

Northeast Site Solutions Denise Sabo 4 Angela's Way, Burlington CT 06013 203-435-3640 denise@northeastsitesolutions.com

September 21, 2021

Members of the Siting Council Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

RE: Tower Share Application 50 Rockland Road, Norwalk CT 06854 Latitude: 41.0817889 Longitude: 73.43042222 Site# 807133 Crown Dish

#### Dear Ms. Bachman:

This letter and attachments are submitted on behalf of Dish Wireless LLC. Dish Wireless LLC plans to install antennas and related equipment to the tower site located at 50 Rockland Road, Norwalk, Connecticut.

Dish Wireless LLC proposes to install three (3) 600/1900 5G MHz antenna and six (6) RRUs, at the 181-foot level of the existing 182-foot monopole tower, one (1) Fiber cables will also be installed. Dish Wireless LLC equipment cabinets will be placed within 7x5 lease area. Included are plans by B&T Group, dated September 1, 2021 Exhibit C. Also included is a structural analysis prepared by Crown Castle, dated June 7, 2021, confirming that the existing tower is structurally capable of supporting the proposed equipment. Attached as Exhibit D. This facility was approved by Connecticut Siting Council, Docket No. 73 on April 1, 1997. Please see attached Exhibit A.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies 16-50aa, of Dish Wireless LLC intent to share a telecommunications facility pursuant to R.C.S.A. 16-50j-88. In accordance with R.C.S.A., a copy of this letter is being sent to Harry Rilling, Mayor, for the City of Norwalk, Steven Kleppin, Director of Planning, as well as the tower owner and property owner (Crown Castle)

The planned modifications of the facility fall squarely within those activities explicitly provided for in R.C.S.A. 16-50j-89.

- 1. The proposed modification will not result in an increase in the height of the existing structure. The top of the tower is 182-feet; Dish Wireless LLC proposed antennas will be located at a center line height of 181-feet.
- 2. The proposed modifications will not result in the increase of the site boundary as depicted on the attached site plan.



- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed local and state criteria. The incremental effect of the proposed changes will be negligent.
- 4. The operation of the proposed antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard. As indicated in the attached power density calculations, the combined site operations will result in a total power density of 24.00% as evidenced by Exhibit F.

Connecticut General Statutes 16-50aa indicates that the Council must approve the shared use of a telecommunications facility provided it finds the shared use is technically, legally, environmentally, and economically feasible and meets public safety concerns. As demonstrated in this letter, Dish Wireless LLC respectfully indicates that the shared use of this facility satisfies these criteria.

- A. Technical Feasibility. The existing monopole has been deemed structurally capable of supporting Dish Wireless LLC proposed loading. The structural analysis is included as Exhibit D.
- B. Legal Feasibility. As referenced above, C.G.S. 16-50aa has been authorized to issue orders approving the shared use of an existing tower such as this support tower in Norwalk. Under the authority granted to the Council, an order of the Council approving the requested shared use would permit Dish Wireless LLC to obtain a building permit for the proposed installation. Further, a Letter of Authorization is included as Exhibit G, authorizing Dish Wireless LLC to file this application for shared use.
- C. Environmental Feasibility. The proposed shared use of this facility would have a minimal environmental impact. The installation of Dish Wireless LLC equipment at the 181-foot level of the existing 182-foot tower would have an insignificant visual impact on the area around the tower. Dish Wireless LLC ground equipment would be installed within the existing facility compound. Dish Wireless LLC shared use would therefore not cause any significant alteration in the physical or environmental characteristics of the existing site. Additionally, as evidenced by Exhibit F, the proposed antennas would not increase radio frequency emissions to a level at or above the Federal Communications Commission safety standard.
- D. Economic Feasibility. Dish Wireless LLC will be entering into an agreement with the owner of this facility to mutually agreeable terms. As previously mentioned, the Letter of Authorization has been provided by the owner to assist Dish Wireless LLC with this tower sharing application.
- E. Public Safety Concerns. As discussed above, the tower is structurally capable of supporting Dish Wireless LLC proposed loading. Dish Wireless LLC is not aware of any public safety concerns relative to the proposed sharing of the existing tower. Dish Wireless LLC intentions of providing new and improved wireless service through the shared use of this facility is expected to enhance the safety and welfare of local residents and individuals traveling through Norwalk.

Sincerely,

#### Denise Sabo

Denise Sabo

Mobile: 203-435-3640 Fax: 413-521-0558

Office: 4 Angela's Way, Burlington CT 06013 Email: denise@northeastsitesolutions.com



Attachments cc: Harry Rilling, Mayor City of Norwalk 125 East Avenue Norwalk, CT 06851

Steven Kleppin, Director of Planning City of Norwalk Planning & Zoning 125 East Avenue Norwalk, CT 06851

Crown Castle, Property and Tower Owner

# Exhibit A

**Original Facility Approval** 

#### DOCKET NO. 73

AN APPLICATION OF METRO MOBILE CTS OF FAIRFIELD COUNTY, INC., FOR CERTIFICATES OF ENVIRONMENTAL COMPATIBILITY AND PUBLIC NEED FOR THE CONSTRUCTION, MAINTENANCE, AND OPERATION OF THREE FACILITIES CONSISTING OF TELECOMMUNICATIONS TOWERS AND ASSOCIATED EQUIPMENT FOR THE PURPOSE OF PROVIDING DOMESTIC PUBLIC CELLULAR RADIO TELECOMMUNICATIONS SERVICE IN THE TOWN OF GREENWICH AND IN THE CITIES OF NORWALK AND STAMFORD, CONNECTICUT.

: CONNECTICUT SITING
COUNCIL

:

April 1, 1987

#### DECISION AND ORDER

Pursuant to the foregoing opinion, the Connecticut Siting Council (Council) hereby directs that a Certificate of Environmental Compatibility and Public Need, as provided by Section 16-50k of the General Statutes of Connecticut (CGS), be issued to Metro Mobile CTS of Fairfield County, Inc., for the construction, operation, and maintenance of cellular mobile telecommunications equipment in the Town of Greenwich, and the Cities of Norwalk and Stamford, Connecticut.

The facilities shall be constructed, operated, and maintained as specified in the Council's record on this matter, and subject to the following conditions.

- The Norwalk tower, including antennas, shall be no taller than necessary to provide the proposed service, and in no event shall exceed 193 feet.
- A fence not lower than eight feet shall surround the Norwalk tower.
- Unless necessary to comply with condition number four, below, no lights shall be installed on the Norwalk tower.
- 4. The facilities shall be constructed in accordance with all applicable federal, state, and municipal laws and regulations.

- 5. The certificate holder shall prepare a development and management (D&M) plan for the Norwalk site in compliance with sections 16-50j-75 through 16-50j-77 of the Regulations of State Agencies. The D&M plan shall provide for evergreen screening around the perimeter of the fence at this site, and for other landscaping to improve the appearance of the facility.
- 6. The receive antennas at the Greenwich and Stamford sites shall be mounted below the high points of the facades of their respective buildings to minimize their visibility.
- 7. No construction activities shall take place outside the hours of 7:00 A.M. to 7:00 P.M., Monday through Saturday.
- 8. The certificate holder or its successor shall notify the Council if and when directional antennas or any equipment other than that listed in this application is added to these facilities.
- 9. The certificate holder or its successor shall permit public or private entities to share space on the Norwalk tower, for due consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing.
- 10. If these facilities do not provide or permanently cease to provide cellular service following completion of construction, this Decision and Order shall be void, and the tower and all associated equipment in this application shall be dismantled and removed or reapplication for any new use shall be made to the Council before any such new use is made.

- 11. Unless otherwise approved by the Council, this Decision and Order shall be void if all construction authorized herein is not completed within three years of the issuance of this Decision and Order, or within three years of the completion of any appeal taken in this Decision.
- 12. The certificate holder shall comply with any future radio frequency (RF) standards promulgated by state or federal regulatory agencies. Upon the establishment of any new governmental RF standards, the facilities granted in this Decision shall continue to be in compliance with such standards.

Pursuant to CGS section 16-50p, we hereby direct that a copy of the Decision and Order be served on each person listed below. A notice of the issuance shall be published in the Stamford Advocate, the Greenwich Times, the Norwalk Hour, and the Bridgeport Post.

The parties to the proceeding are:

Mr. Armand Mascioli General Manager Metro Mobile CTS of Fairfield County, Inc. 5 Eversley Avenue Norwalk, Connecticut 06855

(its attorney)

(Applicant)

Howard L. Slater, Esquire Byrne, Slater, Sandler, Shulman & Rouse, P.C. 330 Main Street P.O. Box 3216 Hartford, Connecticut 06103

(its attorney)

Richard Rubin, Esquire Fleischman and Walsh, P.C. 1725 N Street, N.W. Washington, D.C. 20036 Southern New England Telephone Company (its attorney)

Mr. Peter J. Tyrrell Senior Attorney Southern New England Telephone Company 227 Church Street New Haven, Connecticut 06506

#### CERTIFICATION

The undersigned members of the Connecticut Siting Council hereby certify that they have heard this case or read the record thereof, and that we voted as follows:

Dated at New Britain, Connecticut, this 1st day of April, 1987.

Council Members	Vote Cast
Gloria Dibble Pond, Chairperson	Yes
Commissioner John Downey Designee: Commissioner Peter G. Boucher	Yes
Acting Commissioner John Anderson Designee: Brian Emerjok	Yes
Quen L. Clark	Yes
Fred J. Doocy	Yes
Mortimer A. Gelston	Yes
James G. Horsfall	Absent
William H. Smith	Absent
Colin C. Tait	Yes

STATE OF CONNECTICUT	)					
	:	SS.	New Britain,	April	1,	1987
COUNTY OF HARTFORD	)		- A	17.	- 55	

I hereby certify that the foregoing is a true and correct copy of the decision and order issued by the Connecticut Siting Council, State of Connecticut.

ATTEST:

Executive Director

Connecticut Siting Conneil

I certify that a copy of the opinion and decision and order have been forwarded by mail to all parties of record on April 3, 1987.

ATTEST:

Robert K. Erling Siting Analyst

Connecticut Siting Council

# Exhibit B

**Property Card** 

#### **50 ROCKLAND RD**

**Location** 50 ROCKLAND RD **Mblu** 5/ 82/ 58/ 0/

Acct# 25665 Owner CROWN ATLANTIC COMPANY

LLC

**Assessment** \$3,369,900 **Appraisal** \$4,814,150

PID 25665 Building Count 1

#### **Current Value**

Appraisal					
Valuation Year	Improvements	Land	Total		
2018	\$4,161,500	\$652,650	\$4,814,150		
	Assessment				
Valuation Year	Improvements	Land	Total		
2018	\$2,913,040	\$456,860	\$3,369,900		

#### **Owner of Record**

Owner CROWN ATLANTIC COMPANY LLC Sale Price \$1,600,000

Co-Owner Certificate

Address PMB 353 Book & Page 3701/331

4017 WASHINGTON RD Sale Date 04/16/1999 McMURRAY, PA 15317-0000

#### **Ownership History**

Ownership History							
Owner	Sale Price	Certificate	Book & Page	Sale Date			
CROWN ATLANTIC COMPANY LLC	\$1,600,000		3701/331	04/16/1999			
CELLCO PARTNERSHIP,	\$1,020,000		3489/348	04/03/1998			
DEVIVO MARIO + WENCHE	\$0		0/0				

#### **Building Information**

#### **Building 1: Section 1**

 Year Built:
 1987

 Living Area:
 21,115

 Replacement Cost:
 \$1,257,359

**Building Percent Good:** 66

**Replacement Cost** 

Less Depreciation: \$829,860

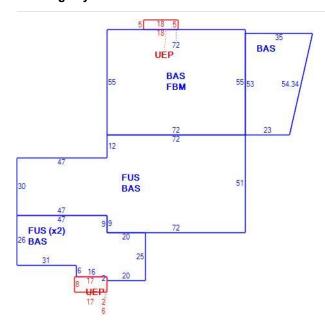
Building	Attributes
Field	Description
STYLE	Light Indust
MODEL	Industrial
Grade	C+
Stories:	3.00
Occupancy	1.00
Exterior Wall 1	Concrete
Exterior Wall 2	
Roof Structure	Flat
Roof Cover	Rolled Compos
Interior Wall 1	Drywall
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Concrete
Heating Fuel	Gas
Heating Type	Forced Air
AC Percent	60
Heat Percent	100
Bldg Use	Industrial
Total Rooms	0
Bedrooms	0
Full Baths	0
Half Baths	6
Extra Fixtures	0
FBM Area	
Heat/AC	Heat/AC Pkg
Frame	Masonry
Plumbing	Average
Foundation	Slab
Partitions	Average
Wall Height	13.00
% Sprinkler	40.00

#### **Building Photo**



(http://images.vgsi.com/photos/NorwalkCTPhotos//00\00\72/74.jpg)

#### **Building Layout**



(ParcelSketch.ashx?pid=25665&bid=25665)

	Building Sub-Areas (sq ft)				
Code	Description	Gross Area	Living Area		
BAS	First Floor	12,397	12,397		
FUS	Finished Upper Story	8,718	8,718		
FBM	Finished Basement	3,960	0		
UEP	Utility Enclosed Porch	226	0		
		25,301	21,115		

#### **Extra Features**

Extra Features <u>Lege</u>						
Code	Description	Size	Value	Bldg #		
ELV1	Commercial	3.00 STOP	\$56,250	1		

A/C	Air Conditioning	12669.00 S.F.	\$38,010	1
SPR	Sprinklers	8446.00 S.F.	\$31,670	1

#### Land

Land Use Land Line Valuation		ation	
Use Code	301	Size (Acres)	0.82
Description	Industrial	Frontage	
Zone	RI	Depth	
Neighborhood	C530	Assessed Value	\$456,860
		Appraised Value	\$652.650

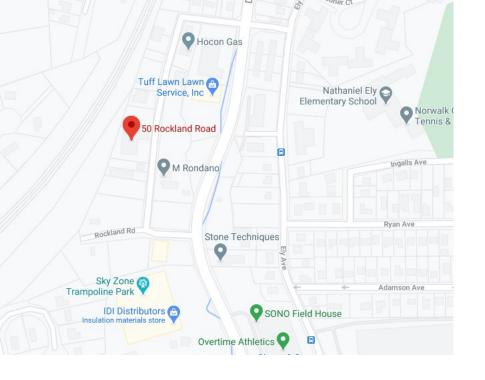
#### Outbuildings

Outbuildings						<u>Legend</u>
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
PAV1	Paving Asph.			16900.00 S.F.	\$21,970	1
FN6	Fence 6'			450.00 L.F.	\$4,090	1
SHD4	Cell Equip	FR	Frame	128.00 S.F.	\$12,800	1
CEL1	Cell Tower		Steel	5.00 UNITS	\$750,000	1
SHD4	Cell Equip	FR	Frame	128.00 S.F.	\$12,800	1

#### **Valuation History**

Appraisal						
Valuation Year	Improvements	Land	Total			
2018	\$4,161,500	\$652,650	\$4,814,150			
2017	\$991,370	\$447,530	\$1,438,900			
2016	\$991,370	\$447,530	\$1,438,900			

Assessment						
Valuation Year	Improvements	Land	Total			
2018	\$2,913,040	\$456,860	\$3,369,900			
2017	\$693,970	\$313,270	\$1,007,240			
2016	\$693,970	\$313,270	\$1,007,240			



# Exhibit C

**Construction Drawings** 

# O i s wireless...

DISH Wireless L.L.C. SITE ID:

#### NJJER01090A

DISH Wireless L.L.C. SITE ADDRESS:

## **50 ROCKLAND ROAD** NORWALK, CT 06854

#### CONNECTICUT CODE COMPLIANCE

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

2018 CT STATE BUILDING CODE/2015 IBC W/ CT AMENDMENTS MECHANICAL ELECTRICAL 2018 CT STATE BUILDING CODE/2015 IMC W/ CT AMENDMENTS
2018 CT STATE BUILDING CODE/2017 NEC W/ CT AMENDMENTS

	SHEET INDEX
SHEET NO.	SHEET TITLE
T-1	TITLE SHEET
A-1	OVERALL AND ENLARGED SITE PLAN
A-2	ELEVATION, ANTENNA LAYOUT AND SCHEDULE
A-3	EQUIPMENT PLATFORM AND H-FRAME DETAILS
A-4	EQUIPMENT DETAILS
A-5	EQUIPMENT DETAILS
A-6	EQUIPMENT DETAILS
E-1	ELECTRICAL/FIBER ROUTE PLAN AND NOTES
E-2	ELECTRICAL DETAILS
E-3	ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE
G-1	GROUNDING PLANS AND NOTES
G-2	GROUNDING DETAILS
G-3	GROUNDING DETAILS
RF-1	RF CABLE COLOR CODE
GN-1	LEGEND AND ABBREVIATIONS
GN-2	GENERAL NOTES
GN-3	GENERAL NOTES
GN-4	GENERAL NOTES

#### SCOPE OF WORK

THIS IS NOT AN ALL INCLUSIVE LIST. CONTRACTOR SHALL UTILIZE SPECIFIED EQUIPMENT PART OR ENGINEER APPROVED EQUIPMENT. CONTRACTOR SHALL VERIFY ALL NEEDED EQUIPMENT TO PROVIDE A FUNCTIONAL SITE. THE PROJECT GENERALLY CONSISTS OF THE FOLLOWING:

TOWER SCOPE OF WORKS

REMOVE EMPTY MOUNT AT 178'-0" CL

INSTALL (3) PROPOSED PANEL ANTENNAS (1 PER SECTOR) INSTALL (3) PROPOSED ANTENNA MOUNTS (1 PER SECTOR)

INSTALL PROPOSED JUMPERS

INSTALL (6) PROPOSED RRUS (2 PER SECTOR)

INSTALL (1) PROPOSED OVER VOLTAGE PROTECTION DEVICE (OVP) INSTALL (1) PROPOSED HYBRID CABLE

GROUND SCOPE OF WORK:
REMOVE ABANDONED CONCRETE PAD

INSTALL (1) PROPOSED METAL PLATFORM INSTALL (1) PROPOSED PPC CABINET

PROPOSED EQUIPMENT CABINET

INSTALL (1) PROPOSED POWER CONDUIT INSTALL (1) PROPOSED TELCO CONDUIT

INSTALL (1 PROPOSED TELCO-FIBER BOX

INSTALL (1) PROPOSED GPS UNIT

PROPOSED SAFETY SWITCH (IF REQUIRED) INSTALL (1) PROPOSED FIBER NID (IF REQUIRED)

#### SITE PHOTO





**UNDERGROUND SERVICE ALERT CBYD 811** UTILITY NOTIFICATION CENTER OF CONNECTICUT (800) 922-4455 WWW.CBYD.COM

CALL 2 WORKING DAYS UTILITY NOTIFICATION PRIOR TO CONSTRUCTION

#### **GENERAL NOTES**

THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON DRAINAGE. NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.

#### 11"x17" PLOT WILL BE HALF SCALE UNLESS OTHERWISE NOTED

CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, AND CONDITIONS ON THE JOB SITE, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK.

#### SITE INFORMATION PROJECT DIRECTORY PROPERTY OWNER: CONNECTICUT LIGHT & POWER DISH Wireless L.L.C. ADDRESS: PO BOX 2957 NORTHEAST UTILITIES, HARTFORD, CT 5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120 TOWER TYPE: SELF-SUPPORT TOWER TOWER OWNER: CROWN CASTLE TOWER CO SITE ID: 807133 2000 CORPORATE DRIVE CANONSBURG, PA 15317 TOWER APP NUMBER: 548868 (877) 486-9377 SITE DESIGNER: B+T GROUP COUNTY: FAIRFIELD 1717 S. BOULDER AVE, SUITE 300 TULSA, OK 74119 LATITUDE (NAD 83): 41° 4' 54 44" N 41.08178889 N (918) 587-4630 LONGITUDE (NAD 83): 73° 25' 49.52" W 73.43042222 W ZONING JURISDICTION: CONNECTICUT SITING COUNCIL SITE ACQUISITION: NICHOLAS CURRY NICHOLAS.CURRY OCROWN.CASTLE.COM ZONING DISTRICT: MICHAEL NARDUCCI CONSTRUCTION MANAGER: NORW-000005-000082-000058 MICHAEL.NARDUCCI PARCEL NUMBER: MURUGABIRAN JAYAPAL OCCUPANCY GROUP: RF ENGINEER: MURUGABIRAN.JAYAPAL CONSTRUCTION TYPE: NORTHEAST UTILITIES

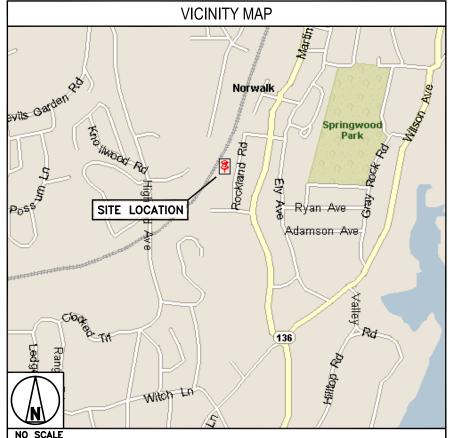
#### **DIRECTIONS**

DIRECTIONS FROM 3 ADP BLVD ROSELAND, NJ:

GET ON 1-280 W FROM LIVINGSTON AVE

TELEPHONE COMPANY: T.B.D.

TAKE I-287 N AND I-95 N TO CT-136 E/TOKENEKE RD IN DARIEN. TAKE EXIT 12 FROM I-95 N CONTINUE ON CT-136 E. TAKE WITCH LN TO ROCKLAND RD IN NORWALK





5701 SOUTH SANTA FF DRIVE LITTLETON, CO 80120



2000 CORPORATE DRIVE CANONSBURG, PA 15317





B&T ENGINEERING, INC. PEC.0001564 Expires 2/10/22

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

DRAWN	BY:	CHECKED	BY:	APPROVED	BY:
LHT		YXI		MDW	

RFDS REV #:

#### CONSTRUCTION **DOCUMENTS**

	SUBMITTALS								
ı	REV	DATE	DESCRIPTION						
ı	A	6/16/21	ISSUED FOR REVIEW						
ı	0	7/29/21	ISSUED FOR REVIEW						
ı	1	9/1/21	ISSUED FOR CONSTRUCTION						
ı									
ı									
ı									

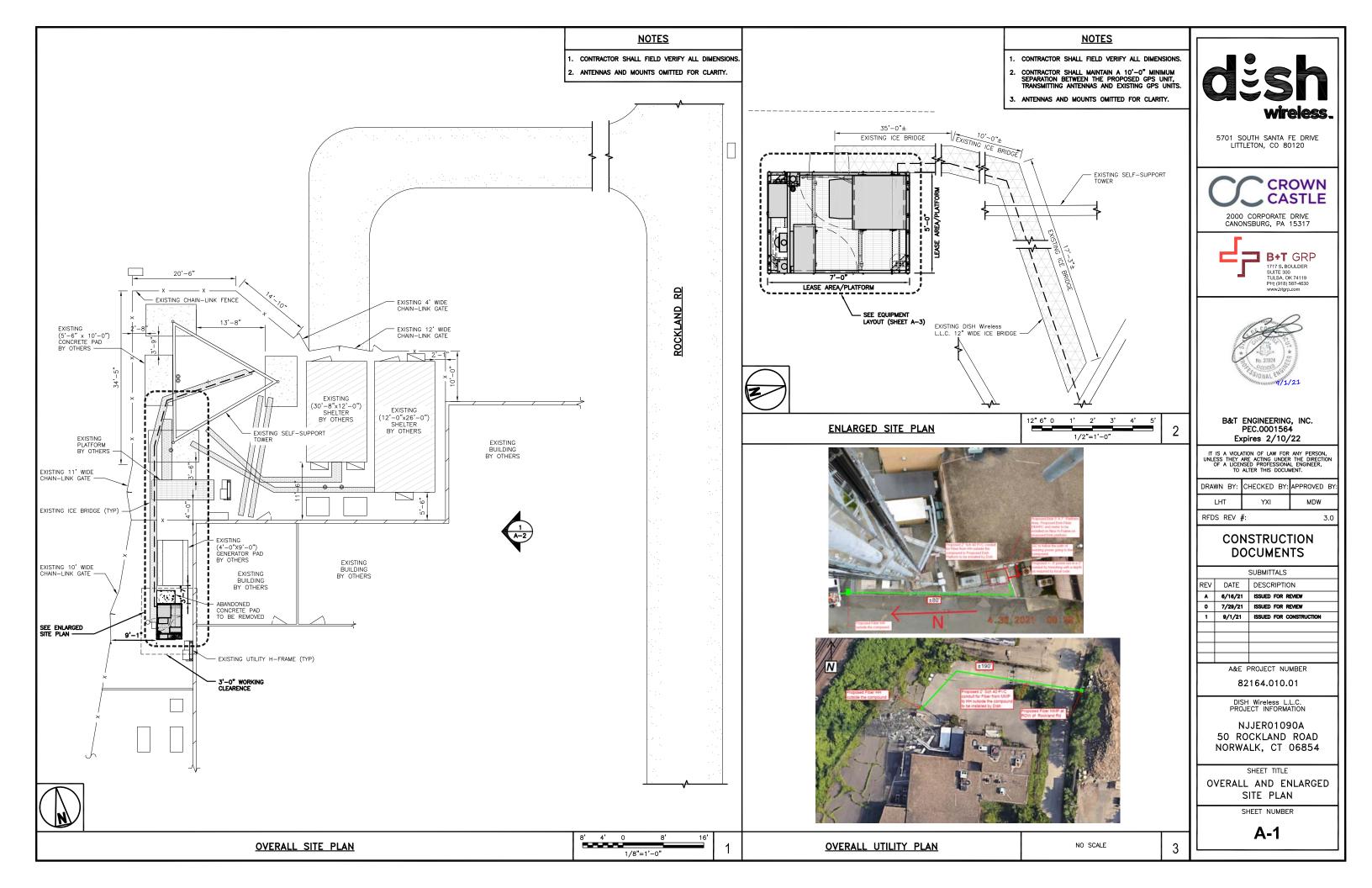
A&E PROJECT NUMBER 82164.010.01

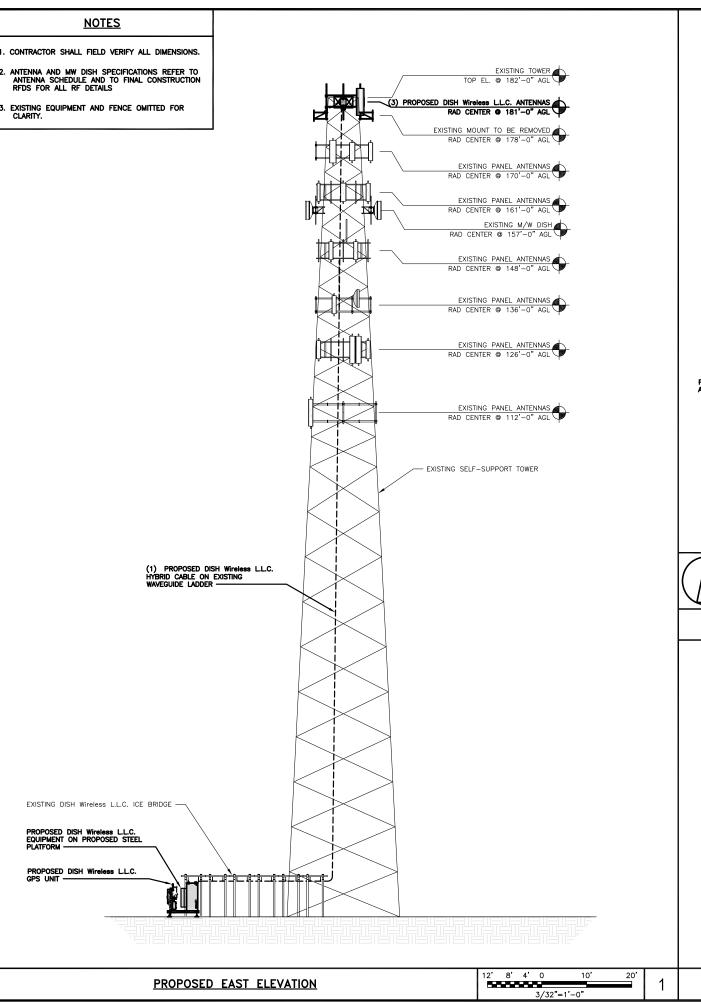
50 ROCKLAND ROAD NORWALK, CT 06854

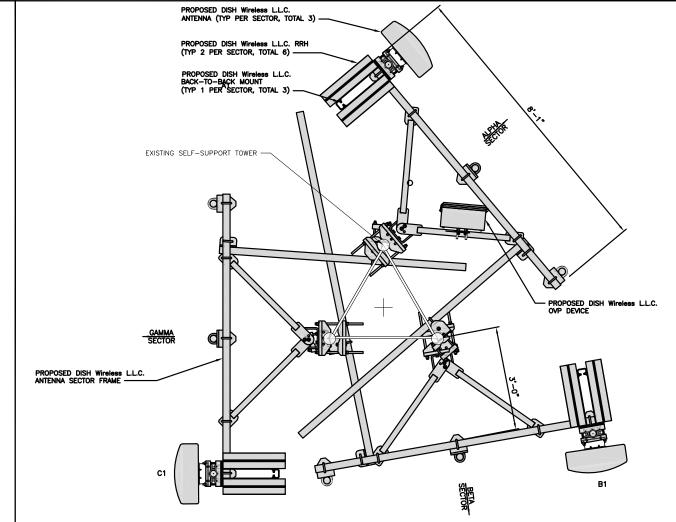
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SHEET NUMBER

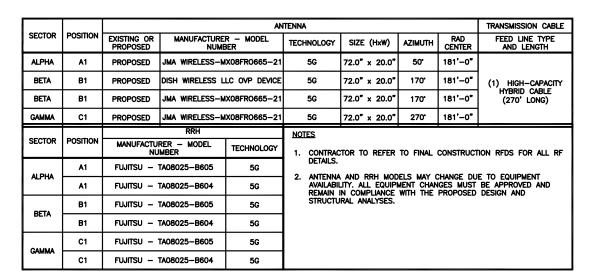
T-1







ANTENNA LAYOUT





5701 SOUTH SANTA FE DRIVE LITTLETON, CO 80120



2000 CORPORATE DRIVE CANONSBURG, PA 15317





B&T ENGINEERING, INC. PEC.0001564 Expires 2/10/22

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

П	DRAWN	BY:	CHECKED	BY:	APPROVED	BY:
	LHT		YXI		MDW	

3.0

RFDS REV #:

## CONSTRUCTION DOCUMENTS

	SUBMITTALS						
REV	DATE	DESCRIPTION					
A	6/16/21	ISSUED FOR REVIEW					
0	7/29/21	ISSUED FOR REVIEW					
1	9/1/21	ISSUED FOR CONSTRUCTION					

