

#### STATE OF CONNECTICUT

#### CONNECTICUT SITING COUNCIL

10 Franklin Square New Britain, Connecticut 06051 Phone: (860) 827-2935 Fax: (860) 827-2950

March 31, 2000

Kenneth E. Lee Real Estate Consultant Springwich Cellular Limited Partnership 500 Enterprise Drive Rocky Hill, CT 06067

RE:

TS-BAM/SCLP-105-000314 - Cellco Partnership d/b/a Bell Atlantic Mobile and Springwich Cellular Limited Partnership request for an order to approve tower sharing at an existing telecommunications facility located at 38 Hatchetts Hill Road in Old Lyme, Connecticut.

Dear Mr. Lee:

At a public meeting held March 22, 2000, the Connecticut Siting Council (Council) ruled that the shared use of this existing tower site is technically, legally, environmentally, and economically feasible and meets public safety concerns, and therefore, in compliance with General Statutes § 16-50aa, the Council has ordered the shared use of this facility. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

The proposed placement of antennas on this existing tower is in technical compliance with General Statutes § 16-50aa; nonetheless, the Council is concerned that the construction of two towers, with municipal approval, adjacent to each other may contradict the intent of tower sharing policy. To fully comply with the spirit of tower sharing policy, all those concerned would have developed only one tower, if feasible. However, the Council did not have jurisdiction over these towers that were constructed. This situation allows the circumvention of tower sharing policy and may encourage the very proliferation of towers that the Connecticut General Assembly sought to avoid. While this particular request is approved, please be advised that the Council may seek to refine its procedure to deal with this type of situation for the long term, and such approval should not be seen as setting a precedent for future requests.

Any additional change to this facility may require an explicit request to this agency pursuant to General Statutes § 16-50aa or notice pursuant to Regulations of Connecticut State Agencies Section 16-50j-73, as applicable. Such request or notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Kenneth E. Lee Springwich Cellular Limited Partnership March 31, 2000 Page 2

This decision applies only to this request for tower sharing and is not applicable to any other request or construction.

The proposed shared use is to be implemented as specified in your letter dated March 14, 2000.

Thank you for your attention and cooperation.

Very truly yours,

Mortimer A. Gelston

Chairman

MAG/RKE/grg

c: Honorable Timothy C. Griswold, First Selectman, Town of Old Lyme Honorable Moira K. Lyons, Representative of the 146<sup>th</sup> District Honorable Robert M. Ward, Representative of the 86<sup>th</sup> District Sandy Carter, Bell Atlantic Mobile

J. Brendan Sharkey, VoiceStream Wireless

#### SPRINGWICH CELLULAR LIMITED PARTNERSHIP

500 Enterprise Drive, 3<sup>rd</sup> Floor Rocky Hill, CT. 06067 860/513-7700

#### **BELL ATLANTIC MOBILE**

20 Alexander Drive Wallingford, CT. 06492 203/294-8519



MAR 1 4 2000

CONNECTICUT SITING COUNCIL

#### HAND DELIVERED

Mr. Mortimer A. Gelston, Chairman Connecticut Siting Council 10 Franklin Square New Britain, Connecticut 06051

March 14, 2000

Re:

Request by Cellco Partnership d/b/a Bell Atlantic Mobile and Springwich Cellular Limited Partnership for an Order to Approve the Shared Use of a Tower Facility, 38 Hatchetts Hill Road, Old Lyme, Connecticut

#### Dear Chairman Gelston:

Pursuant to Connecticut General Statutes (C.G.S.) Sec. 16-50aa, Cellco Partnership d/b/a Bell Atlantic Mobile ("BAM") and Springwich Cellular Limited Partnership ("SCLP") (collectively, the "Applicants") hereby request an order from the Connecticut Siting Council ("Council") to approve the proposed shared use by the Applicants of an existing tower facility located at 38 Hatchetts Hill Road in Old Lyme, Connecticut. The property is owned by Hatchetts Hill, LLC, from which Omnipoint Communications, Inc. leases property for the tower facility. Omnipoint Communications, Inc. owns and operates the tower. As shown on the attached drawings and as further described below, BAM and SCLP each propose to install antennas on the existing tower and to locate two equipment shelters at the base of the tower. The Applicants request the Council to find that the proposed shared use of the tower facility satisfies the criteria stated in C.G.S. Sec. 16-50aa, and to issue an order approving the proposed shared use

#### **Background**

Cellco Partnership d/b/a Bell Atlantic Mobile and Springwich Cellular Limited Partnership are each licensed by the Federal Communications Commission (FCC) to provide cellular telephone service in the New London County New England County Metropolitan Area (NECMA), which includes the area to be served by the Applicants' proposed installations.

The Omnipoint facility at 38 Hatchetts Hill Road, Old Lyme, Connecticut consists of an approximately 2,080 s. f. leased area, on which is located a 190' monopole and one equipment cabinet located at the base of the monopole. The monopole supports several Omnipoint antennas, which provide mobile communications service to the public pursuant to a FCC license. The Applicants and Omnipoint have agreed to the proposed shared use of this tower pursuant to mutually acceptable terms and conditions. Omnipoint has authorized the Applicants to apply for all necessary permits, approvals and authorizations which may be required for the proposed shared use of this facility. On Monday, March 13, 2000, the Old Lyme Planning & Zoning Commission approved the modification to the original Omnipoint site plan. This modification approved the antennas and equipment buildings for BAM and SCLP for the colocation on this existing tower.

BAM proposes to install twelve (12) Decibel Model DB844H80E antennas, approximately 48 inches in height, on a second antenna platform with their center of radiation at approximately 175 feet above ground level ("AGL"). BAM also will install one (1) GPS antenna on its platform at the 175' level of the tower. Equipment associated with these antennas, as well as a 40 KW diesel-fueled emergency stand-by generator, would be located in a new approximately 12-foot x 30-foot equipment building located at the base of Omnipoint's tower.

SCLP proposes to install twelve (12) Allgon Model 7120.16 antennas, approximately 52 inches in height, on a third antenna platform with their center of radiation at approximately 165 feet above ground level ("AGL"). Equipment associated with these antennas will be located in a new approximately 11'-3" x 16 foot equipment building located at the base of Omnipoint's tower

C.G.S. Sec. 16-50aa provides that, upon written request for approval of a proposed shared use, "if the council finds that the proposed shared use of the facility is technically, legally, environmentally and economically feasible and meets public safety concerns, the council shall issue an order approving such shared use." (C.G.S. Sec. 16-50aa(c)(1).)

The shared use of the tower satisfies the criteria in C.G.S. Sec. 16-50aa as follows:

A. Technical Feasibility. The existing tower is structurally sound and capable of supporting the proposed BAM and SCLP antennas. A copy of the tower design by Pirod Inc.was done for Omnipoint showing the pole design for multiple carriers and is included with this application. The proposed shared use of this tower is therefore technically feasible.

B. Legal Feasibility. Under C.G.S. Sec. 16-50aa, the Council has been authorized to issue an order approving the proposed shared use of an existing tower facility such as the facility in Old Lyme. (C.G.S. Sec. 16-50aa(c)(1).) This authority complements the

Council's prior-existing authority under C.G.S. Sec. 16-50p to issue orders approving the construction of new towers that are subject to the Council's jurisdiction. C.G.S. Sec. 16-50x(a) directs the Council to "give such consideration to other state laws and municipal regulations as it shall deem appropriate" in ruling on requests for the shared use of existing tower facilities. Under the authority vested in the Council by C.G.S. Sec. 16-50aa, an order by the Council approving the shared use would permit the Applicants to obtain a building permit for the proposed installation.

- <u>C.</u> Environmental Feasibility. The proposed shared use would have a minimal environmental effect, for the following reasons:
  - 1. The proposed installation would have an insignificant incremental visual impact, and would not cause any significant change or alteration in the physical or environmental characteristics of the property. The addition of the proposed antennas would not increase the height of the tower. The equipment shelters, within a fenced area, will be placed adjacent to Omnipoint's tower.
  - 2. The proposed installation would not increase the noise levels at the existing facility by six decibels or more. The only additional noise will occur during emergency use or periodic exercising of the generator.
  - 3. Operation of the additional antennas will not increase the total radio frequency electromagnetic radiation power density, measured at the tower base, to a level at or above applicable ANSI standards. "Worst-case" exposure calculations for a point at the base of the tower in relation to operation of each of BAM's, SCLP's, and Omnipoint's antenna arrays are as follows:

	Applicable ANSI Stnd.	Calculated "Worst Case"	Percentage of Stnd.
BAM	0.583 mW/cm2	0.0223 mW/cm2	3.82%
SCLP	0.5867 mW/cm2	0.0272 mW/cm2	4.64%
Omnipoint	1.000 mW/cm2	0.0102 mW/cm2	1.02%
		Total	9.48%

The collective "worst-case" exposure would be only 9.48 % of the ANSI standard, as calculated for mixed frequency sites. Power density levels from shared use of the tower facility would thus be well below applicable ANSI standards.

4. The proposed installation would not require any water or sanitary facilities, or generate discharges to water bodies. Operation of the emergency back-up generator will result in limited air emissions; pursuant to R.C.S.A. Section 22a-174-3, the generator will require the issuance of a permit from the Department of Environmental Protection Bureau of Air Management. After construction is complete, the proposed installation would not generate any traffic other than for periodic maintenance visits.

The proposed use of this facility would therefore have a minimal environmental effect, and is environmentally feasible.

- E. Economic Feasibility. As previously mentioned, Omnipoint and the Applicants have entered into a mutual agreement to share use of the existing tower on terms agreeable to the parties. The proposed tower sharing is therefore economically feasible.
- F. Public Safety Concerns. As stated above, the existing tower is structurally capable of supporting the proposed BAM and SCLP antennas. The Applicants are not aware of any other public safety concerns relative to the proposed sharing of the existing tower. In fact, the provision of new or improved cellular communications service in the Old Lyme area through shared use of the existing tower is expected to enhance the safety and welfare of area residents and travelers.

#### Conclusion

For the reasons discussed above, the proposed shared use of the existing tower facility at 38 Hatchetts Hill Road in Old Lyme satisfies the criteria stated in C.G.S. Sec. 16-50aa, and advances the General Assembly's and the Council's goal of preventing the proliferation of towers in Connecticut. The Applicants therefore request that the Council issue an order approving the proposed shared use.

Thank you for your consideration of this matter.

Pursuant to Connecticut General Statutes Sec. 16-50v and Section 16-50v-1(a) of the Regulations of Connecticut State Agencies, BAM and SCLP have enclosed a check in the amount of \$500.00 for the required filing fee.

Respectfully yours,

Kenneth E. Leesc

Kenneth E. Lee Real Estate Consultant for Springwich Cellular Limited Partnership Sandy M. Carter Sandy M. Carter Manager-Regulatory Bell Atlantic Mobile

Attachments

cc: Honorable Timothy Griswold, First Selectman

#### SPRINGWICH CELLULAR LIMITED PARTNERSHI

500 Enterprise Drive, 3<sup>rd</sup> Floor Rocky Hill. CT. 06067 860/513-7700 BELL ATLANTIC MOBILE

20 Alexander Drive Wallingford, CT. 06492 203/294-8519

March 14, 2000

Honorable Timothy Griswold First Selectman Town Hall 52 Lyme Street, P. O. Box 338 Old Lyme, Connecticut 06371

Dear Mr. Griswold:

This letter is to inform you that Cellco Partnership d/b/a Bell Atlantic Mobile and Springwich Cellular Limited Partnership (SCLP) plan to install antennas and associated equipment at the existing tower facility located at 38 Hatchetts Hill Road, Old Lyme, Connecticut. I am enclosing a copy of Bell Atlantic Mobile's and Springwich Cellular Limited Partnership's joint tower sharing application to the Connecticut Siting Council.

