



STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL

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VIA ELECTRONIC MAIL

April 26, 2024

Jeffrey Barbadora
Permitting Specialist
Crown Castle
1800 West Park Drive
Westborough, MA 01581
Jeff.Barbadora@crowncastle.com

RE: **EM-VER-108-230809** - Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 691 Oxford Road, Oxford, Connecticut.
Request for Project Change.

Dear Jeffrey Barbadora:

The Connecticut Siting Council (Council) is in receipt of the correspondence dated April 24, 2024 and the associated Structural Analysis dated October 25, 2023, regarding a project change for the above-referenced exempt modification request acknowledged by the Council on September 6, 2023.

Pursuant to Condition No. 1 of the Council's September 6, 2023 exempt modification approval, the request to increase the number of Kaelus interference mitigation filters to be installed from three to six is hereby approved.

This approval applies only to the project change in the correspondence dated April 24, 2024.

Thank you for your attention and cooperation.

Sincerely,

Melanie A. Bachman
Executive Director

MAB/ANM/laf

c: The Honorable George R. Temple, First Selectperson, Town of Oxford (selectman@oxford-ct.gov)

From: Barbadora, Jeff <Jeff.Barbadora@crowncastle.com>
Sent: Wednesday, April 24, 2024 12:48 PM
To: CSC-DL Siting Council <Siting.Council@ct.gov>
Subject: EM-VER-108-230809 - 691 Oxford Road, Oxford, CT - 873645

Good afternoon,

Would the CSC please update the approval for EM-VER-108-230809 to include a total of 6 filters?

The original SA submitted with the application and dated 7/26/2023 stated only 3 filters and should have stated 6 filters.

Please see updated SA stating 6 filters and let me know if you have any questions.

Thanks,

Jeffrey Barbadora
Permitting Specialist
781-970-0053

Crown Castle
1800 W. Park Drive, Suite 250
Westborough, MA 01581

Date: **October 25, 2023**



Crown Castle
2000 Corporate Drive
Canonsburg, PA 15317
(724) 416-2000

Subject: **Structural Analysis Report**

Carrier Designation: **Verizon Wireless Co-Locate**
Site Number: 5000385034
Site Name: OXFORD NORTH CT

Crown Castle Designation: **BU Number:** 873645
Site Name: Oxford
JDE Job Number: 2103535
Work Order Number: 2265106
Order Number: 658829 Rev. 0

Engineering Firm Designation: **Crown Castle Project Number:** 2265106

Site Data: **691 Oxford RD, OXFORD, NEW HAVEN County, CT**
Latitude 41° 26' 49.51", Longitude -73° 9' 8.316"
150 Foot - Monopole Tower

Crown Castle is pleased to submit this “**Structural Analysis Report**” to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC5: Proposed Equipment Configuration

Sufficient Capacity – 61.7%

This analysis has been performed in accordance with the 2022 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 117 mph. Applicable Standard references and design criteria are listed in Section 2 - "Analysis Criteria".

Structural analysis prepared by: Steven Hu

Respectfully submitted by:

Rohit Soni, P.E.
Senior Project Engineer

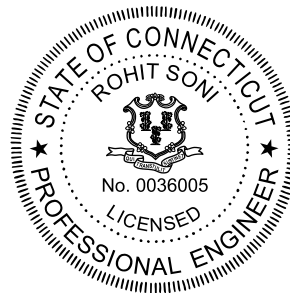


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1) INTRODUCTION

This tower is a 150 ft Monopole tower designed by SUMMIT.

2) ANALYSIS CRITERIA

TIA-222 Revision:	TIA-222-H
Risk Category:	II
Wind Speed:	117 mph
Exposure Category:	C
Topographic Factor:	1
Ice Thickness:	1 in
Wind Speed with Ice:	50 mph
Service Wind Speed:	60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
147.0	150.0	3	commscope	CBC78T-DS-43-2X	7	1-5/8
	148.0	1	raycap	RHSDC-6627-PF-48		
	147.0	6	antel	LPA-80063/6CF w/ Mount Pipe		
		6	commscope	JAHH-65B-R3B w/ Mount Pipe		
		6	kaelus	BSF0020F3V1		
		3	samsung telecommunications	MT6407-77A w/ Mount Pipe		
		3	samsung telecommunications	RFV01U-D1A		
		3	samsung telecommunications	RFV01U-D2A		
		1	tower mounts	Platform Mount [LP 303-1_HR-1]		

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
139.0	140.0	4	andrew	SBNH-1D6565C w/ Mount Pipe	12 1 2 1	1-5/8 3/8 3/4 Conduit
		2	kmw communications	AM-X-CD-16-65-00T-RET w/ Mount Pipe		
		3	powerwave technologies	7770.00 w/ Mount Pipe		
	139.0	3	ericsson	RRUS 11 B12		
		3	ericsson	RRUS 12 B2		
		3	powerwave technologies	TT19-08BP111-001		
		1	raycap	DC6-48-60-18-8F		
		1	tower mounts	Platform Mount [LP 1201-1_KCKR-HR-1]		
		3	communication	DTMABP7819VG12A		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
			components inc.			
125.0	129.0	3	fujitsu	TA08025-B604	1	1-1/2
		3	fujitsu	TA08025-B605		
		1	raycap	RDIDC-9181-PF-48		
	127.0	3	commscope	FFVV-65B-R2 w/ Mount Pipe		
	125.0	1	tower mounts	Sabre_C10801018-32788		

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Reference	Source
4-GEOTECHNICAL REPORTS	2134249	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	1339630	CCISITES
4-TOWER MANUFACTURER DRAWINGS	1339644	CCISITES

3.1) Analysis Method

tnxTower (version 8.1.4.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	150 - 110.75	Pole	TP31.38x24x0.219	1	-13.82	1296.90	38.3	Pass
L2	110.75 - 74.75	Pole	TP37.711x30.19x0.25	2	-19.13	1782.34	61.7	Pass
L3	74.75 - 39.5	Pole	TP43.839x36.318x0.313	3	-26.48	2588.89	60.5	Pass
L4	39.5 - 0	Pole	TP50.64x42.18x0.375	4	-39.28	3674.93	58.1	Pass
							Summary	
						Pole (L2)	61.7	Pass
						Rating =	61.7	Pass

Table 5 - Tower Component Stresses vs. Capacity - LC5

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	49.2	Pass
1	Base Plate	0	42.5	Pass
1	Base Foundation (Structure)	0	36.6	Pass
1	Base Foundation (Soil Interaction)	0	45.7	Pass
Structure Rating (max from all components) =				61.7%

Notes:

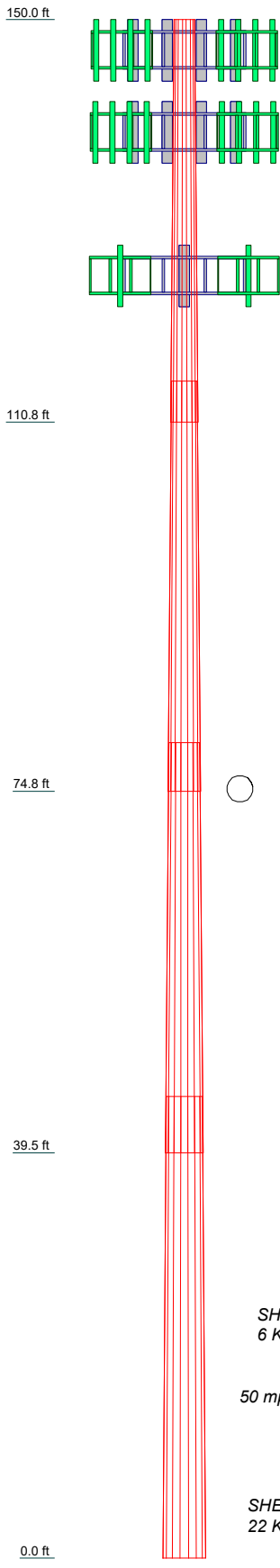
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