A&E PROJECT NUMBER

82164.010.01

DISH Wireless L.L.C. PROJECT INFORMATION

NJJER01090A 50 ROCKLAND ROAD NORWALK, CT 06854

SHEET TITLE

ELEVATION, ANTENNA LAYOUT AND SCHEDULE

SHEET NUMBER

**A-2** 

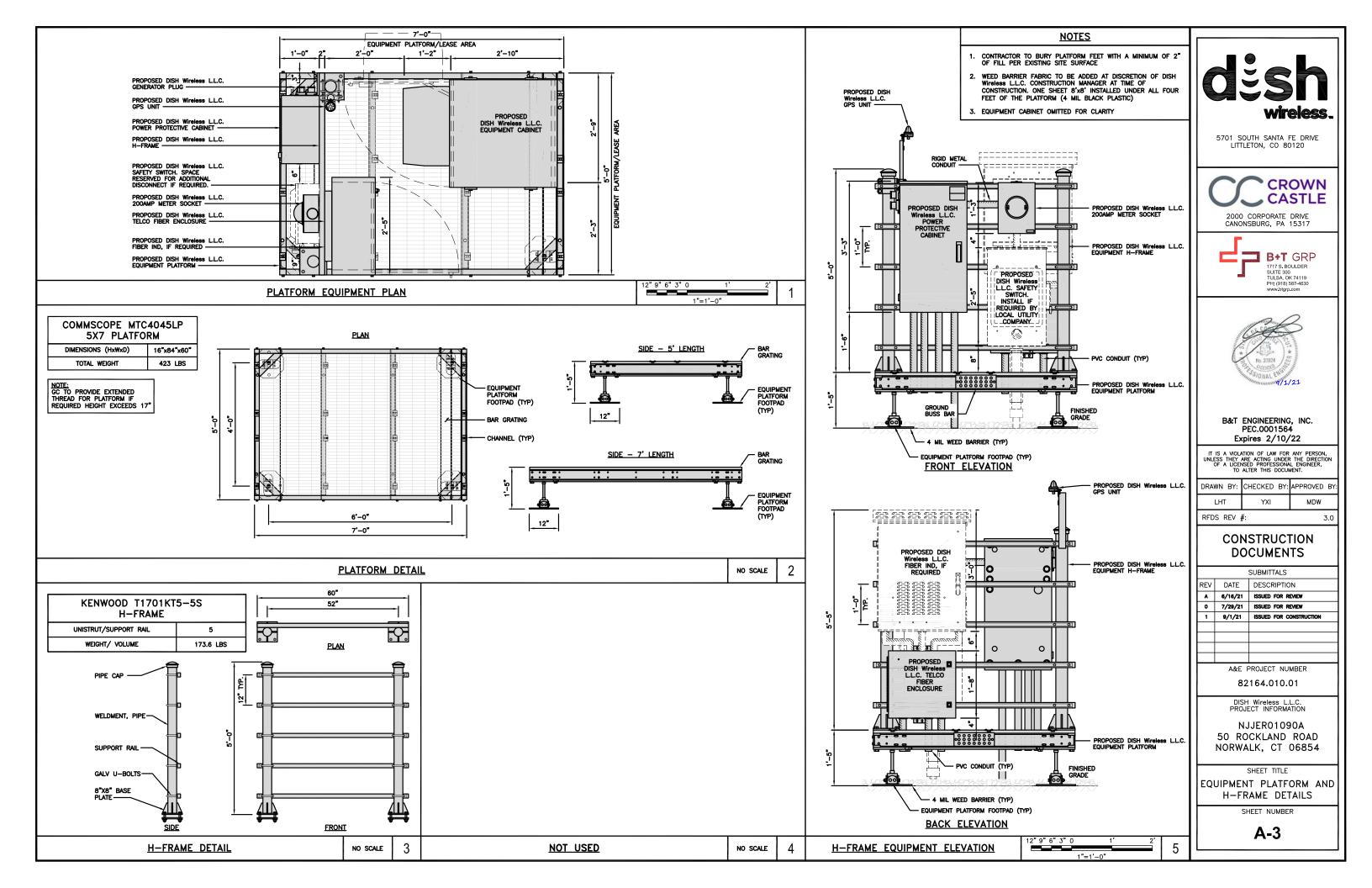
10' 20' ANTENNA SCHEDULE

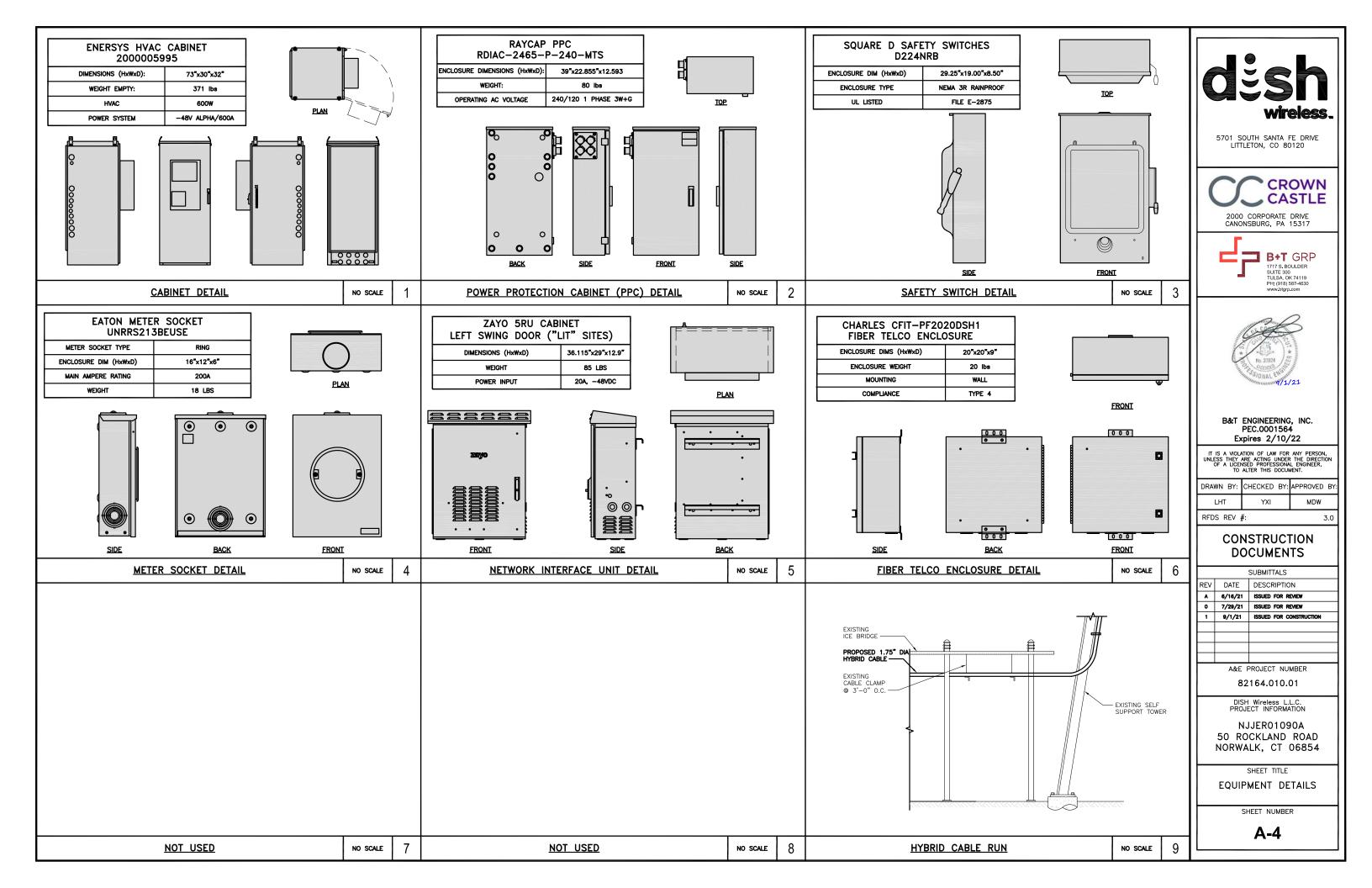
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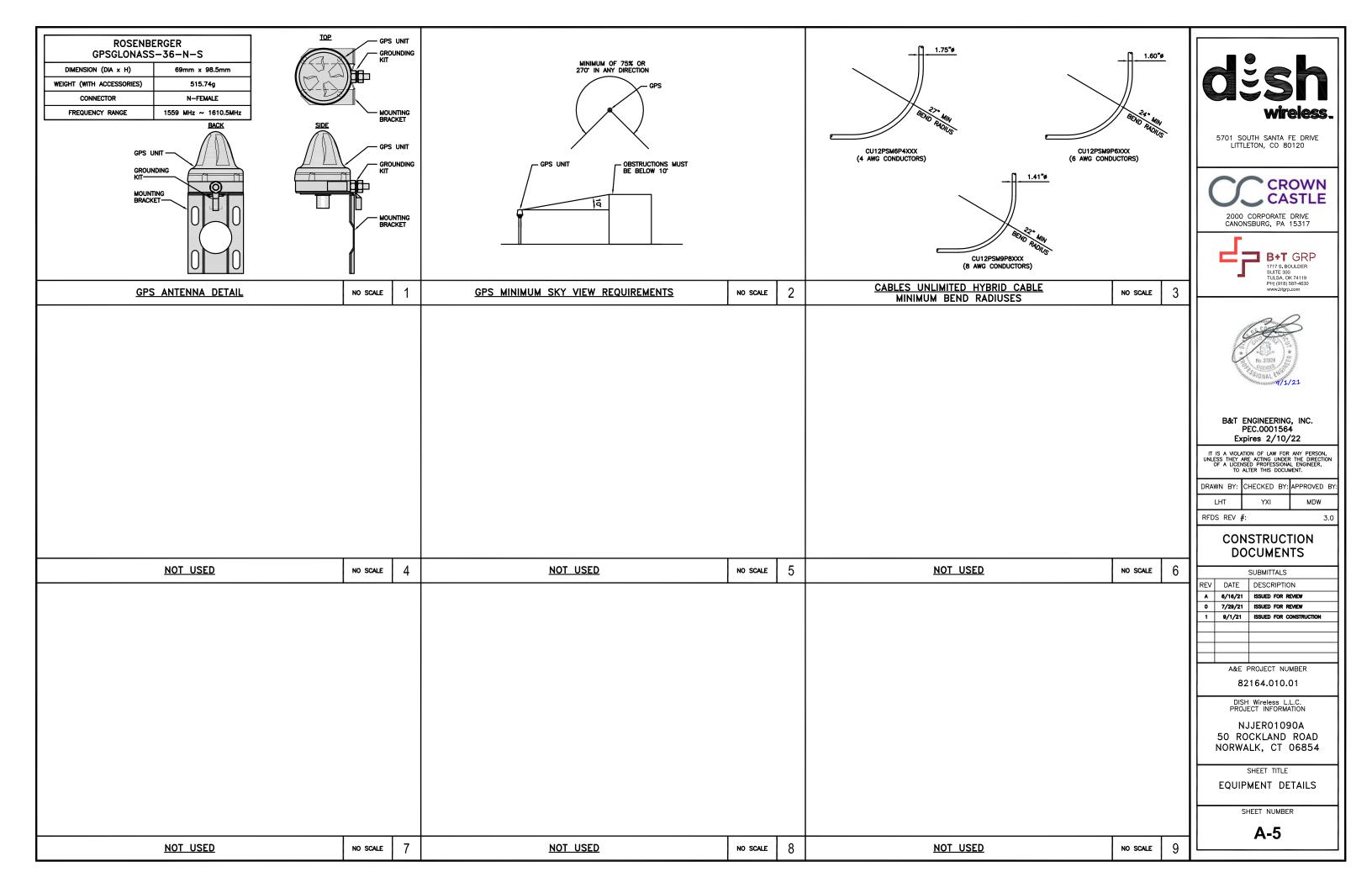
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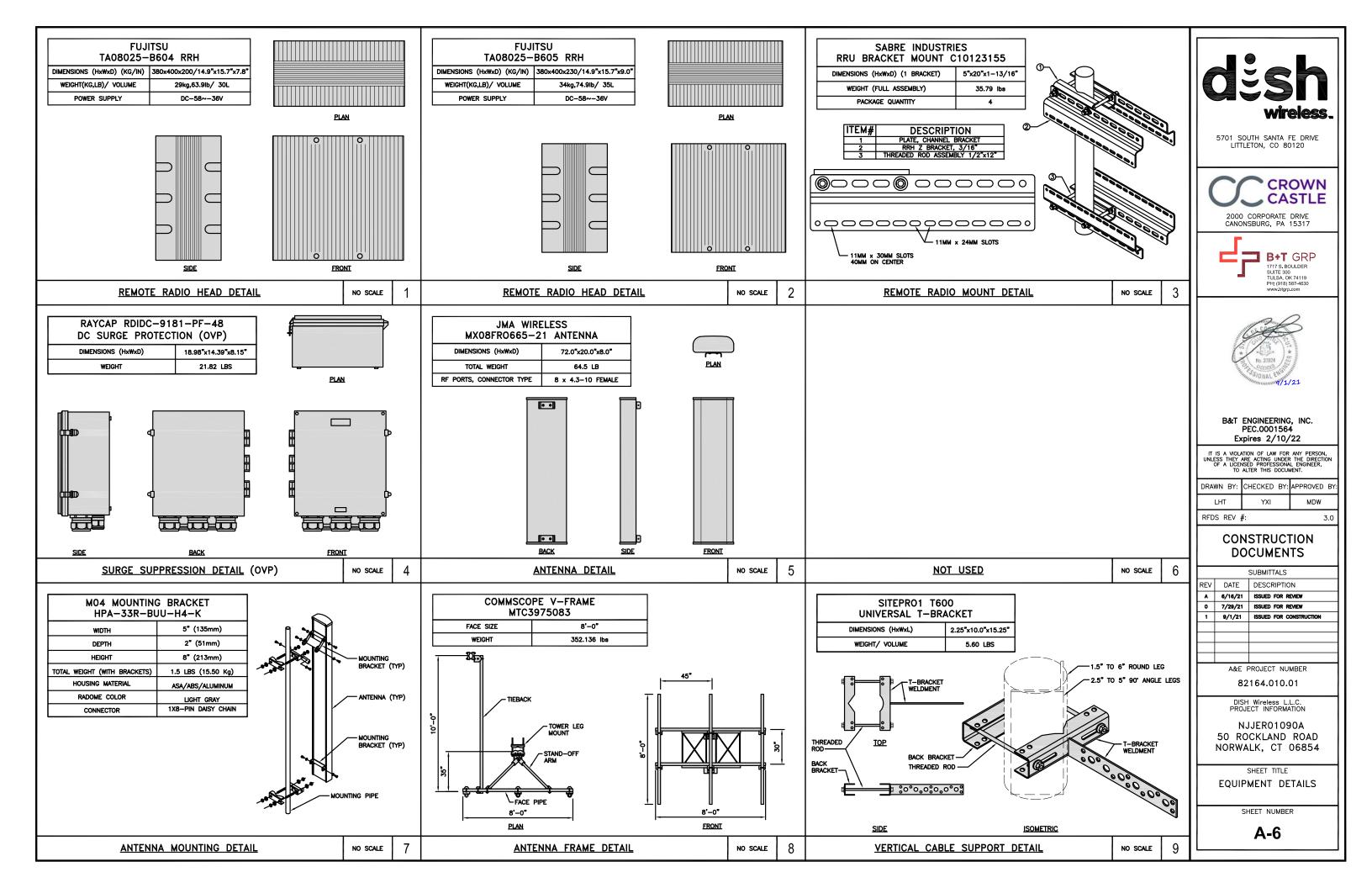
3/4"=1'-0"

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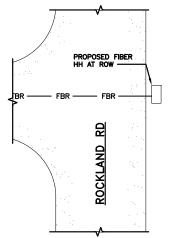






#### **NOTES**

- CONTRACTOR SHALL FIELD VERIFY ALL PROPOSED UNDERGROUND UTILITY CONDUIT ROUTE.
- ANTENNAS AND MOUNTS OMITTED FOR CLARITY.
- THE GROUND LEASE PROVIDES BROAD/BLANKET UTILITY RIGHTS. "PWR" AND "FBR" PATH DEPICTED ON A-1 AND E-1 ARE BASED ON BEST AVAILABLE INFORMATION INCLUDING BUT NOT LIMITED TO FIELD VERIFICATION, PRIOR PROJECT LIMITED TO FILELLY VERTICATION, PRIOR PROJECT DOCUMENTATION AND OTHER REAL PROPERTY RIGHTS DOCUMENTS. WHEN INSTALLING THE UTILITIES PLEASE LOCATE AND FOLLOW EXISTING PATH. IF REVISTING PATH. IS NOT AN OPTION, PLEASE NOTIFY CROWN CASTLE REAL ESTATE AS FURTHER COORDINATION MAY BE NEEDED.



DC POWER WIRING SHALL BE COLOR CODED AT EACH END FOR IDENTIFYING  $\pm 24$ V and  $\pm 48$ V conductors. RED MARKINGS SHALL IDENTIFY  $\pm 24$ V and blue markings shall identify  $\pm 48$ V.

- CONTRACTOR SHALL INSPECT THE EXISTING CONDITIONS PRIOR TO SUBMITTING A BID. ANY QUESTIONS ARISING DURING THE BID PERIOD IN RECARDS TO THE CONTRACTOR'S FUNCTIONS, THE SCOPE OF WORK, OR ANY OTHER ISSUE RELATED TO THIS PROJECT SHALL BE BROUGHT UP DURING THE BID PERIOD WITH THE PROJECT MANAGER FOR CLARIFICATION, NOT AFTER THE CONTRACT HAS BEEN AWARDED.
- ALL ELECTRICAL WORK SHALL BE DONE IN ACCORDANCE WITH CURRENT NATIONAL ELECTRICAL CODES AND ALL STATE AND LOCAL CODES, LAWS, AND ORDINANCES. PROVIDE ALL COMPONENTS AND WIRING SIZES AS REQUIRED TO MEET NEC STANDARDS.
- 3. LOCATION OF EQUIPMENT, CONDUIT AND DEVICES SHOWN ON THE DRAWINGS ARE APPROXIMATE AND SHALL BE COORDINATED WITH FIELD CONDITIONS PRIOR TO CONSTRUCTION.
- 4. CONDUIT ROUGH—IN SHALL BE COORDINATED WITH THE MECHANICAL EQUIPMENT TO AVOID LOCATION CONFLICTS. VERIFY WITH THE MECHANICAL EQUIPMENT CONTRACTOR AND COMPLY AS REQUIRED.
- 5. CONTRACTOR SHALL PROVIDE ALL BREAKERS, CONDUITS AND CIRCUITS AS REQUIRED FOR A COMPLETE SYSTEM.
- 6. CONTRACTOR SHALL PROVIDE PULL BOXES AND JUNCTION BOXES AS REQUIRED BY THE NEC ARTICLE 314.
- 7. CONTRACTOR SHALL PROVIDE ALL STRAIN RELIEF AND CABLE SUPPORTS FOR ALL CABLE ASSEMBLIES. INSTALLATION SHALL BE IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS.
- 8. ALL DISCONNECTS AND CONTROLLING DEVICES SHALL BE PROVIDED WITH ENGRAVED PHENOLIC NAMEPLATES INDICATING EQUIPMENT CONTROLLED, BRANCH CIRCUITS INSTALLED ON, AND PANEL FIELD LOCATIONS FED FROM.
- INSTALL AN EQUIPMENT GROUNDING CONDUCTOR IN ALL CONDUITS PER THE SPECIFICATIONS AND NEC 250.
  THE EQUIPMENT GROUNDING CONDUCTORS SHALL BE BONDED AT ALL JUNCTION BOXES, PULL BOXES, AND ALL
  DISCONNECT SWITCHES, AND EQUIPMENT CABINETS.
- 10. ALL NEW MATERIAL SHALL HAVE A U.L. LABEL.
- 11. PANEL SCHEDULE LOADING AND CIRCUIT ARRANGEMENTS REFLECT POST-CONSTRUCTION EQUIPMENT.
- 12. CONTRACTOR SHALL BE RESPONSIBLE FOR AS-BUILT PANEL SCHEDULE AND SITE DRAWINGS.
- 13. ALL TRENCHES IN COMPOUND TO BE HAND DUG







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A&E PROJECT NUMBER

82164.010.01

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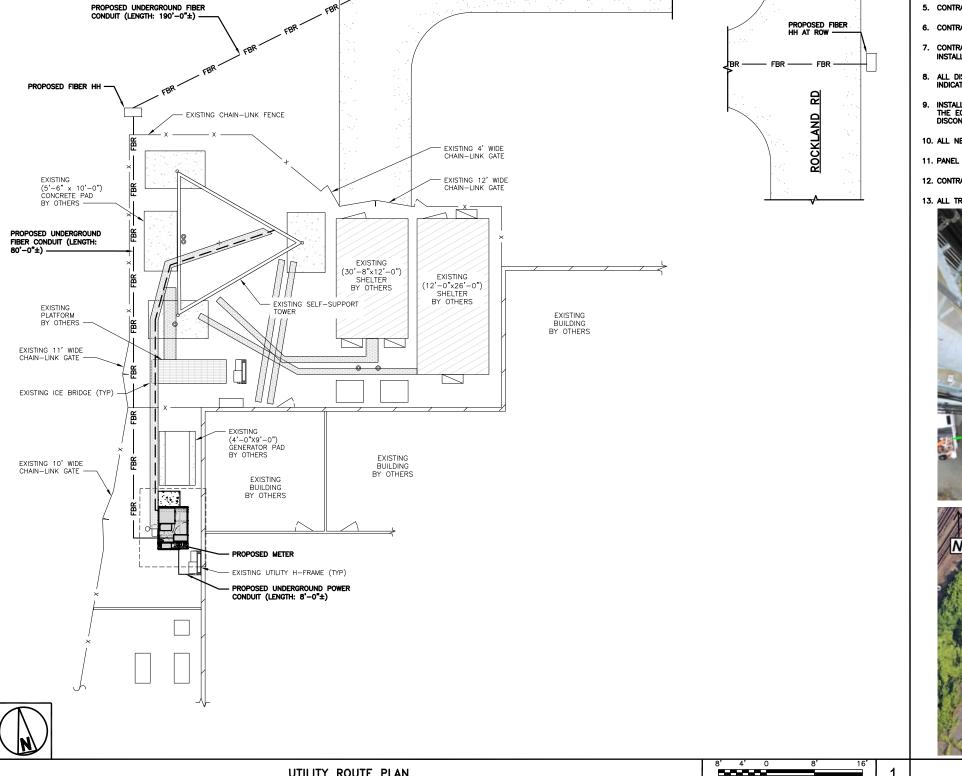
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SHEET TITLE

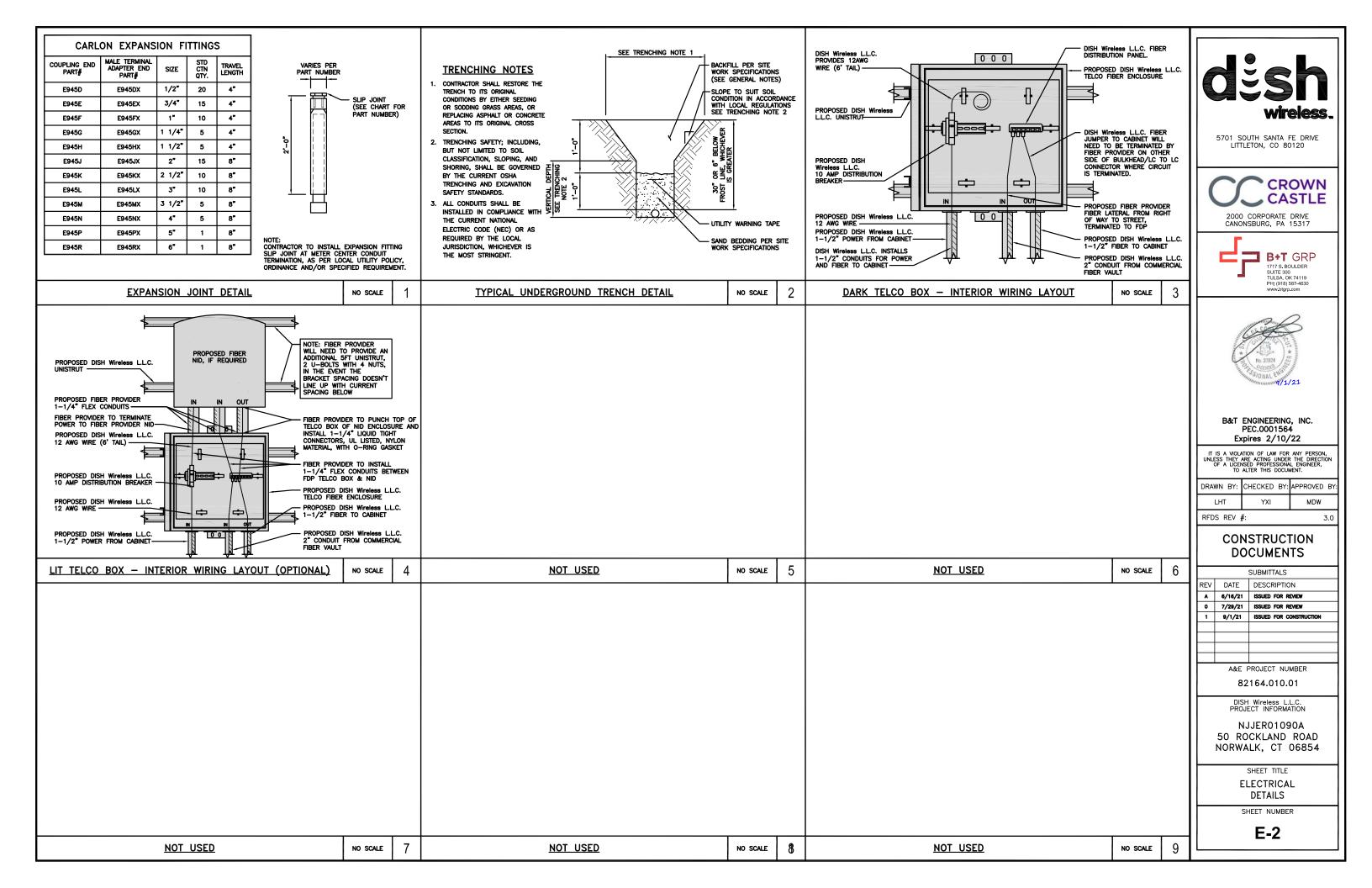
ELECTRICAL/FIBER ROUTE PLAN AND NOTES

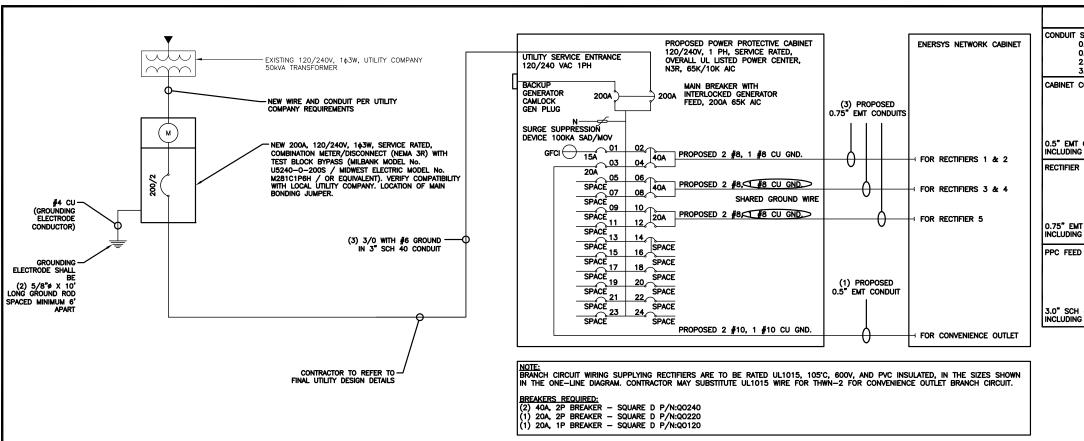
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E-1



— FBR —— FBR —— FBR -





PPC ONE-LINE DIAGRAM

NO SCALE

**NOTES** 

CONDUIT SIZING: AT 40% FILL PER NEC CHAPTER 9, TABLE 4, ARTICLE 358. 0.5" CONDUIT - 0.122 SQ. IN AREA 0.75" CONDUIT - 0.213 SQ. IN AREA 2.0" CONDUIT - 1.316 SQ. IN AREA 3.0" CONDUIT - 2.907 SQ. IN AREA

CABINET CONVENIENCE OUTLET CONDUCTORS (1 CONDUIT): USING THWN-2, CU.

#10 - 0.0211 SQ. IN X 2 = 0.0422 SQ. IN #10 - 0.0211 SQ. IN X 1 = 0.0211 SQ. IN <GROUND = 0.0633 SQ. IN

0.5" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (3) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

RECTIFIER CONDUCTORS (3 CONDUITS): USING UL1015, CU.

#8 - 0.0552 SQ. IN X 2 = 0.1103 SQ. IN #8 - 0.0131 SQ. IN X 1 = 0.0131 SQ. IN <BARE GROUND TOTAL = 0.1234 SQ. IN

0.75" EMT CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (3) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

PPC FEED CONDUCTORS (1 CONDUIT): USING THWN, CU.

NOT USED

3/0 - 0.2679 SQ. IN X 3 = 0.8037 SQ. IN #6 - 0.0507 SQ. IN X 1 = 0.0507 SQ. IN <GROUND = 0.8544 SQ. IN

3.0" SCH 40 PVC CONDUIT IS ADEQUATE TO HANDLE THE TOTAL OF (4) WIRES, INCLUDING GROUND WIRE, AS INDICATED ABOVE.

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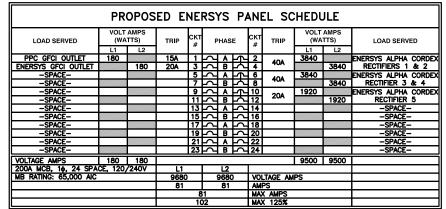
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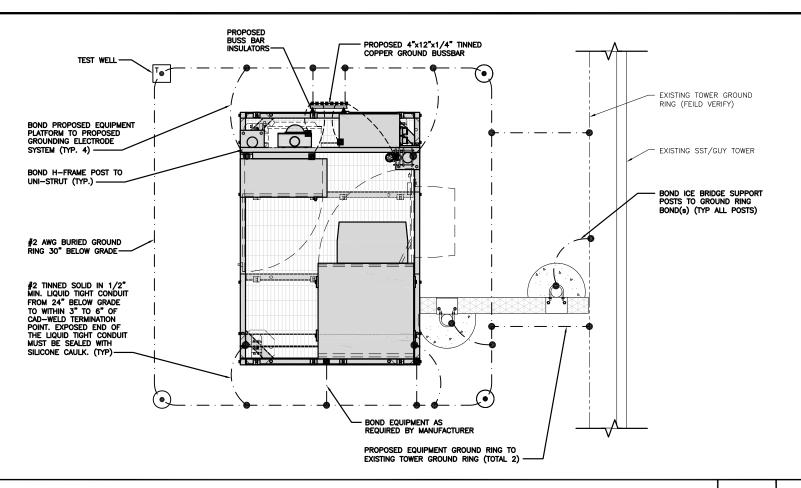
ELECTRICAL ONE-LINE, FAULT CALCS & PANEL SCHEDULE

SHEET NUMBER

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PANEL SCHEDULE



#### TYPICAL EQUIPMENT GROUNDING PLAN

TYPICAL ANTENNA GROUNDING PLAN

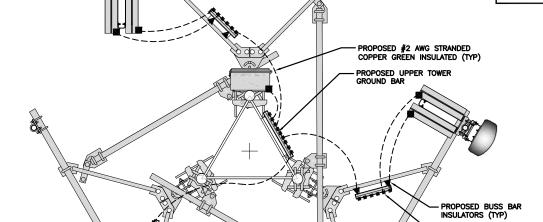
**NOTES** 

NO SCALE

PROPOSED 4"x6"x1/4"

TINNED COPPER SECTOR GROUND BUSSBAR (TYP

ANTENNAS AND OVP SHOWN ARE GENERIC AND NOT REFERENCING TO A SPECIFIC MANUFACTURER. THIS LAYOUT IS FOR REFERENCE



EXOTHERMIC CONNECTION MECHANICAL CONNECTION

T GROUND BUS BAR

GROUND ROD

 $(\bullet)$ 

TEST GROUND ROD WITH INSPECTION SLEEVE

---- #6 AWG STRANDED & INSULATED - · - #2 AWG SOLID COPPER TINNED

▲ BUSS BAR INSULATOR

#### **GROUNDING LEGEND**

- 1. GROUNDING IS SHOWN DIAGRAMMATICALLY ONLY
- CONTRACTOR SHALL GROUND ALL EQUIPMENT AS A COMPLETE SYSTEM. GROUNDING SHALL BE IN COMPLIANCE WITH NEC SECTION 250 AND DISH Wireless L.L.C. GROUNDING AND BONDING REQUIREMENTS AND MANUFACTURER'S SPECIFICATIONS.
- 3. ALL GROUND CONDUCTORS SHALL BE COPPER; NO ALUMINUM CONDUCTORS SHALL BE USED.

#### **GROUNDING KEY NOTES**

- (A) EXTERIOR GROUND RING: #2 AWG SOLID COPPER, BURIED AT A DEPTH OF AT LEAST 30 INCHES BELOW GRADE, OR 6 INCHES BELOW THE FROST LINE AND APPROXIMATELY 24 INCHES FROM THE EXTERIOR WALL OR FOOTING.
- B TOWER GROUND RING: THE GROUND RING SYSTEM SHALL BE INSTALLED AROUND AN ANTENNA TOWER'S LEGS, AND/OR GUY ANCHORS. WHERE SEPARATE SYSTEMS HAVE BEEN BROWNER FOR THE FORMAL TOWER'S LEGS, AND/OR GUY ANCHORS. WHERE SEPARATE SYSTEMS HAVE BEEN PROVIDED FOR THE TOWER AND THE BUILDING, AT LEAST TWO BONDS SHALL BE MADE BETWEEN THE TOWER RING GROUND SYSTEM AND THE BUILDING RING GROUND SYSTEM USING MINIMUM #2 AWG SOLID COPPER CONDUCTORS.
- © Interior ground ring: #2 awg stranded green insulated copper conductor extended around the perimeter of the equipment area. All non-telecommunications related metallic objects found within a site shall be grounded to the interior ground ring with #6 awg stranded green
- D BOND TO INTERIOR GROUND RING: #2 AWG SOLID TINNED COPPER WIRE PRIMARY BONDS SHALL BE PROVIDED AT LEAST AT FOUR POINTS ON THE INTERIOR GROUND RING, LOCATED AT THE CORNERS OF THE
- F CELL REFERENCE GROUND BAR: POINT OF GROUND REFERENCE FOR ALL COMMUNICATIONS EQUIPMENT FRAMES. ALL BONDS ARE MADE WITH #2 AWG UNLESS NOTED OTHERWISE STRANDED GREEN INSULATED COPPER CONDUCTORS. BOND TO GROUND RING WITH (2) #2 SOLID TINNED COPPER CONDUCTORS.
- G HATCH PLATE GROUND BAR: BOND TO THE INTERIOR GROUND RING WITH TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS. WHEN A HATCH-PLATE AND A CELL REFERENCE GROUND BAR ARE BOTH PRESENT, THE CRGB MUST BE CONNECTED TO THE HATCH-PLATE AND TO THE INTERIOR GROUND RING USING (2) TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS EACH.
- (H) EXTERIOR CABLE ENTRY PORT GROUND BARS; LOCATED AT THE ENTRANCE TO THE CELL SITE BUILDING, BOND TO GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTORS WITH AN EXOTHERMIC WELD AND INSPECTION SLEEVE.
- ( ) TELCO GROUND BAR: BOND TO BOTH CELL REFERENCE GROUND BAR OR EXTERIOR GROUND RING.
- J FRAME BONDING: THE BONDING POINT FOR TELECOM EQUIPMENT FRAMES SHALL BE THE GROUND BUS THAT IS NOT ISOLATED FROM THE EQUIPMENTS METAL FRAMEWORK.
- K Interior unit bonds: Metal Frames, Cabinets and Individual Metallic units located with the area of the interior ground ring require a #6 awg stranded green insulated copper bond to the
- L FENCE AND GATE GROUNDING: METAL FENCES WITHIN 7 FEET OF THE EXTERIOR GROUND RING OR OBJECTS BONDED TO THE EXTERIOR GROUND RING SHALL BE BONDED TO THE GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTOR AT AN INTERVAL NOT EXCEEDING 25 FEET. BONDS SHALL BE MADE AT EACH GATE POST AND ACROSS GATE OPENINGS.
- (M) <u>Exterior unit Bonds:</u> Metallic objects, external to or mounted to the Building, shall be bonded to the exterior ground ring. Using #2 tinned solid copper wire
- N ICE BRIDGE SUPPORTS: EACH ICE BRIDGE LEG SHALL BE BONDED TO THE GROUND RING WITH #2 AWG BARE TINNED COPPER CONDUCTOR. PROVIDE EXOTHERMIC WELDS AT BOTH THE ICE BRIDGE LEG AND BURIED
- O DURING ALL DC POWER SYSTEM CHANGES INCLUDING DC SYSTEM CHANGE OUTS, RECTIFIER REPLACEMENTS OR ADDITIONS, BREAKER DISTRIBUTION CHANGES, BATTERY ADDITIONS, BATTERY REPLACEMENTS AND INSTALLATIONS OR CHANGES TO DC CONVERTER SYSTEMS IT SHALL BE REQUIRED THAT SERVICE CONTRACTORS VERIFY ALL DC POWER SYSTEMS ARE EQUIPPED WITH A MASTER DC SYSTEM RETURN GROUND CONDUCTOR FROM THE DC POWER SYSTEM COMMON RETURN BUS DIRECTLY CONNECTED TO THE CELL SITE DEFERENCE COUNTRY BARE. REFERENCE GROUND BAR
- (P) TOWER TOP COLLECTOR BUSS BAR IS TO BE MECHANICALLY BONDED TO TOWER STEEL.

REFER TO DISH Wireless L.L.C. GROUNDING NOTES.

