

1280 Route 46 West, Suite 9, Parsippany NJ, 07054

Ms. Melanie Bachman Executive Director CT Siting Council 10 Franklin Square New Britain, CT 06051

Re: Notice of Exempt Modification Application 2 Hinckley Hill Road, Norwich CT 06360

April 18, 2018

Dear Ms. Bachman:

Sprint Spectrum Realty Company, L.P. ("Sprint"), is submitting to the Connecticut Siting Council for a Notice of Exempt Modification for Proposed Modifications to an Existing Telecommunications Facility located at the above-referenced site. Sprint currently maintains 3 panel antennas and 6 Remote Radio at the 140' level of the Tower. Sprint proposes to add 3 new panel antennas (1 per sector) and 6 new Remote Radio Heads (2 per sector) and further proposes to add 1 new hybrid cable.

The earliest CT Siting Council submission I could find was issued to Sprint on November 29, 2013. The original Building permit for the actual tower construction issued by the Town was unavailable but there is a Building Permit from June 17, 2014. The attached construction and structural documents enclosed reflect the current reality of all the installations on the Tower.

If you have any questions, please feel free to contact me.

Thank you,

By: Paul F. Sagristano

Paul F. Sagristano Cherundolo Consulting 917.841.0247 psagristano@lrivassoc.com



4 Davis Road West, Suite 5 – Old Lyme, CT 06371

Ms. Melanie Bachman Executive Director CT Siting Council 10 Franklin Square New Britain, CT 06051

Re: Notice of Exempt Modification Application 2 Hinckley Hill Road, Norwich, CT 06360

Lat: N 41.5107 Long: W72.0596

April 18, 2018

Dear Ms. Bachman:

Sprint currently maintains 3 panel antennas and 6 Remote Radio Heads at the 140' level of the above noted wireless tower. Sprint proposes to add 3 panel antennas (1 per sector) and add 6 remote radio heads (2 per sector) at the 140' tower level as well as 1 new hybrid cable. Sprint is performing a new high-performance upgrade for cellular mobile communications. It is designed to increase the capacity and speed of mobile telephone networks.

The earliest CT Siting Council approval available was from November 29, 2013. The earliest building permit for the Tower construction was not available but a recent one from June 17, 2014 is included.

Attached is a summary of the planned modifications, including power density calculations reflecting the change in Sprint's operations at the site. Also included is documentation of the structural sufficiency of the tower with proposed modifications to accommodate the revised antenna configuration.

Existing Facility

The Eastford facility is located at 2 Hinckley Hill Road. The address spans part of Preston and Norwich, however, the tower is located in the City of Norwich. The Site coordinates are: N41.5107, W72.0596. The existing facility consists of a 150' Self Supporting Tower. Sprint currently operates wireless communications equipment on a platform on a concrete slab at the facility and has3 antennas and 6 remote radio heads at a centerline of 140' feet on the tower. Please accept this letter as notification to the Council, pursuant to R.C.S.A. Section 16-50j-73, for construction which constitutes an exempt modification pursuant to R.C.S.A. Section 16-50j-72(b)(2).

In compliance with R.C.S.A. Section 16-50j-73, a copy of this letter is being sent to:

Hon. Peter Nystrom – Mayor of Norwich, – Via Fed Ex Deanna Rhodes – City Planner – Via Fed Ex Cordless Data Transfer, Inc., the tower owner – Via Fed Ex Lavern Irwin – Land Owner – Via Fed Ex

Statutory Considerations

The planned modifications to the facility fall within the activities explicitly provided for in R.C.S.A. 16-50j-72(b)(2)

- 1. The height of the overall structure will be unaffected.
- 2. The proposed changes will not require an extension of the property boundaries.
- 3. The proposed additions will not increase the noise level at the existing facility by

six decibels or more, or to levels that exceed state and/or local criteria

- 4. The changes will not increase the calculated "worst case" power density for the combined operations at the site to a level at or above the Federal Communications Commission safety standard.
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Sprint respectfully submits that the proposed changes at the referenced site constitute exempt modifications under R.C.S.A Section §16-50j-72(b)(2).

Respectfully submitted,

<u>Paul F. Sagristano</u>

Paul F. Sagristano Charles Cherundolo Consulting 917-841-0247 psagristano@lrivassoc.com

PFS/mtf

Additional Recipients: Hon. Peter Nystrom – Mayor of Norwich, – Via Fed Ex Deanna Rhodes – City Planner – Via Fed Ex Cordless Data Transfer, Inc., the tower owner – Via Fed Ex

Lavern Irwin – Land Owner – Via Fed Ex



May 7,2018

Dear Customer:

The following is the proof-of-delivery for tracking number **772134902165**.

Delivery Information:

Status: Delivered to: Receptionist/Front Desk

Delivery date:

Signed for by: C.CUPRAK Delivery location: 100 BROADWAY

NORWICH, CT 06360

May 7, 2018 11:00

Service type: FedEx Express Saver

Special Handling: Deliver Weekday

Direct Signature Required



Shipping Information:

Tracking number: 772134902165 **Ship date:** May 2, 2018

Weight: 0.5 lbs/0.2 kg

Recipient:

Hon. Peter Nystrom - Mayor

City of Norwich 100 Broadway Mayor's office

NORWICH, CT 06360 US

Reference

Shipper:

Paul Sagristano

CCC

4 Davis Road West

Suite 5

OLD LYME, CT 06371 US CT23XC114 - CSC to Mayor



May 7,2018

Dear Customer:

The following is the proof-of-delivery for tracking number **772134959750**.

Delivery Information:

Status: Delivered to: Receptionist/Front Desk

Delivery date:

Signed for by: S.SMITH Delivery location: 23 UNION ST

NORWICH, CT 06360

May 7, 2018 11:01

Service type: FedEx Express Saver

Special Handling: Deliver Weekday

Direct Signature Required



Shipper:

CCC

Suite 5

Paul Sagristano

4 Davis Road West

Shipping Information:

Tracking number: 772134959750 **Ship date:** May 2, 2018

Weight: 0.5 lbs/0.2 kg

Recipient:

Deanna Rhodes, City Planner

City of Norwich 23 Union Street

NORWICH, CT 06360 US

OLD LYME, CT 06371 US

Reference CT23XC114 - CSC to Planner



May 9,2018

Dear Customer:

The following is the proof-of-delivery for tracking number **772135064165**.

Delivery Information:

Status: Delivered

Signed for by: L.IRWIN

Service type: FedEx Express Saver

Special Handling: Deliver Weekday

Residential Delivery

Direct Signature Required

Delivered to: Residence

Delivery location: 890 N GRANADA DR

CHANDLER, AZ 85226 May 8, 2018 12:31

Delivery date:

Shipping Information:

 Tracking number:
 772135064165
 Ship date:
 May 2, 2018

 Weight:
 0.5 lbs/0.2 kg

Recipient: Shipper:

Lavern Irwin Paul Sagristano

890 North Granada Drive CCC
CHANDLER, AZ 85226 US 4 Davis Road West

Suite 5

OLD LYME, CT 06371 US

Reference CT23XC114 - CSC to LL



May 7,2018

Dear Customer:

The following is the proof-of-delivery for tracking number 772132696764.

Delivery Information:

Status: Delivered Delivered to: Residence

Signed for by: **Delivery location:** 600 OLD HARTFORD **V.LEGALT**

Delivery date:

ROAD

COLCHESTER, CT 06415

May 7, 2018 14:46

Service type: FedEx Express Saver

Special Handling: **Deliver Weekday**

Residential Delivery

Direct Signature Required

Shipping Information:

Tracking number: Ship date: May 2, 2018 772132696764

Weight: 0.5 lbs/0.2 kg

Recipient:

Shipper: Mark Legault Paul Sagristano CDT, Inc CCC

600 Old Hartford Road 4 Davis Road West

COLCHESTER, CT 06415 US Suite 5

OLD LYME, CT 06371 US CT33XC016 CSC Sub Reference

Google Maps 2 Hinckley Hill Rd



Imagery ©2018 Google, Map data ©2018 Google 100 ft

2B HINCKLEY HILL RD

Location 2B HINCKLEY HILL RD

Mblu 17-0/ HIN1/ 2B/ /

Acct# 00112101

Owner IRWIN JAMES C & LAVERNE G

Assessment \$200

Appraisal \$300

PID 102142

Building Count 1

Current Value

Appraisal							
Valuation Year Improvements Land Total							
2012		\$0	\$300	\$300			
		Assessment					
	Valuation Year Improvements Land Total						
2012		\$0	\$200	\$200			

Owner of Record

Owner

IRWIN JAMES C & LAVERNE G

Co-Owner **Address**

890 N GRANADA DR

CHANDLER, AZ 85226

Sale Price \$0

Certificate Book & Page

Sale Date

Ownership History

Ownership History					
Owner Sale Price Certificate Book & Page Sale Date					
IRWIN JAMES C & LAVERNE G	\$0				

Building Information

Building 1: Section 1

Year Built:

Living Area:

Replacement Cost:

\$0

Building Percent

Good:

Replacement Cost

Less Depreciation: \$0

Building Attributes				
Field	Description			

Style	Vacant Land
Model	
Grade:	
Stories:	
Occupancy	
Exterior Wall 1	
Exterior Wall 2	
Roof Structure:	
Roof Cover	
Interior Wall 1	
Interior Wall 2	
Interior Flr 1	
Interior Flr 2	
Heat Fuel	
Heat Type:	
AC Type:	
Total Bedrooms:	
Total Bthrms:	
Total Half Baths:	
Total Xtra Fixtrs:	
Total Rooms:	
Bath Style:	
Kitchen Style:	

Building Photo



(http://images.vgsi.com/photos/PrestonCTPhotos//default.jpg)

Building Layout

Building Sub-Areas (sq ft)	<u>Legend</u>
No Data for Building Sub-Areas	

Extra Features

Extra Features	<u>Legend</u>
No Data for Extra Features	

Land

Land Use		Land Line Valua	Land Line Valuation	
Use Code	1320	Size (Acres)	0.00	
Description	RES ACLNUD	Frontage		
Zone	R-12	Depth		
Neighborhood	0050	Assessed Value	\$200	
Alt Land Appr	No	Appraised Value	\$300	
Category				

Outbuildings

Outbuildings	<u>Legeno</u>	l
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No Data for Outbuildings

Valuation History

Appraisal				
Valuation Year Improvements Land				
2011	\$0	\$300	\$300	
2006	\$0	\$49,300	\$49,300	

Assessment					
Valuation Year Improvements Land Total					
2011	\$0	\$200	\$200		
2006	\$0	\$34,500	\$34,500		

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RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

SPRINT Existing Facility

Site ID: CT23XC114

Northford / Oshencowski 2 Hinckley Hill Road Preston, CT 06360

October 24, 2017

EBI Project Number: 6217004512

Site Compliance Summary			
Compliance Status:	COMPLIANT		
Site total MPE% of			
FCC general	14.09 %		
population	14.09 /		
allowable limit:			



October 24, 2017

SPRINT Attn: RF Engineering Manager 1 International Boulevard, Suite 800 Mahwah, NJ 07495

Emissions Analysis for Site: CT23XC114 - Northford / Oshencowski

EBI Consulting was directed to analyze the proposed SPRINT facility located at **2 Hinckley Hill Road**, **Preston**, **CT**, for the purpose of determining whether the emissions from the Proposed SPRINT Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter (μ W/cm2). The number of μ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter (μ W/cm²). The general population exposure limits for the 850 MHz Band is approximately 567 μ W/cm². The general population exposure limit for the 1900 MHz (PCS) and 2500 MHz (BRS) bands is 1000 μ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed SPRINT Wireless antenna facility located at **2 Hinckley Hill Road, Preston, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since SPRINT is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 1 CDMA channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.
- 2) 2 LTE channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.
- 3) 5 CDMA channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 16 Watts per Channel.
- 4) 2 LTE channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 5) 8 LTE channels (2500 MHz (BRS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.



- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the RFS APXVSPP18-C-A20, RFS APXV9ERR18-C-A20, KMW ET-X-TU-42-15-37-18-IR-RA and the Commscope DT465B-2XR for transmission in the 850 MHz, 1900 MHz (PCS) and 2500 MHz (BRS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerlines of the proposed antennas are **140 feet** above ground level (AGL) for **Sector A**, **140 feet** above ground level (AGL) for **Sector B** and **140 feet** above ground level (AGL) for Sector C.
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general population threshold limits.



SPRINT Site Inventory and Power Data by Antenna

Sector:	A	Sector:	В	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	KMW ET-X-TU-42-15-37- 18-IR-RA	Make / Model:	RFS APXVSPP18-C-A20	Make / Model:	RFS APXV9ERR18-C- A20
Gain:	12.9 / 15.9 dBd	Gain:	13.4 / 15.9 dBd	Gain:	11.9 / 14.9 dBd
Height (AGL):	140 feet	Height (AGL):	140 feet	Height (AGL):	140 feet
Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)
Channel Count	10	Channel Count	10	Channel Count	10
Total TX Power(W):	220 Watts	Total TX Power(W):	220 Watts	Total TX Power(W):	220 Watts
ERP (W):	7,394.63	ERP (W):	7,537.38	ERP (W):	5,873.76
Antenna A1 MPE%	1.66 %	Antenna B1 MPE%	1.71 %	Antenna C1 MPE%	1.32 %
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	Commscope DT465B-2XR	Make / Model:	Commscope DT465B-2XR	Make / Model:	Commscope DT465B-2XR
Gain:	15.05 dBd	Gain:	15.05 dBd	Gain:	15.05 dBd
Height (AGL):	140 feet	Height (AGL):	140 feet	Height (AGL):	140 feet
Frequency Bands	2500 MHz (BRS)	Frequency Bands	2500 MHz (BRS)	Frequency Bands	2500 MHz (BRS)
Channel Count	8	Channel Count	8	Channel Count	8
Total TX Power(W):	160 Watts	Total TX Power(W):	160 Watts	Total TX Power(W):	160 Watts
ERP (W):	5,118.23	ERP (W):	5,118.23	ERP (W):	5,118.23
Antenna A2 MPE%	1.02 %	Antenna B2 MPE%	1.02 %	Antenna C2 MPE%	1.02 %

Site Composite MPE%					
Carrier	MPE%				
SPRINT – Max per sector	2.73 %				
T-Mobile	2.83 %				
Verizon Wireless	3.41 %				
AT&T	4.74 %				
TSR Paging	0.20 %				
Aquis Paging	0.18 %				
Site Total MPE %:	14.09 %				

SPRINT Sector A Total:	2.68 %
SPRINT Sector B Total:	2.73 %
SPRINT Sector C Total:	2.34 %
Site Total:	14.09 %

SPRINT _ Max Values per Frequency Band / Technology (Sector B)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
Sprint 850 MHz CDMA	1	437.55	140	0.88	850 MHz	567	0.15%
Sprint 850 MHz LTE	2	437.55	140	1.75	850 MHz	567	0.31%
Sprint 1900 MHz (PCS) CDMA	5	622.47	140	6.23	1900 MHz (PCS)	1000	0.62%
Sprint 1900 MHz (PCS) LTE	2	1,556.18	140	6.23	1900 MHz (PCS)	1000	0.62%
Sprint 2500 MHz (BRS) LTE	8	639.78	140	10.25	2500 MHz (BRS)	1000	1.02%
						Total:	2.73%



Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the SPRINT facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

SPRINT Sector	Power Density Value (%)
Sector A:	2.68 %
Sector B:	2.73 %
Sector C:	2.34 %
SPRINT Maximum	2.73 %
Total (Sector B):	2.13 %
Site Total:	14.09 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **14.09** % of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

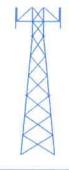
FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



FRED A. NUDD CORPORATION

1743 ROUTE 104, BOX 577 ONTARIO, NY 14519 (315) 524-2531 FAX (315) 524-4249

www.nuddtowers.com



Mark LeGault Cordless Data Transfer, Inc. 600 Old Hartford Road Colchester, CT 06415 January 14, 2017

Fred A. Nudd Job Number: 117-23243.6

Location: 2 Hinkley Hill Road, Norwich, CT 06360, New London County (Lat. & Long: 41-30-53.45, -72-03-42.08)

Subject: Structural Analysis of a 150 ft Self-Supporting Tower

Fred A. Nudd Corporation has completed a three-dimensional, finite element model structural analysis of the above noted self-supporting tower. This tower was analyzed considered appurtenance loads noted in the appurtenance loading table on the following page. The design loading criteria and strength design are per the ANSI/TIA-222-G standard, which is the recommended design standard per the 2012 International Building Code (Sec. 1609 & 3108), , and the 2016 Connecticut State Building Code. Tower and foundation dimensions have been taken from original design drawings by Fred A. Nudd Corporation (Drawing Number 99-6864-1 & 99-6864-2R, dated July 22, 1999 & November 20, 1999). Onsite subsurface conditions were taken from a geotechnical report by Coneco (Project Number C104.0CDT, dated November 15, 1999). The tower is assumed to be in good, undamaged and equivalent to as new condition and has been maintained / inspected per criteria by TIA-222.

The purpose of this analysis is to determine the structure's ability to support new Sprint and AT&T equipment installed at a rad center of 140 ft 115 ft above ground level (AGL), respectively. The new equipment to be installed, which included antennas, coax, mounts and associated hardware are listed on the following page in the appurtenance loading table.

Results of the analysis indicate the tower will be able to the support the design loads noted in the appurtenance loading table on the following page. Specific section design loads, capacities and stress ratios are provided on the following pages. Maximum member usage was found to be 83%. Detailed calculation of the applied forces and member capacities, considering combinations of existing and proposed equipment, are provided in the following pages.

The tower base foundation was analyzed using soil properties from the aforementioned geotechnical report. Based on this analysis, the foundation is capable of supporting the existing and proposed equipment. Detailed calculation of the applied forces and member capacities are provided in the following pages.

In conclusion, the tower superstructure and substructure can support the listed existing and proposed appurtenance loading.

We trust this report satisfies your needs. Please contact us with any questions or concerns regarding this report.

Best Regards,



Fred. A. Nudd Corporation

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Code Design Criteria

ANSI/TIA-222-G

Windspeed = 104 mph, 3-second gust, V_{asd} / 132 mph, 3-second gust, V_{ult}

Exposure = B

Radial Ice = 0.75 inch

Ice Windspeed = 50 mph, 3-second gust

Structure Class = II

Topographic Category = 1

S_s < 1.0, thus seismic loading does not need to be considered

Appurtenance Loading (1) - Currently installed equipment on tower

Height (ft)	Carrier	Appurtenance	Mount	Coax (in)
150	T-Mobile	(3) Ericsson AIR32 B66A/B2A (3) Commscope DBXNH-6565B-A2M (3) Ericsson KRY 112 144/2	(3) 12 ft Boom / Frame	(12) 1-5/8 (1) 1-5/8 Fiber
140	Sprint	(1) KMW ET-X-TU-42-15-37-18-IR-RA (1) RFS APXVSPP18-C-A20 (1) RFS APXV9ERR18-C-A20 (3) Alcatel Lucent 4X45 65 RRU (3) Alcatel Lucent 2X50W RRU (3) Alcatel Lucent 1900 MHz RRH, 65 MHz (3) RFS IBC1900BB-3	(3) 12 ft Boom / Frame	(3) 1-1/4 Hybriflex
127.5	Verizon	(4) RFS APL868013 (6) RFS FD9R6004/2C-3L (2) RFS APL866513 (1) RFS DB-T1-6Z-8AB-0Z (3) Commscope LNX-6514DS-1AM (3) Commscope HBXX-9014DS-VTM (3) Commscope HBXX-6517DS-A2M (3) Alcatel Lucent RH_2x60-AWS (3) Alcatel Lucent RH_2x60-PCS	(3) 12 ft Boom / Frame	(6) 1-5/8 (2) 1-5/8 Fiber Cable
115	AT&T Mobility	(3) Powerwave 7770.00 (6) Powerwave LGP21401 (1) Powerwavce P65-17-XLH-RR (1) KMW AM-X-CD-16-65-00T-RET (6) Ericsson RRUS11 (1) Raycap DC6-48-60-18-8F	(3) 10 ft Boom / Frame	(12) 1-1/4 (2) 0.65 DC (1) 1.34 Fiber

Height measurement taken as distance from top of base foundation to center of appurtenance.

Appurtenance Loading (2) - New Sprint equipment to replace all Sprint equipment noted above

Height (ft)	Carrier	Appurtenance	Mount	Coax (in)
140	Sprint	(3) RFS APXV9ERR18-C-A20 (6) Alcatel Lucent 4x45W, 1900 MHz (3) Alcatel Lucent TD-RRH8x200-25 (6) Alcatel Lucent RRH 2x50, 800 Mhz (3) Commscope DT465B-2XR	(3) 12 ft Boom / Frame	(4) 1-1/4 Hybrid

- Height measurement taken as distance from top of base foundation to center of appurtenance.
- Additional coax to be installed anywhere on the tower.

Appurtenance Loading (3) - New AT&T equipment to replace all AT&T equipment noted above

Height (ft)	Carrier	Appurtenance	Mount	Coax (in)
115	AT&T	(3) Powerwave 7770 (3) KMW EPBQ-654L8H8-L2 (3) CCI OPA-65R-LCUU-H8 (3) Ericsson RRUS-11 (9) Ericsson RRUS-32 (3) Ericsson B14 4478 (6) Powerwave LGP21401 (2) Raycap DC6-48-60-18-8F (1) Raycap DC6-48-60-0-8F	(3) 12 ft Boom / Frame	(12) 1-1/4 (6) 3/4 DC (2) 3/8 Fiber

- Height measurement taken as distance from top of base foundation to center of appurtenance.
- Additional coax to be installed anywhere on the tower.

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Maximum Member Usage

Member	1	1+2	1+3	1+2+3
Leg	64	68	67	73
Diagonal	79	80	81	83
Horizontal	2	2	2	2
Splice/Connection Bolts	79	80	81	83

- Percentage equal to or less than 100% denote member stress levels are satisfactory for loading.
- Percentage greater than 100% indicates member strengthening is required.

Foundation Reaction Usage

Reaction	1	1+2	1+3	1+2+3
Compression / Leg	58	60	60	63
Uplift / Leg	46	48	48	51
Shear / Leg	36	37	37	38

- Percentage equal to or less than 100% denote member stress levels are satisfactory for loading.
- Percentage greater than 100% indicates member strengthening is required.

. 2			

Section	Legs	Leg Grade	Diagonals	Diagonal Grade	Top Girts	Face Width (ft) 18	# Panels @ (ft)	Weight (Ib) 16284 4	0.0 ft	
Ta	PB		L3 1/2×			10	4 @ 9	3948.6		
11	P8x5		L3 1/2x3 1/2x1/4			10.	4 @ 9 33333	3829 5	20.0 ft	
16 1	P8x,322					41		6 0892	40.0 ft	
TS	P6x 28	A500M-54	L3x3x3/16	A36	Y Z	- to	9@625	1986.3	60.0 R	
T4	P5x 258	54	L2 1/2x2 1/2x3/16			10		14850	80 O R	
t	P4x 237		L2x				8	1140 7	100.0 ft	
12	P2 5x 203		L2x2x3/16				8 @ 4.66667	791.8	120 O ft	
F			A		L3x3x1/4		2 @ 4 33333	4215	140 0 ft	

DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Nudd 12' boom	150	Nudd 12' boom	127.5
Nudd 12' boom	150	(2) RFS APL868013 (Verizon)	127.5
Nudd 12 boom	150	(2) RFS APL868013 (Verizon)	127.5
Ericsson AIR 32 B66AA/B2A	150	(2) RFS APL868513 (Verizon)	127.5
(T-Mobile)		(2) RFS FD9R6004/2C-3L (Verizon)	127.5
Ericsson AIR 32 B66AA/B2A	150	(2) RFS FD9R6004/2C-3L (Verizon)	127.5
(T-Mobile)		(2) RFS FD9R6004/2C-3L (Verizon)	127.5
Ericsson AIR 32 B66AA/B2A (T-Mobile)	150	RFS DB-T1-6Z-8AB-0Z (Verizon)	127.5 - 12.75
Commscope DBXHN-65659-A2M (T-Mobile)	150	Commscope LNX-6514DS-VTM (Verizon)	127,5
Commscope DBXHN-6565B-A2M (T-Mobile)	150	Commscope LNX-6514DS-VTM (Verizon)	127.5
Commscope DBXHN-6565B-A2M (T-Mobile)	150	Commscope LNX-6514DS-VTM (Verizon)	127 5
(2) Ericsson KRY112/2 (T-Mobile)	150	Commscope HBXX-9014DS-VTM	127.5
(2) Ericsson KRY112/2 (T-Mobile)	150	(Verizon)	
(2) Ericsson KRY112/2 (T-Mobile)	150	(Verizon)	127.5
Alcatel Lucent 1900 MHz RRH (Sprint)	140	Commscope HBXX-9014DS-VTM	127.5
Alcatel Lucent 1900 MHz RRH (Sprint)	140	(Verizon)	1210
Alcatel Lucent 1900 MHz RRH (Sprint)	140	Commiscope HBXX-8517DS-A2M	127.5
RFS IBC1900BB-3 (Sprint)	140	(Verizon)	
RFS IBC1900BB-3 (Sprint)	140	Commscope HBXX-8517DS-A2M	127.5
RFS IBC1900BB-3 (Sprint)	140	(Verizon)	
Nudd 12' boom (Sprint)	140	Commscope HBXX-6517DS-A2M	127,5
Nudd 12' boom (Sprint)	140	(Verizon)	107.5
Nudd 12' boom (Sprint)	140	Alcatel Lucent RH_2x60-AWS (Verizon)	127.5
Commscope DT465B-2XR (Sprint)	140	Alcatel Lucent RH 2x60-AWS	127 5
Commscope DT465B-2XR (Sprint)	140	(Verizon)	121 0
Commscope DT465B-2XR (Sprint)	140	Nudd 10' boom	115
(2) Alcatel Lucent 4x45 (Sprint)	140	Powerwave 7770.00 (ATI)	115
(2) Alcatel Lucent 4x45 (Sprint)	140	Powerwave 7770.00 (ATI)	115
(2) Alcatel Lucent 4x45 (Sprint)	140	Powerwave 7770.00 (ATI)	115
Alcatel Lucent 8x200-25 (Sprint)	140	KMW EPBQ-654L8H8-L2 (ATI)	115
Alcatel Lucent 8x200-25 (Sprint)	140	KMW EPBQ-654L8H8-L2 (ATI)	115
Alcatel Lucent 8x200-25 (Sprint)	140	KMW EPBQ-654L8H8-L2 (ATI)	115
RFS APXV9ERR18-C-A20 (Sprint)	140	CCI OPA-65R-LCUU-H8 (ATI)	115
RFS APXV9ERR18-C-A20 (Sprint)	140	CCI OPA-65R-LCUU-H8 (ATI)	115
RFS APXV9ERR18-C-A20 (Sprint)	140	CCI OPA-65R-LCUU-H8 (ATI)	115
(2) Alcatel Lucent RRH2x50 (Sprint)	140	Ericsson RRUS11 (ATI)	115
(2) Alcatel Lucent RRH2x50 (Sprint)	140	Ericsson RRUS11 (ATI)	115
(2) Alcatel Lucent RRH2x50 (Sprint)	140	Ericsson RRUS11 (ATI)	115
Alcatel Lucent RH 2x60-AWS	127.5	(3) Ericsson RRUS32 (ATI)	115
(Verizon)		(3) Ericsson RRUS32 (ATI)	115
Alcalel Lucent RH_2x60-70OU	127.5	(3) Ericsson RRUS32 (ATI)	115
(Verizon)		Ericsson B14 4478 (ATI)	115
Alcatel Lucent RH_2x60-70OU	127.5	Ericsson B14 4478 (ATI)	115
(Verizon)	407.5	Ericsson B14 4478 (ATI)	115
Alcatel Lucent RH_2x60-70OU (Verizon)	127.5	(2) Powerwave LGP21401 (ATI)	115
	127.5	(2) Powerwave LGP21401 (ATI)	115
Alcatel Lucent RH_2x60-PCS (Verizon)	121.0	(2) Powerwave LGP21401 (ATI)	115
Alcatel Lucent RH_2x60-PCS	127.5	Nudd 10' boom	115
(Verizon)		Nudd 10' boom	115
Alcatel Lucent RH_2x60-PCS	127.5	Raycap DC6-48-60 (ATI)	115
(Verizon)		Raycap DC6-48-60 (ATI)	115
	127.5		1

SYMBOL LIST

SHEAR	MARK	SIZE	MARK	SIZE	
13086 lb	Α	L1 1/2x1 1/2x3/16			

 TOI
 MATERIAL STRENGTH

 50 mph
 GRADE
 Fy
 Fu
 GRADE
 Fy
 Fu

 A500M-54
 54 ksi
 70 ksi
 A36
 36 ksi
 58 ksi

SHEAR 45008 lb__{__

> TORQUE 14155 lb-ft REACTIONS - 104 mph WIND

	^{lob:} 150' SS Tower Norwich, CT. Analysis							
	Project: 117-23243.6							
	Client: CDT	Drawn by: FAN	App'd:					
		Date: 01/14/18	Scale: NTS					
Phone: FAX:	Path:	Dwg No E-1						

RISATower	Job	Page 1 of 34	
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 150.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 6,00 ft at the top and 18,00 ft at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in New London County, Connecticut.

Basic wind speed of 104 mph.

Structure Class II.

Exposure Category B.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Weld together tower sections have flange connections..

Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications..

Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards...

Welds are fabricated with ER-70S-6 electrodes..