4.1) Recommendations

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A
TNXTOWER OUTPUT

Section	1	2	3	4	
Length (ft)	39.25	40.00	40.00	45.00	
Number of Sides	18	18	18	18	
Thickness (in)	0.219	0.250	0.313	0.375	
Socket Length (ft)	4.00	4.75	5.50		
Top Dia (in)	24.000	30.190	36.318	42.180	
Bot Dia (in)	31.380	37.711	43.839	50.640	
Grade	A607-65				
Weight (K)	2.5	3.6	5.4	8.4	19.9

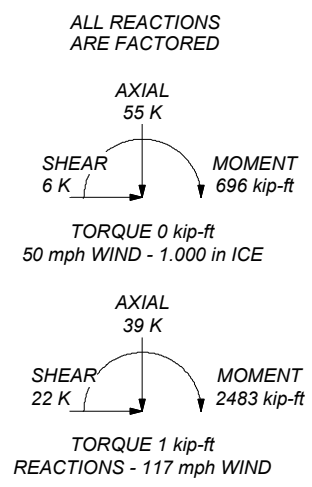


MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A607-65	65 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in New Haven County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-H Standard.
3. Tower designed for a 117 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 50 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. TOWER RATING: 61.7%



Crown Castle
 2000 Corporate Drive
 Canonsburg, PA 15317
 Phone: (724) 416-2000
 FAX:

Job: BU# 873645	Project:	
Client: Crown Castle	Drawn by: SHu	App'd:
Code: TIA-222-H	Date: 10/25/23	Scale: NTS
Path:	Dwg No. E-1	

Tower Input Data

The tower is a monopole.
 This tower is designed using the TIA-222-H standard.
 The following design criteria apply:

- Tower is located in New Haven County, Connecticut.
- Tower base elevation above sea level: 670.00 ft.
- Basic wind speed of 117 mph.
- Risk Category II.
- Exposure Category C.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0.00 ft.
- Nominal ice thickness of 1.000 in.
- Ice thickness is considered to increase with height.
- Ice density of 56 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50 °F.
- Deflections calculated using a wind speed of 60 mph.
- A non-linear (P-delta) analysis was used.
- Pressures are calculated at each section.
- Stress ratio used in pole design is 1.
- Tower analysis based on target reliabilities in accordance with Annex S.
- Load Modification Factors used: $K_{es}(F_w) = 0.95$, $K_{es}(t_i) = 0.85$.
- Maximum demand-capacity ratio is: 1.05.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs
 Consider Moments - Horizontals

Distribute Leg Loads As Uniform
 Assume Legs Pinned

Use ASCE 10 X-Brace Ly Rules
 Calculate Forces in Supporting Bracing Members

- Consider Moments - Diagonals
- Use Moment Magnification
- √ Use Code Stress Ratios
- √ Use Code Safety Factors - Guys
- Escalate Ice
- Always Use Max Kz
- Use Special Wind Profile
- Include Bolts In Member Capacity
- Leg Bolts Are At Top Of Section
- Secondary Horizontal Braces Leg
- Use Diamond Inner Bracing (4 Sided)
- SR Members Have Cut Ends
- SR Members Are Concentric

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
- Use Clear Spans For KL/r
- Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.
- Autocalc Torque Arm Areas
- Add IBC .6D+W Combination
- √ Sort Capacity Reports By Component
- Triangulate Diamond Inner Bracing
- Treat Feed Line Bundles As Cylinder
- Ignore KL/ry For 60 Deg. Angle Legs

- Ignore Redundant Members in FEA
 - SR Leg Bolts Resist Compression
 - All Leg Panels Have Same Allowable
 - Offset Girt At Foundation
 - √ Consider Feed Line Torque
 - Include Angle Block Shear Check
 - Use TIA-222-H Bracing Resist. Exemption
 - Use TIA-222-H Tension Splice Exemption
- Poles
- √ Include Shear-Torsion Interaction
 - Always Use Sub-Critical Flow
 - Use Top Mounted Sockets
 - √ Pole Without Linear Attachments
 - Pole With Shroud Or No Appurtenances
 - Outside and Inside Corner Radii Are Known

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	150.00-110.75	39.25	4.000	18	24.000	31.380	0.219	0.875	A607-65 (65 ksi)

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L2	110.75-74.75	40.00	4.750	18	30.190	37.711	0.250	1.000	A607-65 (65 ksi)
L3	74.75-39.50	40.00	5.500	18	36.318	43.839	0.313	1.250	A607-65 (65 ksi)
L4	39.50-0.00	45.00		18	42.180	50.640	0.375	1.500	A607-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	I in ⁴	r in	C in	I/C in ³	J in ⁴	I/Q in ²	w in	w/t
L1	24.336	16.512	1179.768	8.442	12.192	96.766	2361.088	8.257	3.839	17.55
	31.830	21.636	2654.221	11.062	15.941	166.502	5311.934	10.820	5.138	23.487
L2	31.381	23.758	2690.649	10.629	15.337	175.438	5384.839	11.881	4.874	19.494
	38.254	29.725	5270.144	13.299	19.157	275.100	10547.223	14.865	6.197	24.789
L3	37.737	35.713	5849.225	12.782	18.450	317.040	11706.148	17.860	5.842	18.694
	44.467	43.173	10333.695	15.452	22.270	464.014	20680.987	21.591	7.166	22.93
L4	43.823	49.758	10986.408	14.841	21.427	512.728	21987.273	24.884	6.764	18.036
	51.363	59.828	19097.332	17.844	25.725	742.361	38219.793	29.920	8.253	22.007

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _r	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
L1 150.00- 110.75				1	1	1			
L2 110.75- 74.75				1	1	1			
L3 74.75- 39.50				1	1	1			
L4 39.50-0.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Exclude From Torque Calculation	Componen t Type	Placement ft	Total Number	Number Per Row	Start/En d Position	Width or Diamete r in	Perimete r in	Weight plf
5/8 rod/step	C	No	Surface Ar (CaAa)	150.00 - 0.00	1	1	0.000 0.000	0.200		0.274
Safety Line 3/8	C	No	Surface Ar (CaAa)	150.00 - 0.00	1	1	0.000 0.000	0.375		0.220

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Componen t Type	Placement ft	Total Number	C _A A _A ft ² /ft	Weight plf
147								
AL7-50(1 5/8")	A	No	No	Inside Pole	147.00 - 0.00	6	No Ice 1/2" Ice 1" Ice	0.520 0.520 0.520
HB158-U12S24- XXX-LI(1-5/8)	A	No	No	Inside Pole	147.00 - 0.00	1	No Ice 1/2" Ice	3.200 3.200