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NJJER01090A 50 ROCKLAND ROAD NORWALK, CT 06854

SHEET TITLE

GROUNDING PLANS AND NOTES

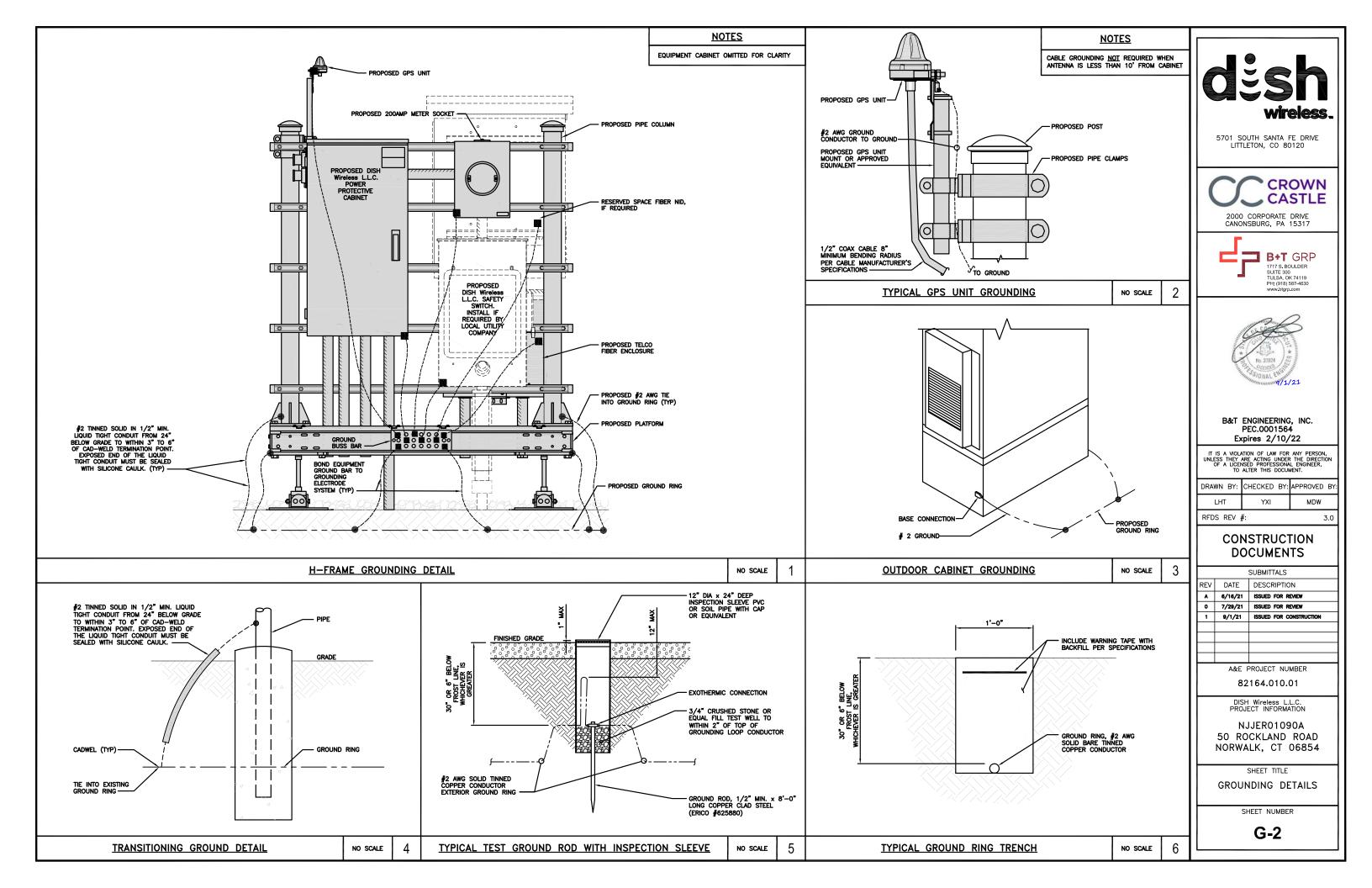
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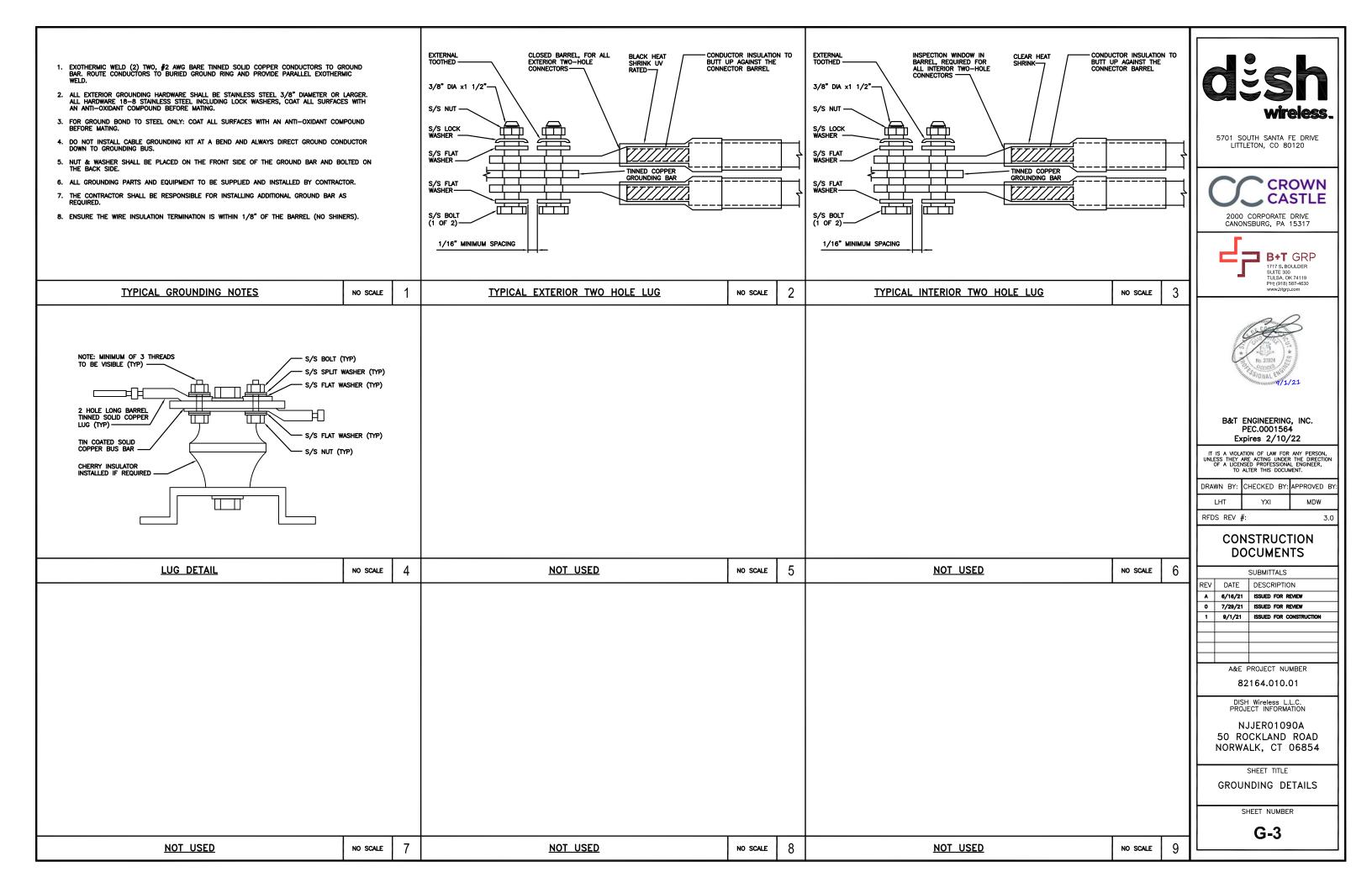
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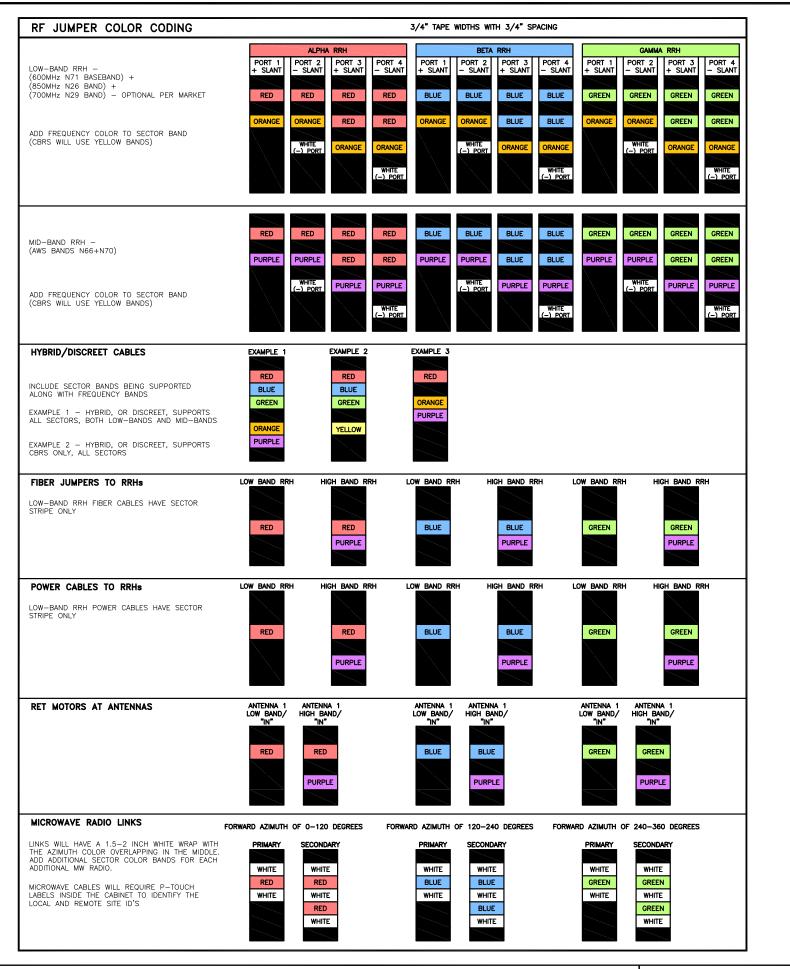
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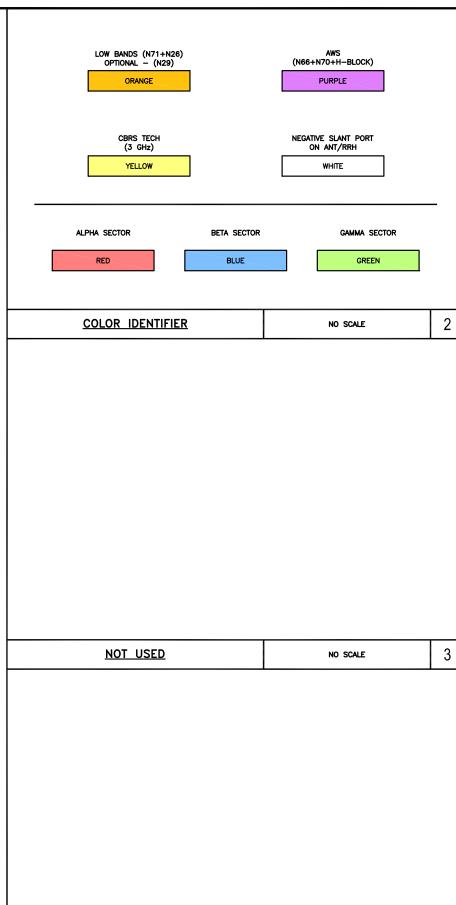
**GROUNDING KEY NOTES** 

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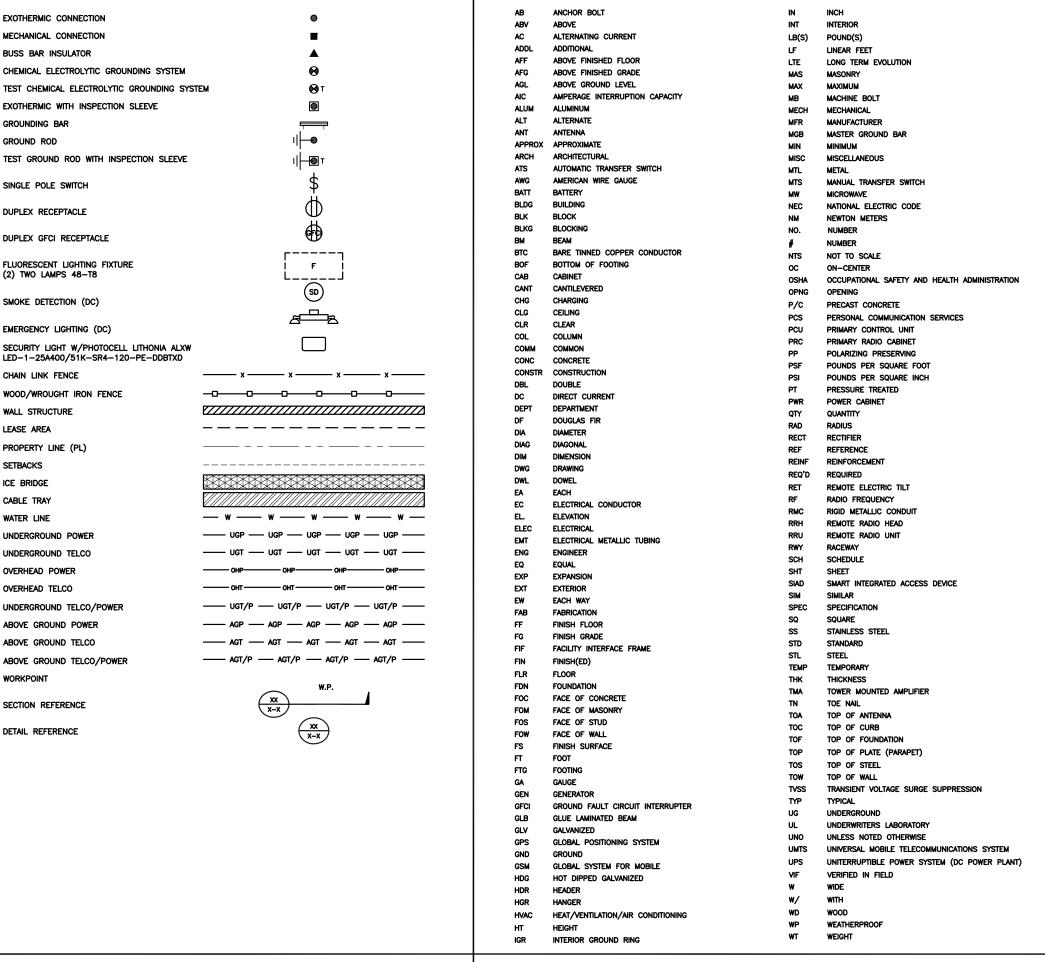
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> SHEET TITLE RF

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LEGEND AND ABBREVIATIONS

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**LEGEND** 

**ABBREVIATIONS** 

#### SITE ACTIVITY REQUIREMENTS:

- 1. NOTICE TO PROCEED NO WORK SHALL COMMENCE PRIOR TO CONTRACTOR RECEIVING A WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE DISH Wireless L.L.C. AND TOWER OWNER NOC & THE DISH Wireless L.L.C. AND TOWER CONSTRUCTION MANAGER.
- 2. "LOOK UP" DISH Wireless L.L.C. AND TOWER OWNER SAFETY CLIMB REQUIREMENT:

THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR DISH Wireless L.L.C. AND DISH Wireless L.L.C. AND TOWER OWNER POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.

- 3. PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.
- 4. ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND DISH WIFELDS L.L.C. AND TOWER OWNER STANDARDS, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA-322 (LATEST EDITION).
- 5. ALL SITE WORK TO COMPLY WITH DISH Wireless L.L.C. AND TOWER OWNER INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON DISH Wireless L.L.C. AND TOWER OWNER TOWER SITE AND LATEST VERSION OF ANSI/TIA-1019-A-2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS."
- 6. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY DISH Wireless L.L.C. AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OVER SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 9. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES INCLUDING PRIVATE LOCATES SERVICES PRIOR TO THE START OF CONSTRUCTION.
- 10. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION E) CONSTRUCTION SAFETY PROCEDURES.
- 11. ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND DISH PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.
- 12. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- 13. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF DISH WIReless L.L.C. AND TOWER OWNER, AND/OR LOCAL UTILITIES.
- 14. THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.
- 15. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER'S EQUIPMENT AND TOWER AREAS.
- 16. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
- 17. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
- 18. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
- 19. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
- 20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS AND RADIOS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 21. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION, TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.
- 22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

#### **GENERAL NOTES:**

1.FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR: GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION

CARRIER:DISH Wireless L.L.C.

TOWER OWNER:TOWER OWNER

- 2. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.
- 3. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY.
- 4. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.
- 5. SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.
- 6. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CARRIER POC AND TOWER OWNER.
- 7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 8. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- 9. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 10. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
- 11. CONTRACTOR IS TO PERFORM A SITE INVESTIGATION, BEFORE SUBMITTING BIDS, TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.
- 12. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR'S EXPENSE TO THE SATISFACTION OF DISH Wireless L.L.C. AND TOWER OWNER
- 13. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 14. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.



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## CONSTRUCTION DOCUMENTS

SUBMITTALS				
REV	DATE	DESCRIPTION		
A	6/16/21	ISSUED FOR REVIEW		
0	7/29/21	ISSUED FOR REVIEW		
1	9/1/21	ISSUED FOR CONSTRUCTION		
	A&E I	PROJECT NUMBER		

82164.010.01

DISH Wireless L.L.C. PROJECT INFORMATION

NJJER01090A

50 ROCKLAND ROAD NORWALK, CT 06854

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

GN-2

#### CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST—IN—PLACE CONCRETE.
- 2. UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED TO BE 1000 psf.
- 3. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) OF 3000 psi at 28 days, unless noted otherwise. No more than 90 minutes shall elapse from batch time to time of placement unless approved by the engineer of record. Temperature of concrete shall not exceed 90'f at time of placement.
- 4. CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL CONTAIN AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER-TO-CEMENT RATIO (W/C) OF 0.45.
- 5. ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE STANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS:

#4 BARS AND SMALLER 40 ksi

#5 BARS AND LARGER 60 ksi

- 6. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
- . CONCRETE EXPOSED TO EARTH OR WEATHER:
- #6 BARS AND LARGER 2"
- #5 BARS AND SMALLER 1-1/2"
- CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
- SLAB AND WALLS 3/4"
- BEAMS AND COLUMNS 1-1/2"
- 7. A TOOLED EDGE OR A 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

#### **ELECTRICAL INSTALLATION NOTES:**

- 1. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
- 2. CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED AND TRIP HAZARDS ARE ELIMINATED.
- 3. WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.
- I. ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.
- 4.1. ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.
- 4.2. ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE PRE THE GOVERNING JURISDICTION.
- 5. EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR—CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.
- 6. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT ID'S).
- 7. PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
- 8. TIE WRAPS ARE NOT ALLOWED.
- 9. ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 10. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 11. POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS OTHERWISE SPECIFIED.
- 12. POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
- 13. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75° C (90° C IF AVAILABLE).
- 14. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.
- 15. ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

- ELECTRICAL METALLIC TUBING (EMT) OR METAL—CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- 17. SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE GRADE PVC CONDUIT.
- 18. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
- 19. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION—TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
- 20. CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND THE NEC.
- 21. WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS (WIREMOLD SPECMATE WIREWAY).
- 22. SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).
- 23. CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE TEMPORARILY CAPPED FLUSH TO FINISH GRADE TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE.
- 24. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3 (OR BETTER) FOR EXTERIOR LOCATIONS.
- 25. METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY—COATED OR NON—CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 26. NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
- 27. THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR DISH Wireless L.L.C. AND TOWER OWNER BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
- 28. THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.
- 29. INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "DISH Wireless L.L.C.".
- 30. ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.



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## CONSTRUCTION DOCUMENTS

3.0

SUBMITTALS				
REV DATE DESCRIPTION				
A	A 6/16/21 ISSUED FOR REVIEW			
0 7/29/21 ISSUED FOR REVIEW				
1 9/1/21 ISSUED FOR CONSTRUCTION				
	A&F F	PROJECT NUMBER		

82164.010.01

DISH Wireless L.L.C.

NJJER01090A 50 ROCKLAND ROAD NORWALK, CT 06854

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

GN-3

#### **GROUNDING NOTES:**

- 1. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- 2. THE CONTRACTOR SHALL PERFORM IEEE FALL—OF—POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.
- 4. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 5. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- 6. EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS.
- 7. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
- 8. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.
- 9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- 10. USE OF  $90^{\circ}$  BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN  $45^{\circ}$  BENDS CAN BE ADEQUATELY SUPPORTED.
- 11. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- 12. ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.
- COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.
- 14. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.
- 15. APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- 16. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
- 17. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
- 18. BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.
- 19. GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.
- 20. ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON-METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD-WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).
- 21. BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/O COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY). DO NOT ATTACH GROUNDING TO FIRE SPRINKLER SYSTEM PIPES.



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## CONSTRUCTION DOCUMENTS

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A						
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1	1 9/1/21 ISSUED FOR CONSTRUCTION					
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A&E PROJECT NUMBER

82164.010.01

DISH Wireless L.L.C PROJECT INFORMATIO

NJJER01090A 50 ROCKLAND ROAD NORWALK, CT 06854

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

GN-4

# Exhibit D

**Structural Analysis Report** 

Date: June 07, 2021



Crown Castle 2000 Corporate Drive Canonsburg, PA 15317 (724) 416-2000

Subject: Structural Analysis Report

Carrier Designation: DISH Network Co-Locate

Site Number: NJJER01090A Site Name: CT-CCI-T-807133

Crown Castle Designation: BU Number: 807133

**Site Name:** BRG 134 943057

 JDE Job Number:
 640174

 Work Order Number:
 1964193

 Order Number:
 548868 Rev. 1

Engineering Firm Designation: Crown Castle Project Number: 1964193

Site Data: 50 ROCKLAND ROAD, SO NORWALK, FAIRFIELD County, CT

Latitude 41° 4' 54.44", Longitude -73° 25' 49.52"

180 Foot - Self Support Tower

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Proposed Equipment Configuration

**Sufficient Capacity-84.8%** 

This analysis utilizes an ultimate 3-second gust wind speed of 120 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - "Analysis Criteria".

Structural analysis prepared by: Melanie Atiles

Respectfully submitted by:

Maribel Dentinger, P.E.

Senior Project Engineer

Maribel

Maribel
Dentinger
Date: 2021.06.08 17:36:28
-04'00'



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#### 1) INTRODUCTION

This tower is a 180 ft Self Support tower designed by ROHN. The tower has been modified in the past to accommodate additional loading.

#### 2) ANALYSIS CRITERIA

TIA-222 Revision: TIA-222-H

Risk Category:

Wind Speed: 120 mph

Exposure Category: C
Topographic Factor: 1
Ice Thickness: 1.5 in
Wind Speed with Ice: 50 mph
Service Wind Speed: 60 mph

**Table 1 - Proposed Equipment Configuration** 

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
		3	fujitsu	TA08025-B604		
		3	fujitsu	TA08025-B605		
181.0	181.0	3	jma wireless	MX08FRO665-21 w/ Mount Pipe	1	1-3/4
		1	raycap	RDIDC-9181-PF-48		
		1	tower mounts	Commscope MTC3975083 (3)		

Table 2 - Non-Carrier Equipment To Be Removed

•	Mounting Level (ft)	Elevation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
	178.0	178.0	2	tower mounts	Side Arm Mount [SO 305-1]	-	-

**Table 3 - Other Considered Equipment** 

Tubic 0	Linei Consi	ucica Equi	D1110111			
Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer		Number of Feed Lines	Feed Line Size (in)
		3	ericsson	AIR 32 B2A/B66AA w/ Mount Pipe		
		3	ericsson	AIR 3246 B66_T-MOBILE w/ Mount Pipe		
		3	ericsson	AIR6449 B41 w/ Mount Pipe		
170.0	170.0	3	ericsson	RADIO 4449 B71 B85A_T- MOBILE	6	1-3/8
		3	ericsson	RRUS 4415 B25_CCIV2		
		3	rfs celwave	APXVAARR24_43-U-NA20 w/ Mount Pipe		
		1	tower mounts	Sector Mount [SM 702-3]		

Mounting Level (ft)	Center Line Elevation (ft)	ine Number Antenna Antenna Model Antennas Manufacturer		Number of Feed Lines	Feed Line Size (in)			
		1	andrew	SBNHH-1D65A				
			2	cci antennas	HPA-65R-BUU-H6		ļ	
		1	cci antennas	OPA65R-BU4D w/ Mount Pipe				
		2	cci antennas	OPA65R-BU6D w/ Mount Pipe				
		3	ericsson	RRUS 11				
		3	ericsson	RRUS 32				
161.0	161.0	3	ericsson	RRUS 32 B2	6	1-5/8 5/8		
101.0	101.0	3	ericsson	RRUS 4426 B66	8 2	3/8		
		3	ericsson	RRUS 4478 B14	_	0,0		
		3	ericsson	RRUS 4478 B5				
		1	quintel technology	QS46512-2				
		2	quintel technology	QS66512-2				
		4	raycap	DC6-48-60-18-8F				
		1	tower mounts	Sector Mount [SM 201-3]				
157.0	157.0	2	-	VHLP2-18	2	7983A		
157.0	157.0	2	tower mounts	Side Arm Mount [SO 203-1]		7963A		
		3	alcatel lucent	800 EXTERNAL NOTCH FILTER				
		3	alcatel lucent	800MHZ 2X50W RRH				
		6	alcatel lucent	PCS 1900MHz 4x45W-65MHz				
	148.0	9	rfs celwave	ACU-A20-N				
148.0		148.0	3	rfs celwave	APXVSPP18-C-A20 w/ Mount Pipe	4	1-1/4	
					3	rfs celwave	APXVTM14-ALU-I20 w/ Mount Pipe	
		1	tower mounts	Sector Mount [SM 502-3]				
	138.0	1	-	VHLP2-23				
		1	-	J - Box		<b>5/40</b>		
136.0		3	argus technologies	LLPX310R w/ Mount Pipe	6	5/16 1/2		
100.0	136.0	3	samsung telecommunications	RRH-2WB	2	Conduit		
		1	tower mounts	Sector Mount [SM 504-3]				
	133.0	3	samsung telecommunications	CBRS w/ Mount Pipe				
	130.0	1	gps	GPS_A				
		6	commscope	JAHH-65C-R3B w/ Mount Pipe				
126.0		4	decibel	DB844G65ZAXY w/ Mount Pipe				
		2	decibel	DB844H80-XY w/ Mount Pipe	7	1-5/8		
	129.0	1	rfs celwave	DB-C1-12C-24AB-0Z	1	1/2		
		3	samsung telecommunications	RFV01U-D1A	,	1/2		
		3	samsung telecommunications	RFV01U-D2A				
	128.0	3	VZW	Sub6 Antenna - VZS01 w/ Mount Pipe				

Mounting Level (ft)	Flevation	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
	126.0	1	tower mounts	Sector Mount [SM 411-3]		
112.0	112.0	3	kathrein	800 10504 w/ Mount Pipe		
112.0	112.0	1	tower mounts	Sector Mount [SM 104-3]	_	_
	6.0	1	decibel	ASPP2933		
5.0	5.0	1	gps	GPS_A	2	1/4
	5.0	1	tower mounts	Side Arm Mount [SO 701-1]		

#### 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided** 

Document	Reference	Source
4-GEOTECHNICAL REPORTS	2311843	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	821566	CCISITES
4-TOWER MANUFACTURER DRAWINGS	392878	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	1257479	CCISITES
4-POST-MODIFICATION INSPECTION	4065020	CCISITES

#### 3.1) Analysis Method

tnxTower (version 8.0.9.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

#### 3.2) Assumptions

- 1) Tower and structures were maintained in accordance with the TIA-222 Standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)** 

Tubic	Cootion Cap	Coolin Supusity (Summary)									
Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail			
T1	180 - 160	Leg	ROHN 3 EH	2	-17.157	116.138	14.8	Pass			
T2	160 - 153.333	Leg	ROHN 4 EH	35	-24.613	167.901	14.7	Pass			
Т3	153.333 - 146.667	Leg	ROHN 4 EH	44	-35.886	167.900	21.4	Pass			
T4	146.667 - 140	Leg	ROHN 4 EH	56	-46.791	167.901	27.9	Pass			
T5	140 - 120	Leg	ROHN 5 EH	68	-85.930	251.347	34.2	Pass			
Т6	120 - 100	Leg	ROHN 6 EHS	89	-128.015	288.515	44.4	Pass			
T7	100 - 80	Leg	ROHN 6 EH	110	-164.542	318.903	51.6	Pass			
Т8	80 - 70	Leg	ROHN 8 EHS	125	-183.519	405.715	45.2	Pass			

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
Т9	70 - 60	Leg	ROHN 8 EHS	135	-202.841	405.715	50.0	Pass
T10	60 - 40	Leg	ROHN 8 EHS	144	-241.080	405.717	59.4	Pass
T11	40 - 20	Leg	ROHN 8 EH	159	-278.864	530.833	52.5	Pass
T12	20 - 0	Leg	ROHN 8 EH	174	-316.499	530.833	59.6	Pass
T1	180 - 160	Diagonal	L2x2x3/16	13	-3.019	10.104	29.9	Pass
T2	160 - 153.333	Diagonal	L2 1/2x2 1/2x1/4	39	-5.019	19.793	25.4	Pass
Т3	153.333 - 146.667	Diagonal	L2 1/2x2 1/2x1/4	51	-5.281	17.900	29.5	Pass
T4	146.667 - 140	Diagonal	L2 1/2x2 1/2x1/4	63	-5.954	16.240	36.7	Pass
T5	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	70	-7.793	12.489	62.4	Pass
T6	120 - 100	Diagonal	L3x3x1/4	91	-9.061	17.566	51.6	Pass
T7	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	113	-9.928	18.890	52.6	Pass
T8	80 - 70	Diagonal	L3 1/2x3 1/2x1/4	128	-10.407	17.632	59.0	Pass
Т9	70 - 60	Diagonal	2L3 1/2x3 1/2x1/4x3/8	137	-11.141	27.539	40.5	Pass
T10	60 - 40	Diagonal	L4x4x1/4	146	-11.622	20.589	56.4	Pass
T11	40 - 20	Diagonal	L4x4x5/16	161	-12.359	21,559	57.3	Pass
T12	20 - 0	Diagonal	2L4x4x5/16x3/8	176	-13.697	31.656	43.3	Pass
T1	180 - 160	Top Girt	L2x2x1/8	4	-0.482	4.230	11.4	Pass
Т3	153.333 - 146.667	Top Girt	L2 1/2x2 1/2x1/8	46	-0.622	4.069	15.3	Pass
T4	146.667 - 140	Top Girt	L2 1/2x2 1/2x1/8	58	-0.812	3.498	23.2	Pass
T1	180 - 160	Mid Girt	L2x2x1/8	9	-0.480	3.097	15.5	Pass
							Summary	
						Leg (T12)	59.6	Pass
						Diagonal (T5)	62.4	Pass
						Top Girt (T4)	23.2	Pass
						Mid Girt (T1)	15.5	Pass
						Bolt Checks	75.8	Pass
						Rating =	75.8	Pass

Table 6 - Tower Component Stresses vs. Capacity - LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	47.1	Pass
1	Base Foundation (Structure)	0	84.8	Pass

Structure Rating (max from all components) =	84.8%
--	-------

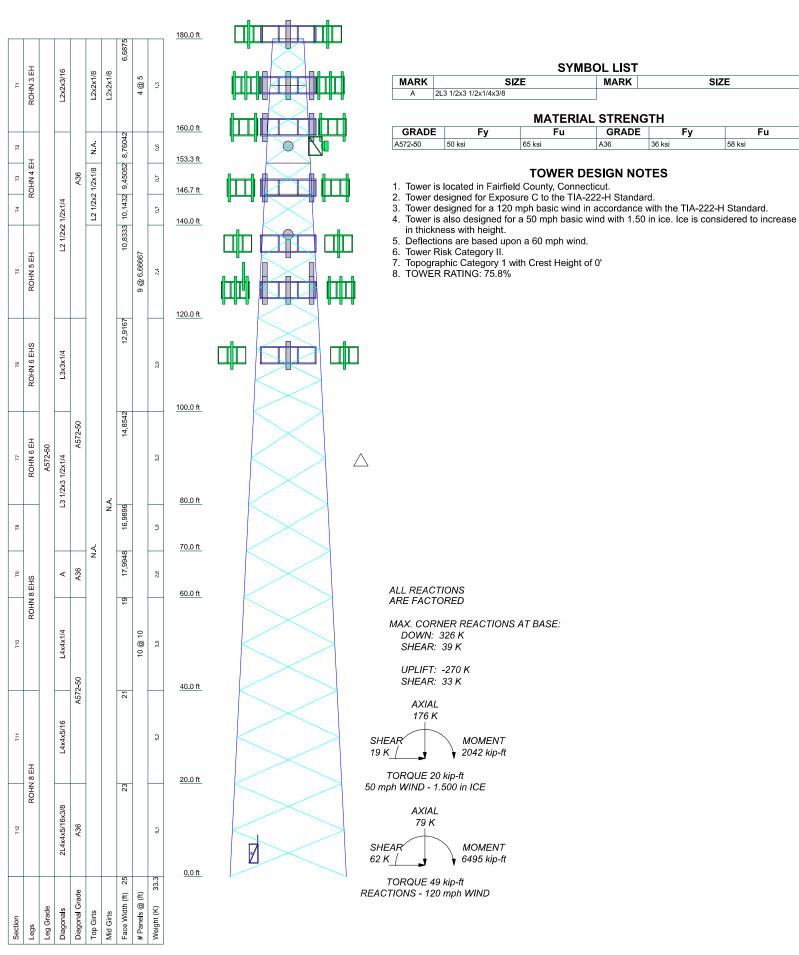
Notes:

#### 4.1) Recommendations

Once the equipment in Table 2 has been removed, the tower and its foundation have sufficient capacity to carry the proposed load configuration. No structural modifications are required at this time.

<sup>1)</sup> See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

# APPENDIX A TNXTOWER OUTPUT





SIZE

58 ksi

Fu

MARK

GRADE

36 ksi

A36

### **Tower Input Data**

The main tower is a 3x free standing tower with an overall height of 180' above the ground line.

The base of the tower is set at an elevation of 0' above the ground line.

The face width of the tower is 6'8-1/4" at the top and 25' at the base.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

- Tower is located in Fairfield County, Connecticut.
- Tower base elevation above sea level: 61'.
- Basic wind speed of 120 mph.
- · Risk Category II.
- Exposure Category C.
- Simplified Topographic Factor Procedure for wind speed-up calculations is used.
- Topographic Category: 1.
- Crest Height: 0'.
- Nominal ice thickness of 1.500 in.
- Ice thickness is considered to increase with height.
- Ice density of 56.000 pcf.
- A wind speed of 50 mph is used in combination with ice.
- Temperature drop of 50.000 °F.
- Deflections calculated using a wind speed of 60 mph.
- Pressures are calculated at each section.
- Stress ratio used in tower member design is 1.
- Tower analysis based on target reliabilities in accordance with Annex S.
- Load Modification Factors used: Kes(Fw) = 0.95, Kes(ti) = 0.85.
- Maximum demand-capacity ratio is: 1.05.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

### **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- √ Use Code Stress Ratios
- ✓ Use Code Safety Factors Guys Escalate Ice
   Always Use Max Kz
   Use Special Wind Profile

Include Bolts In Member Capacity

Leg Bolts Are At Top Of Section

✓ Secondary Horizontal Braces Leg
Use Diamond Inner Bracing (4 Sided)
SR Members Have Cut Ends
SR Members Are Concentric

Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
- √ Use Clear Spans For KL/r
  Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
   √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.

