



56 Prospect Street,
P.O. Box 270
Hartford, CT 06103

Kathleen M. Shanley
Manager – Transmission Siting
Tel: (860) 728-4527

September 10, 2021

Melanie A. Bachman
Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

RE: **Notice of Exempt Modification
Eversource Site Ridgefield 22N
Off Prospect Street, Ridgefield, CT 06877
Latitude: 41-17-00.6 N / Longitude: 73-29.16.3 W**

Dear Ms. Bachman:

The Connecticut Light and Power Company doing business as Eversource Energy (“Eversource”) currently maintains multiple antennas and equipment at various mounting heights on an existing 84-foot steel monopole tower located off Prospect Street in Ridgefield. See [Attachment A](#), Parcel Map and Property Card. The tower and property are owned by Eversource. Eversource is seeking the Connecticut Siting Council’s authorization for the installation of one 15-foot 6-inch tall omni-directional antenna to be mounted at 83 feet above ground level (“AGL”) and the removal of two omni-directional antennas, one upright and one inverted. There will be no other changes to the area of the fenced compound, the tower or the existing antennas and other equipment currently mounted on the tower. The antenna will be mounted to the existing tower on a new 4-foot stand-off mount. See [Attachment B](#), Mount Analysis, dated August 12, 2021. The tower and existing and proposed equipment on the tower are depicted on [Attachment C](#), Construction Drawings, dated August 17, 2021 and [Attachment D](#), Structural Analysis, dated July 29, 2021. The Connecticut Siting Council approved the monopole at this location in Petition No. 1054 in January 2013.

The modification is required to eliminate transmitter induced noise issues from two antennas previously installed as part of Eversource’s program to update its obsolete analog voice radio communications system to a modern digital voice communications system (refer to EM-EVER-118-200724, dated August 17, 2020). The transmitter issue manifests as passive intermodulation, or PIM, noise located on the receive frequencies, which limits the system level coverage capability of the site.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies (“R.C.S.A.”) §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this notice is being delivered to Rudy Marconi, First Selectman for the Town of Ridgefield and Richard Baldelli, Director of Planning & Zoning

for the Town of Ridgefield via the United States Postal Service or private carrier. Proof of delivery is attached. See Attachment E, Proof of Delivery of Notice.

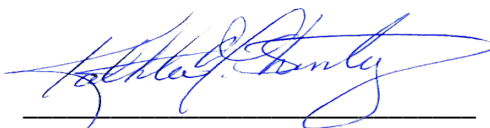
The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2):

1. There will be no change to the height of the existing tower.
2. The modifications will not require the extension of the site boundary.
3. The modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the new antenna will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard as shown in the attached Radio Frequency Emissions Report, dated August 18, 2021 (Attachment F – Power Density Report)¹.
5. The modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Eversource respectfully submits that the proposed modifications to the above referenced telecommunications facility constitute an exempt modification under R.C.S.A. § 16-50j-72(b)(2). One original and two copies of this notice and a check in the amount of \$625 are enclosed.

Communications regarding this Notice of Exempt Modification should be directed to Kathleen Shanley at (860) 728-4527.

By:



Kathleen M. Shanley
Manager – Transmission Siting

cc: Honorable Rudy Marconi, First Selectman, Town of Ridgefield
Richard Baldelli, Director of Planning & Zoning, Town of Ridgefield


Attachments

- A. Parcel Map and Property Card
- B. Mount Analysis
- C. Construction Drawings
- D. Structural Analysis
- E. Proof of Delivery of Notice
- F. Power Density Report

¹ It should be noted that the number of transmitting antennas accounted for in the Power Density Report accounts for two channels on the 88' centerline antenna. Also, the "Antenna Height" column on Table 1 in the Power Density Report reflects the centerline of the Transmit or "TX" antenna centerline.

ATTACHMENT A – PARCEL MAP AND PROPERTY CARD

Legend

 Approximate Tower Location



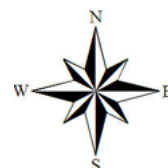
ES-286 Ridgefield22N Parcel

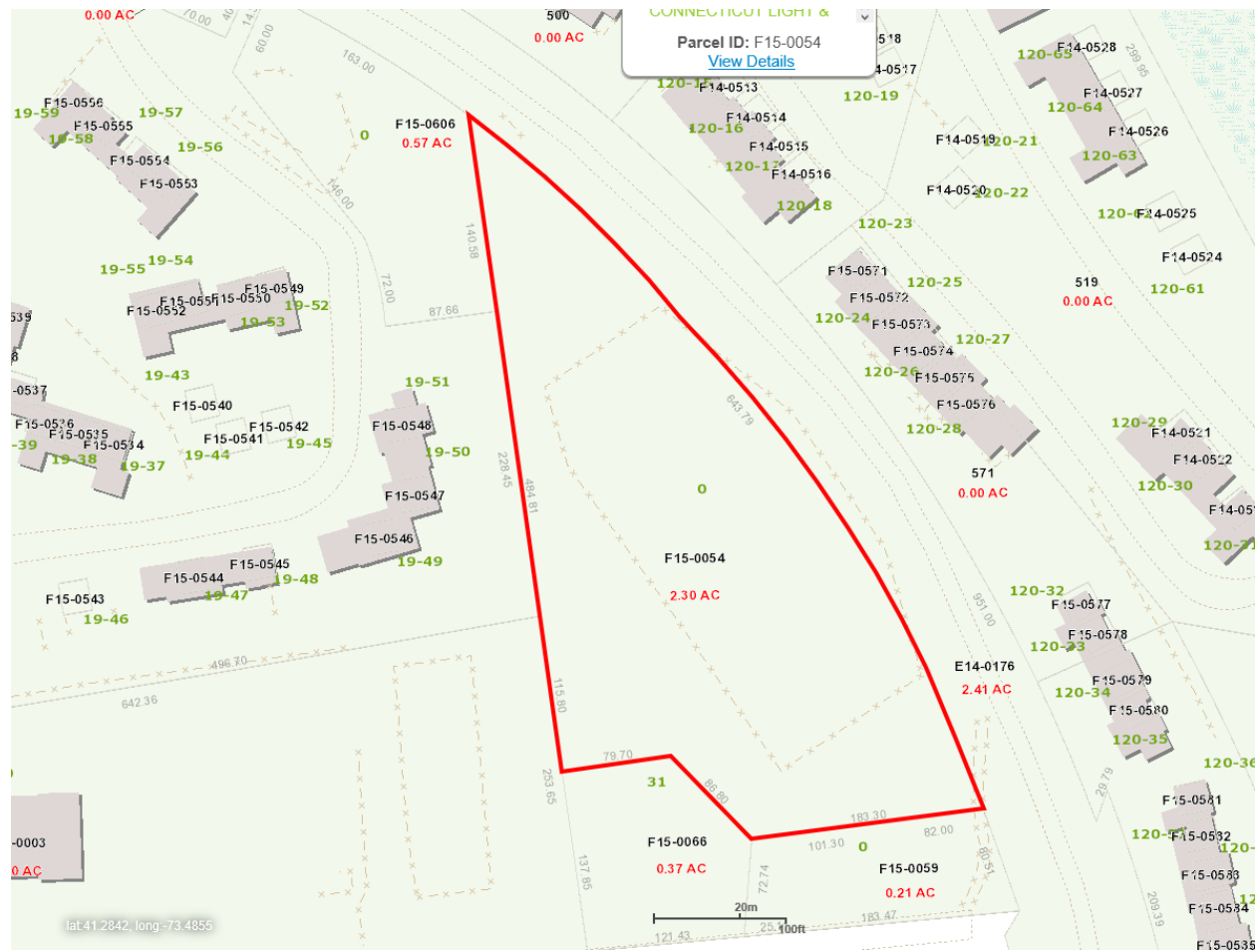
2/20/2020 8:34:14 AM

Scale: 1"=188'

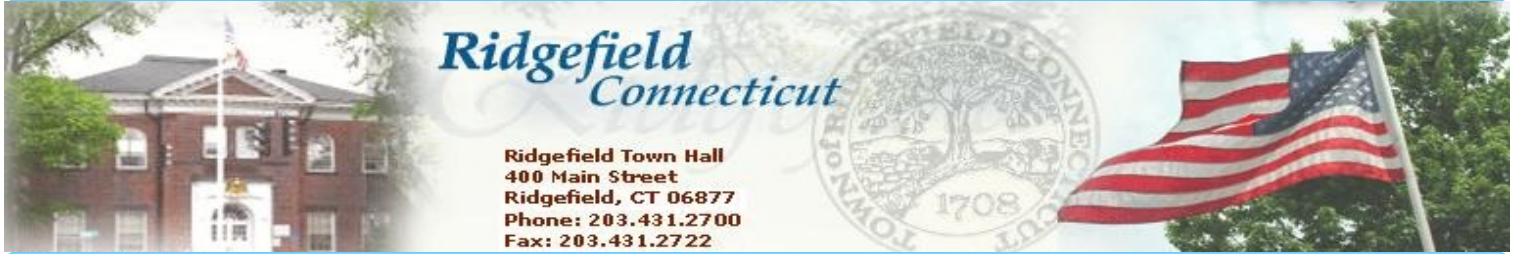
Scale is approximate

The information depicted on this map is for planning purposes only.
It is not adequate for legal boundary definition, regulatory
interpretation, or parcel-level analyses.





The Assessor's office is responsible for the maintenance of records on the ownership of properties. Assessments are computed at 70% of the estimated market value of real property at the time of the last revaluation which was 2017.



Information on the Property Records for the Municipality of Ridgefield was last updated on 2/17/2020.

Parcel Information

| | | | | | |
|-----------------------|-----------|----------------|-------------|----------------|-------------|
| Location: | SUNSET LA | Property Use: | Vacant Land | Primary Use: | Residential |
| Unique ID: | F150054 | Map Block Lot: | F15-0054 | Acres: | 2.30 |
| 490 Acres: | 0.00 | Zone: | RAA | Volume / Page: | 0178/0079 |
| Developers Map / Lot: | | Census: | 2453 | | |

Value Information

| | Appraised Value | Assessed Value |
|-----------------------|-----------------|----------------|
| Land | 98,900 | 69,230 |
| Buildings | 0 | 0 |
| Detached Outbuildings | 0 | 0 |
| Total | 98,900 | 69,230 |

ATTACHMENT B – MOUNT ANALYSIS

August 16, 2021

MOUNT EVALUATION LETTER

Site Number: 848
Site Name: THOMPSON CSP
Site Data: 97 Mountain Hill Road
Thompson, CT 06255
Latitude: 41° 59' 11.76"
Longitude: -71° 54' 49.11"

Black & Veatch Corporation is pleased to submit this "Mount Evaluation Letter" to determine the structural integrity of antenna mounting system on the above-mentioned site. The purpose of this evaluation is to determine the capacity of the system in supporting the final loading in the attached "Loading Summary".

Based on our evaluation we have determined the proposed antenna mounting system to be:

SUFFICIENT

| | |
|---|--------------|
| Structure Rating (max from all components) = | 17.2% |
|---|--------------|

| Proposed Mounting System |
|---|
| SitePro 1 (USF-4U) 48" Ultimate Universal Stand-off Frame |

This analysis analyzes the worst-case scenario for the proposed USF-4U Stand-off Frame. All levels are deemed sufficient. The proposed mounting system will be capable of supporting the proposed equipment, under the following conditions:

- Contractor shall be responsible for the means and methods of construction.
- Contractor shall inspect the condition of all existing and proposed structural members, all relevant members and connections and report any deficiencies to the engineer prior to installation of any new antennas and other equipment.

The scope of this evaluation pertains only to the proposed antenna mounting system and does not include examination of the loads imparted by the antenna mounting system to the existing tower and its structural components. This document was prepared based on information provided to Black & Veatch. If existing conditions do not reflect those represented, this analysis is no longer valid.

Please contact Josh Riley in our Overland Park Office
at 913-458-2522 if you have any questions or comments.

Sincerely,
Black & Veatch Corporation

Prepared By: Shaun Donley
Submitted By: Josh Riley, P.E.

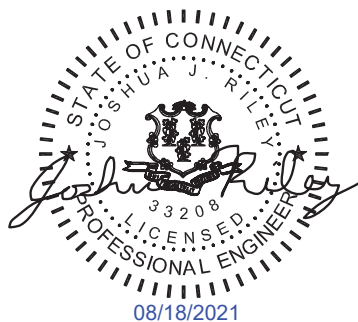




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BLACK & VEATCH

August 16, 2021

THOMPSON CSP

1. LOADING SUMMARY

| Appurtenance | | | | | | | | |
|--------------|----------|--------|----------------------------|--------------------------|-----|--------|--------------|-----------|
| Carrier | Position | Sector | Antenna RAD Center (ft) | Mount Centerline (ft) | Qty | Type | Manufacturer | Model |
| Eversource | 1 | - | 157 | 154 | 1 | Omni | Telewave | ANT220F2 |
| Eversource | 1 | - | 134 | 131 | 1 | Dipole | COMPROD | 871F-70-2 |
| | | | | | | | | |

This analysis analyzes the worst-case scenario for the proposed USF-4U Stand-off Frame. All levels are deemed sufficient



2. ANALYSIS CRITERIA SUMMARY

| ANALYSIS CRITERIA | |
|---------------------|-------------------------------------|
| STANDARD | TIA-222-H |
| WIND SPEED | Ultimate of 140 mph |
| WIND SPEED WITH ICE | 50 mph with 2" radial ice thickness |
| EXPOSURE CATEGORY | B |
| RISK CATEGORY | III |
| TOPO CATEGORY | Hill |
| CREST HEIGHT | 110 ft |

3. REFERENCES

- American Institute of Steel Construction, AISC 15th Edition
- Telecommunications Industry Association Standard, TIA-222-H & 2018 Connecticut State Building Code
- Antenna Mount Assembly Drawing (Model: USF-4U) by SitePro 1, dated 02/16/2011

4. ASSUMPTIONS

This analysis may be affected if any assumptions are not valid or have been made in error. Black & Veatch should be notified to determine the effect on the structural integrity of the antenna mounting system.

- The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- The configuration of antennas, mounts, and other appurtenances are as specified in the Loading Summary and the referenced drawings.
- All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- Sector frame center line: located equidistant between top & bottom boom; Platform center line: located at the base perimeter of platform, unless otherwise specified.
- Steel grades have been assumed as follows, unless noted otherwise:

| | |
|------------------------------------|--------------------|
| Channel, Solid Round, Angle, Plate | ASTM A36 (GR 36) |
| HSS (Rectangular) | ASTM 500 (GR B-46) |
| Pipe | ASTM A53 (GR B-35) |
| Connection Bolts | ASTM A325 |



BLACK & VEATCH

August 16, 2021

THOMPSON CSP

5. RESULTS SUMMARY

| Name | Bending Stress Ratio | | Shear Stress Ratio | |
|--------------------------|----------------------|------|--------------------|------|
| Arm: HSS3X3X3 | 12.8% | Pass | 2.8% | Pass |
| Bracing: Pipe 2.0 Std | 17.2% | Pass | 2.2% | Pass |
| Mount Pipe: Pipe 3.0 Std | 8.7% | Pass | 3.8% | Pass |
| | | | | |

*Von Mises SR = (Max Von Mises Value From RISA-3D)/(0.9*Fy)

**Capacity rating per TIA-222-H Section 15.5.

**APPENDIX 1:
MOUNT ANALYSIS REPORT**



BLACK & VEATCH

Client: Eversource

Site Name: THOMPSON CSP (848)

Computed By: Shaun Donley

Date: 8/16/2021

Verified By: JJ

Title: MOUNT ANALYSIS REPORT

Date: 8/16/2021

Dead and Live Loads

Maintenance Live Load: $L_V = 250$ lb

Installation Live Load: $L_M = 0$ lb

| Appurtenance Dead Loads | |
|-------------------------|----------------|
| Name | Weight (lb) |
| 871F-70-2 | 12.5 |
| | |



Computed By: Shaun Donley

Date: 8/16/2021

Title: MOUNT ANALYSIS REPORT

Date: 8/16/2021

[illegible]



BLACK & VEATCH

Client: Eversource
Site Name: THOMPSON CSP (848)

Computed By: Shaun Donley

Date: 8/16/2021

Verified By: JJ

Title: MOUNT ANALYSIS REPORT

Date: 8/16/2021

Member Wind Loading

Exposure Category = B
Risk Category = III
Topographic Category = 1
Basic Wind Speed, V = 140 mph
Height Above Ground, z = 134 ft
Crest Height, H = 110 ft
Velocity Pressure Coefficient, K_z = 1.07
Topographic Factor, K_{zt} = 1.09
Wind Directionality Factor, K_d = 0.95
Shielding Factor, K_s = 0.90
Ground Elevation Factor, K_e = 1.000
Wind Velocity Pressure, q_z = 55.61 psf
Gust Effect Factor, G_h = 1.00

Equations

$$K_z = 2.01 (z / z_g)^{2/\alpha}$$

$$K_h = e^{(f \cdot z / H)}$$

$$K_{zt} = [1 + K_c K_t / K_h]^2$$

$$K_e = e^{-0.00053 z^{0.25}}$$

$$q_z = 0.00256 K_z K_{zt} K_e K_d V^2$$

$$F_A = q_z G_h (EPA)$$

$$F_M = q_z G_h C_f D_p$$

TIA-222-H

2.6.5.2

2.6.6.2.1

2.6.6.2.1

2.6.8

2.6.11.6

2.6.11.2

2.6.11.2

Member Wind Loads

| Name | Depth (ft) | Width (ft) | C_f | D_p (ft) | F_M (lb) |
|--------------------------|------------|------------|-------|------------|------------|
| Arm: HSS3X3X3 | 0.25 | 0.25 | 2 | 0.25 | 27.80 |
| Bracing: Pipe 2.0 Std | 0.20 | | 1.2 | 0.20 | 13.21 |
| Mount Pipe: Pipe 3.0 Std | 0.29 | | 1.2 | 0.29 | 19.46 |
| | | | | | |

**BLACK & VEATCH**

Client: Eversource
 Site Name: THOMPSON CSP (848)

Computed By: Shaun Donley

Date: 8/16/2021

Verified By: JJ

Title: MOUNT ANALYSIS REPORT

Date: 8/16/2021

Appurtenance Ice Dead Loading

Exposure Category = B
 Risk Category = III
 Topographic Category = 1
 Height Above Ground, z = 134 ft
 Crest Height, H = 110 ft
 Design Ice Thickness, T_i = 2.00 in
 Importance Factor, I = 1.15
 Topographic Factor, K_{zt} = 1.09
 Height Escalation Factor, K_{iz} = 1.15
 Factored Ice Thickness, T_{iz} = 2.72 in
 Grating Ice Dead Load, D_{Gice} = 12.71 psf

Equations

$$K_h = e^{(f \cdot z / H)}$$

$$K_{zt} = [1 + K_c K_t / K_h]^2$$

$$K_{iz} = (z/33)^{0.10}$$

$$T_{iz} = T_i I K_{iz} (K_{zt})^{0.30}$$

$$DL_{ice} = [(H_{ice} \cdot D_{ice} \cdot W_{ice}) - (H \cdot W \cdot D)] \cdot 56 \text{pcf}$$

TIA-222-H

2.6.6.2.1

2.6.6.2.1

2.6.10

2.6.10

Appurtenance Ice Dead Loads

| Name | Height w/ ice (ft) | Width w/ice (ft) | Depth w/ ice (ft) | V_{ice} (ft ³) | DL_{ice} (lb) |
|-----------|-----------------------|---------------------|----------------------|---------------------------------|--------------------|
| 871F-70-2 | 5.95 | 3.04 | 0.61 | 8.82 | 493.98 |
| | | | | | |

**BLACK & VEATCH**Client: Eversource
Site Name: THOMPSON CSP (848)

Computed By: Shaun Donley

Date: 8/16/2021

Verified By: JJ

Title: MOUNT ANALYSIS REPORT

Date: 8/16/2021

Member Ice Dead Loading

Exposure Category = B
 Risk Category = III
 Topographic Category = 1
 Height Above Ground, z = 134 ft
 Crest Height, H = 110 ft
 Design Ice Thickness, T_i = 2.00 in
 Importance Factor, I = 1.15
 Topographic Factor, K_{zt} = 1.09
 Height Escalation Factor, K_{iz} = 1.15
 Factored Ice Thickness, T_{iz} = 2.72 in
 Grating Ice Dead Load, D_{Gice} = 12.71 psf

Equations

$$K_h = e^{(f \cdot z / H)}$$

$$K_{zt} = [1 + K_c K_t / K_h]^2$$

$$K_{iz} = (z/33)^{0.10}$$

$$T_{iz} = T_i I K_{iz} (K_{zt})^{0.35}$$

$$A_{iz} = \pi \cdot T_{iz} \cdot (D_c + T_{iz})$$

$$DL_{ice} = A_{iz} \cdot 56 \text{pcf}$$

TIA-222-H

2.6.6.2.1

2.6.6.2.1

2.6.10

2.6.10

2.6.10

Member Ice Dead Loads

| Name | Depth w/ ice (ft) | Width w/ ice (ft) | Dc (ft) | Aiz (ft ²) | DL _{ice} (lb/ft) |
|--------------------------|----------------------|----------------------|------------|---------------------------|---------------------------|
| Arm: HSS3X3X3 | 0.70 | 0.70 | 0.35 | 0.41 | 23.18 |
| Bracing: Pipe 2.0 Std | 0.65 | | 0.20 | 0.30 | 16.96 |
| Mount Pipe: Pipe 3.0 Std | 0.75 | | 0.29 | 0.37 | 20.71 |
| | | | | | |



Computed By: Shaun Donley

Date: 8/16/2021

Date: 8/16/2021

[illegible]



Client: Eversource
 Site Name: THOMPSON CSP (848)

Computed By: Shaun Donley

Date: 8/16/2021

Verified By: JJ

BLACK & VEATCH

Title: MOUNT ANALYSIS REPORT

Date: 8/16/2021

Member Ice Wind Loading

Exposure Category = B
 Risk Category = III
 Topographic Category = 1
 Ice Wind Speed, V_{ice} = 50 mph
 Height Above Ground, z = 134 ft
 Crest Height, H = 110 ft
 Velocity Pressure Coefficient, K_z = 1.07 psf
 Topographic Factor, K_{zt} = 1.09
 Wind Directionality Factor, K_d = 0.95
 Shielding Factor, K_a = 0.90
 Ground Elevation Factor, K_e = 1.000
 Ice Wind Velocity Pressure, $q_{z(ice)}$ = 7.093
 Factored Ice Thickness, T_{iz} = 2.72 in
 Gust Effect Factor, G_h = 1

Equations

$$K_z = 2.01 (z / z_g)^{2/\alpha}$$

$$K_h = e^{(f \cdot z / H)}$$

$$K_{zt} = [1 + K_c K_t / K_h]^2$$

$$K_e = e^{-0.00053 z^{0.25}}$$

$$q_z = 0.00256 K_z K_{zt} K_e K_d V^2$$

$$F_{A(ice)} = q_{z(ice)} G_h (EPA)_{A(ice)}$$

$$F_{M(ice)} = q_{z(ice)} G_h C_f D_{p(ice)}$$

TIA-222-H

2.6.5.2

2.6.6.2.1

2.6.6.2.1

2.6.8

2.6.11.6

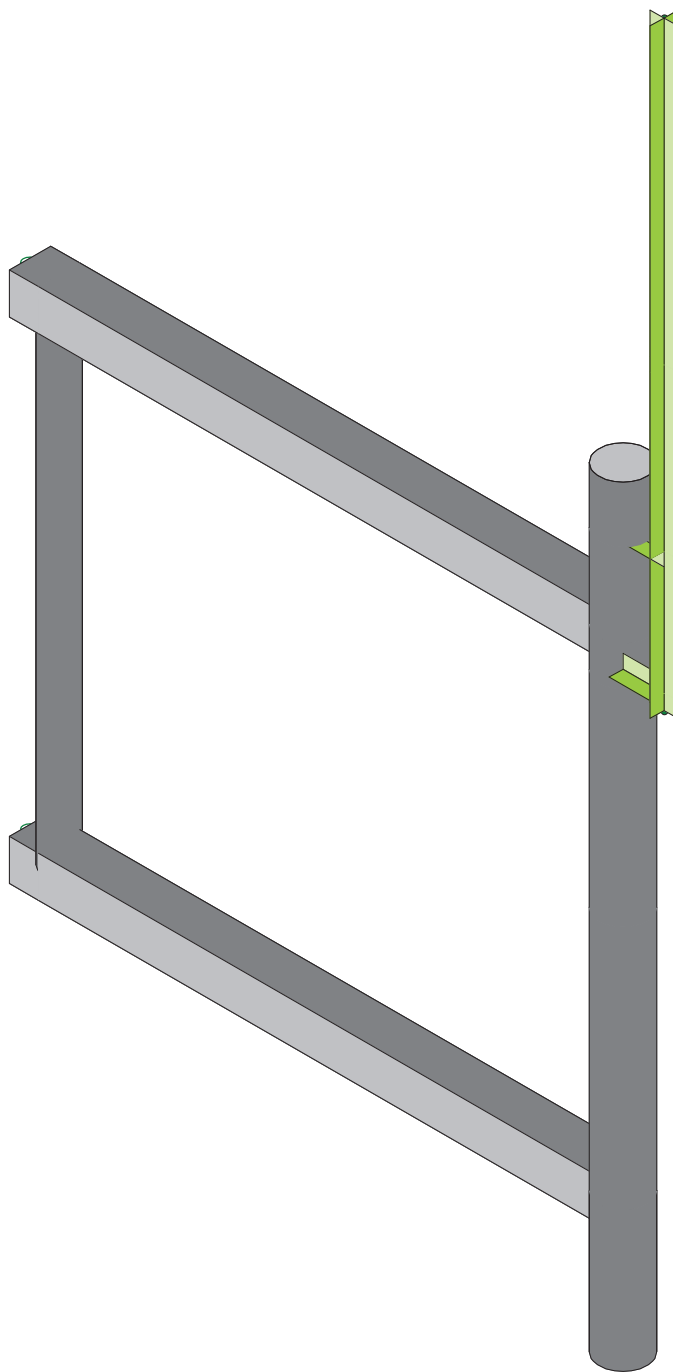
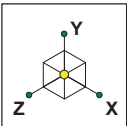
2.6.11.2

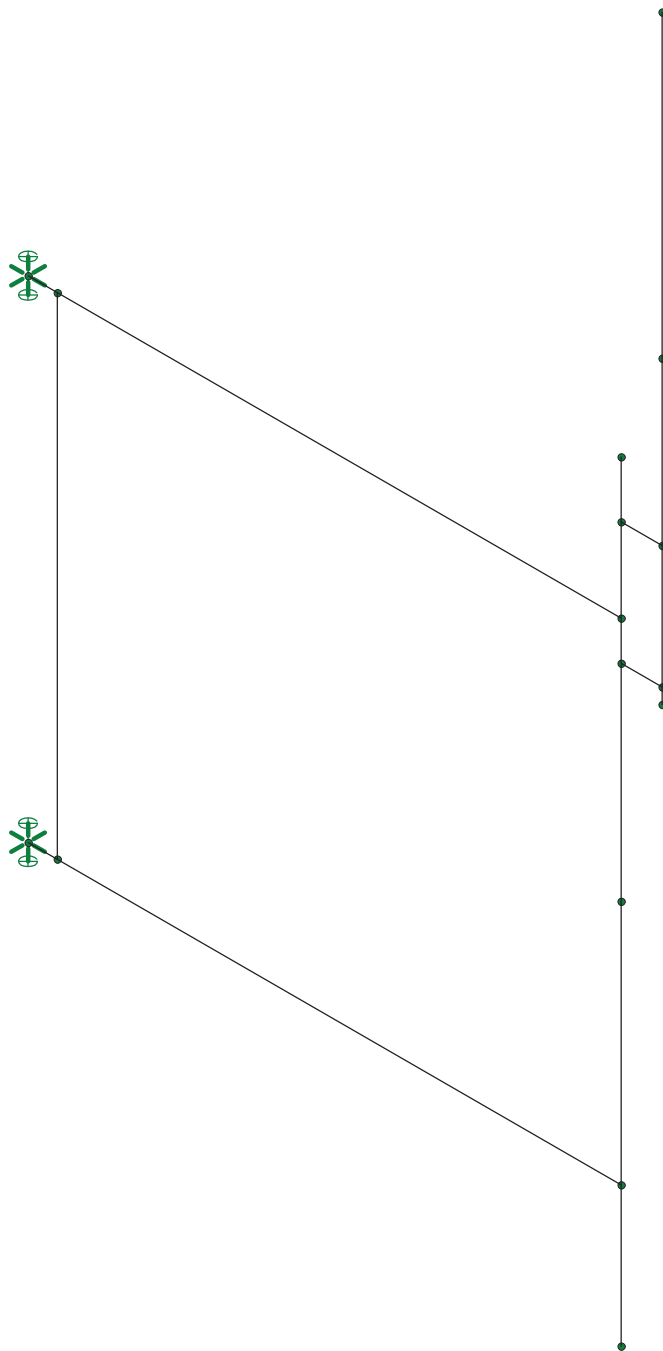
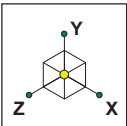
2.6.11.2