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		C _{AA} ft ² /ft	Weight plf
							1" Ice	0.00	3.200
139									
LCF158-50JA-A0(1-5/8)	B	No	No	Inside Pole	139.00 - 0.00	12	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00	0.800 0.800 0.800
FB-L98B-034-XXX(3/8)	B	No	No	Inside Pole	139.00 - 0.00	1	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00	0.057 0.057 0.057
WR-VG86ST-BRD(3/4)	B	No	No	Inside Pole	139.00 - 0.00	2	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00	0.584 0.584 0.584
2" Rigid Conduit	B	No	No	Inside Pole	139.00 - 0.00	1	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00	2.800 2.800 2.800
**									
CU12PSM9P6XXX (1-1/2)	A	No	No	Inside Pole	125.00 - 0.00	1	No Ice 1/2" Ice 1" Ice	0.00 0.00 0.00	2.350 2.350 2.350

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L1	150.00-110.75	A	0.000	0.000	0.000	0.000	0.26
		B	0.000	0.000	0.000	0.000	0.38
		C	0.000	0.000	2.257	0.000	0.02
L2	110.75-74.75	A	0.000	0.000	0.000	0.000	0.31
		B	0.000	0.000	0.000	0.000	0.49
		C	0.000	0.000	2.070	0.000	0.02
L3	74.75-39.50	A	0.000	0.000	0.000	0.000	0.31
		B	0.000	0.000	0.000	0.000	0.48
		C	0.000	0.000	2.027	0.000	0.02
L4	39.50-0.00	A	0.000	0.000	0.000	0.000	0.34
		B	0.000	0.000	0.000	0.000	0.54
		C	0.000	0.000	2.271	0.000	0.02

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L1	150.00-110.75	A	0.975	0.000	0.000	0.000	0.000	0.26
		B		0.000	0.000	0.000	0.000	0.38
		C		0.000	0.000	17.559	0.000	0.14
L2	110.75-74.75	A	0.942	0.000	0.000	0.000	0.000	0.31
		B		0.000	0.000	0.000	0.000	0.49
		C		0.000	0.000	16.105	0.000	0.13
L3	74.75-39.50	A	0.898	0.000	0.000	0.000	0.000	0.31
		B		0.000	0.000	0.000	0.000	0.48
		C		0.000	0.000	15.311	0.000	0.12
L4	39.50-0.00	A	0.808	0.000	0.000	0.000	0.000	0.34
		B		0.000	0.000	0.000	0.000	0.54
		C		0.000	0.000	16.454	0.000	0.12

Feed Line Center of Pressure

Section	Elevation	CP _x	CP _z	CP _x Ice	CP _z Ice
	ft	in	in	in	in
L1	150.00-110.75	0.000	0.526	0.000	1.956
L2	110.75-74.75	0.000	0.528	0.000	2.031
L3	74.75-39.50	0.000	0.530	0.000	2.030
L4	39.50-0.00	0.000	0.531	0.000	1.996

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L1	1	5/8 rod/step	110.75 - 150.00	1.0000	1.0000
L1	2	Safety Line 3/8	110.75 - 150.00	1.0000	1.0000
L2	1	5/8 rod/step	74.75 - 110.75	1.0000	1.0000
L2	2	Safety Line 3/8	74.75 - 110.75	1.0000	1.0000
L3	1	5/8 rod/step	39.50 - 74.75	1.0000	1.0000
L3	2	Safety Line 3/8	39.50 - 74.75	1.0000	1.0000
L4	1	5/8 rod/step	0.00 - 39.50	1.0000	1.0000
L4	2	Safety Line 3/8	0.00 - 39.50	1.0000	1.0000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft
*** Lighting Rod 1/2" x 2' ***	C	None		0.000	150.00
(2) LPA-80063/6CF w/ Mount Pipe	A	From Leg	4.00 0.000 0.000	0.000	147.00
(2) LPA-80063/6CF w/ Mount Pipe	B	From Leg	4.00 0.000 0.000	0.000	147.00
(2) LPA-80063/6CF w/ Mount Pipe	C	From Leg	4.00 0.000 0.000	0.000	147.00
(2) JAHH-65B-R3B w/ Mount Pipe	A	From Leg	4.00 0.000 0.000	0.000	147.00
(2) JAHH-65B-R3B w/ Mount Pipe	B	From Leg	4.00 0.000 0.000	0.000	147.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft	Azimuth Adjustment °	Placement ft
			0.000		
(2) JAHH-65B-R3B w/ Mount Pipe	C	From Leg	0.000 4.00	0.000	147.00
MT6407-77A w/ Mount Pipe	A	From Leg	0.000 4.00	0.000	147.00
MT6407-77A w/ Mount Pipe	B	From Leg	0.000 4.00	0.000	147.00
MT6407-77A w/ Mount Pipe	C	From Leg	0.000 4.00	0.000	147.00
CBC78T-DS-43-2X	A	From Leg	0.000 4.00	0.000	147.00
CBC78T-DS-43-2X	B	From Leg	0.000 3.000 4.00	0.000	147.00
CBC78T-DS-43-2X	C	From Leg	0.000 3.000 4.00	0.000	147.00
RFV01U-D1A	A	From Leg	0.000 3.000 4.00	0.000	147.00
RFV01U-D1A	B	From Leg	0.000 0.000 4.00	0.000	147.00
RFV01U-D1A	C	From Leg	0.000 0.000 4.00	0.000	147.00
RFV01U-D2A	A	From Leg	0.000 0.000 4.00	0.000	147.00
RFV01U-D2A	B	From Leg	0.000 0.000 4.00	0.000	147.00
RFV01U-D2A	C	From Leg	0.000 0.000 4.00	0.000	147.00
(2) BSF0020F3V1	A	From Leg	0.000 0.000 4.00	0.000	147.00
(2) BSF0020F3V1	B	From Leg	0.000 0.000 4.00	0.000	147.00
(2) BSF0020F3V1	C	From Leg	0.000 0.000 4.00	0.000	147.00
RHSDC-6627-PF-48	B	From Leg	0.000 0.000 4.00	0.000	147.00
Platform Mount [LP 303-1_HR-1] ***	C	None	1.000	0.000	147.00
(2) SBNH-1D6565C w/ Mount Pipe	A	From Leg	4.00 0.000 1.000	0.000	139.00
(2) SBNH-1D6565C w/ Mount Pipe	B	From Leg	4.00 0.000 1.000	0.000	139.00
7770.00 w/ Mount Pipe	A	From Leg	4.00 0.000 1.000	0.000	139.00
7770.00 w/ Mount Pipe	B	From Leg	4.00 0.000 1.000	0.000	139.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft
			0.000		
			1.000		
7770.00 w/ Mount Pipe	C	From Leg	4.00	0.000	139.00
			0.000		
			1.000		
(2) AM-X-CD-16-65-00T-RET w/ Mount Pipe	C	From Leg	4.00	0.000	139.00
			0.000		
			1.000		
DTMABP7819VG12A	A	From Leg	4.00	0.000	139.00
			0.000		
			-3.000		
DTMABP7819VG12A	B	From Leg	4.00	0.000	139.00
			0.000		
			-3.000		
DTMABP7819VG12A	C	From Leg	4.00	0.000	139.00
			0.000		
			-3.000		
RRUS 11 B12	A	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
RRUS 11 B12	B	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
RRUS 11 B12	C	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
RRUS 12 B2	A	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
RRUS 12 B2	B	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
RRUS 12 B2	C	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
TT19-08BP111-001	A	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
TT19-08BP111-001	B	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
TT19-08BP111-001	C	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
DC6-48-60-18-8F	C	From Leg	4.00	0.000	139.00
			0.000		
			0.000		
Platform Mount [LP 1201-1_KCKR-HR-1] ***	C	None		0.000	139.00
TA08025-B604	A	From Leg	4.00	0.000	125.00
			0.000		
			4.000		
TA08025-B604	B	From Leg	4.00	0.000	125.00
			0.000		
			4.000		
TA08025-B604	C	From Leg	4.00	0.000	125.00
			0.000		
			4.000		
TA08025-B605	A	From Leg	4.00	0.000	125.00
			0.000		
			4.000		
TA08025-B605	B	From Leg	4.00	0.000	125.00
			0.000		
			4.000		
TA08025-B605	C	From Leg	4.00	0.000	125.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft
			0.000		
FFVV-65B-R2 w/ Mount Pipe	A	From Leg	4.000	0.000	125.00
			4.000		
FFVV-65B-R2 w/ Mount Pipe	B	From Leg	2.000	0.000	125.00
			4.000		
FFVV-65B-R2 w/ Mount Pipe	C	From Leg	0.000	0.000	125.00
			2.000		
RDIDC-9181-PF-48	C	From Leg	4.000	0.000	125.00
			0.000		
(2) 8' x 2" Mount Pipe	A	From Leg	4.000	0.000	125.00
			4.000		
(2) 8' x 2" Mount Pipe	B	From Leg	0.000	0.000	125.00
			4.000		
(2) 8' x 2" Mount Pipe	C	From Leg	0.000	0.000	125.00
			4.000		
Sabre_C10801018-32788	C	None	0.000	0.000	125.00