Autocalc Torque Arm Areas

Add IBC .6D+W Combination

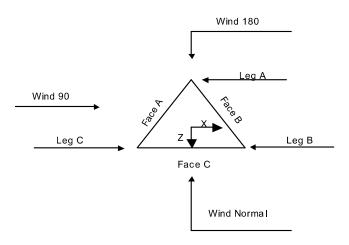
Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs

Use ASCE 10 X-Brace Ly Rules

- Calculate Redundant Bracing Forces Ignore Redundant Members in FEA
- √ SR Leg Bolts Resist Compression
  All Leg Panels Have Same Allowable
  Offset Girt At Foundation
- √ Consider Feed Line Torque
- ✓ Include Angle Block Shear Check Use TIA-222-H Bracing Resist.
   Exemption
   Use TIA-222-H Tension Splice
   Exemption

#### Poles

Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known



Triangular Tower

Tower	Section (	Geometry

Tower	Tower	Assembly	Description	Section	Number	Section
Section	Elevation	Database		Width	of	Length
					Sections	
	ft			ft		ft
T1	180'-160'			6'8-1/4"	1	20'
T2	160'-153'4"			8'9-1/8"	1	6'8"
T3	153'4"-146'8"			9'5-13/32"	1	6'8"
T4	146'8"-140'			10'1-23/32"	1	6'8"
T5	140'-120'			10'10"	1	20'
T6	120'-100'			12'11"	1	20'
<b>T</b> 7	100'-80'			14'10-1/4"	1	20'
T8	80'-70'			16'11-7/8"	1	10'
T9	70'-60'			17'11-15/16"	1	10'
T10	60'-40'			19'	1	20'
T11	40'-20'			21'	1	20'
T12	20'-0'			23'	1	20'

Tower	Tower	Diagonal	Bracing	Has	Has	Top Girt	Bottom Gir
Section	Elevation	Spacing	Type	K Brace	Horizontals	Offset	Offset
				End			
	ft	ft		Panels		in	in
T1	180'-160'	5'	X Brace	No	No	0.000	0.000
T2	160'-153'4"	6'8"	X Brace	No	No	0.000	0.000
T3	153'4"-146'8"	6'8"	X Brace	No	No	0.000	0.000
T4	146'8"-140'	6'8"	X Brace	No	No	0.000	0.000
T5	140'-120'	6'8"	X Brace	No	No	0.000	0.000
T6	120'-100'	6'8"	X Brace	No	No	0.000	0.000
<b>T</b> 7	100'-80'	10'	X Brace	No	No	0.000	0.000
T8	80'-70'	10'	X Brace	No	No	0.000	0.000
<b>T</b> 9	70'-60'	10'	X Brace	No	No	0.000	0.000
T10	60'-40'	10'	X Brace	No	No	0.000	0.000
T11	40'-20'	10'	X Brace	No	No	0.000	0.000

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft		Panels		in	in
T12	20'-0'	10'	X Brace	No	No	0.000	0.000

Tower Section Geometry (cont'd)
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Tower	Leg	Leg	Leg	Diagonal	Diagonal	Diagonal
Elevation ft	Type	Size	Grade	Type	Size	Grade
T1 180'-160'	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Single Angle	L2x2x3/16	A36 (36 ksi)
T2 160'-153'4"	Pipe	ROHN 4 EH	A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T3 153'4"- 146'8"	Pipe	ROHN 4 EH	A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T4 146'8"-140'	Pipe	ROHN 4 EH	A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T5 140'-120'	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Single Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T6 120'-100'	Pipe	ROHN 6 EHS	A572-50 (50 ksi)	Single Angle	L3x3x1/4	A572-50 (50 ksi)
T7 100'-80'	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Single Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T8 80'-70'	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Single Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T9 70'-60'	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Double Equal Angle	2L3 1/2x3 1/2x1/4x3/8	A36 (36 ksi)
T10 60'-40'	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Single Angle	L4x4x1/4	A572-50 (50 ksi)
T11 40'-20'	Pipe	ROHN 8 EH	A572-50 (50 ksi)	Single Angle	L4x4x5/16	A572-50 (50 ksi)
T12 20'-0'	Pipe	ROHN 8 EH	(50 ksi) A572-50 (50 ksi)	Double Equal Angle	2L4x4x5/16x3/8	A36 (36 ksi)

Tower Section	Geometry	(cont'd)
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Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 180'-160'	Equal Angle	L2x2x1/8	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T3 153'4"- 146'8"	Equal Angle	L2 1/2x2 1/2x1/8	`A36 <sup>´</sup> (36 ksi)	Single Angle		`A36 <sup>´</sup> (36 ksi)
T4 146'8"-140'	Single Angle	L2 1/2x2 1/2x1/8	` A36 <sup>′</sup> (36 ksi)	Single Angle		`A36 <sup>´</sup> (36 ksi)

Tower Elevation	No. of Mid	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
ft	Girts						
T1 180'-160'	1	Equal Angle	L2x2x1/8	A36	Single Angle		A36
		. 0		(36 ksi)	5 0		(36 ksi)

<b>Tower Section Geometry</b> (cont'd)	<b>Tower</b>	Section	Geometry	(cont'd)
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Tower	Gusset	Gusset	Gusset Grade	Adjust. Factor	Adjust.	Weight Mult.	Double Angle	Double Angle	Double Angle
Elevation	Area	Thickness		$A_f$	Factor	-	Stitch Bolt	Stitch Bolt	Stitch Bolt
	(per face)				$A_r$		Spacing	Spacing	Spacing
							Diagonals	Horizontals	Redundants
ft	ft²	in					in	in	in
T1 180'-160'	0.000	0.250	A36	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
TO 4001	0.000	0.050	(36 ksi)	4.00	4	4.05	MODE	Mid Di	Mid Di
T2 160'- 153'4"	0.000	0.250	A36	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
	0.000	0.250	(36 ksi)	1.02	1	1.05	Mid Dt	Mid Dt	Mid Dt
T3 153'4"- 146'8"	0.000	0.250	A36 (36 ksi)	1.03	ı	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T4 146'8"-	0.000	0.250	A36	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
140'			(36 ksi)						
T5 140'-120'	0.000	0.250	` A36 <sup>′</sup>	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
			(36 ksi)						
T6 120'-100'	0.000	0.250	A36	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
			(36 ksi)						
T7 100'-80'	0.000	0.375	A36	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
			(36 ksi)						
T8 80'-70'	0.000	0.375	A36	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
			(36 ksi)						
T9 70'-60'	0.000	0.375	A36	1.03	1	1.05	120,000	Mid-Pt	Mid-Pt
			(36 ksi)						
T10 60'-40'	0.000	0.375	A36	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
			(36 ksi)						
T11 40'-20'	0.000	0.375	A36	1.03	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
			(36 ksi)						
T12 20'-0'	0.000	0.375	A36	1.03	1	1.05	152.750	Mid-Pt	Mid-Pt
			(36 ksi)						

						K Fac	ctors1			
Tower Elevation	Calc K Single	Calc K Solid	Legs	X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace
	Angles	Rounds		X	X	X	X	X	X	X
ft				Y	Y	Y	Y	Y	Y	Υ
T1 180'-160'	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1
T2 160'-	Yes	No	1	1	1	1	1	1	1	1
153'4"				1	1	1	1	1	1	1
T3 153'4"-	Yes	No	1	1	1	1	1	1	1	1
146'8"				1	1	1	1	1	1	1
T4 146'8"-	Yes	No	1	1	1	1	1	1	1	1
140'				1	1	1	1	1	1	1
T5 140'-120'	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1
T6 120'-100'	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1
T7 100'-80'	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1
T8 80'-70'	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1
T9 70'-60'	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1
T10 60'-40'	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1
T11 40'-20'	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1
T12 20'-0'	Yes	No	1	1	1	1	1	1	1	1
				1	1	1	1	1	1	1

<sup>&</sup>lt;sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-ofplane direction applied to the overall length.

<b>Tower Section</b>	Geometry	(cont'd)
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Tower Elevation ft	Leg		Diago	nal	Тор С	irt	Botton	n Girt	Mid	Girt	Long Ho	rizontal	Short Ho	rizontal
	Net Width Deduct	U	Net Width	U	Net Width Deduct	U	Net Width	U	Net Width	U	Net Width	U	Net Width	U
	in		Deduct in		in		Deduct in		Deduct in		Deduct in		Deduct in	
T1 180'-160'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T2 160'-	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
153'4"														
T3 153'4"- 146'8"	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T4 146'8"- 140'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T5 140'-120'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T6 120'-100'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T7 100'-80'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T8 80'-70'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T9 70'-60'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T10 60'-40'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T11 40'-20'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T12 20'-0'	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75

Tower Elevation ft	Redund Horizo		Redun Diago		Redundan Diagoi		Redunda Horiza		Redur Vert		Redund	ant Hip	Redund Diago	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 180'-160' T2 160'- 153'4"	0.000 0.000	0.75 0.75	0.000 0.000	0.75 0.75	0.000 0.000	0.75 0.75	0.000 0.000	0.75 0.75	0.000 0.000	0.75 0.75	0.000 0.000	0.75 0.75	0.000 0.000	0.75 0.75
T3 153'4"- 146'8"	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T4 146'8"- 140'	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T5 140'-120'	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T6 120'-100'	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T7 100'-80'	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T8 80'-70'	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T9 70'-60'	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T10 60'-40' T11 40'-20'	0.000	0.75 0.75	0.000	0.75 0.75	0.000	0.75 0.75	0.000	0.75 0.75	0.000	0.75 0.75	0.000	0.75 0.75	0.000	0.75 0.75
T12 20'-0'	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75

Tower Elevation ft	Leg Connection Type	Leg		Diagon	nal	Top G	irt	Bottom	Girt	Mid G	irt	Long Horiz	zontal	Shor Horizor	-
		Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.						
		in		in		in		in		in		in		in	
T1 180'-160'	Flange	0.875	4	0.625	1	0.625	1	0.000	0	0.625	1	0.625	0	0.625	0
		A325N		A325N		A325N		A325X		A325N		A325X		A325N	
T2 160'-	Flange	0.000	0	0.625	1	0.000	0	0.000	0	0.625	0	0.625	0	0.625	0
153'4"		A325N		A325N		A325N		A325X		A325X		A325X		A325N	
T3 153'4"-	Flange	0.000	0	0.625	1	0.625	1	0.000	0	0.625	0	0.625	0	0.625	0
146'8"		A325N		A325N		A325N		A325X		A325X		A325X		A325N	

Tower Elevation ft	Leg Connection Type	Leg		Diagor	nal	Top G	irt	Bottom	Girt	Mid Gi	irt	Long Hori	zontal	Shor Horizor	-
		Bolt Size	No.	Bolt Size	No.	Bolt Size	No.	Bolt Size	No.						
		in		in		in		in		l in		in		in	
T4 146'8"-	Flange	1.000	4	0.625	1	0.625	1	0.000	0	0.625	0	0.625	0	0.625	0
140'	•	A325N		A325N		A325N		A325X		A325X		A325X		A325N	
T5 140'-120'	Flange	1.000	6	0.625	1	0.000	0	0.000	0	0.625	0	0.625	0	0.625	0
	•	A325N		A325N		A325N		A325X		A325X		A325X		A325N	
T6 120'-100'	Flange	1.000	6	0.625	1	0.000	0	0.000	0	0.625	0	0.625	0	0.625	0
	_	A325N		A325N		A325N		A325X		A325X		A325X		A325N	
T7 100'-80'	Flange	1.000	8	0.750	1	0.000	0	0.000	0	0.625	0	0.625	0	0.625	0
	_	A325N		A325N		A325N		A325X		A325X		A325X		A325N	
T8 80'-70'	Flange	0.000	0	0.750	1	0.000	0	0.000	0	0.625	0	0.625	0	0.625	0
	_	A325N		A325N		A325N		A325X		A325X		A325X		A325N	
T9 70'-60'	Flange	1.000	8	0.750	1	0.000	0	0.000	0	0.625	0	0.625	0	0.625	0
	_	A325N		A325N		A325N		A325X		A325X		A325X		A325N	
T10 60'-40'	Flange	1.000	8	0.750	1	0.000	0	0.000	0	0.625	0	0.625	0	0.625	0
	•	A325N		A325N		A325N		A325X		A325X		A325X		A325N	
T11 40'-20'	Flange	1.000	8	0.750	1	0.000	0	0.000	0	0.625	0	0.625	0	0.625	0
	•	A325N		A325N		A325N		A325X		A325X		A325X		A325N	
T12 20'-0'	Flange	1.000	0	0.750	1	0.000	0	0.000	0	0.625	0	0.625	0	0.625	0
		A449		A325N		A325N		A325X		A325X		A325X		A325N	

# Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description		Allow Shield	Exclude From Torque Calculation	Componen t Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacin g in	Width or Per Diameter in	rimete r in	Weight klf
***													
Feedline Ladder (Af) ***	В	No	No	Af (CaAa)	180' - 0'	0.000	0.38	1	1	3.000	3.000		0.008
HCS 6X12 6AWG(1-3/8)	Α	No	No	Ar (CaAa)	170' - 0'	0.000	0.08	6	6	1.380	1.380		0.002
LDF1- 50A(1/4)	Α	No	No	Ar (CaAa)	5' - 0'	0.000	0.15	2	2	0.345	0.345		0.000
Feedline Ladder (Af)	Α	No	No	Af (CaAa)	170' - 0'	0.000	0	1	1	3.000	3.000		0.008
MLE Hybrid 3Power/6Fib er RL 2 10AWG(1- 1/4) ***	Α	No	No	Ar (CaAa)	148' - 0'	0.000	-0.35	4	4	1.250	1,250		0.000
2-1/4" Rigid Conduit	Α	No	No	Ar (CaAa)	136' - 0'	0.000	-0.38	2	2	0.850 0.750	2.250		0.003
LDF4- 50A(1/2)	Α	No	No	Ar (CaAa)	136' - 0'	0.000	-0.395	1	1	0.630	0.630		0.000
9207(5/16)	Α	No	No	Ar (CaAa)	136' - 0'	0.000	-0.38	6	6	0.200	0.330		0.001
7983A(ELLÍP TICAL)	Α	No	No	Ar (CaAa)	157' - 0'	0.000	-0.405	2	1	0.500	0.573		0.000
Feedline Ladder (Af)	Α	No	No	Af (CaAa)	160' - 0'	0.000	-0.36	1	1	3.000	3.000		0.008
Feedline Ladder (Af) ***	С	No	No	Af (CaAa)	180' - 0'	0.000	-0.4	1	1	3.000	3.000		0.008
LDF7-50A(1 5/8)	С	No	No	Ar (CaAa)	126' - 0'	-3.000	-0.4	7	7	1.980	1.980		0.001
LDF4- 50A(1/2)	С	No	No	Ar (CaAa)	126' - 0'	-1.000	-0.455	1	1	0.500	0.630		0.000
Feedline Ladder (Af)	С	No	No	Af (CaAa)	126' - 0'	-1.000	-0.4	1	1	3.000	3.000		0.008
Feedline	С	No	No	Af (CaAa)	161' - 0'	0.000	0.4	1	1	3.000	3.000		800.0
tnxTower Re	eport -	- versi	on 8.0.9.0										

tnxTower Report - version 8.0.9.0

Description	Face		Exclude	Componen	Placement	Face	Lateral	#	#	Clear		Perimete	Weight
	or	Shield	From	_ t	-	Offset	Offset		Per	•	Diameter	r	
	Leg		Torque	Type	ft	in	(Frac FW)		Row	g	in	_	kIf
			Calculation							in		in	
Ladder (Af)													
CR 50 1873(1-5/8)	С	No	No	Ar (CaAa)	161' - 0'	0.000	0.34	6	3	0.850 0.750	1.980		0.001
FB-L98-002- XXX(3/8)	С	No	No	Ar (CaAa)	161' - 0'	0.000	0.36	2	1	0.500	0.394		0.000
WR- ' VG82ST- BRDA(5/8)	С	No	No	Ar (CaAa)	161' - 0'	4.500	0.34	6	6	0.500	0.645		0.000
WR- VG82ST- BRDA(5/8)	С	No	No	Ar (CaAa)	161' - 0'	4.500	0.34	2	2	0.500	0.645		0.000
Safety Line 3/8 ***	В	No	No	Ar (CaAa)	180' - 0'	0.000	0.5	1	1	0.375	0.375		0.000
CU12PSM6P 4XXX(1-3/4) ***	В	No	No	Ar (CaAa)	180' - 0'	0.000	0.48	1	1	1.750	1.750		0.003

Feed Line/Linear Appurtenances - Entered As Area									
Description	Face Allow	Exclude Co	omponen	Placement	Total	$C_A A_A$	Weight		
	or Shield Leg	From Torque	t Type	ft	Number	ft²/ft	kIf		
***		Calculation							

Feed Line/Linear Appurtenances Section Areas
--

Tower	Tower	Face	$A_R$	AF	$C_AA_A$	$C_A A_A$	Weight
Sectio	Elevation				In Face	Out Face	J
n	ft		ft²	ft <sup>2</sup>	ft²	ft²	K
T1	180'-160'	Α	0.000	0.000	13,280	0.000	0.186
		В	0.000	0.000	14.250	0.000	0.227
		С	0.000	0.000	12.283	0.000	0.184
T2	160'-153'4"	Α	0.000	0.000	12,607	0.000	0.181
		В	0.000	0.000	4.750	0.000	0.076
		С	0.000	0.000	18.552	0.000	0.162
T3	153'4"-146'8"	Α	0.000	0.000	13.617	0.000	0.184
		В	0.000	0.000	4.750	0.000	0.076
		С	0.000	0.000	18.552	0.000	0.162
T4	146'8"-140'	Α	0.000	0.000	16.284	0.000	0.193
		В	0.000	0.000	4.750	0.000	0.076
		С	0.000	0.000	18.552	0.000	0.162
T5	140'-120'	Α	0.000	0.000	60,228	0.000	0.736
		В	0.000	0.000	14.250	0.000	0.227
		С	0.000	0.000	67.349	0.000	0.573
T6	120'-100'	Α	0.000	0.000	63.072	0.000	0.775
		В	0.000	0.000	14.250	0.000	0.227
		С	0.000	0.000	94.635	0.000	0.773
<b>T</b> 7	100'-80'	Α	0.000	0.000	63.072	0.000	0.775
		В	0.000	0.000	14.250	0.000	0.227
		С	0.000	0.000	94.635	0.000	0.773
T8	80'-70'	Α	0.000	0.000	31.536	0.000	0.388
		В	0.000	0.000	7.125	0.000	0.113
		С	0.000	0.000	47.317	0.000	0.387
T9	70'-60'	Α	0.000	0.000	31.536	0.000	0.388

Tower Sectio	Tower Elevation	Face	$A_R$	$A_F$	C <sub>A</sub> A <sub>A</sub> In Face	C <sub>A</sub> A <sub>A</sub> Out Face	Weight
	Elevalion ft		ft <sup>2</sup>	ft²	III Face ft²	ft²	K
n	π						
		В	0.000	0.000	7.125	0.000	0.113
		С	0.000	0.000	47.317	0.000	0.387
T10	60'-40'	Α	0.000	0.000	63.072	0.000	0.775
		В	0.000	0.000	14.250	0.000	0.227
		С	0.000	0.000	94.635	0.000	0.773
T11	40'-20'	Α	0.000	0.000	63.072	0.000	0.775
		В	0.000	0.000	14.250	0.000	0.227
		С	0.000	0.000	94.635	0.000	0.773
T12	20'-0'	Α	0.000	0.000	63.417	0.000	0.776
		В	0.000	0.000	14.250	0.000	0.227
		С	0.000	0.000	94.635	0.000	0.773

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	$A_R$	$A_{\digamma}$	$C_A A_A$	$C_A A_A$	Weight
Sectio	Elevation	or	Thickness			In Face	Out Face	
n	ft	Leg	in	ft²	ft²	ft²	ft²	Κ
T1	180'-160'	Α	1.502	0.000	0.000	31.386	0.000	0.548
		В		0.000	0.000	32,275	0.000	0.622
		С		0.000	0.000	21.147	0.000	0.447
T2	160'-153'4"	Α	1.490	0.000	0.000	28.876	0.000	0.516
		В		0.000	0.000	10.710	0.000	0.206
		С		0.000	0.000	39.448	0.000	0.606
T3	153'4"-146'8"	Α	1.483	0.000	0.000	33.094	0.000	0.561
		В		0.000	0.000	10.684	0.000	0.205
		С		0.000	0.000	39.371	0.000	0.604
T4	146'8"-140'	Α	1.477	0.000	0.000	41.334	0.000	0.652
		В		0.000	0.000	10.657	0.000	0.204
		С		0.000	0.000	39.292	0.000	0.601
T5	140'-120'	Α	1.462	0.000	0.000	162.299	0.000	2.441
		В		0.000	0.000	31.798	0.000	0.607
		С		0.000	0.000	146.065	0.000	2.222
T6	120'-100'	Α	1.438	0.000	0.000	170.879	0.000	2.531
		В		0.000	0.000	31.508	0.000	0.598
		С		0.000	0.000	211.836	0.000	3.190
T7	100'-80'	Α	1.410	0.000	0.000	169.592	0.000	2.490
		В		0.000	0.000	31.165	0.000	0.587
		С		0.000	0.000	210.430	0.000	3.139
T8	80'-70'	Α	1.384	0.000	0.000	84.222	0.000	1.227
		В		0.000	0.000	15.430	0.000	0.289
		С		0.000	0.000	104.589	0.000	1.547
T9	70'-60'	Α	1.364	0.000	0.000	83.780	0.000	1.213
		В		0.000	0.000	15.312	0.000	0.285
		С		0.000	0.000	104.105	0.000	1.530
T10	60'-40'	Α	1.329	0.000	0.000	165.970	0.000	2.377
		В		0.000	0.000	30.199	0.000	0.558
		С		0.000	0.000	206.474	0.000	2.998
T11	40'-20'	Α	1.263	0.000	0.000	162.996	0.000	2.286
		В		0.000	0.000	29.405	0.000	0.535
		С		0.000	0.000	203.223	0.000	2.884
T12	20'-0'	Α	1.132	0.000	0.000	159.736	0.000	2.125
		В		0.000	0.000	27.828	0.000	0.491
		С		0.000	0.000	196.781	0.000	2.664

### **Feed Line Center of Pressure**

Section	Elevation	<i>CP</i> <sub>X</sub>	<i>CP</i> z	CP <sub>X</sub>	CPz
				Ice	Ice
	ft	in	in	in	in
T1	180'-160'	3.901	2.082	6.373	3.150
T2	160'-153'4"	-7.859	5.873	-8.694	8.318
T3	153'4"-146'8"	-7.915	5.764	-9.375	8.419

Section	Elevation	CP <sub>X</sub>	CPz	$CP_X$	CPz
				Ice	Ice
	ft	in	in	in	in
T4	146'8"-140'	-9.475	6.515	-11.134	9.321
T5	140'-120'	-9.632	9.171	-10.299	12.353
T6	120'-100'	-3.675	11.038	-2.165	14.971
T7	100'-80'	-4.212	12.692	-2.419	17.209
T8	80'-70'	-4.370	13.111	-2.472	17.921
<b>T</b> 9	70'-60'	-4.531	13,609	-2.520	18.594
T10	60'-40'	-4.534	13.643	-2.502	19.088
T11	40'-20'	-4.790	14.442	-2.468	20.076
T12	20'-0'	-5.069	15.069	-2.328	20.228

# Shielding Factor Ka

Tower	Feed Line	Description	Feed Line	Ka	<b>K</b> a
Section	Record No.	,	Segment	No Ice	Ice
			Elev.		
T1	2	Feedline Ladder (Af)	160.00 -	0.6000	0.6000
		1100 0340 04404 040	180.00	0.0000	0.0000
T1	4	HCS 6X12 6AWG(1-3/8)	160.00 -	0.6000	0.6000
T1	9	Feedline Ladder (Af)	170.00 160.00 -	0.6000	0.6000
l ''	ا	reedline Ladder (Al)	170.00	0.0000	0.0000
T1	20	Feedline Ladder (Af)	160.00 -	0.6000	0.6000
, ,		r coamie zadaci (/ ii)	180.00	0.0000	0,000
T1	33	Feedline Ladder (Af)	160.00 -	0.6000	0.6000
		, ,	161.00		
T1	35	CR 50 1873(1-5/8)	160.00 -	0.6000	0.6000
			161.00		
T1	36	FB-L98-002-XXX(3/8)	160.00 -	0.6000	0.6000
T1	37	WR-VG82ST-BRDA(5/8)	161.00 160.00 -	0.6000	0.6000
''	37	WK-VG0231-BKDA(3/0)	161.00	0.0000	0.0000
T1	38	WR-VG82ST-BRDA(5/8)	160.00 -	0.6000	0.6000
		,	161.00		
T1	40	Safety Line 3/8	160.00 -	0.6000	0.6000
		-	180.00		
T1	42	CU12PSM6P4XXX(1-3/4)	160.00 -	0.6000	0.6000
			180.00	0.0000	0.0000
T2	2	Feedline Ladder (Af)	153.33 -	0.6000	0.6000
T2	4	HCS 6X12 6AWG(1-3/8)	160.00 153.33 -	0.6000	0.6000
'2		1100 0212 02400(1-9/0)	160.00	0.0000	0.0000
T2	9	Feedline Ladder (Af)	153.33 -	0.6000	0.6000
		` '	160.00		
T2	17	7983A(ELLIPTICAL)	153.33 -	0.6000	0.6000
			157.00		
T2	18	Feedline Ladder (Af)	153.33 -	0.6000	0.6000
T2	20	Feedline Ladder (Af)	160.00 153.33 -	0,6000	0.6000
12	20	reedline Ladder (AI)	160.00	0.0000	0.8000
T2	33	Feedline Ladder (Af)	153.33 -	0.6000	0.6000
		. 334 24443. (7.1.)	160.00	0.0000	0.0000
T2	35	CR 50 1873(1-5/8)	153.33 -	0.6000	0.6000
			160.00		
T2	36	FB-L98-002-XXX(3/8)	153.33 -	0.6000	0.6000
	0.7	WE VOOSE BEEN (5/0)	160.00	0.0000	0.0000
T2	37	WR-VG82ST-BRDA(5/8)	153.33 - 160.00	0.6000	0.6000
T2	38	WR-VG82ST-BRDA(5/8)	153.33 -	0.6000	0.6000
'-		(30231 BRDA(370)	160.00	0.0000	0.0000
T2	40	Safety Line 3/8	153.33 -	0.6000	0.6000
		,	160.00		
T2	42	CU12PSM6P4XXX(1-3/4)	153.33 -	0.6000	0.6000
I		l	160.00		<b> </b>

Tower	Feed Line	Description	Feed Line	Ka	<b>K</b> a
Section	Record No.	2 300 i puoli	Segment Elev.	No Ice	Ice
ТЗ	2	Feedline Ladder (Af)	146.67 - 153.33	0.6000	0.6000
Т3	4	HCS 6X12 6AWG(1-3/8)	146.67 -	0.6000	0.6000
Т3	9	Feedline Ladder (Af)	153.33 146.67 -	0.6000	0.6000
Т3	12	MLE Hybrid 3Power/6Fiber	153.33 146.67 -	0.6000	0.6000
Т3	17	RL 2 10AWG(1-1/4) 7983A(ELLIPTICAL)	148.00 146.67 -	0.6000	0.6000
Т3	18	Feedline Ladder (Af)	153.33 146.67 -	0.6000	0.6000
Т3	20	Feedline Ladder (Af)	153.33 146.67 -	0.6000	0.6000
Т3	33	Feedline Ladder (Af)	153.33 146.67 -	0.6000	0.6000
Т3	35	CR 50 1873(1-5/8)	153.33 146.67 -	0.6000	0.6000
ТЗ	36	FB-L98-002-XXX(3/8)	153.33 146.67 -	0.6000	0.6000
Т3	37	WR-VG82ST-BRDA(5/8)	153.33 146.67 -	0.6000	0.6000
Т3	38	WR-VG82ST-BRDA(5/8)	153.33 146.67 -	0.6000	0.6000
ТЗ	40	Safety Line 3/8	153.33 146.67 -	0.6000	0.6000
Т3	42	CU12PSM6P4XXX(1-3/4)	153.33 146.67 -	0.6000	0.6000
T4	2	Feedline Ladder (Af)	153.33 140.00 -	0.6000	0.6000
T4	4	HCS 6X12 6AWG(1-3/8)	146.67 140.00 -	0.6000	0.6000
T4	9	Feedline Ladder (Af)	146.67 140.00 -	0.6000	0.6000
T4	12	MLE Hybrid 3Power/6Fiber	146.67 140.00 -	0.6000	0.6000
T4	17	RL 2 10AWG(1-1/4) 7983A(ELLIPTICAL)	146.67 140.00 -	0.6000	0.6000
T4	18	Feedline Ladder (Af)	146.67 140.00 -	0.6000	0.6000
T4	20	Feedline Ladder (Af)	146.67 140.00 -	0.6000	0.6000
T4	33	Feedline Ladder (Af)	146.67 140.00 -	0.6000	0.6000
T4	35	CR 50 1873(1-5/8)	146.67 140.00 -	0.6000	0.6000
T4	36	FB-L98-002-XXX(3/8)	146.67 140.00 -	0.6000	0.6000
T4	37	WR-VG82ST-BRDA(5/8)	146.67 140.00 -	0.6000	0.6000
T4	38	WR-VG82ST-BRDA(5/8)	146.67 140.00 -	0.6000	0.6000
T4	40	Safety Line 3/8	146.67 140.00 -	0.6000	0.6000
T4	42	CU12PSM6P4XXX(1-3/4)	146.67 140.00 -	0.6000	0.6000
Т5	2	Feedline Ladder (Af)	146.67 120.00 -	0.6000	0.6000
Т5	4	HCS 6X12 6AWG(1-3/8)	140.00 120.00 -	0.6000	0.6000
Т5	9	Feedline Ladder (Af)	140.00 120.00 -	0.6000	0.6000
Т5	12	MLE Hybrid 3Power/6Fiber	140.00 120.00 -	0.6000	0.6000
Т5	14	RL 2 10AWG(1-1/4) 2-1/4" Rigid Conduit	140.00 120.00 -	0.6000	0.6000
Т5	15	LDF4-50A(1/2)	136.00 120.00 -	0.6000	0.6000
Т5	16	9207(5/16)	136.00 120.00 -	0.0000	0.0000

Tower	Feed Line	Description	Feed Line	Ka	Ka
Section	Record No.	Везоприон	Segment	No Ice	Ice
			Elev.		
Т5	17	7983A(ELLIPTICAL)	136.00 120.00 - 140.00	0.6000	0.6000
Т5	18	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
T5	20	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
Т5	22	LDF7-50A(1 5/8)	120.00 - 126.00	0.6000	0.6000
T5	24	LDF4-50A(1/2)	120.00 - 126.00	0.6000	0.6000
Т5	25	Feedline Ladder (Af)	120.00 - 126.00	0.6000	0.6000
T5	33	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
T5	35	CR 50 1873(1-5/8)	120.00 - 140.00	0.6000	0.6000
T5	36	FB-L98-002-XXX(3/8)	120.00 - 140.00	0.6000	0.6000
T5	37	WR-VG82ST-BRDA(5/8)	120.00 - 140.00	0.6000	0.6000
T5	38	WR-VG82ST-BRDA(5/8)	120.00 - 140.00	0.6000	0.6000
T5	40	Safety Line 3/8	120.00 - 140.00	0.6000	0.6000
T5	42	CU12PSM6P4XXX(1-3/4)	120.00 - 140.00	0.6000	0.6000
Т6	2	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
Т6	4	HCS 6X12 6AWG(1-3/8)	100.00 - 120.00	0.6000	0.6000
Т6	9	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
Т6	12	MLE Hybrid 3Power/6Fiber RL 2 10AWG(1-1/4)	100.00 - 120.00	0.6000	0.6000
Т6	14	2-1/4" Rigid Conduit	100.00 - 120.00	0.6000	0.6000
Т6	15	LDF4-50A(1/2)	100.00 - 120.00	0.6000	0.6000
T6	16	9207(5/16)	100.00 - 120.00	0.0000	0.0000
Т6	17	7983A(ELLIPTICAL)	100.00 - 120.00	0.6000	0.6000
Т6	18	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
Т6	20	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T6	22	LDF7-50A(1 5/8)	100.00 - 120.00	0.6000	0.6000
T6	24	LDF4-50A(1/2)	100.00 - 120.00	0.6000	0.6000
T6	25	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T6	33	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T6	35	CR 50 1873(1-5/8)	100.00 - 120.00	0.6000	0.6000
T6	36	FB-L98-002-XXX(3/8)	100.00 - 120.00	0.6000	0.6000
T6 T6	37	WR-VG82ST-BRDA(5/8)	100.00 - 120.00	0.6000	0.6000
T6	38 40	WR-VG82ST-BRDA(5/8) Safety Line 3/8	100.00 - 120.00 100.00 -	0.6000	0.6000 0.6000
T6	40	CU12PSM6P4XXX(1-3/4)	120.00 - 120.00 100.00 -	0.6000	0.6000
T7	42	,	120.00	0.6000	0.6000
'/	2	Feedline Ladder (Af)	80.00 - 100.00	0.0000	0.0000

Tower	Feed Line	Description	Feed Line	Ka	<b>K</b> a
Section	Record No.	Везоприон	Segment	No Ice	Ice
	4	LICC CV12 CAMC(1 2/0)	Elev.	0.6000	0.6000
T7	4	HCS 6X12 6AWG(1-3/8)	80.00 - 100.00	0.6000	0.6000
Т7	9	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T7	12	MLE Hybrid 3Power/6Fiber	80.00 -	0.6000	0.6000
Т7	14	RL 2 10AWG(1-1/4) 2-1/4" Rigid Conduit	100.00 80.00 - 100.00	0.6000	0.6000
Т7	15	LDF4-50A(1/2)	80.00 - 100.00	0.6000	0.6000
T7	16	9207(5/16)	80.00 - 100.00	0.0000	0.0000
Т7	17	7983A(ELLIPTICAL)	80.00 - 100.00	0.6000	0.6000
T7	18	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
Т7	20	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
Т7	22	LDF7-50A(1 5/8)	80.00 - 100.00	0.6000	0.6000
Т7	24	LDF4-50A(1/2)	80.00 - 100.00	0.6000	0.6000
T7	25	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T7	33	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T7	35	CR 50 1873(1-5/8)	80.00 - 100.00	0.6000	0.6000
T7	36	FB-L98-002-XXX(3/8)	80.00 - 100.00	0.6000	0.6000
T7	37	WR-VG82ST-BRDA(5/8)	80.00 - 100.00	0.6000	0.6000
T7	38	WR-VG82ST-BRDA(5/8)	80.00 - 100.00	0.6000	0.6000
Т7	40	Safety Line 3/8	80.00 - 100.00	0.6000	0.6000
Т7	42	CU12PSM6P4XXX(1-3/4)	80.00 - 100.00	0.6000	0.6000
Т8	2	Feedline Ladder (Af)	70.00 - 80.00	0.6000	0.6000
Т8	4	HCS 6X12 6AWG(1-3/8)	70.00 - 80.00	0.6000	0.6000
Т8	9	Feedline Ladder (Af)	70.00 - 80.00	0.6000	0.6000
Т8	12	MLE Hybrid 3Power/6Fiber RL 2 10AWG(1-1/4)	70.00 - 80.00	0.6000	0.6000
Т8	14	2-1/4" Rigid Conduit	70.00 - 80.00	0.6000	0.6000
Т8	15	LDF4-50A(1/2)	70.00 - 80.00	0.6000	0.6000
Т8	16	9207(5/16)	70.00 - 80.00	0.0000	0.0000
Т8	17	7983A(ELLIPTICAL)	70.00 - 80.00	0.6000	0.6000
Т8	18	Feedline Ladder (Af)	70.00 - 80.00	0.6000	0.6000
Т8	20	Feedline Ladder (Af)	70.00 - 80.00	0.6000	0.6000
Т8	22	LDF7-50A(1 5/8)	70.00 - 80.00	0.6000	0.6000
Т8	24	LDF4-50A(1/2)	70.00 - 80.00	0.6000	0.6000
Т8	25	Feedline Ladder (Af)	70.00 - 80.00	0.6000	0.6000
Т8	33	Feedline Ladder (Af)	70.00 - 80.00	0.6000	0.6000
Т8	35	CR 50 1873(1-5/8)	70.00 - 80.00	0.6000	0.6000
Т8	36	FB-L98-002-XXX(3/8)		0.6000	0.6000

Tower	Feed Line	Description	Feed Line	Ka	<b>K</b> a
Section	Record No.	= 500.160011	Segment	No Ice	Ice
			<i>Elev.</i> 80.00		
Т8	37	WR-VG82ST-BRDA(5/8)	70.00 - 80.00	0.6000	0.6000
Т8	38	WR-VG82ST-BRDA(5/8)	70.00 - 80.00	0.6000	0.6000
Т8	40	Safety Line 3/8	70.00 - 80.00	0.6000	0.6000
Т8	42	CU12PSM6P4XXX(1-3/4)	70.00 - 80.00	0.6000	0.6000
Т9	2	Feedline Ladder (Af)	60.00 - 70.00	0.6000	0.6000
Т9	4	HCS 6X12 6AWG(1-3/8)	60.00 - 70.00	0.6000	0.6000
Т9	9	Feedline Ladder (Af)	60.00 - 70.00	0.6000	0.6000
Т9	12	MLE Hybrid 3Power/6Fiber RL 2 10AWG(1-1/4)	60.00 - 70.00	0.6000	0.6000
Т9	14	2-1/4" Rigid Conduit	60.00 - 70.00	0.6000	0.6000
Т9	15	LDF4-50A(1/2)	60.00 - 70.00	0.6000	0.6000
Т9	16	9207(5/16)	60.00 - 70.00	0.0000	0.0000
Т9	17	7983A(ELLIPTICAL)	60.00 - 70.00	0.6000	0.6000
Т9	18	Feedline Ladder (Af)	60.00 - 70.00	0.6000	0.6000
Т9	20	Feedline Ladder (Af)	60.00 - 70.00	0.6000	0.6000
Т9	22	LDF7-50A(1 5/8)	60.00 - 70.00	0.6000	0.6000
Т9	24	LDF4-50A(1/2)	60.00 - 70.00	0.6000	0.6000
Т9	25	Feedline Ladder (Af)	60.00 - 70.00	0.6000	0.6000
T9	33	Feedline Ladder (Af)	60.00 - 70.00	0.6000	0.6000
T9	35	CR 50 1873(1-5/8)	60.00 - 70.00	0.6000	0.6000
T9	36	FB-L98-002-XXX(3/8)	60.00 - 70.00	0.6000	0.6000
T9	37	WR-VG82ST-BRDA(5/8)	60.00 - 70.00	0.6000	0.6000
T9	38	WR-VG82ST-BRDA(5/8)	60.00 - 70.00	0.6000	0.6000
T9	40	Safety Line 3/8	60.00 - 70.00	0.6000	0.6000
T9	42	CU12PSM6P4XXX(1-3/4)	60.00 - 70.00	0.6000	0.6000
T10	2	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T10	4	HCS 6X12 6AWG(1-3/8)	40.00 - 60.00	0.6000	0.6000
T10	9	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T10	12	MLE Hybrid 3Power/6Fiber RL 2 10AWG(1-1/4)	40.00 - 60.00	0.6000	0.6000
T10	14	2-1/4" Rigid Conduit	40.00 - 60.00	0.6000	0.6000
T10	15	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T10	16	9207(5/16)	40.00 - 60.00	0.0000	0.0000
T10	17	7983A(ELLIPTICAL)	40.00 - 60.00	0.6000	0.6000
T10	18	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T10	20	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000

