



# STATE OF CONNECTICUT

## CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

Web Site: [portal.ct.gov/csc](http://portal.ct.gov/csc)

### VIA ELECTRONIC MAIL

May 16, 2024

Hollis M. Redding  
SAI Communications, LLC  
12 Industrial Way  
Salem, NH 03079  
[hredding@saigrp.com](mailto:hredding@saigrp.com)

RE: **EM-AT&T-107-221031** – AT&T notice of intent to modify an existing telecommunications facility located at 298 Ridge Road, Orange, Connecticut. **Request for Project Change.**

Dear Hollis Redding:

The Connecticut Siting Council (Council) is in receipt of your correspondence of May 8, 2024 and associated documents including a revised Structural Analysis performed by Centek Engineering dated April 15, 2024 and stamped and signed by Timothy Lynn; a revised Mount Analysis dated February 16, 2024 performed by TEP Northeast and stamped and signed by Daniel Hamm; and revised construction drawings dated May 1, 2024 prepared by TEP Northeast and stamped and signed by Daniel Hamm; associated with the above-referenced exempt modification request acknowledged by the Council on November 9, 2022.

The Council hereby acknowledges the revised documents with the following conditions:

1. Prior to AT&T's antenna installation, antenna mount modifications shall be installed in accordance with the Mount Analysis prepared by TEP Northeast dated February 16, 2024 and stamped and signed by Daniel Hamm; and
2. Within 45 days following completion of equipment installation, AT&T shall provide documentation certified by a Professional Engineer that its installation complied with the recommendations of the Mount Analysis.

This approval applies only to the project change in the correspondence dated May 8, 2024.

Thank you for your attention and cooperation.

Sincerely,

A handwritten signature in black ink, appearing to read 'Melanie A. Bachman'.

Melanie A. Bachman  
Executive Director

MAB/ANM/dll

c: The Honorable James M. Zeoli, First Selectperson, Town of Orange ( [jzeoli@orange-ct.gov](mailto:jzeoli@orange-ct.gov) )



May 8, 2024

Ms. Melanie A. Bachman  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

Re:

EM-AT&T-107-221031 Change of Approval Request New Cingular Wireless PCS LLC ("AT&T")  
Site CT2267 Facility located at 298 Ridge Road, Orange, CT 06477

Dear Ms. Bachman:

AT&T received CT Siting Council approval on November 28, 2022, to make modifications to its Facility placed on an Eversource Structure located at 298 Ridge Road, Orange, CT. AT&T also received an extension of time for construction to November 28, 2024. After these approvals were received, Eversource requested AT&T rerun the structural & mount analyses to account for the most updated CT building code standards. No changes were made to the equipment per the approvals, but the analysis documents have been updated with the current codes, so the results are different.

AT&T respectfully requests the original CT Siting Council approval be changed to include the updated documentation. Attached are the revised structural & mount analyses, updated plans and original and extension approvals for your review.

Please let me know if you have any questions. Thank you for your time and consideration.  
Sincerely,

*Hollis M. Redding*

Hollis M. Redding  
SAI Communications, LLC  
12 Industrial Way  
Salem, NH 03079  
Mobile: 860-834-6964  
[hredding@saigrp.com](mailto:hredding@saigrp.com)

**Structural Analysis of  
Antenna Mast and Pole**

AT&T Site Ref: CT2267

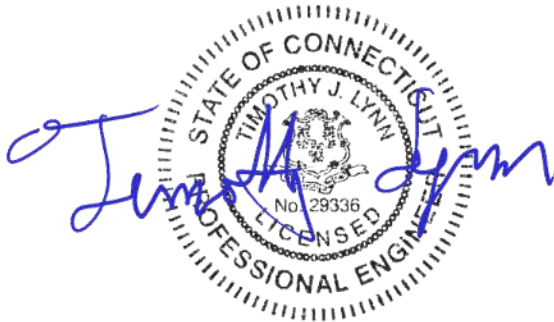
Eversource Structure No. 3848  
96' Electric Transmission Pole

298 Ridge Road  
Orange, CT

CEN TEK Project No. 22021.02

~~Date: April 6, 2022~~

Rev 3: April 15, 2024



**Prepared for:**  
AT&T Mobility  
500 Enterprise Drive, Suite 3A  
Rocky Hill, CT 06067

# **Table of Contents**

## **SECTION 1 - REPORT**

- INTRODUCTION
- PRIMARY ASSUMPTIONS USED IN THE ANALYSIS
- ANALYSIS
- DESIGN BASIS
- RESULTS
- CONCLUSION

## **SECTION 2 - CONDITIONS & SOFTWARE**

- STANDARD ENGINEERING CONDITIONS
- GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAMS
  - RISA 3-D
  - PLS POLE

## **SECTION 3 - DESIGN CRITERIA**

- CRITERIA FOR DESIGN OF PCS FACILITIES ON OR EXTENDING ABOVE METAL ELECTRIC TRANSMISSION TOWERS
- EVERSOURCE DESIGN CRITERIA TABLE
- PCS SHAPE FACTOR CRITERIA
- WIRE LOADS SHEET

## **SECTION 4 - DRAWINGS**

- TOWER AND MAST DRAWINGS

## **SECTION 5 - TIA-222-H LOAD CALCULATIONS FOR MAST ANALYSIS**

- MAST WIND & ICE LOAD

## **SECTION 6 - MAST ANALYSIS PER TIA-222-H**

- RISA 3-D ANALYSIS REPORT
- MAST CONNECTION TO TOWER ANALYSIS

**SECTION 7 - NECS/EVERSOURCE LOAD CALCULATIONS**

- MAST WIND LOAD

**SECTION 8 - MAST ANALYSIS PER NESC/EVERSOURCE**

- RISA 3-D ANALYSIS REPORT

**SECTION 9 - PLS POLE ANALYSIS**

- COAX CABLE LOAD ON UTILITYYY POLE CALCULATION
- PLS REPORT
- ANCHOR BOLT ANALYSIS
- FOUNDATION ANALYSIS

**SECTION 10 - REFERENCE MATERIAL**

- RFDS SHEET
- EQUIPMENT CUT SHEETS

## Introduction

The purpose of this report is to analyze the existing mast and 96' utility pole located on at 298 Ridge Road in Orange, CT for the proposed antenna and equipment upgrade by AT&T.

The existing/proposed loads consist of the following:

- **AT&T MOBILITY (Existing to Remain):**  
**Cables:** Twenty four (24) 1-5/8"  $\varnothing$  coax cables running on the exterior of the pole as indicated in section 4 of this report.  
**Mast:** HSS14x0.625 x 30-ft long pipe conforming to ASTM A500 Gr. B (Fy = 42 ksi).
- **AT&T MOBILITY (Existing to Remove):**  
**Antennas:** Nine (9) Andrew SBNH-1D6565C panel antennas, three (3) CCI OPA-65R-LCUU-H8 panel antennas and twelve (12) CCI DTMABP7819VG12A TMA's mounted on a Triple T-Arm w/ handrail with a RAD center elevation of 111-ft above grade.
- **AT&T MOBILITY (Proposed):**  
**Antennas:** Three (3) CCI TPA65R-BU8D panel antennas, three (3) CCI DMP65R-BU8D panel antennas, three (3) Ericsson AIR6449 panel antennas, three (3) Ericsson AIR6419 panel antennas, three (3) Kaelus TMA2124F03V5-2D TMAs, six (6) Kaelus TMA2104F00V1-1 TMAs and one (1) Raycap DC6 surge arrestor mounted on a Triple T-Arm w/ handrail with a RAD center elevation of 111-ft above grade.  
**Cables:** One (1) fiber cable and two (2) DC cables running on the exterior of the pole as indicated in section 4 of this report.

## Primary assumptions used in the analysis

- ASCE Manual No. 48-19, "Design of Steel Transmission Pole Structures", defines steel stresses for evaluation of the utility pole.
- All utility tower members are adequately protected to prevent corrosion of steel members.
- All proposed antenna mounts are modeled as listed above.
- All coaxial cable will be installed within the antenna mast unless specified otherwise.
- Antenna mast will be properly installed and maintained.
- No residual stresses exist due to incorrect pole erection.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds conform to the requirements of AWS D1.1.
- Antenna mast and utility pole will be in plumb condition.
- Utility pole was properly installed and maintained and all members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
- Any deviation from the analyzed loading will require a new analysis for verification of structural adequacy.
- Properties used in Lpile @ caisson rock socket conservatively based on weak rock as follows:
  - Uniaxial Compressive Strength = 250psi
  - Rock Quality Designation (RQD) = 50%
  - Effective Weight = 135pcf

## A n a l y s i s

Structural analysis of the existing antenna mast was independently completed using the current version of RISA-3D computer program licensed to CENTEK Engineering, Inc.

The existing mast consisting of HSS14x0.625 x 30-ft long pipe connected at two points to the existing tower was analyzed for its ability to resist loads prescribed by the TIA-222-H standard. Section 5 of this report details these gravity and lateral wind loads. NESC prescribed loads were also applied to the mast in order to obtain reactions needed for analyzing the utility pole structure. These loads are developed in Section 7 of this report. Load cases and combinations used in RISA-3D for TIA-222-H loading and for NESC/EVERSOURCE loading are listed in report Sections 6 and 8, respectively.

An envelope solution was first made to determine maximum and minimum forces, stresses, and deflections to confirm the selected section as adequate. Additional analyses were then made to determine the NESC forces to be applied to the pole structure.

The RISA-3D program contains a library of all AISC shapes and corresponding section properties are computed and applied directly within the program. The program's Steel Code Check option was also utilized. The forces calculated in RISA-3D using NESC guidelines were then applied to the pole using PLS-Pole. Maximum usage for the pole was calculated considering the additional forces from the mast and associated appurtenances.

## D e s i g n B a s i s

Our analysis was performed in accordance with ASCE 48-19, "Design of Steel Transmission Pole Structures", NESC C2-2023 and Eversource Design Criteria.

### ▪ UTILITY POLE ANALYSIS

The purpose of this analysis is to determine the adequacy of the existing utility pole to support the proposed antenna loads. The loading and design requirements were analyzed in accordance with the Eversource Design Criteria Table, NESC C2-2023 ~ Construction Grade B, and ASCE Manual No. 48-19.

Load cases considered:

#### Load Case 1: NESC Heavy

Wind Pressure.....	4.0 psf
Radial Ice Thickness.....	0.5"
Vertical Overload Capacity Factor.....	1.50
Wind Overload Capacity Factor.....	2.50
Wire Tension Overload Capacity Factor.....	1.65

#### Load Case 2: NESC Extreme

Wind Speed.....	110 mph <sup>(1)</sup>
Radial Ice Thickness.....	0"

Note 1: NESC C2-2023, Section 25, Rule 250C: Extreme Wind Loading, 1.25 x Gust Response Factor (wind speed: 3-second gust)

- MAST ASSEMBLY ANALYSIS

Mast, appurtenances and connections to the utility tower were analyzed and designed in accordance with TIA-222-H and AISC standards.

Load cases considered:

Load Case 1:

Wind Speed..... 130 mph (2022 CSBC Appendix-P)  
 Radial Ice Thickness..... 0"

Load Case 2:

Wind Pressure..... 50 mph wind pressure  
 Radial Ice Thickness..... 1.0"

Results

- MAST ASSEMBLY

The existing mast was determined to be structurally adequate.

Member	Stress Ratio (% of capacity)	Result
HSS14x0.625	75.2%	PASS
Mast to Base Plate Weld <sup>1</sup>	98.7%	PASS

Note 1 – Critical connection component.

- UTILITY POLE

This analysis finds that the subject utility pole is adequate to support the proposed antenna mast and related appurtenances. The pole stresses meet the requirements set forth by the ASCE Manual No. 48-19, "Design of Steel Transmission Pole Structures", for the applied NESC Heavy and Hi-Wind load cases. The detailed analysis results are provided in Section 6 of this report. The analysis results are summarized as follows:

A maximum usage of **80.63%** occurs in the utility pole under the **NESC Extreme** loading condition.

POLE SECTION:

The utility pole was found to be within allowable limits.

Tower Section	Elevation	Stress Ratio (% of capacity)	Result
Tube Number 1	48.50-96.00' (AGL)	80.63%	PASS

BASE PLATE:

The base plate was found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (percentage of capacity)	Result
Base Plate	Bending	77.75%	PASS



▪ FOUNDATION AND ANCHORS

The existing foundation consists of a 7.0-ft  $\varnothing$  x 18-ft long reinforced concrete caisson. The base of the tower is connected to the foundation by means of (16) 2.25"  $\varnothing$ , ASTM A615 Gr. 75 anchor bolts embedded approximately 7-ft-6-in into the concrete foundation structure. Foundation information was obtained from NUSCO drawing # 01007-60010.

BASE REACTIONS:

From PLS-Pole analysis of pole based on NESC/Eversource prescribed loads.

Load Case	Shear	Axial	Moment
NESC Heavy Wind	17.24 kips	67.00 kips	1329.30 ft-kips
NESC Extreme Wind	39.08 kips	36.38 kips	3183.47 ft-kips

Note 1 – 10% increase to be applied to tower base reactions for foundation verification per OTRM 051

ANCHOR BOLTS:

The anchor bolts were found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (% of capacity)	Result
Anchor Bolts	Tension	60.0%	PASS

FOUNDATION:

The foundation with the proposed modifications was found to be within allowable limits.

Foundation	Design Limit	Proposed Loading <sup>(1)</sup>	Result
Reinforced Concrete Caisson	Moment Capacity	55.1%	PASS
	Shear Capacity	84.8%	PASS
	Lateral Deflection	2.00 in. <sup>(2)</sup>	PASS

Note 1: 10% increase to PLS base reactions used in foundation analysis per OTRM 051.

Note 2: Lateral deflection limited to L/100 per OTRM 051 Rev 5 dated 7/19/10. (L/100 = 18\*12/100=2.16-in)


Conclusion

This analysis shows that the subject utility pole **is adequate** to support the proposed AT&T equipment upgrade.

The analysis is based, in part on the information provided to this office by Eversource and AT&T. If the existing conditions are different than the information in this report, CENTEK engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:

  
 Timothy J. Lynn, PE  
 Structural Engineer



STANDARD CONDITIONS FOR FURNISHING OF  
PROFESSIONAL ENGINEERING SERVICES ON  
EXISTING STRUCTURES

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CENTEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to CENTEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the “as new” condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222.
- All services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. CENTEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

## GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ RISA - 3 D

RISA-3D Structural Analysis Program is an integrated structural analysis and design software package for buildings, bridges, tower structures, etc.

### Modeling Features:

- Comprehensive CAD-like graphic drawing/editing capabilities that let you draw, modify and load elements as well as snap, move, rotate, copy, mirror, scale, split, merge, mesh, delete, apply, etc.
- Versatile drawing grids (orthogonal, radial, skewed)
- Universal snaps and object snaps allow drawing without grids
- Versatile general truss generator
- Powerful graphic select/unselect tools including box, line, polygon, invert, criteria, spreadsheet selection, with locking
- Saved selections to quickly recall desired selections
- Modification tools that modify single items or entire selections
- Real spreadsheets with cut, paste, fill, math, sort, find, etc.
- Dynamic synchronization between spreadsheets and views so you can edit or view any data in the plotted views or in the spreadsheets
- Simultaneous view of multiple spreadsheets
- Constant in-stream error checking and data validation
- Unlimited undo/redo capability
- Generation templates for grids, disks, cylinders, cones, arcs, trusses, tanks, hydrostatic loads, etc.
- Support for all units systems & conversions at any time
- Automatic interaction with RISASection libraries
- Import DXF, RISA-2D, STAAD and ProSteel 3D files
- Export DXF, SDNF and ProSteel 3D files

### Analysis Features:

- Static analysis and P-Delta effects
- Multiple simultaneous dynamic and response spectra analysis using Gupta, CQC or SRSS mode combinations
- Automatic inclusion of mass offset (5% or user defined) for dynamic analysis
- Physical member modeling that does not require members to be broken up at intermediate joints
- State of the art 3 or 4 node plate/shell elements
- High-end automatic mesh generation — draw a polygon with any number of sides to create a mesh of well-formed quadrilateral (NOT triangular) elements.
- Accurate analysis of tapered wide flanges - web, top and bottom flanges may all taper independently
- Automatic rigid diaphragm modeling
- Area loads with one-way or two-way distributions
- Multiple simultaneous moving loads with standard AASHTO loads and custom moving loads for bridges, cranes, etc.
- Torsional warping calculations for stiffness, stress and design
- Automatic Top of Member offset modeling
- Member end releases & rigid end offsets
- Joint master-slave assignments
- Joints detachable from diaphragms
- Enforced joint displacements
- 1-Way members, for tension only bracing, slipping, etc.

- 1-Way springs, for modeling soils and other effects
- Euler members that take compression up to their buckling load, then turn off.
- Stress calculations on any arbitrary shape
- Inactive members, plates, and diaphragms allows you to quickly remove parts of structures from consideration
- Story drift calculations provide relative drift and ratio to height
- Automatic self-weight calculations for members and plates
- Automatic subgrade soil spring generator

#### Graphics Features:

- Unlimited simultaneous model view windows
- Extraordinary “true to scale” rendering, even when drawing
- High-speed redraw algorithm for instant refreshing
- Dynamic scrolling stops right where you want
- Plot & print virtually everything with color coding & labeling
- Rotate, zoom, pan, scroll and snap views
- Saved views to quickly restore frequent or desired views
- Full render or wire-frame animations of deflected model and dynamic mode shapes with frame and speed control
- Animation of moving loads with speed control
- High quality customizable graphics printing

#### Design Features:

- Designs concrete, hot rolled steel, cold formed steel and wood
- ACI 1999/2002, BS 8110-97, CSA A23.3-94, IS456:2000, EC 2-1992 with consistent bar sizes through adjacent spans
- Exact integration of concrete stress distributions using parabolic or rectangular stress blocks
- Concrete beam detailing (Rectangular, T and L)
- Concrete column interaction diagrams
- Steel Design Codes: AISC ASD 9th, LRFD 2nd & 3rd, HSS Specification, CAN/CSA-S16.1-1994 & 2004, BS 5950-1-2000, IS 800-1984, Euro 3-1993 including local shape databases
- AISI 1999 cold formed steel design
- NDS 1991/1997/2001 wood design, including Structural Composite Lumber, multi-ply, full sawn
- Automatic spectra generation for UBC 1997, IBC 2000/2003
- Generation of load combinations: ASCE, UBC, IBC, BOCA, SBC, ACI
- Unbraced lengths for physical members that recognize connecting elements and full lengths of members
- Automatic approximation of K factors
- Tapered wide flange design with either ASD or LRFD codes
- Optimization of member sizes for all materials and all design codes, controlled by standard or user-defined lists of available sizes and criteria such as maximum depths
- Automatic calculation of custom shape properties
- Steel Shapes: AISC, HSS, CAN, ARBED, British, Euro, Indian, Chilean
- Light Gage Shapes: AISI, SSMA, Dale / Incor, Dietrich, Marino\WARE
- Wood Shapes: Complete NDS species/grade database
- Full seamless integration with RISAFoot (Ver 2 or better) for advanced footing design and detailing
- Plate force summation tool

Results Features:

- Graphic presentation of color-coded results and plotted designs
- Color contours of plate stresses and forces with quadratic smoothing, the contours may also be animated
- Spreadsheet results with sorting and filtering of: reactions, member & joint deflections, beam & plate forces/stresses, optimized sizes, code designs, concrete reinforcing, material takeoffs, frequencies and mode shapes
- Standard and user-defined reports
- Graphic member detail reports with force/stress/deflection diagrams and detailed design calculations and expanded diagrams that display magnitudes at any dialed location
- Saved solutions quickly restore analysis and design results.

## GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ PLS - POLE

PLS-POLE provides all of the capabilities a structural engineer requires to design transmission, substation or communications structures. It does so using a simple easy to use graphical interface that rests upon our time tested finite element engine. Regardless of whether you want to model a simple wood pole or a guyed steel X-Frame; PLS-POLE can handle the job simply, reliably and efficiently.

### Modeling Features:

- Structures are made of standard reusable components that are available in libraries. You can easily create your own libraries or get them from a manufacturer
- Structure models are built interactively using interactive menus and graphical commands
- Automatic generation of underlying finite element model of structure
- Steel poles can have circular, 4, 6, 8, 12, 16, or 18-sided, regular, elliptical or user input cross sections (flat-to-flat or tip-to-tip orientations)
- Steel and concrete poles can be selected from standard sizes available from manufacturers
- Automatic pole class selection
- Cross brace position optimizer
- Capability to specify pole ground line rotations
- Capability to model foundation displacements
- Can optionally model foundation stiffness
- Guys are easily handled (modeled as exact cable elements in nonlinear analysis)
- Powerful graphics module (members color-coded by stress usage)
- Graphical selection of joints and components allows graphical editing and checking
- Poles can be shown as lines, wire frames or can be rendered as 3-d polygon surfaces

### Analysis Features:

- Automatic distribution of loads in 2-part suspension insulators (v-strings, horizontal vees, etc.)
- Design checks for ASCE, ANSI/TIA/EIA 222 (Revisions F and G) or other requirements
- Automatic calculation of dead and wind loads
- Automated loading on structure (wind, ice and drag coefficients) according to:
  - ASCE 74-1991
  - NESC 2002
  - NESC 2007
  - IEC 60826:2003
  - EN50341-1:2001 (CENELEC)
  - EN50341-3-9:2001 (UK NNA)
  - EN50341-3-17:2001 (Portugal NNA)
  - ESAA C(b)1-2003 (Australia)
  - TPNZ (New Zealand)
  - REE (Spain)
  - EIA/TIA 222-F
  - ANSI/TIA 222-G
  - CSA S37-01
- Automated microwave antenna loading as per EIA/TIA 222-F and ANSI/TIA 222-G
- Detects buckling by nonlinear analysis

Results Features:

- Detects buckling by nonlinear analysis
- Easy to interpret text, spreadsheet and graphics design summaries
- Automatic determination of allowable wind and weight spans
- Automatic determination of interaction diagrams between allowable wind and weight spans
- Automatic tracking of part numbers and costs

*Criteria for Design of PCS Facilities On or  
Extending Above Metal Electric Transmission  
Towers & Analysis of Transmission Towers  
Supporting PCS Masts* <sup>(1)</sup>

*Introduction*

This criteria is the result from an evaluation of the methods and loadings specified by the separate standards, which are used in designing telecommunications towers and electric transmission towers. That evaluation is detailed elsewhere, but in summary; the methods and loadings are significantly different. This criteria specifies the manner in which the appropriate standard is used to design PCS facilities including masts and brackets (hereafter referred to as “masts”), and to evaluate the electric transmission towers to support PCS masts. The intent is to achieve an equivalent level of safety and security under the extreme design conditions expected in Connecticut and Massachusetts.

ANSI Standard TIA-222-H covering the design of telecommunications structures specifies LRFD design approach. This approach applies the loads from extreme weather loading conditions, and designs the structure so that it does not exceed code defined percentage of failure strength.

ANSI Standard C2-2023 (National Electrical Safety Code) covering the design of electric transmission metal structures is based upon an ultimate strength/yield stress design approach. This approach applies a multiplier (overload capacity factor) to the loads possible from extreme weather loading conditions, and designs the structure so that it does not exceed its ultimate strength (yield stress).

Each standard defines the details of how loads are to be calculated differently. Most of the Eversource effort in “unifying” both codes was to establish what level of strength each approach would provide, and then increasing the appropriate elements of each to achieve a similar level of security under extreme weather loadings.

Two extreme weather conditions are considered. The first is an extreme wind condition (hurricane) based upon a 1700-year recurrence for TIA-222-H risk category III and a 100-year recurrence for NESC Grade B. The second is a winter condition combining wind and ice loadings.

The following sections describe the design criteria for any PCS mast extending above the top of an electric transmission tower, and the analysis criteria for evaluating the loads on the transmission tower from such a mast from the lower portions of such a mast, and loads on the pre-existing electric lower portions of such a mast, and loads on the pre-existing electric transmission tower and the conductors it supports.

| Note 1: *Prepared from documentation provide from Northeast Utilities.*



### *P C S M a s t*

The PCS facility (mast, external cable/trays, including the initial and any planned future support platforms, antennas, etc. extending the full height above the top level of the electric transmission structure) shall be designed in accordance with the provisions of TIA 222-H:

### *E L E C T R I C T R A N S M I S S I O N T O W E R*

The electric transmission tower shall be analyzed using yield stress theory in accordance with the attached table titled “Eversource Design Criteria”. This specifies uniform loadings (different from the TIA loadings) on the each of the following components of the installed facility:

- PCS mast for its total height above ground level, including the initial and planned future support platforms, antennas, etc. above the top of an electric transmission structure.
- Conductors are related devices and hardware.
- Electric transmission structure. The loads from the PCS facility and from the electric conductors shall be applied to the structure at conductor and PCS mast attachment points, where those load transfer to the tower.

The uniform loadings and factors specified for the above components in the table are based upon the National Electrical Safety Code 2023 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to TIA and its loads and factors with the exceptions noted above. (Note that the NESC does not require the projected wind surfaces of structures and equipment to be increased by the ice covering.)

In the event that the electric transmission tower is not sufficient to support the additional loadings of the PCS mast, reinforcement will be necessary to upgrade the strength of the overstressed members.

# Eversource

## Overhead Transmission Standards

### Attachment A Eversource Design Criteria

		Attachment A ES Design Criteria	Basic Wind Speed	Pressure	Height Factor	Gust Factor	Load or Stress Factor	Force Coef. - Shape Factor
			V (MPH)	Q (PSF)	Kz	Gh		
Ice Condition	TIA/EIA	Antenna Mount	TIA	TIA (0.75Wi)	TIA	TIA	TIA, Section 3.1.1.1 disallowed for connection design	TIA
	NESC Heavy	Tower/Pole Analysis with antennas extending above top of Tower/Pole (Yield Stress)	-----	4	1	1	2.5	1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with antennas below top of Tower/Pole (on two faces)	-----	4	1	1	2.5	1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor Loads Provided by ES					
High Wind Condition	TIA/EIA	Antenna Mount	85	TIA	TIA	TIA	TIA, Section 3.1.1.1 disallowed for connection design	TIA
	NESC Extreme Wind	Tower/Pole Analysis with antennas extending above top of Tower/Pole	For wind speed use OTRM 060 Map 1, Rule 250C: Extreme Wind Loading Apply a 1.25 x Gust Response Factor to all telecommunication equipment projected above top of tower/pole and apply a 1.0 x Gust Response Factor to the tower/pole structure					1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with antennas below top of Tower/Pole	For wind speed use OTRM 060 Map 1, Rule 250C: Extreme Wind Loading Height above ground is based on overall height to top of tower/pole					1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor Loads Provided by ES					
NESC Extreme Ice with Wind Condition*	Tower/Pole Analysis with antennas extending above top of Tower/Pole		For wind speed use OTRM 060 Map 1, Rule 250D: Extreme Ice with Wind Loading 4 PSF Wind Load 1.25 x Gust Response Factor Apply a 1.25 x Gust Response Factor to all telecommunication equipment projected above top of tower/pole and apply a 1.0 x Gust Response Factor to the tower/pole structure					1.6 Flat Surfaces 1.3 Round Surfaces
	Tower/Pole Analysis with antennas below top of Tower/Pole		For wind speed use OTRM 060 Map 1, Rule 250D: Extreme Ice with Wind Loading 4 PSF Wind Load Height above ground is based on overall height to top of tower/pole					1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor Loads Provided by ES					

\*Only for structures installed after 2007

### Communication Antennas on Transmission Structures

<b>Eversource</b> Approved by: CPS (CT/WMA) JCC (NH/EMA)	<b>Design</b>	<b>OTRM 059</b> <b>Page 8 of 10</b>	<b>Rev. 1</b> <b>11/19/2018</b>
----------------------------------------------------------------	---------------	----------------------------------------	------------------------------------

**Overhead Transmission Standards**

determined from NESC applied loading conditions (not TIA Loads) on the structure and mount as specified below, and shall include the wireless communication mast and antenna loads per NESC criteria)

The strength reduction factor obtained from the field investigation shall be applied to the members or connections that are showing signs of deterioration from their original condition. With the written approval of Eversource Transmission Line Engineering on a case by case the existing structures may be analyzed initially using the current NESC code, then it is permitted to use the original design code with the original conductor load should the existing tower fail the current NESC code.

The structure shall be analyzed using yield stress theory in accordance with Attachment A, "Eversource Design Criteria." This specifies uniform loadings (different from the TIA loadings) on each of the following components of the installed facility:

- a) Wireless communication mast for its total height above ground level, including the initial and any planned future equipment (Support Platforms, Antennas, TMA's etc.) above the top of an electric transmission structure.
- b) Conductors and related devices and hardware (wire loads will be provided by Eversource).
- c) Electric Transmission Structure

- i) The loads from the wireless communication equipment components based on NESC and Eversource Criteria in Attachment A, and from the electric conductors shall be applied to the structure at conductor and wireless communication mast attachment points, where those loads transfer to the tower. ii)
- ii) Shape Factor Multiplier:

NESC Structure Shape	Cd
Polyround (for polygonal steel poles)	1.3
Flat	1.6
Open Lattice	3.2
Pole with Coaxial Cable	See Below Table

- iii) When Coaxial Cables are mounted alongside the pole structure, the shape multiplier shall be:

Mount Type	Cable Cd	Pole Cd
Coaxial Cables on outside periphery (One layer)	1.45	1.45
Coaxial Cables mounted on stand offs	1.6	1.6

- d) The uniform loadings and factors specified for the above components in Attachment A, "Eversource Design Criteria" are based upon the National Electric Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to the TIA and its loads and factors with the exceptions noted above.

PLS-CADD Version 12.30 11:15:02 AM Friday, December 21, 2012  
 Northeast Utilities System  
 Project Name: 'c:\cps\pls-cadd\mn345 project final models\prj-dev\imagery\peaserd\_eastdevon\_final\_pls.DON'  
 Line Title: '345-kv/115-kv 3827, 1640, 1685, 1776, 1497, & 1350'

Criteria Notes:  
 NESC Heavy per Rule 250B, Page 161  
 Extreme Wind Loading per Rule 250C, Page 161, Coefficients and Gust Response Factors per Equations in Tables 250-2, 250-3  
 90 MPH Basic Wind Speed, 3 second Gust Wind Speed, Figure 250-2 Beginning on Page 166  
 Grade B Construction "Method A" per Table 253-1, Page 173 and Table 261-1A, Page 182  
 Tension Limits per Rule 261H1, Page 179  
 Insulator Strength Reduction per Rule 277, Page 188 Should be applied to Insulator Strengths when Modeling Insulators  
 2002 NESC C2-2002 Criteria File for PLS-CADD Created December 21, 2001

Structure 24277 (just North of Durham Rd) moved 50` south per homeowner request. Structure height increased 5 ft. to maintain ground clearance.  
 1-27-06 Moved structure 24257 100` NE to accommodate homeowner request.  
 Homeowner request - Structures 24102 & 3915 were moved 100 ft. North for aesthetic purposes.  
 Homeowner request - Structure 3910 moved 90 ft. South out of line of sight.  
 Homeowner requests - Neighborhoods of Salem Rd, Country Ct., & Brookwood Ct. requested 135` typ. split phase.  
 do not change these areas structure heights.  
 Homeowner request - Structures 24098 & 3911 moved South to property line.  
 Structure 3808 raised 10 ft. to meet road clearance requirements  
 12-5-06: Structure 3807 moved 15 ft. South to avoid existing 3808 guy wires.  
 3848: Moved 5 ft. South towards 3847 per field request, too close to rock wall and distribution lines.  
 3848: Moved 5 ft. North back to original spot per email from Jason Cabral. Rock wall will be moved, PAR said there should be no impact to adjacent distribution lines.  
 3807: (1-24-07) Moved approx. 20 ft. southwest to get structure away from existing 1640 guy locations to accomodate construction.  
 3849: (1-24-07) Moved approx. 14 ft. southwest to get structure away from existing 1640 guy locations to accomodate construction.  
 3888: (1-24-07) Moved approx. 20 ft. north to get structure away from existing 1640 guy locations to accomodate construction.  
 5-22-07 : Moved structure 24043 20 ft. northeast per homeowner request. This was a field call from Tim Arvin.  
 6-11-07: Moved structure 3911 5 ft. South to avoid underground utilities to two residences.  
 10-17-07: Moved structure 24002 46.5 ft. north per field request.  
 10-24-07: Moved structure 24045 11.75 ft. east per field request.  
 1-4-08: Moved structure 24091 17 ft. south to avoid underground storm water drainage pipe.

Weather Cases

WC #	Description	Air Density Factor (psf/mph^2)	Wind Vel. (mph)	Wind Pres. (psf)	Wire Ice Thick (in)	Wire Ice Density (lbs/ft^3)	Wire Ice Load (lbs/ft)	Wire Temp (deg F)	Ambient Temp (deg F)	Weather Load Factor	NESC Constant (lbs/ft)	Wire Wind Height Adjust Model	Wire Gust Response Factor
1	NESC Heavy	0.00256	40	4.0	0.50	57.000	0.00	0	0	1.00	0.30	None	1
2	0F 4psf w/ ice	0.00256	40	4.0	0.50	57.000	0.00	0	0	1.00	0.00	None	1
3	0F 4psf w/o ice	0.00256	40	4.0	0.00	0.000	0.00	0	0	1.00	0.00	None	1
4	NESC Ext Wind	0.00256	112	32.1	0.00	0.000	0.00	60	60	1.00	0.00	None	1
5	ASCE Ice/Wind	0.00256	50	6.4	0.75	57.000	0.00	32	32	1.00	0.00	None	1
6	Maximum Operating	0.00256	0	0.0	0.00	0.000	0.00	285	285	1.00	0.00	None	1
7	NESC Blowout 6PSF	0.00256	48	6.0	0.00	0.000	0.00	60	60	1.00	0.00	None	1
8	3# Wind (SWING 1)	0.00256	34	3.0	0.00	0.000	0.00	60	60	1.00	0.00	None	1
9	6# Wind (SWING 2)	0.00256	48	6.0	0.00	0.000	0.00	60	60	1.00	0.00	None	1
10	60 mph Wind (SWING 3)	0.00256	60	9.2	0.00	0.000	0.00	60	60	1.00	0.00	None	1
11	GALLOPING (SWING)	0.00256	28	2.0	0.50	57.000	0.00	32	32	1.00	0.00	None	1
12	GALLOPING (SAG)	0.00256	0	0.0	0.50	57.000	0.00	32	32	1.00	0.00	None	1
13	-20 Deg F	0.00256	0	0.0	0.00	0.000	0.00	-20	-20	1.00	0.00	None	1
14	0 Deg F	0.00256	0	0.0	0.00	0.000	0.00	0	0	1.00	0.00	None	1
15	50 Deg F	0.00256	0	0.0	0.00	0.000	0.00	50	50	1.00	0.00	None	1
16	60 Deg F	0.00256	0	0.0	0.00	0.000	0.00	60	60	1.00	0.00	None	1
17	120 Deg F	0.00256	0	0.0	0.00	0.000	0.00	120	120	1.00	0.00	None	1
18	NU Ice	0.00256	0	0.0	1.00	57.000	0.00	0	0	1.00	0.00	None	1
19	NU Blowout	0.00256	60	9.2	0.00	0.000	0.00	60	60	1.00	0.00	None	1
20	Construction	0.00256	30	2.3	0.00	0.000	0.00	30	30	1.00	0.00	None	1
21	UI Ice	0.00256	56	8.0	0.50	57.000	0.00	0	0	1.00	0.00	None	1
22	NU Extreme Wind 90mph	0.00256	90	20.7	0.00	0.000	0.00	60	60	1.00	0.00	None	1
23	9psf Wind 60 deg F	0.00256	59	9.0	0.00	0.000	0.00	60	60	1.00	0.00	None	1
24	Tower High Wind	0.00256	80	16.4	0.00	0.000	0.00	60	60	1.00	0.00	None	1
25	UI Heavy Ice (Tower)	0.00256	0	0.0	1.50	57.000	0.00	32	32	1.00	0.00	None	1
26	ADSS Survey Temp	0.00256	0	0.0	0.00	0.000	0.00	85	85	1.00	0.00	None	1
27	ADSS 90 Deg Case	0.00256	0	0.0	0.00	0.000	0.00	90	90	1.00	0.00	None	1

Structure Loads Criteria

LC #	WC #	Load Case Description	Cable Condition	Wind Dir.	Bisect Wind Angle	Wire Vert. Load Factor	Wire + Struct. Wind Load Factor	Wire Tension Load Factor	Struct. Weight Load Factor	Struct. Wind Area Factor	Struct. Wind Load Model	Struct. Ice Thick (in)	Struct. Ice Density (lbs/ft^3)	Pole Tip Deflection Check	Pole Tip Deflect Limit % or (ft)
1	1	NESC Heavy NA+	Load RS	NA+		1.50	2.50	1.65	1.50	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
2	1	NESC Heavy NA-	Load RS	NA-		1.50	2.50	1.65	1.50	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
3	1	NESC Uplift NA+	Load RS	NA+		1.00	2.50	1.65	1.00	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
4	1	NESC Uplift NA-	Load RS	NA-		1.00	2.50	1.65	1.00	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
5	4	NESC Ext Wind NA	Load RS	NA+		1.10	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
6	4	NESC Ext Wind NA	Load RS	NA-		1.10	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
7	1	NESC Ins NA+	Load RS	NA+		1.00	1.00	1.00	1.00	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
8	1	NESC Ins NA-	Load RS	NA-		1.00	1.00	1.00	1.00	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00

LC	WC	Load Case	Struct	Command 1	Command 1	Command 1	Value	STR 3848	Loads.txt	No Limit	0.00		
9	5	ASCE Ice/Wind NA	Load RS	NA+	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
10	5	ASCE Ice/Wind NA	Load RS	NA-	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
11	18	NU Ice	Load RS	NA+	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
12	24	Tower High Wind	Load RS	NA+	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
13	24	Tower High Wind	Load RS	NA-	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
14	1	Broken SW NA+	Load RS	NA+	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
15	1	Broken SW NA-	Load RS	NA-	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
16	1	Cond SW NA+	Load RS	NA+	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
17	1	Cond SW NA-	Load RS	NA-	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
18	1	Deadend Tower NA	Load RS	NA+	1.00	1.00	1.00	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
19	1	Deadend Tower NA	Load RS	NA-	1.00	1.00	1.00	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
20	25	UI Havey Ice Tow	Load RS	NA+	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00
21	25	UI Havey Ice Tow	Load RS	NA-	1.10	1.10	1.10	1.00	Pre V7 Standard	0.00	0.000	No Limit	0.00

Cable Load Adjustments for each Load Case

LC #	WC #	Load Case Description	Struct Groups On Which To Apply	Command 1 Wire(s) Set: Phase: Side:	Command 1 Value (lbs) (deg) (%)
1	1	NESC Heavy NA+	'All'		
2	1	NESC Heavy NA-	'All'		
3	1	NESC Uplift NA+	'All'		
4	1	NESC Uplift NA-	'All'		
5	4	NESC Ext Wind NA	'All'		
6	4	NESC Ext Wind NA	'All'		
7	1	NESC Ins NA+	'All'		
8	1	NESC Ins NA-	'All'		
9	5	ASCE Ice/Wind NA	'All'		
10	5	ASCE Ice/Wind NA	'All'		
11	18	NU Ice	'All'		
12	24	Tower High Wind	'TOWER'		
13	24	Tower High Wind	'TOWER'		
14	1	Broken SW NA+	'TOWER no DE'	1:1:Ahead # Broken Subconductors	1.0
15	1	Broken SW NA-	'TOWER no DE'	1:1:Ahead # Broken Subconductors	1.0
16	1	Cond SW NA+	'TOWER no DE'	2:1:Ahead # Broken Subconductors	1.0
17	1	Cond SW NA-	'TOWER no DE'	2:1:Ahead # Broken Subconductors	1.0
18	1	Deadend Tower NA	'TOWER has DE'	Ahead Spans # Broken Subconductors	1.0
19	1	Deadend Tower NA	'TOWER has DE'	Ahead Spans # Broken Subconductors	1.0
20	25	UI Havey Ice Tow	'TOWER'		
21	25	UI Havey Ice Tow	'TOWER'		

Span and Wire Summary For Structure Range

Span azimuth is measured clockwise from structure transverse axis (0=transverse, 90=back, 270=ahead)  
Azimuth of structure transverse axis is 147.3608 (deg) measured clockwise from North.

Str. No.	Str. Name	LC #	WC #	Load Case Description	Set No.	Phase No.	Attach. Joint Labels	Back span					Ahead span						
								Cable Name	Len. (ft)	Azi. (deg)	Horiz. Load (lbs/ft)	Vert. Tension (lbs)	Horiz. Load (lbs/ft)	Vert. Tension (lbs)					
3848	11-cdcsp-02-uw.095	1	1	NESC Heavy NA+	1	1	GL:E	brugg_57ay104acs-3c_115.wir	387	90	1.37	1.77	10679	brugg_57ay104acs-3c_115.wir	465	270	1.37	1.77	10680
3848	11-cdcsp-02-uw.095	1	1		2	1	TL	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	464	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	1	1		3	1	ML	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	464	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	1	1		4	1	BL	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	464	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	1	1		5	1	GR:E	19#10_alumoweld_tight.wir	387	90	1.26	1.61	9769	19#10_alumoweld_tight.wir	466	270	1.26	1.61	9770
3848	11-cdcsp-02-uw.095	1	1		6	1	TR	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	466	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	1	1		7	1	MR	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	466	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	1	1		8	1	BR	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	466	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	2	1	NESC Heavy NA-	1	1	GL:E	brugg_57ay104acs-3c_115.wir	387	90	1.37	1.77	10679	brugg_57ay104acs-3c_115.wir	465	270	1.37	1.77	10680
3848	11-cdcsp-02-uw.095	2	1		2	1	TL	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	464	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	2	1		3	1	ML	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	464	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	2	1		4	1	BL	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	464	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	2	1		5	1	GR:E	19#10_alumoweld_tight.wir	387	90	1.26	1.61	9769	19#10_alumoweld_tight.wir	466	270	1.26	1.61	9770
3848	11-cdcsp-02-uw.095	2	1		6	1	TR	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	466	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	2	1		7	1	MR	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	466	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	2	1		8	1	BR	lapwing_tight.wir	387	90	2.09	4.56	22624	lapwing_tight.wir	466	270	2.09	4.56	22624
3848	11-cdcsp-02-uw.095	3	1	NESC Uplift NA+	1	1	GL:E	brugg_57ay104acs-3c_115.wir	387	90	1.37	1.18	10679	brugg_57ay104acs-3c_115.wir	465	270	1.37	1.18	10680
3848	11-cdcsp-02-uw.095	3	1		2	1	TL	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	464	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	3	1		3	1	ML	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	464	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	3	1		4	1	BL	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	464	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	3	1		5	1	GR:E	19#10_alumoweld_tight.wir	387	90	1.26	1.08	9769	19#10_alumoweld_tight.wir	466	270	1.26	1.08	9770
3848	11-cdcsp-02-uw.095	3	1		6	1	TR	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	466	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	3	1		7	1	MR	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	466	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	3	1		8	1	BR	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	466	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	4	1	NESC Uplift NA-	1	1	GL:E	brugg_57ay104acs-3c_115.wir	387	90	1.37	1.18	10679	brugg_57ay104acs-3c_115.wir	465	270	1.37	1.18	10680
3848	11-cdcsp-02-uw.095	4	1		2	1	TL	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	464	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	4	1		3	1	ML	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	464	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	4	1		4	1	BL	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	464	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	4	1		5	1	GR:E	19#10_alumoweld_tight.wir	387	90	1.26	1.08	9769	19#10_alumoweld_tight.wir	466	270	1.26	1.08	9770
3848	11-cdcsp-02-uw.095	4	1		6	1	TR	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	466	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	4	1		7	1	MR	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	466	270	2.09	3.04	22624
3848	11-cdcsp-02-uw.095	4	1		8	1	BR	lapwing_tight.wir	387	90	2.09	3.04	22624	lapwing_tight.wir	466	270	2.09	3.04	22624



3848	11-cdcsp-02-uwx.095	3	1	6	1	TR	1148	404	22624	337	487	22624	
3848	11-cdcsp-02-uwx.095	3	1	7	1	MR	1148	404	22624	337	487	22624	
3848	11-cdcsp-02-uwx.095	3	1	8	1	BR	1148	404	22624	337	487	22624	
3848	11-cdcsp-02-uwx.095	4	1	1	1	GL:E	464	-266	10679	118	-319	10680	
3848	11-cdcsp-02-uwx.095	4	1	2	1	TL	1148	-404	22624	331	-484	22624	
3848	11-cdcsp-02-uwx.095	4	1	3	1	ML	1148	-404	22624	331	-484	22624	
3848	11-cdcsp-02-uwx.095	4	1	4	1	BL	1148	-404	22624	331	-484	22624	
3848	11-cdcsp-02-uwx.095	4	1	5	1	GR:E	420	-244	9769	110	-293	9770	
3848	11-cdcsp-02-uwx.095	4	1	6	1	TR	1148	-404	22624	337	-487	22624	
3848	11-cdcsp-02-uwx.095	4	1	7	1	MR	1148	-404	22624	337	-487	22624	
3848	11-cdcsp-02-uwx.095	4	1	8	1	BR	1148	-404	22624	337	-487	22624	
3848	11-cdcsp-02-uwx.095	5	4	1	1	GL:E	405	368	6836	-84	442	6840	
3848	11-cdcsp-02-uwx.095	5	4	2	1	TL	1062	858	15223	3	1029	15230	
3848	11-cdcsp-02-uwx.095	5	4	3	1	ML	1062	858	15223	3	1028	15230	
3848	11-cdcsp-02-uwx.095	5	4	4	1	BL	1062	858	15223	3	1028	15230	
3848	11-cdcsp-02-uwx.095	5	4	5	1	GR:E	352	290	5737	-56	349	5740	
3848	11-cdcsp-02-uwx.095	5	4	6	1	TR	1062	858	15223	7	1034	15230	
3848	11-cdcsp-02-uwx.095	5	4	7	1	MR	1062	858	15223	7	1034	15230	
3848	11-cdcsp-02-uwx.095	5	4	8	1	BR	1062	858	15223	7	1034	15230	
3848	11-cdcsp-02-uwx.095	6	4	1	1	GL:E	405	-368	6836	-84	-442	6840	
3848	11-cdcsp-02-uwx.095	6	4	2	1	TL	1062	-858	15223	3	-1029	15230	
3848	11-cdcsp-02-uwx.095	6	4	3	1	ML	1062	-858	15223	3	-1028	15230	
3848	11-cdcsp-02-uwx.095	6	4	4	1	BL	1062	-858	15223	3	-1028	15230	
3848	11-cdcsp-02-uwx.095	6	4	5	1	GR:E	352	-290	5737	-56	-349	5740	
3848	11-cdcsp-02-uwx.095	6	4	6	1	TR	1062	-858	15223	7	-1034	15230	
3848	11-cdcsp-02-uwx.095	6	4	7	1	MR	1062	-858	15223	7	-1034	15230	
3848	11-cdcsp-02-uwx.095	6	4	8	1	BR	1062	-858	15223	7	-1034	15230	
3848	11-cdcsp-02-uwx.095	7	1	1	1	GL:E	464	106	6472	118	128	6473	
3848	11-cdcsp-02-uwx.095	7	1	2	1	TL	1148	162	13711	331	194	13712	
3848	11-cdcsp-02-uwx.095	7	1	3	1	ML	1148	162	13711	331	194	13712	
3848	11-cdcsp-02-uwx.095	7	1	4	1	BL	1148	162	13711	331	194	13712	
3848	11-cdcsp-02-uwx.095	7	1	5	1	GR:E	420	97	5921	110	117	5921	
3848	11-cdcsp-02-uwx.095	7	1	6	1	TR	1148	162	13711	337	195	13712	
3848	11-cdcsp-02-uwx.095	7	1	7	1	MR	1148	162	13711	337	195	13712	
3848	11-cdcsp-02-uwx.095	7	1	8	1	BR	1148	162	13711	337	195	13712	
3848	11-cdcsp-02-uwx.095	8	1	1	1	GL:E	464	-106	6472	118	-128	6473	
3848	11-cdcsp-02-uwx.095	8	1	2	1	TL	1148	-162	13711	331	-194	13712	
3848	11-cdcsp-02-uwx.095	8	1	3	1	ML	1148	-162	13711	331	-194	13712	
3848	11-cdcsp-02-uwx.095	8	1	4	1	BL	1148	-162	13711	331	-194	13712	
3848	11-cdcsp-02-uwx.095	8	1	5	1	GR:E	420	-97	5921	110	-117	5921	
3848	11-cdcsp-02-uwx.095	8	1	6	1	TR	1148	-162	13711	337	-195	13712	
3848	11-cdcsp-02-uwx.095	8	1	7	1	MR	1148	-162	13711	337	-195	13712	
3848	11-cdcsp-02-uwx.095	8	1	8	1	BR	1148	-162	13711	337	-195	13712	
3848	11-cdcsp-02-uwx.095	9	5	1	1	GL:E	733	244	7952	216	293	7954	
3848	11-cdcsp-02-uwx.095	9	5	2	1	TL	1536	342	15791	523	409	15792	
3848	11-cdcsp-02-uwx.095	9	5	3	1	ML	1536	342	15791	523	409	15792	
3848	11-cdcsp-02-uwx.095	9	5	4	1	BL	1536	342	15791	523	409	15792	
3848	11-cdcsp-02-uwx.095	9	5	5	1	GR:E	671	228	7283	199	275	7284	
3848	11-cdcsp-02-uwx.095	9	5	6	1	TR	1536	342	15796	530	411	15797	
3848	11-cdcsp-02-uwx.095	9	5	7	1	MR	1536	342	15796	530	411	15797	
3848	11-cdcsp-02-uwx.095	9	5	8	1	BR	1536	342	15796	530	411	15797	
3848	11-cdcsp-02-uwx.095	10	5	1	1	GL:E	733	-244	7952	216	-293	7954	
3848	11-cdcsp-02-uwx.095	10	5	2	1	TL	1536	-342	15791	523	-409	15792	
3848	11-cdcsp-02-uwx.095	10	5	3	1	ML	1536	-342	15791	523	-409	15792	
3848	11-cdcsp-02-uwx.095	10	5	4	1	BL	1536	-342	15791	523	-409	15792	
3848	11-cdcsp-02-uwx.095	10	5	5	1	GR:E	671	-228	7283	199	-275	7284	
3848	11-cdcsp-02-uwx.095	10	5	6	1	TR	1536	-342	15796	530	-411	15797	
3848	11-cdcsp-02-uwx.095	10	5	7	1	MR	1536	-342	15796	530	-411	15797	
3848	11-cdcsp-02-uwx.095	10	5	8	1	BR	1536	-342	15796	530	-411	15797	
3848	11-cdcsp-02-uwx.095	11	18	1	1	GL:E	952	0	9310	366	0	9310	
3848	11-cdcsp-02-uwx.095	11	18	2	1	TL	1902	0	19169	680	0	19169	
3848	11-cdcsp-02-uwx.095	11	18	3	1	ML	1902	0	19169	680	0	19169	
3848	11-cdcsp-02-uwx.095	11	18	4	1	BL	1902	0	19169	680	0	19169	
3848	11-cdcsp-02-uwx.095	11	18	5	1	GR:E	875	0	8492	343	0	8492	
3848	11-cdcsp-02-uwx.095	11	18	6	1	TR	1902	0	19169	689	0	19169	
3848	11-cdcsp-02-uwx.095	11	18	7	1	MR	1902	0	19169	689	0	19169	
3848	11-cdcsp-02-uwx.095	11	18	8	1	BR	1902	0	19169	689	0	19169	

Wire Loads In Structure Coordinate System For Structure Range

Note: Loads in this report include load from counter weights, insulator weight and insulator wind area.

Str. No.	Str. Name	LC #	WC #	Load Case Description	Set No.	Phase No.	Attach. Joint Labels	---Structure Loads---			--Loads from back span--			-Loads from ahead span-			Warnings
								Vert.	Trans.	Long.	Vert.	Trans.	Long.	Vert.	Trans.	Long.	
								------(lbs)-----			------(lbs)-----			------(lbs)-----			
3848	11-cdcsp-02-uwx.095	1	1	NESC Heavy NA+	1	1	GL:E	872	659	-1	696	302	10678	177	356	-10679	
3848	11-cdcsp-02-uwx.095	1	1		2	1	TL	2369	1055	-1	1797	487	22622	572	568	-22623	
3848	11-cdcsp-02-uwx.095	1	1		3	1	ML	2369	1056	-1	1797	487	22622	572	569	-22623	
3848	11-cdcsp-02-uwx.095	1	1		4	1	BL	2369	1057	-1	1797	487	22622	572	570	-22623	
3848	11-cdcsp-02-uwx.095	1	1		5	1	GR:E	795	603	-1	629	277	9768	165	326	-9769	
3848	11-cdcsp-02-uwx.095	1	1		6	1	TR	2377	1055	-1	1797	487	22622	580	568	-22623	
3848	11-cdcsp-02-uwx.095	1	1		7	1	MR	2377	1054	-1	1797	487	22622	580	567	-22623	
3848	11-cdcsp-02-uwx.095	1	1		8	1	BR	2377	1053	-1	1797	487	22622	580	566	-22623	
3848	11-cdcsp-02-uwx.095	2	1	NESC Heavy NA-	1	1	GL:E	872	-511	-1	696	-229	10680	177	-282	-10681	
3848	11-cdcsp-02-uwx.095	2	1		2	1	TL	2369	-743	-1	1797	-332	22625	572	-411	-22626	
3848	11-cdcsp-02-uwx.095	2	1		3	1	ML	2369	-742	-1	1797	-332	22625	572	-410	-22626	









STR 3848 Loads.txt																									
3848	11-cdcsp-02-uwX.095	0.39	9	5	ASCE Ice/Wind NA	1	1	GL:E	0.0	-4.8	95.0	95.0	N	1	1	948	592	-1	733	271	7952	216	321	-7953	brugg_57ay104acs-3c_115.wir
387	90	1.26	1.95	7952	brugg_57ay104acs-3c_115.wir	465	270	1.26	1.95	7954															
3848	11-cdcsp-02-uwX.095	0.39	9	5		2	1	TL	0.0	-9.9	88.5	88.5	N	1	1	2169	867	-1	1591	399	15789	578	468	-15790	lapwing_tight.wir
387	90	1.76	4.28	15791	lapwing_tight.wir	464	270	1.76	4.28	15792															
3848	11-cdcsp-02-uwX.095	0.39	9	5		3	1	ML	0.0	-10.1	75.5	75.5	N	1	1	2168	868	-1	1591	399	15789	578	469	-15790	lapwing_tight.wir
387	90	1.76	4.28	15791	lapwing_tight.wir	464	270	1.76	4.28	15792															
3848	11-cdcsp-02-uwX.095	0.39	9	5		4	1	BL	0.0	-10.2	62.5	62.5	N	1	1	2168	868	-1	1591	399	15789	578	469	-15790	lapwing_tight.wir
387	90	1.76	4.28	15791	lapwing_tight.wir	464	270	1.76	4.28	15792															
3848	11-cdcsp-02-uwX.095	0.39	9	5		5	1	GR:E	0.0	4.8	95.0	95.0	N	1	1	871	552	-1	671	253	7282	199	299	-7283	19#10_alumoweld_tight.wir
387	90	1.18	1.79	7283	19#10_alumoweld_tight.wir	466	270	1.18	1.79	7284															
3848	11-cdcsp-02-uwX.095	0.39	9	5		6	1	TR	0.0	9.9	88.5	88.5	N	1	1	2176	867	-1	1591	399	15795	585	468	-15796	lapwing_tight.wir
387	90	1.76	4.28	15796	lapwing_tight.wir	466	270	1.76	4.28	15797															
3848	11-cdcsp-02-uwX.095	0.39	9	5		7	1	MR	0.0	10.1	75.5	75.5	N	1	1	2176	866	-1	1591	399	15795	585	467	-15796	lapwing_tight.wir
387	90	1.76	4.28	15796	lapwing_tight.wir	466	270	1.76	4.28	15797															
3848	11-cdcsp-02-uwX.095	0.39	9	5		8	1	BR	0.0	10.2	62.5	62.5	N	1	1	2176	866	-1	1591	399	15795	585	466	-15796	lapwing_tight.wir
387	90	1.76	4.28	15796	lapwing_tight.wir	466	270	1.76	4.28	15797															
3848	11-cdcsp-02-uwX.095	0.39	10	5	ASCE Ice/Wind NA	1	1	GL:E	0.0	-4.8	95.0	95.0	N	1	1	948	-482	-1	733	-217	7953	216	-265	-7955	brugg_57ay104acs-3c_115.wir
387	90	1.26	1.95	7952	brugg_57ay104acs-3c_115.wir	465	270	1.26	1.95	7954															
3848	11-cdcsp-02-uwX.095	0.39	10	5		2	1	TL	0.0	-9.9	88.5	88.5	N	1	1	2169	-649	-2	1591	-291	15792	578	-358	-15793	lapwing_tight.wir
387	90	1.76	4.28	15791	lapwing_tight.wir	464	270	1.76	4.28	15792															
3848	11-cdcsp-02-uwX.095	0.39	10	5		3	1	ML	0.0	-10.1	75.5	75.5	N	1	1	2168	-648	-2	1591	-291	15792	578	-357	-15793	lapwing_tight.wir
387	90	1.76	4.28	15791	lapwing_tight.wir	464	270	1.76	4.28	15792															
3848	11-cdcsp-02-uwX.095	0.39	10	5		4	1	BL	0.0	-10.2	62.5	62.5	N	1	1	2168	-647	-2	1591	-291	15792	578	-356	-15793	lapwing_tight.wir
387	90	1.76	4.28	15791	lapwing_tight.wir	464	270	1.76	4.28	15792															
3848	11-cdcsp-02-uwX.095	0.39	10	5		5	1	GR:E	0.0	4.8	95.0	95.0	N	1	1	871	-454	-1	671	-204	7283	199	-250	-7285	19#10_alumoweld_tight.wir
387	90	1.18	1.79	7283	19#10_alumoweld_tight.wir	466	270	1.18	1.79	7284															
3848	11-cdcsp-02-uwX.095	0.39	10	5		6	1	TR	0.0	9.9	88.5	88.5	N	1	1	2176	-653	-1	1591	-291	15797	585	-362	-15799	lapwing_tight.wir
387	90	1.76	4.28	15796	lapwing_tight.wir	466	270	1.76	4.28	15797															
3848	11-cdcsp-02-uwX.095	0.39	10	5		7	1	MR	0.0	10.1	75.5	75.5	N	1	1	2176	-653	-1	1591	-291	15797	585	-362	-15799	lapwing_tight.wir
387	90	1.76	4.28	15796	lapwing_tight.wir	466	270	1.76	4.28	15797															
3848	11-cdcsp-02-uwX.095	0.39	10	5		8	1	BR	0.0	10.2	62.5	62.5	N	1	1	2176	-654	-1	1591	-291	15797	585	-363	-15799	lapwing_tight.wir
387	90	1.76	4.28	15796	lapwing_tight.wir	466	270	1.76	4.28	15797															
3848	11-cdcsp-02-uwX.095	0.39	11	18	NU Ice	1	1	GL:E	0.0	-4.8	95.0	95.0	N	1	1	1318	64	0	952	32	9310	366	33	-9310	brugg_57ay104acs-3c_115.wir
387	90	0.00	2.77	9310	brugg_57ay104acs-3c_115.wir	465	270	0.00	2.77	9310															
3848	11-cdcsp-02-uwX.095	0.39	11	18		2	1	TL	0.0	-9.9	88.5	88.5	N	1	1	2693	132	0	1957	66	19169	735	67	-19169	lapwing_tight.wir
387	90	0.00	5.40	19169	lapwing_tight.wir	464	270	0.00	5.40	19169															
3848	11-cdcsp-02-uwX.095	0.39	11	18		3	1	ML	0.0	-10.1	75.5	75.5	N	1	1	2692	133	0	1957	66	19169	735	68	-19169	lapwing_tight.wir
387	90	0.00	5.40	19169	lapwing_tight.wir	464	270	0.00	5.40	19169															
3848	11-cdcsp-02-uwX.095	0.39	11	18		4	1	BL	0.0	-10.2	62.5	62.5	N	1	1	2692	134	0	1957	66	19169	735	69	-19169	lapwing_tight.wir
387	90	0.00	5.40	19169	lapwing_tight.wir	464	270	0.00	5.40	19169															
3848	11-cdcsp-02-uwX.095	0.39	11	18		5	1	GR:E	0.0	4.8	95.0	95.0	N	1	1	1218	58	-0	875	29	8492	343	28	-8492	19#10_alumoweld_tight.wir
387	90	0.00	2.56	8492	19#10_alumoweld_tight.wir	466	270	0.00	2.56	8492															
3848	11-cdcsp-02-uwX.095	0.39	11	18		6	1	TR	0.0	9.9	88.5	88.5	N	1	1	2701	130	-0	1957	66	19169	744	64	-19169	lapwing_tight.wir
387	90	0.00	5.40	19169	lapwing_tight.wir	466	270	0.00	5.40	19169															
3848	11-cdcsp-02-uwX.095	0.39	11	18		7	1	MR	0.0	10.1	75.5	75.5	N	1	1	2702	129	-0	1957	66	19169	744	64	-19169	lapwing_tight.wir
387	90	0.00	5.40	19169	lapwing_tight.wir	466	270	0.00	5.40	19169															
3848	11-cdcsp-02-uwX.095	0.39	11	18		8	1	BR	0.0	10.2	62.5	62.5	N	1	1	2702	128	-0	1957	66	19169	744	63	-19169	lapwing_tight.wir
387	90	0.00	5.40	19169	lapwing_tight.wir	466	270	0.00	5.40	19169															

☉ AT&T ANTENNAS  
EL. ±111'-0" ATB

HSS14X0.625 X 30' LONG PIPE

☉ MAST CONNECTION  
EL. ±93'-0" ATB

☉ MAST CONNECTION  
EL. ±83'-0" ATB

ATB DENOTES ABOVE  
TOP OF BASE PLATE

AT&T EXISTING TWENTY-FOUR  
(24) 1-5/8" DIA. COAX  
CABLES MOUNTED TO  
EXTERIOR OF THE POLE

EXISTING 96' TALL  
STEEL TRANSMISSION  
STRUCTURE NO. 3848

EXISTING TO REMOVE: NINE (9) ANDREW  
SBNH-1D6565C PANEL ANTENNAS, THREE  
(3) CCI OPA-65R-LCUU-H8 PANEL  
ANTENNAS AND TWELVE (12) CCI  
DTMABP7819VG12A TMA

PROPOSED: THREE (3) CCI TPA65R-BU8D  
PANEL ANTENNAS, THREE (3) CCI  
DMP65R-BU8D PANEL ANTENNAS, THREE  
(3) ERICSSON AIR6449 PANEL ANTENNAS,  
THREE (3) ERICSSON AIR6419 PANEL  
ANTENNAS, THREE (3) KAELUS  
TMA2124F03V5-2D TMAs, SIX (6) KAELUS  
TMA2104F00V1-1 TMAs AND ONE (1)  
DC6 SURGE ARRESTOR.

AT&T PROPOSED TWO (2) DC  
CABLES AND ONE (1) FIBER CABLE

1  
EL-1

# TOWER & MAST ELEVATION

SCALE: NOT TO SCALE

REVISIONS		
00	4/6/22	CONSTRUCTION

**CEN TEK** engineering  
Centered on Solutions™  
www.CentekEng.com  
(203) 488-0580  
(203) 488-8587 Fax  
63-2 North Branford Road, Branford, CT 06405

CT2267  
EVERSOURCE 3848  
298 RIDGE ROAD  
ORANGE, CT 06477

PROJECT NO: 22021.02  
DRAWN BY: T.J.L.  
CHECKED BY: C.F.C.  
SCALE: AS NOTED  
DATE: 4/6/22

TOWER AND MAST  
ELEVATION  
**EL-1**  
DWG. 1 OF 1

**Development of Design Heights, Exposure Coefficients, and Velocity Pressures Per TIA-222-H**

**Wind Speeds**

Basic Wind Speed	$V := 130$	mph	(User Input - 2022 CSBC Appendix P)
Basic Wind Speed with Ice	$V_i := 50$	mph	(User Input per Annex B of TIA-222-H)
Basic Wind Speed Service Loads	$V_{Ser} := 60$	mph	(User Input - TIA-222-H Section 2.8.3)

**Input**

Structure Type =	$Structure\_Type := Pole$		(User Input)
Structure Category =	$SC := III$		(User Input)
Exposure Category =	$Exp := C$		(User Input)
Structure Height =	$h := 96$	ft	(User Input)
Height to Center of Antennas =	$Z_{ant} := 111$	ft	(User Input)
Height to Center of Mast =	$Z_{Mast1} := 108$	ft	(User Input)
Radial Ice Thickness =	$t_i := 1.0$	in	(User Input per Annex B of TIA-222-H)
Radial Ice Density =	$Id := 56.00$	pcf	(User Input)
Topographic Factor =	$K_{zt} := 1.0$		(User Input)
Shielding Factor for Appurtenances =	$K_a := 1.0$		(User Input)
Mean Base Elevation of structure above sea level =	$Z_s := 150$		
Ground Elevation Factor =	$K_e := e^{-0.0000362 \cdot Z_s} = 0.995$		
Gust Response Factor =	$G_H := 1.35$		(User Input - Section 2.6.9.4 of TIA-222-H)

**Output**

Wind Direction Probability Factor =	$K_d := \begin{cases} \text{if } Structure\_Type = Pole & 0.95 \\ \text{if } Structure\_Type = Lattice & 0.85 \end{cases}$	= 0.95 (Per Table 2-2 of TIA-222-H)
Wind Direction Probability Factor (Service) =	$K_{dSer} := 0.85$	(Per Section 2.8.3 of TIA-222-H)
Importance Factors =	$I_{ice} := \begin{cases} \text{if } SC = 1 & 1.15 \\ & 0 \\ \text{if } SC = 2 & 1.00 \\ & 1.15 \\ \text{if } SC = 3 & 1.15 \\ & 1.25 \\ \text{if } SC = 4 & 1.25 \end{cases}$	= 1.15 (Per Table 2-3 of TIA-222-H)

$$K_{iz} := \left( \frac{Z_{ant}}{33} \right)^{0.1} = 1.129$$

Velocity Pressure Coefficient Antennas =

Velocity Pressure w/o Ice Antennas =

Velocity Pressure with Ice Antennas =

Velocity Pressure Service =

$$K_{izMast1} := \left( \frac{Z_{Mast1}}{33} \right)^{0.1} = 1.126$$

Velocity Pressure Coefficient Mast =

Velocity Pressure w/o Ice Mast =

Velocity Pressure with Ice Mast =

Velocity Pressure Service =

$$t_{izant} := t_i \cdot I_{ice} \cdot K_{iz} \cdot K_{zt}^{0.35} = 1.298$$

$$KZ_{ant} := 2.01 \cdot \left( \left( \frac{Z_{ant}}{zg} \right) \right)^{\frac{2}{\alpha}} = 1.294$$

$$qZ_{ant} := 0.00256 \cdot K_{zt} \cdot K_e \cdot K_d \cdot KZ_{ant} \cdot V^2 = 52.886$$

$$qZ_{ice.ant} := 0.00256 \cdot K_{zt} \cdot K_e \cdot K_d \cdot KZ_{ant} \cdot V_i^2 = 7.823$$

$$qZ_{ant.Ser} := 0.00256 \cdot K_{zt} \cdot K_e \cdot K_{dSer} \cdot KZ_{ant} \cdot V_{Ser}^2 = 10.08$$

$$t_{izMast1} := t_i \cdot I_{ice} \cdot K_{izMast1} \cdot K_{zt}^{0.35} = 1.295$$

$$KZ_{Mast1} := 2.01 \cdot \left( \left( \frac{Z_{Mast1}}{zg} \right) \right)^{\frac{2}{\alpha}} = 1.286$$

$$qZ_{Mast1} := 0.00256 \cdot K_{zt} \cdot K_e \cdot K_d \cdot KZ_{Mast1} \cdot V^2 = 52.581$$

$$qZ_{ice.Mast1} := 0.00256 \cdot K_{zt} \cdot K_e \cdot K_d \cdot KZ_{Mast1} \cdot V_i^2 = 7.778$$

$$qZ_{Mast1.Ser} := 0.00256 \cdot K_{zt} \cdot K_e \cdot K_{dSer} \cdot KZ_{Mast1} \cdot V_{Ser}^2 = 10.022$$

**Development of Wind & Ice Load on Mast**

**Mast Data:**

	(HSS14x0.625)	(User Input)
Mast Shape =	Round	(User Input)
Mast Diameter =	$D_{mast} := 14$ in	(User Input)
Mast Length =	$L_{mast} := 30$ ft	(User Input)
Mast Thickness =	$t_{mast} := 0.625$ in	(User Input)
Mast Aspect Ratio =	$A_{r_{mast}} := \frac{12 \cdot L_{mast}}{D_{mast}} = 25.7$	
Mast Force Coefficient =	$C_{a_{mast}} = 1.2$	

**Gravity Loads (without ice)**

Weight of the mast =	Self Weight	(Computed internally by Risa-3D)	plf	<b>BLC 1</b>
----------------------	-------------	----------------------------------	-----	--------------

**Gravity Loads (ice only)**

Ice Area per Linear Foot =  $A_{i_{mast}} := \frac{\pi}{4} \cdot ((D_{mast} + t_{izMast1} \cdot 2)^2 - D_{mast}^2) = 62.2$  sq in

Weight of Ice on Mast =	$W_{iCE_{mast}} := I_d \cdot \frac{A_{i_{mast}}}{144} = 24$	plf	<b>BLC 3</b>
-------------------------	-------------------------------------------------------------	-----	--------------

**Wind Load (with ice)**

Mast Projected Surface Area w/ Ice =  $A_{ICE_{mast}} := \frac{(D_{mast} + 2 \cdot t_{izMast1})}{12} = 1.382$  sf/ft

Total Mast Wind Force w/ Ice =	$qZ_{ice.Mast1} \cdot G_H \cdot C_{a_{mast}} \cdot A_{ICE_{mast}} = 17$	plf	<b>BLC 4</b>
--------------------------------	-------------------------------------------------------------------------	-----	--------------

**Wind Load (without ice)**

Mast Projected Surface Area =  $A_{mast} := \frac{D_{mast}}{12} = 1.167$  sf/ft

Total Mast Wind Force =	$qZ_{Mast1} \cdot G_H \cdot C_{a_{mast}} \cdot A_{mast} = 99$	plf	<b>BLC 5</b>
-------------------------	---------------------------------------------------------------	-----	--------------

**Wind Load (Service)**

Total Mast Wind Force Service Loads =	$qZ_{Mast1.Ser} \cdot G_H \cdot C_{a_{mast}} \cdot A_{mast} = 19$	plf	<b>BLC 6</b>
---------------------------------------	-------------------------------------------------------------------	-----	--------------

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	CCI DMP65R-BU8D	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 96$	in (User Input)
Antenna Width =	$W_{ant} := 20.7$	in (User Input)
Antenna Thickness =	$T_{ant} := 7.7$	in (User Input)
Antenna Weight =	$WT_{ant} := 115$	lbs (User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)
Antenna Aspect Ratio =	$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 4.6$	
Antenna Force Coefficient =	$Ca_{ant} = 1.3$	

**Gravity Load (without ice)**

Weight of All Antennas =  $WT_{ant} \cdot N_{ant} = 345$  lbs **BLC 2**

**Gravity Loads (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 2 \cdot 10^4$  cu in

Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant}) \cdot (T_{ant} + 2 \cdot t_{izant}) - V_{ant} = 8350$  cu in

Weight of Ice on Each Antenna =  $W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 271$  lbs

Weight of Ice on All Antennas =  $W_{ICEant} \cdot N_{ant} = 812$  lbs **BLC 3**

**Wind Load (with ice)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := \frac{(L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant})}{144} = 16$  sf

Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 47.9$  sf

Total Antenna Wind Force w/ Ice =  $F_{iant} := qz_{ice,ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ICEant} = 655$  lbs **BLC 4**

**Wind Load (without ice)**

Surface Area for One Antenna =  $SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 13.8$  sf

Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 41.4$  sf

Total Antenna Wind Force =  $F_{ant} := qz_{ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 3828$  lbs **BLC 5**

**Wind Load (Service)**

Total Antenna Wind Force Service Loads =  $F_{ant,ser} := qz_{ant,ser} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 730$  lbs **BLC 6**



**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	CCI TPA65R-BU8D	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 96$	in (User Input)
Antenna Width =	$W_{ant} := 20.7$	in (User Input)
Antenna Thickness =	$T_{ant} := 7.7$	in (User Input)
Antenna Weight =	$WT_{ant} := 110$	lbs (User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)
Antenna Aspect Ratio =	$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 4.6$	
Antenna Force Coefficient =	$Ca_{ant} = 1.3$	

**Gravity Load (without ice)**

Weight of All Antennas =  $WT_{ant} \cdot N_{ant} = 330$  lbs **BLC 2**

**Gravity Loads (ice only)**

Volume of Each Antenna =	$V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 2 \cdot 10^4$	cu in
Volume of Ice on Each Antenna =	$V_{ice} := (L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant}) \cdot (T_{ant} + 2 \cdot t_{izant}) - V_{ant} = 8350$	cu in
Weight of Ice on Each Antenna =	$W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 271$	lbs
Weight of Ice on All Antennas =	$W_{ICEant} \cdot N_{ant} = 812$	lbs <b>BLC 3</b>

**Wind Load (with ice)**

Surface Area for One Antenna w/ Ice =	$SA_{ICEant} := \frac{(L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant})}{144} = 16$	sf
Antenna Projected Surface Area w/ Ice =	$A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 47.9$	sf
Total Antenna Wind Force w/ Ice =	$F_{ant} := qZ_{ice,ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ICEant} = 655$	lbs <b>BLC 4</b>

**Wind Load (without ice)**

Surface Area for One Antenna =	$SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 13.8$	sf
Antenna Projected Surface Area =	$A_{ant} := SA_{ant} \cdot N_{ant} = 41.4$	sf
Total Antenna Wind Force =	$F_{ant} := qZ_{ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 3828$	lbs <b>BLC 5</b>

**Wind Load (Service)**

Total Antenna Wind Force Service Loads =  $F_{ant, Ser} := qZ_{ant, Ser} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 730$  lbs **BLC 6**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Ericsson AIR6419	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 31.1$	in (User Input)
Antenna Width =	$W_{ant} := 16.1$	in (User Input)
Antenna Thickness =	$T_{ant} := 7.3$	in (User Input)
Antenna Weight =	$WT_{ant} := 56$	lbs (User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)
Antenna Aspect Ratio =	$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 1.9$	
Antenna Force Coefficient =	$Ca_{ant} = 1.2$	

**Gravity Load (without ice)**

Weight of All Antennas =  $WT_{ant} \cdot N_{ant} = 168$  lbs **BLC 2**

**Gravity Loads (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 3655$  cu in

Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant}) \cdot (T_{ant} + 2 \cdot t_{izant}) - V_{ant} = 2580$  cu in

Weight of Ice on Each Antenna =  $W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 84$  lbs

Weight of Ice on All Antennas =  $W_{ICEant} \cdot N_{ant} = 251$  lbs **BLC 3**

**Wind Load (with ice)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := \frac{(L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant})}{144} = 4.4$  sf

Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 13.1$  sf

Total Antenna Wind Force w/ Ice =  $F_{iant} := qz_{ice,ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ICEant} = 166$  lbs **BLC 4**

**Wind Load (without ice)**

Surface Area for One Antenna =  $SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 3.5$  sf

Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 10.4$  sf

Total Antenna Wind Force =  $F_{ant} := qz_{ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 894$  lbs **BLC 5**

**Wind Load (Service)**

Total Antenna Wind Force Service Loads =  $F_{ant,ser} := qz_{ant,ser} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 170$  lbs **BLC 6**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Ericsson AIR6449	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 30.6$	in (User Input)
Antenna Width =	$W_{ant} := 15.9$	in (User Input)
Antenna Thickness =	$T_{ant} := 10.6$	in (User Input)
Antenna Weight =	$WT_{ant} := 96$	lbs (User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)
Antenna Aspect Ratio =	$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 1.9$	
Antenna Force Coefficient =	$Ca_{ant} = 1.2$	

**Gravity Load (without ice)**

Weight of All Antennas =  $WT_{ant} \cdot N_{ant} = 288$  lbs **BLC 2**

**Gravity Loads (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 5157$  cu in

Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant}) \cdot (T_{ant} + 2 \cdot t_{izant}) - V_{ant} = 2946$  cu in

Weight of Ice on Each Antenna =  $W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 95$  lbs

Weight of Ice on All Antennas =  $W_{ICEant} \cdot N_{ant} = 286$  lbs **BLC 3**

**Wind Load (with ice)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := \frac{(L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant})}{144} = 4.3$  sf

Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 12.8$  sf

Total Antenna Wind Force w/ Ice =  $F_{iant} := qz_{ice,ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ICEant} = 162$  lbs **BLC 4**

**Wind Load (without ice)**

Surface Area for One Antenna =  $SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 3.4$  sf

Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 10.1$  sf

Total Antenna Wind Force =  $F_{ant} := qz_{ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 868$  lbs **BLC 5**

**Wind Load (Service)**

Total Antenna Wind Force Service Loads =  $F_{ant,ser} := qz_{ant,ser} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 166$  lbs **BLC 6**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Kaelus TMA2124F03V5-2D	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 9.7$	in (User Input)
Antenna Width =	$W_{ant} := 10.4$	in (User Input)
Antenna Thickness =	$T_{ant} := 8.3$	in (User Input)
Antenna Weight =	$WT_{ant} := 36$	lbs (User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)
Antenna Aspect Ratio =	$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 0.9$	
Antenna Force Coefficient =	$Ca_{ant} = 1.2$	