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp

Comb. No.	Description
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	150 - 110.75	Pole	Max Tension	21	0.00	-0.00	0.00
			Max. Compression	26	-24.77	-0.49	-0.45
			Max. Mx	8	-13.82	-340.09	-0.28
			Max. My	14	-13.83	-0.14	-339.08
			Max. Vy	8	14.53	-340.09	-0.28
			Max. Vx	14	14.48	-0.14	-339.08
			Max. Torque	3			-0.75
L2	110.75 - 74.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-31.54	-0.49	-0.63
			Max. Mx	8	-19.13	-898.21	-0.10
			Max. My	14	-19.14	0.06	-895.61
			Max. Vy	8	17.11	-898.21	-0.10
			Max. Vx	14	17.07	0.06	-895.61
			Max. Torque	17			0.61
L3	74.75 - 39.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-40.42	-0.49	-0.83
			Max. Mx	8	-26.48	-1533.15	0.08
			Max. My	14	-26.48	0.28	-1529.02
			Max. Vy	8	19.64	-1533.15	0.08
			Max. Vx	14	19.59	0.28	-1529.02
			Max. Torque	17			0.61
L4	39.5 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-55.19	-0.49	-1.11
			Max. Mx	8	-39.28	-2483.13	0.31
			Max. My	14	-39.28	0.56	-2477.07
			Max. Vy	8	22.40	-2483.13	0.31
			Max. Vx	14	22.36	0.56	-2477.07
			Max. Torque	17			0.61

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	30	55.19	-6.41	0.00

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Max. H _x	21	29.47	22.38	-0.01
	Max. H _z	3	29.47	-0.01	22.33
	Max. M _x	2	2476.45	-0.01	22.33
	Max. M _z	8	2483.13	-22.38	0.01
	Max. Torsion	17	0.61	11.19	-19.34
	Min. Vert	25	29.47	11.18	19.34
	Min. H _x	9	29.47	-22.38	0.01
	Min. H _z	15	29.47	0.01	-22.33
	Min. M _x	14	-2477.07	0.01	-22.33
	Min. M _z	20	-2483.01	22.38	-0.01
	Min. Torsion	5	-0.60	-11.19	19.34

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	32.75	0.00	0.00	0.24	-0.05	0.00
1.2 Dead+1.0 Wind 0 deg - No Ice	39.29	0.01	-22.33	-2476.45	-0.67	0.55
0.9 Dead+1.0 Wind 0 deg - No Ice	29.47	0.01	-22.33	-2443.61	-0.65	0.56
1.2 Dead+1.0 Wind 30 deg - No Ice	39.29	11.19	-19.34	-2144.94	-1242.13	0.60
0.9 Dead+1.0 Wind 30 deg - No Ice	29.47	11.19	-19.34	-2116.51	-1225.61	0.60
1.2 Dead+1.0 Wind 60 deg - No Ice	39.29	19.38	-11.17	-1238.60	-2150.77	0.49
0.9 Dead+1.0 Wind 60 deg - No Ice	29.47	19.38	-11.17	-1222.22	-2122.17	0.49
1.2 Dead+1.0 Wind 90 deg - No Ice	39.29	22.38	-0.01	-0.31	-2483.13	0.25
0.9 Dead+1.0 Wind 90 deg - No Ice	29.47	22.38	-0.01	-0.38	-2450.10	0.25
1.2 Dead+1.0 Wind 120 deg - No Ice	39.29	19.38	11.16	1238.15	-2150.16	-0.06
0.9 Dead+1.0 Wind 120 deg - No Ice	29.47	19.38	11.16	1221.62	-2121.56	-0.06
1.2 Dead+1.0 Wind 150 deg - No Ice	39.29	11.18	19.34	2144.94	-1241.07	-0.36
0.9 Dead+1.0 Wind 150 deg - No Ice	29.47	11.18	19.34	2116.35	-1224.55	-0.36
1.2 Dead+1.0 Wind 180 deg - No Ice	39.29	-0.01	22.33	2477.07	0.56	-0.56
0.9 Dead+1.0 Wind 180 deg - No Ice	29.47	-0.01	22.33	2444.06	0.57	-0.56
1.2 Dead+1.0 Wind 210 deg - No Ice	39.29	-11.19	19.34	2145.55	1242.02	-0.61
0.9 Dead+1.0 Wind 210 deg - No Ice	29.47	-11.19	19.34	2116.96	1225.52	-0.61
1.2 Dead+1.0 Wind 240 deg - No Ice	39.29	-19.38	11.17	1239.21	2150.65	-0.49
0.9 Dead+1.0 Wind 240 deg - No Ice	29.47	-19.38	11.17	1222.67	2122.08	-0.49
1.2 Dead+1.0 Wind 270 deg - No Ice	39.29	-22.38	0.01	0.92	2483.01	-0.25
0.9 Dead+1.0 Wind 270 deg - No Ice	29.47	-22.38	0.01	0.83	2450.01	-0.25
1.2 Dead+1.0 Wind 300 deg - No Ice	39.29	-19.38	-11.16	-1237.53	2150.04	0.07
0.9 Dead+1.0 Wind 300 deg - No Ice	29.47	-19.38	-11.16	-1221.16	2121.47	0.06
1.2 Dead+1.0 Wind 330 deg - No Ice	39.29	-11.18	-19.34	-2144.32	1240.95	0.36
0.9 Dead+1.0 Wind 330 deg - No Ice	29.47	-11.18	-19.34	-2115.90	1224.47	0.36