Tower	Feed Line	Description	Feed Line	Ka	<b>K</b> a
Section	Record No.	Боолрион	Segment	No Ice	lce
T10	22	LDF7-50A(1 5/8)	<i>Elev.</i> 40.00 -	0.6000	0.6000
			60.00		
T10	24	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T10	25	Feedline Ladder (Af)	40.00 -	0.6000	0.6000
T10	33	Feedline Ladder (Af)	60.00 40.00 - 60.00	0.6000	0.6000
T10	35	CR 50 1873(1-5/8)	40.00 - 60.00	0.6000	0.6000
T10	36	FB-L98-002-XXX(3/8)	40.00 - 60.00	0.6000	0.6000
T10	37	WR-VG82ST-BRDA(5/8)	40.00 - 60.00	0.6000	0.6000
T10	38	WR-VG82ST-BRDA(5/8)	40.00 - 60.00	0.6000	0.6000
T10	40	Safety Line 3/8	40.00 - 60.00	0.6000	0.6000
T10	42	CU12PSM6P4XXX(1-3/4)	40.00 -	0.6000	0.6000
T11	2	Feedline Ladder (Af)	60.00 20.00 - 40.00	0.6000	0.6000
T11	4	HCS 6X12 6AWG(1-3/8)	20.00 - 40.00	0.6000	0.6000
T11	9	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T11	12	MLE Hybrid 3Power/6Fiber RL 2 10AWG(1-1/4)	20.00 - 40.00	0.6000	0.6000
T11	14	2-1/4" Rigid Conduit	20.00 - 40.00	0.6000	0.6000
T11	15	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
T11	16	9207(5/16)	20.00 - 40.00	0.0000	0.0000
T11	17	7983A(ELLIPTICAL)	20.00 - 40.00	0.6000	0.6000
T11	18	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T11	20	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T11	22	LDF7-50A(1 5/8)	20.00 - 40.00	0.6000	0.6000
T11	24	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
T11	25	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T11	33	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T11	35	CR 50 1873(1-5/8)	20.00 - 40.00	0.6000	0.6000
T11	36	FB-L98-002-XXX(3/8)	20.00 - 40.00	0.6000	0.6000
T11	37	WR-VG82ST-BRDA(5/8)	20.00 - 40.00	0.6000	0.6000
T11	38	WR-VG82ST-BRDA(5/8)	20.00 - 40.00	0.6000	0.6000
T11	40	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000
T11	42	CU12PSM6P4XXX(1-3/4)	20.00 - 40.00	0.6000	0.6000
T12	2	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T12	4	HCS 6X12 6AWG(1-3/8)	0.00 - 20.00	0.6000	0.6000
T12	8	LDF1-50A(1/4)	0.00 - 5.00	0.6000	0.6000
T12	9	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T12	12	MLE Hybrid 3Power/6Fiber RL 2 10AWG(1-1/4)	0.00 - 20.00	0.6000	0.6000
T12	14	2-1/4" Rigid Conduit	0.00 - 20.00	0.6000	0.6000
T12	15 16	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T12	16	9207(5/16)	0.00 - 20.00	0.0000	0.0000

Tower	Feed Line	Description	Feed Line	<b>K</b> a	$K_a$
Section	Record No.	·	Segment	No Ice	Ice
			Elev.		
T12	17	7983A(ELLIPTICAL)	0.00 - 20.00	0.6000	0.6000
T12	18	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T12	20	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T12	22	LDF7-50A(1 5/8)	0.00 - 20.00	0.6000	0.6000
T12	24	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T12	25	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T12	33	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T12	35	CR 50 1873(1-5/8)	0.00 - 20.00	0.6000	0.6000
T12	36	FB-L98-002-XXX(3/8)	0.00 - 20.00	0.6000	0.6000
T12	37	WR-VG82ST-BRDA(5/8)	0.00 - 20.00	0.6000	0.6000
T12	38	WR-VG82ST-BRDA(5/8)	0.00 - 20.00	0.6000	0.6000
T12	40	Safety Line 3/8	0.00 - 20.00	0.6000	0.6000
T12	42	CU12PSM6P4XXX(1-3/4)	0.00 - 20.00	0.6000	0.6000

Discrete	Tower	Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement
			ft ft ft	۰	ft
*170*					
AIR 32 B2A/B66AA w/ Mount Pipe	Α	From Leg	4.000 0' 0'	0.000	170'
AIR 32 B2A/B66AA w/ Mount Pipe	В	From Leg	4.000 0' 0'	0.000	170'
AIR 32 B2A/B66AA w/ Mount Pipe	С	From Leg	4.000 0' 0'	0.000	170'
APXVAARR24_43-U-NA20 w/ Mount Pipe	Α	From Leg	4.000 0'	0.000	170'
APXVAARR24_43-U-NA20 w/ Mount Pipe	В	From Leg	0' 4.000 0'	0.000	170'
APXVAARR24_43-U-NA20 w/ Mount Pipe	С	From Leg	0' 4.000 0'	0.000	170'
AIR 3246 B66_T-MOBILE w/ Mount Pipe	Α	From Leg	0' 4.000 0' 0'	0.000	170'
AIR 3246 B66_T-MOBILE w/ Mount Pipe	В	From Leg	4.000 0' 0'	0.000	170'
AIR 3246 B66_T-MOBILE w/ Mount Pipe	С	From Leg	4.000 0' 0'	0.000	170'
AIR6449 B41 w/ Mount Pipe	Α	From Leg	4.000 0' 0'	0.000	170'
AIR6449 B41 w/ Mount Pipe	В	From Leg	4.000 0' 0'	0.000	170'
AIR6449 B41 w/ Mount Pipe	С	From Leg	4.000 0' 0'	0.000	170'
RADIO 4449 B71 B85A_T-MOBILE	Α	From Leg	4.000 0'	0.000	170'

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placemen
	Leg		Vert ft ft ft	o	ft
RADIO 4449 B71 B85A_T-MOBILE	В	From Leg	0' 4.000 0'	0.000	170'
RADIO 4449 B71 B85A_T-MOBILE	С	From Leg	0' 4.000 0'	0.000	170'
RRUS 4415 B25_CCIV2	Α	From Leg	0' 4.000 0'	0.000	170'
RRUS 4415 B25_CCIV2	В	From Leg	0' 4.000 0'	0.000	170'
RRUS 4415 B25_CCIV2	С	From Leg	0' 4.000 0'	0.000	170'
Sector Mount [SM 702-3] *161*	С	None	0'	0.000	170'
HPA-65R-BUU-H6	Α	From Leg	4.000 0'	0.000	161'
HPA-65R-BUU-H6	В	From Leg	0' 4.000 0'	0.000	161'
SBNHH-1D65A	С	From Leg	0' 4.000 0'	0.000	161'
QS66512-2	Α	From Leg	0' 4.000 0'	0.000	161'
QS66512-2	В	From Leg	0' 4.000 0'	0.000	161'
QS46512-2	С	From Leg	0' 4.000 0'	0.000	161'
RRUS 4426 B66	Α	From Leg	0' 4.000 0'	0.000	161'
RRUS 4426 B66	В	From Leg	0' 4.000 0'	0.000	161'
RRUS 4426 B66	С	From Leg	0' 4.000 0'	0.000	161'
RRUS 32 B2	В	From Leg	0' 4.000 0'	0.000	161'
RRUS 32 B2	Α	From Leg	0' 4.000 0'	0.000	161'
RRUS 32 B2	С	From Leg	0' 4.000 0'	0.000	161'
DC6-48-60-18-8F	Α	From Leg	0' 4.000 0'	0.000	161'
DC6-48-60-18-8F	В	From Leg	0' 4.000 0'	0.000	161'
DC6-48-60-18-8F	С	From Leg	0' 4.000 0'	0.000	161'
	Α		0' 4.000		161'

Description	Face	Offset Type	Offsets: Horz	Azimuth	Placement
	or Leg	Type	Horz Lateral	Adjustment	
	J		Vert		
			ft ft	٥	ft
			ft		
DDU2 44		F	0'	0.000	4041
RRUS 11	В	From Leg	4.000 0'	0.000	161'
			0'		
RRUS 11	С	From Leg	4.000	0.000	161'
			0'		
RRUS 32	Α	From Leg	0' 4.000	0.000	161'
KK03 32	A	From Leg	4.000 0'	0.000	101
			0'		
RRUS 32	В	From Leg	4.000	0.000	161'
			0'		
RRUS 32	С	From Leg	0' 4.000	0.000	161'
KKU3 32	C	From Leg	4.000 0'	0.000	101
			0'		
Sector Mount [SM 201-3]	С	None		0.000	161'
OPA65R-BU6D w/ Mount Pipe	Α	From Leg	4.000	0.000	161'
			0' 0'		
OPA65R-BU6D w/ Mount Pipe	В	From Leg	4.000	0.000	161'
or moon book with mount inpo	5	r rom 20g	0'	0.000	101
			0'		
OPA65R-BU4D w/ Mount Pipe	С	From Leg	4.000	0.000	161'
			0' 0'		
RRUS 4478 B5	Α	From Leg	4.000	0.000	161'
14.65 1176 26	, ,	r rom 20g	0'	0.000	101
			0'		
RRUS 4478 B5	В	From Leg	4.000	0.000	161'
			0' 0'		
RRUS 4478 B5	С	From Leg	4.000	0.000	161'
14.65 1116 26	Ū	1 10111 209	0'	0.000	101
			0'		
RRUS 4478 B14	Α	From Leg	4.000	0.000	161'
			0' 0'		
RRUS 4478 B14	В	From Leg	4.000	0.000	161'
			0'		
	_		0'		
RRUS 4478 B14	С	From Leg	4.000	0.000	161'
			0' 0'		
DC6-48-60-18-8F	Α	From Leg	4.000	0.000	161'
		· ·	0'		
*457*			0'		
*157* Side Arm Mount [SO 203-1]	Α	From Leg	1.500	0.000	157'
Side Aim Mount [50 203-1]	A	From Leg	0'	0.000	137
			0'		
Side Arm Mount [SO 203-1]	В	From Leg	1.500	0.000	157'
			0'		
*148*			0'		
APXVTM14-ALU-I20 w/ Mount Pipe	Α	From Leg	4.000	0.000	148'
	-	3	0'		
AD20/T1444	_		0'		
APXVTM14-ALU-I20 w/ Mount Pipe	В	From Leg	4.000	0.000	148'
			0' 0'		
APXVTM14-ALU-I20 w/ Mount Pipe	С	From Leg	4.000	0.000	148'
	-		0'	000	
			0'		
APXVSPP18-C-A20 w/ Mount Pipe	Α	From Leg	4.000	0.000	148'

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement
	Log		Vert ft ft	۰	ft
			ft 0'		
APXVSPP18-C-A20 w/ Mount Pipe	В	From Leg	0' 4.000 0'	0.000	148'
APXVSPP18-C-A20 w/ Mount Pipe	С	From Leg	0' 4.000 0'	0.000	148'
(2) PCS 1900MHz 4x45W-65MHz	Α	From Leg	0' 4.000 0'	0.000	148'
(2) PCS 1900MHz 4x45W-65MHz	В	From Leg	0' 4.000 0'	0.000	148'
(2) PCS 1900MHz 4x45W-65MHz	С	From Leg	0' 4.000 0'	0.000	148'
800MHZ 2X50W RRH	Α	From Leg	0' 4.000 0'	0.000	148'
800MHZ 2X50W RRH	В	From Leg	0' 4.000 0'	0.000	148'
800MHZ 2X50W RRH	С	From Leg	0' 4.000 0'	0.000	148'
800 EXTERNAL NOTCH FILTER	Α	From Leg	0' 4.000 0'	0.000	148'
800 EXTERNAL NOTCH FILTER	В	From Leg	0' 4.000 0'	0.000	148'
800 EXTERNAL NOTCH FILTER	С	From Leg	0' 4.000 0' 0'	0.000	148'
(3) ACU-A20-N	Α	From Leg	4.000 0'	0.000	148'
(3) ACU-A20-N	В	From Leg	0' 4.000 0'	0.000	148'
(3) ACU-A20-N	С	From Leg	0' 4.000 0'	0.000	148'
Sector Mount [SM 502-3]	С	None	0'	0.000	148'
*136* LLPX310R w/ Mount Pipe	Α	From Leg	4.000 0'	0.000	136'
LLPX310R w/ Mount Pipe	В	From Leg	0' 4.000 0'	0.000	136'
LLPX310R w/ Mount Pipe	С	From Leg	0' 4.000 0'	0.000	136'
RRH-2WB	Α	From Leg	0' 4.000 0'	0.000	136'
RRH-2WB	В	From Leg	0' 4.000 0'	0.000	136'
RRH-2WB	С	From Leg	0' 4.000	0.000	136'
			0' 0'		

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placeme
	Leg		Lateral Vert		
			ft ft	•	ft
			ft		
			0' 0'		
(3) 6' x 2" Mount Pipe	Α	From Leg	4.000 0'	0.000	136'
			0'		
(3) 6' x 2" Mount Pipe	В	From Leg	4.000 0'	0.000	136'
(3) 6' x 2" Mount Pipe	С	From Leg	0' 4.000	0.000	136'
(3) 0 XZ Would Fibe	C	i ioni Leg	0'	0.000	130
6' x 3" Mount Pipe	Α	From Leg	0' 4.000	0.000	136'
·		-	0' 0'		
6' x 3" Mount Pipe	С	From Leg	4.000	0.000	136'
			0' 0'		
Sector Mount [SM 504-3] *126*	С	None		0.000	136'
(2) JAHH-65C-R3B w/ Mount Pipe	Α	From Leg	4.000	0.000	126'
			0' 3'		
(2) JAHH-65C-R3B w/ Mount Pipe	В	From Leg	4.000 0'	0.000	126'
(2) IALUL CEC D2D w/ Mount Ding	0	Frank Lan	3'	0.000	400
(2) JAHH-65C-R3B w/ Mount Pipe	С	From Leg	4.000 0'	0.000	126'
CBRS w/ Mount Pipe	Α	From Leg	3' 4.000	0.000	126'
			0' 7'		
CBRS w/ Mount Pipe	В	From Leg	4.000	0.000	126'
			0' 7'		
CBRS w/ Mount Pipe	С	From Leg	4.000 0'	0.000	126'
0.10.1.			7'	0.000	100
Sub6 Antenna - VZS01 w/ Mount Pipe	Α	From Leg	4.000 0'	0.000	126'
Sub6 Antenna - VZS01 w/ Mount Pipe	В	From Leg	2' 4.000	0.000	126'
oubo / titelina	D	r rom Leg	0'	0.000	120
Sub6 Antenna - VZS01 w/ Mount Pipe	С	From Leg	2' 4.000	0.000	126'
			0' 2'		
DB-C1-12C-24AB-0Z	С	From Leg	4.000	0.000	126'
			0' 3'		
(3) RFV01U-D1A	Α	From Leg	4.000 0'	0.000	126'
(2) RFV01U-D2A	В	From Leg	3' 4.000	0.000	126'
(2) IN VOID-DZM	ь	i ioni Leg	0'	0.000	120
RFV01U-D2A	С	From Leg	3' 4.000	0.000	126'
		-	0' 3'		
(2) DB844G65ZAXY w/ Mount Pipe	Α	From Leg	4.000	0.000	126'
			0' 3'		
DB844H80-XY w/ Mount Pipe	В	From Leg	4.000 0'	0.000	126'
DD044100 V0// M 1 B'	5	Facility 1	3'	0.000	400'
DB844H80-XY w/ Mount Pipe	В	From Leg	4.000	0.000	126'

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement
	209		Vert ft ft ft	o	ft
			0'		
(2) DB844G65ZAXY w/ Mount Pipe	С	From Leg	3' 4.000 0'	0.000	126'
GPS_A	В	From Leg	3' 4.000 0'	0.000	126'
(2) 6' x 2" Mount Pipe	В	From Leg	4' 4.000 0'	0.000	126'
6' x 2" Mount Pipe	С	From Leg	0' 4.000 0'	0.000	126'
5' x 2" Pipe Mount	В	From Leg	0' 1.000 0'	0.000	126'
Sector Mount [SM 411-3]	С	None	0'	0.000	126'
*112* 800 10504 w/ Mount Pipe	A	From Leg	4.000 0'	0.000	112'
800 10504 w/ Mount Pipe	В	From Leg	0' 4.000	0.000	112'
800 10504 w/ Mount Pipe	С	From Leg	0' 0' 4.000	0.000	112'
6' x 2" Mount Pipe	Α	From Leg	0' 0' 4.000	0.000	112'
6' x 2" Mount Pipe	В	From Leg	0' 0' 4.000 0'	0.000	112'
6' x 2" Mount Pipe	С	From Leg	0' 4.000 0'	0.000	112'
Sector Mount [SM 104-3]	С	None	0'	0.000	112'
*5* GPS_A	Α	From Face	2.000 0'	0.000	5'
ASPP2933	Α	From Face	0' 0.500 0'	0.000	5'
3' x 2" Pipe Mount	Α	From Face	1' 2.000 0'	0.000	5'
Side Arm Mount [SO 701-1]	Α	From Face	0' 1.500 0'	0.000	5'
**			0'		
MX08FRO665-21 w/ Mount Pipe	Α	From Leg	4.000 0'	0.000	181'
MX08FRO665-21 w/ Mount Pipe	В	From Leg	0' 4.000 0'	0.000	181'
MX08FRO665-21 w/ Mount Pipe	С	From Leg	0' 4.000 0'	0.000	181'
TA08025-B604	Α	From Leg	0' 4.000 0'	0.000	181'
TA08025-B604	В	From Leg	0' 4.000	0.000	181'

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement
	Leg	Туре	Lateral	Aujustinent	
	Log		Vert		
			ft	•	ft
			ft		
			ft		
			0'		
			0'		
TA08025-B604	С	From Leg	4.000	0.000	181'
			0'		
TA 2000 F DOOF			0'	0.000	4041
TA08025-B605	Α	From Leg	4.000	0.000	181'
			0' 0'		
TA08025-B605	В	From Leg	4.000	0.000	181'
1A00023-B003	Ь	i ioni Leg	4.000 0'	0.000	101
			0'		
TA08025-B605	С	From Leg	4.000	0.000	181'
	_		0'		
			0'		
RDIDC-9181-PF-48	Α	From Leg	4.000	0.000	181'
			0'		
			0'		
(2) 8' x 2" Mount Pipe	Α	From Leg	4.000	0.000	181'
			0'		
(O) OL Oll Marrort Direct	Б	<b>F</b>	0'	0.000	4041
(2) 8' x 2" Mount Pipe	В	From Leg	4.000	0.000	181'
			0' 0'		
(2) 8' x 2" Mount Pipe	С	From Leg	4.000	0.000	181'
(2) 0 X 2 Mount 1 Ipe	G	i ioni Leg	4.000 0'	0.000	101
			0'		
Commscope MTC3975083 (3)	С	None	-	0.000	181'
***					

	Dishes								
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	
				ft	۰	0	ft	ft	
VHLP2-18	А	Paraboloid w/Shroud (HP)	From Leg	3.000 0' 0'	-10.000		157'	2.175	
VHLP2-18	В	Paraboloid w/Shroud (HP)	From Leg	3,000 0' 0'	-40.000		157'	2.175	
** VHLP2-23 **	А	Paraboloid w/Shroud (HP)	From Leg	4.000 0' 2'	50.000		136'	2.175	

### **Load Combinations**

Comb.		Description	
No.			
1	Dead Only		

- 1.2 Dead+1.0 Wind 0 deg No Ice 0.9 Dead+1.0 Wind 0 deg No Ice 1.2 Dead+1.0 Wind 30 deg No Ice

Comb. No.	Description
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

# **Maximum Member Forces**

Sectio	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
n	ft	Type		Load		Moment	Moment
No.				Comb.	K	kip-ft	kip-ft
T1	180 - 160	Leg	Max Tension	15	9.841	-0.168	0.016
			Max. Compression	10	-17 157	0.609	0.057
			Max. Mx	22	7.683	-0.670	-0.050
			Max. My	17	-3.485	-0.020	0.560
			Max. Vy	22	-1.348	0.025	0.004
			Max. Vx	16	1.221	0.009	-0.115
		Diagonal	Max Tension	24	3.105	0.000	0.000
		_	Max. Compression	25	-3.019	0.000	0.000
			Max. Mx	32	0.502	0.030	0.004
			Max. My	34	0.963	0.029	0.004
			Max. Vy	28	0.030	0.029	-0.004
			Max Vx	34	-0.002	0.000	0.000
		Top Girt	Max Tension	3	0.426	0.000	0.000
		•	Max. Compression	14	-0.482	0.000	0.000
			Max. Mx	26	-0.087	-0.056	0.000

Sectio n	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axis Moment
No.				Comb.	K	kip-ft	kip-ft
			Max. My	26	-0.085	0.000	0.002
			Max. Vy	26	-0.034	0.000	0.000
			Max Vx	26	-0.001	0.000	0.000
		Mid Girt	Max Tension	18	0.483	0.000	0.000
			Max. Compression	23	-0.480	0.000	0.000
			Max, Mx	26	0.010	-0.075	0.000
			Max. My	26	0.008	0.000	0.002
			Max. Vy	26	-0.039	0.000	0.000
			Max. Vx	26	-0.001	0.000	0.000
T2	160 -	Leg	Max Tension	15	15.867	-0.662	0.071
12	153.333	Leg	Max Terision	13	13.007	-0.002	0.071
	100.000		May Communica	10	04 640	0.000	0.000
			Max. Compression		-24.613	0.200	-0.020
			Max. Mx	22	14.577	-0.670	-0.050
			Max. My	17	-3.532	-0.020	0.560
			Max. Vy	22	-0.206	-0.670	-0.050
			Max. Vx	16	0.215	-0.028	0.560
		Diagonal	Max Tension	13	4.808	0.000	0.000
			Max. Compression	12	-5.019	0.000	0.000
			Max. Mx	30	1.075	0.049	0.006
			Max. My	38	-1.695	0.044	-0.008
			Max. Vý	29	0.042	0.048	0.006
			Max. Vx	38	0.003	0.000	0.000
Т3	153.333 -	Leg	Max Tension	15	25.052	-0.227	0.001
10	146.667	Log	Max Tension	13	20.002	0.221	0.001
	140.007		Max. Compression	10	-35.886	0.457	-0.042
							-0.042
			Max. Mx	6	22.535	0.520	
			Max. My	20	-5.032	-0.026	0.538
			Max. Vy	6	0.756	-0.475	-0.012
			Max. Vx	13	0.714	-0.015	-0.397
		Diagonal	Max Tension	13	4.943	0.000	0.000
			Max. Compression	12	-5.281	0.000	0.000
			Max. Mx	31	0.769	0.055	-0.008
			Max. My	28	0.805	0.054	-0.008
			Max. Vy	29	0.045	0.054	-0.008
			Max. Vx	28	0.003	0.000	0.000
		Top Girt	Max Tension	14	0.886	0.000	0.000
		. op 0t	Max. Compression	11	-0.523	0.000	0.000
			Max. Mx	26	0.525	-0.131	0.000
			Max. My	26	0.507	0.000	0.004
			•	26	0.055	0.000	
			Max. Vy				0.000
			Max. Vx	26	-0.002	0.000	0.000
T4	146.667 -	Leg	Max Tension	15	33.977	-0.468	-0.035
	140						
			Max. Compression	10	-46.791	0.178	-0.027
			Max. Mx	6	31.997	-0.475	-0.012
			Max. My	12	-6.472	-0.020	-0.397
			Max. Vy	2	0.103	0.459	0.036
			Max. Vx	13	-0.103	-0.015	-0.397
		Diagonal	Max Tension	13	5.795	0.000	0.000
		J	Max. Compression	12	-5.954	0.000	0.000
			Max. Mx	31	1.244	0.060	-0.008
			Max. My	35	-1.871	0.050	0.009
			Max. Vy	33	0.047	0.058	0.008
			Max. Vx	35	-0.003	0.000	0.000
		Ton Cirt					
		Top Girt	Max Tension	29	0.515	0.000	0.000
			Max. Compression	11	-0.187	0.000	0.000
			Max. Mx	26	0.430	-0.150	0.000
			Max. My	26	0.424	0.000	0.004
			Max. Vy	26	0.059	0.000	0.000
			Max. Vx	26	-0.002	0.000	0.000
T5	140 - 120	Leg	Max Tension	15	66.042	-0.384	-0.022
		=	Max. Compression	10	-85.931	0.539	-0.044
			Max. Mx	6	40.925	0.778	0.019
			Max. My	12	-10.687	-0.006	-0.785
			Max. Vy	14	-1.026	-0.393	-0.022
			Max. Vx	12	-1.008	-0.039	-0.311
		Diagonal	Max Tension	20	7.772	0.000	0.000
		שומאסווםו	Max. Compression	20	7.772	0.000	0.000
			Max. Mx	33	1.820	0.082	-0.011

Sectio n	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axis Moment
No.			NA= NA	Comb.	K	kip-ft	kip-ft
			Max. My	37	-2.016 0.057	0.075 0.082	0.011
			Max. Vy Max. Vx	33 37	-0.003	0.002	-0.011 0.000
Т6	120 - 100	Leg	Max Tension	15	102.991	-0.484	-0.006
	120 100	Log	Max. Compression	10	-128.015	0.840	-0.090
			Max. Mx	3	-124.738	0.850	0.032
			Max. My	12	-11.095	-0.006	0.785
			Max. Vy	6	-0.463	-0.625	-0.030
			Max. Vx	24	0.437	-0.025	0.458
		Diagonal	Max Tension	20	9.089	0.000	0.000
			Max. Compression	20	-9.061	0.000	0.000
			Max. Mx	35	2.346	0.124	0.015
			Max. My	37	-2.341	0.110	0.016
			Max. Vy	33	0.075	0.121	-0.014
T7	100 00	1	Max. Vx	37	-0.004	0.000	0.000
Т7	100 - 80	Leg	Max Tension	15 10	135.397	-0.550 0.619	-0.028
			Max. Compression Max. Mx	3	-164.542 -140.996	0.850	-0.061 0.032
			Max. My	12	-15.004	-0.030	-0.859
			Max. Vy	6	-0.141	-0.818	-0.060
			Max. Vx	10	-0.181	-0.318	-0.796
		Diagonal	Max Tension	8	9.996	0.000	0.000
		214901141	Max. Compression	8	9.928	0.000	0.000
			Max. Mx	33	2.603	0.194	0.026
			Max. My	31	2.274	0.188	-0.028
			Max. Vy	33	0.097	0.194	0.026
			Max. Vx	31	0.006	0.000	0.000
Т8	80 - 70	Leg	Max Tension	15	152.246	-0.660	-0.016
			Max. Compression	10	-183.519	2.215	-0.201
			Max. Mx	2	-183.437	2.241	0.074
			Max. My	12	-16.609	0.058	-2.040
			Max. Vy	2	-0.291	2.241	0.074
		D'I	Max. Vx	12	0.312	0.058	-2.040
		Diagonal	Max Tension	8	10.283	0.000	0.000
			Max. Compression	8 33	-10.407	0.000 0.216	0.000
			Max. Mx Max. My	33 37	2.253 2.126	0.215	-0.027 0.028
			Max. Vy	33	0.102	0.216	-0.027
			Max. Vx	37	0.005	0.000	0.000
T9	70 - 60	Leg	Max Tension	15	168.910	-2.020	-0.078
		3	Max. Compression	2	-202.841	0.329	-0.025
			Max. Mx	2	-202.596	2.241	0.074
			Max. My	12	-17.372	0.058	-2.040
			Max. Vy	2	0.318	2.241	0.074
			Max. Vx	24	0.319	0.054	2.034
		Diagonal	Max Tension	8	10.966	0.000	0.000
			Max. Compression	8	-11.141	0.000	0.000
			Max. Mx	33	2.672	-0.369	-0.043
			Max. My	37	-3.433	-0.330	-0.053
			Max. Vy Max. Vx	33 31	-0.172 -0.009	-0.369 0.000	-0.043 0.000
T10	60 - 40	Leg	Max Tension	15	201.617	-1.276	-0.028
110	00 - 40	Log	Max. Compression	2	-241.080	1 668	0.037
			Max. Mx	37	19,001	2.511	0.069
			Max. My	12	-19.488	-0.029	-1.417
			Max. Vý	29	0.382	-2.495	-0.040
			Max. Vx	24	-0.251	-0.032	1.406
		Diagonal	Max Tension	8	11.253	0.000	0.000
			Max. Compression	10	-11.622	0.000	0.000
			Max. Mx	33	2.284	0.291	-0.034
			Max. My	37	-3.114	0.262	0.039
			Max. Vy	33	0.128	0.290	0.036
T4.4	40.00	1	Max. Vx	31	0.007	0.000	0.000
T11	40 - 20	Leg	Max Tension	15	233.167	-0.889	-0.005
			Max. Compression	2 37	-278.864	2.725 -5.859	0.069
			Max. Mx Max. My	37 12	22.632 -22.803	-5.859 -0.205	0.082 -1.507
			Max. Vy	29	0.974	-5.824	-0.047
			Max. Vx	12	-0.195	-0.205	-1.507
					51.50	5.250	

Sectio	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
n	ft	Type		Load		Moment	Moment
No.				Comb.	K	kip-ft	kip-ft
		Diagonal	Max Tension	8	11.839	0.000	0.000
			Max. Compression	10	-12.359	0.000	0.000
			Max. Mx	33	1.601	0.390	0.042
			Max. My	31	4.654	0.319	-0.049
			Max. Vy	33	0.147	0.390	0.042
			Max. Vx	31	0.008	0.000	0.000
T12	20 - 0	Leg	Max Tension	15	262.575	-1.119	-0.027
			Max. Compression	2	-316.499	0.000	-0.000
			Max. Mx	37	29.451	-5.859	0.082
			Max. My	12	-26.735	-0.283	-2.877
			Max. Vy	29	-1.142	-5.824	-0.047
			Max. Vx	12	-0.431	-0.283	-2.877
		Diagonal	Max Tension	8	12.809	0.000	0.000
		-	Max. Compression	10	-13.697	0.000	0.000
			Max. Mx	33	-1.081	-0.839	0.073
			Max. My	32	8.039	-0.482	0.099
			Max. Vy	33	-0.267	-0.839	0.073
			Max. Vx	32	-0.014	0.000	0.000

### **Maximum Reactions**

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load	Κ	K	K
		Comb.			
Leg C	Max. Vert	18	322,003	33,271	-18.354
	Max. H <sub>x</sub>	18	322.003	33.271	-18.354
	Max. H <sub>z</sub>	7	-259.329	-27.849	15.272
	Min. Vert	7	-259.329	-27.849	15.272
	Min. H <sub>x</sub>	7	-259.329	-27.849	15.272
	Min. H <sub>z</sub>	18	322.003	33.271	-18.354
Leg B	Max. Vert	10	324.665	-33.938	-18.239
_	Max. H <sub>x</sub>	23	-264.499	28.581	15.215
	Max. H <sub>z</sub>	25	-235.160	24.566	15.699
	Min. Vert	23	-264.499	28.581	15.215
	Min. H <sub>x</sub>	10	324.665	-33.938	-18.239
	Min. H <sub>z</sub>	10	324,665	-33.938	-18.239
Leg A	Max. Vert	2	325,956	-0.408	39.031
	Max. H <sub>x</sub>	21	18.322	3.478	1.734
	Max. H <sub>z</sub>	2	325.956	-0.408	39.031
	Min. Vert	15	-269.802	0.393	-32.977
	Min. H <sub>x</sub>	8	24.692	-3.496	2.360
	Min. H <sub>z</sub>	15	-269.802	0.393	-32.977

### **Tower Mast Reaction Summary**

Load Combination	Vertical	Shear <sub>x</sub>	Shear₂	Overturning Moment. M <sub>×</sub>	Overturning Moment, Mz	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
Dead Only	66.048	0.000	0.000	32,518	11.850	0.000
1.2 Dead+1.0 Wind 0 deg - No Ice	79.258	0.067	-62.284	-6485.162	6.903	-16.907
0.9 Dead+1.0 Wind 0 deg - No Ice	59.444	0.067	-62.284	-6494.918	3.348	-16.907
1.2 Dead+1.0 Wind 30 deg - No Ice	79.258	28.642	-49.665	-5246.099	-3030.820	23.886
0.9 Dead+1.0 Wind 30 deg - No Ice	59.444	28.642	-49.665	-5255.854	-3034.375	23.886
1.2 Dead+1.0 Wind 60 deg - No Ice	79.258	49.147	-28.458	-3003.106	-5235.550	31.157
0.9 Dead+1.0 Wind 60 deg - No Ice	59.444	49.147	-28.458	-3012.862	-5239.105	31.157