Member Ice Wind Loads

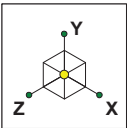
| Name | Depth w/ Ice (ft) | Width w/ Ice (ft) | C_f | $D_{p(ice)}$ (ft) | $F_{M(ice)}$ (lb/ft) |
|--------------------------|----------------------|----------------------|-------|----------------------|-------------------------|
| Arm: HSS3X3X3 | 0.70 | 0.70 | 2 | 0.70 | 9.98 |
| Bracing: Pipe 2.0 Std | 0.65 | | 1.2 | 0.65 | 5.55 |
| Mount Pipe: Pipe 3.0 Std | 0.75 | | 1.2 | 0.75 | 6.35 |
| | | | | | |

**APPENDIX 2:
RISA PRINTOUTS**

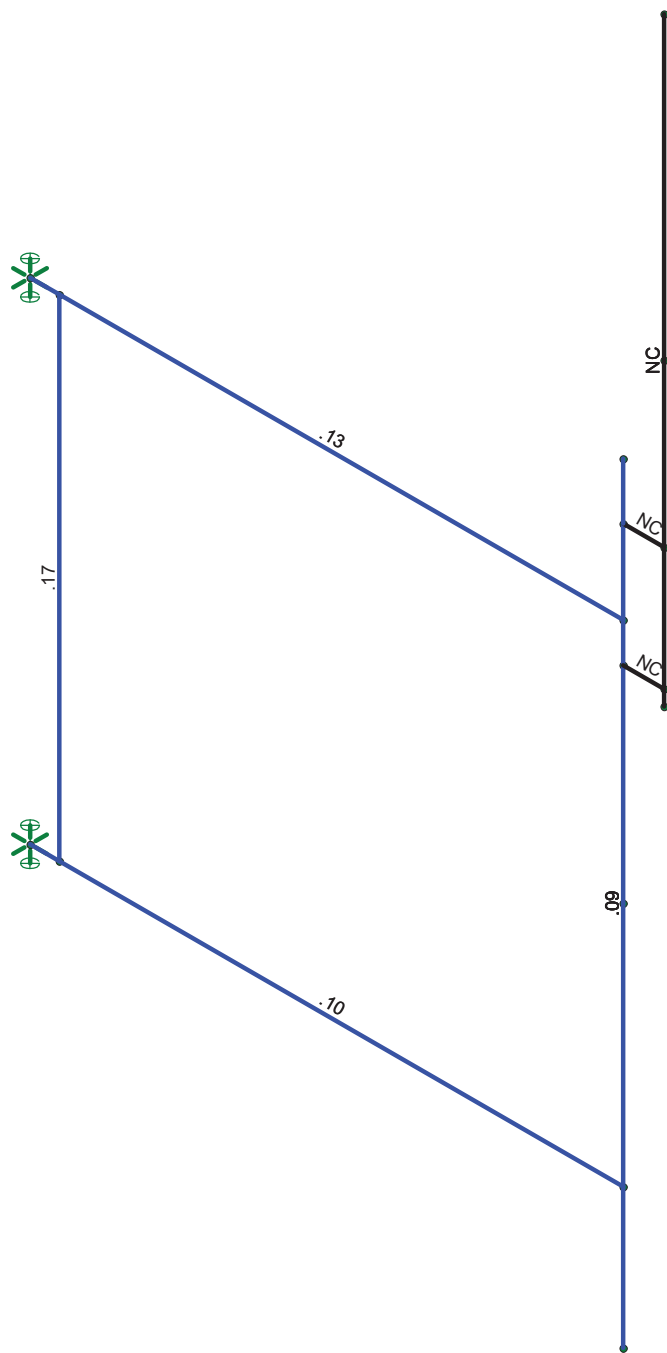




| | | |
|------------------|--------------------------|---------------------------------|
| Black & Veatch | THOMPSONCSP USF-4U Model | SK - 2 |
| Shaun Donley | | Aug 16, 2021 at 12:38 PM |
| 405025.3022.2200 | | 405025.3022.2200 Risa Model.r3d |

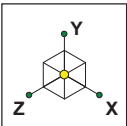


| Code Check (Env) | |
|-----------------------|---------|
| No Calc | > 1.0 |
| | .90-1.0 |
| | .75-.90 |
| | .50-.75 |
| | 0-.50 |



Member Code Checks Displayed (Enveloped)
Envelope Only Solution

| | | |
|------------------|--------------------------|---------------------------------|
| Black & Veatch | THOMPSONCSP USF-4U Model | SK - 3 |
| Shaun Donley | | Aug 16, 2021 at 12:39 PM |
| 405025.3022.2200 | | 405025.3022.2200 Risa Model.r3d |



Shear Check
(Env)

No Calc

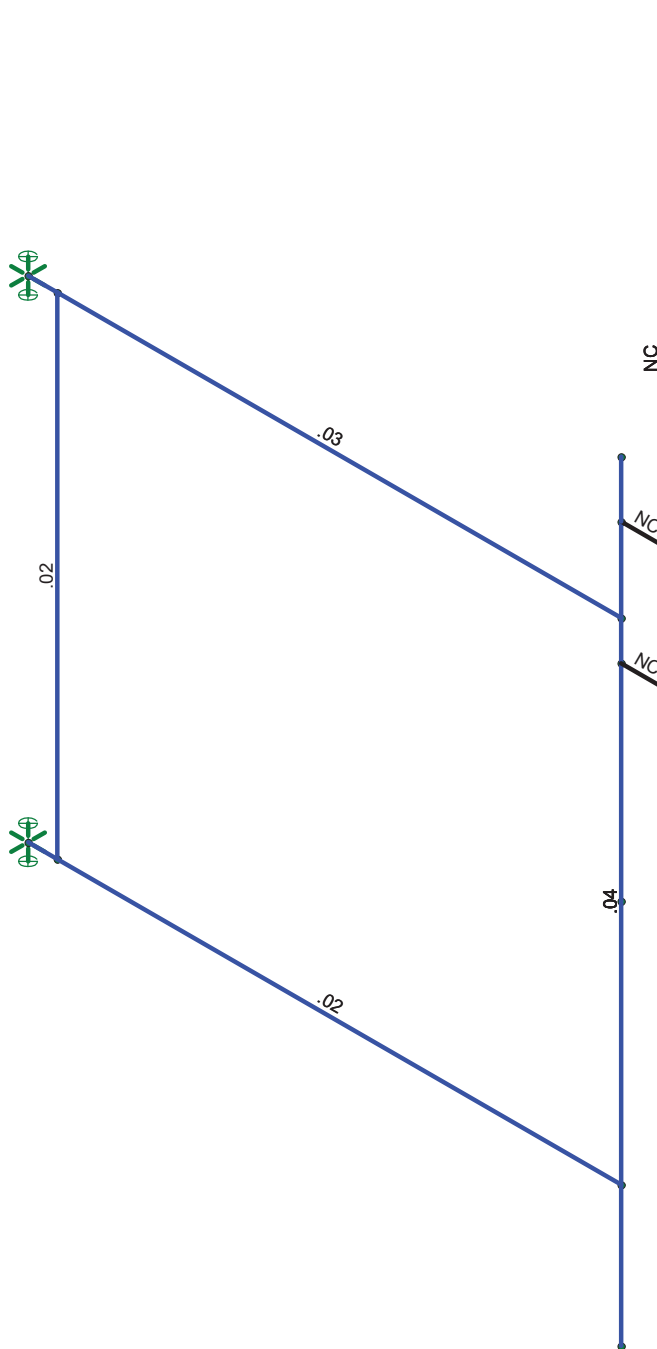
> 1.0

.90-1.0

.75-.90

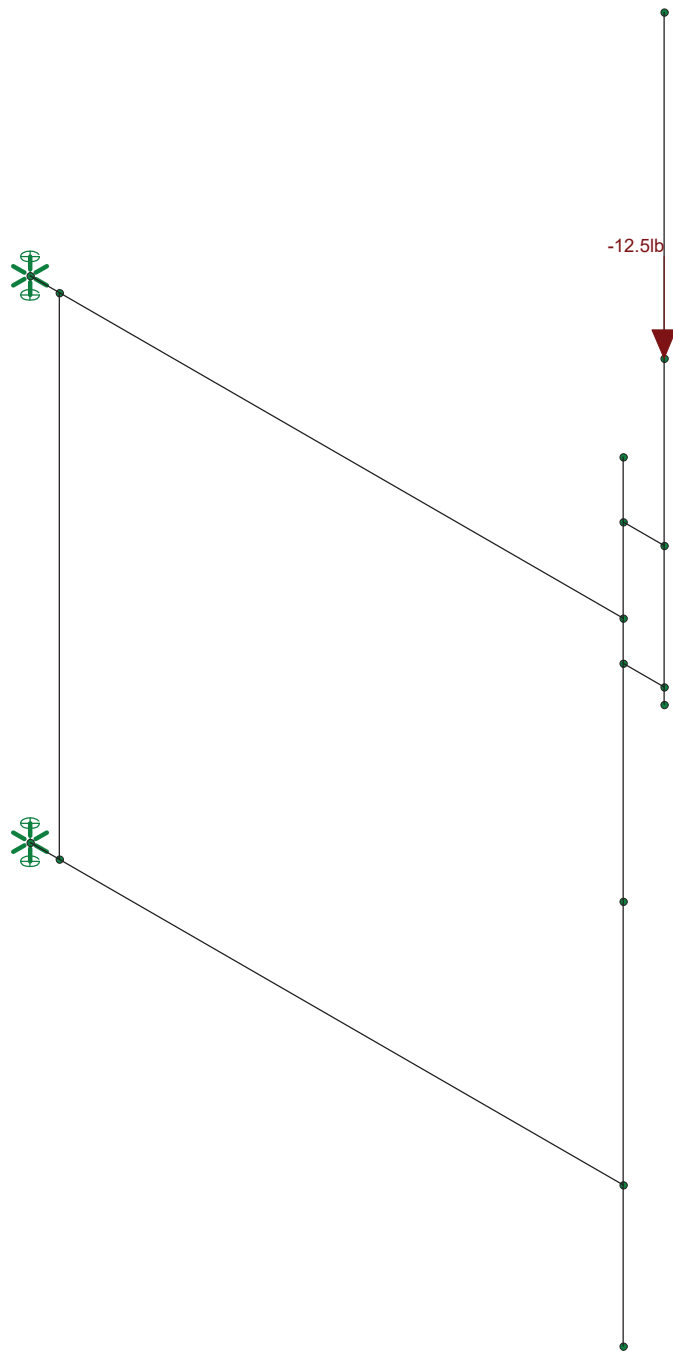
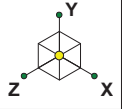
.50-.75

0-.50



Member Shear Checks Displayed (Enveloped)
Envelope Only Solution

| | | |
|------------------|--------------------------|---------------------------------|
| Black & Veatch | THOMPSONCSP USF-4U Model | SK - 4 |
| Shaun Donley | | Aug 16, 2021 at 12:39 PM |
| 405025.3022.2200 | | 405025.3022.2200 Risa Model.r3d |



Loads: BLC 1, DL
Envelope Only Solution

| | | |
|------------------|--------------------------|---------------------------------|
| Black & Veatch | THOMPSONCSP USF-4U Model | SK - 5 |
| Shaun Donley | | Aug 16, 2021 at 12:40 PM |
| 405025.3022.2200 | | 405025.3022.2200 Risa Model.r3d |

(Global) Model Settings

| | |
|--|--------------------|
| Display Sections for Member Calcs | 5 |
| Max Internal Sections for Member Calcs | 97 |
| Include Shear Deformation? | Yes |
| Increase Nailing Capacity for Wind? | Yes |
| Include Warping? | Yes |
| Trans Load Btwn Intersecting Wood Wall? | Yes |
| Area Load Mesh (in^2) | 144 |
| Merge Tolerance (in) | .12 |
| P-Delta Analysis Tolerance | 0.50% |
| Include P-Delta for Walls? | Yes |
| Automatically Iterate Stiffness for Walls? | Yes |
| Max Iterations for Wall Stiffness | 3 |
| Gravity Acceleration (in/sec^2) | 386.4 |
| Wall Mesh Size (in) | 24 |
| Eigensolution Convergence Tol. (1.E-) | 4 |
| Vertical Axis | Y |
| Global Member Orientation Plane | XZ |
| Static Solver | Sparse Accelerated |
| Dynamic Solver | Accelerated Solver |

| | |
|------------------------|-------------------------|
| Hot Rolled Steel Code | AISC 15th(360-16): LRFD |
| Adjust Stiffness? | Yes(Iterative) |
| RISACONNECTION Code | None |
| Cold Formed Steel Code | None |
| Wood Code | None |
| Wood Temperature | < 100F |
| Concrete Code | None |
| Masonry Code | None |
| Aluminum Code | None - Building |
| Stainless Steel Code | None |

| | |
|-------------------------------|--------------------|
| Number of Shear Regions | 4 |
| Region Spacing Increment (in) | 4 |
| Biaxial Column Method | Exact Integration |
| Parame Beta Factor (PCA) | .65 |
| Concrete Stress Block | Rectangular |
| Use Cracked Sections? | Yes |
| Use Cracked Sections Slab? | No |
| Bad Framing Warnings? | No |
| Unused Force Warnings? | Yes |
| Min 1 Bar Diam. Spacing? | No |
| Concrete Rebar Set | REBAR_SET_ASTMA615 |
| Min % Steel for Column | 1 |
| Max % Steel for Column | 8 |

(Global) Model Settings, Continued

| | |
|-----------------------------|-------------|
| Seismic Code | ASCE 7-16 |
| Seismic Base Elevation (in) | Not Entered |
| Add Base Weight? | Yes |
| Ct X | .02 |
| Ct Z | .02 |
| T X (sec) | Not Entered |
| T Z (sec) | Not Entered |
| R X | 3 |
| R Z | 3 |
| Ct Exp. X | .75 |
| Ct Exp. Z | .75 |
| SD1 | 1 |
| SDS | 1 |
| S1 | 1 |
| TL (sec) | 5 |
| Risk Cat | I or II |
| Drift Cat | Other |
| Om Z | 1 |
| Om X | 1 |
| Cd Z | 4 |
| Cd X | 4 |
| Rho Z | 1 |
| Rho X | 1 |

Hot Rolled Steel Properties

| | Label | E [ksi] | G [ksi] | Nu | Therm (/1E... | Density[k/ft... | Yield[ksi] | Ry | Fu[ksi] | Rt |
|---|----------------|---------|---------|----|---------------|-----------------|------------|-----|---------|-----|
| 1 | A992 | 29000 | 11154 | .3 | .65 | .49 | 50 | 1.1 | 65 | 1.1 |
| 2 | A36 Gr.36 | 29000 | 11154 | .3 | .65 | .49 | 36 | 1.5 | 58 | 1.2 |
| 3 | A572 Gr.50 | 29000 | 11154 | .3 | .65 | .49 | 50 | 1.1 | 65 | 1.1 |
| 4 | A500 Gr.B RND | 29000 | 11154 | .3 | .65 | .527 | 42 | 1.4 | 58 | 1.3 |
| 5 | A500 Gr.B Rect | 29000 | 11154 | .3 | .65 | .527 | 46 | 1.4 | 58 | 1.3 |
| 6 | A53 Gr.B | 29000 | 11154 | .3 | .65 | .49 | 35 | 1.6 | 60 | 1.2 |
| 7 | A1085 | 29000 | 11154 | .3 | .65 | .49 | 50 | 1.4 | 65 | 1.3 |

Hot Rolled Steel Section Sets

| | Label | Shape | Type | Design List | Material | Design R... | A [in2] | Iyy [in4] | Izz [in4] | J [in4] |
|---|------------|----------|--------|-------------|----------|-------------|---------|-----------|-----------|---------|
| 1 | Arm | HSS3X3X3 | Beam | SquareTube | A53 Gr.B | Typical | 1.89 | 2.46 | 2.46 | 4.03 |
| 2 | Bracing | PIPE 2.0 | Column | Pipe | A53 Gr.B | Typical | 1.02 | .627 | .627 | 1.25 |
| 3 | Mount Pipe | PIPE 3.0 | Column | Pipe | A53 Gr.B | Typical | 2.07 | 2.85 | 2.85 | 5.69 |

General Material Properties

| | Label | E [ksi] | G [ksi] | Nu | Therm (/1E5 F) | Density[k/ft^3] |
|---|-------------|---------|---------|-----|----------------|-----------------|
| 1 | gen_Conc3NW | 3155 | 1372 | .15 | .6 | .145 |
| 2 | gen_Conc4NW | 3644 | 1584 | .15 | .6 | .145 |
| 3 | gen_Conc3LW | 2085 | 906 | .15 | .6 | .11 |
| 4 | gen_Conc4LW | 2408 | 1047 | .15 | .6 | .11 |
| 5 | gen_Alum | 10100 | 4077 | .3 | 1.29 | .173 |
| 6 | gen_Steel | 29000 | 11154 | .3 | .65 | .49 |
| 7 | gen_Plywood | 1800 | 38 | 0 | .3 | .035 |
| 8 | RIGID | 1e+6 | | .3 | 0 | 0 |

Joint Boundary Conditions

| | Joint Label | X [k/in] | Y [k/in] | Z [k/in] | X Rot.[k-ft/rad] | Y Rot.[k-ft/rad] | Z Rot.[k-ft/rad] |
|---|-------------|----------|----------|----------|------------------|------------------|------------------|
| 1 | N1 | Reaction | Reaction | Reaction | | Reaction | |
| 2 | N3 | Reaction | Reaction | Reaction | | Reaction | |

Member Primary Data

| | Label | I Joint | J Joint | K Joint | Rotate(d...) | Section/Shape | Type | Design List | Material | Design Ru... |
|---|-------|---------|---------|---------|--------------|---------------|--------|-------------|----------|--------------|
| 1 | M1 | N1 | N2 | | | Arm | Beam | SquareTube | A53 Gr.B | Typical |
| 2 | M2 | N3 | N4 | | | Arm | Beam | SquareTube | A53 Gr.B | Typical |
| 3 | M3 | N5 | N6 | | | Bracing | Column | Pipe | A53 Gr.B | Typical |
| 4 | M4 | N7 | N8 | | | Mount Pipe | Column | Pipe | A53 Gr.B | Typical |
| 5 | M5 | N9 | N10 | | | RIGID | None | None | RIGID | Typical |
| 6 | M6 | N12 | N13 | | | RIGID | None | None | RIGID | Typical |
| 7 | M7 | N15 | N14 | | | RIGID | None | None | RIGID | Typical |

Member Advanced Data

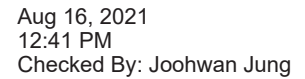
| | Label | I Release | J Release | I Offset[in] | J Offset[in] | T/C Only | Physical | Defl Rat... | Analysis ... | Inactive | Seismic... |
|---|-------|-----------|-----------|--------------|--------------|----------|----------|-------------|--------------|----------|------------|
| 1 | M1 | | | | | | Yes | | | | None |
| 2 | M2 | | | | | | Yes | | | | None |
| 3 | M3 | | | | | | Yes | ** NA ** | | | None |
| 4 | M4 | | | | | | Yes | ** NA ** | | | None |
| 5 | M5 | | | | | | Yes | ** NA ** | | | None |
| 6 | M6 | | | | | | Yes | ** NA ** | | | None |
| 7 | M7 | | | | | | Yes | ** NA ** | | | None |

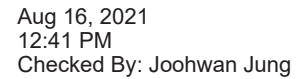
Hot Rolled Steel Design Parameters

| | Label | Shape | Length[in] | Lbyy[in] | Lbzz[in] | Lcomp top[i.. | Lcomp bot[in] | L-torqu... | Kyy | Kzz | Cb | Function |
|---|-------|------------|------------|----------|----------|---------------|---------------|------------|-----|-----|----|----------|
| 1 | M1 | Arm | 43.5 | | | Lbyy | | | | | | Lateral |
| 2 | M2 | Arm | 43.5 | | | Lbyy | | | | | | Lateral |
| 3 | M3 | Bracing | 36 | | | | | | | | | Lateral |
| 4 | M4 | Mount Pipe | 56.5 | | | | | | | | | Lateral |

Basic Load Cases

| | BLC Description | Category | X Gravity | Y Gravity | Z Gravity | Joint | Point | Distributed Area(Me... | Surface(P... |
|----|----------------------|----------|-----------|-----------|-----------|-------|-------|------------------------|--------------|
| 1 | DL | DL | | -1 | | 1 | | | |
| 2 | Maintenance LL - LV | LL | | | | 1 | | | |
| 3 | Installation LL - LM | LL | | | | 1 | | | |
| 4 | Wind - 0 Deg (X) | WL | | | | 1 | | 4 | |
| 5 | Wind - 30 Deg (X) | WL | | | | 1 | | 4 | |
| 6 | Wind - 60 Deg (X) | WL | | | | 1 | | 4 | |
| 7 | Wind - 90 Deg (X) | WL | | | | 1 | | 4 | |
| 8 | Wind - 120 Deg (X) | WL | | | | 1 | | 4 | |
| 9 | Wind - 150 Deg (X) | WL | | | | 1 | | 4 | |
| 10 | Wind - 180 Deg (X) | WL | | | | 1 | | 4 | |
| 11 | Wind - 210 Deg (X) | WL | | | | 1 | | 4 | |
| 12 | Wind - 240 Deg (X) | WL | | | | 1 | | 4 | |
| 13 | Wind - 270 Deg (X) | WL | | | | 1 | | 4 | |
| 14 | Wind - 300 Deg (X) | WL | | | | 1 | | 4 | |
| 15 | Wind - 330 Deg (X) | WL | | | | 1 | | 4 | |
| 16 | Wind - 0 Deg (Z) | WL | | | | 1 | | 4 | |
| 17 | Wind - 30 Deg (Z) | WL | | | | 1 | | 4 | |
| 18 | Wind - 60 Deg (Z) | WL | | | | 1 | | 4 | |



Page 5

**APPENDIX 3:
ATTACHMENTS**



THOMPSON CSP
97 MOUNTAIN HILL ROAD
THOMPSON, CT 06255

PROJECT SUMMARY

THE GENERAL SCOPE OF WORK CONSISTS OF THE FOLLOWING

1. INSTALL (1) OMNI/WHIP ANTENNA AT ELEVATION 159'-1 1/2" AGL AND INSTALL (1) NEW DIPOLE ANTENNA AT ELEVATION 135'-9" AGL INSTEAD OF (1) OMNI/WHIP ANTENNA AT ELEVATION 136'-1 1/2" AGL AT A LATER DATE
2. REMOVE (2) EXISTING ANTENNAS AND THEIR ASSOCIATED COAX, (1) AT ELEVATION 127'-0"± AGL AND (1) AT ELEVATION 47'-0"± AGL
3. INSTALL (1) NEW RACK WITH DMR EQUIPMENT IN EXISTING TELECOM ROOM

GOVERNING CODES

2018 CONNECTICUT STATE BUILDING CODE (2015 IBC BASIS)
2017 NATIONAL ELECTRIC CODE
TIA-222-H

GENERAL NOTES

THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON DRAINAGE; NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.

SITE INFORMATION

SITE NAME: THOMPSON CS
SITE ID NUMBER: #848

SITE ADDRESS: 97 MOUNTAIN HILL ROAD
THOMPSON, CT 06255

MAP: 42
BLOCK: 88
LOT: 165
ZONE: RA8C

LATITUDE: 41° 59' 11.76" N
LONGITUDE: 71° 54' 49.11" W
ELEVATION: 558± AMSL

FEMA/FIRM DESIGNATION: C
ACREAGE: 0.23± AC (BOOK: 0248. PAGE: 0073)

CONTACT INFORMATION

APPLICANTS:
EVERSOURCE ENERGY
107 SELDEN STREET
BERLIN, CT 06037

POWER PROVIDER:
EVERSOURCE ENERGY
(800) 286-2000

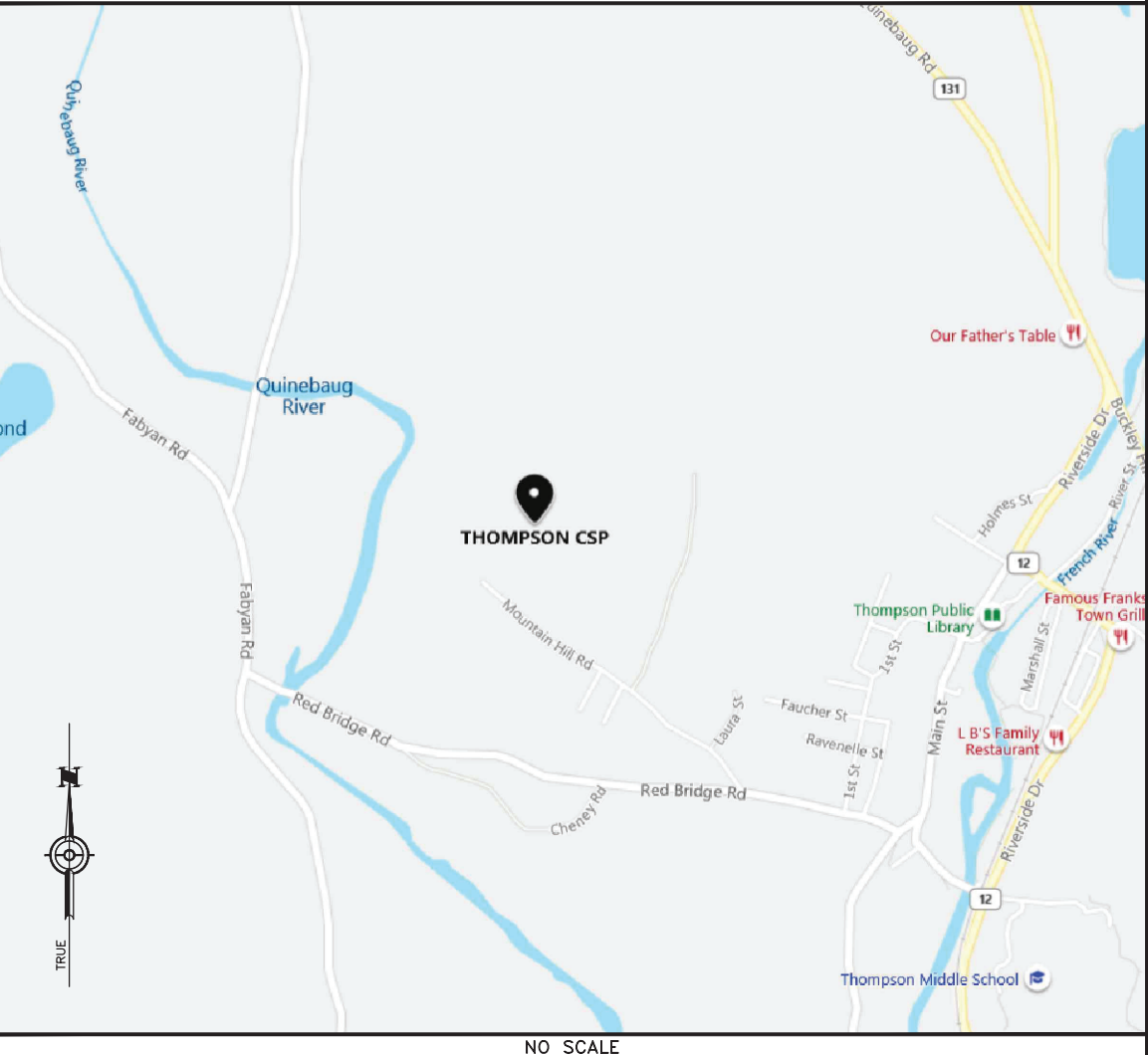
PROPERTY OWNER:
CONNECTICUT STATE POLICE
165 CAPITOL AVE
HARTFORD, CT 06106

TELCO PROVIDER
FRONTIER
(800) 921-8102

EVERSOURCE ENERGY
PROJECT MANAGER:
NIKOLL PRECI
(860) 655-3079

CALL BEFORE YOU DIG
(800) 922-4455

LOCATION MAP



DESIGN TYPE

SITE UPGRADE
SELF-SUPPORT TOWER

DRAWING INDEX

| SHEET NO: | SHEET TITLE |
|-----------|---------------|
| T-1 | TITLE SHEET |
| C-1 | SITE PLAN |
| C-2 | TOWER ELEV. |
| G-1 | GROUNDING |
| N-1 | NOTES & SPEC. |
| N-2 | NOTES & SPEC. |
| N-3 | NOTES & SPEC. |

DO NOT SCALE DRAWINGS

SUBCONTRACTOR SHALL VERIFY ALL PLANS & EXISTING DIMENSIONS & CONDITIONS ON THE JOB SITE & SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME



UNDERGROUND
SERVICE ALERT
UTILITIES PROTECTION CENTER, INC.
811

48 HOURS BEFORE YOU DIE



107 SELDEN STREET
BERLIN, CT 06037
PHONE: (800) 286-2000



BLACK & VEATCH

6800 W 115TH ST, SUITE 2292
OVERLAND PARK, KS 66211
PHONE: (913) 458-2522

PROJECT NO: 405025

DRAWN BY: TYW

CHECKED BY: TH

| | | |
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| 1 | 07/13/21 | ISSUED FOR FILING |
| 0 | 12/16/20 | ISSUED FOR FILING |
| REV | DATE | DESCRIPTION |

IT IS A VIOLATION OF LAW FOR ANY PERSON,
UNLESS THEY ARE ACTING UNDER THE DIRECTION
OF A LICENSED PROFESSIONAL ENGINEER,
TO ALTER THIS DOCUMENT.