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
- No Ice						
1.2 Dead+1.0 Ice+1.0 Temp	55.19	0.00	0.00	1.11	-0.49	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	55.19	0.00	-6.40	-692.90	-0.70	0.10
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	55.19	3.21	-5.54	-599.96	-348.38	0.12
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	55.19	5.55	-3.20	-345.94	-602.86	0.10
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	55.19	6.41	-0.00	1.10	-695.97	0.06
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	55.19	5.55	3.20	348.18	-602.74	0.00
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	55.19	3.20	5.54	602.30	-348.16	-0.06
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	55.19	-0.00	6.40	695.36	-0.45	-0.10
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	55.19	-3.21	5.54	602.42	347.23	-0.12
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	55.19	-5.55	3.20	348.40	601.71	-0.10
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	55.19	-6.41	0.00	1.36	694.81	-0.06
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	55.19	-5.55	-3.20	-345.72	601.59	-0.00
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	55.19	-3.20	-5.54	-599.84	347.01	0.06
Dead+Wind 0 deg - Service	32.75	0.00	-5.53	-608.62	-0.20	0.14
Dead+Wind 30 deg - Service	32.75	2.77	-4.79	-527.12	-305.40	0.15
Dead+Wind 60 deg - Service	32.75	4.80	-2.77	-304.31	-528.77	0.12
Dead+Wind 90 deg - Service	32.75	5.54	-0.00	0.10	-610.48	0.06
Dead+Wind 120 deg - Service	32.75	4.80	2.77	304.56	-528.62	-0.01
Dead+Wind 150 deg - Service	32.75	2.77	4.79	527.48	-305.13	-0.09
Dead+Wind 180 deg - Service	32.75	-0.00	5.53	609.13	0.10	-0.14
Dead+Wind 210 deg - Service	32.75	-2.77	4.79	527.63	305.29	-0.15
Dead+Wind 240 deg - Service	32.75	-4.80	2.77	304.82	528.67	-0.12
Dead+Wind 270 deg - Service	32.75	-5.54	0.00	0.41	610.37	-0.06
Dead+Wind 300 deg - Service	32.75	-4.80	-2.77	-304.05	528.52	0.01
Dead+Wind 330 deg - Service	32.75	-2.77	-4.79	-526.97	305.03	0.09

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-32.75	0.00	0.00	32.75	0.00	0.000%
2	0.01	-39.29	-22.33	-0.01	39.29	22.33	0.000%
3	0.01	-29.47	-22.33	-0.01	29.47	22.33	0.000%
4	11.19	-39.29	-19.34	-11.19	39.29	19.34	0.000%
5	11.19	-29.47	-19.34	-11.19	29.47	19.34	0.000%
6	19.38	-39.29	-11.17	-19.38	39.29	11.17	0.000%
7	19.38	-29.47	-11.17	-19.38	29.47	11.17	0.000%
8	22.38	-39.29	-0.01	-22.38	39.29	0.01	0.000%
9	22.38	-29.47	-0.01	-22.38	29.47	0.01	0.000%
10	19.38	-39.29	11.16	-19.38	39.29	-11.16	0.000%
11	19.38	-29.47	11.16	-19.38	29.47	-11.16	0.000%
12	11.18	-39.29	19.34	-11.18	39.29	-19.34	0.000%
13	11.18	-29.47	19.34	-11.18	29.47	-19.34	0.000%
14	-0.01	-39.29	22.33	0.01	39.29	-22.33	0.000%

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
15	-0.01	-29.47	22.33	0.01	29.47	-22.33	0.000%
16	-11.19	-39.29	19.34	11.19	39.29	-19.34	0.000%
17	-11.19	-29.47	19.34	11.19	29.47	-19.34	0.000%
18	-19.38	-39.29	11.17	19.38	39.29	-11.17	0.000%
19	-19.38	-29.47	11.17	19.38	29.47	-11.17	0.000%
20	-22.38	-39.29	0.01	22.38	39.29	-0.01	0.000%
21	-22.38	-29.47	0.01	22.38	29.47	-0.01	0.000%
22	-19.38	-39.29	-11.16	19.38	39.29	11.16	0.000%
23	-19.38	-29.47	-11.16	19.38	29.47	11.16	0.000%
24	-11.18	-39.29	-19.34	11.18	39.29	19.34	0.000%
25	-11.18	-29.47	-19.34	11.18	29.47	19.34	0.000%
26	0.00	-55.19	0.00	0.00	55.19	0.00	0.000%
27	0.00	-55.19	-6.40	-0.00	55.19	6.40	0.000%
28	3.21	-55.19	-5.54	-3.21	55.19	5.54	0.000%
29	5.55	-55.19	-3.20	-5.55	55.19	3.20	0.000%
30	6.41	-55.19	-0.00	-6.41	55.19	0.00	0.000%
31	5.55	-55.19	3.20	-5.55	55.19	-3.20	0.000%
32	3.20	-55.19	5.54	-3.20	55.19	-5.54	0.000%
33	-0.00	-55.19	6.40	0.00	55.19	-6.40	0.000%
34	-3.21	-55.19	5.54	3.21	55.19	-5.54	0.000%
35	-5.55	-55.19	3.20	5.55	55.19	-3.20	0.000%
36	-6.41	-55.19	0.00	6.41	55.19	-0.00	0.000%
37	-5.55	-55.19	-3.20	5.55	55.19	3.20	0.000%
38	-3.20	-55.19	-5.54	3.20	55.19	5.54	0.000%
39	0.00	-32.75	-5.53	-0.00	32.75	5.53	0.000%
40	2.77	-32.75	-4.79	-2.77	32.75	4.79	0.000%
41	4.80	-32.75	-2.77	-4.80	32.75	2.77	0.000%
42	5.54	-32.75	-0.00	-5.54	32.75	0.00	0.000%
43	4.80	-32.75	2.77	-4.80	32.75	-2.77	0.000%
44	2.77	-32.75	4.79	-2.77	32.75	-4.79	0.000%
45	-0.00	-32.75	5.53	0.00	32.75	-5.53	0.000%
46	-2.77	-32.75	4.79	2.77	32.75	-4.79	0.000%
47	-4.80	-32.75	2.77	4.80	32.75	-2.77	0.000%
48	-5.54	-32.75	0.00	5.54	32.75	-0.00	0.000%
49	-4.80	-32.75	-2.77	4.80	32.75	2.77	0.000%
50	-2.77	-32.75	-4.79	2.77	32.75	4.79	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	5	0.00000001	0.00006720
3	Yes	4	0.00000001	0.00072721
4	Yes	6	0.00000001	0.00015779
5	Yes	6	0.00000001	0.00005305
6	Yes	6	0.00000001	0.00015375
7	Yes	6	0.00000001	0.00005152
8	Yes	5	0.00000001	0.00003545
9	Yes	4	0.00000001	0.00042759
10	Yes	6	0.00000001	0.00015530
11	Yes	6	0.00000001	0.00005209
12	Yes	6	0.00000001	0.00015686
13	Yes	6	0.00000001	0.00005269
14	Yes	5	0.00000001	0.00006522
15	Yes	4	0.00000001	0.00070852
16	Yes	6	0.00000001	0.00015328
17	Yes	6	0.00000001	0.00005135
18	Yes	6	0.00000001	0.00015755
19	Yes	6	0.00000001	0.00005293
20	Yes	5	0.00000001	0.00003707
21	Yes	4	0.00000001	0.00044132
22	Yes	6	0.00000001	0.00015573
23	Yes	6	0.00000001	0.00005227
24	Yes	6	0.00000001	0.00015394