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- √ Use Code Stress Ratios
- ✓ Use Code Safety Factors Guys Escalate Ice
 Always Use Max Kz
 Use Special Wind Profile
- √ Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC 6D+W Combination

Distribute Leg Loads As Uniform Assume Legs Pinned

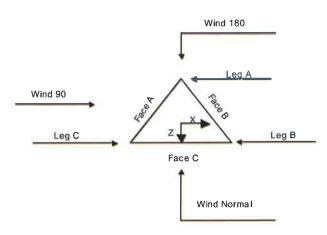
- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
- ✓ Use Clear Spans For KL/r Retension Guys To Initial Tension Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt. Autocalc Torque Arm Areas SR Members Have Cut Ends Sort Capacity Reports By Component Triangulate Diamond Inner Bracing

Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

√ Consider Feedline Torque Include Angle Block Shear Check Poles

Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets

RISATower	Job 150' SS Tower Norwich, CT. Analysis	Page 2 of 34
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Triangular Tower

Tower	Section	Geometr	٧
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Tower	Tower	Assembly	Description	Section	Number	Section
Section	Elevation	Database		Width	of	Length
					Sections	
	ft			fi		ft
T1	150.00-140.00			6.00	1	10.00
T2	140,00-120,00			6,00	. 1	20.00
Т3	120.00-100.00			6.00	1	20.00
T4	100.00-80.00			8.00	1	20.00
T5	80.00-60.00			10.00	1	20.00
T6	60.00-40.00			12.00	1	20.00
T7	40,00-20,00			14.00	1	20.00
Т8	20.00-0.00			16.00	1	20.00

Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Gir
Section	Elevation	Spacing	Type	K Brace	Horizontals	Offset	Offset
				End			
	ft	st		Panels		in	in
T1	150.00-140.00	4.33	X Brace	No	No	8.0000	8.0000
T2	140.00-120.00	4.67	X Brace	No	No	8.0000	8.0000
T3	120.00-100.00	4.67	X Brace	No	No	8.0000	8.0000
T4	100.00-80.00	6.25	X Brace	No	No	7.5000	7.5000
T5	80.00-60.00	6.25	X Brace	No	No	7.5000	7.5000
T6	60.00-40.00	6.25	X Brace	No	No	7.5000	7.5000
T7	40.00-20.00	9.33	X Brace	No	No	8.0000	8.0000

RISATower	Job	150' SS Tower Norwich, CT. Analysis	Page 3 of 34
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Girt
Section	Elevation	Spacing	Type	K Brace	Horizontals	Offset	Offset
				End			
	ft	fl		Panels		in	in
T8	20.00-0.00	9.33	X Brace	No	No	8.0000	8,0000

Tower	Leg	Leg	Leg	Diagonal	Diagonal	Diagonal
Elevation fl	Туре	Size	Grade	Туре	Size	Grade
1 150,00-140.00	Pipe	P2.5x.203	A500M-54 (54 kai)	Equal Angle	L1 1/2x1 1/2x3/16	A36 (36 ksi)
2 140,00-120.00	Pipe	P2.5x.203	A500M-54 (54 ksi)	Equal Angle	L2x2x3/16	A36 (36 ksi)
73 120,00-100,00	Pipe	P4x.237	A500M-54 (54 ksi)	Equal Angle	L2x2x3/16	A36 (36 ksi)
T4 100.00-80,00	Pipe	P5x.258	A500M-54 (54 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A36 (36 ksi)
T5 80.00-60.00	Pipe	P6x.28	A500M-54 (54 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T6 60.00-40.00	Pipe	P8x.322	A500M-54 (54 ksi)	Equal Angle	L3x3x3/16	A36 (36 ksi)
T7 40.00-20.00	Pipe	P8x.5	A500M-54 (54 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)
T8 20.00-0.00	Pipe	P8x.5	A500M-54 (54 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A36 (36 ksi)

Tower	Secondary	Secondary Horizontal	Secondary	Inner Bracing	Inner Bracing Size	Inner Bracing
Elevation	Horizontal Type	Size	Horizontal	Туре		Grade
			Grade			
ft						
T1 150.00-140.00	Solid Round		A572-50	Solid Round	9/16	A572-50
			(50 ksi)			(50 ksi)
T2 140,00-120,00	Solid Round		A572-50	Solid Round	9/16	A572-50
			(50 ksi)			(50 ksi)
T3 120,00-100.00	Solid Round		A572-50	Solid Round	9/16	A572-50
			(50 ksi)			(50 ksi)
T4 100.00-80.00	Solid Round		A572-50	Solid Round	9/16	A572-50
			(50 ksi)			(50 ksi)
T5 80.00-60.00	Solid Round		A572-50	Solid Round	9/16	A572-50
			(50 ksi)			(50 ksi)
T6 60.00-40.00	Solid Round		A572-50	Solid Round	9/16	A572-50
			(50 ksi)			(50 ksi)
T7 40.00-20.00	Solid Round		A572-50	Solid Round	9/16	A572-50
			(50 ksi)			(50 ksi)
T8 20.00-0.00	Solid Round		A572-50	Solid Round	9/16	A572-50
			(50 ksi)			(50 ksi)

Tower Section Geometry (cont'd)

RISATower	Јов 150' SS Tower Norwich, CT. Analys	is Page 4 of 34
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Tower	Gusset	Gusset	Gusset Grade	Adjust. Factor	Adjust	Weight Mult.	Double Angle	Double Angle
Elevation	Area	Thickness		A_f	Factor		Stitch Bolt	Stitch Bolt
	(per face)				A_r		Spacing	Spacing
							Diagonals	Horizontals
ft	ft ²	in					in	in
Tl	0.00	0.0000	A36	1	ı	1	36,0000	36,0000
130.00-140.00			(36 ksi)					
T2	0.00	0.0000	A36	1	1	1	36,0000	36.0000
140.00-120.00			(36 ksi)					
T3	0.00	0.0000	A36	1	1	3	36,0000	36,0000
120.00-100.00			(36 ksi)					
T4	0,00	0.0000	A36	1	1	1	36.0000	36.0000
100.00-80.00			(36 ksi)					
T\$ 80,00-60,00	0,00	0,0000	A36	1	ŧ	1	36.0000	36,0000
			(36 ksi)					
T6 60,00-40,00	0.00	0.0000	A36	1	1	1	36,0000	36,0000
			(36 ksi)					
T7 40.00-20.00	0.00	0.0000	A36	1	1	1	36,0000	36,0000
			(36 ksi)					
T8 20.00-0.00	00,0	0.0000	A36	1	10	1	36.0000	36,0000
			(36 ksi)					

			K Factors ¹							
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
	Angles	Rounds		X	X	X	X	X	X	X
fi				Y	Y	Y	Y	Y	Y	<u> </u>
Tl	Yes	No	1	1.	1	1	1	1	1	1
150,00-140,00				1	1	1	1	1	1	1
T2	Yes	No	1	1	1	1	1	1	1	1
140,00-120.00				1	1	1	1	1	1	1
T3	Yes	No	1	1	1	1	1	1	1	1
120.00-100.00				1	1	1	1	1	1	1
T4	Yes	No	1	1	1	I	1	1	1	1
100.00-80.00				1	1	1	1	1	1	1
T5	Yes	No	1	1	1	1	1	1	1	1
80,00-60.00				1	1	1	1	1	1	1
Т6	Yes	No	1	1	1	1	1	1	1	1
60,00-40.00				1	1	1	1	1	1	1
T7	Yes	No	1	1	1	1	1	1	1	1
40,00-20.00				1	1	1	1	1	1	1
T8 20.00-0.00	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1

Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

Tower Section Geometry (cont'd)

RISATower	lob 150' SS Tow	er Norwich, CT. Analysis	Page 5 of 34
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Tower Elevation fi	Leg		Diago	nal	Top G	irt	Botton	ı Girt	Mid	Girt	Long Ho	rizontal	Short Ho	rizontal
<i>J</i> .	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
Ti	0.0000	1	0.0000	0.73	0,0000	0.73	0.0000	0.75	0,0000	0.75	0.0000	0.75	0.0000	0.73
130.00-140.00														
T2	0.0000	- 1	0.0000	0.75	0.0000	0.73	0.0000	0.73	0.0000	0.73	0.0000	0.75	0.0000	0.75
140.00-120.00														
Т3	0,0000	1	0.0000	0.75	0,0000	0.75	0.0000	0.75	0,0000	0.75	0,0000	0.75	0.0000	0.75
120,00-100,00														
T4	0.0000	-1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.73	0.0000	0.75
100.00-80.00														
T5 80.00-60.00	0.0000	1	0.0000	0.75	0.0000	0.75	0,0000	0.75	0,0000	0,75	0.0000	0.75	0.0000	0.75
T6 60.00-40.00	0.0000	1	0.0000	0.75	0,0000	0.75	0.0000	0.75	0,0000	0.75	0,0000	0.75	0,0000	0.75
T7 40.00-20.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0,0000	0.75
T8 20.00-0.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0,0000	0.75	0.0000	0,75	0.0000	0.75

Tower Elevation	Leg Connection	Leg		Diago	nal	Top G	irt	Bottom	Girt	Mid G	d Girt Long Horizontal		Short Hor	izontal	
ft	Туре	Bolt Size in	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.
T1 150.00-140.00	Flange	0.7500 A325N	4	0.5000 A325N	1	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T2 140.00-120.00	Flange	1,0000 A325N	4	0,6250 A325N	1	0.6250 A325N	0	0.6250 A325N	0	0,6250 A325N	0	0,6250 A325N	0	0.6250 A325N	0
T3 120.00-100.00	Flange	1.0000 A325N	6	0.6250 A325N	1	0.6250 A325N	0	0,6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0,6250 A325N	0
T4 100.00-80.00	Flange	1,0000 A325N	8	0,6250 A325N	1	0.6250 A325N	0	0,6250 A325N	0	0.6250 A325N	0	0,6250 A325N	0	0.6250 A325N	0
T5 80.00-60.00	Flange	1.2500 A325N	8	0,6250 A325N	1	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T6 60.00-40.00	Flange	1,2500 A325N	8	0.6250 A325N	1	0.6250 A325N	0	0.6250 A325N	0	0,6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T7 40.00-20.00	Flange	1,2500 A325N	8	0.7500 A325N	1	0.6250 A325N	0	0.6250 A325N	0	0 6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0
T8 20.00-0.00	Flange	1.5000 F1554-36	8	0.7500 A325N	1	0.6250 A325N	0	0.6250 A325N	0 =	0.6250 A325N	0	0.6250 A325N	0	0.6250 A325N	0

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face	Allow	Component	Placement	Face	Lateral	#	#	Clear	Width or	Perimeter	Weight	_
-	or	Shield	Type		Offset	Offset		Per	Spacing	Diameter			
	Leg			ft	in	(Frac FW)		Row	in	in	in	plf	
LDF7-50A	С	No	Ar (CaAa)	150.00 - 5.00	0.0000	0.25	12	9	1.9800	1.9800		0.82	
(1-5/8 FOAM)			,										
(T-Mobile)													
1 1/4	Α	No	Ar (CaAa)	140.00 - 5.00	0.0000	0.25	4	4	1.5500	1.5500		0.66	
(Sprint)													
LDF7-50A	В	No	Ar (CaAa)	127.50 - 5.00	0.0000	0.25	6	6	1.9800	1.9800		0.82	

RISATower	Job	150' SS Tower Norwich, CT. Analysis	Page 6 of 34
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Description	Face or	Allow Shield	Component Type	Placement	Face Offset	Lateral Offset	#	# Per	Clear Spacing	Width or Diameter	Perimeter	Weight
	Leg	Silicia	1,460	ſì	in	(Frac FW)		Row	in	in	in	plf
(1-3/8 FOAM)												
(Verizon)												
LDF6-30A	C	No	Ar (CaAa)	113.00 - 3.00	0.0000	0.25	12	6	1.3300	1.3300		0.66
1-1/4 FOAM)												
(AT&T)												
Safety Line	C	No	Ar (CaAa)	130.00 = 0.00	0,0000	0	1	- 1	0.3750	0.3730		0.22
3/8												
Feedline	C	No	Ar (CaAa)	150 00 - 0 00	0.0000	0.25	1	1	3,0000	3,0000		8.40
Ladder (Af)												
Feedline	C	No	Ar (CaAa)	115.00 - 0.00	0.0000	0.25	1	1	3,0000	3.0000		8.40
Ladder (Af)												
Feedline	В	No	Ar (CaAa)	127.50 - 0.00	0,0000	0,25	- 1	1	3,0000	3.0000		8.40
Ladder (Af)												
Feedline	Α	No	Ar (CaAa)	140.00 - 0.00	0.0000	0.25	1	1	3,0000	3.0000		8.40
Ladder (Af)												
DC T	C	No	Ar (CaAa)	115.00 - 5.00	0.0000	0.25	6	6	0.7500	0.0000		0.15
(AT&T)									0.6300			
Fiber	C	No	Ar (CaAa)	115.00 - 5.00	0.0000	0.25	2	2	0.3750	0,0000		0.15
(AT&T)			, ,						0.6300			
3" Rigid	C	No	Ar (CaAa)	115,00 - 0,00	0.0000	0.25	-1	- 1	2,0000	3.0000		2.80
Conduit			, ,									
(AT&T)												
LDF7-50A	В	No	Ar (CaAa)	127.50 - 5.00	0,0000	0.25	2	2	1.9800	1.9800		0.82
1-5/8 FOAM)												
(Verizon)												
LDF7-50A	C	No	Ar (CaAa)	150.00 - 5.00	0.0000	0.25	1	1	1.9800	1.9800		0.82
1-5/8 FOAM)			,									
(T-Mobile)												

Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation			_	In Face	Out Face	
	ft		ft²	ft²	ft^2	ft ²	lb
Tl	150.00-140.00	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0,00
		C	0.000	0.000	29.115	0.000	192.80
T2	140,00-120.00	Α	0.000	0.000	18.400	0.000	220.80
		В	0.000	0.000	14.130	0.000	112.20
		C	0.000	0.000	58.230	0.000	385.60
T3	120,00-100,00	Α	0.000	0.000	18.400	0.000	220.80
		В	0.000	0.000	37.680	0.000	299.20
		C	0.000	0.000	95.130	0.000	690.40
T4	100.00-80.00	Α	0.000	0.000	18.400	0.000	220.80
		В	0.000	0.000	37.680	0.000	299.20
		C	0.000	0.000	107.430	0.000	792.00
T5	80.00-60.00	Α	0.000	0.000	18.400	0.000	220.80
		В	0.000	0.000	37,680	0.000	299.20
		C	0.000	0.000	107,430	0.000	792.00
T6	60.00-40.00	Α	0.000	0.000	18,400	0.000	220.80
		В	0.000	0.000	37,680	0.000	299.20
		C	0.000	0.000	107.430	0.000	792.00
T7	40.00-20.00	A	0.000	0.000	18.400	0.000	220.80
		В	0.000	0.000	37.680	0.000	299.20
		C	0.000	0.000	107.430	0.000	792.00
T8	20.00-0.00	Ā	0,000	0.000	15.300	0.000	207.60

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Tower	Tower	Face	A_R	A_F	C_AA_A	C_AA_A	Weight
Section	Elevation				In Face	Out Face	
	ſŧ		ft²	ft²	ft²	ft²	lb
		B	0.000	0.000	29.760	0.000	266.40
		C	0.000	0.000	85.260	0.000	693.10

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A_R	A_F	C_AA_A	C_AA_A	Weight
Section	Elevation	or	Thickness			In Face	Out Face	
	fl	Leg	in	ft²	ft²	ft²	ft²	lb
TI	150,00-140,00	Α	1,739	0.000	0.000	0.000	0.000	0,00
		B		0,000	0.000	0.000	0,000	0.00
		C		0.000	0,000	63,684	0.000	1237.10
T2	140,00-120.00	Α	1.720	0.000	0,000	50.725	0.000	873.69
		В		0,000	0.000	39.172	0,000	625.06
		C		0,000	0.000	127,026	0,000	2454.14
T3	120,00-100,00	Α	1.692	0,000	0,000	50,429	0.000	861.65
		В		0,000	0,000	103.982	0,000	1644.32
		C		0.000	0.000	211.118	0,000	4001.13
T4	100,00-80,00	Α	1.658	0.000	0.000	50.081	0.000	847.54
		В		0.000	0.000	103.419	0.000	1617.93
		С		0.000	0.000	237,766	0,000	4459.64
T5	80.00-60.00	Α	1.617	0.000	0.000	49,655	0,000	830.43
		В		0,000	0.000	102,730	0,000	1585.83
		С		0.000	0.000	235.862	0.000	4378.02
T6	60.00-40.00	Α	1.564	0.000	0.000	49.102	0.000	808.42
		В		0.000	0.000	101,835	0.000	1544.48
		С		0.000	0.000	233,390	0.000	4273.01
T7	40.00-20.00	Α	1.486	0.000	0.000	48,299	0.000	776.89
		В		0.000	0.000	100,535	0.000	1485.05
		С		0.000	0.000	229.794	0,000	4122.43
T8	20.00-0.00	Α	1.331	0.000	0.000	37.863	0.000	614.27
		В		0.000	0.000	76.302	0.000	1104,50
		C		0.000	0.000	177,014	0.000	3092.14

Feed Line Center of Pressure

Section	Elevation	CP_X	CP_z	CP_X	CP_Z
				Ice	Ice
	ft	in	in	in	in
Tl	150.00-140.00	-3.1280	4.1252	-2.4718	3,4222
T2	140.00-120.00	-1.7879	2.0517	-1.3555	1,6185
T3	120.00-100.00	-1.3601	3.0181	-0.8769	2.2458
T4	100,00-80.00	-1.9508	3.8862	-1.3039	2.9444
T5	80.00-60.00	-2.2696	4.4518	-1.5525	3.4610
T6	60.00-40.00	-2.5506	4.9493	-1.7863	3.9502
T7	40.00-20.00	-2.9597	5.6974	-2.0969	4.6168
T8	20.00-0.00	-3.0799	5.7606	-2.2929	4.9116

Shielding Factor Ka

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Tower	Feed Line	Description	Feed Line	K_a	Ka
Section	Record No.	Desc. ipilon	Segment Elev.	No Ice	Ice
TI	1	LDF7-50A (1-5/8 FOAM)	140.00 -	0.6000	0.6000
TI	5	Safety Line 3/8	150.00 140.00 - 150.00	0.6000	0.6000
Tì	6	Feedline Ladder (Af)	140,00 -	0.6000	0.6000
TI	14	LDF7-50A (1-5/8 FOAM)	140.00 - 150.00	0.6000	0.6000
T2	1	LDF7-50A (1-5/8 FOAM)	120.00 - 140.00	0.6000	0.6000
T2	2	1 1/4	120,00 - 140,00	0.6000	0.6000
T2	3	LDF7-50A (1-5/8 FOAM)	120,00 - 127.50	0.6000	0.6000
T2	5	Safety Line 3/8	120,00 - 140.00	0.6000	0.6000
Т2	6	Feedline Ladder (Af)	120,00 - 140,00	0,6000	0.6000
T2	8	Feedline Ladder (Af)	120,00 - 127,50	0.6000	0.6000
T2	9	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
T2	13	LDF7-50A (1-5/8 FOAM)	120,00 - 127.50	0.6000	0.6000
Т2	14	LDF7-50A (1-5/8 FOAM)	120.00 - 140.00	0.6000	0.6000
Т3	1	LDF7-50A (1-5/8 FOAM)	100.00 - 120.00	0.6000	0.6000
Т3	2	1 1/4	100.00 - 120.00	0.6000	0.6000
Т3	3	LDF7-50A (1-5/8 FOAM)	100.00 - 120.00	0,6000	0.6000
Т3	4	LDF6-50A (1-1/4 FOAM)	100.00 - 115.00	0.6000	0.6000
Т3	5	Safety Line 3/8	100.00 - 120.00	0,6000	0.6000
Т3	6	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
Т3	7	Feedline Ladder (Af)	100.00 - 115.00	0,6000	0.6000
Т3	8	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
Т3	9	Feedline Ladder (Af)	100.00 - 120.00	0,6000	0.6000
T3	10	DC	100.00 - 115.00	0.6000	0.6000
T3	11	Fiber	100.00 - 115.00	0.6000	0.6000
Т3	12	3" Rigid Conduit	100.00 - 115.00	0.6000	0.6000
T3	13	LDF7-50A (1-5/8 FOAM)	100.00 - 120.00	0.6000	0.6000
T3	14	LDF7-50A (1-5/8 FOAM)	100.00 -	0.6000	0.6000
T4 T4	1	LDF7-50A (1-5/8 FOAM)	80.00 = 100.00 80.00 = 100.00	0.6000	0.6000
T4	2 3	LDF7-50A (1-5/8 FOAM)		0.6000	0.6000
T4	4	LDF6-50A (1-1/4 FOAM)	80.00 - 100.00	0.6000	0.6000
T4	5	Safety Line 3/8	80.00 - 100.00	0.6000	0.6000
T4	5	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T4	7	Feedline Ladder (Af)		0.6000	0.6000

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Tower	Feed Line	Description	Feed Line	K_a	K_a
Section	Record No.		Segment Elev.	No Ice	Ice
T4	8	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T4	. 9	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T4	10		80.00 - 100.00	0.6000	0.6000
T4	11		80.00 - 100.00	0.6000	0.6000
T4	12	3" Rigid Conduit		0.6000	0.6000
T4	13	LDF7-30A (1-3/8 FOAM)	11 TO	0.6000	0.6000
T4 T5	14	LDF7-30A (1-3/8 FOAM)		0.6000	0.6000
T5	1	LDF7-30A (1-3/8 FOAM)	60.00 - 80.00 60.00 - 80.00	0.6000	0.6000
T5	2 3	1 1/4 LDF7-50A (1-5/8 FOAM)	5/20/00/00	0.6000	0.6000
T5	4	LDF6-30A (1-1/4 FOAM)	60.00 - 80.00	0.6000	0.6000
T5	5	Safety Line 3/8	60.00 - 80.00	0.6000	0.6000
T5	6	Feedline Ladder (A1)	60.00 - 80.00	0.6000	0.6000
T5	7	Feedline Ladder (Af)	60.00 - 80.00	0,6000	0.6000
T5	8	Feedline Ladder (Af)	60.00 - 80.00	0,6000	0.6000
T5	9	Feedline Ladder (Af)	60,00 - 80,00	0,6000	0.6000
T5	10	DC	60.00 - 80.00	0.6000	0.6000
T5	11	Fiber	60.00 - 80.00	0.6000	0.6000
TS	12	3" Rigid Conduit	60.00 - 80.00	0.6000	0.6000
T5	13	LDF7-50A (1-5/8 FOAM)	60.00 - 80.00	0.6000	0.6000
T5	14	LDF7-50A (1-5/8 FOAM)	60,00 - 80,00	0.6000	0.6000
T6	i l	LDF7-50A (1-5/8 FOAM)	40.00 - 60.00	0.6000	0.6000
T6	2 3	1 1/4 1 DET 50A (1 5/0 EOAM)	40.00 - 60.00	0.6000	0.6000
T6 T6	4	LDF7-50A (1-5/8 FOAM) LDF6-50A (1-1/4 FOAM)	40,00 - 60,00 40,00 - 60,00	0.6000	0.6000
T6	5	Safety Line 3/8	40.00 - 60.00	0.6000	0.6000
T6	6	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T6	7	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T6	8	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
Т6	9	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
Т6	10	, DC	40.00 - 60.00	0,6000	0.6000
Т6	11	Fiber	40.00 - 60.00	0.6000	0.6000
Т6	12	3" Rigid Conduit	40.00 - 60.00	0.6000	0.6000
T6	1.3	LDF7-50A (1-5/8 FOAM)	40.00 - 60.00	0,6000	0.6000
T6	14	LDF7-50A (1-5/8 FOAM)	40,00 - 60.00	0,6000	0,6000
T7	1	LDF7-50A (1-5/8 FOAM)	20.00 - 40.00	0.6000	0.6000
T7 T7	2 3	1 1/4 1 DE7 504 (1 5/9 EOAM)	20.00 - 40.00	0.6000	0.6000
T7	4	LDF7-50A (1-5/8 FOAM) LDF6-50A (1-1/4 FOAM)	20.00 - 40.00 20.00 - 40.00	0.6000	0.6000
T7	5	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000
T7	6	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T7	7	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T7	8	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T7	9	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T7	10	DC	20.00 - 40.00	0.6000	0.6000
T7	11	Fiber	20.00 - 40.00	0.6000	0,6000
T7	12	3" Rigid Conduit	20.00 - 40.00	0.6000	0,6000
T7	13	LDF7-50A (1-5/8 FOAM)	20.00 - 40.00	0.6000	0.6000
T7	14	LDF /-50A (1-5/8 FOAM)	20.00 - 40.00	0.6000	0.6000
T8	1	LDF7-50A (1-5/8 FOAM)	5.00 - 20.00	0.6000	0.6000
T8 T8	2	1 1/4 LDF7-50A (1-5/8 FOAM)	5.00 - 20.00 5.00 - 20.00	0.6000	0.6000
T8	4	LDF6-50A (1-3/8 FOAM)	5.00 - 20.00	0.6000	0.6000
T8	5	Safety Line 3/8	0.00 - 20.00	0.6000	0.6000
T8	6	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T8	7	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T8	8	Feedline Ladder (Af)	0.00 - 20.00	0,6000	0.6000
T8	9	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T8	10	DC	5.00 - 20.00	0.6000	0.6000
Т8	11	Fiber	5.00 - 20.00	0.6000	0.6000
T8	12	3" Rigid Conduit	0.00 - 20.00	0.6000	0.6000
T8	13	LDF7-50A (1-5/8 FOAM)	5.00 - 20.00	0.6000	0.6000

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Tower	Feed Line	Description	Feed Line	Ka	K_a
Section	Record No.		Segment Elev.	No Ice	Ice
Ta	14	LDF7-30A (1-5/8 FOAM)	5.00 - 20.00	0.6000	0,6000

		Discrete Tower Loads								
Description	Face	Offset	Offsets:	Azimuth	Placement		C_AA_A	C_AA_A	Weight	
Description	or	Туре	Horz	Adjustment	Tuccment		Front	Side		
	Leg	1)pc	Lateral	114/11011110111						
	208		Vert							
			ft	0	fi		ft^2	ft²	lb	
			fi		J -		,	,		
			fî							
Nudd 12' boom	С	From Leg	1.00	0.0000	150.00	No Ice	17.10	9.30	254.00	
11844 14 000111	•		0.00	50,000,000		1/2" Ice	21.40	21.40	376.00	
			0,00			1" Ice	26,00	26.00	534,00	
Nudd 12' boom	Α	From Leg	1,00	0,0000	150.00	No Ice	17.10	9.30	254.00	
71000 10 000111			0.00			1/2" Ice	21.40	21.40	376,00	
			0,00			1" Ice	26.00	26.00	534.00	
Nudd 12' boom	В	From Leg	1.00	0.0000	150.00	No Ice	17.10	9.30	254.00	
			0.00			1/2" Ice	21.40	21.40	376.00	
			0.00			1" Ice	26,00	26.00	534.00	
Nudd 12 ^t boom	С	From Leg	1.00	0.0000	127.50	No Ice	17.10	9.30	254.00	
	-		0.00			1/2" Ice	21.40	21.40	376,00	
			0.00			1" Ice	26.00	26,00	534.00	
Nudd 12' boom	Α	From Leg	1.00	0.0000	127.50	No Ice	17.10	9,30	254.00	
	i i		0.00			1/2" Ice	21.40	21.40	376.00	
			0.00			1" Ice	26,00	26,00	534.00	
Nudd 12' boom	В	From Leg	1.00	0.0000	127.50	No Ice	17.10	9,30	254.00	
	_		0.00			1/2" Ice	21.40	21.40	376.00	
			0.00			1" Ice	26.00	26,00	534.00	
Nudd 10' boom	С	From Leg	1.00	0.0000	115:00	No Ice	15.50	9.30	255.00	
11444 10 000111	•		0.00			1/2" Ice	19.60	19.60	367.00	
			0.00			1" Ice	24.00	24.00	512.00	
Nudd 10' boom	С	From Leg	1.00	0.0000	115.00	No Ice	15.50	9.30	255.00	
7.122 70 000			0.00			1/2" Ice	19.60	19.60	367.00	
			0.00			1" Ice	24.00	24.00	512.00	
Nudd 10' boom	Α	From Leg	1.00	0.0000	115.00	No Ice	15.50	9.30	255.00	
11222 10 000			0.00			1/2" Ice	19.60	19.60	367.00	
			0.00			1" Ice	24.00	24.00	512.00	
Nudd 12' boom	Α	From Leg	1.00	0.0000	140.00	No Ice	17.10	9.30	254.00	
(Sprint)			0.00			1/2" Ice	21.40	21.40	376.00	
(Optinit)			0.00			1" Ice	26.00	26.00	534.00	
Nudd 12' boom	В	From Leg	1.00	0.0000	140.00	No Ice	17.10	9.30	254.00	
(Sprint)		Trom 20g	0.00	0,000	1.0.00	1/2" Ice	21.40	21.40	376.00	
(opinit)			0.00			1" Ice	26.00	26.00	534.00	
Nudd 12' boom	С	From Leg	1.00	0.0000	140.00	No Ice	17.10	9.30	254.00	
(Sprint)	Ü	Trom Log	0.00	0.0000	110.00	1/2" Ice	21.40	21,40	376.00	
(Oprinit)		25	0.00			1" Ice	26.00	26.00	534.00	
catel Lucent 1900 MHz	Α	From Leg	4.00	0.0000	140.00	No Ice	2.58	2.54	60.00	
RRH			0.00			1/2" Ice	2.77	2.73	86.50	
(Sprint)			0.00			1" Ice	2.96	1.00	110.20	
catel Lucent 1900 MHz	В	From Leg	4.00	0.0000	140.00	No Ice	2.91	3.80	60.00	
RRH	D	, rom Leg	0.00	0.0000	140,00	1/2" Ice	3.11	4.03	86.50	
(Sprint)			0.00			1" Ice	3.33	4.27	110.20	
catel Lucent 1900 MHz	С	From Leg	4.00	0.0000	140.00	No Ice	2.91	3.80	60.00	
RRH	C	1 tolli reg	0.00	0.0000	140.00	1/2" Ice	3.11	4.03	68.50	

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Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
	Leg		Lateral						
			Vert ft	o	ft		ft²	ft²	lb
			ft		н		Ji	Ji	10
			ft						
(Sprint)			0.00			1" loe	3.33	4.27	110.20
RFS IBC1900BB-3	A	From Log	4.00	0.0000	140.00	No Ice	1.41	0.63	22 00
(Sprint)	~	LIGHT FAR	0.00	0.0000	140.00	1/2" Ice	1.33	0.74	31.00
(ohum)			0.00			1" Ice	1.71	0.87	42.20
DECIDERONDED 3	ъ	From Loa		0.0000	140.00			0.63	
RFS IBC1900BB-3	B	From Leg	4.00	0.0000	140.00	No Ice	1,41		22.00
(Sprint)			0.00			1/2" Ice	1.55	0.74	31.00
DECIDATIONED 2		E 1	0.00	0.0000	1.40.00	1" Ice	1.71	0.87	42,20
RFS IBC1900BB-3	C	From Log	4.00	0.0000	140,00	No Ice	1,41	0.63	22,00
(Sprint)			0.00			1/2" Ice	1.35	0.74	31,00
			0,00			1" Ice	1.71	0.87	42.20
(2) RFS APL868013	C	From Leg	4.00	0.0000	127,50	No Ice	2,87	3.61	8.20
(Verizon)			0.00			1/2" Ice	3.17	3.92	33,60
			0.00			l" Ice	3.52	4.48	64.96
(2) RFS APL868013	Α	From Leg	4.00	0,0000	127.50	No Ice	2.87	3.61	8.20
(Verizon)		•	0.00			1/2" Ice	3.17	3.92	33.60
			0.00			1" Ice	3,52	4.48	64.96
(2) RFS APL866513	В	From Leg	4.00	0.0000	127.50	No Ice	4.29	3.73	15.70
(Verizon)			0.00	0,0000		1/2" Ice	4.62	4.05	47.00
(10112011)			0.00			1" Ice	4.95	4,38	82.70
RFS FD9R6004/2C-3L	C	From Leg	4.00	0,0000	127.50	No Ice	0.31	0.08	3.10
	•	r totti Leg	0.00	0,000	127.50	1/2" Ice	0.38	0.12	5.40
(Verizon)									
DEC EDOD (004/00 31		F	0.00	0.0000	102.50	1" Ice	3.52	4.48	64.96
RFS FD9R6004/2C-3L	Α	From Leg	4.00	0.0000	127.50	No Ice	0.31	0.08	3.10
(Verizon)			0.00			1/2" Ice	0.38	0.12	5.40
	_		0.00			1" Ice	3.52	4.48	64,96
RFS FD9R6004/2C-3L	В	From Leg	4.00	0.0000	127.50	No Ice	0.31	0.08	3.10
(Verizon)			0.00			1/2" Ice	0.38	0.12	5,40
			0.00			1" Ice	3.52	4.48	64.96
S DB-T1-6Z-8AB-0Z	C	From Leg	4.00	0.0000	12.75 - 127.50	No Ice	4.80	2.00	10.00
(Verizon)		_	0.00			1/2" Ice	5.04	2.17	46.10
			0.00			1" Ice	5.28	2.34	82.20
Commscope	Α	From Leg	4.00	0.0000	127.50	No Ice	8.17	5.41	38.80
LNX-6514DS-VTM			0.00			1/2" Ice	8.63	5,88	89.30
(Verizon)			0.00			1" Ice	3.52	4.48	64.96
Commscope	В	From Leg	4.00	0.0000	127.50	No Ice	8.17	5.41	38.80
LNX-6514DS-VTM		- 10111 1105	0.00	0.0000	.2.,50	1/2" Ice	8.63	5.88	89.30
(Verizon)			0.00			1" Ice	3.52	4.48	64.96
	C	From I ag	4.00	0.0000	127.50	No Ice	8.17	5.41	38.80
Commscope		From Leg		0.0000	141,30	1/2" Ice		5.88	89.30
LNX-6514DS-VTM			0.00				8.63		
(Verizon)		F 1	0.00	0.0000	107.50	1" Ice	3.52	4.48	64.96
Commscope =	Α	From Leg	4.00	0.0000	127,50	No Ice	5.42	3.28	29.80
BXX-9014DS-VTM			0.00			1/2" Ice	5.74	3.60	65.10
(Verizon)	-		0.00	0.05		1" Ice	3.52	4.48	64.96
Commscope	В	From Leg	4.00	0.0000	127.50	No Ice	5.42	3.28	29.80
BXX-9014DS-VTM			0,00			1/2" Ice	5.74	3.60	65.10
(Verizon)			0.00			I" Ice	3.52	4.48	64.96
Commscope	C	From Leg	4.00	0.0000	127.50	No Ice	5.42	3.28	29.80
BXX-9014DS-VTM		-	0.00			1/2" Ice	5.74	3.60	65.10
(Verizon)			0.00			1" Ice	3.52	4.48	64.96
Commscope	Α	From Leg	4.00	0.0000	127.50	No Ice	8.53	5.24	43.00
BXX-6517DS-A2M			0.00			1/2" Ice	9.00	5.74	93.50
(Verizon)			0.00			1" Ice	3.52	4.48	64.96
Commscope	В	From Leg	4.00	0.0000	127.50	No Ice	8.53	5.24	43.00
BXX-6517DS-A2M	D	1 TOTAL LUE	0.00	0.0000	121.50	1/2" Ice	9.00	5.74	93.50
(Verizon)						1" Ice			64.96
LVCHZUIII			0.00				3.52	4.48	
` '		Ceons I							
Commscope XX-6517DS-A2M	C	From Leg	4.00 0.00	0.0000	127.50	No Ice 1/2" Ice	8.53 9.00	5.24 5.74	43,00 93,50