The application fully sets forth the Company's proposal. However, if you have any questions or require further information on our plans or the Siting Council's procedures, please contact me at (203) 294-8519 or Mr. Kenneth Lee at (203) 556-1655 or Mr. Joel Rinebold, Executive Director of the Connecticut Siting Council at (860) 827-2935.

Sincerely,

Sandy M. Carter Sandy M. Carter Manager – Regulatory

Bell Atlantic Mobile

Kenneth E. Leese Kenneth E. Lee

Real Estate Consultant for Springwich Cellular

Limited Partnership



100 Filley Street Bloomfield, CT 06002

Telephone: (860) 692-7156

Fax: (860) 692-7159

February 16, 2000

Ms. Sandy Carter Regulatory Manager Bell Atlantic Mobile 20 Alexander Drive Wallingford, CT 06492

Re:

CT-11-038 (Old Lyme)

38 Hatchetts Hill Road

Old Lyme, CT

Co-ordinates: 41-19-07 72-16-14

Dear Ms. Carter:

Please be advised that Bell Atlantic Mobile is authorized to proceed in seeking Connecticut Siting Council approvals for the above referenced Omnipoint tower site located at 38 Hatchetts Hill Road, Old Lyme, CT.

If you need any further assistance, please don't hesitate to contact me.

Sincerely,

Sherry Sukow

Collocations Manager

Omnipoint Communications, Inc.

cc:

Master File



### Town of Old Lyme, Old Lyme, CT 06371-0160

January 28, 1999

PLANNING COMMISSION 52 Lyme Street P.O. Box 160 (860) 434-9174 FAX (860) 434-9283

Brendan Sharkey, Attorney Omnipoint Communications 25 Van Zant Street, Suite 18E East Norwalk, CT 06855

RE: Application for Special Exception - Omnipoint Communications 36 Hatchetts Hill Road, Old Lyme, CT 06371

Dear Attorney Sharkey,

On January 14, 1999, the Planning Commission granted Special Exception approval for the above referenced application in accordance site plan dated September 10, 1998 and revised through December 9, 1998, with the following modifications:

1. Paragraph 13 be amended in accordance with Attorney Mattern's letter of January 13, 1999.

Notice of said decision was published in The Day on January 25, 1999. The Certificate of Decision and the mylar and legal documents as modified may be filed on the Land Records in the office of the Town Clerk on February 11, 1999. The mylars must be signed by the Commission Chairman prior to filing. All required documentation must be filed prior to issuance of any permits.

Please note that all site work must comply with the approved site plan. Changes may not be made without prior approval from the Commission or Zoning Enforcement Officer. Also, as-built mylars are required by Regulation to be filed prior to the issuance of a Certificate of Zoning Compliance.

If you have any questions, please do not hesitate to contact me.

Sincerely yours,

Marilyn M. Ózols

Zoning Enforcement Officer

#### LOG-PREIODIC REFLECTOR ANTENNA 110 Degrees 11 dBd

#### Features:

Broadbanded. (800-900 MHz)
Low backlobe radiation. Front to back ratio better than 25 dB.
Low Intermodulation products.
Low wind-load.
Low weight.

Please see the following pages including radiation patterns for ALP 11011-N.

SNE

### Electrical Specifications:

Frequency range: Impedance: Connector:

806-896 MHz 50 Ohm N female

VSWR:

Polarization:

Typ. 1,3:1 max 1,5:1 Vertical

Gain:

11 dBd >25 dB

Front to back ratio: Intermodulation: (2 x 25 W)

Small size.

Rugged design.

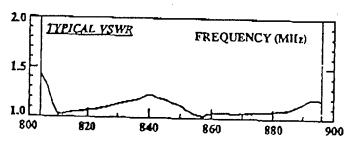
IM5 - 107 dBm 500·W

Power Rating: H-Plane: -3 dB E-Plane: -3 oB

110° 15°

Lightning Protection:

DC Grounded



#### Mechanical Specifications:

Overall height:

51 In. (1320 mm) 8.3 In. (210 mm)

Width: Depth:

11.4 in. (290 mm) 24.5 lbs (11 Kg)

Weight Incl. mounting Items: Rated wind velocity: Wind Area (CxA/Front):

113 mph (180 Km/h) 3.7 sq.ft (0.34 sq.m)

Lateral thrust at rated wind: Worst case

530 N

#### Materials:

Radiating elements: Element housing:

Aluminum **Grey PVC** 

Reflector:

Aluminum

Mounting Hardware clamps:

Hot dip gaivanized steel Stainless steel

bolts:

Manufactured by: Aligon System AB

7120.16



#### 3.42 M 9.6 M + C7 dB DIRECTOR™ LOG PERIODIC ANTENNAS 3.44 M 9.6 M 9-13 dBd GAIN, 40 dB F/B RATIO, 806-960 MHz

MHz

Ideal for cellular and trunking/ESMR applications, these high quality log periodics are now available from Decibel in four new models with 80 or 90 degree horizontal apertures. They're compact, lightweight, and provide an **unmatched front-to-back ratio of 40 dB**.

- Lass Wind Coacing They measure only 24 or 48 inches (610 or 1219 mm) tall, 8.5 inches deep (216 mm), and 6 inches wide (152 mm). They weigh only 5 or 10 pounds.
- Commune Electrical downtilt is available on all 4-foot models, 6°, 8°, 11°, 13°, or for mechanical downtilt, order DB5083 bracket.
- ্ াথা-না Four-foot models provide null-fill and upper lobe suppression.
- Most Stringent IM Test Each antenna is tested for the absence of IM with 16 carriers at 500 watts of composite power.
- Sturdy Construction Made in the U.S. of high-strength aluminum alloy backs, brass elements and UV resistant ABS plastic radomes. No rivets are used!
- Lightning Resistant All metal parts are grounded.

Terminations and Slounts - All models are available with N-Female or 7/16 DIN connectors. DB380 pipe mount is included.

Ordering Information - See table for models to fit your requirements.

UPS Shippable

Model*	DB842H80N-XY	DB844H80N-XY	DB842H90N-XY	DB844H90N-XY
Gain – dBd/dBi	10/12.1	13/15.1	9/11.1	12/14.1
F/B Ratio – dB	40	40	40	40
Horizontal beamwidth**	80°	80°	90°	90°
Vertical beamwidth**	30°	15°	30°	15°
Height – in. (mm)	24 (610)	48 (1219)	24 (610)	48 (1219)
Weight – Ibs. (kg)	5 (2.3)	10 (4.6)	5 (2.3)	10 (4.6)
Shipping weight – lbs. (kg)	8 (3.6)	15 (6.8)	8 (3.6)	15 (6.8)

- \* For 7/16 DIN connectors substitute "E" for "N" in the model numbers. Example: DB842H80E-XY.
- \*\* 3 dB from maximum.

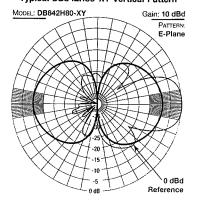
Side offset mounting bracket is included. For electrical downtilt of 6°, 8°, 11° or 13° add T6, T8, T11 or T13 before the "N" or "E" in any 4-foot model number. Example: DB844H80T6N-XY. Note: Electrical downtilt causes a gain loss of .05 dB, or , at the horizon, a reduction of 3, 6, 9 or 12 dB on downtilts of 6°, 8°, 11° or 13° respectively. For mechanical downtilt order DB5083 bracket.

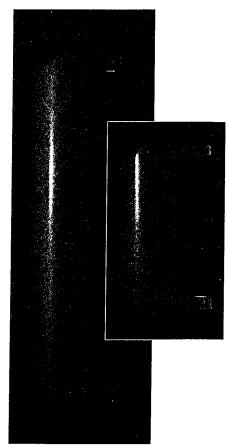
Manneri	pai Bala
Width - in. (mm)	6 (152)
Depth – in. (mm)	8.5 (216)
Height	See table above
Maximum wind speed – Wind area – ft² (m²)	mph (km/h) 125 (200)
24" (610 mm) antenn	na 1 (.093)
48" (1219 mm) anten	ına 2 (.186)
Wind load (at 100 mph/1)	61 km/h) – lbf (N) kp
24" (610 mm) antenn	na 40 (178) 18
48" (1219 mm) anten	ına 80 (356) 36
Radome	Gray ABS
Backplate	Passivated aluminum
Radiators	Brass
Mounting hardware	Galvanized steel
Weight	See table above

Division of the second of the

Electri	cal Cata
Frequency Range - MH:	z 806-960
Gain - dBd	See table above
Front-to-back ratio - dE Beamwidths VSWR	See table above <1.5:1
Null-fill and secondary	On 48" (1219 mm)
lobe suppression	models only
Maximum power input - Nominal impedance - o	
Lightning protection	All metal parts grounded
Termination	N-Female or 7/16 DIN

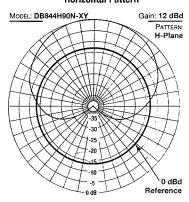
#### Typical DB842H80-XY Vertical Pattern