**Gravity Load (without ice)**

Weight of All Antennas =  $WT_{ant} \cdot N_{ant} = 108$  lbs **BLC 2**

**Gravity Loads (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 837$  cu in

Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant}) \cdot (T_{ant} + 2 \cdot t_{izant}) - V_{ant} = 904$  cu in

Weight of Ice on Each Antenna =  $W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 29$  lbs

Weight of Ice on All Antennas =  $W_{ICEant} \cdot N_{ant} = 88$  lbs **BLC 3**

**Wind Load (with ice)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := \frac{(L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant})}{144} = 1.1$  sf

Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 3.3$  sf

Total Antenna Wind Force w/ Ice =  $F_{iant} := qz_{ice,ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ICEant} = 42$  lbs **BLC 4**

**Wind Load (without ice)**

Surface Area for One Antenna =  $SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 0.7$  sf

Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 2.1$  sf

Total Antenna Wind Force =  $F_{ant} := qz_{ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 180$  lbs **BLC 5**

**Wind Load (Service)**

Total Antenna Wind Force Service Loads =  $F_{ant,ser} := qz_{ant,ser} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 34$  lbs **BLC 6**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Kaelus TMA2104F00V1-1	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 8.7$	in (User Input)
Antenna Width =	$W_{ant} := 7.9$	in (User Input)
Antenna Thickness =	$T_{ant} := 4.1$	in (User Input)
Antenna Weight =	$WT_{ant} := 17$	lbs (User Input)
Number of Antennas =	$N_{ant} := 6$	(User Input)
Antenna Aspect Ratio =	$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 1.1$	
Antenna Force Coefficient =	$Ca_{ant} = 1.2$	

**Gravity Load (without ice)**

Weight of All Antennas =  $WT_{ant} \cdot N_{ant} = 102$  lbs **BLC 2**

**Gravity Loads (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 282$  cu in

Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant}) \cdot (T_{ant} + 2 \cdot t_{izant}) - V_{ant} = 512$  cu in

Weight of Ice on Each Antenna =  $W_{ICEant} := \frac{V_{ice}}{1728} \cdot Id = 17$  lbs

Weight of Ice on All Antennas =  $W_{ICEant} \cdot N_{ant} = 100$  lbs **BLC 3**

**Wind Load (with ice)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := \frac{(L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant})}{144} = 0.8$  sf

Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 4.9$  sf

Total Antenna Wind Force w/ Ice =  $F_{iant} := qz_{ice,ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ICEant} = 63$  lbs **BLC 4**

**Wind Load (without ice)**

Surface Area for One Antenna =  $SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 0.5$  sf

Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 2.9$  sf

Total Antenna Wind Force =  $F_{ant} := qz_{ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 245$  lbs **BLC 5**

**Wind Load (Service)**

Total Antenna Wind Force Service Loads =  $F_{ant,ser} := qz_{ant,ser} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 47$  lbs **BLC 6**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Raycap DC6-48-60-18-8C	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 31.4$	in (User Input)
Antenna Width =	$W_{ant} := 18.28$	in (User Input)
Antenna Thickness =	$T_{ant} := 10.24$	in (User Input)
Antenna Weight =	$WT_{ant} := 26$	lbs (User Input)
Number of Antennas =	$N_{ant} := 1$	(User Input)
Antenna Aspect Ratio =	$Ar_{ant} := \frac{L_{ant}}{W_{ant}} = 1.7$	
Antenna Force Coefficient =	$Ca_{ant} = 1.2$	

**Gravity Load (without ice)**

Weight of All Antennas =  $WT_{ant} \cdot N_{ant} = 26$  lbs **BLC 2**

**Gravity Loads (ice only)**

Volume of Each Antenna =  $V_{ant} = L_{ant} \cdot W_{ant} \cdot T_{ant} = 5878$  cu in

Volume of Ice on Each Antenna =  $V_{ice} = (L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant}) \cdot (T_{ant} + 2 \cdot t_{izant}) - V_{ant} = 3233$  cu in

Weight of Ice on Each Antenna =  $W_{ICEant} = \frac{V_{ice}}{1728} \cdot Id = 105$  lbs

Weight of Ice on All Antennas =  $W_{ICEant} \cdot N_{ant} = 105$  lbs **BLC 3**

**Wind Load (with ice)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := \frac{(L_{ant} + 2 \cdot t_{izant}) \cdot (W_{ant} + 2 \cdot t_{izant})}{144} = 4.9$  sf

Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 4.9$  sf

Total Antenna Wind Force w/ Ice =  $F_{iant} := qz_{ice,ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ICEant} = 62$  lbs **BLC 4**

**Wind Load (without ice)**

Surface Area for One Antenna =  $SA_{ant} := \frac{L_{ant} \cdot W_{ant}}{144} = 4$  sf

Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 4$  sf

Total Antenna Wind Force =  $F_{ant} := qz_{ant} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 342$  lbs **BLC 5**

**Wind Load (Service)**

Total Antenna Wind Force Service Loads =  $F_{ant,ser} := qz_{ant,ser} \cdot G_H \cdot Ca_{ant} \cdot K_a \cdot A_{ant} = 65$  lbs **BLC 6**

**Development of Wind & Ice Load on Antenna Mounts**

**Mount Data:**

Mount Type:	Platform w/ Handrail		
Mount Shape =	Flat		
Mount Projected Surface Area =	CaAa := 24	sf	(User Input)
Mount Projected Surface Area w/ Ice =	CaAa <sub>ice</sub> := 28	sf	(User Input)
Mount Weight =	WT <sub>mnt</sub> := 3300	lbs	(User Input)
Mount Weight w/ Ice =	WT <sub>mnt,ice</sub> := 3800	lbs	(User Input)

**Gravity Loads (without ice)**

Weight of All Mounts =  $WT_{mnt} = 3300$  lbs **BLC 2**

**Gravity Loads (ice only)**

Weight of Ice on All Mounts =  $WT_{mnt,ice} - WT_{mnt} = 500$  lbs **BLC 3**

**Wind Load (with ice)**

Total Mount Wind Force =  $F_{mnt} := qZ_{ice,ant} \cdot G_H \cdot CaAa_{ice} = 296$  lbs **BLC 4**

**Wind Load (without ice)**

Total Mount Wind Force =  $F_{mnt} := qZ_{ant} \cdot G_H \cdot CaAa = 1713$  lbs **BLC 5**

**Wind Load (Service)**

Total Mount Wind Force =  $F_{mnt} := qZ_{ant,ser} \cdot G_H \cdot CaAa = 327$  lbs **BLC 6**

**Development of Wind & Ice Load on Coax Cables**

**Cable Data:**

Type =	1-5/8"		
Shape =	Round		(User Input)
Coax Outside Diameter =	$D_{coax} := 1.98$	in	(User Input)
Coax Cable Length =	$L_{coax} := 10$	ft	(User Input)
Weight of Coax per foot =	$Wt_{coax} := 1.04$	plf	(User Input)
Total Number of Coax =	$N_{coax} := 27$		(User Input - 24 Coax, 1 Hybrid & 2 DC)
No. of Coax Projecting Outside Face of Mast =	$NP_{coax} := 6$		(User Input)
Coax aspect ratio,	$Ar_{coax} := \frac{(L_{coax} \cdot 12)}{D_{coax}} = 60.6$		
Coax Cable Force Factor Coefficient =	$Ca_{coax} = 1.2$		

**Gravity Loads (without ice)**

Weight of all cables w/o ice  $WT_{coax} := Wt_{coax} \cdot N_{coax} = 28$  plf **BLC 2**

**Gravity Loads (ice only)**

Ice Area per Linear Foot =  $Ai_{coax} := \frac{\pi}{4} \cdot ((D_{coax} + 2 \cdot t_{izMast1})^2 - D_{coax}^2) = 13.3$  sq in

Ice Weight All Coax per foot =  $WTi_{coax} := N_{coax} \cdot Id \cdot \frac{Ai_{coax}}{144} = 140$  plf **BLC 3**

**Wind Load (with ice)**

Coax projected surface area w/ Ice =  $AICE_{coax} := \frac{(NP_{coax} \cdot D_{coax} + 2 \cdot t_{izMast1})}{12} = 1.2$  sf/ft

Total Coax Wind Force w/ Ice =  $Fi_{coax} := Ca_{coax} \cdot qZ_{ice.Mast1} \cdot G_H \cdot AICE_{coax} = 15$  plf **BLC 4**

**Wind Load (without ice)**

Coax projected surface area =  $A_{coax} := \frac{(NP_{coax} \cdot D_{coax})}{12} = 1$  sf/ft

Total Coax Wind Force =  $F_{coax} := Ca_{coax} \cdot qZ_{Mast1} \cdot G_H \cdot A_{coax} = 84$  plf **BLC 5**

**Wind Load (Service )**

Total Coax Wind Force Service Loads =  $F_{coax} := Ca_{coax} \cdot qZ_{Mast1.Ser} \cdot G_H \cdot A_{coax} = 16$  plf **BLC 6**





Company : CENTEK Engineering, INC.  
 Designer : T.JL  
 Job Number : 22021.02 /AT&T 2267  
 Model Name : Strcuture #3848 - Mast

Checked By : CFC

**Model Settings**

Number of Reported Sections	5
Number of Internal Sections	100
Member Area Load Mesh Size (in <sup>2</sup> )	144
Consider Shear Deformation	Yes
Consider Torsional Warping	Yes
Approximate Mesh Size (in)	12
Transfer Forces Between Intersecting Wood Walls	Yes
Increase Wood Wall Nailing Capacity for Wind Loads	Yes
Include P-Delta for Walls	Yes
Optimize Masonry and Wood Walls	No
Maximum Number of Iterations	3
Single	No
Multiple (Optimum)	Yes
Maximum	No

Global Axis corresponding to vertical direction	Y
Convert Existing Data	Yes
Default Global Plane for z-axis	XZ
Plate Local Axis Orientation	Nodal

Hot Rolled Steel	AISC 15th (360-16): LRFD
Stiffness Adjustment	Yes (Iterative)
Notional Annex	None
Connections	AISC 14th (360-10): ASD
Cold Formed Steel	AISI 1999: ASD
Stiffness Adjustment	Yes (Iterative)
Wood	AF&PA NDS-91/97: ASD
Temperature	< 100F
Concrete	ACI 318-02
Masonry	ACI 530-05: ASD
Aluminum	AA ADM1-05: ASD
Structure Type	Building
Stiffness Adjustment	Yes (Iterative)
Stainless	AISC 14th (360-10): ASD
Stiffness Adjustment	Yes (Iterative)

Analysis Methodology	PCA Load Contour Method
Parme Beta Factor	0.65

Compression Stress Block	Rectangular Stress Block
Analyze using Cracked Sections	Yes
Leave room for horizontal rebar splices (2*d bar spacing)	Yes
List forces which were ignored for design in the Detail Report	Yes

Column Min Steel	1
Column Max Steel	8
Rebar Material Spec	ASTM A615
Warn if beam-column framing arrangement is not understood	No
Number of Shear Regions	4
Region 2 & 3 Spacing Increase Increment (in)	4

Code	UBC 1997
------	----------



Company : CENTEK Engineering, INC.  
Designer : T.JL  
Job Number : 22021.02 /AT&T 2267  
Model Name : Strcuture #3848 - Mast

Checked By : CFC

**Model Settings (Continued)**

Occupancy Cat	4
Seismic Zone	3
Base Elevation (ft)	
Include the weight of the structure in base shear calcs	No
C <sub>s</sub>	0.36
C <sub>v</sub>	0.54
T Z (sec)	
T X (sec)	
C <sub>Z</sub>	0.035
C <sub>X</sub>	0.035
R Z	8.5
R X	8.5
Ω <sub>z</sub>	1
Ω <sub>x</sub>	1
ρ Z	1
ρ X	1

**Hot Rolled Steel Properties**

	Label	E [ksj]	G [ksj]	Nu	Therm. Coeff. [ $1e^{-5}F^{-1}$ ]	Density [k/ft <sup>3</sup> ]	Yield [ksj]	Ry	Fu [ksj]	Rt
1	A36 Gr.36	29000	11154	0.3	0.65	0.49	36	1.5	58	1.2
2	A572 Gr.50	29000	11154	0.3	0.65	0.49	50	1.1	65	1.2
3	A992	29000	11154	0.3	0.65	0.49	50	1.1	65	1.2
4	A500 Gr.42	29000	11154	0.3	0.65	0.49	42	1.3	58	1.1
5	A500 Gr.46	29000	11154	0.3	0.65	0.49	46	1.2	62	1.1
6	A53 Gr. B	29000	11154	0.3	0.65	0.49	35	1.5	60	1.2

**Hot Rolled Steel Section Sets**

	Label	Shape	Type	Design List	Material	Design Rule	Area [in <sup>2</sup> ]	Iyy [in <sup>4</sup> ]	Izz [in <sup>4</sup> ]	J [in <sup>4</sup> ]
1	Mast	HSS14.000X0.625	Column	Pipe	A500 Gr.42	Typical	24.5	552	552	1100

**Hot Rolled Steel Design Parameters**

	Label	Shape	Length [ft]	Channel Conn.	a [ft]	Function
1	M1	Mast	30	N/A	N/A	Lateral

**Member Primary Data**

	Label	I Node	J Node	Section/Shape	Type	Design List	Material	Design Rule
1	M1	BOTCONN	TOPMAST	Mast	Column	Pipe	A500 Gr.42	Typical

**Node Coordinates**

	Label	X [ft]	Y [ft]	Z [ft]	Detach From Diaphragm
1	BOTCONN	0	0	0	
2	TOPCONN	0	10	0	
3	TOPMAST	0	30	0	

**Node Boundary Conditions**

	Node Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot [k-ft/rad]	Y Rot [k-ft/rad]	Z Rot [k-ft/rad]
1	BOTCONN	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
2	TOPCONN	Reaction		Reaction			

**Member Point Loads (BLC 2 : Weight of Appurtenances)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
1	M1	Y	-0.345	28
2	M1	Y	-0.33	28
3	M1	Y	-0.168	28
4	M1	Y	-0.288	28
5	M1	Y	-0.108	28
6	M1	Y	-0.102	28
7	M1	Y	-0.026	28
8	M1	Y	-3.3	28

**Member Point Loads (BLC 3 : Weight of Ice Only)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
1	M1	Y	-0.812	28
2	M1	Y	-0.812	28

**Member Point Loads (BLC 3 : Weight of Ice Only) (Continued)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
3	M1	Y	-0.251	28
4	M1	Y	-0.286	28
5	M1	Y	-0.088	28
6	M1	Y	-0.1	28
7	M1	Y	-0.105	28
8	M1	Y	-0.5	28

**Member Point Loads (BLC 4 : TIA Wind with Ice)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
1	M1	X	0.655	28
2	M1	X	0.655	28
3	M1	X	0.166	28
4	M1	X	0.162	28
5	M1	X	0.042	28
6	M1	X	0.063	28
7	M1	X	0.062	28
8	M1	X	0.296	28

**Member Point Loads (BLC 5 : TIA Wind)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
1	M1	X	3.828	28
2	M1	X	3.828	28
3	M1	X	0.894	28
4	M1	X	0.868	28
5	M1	X	0.18	28
6	M1	X	0.245	28
7	M1	X	0.342	28
8	M1	X	1.713	28

**Member Point Loads (BLC 6 : Service Wind)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
1	M1	X	0.73	28
2	M1	X	0.73	28
3	M1	X	0.17	28
4	M1	X	0.166	28
5	M1	X	0.034	28
6	M1	X	0.047	28
7	M1	X	0.065	28
8	M1	X	0.327	28

**Member Distributed Loads (BLC 2 : Weight of Appurtenances)**

	Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
1	M1	Y	-0.028	-0.028	0	26

**Member Distributed Loads (BLC 3 : Weight of Ice Only)**

	Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
1	M1	Y	-0.024	-0.024	20	%100
2	M1	Y	-0.024	-0.024	0	20
3	M1	Y	-0.14	-0.14	0	26

**Member Distributed Loads (BLC 4 : TIA Wind with Ice)**

Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
1 M1	X	0.017	0.017	20	%100
2 M1	X	0.017	0.017	0	20
3 M1	X	0.015	0.015	0	26

**Member Distributed Loads (BLC 5 : TIA Wind)**

Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
1 M1	X	0.099	0.099	20	%100
2 M1	X	0.099	0.099	0	20
3 M1	X	0.084	0.084	0	26

**Member Distributed Loads (BLC 6 : Service Wind)**

Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
1 M1	X	0.019	0.019	20	%100
2 M1	X	0.019	0.019	0	20
3 M1	X	0.016	0.016	0	26

**Basic Load Cases**

	BLC Description	Category	Y Gravity	Point	Distributed
1	Self Weight	None	-1		
2	Weight of Appurtenances	None		8	1
3	Weight of Ice Only	None		8	3
4	TIA Wind with Ice	None		8	3
5	TIA Wind	None		8	3
6	Service Wind	None		8	3

**Load Combinations**

	Description	Solve	P-Delta	BLC	Factor	BLC	Factor	BLC	Factor	BLC	Factor
1	1.2D + 1.0W	Yes	Y	1	1.2	2	1.2	5	1		
2	0.9D + 1.0W	Yes	Y	1	0.9	2	0.9	5	1		
3	1.2D + 1.0Di + 1.0W	Yes	Y	1	1.2	2	1.2	3	1	4	1
4	1.0D + 1.0WService	Yes	Y	1	1	2	1	6	1		

**Envelope Node Reactions**

Node Label		X [k]	LC	Y [k]	LC	Z [k]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
0 BOTCONN	max	35.198	1	16.789	3	0	4	0	4	0	4	-20.292	3
1	min	6.243	3	7.106	2	0	1	0	1	0	1	-114.435	1
2 TOPCONN	max	-9.244	3	0	4	0	4	0	4	0	4	0	4
3	min	-52.25	1	0	1	0	1	0	1	0	1	0	1
4 Totals:	max	-3.001	3	16.789	3	0	4						
5	min	-17.052	1	7.106	2	0	1						

**Envelope Node Displacements**

Node Label		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation [rad]	LC	Y Rotation [rad]	LC	Z Rotation [rad]	LC
0 BOTCONN	max	0	3	0	2	0	4	0	4	0	4	0	1
1	min	0	1	0	3	0	1	0	1	0	1	0	3
2 TOPCONN	max	0	1	-0.001	2	0	4	0	4	0	4	-1.288e-3	3



Company : CENTEK Engineering, INC.  
 Designer : T.JL  
 Job Number : 22021.02 /AT&T 2267  
 Model Name : Strcuture #3848 - Mast

Checked By : CFC

**Envelope Node Displacements (Continued)**

Node Label		X [in]	LC	Y [in]	LC	Z [in]	LC	X Rotation [rad]	LC	Y Rotation [rad]	LC	Z Rotation [rad]	LC
3	min	0	3	-0.003	3	0	1	0	1	0	1	-7.264e-3	1
4	TOPMAST	max	5.833	1	-0.003	2	0	4	4	0	4	-5.552e-3	3
5	min	1.035	3	-0.007	3	0	1	0	1	0	1	-3.129e-2	1

**Envelope AISC 15TH (360-16): LRFD Member Steel Code Checks**

Member	Shape	Code Check	Loc[ft]	LC	Shear	Check	Loc[ft]	LC	phi*Pnc [k]	phi*Pnt [k]	phi*Mn y-y [k-ft]	phi*Mn z-z [k-ft]	Cb	Eqn
0	M1	HSS14.000X0.625	0.752	10	1	0.133	10	1	650.468	926.1	330.75	330.75	1	H1-1b



Company : CENTEK Engineering, INC.  
Designer : T.JL  
Job Number : 22021.02 /AT&T 2267  
Model Name : Strcuture #3848 - Mast

Checked By : CFC

**Node Reactions**

	LC	Node Label	X [k]	Y [k]	Z [k]	MX [k-ft]	MY [k-ft]	MZ [k-ft]
0	1	BOTCONN	35.198	9.475	0	0	0	-114.435
1	1	TOPCONN	-52.25	0	0	0	0	0
2	1	Totals:	-17.052	9.475	0			
3	1	COG (ft):	X: 0	Y: 22.499	Z: 0			



Company : CENTEK Engineering, INC.  
Designer : T.JL  
Job Number : 22021.02 /AT&T 2267  
Model Name : Strcuture #3848 - Mast

Checked By : CFC

---

**Node Reactions**

---

	LC	Node Label	X [k]	Y [k]	Z [k]	MX [k-ft]	MY [k-ft]	MZ [k-ft]
0	2	BOTCONN	35.125	7.106	0	0	0	-114.199
1	2	TOPCONN	-52.177	0	0	0	0	0
2	2	Totals:	-17.052	7.106	0			
3	2	COG (ft):	X: 0	Y: 22.499	Z: 0			





Company : CENTEK Engineering, INC.  
Designer : T.JL  
Job Number : 22021.02 /AT&T 2267  
Model Name : Strcuture #3848 - Mast

Checked By : CFC

**Node Reactions**

	LC	Node Label	X [k]	Y [k]	Z [k]	MX [k-ft]	MY [k-ft]	MZ [k-ft]
0	3	BOTCONN	6.243	16.789	0	0	0	-20.292
1	3	TOPCONN	-9.244	0	0	0	0	0
2	3	Totals:	-3.001	16.789	0			
3	3	COG (ft):	X: 0	Y: 21.086	Z: 0			



Code Check (Env)	
Black	No Calc
Red	> 1.0
Magenta	.90-.1.0
Green	.75-.90
Cyan	.50-.75
Blue	0-.50



Member Code Checks Displayed (Enveloped)  
Envelope Only Solution

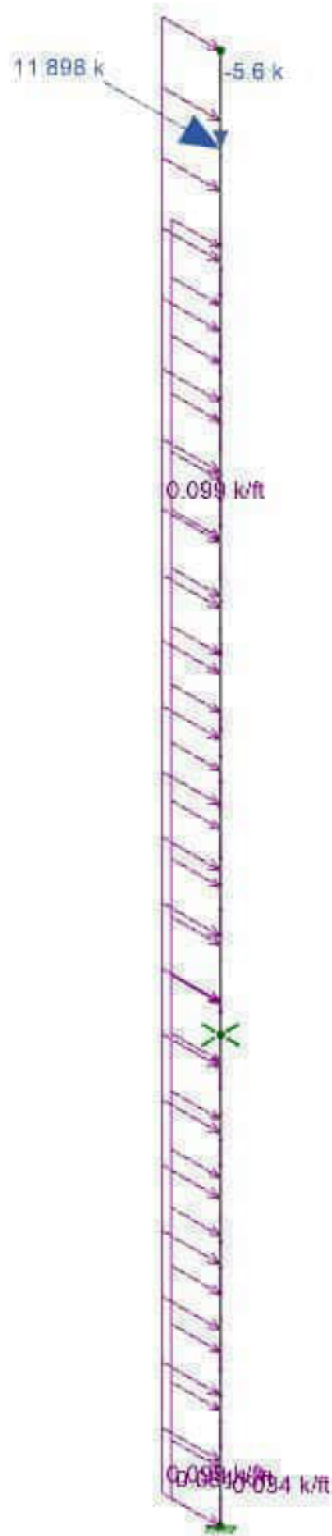


CENTEK Engineering, INC.  
TJL  
22021.02 /AT&T 2267

Structure #3848 - Mast

SK-1

TIA.r3d



Loads: LG 1, 1.2D + 1.0W



CENTEK Engineering, INC.  
TJL  
22021.02 /AT&T 2267

Structure #3848 - Mast

SK-1

TIA.r3d



Member Code Checks Displayed  
Results for LC 1, 1.2D + 1.0W  
Reaction and Moment Units are kips and kip-ft.

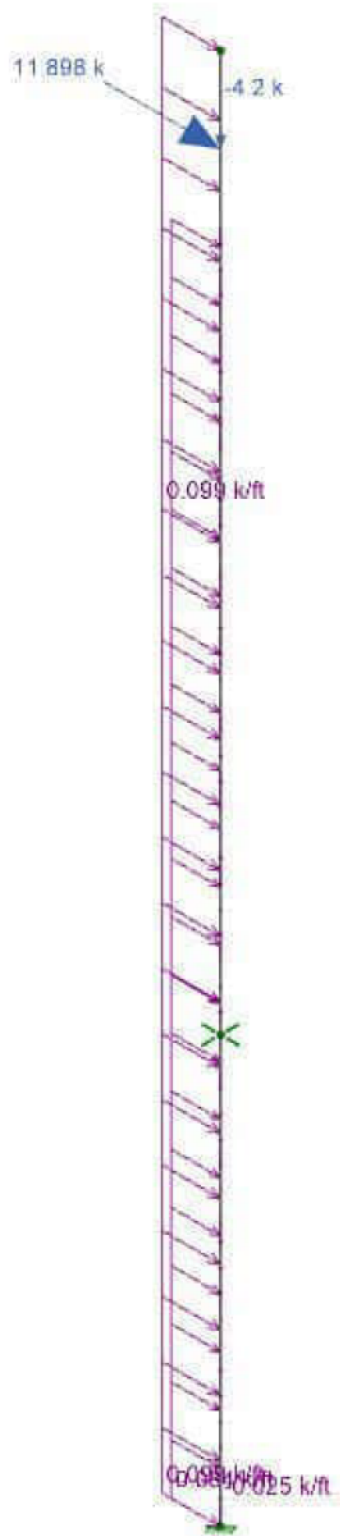


CEN TEK Engineering, INC.  
TJL  
22021.02 /AT&T 2267

Structure #3848 - Mast

SK-2

TIA.r3d



Loads: LC 2, 0.9D + 1.0W



CENTEK Engineering, INC.  
TJL  
22021.02 /AT&T 2267

Structure #3848 - Mast

SK-2

TIA.r3d



Member Code Checks Displayed  
Results for LC 2, 0.9D + 1.0W  
Reaction and Moment Units are kips and kip-ft.



CEN TEK Engineering, INC.  
TJL  
22021.02 /AT&T 2267

Structure #3848 - Mast

SK-3

TIA.r3d



Loads: LC 3, 1.2D + 1.0Di + 1.0Wi



CENTEK Engineering, INC.  
TJL  
22021.02 / AT&T 2267

Structure #3848 - Mast

SK-4

TIA.r3d



Member Code Checks Displayed  
Results for LC 3, 1.2D + 1.0D + 1.0W1  
Reaction and Moment Units are kips and kip-ft.



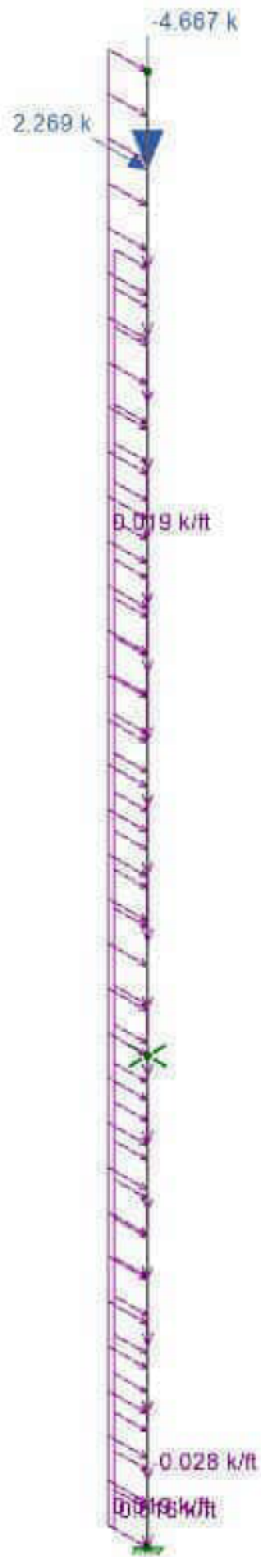
CEN TEK Engineering, INC.  
TJL  
22021.02 / AT&T 2267

Structure #3848 - Mast

SK-4

TIA.r3d





Loads: LG 4, 1.0D + 1.0W Service



CEN TEK Engineering, INC.  
TJL  
22021.02 / AT&T 2267

Structure #3848 - Mast

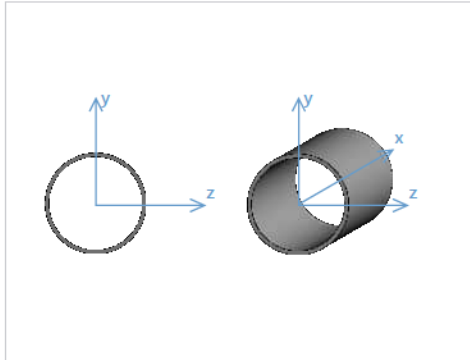
SK-5

TIA.r3d

## Detail Report: M1

Load Combination: LC 4: 1.0D +  
1.0WService

Code check: 0.147 (axial/bending)

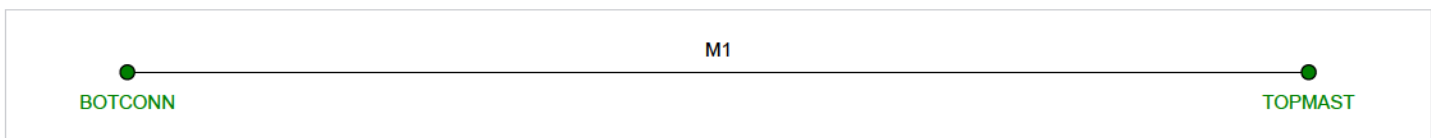


Input Data			
Shape:	HSS14.000X0.625	I Node:	BOTCONN
Member Type:	Column	J Node:	TOPMAST
Length (ft):	30	I Release:	Fixed
Material Type:	Hot Rolled Steel	J Release:	Fixed
Design Rule:	Typical	I Offset:	N/A
Internal Sections:	97	J Offset:	N/A
Design Code:	AISC 15th (360-16): LRFD	T/C Only:	Both Way

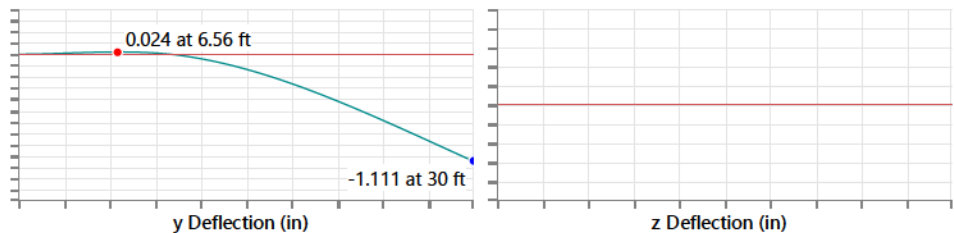
Material Properties			
Material:	A500 Gr.42	Therm. Coeff. (/1E5 F):	0.65
E (ksi):	29000	Density (k/ft <sup>3</sup> ):	0.49
G (ksi):	11154	F <sub>y</sub> (ksi):	42
Nu:	0.3	R <sub>y</sub> :	1.3
		F <sub>u</sub> (ksi):	58
		R <sub>t</sub> :	1.1

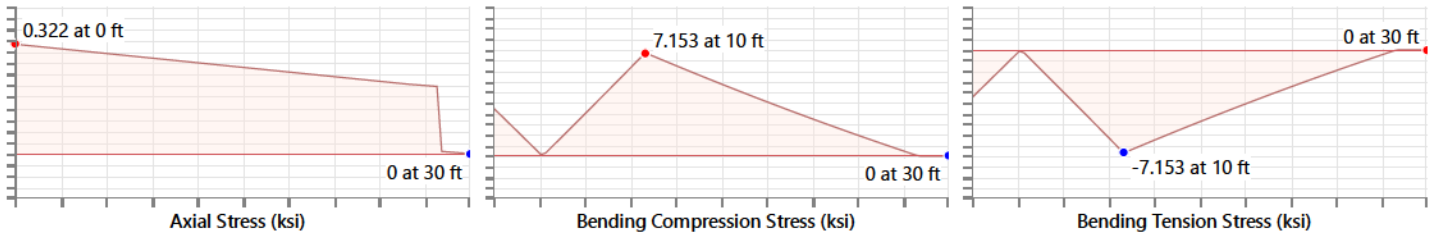
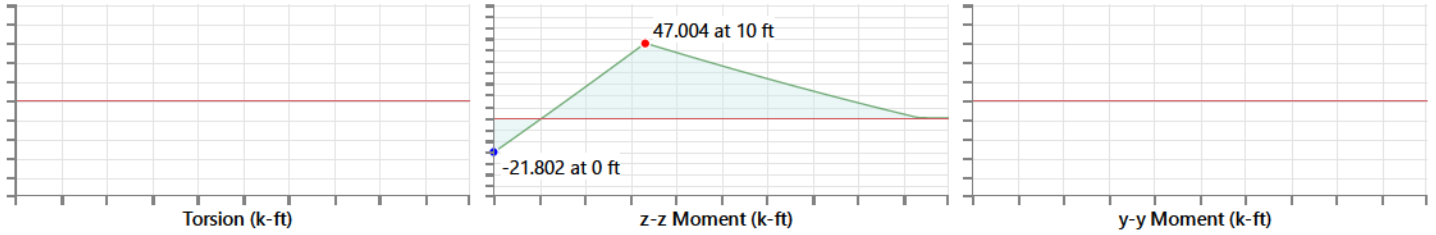
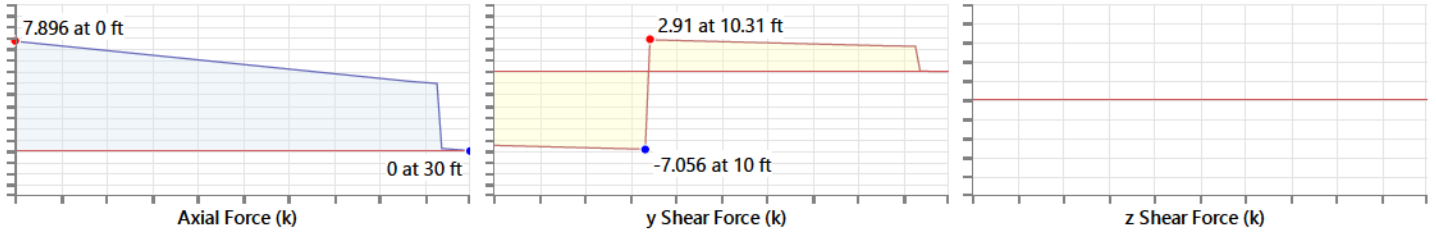
Shape Properties			
d (in):	14	Area (in <sup>2</sup> ):	24.5
t (in):	0.581	J (in <sup>4</sup> ):	1100
Z (in <sup>3</sup> ):	105	I <sub>yy</sub> (in <sup>4</sup> ):	552
		I <sub>zz</sub> (in <sup>4</sup> ):	552

Design Properties			
L <sub>b y-y</sub> (ft):	30	K <sub>y-y</sub> :	1
L <sub>b z-z</sub> (ft):	30	K <sub>z-z</sub> :	1
L <sub>comp top</sub> (ft):	30	y sway:	No
L <sub>comp bot</sub> (ft):	30	z sway:	No
L <sub>torque</sub> (ft):	30	Function:	Lateral
		Seismic DR:	None
		Max Defl Ratio:	L/324
		Max Defl Location:	30
		Span:	N/A
		τ <sub>b</sub> :	1



**Diagrams:**





### AISC 15th (360-16): LRFD Code Check

Limit State	Required	Available	Unity Check	Result
Applied Loading - Bending/Axial	-	-	-	-
Applied Loading - Shear + Torsion	-	-	-	-
Axial Tension Analysis	0 k	926.1 k	-	-
Axial Compression Analysis	6.782 k	650.468 k	-	-
Flexural Analysis	47.004 k-ft	330.75 k-ft	-	-
Shear Analysis	7.056 k	277.83 k	0.025	PASS
Bending & Axial Interaction Check (UC Bending Max)	-	-	0.147	PASS
Torsional Analysis	0 k-ft	310.598 k-ft	0	PASS

**Mast Top Connection:**

**Maximum Design Reactions at Brace:**

Vertical =  $Vert := 0 \cdot kips$  (User Input)

Horizontal =  $Horz := 52.5 \cdot kips$  (User Input)

Moment =  $Moment := 0$  (User Input)

**Bolt Data:**

Bolt Grade = A325 (User Input)

Number of Bolts =  $n_b := 6$  (User Input)

Bolt Diameter =  $d_b := 0.75 \cdot in$  (User Input)

Nominal Tensile Strength =  $F_{nt} := 90 \cdot ksi$  (User Input)

Nominal Shear Strength =  $F_{nv} := 54 \cdot ksi$  (User Input)

Resistance Factor =  $\phi := 0.75$  (User Input)

Bolt Eccentricity from C.L. Mast =  $e := 22.25 \cdot in$  (User Input)

Vertical Spacing Between Top and Bottom Bolts =  $S_{vert} := 9 \cdot in$  (User Input)

Horizontal Spacing Between Bolts =  $S_{horz} := 24 \cdot in$  (User Input)

Bolt Area =  $a_b := \frac{1}{4} \cdot \pi \cdot d_b^2 = 0.442 \text{ in}^2$

**Check Bolt Stresses:**

**Wind Acting Parallel to Stiffener Plate:**

Shear Stress per Bolt =

$$f_v := \frac{Vert}{n_b \cdot a_b} = 0 \text{ ksi}$$

Condition1 := if ( $f_v < \phi \cdot F_{nv}$ , "OK", "Overstressed")

Condition1 = "OK"

$$\frac{f_v}{(\phi \cdot F_{nv})} = 0$$

Tensile Stress Adjusted for Shear =

$$F'_{nt} := \begin{cases} \text{if } 1.3 \cdot F_{nt} - \frac{F_{nt}}{\phi \cdot F_{nv}} \cdot f_v \leq F_{nt} & = 90 \text{ ksi} \\ \left( 1.3 \cdot F_{nt} - \frac{F_{nt}}{\phi \cdot F_{nv}} \cdot f_v \right) \\ \text{else} \\ F_{nt} \end{cases}$$

Tension Force Each Bolt =

Tension Stress Each Bolt =

$$F_{tension.bolt} := \frac{Horz}{n_b} + \frac{Vert \cdot e}{S_{vert} \cdot 2} = 8.75 \text{ kips}$$

$$f_t := \frac{F_{tension.bolt}}{a_b} = 19.8 \text{ ksi}$$

Condition2 := if ( $f_t < \phi \cdot F'_{nt}$ , "OK", "Overstressed")

Condition2 = "OK"

$$\frac{f_t}{(\phi \cdot F'_{nt})} = 29.3\%$$

**Wind Acting Perpendicular to Stiffener Plate:**

Shear Stress per Bolt =

$$f_v := \frac{\sqrt{Vert^2 + Horz^2}}{n_b \cdot a_b} = 19.806 \text{ ksi}$$

Condition3 := if ( $f_v < \phi \cdot F_{nv}$ , "OK", "Overstressed")

Condition3 = "OK"

$$\frac{f_v}{(\phi \cdot F_{nv})} = 48.9\%$$

Tensile Stress Adjusted for Shear =

$$F'_{nt} := \begin{cases} \text{if } 1.3 \cdot F_{nt} - \frac{F_{nt}}{\phi \cdot F_{nv}} \cdot f_v \leq F_{nt} & = 72.99 \text{ ksi} \\ \left( 1.3 \cdot F_{nt} - \frac{F_{nt}}{\phi \cdot F_{nv}} \cdot f_v \right) \\ \text{else} \\ F_{nt} \end{cases}$$

Tension Force per Bolt =

Tension Stress Each Bolt =

$$F_{tension.conn} := \frac{Horz \cdot e}{S_{horz} \cdot \frac{n_b}{2}} + \frac{Vert \cdot e}{S_{vert} \cdot 2} = 16.224 \text{ kips}$$

$$f_t := \frac{F_{tension.conn}}{a_b} = 36.724 \text{ ksi}$$

Condition4 := if ( $f_t < \phi \cdot F'_{nt}$ , "OK", "Overstressed")

Condition4 = "OK"

$$\frac{f_t}{(\phi \cdot F'_{nt})} = 67.1\%$$

**Mast Connection to Bottom Bracket:**

**Design Reactions at Brace:**

Axial (Max) =  $Axial_{max} := 16.8 \cdot kip$  (User Input)

Axial (Min) =  $Axial_{min} := 7.1 \cdot kip$  (User Input)

Horz =  $Horz := 35.4 \cdot kip$  (User Input)

Moment =  $Moment := 115 \cdot kip \cdot ft$  (User Input)

**Anchor Bolt Data:**

Bolt Grade = A325 (User Input)

Design Shear Stress =  $F_v := 40.5 \cdot ksi$  (User Input)

Design Tension Stress =  $F_T := 67.5 \cdot ksi$  (User Input)

Total Number of Bolts =  $n_b := 4$  (User Input)

Number of Bolts Tension Side Parallel =  $n_{b,par} := 2$  (User Input)

Number of Bolts Tension Side Diagonal =  $n_{b,diag} := 1$  (User Input)

Bolt Diameter =  $d_b := 1.25 \cdot in$  (User Input)

Bolt Spacing X Direction =  $S_x := 14 \cdot in$  (User Input)

Bolt Spacing Z Direction =  $S_z := 14 \cdot in$  (User Input)

**Base Plate Data:**

Base Plate Steel = A36 (User Input)

Allowable Yield Stress =  $F_y := 36 \cdot ksi$  (User Input)

Base Plate Width =  $Pl_w := 18 \cdot in$  (User Input)

Base Plate Thickness =  $Pl_t := 1.75 \cdot in$  (User Input)

Bolt Edge Distance =  $B_E := 2 \cdot in$  (User Input)

Pole Diameter =  $D_p := 14 \cdot in$  (User Input)

**Base Plate Data:**

Weld Grade = E70XX (User Input)

Weld Yield Stress =  $F_{yw} := 70 \cdot ksi$  (User Input)

Weld Size =  $sw := 0.5 \cdot in$  (User Input)

**Anchor Bolt Check:**

Bolt Area =

$$a_b := \frac{1}{4} \cdot \pi \cdot d_b^2 = 1.227 \text{ in}^2$$

Shear per bolt =

$$V_{\text{bolt}} := \frac{\text{Horz}}{n_b} = 8.85 \text{ kip}$$

Actual Shear Stress =

$$f_v := \frac{V_{\text{bolt}}}{a_b} = 7.21 \text{ ksi}$$

Condition1 := if ( $f_v < F_v$ , "OK", "Overstressed")

Condition1 = "OK"

Bolt Spacing Diag. Direction =

$$S_{\text{diag}} := \sqrt{S_x^2 + S_z^2} = 19.8 \text{ in}$$

Tension Load per Bolt Parallel =

$$T_{\text{par}} := \frac{\text{Moment}}{S_x \cdot n_{b,\text{par}}} - \frac{\text{Axial}_{\text{min}}}{n_b} = 47.51 \text{ kip}$$

Tension Load per Bolt Diagonal =

$$T_{\text{diag}} := \frac{\text{Moment}}{S_{\text{diag}} \cdot n_{b,\text{diag}}} - \frac{\text{Axial}_{\text{min}}}{n_b} = 67.93 \text{ kip}$$

Tension per bolt =

$$T := \text{if} (T_{\text{par}} > T_{\text{diag}}, T_{\text{par}}, T_{\text{diag}}) = 67.926 \text{ kip}$$

Actual Tensile Stress =

$$f_t := \frac{T}{a_b} = 55.35 \text{ ksi}$$

Condition2 := if ( $f_t < F_T$ , "OK", "Overstressed")

Condition2 = "OK"

**Base Plate Check:**

Design Bending Stress =

$$F_b := 0.9 \cdot F_y = 32.4 \text{ ksi}$$

Plate Bending Width =

$$Z := (P_{lw} \cdot \sqrt{2} - D_p) = 11.46 \text{ in}$$

Moment Arm =

$$K := \frac{(S_{\text{diag}} - D_p)}{2} = 2.9 \text{ in}$$

Load per Bolt Diagonal =

$$P_{\text{diag}} := \frac{\text{Moment}}{S_{\text{diag}} \cdot n_{b,\text{diag}}} + \frac{\text{Axial}_{\text{max}}}{n_b} = 73.9 \text{ kip}$$

Moment in Base Plate =

$$M := K \cdot P_{\text{diag}} = 214.27 \text{ kip} \cdot \text{in}$$

Plastic Section Modulus =

$$Z := \frac{1}{4} \cdot Z \cdot P_{lw}^2 = 8.77 \text{ in}^3$$

Bending Stress =

$$f_b := \frac{M}{Z} = 24.43 \text{ ksi}$$

Condition3 := if ( $f_b < F_b$ , "OK", "Overstressed")

Condition3 = "OK"

**Base Plate to PCS Mast Weld Check:**

Design Weld Stress =

$$F_w := 0.45 \cdot F_{yw} = 31.5 \text{ ksi}$$

Weld Area =

$$A_w := \frac{\pi}{4} \cdot ((D_p + 2 \cdot sw \cdot 0.707)^2 - D_p^2) = 15.94 \text{ in}^2$$

Weld Moment of Inertia =

$$I_w := \frac{\pi}{64} \cdot ((D_p + 2 \cdot sw \cdot 0.707)^4 - D_p^4) = 410.76 \text{ in}^4$$

$$c := \frac{D_p}{2} + sw \cdot 0.707 = 7.35 \text{ in}$$

Section Modulus of Weld =

$$S_w := \frac{I_w}{c} = 55.86 \text{ in}^3$$

Weld Stress =

$$f_w := \frac{\text{Moment}}{S_w} + \frac{\text{Horz}}{A_w} = 26.93 \text{ ksi}$$

Condition4 := if ( $f_w < F_w$ , "OK", "Overstressed")

Condition4 = "OK"



**Gusset Plate Data:**

Yield Strength =	$F_y := 36 \cdot ksi$	(User Input)
Tensile Strength =	$F_u := 58 \cdot ksi$	(User Input)
Plate Height =	$Pl_h := 15 \cdot in$	(User Input)
Plate Thickness =	$Pl_t := 0.5 \cdot in$	(User Input)
Number of Plates =	$n_{pl} := 3$	(User Input)
Distance from CL Pole to Face of Collar =	$d := 9.5 \cdot in$	(User Input)
Section Modulus Gusset Assembly =	$S_x := 92 \cdot in^3$	(User Input)
Dist. Between Outer 2 Gusset Plates =	$d_{pl} := 19.5 \cdot in$	(User Input)

**Resistance Factors:**

Yielding Factor =	$\phi_t := 0.9$	(User Input)
Rupture Factor =	$\phi_r := 0.75$	(User Input)
Shear Factor =	$\phi_v := 0.9$	(User Input)

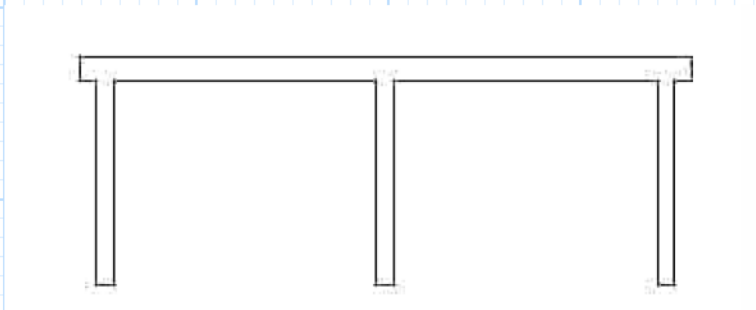


Plate Gross Area =	$A_g := Pl_h \cdot Pl_t = 7.5 \cdot in^2$
Effective Net Area =	$A_{en} := A_g = 7.5 \cdot in^2$
Tensile Yielding =	$P_{at} := \phi_t \cdot F_y \cdot A_g = 243 \cdot kip$
Tensile Rupture =	$P_{ar} := \phi_r \cdot F_u \cdot A_{en} = 326.25 \cdot kip$
Design Tension =	$P_a := \min(P_{at}, P_{ar}) = 243 \cdot kip$
Design Shear =	$V_n := \phi_v \cdot 0.6 \cdot F_y \cdot A_g = 145.8 \cdot kip$
Design Bending Stress =	$F_b := 0.9 \cdot F_y = 32.4 \cdot ksi$

**Wind Acting Parallel to Stiffener Plates:**

Moment Parallel =  $M_{par} := \text{Moment} + \text{Axial}_{max} \cdot d = (1.5 \cdot 10^3) \text{ kip} \cdot \text{in}$

Bending Stress =  $f_b := \frac{M_{par}}{S_x} = 16.73 \text{ ksi}$

Max Tension =  $T_{max} := \frac{\text{Horz}}{n_{plt}} = 11.8 \text{ kip}$

Max Shear =  $V_{max} := \frac{\text{Axial}_{max}}{n_{plt}} = 5.6 \text{ kip}$

Condition4 := if  $\left( \frac{f_b}{F_b} + \frac{T_{max}}{Pa.} + \frac{V_{max}}{V_n} < 1, \text{"OK"}, \text{"Overstressed"} \right)$

Condition4 = "OK"  $\frac{f_b}{F_b} + \frac{T_{max}}{Pa.} + \frac{V_{max}}{V_n} = 60.3\%$

**Wind Acting Perpendicular to Stiffener Plates:**

Moment Parallel =  $M_{par} := \text{Axial}_{max} \cdot d = 159.6 \text{ in} \cdot \text{kip}$

Moment Perpendicular =  $M_{perp} := \text{Horz} \cdot d = 336.3 \text{ in} \cdot \text{kip}$

Bending Stress =  $f_b := \frac{M_{par}}{S_x} = 1.73 \text{ ksi}$

Max Tension =  $T_{max} := \frac{M_{perp}}{d_{plt}} = 17.246 \text{ kip}$

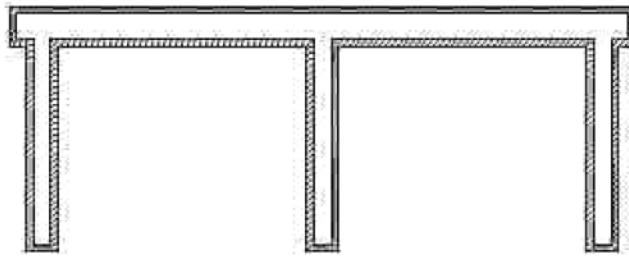
Max Shear =  $V_{max} := \frac{\text{Axial}_{max} + \text{Horz}}{n_{plt}} + \frac{\text{Moment}}{d_{plt}} = 88.169 \text{ kip}$

Condition5 := if  $\left( \frac{f_b}{F_b} + \frac{T_{max}}{Pa.} + \frac{V_{max}}{V_n} < 1, \text{"OK"}, \text{"Overstressed"} \right)$

Condition5 = "OK"  $\frac{f_b}{F_b} + \frac{T_{max}}{Pa.} + \frac{V_{max}}{V_n} = 72.9\%$

**Weld Data:**

Weld Yield Stress =	$F_{yw} := 70 \cdot \text{ksi}$	(User Input)
Design Weld Stress =	$F_w := 0.45 \cdot F_{yw} = 31.5 \text{ ksi}$	
Weld Size =	$sw := 0.3125 \cdot \text{in}$	(User Input)
Weld Area =	$A_w := 29.5 \cdot \text{in}^2$	(User Input)
Section Modulus of Weld =	$S_x := 82.4 \cdot \text{in}^3$	(User Input)
Section Modulus of Weld =	$S_z := 160 \cdot \text{in}^3$	(User Input)
Weld Area of 1 Gusset =	$A_{w1} := 2.8 \cdot \text{in}^2$	(User Input)



**Wind Acting Parallel to Stiffener Plates:**

Moment Parallel =	$M_{par} := \text{Moment} + \text{Axial}_{max} \cdot d = (1.5 \cdot 10^3) \text{ in} \cdot \text{kip}$	
Weld Stress =	$f_w := \frac{M_{par}}{S_x} + \frac{\text{Axial}_{max} + \text{Horz}}{A_w} = 20.45 \text{ ksi}$	
	Condition6 := if ( $f_w < F_w$ , "OK", "Overstressed")	$\frac{f_w}{F_w} = 64.9\%$
	Condition6 = "OK"	

**Wind Acting Perpendicular to Stiffener Plates:**

Moment Parallel =	$M_{par} := \text{Axial}_{max} \cdot d = 159.6 \text{ in} \cdot \text{kip}$	
Moment Perpendicular =	$M_{perp} := \text{Horz} \cdot d = 336.3 \text{ in} \cdot \text{kip}$	
Weld Stress =	$f_w := \frac{M_{par}}{S_x} + \frac{M_{perp}}{S_z} + \frac{\text{Axial}_{max} + \text{Horz}}{A_w} + \frac{\text{Moment}}{(d_{plt} \cdot A_{w1})} = 31.08 \text{ ksi}$	
	Condition7 := if ( $f_w < F_w$ , "OK", "Overstressed")	$\frac{f_w}{F_w} = 98.7\%$
	Condition7 = "OK"	

**Mast Bottom Connection:**

**Maximum Design Reactions at Brace:**

Vertical =  $Vert := 16.8 \cdot kips$  (User Input)

Horizontal =  $Horz := 35.4 \cdot kips$  (User Input)

Moment =  $Moment := 115 \cdot ft \cdot kips$  (User Input)

**Bolt Data:**

Bolt Grade = A325 (User Input)

Number of Bolts =  $n_b := 12$  (User Input)

Bolt Diameter =  $d_b := 1 \cdot in$  (User Input)

Nominal Tensile Strength =  $F_{nt} := 90 \cdot ksi$  (User Input)

Nominal Shear Strength =  $F_{nv} := 54 \cdot ksi$  (User Input)

Resistance Factor =  $\phi := 0.75$  (User Input)

Bolt Eccentricity from C.L. Mast =  $e := 22.25 \cdot in$  (User Input)

Horizontal Spacing Between Bolts =  $S_{horz} := 29.375 \cdot in$  (User Input)

Bolt Area =  $a_b := \frac{1}{4} \cdot \pi \cdot d_b^2 = 0.785 \text{ in}^2$

Vertical Spacing From Plate CL to Bolt 1 =  $d_1 := 2 \cdot in$  (User Input)

Vertical Spacing From Plate CL to Bolt 2 =  $d_2 := 6 \cdot in$  (User Input)

Vertical Spacing From Plate CL to Bolt 3 =  $d_3 := 10 \cdot in$  (User Input)

Bolt Polar Moment of Inertia =  $I_p := 4 \cdot (d_1)^2 + 4 \cdot (d_2)^2 + 4 \cdot (d_3)^2 = 560 \text{ in}^2$

**Check Bolt Stresses:**

**Wind Acting Parallel to Stiffener Plate:**

Shear Stress per Bolt =

$$f_v := \frac{Vert}{n_b \cdot a_b} = 1.783 \text{ ksi}$$

Condition1 := if ( $f_v < \phi \cdot F_{nv}$ , "OK", "Overstressed")

$$\frac{f_v}{(\phi \cdot F_{nv})} = 4.4\%$$

Condition1 = "OK"

Tensile Stress Adjusted for Shear =

$$F'_{nt} := \begin{cases} \text{if } 1.3 \cdot F_{nt} - \frac{F_{nt}}{\phi \cdot F_{nv}} \cdot f_v \leq F_{nt} & = 90 \text{ ksi} \\ \left( 1.3 \cdot F_{nt} - \frac{F_{nt}}{\phi \cdot F_{nv}} \cdot f_v \right) \\ \text{else} \\ F_{nt} \end{cases}$$

Tension Force Each Bolt =

$$F_{tension.bolt} := \frac{Horz}{n_b} + \frac{(Vert \cdot e + Moment) \cdot d_3}{I_p} = 34.3 \text{ kips}$$

Tension Stress Each Bolt =

$$f_t := \frac{F_{tension.bolt}}{a_b} = 43.6 \text{ ksi}$$

Condition2 := if ( $f_t < \phi \cdot F'_{nt}$ , "OK", "Overstressed")

$$\frac{f_t}{(\phi \cdot F'_{nt})} = 64.6\%$$

Condition2 = "OK"

**Wind Acting Perpendicular to Stiffener Plate:**

Shear Stress per Bolt =

$$f_v := \sqrt{\left( \frac{Vert}{n_b \cdot a_b} + \frac{(Moment) \cdot 2}{S_{horz} \cdot n_b \cdot a_b} \right)^2 + \left( \frac{Horz}{n_b \cdot a_b} \right)^2} = 12.337 \text{ ksi}$$

Condition3 := if ( $f_v < \phi \cdot F_{nv}$ , "OK", "Overstressed")

$$\frac{f_v}{(\phi \cdot F_{nv})} = 30.5\%$$

Condition3 = "OK"

Tensile Stress Adjusted for Shear =

$$F'_{nt} := \begin{cases} \text{if } 1.3 \cdot F_{nt} - \frac{F_{nt}}{\phi \cdot F_{nv}} \cdot f_v \leq F_{nt} & = 89.58 \text{ ksi} \\ \left( 1.3 \cdot F_{nt} - \frac{F_{nt}}{\phi \cdot F_{nv}} \cdot f_v \right) \\ \text{else} \\ F_{nt} \end{cases}$$

Tension Force per Bolt =

$$F_{tension.conn} := \frac{Horz \cdot e}{S_{horz} \cdot \frac{n_b}{2}} + \frac{Vert \cdot e \cdot d_3}{I_p} = 11.144 \text{ kips}$$

Tension Stress Each Bolt =

$$f_t := \frac{F_{tension.conn}}{a_b} = 14.189 \text{ ksi}$$

Condition4 := if ( $f_t < \phi \cdot F'_{nt}$ , "OK", "Overstressed")

$$\frac{f_t}{(\phi \cdot F'_{nt})} = 21.1\%$$

Condition4 = "OK"

**Basic Components**

Heavy Wind Pressure =	$p := 4.00 \cdot psf$	(User Input NESC 2023 Figure 250-1 & Table 250-1)
Basic Windspeed =	$V := 110 \quad mph$	(User Input)
Radial Ice Thickness =	$lr := 0.50 \cdot in$	(User Input NESC 2023 Figure 250-1 & Table 250-1)
Radial Ice Density =	$ld := 56.0 \quad pcf$	(User Input)

**Factors for Extreme Wind Calculation**

CL Elevation of Antennas =	$TME := 113 \quad ft$	(User Input)
Multiplier Gust Response Factor =	$m := 1.25$	(User Input - Only for NESC Extreme wind case)
Velocity Pressure Coefficient =	$Kz := 2.01 \cdot \left(\frac{TME}{900}\right)^{\frac{2}{9.5}} = 1.299$	(NESC 2023 Table 250-2)
Turbulence Intensity Constant =	$C_{exp} := 0.2$	(NESC 2023 Table 250-3)
Integral Length Scale of Turbulence Constant =	$L_s := 220$	(NESC 2023 Table 250-3)
Effective Height =	$z_s := 0.67 \cdot TME = 75.71$	(NESC 2023 Table 250-3)
Turbulence Intensity =	$I_z := C_{exp} \cdot \left(\frac{33}{z_s}\right)^{\frac{1}{6}} = 0.174$	(NESC 2023 Table 250-3)
Response Term =	$B_t := \left(\frac{1}{\left(1 + \left(0.56 \cdot \frac{z_s}{L_s}\right)\right)}\right)^{0.5} = 0.916$	(NESC 2023 Table 250-3)
Gust Response Factor =	$G_{rf} := \frac{(1 + (4.61 \cdot I_z \cdot B_t))}{(1 + 6.1 \cdot I_z)} = 0.841$	(NESC 2023 Table 250-3)
Wind Pressure =	$qz := 0.00256 \cdot Kz \cdot V^2 \cdot G_{rf} \cdot psf = 33.8 \frac{lb}{ft^2}$	(NESC 2023 Section 250.C.1)

**Shape Factors**

Shape Factor for Round Members =	$Cd_R := 1.3$	(User Input)
Shape Factor for Flat Members =	$Cd_F := 1.6$	(User Input)
Shape Factor for Open Lattice =	$Cd_{OL} := 3.2$	(User Input)
Shape Factor for Coax Cables Attached to Pole =	$Cd_{coax} := 1.6$	(User Input)

**Overload Factors**

**Overload Factors for Wind Loads:**

NESC Heavy Loading =	2.5	(User Input)	Apply in Risa-3D Analysis
NESC Extreme Loading =	1.0	(User Input)	Apply in Risa-3D Analysis
NESC Extreme Ice w/ Wind Loading =	1.0	(User Input)	Apply in Risa-3D Analysis

**Overload Factors for Vertical Loads:**

NESC Heavy Loading =	1.5	(User Input)	Apply in Risa-3D Analysis
NESC Extreme Loading =	1.0	(User Input)	Apply in Risa-3D Analysis
NESC Extreme Ice w/ Wind Loading =	1.0	(User Input)	Apply in Risa-3D Analysis

**Development of Wind & Ice Load on Mast**

**Mast Data:**

	(HSS14x0.625)	
Mast Shape =	Round	(User Input)
Mast Diameter =	$D_{mast} := 14 \cdot in$	(User Input)
Mast Length =	$L_{mast} := 30 \cdot ft$	(User Input)
Mast Thickness =	$t_{mast} := 0.625 \cdot in$	(User Input)

**Gravity Loads (without ice)**

Weight of the Mast =	Self Weight (Computed internally by Risa-3D)	BLC 1
----------------------	----------------------------------------------	-------

**Gravity Loads (ice only)**

Ice Area per Linear Foot =  $A_{i_{mast}} := \frac{\pi}{4} \cdot ((D_{mast} + 1r \cdot 2)^2 - D_{mast}^2) = 0.2 \text{ ft}^2$

Weight of Ice on Mast =	$W_{ICE_{mast}} := 1d \cdot A_{i_{mast}} = 9 \frac{lb}{ft}$	BLC 3
-------------------------	-------------------------------------------------------------	-------

**Wind Load (NESE Heavy)**

Mast Projected Surface Area w/ Ice =  $A_{ICE_{mast}} := (D_{mast} + 2 \cdot 1r) = 1.25 \text{ ft}$  (per ft)

Total Mast Wind Force w/ Ice =	$p \cdot C_{d_{coax}} \cdot A_{ICE_{mast}} = 8 \frac{lb}{ft}$	BLC 4
--------------------------------	---------------------------------------------------------------	-------

**Wind Load (NESC Extreme)**

Mast Projected Surface Area =  $A_{mast} := D_{mast} = 1.167 \text{ ft}$  (per ft)

Total Mast Wind Force (Above Structure) =	$qz \cdot C_{d_{coax}} \cdot A_{mast} \cdot m = 79 \frac{lb}{ft}$	BLC 5
-------------------------------------------	-------------------------------------------------------------------	-------

Total Mast Wind Force (Below Structure) =	$qz \cdot C_{d_{coax}} \cdot A_{mast} = 63 \frac{lb}{ft}$	BLC 5
-------------------------------------------	-----------------------------------------------------------	-------

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	CCI DMP65R-BU8D	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 96 \cdot in$	(User Input)
Antenna Width =	$W_{ant} := 20.7 \cdot in$	(User Input)
Antenna Thickness =	$T_{ant} := 7.7 \cdot in$	(User Input)
Antenna Weight =	$WT_{ant} := 115 \cdot lb$	(User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)

**Gravity Load (without ice)**

Weight of All Antennas =  $W_{ant1} := WT_{ant} \cdot N_{ant} = 345 \text{ lb}$  **BLC 2**

**Gravity Load (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 15301 \text{ in}^3$   
 Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot lr) \cdot (W_{ant} + 2 \cdot lr) \cdot (T_{ant} + 2 \cdot lr) - V_{ant} = 3011 \text{ in}^3$   
 Weight of Ice on Each Antenna =  $W_{ICEant} := V_{ice} \cdot Id = 98 \text{ lb}$

Weight of Ice on All Antennas =  $W_{ice,ant1} := W_{ICEant} \cdot N_{ant} = 293 \text{ lb}$  **BLC 3**

**Wind Load (NESC Heavy)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := (L_{ant} + 2 \cdot lr) \cdot (W_{ant} + 2 \cdot lr) = 14.6 \text{ ft}^2$   
 Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 43.9 \text{ ft}^2$

Total Antenna Wind Force w/ Ice =  $F_{ant1} := p \cdot Cd_F \cdot A_{ICEant} = 281 \text{ lb}$  **BLC 4**

**Wind Load (NESC Extreme)**

Surface Area for One Antenna =  $SA_{ant} := L_{ant} \cdot W_{ant} = 13.8 \text{ ft}^2$   
 Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 41.4 \text{ ft}^2$

Total Antenna Wind Force =  $F_{ant1} := qz \cdot Cd_F \cdot A_{ant} \cdot m = 2802 \text{ lb}$  **BLC 5**



**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	CCI TPA65R-BU8D	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 96 \cdot in$	(User Input)
Antenna Width =	$W_{ant} := 20.7 \cdot in$	(User Input)
Antenna Thickness =	$T_{ant} := 7.7 \cdot in$	(User Input)
Antenna Weight =	$WT_{ant} := 110 \cdot lb$	(User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)

**Gravity Load (without ice)**

Weight of All Antennas =  $Wt_{ant2} := WT_{ant} \cdot N_{ant} = 330 \text{ lb}$  **BLC 2**

**Gravity Load (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 15301 \text{ in}^3$   
 Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot Ir) \cdot (W_{ant} + 2 \cdot Ir) \cdot (T_{ant} + 2 \cdot Ir) - V_{ant} = 3011 \text{ in}^3$   
 Weight of Ice on Each Antenna =  $W_{ICEant} := V_{ice} \cdot Id = 98 \text{ lb}$

Weight of Ice on All Antennas =  $Wt_{ice,ant2} := W_{ICEant} \cdot N_{ant} = 293 \text{ lb}$  **BLC 3**

**Wind Load (NESC Heavy)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := (L_{ant} + 2 \cdot Ir) \cdot (W_{ant} + 2 \cdot Ir) = 14.6 \text{ ft}^2$   
 Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 43.9 \text{ ft}^2$   
 Total Antenna Wind Force w/ Ice =  $F_{iant2} := p \cdot Cd_F \cdot A_{ICEant} = 281 \text{ lb}$  **BLC 4**

**Wind Load (NESC Extreme)**

Surface Area for One Antenna =  $SA_{ant} := L_{ant} \cdot W_{ant} = 13.8 \text{ ft}^2$   
 Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 41.4 \text{ ft}^2$   
 Total Antenna Wind Force =  $F_{ant2} := qz \cdot Cd_F \cdot A_{ant} \cdot m = 2802 \text{ lb}$  **BLC 5**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Ericsson AIR6419	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 31.1 \cdot in$	(User Input)
Antenna Width =	$W_{ant} := 16.1 \cdot in$	(User Input)
Antenna Thickness =	$T_{ant} := 7.3 \cdot in$	(User Input)
Antenna Weight =	$WT_{ant} := 56 \cdot lb$	(User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)

**Gravity Load (without ice)**

Weight of All Antennas =  $W_{ant3} := WT_{ant} \cdot N_{ant} = 168 \text{ lb}$  **BLC 2**

**Gravity Load (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 3655 \text{ in}^3$   
 Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot Ir) \cdot (W_{ant} + 2 \cdot Ir) \cdot (T_{ant} + 2 \cdot Ir) - V_{ant} = 901 \text{ in}^3$   
 Weight of Ice on Each Antenna =  $W_{ICEant} := V_{ice} \cdot Id = 29 \text{ lb}$

Weight of Ice on All Antennas =  $W_{ice.ant3} := W_{ICEant} \cdot N_{ant} = 88 \text{ lb}$  **BLC 3**

**Wind Load (NESC Heavy)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := (L_{ant} + 2 \cdot Ir) \cdot (W_{ant} + 2 \cdot Ir) = 3.8 \text{ ft}^2$   
 Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 11.4 \text{ ft}^2$

Total Antenna Wind Force w/ Ice =  $F_{ant3} := p \cdot Cd_F \cdot A_{ICEant} = 73 \text{ lb}$  **BLC 4**

**Wind Load (NESC Extreme)**

Surface Area for One Antenna =  $SA_{ant} := L_{ant} \cdot W_{ant} = 3.5 \text{ ft}^2$   
 Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 10.4 \text{ ft}^2$

Total Antenna Wind Force =  $F_{ant3} := qz \cdot Cd_F \cdot A_{ant} \cdot m = 706 \text{ lb}$  **BLC 5**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Ericsson AIR6449	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 30.6 \cdot in$	(User Input)
Antenna Width =	$W_{ant} := 15.9 \cdot in$	(User Input)
Antenna Thickness =	$T_{ant} := 10.6 \cdot in$	(User Input)
Antenna Weight =	$WT_{ant} := 96 \cdot lb$	(User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)

**Gravity Load (without ice)**

Weight of All Antennas =  $W_{ant4} := WT_{ant} \cdot N_{ant} = 288 \text{ lb}$  **BLC 2**

**Gravity Load (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 5157 \text{ in}^3$   
 Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot Ir) \cdot (W_{ant} + 2 \cdot Ir) \cdot (T_{ant} + 2 \cdot Ir) - V_{ant} = 1038 \text{ in}^3$   
 Weight of Ice on Each Antenna =  $W_{ICEant} := V_{ice} \cdot Id = 34 \text{ lb}$

Weight of Ice on All Antennas =  $W_{ice.ant4} := W_{ICEant} \cdot N_{ant} = 101 \text{ lb}$  **BLC 3**

**Wind Load (NESC Heavy)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := (L_{ant} + 2 \cdot Ir) \cdot (W_{ant} + 2 \cdot Ir) = 3.7 \text{ ft}^2$   
 Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 11.1 \text{ ft}^2$

Total Antenna Wind Force w/ Ice =  $F_{ant4} := p \cdot Cd_F \cdot A_{ICEant} = 71 \text{ lb}$  **BLC 4**

**Wind Load (NESC Extreme)**

Surface Area for One Antenna =  $SA_{ant} := L_{ant} \cdot W_{ant} = 3.4 \text{ ft}^2$   
 Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 10.1 \text{ ft}^2$

Total Antenna Wind Force =  $F_{ant4} := qZ \cdot Cd_F \cdot A_{ant} \cdot m = 686 \text{ lb}$  **BLC 5**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Kaelus TMA2124F03V5-2D	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 9.7 \cdot in$	(User Input)
Antenna Width =	$W_{ant} := 10.4 \cdot in$	(User Input)
Antenna Thickness =	$T_{ant} := 8.3 \cdot in$	(User Input)
Antenna Weight =	$WT_{ant} := 36 \cdot lb$	(User Input)
Number of Antennas =	$N_{ant} := 3$	(User Input)

**Gravity Load (without ice)**

Weight of All Antennas =  $W_{ant5} := WT_{ant} \cdot N_{ant} = 108 \text{ lb}$  **BLC 2**

**Gravity Load (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 837 \text{ in}^3$

Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot lr) \cdot (W_{ant} + 2 \cdot lr) \cdot (T_{ant} + 2 \cdot lr) - V_{ant} = 297 \text{ in}^3$

Weight of Ice on Each Antenna =  $W_{ICEant} := V_{ice} \cdot Id = 10 \text{ lb}$

Weight of Ice on All Antennas =  $W_{ice,ant5} := W_{ICEant} \cdot N_{ant} = 29 \text{ lb}$  **BLC 3**

**Wind Load (NESC Heavy)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := (L_{ant} + 2 \cdot lr) \cdot (W_{ant} + 2 \cdot lr) = 0.8 \text{ ft}^2$

Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 2.5 \text{ ft}^2$

Total Antenna Wind Force w/ Ice =  $F_{ant5} := p \cdot Cd_F \cdot A_{ICEant} = 16 \text{ lb}$  **BLC 4**

**Wind Load (NESC Extreme)**

Surface Area for One Antenna =  $SA_{ant} := L_{ant} \cdot W_{ant} = 0.7 \text{ ft}^2$

Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 2.1 \text{ ft}^2$

Total Antenna Wind Force =  $F_{ant5} := qz \cdot Cd_F \cdot A_{ant} \cdot m = 142 \text{ lb}$  **BLC 5**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Kaelus TMA2104F00V1-1	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 8.7 \cdot in$	(User Input)
Antenna Width =	$W_{ant} := 7.9 \cdot in$	(User Input)
Antenna Thickness =	$T_{ant} := 4.1 \cdot in$	(User Input)
Antenna Weight =	$WT_{ant} := 17 \cdot lb$	(User Input)
Number of Antennas =	$N_{ant} := 6$	(User Input)

**Gravity Load (without ice)**

Weight of All Antennas =  $W_{ant6}^t := WT_{ant} \cdot N_{ant} = 102 \text{ lb}$  **BLC 2**

**Gravity Load (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 282 \text{ in}^3$

Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot lr) \cdot (W_{ant} + 2 \cdot lr) \cdot (T_{ant} + 2 \cdot lr) - V_{ant} = 158 \text{ in}^3$

Weight of Ice on Each Antenna =  $W_{ICEant} := V_{ice} \cdot Id = 5 \text{ lb}$

Weight of Ice on All Antennas =  $W_{ice,ant6}^t := W_{ICEant} \cdot N_{ant} = 31 \text{ lb}$  **BLC 3**

**Wind Load (NESC Heavy)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := (L_{ant} + 2 \cdot lr) \cdot (W_{ant} + 2 \cdot lr) = 0.6 \text{ ft}^2$

Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 3.6 \text{ ft}^2$

Total Antenna Wind Force w/ Ice =  $F_{ant6}^i := p \cdot Cd_F \cdot A_{ICEant} = 23 \text{ lb}$  **BLC 4**

**Wind Load (NESC Extreme)**

Surface Area for One Antenna =  $SA_{ant} := L_{ant} \cdot W_{ant} = 0.5 \text{ ft}^2$

Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 2.9 \text{ ft}^2$

Total Antenna Wind Force =  $F_{ant6} := qZ \cdot Cd_F \cdot A_{ant} \cdot m = 194 \text{ lb}$  **BLC 5**

**Development of Wind & Ice Load on Antennas**

**Antenna Data:**

Antenna Model =	Raycap DC6-48-60-18-8C	
Antenna Shape =	Flat	(User Input)
Antenna Height =	$L_{ant} := 31.4 \cdot in$	(User Input)
Antenna Width =	$W_{ant} := 18.28 \cdot in$	(User Input)
Antenna Thickness =	$T_{ant} := 10.24 \cdot in$	(User Input)
Antenna Weight =	$WT_{ant} := 26 \cdot lb$	(User Input)
Number of Antennas =	$N_{ant} := 1$	(User Input)

**Gravity Load (without ice)**

Weight of All Antennas =  $W_{ant7}^t := WT_{ant} \cdot N_{ant} = 26 \text{ lb}$  BLC 2

**Gravity Load (ice only)**

Volume of Each Antenna =  $V_{ant} := L_{ant} \cdot W_{ant} \cdot T_{ant} = 5878 \text{ in}^3$

Volume of Ice on Each Antenna =  $V_{ice} := (L_{ant} + 2 \cdot Ir) \cdot (W_{ant} + 2 \cdot Ir) \cdot (T_{ant} + 2 \cdot Ir) - V_{ant} = 1144 \text{ in}^3$

Weight of Ice on Each Antenna =  $W_{ICEant} := V_{ice} \cdot Id = 37 \text{ lb}$

Weight of Ice on All Antennas =  $W_{ice.ant7}^t := W_{ICEant} \cdot N_{ant} = 37 \text{ lb}$  BLC 3

**Wind Load (NESC Heavy)**

Surface Area for One Antenna w/ Ice =  $SA_{ICEant} := (L_{ant} + 2 \cdot Ir) \cdot (W_{ant} + 2 \cdot Ir) = 4.3 \text{ ft}^2$

Antenna Projected Surface Area w/ Ice =  $A_{ICEant} := SA_{ICEant} \cdot N_{ant} = 4.3 \text{ ft}^2$

Total Antenna Wind Force w/ Ice =  $F_{ant7}^i := p \cdot Cd_F \cdot A_{ICEant} = 28 \text{ lb}$  BLC 4

**Wind Load (NESC Extreme)**

Surface Area for One Antenna =  $SA_{ant} := L_{ant} \cdot W_{ant} = 4 \text{ ft}^2$

Antenna Projected Surface Area =  $A_{ant} := SA_{ant} \cdot N_{ant} = 4 \text{ ft}^2$

Total Antenna Wind Force =  $F_{ant7} := qZ \cdot Cd_F \cdot A_{ant} \cdot m = 270 \text{ lb}$  BLC 5

**Development of Wind & Ice Load on Antenna Mounts**

**Mount Data:**

Mount Type = Platform with Handrail

Mount Shape = Flat (User Input)

Mount Projected Surface Area =  $CdAa := 24 \cdot sf$  (User Input)

Mount Projected Surface Area w/ Ice =  $CdAa_{ice} := 28 \cdot sf$  (User Input)

Mount Weight =  $WT_{mnt} := 3300 \cdot lb$  (User Input)

Mount Weight w/ Ice =  $WT_{mnt,ice} := 3800 \cdot lb$  (User Input)

**Gravity Loads (without ice)**

Weight of All Mounts =  $Wt_{mnt1} := WT_{mnt} = 3300 \text{ lb}$  lbs **BLC 2**

**Gravity Load (ice only)**

Weight of Ice on All Mounts =  $Wt_{ice,mnt1} := (WT_{mnt,ice} - WT_{mnt}) = 500 \text{ lb}$  lbs **BLC 3**

**Wind Load (NESC Heavy)**

Total Mount Wind Force w/ Ice =  $F_{i,mnt1} := p \cdot CdAa_{ice} = 112 \text{ lb}$  lbs **BLC 4**

**Wind Load (NESC Extreme)**

Total Mount Wind Force =  $F_{mnt1} := qz \cdot CdAa \cdot m = 1015 \text{ lb}$  lbs **BLC 5**

**Development of Wind & Ice Load on Coax Cables**

**Coax Cable Data:**

Coax Type =	1-5/8" Coax Cable	
Shape =	Round	(User Input)
Coax Outside Diameter =	$D_{coax} := 1.98 \cdot in$	(User Input)
Coax Cable Length =	$L_{coax} := 10 \cdot ft$	(User Input)
Weight of Coax per foot =	$Wt_{coax} := 1.04 \cdot plf$	(User Input)
Total Number of Coax =	$N_{coax} := 27$	(User Input - 24 Coax, 1 Hybrid & 2 DC)
No. of Coax Projecting Outside Face of Member =	$NP_{coax} := 6$	(User Input)