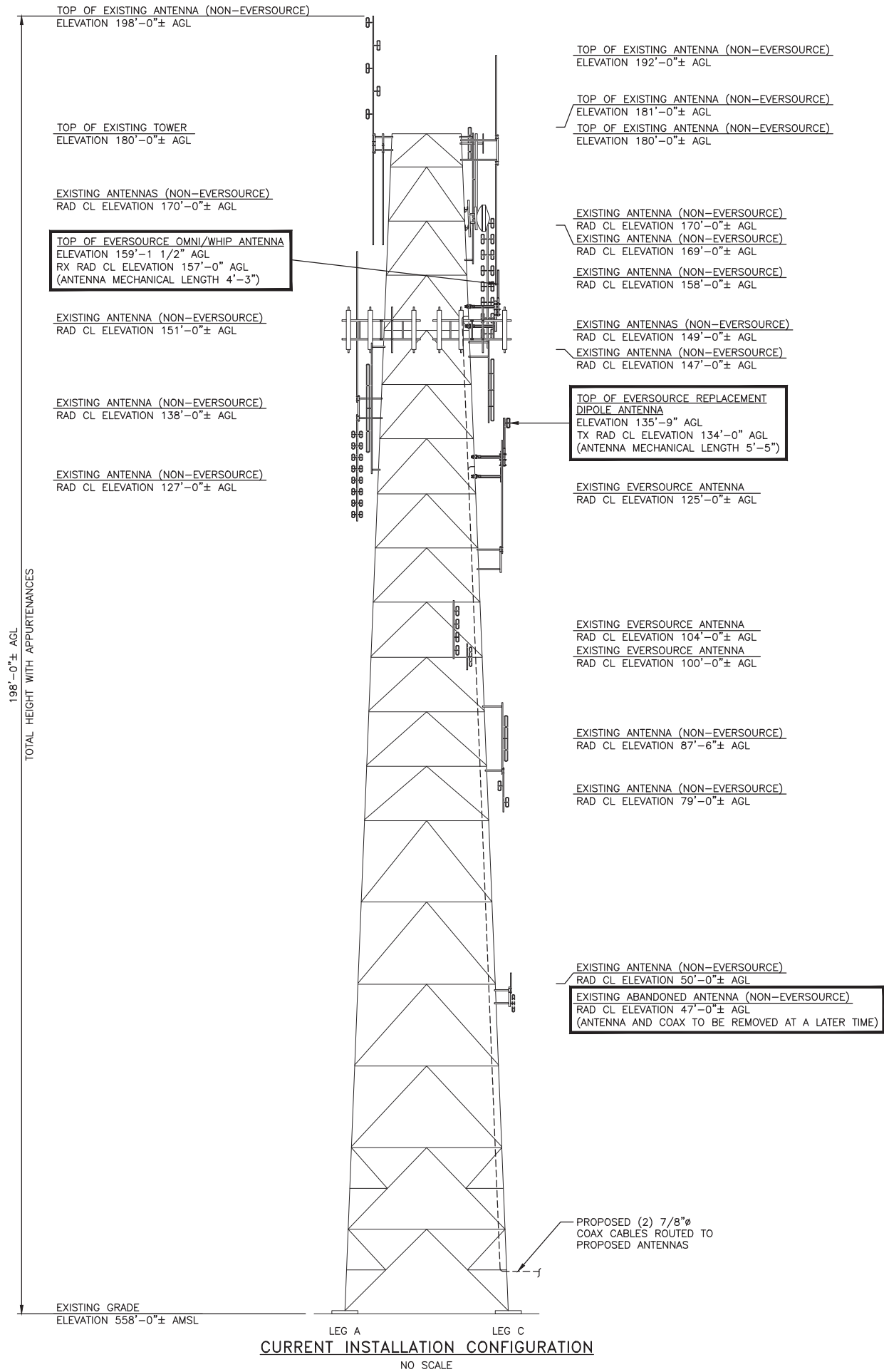
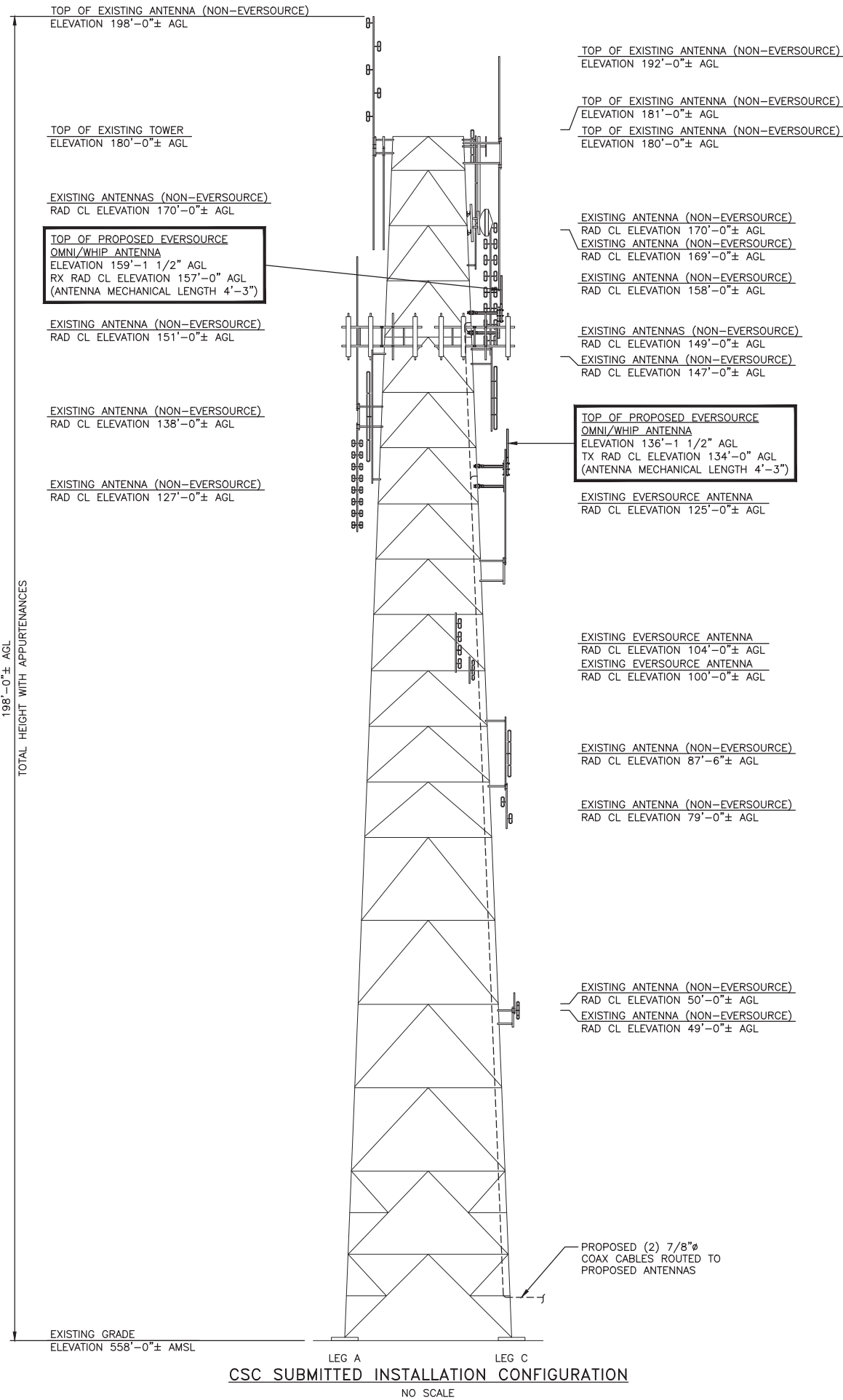
THOMPSON CSP
97 MOUNTAIN HILL ROAD
THOMPSON, CT 06255

SHEET TITLE

TITLE SHEET

SHEET NUMBER

[-1



EVERSOURCE
ENERGY

107 SELDEN STREET
BERLIN, CT 06037
PHONE: (800) 286-2000



BLACK & VEATCH

6800 W 115TH ST, SUITE 2292
OVERLAND PARK, KS 66211
PHONE: (913) 458-2522

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|-------------|--------|
| PROJECT NO: | 405025 |
| DRAWN BY: | TYW |
| CHECKED BY: | TH |

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TO ALTER THIS DOCUMENT.

THOMPSON CSP
97 MOUNTAIN HILL ROAD
THOMPSON, CT 06255

SHEET TITLE
TOWER ELEVATION &
ANTENNA EQUIPMENT

SHEET NUMBER

C-2

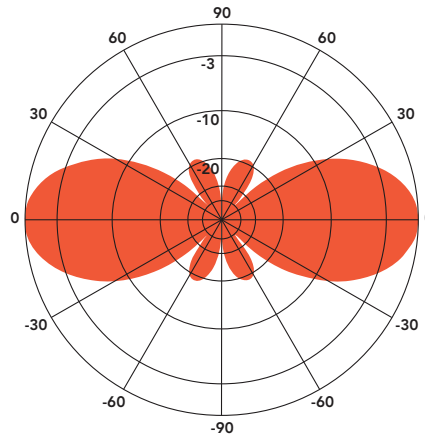
ANT220F2DIN

FIBERGLASS COLLINEAR ANTENNA 2.5 dBd

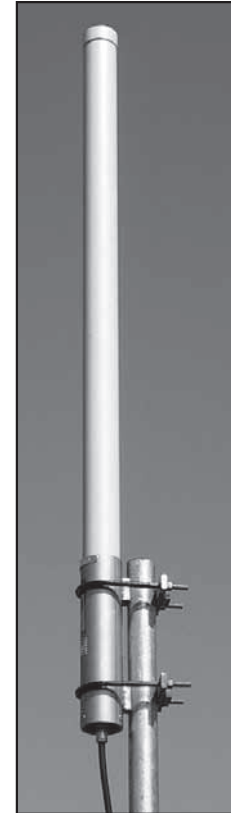
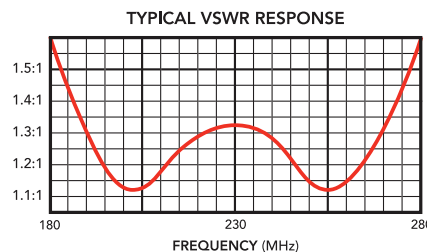
The Telewave ANT220F2 is an extremely rugged collinear antenna, with moderate gain and wide vertical beamwidth. This compact antenna produces 2.5 dBd gain, and is designed for operation in all environmental conditions. The antenna is constructed with brass and copper elements, with a path to ground potential for lightning impulse protection. The ANT220F2 is an excellent choice for wireless PTC systems in urban or rural areas.

All junctions are fully soldered to prevent RF intermodulation, and each antenna is completely protected within a rugged, high-tech radome to ensure survivability in the worst environments. The "Cool Blue" radome provides maximum protection from corrosive gases, ultraviolet radiation, icing, salt spray, acid rain, and wind blown abrasives.

The ANT220F2 includes the ANTC485 dual clamp set for mounting to a 1.5" to 3" O.D. support pipe, and a 24" removable RG-213 DIN-Male jumper.



ANT220F2 - 230 MHz
Vertical Plane
Gain = 2.58 dBd



SPECIFICATIONS

| | | | |
|------------------------|-----------------|-------------------------------------|----------------------|
| Frequency (continuous) | 195-260 MHz | Dimensions (L x base diam.) in. | 51 x 2.75 |
| Gain | 2.5 dBd | Tower weight (antenna + clamps) | 11 lb. |
| Power rating (typ.) | 500 watts | Shipping weight | 14 lb. |
| Impedance | 50 ohms | Wind rating / with 0.5" ice | 200 / 150 MPH |
| VSWR | 1.5:1 or less | Maximum exposed area | 1.1 ft. ² |
| Pattern | Omnidirectional | Lateral thrust at 100 MPH | 44 lb. |
| Vertical beamwidth | 38° | Bending moment at top clamp | 47 ft. lb. |
| Termination | 7-16 DIN-F | (100 MPH, 40 PSF flat plate equiv.) | |

870 Series 220MHz Exposed Dipoles

The 870 Series 220MHz Exposed Dipoles are available in 1, 2, 4, 8 dipole configurations. All our antennas can be completely customized to your particular applications. Our antennas can be black anodized, adjustable, or fixed, side mount or top mount, and heavy-duty versions are available.

- Each antenna is offered in a 1/4, 3/8 or 1/2 wave spacing versions.
- The 87XA-70 has external cabling and a field-adjustable pattern.
- The 87XF-70 has internal cabling and fixed dipole-mast spacing.
- Heavy-duty versions are available. Please contact our Technical Support team for consultation.

| Electrical Specifications | 871F-70-2 | 872F-70-2 | 874F-70-2 |
|---|---------------|-------------|-------------|
| Frequency Range, MHz | 215-225 | 215-225 | 215-225 |
| Nominal Gain, dBd | 2.0-2.5 | 5.0-5.5 | 8.0-8.5 |
| Number of Dipoles | 1 | 2 | 4 |
| Bandwidth 1.5:1 VSWR, MHz | 10 | 10 | 10 |
| Polarization | Vertical | Vertical | Vertical |
| Pattern | Offset / bi | Offset / bi | Offset / bi |
| Power Rating, Watts | 200 | 300 | 500 |
| Nominal Impedance, Ohms | 50 | 50 | 50 |
| Lightning Protection | DC Ground | DC Ground | DC Ground |
| Standard Termination | Type DIN Male | Type N Male | Type N Male |
| Mechanical Specifications | 871F-70-2 | 872F-70-2 | 874F-70-2 |
| Length, in (mm) | 66 (1676) | 112 (2845) | 200 (5080) |
| Width (1/2 Wave Spacing), in (mm) | 31 (787) | 31 (787) | 32 (813) |
| Weight, lbs. (kg) | 12.5 (5.7) | 21 (9.5) | 51 (23) |
| Rated Wind Velocity, No Ice, mph (km/h) | 165 (266) | 150 (241) | 145 (233) |
| Rated Wind Velocity, 0.5" (13mm) ice, mph (km/h) | 140 (225) | 130 (209) | 105 (177) |
| Lateral Thrust @ 100 mph, wind, lbs. (kg) | 40 (18) | 66 (30) | 143 (65) |
| Bending Moment @ top clamp: 100 mph, ft.*lb (kg*m) | 58 (8) | 150 (21) | 610 (84) |
| Projected Area, ft ² (m ²) | 1.5 (0.14) | 2.6 (0.24) | 5.5 (0.51) |
| Mounting Information Mast O.D. (mm) | 1.9" (48) | 1.9" (48) | 2.4" (60) |
| * See next page for ordering information (page 3) * | | | |

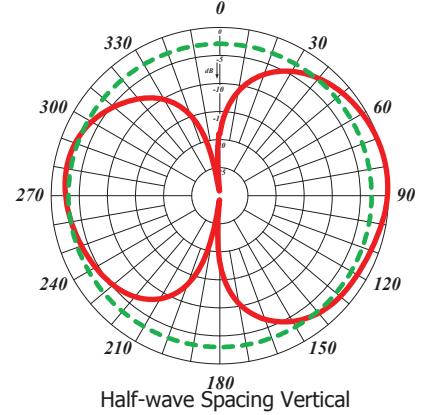
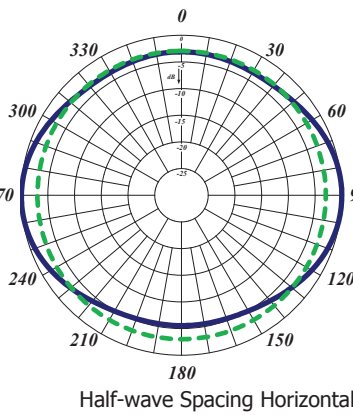
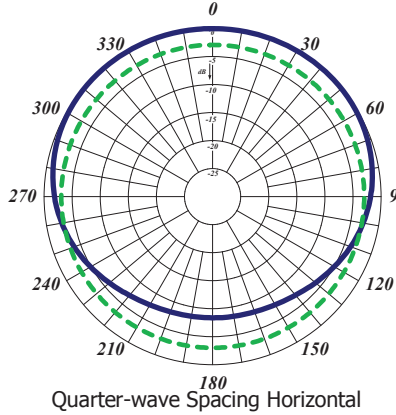


220MHz EXPOSED DIPOLES

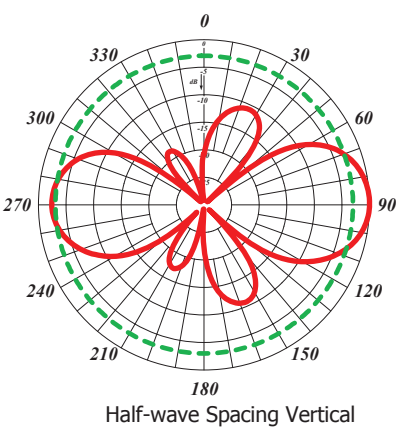
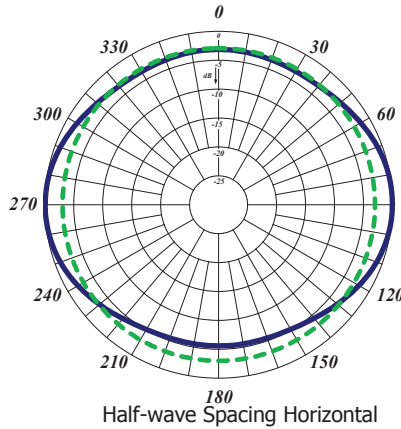
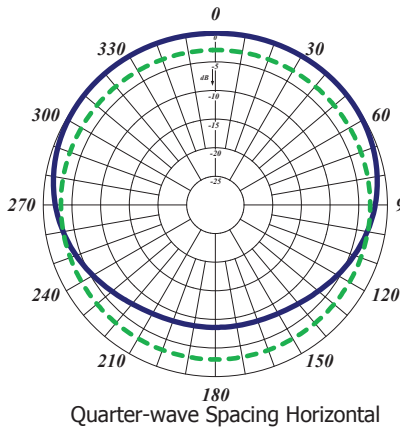
215-225 MHz



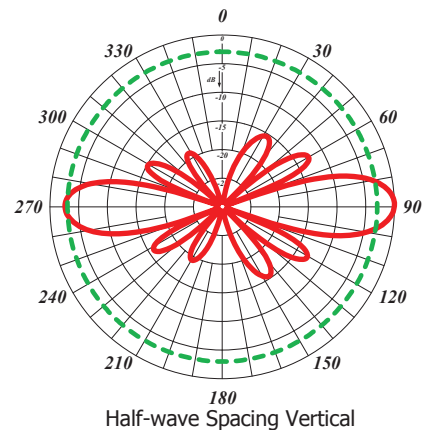
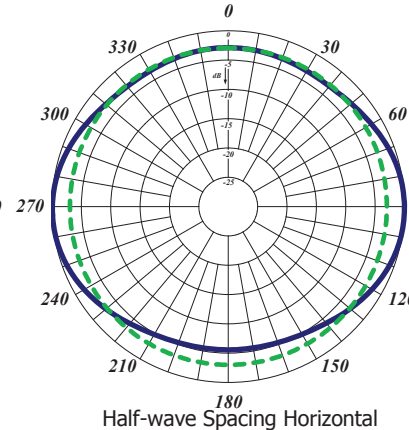
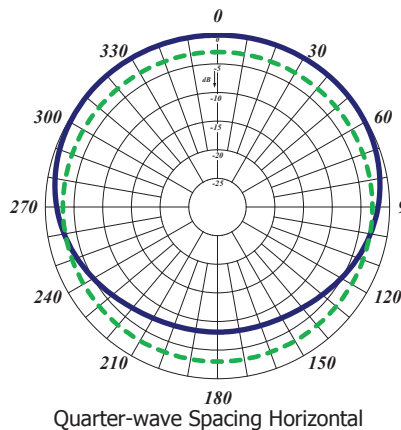
871F-70-2



872F-70-2



874F-70-2



ORIGINAL TRANSMIT (TX) ANTENNA, REMOVED AND REPLACED

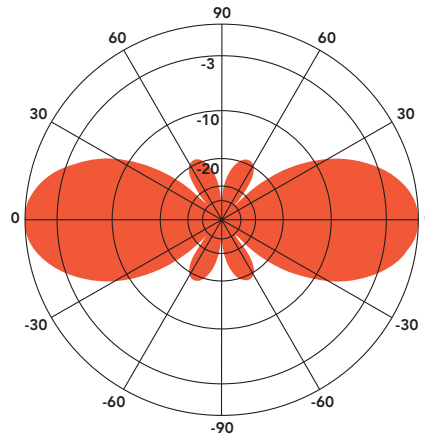
ANT220F2DIN

FIBERGLASS COLLINEAR ANTENNA 2.5 dBd

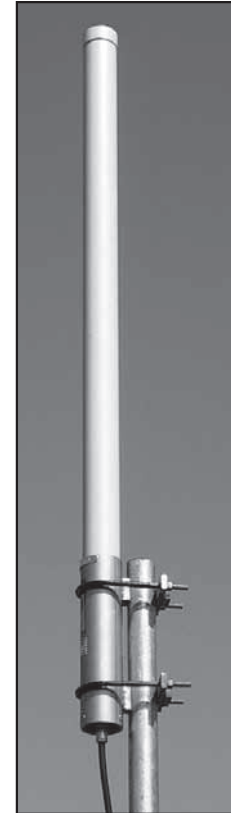
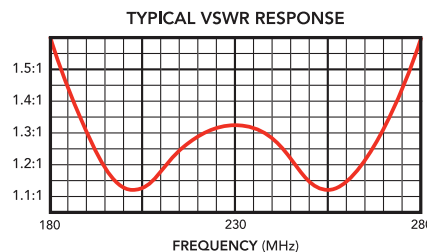
The Telewave ANT220F2 is an extremely rugged collinear antenna, with moderate gain and wide vertical beamwidth. This compact antenna produces 2.5 dBd gain, and is designed for operation in all environmental conditions. The antenna is constructed with brass and copper elements, with a path to ground potential for lightning impulse protection. The ANT220F2 is an excellent choice for wireless PTC systems in urban or rural areas.

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The ANT220F2 includes the ANTC485 dual clamp set for mounting to a 1.5" to 3" O.D. support pipe, and a 24" removable RG-213 DIN-Male jumper.



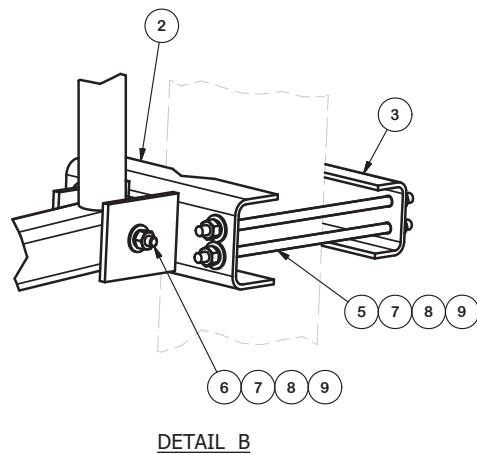
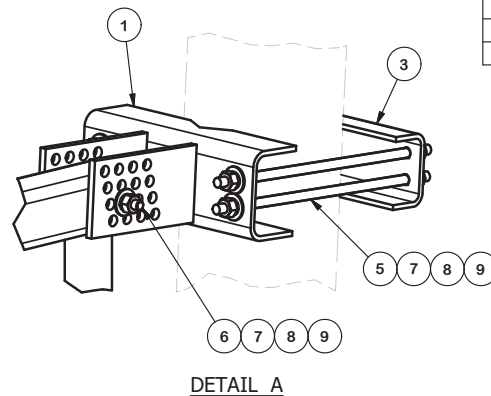
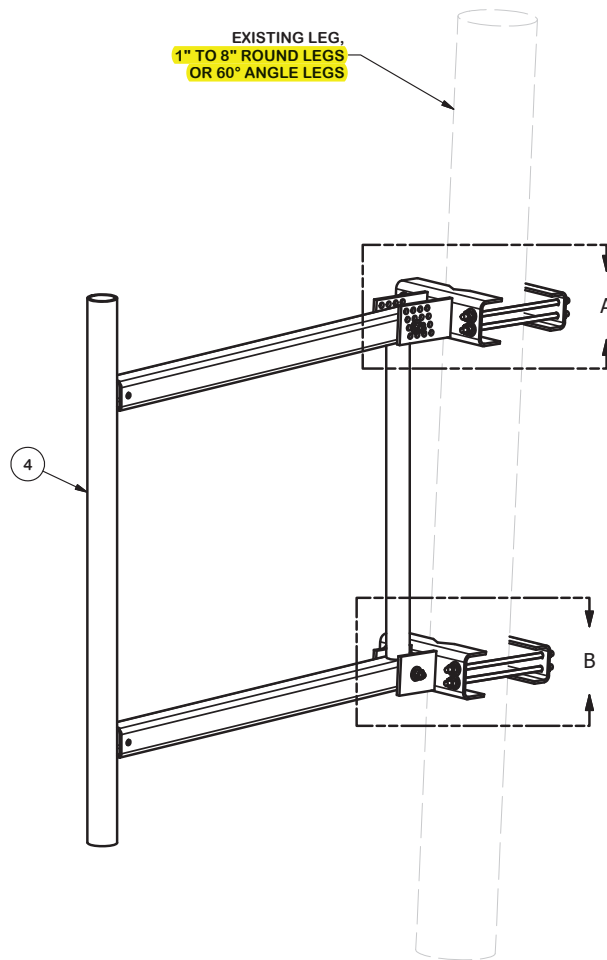
ANT220F2 - 230 MHz
Vertical Plane
Gain = 2.58 dBd



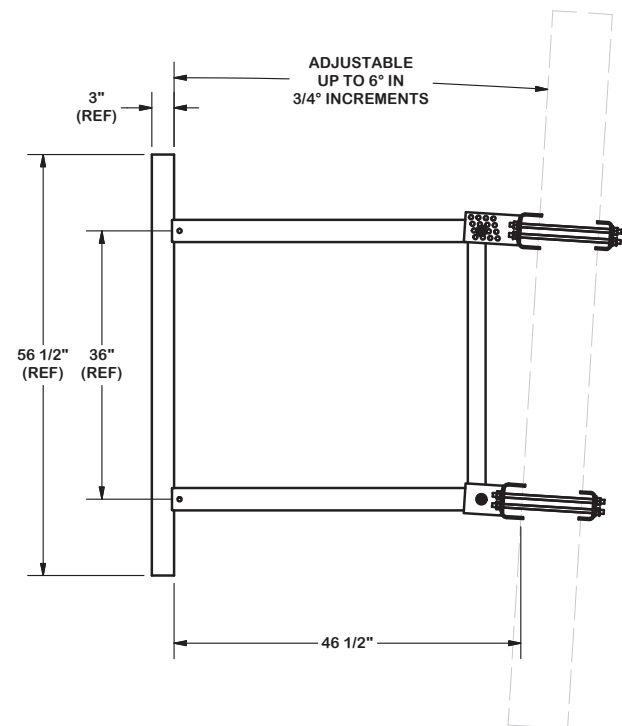
SPECIFICATIONS

| | | | |
|------------------------|-----------------|-------------------------------------|----------------------|
| Frequency (continuous) | 195-260 MHz | Dimensions (L x base diam.) in. | 51 x 2.75 |
| Gain | 2.5 dBd | Tower weight (antenna + clamps) | 11 lb. |
| Power rating (typ.) | 500 watts | Shipping weight | 14 lb. |
| Impedance | 50 ohms | Wind rating / with 0.5" ice | 200 / 150 MPH |
| VSWR | 1.5:1 or less | Maximum exposed area | 1.1 ft. ² |
| Pattern | Omnidirectional | Lateral thrust at 100 MPH | 44 lb. |
| Vertical beamwidth | 38° | Bending moment at top clamp | 47 ft. lb. |
| Termination | 7-16 DIN-F | (100 MPH, 40 PSF flat plate equiv.) | |

TOWER/MAST SIZE AT PROPOSED ANTENNA ATTACHMENT = 5.0"± DIAMETER.



| PARTS LIST | | | | | |
|------------|-----|----------|-------------------------------|-------------|------------------|
| ITEM | QTY | PART NO. | PART DESCRIPTION | LENGTH | UNIT WT. NET WT. |
| 1 | 1 | CFM | UPPER GATE FOOT WELDMENT | | 13.90 13.90 |
| 2 | 1 | CFS | LOWER GATE FOOT WELDMENT | | 12.72 12.72 |
| 3 | 2 | GBB | GATE BACKING BAR | | 4.53 9.06 |
| 4 | 1 | 4PBG | 48" PIPE MOUNT STANDOFF ARM | | 113.96 113.96 |
| 5 | 8 | G12R-12 | 1/2" x 12" GALV. THREADED ROD | | 0.67 5.35 |
| 5 | 8 | G12R-15 | 1/2" x 15" GALV. THREADED ROD | | 0.84 6.69 |
| 6 | 2 | A1205 | 1/2" x 5" A325 HDG BOLT | | 0.34 0.69 |
| 7 | 18 | G12FW | 1/2" HDG USS FLATWASHER | | 0.03 0.61 |
| 8 | 18 | G12LW | 1/2" HDG LOCKWASHER | | 0.01 0.25 |
| 9 | 18 | G12NUT | 1/2" HDG HEAVY 2H HEX NUT | | 0.07 1.29 |
| | | | | TOTAL WT. # | 164.53 |



TOLERANCE NOTES

TOLERANCES ON DIMENSIONS, UNLESS OTHERWISE NOTED ARE:
SAWED, SHEARED AND GAS CUT EDGES ($\pm 0.030"$)
DRILLED AND GAS CUT HOLES ($\pm 0.030"$) - NO CONING OF HOLES
LASER CUT EDGES AND HOLES ($\pm 0.010"$) - NO CONING OF HOLES
BENDS ARE $\pm 1/2$ DEGREE
ALL OTHER MACHINING ($\pm 0.030"$)
ALL OTHER ASSEMBLY ($\pm 0.060"$)

PROPRIETARY NOTE:
THE DATA AND TECHNIQUES CONTAINED IN THIS DRAWING ARE PROPRIETARY INFORMATION OF VALMONT INDUSTRIES AND CONSIDERED A TRADE SECRET. ANY USE OR DISCLOSURE WITHOUT THE CONSENT OF VALMONT INDUSTRIES IS STRICTLY PROHIBITED.

