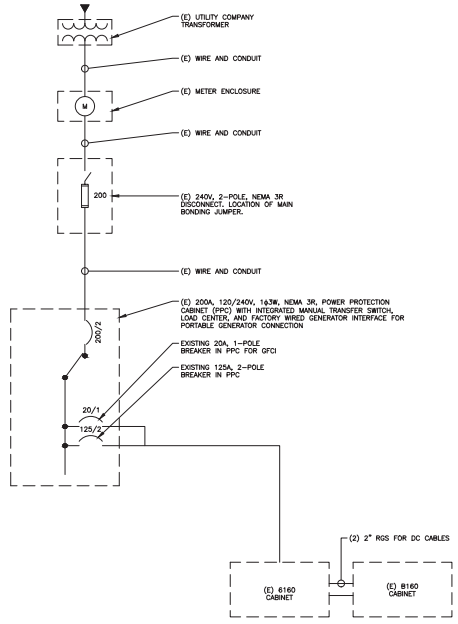
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SHEET NUMBER:	REVISION:
C-5	0

1:36290:080:01\_086 8/3/2018:eng - SheetC-5 - User: cmhlypope - May 27, 2022 - 8:35am

FINAL PANEL SCHEDULE							
LOAD	POLES	AMPS	BUS		AMPS	POLES	LOAD
			L1	L2			
TVSS	2	30A	1	2	60A	2	BTS 1
SITE LIGHT	1	15A	5	6	20A	1	GFC BELOW
6160	2	125A	7	8	20A	1	FIBER CAB
			9	10	20A	1	6160 PLUG
			11	12			
			13	14			
			15	16			
			17	18			
			19	20			
			21	22			
			23	24			

RATED VOLTAGE 120/240 3 PHASE 3 WIRE BRANCH POLES 015 024 030 042 APPROVED MPMS  
 RATIO AMPS 0100 0200 0400 0  
 CHAIN LUGS ONLY MAX 200 AMPS BREAKER CLOSED SWITCH BRASS DOOR TO BE GFCI BREAKERS  
 DIFFUSED BREAKER BRANCH DEVICES BRASS DOOR TO BE GFCI BREAKERS FULL NEUTRAL BUS GROUND BAR  
 ALL BREAKERS MUST BE RATED TO INTERRUPT A SHORT CIRCUIT ICS OF 10,000 AMPS SYMMETRICAL  
 FINAL PANEL DESIGN AND CALCULATIONS FOR WIRE SIZE WERE BASED OFF OF EXISTING DOCUMENTS AND PHOTOS



- NOTES:
- ALL NEW CONDUCTORS TO BE INSTALLED SHALL BE COPPER. ALL CONDUCTORS SHALL BE THW, THHN, THWN-2, XHHW, XHHW-2, RHW, THW, OR RHW-2 UNLESS NOTED OTHERWISE.
  - CONTRACTOR IS TO FIELD VERIFY ALL EXISTING ITEMS SHOWN ON THE ELECTRICAL ONE-LINE DIAGRAM AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES.
  - ALL GROUNDING AND BONDING PER THE NEC.

2 ONE LINE DIAGRAM  
 SCALE: NOT TO SCALE

1 FINAL T-MOBILE PANEL DETAIL  
 SCALE: NOT TO SCALE

**T-Mobile**  
 55 GRIFFIN ROAD  
 BLOOMFIELD, CT 06002

---

**CROWN CASTLE**  
 1200 MACARTHUR BLVD, SUITE 200  
 MAHWAH, NJ 07440

---

**B+T GRP**  
 1717 E. BOULDER  
 SUITE 200  
 TULSA, OK 74119  
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---


METRO PCS SITE NUMBER:  
**CTHA534A**  
 BU #: 806378  
 HRT 086 943248  
 126 PIONEER HEIGHTS RD  
 SOMERS, CT 06071  
 EXISTING  
 161'-5" SELF-SUPPORT TOWER

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SHEET NUMBER: **E-1**      REVISION: **0**