RISATower	Job	150' SS Tower Norwich, CT. Analysis	Page 12 of 34
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Description	Face	Offset	Offsets:	Azimuth	Placement		$C_{4}A_{1}$	C_AA_A	Weight
7	or	Туре	Horz	Adjustment			Front	Side	
	Leg		Lateral						
			Vert	٥	a		ft²	ft²	lb
			ft ft	-	ft		Ji	Ji	10
			ft						
(Verizon)			0.00			1" Ice	3.52	4.48	64.96
Alcatel Lucent	Α	From Leg	4.00	0.0000	127.50	No Ice	1.88	1.24	44.00
RH_2x60-AWS			0.00			1/2" Ice	2.03	1.37	60.00
(Verizon)	-		0.00	0.0000	100.00	1" Ice	3.52	4.48	64.96
Alcatel Lucent	B	From Leg	4.00 0.00	0.0000	127.50	No Ice 1/2" Ice	1.88 2.03	1.24 1.37	44.00 60.00
RH_2x60-AWS (Verizon)			0,00			1" Ice	3.52	4.48	64.96
Alcatel Lucent	C	From Leg	4.00	0.0000	127.50	No Ice	1,88	1.24	44.00
RH_2x60-AWS		Trom tog	0.00	0.0000	121/00	1/2" Ice	2.03	1.37	60,00
(Verizon)			0.00			1" Ice	3.52	4.48	64.96
Alcatel Lucent	Α	From Leg	4.00	0.0000	127.50	No Ice	2.16	1.62	44,00
RH_2x60-70OU			0,00			1/2" Ice	2.33	1.77	63.60
(Verizon)			0.00			1" Ice	3,52	4.48	64.96
Alcatel Lucent	В	From Leg	4.00	0.0000	127.50	No Ice	2.16	1.62	44.00
RH_2x60-70OU			0.00			1/2" Ice	2.33	1.77	63,60
(Verizon) Alcatel Lucent	C	From Leg	0.00 4,00	0.0000	127.50	1" Ice No Ice	3.52 2.16	4.48 1.62	64.96 44.00
RH 2x60-70OU	C	riom reg	0.00	0.0000	127.50	1/2" Ice		1.77	63.60
(Verizon)			0.00			1" Ice	3,52	4.48	64.96
Alcatel Lucent	Α	From Leg	4.00	0,0000	127.50	No Ice	1.84	1.34	46.00
RH_2x60-PCS			0.00			1/2" Ice	2.00	1.48	62.60
(Verizon)			0.00			1" lce	3.52	4.48	64,96
Alcatel Lucent	В	From Leg	4.00	0.0000	127.50	No Ice	1.84	1.34	46.00
RH_2x60-PCS			0.00			1/2" Ice	2.00	1.48	62,60
(Verizon)	_	-	0.00		107.50	1" Ice	3.52	4.48	64.96
Alcatel Lucent	C	From Leg	4.00	0.0000	127.50	No Ice 1/2" Ice	1.84 2.00	1,34 1,48	46.00 62.60
RH_2x60-PCS			0.00			1" Ice	3.52	4.48	64.96
(Verizon) Ericsson AIR 32	Α	From Leg	4.00	0.0000	150.00	No Ice	6.51	4.71	100,00
B66AA/B2A	^	I TOTAL LOG	0.00	0,0000	130.00	1/2" Ice	6.87	5.07	145.80
(T-Mobile)			0.00			l" Ice	7.24	5.43	196.90
Ericsson AIR 32	В	From Leg	4.00	0.0000	150.00	No Ice	6.51	4.71	100,00
B66AA/B2A		_	0.00			1/2" Ice	6.87	5.07	145.80
(T-Mobile)			0.00			l" Ice	7.24	5.43	196.90
Ericsson AIR 32	C	From Leg	4.00	0.0000	150.00	No Ice	6.51	4.71	100.00
B66AA/B2A			0.00			1/2" Ice	6.87	5.07	145.80
(T-Mobile)		E I	0.00 4.00	0.0000	150,00	1" Ice No Ice	7.24 8.17	5.43 3.13	196,90 46,30
Commscope DBXHN-6565B-A2M	Α	From Leg	0.00	0.0000	130.00	1/2" Ice	8.63	3.60	96.80
(T-Mobile)			0.00			1" Ice	3.60	4.07	153.50
Commscope	В	From Leg	4.00	0.0000	150.00	No Ice	8.17	3.13	46.30
DBXHN-6565B-A2M	_		0.00			1/2" Ice	8.63	3.60	96.80
(T-Mobile)			0.00			1" Ice	3.60	4.07	153.50
Commscope	C	From Leg	4.00	0.0000	150.00	No Ice	8.17	3,13	46.30
DBXHN-6565B-A2M			0.00			1/2" Ice	8.63	3.60	96,80
(T-Mobile)			0.00			1" Ice	3.60	4.07	153.50
(2) Ericsson KRY112/2	Α	From Leg	4.00	0.0000	150,00	No Ice	0.35	0.16	11.00
(T-Mobile)			0.00 0.00			1/2" Ice 1" Ice	0.42 0.50	0.21 0.27	14.10 18.40
(2) Ericsson KRY112/2	В	From Leg	4.00	0.0000	150.00	No Ice	0.35	0.16	11.00
(T-Mobile)	D	1 Tolli Leg	0.00	0.0000	150.00	1/2" Ice	0.33	0.10	14.10
(1-14100116)			0.00			I" Ice	0.50	0.27	18.40
(2) Ericsson KRY112/2	С	From Leg	4.00	0.0000	150.00	No Ice	0.35	0.16	11.00
(T-Mobile)	-		0.00			1/2" Ice	0.42	0.21	14.10
, ,			0.00			l" Ice	0.50	0.27	18.40
Commscope DT465B-2XR	Α	From Leg	4.00	0.0000	140.00	No Ice	9.22	5.87	50.00
(Sprint)			0.00			1/2" Ice	10.14	6.79	172.40

RISATower	Job	150' SS Tower Norwich, CT. Analysis	Page 13 of 34
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		C_AA_A Front	C ₄ A _{.1} Side	Weight
	Leg		Lateral	,					
			Vert	0			e2	c?	,,
			ft ft	Ü	fi		ft²	ft²	lb
			ft						
			0.00			1" Ice	11.07	7.70	320.70
Commscope DT463B-2XR	B	From Leg	4.00	0.0000	140.00	No Ice	9.22	3.87	30.00
(Sprint)		•	0.00			1/2" Ice	10.14	6.79	172,40
			0.00			1" lee	11.07	7.70	320.70
Commscope DT465B-2XR	C	From Leg	4.00	0.0000	140.00	No Ice	9.22	3.87	30.00
(Sprint)			0.00			1/2" 100	10.14	6.79	172,40
			0.00			1" Ice	11.07	7.70	320,70
(2) Alcatel Lucent 4x45	Α	From Leg	4.00	0.0000	140.00	No Ice	2.54	1,61	51.00
(Sprint)			0,00			1/2" Ice	2,92	1.96	94,30
			0.00	0.0000	1.40.00	1" Ice	3,35	2,33	150.90
(2) Alcatel Lucent 4x45	В	From Leg	4.00	0.0000	140.00	No Ice	2.54	1.61	51.00
(Sprint)			00.0			1/2" Ice	2.92	1.96	94,30
(2) Alcotel I mount And	C	From I an	0,00	0.0000	140.00	l" Ice	3,35	2.33	150,90
(2) Alcatel Lucent 4x45 (Sprint)	С	From Leg	4,00 0.00	0,0000	140.00	No Ice 1/2" Ice	2.54 2.92	1.61 1.96	51,00 94.30
(ahint)			0.00			1" lce	3.35	2.33	150.90
Alcatel Lucent 8x200-25	Α	From Leg	4,00	0.0000	140.00	No Ice	4.05	1.53	70.00
(Sprint)	Α	r tom Leg	0.00	0.0000	140.00	1/2" Ice	4.50	1.88	127.80
(бринк)			0.00			1" Ice	4,99	2.26	200,50
Alcatel Lucent 8x200-25	В	From Leg	4,00	0.0000	140.00	No Ice	4.05	1.53	70.00
(Sprint)	42		0,00	0,0000	11000	1/2" Ice	4.50	1.88	127,80
(2)			0.00			1" Ice	4.99	2.26	200.50
Alcatel Lucent 8x200-25	С	From Leg	4.00	0.0000	140.00	No Ice	4.05	1.53	70.00
(Sprint)			0.00			1/2" Ice	4.50	1.88	127.80
			0.00			l" Ice	4,99	2,26	200,50
FS APXV9ERR18-C-A20	Α	From Leg	4.00	0.0000	140.00	No Ice	8.02	5.81	62,00
(Sprint)			0.00			1/2" Ice	8.93	6.73	172.10
			0.00			1" Ice	9.86	7.64	307.60
FS APXV9ERR18-C-A20	В	From Leg	4.00	0.0000	140.00	No Ice	8.02	5.81	62.00
(Sprint)			0.00			1/2" Ice	8.93	6.73	172.10
			0.00			1" Ice	9.86	7.64	307.60
FS APXV9ERR18-C-A20	C	From Leg	4.00	0.0000	140.00	No Ice	8.02	5.81	62.00
(Sprint)			0.00			1/2" Ice	8.93	6.73	172,10
			0.00			1" Ice	9.86	7.64	307.60
) Alcatel Lucent RRH2x50	Α	From Leg	4.00	0.0000	140.00	No Ice	2.27	1.35	42.00
(Sprint)			0.00			1/2" Ice	2.64	1.68	79.60
D ###0.00			0.00	0.0000	11500	1" Ice	3.05	2.04	129.60
Powerwave 7770.00	Α	From Leg	4.00	0.0000	115.00	No Ice	5.51	2.93	35.00
(AT&T)			0.00			1/2" Ice	6.21	3.64	105.10
Powerwave 7770.00	В	Erom I oo	0.00 4.00	0.0000	115.00	l" Ice No Ice	6.93 5.51	4.33 2.93	195.10 35.00
(AT&T)	Ь	From Leg	0.00	0.0000	113.00	1/2" Ice	6.21	3.64	105.10
(AI&I)			0.00			1" Ice	6.93	4.33	195.10
Powerwave 7770.00	C	From Leg	4.00	0.0000	115.00	No Ice	5.51	2.93	35.00
(AT&T)		I Iom Leg	0.00	0.0000	115.00	1/2" Ice	6.21	3.64	105.10
(11.6.1)			0.00			1" Ice	6.93	4.33	195.10
MW EPBQ-654L8H8-L2	Α	From Leg	4.00	0.0000	115.00	No Ice	18.09	7.03	200.00
(AT&T)			0.00	2,2200		1/2" Ice	19.31	8.35	394.70
(<i>-</i>)			0.00			1" Ice	20.55	9,57	622.90
MW EPBQ-654L8H8-L2	В	From Leg	4.00	0.0000	115.00	No Ice	18.09	7.03	200.00
(AT&T)	_	248	0.00			1/2" Ice	19.31	8.35	394.70
\/			0.00			1" Ice	20.55	9.57	622.90
MW EPBQ-654L8H8-L2	C	From Leg	4.00	0.0000	115.00	No Ice	18.09	7.03	200.00
(AT&T)		J	0.00			1/2" Ice	19.31	8.35	394.70
, ,			0.00			1" Ice	20.55	9.57	622.90
CCI OPA-65R-LCUU-H8	Α	From Leg	4.00	0.0000	115.00	No Ice	12.76	7.48	70.90
(AT&T)		-	0.00			1/2" Ice	13.94	8.72	223.30

RISATower	Job	150' SS Tower Norwich, CT. Analysis	Page 14 of 34
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		C₄A₄ Front	C₄A₄ Side	Weight
	Leg	Туре	Lateral	лијизинени			170111	Diue	
			Vert	٥			ft²	ft²	lb
			ft ft	-	ft		Jt	Ji	10
			ft ft						
			0,00			1" Ice	13.11	9.90	47.00
CCI OPA-65R-LCUU-H8	B	From Leg	4.00	0,0000	115.00	No Ice	12.76	7.48	70.90
(AT&T)		-	0.00			1/2" Ice	13.94	8.72	223.30
			0,00			l" Ice	15:11	9.90	47.00
CCI OPA-65R-LCUU-H8	C	From Leg	4.00	0.0000	115.00	No Ice	12.76	7.48	70.90
(AT&T)			0.00			1/2" Ice	13.94	8.72	223,30
			0,00			1" Ice	15.11	9.90	47,00
Ericsson RRUS11	Α	From Leg	4.00	0.0000	115.00	No Ice	2.78	1,19	55.00
(AT&T)			0,00			1/2" Ice	3.16	1.47	99,60
			0.00			1" Ice	3,57	1,79	1 57 .10
Ericsson RRUS11	B	From Leg	4.00	0.0000	115.00	No Ice	2.78	1.19	55,00
(T&TA)			0,00			1/2" Ice	3.16	1.47	99.60
		_	0.00			1" Ice	3,57	1.79	157.10
Ericsson RRUS11	C	From Leg	4.00	0.0000	115.00	No Ice	2.78	1.19	55.00
(AT&T)			0.00			1/2" Ice	3.16	1.47	99.60
		_	0.00			1" Ice	3.57	1.79	157.10
(3) Ericsson RRUS32	Α	From Leg	4.00	0.0000	115.00	No Ice	2.06	0.50	22.00
(AT&T)			0.00			1/2" Ice	2.39	0.72	49.80
	_		0.00			l" Ice	2.75	0.97	88,20
(3) Ericsson RRUS32	В	From Leg	4,00	0.0000	115.00	No Ice	2.06	0.50	22.00
(AT&T)			0.00			1/2" Ice	2.39	0.72	49.80
			0.00		11.500	1" lce	2.75	0.97	88.20
(3) Ericsson RRUS32	C	From Leg	4.00	0.0000	115.00	No Ice	2.06	0.50	22.00
(AT&T)			0.00			1/2" Ice	2.39	0.72	49.80
T : D14 44E0			0.00	0.0000	115.00	1" Ice	2.75	0.97	88.20
Ericsson B14 4478	Α	From Leg	4.00	0.0000	115.00	No Ice	1.84	1.06	50.00
(AT&T)			0.00			1/2" Ice	2.15	1.31	84.40
F: D14 4450		г .	0.00	0.0000	115.00	1" Ice	2.50	1.61	130.10
Ericsson B14 4478	В	From Leg	4.00	0.0000	115.00	No Ice	1.84	1.06	50.00
(AT&T)			0.00			1/2" lce	2.15	1.31	84.40
Friegen D14 4430	С	F I	0.00	0.0000	115.00	1" Ice No Ice	2.50 1.84	1.61 1.06	130,10 50.00
Ericsson B14 4478	C	From Leg	4.00	0.0000	115.00	1/2" lce	2.15	1.31	84.40
(AT&T)			0.00 0.00			1" Ice	2.13	1.61	130.10
) Povenena I GP21401	Α	From Log	4.00	0.0000	115.00	No Ice	1.67	0.47	31.00
?) Powerwave LGP21401 (A'1&'1)	А	From Leg	0.00	0.0000	113,00	1/2" Ice	1.96	0.47	55.30
(AIWI)			0.00			1" Ice	2.30	0.90	89.40
2) Powerwave LGP21401	Α	From Leg	4.00	0.0000	115.00	No Ice	1.67	0.47	31.00
(AT&T)		2.000 Dog	0.00	0.0000	115.00	1/2" Ice	1.96	0.67	55.30
(*******)			0.00			1" Ice	2.30	0.90	89.40
) Powerwave LGP21401	Α	From Leg	4.00	0.0000	115.00	No Ice	1.67	0.47	31.00
(AT&T)			0.00			1/2" Ice	1.96	0.67	55.30
·/			0.00			l" Ice	2.30	0.90	89.40
Alcatel Lucent RRH2x50	В	From Leg	4.00	0.0000	140.00	No Ice	2.27	1.35	42.00
(Sprint)	_		0.00			1/2" Ice	2.64	1.68	79.60
/- i			0.00			1" Ice	3.05	2.04	129.60
Alcatel Lucent RRH2x50	C	From Leg	4.00	0.0000	140.00	No Ice	2.27	1.35	42.00
(Sprint)	•		0.00	•	•	1/2" Ice	2.64	1.68	79.60
7			0.00			1" Ice	3.05	2.04	129.60
Raycap DC6-48-60	Α	From Leg	4.00	0.0000	115.00	No Ice	1.28	1.28	31,80
(AT&T)		-	0.00			1/2" Ice	1.64	1.64	80.10
` '			0.00			I" Ice	2.04	2.04	141.90
Raycap DC6-48-60	В	From Leg	4.00	0.0000	115.00	No Ice	1.28	1.28	31.80
(AT&T)		_	0.00			1/2" Ice	1.64	1.64	80.10
, ,			0.00			1" Ice	2.04	2.04	141.90
Raycap DC6-48-60	C	From Leg	4.00	0.0000	115.00	No Ice	1.28	1.28	31.80
(AT&T)			0.00			1/2" Ice	1.64	1.64	80.10

RISATower	Job 1	50' SS Tower Norwich, CT. Analysis	Page 15 of 34
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
			fi fi fi	0	fi		ft²	ft²	lb
			0,00			1" Ice	2.04	2.04	141.90

Tower Pressures - No Ice

 $G_H = 0.850$

Section	Z	Kz	q_z	A_G	F	A_F	A_R	Aleg	Leg	C_AA_A	$C_A A_A$
Elevation					а			"	%	In	Out
l 1					С					Face	Face
fi	fi		psf	fî ²	e	ft ²	ft²	fi²		ft²	ft²
TI	145.00	1.099	26	62,396	Α	4.993	4.792	4.792	48,97	0.000	0.000
150,00-140,00					В	4,993	4.792		48.97	0,000	0,000
					C	4.993	4.792		48.97	29.115	0.000
T2	130.00	1.065	25	124.792	Α	9,730	9.583	9.583	49.62	18.400	0.000
140.00-120.00					В	9,730	9,583		49.62	14,130	0,000
					С	9.730	9,583	1	49.62	58.230	0.000
Т3	110.00	1.016	24	147.509	Α	10.623	15.025	15.025	58.58	18.400	0.000
120.00-100.00					В	10.623	15.025		58.58	37.680	0.000
					С	10.623	15.025		58.58	95.130	0.000
T4	90.00	0.959	23	189,283	Α	12.997	18.574	18.574	58.83	18,400	0.000
100.00-80.00					В	12.997	18.574		58.83	37.680	0.000
					C	12.997	18.574	ll i	58.83	107.430	0.000
T5 80.00-60.00	70.00	0.892	21	231.055	Α	18.030	22.120	22.120	55.09	18.400	0.000
					В	18.030	22.120		55.09	37.680	0.000
					С	18.030	22,120	1	55.09	107.430	0.000
T6 60.00-40.00	50.00	0,811	19	274.393	Α	20,444	28.798	28.798	58.48	18.400	0.000
					В	20.444	28.798		58.48	37.680	0.000
					C	20.444	28.798		58.48	107.430	0.000
T7 40.00-20.00	30.00	0.701	16	314.393	Α	19.628	28.798	28.798	59.47	18.400	0.000
	- 1		2.		В	19.628	28.798		59.47	37.680	0.000
			1		C	19.628	28.798		59.47	107.430	0.000
T8 20.00-0.00	10.00	0.7	16	354.393	Α	21.673	28.798	28.798	57.06	15.300	0.000
					В	21.673	28.798		57.06	29.760	0.000
			1		C	21.673	28.798		57.06	85.260	0.000

Tower Pressure - With Ice

 $G_H = \theta.85\theta$

Section Elevation	z	Kz	q_z	t _Z	A_G	F a	A_F	A_R	A _{leg}	Leg %	C _A A _A In	C _A A _A Out
ft	ſi		psf	in	ft²	c e	ft²	ft²	ft²		Face ft²	Face _ft²
TI	145.00	1.099	6	1.7393	65.295	Α	4.993	20.499	10.589	41.54	0.000	0,000
150.00-140.00	- 1			- 1		В	4.993	20.499	- 1	41.54	0.000	0.000
-: 2/2/2/3	- 1		YI.			C	4.993	20.499		41.54	63.684	0.000
T2	130.00	1.065	6	1.7204	130.526	Α	9.730	37.793	21.053	44.30	50.725	0.000

RISATower	Job	150' SS Tower Norwich, CT. Analysis	Page 16 of 34
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Section	z	Kz	q_z	tz	A_G	F	A_F	A_R	Aleg	Leg	C_4A_A	C_AA_A
Elevation						a	1 1			%	In	Out
				în	ft²	С	ft ²	ft²	fi²		Face ft ²	Face ft²
JI	ft		psf	m	Il	е			JI		_	
140.00-120.00						B	9.730	37.793		44.30		0.000
						C	9.730	37.793		44.30		
T3	110.00	1.016	6	1.6919	133.136	A	10.623	44.297	26.323	47.93	50.429	0,000
120.00-100.00						B	10.623	44.297		47,93	103.982	0.000
						C	10.623	44,297		47.93	211.118	0,000
T4 100.00-80.00	90,00	0.939	5	1.6583	194.818	A	12.997	46,890	29.648	49.31	50.081	0.000
						В	12,997	46,890		49.51	103,419	0.000
l						C	12.997	46.890		49.51	237.766	0.000
T3 80,00-60.00	70.00	0.892	5	1,6171	236.453	A	18,030	52.357	32,919	46.77	49.635	0.000
	,	1	· ·	******		В	18,030	52.357		46.77	102.730	0.000
						Ĉ	18.030	52.357		46.77	235.862	0,000
T6 60,00-40,00	\$0.00	0.811	4	1.5636	279,612	Ā	20.444	60.551	39,240		49.102	0.000
10 00,00,40,00	20,00	0.011		1.2020	212,010	В	20,444	60.351	0.2.2 Tu	48.45	101.835	0.000
						C	20,444	60.551		48.45	233.390	0,000
T7 40,00-20.00	30.00	0.701	ا	1,4858	319.352	Ā	19.628	55.384	38.720		48.299	0.000
1740,00-20.00	30.00	0.701	"	1,4020	319.332	B	19.628	55.384	30,720	51.62	100.535	0,000
					- 1	D					229.794	0,000
WO 40 00 000	10.00			1 2212	250.026	Č	19,628	55.384	37.607	51.62		
T8 20.00-0.00	10.00	0.7	4	1.3312	358,836	A	21,673	54.173	37.687	49,69	37,863	0,000
						В	21.673	54.173		49,69	76,302	0,000
						C	21.673	54.173		49.69	177.014	0.000

Tower Pressure - Service

 $G_H = 0.850$

Section	Z	Kz	q_z	A_G	F	A_F	A_R	Aleg	Leg	$C_A A_A$	C_AA_A
Elevation					а				%	In	Out
					С					Face	Face
ft	ft		psf	ft²	е	ft²	ft²	ft²		ft²	ft²
T1	145.00	1.099	9	62,396	Α	4,993	4.792	4.792	48.97	0.000	0.000
150.00-140.00		1			В	4.993	4.792		48.97	0.000	0.000
					C	4.993	4.792		48.97	29.115	0.000
T2	130.00	1.065	8	124.792	Α	9.730	9.583	9.583	49.62	18.400	0.000
140.00-120.00					В	9.730	9.583		49.62	14.130	0.000
		ľ			С	9.730	9.583		49.62	58.230	0.000
T3	110.00	1.016	8	147.509	Α	10.623	15.025	15.025	58.58	18.400	0.000
120.00-100.00					В	10.623	15.025		58.58	37.680	0.000
0 55					C	10.623	15.025		58.58	95.130	0.000
T4	90.00	0.959	8	189.283	Α	12,997	18.574	18.574	58.83	18,400	0.000
100.00-80.00					В	12,997	18.574		58.83	37.680	0.000
1 1					C	12.997	18.574		58.83	107.430	0.000
T5 80.00-60.00	70.00	0.892	7	231.055	Α	18.030	22.120	22.120	55.09	18.400	0.000
			ľ		В	18.030	22.120		55.09	37.680	0.000
					C	18.030	22.120		55.09	107.430	0.000
T6 60.00-40.00	50.00	0.811	6	274.393	Α	20.444	28.798	28.798	58.48	18.400	0.000
27		10.1			В	20.444	28.798		58.48	37.680	0.000
					C	20.444	28.798		58.48	107.430	0.000
T7 40.00-20.00	30.00	0.701	5	314.393	Α	19.628	28.798	28.798	59.47	18.400	0.000
					В	19.628	28.798		59.47	37.680	0.000
					C	19.628	28.798		59,47	107.430	0.000
T8 20.00-0.00	10,00	0.7	5	354.393	Α	21.673	28.798	28.798	57.06	15.300	0.000
					В	21.673	28.798		57.06	29.760	0.000
					C	21.673	28.798		57.06	85.260	0.000

RISATower	Job	150' SS Tower Norwich, CT. Analysis	17 of 34
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Tower Forces - No Ice - Wind Normal To Face

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	\dot{A}_E	F	w	Ctrl.
Elevation	Weight	Weight	а			1-	20052					Face
			c			psf			1			
fi	1b	lb	e						ft ²	1b	plf	l
TI	192.80	421.53	A	0.137	2.747	26	1	1	7.716	849,93	84.99	C
130.00-140.00			В	0.137	2.747		1	1	7.716			
			C	0.157	2.747		1	1	7.716			
T2	718.60	791.76	Α	0.135	2.754	25	- 1	1	15,174	2030.87	102.54	C
140.00-120.00		. 5 4.641155	В	0.155	2.754		- 1	1	15.174			
			C	0.155	2.754		1	1	15.174			
Т3	1210.40	1140.72	Α	0.174	2,686	24	i i	1	18,716	2864.29	143.21	C
120.00-100.00			В	0.174	2.686		1	1	18.716			
		1	C	0.174	2.686		1	1	18.716			
T4	1312.00	1485.02	Α	0.167	2,711	23	1	1	22,320	3042.67	152.13	C
100,00-80,00			В	0.167	2.711		1	1	22,320			
			C	0.167	2.711	11	1	1	22.320			l
T5	1312.00	1986.33	Α	0,174	2.686	21	1	1	28,506	3118.68	155.93	C
80,00-60.00			B	0.174	2.686		1	1	28,506			1
			C	0.174	2.686		1	1	28,506			
Т6	1312.00	2680.91	Α	0,179	2.666	19	1	1	32,950	3015,74	150,79	C
60.00-40.00			В	0.179	2.666		1	1	32.950			
			C	0.179	2.666		1	1	32.950			
T7	1312.00	3829.48	Α	0.154	2.757	16	1	1	31,934	2608.86	130.44	С
40.00-20.00		~	В	0.154	2,757		ì	1	31.934			
			С	0.154	2.757		1	1	31.934			
T8 20.00-0.00	1167.10	3948.61	Α	0,142	2.8	16	1	1	33.820	2420.89	121.04	С
			В	0.142	2.8		1	1	33.820			
	i i		C	0.142	2.8		1	1	33.820			
Sum Weight:	8536.90	16284.35						OTM	1450335.4	19971.93		
									0 lb-ft			

Tower Forces - No Ice - Wind 60 To Face

Section	Add	Self	F	e	C_F	q:	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а					1			1	Face
			С			psf						
ft	lb	lb	е						ft²	lb	plf	
T1	192.80	421.53	Α	0.157	2.747	26	0.8	1	6.717	789.64	78.96	С
150.00-140.00			В	0.157	2.747		0.8	1	6.717			
			C	0.157	2.747		0.8	1	6.717			
T2	718.60	791.76	Α	0.155	2.754	25	0.8	1	13.228	1936.67	96.83	C
140.00-120.00	1		В	0.155	2.754		0.8	Ī	13,228			
			C	0.155	2.754		0.8	1	13.228			
T3	1210.40	1140.72	Α	0.174	2.686	24	0.8	1	16.592	2748.37	137.42	C
120.00-100.00			В	0.174	2.686		0.8	1	16.592			
			C	0.174	2.686		0.8	1	16,592			
T4	1312.00	1485.02	Α	0.167	2.711	23	0.8	1	19.721	2907.50	145.38	C
100.00-80.00			В	0.167	2.711		0.8	1	19.721			
			C	0.167	2:711		0.8	1	19.721			
T5	1312.00	1986.33	Α	0.174	2,686	21	0.8	1	24.900	2945.75	147.29	C
80.00-60.00			В	0.174	2.686		0.8	1	24.900			
			C	0.174	2.686		0.8	1	24.900			
Т6	1312.00	2680.91	Α	0.179	2.666	19	0.8	1	28,861	2838.95	141.95	C
60.00-40.00	- 1		В	0.179	2.666		0.8	1	28.861			

RISATower	Job	150' SS Tower Norwich, CT. Analysis	18 of 34
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Section Elevation	Add Weight	Self Weight	F a	е	C_F	q ₌	D_F	D_R	A_E	F	w	Ctrl. Face
fi	lb	lb	c e			psf			ft²	lb	plf	
T7 40.00-20.00	1312.00	3829.48	C A B	0.179 0.154 0.154	2.666 2.757 2.757	16	0.8 0.8 0.8	1	28.861 28.009 28.009	2457.19	122.86	c
T8 20.00-0.00	1167.10	3948.61	C A B	0.154 0.142 0.142	2.757 2.8 2.8	16	0.8 0.8 0.8	1	28,009 29,485 29,485	2250,93	112,55	c
Sum Weight:	8536.90	16284.35	C	0.142	2.8		0.8	ОТМ	29.485 29.485 1374635.9 7 lb-ft	18875.02		

Tower Forces - No Ice - Wind 90 To Face

Section	Add	Self	F	e	C_F	q ₌	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a						1 1			Face
			c			psf						
fi	lb	lb	e						ft²	lb	plf	
TI	192.80	421.53	A	0.157	2.747	26	0.85	1	6.967	804.71	80.47	C
150,00-140,00			В	0.157	2.747		0.85	1	6.967			
			С	0.157	2.747		0.85	1	6.967			
T2	718.60	791.76	Α	0.155	2.754	25	0.85	1	13.714	1965.22	98.26	С
140.00-120.00			В	0.155	2.754		0.85	1	13.714			
			С	0,155	2.754		0.85	1	13,714			
T3	1210.40	1140.72	Α	0.174	2.686	24	0.85	1	17.123	2777.35	138.87	С
120.00-100.00			В	0,174	2,686		0.85	1	17,123			
			C	0.174	2.686		0.85	1	17.123			
T4	1312.00	1485.02	Α	0,167	2.711	23	0.85	1	20,371	2941.29	147.06	С
100.00-80.00			В	0.167	2.711		0.85	1	20,371			
			C	0.167	2,711		0.85	1	20,371			
T5	1312.00	1986.33	Α	0.174	2.686	21	0.85	1	25.802	2988.98	149.45	С
80.00-60.00			В	0.174	2,686		0.85	1	25.802			
			C	0.174	2.686		0.85	1	25.802			
Т6	1312.00	2680.91	A	0.179	2.666	19	0.85	1	29,883	2883.15	144.16	С
60.00-40.00			В	0.179	2.666		0.85	1	29.883			
			C	0.179	2.666		0.85	1	29.883			
T7	1312.00	3829.48	Α	0.154	2.757	16	0.85	1	28.990	2495.10	124.76	С
40.00-20.00			В	0.154	2.757		0.85	1	28.990			
= =			С	0.154	2.757		0.85	1	28.990			
T8 20.00-0.00	1167.10	3948.61	Α	0.142	2.8	16	0.85	1	30.569	2293.44	114.67	С
			В	0.142	2.8		0.85	Ī	30.569			
			С	0.142	2.8		0.85	1	30.569			
Sum Weight:	8536.90	16284.35					- 2	OTM	1393560.8	19149.25		
									2 lb-ft			

Tower Forces - With Ice - Wind Normal To Face

Section	Add	Self	F	е	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf			_			
ft	lb	lb	e						ft ²	lb	plf	