DESCRIPTION

48" ULTIMATE UNIVERSAL
STANDOFF FRAME

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| CPD NO. | DRAWN BY | ENG. APPROVAL |
| | RCH 2/4/2011 | |
| CLASS | DRAWING USAGE | CHECKED BY |
| 81 | 01 CUSTOMER | BMC 2/16/2011 |

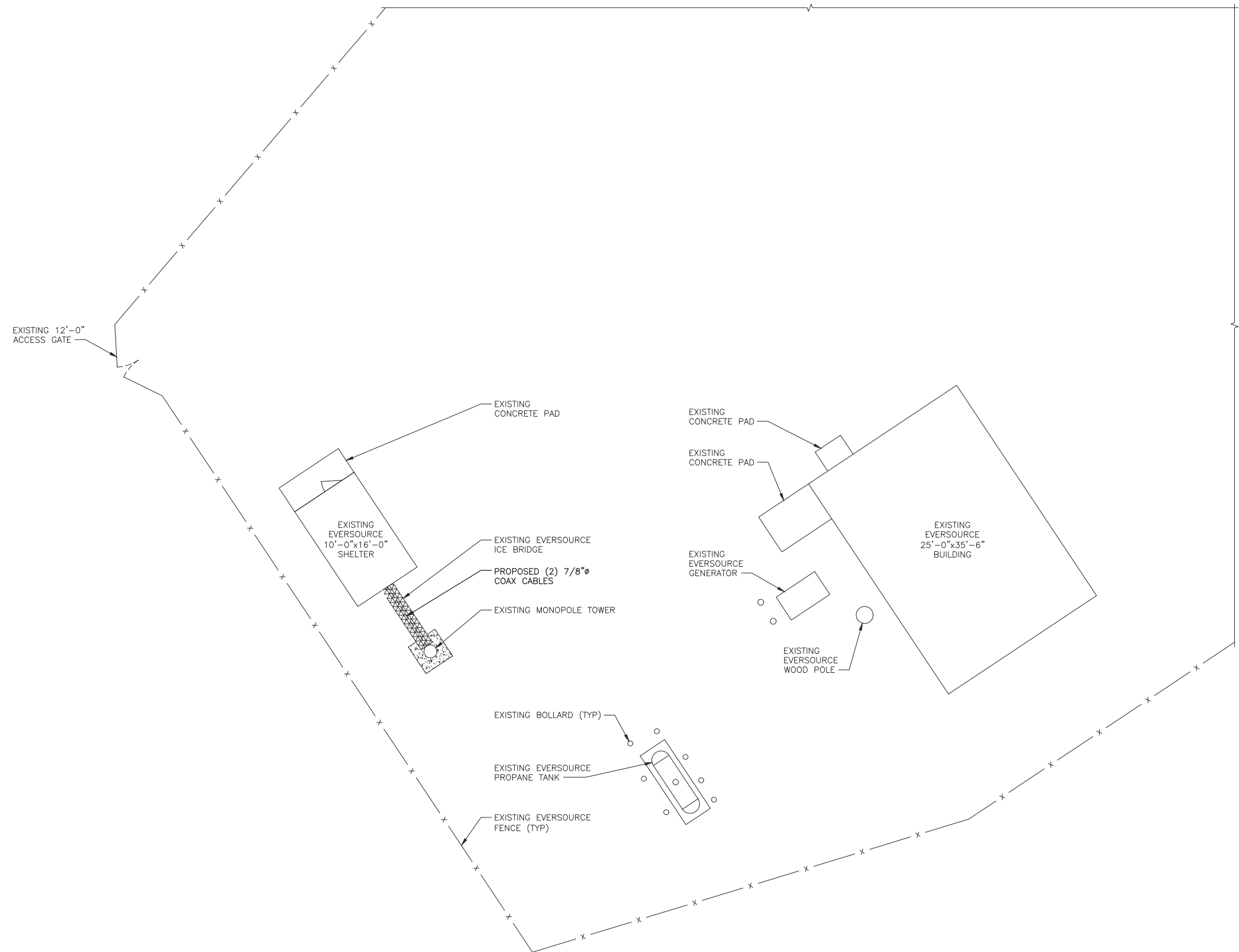


Engineering
Support Team:
1-888-753-7446

Locations:
New York, NY
Atlanta, GA
Los Angeles, CA
Plymouth, IN
Salem, OR
Dallas, TX

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| PART NO. | USF-4U | PAGE |
| DWG. NO. | USF-4U | 1 OF 1 |

ATTACHMENT C – CONSTRUCTION DRAWINGS



SITE PLAN
NO SCALE

EVERSOURCE
ENERGY

107 SELDEN STREET
BERLIN, CT 06037
PHONE: (800) 286-2000

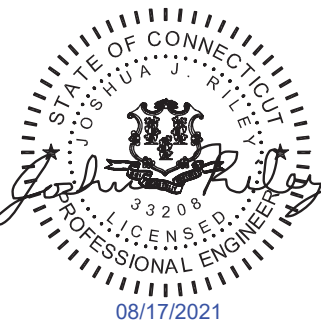


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PHONE: (913) 458-3595

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| CHECKED BY: | JR |

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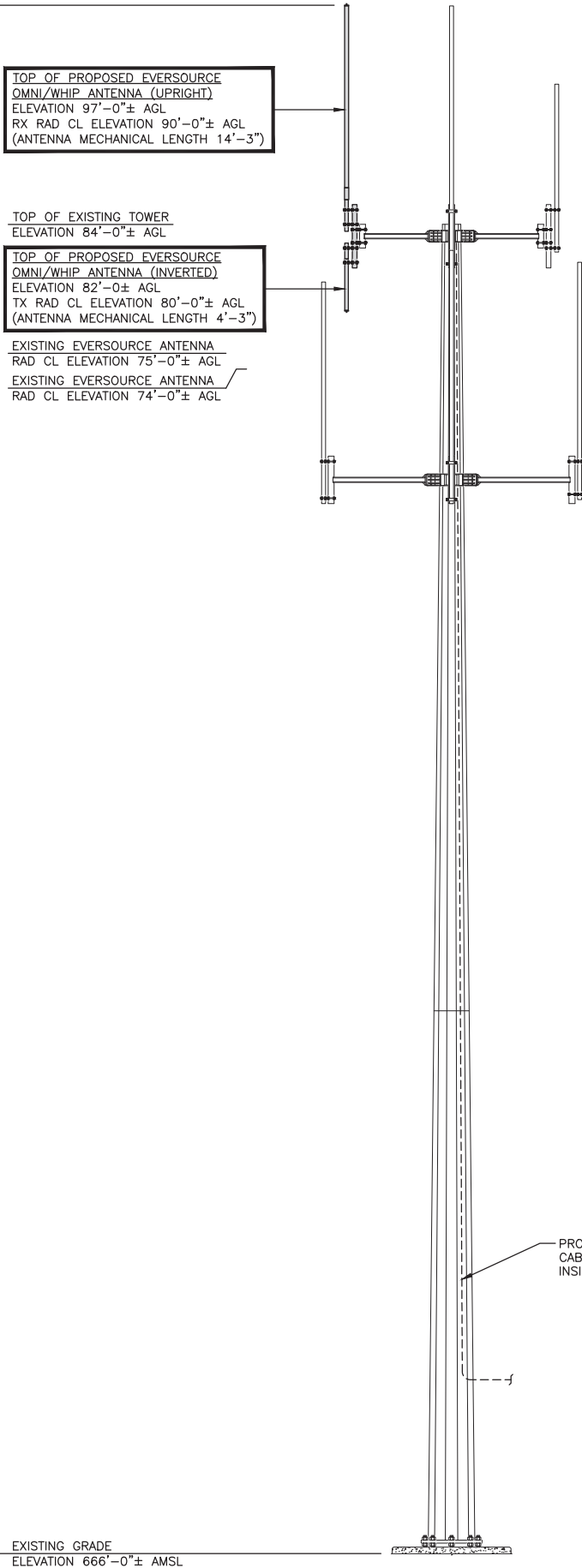
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RIDGEFIELD 22N
1 PROSPECT STREET
RIDGEFIELD, CT 06877

SHEET TITLE
SITE PLAN

SHEET NUMBER
C-1

97'-0"± AGL
TOTAL HEIGHT WITH APPURTENANCES



EXISTING GRADE
ELEVATION 666'-0"± AMSL

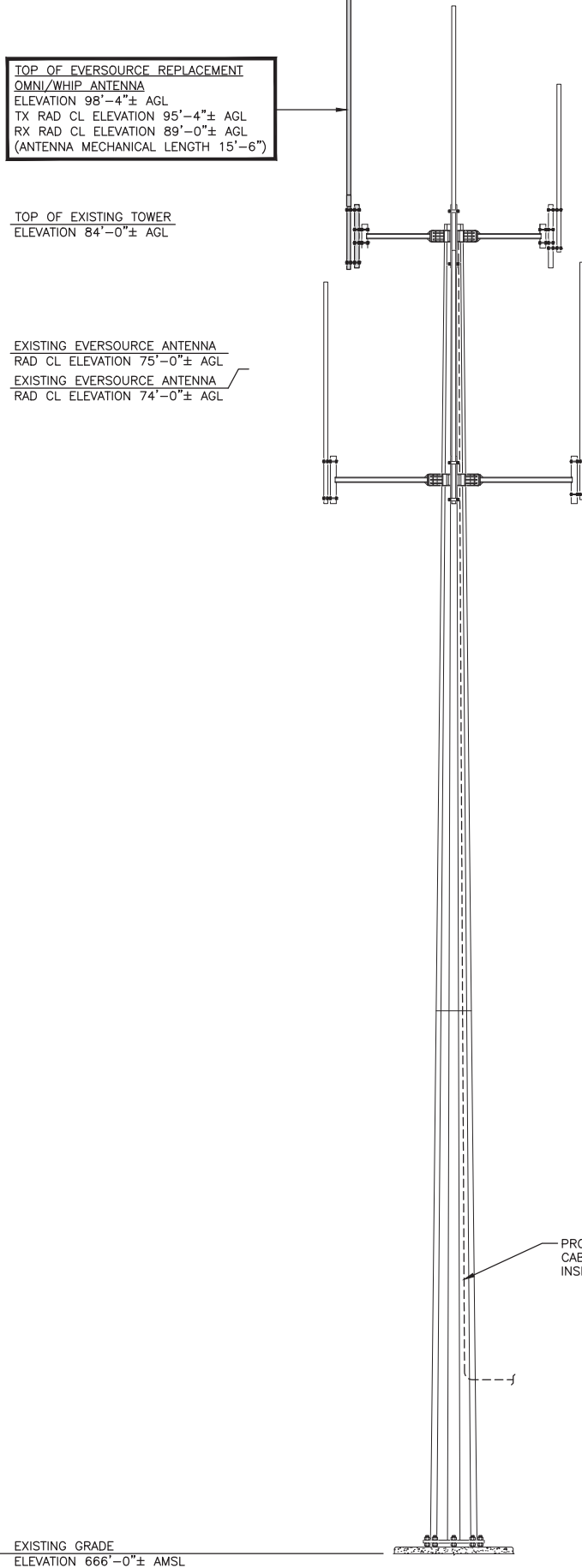
CSC SUBMITTED INSTALLATION CONFIGURATION
NO SCALE

TOP OF EXISTING EVERSOURCE ANTENNA
ELEVATION 97'-0"± AGL

TOP OF EXISTING EVERSOURCE ANTENNA
ELEVATION 92'-0"± AGL

EXISTING RELOCATED EVERSOURCE ANTENNA
RAD CL ELEVATION 75'-0"± AGL

98'-4"± AGL
TOTAL HEIGHT WITH APPURTENANCES



EXISTING GRADE
ELEVATION 666'-0"± AMSL

CURRENT INSTALLATION CONFIGURATION
NO SCALE

TOP OF EXISTING EVERSOURCE ANTENNA
ELEVATION 97'-0"± AGL

TOP OF EXISTING EVERSOURCE ANTENNA
ELEVATION 92'-0"± AGL

EXISTING RELOCATED EVERSOURCE ANTENNA
RAD CL ELEVATION 75'-0"± AGL

EVERSOURCE
ENERGY

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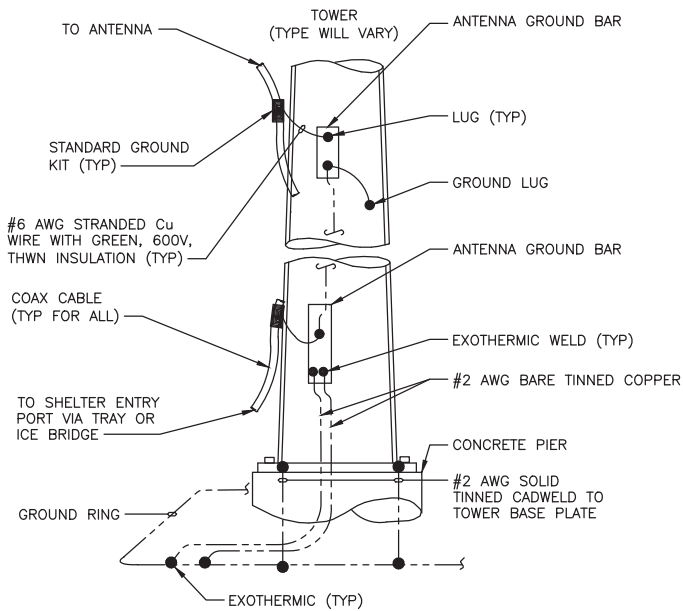


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RIDGEFIELD 22N
1 PROSPECT STREET
RIDGEFIELD, CT 06877

SHEET TITLE
TOWER
ELEVATION

SHEET NUMBER
C-2

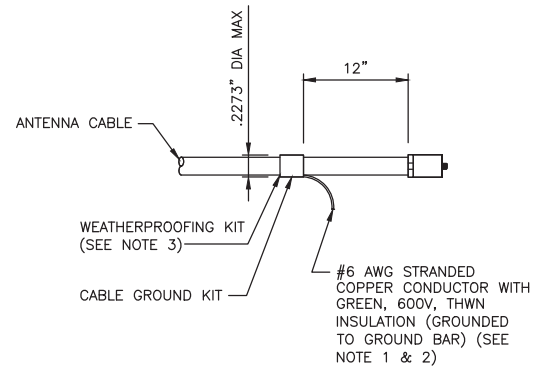


NOTE

1. NUMBER OF GROUND BARS MAY VARY DEPENDING ON THE TYPE OF TOWER, ANTENNA LOCATION AND CONNECTION ORIENTATION. PROVIDE AS REQUIRED.

ANTENNA CABLE GROUNDING

NO SCALE

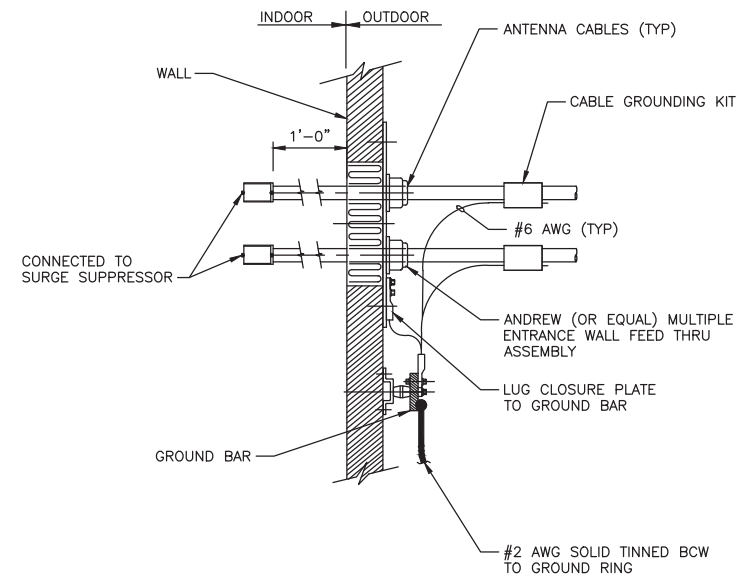


NOTES

1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR.
2. GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.
3. WEATHER PROOFING SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.

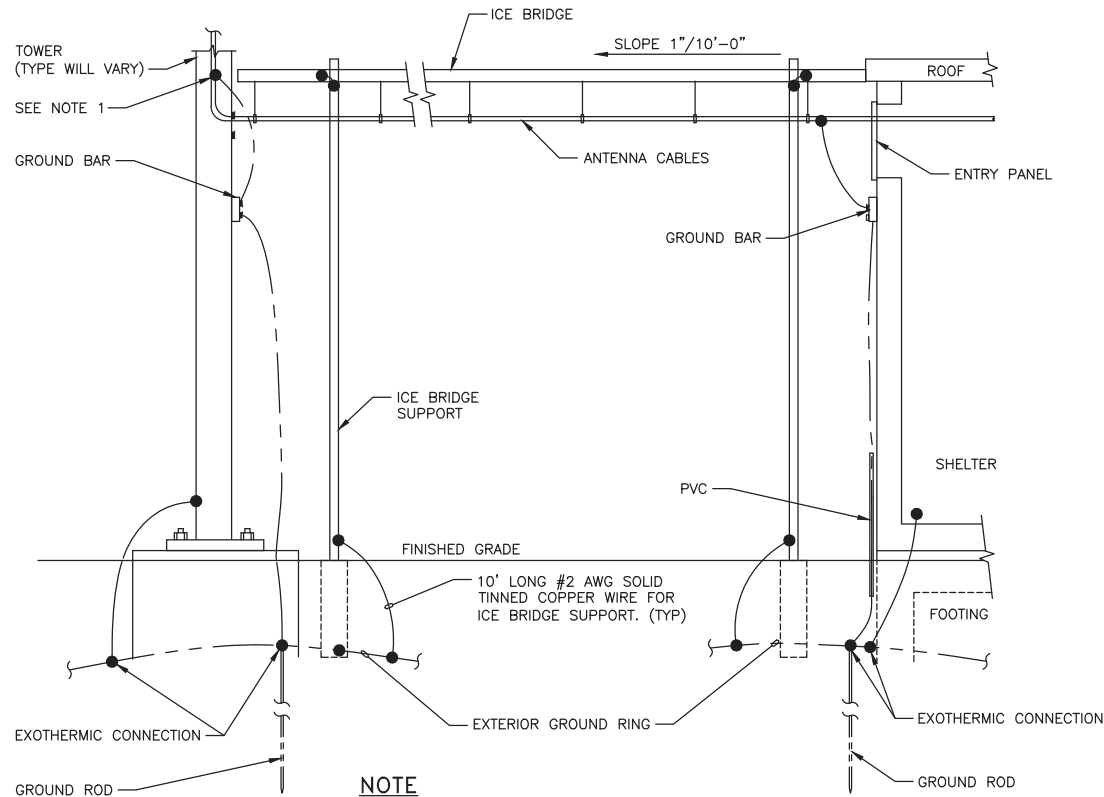
CONNECTION OF CABLE GROUND KIT TO ANTENNA CABLE

NO SCALE



CABLE INSTALLATION WITH WALL FEED THRU ASSEMBLY

NO SCALE



NOTE

1. PROVIDE GROUND KIT 6" BEFORE TURN

ICE BRIDGE AND ANTENNA CABLE DETAIL

NO SCALE

EVERSOURCE
ENERGY

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RIDGEFIELD 22N
1 PROSPECT STREET
RIDGEFIELD, CT 06877

SHEET TITLE
GROUNDING
DETAILS

SHEET NUMBER

G-1

DESIGN BASIS

1. GOVERNING CODE: 2018 CONNECTICUT STATE BUILDING CODE (2015 IBC BASIS).

GENERAL CONDITIONS

1. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO COMPLY WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL BUILDING CODES, PERMIT CONDITIONS AND SAFETY CODES DURING CONSTRUCTION.
2. THE ENGINEER IS NOT: A GUARANTOR OF THE INSTALLING CONTRACTOR'S WORK; RESPONSIBLE FOR SAFETY IN, ON OR ABOUT THE WORK SITE; IN CONTROL OF THE SAFETY OR ADEQUACY OF ANY BUILDING COMPONENT, SCAFFOLDING OR SUPERINTENDING THE WORK.
3. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL PERMITS, INSPECTIONS, TESTING AND CERTIFICATES NEEDED FOR LEGAL OCCUPANCY OF THE FINISHED PROJECT.
4. THE CONTRACTOR IS RESPONSIBLE TO REVIEW THIS COMPLETE PLAN SET AND VERIFY THE EXISTING CONDITIONS SHOWN IN THESE PLANS AS THEY RELATE TO THE WORK PRIOR TO SUBMITTING PRICE. SIGNIFICANT DEVIATIONS FROM WHAT IS SHOWN AFFECTING THE WORK SHALL BE REPORTED IMMEDIATELY TO THE CONSTRUCTION MANAGER.
5. DETAILS INCLUDED IN THIS PLAN SET ARE TYPICAL AND APPLY TO SIMILAR CONDITIONS.
6. EXISTING ELECTRICAL AND MECHANICAL FIXTURES, PIPING, WIRING, AND EQUIPMENT OBSTRUCTING THE WORK SHALL BE REMOVED AND/OR RELOCATED AS DIRECTED BY THE CONSTRUCTION MANAGER. TEMPORARY SERVICE INTERRUPTIONS MUST BE COORDINATED WITH OWNER.
7. THE CONTRACTOR SHALL DILIGENTLY PROTECT THE EXISTING BUILDING/SITE CONDITIONS AND THOSE OF ANY ADJOINING BUILDING/SITES AND RESTORE ANY DAMAGE CAUSED BY HIS ACTIVITIES TO THE PRE-CONSTRUCTION CONDITION.
8. THE CONTRACTOR SHALL SAFEGUARD AGAINST: CREATING A FIRE HAZARD, AFFECTING TENANT EGRESS OR COMPROMISING BUILDING SITE SECURITY MEASURES.
9. THE CONTRACTOR SHALL REMOVE ALL DEBRIS AND CONSTRUCTION WASTE FROM THE SITE EACH DAY. WORK AREAS SHALL BE SWEEPED AND MADE CLEAN AT THE END OF EACH WORK DAY.
10. THE CONTRACTOR'S HOURS OF WORK SHALL BE IN ACCORDANCE WITH LOCAL CODES AND ORDINANCES AND BE APPROVED BY OWNER.
11. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE CONSTRUCTION MANAGER IF ASBESTOS IS ENCOUNTERED DURING THE EXECUTION OF HIS WORK. THE CONTRACTOR SHALL CEASE ALL ACTIVITIES WHERE THE ASBESTOS MATERIAL IS FOUND UNTIL NOTIFIED BY THE CONSTRUCTION MANAGER TO RESUME OPERATIONS.

THERMAL & MOISTURE PROTECTION

1. FIRE-STOP ALL PENETRATIONS FOR ELECTRICAL CONDUITS OR WAVEGUIDE CABLING THROUGH BUILDING WALLS, FLOORS, AND CEILINGS SHALL BE FIRESTOPPED WITH ACCEPTED MATERIALS TO MAINTAIN THE FIRE RATING OF THE EXISTING ASSEMBLY. ALL FILL MATERIAL SHALL BE SHAPED, FITTED, AND PERMANENTLY SECURED IN PLACE. FIRESTOPPING SHALL BE INSTALLED IN ACCORDANCE WITH ASTM E814.
2. HILTI CP620 FIRE FOAM OR 3M FIRE BARRIER FILL, VOID OR CAVITY MATERIAL OR ACCEPTED EQUAL SHALL BE APPLIED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND ASSOCIATED UNDERWRITERS LABORATORIES (UL) SYSTEM NUMBER.
3. FIRESTOPPING SHALL BE APPLIED AS SOON AS PRACTICABLE AFTER PENETRATIONS ARE MADE AND EQUIPMENT INSTALLED.
4. FIRESTOPPED PENETRATIONS SHALL BE LEFT EXPOSED AND MADE AVAILABLE FOR INSPECTION BEFORE CONCEALING SUCH PENETRATIONS. FIRESTOPPING MATERIAL CERTIFICATES SHALL BE MADE AVAILABLE AT THE TIME OF INSPECTION.
5. ANY BUILDING ROOF PENETRATION AND/OR RESTORATION SHALL BE PERFORMED SO THAT THE ROOF WARRANTY IN PLACE IS NOT COMPROMISED. CONTRACTOR SHALL ARRANGE FOR OWNER'S ROOFING CONTRACTOR TO PERFORM ANY AND ALL ROOFING WORK IF SO REQUIRED BY EXISTING ROOF WARRANTY. OTHERWISE, ROOF SHALL BE MADE WATERTIGHT WITH LIKE CONSTRUCTION AS SOON AS PRACTICABLE AND AT COMPLETION OF CONSTRUCTION.
6. ALL PENETRATIONS INTO AND/OR THROUGH BUILDING EXTERIOR WALLS SHALL BE SEALED WITH SILICONE SEALER.
7. WHERE CONDUIT AND CABLES PENETRATES FIRE RATED WALLS AND FLOORS, FIRE GROUT ALL PENETRATIONS IN ORDER TO MAINTAIN THE FIRE RATING USING A LISTED FIRE SEALING DEVICE OR GROUT.
8. CONTRACTOR TO REMOVE AND RE-INSTALL ALL FIRE PROOFING AS REQUIRED DURING CONSTRUCTION.

SUBMITTALS

1. CONTRACTOR TO SUBMIT SHOP DRAWINGS TO ENGINEER FOR REVIEW PRIOR TO FABRICATION.
2. CONTRACTOR TO NOTIFY ENGINEER FOR INSPECTION PRIOR TO CLOSING PENETRATIONS.
3. CONTRACTORS SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. THE ENGINEER SHALL BE NOTIFIED OF ANY CONDITIONS WHICH PRECLUDE COMPLETION OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
4. ALL STEEL MATERIAL EXPOSED TO WEATHER SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 " ZINC (HOT-DIPPED GALVANIZED) COATINGS" ON IRON AND STEEL PRODUCTS.
5. THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS FOR REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW.