25	Yes	6	0.00000001	0.00005162
26	Yes	4	0.00000001	0.00000001
27	Yes	5	0.00000001	0.00052297
28	Yes	5	0.00000001	0.00068095
29	Yes	5	0.00000001	0.00067790
30	Yes	5	0.00000001	0.00052632
31	Yes	5	0.00000001	0.00068303
32	Yes	5	0.00000001	0.00068345
33	Yes	5	0.00000001	0.00052562
34	Yes	5	0.00000001	0.00067809
35	Yes	5	0.00000001	0.00068205
36	Yes	5	0.00000001	0.00052428
37	Yes	5	0.00000001	0.00067633
38	Yes	5	0.00000001	0.00067502
39	Yes	4	0.00000001	0.00011289
40	Yes	4	0.00000001	0.00073329
41	Yes	4	0.00000001	0.00067867
42	Yes	4	0.00000001	0.00009893
43	Yes	4	0.00000001	0.00070038
44	Yes	4	0.00000001	0.00072083
45	Yes	4	0.00000001	0.00011278
46	Yes	4	0.00000001	0.00067486
47	Yes	4	0.00000001	0.00072961
48	Yes	4	0.00000001	0.00009904
49	Yes	4	0.00000001	0.00070353
50	Yes	4	0.00000001	0.00068301

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	150 - 110.75	22.588	42	1.295	0.002
L2	114.75 - 74.75	13.378	42	1.138	0.001
L3	79.5 - 39.5	6.239	42	0.758	0.000
L4	45 - 0	1.970	42	0.401	0.000

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
150.00	Lighting Rod 1/2" x 2'	42	22.588	1.295	0.002	43842
147.00	(2) LPA-80063/6CF w/ Mount Pipe	42	21.771	1.287	0.002	43842
139.00	(2) SBNH-1D6565C w/ Mount Pipe	42	19.602	1.263	0.002	19928
125.00	TA08025-B604	42	15.916	1.204	0.001	8768

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	150 - 110.75	92.006	8	5.282	0.008
L2	114.75 - 74.75	54.496	8	4.639	0.004
L3	79.5 - 39.5	25.411	8	3.091	0.002
L4	45 - 0	8.020	8	1.633	0.001

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
150.00	Lighting Rod 1/2" x 2'	8	92.006	5.282	0.008	10918
147.00	(2) LPA-80063/6CF w/ Mount Pipe	8	88.676	5.248	0.008	10918
139.00	(2) SBNH-1D6565C w/ Mount Pipe	8	79.843	5.150	0.006	4962
125.00	TA08025-B604	8	64.831	4.912	0.005	2180

Compression Checks

Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	KI/r	A in ²	P _u K	φP _n K	Ratio P _u / φP _n
L1	150 - 110.75 (1)	TP31.38x24x0.219	39.25	0.00	0.0	21.114	-13.82	1235.14	0.011
L2	110.75 - 74.75 (2)	TP37.711x30.19x0.25	40.00	0.00	0.0	29.017	-19.13	1697.47	0.011
L3	74.75 - 39.5 (3)	TP43.839x36.318x0.313	40.00	0.00	0.0	42.147	-26.48	2465.61	0.011
L4	39.5 - 0 (4)	TP50.64x42.18x0.375	45.00	0.00	0.0	59.828	-39.28	3499.93	0.011

Pole Bending Design Data

Section No.	Elevation ft	Size	M _{ux} kip-ft	φM _{nx} kip-ft	Ratio M _{ux} / φM _{nx}	M _{uy} kip-ft	φM _{ny} kip-ft	Ratio M _{uy} / φM _{ny}
L1	150 - 110.75 (1)	TP31.38x24x0.219	340.09	873.35	0.389	0.00	873.35	0.000
L2	110.75 - 74.75 (2)	TP37.711x30.19x0.25	898.22	1414.33	0.635	0.00	1414.33	0.000
L3	74.75 - 39.5 (3)	TP43.839x36.318x0.313	1533.15	2456.62	0.624	0.00	2456.62	0.000
L4	39.5 - 0 (4)	TP50.64x42.18x0.375	2483.13	4146.91	0.599	0.00	4146.91	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V _u K	φV _n K	Ratio V _u / φV _n	Actual T _u kip-ft	φT _n kip-ft	Ratio T _u / φT _n
L1	150 - 110.75 (1)	TP31.38x24x0.219	14.53	370.54	0.039	0.25	986.78	0.000
L2	110.75 - 74.75 (2)	TP37.711x30.19x0.25	17.11	509.24	0.034	0.25	1630.82	0.000
L3	74.75 - 39.5 (3)	TP43.839x36.318x0.313	19.64	739.68	0.027	0.25	2752.55	0.000
L4	39.5 - 0 (4)	TP50.64x42.18x0.375	22.40	1049.98	0.021	0.25	4621.97	0.000

Pole Interaction Design Data

Section No.	Elevation ft	Ratio P_u ϕP_n	Ratio M_{ux} ϕM_{nx}	Ratio M_{uy} ϕM_{ny}	Ratio V_u ϕV_n	Ratio T_u ϕT_n	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	150 - 110.75 (1)	0.011	0.389	0.000	0.039	0.000	0.402	1.050	4.8.2
L2	110.75 - 74.75 (2)	0.011	0.635	0.000	0.034	0.000	0.647	1.050	4.8.2
L3	74.75 - 39.5 (3)	0.011	0.624	0.000	0.027	0.000	0.636	1.050	4.8.2
L4	39.5 - 0 (4)	0.011	0.599	0.000	0.021	0.000	0.610	1.050	4.8.2

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail	
L1	150 - 110.75	Pole	TP31.38x24x0.219	1	-13.82	1296.90	38.3	Pass	
L2	110.75 - 74.75	Pole	TP37.711x30.19x0.25	2	-19.13	1782.34	61.7	Pass	
L3	74.75 - 39.5	Pole	TP43.839x36.318x0.313	3	-26.48	2588.89	60.5	Pass	
L4	39.5 - 0	Pole	TP50.64x42.18x0.375	4	-39.28	3674.93	58.1	Pass	
							Summary		
							Pole (L2)	61.7	Pass
							RATING =	61.7	Pass

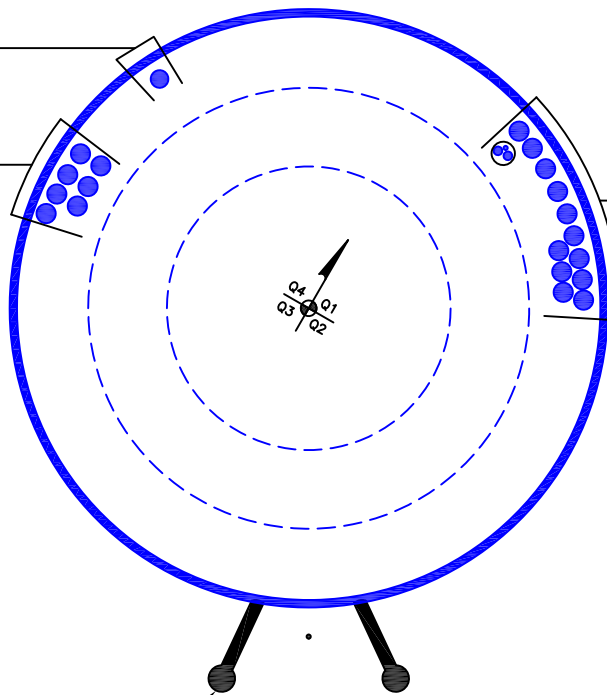
APPENDIX B
BASE LEVEL DRAWING



(OTHER CONSIDERED EQUIPMENT)
(1) 1-1/2" TO 125 FT LEVEL

(PROPOSED EQUIPMENT CONFIGURATION)
(7) 1-5/8" TO 147 FT LEVEL

(OTHER CONSIDERED EQUIPMENT-IN CONDUIT)
(1) 3/8" TO 139 FT LEVEL
(2) 3/4" TO 139 FT LEVEL
(OTHER CONSIDERED EQUIPMENT)
(12) 1-5/8" TO 139 FT LEVEL