RISATower	Job 15	60' SS Tower Norwich, CT. Analysis	Page 19 of 34
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Section	Add	Self	F	e	C_F	q _z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a	, i	-7	7-		27		1		Face
			c			psf			1 1			7 400
ft	lb	lb	e			l Pag			ft ²	lb	plf	
TI	1237.10	1673.10	A	0.39	2.083	6	1	1	17.931	384.19	38.42	C
130.00-140.00	1000	Sin	В	0.39	2 083	1	i	i	17.931	30,000		, i
			C	0.39	2.083		i	ì	17.931	- 1		
Т2	3952.89	3115.53	A	0.364	2 139	6	î	î	33.216	991.07	49,33	C
140.00-120.00			B	0.364	2.139		î	1	33.216	11.00		1
			C	0.364	2.139		i	1	33.216			
ТЭ	6307.10	3797.93	A	0.359	2.152	6	ì	ì	38.037	1414.40	70.72	C
120 00-100 00			В	0.359	2.152		î	1	38.037			
			C	0.359	2.152		î.	ì	38.057		11	
T4	6925,11	4440.73	A	0.307	2.277	5.	i	1	41.190	1456.72	72.84	C
100,00-80,00			В	0.307	2.277		ì	1	41.190			
			C	0,307	2.277		î	î	41.190			
T5	6794.28	5602.28	A	0.298	2.302	5	î	ì	49,350	1430.21	71.51	C
80.00-60.00			В	0.298	2.302		1	1	49.350		, , , , ,	
			C	0,298	2.302		1	1	49,350			
Т6	6625.91	6731.16	A	0.29	2.324	4	1	ĩ	56.519	1356.75	67.84	C
60.00-40.00			В	0.29	2.324		1.1	1	56.519			_
			C	0,29	2,324		1	1	56.519			
T7	6384.37	7409.58	Α	0.235	2.484	4	1	1	51.831	1152.96	57.65	C
40.00-20.00			В	0.235	2.484		1	1	51.831			
			C	0.235	2.484		1	ĩ	51.831			
T8 20.00-0.00	4810.91	7299,43	A	0.211	2.558	4	1	1	52.907	1003.63	50.18	C
			В	0.211	2.558		1	1	52,907			-
			C	0.211	2.558		1	1	52,907			
Sum Weight:	43237,67	40069.74					51	OTM	683812.12	9189.93		
									lb-ft			

Tower Forces - With Ice - Wind 60 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а	h		.~						Face
			c			psf						
ft	lb	lb	е						ft ²	lb	plf	
T1	1237.10	1673.10	Α	0.39	2,083	6	0.8	1	16.952	373.62	37.36	С
150.00-140.00			В	0.39	2.083		0.8	1	16.952			
			C	0.39	2.083		0.8	1	16.952			
T2	3952.89	3115.53	Α	0.364	2.139	6	0.8	1	31.270	970.56	48.53	C
140.00-120.00			В	0.364	2.139		0.8	1	31.270			
			C	0.364	2.139		0.8	1	31.270			
T3	6507.10	3797.93	Α	0.359	2.152	6	0.8	1	35.932	1392.93	69.65	С
120.00-100.00			В	0.359	2.152		0.8	1	35.932			
			C	0.359	2,152		0.8	1	35.932			
T4	6925.11	4440.73	Α	0.307	2.277	5	0.8	1	38.591	1430.48	71,52	C
100.00-80.00		1	В	0.307	2,277	- 1	0.8	1	38.591			
			C	0.307	2,277		0.8	1	38.591	- 1		
T5	6794.28	5602.28	Α	0,298	2.302	5	0.8	1	45.744	1395,95	69.80	C
80.00-60.00			В	0.298	2.302		0.8	1	45.744	- 1		
			C	0,298	2.302		0.8	1	45.744			
Т6	6625.91	6731.16	Α	0.29	2.324	4	0.8	1	52.430	1321,13	66.06	C
60,00-40.00			В	0.29	2.324		0.8	1	52.430	ı		
		"	C	0.29	2.324		0.8	1	52.430	- 1		
Т7	6384.37	7409.58	Α	0.235	2.484	4	0.8	1	47.905	1121.38	56.07	C
40,00-20,00			В	0.235	2,484		0.8	1	47.905	- 1		
			C	0.235	2.484		0.8	1	47.905	- 1		

RISATower	Job 150' SS	Tower Norwich, CT. Analysis	20 of 34
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Section Elevation	Add Weight	Self Weight	F a	е	C_F	q ₌	D_F	D_R	A_E	F	w	Ctrl. Face
ft	Ib	lb	c e			psf			ft²	<i>lb</i>	plf	
T8 20,00-0,00	4810,91	7299.43	A B C	0.211 0.211 0.211	2.558 2.558 2.558	4	0.8	1	48.573 48.573 48.573	967 74	48.39	C
Sum Weight:	43237,67	40069.74						OTM	669404.94 lb=ft	8973.79		

Tower Forces - With Ice - Wind 90 To Face

Section	Add	Self	F	e	C_F	q=	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			c			psf						
ft	lb	lb	e						ft ²	lb lb	plf	
TI	1237.10	1673.10	Α	0.39	2.083	6	0.85	1	17.202	376.26	37.63	C
150.00-140.00			В	0.39	2.083		0.85	1	17.202			
			C	0.39	2.083		0.85	1	17,202			
T2	3952.89	3115.53	Α	0.364	2.139	6	0.85	1	31.757	975,69	48.78	С
140.00-120.00			В	0.364	2.139		0.85	1	31.757			
			C	0.364	2.139		0.85	1	31.757			
Т3	6507,10	3797.93	Α	0.359	2.152	6	0.85	1	36.463	1398.29	69.91	C
120.00-100.00			В	0.359	2.152		0.85	1	36.463			
			C	0.359	2.152		0.85	1	36.463			
T4	6925.11	4440.73	Α	0.307	2.277	5	0.85	1	39.240	1437.04	71.85	С
100.00-80.00			В	0.307	2.277		0.85	1	39,240			
			C	0.307	2.277		0.85	1)	39.240			
T5	6794.28	5602.28	Α	0.298	2.302	5	0.85	1	46.646	1404.51	70.23	C
80.00-60.00			В	0.298	2.302		0.85	1	46.646			
			C	0.298	2.302		0.85	1	46.646			- 13
T6	6625.91	6731.16	Α	0.29	2.324	4	0.85	1	53.452	1330.04	66.50	C
60,00-40.00			В	0.29	2.324		0.85	1	53.452			
			C	0.29	2.324		0.85	1	53.452			
T7	6384.37	7409.58	Α	0.235	2.484	4	0.85	1	48.886	1129.28	56.46	C
40.00-20.00			В	0,235	2.484		0.85	1	48.886		-	
			C	0,235	2.484		0.85	1	48.886			
T8 20.00-0.00	4810.91	7299.43	Α	0.211	2.558	4	0.85	1	49.656	976.71	48.84	C
			В	0,211	2,558		0.85	1	49.656			
			C	0.211	2.558		0.85	1	49,656			
Sum Weight:	43237.67	40069.74						OTM	673006.73	9027.82		
									lb-ft			

Tower Forces - Service - Wind Normal To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf						
ſŧ	lb	lb	e						ft ²	lb	plf	
T1	192.80	421.53	A	0.157	2.747	9	1	1	7.716	282.89	28.29	C
150.00-140.00	22.455		В	0.157	2.747	~	1	1	7.716			
			C	0.157	2.747		1	1	7.716			
T2	718.60	791.76	A	0.155	2.754	8	1	1	15.174	682.61	34.13	С

RISATower	Job	150' SS Tower Norwich, CT. Analysis	Page 21 of 34
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Section	Add	Self	F	e	C_F	q _z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			c			psf						
ft	lb	lb	e						ft²	lb	plf	
140 00-120 00			B	0.133	2.734		1	- 1	15.174			
			C	0.133	2.734		1	1	15.174			
£Ţ	1210.40	1140.72	Α	0.174	2.686	8	1	1	18.716	953.35	47,67	C
120.00-100.00			B	0.174	2.686		1	1	18.716			
			C	0.174	2.686		1	1.	18.716			
T4	1312.00	1485.02	Α	0.167	2,711	8	1	1	22,320	1012,72	30,64	C
100.08-00.001			B	0.167	2.711		1	1	22.320			
			C	0.167	2.711		1	Ĭ.	22.320			١.
T5	1312.00	1986.33	Α	0.174	2.686	7	1	1	28,506	1038.02	31.90	C
80,00-60,00			В	0.174	2.686		1	1	28,506			
			C	0.174	2.686		Į.	1	28,506	1000 5		_
Т6	1312.00	2680.91	A	0.179	2.666	6	1	ı,	32,950	1003,76	30,19	C
60,00-40,00		-	В	0.179	2.666		1	1	32.950			
		*****	C	0.179	2.666		1	1	32.950	040.00	40.40	
T7	1312.00	3829.48	Ā	0.154	2.757	5	1	1	31,934	868,33	43.42	C
40.00-20.00			В	0.154	2.757		Į.	1	31,934			
mn an an an	11/8/10	2010 (1	C	0,154	2,757		1	1	31.934	004.44	40.00	_
T8 20.00-0.00	1167,10	3948,61	A	0,142	2.8	5	1	1	33.820	805.77	40,29	C
			В	0.142	2,8		1	1	33,820			
	0.837.00	14004.55	С	0.142	2.8		10	0777.1	33.820			
Sum Weight:	8536.90	16284.35						OTM	482729.98	6647.46		
									lb-ft			

Tower Forces - Service - Wind 60 To Face

Section	Add	Self	F	e	C_F	q=	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			c			psf						
ft	lb	lb	е						ft ²	lb	plf	
T1	192.80	421.53	Α	0:157	2.747	9	0.8	1	6.717	262,82	26.28	C
150.00-140.00			В	0.157	2.747		0.8	1	6.717			
			C	0.157	2.747		0.8	1	6.717			
T2	718.60	791.76	Α	0.155	2.754	8	0.8	1	13.228	644.60	32.23	С
140.00-120.00			В	0.155	2.754		0.8	1	13.228			
			C	0.155	2.754		0.8	1	13.228			
T3	1210.40	1140.72	Α	0.174	2.686	8	0.8	1	16.592	914.77	45.74	С
120.00-100.00			В	0.174	2.686		0.8	1	16.592			
			C	0.174	2.686		0.8	1	16.592			
T4	1312.00	1485.02	Α	0.167	2.711	8	0.8	1	19.721	967.73	48.39	C
100.00-80.00			В	0.167	2.711		0.8	-1	19,721			
			C	0.167	2,711		0.8	1	19,721			
T5	1312.00	1986.33	Α	0.174	2.686	7	0.8	1	24,900	980.46	49.02	C
80.00-60.00		1	В	0.174	2.686		0.8	1	24.900			
			C	0,174	2.686		0.8	1	24,900			
T6	1312.00	2680.91	Α	0.179	2,666	6	0.8	1	28.861	944.92	47.25	C
60.00-40.00			В	0.179	2.666		0.8	1	28.861			
		1	C	0.179	2.666		0,8	1	28.861			
T7	1312.00	3829.48	Α	0.154	2.757	5	0.8	1	28.009	817,85	40,89	C
40.00-20.00			В	0.154	2.757		0.8	1	28,009			
l			C	0.154	2.757		0.8	1	28.009			_
T8 20.00-0.00	1167.10	3948.61	A	0.142	2.8	5	0.8	1	29.485	749.21	37.46	C
			В	0.142	2.8		0.8	1	29.485			
			C	0.142	2.8		0.8	1	29.485			
Sum Weight:	8536.90	16284.35						OTM	457534.16	6282.37		

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Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
	,,	.,	C			psf			62	,,	10	
JI	ID	lb	e						Jr .	ID	plf	
				·					lb-ft			

Tower Forces - Service - Wind 90 To Face

Section	Add	Self	F	e	C_F	q_z	D_F	D_R	A_E	F	w	Ctrl
Elevation	Weight	Weight	а									Fac
			с			psf						
ft	lb	lb	e						ft ²	lb	plf	
TI	192.80	421.53	Α	0.157	2.747	9	0.85	1	6.967	267.84	26.78	C
150.00-140,00			В	0.157	2,747		0.85	1	6.967			
			C	0.157	2,747		0.85	1	6.967			
T2	718.60	791.76	A	0.155	2.754	8	0.85	1	13.714	654.11	32.71	C
140.00-120.00			В	0.155	2.754		0.85	1	13,714			
			С	0,155	2.754		0,85	1	13.714			
T3	1210,40	1140.72	Α	0.174	2,686	8	0.85	1	17.123	924.41	46.22	C
120.00-100.00			В	0.174	2.686		0.85	1	17,123			
- 1			C	0.174	2.686		0.85	1	17,123			
T4	1312,00	1485.02	Α	0.167	2,711	8	0.85	1	20.371	978.98	48.95	C
100.00-80.00			В	0.167	2.711	.	0.85	1	20.371			
٠ .			C	0.167	2.711		0.85	1	20,371			
T5	1312.00	1986.33	Α	0.174	2.686	7	0.85	1	25.802	994.85	49.74	C
80.00-60.00			В	0.174	2,686		0.85	1	25.802			
			C	0.174	2.686		0.85	-1	25.802			
T6	1312.00	2680.91	Α	0.179	2.666	6	0.85	1	29.883	959.63	47.98	С
60.00-40.00			В	0.179	2.666		0.85	1	29.883			
			C	0.179	2.666		0.85	1	29.883			
T7	1312.00	3829.48	Α	0.154	2.757	5	0.85	1	28.990	830.47	41.52	C
40.00-20.00			В	0.154	2.757		0.85	1	28.990			
			C	0.154	2.757		0.85	1	28.990			
T8 20,00-0.00	1167.10	3948.61	Α	0.142	2.8	5	0.85	1	30.569	763,35	38.17	С
0	I		В	0,142	2.8		0.85	1	30.569			
	I		c	0.142	2.8		0.85	1	30.569			
Sum Weight:	8536.90	16284.35						OTM	463833.11	6373.64		
		2.1							lb-ft		3	

Force Totals

Load	Vertical	Sum of	Sum of	Sum of	Sum of	Sum of Torques
Case	Forces	Forces	Forces	Overturning	Overturning	
		X	Z	Moments, M _x	Moments, Mz	
	lb	lb	lb	lb-ft	lb-fl	lb-ft
Leg Weight	10129.62	THE RESERVE			Style All Lea	
Bracing Weight	6154.73		LA STATE			200 100 100 100 100
Total Member Self-Weight	16284.35			11535.77	10513.67	The Course
Total Weight	32268.85		- 1	11535.77	10513.67	Name of the Control o
Wind 0 deg - No Ice		81.10	-28129.64	-2501715.82	2203.54	-5253.55
Wind 30 deg - No Ice		13677.38	-23689.07	-2119990.80	-1219080.05	-1000.18
Wind 60 deg - No Ice	100000	24321.29	-14135.05	-1252286.81	-2160063.21	3536.94
Wind 90 deg - No Ice		27214.30	-81,10	3225.65	-2434280.20	6814.36

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Load	Vertical	Sum of	Sum of	Sum of	Sum of	Sum of Torques
Case	Forces	Forces	Forces	Overturning	Overturning	W 167
2		X	Z	Moments, M _x	Moments, M.	
	lb lb	lb	lb	lb-fi	lb-fi	lb-ft
Wind 120 deg - No Ice	The second second	24240.19	13994.59	1260964.79	-2151753.09	8790.48
Wind 150 deg - No lee		13336.92	23607.97	2134752.22	-1204686.48	7814.54
Wind 180 deg - No Ice	(Street of the	-81.10	28129.64	2524787.37	18823.79	5253.55
Wind 210 deg - No Ice		-13677.38	23689.07	2143062.35	1240107.38	1000,18
Wind 240 deg - No Ice	III A STATE OF THE PARTY OF	-24321.29	14135.05	1275358.35	2181090.55	-3536.94
Wind 270 deg - No Ice		-27214.30	81.10	19845.90	2455307.53	-6814.36
Wind 300 deg - No Ice		-24240.19	13994.59	-1237893.24	2172780.42	-8790.48
Wind 330 deg - No lee	The State of the S	-13536.92	-23607.97	-2111680.67	1225713.82	-7814.54
Member Ice	23785.39	THE PERSON NAMED IN		THE RESERVE	STREET, SQUARE, SQUARE	AND DESCRIPTION OF THE PERSON NAMED IN
Total Weight Ice	117112.05	THE RESERVE		68628.88	49693.32	
Wind 0 deg - Ice	Contract Con	2.86	-13087.04	-1126812,42	49576.96	-2242.56
Wind 30 deg - Ice	ALC: NO SECOND	6441.66	-11194.76	-957354.09	-539906,53	-1003.34
Wind 60 deg - Ice	771 34 17 17 17	11294.80	-6546.00	-529192,54	-980764.96	511.83
Wind 90 deg - Ice	THE PARTY	12878.36	-2.86	68512,52	-1129304.84	1837.24
Wind 120 deg - Ice		11291.94	6541.04	666248.76	-980648.60	2754.39
Wind 150 deg - Ice		6436.70	11191.89	1094495.49	-539704.99	2840.58
Wind 180 deg - Ice	- 1 - 12 - 1	-2.86	13087.04	1264070.18	49809.68	2242.56
Wind 210 deg - Ice	SHIRALA	-6441.66	11194.76	1094611.85	639293.17	1003.34
Wind 240 deg - Ice		-11294.80	6546.00	666450.30	1080151.60	-511.83
Wind 270 deg - Ice		-12878.36	2.86	68745,24	1228691.48	-1837.24
Wind 300 deg - Ice		-11291.94	-6541.04	-528991,00	1080035.25	-2754.39
Wind 330 deg - Ice	To the same of the	-6436,70	-11191.89	-957237.73	639091.63	-2840.58
Total Weight	32268.85			11535,77	10513.67	OT STATE
Wind 0 deg - Service		26.99	-9362,68	-837845.50	-674.03	-1748.59
Wind 30 deg - Service	Home of the Add	4552.38	-7884.68	-710792.06	-407166.35	-332.90
Wind 60 deg - Service	A STATE OF THE PARTY OF THE PAR	8095.10	-4704.71	-421985.25	-720363.41	1177.23
Wind 90 deg - Service	Bridge Long Park	9058.01	-26.99	-4100.19	-811633.86	2268.09
Wind 120 deg - Service	A 18 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	8068.11	4657,96	414526.00	-717597.47	2925.83
Wind 150 deg - Service	315 55 115	4505.63	7857.68	705357.62	-402375.59	2600.99
Wind 180 deg - Service		-26.99	9362.68	835177.01	4857.86	1748.59
Wind 210 deg - Service	100	-4552.38	7884.68	708123.56	411350.18	332.90
Wind 240 deg - Service	J. 1. 1. 1. 1	-8095.10	4704.71	419316.76	724547.24	-1177.23
Wind 270 deg - Service	Table 1	-9058.01	26.99	1431.70	815817.68	-2268.09
Wind 300 deg - Service	E AND THE STREET	-8068.11	-4657.96	-417194.50	721781.29	-2925.83
Wind 330 deg - Service	The House	-4505.63	-7857.68	-708026.11	406559.42	-2600.99

Load Combinations

Comb. No₌	Desc	ription
1	Dead Only	
2	1.2 Dead+1.6 Wind 0 deg - No Ice	
3	0.9 Dead+1.6 Wind 0 deg - No Ice	
4	1.2 Dead+1.6 Wind 30 deg - No Ice	
5	0.9 Dead+1.6 Wind 30 deg - No Ice	
6	1.2 Dead+1.6 Wind 60 deg - No Ice	
7	0.9 Dead+1.6 Wind 60 deg - No Ice	
8	1.2 Dead+1.6 Wind 90 deg - No Ice	
9	0.9 Dead+1.6 Wind 90 deg - No Ice	
10	1.2 Dead+1.6 Wind 120 deg - No Ice	
11	0.9 Dead+1.6 Wind 120 deg - No Ice	
12	1.2 Dead+1.6 Wind 150 deg - No Ice	
13	0.9 Dead+1.6 Wind 150 deg - No Ice	
14	1.2 Dead+1.6 Wind 180 deg - No Ice	
15	0.9 Dead+1.6 Wind 180 deg - No Ice	
16	1.2 Dead+1.6 Wind 210 deg - No Ice	

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Comb.	Description		
No.	0.0 D 114 (WE 1010 1 . No. 1		
17	0.9 Dead+1.6 Wind 210 deg - No Ice		
18	1.2 Dead+1.6 Wind 240 deg - No Ice		
19	0.9 Dead+1.6 Wind 240 deg - No lee		
20	1.2 Dead+1.6 Wind 270 deg - No loe		
21	0.9 Dead+1.6 Wind 270 deg - No Ice		
22	1.2 Dead+1.6 Wind 300 deg - No Ice		
23	0.9 Dead+1.6 Wind 300 deg - No Ice		
24	1.2 Dead+1.6 Wind 330 deg - No Ice		
23	0.9 Dead+1.6 Wind 330 deg - No Ice		
26	1.2 Dead+1.0 Ice+1.0 Temp		
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp		
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp		
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp		
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp		
31	1:2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp		
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp		
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp		
34	1,2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp		
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp		
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp		
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp		
38 39	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp		
40	Dead+Wind 0 deg - Service		
40	Dead+Wind 30 deg - Service Dead+Wind 60 deg - Service		
42	Dead+Wind 90 deg - Service Dead+Wind 90 deg - Service		
43	Dead+Wind 120 deg - Service		
43	Dead+Wind 150 deg - Service		
45	Dead+Wind 180 deg - Service		
46	Dead+Wind 210 deg - Service		
47	Dead+Wind 240 deg - Service		
48	Dead+Wind 270 deg - Service		
49	Dead+Wind 300 deg - Service		
50	Dead+Wind 330 deg - Service		

Maximum Reactions

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load	lb	lb	lb
		Comb.			
Leg C	Max. Vert	18	272806.66	23814.42	-13543,39
_	Max, H_x	18	272806.66	23814.42	-13543.39
	Max. Hz	5	-208750.54	-18078.01	12445.05
	Min. Vert	7	-240181.19	-21809.66	12390.38
	Min. H _x	7	-240181.19	-21809.66	12390.38
	Min. H _z	18	272806.66	23814.42	-13543.39
Leg B	Max. Vert	10	269916.79	-23819.74	-13214.11
· ·	Max. H _x	23	-239754.99	21844.03	12093.41
	Max: Hz	23	-239754.99	21844.03	12093.41
	Min. Vert	23	-239754.99	21844.03	12093.41
	Min. H _x	10	269916,79	-23819.74	-13214.11
	Min. Hz	10	269916.79	-23819.74	-13214.11
Leg A	Max. Vert	2	270928.95	-287.75	27361.05
J	Max. H _x	21	8156.43	2882.53	568.14
	Max. H _z	2	270928,95	-287.75	27361,05
	Min. Vert	15	-241871.06	274.43	-25113.60
	Min. H _x	9	9866.39	-2904.15	726.10
	Min. Hz	15	-241871.06	274.43	-25113.60

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Tower Mast Reaction Summary

Load Combination	Vertical	Shearx	Shear ₌	Overturning Moment, M _x	Overturning Moment, M _z	Torque
	lb	lb	lb	lb-ft	lb-fl	lb-ft
Dead Only 1.2 Dead+1.6 Wind 0 deg - No	32268.85 38722 62	0.00 129,76	-0.00 -45006,84	11538.53 -4022155.66	10517.42 -636.93	0,00 -8489.63
loo 0.9 Dead+1.6 Wind 0 deg - No	29041.96	129,76	-45007.00	-4021880.76	-3804.10	-8468.15
lce 1.2 Dead+1.6 Wind 30 deg - No	38722.62	21883,48	-37902.00	-3409204.07	-1961954.76	-1640.14
lce 0.9 Dead+1.6 Wind 30 deg - No	29041.96	21883.57	-37902.14	-3409488,89	-1963292,10	-1628,83
Ice 1,2 Dead+1.6 Wind 60 deg - No Ice	38722.62	37393.59	-21738.24	-1954940,59	-3367859,56	5664.59
0.9 Dead+1.6 Wind 60 deg - No Ice	29041.96	37393.73	-21738.32	-1956579.92	-3367881.18	5663.24
1.2 Dead+1.6 Wind 90 deg - No Ice	38722.62	43542,27	-129.73	570.54	-3913494.44	10952.72
0.9 Dead+1.6 Wind 90 deg - No Ice	29041,96	43542.43	-129.73	-2895.36	-3913010,93	10939.05
1.2 Dead+1.6 Wind 120 deg - No Ice	38722.62	38783.80	22391.04	2020385.87	-3459696.20	14155.39
0.9 Dead+1.6 Wind 120 deg - No Ice	29041.96	38783.94	22391,13	2015044.57	-3459653.46	14137,12
1.2 Dead+1.6 Wind 150 deg - No Ice	38722.62	21658.80	37772.21	3423641.99	-1938822.28	12606.46
0.9 Dead+1.6 Wind 150 deg - No Ice	29041.96	21658.87	37772.35	3416982.96	-1940179.67	12581.45
1.2 Dead+1.6 Wind 180 deg - No Ice	38722.62	-129.75	43251.73	3928437.07	26042.04	8487.72
0.9 Dead+1.6 Wind 180 deg - No Ice	29041.96	-129.75	43251.90	3921303.16	22852.73	8466.91
1.2 Dead+1.6 Wind 210 deg - No Ice	38722.62	-21883.53	37901.97	3436952.84	1987314.89	1639.90
0.9 Dead+1.6 Wind 210 deg - No Ice	29041.96	-21883.61	37902.11	3430283,25	1982296.65	1628,63
1,2 Dead+1,6 Wind 240 deg - No Ice	38722.62	-38913,55	22615,79	2043459.37	3498392.41	-5665.71
0.9 Dead+1.6 Wind 240 deg - No Ice	29041.96	-38913.69	22615.87	2038097.24	3491975.43	-5664.24
1.2 Dead+1.6 Wind 270 deg - No Ice	38722.62	-43542.27	129.78	27247.46	3938834.73	-10953.22
0.9 Dead+1.6 Wind 270 deg - No Ice	29041,96 38722.62	-43542 ₄ 43 -37263.83	129.78 -21513.50	23760.30 -1931811.47	3931994,99 3379876,25	-10939.61 -14152.27
2 Dead+1.6 Wind 300 deg - No Ice	29041.96	-37263.83	-21513.58	-1931811.47		
9.9 Dead+1,6 Wind 300 deg - No Ice 1.2 Dead+1.6 Wind 330 deg -	38722.62	-3/263.97	-21313.38 -37772.24	-3395839.86	3373552.66 1964236.09	-14130.12 -12605.63
No Ice 0.9 Dead+1.6 Wind 330 deg -	29041.96	-21658.82	-37772.38	-3395135.41	1959236.29	-12580,63
No Ice .2 Dead+1.0 Ice+1.0 Temp	123565.82	0,05	-0.04	71555.05	52562.00	-0.52
.2 Dead+1.0 Wind 0 deg+1.0 ce+1.0 Temp	123565.81	2.91	-13086.22	-1140780.24	52557.63	-2347.34
.2 Dead+1.0 Wind 30 deg+1.0 ce+1.0 Temp	123565.81	6441.27	-11194.05	-968936.64	-545332.92	-1048.92
.2 Dead+1.0 Wind 60 deg+1.0	123565.81	11106.92	-6437.53	-527366.90	-979835.09	536,63

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Load	Vertical	$Shear_x$	Shear-	Overturning	Overturning	Torque
Combination	22			Moment, M_x	Moment, M ₋	11.6
	lb .	lb	Ib	lb-fi	lb-ft	lb-ft
Ico+1 0 Temp						
1.2 Dead+1.0 Wind 90 deg+1.0	123363 81	12877.55	-2.89	71362.89	-1143141.23	1923 68
lco+1 0 Temp						
1.2 Dead+1.0 Wind 120	123363.81	11291.25	6340.37	677809.31	-992339.87	2884.20
dog+1.0 lco+1.0 Tomp						
1.2 Dead+1.0 Wind 130	123563.81	6436.33	11191.10	1112177.96	-543136.31	2974.41
deg+1.0 lce+1.0 Temp						
1.2 Dead+1.0 Wind 180	123565.81	-2.81	12869.99	1269566.85	52783.45	2346.08
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 210	123565.81	-6441.18	11193.96	1112291.42	650671.07	1048.77
deg+1.0 Ice+1.0 Temp						
.2 Dead+1.0 Wind 240	123565,81	-11294.01	6545.53	678007,62	1097789.08	-536,90
deg+1.0 lce+1.0 Temp						
1.2 Dead+1.0 Wind 270	123565.81	-12877.45	2.83	71794.94	1248475.94	-1925.60
deg+1.0 Ice+1.0 Temp						
.2 Dead+1.0 Wind 300	123565.81	-11103.96	-6432.58	-527163.23	1085058.72	-2882.70
leg+1.0 lce+1.0 Temp						
1.2 Dead+1.0 Wind 330	123565.81	-6436.21	-11191.19	-968817.91	650476.75	-2974.4
leg+1.0 Ice+1.0 Temp						
Dead+Wind 0 deg - Service	32268.85	26,99	-9362.57	-827535.78	7779.43	-1763.23
Dead+Wind 30 deg - Service	32268.85	4552.33	-7884.58	-700098.57	-399975.90	-343.51
Dead+Wind 60 deg - Service	32268.85	7778.83	-4522.11	-397758.11	-692260.69	1178.10
Dead+Wind 90 deg - Service	32268.85	9057.91	-26.99	8790.73	-805700.09	2280.6
Dead+Wind 120 deg - Service	32268.85	8068.02	4657,91	428710.81	-711362,03	2941.5
Dead+Wind 150 deg - Service	32268.85	4505.58	7857.59	720450.03	-395170.81	2615.39
Dead+Wind 180 deg - Service	32268.85	-26,99	8997.47	825399.81	13326.67	1762.84
Dead+Wind 210 deg - Service	32268.85	-4552.33	7884.58	723222.20	421080.26	343.4
Dead+Wind 240 deg - Service	32268.85	-8095,01	4704.66	433513.08	735239.69	-1178.40
Dead+Wind 270 deg - Service	32268.85	-9057.90	26.99	14336.68	826803.83	-2280.54
Dead+Wind 300 deg - Service	32268.85	-77 51.83	-4475.36	-392953,56	710591.30	-2941,01
Dead+Wind 330 deg - Service	32268,85	-4505,57	-7857.59	-697324.15	416277.24	-2615.35

Solution Summary

	Sui	n of Applied Forces	5		Sum of Reaction	ıs	
Load	PX	PY	PZ	PX	PY	PZ	% Erro
Comb.	lb	lb	lb	lb	lb	lb	
1	0.00	-32268.85	-0.00	-0.00	32268.85	0.00	0.000%
2	129.76	-38722.62	-45007.43	-129.76	38722.62	45006.84	0.0019
3	129.76	-29041.97	-45007.43	-129.76	29041.96	45007.00	0.0019
4	21883.81	-38722.62	-37902.51	-21883.48	38722.62	37902.00	0.0019
5	21883.81	-29041.97	-37902.51	-21883.57	29041.96	37902,14	0.001%
6	37394.13	-38722.62	-21738.56	-37393.59	38722.62	21738.24	0.0019
7	37394.13	-29041.97	-21738.56	-37393.73	29041.96	21738.32	0.0019
8	43542.88	-38722.62	-129.76	-43542.27	38722.62	129.73	0.0019
9	43542.88	-29041.97	-129.76	-43542.43	29041.96	129.73	0.0019
10	38784.30	-38722.62	22391.34	-38783.80	38722.62	-22391.04	0.0019
11	38784.30	-29041.97	22391.34	-38783.94	29041.96	-22391:13	0.0019
12	21659.07	-38722.62	37772.75	-21658.80	38722.62	-37772:21	0.0019
13	21659.07	-29041.97	37772.75	-21658.87	29041.96	-37772.35	0.0019
14	-129.76	-38722.62	43252.37	129.75	38722.62	-43251.73	0.0019
15	-129.76	-29041.97	43252.37	129.75	29041.96	-43251.90	0.0019
16	-21883.81	-38722.62	37902.51	21883.53	38722 62	-37901.97	0.0019
17	-21883.81	-29041.97	37902.51	21883.61	29041.96	-37902.11	0,0019
18	-38914.06	-38722.62	22616.09	38913.55	38722.62	-22615.79	0.0019
19	-38914.06	-29041.97	22616.09	38913.69	29041.96	-22615.87	0.0019
20	-43542.88	-38722.62	129.76	43542.27	38722.62	-129.78	0.0019
21	-43542.88	-29041.97	129.76	43542.43	29041.96	-129.78	0.0019