STEEL

1. MATERIAL:
- WIDE FLANGE: ASTM A572, GR 50
TUBING: ASTM A500, GR C
PIPE: ASTM A53, GR B
BOLTS: ASTM A325
GRATING: TYPE GW-2 (1"x3/16" BARS)
MISC. MATERIAL: ASTM A36
- ALL STEEL SHAPES SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 WITH A COATING WEIGHT OF 2 OZ/SF.
2. DAMAGED GALVANIZED SURFACES SHALL BE CLEANED WITH A WIRE BRUSH AND PAINTED WITH TWO COATS OF COLD ZINC, "GALVANOX", "DRY GALV", "ZINC IT", OR APPROVED EQUIVALENT, IN ACCORDANCE WITH MANUFACTURER'S GUIDELINES. TOUCH UP DAMAGED NON GALVANIZED STEEL WITH SAME PAINT IN SHOP OR FIELD.
3. DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AISC "MANUAL OF STEEL CONSTRUCTION" 13TH EDITION.
4. THE STEEL STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER COMPLETION. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION.
5. ALL STEEL ELEMENTS SHALL BE INSTALLED PLUMB AND LEVEL.
6. TOWER MANUFACTURER'S DESIGNS SHALL PREVAIL FOR TOWER.

SITE GENERAL

1. CONTRACTOR SHALL FOLLOW CONDITIONS OF ALL APPLICABLE PERMITS AND WORK IN ACCORDANCE WITH OSHA REGULATIONS.
2. THESE PLANS DEPICT KNOWN UNDERGROUND STRUCTURES, CONDUITS, AND/OR PIPELINES. THE LOCATIONS FOR THESE ELEMENTS ARE BASED UPON THE VARIOUS RECORD DRAWINGS AVAILABLE. THE CONTRACTOR IS HEREBY ADVISED THAT THESE DRAWINGS MAY NOT ACCURATELY DEPICT AS-BUILT LOCATIONS AND OTHER UNKNOWN STRUCTURES. THE CONTRACTOR SHALL THEREFORE DETERMINE THE EXACT LOCATION OF EXISTING UNDERGROUND ELEMENTS AND EXCAVATE WITH CARE AFTER CALLING MARKOUT SERVICE AT 1-800-272-4480 48 HOURS BEFORE DIGGING, DRILLING OR BLASTING.
3. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, FIBER OPTIC, AND OTHER UTILITIES WHERE ENCOUNTERED, SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION, SHALL BE RELOCATED AS DIRECTED BY ENGINEER. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR PIER DRILLING AROUND OR NEAR UTILITIES. CONTRACTOR SHALL HAND DIG UTILITIES AS NEEDED. CONTRACTOR SHALL PROVIDE, BUT IS NOT LIMITED TO, APPROPRIATE A) FALL PROTECTION, B) CONFINED SPACE ENTRY, C) ELECTRICAL SAFETY, AND D) TRENCHING AND EXCAVATION.
4. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES, AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
5. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC, FIBER OPTIC, OR OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED, AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT THE POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF THE CONSTRUCTION MANAGER.
6. CONTRACTOR IS RESPONSIBLE FOR REPAIRING OR REPLACING STRUCTURES OR UTILITIES DAMAGED DURING CONSTRUCTION.
7. CONTRACTOR SHALL PROTECT EXISTING PAVED AND GRAVEL SURFACES, CURBS, LANDSCAPE AND STRUCTURES AND RESTORE SITE OR PRE-CONSTRUCTION CONDITION WITH AS GOOD, OR BETTER, MATERIALS. NEW MATERIALS SHALL MATCH EXISTING THICKNESS AND TYPE.
8. THE CONTRACTOR SHALL SHORE ALL TRENCH EXCAVATIONS GREATER THAN 5 FEET IN DEPTH OR LESS WHERE SOIL CONDITIONS ARE DEEMED UNSTABLE. ALL SHEETING AND/OR SHORING METHODS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER.
9. THE CONTRACTOR IS RESPONSIBLE FOR MANAGING GROUNDWATER LEVELS IN THE VICINITY OF EXCAVATIONS TO PROTECT ADJACENT PROPERTIES AND NEW WORK. GROUNDWATER SHALL BE DRAINED IN ACCORDANCE WITH LOCAL SEDIMENTATION AND EROSION CONTROL GUIDELINES.



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STATE OF CONNECTICUT
JOSHUA J. RILEY
33208
LICENSED PROFESSIONAL ENGINEER

08/17/2021

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RIDGEFIELD 22N
1 PROSPECT STREET
RIDGEFIELD, CT 06877

SHEET TITLE
NOTES
& SPECIFICATIONS

SHEET NUMBER

N-1

ELECTRICAL

- CONTRACTOR SHALL VERIFY EXISTING ELECTRIC SERVICE TYPE AND CAPACITY AND ORDER NEW ELECTRIC SERVICE FROM LOCAL ELECTRIC UTILITY, WHERE APPLICABLE.
- ALL ELECTRICAL WORK SHALL BE IN ACCORDANCE WITH ALL APPLICABLE CODES, AND SHALL BE ACCEPTABLE TO ALL AUTHORITIES HAVING JURISDICTION. WHERE A CONFLICT EXISTS BETWEEN CODES, PLAN AND SPECIFICATIONS, OR AUTHORITIES HAVING JURISDICTION, THE MORE STRINGENT AUTHORITIES SHALL APPLY.
- CONTRACTOR SHALL PROVIDE ALL LABOR, MATERIALS, INSURANCE, EQUIPMENT, INSTALLATION, CONSTRUCTION TOOLS, TRANSPORTATION, ETC, FOR A COMPLETE AND PROPERLY OPERATIVE SYSTEM ENERGIZED THROUGHOUT AND AS INDICATED ON THE DRAWINGS AND AS SPECIFIED HEREIN AND/OR OTHERWISE REQUIRED.
- ALL ELECTRICAL CONDUCTORS SHALL BE 100% COPPER AND SHALL HAVE TYPE THHN INSULATION UNLESS INDICATED OTHERWISE.
- CONDUIT SHALL BE THREADED RIGID GALVANIZED STEEL OR EMT WITH ONLY COMPRESSION TYPE COUPLINGS AND CONNECTORS, ALL MADE UP WRENCH TIGHT.
- ALL BURIED CONDUIT SHALL BE MINIMUM SCH 40 PVC UNLESS NOTED OTHERWISE, OR AS PER LOCAL CODE REQUIREMENTS.
- PROVIDE FLEXIBLE STEEL CONDUIT OR LIQUID TIGHT FLEXIBLE STEEL CONDUIT TO ALL VIBRATING EQUIPMENT, INCLUDING HVAC UNITS, TRANSFORMERS, MOTORS, ETC, OR WHERE EQUIPMENT IS PLACED UPON A SLAB ON GRADE.
- ALL BRANCH CIRCUITS AND FEEDERS SHALL HAVE A SEPARATE GREEN INSULATED EQUIPMENT GROUNDING CONDUCTOR BONDED TO ALL ENCLOSURES, PULLBOXES, ETC.
- CONDUIT AND CABLE WITHIN CORRIDORS SHALL BE CONCEALED AND EXPOSED ELSEWHERE, UNLESS NOTED OTHERWISE.
- ELECTRICAL MATERIALS INSTALLED ON ROOFTOP SHALL BE LISTED FOR NEMA 3R USE. –AND ALL WIRING WITHIN A VENTILATION DUCT SHALL BE LISTED FOR SUCH USE. IN GENERAL WIRING METHODS WITHIN A DUCT SHALL BE AN MC CABLE WITH SMOOTH OR CORRUGATED METAL JACKET AND HAVE NO OUTER COVERING OVER THE METAL JACKET. INTERLOCKED ARMOR TYPE OF MC CABLE IS NOT ACCEPTABLE FOR THIS APPLICATION. CONTRACTOR CAN ALSO USE TYPE MI CABLE IN THE VENTILATION DUCT PROVIDED IT DOES NOT HAVE ANY OUTER COVERINGS OVER THE METAL EXTERIOR.
- WIRING DEVICES SHALL BE SPECIFICATION GRADE, AND WIRING DEVICE COVER PLATES SHALL BE PLASTIC WITH ENGRAVING AS SPECIFIED.
- GROUNDING SYSTEM RESISTANCE SHALL BE MEASURED, RECORDED, AND DATED USING MEGGER DET14 OR SIMILAR INSTRUMENT. GROUND RESISTANCE SHALL NOT EXCEED 5 OHMS. IF THE RESISTANCE VALUE IS EXCEEDED, NOTIFY CONSTRUCTION MANAGER FOR FURTHER INSTRUCTION.
- COORDINATE WITH BUILDING MANAGEMENT BEFORE PERFORMING ANY WORK INVOLVING EXISTING SYSTEMS OR EQUIPMENT IN ORDER TO DETERMINE THE EFFECT, IF ANY, ON OTHER TENANTS WITHIN THE BUILDING, AND TO DETERMINE THE APPROPRIATE TIME FOR PERFORMING THIS WORK.
- THE CONTRACTOR SHALL BE REQUIRED TO VISIT THE SITE PRIOR TO SUBMITTING BID IN ORDER TO DETERMINE THE EXTENT OF THE EXISTING CONDITIONS.
- ALL CONDUCTOR ENDS SHALL BE TAGGED AND ELECTRICAL EQUIPMENT LABELED WITH ENGRAVED IDENTIFICATION PLATES.
- CONTRACTOR IS RESPONSIBLE FOR ALL CONTROL WIRING AND ALARM TIE–INS.

GROUNDING

- #6 THWN SHALL BE STRANDED #6 COPPER WITH GREEN THWN INSULATION SUITABLE FOR WET INSTALLATIONS.
- #2 THWN SHALL BE STRANDED #2 COPPER WITH THWN INSULATION SUITABLE FOR WET INSTALLATIONS.
- ALL LUGS SHALL BE 2–HOLE, LONG BARREL, TINNED SOLID COPPER UNLESS OTHERWISE SPECIFIED, LUGS SHALL BE THOMAS AND BETTS SERIES 548##BE OR EQUIVALENT (IE #2 THWN – 54856BE, #2 SOLID – 54856BE, AND #6 THWN – 54852BE).
- ALL HARDWARE, BOLTS, NUTS, AND WASHERS SHALL BE 18–8 STAINLESS STEEL. EVERY CONNECTION SHALL BE BOLT–FLAT WASHER–BUSS–LUG–FLAT WASHER–BELLEVILLE WASHER–NUT IN THAT EXACT ORDER. BACK–TO–BACK LUGGING, BOLT–FLAT WASHER–LUG–BUSS–LUG–FLAT WASHER–BELLEVILLE WASHER–NUT, IN THAT EXACT ORDER, IS ACCEPTED WHERE NECESSARY TO CONNECT MANY LUGS TO A BUSS BAR. STACKING OF LUGS, BUSS–LUG–LUG, IS NOT ACCEPTABLE.
- WHERE CONNECTIONS ARE MADE TO STEEL OR DISSIMILAR METALS, A THOMAS AND BETTS DRAGON TOOTH WASHER MODEL DTWXXX SHALL BE USED BETWEEN THE LUG AND THE STEEL, BOLT–FLAT WASHER–STEEL–DRAGON TOOTH WASHER–LUG–FLAT WASHER–BELEVILE WASHER–NUT.
- ALL CONNECTIONS, INTERIOR AND EXTERIOR, SHALL BE MADE WITH THOMAS AND BETTS KPOR–SHIELD. COAT ALL WIRES BEFORE LUGGING AND COAT ALL SURFACES BEFORE CONNECTING.
- THE MINIMUM BEND RADIUS SHALL BE 8 INCHES FOR #6 WIRE AND SMALLER AND 12 INCHES FOR WIRE LARGER THAN #6.
- BOND THE FENCE TO THE GROUND RING AT EACH CORNER, AND AT EACH GATE POST WITH #2 SOLID TINNED WIRE. EXOTHERMIC WELD BOTH ENDS.
- GROUND KITS SHALL BE SOLID COPPER STRAP WITH #6 WIRE 2–HOLE COMPRESSION CRIMPED LUGS AND SHALL BE SEALED ACCORDING TO MANUFACTURER INSTRUCTIONS.
- FERROUS METAL CLIPS WHICH COMPLETELY SURROUND THE GROUNDING CONDUCTOR SHALL BE USED.
- GROUND BARS SHALL BE FURNISHED AND INSTALLED WITH PRE–DRILLED HOLE DIAMETERS AND SPACINGS. GROUND BARS SHALL NEITHER BE FIELD FABRICATED NOR NEW HOLES DRILLED. GROUND LUGS SHALL MATCH THE SPACING ON THE BAR. HARDWARE DIAMETER SHALL BE MINIMUM 3.8 INCH.

ANTENNA & CABLE NOTES

- THE CONTRACTOR SHALL FURNISH AND INSTALL ALL TRANSMISSION CABLES, JUMPERS, CONNECTORS, GROUNDING STRAPS, ANTENNAS, MOUNTS AND HARDWARE. ALL MATERIALS SHALL BE INSPECTED BY THE CONTRACTOR FOR DAMAGE UPON DELIVERY. JUMPERS SHALL BE SUPPLIED AT ANTENNAS AND EQUIPMENT INSIDE SHELTER COORDINATE LENGTH OF JUMP CABLES WITH EVERSOURCE. COORDINATE AND VERIFY ALL OF THE MATERIALS TO BE PROVIDED WITH EVERSOURCE PRIOR TO SUBMITTING BID AND ORDERING MATERIALS.
- AFTER INSTALLATION, THE TRANSMISSION LINE SYSTEM SHALL BE PIM/SWEEP TESTED FOR PROPER INSTALLATION AND DAMAGE WITH ANTENNAS CONNECTED. CONTRACTOR TO OBTAIN LATEST TESTING PROCEDURES FROM EVERSOURCE PRIOR TO BIDDING.
- ANTENNA CABLES SHALL BE COLOR CODED AT THE FOLLOWING LOCATIONS:
 - AT THE ANTENNAS.
 - AT THE WAVEGUIDE ENTRY PLATE ON BOTH SIDES OF THE EQUIPMENT SHELTER WALL.
 - JUMPER CABLES AT THE EQUIPMENT ENTER.
- SYSTEM INSTALLATION:
THE CONTRACTOR SHALL INSTALL ALL CABLES AND ANTENNAS TO THE MANUFACTURER’S SPECIFICATIONS. THE CONTRACTOR IS RESPONSIBLE FOR THE PROCUREMENT AND INSTALLATION OF THE FOLLOWING:
 - ALL CONNECTORS, ASSOCIATED CABLE MOUNTING, AND GROUNDING HARDWARE.
 - WALL MOUNTS, STANDOFFS, AND ASSOCIATED HARDWARE.
 - 1/2 INCH HELIAX ANTENNA JUMPERS OF APPROPRIATE LENGTHS.
- MINIMUM BENDING RADIUS FOR COAXIAL CABLES:
 - 7/8 INCH, RMIN = 15 INCHES
 - 1 5/8 INCH, RMIN = 25 INCHES
- CABLE SHALL BE INSTALLED WITH A MINIMUM NUMBER OF BENDS WHERE POSSIBLE. CABLE SHALL NOT BE LEFT UNTERMINATED AND SHALL BE SEALED IMMEDIATELY AFTER BEING INSTALLED.
- ALL CABLE CONNECTIONS OUTSIDE SHALL BE COVERED WITH WATERPROOF SPLICING KIT.
- CONTRACTOR SHALL VERIFY EXACT LENGTH AND DIRECTION OF TRAVEL IN FIELD PRIOR TO CONSTRUCTION.
- CABLE SHALL BE FURNISHED WITHOUT SPLICES AND WITH CONNECTORS AT EACH END.



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| PROJECT NO: | 403093 |
| DRAWN BY: | TCG |
| CHECKED BY: | JR |

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RIDGEFIELD 22N
1 PROSPECT STREET
RIDGEFIELD, CT 06877

SHEET TITLE
NOTES
& SPECIFICATIONS

SHEET NUMBER

N-2

SYMBOLS

| | |
|--------------------------------|---|
| | EXOTHERMIC CONNECTION |
| | COMPRESSION CONNECTION |
| | 5/8"Øx10-0" COPPER CLAD STEEL GROUND ROD. |
| | TEST GROUND ROD WITH INSPECTION SLEEVE |
| | GROUNDING CONDUCTOR |
| | KEY NOTES |
| CHAINLINK FENCE | |
| WOOD FENCE | |
| LEASE AREA | |
| ICE BRIDGE | |
| CABLE TRAY | |
| GAS LINE | |
| UNDERGROUND ELECTRICAL/TELCO | |
| UNDERGROUND ELECTRICAL/CONTROL | |
| UNDERGROUND ELECTRICAL | |
| UNDERGROUND TELCO | |
| PROPERTY LINE (PL) | |

ABBREVIATIONS

| | | | |
|------|-----------------------------------|------|---|
| AC | ALTERNATING CURRENT | MGB | MASTER GROUNDING BAR |
| AIC | AMPERAGE INTERRUPTION CAPACITY | MIN | MINIMUM |
| ANI | AUXILIARY NETWORK INTERFACE | MW | MICROWAVE |
| ATM | ASYNCHRONOUS TRANSFER MODE | MTS | MANUAL TRANSFER SWITCH |
| ATS | AUTOMATIC TRANSFER SWITCH | NEC | NATIONAL ELECTRICAL CODE |
| AWG | AMERICAN WIRE GAUGE | OC | ON CENTER |
| AWS | ADVANCED WIRELESS SERVICES | PP | POLARIZING PRESERVING |
| BATT | BATTERY | PCU | PRIMARY CONTROL UNIT |
| BBU | BASEBAND UNIT | PDU | PROTOCOL DATA UNIT |
| BTC | BARE TINNED COPPER CONDUCTOR | PWR | POWER |
| BTS | BASE TRANSCEIVER STATION | RECT | RECTIFIER |
| CCU | CLIMATE CONTROL UNIT | RET | REMOTE ELECTRICAL TILT |
| CDMA | CODE DIVISION MULTIPLE ACCESS | RMC | RIGID METALLIC CONDUIT |
| CHG | CHARGING | RF | RADIO FREQUENCY |
| CLU | CLIMATE UNIT | RUC | RACK USER COMMISSIONING |
| COMM | COMMON | RRH | REMOTE RADIO HEAD |
| DC | DIRECT CURRENT | RRU | REMOTE RADIO UNIT |
| DIA | DIAMETER | RWY | RACEWAY |
| DWG | DRAWING | SFP | SMALL FORM-FACTOR PLUGGABLE |
| EC | ELECTRICAL CONDUCTOR | SIAD | SMART INTEGRATED ACCESS DEVICE |
| EMT | ELECTRICAL METALLIC TUBING | SSC | SITE SOLUTIONS CABINET |
| FIF | FACILITY INTERFACE FRAME | T1 | 1544KBPS DIGITAL LINE |
| GEN | GENERATOR | TDMA | TIME-DIVISION MULTIPLE ACCESS |
| GPS | GLOBAL POSITIONING SYSTEM | TMA | TOWER MOUNT AMPLIFIER |
| GSM | GLOBAL SYSTEM FOR MOBILE | TVSS | TRANSIENT VOLTAGE SUPPRESSION SYSTEM |
| HVAC | HEAT/VENTILATION/AIR CONDITIONING | TYP | TYPICAL |
| ICF | INTERCONNECTION FRAME | UMTS | UNIVERSAL MOBILE TELECOMMUNICATION SYSTEM |
| IGR | INTERIOR GROUNDING RING (HALO) | UPS | UNINTERRUPTIBLE POWER SUPPLY (DC POWER PLANT) |
| LTE | LONG TERM EVOLUTION | | |

EVERSOURCE
ENERGY

107 SELDEN STREET
BERLIN, CT 06037
PHONE: (800) 286-2000

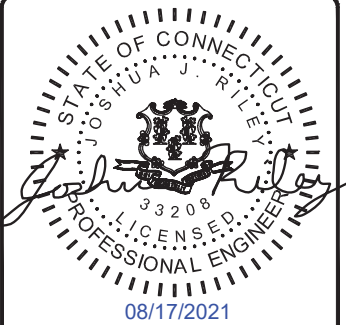


BLACK & VEATCH

6800 W 115TH ST, SUITE 2292
OVERLAND PARK, KS 66211
PHONE: (913) 458-3595

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| PROJECT NO: | 403093 |
| DRAWN BY: | TCG |
| CHECKED BY: | JR |

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RIDGEFIELD 22N
1 PROSPECT STREET
RIDGEFIELD, CT 06877

SHEET TITLE
NOTES
& SPECIFICATIONS

SHEET NUMBER

N-3

REPLACEMENT ANTENNA

220 MHz Antenna – Omnidirectional, Low-PIM/Hi-PIP, Unity Gain Models - SP2D00P36D-D

| Specifications | |
|---|---|
| Design Type | True Corporate Feed |
| Frequency Range | 217-220 MHz |
| Passive Intermodulation – PIM (2 x 20W sources) | -150 dBc, 3 rd Order |
| Bandwidth | 3 MHz |
| Gain - dBd (average over BW) | 0 dBd |
| Isolation, min. | 40 dB |
| Configuration | Dual antenna |
| Beam Tilt (electrical down-tilt) | None (0°) |
| Vertical Beamwidth (E-Plane) | 60° |
| Impedance -- Ohms | 50 |
| VSWR / Return Loss -- dB | 1.5 : 1 / 14 dB (min.) |
| Average Power Rating | 500 W (each antenna) |
| Peak Instantaneous Power | 25 kW (each antenna) |
| Polarization | Vertical |
| Lightning Protection | Direct Ground |
| Connector | 7/16 DIN female |
| Equivalent Flat-Plate Area | 2.59 sq. ft. |
| Lateral Wind-load Thrust @100mph | 109 lbf. |
| Wind Speed rating | 160 mph (without ice) 136 mph (½" radial ice) |
| Total Length | 15.6 feet |
| Mounting Mast Length | 35 inches |
| Mounting Hardware (Included) | DSH3V4N |
| Top Sway Brace (Recommended if side mounting antennas) | DSH2H3S (order separately) |
| Mast O.D. | 3.5 inches |
| Radome color | Horizon Blue |
| Radome O.D. | 3.0 inches |
| Weight, antenna, and hardware | 45 lbs. (approx.) |
| Shipping Weight | 80 lbs. (approx.) |
| Invertibility | Antennas are physically invertible, but the patterns are optimized for upright mount. |



Features and Benefits

Antennas from dbSpectra provide long term, trouble-free service in severe environments!

Design is tested to stringent Peak Instantaneous Power (PIP) levels of 25 KW using dbSpectra's 12-channel P25 PIP test bed. High PIP level is demanded by today's digital systems.

True Corporate Feed Array – provides for excellent gain and pattern consistency across a wider frequency range.

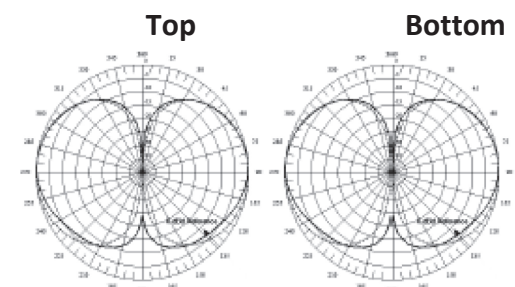
PIM Rated Design – better than -150 dBc.

Sturdy Construction – Heavy-wall fiberglass radome minimizes tip deflection.

Excellent Lightning Protection – heavy internal conductor DC ground.

Radiation Pattern

Vertical (No-Tilt)



REMOVED AND REPLACED

ANT220F6 DIN

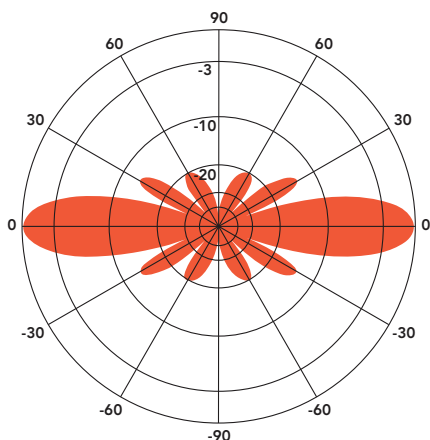
FIBERGLASS COLLINEAR ANTENNA 6 dBd

The Telewave ANT220F6 is an extremely rugged, medium-gain, fiberglass collinear antenna, designed for operation in all environmental conditions. The antenna is constructed with brass and copper elements, connected at DC ground potential for lightning impulse protection. The ANT220F6 is an excellent choice for wireless PTC systems in urban or rural areas.

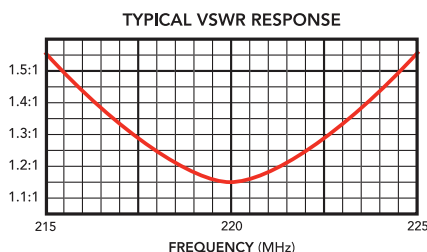
All junctions are fully soldered to prevent RF intermodulation, and each antenna is completely protected within a rugged, high-tech radome to ensure survivability in the worst environments. The "Cool Blue" radome provides maximum protection from corrosive gases, ultraviolet radiation, icing, salt spray, acid rain, and wind blown abrasives.

The ANT220F6 includes an ANTC482 dual clamp set for mounting to a 1.5" to 3.5" O.D. support pipe, and a 24" removable RG-213 DIN-Male jumper. Stand-off and top mounts are also available.

NOTE: THIS ANTENNA IS SHIPPED VIA TRUCK FREIGHT ONLY



ANT220F6 - 221 MHz
Vertical Plane
Gain = 6.11 dBd



SPECIFICATIONS

| | | | |
|------------------------|-----------------|-------------------------------------|----------------------|
| Frequency (continuous) | 216-225 MHz | Dimensions (L x base diam.) in. | 171 x 2.75 |
| Gain | 6 dBd | Tower weight (antenna + clamps) | 35 lb. |
| Power rating (typ.) | 500 watts | Shipping weight | 50 lb. |
| Impedance | 50 ohms | Wind rating / with 0.5" ice | 150 / 125 MPH |
| VSWR | 1.5:1 or less | Maximum exposed area | 3.1 ft. ² |
| Pattern | Omnidirectional | Lateral thrust at 100 MPH | 122 lb. |
| Vertical beamwidth | 20° | Bending moment at top clamp | 494 ft. lb. |
| Termination | 7-16 DIN-F | (100 MPH, 40 PSF flat plate equiv.) | |

ANT220F2-I w/DIN CONNECTOR to be used for the inverted antenna.

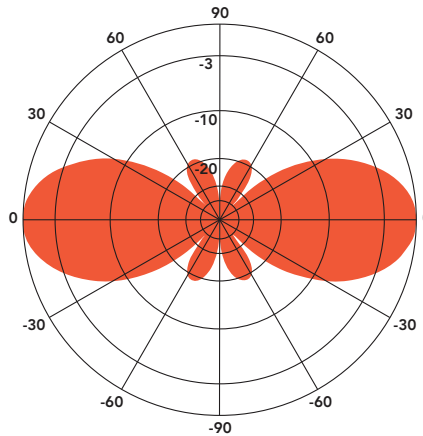
ANT220F2

FIBERGLASS COLLINEAR ANTENNA 2.5 dBd

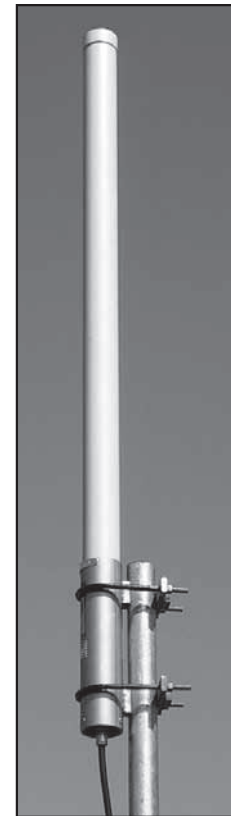
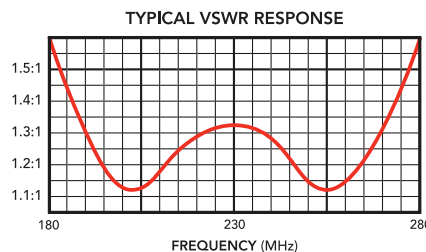
The Telewave ANT220F2 is an extremely rugged collinear antenna, with moderate gain and wide vertical beamwidth. This compact antenna produces 2.5 dBd gain, and is designed for operation in all environmental conditions. The antenna is constructed with brass and copper elements, with a path to ground potential for lightning impulse protection. The ANT220F2 is an excellent choice for wireless PTC systems in urban or rural areas.