Load Combination	Vertical	Shear <sub>x</sub>	Shear <sub>z</sub>	Overturning Moment, M <sub>x</sub>	Overturning Moment, Mz	Torque
1.2 Dead+1.0 Wind 90 deg -	<i>К</i> 79.258	<i>K</i> 56.273	-0.030	kip-ft 37.403	kip-ft -6033.471	kip-ft 32.343
No Ice	79.230	30.273	-0.030	37.403	-0033.471	32.343
0.9 Dead+1.0 Wind 90 deg -	59.444	56.273	-0.030	27.647	-6037.027	32.343
No Ice 1.2 Dead+1.0 Wind 120 deg	79.258	53.052	30,679	3271,145	-5567.536	49,244
- No Ice	79.236	55.052	30.079	3271.143	-5507.550	49.244
0.9 Dead+1.0 Wind 120 deg	59.444	53.052	30.679	3261.389	-5571.091	49.244
- No Ice 1.2 Dead+1.0 Wind 150 deg	79.258	30.248	52.506	5574.888	-3173.490	45.222
- No Ice	13.230	30.240	32.300	3374.000	-3173.430	45.222
0.9 Dead+1.0 Wind 150 deg	59.444	30.248	52.506	5565.132	-3177.045	45.222
- No Ice 1.2 Dead+1.0 Wind 180 deg	79.258	-0.075	58.854	6280.146	22.375	17.190
- No Ice						
0.9 Dead+1.0 Wind 180 deg - No Ice	59.444	-0.075	58.854	6270.390	18.820	17.190
1.2 Dead+1.0 Wind 210 deg	79.258	-28.698	49.724	5332.970	3067.521	-23.743
- No Ice	50 444	00.000	10.704	5000.044	2022.222	00.740
0.9 Dead+1.0 Wind 210 deg - No Ice	59.444	-28.698	49.724	5323.214	3063.966	-23.743
1.2 Dead+1.0 Wind 240 deg	79.258	-52.211	30.244	3233.295	5522.842	-31.201
- No Ice 0.9 Dead+1.0 Wind 240 deg	59,444	-52.211	30.244	3223.539	5519.287	-31.201
- No Ice	33.444	-52.211	30.244	3223.339	3313.207	-51.201
1.2 Dead+1.0 Wind 270 deg	79.258	-56.338	0.041	42.066	6071.492	-32.262
- No Ice 0.9 Dead+1.0 Wind 270 deg	59.444	-56.338	0.041	32.310	6067.937	-32.262
- No Ice						
1.2 Dead+1.0 Wind 300 deg - No Ice	79.258	-50.074	-28.896	-3041.376	5349.826	-49.328
0.9 Dead+1.0 Wind 300 deg	59.444	-50.074	-28.896	-3051.131	5346.271	-49.328
- No Ice 1.2 Dead+1.0 Wind 330 deg	79.258	-30.248	-52,468	-5490.814	3202.153	-45.483
- No Ice	79.236	-30.246	-32.400	-5490.614	3202.133	-45.465
0.9 Dead+1.0 Wind 330 deg	59.444	-30.248	-52.468	-5500.569	3198.598	-45.483
- No Ice 1.2 Dead+1.0 Ice+1.0 Temp	175.612	0.000	-0.000	132.760	52.679	0.000
1.2 Dead+1.0 Wind 0	175.612	0.018	-18.676	-1818.040	50.701	-3.669
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 30	175,612	8.750	-15.161	-1474.711	-874.673	8,369
deg+1.0 Ice+1.0 Temp	175.012	0.750	-13.101	-14/4./11	-074.073	0.509
1.2 Dead+1.0 Wind 60	175.612	14.621	-8.459	-775.366	-1516.376	11.603
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 90	175.612	17.316	-0.010	132,001	-1801.372	14.734
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	175.612	16.012	9.251	1108.725	-1635.323	20.447
1.2 Dead+1.0 Wind 150	175.612	9,274	16.088	1821,708	-920.791	17.671
deg+1.0 Ice+1.0 Temp	475.040	0.040		0044.000	54.000	0.700
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	175.612	-0.019	18.149	2041.388	54.839	3.730
1.2 Dead+1.0 Wind 210	175.612	-8.762	15.173	1742.122	981.801	-8.338
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 240	175.612	-15.098	8.738	1064.247	1661.196	-11.612
deg+1.0 Ice+1.0 Temp	175.012	-13.030	0.730	1004.247	1001.190	-11.012
1.2 Dead+1.0 Wind 270	175.612	-17.330	0.012	133.825	1908.782	-14.717
deg+1.0 Ice+1.0 Temp 1.2 Dead+1.0 Wind 300	175.612	-15.554	-8.973	-819.934	1703.940	-20.465
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	175.612	-9.275	-16.080	-1554.900	1026.196	-17.727
Dead+Wind 0 deg - Service	66.048	0.018	-16.903	-1720.482	9.925	-4.449
Dead+Wind 30 deg - Service	66.048	7.793	-13.513	-1389.574	-807.531	6.282
Dead+Wind 60 deg - Service	66.048	13.377	-7.745	-786.096	-1400.940	8.193
Dead+Wind 90 deg - Service	66.048 66.048	15.321 14.405	-0.008 8.330	32.092 901.132	-1615.757 1488 305	8.504 12.952
Dead+Wind 120 deg - Service	00.046	14.405	0,330	901.132	-1488.305	12.902
Dead+Wind 150 deg -	66.048	8.216	14.261	1520.597	-845.075	11.897
Service						

Load Combination	Vertical	Shear <sub>x</sub>	Shearz	Overturning Moment. Mx	Overturning Moment. M <sub>z</sub>	Torque
o o mamada m	K	K	K	kip-ft	kip-ft	kip-ft
Dead+Wind 180 deg - Service	66.048	-0.020	16.000	1711.029	13.996	4.524
Dead+Wind 210 deg - Service	66.048	-7.808	13.529	1456.934	833.405	-6.244
Dead+Wind 240 deg - Service	66.048	-14.183	8.215	891.171	1492.759	-8.204
Dead+Wind 270 deg - Service	66.048	-15.338	0.011	33.319	1641.978	-8.483
Dead+Wind 300 deg - Service	66.048	-13.621	-7.860	-796.167	1447.228	-12.975
Dead+Wind 330 deg - Service	66.048	-8.216	-14.251	-1453.973	868.835	-11.966

# **Solution Summary**

		n of Applied Force			ns		
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0.000	-66.048	0.000	0.000	66.048	0.000	0.000%
2	0.067	-79.258	-62.284	-0.067	79.258	62.284	0.000%
3	0.067	-59.444	-62.284	-0.067	59.444	62.284	0.000%
4	28.642	-79.258	-49.665	-28.642	79.258	49.665	0.000%
5	28.642	-59.444	-49.665	-28.642	59.444	49.665	0.000%
6	49.147	-79.258	-28.458	-49 147	79.258	28.458	0.000%
7	49.147	-59.444	-28.458	-49.147	59.444	28.458	0.000%
8	56.273	-79.258	-0.030	-56.273	79.258	0.030	0.000%
9	56.273	-59.444	-0.030	-56.273	59.444	0.030	0.000%
10	53.052	-79.258	30.679	-53.052	79.258	-30.679	0.000%
11	53.052	-59.444	30.679	-53.052	59.444	-30.679	0.000%
12	30.248	-79.258	52.506	-30.248	79.258	-52.506	0.000%
13	30.248	-59.444	52.506	-30.248	59.444	-52.506	0.000%
14	-0.075	-79.258	58.854	0.075	79.258	-58.854	0.000%
15	-0.075	-59.444	58.854	0.075	59.444	-58.854	0.000%
16	-28.698	-79.258	49.724	28.698	79.258	-49.724	0.000%
17	-28.698	-59.444	49.724	28.698	59.444	-49.724	0.000%
18	-52.211	-79.258	30.244	52.211	79.258	-30.244	0.000%
19	-52.211	-59.444	30.244	52.211	59.444	-30.244	0.000%
20	-56,338	-79.258	0.041	56.338	79.258	-0.041	0.000%
21	-56.338	-59.444	0.041	56.338	59.444	-0.041	0.000%
22	-50.074	-79.258	-28.896	50.074	79.258	28.896	0.000%
23	-50.074	-59.444	-28.896	50.074	59.444	28.896	0.000%
24	-30.248	-79.258	-52.468	30.248	79.258	52.468	0.000%
25	-30.248	-59.444	-52.468	30.248	59 <b>.</b> 444	52.468	0.000%
26	0.000	-175.612	0.000	0.000	175,612	0.000	0.000%
27	0.018	-175.612	-18.676	-0.018	175.612	18.676	0.000%
28	8,750	-175.612	-15.161	-8.750	175,612	15,161	0.000%
29	14,621	-175,612	-8.459	14.621	175,612	8,459	0.000%
30	17.316	-175.612	-0.010	17.316	175.612	0.010	0.000%
31	16.012	-175.612	9.251	-16.012	175.612	-9,251	0.000%
32	9.274	-175.612 -175.612	16.088	-9.274	175.612	-16.088	0.000%
33	-0.019	-175.612 -175.612	18.149	0.019	175.612	-18.149	0.000%
34	-8.762	-175.612	15.173	8.762	175.612	-15.173	0.000%
3 <del>4</del> 35	-6.762 -15.098	-175.612 -175.612	8.738	15.098	175.612	-13.173 -8.738	0.000%
36	-17.330	-175.612	0.012	17.330	175.612	-0.012	0.000%
37	-15.554	-175.612	-8.973	15.554	175.612	8.973	0.000%
38	-9.275	-175.612	-16.080	9.275	175.612	16.080	0.000%
39	0.018	-66.048	-16.903	-0.018	66.048	16.903	0.000%
40	7.793	-66.048	-13.513	-7.793	66.048	13.513	0.000%
41	13.377	-66.048	-7.745	-13.377	66.048	7.745	0.000%
42	15.321	-66.048	-0.008	-15.321	66.048	0.008	0.000%
43	14.405	-66.048	8.330	-14.405	66.048	-8.330	0.000%
44	8.216	-66.048	14.261	-8.216	66.048	-14.261	0.000%
45	-0.020	-66.048	16.000	0.020	66.048	-16.000	0.000%
46	-7.808	-66.048	13.529	7.808	66.048	-13.529	0.000%
47	-14.183	-66.048	8.215	14.183	66.048	-8.215	0.000%
48	-15.338	-66.048	0.011	15.338	66.048	-0.011	0.000%

	Sum of Applied Forces				Sum of Reactions			
Load	PX	PY	PZ	PX	PY	PZ	% Error	
Comb.	K	K	K	K	K	K		
49	-13.621	-66.048	-7.860	13.621	66.048	7.860	0.000%	
50	-8.216	-66.048	-14.251	8.216	66.048	14.251	0.000%	

### **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
T1	180 - 160	3.117	43	0.137	0.011
T2	160 - 153.333	2.537	43	0.133	0.011
T3	153.333 -	2.347	43	0.130	0.012
	146.667				
T4	146.667 - 140	2.162	43	0.126	0.012
T5	140 - 120	1.982	43	0.121	0.012
T6	120 - 100	1.473	43	0.107	0.011
T7	100 - 80	1.029	43	0.088	0.009
T8	80 - 70	0.670	43	0.070	0.007
<b>T</b> 9	70 - 60	0.517	43	0.061	0.006
T10	60 - 40	0.390	43	0.052	0.005
T11	40 - 20	0.182	43	0.032	0.003
T12	20 - 0	0.051	39	0.017	0.001

### Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	Curvature ft
181'	MX08FRO665-21 w/ Mount Pipe	43	3.117	0.137	0.011	Inf
170'	AIR 32 B2A/B66AA w/ Mount	43	2.826	0.135	0.011	508150
	Pipe					
161'	HPA-65R-BUU-H6	43	2,566	0.133	0.011	244090
157'	VHLP2-18	43	2.451	0.132	0.012	140917
148'	APXVTM14-ALU-I20 w/ Mount	43	2.199	0.127	0.012	130926
	Pipe					
138'	VHLP2-23	43	1.928	0.120	0.012	96681
136'	LLPX310R w/ Mount Pipe	43	1.875	0.118	0.012	93511
126'	(2) JAHH-65C-R3B w/ Mount	43	1.620	0.111	0.011	82881
	Pipe					
112'	800 10504 w/ Mount Pipe	43	1.286	0.099	0.010	63615
5'	GPS_A	39	0.009	0.004	0.000	187913

# **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	٥
T1	180 - 160	11.678	3	0.511	0.041
T2	160 - 153.333	9.505	3	0.497	0.043
Т3	153.333 -	8.794	3	0.486	0.045
	146.667				
T4	146.667 - 140	8.101	3	0.472	0.046
T5	140 - 120	7.423	3	0.454	0.046
T6	120 - 100	5.515	3	0.399	0.041
T7	100 - 80	3.853	3	0.329	0.034
T8	80 - 70	2.505	3	0.260	0.027
Т9	70 - 60	1.935	3	0.228	0.022
T10	60 - 40	1.461	3	0.193	0.020
T11	40 - 20	0.682	3	0.121	0.012
T12	20 - 0	0.189	3	0.062	0.004

### Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
181'	MX08FRO665-21 w/ Mount Pipe	3	11.678	0.511	0.041	284531
170'	AIR 32 B2A/B66AA w/ Mount	3	10.587	0.506	0.042	142265
	Pipe					
161'	HPA-65R-BUU-H6	3	9.613	0.498	0.043	67736
157'	VHLP2-18	3	9.183	0.492	0.044	38094
148'	APXVTM14-ALU-I20 w/ Mount	3	8.238	0.475	0.046	35262
	Pipe					
138'	VHLP2-23	3	7.223	0.448	0.046	25619
136'	LLPX310R w/ Mount Pipe	3	7.025	0.443	0.046	24759
126'	(2) JAHH-65C-R3B w/ Mount	3	6.065	0.416	0.043	21896
	Pipe					
112'	800 10504 w/ Mount Pipe	3	4.815	0.373	0.039	16844
5'	GPS A	3	0.032	0.016	0.001	50252

### **Bolt Design Data**

Section No.	Elevation	Component Type	Bolt Grade	Bolt Size	Number Of	Maximum Load	Allowable Load	Ratio Load	Allowable Ratio	Criteria
740.	ft	1 3 00	Orado	in	Bolts	per Bolt K	per Bolt K	Allowable	-	
T1	180	Leg	A325N	0.875	4	2.460	41.556	0.059	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	3.105	8.135	0.382	1.05	Member Block Shear
		Top Girt	A325N	0.625	1	0.426	5.423	0.079	1.05	Member Block Shear
		Mid Girt	A325N	0.625	1	0.483	5.423	0.089	1.05	Member Block Shear
T2	160	Diagonal	A325N	0.625	1	4.808	11.310	0.425	1.05	Member Bearing
Т3	153.333	Diagonal	A325N	0.625	1	4.943	11.310	0.437	1.05	Member Bearing
		Top Girt	A325N	0.625	1	0.886	5.655	0.157	1.05	Member Bearing
T4	146,667	Leg	A325N	1.000	4	8.494	54.517	0.156	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	5.795	11.310	0.512	1.05	Member Bearing
		Top Girt	A325N	0.625	1	0.812	5.655	0.144	1.05	Member Bearing
T5	140	Leg	A325N	1.000	6	11.007	54.517	0.202	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	7.772	11.310	0.687	1.05	Member Bearing
T6	120	Leg	A325N	1.000	6	17.165	54.517	0.315	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	9.089	12.675	0.717	1.05	Member Bearing
<b>T</b> 7	100	Leg	A325N	1.000	8	16.925	54.517	0.310	1.05	Bolt Tension
		Diagonal	A325N	0.750	1	9.996	14.137	0.707	1.05	Member Bearing
Т8	80	Diagonal	A325N	0.750	1	10.283	14.137	0.727	1.05	Member Bearing
T9	70	Leg	A325N	1.000	8	21.114	54.517	0.387	1.05	Bolt Tension
		Diagonal	A325N	0.750	1	10.966	20.227	0.542	1.05	Gusset Bearing
T10	60	Leg	A325N	1.000	8	25.202	54.517	0.462	1.05	Bolt Tension
		Diagonal	A325N	0.750	1	11.253	14.137	0.796	1.05	Member Bearing
T11	40	Leg	A325N	1.000	8	29.146	54.517	0.535	1.05	<b>Bolt Tension</b>
		Diagonal	A325N	0.750	1	11.839	17.672	0.670	1.05	Member Bearing
T12	20	Diagonal	A325N	0.750	1	12.809	20.227	0.633	1.05	Gusset Bearing

### **Compression Checks**

## Leg Design Data (Compression)

Section No.	Elevation	Size	L	Lu	KI/r	Α	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in <sup>2</sup>	K	Κ	
T1	180 - 160	ROHN 3 EH	20'7/16'	5'3/32"	52.9 K=1.00	3.016	-17.157	110.608	0.155 <sup>1</sup>
T2	160 - 153.333	ROHN 4 EH	6'8- 5/32''	6'8- 5/32"	54.3 K=1.00	4.407	-24.613	159.906	0.154 <sup>1</sup>
Т3	153.333 - 146.667	ROHN 4 EH	6'8- 5/32''	6'8- 5/32"	54.3 K=1.00	4.407	-35.886	159.905	0.224 1
T4	146.667 - 140	ROHN 4 EH	6'8- 5/32''	6'8- 5/32"	54.3 K=1.00	4.407	-46.791	159.906	0.293 <sup>1</sup>
<b>T</b> 5	140 - 120	ROHN 5 EH	20'7/16'	6'8- 5/32"	43.6 K=1.00	6.112	-85.930	239.378	0.359 <sup>1</sup>
Т6	120 - 100	ROHN 6 EHS	20'3/8"	6'8-1/8"	36.0 K=1.00	6.713	-128.015	274.776	0.466 <sup>1</sup>
T7	100 - 80	ROHN 6 EH	20'15/3 2"	10'7/32'	54.8 K=1.00	8.405	-164.542	303.717	0.542 <sup>1</sup>
Т8	80 - 70	ROHN 8 EHS	10'7/32'	10'7/32'	41.2 K=1.00	9.719	-183.519	386.395	0.475 <sup>1</sup>
Т9	70 - 60	ROHN 8 EHS	10'7/32'	10'7/32'	41.2 K=1.00	9.719	-202.841	386.395	0.525 <sup>1</sup>
T10	60 - 40	ROHN 8 EHS	20'13/3 2"	10'7/32'	41.2 K=1.00	9.719	-241.080	386.397	0.624 <sup>1</sup>
T11	40 - 20	ROHN 8 EH	20'13/3 2"	10'7/32'	41.8 K=1.00	12.763	-278.864	505.555	0.552 <sup>1</sup>
T12	20 - 0	ROHN 8 EH	20'13/3 2"	10'7/32'	41.8 K=1.00	12.763	-316.499	505.555	0.626 <sup>1</sup>

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

## **Diagonal Design Data (Compression)**

Section No.	Elevation	Size	L	$L_u$	KI/r	Α	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in <sup>2</sup>	K	K	$\phi P_n$
T1	180 - 160	L2x2x3/16	9'10- 3/8"	4'9- 15/32"	145.8 K=1.00	0.715	-3.019	9.623	0.314 1
T2	160 - 153.333	L2 1/2x2 1/2x1/4	11'3- 7/16"	5'6"	134.4 K=1.00	1.190	-5.019	18.851	0.266 <sup>1</sup>
Т3	153.333 - 146.667	L2 1/2x2 1/2x1/4	11'10- 7/32"	5'9- 13/32"	141.4 K=1.00	1.190	-5.281	17.047	0.310 <sup>1</sup>
T4	146.667 - 140	L2 1/2x2 1/2x1/4	12'5- 5/32"	6'7/8"	148.4 K=1.00	1.190	-5.954	15.466	0.385 1
T5	140 - 120	L2 1/2x2 1/2x1/4	14'2- 3/4"	6'11- 3/32"	169.2 K=1.00	1.190	-7.793	11.895	0.655 <sup>1</sup>
Т6	120 - 100	L3x3x1/4	15'11- 7/8"	7'8- 29/32"	157.0 K=1.00	1.440	-9.061	16.730	0.542 <sup>1</sup>
T7	100 - 80	L3 1/2x3 1/2x1/4	19'3- 3/32"	9'5- 25/32"	164.0 K=1.00	1.690	-9.928	17.990	0.552 <sup>1</sup>
Т8	80 - 70	L3 1/2x3 1/2x1/4	20'1- 13/16"	9'9- 25/32"	169.7 K=1.00	1.690	-10.407	16.792	0.620 <sup>1</sup>
Т9	70 - 60	2L3 1/2x3 1/2x1/4x3/8	21'11/3 2"	10'3- 3/32"	189.4 K=1.00	3.380	-11.141	26.228	0.425 1
		2L 'a' > 58.773 in - 137							
T10	60 - 40	L4x4x1/4	22'9- 23/32"	11'1- 25/32"	168.3 K=1.00	1.940	-11.622	19.609	0.593 <sup>1</sup>
T11	40 - 20	L4x4x5/16	24'7- 1/2"	12'11/1 6"	182.9 K=1.00	2.400	-12.359	20.532	0.602 <sup>1</sup>
T12	20 - 0	2L4x4x5/16x3/8	26'5- 9/16"	12'11- 3/4"	211.6 K=1.00	4.800	-13.697	30.149	0.454 <sup>1</sup>
		2L 'a' > 74.511 in - 176	2. 10						

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

	Top Girt Design Data (Compression)										
Section No.	Elevation	Size	L	Lu	KI/r	Α	$P_u$	φPn	Ratio Pu		
	ft		ft	ft		in²	K	K	φ <i>P</i> <sub>n</sub>		
T1	180 - 160	L2x2x1/8	6'8-1/4"	6'1-3/4"	185.5 K=1.00	0.484	-0.482	4.028	0.120 <sup>1</sup>		
Т3	153.333 - 146.667	L2 1/2x2 1/2x1/8	9'5- 13/32"	8'9- 29/32"	212.2 K=1.00	0.609	-0.622	3.875	0.161 <sup>1</sup>		
T4	146.667 - 140	KL/R > 200 (C) - 46 L2 1/2x2 1/2x1/8	10'1- 23/32"	9'6- 7/32"	228.8 K=1.00	0.609	-0.812	3.331	0.244 <sup>1</sup>		
		KL/R > 200 (C) - 58	23/32	1/32	K-1.00						

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

Mid Girt Design Data (Compression)										
Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	φPn	Ratio Pu	
	ft		ft	ft		in²	K	K	$\frac{P_u}{\phi P_n}$	
T1	180 - 160	L2x2x1/8	7'8- 11/16"	7'2- 3/16"	216.8 K=1.00	0.484	-0.480	2.950	0.163 <sup>1</sup>	
		KL/R > 200 (C) - 9								

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

### **Tension Checks**

#### Leg Design Data (Tension) KI/r Section Elevation Size L Lu Α $P_u$ Ratio $\phi P_n$ No. $P_u$ $in^2$ ft ft ft Κ Κ $\phi P_n$ T1 180 - 160 **ROHN 3 EH** 20'7/16' 5'3/32" 52.9 3.016 9.841 135.717 0.073 1 T2 160 - 153.333 **ROHN 4 EH** 6'8-6'8-54.3 4.407 15.867 198.335 0.080 1 5/32" 5/32" T3 153.333 -**ROHN 4 EH** 6'8-6'8-54.3 4.407 25.052 198.335 0.126 1 146.667 5/32" 5/32" T4 146.667 - 140 **ROHN 4 EH** 6'8-6'8-54.3 4.407 33.977 198.335 0.171 1 5/32" 5/32" 140 - 120 **ROHN 5 EH** 66.042 275.039 0.240 1 T5 20'7/16' 6'8-43.6 6.112 5/32" **ROHN 6 EHS** 20'3/8" 36.0 102.991 302.097 0.341 1 T6 120 - 100 6'8-1/8" 6.713 10'7/32' T7 100 - 80 **ROHN 6 EH** 20'15/3 54.8 8.405 135.397 378.222 $0.358^{1}$ 9.719 T8 80 - 70 **ROHN 8 EHS** 10'7/32' 10'7/32' 152.246 437.369 0.348 1 41.2 T9 70 - 60 **ROHN 8 EHS** 10'7/32' 10'7/32' 41.2 9.719 168.910 437.369 0.386 1 T10 60 - 40 **ROHN 8 EHS** 20'13/3 10'7/32' 41.2 9.719 201.617 437.369 0.461 1 2" **ROHN 8 EH** 20'13/3 10'7/32' T11 40 - 20 41.8 12.763 233.167 574.322 0.406 1 2" **ROHN 8 EH** 20'13/3 10'7/32' 12.763 574.322 0.457 1 T12 20 - 0 41.8 262.575 2"

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

Diagonal	Design	Data	(Tension)
agoa.		_ ~~~	(

Section No.	Elevation	Size	L	$L_u$	KI/r	Α	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in <sup>2</sup>	K	K	$\overline{\phi P_n}$
T1	180 - 160	L2x2x3/16	9'10- 3/8"	4'9- 15/32"	95.6	0.431	3.105	18.739	0.166 <sup>1</sup>
T2	160 - 153.333	L2 1/2x2 1/2x1/4	11'3- 7/16"	5'6"	87.8	0.752	4.808	32.707	0.147 <sup>1</sup>
Т3	153.333 - 146.667	L2 1/2x2 1/2x1/4	11'10- 7/32"	5'9- 13/32"	92.2	0.752	4.943	32.707	0.151 <sup>1</sup>
T4	146.667 - 140	L2 1/2x2 1/2x1/4	12'5- 5/32"	6'7/8"	96.7	0.752	5.795	32.707	0.177 <sup>1</sup>
<b>T</b> 5	140 - 120	L2 1/2x2 1/2x1/4	14'2- 3/4"	6'11- 3/32"	110.0	0.752	7.772	32.707	0.238 <sup>1</sup>
Т6	120 - 100	L3x3x1/4	15'11- 7/8"	7'8- 29/32"	101.5	0.939	9.089	45.794	0.198 <sup>1</sup>
T7	100 - 80	L3 1/2x3 1/2x1/4	19'3- 3/32"	9'5- 25/32"	105.9	1.103	9.996	53.793	0.186 <sup>1</sup>
Т8	80 - 70	L3 1/2x3 1/2x1/4	20'1- 13/16"	9'9- 25/32"	109.6	1.103	10.283	53.793	0.191 <sup>1</sup>
Т9	70 - 60	2L3 1/2x3 1/2x1/4x3/8	21'11/3 2"	10'3- 3/32"	114.4	2.207	10.966	95.999	0.114 <sup>1</sup>
		2L 'a' > 58.773 in - 136							
T10	60 - 40	L4x4x1/4	22'9- 23/32"	11'1- 25/32"	108.3	1.291	11.253	62.933	0.179 <sup>1</sup>
T11	40 - 20	L4x4x5/16	23'8- 9/16"	11'7- 1/4"	113.6	1.595	11.839	77.752	0.152 <sup>1</sup>
T12	20 - 0	2L4x4x5/16x3/8	26'5- 9/16"	12'11- 3/4"	126.9	3.190	12.809	138.758	0.092 1
		2L 'a' > 74.511 in - 175							

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

<b>Top Girt Design D</b>	ata (Tension)
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Section No.	Elevation	Size	L	Lu	KI/r	Α	$P_u$	$\phi P_n$	Ratio Pu
	ft		ft	ft		in²	Κ	K	$\phi P_n$
T1	180 - 160	L2x2x1/8	6'8-1/4"	6'1-3/4"	122.6	0.293	0.426	12.744	0.033 <sup>1</sup>
Т3	153.333 - 146.667	L2 1/2x2 1/2x1/8	9'5- 13/32"	8'9- 29/32"	138.3	0.387	0.886	16.822	0.053 <sup>1</sup>
T4	146.667 - 140	L2 1/2x2 1/2x1/8	10'1- 23/32"	9'6- 7/32"	148.9	0.387	0.812	16.822	0.048 <sup>1</sup>

<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

Section No.	Elevation	Size	L	Lu	KI/r	Α	Pu	φPn	Ratio Pu
	ft		ft	ft		in²	K	Κ	$\overline{\phi P_n}$
T1	180 - 160	L2x2x1/8	7'8- 11/16"	7'2- 3/16"	142.4	0.293	0.483	12.744	0.038 1

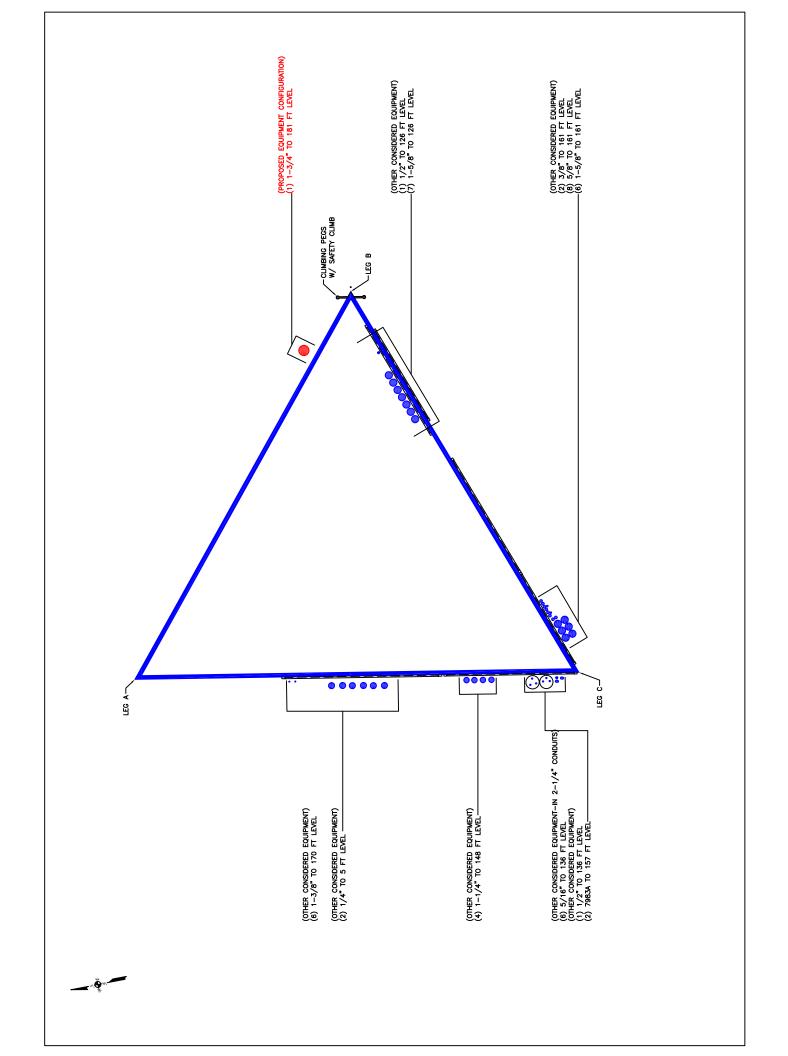
<sup>&</sup>lt;sup>1</sup>  $P_u$  /  $\phi P_n$  controls

Sect	tion (	Capac	ity Tak	ole

Section	Elevation	Component	Size	Critical	P	ø $P_{allow}$	%	Pass
No.	ft	Type		Element	K	K	Capacity	Fail

Section	Elevation	Component	Size	Critical	P	øP <sub>allow</sub>	%	Pass
No.	ft	Type		Element	K	K	Capacity	Fail
T1	180 - 160	Leg	ROHN 3 EH	2	-17.157	116.138	14.8	Pass
T2	160 - 153.333	Leg	ROHN 4 EH	35	-24.613	167.901	14.7	Pass
Т3	153.333 -	Leg	ROHN 4 EH	44	-35.886	167.900	21.4	Pass
	146.667							
T4	146.667 - 140	Leg	ROHN 4 EH	56	-46.791	167.901	27.9	Pass
T5	140 - 120	Leg	ROHN 5 EH	68	-85.930	251.347	34.2	Pass
Т6	120 - 100	Leg	ROHN 6 EHS	89	-128.015	288.515	44.4	Pass
T7	100 - 80	Leg	ROHN 6 EH	110	-164.542	318.903	51.6	Pass
Т8	80 - 70	Leg	ROHN 8 EHS	125	-183.519	405.715	45.2	Pass
Т9	70 - 60	Leg	ROHN 8 EHS	135	-202.841	405.715	50.0	Pass
T10	60 - 40	Leg	ROHN 8 EHS	144	-241.080	405.717	59.4	Pass
T11	40 - 20	Leg	ROHN 8 EH	159	-278.864	530.833	52.5	Pass
T12	20 - 0	Leg	ROHN 8 EH	174	316.499	530.833	59.6	Pass
T1	180 - 160	Diagonal	L2x2x3/16	13	-3.019	10.104	29.9	Pass
T2	160 - 153.333	Diagonal	L2 1/2x2 1/2x1/4	39	-5.019	19.793	25.4	Pass
Т3	153.333 - 146.667	Diagonal	L2 1/2x2 1/2x1/4	51	-5.281	17.900	29.5	Pass
T4	146.667 - 140	Diagonal	L2 1/2x2 1/2x1/4	63	-5.954	16.240	36.7	Pass
T5	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	70	-7.793	12.489	62.4	Pass
<b>T</b> 6	120 - 100	Diagonal	L3x3x1/4	91	-9.061	17.566	51.6	Pass
T7	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	113	-9.928	18.890	52.6	Pass
T8	80 - 70	Diagonal	L3 1/2x3 1/2x1/4	128	-10.407	17.632	59.0	Pass
T9	70 - 60	Diagonal	2L3 1/2x3 1/2x1/4x3/8	137	-11.141	27.539	40.5	Pass
T10	60 - 40	Diagonal	L4x4x1/4	146	-11.622	20.589	56.4	Pass
T11	40 - 20	Diagonal	L4x4x5/16	161	-12.359	21.559	57.3	Pass
T12	20 - 0	Diagonal	2L4x4x5/16x3/8	176	-13.697	31.656	43.3	Pass
T1	180 - 160	Top Girt	L2x2x1/8	4	-0.482	4.230	11.4	Pass
Т3	153.333 - 146.667	Top Girt	L2 1/2x2 1/2x1/8	46	-0.622	4.069	15.3	Pass
T4	146.667 - 140	Top Girt	L2 1/2x2 1/2x1/8	58	-0.812	3.498	23.2	Pass
T1	180 - 160	Mid Girt	L2x2x1/8	9	-0.480	3.097	15.5	Pass
							Summary	
						Leg (T12)	59.6	Pass
						Diagonal (T5)	62.4	Pass
						Top Girt (T4)	23.2	Pass
						Mid Girt (T1)	15.5	Pass
						Bolt Checks	75.8	Pass
						RATING =	75.8	Pass

# APPENDIX B BASE LEVEL DRAWING



# APPENDIX C ADDITIONAL CALCULATIONS

### **Self Support Anchor Rod Capacity**



Site Info		
	BU#	807133
	Site Name	BRG 134 943057
	Order#	548868, Rev. 1

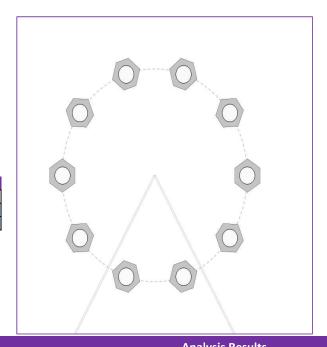
Analysis Considerations				
TIA-222 Revision	Н			
Grout Considered:	Yes			
I <sub>ar</sub> (in)	1.5			

Applied Loads						
	Comp.	Uplift				
Axial Force (kips)	325.96	269.80				
Shear Force (kips)	39.03	32.98				

<sup>\*</sup>TIA-222-H Section 15.5 Applied

Considered Eccentricity					
Leg Mod Eccentricity (in)	0.000				
Anchor Rod N.A Shift (in)	0.000				
Total Eccentricity (in)	0.000				

<sup>\*</sup>Anchor Rod Eccentricity Applied



_		_	
Conr	iection	Pro	perties

Connection Properties	A	Analysis Results			
Anchor Rod Data	Anchor Rod Summary		(units of kips, kip-in)		
(10) 1" ø bolts (A449 N; Fy=92 ksi, Fu=120 ksi)	Pu_t = 26.98	φPn_t = 54.54	Stress Rating		
I <sub>ar</sub> (in): 1.5	Vu = 3.3	φVn = 35.34	47.1%		
	Mu = n/a	φMn = n/a	Pass		

CCIplate - Version 4.1.1 Analysis Date: 6/7/2021 BU: 807133 WO: 1964193

Done By: MAA Checked By: Date: 6/7/2021



### Foundation Analysis - Rock Anchors

#### Applied Loads:

U := 270kip

C := 326kip

 $d_{pier} := 9 ft$   $b_{pier} := 6.25 ft$ 

 $L_{pier} := 9ft$ 

n := 4

 $W_{conc} := \gamma_c \cdot L_{pier} \cdot d_{pier} \cdot b_{pier} = 75.94 \cdot kip$ 

 $R_{u} := U - 0.9 \cdot W_{conc} = 201.66 \cdot kip$ 

 $R_c := C + 1.2 \cdot W_{conc} = 417.13 \cdot kip$ 

Uplift Force per Leg

Compression Force per Leg

Pier Dimensions

Pier Length

Number of Anchors

Pier Weight

Applied Uplift Force

Applied Compression Force

### Compression Analysis:

 $q_{ult} := 30ksf$ 

 $\phi := 0.75$ 

 $A_{bearing} := d_{pier} \cdot b_{pier} = 56.25 \text{ ft}^2$ 

 $R_{n\_bearing} := q_{ult} \cdot A_{bearing} = 1687.5 \cdot kip$ 

 $\phi R_{n \text{ bearing}} := \phi \cdot R_{n \text{ bearing}} = 1265.63 \cdot \text{kip}$ 