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	Su	m of Applied Forces	5		Sum of Reaction	S	
Load	PX	PY	PZ	PX	$\dot{P}Y$	PZ	% Erro
Comb.	lb	lb	1b	lb	lb	lb	
22	-37264.38	-38722.62	-21513.81	37263.83	38722.62	21313.50	0.0019
23	-37264.38	-29041.97	-21313 81	37263.97	29041.96	21313.38	0.0019
24	-21639.07	-38722.62	-37772.73	21638.73	38722 62	37772.24	0.0019
23	-21639.07	-29041.97	-37772.75	21638.82	29041.96	37772.38	0.0019
26	0,00	-123363.82	-0.00	-0.03	123363.82	0.04	0.0009
27	2.86	-123563.82	-13087.04	-2.91	123363.81	13086.22	0.0019
28	6441.66	-123563.82	-11194.76	-6441.27	123363.81	11194.03	0.001%
29	11107.62	-123563.82	-6437.93	-11106.92	123365.81	6437.53	0.0019
30	12878.36	-123565.82	-2.86	-12877.55	123565.81	2.89	0.0019
31	11291,94	-123565.82	6541.04	-11291.25	123365.81	-6540.37	0.0019
32	6436.70	-123565.82	11191.89	-6436.33	123565.81	-11191.10	0.0019
33	-2.86	-123565.82	12870.90	2.81	123565.81	-12869.99	0.001%
34	-6441.66	-123565,82	11194.76	6441.18	123565.81	-11193.96	0.0019
35	-11294.80	-123565.82	6546.00	11294,01	123565.81	-6545.53	0,0019
36	-12878.36	-123565.82	2,86	12877.45	123565,81	-2.83	0.0019
37	-11104.76	-123565.82	-6432.97	11103.96	123565.81	6432.58	0.001%
38	-6436.70	-123565.82	-11191.89	6436.21	123565,81	11191.19	0.0019
39	26,99	-32268.85	-9362.68	-26,99	32268,85	9362.57	0.0009
40	4552.38	-32268,85	-7884.68	-4552,33	32268.85	7884.58	0.0009
41	7778.92	-32268,85	-4522.17	-7778.83	32268.85	4522.11	0,000%
42	9058.01	-32268.85	-26,99	-9057.91	32268.85	26.99	0.000%
43	8068.11	-32268,85	4657.96	-8068.02	32268.85	4 657.91	0.000%
44	4505,63	-32268,85	7857.68	-4505,58	32268.85	-7857,59	0.000%
45	-26.99	-32268,85	8997.58	26,99	32268.85	-8997,47	0.000%
46	-4552.38	-32268.85	7884.68	4552.33	32268.85	-7884.58	0.000%
47	-8095.10	-32268.85	4704.71	8095.01	32268.85	-4704.66	0.000%
48	-9058.01	-32268.85	26.99	9057,90	32268.85	-26.99	0.000%
49	-7751.93	-32268.85	-4475.41	7751.83	32268.85	4475.36	0.000%
50	-4505.63	-32268,85	-7857.68	4505.57	32268.85	7857.59	0.000%

Non-Linear Convergence Results

Load	Converged?	Number	Displacement	Force
Combination		of Cycles	Tolerance	Tolerance
1	Yes	4	0.00000001	0,00000001
2	Yes	4	0.00000001	0,00011615
3	Yes	4	0.00000001	0.00008419
4	Yes	4	0.00000001	0.00012001
5	Yes	4	0.00000001	0.00008794
6	Yes	4	0.00000001	0.00012364
7	Yes	4	0.00000001	0.00009145
8	Yes	4	0.00000001	0.00012012
9	Yes	4	0.00000001	0.00008807
10	Yes	4	0.00000001	0.00011616
11	Yes	4	0.00000001	0.00008424
12	Yes	4	0.00000001	0.00012024
13	Yes	4	0.00000001	0.00008816
14	Yes	4	0.0000001	0.00012372
15	Yes	4	0.00000001	0.00009151
16	Yes	4	0.00000001	0.00011994
17	Yes	4	0.00000001	0.00008790
18	Yes	4	0.00000001	0.00011593
19	Yes	4	0.00000001	0.00008403
20	Yes	4	0.00000001	0.00012009
21	Yes	4	0.00000001	0.00008800
22	Yes	4	0.00000001	0.00012389

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23	Yes	4	0.00000001	0.00009161
24	Yes	4	0.00000001	0.00012028
23	Yes	4	0.00000001	0.00008813
26	Yes	4	0.00000001	0.00003100
27	Yes	4	0.00000001	0.00044825
28	Yes	4	0.00000001	0.00044169
29	Yes	4	0.00000001	0.00043902
30	Yes	4	0.00000001	0.00043946
31	Yes	4	0.00000001	0.00044497
32	Yes	4	0.00000001	0.00045274
33	Yes	4	0.00000001	0.00046003
34	Yes	4	0.00000001	0.00046330
33	Yes	4	0.00000001	0.00046461
36	Yes	4	0.00000001	0.00046426
37	Yes	4	0.00000001	0.00046210
38	Yes	4	0.00000001	0.00045576
39	Yes	4	0.00000001	0.00009718
40	Yes	4	0.00000001	0,00009770
41	Yes	4	0.00000001	0.00009824
42	Yes	4	0.00000001	0.00009745
43	Yes	4	0.0000001	0.00009676
44	Yes	4	0,0000001	0.00009770
45	Yes	4	0,00000001	0.00009849
46	Yes	4	0.00000001	0.00009785
47	Yes	4	0,0000001	0.00009718
48	Yes	4	0,0000001	0.00009809
49	Yes	4	0,0000001	0,00009893
50	Yes	4	0.0000001	0.00009819

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
T1	150 - 140	3.104	47	0.1977	0.0184
T2	140 - 120	2.672	47	0.1959	0.0175
T3	120 - 100	1.854	47	0.1649	0.0145
T4	100 - 80	1.194	47	0.1256	0.0105
T5	80 - 60	0.712	47	0.0870	0.0075
Т6	60 - 40	0.387	47	0.0539	0.0051
T7	40 - 20	0.177	47	0.0311	0.0028
Т8	20 - 0	0.056	47	0.0156	0.0014

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	o	0	fi
150.00	Nudd 12' boom	47	3.104	0.1977	0.0184	313572
140.00	Nudd 12' boom	47	2.672	0.1959	0.0175	139521
127.50	Nudd 12' boom	47	2.147	0.1794	0.0158	37588
122.28	RFS DB-T1-6Z-8AB-0Z	47	1.941	0.1695	0.0149	28688
117.07	RFS DB-T1-6Z-8AB-0Z	47	1.745	0.1591	0.0140	25998
115.00	Nudd 10' boom	47	1.672	0.1551	0.0135	26042
111.85	RFS DB-T1-6Z-8AB-0Z	47	1,563	0.1489	0.0129	26213
106.64	RFS DB-T1-6Z-8AB-0Z	47	1.393	0.1387	0.0118	26502

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Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
	••	Load				Curvature
ft		Comb.	in	o	٥	ft
101.42	RFS DB-T1-6Z-8AB-0Z	47	1.233	0.1284	0.0108	26813
96.20	RFS DB-T1-6Z-8AB-0Z	47	1.089	0.1182	0.0098	27301
90.99	RFS DB-T1-6Z-8AB-0Z	47	0.936	0.1079	0.0090	27902
85.77	RFS DB-T1-6Z-8AB-0Z	47	0.834	0.0979	0.0083	28330
80.56	RFS DB-T1-6Z-8AB-0Z	47	0.723	0.0880	0.0076	29402
75.34	RFS DB-T1-6Z-8AB-0Z	47	0.624	0.0786	0.0070	31765
70.13	RFS DB-T1-6Z-8AB-0Z	47	0.334	0.0696	0.0064	35133
64.91	RFS DB-T1-6Z-8AB-0Z	47	0.455	0.0611	0.0037	39300
39.69	RFS DB-T1-6Z-8AB-0Z	47	0.383	0.0534	0.0031	43847
34.48	RFS DB-T1-6Z-8AB-0Z	47	0.319	0.0463	0.0045	46480
49.26	RFS DB-T1-6Z-8AB-0Z	47	0.262	0.0404	0.0038	48816
44.05	RFS DB-T1-6Z-8AB-0Z	47	0.211	0.0349	0.0032	51399
38.83	RFS DB-T1-6Z-8AB-0Z	47	0.167	0.0300	0.0027	54138
33.61	RFS DB-T1-6Z-8AB-0Z	47	0.129	0.0257	0.0022	56870
28.40	RFS DB-T1-6Z-8AB-0Z	47	0.097	0.0218	0.0019	59853
23.18	RFS DB-T1-6Z-8AB-0Z	47	0.070	0.0179	0.0016	63262
17.97	RFS DB-T1-6Z-8AB-0Z	47	0.049	0.0141	0.0012	73329
12.75	RFS DB-T1-6Z-8AB-0Z	47	0.031	0.0101	0.0009	102547

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	(0)
T1	150 - 140	14.715	18	0.9338	0.0886
T2	140 - 120	12.671	18	0,9252	0.0842
Т3	120 - 100	8.799	18	0.7794	0.0700
T4	100 - 80	5.674	18	0.5945	0.0506
T5	80 - 60	3.387	18	0.4120	0.0363
Т6	60 - 40	1.844	18	0,2552	0.0248
T7	40 - 20	0.844	18	0.1472	0.0134
T8	20 - 0	0,271	18	0.0740	0.0065

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	o	ft
150.00	Nudd 12' boom	18	14,715	0.9338	0.0886	70033
140.00	Nudd 12' boom	18	12.671	0.9252	0.0842	30757
127.50	Nudd 12' boom	18	10.188	0.8476	0.0760	8004
122.28	RFS DB-T1-6Z-8AB-0Z	18	9.210	0.8007	0.0719	6093
117.07	RFS DB-T1-6Z-8AB-0Z	18	8.287	0.7522	0.0673	5517
115.00	Nudd 10' boom	18	7.937	0.7330	0.0652	5526
111.85	RFS DB-T1-6Z-8AB-0Z	18	7.422	0.7039	0.0620	5562
106.64	RFS DB-T1-6Z-8AB-0Z	18	6.616	0.6558	0.0568	5623
101.42	RFS DB-T1-6Z-8AB-0Z	18	5.868	0.6076	0.0519	5689
96.20	RFS DB-T1-6Z-8AB-0Z	18	5.178	0.5593	0.0475	5787
90.99	RFS DB-T1-6Z-8AB-0Z	18	4.544	0.5110	0.0435	5906
85.77	RFS DB-T1-6Z-8AB-0Z	18	3.965	0.4634	0.0399	6029
80.56	RFS DB-T1-6Z-8AB-0Z	18	3.440	0.4169	0.0367	6205
75.34	RFS DB-T1-6Z-8AB-0Z	18	2.968	0.3720	0.0336	6705

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Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load	in	0	0	Curvature
ft		Comb.				ft
70.13	RFS DB-T1-6Z-8AB-0Z	18	2 343	0.3294	0.0306	7422
64.91	RFS DB-T1-6Z-8AB-0Z	18	2.163	0.2896	0.0277	8310
39.69	RFS DB-T1-6Z-8AB-0Z	18	1.823	0.2331	0.0246	9277
54.48	RFS DB-T1-6Z-8AB-0Z	18	1.321	0.2204	0.0215	9821
49.26	RFS DB-T1-6Z-8AB-0Z	18	1.249	0.1912	0.0183	10294
44.05	RFS DB-T1-6Z-8AB-0Z	18	1,009	0.1653	0.0154	10813
38.83	RFS DB-T1-6Z-8AB-0Z	18	0.799	0.1423	0.0128	11379
33.61	RFS DB-T1-6Z-8AB-0Z	18	0.618	0.1219	0.0108	11971
28.40	RFS DB-T1-6Z-8AB-0Z	18	0.463	0.1031	0.0090	12624
23.18	RFS DB-T1-6Z-8AB-0Z	18	0.337	0.0851	0.0075	13373
17.97	RFS DB-T1-6Z-8AB-0Z	18	0.233	0.0668	0.0059	15523
12.75	RFS DB-T1-6Z-8AB-0Z	18	0.151	0.0479	0,0043	21710

Bo	lt.	Des	ian	Data
-				Dutu

Section	Elevation	Component	Bolt Grade	Bolt Size	Number Of	Maximum Load per	Allowable Load	Ratio Load		Allowable Ratio	Criteria
No	fl	Туре	Graue	in	Bolts	Bolt Ib	lb	Allowa		2.0.110	
T 1	150	Leg	A325N	0.7500	4	1236.07	29820.60	0.041	1	1	Bolt Tension
		Diagonal	A325N	0.5000	1	1705.87	7952.16	0.215	1	1	Bolt Shear
T2	140	Leg	A325N	1.0000	4	9945.66	53014.40	0.188	1	1	Bolt Tension
		Diagonal	A325N	0.6250	1	6334.85	9787.50	0.647	/	1	Member Bearing
Т3	120	Leg	A325N	1.0000	6	13755.70	53014.40		1	1	Bolt Tension
		Diagonal	A325N	0.6250	1	6634.20	9787.50		1	1	Member Bearin
T4	100	Leg	A325N	1,0000	8	15082.30	53014.40	0.284	1	1	Bolt Tension
		Diagonal	A325N	0.6250	1	7175.78	9787.50	0.733	/	1	Member Bearing
T5	80	Leg	A325N	1.2500	8	19322.10	82835.00	0.233	1	1	Bolt Tension
		Diagonal	A325N	0.6250	1	7417.88	9787.50	0.758	1	1	Member Bearin
Т6	60	Leg	A325N	1,2500	8	23279.90	82835.00	0.281	1	1	Bolt Tension
		Diagonal	A325N	0.6250	1	8102.48	9787.50		1	1	Member Bearin
T7	40	Leg	A325N	1.2500	8	26947.60	82835.00	0.020	1	1	Bolt Tension
		Diagonal	A325N	0.7500	1	8988.72	14355.00	0.626	1	1	Member Bearin
Т8	20	Leg	F1554-36	1.5000	8	30369.50	57653.10		1	1	Bolt Tension
		Diagonal	A325N	0.7500	1	9429.93	14355.00	0.657	/	1	Member Bearin

Compression Checks

Leg Design Data (Compression)

RISATower	Job	150' SS Tower Norwich, CT. Analysis	91 of 34
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Section No.	Elevation	Size	L	L_{u}	Kl/r	Α	P_u		Ratio Pu
	ft		ft	ft		in ²	lb	<i>lb</i>	bP_n
Tl	150 - 140	P2.5x 203	10.00	0.67	8.4 K=1.00	1.7040	-6345 62	82351.90	0.077
T2	140 - 120	P2.5x.203	20.00	4.67	59.1 K=1.00	1.7040	-39142.50	62849.30	0.623
T3	120 - 100	P4x.237	20.03	4.67	37.2 K=1.00	3,1741	-86765 70	138323.00	0.627
T4	100 - 80	P5x.258	20,03	6,26	40.0 K=1.00	4 2999	-133951.00	184163.00	0.727
T5	80 - 60	P6x 28	20,03	6.26	33.5 K=1.00	5.5813	-171372.00	248307.00	0,690
Т6	60 - 40	P8x.322	20.03	6.26	25,6 K≖1,00	8.3993	-207170,00	387660.00	0.534
T7	40 - 20	P8x.5	20.03	9.35	39.0 K=1.00	12.7627	-232686,00	550137.00	0.423 1
T8	20 - 0	P8x.5	20,03	9,35	39.0 K≖1.00	12.7627	-264972.00	550137.00	0.482 1

 $^{{}^{1}}P_{u}$ / ϕP_{n} controls

Diagonal Design Data (Compression)

Section No.	Elevation	Size	L	L_{u}	Kl/r	À	P_u	ϕP_n	Ratio P _u
	ft		ft	ft		in ²	lb	lb	φP _n
TI	150 - 140	L1 1/2x1 1/2x3/16	7.40	3.42	139.8 K=1.00	0,5273	-1644.62	6096,07	0.270
T2	140 - 120	L2x2x3/16	7.60	3,51	110.3 K=1.03	0.7150	-6285.55	12215.70	0.515 1
T3	120 - 100	L2x2x3/16	9.00	4.28	130.5 K=1.00	0.7150	-6585.27	9453.47	0.697 1
T4	100 - 80	L2 1/2x2 1/2x3/16	11.48	5,51	133.7 K=1.00	0.9020	-7097.17	11410.90	0,622 1
T5	80 - 60	L3x3x3/16	13.20	6.33	127.4 K=1.00	1.0900	-7487.52	14941.00	0.501 1
Т6	60 - 40	L3x3x3/16	14.99	7.14	143.7 K=1.00	1.0900	-8214.75	11926.10	0.689 1
T7	40 - 20	L3 1/2x3 1/2x1/4	18.07	8.74	151.1 K=1.00	1.6900	-9098.83	16715.90	0.544 1
Т8	20 - 0	L3 1/2x3 1/2x1/4	19.81	9.61	166.2 K=1.00	1.6900	-9604.27	13815.30	0.695

 $^{^{1}}$ P_{u} / ϕP_{n} controls

Top Girt Design Data (Compression)

RISATower	Job	150' SS Tower Norwich, CT. Analysis	32 of 34
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Section No.	Elevation	Size	L	L_{u}	KI/r	Α	P_u		Ratio P.,
	ft		ft	ft		in ²	lb	lb	$\bullet P_n$
TI	150 - 140	L3x3x1/4	6.00	3.76	118.4 K=1.01	1.4400	-391.71	22309.60	0.018

 $^{{}^{\}dagger}P_{u}$ / ${}^{\dagger}P_{n}$ controls

Tension Checks

_			Leg Des	sign L	vata (lensio	on)		
Section No.	Elevation	Size	L	L_{u}	KI/r	A	P_u	ϕP_n	Ratio P _u
	ſŧ		fl	ft		in ²	lb	lb	ϕP_n
TI	150 - 140	P2.5x.203	10.00	0.67	8.4	1.7040	4944.28	82816.80	0.060
T2	140 - 120	P2.5x.203	20,00	0.67	8.4	1.7040	39782.60	82816,80	0.480 1
Т3	120 - 100	P4x.237	20.03	0.67	5.3	3.1741	82534.50	154259,00	0.535 1
T4	100 - 80	P5x.258	20.03	0.63	4.0	4.2999	120658.00	208974.00	0.577 1
T5	80 - 60	P6x.28	20.03	0.63	3.3	5,5813	154577.00	271254.00	0.570 1
Т6	60 - 40	P8x.322	20.03	0.63	2.6	8.3993	186239.00	408204.00	0.456
T 7	40 - 20	P8x.5	20.03	0.67	2.8	12.7627	215580.00	620268.00	0.348
T8	20 - 0	P8x.5	20.03	0.67	2.8	12.7627	242956.00	620268.00	0,392 1

 $^{^{1}}$ P_{u} / ϕP_{n} controls

	Diagonal Design Data (Tension)										
Section No.	Elevation	Size	L	L_{θ}	Kl/r	A	P_u	ϕP_n	Ratio P _u		
	ft		fl	ft		in ²	lb	lb	ϕP_n		
TI	150 - 140	L1 1/2x1 1/2x3/16	7,40	3,42	93.4	0.3076	1705.87	13381.30	0.127		
T2	140 - 120	L2x2x3/16	7.60	3,51	71.0	0.4308	6334.85	18739.00	0.338 1		
Т3	120 - 100	L2x2x3/16	9.00	4.28	86.0	0.4308	6634.20	18739.00	0.354		
T4	100 - 80	L2 1/2x2 1/2x3/16	10.45	5.01	79.3	0.5710	7175,78	24839.90	0.289		
T5	80 - 60	L3x3x3/16	12.11	5.79	75.7	0.7120	7417.88	30973.40	0.239 1		

RISATower	Job	150' SS Tower Norwich, CT. Analysis	33 of 34
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Section No.	Elevation	Size	L	L_u	Kl/r	A	P_u		Ratio P _u
	fi		fî	ft		in ²	lb	1b	P_n
Т6	60 - 40	L3x3x3/16	14.99	7.14	92,9	0.7120	8102.48	30973.40	0.262
T7	40 - 20	1.3 1/2x3 1/2x1/4	18.07	8.74	97.8	1.1034	8988.72	47999.30	0.187
Т8	20 - 0	L3 1/2x3 1/2x1/4	19.81	9.61	107.4	1.1034	9429.93	47999.50	0.196 1

 $^{{}^{1}}P_{u}$ / ${}_{\bullet}P_{n}$ controls

		То	p Girt [)esigı	n Data	a (Tens	sion)		
Section No.	Elevation	Size	L	L_{μ}	Kl/r	A	P_u	ϕP_n	Ratio Pu
	ft		fi	fi		in ²	lb	lb	ΦP_n
TI	150 - 140	L3x3x1/4	6.00	5,76	74.3	1.4400	363.47	46656.00	0.0081

 $^{^{1}}P_{u}$ / ϕP_{n} controls

Section Capacity Table

Section	Elevation	Component	Size	Critical	P	θP_{allow}	%	Pass
No.	ft	Туре		Element	lb	lb	Capacity	Fail
Tl	150 - 140	Leg	P2.5x.203	1	-6345.62	82351.90	7.7	Pass
		Diagonal	L1 1/2x1 1/2x3/16	8	-1644.62	6096.07	27.0	Pass
		Top Girt	L3x3x1/4	4	-391.71	22309.60	1.8	Pass
T2	140 - 120	Leg	P2.5x.203	19	-39142,50	62849.30	62.3	Pass
		Diagonal	L2x2x3/16	23	-6285.55	12215.70	51.5	Pass
							64.7 (b)	
T3	120 - 100	Leg	P4x.237	46	-86765.70	138323.00	62.7	Pass
		Diagonal	L2x2x3/16	53	-6585.27	9453.47	69.7	Pass
T4	100 - 80	Leg	P5x.258	73	-133951.00	184163.00	72.7	Pass
		Diagonal	L2 1/2x2 1/2x3/16	77	-7097.17	11410.90	62.2	Pass
							73.3 (b)	
T5	80 - 60	Leg	P6x.28	94	-171372.00	248307.00	69.0	Pass
		Diagonal	L3x3x3/16	98	-7487,52	14941.00	50.1	Pass
		_					75.8 (b)	
T6	60 - 40	Leg	P8x.322	115	-207170.00	387660.00	53.4	Pass
		Diagonal	L3x3x3/16	119	-8214.75	11926.10	68.9	Pass
		_					82.8 (b)	
T7	40 - 20	Leg	P8x.5	136	-232686.00	550137.00	42.3	Pass
		Diagonal	L3 1/2x3 1/2x1/4	140	-9098.83	16715.90	54.4	Pass
		_					62.6 (b)	
T8	20 - 0	Leg	P8x.5	151	-264972.00	550137.00	48.2	Pass
		_					52.7 (b)	
		Diagonal	L3 1/2x3 1/2x1/4	155	-9604.27	13815.30	69.5	Pass
		-					Summary	
						Leg (T4)	72.7	Pass
						Diagonal	82.8	Pass

RISATower	Job 1	50' SS Tower Norwich, CT. Analysis	Page 34 of 34
	Project	117-23243.6	Date 23:47:29 01/14/18
Phone: FAX:	Client	CDT	Designed by FAN

Section No.	Elevation ft	Component Type	Size	Critical Element	P Ib	øP _{allow} Ib	% Capacity	Pass Fail
		-				(T6) Top Girt (T1)	1.8	Pass
						Balt Checks	82.8	Pass
						RATING =	82.8	Pass

Site Name: Client:

CDT 117-23243.6

Project Number:

1/14/2018

Norwich, CT

Date:

Design Base Loads (Factored) - Design per TIA-222-G Standard

Moment (Overturning) (Mu): 0.0 k-ft
Shear/Leg (Vu): 27.4 k
Compression/Leg (Pu): 272.8 k
Uplift/Leg (Tu): 241.9 k

Diameter of Caisson (d): Length of Caisson (i):

Caisson Height Above Ground (h):

Depth Below Ground Surface to Water Table (w):

Unit Weight of Concrete: Unit Weight of Soil: Unit Weight of Water:

Ultimate Compressive Bearing Pressure:

Pullout Angle: Rod Diameter:

Rod Ultimate Strength:

Rod Net Area: Number of Rods:

Diameter of Cored Hole:

Ultimate Grout / Rock Interface Bond Strength:

Rod Embedment Length:

Rod Exposure Above Lock Off Nut in Foundation:

Rod Embedment Circle: Free Stress Length: Volume of Concrete:

Weight of Concrete (Buoyancy Effect Considered):

Compressive Bearing Resistance:

Pullout Weight: Rod Bond Strength: Williams Rod Strength: Maximum Lock Off Load:

Nominal Uplift Capacity per Leg ($\phi_s T_n$):

Nominal Compressive Capacity per Leg ($\phi_s P_n$):

T_u: P_u:

 T_u/ϕ_sT_n : P_u/ϕ_sP_n : 3.5 ft 4.3 ft

0.5 ft

100.0 ft 150.0 pcf

135.0 pcf 62.4 pcf

60000 psf

30.0 degrees

150 ksi 0.85 in²

5 3.00 in

250 psi 228 in

60 in 26 in

150 in

6.3 k 577.3 k

2428.7 k 918.9 k

637.5 k

95.6 k > Design Lock Off Load, OK

478.1 k 433.0 k 241.9 k 273.5 k

> 0.51 Result: OK 0.63 Result: OK

Lateral Capacity

Dep	oth (ft)	Ultimate Lateral	Increment	Ysoil	Cohesian	•
Top	Bottom	Bearing Pressure (psf)	(psf/ft)	(pef)	(psf)	(degree)
0.0	0.5	0.0	100.0	100	0	0
0.5	1.0	47.9	100.0	100	0	0
1.0	1.5	100.0	100.0	100	0	0
1.5	2,5	41636.6	567.5	135	10000	38
2,5	3,0	42204.1	567.5	135	10000	38
3.0	3.5	42274.6	567.5	135	10000	38
3.5	3.9	42327.6	567.5	135	10000	38
3.9	3,8	42389.4	567.5	135	10000	38

Total Lateral Resistance:

Inflection Point (Below Ground Surface):

Design Overturning Moment At Inflection Point (Muin):

Nominal Moment Capacity per Leg (\$ Mn):

 $M_{uip}/\phi_s M_n$:

Caisson Strength Canacity

Concrete Compressive Strength (f'c):

Vertical Steel Rebar Size #:

Vertical Steel Rebar Area:

of Vertical Steel Rebars:

Vertical Steel Rebar Yield Strength (F_v):

Horizontal Tie / Stirrup Size #:

Horizontal Tie / Stirrup Area:

Horizontal Tie / Stirrup Spacing:

Horizontal Tie / Stirrup Steel Yield Strength (F_v):

Rebar Cage Diameter:

Strength Bending/Tension Reduction Factor (ϕ_B):

Strength Shear Reduction Factor (ϕ_V):

Strength Compression/Bearing Reduction Factor $(\phi_{P/B})$:

Steel Elastic Modulus:

Design Moment (M_u):

Nominal Moment Capacity ($\phi_B M_n$):

 M_u/ϕ_BM_n :

Design Shear (V_u):

Nominal Shear Capacity ($\phi_V V_n$):

 V_u/ϕ_vV_n :

Design Tension (T_u):

Nominal Tension Capacity $(\phi_T T_n)$:

 $T_u/\phi_T T_n$:

Design Compression (P_u):

Nominal Compression Capacity ($\phi_P P_n$):

 $P_u/\phi_P P_n$:

Bending Reinforcement Ratio:

3000 psi

461.1 k

3.8 ft

118.7 k-ft

310.4 k-ft

0.38 Result: OK

6

0.44 in²

23 Minimum # of vertical rebar met

60 ksi

4

0.20 in²

12.0 in

40 ksi

34.0 in

0.90 ACI318-05 - 9.3.2.1

0.75 ACI318-05 - 9.3.2.3

0.65 ACI318-05 - 9.3.2.2

29000 ksi

118.7 k-ft

692.9 k-ft - ACI318-005 - 10.2

0.17 Result: OK

27.4 k

158.6 k - ACI318-05 - 11.3.1.1 or 11.5.7.2

0.17 Result: OK

241.9 k

546.5 k - ACI318-05 - 10.2

0.44 Result: OK

272.8 k

2145.5 k - ACI318-05 - 10.3.6.2

0.13 Result: OK

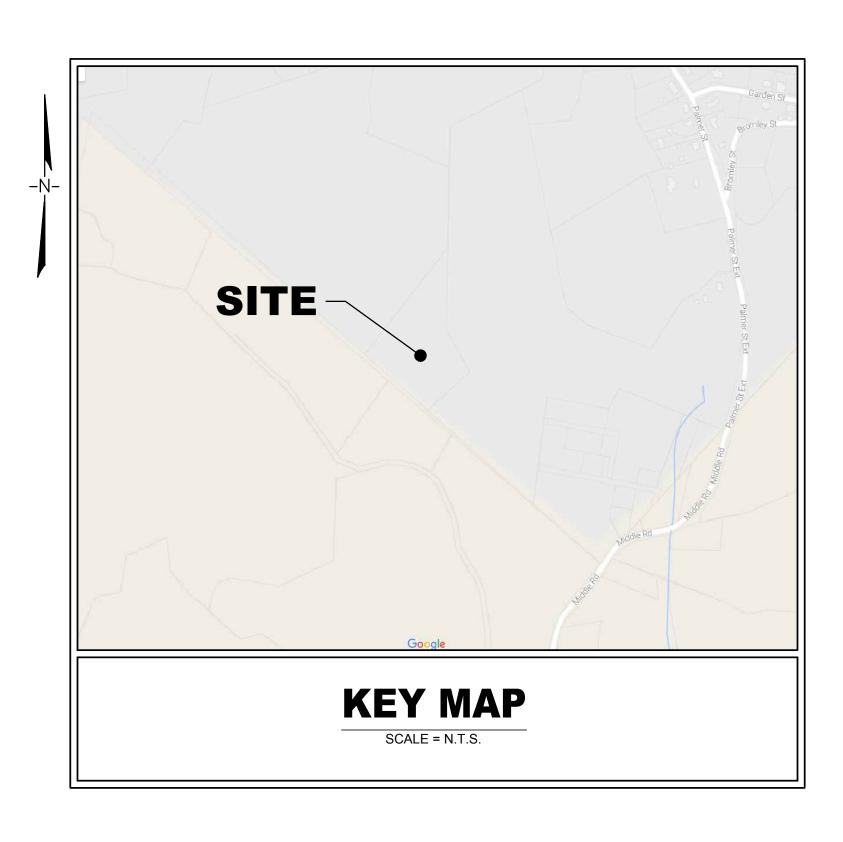
0.007 Reinforcement Ratio is Satisfactory - ACI318-05 - 10.8.4 & 10.9.1

#



"DO MACRO UPGRADE"

CT23XC114 2 HINCKLEY HILL ROAD PRESTON, CT 06360



SITE LOC	ATION INFORMATION
SITE ID NUMBER:	CT23XC114
SITE NAME:	MONTVILLE / NORWICH CDT
SITE ADDRESS:	2 HINCKLEY HILL ROAD PRESTON, CT 06360
PARCEL ID:	17-0 HIN1 2A
CENSUS TRACT:	700100
CENSUS BLOCK:	3012
PROPERTY OWNER:	JAMES C IRWIN 2 -A HINCKLEY HILL RD PRESTON, CT 06365
APPLICANT:	SPRINT 6100 SPRINT PARKWAY OVERLAND PARK, KS 66251

SITE CHARACTERISTICS		
LATITUDE: LONGITUDE:	41.510732 -72.059624	
STRUCTURE TYPE:	SELF-SUPPORT TOWER	
LOCATION OF PROPOSED EQUIPMENT: STRUCTURE HEIGHT:	EXISTING EQUIPMENT PLATFORM ±150'-0" AGL	
ANTENNA (RAD CENTER):	±140'-0" AGL (ALPHA) ±140'-0" AGL (BETA) ±140'-0" AGL (GAMMA)	

SHEET INDEX			
SHEET NO.	SHEET DESCRIPTION		
T-1	TITLE SHEET		
C-1	COMPOUND PLAN & GENERAL NOTES		
C-2	EXISTING & FINAL ANTENNA PLANS		
C-3	TOWER ELEVATION & FINAL EQUIPMENT PLAN		
C-4	CONSTRUCTION DETAILS		
C-5	FIBER PLUMBING DIAGRAM		
C-6	CABLE COLOR CODING		
C-7	EQUIPMENT DETAILS		
E-1	GROUNDING DETAILS		
E-2	DC POWER DETAILS & PANEL SCHEDULES		

SCOPE OF WORK

THE APPLICANT PROPOSES TO INSTALL THREE (3) NEW ANTENNAS AND SIX (6) NEW RADIO HEADS ON EXISTING ANTENNA MOUNTS ON THE EXISTING TOWER.