All junctions are fully soldered to prevent RF intermodulation, and each antenna is completely protected within a rugged, high-tech radome to ensure survivability in the worst environments. The "Cool Blue" radome provides maximum protection from corrosive gases, ultraviolet radiation, icing, salt spray, acid rain, and wind blown abrasives.

The ANT220F2 includes the ANTC485 dual clamp set for mounting to a 1.5" to 3" O.D. support pipe, and a 24" removable RG-213 DIN-Male jumper.



ANT220F2 - 230 MHz
Vertical Plane
Gain = 2.58 dBd



SPECIFICATIONS

| | | | |
|------------------------|--------------------------------------|-------------------------------------|----------------------|
| Frequency (continuous) | 195-260 MHz | Dimensions (L x base diam.) in. | 51 x 2.75 |
| Gain | 2.5 dBd | Tower weight (antenna + clamps) | 11 lb. |
| Power rating (typ.) | 500 watts | Shipping weight | 14 lb. |
| Impedance | 50 ohms | Wind rating / with 0.5" ice | 200 / 150 MPH |
| VSWR | 1.5:1 or less | Maximum exposed area | 1.1 ft. ² |
| Pattern | Omnidirectional | Lateral thrust at 100 MPH | 44 lb. |
| Vertical beamwidth | 38° | Bending moment at top clamp | 47 ft. lb. |
| Termination | Recessed N Female 7-16 DIN-F opt. | (100 MPH, 40 PSF flat plate equiv.) | |

ATTACHMENT D – STRUCTURAL ANALYSIS REPORT

Date: July 29, 2021



Black & Veatch Corp.
6800 W. 115th St., Suite 2292
Overland Park, KS 66211
(913) 458-2522

Subject: Structural Analysis Report

Eversource Designation: **Site Number:** ES-286
Site Name: Ridgefield22N

Engineering Firm Designation: **Black & Veatch Corp. Project Number:** 405025

Site Data: **Off Prospect Street, Ridgefield, Fairfield County, CT**
Latitude 41° 17' 0.59", Longitude -73° 29' 16.27"
84 Foot - Monopole Tower

Black & Veatch Corp. is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC1: Proposed Equipment Configuration

Sufficient Capacity – 53.4%

This analysis utilizes an ultimate 3-second gust wind speed of 125 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Anthony Reyes / Joshua J. Riley

Respectfully submitted by:

Joshua J. Riley, P.E.
Professional Engineer

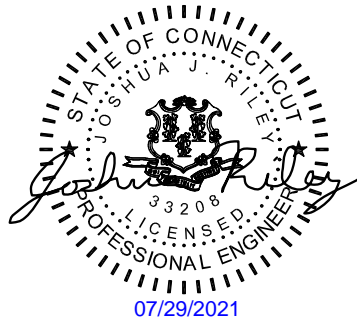


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tnxTower Output

6) APPENDIX B

Base Level Drawing

7) APPENDIX C

Additional Calculations

1) INTRODUCTION

This tower is an 84 ft Monopole tower designed by Valmont in July of 2012.

2) ANALYSIS CRITERIA

| | |
|-------------------------------|------------------|
| TIA-222 Revision: | TIA-222-H |
| Risk Category: | III |
| Wind Speed: | 125 mph ultimate |
| Exposure Category: | C |
| Topographic Factor: | 1 |
| Ice Thickness: | 1.5 in |
| Wind Speed with Ice: | 50 mph |
| Seismic S_s: | 0.229 |
| Seismic S₁: | 0.068 |
| Service Wind Speed: | 60 mph |

Table 1 - Proposed Equipment Configuration

| Mounting Level (ft) | Center Line Elevation (ft) | Number of Antennas | Antenna Manufacturer | Antenna Model | Number of Feed Lines | Feed Line Size (in) | Note |
|---------------------|----------------------------|--------------------|----------------------|------------------|----------------------|---------------------|------|
| 83.0 | 90.67 | 1 | dbspecta | SP2D00P36D-D | 2 | 7/8 | 1 |
| | 83.0 | 1 | generic | 4'x3" Mount Pipe | | | |

Note:

- 1) Proposed equipment to be installed on existing relocated antenna's original antenna mount at 83.0ft Mounting Level.

Table 2 - Other Considered Equipment

| Mounting Level (ft) | Center Line Elevation (ft) | Number of Antennas | Antenna Manufacturer | Antenna Model | Number of Feed Lines | Feed Line Size (in) | Note |
|---------------------|----------------------------|--------------------|----------------------|------------------------------|----------------------|---------------------|------|
| 83.0 | 90.0 | 1 | kreco | CO-41A | 2 | 7/8 | 1 |
| | 88.0 | 1 | commscope | DB589-Y | | | |
| | 83.0 | 1 | tower mounts | Side Arm Mount [4' SO 701-3] | | | |
| 67.0 | 74.0 | 1 | celwave | 1151-3 | 1 | 7/8 | 2 |
| | | 1 | kreco | CO-41A | 2 | 7/8 | 1 |
| | 73.0 | 1 | kreco | CO-41A | | | |
| | 67.0 | 1 | tower mounts | Side Arm Mount [6' SO 701-3] | | | |

Note:

- 1) Existing equipment
2) Existing equipment to be relocated from 83.0ft Mounting Level to empty antenna mount on 67.0ft Mounting Level.

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

| Document | Remarks | Reference | Source |
|--|--|-----------|------------|
| GEOTECHNICAL REPORTS | Dr. Clarence Welti, P.E., P.C., dated 06/14/2012 | - | Eversource |
| TOWER FOUNDATION DRAWINGS/DESIGN/SPECS | Valmont, dated 7/27/2012 | - | Eversource |

| Document | Remarks | Reference | Source |
|-----------------------------|--------------------------|-----------|------------|
| TOWER MANUFACTURER DRAWINGS | Valmont, dated 7/27/2012 | - | Eversource |

3.1) Analysis Method

tnxTower (version 8.1.1.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) Tower and structures were built and maintained in accordance with the manufacturer's specifications.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 3) This analysis was performed under the assumption that all information provided to Black & Veatch is current and correct. This is to include site data, appurtenance loading, tower/foundation details, and geotechnical data.

This analysis may be affected if any assumptions are not valid or have been made in error. Black & Veatch Corp. should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 5 - Tower Component Stresses vs. Capacity - LC1

| Section No. | Elevation (ft) | Component Type | Size | Critical Element | P (K) | SF*P_allow (K) | % Capacity | Pass / Fail |
|-------------|----------------|----------------|------------------------|------------------|-------|----------------|------------|-------------|
| L1 | 84 - 34.25 | Pole | TP18.145x12.001x0.1875 | 1 | -2.07 | 639.52 | 37.8 | Pass |
| L2 | 34.25 - 0 | Pole | TP22x17.3069x0.2188 | 2 | -4.62 | 928.93 | 53.4 | Pass |
| | | | | | | | Summary | |
| | | | | | | Pole (L2) | 53.4 | Pass |
| | | | | | | Rating = | 53.4 | Pass |

Table 4 - Tower Component Stresses vs. Capacity - LC1

| Notes | Component | Elevation (ft) | % Capacity | Pass / Fail |
|-------|----------------------------------|----------------|------------|-------------|
| 1 | Anchor Rods | 0 | 52.1 | Pass |
| | Base Plate | | 22.9 | Pass |
| 1 | Base Foundation | 0 | 34.7 | Pass |
| | Base Foundation Soil Interaction | | 47.6 | Pass |

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity. Rating per TIA-222-H Section 15.5.

| | |
|---|--------------|
| Structure Rating (max from all components) = | 53.4% |
|---|--------------|

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

Maximum Tower Deflections - Service Wind

| <i>Section No.</i> | <i>Elevation ft</i> | <i>Horz. Deflection in</i> | <i>Gov. Load Comb.</i> | <i>Tilt °</i> | <i>Twist °</i> | <i>Check*</i> |
|------------------------|------------------------------|---|-------------------------------------|------------------------|-------------------------|---------------|
| L1 | 84 - 34.25 | 11.155 | 44 | 1.0486 | 0.0208 | OK |
| L2 | 38 - 0 | 2.598 | 44 | 0.6092 | 0.0051 | OK |

*Limit State Deformation (TIA-222-H Section 2.8.2)

1) Maximum Rotation = 4 Degrees

2) Maximum Deflection = 0.03 * Tower Height = 30 in.

Maximum Tower Deflections - Design Wind

| <i>Section No.</i> | <i>Elevation ft</i> | <i>Horz. Deflection in</i> | <i>Gov. Load Comb.</i> | <i>Tilt °</i> | <i>Twist °</i> | <i>Combined Max</i> | <i>Check*</i> |
|------------------------|------------------------------|---|-------------------------------------|------------------------|-------------------------|-------------------------|---------------|
| L1 | 84 - 34.25 | 28.84 | 44 | 2.6937 | 0.0543 | 2.694 | OK** |
| L2 | 38 - 0 | 6.746 | 44 | 1.5807 | 0.0133 | 1.581 | OK** |

*Up to 0.5 degree is considered acceptable per SUB090 Section 7

** Deflection approved by Eversource Energy

APPENDIX A

TNXTOWER OUTPUT

| MATERIAL STRENGTH | | | | | |
|-------------------|--------|--------|-------|----|----|
| GRADE | Fy | Fu | GRADE | Fy | Fu |
| A572-65 | 65 ksi | 80 ksi | | | |

1. Tower is located in Fairfield County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-H Standard.
3. Tower designed for a 125 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category III.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 53.4%

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- Tower is located in Fairfield County, Connecticut.
- Tower base elevation above sea level: 666.00 ft.
- Basic wind speed of 125 mph.
- Risk Category III.
- Exposure Category C.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0.00 ft.
- Nominal ice thickness of 1.5000 in.
- Ice thickness is considered to increase with height.
- Ice density of 56 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 60 mph.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.
- Tower analysis based on target reliabilities in accordance with Annex S.
- Load Modification Factors used: $K_{es}(F_w) = 1.0$, $K_{es}(t_i) = 1.0$.
- Maximum demand-capacity ratio is: 1.05.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

| | | |
|-------------------------------------|-------------------------------------|------------------------------------|
| Consider Moments - Legs | Distribute Leg Loads As Uniform | Use ASCE 10 X-Brace Ly Rules |
| Consider Moments - Horizontals | Assume Legs Pinned | Calculate Redundant Bracing Forces |
| Consider Moments - Diagonals | ✓ Assume Rigid Index Plate | Ignore Redundant Members in FEA |
| Use Moment Magnification | ✓ Use Clear Spans For Wind Area | SR Leg Bolts Resist Compression |
| ✓ Use Code Stress Ratios | Use Clear Spans For KL/r | All Leg Panels Have Same Allowable |
| ✓ Use Code Safety Factors - Guys | Retention Guys To Initial Tension | Offset Girt At Foundation |
| Escalate Ice | ✓ Bypass Mast Stability Checks | ✓ Consider Feed Line Torque |
| Always Use Max Kz | ✓ Use Azimuth Dish Coefficients | Include Angle Block Shear Check |
| Use Special Wind Profile | ✓ Project Wind Area of Appurt. | Use TIA-222-H Bracing Resist. |
| Include Bolts In Member Capacity | Autocalc Torque Arm Areas | Exemption |
| Leg Bolts Are At Top Of Section | Add IBC .6D+W Combination | Use TIA-222-H Tension Splice |
| Secondary Horizontal Braces Leg | Sort Capacity Reports By Component | Exemption |
| Use Diamond Inner Bracing (4 Sided) | Triangulate Diamond Inner Bracing | |
| SR Members Have Cut Ends | Treat Feed Line Bundles As Cylinder | |
| SR Members Are Concentric | Ignore KL/ry For 60 Deg. Angle Legs | |

Poles

- ✓ Include Shear-Torsion Interaction
- Always Use Sub-Critical Flow
- Use Top Mounted Sockets
- Pole Without Linear Attachments
- Pole With Shroud Or No
- Appurtenances
- Outside and Inside Corner Radii Are Known

Tapered Pole Section Geometry

| Section | Elevation ft | Section Length ft | Splice Length ft | Number of Sides | Top Diameter in | Bottom Diameter in | Wall Thickness in | Bend Radius in | Pole Grade |
|---------|-----------------|-------------------------|------------------------|-----------------------|-----------------------|--------------------------|-------------------------|----------------------|---------------------|
| L1 | 84.00-34.25 | 49.75 | 3.75 | 18 | 12.0010 | 18.1450 | 0.1875 | 0.7500 | A572-65 (65 ksi) |
| L2 | 34.25-0.00 | 38.00 | | 18 | 17.3069 | 22.0000 | 0.2188 | 0.8750 | A572-65 (65 ksi) |

Tapered Pole Properties

| Section | Tip Dia. in | Area in ² | I in ⁴ | r in | C in | I/C in ³ | J in ⁴ | It/Q in ² | w in | w/t |
|---------|----------------|-------------------------|----------------------|---------|---------|------------------------|----------------------|-------------------------|---------|--------|
| L1 | 12.1572 | 7.0305 | 123.9600 | 4.1938 | 6.0965 | 20.3329 | 248.0830 | 3.5159 | 1.7822 | 9.505 |
| | 18.3960 | 10.6870 | 435.3948 | 6.3749 | 9.2177 | 47.2349 | 871.3626 | 5.3445 | 2.8635 | 15.272 |
| L2 | 18.0104 | 11.8645 | 437.6998 | 6.0663 | 8.7919 | 49.7845 | 875.9756 | 5.9334 | 2.6610 | 12.165 |
| | 22.3056 | 15.1230 | 906.4437 | 7.7323 | 11.1760 | 81.1063 | 1814.0801 | 7.5629 | 3.4870 | 15.941 |

| Tower Elevation ft | Gusset Area (per face) ft ² | Gusset Thickness in | Gusset Grade | Adjust. Factor A _r | Adjust. Factor A _r | Weight Mult. | Double Angle Stitch Bolt Spacing Diagonals in | Double Angle Stitch Bolt Spacing Horizontal in | Double Angle Stitch Bolt Spacing Redundants in |
|--------------------------|---|---------------------------|--------------|----------------------------------|-------------------------------------|--------------|---|--|--|
| L1 84.00- 34.25 | | | | 1 | 1 | 1 | | | |
| L2 34.25-0.00 | | | | 1 | 1 | 1 | | | |

Feed Line/Linear Appurtenances - Entered As Round Or Flat

| Description | Sector | Exclude From Torque Calculation | Component Type | Placement ft | Total Number | Number Per Row | Start/End Position | Width or Diameter in | Perimeter in | Weight plf |
|-------------------------------|--------|--|----------------------|------------------|-----------------|-------------------|-----------------------|----------------------------|-----------------|---------------|
| ***misc*** Safety Line 3/8 | C | No | Surface Ar (CaAa) | 84.00 - 10.00 | 1 | 1 | 0.000 0.010 | 0.3750 | | 0.22 |

Feed Line/Linear Appurtenances - Entered As Area

| Description | Face or Leg | Allow Shield | Exclude From Torque Calculation | Component Type | Placement ft | Total Number | | C _A A _A ft²/ft | Weight plf |
|-----------------------------|-------------------|-----------------|--|-------------------|-----------------|-----------------|--|---|------------------------------|
| ***83*** LDF5-50A(7/8) | C | No | No | Inside Pole | 83.00 - 0.00 | 2 | No Ice 1/2" Ice 1" Ice 2" Ice | 0.00 0.00 0.00 0.00 | 0.33 0.33 0.33 0.33 |
| ***66*** LDF5-50A(7/8) | C | No | No | Inside Pole | 67.00 - 0.00 | 3 | No Ice 1/2" Ice 1" Ice 2" Ice | 0.00 0.00 0.00 0.00 | 0.33 0.33 0.33 0.33 |
| *Proposed* LDF5-50A(7/8) | A | No | No | Inside Pole | 83.00 - 0.00 | 2 | No Ice 1/2" Ice 1" Ice 2" Ice | 0.00 0.00 0.00 0.00 | 0.33 0.33 0.33 0.33 |

Feed Line/Linear Appurtenances Section Areas

| Tower Section n | Tower Elevation ft | Face | A_R ft ² | A_F ft ² | $C_{AA}A_A$ In Face ft ² | $C_{AA}A_A$ Out Face ft ² | Weight K |
|-----------------------|--------------------------|------|--------------------------|--------------------------|---|--|-------------|
| L1 | 84.00-34.25 | A | 0.000 | 0.000 | 0.000 | 0.000 | 0.03 |
| | | B | 0.000 | 0.000 | 0.000 | 0.000 | 0.00 |
| | | C | 0.000 | 0.000 | 1.866 | 0.000 | 0.08 |
| L2 | 34.25-0.00 | A | 0.000 | 0.000 | 0.000 | 0.000 | 0.02 |
| | | B | 0.000 | 0.000 | 0.000 | 0.000 | 0.00 |
| | | C | 0.000 | 0.000 | 0.909 | 0.000 | 0.06 |

Feed Line/Linear Appurtenances Section Areas - With Ice

| Tower Section n | Tower Elevation ft | Face or Leg | Ice Thickness in | A_R ft ² | A_F ft ² | $C_{AA}A_A$ In Face ft ² | $C_{AA}A_A$ Out Face ft ² | Weight K |
|-----------------------|--------------------------|-------------------|------------------------|--------------------------|--------------------------|---|--|-------------|
| L1 | 84.00-34.25 | A | 1.825 | 0.000 | 0.000 | 0.000 | 0.000 | 0.03 |
| | | B | | 0.000 | 0.000 | 0.000 | 0.000 | 0.00 |
| | | C | | 0.000 | 0.000 | 20.025 | 0.000 | 0.32 |
| L2 | 34.25-0.00 | A | 1.614 | 0.000 | 0.000 | 0.000 | 0.000 | 0.02 |
| | | B | | 0.000 | 0.000 | 0.000 | 0.000 | 0.00 |
| | | C | | 0.000 | 0.000 | 9.761 | 0.000 | 0.18 |

Feed Line Center of Pressure

| Section | Elevation ft | CP_x in | CP_z in | CP_x Ice in | CP_z Ice in |
|---------|-----------------|--------------|--------------|---------------------|---------------------|
| L1 | 84.00-34.25 | -0.0031 | 0.2996 | -0.0142 | 1.3545 |
| L2 | 34.25-0.00 | -0.0022 | 0.2081 | -0.0112 | 1.0668 |

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

| Tower Section | Feed Line Record No. | Description | Feed Line Segment Elev. | K_a No Ice | K_a Ice |
|------------------|-------------------------|-----------------|-------------------------------|-----------------|--------------|
| L1 | 2 | Safety Line 3/8 | 34.25 - 84.00 | 1.0000 | 1.0000 |
| L2 | 2 | Safety Line 3/8 | 10.00 - 34.25 | 1.0000 | 1.0000 |

Discrete Tower Loads

| Description | Face or Leg | Offset Type | Offsets: Horz Lateral Vert ft ft ft | Azimuth Adjustmen t ° | Placement ft | | C _A A _A Front ft ² | C _A A _A Side ft ² | Weight K |
|---------------------------------|-------------------|----------------|---|--------------------------------|-----------------|--------|---|--|-------------|
| ***83*** | | | | | | | | | |
| Side Arm Mount [4' SO 701-1] | C | None | | 0.0000 | 83.00 | No Ice | 1.13 | 2.23 | 0.09 |
| | | | | | | 1/2" | 1.52 | 3.12 | 0.11 |
| | | | | | | Ice | 1.91 | 4.01 | 0.12 |
| | | | | | | 1" Ice | 2.68 | 5.80 | 0.16 |
| | | | | | | 2" Ice | | | |
| 3' x 2" Pipe Mount | A | From Face | 6.00 | 0.0000 | 83.00 | No Ice | 0.58 | 0.58 | 0.01 |
| | | | 0.00 | | | 1/2" | 0.77 | 0.77 | 0.02 |
| | | | 0.00 | | | Ice | 0.97 | 0.97 | 0.02 |
| | | | | | | 1" Ice | 1.39 | 1.39 | 0.05 |
| | | | | | | 2" Ice | | | |
| 3' x 2" Pipe Mount | B | From Face | 6.00 | 0.0000 | 83.00 | No Ice | 0.58 | 0.58 | 0.01 |
| | | | 0.00 | | | 1/2" | 0.77 | 0.77 | 0.02 |
| | | | 0.00 | | | Ice | 0.97 | 0.97 | 0.02 |
| | | | | | | 1" Ice | 1.39 | 1.39 | 0.05 |
| | | | | | | 2" Ice | | | |
| 3' x 2" Pipe Mount | C | From Face | 6.00 | 0.0000 | 83.00 | No Ice | 0.58 | 0.58 | 0.01 |
| | | | 0.00 | | | 1/2" | 0.77 | 0.77 | 0.02 |
| | | | 0.00 | | | Ice | 0.97 | 0.97 | 0.02 |
| | | | | | | 1" Ice | 1.39 | 1.39 | 0.05 |
| | | | | | | 2" Ice | | | |
| DB589-Y | C | From Face | 6.00 | 0.0000 | 83.00 | No Ice | 1.38 | 1.38 | 0.01 |
| | | | 0.00 | | | 1/2" | 2.31 | 2.31 | 0.02 |
| | | | 5.00 | | | Ice | 3.27 | 3.27 | 0.04 |
| | | | | | | 1" Ice | 4.81 | 4.81 | 0.09 |
| | | | | | | 2" Ice | | | |
| CO-41A | A | From Face | 6.00 | 0.0000 | 83.00 | No Ice | 3.15 | 3.15 | 0.01 |
| | | | 0.00 | | | 1/2" | 4.38 | 4.38 | 0.04 |
| | | | 6.00 | | | Ice | 5.63 | 5.63 | 0.07 |
| | | | | | | 1" Ice | 7.77 | 7.77 | 0.15 |
| | | | | | | 2" Ice | | | |
| ***Relocated to 67*** | | | | | | | | | |
| 1151-3 | C | From Face | 6.00 | 0.0000 | 67.00 | No Ice | 4.18 | 4.18 | 0.02 |
| | | | 0.00 | | | 1/2" | 5.73 | 5.73 | 0.05 |
| | | | 7.00 | | | Ice | 7.30 | 7.30 | 0.09 |
| | | | | | | 1" Ice | 10.48 | 10.48 | 0.20 |
| | | | | | | 2" Ice | | | |
| ***67*** | | | | | | | | | |
| Side Arm Mount [6' SO 701-1] | C | None | | 0.0000 | 67.00 | No Ice | 1.70 | 3.34 | 0.13 |
| | | | | | | 1/2" | 2.28 | 4.68 | 0.16 |
| | | | | | | Ice | 2.86 | 6.02 | 0.19 |
| | | | | | | 1" Ice | 4.02 | 8.70 | 0.24 |
| | | | | | | 2" Ice | | | |
| 3' x 2" Pipe Mount | A | From Face | 6.00 | 0.0000 | 67.00 | No Ice | 0.58 | 0.58 | 0.01 |
| | | | 0.00 | | | 1/2" | 0.77 | 0.77 | 0.02 |
| | | | 0.00 | | | Ice | 0.97 | 0.97 | 0.02 |
| | | | | | | 1" Ice | 1.39 | 1.39 | 0.05 |
| | | | | | | 2" Ice | | | |
| 3' x 2" Pipe Mount | B | From Face | 6.00 | 0.0000 | 67.00 | No Ice | 0.58 | 0.58 | 0.01 |
| | | | 0.00 | | | 1/2" | 0.77 | 0.77 | 0.02 |
| | | | 0.00 | | | Ice | 0.97 | 0.97 | 0.02 |
| | | | | | | 1" Ice | 1.39 | 1.39 | 0.05 |
| | | | | | | 2" Ice | | | |
| 3' x 2" Pipe Mount | C | From Face | 6.00 | 0.0000 | 67.00 | No Ice | 0.58 | 0.58 | 0.01 |
| | | | 0.00 | | | 1/2" | 0.77 | 0.77 | 0.02 |
| | | | 0.00 | | | Ice | 0.97 | 0.97 | 0.02 |
| | | | | | | 1" Ice | 1.39 | 1.39 | 0.05 |
| | | | | | | 2" Ice | | | |
| CO-41A | A | From Face | 6.00 | 0.0000 | 67.00 | No Ice | 3.15 | 3.15 | 0.01 |
| | | | 0.00 | | | 1/2" | 4.38 | 4.38 | 0.04 |
| | | | 7.00 | | | Ice | 5.63 | 5.63 | 0.07 |
| | | | | | | 1" Ice | 7.77 | 7.77 | 0.15 |
| | | | | | | 2" Ice | | | |
| CO-41A | B | From Face | 6.00 | 0.0000 | 67.00 | No Ice | 3.15 | 3.15 | 0.01 |
| | | | 0.00 | | | 1/2" | 4.38 | 4.38 | 0.04 |

| Description | Face or Leg | Offset Type | Offsets: Horz Lateral Vert ft ft ft | Azimuth Adjustmen t ° | Placement ft | | C _A A _A Front ft ² | C _A A _A Side ft ² | Weight K |
|------------------|-------------------|----------------|---|--------------------------------|-----------------|--------|---|--|-------------|
| | | | 6.00 | | | Ice | 5.63 | 5.63 | 0.07 |
| | | | | | | 1" Ice | 7.77 | 7.77 | 0.15 |
| | | | | | | 2" Ice | | | |
| *Proposed* | | | | | | | | | |
| SP2D00P36D-D | B | From Leg | 4.50 | 0.0000 | 83.00 | No Ice | 5.36 | 5.36 | 0.05 |
| | | | 0.00 | | | 1/2" | 7.06 | 7.06 | 0.08 |
| | | | 7.67 | | | Ice | 8.67 | 8.67 | 0.13 |
| | | | | | | 1" Ice | 11.95 | 11.95 | 0.26 |
| | | | | | | 2" Ice | | | |
| 4'x3" Mount Pipe | B | From Leg | 4.00 | 0.0000 | 83.00 | No Ice | 1.09 | 1.09 | 0.03 |
| | | | 0.00 | | | 1/2" | 1.36 | 1.36 | 0.04 |
| | | | 1.00 | | | Ice | 1.62 | 1.62 | 0.05 |
| | | | | | | 1" Ice | 2.16 | 2.16 | 0.09 |
| | | | | | | 2" Ice | | | |
| *** | | | | | | | | | |
| *** | | | | | | | | | |

Load Combinations

| Comb. No. | Description |
|--------------|--|
| 1 | Dead Only |
| 2 | 1.2 Dead+1.0 Wind 0 deg - No Ice |
| 3 | 0.9 Dead+1.0 Wind 0 deg - No Ice |
| 4 | 1.2 Dead+1.0 Wind 30 deg - No Ice |
| 5 | 0.9 Dead+1.0 Wind 30 deg - No Ice |
| 6 | 1.2 Dead+1.0 Wind 60 deg - No Ice |
| 7 | 0.9 Dead+1.0 Wind 60 deg - No Ice |
| 8 | 1.2 Dead+1.0 Wind 90 deg - No Ice |
| 9 | 0.9 Dead+1.0 Wind 90 deg - No Ice |
| 10 | 1.2 Dead+1.0 Wind 120 deg - No Ice |
| 11 | 0.9 Dead+1.0 Wind 120 deg - No Ice |
| 12 | 1.2 Dead+1.0 Wind 150 deg - No Ice |
| 13 | 0.9 Dead+1.0 Wind 150 deg - No Ice |
| 14 | 1.2 Dead+1.0 Wind 180 deg - No Ice |
| 15 | 0.9 Dead+1.0 Wind 180 deg - No Ice |
| 16 | 1.2 Dead+1.0 Wind 210 deg - No Ice |
| 17 | 0.9 Dead+1.0 Wind 210 deg - No Ice |
| 18 | 1.2 Dead+1.0 Wind 240 deg - No Ice |
| 19 | 0.9 Dead+1.0 Wind 240 deg - No Ice |
| 20 | 1.2 Dead+1.0 Wind 270 deg - No Ice |
| 21 | 0.9 Dead+1.0 Wind 270 deg - No Ice |
| 22 | 1.2 Dead+1.0 Wind 300 deg - No Ice |
| 23 | 0.9 Dead+1.0 Wind 300 deg - No Ice |
| 24 | 1.2 Dead+1.0 Wind 330 deg - No Ice |
| 25 | 0.9 Dead+1.0 Wind 330 deg - No Ice |
| 26 | 1.2 Dead+1.0 Ice+1.0 Temp |
| 27 | 1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp |
| 28 | 1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp |
| 29 | 1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp |
| 30 | 1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp |
| 31 | 1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp |
| 32 | 1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp |
| 33 | 1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp |
| 34 | 1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp |
| 35 | 1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp |
| 36 | 1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp |
| 37 | 1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp |
| 38 | 1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp |

| Comb. No. | Description |
|-----------|-----------------------------|
| 39 | Dead+Wind 0 deg - Service |
| 40 | Dead+Wind 30 deg - Service |
| 41 | Dead+Wind 60 deg - Service |
| 42 | Dead+Wind 90 deg - Service |
| 43 | Dead+Wind 120 deg - Service |
| 44 | Dead+Wind 150 deg - Service |
| 45 | Dead+Wind 180 deg - Service |
| 46 | Dead+Wind 210 deg - Service |
| 47 | Dead+Wind 240 deg - Service |
| 48 | Dead+Wind 270 deg - Service |
| 49 | Dead+Wind 300 deg - Service |
| 50 | Dead+Wind 330 deg - Service |