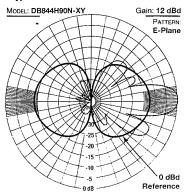


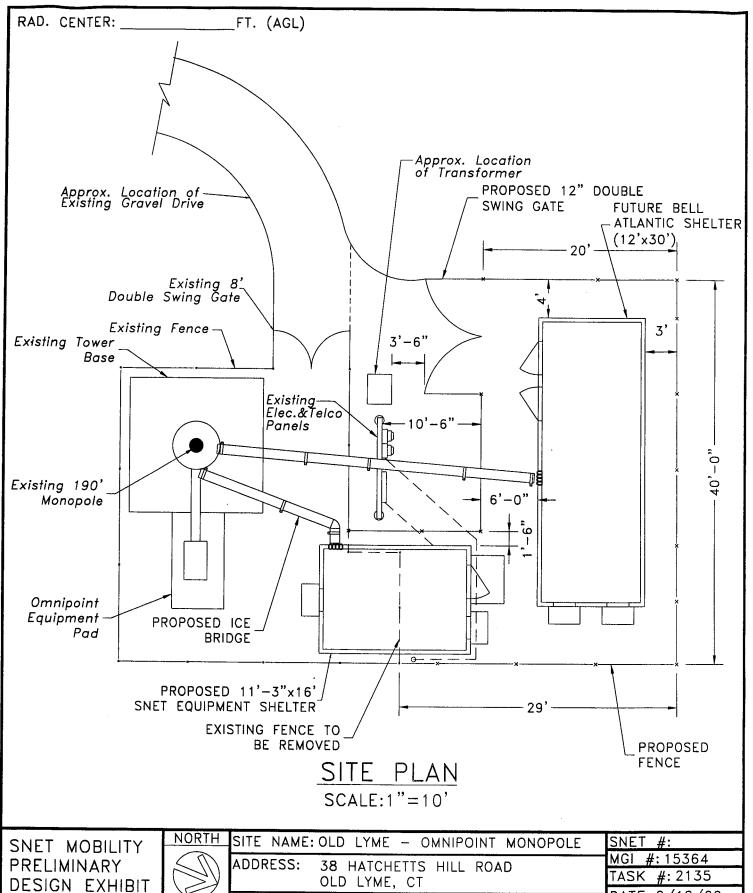
4-Foot and 2-Foot dB DIRECTORS

#### Typical DB842H90N-XY, DB844H90N-XY Horizontal Pattern



#### Typical DB844H90N-XY Vertical Pattern





MAGUIRE GROUP

Maguire Group Inc. Architects Engineers Planners

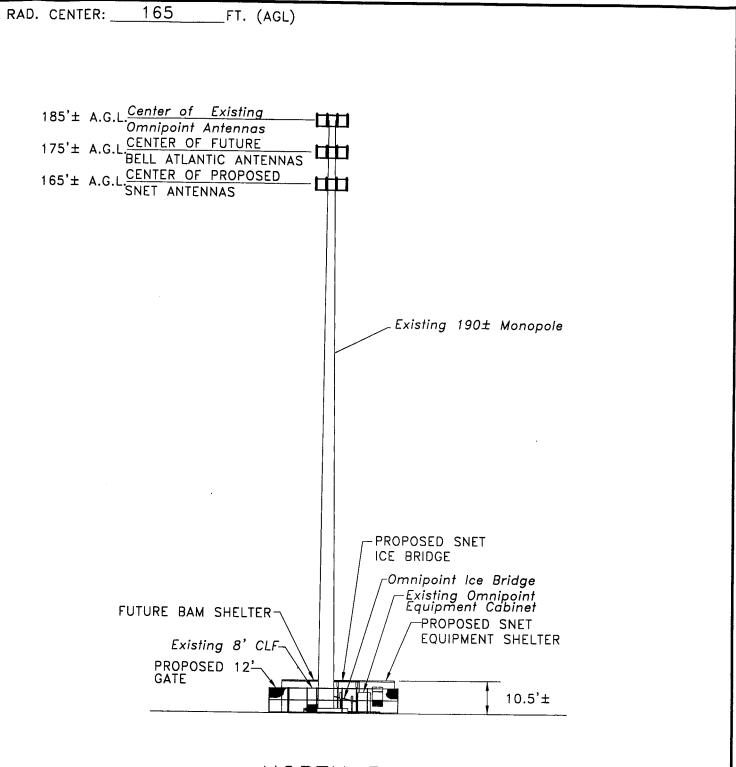
One Court Street New Britain, Connecticut 06051

OLD LYME, CT

THIS DRAWING AND ALL DATA CONTAINED HEREIN IS FOR INFORMATIONAL PURPOSES ONLY. NOT INTENDED FOR DESIGN OR CONSTRUCTION USE. ALL DATA SHOULD BE VERIFIED

DATE: 2/18/00





### NORTH ELEVATION

SCALE:1"=30'

SNET MOBILITY	SITE NAME: OLD LYME - OMNIPOINT MONOPOLE			SNET #:	
PRELIMINARY DESIGN EXHIBIT		0 LD L 1 14 L , C			MGI #: 15364 TASK #: 2135
	L	DRAWN:GMP	CHECKED:RGT	SCALE: 1"=30'	DATE: 2/18/00
MACUIRE Architects Engin	Maguire Group Inc.		THIS DRAWING AND ALL DATA CONTAINED HEREIN IS FOR INFORMATIONAL PURPOSES ONLY. NOT INTENDED FOR DESIGN OR CONSTRUCTION USE. ALL DATA SHOULD BE VERIFIED		

H. E. Bergeron Engineers	r	P.O. Box 440	 65 Po
Civil • Structural • Land Surveying	Ш	2605 White Mountain Highway North Conway, NH 03860	(20
		(603) 356-6936	(20
		(603) 356-7715 (fax)	W٨

65 W. Commercial Street Portland, ME 04101 (207) 780-1100 (207) 780-1101 (fax) www.hebcivil.com



#### STRUCTURAL ANALYSIS REPORT of 191' PIROD MONOPOLE TOWER OLD LYME, CONNECTICUT

Prepared for Maguire Group, Inc.

March 3, 2000



Prepared by:

H. E. Bergeron Engineers

P.O. Box 440, 2605 White Mountain Highway

North Conway, New Hampshire 03860

HEB Project No. 2000-032

H. E. Bergeron Engineers  • Civil • Structural • Land Surveying	P.O. Box 440 2605 White Mountain Highway North Conway, NH 03860 (603) 356-6936 (603) 356-7715 (fax)
	STRUCTU

65 W. Commercial Street
Portland, ME 04101
(207) 780-1100
(207) 780-1101 (fax)
www.hebcivil.com

of
191' PIROD MONOPOLE
OLD LYME, CONNECTICUT
prepared for
MAGUIRE GROUP, INC.

#### **EXECUTIVE SUMMARY:**

H. Edmund Bergeron Civil Engineers, P.A. (HEB) performed a structural analysis of this 191-foot PiROD monopole tower. The analysis was performed with the addition of two twelve-panel antenna arrays mounted on low-profile platforms at centerline elevations of 165' and 175' respectively.

Our analysis indicates the tower and its foundation are capable of supporting the proposed antennas.

#### **INTRODUCTION:**

A structural analysis of this communications tower was performed by H. Edmund Bergeron Civil Engineers, P.A. (HEB) for Maguire Group, Inc. (Maguire). HEB did not visit the tower site. This analysis was based on information provided by Maguire, which included design drawings by PiROD, Inc., lease exhibits by Maguire and Techstar Communications, and antennas proposed by SNET Mobility and Bell Atlantic Mobile.

The structure is a 191-foot, galvanized steel monopole manufactured by PiROD. This analysis was conducted with the following antenna loads:

- (4) RV90-17 and (2) RV33-20 panel antennas on a low-profile platform at the top of the tower
- (12) ALP9212 panel antennas on a low-profile platform at 175'
- (12) ALP7120.16 panel antennas on a low-profile platform at 165'

For the purpose of the analysis, all waveguide cables were assumed to be 1-5/8" diameter installed on the inside of the pole.



#### STRUCTURAL ANALYSIS:

#### Methodology:

The structural analysis was done in accordance with TIA/EIA-222-F (EIA), Structural Standards for Steel Antenna Towers and Antenna Supporting Structures; and the American Institute of Steel Construction (AISC), Manual of Steel Construction, Allowable Stress Design, Ninth Edition.

The analysis was conducted using a wind speed of 85 miles per hour and one-half inch of radial ice over the entire structure and all appurtenances. The TIA/EIA Standard requires a minimum of 85-mph wind load for New London County, Connecticut.

Two analytical methods were used to evaluate the structure: a two-dimensional linear computer model developed by HEB, and a P-delta analysis using CSTRAAD finite element software distributed by ECOM Associates. The HEB 2-D model was used to generate dead loads of the tower and all of its appurtenances, radial ice loads and the resultant wind loading. The maximum bending moments and axial loads were used to calculate combined axial and bending stresses at intervals on the monopole, which were compared to allowable stresses according to AISC and TIA/EIA.

Loads generated in the 2-D model were input into the CSTRAAD program to evaluate secondary bending moments induced during deflection of the structure under load and to independently evaluate stresses. Evaluation of secondary bending moments is required by EIA paragraph 3.1.15. Our analysis indicates that the secondary moments exceed those of the linear analysis, and therefore govern in determining the capacity of the structure.

EIA requires two loading conditions to be evaluated to determine the tower's capacity. The higher stresses resulting from the two cases is used to calculate the tower capacity:

- Case 1 = Wind Load (without ice) + Tower Dead Load (controls)
- Case 2 = 0.75 Wind Load (with ice) + Ice Load + Tower Dead Load

EIA permits a one-third increase in allowable stresses for towers less than 700-feet tall. Allowable stresses of tower members were increased by one-third in computing the load capacity values indicated herein.



#### **ANALYSIS RESULTS:**

Our analysis determined the tower will support the proposed antennae in addition to its current loading. Supporting calculations are provided in Appendix A.

The following table summarizes the capacity of the tower based on combined axial and bending stresses:

Elevation	Capacity
0'-20'	76%
20'-40'	80%
40'-60'	88%
60'-80'	87%
80'-100'	87%
100'-120'	85%
120'-140'	81%
140'-160'	71%
160'-191'	46%

The capability of the existing foundation to support the proposed loading was evaluated by comparing calculated values of shear, compression and overturning moment to design reactions provided on the PiROD record drawings. We found that the shear and overturning moment reactions were less than the design values, but the compression load slightly exceeded the design value (0.2 kips). Based on these figures, the tower's foundation is adequate to support the proposed loading, provided the foundations were designed and built to the requirements of the original design drawing.

#### **CONCLUSIONS AND SUGGESTIONS:**

As detailed above, our analysis indicates that the existing 191' PiROD monopole tower and foundation are capable of supporting the additional antenna loading proposed by SNET Mobility and Bell Atlantic Mobile.



#### LIMITATIONS:

This report is based on the following:

- 1. Tower is properly installed and maintained.
- 2. All members are in new condition.
- 3. All required members are in place.
- 4. All bolts are in place and are properly tightened.
- 5. Tower is in plumb condition.
- 6. All members are galvanized.
- 7. All tower members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
- 8. Record drawings accurately reflect tower dimensions and height.

H. Edmund Bergeron Civil Engineers, P.A. (HEB) is not responsible for any modifications completed prior to or hereafter which HEB is not or was not directly involved. Modifications include but are not limited to:

- 1. Adding or relocating antennas.
- 2. Installing antenna mounting gates or side arms.
- 3. Extending tower.

HEB hereby states that this document represents the entire report and that it assumes no liability for any factual changes that may occur after the date of this report. All representations, recommendations, and conclusions are based upon the information contained and set forth herein. If you are aware of any information which conflicts with that which is contained herein, or you are aware of any defects arising from original design, material, fabrication, or erection deficiencies, you should disregard this report and immediately contact HEB. HEB disclaims all liability for any representation, recommendation, or conclusion not expressly stated herein.

## Appendix A

Calculations

2605 White Mountain Highway, PO Box 440 North Conway, NH 03860 (603) 356-6936

Client:

Maguire Group

Job:

Old Lyme, CT

Job No.: 2000-032

Calculated By: Checked By:

R. Adair

Date: 3-Mar-00

Date:

#### **Monopole Summary**

Wind Speed =

85 mph

Height of the Tower =

191 ft

Section	Length (ft.)	Midpt Elev.	Base Width (in.)	Top Width (in.)	Area (sf) w/o lce	Area (sf) w/ Ice	Wall Thknss	Wt. (lbs) Tower	Wt. (lbs)
10 ·		0.00			0.00	0.00	TIKII33	100001	106
9	31.00	175.50	24.00	24.00	62.00	64.58	0.375	2936	464
8	20.00	150.00	30.00	30.00	50.00	51.67	0.375	2375	373
7	20.00	130.00	36.00	36.00	60.00	61.67	0.375	2856	446
6	20.00	110.00	42.00	42.00	70.00	71.67	0.375	3337	519
. 5	20.00	90.00	48.00	48.00	80.00	81.67	0.375	3818	593
4	20.00	70.00	54.00	54.00	90.00	91.67	0.375	4299	666
3	20.00	50.00	60.00	60.00	100.00	101.67	0.375	4781	739
2	20.00	30.00	60.00	60.00	100.00	101.67	0.500	6361	739
1	20.00	10.00	60.00	60.00	100.00	101.67	0.625	7934	739

Total 38698 5278

**Section Properties** 

Section	I	r	S	Area
	in⁴ .	in	in³	in⁴
10	0	#DIV/0!	#DIV/0!	0.00
9	1942	8.35	162	27.83
8	3829	10.47	255	34.90
7	6659	12.60	370	41.97
6	10621	14.72	506	49.04
5	15908	16.84	663	56.11
4	22710	18.96	841	63.18
3	31217	21.08	1041	70.24
2	41363	21.04	1379	93.46
1	51380	20.99	1713	116.58

**Tower Dead Load Summary** 

Elev.	Tower	lce
	(lbs)	(lbs)
191	0	0
160	2936	464
140	<sub>့</sub> 5311	837
120	8167	1283
100	11505	1802
80	15323	2394
60	19623	3060
40	24403	3799
20	30764	4538
0	38698	5278

2605 White Mountain Highway, PO Box 440 North Conway, NH 03860 (603) 356-6936

Client:

Maguire Group

Job:

Old Lyme, CT

Job No.: 2000-032

Calculated By:

R. Adair

Date: Date:

3-Mar-00

Checked By:

#### Antennae Summary

Calculations based on EIA/TIA-222-F, using the following formulas:

Force on discrete appurtenance: F=Qz\*Gh\*Ca\*A

Force on microwave antennae: F=Cr\*A\*Gh\*Kz\*V^2, where Cr=((Ca^2)+(Cs^2))^(1/2)

Gh=1.69 for monopoles Gh= 1.69

V as specified EIA-222-F

Input:

Wind Velocity=

85 mph

Tower Hgt=

191 ft.