CLIMBING PEGS
W/ SAFETY CLIMB

APPENDIX C
ADDITIONAL CALCULATIONS

Monopole Base Plate Connection

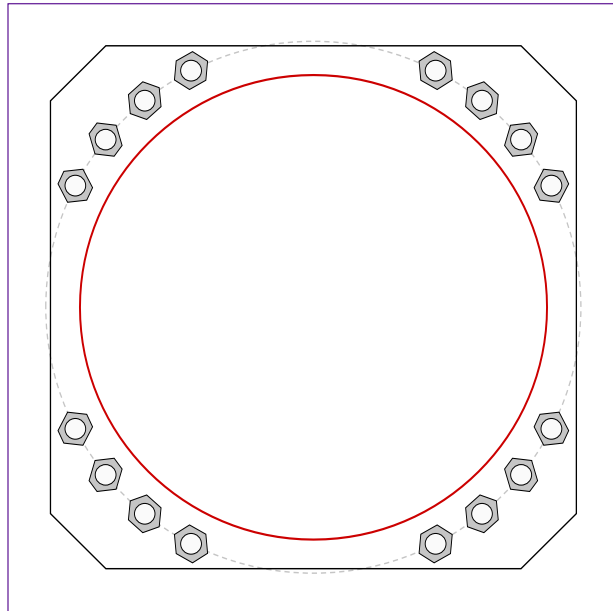


Site Info	
BU #	873645
Site Name	Oxford
Order #	658829 Rev.0

Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	No
l_{ar} (in)	0.75

Applied Loads	
Moment (kip-ft)	2483.13
Axial Force (kips)	39.28
Shear Force (kips)	22.40

*TIA-222-H Section 15.5 Applied



Connection Properties	Analysis Results
-----------------------	------------------

Anchor Rod Data
 (16) 2-1/4" ϕ bolts (A615-75 N; $F_y=75$ ksi, $F_u=100$ ksi) on 58" BC
 Anchor Spacing: 6 in

Base Plate Data
 57" W x 3" Plate (A572-55; $F_y=55$ ksi, $F_u=70$ ksi); Clip: 6 in

Stiffener Data
 N/A

Pole Data
 50.64" x 0.375" 18-sided pole (A607-65; $F_y=65$ ksi, $F_u=80$ ksi)

Anchor Rod Summary (units of kips, kip-in)

$P_{u,t} = 125.9$	$\phi P_{n,t} = 243.75$	Stress Rating
$V_u = 1.4$	$\phi V_n = 149.1$	49.2%
$M_u = n/a$	$\phi M_n = n/a$	Pass

Base Plate Summary

Max Stress (ksi):	22.07	(Flexural)
Allowable Stress (ksi):	49.5	
Stress Rating:	42.5%	Pass

Pier and Pad Foundation



BU # :	873645
Site Name:	Oxford
App. Number:	65882 Rev.0

TIA-222 Revision:	H
Tower Type:	Monopole

Top & Bot. Pad Rein. Different?:	<input type="checkbox"/>
Block Foundation?:	<input type="checkbox"/>
Rectangular Pad?:	<input type="checkbox"/>

Superstructure Analysis Reactions		
Compression, P_{comp} :	39.29	kips
Base Shear, V_{u_comp} :	22.38	kips
Moment, M_u :	2483.13	ft-kips
Tower Height, H :	150	ft
BP Dist. Above Fdn, bp_{dist} :	3	in

Foundation Analysis Checks				
	Capacity	Demand	Rating*	Check
<i>Lateral (Sliding) (kips)</i>	227.35	22.38	9.4%	Pass
<i>Bearing Pressure (ksf)</i>	9.00	2.09	22.1%	Pass
<i>Overturing (kip*ft)</i>	5816.43	2656.58	45.7%	Pass
<i>Pier Flexure (Comp.) (kip*ft)</i>	6720.89	2583.84	36.6%	Pass
<i>Pier Compression (kip)</i>	23390.64	78.98	0.3%	Pass
<i>Pad Flexure (kip*ft)</i>	4415.72	836.62	18.0%	Pass
<i>Pad Shear - 1-way (kips)</i>	720.43	137.92	18.2%	Pass
<i>Pad Shear - 2-way (Comp) (ksi)</i>	0.164	0.027	15.6%	Pass
<i>Flexural 2-way (Comp) (kip*ft)</i>	5938.20	1550.30	24.9%	Pass

Pier Properties		
Pier Shape:	Square	
Pier Diameter, $dpier$:	7	ft
Ext. Above Grade, E :	0.5	ft
Pier Rebar Size, Sc :	11	
Pier Rebar Quantity, mc :	28	
Pier Tie/Spiral Size, St :	4	
Pier Tie/Spiral Quantity, mt :	7	
Pier Reinforcement Type:	Tie	
Pier Clear Cover, cc_{pier} :	3	in

*Rating per TIA-222-H Section 15.5

Structural Rating*:	36.6%
Soil Rating*:	45.7%

Pad Properties		
Depth, D :	7	ft
Pad Width, W_1 :	23.5	ft
Pad Thickness, T :	3	ft
Pad Rebar Size (Bottom dir. 2), Sp_2 :	10	
Pad Rebar Quantity (Bottom dir. 2), mp_2 :	26	
Pad Clear Cover, cc_{pad} :	3	in

Material Properties		
Rebar Grade, F_y :	60	ksi
Concrete Compressive Strength, F'_c :	3	ksi
Dry Concrete Density, δ_c :	150	pcf

Soil Properties		
Total Soil Unit Weight, γ :	125	pcf
Ultimate Gross Bearing, Q_{ult} :	12.000	ksf
Cohesion, C_u :	0.000	ksf
Friction Angle, ϕ :	30	degrees
SPT Blow Count, N_{blows} :		
Base Friction, μ :	0.3	
Neglected Depth, N :	3.50	ft
Foundation Bearing on Rock?	No	
Groundwater Depth, gw :	None	ft