**Gravity Loads (without ice)**

Weight of all cables w/o ice =  $WT_{coax} := Wt_{coax} \cdot N_{coax} = 28 \frac{lb}{ft}$  **BLC 2**

**Gravity Load (ice only)**

Ice Area per Linear Foot =  $Ai_{coax} := \frac{\pi}{4} \cdot ((D_{coax} + 2 \cdot lr)^2 - D_{coax}^2) = 3.9 in^2$

Ice Weight All Coax per foot =  $WTi_{coax} := N_{coax} \cdot ld \cdot Ai_{coax} = 41 \frac{lb}{ft}$  **BLC 3**

**Wind Load (NESC Heavy)**

Coax projected surface area w/ Ice =  $AICE_{coax} := NP_{coax} \cdot (D_{coax} + 2 \cdot lr) = 17.9 in$

Total Coax Wind Force w/ Ice =  $Fi_{coax} := p \cdot Cd_{coax} \cdot AICE_{coax} = 10 \frac{lb}{ft}$  **BLC 4**

**Wind Load (NESC Extreme)**

Coax projected surface area =  $A_{coax} := (NP_{coax} \cdot D_{coax}) = 11.9 in$

Total Coax Wind Force (Above NU Structure) =  $F_{coax} := qz \cdot Cd_{coax} \cdot A_{coax} \cdot m = 67 \frac{lb}{ft}$  **BLC 5**



**Model Settings**

Number of Reported Sections	5
Number of Internal Sections	100
Member Area Load Mesh Size (in <sup>2</sup> )	144
Consider Shear Deformation	Yes
Consider Torsional Warping	Yes
Approximate Mesh Size (in)	12
Transfer Forces Between Intersecting Wood Walls	Yes
Increase Wood Wall Nailing Capacity for Wind Loads	Yes
Include P-Delta for Walls	Yes
Optimize Masonry and Wood Walls	No
Maximum Number of Iterations	3
Single	No
Multiple (Optimum)	Yes
Maximum	No

Global Axis corresponding to vertical direction	Y
Convert Existing Data	Yes
Default Global Plane for z-axis	XZ
Plate Local Axis Orientation	Nodal

Hot Rolled Steel	AISC 15th (360-16): LRFD
Stiffness Adjustment	Yes (Iterative)
Notional Annex	None
Connections	AISC 14th (360-10): ASD
Cold Formed Steel	AISI 1999: ASD
Stiffness Adjustment	Yes (Iterative)
Wood	AF&PA NDS-91/97: ASD
Temperature	< 100F
Concrete	ACI 318-02
Masonry	ACI 530-05: ASD
Aluminum	AA ADM1-05: ASD
Structure Type	Building
Stiffness Adjustment	Yes (Iterative)
Stainless	AISC 14th (360-10): ASD
Stiffness Adjustment	Yes (Iterative)

Analysis Methodology	PCA Load Contour Method
Parme Beta Factor	0.65

Compression Stress Block	Rectangular Stress Block
Analyze using Cracked Sections	Yes
Leave room for horizontal rebar splices (2*d bar spacing)	Yes
List forces which were ignored for design in the Detail Report	Yes

Column Min Steel	1
Column Max Steel	8
Rebar Material Spec	ASTM A615
Warn if beam-column framing arrangement is not understood	No
Number of Shear Regions	4
Region 2 & 3 Spacing Increase Increment (in)	4

Code	UBC 1997
------	----------



Company : CENTEK Engineering, Inc.  
Designer : T.JL  
Job Number : 22021.02 /AT&T 2267  
Model Name : Structure # 3848 - Mast

Checked By : CFC

**Model Settings (Continued)**

Occupancy Cat	4
Seismic Zone	3
Base Elevation (ft)	
Include the weight of the structure in base shear calcs	No
C <sub>s</sub>	0.36
C <sub>v</sub>	0.54
T Z (sec)	
T X (sec)	
C <sub>Z</sub>	0.035
C <sub>X</sub>	0.035
R Z	8.5
R X	8.5
Ω <sub>z</sub>	1
Ω <sub>x</sub>	1
ρ Z	1
ρ X	1

**Hot Rolled Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm. Coeff. [ $10^{-6}F^{-1}$ ]	Density [k/ft <sup>3</sup> ]	Yield [ksi]	Ry	Fu [ksi]	Rt
1	A36 Gr.36	29000	11154	0.3	0.65	0.49	36	1.5	58	1.2
2	A572 Gr.50	29000	11154	0.3	0.65	0.49	50	1.1	58	1.2
3	A992	29000	11154	0.3	0.65	0.49	50	1.1	58	1.2
4	A500 Gr.42	29000	11154	0.3	0.65	0.49	42	1.3	58	1.1
5	A500 Gr.46	29000	11154	0.3	0.65	0.49	46	1.2	58	1.1
6	A53 Gr. B	29000	11154	0.3	0.65	0.49	35	1.5	58	1.2

**Hot Rolled Steel Section Sets**

	Label	Shape	Type	Design List	Material	Design Rule	Area [in <sup>2</sup> ]	Iyy [in <sup>4</sup> ]	Izz [in <sup>4</sup> ]	J [in <sup>4</sup> ]
1	Mast	HSS14.000X0.625	Column	Pipe	A500 Gr.42	Typical	24.5	552	552	1100

**Hot Rolled Steel Design Parameters**

	Label	Shape	Length [ft]	Channel Conn.	a [ft]	Function
1	M1	Mast	30	N/A	N/A	Lateral

**Member Primary Data**

	Label	I Node	J Node	Section/Shape	Type	Design List	Material	Design Rule
1	M1	BOTCONN	TOPMAST	Mast	Column	Pipe	A500 Gr.42	Typical

**Node Coordinates**

	Label	X [ft]	Y [ft]	Z [ft]	Detach From Diaphragm
1	BOTCONN	0	0	0	
2	TOPCONN	0	10	0	
3	TOPMAST	0	30	0	

**Node Boundary Conditions**

	Node Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot [k-ft/rad]	Y Rot [k-ft/rad]	Z Rot [k-ft/rad]
1	BOTCONN	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction
2	TOPCONN	Reaction		Reaction			

**Member Point Loads (BLC 2 : Weight of Appurtenances)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
1	M1	Y	-0.345	28
2	M1	Y	-0.33	28
3	M1	Y	-0.168	28
4	M1	Y	-0.288	28
5	M1	Y	-0.108	28
6	M1	Y	-0.102	28
7	M1	Y	-0.026	28
8	M1	Y	-3.3	28

**Member Point Loads (BLC 3 : Weight of Ice Only)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
1	M1	Y	-0.293	28
2	M1	Y	-0.293	28

**Member Point Loads (BLC 3 : Weight of Ice Only) (Continued)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
3	M1	Y	-0.088	28
4	M1	Y	-0.101	28
5	M1	Y	-0.029	28
6	M1	Y	-0.031	28
7	M1	Y	-0.037	28
8	M1	Y	-0.5	28

**Member Point Loads (BLC 4 : NESC Heavy Wind)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
1	M1	X	0.281	28
2	M1	X	0.281	28
3	M1	X	0.073	28
4	M1	X	0.071	28
5	M1	X	0.016	28
6	M1	X	0.023	28
7	M1	X	0.028	28
8	M1	X	0.112	28

**Member Point Loads (BLC 5 : NESC Extreme Wind)**

	Member Label	Direction	Magnitude [k, k-ft]	Location [(ft, %)]
1	M1	X	2.802	28
2	M1	X	2.802	28
3	M1	X	0.706	28
4	M1	X	0.686	28
5	M1	X	0.142	28
6	M1	X	0.194	28
7	M1	X	0.27	28
8	M1	X	1.015	28

**Member Distributed Loads (BLC 2 : Weight of Appurtenances)**

	Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
1	M1	Y	-0.028	-0.028	0	26

**Member Distributed Loads (BLC 3 : Weight of Ice Only)**

	Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
1	M1	Y	-0.009	-0.009	0	%100
2	M1	Y	-0.041	-0.041	0	26

**Member Distributed Loads (BLC 4 : NESC Heavy Wind)**

	Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
1	M1	X	0.008	0.008	0	%100
2	M1	X	0.01	0.01	0	26

**Member Distributed Loads (BLC 5 : NESC Extreme Wind)**

	Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
1	M1	X	0.063	0.063	0	13
2	M1	X	0.079	0.079	13	%100



Company : CENTEK Engineering, Inc.  
 Designer : T.JL  
 Job Number : 22021.02 /AT&T 2267  
 Model Name : Structure # 3848 - Mast

Checked By : CFC

**Member Distributed Loads (BLC 5 : NESC Extreme Wind) (Continued)**

Member Label	Direction	Start Magnitude [k/ft, F, ksf, k-ft/ft]	End Magnitude [k/ft, F, ksf, k-ft/ft]	Start Location [(ft, %)]	End Location [(ft, %)]
3	M1	X	0.055	0.055	0 13
4	M1	X	0.067	0.067	13 26

**Basic Load Cases**

	BLC Description	Category	Y Gravity	Point	Distributed
1	Self Weight	None	-1		
2	Weight of Appurtenances	None		8	1
3	Weight of Ice Only	None		8	2
4	NESC Heavy Wind	None		8	2
5	NESC Extreme Wind	None		8	4

**Load Combinations**

Description	Solve	P-Delta	BLC	Factor	BLC	Factor	BLC	Factor	BLC	Factor
1 NESC Heavy Wind	Yes	Y	1	1.5	2	1.5	3	1.5	4	2.5
2 NESC Extreme Wind	Yes	Y	1	1	2	1	5	1		



Company : CENTEK Engineering, Inc.  
Designer : T.JL  
Job Number : 22021.02 /AT&T 2267  
Model Name : Structure # 3848 - Mast

Checked By : CFC

---

**Node Reactions**

---

	LC	Node Label	X [k]	Y [k]	Z [k]	MX [k-ft]	MY [k-ft]	MZ [k-ft]
0	1	BOTCONN	6.739	15.906	0	0	0	-21.987
1	1	TOPCONN	-10.202	0	0	0	0	0
2	1	Totals:	-3.463	15.906	0			
3	1	COG (ft):	X: 0	Y: 22.065	Z: 0			

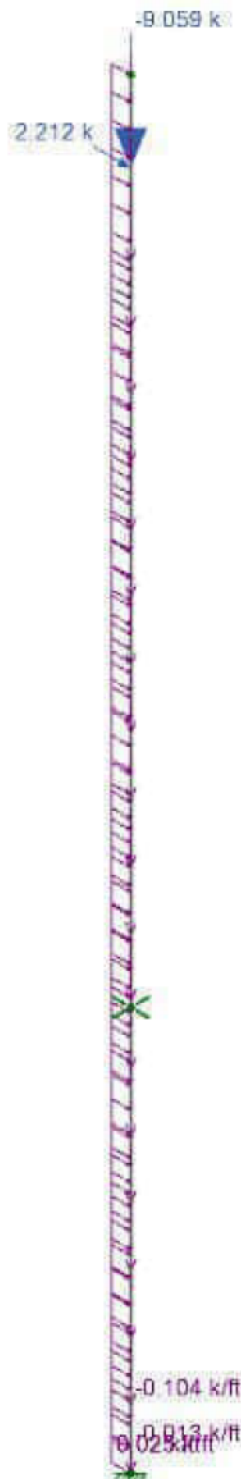


Company : CENTEK Engineering, Inc.  
Designer : T.JL  
Job Number : 22021.02 /AT&T 2267  
Model Name : Structure # 3848 - Mast

Checked By : CFC

**Node Reactions**

	LC	Node Label	X [k]	Y [k]	Z [k]	MX [k-ft]	MY [k-ft]	MZ [k-ft]
0	2	BOTCONN	25.861	7.896	0	0	0	-83.953
1	2	TOPCONN	-38.226	0	0	0	0	0
2	2	Totals:	-12.365	7.896	0			
3	2	COG (ft):	X: 0	Y: 22.499	Z: 0			



Loads: LC 1, NESC Heavy Wind



CEN TEK Engineering, Inc.  
TJL  
22021.02 /AT&T 2267

Structure # 3848 - Mast

NESC.r3d





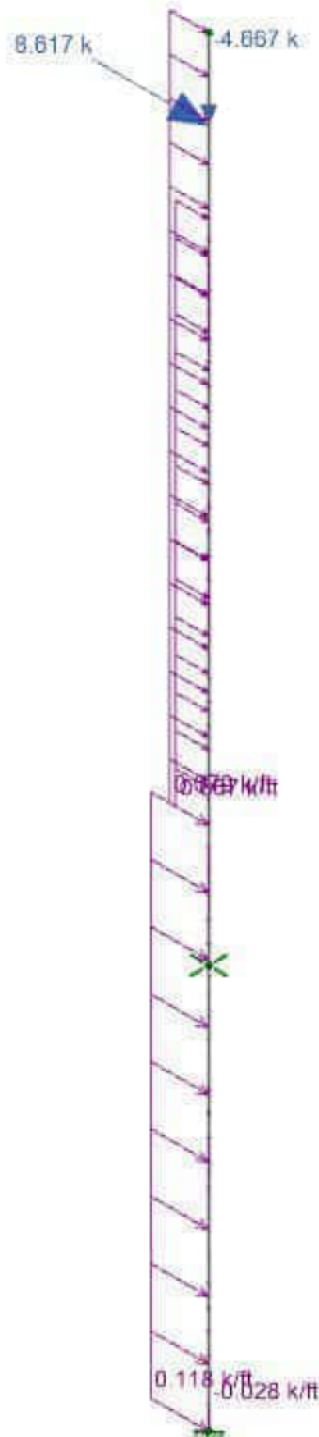
Results for LC 1, NESC Heavy Wind  
Reaction and Moment Units are kips and kip-ft.



CEN TEK Engineering, Inc.  
TJL  
22021.02 / AT&T 2267

Structure # 3848 - Mast

NESC.r3d



Loads: LC 2, NESC Extreme Wind



CENTEK Engineering, Inc.  
TJL  
22021.02 /AT&T 2267

Structure # 3848 - Mast

NESC.r3d



Results for LC 2, NESC Extreme Wind  
Reaction and Moment Units are kips and kip-ft.



CEN TEK Engineering, Inc.  
TJL  
22021.02 /AT&T 2267

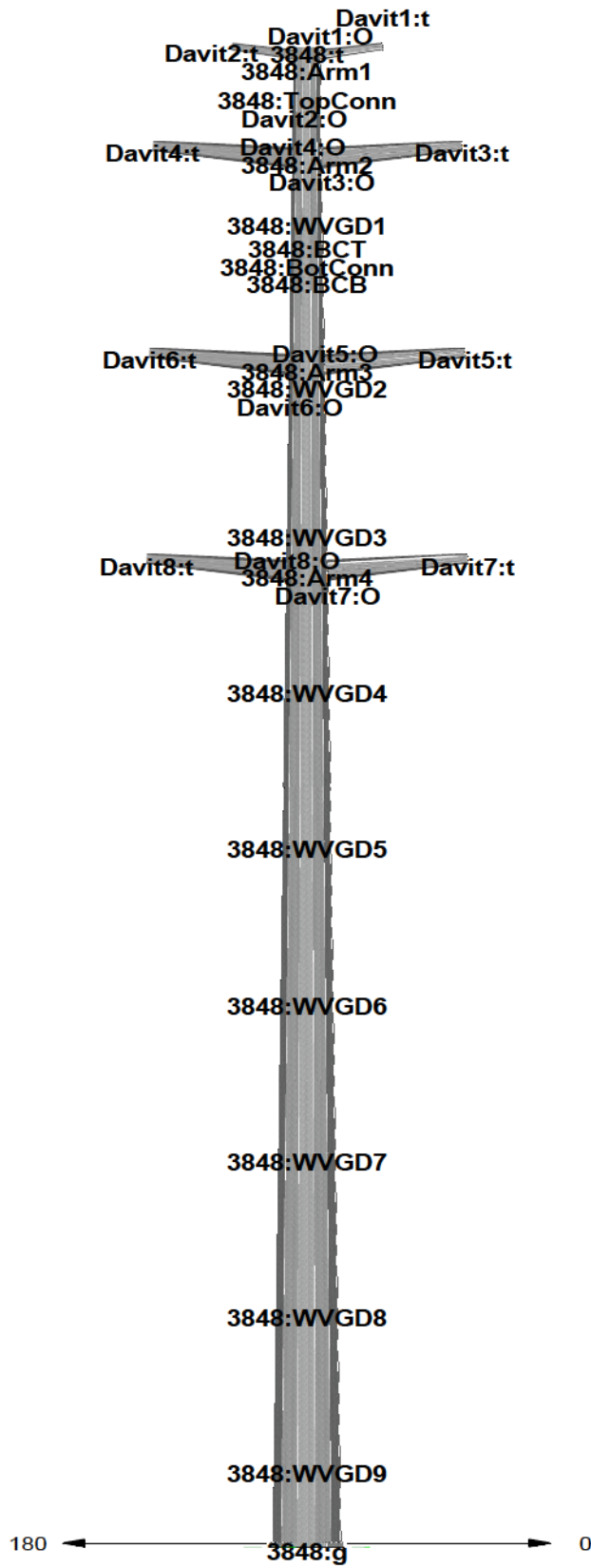
Structure # 3848 - Mast

NESC.r3d

**Coax Cable on Pole**

Coaxial Cable Span =	$Coax_{span} := 10 \cdot ft$	(User Input)
Heavy Wind Pressure =	$p := 4 \cdot psf$	(User Input NESC 2023 Figure 250-1 & Table 250-1)
Radial Ice Thickness =	$lr := 0.5 \cdot in$	(User Input NESC 2023 Figure 250-1 & Table 250-1)
Radial Ice Density =	$ld := 56 \cdot pcf$	(User Input)
Basic Windspeed =	$V := 110 \quad mph$	(User Input)
Height to Top of Coax Above Grade =	$TC := 96 \quad ft$	(User Input)
Multiplier Gust Response Factor =	$m := 1.00$	(User Input - Only for NESC Extreme wind case)
Velocity Pressure Coefficient =	$Kz := 2.01 \cdot \left( \frac{0.67 \cdot TC}{900} \right)^{\frac{2}{9.5}} = 1.153$	(NESC 2023 Table 250-2)
Turbulence Intensity Constant =	$C_{exp} := 0.2$	(NESC 2023 Table 250-3)
Integral Length Scale of Turbulence Constant =	$L_s := 220$	(NESC 2023 Table 250-3)
Effective Height =	$z_s := 0.67 \cdot TC = 64.32$	(NESC 2023 Table 250-3)
Turbulence Intensity =	$I_z := C_{exp} \cdot \left( \frac{33}{z_s} \right)^{\frac{1}{6}} = 0.179$	(NESC 2023 Table 250-3)
Response Term =	$B_t := \left( \frac{1}{\left( 1 + \left( 0.56 \cdot \frac{z_s}{L_s} \right) \right)} \right)^{0.5} = 0.927$	(NESC 2023 Table 250-3)
Gust Response Factor =	$Grf := \frac{1 + (4.61 \cdot I_z \cdot B_t)}{1 + 6.1 \cdot I_z} = 0.844$	(NESC 2023 Table 250-3)
Wind Pressure =	$qz := 0.00256 \cdot Kz \cdot V^2 \cdot Grf = 30.1 \quad psf$	(NESC 2023 Section 250.C.1)
Diameter of Coax Cable =	$D_{coax} := 1.98 \cdot in$	(User Input)
Weight of Coax Cable =	$W_{coax} := 1.04 \cdot plf$	(User Input)
Number of Coax Cables =	$N_{coax} := 27$	(User Input)
Number of Projected Coax Cables =	$NP_{coax} := 6$	(User Input)

Shape Factor =	$Cd_{coax} := 1.6$	(User Input)
Overload Factor for NESC Heavy Wind Transverse Load =	$OF_{HWT} := 2.5$	(User Input)
Overload Factor for NESC Heavy Wind Vertical Load =	$OF_{HWV} := 1.5$	(User Input)
Overload Factor for NESC Extreme Wind Transverse Load =	$OF_{EWT} := 1.0$	(User Input)
Overload Factor for NESC Extreme Wind Vertical Load =	$OF_{E WV} := 1.0$	(User Input)
Projected width without Ice =	$A := (NP_{coax} \cdot D_{coax}) = 11.88 \text{ in}$	
Projected width with Ice =	$A_{ice} := (NP_{coax} \cdot D_{coax} + 2 \cdot Ir) = 12.88 \text{ in}$	
Ice Area per Liner Ft =	$Ai_{coax} := \frac{\pi}{4} \cdot ((D_{coax} + 2 \cdot Ir)^2 - D_{coax}^2) = 0.027 \text{ ft}^2$	
Weight of Ice on All Coax Cables =	$W_{ice} := Ai_{coax} \cdot Id \cdot N_{coax} = 40.904 \text{ plf}$	
Heavy Wind Vertical Load =		
$Heavy\_Wind_{Vert} := ((N_{coax} \cdot W_{coax} + W_{ice}) \cdot Coax_{Span} \cdot OF_{HWV})$		
Heavy Wind Transverse Load =		
$Heavy\_Wind_{Trans} := (p \cdot A_{ice} \cdot Cd_{coax} \cdot Coax_{Span} \cdot OF_{HWT})$	$Heavy\_Wind_{Vert} = 1035 \text{ lb}$	$Heavy\_Wind_{Trans} = 172 \text{ lb}$
Extreme Wind Vertical Load =		
$Extreme\_Wind_{Vert} := (N_{coax} \cdot W_{coax} \cdot Coax_{Span} \cdot OF_{E WV})$		
Extreme Wind Transverse Load =		
$Extreme\_Wind_{Trans} := ((qz \cdot psf \cdot A \cdot Cd_{coax}) \cdot Coax_{Span} \cdot OF_{EWT})$	$Extreme\_Wind_{Vert} = 281 \text{ lb}$	$Extreme\_Wind_{Trans} = 477 \text{ lb}$



Project Name : 22021.02 - Orange, CT  
 Project Notes: Pole # 3848/ AT&T - CT2267  
 Project File : J:\Jobs\2202100.WI\02\_CT2267\05\_Structural\Tower\Backup Documentation\Rev (2)\Calcs\PLS Pole\cl&p structure # 3848.pol  
 Date run : 2:14:46 PM Monday, February 26, 2024  
 by : PLS-POLE Version 18.01  
 Licensed to : Centek Engineering Inc

Successfully performed nonlinear analysis

The model has 0 warnings.

Loads from file: J:\Jobs\2202100.WI\02\_CT2267\05\_Structural\Tower\Backup Documentation\Rev (2)\Calcs\PLS Pole\cl&p #3848.lca

\*\*\* Analysis Results:

Maximum element usage is 80.63% for Steel Pole "3848" in load case "NESC Extreme Wind"  
 Maximum insulator usage is 83.95% for Clamp "Clamp20" in load case "NESC Extreme Wind"

Foundation Design Forces For All Load Cases:

Note: loads are factored.

Load Case	Foundation Description	Axial Force (kips)	Shear Force (kips)	Resultant Force (kips)	Bending Moment (ft-k)	Foundation Usage %
NESC Heavy Wind	3848:g	67.00	17.24	69.19	1329.30	0.00
NESC Extreme Wind	3848:g	36.38	39.08	53.39	3183.47	0.00

Summary of Joint Support Reactions For All Load Cases:

Load Case	Joint Label	Long. Force (kips)	Tran. Force (kips)	Vert. Force (kips)	Shear Force (kips)	Tran. Moment (ft-k)	Long. Moment (ft-k)	Bending Moment (ft-k)	Vert. Moment (ft-k)	Found. Usage %
NESC Heavy Wind	3848:g	-0.11	-17.24	-67.00	17.24	1329.29	-4.56	1329.30	-0.00	0.00
NESC Extreme Wind	3848:g	0.01	-39.08	-36.38	39.08	3183.47	2.35	3183.47	-0.03	0.00

Summary of Tip Deflections For All Load Cases:

Note: positive tip load results in positive deflection

Load Case	Joint Label	Long. Defl. (in)	Tran. Defl. (in)	Vert. Defl. (in)	Resultant Defl. (in)	Long. Rot. (deg)	Tran. Rot. (deg)	Twist (deg)
NESC Heavy Wind	3848:t	0.04	20.66	-0.30	20.66	0.00	-2.02	0.00
NESC Extreme Wind	3848:t	-0.04	52.67	-1.74	52.70	-0.00	-5.40	0.00

Tubes Summary:

Pole Label	Tube Num.	Weight (lbs)	Load Case	Maximum Usage %	Resultant Moment (ft-k)
3848	1	4530	NESC Extreme Wind	80.63	1309.81
3848	2	3652	NESC Extreme Wind	72.60	2092.19
3848	3	5711	NESC Extreme Wind	77.23	3183.47

\*\*\* Overall summary for all load cases - Usage = Maximum Stress / Allowable Stress

Summary of Steel Pole Usages:

Steel Pole Label	Maximum Usage %	Load Case	Height AGL (ft)	Segment Number	Weight (lbs)
3848	80.63	NESC Extreme Wind	50.8	17	16194.8

Summary of Tubular Davit Usages:

Tubular Davit Label	Maximum Usage %	Load Case	Height AGL (ft)	Segment Number	Weight (lbs)
Davit1	9.76	NESC Heavy Wind	95.8	1	49.5
Davit2	8.90	NESC Heavy Wind	95.8	1	49.5
Davit3	10.19	NESC Heavy Wind	89.1	1	280.0
Davit4	9.49	NESC Heavy Wind	89.1	1	280.0
Davit5	10.21	NESC Heavy Wind	75.8	1	280.0
Davit6	9.52	NESC Heavy Wind	75.8	1	280.0
Davit7	10.23	NESC Heavy Wind	62.6	1	280.0
Davit8	9.55	NESC Heavy Wind	62.6	1	280.0

\*\*\* Maximum Stress Summary for Each Load Case

Summary of Maximum Usages by Load Case:

Load Case	Maximum Usage %	Element Label	Element Type
NESC Heavy Wind	52.52	3848 Base Plate	
NESC Extreme Wind	80.63	3848 Steel Pole	

Summary of Steel Pole Usages by Load Case:

Load Case	Maximum Usage %	Steel Pole Label	Height AGL (ft)	Segment Number
NESC Heavy Wind	33.59	3848	2.5	28
NESC Extreme Wind	80.63	3848	50.8	17

Summary of Base Plate Usages by Load Case:

Load Case	Pole Label	Bend Line #	Length (in)	Vertical Load (kips)	X Moment (ft-k)	Y Bending Moment (ft-k)	Stress (ksi)	Bolt Moment Sum (ft-k)	# Bolts	Max Bolt Load For Bend Line (kips)	Minimum Plate Thickness (in)	Usage %
NESC Heavy Wind	3848	1	14.076	64.700	2085.347	-7.146	31.512	46.588	-2	98.231	1.993	52.52
NESC Extreme Wind	3848	8	14.076	34.080	3183.472	2.351	46.651	68.970	2	145.714	2.425	77.75

Summary of Tubular Davit Usages by Load Case:

Load Case	Maximum Usage %	Tubular Davit Label	Height AGL (ft)	Segment Number
NESC Heavy Wind	10.23	Davit7	62.6	1



NESC Extreme Wind 5.46 Davit7 62.6 1

Summary of Insulator Usages:

Insulator Label	Insulator Type	Maximum Usage %	Load Case	Weight (lbs)
Clamp1	Clamp	1.00	NESC Heavy Wind	0.0
Clamp2	Clamp	1.09	NESC Heavy Wind	0.0
Clamp3	Clamp	2.60	NESC Heavy Wind	0.0
Clamp4	Clamp	2.59	NESC Heavy Wind	0.0
Clamp5	Clamp	2.60	NESC Heavy Wind	0.0
Clamp6	Clamp	2.59	NESC Heavy Wind	0.0
Clamp7	Clamp	2.60	NESC Heavy Wind	0.0
Clamp8	Clamp	2.59	NESC Heavy Wind	0.0
Clamp9	Clamp	1.05	NESC Heavy Wind	0.0
Clamp10	Clamp	1.05	NESC Heavy Wind	0.0
Clamp11	Clamp	1.05	NESC Heavy Wind	0.0
Clamp12	Clamp	1.05	NESC Heavy Wind	0.0
Clamp13	Clamp	1.05	NESC Heavy Wind	0.0
Clamp14	Clamp	1.05	NESC Heavy Wind	0.0
Clamp15	Clamp	1.05	NESC Heavy Wind	0.0
Clamp16	Clamp	1.05	NESC Heavy Wind	0.0
Clamp17	Clamp	1.05	NESC Heavy Wind	0.0
Clamp18	Clamp	38.23	NESC Extreme Wind	0.0
Clamp19	Clamp	27.04	NESC Extreme Wind	0.0
Clamp20	Clamp	83.95	NESC Extreme Wind	0.0
Clamp21	Clamp	83.95	NESC Extreme Wind	0.0

\*\*\* Weight of structure (lbs):  
Weight of Tubular Davit Arms: 1779.3  
Weight of Steel Poles: 16194.8  
Total: 17974.1

\*\*\* End of Report

```

*****
*
*                PLS-POLE
*          POLE AND FRAME ANALYSIS AND DESIGN
*      Copyright Power Line Systems 1999-2023
*
*****

```

```

Project Name : 22021.02 - Orange, CT
Project Notes: Pole # 3848/ AT&T - CT2267
Project File : J:\Jobs\2202100.WI\02_CT2267\05_Structural\Tower\Backup Documentation\Rev (2)\Calcs\PLS Pole\cl&p structure # 3848.pol
Date run      : 2:14:45 PM Monday, February 26, 2024
by           : PLS-POLE Version 18.01
Licensed to  : Centek Engineering Inc

```

Successfully performed nonlinear analysis

The model has 0 warnings.



Modeling options:

```

Offset Arms from Pole/Mast: Yes
Offset Braces from Pole/Mast: Yes
Offset Guys from Pole/Mast: Yes
Offset Posts from Pole/Mast: Yes
Offset Strains from Pole/Mast: Yes
Use Alternate Convergence Process: No
Steel poles and tubular arms checked with ASCE/SEI 48-19

```

```

Default Modulus of Elasticity for Steel = 29000.00 (ksi)
Default Weight Density for Steel = 490.00 (lbs/ft^3)

```

Steel Pole Properties:

Steel Pole Ultimate Property Number	Stock Length Texture	Default Embedded	Base Plate	Shape	Tip Diameter	Base Diameter	Taper	Default Drag	Tubes	Modulus of Elasticity	Weight Density	Shape At	Strength Check	Distance From	Ultimate Trans.
-------------------------------------	----------------------	------------------	------------	-------	--------------	---------------	-------	--------------	-------	-----------------------	----------------	----------	----------------	---------------	-----------------

Long. Label	Length	Coef.	Override	Override	Base	Type	Tip	Load	
(kips)	(ft)	(ft)	(in)	(in)	(in/ft)	(ksi)	(lbs/ft^3)	(ft)	(kips)

CL&P3848	3848	96.00	0	Yes	12F	20	52.53	0	1.6	3 tubes	0	0	Calculated	0.000	0.0000
----------	------	-------	---	-----	-----	----	-------	---	-----	---------	---	---	------------	-------	--------

0.0000 Galvanized Steel

**Steel Tubes Properties:**

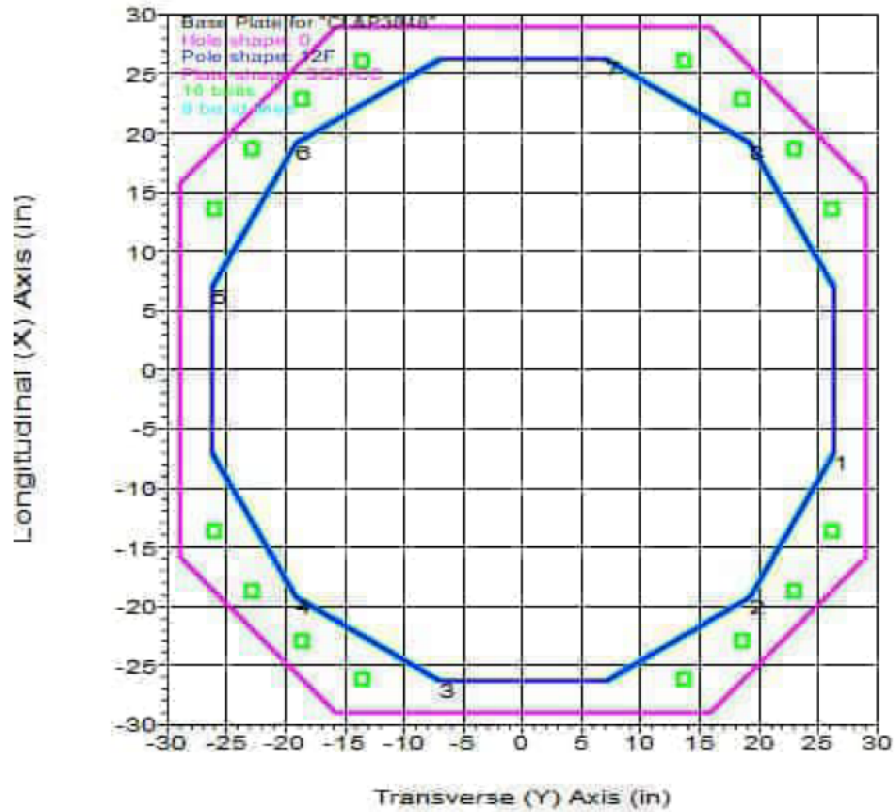
Pole Property	Tube No.	Length (ft)	Thickness (in)	Lap Length (ft)	Lap Factor	Lap Butt Offset (in)	Gap or Stress (ksi)	Yield Moment Override (ft-k)	Cap. Weight (lbs)	Tube Center of Gravity (ft)	Calculated Taper (in/ft)	Tube Top Diameter (in)	Tube Bot. Diameter (in)	1.5x Lap Length (ft)	Diam. Lap Length (ft)	Actual Overlap (ft)
CL&P3848	1	47.5	0.3125	4.583	0.000	0.000	65.000	0.000	4530	26.08	0.34538	20.00	36.41	4.473	4.583	4.583
CL&P3848	2	23.5	0.375	0.000	0.000	0.000	65.000	0.000	3652	12.17	0.34538	34.20	42.31	5.195	0.000	0.000
CL&P3848	3	29.583	0.375	0.000	0.000	0.000	65.000	0.000	5711	15.33	0.34538	42.31	52.53	0.000	0.000	0.000

**Base Plate Properties:**

Pole Property	Plate Diam. (in)	Plate Shape	Plate Thick. (in)	Plate Weight (lbs)	Plate Bend Length Override (in)	Line Length (in)	Hole Diam. (in)	Hole Shape	Steel Density (lbs/ft^3)	Steel Yield Stress (ksi)	Bolt Diam. (in)	Bolt Pattern (in)	Num. Of Bolts	Bolt Cage X Inertia (in^4)	Bolt Cage Y Inertia (in^4)
CL&P3848	58.000	SQP/CC	2.750	2302	0.000	0.000	0.000	0	490.00	60.000	2.250	59.000	16	27647.55	27647.55

**Base Plate Bolt Coordinates for Property "CL&P3848":**

Bolt X Coord.	Bolt Y Coord.	Bolt Angle (deg)
0.4619	0.8856	0
0.6314	0.7754	0
0.7754	0.6314	0
0.8856	0.4619	0



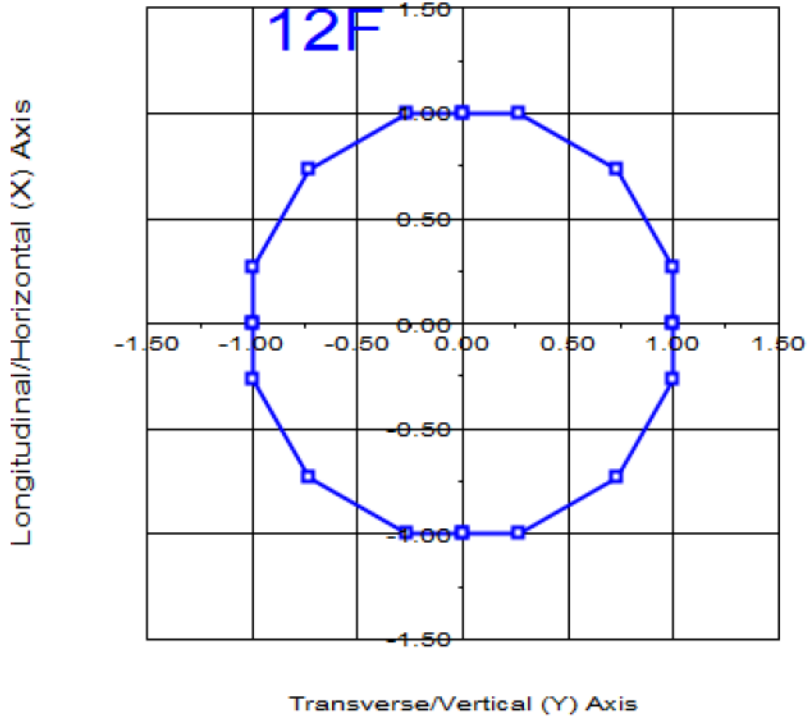
**Steel Pole Connectivity:**

Pole Label	Tip Joint	Base Joint	X of Base (ft)	Y of Base (ft)	Z of Base (ft)	Inclin. About X (deg)	Inclin. About Y (deg)	Property Set	Attach. Labels	Base Connect	Embed % Override	Embed C. Override (ft)
3848			0	0	0	0	0	CL&P3848	17 labels		0.00	0

**Relative Attachment Labels for Steel Pole "3848":**

Joint Label	Distance From Origin/Top Joint (ft)	Global Z of Attach (ft)
3848:Arm1	0.00	95.54
3848:Arm2	0.00	88.90
3848:Arm3	0.00	75.65
3848:Arm4	0.00	62.40
3848:WVGD1	0.00	85.00

3848:WVGD2	0.00	75.00
3848:WVGD3	0.00	65.00
3848:WVGD4	0.00	55.00
3848:WVGD5	0.00	45.00
3848:WVGD6	0.00	35.00
3848:WVGD7	0.00	25.00
3848:WVGD8	0.00	15.00
3848:WVGD9	0.00	5.00
3848:TopConn	0.00	93.00
3848:BotConn	0.00	83.00
3848:BCT	0.00	83.50
3848:BCB	0.00	82.50



**Pole Steel Properties:**

Element Label	Joint Label	Joint Position	Rel. Dist. (ft)	Outer Diam. (in)	Area (in <sup>2</sup> )	T-Moment Inertia (in <sup>4</sup> )	L-Moment Inertia (in <sup>4</sup> )	D/t	W/t Max.	Fy (ksi)	Fa Min. (ksi)	T-Moment Capacity (ft-k)	L-Moment Capacity (ft-k)
3848	3848:t	3848:t Ori	0.00	20.00	19.78	981.62	981.62	0.00	14.5	65.00	65.00	531.71	531.71
3848	3848:Arm1	3848:Arm1 End	0.46	20.16	19.94	1005.47	1005.47	0.00	14.6	65.00	65.00	540.35	540.35
3848	3848:Arm1	3848:Arm1 Ori	0.46	20.16	19.94	1005.47	1005.47	0.00	14.6	65.00	65.00	540.35	540.35
3848	3848:TopConn	3848:TopConn End	3.00	21.04	20.82	1144.88	1144.88	0.00	15.4	65.00	65.00	589.60	589.60
3848	3848:TopConn	3848:TopConn Ori	3.00	21.04	20.82	1144.88	1144.88	0.00	15.4	65.00	65.00	589.60	589.60

3848	3848:Arm2	3848:Arm2	End	7.10	22.45	22.25	1396.19	1396.19	0.00	16.6	65.00	65.00	673.63	673.63
3848	3848:Arm2	3848:Arm2	Ori	7.10	22.45	22.25	1396.19	1396.19	0.00	16.6	65.00	65.00	673.63	673.63
3848	3848:WVGD1	3848:WVGD1	End	11.00	23.80	23.60	1666.49	1666.49	0.00	17.7	65.00	65.00	758.58	758.58
3848	3848:WVGD1	3848:WVGD1	Ori	11.00	23.80	23.60	1666.49	1666.49	0.00	17.7	65.00	65.00	758.58	758.58
3848	3848:BCT	3848:BCT	End	12.50	24.32	24.12	1779.21	1779.21	0.00	18.2	65.00	65.00	792.64	792.64
3848	3848:BCT	3848:BCT	Ori	12.50	24.32	24.12	1779.21	1779.21	0.00	18.2	65.00	65.00	792.64	792.64
3848	3848:BotConn	3848:BotConn	End	13.00	24.49	24.29	1817.88	1817.88	0.00	18.3	65.00	65.00	804.15	804.15
3848	3848:BotConn	3848:BotConn	Ori	13.00	24.49	24.29	1817.88	1817.88	0.00	18.3	65.00	65.00	804.15	804.15
3848	3848:BCB	3848:BCB	End	13.50	24.66	24.47	1857.11	1857.11	0.00	18.5	65.00	65.00	815.75	815.75
3848	3848:BCB	3848:BCB	Ori	13.50	24.66	24.47	1857.11	1857.11	0.00	18.5	65.00	65.00	815.75	815.75
3848	#3848:0	Tube 1	End	16.93	25.85	25.66	2141.26	2141.26	0.00	19.5	65.00	65.00	897.50	897.50
3848	#3848:0	Tube 1	Ori	16.93	25.85	25.66	2141.27	2141.27	0.00	19.5	65.00	65.00	897.50	897.50
3848	3848:Arm3	3848:Arm3	End	20.35	27.03	26.85	2453.02	2453.02	0.00	20.5	65.00	65.00	983.15	983.15
3848	3848:Arm3	3848:Arm3	Ori	20.35	27.03	26.85	2453.03	2453.03	0.00	20.5	65.00	65.00	983.15	983.15
3848	3848:WVGD2	3848:WVGD2	End	21.00	27.25	27.07	2514.99	2514.99	0.00	20.7	65.00	65.00	999.73	999.73
3848	3848:WVGD2	3848:WVGD2	Ori	21.00	27.25	27.07	2514.99	2514.99	0.00	20.7	65.00	65.00	999.73	999.73
3848	#3848:1	Tube 1	End	26.00	28.98	28.81	3030.24	3030.24	0.00	22.2	65.00	65.00	1132.77	1132.77
3848	#3848:1	Tube 1	Ori	26.00	28.98	28.81	3030.24	3030.24	0.00	22.2	65.00	65.00	1132.77	1132.77
3848	3848:WVGD3	3848:WVGD3	End	31.00	30.71	30.54	3611.46	3611.46	0.00	23.6	65.00	65.00	1274.12	1274.12
3848	3848:WVGD3	3848:WVGD3	Ori	31.00	30.71	30.54	3611.46	3611.46	0.00	23.6	65.00	65.00	1274.12	1274.12
3848	3848:Arm4	3848:Arm4	End	33.60	31.61	31.44	3941.61	3941.61	0.00	24.4	65.00	65.00	1351.03	1351.03
3848	3848:Arm4	3848:Arm4	Ori	33.60	31.61	31.44	3941.61	3941.61	0.00	24.4	65.00	65.00	1351.03	1351.03
3848	#3848:2	Tube 1	End	37.30	32.88	32.73	4444.15	4444.15	0.00	25.5	65.00	65.00	1464.11	1464.11
3848	#3848:2	Tube 1	Ori	37.30	32.88	32.73	4444.15	4444.15	0.00	25.5	65.00	65.00	1464.11	1464.11
3848	3848:WVGD4	3848:WVGD4	End	41.00	34.16	34.01	4987.70	4987.70	0.00	26.6	65.00	65.00	1581.75	1581.75
3848	3848:WVGD4	3848:WVGD4	Ori	41.00	34.16	34.01	4987.70	4987.70	0.00	26.6	65.00	65.00	1581.75	1581.75
3848	#3848:3	SpliceT	End	42.92	34.82	34.68	5286.13	5286.13	0.00	27.2	65.00	65.00	1644.52	1644.52
3848	#3848:3	SpliceT	Ori	42.92	34.82	34.68	5286.13	5286.13	0.00	27.2	65.00	65.00	1644.52	1644.52
3848	#3848:4	SpliceB	End	47.50	35.78	42.69	6850.22	6850.22	0.00	22.9	65.00	65.00	2074.05	2074.05
3848	#3848:4	SpliceB	Ori	47.50	35.78	42.69	6850.22	6850.22	0.00	22.9	65.00	65.00	2074.05	2074.05
3848	3848:WVGD5	3848:WVGD5	End	51.00	36.99	44.15	7576.04	7576.04	0.00	23.8	65.00	65.00	2218.85	2218.85
3848	3848:WVGD5	3848:WVGD5	Ori	51.00	36.99	44.15	7576.04	7576.04	0.00	23.8	65.00	65.00	2218.85	2218.85
3848	#3848:5	Tube 2	End	56.00	38.72	46.23	8699.27	8699.27	0.00	25.0	65.00	65.00	2434.18	2434.18
3848	#3848:5	Tube 2	Ori	56.00	38.72	46.23	8699.28	8699.28	0.00	25.0	65.00	65.00	2434.18	2434.18
3848	3848:WVGD6	3848:WVGD6	End	61.00	40.44	48.31	9928.38	9928.38	0.00	26.2	65.00	65.00	2659.47	2659.47
3848	3848:WVGD6	3848:WVGD6	Ori	61.00	40.44	48.31	9928.38	9928.38	0.00	26.2	65.00	65.00	2659.47	2659.47
3848	#3848:6	Tube 2	End	63.71	41.38	49.44	10640.09	10640.09	0.00	26.9	65.00	65.00	2785.68	2785.68
3848	#3848:6	Tube 2	Ori	63.71	41.38	49.44	10640.09	10640.09	0.00	26.9	65.00	65.00	2785.68	2785.68
3848	#3848:7	SpliceT	End	66.42	42.31	50.57	11385.02	11385.02	0.00	27.6	65.00	65.00	2914.82	2914.82
3848	#3848:7	SpliceT	Ori	66.42	42.31	50.57	11385.02	11385.02	0.00	27.6	65.00	65.00	2914.82	2914.82
3848	3848:WVGD7	3848:WVGD7	End	71.00	43.90	52.48	12723.30	12723.30	0.00	28.7	65.00	65.00	3139.99	3139.99
3848	3848:WVGD7	3848:WVGD7	Ori	71.00	43.90	52.48	12723.30	12723.30	0.00	28.7	65.00	65.00	3139.99	3139.99
3848	#3848:8	Tube 3	End	76.00	45.62	54.56	14298.64	14298.64	0.00	29.9	65.00	64.88	3388.85	3388.85
3848	#3848:8	Tube 3	Ori	76.00	45.62	54.56	14298.64	14298.64	0.00	29.9	65.00	64.88	3388.85	3388.85
3848	3848:WVGD8	3848:WVGD8	End	81.00	47.35	56.64	15998.93	15998.93	0.00	31.2	65.00	63.67	3585.33	3585.33
3848	3848:WVGD8	3848:WVGD8	Ori	81.00	47.35	56.64	15998.93	15998.93	0.00	31.2	65.00	63.67	3585.33	3585.33
3848	#3848:9	Tube 3	End	86.00	49.08	58.72	17828.94	17828.94	0.00	32.4	65.00	62.46	3781.51	3781.51
3848	#3848:9	Tube 3	Ori	86.00	49.08	58.72	17828.94	17828.94	0.00	32.4	65.00	62.46	3781.51	3781.51
3848	3848:WVGD9	3848:WVGD9	End	91.00	50.80	60.81	19793.44	19793.44	0.00	33.6	65.00	61.24	3976.82	3976.82
3848	3848:WVGD9	3848:WVGD9	Ori	91.00	50.80	60.81	19793.44	19793.44	0.00	33.6	65.00	61.24	3976.82	3976.82
3848	3848:g	3848:g	End	96.00	52.53	62.89	21897.19	21897.19	0.00	34.9	65.00	60.03	4170.72	4170.72

**Tubular Davit Properties:**

Davit Steel Texture Property Shape	Stock Number	Steel Shape	Thickness	Base Diameter	Tip Diameter	Taper Coef.	Drag	Modulus of	Geometry of	Strength Check	Vertical Capacity	Tension Capacity	Compres. Capacity	Long. Capacity	Yield Stress	Weight Density
------------------------------------	--------------	-------------	-----------	---------------	--------------	-------------	------	------------	-------------	----------------	-------------------	------------------	-------------------	----------------	--------------	----------------

Label At End	or Depth or Depth				Elasticity			Type	Override						
	(in)	(in)	(in)	(in/ft)	(ksi)		(lbs)		(lbs)	(lbs)	(lbs)	(ksi)	(lbs/ft^3)		
20702-M 20702-M	8T	0.1875	7	5	0	1.3	29000	1 point	Calculated	0	0	0	0	65	0
20702-N 20702-N	8T	0.25	15	7.5	0	1.3	29000	1 point	Calculated	0	0	0	0	65	0

Intermediate Joints for Davit Property "20702-M":

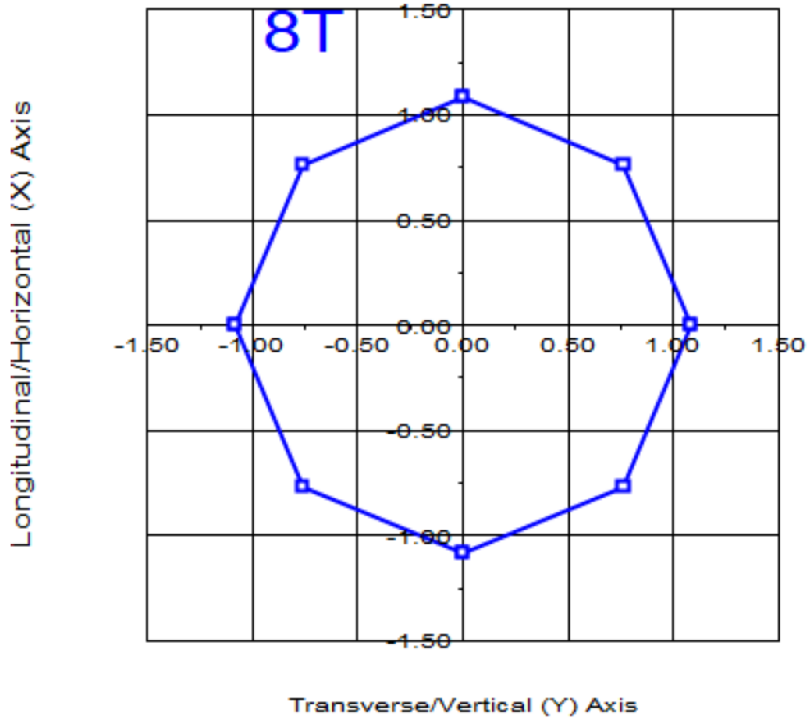
Joint Label	Horz. Offset (ft)	Vert. Offset (ft)
t	4	-0.5

Intermediate Joints for Davit Property "20702-N":

Joint Label	Horz. Offset (ft)	Vert. Offset (ft)
t	9	-0.75

Tubular Davit Arm Connectivity:

Davit Label	Attach Label	Davit Property Set	Azimuth (deg)
Davit1	3848:Arm1	20702-M	0
Davit2	3848:Arm1	20702-M	180
Davit3	3848:Arm2	20702-N	0
Davit4	3848:Arm2	20702-N	180
Davit5	3848:Arm3	20702-N	0
Davit6	3848:Arm3	20702-N	180
Davit7	3848:Arm4	20702-N	0
Davit8	3848:Arm4	20702-N	180



**Tubular Davit Arm Steel Properties:**

Element Label	Joint Label	Joint Position	Rel. Dist. (ft)	Outer Diam. (in)	Area (in <sup>2</sup> )	V-Moment Inertia (in <sup>4</sup> )	H-Moment Inertia (in <sup>4</sup> )	D/t	W/t Max.	Fy (ksi)	Fa Min. (ksi)	V-Moment Capacity (ft-k)	H-Moment Capacity (ft-k)
Davit1	Davit1:O	Origin	0.00	7.00	4.23	25.98	25.98	0.00	11.3	65.00	65.00	37.15	37.15
Davit1	Davit1:t	End	4.03	5.00	2.99	9.16	9.16	0.00	6.9	65.00	65.00	18.35	18.35
Davit2	Davit2:O	Origin	0.00	7.00	4.23	25.98	25.98	0.00	11.3	65.00	65.00	37.15	37.15
Davit2	Davit2:t	End	4.03	5.00	2.99	9.16	9.16	0.00	6.9	65.00	65.00	18.35	18.35
Davit3	Davit3:O	Origin	0.00	15.00	12.22	351.41	351.41	0.00	20.7	65.00	65.00	234.48	234.48
Davit3	#Davit3:O	End	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit3	#Davit3:O	Origin	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit3	Davit3:t	End	9.03	7.50	6.01	41.77	41.77	0.00	8.3	65.00	65.00	55.74	55.74
Davit4	Davit4:O	Origin	0.00	15.00	12.22	351.41	351.41	0.00	20.7	65.00	65.00	234.48	234.48
Davit4	#Davit4:O	End	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit4	#Davit4:O	Origin	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit4	Davit4:t	End	9.03	7.50	6.01	41.77	41.77	0.00	8.3	65.00	65.00	55.74	55.74
Davit5	Davit5:O	Origin	0.00	15.00	12.22	351.41	351.41	0.00	20.7	65.00	65.00	234.48	234.48



Davit5	#Davit5:0	End	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit5	#Davit5:0	Origin	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit5	Davit5:t	End	9.03	7.50	6.01	41.77	41.77	0.00	8.3	65.00	65.00	55.74	55.74
Davit6	Davit6:0	Origin	0.00	15.00	12.22	351.41	351.41	0.00	20.7	65.00	65.00	234.48	234.48
Davit6	#Davit6:0	End	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit6	#Davit6:0	Origin	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit6	Davit6:t	End	9.03	7.50	6.01	41.77	41.77	0.00	8.3	65.00	65.00	55.74	55.74
Davit7	Davit7:0	Origin	0.00	15.00	12.22	351.41	351.41	0.00	20.7	65.00	65.00	234.48	234.48
Davit7	#Davit7:0	End	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit7	#Davit7:0	Origin	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit7	Davit7:t	End	9.03	7.50	6.01	41.77	41.77	0.00	8.3	65.00	65.00	55.74	55.74
Davit8	Davit8:0	Origin	0.00	15.00	12.22	351.41	351.41	0.00	20.7	65.00	65.00	234.48	234.48
Davit8	#Davit8:0	End	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit8	#Davit8:0	Origin	4.52	11.25	9.11	145.79	145.79	0.00	14.5	65.00	65.00	129.70	129.70
Davit8	Davit8:t	End	9.03	7.50	6.01	41.77	41.77	0.00	8.3	65.00	65.00	55.74	55.74

\*\*\* Insulator Data

**Clamp Properties:**

Label	Stock Number	Holding Capacity (lbs)	Hardware Capacity (lbs)	Notes
clamp2	clamp2	1e+05	0	

-----

**Clamp Insulator Connectivity:**

Clamp Label	Structure And Tip Attach	Property Set	Min. Required Vertical Load (uplift) (lbs)
Clamp1	Davit1:t	clamp2	No Limit
Clamp2	Davit2:t	clamp2	No Limit
Clamp3	Davit3:t	clamp2	No Limit
Clamp4	Davit4:t	clamp2	No Limit
Clamp5	Davit5:t	clamp2	No Limit
Clamp6	Davit6:t	clamp2	No Limit
Clamp7	Davit7:t	clamp2	No Limit
Clamp8	Davit8:t	clamp2	No Limit
Clamp9	3848:WVGD1	clamp2	No Limit
Clamp10	3848:WVGD2	clamp2	No Limit
Clamp11	3848:WVGD3	clamp2	No Limit
Clamp12	3848:WVGD4	clamp2	No Limit
Clamp13	3848:WVGD5	clamp2	No Limit
Clamp14	3848:WVGD6	clamp2	No Limit
Clamp15	3848:WVGD7	clamp2	No Limit
Clamp16	3848:WVGD8	clamp2	No Limit
Clamp17	3848:WVGD9	clamp2	No Limit
Clamp18	3848:TopConn	clamp2	No Limit
Clamp19	3848:BotConn	clamp2	No Limit
Clamp20	3848:BCT	clamp2	No Limit
Clamp21	3848:BCB	clamp2	No Limit

Material List Options:

Show Parts: YES

Decompose Assemblies: NO

Show Assemblies: YES

Material List

Stock Number	Item Description	Quantity	Unit of Measure
20702-M	Tubular Davit property: 20702-M	2.00	Each
20702-N	Tubular Davit property: 20702-N	6.00	Each
clamp2	Clamp property: clamp2	21.00	Each
3848	Steel Pole property: CL&P3848	1.00	Each

\*\*\* Loads Data

Loads from file: J:\Jobs\2202100.WI\02\_CT2267\05\_Structural\Tower\Backup Documentation\Rev (2)\Calcs\PLS Pole\cl&p #3848.lca

Insulator dead and wind loads are already included in the point loads printed below.

Loading Method Parameters:

Structure Height Summary (used for calculating wind/ice adjust with height):

Z of ground for wind height adjust 0.00 (ft) and structure Z coordinate that will be put on the centerline ground profile in PLS-CADD.  
 Ground elevation shift 0.00 (ft)  
 Z of ground with shift 0.00 (ft)  
 Z of structure top (highest joint) 96.04 (ft)  
 Structure height 96.04 (ft)  
 Structure height above ground 96.04 (ft)

Vector Load Cases:

Trans.	Load Case	Dead	Wind	SF for	SF for	SF for	SF for	SF for	SF for	SF for	SF for	SF for	SF for	SF for	SF for	Point	Wind/Ice
	Longit.	Ice	Ice	Temperature	Pole	Pole	Conc.	Conc.	Guys	Non	Braces	Insuls.	Hardware	Found.	Loads	Model	
Wind	Wind Thick.	Density	Factor	Factor	Tubular	Arms	Poles	Ult.	First	Zero	and	Tubular					
Pressure	Pressure																
(psf)	(psf)	(in)	(lbs/ft^3)		and Towers		Check	Limit									
					(deg F)		% or	(ft)									
4	NESC Heavy Wind	1.5000	2.5000	1.00000	0.6500	0.0000	1.0000	0.0000	1.0000	1.0000	1.0000	1.0000	0.0000	1.0000	21 loads	Wind on All	
	0 0.000	56.000		60.0	No Limit		0										
31	NESC Extreme Wind	1.0000	1.0000	1.00000	0.6500	0.0000	1.0000	0.0000	1.0000	1.0000	1.0000	1.0000	0.0000	1.0000	21 loads	NESC 2023	
	0 0.000	0.000		60.0	No Limit		0										

Point Loads for Load Case "NESC Heavy Wind":

Joint Label	Vertical Load (lbs)	Transverse Load (lbs)	Longitudinal Load (lbs)	Load Comment
Davit1:t	795	603	-1	Shield Wire
Davit2:t	872	659	-1	Shield Wire
Davit3:t	2377	1055	-1	Conductor
Davit4:t	2369	1055	-1	Conductor
Davit5:t	2377	1054	-1	Conductor
Davit6:t	2369	1056	-1	Conductor
Davit7:t	2377	1053	-1	Conductor
Davit8:t	2369	1057	-1	Conductor
3848:TopConn	0	10202	0	Top Connection
3848:BotConn	15906	-6739	0	Bottom Connection
3848:BCT	0	21987	0	
3848:BCB	0	-21987	0	
3848:WVGD1	1035	172	0	Coax Cables
3848:WVGD2	1035	172	0	Coax Cables
3848:WVGD3	1035	172	0	Coax Cables
3848:WVGD4	1035	172	0	Coax Cables

3848:WVGD5	1035	172	0	Coax Cables
3848:WVGD6	1035	172	0	Coax Cables
3848:WVGD7	1035	172	0	Coax Cables
3848:WVGD8	1035	172	0	Coax Cables
3848:WVGD9	1035	172	0	Coax Cables

Point Loads for Load Case "NESC Extreme Wind":

Label	Joint Vertical Load (lbs)	Transverse Load (lbs)	Longitudinal Load (lbs)	Load Comment
Davit1:t	296	678	-3	Shield Wire
Davit2:t	321	858	-3	Shield Wire
Davit3:t	1179	2030	-7	Conductor
Davit4:t	1175	2027	-6	Conductor
Davit5:t	1179	2029	-7	Conductor
Davit6:t	1175	2028	-6	Conductor
Davit7:t	1179	2029	-7	Conductor
Davit8:t	1175	2028	-6	Conductor
3848:TopConn	0	38226	0	Top Connection
3848:BotConn	7896	-25861	0	Bottom Connection
3848:BCT	0	83953	0	
3848:BCB	0	-83953	0	
3848:WVGD1	281	477	0	Coax Cables
3848:WVGD2	281	477	0	Coax Cables
3848:WVGD3	281	477	0	Coax Cables
3848:WVGD4	281	477	0	Coax Cables
3848:WVGD5	281	477	0	Coax Cables
3848:WVGD6	281	477	0	Coax Cables
3848:WVGD7	281	477	0	Coax Cables
3848:WVGD8	281	477	0	Coax Cables
3848:WVGD9	281	477	0	Coax Cables

Detailed Pole Loading Data for Load Case "NESC Extreme Wind":

Notes: Does not include loads from equipment, arms, guys, braces, etc. or user input loads.

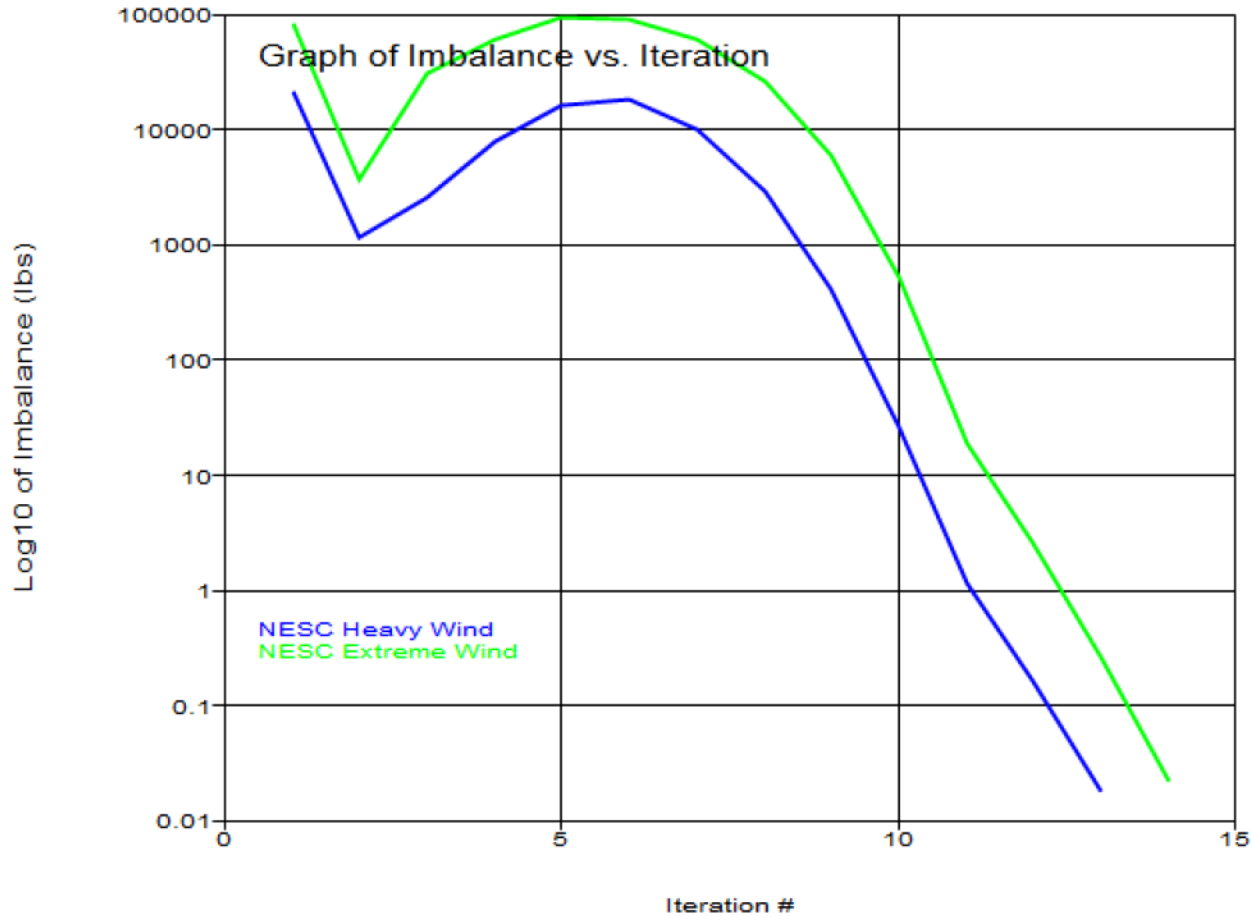
Wind load is calculated for the undeformed shape of a pole.

Pole Label	Top Joint	Bottom Joint	Section Top Z (ft)	Section Bottom Z (ft)	Section Average Elevation (ft)	Outer Diameter (in)	Reynolds Number	Drag Coef.	Adjusted Wind Pressure (psf)	Adjusted Ice Thickness (in)	Pole Vert. Load (lbs)	Pole Wind Load (lbs)	Pole Ice Vertical Load (lbs)	Pole Ice Wind Load (lbs)	Tran. Wind Load (lbs)	Long. Wind Load (lbs)
3848	3848:t	3848:Arm1	96.00	95.54	95.77	20.079	1.65e+06	1.000	30.13	0.00	30.95	23.09	0.00	0.00	23.09	0.00
3848	3848:Arm1	3848:TopConn	95.54	93.00	94.27	20.597	1.69e+06	1.000	30.13	0.00	176.30	131.45	0.00	0.00	131.45	0.00
3848	3848:TopConn	3848:Arm2	93.00	88.90	90.95	21.745	1.79e+06	1.000	30.13	0.00	300.74	224.05	0.00	0.00	224.05	0.00
3848	3848:Arm2	3848:WVGD1	88.90	85.00	86.95	23.126	1.9e+06	1.000	30.13	0.00	303.90	226.20	0.00	0.00	226.20	0.00
3848	3848:WVGD1	3848:BCT	85.00	83.50	84.25	24.058	1.98e+06	1.000	30.13	0.00	121.78	90.60	0.00	0.00	90.60	0.00
3848	3848:BCT	3848:BotConn	83.50	83.00	83.25	24.404	2.01e+06	1.000	30.13	0.00	41.19	30.63	0.00	0.00	30.63	0.00
3848	3848:BotConn	3848:BCB	83.00	82.50	82.75	24.576	2.02e+06	1.000	30.13	0.00	41.48	30.85	0.00	0.00	30.85	0.00
3848	3848:BCB		82.50	79.07	80.79	25.254	2.08e+06	1.000	30.13	0.00	292.25	217.28	0.00	0.00	217.28	0.00
3848		3848:Arm3	79.07	75.65	77.36	26.438	2.17e+06	1.000	30.13	0.00	306.12	227.47	0.00	0.00	227.47	0.00
3848	3848:Arm3	3848:WVGD2	75.65	75.00	75.32	27.141	2.23e+06	1.000	30.13	0.00	59.26	44.02	0.00	0.00	44.02	0.00
3848	3848:WVGD2		75.00	70.00	72.50	28.116	2.31e+06	1.000	30.13	0.00	475.33	352.94	0.00	0.00	352.94	0.00
3848		3848:WVGD3	70.00	65.00	67.50	29.843	2.45e+06	1.000	30.13	0.00	504.85	374.62	0.00	0.00	374.62	0.00
3848	3848:WVGD3	3848:Arm4	65.00	62.40	63.70	31.156	2.56e+06	1.000	30.13	0.00	274.62	203.69	0.00	0.00	203.69	0.00
3848	3848:Arm4		62.40	58.70	60.55	32.245	2.65e+06	1.000	30.13	0.00	403.75	299.36	0.00	0.00	299.36	0.00
3848		3848:WVGD4	58.70	55.00	56.85	33.522	2.75e+06	1.000	30.13	0.00	419.90	311.22	0.00	0.00	311.22	0.00

3848	3848:WVGD4		55.00	53.08	54.04	34.492	2.83e+06	1.000	30.13	0.00	224.03	166.00	0.00	0.00	166.00	0.00
3848			53.08	48.50	50.79	35.302	2.9e+06	1.000	30.13	0.00	1204.05	406.18	0.00	0.00	406.18	0.00
3848		3848:WVGD5	48.50	45.00	46.75	36.385	2.99e+06	1.000	30.13	0.00	517.20	319.72	0.00	0.00	319.72	0.00
3848	3848:WVGD5		45.00	40.00	42.50	37.853	3.11e+06	1.000	30.13	0.00	768.85	475.16	0.00	0.00	475.16	0.00
3848		3848:WVGD6	40.00	35.00	37.50	39.580	3.25e+06	1.000	30.13	0.00	804.28	496.84	0.00	0.00	496.84	0.00
3848	3848:WVGD6		35.00	32.29	33.65	40.911	3.36e+06	1.000	30.13	0.00	450.47	278.19	0.00	0.00	278.19	0.00
3848			32.29	29.58	30.94	41.846	3.44e+06	1.000	30.13	0.00	460.87	284.55	0.00	0.00	284.55	0.00
3848		3848:WVGD7	29.58	25.00	27.29	43.105	3.54e+06	1.000	30.13	0.00	803.50	495.97	0.00	0.00	495.97	0.00
3848	3848:WVGD7		25.00	20.00	22.50	44.760	3.68e+06	1.000	30.13	0.00	910.56	561.87	0.00	0.00	561.87	0.00
3848		3848:WVGD8	20.00	15.00	17.50	46.487	3.82e+06	1.000	30.13	0.00	945.98	583.55	0.00	0.00	583.55	0.00
3848	3848:WVGD8		15.00	10.00	12.50	48.214	3.96e+06	1.000	30.13	0.00	981.41	605.23	0.00	0.00	605.23	0.00
3848		3848:WVGD9	10.00	5.00	7.50	49.941	4.1e+06	1.000	30.13	0.00	1016.84	626.91	0.00	0.00	626.91	0.00
3848	3848:WVGD9	3848:g	5.00	0.00	2.50	51.668	4.25e+06	1.000	30.13	0.00	1052.27	648.58	0.00	0.00	648.58	0.00

\*\*\* Analysis Results:

Maximum element usage is 80.63% for Steel Pole "3848" in load case "NESC Extreme Wind"  
 Maximum insulator usage is 83.95% for Clamp "Clamp20" in load case "NESC Extreme Wind"



\*\*\* Analysis Results for Load Case No. 1 "NESC Heavy Wind" - Number of iterations in SAPS 13

Equilibrium Joint Positions and Rotations for Load Case "NESC Heavy Wind":

Joint Label	X-Displ (ft)	Y-Displ (ft)	Z-Displ (ft)	X-Rot (deg)	Y-Rot (deg)	Z-Rot (deg)	X-Pos (ft)	Y-Pos (ft)	Z-Pos (ft)
3848:g	0	0	0	0.0000	0.0000	0.0000	0	0	0
3848:t	0.003552	1.721	-0.02467	-2.0204	0.0032	0.0001	0.003552	1.721	95.98
3848:Arm1	0.003527	1.705	-0.02439	-2.0204	0.0032	0.0001	0.003527	1.705	95.52

3848:TopConn	0.003385	1.616	-0.0228	-2.0190	0.0032	0.0001	0.003385	1.616	92.98
3848:Arm2	0.003155	1.471	-0.02025	-1.9935	0.0032	0.0000	0.003155	1.471	88.88
3848:WVGD1	0.002938	1.337	-0.0179	-1.9348	0.0032	0.0000	0.002938	1.337	84.98
3848:BCT	0.002855	1.287	-0.01704	-1.9053	0.0032	0.0000	0.002855	1.287	83.48
3848:BotConn	0.002827	1.27	-0.01675	-1.8942	0.0032	0.0000	0.002827	1.27	82.98
3848:BCB	0.002799	1.254	-0.01646	-1.8822	0.0032	0.0000	0.002799	1.254	82.48
3848:Arm3	0.002422	1.039	-0.01285	-1.7080	0.0031	0.0000	0.002422	1.039	75.63
3848:WVGD2	0.002387	1.02	-0.01253	-1.6913	0.0031	0.0000	0.002387	1.02	74.99
3848:WVGD3	0.00186	0.7469	-0.0084	-1.4258	0.0029	0.0000	0.00186	0.7469	64.99
3848:Arm4	0.001729	0.6836	-0.007526	-1.3561	0.0028	0.0000	0.001729	0.6836	62.39
3848:WVGD4	0.001377	0.521	-0.005406	-1.1566	0.0026	0.0000	0.001377	0.521	54.99
3848:WVGD5	0.0009517	0.3409	-0.003378	-0.9086	0.0022	0.0000	0.0009517	0.3409	45
3848:WVGD6	0.0005933	0.2017	-0.002037	-0.6833	0.0018	0.0000	0.0005933	0.2017	35
3848:WVGD7	0.0003117	0.1008	-0.001162	-0.4711	0.0014	0.0000	0.0003117	0.1008	25
3848:WVGD8	0.0001159	0.03567	-0.0005886	-0.2728	0.0008	0.0000	0.0001159	0.03567	15
3848:WVGD9	1.382e-05	0.003997	-0.0001797	-0.0878	0.0003	0.0000	1.382e-05	0.003997	5
Davit1:O	0.003524	1.705	-0.054	-2.0204	0.0032	0.0001	0.003524	2.545	95.49
Davit1:t	0.003534	1.72	-0.2008	-2.1364	0.0032	0.0002	0.003534	6.56	95.84
Davit2:O	0.003529	1.706	0.005225	-2.0204	0.0032	0.0001	0.003529	0.8657	95.55
Davit2:t	0.003562	1.725	0.141	-1.9160	0.0032	-0.0001	0.003562	-3.115	96.18
Davit3:O	0.003153	1.471	-0.0528	-1.9935	0.0032	0.0000	0.003153	2.406	88.84
Davit3:t	0.003163	1.492	-0.3817	-2.1614	0.0032	0.0001	0.003163	11.43	89.26
Davit4:O	0.003158	1.472	0.01229	-1.9935	0.0032	0.0000	0.003158	0.5364	88.91
Davit4:t	0.003219	1.502	0.3107	-1.8382	0.0032	-0.0000	0.003219	-8.434	89.96
Davit5:O	0.002419	1.038	-0.04641	-1.7080	0.0031	0.0000	0.002419	2.165	75.6
Davit5:t	0.002434	1.058	-0.3304	-1.8762	0.0031	0.0001	0.002434	11.18	76.07
Davit6:O	0.002424	1.039	0.02072	-1.7080	0.0031	0.0000	0.002424	-0.08688	75.67
Davit6:t	0.002479	1.064	0.2744	-1.5523	0.0031	-0.0000	0.002479	-9.062	76.67
Davit7:O	0.001727	0.6832	-0.03869	-1.3561	0.0028	0.0000	0.001727	2	62.36
Davit7:t	0.001745	0.6994	-0.2674	-1.5248	0.0028	0.0001	0.001745	11.02	62.88
Davit8:O	0.001731	0.684	0.02364	-1.3561	0.0028	0.0000	0.001731	-0.633	62.42
Davit8:t	0.001776	0.7028	0.2222	-1.2000	0.0028	-0.0000	0.001776	-9.614	63.37

Joint Support Reactions for Load Case "NESC Heavy Wind":

Joint Label	X Force (kips)	X Usage %	Y Force (kips)	Y Usage %	H-Shear Usage %	Z Comp. Force (kips)	Z Usage %	Uplift Usage %	Result. Force (kips)	Result. Usage %	X Moment (ft-k)	X-M. Usage %	Y Moment (ft-k)	Y-M. Usage %	H-Bend-M Usage %	Z Moment (ft-k)	Z-M. Usage %	Max. Usage %
3848:g	-0.11	0.0	-17.24	0.0	0.0	-67.00	0.0	0.0	69.19	0.0	1329.29	0.0	-4.6	0.0	0.0	-0.00	0.0	0.0

Detailed Steel Pole Usages for Load Case "NESC Heavy Wind":

Element Label	Joint Label	Joint Position	Rel. Dist. (ft)	Trans. Defl. (in)	Long. Defl. (in)	Vert. Defl. (in)	Trans. Mom. (Local Mx) (ft-k)	Long. Mom. (Local My) (ft-k)	Tors. Mom. (ft-k)	Axial Force (kips)	Tran. Shear (kips)	Long. Shear (kips)	P/A (ksi)	M/S. (ksi)	V/Q. (ksi)	T/R. (ksi)	Res. (ksi)	Max. Usage %	At Pt.
3848	3848:t	Origin	0.00	20.66	0.04	-0.30	-0.00	-0.00	-0.0	-0.02	0.01	-0.00	-0.00	0.00	0.00	0.00	0.00	0.0	5
3848	3848:Arm1	End	0.46	20.46	0.04	-0.29	0.00	-0.00	-0.0	-0.02	0.01	-0.00	-0.00	0.00	0.00	0.00	0.00	0.0	4
3848	3848:Arm1	Origin	0.46	20.46	0.04	-0.29	0.30	0.00	0.0	-1.95	1.38	0.00	-0.10	0.00	0.14	0.00	0.26	0.4	5
3848	3848:TopConn	End	3.00	19.39	0.04	-0.27	3.80	0.00	0.0	-1.95	1.38	0.00	-0.09	0.42	0.00	0.00	0.51	0.8	2
3848	3848:TopConn	Origin	3.00	19.39	0.04	-0.27	3.80	0.00	0.0	-1.95	11.68	-0.00	-0.09	0.00	1.14	0.00	1.98	3.0	5
3848	3848:Arm2	End	7.10	17.66	0.04	-0.24	51.74	-0.01	0.0	-1.95	11.68	-0.00	-0.09	4.99	0.00	0.00	5.08	7.8	2
3848	3848:Arm2	Origin	7.10	17.66	0.04	-0.24	53.51	-0.01	0.0	-7.92	14.12	-0.00	-0.36	5.16	0.00	0.00	5.52	8.5	2
3848	3848:WVGD1	End	11.00	16.05	0.04	-0.21	108.50	-0.02	0.0	-7.92	14.12	-0.00	-0.34	9.30	0.00	0.00	9.63	14.8	2
3848	3848:WVGD1	Origin	11.00	16.05	0.04	-0.21	108.50	-0.02	0.0	-9.28	14.41	-0.01	-0.39	9.30	0.00	0.00	9.69	14.9	2
3848	3848:BCT	End	12.50	15.45	0.03	-0.20	130.12	-0.03	0.0	-9.28	14.41	-0.01	-0.38	10.67	0.00	0.00	11.06	17.0	2
3848	3848:BCT	Origin	12.50	15.45	0.03	-0.20	130.12	-0.03	0.0	-8.67	36.42	-0.01	-0.36	10.67	0.00	0.00	11.03	17.0	2