Maximum Member Forces

| Section No. | Elevation ft | Component Type | Condition | Gov. Load Comb. | Axial K | Major Axis Moment kip-ft | Minor Axis Moment kip-ft |
|-------------|--------------|----------------|------------------|-----------------|---------|--------------------------|--------------------------|
| L1 | 84 - 34.25 | Pole | Max Tension | 1 | 0.00 | 0.00 | 0.00 |
| | | | Max. Compression | 26 | -5.51 | -0.71 | -1.51 |
| | | | Max. Mx | 8 | -2.07 | -108.95 | -0.35 |
| | | | Max. My | 14 | -2.07 | -0.33 | -108.97 |
| | | | Max. Vy | 8 | 3.55 | -108.95 | -0.35 |
| | | | Max. Vx | 14 | 3.55 | -0.33 | -108.97 |
| | | | Max. Torque | 19 | | | 1.15 |
| L2 | 34.25 - 0 | Pole | Max Tension | 1 | 0.00 | 0.00 | 0.00 |
| | | | Max. Compression | 26 | -9.78 | -0.73 | -1.68 |
| | | | Max. Mx | 8 | -4.62 | -275.32 | -0.37 |
| | | | Max. My | 14 | -4.62 | -0.34 | -275.35 |
| | | | Max. Vy | 8 | 5.16 | -275.32 | -0.37 |
| | | | Max. Vx | 14 | 5.16 | -0.34 | -275.35 |
| | | | Max. Torque | 19 | | | 1.15 |

Maximum Reactions

| Location | Condition | Gov. Load Comb. | Vertical K | Horizontal, X K | Horizontal, Z K |
|----------|---------------------|-----------------|------------|-----------------|-----------------|
| Pole | Max. Vert | 33 | 9.78 | -0.00 | -1.73 |
| | Max. H _x | 21 | 3.47 | 5.15 | -0.00 |
| | Max. H _z | 2 | 4.62 | -0.00 | 5.15 |
| | Max. M _x | 2 | 274.60 | -0.00 | 5.15 |
| | Max. M _z | 8 | 275.32 | -5.15 | -0.00 |
| | Max. Torsion | 19 | 1.15 | 4.46 | -2.58 |
| | Min. Vert | 7 | 3.47 | -4.46 | 2.58 |
| | Min. H _x | 8 | 4.62 | -5.15 | -0.00 |
| | Min. H _z | 14 | 4.62 | -0.00 | -5.15 |
| | Min. M _x | 14 | -275.35 | -0.00 | -5.15 |
| | Min. M _z | 20 | -274.63 | 5.15 | -0.00 |
| | Min. Torsion | 7 | -1.15 | -4.46 | 2.58 |

Tower Mast Reaction Summary

| Load Combination | Vertical K | Shear _x K | Shear _z K | Overturning Moment, M _x kip-ft | Overturning Moment, M _z kip-ft | Torque kip-ft |
|----------------------------------|------------|----------------------|----------------------|---|---|---------------|
| Dead Only | 3.85 | 0.00 | 0.00 | 0.31 | -0.29 | 0.00 |
| 1.2 Dead+1.0 Wind 0 deg - No Ice | 4.62 | 0.00 | -5.15 | -274.60 | -0.34 | 0.47 |

| Load Combination | Vertical K | Shear _x K | Shear _z K | Overturning Moment, M _x kip-ft | Overturning Moment, M _z kip-ft | Torque kip-ft |
|---|---------------|-------------------------|-------------------------|---|---|------------------|
| 0.9 Dead+1.0 Wind 0 deg - No Ice | 3.47 | 0.00 | -5.15 | -272.87 | -0.26 | 0.48 |
| 1.2 Dead+1.0 Wind 30 deg - No Ice | 4.62 | 2.58 | -4.46 | -237.76 | -137.83 | 0.93 |
| 0.9 Dead+1.0 Wind 30 deg - No Ice | 3.47 | 2.58 | -4.46 | -236.28 | -136.83 | 0.94 |
| 1.2 Dead+1.0 Wind 60 deg - No Ice | 4.62 | 4.46 | -2.58 | -137.11 | -238.48 | 1.15 |
| 0.9 Dead+1.0 Wind 60 deg - No Ice | 3.47 | 4.46 | -2.58 | -136.30 | -236.81 | 1.15 |
| 1.2 Dead+1.0 Wind 90 deg - No Ice | 4.62 | 5.15 | 0.00 | 0.37 | -275.32 | 1.05 |
| 0.9 Dead+1.0 Wind 90 deg - No Ice | 3.47 | 5.15 | 0.00 | 0.28 | -273.41 | 1.05 |
| 1.2 Dead+1.0 Wind 120 deg - No Ice | 4.62 | 4.46 | 2.58 | 137.86 | -238.48 | 0.67 |
| 0.9 Dead+1.0 Wind 120 deg - No Ice | 3.47 | 4.46 | 2.58 | 136.85 | -236.81 | 0.67 |
| 1.2 Dead+1.0 Wind 150 deg - No Ice | 4.62 | 2.58 | 4.46 | 238.51 | -137.83 | 0.12 |
| 0.9 Dead+1.0 Wind 150 deg - No Ice | 3.47 | 2.58 | 4.46 | 236.84 | -136.83 | 0.11 |
| 1.2 Dead+1.0 Wind 180 deg - No Ice | 4.62 | 0.00 | 5.15 | 275.35 | -0.34 | -0.47 |
| 0.9 Dead+1.0 Wind 180 deg - No Ice | 3.47 | 0.00 | 5.15 | 273.43 | -0.26 | -0.48 |
| 1.2 Dead+1.0 Wind 210 deg - No Ice | 4.62 | -2.58 | 4.46 | 238.51 | 137.14 | -0.93 |
| 0.9 Dead+1.0 Wind 210 deg - No Ice | 3.47 | -2.58 | 4.46 | 236.83 | 136.32 | -0.94 |
| 1.2 Dead+1.0 Wind 240 deg - No Ice | 4.62 | -4.46 | 2.58 | 137.86 | 237.79 | -1.15 |
| 0.9 Dead+1.0 Wind 240 deg - No Ice | 3.47 | -4.46 | 2.58 | 136.85 | 236.30 | -1.15 |
| 1.2 Dead+1.0 Wind 270 deg - No Ice | 4.62 | -5.15 | 0.00 | 0.37 | 274.63 | -1.05 |
| 0.9 Dead+1.0 Wind 270 deg - No Ice | 3.47 | -5.15 | 0.00 | 0.28 | 272.90 | -1.05 |
| 1.2 Dead+1.0 Wind 300 deg - No Ice | 4.62 | -4.46 | -2.58 | -137.11 | 237.79 | -0.67 |
| 0.9 Dead+1.0 Wind 300 deg - No Ice | 3.47 | -4.46 | -2.58 | -136.30 | 236.30 | -0.67 |
| 1.2 Dead+1.0 Wind 330 deg - No Ice | 4.62 | -2.58 | -4.46 | -237.76 | 137.14 | -0.12 |
| 0.9 Dead+1.0 Wind 330 deg - No Ice | 3.47 | -2.58 | -4.46 | -236.28 | 136.32 | -0.11 |
| 1.2 Dead+1.0 Ice+1.0 Temp | 9.78 | 0.00 | 0.00 | 1.68 | -0.73 | 0.00 |
| 1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp | 9.78 | 0.00 | -1.73 | -98.49 | -0.73 | 0.12 |
| 1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp | 9.78 | 0.87 | -1.50 | -85.07 | -50.82 | 0.33 |
| 1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp | 9.78 | 1.50 | -0.87 | -48.40 | -87.48 | 0.44 |
| 1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp | 9.78 | 1.73 | 0.00 | 1.68 | -100.91 | 0.44 |
| 1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp | 9.78 | 1.50 | 0.87 | 51.77 | -87.48 | 0.33 |
| 1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp | 9.78 | 0.87 | 1.50 | 88.44 | -50.82 | 0.12 |
| 1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp | 9.78 | 0.00 | 1.73 | 101.86 | -0.73 | -0.12 |
| 1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp | 9.78 | -0.87 | 1.50 | 88.44 | 49.36 | -0.33 |
| 1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp | 9.78 | -1.50 | 0.87 | 51.77 | 86.03 | -0.44 |
| 1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp | 9.78 | -1.73 | 0.00 | 1.68 | 99.45 | -0.44 |
| 1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp | 9.78 | -1.50 | -0.87 | -48.40 | 86.03 | -0.32 |

| Load Combination | Vertical K | Shear _x K | Shear _z K | Overturning Moment, M _x kip-ft | Overturning Moment, M _z kip-ft | Torque kip-ft |
|--|---------------|-------------------------|-------------------------|---|---|------------------|
| 1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp | 9.78 | -0.87 | -1.50 | -85.07 | 49.36 | -0.12 |
| Dead+Wind 0 deg - Service | 3.85 | 0.00 | -1.06 | -56.30 | -0.29 | 0.10 |
| Dead+Wind 30 deg - Service | 3.85 | 0.53 | -0.92 | -48.71 | -28.60 | 0.20 |
| Dead+Wind 60 deg - Service | 3.85 | 0.92 | -0.53 | -27.99 | -49.32 | 0.24 |
| Dead+Wind 90 deg - Service | 3.85 | 1.06 | 0.00 | 0.31 | -56.90 | 0.22 |
| Dead+Wind 120 deg - Service | 3.85 | 0.92 | 0.53 | 28.62 | -49.32 | 0.14 |
| Dead+Wind 150 deg - Service | 3.85 | 0.53 | 0.92 | 49.34 | -28.60 | 0.02 |
| Dead+Wind 180 deg - Service | 3.85 | 0.00 | 1.06 | 56.93 | -0.29 | -0.10 |
| Dead+Wind 210 deg - Service | 3.85 | -0.53 | 0.92 | 49.34 | 28.02 | -0.20 |
| Dead+Wind 240 deg - Service | 3.85 | -0.92 | 0.53 | 28.62 | 48.74 | -0.24 |
| Dead+Wind 270 deg - Service | 3.85 | -1.06 | 0.00 | 0.31 | 56.32 | -0.22 |
| Dead+Wind 300 deg - Service | 3.85 | -0.92 | -0.53 | -27.99 | 48.74 | -0.14 |
| Dead+Wind 330 deg - Service | 3.85 | -0.53 | -0.92 | -48.71 | 28.02 | -0.02 |

Solution Summary

| Load Comb. | Sum of Applied Forces | | | Sum of Reactions | | | % Error |
|------------|-----------------------|---------|---------|------------------|---------|---------|---------|
| | PX K | PY K | PZ K | PX K | PY K | PZ K | |
| 1 | 0.00 | -3.85 | 0.00 | 0.00 | 3.85 | 0.00 | 0.000% |
| 2 | 0.00 | -4.62 | -5.15 | -0.00 | 4.62 | 5.15 | 0.000% |
| 3 | 0.00 | -3.47 | -5.15 | -0.00 | 3.47 | 5.15 | 0.000% |
| 4 | 2.58 | -4.62 | -4.46 | -2.58 | 4.62 | 4.46 | 0.000% |
| 5 | 2.58 | -3.47 | -4.46 | -2.58 | 3.47 | 4.46 | 0.000% |
| 6 | 4.46 | -4.62 | -2.58 | -4.46 | 4.62 | 2.58 | 0.000% |
| 7 | 4.46 | -3.47 | -2.58 | -4.46 | 3.47 | 2.58 | 0.000% |
| 8 | 5.15 | -4.62 | 0.00 | -5.15 | 4.62 | -0.00 | 0.000% |
| 9 | 5.15 | -3.47 | 0.00 | -5.15 | 3.47 | -0.00 | 0.000% |
| 10 | 4.46 | -4.62 | 2.58 | -4.46 | 4.62 | -2.58 | 0.000% |
| 11 | 4.46 | -3.47 | 2.58 | -4.46 | 3.47 | -2.58 | 0.000% |
| 12 | 2.58 | -4.62 | 4.46 | -2.58 | 4.62 | -4.46 | 0.000% |
| 13 | 2.58 | -3.47 | 4.46 | -2.58 | 3.47 | -4.46 | 0.000% |
| 14 | 0.00 | -4.62 | 5.15 | -0.00 | 4.62 | -5.15 | 0.000% |
| 15 | 0.00 | -3.47 | 5.15 | -0.00 | 3.47 | -5.15 | 0.000% |
| 16 | -2.58 | -4.62 | 4.46 | 2.58 | 4.62 | -4.46 | 0.000% |
| 17 | -2.58 | -3.47 | 4.46 | 2.58 | 3.47 | -4.46 | 0.000% |
| 18 | -4.46 | -4.62 | 2.58 | 4.46 | 4.62 | -2.58 | 0.000% |
| 19 | -4.46 | -3.47 | 2.58 | 4.46 | 3.47 | -2.58 | 0.000% |
| 20 | -5.15 | -4.62 | 0.00 | 5.15 | 4.62 | -0.00 | 0.000% |
| 21 | -5.15 | -3.47 | 0.00 | 5.15 | 3.47 | -0.00 | 0.000% |
| 22 | -4.46 | -4.62 | -2.58 | 4.46 | 4.62 | 2.58 | 0.000% |
| 23 | -4.46 | -3.47 | -2.58 | 4.46 | 3.47 | 2.58 | 0.000% |
| 24 | -2.58 | -4.62 | -4.46 | 2.58 | 4.62 | 4.46 | 0.000% |
| 25 | -2.58 | -3.47 | -4.46 | 2.58 | 3.47 | 4.46 | 0.000% |
| 26 | 0.00 | -9.78 | 0.00 | -0.00 | 9.78 | -0.00 | 0.000% |
| 27 | 0.00 | -9.78 | -1.73 | -0.00 | 9.78 | 1.73 | 0.000% |
| 28 | 0.87 | -9.78 | -1.50 | -0.87 | 9.78 | 1.50 | 0.000% |
| 29 | 1.50 | -9.78 | -0.87 | -1.50 | 9.78 | 0.87 | 0.000% |
| 30 | 1.73 | -9.78 | 0.00 | -1.73 | 9.78 | -0.00 | 0.000% |
| 31 | 1.50 | -9.78 | 0.87 | -1.50 | 9.78 | -0.87 | 0.000% |
| 32 | 0.87 | -9.78 | 1.50 | -0.87 | 9.78 | -1.50 | 0.000% |
| 33 | 0.00 | -9.78 | 1.73 | -0.00 | 9.78 | -1.73 | 0.000% |
| 34 | -0.87 | -9.78 | 1.50 | 0.87 | 9.78 | -1.50 | 0.000% |
| 35 | -1.50 | -9.78 | 0.87 | 1.50 | 9.78 | -0.87 | 0.000% |
| 36 | -1.73 | -9.78 | 0.00 | 1.73 | 9.78 | -0.00 | 0.000% |
| 37 | -1.50 | -9.78 | -0.87 | 1.50 | 9.78 | 0.87 | 0.000% |
| 38 | -0.87 | -9.78 | -1.50 | 0.87 | 9.78 | 1.50 | 0.000% |

| Load Comb. | Sum of Applied Forces | | | Sum of Reactions | | | % Error |
|------------|-----------------------|---------|---------|------------------|---------|---------|---------|
| | PX K | PY K | PZ K | PX K | PY K | PZ K | |
| 39 | 0.00 | -3.85 | -1.06 | 0.00 | 3.85 | 1.06 | 0.000% |
| 40 | 0.53 | -3.85 | -0.92 | -0.53 | 3.85 | 0.92 | 0.000% |
| 41 | 0.92 | -3.85 | -0.53 | -0.92 | 3.85 | 0.53 | 0.000% |
| 42 | 1.06 | -3.85 | 0.00 | -1.06 | 3.85 | 0.00 | 0.000% |
| 43 | 0.92 | -3.85 | 0.53 | -0.92 | 3.85 | -0.53 | 0.000% |
| 44 | 0.53 | -3.85 | 0.92 | -0.53 | 3.85 | -0.92 | 0.000% |
| 45 | 0.00 | -3.85 | 1.06 | 0.00 | 3.85 | -1.06 | 0.000% |
| 46 | -0.53 | -3.85 | 0.92 | 0.53 | 3.85 | -0.92 | 0.000% |
| 47 | -0.92 | -3.85 | 0.53 | 0.92 | 3.85 | -0.53 | 0.000% |
| 48 | -1.06 | -3.85 | 0.00 | 1.06 | 3.85 | 0.00 | 0.000% |
| 49 | -0.92 | -3.85 | -0.53 | 0.92 | 3.85 | 0.53 | 0.000% |
| 50 | -0.53 | -3.85 | -0.92 | 0.53 | 3.85 | 0.92 | 0.000% |

Non-Linear Convergence Results

| Load Combination | Converged? | Number of Cycles | Displacement Tolerance | Force Tolerance |
|------------------|------------|------------------|------------------------|-----------------|
| 1 | Yes | 4 | 0.00000001 | 0.00000001 |
| 2 | Yes | 4 | 0.00000001 | 0.00045030 |
| 3 | Yes | 4 | 0.00000001 | 0.00026285 |
| 4 | Yes | 5 | 0.00000001 | 0.00007940 |
| 5 | Yes | 5 | 0.00000001 | 0.00003247 |
| 6 | Yes | 5 | 0.00000001 | 0.00005347 |
| 7 | Yes | 5 | 0.00000001 | 0.00000001 |
| 8 | Yes | 4 | 0.00000001 | 0.00094922 |
| 9 | Yes | 4 | 0.00000001 | 0.00055649 |
| 10 | Yes | 5 | 0.00000001 | 0.00007431 |
| 11 | Yes | 5 | 0.00000001 | 0.00003003 |
| 12 | Yes | 5 | 0.00000001 | 0.00006135 |
| 13 | Yes | 5 | 0.00000001 | 0.00000001 |
| 14 | Yes | 4 | 0.00000001 | 0.00045117 |
| 15 | Yes | 4 | 0.00000001 | 0.00026295 |
| 16 | Yes | 5 | 0.00000001 | 0.00005374 |
| 17 | Yes | 5 | 0.00000001 | 0.00000001 |
| 18 | Yes | 5 | 0.00000001 | 0.00008390 |
| 19 | Yes | 5 | 0.00000001 | 0.00003446 |
| 20 | Yes | 4 | 0.00000001 | 0.00094807 |
| 21 | Yes | 4 | 0.00000001 | 0.00055664 |
| 22 | Yes | 5 | 0.00000001 | 0.00005475 |
| 23 | Yes | 5 | 0.00000001 | 0.00000001 |
| 24 | Yes | 5 | 0.00000001 | 0.00006353 |
| 25 | Yes | 5 | 0.00000001 | 0.00000001 |
| 26 | Yes | 4 | 0.00000001 | 0.00003689 |
| 27 | Yes | 5 | 0.00000001 | 0.00008590 |
| 28 | Yes | 5 | 0.00000001 | 0.00014014 |
| 29 | Yes | 5 | 0.00000001 | 0.00012552 |
| 30 | Yes | 5 | 0.00000001 | 0.00010894 |
| 31 | Yes | 5 | 0.00000001 | 0.00015104 |
| 32 | Yes | 5 | 0.00000001 | 0.00013248 |
| 33 | Yes | 5 | 0.00000001 | 0.00009114 |
| 34 | Yes | 5 | 0.00000001 | 0.00012732 |
| 35 | Yes | 5 | 0.00000001 | 0.00015389 |
| 36 | Yes | 5 | 0.00000001 | 0.00010589 |
| 37 | Yes | 5 | 0.00000001 | 0.00011911 |
| 38 | Yes | 5 | 0.00000001 | 0.00012464 |
| 39 | Yes | 4 | 0.00000001 | 0.00000001 |
| 40 | Yes | 4 | 0.00000001 | 0.00007604 |
| 41 | Yes | 4 | 0.00000001 | 0.00005715 |
| 42 | Yes | 4 | 0.00000001 | 0.00006222 |
| 43 | Yes | 4 | 0.00000001 | 0.00006224 |
| 44 | Yes | 4 | 0.00000001 | 0.00000001 |
| 45 | Yes | 4 | 0.00000001 | 0.00000001 |
| 46 | Yes | 4 | 0.00000001 | 0.00000001 |
| 47 | Yes | 4 | 0.00000001 | 0.00008699 |
| 48 | Yes | 4 | 0.00000001 | 0.00006095 |

| | | | | |
|----|-----|---|------------|------------|
| 49 | Yes | 4 | 0.00000001 | 0.00000001 |
| 50 | Yes | 4 | 0.00000001 | 0.00000001 |

Maximum Tower Deflections - Service Wind

| Section No. | Elevation ft | Horz. Deflection in | Gov. Load Comb. | Tilt ° | Twist ° |
|-------------|-----------------|---------------------------|-----------------------|-----------|------------|
| L1 | 84 - 34.25 | 11.155 | 44 | 1.0486 | 0.0208 |
| L2 | 38 - 0 | 2.598 | 44 | 0.6092 | 0.0051 |

Critical Deflections and Radius of Curvature - Service Wind

| Elevation ft | Appurtenance | Gov. Load Comb. | Deflection in | Tilt ° | Twist ° | Radius of Curvature ft |
|-----------------|------------------------------|-----------------------|------------------|-----------|------------|------------------------------|
| 83.00 | Side Arm Mount [4' SO 701-1] | 44 | 10.936 | 1.0408 | 0.0204 | 26287 |
| 67.00 | 1151-3 | 44 | 7.520 | 0.9122 | 0.0142 | 7731 |

Maximum Tower Deflections - Design Wind

| Section No. | Elevation ft | Horz. Deflection in | Gov. Load Comb. | Tilt ° | Twist ° |
|-------------|-----------------|---------------------------|-----------------------|-----------|------------|
| L1 | 84 - 34.25 | 53.438 | 12 | 4.9853 | 0.0982 |
| L2 | 38 - 0 | 12.525 | 12 | 2.9349 | 0.0240 |

Critical Deflections and Radius of Curvature - Design Wind

| Elevation ft | Appurtenance | Gov. Load Comb. | Deflection in | Tilt ° | Twist ° | Radius of Curvature ft |
|-----------------|------------------------------|-----------------------|------------------|-----------|------------|------------------------------|
| 83.00 | Side Arm Mount [4' SO 701-1] | 12 | 52.395 | 4.9496 | 0.0963 | 5584 |
| 67.00 | 1151-3 | 12 | 36.068 | 4.3587 | 0.0668 | 1640 |

Compression Checks

Pole Design Data

| Section No. | Elevation ft | Size | L ft | L _u ft | KI/r | A in ² | P _u K | φP _n K | Ratio P _u / φP _n |
|-------------|-----------------|------------------------|---------|----------------------|------|----------------------|---------------------|----------------------|--|
| L1 | 84 - 34.25 (1) | TP18.145x12.001x0.1875 | 49.75 | 0.00 | 0.0 | 10.411 3 | -2.07 | 609.06 | 0.003 |
| L2 | 34.25 - 0 (2) | TP22x17.3069x0.2188 | 38.00 | 0.00 | 0.0 | 15.123 0 | -4.62 | 884.70 | 0.005 |

Pole Bending Design Data

| Section No. | Elevation ft | Size | M_{ux} kip-ft | ϕM_{nx} kip-ft | Ratio $\frac{M_{ux}}{\phi M_{nx}}$ | M_{uy} kip-ft | ϕM_{ny} kip-ft | Ratio $\frac{M_{uy}}{\phi M_{ny}}$ |
|-------------|-----------------|------------------------|--------------------|-------------------------|---------------------------------------|--------------------|-------------------------|---------------------------------------|
| L1 | 84 - 34.25 (1) | TP18.145x12.001x0.1875 | 109.09 | 277.48 | 0.393 | 0.00 | 277.48 | 0.000 |
| L2 | 34.25 - 0 (2) | TP22x17.3069x0.2188 | 275.46 | 496.47 | 0.555 | 0.00 | 496.47 | 0.000 |

Pole Shear Design Data

| Section No. | Elevation ft | Size | Actual V_u K | ϕV_n K | Ratio $\frac{V_u}{\phi V_n}$ | Actual T_u kip-ft | ϕT_n kip-ft | Ratio $\frac{T_u}{\phi T_n}$ |
|-------------|-----------------|------------------------|----------------------|-----------------|---------------------------------|---------------------------|----------------------|---------------------------------|
| L1 | 84 - 34.25 (1) | TP18.145x12.001x0.1875 | 3.55 | 182.72 | 0.019 | 0.67 | 279.94 | 0.002 |
| L2 | 34.25 - 0 (2) | TP22x17.3069x0.2188 | 5.16 | 265.41 | 0.019 | 0.67 | 506.26 | 0.001 |

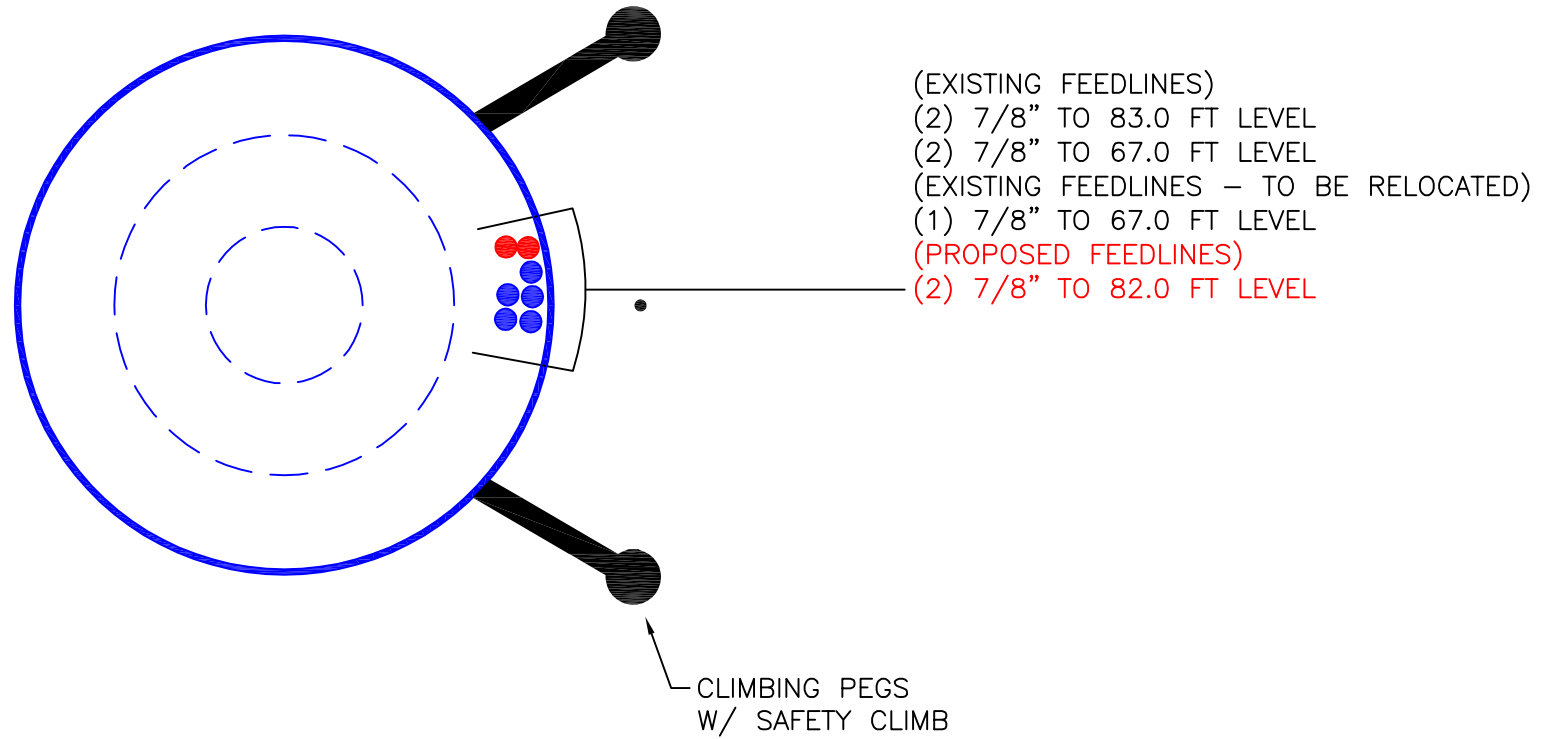
Pole Interaction Design Data

| Section No. | Elevation ft | Ratio P_u ϕP_n | Ratio M_{ux} ϕM_{nx} | Ratio M_{uy} ϕM_{ny} | Ratio V_u ϕV_n | Ratio T_u ϕT_n | Comb. Stress Ratio | Allow. Stress Ratio | Criteria |
|-------------|-----------------|------------------------------|------------------------------------|------------------------------------|------------------------------|------------------------------|--------------------------|---------------------------|----------|
| L1 | 84 - 34.25 (1) | 0.003 | 0.393 | 0.000 | 0.019 | 0.002 | 0.397 | 1.050 | 4.8.2 |
| L2 | 34.25 - 0 (2) | 0.005 | 0.555 | 0.000 | 0.019 | 0.001 | 0.560 | 1.050 | 4.8.2 |