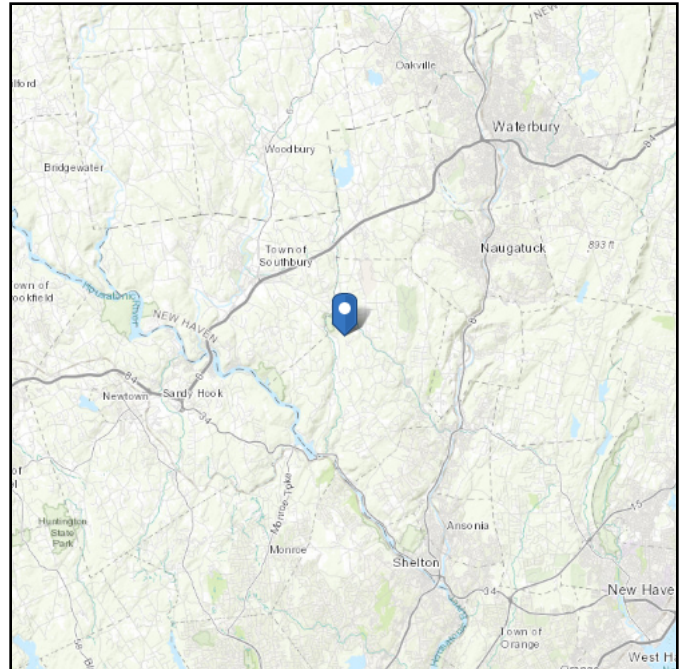
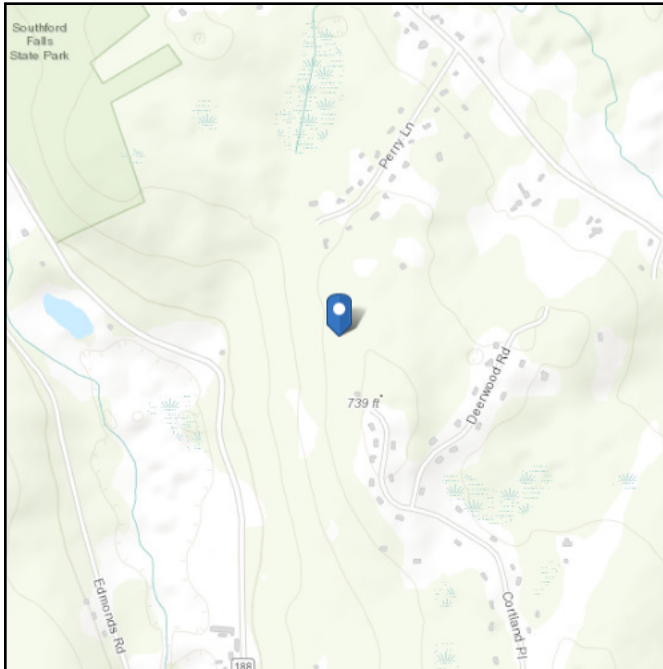
<-- Toggle between Gross and Net

ASCE 7 Hazards Report

Address:
No Address at This Location

Standard: ASCE/SEI 7-16
Risk Category: II
Soil Class: D - Default (see Section 11.4.3)

Latitude: 41.447086
Longitude: -73.15231
Elevation: 669.86 ft (NAVD 88)



Wind

Results:

Wind Speed	117 Vmph
10-year MRI	75 Vmph
25-year MRI	84 Vmph
50-year MRI	90 Vmph
100-year MRI	97 Vmph

Data Source: ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2

Date Accessed: Fri Dec 16 2022

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

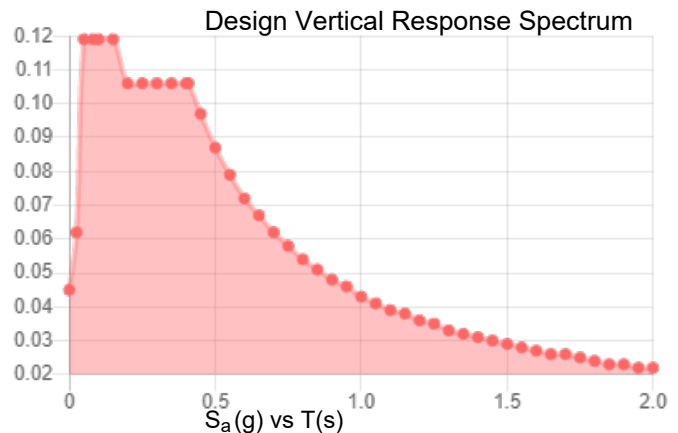
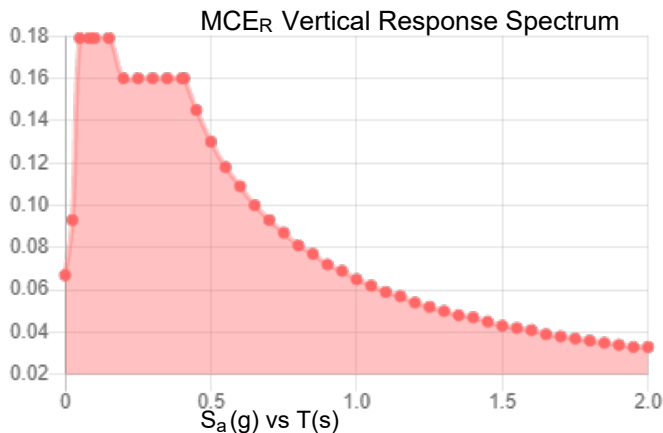
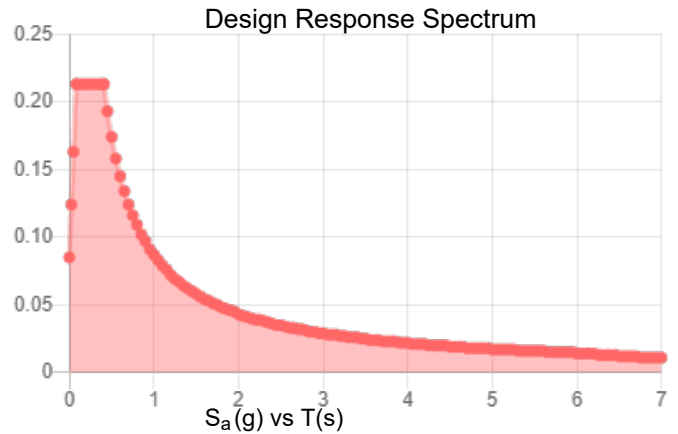
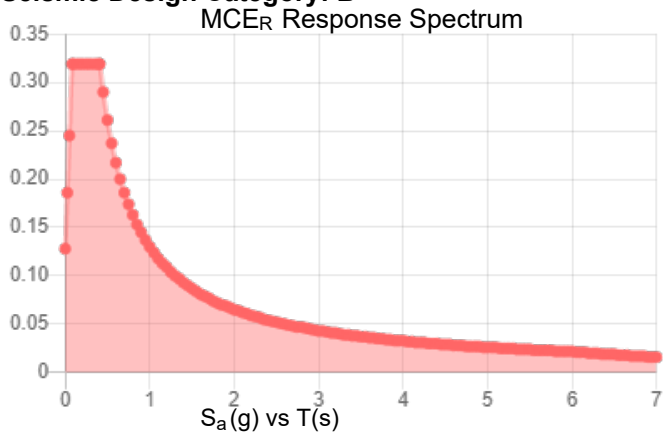
Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings need not be protected against wind-borne debris.

Site Soil Class:

Results:

S_s :	0.199	S_{D1} :	0.087
S_1 :	0.054	T_L :	6
F_a :	1.6	PGA :	0.111
F_v :	2.4	PGA _M :	0.176
S_{MS} :	0.319	F_{PGA} :	1.577
S_{M1} :	0.13	I_e :	1
S_{DS} :	0.213	C_v :	0.7

Seismic Design Category: B



Data Accessed:

Fri Dec 16 2022

Date Source:

USGS Seismic Design Maps based on ASCE/SEI 7-16 and ASCE/SEI 7-16 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-16 Ch. 21 are available from USGS.

Ice

Results:

Ice Thickness: 1.00 in.
Concurrent Temperature: 15 F
Gust Speed 50 mph

Data Source: Standard ASCE/SEI 7-16, Figs. 10-2 through 10-8

Date Accessed: Fri Dec 16 2022

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 500-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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