3848	3848:BotConn	End	13.00	15.25	0.03	-0.20	148.33	-0.03	0.0	-8.67	36.42	-0.01	-0.36	11.99	0.00	0.00	12.35	19.0	2
3848	3848:BotConn	Origin	13.00	15.25	0.03	-0.20	148.33	-0.03	0.0	-24.86	30.23	-0.01	-1.02	11.99	0.00	0.00	13.01	20.0	2
3848	3848:BCB	End	13.50	15.05	0.03	-0.20	163.44	-0.04	0.0	-24.86	30.23	-0.01	-1.02	13.02	0.00	0.00	14.04	21.6	2
3848	3848:BCB	Origin	13.50	15.05	0.03	-0.20	163.44	-0.04	0.0	-25.85	8.30	-0.01	-1.06	13.02	0.00	0.00	14.08	21.7	2
3848	Tube 1	End	16.93	13.73	0.03	-0.17	191.89	-0.07	0.0	-25.85	8.30	-0.01	-1.01	13.90	0.00	0.00	14.91	22.9	2
3848	Tube 1	Origin	16.93	13.73	0.03	-0.17	191.89	-0.07	0.0	-26.31	8.39	-0.01	-1.03	13.90	0.00	0.00	14.92	23.0	2
3848	3848:Arm3	End	20.35	12.47	0.03	-0.15	220.65	-0.12	0.0	-26.31	8.39	-0.01	-0.98	14.59	0.00	0.00	15.57	24.0	2
3848	3848:Arm3	Origin	20.35	12.47	0.03	-0.15	222.41	-0.12	0.0	-32.11	10.72	-0.01	-1.20	14.71	0.00	0.00	15.90	24.5	2
3848	3848:WVGD2	End	21.00	12.24	0.03	-0.15	229.33	-0.13	0.0	-32.11	10.72	-0.01	-1.19	14.91	0.00	0.00	16.10	24.8	2
3848	3848:WVGD2	Origin	21.00	12.24	0.03	-0.15	229.33	-0.13	0.0	-33.55	11.00	-0.02	-1.24	14.91	0.00	0.00	16.15	24.8	2
3848	Tube 1	End	26.00	10.53	0.03	-0.12	284.34	-0.21	0.0	-33.55	11.00	-0.02	-1.16	16.32	0.00	0.00	17.48	26.9	2
3848	Tube 1	Origin	26.00	10.53	0.03	-0.12	284.34	-0.21	0.0	-34.31	11.14	-0.02	-1.19	16.32	0.00	0.00	17.51	26.9	2
3848	3848:WVGD3	End	31.00	8.96	0.02	-0.10	340.02	-0.32	0.0	-34.31	11.14	-0.02	-1.12	17.35	0.00	0.00	18.47	28.4	2
3848	3848:WVGD3	Origin	31.00	8.96	0.02	-0.10	340.02	-0.32	0.0	-35.95	11.44	-0.03	-1.18	17.35	0.00	0.00	18.53	28.5	2
3848	3848:Arm4	End	33.60	8.20	0.02	-0.09	369.81	-0.38	0.0	-35.95	11.44	-0.03	-1.14	17.80	0.00	0.00	18.94	29.1	2
3848	3848:Arm4	Origin	33.60	8.20	0.02	-0.09	371.54	-0.38	0.0	-42.01	13.77	-0.03	-1.34	17.88	0.00	0.00	19.22	29.6	2
3848	Tube 1	End	37.30	7.19	0.02	-0.08	422.46	-0.49	0.0	-42.01	13.77	-0.03	-1.28	18.76	0.00	0.00	20.04	30.8	2
3848	Tube 1	Origin	37.30	7.19	0.02	-0.08	422.46	-0.49	0.0	-42.65	13.87	-0.03	-1.30	18.76	0.00	0.00	20.06	30.9	2
3848	3848:WVGD4	End	41.00	6.25	0.02	-0.06	473.75	-0.61	0.0	-42.65	13.87	-0.03	-1.25	19.47	0.00	0.00	20.73	31.9	2
3848	3848:WVGD4	Origin	41.00	6.25	0.02	-0.06	473.75	-0.61	0.0	-44.19	14.14	-0.04	-1.30	19.47	0.00	0.00	20.77	32.0	2
3848	SpliceT	End	42.92	5.80	0.02	-0.06	500.86	-0.67	0.0	-44.19	14.14	-0.04	-1.27	19.80	0.00	0.00	21.08	32.4	2
3848	SpliceT	Origin	42.92	5.80	0.02	-0.06	500.86	-0.67	0.0	-45.28	14.25	-0.04	-1.31	19.80	0.00	0.00	21.11	32.5	2
3848	SpliceB	End	47.50	4.79	0.01	-0.05	566.17	-0.85	0.0	-45.28	14.25	-0.04	-1.06	17.75	0.00	0.00	18.81	28.9	2
3848	SpliceB	Origin	47.50	4.79	0.01	-0.05	566.17	-0.85	0.0	-46.59	14.39	-0.04	-1.09	17.75	0.00	0.00	18.84	29.0	2
3848	3848:WVGD5	End	51.00	4.09	0.01	-0.04	616.52	-1.01	0.0	-46.59	14.39	-0.04	-1.06	18.07	0.00	0.00	19.12	29.4	2
3848	3848:WVGD5	Origin	51.00	4.09	0.01	-0.04	616.52	-1.01	0.0	-48.61	14.72	-0.05	-1.10	18.07	0.00	0.00	19.17	29.5	2
3848	Tube 2	End	56.00	3.20	0.01	-0.03	690.12	-1.26	0.0	-48.61	14.72	-0.05	-1.05	18.44	0.00	0.00	19.49	30.0	2
3848	Tube 2	Origin	56.00	3.20	0.01	-0.03	690.12	-1.26	0.0	-49.82	14.90	-0.06	-1.08	18.44	0.00	0.00	19.51	30.0	2
3848	3848:WVGD6	End	61.00	2.42	0.01	-0.02	764.60	-1.54	0.0	-49.82	14.90	-0.06	-1.03	18.70	0.00	0.00	19.73	30.4	2
3848	3848:WVGD6	Origin	61.00	2.42	0.01	-0.02	764.60	-1.54	0.0	-51.82	15.22	-0.06	-1.07	18.70	0.00	0.00	19.77	30.4	2
3848	Tube 2	End	63.71	2.05	0.01	-0.02	805.84	-1.71	0.0	-51.82	15.22	-0.06	-1.05	18.81	0.00	0.00	19.86	30.6	2
3848	Tube 2	Origin	63.71	2.05	0.01	-0.02	805.84	-1.71	0.0	-52.52	15.33	-0.07	-1.06	18.81	0.00	0.00	19.88	30.6	2
3848	SpliceT	End	66.42	1.71	0.01	-0.02	847.35	-1.88	0.0	-52.52	15.33	-0.07	-1.04	18.91	0.00	0.00	19.95	30.7	2
3848	SpliceT	Origin	66.42	1.71	0.01	-0.02	847.35	-1.88	0.0	-53.49	15.47	-0.07	-1.06	18.91	0.00	0.00	19.96	30.7	2
3848	3848:WVGD7	End	71.00	1.21	0.00	-0.01	918.26	-2.21	0.0	-53.49	15.47	-0.07	-1.02	19.02	0.00	0.00	20.04	30.8	2
3848	3848:WVGD7	Origin	71.00	1.21	0.00	-0.01	918.26	-2.21	0.0	-55.83	15.85	-0.08	-1.06	19.02	0.00	0.00	20.08	30.9	2
3848	Tube 3	End	76.00	0.77	0.00	-0.01	997.51	-2.60	0.0	-55.83	15.85	-0.08	-1.02	19.11	0.00	0.00	20.13	31.0	2
3848	Tube 3	Origin	76.00	0.77	0.00	-0.01	997.51	-2.60	0.0	-57.25	16.07	-0.09	-1.05	19.11	0.00	0.00	20.16	31.1	2
3848	3848:WVGD8	End	81.00	0.43	0.00	-0.01	1077.84	-3.03	0.0	-57.25	16.07	-0.09	-1.01	19.15	0.00	0.00	20.17	31.7	2
3848	3848:WVGD8	Origin	81.00	0.43	0.00	-0.01	1077.84	-3.03	0.0	-59.76	16.47	-0.09	-1.06	19.15	0.00	0.00	20.21	31.7	2
3848	Tube 3	End	86.00	0.19	0.00	-0.00	1160.18	-3.49	0.0	-59.76	16.47	-0.09	-1.02	19.18	0.00	0.00	20.19	32.3	2
3848	Tube 3	Origin	86.00	0.19	0.00	-0.00	1160.18	-3.49	0.0	-61.28	16.70	-0.10	-1.04	19.18	0.00	0.00	20.22	32.4	2
3848	3848:WVGD9	End	91.00	0.05	0.00	-0.00	1243.69	-4.00	0.0	-61.28	16.70	-0.10	-1.01	19.17	0.00	0.00	20.18	32.9	2
3848	3848:WVGD9	Origin	91.00	0.05	0.00	-0.00	1243.69	-4.00	0.0	-63.90	17.12	-0.11	-1.05	19.17	0.00	0.00	20.22	33.0	2
3848	3848:g	End	96.00	0.00	0.00	0.00	1329.29	-4.56	0.0	-63.90	17.12	-0.11	-1.02	19.15	0.00	0.00	20.17	33.6	2

Detailed Tubular Davit Arm Usages for Load Case "NESC Heavy Wind":

Element Label	Joint Label	Joint Position	Rel. Dist. (ft)	Trans. Defl. (in)	Long. Defl. (in)	Vert. Defl. (in)	Vert. Mom. (ft-k)	Horz. Mom. (ft-k)	Tors. Mom. (ft-k)	Axial Force (kips)	Vert. Shear (kips)	Horz. Shear (kips)	P/A (ksi)	M/S. (ksi)	V/Q. (ksi)	T/R. (ksi)	Res. (ksi)	Max. Usage %	At Pt.
Davit1	Davit1:O	Origin	0.00	20.46	0.04	-0.65	-3.56	0.00	-0.0	0.53	0.88	-0.00	0.12	6.22	0.00	0.00	6.35	9.8	1
Davit1	Davit1:t	End	4.03	20.64	0.04	-2.41	0.00	0.00	-0.0	0.53	0.88	-0.00	0.18	0.00	0.61	0.00	1.08	1.7	3
Davit2	Davit2:O	Origin	0.00	20.47	0.04	0.06	-3.20	-0.00	0.0	-0.79	0.79	0.00	-0.19	5.60	0.00	0.00	5.79	8.9	1
Davit2	Davit2:t	End	4.03	20.70	0.04	1.69	-0.00	0.00	0.0	-0.79	0.79	0.00	-0.27	0.00	0.55	0.00	0.99	1.5	3



Davit3	Davit3:O	Origin	0.00	17.65	0.04	-0.63	-23.62	0.01	-0.0	0.93	2.72	-0.00	0.08	6.55	0.00	0.00	6.62	10.2	1
Davit3	#Davit3:O	End	4.52	17.78	0.04	-2.56	-11.33	0.00	-0.0	0.93	2.72	-0.00	0.10	5.68	0.00	0.00	5.78	8.9	1
Davit3	#Davit3:O	Origin	4.52	17.78	0.04	-2.56	-11.33	0.00	0.0	0.94	2.51	-0.00	0.10	5.68	0.00	0.00	5.78	8.9	1
Davit3	Davit3:t	End	9.03	17.91	0.04	-4.58	0.00	0.00	0.0	0.94	2.51	-0.00	0.16	0.00	0.87	0.00	1.51	2.3	3
Davit4	Davit4:O	Origin	0.00	17.66	0.04	0.15	-21.86	-0.01	0.0	-1.36	2.52	0.00	-0.11	6.06	0.00	0.00	6.17	9.5	1
Davit4	#Davit4:O	End	4.52	17.85	0.04	1.98	-10.47	-0.00	0.0	-1.36	2.52	0.00	-0.15	5.24	0.00	0.00	5.39	8.3	1
Davit4	#Davit4:O	Origin	4.52	17.85	0.04	1.98	-10.47	-0.00	0.0	-1.33	2.32	0.00	-0.15	5.24	0.00	0.00	5.39	8.3	1
Davit4	Davit4:t	End	9.03	18.02	0.04	3.73	-0.00	0.00	0.0	-1.33	2.32	0.00	-0.22	0.00	0.80	0.00	1.41	2.2	3
Davit5	Davit5:O	Origin	0.00	12.46	0.03	-0.56	-23.66	0.01	-0.0	0.91	2.73	-0.00	0.07	6.56	0.00	0.00	6.63	10.2	1
Davit5	#Davit5:O	End	4.52	12.57	0.03	-2.22	-11.35	0.00	-0.0	0.91	2.73	-0.00	0.10	5.69	0.00	0.00	5.79	8.9	1
Davit5	#Davit5:O	Origin	4.52	12.57	0.03	-2.22	-11.35	0.00	0.0	0.93	2.51	-0.00	0.10	5.69	0.00	0.00	5.79	8.9	1
Davit5	Davit5:t	End	9.03	12.69	0.03	-3.96	0.00	0.00	0.0	0.93	2.51	-0.00	0.15	0.00	0.87	0.00	1.51	2.3	3
Davit6	Davit6:O	Origin	0.00	12.47	0.03	0.25	-21.92	-0.01	0.0	-1.35	2.53	0.00	-0.11	6.08	0.00	0.00	6.19	9.5	1
Davit6	#Davit6:O	End	4.52	12.63	0.03	1.81	-10.50	-0.00	0.0	-1.35	2.53	0.00	-0.15	5.26	0.00	0.00	5.41	8.3	1
Davit6	#Davit6:O	Origin	4.52	12.63	0.03	1.81	-10.50	-0.00	0.0	-1.32	2.32	0.00	-0.14	5.26	0.00	0.00	5.40	8.3	1
Davit6	Davit6:t	End	9.03	12.77	0.03	3.29	-0.00	0.00	0.0	-1.32	2.32	0.00	-0.22	0.00	0.80	0.00	1.41	2.2	3
Davit7	Davit7:O	Origin	0.00	8.20	0.02	-0.46	-23.71	0.01	-0.0	0.89	2.73	-0.00	0.07	6.57	0.00	0.00	6.65	10.2	1
Davit7	#Davit7:O	End	4.52	8.29	0.02	-1.79	-11.38	0.00	-0.0	0.89	2.73	-0.00	0.10	5.70	0.00	0.00	5.80	8.9	1
Davit7	#Davit7:O	Origin	4.52	8.29	0.02	-1.79	-11.38	0.00	-0.0	0.91	2.52	-0.00	0.10	5.70	0.00	0.00	5.80	8.9	1
Davit7	Davit7:t	End	9.03	8.39	0.02	-3.21	0.00	0.00	-0.0	0.91	2.52	-0.00	0.15	0.00	0.87	0.00	1.52	2.3	3
Davit8	Davit8:O	Origin	0.00	8.21	0.02	0.28	-21.99	-0.01	0.0	-1.33	2.54	0.00	-0.11	6.10	0.00	0.00	6.21	9.5	1
Davit8	#Davit8:O	End	4.52	8.32	0.02	1.52	-10.53	-0.00	0.0	-1.33	2.54	0.00	-0.15	5.28	0.00	0.00	5.42	8.3	1
Davit8	#Davit8:O	Origin	4.52	8.32	0.02	1.52	-10.53	-0.00	0.0	-1.31	2.33	0.00	-0.14	5.28	0.00	0.00	5.42	8.3	1
Davit8	Davit8:t	End	9.03	8.43	0.02	2.67	-0.00	0.00	0.0	-1.31	2.33	0.00	-0.22	0.00	0.81	0.00	1.41	2.2	3

Summary of Clamp Capacities and Usages for Load Case "NESC Heavy Wind":

Clamp Label	Force (kips)	Input Holding Capacity (kips)	Factored Holding Capacity (kips)	Holding Usage %	Input Hardware Capacity (kips)	Factored Hardware Capacity (kips)	Hardware Usage %	Max. Usage %
Clamp1	0.998	100.00	100.00	1.00	0.00	0.00	0.00	1.00
Clamp2	1.093	100.00	100.00	1.09	0.00	0.00	0.00	1.09
Clamp3	2.601	100.00	100.00	2.60	0.00	0.00	0.00	2.60
Clamp4	2.593	100.00	100.00	2.59	0.00	0.00	0.00	2.59
Clamp5	2.600	100.00	100.00	2.60	0.00	0.00	0.00	2.60
Clamp6	2.594	100.00	100.00	2.59	0.00	0.00	0.00	2.59
Clamp7	2.600	100.00	100.00	2.60	0.00	0.00	0.00	2.60
Clamp8	2.594	100.00	100.00	2.59	0.00	0.00	0.00	2.59
Clamp9	1.049	100.00	100.00	1.05	0.00	0.00	0.00	1.05
Clamp10	1.049	100.00	100.00	1.05	0.00	0.00	0.00	1.05
Clamp11	1.049	100.00	100.00	1.05	0.00	0.00	0.00	1.05
Clamp12	1.049	100.00	100.00	1.05	0.00	0.00	0.00	1.05
Clamp13	1.049	100.00	100.00	1.05	0.00	0.00	0.00	1.05
Clamp14	1.049	100.00	100.00	1.05	0.00	0.00	0.00	1.05
Clamp15	1.049	100.00	100.00	1.05	0.00	0.00	0.00	1.05
Clamp16	1.049	100.00	100.00	1.05	0.00	0.00	0.00	1.05
Clamp17	1.049	100.00	100.00	1.05	0.00	0.00	0.00	1.05
Clamp18	10.202	100.00	100.00	10.20	0.00	0.00	0.00	10.20
Clamp19	17.275	100.00	100.00	17.27	0.00	0.00	0.00	17.27
Clamp20	21.987	100.00	100.00	21.99	0.00	0.00	0.00	21.99
Clamp21	21.987	100.00	100.00	21.99	0.00	0.00	0.00	21.99



Equilibrium Joint Positions and Rotations for Load Case "NESC Extreme Wind":

Joint Label	X-Displ (ft)	Y-Displ (ft)	Z-Displ (ft)	X-Rot (deg)	Y-Rot (deg)	Z-Rot (deg)	X-Pos (ft)	Y-Pos (ft)	Z-Pos (ft)
3848:g	0	0	0	0.0000	0.0000	0.0000	0	0	0
3848:t	-0.003232	4.389	-0.1447	-5.3993	-0.0035	0.0001	-0.003232	4.389	95.86
3848:Arm1	-0.003204	4.346	-0.1426	-5.3993	-0.0035	0.0001	-0.003204	4.346	95.4
3848:TopConn	-0.003047	4.107	-0.1313	-5.3974	-0.0035	0.0001	-0.003047	4.107	92.87
3848:Arm2	-0.002795	3.722	-0.1133	-5.3174	-0.0035	0.0001	-0.002795	3.722	88.78
3848:WVGD1	-0.002558	3.366	-0.09697	-5.1299	-0.0034	0.0001	-0.002558	3.366	84.9
3848:BC1	-0.002468	3.233	-0.09105	-5.0358	-0.0034	0.0001	-0.002468	3.233	83.41
3848:BotConn	-0.002438	3.189	-0.08911	-5.0006	-0.0034	0.0001	-0.002438	3.189	82.91
3848:BC2	-0.002408	3.146	-0.08721	-4.9615	-0.0034	0.0001	-0.002408	3.146	82.41
3848:Arm3	-0.002013	2.585	-0.06409	-4.4167	-0.0032	0.0001	-0.002013	2.585	75.58
3848:WVGD2	-0.001977	2.535	-0.06217	-4.3663	-0.0032	0.0001	-0.001977	2.535	74.94
3848:WVGD3	-0.001454	1.839	-0.03767	-3.6024	-0.0028	0.0001	-0.001454	1.839	64.96
3848:Arm4	-0.00133	1.68	-0.03274	-3.4100	-0.0026	0.0001	-0.00133	1.68	62.36
3848:WVGD4	-0.00101	1.273	-0.0214	-2.8743	-0.0023	0.0001	-0.00101	1.273	54.98
3848:WVGD5	-0.0006539	0.8287	-0.01127	-2.2322	-0.0018	0.0000	-0.0006539	0.8287	44.99
3848:WVGD6	-0.0003812	0.4881	-0.005254	-1.6650	-0.0013	0.0000	-0.0003812	0.4881	34.99
3848:WVGD7	-0.0001871	0.243	-0.002039	-1.1407	-0.0009	0.0000	-0.0001871	0.243	25
3848:WVGD8	-6.481e-05	0.08571	-0.000596	-0.6572	-0.0005	0.0000	-6.481e-05	0.08571	15
3848:WVGD9	-7.037e-06	0.009568	-0.0001025	-0.2106	-0.0002	0.0000	-7.037e-06	0.009568	5
Davit1:0	-0.0032	4.343	-0.2217	-5.3993	-0.0035	0.0001	-0.0032	5.183	95.32
Davit1:t	-0.003234	4.372	-0.6024	-5.4442	-0.0035	0.0005	-0.003234	9.212	95.44
Davit2:0	-0.003207	4.35	-0.06359	-5.3993	-0.0035	0.0001	-0.003207	3.51	95.48
Davit2:t	-0.003273	4.415	0.3097	-5.3796	-0.0036	-0.0003	-0.003273	-0.4252	96.35
Davit3:0	-0.002791	3.718	-0.2	-5.3174	-0.0035	0.0001	-0.002791	4.654	88.7
Davit3:t	-0.002843	3.749	-1.045	-5.3991	-0.0035	0.0006	-0.002843	13.68	88.6
Davit4:0	-0.002799	3.726	-0.02658	-5.3174	-0.0035	0.0001	-0.002799	2.791	88.87
Davit4:t	-0.002916	3.834	0.7989	-5.2592	-0.0036	-0.0003	-0.002916	-6.102	90.44
Davit5:0	-0.00201	2.581	-0.1508	-4.4167	-0.0032	0.0001	-0.00201	3.708	75.5
Davit5:t	-0.00207	2.612	-0.8539	-4.5006	-0.0032	0.0006	-0.00207	12.74	75.54
Davit6:0	-0.002016	2.588	0.02264	-4.4167	-0.0032	0.0001	-0.002016	1.462	75.67
Davit6:t	-0.002116	2.672	0.708	-4.3562	-0.0033	-0.0003	-0.002116	-7.455	77.1
Davit7:0	-0.001329	1.677	-0.1111	-3.4100	-0.0026	0.0001	-0.001329	2.994	62.28
Davit7:t	-0.001392	1.706	-0.6556	-3.4961	-0.0027	0.0005	-0.001392	12.02	62.49
Davit8:0	-0.001332	1.682	0.04559	-3.4100	-0.0026	0.0001	-0.001332	0.365	62.44
Davit8:t	-0.001415	1.742	0.5738	-3.3469	-0.0027	-0.0003	-0.001415	-8.575	63.72

Joint Support Reactions for Load Case "NESC Extreme Wind":

Joint Label	X Force (kips)	X Usage %	Y Force (kips)	Y Usage %	H-Shear Usage %	Z Force (kips)	Comp. Usage %	Uplift Usage %	Result. Force (kips)	Result. Usage %	X Moment (ft-k)	X-M. Usage %	Y Moment (ft-k)	Y-M. Usage %	H-Bend-M Usage %	Z Moment (ft-k)	Z-M. Usage %	Max. Usage %
3848:g	0.01	0.0	-39.08	0.0	0.0	-36.38	0.0	0.0	53.39	0.0	3183.47	0.0	2.4	0.0	0.0	-0.03	0.0	0.0

Detailed Steel Pole Usages for Load Case "NESC Extreme Wind":

Element Label	Joint Label	Joint Position	Rel. Dist. (ft)	Trans. Defl. (in)	Long. Defl. (in)	Vert. Defl. (in)	Trans. Mom. (Local Mx) (ft-k)	Long. Mom. (Local My) (ft-k)	Tors. Mom. (ft-k)	Axial Force (kips)	Tran. Shear (kips)	Long. Shear (kips)	P/A (ksi)	M/S. (ksi)	V/Q. (ksi)	T/R. (ksi)	Res. (ksi)	Max. Usage %	At Pt.
---------------	-------------	----------------	-----------------	-------------------	------------------	------------------	-------------------------------	------------------------------	-------------------	--------------------	--------------------	--------------------	-----------	------------	------------	------------	------------	--------------	--------

3848	3848:t	Origin	0.00	52.67	-0.04	-1.74	-0.00	0.00	-0.0	-0.02	0.01	-0.00	-0.00	0.00	0.00	0.00	0.0	5
3848	3848:Arm1	End	0.46	52.16	-0.04	-1.71	0.01	-0.00	-0.0	-0.02	0.01	-0.00	-0.00	0.00	0.00	0.00	0.0	4
3848	3848:Arm1	Origin	0.46	52.16	-0.04	-1.71	0.77	0.00	0.0	-0.68	1.70	0.01	-0.03	0.00	0.17	0.00	0.30	5
3848	3848:TopConn	End	3.00	49.29	-0.04	-1.58	5.08	0.02	0.0	-0.68	1.70	0.01	-0.03	0.56	0.00	0.00	0.59	2
3848	3848:TopConn	Origin	3.00	49.29	-0.04	-1.58	5.08	0.02	0.0	2.66	39.95	0.01	0.13	0.00	3.91	0.00	6.77	5
3848	3848:Arm2	End	7.10	44.67	-0.03	-1.36	169.05	0.04	0.0	2.66	39.95	0.01	0.12	16.31	0.00	0.00	16.43	2
3848	3848:Arm2	Origin	7.10	44.67	-0.03	-1.36	172.32	0.05	0.0	-0.25	44.52	0.02	-0.01	16.63	0.00	0.00	16.64	2
3848	3848:WVGD1	End	11.00	40.40	-0.03	-1.16	345.76	0.12	0.0	-0.25	44.52	0.02	-0.01	29.63	0.00	0.00	29.64	2
3848	3848:WVGD1	Origin	11.00	40.40	-0.03	-1.16	345.76	0.12	0.0	-0.82	45.19	0.02	-0.03	29.63	0.00	0.00	29.66	2
3848	3848:BCT	End	12.50	38.80	-0.03	-1.09	413.54	0.14	0.0	-0.82	45.19	0.02	-0.03	33.92	0.00	0.00	33.95	2
3848	3848:BCT	Origin	12.50	38.80	-0.03	-1.09	413.54	0.14	0.0	6.44	128.89	0.02	0.27	33.92	0.00	0.00	34.18	2
3848	3848:BotConn	End	13.00	38.27	-0.03	-1.07	477.99	0.15	0.0	6.44	128.89	0.02	0.27	38.64	0.00	0.00	38.90	2
3848	3848:BotConn	Origin	13.00	38.27	-0.03	-1.07	477.99	0.15	0.0	-3.81	103.85	0.02	-0.16	38.64	0.00	0.00	38.80	2
3848	3848:BCB	End	13.50	37.75	-0.03	-1.05	529.91	0.16	0.0	-3.81	103.85	0.02	-0.16	42.23	0.00	0.00	42.38	2
3848	3848:BCB	Origin	13.50	37.75	-0.03	-1.05	529.91	0.16	0.0	-11.36	20.32	0.02	-0.46	42.23	0.00	0.00	42.69	2
3848	Tube 1	End	16.93	34.28	-0.03	-0.90	599.55	0.21	0.0	-11.36	20.32	0.02	-0.44	43.43	0.00	0.00	43.87	2
3848	Tube 1	Origin	16.93	34.28	-0.03	-0.90	599.55	0.21	0.0	-11.76	20.51	0.02	-0.46	43.43	0.00	0.00	43.88	2
3848	3848:Arm3	End	20.35	31.02	-0.02	-0.77	669.85	0.27	0.0	-11.76	20.51	0.02	-0.44	44.29	0.00	0.00	44.73	2
3848	3848:Arm3	Origin	20.35	31.02	-0.02	-0.77	673.09	0.28	0.0	-14.59	24.90	0.03	-0.54	44.51	0.00	0.00	45.05	2
3848	3848:WVGD2	End	21.00	30.42	-0.02	-0.75	689.17	0.30	0.0	-14.59	24.90	0.03	-0.54	44.81	0.00	0.00	45.35	2
3848	3848:WVGD2	Origin	21.00	30.42	-0.02	-0.75	689.17	0.30	0.0	-15.19	25.56	0.03	-0.56	44.81	0.00	0.00	45.37	2
3848	Tube 1	End	26.00	26.05	-0.02	-0.59	816.94	0.43	0.0	-15.19	25.56	0.03	-0.53	46.88	0.00	0.00	47.41	2
3848	Tube 1	Origin	26.00	26.05	-0.02	-0.59	816.94	0.43	0.0	-15.85	25.85	0.03	-0.55	46.88	0.00	0.00	47.43	2
3848	3848:WVGD3	End	31.00	22.07	-0.02	-0.45	946.18	0.57	0.0	-15.85	25.85	0.03	-0.52	48.28	0.00	0.00	48.80	2
3848	3848:WVGD3	Origin	31.00	22.07	-0.02	-0.45	946.18	0.57	0.0	-16.62	26.57	0.03	-0.54	48.28	0.00	0.00	48.82	2
3848	3848:Arm4	End	33.60	20.15	-0.02	-0.39	1015.37	0.63	0.0	-16.62	26.57	0.03	-0.53	48.86	0.00	0.00	49.39	2
3848	3848:Arm4	Origin	33.60	20.15	-0.02	-0.39	1018.58	0.64	0.0	-19.74	30.99	0.04	-0.63	49.01	0.00	0.00	49.64	2
3848	Tube 1	End	37.30	17.61	-0.01	-0.32	1133.19	0.78	0.0	-19.74	30.99	0.04	-0.60	50.32	0.00	0.00	50.92	2
3848	Tube 1	Origin	37.30	17.61	-0.01	-0.32	1133.19	0.78	0.0	-20.29	31.23	0.04	-0.62	50.32	0.00	0.00	50.94	2
3848	3848:WVGD4	End	41.00	15.28	-0.01	-0.26	1248.66	0.91	0.0	-20.29	31.23	0.04	-0.60	51.32	0.00	0.00	51.92	2
3848	3848:WVGD4	Origin	41.00	15.28	-0.01	-0.26	1248.66	0.91	0.0	-20.98	31.90	0.04	-0.62	51.32	0.00	0.00	51.94	2
3848	SpliceT	End	42.92	14.15	-0.01	-0.23	1309.81	0.98	0.0	-20.98	31.90	0.04	-0.61	51.78	0.00	0.00	52.39	2
3848	SpliceT	Origin	42.92	14.15	-0.01	-0.23	1309.81	0.98	0.0	-21.82	32.14	0.03	-0.63	51.78	0.00	0.00	52.41	2
3848	SpliceB	End	47.50	11.66	-0.01	-0.17	1457.10	1.14	0.0	-21.82	32.14	0.03	-0.51	45.67	0.00	0.00	46.19	2
3848	SpliceB	Origin	47.50	11.66	-0.01	-0.17	1457.10	1.14	0.0	-22.82	32.44	0.03	-0.53	45.67	0.00	0.00	46.21	2
3848	3848:WVGD5	End	51.00	9.94	-0.01	-0.14	1570.63	1.26	0.0	-22.82	32.44	0.03	-0.52	46.02	0.00	0.00	46.54	2
3848	3848:WVGD5	Origin	51.00	9.94	-0.01	-0.14	1570.63	1.26	0.0	-23.87	33.25	0.03	-0.54	46.02	0.00	0.00	46.56	2
3848	Tube 2	End	56.00	7.75	-0.01	-0.09	1736.86	1.42	0.0	-23.87	33.25	0.03	-0.52	46.39	0.00	0.00	46.91	2
3848	Tube 2	Origin	56.00	7.75	-0.01	-0.09	1736.86	1.42	0.0	-24.82	33.64	0.03	-0.54	46.39	0.00	0.00	46.93	2
3848	3848:WVGD6	End	61.00	5.86	-0.00	-0.06	1905.04	1.57	0.0	-24.82	33.64	0.03	-0.51	46.57	0.00	0.00	47.08	2
3848	3848:WVGD6	Origin	61.00	5.86	-0.00	-0.06	1905.04	1.57	0.0	-25.84	34.43	0.03	-0.53	46.57	0.00	0.00	47.11	2
3848	Tube 2	End	63.71	4.95	-0.00	-0.05	1998.31	1.64	0.0	-25.84	34.43	0.03	-0.52	46.64	0.00	0.00	47.16	2
3848	Tube 2	Origin	63.71	4.95	-0.00	-0.05	1998.31	1.64	0.0	-26.38	34.66	0.03	-0.53	46.64	0.00	0.00	47.17	2
3848	SpliceT	End	66.42	4.13	-0.00	-0.04	2092.19	1.72	0.0	-26.38	34.66	0.03	-0.52	46.67	0.00	0.00	47.19	2
3848	SpliceT	Origin	66.42	4.13	-0.00	-0.04	2092.19	1.72	0.0	-27.13	34.98	0.03	-0.54	46.67	0.00	0.00	47.20	2
3848	3848:WVGD7	End	71.00	2.92	-0.00	-0.02	2252.49	1.84	0.0	-27.13	34.98	0.03	-0.52	46.64	0.00	0.00	47.16	2
3848	3848:WVGD7	Origin	71.00	2.92	-0.00	-0.02	2252.49	1.84	0.0	-28.41	35.89	0.02	-0.54	46.64	0.00	0.00	47.18	2
3848	Tube 3	End	76.00	1.85	-0.00	-0.01	2431.94	1.96	0.0	-28.41	35.89	0.02	-0.52	46.57	0.00	0.00	47.09	2
3848	Tube 3	Origin	76.00	1.85	-0.00	-0.01	2431.94	1.96	0.0	-29.49	36.36	0.02	-0.54	46.57	0.00	0.00	47.11	2
3848	3848:WVGD8	End	81.00	1.03	-0.00	-0.01	2613.71	2.08	0.0	-29.49	36.36	0.02	-0.52	46.42	0.00	0.00	46.94	2
3848	3848:WVGD8	Origin	81.00	1.03	-0.00	-0.01	2613.71	2.08	0.0	-30.88	37.32	0.02	-0.55	46.42	0.00	0.00	46.97	2
3848	Tube 3	End	86.00	0.45	-0.00	-0.00	2800.30	2.18	0.0	-30.88	37.32	0.02	-0.53	46.26	0.00	0.00	46.79	2
3848	Tube 3	Origin	86.00	0.45	-0.00	-0.00	2800.30	2.18	0.0	-32.02	37.82	0.02	-0.55	46.26	0.00	0.00	46.81	2
3848	3848:WVGD9	End	91.00	0.11	-0.00	-0.00	2989.39	2.27	0.0	-32.02	37.82	0.02	-0.53	46.05	0.00	0.00	46.57	2
3848	3848:WVGD9	Origin	91.00	0.11	-0.00	-0.00	2989.39	2.27	0.0	-33.48	38.82	0.02	-0.55	46.05	0.00	0.00	46.60	2
3848	3848:g	End	96.00	0.00	0.00	0.00	3183.47	2.35	0.0	-33.48	38.82	0.02	-0.53	45.83	0.00	0.00	46.36	2

Detailed Tubular Davit Arm Usages for Load Case "NESC Extreme Wind":

Element Label	Joint Label	Joint Position	Rel. Dist. (ft)	Trans. Defl. (in)	Long. Defl. (in)	Vert. Defl. (in)	Vert. Mom. (ft-k)	Horz. Mom. (ft-k)	Tors. Mom. (ft-k)	Axial Force (kips)	Vert. Shear (kips)	Horz. Shear (kips)	P/A (ksi)	M/S. (ksi)	V/Q. (ksi)	T/R. (ksi)	Res. (ksi)	Max. Usage %	At Pt.
Davit1	Davit1:O	Origin	0.00	52.11	-0.04	-2.66	-1.37	0.01	-0.0	0.67	0.34	-0.00	0.16	2.40	0.00	0.00	2.56	3.9	1
Davit1	Davit1:t	End	4.03	52.46	-0.04	-7.23	0.00	-0.00	-0.0	0.67	0.34	-0.00	0.22	0.00	0.24	0.00	0.47	0.7	3
Davit2	Davit2:O	Origin	0.00	52.20	-0.04	-0.76	-0.60	-0.01	-0.0	-0.91	0.15	0.00	-0.22	1.06	0.00	0.00	1.27	2.0	1
Davit2	Davit2:t	End	4.03	52.98	-0.04	3.72	-0.00	-0.00	-0.0	-0.91	0.15	0.00	-0.31	0.00	0.10	0.00	0.35	0.5	3
Davit3	Davit3:O	Origin	0.00	44.62	-0.03	-2.40	-11.61	0.06	0.0	2.04	1.36	-0.01	0.17	3.22	0.00	0.00	3.39	5.2	1
Davit3	#Davit3:O	End	4.52	44.81	-0.03	-7.45	-5.49	0.03	0.0	2.04	1.36	-0.01	0.22	2.75	0.00	0.00	2.97	4.6	1
Davit3	#Davit3:O	Origin	4.52	44.81	-0.03	-7.45	-5.49	0.03	-0.0	2.04	1.21	-0.01	0.22	2.75	0.00	0.00	2.97	4.6	1
Davit3	Davit3:t	End	9.03	44.99	-0.03	-12.54	0.00	-0.00	-0.0	2.04	1.21	-0.01	0.34	0.00	0.42	0.00	0.80	1.2	3
Davit4	Davit4:O	Origin	0.00	44.72	-0.03	-0.32	-8.35	-0.05	-0.0	-2.24	0.99	0.01	-0.18	2.32	0.00	0.00	2.50	3.8	1
Davit4	#Davit4:O	End	4.52	45.36	-0.03	4.65	-3.88	-0.03	-0.0	-2.24	0.99	0.01	-0.25	1.94	0.00	0.00	2.19	3.4	1
Davit4	#Davit4:O	Origin	4.52	45.36	-0.03	4.65	-3.88	-0.03	-0.0	-2.21	0.86	0.01	-0.24	1.94	0.00	0.00	2.19	3.4	1
Davit4	Davit4:t	End	9.03	46.01	-0.03	9.59	-0.00	-0.00	-0.0	-2.21	0.86	0.01	-0.37	0.00	0.30	0.00	0.63	1.0	3
Davit5	Davit5:O	Origin	0.00	30.98	-0.02	-1.81	-11.90	0.06	0.0	2.02	1.39	-0.01	0.17	3.30	0.00	0.00	3.46	5.3	1
Davit5	#Davit5:O	End	4.52	31.16	-0.02	-6.01	-5.63	0.03	0.0	2.02	1.39	-0.01	0.22	2.82	0.00	0.00	3.04	4.7	1
Davit5	#Davit5:O	Origin	4.52	31.16	-0.02	-6.01	-5.63	0.03	-0.0	2.02	1.25	-0.01	0.22	2.82	0.00	0.00	3.04	4.7	1
Davit5	Davit5:t	End	9.03	31.35	-0.02	-10.25	0.00	-0.00	-0.0	2.02	1.25	-0.01	0.34	0.00	0.43	0.00	0.82	1.3	3
Davit6	Davit6:O	Origin	0.00	31.06	-0.02	0.27	-8.67	-0.05	-0.0	-2.22	1.03	0.01	-0.18	2.40	0.00	0.00	2.59	4.0	1
Davit6	#Davit6:O	End	4.52	31.56	-0.02	4.40	-4.04	-0.03	-0.0	-2.22	1.03	0.01	-0.24	2.02	0.00	0.00	2.27	3.5	1
Davit6	#Davit6:O	Origin	4.52	31.56	-0.02	4.40	-4.04	-0.03	-0.0	-2.20	0.89	0.01	-0.24	2.02	0.00	0.00	2.26	3.5	1
Davit6	Davit6:t	End	9.03	32.06	-0.03	8.50	-0.00	-0.00	-0.0	-2.20	0.89	0.01	-0.37	0.00	0.31	0.00	0.65	1.0	3
Davit7	Davit7:O	Origin	0.00	20.13	-0.02	-1.33	-12.22	0.06	0.0	2.00	1.42	-0.01	0.16	3.39	0.00	0.00	3.55	5.5	1
Davit7	#Davit7:O	End	4.52	20.30	-0.02	-4.58	-5.79	0.03	0.0	2.00	1.42	-0.01	0.22	2.90	0.00	0.00	3.12	4.8	1
Davit7	#Davit7:O	Origin	4.52	20.30	-0.02	-4.58	-5.79	0.03	-0.0	2.00	1.28	-0.01	0.22	2.90	0.00	0.00	3.12	4.8	1
Davit7	Davit7:t	End	9.03	20.47	-0.02	-7.87	0.00	-0.00	-0.0	2.00	1.28	-0.01	0.33	0.00	0.44	0.00	0.84	1.3	3
Davit8	Davit8:O	Origin	0.00	20.18	-0.02	0.55	-9.03	-0.05	-0.0	-2.20	1.07	0.01	-0.18	2.50	0.00	0.00	2.68	4.1	1
Davit8	#Davit8:O	End	4.52	20.54	-0.02	3.73	-4.21	-0.03	-0.0	-2.20	1.07	0.01	-0.24	2.11	0.00	0.00	2.35	3.6	1
Davit8	#Davit8:O	Origin	4.52	20.54	-0.02	3.73	-4.21	-0.03	-0.0	-2.18	0.93	0.01	-0.24	2.11	0.00	0.00	2.35	3.6	1
Davit8	Davit8:t	End	9.03	20.90	-0.02	6.89	-0.00	-0.00	-0.0	-2.18	0.93	0.01	-0.36	0.00	0.32	0.00	0.67	1.0	3

Summary of Clamp Capacities and Usages for Load Case "NESC Extreme Wind":

Clamp Label	Force (kips)	Input Holding Capacity (kips)	Factored Holding Capacity (kips)	Holding Usage %	Input Hardware Capacity (kips)	Factored Hardware Capacity (kips)	Hardware Usage %	Max. Usage %
Clamp1	0.740	100.00	100.00	0.74	0.00	0.00	0.00	0.74
Clamp2	0.916	100.00	100.00	0.92	0.00	0.00	0.00	0.92
Clamp3	2.348	100.00	100.00	2.35	0.00	0.00	0.00	2.35
Clamp4	2.343	100.00	100.00	2.34	0.00	0.00	0.00	2.34
Clamp5	2.347	100.00	100.00	2.35	0.00	0.00	0.00	2.35
Clamp6	2.344	100.00	100.00	2.34	0.00	0.00	0.00	2.34
Clamp7	2.347	100.00	100.00	2.35	0.00	0.00	0.00	2.35
Clamp8	2.344	100.00	100.00	2.34	0.00	0.00	0.00	2.34
Clamp9	0.554	100.00	100.00	0.55	0.00	0.00	0.00	0.55

Clamp10	0.554	100.00	100.00	0.55	0.00	0.00	0.00	0.55
Clamp11	0.554	100.00	100.00	0.55	0.00	0.00	0.00	0.55
Clamp12	0.554	100.00	100.00	0.55	0.00	0.00	0.00	0.55
Clamp13	0.554	100.00	100.00	0.55	0.00	0.00	0.00	0.55
Clamp14	0.554	100.00	100.00	0.55	0.00	0.00	0.00	0.55
Clamp15	0.554	100.00	100.00	0.55	0.00	0.00	0.00	0.55
Clamp16	0.554	100.00	100.00	0.55	0.00	0.00	0.00	0.55
Clamp17	0.554	100.00	100.00	0.55	0.00	0.00	0.00	0.55
Clamp18	38.226	100.00	100.00	38.23	0.00	0.00	0.00	38.23
Clamp19	27.040	100.00	100.00	27.04	0.00	0.00	0.00	27.04
Clamp20	83.953	100.00	100.00	83.95	0.00	0.00	0.00	83.95
Clamp21	83.953	100.00	100.00	83.95	0.00	0.00	0.00	83.95

\*\*\* Overall summary for all load cases - Usage = Maximum Stress / Allowable Stress

Summary of Steel Pole Usages:

Steel Pole Label	Maximum Usage %	Load Case	Height AGL (ft)	Segment Number	Weight (lbs)
3848	80.63	NESC Extreme Wind	50.8	17	16194.8

Base Plate Results by Bend Line:

Pole Label	Load Case	Bend Line #	Start X (ft)	Start Y (ft)	End X (ft)	End Y (ft)	Length (in)	Bending Stress (ksi)	Mom. (ft-k)	Bolt #	Bolts Acting	Bolt Max Load (kips)	Min Plate Thickness (in)	Actual Thickness (in)	Usage %	
3848	NESC Heavy Wind	1	-0.586	2.189	-1.602	1.602	14.076	31.512	46.588	-2	98.231	1.993	2.750	52.52	Note: actual load overridden by one half of pole moment capacity at the base as per ASCE/SEI 48-19 6.4.2	
3848	NESC Heavy Wind	2	-1.602	1.602	-2.189	0.586	14.076	21.042	31.109	-2	71.358	1.629	2.750	35.07	Note: actual load overridden by one half of pole moment capacity at the base as per ASCE/SEI 48-19 6.4.2	
3848	NESC Heavy Wind	3	-2.189	-0.586	-1.602	-1.602	14.076	18.087	26.741	-2	-62.706	1.510	2.750	30.15	Note: actual load overridden by one half of pole moment capacity at the base as per ASCE/SEI 48-19 6.4.2	
3848	NESC Heavy Wind	4	-1.602	-1.602	-0.586	-2.189	14.076	28.629	42.326	-2	-89.808	1.900	2.750	47.71	Note: actual load overridden by one half of pole moment capacity at the base as per ASCE/SEI 48-19 6.4.2	
3848	NESC Heavy Wind	5	0.586	-2.189	1.602	-1.602	14.076	28.763	42.524	-2	-90.144	1.904	2.750	47.94	Note: actual load overridden by one half of pole moment capacity at the base as per ASCE/SEI 48-19 6.4.2	
3848	NESC Heavy Wind	6	1.602	-1.602	2.189	-0.586	14.076	18.293	27.045	-2	-63.271	1.518	2.750	30.49	Note: actual load overridden by one half of pole moment capacity at the base as per ASCE/SEI 48-19 6.4.2	
3848	NESC Heavy Wind	7	2.189	0.586	1.602	1.602	14.076	20.836	30.804	-2	70.794	1.621	2.750	34.73	Note: actual load overridden by one half of pole moment capacity at the base as per ASCE/SEI 48-19 6.4.2	
3848	NESC Heavy Wind	8	1.602	1.602	0.586	2.189	14.076	31.378	46.390	-2	97.895	1.989	2.750	52.30	Note: actual load overridden by one half of pole moment capacity at the base as per ASCE/SEI 48-19 6.4.2	
3848	NESC Extreme Wind	1	-0.586	2.189	-1.602	1.602	14.076	46.607	68.905	2	145.604	2.424	2.750	77.68		
3848	NESC Extreme Wind	2	-1.602	1.602	-2.189	0.586	14.076	30.557	45.177	2	104.368	1.963	2.750	50.93		
3848	NESC Extreme Wind	3	-2.189	-0.586	-1.602	-1.602	14.076	29.177	43.136	2	-100.294	1.918	2.750	48.63		
3848	NESC Extreme Wind	4	-1.602	-1.602	-0.586	-2.189	14.076	45.203	66.830	2	-141.455	2.387	2.750	75.34		
3848	NESC Extreme Wind	5	0.586	-2.189	1.602	-1.602	14.076	45.159	66.765	2	-141.344	2.386	2.750	75.26		
3848	NESC Extreme Wind	6	1.602	-1.602	2.189	-0.586	14.076	29.109	43.036	2	-100.108	1.915	2.750	48.52		
3848	NESC Extreme Wind	7	2.189	0.586	1.602	1.602	14.076	30.625	45.277	2	104.554	1.965	2.750	51.04		
3848	NESC Extreme Wind	8	1.602	1.602	0.586	2.189	14.076	46.651	68.970	2	145.714	2.425	2.750	77.75		

Summary of Tubular Davit Usages:

Tubular Davit Label	Maximum Usage %	Load Case	Height AGL (ft)	Segment Number	Weight (lbs)
Davit1	9.76	NESC Heavy Wind	95.8	1	49.5
Davit2	8.90	NESC Heavy Wind	95.8	1	49.5
Davit3	10.19	NESC Heavy Wind	89.1	1	280.0
Davit4	9.49	NESC Heavy Wind	89.1	1	280.0
Davit5	10.21	NESC Heavy Wind	75.8	1	280.0
Davit6	9.52	NESC Heavy Wind	75.8	1	280.0
Davit7	10.23	NESC Heavy Wind	62.6	1	280.0
Davit8	9.55	NESC Heavy Wind	62.6	1	280.0

\*\*\* Maximum Stress Summary for Each Load Case

Summary of Maximum Usages by Load Case:

Load Case	Maximum Usage %	Element Label	Element Type
NESC Heavy Wind	52.52	3848 Base Plate	
NESC Extreme Wind	80.63	3848 Steel Pole	

Summary of Steel Pole Usages by Load Case:

Load Case	Maximum Usage %	Steel Pole Label	Height AGL (ft)	Segment Number
NESC Heavy Wind	33.59	3848	2.5	28
NESC Extreme Wind	80.63	3848	50.8	17

Summary of Base Plate Usages by Load Case:

Load Case	Pole Label	Bend Line #	Length (in)	Vertical Load (kips)	X Moment (ft-k)	Y Bending Moment (ft-k)	Stress (ksi)	Bolt Moment Sum (ft-k)	# Bolts Acting On Bend Line	Max Bolt Load For Bend Line (kips)	Minimum Plate Thickness (in)	Usage %
NESC Heavy Wind	3848	1	14.076	64.700	2085.347	-7.146	31.512	46.588	-2	98.231	1.993	52.52
NESC Extreme Wind	3848	8	14.076	34.080	3183.472	2.351	46.651	68.970	2	145.714	2.425	77.75

Summary of Tubular Davit Usages by Load Case:

Load Case	Maximum Usage %	Tubular Davit Label	Height AGL (ft)	Segment Number
NESC Heavy Wind	10.23	Davit7	62.6	1
NESC Extreme Wind	5.46	Davit7	62.6	1

Summary of Insulator Usages:

Insulator Label	Insulator Type	Maximum Usage %	Load Case	Weight (lbs)
Clamp1	Clamp	1.00	NESC Heavy Wind	0.0
Clamp2	Clamp	1.09	NESC Heavy Wind	0.0
Clamp3	Clamp	2.60	NESC Heavy Wind	0.0
Clamp4	Clamp	2.59	NESC Heavy Wind	0.0
Clamp5	Clamp	2.60	NESC Heavy Wind	0.0
Clamp6	Clamp	2.59	NESC Heavy Wind	0.0
Clamp7	Clamp	2.60	NESC Heavy Wind	0.0
Clamp8	Clamp	2.59	NESC Heavy Wind	0.0
Clamp9	Clamp	1.05	NESC Heavy Wind	0.0
Clamp10	Clamp	1.05	NESC Heavy Wind	0.0
Clamp11	Clamp	1.05	NESC Heavy Wind	0.0
Clamp12	Clamp	1.05	NESC Heavy Wind	0.0
Clamp13	Clamp	1.05	NESC Heavy Wind	0.0
Clamp14	Clamp	1.05	NESC Heavy Wind	0.0
Clamp15	Clamp	1.05	NESC Heavy Wind	0.0
Clamp16	Clamp	1.05	NESC Heavy Wind	0.0
Clamp17	Clamp	1.05	NESC Heavy Wind	0.0



Clamp18	Clamp	38.23	NESC Extreme Wind	0.0
Clamp19	Clamp	27.04	NESC Extreme Wind	0.0
Clamp20	Clamp	83.95	NESC Extreme Wind	0.0
Clamp21	Clamp	83.95	NESC Extreme Wind	0.0

**Loads At Insulator Attachments For All Load Cases:**

Load Case	Insulator Label	Insulator Type	Structure Attach Label	Structure Attach Load X (kips)	Structure Attach Load Y (kips)	Structure Attach Load Z (kips)	Structure Attach Load Res. (kips)
NESC Heavy Wind	Clamp1	Clamp	Davit1:t	-0.001	0.603	0.795	0.998
NESC Heavy Wind	Clamp2	Clamp	Davit2:t	-0.001	0.659	0.872	1.093
NESC Heavy Wind	Clamp3	Clamp	Davit3:t	-0.001	1.055	2.377	2.601
NESC Heavy Wind	Clamp4	Clamp	Davit4:t	-0.001	1.055	2.369	2.593
NESC Heavy Wind	Clamp5	Clamp	Davit5:t	-0.001	1.054	2.377	2.600
NESC Heavy Wind	Clamp6	Clamp	Davit6:t	-0.001	1.056	2.369	2.594
NESC Heavy Wind	Clamp7	Clamp	Davit7:t	-0.001	1.053	2.377	2.600
NESC Heavy Wind	Clamp8	Clamp	Davit8:t	-0.001	1.057	2.369	2.594
NESC Heavy Wind	Clamp9	Clamp	3848:WVGD1	0.000	0.172	1.035	1.049
NESC Heavy Wind	Clamp10	Clamp	3848:WVGD2	0.000	0.172	1.035	1.049
NESC Heavy Wind	Clamp11	Clamp	3848:WVGD3	0.000	0.172	1.035	1.049
NESC Heavy Wind	Clamp12	Clamp	3848:WVGD4	0.000	0.172	1.035	1.049
NESC Heavy Wind	Clamp13	Clamp	3848:WVGD5	0.000	0.172	1.035	1.049
NESC Heavy Wind	Clamp14	Clamp	3848:WVGD6	0.000	0.172	1.035	1.049
NESC Heavy Wind	Clamp15	Clamp	3848:WVGD7	0.000	0.172	1.035	1.049
NESC Heavy Wind	Clamp16	Clamp	3848:WVGD8	0.000	0.172	1.035	1.049
NESC Heavy Wind	Clamp17	Clamp	3848:WVGD9	0.000	0.172	1.035	1.049
NESC Heavy Wind	Clamp18	Clamp	3848:TopConn	0.000	10.202	0.000	10.202
NESC Heavy Wind	Clamp19	Clamp	3848:BotConn	0.000	-6.739	15.906	17.275
NESC Heavy Wind	Clamp20	Clamp	3848:BCT	0.000	21.987	0.000	21.987
NESC Heavy Wind	Clamp21	Clamp	3848:BCB	0.000	-21.987	0.000	21.987
NESC Extreme Wind	Clamp1	Clamp	Davit1:t	-0.003	0.678	0.296	0.740
NESC Extreme Wind	Clamp2	Clamp	Davit2:t	-0.003	0.858	0.321	0.916
NESC Extreme Wind	Clamp3	Clamp	Davit3:t	-0.007	2.030	1.179	2.348
NESC Extreme Wind	Clamp4	Clamp	Davit4:t	-0.006	2.027	1.175	2.343
NESC Extreme Wind	Clamp5	Clamp	Davit5:t	-0.007	2.029	1.179	2.347
NESC Extreme Wind	Clamp6	Clamp	Davit6:t	-0.006	2.028	1.175	2.344
NESC Extreme Wind	Clamp7	Clamp	Davit7:t	-0.007	2.029	1.179	2.347
NESC Extreme Wind	Clamp8	Clamp	Davit8:t	-0.006	2.028	1.175	2.344
NESC Extreme Wind	Clamp9	Clamp	3848:WVGD1	0.000	0.477	0.281	0.554
NESC Extreme Wind	Clamp10	Clamp	3848:WVGD2	0.000	0.477	0.281	0.554
NESC Extreme Wind	Clamp11	Clamp	3848:WVGD3	0.000	0.477	0.281	0.554
NESC Extreme Wind	Clamp12	Clamp	3848:WVGD4	0.000	0.477	0.281	0.554
NESC Extreme Wind	Clamp13	Clamp	3848:WVGD5	0.000	0.477	0.281	0.554
NESC Extreme Wind	Clamp14	Clamp	3848:WVGD6	0.000	0.477	0.281	0.554
NESC Extreme Wind	Clamp15	Clamp	3848:WVGD7	0.000	0.477	0.281	0.554
NESC Extreme Wind	Clamp16	Clamp	3848:WVGD8	0.000	0.477	0.281	0.554
NESC Extreme Wind	Clamp17	Clamp	3848:WVGD9	0.000	0.477	0.281	0.554
NESC Extreme Wind	Clamp18	Clamp	3848:TopConn	0.000	38.226	0.000	38.226
NESC Extreme Wind	Clamp19	Clamp	3848:BotConn	0.000	-25.861	7.896	27.040
NESC Extreme Wind	Clamp20	Clamp	3848:BCT	0.000	83.953	0.000	83.953
NESC Extreme Wind	Clamp21	Clamp	3848:BCB	0.000	-83.953	0.000	83.953

**Overturning Moments For User Input Concentrated Loads:**

Moments are static equivalents based on central axis of 0,0 (i.e. a single pole).

Load Case	Total Tran. Load (kips)	Total Long. Load (kips)	Total Vert. Load (kips)	Transverse Overturning Moment (ft-k)	Longitudinal Overturning Moment (ft-k)	Torsional Moment (ft-k)
NESC Heavy Wind	12.603	-0.008	41.126	1085.758	0.650	0.000
NESC Extreme Wind	30.365	-0.045	18.104	2763.030	3.556	-0.030

\*\*\* Weight of structure (lbs):  
Weight of Tubular Davit Arms: 1779.3  
Weight of Steel Poles: 16194.8  
Total: 17974.1

\*\*\* End of Report

**Anchor Bolt Analysis:**

**Input Data:**

Bolt Force:

Maximum Tensile Force =	$T_{Max} := 146 \cdot kips$	(User Input from PLS-Pole)
Maximum Shear Force at Base =	$V_{base} := 39 \cdot kips$	(User Input from PLS-Pole)

Anchor Bolt Data:

Use ASTM A615 Grade 75

Number of Anchor Bolts =  $N := 16$  (User Input)

Bolt "Column" Distance =  $l := 3.0 \cdot in$  (User Input)

Bolt Ultimate Strength =  $F_u := 100 \cdot ksi$  (User Input)

Bolt Yield Strength =  $F_y := 75 \cdot ksi$  (User Input)

Bolt Modulus =  $E := 29000 \cdot ksi$  (User Input)

Diameter of Anchor Bolts =  $D := 2.25 \cdot in$  (User Input)

Threads per Inch =  $n := 4.5$  (User Input)

**Anchor Bolt Analysis:**

Stress Area of Bolt =  $A_s := \frac{\pi}{4} \cdot \left( D - \frac{0.9743 \cdot in}{n} \right)^2 = 3.248 \text{ in}^2$

Maximum Shear Force per Bolt =  $V_{Max} := \frac{V_{base}}{N} = 2.4 \text{ kips}$

Shear Stress per Bolt =  $f_v := \frac{V_{Max}}{A_s} = 750.5 \text{ psi}$

Tensile Stress Permitted =  $F_t := 0.75 \cdot F_u = 75 \text{ ksi}$

Shear Stress Permitted =  $F_v := 0.35 \cdot F_u = 35 \text{ ksi}$

Permitted Axial Tensile Stress in Conjunction with Shear =  $F_{tv} := F_t \cdot \sqrt{1 - \left( \frac{f_v}{F_v} \right)^2} = 74.98 \text{ ksi}$

Bolt Tension % of Capacity =  $\frac{T_{Max}}{F_{tv} \cdot A_s} = 60\%$

Condition1 =  $Condition1 := \text{if} \left( \frac{T_{Max}}{F_{tv} \cdot A_s} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$

Condition1 = "OK"

**Caisson Foundation:**

Input Data:

Shear Force =	$S := 39.1 \cdot k \cdot 1.1 = 43 \text{ kips}$	USER INPUT-FROM PLS-Pole
Overturning Moment =	$M := 3183.5 \cdot \text{ft} \cdot k \cdot 1.1 = 3502 \text{ ft} \cdot k$	USER INPUT-FROM PLS-Pole
Applied Axial Load =	$A1 := 36.4 \cdot k \cdot 1.1 = 40 \text{ kips}$	USER INPUT-FROM PLS-Pole
Bending Moment =	$M_u := 3675 \cdot \text{ft} \cdot k$	USER INPUT-FROM LPILE
Moment Capacity =	$M_n := 7416 \cdot \text{ft} \cdot k$	USER INPUT-FROM LPILE
Max Shear =	$V_u := 770945 \cdot \text{lb}$	USER INPUT-FROM LPILE
Foundation Diameter =	$d := 7 \cdot \text{ft}$	USER INPUT
Overall Length of Caisson =	$L_c := 18 \cdot \text{ft}$	USER INPUT
Depth From Top of Caisson to Grade =	$L_{\text{pag}} := 1.0 \cdot \text{ft}$	USER INPUT
Number of Rebar =	$n := 27$	USER INPUT
Area of Rebar =	$A_r := 1.56 \cdot \text{in}^2$	USER INPUT
Rebar Yield Strength =	$f_y := 60 \cdot \text{ksi}$	USER INPUT
Concrete Comp Strength =	$f'_c := 3000 \text{ psi}$	USER INPUT
Area of Shear Reinforcement =	$A_v := 1.2 \cdot \text{in}^2$	USER INPUT = (2)*(Area of #7) per 11.4.7.3
Spacing of Shear Reinforcement =	$s := 8 \cdot \text{in}$	

Check Moment Capacity:

Factor of Safety =	$FS := \frac{0.9 \cdot M_n}{M_u} = 1.82$	
Factor of Safety Required =	$FS_{\text{reqd}} := 1.0$	
	$FOS_{\text{Check}} := \text{if } (FS \geq FS_{\text{reqd}}, \text{"OK"}, \text{"NO GOOD"})$	$\frac{FS_{\text{reqd}}}{FS} = 55.1\%$
	<b>FOSCheck = "OK"</b>	

Check Shear Capacity:

Shear Strength Reduction Factor =	$\phi := 0.75$	
Area of Concrete Pier =	$A_c := \frac{1}{4} \cdot \pi \cdot d^2 = 5542 \text{ in}^2$	
Nominal Shear Strength by Concrete =	$V_c := 2 \cdot \sqrt{f'_c} \cdot \text{psi} \cdot A_c = 607 \text{ kips}$	
Nominal Shear Strength by Steel =	$V_s := \frac{(A_v \cdot f_y \cdot 0.8 \cdot d)}{s} = 605 \text{ kips}$	
Design Shear Strength =	$\phi V_n := \phi \cdot (V_c + V_s) = 909 \text{ kips}$	
	$Shear_{\text{Check}} := \text{if } (\phi V_n \geq V_u, \text{"OK"}, \text{"NO GOOD"})$	$\frac{V_u}{\phi V_n} = 84.8\%$
	<b>ShearCheck = "OK"</b>	

=====  
LPIle for Windows, Version 2022-12.010

Analysis of Individual Piles and Drilled Shafts  
Subjected to Lateral Loading Using the p-y Method  
© 1985-2022 by Ensoft, Inc.  
All Rights Reserved

=====  
This copy of LPIle is being used by:

Centek  
Centek

Serial Number of Security Device: 156264506

This copy of LPIle is licensed for exclusive use by:

Natcomm, Inc., Branford, CT

Use of this software by employees of Natcomm, Inc.  
other than those of the office site in Branford, CT  
is a violation of the software license agreement.

-----  
Files Used for Analysis  
-----

Path to file locations:

\Jobs\2202100.WI\02\_CT2267\05\_Structural\Tower\Backup Documentation\Rev  
(2)\Calcs\MathCAD\Foundation\

Name of input data file:

Caisson Analysis.lp12d

Name of output report file:

Caisson Analysis.lp12o

Name of plot output file:

Caisson Analysis.lp12p

Name of runtime message file:

Caisson Analysis.lp12r  
-----

Date and Time of Analysis

Date: February 26, 2024

Time: 14:29:56

Problem Title

22021.02/ CT2267 - Orange/ Pole # 3848

Job Number:

Client:

Engineer:

Description:

Program Options and Settings

Computational Options:

- Conventional Analysis

Engineering Units Used for Data Input and Computations:

- US Customary System Units (pounds, feet, inches)

Analysis Control Options:

- Maximum number of iterations allowed = 250
- Deflection tolerance for convergence = 1.0000E-05 in
- Maximum allowable deflection = 100.0000 in
- Number of pile increments = 100

Loading Type and Number of Cycles of Loading:

- Static loading specified
- Use of p-y modification factors for p-y curves not selected
- Analysis uses layering correction (Method of Georgiadis)
- No distributed lateral loads are entered
- Loading by lateral soil movements acting on pile not selected
- Input of shear resistance at the pile tip not selected
- Input of moment resistance at the pile tip not selected
- Compute pile-head foundation stiffness matrix
- Push-over analysis of pile not selected
- Buckling analysis of pile not selected

Output Options:

- Output files use decimal points to denote decimal symbols.
- Report only summary tables of pile-head deflection, maximum bending moment, and maximum shear force in output report file.
- No p-y curves to be computed and reported for user-specified depths
- Print using wide report formats

-----  
 Pile Structural Properties and Geometry  
 -----

Number of pile sections defined = 1  
 Total length of pile = 18.000 ft  
 Depth of ground surface below top of pile = 1.0000 ft

Pile diameters used for p-y curve computations are defined using 2 points.

p-y curves are computed using pile diameter values interpolated with depth over the length of the pile. A summary of values of pile diameter vs. depth follows.

Point No.	Depth Below Pile Head feet	Pile Diameter inches
1	0.000	84.0000
2	18.000	84.0000

Input Structural Properties for Pile Sections:  
 -----

Pile Section No. 1:

Section 1 is a round drilled shaft, bored pile, or CIDH pile  
 Length of section = 18.000000 ft

Shaft Diameter = 84.000000 in

-----  
Control Data for Pile-head Stiffness Computations  
-----

Computation Method 0 - Use loads from Load Case 1

Number of K-matrix points to generate = 10  
Point distribution method = logarithmic distribution

-----  
Soil and Rock Layering Information  
-----

The soil profile is modelled using 2 layers

Layer 1 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 1.000000 ft  
Distance from top of pile to bottom of layer = 11.000000 ft  
Effective unit weight at top of layer = 100.224000 pcf  
Effective unit weight at bottom of layer = 100.224000 pcf  
Friction angle at top of layer = 30.000000 deg.  
Friction angle at bottom of layer = 30.000000 deg.  
Subgrade k at top of layer = 90.000000 pci  
Subgrade k at bottom of layer = 90.000000 pci

Layer 2 is weak rock, p-y criteria by Reese, 1997

Distance from top of pile to top of layer = 11.000000 ft  
Distance from top of pile to bottom of layer = 18.000000 ft  
Effective unit weight at top of layer = 134.784000 pcf  
Effective unit weight at bottom of layer = 134.784000 pcf  
Uniaxial compressive strength at top of layer = 250.000000 psi  
Uniaxial compressive strength at bottom of layer = 250.000000 psi  
Initial modulus of rock at top of layer = 500000. psi  
Initial modulus of rock at bottom of layer = 500000. psi  
RQD of rock at top of layer = 50.000000 %  
RQD of rock at bottom of layer = 50.000000 %  
k<sub>rm</sub> of rock at top of layer = 0.0005000  
k<sub>rm</sub> of rock at bottom of layer = 0.0005000



(Depth of the lowest soil layer extends 0.000 ft below the pile tip)

-----  
 Summary of Input Soil Properties  
 -----

Layer Num. RQD %	Soil Type E50 Name or (p-y Curve Type) krm	Layer Rock Mass Depth ft	Effective Unit Wt. Modulus pcf psi	Angle of Friction deg.	Uniaxial qu psi
	kpy pci				
1	Sand	1.0000	100.2240	30.0000	--
--	--	90.0000	--	--	--
--	(Reese, et al.)	11.0000	100.2240	30.0000	--
--	--	90.0000	--	--	--
2	Weak	11.0000	134.7840	--	250.0000
50.0000	5.00E-04	--	500000.	--	--
50.0000	Rock	18.0000	134.7840	--	250.0000
	5.00E-04	--	500000.	--	--

-----  
 Static Loading Type  
 -----

Static loading criteria were used when computing p-y curves for all analyses.

-----  
 Pile-head Loading and Pile-head Fixity Conditions  
 -----

Number of loads specified = 1

Load Compute No.	Load Top y Type	Condition Run Analysis 1	Condition 2	Axial Thrust Force, lbs
1	1	V = 43000. lbs	M = 42024000. in-lbs	40000.
No		Yes		

V = shear force applied normal to pile axis

M = bending moment applied to pile head  
 y = lateral deflection normal to pile axis  
 S = pile slope relative to original pile batter angle  
 R = rotational stiffness applied to pile head  
 Values of top y vs. pile lengths can be computed only for load types with specified shear loading (Load Types 1, 2, and 3).  
 Thrust force is assumed to be acting axially for all pile batter angles.

-----  
 Computations of Nominal Moment Capacity and Nonlinear Bending Stiffness  
 -----

Axial thrust force values were determined from pile-head loading conditions

Number of Pile Sections Analyzed = 1

Pile Section No. 1:  
 -----

Dimensions and Properties of Drilled Shaft (Bored Pile):  
 -----

Length of Section	=	18.000000 ft
Shaft Diameter	=	84.000000 in
Concrete Cover Thickness (to edge of long. rebar)	=	2.295327 in
Number of Reinforcing Bars	=	27 bars
Yield Stress of Reinforcing Bars	=	60000. psi
Modulus of Elasticity of Reinforcing Bars	=	29000000. psi
Gross Area of Shaft	=	5542. sq. in.
Total Area of Reinforcing Steel	=	42.120000 sq. in.
Area Ratio of Steel Reinforcement	=	0.76 percent
Edge-to-Edge Bar Spacing	=	7.645171 in
Maximum Concrete Aggregate Size	=	0.750000 in
Ratio of Bar Spacing to Aggregate Size	=	10.19
Offset of Center of Rebar Cage from Center of Pile	=	0.0000 in

Axial Structural Capacities:  
 -----

Nom. Axial Structural Capacity = $0.85 F_c A_c + F_y A_s$	=	16551.306 kips
Tensile Load for Cracking of Concrete	=	-2132.635 kips
Nominal Axial Tensile Capacity	=	-2527.200 kips

Reinforcing Bar Dimensions and Positions Used in Computations:

Bar Number	Bar Diam. inches	Bar Area sq. in.	X inches	Y inches
---------------	---------------------	---------------------	-------------	-------------

1	1.410000	1.560000	38.999673	0.00000
2	1.410000	1.560000	37.948431	8.993943
3	1.410000	1.560000	34.851380	17.503021
4	1.410000	1.560000	29.875482	25.068506
5	1.410000	1.560000	23.288990	31.282542
6	1.410000	1.560000	15.446981	35.810128
7	1.410000	1.560000	6.772222	38.407180
8	1.410000	1.560000	-2.26763	38.933691
9	1.410000	1.560000	-11.18523	37.361277
10	1.410000	1.560000	-19.49984	33.774707
11	1.410000	1.560000	-26.76320	28.367334
12	1.410000	1.560000	-32.58375	21.430670
13	1.410000	1.560000	-36.64770	13.338674
14	1.410000	1.560000	-38.73597	4.527586
15	1.410000	1.560000	-38.73597	-4.52759
16	1.410000	1.560000	-36.64770	-13.33867
17	1.410000	1.560000	-32.58375	-21.43067
18	1.410000	1.560000	-26.76320	-28.36733
19	1.410000	1.560000	-19.49984	-33.77471
20	1.410000	1.560000	-11.18523	-37.36128
21	1.410000	1.560000	-2.26763	-38.93369
22	1.410000	1.560000	6.772222	-38.40718
23	1.410000	1.560000	15.446981	-35.81013
24	1.410000	1.560000	23.288990	-31.28254
25	1.410000	1.560000	29.875482	-25.06851
26	1.410000	1.560000	34.851380	-17.50302
27	1.410000	1.560000	37.948431	-8.99394

NOTE: The positions of the above rebars were computed by LPile

Minimum spacing between any two bars not equal to zero = 7.645 inches  
between bars 18 and 19.

Ratio of bar spacing to maximum aggregate size = 10.19

#### Concrete Properties:

-----

Compressive Strength of Concrete	=	3000. psi
Modulus of Elasticity of Concrete	=	3122019. psi
Modulus of Rupture of Concrete	=	-410.79192 psi
Compression Strain at Peak Stress	=	0.001634
Tensile Strain at Fracture of Concrete	=	-0.0001160
Maximum Coarse Aggregate Size	=	0.750000 in

Number of Axial Thrust Force Values Determined from Pile-head Loadings = 1

Number	Axial Thrust Force kips
1	40.000

-----  
 Summary of Results for Nominal Moment Capacity for Section 1  
 -----

Moment values interpolated at maximum compressive strain = 0.003  
 or maximum developed moment if pile fails at smaller strains.

Load Tens. No. Strain	Axial Thrust kips	Nominal Mom. Cap. in-kip	Max. Comp. Strain	Max.
1 -0.01470111	40.000	88993.752	0.00300000	

Note that the values of moment capacity in the table above are not factored by a strength reduction factor (phi-factor).

In ACI 318, the value of the strength reduction factor depends on whether the transverse reinforcing steel bars are tied hoops (0.65) or spirals (0.75).

The above values should be multiplied by the appropriate strength reduction factor to compute ultimate moment capacity according to ACI 318, or the value required by the design standard being followed.

The following table presents factored moment capacities and corresponding bending stiffnesses computed for common resistance factor values used for reinforced concrete sections.

Axial Stiff. Load Ult Mom No. kip-in^2	Resist. Factor	Nominal Ax. Thrust kips	Nominal Moment Cap in-kips	Ult. (Fac) Ax. Thrust kips	Ult. (Fac) Moment Cap in-kips	Bend. at
1 1.8481E+09	0.65	40.000000	88994.	26.000000	57846.	
1 1.7948E+09	0.75	40.000000	88994.	30.000000	66745.	

↑  
 MAX CAPACITY = 88994 IN-KIPS /12 = 7,416 FT-KIPS

1      0.90                      40.000000                      88994.                      36.000000                      80094.  
 1.2199E+09

-----  
 Layering Correction Equivalent Depths of Soil & Rock Layers  
 -----

Layer No.	Top of Layer Below Pile Head ft	Equivalent Top Depth Below Grnd Surf ft	Same Layer Type As Layer Above	Layer is Rock or is Below Rock Layer	F0 Integral for Layer lbs	F1 Integral for Layer lbs
1	1.0000	0.00	N.A.	No	0.00	327706.
2	11.0000	10.0000	No	Yes	N.A.	N.A.

Notes: The F0 integral of Layer n+1 equals the sum of the F0 and F1 integrals for Layer n. Layering correction equivalent depths are computed only for soil types with both shallow-depth and deep-depth expressions for peak lateral load transfer. These soil types are soft and stiff clays, non-liquefied sands, and cemented c-phi soil.

-----  
 Summary of Pile-head Responses for Conventional Analyses  
 -----

Definitions of Pile-head Loading Conditions:

- Load Type 1: Load 1 = Shear, V, lbs, and Load 2 = Moment, M, in-lbs
- Load Type 2: Load 1 = Shear, V, lbs, and Load 2 = Slope, S, radians
- Load Type 3: Load 1 = Shear, V, lbs, and Load 2 = Rot. Stiffness, R, in-lbs/rad.
- Load Type 4: Load 1 = Top Deflection, y, inches, and Load 2 = Moment, M, in-lbs
- Load Type 5: Load 1 = Top Deflection, y, inches, and Load 2 = Slope, S, radians

Case No.	Load Type	Load 1	Load 2	Axial Loading lbs	Pile-head Deflection inches	Pile-head Rotation radians	Max in lbs
1	V, lb	43000.	M, in-lb	4.20E+07	40000.	2.0079	-0.01331
		-770945.					
		4.41E+07					

MAX SHEAR = 770945 LBS

MAX MOMENT = 44100000 IN-LBS /12000 = 3,675 FT-KIPS

Maximum pile-head deflection = 2.0079130672 inches  
 Maximum pile-head rotation = -0.0133135993 radians = -0.762813 deg.

-----  
 Computed Pile-head Stiffness Matrix Values  
 K[2,2], K[2,3], K[3,2], K[3,3] for Pile Head  
 -----

Computations are based on the pile-head loads defined in Load Case 1

The K[2,2] and K[3,2] stiffnesses are computed using the specified pile-head shear force and rotation (Type 2) pile-head condition.