	•				Area	Area	Force	Force				
<u>Type</u>	Elev. (z)	Coeff. (C)	<u>Kz</u>	Qz	(no ice)	(ice)	(no ice)	(ice)	Weight			
(4) RV90-17 and	187	1.4	1.64	30.36	30.0	32.4	2155	2327	240			
(2) RV33-20 on platforn	187	1.4	1.64	30.36	12.6	16.8	903	1207	1600			
			1.00	18.50			0	0				
(12) ALP9212 Panels	175	1.4	1.61	29.79	46.8	49.2	3299	3468	360			
Low-profile platform	175	1.4	1.61	29.79	12.6	16.8	886	1184	1600			
			1.00	18.50			0	0				
(12) ALP7120.16	165	1.4	1.58	29.29	44.4	46.8	3077	3244	300			
Low-profile platform	165	1.4	1.58	29.29	12.6	16.8	871	1164	1600			
			1.00	18.50			0	0				
			1.00	18.50			0	0				
<u>DISHES</u>										Orient	<u>Ca</u>	<u>Cs</u>
		0.00000	1.00	18.50			0	0				
		0.00000	1.00	18.50			0	0				
		0.00000	1.00	18.50			0	0				
		0.00000	1.00	18.50			0	. 0				
		0.00000	1.00	18.50			0	0				
									5700			

#### **CABLES & LINEAR APPURT.**

Section	Area w/o ice	Area w/ Ice	Weight w/o lce	Weight w/ Ice
10				
9	0.97	3.55	418	449
8	0.63	2.29	769	789
7	0.63	2.29	769	789
6	0.63	2.29	769	789
5	0.63	2.29	769	789
4	0.63	2.29	769	789
3	0.63	2.29	769	789
2	0.63	2.29	769	789
1	0.63	2.29	769	789

2605 White Mountain Highway, PO Box 440 North Conway, NH 03860 (603) 356-6936

Client:

Maguire Group

Job:

Old Lyme, CT

Job No.: 2000-032

3-Mar-00

Calculated By:

R. Adair

Date:

Date:

Checked By:

Wind Load Summary

Wind Velocity =

Height of Tower =

85 mph

191 feet

Gh = Gust response factor = 1.69

Cf = Structure force coefficient from Table 1 of TIA/EIA

Qz = Velocity pressure =  $.00256*Kz*V^2$ 

Aa and Ai = Areas of linear apputenances, w/o & with ice

 $Kz = Exposure coefficient = (z/33)^{2/7}$ ; 1.00<=Kz<=2.58

Ae = Effective area = Avg. width\*section length

Force = Qz\*Gh\*(Cf\*Ae+Ca\*Aa)

#### Wind Load Without Ice

	Midpoint	Are	as	-					
Section	Height	Ae	Aa	Kz	Qz	Gh	Cf	Wind Load	Wind Load
10	0	0.0	0.00	1.00	18.50	1.69	0.59	0 lbs.	#DIV/0! plf.
9	175.5	62.0	0.97	1.61	29.82	1.69	0.59	1902 lbs.	61 plf.
8	150	50.0	0.63	1.54	28.51	1.69	0.59	1457 lbs.	73 plf.
7	130	60.0	0.63	1.48	27.37	1.69	0.59	1672 lbs.	84 plf.
6	110	70.0	0.63	1.41	26.09	1.69	0.59	1854 lbs.	93 plf.
5	90	80.0	0.63	1.33	24.64	1.69	0.59	1996 lbs.	100 plf.
4	70	90.0	0.63	1.24	22.93	1.69	0.59	2087 lbs.	104 pif.
3	50	100.0	0.63	1.13	20.83	1.69	0.59	2103 lbs.	105 plf.
2	30	100.0	0.63	1.00	18.50	1.69	0.59	1868 lbs.	93 plf.
1	10	100.0	0.63	1.00	18.50	1.69	0.59	1868 lbs.	93 plf.

#### Wind Load With Ice

	Midpoint	Are	as	<del>- 11 </del>		<u> </u>	Γ		<u> </u>
Section	Height	Ae	Ai	Kz	Qz	Gh	Cf	Wind Load	75% Wind Load
10	0	0.0	0.00	1.00	18.50	1.69	0.59	0 lbs.	#DIV/0! plf.
9	175.5	64.6	3.55	1.61	29.82	1.69	0.59	2135 lbs.	52 plf.
8	150	51.7	2.29	1.54	28.51	1.69	0.59	1601 lbs.	60 plf.
7	130	61.7	2.29	1.48	27.37	1.69	0.59	1810 lbs.	68 plf.
6	110	71.7	2.29	1.41	26.09	1.69	0.59	1986 lbs.	74 plf.
5	90	81.7	2.29	1.33	24.64	1.69	0.59	2121 lbs.	80 plf.
4	70	91.7	2.29	1.24	22.93	1.69	0.59	2202 lbs.	83 plf.
3	50	101.7	2.29	1.13	20.83	1.69	0.59	2208 lbs.	83 plf.
2	30	101.7	2.29	1.00	18.50	1.69	0.59	1961 lbs.	74 plf.
1	10	101.7	2.29	1.00	18.50	1.69	0.59	1961 lbs.	74 plf.

	_		.=======	NOD	A L									DITI	оия			
	REBAN	D		ORDII	NATE			*^p" #F	up.	ALPHA		BETA	GAMMA	•	ŗ	DDQQQQ XYZXYZ		FNESS
NO	NO =====	=====	X :======	Y	======	Z =======	1 ======	ODE TE	MP =====	ALPRA	====							
Jnits			Ft	Ft		Ft		F		Deg		Deg	Deg				к /:	n /Deg
1	1		0.00	(	0.00	0.	00		0.00	0.0	0	0.00	0.00	)	F	FFFFF		
2	2		0.00	20	0.00	0.	00		0.00	0.0		0.00	0.00					
3	3		0.00	40		0.			0.00	0.0		0.00	0.00					
4	4		0.00	6		0.			0.00	0.0		0.00	0.00					
5	5		0.00	81	0.00	0.			0.00	0.0		0.00	0.00					
6	6		0.00	100 120	3.00	0.			0.00	0.0		0.00	0.00					
7	7		0.00	140	2.00	0. 0.	00		0.00 0.00	0.0			0.00					
8 9	8 9		0.00		3.00	0.			0.00	0.0			0.00					
10	10		0.00	19:		0.			0.00	0.0		0.00	0.00					
			CTIVE NOD QUATIONS		10 54													
5====	=====	## <b>#</b>	========	=======													======	:======
						ODE							MENT		em.		STIFF	2231
ELEM NO	NO	NO	ALPHA	BETA		LENGTH	TYPE	TYPE	NE	EASE PE	TE	MP D	IR N	Œ	PE		NE	. PĒ
===== Units		****	======= Deg	Deg	Deg	Ft	-====	======	=====	=====	==== F		====== F	:======= `t	Ft	K /I	n /Deg	K /In /
		_		-	_			_										
	1		90.00			20.00	1											
2	2	-	90.00			20.00	1											
3 4	3 4		90.00 90.00			20.00	1											
5	5		90.00			20.00	1											
6	6		90.00			20.00	î											
7	7		90.00			20.00	ī											
8	8		90.00			20.00	1											
9	9	10	90.00	-90.00	0.00	31.00	1	9										
			CTIVE PRI						-									
24222	=====	====			-======	MATE		A L	PRO	PERT	ΙE	S						
MATL NO			GNATION		DULUS		TIO	C	DEFF	D	ENSIT	Y	WEIG! DENSI	TY		:========	======	£======
Units					/In ^2					Slug/F			Lb/F					
1	Pole			2	.9e+004	0	.250	6.5	e-006		15.	2	4	90				
====		=====														.======	##=====	=======
PROP	DESIG	NATIO		2 NO	DE P	RISM				ELE J		N T IXY	PROP SFY	SFX	ES	CW		
		====	*========										======			In^6	======	=======
Units	:			In	^2	In^4		In^4	1	Ω4		In^4				In o		
1	Secti	on 1			117 5	.14e+004	5.1	L4e+004	1.0	3e+005		0	1.000	1.000		0		
	Secti					.14e+004				7e+004			1.000			0		
	Sect.					.12e+004		L2e+004					1.000			0		
	Sect.					.27e+004		27e+004					1.000			0		
	Sect.			!		.59e+004		9e+004					1.000			0		
	Sect.					.06e+004		6e+004		2e+004			1.000			0		
						.66e+003		66e+003		3e+004			1.000			0		
	Sect. Sect.					.83e+003		33e+003 94e+003		8e+003			1.000			ŏ		
=====	=====	=====	******		=======	*======	33722		=====		37## <b>=</b>	*****	========	******	.=====		======	
REC						AVIT												
NO			F	x		PY		PZ										
DESCR LOAD	IPTION CASES	: P	ole dead				*****		=====	======	2222	=====	<b></b>		.====		######	=======
	NT LIS	T : 1	-															
1			C	0.000	-	1.300		0.0	00									

EC LOAD				DIST	PX	PY	PZ	MX	MY	MZ
=======	213	======	======	######################################	**************************************	======================================	K	Ft-K	========= Ft-K	======================================
nits:					K.	•				
ESCRIPTION	: 1	ind on	Sectio	n 1						
LEMENT LIS	5T : 1									
1 UNI	GLO	FRAC	B E	0.000	0.093	0.000	0.000 0.000	0.000 0.000	0.000	0.000 0.000
				1.000	0.055					
ESCRIPTION OAD CASES	1 : W	ind on	2							
ELEMENT LI	ST : 2									0.000
2 UNI	F GLO	FRAC	B E	0.000	0.093 0.093	0.000 0.000	0.000 0.000	0.000	0.000	0.000
				11000	*****					
DESCRIPTION LOAD CASES	N : W		3							
LEMENT LI	ST : 3									0.000
3 UNI	F GLC	FRAC	B E	0.000 1.000	0.105 0.105	0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000
	_	st 2	_		- · • • •	- · - · <del>-</del>				
DESCRIPTIO LOAD CASES	: 1		4							
ELEMENT LI	ST : 4								2 222	0.000
4 UNI	F GLC	FRAC	B E	0.000	0.104 0.104	0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000
		ind on		2						
DESCRIPTIO LOAD CASES	: 1		5							
ELEMENT LI	ST : 5	•								0.000
5 UNI	F GLO	FRAC	B E	0.000 1.000	0.100 0.100	0.000	0.000 0.000	0.000 0.000	0.000	0.000
DESCRIPTIO	N . 1	lind on	6							
LOAD CASES	. 1		Ü							
ELEMENT LI							0.000	0.000	0.000	0.000
6 UNI	F GLO	FRAC	B E	0.000 1.000	0.093 0.093	0.000 0.000	0.000 0.000	0.000	0.000	0.000
DESCRIPTIO	N	lind on	7							
LOAD CASES	: :		•							
ELEMENT LI						0.000	0.000	0.000	0.000	0.000
7 UNI	F GLO	FRAC	B E	0.000 1.000	0.084 0.084	0.000	0.000	0.000	0.000	0.000
DESCRIPTIO	N : 1	lind on	8							
LOAD CASES	: :	L	-							
ELEMENT LI					0.053	0.000	0.000	0.000	0.000	0.000
8 UNI	F GLO	FRAC	B E	0.000 1.000	0.073 0.073	0.000	0.000	0.000	0.000	0.000
DESCRIPTIO	N : 1	Vind on	9							
LOAD CASES ELEMENT LI	: :	L								
•			_	0.000	0.001	0.000	0.000	0.000	0.000	0.000
9 UNI	F GLO	FRAC	B E	0.000	0.061 0.061	0.000	0.000	0.000	0.000	0.000
DESCRIPTIO	on :	(4) RV9	0-17 a:	n (2) RV33-20	on platform a	it 187'				
LOAD CASES	: :	L								
DISTANCES	: :									
101 CON	IC GL	DIST			3.058	-1.840	0.000	0.000	0.000	0.000
DESCRIPTION	on :	(12) AL	P9212	panels on pla	tform @ 175'					
LOAD CASES	· :	1		-						
DISTANCES	:									
102 CON	IC GL	O DIST			4.185	-1.960	0.000	0.000	0.000	0.000