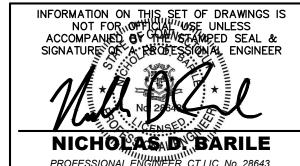


SCHEDULE OF REVISIONS

REV.		
0	05/18/17	INITIAL SUBMISSION
1	09/22/17	REVISED PER RFDS
2	10/04/17	REVISED PER RFDS
3	10/09/17	ISSUED FOR CONSTRUCTION
4	11/21/17	REVISED PER COMMENT
5		
6		
7		

DRAWN BY:	AM
CHECKED BY:	NDB
SCALE:	AS NOTED
JOB NO:	17043-CHE

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CT23XC114
2 HINCKLEY HILL ROAD
PRESTON, CT 06360

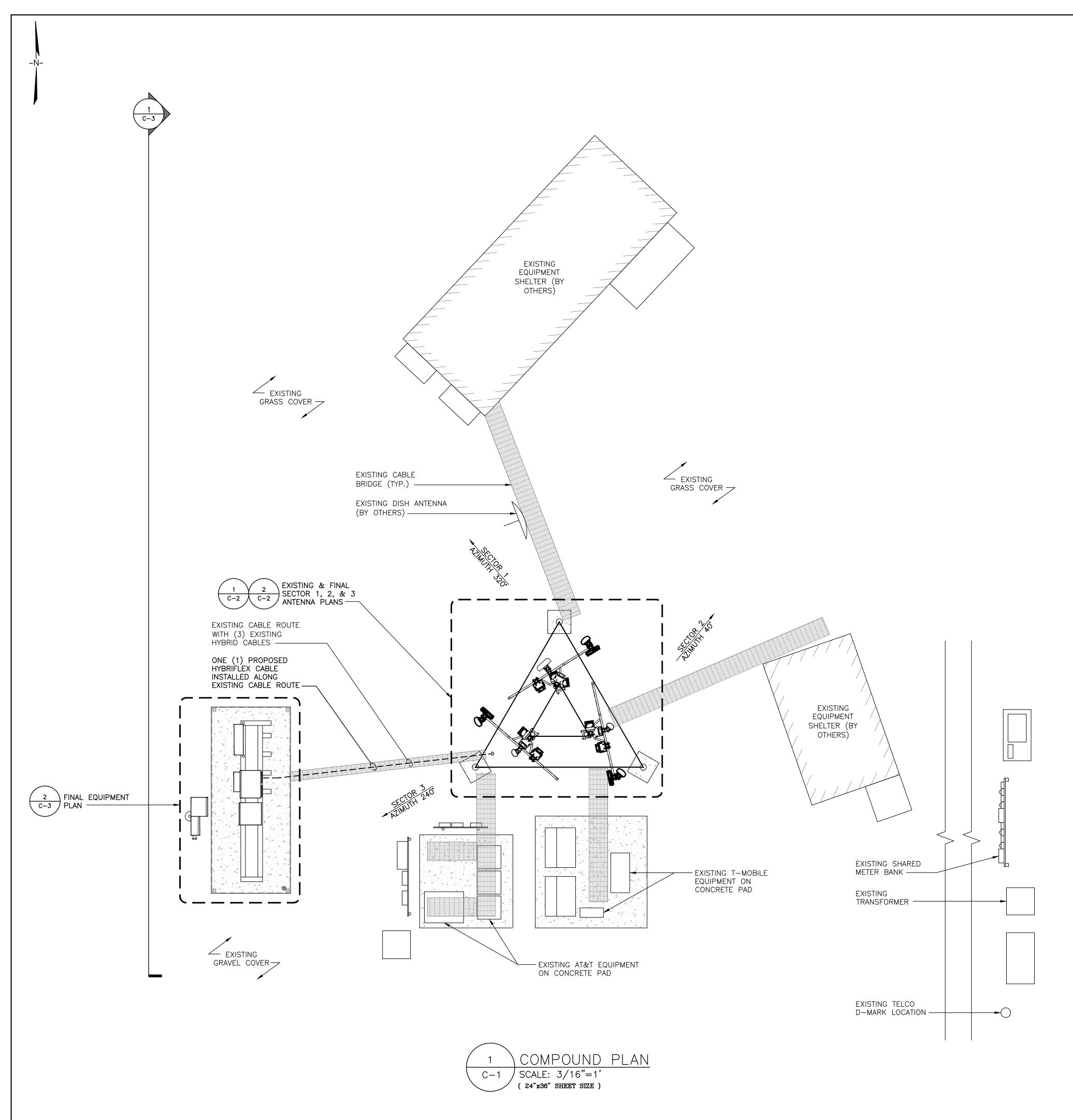
DRAWING TITLE:

TITLE SHEET

DRAWING SHEET: 1 OF 10

T-1

SIGNATURE BLOCK	:
SPRINT REPRESENTATIVE:	DATE
SPRINT RF ENGINEER:	DATE
PROPERTY OWNER:	DATE



GENERAL NOTES:

- 1. SUBJECT PROPERTY IS KNOWN AS TAX PARCEL ID 17-0 HIN1 2A, CENSUS TRACT 700100, CENSUS BLOCK 3012 AS SHOWN THE OFFICIAL TAX MAP OF THE TOWN OF PRESTON, CT.
- 2. THE APPLICANT PROPOSES TO INSTALL THREE (3) NEW ANTENNAS AND SIX (6) NEW RADIO HEADS ON EXISTING ANTENNA MOUNTS ON THE EXISTING TOWER.
- 3. CONTRACTOR SHALL NOT COMMENCE ANY WORK UNTIL HE OBTAINS, AT HIS OWN EXPENSE, ALL INSURANCE REQUIRED BY SPRINT, THE PROPERTY OWNER AND/OR PROPERTY MANAGEMENT COMPANY.
- 4. THIS SET OF PLANS HAS BEEN PREPARED FOR THE PURPOSES OF MUNICIPAL AND AGENCY REVIEW AND APPROVAL. THIS SET OF PLANS SHALL NOT BE UTILIZED AS CONSTRUCTION DOCUMENTS UNTIL ALL CONDITIONS OF APPROVAL HAVE BEEN SATISFIED AND EACH OF THE DRAWINGS HAVE BEEN REVISED TO INDICATED "ISSUED FOR CONSTRUCTION".
- 5. SITE INFORMATION SHOWN TAKEN FROM PLANS PREPARED BY FULLERTON ENGINEERING DESIGN FOR SPRINT'S INSTALLATION ON THIS FACILITY. DRAWINGS ENTITLED "SPRINT, SITE NAME: NORTHERN CT, SPRINT NUMBER: CT23XC114" DATED 09/26/12 REVISED 05/28/14. ADDITIONAL SITE INFORMATION WAS SUPPLEMENTED WITH A LIMITED SITE VISIT BY COM-EX CONSULTANTS 05/10/17.
- 6. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE CODES, ORDINANCES, LAWS AND REGULATIONS OF ALL MUNICIPALITIES, UTILITIES OR OTHER PUBLIC AUTHORITIES.
- 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS THAT MAY BE REQUIRED BY ANY FEDERAL, STATE, COUNTY OR MUNICIPAL AUTHORITIES.
- 8. THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER, IN WRITING, OF ANY CONFLICTS, ERRORS OR OMISSIONS PRIOR TO THE SUBMISSION OF BIDS OR PERFORMANCE OF WORK. MINOR OMISSIONS OR ERRORS IN THE BID DOCUMENTS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING THIS PROJECT IN ACCORDANCE WITH THE OVERALL INTENT OF THESE DRAWINGS.
- 9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL EXISTING SITE IMPROVEMENTS PRIOR TO COMMENCING CONSTRUCTION. THE CONTRACTOR SHALL REPAIR ANY DAMAGE CAUSED AS A RESULT OF CONSTRUCTION OF THIS FACILITY.
- 10. THE SCOPE OF WORK FOR THIS PROJECT SHALL INCLUDE PROVIDING ALL MATERIALS, EQUIPMENT AND LABOR REQUIRED TO COMPLETE THIS PROJECT. ALL EQUIPMENT SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- 11. THE CONTRACTOR SHALL VISIT THE PROJECT SITE PRIOR TO SUBMITTING A BID TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- 12. CONTRACTOR SHALL VERIFY ANTENNA ELEVATION AND AZIMUTH WITH RF ENGINEERING PRIOR TO INSTALLATION.
- 13. ALL STRUCTURAL ELEMENTS SHALL BE HOT DIPPED GALVANIZED STEEL.
- 14. THE CONSTRUCTION CONTRACTOR IS SOLELY RESPONSIBLE FOR DETERMINING ALL CONSTRUCTION MEANS AND METHODS. THE CONSTRUCTION CONTRACTOR IS ALSO RESPONSIBLE FOR ALL JOB SITE SAFETY.
- 15. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES AND EXISTING CONDITIONS AT THE SITE PRIOR TO FABRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA AND SUBMIT TO THE ENGINEER ANY DISCREPANCIES FROM THE DRAWINGS.
- 16. THE CONTRACTOR IS TO REVIEW ALL DRAWINGS AND SPECIFICATIONS IN THE CONTRACT DOCUMENT SET. THE CONTRACTOR SHALL COORDINATE ALL WORK SHOWN IN THE SET OF DRAWINGS. THE CONTRACTOR SHALL PROVIDE A COMPLETE SET OF DRAWINGS TO ALL SUBCONTRACTORS AND RELATED PARTIES. THE SUBCONTRACTOR SHALL EXAMINE ALL THE DRAWINGS AND SPECIFICATIONS FOR THE INFORMATION THAT EFFECTS THEIR WORK.
- 17. THE CONTRACTOR SHALL MAINTAIN A CURRENT SET OF DRAWINGS AND SPECIFICATIONS ON THE SITE AT ALL TIMES AND INSURE THE DISTRIBUTION OF NEW DRAWINGS TO SUBCONTRACTORS AND OTHER RELEVANT PARTIES AS SOON AS THEY ARE MADE AVAILABLE. OLD DRAWINGS SHALL BE MARKED VOID AND REMOVED FROM THE CONTRACT AREA CONTRACTOR FURNISH 3 SETS OF REDLINE "AS-BUILT" DRAWINGS TO SPRINT UPON COMPLETION OF THE WORK.
- 18. DETAILS ARE INTENDED TO SHOW END RESULT OF DESIGN. MINOR MODIFICATIONS MAT BE REQUIRED TO SUIT JOB DIMENSIONS OR CONDITIONS, AND SUCH MODIFICATIONS SHALL INCLUDED AS PART OF THE WORK.
- 19. ALL MATERIAL PROVIDED BY IS TO BE REVIEWED BY THE CONTRACTOR AND ALL APPLICABLE SUB-CONTRACTOR PRIOR TO INSTALLATION. ANY DEFICIENCIES TO PROVIDE MATERIALS SHALL BE BROUGHT TO THE CONSTRUCTION MANAGERS ATTENTION IMMEDIATELY.
- 20. THE MATERIALS INSTALLED SHALL MEET REQUIREMENTS OF CONTRACTORS DOCUMENTS. NO SUBSTITUTIONS ARE ALLOWED.
- 21. THE CONTRACTOR SHALL COORDINATE ALL CIVIL, STRUCTURAL AND ELECTRICAL DRAWINGS FOR THE LOCATIONS OF ALL OPENINGS, RECESSES, BUILT—IN WORK, ETC..
- 22. THE CONTRACTOR SHALL RECEIVE CLARIFICATION IN WRITING AND SHALL RECEIVE IN WRITING AUTHORIZATION TO PROCEED BEFORE STARTING WORK ON ANY ITEMS NOT CLEARLY DEFINED OR IDENTIFIED BY THE CONTACT DOCUMENTS.
- 23. THE CONTRACTOR SHALL NOTIFY THE CONSTRUCTION MANAGER OF ALL PRODUCTS OR ITEMS NOTED AS "EXISTING" WHICH ARE NOT FOUND TO BE IN THE FIELD.
- 24. ERECTION SHALL BE DONE IN A WORKMANLIKE MANNER BY COMPETENT EXPERIENCED WORKMEN IN ACCORDANCE WITH APPLICABLE CODES AND THE BEST—ACCEPTED PRACTICE. ALL MEMBERS SHALL BE LAND PLUMB AND TRUE AS INDICATED ON THE DRAWINGS.
- 25. THE CONTRACTOR SHALL COORDINATE HIS WORK AND SCHEDULE HIS ACTIVITIES AND WORKING HOURS IN ACCORDANCE WITH THE REQUIREMENTS OF THE PROPERTY OWNER AND/OR PROPERTY MANAGEMENT
- 26. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING HIS WORK WITH THE WORK OF OTHERS AS IT MAY RELATE TO RADIO EQUIPMENT, ANTENNAS AND ANY OTHER PORTIONS OF THE WORK.
- 27. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH THE MANUFACTURE'S RECOMMENDATIONS UNLESS SPECIFICALLY INDICATED OR WHERE LOCAL CODES OR REGULATIONS MAY TAKE PRECEDENCE.
- 28. THE CONTRACTOR SHALL REPAIR ALL EXISTING SURFACES DAMAGED DURING CONSTRUCTION SUCH THAT THEY MATCH AND BLEND WITH ADJACENT SURFACES.
- 29. THE CONTRACTOR SHALL KEEP CONTRACT AREA CLEAN, HAZARD FREE AND DISPOSE OF ALL DEBRIS AND RUBBISH. LEAVE PREMISES IN CLEAN CONDITION AND FREE FROM PAINT SPOTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL ITEMS UNTIL COMPLETION OF CONSTRUCTION.
- 30. BEFORE FINAL ACCEPTANCE OF THE WORK, THE CONTRACTOR SHALL REMOVE ALL EQUIPMENT, TEMPORARY WORKS, UNUSED AND USELESS MATERIALS, RUBBISH AND TEMPORARY STRUCTURES.
- 31. DESIGN REQUIREMENTS PER INTERNATIONAL BUILDING CODE 2015 AND THE EIA/TIA-222-G STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES.





OVERLAND PARK, KS 66251



7		
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4	11/21/17	REVISED PER COMMENT
3	10/09/17	ISSUED FOR CONSTRUCTION
2	10/04/17	REVISED PER RFDS
1	09/22/17	REVISED PER RFDS
0	05/18/17	INITIAL SUBMISSION

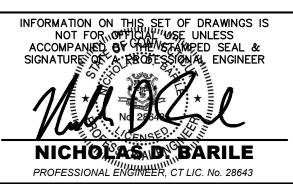
DESCRIPTION OF CHANGES

SCHEDULE OF REVISIONS

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SCALE:	AS NOTED
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DATE

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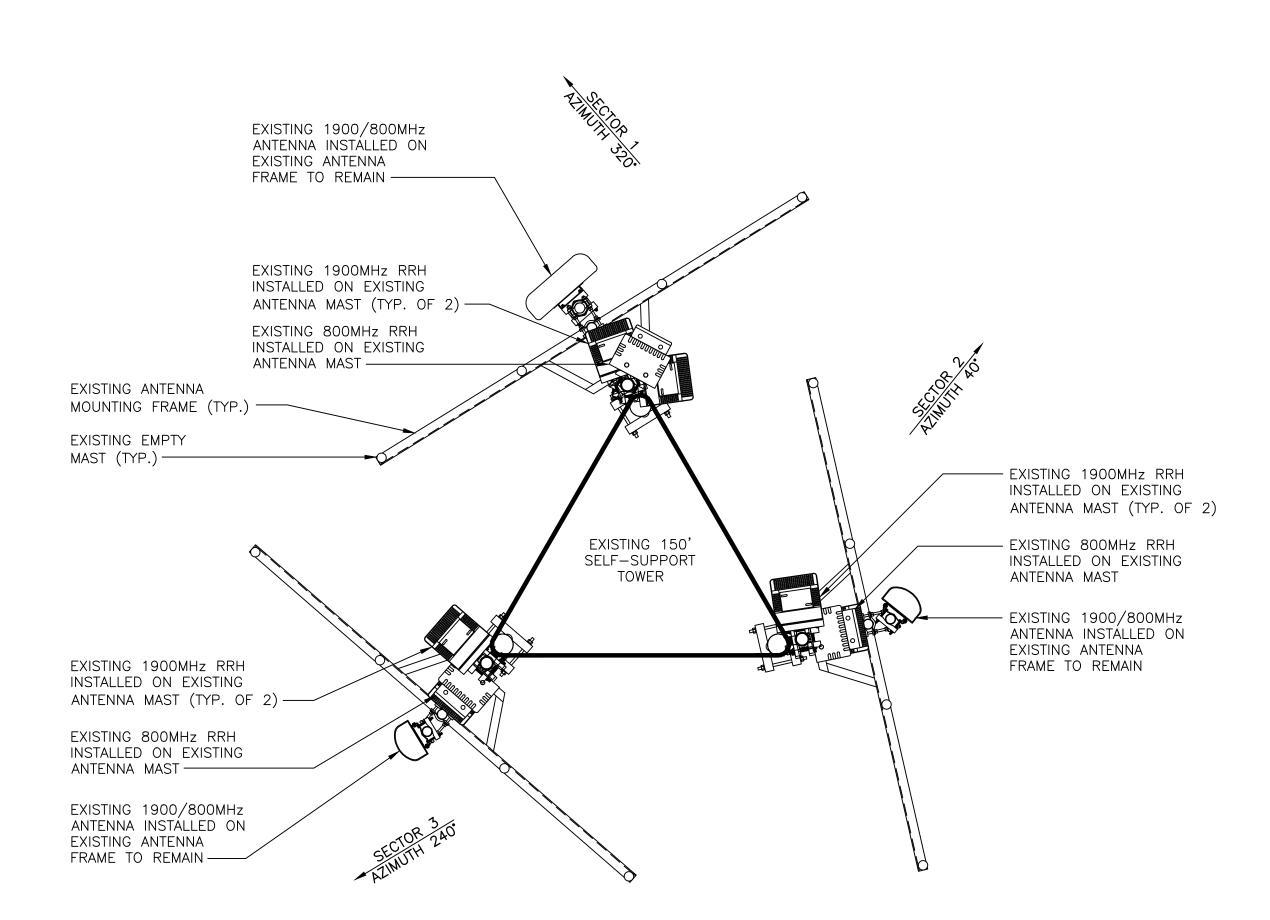


CT23XC114
2 HINCKLEY HILL ROAD
PRESTON, CT 06360

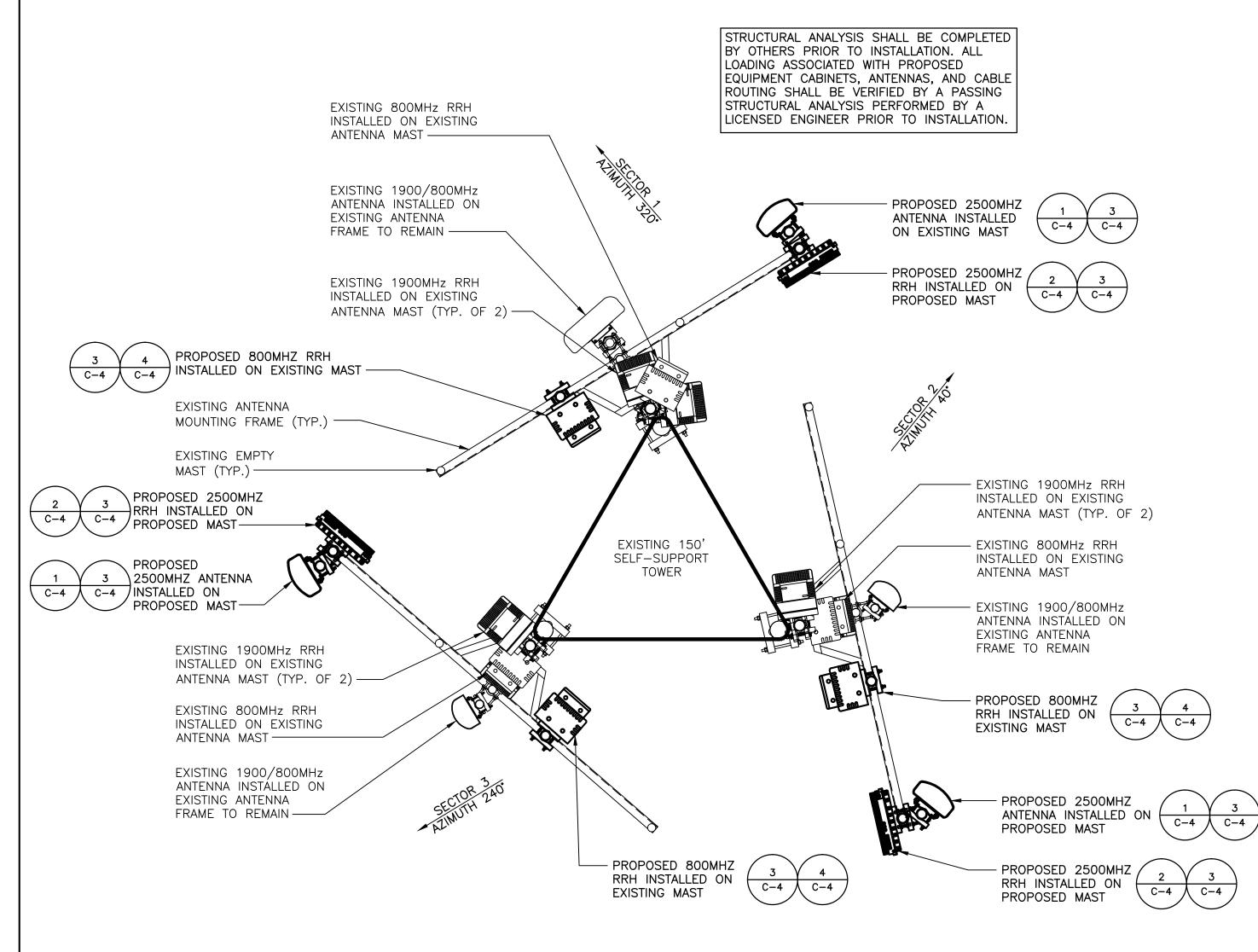
DRAWING TITLE:

COMPOUND PLAN &
GENERAL NOTES

DRAWING SHEET: 2 OF 10



1 EXISTING SECTOR 1, 2, & 3 ANTENNA PLAN C-2 SCALE: 1/2"=1"



FINAL SECTOR 1, 2, & 3 ANTENNA PLAN

C-2 | SCALE: 1/2"=1'

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Suite E39

Mountain Lakes, NJ 07046
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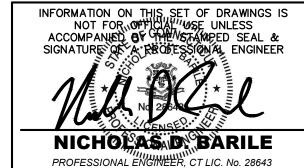


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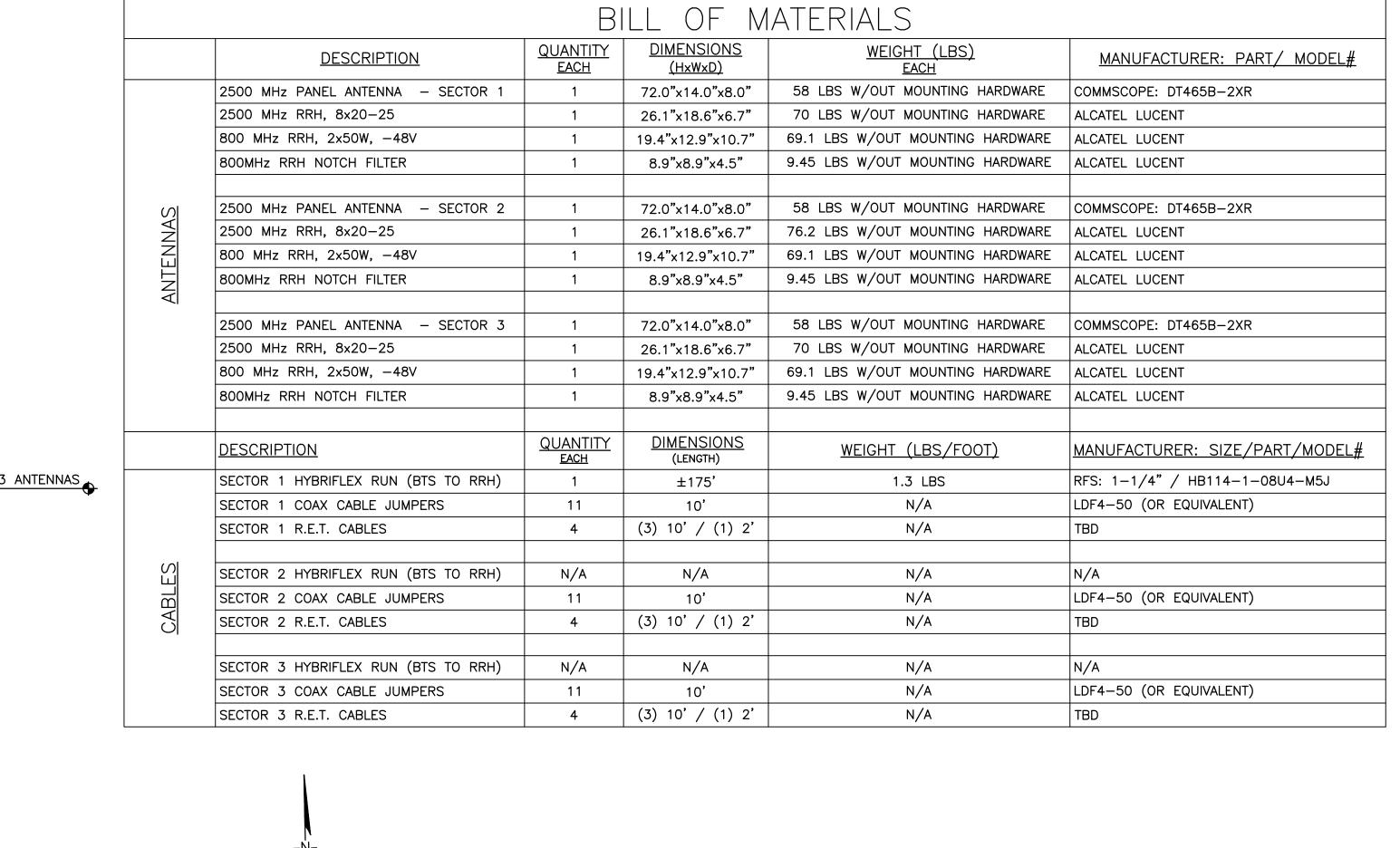


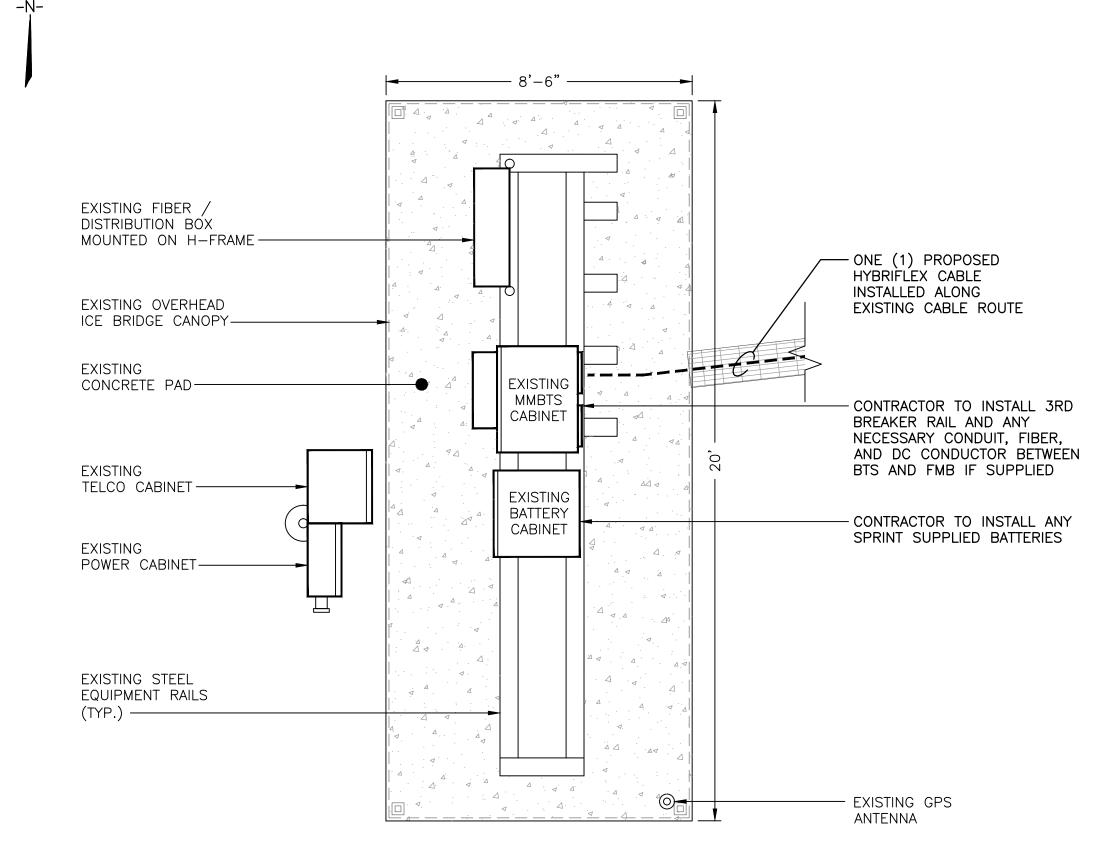
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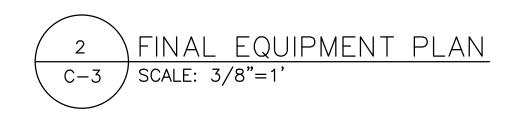
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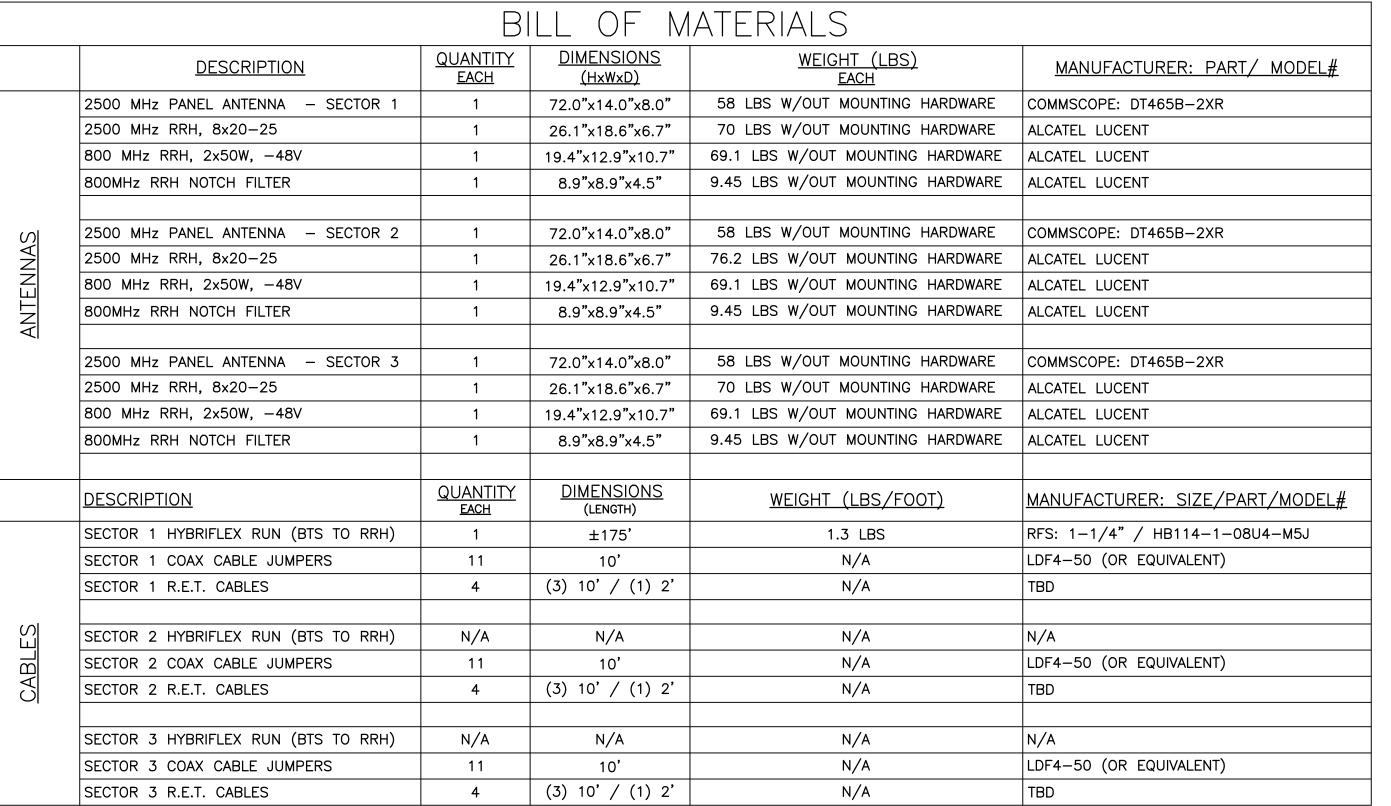
EXISTING & FINAL ANTENNA PLANS

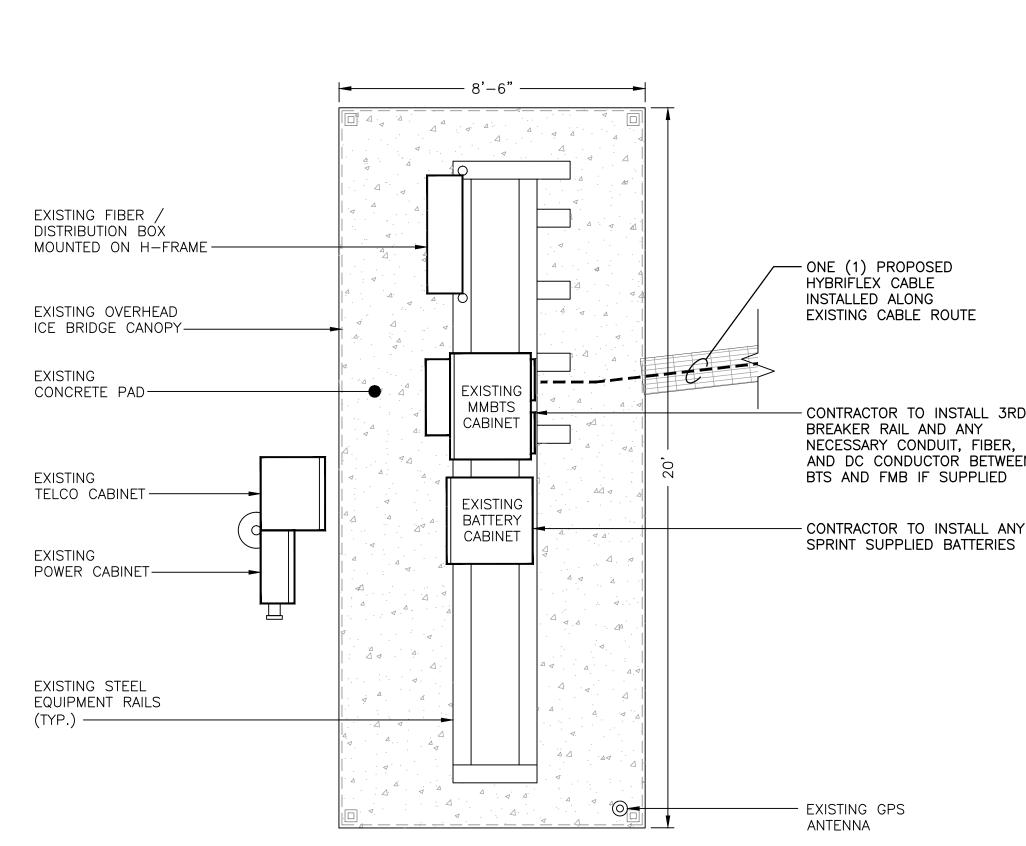
DRAWING SHEET: 3 OF 10











& FINAL **EQUIPMENT PLAN**

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SIGNATURE OF A RECORDS ON ALL ENGINEER

NICHOLAS DEBARILE

PROFESSIONAL ENGINEER, CT LIC. No. 28643

CT23XC114

2 HINCKLEY HILL ROAD

PRESTON, CT 06360

TOWER

ELEVATION, B.O.M.