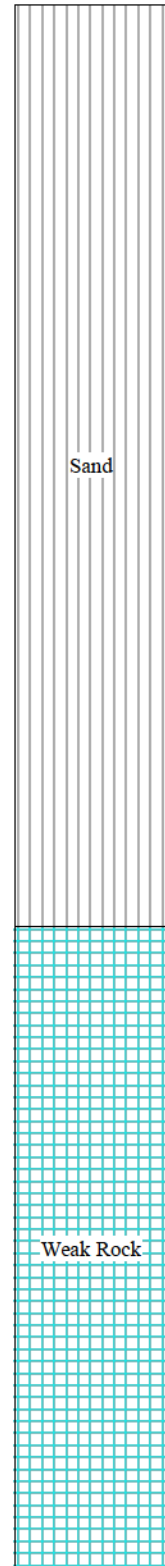
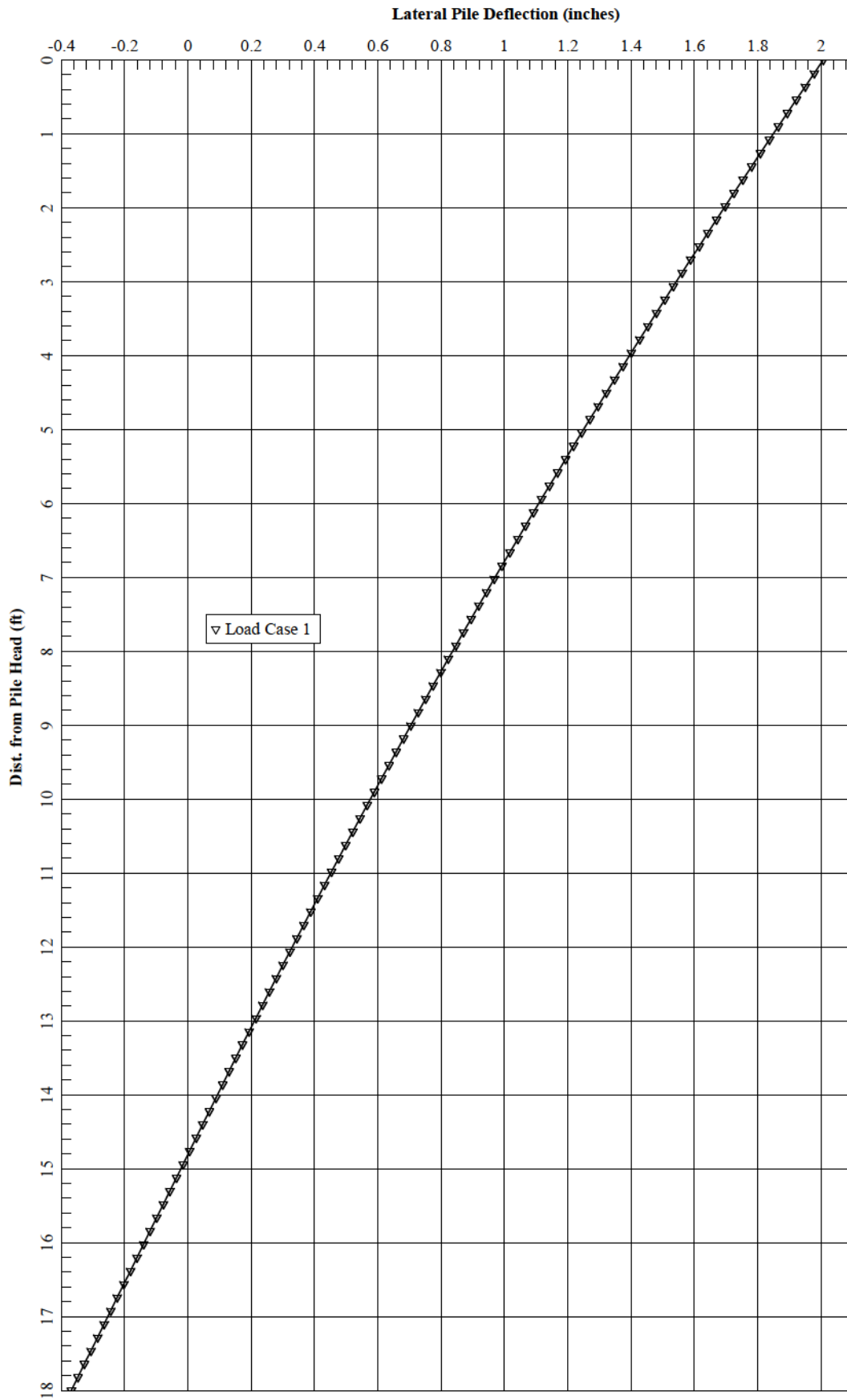
K[2,2] = abs(Shear Reaction/Top y)			K[3,2] = abs(Moment Reaction/Top y)		
Pile-Top Deflection inches	Pile-Top Rotation radians	Pile-Top Shear Reac. lbs	Pile-Top Mom. Reac. in-lbs	K[2,2] V/y lb/in.	K[3,2] M/y in-lb/in.
0.0001558	0.00000	4300.	346249.	27599899.	2222428597.
0.0006281	0.00000	15950.	1319107.	25395227.	2100270671.
0.0009485	0.00000	22765.	1916286.	23999590.	2020246210.
0.0011905	0.00000	27600.	2348378.	23182648.	1972541994.
0.0013908	0.00000	31350.	2690596.	22541244.	1934580741.
0.0015553	0.00000	34414.	2970454.	22127766.	1909939350.
0.0017037	0.00000	37005.	3212172.	21721073.	1885455046.
0.0018319	0.00000	39250.	3421153.	21426167.	1867591757.
0.0019559	0.00000	41229.	3611513.	21079516.	1846481951.
0.0020645	0.00000	43000.	3780466.	20827937.	1831146862.

The K[2,3] and K[3,3] stiffnesses are computed using the specified deflection and moment (Type 4) pile-head condition.

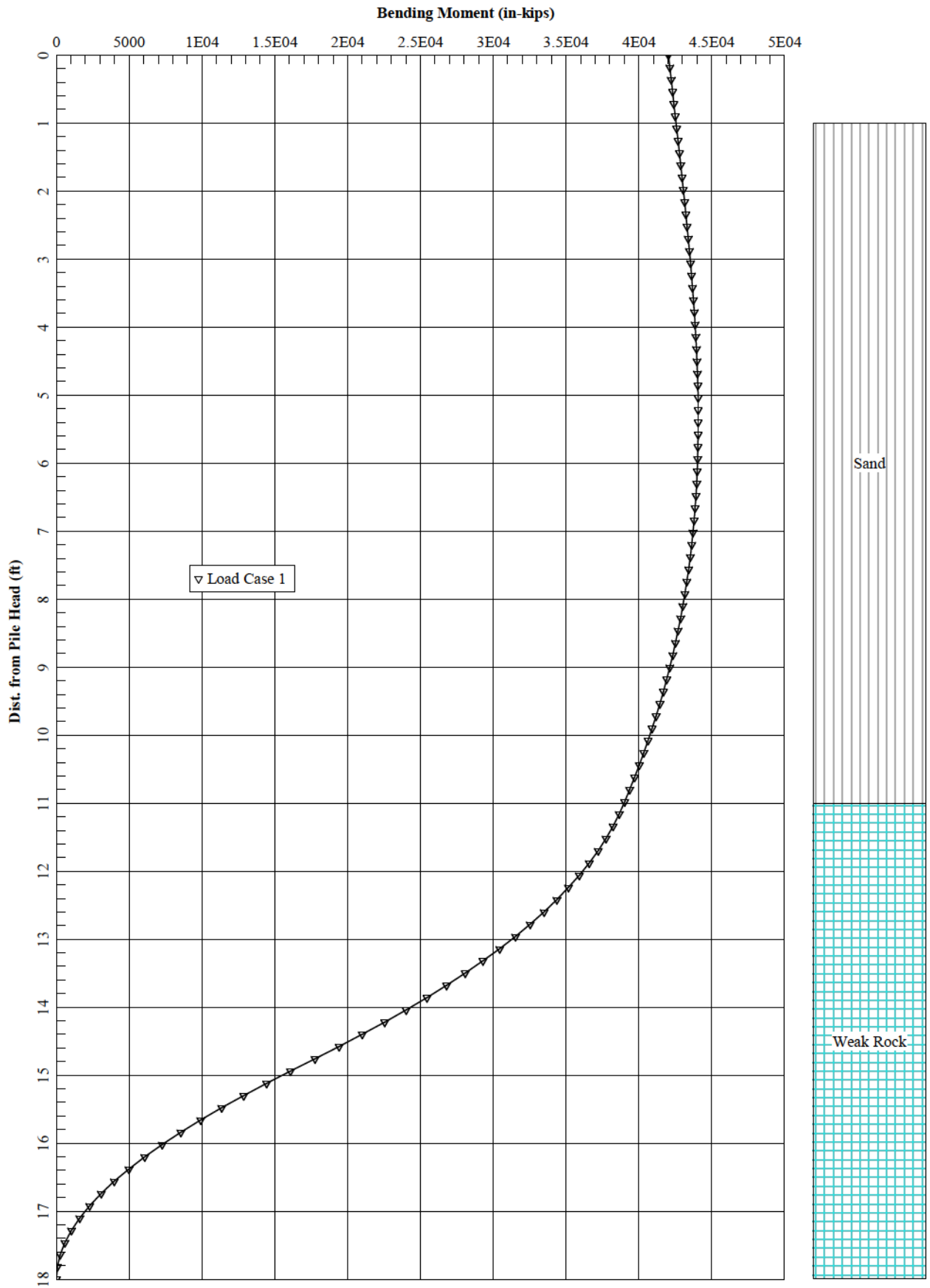
K[2,3] = abs(Shear Force/Top Rotation)			K[3,3] = abs(Moment/Top Rotation)		
Pile-Top Deflection inches	Pile-Top Rotation radians	Pile-Top Shear Reac. lbs	Pile-Top Mom. Reac. in-lbs	K[2,3] V/rot. lb/rad	K[3,3] M/rot. in-lb/rad
0.00000	0.00001858	36856.	4202400.	1983280859.	2.26140E+11
0.00000	0.00008048	118472.	15587836.	1472088484.	1.93688E+11
0.00000	0.0001214	159842.	22247889.	1316505726.	1.83240E+11
0.00000	0.0002849	189211.	26973272.	664160609.	9.46804E+10
0.00000	0.0004880	214654.	30638564.	439877761.	6.27857E+10
0.00000	0.0006203	235110.	33633325.	379021732.	5.42203E+10
0.00000	0.0007368	252680.	36165360.	342933102.	4.90830E+10

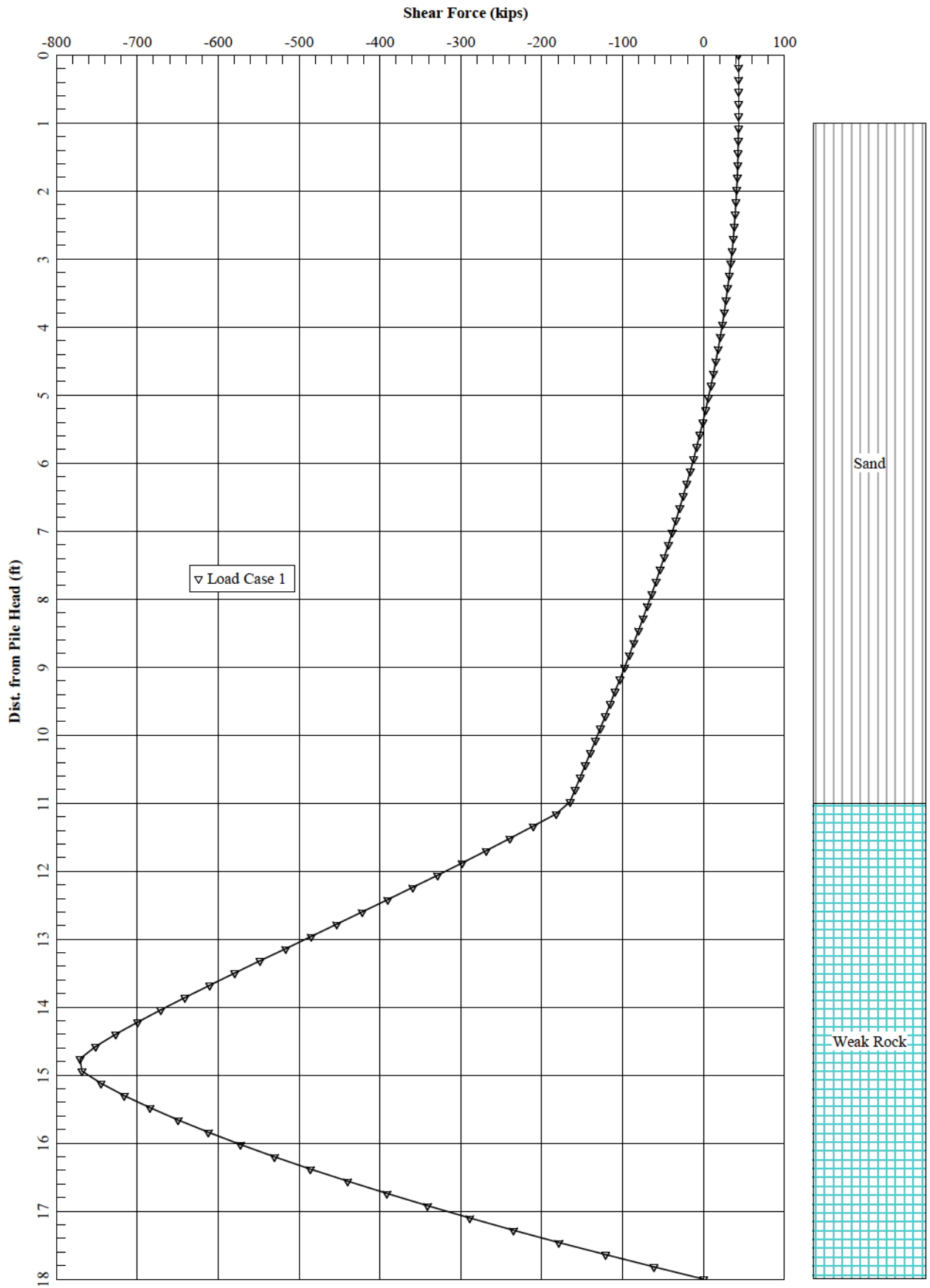
0.00000	0.0008315	267660.	38358708.	321908643.	4.61331E+10
0.00000	0.0009092	280555.	40293378.	308585157.	4.43190E+10
0.00000	0.0009716	291697.	42024000.	300229309.	4.32532E+10

The analysis ended normally.









Section 1 - RFD'S GENERAL INFORMATION

RFD'S NAME	CT2287	DATE	6/25/2014	RF DESIGN ENG	Mc Mahon	RF PERM DATE		RFD'S PROGRAM TYPE	2011 5G NR Radio	
ISSUE	Device Standard	Approved? (Y/N)	Yes	RF DESIGN PHONE	8622569382	RF PERM PHONE		RFD'S TECHNOLOGY	5G NR 15R CBAND	
REVISION	Final	RF MANAGER	Cameron Syme	RF DESIGN EMAIL	LM333@ATT.COM	RF PERM EMAIL		STATUS/STATUS	Final/Approved	
ADDITIONAL WORKFLOW NOTIFICATIONS:							RFD'S ID:			4732038
RFD'S VERSION:							Created By:	mh705	Updated By:	mh705
LIMITS FREQUENCY:							Created:	6/25/2014	Updated:	1/14/2022
LTE FREQUENCY:							Estimated SQM:	15,899	Expiration:	
5G FREQUENCY:							REF Initiative:		Calculation ID:	20111100616051203
PLAN JOB # 1:							PRD   SUB GRP #:	5G NR Radio   5G NR 15R CBand		
PLAN JOB # 2:							PRD   SUB GRP #:	Cell Site RF Modifications   BRU Add		
PLAN JOB # 3:							PRD   SUB GRP #:	Cell Site RF Modifications   5G NR Upgrade		
PLAN JOB # 4:							PRD   SUB GRP #:	5G NR Radio   5G NR 15R-1		
PLAN JOB # 5:							PRD   SUB GRP #:	LTE Software Center   LTE 4G		
PLAN JOB # 6:							PRD   SUB GRP #:	LTE Next Center   LTE 3G		
PLAN JOB # 7:							PRD   SUB GRP #:	5G NR Software Radio   5G NR Activation		
PLAN JOB # 8:							PRD   SUB GRP #:	5G NR Radio   5G NR 15R CBand		

C-Band, DuD and Multicarrier SCW

Section 2 - LOCATION INFORMATION

URID:	138248	FA LOCATION CODE:	19141389	LOCATION NAME:	ORANGE CT RIDGE ROAD	ORACLE PLUT # 1:	2051A02L7	FACE JOB #1:	MRCTB052845
REGION:	NORTHEAST	MARKET CLUSTER:	NEW ENGLAND	MARKET:	CONNECTICUT	ORACLE PLUT # 2:	2051A0270W	FACE JOB #2:	MRCTB051144
ADDRESS:	298 RIDGE ROAD	CITY:	ORANGE	STATE:	CT	ORACLE PLUT # 3:	2051A027EF	FACE JOB #3:	MRCTB050790
ZIP CODE:	06477	COUNTY:	NEW HAVEN	LONG (DEC. DECS):	-73.042600	ORACLE PLUT # 4:	2051A027AD	FACE JOB #4:	MRCTB051256
412 15m:	30.4559	LONGITUDE (D-M-S):	-73.2m 30.3359	LAT (DEC. DECS):	-41.258400	ORACLE PLUT # 5:	2051A027E5	FACE JOB #5:	MRCTB050015
DIRECTIONAL, ACCESS AND EQUIPMENT LOCATION:						ORACLE PLUT # 6:	2051A02794	FACE JOB #6:	MRCTB051353
FROM HARTFORD, TAKE RT 91 SOUTH TO EXIT 17 ROUTE 15 SOUTH. TAKE EXIT 58 AND TURN RIGHT ONTO GRASSY HILL ROAD. TURN LEFT ONTO CLARK LANE. TAKE SECOND RIGHT ONTO RIDGE ROAD. RFD'S ARE IN THE SHELTER, ALL LOOK UP THE POLE. THE COMBO IS 0943 ON THE GATE, 12345678 DOOR COMBO 5509 NU GATE.						ORACLE PLUT # 7:		FACE JOB #7:	MRCTB051428
						ORACLE PLUT # 8:	2051A027GK	FACE JOB #8:	MRCTB051476
						BORDER CELL WITH CONTOUR COORDS:		SEARCH RING NAME:	
						AM STUDY RFD ID (Y/N):	No	SEARCH RING ID:	
						RING COORD:		BTA:	MSA / RSA:
						LAC(UMTS):	05991		
						RF DISTRICT:	T8D		
						RF ZONE:	T8D	RNC(UMTS):	BRIDGEPORT RNC05
								MRE POOL (DLTE):	FF01
						PARENT NAME(UMTS):	BRPCT04CR0R05		

Section 3 - LICENSE COVERAGE/FILING INFORMATION

COBA - NO FILING REQUIRED (Yes/No):	No	COBA LOSS:		PCS REDUCED - UPS ZP:		COBA CALL SIGN:	
COBA - MINOR FILING NEEDED (Yes/No):	No	COBA EXT ASMT NEEDED:		PCS POPS REDUCED:			
COBA - MAJOR FILING NEEDED (Yes/No):	Yes	COBA SCORECARD UPDATED:					

Section 4 - TOWER/REGULATORY INFORMATION

STRUCTURE AT ST OWNED?:	Yes	GROUND ELEVATION (FT):	0	STRUCTURE UTILITY TYPE:	MARKET LOCATION 700 Mhz Band
ADDITIONAL REGULATORY?:	Yes	HEIGHT OVERALL (FT):	97.50	FCC ABR #:	MARKET LOCATION 800 Mhz Band
SUB-LEASE RIGHTS?:	Yes	STRUCTURE HEIGHT (FT):	97.50	MARKET LOCATION 1900 Mhz Band:	
LIGHTING TYPE:	NOT REQUIRED			MARKET LOCATION AWS Band:	
				MARKET LOCATION WCS Band:	
				MARKET LOCATION Future Band:	

Section 5 - E-911 INFORMATION - existing

SECTOR	PSAP NAME:	PSAP ID:	E911 PHASE:	MFC SVC PROVIDER:	LIQU REQUIRED:	ESRN:	DATE LIVE PH1:	DATE LIVE PH2:
SECTOR A	E-911			INTRADO	0			
SECTOR B				INTRADO	0			
SECTOR C				INTRADO	0			
SECTOR D								
SECTOR E								
SECTOR F								
OMN								

Section 5 - E-911 INFORMATION - final

SECTOR	PSAP NAME:	PSAP ID:	E911 PHASE:	MFC SVC PROVIDER:	LIQU REQUIRED:	ESRN:	DATE LIVE PH1:	DATE LIVE PH2:
SECTOR A	E-911			INTRADO	0			
SECTOR B				INTRADO	0			
SECTOR C				INTRADO	0			
SECTOR D								
SECTOR E								
SECTOR F								
OMN								

Section 67 - BBU INFORMATION - existing

	BBU 1	BBU 2
BBU ID	133304	43555
TECHNOLOGY	UMTS	LTE
BBU NAME	CTU2267	CTU2267
BBU UUID	133348	133348
CELL ID / BCF	CTU2267	CTU2267
STATE	IL	IL
4-DIGIT SITE ID	2267	2267
COW OR TOPT	No	No
CELL SITE TYPE	SECTORIZED	SECTORIZED
SITE TYPE	MACRO-CONVENTIONAL	MACRO-CONVENTIONAL
BTS LOCATION ID	INTERNAL	INTERNAL
BASE STATION TYPE	BASE	OVERLAY
EQUIPMENT NAME	NORTH STREET, MILFORD CT UMS	NORTH STREET, MILFORD CT LTE
DISASTER PRIORITY	0	0
EQUIPMENT VENDOR	ERICSSON	ERICSSON
EQUIPMENT TYPE (Model)	8801 INDOOR MU	
BANDWIDTH CONFIGURATION		
MARKET STATE CODE		CT
NODE B NUMBER	0	2267
SIDEHAUL SWITCH VENDOR		
SIDEHAUL SWITCH MODEL		
SIDEHAUL SWITCH NAME		
CBS - CTS COMMON ID	CTU2267	CTU2267
CBS - SECONDARY FUNCTION ID		

Section 67 - BBU INFORMATION - final

	BBU 1	BBU 2
BBU ID	43555	0
TECHNOLOGY	LTE	LTE, 2G
BBU NAME	CTU2267	CTU2267, CTU2267
BBU UUID	133348	133348
CELL ID / BCF	CTU2267	CTU2267
STATE	IL	IL
4-DIGIT SITE ID	2267	2267
COW OR TOPT	No	No
CELL SITE TYPE	SECTORIZED	SECTORIZED
SITE TYPE	MACRO-CONVENTIONAL	MACRO-CONVENTIONAL
BTS LOCATION ID	INTERNAL	INTERNAL
BASE STATION TYPE	OVERLAY	OVERLAY
EQUIPMENT NAME	NORTH STREET, MILFORD CT LTE	NORTH STREET, MILFORD CT LTE
DISASTER PRIORITY	0	0
EQUIPMENT VENDOR	ERICSSON	ERICSSON
EQUIPMENT TYPE (Model)	BASEBAND 603	BASEBAND 603
BANDWIDTH CONFIGURATION	1x600 / 1x600 / 1x600/3	none / 1x600 / none + 10x
MARKET STATE CODE	CT	CT, CT
NODE B NUMBER	2267	2267, 2267
SIDEHAUL SWITCH VENDOR		
SIDEHAUL SWITCH MODEL		
SIDEHAUL SWITCH NAME		
CBS - CTS COMMON ID	CTU2267	
CBS - SECONDARY FUNCTION ID		

Section 7b - Radio INFORMATION - existing

Section 7b - Radio INFORMATION - final

Section 8 - RBS/SECTOR ASSOCIATION - existing

	BBU 1	BBU 2
CTS Common ID	CTU2267	CTU2267
RBS Sector ID	CTU2267	CTU2267_RA_1
	CTU2267B	CTU2267_RB_1
	CTU2267C	CTU2267_RC_1
	CTU2267D	CTU2267_RA_1
	CTU2267E	CTU2267_RA_2
	CTU2267F	CTU2267_RB_1
	CTU2267A	CTU2267_RB_2
	CTU2267B	CTU2267_RC_1
	CTU2267C	CTU2267_RC_2



Section 15A - CURRENT TOWER CONFIGURATION - SECTOR A (OR OMNI)

ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
ANTENNA MAKE / MODEL	BBN1-1D6565C	BBN1-1D6565C	BBN1-1D6565C	CPA45SR-LCUJ-48		
ANTENNA VENDOR	Andrew	Andrew	Andrew	CCI Products		
ANTENNA SIZE (H x W x D)	96.4X11.8X7.1	96.4X11.8X7.1	96.4X11.8X7.1	62.7X14.4X7		
ANTENNA WEIGHT	80.8	80.8	80.8	88		
AZIMUTH	70	70	70	70		
MAGNETIC DECLINATION						
RADIATION CENTER (feet)	111	111	111	111		
ANTENNA TIP HEIGHT	115	115	115	115		
MECHANICAL DOWNTILT	0	0	0	0		
FEEDER ANGLE	4	2	2			
VERTICAL SEPARATION from ANTENNA ABOVE (TP to TP)						
VERTICAL SEPARATION from ANTENNA BELOW (TP to TP)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # if not both)						
Antenna RET Model (QTY/MODEL)	Built-in	Built-in	Built-in	Built-in		
SURGE ARRESTOR (QTY/MODEL)				8	#P10C-60FDMA-DBW	
DUPLEXER (QTY/MODEL)	2	2	2	2	CM1007-DBP95C-003	
DUPLEXER (QTY/MODEL)						
Antenna RET CONTROL UNIT (QTY/MODEL)	1	Powermax 7070				
DC BLOCK (QTY/MODEL)						
TUNING MATCH (QTY/MODEL)	1	12A (Twin 700850 Bypass)	1	12A (Twin 700850 Bypass)	1	12A (Twin 700850 Bypass)
CURRENT INJECTORS FOR TMA (QTY/MODEL)		ASQ Diplexer		ASQ Diplexer		
POU FOR TMA (QTY/MODEL)						
FILTER (QTY/MODEL)						
SOLID (QTY/MODEL)						
FEEDER TRUNK (QTY/MODEL)						
DC TRUNK (QTY/MODEL)						
REPEATER (QTY/MODEL)						
RRH - 700 band (QTY/MODEL)				1	RRH-11 B12	
RRH - 850 band (QTY/MODEL)						
RRH - 1900 band (QTY/MODEL)				1	RRH-12 B2 + RRH-12 B2S	
RRH - 800S band (QTY/MODEL)						
RRH - WCS band (QTY/MODEL)						
Additional RRH #1 - any band (QTY/MODEL)						
Additional RRH #2 - any band (QTY/MODEL)						
RRH_7B_1 (QTY/MODEL)						
RRH_7B_2 (QTY/MODEL)						
RRH_7B_3 (QTY/MODEL)						
Additional Component 1 (QTY/MODEL)						
Additional Component 2 (QTY/MODEL)						
Additional Component 3 (QTY/MODEL)						
Local Market Note 1						
Local Market Note 2						
Local Market Note 3						

PORT SPECIFIC RELO	PORT NUMBER	USED (CSBng)	USED (Awh)	ATOLL TXID	ATOLL CELL ID	TWRX?	TECHNOLOGY / FREQUENCY	ANTENNA ATOLL	ANTENNA GAN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	ROAT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPANCPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID(111ng)	
ANTENNA POSITION 1	PORT 1		135348.A.850.3 0.1	CTV22671	CTV22671		UMTS 850	1D6565C_850M RS_04DT	13.41	70	4	Bottom	Comscape 1-5B (850)	140						835.33		1		
	PORT 2		135348.A.850.3 0.2	CTV2267A	CTV2267A		UMTS 850	1D6565C_850M RS_04DT	13.41	70	4	Bottom	Comscape 1-5B (850)	140							835.33		1	
	PORT 3		135348.A.1900.3 0.1	CTU22677	CTU22677		UMTS 1900	1D6565C_1900 MS_04DT	17	70	4	Bottom	Comscape 1-5B (1900)	140							1172.2		2	
ANTENNA POSITION 2	PORT 1					GSM			70	0														
ANTENNA POSITION 3	PORT 1					GSM			70	0														
ANTENNA POSITION 4	PORT 1		135348.A.700.4 0.1	CTL02287_7A.1	CTL02287_7A.1		LTE 700	RR_7194MHz_04 DT	15.3	70	4	Bottom	Comscape 1-5B (700)	140							931.1078		7	
	PORT 3		135348.A.1900.4 0.1	CTL02287_BA.1	CTL02287_BA.1		LTE 1900	RR_1948MHz_0 4DT	15.3	70	4	Bottom	Comscape 1-5B (700)	140							3380.8483		8	
	PORT 2		135348.A.1900.4 0.2	CTL02287_BA.2	CTL02287_BA.2		LTE 1900	RR_1948MHz_0 4DT	15.3	70	4	Bottom	Comscape 1-5B (700)	140							3380.8483		8	
	PORT 4		135348.A.1900.4 0.2	CTL02287_BA.2	CTL02287_BA.2		LTE 1900	RR_1948MHz_0 4DT	15.3	70	4	Bottom	Comscape 1-5B (700)	140							3380.8483		8	

Section 15B - CURRENT TOWER CONFIGURATION - SECTOR B

ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
ANTENNA MAKE / MODEL	BBN1-1D6565C	BBN1-1D6565C	BBN1-1D6565C	CPA45SR-LCUJ-48		
ANTENNA VENDOR	Andrew	Andrew	Andrew	CCI Products		
ANTENNA SIZE (H x W x D)	96.4X11.8X7.1	96.4X11.8X7.1	96.4X11.8X7.1	62.7X14.4X7		
ANTENNA WEIGHT	80.8	80.8	80.8	88		
AZMUTH	190	190	190	190		
MAGNETIC DECLINATION						
RADIATION CENTER (Inch)	111	111	111	111		
ANTENNA TIP HEIGHT	115	115	115	115		
MECHANICAL DOWNTILT	0	0	0	0		
FEEDER AMOUNT	4	2	2			
VERTICAL SEPARATION from ANTENNA ABOVE (TP to TP)						
VERTICAL SEPARATION from ANTENNA BELOW (TP to TP)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # it is of)						
Antenna RET Motor (QTY/MODEL)	Built-in	Built-in	Built-in	Built-in		
SURGE ARRESTOR (QTY/MODEL)				8	8P100-80FDMA-DBW	
DUPLEXER (QTY/MODEL)	2	2	2	2	CM1007-D8P95C-003	
DUPLEXER (QTY/MODEL)						
Antenna RET CONTROL UNIT (QTY/MODEL)						
DC BLOCK (QTY/MODEL)						
TMA/NA (QTY/MODEL)	1	1	1	1	12A (Twin 700R50 Bypass)	
CURRENT INJECTORS FOR TMA (QTY/MODEL)	ASQ Diplexer			ASQ Diplexer		
POU FOR TMA# (QTY/MODEL)						
FILTER (QTY/MODEL)						
SOLID (QTY/MODEL)						
FEEDER TRUNK (QTY/MODEL)						
DC TRUNK (QTY/MODEL)						
REPEATER (QTY/MODEL)						
RRH - 700 band (QTY/MODEL)				1	RRH-11 B12	
RRH - 850 band (QTY/MODEL)						
RRH - 1900 band (QTY/MODEL)				1	RRH-12 B2 + RRH-12 B2S	
RRH - 800S band (QTY/MODEL)						
RRH - WCS band (QTY/MODEL)						
Additional RRH #1 - any band (QTY/MODEL)						
Additional RRH #2 - any band (QTY/MODEL)						
RRH_7B_1 (QTY/MODEL)						
RRH_7B_2 (QTY/MODEL)						
RRH_7B_3 (QTY/MODEL)						
Additional Component 1 (QTY/MODEL)						
Additional Component 2 (QTY/MODEL)						
Additional Component 3 (QTY/MODEL)						
Local Market Note 1						
Local Market Note 2						
Local Market Note 3						

PORT SPECIFIC RELO	PORT NUMBER	USED (CSBng)	USED (Awh)	ATOLL TXID	ATOLL CELL ID	TWRX?	TECHNOLOGY / FREQUENCY	ANTENNA ATOLL	ANTENNA GAN	ELECTRICAL AZMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (ft)	ROAT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPAN/CPA MODULE?	HATCHPLATE POWER (Watts)	SWP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID(s+ing)	
ANTENNA POSITION 1	PORT 1		135348.B.850.3 0.1	CTV22672	CTV22672		UMTS 850	1D6565C_850M RS_840T	13.41	190	4	Bottom	Comscape 1-5B (850)	140						835.33		8		
	PORT 2		135348.B.850.3 0.2	CTV22678	CTV22678		UMTS 850	1D6565C_850M RS_840T	13.41	190	4	Bottom	Comscape 1-5B (850)	140							835.33		8	
	PORT 3		135348.B.1900.3 0.1	CTU22678	CTU22678		UMTS 1900	1D6565C_1900 MS_840T	17	190	4	Bottom	Comscape 1-5B (1900)	140							1172.2		15	
ANTENNA POSITION 2	PORT 1					GSM			190	0														
ANTENNA POSITION 3	PORT 1					GSM			190	0														
ANTENNA POSITION 4	PORT 1		135348.B.700.4 0.1	CTL02287_7B_1	CTL02287_7B_1		LTE 700	RR_719MHz_04 DT	15.3	190	4	Bottom	Comscape 1-5B (700)	140							931.1078		15	
	PORT 3		135348.B.1900.4 0.1	CTL02287_8B_1	CTL02287_8B_1		LTE 1900	RR_1948MHz_0 SDT	15.3	190	3	Bottom	Comscape 1-5B (700)	140							3380.8483		18	
	PORT 2		135348.B.1900.4 0.2	CTL02287_8B_2	CTL02287_8B_2		LTE 1900	RR_1948MHz_0 SDT	15.3	190	3	Bottom	Comscape 1-5B (700)	140							3380.8483		18	
	PORT 4		135348.B.1900.4 0.2	CTL02287_8B_2	CTL02287_8B_2		LTE 1900	RR_1948MHz_0 SDT	15.3	190	3	Bottom	Comscape 1-5B (700)	140							3380.8483		18	

Section 15C - CURRENT TOWER CONFIGURATION - SECTOR C

ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
ANTENNA MAKE / MODEL	BBN1-1D6565C	BBN1-1D6565C	BBN1-1D6565C	CPA45SR-LCUJ-148		
ANTENNA VENDOR	Andrew	Andrew	Andrew	CCI Products		
ANTENNA SIZE (H x W x D)	96.4X11.8X7.1	96.4X11.8X7.1	96.4X11.8X7.1	62.7X14.4X7		
ANTENNA WEIGHT	80.8	80.8	80.8	88		
AZIMUTH	310	310	310	310		
MAGNETIC DECLINATION						
RADIATION CENTER (Inch)	111	111	111	111		
ANTENNA TIP HEIGHT	115	115	115	115		
MECHANICAL DOWNTILT	0	0	0	0		
FEEDER AMOUNT	4	2	2			
VERTICAL SEPARATION from ANTENNA ABOVE (TP to TP)						
VERTICAL SEPARATION from ANTENNA BELOW (TP to TP)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # / # of inches)						
Antenna RET Motor (QTY/MODEL)	Built-in	Built-in	Built-in	Built-in		
SURGE ARRESTOR (QTY/MODEL)				8	#P10C-80FDMA-DBW	
DUPLEXER (QTY/MODEL)	2	2	2	2	CM1007-D8P95C-003	
DUPLEXER (QTY/MODEL)						
Antenna RET CONTROL LIMIT (QTY/MODEL)						
DC BLOCK (QTY/MODEL)						
TUNING MATCH (QTY/MODEL)	1	1	1	1	12A (Twin 700850 Bypass)	
CURRENT INJECTORS FOR TMA (QTY/MODEL)	ASQ Diplexer				ASQ Diplexer	
POU FOR TMA8 (QTY/MODEL)						
FILTER (QTY/MODEL)						
SOLID (QTY/MODEL)						
FEEDER TRUNK (QTY/MODEL)						
DC TRUNK (QTY/MODEL)						
REPEATER (QTY/MODEL)						
RRH - 700 band (QTY/MODEL)				1	RRH-11 B12	
RRH - 850 band (QTY/MODEL)						
RRH - 1900 band (QTY/MODEL)				1	RRH-12 B2 + RRH-12 B2S	
RRH - 800S band (QTY/MODEL)						
RRH - WCS band (QTY/MODEL)						
Additional RRH #1 - any band (QTY/MODEL)						
Additional RRH #2 - any band (QTY/MODEL)						
RRH_7B_1 (QTY/MODEL)						
RRH_7B_2 (QTY/MODEL)						
RRH_7B_3 (QTY/MODEL)						
Additional Component 1 (QTY/MODEL)						
Additional Component 2 (QTY/MODEL)						
Additional Component 3 (QTY/MODEL)						
Local Market Note 1						
Local Market Note 2						
Local Market Note 3						

PORT SPECIFIC RELO	PORT NUMBER	USED (CSBng)	USED (Awh)	ATOLL TXID	ATOLL CELL ID	TWRX?	TECHNOLOGY / FREQUENCY	ANTENNA ATOLL	ANTENNA GAN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (ft)	ROAT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPAN/CPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID(111ng)	
ANTENNA POSITION 1	PORT 1		135348.C.850.3	G1	CTV22673	CTV22673	UMTS 850	1D6565C_850M	HS_84DT	13.41	310	4	Bottom	CommScope 1-5/8 (850)	140					835.33		17		
	PORT 2		135348.C.850.3	G2	CTV2267C	CTV2267C	UMTS 850	1D6565C_850M	HS_84DT	13.41	310	4	Bottom	CommScope 1-5/8 (850)	140					835.33		17		
	PORT 3		135348.C.1900	SG1	CTU22679	CTU22679	UMTS 1900	1D6565C_1900	MS_84DT	17	310	4	Bottom	CommScope 1-5/8 (1900)	140					1172.2		18		
ANTENNA POSITION 2	PORT 1						GSM			310	0													
ANTENNA POSITION 3	PORT 1						GSM			310	0													
ANTENNA POSITION 4	PORT 1		135348.C.700.4	G1	CTL02287_7C_1	CTL02287_7C_1	LTE 700	HR_7194MHz_04	DT	15.3	310	4	Bottom	CommScope 1-5/8 (700)	140					931.1078		23		
	PORT 3		135348.C.1900	AG1	CTL02287_9C_1	CTL02287_9C_1	LTE 1900	HR_1948MHz_0	SDT	15.3	310	3	Bottom	CommScope 1-5/8 (700)	140					3380.8483		24		
	PORT 3		135348.C.1900	AG2	CTL02287_9C_2	CTL02287_9C_2	LTE 1900	HR_1948MHz_0	SDT	15.3	310	3	Bottom	CommScope 1-5/8 (700)	140					3380.8483		24		
	PORT 4		135348.C.1900	AG2	CTL02287_9C_2	CTL02287_9C_2	LTE 1900	HR_1948MHz_0	SDT	15.3	310	3	Bottom	CommScope 1-5/8 (700)	140					3380.8483		24		



Section 16A - PLANNED/PROPOSED TOWER CONFIGURATION - SECTOR A (OR OMNI)

ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
* Following Antennas are						
ANTENNA MAKE - MODEL	TPABER-SUBDA-K	ARR449 B770-ARR519 B770 STACSED	DMPEER-SUBDA			
ANTENNA VENDOR	CCI	Efason	CCI			
ANTENNA SIZE (H x W x D)	98.000 7.0 7	30.4X15.9X8.1	98.000 7.0 7			
ANTENNA WEIGHT	87.1	81.8	85.7			
AZIMUTH	70	70	70			
MAGNETIC DECLINATION						
RADIATION CENTER (feet)	111	111	111			
ANTENNA TIP HEIGHT	115	115	115			
MECHANICAL DOWNTILT	0	0	0			
FEEDER AMOUNT		Fiber				
VERTICAL SEPARATION from ANTENNA ABOVE (TP to TP)						
VERTICAL SEPARATION from ANTENNA BELOW (TP to TP)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # 1 of 2)						
Antenna RET Motor (QTY/MODEL)		Built in	Built in	Built in		
SURGE ARRESTOR (QTY/MODEL)	12	TSDCC-4110FM	8	TSDCC-4110FM		
DUPLEXER (QTY/MODEL)	2	CB0819237-06-43	2	DB00115F1W1-1-2		
DUPLEXER (QTY/MODEL)						
Antenna RET CONTROL UNIT (QTY/MODEL)						
DC BLOCK (QTY/MODEL)						
TRIALNA (QTY/MODEL)	1	TMC2104FD0V-2D	2	TMC2104FD0V-1		
CURRENT INJECTORS FOR TMA (QTY/MODEL)						
POU FOR TMA (QTY/MODEL)						
FILTER (QTY/MODEL)						
BOARD (QTY/MODEL)		1	DCS-48-85-18			
FIBER TRUNK (QTY/MODEL)						
DC TRUNK (QTY/MODEL)						
REPEATER (QTY/MODEL)						
RRH - 700 band (QTY/MODEL)	1	4478 B14	1	4449 B0812 with another band		
RRH - 850 band (QTY/MODEL)						
RRH - 1900 band (QTY/MODEL)	1	8543 S2856A with another band				
RRH - AWS band (QTY/MODEL)						
RRH - WCS band (QTY/MODEL)						
Additional RRH #1 - any band (QTY/MODEL)		1	Integrated with ARR449 B770			
Additional RRH #2 - any band (QTY/MODEL)		1	Integrated with ARR519 B770			
RRH_7B_1 (QTY/MODEL)						
RRH_7B_2 (QTY/MODEL)						
RRH_7B_3 (QTY/MODEL)						
Additional Component 1 (QTY/MODEL)	2	K 887 782-11055	2	K 887 782-11055		
Additional Component 2 (QTY/MODEL)	2	AP70C-80FDMA-DB	2	AP70C-80FDMA-DB		
Additional Component 3 (QTY/MODEL)						
Local Market Note 1	- Follow Antenna and radio positions as per PD.					
Local Market Note 2	- Decom UMTS and remove unused line elements.					
Local Market Note 3	2x693D+1x08M+1xL+8549+2x4de					

PORT SPECIFIC FILEDS	PORT NUMBER	USED (CBS#)	USED (A#B)	ATOLL TXID	ATOLL CELL ID	TURX?	TECHNOLOGY / FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	RRH RET MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCF/AMC/PA MODULE?	MATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID(#+sig)	
ANTENNA POSITION 2	PORT 1			CTL02287_FA_3 F	CTL02287_FA_3 F		LTE 700	R	17198d, G4DT	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					831.1078		5		
	PORT 3			CTL09287_BA_1	CTL09287_BA_1		LTE 1900	R	1948Mq, G4D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
	PORT 4			CTL09287_BA_2	CTL09287_BA_2		LTE 1900	R	1948Mq, G4D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
	PORT 7			CTL09287_BA_2	CTL09287_BA_2		LTE AWS	R	1948Mq, G4D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
	PORT 8			CTCN002287_N 095A_1	CTCN002287_N 095A_1		5G AWS	R	1948Mq, G4D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
	PORT 11			CTCN002287_N 095A_1	CTCN002287_N 095A_1		5G 1900	R	1948Mq, G4D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
ANTENNA POSITION 3	PORT 1						5G CBAND			70	0	Integrated	Fiber	0								5		
ANTENNA POSITION 4	PORT 1			CTL02287_FA_1	CTL02287_FA_1		LTE 700	BURDA_7198d G4DT	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140						831.1078		7		
	PORT 2			CTCN002287_N 095A_1	CTCN002287_N 095A_1		5G 850	BURDA_7198d G4DT	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140						831.1078		7		

Section 16B - PLANNED/PROPOSED TOWER CONFIGURATION - SECTOR B

ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
*Following Antennas are LEFT to RIGHT from BACK OF ANTENNA (unless otherwise specified)						
Existing Antenna?						
ANTENNA MAKE - MODEL	TPABER-SUBDA-K	ARR449 B770-ARR54 19 B770 STACHED	DMPEER-SUBDA			
ANTENNA VENDOR	CCI	Eaton	CCI			
ANTENNA SIZE (H x W x D)	98.000 7X2 7	30.4X15 8X8 1	98.000 7X2 7			
ANTENNA WEIGHT	87.1	81.8	85.7			
AZIMUTH	190	190	190			
MAGNETIC DECLINATION						
RADIATION CENTER (feet)	111	111	111			
ANTENNA TIP HEIGHT	115	115	115			
MECHANICAL DOWNTILT	0	0	0			
FEEDER AMOUNT		Fiber				
VERTICAL SEPARATION from ANTENNA ABOVE (TP to TP)						
VERTICAL SEPARATION from ANTENNA BELOW (TP to TP)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # 1 of both)						
Antenna RET Motor (QTY/MODEL)		Built in	Built in	Built in		
SURGE ARRESTOR (QTY/MODEL)	12	TSDCC-4110FM	8	TSDCC-4110FM		
DUPLEXER (QTY/MODEL)	2	CB0119237-D6-43	2	DB00115F1W1-1-2		
DUPLEXER (QTY/MODEL)						
Antenna RET CONTROL UNIT (QTY/MODEL)						
DC BLOCK (QTY/MODEL)						
TRIALNA (QTY/MODEL)	1	TMC2104FD0V-2D	2	TMC2104FD0V-1		
CURRENT INJECTORS FOR TMA (QTY/MODEL)						
POU FOR TMA (QTY/MODEL)						
FILTER (QTY/MODEL)						
BOARD (QTY/MODEL)						
FIBER TRUNK (QTY/MODEL)						
DC TRUNK (QTY/MODEL)						
REPEATER (QTY/MODEL)						
RRH - 700 band (QTY/MODEL)	1	4478 B14	1	4449 B5B12 with another band		
RRH - 800 band (QTY/MODEL)						
RRH - 1900 band (QTY/MODEL)	1	8643 S2866A with another band				
RRH - AWS band (QTY/MODEL)						
RRH - WCS band (QTY/MODEL)						
Additional RRH #1 - any band (QTY/MODEL)		1	Integrated with ARR449 B770			
Additional RRH #2 - any band (QTY/MODEL)		1	Integrated with ARR449 B770			
RRH_7B_1 (QTY/MODEL)						
RRH_7B_2 (QTY/MODEL)						
RRH_7B_3 (QTY/MODEL)						
Additional Component 1 (QTY/MODEL)	2	K 887 780-11055	2	K 887 780-11055		
Additional Component 2 (QTY/MODEL)	2	AP70C-80FDMA-DB	2	AP70C-80FDMA-DB		
Additional Component 3 (QTY/MODEL)						
Local Market Note 1	- Follow Antenna and radio positions as per PD.					
Local Market Note 2	- Decom UMTS and remove unused line elements.					
Local Market Note 3	2x693D+1x08M+1xL+8649+3x4de					

PORT SPECIFIC FIELDS	PORT NUMBER	USED (CBS#)	USED (A#B)	ATOLL TXID	ATOLL CELL ID	TURX?	TECHNOLOGY / FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrate/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	RRH RET MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCF/AMC/PA MODULE?	MATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID(+sig)
ANTENNA POSITION 2	PORT 1			CTL02287_7B_3-F	CTL02287_7B_3-F		LTE 700	K 7198M4_04DT	15.3	190	4	Bottom	Comscape 1-5/8 (700)	140						831.1078		11	
	PORT 3			CTL06287_8B_1	CTL06287_8B_1		LTE 1900	K 1848M4_03D	15.3	190	3	Bottom	Comscape 1-5/8 (700)	140						3380.6483		12	
	PORT 4			CTL06287_8B_2	CTL06287_8B_2		LTE 1900	K 1848M4_03D	15.3	190	3	Bottom	Comscape 1-5/8 (700)	140						3380.6483		12	
	PORT 7			CTL06287_2B_2	CTL06287_2B_2		LTE AWS	K 1848M4_03D	15.3	190	3	Bottom	Comscape 1-5/8 (700)	140						3380.6483		12	
	PORT 8			CTC002287_N 005B_1	CTC002287_N 005B_1		5G AWS	K 1848M4_03D	15.3	190	3	Bottom	Comscape 1-5/8 (700)	140						3380.6483		12	
	PORT 11			CTC002287_N 005B_1	CTC002287_N 005B_1		5G 1900	K 1848M4_03D	15.3	190	3	Bottom	Comscape 1-5/8 (700)	140						3380.6483		12	
ANTENNA POSITION 3	PORT 1						5G CBAND			190	0	Integrated	Fiber	0								13	
ANTENNA POSITION 4	PORT 1			CTL02287_7B_1	CTL02287_7B_1		LTE 700	BURDA_7198M4_04DT	15.3	190	4	Bottom	Comscape 1-5/8 (700)	140						831.1078		15	
	PORT 2			CTC002287_N 005B_1	CTC002287_N 005B_1		5G 850	BURDA_7198M4_04DT	15.3	190	4	Bottom	Comscape 1-5/8 (700)	140						831.1078		15	

Section 16C - PLANNED/PROPOSED TOWER CONFIGURATION - SECTOR C

ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
*Following Antennas*						
ANTENNA MAKE - MODEL	TPMSR-SUBDA-K	ARR449 B770+ARR419 B770 STACDED	TPMSR-SUBDA			
ANTENNA VENDOR	CCI	Efason	CCI			
ANTENNA SIZE (H x W x D)	98.000 7X2 7	30.4X15.9X8.1	98.000 7X2 7			
ANTENNA WEIGHT	87.1	81.8	85.7			
AZIMUTH	310	310	310			
MAGNETIC DECLINATION						
RADIATION CENTER (feet)	111	111	111			
ANTENNA TIP HEIGHT	115	115	115			
MECHANICAL DOWNTILT	0	0	0			
FEDER AMOUNT		Fiber				
VERTICAL SEPARATION from ANTENNA ABOVE (TP to TP)						
VERTICAL SEPARATION from ANTENNA BELOW (TP to TP)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # 1 of 2)						
Antenna RET Model (QTY/MODEL)		Built in	Built in	Built in		
SURGE ARRESTOR (QTY/MODEL)	12	TSDCC-4110FM	8	TSDCC-4110FM		
DUPLEXER (QTY/MODEL)	2	CBCH1923T-D8-43	2	DBCH115F1W-1-2		
DUPLEXER (QTY/MODEL)						
Antenna RET CONTROL UNIT (QTY/MODEL)						
DC BLOCK (QTY/MODEL)						
TRIALNA (QTY/MODEL)	1	TMC2104FD0V-2D	2	TMC2104FD0V-1		
CURRENT INJECTORS FOR TMA (QTY/MODEL)						
POU FOR TMA5 (QTY/MODEL)						
FILTER (QTY/MODEL)						
BOARD (QTY/MODEL)						
FIBER TRUNK (QTY/MODEL)						
DC TRUNK (QTY/MODEL)						
REPEATER (QTY/MODEL)						
RRH - 700 band (QTY/MODEL)	1	4478 B14	1	4449 B5B12 with another band		
RRH - 800 band (QTY/MODEL)						
RRH - 1900 band (QTY/MODEL)	1	8843 S2858A with another band				
RRH - AWS band (QTY/MODEL)						
RRH - WCS band (QTY/MODEL)						
Additional RRH #1 - any band (QTY/MODEL)		1	Integrated with ARR449 B770			
Additional RRH #2 - any band (QTY/MODEL)		1	Integrated with ARR419 B770			
RRH_7B_1 (QTY/MODEL)						
RRH_7B_2 (QTY/MODEL)						
RRH_7B_3 (QTY/MODEL)						
Additional Component 1 (QTY/MODEL)	2	K 887 782-11055	2	K 887 782-11055		
Additional Component 2 (QTY/MODEL)	2	AP7DC-80FDMA-DB	2	AP7DC-80FDMA-DB		
Additional Component 3 (QTY/MODEL)						
Local Market Note 1	- Follow Antenna and radio positions as per PD.					
Local Market Note 2	- Decom UMTS and remove unused line elements.					
Local Market Note 3	2x693D+1x08MJ+DL+8849+3x4de					

PORT SPECIFIC FIELDS	PORT NUMBER	USED (CBS#)	USED (A#B)	ATOLL TXID	ATOLL CELL ID	TURX?	TECHNOLOGY / FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEDERS TYPE	FEDER LENGTH (feet)	RRH RET MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCF/ANCO/A-MODULE?	MATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID(+sig)	
ANTENNA POSITION 2	PORT 1			CTL02287_7C_3,F	CTL02287_7C_3,F		LTE 700	7198W_04DT	15.3	310	4	Bottom	Comscape 1-5/8 (700)	140						831.1078		19		
	PORT 2			CTL06287_9C_1	CTL06287_9C_1		LTE 1900	1848MF_03D	15.3	310	3	Bottom	Comscape 1-5/8 (700)	140						3380.6483		20		
	PORT 4			CTL06287_9C_2	CTL06287_9C_2		LTE 1900	1848MF_03D	15.3	310	2	Bottom	Comscape 1-5/8 (700)	140						3380.6483		20		
	PORT 7			CTL06287_9C_2	CTL06287_9C_2		LTE AWS	1848MF_03D	15.3	310	3	Bottom	Comscape 1-5/8 (700)	140						3380.6483		20		
	PORT 8			CTCN002287_N 886C_1	CTCN002287_N 886C_1		5G AWS	1848MF_03D	15.3	310	3	Bottom	Comscape 1-5/8 (700)	140						3380.6483		20		
	PORT 11			CTCN002287_N 895C_1	CTCN002287_N 895C_1		5G 1900	1848MF_03D	15.3	310	1	Bottom	Comscape 1-5/8 (700)	140						3380.6483		20		
ANTENNA POSITION 3	PORT 1						5G CBAND			310	0	Integrated	Fiber	0									21	
ANTENNA POSITION 4	PORT 1			CTL02287_7C_1	CTL02287_7C_1		LTE 700	BURDA_7198W_04DT	15.3	310	4	Bottom	Comscape 1-5/8 (700)	140						831.1078		23		
	PORT 2			CTCN002287_N 895C_1	CTCN002287_N 895C_1		5G 850	BURDA_7198W_04DT	15.3	310	4	Bottom	Comscape 1-5/8 (700)	140						831.1078		23		

Section 16.5A - SCOPING TOWER CONFIGURATION - SECTOR A (OR OMNI)

Section 17A - FINAL TOWER CONFIGURATION - SECTOR A (OR OMNI)

ANTENNA POSITION 1 LEFT to RIGHT from BACK OF ANTENNA (unless otherwise specified)	ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
ANTENNA MAKE / MODEL	TPABER-BURDAK		ARR449 B770+ARR419 B770 STACSED	DMWESR-BURDA			
ANTENNA VENDOR	CCI		Etoson	CCI			
ANTENNA SIZE (H x W x D)	98.000 7.0 7		30.4X15.9X8.1	98.000 7.0 7			
ANTENNA WEIGHT	87.1		81.8	85.7			
AZIMUTH	70		70	70			
MAGNETIC DECLINATION							
RADIATION CENTER (feet)	111		111	111			
ANTENNA TIP HEIGHT	115		115	115			
MECHANICAL DOWNTILT	0		0	0			
FEEDER AMOUNT	4		Fiber	4			
VERTICAL SEPARATION from ANTENNA ABOVE (TP to TP)							
VERTICAL SEPARATION from ANTENNA BELOW (TP to TP)							
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)							
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)							
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # 1 of both)							
Antenna RET Motor (QTY/MODEL)			Built-in			Built-in	
SURGE ARRESTOR (QTY/MODEL)	12	TSDCC-4110FM		8	TSDCC-4110FM		
DUPLEXER (QTY/MODEL)	2	CBCH1923T-D8-43		2	DBCO115F1W1-1-2		
DUPLEXER (QTY/MODEL)							
Antenna RET CONTROL UNIT (QTY/MODEL)							
DC BLOCK (QTY/MODEL)							
TRIALNA (QTY/MODEL)	1	TMC2104FDV95-2D		2	TMC2104FDV95-1		
CURRENT INJECTORS FOR TMA (QTY/MODEL)							
POU FOR TMA5 (QTY/MODEL)							
FILTER (QTY/MODEL)							
BOARD (QTY/MODEL)			1	DCS-48-85-18			
FIBER TRUNK (QTY/MODEL)							
DC TRUNK (QTY/MODEL)							
REPEATER (QTY/MODEL)							
RRH - 700 band (QTY/MODEL)	1	4478 B14		1	4449 B5B12 with another band		
RRH - 850 band (QTY/MODEL)							
RRH - 1900 band (QTY/MODEL)	1	8543 S2856A					
RRH - AWS band (QTY/MODEL)							
RRH - WCS band (QTY/MODEL)							
Additional RRH #1 - any band (QTY/MODEL)			1	Integrated with ARR449 B770			
Additional RRH #2 - any band (QTY/MODEL)			1	Integrated with ARR419 B770			
RRH_7B_1 (QTY/MODEL)							
RRH_7B_2 (QTY/MODEL)							
RRH_7B_3 (QTY/MODEL)							
Additional Component 1 (QTY/MODEL)	2	K 887 782-11055		2	K 887 782-11055		
Additional Component 2 (QTY/MODEL)	2	AP7DC-80FDMA-DB		2	AP7DC-80FDMA-DB		
Additional Component 3 (QTY/MODEL)							
Local Market Note 1	- Follow Antenna and radio positions as per PD.						
Local Market Note 2	- Decom UMTS and remove unused line elements.						
Local Market Note 3	2x850+1x850+1x1.9+1x850+1x850						

PORT SPECIFIC FIELDS	PORT NUMBER	USED (CBS#)	USED (A#B)	ATOLL TXID	ATOLL CELL ID	TURX?	TECHNOLOGY / FREQUENCY	ANTENNA ATOLL	ANTENNA GAIN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	RRH RET MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCF/AMC/PA MODULE?	MATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID(4+4)	
ANTENNA POSITION 2	PORT 1	135348.A.700.4		CTL02287_FA_3	CTL02287_FA_3		LTE 700	N	7198dG_04DT	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					831.1078		5		
	PORT 2	135348.A.1900.4		CTL02287_FA_3	CTL02287_FA_3		LTE 1900	N	1948Mfg_04D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
	PORT 3	135348.A.1900.4		CTL02287_FA_3	CTL02287_FA_3		LTE 1900	N	1948Mfg_04D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
	PORT 4	135348.A.1900.4		CTL02287_FA_3	CTL02287_FA_3		LTE 1900	N	1948Mfg_04D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
	PORT 5	135348.A.1900.4		CTL02287_FA_3	CTL02287_FA_3		LTE 1900	N	1948Mfg_04D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
	PORT 6	135348.A.1900.5		CTL02287_FA_3	CTL02287_FA_3		5G AWS	N	1948Mfg_04D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
	PORT 7	135348.A.1900.5		CTL02287_FA_3	CTL02287_FA_3		5G AWS	N	1948Mfg_04D	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140					3380.6483		4		
ANTENNA POSITION 3	PORT 1						5G CBAND			70	0	Integrated	Fiber	0								5		
ANTENNA POSITION 4	PORT 1	135348.A.700.4		CTL02287_FA_1	CTL02287_FA_1		LTE 700	BURDA_7198dG_04DT	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140						831.1078		7		
	PORT 2	135348.A.850.3		CTL02287_N_05A_1	CTL02287_N_05A_1		5G B50	BURDA_7198dG_04DT	15.3	70	4	Bottom	Cumscope 1-5/8 (700)	140						831.1078		7		

Section 17B - FINAL TOWER CONFIGURATION - SECTOR B

ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
ANTENNA MAKE / MODEL	TPMSR-SURDA-K	NR549 B770-NR549 B770 STACKED	DMP5SR-SURDA			
ANTENNA VENDOR	CCI	Edison	CCI			
ANTENNA SIZE (H x W x D)	30X20.7X7.7	30.4X15.8X8.1	36.0X20.7X7.7			
ANTENNA WEIGHT	87.1	81.8	95.7			
AZIMUTH	190	190	190			
MAGNETIC DECLINATION						
RADIATION CENTER (feet)	111	111	111			
ANTENNA TIP HEIGHT	115	115	115			
MECHANICAL DOWNTILT	0	0	0			
FEEDER ANGLE	4	Fiber	4			
VERTICAL SEPARATION from ANTENNA ABOVE (TP to TP)						
VERTICAL SEPARATION from ANTENNA BELOW (TP to TP)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # it is of)						
Antenna RET Motor (QTY/MODEL)		Built in	Built in	Built in		
SURGE ARRESTOR (QTY/MODEL)	12	TBDC-4310FM	8	TBDC-4310FM		
DUPLEXER (QTY/MODEL)	2	CBC81923T-DS-49	2	DBC0115F1W1-2		
DUPLEXER (QTY/MODEL)						
Antenna RET CONTROL UNIT (QTY/MODEL)						
DC BLOCK (QTY/MODEL)						
TMA/NA (QTY/MODEL)	1	TMA2124F0V5-2D	2	TMA2104F0V1-1		
CURRENT INJECTORS FOR TMA (QTY/MODEL)						
POU FOR TMA3 (QTY/MODEL)						
FILTER (QTY/MODEL)						
SOLID (QTY/MODEL)						
FIBER TRUNK (QTY/MODEL)						
DC TRUNK (QTY/MODEL)						
REPEATER (QTY/MODEL)						
RRH - 700 band (QTY/MODEL)	1	4478 B14	1	4449 B0812 with another band		
RRH - 850 band (QTY/MODEL)						
RRH - 1900 band (QTY/MODEL)	1	8543 B2856A with another band				
RRH - 800S band (QTY/MODEL)						
RRH - WCS band (QTY/MODEL)						
Additional RRH #1 - any band (QTY/MODEL)			1	Integrated with: NR549 B770		
Additional RRH #2 - any band (QTY/MODEL)			1	Integrated with: NR549 B770		
RRH_7B_1 (QTY/MODEL)						
RRH_7B_2 (QTY/MODEL)						
RRH_7B_3 (QTY/MODEL)						
Additional Component 1 (QTY/MODEL)	2	K BBT 782-11055	2	K BBT 782-11055		
Additional Component 2 (QTY/MODEL)	2	APTDC-8CFDM-DB	2	APTDC-8CFDM-DB		
Additional Component 3 (QTY/MODEL)						
Local Market Note 1	- Follow Antenna and radion positions as per PD.					
Local Market Note 2	- Decon UMPS and remove unused line elements.					
Local Market Note 3	2x8550+1x0AMU+DLx4844+Kcde					

PORT SPECIFIC RELOS	PORT NUMBER	USED (CSBmg)	USED (Awb)	ATOLL TXID	ATOLL CELL ID	TWRX?	TECHNOLOGY / FREQUENCY	ANTENNA ATOLL	ANTENNA GAN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	ROUT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPAN/CPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID(s) (mg)		
ANTENNA POSITION 2	PORT 1	135348 B.700.4 0.3mg		CTL02287_7B_3	CTL02287_7B_3		LTE 700	R	719MHz_04DT	15.3	190	4	Bottom	Commscope 1-5/8 (700)	140					931.1078		11			
	PORT 3	135348 B.1900.4 0.3mg		CTL09287_8B_1	CTL09287_8B_1		LTE 1900	R	1848MHz_03D	15.3	190	3	Bottom	Commscope 1-5/8 (700)	140					3380.6483			12		
	PORT 4	135348 B.1900.4 0.3mg		CTL09287_8B_2	CTL09287_8B_2		LTE 1900	R	1848MHz_03D	15.3	190	3	Bottom	Commscope 1-5/8 (700)	140					3380.6483			12		
	PORT 7	135348 B.AW5.4 0.3mg		CTL09287_2B_2	CTL09287_2B_2		LTE AW5	R	1848MHz_03D	15.3	190	3	Bottom	Commscope 1-5/8 (700)	140					3380.6483			12		
	PORT 8	135348 B.AW5.4 0.3mg		CTL09287_2B_2	CTL09287_2B_2		LTE AW5	R	1848MHz_03D	15.3	190	3	Bottom	Commscope 1-5/8 (700)	140					3380.6483			12		
	PORT 11	135348 B.1900.5 0.3mg		CTL09287_8B_1	CTL09287_8B_1		5G 1900	R	1848MHz_03D	15.3	190	3	Bottom	Commscope 1-5/8 (700)	140					3380.6483			12		
	ANTENNA POSITION 3	PORT 1					5G CBAND				190	0	Integrated	Fiber	0									13	
	ANTENNA POSITION 4	PORT 1	135348 B.700.4 0.3		CTL02287_7B_1	CTL02287_7B_1		LTE 700	BURDA_719MHz_04DT	15.3	190	4	Bottom	Commscope 1-5/8 (700)	140						931.1078			15	
		PORT 2	135348 B.850.5 0.3mg		CTL09287_8B_1	CTL09287_8B_1		5G 850	BURDA_719MHz_04DT	15.3	190	4	Bottom	Commscope 1-5/8 (700)	140						931.1078			15	

Section 17C - FINAL TOWER CONFIGURATION - SECTOR C

ANTENNA POSITION 1	ANTENNA POSITION 2	ANTENNA POSITION 3	ANTENNA POSITION 4	ANTENNA POSITION 5	ANTENNA POSITION 6	ANTENNA POSITION 7
ANTENNA MAKE / MODEL	TPMSR-EURO-K	NR549 B770-NR549 B770 STACKED	DMP5R-EURO-A			
ANTENNA VENDOR	CCI	Etteplan	CCI			
ANTENNA SIZE (H x W x D)	80X20.7X7	30.4X15.8X8.1	66.0X20.7X7			
ANTENNA WEIGHT	87.1	81.8	95.7			
AZIMUTH	310	310	310			
MAGNETIC DECLINATION						
RADIATION CENTER (Inch)	111	111	111			
ANTENNA TIP HEIGHT	115	115	115			
MECHANICAL DOWNTILT	0	0	0			
FEEDER ANGLE	4	Fiber	4			
VERTICAL SEPARATION from ANTENNA ABOVE (TP to TP)						
VERTICAL SEPARATION from ANTENNA BELOW (TP to TP)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to LEFT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from CLOSEST ANTENNA to RIGHT (CENTERLINE to CENTERLINE)						
HORIZONTAL SEPARATION from ANOTHER ANTENNA (which antenna # if of interest)						
Antenna RET Motor (QTY/MODEL)		Built in	Built in	Built in		
SURGE ARRESTOR (QTY/MODEL)	12	TBDC-4310FM	8	TBDC-4310FM		
DUPLEXER (QTY/MODEL)	2	CBC81923T-DS-49	2	DBC0115F1W1-2		
DUPLEXER (QTY/MODEL)						
Antenna RET CONTROL UNIT (QTY/MODEL)						
DC BLOCK (QTY/MODEL)						
TMA/NA (QTY/MODEL)	1	TMA2124F03V5-2D	2	TMA2104F00V-1		
CURRENT INJECTORS FOR TMA (QTY/MODEL)						
POU FOR TMA3 (QTY/MODEL)						
FILTER (QTY/MODEL)						
SOLID (QTY/MODEL)						
FIBER TRUNK (QTY/MODEL)						
DC TRUNK (QTY/MODEL)						
REPEATER (QTY/MODEL)						
RRH - 700 band (QTY/MODEL)	1	4478 B14	1	4449 B0812		
RRH - 850 band (QTY/MODEL)				with another band		
RRH - 1900 band (QTY/MODEL)	1	8543 B2856A				
RRH - 8WS band (QTY/MODEL)				with another band		
RRH - WCS (QTY/MODEL)						
Additional RRH #1 - any band (QTY/MODEL)			1	Integrated with: NR549 B770		
Additional RRH #2 - any band (QTY/MODEL)			1	Integrated with: NR549 B770		
RRH_7B_1 (QTY/MODEL)						
RRH_7B_2 (QTY/MODEL)						
RRH_7B_3 (QTY/MODEL)						
Additional Component 1 (QTY/MODEL)	2	K BBT 782-11055	2	K BBT 782-11055		
Additional Component 2 (QTY/MODEL)	2	APTDC-8CFDM-DB	2	APTDC-8CFDM-DB		
Additional Component 3 (QTY/MODEL)						
Local Market Note 1	- Follow Antenna and radion positions as per PD.					
Local Market Note 2	- Decon UMPS and remove unused line elements.					
Local Market Note 3	2x8530-1x0MJ+DLx4943+Kcode					

PORT SPECIFIC RELOS	PORT NUMBER	USED (CSBmg)	USED (Awb)	ATOLL TXID	ATOLL CELL ID	TWRX?	TECHNOLOGY / FREQUENCY	ANTENNA ATOLL	ANTENNA GAN	ELECTRICAL AZIMUTH	ELECTRICAL TILT	RRH LOCATION (Top/Bottom/Integrated/None)	FEEDERS TYPE	FEEDER LENGTH (feet)	ROUT KIT MODULE?	TRIPLEXER or LLC (QTY)	TRIPLEXER or LLC (MODEL)	SCPAN/CPA MODULE?	HATCHPLATE POWER (Watts)	ERP (Watts)	Antenna RET Name	CABLE NUMBER	CABLE ID(s) (mg)		
ANTENNA POSITION 2	PORT 1	135348 C.700.4 0.3mg		CTL02287_7C_1	CTL02287_7C_1		LTE 700	R 719MHz 04DT	15.3	310	4	Bottom	Commscope 1-5/8 (700)	140						931.1078		19			
	PORT 3	135348 C.1900 40.3mg		CTL06287_9C_1	CTL06287_9C_1		LTE 1900	R 1848MHz 03D	15.3	310	3	Bottom	Commscope 1-5/8 (700)	140						3380.6483		20			
	PORT 4	135348 C.1900 40.3mg		CTL06287_9C_2	CTL06287_9C_2		LTE 1900	R 1848MHz 03D	15.3	310	3	Bottom	Commscope 1-5/8 (700)	140						3380.6483		20			
	PORT 7	135348 C.AWB.4 0.3mg		CTL06287_9C_2	CTL06287_9C_2		LTE AWB	R 1848MHz 03D	15.3	310	3	Bottom	Commscope 1-5/8 (700)	140						3380.6483		20			
	PORT 8	135348 C.AWB.5 0.3mg		CTCN02287_N 005C_1	CTCN02287_N 005C_1		5G AWB	R 1848MHz 03D	15.3	310	3	Bottom	Commscope 1-5/8 (700)	140						3380.6483		20			
	PORT 11	135348 C.1900 50.3mg		CTCN02287_N 005C_1	CTCN02287_N 005C_1		5G 1900	R 1848MHz 03D	15.3	310	3	Bottom	Commscope 1-5/8 (700)	140						3380.6483		20			
	ANTENNA POSITION 3	PORT 1						5G CBAND			310	0	Integrated	Fiber	0									21	
	ANTENNA POSITION 4	PORT 1	135348 C.700.4 0.3		CTL02287_7C_1	CTL02287_7C_1		LTE 700	BURDA_719MHz 04DT	15.3	310	4	Bottom	Commscope 1-5/8 (700)	140						931.1078		23		
PORT 2		135348 C.850.5 0.3mg		CTCN02287_N 005C_1	CTCN02287_N 005C_1		5G 850	BURDA_719MHz 04DT	15.3	310	4	Bottom	Commscope 1-5/8 (700)	140						931.1078		23			



- Eight foot (2.4 m) internally multiplexed MultiBand antenna, including eight external RF ports (12 RF ports internal), with a 65° azimuth beamwidth covering 698-896 MHz and 1695-2400 MHz frequencies
- Four wide high band ports covering 1695-2400 MHz and four wide low band ports covering 698-896 MHz in a single antenna enclosure
- Innovative Multiplexed/RET Control configuration, supporting Dual Band Radio Configurations (B12/B5 and B29/B5). The antenna provides Dual 4T4R (4x4 MIMO) capability, while providing independent RET control, an Industry First
- Innovative Low and High Band Array configuration allows for 4T4R (4x4 MIMO) on Low Band and 4T4R (4x4 MIMO) High Band Arrays, using full length arrays (non stacked), all in a 20.7" (525 mm) width enclosure, an Industry First
- Industry leading antenna topology and RET shielding techniques drastically mitigate PIM propagation from B12/B14/B29 operations, allowing for superior Network performance
- Full Spectrum Compliance for PCS, AWS-3 and WCS frequencies and 700/850 MHz Dual Band Radio Configurations
- LTE Optimized FBR and SPR performance, providing for an efficient use of valuable radio capacity
- LTE Optimized Boresight and Sector XPD and USL performance, essential for LTE Performance
- Exceeds minimum PIM performance requirements
- Equipped with new 4.3-10 connector, which is 40% smaller than traditional 7/16 DIN connector
- Ordering options for External RET Controllers (Type 1) or Internally Integrated RET Controllers (Type 17)

### Overview

The CCI internally multiplexed MultiBand array is an eight port (12 RF ports internal) antenna, with four wide band ports covering 1695-2400 MHz and four low band ports covering 698-896 MHz. The antenna provides the capability to deploy 4T4R (4x4 MIMO) in the high band, with separate RET control. The antenna also provides the capability to provide independent RET control for 700/850 MHz Dual Band Radio Configurations, while maintaining 4T4R (4x4 MIMO) across the low band ports.

CCI antennas are designed and produced to ISO 9001 certification standards for reliability and quality in our state-of-the-art manufacturing facilities.

### Applications

- 4x4 MIMO for the High Band and 4X4 MIMO Low Band ports
- Ready for Network Standardization on 4.3-10 DIN connectors
- With CCI's multiband antennas, wireless providers can connect multiple platforms to a single antenna, reducing tower load, lease expense, deployment time and installation costs



SPECIFICATIONS

Diplexed Multi-Band Antenna

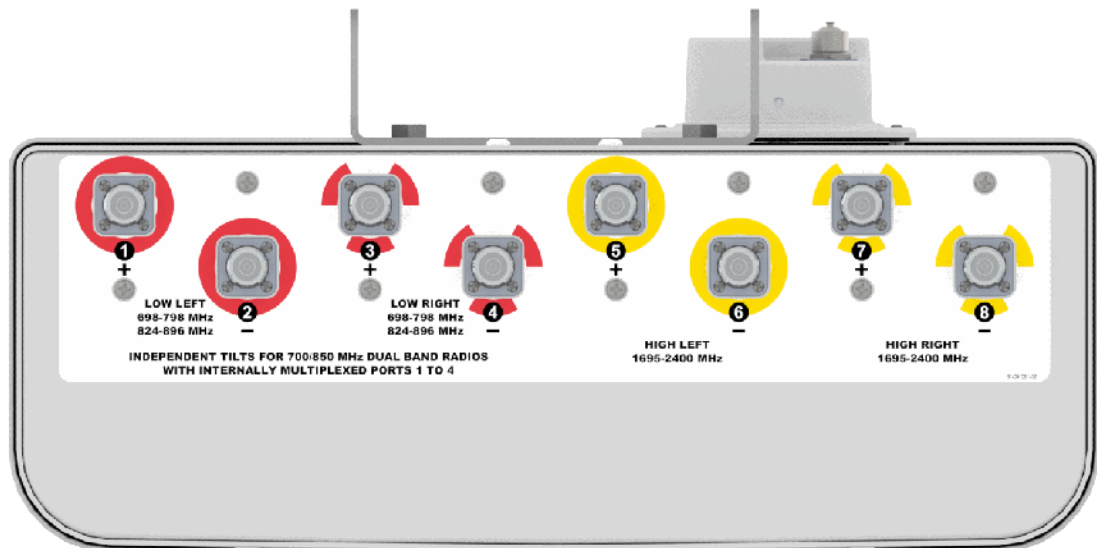
DMP65R-BU8D

Mechanical

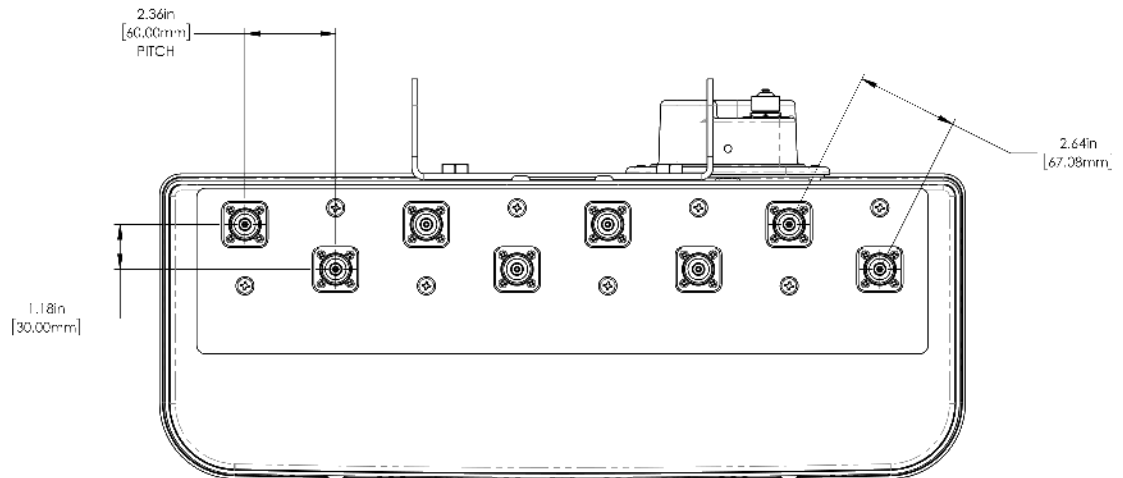
Dimensions (LxWxD)	96.0x20.7x7.7 in (2438x525x197 mm)
Survival Wind Speed	> 150 mph (> 241 kph)
Front Wind Load	457 lbs (2033 N) @ 100 mph (161 kph)
Side Wind Load	209 lbs (929 N) @ 100 mph (161 kph)
Equivalent Flat Plate Area	17.9 ft <sup>2</sup> (1.7 m <sup>2</sup> )
Weight *	95.7 lbs (43.4 kg)
Connector	8 x 4.3-10 female
Mounting Pole	2 to 5 in (5 to 12 cm)

\* Weight excludes mounting

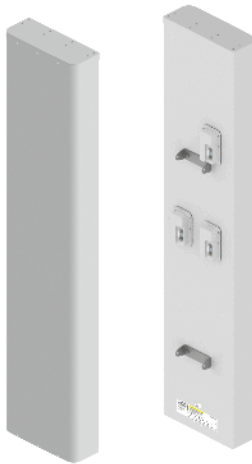
Bottom View



Connector Spacing







- Eight foot (2.4 m) multiband, twelve port antenna with a 65° azimuth beamwidth covering 698-896 MHz and 1695-2400 MHz frequencies
- Eight high band ports covering 1695-2400 MHz and four low band ports covering 698-896 MHz in a single antenna enclosure
- Innovative Low and High Band Array configuration allows for 4T4R (4x4 MIMO) on Low Band and Dual 4T4R (4x4 MIMO) High Band Arrays, using full length arrays (non stacked), all in under a 20.7" (525 mm) width enclosure, an Industry First
- Full Spectrum Compliance for WCS and AWS-3 frequencies and Band 14 Operations
- Array configuration allows for 4T4R (4X4 MIMO) on Low Band, essential for Band 14 Operations
- LTE Optimized FBR and SPR performance, providing for an efficient use of valuable radio capacity
- LTE Optimized Boresight and Sector XPD and USL performance, essential for LTE Performance
- Exceeds minimum PIM performance requirements
- Equipped with new 4.3-10 connector, which is 40% smaller than traditional 7/16 DIN connector
- Equipped with 3 field replaceable, integrated AISG 2.0 compliant Remote Electrical Tilt (RET) Controllers (Type 1 External)
- Ordering options for External RET Controllers (Type 1) or Internally Integrated RET Controllers (Type 17)

### Overview

The CCI 12-Port multiband array is a twelve port antenna, with eight wide band ports covering 1695-2400 MHz and four low band ports covering 698-896 MHz. The antenna provides the capability to deploy Dual 4x4 Multiple-input Multiple-output (MIMO) in the high band and 4X4 Multiple-input Multiple-output (MIMO) across low band ports. The CCI 12-Port allows independent tilt control between the low band ports and high band ports and independent tilt control between left and right antenna arrays.