Section Capacity Table

| Section No. | Elevation ft | Component Type | Size | Critical Element | P K | ϕP_{allow} K | % Capacity | Pass Fail |
|-------------|-----------------|-------------------|------------------------|---------------------|--------|-----------------------|-----------------|--------------|
| L1 | 84 - 34.25 | Pole | TP18.145x12.001x0.1875 | 1 | -2.07 | 639.52 | 37.8 | Pass |
| L2 | 34.25 - 0 | Pole | TP22x17.3069x0.2188 | 2 | -4.62 | 928.93 | 53.4 | Pass |
| | | | | | | | Summary | |
| | | | | | | | Pole (L2) | 53.4 |
| | | | | | | | RATING = | 53.4 |
| | | | | | | | | Pass |

APPENDIX B
BASE LEVEL DRAWING



RIDGEFIELD 22N

APPENDIX C

ADDITIONAL CALCULATIONS

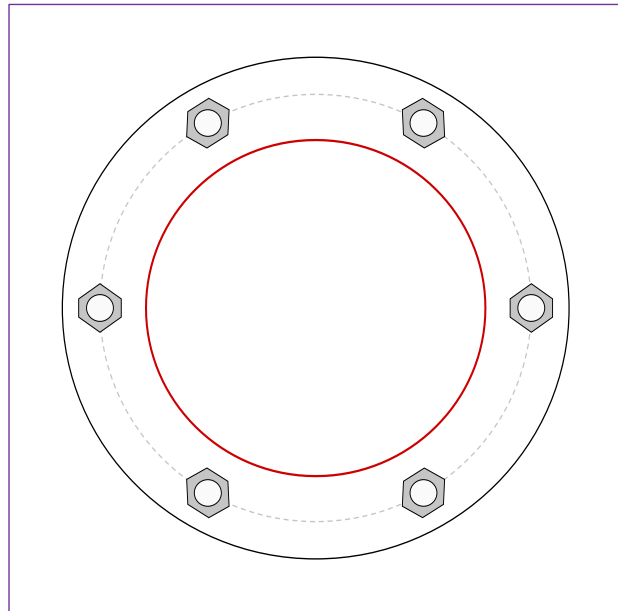
Monopole Base Plate Connection

| Site Info | |
|-----------|--------------|
| | ES-074 |
| | NewMilfordRS |
| | |

| Analysis Considerations | |
|-------------------------|-------|
| TIA-222 Revision | H |
| Grout Considered: | No |
| l_{ar} (in) | 2.125 |

| Applied Loads | |
|--------------------|--------|
| Moment (kip-ft) | 275.47 |
| Axial Force (kips) | 4.62 |
| Shear Force (kips) | 5.16 |

*TIA-222-H Section 15.5 Applied



| Connection Properties | | Analysis Results | |
|---|--|--|--|
| Anchor Rod Data | | Anchor Rod Summary <i>(units of kips, kip-in)</i> | |
| (6) 1-3/4" ϕ bolts (A615-75 N; Fy=75 ksi, Fu=100 ksi) on 27.96" BC | | Pu_t = 77.93 | $\phi Pn_t = 142.5$ Stress Rating |
| Base Plate Data | | Vu = 0.86 | $\phi Vn = 90.2$ 52.1% |
| 32.84" OD x 2.25" Plate (A572-50; Fy=50 ksi, Fu=65 ksi) | | Mu = 1.19 | $\phi Mn = 60.29$ Pass |
| Stiffener Data | | Base Plate Summary | |
| N/A | | Max Stress (ksi): | 10.84 (Flexural) |
| Pole Data | | Allowable Stress (ksi): | 45 |
| 22" x 0.21875" 18-sided pole (A572-65; Fy=65 ksi, Fu=80 ksi) | | Stress Rating: | 22.9% Pass |

Pier and Pad Foundation

| |
|---------------|
| ES-286 |
| Ridgefield22N |
| |

| | |
|-------------------|----------|
| TIA-222 Revision: | H |
| Tower Type: | Monopole |

| | |
|----------------------------------|-------------------------------------|
| Top & Bot. Pad Rein. Different?: | <input checked="" type="checkbox"/> |
| Block Foundation?: | <input type="checkbox"/> |
| Rectangular Pad?: | <input type="checkbox"/> |

| Superstructure Analysis Reactions | | |
|--|--------|---------|
| Compression, P_{comp} : | 4.62 | kips |
| Base Shear, Vu_{comp} : | 5.15 | kips |
| | | |
| | | |
| Moment, M_u : | 275.47 | ft-kips |
| Tower Height, H : | 84 | ft |
| | | |
| BP Dist. Above Fdn, bp_{dist} : | 6 | in |

| Pier Properties | | |
|--|----------|----|
| Pier Shape: | Circular | |
| Pier Diameter, dpier : | 4.5 | ft |
| Ext. Above Grade, E : | 0.5 | ft |
| Pier Rebar Size, Sc : | 9 | |
| Pier Rebar Quantity, mc : | 12 | |
| Pier Tie/Spiral Size, St : | 4 | |
| Pier Tie/Spiral Quantity, mt : | 7 | |
| Pier Reinforcement Type: | Tie | |
| Pier Clear Cover, cc_{pier} : | 6 | in |

| Pad Properties | | |
|---|-----|----|
| Depth, D : | 5.5 | ft |
| Pad Width, W₁ : | 12 | ft |
| Pad Thickness, T : | 2 | ft |
| Pad Rebar Size (Top dir. 2), Sp_{top2} : | 4 | |
| Pad Rebar Quantity (Top dir. 2), mp_{top2} : | 9 | |
| Pad Rebar Size (Bottom dir. 2), Sp₂ : | 6 | |
| Pad Rebar Quantity (Bottom dir. 2), mp₂ : | 11 | |
| Pad Clear Cover, cc_{pad} : | 3 | in |

| Material Properties | | |
|---|-----|-----|
| Rebar Grade, Fy : | 60 | ksi |
| Concrete Compressive Strength, F'c : | 3 | ksi |
| Dry Concrete Density, δc : | 150 | pcf |

| Soil Properties | | |
|--|-------|---------|
| Total Soil Unit Weight, γ : | 125 | pcf |
| Ultimate Gross Bearing, Qult : | 8.000 | ksf |
| Cohesion, Cu : | | ksf |
| Friction Angle, φ : | 34 | degrees |
| SPT Blow Count, N_{blows} : | | |
| Base Friction, μ : | 0.6 | |
| Neglected Depth, N : | 3.50 | ft |
| Foundation Bearing on Rock? | No | |
| Groundwater Depth, gw : | N/A | ft |

| Foundation Analysis Checks | | | | |
|--------------------------------|----------|--------|---------|-------|
| | Capacity | Demand | Rating* | Check |
| | | | | |
| Lateral (Sliding) (kips) | 81.43 | 5.15 | 6.0% | Pass |
| Bearing Pressure (ksf) | 6.00 | 1.71 | 28.5% | Pass |
| Overturning (kip*ft) | 648.41 | 308.95 | 47.6% | Pass |
| Pier Flexure (Comp.) (kip*ft) | 1147.42 | 296.07 | 24.6% | Pass |
| | | | | |
| Pier Compression (kip) | 7592.08 | 16.07 | 0.2% | Pass |
| Pad Flexure (kip*ft) | 424.27 | 82.23 | 18.5% | Pass |
| Pad Shear - 1-way (kips) | 235.14 | 25.59 | 10.4% | Pass |
| Pad Shear - 2-way (Comp) (ksi) | 0.164 | 0.013 | 7.4% | Pass |
| Flexural 2-way (Comp) (kip*ft) | 487.12 | 177.64 | 34.7% | Pass |

*Rating per TIA-222-H Section 15.5

| | |
|---------------------|-------|
| Structural Rating*: | 34.7% |
| Soil Rating*: | 47.6% |

<--Toggle between Gross and Net

ATTACHMENT E – PROOF OF DELIVERY OF NOTICE

ORIGIN ID:SKKA (860) 798-6597
 BRIAN GAUDET
 ALL-POINTS TECHNOLOGY CORP. P.C.
 567 VAUXHALL STREET EXTENSION
 SUITE 311
 WATERFORD, CT 06385
 UNITED STATES US

SHIP DATE: 10SEP21
 ACTWGT: 3.00 LB
 CAD: 4762401/INET1400

BILL SENDER

TO CONNECTICUT SITING COUNCIL

10 FRANKLIN SQ

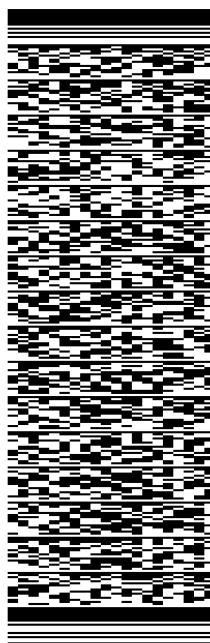
NEW BRITAIN CT 06051

(860) 827-2935

REF: CT578110, CT578140, CT578120

INV:

DEPT:



56DJ3/169AFE4A

2 of 3

MPS# 7747 6682 7436

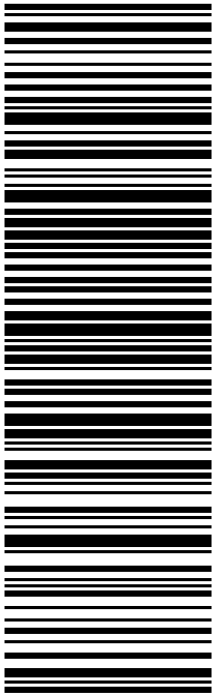
Mstr# 7747 6682 7116

0201

MON - 13 SEP 10:30A
 PRIORITY OVERNIGHT

00 BDLA

06051
 CT-US BDL



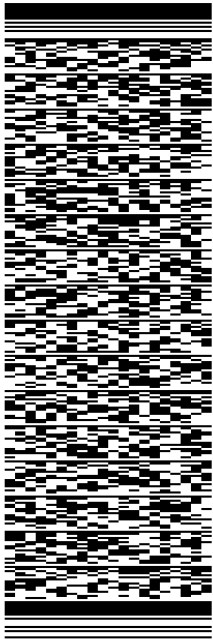

After printing this label:

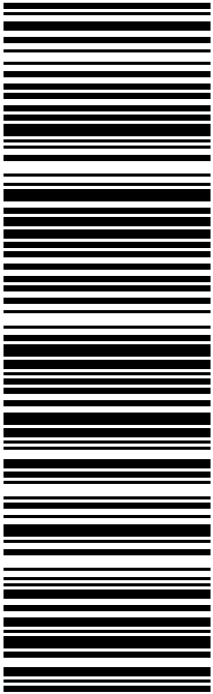
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| | |
|---|---|
| ORIGIN ID:SKKA (860) 798-6597 BRIAN GAUDET ALL-POINTS TECHNOLOGY CORP. P.C. 567 VAUXHALL STREET EXTENSION SUITE 311 WATERFORD, CT 06385 UNITED STATES US | SHIP DATE: 10SEP21 ACTWGT: 1.00 LB CAD: 4762401/INET1400 BILL SENDER |
| TO RICHARD BALDELLI TOWN OF RIDGEFIELD 400 MAIN STREET PLANNING & ZONING RIDGEFIELD CT 06877 (000) 000-0000 REF: CT578140 INV: DEPT: PO: | |



SH WODA
 CT-US **SWF**
06877

TRK# 7747 6784 7515
0201
MON - 13 SEP 10:30A
PRIORITY OVERNIGHT

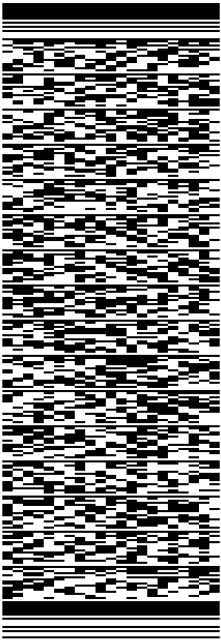

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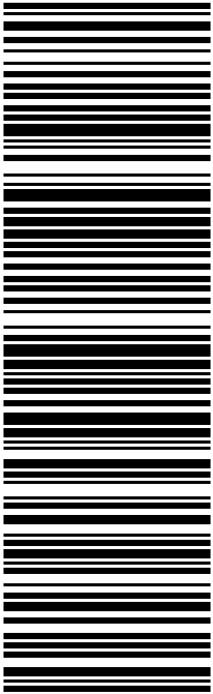
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| TO HONORABLE RUDY MARCONI TOWN OF RIDGEFIELD 400 MAIN STREET SELECTMAN'S OFFICE RIDGEFIELD CT 06877 (000) 000-0000 INV: REF: CT578140 PO: DEPT: | | BILL SENDER |

J212021070901uv

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 PRIORITY OVERNIGHT
 TRK# 7747 6756 5413
 0201
 SH WODA
 CT-US
 06877
 SWF


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ATTACHMENT F - POWER DENSITY REPORT



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Calculated Radio Frequency Emissions Report



ES-286

Off Prospect Street

Ridgefield, CT 06877

August 18, 2021

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1. Introduction

The purpose of this report is to investigate compliance with applicable FCC regulations for the existing Eversource installation located off Prospect Street in Ridgefield, CT.

Eversource has recently installed one omnidirectional antenna for both transmit and receive purposes as part of its 220 MHz communications system. The original proposal consisted of two omnidirectional antennas – separate transmit and receive antennas. This report provides an updated analysis based on the current installation as reflected in the update site plans¹.

This report considers the existing antenna configuration as provided by Eversource along with power density information of the other existing antennas to calculate the overall % MPE (Maximum Permissible Exposure) of the facility at ground level.

2. FCC Guidelines for Evaluating RF Radiation Exposure Limits

In 1985, the FCC established rules to regulate radio frequency (RF) exposure from FCC licensed antenna facilities. In 1996, the FCC updated these rules, which were further amended in August 1997 by OET Bulletin 65 Edition 97-01. These new rules include Maximum Permissible Exposure (MPE) limits for transmitters operating between 300 kHz and 100 GHz. The FCC MPE limits are based upon those recommended by the National Council on Radiation Protection and Measurements (NCRP), developed by the Institute of Electrical and Electronics Engineers, Inc., (IEEE) and adopted by the American National Standards Institute (ANSI).

The FCC general population/uncontrolled limits set the maximum exposure to which most people may be subjected. General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure.

Public exposure to radio frequencies is regulated and enforced in units of milliwatts per square centimeter (mW/cm²). The general population exposure limits for the various frequency ranges are defined in the attached “FCC Limits for Maximum Permissible Exposure (MPE)” in Attachment B of this report.

Higher exposure limits are permitted under the occupational/controlled exposure category, but only for persons who are exposed as a consequence of their employment and who have been made fully aware of the potential for exposure, and they must be able to exercise control over their exposure. General population/uncontrolled limits are five times more stringent than the levels that are acceptable for occupational, or radio frequency trained individuals. Attachment B contains excerpts from OET Bulletin 65 and defines the Maximum Exposure Limit.

Finally, it should be noted that the MPE limits adopted by the FCC for both general population/uncontrolled exposure and for occupational/controlled exposure incorporate a substantial margin of safety and have been established to be well below levels generally accepted as having the potential to cause adverse health effects.

¹ Stamped Black & Veatch site drawings dated 8/13/2021 (Rev. 1).

3. Power Density Calculation Methods

The power density calculation results were generated using the following formula as outlined in FCC bulletin OET 65, and Connecticut Siting Council recommendations:

$$\text{Power Density} = \left(\frac{1.6^2 \times 1.64 \times \text{ERP}}{4\pi \times R^2} \right) \times \text{Off Beam Loss}$$

Where:

EIRP = Effective Isotropic Radiated Power = $1.64 \times \text{ERP}$

R = Radial Distance = $\sqrt{H^2 + V^2}$

H = Horizontal Distance from antenna

V = Vertical Distance from radiation center of antenna

Ground reflection factor of 1.6

Off Beam Loss is determined by the selected antenna pattern

These calculations assume that the antennas are operating at 100 percent capacity and full power, and that all antenna channels are transmitting simultaneously. Obstructions (trees, buildings, etc.) that would normally attenuate the signal are not taken into account. The calculations assume even terrain in the area of study and do not consider actual terrain elevations which could attenuate the signal. As a result, the calculated power density and corresponding % MPE levels reported below are much higher than the actual levels will be from the final installation.

4. Calculated % MPE Results

Table 1 below outlines the power density information for the site. The new Eversource omnidirectional antenna has a vertical beamwidth of 60°; therefore, the majority of the RF power is focused out towards the horizon. Please refer to Attachment C for the vertical pattern of the recently installed 220 MHz Eversource antenna. Likewise, the other transmit antennas exhibit similar directionality of varying vertical beamwidths. As a result, there will be less RF power directed below the antennas relative to the horizon, and consequently lower power density levels around the base of the facility. The calculated results in Table 1 include a nominal 10 dB off-beam pattern loss to account for the lower relative gain below the antennas. Any inactive or receive-only antennas are not listed in the table, as they are irrelevant in terms of the % MPE calculations.

| Carrier | Antenna Height (Feet) | Operating Frequency (MHz) | Number of Trans. | ERP Per Transmitter (Watts) | Power Density (mw/cm ²) | Limit | %MPE |
|------------|-----------------------|---------------------------|------------------|-----------------------------|-------------------------------------|--------------|--------------|
| CL&P | 85 | 37.74 | 1 | 100 | 0.0006 | 0.2000 | 0.29% |
| CL&P | 70 | 44 | 1 | 100 | 0.0009 | 0.2000 | 0.44% |
| CL&P | 70 | 48 | 1 | 100 | 0.0009 | 0.2000 | 0.44% |
| CL&P | 85 | 450 | 1 | 251 | 0.0014 | 0.3000 | 0.48% |
| CL&P | 85 | 937 | 2 | 240 | 0.0028 | 0.6247 | 0.44% |
| Eversource | 90 | 44.34 | 1 | 100 | 0.0005 | 0.2000 | 0.25% |
| Eversource | 88 | 936.6375 | 1 | 240 | 0.0013 | 0.6244 | 0.21% |
| Eversource | 88 | 938.45 | 1 | 240 | 0.0013 | 0.6256 | 0.21% |
| Eversource | 74 | 37.74 | 1 | 100 | 0.0008 | 0.2000 | 0.39% |
| Eversource | 74 | 451.675 | 1 | 251 | 0.0020 | 0.3011 | 0.65% |
| Eversource | 73 | 48.34 | 1 | 100 | 0.0008 | 0.2000 | 0.40% |
| Eversource | 95.3 | 217 | 4 | 124 | 0.0022 | 0.2000 | 1.12% |
| | | | | | | Total | 3.22% |

Table 1: Proposed Facility % MPE ^{2 3}

The CT Siting Council power density database reflects entries for pre-existing Eversource (f.k.a. CL&P) antennas. These entries are shown as grey in the table above and should be replaced by the unshaded entries, which are based upon updated operating parameters provided by Eversource as part of this project. The blue entry reflects the parameters of the recently installed Eversource antenna. Therefore, the total % MPE calculated is based upon only the unshaded and blue entries.

² Please note that % MPE values listed are rounded to two decimal points and the total % MPE listed is a summation of each unrounded contribution. Therefore, summing each rounded value may not identically match the total value reflected in the table.

³ The antenna heights listed for Eversource are in reference to the Black & Veatch Structural Analysis Report dated 07/29/2021.

5. Conclusion

The above analysis concludes that RF exposure at ground level with the new Eversource 220 MHz antenna installation will be below the maximum power density limits as outlined by the FCC in the OET Bulletin 65 Ed. 97-01. Using the conservative calculation methods discussed herein, the highest expected percent of Maximum Permissible Exposure at ground level from the existing installation is **3.22% of the FCC General Population/Uncontrolled limit.**

As noted previously, the calculated % MPE levels are more conservative (higher) than the actual measured levels will be from the installation.

6. Statement of Certification

I certify to the best of my knowledge that the statements in this report are true and accurate. The calculations follow guidelines set forth in FCC OET Bulletin 65 Edition 97-01, IEEE Std. C95.1, and IEEE Std. C95.3.

Report Prepared By: Keith Vellante
Keith Vellante
Director – RF Services
C Squared Systems, LLC

August 18, 2021
Date

Attachment A: References

OET Bulletin 65 - Edition 97-01 - August 1997 Federal Communications Commission Office of Engineering & Technology

IEEE C95.1-2005, IEEE Standard Safety Levels With Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz IEEE-SA Standards Board

IEEE C95.3-2002 (R2008), IEEE Recommended Practice for Measurements and Computations of Radio Frequency Electromagnetic Fields With Respect to Human Exposure to Such Fields, 100 kHz-300 GHz IEEE-SA Standards Board

Attachment B: FCC Limits for Maximum Permissible Exposure (MPE)

(A) Limits for Occupational/Controlled Exposure⁴

| Frequency Range (MHz) | Electric Field Strength (E) (V/m) | Magnetic Field Strength (E) (A/m) | Power Density (S) (mW/cm ²) | Averaging Time E ² , H ² or S (minutes) |
|-----------------------|-----------------------------------|-----------------------------------|---|---|
| 0.3-3.0 | 614 | 1.63 | (100)* | 6 |
| 3.0-30 | 1842/f | 4.89/f | (900/f ²)* | 6 |
| 30-300 | 61.4 | 0.163 | 1.0 | 6 |
| 300-1500 | - | - | f/300 | 6 |
| 1500-100,000 | - | - | 5 | 6 |

(B) Limits for General Population/Uncontrolled Exposure⁵

| Frequency Range (MHz) | Electric Field Strength (E) (V/m) | Magnetic Field Strength (E) (A/m) | Power Density (S) (mW/cm ²) | Averaging Time E ² , H ² or S (minutes) |
|-----------------------|-----------------------------------|-----------------------------------|---|---|
| 0.3-1.34 | 614 | 1.63 | (100)* | 30 |
| 1.34-30 | 824/f | 2.19/f | (180/f ²)* | 30 |
| 30-300 | 27.5 | 0.073 | 0.2 | 30 |
| 300-1500 | - | - | f/1500 | 30 |
| 1500-100,000 | - | - | 1.0 | 30 |

f = frequency in MHz * Plane-wave equivalent power density

Table 2: FCC Limits for Maximum Permissible Exposure (MPE)

⁴ Occupational/controlled limits apply in situations in which persons are exposed as a consequence of their employment provided those persons are fully aware of the potential for exposure and can exercise control over their exposure. Limits for occupational/controlled exposure also apply in situations when an individual is transient through a location where occupational/controlled limits apply provided he or she is made aware of the potential for exposure

⁵ General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure

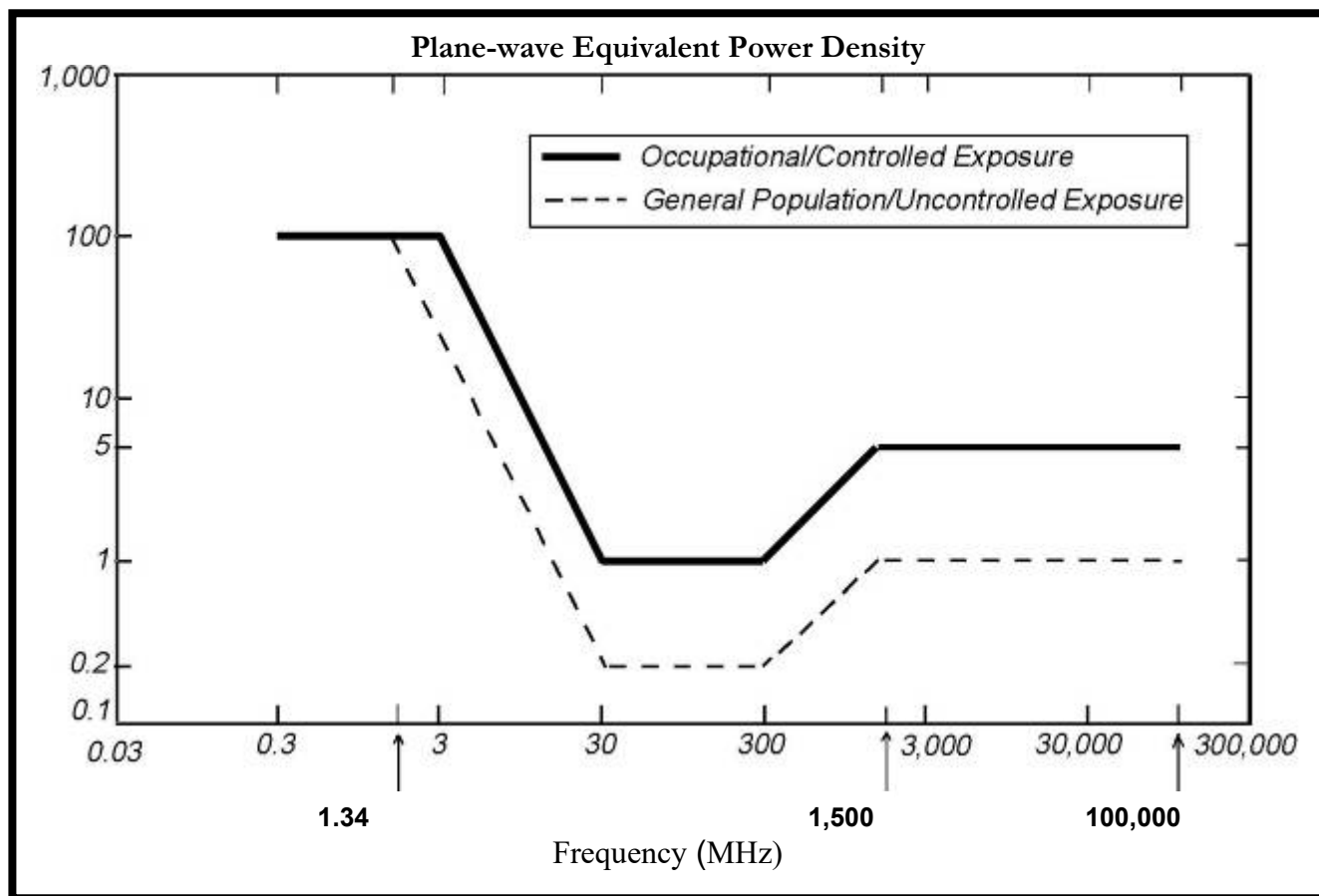
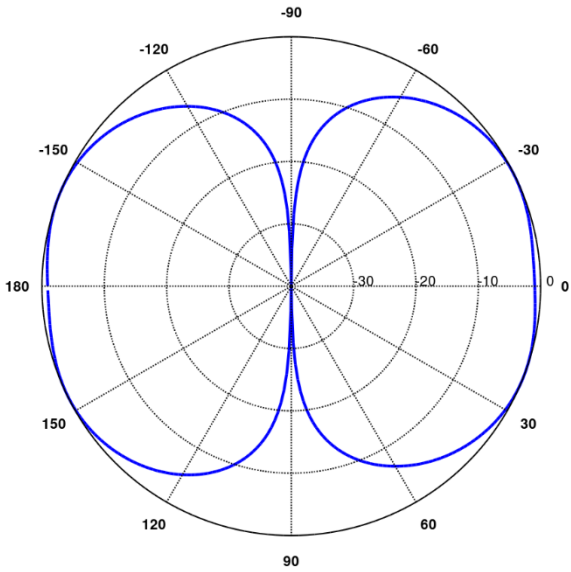


Figure 1: Graph of FCC Limits for Maximum Permissible Exposure (MPE)

Attachment C: Eversource Antenna Data Sheets and Electrical Patterns

| 217 MHz | |  |
|-----------------------|--------------|--|
| Manufacturer: | dbSpectra | |
| Model #: | SP2D00P36D-D | |
| Frequency Band: | 217-220 MHz | |
| Gain: | 0 dBd | |
| Vertical Beamwidth: | 60° | |
| Horizontal Beamwidth: | 360° | |
| Polarization: | Vertical | |
| Length: | 15.6' | |