DESCRIPTION OF CHANGES

AM

NDB

AS NOTED

17043-CHE

10/04/17 REVISED PER RFDS

ISSUED FOR CONSTRUCTION

Consulting

6100 SPRINT PARKWAY

11/21/17

10/09/17

09/22/17

05/18/17

DATE

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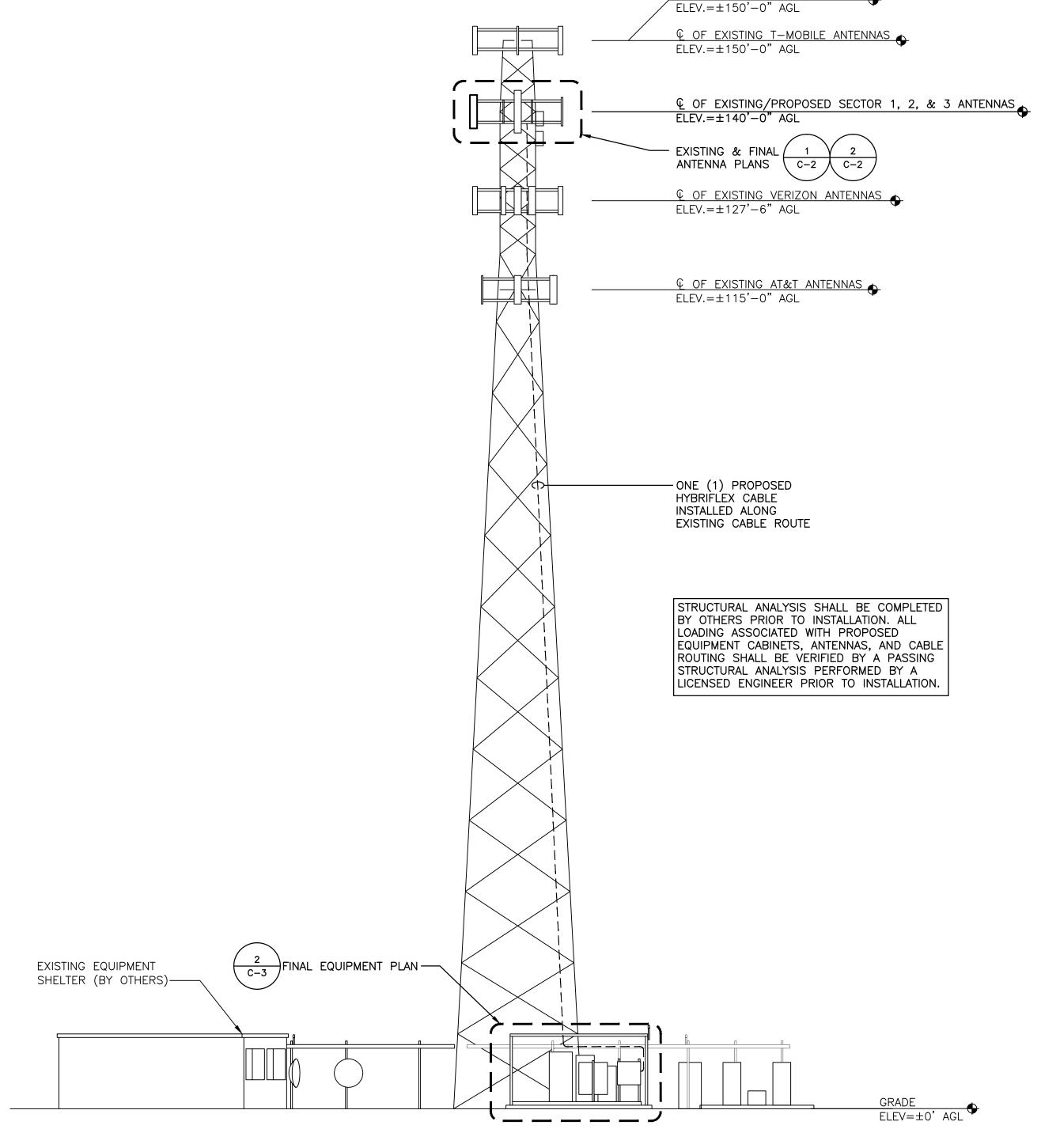
JOB NO:

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OVERLAND PARK, KS 66251

DRAWING SHEET: 4 OF 10

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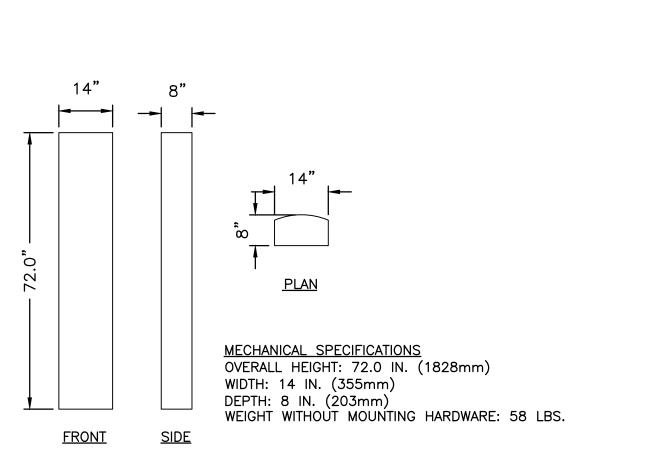


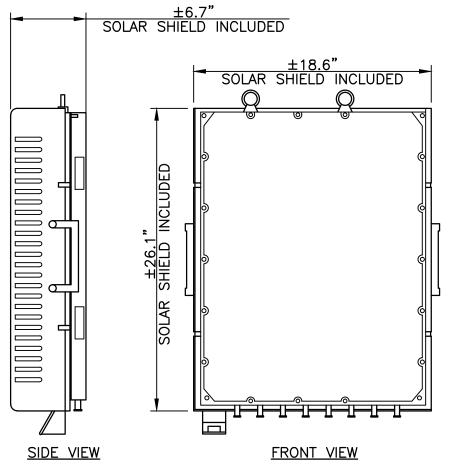
TOWER ELEVATION

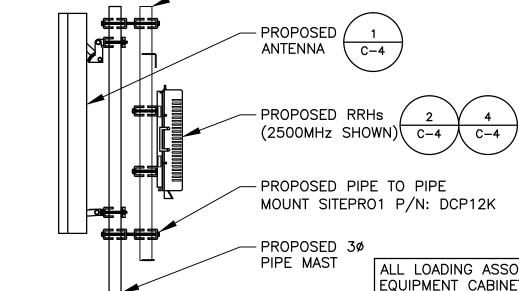
C-3 / SCALE: 3/32"=1'

(24"x36" SHEET SIZE)

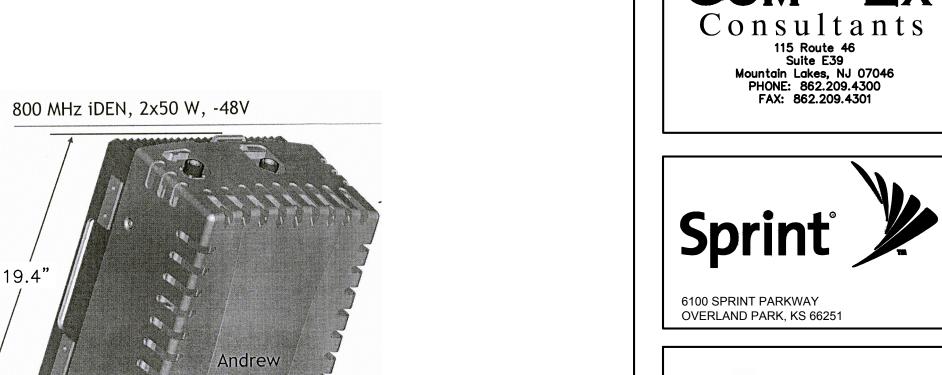
TOP OF EXISTING TOWER







- EXISTING ANTENNA MOUNTING FRAME



PLAN VIEW

ALL LOADING ASSOCIATED WITH PROPOSED EQUIPMENT CABINETS, ANTENNAS, AND CABLE ROUTING SHALL BE VERIFIED BY A PASSING STRUCTURAL ANALYSIS PERFORMED BY A LICENSED ENGINEER PRIOR TO INSTALLATION

Cherundolo Consulting

SCHEDULE OF REVISIONS

Com ≫ EX

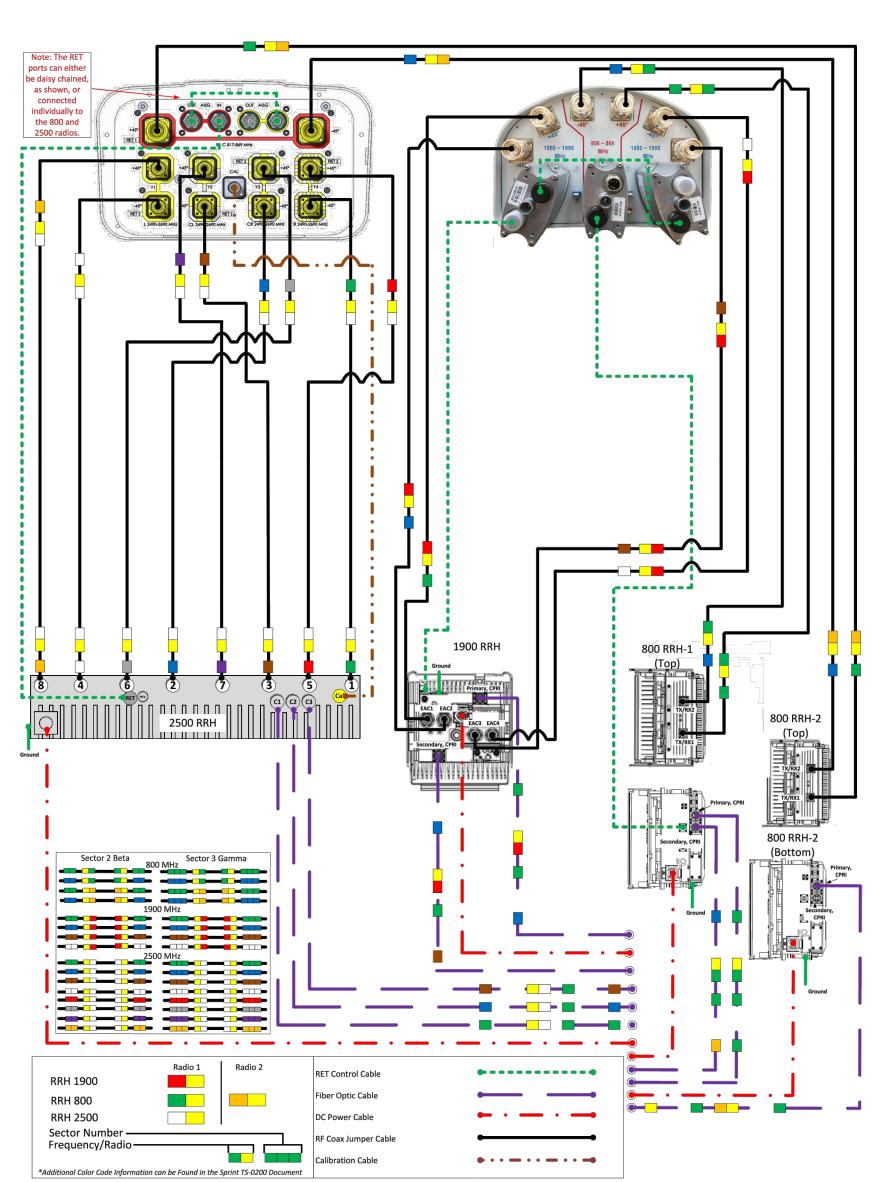
2500MHz ANTENNA COMMSCOPE: DT465B-2XR C-4 / SCALE: NTS

2500MHz RRH DETAIL SCALE: N.T.S.

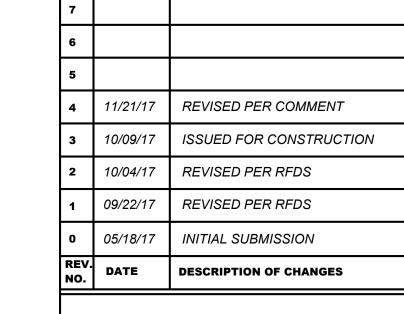
SECTOR 1, 2, & 3 ANTENNA & RRH INSTALLATION DETAIL C-4 / SCALE: N.T.S.

\800MHz RRH DETAIL (ANT-005) SCALE: N.T.S.

19.4

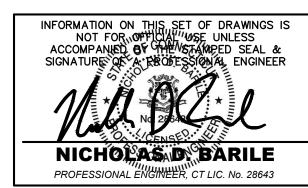


ANTENNA PLUMBING DIAGRAM (ANT-005) SCALE: N.T.S.



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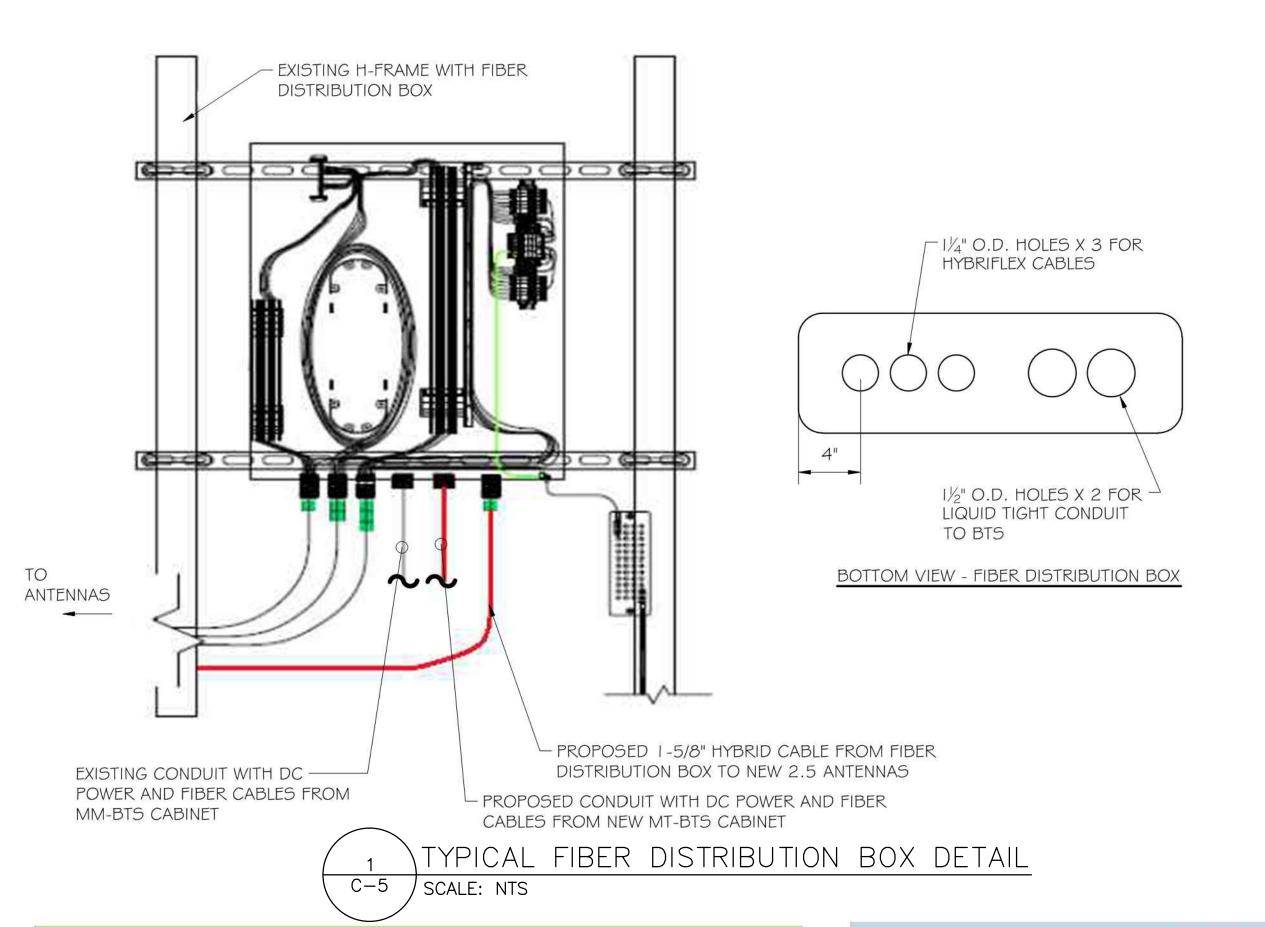


CT23XC114 2 HINCKLEY HILL ROAD PRESTON, CT 06360

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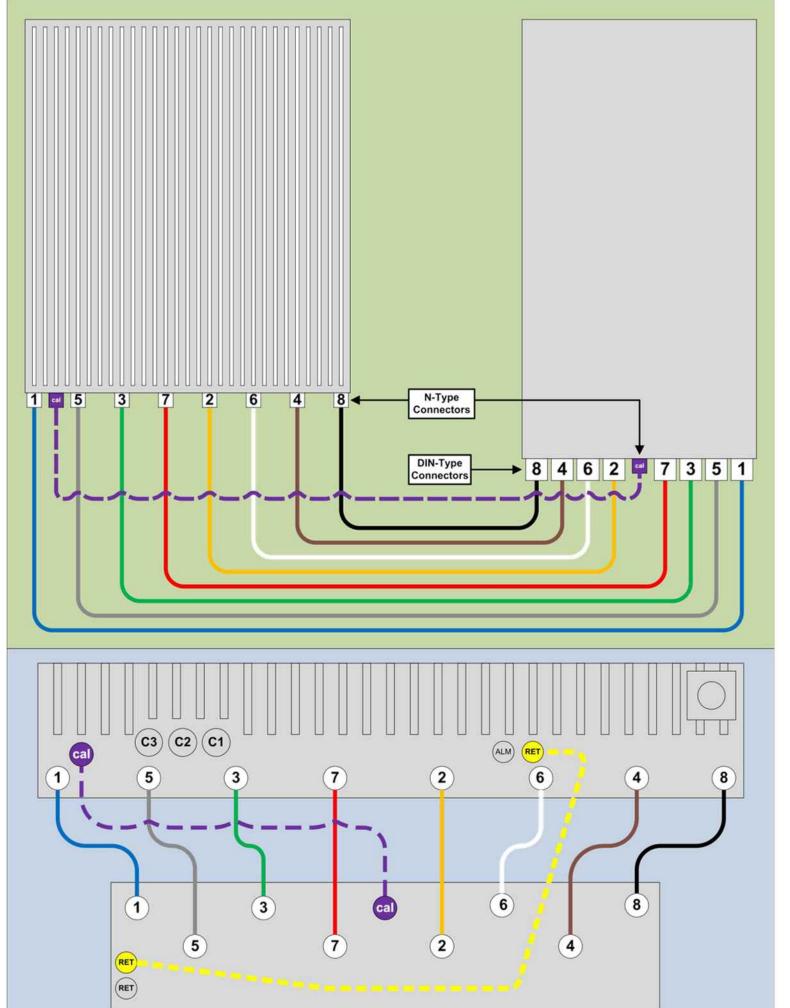
CONSTRUCTION **DETAILS**

DRAWING SHEET: 5 OF 10



Under 200 Feet / Three (3) to Nine (9) Existing RRHs Alpha 800 MHz **Existing SNV Hybriflex** 1900 MHz Alpha 8 AWG Power Alpha 1900 MHz Alpha 2.5 GHz Beta 800 MHz **Existing SNV Hybriflex** 1900 MHz Beta 8 AWG Power 1900 MHz Beta 2.5 GHz 800 MHz Gamma **Existing SNV Hybriflex** 1900 MHz Gamma 8 AWG Power Gamma 1900 MHz Gamma 2.5 GHz

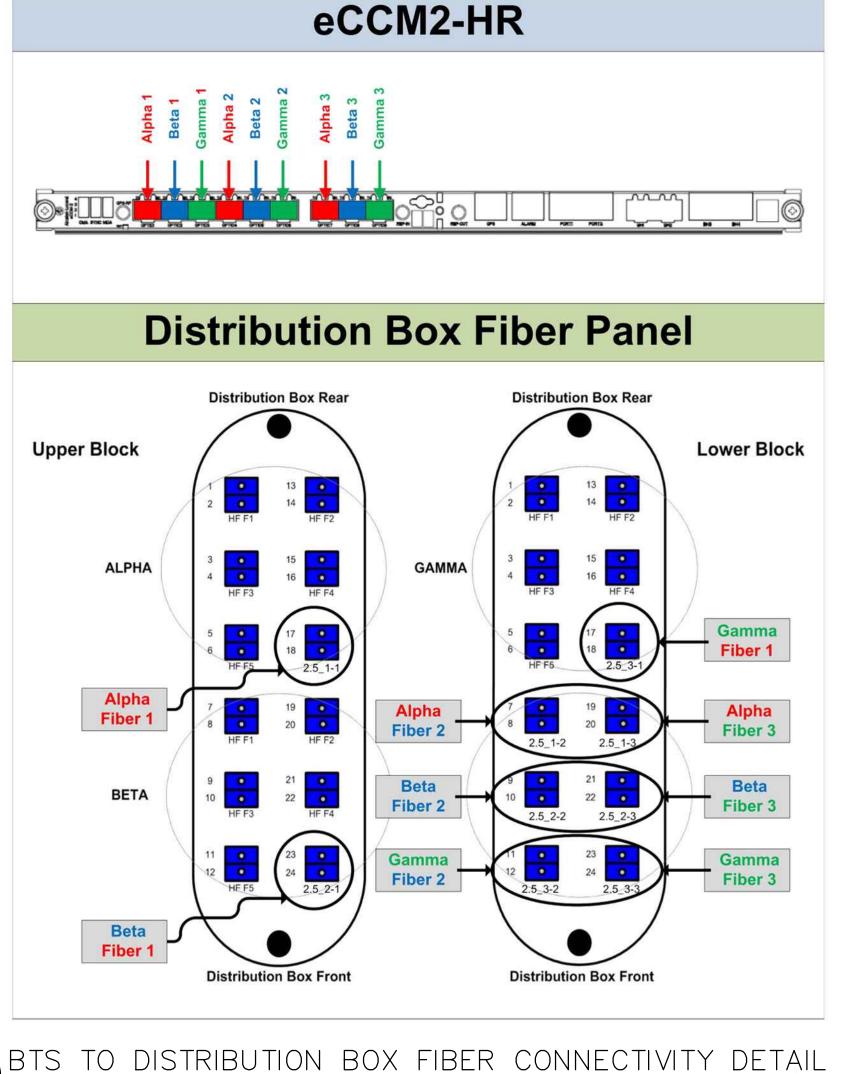
2 RRH TO DISTRIBUTION BOX POWER CONNECTIVITY DETAIL C-5 SCALE: NTS

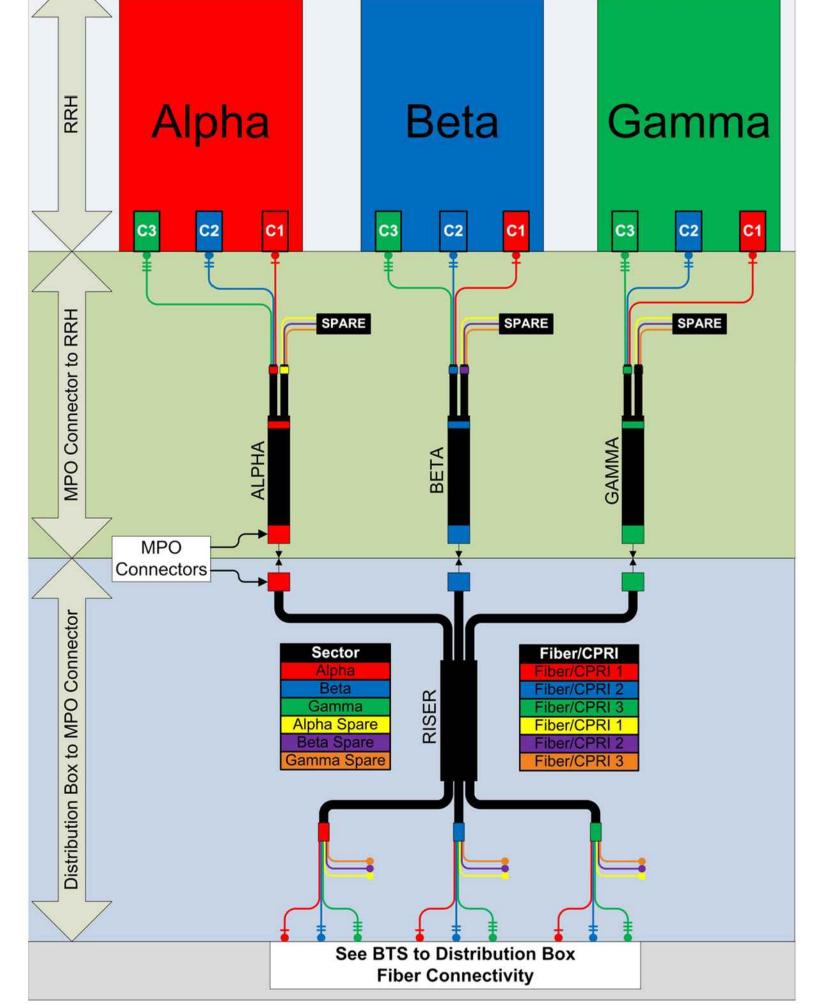


8T8R DETAIL

SCALE: NTS

C-5 / SCALE: NTS





RRH TO DISTRIBUTION BOX FIBER CONNECTIVITY DETAIL

C-5 | SCALE: NTS

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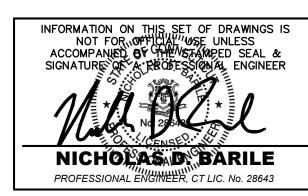




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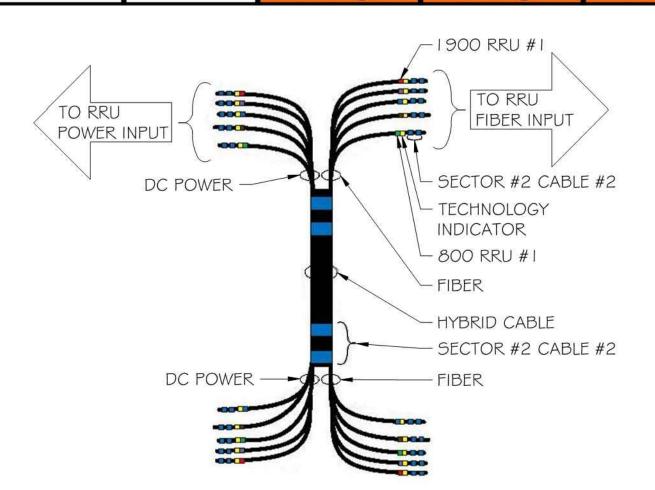
FIBER PLUMBING DIAGRAM

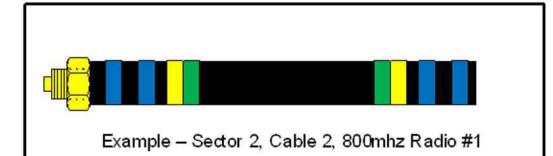
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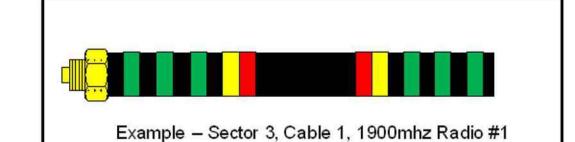
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2500 -1	YEL	WHT	GRN
2500 -2	YEL	WHT	RED
2500 -3	YEL	WHT	BRN
2500 -4	YEL	WHT	BLU
2500 -5	YEL	WHT	SLT
2500 -6	YEL	WHT	ORG
2500 -7	YEL	WHT	WHT
2500 -8	YEL	WHT	PPL

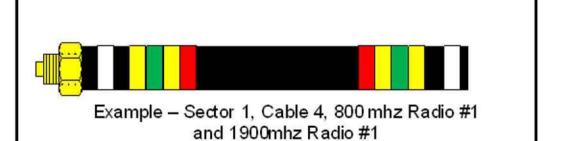
NV		
FREQUENCY	INDICATOR	ID
800-1	YEL	GRN
1900-1	YEL	RED
1900-2	YEL	BRN
1900-3	YEL	BLU
1900-4	YEL	SLT
800-1	YEL	ORG
RESERVED	YEL	WHT
RESERVED	YEL	PPL

			Second	
Sector	Cable	First Ring	Ring	Third Ring
1 Alpha	1	Green	No Tape	No Tape
1	2		No Tape	No Tape
1	3	Brown	No Tape	No Tape
1	4	White	No Tape	No Tape
1	5	Red	No Tape	No Tape
1	6	Grey	No Tape	No Tape
1	7	Purple	No Tape	No Tape
1	8	Orange	No Tape	No Tape
2 Beta	1	Green	Green	No Tape
2	2	Blue		No Tape
2	3	Brown	Brown	No Tape
2	4	White	White	No Tape
2	5	Red	Red	No Tape
2	6	Grey	Grey	No Tape
2	7	Purple	Purple	No Tape
2	8	Orange	Orange	No Tape
3 Gamma	1	Green	Green	Green
3	2			Blue
3	3	Brown	Brown	Brown
3	4	White	White	White
3	5	Red	Red	Red
3	6	Grey	Grey	Grey
3	7	Purple	Purple	Purple
3	8	Orange	Orange	Orange











CABLE MARKING NOTES

- ALL CABLES SHALL BE MARKED WITH 2" WIDE, UV STABILIZED, UL APPROVED TAPE.
- 2. THE FIRST RING SHALL BE CLOSEST TO THE END OF THE CABLE AND SPACED APPROXIMATELY 2" FROM THE END CONNECTOR, WEATHERPROOFING, OR BREAKOUT UNIT. THERE SHALL BE 1" SPACE BETWEEN EACH RING.
- 3. A 2" GAP SHALL SEPARATE THE CABLE COLOR CODE FROM THE FREQUENCY COLOR CODE. THE 2" COLOR RINGS FOR THE FREQUENCY CODE SHALL BE PLACED NEXT TO EACH OTHER WITH NO SPACES.
- 4. THE 2" COLORED TAPE(S) SHALL BE WRAPPED A MINIMUM OF 3 TIMES AROUND THE INDIVIDUAL CABLES, AND THE TAPE SHALL BE KEPT IN THE SAME LOCATION AS MUCH AS POSSIBLE.
- 5. SITES WITH MORE THAN FOUR (4) SECTORS WILL REQUIRE ADDITIONAL RINGS FOR EACH SECTOR, FOLLOWING THE PATTERN. HIGH CAPACITY SITES WILL USE THE SECOND CABLE IDENTIFIED BY BLUE BANDS OF TAPE
- 6. HYBRID FIBER CABLE SHALL BE SECTOR IDENTIFIED INSIDE THE CABINET ON FREQUENCY BUNDLES, ON THE SEALTITE, ON THE MAIN LINE UPON EXIT OF SEALTITE, AND BEFORE AND AFTER THE BREAKOUT UNIT (MEDUSA), AS WELL AS BEFORE AND AFTER ANY ENTRANCE OR EXIT.
- 7. HFC "MAIN TRUNK" WILL NOT BE MARKED WITH THE FREQUENCY CODES, AS IT CONTAINS ALL FREQUENCIES.
- 8. INDIVIDUAL POWER PAIRS AND FIBER BUNDLES SHALL BE LABELED WITH BOTH THE CABLE AND FREQUENCY.