In this three RET configuration, the 1st RET is dedicated for the four Low Band ports. The 2nd RET is dedicated for the four Left High Band ports and the 3th RET is dedicated for the four Right High Band ports. This RET arrangement allows for complete flexibility in coverage control between left and right antenna arrays.

CCI antennas are designed and produced to ISO 9001 certification standards for reliability and quality in our state-of-the-art manufacturing facilities.

### Applications

- Dual 4x4 MIMO for the High Band and 4X4 MIMO Low Band ports
- Ready for Network Standardization on 4.3-10 DIN connectors
- With CCI's multiband antennas, wireless providers can connect multiple platforms to a single antenna, reducing tower load, lease expense, deployment time and installation costs



SPECIFICATIONS

Multi-Band Twelve-Port Antenna

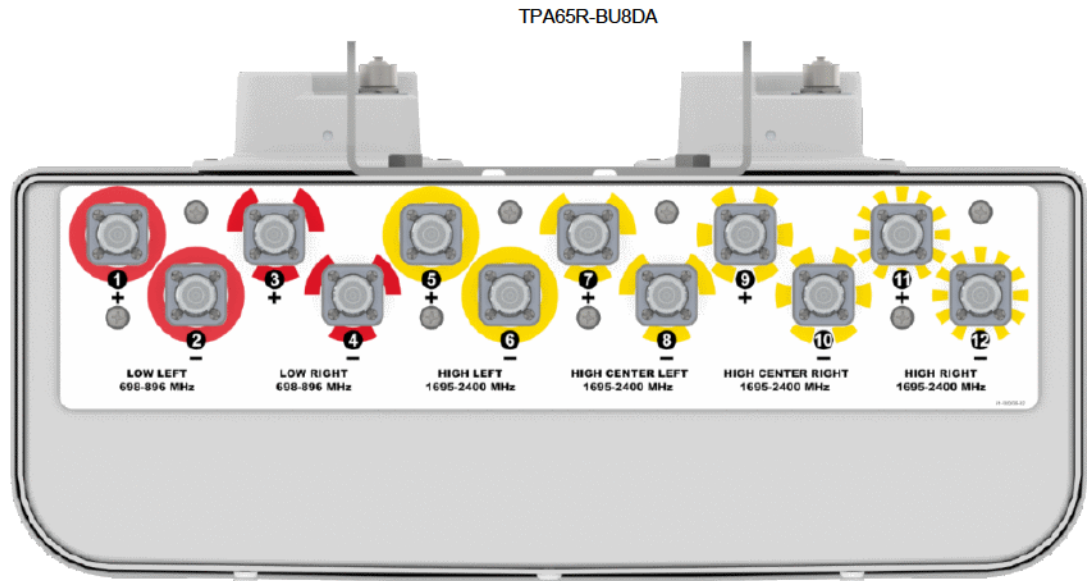
TPA65R-BU8D

Mechanical

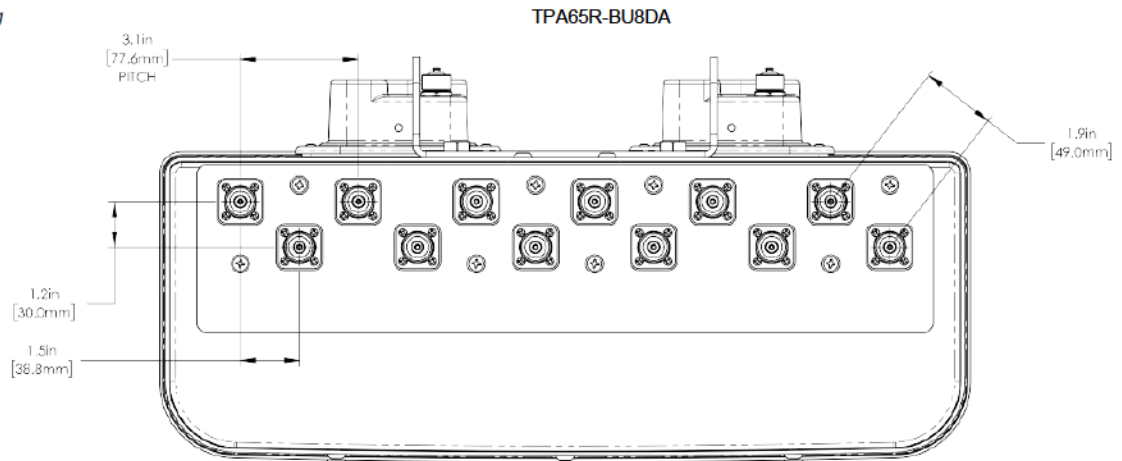
<b>Dimensions (LxWxD)</b>	96.0x20.7x7.7 in (2438x525x197 mm)
<b>Survival Wind Speed</b>	> 150 mph (> 241 kph)
<b>Front Wind Load</b>	457 lbs (2033 N) @ 100 mph (161 kph)
<b>Side Wind Load</b>	209 lbs (929 N) @ 100 mph (161 kph)
<b>Equivalent Flat Plate Area</b>	17.9 ft <sup>2</sup> (1.7 m <sup>2</sup> )
<b>Weight *</b>	87.1 lbs (39.5 kg)
<b>Connector</b>	12 x 4.3-10 female
<b>Mounting Pole</b>	2 to 5 in (5 to 12 cm)
<b>Package Dimensions (LxWxD)</b>	104.3x28.7x16.9 in (2650x730x430 mm)
<b>Package Weight</b>	145 lbs (65.8 kg)

\* Weight excludes mounting kit

Bottom View



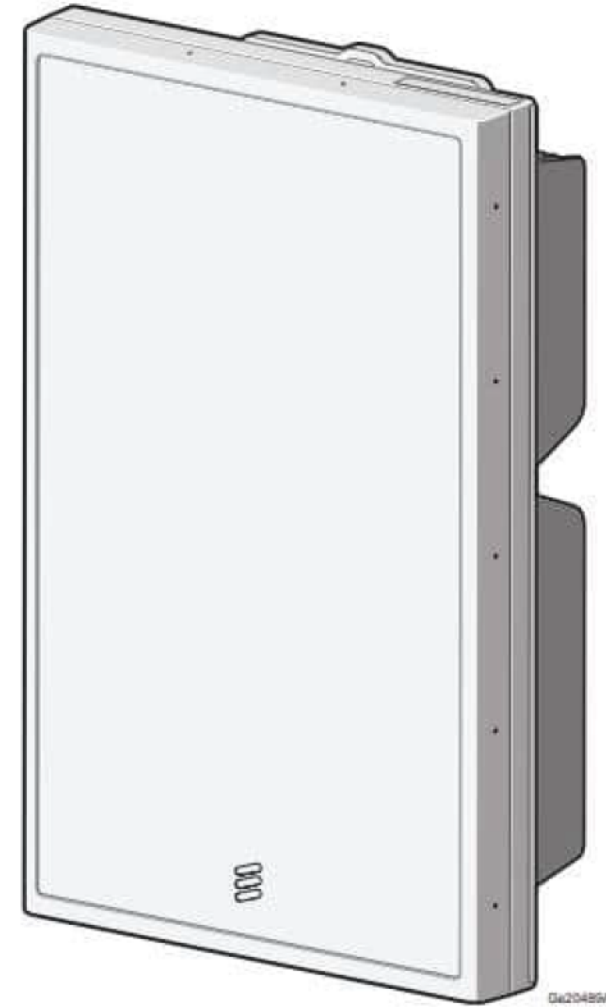
Connector Spacing



# ERICSSON AIR 6419 B77G



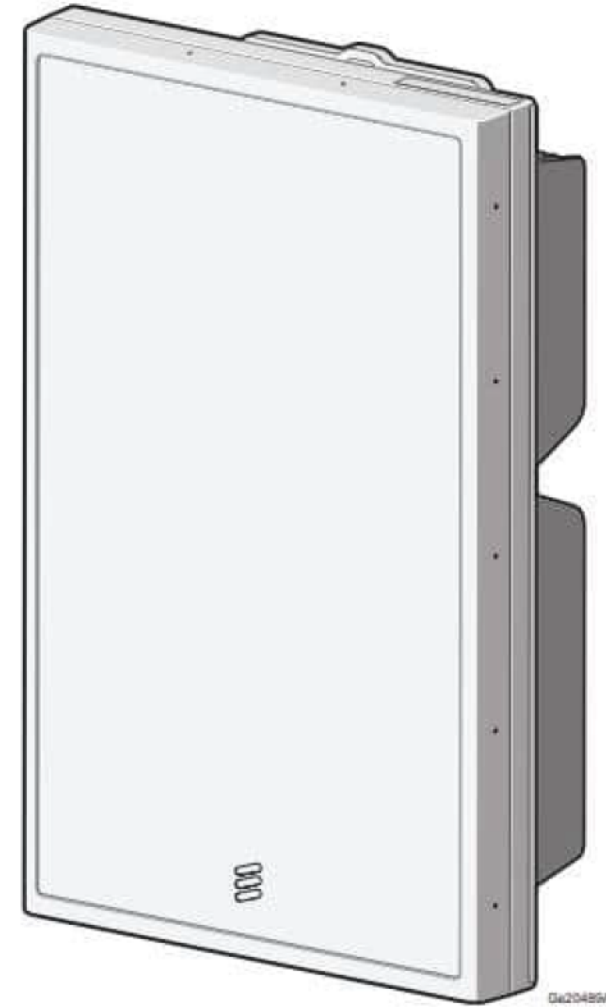
- › ERICSSON AIR 6419 has a total of **2** ECPRI connections @ 25.8 Gbps, 1 DC Power cable connection
- › Operates over B77G DOD band (3.4-3.6 GHz)
- › Breaker size = **45A** DC, DC Power Consumption = **1280W** (for dimensioning)
- › Dimensions
  - Height: 31.1" (790 mm)
  - Width: 16.1" (408 mm)
  - Depth: 7.3" (186 mm)
- › Weight, excl. mounting hardware = **44 lbs (20 kg)**
- › Weight with Mounting Hardware = **55.4 lbs (25.2 kg)**
- › Max Frontal Wind Load @ 42m/s = **454 N**
- › Horizontal Separation Required between AIR 6419 = **100mm**
- › Minimum Vertical Space Required below/above AIR 6419 = **300mm**
- › Minimum Height Above Users = **5m**
- › Outdoor Installation locations to avoid:
  - Hot microclimates caused by, for example, heat radiated or reflected from dark or metallic walls or floors
  - Chimney mouths or ventilation system outlets
  - In front of Large glass surfaces or concrete surfaces
- › Avoid radio interference by keeping the area directly in front of the antenna clear of metal surfaces such as railing, ladders or chains or equipment generating electromagnetic fields, for example, electric motors in air conditioners or diesel generators in front of antenna
- › Do not use metallic paint to cover the AIR 6419 If painting is required. Do not paint underside of AIR 6419.



# ERICSSON AIR 6449 B77



- › ERICSSON AIR 6449 has a total of 4 ECPRI connections @ 25 Gbps
- › Operates over B77 band (3.3-4.2 GHz)
- › Breaker size = 50A DC, DC Power Consumption = **1280W (for dimensioning)**
- › Dimensions
  - Height: 30.6" (778 mm)
  - Width: 15.9" (403 mm)
  - Depth: 10.6" (268 mm)
- › Weight, excl. mounting hardware = **82.5 lbs (37.5 kg)**
- › Weight with Mounting Hardware = **95.5 lbs (43.4 kg)**
- › Max Frontal Wind Load @ 42m/s = **478 N**
- › Horizontal Separation Required between AIR 6449 = **100mm**
- › Minimum Vertical Space Required below AIR 6449 = **300mm**
- › Minimum Height Above Users = **5m**
- › Outdoor Installation locations to avoid:
  - Hot microclimates caused by, for example, heat radiated or reflected from dark or metallic walls or floors
  - Chimney mouths or ventilation system outlets
  - In front of Large glass surfaces or concrete surfaces
- › Avoid radio interference by keeping the area directly in front of the antenna clear of metal surfaces such as railing, ladders or chains or equipment generating electromagnetic fields, for example, electric motors in air conditioners or diesel generators in front of antenna
- › Do not use metallic paint to cover the AIR 6449 If painting is required. Do not paint underside of AIR 6449.



# TMA2124F03V5-2D

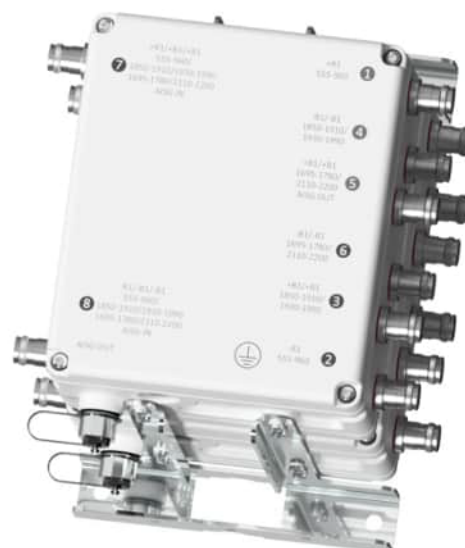
## DUAL TWIN TMA 1900/AWS/LOWPASS 555-960MHZ 6 ANT

### NON-DIPLEXED 1900/AWS ANTENNA PORTS

Designed to be deployed in co-located AWS & 1900 networks, the Kaelus TMA2124 provides gain in 1900 and AWS uplink, using independent LNAs per band and per channel. Low loss bypass 555-960MHz signal to low band antennas is also provided.

### FEATURES

- Improved base station sensitivity through excellent noise figure performance and linearity
- AISG 2.0 compatible, full software upgradable using AISG "personality" upload
- DC/AISG passthrough to AWS antenna (port 5)
- AISG OUT connector disabled when AISG device (SBT equipped antenna) present on Port 3 +R1/+R1
- One AISG subunit per LNA, 4 in total. All fixed gain
- 555-960 bypass to low band antenna



### TECHNICAL SPECIFICATIONS

BAND NAME	1900	AWS
<b>DOWNLINK</b>		
Passband	1930 - 1990MHz	2110 - 2200MHz
Insertion loss	0.4dB typical	0.3dB typical
Return loss	22dB typical	
Maximum input power	160W (average) / 2kW (PEP)	160W (average) / 2kW (PEP)
Intermodulation products	-155dBc maximum, at antenna port in RX band with 2 x 20W carriers	-163dBc maximum, at antenna port in RX band with 2 x 20W carriers
<b>UPLINK</b>		
Passband	1850 - 1910MHz	1695 - 1780MHz
Gain	13dB	
Gain variation	±1dB maximum	
Return loss	22dB typical	
Bypass return loss	14dB typical	
Bypass loss	3dB typical	
Noise figure	1.2dB typical @ 13dB gain	1.0dB typical @ 13dB gain
Output IP3	+28dBm typical	
Maximum input power with no damage	+12dBm	
<b>555-960 LOWPASS FILTER</b>		
Passband	555 - 960MHz	
Insertion loss	0.2dB typical	
Return loss	21dB typical	
Maximum input power	250W (average) / 2.5kW (PEP)	
Intermodulation products	-155dBc maximum, at antenna port with 2 x 20W carriers	
<b>ELECTRICAL</b>		
Impedance	50Ohms	

### POWER SUPPLY AND ALARM (CURRENT WINDOW ALARM MODE, DEFAULT)

Current window alarm mode (CWA) is the default operating mode and can be configured to specific customer requirements. The TMA2124F03V4 is configured so that both channels are independently powered and monitored via their respective BTS port, 7 or 8. The BTS port sinks additional current to indicate an alarm state in its uplink path. Normal operating and alarm current values are configured independently via a field-loadable personality file. Please contact Kaelus for more information.

DC supply voltage	+8.5 to +18V DC, case is DC ground
DC supply	Each BTS port powered individually
DC supply current, normal mode	200mA per port typical (both ports are powered)
DC supply current, alarm mode	300mA per port typical (both ports are powered)

### AISG MODE OF OPERATION (AUTO SELECTED ON VALID AISG 2.0 FRAMES)

AISG signals can be applied to port 7 or port 8. The TMA unit switches to AISG mode when valid frames are detected on either port 7 or 8. All LNAs take DC power from the port with the AISG frames or, if DC is present on both ports, power will be supplied equally between the ports. Each LNA is controlled uniquely by its sub-unit number.

DC supply voltage	+7.5V to +30V DC
AISG version	2.0 (1.1 optional)
Supply current, AISG mode	500mA @ 7.5V, 135mA @ 30V typical
AISG connector, current rating	IEC60130-9, 8-pin female, < 4A peak, 2A continuous, pin 6
Field firmware upgradable	Yes (R951022ATA2.0 Rev 2.9.12)
AISG pass through to antenna port	Yes

### ANTENNA AISG OOK + DC

When DC is applied it is quickly switched through to port 5. If an over-current condition is detected, DC & AISG are disconnected from port 5. If DC remains connected to the load at port 5, DC and AISG are disconnected from the AISG OUT 8 pin connector. If DC is disconnected from port 5, DC and AISG are enabled at the AISG OUT 8 pin connector. If a short circuit is detected at the AISG OUT 8 pin connector, DC and AISG are disabled.

Mode of Operation	Voltage at Port 5	Assumption	"Autosense + Protection" Switch Status	Comment
AISG or CWA	High	Device present or open circuit	Close	DC & AISG OOK will be supplied to port 5. DC & AISG is removed from the AISG OUT 8 pin port
AISG or CWA	Low	DC short circuit or low DC resistance	Open	DC & AISG OOK will not be supplied to port 5. DC & AISG are supplied to the AISG OUT 8 pin port

### ENVIRONMENTAL

For further details of environmental compliance, please contact Kaelus.

Temperature range	-40°C to +65°C   -40°F to +149°F
Ingress protection	IP67
Altitude	3,000m   10,000ft
Lightning protection	IEC61312-1, RF: ±5kA maximum (8/20us), AISG: ±2kA maximum (8/20us)
MTBF	>1,000,000 hours
Compliance	FCC Part 15 subpart B

### MECHANICAL

Dimensions H x D x W (single unit)	245 x 263 x 210mm   9.7 x 10.4 x 8.3in excluding brackets and connectors
Weight	16.2kg   35.71lbs
Finish	Painted, light grey (RAL 7035)
Connectors	4.3-10 (F) x 16 long neck, AISG (F) x 2
Wind Load	Front 390N, Side 147N (Single) Front 251N, Side 409N (Twin) At 74m/s (AS/NZS 1170-2-2011 Structural design - Wind actions - Cyclone areas)
Mounting	Pole/wall bracket supplied with two metal clamps 45-178mm diameter poles

### ORDERING INFORMATION

PART NUMBER	CONFIGURATION	OPTIONAL FEATURES	CONNECTORS
TMA2124F03V5-1D	TWIN 2 in / 6 out	STANDARD	4.3-10 (F)
TMA2124F03V5-2D	QUAD 4 in / 12 out	STANDARD	4.3-10 (F)
TMA2124F03V5-1S	TWIN 2 in / 6 out	STANDARD	4.3-10 (F)
TMA2124F03V5-2S	QUAD 4 in / 12 out	STANDARD	4.3-10 (F)

# TMA2104F00V1-1

## TWIN TMA 2300, LOWPASS

Designed to be deployed in 2300MHz WCS Band, the Kaelus TMA2104 provides gain at the 2300MHz uplink band while allowing 700/850 services to pass through to a low-band antenna. This saves OPEX, CAPEX and gives excellent system performance as well as full AISG capabilities.

### FEATURES

- Improved base station sensitivity through gain in 2300MHz bands
- High linearity and low noise performance
- Bypass provided for 700/850 MHz services
- Hardware and software configuration using AISG "Personality" upload
- Fail safe bypass mode with lightning protection



### TECHNICAL SPECIFICATIONS

2300MHZ DOWNLINK (TX) PATH	
Passband	2350 - 2360MHz
Return loss	18dB minimum
Insertion loss	0.5dB typical
Maximum input power with no damage	120W (average) / 1.2kW (PEP)
Intermodulation @ antenna ports	-153dBc maximum in RX band with 2 x 20W carriers
2300MHZ UPLINK AWS (TX) PATH	
Passband	2305 - 2315MHz
Nominal gain	12dB
Gain variation over frequency, temperature	± 1dB maximum
Noise figure	1.7dB typical
Return loss	18dB minimum operating, 15dB minimum in bypass mode
Bypass loss	3.3dB typical
Output IP3	+30dBm typical
Maximum input power with no damage	12dBm maximum
Rejection Rx in 2324.54 - 2341.285MHz	27.5dB minimum
Rejection Rx in 698 - 2170MHz	75dB minimum
LOW BAND PATH	
Passband	698 - 894MHz
Return loss	18dB minimum
Insertion loss	0.35dB typical
Maximum input power with no damage	200W (average) / 2kW (PEP)
Intermodulation @ antenna ports	-153dBc maximum in RX band with 2 x 20W carriers
ELECTRICAL	
Impedance	50ohms

POWER SUPPLY AND ALARM (CURRENT WINDOW ALARM MODE, DEFAULT)	
Current window alarm mode (CWA) is the default TMA operating mode and can be configured to specific customer requirements. The generic personality (F00V1) is configured so that both channels are independently powered and monitored via the respective BTS port. The BTS port sinks additional current to indicate an alarm state in its uplink path. Normal operating and alarm current values are configured independently via a field-loadable personality file, Please contact Kaelus for more information.	
DC supply voltage	8.5V to 30V DC, case is DC ground
DC supply	Each BTS powered individually (programmable)
DC supply current, normal mode	100 ± 20mA per port (programmable)
DC supply current, alarm mode programmable range	180 ± 30mA per port

AISG MODE OF OPERATION (AUTO SELECTED ON VALID AISG 2.0 FRAMES)	
AISG signals can be applied to either BTS1 or BTS2 ports. The TMA2104FxxVx-1 unit switches to AISG mode when valid frames are detected on one of the BTS ports. Both LNAs take DC power from the port with AISG frames or, if DC is present on both ports, both channels supply equal power to the TMA2104FxxVx-1.	
DC supply voltage	+8V to +30V DC
AISG version	2.0 (1.1 optional)
Supply current, AISG mode	215mA @ 8.5V, 70mA @ 30V typical
AISG connector, current rating	IEC60130-9, 8-pin female, < 4A peak, 2A continuous, pin 6
Field firmware upgradable	Yes
AISG pass through to antenna port	Yes (optional)

ENVIRONMENTAL	
For further details of environmental compliance, please contact Kaelus.	
Temperature range	-40° to +65°C   -40° to +149°F
Ingress protection	IP67
Altitude	2,000m   6,561ft
Lightning protection	IEC61312-1, RF: ±5kA maximum (8/20us), AISG: ±2kA maximum (8/20us)
MTBF	>1,000,000 (hours)
Compliance	FCC part 15, ETSI EN 300 019 class 4.1, RoHS

MECHANICAL	
Dimensions H x D x W	220 x 200 x 104mm   8.7 x 7.9 x 4.1in (excluding brackets and connectors)
Weight	7.7kg   17lbs
Finish	Painted, light grey (RAL7035)
Connectors	DIN 7-16 (F) x 6 long shank, AISG (F) x 1
Mounting	Pole/wall bracket supplied with two metal clamps 45-178mm diameter poles

## ORDERING INFORMATION

PART NUMBER	CONFIGURATION	OPTIONAL FEATURES	CONNECTORS
TMA2104F00V1-1	TWIN 2 in / 4 out	Generic CWA configuration	DIN 7-16 (F)
TMA2104F00V1-2	Quad 4 in / 8 out	Generic CWA configuration	DIN 7-16 (F)



**Rooftop / Towntop**

The DC6-48-60-18-8C-EV is designed to provide the ultimate coordination between the SPD and the RRH/RRU by offering industry-leading low-clamping voltage of 160V and extremely robust protection for use in a high DC voltage environment. Capable of providing 12.5kA (10/350  $\mu$ s) max per circuit surge capacity for up to 6 -48V DC circuits.

powered by  
**Strikesorb®**

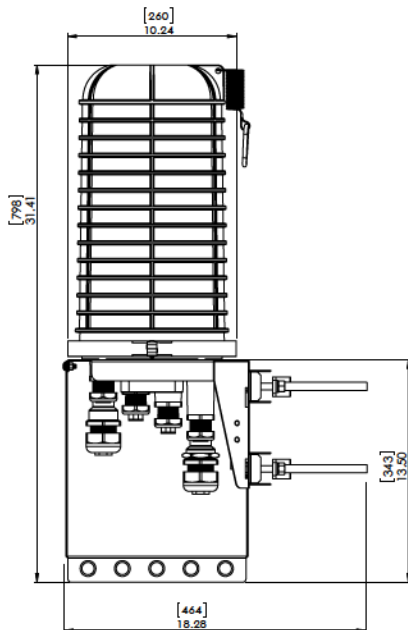


**Features**

- Provides discrete protection for six individual -48V DC circuits
- Surge protection of 90kA 8/20  $\mu$ s
- Maximum impulse current 12.5kA 10/350  $\mu$ s
- Fiber connections for up to 18 fiber pair
- Simplifies inter-connectivity and cable management for DC conductors
- UL 1449 4th Edition Type 2 protective device
- IEC 61643-11 Class I protection for DC applications
- Form C relay contacts included, allowing remote monitoring of suppressor status
- Copper-coated lid to reduce power line interference
- Patented design
- Patented Strikesorb technology ensures lowest let-through voltage available in the industry, providing enhanced coordination with the RRH/RRU
- Raycap recommends that DC protection system be installed within 5 meters of the radio

**Benefits**

- Strikesorb modules are fully recognized to UL 1449 4th Edition, and IEC 61643-11 Safety Standards, meeting all intermediate and high current fault requirements to facilitate use in original equipment manufacturers (OEM) applications
- Strikesorb offers unique maintenance-free protection against direct lightning currents
- Design provides maximum flexibility for installation
- NEMA 4X enclosure allows for indoor or outdoor installation



Strikesorb is a registered trademark of Raycap  
© 2018 Raycap All rights reserved.  
G02-01-203 180122

**SPECIFICATIONS**

**DC Surge Protection Solutions**

**DC6-48-60-18-8C-EV**

**Overvoltage Protection and Fiber Distribution/Cable Management Solution**

powered by

**Strikesorb®**

**Electrical**

Model Number	DC6-48-60-18-8C-EV	
CEQ / ANT Number	CEQ.18537	
Number of Circuits Protected	6	
Surge Protective Device (SPD) Type per UL 1449 4th Edition	Type 2	
Surge Protection Class as per IEC 61643-11	Class I	
Nominal Operating DC Voltage [U <sub>n</sub> ]	48 V	
Nominal Discharge Current [I <sub>n</sub> ] per UL 1449 4th Edition	20kA 8/20 μs	
Maximum Surge Current [I <sub>max</sub> ] per IEC 61643-11	90kA 8/20 μs	
Maximum Impulse (Lightning) Current [I <sub>imp</sub> ] per IEC 61643-11	12.5kA 10/350 μs	
Maximum Continuous Operating DC Voltage [U <sub>c</sub> ] (MCOV)	60 VDC	
Voltage Protection Level [U <sub>p</sub> ] per IEC 61643-11	160 V	
Voltage Protection Rating (VPR) per UL 1449 4th Edition	330 V	
Suppression Technology	MOV	
Strikesorb Module Type 2CA (UL 1449 4th edition)	30-V1-EV	
Protection Modes:	Normal Mode	-48V to Return
	Common Mode	Return to Ground

**Mechanical**

Connection Terminal (Alarm) Method	Form C Hardwired, #22 to #12 AWG [0.34 to 4 mm <sup>2</sup> ]	
Connection Terminal (Suppression) Method (for all power cables)	Compression lug 2 hole, #10, 5/8 pitch, #12 – #4 AWG [3.3 – 21.15 mm <sup>2</sup> ]	
Connection Terminal (Terminal Block) Method	Copper	#12 to #4 AWG [3.3 – 21.15 mm <sup>2</sup> ]
Fiber Connection Method	LC-LC Single Mode	
Environmental Ingress Protection (IP) Rating	IP 68	
Operating Temperature (°C)	-40° C to +100° C	
Storage Temperature (°C)	-70° C to +80° C	
Cold Temperature Cycling IEC 61300-2-22	-30° C to +60° C 200 hrs @5 PSI	
Resistance to Aggressive Materials CEI IEC 61073-2	Including Acids and Bases	
UV Protection ISO 4892-2 Method A	Xenon-Arc 2160 hrs	
Enclosure Type	Outdoor NEMA 4X	
Enclosure Dimensions (L x W x H)	18.28" x 10.24" x 31.4" [464 x 260 x 797 mm]	
Weight*	System: 16.0 lbs [7.25 kg] Mount: 10.2 lbs [4.62 kg] Total: 26.2 lbs [11.87 kg]	
Combined Wind Loading	Sustained	150 mph Sustained: 105.7 lbs [470 N]
	Gust	195 mph Gust: 213.6 lbs [950 N]

**Standards Compliance & Certifications**

NEBS certified to:	GR-63-CORE Issue 4, GR-1089-CORE Issue 6, GR-3108-CORE Issue 3, GR-487-CORE Issue 4, ATT-TP-76200 Issue 18
Strikesorb modules are compliant to the following Surge Protection Device Standards:	
Standards:	UL 1449 4th Edition: 2011, IEC 61643-11: 2011, EN 61643-11: 2012, IEEE C62.11: 2005, IEEE C62.41: 2002, IEEE C62.45: 2002, NEMA-LS-1
Certifications:	UL, VDE, CE

AWG=American Wire Gauge



**Raycap**

www.raycap.com

G02-01-203 180122

January 21, 2022  
February 16, 2024 (Rev.1)



SAI Communications  
12 Industrial Way  
Salem NH, 03079

RE: AT&T Site Number: CT2267 (C-BAND)  
FA Number: 10141359  
PACE Number: MRCTB051476  
PT Number: 2051A0Z7QX  
TEP Project Number: 316500.928356  
AT&T Site Name: ORANGE CT RIDGE ROAD  
Site Address: 298 Ridge Road  
Orange, CT 06477

To Whom It May Concern:

TEP Northeast (TEP NE) has been authorized by SAI Communications to perform a mount analysis on the existing AT&T antenna mount to determine their capability of supporting the following loading:

- **(3) TPA65R-BU8DA-K Antennas (96.0"x20.7"x7.7" – Wt. = 87 lbs. /each)**
- **(3) AIR6449 Antennas (30.6"x15.9"x10.6" – Wt. 84 lbs. /each)**
- **(3) AIR6419 Antennas (31.2"x16.1"x9.1" – Wt. = 66 lbs. /each)**
- **(3) DMP65R-BU8DA Antennas (96.0"x20.7"x7.7" – Wt. = 119 lbs. /each)**
- **(3) TMA2124F03V5-2D TMA's (10.4"x9.7"x8.3" – Wt. = 36 lbs. /each) (Pos.2)**
- **(6) TMA2104F00V1-1 TMA's (8.7"x7.9"x4.1" – Wt. = 8 lbs. /each) (Pos.4)**
- **(1) DC6-48-60-18 Surge Arrestor (31.4"x10.2"Ø – Wt. = 29 lbs. /each) (Standoff)**

*\*Proposed equipment shown in bold.*

Fabrication drawings prepared Sitepro1, P/N RMV12-496, dated July 2, 2015, were available for the existing mount. TEP NE conducted a ground audit of the existing antenna mounts on March 29, 2022.

Mount Analysis Methods:

- This analysis was conducted in accordance with EIA/TIA-222-H, Structural Standards for Steel Antenna Towers and Antenna Supporting Structures, the International Building Code 2021 with 2022 Connecticut State Building Code, and AT&T Mount Technical Directive – R22.
- TEP NE considers this mount to be asymmetrical and has applied wind loads in 30 degree increments all around the mount. Per TIA-222-H and Appendix P of the Connecticut State Building Code, the max basic wind speed for this site is equal to 130 mph with a max basic wind speed with ice of 50 mph and a max ice thickness of 1.0 in. An escalated ice thickness of 1.30 in was used for this analysis.
- TEP NE considers this site to be exposure category C; tower is located near large, flat, open, terrain/grasslands.
- TEP NE considers this site to be topographic category 1; tower is located on flat terrain or the bottom of a hill or ridge.
- TEP NE considers this site to have a spectral response acceleration parameter at short periods,  $S_s$ , of 0.201 and a spectral response acceleration parameter at a period of 1 second,  $S_1$ , of 0.054.
- AT&T policy forbids walking on or suspending below T-arm mounts. This analysis does not include live load conditions for this mount.
- The existing mount is secured to the existing pole extension with ring mounts and threaded rods. TEP NE considers the threaded rods to be the governing connection member.

Based on our evaluation, we have determined that the existing mount **IS NOT CAPABLE** of supporting the proposed installation. TEP NE recommends the following modifications:

- **Remove existing antenna pipe mast and install proposed 2-1/2" std. (2.88" O.D.) pipe mast secured to the existing mount and handrail (typ. of 3 per sector, total of 9).**
- **Install proposed 2" std. (2.38" O.D.) pipe brace secured to the mount face and adjacent standoff (typ. of 1 per sector, total of 3).**

	Component	Controlling Load Case	Stress Ratio	Pass/Fail
<b>Existing Mount Rating</b>	8	LC1	156%	<b>FAIL</b>
<b>Modified Mount Rating</b>	54	LC1	84%	<b>PASS</b>

Reference Documents:

- Fabrication drawings prepared Sitepro1, P/N RMV12-496, dated July 2, 2015.

This determination was based on the following limitations and assumptions:

1. TEP NE is not responsible for any modifications completed prior to and hereafter which TEP NE was not directly involved.
2. All structural members and their connections are assumed to be in good condition and are free from defects with no deterioration to its member capacities.
3. All antennas, coax cables and waveguide cables are assumed to be properly installed and supported as per the manufacturer's requirements.
4. The existing mount has been adequately secured to the tower structure per the mount manufacturer's specifications.
5. All components pertaining to AT&T's mount must be tightened and re-plumbed prior to the installation of new appurtenances.
6. TEP NE performed a localized analysis on the mount itself and not on the supporting tower structure.

Please feel free to contact our office should you have any questions.

Respectfully Submitted,  
TEP Northeast



Michael Cabral  
Director



Daniel P. Hamm, PE  
Vice President

FIELD PHOTOS:





**Wind & Ice  
Calculations**



Date: 2/15/2024  
 Project Name: ORANGE CT RIDGE ROAD  
 Project No.: CT2267  
 Designed By: KM Checked By: MSC



**2.6.5.2 Velocity Pressure Coeff:**

$$K_z = 2.01 (z/z_g)^{2/\alpha}$$

$K_z =$  **1.294**

$z =$  **111** (ft)  
 $z_g =$  **900** (ft)  
 $\alpha =$  **9.5**

$K_{zmin} \leq K_z \leq 2.01$

**Table 2-4**

Exposure	Z <sub>g</sub>	α	K <sub>zmin</sub>	K <sub>c</sub>
B	1200 ft	7.0	0.70	0.9
C	900 ft	9.5	0.85	1.0
D	700 ft	11.5	1.03	1.1

**2.6.6.2 Topographic Factor:**

**Table 2-5**

Topo. Category	K <sub>t</sub>	f
2	0.43	1.25
3	0.53	2.0
4	0.72	1.5

$$K_{zt} = [1 + (K_c K_t / K_h)]^2$$

$$K_h = e^{(fz/H)}$$

$K_{zt} =$  **1**

$K_h =$  **1**

$K_c =$  **1.0** (from Table 2-4)

$K_t =$  **0** (from Table 2-5)

$f =$  **0** (from Table 2-5)

$z =$  **111**

$z_s =$  **145** (Mean elevation of base of structure above sea level)

$H =$  **0** (Ht. of the crest above surrounding terrain)

$K_{zt} =$  **1.00** (from 2.6.6.2.1)

$K_e =$  **0.99** (from 2.6.8)

*(If Category 1 then K<sub>zt</sub>=1.0)*

**Category = 1**

**2.6.10 Design Ice Thickness**

Max Ice Thickness =  
 Importance Factor =

$t_i =$  **1.00** in

$I =$  **1.15** (from Table 2-3)

$K_{iz} =$  **1.13** (from Sec. 2.6.10)

$$t_{iz} = t_i * I * K_{iz} * (K_{zt})^{0.35}$$

$t_{iz} =$  **1.30** in

Date: 2/15/2024  
 Project Name: ORANGE CT RIDGE ROAD  
 Project No.: CT2267  
 Designed By: KM Checked By: MSC



**2.6.9 Gust Effect Factor**

2.6.9.1 Self Supporting Lattice Structures

$G_h = 1.0$  Latticed Structures > 600 ft

$G_h = 0.85$  Latticed Structures 450 ft or less

$G_h = 0.85 + 0.15 [h/150 - 3.0]$

h= ht. of structure

h= 114

$G_h = 0.85$

2.6.9.2 Guyed Masts

$G_h = 0.85$

2.6.9.3 Pole Structures

$G_h = 1.1$

2.6.9 Appurtenances

$G_h = 1.0$

2.6.9.4 Structures Supported on Other Structures

(Cantilevered tubular or latticed spines, pole, structures on buildings (ht. : width ratio > 5)

$G_h = 1.35$

$G_h = 1.00$

**2.6.11.2 Design Wind Force on Appurtenances**

$F = q_z * G_h * (EPA)_A$

$q_z = 0.00256 * K_z * K_{zt} * K_s * K_e * K_d * V_{max}^2$

$K_z = 1.294$  (from 2.6.5.2)

$K_{zt} = 1.0$  (from 2.6.6.2.1)

$K_s = 1.0$  (from 2.6.7)

$K_e = 0.99$  (from 2.6.8)

$K_d = 0.95$  (from Table 2-2)

$V_{max} = 130$  mph (Ultimate Wind Speed)

$V_{max(ice)} = 50$  mph

$V_{30} = 30$  mph

$q_z =$	<b>52.90</b>
$q_z(ice) =$	<b>7.82</b>
$q_z(30) =$	<b>2.82</b>

**Table 2-2**

Structure Type	Wind Direction Probability Factor, $K_d$
Latticed structures with triangular, square or rectangular cross sections	0.85
Tubular pole structures, latticed structures with other cross sections, appurtenances	0.95
Tubular pole structures supporting antennas enclosed within a cylindrical shroud	1.00

Date: 2/16/2024  
 Project Name: ORANGE CT RIDGE ROAD  
 Project No.: CT2267  
 Designed By: KM Checked By: MSC



**Determine Ca:**

**Table 2-9**

Force Coefficients (Ca) for Appurtenances				
Member Type		Aspect Ratio ≤ 2.5	Aspect Ratio = 7	Aspect Ratio ≥ 25
		Ca	Ca	Ca
Flat		1.2	1.4	2.0
Square/Rectangular HSS		1.2 - 2.8(r <sub>s</sub> ) ≥ 0.85	1.4 - 4.0(r <sub>s</sub> ) ≥ 0.90	2.0 - 6.0(r <sub>s</sub> ) ≥ 1.25
Round	C < 39 (Subcritical)	0.7	0.8	1.2
	39 ≤ C ≤ 78 (Transitional)	4.14/(C <sup>0.485</sup> )	3.66/(C <sup>0.415</sup> )	46.8/(C <sup>1.0</sup> )
	C > 78 (Supercritical)	0.5	0.6	0.6

Aspect Ratio is the overall length/width ratio in the plane normal to the wind direction.  
 (Aspect ratio is independent of the spacing between support points of a linear appurtenance.)

Note: Linear interpolation may be used for aspect ratios other than those shown.

Ice Thickness = 1.30 in      Angle = 0 (deg)      Equivalent Angle = 180 (deg)

Appurtenances	Height	Width	Depth	Flat Area	Aspect Ratio	Ca	Force (lbs)	Force (lbs) (w/ Ice)
TPA65R-BU8DA-K Antenna	96.0	20.7	7.7	13.80	4.64	1.30	945	162
AIR6449 Antenna	30.6	15.9	10.6	3.38	1.92	1.20	214	40
AIR6419 Antenna	31.2	16.1	9.1	3.49	1.94	1.20	221	41
DMP65R-BU8DA Antenna	96.0	20.7	7.7	13.80	4.64	1.30	945	162
TMA2124F03V5-2D TMA	10.4	9.7	8.3	0.70	1.07	1.20	44	10
TMA2124F03V5-2D TMA (Shielded)	10.4	0.0	8.3	0.00	0.00	1.20	0	2
TMA2104F00V1-1 TMA	8.7	4.1	7.9	0.25	2.12	1.20	16	5
TMA2104F00V1-1 TMA (Shielded)	8.7	0.0	7.9	0.00	0.00	1.20	0	2
DC6 Surge Arrestor	31.4	10.2	10.2	2.22	3.08	0.70	82	17
HSS 4x4	4.0	12.0	-	0.33	0.33	1.25	22	
4" Pipe	4.5	12.0	-	0.38	0.38	1.20	24	
3" Pipe	3.5	12.0	-	0.29	0.29	1.20	19	
2-1/2" Pipe	2.9	12.0	-	0.24	0.24	1.20	15	
2" Pipe	2.4	12.0	-	0.20	0.20	1.20	13	

Date: 2/15/2024  
 Project Name: ORANGE CT RIDGE ROAD  
 Project No.: CT2267  
 Designed By: KM Checked By: MSC



WIND LOADS

Angle = 30 (deg)      Ice Thickness = 1.30 in.      Equivalent Angle = 210 (deg)

WIND LOADS WITH NO ICE:

Appurtenances	Height	Width	Depth	Flat Area (normal)	Flat Area (side)	Aspect Ratio	Aspect Ratio	Ca (normal)	Ca (side)	Force (lbs) (normal)	Force (lbs) (side)	Force (lbs) (angle)
TPA65R-BU8DA-K Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	816
AIR6449 Antenna	30.6	15.9	10.6	3.38	2.25	1.92	2.89	1.20	1.22	214	145	197
AIR6419 Antenna	31.2	16.1	9.1	3.49	1.97	1.94	3.43	1.20	1.24	221	129	198
DMP65R-BU8DA Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	816
TMA2124F03V5-2D TMA	10.4	9.7	8.3	0.70	0.60	1.07	1.25	1.20	1.20	44	38	43
TMA2124F03V5-2D TMA (Shielded)	10.4	4.9	8.3	0.35	0.60	2.14	1.25	1.20	1.20	22	38	26
TMA2104F00V1-1 TMA	8.7	4.1	7.9	0.25	0.48	2.12	1.10	1.20	1.20	16	30	19
TMA2104F00V1-1 TMA (Shielded)	8.7	2.1	7.9	0.12	0.48	4.24	1.10	1.28	1.20	8	30	14

WIND LOADS WITH ICE:

TPA65R-BU8DA-K Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	140
AIR6449 Antenna	33.2	18.5	13.2	4.26	3.04	1.79	2.52	1.20	1.20	40	29	37
AIR6419 Antenna	33.8	18.7	11.7	4.39	2.75	1.81	2.89	1.20	1.22	41	26	37
DMP65R-BU8DA Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	140
TMA2124F03V5-2D TMA	13.0	12.3	10.9	1.11	0.98	1.06	1.19	1.20	1.20	10	9	10
TMA2124F03V5-2D TMA (Shielded)	13.0	6.1	10.9	0.55	0.98	2.11	1.19	1.20	1.20	5	9	6
TMA2104F00V1-1 TMA	11.3	6.7	10.5	0.53	0.82	1.69	1.08	1.20	1.20	5	8	6
TMA2104F00V1-1 TMA (Shielded)	11.3	3.3	10.5	0.26	0.82	3.37	1.08	1.24	1.20	3	8	4

Date: 2/15/2024  
 Project Name: ORANGE CT RIDGE ROAD  
 Project No.: CT2267  
 Designed By: KM Checked By: MSC



WIND LOADS

Angle = 60 (deg)      Ice Thickness = 1.30 in.      Equivalent Angle = 240 (deg)

WIND LOADS WITH NO ICE:

Appurtenances	Height	Width	Depth	Flat Area (normal)	Flat Area (side)	Ratio (normal)	Ratio (side)	Ca (normal)	Ca (side)	Force (lbs) (normal)	Force (lbs) (side)	Force (lbs) (angle)
TPA65R-BU8DA-K Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	559
AIR6449 Antenna	30.6	15.9	10.6	3.38	2.25	1.92	2.89	1.20	1.22	214	145	162
AIR6419 Antenna	31.2	16.1	9.1	3.49	1.97	1.94	3.43	1.20	1.24	221	129	152
DMP65R-BU8DA Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	559
TMA2124F03V5-2D TMA	10.4	9.7	8.3	0.70	0.60	1.07	1.25	1.20	1.20	44	38	40
TMA2124F03V5-2D TMA (Shielded)	10.4	7.3	8.3	0.53	0.60	1.43	1.25	1.20	1.20	33	38	37
TMA2104F00V1-1 TMA	8.7	4.1	7.9	0.25	0.48	2.12	1.10	1.20	1.20	16	30	27
TMA2104F00V1-1 TMA (Shielded)	8.7	3.1	7.9	0.19	0.48	2.83	1.10	1.21	1.20	12	30	26

WIND LOADS WITH ICE:

TPA65R-BU8DA-K Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	101
AIR6449 Antenna	33.2	18.5	13.2	4.26	3.04	1.79	2.52	1.20	1.20	40	29	31
AIR6419 Antenna	33.8	18.7	11.7	4.39	2.75	1.81	2.89	1.20	1.22	41	26	30
DMP65R-BU8DA Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	101
TMA2124F03V5-2D TMA	13.0	12.3	10.9	1.11	0.98	1.06	1.19	1.20	1.20	10	9	10
TMA2124F03V5-2D TMA (Shielded)	13.0	9.2	10.9	0.83	0.98	1.41	1.19	1.20	1.20	8	9	9
TMA2104F00V1-1 TMA	11.3	6.7	10.5	0.53	0.82	1.69	1.08	1.20	1.20	5	8	7
TMA2104F00V1-1 TMA (Shielded)	11.3	5.0	10.5	0.39	0.82	2.25	1.08	1.20	1.20	4	8	7

Date: 2/15/2024  
 Project Name: ORANGE CT RIDGE ROAD  
 Project No.: CT2267  
 Designed By: KM Checked By: MSC



WIND LOADS

Angle = 90 (deg)      Ice Thickness = 1.30 in.      Equivalent Angle = 270 (deg)

WIND LOADS WITH NO ICE:

Appurtenances	Height	Width	Depth	Flat Area (normal)	Flat Area (side)	Ratio (normal)	Ratio (side)	Ca (normal)	Ca (side)	Force (lbs) (normal)	Force (lbs) (side)	Force (lbs) (angle)
TPA65R-BU8DA-K Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	430
AIR6449 Antenna	30.6	15.9	10.6	3.38	2.25	1.92	2.89	1.20	1.22	214	145	145
AIR6419 Antenna	31.2	16.1	9.1	3.49	1.97	1.94	3.43	1.20	1.24	221	129	129
DMP65R-BU8DA Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	430
TMA2124F03V5-2D TMA	10.4	9.7	8.3	0.70	0.60	1.07	1.25	1.20	1.20	44	38	38
TMA2124F03V5-2D TMA (Shielded)	10.4	0.0	8.3	0.00	0.60	0.00	1.25	1.20	1.20	0	38	38
TMA2104F00V1-1 TMA	8.7	4.1	7.9	0.25	0.48	2.12	1.10	1.20	1.20	16	30	30
TMA2104F00V1-1 TMA (Shielded)	8.7	0.0	7.9	0.00	0.48	0.00	1.10	1.20	1.20	0	30	30

WIND LOADS WITH ICE:

TPA65R-BU8DA-K Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	82
AIR6449 Antenna	33.2	18.5	13.2	4.26	3.04	1.79	2.52	1.20	1.20	40	29	29
AIR6419 Antenna	33.8	18.7	11.7	4.39	2.75	1.81	2.89	1.20	1.22	41	26	26
DMP65R-BU8DA Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	82
TMA2124F03V5-2D TMA	13.0	12.3	10.9	1.11	0.98	1.06	1.19	1.20	1.20	10	9	9
TMA2124F03V5-2D TMA (Shielded)	13.0	2.6	10.9	0.23	0.98	5.01	1.19	1.31	1.20	2	9	9
TMA2104F00V1-1 TMA	11.3	6.7	10.5	0.53	0.82	1.69	1.08	1.20	1.20	5	8	8
TMA2104F00V1-1 TMA (Shielded)	11.3	2.6	10.5	0.20	0.82	4.35	1.08	1.28	1.20	2	8	8

Date: 2/15/2024  
 Project Name: ORANGE CT RIDGE ROAD  
 Project No.: CT2267  
 Designed By: KM Checked By: MSC



WIND LOADS

Angle = 120 (deg) Ice Thickness = 1.30 in. Equivalent Angle = 300 (deg)

WIND LOADS WITH NO ICE:

Appurtenances	Height	Width	Depth	Flat Area (normal)	Flat Area (side)	Ratio (normal)	Ratio (side)	Ca (normal)	Ca (side)	Force (lbs) (normal)	Force (lbs) (side)	Force (lbs) (angle)
TPA65R-BU8DA-K Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	559
AIR6449 Antenna	30.6	15.9	10.6	3.38	2.25	1.92	2.89	1.20	1.22	214	145	162
AIR6419 Antenna	31.2	16.1	9.1	3.49	1.97	1.94	3.43	1.20	1.24	221	129	152
DMP65R-BU8DA Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	559
TMA2124F03V5-2D TMA	10.4	9.7	8.3	0.70	0.60	1.07	1.25	1.20	1.20	44	38	40
TMA2124F03V5-2D TMA (Shielded)	10.4	7.3	8.3	0.53	0.60	1.43	1.25	1.20	1.20	33	38	37
TMA2104F00V1-1 TMA	8.7	4.1	7.9	0.25	0.48	2.12	1.10	1.20	1.20	16	30	27
TMA2104F00V1-1 TMA (Shielded)	8.7	3.1	7.9	0.19	0.48	2.83	1.10	1.21	1.20	12	30	26

WIND LOADS WITH ICE:

TPA65R-BU8DA-K Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	101
AIR6449 Antenna	33.2	18.5	13.2	4.26	3.04	1.79	2.52	1.20	1.20	40	29	31
AIR6419 Antenna	33.8	18.7	11.7	4.39	2.75	1.81	2.89	1.20	1.22	41	26	30
DMP65R-BU8DA Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	101
TMA2124F03V5-2D TMA	13.0	12.3	10.9	1.11	0.98	1.06	1.19	1.20	1.20	10	9	10
TMA2124F03V5-2D TMA (Shielded)	13.0	9.2	10.9	0.83	0.98	1.41	1.19	1.20	1.20	8	9	9
TMA2104F00V1-1 TMA	11.3	6.7	10.5	0.53	0.82	1.69	1.08	1.20	1.20	5	8	7
TMA2104F00V1-1 TMA (Shielded)	11.3	5.0	10.5	0.39	0.82	2.25	1.08	1.20	1.20	4	8	7

Date: 2/15/2024  
 Project Name: ORANGE CT RIDGE ROAD  
 Project No.: CT2267  
 Designed By: KM Checked By: MSC



WIND LOADS

Angle = 150 (deg) Ice Thickness = 1.30 in. Equivalent Angle = 330 (deg)

WIND LOADS WITH NO ICE:

Appurtenances	Height	Width	Depth	Flat Area (normal)	Flat Area (side)	Ratio (normal)	Ratio (side)	Ca (normal)	Ca (side)	Force (lbs) (normal)	Force (lbs) (side)	Force (lbs) (angle)
TPA65R-BU8DA-K Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	816
AIR6449 Antenna	30.6	15.9	10.6	3.38	2.25	1.92	2.89	1.20	1.22	214	145	197
AIR6419 Antenna	31.2	16.1	9.1	3.49	1.97	1.94	3.43	1.20	1.24	221	129	198
DMP65R-BU8DA Antenna	96.0	20.7	7.7	13.80	5.13	4.64	12.47	1.30	1.58	945	430	816
TMA2124F03V5-2D TMA	10.4	9.7	8.3	0.70	0.60	1.07	1.25	1.20	1.20	44	38	43
TMA2124F03V5-2D TMA (Shielded)	10.4	4.9	8.3	0.35	0.60	2.14	1.25	1.20	1.20	22	38	26
TMA2104F00V1-1 TMA	8.7	4.1	7.9	0.25	0.48	2.12	1.10	1.20	1.20	16	30	19
TMA2104F00V1-1 TMA (Shielded)	8.7	2.1	7.9	0.12	0.48	4.24	1.10	1.28	1.20	8	30	14

WIND LOADS WITH ICE:

TPA65R-BU8DA-K Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	140
AIR6449 Antenna	33.2	18.5	13.2	4.26	3.04	1.79	2.52	1.20	1.20	40	29	37
AIR6419 Antenna	33.8	18.7	11.7	4.39	2.75	1.81	2.89	1.20	1.22	41	26	37
DMP65R-BU8DA Antenna	98.6	23.3	10.3	15.95	7.05	4.23	9.58	1.28	1.49	159	82	140
TMA2124F03V5-2D TMA	13.0	12.3	10.9	1.11	0.98	1.06	1.19	1.20	1.20	10	9	10
TMA2124F03V5-2D TMA (Shielded)	13.0	6.1	10.9	0.55	0.98	2.11	1.19	1.20	1.20	5	9	6
TMA2104F00V1-1 TMA	11.3	6.7	10.5	0.53	0.82	1.69	1.08	1.20	1.20	5	8	6
TMA2104F00V1-1 TMA (Shielded)	11.3	3.3	10.5	0.26	0.82	3.37	1.08	1.24	1.20	3	8	4



Date: 2/16/2024

Project Name: ORANGE CT RIDGE ROAD

Project No.: CT2267

Designed By: KM Checked By: MSC



ICE WEIGHT CALCULATIONS

Thickness of ice: 1.30 in.
Density of ice: 56 pcf

TPA65R-BU8DA-K Antenna

Weight of ice based on total radial SF area:
Height (in): 96.0
Width (in): 20.7
Depth (in): 7.7
Total weight of ice on object: 297 lbs
Weight of object: 87.0 lbs
Combined weight of ice and object: 384 lbs

AIR6449 Antenna

Weight of ice based on total radial SF area:
Height (in): 30.6
Width (in): 15.9
Depth (in): 10.6
Total weight of ice on object: 83 lbs
Weight of object: 84.0 lbs
Combined weight of ice and object: 167 lbs

AIR6419 Antenna

Weight of ice based on total radial SF area:
Height (in): 31.2
Width (in): 16.1
Depth (in): 9.1
Total weight of ice on object: 82 lbs
Weight of object: 66.0 lbs
Combined weight of ice and object: 148 lbs

DMP65R-BU8DA Antenna

Weight of ice based on total radial SF area:
Height (in): 96.0
Width (in): 20.7
Depth (in): 7.7
Total weight of ice on object: 297 lbs
Weight of object: 119.0 lbs
Combined weight of ice and object: 416 lbs

TMA2124F03V5-2D TMA

Weight of ice based on total radial SF area:
Height (in): 10.4
Width (in): 9.7
Depth (in): 8.3
Total weight of ice on object: 19 lbs
Weight of object: 36.0 lbs
Combined weight of ice and object: 55 lbs

TMA2104F00V1-1 TMA

Weight of ice based on total radial SF area:
Height (in): 8.7
Width (in): 4.1
Depth (in): 7.9
Total weight of ice on object: 12 lbs
Weight of object: 8.0 lbs
Combined weight of ice and object: 20 lbs

DC6 Surge Arrestor

Weight of ice based on total radial SF area:
Depth (in): 31.4
Diameter(in): 10.2
Total weight of ice on object: 48 lbs
Weight of object: 29 lbs
Combined weight of ice and object: 77 lbs

HSS 4x4

Weight of ice based on total radial SF area:
Height (in): 4
Width (in): 4
Per foot weight of ice on object: 11 plf

4" Pipe

Per foot weight of ice:
diameter (in): 4.5
Per foot weight of ice on object: 9 plf

3" Pipe

Per foot weight of ice:
diameter (in): 3.5
Per foot weight of ice on object: 8 plf

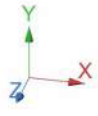
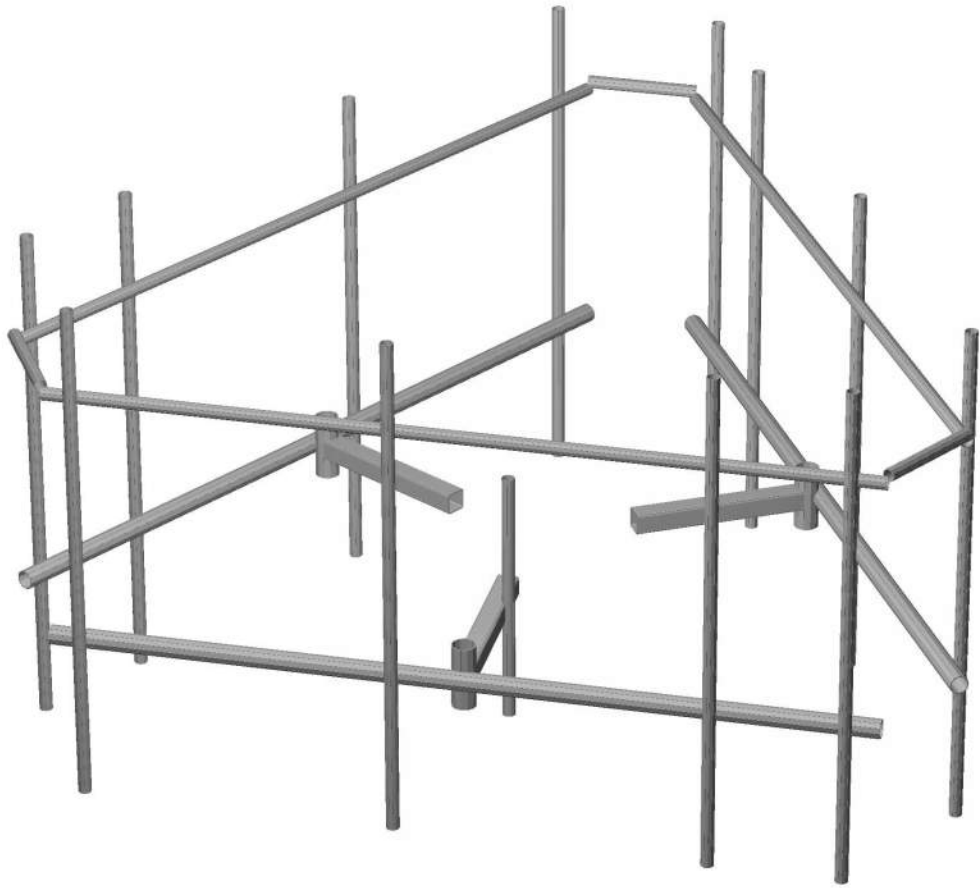
2-1/2" Pipe

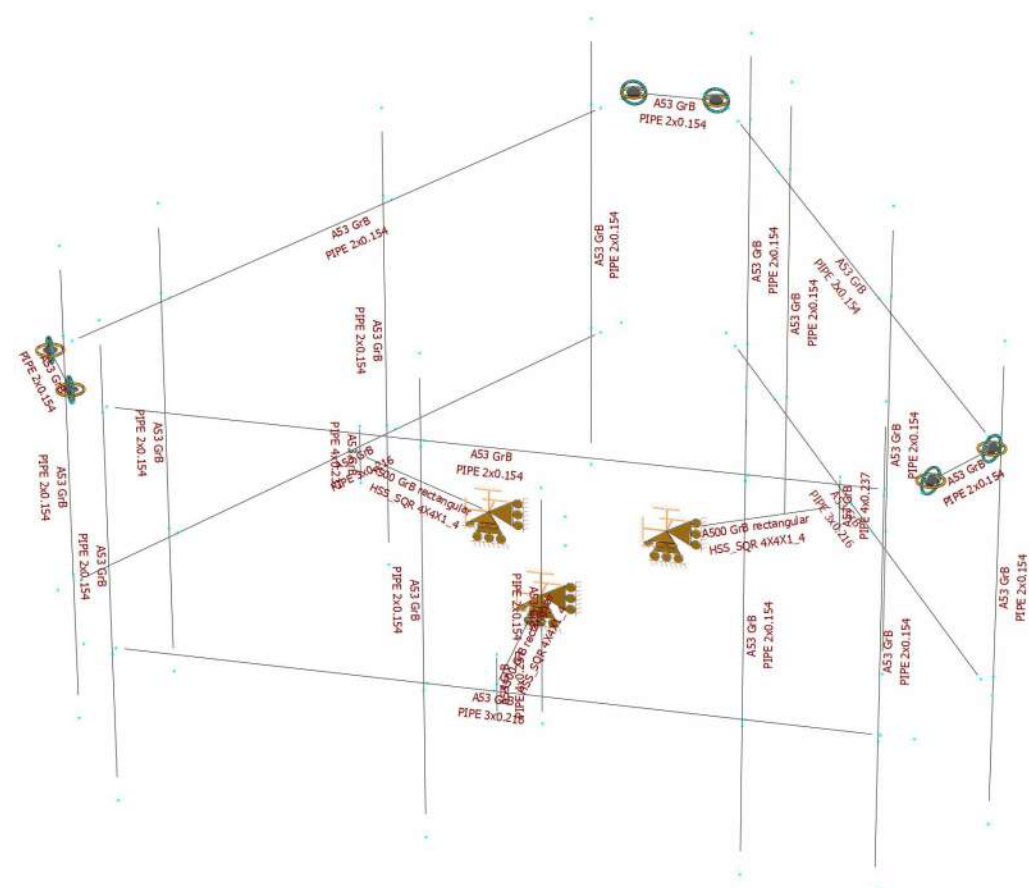
Per foot weight of ice:
diameter (in): 2.88
Per foot weight of ice on object: 7 plf

2" Pipe

Per foot weight of ice:
diameter (in): 2.38
Per foot weight of ice on object: 6 plf

**Mount Calculations  
(Existing Conditions)**

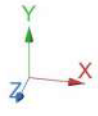
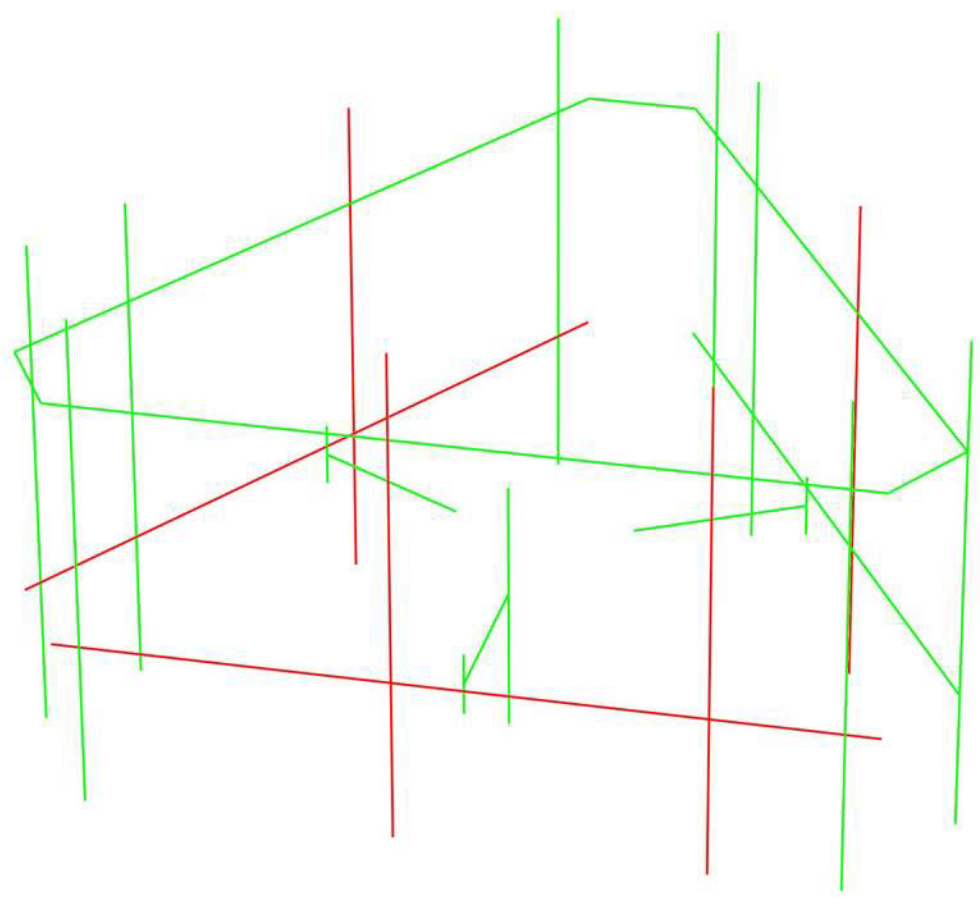


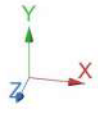
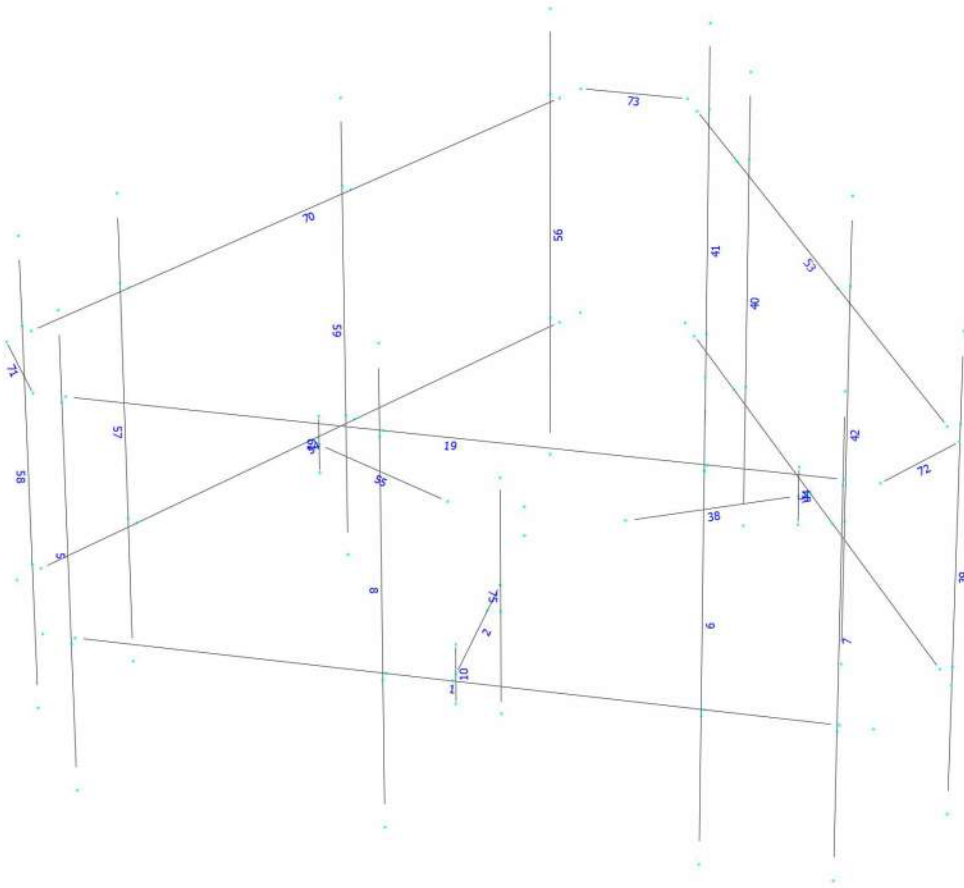




Design status

- Not designed
- Error on design
- Design O.K.
- With warnings





## Load data

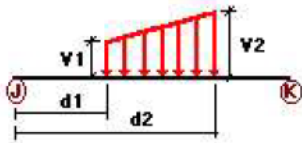
### GLOSSARY

Comb : Indicates if load condition is a load combination

### Load Conditions

Condition	Description	Comb.	Category
DL	Dead Load	No	DL
W0	Wind Load 0/60/120 deg	No	WIND
W30	Wind Load 30/90/150 deg	No	WIND
Di	Ice Load	No	LL
Wi0	Ice Wind Load 0/60/120 deg	No	WIND
Wi30	Ice Wind Load 30/90/150 deg	No	WIND

### Distributed force on members

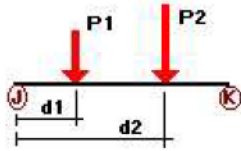


Condition	Member	Dir1	Val1 [Kip/ft]	Val2 [Kip/ft]	Dist1 [ft]	%	Dist2 [ft]	%
W0	1	z	-0.019	-0.019	0.00	No	100.00	Yes
	7	z	-0.013	-0.013	0.00	No	100.00	Yes
	10	z	-0.024	-0.024	0.00	No	100.00	Yes
	19	z	-0.013	-0.013	0.00	No	100.00	Yes
	37	z	-0.019	-0.019	0.00	No	100.00	Yes
	38	z	-0.022	-0.022	0.00	No	100.00	Yes
	39	z	-0.013	-0.013	0.00	No	100.00	Yes
	40	z	-0.013	-0.013	0.00	No	100.00	Yes
	41	z	-0.013	-0.013	0.00	No	100.00	Yes
	42	z	-0.013	-0.013	0.00	No	100.00	Yes
	44	z	-0.024	-0.024	0.00	No	100.00	Yes
	53	z	-0.013	-0.013	0.00	No	100.00	Yes
	54	z	-0.019	-0.019	0.00	No	100.00	Yes
	55	z	-0.022	-0.022	0.00	No	100.00	Yes
	56	z	-0.013	-0.013	0.00	No	100.00	Yes
	57	z	-0.013	-0.013	0.00	No	100.00	Yes
	58	z	-0.013	-0.013	0.00	No	100.00	Yes
	59	z	-0.013	-0.013	0.00	No	100.00	Yes
	61	z	-0.024	-0.024	0.00	No	100.00	Yes
	70	z	-0.013	-0.013	0.00	No	100.00	Yes
71	z	-0.013	-0.013	0.00	No	100.00	Yes	
72	z	-0.013	-0.013	0.00	No	100.00	Yes	
73	z	-0.013	-0.013	0.00	No	100.00	Yes	
75	z	-0.013	-0.013	0.00	No	100.00	Yes	

W30	2	x	-0.022	-0.022	0.00	No	100.00	Yes
	5	x	-0.013	-0.013	0.00	No	100.00	Yes
	6	x	-0.013	-0.013	0.00	No	100.00	Yes
	7	x	-0.013	-0.013	0.00	No	100.00	Yes
	8	x	-0.013	-0.013	0.00	No	100.00	Yes
	10	x	-0.024	-0.024	0.00	No	100.00	Yes
	19	x	-0.013	-0.013	0.00	No	100.00	Yes
	37	x	-0.019	-0.019	0.00	No	100.00	Yes
	38	x	-0.022	-0.022	0.00	No	100.00	Yes
	41	x	-0.013	-0.013	0.00	No	100.00	Yes
	44	x	-0.024	-0.024	0.00	No	100.00	Yes
	53	x	-0.013	-0.013	0.00	No	100.00	Yes
	54	x	-0.019	-0.019	0.00	No	100.00	Yes
	55	x	-0.022	-0.022	0.00	No	100.00	Yes
	56	x	-0.013	-0.013	0.00	No	100.00	Yes
	57	x	-0.013	-0.013	0.00	No	100.00	Yes
	58	x	-0.013	-0.013	0.00	No	100.00	Yes
	59	x	-0.013	-0.013	0.00	No	100.00	Yes
	61	x	-0.024	-0.024	0.00	No	100.00	Yes
	70	x	-0.013	-0.013	0.00	No	100.00	Yes
	71	x	-0.013	-0.013	0.00	No	100.00	Yes
	72	x	-0.013	-0.013	0.00	No	100.00	Yes
	73	x	-0.013	-0.013	0.00	No	100.00	Yes
	75	x	-0.013	-0.013	0.00	No	100.00	Yes
	Di	1	y	-0.008	-0.008	0.00	No	100.00
2		y	-0.011	-0.011	0.00	No	100.00	Yes
5		y	-0.006	-0.006	0.00	No	100.00	Yes
6		y	-0.006	-0.006	0.00	No	100.00	Yes
7		y	-0.006	-0.006	0.00	No	100.00	Yes
8		y	-0.006	-0.006	0.00	No	100.00	Yes
10		y	-0.009	-0.009	0.00	No	100.00	Yes
19		y	-0.006	-0.006	0.00	No	100.00	Yes
37		y	-0.008	-0.008	0.00	No	100.00	Yes
38		y	-0.011	-0.011	0.00	No	100.00	Yes
39		y	-0.006	-0.006	0.00	No	100.00	Yes
40		y	-0.006	-0.006	0.00	No	100.00	Yes
41		y	-0.006	-0.006	0.00	No	100.00	Yes
42		y	-0.006	-0.006	0.00	No	100.00	Yes
44		y	-0.009	-0.009	0.00	No	100.00	Yes
53		y	-0.006	-0.006	0.00	No	100.00	Yes
54		y	-0.008	-0.008	0.00	No	100.00	Yes
55		y	-0.011	-0.011	0.00	No	100.00	Yes
56		y	-0.006	-0.006	0.00	No	100.00	Yes
57		y	-0.006	-0.006	0.00	No	100.00	Yes
58		y	-0.006	-0.006	0.00	No	100.00	Yes
59		y	-0.006	-0.006	0.00	No	100.00	Yes
61		y	-0.009	-0.009	0.00	No	100.00	Yes
70		y	-0.006	-0.006	0.00	No	100.00	Yes
71		y	-0.006	-0.006	0.00	No	100.00	Yes
72	y	-0.006	-0.006	0.00	No	100.00	Yes	
73	y	-0.006	-0.006	0.00	No	100.00	Yes	
75	y	-0.006	-0.006	0.00	No	100.00	Yes	



### Concentrated forces on members



Condition	Member	Dir1	Value1 [Kip]	Dist1 [ft]	%
DL	5	y	-0.06	0.50	No
		y	-0.06	7.50	No
		y	-0.008	4.00	No
		y	-0.008	4.00	No
	6	y	-0.044	0.50	No
		y	-0.044	7.50	No
		y	-0.036	4.00	No
	8	y	-0.033	1.00	No
		y	-0.033	3.00	No
		y	-0.041	5.00	No
		y	-0.041	7.00	No
	39	y	-0.06	0.50	No
		y	-0.06	7.50	No
		y	-0.008	4.00	No
		y	-0.008	4.00	No
	40	y	-0.044	0.50	No
		y	-0.044	7.50	No
		y	-0.036	4.00	No
		y	-0.033	1.00	No
	42	y	-0.033	3.00	No
		y	-0.041	5.00	No
		y	-0.041	7.00	No
		y	-0.06	0.50	No
	56	y	-0.06	7.50	No
y		-0.008	4.00	No	
y		-0.008	4.00	No	
y		-0.044	0.50	No	
57	y	-0.044	7.50	No	
	y	-0.036	4.00	No	
	y	-0.033	1.00	No	
59	y	-0.033	3.00	No	
	y	-0.041	5.00	No	
	y	-0.041	7.00	No	
	y	-0.029	1.00	No	
W0	5	z	-0.473	0.50	No
		z	-0.473	7.50	No
		z	-0.473	0.50	No
		z	-0.473	7.50	No
	8	z	-0.111	1.00	No
		z	-0.111	3.00	No
		z	-0.107	5.00	No
		z	-0.107	7.00	No
	39	z	-0.28	0.50	No
		z	-0.28	7.50	No
		z	-0.026	4.00	No
		z	-0.28	0.50	No
	40	z	-0.28	7.50	No
		z	-0.037	4.00	No
		z	-0.076	1.00	No
		z	-0.076	3.00	No
	42	z	-0.081	5.00	No
		z	-0.081	7.00	No
		z	-0.28	0.50	No
		z	-0.28	7.50	No

		z	-0.026	4.00	No
	57	z	-0.28	0.50	No
		z	-0.28	7.50	No
		z	-0.037	4.00	No
	59	z	-0.076	1.00	No
		z	-0.076	3.00	No
		z	-0.081	5.00	No
		z	-0.081	7.00	No
	75	z	-0.082	1.00	No
W30	5	x	-0.215	0.50	No
		x	-0.215	7.50	No
		x	-0.03	4.00	No
	6	x	-0.215	0.50	No
		x	-0.215	7.50	No
		x	-0.038	4.00	No
	8	x	-0.065	1.00	No
		x	-0.065	3.00	No
		x	-0.073	5.00	No
		x	-0.073	7.00	No
	39	x	-0.408	0.50	No
		x	-0.408	7.50	No
		x	-0.014	4.00	No
	40	x	-0.408	0.50	No
		x	-0.408	7.50	No
		x	-0.026	4.00	No
	42	x	-0.099	1.00	No
		x	-0.099	3.00	No
		x	-0.099	5.00	No
		x	-0.099	7.00	No
	56	x	-0.408	0.50	No
		x	-0.408	7.50	No
		x	-0.014	4.00	No
	57	x	-0.408	0.50	No
		x	-0.408	7.50	No
		x	-0.026	4.00	No
	59	x	-0.099	1.00	No
		x	-0.099	3.00	No
		x	-0.099	5.00	No
		x	-0.099	7.00	No
	75	x	-0.082	1.00	No
Di	5	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.012	4.00	No
		y	-0.012	4.00	No
	6	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.019	4.00	No
	8	y	-0.041	1.00	No
		y	-0.041	3.00	No
		y	-0.042	5.00	No
		y	-0.042	7.00	No
	39	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.012	4.00	No
		y	-0.012	4.00	No
	40	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.019	4.00	No
	42	y	-0.041	1.00	No
		y	-0.041	3.00	No

		y	-0.042	5.00	No
		y	-0.042	7.00	No
	56	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.012	4.00	No
		y	-0.012	4.00	No
	57	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.019	4.00	No
	59	y	-0.041	1.00	No
		y	-0.041	3.00	No
		y	-0.042	5.00	No
		y	-0.042	7.00	No
	75	y	-0.048	1.00	No
Wi0	5	z	-0.081	0.50	No
		z	-0.081	7.50	No
		z	-0.002	4.00	No
	6	z	-0.081	0.50	No
		z	-0.081	7.50	No
		z	-0.002	4.00	No
	8	z	-0.021	1.00	No
		z	-0.021	3.00	No
		z	-0.02	5.00	No
		z	-0.02	7.00	No
	39	z	-0.051	0.50	No
		z	-0.051	7.50	No
		z	-0.007	4.00	No
	40	z	-0.051	0.50	No
		z	-0.051	7.50	No
		z	-0.009	4.00	No
	42	z	-0.015	1.00	No
		z	-0.015	3.00	No
		z	-0.016	5.00	No
		z	-0.016	7.00	No
	56	z	-0.051	0.50	No
		z	-0.051	7.50	No
		z	-0.007	4.00	No
	57	z	-0.051	0.50	No
		z	-0.051	7.50	No
		z	-0.009	4.00	No
	59	z	-0.015	1.00	No
		z	-0.015	3.00	No
		z	-0.016	5.00	No
		z	-0.016	7.00	No
	75	z	-0.017	1.00	No
Wi30	5	x	-0.041	0.50	No
		x	-0.041	7.50	No
		x	-0.008	4.00	No
	6	x	-0.041	0.50	No
		x	-0.041	7.50	No
		x	-0.009	4.00	No
	8	x	-0.013	1.00	No
		x	-0.013	3.00	No
		x	-0.015	5.00	No
		x	-0.015	7.00	No
	39	x	-0.07	0.50	No
		x	-0.07	7.50	No
		x	-0.004	4.00	No
	40	x	-0.07	0.50	No
		x	-0.07	7.50	No

	x	-0.006	4.00	No
42	x	-0.019	1.00	No
	x	-0.019	3.00	No
	x	-0.019	5.00	No
	x	-0.019	7.00	No
56	x	-0.07	0.50	No
	x	-0.07	7.50	No
	x	-0.004	4.00	No
57	x	-0.07	0.50	No
	x	-0.07	7.50	No
	x	-0.006	4.00	No
59	x	-0.019	1.00	No
	x	-0.019	3.00	No
	x	-0.019	5.00	No
	x	-0.019	7.00	No
75	x	-0.017	1.00	No