DESCRIPTION : (12) ALP7120.16 panels on platform @ 165' LOAD CASES : 1 ELEMENT LIST : 9

DISTANCES

0.000 0.000 0.000 0.000 CONC GLO DIST 3.948 -1.900

REC NO	ALPHA	BETA	GAMMA	PX	NODAL PY	LOADS PZ	мх	MY	MZ
======= Units:	Deg	Deg	Deg	K K	**************************************	к	Ft-K	Ft-K	Ft-K
DESCRIPT LOAD CAS NODE LIS	SES :	1	cables:dea	d load					
1	0.00	0.00	0.00	0.000	-0.769	0.000	0.000	0.000	0.000
DESCRIPT LOAD CAT NODE LI	SES : 3		d.1.						
2	0.00	0.00	0.00	0.000	-0.418	0.000	0.000	0.000	0.000

LINEAR ANALYSIS RESULTS STRUCTURE LOAD COMBINATIONS LIST OF FACTORS \* CASES LOAD COMBINATIONS: COMB 1 (Wind + dead loads) : 1.00 X CASE 1 NODAL DISPLACEMENTS NODE LOAD (\* Indicates Displacements Occur in Nodal Local System) COMB DX DY DZ ox OY NO ΟZ ======== ========= In In Deg Deg 1 0.0000 0.0000 0.0000 . 0.0000 0.0000 0.0000 0.7589 0.0000 0.0000 -0.0041 0.0000 -0.3501 3.0151 -0.0083 0.0000 0.0000 0.0000 -0.7130 6.8604 -0.0129 0.0000 0.0000 1 0.0000 -1.105612.4332 -0.0172 0.0000 0.0000 0.0000 -1.5343 6 1 19.8951 -0.0211 0.0000 0.0000 0.0000 -2.0024 29.4163 -0.0247 0.0000 0.0000 0.0000 -2.5096 41.1402 0.0000 0.0000 -0.0279 0.0000 9 1 55.0561 -0.0308 0.0000 0.0000 0.0000 -3.5313 10 79.3791 0.0000 -0.0332 0.0000 0.0000 -3.8151 NODE PRISMATIC BEAM ELEMENT --FORCES AND MOMENTS ELEM LOAD NODE SIGN CONVENTION : BEAM DESIGNERS NO AXIAL TORSION COMB MOMENT X NO SHEAR X SHEAR Y MOMENT Y Units: ĸ K K -Ft 1 -62.5735 0.0000 27,9820 -3434.5115 0.0000 0.0000 2 0.0000 -52,2577 0.0000 26.1220 -2893.4715 0.0000 2 -51.4887 0.0000 26.1220 -2893.4715 0.0000 0.0000 -43.2165 0.0000 24.2620 -2389.6315 0.0000 0.0000 -42,4475 0.0000 24.2620 -2389.6315 0.0000 0.0000 -36.2368 -1925.3915 0.0000 0.0000 22.1620 0.0000 1 -35.4678 0.0000 22.1620 -1925.3915 0.0000 0.0000 5 -29.8781 0.0000 20.0820 -1502.9515 0.0000 0.0000 0.0000 -29.1091 0.0000 20.0820 -1502.9515 0.0000 -1121.3115 0.0000 -24.1449 0.0000 18.0820 0.0000 6 -23.3759 0.0000 18.0820 -1121.3115 0.0000 0.0000 7 -19.0408 0.0000 16.2220 -778.2715 0.0000 0.0000 -18.2718 0.0000 16.2220 -778.2715 0.0000 0.0000 0.0000 -14.5559 0.0000 14.5420 -470.6315 0.0000 1 8 -13.7869 0.000014.5420 -470.6315 0.0000 0.0000 9 -10.6993 0.0000 13.0820 -194.3915 0.0000 0.0000 9 -9.9303 0.0000 0.0000 13.0820 -194.3915 0.0000 10 -0.4180 0.0000 0.0000 0.0000 0.0000 0.0000

REACTIONS NODE (\* Indicates Reactions Occur in Nodal Local System) NO COMB РX PΥ PZ MX MY Units: K K K K -Ft K -Ft K -Ft -27.9820 62.5735 0.0000 0.0000 0.0000 3434.5115

								=======================================
*= * * * * * * * * * * * * * * * * * *	=======================================	=========			=======================================	NATIONS	#==========	=======================================
COMB L	IST OF FACT	ORS * CASES					=======================================	
LOAD COM	:======= :BINATIONS:		5					
сомв 1	(Wind + dea	d loads) :	1.00 X CASE 1					
========	:========						=======================================	=======================================
NODE NO	LOAD		(* Indic	ates Displacement	s Occur in Nodal	Local System)	OY	OZ ====================================
======= Units:		==========	======================================	======================================	In	Deg	Deg	Deg
1	1		0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
2	1		0.7858	-0.0054	0.0000	0.0000	0.0000	-0.3628
-	1		3.1274	-0.0209	0.0000	0.0000	0.0000	-0.7408
3			7.1272	-0.0589	0.0000	0.0000	0.0000	-1.1512
4	1			-0.1334	0.0000	0.0000	0.0000	-1.6007
. 5	1		12.9356		0.0000	0.0000	0.0000	-2.0928
6	1		20.7268	-0.2637	• • • • • • • • • • • • • • • • • • • •	0.0000	0.0000	-2.6265
7	1		30.6829	-0.4738	0.0000		0.0000	-3.1858
8	1		42.9552	-0.7908	0.0000	0.0000		-3.7022
9	1		57.5315	-1.2366	0.0000	0.0000	0.0000	
10	1		83.0240	-2.1133	0.0000	0.0000	0.0000	-4.0027
========			RISMATIC	BEAM EL	======================================	FORCES	AND MOME	======================================
ELEM	2 1 LOAD	NODE F NODE	RISMATIC	N CONVENTION : B				
	LOAD					MOMENT X	MOMENT Y	TORSION
NO	COMB		AXIAL	SHEAR X	SHEAR Y	MOMENT X	MOMENT Y	TORSION ========= K -Ft
NO ======= Units:	COMB			SHEAR X	SHEAR Y =========== K	K -Ft	**************************************	K -Ft
NO	COMB		AXIAL	SHEAR X	SHEAR Y	*******	MOMENT Y K -Ft 0.0000 0.0000	TORSION
NO ======= Units:	COMB	NO ====================================	AXIAL K -62.4816 -52.1719	SHEAR X ************************************	SHEAR Y ========== K 28.1867	K -Ft -3551.9011 -3007.1038 -3007.1039	K -Ft 0.0000 0.0000	K -Ft 0.0000 0.0000
NO ======= Units:	COMB	NO ====================================	AXIAL K -62.4816	SHEAR X K 0.0000 0.0000	SHEAR Y	K -Ft -3551.9011 -3007.1038	K -Ft 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000
NO ======= Units:	COMB	NO  1 2 2 3 3	AXIAL  -62.4816 -52.1719 -51.2314 -42.9778 -42.0373	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  ===================================	K -Ft -3551.9011 -3007.1038 -3007.1039 -2494.0475 -2494.0489	K -Ft 0.0000 0.0000	K -Ft 0.0000 0.0000
NO ======= Units: 1	COMB 1 1	1 2 2 3 3 4	AXIAL  -62.4816 -52.1719  -51.2314 -42.9778  -42.0373 -35.8624	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  ===================================	K -Ft -3551.9011 -3007.1038 -3007.1039 -2494.0475 -2494.0489 -2016.7593	K -Ft  0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000
NO ======= Units: 1	COMB 1 1	NO  1 2 2 3 3	AXIAL  -62.4816 -52.1719 -51.2314 -42.9778 -42.0373	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  ===================================	K -Ft -3551.9011 -3007.1038 -3007.1039 -2494.0475 -2494.0489	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
NO THE SERVICE STATES TO SERVI	1 1 1 1	NO 1 2 2 3 3 4 4 4	AXIAL  -62.4816 -52.1719 -51.2314 -42.9778 -42.0373 -35.8624 -34.9210	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  ===================================	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7593  -2016.7613 -1578.6302	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
NO Units:  1 2	1 1	1 2 2 3 3 4 4 5 5	AXIAL  -62.4816 -52.1719 -51.2314 -42.9778 -42.0373 -35.8624 -34.9210 -29.3833	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  28.1867 26.2930  26.6231 24.6825  24.9661 22.7629  23.0139 20.7992 21.0164 18.8563	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7593  -2016.7613 -1578.6297  -1578.6302 -1179.9033	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
NO THE SERVICE STATES TO SERVI	1 1 1 1	1 2 2 3 3 4 4 5 5 6 6 6	AXIAL  -62.4816 -52.1719  -51.2314 -42.9778  -42.0373 -35.8624  -34.9210 -29.3833  -28.4419 -23.5453  -22.6058	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  ===================================	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7593  -2016.7613 -1578.6302	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
NO ====================================	1 1 1 1 1 1	1 2 2 3 3 4 4 5 5 6 6 6 7	AXIAL  -62.4816 -52.1719 -51.2314 -42.9778 -42.0373 -35.8624 -34.9210 -29.3833 -28.4419 -23.5453 -22.6058 -18.3516	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  28.1867 26.2930  26.6231 24.6825  24.9661 22.7629  23.0139 20.7992  21.0164 18.8563  19.0362 16.9980	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7593  -2016.7613 -1578.6297  -1578.6302 -1179.9054	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
NO	1 1 1 1 1	1 2 2 3 3 4 4 5 5 6 6 6	AXIAL  -62.4816 -52.1719  -51.2314 -42.9778  -42.0373 -35.8624  -34.9210 -29.3833  -28.4419 -23.5453  -22.6058	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  ===================================	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7613 -1578.6302 -1179.9033  -1179.9054 -819.5632	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
NO ====================================	1 1 1 1 1 1 1	NO 1 2 2 3 3 4 4 5 5 6 6 7 7	AXIAL  -62.4816 -52.1719 -51.2314 -42.9778 -42.0373 -35.8624 -34.9210 -29.3833 -28.4419 -23.5453 -22.6058 -18.3516 -17.4179 -13.7928 -12.8791	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  ===================================	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7593  -2016.7613 -1578.6297 -1578.6302 -1179.9033 -1179.9054 -819.5632 -819.5679 -495.5383	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
NO = = = = = = = = = = = = = = = = = = =	1 1 1 1 1 1 1 1 1 1	1 2 2 3 4 4 5 5 6 6 7 7 8 8	AXIAL  -62.4816 -52.1719 -51.2314 -42.9778 -42.0373 -35.8624 -34.9210 -29.3833 -28.4419 -23.5453 -22.6058 -18.3516 -17.4179 -13.7928 -12.8791 -9.8858	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  28.1867 26.2930  26.6231 24.6825  24.9661 22.7629  23.0139 20.7992  21.0164 18.8563  19.0362 16.9980  17.1353 15.2675  15.3521 13.7073	K -Ft  -3551.9011 -3007.1038 -3007.1039 -2494.0475 -2494.0489 -2016.7693 -2016.7613 -1578.6297 -1578.6302 -1179.9054 -819.5632 -819.5679 -495.5383 -204.9443	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
NO = = = = = = = = = = = = = = = = = = =	1 1 1 1 1 1 1 1 1 1	NO 1 2 2 3 3 4 4 5 5 6 6 7 7 8 8 8	AXIAL  -62.4816 -52.1719 -51.2314 -42.9778 -42.0373 -35.8624 -34.9210 -29.3833 -28.4419 -23.5453 -22.6058 -18.3516 -17.4179 -13.7928 -12.8791	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  ===================================	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7593  -2016.7613 -1578.6297 -1578.6302 -1179.9033 -1179.9054 -819.5632 -819.5679 -495.5383	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000
NO	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	NO 1 2 2 3 3 4 4 5 5 6 6 7 7 8 8 8 9 9 10	AXIAL  -62.4816 -52.1719 -51.2314 -42.9778 -42.0373 -35.8624 -34.9210 -29.3833 -28.4419 -23.5453 -22.6058 -18.3516 -17.4179 -13.7928 -12.8791 -9.8858 -9.0097 -0.4162	SHEAR X  0.0000	SHEAR Y  28.1867 26.2930  26.6231 24.6825  24.9661 22.7629  23.0139 20.7992  21.0164 18.8563  19.0362 16.9980  17.1353 15.2675  15.3521 13.7073  13.7318 0.0287	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7593  -2016.7613 -1578.6302 -1179.9054 -819.5632  -819.5679 -495.5383 -204.9443  -204.9429 0.0013	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000
NO	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	NO 1 2 2 3 3 4 4 5 5 6 6 7 7 8 8 8 9 9 10	AXIAL  K  -62.4816 -52.1719  -51.2314 -42.9778  -42.0373 -35.8624  -34.9210 -29.3833  -28.4419 -23.5453  -22.6058 -18.3516  -17.4179 -13.7928  -12.8791 -9.8858 -9.0097 -0.4162	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  28.1867 26.2930  26.6231 24.6825  24.9661 22.7629  23.0139 20.7992  21.0164 18.8563  19.0362 16.9980  17.1353 15.2675  15.3521 13.7073  13.7318 0.0287	K -Ft  -3551.9011 -3007.1038 -3007.1039 -2494.0475 -2494.0489 -2016.7613 -1578.6297 -1578.6302 -1179.9033 -1179.9054 -819.5632 -819.5679 -495.5383 -204.9443 -204.9443	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000
NO ====================================	1 1 1 1 1 1 1 1 1 1 LOAD	NO 1 2 2 3 3 4 4 5 5 6 6 7 7 8 8 8 9 9 10	AXIAL  K  -62.4816 -52.1719  -51.2314 -42.9778  -42.0373 -35.8624  -34.9210 -29.3833  -28.4419 -23.5453  -22.6058 -18.3516  -17.4179 -13.7928  -12.8791 -9.8858 -9.0097 -0.4162	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  28.1867 26.2930  26.6231 24.6825  24.9661 22.7629  23.0139 20.7992  21.0164 18.8563  19.0362 16.9980  17.1353 15.2675  15.3521 13.7073  13.7318 0.0287	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7613 -1578.6297  -1578.6302 -1179.9033 -1179.9054 -819.5632  -819.5679 -495.5391  -495.5383 -204.9443  -204.9429 0.0013	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000
NO ====================================	COMB  1  1  1  1  1  1  1  LOAD COMB	NO 1 2 2 3 3 4 4 5 5 6 6 7 7 8 8 8 9 9 10	AXIAL  K  -62.4816 -52.1719  -51.2314 -42.9778  -42.0373 -35.8624  -34.9210 -29.3833  -28.4419 -23.5453  -22.6058 -18.3516  -17.4179 -13.7928  -12.8791 -9.8858 -9.0097 -0.4162	SHEAR X  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	SHEAR Y  28.1867 26.2930  26.6231 24.6825  24.9661 22.7629  23.0139 20.7992  21.0164 18.8563  19.0362 16.9980  17.1353 15.2675  15.3521 13.7073  13.7318 0.0287	K -Ft  -3551.9011 -3007.1038  -3007.1039 -2494.0475  -2494.0489 -2016.7613 -1578.6297  -1578.6302 -1179.9033 -1179.9054 -819.5632  -819.5679 -495.5391  -495.5383 -204.9443  -204.9429 0.0013	K -Ft  0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000	K -Ft  0.0000