13628010801\_001\_001 - SheetE-1 - User: cathy.pope - May 27, 2022 - 8:28am



5 GRIFFIN ROAD  
BLOOMFIELD, CT 06002



1200 MACARTHUR BLVD, SUITE 200  
MAYWAH, NJ 07430



1717 E. BOULDER  
SUITE 200  
TULSA, OK 74119  
PH: (918) 587-4830  
www.btgps.com

METRO PCS SITE NUMBER:  
**CTHA534A**

BU #: 806378  
HRT 086 943248

126 PIONEER HEIGHTS RD  
SOMERS, CT 06071

EXISTING  
161'-5" SELF-SUPPORT TOWER

**ISSUED FOR:**

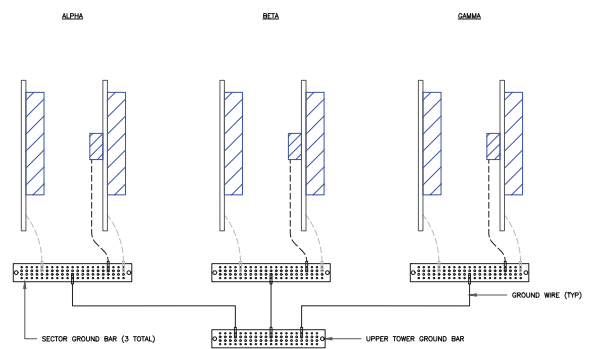
REV	DATE	BY	DESCRIPTION	DES. Q2
A	4/26/22	ANP	PRELIMINARY REVIEW	MTJ
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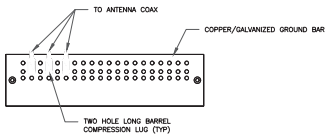
SHEET NUMBER: **G-1** REVISION: **0**



**NOTE:**  
ALL NEW GROUNDS TO BE #6 STRANDED  
COPPER WITH GREEN INSULATION UNLESS  
NOTED OTHERWISE.

1 ANTENNA GROUNDING DIAGRAM  
SCALE: NOT TO SCALE

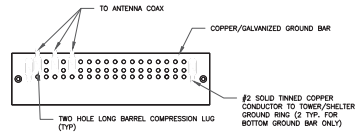
136280086001\_HRT\_086\_943248.dwg - SheetG-1 - User: cmhlypope - May 27, 2022 - 8:35am



NOTES:

1. DOUBLING UP "OR STACKING" OF CONNECTIONS IS NOT PERMITTED.
2. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
3. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO ANTENNA MOUNT STEEL.

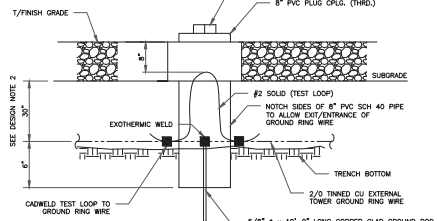
1 ANTENNA SECTOR GROUND BAR DETAIL  
SCALE: NOT TO SCALE



NOTES:

1. EXTERIOR ANTIOXIDANT JOINT COMPOUND TO BE USED ON ALL EXTERIOR CONNECTIONS.
2. GROUND BAR SHALL NOT BE ISOLATED FROM TOWER. MOUNT DIRECTLY TO TOWER STEEL (TOWER ONLY).
3. GROUND BAR SHALL BE ISOLATED FROM BUILDING OR SHELTER.

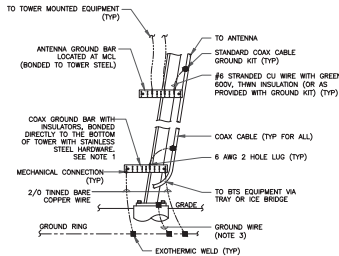
2 TOWER/SHELTER GROUND BAR DETAIL  
SCALE: NOT TO SCALE



NOTES:

1. GROUND ROD SHALL BE DRIVEN VERTICALLY, NOT TO EXCEED 45 DEGREES FROM THE VERTICAL.
2. GROUND WIRE SHALL BE MIN. 30" BELOW GRADE OR 6" BELOW FROST LINE, (WHICH EVER IS GREATER) AS PER N.E.C. ARTICLE 250-50(D).

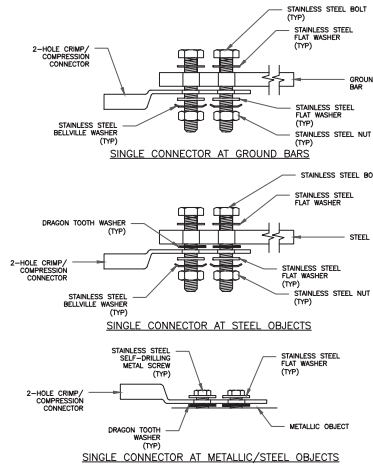
3 INSPECTION WELL DETAIL  
SCALE: NOT TO SCALE



NOTES:

1. NUMBER OF GROUNDING BARS MAY VARY DEPENDING ON THE TYPE OF TOWER, ANTENNA LOCATIONS AND CONNECTION ORIENTATION. COAXIAL CABLES EXCEEDING 200 FEET ON THE TOWER SHALL HAVE GROUND KITS AT THE MIDPOINT, PROVIDED AS REQUIRED.
2. ONLY MECHANICAL CONNECTIONS ARE ALLOWED TO BE MADE TO CROWN CASTLE USA INC. TOWERS. ALL MECHANICAL CONNECTIONS SHALL BE TREATED WITH AN ANTI-OXIDANT COATING.
3. ALL TOWER GROUNDING SYSTEMS SHALL COMPLY WITH THE REQUIREMENTS OF THE RECOGNIZED EDITION OF ANSI/TIA 222 AND NFPA 700.