 $R_c = 417.13 \cdot kip$ 

Compression := ·  $- = 32.96 \cdot \%$ φR<sub>n bearing</sub>

**Ultimate Bearing Capacity** 

Bearing Strength Reduction Factor

Bearing Area

Nominal Bearing Capacity

**Bearing Capacity** 

Applied Compression Force

**Bearing Stress Rating** 

### Lateral Analysis:

 $\mu := 0.3$ 

 $\phi := 0.75$ 

 $R_v := 44 \text{kip}$ 

 $R_c = 417.13 \cdot kip$ 

 $R_s := R_c \cdot \mu = 125.14 \cdot \text{kip}$ 

 $\phi R_s := \phi \cdot R_s = 93.85 \cdot \text{kip}$ 

Sliding Friction Factor

Sliding Strength Reduction Factor

Compression Shear per Leg

Applied Compression Force per Leg

Nominal Lateral Resistance

Lateral Resistance

Lateral Stress Rating

BU: 807133 WO: 1964193 Done By: MAA Checked By: Date: 6/7/2021



#### **Uplift Analysis:**

#### 8.1.a Steel Anchor Nominal Tensile Strength:

$$F_u := 90ksi$$

$$A_{\text{net}} := 1.56 \text{in}^2$$

$$R_{11} = 201.66 \cdot kip$$

$$R_{n \text{ steel}} := F_{u} \cdot A_{net} = 140.4 \cdot \text{kip}$$

Nominal Steel Anchor Strength per Anchor

#### 8.1.b Steel-to-Grout Nominal Bonding Strength:

$$d_{\text{hole}} := 2.25 \text{in}$$

$$\theta := 0^{\circ}$$

$$f_c := 4000psi$$

$$A_s := \pi \cdot d_{hole} \cdot \left(\frac{L}{\cos(\theta)}\right) = 721 \cdot in^2$$

$$F_{s\_g} := 6 \cdot \sqrt{f_c} \cdot \sqrt{psi} = 379.47 \cdot psi$$

$$R_{n \text{ steel to grout}} := A_s \cdot F_{s \text{ g}} = 273.6 \cdot \text{kip}$$

Nominal Steel-to-Grout Bond Strength per Anchor

#### 8.1.c Grout-Rock Nominal Bonding Strength:

$$L_{rock} := 8.5 ft$$

$$F_{r\_g} := 110psi$$

$$A_b := \pi \cdot d_{hole} \cdot \left( \frac{L_{rock}}{cos(\theta)} \right) = 721 \cdot in^2$$

$$R_{n \text{ rock grout}} := F_{r \text{ g}} \cdot A_b = 79.31 \cdot \text{kip}$$

Nominal Grout-Rock Bond Strength per Anchor



#### 8.1.d Nominal Weight of Rock Prism

$$L_{eff} := 11.25 ft$$

$$d_{anchors} := 2.083 ft$$

$$\phi_{rock} := 40^{\circ}$$

$$\gamma_{\text{rock}} := 140 \text{pcf}$$

$$h_{soil} := 20ft$$

$$\phi_{\text{soil}} := 40^{\circ}$$

$$\gamma_{\text{soil}} := 135 \text{pcf}$$

$$d_1 := d_{anchors} = 2.083 \text{ ft}$$

$$d_2 := 2 \cdot L_{eff} \cdot tan(\phi_{rock}) + d_{anchors} = 20.96 \text{ ft}$$

$$d_3 := d_2 + 2 \cdot h_{soil} \cdot tan(\phi_{soil}) = 54.53 \text{ ft}$$

$$V_{rock} := \frac{\pi \cdot L_{eff}}{3} \cdot \left[ \left( \frac{d_2}{2} \right)^2 + \left( \frac{d_2}{2} \right) \left( \frac{d_1}{2} \right) + \left( \frac{d_1}{2} \right)^2 \right] = 1435.63 \cdot ft^3$$

$$V_{soil} := \frac{\pi \cdot h_{soil}}{3} \cdot \left[ \left( \frac{d_3}{2} \right)^2 + \left( \frac{d_3}{2} \right) \left( \frac{d_2}{2} \right) + \left( \frac{d_2}{2} \right)^2 \right] = 23853.22 \cdot ft^3$$

$$W_{rock} := \gamma_{rock} \cdot V_{rock} = 200.99 \cdot kip$$

$$W_{soil} := \gamma_{soil} \cdot V_{soil} = 3220.19 \cdot kip$$

$$R_{\text{n rock}} := W_{\text{rock}} + W_{\text{soil}} = 3421.17 \cdot \text{kip}$$

$$R_n := \min(R_{n \text{ steel}}, R_{n \text{ steel to grout}}, R_{n \text{ rock grout}}, R_{n \text{ rock}}) = 79.31 \cdot \text{kip}$$

$$\Phi R_n := \Phi \cdot R_n = 59.48 \cdot \text{kip}$$

$$P_{u} := \frac{R_{u}}{n} = 50.41 \cdot kip$$

Uplift := 
$$\frac{P_u}{\phi R_n}$$
 = 84.75·%



#### Address:

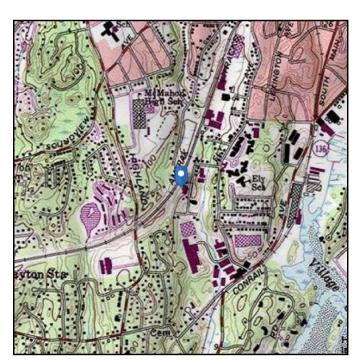
No Address at This Location

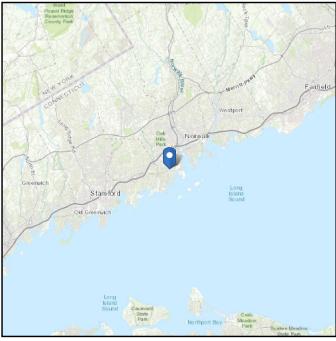
## **ASCE 7 Hazards Report**

Standard: ASCE/SEI 7-10 Elevation: 60.78 ft (NAVD 88)

Risk Category: || Latitude: 41.081789

Soil Class: D - Stiff Soil Longitude: -73.430422





#### Wind

#### Results:

Wind Speed: 120 Vmph
10-year MRI 76 Vmph
25-year MRI 86 Vmph
50-year MRI 92 Vmph
100-year MRI 98 Vmph

Date Somessed: ASCE/BET7202,1Fig. 26.5-1A and Figs. CC-1—CC-4, and Section 26.5.2, incorporating errata of March 12, 2014

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

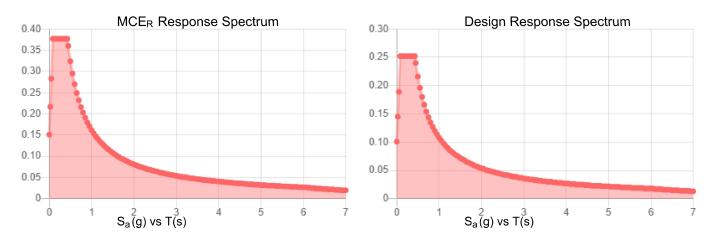
Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.



#### Seismic

Site Soil Class: Results:	D - Stiff Soil			
S <sub>s</sub> :	0.236	$S_{ t DS}$ :	0.252	
$S_1$ :	0.068	S <sub>D1</sub> :	0.108	
F <sub>a</sub> :	1.6	T <sub>L</sub> :	6	
F <sub>v</sub> :	2.4	PGA:	0.134	
S <sub>MS</sub> :	0.377	PGA <sub>M</sub> :	0.206	
S <sub>M1</sub> :	0.162	F <sub>PGA</sub> :	1.531	
		l <sub>o</sub> '	1	

#### Seismic Design Category B



Data Accessed: Thu Apr 15 2021

Date Source: USGS Seismic Design Maps based on ASCE/SEI 7-10, incorporating

Supplement 1 and errata of March 31, 2013, and ASCE/SEI 7-10 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with

ASCE/SEI 7-10 Ch. 21 are available from USGS.



#### lce

Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 15 F

Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Thu Apr 15 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

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## Exhibit E

**Mount Analysis** 

Date: July 27, 2021

Darcy Tarr Crown Castle 3530 Tornigdon Way, Suite 300 Charlotte, NC 28277 (704) 405-6589



Trylon 1825 W. Walnut Hill Lane, Suite 302 Irving, TX 75038 214-930-1730

Subject: Mount Replacement Analysis Report

Carrier Designation: Dish Network Dish 5G

Carrier Site Number: NJJER01090A Carrier Site Name: CT-CCI-T-807133

Crown Castle Designation: Crown Castle BU Number: 807133

Crown Castle Site Name: BRG 134 943057

Crown Castle JDE Job Number: 640174 Crown Castle Order Number: 548868 Rev. 1

Engineering Firm Designation: Trylon Report Designation: 188628

Site Data: 50 Rockland Road, Norwalk OFC – MTSO, So Norwalk, Fairfield County,

CT, 06854

Latitude 41°4'54.44" Longitude -73°25'49.52"

Structure Information: Tower Height & Type: 180.0 ft Self Support

Mount Elevation: 181.0 ft

Mount Type: 8.0 ft Sector Frame

Dear Darcy Tarr,

Trylon is pleased to submit this "Mount Replacement Analysis Report" to determine the structural integrity of Dish Network's antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

Sector Frame Sufficient\*
\*Sufficient upon completion of the changes listed in the 'Recommendations' section of this report.

This analysis utilizes an ultimate 3-second gust wind speed of 120 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Aura Baltoiu

Respectfully Submitted by: Cliff Abernathy, P.E.



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#### 7) APPENDIX C

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#### 8) APPENDIX D

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Supplemental Drawings

#### 1) INTRODUCTION

This is a proposed 3 sector 8.0 ft Sector Frame, designed by Commscope.

#### 2) ANALYSIS CRITERIA

**Building Code:** 2015 IBC TIA-222 Revision: TIA-222-H

Risk Category:

Ultimate Wind Speed: 120 mph

**Exposure Category:** Topographic Factor at Base: 1.00 **Topographic Factor at Mount:** 1.00 Ice Thickness: 1.5 in Wind Speed with Ice: 50 mph Seismic S<sub>s</sub>: 0.232 Seismic S<sub>1</sub>: 0.067 **Live Loading Wind Speed:** 30 mph Man Live Load at Mid/End-Points: 250 lb Man Live Load at Mount Pipes: 500 lb

**Table 1 - Proposed Equipment Configuration** 

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details
		3	JMA Wireless	MX08FRO665-21	8.0 ft Sector Frame
181.0	181.0	3	Fujitsu	TA08025-B604	[Commscope,
101.0		3	Fujitsu	TA08025-B605	• • •
		1	Raycap	RDIDC-9181-PF-48	MTC3975083]

#### 3) ANALYSIS PROCEDURE

Table 2 - Documents Provided

Table 2 Decaments Floridea								
Document	Remarks	Reference	Source					
Crown Application	Dish Network Application	548868, Rev.1	CCI Sites					
Mount Manufacturer Drawings	Commscope	MTC3975083	Trylon					
Exposure Category Determination	Crown Castle	6995378	CCI Sites					

#### 3.1) Analysis Method

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed, using Microsoft Excel, by Trylon was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision B).

#### 3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 5) Prior structural modifications to the tower mounting system are assumed to be installed as shown per available data.
- 6) Steel grades have been assumed as follows, unless noted otherwise:

Channel, Solid Round, Angle, Plate

HSS (Rectangular)

Pipe

ASTM A36 (GR 36)

ASTM A500 (GR B-46)

ASTM A53 (GR 35)

ASTM A325

This analysis may be affected if any assumptions are not valid or have been made in error. Trylon should be notified to determine the effect on the structural integrity of the antenna mounting system.

#### 4) ANALYSIS RESULTS

Table 3 - Mount Component Stresses vs. Capacity (Sector Frame, Worst Case Sector)

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
	Mount Pipe(s)	MP23		46.3	Pass
	Horizontal(s)	TH		45.8	Pass
	Standoff(s)	SA4		46.6	Pass
1,2	Bracing(s)	B3	181.0	40.6	Pass
1,2	Vertical(s)	V4		23.9	Pass
	Plate(s)	MP9		48.1	Pass
	Tieback(s)	MP25		4.6	Pass
	Mount Connection(s)	-		15.9	Pass

Structure Rating (max from all components) =	48.1%

Notes:

- 1) See additional documentation in "Appendix C Software Analysis Output" for calculations supporting the % capacity consumed.
- 2) Rating per TIA-222-H, Section 15.5

#### **Table 4 - Tieback Connection Data Table**

Tower Connection Node No.	Existing / Proposed	Resultant End Reaction (lb)	Connected Member Type	Connected Member Size	Member Compressive Capacity (lb) <sup>3</sup>	Notes
N86A	Proposed	814.44	Leg	ROHN 3 EH	5,530.4	1

Notes:

- 1) Tieback connection point is within 25% of either end of the connected tower member
- 2) Tieback connection point is NOT within 25% of either end of the connected tower member
- Reduced member compressive capacity according to CED-STD-10294 Standard for Installation of Mounts and Appurtenances

#### 4.1) Recommendations

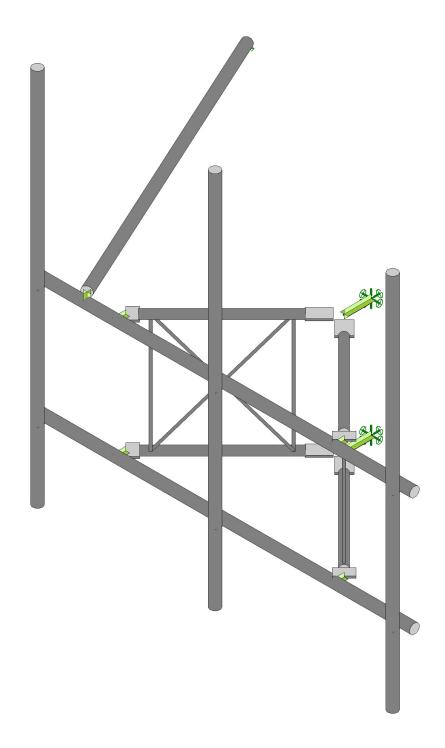
The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the proposed mount listed below must be installed.

1. Commscope, MTC3975083. Install the tiebacks at approximately 12" from the mount edge (from pipe mount position #3). The connection point needs to be within 25% ends of tower leg.

No structural modifications are required at this time, provided that the above-listed changes are implemented.

## APPENDIX A WIRE FRAME AND RENDERED MODELS

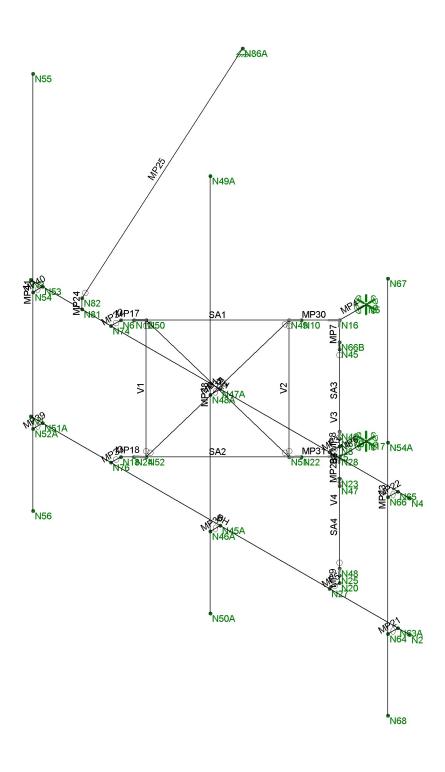




**Envelope Only Solution** 

Trylon		SK - 1
AB	807133	July 27, 2021 at 4:02 PM
188628		807133.r3d





#### **Envelope Only Solution**

Trylon		SK - 2
AB	807133	July 27, 2021 at 4:02 PM
188628		807133.r3d

## APPENDIX B SOFTWARE INPUT CALCULATIONS



#### Address:

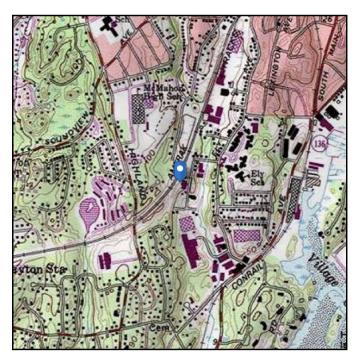
No Address at This Location

## **ASCE 7 Hazards Report**

Standard: ASCE/SEI 7-10 Elevation: 60.78 ft (NAVD 88)

Risk Category: || Latitude: 41.081789

Soil Class: D - Stiff Soil Longitude: -73.430422





#### Ice

#### Results:

Ice Thickness: 0.75 in.

Concurrent Temperature: 15 F

Gust Speed: 50 mph

**Data Source:** Standard ASCE/SEI 7-10, Figs. 10-2 through 10-8

Date Accessed: Mon Jul 26 2021

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 50-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.



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#### **TIA LOAD CALCULATOR 2.0**

PROJECT DATA		
Job Code:	188628	
Carrier Site ID:	NEW JERSEY	
Carrier Site Name:	•	

CODES AND STANDARDS			
Building Code:	2015 IBC		
Local Building Code:	2018 CSBC		
Design Standard:	TIA-222-H		

STRUCTURE DETAILS			
Mount Type:	Sector Frame		
Mount Elevation:	181.0	ft.	
Number of Sectors:	3		
Structure Type:	Self Support Tower		
Structure Height:	180.0	ft.	

ANALYSIS CRITERIA				
Structure Risk Category:	=			
Exposure Category:	С			
Site Class:	D - Stiff Soil			
Ground Elevation:	60.78	ft.		

TOPOGRAPHIC DATA		
Topographic Category:	1.00	
Topographic Feature:	N/A	
Crest Point Elevation:	0.00	ft.
Base Point Elevation:	0.00	ft.
Crest to Mid-Height (L/2):	0.00	ft.
Distance from Crest (x):	0.00	ft.
Base Topo Factor (K <sub>zt</sub> ):	1.00	
Mount Topo Factor (K <sub>zt</sub> ):	1.00	

WIND PARAMETERS			
Design Wind Speed:	120	mph	
Wind Escalation Factor (K <sub>s</sub> ):	1.00		
Velocity Coefficient (K <sub>z</sub> ):	1.43		
Directionality Factor (K <sub>d</sub> ):	0.95		
Gust Effect Factor (Gh):	1.00		
Shielding Factor (K <sub>a</sub> ):	0.90		
Velocity Pressure (qz):	50.11	psf	

ICE PARAMETERS				
Design Ice Wind Speed:	50	mph		
Design Ice Thickness (t <sub>i</sub> ):	1.50	in		
Importance Factor (I <sub>i</sub> ):	1.00			
Ice Velocity Pressure (qzi):	50.11	psf		
Mount Ice Thickness (t <sub>iz</sub> ):	1.78	in		

WIND STRUCTURE CALCULATIONS		
Flat Member Pressure:	90.20	psf
Round Member Pressure:	54.12	psf
Ice Wind Pressure:	7.77	psf

SEISMIC PARAMETERS		
Importance Factor (I <sub>e</sub> ):	1.00	
Short Period Accel .(S <sub>s</sub> ):	0.232	g
1 Second Accel (S <sub>1</sub> ):	0.067	g
Short Period Des. $(S_{DS})$ :	0.25	g
1 Second Des. (S <sub>D1</sub> ):	0.11	g
Short Period Coeff. (F <sub>a</sub> ):	1.60	
1 Second Coeff. (F <sub>v</sub> ):	2.40	
Response Coefficient (Cs):	0.12	
Amplification Factor (A <sub>S</sub> ):	1.20	-

## **LOAD COMBINATIONS [LRFD]**

#	Description
1	1.4DL
2	1.2DL + 1WL 0 AZI
3	1.2DL + 1WL 30 AZI
4	1.2DL + 1WL 45 AZI
5	1.2DL + 1WL 60 AZI
6	1.2DL + 1WL 90 AZI
7	1.2DL + 1WL 120 AZI
8	1.2DL + 1WL 135 AZI
9	1.2DL + 1WL 150 AZI
10	1.2DL + 1WL 180 AZI
11	1.2DL + 1WL 210 AZI
12	1.2DL + 1WL 225 AZI
13	1.2DL + 1WL 240 AZI
14	1.2DL + 1WL 270 AZI
15	1.2DL + 1WL 300 AZI
16	1.2DL + 1WL 315 AZI
17	1.2DL + 1WL 330 AZI
18	0.9DL + 1WL 0 AZI
19	0.9DL + 1WL 30 AZI
20	0.9DL + 1WL 45 AZI
21	0.9DL + 1WL 60 AZI
22	0.9DL + 1WL 90 AZI
23	0.9DL + 1WL 120 AZI
24	0.9DL + 1WL 135 AZI
25	0.9DL + 1WL 150 AZI
26	0.9DL + 1WL 180 AZI
27	0.9DL + 1WL 210 AZI
28	0.9DL + 1WL 225 AZI
29	0.9DL + 1WL 240 AZI
30	0.9DL + 1WL 270 AZI
31	0.9DL + 1WL 300 AZI
32	0.9DL + 1WL 315 AZI
33	0.9DL + 1WL 330 AZI
34	1.2DL + 1DLi + 1WLi 0 AZI
35	1.2DL + 1DLi + 1WLi 30 AZI
36	1.2DL + 1DLi + 1WLi 45 AZI
37	1.2DL + 1DLi + 1WLi 60 AZI
38	1.2DL + 1DLi + 1WLi 90 AZI
39	1.2DL + 1DLi + 1WLi 120 AZI
40	1.2DL + 1DLi + 1WLi 135 AZI
41	1.2DL + 1DLi + 1WLi 150 AZI

#	Description
42	1.2DL + 1DLi + 1WLi 180 AZI
43	1.2DL + 1DLi + 1WLi 210 AZI
44	1.2DL + 1DLi + 1WLi 225 AZI
45	1.2DL + 1DLi + 1WLi 240 AZI
46	1.2DL + 1DLi + 1WLi 270 AZI
47	1.2DL + 1DLi + 1WLi 300 AZI
48	1.2DL + 1DLi + 1WLi 315 AZI
49	1.2DL + 1DLi + 1WLi 330 AZI
50	(1.2+0.2Sds) + 1.0E 0 AZI
51	(1.2+0.2Sds) + 1.0E 30 AZI
52	(1.2+0.2Sds) + 1.0E 45 AZI
53	(1.2+0.2Sds) + 1.0E 60 AZI
54	(1.2+0.2Sds) + 1.0E 90 AZI
55	(1.2+0.2Sds) + 1.0E 120 AZI
56	(1.2+0.2Sds) + 1.0E 135 AZI
57	(1.2+0.2Sds) + 1.0E 150 AZI
58	(1.2+0.2Sds) + 1.0E 180 AZI
59	(1.2+0.2Sds) + 1.0E 210 AZI
60	(1.2+0.2Sds) + 1.0E 225 AZI
61	(1.2+0.2Sds) + 1.0E 240 AZI
62	(1.2+0.2Sds) + 1.0E 270 AZI
63	(1.2+0.2Sds) + 1.0E 300 AZI
64	(1.2+0.2Sds) + 1.0E 315 AZI
65	(1.2+0.2Sds) + 1.0E 330 AZI
66	(0.9-0.2Sds) + 1.0E 0 AZI
67	(0.9-0.2Sds) + 1.0E 30 AZI
68	(0.9-0.2Sds) + 1.0E 45 AZI
69	(0.9-0.2Sds) + 1.0E 60 AZI
70	(0.9-0.2Sds) + 1.0E 90 AZI
71	(0.9-0.2Sds) + 1.0E 120 AZI
72	(0.9-0.2Sds) + 1.0E 135 AZI
73	(0.9-0.2Sds) + 1.0E 150 AZI
74	(0.9-0.2Sds) + 1.0E 180 AZI
75	(0.9-0.2Sds) + 1.0E 210 AZI
76	(0.9-0.2Sds) + 1.0E 225 AZI
77	(0.9-0.2Sds) + 1.0E 240 AZI
78	(0.9-0.2Sds) + 1.0E 270 AZI
79	(0.9-0.2Sds) + 1.0E 300 AZI
80	(0.9-0.2Sds) + 1.0E 315 AZI
81	(0.9-0.2Sds) + 1.0E 330 AZI
82-88	,
02-00	1.∠U + 1.3 LV1

#	Description
89	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP1
90	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP1
91	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP1
92	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP1
93	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP1
94	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP1
95	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP1
96	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP1
97	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP1
98	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP1
99	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP1
100	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP1
101	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP1
102	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP1
103	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP1
104	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP1
105	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP2
106	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP2
107	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP2
108	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP2
109	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP2
110	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP2
111	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP2
112	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP2
113	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP2
114	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP2
115	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP2
116	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP2
117	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP2
118	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP2
119	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP2
120	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP2

#	Description
121	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP3
122	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP3
123	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP3
124	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP3
125	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP3
126	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP3
127	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP3
128	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP3
129	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP3
130	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP3
131	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP3
132	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP3
133	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP3
134	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP3
135	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP3
136	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP3
137	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP4
138	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP4
139	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP4
140	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP4
141	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP4
142	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP4
143	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP4
144	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP4
145	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP4
146	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP4
147	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP4
148	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP4
149	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP4
150	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP4
151	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP4
152	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP4

<sup>\*</sup>This page shows an example of maintenance loads for (4) pipes, the number of mount pipe LCs may vary per site

## **EQUIPMENT LOADING**

Appurtenance Name/Location	Qty.	Elevation [ft]		EPA <sub>N</sub> (ft2)	EPA <sub>T</sub> (ft2)	Weight (lbs)
MX08FRO665-21	1	181	No Ice	8.01	3.21	82.50
MP23, 0			w/ Ice	9.62	4.62	293.26
TA08025-B604	1	181	No Ice	1.96	0.98	63.90
MP23, 0			w/ Ice	2.40	1.32	72.29
TA08025-B605	1	181	No Ice	1.96	1.13	75.00
MP23, 0			w/ Ice	2.40	1.48	76.99
RDIDC-9181-PF-48	1	181	No Ice	2.01	1.17	21.85
MP23, 0	-		w/ Ice	2.46	1.54	75.88
			No Ice			
<del></del>			w/ Ice			
			No Ice			
<del></del>			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
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			No Ice			
			w/ Ice			
			No Ice			
	-		w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			

## **EQUIPMENT LOADING [CONT.]**

Appurtenance Name/Location	Qty.	Elevation [ft]		EPA <sub>N</sub> (ft2)	EPA <sub>T</sub> (ft2)	Weight (lbs)
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			
			No Ice			
			w/ Ice			

## **EQUIPMENT WIND CALCULATIONS**

Appurtenance Name	Qty.	Elevation [ft]	<b>K</b> <sub>zt</sub>	Kz	K <sub>d</sub>	<b>t</b> <sub>d</sub>	<b>q</b> <sub>z</sub> [psf]	<b>q</b> <sub>zi</sub> [psf]
MX08FRO665-21	1	181	1.00	1.43	0.95	1.78	50.11	8.70
TA08025-B604	1	181	1.00	1.43	0.95	1.78	50.11	8.70
TA08025-B605	1	181	1.00	1.43	0.95	1.78	50.11	8.70
RDIDC-9181-PF-48	1	181	1.00	1.43	0.95	1.78	50.11	8.70

## **EQUIPMENT LATERAL WIND FORCE CALCULATIONS**

Appurtenance Name	Qty.		0° 180°	30° 210°	60° 240°	90° 270°	120° 300°	150° 330°
MX08FRO665-21	1	No Ice	361.24	198.89	307.12	144.77	307.12	198.89
MP23, 0		w/ Ice	75.33	45.98	65.55	36.20	65.55	45.98
TA08025-B604	1	No Ice	88.55	55.32	77.48	44.25	77.48	55.32
MP23, 0		w/ Ice	18.80	12.46	16.68	10.34	16.68	12.46
TA08025-B605	1	No Ice	88.55	60.34	79.15	50.94	79.15	60.34
MP23, 0		w/ Ice	18.80	13.41	17.00	11.62	17.00	13.41
RDIDC-9181-PF-48	1	No Ice	90.73	62.20	81.22	52.68	81.22	62.20
MP23, 0		w/ Ice	19.22	13.83	17.43	12.03	17.43	13.83
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
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		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		W/ ICE						

## **EQUIPMENT LATERAL WIND FORCE CALCULATIONS [CONT.]**

Appurtenance Name	Qty.		0° 180°	30° 210°	60° 240°	90° 270°	120° 300°	150° 330°
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
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		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						
		No Ice						
		w/ Ice						

## **EQUIPMENT SEISMIC FORCE CALCULATIONS**

Appurtenance Name	Qty.	Elevation [ft]	Weight [lbs]	<b>F</b> <sub>p</sub> [lbs]
MX08FRO665-21	1	181	82.5	12.25
TA08025-B604	1	181	63.9	9.49
TA08025-B605	1	181	75	11.14
RDIDC-9181-PF-48	1	181	21.85	3.24

## APPENDIX C SOFTWARE ANALYSIS OUTPUT



Company : 7
Designer : 7
Job Number : 8
Model Name : 8

: Trylon : AB : 188628 : 807133

### (Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include W arping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P -Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec ^2)	386.4
Wall Mesh Size (in)	24
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Z
Global Member Orientation Plane	XY
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
R ISAC onnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AIS I S 100-16: LRFD
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	AISC 14th(360-10): LRFD
Adjust Stiffness?	Yes(Iterative)

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parme Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR_SET_ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8

Company Designer Job Number

: Trylon : AB : 188628 : 807133

## (Global) Model Settings, Continued

Seismic Code	ASCE 7-16
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
CtX	.02
CtZ	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
RX	3
RZ	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	l or II
Drift Cat	O ther
O m Z	1
OmX	1
C d Z	1
CdX	1
R ho Z	1
R ho X	1

## **Hot Rolled Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E5F)	Density[k/ft^3]	Y ield[ks i]	Ry	Fu[ksi]	Rt
1	A992	29000	11154	.3	.65	.49	50	1.1	65	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
3	A572 G r.50	29000	11154	.3	.65	.49	50	1.1	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42	1.4	58	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46	1.4	58	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2
7	A1085	29000	11154	.3	.65	.49	50	1.4	65	1.3

### **Cold Formed Steel Properties**

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E5F)	Density[k/ft^3]	Y ield[ks i]	Fu[ksi]
1	A653 S S G r33	29500	11346	.3	.65	.49	33	45
2	A653 S S G r50/1	29500	11346	.3	.65	.49	50	65

## Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design	A [in2]	lyy [in4]	lzz [in4]	J [in4]
1	PIPE_2.0	PIPE_2.0	Beam	Pipe	A53 Gr.B	Typical	1.02	.627	.627	1.25
2	PIPE_1.5	PIPE_1.5	Beam	Pipe	A53 Gr.B	Typical	.749	.293	.293	.586
3	SR 5/8	SR 5/8	Beam	BAR	A36 Gr.36	Typical	.307	.007	.007	.015
4	3.5x0.5	3.5x0.5	Beam	RECT	A36 Gr.36	Typical	1.75	.036	1.786	.133
5	4.25x0.5	4.25x0.5	Beam	RECT	A36 Gr.36	Typical	2.125	.044	3.199	.164
6	SR 1/2"	SR 1/2"	Beam	BAR	A36 Gr.36	Typical	.196	.003	.003	.006

Company Designer Job Number

: Trylon : AB : 188628 : 807133

### **Cold Formed Steel Section Sets**

	Label	Shape	Type	Design List	Material	Design R	A [in2]	lyy [in4]	lzz [in4]	J [in4]	
1	CF1	162T125-18	Beam	None	A653 S S G r33	Typical	.078	.013	.042	9e-6	

## Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	N5	Reaction	Reaction	Reaction	Reaction	Reaction	
2	N17	Reaction	Reaction	Reaction	Reaction	Reaction	
3	N86A	Reaction	Reaction	Reaction			

## **Basic Load Cases**

	BLC Description	Category	X Gravity	Y Gravity	Z G ravity	Joint	Point	Distribu	.A rea (M	Surface
1	Self Weight	DL			-1		5			
2	Structure Wind X	WLX						26		
3	Structure Wind Y	WLY						26		
4	Wind Load 0 AZI	WLX					5			
5	Wind Load 30 AZI	None					10			
6	Wind Load 45 AZI	None					10			
7	Wind Load 60 AZI	None					10			
8	Wind Load 90 AZI	WLY					5			
9	Wind Load 120 AZI	None					10			
10	Wind Load 135 AZI	None					10			
11	Wind Load 150 AZI	None					10			
12	Ice Weight	OL1					5	26		
13	Structure Ice Wind X	OL2						26		
14	Structure Ice Wind Y	OL3						26		
15	Ice Wind Load 0 AZI	OL2					5			
16	Ice Wind Load 30 AZI	None					10			
17	Ice Wind Load 45 AZI	None					10			
18	Ice Wind Load 60 AZI	None					10			
19	Ice Wind Load 90 AZI	OL3					5			
20	Ice Wind Load 120 AZI	None					10			
21	Ice Wind Load 135 AZI	None					10			
22	Ice Wind Load 150 AZI	None					10			
23	Seismic Load X	ELX	148				5			
24	Seismic Load Y	ELY		148			5			
25	Live Load 1 (Lv)	LL					1			
26	Live Load 2 (Lv)	LL					1			
27	Maintenance Load 1 (Lm)	None					1			
28	Maintenance Load 2 (Lm)	None					1			
29	Maintenance Load 3 (Lm)	None					1			
33		ELX								
34		ELY								
35		LL								
36		LL								
41		LL								

Company Designer Job Number Model Name

: Trylon : AB : 188628 : 807133

### Load Combinations

	Des cription	Solve	PD	.SRB	Factor	BLC	Factor	В	.Fa	B	.Fa	.BLC	Fa	В	Fa	В	Fa	.В	.Fa	В	Fa	В	 Fa
1	1.4DL	Yes	Υ	DL	1.4																		
2	1.2DL + 1WL 0 AZI	Yes	Υ	DL	1.2	2	1	3		4	1												
3	1.2DL + 1WL 30 AZI	Yes	Υ	DL	1.2	2	.866	3	.5	5	1												
4	1.2DL + 1WL 45 AZI	Yes	Υ	DL	1.2	2	.707	3	.707	6	1												
5	1.2DL + 1WL 60 AZI	Yes	Υ	DL	1.2	2	.5	3	.866	7	1												
6	1.2DL + 1WL 90 AZI	Yes	Υ	DL	1.2	2		3	1	8	1												
7	1.2DL + 1WL 120 AZI	Yes	Υ	DL	1.2	2	5	3	.866	9	1												
8	1.2DL + 1WL 135 AZI	Yes	Υ	DL	1.2	2	707	3	.707	10	1												
9	1.2DL + 1WL 150 AZI	Yes	Υ	DL	1.2	2	866	3	.5	11	1												
10	1.2DL + 1WL 180 AZI	Yes	Υ	DL	1.2	2	-1	3		4	-1												
11	1.2DL + 1WL 210 AZI	Yes	Υ	DL	1.2	2	866	3	5	5	-1												
12	1.2DL + 1WL 225 AZI	Yes	Υ	DL	1.2	2	707	3	7.		-1												
13	1.2DL + 1WL 240 AZI	Yes	Υ	DL	1.2	2	5	3	8.		-1												
14	1.2DL + 1WL 270 AZI	Yes	Υ	DL	1.2	2		3	-1		-1												
15	1.2DL + 1WL 300 AZI	Yes	Υ	DL	1.2	2	.5	_	8.	_	-1												
16	1.2DL + 1WL 315 AZI	Yes	Υ	DL	1.2	2	.707	_	7.	_	_												
17	1.2DL + 1WL 330 AZI	Yes	Υ	DL	1.2	2	.866	3	5														
18	0.9DL + 1WL 0 AZI	Yes	Υ	DL	.9	2	1	3		4	1												
19	0.9DL + 1WL 30 AZI	Yes	Υ	DL	.9	2	.866	3			1												
20	0.9DL + 1WL 45 AZI	Yes	Υ	DL	.9	2	.707		.707		1												
21	0.9DL + 1WL 60 AZI	Yes	Υ	DL	.9	2	.5		.866	_	1												
22	0.9DL + 1WL 90 AZI	Yes	Υ	DL	.9	2		3	1	8	1												
23	0.9DL + 1WL 120 AZI	Yes	Υ	DL	.9	2	5		.866		1												
24	0.9DL + 1WL 135 AZI	Yes	Υ	DL	.9	2	707		.707														
25	0.9DL + 1WL 150 AZI	Yes	Υ	DL	.9	2	866	3	.5	11													
26	0.9DL + 1WL 180 AZI	Yes	Y	DL	.9	2	<u>-1</u>	3		4	-1												
27	0.9DL + 1WL 210 AZI	Yes	Υ	DL	.9	2	866		5		-1												
28	0.9DL + 1WL 225 AZI	Yes	Υ	DL	.9	2	707	_	7.		-1												
29	0.9DL + 1WL 240 AZI	Yes	Υ	DL	.9	2	5	_	8.	_	-1												
30	0.9DL + 1WL 270 AZI	Yes	Υ	DL	.9	2		3	-1		-1												
	0.9DL + 1WL 300 AZI	Yes	Υ	DL	.9	2	.5	_	8.	_	-1											_	_
32	0.9DL + 1WL 315 AZI	Yes	Υ	DL	.9	2	.707	_	7.	_	_												
	0.9DL + 1WL 330 AZI	Yes	Y	DL	.9	2	.866	3	_														
34	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1	13	_	14		15	1										
35	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1	_	.866	_	_	16	1										
36	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1				.707		1										
37	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1				.866		1										
38	1.2DL + 1DLi + 1W L	Yes	Y	DL		OL1	1					19	1										
39	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1					20	1										
40	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1	_	_	_		21	1										
41	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1	_		_	.5		1										
42	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1		-1			15											
43	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1	_	_	_	5												
44	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1	_		_		. 17											
45	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1					. 18	-1										
46	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1	13	_			19	-1										
47	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1					. 20	-1										
48	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1					. 21	-1										
49	1.2DL + 1DLi + 1W L	Yes	Y	DL	1.2	OL1	1			114	5	22	-1										
50	(1.2+0.2S ds) + 1.0E	Yes	Y	DL			1 000	E						-									
51	(1.2+0.2Sds) +1.0E	Yes	Υ	DL	1.249	LLX	.866	□	.5	1													