2605 White Mountain Highway, PO Box 440 North Conway, NH 03860 (603) 356-6936

Client:

Maguire Group

Job:

Old Lyme, CT

Job No.: 2000-032

Calculated By: Checked By: R. Adair

Date: 3-

Date:

3-Mar-00

Moments from Tower Without Ice @ Sections

Total (ft-kips)		1479	1162	882	641	443	285	166	82	29	0
0	0	. 0	0	0	0	0	0	0	0	0	C
1902	176	333759	295724	257689	219653	181618	143583	105548	67513	29477	C
1457	150	218605	189458	160310	131163	102016	72868	43721	14574	0	C
1672	130	217340	183903	150466	117029	83592	50155	16718	0	0	C
1854	110	203947	166866	129784	92703	55622	18541	0	0	0	
1996	90	179676	139748	99820	59892	19964	0	0	0	0	C
2087	70	146069	104335	62601	20867	0	o	0	0	0	C
2103	50	105155	63093	21031	0	0	0	0	0	0	
1868	30	56030	18677	0	0	0	0	. 0	0	0	(
1868	10	18677	0	0	0	0	0	0	0	0	
Ice	Force	0	20	40	60	80	100	120	140	160	191
lower Force w/o	Elev. of										

Moments from Tower With Ice @ Sections

Tower Force w/	Elev. of										
Ice	Force	0	20	40	60	80	100	120	140	160	191
1961	10	19609	0	0	0	0	0	0	0	0	C
1961	30	58828	19609	0	0	0	0	0	0	0	C
2208	50	110406	66243	22081	0	0	0	0	0	0	C
2202	70	154161	110115	66069	22023	0	0	0	0	0	C
2121	90	190855	148443	106030	63618	21206	0	0	0	0	C
1986	110	218416	178704	138992	99280	59568	19856	0	0	0	C
1810	130	235276	199080	162883	126687	90491	54294	18098	0	0	C
1601	150	240165	208143	176121	144099	112077	80055	48033	16011	0	C
2135	176	374651	331956	289260	246565	203870	161175	118479	75784	33089	C
아	이	0	0	0	0	0	0	0	0	0	0
Total (ft-kips)		1602	1262	961	702	487	315	185	92	33	0

Total Moment (Tower & Antennas)

Axial Loads (kips) Shear

	Mom.	75% Mom	100% Mom	P-Delta	D+A	D+A+!	P-Delta	Tower	Antenna	Total	P-Delta
Elevation	w/o lce	w/ lce	w/ ice	Secondary	Force	Force	Secondary	(lbs.)	(ibs)	(kips)	Secondary
0	3434.8	2853.5	3804.7	3551.9	45.2	50.5	62.5	17984	10305	28.29	28.2
20	2893.6	2409.6	3212.8	3007.1	37.2	42.5	51.2	16023	10305	26.33	26.6
40	2389.6	1995.0	2660.0	2494.0	30.9	35.4	42.0	14062	10305	24.37	25.0
60	1925.4	1611.7	2149.0	2016.7	26.1	29.9	34.9	11854	10305	22.16	23.0
80	1503.1	1261.5	1682.0	1578.6	21.8	24.9	28.4	9652	10305	19.96	21,0
100	1121.7	943.7	1258.3	1179.9	18.0	20.4	22.6	7531	10305	17.84	19.0
120	778.7	656.7	875.6	819.6	14.6	16.5	17.4	5546	10305	15.85	17.1
140	471.0	398.2	530.9	495.5	6.8	13.1	12.9	3736	10305	14.04	15.4
160	194.5	165.3	220.3	204.9	9.1	9.9	9.0	2135	10305	12.44	13.7
191	0.0	0.0	0.0		0.0	0.5		0	o	0.00	

2605 White Mountain Highway, PO Box 440 North Conway, NH 03860 (603) 356-6936

Client:

Maguire Group

Job:

Old Lyme, CT

Job No.: 2000-032

#########

Calculated By: Checked By:

R. Adair

Date:

Date:

**Evaluation of Monopole** 

fb= Moment/Section Modulus

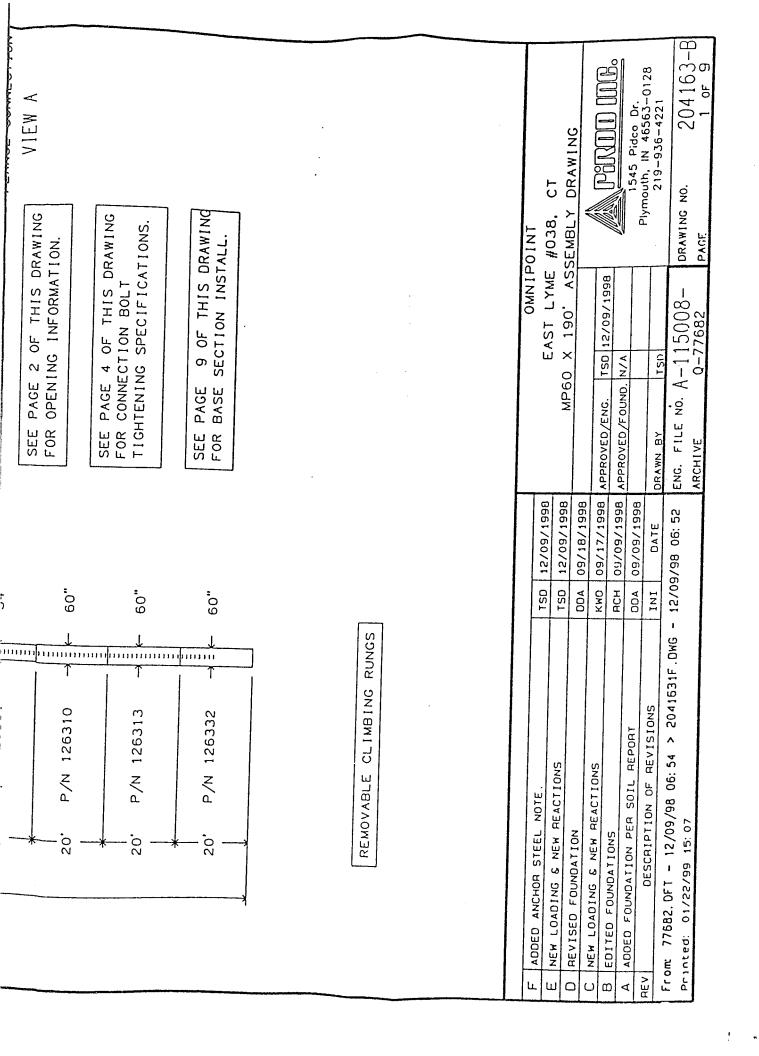
	Axial Force						
	Axial Force		Actual				
Elev.	w/o ice	Area	Fy	w/o ice			
0	62.5	116.58	42	0.54			
20	51.2	93.46	42	0.55			
40	42.0	70.24	42	0.60			
60	34.9	63.18	42	0.55			
80	28.4	56.11	42	0.51			
100	22.6	49.04	42	0.46			
120	17.4	41.97	42	0.41			
140	12.9	34.90	42	0.37			
160	9.0	27.83	42	0.32			
191	0.0	0.00	42	#DIV/0!			