### Self weight multipliers for load conditions

Condition	Description	Self weight multiplier			
		Comb.	MultX	MultY	MultZ
DL	Dead Load	No	0.00	-1.00	0.00
W0	Wind Load 0/60/120 deg	No	0.00	0.00	0.00
W30	Wind Load 30/90/150 deg	No	0.00	0.00	0.00
Di	Ice Load	No	0.00	0.00	0.00
Wi0	Ice Wind Load 0/60/120 deg	No	0.00	0.00	0.00
Wi30	Ice Wind Load 30/90/150 deg	No	0.00	0.00	0.00

## Steel Code Check

Report: Summary - Group by member

Load conditions to be included in design :

- LC1=1.2DL+W0
- LC2=1.2DL+W30
- LC3=1.2DL-W0
- LC4=1.2DL-W30
- LC5=0.9DL+W0
- LC6=0.9DL+W30
- LC7=0.9DL-W0
- LC8=0.9DL-W30
- LC9=1.2DL+Di+Wi0
- LC10=1.2DL+Di+Wi30
- LC11=1.2DL+Di-Wi0
- LC12=1.2DL+Di-Wi30
- LC13=1.4DL
- LC16=1.2DL

Description	Section	Member	Ctrl Eq.	Ratio	Status	Reference
	<i>HSS_SQR 4X4X1_4</i>	2	LC2 at 0.00%	0.53	OK	
		38	LC1 at 0.00%	0.64	OK	
		55	LC1 at 0.00%	<b>0.68</b>	<b>OK</b>	
	<i>PIPE 2x0.154</i>	5	LC1 at 66.67%	0.98	OK	
		6	LC1 at 66.67%	<b>1.02</b>	N.G.	
		7	LC1 at 66.67%	0.63	OK	
		8	LC1 at 66.67%	<b>1.56</b>	<b>N.G.</b>	
		19	LC1 at 41.25%	0.49	OK	
		39	LC2 at 66.67%	0.82	OK	
		40	LC2 at 66.67%	0.90	OK	
		41	LC2 at 66.67%	0.63	OK	
		42	LC2 at 66.67%	<b>1.32</b>	N.G.	
		53	LC4 at 42.50%	0.62	OK	
		56	LC4 at 66.67%	0.85	OK	
		57	LC4 at 66.67%	0.87	OK	
		58	LC4 at 66.67%	0.52	OK	
		59	LC4 at 66.67%	<b>1.37</b>	N.G.	
		70	LC4 at 42.50%	0.56	OK	
		71	LC3 at 100.00%	0.01	OK	
	72	LC4 at 50.00%	0.00	OK		
	73	LC2 at 0.00%	0.01	OK		
	75	LC2 at 53.13%	0.07	OK		
	<i>PIPE 3x0.216</i>	1	LC1 at 48.96%	<b>1.01</b>	N.G.	
		37	LC2 at 50.00%	1.00	OK	
		54	LC4 at 48.96%	<b>1.12</b>	<b>N.G.</b>	
	<i>PIPE 4x0.237</i>	10	LC2 at 50.00%	<b>0.00</b>	<b>OK</b>	
		44	LC4 at 50.00%	0.00	OK	
		61	LC1 at 50.00%	0.00	OK	

## Geometry data

### GLOSSARY

Cb22, Cb33	: Moment gradient coefficients
Cm22, Cm33	: Coefficients applied to bending term in interaction formula
d0	: Tapered member section depth at J end of member
DJX	: Rigid end offset distance measured from J node in axis X
DJY	: Rigid end offset distance measured from J node in axis Y
DJZ	: Rigid end offset distance measured from J node in axis Z
DKX	: Rigid end offset distance measured from K node in axis X
DKY	: Rigid end offset distance measured from K node in axis Y
DKZ	: Rigid end offset distance measured from K node in axis Z
dL	: Tapered member section depth at K end of member
Ig factor	: Inertia reduction factor (Effective Inertia/Gross Inertia) for reinforced concrete members
K22	: Effective length factor about axis 2
K33	: Effective length factor about axis 3
L22	: Member length for calculation of axial capacity
L33	: Member length for calculation of axial capacity
LB pos	: Lateral unbraced length of the compression flange in the positive side of local axis 2
LB neg	: Lateral unbraced length of the compression flange in the negative side of local axis 2
RX	: Rotation about X
RY	: Rotation about Y
RZ	: Rotation about Z
TO	: 1 = Tension only member    0 = Normal member
TX	: Translation in X
TY	: Translation in Y
TZ	: Translation in Z

### Nodes

Node	X [ft]	Y [ft]	Z [ft]	Rigid Floor
1	-6.25	0.00	0.00	0
2	6.25	0.00	0.00	0
3	0.00	0.00	-0.20	0
4	0.00	0.00	-3.00	0
8	-5.75	5.50	0.20	0
9	3.75	5.50	0.20	0
10	5.75	5.50	0.20	0
11	-1.00	5.50	0.20	0
12	-5.75	-2.50	0.20	0
13	3.75	-2.50	0.20	0
14	5.75	-2.50	0.20	0
15	-1.00	-2.50	0.20	0
16	0.00	0.00	0.00	0
17	0.00	0.50	-0.20	0
18	0.00	-0.50	-0.20	0
19	-5.75	0.00	0.00	0
20	-1.00	0.00	0.00	0
21	3.75	0.00	0.00	0
22	5.75	0.00	0.00	0
23	5.75	0.00	0.20	0
24	3.75	0.00	0.20	0
25	-1.00	0.00	0.20	0
26	-5.75	0.00	0.20	0

27	-6.25	4.00	0.00	0
28	-5.75	4.00	0.00	0
29	-5.75	4.00	0.20	0
30	-1.00	4.00	0.00	0
31	-1.00	4.00	0.20	0
32	6.25	4.00	0.00	0
33	3.75	4.00	0.00	0
34	5.75	4.00	0.00	0
35	5.75	4.00	0.20	0
36	3.75	4.00	0.20	0
37	0.00	0.00	-4.62	0
38	0.00	0.50	-4.62	0
72	7.126	0.00	-1.5173	0
73	0.876	0.00	-12.3427	0
74	3.8278	0.00	-6.83	0
75	1.403	0.00	-5.43	0
76	7.0492	5.50	-2.0504	0
77	2.2992	5.50	-10.2776	0
78	1.2992	5.50	-12.0096	0
79	4.6742	5.50	-6.164	0
80	7.0492	-2.50	-2.0504	0
81	2.2992	-2.50	-10.2776	0
82	1.2992	-2.50	-12.0096	0
83	4.6742	-2.50	-6.164	0
84	4.001	0.00	-6.93	0
85	3.8278	0.50	-6.83	0
86	3.8278	-0.50	-6.83	0
87	6.876	0.00	-1.9504	0
88	4.501	0.00	-6.064	0
89	2.126	0.00	-10.1776	0
90	1.126	0.00	-11.9096	0
91	1.2992	0.00	-12.0096	0
92	2.2992	0.00	-10.2776	0
93	4.6742	0.00	-6.164	0
94	7.0492	0.00	-2.0504	0
95	7.126	4.00	-1.5173	0
96	6.876	4.00	-1.9504	0
97	7.0492	4.00	-2.0504	0
98	4.501	4.00	-6.064	0
99	4.6742	4.00	-6.164	0
100	0.876	4.00	-12.3427	0
101	2.126	4.00	-10.1776	0
102	1.126	4.00	-11.9096	0
103	1.2992	4.00	-12.0096	0
104	2.2992	4.00	-10.2776	0
105	-0.876	0.00	-12.3427	0
106	-7.126	0.00	-1.5173	0
107	-3.8278	0.00	-6.83	0
108	-1.403	0.00	-5.43	0
109	-1.2992	5.50	-12.0096	0
110	-6.0492	5.50	-3.7824	0
111	-7.0492	5.50	-2.0504	0
112	-3.6742	5.50	-7.896	0
113	-1.2992	-2.50	-12.0096	0
114	-6.0492	-2.50	-3.7824	0
115	-7.0492	-2.50	-2.0504	0
116	-3.6742	-2.50	-7.896	0
117	-4.001	0.00	-6.93	0
118	-3.8278	0.50	-6.83	0
119	-3.8278	-0.50	-6.83	0

120	-1.126	0.00	-11.9096	0
121	-3.501	0.00	-7.796	0
122	-5.876	0.00	-3.6824	0
123	-6.876	0.00	-1.9504	0
124	-7.0492	0.00	-2.0504	0
125	-6.0492	0.00	-3.7824	0
126	-3.6742	0.00	-7.896	0
127	-1.2992	0.00	-12.0096	0
128	-0.876	4.00	-12.3427	0
129	-1.126	4.00	-11.9096	0
130	-1.2992	4.00	-12.0096	0
131	-3.501	4.00	-7.796	0
132	-3.6742	4.00	-7.896	0
133	-7.126	4.00	-1.5173	0
134	-5.876	4.00	-3.6824	0
135	-6.876	4.00	-1.9504	0
136	-7.0492	4.00	-2.0504	0
137	-6.0492	4.00	-3.7824	0
138	0.00	0.00	-2.20	0
139	0.20	0.00	-2.20	0
140	0.20	2.25	-2.20	0
141	0.20	-1.75	-2.20	0

## Restraints

Node	TX	TY	TZ	RX	RY	RZ
4	1	1	1	1	1	1
75	1	1	1	1	1	1
108	1	1	1	1	1	1

## Members

Member	NJ	NK	Description	Section	Material	d0 [in]	dL [in]	Ig factor
1	1	2		PIPE 3x0.216	A53 GrB	0.00	0.00	0.00
2	4	3		HSS_SQR 4X4X1_4	A500 GrB rectangular	0.00	0.00	0.00
5	8	12		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
6	9	13		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
7	10	14		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
8	11	15		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
10	18	17		PIPE 4x0.237	A53 GrB	0.00	0.00	0.00
19	27	32		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
37	72	73		PIPE 3x0.216	A53 GrB	0.00	0.00	0.00
38	75	74		HSS_SQR 4X4X1_4	A500 GrB rectangular	0.00	0.00	0.00
39	76	80		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
40	77	81		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
41	78	82		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
42	79	83		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
44	86	85		PIPE 4x0.237	A53 GrB	0.00	0.00	0.00
53	95	100		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
54	105	106		PIPE 3x0.216	A53 GrB	0.00	0.00	0.00



55	108	107	HSS_SQR 4X4X1_4	A500 GrB rectangular	0.00	0.00	0.00
56	109	113	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
57	110	114	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
58	111	115	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
59	112	116	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
61	119	118	PIPE 4x0.237	A53 GrB	0.00	0.00	0.00
70	128	133	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
71	27	133	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
72	32	95	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
73	100	128	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
75	140	141	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00

### Orientation of local axes

Member	Rotation [Deg]	Axes23	NX	NY	NZ
5	315.00	0	0.00	0.00	0.00
6	315.00	0	0.00	0.00	0.00
7	315.00	0	0.00	0.00	0.00
8	315.00	0	0.00	0.00	0.00
39	0.00	2	-0.9659	0.00	-0.2588
40	0.00	2	-0.9659	0.00	-0.2588
41	0.00	2	-0.9659	0.00	-0.2588
42	0.00	2	-0.9659	0.00	-0.2588
44	0.00	2	0.50	0.00	0.866
56	0.00	2	0.2588	0.00	0.9659
57	0.00	2	0.2588	0.00	0.9659
58	0.00	2	0.2588	0.00	0.9659
59	0.00	2	0.2588	0.00	0.9659
61	0.00	2	0.50	0.00	-0.866

### Rigid end offsets

Member	DJX [in]	DJY [in]	DJZ [in]	DKX [in]	DKY [in]	DKZ [in]
71	0.00	1.50	0.00	0.00	1.50	0.00
72	0.00	1.50	0.00	0.00	1.50	0.00
73	0.00	1.50	0.00	0.00	1.50	0.00

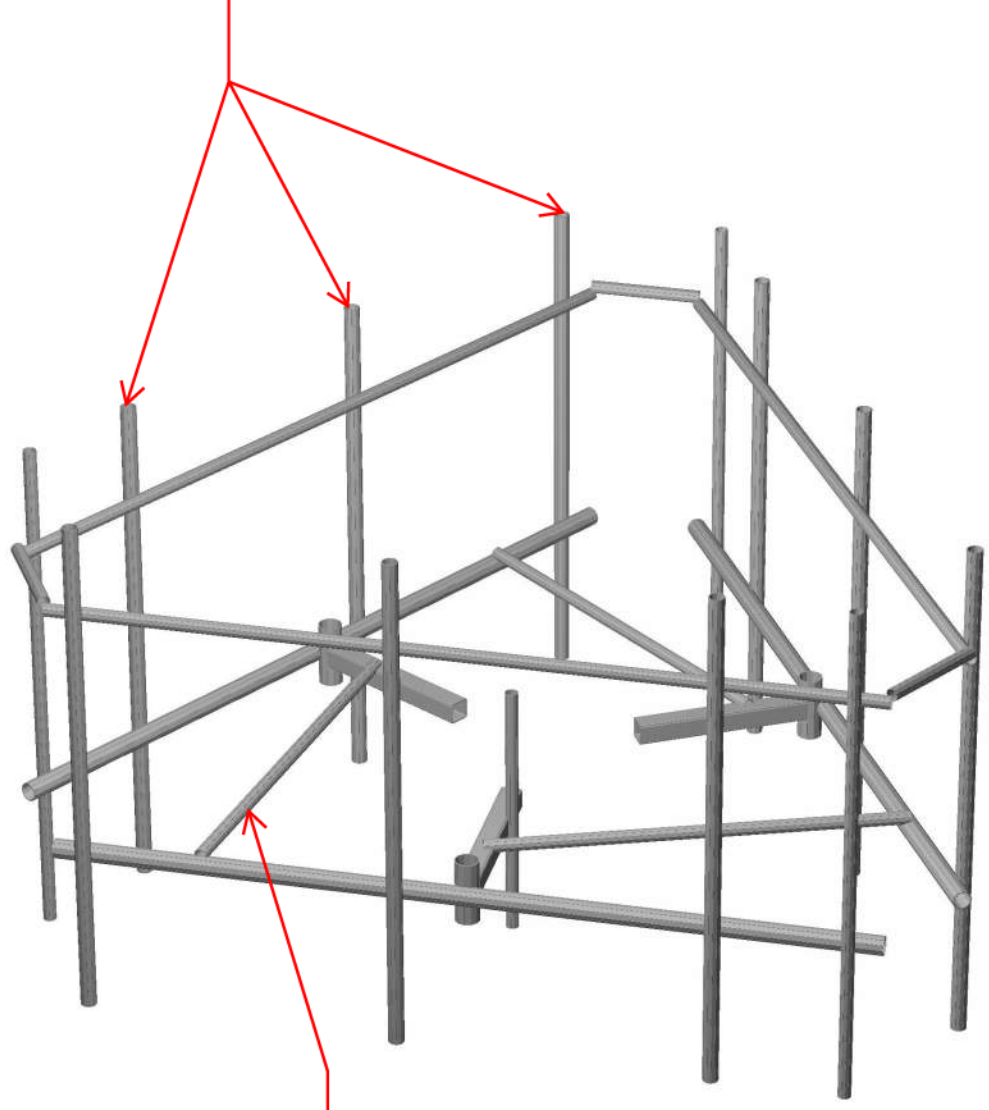
### Hinges

Member	Node-J				Node-K				TOR	AXL	Axial rigidity
	M33	M22	V3	V2	M33	M22	V3	V2			
71	1	1	0	0	1	1	0	0	0	0	Full
72	1	1	0	0	1	1	0	0	0	0	Full
73	1	1	0	0	1	1	0	0	0	0	Full

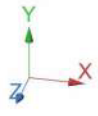
**Mount Calculations  
(Modified Conditions)**

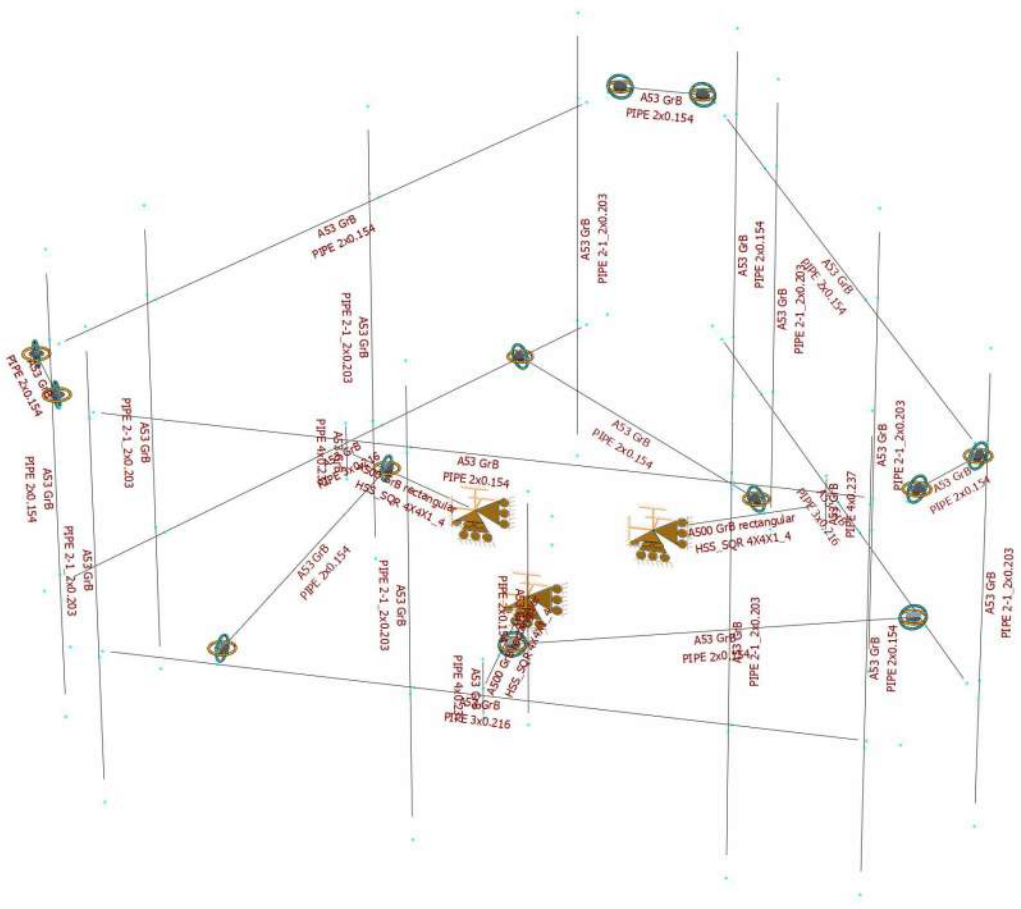


Remove existing antenna pipe mast and install proposed 2-1/2" std. (2.88" O.D.) pipe mast secured to the existing mount and handrail (typ. of 3 per sector, total of 9).



Install proposed 2" std. (2.38" O.D.) pipe brace secured to the mount face and adjacent standoff (typ. of 1 per sector, total of 3).

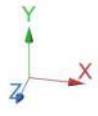
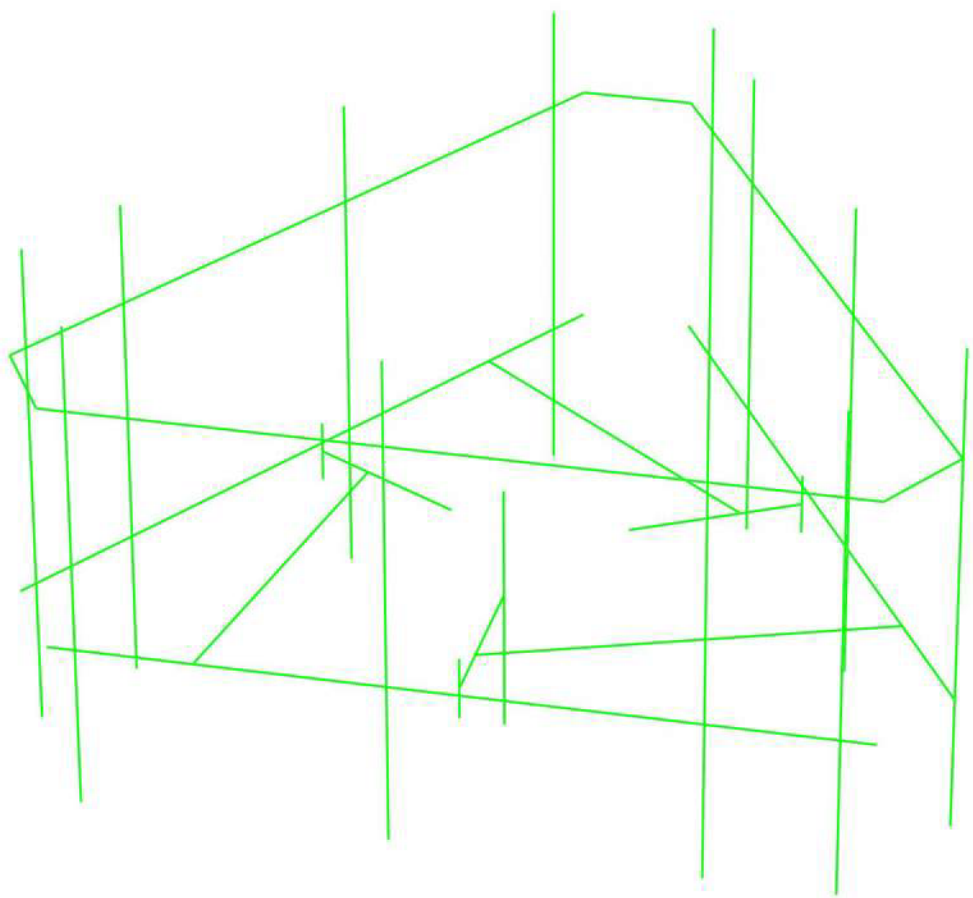


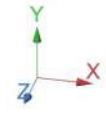
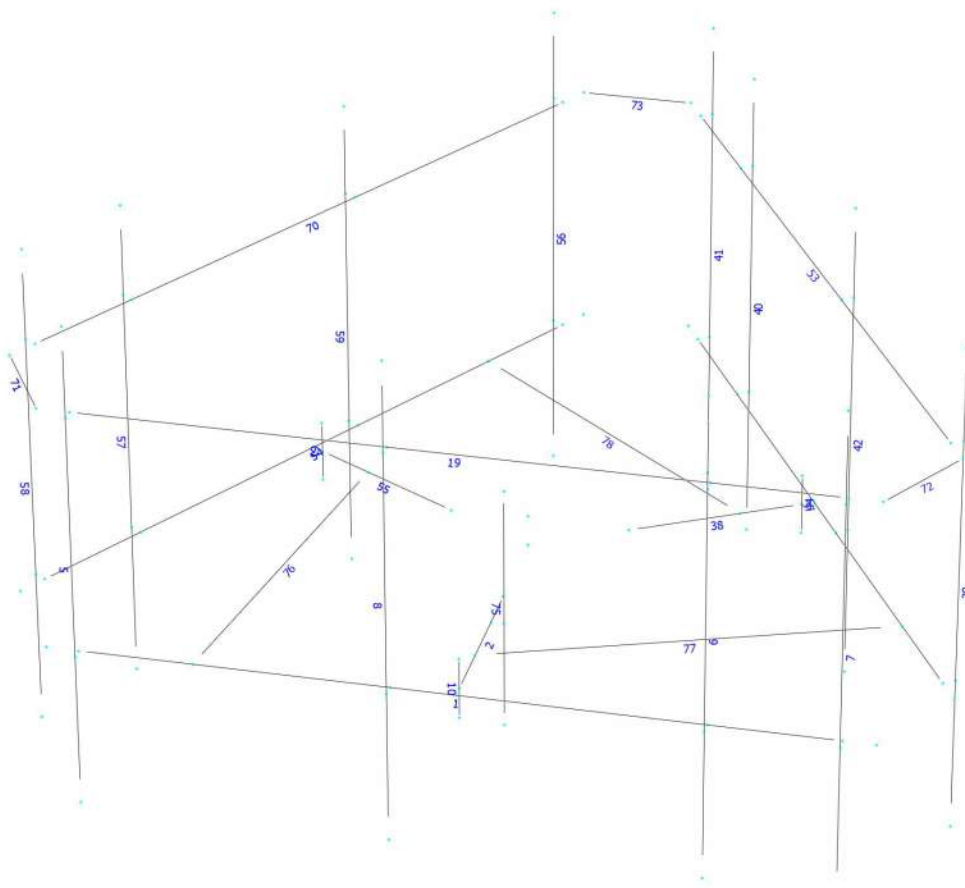




Design status

- Not designed
- Error on design
- Design O.K.
- With warnings





## Load data

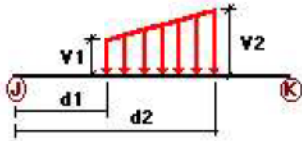
### GLOSSARY

Comb : Indicates if load condition is a load combination

### Load Conditions

Condition	Description	Comb.	Category
DL	Dead Load	No	DL
W0	Wind Load 0/60/120 deg	No	WIND
W30	Wind Load 30/90/150 deg	No	WIND
Di	Ice Load	No	LL
Wi0	Ice Wind Load 0/60/120 deg	No	WIND
Wi30	Ice Wind Load 30/90/150 deg	No	WIND

### Distributed force on members

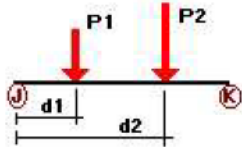


Condition	Member	Dir1	Val1 [Kip/ft]	Val2 [Kip/ft]	Dist1 [ft]	%	Dist2 [ft]	%
W0	1	z	-0.019	-0.019	0.00	No	100.00	Yes
	7	z	-0.013	-0.013	0.00	No	100.00	Yes
	10	z	-0.024	-0.024	0.00	No	100.00	Yes
	19	z	-0.013	-0.013	0.00	No	100.00	Yes
	37	z	-0.019	-0.019	0.00	No	100.00	Yes
	38	z	-0.022	-0.022	0.00	No	100.00	Yes
	39	z	-0.015	-0.015	0.00	No	100.00	Yes
	40	z	-0.015	-0.015	0.00	No	100.00	Yes
	41	z	-0.013	-0.013	0.00	No	100.00	Yes
	42	z	-0.015	-0.015	0.00	No	100.00	Yes
	44	z	-0.024	-0.024	0.00	No	100.00	Yes
	53	z	-0.013	-0.013	0.00	No	100.00	Yes
	54	z	-0.019	-0.019	0.00	No	100.00	Yes
	55	z	-0.022	-0.022	0.00	No	100.00	Yes
	56	z	-0.015	-0.015	0.00	No	100.00	Yes
	57	z	-0.015	-0.015	0.00	No	100.00	Yes
	58	z	-0.013	-0.013	0.00	No	100.00	Yes
	59	z	-0.015	-0.015	0.00	No	100.00	Yes
	61	z	-0.024	-0.024	0.00	No	100.00	Yes
	70	z	-0.013	-0.013	0.00	No	100.00	Yes
71	z	-0.013	-0.013	0.00	No	100.00	Yes	
72	z	-0.013	-0.013	0.00	No	100.00	Yes	
73	z	-0.013	-0.013	0.00	No	100.00	Yes	
75	z	-0.013	-0.013	0.00	No	100.00	Yes	

	76	z	-0.013	-0.013	0.00	No	100.00	Yes
	77	z	-0.013	-0.013	0.00	No	100.00	Yes
	78	z	-0.013	-0.013	0.00	No	100.00	Yes
W30	2	x	-0.022	-0.022	0.00	No	100.00	Yes
	5	x	-0.015	-0.015	0.00	No	100.00	Yes
	6	x	-0.015	-0.015	0.00	No	100.00	Yes
	7	x	-0.013	-0.013	0.00	No	100.00	Yes
	8	x	-0.015	-0.015	0.00	No	100.00	Yes
	10	x	-0.024	-0.024	0.00	No	100.00	Yes
	19	x	-0.013	-0.013	0.00	No	100.00	Yes
	37	x	-0.019	-0.019	0.00	No	100.00	Yes
	38	x	-0.022	-0.022	0.00	No	100.00	Yes
	41	x	-0.013	-0.013	0.00	No	100.00	Yes
	44	x	-0.024	-0.024	0.00	No	100.00	Yes
	53	x	-0.013	-0.013	0.00	No	100.00	Yes
	54	x	-0.019	-0.019	0.00	No	100.00	Yes
	55	x	-0.022	-0.022	0.00	No	100.00	Yes
	56	x	-0.015	-0.015	0.00	No	100.00	Yes
	57	x	-0.015	-0.015	0.00	No	100.00	Yes
	58	x	-0.013	-0.013	0.00	No	100.00	Yes
	59	x	-0.015	-0.015	0.00	No	100.00	Yes
	61	x	-0.024	-0.024	0.00	No	100.00	Yes
	70	x	-0.013	-0.013	0.00	No	100.00	Yes
	71	x	-0.013	-0.013	0.00	No	100.00	Yes
	72	x	-0.013	-0.013	0.00	No	100.00	Yes
	73	x	-0.013	-0.013	0.00	No	100.00	Yes
	75	x	-0.013	-0.013	0.00	No	100.00	Yes
	76	x	-0.013	-0.013	0.00	No	100.00	Yes
	77	x	-0.013	-0.013	0.00	No	100.00	Yes
	78	x	-0.013	-0.013	0.00	No	100.00	Yes
Di	1	y	-0.008	-0.008	0.00	No	100.00	Yes
	2	y	-0.011	-0.011	0.00	No	100.00	Yes
	5	y	-0.007	-0.007	0.00	No	100.00	Yes
	6	y	-0.007	-0.007	0.00	No	100.00	Yes
	7	y	-0.006	-0.006	0.00	No	100.00	Yes
	8	y	-0.007	-0.007	0.00	No	100.00	Yes
	10	y	-0.009	-0.009	0.00	No	100.00	Yes
	19	y	-0.006	-0.006	0.00	No	100.00	Yes
	37	y	-0.008	-0.008	0.00	No	100.00	Yes
	38	y	-0.011	-0.011	0.00	No	100.00	Yes
	39	y	-0.007	-0.007	0.00	No	100.00	Yes
	40	y	-0.007	-0.007	0.00	No	100.00	Yes
	41	y	-0.006	-0.006	0.00	No	100.00	Yes
	42	y	-0.007	-0.007	0.00	No	100.00	Yes
	44	y	-0.009	-0.009	0.00	No	100.00	Yes
	53	y	-0.006	-0.006	0.00	No	100.00	Yes
	54	y	-0.008	-0.008	0.00	No	100.00	Yes
	55	y	-0.011	-0.011	0.00	No	100.00	Yes
	56	y	-0.007	-0.007	0.00	No	100.00	Yes
	57	y	-0.007	-0.007	0.00	No	100.00	Yes
	58	y	-0.006	-0.006	0.00	No	100.00	Yes
	59	y	-0.007	-0.007	0.00	No	100.00	Yes
	61	y	-0.009	-0.009	0.00	No	100.00	Yes
	70	y	-0.006	-0.006	0.00	No	100.00	Yes
	71	y	-0.006	-0.006	0.00	No	100.00	Yes
	72	y	-0.006	-0.006	0.00	No	100.00	Yes
	73	y	-0.006	-0.006	0.00	No	100.00	Yes
	75	y	-0.006	-0.006	0.00	No	100.00	Yes
	76	y	-0.006	-0.006	0.00	No	100.00	Yes
	77	y	-0.006	-0.006	0.00	No	100.00	Yes



**Concentrated forces on members**



Condition	Member	Dir1	Value1 [Kip]	Dist1 [ft]	%
DL	5	y	-0.06	0.50	No
		y	-0.06	7.50	No
		y	-0.008	4.00	No
	6	y	-0.008	4.00	No
		y	-0.044	0.50	No
		y	-0.044	7.50	No
	8	y	-0.036	4.00	No
		y	-0.033	1.00	No
		y	-0.033	3.00	No
	39	y	-0.041	5.00	No
		y	-0.041	7.00	No
		y	-0.06	0.50	No
	40	y	-0.06	7.50	No
		y	-0.008	4.00	No
		y	-0.008	4.00	No
	42	y	-0.044	0.50	No
		y	-0.044	7.50	No
		y	-0.036	4.00	No
	56	y	-0.033	1.00	No
		y	-0.033	3.00	No
		y	-0.041	5.00	No
	57	y	-0.041	7.00	No
		y	-0.06	0.50	No
		y	-0.06	7.50	No
59	y	-0.008	4.00	No	
	y	-0.008	4.00	No	
	y	-0.044	0.50	No	
75	y	-0.044	7.50	No	
	y	-0.036	4.00	No	
	y	-0.033	1.00	No	
W0	5	y	-0.033	3.00	No
		y	-0.041	5.00	No
		y	-0.041	7.00	No
	6	z	-0.029	1.00	No
		z	-0.473	0.50	No
		z	-0.473	7.50	No
	8	z	-0.473	0.50	No
		z	-0.473	7.50	No
		z	-0.111	1.00	No
	39	z	-0.111	3.00	No
		z	-0.107	5.00	No
		z	-0.107	7.00	No
39	z	-0.107	7.00	No	
	z	-0.28	0.50	No	
		z	-0.28	7.50	No

		z	-0.026	4.00	No
	40	z	-0.28	0.50	No
		z	-0.28	7.50	No
		z	-0.037	4.00	No
	42	z	-0.076	1.00	No
		z	-0.076	3.00	No
		z	-0.081	5.00	No
		z	-0.081	7.00	No
	56	z	-0.28	0.50	No
		z	-0.28	7.50	No
		z	-0.026	4.00	No
	57	z	-0.28	0.50	No
		z	-0.28	7.50	No
		z	-0.037	4.00	No
	59	z	-0.076	1.00	No
		z	-0.076	3.00	No
		z	-0.081	5.00	No
		z	-0.081	7.00	No
	75	z	-0.082	1.00	No
W30	5	x	-0.215	0.50	No
		x	-0.215	7.50	No
		x	-0.03	4.00	No
	6	x	-0.215	0.50	No
		x	-0.215	7.50	No
		x	-0.038	4.00	No
	8	x	-0.065	1.00	No
		x	-0.065	3.00	No
		x	-0.073	5.00	No
		x	-0.073	7.00	No
	39	x	-0.408	0.50	No
		x	-0.408	7.50	No
		x	-0.014	4.00	No
	40	x	-0.408	0.50	No
		x	-0.408	7.50	No
		x	-0.026	4.00	No
	42	x	-0.099	1.00	No
		x	-0.099	3.00	No
		x	-0.099	5.00	No
		x	-0.099	7.00	No
	56	x	-0.408	0.50	No
		x	-0.408	7.50	No
		x	-0.014	4.00	No
	57	x	-0.408	0.50	No
		x	-0.408	7.50	No
		x	-0.026	4.00	No
	59	x	-0.099	1.00	No
		x	-0.099	3.00	No
		x	-0.099	5.00	No
		x	-0.099	7.00	No
	75	x	-0.082	1.00	No
Di	5	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.012	4.00	No
		y	-0.012	4.00	No
	6	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.019	4.00	No
	8	y	-0.041	1.00	No
		y	-0.041	3.00	No
		y	-0.042	5.00	No

		y	-0.042	7.00	No
	39	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.012	4.00	No
		y	-0.012	4.00	No
	40	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.019	4.00	No
	42	y	-0.041	1.00	No
		y	-0.041	3.00	No
		y	-0.042	5.00	No
		y	-0.042	7.00	No
	56	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.012	4.00	No
		y	-0.012	4.00	No
	57	y	-0.149	0.50	No
		y	-0.149	7.50	No
		y	-0.019	4.00	No
	59	y	-0.041	1.00	No
		y	-0.041	3.00	No
		y	-0.042	5.00	No
		y	-0.042	7.00	No
	75	y	-0.048	1.00	No
Wi0	5	z	-0.081	0.50	No
		z	-0.081	7.50	No
		z	-0.002	4.00	No
	6	z	-0.081	0.50	No
		z	-0.081	7.50	No
		z	-0.002	4.00	No
	8	z	-0.021	1.00	No
		z	-0.021	3.00	No
		z	-0.02	5.00	No
		z	-0.02	7.00	No
	39	z	-0.051	0.50	No
		z	-0.051	7.50	No
		z	-0.007	4.00	No
	40	z	-0.051	0.50	No
		z	-0.051	7.50	No
		z	-0.009	4.00	No
	42	z	-0.015	1.00	No
		z	-0.015	3.00	No
		z	-0.016	5.00	No
		z	-0.016	7.00	No
	56	z	-0.051	0.50	No
		z	-0.051	7.50	No
		z	-0.007	4.00	No
	57	z	-0.051	0.50	No
		z	-0.051	7.50	No
		z	-0.009	4.00	No
	59	z	-0.015	1.00	No
		z	-0.015	3.00	No
		z	-0.016	5.00	No
		z	-0.016	7.00	No
	75	z	-0.017	1.00	No
Wi30	5	x	-0.041	0.50	No
		x	-0.041	7.50	No
		x	-0.008	4.00	No
	6	x	-0.041	0.50	No
		x	-0.041	7.50	No

	x	-0.009	4.00	No
8	x	-0.013	1.00	No
	x	-0.013	3.00	No
	x	-0.015	5.00	No
	x	-0.015	7.00	No
39	x	-0.07	0.50	No
	x	-0.07	7.50	No
	x	-0.004	4.00	No
40	x	-0.07	0.50	No
	x	-0.07	7.50	No
	x	-0.006	4.00	No
42	x	-0.019	1.00	No
	x	-0.019	3.00	No
	x	-0.019	5.00	No
	x	-0.019	7.00	No
56	x	-0.07	0.50	No
	x	-0.07	7.50	No
	x	-0.004	4.00	No
57	x	-0.07	0.50	No
	x	-0.07	7.50	No
	x	-0.006	4.00	No
59	x	-0.019	1.00	No
	x	-0.019	3.00	No
	x	-0.019	5.00	No
	x	-0.019	7.00	No
75	x	-0.017	1.00	No

### Self weight multipliers for load conditions

Condition	Description	Self weight multiplier			
		Comb.	MultX	MultY	MultZ
DL	Dead Load	No	0.00	-1.00	0.00
W0	Wind Load 0/60/120 deg	No	0.00	0.00	0.00
W30	Wind Load 30/90/150 deg	No	0.00	0.00	0.00
Di	Ice Load	No	0.00	0.00	0.00
Wi0	Ice Wind Load 0/60/120 deg	No	0.00	0.00	0.00
Wi30	Ice Wind Load 30/90/150 deg	No	0.00	0.00	0.00

## Steel Code Check

Report: Summary - Group by member

Load conditions to be included in design :

- LC1=1.2DL+W0
- LC2=1.2DL+W30
- LC3=1.2DL-W0
- LC4=1.2DL-W30
- LC5=0.9DL+W0
- LC6=0.9DL+W30
- LC7=0.9DL-W0
- LC8=0.9DL-W30
- LC9=1.2DL+Di+Wi0
- LC10=1.2DL+Di+Wi30
- LC11=1.2DL+Di-Wi0
- LC12=1.2DL+Di-Wi30
- LC13=1.4DL
- LC16=1.2DL

Description	Section	Member	Ctrl Eq.	Ratio	Status	Reference
	<i>HSS_SQR 4X4X1_4</i>	2	LC4 at 0.00%	0.54	OK	
		38	LC4 at 0.00%	<b>0.74</b>	<b>OK</b>	
		55	LC1 at 0.00%	0.71	OK	
	<i>PIPE 2-1_2x0.203</i>	5	LC1 at 66.67%	0.69	OK	
		6	LC1 at 66.67%	0.55	OK	
		8	LC1 at 66.67%	<b>0.84</b>	<b>OK</b>	
		39	LC2 at 66.67%	0.49	OK	
		40	LC2 at 66.67%	0.46	OK	
		42	LC2 at 66.67%	0.69	OK	
		56	LC4 at 66.67%	0.68	OK	
		57	LC4 at 66.67%	0.48	OK	
		59	LC4 at 66.67%	0.77	OK	
		<i>PIPE 2x0.154</i>	7	LC1 at 66.67%	0.23	OK
	19		LC1 at 41.25%	0.46	OK	
	41		LC2 at 66.67%	0.24	OK	
	53		LC2 at 41.25%	0.51	OK	
	58		LC4 at 18.75%	0.23	OK	
	70		LC2 at 41.25%	<b>0.52</b>	<b>OK</b>	
	71		LC2 at 0.00%	0.01	OK	
	72		LC3 at 100.00%	0.01	OK	
	73		LC1 at 50.00%	0.01	OK	
	75		LC3 at 53.13%	0.07	OK	
	76		LC2 at 50.00%	0.06	OK	
	77		LC3 at 50.00%	0.07	OK	
	78		LC4 at 0.00%	0.07	OK	
	<i>PIPE 3x0.216</i>	1	LC2 at 49.11%	0.69	OK	
		37	LC2 at 50.00%	0.76	OK	
		54	LC1 at 49.11%	<b>0.84</b>	<b>OK</b>	
	<i>PIPE 4x0.237</i>	10	LC6 at 50.00%	<b>0.00</b>	<b>OK</b>	
		44	LC1 at 50.00%	0.00	OK	
		61	LC1 at 50.00%	0.00	OK	

## Geometry data

### GLOSSARY

Cb22, Cb33	: Moment gradient coefficients
Cm22, Cm33	: Coefficients applied to bending term in interaction formula
d0	: Tapered member section depth at J end of member
DJX	: Rigid end offset distance measured from J node in axis X
DJY	: Rigid end offset distance measured from J node in axis Y
DJZ	: Rigid end offset distance measured from J node in axis Z
DKX	: Rigid end offset distance measured from K node in axis X
DKY	: Rigid end offset distance measured from K node in axis Y
DKZ	: Rigid end offset distance measured from K node in axis Z
dL	: Tapered member section depth at K end of member
Ig factor	: Inertia reduction factor (Effective Inertia/Gross Inertia) for reinforced concrete members
K22	: Effective length factor about axis 2
K33	: Effective length factor about axis 3
L22	: Member length for calculation of axial capacity
L33	: Member length for calculation of axial capacity
LB pos	: Lateral unbraced length of the compression flange in the positive side of local axis 2
LB neg	: Lateral unbraced length of the compression flange in the negative side of local axis 2
RX	: Rotation about X
RY	: Rotation about Y
RZ	: Rotation about Z
TO	: 1 = Tension only member    0 = Normal member
TX	: Translation in X
TY	: Translation in Y
TZ	: Translation in Z

### Nodes

Node	X [ft]	Y [ft]	Z [ft]	Rigid Floor
1	-6.25	0.00	0.00	0
2	6.25	0.00	0.00	0
3	0.00	0.00	-0.20	0
4	0.00	0.00	-3.00	0
8	-5.75	5.50	0.20	0
9	3.75	5.50	0.20	0
10	5.75	5.50	0.20	0
11	-1.00	5.50	0.20	0
12	-5.75	-2.50	0.20	0
13	3.75	-2.50	0.20	0
14	5.75	-2.50	0.20	0
15	-1.00	-2.50	0.20	0
16	0.00	0.00	0.00	0
17	0.00	0.50	-0.20	0
18	0.00	-0.50	-0.20	0
19	-5.75	0.00	0.00	0
20	-1.00	0.00	0.00	0
21	3.75	0.00	0.00	0
22	5.75	0.00	0.00	0
23	5.75	0.00	0.20	0
24	3.75	0.00	0.20	0
25	-1.00	0.00	0.20	0
26	-5.75	0.00	0.20	0

27	-6.25	4.00	0.00	0
28	-5.75	4.00	0.00	0
29	-5.75	4.00	0.20	0
30	-1.00	4.00	0.00	0
31	-1.00	4.00	0.20	0
32	6.25	4.00	0.00	0
33	3.75	4.00	0.00	0
34	5.75	4.00	0.00	0
35	5.75	4.00	0.20	0
36	3.75	4.00	0.20	0
37	0.00	0.00	-4.62	0
38	0.00	0.50	-4.62	0
72	7.126	0.00	-1.5173	0
73	0.876	0.00	-12.3427	0
74	3.8278	0.00	-6.83	0
75	1.403	0.00	-5.43	0
76	7.0492	5.50	-2.0504	0
77	2.2992	5.50	-10.2776	0
78	1.2992	5.50	-12.0096	0
79	4.6742	5.50	-6.164	0
80	7.0492	-2.50	-2.0504	0
81	2.2992	-2.50	-10.2776	0
82	1.2992	-2.50	-12.0096	0
83	4.6742	-2.50	-6.164	0
84	4.001	0.00	-6.93	0
85	3.8278	0.50	-6.83	0
86	3.8278	-0.50	-6.83	0
87	6.876	0.00	-1.9504	0
88	4.501	0.00	-6.064	0
89	2.126	0.00	-10.1776	0
90	1.126	0.00	-11.9096	0
91	1.2992	0.00	-12.0096	0
92	2.2992	0.00	-10.2776	0
93	4.6742	0.00	-6.164	0
94	7.0492	0.00	-2.0504	0
95	7.126	4.00	-1.5173	0
96	6.876	4.00	-1.9504	0
97	7.0492	4.00	-2.0504	0
98	4.501	4.00	-6.064	0
99	4.6742	4.00	-6.164	0
100	0.876	4.00	-12.3427	0
101	2.126	4.00	-10.1776	0
102	1.126	4.00	-11.9096	0
103	1.2992	4.00	-12.0096	0
104	2.2992	4.00	-10.2776	0
105	-0.876	0.00	-12.3427	0
106	-7.126	0.00	-1.5173	0
107	-3.8278	0.00	-6.83	0
108	-1.403	0.00	-5.43	0
109	-1.2992	5.50	-12.0096	0
110	-6.0492	5.50	-3.7824	0
111	-7.0492	5.50	-2.0504	0
112	-3.6742	5.50	-7.896	0
113	-1.2992	-2.50	-12.0096	0
114	-6.0492	-2.50	-3.7824	0
115	-7.0492	-2.50	-2.0504	0
116	-3.6742	-2.50	-7.896	0
117	-4.001	0.00	-6.93	0
118	-3.8278	0.50	-6.83	0
119	-3.8278	-0.50	-6.83	0

120	-1.126	0.00	-11.9096	0
121	-3.501	0.00	-7.796	0
122	-5.876	0.00	-3.6824	0
123	-6.876	0.00	-1.9504	0
124	-7.0492	0.00	-2.0504	0
125	-6.0492	0.00	-3.7824	0
126	-3.6742	0.00	-7.896	0
127	-1.2992	0.00	-12.0096	0
128	-0.876	4.00	-12.3427	0
129	-1.126	4.00	-11.9096	0
130	-1.2992	4.00	-12.0096	0
131	-3.501	4.00	-7.796	0
132	-3.6742	4.00	-7.896	0
133	-7.126	4.00	-1.5173	0
134	-5.876	4.00	-3.6824	0
135	-6.876	4.00	-1.9504	0
136	-7.0492	4.00	-2.0504	0
137	-6.0492	4.00	-3.7824	0
138	0.00	0.00	-2.20	0
139	0.20	0.00	-2.20	0
140	0.20	2.25	-2.20	0
141	0.20	-1.75	-2.20	0
142	0.00	0.00	-1.20	0
143	2.9618	0.00	-6.33	0
144	-2.9618	0.00	-6.33	0
145	-4.00	0.00	0.00	0
146	6.001	0.00	-3.4659	0
147	-2.001	0.00	-10.3941	0

## Restraints

Node	TX	TY	TZ	RX	RY	RZ
4	1	1	1	1	1	1
75	1	1	1	1	1	1
108	1	1	1	1	1	1

## Members

Member	NJ	NK	Description	Section	Material	d0 [in]	dL [in]	Ig factor
1	1	2		PIPE 3x0.216	A53 GrB	0.00	0.00	0.00
2	4	3		HSS_SQR 4X4X1_4	A500 GrB rectangular	0.00	0.00	0.00
5	8	12		PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
6	9	13		PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
7	10	14		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
8	11	15		PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
10	18	17		PIPE 4x0.237	A53 GrB	0.00	0.00	0.00
19	27	32		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
37	72	73		PIPE 3x0.216	A53 GrB	0.00	0.00	0.00
38	75	74		HSS_SQR 4X4X1_4	A500 GrB rectangular	0.00	0.00	0.00
39	76	80		PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00



40	77	81	PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
41	78	82	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
42	79	83	PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
44	86	85	PIPE 4x0.237	A53 GrB	0.00	0.00	0.00
53	95	100	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
54	105	106	PIPE 3x0.216	A53 GrB	0.00	0.00	0.00
55	108	107	HSS_SQR 4X4X1_4	A500 GrB rectangular	0.00	0.00	0.00
56	109	113	PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
57	110	114	PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
58	111	115	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
59	112	116	PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
61	119	118	PIPE 4x0.237	A53 GrB	0.00	0.00	0.00
70	128	133	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
71	27	133	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
72	32	95	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
73	100	128	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
75	140	141	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
76	145	144	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
77	142	146	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
78	143	147	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00

### Orientation of local axes

Member	Rotation [Deg]	Axes23	NX	NY	NZ
5	315.00	0	0.00	0.00	0.00
6	315.00	0	0.00	0.00	0.00
7	315.00	0	0.00	0.00	0.00
8	315.00	0	0.00	0.00	0.00
39	0.00	2	-0.9659	0.00	-0.2588
40	0.00	2	-0.9659	0.00	-0.2588
41	0.00	2	-0.9659	0.00	-0.2588
42	0.00	2	-0.9659	0.00	-0.2588
44	0.00	2	0.50	0.00	0.866
56	0.00	2	0.2588	0.00	0.9659
57	0.00	2	0.2588	0.00	0.9659
58	0.00	2	0.2588	0.00	0.9659
59	0.00	2	0.2588	0.00	0.9659
61	0.00	2	0.50	0.00	-0.866

### Rigid end offsets

Member	DJX [in]	DJY [in]	DJZ [in]	DKX [in]	DKY [in]	DKZ [in]
71	0.00	1.50	0.00	0.00	1.50	0.00
72	0.00	1.50	0.00	0.00	1.50	0.00
73	0.00	1.50	0.00	0.00	1.50	0.00
76	0.00	2.00	0.00	0.00	2.00	0.00
77	0.00	2.00	0.00	0.00	2.00	0.00
78	0.00	2.00	0.00	0.00	2.00	0.00

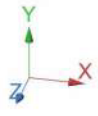
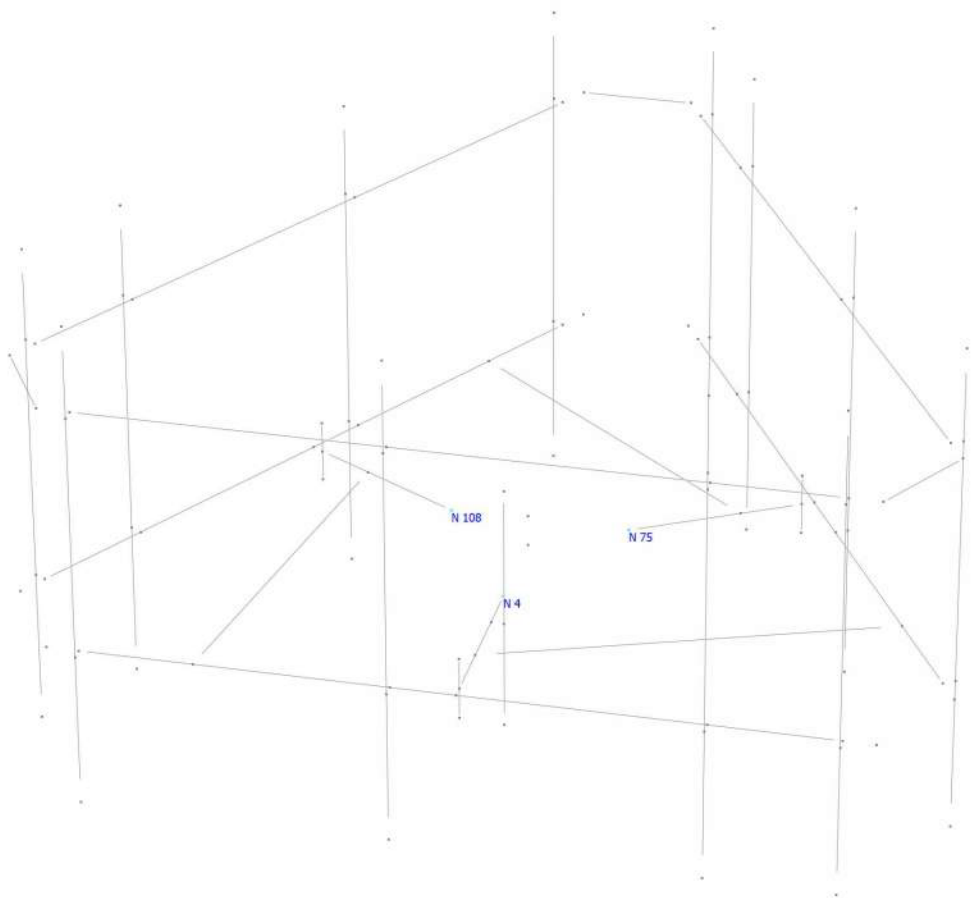
## Hinges

---

Member	Node-J				Node-K				TOR	AXL	Axial rigidity
	M33	M22	V3	V2	M33	M22	V3	V2			
71	1	1	0	0	1	1	0	0	0	0	Full
72	1	1	0	0	1	1	0	0	0	0	Full
73	1	1	0	0	1	1	0	0	0	0	Full
76	1	1	0	0	1	1	0	0	0	0	Full
77	1	1	0	0	1	1	0	0	0	0	Full
78	1	1	0	0	1	1	0	0	0	0	Full

---

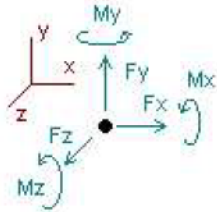
## Connection Check



## Analysis result

### Envelope for nodal reactions

Note.- Ic is the controlling load condition



Direction of positive forces and moments

Envelope of nodal reactions for :

- LC1=1.2DL+W0
- LC2=1.2DL+W30
- LC3=1.2DL-W0
- LC4=1.2DL-W30
- LC5=0.9DL+W0
- LC6=0.9DL+W30
- LC7=0.9DL-W0
- LC8=0.9DL-W30
- LC9=1.2DL+Di+Wi0
- LC10=1.2DL+Di+Wi30
- LC11=1.2DL+Di-Wi0
- LC12=1.2DL+Di-Wi30
- LC13=1.4DL
- LC16=1.2DL

Node		Forces						Moments					
		Fx	Ic	Fy	Ic	Fz	Ic	Mx	Ic	My	Ic	Mz	Ic
		[Kip]		[Kip]		[Kip]		[Kip*ft]		[Kip*ft]		[Kip*ft]	
4	Max	2.633	LC2	2.102	LC12	2.249	LC1	1.64140	LC5	5.72379	LC6	3.05742	LC8
	Min	-2.583	LC8	0.744	LC5	-2.144	LC7	-6.24962	LC3	-5.75099	LC4	-3.68527	LC2
75	Max	3.184	LC2	2.001	LC9	1.984	LC5	5.01998	LC1	6.28569	LC8	5.93051	LC4
	Min	-3.127	LC8	0.701	LC7	-2.077	LC3	-3.24223	LC7	-6.37136	LC2	-1.70921	LC6
108	Max	1.793	LC6	2.001	LC11	3.448	LC5	5.19062	LC1	7.17530	LC5	2.59886	LC8
	Min	-1.901	LC4	0.701	LC8	-3.460	LC3	-2.32653	LC7	-7.24682	LC3	-6.23440	LC2



Site Number: CT2267  
TEP No. 316500.928356  
Analysis By: KM 2/16/2024  
Checked By: MSC 2/16/2024

## Moment Bolt Group - Plate Connection to Collar

Code Revisions:	ANSI/TIA-222-H
Bolt Type:	Headed Bolts

### Connection Inputs:

Bolt Size:	0.750	in
# Bolts:	4	
Plate Width:	8.00	in
Plate Height:	8.00	in
Bolt H Gap:	6.00	in
Bolt V Gap:	6.00	in
Plate T:	0.625	in
Slip Member $\emptyset$ :	N/A	in
Bolt Grade:	A325N	

### Capacities:

Bolt Capacity=	53.3%	PASS*
Plate Capacity=	99.0%	PASS*

\*Value Adjusted per TIA-H Section 15.5

### Bolt Properties:

$F_{y\text{bolt}}$ :	92.0	ksi
$F_{u\text{bolt}}$ :	120.0	ksi
r:	4.2	in
J:	72.0	in <sup>4</sup> /in <sup>2</sup>
$A_{\text{bolt}}$ :	0.4	in <sup>2</sup>
$A_{\text{bolt, Net Tensile}}$ :	0.3	in <sup>2</sup>
Pretension:	28.1	kips

### Member Properties:

Member Shape:	Flat	
Plate $F_y$ :	36.0	ksi
Plate $F_u$ :	60.0	ksi
Member Height:	4.0	in
Member Width:	4.0	in

### Member Forces:

Reaction in X Direction:	2.633	kips
Reaction in Y Direction:	2.102	kips
Reaction in Z Direction:	2.249	kips
Moment in X Axis:	6.25	kip-ft
Moment in Y Axis:	5.751	kip-ft
Moment in Z Axis:	3.685	kip-ft

**PROJECT INFORMATION**

SCOPE OF WORK:

- NEW AT&T ANTENNAS: AIR6419 B77G (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T ANTENNAS: AIR6449 B77D (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T ANTENNAS: TPA65R-BU8DA-K (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T ANTENNAS: DMP65R-BU8DA (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T SURGE ARRESTOR: DC6-48-60-18-8C (TOTAL OF 1) W/ (1) FIBER & (2) AWG6 DC TRUNK.
- NEW AT&T TMA'S: TMA2124F03V5-2D (TYP OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T TMA'S: TMA2104F00V1-1 (TYP OF 2 PER SECTOR, TOTAL OF 6).

ITEMS TO BE MOUNTED AT EQUIPMENT LOCATION:

- ADD (1) 6648 + XCEDE CABLE
- NEW AT&T RRUS: 4478 B14 (700) (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T RRUS: 8843 B2/B66A (AWS/PCS) (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T RRUS: 4449 B5/B12 (700/850) (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- NEW AT&T DIPLEXERS: DBC0115F1V91-2 (TYP. OF 2 PER SECTOR, TOTAL OF 6).
- NEW AT&T TRIPLEXERS: CBC61923T-DS-43 (TYP. OF 2 PER SECTOR, TOTAL OF 6).
- NEW AT&T (12) SMART BIAS TEE.
- ADD (5) RECTIFIERS.
- ADD (2) DC12.

ITEMS TO BE REMOVED:

- EXISTING AT&T LTE ANTENNA: SBNH-1D6565C (TYP. OF 3 PER SECTOR, TOTAL OF 9).
- EXISTING AT&T LTE ANTENNA: OPA-65R-LCUU-H8 (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- EXISTING AT&T TMA: DTMAP7819VG12A (TYP. OF 4 PER SECTOR, TOTAL OF 12).
- EXISTING AT&T DIPLEXERS: CM1007-DBPXC-003 (TYP. OF 6 PER SECTOR, TOTAL OF 24).
- EXISTING AT&T RRUS: RRUS-11 B12 (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- EXISTING AT&T RRUS: RRUS-12 B2+A2 (TYP. OF 1 PER SECTOR, TOTAL OF 3).
- DECOMM UMTS & LINE COMPONENTS.

ITEMS TO REMAIN:

- (24) COAX CABLES

SITE ADDRESS: 298 RIDGE ROAD  
ORANGE, CT 06477

LATITUDE: 41.2584600° N, 41° 15' 30.456" N

LONGITUDE: 73.0442600° W, 73° 2' 39.336" W

TYPE OF SITE: UTILITY POLE / INDOOR EQUIPMENT

STRUCTURE HEIGHT: 96'-0"±

RAD CENTER: 111'-0"± (LTE), 113'-8"± (DOD), 110'-0"± (C-BAND)

CURRENT USE: TELECOMMUNICATIONS FACILITY

PROPOSED USE: TELECOMMUNICATIONS FACILITY

**DRAWING INDEX**

SHEET NO.	DESCRIPTION	REV.
T-1	TITLE SHEET	3
GN-1	GENERAL NOTES	3
A-1	COMPOUND & EQUIPMENT PLANS	3
A-2	ANTENNA LAYOUTS & ELEVATION	3
A-3	DETAILS	3
SN-1	STRUCTURAL NOTES	3
S-1	MOUNT MODIFICATION DESIGN	3
G-1	GROUNDING DETAILS	3
RF-1	RF PLUMBING DIAGRAM	3

NOTE TO GENERAL CONTRACTOR: (PRIOR/DURING CONSTRUCTION)

CONTRACTOR TO CONTACT E.O.R. (TEP NORTHEAST, TEP OPCO,LLC) PRIOR TO ROOF/WALL OPENINGS TO COORDINATE/SCHEDULE THE FOLLOWING:

- INSPECTION OF EXISTING CONDITIONS AND LOCATIONS WHERE CONNECTIONS ARE BEING PROPOSED, INCLUDING INSPECTIONS OF STUB-UP ANCHORS AND/OR WALL ANCHORS PRIOR TO CONCEALING.



**AT&T**

**SITE NUMBER: CTL02267**

**SITE NAME: ORANGE CT RIDGE ROAD**

**FA CODE: 10141359**

**EVERSOURCE STRUCTURE: 3848**

**PACE ID: MRCTB052645,MRCTB051144,MRCTB050790,MRCTB051256,MRCTB050915,MRCTB051353,MRCTB051426, MRCTB051476**

**PROJECT: 5G NR 1SR CBAND ANTENNA MODIFICATION II 4TX4RX SOFTWARE RETROIT UPGRADE**

**VICINITY MAP**

DIRECTIONS TO SITE:

START OUT GOING EAST ON ENTERPRISE DR TOWARD CAPITAL BLVD. TURN LEFT ONTO CAPITAL BLVD. TURN LEFT ONTO WEST ST. MERGE ONTO I-91 S VIA THE RAMP ON THE LEFT TOWARD NEW HAVEN. MERGE ONTO CT-15 S VIA EXIT 17 TOWARD E MAIN ST. TAKE THE CT-121 EXIT, EXIT 56, TOWARD ORANGE. STAY STRAIGHT TO GO ONTO TURKEY HILL RD. TURN RIGHT ONTO GRASSY HILL RD/CT-121. TURN LEFT ONTO CLARK LN. CLARK LN IS 0.4 MILES PAST SPORTSMAN RD. TURN RIGHT ONTO RIDGE RD. RIDGE RD IS JUST PAST RIDGEVIEW RD. 298 RIDGE RD, ORANGE, CT 06477-IS ON THE RIGHT.



**GENERAL NOTES**

1. THIS DOCUMENT IS THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF AT&T. ANY DUPLICATION OR USE WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED. DUPLICATION AND USE BY GOVERNMENT AGENCIES FOR THE PURPOSES OF CONDUCTING THEIR LAWFULLY AUTHORIZED REGULATORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.
2. THE FACILITY IS AN UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ACCESSED BY TRAINED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES NOT REQUIRE ANY WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.
3. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE AT&T MOBILITY REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
4. CONSTRUCTION DRAWINGS ARE VALID FOR SIX MONTHS AFTER ENGINEER OF RECORD'S STAMPED AND SIGNED SUBMITTAL DATE LISTED HEREIN.

**72 HOURS**



**CALL BEFORE YOU DIG**



**CALL TOLL FREE 1-800-922-4455**  
**OR CALL 811**

**UNDERGROUND SERVICE ALERT**



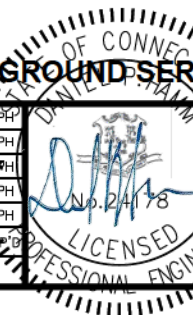
**SITE NUMBER: CTL02267**  
**SITE NAME: ORANGE CT RIDGE ROAD**

298 RIDGE ROAD  
ORANGE, CT 06477  
NEW HAVEN COUNTY



84 DEERFIELD LANE  
MERIDEN, CT 06450

NO.	DATE	REVISIONS	BY	CHK	APP'D
3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	OPPH
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	OPPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	OPPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	OPPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	OPPH



<b>AT&amp;T</b>		
<b>TITLE SHEET</b>		
<b>5G NR 1SR CBAND ANTENNA MODIFICATION II 4TX4RX SOFTWARE RETROIT UPGRADE</b>		
SITE NUMBER	DRAWING NUMBER	REV
CTL02267	T-1	3

SCALE: AS SHOWN    DESIGNED BY: HC    DRAWN BY: MR

**GROUNDING NOTES**

1. THE SUBCONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE-SPECIFIC (UL, LPI, OR NFPA) LIGHTING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE SUBCONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE CONTRACTOR FOR RESOLUTION.
2. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
3. THE SUBCONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81 STANDARDS) FOR NEW GROUND ELECTRODE SYSTEMS. THE SUBCONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
4. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
5. EACH BTS CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 AWG STRANDED COPPER OR LARGER FOR INDOOR BTS AND #2 AWG STRANDED COPPER FOR OUTDOOR BTS.
6. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
7. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
8. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO GROUND BAR.
9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
10. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
11. METAL CONDUIT SHALL BE MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 AWG COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
12. ALL NEW STRUCTURES WITH A FOUNDATION AND/OR FOOTING HAVING 20 FT. OR MORE OF 1/2 IN. OR GREATER ELECTRICALLY CONDUCTIVE REINFORCING STEEL MUST HAVE IT BONDED TO THE GROUND RING USING AN EXOTHERMIC WELD CONNECTION USING #2 AWG SOLID BARE TINNED COPPER GROUND WIRE, PER NEC 250.50

**GENERAL NOTES**

1. FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:  
 CONTRACTOR – SAI  
 SUBCONTRACTOR – GENERAL CONTRACTOR (CONSTRUCTION)  
 OWNER – AT&T MOBILITY
2. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.
3. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
4. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.
5. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
6. "KITTING LIST" SUPPLIED WITH THE BID PACKAGE IDENTIFIES ITEMS THAT WILL BE SUPPLIED BY CONTRACTOR. ITEMS NOT INCLUDED IN THE BILL OF MATERIALS AND KITTING LIST SHALL BE SUPPLIED BY THE SUBCONTRACTOR.
7. THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
8. IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION SPACE FOR APPROVAL BY THE CONTRACTOR.
9. SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR.
10. THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
12. SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION.
13. ALL CONCRETE REPAIR WORK SHALL BE DONE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301.

14. ANY NEW CONCRETE NEEDED FOR THE CONSTRUCTION SHALL BE AIR-ENTRAINED AND SHALL HAVE 4000 PSI STRENGTH AT 28 DAYS. ALL CONCRETE WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS.
15. ALL STRUCTURAL STEEL WORK SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS. ALL STRUCTURAL STEEL SHALL BE ASTM A36 (Fy = 36 ksi) UNLESS OTHERWISE NOTED. PIPES SHALL BE ASTM A53 TYPE E (Fy = 36 ksi). ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED. TOUCH UP ALL SCRATCHES AND OTHER MARKS IN THE FIELD AFTER STEEL IS ERECTED USING A COMPATIBLE ZINC RICH PAINT.
16. CONSTRUCTION SHALL COMPLY WITH SPECIFICATIONS AND "GENERAL CONSTRUCTION SERVICES FOR CONSTRUCTION OF AT&T SITES."
17. SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.
18. THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.
19. SINCE THE CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE ADVISED TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.
20. **APPLICABLE BUILDING CODES:**  
 SUBCONTRACTOR'S WORK SHALL COMPLY WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AS ADOPTED BY THE LOCAL AUTHORITY HAVING JURISDICTION (AHJ) FOR THE LOCATION. THE EDITION OF THE AHJ ADOPTED CODES AND STANDARDS IN EFFECT ON THE DATE OF CONTRACT AWARD SHALL GOVERN THE DESIGN.

**BUILDING CODE: IBC 2021 WITH 2022 CT STATE BUILDING CODE AMENDMENTS  
 ELECTRICAL CODE: 2020 NATIONAL ELECTRICAL CODE (NFPA 70-2020)**

SUBCONTRACTOR'S WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS:

**AMERICAN CONCRETE INSTITUTE (ACI) 318; BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE;**

**AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) MANUAL OF STEEL CONSTRUCTION, ASD, FOURTEENTH EDITION;**

**TELECOMMUNICATIONS INDUSTRY ASSOCIATION (TIA) 222-H, STRUCTURAL STANDARDS FOR STEEL**

FOR ANY CONFLICTS BETWEEN SECTIONS OF LISTED CODES AND STANDARDS REGARDING MATERIAL, METHODS OF CONSTRUCTION, OR OTHER REQUIREMENTS, THE MOST RESTRICTIVE REQUIREMENT SHALL GOVERN. WHERE THERE IS CONFLICT BETWEEN A GENERAL REQUIREMENT AND A SPECIFIC REQUIREMENT, THE SPECIFIC REQUIREMENT SHALL GOVERN.

**ABBREVIATIONS**

AGL	ABOVE GRADE LEVEL	EQ	EQUAL	REQ	REQUIRED
AWG	AMERICAN WIRE GAUGE	GC	GENERAL CONTRACTOR	RF	RADIO FREQUENCY
BBU	BATTERY BACKUP UNIT	GRC	GALVANIZED RIGID CONDUIT	TBD	TO BE DETERMINED
BTCW	BARE TINNED SOLID COPPER WIRE	MGB	MASTER GROUND BAR	TBR	TO BE REMOVED
BGR	BURIED GROUND RING	MIN	MINIMUM	TBRR	TO BE REMOVED AND REPLACED
BTS	BASE TRANSCEIVER STATION	P	PROPOSED	TYP	TYPICAL
E	EXISTING	NTS	NOT TO SCALE	UG	UNDER GROUND
EGB	EQUIPMENT GROUND BAR	RAR	REFERENCE CENTER LINE	VIF	VERIFY IN FIELD
EGR	EQUIPMENT GROUND RING		REFERENCE		



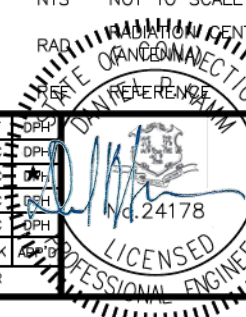
**SITE NUMBER: CTL02267  
 SITE NAME: ORANGE CT RIDGE ROAD**

**298 RIDGE ROAD  
 ORANGE, CT 06477  
 NEW HAVEN COUNTY**



3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	OPH
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	OPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	OPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	OPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	OPH
NO.	DATE	REVISIONS	BY	CHK	APP'D

SCALE: AS SHOWN DESIGNED BY: HC DRAWN BY: MR



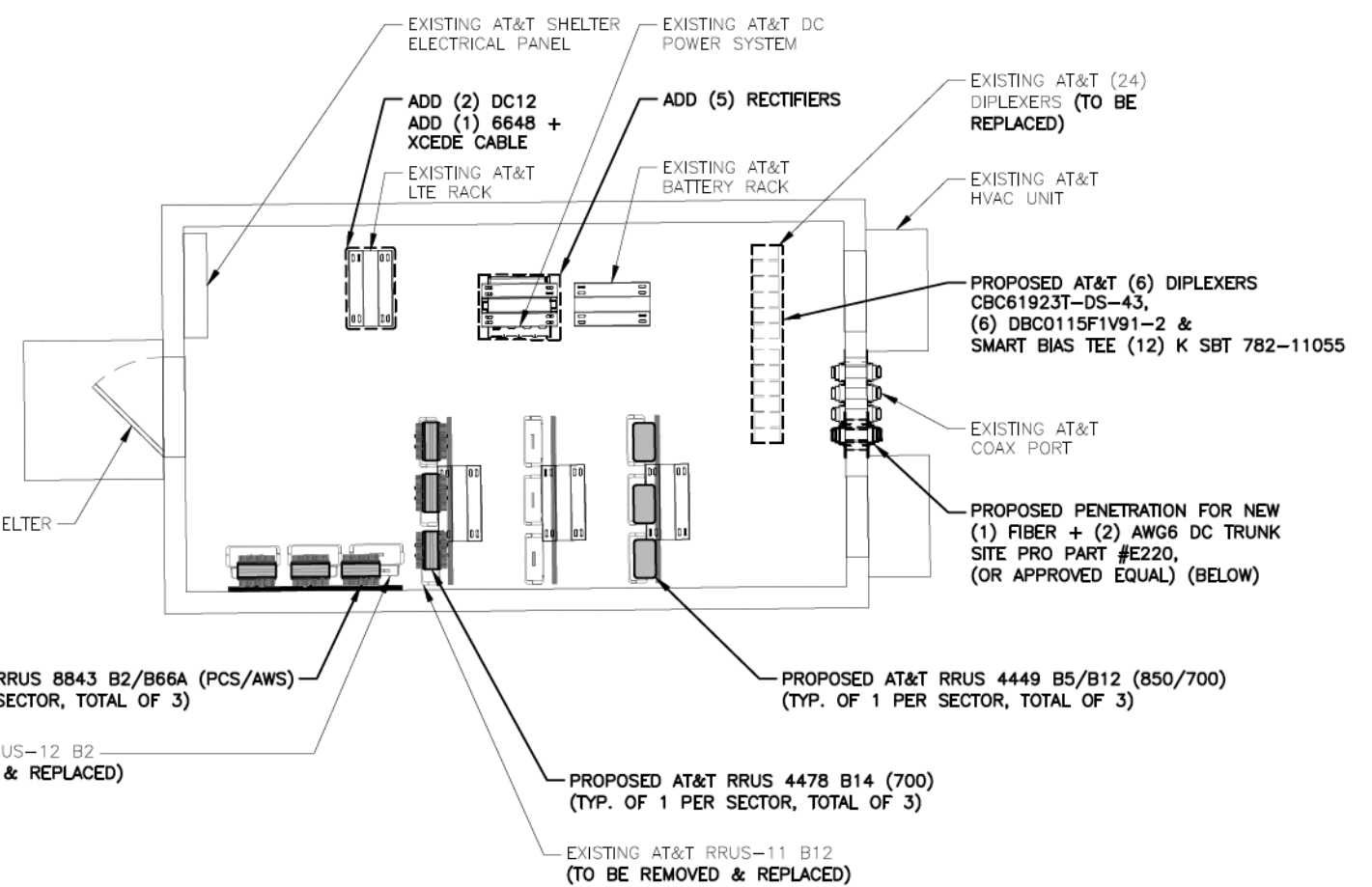
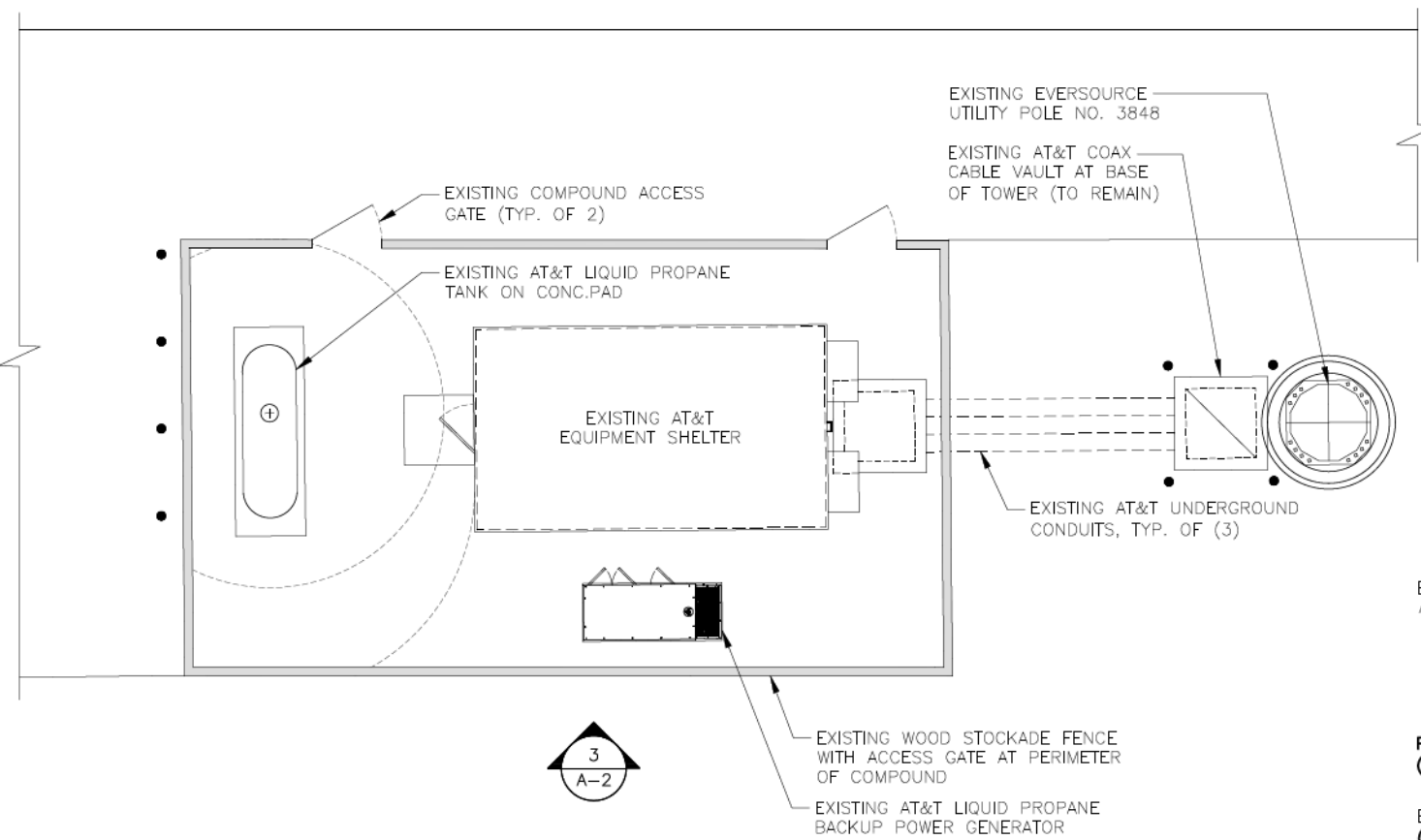
<b>AT&amp;T</b>		
<b>GENERAL NOTES              5G NR 1SR CBAND_ANTENNA MODIFICATION II 4TX4RX              SOFTWARE RETROFIT UPGRADE</b>		
SITE NUMBER	DRAWING NUMBER	REV
CTL02267	GN-1	3



NOTE:  
REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

NOTE:  
AN ANALYSIS FOR THE CAPACITY OF THE EXISTING ANTENNA MOUNT TO SUPPORT THE PROPOSED LOADING HAS BEEN COMPLETED BY:  
TEP NORTHEAST (TEP OPCO, LLC.)  
DATED: FEBRUARY 16, 2024 (REV.1)

NOTE:  
REFER TO STRUCTURAL ANALYSIS BY: CENTEK ENGINEERING  
DATED: APRIL 15 2024, (REV.3)  
FOR THE CAPACITY OF THE EXISTING STRUCTURES TO SUPPORT THE PROPOSED EQUIPMENT.