Company :
Designer :
Job Number :
Model Name :

: Trylon : AB : 188628 : 807133

## Load Combinations (Continued)

	Des cription	Solve	PD	.SRB	Factor	BLC	Factor	BF	aB.	Fa	.BLC	Fa	Bl	Fa	.B	Fa	.B	.Fa	.B	.Fa	.В	Fa
52	(1.2+0.2Sds) +1.0E	Yes	Υ	DL	1.249	ELX	.707	E7	707													
53	(1.2+0.2Sds) +1.0E	Yes	Υ	DL	1.249	ELX	.5	E8	66													
54	(1.2+0.2Sds) +1.0E	Yes	Υ	DL	1.249	ELX			1													
55	(1.2+0.2Sds) +1.0E	Yes	Υ	DL	1.249	ELX	5	E8	66													
56	(1.2+0.2Sds) +1.0E	Yes	Υ			ELX	707	E7	07													
57	(1.2+0.2Sds) +1.0E	Yes	Υ	DL	1.249	ELX	866	E	5													
58	(1.2+0.2Sds) +1.0E	Yes	Υ			ELX	-1	E														
59	(1.2+0.2Sds) +1.0E	Yes	Υ	DL		ELX	866	E														
60	(1.2+0.2Sds) + 1.0E	Yes	Υ	DL		ELX	707	E														
61	(1.2+0.2Sds) + 1.0E	Yes	Υ	DL	1.249	ELX	5	E	_								┖					
62	(1.2+0.2Sds) +1.0E	Yes	Υ			ELX			1													
63	(1.2+0.2Sds) + 1.0E	Yes	Υ			ELX	.5	E													$\square$	
64	(1.2+0.2Sds) +1.0E	Yes	Υ			ELX	.707	E	_													
65	(1.2+0.2Sds) +1.0E	Yes	Y			ELX	866		.5													
66	(0.9-0.2Sds) + 1.0E	Yes	Υ	DL	.851	ELX	1	E														
67	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	.866		5													
68	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	.707	E7	_													
69	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	.5	E8	-													
70	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX			1													
71	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	5 5	E8	_													
72	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	707	E7	_													
73	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	866	E	5													
74	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	-1	E	_													
75	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	866	E	_													
76	(0.9-0.2Sds) + 1.0E (0.9-0.2Sds) + 1.0E	Yes	Y	DL DL	.851	ELX	707	E	_													
77	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	5															
78	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	<u>.851</u> .851	ELX	.5	E	.1													
79 80	(0.9-0.2Sds) + 1.0E	Yes Yes	Y	DL	.851	ELX	<u>.5</u> .707	E														
81	(0.9-0.2Sds) + 1.0E	Yes	Y	DL	.851	ELX	.866	E														
82	1.2D + 1.5 Lv1	Yes	Y	DL	1.2	25	1.5		.5													
83	1.2D + 1.5 Lv2	Yes	Y	DL	1.2	26	1.5															
84	1.2D + 1.5Lm + 1.0	Yes	Y	DL	1.2	27	1.5	4 (	63 2	.063	3											
85	1.2D + 1.5Lm + 1.0	Yes	Τ̈́Υ	DL	1.2	27	1.5		63 2			.031										
86	1.2D + 1.5Lm + 1.0	Yes	Y	DL	1.2	27	1.5		63 2			.044										
87	1.2D + 1.5Lm + 1.0	Yes	Ϋ́	DL	1.2	27	1.5		63 2			.054										
88	1.2D + 1.5Lm + 1.0	Yes	Ϋ́	DL	1.2	27	1.5		63 2		3	.063										
89	1.2D + 1.5Lm + 1.0	Yes	Ϋ́	DL	1.2	27	1.5		63 2			.054									$\Box$	
90	1.2D + 1.5Lm + 1.0	Yes	Y	DL	1.2	27	1.5		63 2			.044										
91	1.2D + 1.5Lm + 1.0	Yes	Y	DL	1.2	27	1.5		63 2	_		.031										
92		Yes	Y	DL	1.2	27	1.5		63 2	_		7										
93	1.2D + 1.5Lm + 1.0	Yes	Y	DL	1.2	27	1.5		63 2	_		0										
94	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	27	1.5		63 2			0										
95	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	27	1.5		63 2			0	.									
96	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	27	1.5	8 .0	63 2	-1	. 3	0										
97	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	27	1.5	9 .0	63 2	.031	3	0										
98	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	27	1.5	10.0	63 2	.044	3	0										
99	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	27	1.5	11.0	63 2	.054	3	0										
	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	28	1.5		63 2	_	3											
	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	28	1.5		63 2			.031										
	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	28	1.5		63 2			.044										
103	1.2D + 1.5Lm + 1.0	Yes	Υ	DL	1.2	28	1.5	7 .0	63 2	.031	3	.054									لي	

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## Load Combinations (Continued)

Des cription	Solve	PD S	RB Factor	BLC	Factor	BFaBF	aBL	C FaBFa	.BFaE	3FaE	3Fa	.BFa
104 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	8 .063 2 3	3	.063				
105   1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	9 .063 2 -	.0 3	.054				
106   1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	10.063 2 -	.0 3	.044				
107 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	11.063 2 -	.0 3	.031				
108 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	4 .063 2 -	.0 3	7				
109 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	5 .063 2 -	.0 3	0				
110 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	6 .063 2 -	.0 3	0				
111 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	7 .063 2 -	.0 3	0				
112 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	8 .063 2 -	1 3	0				
113 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	9 .063 2 .0	031 3	0				
114 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	10.063 2 .0	044 3	0				
115 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	28	1.5	11.063 2 .0	054 3	0				
116 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	4 .063 2 .0	063 3					
117 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	5 .063 2 .0	054 3	.031				
118 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	6 .063 2 .0	)44 3	.044				
119 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	7 .063 2 .0	031 3	.054				
120 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	8 .063 2 3	3	.063				
121 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	9 .063 2 -	.0 3	.054				
122 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	10.063 2 -	.0 3	.044				
123 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	11.063 2 -	.0 3	.031				
124 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	4 .063 2 -	.0 3	7				
125 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	5 .063 2 -	.0 3	0				
126 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	6 .063 2 -	.0 3	0				
127 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	7 .063 2 -	.0 3	0				
128 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	8 .063 2 -	1 3	0				
129 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5		031 3	0				
130 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	10.063 2 .0	)44 3	0				
131 1.2D + 1.5Lm + 1.0	Yes	Υ	DL 1.2	29	1.5	11.063 2 .0	054 3	0				

#### **Envelope Joint Reactions**

	Joint		X <b>[</b> b]	LC	Y <b>[</b> b]	LC	Z [ <b>l</b> b]	LC	MX [lb-ft]	LC	MY [lb-ft]	LC	MZ [lb-ft]	LC
1	N5	max	1351.35	18	642.51	118	907.61	34	269.69	91	<b>-</b> 156.93	26	0	131
2		min	-2199.4	10	-1350	99	189.14	26	-129.89	125	-765.94	34	0	1
3	N 17	max	1459.45	37	1345.75	88	784.78	42	231.15	91	-147.45	18	0	131
4		min	289.42	29	-614.16	127	187.08	18	-102.88	125	-663.17	42	0	1
5	N86A	max	721.9	9	353.1	9	45.46	41	0	131	0	131	0	131
6		min	-722.11	17	-354.68	17	10.16	79	0	1	0	1	0	1
7	Totals:	max	1204.66	2	704.95	22	1731.06	43						
8		min	-1204.66	26	-704.95	14	397.18	66						

## Envelope AISC 15th (360-16): LRFD Steel Code Checks

	Member	Shape	Code Check	Loc[in]	LC	She Lo	. phi*P phi*P	phi*Mphi*M Eqn
1	MP9	4.25x0.5	.506	.21	91			0 717.19 6096H1-1b
2	MP8	4.25x0.5	.501	.21	84	.110 2.51 y	. 666206885	0 717.19 6071 1 H1-1b
3	SA4	PIPE 1.5	.490	27.81	91	.116 27 84	4 20972 2359	311051105H1 <b>-</b> 1b
4	MP23	PIPE 2.0	.487	48	2	.182 48 10	0 137873213	0 18711871H1 <b>-</b> 1b
5	TH	PIPE 2.0	.481	75	10	.188 76 2	249763213	0 18711871 <b>1</b> H1 <b>-</b> 1b
6	MP7	3.5x0.5	.458	0	91	136 4.81 y	. 548635670	0 590.63 4134 1 H1-1b
7	SA3	PIPE_1.5	.440	27.81	84	.118 27 92	2 20972 2359	3 <mark>.1105</mark> 1105 <mark></mark> H1 <b>-</b> 1b

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### Envelope AISC 15th (360-16): LRFD Steel Code Checks (Continued)

	Member	Shape	Code Check	Loc[in]	LC	SheLo phi*P phi*P phi*Mphi*M Eqn
8	В3	SR 1/2"	.427	0	90	.009 39 1056 6361 53.02 53.02 1 H1
9	BH	PIPE 2.0	.420	76	84	.085 76 84 24976 32130 1871 1871 1 H1-1b
10	MP29	3.5x0.5	.403	0	99	.142 4.81 y 54863 56700 590.63 4134H1-1b
11	MP18	4.25x0.5	.360	.21	124	.114 .21 y 66620. 68850 717.19 6071 1 H1-1b
12	MP17	4.25x0.5	.359	.21	116	.114 .21 y 66620 68850 717.19 6096 H1-1b
13	SA2	PIPE 1.5	.346	27.81	124	.091 27 20972. 23593 1105 1105 H1-1b
14	SA1	PIPE 1.5	.314	2.19	122	.093 2.19 20972. 23593. 1105 1105 H1-1b
15	B1	SR 1/2"	.301	0	124	.011 0 99 1056 6361 53.02 53.02 1 H1
16	MP30	3.5x0.5	.293	0	122	.141 0 y 5486356700 590.63 4134H1-1b
17	MP31	3.5x0.5	.253	0	130	.141 0 y 54863 56700 590.63 4134 1 H1-1b
18	V4	SR 5/8	.251	12.81	84	.027 0 84 4134 9946.8 96.77 96.77 1 H1-1a
19	V3	SR 5/8	.236	17.5	91	.063 0 91 4134 9946.8 96.77 96.77 1 H1-1a
20	MP38	PIPE_2.0	.194	48	10	.070 48 91 1378732130 18711871H1-1b
21	V1	SR 5/8	.177	0	116	.027 0 94 4134 9946.8 96.77 96.77 1 H1
22	V2	SR 5/8	.165	30	124	.064 0 91 4134 9946.8 96.77 96.77 1 H1
23	MP41	PIPE 2.0	.079	48	17	.071 48 9 1378732130 18711871H1-1b
24	MP25	PIPE 2.0	.049	41.3	7	.005 82 46 1820632130 18711871H1-1b
25	B2	SR 1/2"	.018	0	91	.012 0 88 1056 6361 53.02 53.02 1 H1
26	B4	SR 1/2"	.000	0	131	.007 39 1056 6361 53.02 53.02 1 H1-1a

## Envelope AISIS 100-16: LRFD Cold Formed Steel Code Checks

Member Shape	Code Check	Loc[in]LC SheaLoc[iDirLC phi*Pn[phi*Tn[phi*Mnphi*Mnphi* phi* Cb	Eqn		
No Data to Print					

# APPENDIX D ADDITIONAL CALCUATIONS

Analysis date: 07/27/21

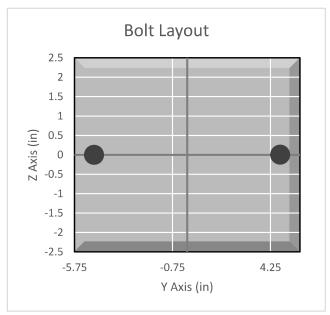


#### **BOLT TOOL 1.5.2**

Project Data				
Job Code:	188628			
Carrier Site ID:	NJJER01090A			
Carrier Site Name:	CT-CCI-T-807133			

Code				
Design Standard:	TIA-222-H			
Slip Check:	No			
Pretension Standard:	TIA-222-H			

Bolt Properties					
Connection Type:	Threaded Rod				
Diameter:	0.75	in			
Grade:	A307				
Yield Strength (Fy):	36	ksi			
Ultimate Strength (Fu):	60	ksi			
Number of Bolts:	2				
Threads Included:	Yes				
Double Shear:	No				
Connection Pipe Size:	9.5	in			



Connection Description				
Mount to Tower				

Bolt Check*					
Tensile Capacity (φT <sub>n</sub> ):		lbs			
Shear Capacity (φV <sub>n</sub> ):	9940.2	lbs			
Tension Force (T <sub>u</sub> ):	642.5	lbs			
Shear Force (V <sub>u</sub> ):	967.5	lbs			
Tension Usage:	4.1%				
Shear Usage:	9.3%				
Interaction:	9.3%	Pass			
Controlling Member:	MP4				
Controlling LC:	99				

<sup>\*</sup>Rating per TIA-222-H Section 15.5

Analysis date: 07/27/21

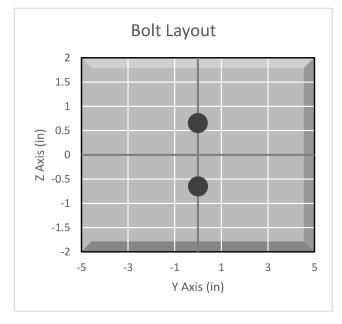


#### **BOLT TOOL 1.5.2**

Project Data				
Job Code:	188628			
Carrier Site ID:	NJJER01090A			
Carrier Site Name:	CT-CCI-T-807133			

Code				
Design Standard:	TIA-222-H			
Slip Check:	No			
Pretension Standard:	AISC			

Bolt Properties				
Connection Type:	Bolt			
Diameter:	0.5 in			
Grade:	A325			
Yield Strength (Fy):	92	ksi		
Ultimate Strength (Fu):	120	ksi		
Number of Bolts:	2			
Threads Included:	Yes			
Double Shear:	No			
Connection Pipe Size:	-	in		

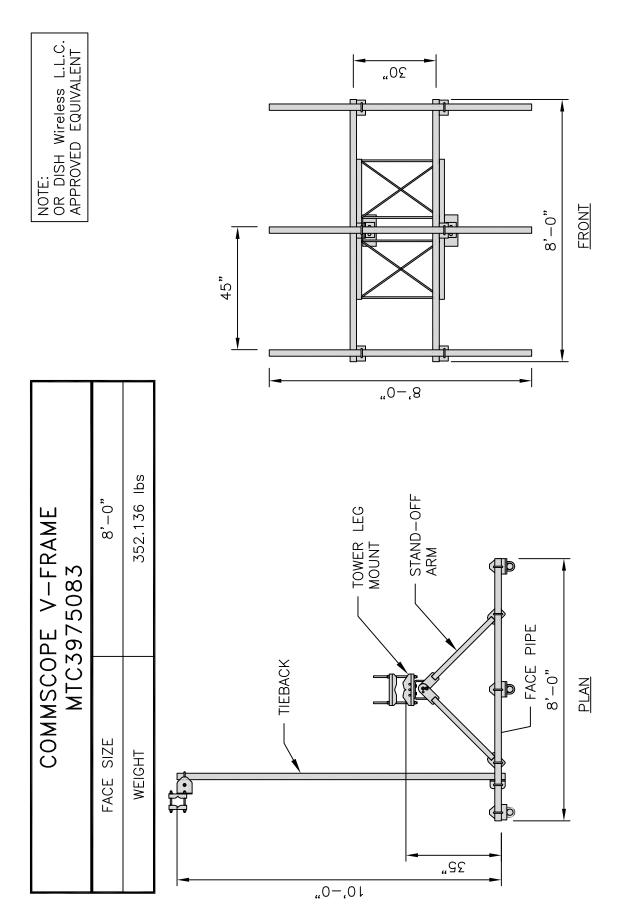


Connection Description
Stand-off arm to Tower connection kit

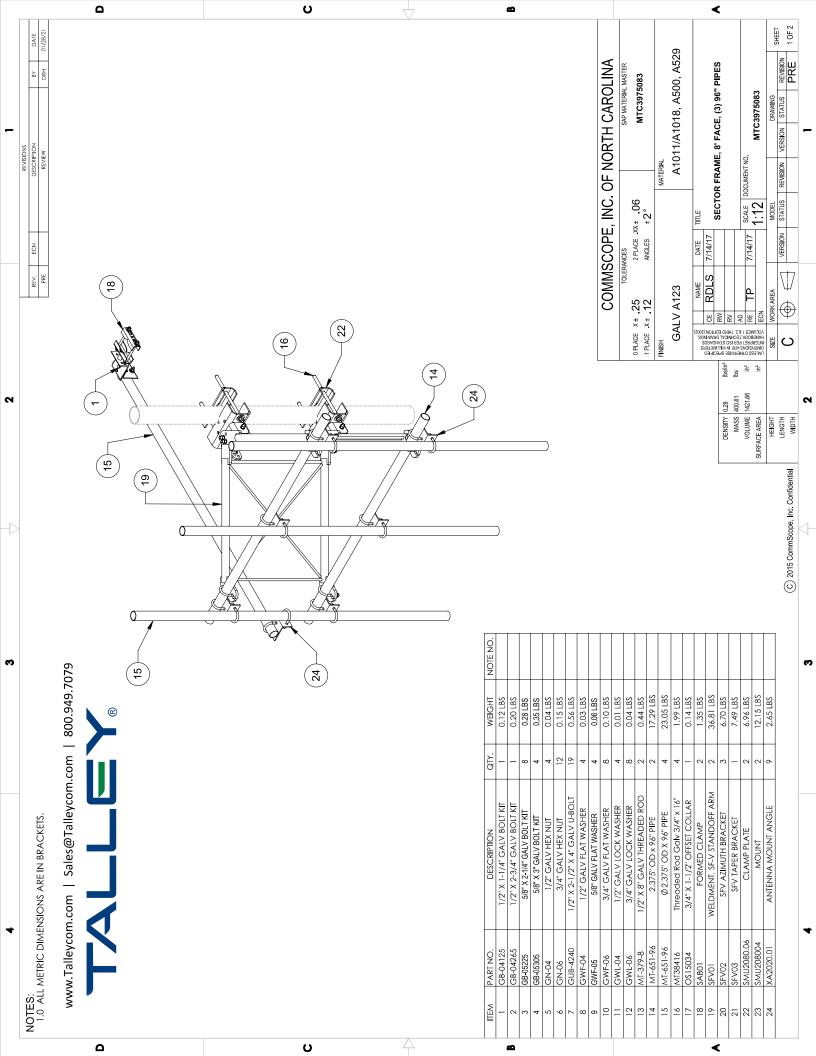
Bolt Check*				
Tensile Capacity $(\phi T_n)$ :	12770.9	lbs		
Shear Capacity (φV <sub>n</sub> ):	8835.7	lbs		
Tension Force (T <sub>u</sub> ):	2134.7	lbs		
Shear Force (V <sub>u</sub> ):	946.6	lbs		
Tension Usage:	15.9%			
Shear Usage:	10.2%			
Interaction:	15.9%	Pass		
Controlling Member:	MP4			
Controlling LC:	91			

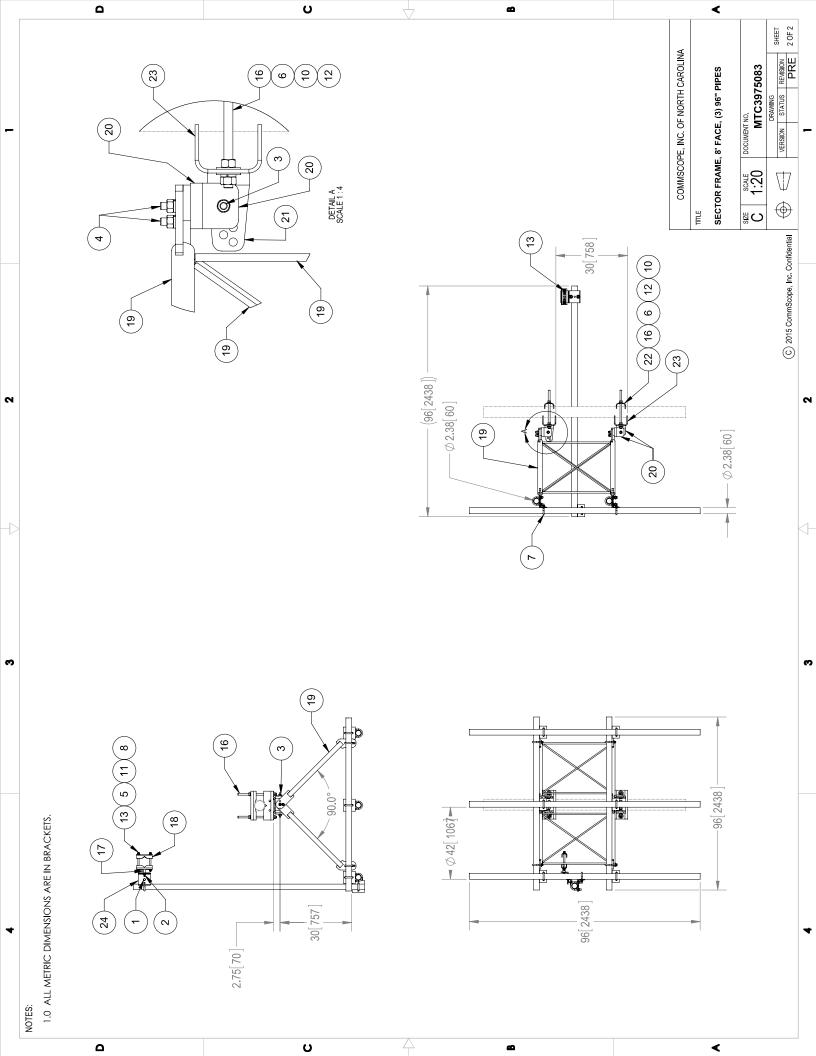
<sup>\*</sup>Rating per TIA-222-H Section 15.5

# APPENDIX E SUPPLEMENTAL DRAWINGS



ANTENNA FRAME DETAIL





## Exhibit F

**Power Density/RF Emissions Report** 



## RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

Dish Wireless Existing Facility

Site ID: NJJER01090A

807133

50 Rockland Road Norwalk, Connecticut 06854

September 9, 2021

EBI Project Number: 6221004862

Site Compliance Summary		
Compliance Status:	COMPLIANT	
Site total MPE% of FCC general population allowable limit:	24.00%	



September 9, 2021

Dish Wireless

Emissions Analysis for Site: NJJER01090A - 807133

EBI Consulting was directed to analyze the proposed Dish Wireless facility located at **50 Rockland Road** in **Norwalk, Connecticut** for the purpose of determining whether the emissions from the Proposed Dish Wireless Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm²). The number of  $\mu$ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm²). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately 400  $\mu$ W/cm² and 467  $\mu$ W/cm², respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is 1000  $\mu$ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure.



Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

#### **CALCULATIONS**

Calculations were done for the proposed Dish Wireless Wireless antenna facility located at 50 Rockland Road in Norwalk, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since Dish Wireless is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 4 n71 channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 4 n70 channels (PCS Band 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 3) 4 n66 channels (AWS Band 2190 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 5) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative



estimate as gain reductions for these particular antennas are typically much higher in this direction.

- 6) The antennas used in this modeling are the JMA MX08FRO665-20 for the 600 MHz / 1900 MHz / 2190 MHz channel(s) in Sector A, the JMA MX08FRO665-20 for the 600 MHz / 1900 MHz / 2190 MHz channel(s) in Sector B, the JMA MX08FRO665-20 for the 600 MHz / 1900 MHz / 2190 MHz channel(s) in Sector C. This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 20 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antenna mounting height centerline of the proposed antennas is 181 feet above ground level (AGL).
- 8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 9) All calculations were done with respect to uncontrolled / general population threshold limits.



## Dish Wireless Site Inventory and Power Data

Sector:	Α	Sector:	В	Sector:	С
Antenna #:	I	Antenna #:	I	Antenna #:	I
Make / Model:	JMA MX08FRO665- 20	Make / Model:	JMA MX08FRO665- 20	Make / Model:	JMA MX08FRO665- 20
Frequency Bands:	600 MHz / 1900 MHz / 2190 MHz	Frequency Bands:	600 MHz / 1900 MHz / 2190 MHz	Frequency Bands:	600 MHz / 1900 MHz / 2190 MHz
Gain:	17.45 dBd / 22.65 dBd / 22.65 dBd	Gain:	17.45 dBd / 22.65 dBd / 22.65 dBd	Gain:	17.45 dBd / 22.65 dBd / 22.65 dBd
Height (AGL):	181 feet	Height (AGL):	181 feet	Height (AGL):	181 feet
Channel Count:	12	Channel Count:	12	Channel Count:	12
Total TX Power (W):	440 Watts	Total TX Power (W):	440 Watts	Total TX Power (W):	440 Watts
ERP (W):	5,236.31	ERP (W):	5,236.31	ERP (W):	5,236.31
Antenna A1 MPE %:	0.77%	Antenna B1 MPE %:	0.77%	Antenna C1 MPE %:	0.77%

## environmental | engineering | due diligence

Site Composite MPE %		
Carrier	MPE %	
Dish Wireless (Max at Sector A):	0.77%	
AT&T	3.45%	
Metro PCS	1.56%	
Verizon	5.01%	
T-Mobile	10.58%	
Sprint	2.63%	
Site Total MPE % :	24.00%	

Dish Wireless MPE % Per Sector		
Dish Wireless Sector A Total:	0.77%	
Dish Wireless Sector B Total:	0.77%	
Dish Wireless Sector C Total:	0.77%	
Site Total MPE % :	24.00%	

Dish Wireless Maximum MPE Power Values (Sector A)								
Dish Wireless Frequency Band / Technology (Sector A)  # Channels   Watts ERP (Per Channel)   Height (feet)   Total Power Density (µW/cm²)   Frequency (MHz)   Calculated % MI								
Dish Wireless 600 MHz n71	4	223.68	181.0	1.05	600 MHz n71	400	0.26%	
Dish Wireless 1900 MHz n70	4	542.70	181.0	2.55	1900 MHz n70	1000	0.25%	
Dish Wireless 2190 MHz n66	4	542.70	181.0	2.55	2190 MHz n66	1000	0.25%	
						Total:	0.77%	

<sup>•</sup> NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.



## **Summary**

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the Dish Wireless facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

Dish Wireless Sector	Power Density Value (%)
Sector A:	0.77%
Sector B:	0.77%
Sector C:	0.77%
Dish Wireless	
Maximum MPE %	0.77%
(Sector A):	
Site Total:	24.00%
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **24.00**% of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

## Exhibit G

## **Letter of Authorization**



4545 E River Rd, Suite 320 West Henrietta, NY 14586 Phone: (585) 445-5896 Fax: (724) 416-4461 www.crowncastle.com

#### **Crown Castle Letter of Authorization**

#### CT - CONNECTICUT SITING COUNCIL

Melanie A. Bachman Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

**Re:** Tower Share Application

**Crown Castle telecommunications site at:** 

50 ROCKLAND ROAD NORWALK OFC - MTSO, SO NORWALK, CT 06854

CROWN ATLANTIC COMPANY LLC ("Crown Castle") hereby authorizes DISH Wireless LLC, including their Agent, to act as our Agent in the processing of all zoning applications, building permits and approvals through the CT - CONNECTICUT SITING COUNCIL for the existing wireless communications site described below:

Crown Site ID/Name: 807133/BRG 134 943057 Customer Site ID: NJJER01090A/CT-CCI-T-807133

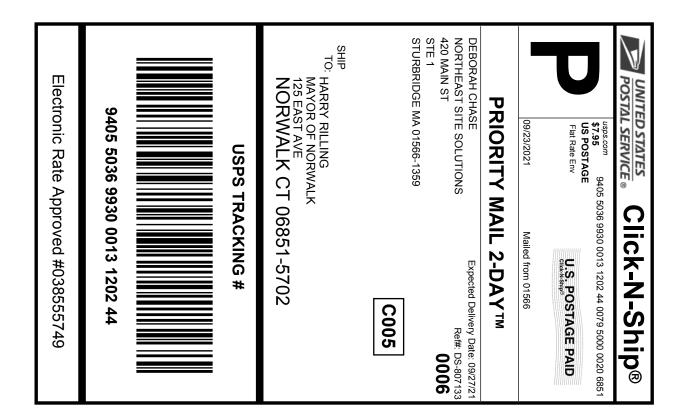
Site Address: 50 ROCKLAND ROAD NORWALK OFC - MTSO, SO

NORWALK, CT 06854

Crown Ca	stle			
	ichard Zajac ite Acquisition Specialist	Date:	8/30/2021	

# Exhibit H

**Recipient Mailings** 





Cut on dotted line.

#### Instructions

- 1. Each Click-N-Ship® label is unique. Labels are to be used as printed and used only once. DO NOT PHOTO **COPY OR ALTER LABEL.**
- 2. Place your label so it does not wrap around the edge of the package.
- 3. Adhere your label to the package. A self-adhesive label is recommended. If tape or glue is used, DO NOT TAPE OVER BARCODE. Be sure all edges are secure.
- 4. To mail your package with PC Postage®, you may schedule a Package Pickup online, hand to your letter carrier, take to a Post Office™, or drop in a USPS collection box.
- 5. Mail your package on the "Ship Date" you selected when creating this label.

## Click-N-Ship® Label Record

#### **USPS TRACKING #:** 9405 5036 9930 0013 1202 44

544355104 09/23/2021 09/23/2021 Trans. #: Print Date: Ship Date: 09/27/2021 Delivery Date:

Priority Mail® Postage: Total:

\$7.95 \$7.95

Ref#: DS-807133

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

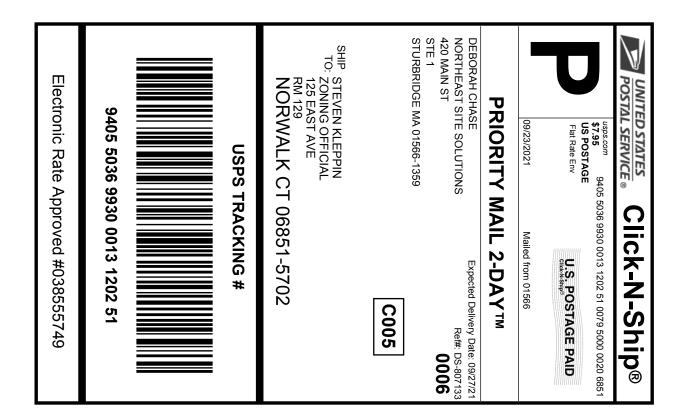
**STURBRIDGE MA 01566-1359** 

HARRY RILLING

MAYOR OF NORWALK 125 EAST AVE

NORWALK CT 06851-5702

Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.





Cut on dotted line.

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- 5. Mail your package on the "Ship Date" you selected when creating this label.

## Click-N-Ship® Label Record

#### **USPS TRACKING #:** 9405 5036 9930 0013 1202 51

544355104 09/23/2021 09/23/2021 Trans. #: Print Date: Ship Date: 09/27/2021 Delivery Date:

Total:

Priority Mail® Postage: \$7.95 \$7.95

Ref#: DS-807133

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

**STURBRIDGE MA 01566-1359** 

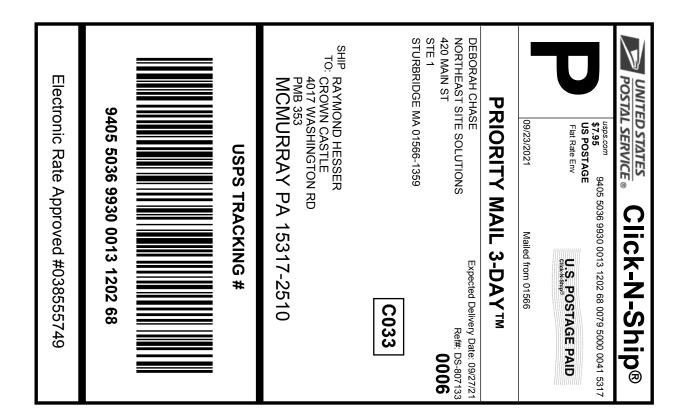
STEVEN KLEPPIN

**ZONING OFFICIAL** 125 EAST AVE

RM 129

NORWALK CT 06851-5702

\* Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.





Cut on dotted line.

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## Click-N-Ship® Label Record

#### **USPS TRACKING #:** 9405 5036 9930 0013 1202 68

544355104 09/23/2021 09/23/2021 Trans. #: Print Date: Ship Date: 09/27/2021 Delivery Date:

Priority Mail® Postage: Total:

\$7.95 \$7.95

Ref#: DS-807133

From: DEBORAH CHASE

NORTHEAST SITE SOLUTIONS

420 MAIN ST

STE 1

**STURBRIDGE MA 01566-1359** 

RAYMOND HESSER

**CROWN CASTLE** 4017 WASHINGTON RD

PMB 353

MCMURRAY PA 15317-2510

\* Retail Pricing Priority Mail rates apply. There is no fee for USPS Tracking® service on Priority Mail service with use of this electronic rate shipping label. Refunds for unused postage paid labels can be requested online 30 days from the print date.

## 807133- Norwalk



FISKDALE 458 MAIN ST FISKDALE, MA 01518-9998

09/24/2021			11:16 AM
Product			Price
Prepaid Mail Norwalk, CT 060 Weight: 1 lb Acceptance Date Fri 09/24/2 Tracking #: 9405 5036 S	1 851 7.60 oz e: 2021		\$0.00
Prepaid Mail Norwalk, CT 068 Weight: 1 lb 7 Acceptance Date Fri 09/24/2 Tracking #: 9405 5036 9	'.50 oz : 021	3 1202 44	\$0.00
Prepaid Mail Canonsburg, PA Weight: 1 lb 7 Acceptance Date Fri 09/24/20 Tracking #: 9405 5036 99	15317 .50 oz : 021 930 0013		\$0.00
Grand Total:		THE REP CHE CHE WAS NOT THE WAY TO	40.00

USPS is experiencing unprecedented volume increases and limited employee