	Bending Force	Bending Stress			
			Allo	wable	Actual
Elev.	w/o ice	S	Fb	1.33 Fb	w/o ice
0	3551.9	1712.67	25.2	33.52	24.89
20	3007.1	1378.75	25.2	33.52	26.17
40	2494.0	1040.56	25.2	33.52	28.76
60	2016.7	841.10	25.2	33.52	28.77
80	1578.6	662.84	25.2	33.52	28.58
100	1179.9	505.79	25.2	33.52	27.99
120	819.6	369.94	25.2	33.52	26.59
140	495.5	255.29	25.2	33.52	23.29
160	204.9	161.86	25.2	33.52	15.19
191	0.0	#DIV/0!	25.2	33.52	#DIV/0!

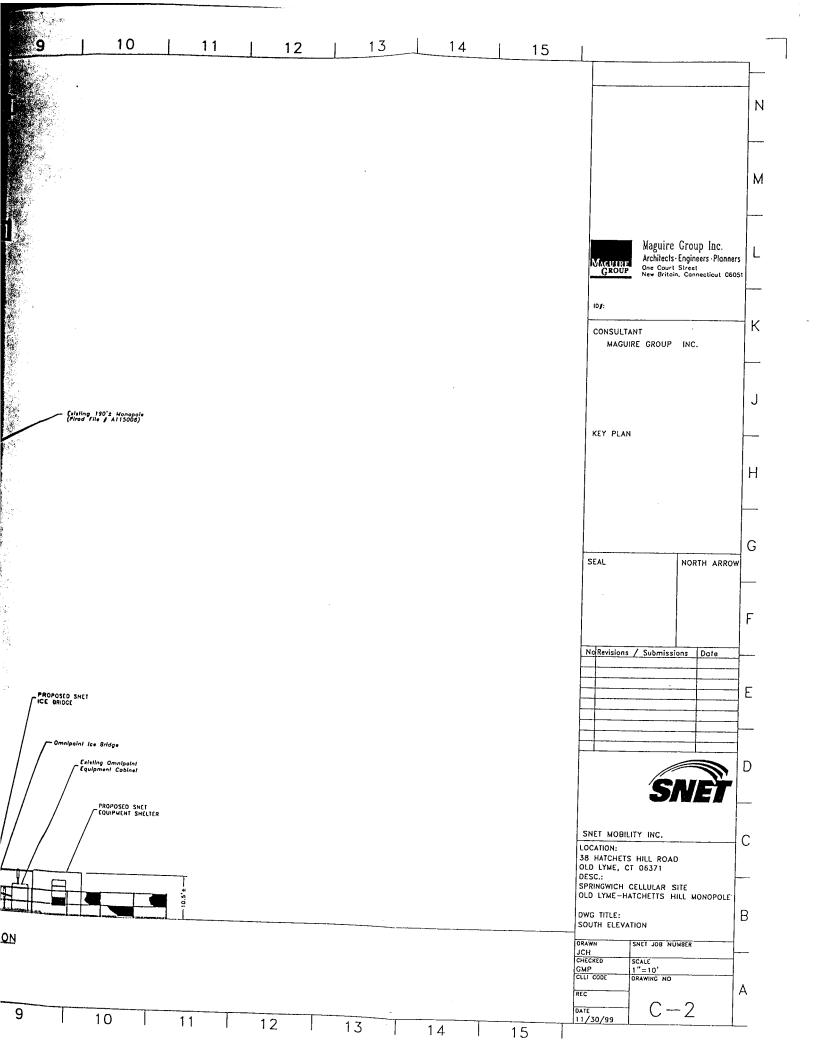
Elev.	Comb. Loads	Capacity
0	0.759	76%
20	0.797	80%
40	0.876	88%
60	0.875	87%
80	0.868	87%
100	0.849	85%
120	0.806	81%
140	0.706	71%
160	0.463	46%
191	#DIV/0!	#DIV/0!

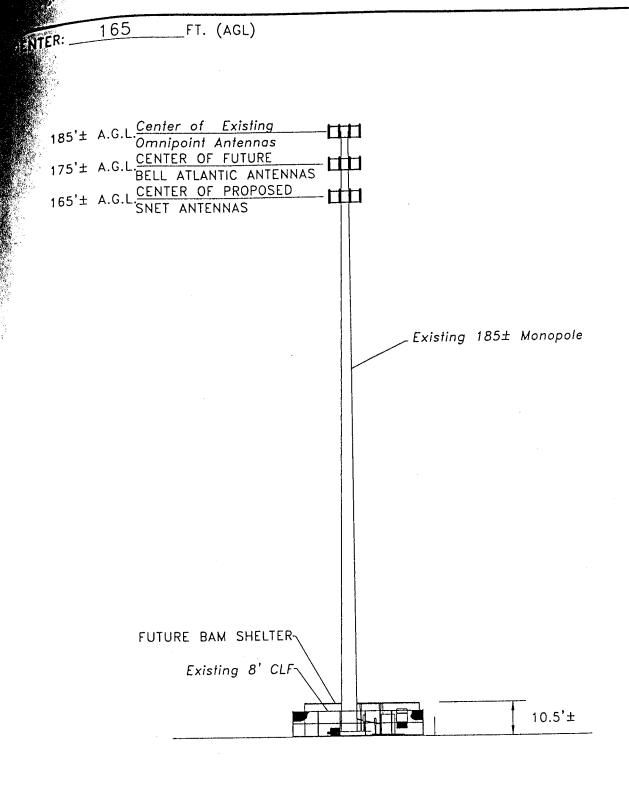
### Appendix B

Drawings



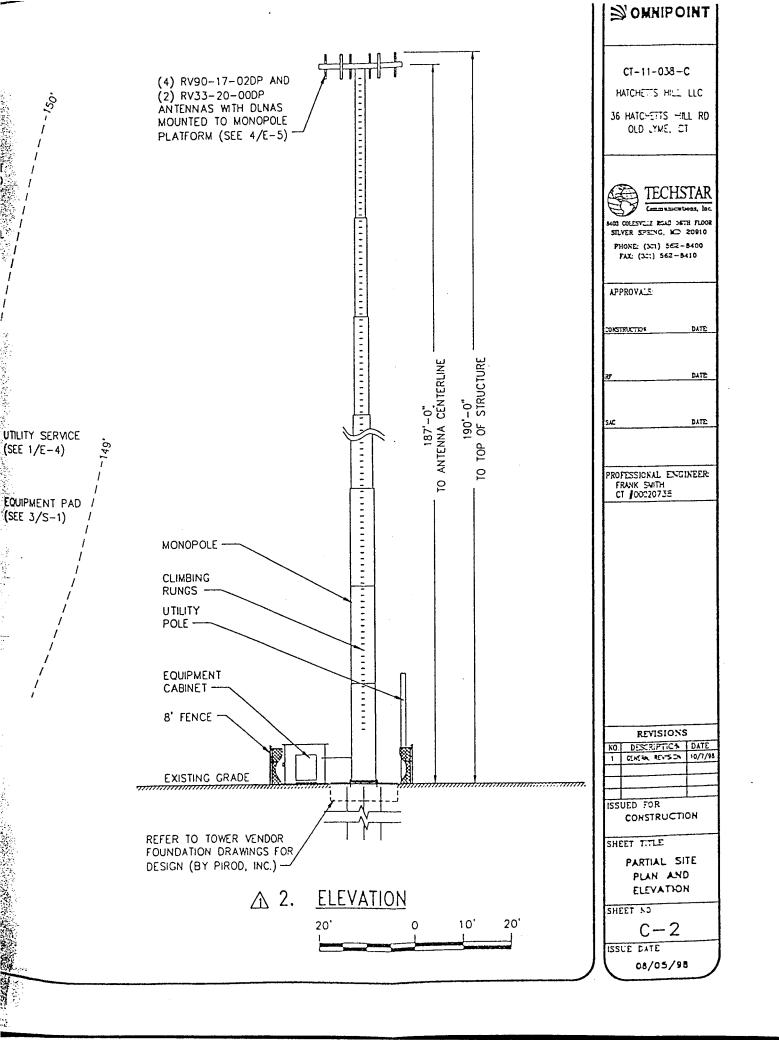
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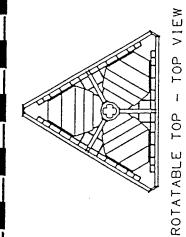


# NORTH ELEVATION SCALE: 1"=30"

ET N	MOBILITY INARY EXHIBIT	NORTH	SITE NAME: C	OLD LYME - ON	MNIPOINT MONO		SNET #: MGI #:14777
ELIMI	INARY			38 HATCHETS H	HILL ROAD		TASK #: 1135
SIGN	EXHIBIT			OLD LYME, CT	SCALE: 1"=30	<del>,                                    </del>	DATE: 2/3/00
	Magnire Group						
CUIRE POUP	Maguire Group In  Architects Engineers One Court Street		THIS DRAWING INFORMATIONAL OR CONSTRUCT	AND ALL DATA CON PURPOSES ONLY. N ION USE. ALL DATA	ITAINED HEREIN IS F NOT INTENDED FOR SHOULD BE VERIFII	OR DESIGN ID	SNET



ā



												<u>,                                     </u>
	T	7#		3	n	m	η.	m	m			<u> </u>
BOTTOM OF SECTION)	PILOT BOLT	LENGTH		5	5"	5	5"	5"	5			
	PILO	DIAM		1	1	1	1		1			
110		*	80	17	21	25	29	33	45	64	64	
80	-	エト	/2	/2"	/2"	/2"	/2"	12.	/2"	/2"	/2"	
	T B01	LENG	4-1/2"	4-1/2"17	4-1/2"21	4-1/2"25	4-1/2"29	4-1/2"33	4-1/2" 45	4-1/2" 64	4-1/2" 64	
	CONNECT BOLT	DIAM  LENGTH #	1		1	1		1		5701#1-1/4"	7779# 1-1/4"	
¥ - ¥ D - z		WT. *		3226#	2747#	3290#	3833#	4376#	4918#	5701#	17779#	60" 0. 625" 10007#
MONOR OLE SECTION DATA		WALL	N/A	24" 0.375"	30" 0.375"	0.375"	0.375"	0.375"	0.375"	60" 0.375"	60" 0.500"	). 625"
	NOI	SIZE	18"	24" (	30	36" (	42" (	48" (	54"	.09	.09	.09
	SECTION	PART# SIZE WALL	801458	126248	126057	126058	126059	126060	126061	126310	126313	126332
		ENGTH	-	30.	20.	20,	20,	20,	20,	20,	20,	20.

THEORETICAL, THE ACTUAL WEIGHTS
SHOULD BE CONFIRMED IN THE FIELD THE WEIGHTS LISTED ARE WILL VARY. ALL WEIGHTS PRIOR TO ERECTION

TOP 1' CONSISTS OF ROTATABLE TOP ASSEMBLY.
SEE DWG # 130555-B FOR INSTALLATION DETAILS.
JAM NUTS NOT REQUIRED.

54" 18" 24" 30" 36" 42" 48, .09 60" .09 126058 126059 126061 126060 332 126057 126310  $\mathcal{C}$ 801458 P/N 126248 12631 126. P\_N P/N P/N Z P N PN Z Z ۵.` ۵. ۵. ۵. 20, 20, 20, 20. 20, 20. 20 20, 91.