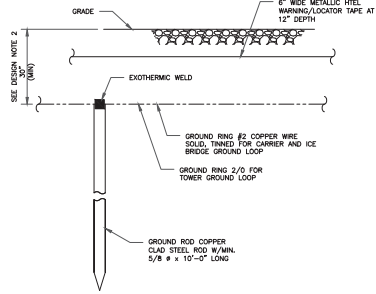
4 TYPICAL ANTENNA CABLE GROUNDING  
SCALE: NOT TO SCALE



NOTES:

1. GROUND ROD SHALL BE DRIVEN VERTICALLY, NOT TO EXCEED 45 DEGREES FROM THE VERTICAL.
2. GROUND WIRE SHALL BE MIN. 30" BELOW GRADE OR 6" BELOW FROST LINE, (WHICH EVER IS GREATER) AS PER N.E.C. ARTICLE 250-50(D).

5 HARDWARE DETAIL FOR EXTERIOR CONNECTIONS  
SCALE: NOT TO SCALE



NOTES:

1. GROUND ROD SHALL BE DRIVEN VERTICALLY, NOT TO EXCEED 45 DEGREES FROM THE VERTICAL.
2. GROUND WIRE SHALL BE MIN. 30" BELOW GRADE OR 6" BELOW FROST LINE, (WHICH EVER IS GREATER) AS PER N.E.C. ARTICLE 250-50(D).

6 GROUND ROD DETAIL  
SCALE: NOT TO SCALE



METRO PCS SITE NUMBER:  
CTHA534A

BU #: 806378  
HRT 086 943248

126 PIONEER HEIGHTS RD  
SOMERS, CT 06071

EXISTING  
161'-5" SELF-SUPPORT TOWER

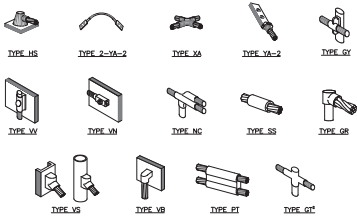
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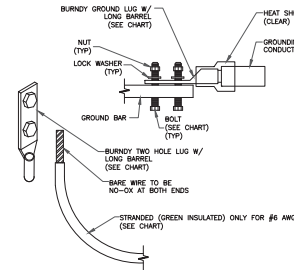
SHEET NUMBER: **G-2** REVISION: **0**



- NOTE:**
1. ERCO EXOTHERMIC "MOLD TYPES" SHOWN HERE ARE EXAMPLES. CONSULT WITH CONSTRUCTION MANAGER FOR SPECIFIC MOLDS TO BE USED FOR THIS PROJECT.
  2. MOLD TYPE ONLY TO BE USED BELOW GRADE WHEN CONNECTING GROUND RING TO GROUND ROD.

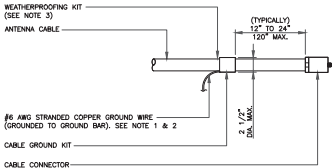
① CADWELD GROUNDING CONNECTIONS  
SCALE: NOT TO SCALE

WIRE SIZE	BURNDY LUG	BOLT SIZE
#8 AWG GREEN INSULATED	YA2C-2TC38	3/8" - 16 NC S 2 BOLT
#2 AWG SOLID TINNED	YA2C-2TC38	3/8" - 16 NC S 2 BOLT
#2 AWG STRANDED	YA2C-2TC38	3/8" - 16 NC S 2 BOLT
#2/0 AWG STRANDED	YA2B-2TC38	3/8" - 16 NC S 2 BOLT
#4/0 AWG STRANDED	YA2B-2N	1/2" - 16 NC S 2 BOLT



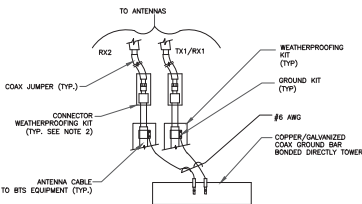
- NOTES:**
1. ALL GROUNDING LUGS ARE TO BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS. ALL HARDWARE BOLTS, NUTS, LOCK WASHERS SHALL BE STAINLESS STEEL. ALL HARDWARE ARE TO BE AS FOLLOWS: BOLT, FLAT WASHER, GROUND BAR, GROUND LUG, FLAT WASHER AND NUT.