**COMPOUND PLAN**  
22x34 SCALE: 3/16"=1'-0"  
11x17 SCALE: 3/32"=1'-0"  
1  
A-1

**EQUIPMENT PLAN**  
22x34 SCALE: 3/8"=1'-0"  
11x17 SCALE: 3/16"=1'-0"  
2  
A-1

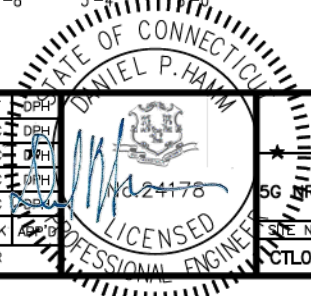


SITE NUMBER: CTL02267  
SITE NAME: ORANGE CT RIDGE ROAD  
298 RIDGE ROAD  
ORANGE, CT 06477  
NEW HAVEN COUNTY

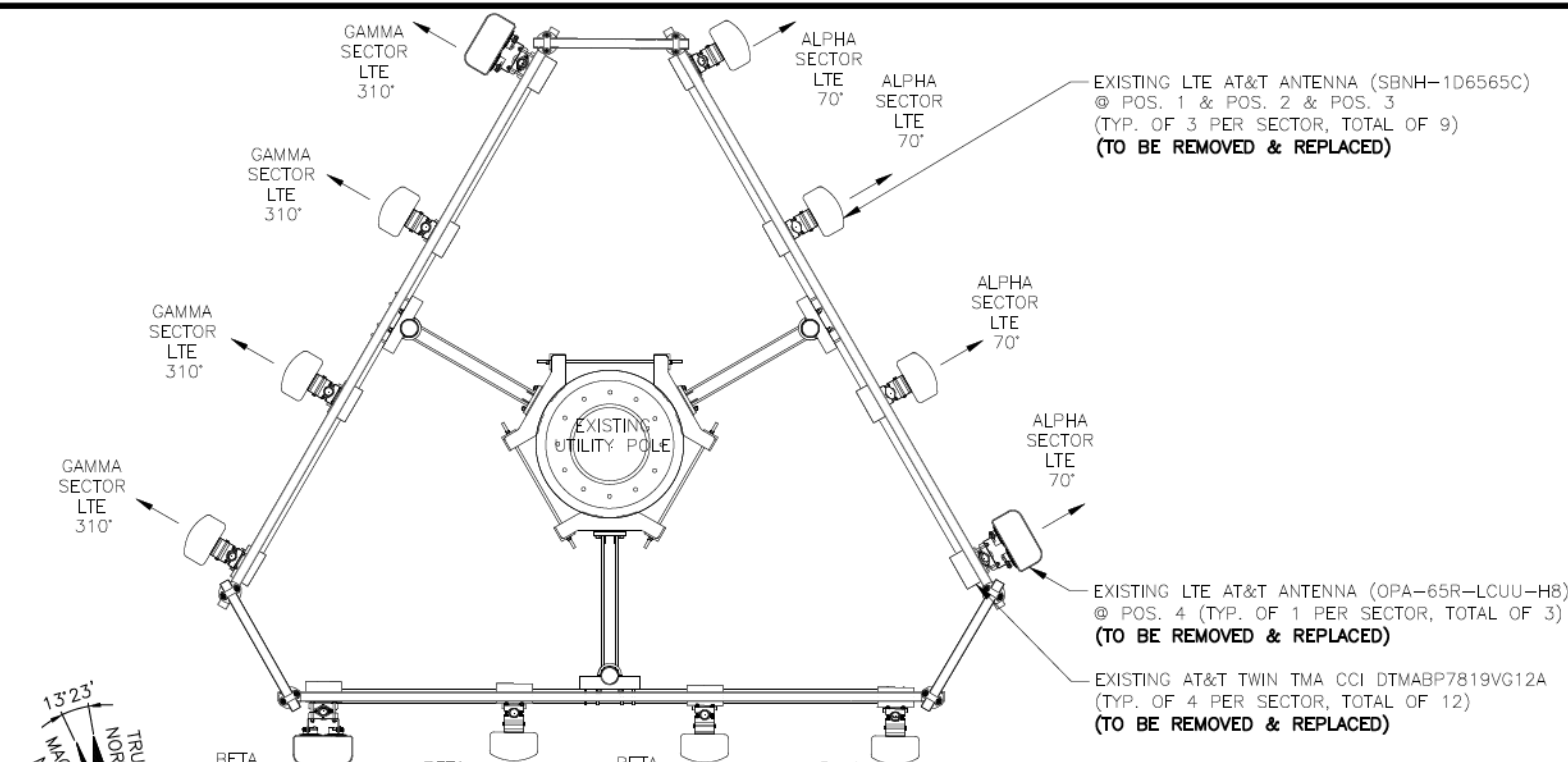


NO.	DATE	REVISIONS	BY	CHK	APP'D
3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	DP/EP
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	DPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	DPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	DPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	DPH

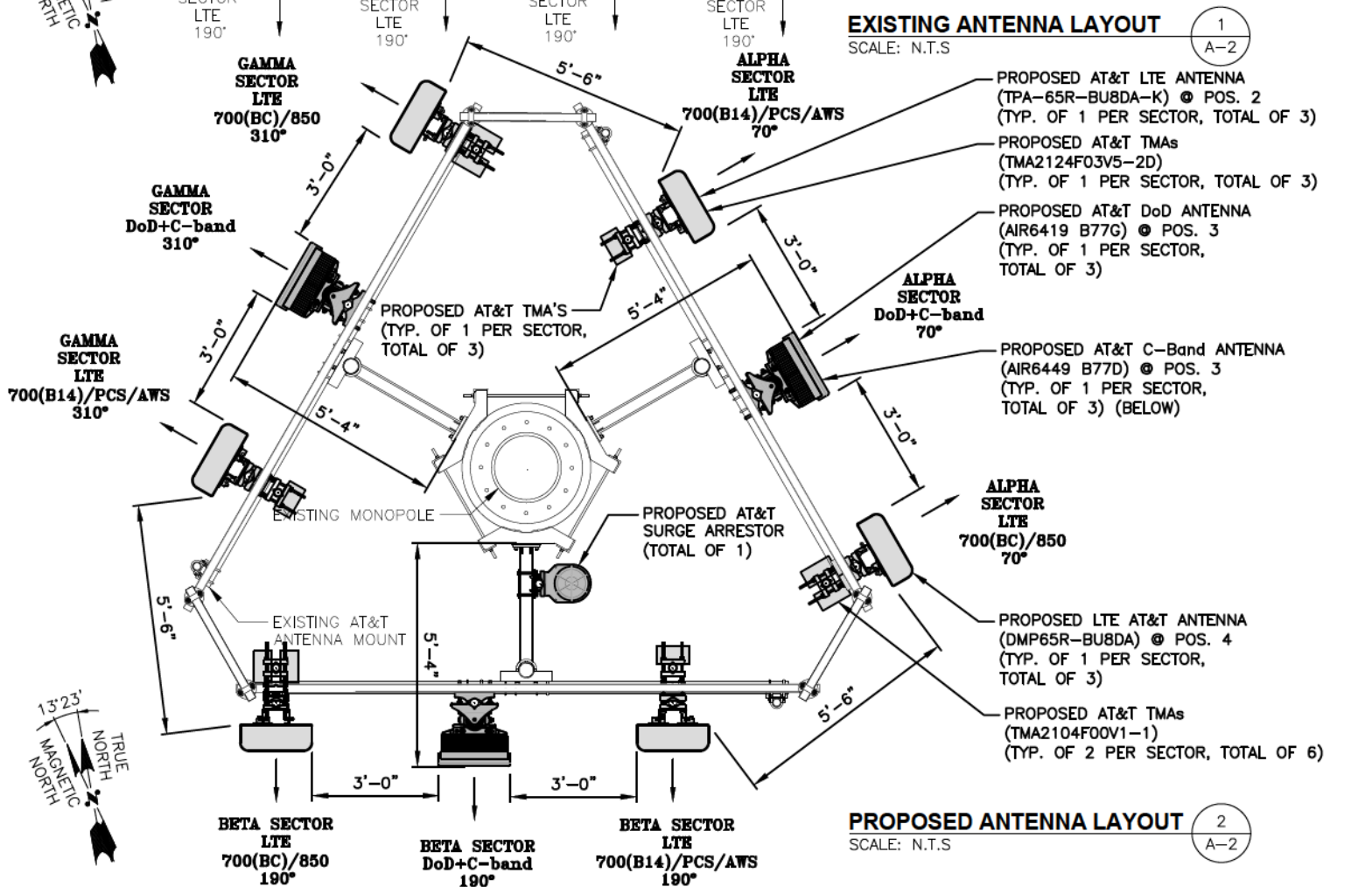
SCALE: AS SHOWN  
DESIGNED BY: HC  
DRAWN BY: MR



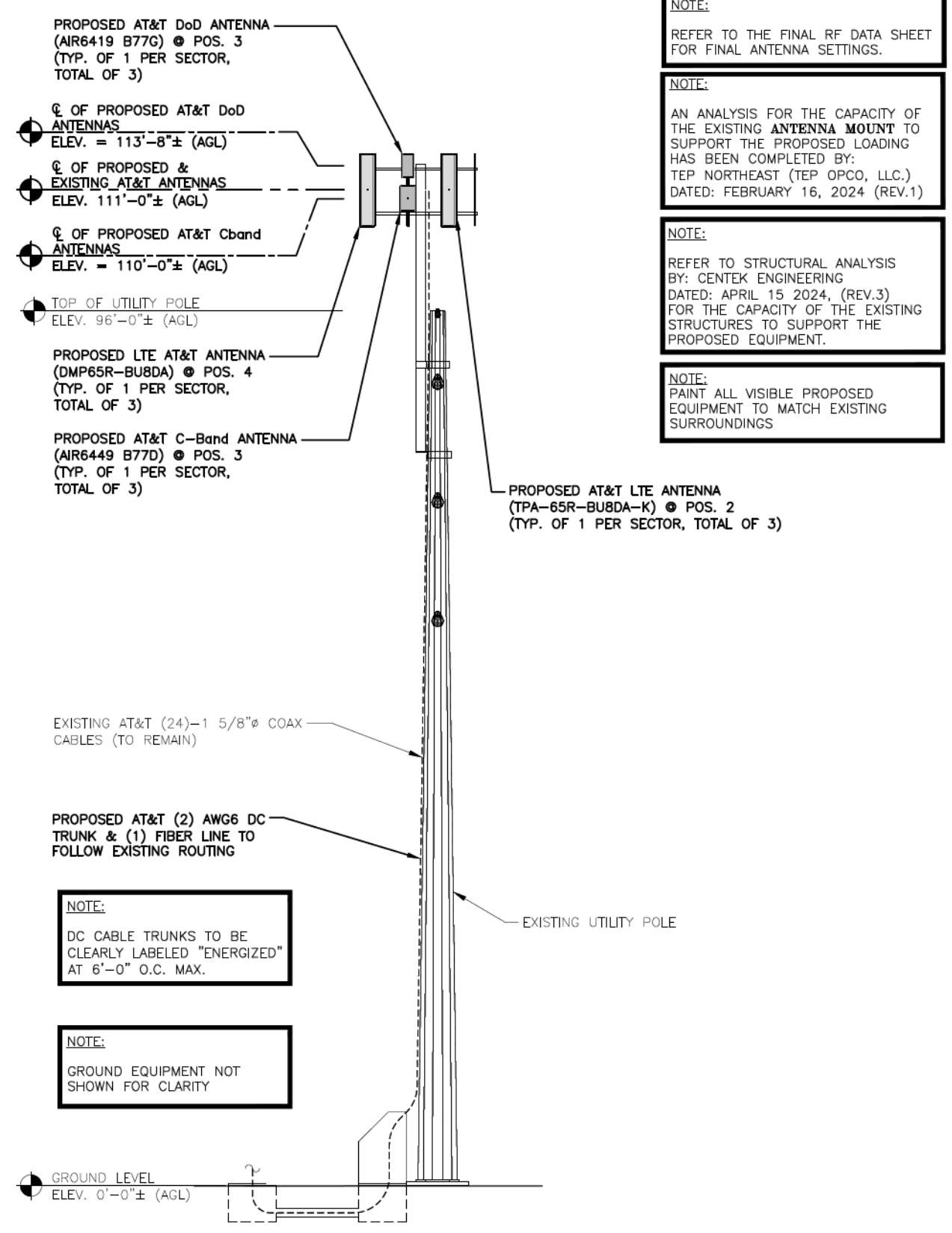
AT&T		
COMPOUND & EQUIPMENT PLANS		
5G NR 1SR CBAND_ANTENNA MODIFICATION II 4TX4RX SOFTWARE RETROFIT UPGRADE		
SITE NUMBER	DRAWING NUMBER	REV
CTL02267	A-1	3



**EXISTING ANTENNA LAYOUT** 1  
SCALE: N.T.S.



**PROPOSED ANTENNA LAYOUT** 2  
SCALE: N.T.S.



**ELEVATION** 3  
22x34 SCALE: 1/8"=1'-0"  
11x17 SCALE: 1/16"=1'-0"

**NOTE:**  
REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

**NOTE:**  
AN ANALYSIS FOR THE CAPACITY OF THE EXISTING ANTENNA MOUNT TO SUPPORT THE PROPOSED LOADING HAS BEEN COMPLETED BY:  
TEP NORTHEAST (TEP OPCO, LLC.)  
DATED: FEBRUARY 16, 2024 (REV.1)

**NOTE:**  
REFER TO STRUCTURAL ANALYSIS BY: CENTEK ENGINEERING  
DATED: APRIL 15 2024, (REV.3)  
FOR THE CAPACITY OF THE EXISTING STRUCTURES TO SUPPORT THE PROPOSED EQUIPMENT.

**NOTE:**  
PAINT ALL VISIBLE PROPOSED EQUIPMENT TO MATCH EXISTING SURROUNDINGS

**NOTE:**  
DC CABLE TRUNKS TO BE CLEARLY LABELED "ENERGIZED" AT 6'-0" O.C. MAX.

**NOTE:**  
GROUND EQUIPMENT NOT SHOWN FOR CLARITY



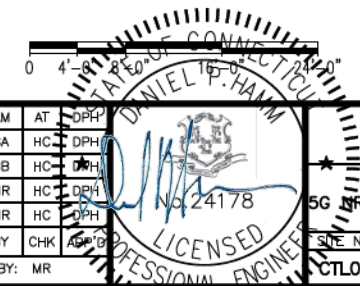
**SITE NUMBER: CTL02267**  
**SITE NAME: ORANGE CT RIDGE ROAD**

298 RIDGE ROAD  
ORANGE, CT 06477  
NEW HAVEN COUNTY



NO.	DATE	REVISIONS	BY	CHK	APP'D
3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	OPPH
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	OPPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	OPPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	OPPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	OPPH

SCALE: AS SHOWN    DESIGNED BY: HC    DRAWN BY: MR



AT&T	
SITE NUMBER	DRAWING NUMBER
CTL02267	A-2
REV	DESCRIPTION
3	ANTENNA LAYOUTS & ELEVATION 5G NR 1SR CBAND_ANTENNA MODIFICATION II 4TX4RX SOFTWARE RETROFIT UPGRADE

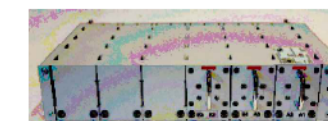
ANTENNA SCHEDULE

SECTOR	EXISTING/PROPOSED	BAND	ANTENNA	SIZE (INCHES) (L x W x D)	ANTENNA HEIGHT	AZIMUTH	TMA/DIPLEXER	RRU	SIZE (INCHES) (L x W x D)	FEEDER	RAYCAP
A1	-	-	-	-	-	-	-	-	-	(8)1-5/8 COAX	(P) (1) RAYCAP DC6-48-60-18-8C
A2	PROPOSED	LTE 700(B14)/ PCS/AWS	TPA65R-BU8DA-K	96X20.7X7.7	111'-0"±	70°	(P)(1) TMA2124F03V5-2D	(P)(G)(1) 4478 B14 (700) (P)(G)(1) 8843 B2/B66A (AWS/PCS)	18.1"x13.4"x8.3" 14.9"x13.2"x10.9"	(P)(2) DC POWER & (1) FIBER	
A3	PROPOSED	DOD + CBAND	AIR6419 B77G + AIR6449 B77D (STACKED)	31.1X16.1X7.3 30.4X15.9X8.1	113'-0"± 110'-0"±	70°	-	-	-	-	
A4	PROPOSED	LTE 700(BC)/850	DMP65R-BU8DA	96.0X20.7X7.7	111'-0"±	70°	(P)(2) TMA2104F00V1-1	(P)(G)(1) 4449 B5/B12 (850/700)	17.9"x13.2"x10.4"	-	
B1	-	-	-	-	-	-	-	-	-	(8)1-5/8 COAX	1
B2	PROPOSED	LTE 700(B14)/ PCS/AWS	TPA65R-BU8DA-K	96X20.7X7.7	111'-0"±	190°	(P)(1) TMA2124F03V5-2D	(P)(G)(1) 4478 B14 (700) (P)(G)(1) 8843 B2/B66A (AWS/PCS)	18.1"x13.4"x8.3" 14.9"x13.2"x10.9"	-	
B3	PROPOSED	DOD + CBAND	AIR6419 B77G + AIR6449 B77D (STACKED)	31.1X16.1X7.3 30.4X15.9X8.1	113'-0"± 110'-0"±	190°	-	-	-	-	
B4	PROPOSED	LTE 700(BC)/850	DMP65R-BU8DA	96.0X20.7X7.7	111'-0"±	190°	(P)(2) TMA2104F00V1-1	(P)(G)(1) 4449 B5/B12 (850/700)	17.9"x13.2"x10.4"	-	
C1	-	-	-	-	-	-	-	-	-	(8)1-5/8 COAX	1
C2	PROPOSED	LTE 700(B14)/ PCS/AWS	TPA65R-BU8DA-K	96X20.7X7.7	111'-0"±	310°	(P)(1) TMA2124F03V5-2D	(P)(G)(1) 4478 B14 (700) (P)(G)(1) 8843 B2/B66A (AWS/PCS)	18.1"x13.4"x8.3" 14.9"x13.2"x10.9"	-	
C3	PROPOSED	DOD + CBAND	AIR6419 B77G + AIR6449 B77D (STACKED)	31.1X16.1X7.3 30.4X15.9X8.1	113'-0"± 110'-0"±	310°	-	-	-	-	
C4	PROPOSED	LTE 700(BC)/850	DMP65R-BU8DA	96.0X20.7X7.7	111'-0"±	310°	(P)(2) TMA2104F00V1-1	(P)(G)(1) 4449 B5/B12 (850/700)	17.9"x13.2"x10.4"	-	

NOTE:  
REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

NOTE:  
AN ANALYSIS FOR THE CAPACITY OF THE EXISTING ANTENNA MOUNT TO SUPPORT THE PROPOSED LOADING HAS BEEN COMPLETED BY:  
TEP NORTHEAST (TEP OPCO, LLC.)  
DATED: FEBRUARY 16, 2024 (REV.1)

NOTE:  
REFER TO STRUCTURAL ANALYSIS BY: CENTEK ENGINEERING  
DATED: APRIL 15 2024, (REV.3)  
FOR THE CAPACITY OF THE EXISTING STRUCTURES TO SUPPORT THE PROPOSED EQUIPMENT.



NOTE:  
MOUNT PER MANUFACTURER'S SPECIFICATIONS.  
PROPOSED DC12 RAYCAP  
P/N DC12-48-60-RM  
WEIGHT: 15.0 LBS.

DC12 DETAIL 6  
SCALE: N.T.S. A-3

FINAL ANTENNA SCHEDULE 1  
SCALE: N.T.S. A-3

QUANTITY	MODEL	SIZE (L x W x D)
P(3)	4449 B5/B12 (700/850)	17.9"x13.2"x10.4"
P(3)	8843 B2/B66A (AWS/PCS)	14.9"x13.2"x10.9"
P(3)	4478 B14 (700)	18.1"x13.4"x8.3"

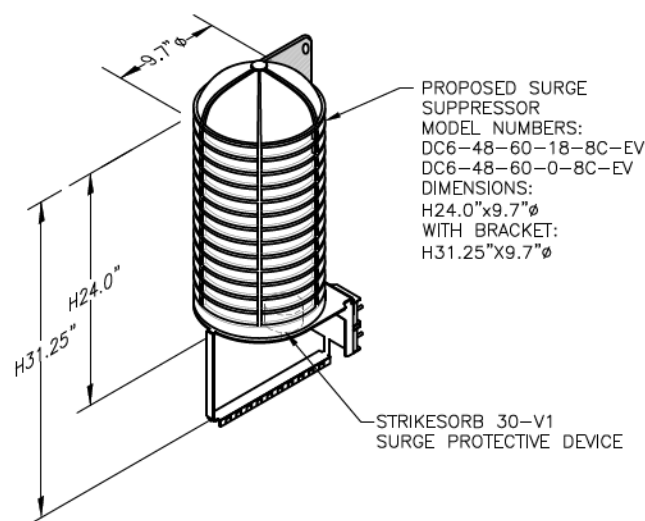
NOTE:  
MOUNT PER MANUFACTURER'S SPECIFICATIONS

NOTE:  
SEE RFDS FOR RRH FREQUENCY AND MODEL NUMBER

PROPOSED RRU REFER TO THE FINAL RFDS AND CHART FOR QUANTITY, MODEL AND DIMENSIONS

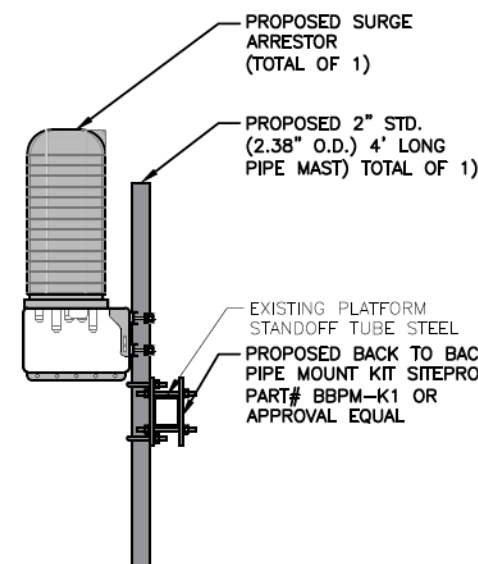
NOTE:  
MOUNT PER MANUFACTURER'S SPECIFICATIONS.

PROPOSED RRUS DETAIL 2  
SCALE: N.T.S. A-3

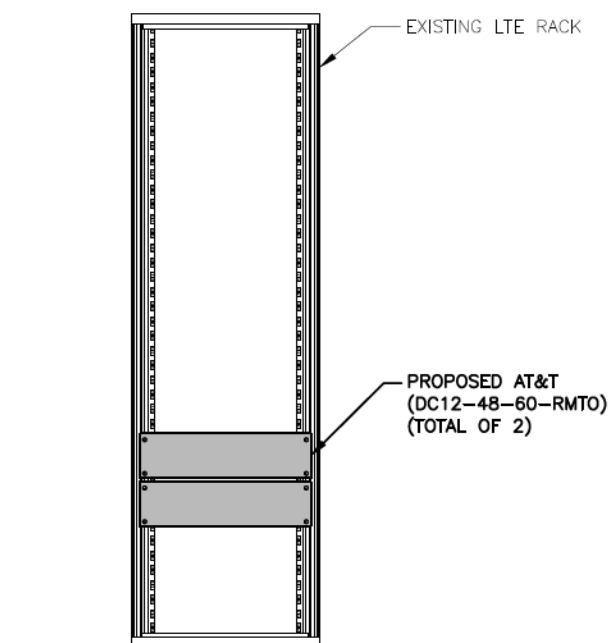


NOTE:  
MOUNT PER MANUFACTURER'S SPECIFICATIONS.

DC SURGE SUPPRESSOR DETAIL 3  
SCALE: N.T.S. A-3



PROPOSED SURGE ARRESTOR MOUNTING DETAIL 4  
SCALE: N.T.S. A-5



PROPOSED DC12 MOUNTING DETAIL 5  
SCALE: N.T.S. A-5



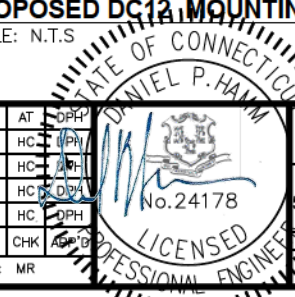
SITE NUMBER: CTL02267  
SITE NAME: ORANGE CT RIDGE ROAD

298 RIDGE ROAD  
ORANGE, CT 06477  
NEW HAVEN COUNTY



NO.	DATE	REVISIONS	BY	CHK	APP'D
3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	DPH
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	DPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	DPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	DPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	DPH

SCALE: AS SHOWN DESIGNED BY: HC DRAWN BY: MR



SITE NUMBER	DRAWING NUMBER	REV
CTL02267	A-3	3

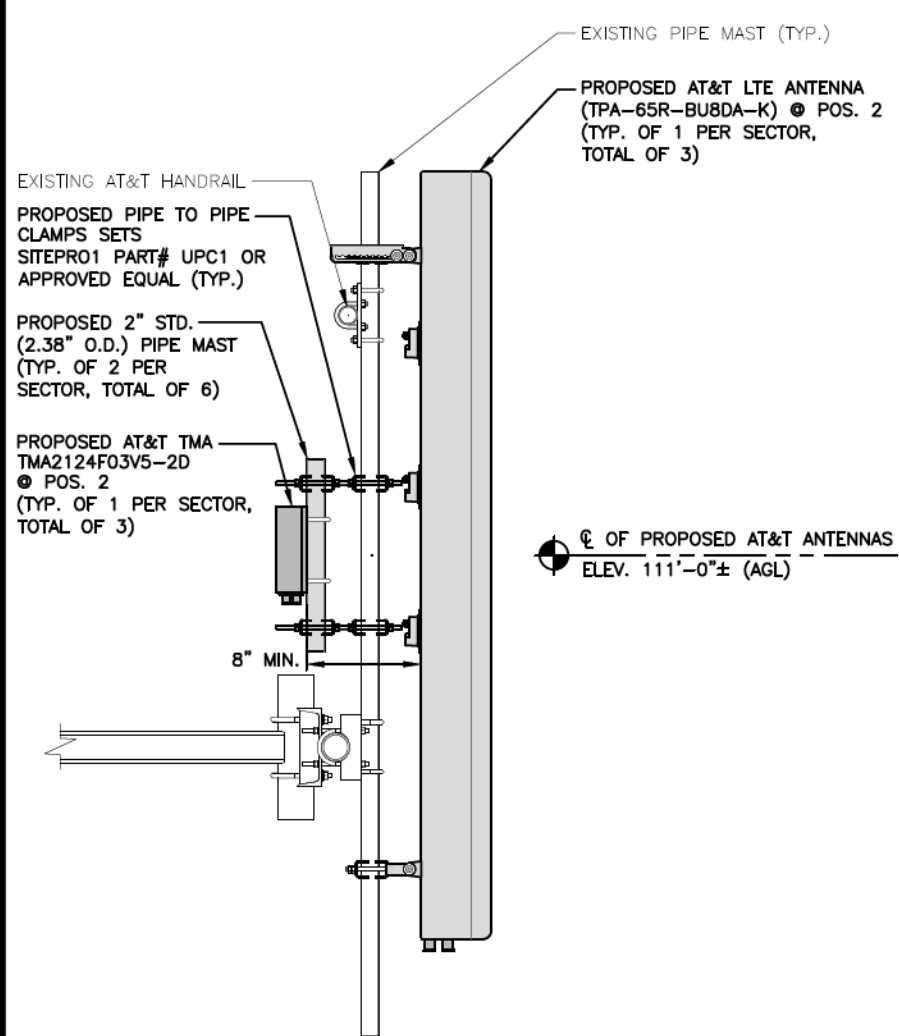
AT&T  
DETAILS  
5G NR 1SR CBAND ANTENNA MODIFICATION II 4TX4RX  
SOFTWARE RETROFIT UPGRADE

**NOTE:**  
REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

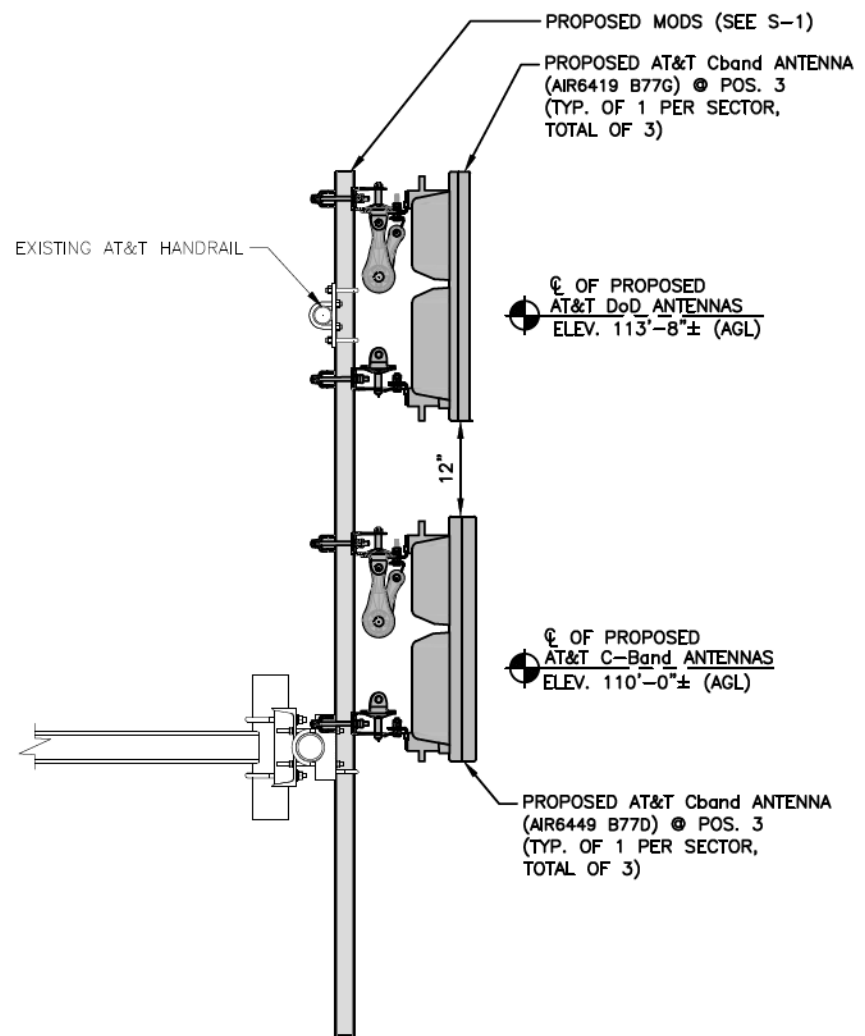
**NOTE:**  
AN ANALYSIS FOR THE CAPACITY OF THE EXISTING ANTENNA MOUNT TO SUPPORT THE PROPOSED LOADING HAS BEEN COMPLETED BY:  
TEP NORTHEAST (TEP OPCO, LLC.)  
DATED: FEBRUARY 16, 2024 (REV.1)

**NOTE:**  
REFER TO STRUCTURAL ANALYSIS BY: CENTEK ENGINEERING  
DATED: APRIL 15 2024, (REV.3)  
FOR THE CAPACITY OF THE EXISTING STRUCTURES TO SUPPORT THE PROPOSED EQUIPMENT.

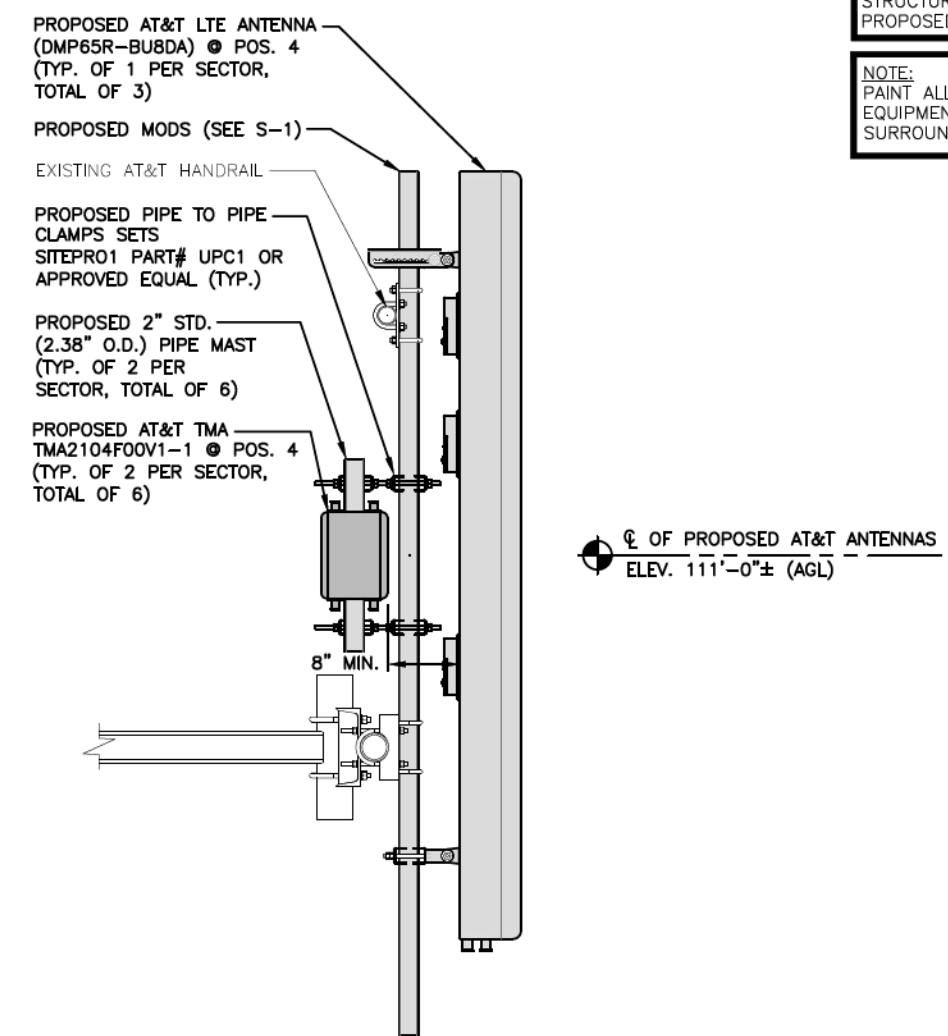
**NOTE:**  
PAINT ALL VISIBLE PROPOSED EQUIPMENT TO MATCH EXISTING SURROUNDINGS



**PROPOSED LTE ANTENNA MOUNTING DETAIL**  
22x34 SCALE: 1"=1'-0"  
11x17 SCALE: 1/2"=1'-0"  
1  
A-4



**PROPOSED DoD + C-BAND ANTENNA MOUNTING DETAIL**  
22x34 SCALE: 1"=1'-0"  
11x17 SCALE: 1/2"=1'-0"  
2  
A-4



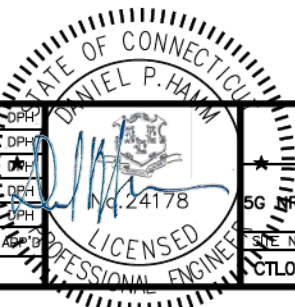
**PROPOSED LTE ANTENNA MOUNTING DETAIL**  
22x34 SCALE: 1"=1'-0"  
11x17 SCALE: 1/2"=1'-0"  
3  
A-4



**SITE NUMBER: CTL02267**  
**SITE NAME: ORANGE CT RIDGE ROAD**  
  
298 RIDGE ROAD  
ORANGE, CT 06477  
NEW HAVEN COUNTY



NO.	DATE	REVISIONS	BY	CHK	APP'D
3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	DPH
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	DPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	DPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	DPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	DPH



<b>AT&amp;T</b>		
<b>DETAILS</b>		
<b>5G NR 1SR CBAND ANTENNA MODIFICATION II 4TX4RX SOFTWARE RETROFIT UPGRADE</b>		
SITE NUMBER	DRAWING NUMBER	REV
CTL02267	A-4	3

**STRUCTURAL NOTES:**

- DESIGN REQUIREMENTS ARE PER STATE BUILDING CODE AND APPLICABLE SUPPLEMENTS, INTERNATIONAL BUILDING CODE, EIA/TIA-222-H STRUCTURAL STANDARDS FOR STEEL ANTENNA, TOWERS AND ANTENNA SUPPORTING STRUCTURES.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. ANY UNUSUAL CONDITIONS SHALL BE REPORTED TO THE ATTENTION OF THE CONSTRUCTION MANAGER AND ENGINEER OF RECORD.
- DESIGN AND CONSTRUCTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- STRUCTURAL STEEL SHALL CONFORM TO ASTM A992 (Fy=50 ksi), MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A36 UNLESS OTHERWISE INDICATED.
- STEEL PIPE SHALL CONFORM TO ASTM A500 "COLD-FORMED WELDED & SEAMLESS CARBON STEEL STRUCTURAL TUBING", GRADE B, OR ASTM A53 PIPE STEEL BLACK AND HOT-DIPPED ZINC-COATED WELDED AND SEAMLESS TYPE E OR S, GRADE B. PIPE SIZES INDICATED ARE NOMINAL. ACTUAL OUTSIDE DIAMETER IS LARGER.
- STRUCTURAL CONNECTION BOLTS SHALL BE HIGH STRENGTH BOLTS (BEARING TYPE) AND CONFORM TO ASTM A325 TYPE-X "HIGH STRENGTH BOLTS FOR STRUCTURAL JOINTS, INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS". ALL BOLTS SHALL BE 3/4" DIA UON.
- ALL STEEL MATERIALS SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS", UNLESS OTHERWISE NOTED.
- ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC-COATING (HOT-DIP) ON IRON AND STEEL HARDWARE", UNLESS OTHERWISE NOTED.
- FIELD WELDS, DRILL HOLES, SAW CUTS AND ALL DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED WITH AN ORGANIC ZINC REPAIR PAINT COMPLYING WITH REQUIREMENTS OF ASTM A780. GALVANIZING REPAIR PAINT SHALL HAVE 65 PERCENT ZINC BY WEIGHT, ZIRP BY DUNCAN GALVANIZING, GALVA BRIGHT PREMIUM BY CROWN OR EQUAL. THICKNESS OF APPLIED GALVANIZING REPAIR PAINT SHALL BE NOT LESS THAN 4 COATS (ALLOW TIME TO DRY BETWEEN COATS) WITH A RESULTING COATING THICKNESS REQUIRED BY ASTM A123 OR A153 AS APPLICABLE.
- CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES, APPEARANCE AND QUALITY OF WELDS, AND FOR METHODS USED IN CORRECTING WELDING. ALL WELDERS AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES". ALL WELDING SHALL BE DONE USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC AND D.I. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "STEEL CONSTRUCTION MANUAL". 14TH EDITION.
- INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NON-CONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE CONSTRUCTION MANAGER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE CONSTRUCTION MANAGER APPROVAL.
- UNISTRUT SHALL BE FORMED STEEL CHANNEL STRUT FRAMING AS MANUFACTURED BY UNISTRUT CORP., WAYNE, MI OR EQUAL. STRUT MEMBERS SHALL BE 1 5/8"x1 5/8"x12GA, UNLESS OTHERWISE NOTED, AND SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION.
- EPOXY ANCHOR ASSEMBLY SHALL CONSIST OF STAINLESS STEEL ANCHOR ROD WITH NUTS & WASHERS. AN INTERNALLY THREADED INSERT, A SCREEN TUBE AND A EPOXY ADHESIVE. THE ANCHORING SYSTEM SHALL BE THE HILTI-HIT HY-270 AND OR HY-200 SYSTEMS (AS SPECIFIED IN DWG.) OR ENGINEERS APPROVED EQUAL.
- EXPANSION BOLTS SHALL CONFORM TO FEDERAL SPECIFICATION FF-S-325, GROUP II, TYPE 4, CLASS I, HILTI KWIK BOLT III OR APPROVED EQUAL. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- LUMBER SHALL COMPLY WITH THE REQUIREMENTS OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION AND THE NATIONAL FOREST PRODUCTS ASSOCIATION'S NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION. ALL LUMBER SHALL BE PRESSURE TREATED AND SHALL BE STRUCTURAL GRADE NO. 2 OR BETTER.
- WHERE ROOF PENETRATIONS ARE REQUIRED, THE CONTRACTOR SHALL CONTACT AND COORDINATE RELATED WORK WITH THE BUILDING OWNER AND THE EXISTING ROOF INSTALLER. WORK SHALL BE PERFORMED IN SUCH A MANNER AS TO NOT VOID THE EXISTING ROOF WARRANTY. ROOF SHALL BE WATERTIGHT.
- ALL FIBERGLASS MEMBERS USED ARE AS MANUFACTURED BY STRONGWELL COMPANY OF BRISTOL, VA 24203. ALL DESIGN CRITERIA FOR THESE MEMBERS IS BASED ON INFORMATION PROVIDED IN THE DESIGN MANUAL. ALL REQUIREMENTS PUBLISHED IN SAID MANUAL MUST BE STRICTLY ADHERED TO.
- NO MATERIALS TO BE ORDERED AND NO WORK TO BE COMPLETED UNTIL SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED IN WRITING.
- SUBCONTRACTOR SHALL FIREPROOF ALL STEEL TO PRE-EXISTING CONDITIONS.

**SPECIAL INSPECTIONS (REFERENCE IBC CHAPTER 17):**

**GENERAL:** WHERE APPLICATION IS MADE FOR CONSTRUCTION, THE OWNER OR THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE ACTING AS THE OWNER'S AGENT SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PERFORM INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE INSPECTION CHECKLIST ABOVE.

THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND ENGINEERS OF RECORD INVOLVED IN THE DESIGN OF THE PROJECT ARE PERMITTED TO ACT AS THE APPROVED AGENCY AND THEIR PERSONNEL ARE PERMITTED TO ACT AS THE SPECIAL INSPECTOR FOR THE WORK DESIGNED BY THEM, PROVIDED THOSE PERSONNEL MEET THE QUALIFICATION REQUIREMENTS.

STATEMENT OF SPECIAL INSPECTIONS: THE APPLICANT SHALL SUBMIT A STATEMENT OF SPECIAL INSPECTIONS PREPARED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE IN ACCORDANCE WITH SECTION 107.1 AS A CONDITION FOR ISSUANCE. THIS STATEMENT SHALL BE IN ACCORDANCE WITH SECTION 1705.

REPORT REQUIREMENT: SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS SHALL BE SUBMITTED.

**SPECIAL INSPECTION CHECKLIST**

**BEFORE CONSTRUCTION**

CONSTRUCTION/INSTALLATION INSPECTIONS AND TESTING REQUIRED (COMPLETED BY ENGINEER OF RECORD)	REPORT ITEM
N/A	ENGINEER OF RECORD APPROVED SHOP DRAWINGS <sup>1</sup>
N/A	MATERIAL SPECIFICATIONS REPORT <sup>2</sup>
N/A	FABRICATOR NDE INSPECTION
<b>REQUIRED</b>	PACKING SLIPS <sup>3</sup>

ADDITIONAL TESTING AND INSPECTIONS:

**DURING CONSTRUCTION**

CONSTRUCTION/INSTALLATION INSPECTIONS AND TESTING REQUIRED (COMPLETED BY ENGINEER OF RECORD)	REPORT ITEM
<b>REQUIRED</b>	STEEL INSPECTIONS
N/A	HIGH STRENGTH BOLT INSPECTIONS
N/A	HIGH WIND ZONE INSPECTIONS <sup>4</sup>
N/A	FOUNDATION INSPECTIONS
N/A	CONCRETE COMP. STRENGTH, SLUMP TESTS AND PLACEMENT
N/A	POST INSTALLED ANCHOR VERIFICATION <sup>5</sup>
N/A	GROUT VERIFICATION
N/A	CERTIFIED WELD INSPECTION
N/A	EARTHWORK: LIFT AND DENSITY
N/A	ON SITE COLD GALVANIZING VERIFICATION
N/A	GUY WIRE TENSION REPORT

ADDITIONAL TESTING AND INSPECTIONS:

**AFTER CONSTRUCTION**

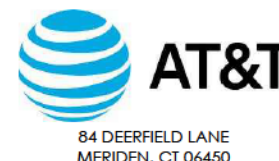
CONSTRUCTION/INSTALLATION INSPECTIONS AND TESTING REQUIRED (COMPLETED BY ENGINEER OF RECORD)	REPORT ITEM
<b>REQUIRED</b>	MODIFICATION INSPECTOR REDLINE OR RECORD DRAWINGS <sup>6</sup>
N/A	POST INSTALLED ANCHOR PULL-OUT TESTING
<b>REQUIRED</b>	PHOTOGRAPHS

ADDITIONAL TESTING AND INSPECTIONS:



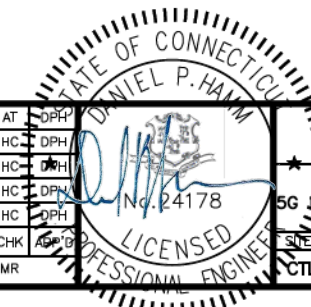
**SITE NUMBER: CTL02267**  
**SITE NAME: ORANGE CT RIDGE ROAD**

298 RIDGE ROAD  
ORANGE, CT 06477  
NEW HAVEN COUNTY



NO.	DATE	REVISIONS	BY	CHK	APP'D
3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	DPH
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	DPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	DPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	DPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	DPH

SCALE: AS SHOWN    DESIGNED BY: HC    DRAWN BY: MR



AT&T

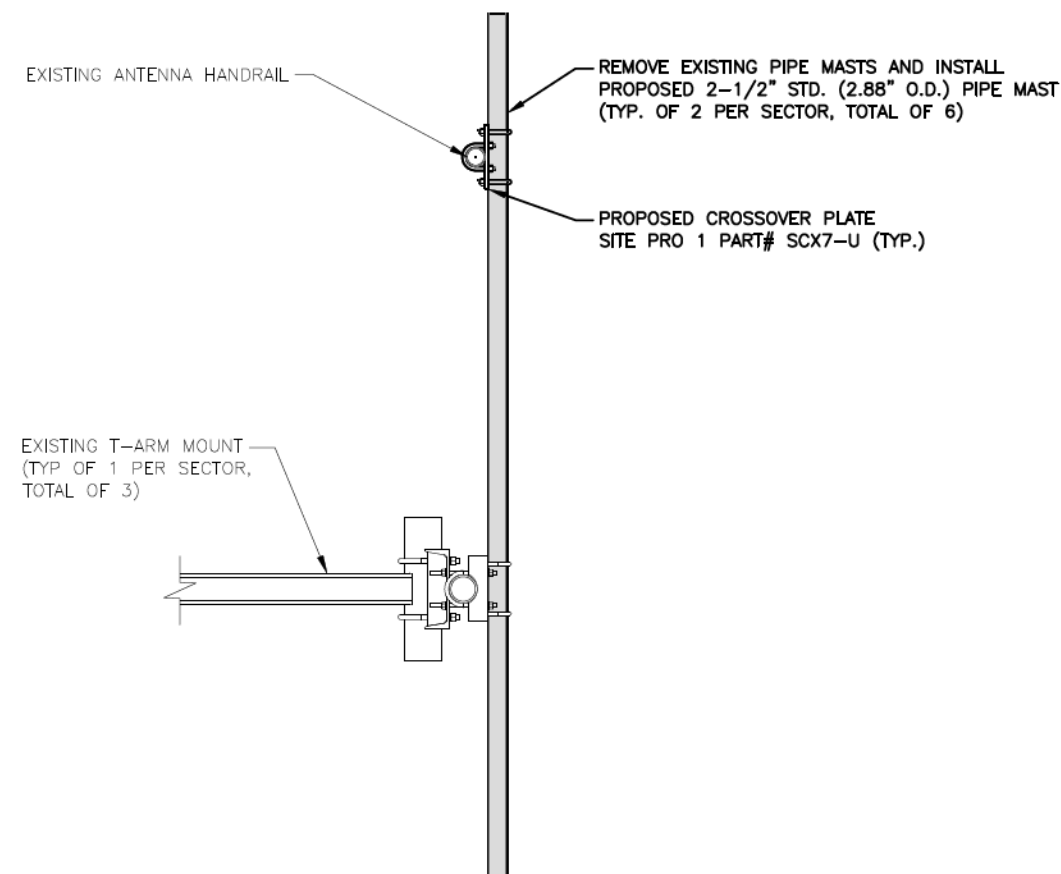
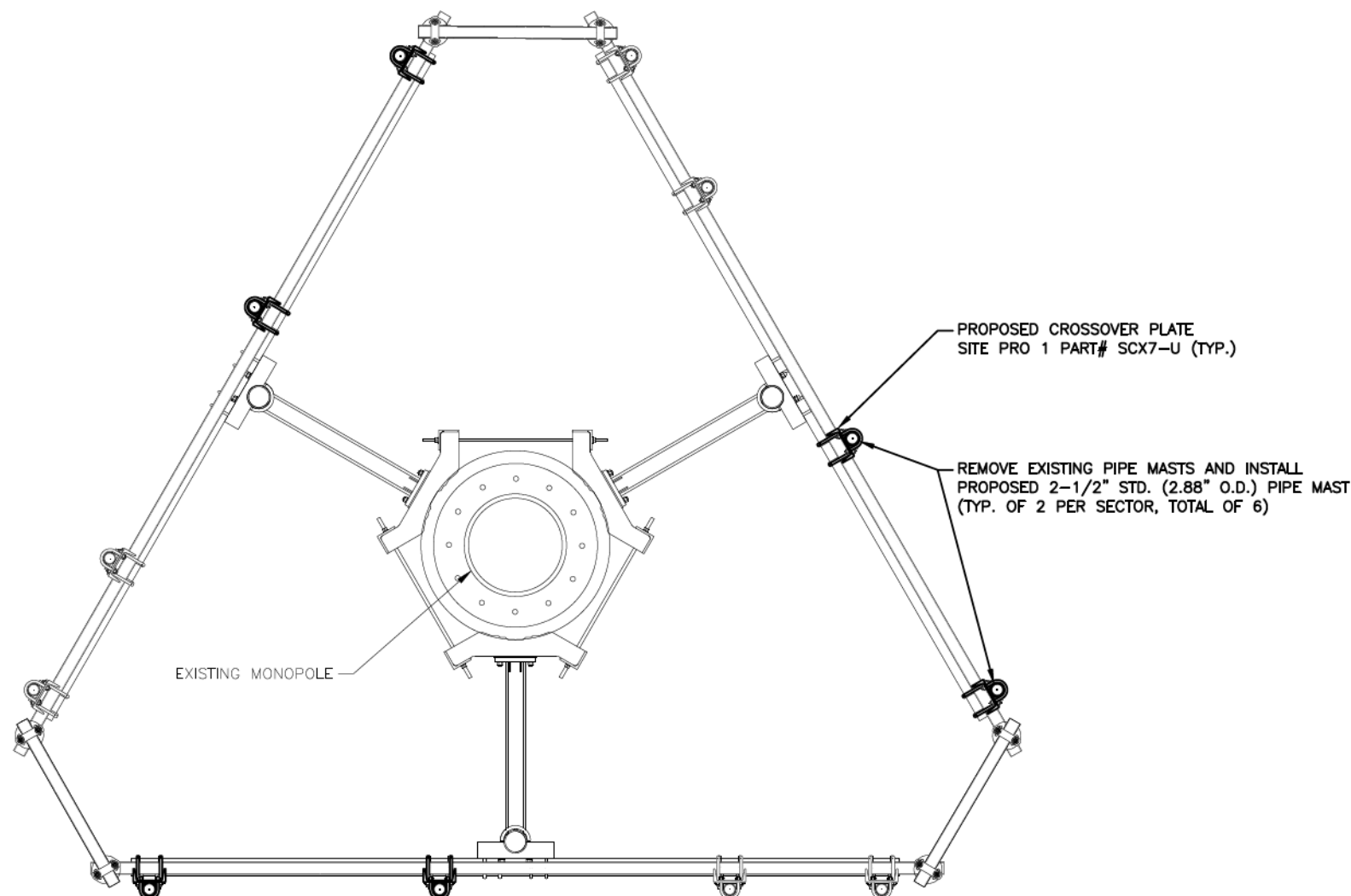
STRUCTURAL NOTES  
5G NR 1SR CBAND\_ANTENNA MODIFICATION II 4TX4RX  
SOFTWARE RETROFIT UPGRADE

SITE NUMBER	DRAWING NUMBER	REV
CTL02267	SN-1	3

**NOTE:**  
REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.

**NOTE:**  
AN ANALYSIS FOR THE CAPACITY OF THE EXISTING ANTENNA MOUNT TO SUPPORT THE PROPOSED LOADING HAS BEEN COMPLETED BY:  
TEP NORTHEAST (TEP OPCO, LLC.)  
DATED: FEBRUARY 16, 2024 (REV.1)

**NOTE:**  
REFER TO STRUCTURAL ANALYSIS BY: CENTEK ENGINEERING  
DATED: APRIL 15 2024, (REV.3)  
FOR THE CAPACITY OF THE EXISTING STRUCTURES TO SUPPORT THE PROPOSED EQUIPMENT.



**PROPOSED MOUNT MODIFICATIONS PLAN**

22x34 SCALE: 3/4"=1'-0"  
11x17 SCALE: 3/8"=1'-0"

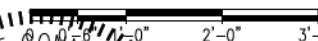
1  
S-1



**PROPOSED MOUNT MODIFICATIONS DETAIL**

22x34 SCALE: 1"=1'-0"  
11x17 SCALE: 1/2"=1'-0"

2  
S-1

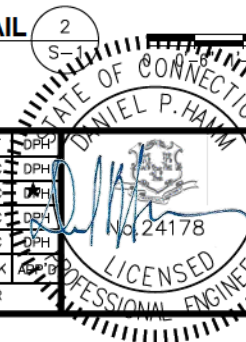


**SITE NUMBER: CTL02267**  
**SITE NAME: ORANGE CT RIDGE ROAD**

298 RIDGE ROAD  
ORANGE, CT 06477  
NEW HAVEN COUNTY



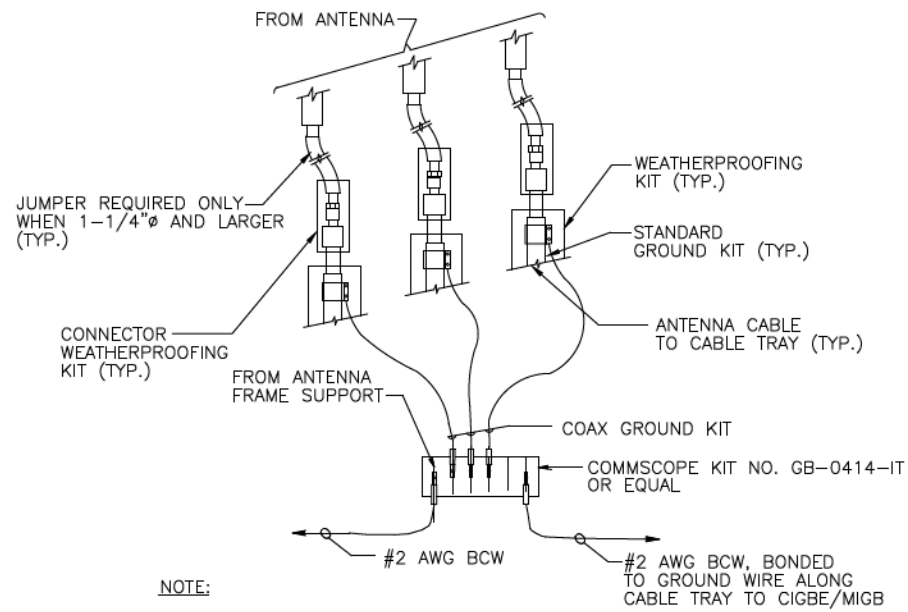
NO.	DATE	REVISIONS	BY	CHK	APP'D
3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	DPH
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	DPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	DPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	DPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	DPH



AT&T

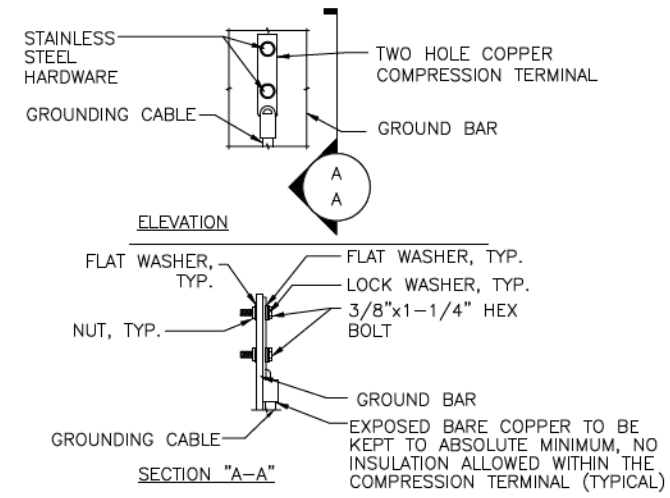
5G NR 1SR CBAND\_ANTENNA MODIFICATION II 4TX4RX  
MOUNT MODIFICATION DESIGN  
SOFTWARE RETROFIT UPGRADE

SITE NUMBER	DRAWING NUMBER	REV
CTL02267	S-1	3



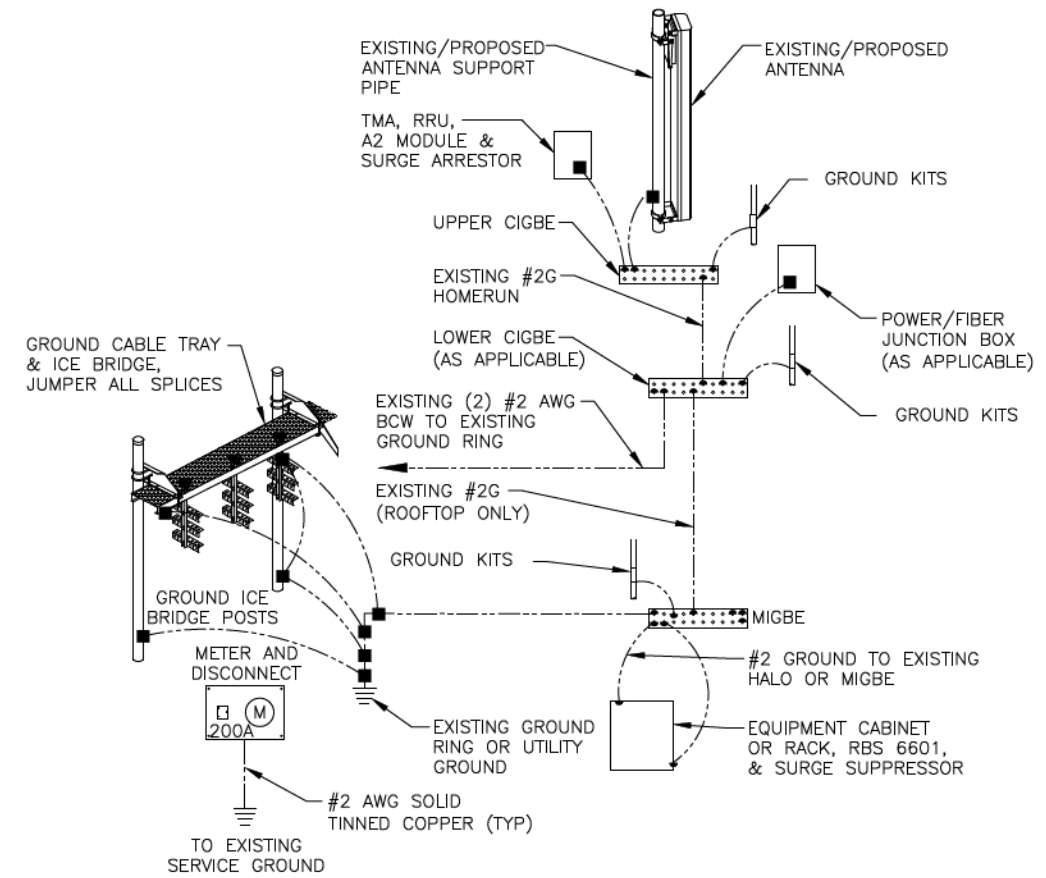
NOTE:  
1. DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO CIGBE.

**GROUND WIRE TO GROUND BAR CONNECTION DETAIL** (1)  
SCALE: N.T.S. G-1



NOTES:  
1. "DOUBLING UP" OR "STACKING" OF CONNECTION IS NOT PERMITTED.  
2. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATION.  
3. CADWELD DOWNLEADS FROM UPPER EGB, LOWER EGB, AND MGB

**TYPICAL GROUND BAR CONNECTION DETAIL** (3)  
SCALE: N.T.S. G-1

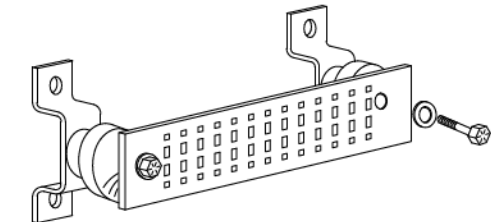


**GROUNDING RISER DIAGRAM** (2)  
SCALE: N.T.S. G-1

**AT&T GROUNDING STANDARDS TO BE FOLLOWED:**  
ATT-TP-76416  
ATT-TP-76300  
ATT-CEM-18002  
ATT-002-290-531  
ATT-002-290-701  
ATT-CEM-23001

EACH GROUND CONDUCTOR TERMINATING ON ANY GROUND BAR SHALL HAVE AN IDENTIFICATION TAG ATTACHED AT EACH END THAT WILL IDENTIFY ITS ORIGIN AND DESTINATION.

- SECTION "P" - SURGE PRODUCERS**
- CABLE ENTRY PORTS (HATCH PLATES) (#2 AWG)
  - GENERATOR FRAMEWORK (IF AVAILABLE) (#2 AWG)
  - TELCO GROUND BAR
  - COMMERCIAL POWER COMMON NEUTRAL/GROUND BOND (#2 AWG)
  - +24V POWER SUPPLY RETURN BAR (#2 AWG)
  - 48V POWER SUPPLY RETURN BAR (#2 AWG)
  - RECTIFIER FRAMES.
- SECTION "A" - SURGE ABSORBERS**
- INTERIOR GROUND RING (#2 AWG)
  - EXTERNAL EARTH GROUND FIELD (BURIED GROUND RING) (#2 AWG)
  - METALLIC COLD WATER PIPE (IF AVAILABLE) (#2 AWG)
  - BUILDING STEEL (IF AVAILABLE) (#2 AWG)



**GROUND BAR - DETAIL (AS REQUIRED)**  
SCALE: N.T.S.

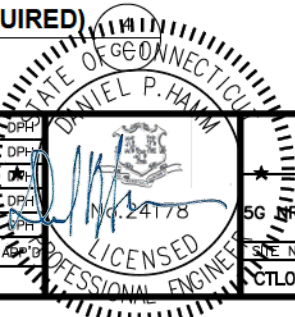


**SITE NUMBER: CTL02267**  
**SITE NAME: ORANGE CT RIDGE ROAD**  
  
298 RIDGE ROAD  
ORANGE, CT 06477  
NEW HAVEN COUNTY



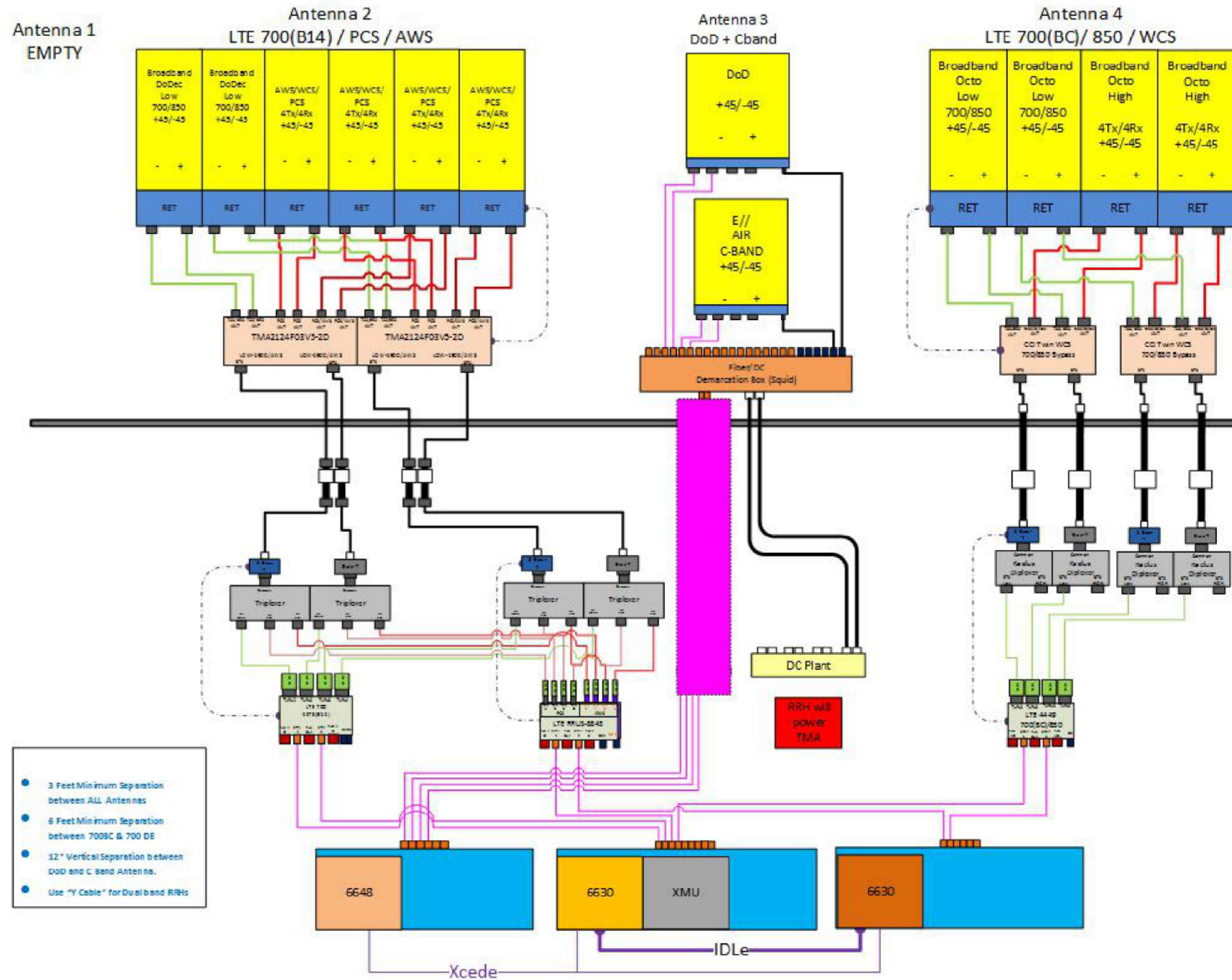
NO.	DATE	REVISIONS	BY	CHK	APP
3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	DPH
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	DPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	DPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	DPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	DPH

SCALE: AS SHOWN    DESIGNED BY: HC    DRAWN BY: MR



AT&T		
GROUNDING DETAILS		
5G NR 1SR CBAND_ANTENNA MODIFICATION II 4TX4RX SOFTWARE RETROFIT UPGRADE		
SITE NUMBER	DRAWING NUMBER	REV
CTL02267	G-1	3

**NOTE:**  
 REV: 5  
 DATED: 05/13/22  
 RFDS ID: 4732036



- 3 Feet Minimum Separation between ALL Antennas
- 6 Feet Minimum Separation between 700BC & 700 DE
- 12" Vertical Separation between DoD and C Band Antenna.
- Use "Y Cable" for Dual band RRHs

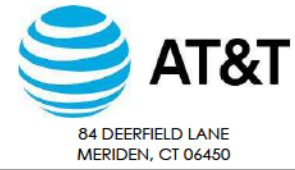
**RF PLUMBING DIAGRAM** 1  
 SCALE: N.T.S. RF-1

**NOTE:**  
 1. CONTRACTOR TO CONFIRM ALL PARTS.  
 2. INSTALL ALL EQUIPMENT TO MANUFACTURER'S RECOMMENDATIONS

**NOTE:**  
 REFER TO THE FINAL RF DATA SHEET FOR FINAL ANTENNA SETTINGS.



**SITE NUMBER: CTL02267**  
**SITE NAME: ORANGE CT RIDGE ROAD**  
 298 RIDGE ROAD  
 ORANGE, CT 06477  
 NEW HAVEN COUNTY



NO.	DATE	REVISIONS	BY	CHK	APP'D
3	05/01/24	ISSUED FOR CONSTRUCTION	AM	AT	DPH
2	10/11/22	ISSUED FOR CONSTRUCTION	GA	HC	DPH
1	09/26/22	ISSUED FOR REVIEW	BB	HC	DPH
0	09/19/22	ISSUED FOR REVIEW	MR	HC	DPH
A	02/24/22	ISSUED FOR REVIEW	MR	HC	DPH

SCALE: AS SHOWN    DESIGNED BY: HC    DRAWN BY: MR

AT&T		
RF PLUMBING DIAGRAM		
5G NR 1SR CBAND_ANTENNA MODIFICATION II 4TX4RX SOFTWARE RETROFIT UPGRADE		
SITE NUMBER	DRAWING NUMBER	REV
CTL02267	RF-1	3





STATE OF CONNECTICUT  
*CONNECTICUT SITING COUNCIL*

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

Web Site: [portal.ct.gov/csc](http://portal.ct.gov/csc)

**VIA ELECTRONIC MAIL**

November 28, 2022

Hollis M. Redding  
SAI Communications, LLC  
12 Industrial Way  
Salem, NH 03079  
[hredding@saigrp.com](mailto:hredding@saigrp.com)

RE: **EM-AT&T-107-221031** – AT&T notice of intent to modify an existing telecommunications facility located at 298 Ridge Road, Orange, Connecticut.

Dear Hollis Redding:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

1. Prior to the commencement of construction, AT&T shall provide a copy of the Structural Analysis referencing the Connecticut State Building Code effective October 1, 2022;
2. Prior to AT&T's antenna installation, antenna mount modifications shall be installed in accordance with the Mount Analysis prepared by Hudson Design Group dated January 21, 2022, stamped and signed by Daniel Hamm;
3. Within 45 days following completion of equipment installation, AT&T shall provide documentation certified by a Professional Engineer that its installation complied with the recommendations of the Mount Analysis;
4. RF access restriction and caution signage shall be installed at the site in compliance with FCC guidance;
5. Any deviation from the proposed modification as specified in this notice and supporting materials submitted to the Council shall render this acknowledgement invalid;
6. Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
7. The Council shall be notified in writing at least two weeks prior to the commencement of site construction activities;
8. Within 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
9. Deployment of any 5G services must comply with FCC and FAA guidance relative to air navigation, as applicable;

10. Any nonfunctioning antenna and associated antenna mounting equipment, or other equipment at this facility owned and operated by AT&T shall be removed within 60 days of the date the antenna or equipment ceased to function;
11. The validity of this action shall expire one year from the date of this letter; and
12. AT&T may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration.

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated October 28, 2022. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site by any dimension, increase noise levels at the tower site boundary by six decibels or more, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standards adopted by the Federal Communications Commission pursuant to Section 704 of the Telecommunications Act of 1996 and by the state Department of Energy and Environmental Protection pursuant to Connecticut General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below state and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

Sincerely,



Melanie A. Bachman  
Executive Director

MAB/RDM/emr

c: The Honorable James M. Zeoli, First Selectperson, Town of Orange (jzeoli@orange-ct.gov)



STATE OF CONNECTICUT  
CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

Web Site: [portal.ct.gov/csc](http://portal.ct.gov/csc)

VIA ELECTRONIC MAIL

November 8, 2023

Hollis M. Redding  
SAI Communications, LLC  
12 Industrial Way  
Salem, NH 03079  
[hredding@saigrp.com](mailto:hredding@saigrp.com)

RE: **Requests for Extensions of Construction:**

**EM-AT&T-103-221026** – AT&T notice of intent to modify an existing telecommunications facility located 284 New Canaan Avenue, Norwalk, Connecticut.

**EM-AT&T-107-221031** – AT&T notice of intent to modify an existing telecommunications facility located at 298 Ridge Road, Orange, Connecticut.

Dear Hollis Redding:

The Connecticut Siting Council (Council) is in receipt of your correspondence dated November 6, 2023, submitted on behalf of AT&T requesting extensions of time to submit notices of completion of construction for the above-referenced exempt modifications.

The Council hereby grants an extension of time until November 28, 2024, to submit notices of completion of construction.

The extensions are granted with the understanding that the Council will be notified should AT&T need additional time to submit notices of completion or decides not to proceed with construction on any of the above-referenced exempt modifications.

Thank you for your attention to this matter.  
Sincerely,

Melanie A. Bachman  
Executive Director

MAB/ANM/laf



STATE OF CONNECTICUT  
*CONNECTICUT SITING COUNCIL*

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

Web Site: [portal.ct.gov/csc](http://portal.ct.gov/csc)

**VIA ELECTRONIC MAIL**

November 8, 2023

Hollis M. Redding  
SAI Communications, LLC  
12 Industrial Way  
Salem, NH 03079  
[hredding@saigrp.com](mailto:hredding@saigrp.com)

**RE: Requests for Extensions of Construction:**

**EM-AT&T-103-221026** – AT&T notice of intent to modify an existing telecommunications facility located 284 New Canaan Avenue, Norwalk, Connecticut.

**EM-AT&T-107-221031** – AT&T notice of intent to modify an existing telecommunications facility located at 298 Ridge Road, Orange, Connecticut.

Dear Hollis Redding:

The Connecticut Siting Council (Council) is in receipt of your correspondence dated November 6, 2023, submitted on behalf of AT&T requesting extensions of time to submit notices of completion of construction for the above-referenced exempt modifications.

The Council hereby grants an extension of time until November 28, 2024, to submit notices of completion of construction.

The extensions are granted with the understanding that the Council will be notified should AT&T need additional time to submit notices of completion or decides not to proceed with construction on any of the above-referenced exempt modifications.

Thank you for your attention to this matter.  
Sincerely,

A handwritten signature in black ink, appearing to read "Melanie A. Bachman".

Melanie A. Bachman  
Executive Director

MAB/ANM/laf