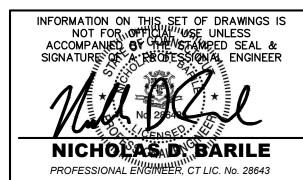
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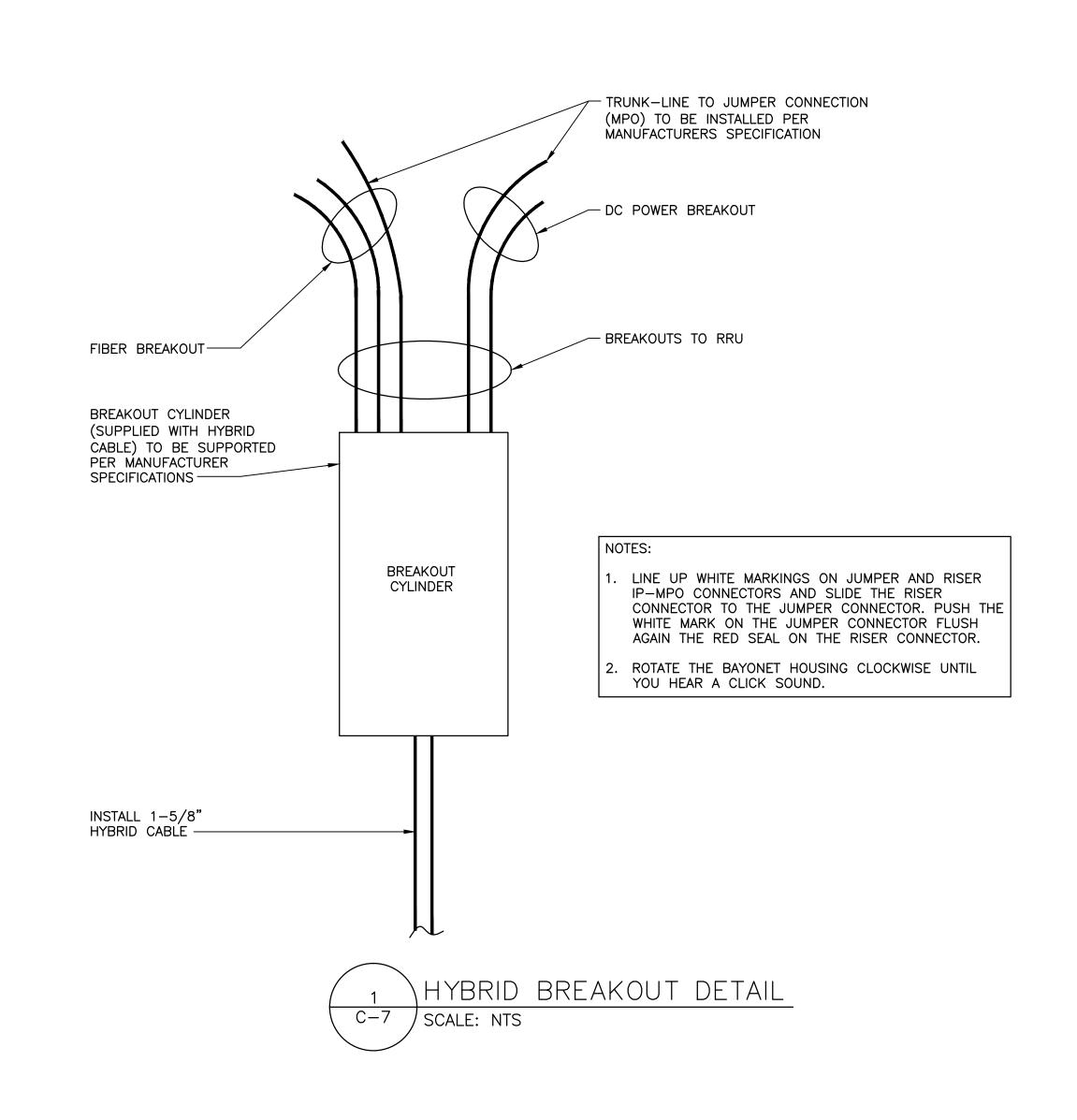


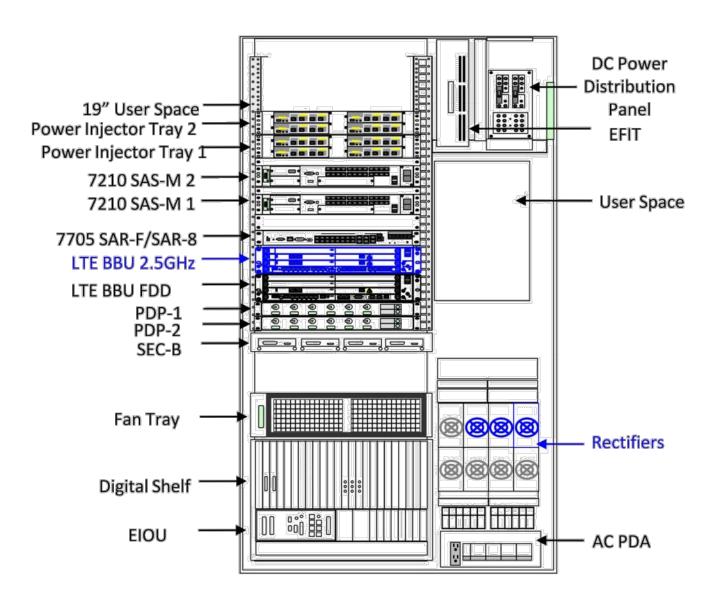
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DRAWING TITLE:

CABLE COLOR CODING

DRAWING SHEET: 7 OF 10











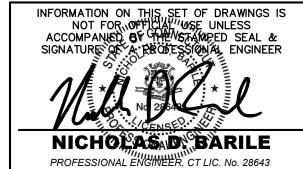
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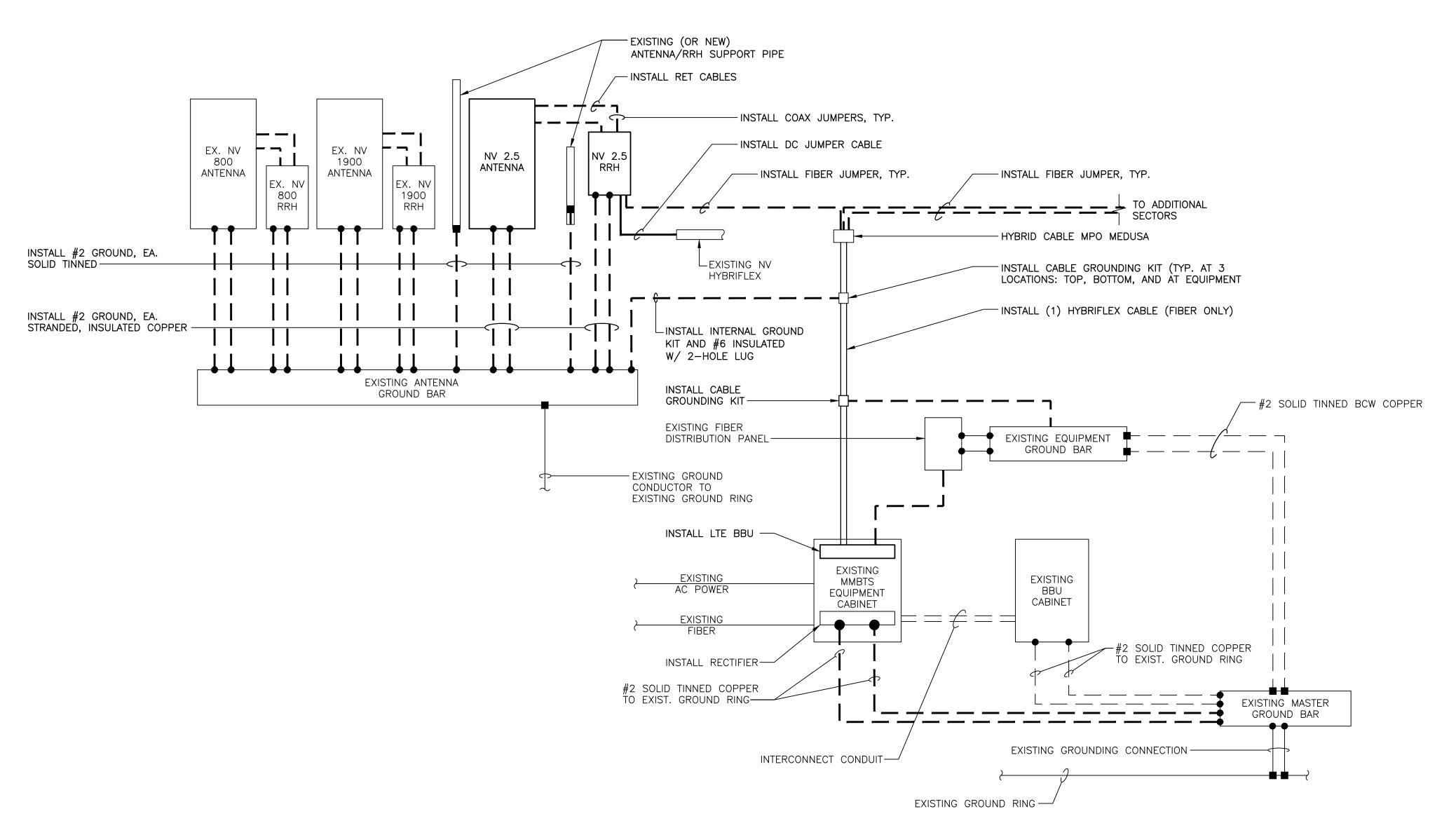


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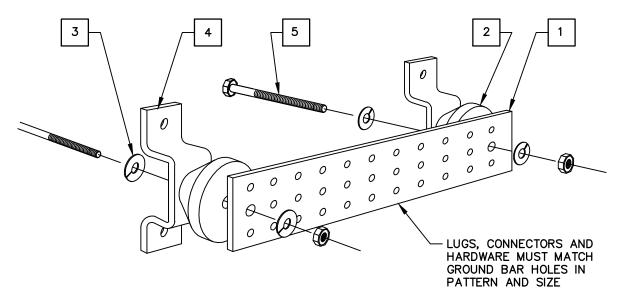
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EQUIPMENT DETAILS

DRAWING SHEET: 8 OF 10



TYPICAL POWER & GROUNDING ONE-LINE DIAGRAM SCALE: N.T.S.

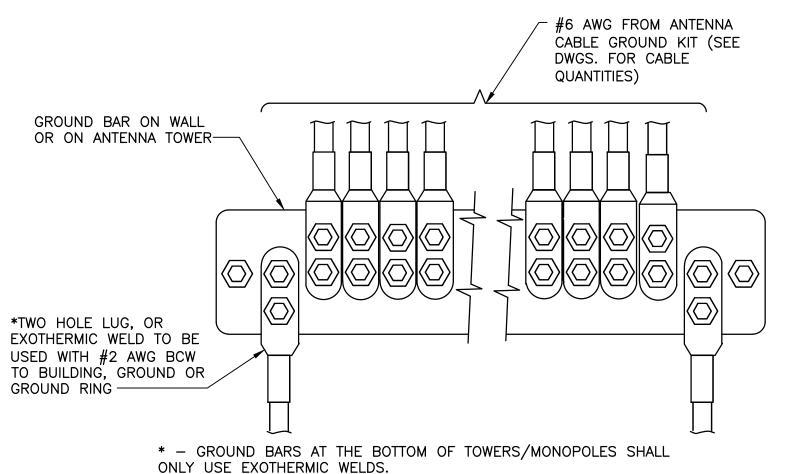


LEGEND

COPPER GROUND BAR, 7/16"X 4" X 20", NEWTON INSTRUMENT CO. CAT. NO. B-6142. HOLE CENTERS TO MATCH NEMA DOUBLE LUG CONFIGURATION. INSULATORS, NEWTON INSTRUMENT CAT. NO. 3061-4. 5/8" LOCKWASHERS, NEWTON INSTRUMENT CO. CAT. NO. 3015-8. WALL MOUNTING BRACKET, NEWTON INSTRUMENT CO. CAT NO. A-6056. 5/8-11 X 1" H.H.C.S.BOLTS, NEWTON INSTRUMENT CO. CAT NO. 3012-1

GROUND BAR SCHEDULE						
TYPE	QTY.	MANUFACTURER	CAT. NO.	REMARKS		
MGB	2	HARGER	GB14420TMGB	OR EQUAL		
CGB 3 HARGER GB14412TMGB OR EQUAL						





- ATTACH "DO NOT DISCONNECT" LABELS TO GROUND BARS. CAN

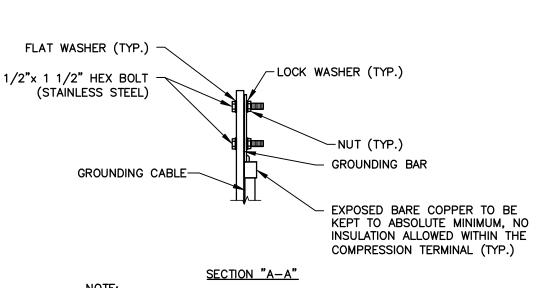
USE BRASS TAG "DO NOT DISCONNECT" AT EACH HYBRIFLEX GROUND POINT OR BACK-A-LITE PLATE LABEL ON GROUND BAR.

BAR/NO-OX/WASHER/LOCK-WASHER/NUT. THIS IS REPEATED FOR

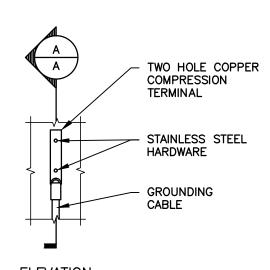
CONNECT SEQUENCE— BOLT/WASHER/NO—OX/GROUND

SCALE: NTS

EACH LUG CONNECTION POINT. TYPICAL GROUND BAR CONNECTION PLAN



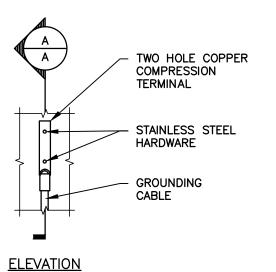
1. "DOUBLING UP" OR "STACKING" OF CONNECTIONS IS NOT PERMITTED. 2. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS.



TYPICAL GROUND BAR CONNECTION DETAIL SCALE: NTS

ELECTRICAL AND GROUNDING NOTES

- 1. ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE NATIONAL ELECTRICAL CODE (NEC) AS WELL AS APPLICABLE STATE AND LOCAL CODES.
- 2. ALL ELECTRICAL ITEMS SHALL BE U.L. APPROVED OR LISTED AND PROCURED PER SPECIFICATION REQUIREMENTS.
- 3. ELECTRICAL AND TELCO WIRING OUTSIDE A BUILDING AND EXPOSED TO WEATHER SHALL BE IN WATER TIGHT GALVANIZED RIGID STEEL CONDUITS OR SCHEDULE 80 PVC (AS PERMITTED BY CODE) AND WHERE REQUIRED IN LIQUID TIGHT FLEXIBLE METAL OR NONMETALLIC CONDUITS.
- 4. BURIED CONDUIT SHALL BE SCHEDULE 40 PVC.
- 5. ELECTRICAL WIRING SHALL BE COPPER WITH TYPE XHHW, THWN, OR THNN INSULATION.
- 6. RUN TELCO CONDUIT OR CABLE BETWEEN TELEPHONE UTILITY DEMARCATION POINT AND PROJECT OWNER CELL SITE TELCO CABINET AND BTS CABINET AS INDICATED ON THIS DRAWING PROVIDE FULL LENGTH PULL ROPE IN INSTALLED TELCO CONDUIT. PROVIDE GREENLEE CONDUIT MEASURING TAPE AT EACH END.
- WHERE CONDUIT BETWEEN BTS AND PROJECT OWNER CELL SITE PPC AND BETWEEN BTS AND PROJECT OWNER CELL SITE TELCO SERVICE CABINET ARE UNDERGROUND USE PVC, SCHEDULE 40 CONDUIT. ABOVE THE GROUND PORTION OF THESE CONDUITS SHALL BE PVC CONDUIT.
- 8. ALL EQUIPMENT LOCATED OUTSIDE SHALL HAVE NEMA 3R ENCLOSURE.
- 9. GROUNDING SHALL COMPLY WITH NEC ART. 250.
- 10. GROUND HYBRIFLEX CABLE SHIELDS AT 3 LOCATIONS USING MANUFACTURER'S HYBRIFLEX CABLE GROUNDING KITS SUPPLIED BY PROJECT OWNER.
- 11. USE #6 COPPER STRANDED WIRE WITH GREEN COLOR INSULATION FOR ABOVE GRADE GROUNDING (UNLESS OTHERWISE SPECIFIED) AND #2 SOLID TINNED BARE COPPER WIRE FOR BELOW GRADE GROUNDING AS INDICATED ON THE DRAWING.
- 12. ALL GROUND CONNECTIONS TO BE BURNDY HYGROUND COMPRESSION TYPE CONNECTORS OR CADWELD EXOTHERMIC WELD. DO NOT ALLOW BARE COPPER WIRE TO BE IN CONTACT WITH GALVANIZED STEEL.
- 13. ROUTE GROUNDING CONDUCTORS ALONG THE SHORTEST AND STRAIGHTEST PATH POSSIBLE, EXCEPT AS OTHERWISE INDICATED. GROUNDING LEADS SHOULD NEVER BE BENT AT RIGHT ANGLE. ALWAYS MAKE AT LEAST 12" RADIUS BENDS. #6 WIRE CAN BE BENT AT 6" RADIUS WHEN NECESSARY. BOND ANY METAL OBJECTS WITHIN 6 FEET OF PROJECT OWNER EQUIPMENT OR CABINET TO MASTER GROUND BAR OR GROUNDING RING.
- 14. CONNECTIONS TO GROUND BARS SHALL BE MADE WITH TWO HOLE COMPRESSION TYPE COPPER LUGS. APPLY OXIDE INHIBITING COMPOUND TO ALL LOCATIONS.
- 15. APPLY OXIDE INHIBITING COMPOUND TO ALL COMPRESSION TYPE GROUND CONNECTIONS.
- 16. BOND ANTENNA MOUNTING BRACKETS, HYBRIFLEX CABLE GROUND KITS. AND RRHs TO EGB PLACED NEAR THE ANTENNA LOCATION.
- 17. BOND ANTENNA EGB'S AND MGB TO GROUND RING.
- 18. CONTRACTOR SHALL TEST COMPLETED GROUND SYSTEM AND RECORD RESULT FOR PROJECT CLOSE-OUT DOCUMENTATION. 5 OHMS MINIMUM RESISTANCE REQUIRED.
- 19. CONTRACTOR SHALL CONDUCT ANTENNA, HYBRIFLEX CABLES, AND RRH RETURN-LOSS AND DISTANCE- TO-FAULT MEASUREMENTS (SWEEP TESTS) AND RECORD RESULTS FOR PROJECT CLOSE OUT.
- 20. CONTRACTOR (CERTIFIED ELECTRICIAN) SHALL CHECK CAPACITY OF EXISTING SERVICE & PANEL ON SITE TO DETERMINE IF CAPACITY EXISTS TO ACCOMMODATE THE ADDED LOAD OF THIS PROJECT. ADVISE ENGINEER OF ANY DISCREPANCY.







OVERLAND PARK, KS 66251

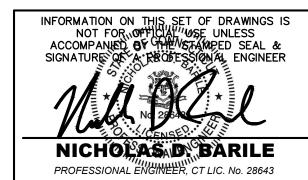


	SCHEDULE OF REVISIONS		
7			
6			
5			
4	11/21/17	REVISED PER COMMENT	
3	10/09/17	ISSUED FOR CONSTRUCTION	
2	10/04/17	REVISED PER RFDS	
1	09/22/17	REVISED PER RFDS	
0	05/18/17	INITIAL SUBMISSION	

DESCRIPTION OF CHANGES

DRAWN BY:	AM
CHECKED BY:	NDB
SCALE:	AS NOTED
JOB NO:	17043-CHE

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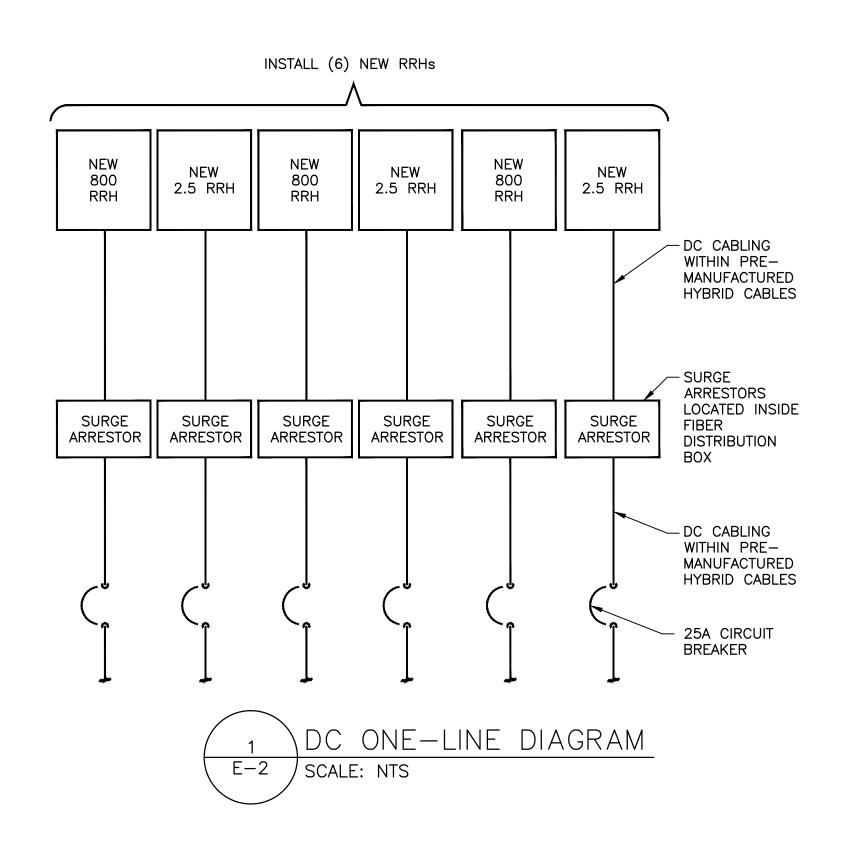


CT23XC114 2 HINCKLEY HILL ROAD PRESTON, CT 06360

DRAWING TITLE:

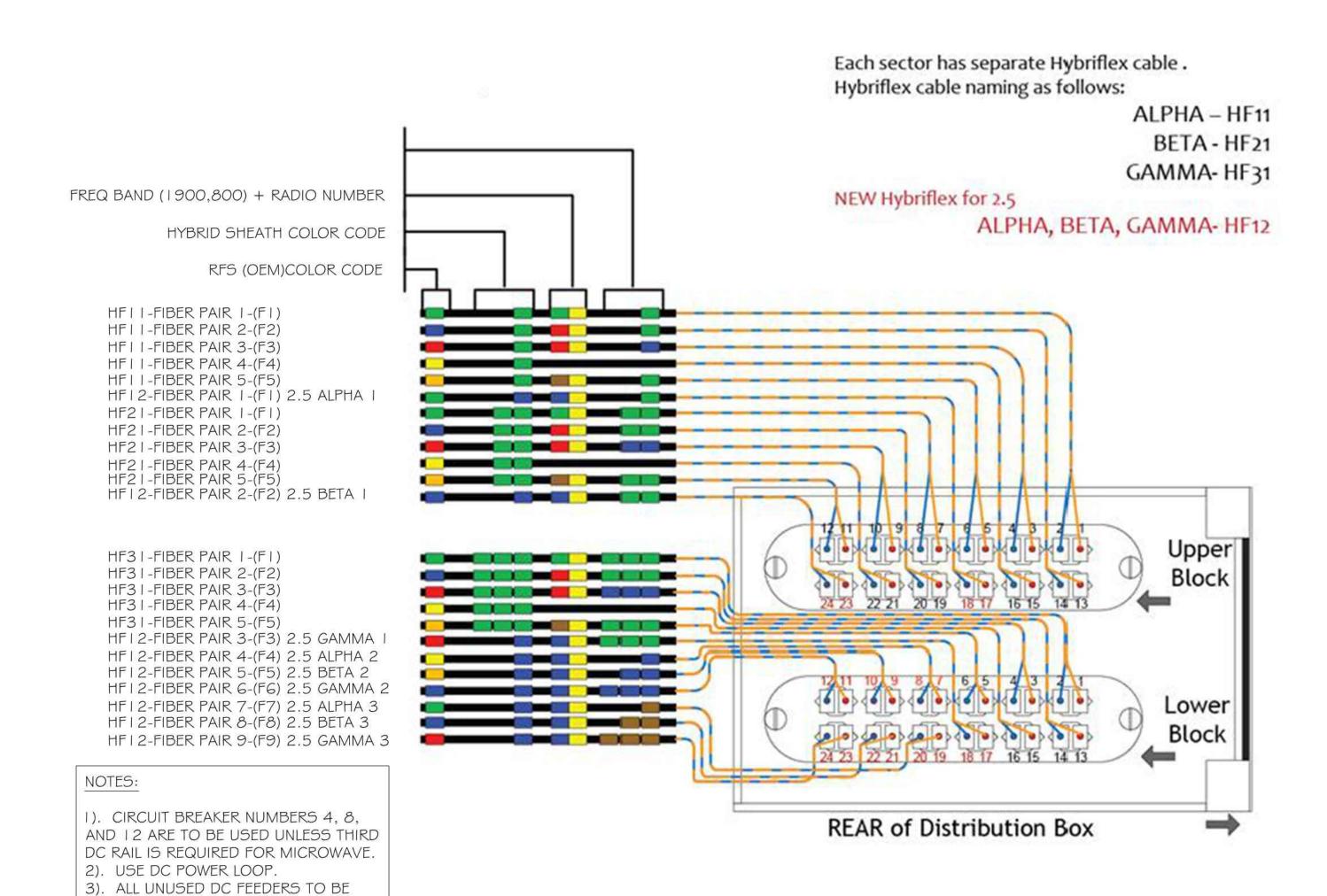
GROUNDING DETAILS

DRAWING SHEET: 9 OF 10



		A/C PAN	EL SCHEDULE		
VOLTAGE:	240V/120	PANEL STATUS:	EXISTING	N TO GROUND BOND:	YES
MAIN BREAKER:	200 AMP	MODEL NUMBER:	TBD	INTERNAL TVSS:	YES
MOUNT:	GRADE	PHASE:	1	WIRE:	3
ENCLOSURE:	NEMA 3R	BUSS RATING:	200 AMP	GROUND BAR:	YES
		NEUTRAL BAR:	YES		





SCALE: NTS

YPICAL FIBER DISTRIBUTION

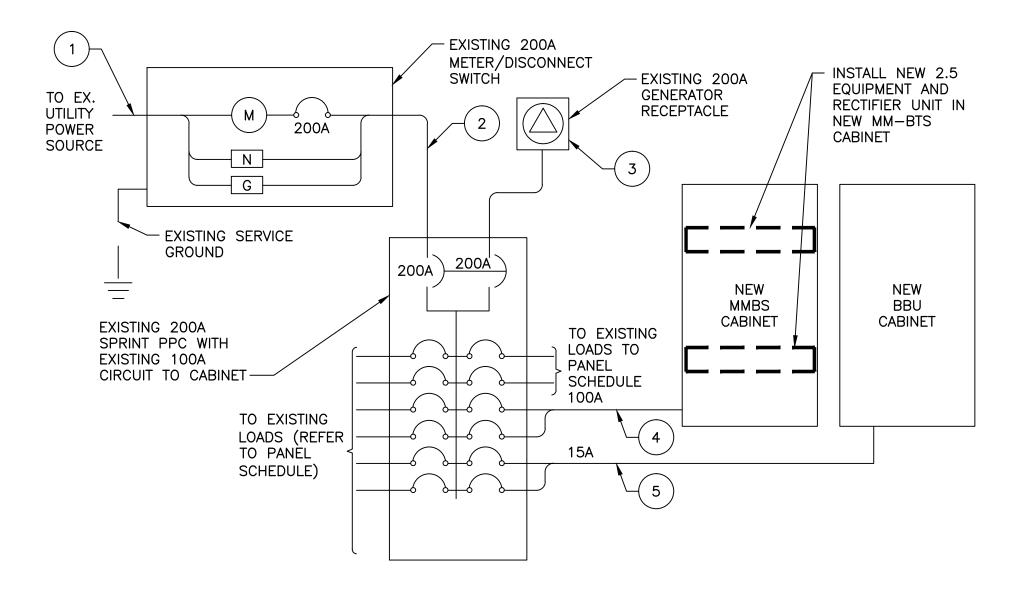
TERMINATED WITH WIRE NUTS AND

INTERIOR OF FIBER DISTRIBUTION BOX

4). REMOVE ALL DEBRIS FROM

TAPED.

WHEN COMPLETE.



<u>CIRCUIT SCHEDULE</u>					
NO.	FROM	ТО	CONFIGURATION		
1	UTILITY SOURCE	METER/ DISCONNECT	EXISTING		
2	METER/ DISCONNECT	TRANSFER & LOAD CENTER	EXISTING		
3	TRANSFER & LOAD CENTER	GENERATOR RECEPTACLE	EXISTING		
4	TRANSFER & LOAD CENTER	EX. MMBS CABINET	(3) #2 AWG, (1) #8 GND IN 1-1/2" CONDUIT		
5	TRANSFER & LOAD CENTER	EX. BBU CABINET	(2) #12 AWG, (1) #12 GND IN 3/4" CONDUIT		

ELECTRICAL ONE-LINE DIAGRAM

E-2 SCALE: NTS



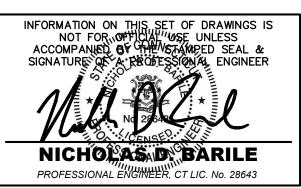




REV.	DATE	DESCRIPTION OF CHANGES	
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SCALE:	AS NOTED
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DRAWING TITLE:

DC POWER
DETAILS & PANEL
SCHEDULES

DRAWING SHEET: 10 OF 10

E-2