PAGE 2 OF THIS DRAWING OPENING INFORMATION. SEE FOR

FLANGE CONNECTION

VIEW A

TYPICAL FLUSH

CONNECTIONS
A-325 BOLTS
TABLE ABOVE
SIZE & QTY.

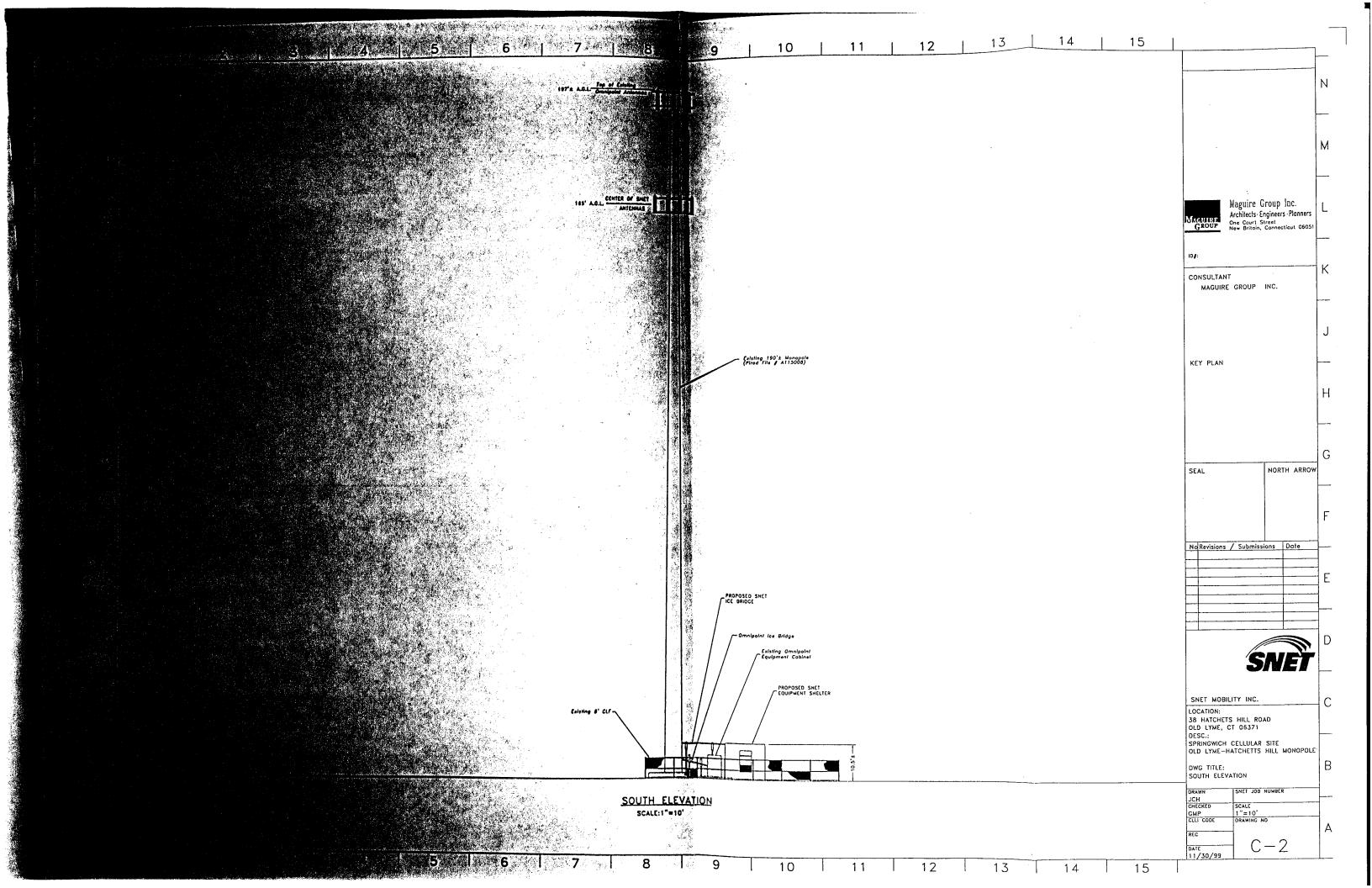
ALL ARE SEE FOR

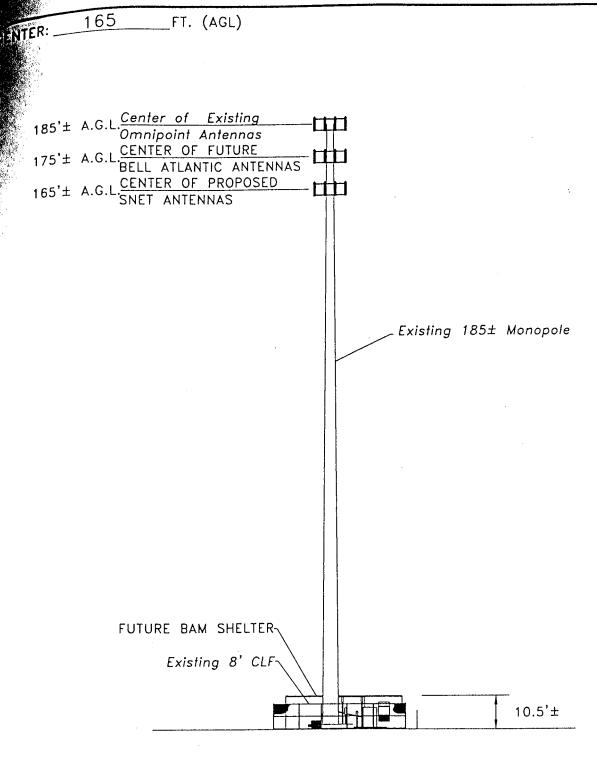
SEE PAGE 4 OF THIS DRAWING FOR CONNECTION BOLT TIGHTENING SPECIFICATIONS.

9 OF THIS DRAWING SECTION INSTALL. PAGE BASE SEE FOR

> CLIMBING RUNGS REMOVABLE

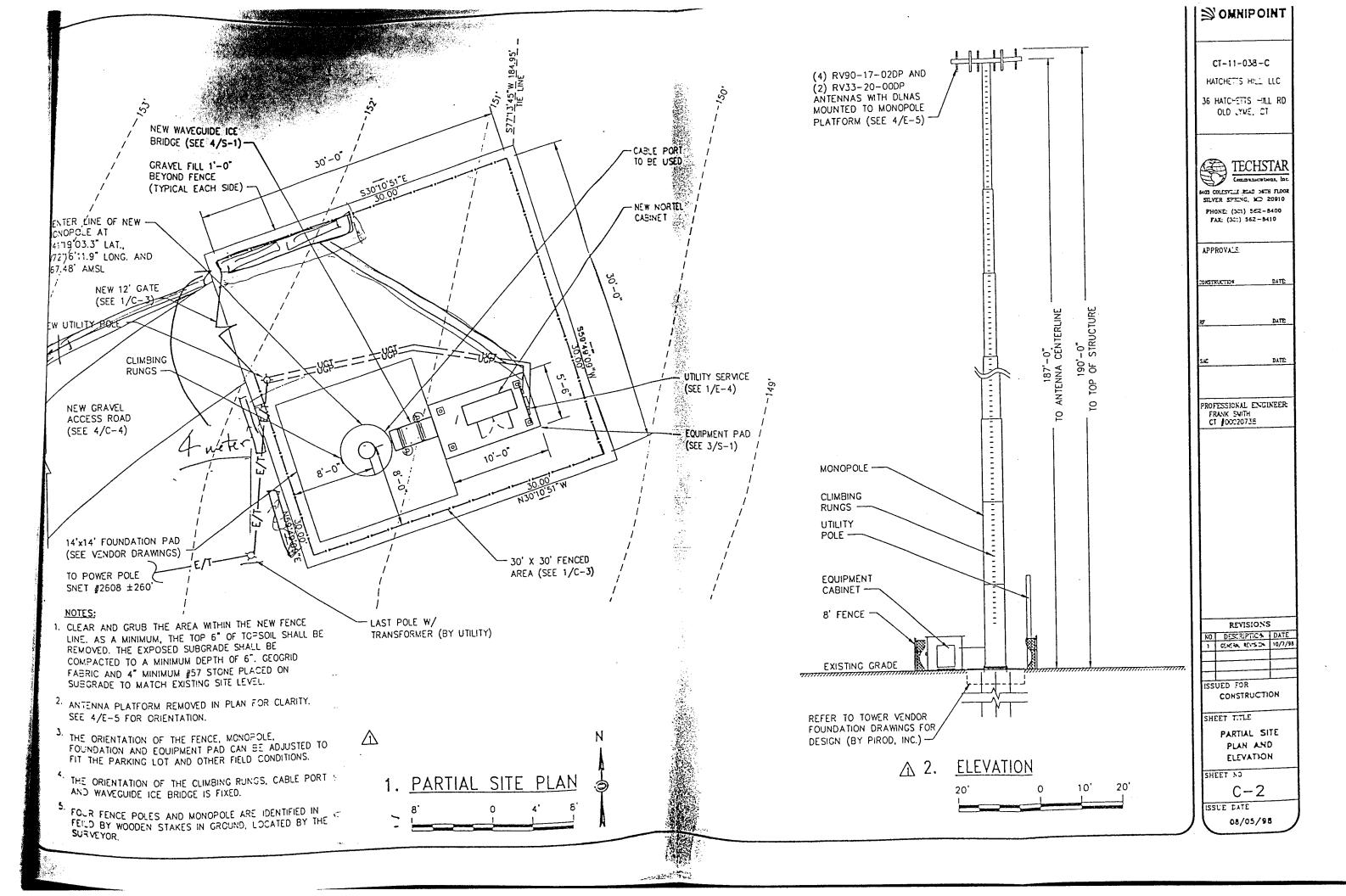
					TNIPOINT	OINT
u.	ADDED ANCHOR STEEL NOTE.	150	TSD 12/09/1998		EAST LYME #038. CT	#038. CT
Ш	NEW LOADING & NEW REACTIONS	150	150 12/09/1998	MP60	X 190' ASS	MP60 X 190' ASSEMBLY DRAWING
۵	REVISED FOUNDATION	00A	DDA 09/18/1998			
ပ	NEW LOADING & NEW REACTIONS	KWD	09/17/1998	KWD 09/17/1998 APPROVED /FNG 150/1998	12/09/1998	
മ	EDITED FOUNDATIONS	HCH	09/09/1998	ЯСН 09/09/1998 APPROVED /FOLIND N/A		
٨	ADDEO FOUNDATION PER SOIL REPORT	DDA	DDA 09/09/1998			1545 Pidco Dr. Plymouth IN 46561-0128
AEV	DESCRIPTION OF REVISIONS	INI	DATE	DRAWN BY	150	219-936-4221
<u>ر</u> ت	From 77682, DFT - 12/09/98 06: 54 > 2041631F, DWG -	12/05	- 12/09/98 06: 52	ENG. FILE NO. A-115008-	115008-	DRAWING NO.
٩	Printed: 01/22/99 15:07			A BCHIVE	0,000	





# NORTH ELEVATION SCALE:1"=30"

ET M	OBILITY	NORTH	SITE NAME: C	DLD LYME - OF	MNIPOINT MON		SNET #:
ELIMI	NARY EXHIBIT		ADDRESS: 38 HATCHEIS HILL ROAD OLD LYMF, CT			MGI #: 14777 TASK #: 1135 DATE: 2/3/00	
		<u></u>	DRAWN:GMP	CHECKED: RGT	SCALE: 1"=	30'	DATE: 2/3/00
CUIRE	A	eers ·Planners	INI OKMANONAL	AND ALL DATA CON PURPOSES ONLY, N			SNET



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