② MECHANICAL LUG CONNECTION  
SCALE: NOT TO SCALE



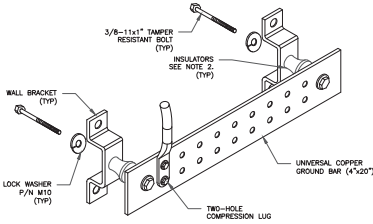
- NOTES:**
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
  2. GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
  3. WEATHER PROOFING SHALL BE TWO-PART TAPE KIT. COLD SHRINK SHALL NOT BE USED.

③ CABLE GROUND KIT CONNECTION  
SCALE: NOT TO SCALE



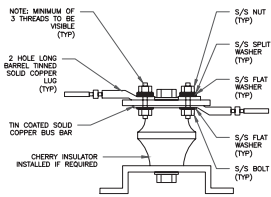
- NOTES:**
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO ANTENNA GROUND BAR.
  2. WEATHER PROOFING SHALL BE TWO-PART TAPE KIT. COLD SHRINK SHALL NOT BE USED.

④ GROUND CABLE CONNECTION  
SCALE: NOT TO SCALE

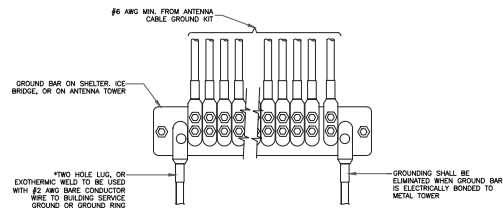


- NOTES:**
1. DOWN LEAD (HOME RUN) CONDUCTORS ARE NOT TO BE INSTALLED ON CROWN CASTLE USA INC. TOWER. PER THE GROUNDING DOWN CONDUCTOR POLICY (GDC-01-10091) NO MODIFICATION OR DRILLING TO TOWER STEEL IS ALLOWED IN ANY FORM OR FASHION. CAG-HELD ON THE TOWER AND/OR IN THE AIR ARE NOT PERMITTED.
  2. OMT INSULATOR WHEN MOUNTING TO TOWER STEEL OR PLATFORM STEEL. USE INSULATORS WHEN ATTACHING TO BUILDING OR SHELTERS.

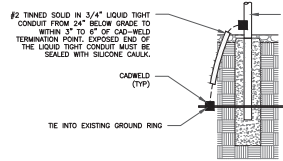
⑥ GROUND BAR DETAIL  
SCALE: NOT TO SCALE



⑦ LUG DETAIL  
SCALE: NOT TO SCALE



⑤ GROUNDWIRE INSTALLATION  
SCALE: NOT TO SCALE



⑧ TRANSITIONING GROUND DETAIL  
SCALE: NOT TO SCALE



35 GRIFFIN ROAD  
BLOOMFIELD, CT 06002



1200 MACARTHUR BLVD, SUITE 200  
MAYFAH, NJ 07440



177 E. BOULDER  
SUITE 200  
TULSA, OK 74119  
PH: 918.587.4830  
www.btgs.com

METRO PCS SITE NUMBER:  
CTHA534A

BU #: 806378  
HRT 086 943248

126 PIONEER HEIGHTS RD  
SOMERS, CT 06071

EXISTING  
161'-5" SELF-SUPPORT TOWER

**ISSUED FOR:**

REV	DATE	BY	DESCRIPTION	CHKD
A	4/26/22	ANP	PRELIMINARY REVIEW	MT
B	5/27/22	ANP	CONSTRUCTION SET	MT



B&T ENGINEERING, INC.  
PEC.0001564  
Expires 2/10/23

IT IS A VIOLATION OF LAW FOR ANY PERSON,  
UNLESS THEY ARE SO TRAINED UNDER THE SUPERVISION  
OF A LICENSED PROFESSIONAL ENGINEER,  
TO ALTER THIS DOCUMENT.

SHEET NUMBER: **G-3** REVISION: **0**

13629008001\_HRT\_086\_843248.dwg - SheetG-3 - User: cmhlypope - May 27, 2022 - 8:35am

CROWN CASTLE - STA PROPERTY  
8000 AVALON BLVD., SUITE 700  
ALPHARETTA, GA 30009

308619

DATE 6-7-22

32-61/1110

PAY  
TO THE  
ORDER OF

Connecticut Sitting Council

\$ 625.00

Six hundred twenty five dollars

DOLLARS

CHASE

JPMorgan Chase Bank, N.A.  
www.chase.com

VOID AFTER 180 DAYS

FOR C T H A-534A-806378-709736-609062

*[Signature]*

⑆ 308619⑆ ⑆ 11000614⑆

⑆ 64638126⑆